



TRAILS WEST: COPE DRIVE UNITS
Servicing and Stormwater Management
Report

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Trails West: Cope Drive Units

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Trails West: Cope Drive Units

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1 Background

Stantec Consulting Ltd. is commissioned by 1230374 Ontario Inc. to prepare a Servicing and Stormwater Management Report for the Trails West – Cope Drive Units development. The subject properties include multiple blocks located north and south of Cope Drive from Northgraves Crescent to Akerson Road / Carronbridge Circle. From the original subdivision development process, the blocks are within the Soho West Phase 1 and Phase 2 development areas. The current reference to the area is now the Trails West neighbourhood. The properties are indicated in **Figure 1** below.

The proposed 1.62 ha-development consists of three residential blocks containing 44 townhomes and 48 back-to-back units located in the ward of Kanata South in the City of Ottawa. The subject properties are zoned General Mixed-Use, Exception 2353 and 2354, Maximum Building Height 14 metres (GM [2353/2354] H (14)) and Residential Third Density, Subzone X, Exception 2355 (R3X [2355]) in the City's Comprehensive Zoning By-law (2008-250). The proposed development is permitted under the current zoning.

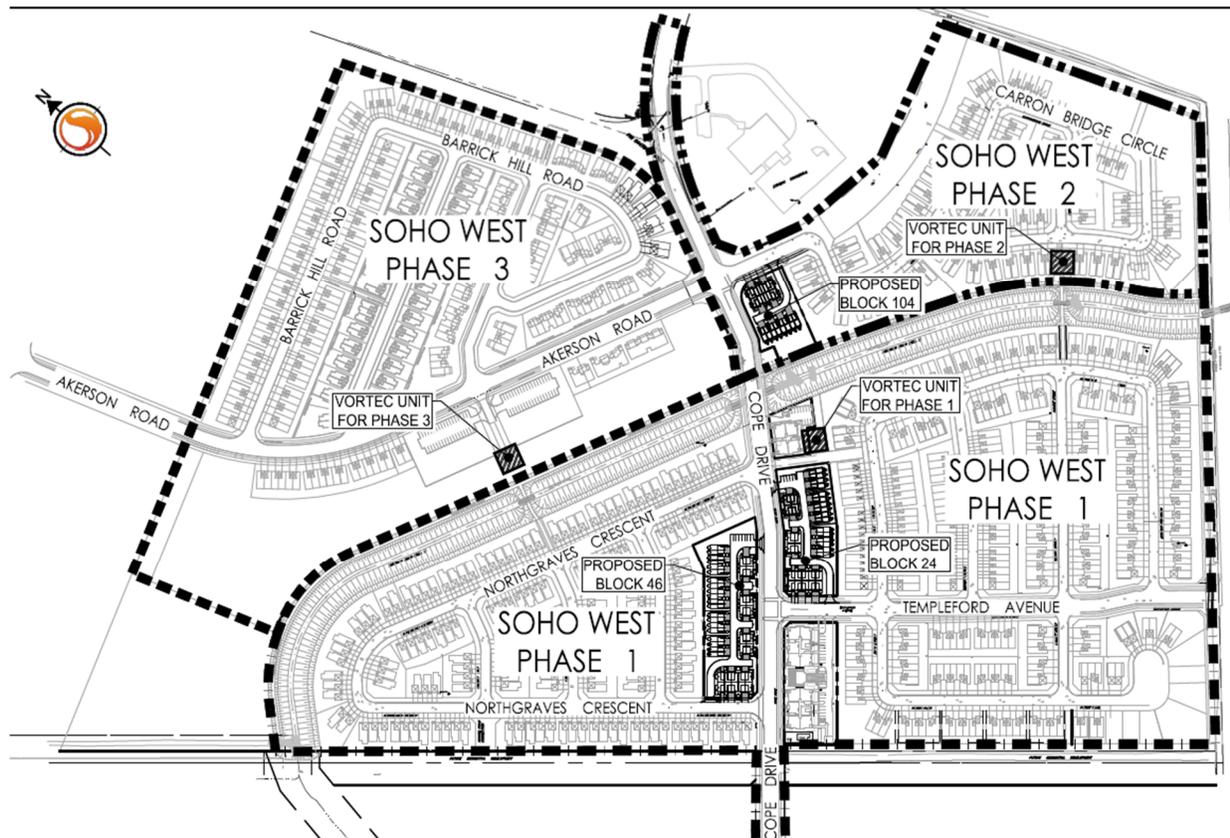


Figure 1: Key Map of Cope Drive Units Location



Trails West: Cope Drive Units

1 Background

This Servicing and Stormwater Management Report is prepared to present a servicing scheme that is free of conflicts and presents a suitable servicing approach that is consistent with the relevant City design guidelines. Information related to existing infrastructure is gathered from as-built drawings, approved design reports and through consultation with City of Ottawa staff and design engineers. Infrastructure requirements for water supply, sanitary sewer, and storm sewer services are presented in this report.

As coordinated with the City of Ottawa, proposed development within Block 43 (110 Cope Drive) will be submitted through a Building Permit application.



2 References

The following documents are referenced in the preparation of this report:

- Cavanagh Construction – Soho West (Phases 1 and 2), Kanata South, City of Ottawa Stormwater Management Report, Stantec Consulting Ltd., October 2007.
- Serviceability Report, Kavanagh Construction Ltd./Karam – Soho West, Rev 3, Stantec Consulting Ltd., October 31, 2007
- Summary of Geotechnical Investigation – Sohwest Area B, Eagleson Road at Fernbank Road, Ottawa, Ontario, Houle Chevrier Engineering, May 2010.
- Summary of Geotechnical Investigation – Proposed Trail west Condo Developments, Akerson Road and Cope Drive, Ottawa, Ontario, Houle Chevrier Engineering, July 21, 2015.
- Geotechnical Investigation Akerson Road and Cope Drive Development, Ottawa, Ontario, Houle Chevrier Engineering, March 3, 2016.
- Letter, Cope Drive Blocks 24,43, 46, and 104 Trails West Subdivision, Gemtec Consulting Engineers and Scientists (formerly Houle Chevrier Engineering), May 23, 2023
- City of Ottawa Design Guidelines – Water Distribution, Infrastructure Services Department, City of Ottawa, First Edition, July 2010 (and all subsequent technical bulletins).
- City of Ottawa Sewer Design Guidelines, City of Ottawa, 2014 (and all subsequent technical bulletins).



3 Potable Water Servicing

3.1 Existing Conditions

The proposed development is located within Zone 3W of the City of Ottawa water distribution system. The 3W pressure zone is fed by the Glen Cairn booster pumping station with the Stittsville elevated storage tank providing balanced storage for peak flows and demands. A 400mm diameter watermain exists along Akerson Road / Carronbridge Circle, a 300mm diameter watermain exists along Cope Drive, and 200mm diameter watermains exist along Northgraves Crescent and Templeford Avenue.

3.2 Proposed Watermain Sizing and Layout

The existing and proposed watermain alignment and sizing for each development area block is shown on **Drawing SSP-1**. Proposed watermain diameters of 150mm (nominal) are used for each block. A hydrant relocation on Carronbridge Circle is needed to accommodate to entrance to the Block 104 site.

3.2.1 CONNECTION TO EXISTING INFRASTRUCTURE

Water supply for each development area block is proposed via connections to the existing mains located within the roadways adjacent to each block. The service connections and the internal servicing layout for each development area block is shown on **Drawing SSP-1**.

The following table summarizes the number of units associated with each block and the related connection point to the existing infrastructure system.

Table 1: Connections to Existing Infrastructure

Development Area	Number of Units	Building Type	Connection Location	Existing Watermain Diameter (mm)
Block 24	32	16 Back-to-Back 16 Townhouse	Templeford Avenue	200
Block 43	8	8 Townhouse	Cope Drive	300
Block 46	40	20 Back-to-Back 20 Townhouse	Northgraves Crescent	200
Block 104	16	8 Back-to-Back 8 Townhouse	Carronbridge Circle	400

3.2.2 WATER DEMAND

The water demand for each development area block is calculated using the City of Ottawa's Water Distribution Design Guidelines with detailed calculations provided in **Appendix A.1**.



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For residential developments, the average day (AVDY) per capita water demand is 280 L/cap/d. For maximum day (MXDY) demand, AVDY is multiplied by a factor of 2.5 and for peak hour (PKHR) demand, MXDY is multiplied by a factor of 2.2. The population density is taken at 2.7 person per unit for Townhomes and Back-to-Back units. The calculated residential water demand is represented in the following table.

Table 2: Residential Water Demands

Development Area	Number of Units	Population	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Block 24	32	89	0.29	0.72	1.59
Block 43	8	22	0.07	0.19	0.41
Block 46	40	111	0.36	0.90	1.98
Block 104	16	44	0.14	0.36	0.78

3.2.3 FIRE FLOW DEMAND

The Fire Underwriter Survey (FUS) fire flow calculation spreadsheets for the governing fire flow demand scenarios are generated to calculate the expected fire flow demand for each development area block. For assessment of the worst-case fire flow requirement, building exposures were reviewed for the largest dwelling unit type within each block.

The detailed calculations are provided in **Appendix A.2** and summarized in the following table.

Table 3: FUS Fire Flow

Development Area	Number of Units	Building Type	Fire Flow (L/s)
Block 24	8	Back-to-Back - 3 Story	12,000
	6	Townhouse - 3 Story	14,000
Block 43	6	Townhouse - 3 Story	10,000
Block 46	8	Back-to-Back - 3 Story	12,000
	6	Townhouse - 3 Story	14,000
Block 104	8	Back-to-Back - 3 Story	13,000
	8	Townhouse - 3 Story	13,000

Exposure distance accounts for the difference in fire flow value for the 8-unit, 3-story back-to-back townhomes in Block 104 compared to similar buildings in Block 24 and 46.

Additional fire walls between units within each building are not considered in the FUS calculations.



3.3 Level of Service

3.3.1 ALLOWABLE PRESSURES

The City of Ottawa Water Distribution Design Guidelines state that the desired range of system pressures under normal demand conditions (i.e., basic day, maximum day, and peak hour) should be in the range of 350 to 552 kPa (50 to 80 psi) and no less than 275 kPa (40 psi) at the ground elevation in the streets (i.e., at hydrant level). The maximum pressure at any point in the distribution system in occupied areas outside of the public right-of-way is 552 kPa (80 psi).

As per the Ontario Building Code (OBC), if pressures greater than 552 kPa (80 psi) are anticipated, pressure relief measures are required. The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi). Under emergency fire flow conditions, the minimum pressure objective in the distribution system is 138 kPa (20 psi).

Residential buildings greater than two stories require an additional 35 kPa (5 psi) for every additional story over two stories. This additional pressure is to account for the change in elevation head and additional head loss. For example, the minimum pressure required for a two-story building is 276 kPa (40 psi) whereas a three-story building requires at least 310 kPa (45 psi) at the service connection point to provide equivalent minimum pressure on the higher floor.

3.3.2 FIRE FLOW

As per the City's technical bulletin ISDTB-2014-02 (City of Ottawa, 2014), fire flow shall be capped at 10,000 L/min for traditional side-by-side townhomes and single-family units constructed in accordance with the OBC and with a minimum separation of 10 meters between the back of adjacent units.

3.4 Hydraulic Analysis

The boundary conditions provided by the City of Ottawa are summarized in the following table. The related boundary conditions request and correspondence with the City of Ottawa are included in **Appendix A.3**.



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Table 4: Boundary Conditions

Development Area	Maximum HGL		Peak Hour		Max Day plus Fire	
	Head (m)	Pressure (psi)	Head (m)	Pressure (psi)	Head (m)	Pressure (psi)
Block 24	160.9	90.8	156.4	84.4	144.3	67.1
Ground Elevation = 97.1 m						
Block 43	161.0	92.0	156.4	85.5	154.6	82.8
Ground Elevation = 96.3 m						
Block 46	160.9	90.9	156.4	84.5	143.4	66.1
Ground Elevation = 97.0 m						
Block 104	161.0	91.8	156.4	85.3	154.1	82.0
Ground Elevation = 96.4 m						

Based on the boundary conditions provided by the city, the existing watermain network provides adequate capacity to support both the residential water (Peak Hour > 40 psi) and fire flow (Max Day plus Fire >20 psi) demands.

With the operating pressures during peak hour demands anticipated to be greater than 80psi (552 kPa), pressure reducing measures are required to comply with OBC guidelines.



4 Wastewater Servicing

4.1 Existing Conditions

The Trails West development is located within the South Glen Cairn wastewater catchment area. Wastewater flow from the development is supported by the South Glen Cairn Trunk Sewer and the Hazeldean Pump Station.

The South Glenn Cairn wastewater catchment includes the Kanata South Business Park which was originally identified to be developed as mixed light industrial and commercial land uses. A serviceability report was prepared for the Soho West (now Trails West) development which examined and compared the expected peak wastewater flow rate from the study area developed as a business park versus a mixed-use development with a variety of housing types, commercial, and business park uses.

The Soho West Serviceability report concluded that the expected wastewater peak flow from the gross site area developed entirely as business park was estimated to be 88.7 L/s. The expected wastewater peak flow from the Trails West residential development and related the commercial block was found to be 67.0 L/s. Since the expected wastewater peak flow from the Trails West development was less than the peak flows from the development as a business park, it was concluded that the South Glen Cairn collector and the Hazeldean Pump Station are adequately sized to receive wastewater from the Trails West development.

4.1.1 ALLOWABLE WASTEWATER PEAK FLOW

A 450mm diameter sewer exists along Akerson Road / Carronbridge Circle, a 525mm diameter sewer exists along Cope Drive, and a 200mm diameter sewer exists in the servicing corridor south of Cope Drive at Northgraves Crescent.

A copy of the original 2007 subdivision sanitary sewer design sheet and associated Sanitary Drainage Plans are included in **Appendix B.1** for reference. From this 2007 subdivision information, the previously allocated wastewater dry weather peak flow (DWPF) and wet weather peak flow (WWPF) for the Trails West – Cope Drive Units development blocks are extracted for each development block area. The following table summarizes the information taken from the 2007 subdivision design.

Table 5: 2007 Wastewater Peak Flow

Development Area	2007 Area Reference	Number of Units	Population	DWPF (L/s)	Extraneous Flow Allowance (L/s)	WWPF (L/s)
Block 24	Block 84-A	44	119	1.93	0.16	2.09
Block 43	Cope Drive Area '32'	12	32	0.52	0.05	0.57
Block 46	Block 34	60	162	2.63	0.20	2.83
Block 104	Block 85	24	65	1.05	0.10	1.15



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4 Wastewater Servicing

For Block 43 (2007 Area '32'), the 2007 data is interpreted based on the calculation methodology and the current 0.16 ha design area of the applicable block.

4.2 Design Criteria

As outlined in the City's Sewer Design Guidelines, the following design parameters are used to calculate estimated wastewater flow rates and to size on-site sanitary sewers.

Table 6: Wastewater Servicing Parameters

Parameter	Value
Minimum Full Flow Velocity	0.6 m/s (0.8 m/s for upstream sections)
Maximum Full Flow Velocity	3.0 m/s
Manning's roughness coefficient	0.013
Townhouse Persons per unit	2.7
Back-to-Back Persons per unit	2.7
Average Residential Flow	280 L/p/day
Harmon Peaking Factor Maximum	4.0
Extraneous Flow Allowance	0.33 L/s/ha
Maintenance Hole Spacing	120 m
Minimum Cover	2.5 m

4.3 Proposed Sewer Sizing and Layout

Wastewater servicing for the proposed development is provided through the existing wastewater collection system in the Trails West development.

4.3.1 CONNECTION TO EXISTING INFRASTRUCTURE

The sewer service for each development area block is proposed via connections to the existing sewer located within the roadways and service corridors adjacent to each block. The service connections and the internal servicing layout for each development area block is shown on **Drawing SSP-1**.

Excepting Block 46, the proposed sewer connection locations are generally consistent with the original 2007 subdivision design conditions. The 2007 connection location for Block 46 was anticipated to occur along Northgraves Crescent. The current proposed connection for Block 46 along Cope Drive is downstream of the 2007 location so there is no relative change in the design flow condition downstream of the new connection location.

At Block 24, 46, and 104 there are Back-to-back Townhouse units that front directly onto an adjacent public road. These units are to have direct service connections to the existing sewer main in each respective public road.



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4 Wastewater Servicing

For Block 24 and 46, the direct service connections to the 200mm sewer mains in Templeford Avenue and Northgraves Crescent are considered offset by the change in the overall service connection for Block 46.

At Block 104, the allocation of the direct service connections is attributed to the same maintenance hole location in Carronbridge Circle as the rest of the development area block.

No evaluation of the service capacity of the existing sewer in the public roads downstream of the direct service connections from Block 24, 46, and 104 is completed.

The following table summarizes the number of units associated with each block and the related connection point to the existing infrastructure system.

Table 7: Connections to Existing Infrastructure

Development Area	Number of Units	Building Type	Connection Location	Existing Sewer Diameter (mm)
Block 24	30	14 Back-to-Back 16 Townhouse	Servicing Corridor	200
	2	2 Back-to-Back	Templeford Avenue	200
Block 43	8	8 Townhouse	Cope Drive	525
Block 46	36	16 Back-to-Back 20 Townhouse	Cope Drive	525
	4	4 Back-to-Back	Northgraves Crescent	200
Block 104	16	8 Back-to-Back 8 Townhouse	Carronbridge Circle	450

4.3.2 WASTEWATER PEAK FLOW

The associated service areas are shown on **Drawing SA-1**. A sanitary sewer design sheet is included in **Appendix B.2**. The anticipated wastewater dry weather peak flow (DWPF) and wet weather peak flow (WWPF) for the Trails West – Cope Drive Units development blocks are summarized in the following table. The corresponding WWPF from the original 2007 subdivision design is also included for reference.



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4 Wastewater Servicing**

Table 8: Wastewater Peak Flow

Development Area	Number of Units	Population	DWPF (L/s)	Extraneous Flow Allowance (L/s)	WWPF (L/s)	2007 WWPF (L/s)
Block 24	30	83	0.97	0.17	1.14	2.09
	2	6	0.07	0.02	0.09	N/A
Block 43	8	22	0.26	0.05	0.31	0.57
Block 46	36	100	1.17	0.23	1.39	2.83
	4	11	0.13	0.02	0.15	N/A
Block 104	16	44	0.53	0.11	0.64	1.15

Relative to the original 2007 subdivision condition, there is no concern associated with the capacity or function of the wastewater servicing systems supporting the Cope Drive Units development area blocks.

4.3.3 HAZELDEAN PUMP STATION OVERFLOW

The Hazeldean Pump Station is designed with an overflow elevation of 95.00 m. The pump station design indicates that the water level in the pump station would be approximately 95.30 m during an overflow.

The proposed underside-of-footing (USF) elevations for the Cope Drive Units development blocks are all below the pump station overflow elevation so backwater conditions are anticipated in the event of an overflow at the pump station. To mitigate the potential impact to the Cope Drive Units development blocks, backwater valves are required on all sanitary service connections.



5 Stormwater Management

5.1 Existing Conditions

The Trails West development is located within the stormwater catchment area for Cell 1 of the Monohan Drain stormwater management (SWM) facility. Minor and major system runoff from the catchment is directed to the facility. Upstream of the minor system inlets to the Monohan Drain, water quality treatment is also provided for all applicable contributing areas.

The proposed site layouts within the Cope Drive Units development block areas are consistent with the design conditions considered by the Monohan Drain and no further evaluation of the existing facility is required.

5.1.1 ALLOWABLE STORM SEWER PEAK FLOW

A 450mm diameter storm sewer exists along Carronbridge Circle, and a 1800mm x 1200mm box section storm sewer exists along Cope Drive and in the servicing corridor south of Cope Drive at Northgraves Crescent.

A copy of the original 2007 subdivision storm sewer design sheet and associated Storm Drainage Plans are included in **Appendix C.1** for reference. From this 2007 subdivision information, the previously allocated controlled storm sewer peak flows for the Trails West – Cope Drive Units areas are extracted for each development block area. The uncontrolled storm water runoff from the permitter of each development area block is also extracted as the representative contributing area and runoff coefficient value.

The following table summarizes the information taken from the 2007 subdivision design.

Table 9: 2007 Storm Water Controlled and Uncontrolled Flow

Development Area	2007 Area Reference	100-Year Controlled Storm Sewer Peak Flow (L/s)	Uncontrolled Runoff Area (ha)	Uncontrolled Runoff Coefficient, C
Block 24	1-52A, B	66	0.24	0.66
Block 43	N/A	N/A	0.14	0.70
Block 46	1-43D, E, F, G	88	0.34	0.64
Block 104	2-OA	44	0.17	0.64

Runoff from Block 43 is accommodated by the existing storm sewer system as uncontrolled runoff with no direct storm sewer connection. There is no applicable controlled storm sewer peak flow to reference.



5.1.2 HYDRAULIC GRADE LINE CONDITION

In association with the controlled flow allocations from the development block areas, the hydraulic grade line (HGL) condition associated with the 2007 subdivision design is also considered. The following table summarizes the HGL results for the maintenance hole (MH) locations associated with the Cope Drive Units. The data is taken from the reference document - Cavanagh Construction – Soho West (Phases 1 and 2) SWM Report (Stantec).

Table 10: 2007 Subdivision Hydraulic Grade Line Elevations

Location Reference	MH Reference	HGL Elevation (m)
Phase 1		
Northgraves Crescent	1043	94.83
	1043A	94.68
Templeford Avenue Cope Drive	1012	94.61
	1013	94.63
	1014	94.62
	1015	94.61
	1016	94.58
	1048	94.55
Service Corridor	1048A	94.57
	1052	94.50
	Diversion-1053	94.50
Phase 2		
Carronbridge Circle	2000A	94.99
	2000	94.78
	2001	94.77
	2002	94.73
	2003	94.72
	2004	94.69
	2005	94.65

The HGL conditions associated with the 2007 subdivision design are the primary comparison metric for the current SWM design associated with the Cope Drive Units.

5.2 Design Criteria and Constraints

Criteria were established by combining current design practices outlined by the City of Ottawa Design Guidelines (2012), through the various background documents and through consultation with City of Ottawa staff. The following summarizes the criteria, with the source of each criterion indicated in brackets:



Trails West: Cope Drive Units

5 Stormwater Management

General

- Major and minor flow to be conveyed the Monahan Drain, Cell 1 SWM facility for quality (80% TSS removal via upstream Vortechs® units) and quantity control.
- Use of the dual drainage principle (City of Ottawa).
- Assess impact of 100-year event and climate change event outlined in the City of Ottawa Sewer Design Guidelines on major & minor drainage system (City of Ottawa).

Storm Sewer & Inlet Controls

- Boundary conditions and HGL elevations for the storm sewer outlets based on the hydraulic conditions described in the Cavanagh Construction – Soho West (Phases 1 and 2) SWM Report, (Stantec).
- Size storm sewers to convey the 5-year design storm event under free-flow conditions (City of Ottawa).
- The 100-year HGL is to be a minimum of 0.30 m below building foundation footing (City of Ottawa).
- Climate Change event HGL to be below building foundation footing (City of Ottawa).

Surface Storage & Overland Flow

- No surface ponding is to be permitted on site roads during the 2-year or 5-year storm event (City of Ottawa).
- Maximum depth of flow under either static or dynamic conditions shall be less than 0.35m for design storm events (i.e., up to 100-year storm) (City of Ottawa).
- Minimum clearance depth of 0.30m to be provided from rear yard spill elevation to the ground elevation at the adjacent building envelope (City of Ottawa).
- Minimum clearance depth of 0.15m to be provided from spill elevations within the proposed rights-of-way to building envelopes in proximity of overland flow routes or ponding areas (City of Ottawa).
- Water must not encroach upon proposed building envelopes and must remain below all proposed building openings during the climate change event (City of Ottawa).
- Provide adequate emergency overflow conveyance off-site (City of Ottawa).
- No rear-yard ponding volumes to be accounted for in SWM model preparation (City of Ottawa).
- The product of depth times velocity on streets not to be greater than 0.6 during the 100-year storm (City of Ottawa).



5.2.1 MODELING

A comprehensive hydrologic modeling exercise is completed with PCSWMM, accounting for the estimated major and minor systems to evaluate the storm sewer infrastructure. The use of PCSWMM for modeling of the development block area hydrology and hydraulics allowed for an analysis of the systems response during various storm events. Surface storage estimates are based on the final grading plan design (see **Drawing GP-1**). The following assumptions are applied to PCSWMM:

- Hydrologic parameters as per the City of Ottawa Sewer Design Guidelines (OSDG), including Manning's 'n', and depression storage values.
- Subcatchment infiltration parameters per Horton Infiltration method per Ottawa Sewer Design Guidelines.
- The following storm distributions are assessed: 3-hour Chicago Storm distribution for the 2, 5, and 100-year analysis.
- To 'stress test' the system a 'climate change' scenario is created by adding 20% of the individual intensity values of the 100-year Chicago storm event at their specified time step.
- Percent imperviousness calculated based on actual soft and hard surfaces on each subcatchment, converted to equivalent Runoff Coefficient using the relationship $C = (\text{Imp.} \times 0.7) + 0.2$
- Subcatchment areas are defined from high-point to high-point where sags occur. Subcatchment width is determined by multiplying street segment length x 2 (length of overland flow path measured from high-point to high-point) for street (double-sided) catchments, multiplying by 1.5 for single-loaded roads, multiplying by 1.0 for single-sided catchments, or by multiplying the subcatchment area by 225m where a street segment flow path has not otherwise been defined.
- Number of catch basins (CBs) based on the proposed servicing plan. (**Drawing SSP-1**)
- CB inflow restricted with inlet-control devices (ICDs) as necessary to maintain inflow target rate, maximize use of surface storage, and eliminate standing water during the 2-year event (5-year event level of service for collector roads). CBs are not to be interconnected.
- The CB rim elevation value is set based on the top-of-grate elevation + 0.30 m for the modelling analysis. This allows the water level to rise above the rim elevation during the major storm events and allows major system routing without loss of flow (noted within the 'flooding' section of model results) in the modeled system.
- For storage on roads with defined cross-sections, active storage was modeled based on actual conduit flow using cross-sections as detailed on **Drawing DS-1**.
- HGL elevations at the downstream boundary set based on the HGL conditions identified in the 2007 Cavanagh Construction – Soho West (Phases 1 and 2) SWM Report (Stantec).



5.2.1.1 SWMM Dual Drainage Methodology

The proposed subdivision is modeled in PCSWMM as a dual conduit system (see **Figure 2**), with: 1) circular conduits representing the sewers & junction nodes representing maintenance holes (MH); 2) irregular conduits using street-shaped cross-sections to represent the sawtoothed overland road network from high-point to low-point and storage nodes representing catchbasins. The dual drainage systems are connected via orifice link objects (or outlets) from storage node (i.e., CB) to junction (i.e., MH), and represent ICDs. Subcatchments are linked to the storage node representing the CB to direct runoff hydrographs to the minor system as the initial analysis condition.

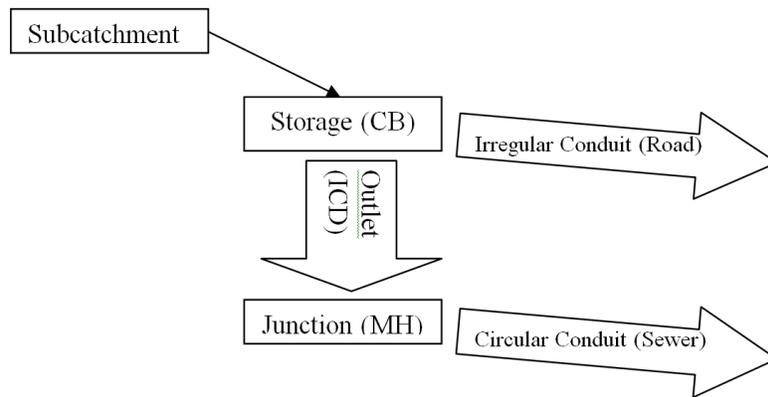


Figure 2: Schematic Representing Model Object Roles

Storage nodes are used in the model to represent CBs as well as major system junctions. For storage nodes representing CBs, the invert of the storage node represents the invert of the CB and the rim of the storage node is set at a constant increment elevation to allow for representation of depth of surface water over the storage node (to be limited to a maximum of 0.35m during the 100-year event). The additional depth is added to rim elevations to allow routing from one surface storage to the next. Storage nodes that represent CBs at sags are surrounded by two or more transects that represent the road segments forming the sag. The storage value assigned to the storage node represents only the volume available within the MH structure. If the available storage volume in a storage node is exceeded, flows spill above the storage node and into the sag in the irregular conduits (transects representing roads). The volume stored within the road sags includes the total static volume and the ponded depth above the node representing the dynamic flow depth. Flow storage volumes exceeding the sag storage available in the transect (roadway) will spill at the downstream highpoint into the next sag and continue routing through the system until ultimately flows either re-enter the minor system or reach the outfall of the major system. Storage nodes representing high points are assigned an invert elevation equal to the transect invert (spill elevation at edge of pavement) and a rim elevation equal to the maximum allowable flow depth elevation above the storage node (equal to the spill elevation at edge of pavement plus an additional 0.50 m). A storage value of zero is assigned to the high point nodes to disable linear volume calculations. No storage is considered within storage nodes at high points. In this manner, storage accumulates according to the actual ponding depths before spilling along the roadway conduit, and to the next downstream road conduit.



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ICDs as represented by orifice/outlet links using a user-specified diameter and discharge coefficient or functional head relationship taken from manufacturer's specifications and City guidelines for the chosen ICD model.

Subcatchment imperviousness is calculated via impervious area measured from **Drawing SSP-1**. It is of note that recent changes in interpretation of the OSDG have introduced the requirement to determine the proposed subcatchment imperviousness based on maximum zoning constraints rather than those of the builder anticipated maximum building size or based on other prevailing criteria such as minimum tree setbacks.

5.2.1.2 Boundary Conditions

The detailed PCSWMM hydrology and the proposed storm sewers are used to assess the peak inflows and HGL for the site. HGL elevations at the downstream boundary are set based on the HGL conditions identified in the Cavanagh Construction – Soho West (Phases 1 and 2) SWM Report.

To assess the relative changes between the 2007 subdivision HGL results and the current SWM design associated with the Cope Drive Units, it is necessary to re-create an equivalence to the 2007 conditions and analysis for comparison. The 2007 analysis used a combination of functions within the models DDSWMM and XP-SWMM. These models no longer being used but applicable data from these 2007 models is extracted and used as input into PCSWMM to re-create the 2007 conditions and HGL analysis.

The key data extracted from the 2007 DDSWMM and XP-SWMM files includes storm sewer design data, minor system inflow hydrograph data, and outfall tail water elevations. The minor system inflow hydrograph data is included in **Appendix C.1**. The storm sewer and outfall tailwater data is included in the following section with the rest of the input parameters applied to the Cope Drive Units SWM evaluation.

5.2.2 INPUT PARAMETERS

The following sections present and describe the input parameters applied to the PCSWMM analysis of the SWM design for the Cope Drive Units.

Drawing SD-1 illustrates the discretized subcatchments used in the analysis of the proposed site and outlines the major overland flow paths.

Appendix C.2 provides a PCSWMM layout figure and the input file for the 100-year 3hr Chicago storm. The PCSWMM data for the re-creation of the 2007 HGL analysis is also included in **Appendix C.2**. All other input files and results of storm scenarios are within the electronic model files provided with this report. Although this analysis was performed using PCSWMM, which provides a unique front-end graphic-user-interface to the EPA-SWMM engine, model data files can be examined in any program which can read EPA-SWMM files version 5.1.015.



5.2.2.1 Hydrologic Parameters

The following table presents the general subcatchment parameters used in the model.

Table 11: General Subcatchment Parameters

Parameter	Value
Infiltration Method	Horton
Max. Infiltration Rate (mm/hr)	76.2
Min. Infiltration Rate (mm/hr)	13.2
Decay Constant (1/hr)	4.14
Drying Time (d)	7
N Impervious	0.013
N Pervious	0.25
Dstore Imperv. (mm)	1.57
Dstore Perv. (mm)	4.67
Zero Imperv. (%)	0

The following table presents the individual parameters that vary for each of the proposed subcatchments. Allowance for the uncontrolled runoff areas around the perimeter of the development block areas is included within the minor system inflow hydrograph data extracted from the 2007 DDSWMM data files.

Table 12: General Subcatchment Parameters

Name	Outlet	Area (ha)	Width (m)	Slope (%)	Imperviousness (%)
Block 24					
C304A	CB-304	0.15	70	2	87
C306A	CB-306	0.12	70	2	87
C310A	CB-310	0.09	86	2	67
Block 46					
C502A	CB-502	0.12	82	2	54
C504A	CB-504	0.07	64	2	86
C506A	CB-506	0.11	64	2	89
C508A	CB-508	0.10	64	2	87
C512A	CB-512	0.09	85	2	69
Block 104					
C102A	CB-102	0.14	78	2	80

The following table summarizes the storage node parameters used in the model. Rim elevations for each node correspond to the rim elevation of the associated area CB plus maximum depth of storage plus an additional buffer depth to allow for potential conveyance of overland flow across high points in the road



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profile. Storage curves noted as 'functional' are to prevent any additional storage for the node from being applied. Storage will occur within the major system conduit (transect) connecting the storage nodes within the model.

A single junction node is also included in the table. The junction is used to connect converging major system segments in Block 24.

Table 13: Storage Node Parameters

Name	Invert (m)	Rim (m)	Depth (m)	Storage Curve	Curve Name
Phase 1 Subdivision					
1001	95.32	98.25	2.93	FUNCTIONAL	*
1002	95.07	98.03	2.96	FUNCTIONAL	*
1004	94.91	97.90	2.99	FUNCTIONAL	*
1005	94.67	97.90	3.23	FUNCTIONAL	*
1006	94.44	97.75	3.31	FUNCTIONAL	*
1007	95.16	98.00	2.84	FUNCTIONAL	*
1008	94.89	97.95	3.06	FUNCTIONAL	*
1009	94.69	97.75	3.06	FUNCTIONAL	*
1010	94.44	97.65	3.21	FUNCTIONAL	*
1011	94.11	97.40	3.29	FUNCTIONAL	*
1012	93.91	97.22	3.31	FUNCTIONAL	*
1013	93.61	96.60	2.99	FUNCTIONAL	*
1014	93.53	96.82	3.29	FUNCTIONAL	*
1015	93.43	96.91	3.48	FUNCTIONAL	*
1016	93.33	96.78	3.45	FUNCTIONAL	*
1017	95.14	97.70	2.56	FUNCTIONAL	*
1018	94.85	97.80	2.95	FUNCTIONAL	*
1019	94.59	97.50	2.91	FUNCTIONAL	*
1020	94.49	97.45	2.96	FUNCTIONAL	*
1021	94.38	97.30	2.92	FUNCTIONAL	*
1022	94.88	97.50	2.62	FUNCTIONAL	*
1023	94.57	97.40	2.83	FUNCTIONAL	*
1024	94.18	97.20	3.02	FUNCTIONAL	*
1025	94.04	97.25	3.21	FUNCTIONAL	*
1026	94.76	97.55	2.79	FUNCTIONAL	*
1027	94.44	97.43	2.99	FUNCTIONAL	*
1028	94.18	97.10	2.92	FUNCTIONAL	*
1029	93.91	97.05	3.14	FUNCTIONAL	*
1030	94.38	97.20	2.82	FUNCTIONAL	*



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Name	Invert (m)	Rim (m)	Depth (m)	Storage Curve	Curve Name
1031	94.01	97.10	3.09	FUNCTIONAL	*
1033	95.23	97.63	2.40	FUNCTIONAL	*
1034	95.11	97.50	2.39	FUNCTIONAL	*
1035	94.8	97.25	2.45	FUNCTIONAL	*
1036	94.6	97.45	2.85	FUNCTIONAL	*
1037	95.13	97.60	2.47	FUNCTIONAL	*
1038	94.94	97.45	2.51	FUNCTIONAL	*
1039	94.46	97.40	2.94	FUNCTIONAL	*
1040	94.76	97.25	2.49	FUNCTIONAL	*
1041	94.48	97.20	2.72	FUNCTIONAL	*
1042	94.09	97.20	3.11	FUNCTIONAL	*
1043	94.44	97.13	2.69	FUNCTIONAL	*
1044	94.65	97.10	2.45	FUNCTIONAL	*
1045	94.25	97.05	2.8	FUNCTIONAL	*
1046	93.86	96.70	2.84	FUNCTIONAL	*
1047	93.74	96.67	2.93	FUNCTIONAL	*
1048	93.24	96.50	3.26	FUNCTIONAL	*
1049	93.72	96.90	3.18	FUNCTIONAL	*
1050	93.7	96.80	3.10	FUNCTIONAL	*
1051	93.57	96.67	3.10	FUNCTIONAL	*
1052	93.1	97.10	4.00	FUNCTIONAL	*
1053	94.72	97.30	2.58	FUNCTIONAL	*
1006A	94.34	97.70	3.36	FUNCTIONAL	*
1012A	94.67	97.49	2.82	FUNCTIONAL	*
1012B	94.83	97.48	2.65	FUNCTIONAL	*
1043A	94.23	96.96	2.73	FUNCTIONAL	*
1048A	94.14	96.10	1.96	FUNCTIONAL	*
Phase 2 Subdivision					
2000	94.33	96.36	2.03	FUNCTIONAL	*
2001	94.24	96.96	2.72	FUNCTIONAL	*
2002	94.14	97.61	3.47	FUNCTIONAL	*
2003	94.11	97.62	3.51	FUNCTIONAL	*
2004	93.87	97.43	3.56	FUNCTIONAL	*
Block 24					
STM301	93.167	96.56	3.393	FUNCTIONAL	*
302_(C-STRM-NEW)	93.961	96.659	2.698	FUNCTIONAL	*
303_(C-STRM-NEW)	94.177	96.815	2.638	FUNCTIONAL	*



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Name	Invert (m)	Rim (m)	Depth (m)	Storage Curve	Curve Name
304_(C-STRM-NEW)	94.040	96.575	2.535	FUNCTIONAL	*
305_(C-STRM-NEW)	94.250	96.687	2.437	FUNCTIONAL	*
306_(C-STRM-NEW)	94.310	96.882	2.572	FUNCTIONAL	*
307_(C-STRM-NEW)	94.440	97.044	2.604	FUNCTIONAL	*
308_(C-STRM-NEW)	94.400	96.850	2.450	FUNCTIONAL	*
309_(C-STRM-NEW)	94.47	96.774	2.304	FUNCTIONAL	*
310_(C-STRM-NEW)	94.519	96.813	2.294	FUNCTIONAL	*
CB-304	95.070	96.750	1.680	TABULAR	Curve7
CB-306	95.100	96.780	1.680	TABULAR	Curve7
CB-310	95.310	96.900	1.590	TABULAR	Curve6
J1	96.60	96.60	0.00	N/A	N/A
Block 46					
STM501	93.348	96.78	3.432	FUNCTIONAL	*
502_(C-STRM-NEW)	94.146	96.573	2.427	FUNCTIONAL	*
503_(C-STRM-NEW)	94.198	96.649	2.451	FUNCTIONAL	*
504_(C-STRM-NEW)	94.294	96.740	2.446	FUNCTIONAL	*
505_(C-STRM-NEW)	94.510	96.948	2.438	FUNCTIONAL	*
506_(C-STRM-NEW)	94.458	96.787	2.329	FUNCTIONAL	*
507_(C-STRM-NEW)	94.600	97.065	2.465	FUNCTIONAL	*
508_(C-STRM-NEW)	94.639	96.840	2.201	FUNCTIONAL	*
509_(C-STRM-NEW)	94.768	96.997	2.229	FUNCTIONAL	*
510_(C-STRM-NEW)	94.741	96.829	2.088	FUNCTIONAL	*
511_(C-STRM-NEW)	94.811	96.807	1.996	FUNCTIONAL	*
512_(C-STRM-NEW)	94.878	97.093	2.215	FUNCTIONAL	*
CB-502	95.100	96.870	1.770	TABULAR	Curve5
CB-504	95.190	96.880	1.690	TABULAR	Curve4
CB-506	95.220	96.810	1.590	TABULAR	Curve3
CB-508	95.180	96.860	1.680	TABULAR	Curve2
CB-512	95.250	96.870	1.620	TABULAR	Curve1
Block 104					
STM100	94.400	96.251	1.851	FUNCTIONAL	*
102_(C-STRM-NEW)	94.526	96.107	1.581	FUNCTIONAL	*
103_(C-STRM-NEW)	94.590	96.211	1.621	FUNCTIONAL	*
104_(C-STRM-NEW)	94.730	96.473	1.743	FUNCTIONAL	*
CB-102	94.700	96.340	1.640	TABULAR	Curve8
For the 2007 Subdivision HGL Re-Creation Only					
1043D	94.53	97.03	2.50	FUNCTIONAL	*



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Name	Invert (m)	Rim (m)	Depth (m)	Storage Curve	Curve Name
1043E	94.74	97.00	2.26	FUNCTIONAL	*
1043F	94.91	96.86	1.95	FUNCTIONAL	*
CB1043G	95.01	96.92	1.78	FUNCTIONAL	*
1052A	94.20	97.15	2.95	FUNCTIONAL	*
1052B	94.37	97.34	2.97	FUNCTIONAL	*
2000A	94.79	96.77	1.98	FUNCTIONAL	*

5.2.2.2 Hydraulic Parameters

As per the OSDG, Manning’s roughness values of 0.013 are used for sewer modeling and overland flow corridors representing roadways. Subcatchment width is determined as described in **Section 5.2.1**. A conservative slope of 2-3% is applied and runoff coefficients remain consistent with the 2007 subdivision SWM Report (Stantec).

Storm sewers are modeled to confirm flow capacities and HGL elevations in the ultimate condition. The detailed storm sewer design sheet is included in **Appendix C.3**.

Maintenance hole loss coefficients are applied as conduit exit losses with values assigned per Appendix 6-B of the OSDG assuming no flow deflector in the maintenance hole.

The following table presents the parameters for the orifice objects in the model, which represent ICDs. All orifices are assigned a discharge coefficient of 0.572 to correspond to manufacturer supplied discharge curves for IPEX Tempest HF/MHF models. Should an approved equivalent model be required, the peak outlet rate of the selected model will be required to match that of the modeled ICD at the maximum head noted in the model results portion of this report.

Table 14: Orifice Parameters

Name	Inlet	Outlet	Inlet Elev. (m)	Type	Diameter (m)
Block 24					
CB-304A-1	CB-304	304_(C-STRM-NEW)	95.07	CIRCULAR	0.095
CB-304A-2	CB-304	304_(C-STRM-NEW)	95.07	CIRCULAR	0.095
CB-306A-1	CB-306	306_(C-STRM-NEW)	95.10	CIRCULAR	0.083
CB-306A-2	CB-306	306_(C-STRM-NEW)	95.10	CIRCULAR	0.083
CB-310A-1	CB-310	310_(C-STRM-NEW)	95.31	CIRCULAR	0.083
CB-310A-2	CB-310	310_(C-STRM-NEW)	95.31	CIRCULAR	0.083
Block 46					
CB-502A-1	CB-502	502_(C-STRM-NEW)	95.10	CIRCULAR	0.102
CB-504A-1	CB-504	504_(C-STRM-NEW)	95.19	CIRCULAR	0.075
CB-504A-2	CB-504	504_(C-STRM-NEW)	95.19	CIRCULAR	0.075



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Name	Inlet	Outlet	Inlet Elev. (m)	Type	Diameter (m)
CB-506A-1	CB-506	506_(C-STRM-NEW)	95.22	CIRCULAR	0.083
CB-506A-2	CB-506	506_(C-STRM-NEW)	95.22	CIRCULAR	0.083
CB-508A-1	CB-508	508_(C-STRM-NEW)	95.18	CIRCULAR	0.075
CB-508A-2	CB-508	508_(C-STRM-NEW)	95.18	CIRCULAR	0.083
CB-512A-1	CB-512	512_(C-STRM-NEW)	95.25	CIRCULAR	0.075
CB-512A-2	CB-512	512_(C-STRM-NEW)	95.25	CIRCULAR	0.070
Block 104					
CB-102A-1	CB-102	102_(C-STRM-NEW)	94.70	CIRCULAR	0.102
CB102-A-2	CB-102	102_(C-STRM-NEW)	94.70	CIRCULAR	0.102

To achieve the desired flow control condition, custom ICD sizes (< 0.083m diameter equivalent) are required within Block 46.

The following table presents the parameter applied to the PCSWMM Outfall locations for the 2007 subdivision HGL re-creation and the 100-year HGL analyses. The “outfall” locations selected are upstream of the actual outlets into Cell 1 of the Monahan Drain. However, for the purpose of this analysis they are considered representative of a suitable downstream reference location.

Table 15: Outfall Parameters

Name	Invert Elev. (m)	Rim Elev. (m)	Tide Gate	Type	Fixed Stage (m)
Diversion-1053	93.09	97.0	Yes	Fixed	94.48
2005	93.73	97.4	Yes	Fixed	94.65

For Block 24 and 46 the diversion MH ‘1053’ is selected as the “outfall” location. This simplifies the analysis by not requiring additional inputs to account for the flow diversion condition used to support the water quality enhancement system. The fixed tail water (stage) elevations applied at this Outfall is based on the 2007 subdivision analysis. However, with the difference in the nature of the 2007 models to the current model, the tail water elevation is adjusted from the 2007 analysis. This is done to generate results in the upstream storm sewer system that more closely resemble the 2007 HGL elevations at the reference MH locations. The nature of the results achieved with the HGL analysis re-creation is discussed further in **Section 5.3.3**.

For Block 104, MH ‘2—5’ is selected as the “outfall” location. This location is selected as a point that is far enough downstream of the connection point to show a representative comparison to the 2007 subdivision design. There is no adjustment to the fixed tail water (stage) elevations relative to the 2007 subdivision design.

Tide Gates are applied at each Outfall to prevent the fixed tail water elevation from moving upstream through the storm sewer system at the start of the simulation.



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For the 5-year and 2-year analyses, the MH ‘STM301’ at Block 24, ‘STM501’ at Block 46, and ‘STM100’ at Block 104 are converted to Outfalls with a ‘Free’ (no tailwater) condition applied.

5.3 Proposed Storm Sewer Sizing and Layout

Storm sewer servicing for the proposed development is provided through the existing storm sewer system in the Trails West development.

5.3.1 CONNECTION TO EXISTING INFRASTRUCTURE

The storm sewer service for each development area block is proposed via connections to the existing storm sewer located within the roadways and service corridors adjacent to each block. The service connections and the internal servicing layout for each development area block is shown on **Drawing SSP-1**.

Excepting Block 46, the proposed storm sewer connection locations are generally consistent with the original 2007 subdivision design conditions. The 2007 connection location for Block 46 was anticipated to occur along Northgraves Crescent. The current proposed connection for Block 46 along Cope Drive is downstream of the 2007 location so there is no relative change in the design flow condition downstream of the new connection location.

At Block 24 and 46 there are Back-to-Back Townhouse units that front directly onto an adjacent public road. These units are to have direct foundation drain connections to the existing storm sewer main in each respective public road. At Block 104, the foundation drain connection for the Back-to-Back Townhouse units that front Carronbridge Circle will be connected to storm sewer system within the Block.

The following table summarizes the number of units associated with each block and the related connection point to the existing infrastructure system.

Table 16: Connections to Existing Infrastructure

Development Area	Number of Units	Building Type	Connection Location	Existing Sewer Diameter (mm)
Block 24	30	14 Back-to-Back 16 Townhouse	Servicing Corridor	1800mm x 1200mm Box Section
	2	2 Back-to-Back	Templeford Avenue	825
Block 43	8	8 Townhouse	Monahan Drain	N/A
Block 46	36	16 Back-to-Back 20 Townhouse	Cope Drive	1800mm x 1200mm Box Section
	4	4 Back-to-Back	Northgraves Crescent	600
Block 104	16	8 Back-to-Back 8 Townhouse	Carronbridge Circle	450



5.3.2 MINOR AND MAJOR SYSTEM PEAK FLOW

A summary of both the uncontrolled and controlled conditions associated with the Cope Drive Units development block areas is provided in the following table.

Table 17: Storm Water Controlled and Uncontrolled Flow

Development Area	100-Year Controlled Storm Sewer Peak Flow (L/s)	Uncontrolled Runoff Area (ha)	Uncontrolled Runoff Coefficient, C
Block 24	110	0.19	0.55
Block 43	N/A	0.14	0.70
Block 46	140	0.24	0.52
Block 104	46	0.19	0.45

The storm sewer connection at Block 43 is for foundation drain support only. Runoff from Block 43 is routed directly to Cell 1 of the Monahan Drain south of Cope Drive. There is no applicable controlled storm sewer peak flow to reference.

To ensure that the 2-year and 5-year conditions are completely captured by the minor system, the 100-year storm sewer peak flow from the development block areas is greater than that previously considered with the original 2007 subdivision condition.

The uncontrolled flow area and related runoff coefficients for Block 24 and Block 46 are less than that previously considered with the original 2007 subdivision condition. Block 104 shows a 0.02 ha increase in total area and a decrease in the overall runoff coefficient. The change in the area is attributed to the nature of the current site layout. The increased area is considered part of the surface runoff that is routed directly to Cell 1 of the Monahan Drain. No further evaluation of the increase in uncontrolled runoff area from Block 104 is considered.

The following tables demonstrate the peak outflow from each modeled connection point to downstream infrastructure during the design storm events assessed.

Table 18: Storm Event Peak Discharge Rates (Minor System)

Development Area	Outlet Node	2-Year Peak Flow (m ³ /s)	5-Year Peak Flow (m ³ /s)	100-Year Peak Flow (m ³ /s)	100-Year + 20% Peak Flow (m ³ /s)
Block 24	302_(C-STRM-NEW)	0.108	0.108	0.110	0.111
Block 43	N/A	N/A	N/A	N/A	N/A
Block 46	502_(C-STRM-NEW)	0.079	0.114	0.140	0.143
Block 104	102_(C-STRM-NEW)	0.024	0.036	0.046	0.047



Table 19: Storm Event Peak Discharge Rates (Major System)

Development Area	Outlet Node	100-Year Peak Flow (m ³ /s)	100-Year + 20% Peak Flow (m ³ /s)
Block 24	OF2	0	0
Block 43	N/A	N/A	N/A
Block 46	OF1	0	0
Block 104	OF3	0	0.002

To confirm that there is no concern associated with the capacity or function of the storm sewer systems supporting the Cope Drive Units development area blocks, a review of the subdivision HGL conditions is completed.

5.3.3 HYDRAULIC GRADE LINE CONDITION

As noted in Section 5.2.1.2, a re-creation of the HGL analysis associated with the 2007 subdivision design is completed. The related PCSWMM information is included in **Appendix C.1** (2007 data hydrographs) and **Appendix C.2** (re-creation of 2007 analysis). The following table summarizes the results for the re-creation of the 2007 HGL analysis.

Table 20: Re-Created 2007 Subdivision Hydraulic Grade Line Elevations

Location Reference	MH Reference	HGL Elevation (m)	Difference from 2007 Result
Phase 1			
Northgraves Crescent	1043	94.79	- 0.04
	1043A	94.69	+ 0.01
Templeford Avenue Cope Drive	1012	94.66	+ 0.05
	1013	94.67	+ 0.04
	1014	94.65	+ 0.03
	1015	94.61	0.00
	1016	94.59	+ 0.01
	1048	94.55	0.00
Service Corridor	1048A	94.58	+ 0.01
	1052	94.49	- 0.01
	Diversion-1053	94.48	- 0.02
Phase 2			
Carronbridge Circle	2000A	94.91	- 0.08
	2000	94.80	+ 0.02
	2001	94.78	+ 0.01
	2002	94.73	0.00
	2003	94.72	0.00



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Location Reference	MH Reference	HGL Elevation (m)	Difference from 2007 Result
	2004	94.69	0.00
	2005	94.65	0.00

Differences between the original 2007 HGL results and the re-created 2007 subdivision HGL elevations are expected. The re-created conditions are considered generally consistent with the original results associated with the 2007 subdivision design.

For the applicable design storms, the following table presents the HGL conditions for the Cope Drive Unit development block areas and the relationship to the associated Underside-of-Footing (USF) elevations.

For Block 43, the HGL condition is taken at the adjacent subdivision maintenance hole to which the storm sewer supporting only the foundation drains within the block is connected. Because there is no storm sewer design flow from Block 43, the HGL elevation within Block 43 is considered the same as that at the adjacent subdivision maintenance hole '1048A'.

Table 21: Cope Drive Units Hydraulic Grade Line Elevations

Node ID	100-Year, 3-Hour HGL (m)	Lowest Adjacent USF Elevation (m)	100-Year HGL Depth below USF (m)	100-Year, 3-Hour + 20% HGL (m)	100-Year + 20% HGL Depth below USF (m)
Block 24					
STM301	94.53	N/A	N/A	94.53	N/A
302_(C-STRM-NEW)	94.59	94.95	0.36	94.60	0.35
303_(C-STRM-NEW)	94.59	94.95	0.36	94.60	0.35
304_(C-STRM-NEW)	94.65	94.95	0.30	94.65	0.30
305_(C-STRM-NEW)	94.65	94.95	0.30	94.65	0.30
306_(C-STRM-NEW)	94.82	95.12	0.30	94.83	0.29
307_(C-STRM-NEW)	94.82	95.12	0.30	94.83	0.29
308_(C-STRM-NEW)	94.84	95.16	0.32	94.85	0.31
309_(C-STRM-NEW)	94.86	N/A	N/A	94.87	N/A
310_(C-STRM-NEW)	94.87	N/A	N/A	94.88	N/A
1012	94.67	95.12	0.46	94.67	0.46
Block 43					
1048A	94.59	94.97	0.38	94.58	0.39
Block 46					
STM501	94.60	N/A	N/A	94.60	N/A
502_(C-STRM-NEW)	94.70	95.11	0.41	94.70	0.41
503_(C-STRM-NEW)	94.73	N/A	N/A	94.73	N/A
504_(C-STRM-NEW)	94.79	95.09	0.30	94.80	0.29
505_(C-STRM-NEW)	94.79	95.11	0.32	94.80	0.31



**Trails West: Cope Drive Units
5 Stormwater Management**

Node ID	100-Year, 3-Hour HGL (m)	Lowest Adjacent USF Elevation (m)	100-Year HGL Depth below USF (m)	100-Year, 3-Hour + 20% HGL (m)	100-Year + 20% HGL Depth below USF (m)
506_(C-STRM-NEW)	94.88	95.18	0.30	94.89	0.29
507_(C-STRM-NEW)	94.88	95.20	0.32	94.89	0.31
508_(C-STRM-NEW)	94.98	95.28	0.30	94.99	0.30
509_(C-STRM-NEW)	94.98	95.37	0.39	94.99	0.38
510_(C-STRM-NEW)	95.00	95.37	0.37	95.01	0.36
511_(C-STRM-NEW)	95.01	N/A	N/A	95.02	N/A
512_(C-STRM-NEW)	95.04	N/A	N/A	95.04	N/A
1043A	94.68	95.37	0.69	94.68	0.69
Block 104					
STM100	94.91	N/A	N/A	94.92	N/A
102_(C-STRM-NEW)	94.98	N/A	N/A	94.98	N/A
103_(C-STRM-NEW)	94.98	95.33	0.35	94.98	0.35
104_(C-STRM-NEW)	94.98	95.33	0.35	94.98	0.35

As demonstrated in the table above, the HGL elevations remain at least 0.30 m below the proposed underside of footings, and HGL elevations remain below the proposed underside of footing elevations during the 20% increased intensity 'climate change' scenario.

The results for the 100-Year + 20% HGL condition maintain the 2007 minor system flow hydrographs for the 100-year condition. Applicable data from a 100-year + 20% condition is not available from the 2007 Cavanagh Construction – Soho West (Phases 1 and 2) SWM Report (Stantec).

With the Cope Drive Unit conditions considered, the following table provides a summary of the resultant HGL conditions at the applicable adjacent subdivision reference locations. The updated HGL elevations are also compared to the USF elevations described in the 2007 Cavanagh Construction – Soho West (Phases 1 and 2) SWM Report (Stantec).

Table 22: 2023 Subdivision Hydraulic Grade Line Elevations

Location Reference	MH Reference	2023 HGL Elevation (m)	HGL Elevation Change (m)	2007 Lowest Adjacent USF Elevation (m)	2023 HGL Depth below 2007 USF (m)
Phase 1					
Northgraves Crescent	1043	94.79	0.00	95.27	0.48
	1043A	94.68	- 0.01	95.27	0.59
Templeford Avenue	1012	94.67	+ 0.01	N/A	N/A
Cope Drive	1013	94.66	- 0.01	N/A	N/A
	1014	94.63	- 0.02	N/A	N/A
	1015	94.62	+ 0.01	N/A	N/A



**Trails West: Cope Drive Units
5 Stormwater Management**

Location Reference	MH Reference	2023 HGL Elevation (m)	HGL Elevation Change (m)	2007 Lowest Adjacent USF Elevation (m)	2023 HGL Depth below 2007 USF (m)
	1016	94.59	0.00	N/A	N/A
	1048	94.56	+ 0.01	N/A	N/A
	1048A	94.59	+ 0.01	N/A	N/A
Service Corridor	1052	94.49	0.00	N/A	N/A
	Diversion-1053	94.48	0.00	N/A	N/A
Phase 2					
Carronbridge Circle	2000A (STM100)	94.91	0.00	N/A	N/A
	2000	94.77	- 0.03	N/A	N/A
	2001	94.75	- 0.03	95.39	0.64
	2002	94.72	- 0.01	95.39	0.67
	2003	94.71	- 0.01	95.20	0.49
	2004	94.68	- 0.01	95.22	0.54
	2005	94.65	0.00	95.33	0.68

The change in the HGL conditions within the adjacent subdivision resulting from the controlled storm sewer peak flow of the Cope Drive Units is considered negligible. There is no impact on the original lowest USF elevations from the 2007 subdivision design.

5.3.4 ON-SITE STORAGE

The following table presents the maximum total surface water depths (static ponding depth + dynamic flow) above the catch basin top-of-grate elevation for the 100-year design storm and 100-Year + 20% 'climate change' storm.

Table 23: Cope Drive Units Maximum Surface Water Depths

Storage Node ID	Top of Grate Elevation (m)	100-Year, 3-Hour		100-Year, 3-Hour + 20%		Adjacent Building Opening Elevation (m)
		Surface HGL (m)	Surface Depth (m)	Surface HGL (m)	Surface Depth (m)	
Block 24						
CB-304	96.45	96.60	0.15	96.64	0.19	96.83
CB-306	96.48	96.61	0.13	96.65	0.17	96.85
CB-310	96.69	96.78	0.09	96.82	0.13	N/A
Block 46						
CB-502	96.48	96.60	0.12	96.63	0.15	96.95
CB-504	96.57	96.66	0.09	96.71	0.14	96.95
CB-506	96.60	96.73	0.13	96.75	0.15	96.98
CB-508	96.56	96.70	0.14	96.75	0.19	96.96



**Trails West: Cope Drive Units
5 Stormwater Management**

CB-512	96.63	96.77	0.14	96.80	0.17	97.20
Block 104						
CB-102	96.08	96.21	0.13	96.26	0.18	96.54

Based on the analysis results, the total ponding depth (static + dynamic) does not exceed the required 0.35m maximum during the 100-year event. Total ponding depths during the climate change scenario are below adjacent building openings and are not anticipated to impact proposed buildings within the development block areas.

There is no major system overflow from the Cope Drive Units in the 100-year design storm scenario. For the 100-year + 20% 'climate change' design storm scenario, a 0.002 m³/s overflow occurs at Block 104 only. This overflow is not considered a significant impact to the downstream drainage system.

The following table presents the anticipated ponding depths during the 2-year and 5-year minor system design storm events.

Table 24: Cope Drive Units Minor System Design Storm Ponding Depths

Storage Node ID	Top of Grate Elevation (m)	2-Year, 3-Hour		5-Year, 3-Hour	
		Maximum HGL (m)	Surface Depth (m)	Maximum HGL (m)	Surface Depth (m)
Block 24					
CB-304	96.45	95.75	0	96.42	0
CB-306	96.48	95.80	0	96.47	0
CB-310	96.69	95.60	0	95.98	0
Block 46					
CB-502	96.48	95.58	0	96.28	0
CB-504	96.57	95.59	0	95.97	0
CB-506	96.60	95.82	0	95.39	0
CB-508	96.56	95.83	0	96.46	0
CB-512	96.63	95.69	0	96.26	0
Block 104					
CB-102	96.08	95.10	0	95.52	0

Based on the analysis results, there is no surface ponding during the 2-year and 5-year minor system design storm events within the development block areas.

5.4 Water Quality Control

Water quality control for the proposed development is provided in the existing Vortechs® Systems located upstream of the Phases 1 and 2 outlets from the Trail West development in the Monohan Drain. The Vortechs® units for the Trail West development are sized for a minimum of 80% total net annual TSS removal.



Trails West: Cope Drive Units
5 Stormwater Management

Blocks 24 and 46 are accommodated in the Vortechs® Model 1929CIP supporting the Phase 1 outlet.
Block 104 is accommodated in the Vortechs® Model 9000 supporting the Phase 2 outlet.

As considered by the 2007 subdivision design, runoff from Block 43 is permitted to contribute directly to the Monohan Drain.



6 Utilities

All Utilities (Hydro Ottawa, Bell Canada, Rogers Ottawa, and Enbridge Gas) are available to service the Cope Drive Units development block areas in the Trails West development.



7 Geotechnical Information and Grading

The proposed Trails West – Cope Drive Units are currently vacant properties and are generally covered with granular fill or sparse vegetation. Based on the geotechnical site investigation (Houle Chevrier, March 2016), the site subsurface conditions can generally be described as; a thin layer of fill ranging from about 0.6 to 1.5 m thick, overlying a 0.15 m thick layer of organics, over a layer of silty sand ranging from about 0.3 to 2.8 m in thickness, overlying grey silty clay ranging from about 1.3 to 3.7 m below existing surface grade.

Groundwater levels were measured on July 14, 2015 in two standpipe piezometers and were found to be between 1.4 and 1.7 m below existing grades. It should be noted that groundwater levels may be higher during wet periods of the year such as the early spring or following periods of precipitation, particularly within the upper silty sand deposits.

As described in the 2023 geotechnical investigation letter included in **Appendix D**, the development block areas proposed for the residential buildings are underlain by thick deposits of compressible silty clay which has a reduced capacity to support loads imposed by grade raise fill materials, pavement structures, etc. Groundwater lowering also causes a stress increase on the underlying silty clay deposits. As a result, the proposed site grading attempts to limit grade raises to 1.0m or less. It is noted that there are isolated areas where the grade raise exceeds 1.0m, however coordination with the Geotechnical Engineer during construction will be completed to implement appropriate mitigation measures at each applicable location.

Grading of the proposed development sites is controlled by existing grades in the adjacent properties and streets as well as stormwater management requirements such as the 0.3m clearance between the spill grade elevation in the access roads and the grade at the proposed buildings. The proposed grading plan is designed to provide overland flow outlets for each block, as outlined in the Soho West Phases 1 and 2 SWM Report (Stantec). **Drawing GP-1** illustrates the proposed grading plan.

The roads within each development area block are designed with a saw-tooth pattern to create storm water storage opportunities. Depths of ponding do not exceed 0.30 m.

Due to the presence of peat and the expected grade raises the geotechnical engineer will be consulted to confirm the most appropriate method of construction in the geotechnical sensitive regions. Upon excavation of the peat at time of construction, the Geotechnical Engineer will confirm recommendations on subgrade depths for roads, footing requirements, and pipe support.

For further details from the Geotechnical Investigations please refer to the complete reports submitted under separate cover.



8 Erosion and Sediment Control

To control erosion and migration of sediment-laden runoff off site during construction, an erosion and sediment control plan will be required for the proposed Trails West – Cope Drive Units. Additionally, an appropriate inspection and maintenance program employed by the contractor is required and will consider the following goals:

- Protection from sedimentation of the downstream existing infrastructure and water course using, but not limited to, temporary sediment traps/basins, silt fence, rock-check dams, diversion swales/berms, raised and/or covered catch basins with filter fabric, sandbags, etc.
- Re-vegetating exposed areas and slopes as soon as possible to control sediment wash-off and erosion.
- Protect exposed slopes with plastic or synthetic mulches.
- Frequent inspection of all controls during construction and after significant rainfall events (greater than 13 mm) for sediment accumulation and erosion.
- Immediate repair of all noticeable erosion, with investigation into the cause so implementation of mitigation measures to prevent recurrence will be more successful.
- Maintenance of the erosion control measures during construction.
- Preparation of monitoring reports outlining the condition of erosion control works, their overall performance, and any actions such as repairs, replacement, or modification.

The contractor will, at every rainfall, complete inspections and guarantee proper performance. The inspection is to include:

1. Verification that water is not flowing under silt barriers.
2. Clean and change sediment traps at catch basins.

Refer to **Drawing EC/DS-1** for the proposed location of silt fences, straw bales, catch basin sediment traps, and other erosion control measures.



9 Approvals

An Environmental Compliance Approval (ECA) issued by the Ontario Ministry of Environment, Conservation, and Parks (MECP) is not expected to be required for the Cope Drive Units development block areas. Each block is intended to be held as a private single ownership.

An MECP Permit to Take Water (PTTW) may be required for the development if more than 400,000 L/day of ground or surface water is to be pumped during the construction works. If between 50,000 L/day and 400,000 L/day are expected to be pumped during the construction works, registration on the Environmental Activity and Sector Registry (EASR) are required. The geotechnical consultant shall confirm prior to construction whether a PTTW or EASR registry are required for construction dewatering purposes.

No submission to the South Nation Conservation Authority (SNCA) is required.

Typical development approval requirements from The City of Ottawa, as the municipal regulatory agency, are anticipated.



10 Closing

The following summarizes key considerations for the primary servicing and stormwater management infrastructure systems supporting the Trails West – Cope Drive Units development.

10.1 Potable Water Servicing

The proposed development is located within Zone 3W of the City of Ottawa water distribution system. The 3W pressure zone is fed by the Glen Cairn booster pumping station with the Stittsville elevated storage tank providing balanced storage for peak flows and demands. A 400mm diameter watermain exists along Akerson Road / Carronbridge Circle, a 300mm diameter watermain exists along Cope Drive, and 200mm diameter watermains exist along Northgraves Crescent and Templeford Avenue.

The proposed watermain alignment and sizing achieves the desired level of service in the proposed development block area. Based on the design and related hydraulic analysis, the following is noted.

- Proposed watermain diameters of 150mm are used in each development block area.
- During peak hour (PKHR) conditions, the supporting watermain network operates above the minimum pressure objective of 40 psi (276 kPa).
- During peak hour (PKHR) conditions, the supporting watermain network operates above the maximum pressure objective of 80psi (552 kPa). Pressure reducing measures are required to comply with Ontario Building Code guidelines.
- The supporting watermain network provides sufficient fire flow while maintaining a residual pressure of 20 psi (138 kPa) in all applicable areas.

10.2 Wastewater Servicing

The Trails West development is located within the South Glen Cairn wastewater catchment area. Wastewater flow from the development is supported by the South Glen Cairn Trunk Sewer and the Hazeldean Pump Station.

A 450mm diameter sewer exists along Akerson Road / Carronbridge Circle, a 525mm diameter sewer exists along Cope Drive, and a 200mm diameter sewer exists in the servicing corridor south of Cope Drive at Northgraves Crescent. These existing sewers provide the service connections to the respective development block areas associated with the Cope Drive Units development.

The preferred cover requirement of 2.5 m for the sanitary sewer system is satisfied in all locations, and requirements for slope and velocities are met within the local internal sewers. There is no concern associated with the capacity or function of the wastewater servicing systems supporting the Cope Drive Units development area blocks.



Trails West: Cope Drive Units 10 Closing

The proposed underside-of-footing elevations for the Cope Drive Units development blocks are all below the Hazeldean Pump Station overflow elevation so backwater conditions are anticipated in the event of an overflow at the pump station. To mitigate the potential impact to the Cope Drive Units development blocks, backwater valves are required on all sanitary service connections.

10.3 Stormwater Management

The Trails West development is located within the stormwater catchment area for Cell 1 of the Monohan Drain stormwater management (SWM) facility. Minor and major system runoff from the catchment is directed to the facility. Upstream of the minor system inlets to the Monohan Drain, water quality treatment is also provided for all contributing areas.

The proposed stormwater management plan is consistent with the goals specified in the background reports and the 2012 City of Ottawa Sewer Guidelines:

- Inlet control devices are proposed to limit inflow from the site area into the minor system based on design storms derived from the City of Ottawa IDF curves.
- No ponding occurs during the 2-year and 5-year design storm events.
- The storm sewer hydraulic grade line is maintained at least 0.30 m below the underside of footing in the subdivision and downstream properties during design storm events.
- All dynamic surface water depths are less than or equal to 0.35 m during all design storm events up to the 100-year event.
- The downstream Monahan Drian Cell 1 SWM Facility has sufficient volume capacity to receive runoff volumes from the proposed site and provide the required water quantity and quality control as considered by the supporting subdivision design described in the Cavanagh Construction – Soho West (Phases 1 and 2), Kanata South, City of Ottawa Stormwater Management Report, Stantec Consulting Ltd., October 2007.

10.4 Grading

The grading for this site is designed to allow for an emergency overland flow outlet to downstream rights-of-way as per City standards and to minimize the grade raise per restrictions as recommended by the Geotechnical Investigation by Gemtec (formerly Houle Chevrier).

10.5 Approvals

An Environmental Compliance Approval (ECA) issued by the Ontario Ministry of Environment, Conservation, and Parks (MECP) is not expected to be required for the Cope Drive Units development block areas. Each block is intended to be held as a private single ownership.



Trails West: Cope Drive Units
10 Closing

An MECP Permit to Take Water (PTTW) may be required for the development if more than 400,000 L/day of ground or surface water is to be pumped during the construction works. If between 50,000 L/day and 400,000 L/day are expected to be pumped during the construction works, registration on the Environmental Activity and Sector Registry (EASR) are required. The geotechnical consultant shall confirm prior to construction whether a PTTW or EASR registry are required for construction dewatering purposes.

No submission to the South Nation Conservation Authority (SNCA) is required.

Typical development approval requirements from The City of Ottawa, as the municipal regulatory agency, are anticipated.



APPENDIX A



Appendix A Potable Water Servicing

A.1 Water Demand Calculations



Domestic Water Demand Estimates

Trails West: Cope Drive Units - 80 Cope Drive (Block 104)

Based on Site Plan from M. David Blakely Architect Inc. Dated November 2022

Densities as per City Guidelines:		
Townhomes (Row)	2.7	ppu
Back-to-Back Townhomes	2.7	ppu

Building ID	No. of Units	Population	Daily Rate of Demand (L/cap/day)	Avg Day Demand		Max Day Demand ¹		Peak Hour Demand ²	
				(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Back-to-Back Townhomes Block 1	8	22	280	4.3	0.07	10.7	0.18	23.5	0.39
Townhouse Blocks Block 2	8	22	280	4.3	0.07	10.7	0.18	23.5	0.39
Total Site :	16	44		8.6	0.14	21.4	0.36	47.1	0.78

1. Average day water demand for residential areas: 280 L/cap/d

2. The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

maximum day demand rate = 2.5 x average day demand rate for residential

peak hour demand rate = 2.2 x maximum day demand rate for residential

Domestic Water Demand Estimates

Trails West: Cope Drive Units - 110 Cope Drive (Block 43)

Based on Site Plan from M. David Blakely Architect Inc. Dated November 2022

Densities as per City Guidelines:		
Townhomes (Row)	2.7	ppu
Back-to-Back Townhomes	2.7	ppu

Building ID	No. of Units	Population	Daily Rate of Demand (L/cap/day)	Avg Day Demand		Max Day Demand ¹		Peak Hour Demand ²	
				(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Townhouse Blocks									
Block 1	6	17	280	3.3	0.06	8.3	0.14	18.2	0.30
Block 2	2	6	280	1.2	0.02	2.9	0.05	6.4	0.11
Total Site :	8	23		4.5	0.07	11.2	0.19	24.6	0.41

1. Average day water demand for residential areas: 280 L/cap/d

2. The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

maximum day demand rate = 2.5 x average day demand rate for residential

peak hour demand rate = 2.2 x maximum day demand rate for residential

Domestic Water Demand Estimates

Trails West: Cope Drive Units - 140 Cope Drive (Block 24)

Based on Site Plan from M. David Blakely Architect Inc. Dated November 2022

Densities as per City Guidelines:		
Townhomes (Row)	2.7	ppu
Back-to-Back Townhomes	2.7	ppu

Building ID	No. of Units	Population	Daily Rate of Demand (L/cap/day)	Avg Day Demand		Max Day Demand ¹		Peak Hour Demand ²	
				(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Back-to-Back Townhomes									
Block 1	8	22	280	4.3	0.07	10.7	0.18	23.5	0.39
Block 2	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Block 3	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Townhouse Blocks									
Block 4	6	17	280	3.3	0.06	8.3	0.14	18.2	0.30
Block 5	6	17	280	3.3	0.06	8.3	0.14	18.2	0.30
Block 6	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Total Site :	32	89		17.3	0.29	43.3	0.72	95.2	1.59

1. Average day water demand for residential areas: 280 L/cap/d

2. The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

maximum day demand rate = 2.5 x average day demand rate for residential

peak hour demand rate = 2.2 x maximum day demand rate for residential

Domestic Water Demand Estimates

Trails West: Cope Drive Units - 151 Cope Drive (Block 46)

Based on Site Plan from M. David Blakely Architect Inc. Dated November 2022

Densities as per City Guidelines:		
Townhomes (Row)	2.7	ppu
Back-to-Back Townhomes	2.7	ppu

Building ID	No. of Units	Population	Daily Rate of Demand (L/cap/day)	Avg Day Demand		Max Day Demand ¹		Peak Hour Demand ²	
				(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Back-to-Back Townhomes									
Block 1	8	22	280	4.3	0.07	10.7	0.18	23.5	0.39
Block 2	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Block 3	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Block 4	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Townhouse Blocks									
Block 5	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Block 6	4	11	280	2.1	0.04	5.3	0.09	11.8	0.20
Block 7	6	17	280	3.3	0.06	8.3	0.14	18.2	0.30
Block 8	6	17	280	3.3	0.06	8.3	0.14	18.2	0.30
Total Site :	40	111		21.6	0.36	54.0	0.90	118.7	1.98

1. Average day water demand for residential areas: 280 L/cap/d

2. The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

maximum day demand rate = 2.5 x average day demand rate for residential

peak hour demand rate = 2.2 x maximum day demand rate for residential

A.2 Fire Flow Calculations





FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 140 Cope Drive (Block 24)

Date: 3/23/2023

Fire Flow Calculation #: 1

Description: 8 Unit, Back-to-Back Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		430 430 430	1290	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	12000
4	Determine Occupancy Charge	Limited Combustible	-15%	10200
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North 10.1 to 20 25 3 61-80 Type V NO	13%	1530
		East 20.1 to 30 18 2 21-49 Type V NO	2%	
		South > 30 25 3 61-80 Type V NO	0%	
		West > 30 18 3 41-60 Type V NO	0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		12000
		Total Required Fire Flow in L/s		200.0
		Required Duration of Fire Flow (hrs)		2.50
		Required Volume of Fire Flow (m ³)		1800



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 140 Cope Drive (Block 24)

Date: 3/23/2023

Fire Flow Calculation #: 2

Description: 6 Unit, Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		335 335 335	1005	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	10000
4	Determine Occupancy Charge	Limited Combustible	-15%	8500
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North 0 to 3 13 3 21-49 Type V NO	21%	5780
		East 10.1 to 20 28 2 41-60 Type V NO	12%	
		South 0 to 3 13 3 21-49 Type V NO	21%	
		West 10.1 to 20 28 3 81-100 Type V NO	14%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		14000
		Total Required Fire Flow in L/s		233.3
		Required Duration of Fire Flow (hrs)		3.00
		Required Volume of Fire Flow (m ³)		2520



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 110 Cope Drive (Block 43)

Date: 3/23/2023

Fire Flow Calculation #: 3

Description: 6 Unit, Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		335 335 335	1005	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	10000
4	Determine Occupancy Charge	Limited Combustible	-15%	8500
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North 10.1 to 20 13 3 21-49 Type V NO	11%	1785
		East 20.1 to 30 13 2 21-49 Type V NO	2%	
		South 20.1 to 30 28 3 81-100 Type V NO	8%	
		West > 30 13 2 21-49 Type V NO	0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		10000
		Total Required Fire Flow in L/s		166.7
		Required Duration of Fire Flow (hrs)		2.00
		Required Volume of Fire Flow (m ³)		1200



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 151 Cope Drive (Block 46)

Date: 3/23/2023

Fire Flow Calculation #: 4

Description: 8 Unit, Back-to-Back Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		430 430 430	1290	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	12000
4	Determine Occupancy Charge	Limited Combustible	-15%	10200
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North 10.1 to 20 25 3 61-80 Type V NO	13%	1938
		East > 30 18 3 41-60 Type V NO	0%	
		South 20.1 to 30 25 2 41-60 Type V NO	4%	
		West 20.1 to 30 18 2 21-49 Type V NO	2%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		12000
		Total Required Fire Flow in L/s		200.0
		Required Duration of Fire Flow (hrs)		2.50
		Required Volume of Fire Flow (m ³)		1800



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 151 Cope Drive (Block 46)

Date: 3/23/2023

Fire Flow Calculation #: 5

Description: 6 Unit, Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		335 335 335	1005	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	10000
4	Determine Occupancy Charge	Limited Combustible	-15%	8500
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North 0 to 3 13 3 21-49 Type V NO	21%	5780
		East 10.1 to 20 28 3 81-100 Type V NO	14%	
		South 0 to 3 13 3 21-49 Type V NO	21%	
		West 10.1 to 20 28 2 41-60 Type V NO	12%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		14000
		Total Required Fire Flow in L/s		233.3
		Required Duration of Fire Flow (hrs)		3.00
		Required Volume of Fire Flow (m ³)		2520



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 80 Cope Drive (Block 104)

Date: 3/23/2023

Fire Flow Calculation #: 6

Description: 8 Unit, Back-to-Back Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		430 430 430	1290	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	12000
4	Determine Occupancy Charge	Limited Combustible	-15%	10200
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North > 30 25 0 0-20 Type V NO	0%	2346
		East 10.1 to 20 18 2 21-49 Type V NO	11%	
		South 10.1 to 20 25 2 41-60 Type V NO	12%	
		West > 30 18 2 21-49 Type V NO	0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		13000
		Total Required Fire Flow in L/s		216.7
		Required Duration of Fire Flow (hrs)		2.50
		Required Volume of Fire Flow (m ³)		1950



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401718

Project Name: Trails West: Cope Drive Units, 80 Cope Drive (Block 104)

Date: 3/23/2023

Fire Flow Calculation #: 7

Description: 8 Unit, Townhome, Wood Frame Townhouse Block

Notes: 3 Story, No fire separation

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type V - Wood Frame / Type IV-D - Mass Timber Construction	1.5	-
2	Determine Effective Floor Area	Sum of All Floor Areas	-	-
		450 450 450	1350	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	12000
4	Determine Occupancy Charge	Limited Combustible	-15%	10200
5	Determine Sprinkler Reduction	None	0%	0
		Non-Standard Water Supply or N/A	0%	
		Not Fully Supervised or N/A	0%	
		% Coverage of Sprinkler System	0%	
6	Determine Increase for Exposures (Max. 75%)	Direction Exposure Distance (m) Exposed Length (m) Exposed Height (Stories) Length-Height Factor (m x stories) Construction of Adjacent Wall Firewall / Sprinklered ?	-	-
		North 10.1 to 20 37 3 > 100 Type V NO	15%	2652
		East 10.1 to 20 13 2 21-49 Type V NO	11%	
		South > 30 37 0 0-20 Type V NO	0%	
		West > 30 13 2 21-49 Type V NO	0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min		13000
		Total Required Fire Flow in L/s		216.7
		Required Duration of Fire Flow (hrs)		2.50
		Required Volume of Fire Flow (m ³)		1950

A.3 City of Ottawa Water Boundary Conditions



Boundary Conditions
80, 110, 140 & 151 Cope Drive

Provided Information

80 Cope Drive

Scenario	Demand	
	L/min	L/s
Average Daily Demand	8	0.14
Maximum Daily Demand	22	0.36
Peak Hour	47	0.78
Fire Flow Demand #1	13,000	216.67

110 Cope Drive

Scenario	Demand	
	L/min	L/s
Average Daily Demand	4	0.07
Maximum Daily Demand	11	0.19
Peak Hour	25	0.41
Fire Flow Demand #1	10,000	166.67

140 Cope Drive

Scenario	Demand	
	L/min	L/s
Average Daily Demand	17	0.29
Maximum Daily Demand	43	0.72
Peak Hour	95	1.59
Fire Flow Demand #1	14,000	233.33

151 Cope Drive

Scenario	Demand	
	L/min	L/s
Average Daily Demand	22	0.36
Maximum Daily Demand	54	0.90
Peak Hour	119	1.98
Fire Flow Demand #1	14,000	233.33

Location



Results

Connection 1 (80 Cope Drive) – Carronbridge Circle

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	161.0	91.8
Peak Hour	156.4	85.3
Max Day plus Fire Flow	154.1	82.0

¹ Ground Elevation = 96.4 m

Connection 2 (110 Cope Drive) – Cope Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	161.0	92.0
Peak Hour	156.4	85.5
Max Day plus Fire Flow	154.6	82.8

¹ Ground Elevation = 96.3 m

Connection 3 (140 Cope Drive) – Templeford Avenue

Demand Scenario	Head (m)	Pressure ¹ (psi)
-----------------	----------	-----------------------------

Maximum HGL	160.9	90.8
Peak Hour	156.4	84.4
Max Day plus Fire Flow	144.3	67.1

¹ Ground Elevation = 97.1 m

Connection 4 (151 Cope Drive) – Northgraves Crescent

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	160.9	90.9
Peak Hour	156.4	84.5
Max Day plus Fire Flow	143.4	66.1

¹ Ground Elevation = 97.0 m

Notes

1. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
 - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

APPENDIX B



Appendix B Wastewater Servicing

B.1 2007 Subdivision Sanitary Design





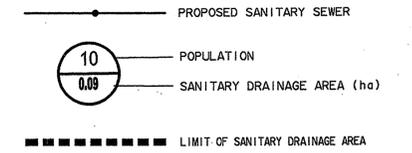
Stantec Consulting Ltd.
1505 Laperriere Avenue
Ottawa ON Canada
K1Z 7T1
Tel. 613.722.4420
Fax. 613.722.2799
www.stantec.com

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Legend



Notes

3	REVISED AS PER CITY COMMENTS AND FINAL APPROVAL	KJK	JBL	07.10.28
2	REVISED AS PER CITY COMMENTS	KJK	JBL	07.08.17
1	ISSUED FOR CITY COMMENTS	KJK	JBL	07.07.12
Revision		By	Appd.	YY.MM.DD

File Name: 160400502C-SAN
Dwn. Chkd. Degn. NI JBL KJK 07.03.14
YY.MM.DD

Permit-Seal



Client/Project

CAVANAGH CONSTRUCTION LTD.

SOHO - KANATA SOUTH

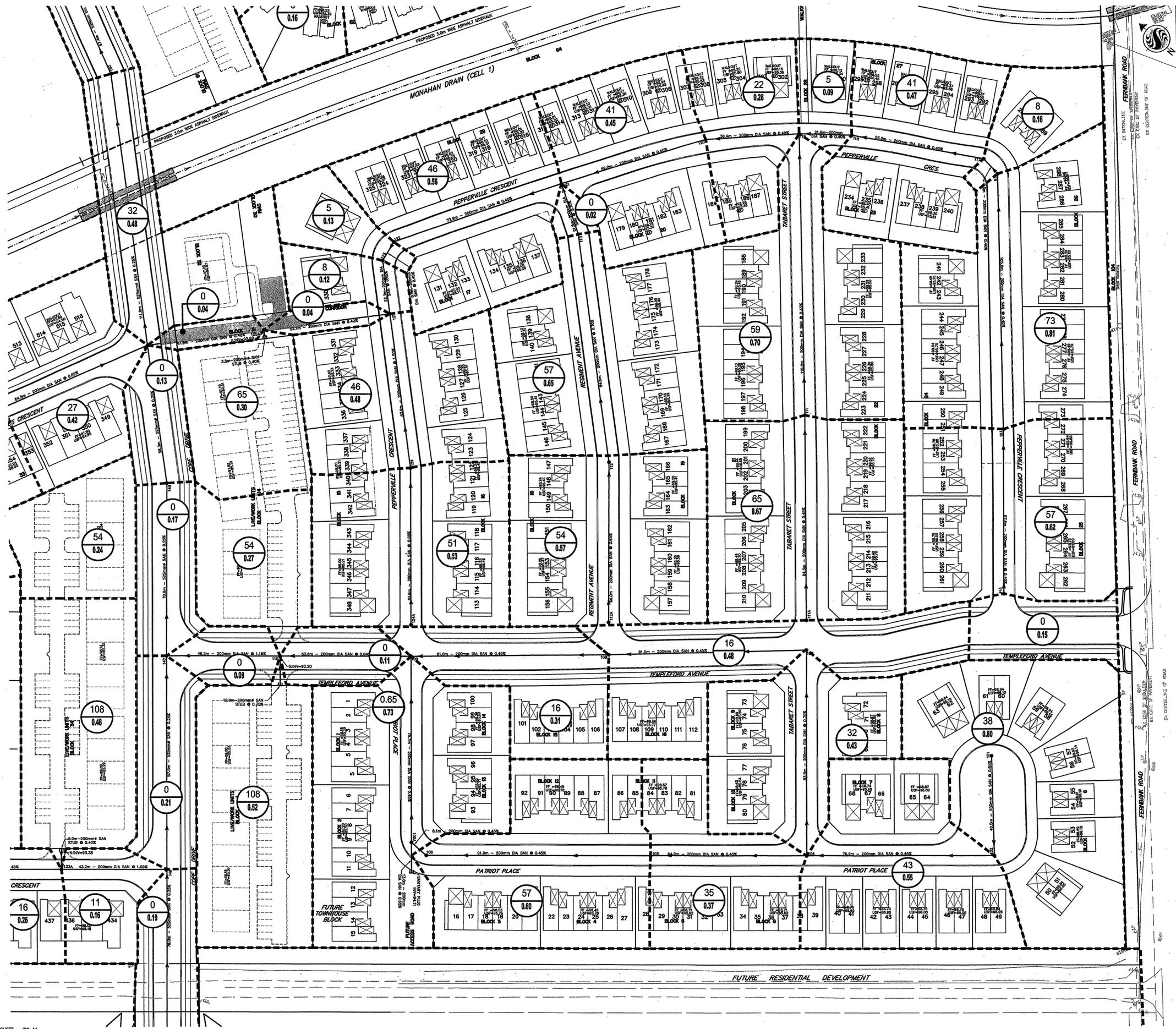
Ottawa ON Canada

Title

SANITARY DRAINAGE PLAN

Project No. 160400502C Scale 0 7.5 22.5 37.5m
1:750

Drawing No. SAN-2 Sheet 55 of 58 Revision 3



160400502C (SAN) Design (Issued) 160400502C - SAN.DWG
 160400502C (SAN) Design (Issued) 160400502C - SAN.DWG
 ORIGINAL SHEET - ISO A1



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Legend

- PROPOSED SANITARY SEWER
- POPULATION
- SANITARY DRAINAGE AREA (ha)
- LIMIT OF SANITARY DRAINAGE AREA

Notes

3	REVISED AS PER CITY COMMENTS AND FINAL APPROVAL	KJK	JBL	07.10.29	
2	REVISED AS PER CITY COMMENTS	KJK	JBL	07.08.17	
1	ISSUED FOR CITY COMMENTS	KJK	JBL	07.07.12	
Revision		By	Appd.	YY.MM.DD	
File Name:	160400502C-SAN	Ni	JBL	KJK	07.03.14
		Dwn.	Chkd.	Degn.	YY.MM.DD

Permit-Seal



Client/Project

CAVANAGH CONSTRUCTION LTD.

SOHO - KANATA SOUTH

Ottawa ON Canada

Title

SANITARY DRAINAGE PLAN

Project No.	Scale	0	7.5	22.5	37.5m
160400502C	1:750				
Drawing No.	Sheet	Revision			
SAN-3	56 of 58	3			



ORIGINAL SHEET - ISO A1

B.2 Sanitary Sewer Design Sheet



APPENDIX C



Appendix C Stormwater Management

C.1 2007 Subdivision Storm Design





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Legend

- DRAINAGE AREA NO.
RUNOFF COEFFICIENT
- STORM DRAINAGE AREA (ha)
- DRAINAGE AREA BOUNDARY
- PROPOSED STORM SEWER & MANHOLE
- PROPOSED CATCH BASIN (ALL ROAD CB'S TO INCLUDE PERFORATED STUB DRAINS EXTENDING OUT FROM THE CB IN TWO DIRECTIONS PARALLEL TO THE ROADWAY. THESE DRAINS ARE TO BE INSTALLED AT THE BOTTOM OF THE SUBBASE LAYER.)
- PROPOSED SUBDRAIN CATCH BASIN
- PROPOSED 250# PERFORATED PIPE
- STREET CATCHBASINS TO BE INTERCONNECTED WITH ONLY ONE CONNECTION TO STORM SEWER PER PAIR WHERE NOTED.
- IPEX TYPE 'A' TO BE INSTALLED IN STREET AND REAR YARD CATCHBASINS WHERE NOTED.
- PROPOSED CATCH BASIN / MANHOLE c/w IPEX INLET-CONTROL DEVICE TYPE 'A' OR APPROVED EQUIVALENT
- PONDING AREA LIMITS
- MAXIMUM PONDING DEPTH DEPTH=0.20m
- DIRECTION OF OVERLAND FLOW

Notes

1. IPEX TYPE 'A' TO RESTRICT FLOWS TO THE STORM SEWER TO 22L/s AT 1.8m HEAD.

3	REVISED AS PER CITY COMMENTS AND FINAL APPROVAL	KJK	JBL	07.10.29
2	REVISED AS PER CITY COMMENTS	KJK	JBL	07.08.17
1	ISSUED FOR CITY COMMENTS	KJK	JBL	07.07.12
Revision		By	Appd.	YY.MM.DD
File Name: 160400502C-SD		KJK	JBL	07.03.14
		Dwn.	Chkd.	Dagn.
				YY.MM.DD

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Ottawa ON Canada

Title

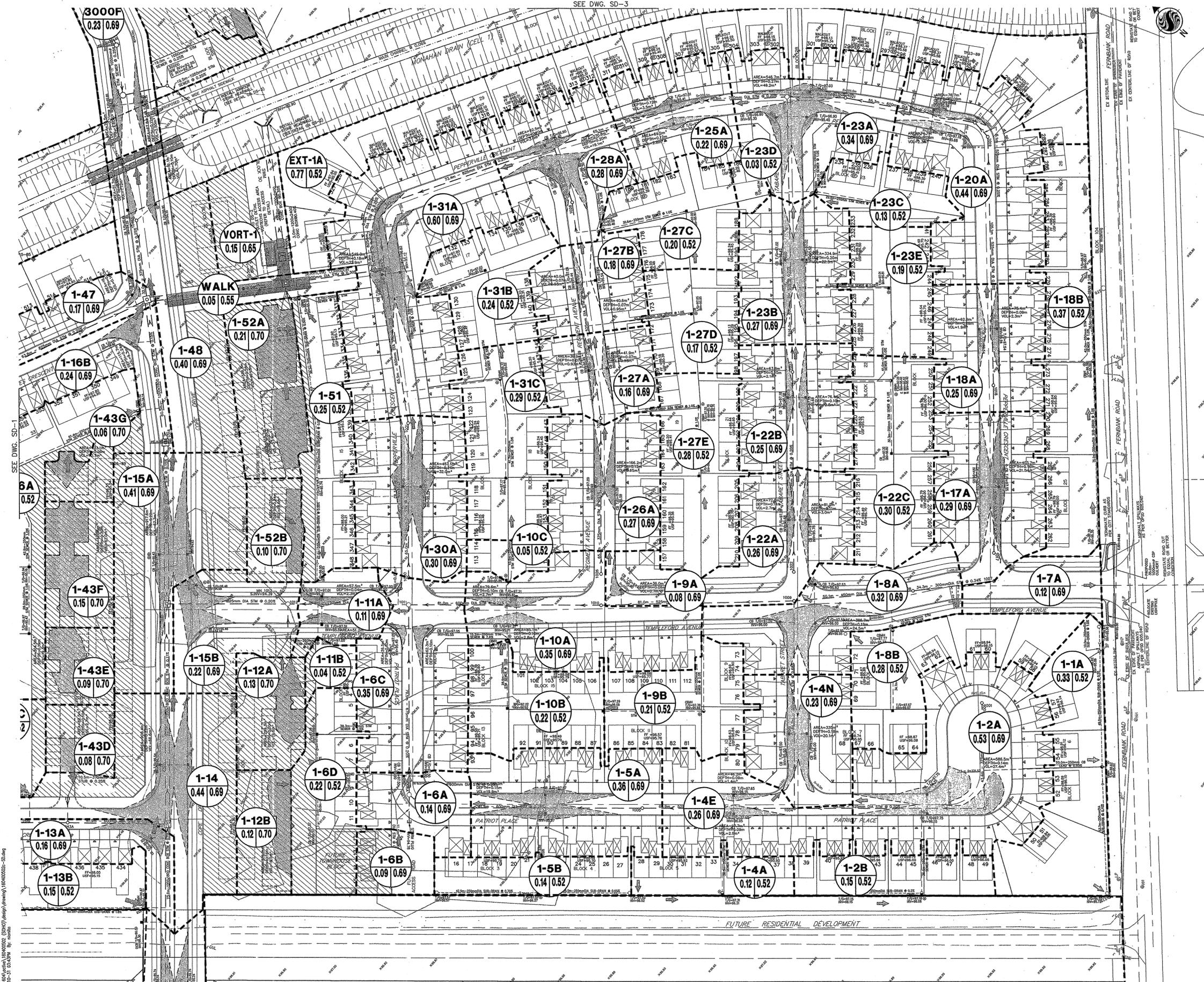
STORM DRAINAGE PLAN

Project No.	Scale	0	7.5	22.5	37.5m
160400502C	1:750				
Drawing No.	Sheet	Revision			

SD-2

51 of 58

3



V:\01-604\soho\160400502 (SD-01)\soho\storm.dwg
 2007-10-31 03:53PM By: nash



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Legend

- DRAINAGE AREA NO.
RUNOFF COEFFICIENT
- STORM DRAINAGE AREA (ha)
- DRAINAGE AREA BOUNDARY
- PROPOSED STORM SEWER & MANHOLE
- PROPOSED CATCH BASIN (ALL ROAD CB'S TO INCLUDE PERFORATED STUB DRAINS EXTENDING OUT FROM THE CB IN TWO DIRECTIONS PARALLEL TO THE ROADWAY. THESE DRAINS ARE TO BE INSTALLED AT THE BOTTOM OF THE SUBBASE LAYER.)
- PROPOSED SUBDRAIN CATCH BASIN
- PROPOSED 250mm PERFORATED PIPE
- STREET CATCHBASINS TO BE INTERCONNECTED WITH ONLY ONE CONNECTION TO STORM SEWER PER PAIR WHERE NOTED.
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- PONDING AREA LIMITS
- MAXIMUM PONDING DEPTH
- DIRECTION OF OVERLAND FLOW

Notes

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3	REVISED AS PER CITY COMMENTS AND FINAL APPROVAL	KJK	JBL	07.10.29
2	REVISED AS PER CITY COMMENTS	KJK	JBL	07.08.17
1	ISSUED FOR CITY COMMENTS	KJK	JBL	07.07.12
Revision		By Appd. YY.MM.DD		
File Name: 160400502C-SD		KJK	JBL	KJK
		Dwn.	Chkd.	Dagn.
				07.03.14
				YY.MM.DD

Permit-Seal



Client/Project
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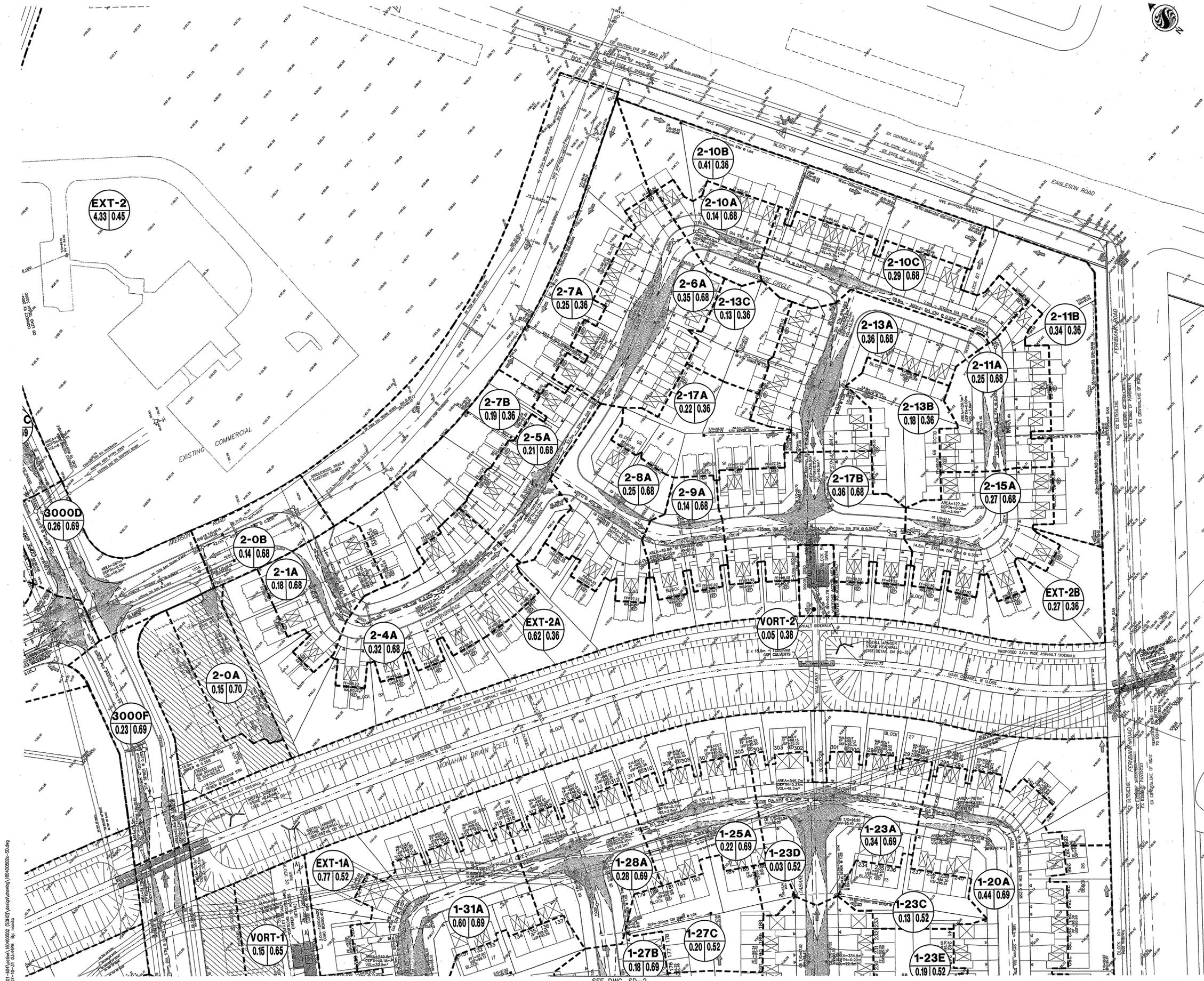
SOHO - KANATA SOUTH

Ottawa ON Canada

Title

STORM DRAINAGE PLAN

Project No.	Scale	0	7.5	22.5	37.5m
160400502C	1:750				
Drawing No.	Sheet	Revision			
SD-3	52 of 58	3			



2010 - 2011 (160400502C) (SD) Design (Drawing) 160400502C-SD.dwg
 2010-10-15 10:45:00 AM J. B. LACHANCE P. Eng.



Soho West Development
 DATE: March 6, 2007
 REVISION: October 25, 2007
 DESIGNED BY: NPC
 CHECKED BY: AMP

**STORM SEWER
 DESIGN SHEET**
 (City of Ottawa)
 Phase 1
 FILE NUMBER: 1604-00502

DESIGN PARAMETERS
 $I = a / (t+b)^c$
 (As per City of Ottawa Guidelines, 2004)

	1:5 yr	1:10 yr
a =	998.07	1174.184
b =	6.053	6.014
c =	0.814	0.816

MANNING'S n = 0.013
 MINIMUM COVER: 2.00 m
 TIME OF ENTRY: 15 min

LOCATION STREET / AREA ID	FROM M.H.	TO M.H.	DRAINAGE AREA							I (mm/h)	Q _{ACT} (L/s)	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	PIPE SELECTION					us			ds			d/s pipe Cover (m)				
			AREA (ha)	C (-)	ACCUM. AREA (ha)	A x C (ha)	ACCUM. A x C (ha)	T of C (min)	Q _{ACT} (L/s)						Conduit Type	Pipe size Box/Elliptical (height)	Box Width	Q _{CAP} (FULL) (L/s)	Q _{ACT} (FULL) (L/s)	VEL. (FULL) (m/s)	VEL. (ACT) (m/s)	TIME OF FLOW (min)	Ground Elevation (m)	us Obvert Elevation (m)	us Invert Elevation (m)		us pipe Cover (m)	Ground Elevation (m)	ds Obvert Elevation (m)	ds Invert Elevation (m)
COPE (1-48)	1048A	1048	0.40	0.69	0.40	0.28	0.28	15.00	83.56	64.1	32.0	375	0.25				91.5	0.70	0.80	0.87	0.61	96.15	94.52	94.14	1.63	96.50	94.44	94.06	2.06	
WALKWAY FROM COPE (walkway)	1048	1052	0.05	0.55	25.64	0.03	16.24	23.57	63.28	2854.2	52.8	1500	0.20	BOX	1200	1800	3298.1	0.87	1.81	2.05	0.43	96.50	94.44	93.24	2.06	97.04	94.33	93.13	2.71	
PEPPERVILLE (1-30A)	1030	1031	0.30	0.69	0.30	0.21	0.21	15.00	83.56	48.0	65.0	375	0.25				91.5	0.53	0.80	0.81	1.34	97.16	94.77	94.39	2.39	97.04	94.61	94.23	2.43	
PEPPERVILLE (1-31A, 1-31B, 1-31C)	1031	1051	1.18	0.61	1.48	0.72	0.92	16.34	79.47	203.7	65.5	600	0.20				286.5	0.71	0.98	1.07	1.02	97.04	94.61	94.01	2.43	96.75	94.48	93.88	2.27	
PEPPERVILLE (1-17A)	1017	1018	0.29	0.69	0.29	0.20	0.20	15.00	83.56	46.4	70.8	450	0.30				162.9	0.29	0.99	0.84	1.40	97.86	95.59	95.14	2.27	97.78	95.38	94.93	2.40	
PEPPERVILLE (1-18A, 1-18B)	1018	1019	0.62	0.59	0.91	0.36	0.57	16.40	79.29	124.4	105.1	525	0.25				224.3	0.55	1.00	1.02	1.71	97.78	95.38	94.85	2.40	97.44	95.12	94.59	2.32	
PEPPERVILLE	1019	1020			0.91	0.00	0.57	18.11	74.69	117.2	8.1	525	0.25				224.3	0.52	1.00	1.01	0.13	97.44	95.12	94.59	2.32	97.40	95.09	94.57	2.31	
PEPPERVILLE (1-20A)	1020	1021	0.44	0.69	1.35	0.30	0.87	18.24	74.36	179.4	55.3	600	0.20				286.5	0.63	0.98	1.04	0.89	97.40	95.09	94.49	2.31	97.25	94.98	94.38	2.27	
PEPPERVILLE	1021	1024			1.35	0.00	0.87	19.13	72.22	174.3	24.6	600	0.20				286.5	0.61	0.98	1.03	0.40	97.25	94.98	94.38	2.27	97.19	94.93	94.33	2.26	
TABARET (1-22A, 1-22B, 1-22C)	1022	1023	0.81	0.63	0.81	0.51	0.51	15.00	83.56	117.9	84.4	450	0.20				133.0	0.89	0.81	0.92	1.52	97.53	95.34	94.89	2.19	97.34	95.17	94.72	2.17	
TABARET (1-23A, 1-23B, 1-23C, 1-23D, 1-23E)	1023	1024	0.96	0.62	1.77	0.59	1.10	16.52	78.94	241.3	116.0	600	0.20				286.5	0.84	0.98	1.11	1.74	97.34	95.17	94.57	2.17	97.19	94.93	94.33	2.26	
PEPPERVILLE	1024	1025			3.12	0.00	1.97	19.53	71.31	390.0	35.8	750	0.20				519.4	0.75	1.14	1.26	0.47	97.19	94.93	94.18	2.26	97.17	94.86	94.11	2.31	
PEPPERVILLE (1-25A)	1025	1029	0.22	0.69	3.34	0.15	2.12	20.00	70.25	413.8	65.1	825	0.20				669.7	0.62	1.21	1.27	0.85	97.17	94.86	94.04	2.31	96.95	94.73	93.91	2.22	
REGIMENT (1-26A)	1026	1027	0.27	0.69	0.27	0.19	0.19	15.00	83.56	43.2	65.7	375	0.25				91.5	0.47	0.80	0.79	1.39	97.55	95.13	94.76	2.42	97.31	94.97	94.59	2.34	
REGIMENT (1-27A, 1-27B, 1-27C, 1-27D, 1-27E)	1027	1028	0.99	0.58	1.26	0.58	0.76	16.39	79.31	168.3	91.5	525	0.20				200.7	0.84	0.90	1.01	1.51	97.31	94.97	94.44	2.34	97.09	94.78	94.26	2.31	
REGIMENT (1-28A)	1028	1029	0.28	0.69	1.54	0.19	0.96	17.90	75.22	200.0	25.4	600	0.20				286.5	0.70	0.98	1.06	0.40	97.09	94.78	94.18	2.31	96.95	94.73	94.13	2.22	
PEPPERVILLE	1029	1049			4.88	0.00	3.08	20.85	68.43	585.1	73.4	825	0.25				748.8	0.78	1.36	1.51	0.81	96.95	94.73	93.91	2.22	96.86	94.55	93.72	2.31	
PEPPERVILLE	1049	1050			4.88	0.00	3.08	21.66	66.81	571.2	8.5	825	0.25				748.8	0.76	1.36	1.51	0.09	96.86	94.55	93.72	2.31	96.83	94.53	93.70	2.30	
PEPPERVILLE	1050	1051			4.88	0.00	3.08	21.75	66.62	569.6	20.9	825	0.25				748.8	0.76	1.36	1.51	0.23	96.83	94.53	93.70	2.30	96.75	94.48	93.65	2.27	
WALKWAY (1-51)	1051	1052	0.25	0.52	6.61	0.13	4.13	21.99	66.17	759.3	47.7	900	0.30				1034.5	0.73	1.58	1.73	0.46	96.75	94.48	93.58	2.27	97.04	94.33	93.43	2.71	
Parking Lot (1-52B)	1052B	1052A	0.10	0.70	0.10	0.07	0.07	15.00	83.56	16.2	50.0	375	0.35				108.2	0.15	0.95	0.65	1.27	97.34	94.75	94.38	2.59	97.15	94.58	94.20	2.57	
Parking Lot (1-52A)	1052A	1052	0.21	0.70	0.31	0.15	0.22	16.27	79.66	48.0	69.5	375	0.35				108.2	0.44	0.95	0.92	1.26	97.15	94.58	94.20	2.57	97.04	94.33	93.96	2.71	
ACCESS ROAD	1052	1053			32.56	0.00	20.59	24.00	62.54	3576.4	10.3	1500	0.35	Box	1200	2400	4363.0	0.82	2.39	2.68	0.06	97.04	94.33	93.13	2.71	96.65	94.30	93.10	2.35	
ACCESS ROAD	1053	1053A			32.56	0.00	20.59	24.06	62.43	3570.2	3.0	1500	0.40	Box	1200	2400	4664.2	0.77	2.56	2.84	0.02	96.65	94.29	93.09	2.36	97.04	94.28	93.08	2.77	
ACCESS ROAD	1053A	outlet			32.56	0.00	20.59	24.13	62.33	3564.0	50.0	1500	0.35	Box	1200	2400	4363.0	0.82	2.39	2.68	0.31	97.04	94.28	93.08	2.77	97.04	94.10	92.90	2.94	
* Future Van Gahl Lands	12.39	ha																												
Boundary Condition:																														
invert at drain	92.9	Check	32.56		32.56	20.59	20.59																							

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hydraulic grade line (HGL) in the subdivision. The 100 year HGL elevation in Cell 1 of the Monahan Drain was obtained through conversations with Novatech Engineering staff (see **Appendix F** for correspondence). Previous reports for the Monahan Drain Constructed Wetland estimated the 100 year water level in Cell 1 to be approximately 94.94 m. However, a set of twin culverts has been proposed to cross Fernbank Road, as well as Cope Drive, which will decrease the water level in Cell 1 to 94.55 m. In addition, the latest revision to the Monahan Drain Constructed Wetlands EPA SWMM model by Novatech Engineering resulted in a lower 100 year water elevation in Cell 1 equal to 94.38 m. **Appendix C** presents the proposed storm sewer design sheets.

The detailed DDSWMM hydrology was interfaced with the XP-SWMM models to determine the resulting HGL in the subdivision. The configuration and number of ICDs were iterated between DDSWMM and XP-SWMM to meet the HGL requirements for the 100 year storm. **Table 4.3** and **Table 4.4** summarize the HGL modeling results.

Table 4.3: Phase 1 - 100 Year Hydraulic Grade Line Results

Node (CB/MH)	Ground Elevation (m)	Lowest Underside of Footing (m)	Worst-case HGL (m)	Separation (m)
1001	98.17	96.05	95.45	0.60
1002	97.85	95.88	95.29	0.59
1004E	97.91	95.78	95.18	0.60
1004N	97.91	95.84	95.53	0.31
1005	97.89	95.65	94.98	0.67
1006	97.70	95.39	94.76	0.63
1007	98.02	N/A	95.16	-
1008	97.95	N/A	95.11	-
1009	97.74	95.84	94.99	0.85
1010	97.67	N/A	94.78	-
1011	97.35	95.39	94.61	0.78
1012	97.02	95.23	94.61	0.62
1012A	97.49	95.23	94.87	0.37
1012B	97.48	95.36	94.96	0.40
1013	97.99	N/A	94.63	-
1014	96.82	95.15	94.62	0.53
1015	97.49	N/A	94.61	-
1016	96.70	N/A	94.58	-
1017	97.86	95.85	95.26	0.59
1018	97.78	95.60	95.05	0.55
1019	97.44	95.67	94.78	0.89
1020	97.40	95.41	94.76	0.65
1021	97.25	95.34	94.71	0.63
1022	97.53	95.65	95.12	0.53
1023	97.34	95.29	94.92	0.37
1024	97.19	95.34	94.70	0.64
1025	97.17	95.22	94.67	0.55
1026	97.55	95.41	94.89	0.52
1027	97.31	95.31	94.78	0.53
1028	97.09	95.31	94.67	0.64
1029	96.95	95.22	94.65	0.57
1030	98.00	95.15	94.59	0.56
1031	98.00	95.07	94.57	0.50

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Node (CB/MH)	Ground Elevation (m)	Lowest Underside of Footing (m)	Worst-case HGL (m)	Separation (m)
1033N	97.50	95.73	95.23	0.50
1033S	97.50	95.90	95.32	0.58
1034	97.43	95.80	95.32	0.48
1035E	97.20	95.55	95.19	0.36
1035N	97.20	95.45	95.03	0.42
1036	97.34	95.30	94.89	0.41
1037	97.50	95.73	95.13	0.60
1038	97.33	95.47	95.13	0.34
1039	97.26	95.26	94.80	0.47
1040	97.15	95.35	94.89	0.46
1041	97.14	95.28	94.72	0.56
1042	97.19	95.14	94.64	0.50
1043	97.07	95.23	94.83	0.40
1043A	96.90	95.15	94.68	0.47
1043D	97.03	95.29	94.74	0.55
1043E	97.00	95.29	94.92	0.37
1043F	96.86	95.35	95.05	0.30
1044	97.02	95.16	94.78	0.38
1045	97.00	95.09	94.62	0.47
1046	96.70	94.95	94.61	0.34
1047	96.63	94.90	94.58	0.32
1048	96.50	94.87	94.55	0.32
1048A	96.15	94.87	94.57	0.30
1049	96.86	95.05	94.59	0.47
1050	96.83	95.02	94.58	0.44
1051	96.75	95.02	94.56	0.46
1052	97.04	94.90	94.50	0.40
1052A	97.15	95.10	94.54	0.56
1052B	97.34	95.21	94.55	0.66
1053	97.22	N/A	95.05	-
1053B	97.20	N/A	94.41	-
CB-1043G	96.92	95.54	95.24	0.30
DIVERSION	95.65	N/A	94.50	-
Monahan	95.05	N/A	94.38	-

Table 4.4: Phase 2 - 100 Year Hydraulic Grade Line Results

Node (CB/MH)	Ground Elevation (m)	Lowest Underside of Footing (m)	Worst-case HGL (m)	Separation (m)
2000	97.01	95.42	94.78	0.64
2000A	96.77	95.42	94.99	0.44
2001	97.69	95.39	94.77	0.63
2002	97.62	95.39	94.73	0.67
2003	97.62	95.21	94.72	0.49
2004	97.44	95.21	94.69	0.52
2005	97.40	95.33	94.65	0.68
2006E	97.50	95.36	94.73	0.63
2006W	97.50	95.32	94.73	0.59
2007	97.35	95.22	94.70	0.52
2008	97.29	95.20	94.62	0.58
2009	97.10	95.12	94.54	0.58

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Node (CB/MH)	Ground Elevation (m)	Lowest Underside of Footing (m)	Worst-case HGL (m)	Separation (m)
2010	97.50	95.36	94.73	0.63
2011N	97.50	95.58	94.64	0.94
2011S	97.50	95.39	94.70	0.69
2012	97.55	95.58	94.64	0.94
2013	97.29	95.26	94.63	0.63
2014	97.38	95.25	94.55	0.70
2015	97.30	95.12	94.54	0.58
2016	97.30	95.08	94.51	0.58
2017	96.90	95.10	94.56	0.54
2018	96.90	95.00	94.47	0.53
2019	97.05	95.00	94.50	0.50
2020	96.90	N/A	94.41	-
2020B	96.90	N/A	94.41	-
BYPASS	96.85	N/A	94.45	-
Monahan2	96.85	N/A	94.38	-

As is demonstrated in **Tables 4.3 and 4.4**, the worst-case scenario results in HGL elevations that remain at least 0.30 m below the proposed underside of footings (USFs) in all locations. **Appendix B** summarizes the results of the hydraulic modeling and output files for the subject site area.

Inflow Time Series - Existing Subdivision Design, Phase 1 October 2007
 Cavanagh - Cope Drive Site Plans
 Block 46 and Block 24

Data taken from

Path: \\ca0218-ppfss01\work_group\01-604\active\160400502 (SOHO)\design\analysis\SWM\Subm2(October2007)\DDSWM\PH1
 File: DDPH1_100-3.CSV

1001	1002	1004/1004E		1005	1006		1008	1009	
Time	Flow	Time	Flow	Time	Flow	Time	Flow	Time	Flow
0:00	0	0:00	0	0:00	0	0:00	0	0:00	0
0:10	0	0:10	0	0:10	0	0:10	0	0:10	0
0:20	0	0:20	0.001	0:20	0.002	0:20	0.004	0:20	0.001
0:30	0.001	0:30	0.005	0:30	0.006	0:30	0.015	0:30	0.006
0:40	0.002	0:40	0.011	0:40	0.014	0:40	0.034	0:40	0.011
0:50	0.017	0:50	0.022	0:50	0.033	0:50	0.095	0:50	0.044
1:00	0.022	1:00	0.044	1:00	0.044	1:00	0.11	1:00	0.066
1:10	0.022	1:10	0.044	1:10	0.042	1:10	0.044	1:10	0.066
1:20	0.022	1:20	0.039	1:20	0.024	1:20	0.063	1:20	0.065
1:30	0.022	1:30	0.033	1:30	0.016	1:30	0.026	1:30	0.053
1:40	0.012	1:40	0.025	1:40	0.01	1:40	0.019	1:40	0.027
1:50	0.008	1:50	0.022	1:50	0.008	1:50	0.015	1:50	0.018
2:00	0.005	2:00	0.017	2:00	0.006	2:00	0.011	2:00	0.011
2:10	0.004	2:10	0.014	2:10	0.005	2:10	0.009	2:10	0.009
2:20	0.003	2:20	0.012	2:20	0.005	2:20	0.008	2:20	0.007
2:30	0.003	2:30	0.011	2:30	0.004	2:30	0.007	2:30	0.007
2:40	0.002	2:40	0.009	2:40	0.004	2:40	0.006	2:40	0.006
2:50	0.002	2:50	0.008	2:50	0.004	2:50	0.005	2:50	0.005
3:00	0.002	3:00	0.008	3:00	0.003	3:00	0.007	3:00	0.005
3:10	0.002	3:10	0.007	3:10	0.003	3:10	0.004	3:10	0.004
3:20	0.002	3:20	0.005	3:20	0.002	3:20	0.004	3:20	0.003
3:30	0.001	3:30	0.005	3:30	0.001	3:30	0.003	3:30	0.003
3:40	0.001	3:40	0.004	3:40	0.001	3:40	0.002	3:40	0.002
3:50	0.001	3:50	0.003	3:50	0.001	3:50	0.002	3:50	0.002
4:00	0.001	4:00	0.003	4:00	0.001	4:00	0.002	4:00	0.002
4:10	0.001	4:10	0.002	4:10	0.001	4:10	0.001	4:10	0.001
4:20	0.001	4:20	0.002	4:20	0.001	4:20	0.001	4:20	0.001
4:30	0.001	4:30	0.002	4:30	0.001	4:30	0.001	4:30	0.001
4:40	0.001	4:40	0.002	4:40	0.001	4:40	0.001	4:40	0.001
4:50	0.001	4:50	0.001	4:50	0.001	4:50	0.001	4:50	0.001
5:00	0	5:00	0.001	5:00	0.001	5:00	0.001	5:00	0.001
5:10	0	5:10	0.001	5:10	0	5:10	0.001	5:10	0.001
5:20	0	5:20	0.001	5:20	0	5:20	0.001	5:20	0.001
5:30	0	5:30	0.001	5:30	0	5:30	0.001	5:30	0.001
5:40	0	5:40	0.001	5:40	0	5:40	0.001	5:40	0.001
5:50	0	5:50	0.001	5:50	0	5:50	0.001	5:50	0.001
6:00	0	6:00	0.001	6:00	0	6:00	0.001	6:00	0.001
6:10	0	6:10	0.001	6:10	0	6:10	0.001	6:10	0
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7:00	0	7:00	0.001	7:00	0	7:00	0.001	7:00	0
7:10	0	7:10	0.001	7:10	0	7:10	0.001	7:10	0
7:20	0	7:20	0.001	7:20	0	7:20	0.001	7:20	0
7:30	0	7:30	0.001	7:30	0	7:30	0.001	7:30	0
7:40	0	7:40	0.001	7:40	0	7:40	0.001	7:40	0
7:50	0	7:50	0.001	7:50	0	7:50	0	7:50	0
8:00	0	8:00	0.001	8:00	0	8:00	0	8:00	0
8:10	0	8:10	0	8:10	0	8:10	0	8:10	0
8:20	0	8:20	0	8:20	0	8:20	0	8:20	0
8:30	0	8:30	0	8:30	0	8:30	0	8:30	0
8:40	0	8:40	0	8:40	0	8:40	0	8:40	0
8:50	0	8:50	0	8:50	0	8:50	0	8:50	0
9:00	0	9:00	0	9:00	0	9:00	0	9:00	0
9:10	0	9:10	0	9:10	0	9:10	0	9:10	0
9:20	0	9:20	0	9:20	0	9:20	0	9:20	0
9:30	0	9:30	0	9:30	0	9:30	0	9:30	0
9:40	0	9:40	0	9:40	0	9:40	0	9:40	0
9:50	0	9:50	0	9:50	0	9:50	0	9:50	0
10:00	0	10:00	0	10:00	0	10:00	0	10:00	0
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10:30	0	10:30	0	10:30	0	10:30	0	10:30	0
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12:20	0	12:20	0	12:20	0	12:20	0	12:20	0
12:30	0	12:30	0	12:30	0	12:30	0	12:30	0
12:40	0	12:40	0	12:40	0	12:40	0	12:40	0
12:50	0	12:50	0	12:50	0	12:50	0	12:50	0
13:00	0	13:00	0	13:00	0	13:00	0	13:00	0
13:10	0	13:10	0	13:10	0	13:10	0	13:10	0

Inflow Time Series - Existing Subdivision Design, Phase 1 October 2007
 Cavanagh - Cope Drive Site Plans
 Block 46 and Block 24

Data taken from

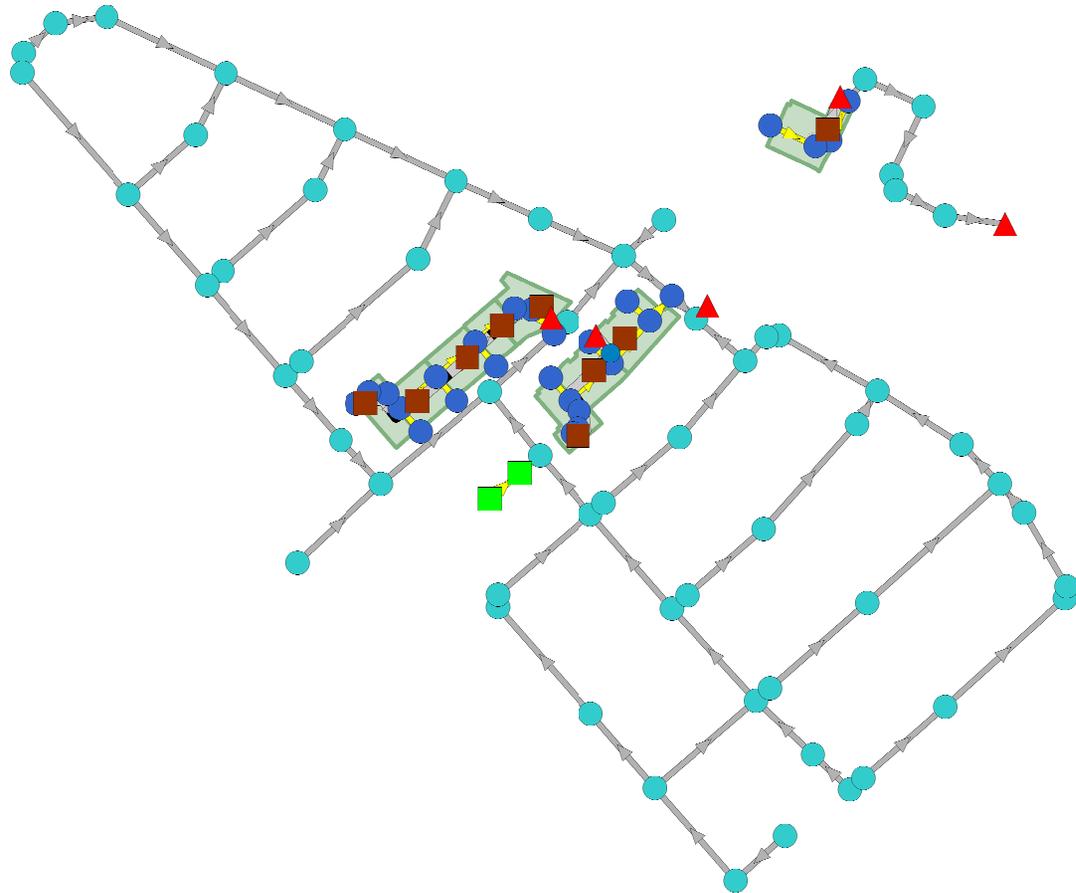
Path: \\ca0218-ppfss01\work_group\01-604\active\160400502 (SOHO)\design\analysis\SWM\Subm2(October2007)\DDSWMM\PH2

File: DDPH1_100-3.CSV

2000A		2000		2001		2004	
Time	Flow	Time	Flow	Time	Flow	Time	Flow
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0:10	0	0:10	0	0:10	0	0:10	0
0:20	0	0:20	0.001	0:20	0.001	0:20	0.002
0:30	0.001	0:30	0.003	0:30	0.003	0:30	0.007
0:40	0.003	0:40	0.008	0:40	0.009	0:40	0.017
0:50	0.044	0:50	0.022	0:50	0.022	0:50	0.044
1:00	0.041	1:00	0.022	1:00	0.022	1:00	0.044
1:10	0.024	1:10	0.011	1:10	0.016	1:10	0.033
1:20	0.009	1:20	0.008	1:20	0.01	1:20	0.019
1:30	0.007	1:30	0.005	1:30	0.008	1:30	0.011
1:40	0.006	1:40	0.004	1:40	0.005	1:40	0.008
1:50	0.004	1:50	0.003	1:50	0.004	1:50	0.007
2:00	0.004	2:00	0.002	2:00	0.003	2:00	0.005
2:10	0.003	2:10	0.002	2:10	0.003	2:10	0.005
2:20	0.003	2:20	0.002	2:20	0.003	2:20	0.004
2:30	0.003	2:30	0.002	2:30	0.002	2:30	0.004
2:40	0.002	2:40	0.002	2:40	0.002	2:40	0.004
2:50	0.002	2:50	0.001	2:50	0.002	2:50	0.003
3:00	0.002	3:00	0.001	3:00	0.002	3:00	0.003
3:10	0.002	3:10	0.001	3:10	0.001	3:10	0.002
3:20	0.002	3:20	0.001	3:20	0.001	3:20	0.002
3:30	0.002	3:30	0.001	3:30	0.001	3:30	0.001
3:40	0.001	3:40	0.001	3:40	0.001	3:40	0.001
3:50	0.001	3:50	0.001	3:50	0.001	3:50	0.001
4:00	0.001	4:00	0.001	4:00	0.001	4:00	0.001
4:10	0.001	4:10	0	4:10	0	4:10	0.001
4:20	0.001	4:20	0	4:20	0	4:20	0.001
4:30	0.001	4:30	0	4:30	0	4:30	0.001
4:40	0.001	4:40	0	4:40	0	4:40	0.001
4:50	0.001	4:50	0	4:50	0	4:50	0.001
5:00	0.001	5:00	0	5:00	0	5:00	0.001
5:10	0.001	5:10	0	5:10	0	5:10	0.001
5:20	0.001	5:20	0	5:20	0	5:20	0
5:30	0.001	5:30	0	5:30	0	5:30	0
5:40	0.001	5:40	0	5:40	0	5:40	0
5:50	0.001	5:50	0	5:50	0	5:50	0
6:00	0.001	6:00	0	6:00	0	6:00	0
6:10	0.001	6:10	0	6:10	0	6:10	0
6:20	0.001	6:20	0	6:20	0	6:20	0
6:30	0	6:30	0	6:30	0	6:30	0
6:40	0	6:40	0	6:40	0	6:40	0
6:50	0	6:50	0	6:50	0	6:50	0
7:00	0	7:00	0	7:00	0	7:00	0

C.2 PCSWMM Data





Legend

Junctions

- Visible
- Visible
- Visible

- ▲ Outfalls

Storages

- Visible
- Visible
- Visible
- Existing MH

Conduits

- Visible
- Major System
- Existing Minor
- Orifices

Subcatchments

- Visible
- Visible



150 m

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

[TITLE]
 ;;Project Title/Notes
 Cope Drive Units
 Option 1: Increase allowable minor system discharge
 HGL-2023-100yr: Subdivision HGL with current site design, 100-year 3-hour design storm
 v0: No conduit exit loss
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[OPTIONS]
 ;;Option Value
 FLOW_UNITS CMS
 INFILTRATION HORTON
 FLOW_ROUTING DYNWAVE
 LINK_OFFSETS ELEVATION
 MIN_SLOPE 0
 ALLOW_PONDING NO
 SKIP_STEADY_STATE NO

START_DATE 02/10/2023
 START_TIME 00:00:00
 REPORT_START_DATE 02/10/2023
 REPORT_START_TIME 00:00:00
 END_DATE 02/11/2023
 END_TIME 00:00:00
 SWEEP_START 01/01
 SWEEP_END 12/31
 DRY_DAYS 0
 REPORT_STEP 00:01:00
 WET_STEP 00:05:00
 DRY_STEP 00:05:00
 ROUTING_STEP 5
 RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
 NORMAL_FLOW_LIMITED BOTH
 FORCE_MAIN_EQUATION H-W
 VARIABLE_STEP 0
 LENGTHENING_STEP 0
 MIN_SURFAREA 0
 MAX_TRIALS 8
 HEAD_TOLERANCE 0.0015
 SYS_FLOW_TOL 5
 LAT_FLOW_TOL 5
 MINIMUM_STEP 0.5
 THREADS 4

[EVAPORATION]
 ;;Data Source Parameters
 ;;-----
 CONSTANT 0.0
 DRY_ONLY NO

[RAINGAGES]
 ;;Name Format Interval SCF Source
 ;;-----
 August4_1988 INTENSITY 0:05 1.0 TIMESERIES
 August4_1988
 August8_1996 INTENSITY 0:05 1.0 TIMESERIES
 August8_1996
 Chicago_100yr_3h INTENSITY 0:10 1.0 TIMESERIES
 Chicago_100yr_3h
 Chicago_100yr+20%_3h INTENSITY 0:10 1.0 TIMESERIES
 Chicago_100yr+20%_3h
 Chicago_2yr_3h INTENSITY 0:10 1.0 TIMESERIES
 Chicago_2yr_3h
 Chicago_5yr_3h INTENSITY 0:10 1.0 TIMESERIES
 Chicago_5yr_3h
 July1_1979 INTENSITY 1:00 1.0 TIMESERIES
 July1_1979
 SCS_Type_II_100yr_103.2mm INTENSITY 1:00 1.0 TIMESERIES
 SCS_Type_II_100yr_103.2mm

[SUBCATCHMENTS]
 ;;Name Rain Gage Outlet Area
 %Imperv Width %Slope CurbLen SnowPack
 ;;-----
 ;;BLK-104
 C102A Chicago_100yr_3h CB-102 0.141479 80
 78 2 0
 ;;BLK-24
 C304A Chicago_100yr_3h CB-304 0.153133 87.14
 70 2 0
 ;;BLK-24
 C306A Chicago_100yr_3h CB-306 0.118501 87.14
 70 2 0
 ;;BLK-24

C310A Chicago_100yr_3h CB-310 0.092817 67.14
 86 2 0
 ;;BLK-46
 C502A Chicago_100yr_3h CB-502 0.115174 54.29
 82 2 0
 ;;BLK-46
 C504A Chicago_100yr_3h CB-504 0.072639 85.71
 64 2 0
 ;;BLK-46
 C506A Chicago_100yr_3h CB-506 0.108034 88.57
 64 2 0
 ;;BLK-46
 C508A Chicago_100yr_3h CB-508 0.104224 87.14
 64 2 0
 ;;BLK-46
 C512A Chicago_100yr_3h CB-512 0.089376 68.57
 85 2 0

[SUBAREAS]
 ;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv
 PctZero RouteTo PctRouted
 ;;-----
 C102A 0.013 0.25 1.57 4.67 0
 OUTLET
 C304A 0.013 0.25 1.57 4.67 0
 OUTLET
 C306A 0.013 0.25 1.57 4.67 0
 OUTLET
 C310A 0.013 0.25 1.57 4.67 0
 OUTLET
 C502A 0.013 0.25 1.57 4.67 0
 OUTLET
 C504A 0.013 0.25 1.57 4.67 0
 OUTLET
 C506A 0.013 0.25 1.57 4.67 0
 OUTLET
 C508A 0.013 0.25 1.57 4.67 0
 OUTLET
 C512A 0.013 0.25 1.57 4.67 0
 OUTLET

[INFILTRATION]
 ;;Subcatchment Param1 Param2 Param3 Param4
 Param5
 ;;-----
 C102A 76.2 13.2 4.14 7 0
 C304A 76.2 13.2 4.14 7 0
 C306A 76.2 13.2 4.14 7 0
 C310A 76.2 13.2 4.14 7 0
 C502A 76.2 13.2 4.14 7 0
 C504A 76.2 13.2 4.14 7 0
 C506A 76.2 13.2 4.14 7 0
 C508A 76.2 13.2 4.14 7 0
 C512A 76.2 13.2 4.14 7 0

[JUNCTIONS]
 ;;Name Elevation MaxDepth InitDepth SurDepth
 Aponed
 ;;-----
 J1 96.6 0 0 0 0

[OUTFALLS]
 ;;Name Elevation Type Stage Data Gated
 Route To
 ;;-----
 2005 93.73 FIXED 94.65 YES
 Diversion-1053 93.09 FIXED 94.48 YES
 OF1 96.6 FREE NO
 OF2 96.5 FREE NO
 OF3 96 FREE NO

[STORAGE]
 ;;Name Elev. MaxDepth InitDepth Shape Curve
 Name/Params N/A Fevap Psi Ksat IMD
 ;;-----
 1001 95.32 2.93 0 FUNCTIONAL 0
 0 1.13 0 0
 1002 95.07 2.96 0 FUNCTIONAL 0
 0 1.13 0 0
 1004 94.91 2.99 0 FUNCTIONAL 0
 0 1.13 0 0
 1005 94.67 3.23 0 FUNCTIONAL 0
 0 1.13 0 0
 1006 94.44 3.31 0 FUNCTIONAL 0
 0 1.13 0 0
 1006A 94.34 3.36 0 FUNCTIONAL 0
 0 1.13 0 0
 1007 95.16 2.84 0 FUNCTIONAL 0
 0 1.13 0 0
 1008 94.89 3.06 0 FUNCTIONAL 0
 0 1.13 0 0
 1009 94.69 3.06 0 FUNCTIONAL 0
 0 1.13 0 0
 1010 94.44 3.21 0 FUNCTIONAL 0
 0 1.13 0 0
 1011 94.11 3.29 0 FUNCTIONAL 0
 0 1.13 0 0
 ;;0.7
 1012 93.91 3.31 0 FUNCTIONAL 0
 0 1.13 0 0
 1012A 94.67 2.82 0 FUNCTIONAL 0
 0 1.13 0 0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

				L110	1033	1034	12.928
				0.013	95.31	95.26	0
				L111	1034	1035	105.17
				0.013	95.11	94.8	0
				L112	1035	1036	58.34
				0.013	94.8	94.68	0
				L113	1036	1039	44.883
				0.013	94.6	94.53	0
				L114	1039	1042	85.724
				0.013	94.46	94.24	0
				L115	1042	1046	79.822
				0.013	94.09	93.93	0
				L116	1046	1047	60.021
				0.013	93.86	93.74	0
				L117	1047	1048	59.797
				0.013	93.74	93.61	0
				L118	1033	1037	28.067
				0.013	95.23	95.13	0
				L119	1037	1038	33.831
				0.013	95.13	95.02	0
				L120	1038	1039	85.977
				0.013	94.94	94.68	0
				L121	1035	1053	78.006
				0.013	94.99	94.79	0
				L122	1053	1043	78.006
				0.013	94.72	94.52	0
				L123	1043	1043A	55.446
				0.013	94.44	94.31	0
				L124	1043A	1014	38.784
				0.013	94.23	94.13	0
				L125	1040	1041	79.903
				0.013	94.76	94.56	0
				L126	1041	1042	43.951
				0.013	94.48	94.39	0
				L127	1044	1045	101.082
				0.013	94.65	94.4	0
				L128	1045	1046	56.656
				0.013	94.25	94.16	0
				L129	1008	1009	51.231
				0.013	94.89	94.76	0
				L130	1025	1029	65.085
				0.013	94.04	93.91	0
				L131	1016	1048	60.5
				0.013	93.33	93.24	0
				L138	1012B	1012A	25.59
				0.013	94.83	94.67	0
				L139	1012A	1012	17.492
				0.013	94.67	94.43	0
				L140	1048A	1048	34.829
				0.013	94.14	94.06	0
				L31	2001	2002	49.342
				0.013	94.24	94.14	0
				L32	2002	2003	10.582
				0.013	94.14	94.11	0
				L33	2003	2004	36.326
				0.013	94.11	93.95	0
				L34	2004	2005	39.598
				0.013	93.87	93.73	0
				L46-1	1015	STM501	55.901
				0.013	93.43	93.348	0
				L46-2	STM501	1016	11.88
				0.013	93.348	93.33	0
				L47-1	1048	STM301	40.657
				0.013	93.24	93.167	0
				L47-2	STM301	1052	21.86
				0.013	93.167	93.13	0
				L48	1052	Diversion-1053	11.237
				0.013	93.1	93.09	0
				L49	1013	1014	74.913
				0.013	93.61	93.53	0
				L50	1014	1015	92.1
				0.013	93.53	93.43	0
				L56	2000	2001	42.002
				0.013	94.33	94.24	0
				L57	STM100	2000	17.842
				0.013	94.79	94.48	0
				L59	1011	1012	50.634
				0.013	94.11	93.98	0
				L60	1012	1015	52.722
				0.013	93.91	93.8	0
				L83	1017	1018	70.77
				0.013	95.14	94.93	0
				L84	1018	1019	105.059
				0.013	94.85	94.59	0
				L85	1019	1020	8.1
				0.013	94.59	94.57	0
				L86	1020	1021	55.271
				0.013	94.49	94.38	0
				L87	1021	1024	24.594
				0.013	94.38	94.33	0
				L88	1024	1025	35.837
				0.013	94.18	94.11	0
				L89	1029	1049	73.44
				0.013	93.91	93.72	0
				L90	1049	1050	8.491
				0.013	93.72	93.7	0
				L91	1050	1051	20.886
				0.013	93.7	93.65	0
				L92	1051	1052	41.983
				0.013	93.57	93.43	0
				L93	1022	1023	84.451
				0.013	94.88	94.72	0
				L94	1023	1024	116.004
				0.013	94.57	94.33	0
				L95	1026	1027	65.709
				0.013	94.76	94.59	0
				L96	1027	1028	91.482
				0.013	94.44	94.26	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

Node	Flow Type	Inflow	Outflow	Storage	Node	Flow Type	Inflow	Outflow	Storage
L60	CIRCULAR	0.825	0	0	1011	FLOW	1011	FLOW	1.0
L83	CIRCULAR	0.45	0	0	1012A	FLOW	1012A	FLOW	1.0
L84	CIRCULAR	0.525	0	0	1012B	FLOW	1012B	FLOW	1.0
L85	CIRCULAR	0.525	0	0	1013	FLOW	1013	FLOW	1.0
L86	CIRCULAR	0.6	0	0	1014	FLOW	1014	FLOW	1.0
L87	CIRCULAR	0.6	0	0	1015	FLOW	1015	FLOW	1.0
L88	CIRCULAR	0.75	0	0	1016	FLOW	1016	FLOW	1.0
L89	CIRCULAR	0.825	0	0	1017	FLOW	1017	FLOW	1.0
L90	CIRCULAR	0.825	0	0	1018	FLOW	1018	FLOW	1.0
L91	CIRCULAR	0.825	0	0	1020	FLOW	1020	FLOW	1.0
L92	CIRCULAR	0.9	0	0	1022	FLOW	1022	FLOW	1.0
L93	CIRCULAR	0.45	0	0	1023	FLOW	1023	FLOW	1.0
L94	CIRCULAR	0.6	0	0	1025	FLOW	1025	FLOW	1.0
L95	CIRCULAR	0.375	0	0	1026	FLOW	1026	FLOW	1.0
L96	CIRCULAR	0.525	0	0	1027	FLOW	1027	FLOW	1.0
L97	CIRCULAR	0.6	0	0	1028	FLOW	1028	FLOW	1.0
L98	CIRCULAR	0.375	0	0	1030	FLOW	1030	FLOW	1.0
CB-102A-1	CIRCULAR	0.102	0	0	1031	FLOW	1031	FLOW	1.0
CB102-A-2	CIRCULAR	0.102	0	0	1034	FLOW	1034	FLOW	1.0
CB-304A-1	CIRCULAR	0.095	0	0	1035	FLOW	1035	FLOW	1.0
CB-304A-2	CIRCULAR	0.095	0	0	1036	FLOW	1036	FLOW	1.0
CB-306A-1	CIRCULAR	0.083	0	0	1038	FLOW	1038	FLOW	1.0
CB-306A-2	CIRCULAR	0.083	0	0	1039	FLOW	1039	FLOW	1.0
CB-310A-1	CIRCULAR	0.083	0	0	1043	FLOW	1043	FLOW	1.0
CB-310A-2	CIRCULAR	0.083	0	0	1044	FLOW	1044	FLOW	1.0
CB-502A-1	CIRCULAR	0.102	0	0	1045	FLOW	1045	FLOW	1.0
CB-504A-1	CIRCULAR	0.075	0	0	1046	FLOW	1046	FLOW	1.0
CB-504A-2	CIRCULAR	0.075	0	0	1047	FLOW	1047	FLOW	1.0
CB-506A-1	CIRCULAR	0.083	0	0	1048A	FLOW	1048A	FLOW	1.0
CB-506A-2	CIRCULAR	0.083	0	0	1051	FLOW	1051	FLOW	1.0
CB-508A-1	CIRCULAR	0.075	0	0	1053	FLOW	1053	FLOW	1.0
CB-508A-2	CIRCULAR	0.083	0	0	2000	FLOW	2000	FLOW	1.0
CB-512A-1	CIRCULAR	0.075	0	0	2001	FLOW	2001	FLOW	1.0
CB-512A-2	CIRCULAR	0.07	0	0	2004	FLOW	2004	FLOW	1.0

```
[TRANSECTS]
;;Transect Data in HEC-2 format
;
NC 0.025 0.025 0.013
X1 18mROW 7 4.75 13.25 0.0 0.0
GR 0.3 0 0.15 4.75 0 4.75 0.13
9
GR 0.15 13.25 0.3 18
```

```
[LOSSES]
;;Link
Seepage
-----
L47-2 0 0.4 0 NO 0
L92 0 0.4 0 NO 0
```

```
[INFLOWS]
;;Node
Mfactor Sfactor Constituent Time Series Type
Baseline Pattern
-----
1001 FLOW 1001 FLOW 1.0
1 0
1002 FLOW 1002 FLOW 1.0
1 0
1004 FLOW 1004 FLOW 1.0
1 0
1005 FLOW 1005 FLOW 1.0
1 0
1006 FLOW 1006 FLOW 1.0
1 0
1008 FLOW 1008 FLOW 1.0
1 0
1009 FLOW 1009 FLOW 1.0
1 0
1010 FLOW 1010 FLOW 1.0
1 0
```

```
[CURVES]
;;Name Type X-Value Y-Value
-----
Curve1 Storage 0 0
Curve1 1.38 0
Curve1 1.52 93.1
Curve2 Storage 0 0
Curve2 1.38 0
Curve2 1.58 177.9
Curve3 Storage 0 0
Curve3 1.38 0
Curve3 1.49 96.1
Curve4 Storage 0 0
Curve4 1.38 0
Curve4 1.49 87.7
Curve5 Storage 0 0
Curve5 1.38 0
Curve5 1.57 287.8
Curve6 Storage 0 0
Curve6 1.38 0
Curve6 1.49 87.1
Curve7 Storage 0 0
Curve7 1.38 0
Curve7 1.58 258.5
Curve8 Storage 0 0
Curve8 1.38 0
Curve8 1.54 165.2
```

```
[TIMESERIES]
;;Name Date Time Value
-----
1001 0:00 0
1001 0:10 0
```

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1001	0:20	0	1002	5:00	0.001
1001	0:30	0.001	1002	5:10	0.001
1001	0:40	0.002	1002	5:20	0.001
1001	0:50	0.017	1002	5:30	0.001
1001	1:00	0.022	1002	5:40	0.001
1001	1:10	0.022	1002	5:50	0.001
1001	1:20	0.022	1002	6:00	0.001
1001	1:30	0.022	1002	6:10	0.001
1001	1:40	0.012	1002	6:20	0.001
1001	1:50	0.008	1002	6:30	0.001
1001	2:00	0.005	1002	6:40	0.001
1001	2:10	0.004	1002	6:50	0.001
1001	2:20	0.003	1002	7:00	0.001
1001	2:30	0.003	1002	7:10	0.001
1001	2:40	0.002	1002	7:20	0.001
1001	2:50	0.002	1002	7:30	0.001
1001	3:00	0.002	1002	7:40	0.001
1001	3:10	0.002	1002	7:50	0.001
1001	3:20	0.002	1002	8:00	0.001
1001	3:30	0.001	1002	8:10	0
1001	3:40	0.001	1002	8:20	0
1001	3:50	0.001	1002	8:30	0
1001	4:00	0.001	1002	8:40	0
1001	4:10	0.001	1002	8:50	0
1001	4:20	0.001	1002	9:00	0
1001	4:30	0.001	1002	9:10	0
1001	4:40	0.001	1002	9:20	0
1001	4:50	0.001	1002	9:30	0
1001	5:00	0	1002	9:40	0
1001	5:10	0	1002	9:50	0
1001	5:20	0	1002	10:00	0
1001	5:30	0	1002	10:10	0
1001	5:40	0	1002	10:20	0
1001	5:50	0	1002	10:30	0
1001	6:00	0	1002	10:40	0
1001	6:10	0	1002	10:50	0
1001	6:20	0	1002	11:00	0
1001	6:30	0	1002	11:10	0
1001	6:40	0	1002	11:20	0
1001	6:50	0	1002	11:30	0
1001	7:00	0	1002	11:40	0
1001	7:10	0	1002	11:50	0
1001	7:20	0	1002	12:00	0
1001	7:30	0	1002	12:10	0
1001	7:40	0	1002	12:20	0
1001	7:50	0	1002	12:30	0
1001	8:00	0	1002	12:40	0
1001	8:10	0	1002	12:50	0
1001	8:20	0	1002	13:00	0
1001	8:30	0	1002	13:10	0
1001	8:40	0			
1001	8:50	0	1004	0:00	0
1001	9:00	0	1004	0:10	0
1001	9:10	0	1004	0:20	0.002
1001	9:20	0	1004	0:30	0.006
1001	9:30	0	1004	0:40	0.014
1001	9:40	0	1004	0:50	0.033
1001	9:50	0	1004	1:00	0.044
1001	10:00	0	1004	1:10	0.042
1001	10:10	0	1004	1:20	0.024
1001	10:20	0	1004	1:30	0.016
1001	10:30	0	1004	1:40	0.01
1001	10:40	0	1004	1:50	0.008
1001	10:50	0	1004	2:00	0.006
1001	11:00	0	1004	2:10	0.005
1001	11:10	0	1004	2:20	0.005
1001	11:20	0	1004	2:30	0.004
1001	11:30	0	1004	2:40	0.004
1001	11:40	0	1004	2:50	0.004
1001	11:50	0	1004	3:00	0.003
1001	12:00	0	1004	3:10	0.003
1001	12:10	0	1004	3:20	0.002
1001	12:20	0	1004	3:30	0.001
1001	12:30	0	1004	3:40	0.001
1001	12:40	0	1004	3:50	0.001
1001	12:50	0	1004	4:00	0.001
1001	13:00	0	1004	4:10	0.001
1001	13:10	0	1004	4:20	0.001
			1004	4:30	0.001
1002	0:00	0	1004	4:40	0.001
1002	0:10	0	1004	4:50	0.001
1002	0:20	0.001	1004	5:00	0.001
1002	0:30	0.005	1004	5:10	0
1002	0:40	0.011	1004	5:20	0
1002	0:50	0.022	1004	5:30	0
1002	1:00	0.044	1004	5:40	0
1002	1:10	0.044	1004	5:50	0
1002	1:20	0.039	1004	6:00	0
1002	1:30	0.033	1004	6:10	0
1002	1:40	0.025	1004	6:20	0
1002	1:50	0.022	1004	6:30	0
1002	2:00	0.017	1004	6:40	0
1002	2:10	0.014	1004	6:50	0
1002	2:20	0.012	1004	7:00	0
1002	2:30	0.011	1004	7:10	0
1002	2:40	0.009	1004	7:20	0
1002	2:50	0.008	1004	7:30	0
1002	3:00	0.008	1004	7:40	0
1002	3:10	0.007	1004	7:50	0
1002	3:20	0.005	1004	8:00	0
1002	3:30	0.005	1004	8:10	0
1002	3:40	0.004	1004	8:20	0
1002	3:50	0.003	1004	8:30	0
1002	4:00	0.003	1004	8:40	0
1002	4:10	0.002	1004	8:50	0
1002	4:20	0.002	1004	9:00	0
1002	4:30	0.002	1004	9:10	0
1002	4:40	0.002	1004	9:20	0
1002	4:50	0.001	1004	9:30	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1004	9:40	0	1006	0:50	0.095
1004	9:50	0	1006	1:00	0.11
1004	10:00	0	1006	1:10	0.095
1004	10:10	0	1006	1:20	0.063
1004	10:20	0	1006	1:30	0.034
1004	10:30	0	1006	1:40	0.019
1004	10:40	0	1006	1:50	0.017
1004	10:50	0	1006	2:00	0.012
1004	11:00	0	1006	2:10	0.011
1004	11:10	0	1006	2:20	0.01
1004	11:20	0	1006	2:30	0.009
1004	11:30	0	1006	2:40	0.008
1004	11:40	0	1006	2:50	0.007
1004	11:50	0	1006	3:00	0.007
1004	12:00	0	1006	3:10	0.005
1004	12:10	0	1006	3:20	0.004
1004	12:20	0	1006	3:30	0.003
1004	12:30	0	1006	3:40	0.002
1004	12:40	0	1006	3:50	0.002
1004	12:50	0	1006	4:00	0.002
1004	13:00	0	1006	4:10	0.002
1004	13:10	0	1006	4:20	0.002
			1006	4:30	0.001
1005	0:00	0	1006	4:40	0.001
1005	0:10	0	1006	4:50	0.001
1005	0:20	0.001	1006	5:00	0.001
1005	0:30	0.004	1006	5:10	0.001
1005	0:40	0.009	1006	5:20	0.001
1005	0:50	0.024	1006	5:30	0.001
1005	1:00	0.044	1006	5:40	0.001
1005	1:10	0.044	1006	5:50	0.001
1005	1:20	0.034	1006	6:00	0.001
1005	1:30	0.026	1006	6:10	0.001
1005	1:40	0.019	1006	6:20	0.001
1005	1:50	0.015	1006	6:30	0.001
1005	2:00	0.011	1006	6:40	0.001
1005	2:10	0.009	1006	6:50	0.001
1005	2:20	0.008	1006	7:00	0.001
1005	2:30	0.007	1006	7:10	0.001
1005	2:40	0.006	1006	7:20	0.001
1005	2:50	0.005	1006	7:30	0.001
1005	3:00	0.005	1006	7:40	0.001
1005	3:10	0.004	1006	7:50	0
1005	3:20	0.004	1006	8:00	0
1005	3:30	0.003	1006	8:10	0
1005	3:40	0.002	1006	8:20	0
1005	3:50	0.002	1006	8:30	0
1005	4:00	0.002	1006	8:40	0
1005	4:10	0.001	1006	8:50	0
1005	4:20	0.001	1006	9:00	0
1005	4:30	0.001	1006	9:10	0
1005	4:40	0.001	1006	9:20	0
1005	4:50	0.001	1006	9:30	0
1005	5:00	0.001	1006	9:40	0
1005	5:10	0.001	1006	9:50	0
1005	5:20	0.001	1006	10:00	0
1005	5:30	0.001	1006	10:10	0
1005	5:40	0.001	1006	10:20	0
1005	5:50	0.001	1006	10:30	0
1005	6:00	0.001	1006	10:40	0
1005	6:10	0.001	1006	10:50	0
1005	6:20	0.001	1006	11:00	0
1005	6:30	0.001	1006	11:10	0
1005	6:40	0	1006	11:20	0
1005	6:50	0	1006	11:30	0
1005	7:00	0	1006	11:40	0
1005	7:10	0	1006	11:50	0
1005	7:20	0	1006	12:00	0
1005	7:30	0	1006	12:10	0
1005	7:40	0	1006	12:20	0
1005	7:50	0	1006	12:30	0
1005	8:00	0	1006	12:40	0
1005	8:10	0	1006	12:50	0
1005	8:20	0	1006	13:00	0
1005	8:30	0	1006	13:10	0
1005	8:40	0			
1005	8:50	0	1008	0:00	0
1005	9:00	0	1008	0:10	0
1005	9:10	0	1008	0:20	0.001
1005	9:20	0	1008	0:30	0.006
1005	9:30	0	1008	0:40	0.011
1005	9:40	0	1008	0:50	0.044
1005	9:50	0	1008	1:00	0.066
1005	10:00	0	1008	1:10	0.066
1005	10:10	0	1008	1:20	0.065
1005	10:20	0	1008	1:30	0.053
1005	10:30	0	1008	1:40	0.027
1005	10:40	0	1008	1:50	0.018
1005	10:50	0	1008	2:00	0.011
1005	11:00	0	1008	2:10	0.009
1005	11:10	0	1008	2:20	0.007
1005	11:20	0	1008	2:30	0.007
1005	11:30	0	1008	2:40	0.006
1005	11:40	0	1008	2:50	0.005
1005	11:50	0	1008	3:00	0.005
1005	12:00	0	1008	3:10	0.004
1005	12:10	0	1008	3:20	0.003
1005	12:20	0	1008	3:30	0.003
1005	12:30	0	1008	3:40	0.002
1005	12:40	0	1008	3:50	0.002
1005	12:50	0	1008	4:00	0.002
1005	13:00	0	1008	4:10	0.001
1005	13:10	0	1008	4:20	0.001
			1008	4:30	0.001
1006	0:00	0	1008	4:40	0.001
1006	0:10	0	1008	4:50	0.001
1006	0:20	0.004	1008	5:00	0.001
1006	0:30	0.015	1008	5:10	0.001
1006	0:40	0.034	1008	5:20	0.001

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1008	5:30	0.001	1009	10:10	0
1008	5:40	0.001	1009	10:20	0
1008	5:50	0.001	1009	10:30	0
1008	6:00	0.001	1009	10:40	0
1008	6:10	0	1009	10:50	0
1008	6:20	0	1009	11:00	0
1008	6:30	0	1009	11:10	0
1008	6:40	0	1009	11:20	0
1008	6:50	0	1009	11:30	0
1008	7:00	0	1009	11:40	0
1008	7:10	0	1009	11:50	0
1008	7:20	0	1009	12:00	0
1008	7:30	0	1009	12:10	0
1008	7:40	0	1009	12:20	0
1008	7:50	0	1009	12:30	0
1008	8:00	0	1009	12:40	0
1008	8:10	0	1009	12:50	0
1008	8:20	0	1009	13:00	0
1008	8:30	0	1009	13:10	0
1008	8:40	0			
1008	8:50	0	1010	0:00	0
1008	9:00	0	1010	0:10	0
1008	9:10	0	1010	0:20	0.001
1008	9:20	0	1010	0:30	0.009
1008	9:30	0	1010	0:40	0.021
1008	9:40	0	1010	0:50	0.052
1008	9:50	0	1010	1:00	0.054
1008	10:00	0	1010	1:10	0.049
1008	10:10	0	1010	1:20	0.047
1008	10:20	0	1010	1:30	0.028
1008	10:30	0	1010	1:40	0.016
1008	10:40	0	1010	1:50	0.013
1008	10:50	0	1010	2:00	0.01
1008	11:00	0	1010	2:10	0.008
1008	11:10	0	1010	2:20	0.007
1008	11:20	0	1010	2:30	0.006
1008	11:30	0	1010	2:40	0.006
1008	11:40	0	1010	2:50	0.005
1008	11:50	0	1010	3:00	0.005
1008	12:00	0	1010	3:10	0.004
1008	12:10	0	1010	3:20	0.003
1008	12:20	0	1010	3:30	0.002
1008	12:30	0	1010	3:40	0.002
1008	12:40	0	1010	3:50	0.002
1008	12:50	0	1010	4:00	0.001
1008	13:00	0	1010	4:10	0.001
1008	13:10	0	1010	4:20	0.001
			1010	4:30	0.001
1009	0:00	0	1010	4:40	0.001
1009	0:10	0	1010	4:50	0.001
1009	0:20	0.001	1010	5:00	0.001
1009	0:30	0.004	1010	5:10	0.001
1009	0:40	0.008	1010	5:20	0.001
1009	0:50	0.044	1010	5:30	0.001
1009	1:00	0.043	1010	5:40	0.001
1009	1:10	0.03	1010	5:50	0.001
1009	1:20	0.016	1010	6:00	0.001
1009	1:30	0.012	1010	6:10	0.001
1009	1:40	0.006	1010	6:20	0.001
1009	1:50	0.006	1010	6:30	0.001
1009	2:00	0.003	1010	6:40	0
1009	2:10	0.003	1010	6:50	0
1009	2:20	0.003	1010	7:00	0
1009	2:30	0.003	1010	7:10	0
1009	2:40	0.002	1010	7:20	0
1009	2:50	0.002	1010	7:30	0
1009	3:00	0.002	1010	7:40	0
1009	3:10	0.001	1010	7:50	0
1009	3:20	0.001	1010	8:00	0
1009	3:30	0.001	1010	8:10	0
1009	3:40	0.001	1010	8:20	0
1009	3:50	0.001	1010	8:30	0
1009	4:00	0.001	1010	8:40	0
1009	4:10	0.001	1010	8:50	0
1009	4:20	0	1010	9:00	0
1009	4:30	0	1010	9:10	0
1009	4:40	0	1010	9:20	0
1009	4:50	0	1010	9:30	0
1009	5:00	0	1010	9:40	0
1009	5:10	0	1010	9:50	0
1009	5:20	0	1010	10:00	0
1009	5:30	0	1010	10:10	0
1009	5:40	0	1010	10:20	0
1009	5:50	0	1010	10:30	0
1009	6:00	0	1010	10:40	0
1009	6:10	0	1010	10:50	0
1009	6:20	0	1010	11:00	0
1009	6:30	0	1010	11:10	0
1009	6:40	0	1010	11:20	0
1009	6:50	0	1010	11:30	0
1009	7:00	0	1010	11:40	0
1009	7:10	0	1010	11:50	0
1009	7:20	0	1010	12:00	0
1009	7:30	0	1010	12:10	0
1009	7:40	0	1010	12:20	0
1009	7:50	0	1010	12:30	0
1009	8:00	0	1010	12:40	0
1009	8:10	0	1010	12:50	0
1009	8:20	0	1010	13:00	0
1009	8:30	0	1010	13:10	0
1009	8:40	0			
1009	8:50	0	1011	0:00	0
1009	9:00	0	1011	0:10	0
1009	9:10	0	1011	0:20	0
1009	9:20	0	1011	0:30	0.002
1009	9:30	0	1011	0:40	0.005
1009	9:40	0	1011	0:50	0.028
1009	9:50	0	1011	1:00	0.032
1009	10:00	0	1011	1:10	0.027

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1012B	10:20	0.002	1014	1:20	0.022
1012B	10:30	0.002	1014	1:30	0.022
1012B	10:40	0.002	1014	1:40	0.022
1012B	10:50	0.002	1014	1:50	0.021
1012B	11:00	0.002	1014	2:00	0.02
1012B	11:10	0.002	1014	2:10	0.017
1012B	11:20	0.002	1014	2:20	0.015
1012B	11:30	0.002	1014	2:30	0.013
1012B	11:40	0.002	1014	2:40	0.012
1012B	11:50	0.002	1014	2:50	0.01
1012B	12:00	0.001	1014	3:00	0.009
1012B	12:10	0.001	1014	3:10	0.008
1012B	12:20	0.001	1014	3:20	0.007
1012B	12:30	0.001	1014	3:30	0.006
1012B	12:40	0.001	1014	3:40	0.005
1012B	12:50	0.001	1014	3:50	0.005
1012B	13:00	0.001	1014	4:00	0.004
1012B	13:10	0.001	1014	4:10	0.003
			1014	4:20	0.003
			1014	4:30	0.003
			1014	4:40	0.002
			1014	4:50	0.002
			1014	5:00	0.002
			1014	5:10	0.002
			1014	5:20	0.001
			1014	5:30	0.001
			1014	5:40	0.001
			1014	5:50	0.001
			1014	6:00	0.001
			1014	6:10	0.001
			1014	6:20	0.001
			1014	6:30	0.001
			1014	6:40	0.001
			1014	6:50	0.001
			1014	7:00	0.001
			1014	7:10	0.001
			1014	7:20	0.001
			1014	7:30	0
			1014	7:40	0
			1014	7:50	0
			1014	8:00	0
			1014	8:10	0
			1014	8:20	0
			1014	8:30	0
			1014	8:40	0
			1014	8:50	0
			1014	9:00	0
			1014	9:10	0
			1014	9:20	0
			1014	9:30	0
			1014	9:40	0
			1014	9:50	0
			1014	10:00	0
			1014	10:10	0
			1014	10:20	0
			1014	10:30	0
			1014	10:40	0
			1014	10:50	0
			1014	11:00	0
			1014	11:10	0
			1014	11:20	0
			1014	11:30	0
			1014	11:40	0
			1014	11:50	0
			1014	12:00	0
			1014	12:10	0
			1014	12:20	0
			1014	12:30	0
			1014	12:40	0
			1014	12:50	0
			1014	13:00	0
			1014	13:10	0
			1015	0:00	0
			1015	0:10	0
			1015	0:20	0.001
			1015	0:30	0.004
			1015	0:40	0.011
			1015	0:50	0.044
			1015	1:00	0.044
			1015	1:10	0.044
			1015	1:20	0.044
			1015	1:30	0.041
			1015	1:40	0.036
			1015	1:50	0.031
			1015	2:00	0.027
			1015	2:10	0.022
			1015	2:20	0.018
			1015	2:30	0.015
			1015	2:40	0.013
			1015	2:50	0.011
			1015	3:00	0.01
			1015	3:10	0.008
			1015	3:20	0.007
			1015	3:30	0.006
			1015	3:40	0.005
			1015	3:50	0.004
			1015	4:00	0.004
			1015	4:10	0.003
			1015	4:20	0.003
			1015	4:30	0.002
			1015	4:40	0.002
			1015	4:50	0.002
			1015	5:00	0.002
			1015	5:10	0.001
			1015	5:20	0.001
			1015	5:30	0.001
			1015	5:40	0.001
			1015	5:50	0.001
			1014	0:00	0
			1014	0:10	0
			1014	0:20	0
			1014	0:30	0.002
			1014	0:40	0.005
			1014	0:50	0.022
			1014	1:00	0.022
			1014	1:10	0.022

;Design inflow to MH1013 from Oct 2007 Phl Subdivision design

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1015	6:00	0.001	1016	10:40	0
1015	6:10	0.001	1016	10:50	0
1015	6:20	0.001	1016	11:00	0
1015	6:30	0.001	1016	11:10	0
1015	6:40	0.001	1016	11:20	0
1015	6:50	0.001	1016	11:30	0
1015	7:00	0.001	1016	11:40	0
1015	7:10	0.001	1016	11:50	0
1015	7:20	0.001	1016	12:00	0
1015	7:30	0.001	1016	12:10	0
1015	7:40	0.001	1016	12:20	0
1015	7:50	0.001	1016	12:30	0
1015	8:00	0.001	1016	12:40	0
1015	8:10	0.001	1016	12:50	0
1015	8:20	0.001	1016	13:00	0
1015	8:30	0.001	1016	13:10	0
1015	8:40	0			
1015	8:50	0	1017	0:00	0
1015	9:00	0	1017	0:10	0
1015	9:10	0	1017	0:20	0.001
1015	9:20	0	1017	0:30	0.003
1015	9:30	0	1017	0:40	0.008
1015	9:40	0	1017	0:50	0.022
1015	9:50	0	1017	1:00	0.022
1015	10:00	0	1017	1:10	0.021
1015	10:10	0	1017	1:20	0.02
1015	10:20	0	1017	1:30	0.016
1015	10:30	0	1017	1:40	0.012
1015	10:40	0	1017	1:50	0.009
1015	10:50	0	1017	2:00	0.007
1015	11:00	0	1017	2:10	0.006
1015	11:10	0	1017	2:20	0.005
1015	11:20	0	1017	2:30	0.004
1015	11:30	0	1017	2:40	0.004
1015	11:40	0	1017	2:50	0.003
1015	11:50	0	1017	3:00	0.003
1015	12:00	0	1017	3:10	0.003
1015	12:10	0	1017	3:20	0.002
1015	12:20	0	1017	3:30	0.002
1015	12:30	0	1017	3:40	0.001
1015	12:40	0	1017	3:50	0.001
1015	12:50	0	1017	4:00	0.001
1015	13:00	0	1017	4:10	0.001
1015	13:10	0	1017	4:20	0.001
			1017	4:30	0.001
1016	0:00	0	1017	4:40	0.001
1016	0:10	0	1017	4:50	0
1016	0:20	0.001	1017	5:00	0
1016	0:30	0.006	1017	5:10	0
1016	0:40	0.008	1017	5:20	0
1016	0:50	0.022	1017	5:30	0
1016	1:00	0.044	1017	5:40	0
1016	1:10	0.044	1017	5:50	0
1016	1:20	0.044	1017	6:00	0
1016	1:30	0.044	1017	6:10	0
1016	1:40	0.032	1017	6:20	0
1016	1:50	0.021	1017	6:30	0
1016	2:00	0.012	1017	6:40	0
1016	2:10	0.01	1017	6:50	0
1016	2:20	0.007	1017	7:00	0
1016	2:30	0.007	1017	7:10	0
1016	2:40	0.006	1017	7:20	0
1016	2:50	0.006	1017	7:30	0
1016	3:00	0.006	1017	7:40	0
1016	3:10	0.005	1017	7:50	0
1016	3:20	0.004	1017	8:00	0
1016	3:30	0.003	1017	8:10	0
1016	3:40	0.003	1017	8:20	0
1016	3:50	0.002	1017	8:30	0
1016	4:00	0.002	1017	8:40	0
1016	4:10	0.002	1017	8:50	0
1016	4:20	0.001	1017	9:00	0
1016	4:30	0.001	1017	9:10	0
1016	4:40	0.001	1017	9:20	0
1016	4:50	0.001	1017	9:30	0
1016	5:00	0.001	1017	9:40	0
1016	5:10	0.001	1017	9:50	0
1016	5:20	0.001	1017	10:00	0
1016	5:30	0.001	1017	10:10	0
1016	5:40	0.001	1017	10:20	0
1016	5:50	0.001	1017	10:30	0
1016	6:00	0.001	1017	10:40	0
1016	6:10	0.001	1017	10:50	0
1016	6:20	0.001	1017	11:00	0
1016	6:30	0.001	1017	11:10	0
1016	6:40	0	1017	11:20	0
1016	6:50	0	1017	11:30	0
1016	7:00	0	1017	11:40	0
1016	7:10	0	1017	11:50	0
1016	7:20	0	1017	12:00	0
1016	7:30	0	1017	12:10	0
1016	7:40	0	1017	12:20	0
1016	7:50	0	1017	12:30	0
1016	8:00	0	1017	12:40	0
1016	8:10	0	1017	12:50	0
1016	8:20	0	1017	13:00	0
1016	8:30	0	1017	13:10	0
1016	8:40	0			
1016	8:50	0	1018	0:00	0
1016	9:00	0	1018	0:10	0
1016	9:10	0	1018	0:20	0.001
1016	9:20	0	1018	0:30	0.007
1016	9:30	0	1018	0:40	0.012
1016	9:40	0	1018	0:50	0.022
1016	9:50	0	1018	1:00	0.044
1016	10:00	0	1018	1:10	0.044
1016	10:10	0	1018	1:20	0.039
1016	10:20	0	1018	1:30	0.032
1016	10:30	0	1018	1:40	0.02

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1018	1:50	0.017	1020	6:30	0
1018	2:00	0.011	1020	6:40	0
1018	2:10	0.009	1020	6:50	0
1018	2:20	0.007	1020	7:00	0
1018	2:30	0.007	1020	7:10	0
1018	2:40	0.006	1020	7:20	0
1018	2:50	0.005	1020	7:30	0
1018	3:00	0.005	1020	7:40	0
1018	3:10	0.004	1020	7:50	0
1018	3:20	0.003	1020	8:00	0
1018	3:30	0.003	1020	8:10	0
1018	3:40	0.002	1020	8:20	0
1018	3:50	0.002	1020	8:30	0
1018	4:00	0.002	1020	8:40	0
1018	4:10	0.001	1020	8:50	0
1018	4:20	0.001	1020	9:00	0
1018	4:30	0.001	1020	9:10	0
1018	4:40	0.001	1020	9:20	0
1018	4:50	0.001	1020	9:30	0
1018	5:00	0.001	1020	9:40	0
1018	5:10	0.001	1020	9:50	0
1018	5:20	0.001	1020	10:00	0
1018	5:30	0.001	1020	10:10	0
1018	5:40	0.001	1020	10:20	0
1018	5:50	0.001	1020	10:30	0
1018	6:00	0.001	1020	10:40	0
1018	6:10	0	1020	10:50	0
1018	6:20	0	1020	11:00	0
1018	6:30	0	1020	11:10	0
1018	6:40	0	1020	11:20	0
1018	6:50	0	1020	11:30	0
1018	7:00	0	1020	11:40	0
1018	7:10	0	1020	11:50	0
1018	7:20	0	1020	12:00	0
1018	7:30	0	1020	12:10	0
1018	7:40	0	1020	12:20	0
1018	7:50	0	1020	12:30	0
1018	8:00	0	1020	12:40	0
1018	8:10	0	1020	12:50	0
1018	8:20	0	1020	13:00	0
1018	8:30	0	1020	13:10	0
1018	8:40	0			
1018	8:50	0	1022	0:00	0
1018	9:00	0	1022	0:10	0
1018	9:10	0	1022	0:20	0.003
1018	9:20	0	1022	0:30	0.012
1018	9:30	0	1022	0:40	0.026
1018	9:40	0	1022	0:50	0.054
1018	9:50	0	1022	1:00	0.066
1018	10:00	0	1022	1:10	0.066
1018	10:10	0	1022	1:20	0.066
1018	10:20	0	1022	1:30	0.046
1018	10:30	0	1022	1:40	0.022
1018	10:40	0	1022	1:50	0.018
1018	10:50	0	1022	2:00	0.014
1018	11:00	0	1022	2:10	0.011
1018	11:10	0	1022	2:20	0.01
1018	11:20	0	1022	2:30	0.009
1018	11:30	0	1022	2:40	0.008
1018	11:40	0	1022	2:50	0.007
1018	11:50	0	1022	3:00	0.007
1018	12:00	0	1022	3:10	0.005
1018	12:10	0	1022	3:20	0.004
1018	12:20	0	1022	3:30	0.003
1018	12:30	0	1022	3:40	0.003
1018	12:40	0	1022	3:50	0.002
1018	12:50	0	1022	4:00	0.002
1018	13:00	0	1022	4:10	0.002
1018	13:10	0	1022	4:20	0.002
			1022	4:30	0.001
1020	0:00	0	1022	4:40	0.001
1020	0:10	0	1022	4:50	0.001
1020	0:20	0.002	1022	5:00	0.001
1020	0:30	0.008	1022	5:10	0.001
1020	0:40	0.019	1022	5:20	0.001
1020	0:50	0.022	1022	5:30	0.001
1020	1:00	0.022	1022	5:40	0.001
1020	1:10	0.022	1022	5:50	0.001
1020	1:20	0.022	1022	6:00	0.001
1020	1:30	0.018	1022	6:10	0.001
1020	1:40	0.012	1022	6:20	0.001
1020	1:50	0.01	1022	6:30	0.001
1020	2:00	0.008	1022	6:40	0.001
1020	2:10	0.007	1022	6:50	0.001
1020	2:20	0.006	1022	7:00	0.001
1020	2:30	0.006	1022	7:10	0.001
1020	2:40	0.005	1022	7:20	0.001
1020	2:50	0.005	1022	7:30	0.001
1020	3:00	0.004	1022	7:40	0.001
1020	3:10	0.004	1022	7:50	0.001
1020	3:20	0.003	1022	8:00	0
1020	3:30	0.002	1022	8:10	0
1020	3:40	0.002	1022	8:20	0
1020	3:50	0.002	1022	8:30	0
1020	4:00	0.001	1022	8:40	0
1020	4:10	0.001	1022	8:50	0
1020	4:20	0.001	1022	9:00	0
1020	4:30	0.001	1022	9:10	0
1020	4:40	0.001	1022	9:20	0
1020	4:50	0.001	1022	9:30	0
1020	5:00	0.001	1022	9:40	0
1020	5:10	0.001	1022	9:50	0
1020	5:20	0.001	1022	10:00	0
1020	5:30	0.001	1022	10:10	0
1020	5:40	0.001	1022	10:20	0
1020	5:50	0.001	1022	10:30	0
1020	6:00	0.001	1022	10:40	0
1020	6:10	0.001	1022	10:50	0
1020	6:20	0.001	1022	11:00	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1022	11:10	0	1025	2:20	0.003
1022	11:20	0	1025	2:30	0.003
1022	11:30	0	1025	2:40	0.003
1022	11:40	0	1025	2:50	0.002
1022	11:50	0	1025	3:00	0.002
1022	12:00	0	1025	3:10	0.002
1022	12:10	0	1025	3:20	0.001
1022	12:20	0	1025	3:30	0.001
1022	12:30	0	1025	3:40	0.001
1022	12:40	0	1025	3:50	0.001
1022	12:50	0	1025	4:00	0.001
1022	13:00	0	1025	4:10	0.001
1022	13:10	0	1025	4:20	0.001
			1025	4:30	0.001
1023	0:00	0	1025	4:40	0.001
1023	0:10	0	1025	4:50	0.001
1023	0:20	0.002	1025	5:00	0
1023	0:30	0.009	1025	5:10	0
1023	0:40	0.021	1025	5:20	0
1023	0:50	0.099	1025	5:30	0
1023	1:00	0.116	1025	5:40	0
1023	1:10	0.113	1025	5:50	0
1023	1:20	0.102	1025	6:00	0
1023	1:30	0.085	1025	6:10	0
1023	1:40	0.052	1025	6:20	0
1023	1:50	0.037	1025	6:30	0
1023	2:00	0.026	1025	6:40	0
1023	2:10	0.02	1025	6:50	0
1023	2:20	0.016	1025	7:00	0
1023	2:30	0.013	1025	7:10	0
1023	2:40	0.011	1025	7:20	0
1023	2:50	0.01	1025	7:30	0
1023	3:00	0.009	1025	7:40	0
1023	3:10	0.007	1025	7:50	0
1023	3:20	0.006	1025	8:00	0
1023	3:30	0.005	1025	8:10	0
1023	3:40	0.004	1025	8:20	0
1023	3:50	0.003	1025	8:30	0
1023	4:00	0.003	1025	8:40	0
1023	4:10	0.002	1025	8:50	0
1023	4:20	0.002	1025	9:00	0
1023	4:30	0.002	1025	9:10	0
1023	4:40	0.002	1025	9:20	0
1023	4:50	0.001	1025	9:30	0
1023	5:00	0.001	1025	9:40	0
1023	5:10	0.001	1025	9:50	0
1023	5:20	0.001	1025	10:00	0
1023	5:30	0.001	1025	10:10	0
1023	5:40	0.001	1025	10:20	0
1023	5:50	0.001	1025	10:30	0
1023	6:00	0.001	1025	10:40	0
1023	6:10	0.001	1025	10:50	0
1023	6:20	0.001	1025	11:00	0
1023	6:30	0.001	1025	11:10	0
1023	6:40	0.001	1025	11:20	0
1023	6:50	0.001	1025	11:30	0
1023	7:00	0.001	1025	11:40	0
1023	7:10	0.001	1025	11:50	0
1023	7:20	0.001	1025	12:00	0
1023	7:30	0.001	1025	12:10	0
1023	7:40	0.001	1025	12:20	0
1023	7:50	0.001	1025	12:30	0
1023	8:00	0.001	1025	12:40	0
1023	8:10	0	1025	12:50	0
1023	8:20	0	1025	13:00	0
1023	8:30	0	1025	13:10	0
1023	8:40	0			
1023	8:50	0	1026	0:00	0
1023	9:00	0	1026	0:10	0
1023	9:10	0	1026	0:20	0.001
1023	9:20	0	1026	0:30	0.005
1023	9:30	0	1026	0:40	0.012
1023	9:40	0	1026	0:50	0.022
1023	9:50	0	1026	1:00	0.022
1023	10:00	0	1026	1:10	0.021
1023	10:10	0	1026	1:20	0.017
1023	10:20	0	1026	1:30	0.012
1023	10:30	0	1026	1:40	0.008
1023	10:40	0	1026	1:50	0.006
1023	10:50	0	1026	2:00	0.005
1023	11:00	0	1026	2:10	0.004
1023	11:10	0	1026	2:20	0.004
1023	11:20	0	1026	2:30	0.003
1023	11:30	0	1026	2:40	0.003
1023	11:40	0	1026	2:50	0.003
1023	11:50	0	1026	3:00	0.003
1023	12:00	0	1026	3:10	0.002
1023	12:10	0	1026	3:20	0.001
1023	12:20	0	1026	3:30	0.001
1023	12:30	0	1026	3:40	0.001
1023	12:40	0	1026	3:50	0.001
1023	12:50	0	1026	4:00	0.001
1023	13:00	0	1026	4:10	0.001
1023	13:10	0	1026	4:20	0.001
			1026	4:30	0.001
1025	0:00	0	1026	4:40	0
1025	0:10	0	1026	4:50	0
1025	0:20	0.001	1026	5:00	0
1025	0:30	0.003	1026	5:10	0
1025	0:40	0.008	1026	5:20	0
1025	0:50	0.022	1026	5:30	0
1025	1:00	0.022	1026	5:40	0
1025	1:10	0.022	1026	5:50	0
1025	1:20	0.022	1026	6:00	0
1025	1:30	0.019	1026	6:10	0
1025	1:40	0.011	1026	6:20	0
1025	1:50	0.005	1026	6:30	0
1025	2:00	0.004	1026	6:40	0
1025	2:10	0.004	1026	6:50	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1026	7:00	0	1027	11:40	0
1026	7:10	0	1027	11:50	0
1026	7:20	0	1027	12:00	0
1026	7:30	0	1027	12:10	0
1026	7:40	0	1027	12:20	0
1026	7:50	0	1027	12:30	0
1026	8:00	0	1027	12:40	0
1026	8:10	0	1027	12:50	0
1026	8:20	0	1027	13:00	0
1026	8:30	0	1027	13:10	0
1026	8:40	0			
1026	8:50	0	1028	0:00	0
1026	9:00	0	1028	0:10	0
1026	9:10	0	1028	0:20	0.001
1026	9:20	0	1028	0:30	0.004
1026	9:30	0	1028	0:40	0.008
1026	9:40	0	1028	0:50	0.044
1026	9:50	0	1028	1:00	0.044
1026	10:00	0	1028	1:10	0.044
1026	10:10	0	1028	1:20	0.044
1026	10:20	0	1028	1:30	0.036
1026	10:30	0	1028	1:40	0.02
1026	10:40	0	1028	1:50	0.01
1026	10:50	0	1028	2:00	0.006
1026	11:00	0	1028	2:10	0.005
1026	11:10	0	1028	2:20	0.004
1026	11:20	0	1028	2:30	0.004
1026	11:30	0	1028	2:40	0.003
1026	11:40	0	1028	2:50	0.003
1026	11:50	0	1028	3:00	0.003
1026	12:00	0	1028	3:10	0.002
1026	12:10	0	1028	3:20	0.002
1026	12:20	0	1028	3:30	0.001
1026	12:30	0	1028	3:40	0.001
1026	12:40	0	1028	3:50	0.001
1026	12:50	0	1028	4:00	0.001
1026	13:00	0	1028	4:10	0.001
1026	13:10	0	1028	4:20	0.001
			1028	4:30	0.001
			1028	4:40	0.001
1027	0:00	0	1028	4:50	0.001
1027	0:10	0	1028	5:00	0.001
1027	0:20	0.004	1028	5:10	0
1027	0:30	0.014	1028	5:20	0
1027	0:40	0.031	1028	5:30	0
1027	0:50	0.1	1028	5:40	0
1027	1:00	0.11	1028	5:50	0
1027	1:10	0.104	1028	6:00	0
1027	1:20	0.088	1028	6:10	0
1027	1:30	0.051	1028	6:20	0
1027	1:40	0.026	1028	6:30	0
1027	1:50	0.019	1028	6:40	0
1027	2:00	0.013	1028	6:50	0
1027	2:10	0.011	1028	7:00	0
1027	2:20	0.01	1028	7:10	0
1027	2:30	0.009	1028	7:20	0
1027	2:40	0.008	1028	7:30	0
1027	2:50	0.007	1028	7:40	0
1027	3:00	0.006	1028	7:50	0
1027	3:10	0.005	1028	8:00	0
1027	3:20	0.004	1028	8:10	0
1027	3:30	0.003	1028	8:20	0
1027	3:40	0.002	1028	8:30	0
1027	3:50	0.002	1028	8:40	0
1027	4:00	0.002	1028	8:50	0
1027	4:10	0.002	1028	9:00	0
1027	4:20	0.001	1028	9:10	0
1027	4:30	0.001	1028	9:20	0
1027	4:40	0.001	1028	9:30	0
1027	4:50	0.001	1028	9:40	0
1027	5:00	0.001	1028	9:50	0
1027	5:10	0.001	1028	10:00	0
1027	5:20	0.001	1028	10:10	0
1027	5:30	0.001	1028	10:20	0
1027	5:40	0.001	1028	10:30	0
1027	5:50	0.001	1028	10:40	0
1027	6:00	0.001	1028	10:50	0
1027	6:10	0.001	1028	11:00	0
1027	6:20	0.001	1028	11:10	0
1027	6:30	0.001	1028	11:20	0
1027	6:40	0.001	1028	11:30	0
1027	6:50	0.001	1028	11:40	0
1027	7:00	0.001	1028	11:50	0
1027	7:10	0.001	1028	12:00	0
1027	7:20	0	1028	12:10	0
1027	7:30	0	1028	12:20	0
1027	7:40	0	1028	12:30	0
1027	7:50	0	1028	12:40	0
1027	8:00	0	1028	12:50	0
1027	8:10	0	1028	13:00	0
1027	8:20	0	1028	13:10	0
1027	8:30	0			
1027	8:40	0			
1027	8:50	0	1030	0:00	0
1027	9:00	0	1030	0:10	0
1027	9:10	0	1030	0:20	0
1027	9:20	0	1030	0:30	0.002
1027	9:30	0	1030	0:40	0.006
1027	9:40	0	1030	0:50	0.022
1027	9:50	0	1030	1:00	0.022
1027	10:00	0	1030	1:10	0.022
1027	10:10	0	1030	1:20	0.021
1027	10:20	0	1030	1:30	0.018
1027	10:30	0	1030	1:40	0.015
1027	10:40	0	1030	1:50	0.012
1027	10:50	0	1030	2:00	0.01
1027	11:00	0	1030	2:10	0.008
1027	11:10	0	1030	2:20	0.007
1027	11:20	0	1030	2:30	0.006
1027	11:30	0	1030	2:40	0.005

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1030	2:50	0.004	1031	7:30	0
1030	3:00	0.004	1031	7:40	0
1030	3:10	0.003	1031	7:50	0
1030	3:20	0.003	1031	8:00	0
1030	3:30	0.002	1031	8:10	0
1030	3:40	0.002	1031	8:20	0
1030	3:50	0.002	1031	8:30	0
1030	4:00	0.001	1031	8:40	0
1030	4:10	0.001	1031	8:50	0
1030	4:20	0.001	1031	9:00	0
1030	4:30	0.001	1031	9:10	0
1030	4:40	0.001	1031	9:20	0
1030	4:50	0.001	1031	9:30	0
1030	5:00	0.001	1031	9:40	0
1030	5:10	0.001	1031	9:50	0
1030	5:20	0.001	1031	10:00	0
1030	5:30	0.001	1031	10:10	0
1030	5:40	0	1031	10:20	0
1030	5:50	0	1031	10:30	0
1030	6:00	0	1031	10:40	0
1030	6:10	0	1031	10:50	0
1030	6:20	0	1031	11:00	0
1030	6:30	0	1031	11:10	0
1030	6:40	0	1031	11:20	0
1030	6:50	0	1031	11:30	0
1030	7:00	0	1031	11:40	0
1030	7:10	0	1031	11:50	0
1030	7:20	0	1031	12:00	0
1030	7:30	0	1031	12:10	0
1030	7:40	0	1031	12:20	0
1030	7:50	0	1031	12:30	0
1030	8:00	0	1031	12:40	0
1030	8:10	0	1031	12:50	0
1030	8:20	0	1031	13:00	0
1030	8:30	0	1031	13:10	0
1030	8:40	0			
1030	8:50	0	1034	0:00	0
1030	9:00	0	1034	0:10	0
1030	9:10	0	1034	0:20	0.001
1030	9:20	0	1034	0:30	0.004
1030	9:30	0	1034	0:40	0.011
1030	9:40	0	1034	0:50	0.054
1030	9:50	0	1034	1:00	0.066
1030	10:00	0	1034	1:10	0.065
1030	10:10	0	1034	1:20	0.056
1030	10:20	0	1034	1:30	0.044
1030	10:30	0	1034	1:40	0.033
1030	10:40	0	1034	1:50	0.028
1030	10:50	0	1034	2:00	0.023
1030	11:00	0	1034	2:10	0.019
1030	11:10	0	1034	2:20	0.016
1030	11:20	0	1034	2:30	0.014
1030	11:30	0	1034	2:40	0.013
1030	11:40	0	1034	2:50	0.011
1030	11:50	0	1034	3:00	0.01
1030	12:00	0	1034	3:10	0.009
1030	12:10	0	1034	3:20	0.008
1030	12:20	0	1034	3:30	0.006
1030	12:30	0	1034	3:40	0.005
1030	12:40	0	1034	3:50	0.005
1030	12:50	0	1034	4:00	0.004
1030	13:00	0	1034	4:10	0.003
1030	13:10	0	1034	4:20	0.003
			1034	4:30	0.003
			1034	4:40	0.002
1031	0:00	0	1034	4:50	0.002
1031	0:10	0	1034	5:00	0.002
1031	0:20	0.002	1034	5:10	0.002
1031	0:30	0.01	1034	5:20	0.002
1031	0:40	0.02	1034	5:30	0.001
1031	0:50	0.055	1034	5:40	0.001
1031	1:00	0.066	1034	5:50	0.001
1031	1:10	0.066	1034	6:00	0.001
1031	1:20	0.066	1034	6:10	0.001
1031	1:30	0.055	1034	6:20	0.001
1031	1:40	0.035	1034	6:30	0.001
1031	1:50	0.029	1034	6:40	0.001
1031	2:00	0.022	1034	6:50	0.001
1031	2:10	0.019	1034	7:00	0.001
1031	2:20	0.016	1034	7:10	0.001
1031	2:30	0.014	1034	7:20	0.001
1031	2:40	0.013	1034	7:30	0.001
1031	2:50	0.012	1034	7:40	0.001
1031	3:00	0.01	1034	7:50	0.001
1031	3:10	0.008	1034	8:00	0.001
1031	3:20	0.007	1034	8:10	0.001
1031	3:30	0.005	1034	8:20	0.001
1031	3:40	0.004	1034	8:30	0.001
1031	3:50	0.003	1034	8:40	0
1031	4:00	0.003	1034	8:50	0
1031	4:10	0.002	1034	9:00	0
1031	4:20	0.002	1034	9:10	0
1031	4:30	0.002	1034	9:20	0
1031	4:40	0.001	1034	9:30	0
1031	4:50	0.001	1034	9:40	0
1031	5:00	0.001	1034	9:50	0
1031	5:10	0.001	1034	10:00	0
1031	5:20	0.001	1034	10:10	0
1031	5:30	0.001	1034	10:20	0
1031	5:40	0.001	1034	10:30	0
1031	5:50	0.001	1034	10:40	0
1031	6:00	0.001	1034	10:50	0
1031	6:10	0.001	1034	11:00	0
1031	6:20	0.001	1034	11:10	0
1031	6:30	0.001	1034	11:20	0
1031	6:40	0.001	1034	11:30	0
1031	6:50	0.001	1034	11:40	0
1031	7:00	0	1034	11:50	0
1031	7:10	0	1034	12:00	0
1031	7:20	0	1034		

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1034	12:10	0	1036	3:20	0.002
1034	12:20	0	1036	3:30	0.002
1034	12:30	0	1036	3:40	0.001
1034	12:40	0	1036	3:50	0.001
1034	12:50	0	1036	4:00	0.001
1034	13:00	0	1036	4:10	0.001
1034	13:10	0	1036	4:20	0.001
			1036	4:30	0.001
1035	0:00	0	1036	4:40	0.001
1035	0:10	0	1036	4:50	0.001
1035	0:20	0	1036	5:00	0.001
1035	0:30	0.002	1036	5:10	0.001
1035	0:40	0.004	1036	5:20	0
1035	0:50	0.022	1036	5:30	0
1035	1:00	0.044	1036	5:40	0
1035	1:10	0.039	1036	5:50	0
1035	1:20	0.028	1036	6:00	0
1035	1:30	0.024	1036	6:10	0
1035	1:40	0.019	1036	6:20	0
1035	1:50	0.016	1036	6:30	0
1035	2:00	0.013	1036	6:40	0
1035	2:10	0.012	1036	6:50	0
1035	2:20	0.01	1036	7:00	0
1035	2:30	0.009	1036	7:10	0
1035	2:40	0.007	1036	7:20	0
1035	2:50	0.007	1036	7:30	0
1035	3:00	0.006	1036	7:40	0
1035	3:10	0.005	1036	7:50	0
1035	3:20	0.004	1036	8:00	0
1035	3:30	0.004	1036	8:10	0
1035	3:40	0.003	1036	8:20	0
1035	3:50	0.003	1036	8:30	0
1035	4:00	0.003	1036	8:40	0
1035	4:10	0.002	1036	8:50	0
1035	4:20	0.002	1036	9:00	0
1035	4:30	0.002	1036	9:10	0
1035	4:40	0.002	1036	9:20	0
1035	4:50	0.001	1036	9:30	0
1035	5:00	0.001	1036	9:40	0
1035	5:10	0.001	1036	9:50	0
1035	5:20	0.001	1036	10:00	0
1035	5:30	0.001	1036	10:10	0
1035	5:40	0.001	1036	10:20	0
1035	5:50	0.001	1036	10:30	0
1035	6:00	0.001	1036	10:40	0
1035	6:10	0.001	1036	10:50	0
1035	6:20	0.001	1036	11:00	0
1035	6:30	0.001	1036	11:10	0
1035	6:40	0.001	1036	11:20	0
1035	6:50	0.001	1036	11:30	0
1035	7:00	0.001	1036	11:40	0
1035	7:10	0	1036	11:50	0
1035	7:20	0	1036	12:00	0
1035	7:30	0	1036	12:10	0
1035	7:40	0	1036	12:20	0
1035	7:50	0	1036	12:30	0
1035	8:00	0	1036	12:40	0
1035	8:10	0	1036	12:50	0
1035	8:20	0	1036	13:00	0
1035	8:30	0	1036	13:10	0
1035	8:40	0			
1035	8:50	0	1038	0:00	0
1035	9:00	0	1038	0:10	0
1035	9:10	0	1038	0:20	0.001
1035	9:20	0	1038	0:30	0.005
1035	9:30	0	1038	0:40	0.013
1035	9:40	0	1038	0:50	0.044
1035	9:50	0	1038	1:00	0.044
1035	10:00	0	1038	1:10	0.033
1035	10:10	0	1038	1:20	0.027
1035	10:20	0	1038	1:30	0.024
1035	10:30	0	1038	1:40	0.018
1035	10:40	0	1038	1:50	0.015
1035	10:50	0	1038	2:00	0.012
1035	11:00	0	1038	2:10	0.01
1035	11:10	0	1038	2:20	0.008
1035	11:20	0	1038	2:30	0.007
1035	11:30	0	1038	2:40	0.006
1035	11:40	0	1038	2:50	0.006
1035	11:50	0	1038	3:00	0.005
1035	12:00	0	1038	3:10	0.005
1035	12:10	0	1038	3:20	0.004
1035	12:20	0	1038	3:30	0.003
1035	12:30	0	1038	3:40	0.003
1035	12:40	0	1038	3:50	0.002
1035	12:50	0	1038	4:00	0.002
1035	13:00	0	1038	4:10	0.002
1035	13:10	0	1038	4:20	0.002
			1038	4:30	0.001
1036	0:00	0	1038	4:40	0.001
1036	0:10	0	1038	4:50	0.001
1036	0:20	0.001	1038	5:00	0.001
1036	0:30	0.006	1038	5:10	0.001
1036	0:40	0.013	1038	5:20	0.001
1036	0:50	0.044	1038	5:30	0.001
1036	1:00	0.044	1038	5:40	0.001
1036	1:10	0.044	1038	5:50	0.001
1036	1:20	0.044	1038	6:00	0.001
1036	1:30	0.032	1038	6:10	0.001
1036	1:40	0.017	1038	6:20	0.001
1036	1:50	0.012	1038	6:30	0.001
1036	2:00	0.008	1038	6:40	0.001
1036	2:10	0.007	1038	6:50	0.001
1036	2:20	0.006	1038	7:00	0.001
1036	2:30	0.005	1038	7:10	0.001
1036	2:40	0.005	1038	7:20	0.001
1036	2:50	0.004	1038	7:30	0.001
1036	3:00	0.004	1038	7:40	0.001
1036	3:10	0.003	1038	7:50	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1038	8:00	0	1039	12:40	0
1038	8:10	0	1039	12:50	0
1038	8:20	0	1039	13:00	0
1038	8:30	0	1039	13:10	0
1038	8:40	0			
1038	8:50	0	;Design inflow to MH1043 from Oct 2007 Phl Subdivision design		
1038	9:00	0	1043	0:00	0
1038	9:10	0	1043	0:10	0
1038	9:20	0	1043	0:20	0.001
1038	9:30	0	1043	0:30	0.004
1038	9:40	0	1043	0:40	0.01
1038	9:50	0	1043	0:50	0.052
1038	10:00	0	1043	1:00	0.066
1038	10:10	0	1043	1:10	0.062
1038	10:20	0	1043	1:20	0.031
1038	10:30	0	1043	1:30	0.015
1038	10:40	0	1043	1:40	0.009
1038	10:50	0	1043	1:50	0.007
1038	11:00	0	1043	2:00	0.005
1038	11:10	0	1043	2:10	0.005
1038	11:20	0	1043	2:20	0.004
1038	11:30	0	1043	2:30	0.004
1038	11:40	0	1043	2:40	0.003
1038	11:50	0	1043	2:50	0.003
1038	12:00	0	1043	3:00	0.003
1038	12:10	0	1043	3:10	0.002
1038	12:20	0	1043	3:20	0.002
1038	12:30	0	1043	3:30	0.001
1038	12:40	0	1043	3:40	0.001
1038	12:50	0	1043	3:50	0.001
1038	13:00	0	1043	4:00	0.001
1038	13:10	0	1043	4:10	0.001
			1043	4:20	0.001
1039	0:00	0	1043	4:30	0.001
1039	0:10	0	1043	4:40	0.001
1039	0:20	0	1043	4:50	0.001
1039	0:30	0.002	1043	5:00	0.001
1039	0:40	0.006	1043	5:10	0.001
1039	0:50	0.022	1043	5:20	0.001
1039	1:00	0.022	1043	5:30	0.001
1039	1:10	0.022	1043	5:40	0.001
1039	1:20	0.022	1043	5:50	0
1039	1:30	0.019	1043	6:00	0
1039	1:40	0.014	1043	6:10	0
1039	1:50	0.01	1043	6:20	0
1039	2:00	0.008	1043	6:30	0
1039	2:10	0.006	1043	6:40	0
1039	2:20	0.005	1043	6:50	0
1039	2:30	0.004	1043	7:00	0
1039	2:40	0.004	1043	7:10	0
1039	2:50	0.003	1043	7:20	0
1039	3:00	0.003	1043	7:30	0
1039	3:10	0.003	1043	7:40	0
1039	3:20	0.002	1043	7:50	0
1039	3:30	0.002	1043	8:00	0
1039	3:40	0.002	1043	8:10	0
1039	3:50	0.001	1043	8:20	0
1039	4:00	0.001	1043	8:30	0
1039	4:10	0.001	1043	8:40	0
1039	4:20	0.001	1043	8:50	0
1039	4:30	0.001	1043	9:00	0
1039	4:40	0.001	1043	9:10	0
1039	4:50	0.001	1043	9:20	0
1039	5:00	0.001	1043	9:30	0
1039	5:10	0.001	1043	9:40	0
1039	5:20	0.001	1043	9:50	0
1039	5:30	0.001	1043	10:00	0
1039	5:40	0	1043	10:10	0
1039	5:50	0	1043	10:20	0
1039	6:00	0	1043	10:30	0
1039	6:10	0	1043	10:40	0
1039	6:20	0	1043	10:50	0
1039	6:30	0	1043	11:00	0
1039	6:40	0	1043	11:10	0
1039	6:50	0	1043	11:20	0
1039	7:00	0	1043	11:30	0
1039	7:10	0	1043	11:40	0
1039	7:20	0	1043	11:50	0
1039	7:30	0	1043	12:00	0
1039	7:40	0	1043	12:10	0
1039	7:50	0	1043	12:20	0
1039	8:00	0	1043	12:30	0
1039	8:10	0	1043	12:40	0
1039	8:20	0	1043	12:50	0
1039	8:30	0	1043	13:00	0
1039	8:40	0	1043	13:10	0
1039	8:50	0			
1039	9:00	0	1044	0:00	0
1039	9:10	0	1044	0:10	0
1039	9:20	0	1044	0:20	0.001
1039	9:30	0	1044	0:30	0.004
1039	9:40	0	1044	0:40	0.011
1039	9:50	0	1044	0:50	0.022
1039	10:00	0	1044	1:00	0.022
1039	10:10	0	1044	1:10	0.022
1039	10:20	0	1044	1:20	0.019
1039	10:30	0	1044	1:30	0.014
1039	10:40	0	1044	1:40	0.009
1039	10:50	0	1044	1:50	0.007
1039	11:00	0	1044	2:00	0.006
1039	11:10	0	1044	2:10	0.005
1039	11:20	0	1044	2:20	0.004
1039	11:30	0	1044	2:30	0.004
1039	11:40	0	1044	2:40	0.004
1039	11:50	0	1044	2:50	0.003
1039	12:00	0	1044	3:00	0.003
1039	12:10	0	1044	3:10	0.003
1039	12:20	0	1044	3:20	0.002
1039	12:30	0	1044	3:30	0.002

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1044	3:40	0.001	1045	8:20	0
1044	3:50	0.001	1045	8:30	0
1044	4:00	0.001	1045	8:40	0
1044	4:10	0.001	1045	8:50	0
1044	4:20	0.001	1045	9:00	0
1044	4:30	0.001	1045	9:10	0
1044	4:40	0.001	1045	9:20	0
1044	4:50	0.001	1045	9:30	0
1044	5:00	0.001	1045	9:40	0
1044	5:10	0.001	1045	9:50	0
1044	5:20	0.001	1045	10:00	0
1044	5:30	0	1045	10:10	0
1044	5:40	0	1045	10:20	0
1044	5:50	0	1045	10:30	0
1044	6:00	0	1045	10:40	0
1044	6:10	0	1045	10:50	0
1044	6:20	0	1045	11:00	0
1044	6:30	0	1045	11:10	0
1044	6:40	0	1045	11:20	0
1044	6:50	0	1045	11:30	0
1044	7:00	0	1045	11:40	0
1044	7:10	0	1045	11:50	0
1044	7:20	0	1045	12:00	0
1044	7:30	0	1045	12:10	0
1044	7:40	0	1045	12:20	0
1044	7:50	0	1045	12:30	0
1044	8:00	0	1045	12:40	0
1044	8:10	0	1045	12:50	0
1044	8:20	0	1045	13:00	0
1044	8:30	0	1045	13:10	0
1044	8:40	0			
1044	8:50	0	1046	0:00	0
1044	9:00	0	1046	0:10	0
1044	9:10	0	1046	0:20	0
1044	9:20	0	1046	0:30	0.002
1044	9:30	0	1046	0:40	0.008
1044	9:40	0	1046	0:50	0.022
1044	9:50	0	1046	1:00	0.022
1044	10:00	0	1046	1:10	0.022
1044	10:10	0	1046	1:20	0.018
1044	10:20	0	1046	1:30	0.012
1044	10:30	0	1046	1:40	0.009
1044	10:40	0	1046	1:50	0.006
1044	10:50	0	1046	2:00	0.005
1044	11:00	0	1046	2:10	0.004
1044	11:10	0	1046	2:20	0.004
1044	11:20	0	1046	2:30	0.003
1044	11:30	0	1046	2:40	0.003
1044	11:40	0	1046	2:50	0.003
1044	11:50	0	1046	3:00	0.003
1044	12:00	0	1046	3:10	0.003
1044	12:10	0	1046	3:20	0.002
1044	12:20	0	1046	3:30	0.002
1044	12:30	0	1046	3:40	0.002
1044	12:40	0	1046	3:50	0.001
1044	12:50	0	1046	4:00	0.001
1044	13:00	0	1046	4:10	0.001
1044	13:10	0	1046	4:20	0.001
			1046	4:30	0.001
1045	0:00	0	1046	4:40	0.001
1045	0:10	0	1046	4:50	0.001
1045	0:20	0.001	1046	5:00	0.001
1045	0:30	0.006	1046	5:10	0.001
1045	0:40	0.011	1046	5:20	0.001
1045	0:50	0.044	1046	5:30	0.001
1045	1:00	0.044	1046	5:40	0.001
1045	1:10	0.044	1046	5:50	0.001
1045	1:20	0.044	1046	6:00	0.001
1045	1:30	0.041	1046	6:10	0.001
1045	1:40	0.028	1046	6:20	0.001
1045	1:50	0.022	1046	6:30	0.001
1045	2:00	0.016	1046	6:40	0
1045	2:10	0.013	1046	6:50	0
1045	2:20	0.011	1046	7:00	0
1045	2:30	0.009	1046	7:10	0
1045	2:40	0.008	1046	7:20	0
1045	2:50	0.007	1046	7:30	0
1045	3:00	0.007	1046	7:40	0
1045	3:10	0.005	1046	7:50	0
1045	3:20	0.005	1046	8:00	0
1045	3:30	0.004	1046	8:10	0
1045	3:40	0.003	1046	8:20	0
1045	3:50	0.003	1046	8:30	0
1045	4:00	0.002	1046	8:40	0
1045	4:10	0.002	1046	8:50	0
1045	4:20	0.002	1046	9:00	0
1045	4:30	0.002	1046	9:10	0
1045	4:40	0.001	1046	9:20	0
1045	4:50	0.001	1046	9:30	0
1045	5:00	0.001	1046	9:40	0
1045	5:10	0.001	1046	9:50	0
1045	5:20	0.001	1046	10:00	0
1045	5:30	0.001	1046	10:10	0
1045	5:40	0.001	1046	10:20	0
1045	5:50	0.001	1046	10:30	0
1045	6:00	0.001	1046	10:40	0
1045	6:10	0.001	1046	10:50	0
1045	6:20	0.001	1046	11:00	0
1045	6:30	0.001	1046	11:10	0
1045	6:40	0.001	1046	11:20	0
1045	6:50	0.001	1046	11:30	0
1045	7:00	0.001	1046	11:40	0
1045	7:10	0.001	1046	11:50	0
1045	7:20	0.001	1046	12:00	0
1045	7:30	0	1046	12:10	0
1045	7:40	0	1046	12:20	0
1045	7:50	0	1046	12:30	0
1045	8:00	0	1046	12:40	0
1045	8:10	0	1046	12:50	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1046	13:00	0		1048A	3:50	0.001
1046	13:10	0		1048A	4:00	0.001
				1048A	4:10	0.001
;Design inflow to MH1047 from Oct 2007 Phl Subdivision design				1048A	4:20	0.001
1047	0:00	0		1048A	4:30	0.001
1047	0:10	0		1048A	4:40	0.001
1047	0:20	0.001		1048A	4:50	0.001
1047	0:30	0.003		1048A	5:00	0.001
1047	0:40	0.008		1048A	5:10	0.001
1047	0:50	0.022		1048A	5:20	0.001
1047	1:00	0.022		1048A	5:30	0.001
1047	1:10	0.022		1048A	5:40	0.001
1047	1:20	0.011		1048A	5:50	0.001
1047	1:30	0.006		1048A	6:00	0.001
1047	1:40	0.005		1048A	6:10	0.001
1047	1:50	0.004		1048A	6:20	0
1047	2:00	0.004		1048A	6:30	0
1047	2:10	0.002		1048A	6:40	0
1047	2:20	0.003		1048A	6:50	0
1047	2:30	0.002		1048A	7:00	0
1047	2:40	0.002		1048A	7:10	0
1047	2:50	0.002		1048A	7:20	0
1047	3:00	0.002		1048A	7:30	0
1047	3:10	0.002		1048A	7:40	0
1047	3:20	0.002		1048A	7:50	0
1047	3:30	0.001		1048A	8:00	0
1047	3:40	0.001		1048A	8:10	0
1047	3:50	0.001		1048A	8:20	0
1047	4:00	0.001		1048A	8:30	0
1047	4:10	0.001		1048A	8:40	0
1047	4:20	0.001		1048A	8:50	0
1047	4:30	0.001		1048A	9:00	0
1047	4:40	0.001		1048A	9:10	0
1047	4:50	0.001		1048A	9:20	0
1047	5:00	0.001		1048A	9:30	0
1047	5:10	0.001		1048A	9:40	0
1047	5:20	0.001		1048A	9:50	0
1047	5:30	0		1048A	10:00	0
1047	5:40	0		1048A	10:10	0
1047	5:50	0		1048A	10:20	0
1047	6:00	0		1048A	10:30	0
1047	6:10	0		1048A	10:40	0
1047	6:20	0		1048A	10:50	0
1047	6:30	0		1048A	11:00	0
1047	6:40	0		1048A	11:10	0
1047	6:50	0		1048A	11:20	0
1047	7:00	0		1048A	11:30	0
1047	7:10	0		1048A	11:40	0
1047	7:20	0		1048A	11:50	0
1047	7:30	0		1048A	12:00	0
1047	7:40	0		1048A	12:10	0
1047	7:50	0		1048A	12:20	0
1047	8:00	0		1048A	12:30	0
1047	8:10	0		1048A	12:40	0
1047	8:20	0		1048A	12:50	0
1047	8:30	0		1048A	13:00	0
1047	8:40	0		1048A	13:10	0
1047	8:50	0				
1047	9:00	0		;Design inflow to MH1051 from Oct 2007 Phl Subdivision design		
1047	9:10	0		1051	0:00	0
1047	9:20	0		1051	0:10	0
1047	9:30	0		1051	0:20	0
1047	9:40	0		1051	0:30	0.002
1047	9:50	0		1051	0:40	0.002
1047	10:00	0		1051	0:50	0
1047	10:10	0		1051	1:00	0.022
1047	10:20	0		1051	1:10	0.022
1047	10:30	0		1051	1:20	0.022
1047	10:40	0		1051	1:30	0.016
1047	10:50	0		1051	1:40	0.007
1047	11:00	0		1051	1:50	0.006
1047	11:10	0		1051	2:00	0.003
1047	11:20	0		1051	2:10	0.003
1047	11:30	0		1051	2:20	0.002
1047	11:40	0		1051	2:30	0.002
1047	11:50	0		1051	2:40	0.002
1047	12:00	0		1051	2:50	0.002
1047	12:10	0		1051	3:00	0.002
1047	12:20	0		1051	3:10	0.001
1047	12:30	0		1051	3:20	0.001
1047	12:40	0		1051	3:30	0.001
1047	12:50	0		1051	3:40	0.001
1047	13:00	0		1051	3:50	0
1047	13:10	0		1051	4:00	0
				1051	4:10	0
;Design inflow to MH1048A from Oct 2007 Phl Subdivision design				1051	4:20	0
1048A	0:00	0		1051	4:30	0
1048A	0:10	0		1051	4:40	0
1048A	0:20	0.001		1051	4:50	0
1048A	0:30	0.007		1051	5:00	0
1048A	0:40	0.01		1051	5:10	0
1048A	0:50	0.044		1051	5:20	0
1048A	1:00	0.044		1051	5:30	0
1048A	1:10	0.044		1051	5:40	0
1048A	1:20	0.044		1051	5:50	0
1048A	1:30	0.044		1051	6:00	0
1048A	1:40	0.044		1051	6:10	0
1048A	1:50	0.034		1051	6:20	0
1048A	2:00	0.012		1051	6:30	0
1048A	2:10	0.006		1051	6:40	0
1048A	2:20	0.008		1051	6:50	0
1048A	2:30	0.005		1051	7:00	0
1048A	2:40	0.006		1051	7:10	0
1048A	2:50	0.005		1051	7:20	0
1048A	3:00	0.005		1051	7:30	0
1048A	3:10	0.004		1051	7:40	0
1048A	3:20	0.003		1051	7:50	0
1048A	3:30	0.002		1051	8:00	0
1048A	3:40	0.002		1051	8:10	0

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

1051	8:20	0	1053	13:00	0
1051	8:30	0	1053	13:10	0
1051	8:40	0			
1051	8:50	0			
1051	9:00	0	;Design inflow to MH2000 from Oct 2007 Ph2 Subdivision design		
1051	9:10	0	2000	0:00	0
1051	9:20	0	2000	0:10	0
1051	9:30	0	2000	0:20	0.001
1051	9:40	0	2000	0:30	0.003
1051	9:50	0	2000	0:40	0.008
1051	10:00	0	2000	0:50	0.022
1051	10:10	0	2000	1:00	0.022
1051	10:20	0	2000	1:10	0.011
1051	10:30	0	2000	1:20	0.008
1051	10:40	0	2000	1:30	0.005
1051	10:50	0	2000	1:40	0.004
1051	11:00	0	2000	1:50	0.003
1051	11:10	0	2000	2:00	0.002
1051	11:20	0	2000	2:10	0.002
1051	11:30	0	2000	2:20	0.002
1051	11:40	0	2000	2:30	0.002
1051	11:50	0	2000	2:40	0.002
1051	12:00	0	2000	2:50	0.001
1051	12:10	0	2000	3:00	0.001
1051	12:20	0	2000	3:10	0.001
1051	12:30	0	2000	3:20	0.001
1051	12:40	0	2000	3:30	0.001
1051	12:50	0	2000	3:40	0.001
1051	13:00	0	2000	3:50	0.001
1051	13:10	0	2000	4:00	0.001
			2000	4:10	0
			2000	4:20	0
			2000	4:30	0
			2000	4:40	0
1053	0:00	0	2000	4:50	0
1053	0:10	0	2000	5:00	0
1053	0:20	0.002	2000	5:10	0
1053	0:30	0.008	2000	5:20	0
1053	0:40	0.019	2000	5:30	0
1053	0:50	0.082	2000	5:40	0
1053	1:00	0.088	2000	5:50	0
1053	1:10	0.078	2000	6:00	0
1053	1:20	0.047	2000	6:10	0
1053	1:30	0.038	2000	6:20	0
1053	1:40	0.025	2000	6:30	0
1053	1:50	0.02	2000	6:40	0
1053	2:00	0.015	2000	6:50	0
1053	2:10	0.012	2000	7:00	0
1053	2:20	0.01			
1053	2:30	0.009	2000		
1053	2:40	0.008			
1053	2:50	0.007	2000A	0:00	0
1053	3:00	0.006	2000A	0:10	0
1053	3:10	0.005	2000A	0:20	0
1053	3:20	0.004	2000A	0:30	0.001
1053	3:30	0.004	2000A	0:40	0.003
1053	3:40	0.003	2000A	0:50	0.044
1053	3:50	0.003	2000A	1:00	0.041
1053	4:00	0.002	2000A	1:10	0.024
1053	4:10	0.002	2000A	1:20	0.009
1053	4:20	0.002	2000A	1:30	0.007
1053	4:30	0.002	2000A	1:40	0.006
1053	4:40	0.001	2000A	1:50	0.004
1053	4:50	0.001	2000A	2:00	0.004
1053	5:00	0.001	2000A	2:10	0.003
1053	5:10	0.001	2000A	2:20	0.003
1053	5:20	0.001	2000A	2:30	0.003
1053	5:30	0.001	2000A	2:40	0.002
1053	5:40	0.001	2000A	2:50	0.002
1053	5:50	0.001	2000A	3:00	0.002
1053	6:00	0.001	2000A	3:10	0.002
1053	6:10	0.001	2000A	3:20	0.002
1053	6:20	0.001	2000A	3:30	0.002
1053	6:30	0.001	2000A	3:40	0.001
1053	6:40	0.001	2000A	3:50	0.001
1053	6:50	0.001	2000A	4:00	0.001
1053	7:00	0.001	2000A	4:10	0.001
1053	7:10	0.001	2000A	4:20	0.001
1053	7:20	0.001	2000A	4:30	0.001
1053	7:30	0.001	2000A	4:40	0.001
1053	7:40	0.001	2000A	4:50	0.001
1053	7:50	0.001	2000A	5:00	0.001
1053	8:00	0.001	2000A	5:10	0.001
1053	8:10	0.001	2000A	5:20	0.001
1053	8:20	0	2000A	5:30	0.001
1053	8:30	0	2000A	5:40	0.001
1053	8:40	0	2000A	5:50	0.001
1053	8:50	0	2000A	6:00	0.001
1053	9:00	0	2000A	6:10	0.001
1053	9:10	0	2000A	6:20	0.001
1053	9:20	0	2000A	6:30	0
1053	9:30	0	2000A	6:40	0
1053	9:40	0	2000A	6:50	0
1053	9:50	0	2000A	7:00	0
1053	10:00	0			
1053	10:10	0	;Design inflow to MH2001 from Oct 2007 Ph2 Subdivision design		
1053	10:20	0	2001	0:00	0
1053	10:30	0	2001	0:10	0
1053	10:40	0	2001	0:20	0.001
1053	10:50	0	2001	0:30	0.003
1053	11:00	0	2001	0:40	0.009
1053	11:10	0	2001	0:50	0.022
1053	11:20	0	2001	1:00	0.022
1053	11:30	0	2001	1:10	0.016
1053	11:40	0	2001	1:20	0.01
1053	11:50	0	2001	1:30	0.008
1053	12:00	0	2001	1:40	0.005
1053	12:10	0	2001	1:50	0.004
1053	12:20	0	2001	2:00	0.003
1053	12:30	0	2001	2:10	0.003
1053	12:40	0	2001	2:20	0.003
1053	12:50	0	2001	2:30	0.002

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

2001	2:40	0.002	August4_1988	3:00	12.8
2001	2:50	0.002	August4_1988	3:05	14
2001	3:00	0.002	August4_1988	3:10	22.2
2001	3:10	0.001	August4_1988	3:15	21.8
2001	3:20	0.001	August4_1988	3:20	1.4
2001	3:30	0.001	August4_1988	3:25	0.2
2001	3:40	0.001	August4_1988	3:30	0.2
2001	3:50	0.001	August4_1988	3:35	0.2
2001	4:00	0.001	August4_1988	3:40	0.2
2001	4:10	0	August4_1988	3:45	0.2
2001	4:20	0	August4_1988	3:50	0.2
2001	4:30	0	August4_1988	3:55	0.2
2001	4:40	0	August4_1988	4:00	0.2
2001	4:50	0	August4_1988	4:05	0.2
2001	5:00	0	August4_1988	4:10	0.2
2001	5:10	0	August4_1988	4:15	0.2
2001	5:20	0	August4_1988	4:20	0.2
2001	5:30	0	August4_1988	4:25	0.2
2001	5:40	0	August4_1988	4:30	0.2
2001	5:50	0	August4_1988	4:35	0.2
2001	6:00	0	August4_1988	4:40	0.2
2001	6:10	0	August4_1988	4:45	0.2
2001	6:20	0	August4_1988	4:50	0.2
2001	6:30	0	August4_1988	4:55	0.2
2001	6:40	0	August4_1988	5:00	2.9
2001	6:50	0	August4_1988	5:05	7.8
2001	7:00	0	August4_1988	5:10	10
			August4_1988	5:15	6.3
			August4_1988	5:20	5.1
			August4_1988	5:25	9.8
			August4_1988	5:30	2.6
			August4_1988	5:35	1.7
;Design inflow to MH2004 from Oct 2007 Ph2 Subdivision design					
2004	0:00	0			
2004	0:10	0			
2004	0:20	0.002			
2004	0:30	0.007			
2004	0:40	0.017	August8_1996	0:00	4
2004	0:50	0.044	August8_1996	0:05	11.9
2004	1:00	0.044	August8_1996	0:10	26.5
2004	1:10	0.033	August8_1996	0:15	13.3
2004	1:20	0.019	August8_1996	0:20	0
2004	1:30	0.011	August8_1996	0:25	2.7
2004	1:40	0.008	August8_1996	0:30	0
2004	1:50	0.007	August8_1996	0:35	8
2004	2:00	0.005	August8_1996	0:40	18.6
2004	2:10	0.005	August8_1996	0:45	10.6
2004	2:20	0.004	August8_1996	0:50	21.2
2004	2:30	0.004	August8_1996	0:55	2.7
2004	2:40	0.004	August8_1996	1:00	2.7
2004	2:50	0.003	August8_1996	1:05	15.9
2004	3:00	0.003	August8_1996	1:10	66.3
2004	3:10	0.002	August8_1996	1:15	55.7
2004	3:20	0.002	August8_1996	1:20	122
2004	3:30	0.001	August8_1996	1:25	88.9
2004	3:40	0.001	August8_1996	1:30	9.63
2004	3:50	0.001	August8_1996	1:35	8
2004	4:00	0.001	August8_1996	1:40	4
2004	4:10	0.001	August8_1996	1:45	0
2004	4:20	0.001	August8_1996	1:50	2.7
2004	4:30	0.001	August8_1996	1:55	0
2004	4:40	0.001	August8_1996	2:00	0
2004	4:50	0.001	August8_1996	2:05	0
2004	5:00	0.001	August8_1996	2:10	5.3
2004	5:10	0.001	August8_1996	2:15	0
2004	5:20	0	August8_1996	2:20	0
2004	5:30	0	August8_1996	2:25	0
2004	5:40	0	August8_1996	2:30	0
2004	5:50	0	August8_1996	2:35	0
2004	6:00	0	August8_1996	2:40	0
2004	6:10	0	August8_1996	2:45	4
2004	6:20	0	August8_1996	2:50	53.1
2004	6:30	0	August8_1996	2:55	69
2004	6:40	0	August8_1996	3:00	63.7
2004	6:50	0	August8_1996	3:05	58.4
2004	7:00	0	August8_1996	3:10	47.8
			August8_1996	3:15	15.9
August4_1988	0:00	0	August8_1996	3:20	13.3
August4_1988	0:05	0.1	August8_1996	3:25	8
August4_1988	0:10	0.1	August8_1996	3:30	5.3
August4_1988	0:15	0	August8_1996	3:35	6.6
August4_1988	0:20	3.7	August8_1996	3:40	2.7
August4_1988	0:25	6.2	August8_1996	3:45	4
August4_1988	0:30	101.5	August8_1996	3:50	2.7
August4_1988	0:35	15.5	August8_1996	3:55	4
August4_1988	0:40	29.3	August8_1996	4:00	2.7
August4_1988	0:45	19.8	August8_1996	4:05	5.3
August4_1988	0:50	1.5	August8_1996	4:10	4
August4_1988	0:55	1.7	August8_1996	4:15	2.7
August4_1988	1:00	5.4	August8_1996	4:20	4
August4_1988	1:05	24.6	August8_1996	4:25	2.7
August4_1988	1:10	26.5	August8_1996	4:30	1.3
August4_1988	1:15	34.9	August8_1996	4:35	1.3
August4_1988	1:20	10.2	August8_1996	4:40	0
August4_1988	1:25	27.1	August8_1996	4:45	0
August4_1988	1:30	104.4	August8_1996	4:50	0
August4_1988	1:35	27.5	August8_1996	4:55	0
August4_1988	1:40	62.5	August8_1996	5:00	2.7
August4_1988	1:45	31.8	August8_1996	5:05	0
August4_1988	1:50	79.8	August8_1996	5:10	0
August4_1988	1:55	67.5	August8_1996	5:15	0
August4_1988	2:00	156.2	August8_1996	5:20	0
August4_1988	2:05	5.1	August8_1996	5:25	0
August4_1988	2:10	0.2	August8_1996	5:30	0
August4_1988	2:15	0.2	August8_1996	5:35	0
August4_1988	2:20	0.2	August8_1996	5:40	1.3
August4_1988	2:25	0.2			
August4_1988	2:30	0.2			
August4_1988	2:35	0.2	Chicago_100yr_3h	0:00	0
August4_1988	2:40	0.2	Chicago_100yr_3h	0:10	6.05
August4_1988	2:45	0.2	Chicago_100yr_3h	0:20	7.54
August4_1988	2:50	0.2	Chicago_100yr_3h	0:30	10.16
August4_1988	2:55	0.2	Chicago_100yr_3h	0:40	15.97
August4_1988			Chicago_100yr_3h	0:50	40.65

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

SCS_Type_II_100yr_103.2mm	17:40	1.92	Node	2002	ExMH
SCS_Type_II_100yr_103.2mm	17:50	1.92	Node	2003	ExMH
SCS_Type_II_100yr_103.2mm	18:00	1.92	Node	2004	ExMH
SCS_Type_II_100yr_103.2mm	18:10	1.92	Node	302_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	18:20	1.92	Node	303_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	18:30	1.92	Node	304_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	18:40	1.92	Node	305_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	18:50	1.92	Node	306_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	19:00	1.92	Node	307_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	19:10	1.92	Node	308_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	19:20	1.92	Node	309_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	19:30	1.92	Node	310_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	19:40	1.92	Node	502_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	19:50	1.92	Node	503_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	20:00	1.28	Node	504_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	20:10	1.28	Node	505_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	20:20	1.28	Node	506_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	20:30	1.28	Node	507_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	20:40	1.28	Node	508_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	20:50	1.28	Node	509_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	21:00	1.28	Node	510_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	21:10	1.28	Node	511_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	21:20	1.28	Node	512_(C-STRM-NEW)	MH
SCS_Type_II_100yr_103.2mm	21:30	1.28	Node	CB-102	CB
SCS_Type_II_100yr_103.2mm	21:40	1.28	Node	CB-304	CB
SCS_Type_II_100yr_103.2mm	21:50	1.28	Node	CB-306	CB
SCS_Type_II_100yr_103.2mm	22:00	1.28	Node	CB-310	CB
SCS_Type_II_100yr_103.2mm	22:10	1.28	Node	CB-502	CB
SCS_Type_II_100yr_103.2mm	22:20	1.28	Node	CB-504	CB
SCS_Type_II_100yr_103.2mm	22:30	1.28	Node	CB-506	CB
SCS_Type_II_100yr_103.2mm	22:40	1.28	Node	CB-508	CB
SCS_Type_II_100yr_103.2mm	22:50	1.28	Node	CB-512	CB
SCS_Type_II_100yr_103.2mm	23:00	1.28	Node	STM100	MH
SCS_Type_II_100yr_103.2mm	23:10	1.28	Node	STM301	MH
SCS_Type_II_100yr_103.2mm	23:20	1.28	Node	STM501	MH
SCS_Type_II_100yr_103.2mm	23:30	1.28	Link	102-100_(C-STRM-NEW)	MinorSystem
SCS_Type_II_100yr_103.2mm	23:40	1.28	Link	103-102_(C-STRM-NEW)	MinorSystem
SCS_Type_II_100yr_103.2mm	23:50	1.28	Link	104-103_(C-STRM-NEW)	MinorSystem

[REPORT]

```

;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]
Node 2005 MH
Node Diversion-1053 MH
Node 1001 ExMH
Node 1002 ExMH
Node 1004 ExMH
Node 1005 ExMH
Node 1006 ExMH
Node 1006A ExMH
Node 1007 ExMH
Node 1008 ExMH
Node 1009 ExMH
Node 1010 ExMH
Node 1011 ExMH
Node 1012 ExMH
Node 1013 ExMH
Node 1014 ExMH
Node 1015 ExMH
Node 1016 ExMH
Node 1017 ExMH
Node 1018 ExMH
Node 1019 ExMH
Node 102_(C-STRM-NEW) MH
Node 1020 ExMH
Node 1021 ExMH
Node 1022 ExMH
Node 1023 ExMH
Node 1024 ExMH
Node 1025 ExMH
Node 1026 ExMH
Node 1027 ExMH
Node 1028 ExMH
Node 1029 ExMH
Node 103_(C-STRM-NEW) MH
Node 1030 ExMH
Node 1031 ExMH
Node 1033 ExMH
Node 1034 ExMH
Node 1035 ExMH
Node 1036 ExMH
Node 1037 ExMH
Node 1038 ExMH
Node 1039 ExMH
Node 104_(C-STRM-NEW) MH
Node 1040 ExMH
Node 1041 ExMH
Node 1042 ExMH
Node 1043 ExMH
Node 1043A ExMH
Node 1044 ExMH
Node 1045 ExMH
Node 1046 ExMH
Node 1047 ExMH
Node 1048 ExMH
Node 1048A ExMH
Node 1049 ExMH
Node 1050 ExMH
Node 1051 ExMH
Node 1052 ExMH
Node 1053 ExMH
Node 2000 ExMH
Node 2001 ExMH

Node 102_(C-STRM-NEW) MH
Node 103_(C-STRM-NEW) MH
Node 302_(C-STRM-NEW) MH
Node 303_(C-STRM-NEW) MH
Node 304_(C-STRM-NEW) MH
Node 305_(C-STRM-NEW) MH
Node 306_(C-STRM-NEW) MH
Node 307_(C-STRM-NEW) MH
Node 308_(C-STRM-NEW) MH
Node 309_(C-STRM-NEW) MH
Node 310_(C-STRM-NEW) MH
Node 502_(C-STRM-NEW) MH
Node 503_(C-STRM-NEW) MH
Node 504_(C-STRM-NEW) MH
Node 505_(C-STRM-NEW) MH
Node 506_(C-STRM-NEW) MH
Node 507_(C-STRM-NEW) MH
Node 508_(C-STRM-NEW) MH
Node 509_(C-STRM-NEW) MH
Node 510_(C-STRM-NEW) MH
Node 511_(C-STRM-NEW) MH
Node 512_(C-STRM-NEW) MH
Node CB-102 CB
Node CB-304 CB
Node CB-306 CB
Node CB-310 CB
Node CB-502 CB
Node CB-504 CB
Node CB-506 CB
Node CB-508 CB
Node CB-512 CB
Node STM100 MH
Node STM301 MH
Node STM501 MH
Link 102-100_(C-STRM-NEW) MinorSystem
Link 103-102_(C-STRM-NEW) MinorSystem
Link 104-103_(C-STRM-NEW) MinorSystem
Link 302-301_(C-STRM-NEW) MinorSystem
Link 303-302_(C-STRM-NEW) MinorSystem
Link 304-302_(C-STRM-NEW) MinorSystem
Link 305-304_(C-STRM-NEW) MinorSystem
Link 306-304_(C-STRM-NEW) MinorSystem
Link 307-306_(C-STRM-NEW) MinorSystem
Link 308-306_(C-STRM-NEW) MinorSystem
Link 309-308_(C-STRM-NEW) MinorSystem
Link 310-309_(C-STRM-NEW) MinorSystem
Link 502-501_(C-STRM-NEW) MinorSystem
Link 503-502_(C-STRM-NEW) MinorSystem
Link 504-503_(C-STRM-NEW) MinorSystem
Link 505-504_(C-STRM-NEW) MinorSystem
Link 506-504_(C-STRM-NEW) MinorSystem
Link 507-506_(C-STRM-NEW) MinorSystem
Link 508-506_(C-STRM-NEW) MinorSystem
Link 509-508_(C-STRM-NEW) MinorSystem
Link 510-508_(C-STRM-NEW) MinorSystem
Link 511-510_(C-STRM-NEW) MinorSystem
Link 512-511_(C-STRM-NEW) MinorSystem
Link C10 MajorSystem
Link C11 MajorSystem
Link C12 MajorSystem
Link C13 MajorSystem
Link C14 MajorSystem
Link C2 MajorSystem
Link C4 MajorSystem
Link C6 MajorSystem
Link C68 ExMinorSystem
Link C8 MajorSystem
Link C9 MajorSystem
Link L101 ExMinorSystem
Link L102 ExMinorSystem
Link L103A ExMinorSystem
Link L103B ExMinorSystem
Link L104 ExMinorSystem
Link L105 ExMinorSystem
Link L106 ExMinorSystem
Link L107 ExMinorSystem
Link L108 ExMinorSystem
Link L109 ExMinorSystem
Link L110 ExMinorSystem
Link L111 ExMinorSystem
Link L112 ExMinorSystem
Link L113 ExMinorSystem
Link L114 ExMinorSystem
Link L115 ExMinorSystem
Link L116 ExMinorSystem
Link L117 ExMinorSystem
Link L118 ExMinorSystem
Link L119 ExMinorSystem
Link L120 ExMinorSystem
Link L121 ExMinorSystem
Link L122 ExMinorSystem
Link L123 ExMinorSystem
Link L124 ExMinorSystem
Link L125 ExMinorSystem
Link L126 ExMinorSystem
Link L127 ExMinorSystem
Link L128 ExMinorSystem
Link L129 ExMinorSystem
Link L130 ExMinorSystem
Link L131 ExMinorSystem
Link L138 FutMinor
Link L139 FutMinor
Link L140 ExMinorSystem
Link L31 ExMinorSystem
Link L32 ExMinorSystem
Link L33 ExMinorSystem
Link L34 ExMinorSystem
Link L46-1 ExMinorSystem

```

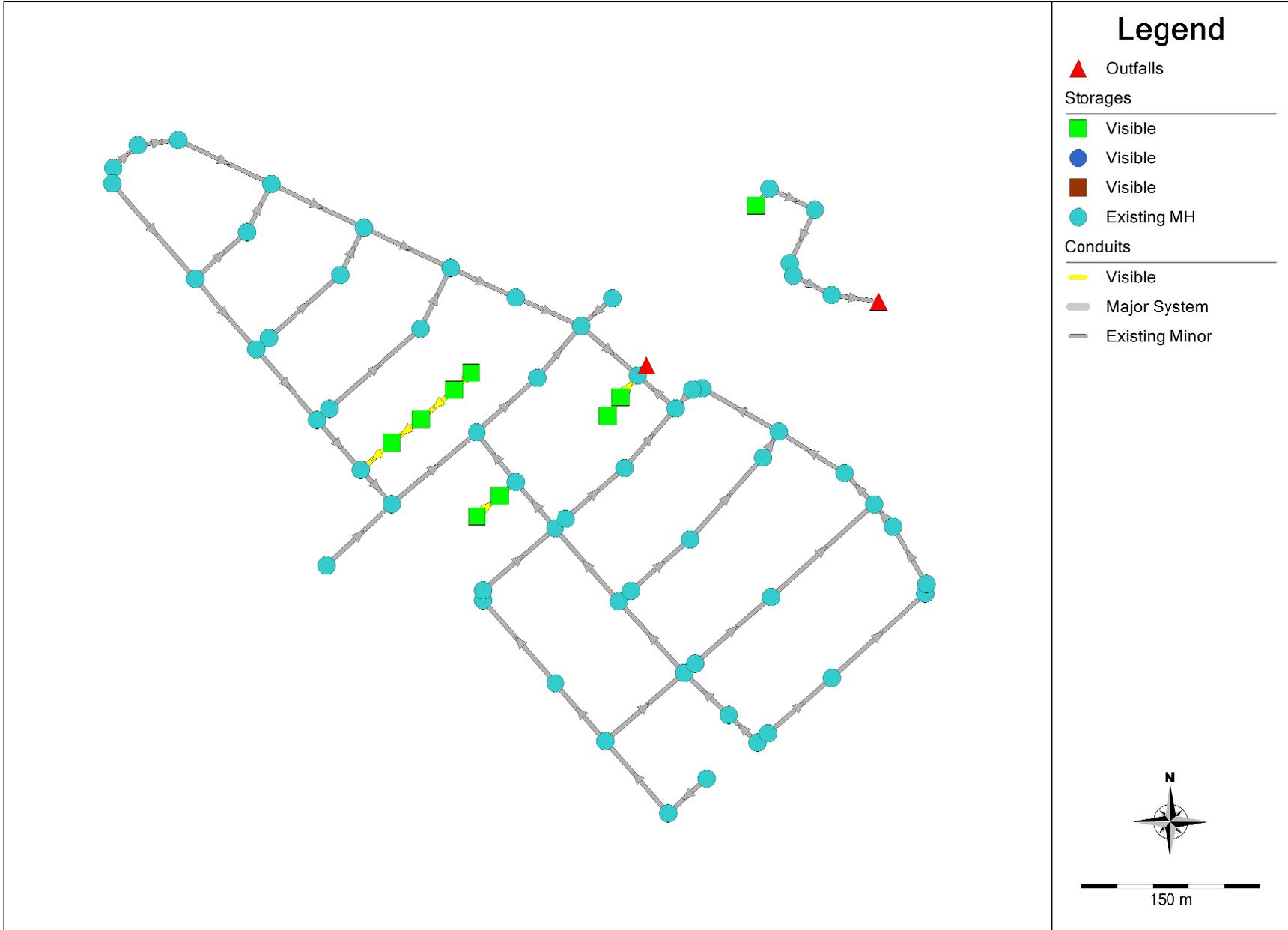

Trails West: Cope Drive Units – PCSWMM Input Data

Design Storm: 100 Year 3-hour Chicago Storm

C512A	354121.134	5015597.19
C512A	354124.975	5015592.577
C512A	354124.975	5015592.577
C512A	354129.181	5015587.726
C512A	354129.181	5015587.726
C512A	354132.286	5015580.787
C512A	354132.286	5015580.787
C512A	354134.454	5015573.402
C512A	354134.454	5015573.402
C512A	354145.33	5015560.857
C512A	354145.33	5015560.857
C512A	354142.912	5015558.761
C512A	354142.912	5015558.761
C512A	354139.094	5015555.563
C512A	354139.094	5015555.563
C512A	354132.518	5015549.701
C512A	354132.518	5015549.701
C512A	354111.985	5015573.383
C512A	354111.985	5015573.383
C512A	354106.523	5015572.61

[SYMBOLS]

```
;; Gage      X-Coord      Y-Coord  
;;-----
```



Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

```

[TITLE]
1001          95.32  2.93  0          FUNCTIONAL 0
;;Project Title/Notes
Cope Drive Units
1002          95.07  2.96  0          FUNCTIONAL 0
Option 1: Increase allowable minor system discharge
HGL-2007-100yr: Re-creation of 2007 subdivision HGL using DDSWMM
data files from Soho West Phase 1 and 2 (160400502)
1004          94.91  2.99  0          FUNCTIONAL 0
v0: Original simulation
0           1.13  0       0
1005          94.67  3.23  0          FUNCTIONAL 0
Official Disclaimer - Stantec Electronic Documents
0           1.13  0       0
*****
1006          94.44  3.31  0          FUNCTIONAL 0
Stantec assumes no responsibility for data supplied in electronic
format.
0           1.13  0       0
1006A         94.34  3.36  0          FUNCTIONAL 0
Such data is provided for convenience only and the recipient
accepts full responsibility for verifying the accuracy and
completeness of the data. The original hard copy of the data,
1007          95.16  2.84  0          FUNCTIONAL 0
which has been sealed and signed, shall constitute the official
documents of record for working purposes. In the event of
1008          94.89  3.06  0          FUNCTIONAL 0
inconsistencies between the electronic data and the hard copy
data, the hard copy data shall prevail.
1009          94.69  3.06  0          FUNCTIONAL 0
The recipient releases Stantec, its officers, employees,
consultants and agents, from any and all claims arising in any
1010          94.44  3.21  0          FUNCTIONAL 0
way from the content or provision of the data. Nothing herein
shall reduce or diminish Stantec's ownership of or copyright in
1011          94.11  3.29  0          FUNCTIONAL 0
the data or its compilation or arrangement. Any analyses,
programs, systems, software or formatting in the data shall be
1012          93.91  3.31  0          FUNCTIONAL 0
the property of Stantec. The recipient of this data is prohibited
from redistributing and from using any design or drawing
1012A         94.67  2.82  0          FUNCTIONAL 0
information contained within the data, in whole or in part, for
any other purpose than that for which it was originally designed
1012B         94.83  2.65  0          FUNCTIONAL 0
without the express written consent of Stantec.
0           1.13  0       0
;1.02
1013          93.61  2.99  0          FUNCTIONAL 0
[OPTIONS]
;Option
Value
FLOW_UNITS       CMS
;1.09
INFILTRATION     HORTON
FLOW_ROUTING     DYNWAVE
1014          93.53  3.29  0          FUNCTIONAL 0
LINK_OFFSETS     ELEVATION
MIN_SLOPE        0
;1.18
ALLOW_PONDING    NO
1015          93.43  3.48  0          FUNCTIONAL 0
SKIP_STEADY_STATE NO
;1.25
1016          93.33  3.45  0          FUNCTIONAL 0
START_DATE       02/10/2023
START_TIME       00:00:00
1017          95.14  2.56  0          FUNCTIONAL 0
REPORT_START_DATE 02/10/2023
REPORT_START_TIME 00:00:00
1018          94.85  2.95  0          FUNCTIONAL 0
END_DATE         02/11/2023
END_TIME         00:00:00
1019          94.59  2.91  0          FUNCTIONAL 0
SWEEP_START      01/01
SWEEP_END        12/31
1020          94.49  2.96  0          FUNCTIONAL 0
DRY_DAYS         0
REPORT_STEP      00:01:00
1021          94.38  2.92  0          FUNCTIONAL 0
WET_STEP         00:05:00
DRY_STEP         00:05:00
1022          94.88  2.62  0          FUNCTIONAL 0
ROUTING_STEP     5
RULE_STEP        00:00:00
1023          94.57  2.83  0          FUNCTIONAL 0
INERTIAL_DAMPING PARTIAL
1024          94.18  3.02  0          FUNCTIONAL 0
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
1025          94.04  3.21  0          FUNCTIONAL 0
VARIABLE_STEP    0
LENGHTENING_STEP 0
1026          94.76  2.79  0          FUNCTIONAL 0
MIN_SURFAREA     0
MAX_TRIALS       8
1027          94.44  2.99  0          FUNCTIONAL 0
HEAD_TOLERANCE   0.0015
SYS_FLOW_TOL     5
1028          94.18  2.92  0          FUNCTIONAL 0
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
1029          93.91  3.14  0          FUNCTIONAL 0
THREADS          4
1030          94.38  2.82  0          FUNCTIONAL 0
[EVAPORATION]
;Data Source
Parameters
;-----
CONSTANT          0.0
1031          94.01  3.09  0          FUNCTIONAL 0
DRY_ONLY          NO
0           1.13  0       0
1033          95.23  2.4  0          FUNCTIONAL 0
1034          95.11  2.39  0          FUNCTIONAL 0
[RAINGAGES]
;Name          Format      Interval SCF      Source
;-----
August4_1988   INTENSITY 0:05     1.0    TIMESERIES
0           1.13  0       0
August4_1988   INTENSITY 0:05     1.0    TIMESERIES
1036          94.6  2.85  0          FUNCTIONAL 0
August8_1996   INTENSITY 0:05     1.0    TIMESERIES
0           1.13  0       0
August8_1996   INTENSITY 0:05     1.0    TIMESERIES
1037          95.13  2.47  0          FUNCTIONAL 0
Chicago_100yr_3h INTENSITY 0:10     1.0    TIMESERIES
0           1.13  0       0
Chicago_100yr_3h INTENSITY 0:10     1.0    TIMESERIES
1038          94.94  2.51  0          FUNCTIONAL 0
Chicago_100yr+20%_3h INTENSITY 0:10     1.0    TIMESERIES
0           1.13  0       0
Chicago_100yr+20%_3h INTENSITY 0:10     1.0    TIMESERIES
1039          94.46  2.94  0          FUNCTIONAL 0
Chicago_2yr_3h INTENSITY 0:10     1.0    TIMESERIES
0           1.13  0       0
Chicago_2yr_3h INTENSITY 0:10     1.0    TIMESERIES
1040          94.76  2.49  0          FUNCTIONAL 0
Chicago_5yr_3h INTENSITY 0:10     1.0    TIMESERIES
0           1.13  0       0
Chicago_5yr_3h INTENSITY 0:10     1.0    TIMESERIES
1041          94.48  2.72  0          FUNCTIONAL 0
July1_1979     INTENSITY 1:00     1.0    TIMESERIES
0           1.13  0       0
July1_1979     INTENSITY 1:00     1.0    TIMESERIES
1042          94.09  3.11  0          FUNCTIONAL 0
SCS_Type_II_100yr_103.2mm INTENSITY 1:00     1.0    TIMESERIES
0           1.13  0       0
SCS_Type_II_100yr_103.2mm INTENSITY 1:00     1.0    TIMESERIES
1043          94.44  2.69  0          FUNCTIONAL 0
;0.45
1043A         94.23  2.73  0          FUNCTIONAL 0
[OUTFALLS]
;Name          Elevation Type      Stage Data      Gated
Route To
;-----
1043D         94.53  2.5  0          FUNCTIONAL 0
1043E         94.74  2.26 0          FUNCTIONAL 0
2005          93.73  FIXED   94.65         YES
1043F         94.91  1.95 0          FUNCTIONAL 0
Diversion-1053 93.09  FIXED   94.48         YES
1044          94.65  2.45 0          FUNCTIONAL 0
[STORAGE]
;Name          Elev.      MaxDepth      InitDepth      Shape      Curve
Name/Params    N/A       Fevap         Psi            Ksat        IMD
;-----
1045          94.25  2.8  0          FUNCTIONAL 0
1046          93.86  2.84 0          FUNCTIONAL 0
0           1.13  0       0
;0.84

```


Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

Node	Shape	Length (ft)	Flow (cfs)	Velocity (ft/s)	Flow (cfs)					
L111	CIRCULAR	0.45	0	0						
L112	CIRCULAR	0.45	0	0						
L113	CIRCULAR	0.525	0	0						
L114	CIRCULAR	0.6	0	0						
L115	CIRCULAR	0.75	0	0						
L116	CIRCULAR	0.825	0	0						
L117	CIRCULAR	0.825	0	0						
L118	CIRCULAR	0.3	0	0						
L119	CIRCULAR	0.3	0	0						
L120	CIRCULAR	0.375	0	0						
L121	CIRCULAR	0.375	0	0						
L122	CIRCULAR	0.45	0	0						
L123	CIRCULAR	0.525	0	0						
L124	CIRCULAR	0.6	0	0						
L125	CIRCULAR	0.375	0	0						
L126	CIRCULAR	0.45	0	0						
L127	CIRCULAR	0.375	0	0						
L128	CIRCULAR	0.525	0	0						
L129	CIRCULAR	0.45	0	0						
L130	CIRCULAR	0.825	0	0						
L131	RECT_CLOSED	1.2	1.8	0						
L133	CIRCULAR	0.25	0	0						
L134	CIRCULAR	0.3	0	0						
L135	CIRCULAR	0.3	0	0						
L136	CIRCULAR	0.375	0	0						
L138	CIRCULAR	0.3	0	0						
L139	CIRCULAR	0.3	0	0						
L140	CIRCULAR	0.375	0	0						
L141	CIRCULAR	0.375	0	0						
L142	CIRCULAR	0.375	0	0						
L31	CIRCULAR	0.45	0	0						
L32	CIRCULAR	0.45	0	0						
L33	CIRCULAR	0.45	0	0						
L34	CIRCULAR	0.525	0	0						
L46	RECT_CLOSED	1.2	1.8	0						
L47	RECT_CLOSED	1.2	1.8	0						
L48	RECT_CLOSED	1.2	2.4	0						
L49	RECT_CLOSED	1.2	1.8	0						
L50	RECT_CLOSED	1.2	1.8	0						
L56	CIRCULAR	0.45	0	0						
L57	CIRCULAR	0.3	0	0						
L59	CIRCULAR	0.75	0	0						
L60	CIRCULAR	0.825	0	0						
L83	CIRCULAR	0.45	0	0						
L84	CIRCULAR	0.525	0	0						
L85	CIRCULAR	0.525	0	0						
L86	CIRCULAR	0.6	0	0						
L87	CIRCULAR	0.6	0	0						
L88	CIRCULAR	0.75	0	0						
L89	CIRCULAR	0.825	0	0						
L90	CIRCULAR	0.825	0	0						
L91	CIRCULAR	0.825	0	0						
L92	CIRCULAR	0.9	0	0						
L93	CIRCULAR	0.45	0	0						
L94	CIRCULAR	0.6	0	0						
L95	CIRCULAR	0.375	0	0						
L96	CIRCULAR	0.525	0	0						
L97	CIRCULAR	0.6	0	0						
L98	CIRCULAR	0.375	0	0						

[TRANSECTS]									
;;Transect Data in HEC-2 format									
Node	Length (ft)	Flow (cfs)							
NC 0.025	0.025	0.013							
X1 18mROW	7	4.75	13.25	0.0	0.0				
GR 0.3	0	0.15	4.75	0	4.75	0.13			
GR 0.15	13.25	0.3	18						

[LOSSES]					
;;Link Seepage					
Node	Kentry	Kexit	Kavg	Flap Gate	
L142	0	0.4	0	NO	0
L47	0	0.4	0	NO	0
L92	0	0.4	0	NO	0

[INFLOWS]					
;;Node					
Mfactor	Sfactor	Constituent	Time Series	Type	
		Baseline Pattern			
;;					
1001		FLOW	1001	FLOW	1.0
1002		FLOW	1002	FLOW	1.0
1004		FLOW	1004	FLOW	1.0
1005		FLOW	1005	FLOW	1.0
1006		FLOW	1006	FLOW	1.0
1008		FLOW	1008	FLOW	1.0
1009		FLOW	1009	FLOW	1.0
1010		FLOW	1010	FLOW	1.0
1011		FLOW	1011	FLOW	1.0
1012A		FLOW	1012A	FLOW	1.0
1012B		FLOW	1012B	FLOW	1.0
1013		FLOW	1013	FLOW	1.0
1014		FLOW	1014	FLOW	1.0
1015		FLOW	1015	FLOW	1.0
1016		FLOW	1016	FLOW	1.0
1017		FLOW	1017	FLOW	1.0
1018		FLOW	1018	FLOW	1.0
1020		FLOW	1020	FLOW	1.0
1022		FLOW	1022	FLOW	1.0
1023		FLOW	1023	FLOW	1.0
1025		FLOW	1025	FLOW	1.0
1026		FLOW	1026	FLOW	1.0
1027		FLOW	1027	FLOW	1.0
1028		FLOW	1028	FLOW	1.0
1030		FLOW	1030	FLOW	1.0
1031		FLOW	1031	FLOW	1.0
1034		FLOW	1034	FLOW	1.0
1035		FLOW	1035	FLOW	1.0
1036		FLOW	1036	FLOW	1.0
1038		FLOW	1038	FLOW	1.0
1039		FLOW	1039	FLOW	1.0
1043		FLOW	1043	FLOW	1.0
1043D		FLOW	1043D	FLOW	1.0
1043E		FLOW	1043E	FLOW	1.0
1043F		FLOW	1043F	FLOW	1.0
1044		FLOW	1044	FLOW	1.0
1045		FLOW	1045	FLOW	1.0

Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

1002	11:30	0	1005	2:40	0.006
1002	11:40	0	1005	2:50	0.005
1002	11:50	0	1005	3:00	0.005
1002	12:00	0	1005	3:10	0.004
1002	12:10	0	1005	3:20	0.004
1002	12:20	0	1005	3:30	0.003
1002	12:30	0	1005	3:40	0.002
1002	12:40	0	1005	3:50	0.002
1002	12:50	0	1005	4:00	0.002
1002	13:00	0	1005	4:10	0.001
1002	13:10	0	1005	4:20	0.001
			1005	4:30	0.001
1004	0:00	0	1005	4:40	0.001
1004	0:10	0	1005	4:50	0.001
1004	0:20	0.002	1005	5:00	0.001
1004	0:30	0.006	1005	5:10	0.001
1004	0:40	0.014	1005	5:20	0.001
1004	0:50	0.033	1005	5:30	0.001
1004	1:00	0.044	1005	5:40	0.001
1004	1:10	0.042	1005	5:50	0.001
1004	1:20	0.024	1005	6:00	0.001
1004	1:30	0.016	1005	6:10	0.001
1004	1:40	0.01	1005	6:20	0.001
1004	1:50	0.008	1005	6:30	0.001
1004	2:00	0.006	1005	6:40	0
1004	2:10	0.005	1005	6:50	0
1004	2:20	0.005	1005	7:00	0
1004	2:30	0.004	1005	7:10	0
1004	2:40	0.004	1005	7:20	0
1004	2:50	0.004	1005	7:30	0
1004	3:00	0.003	1005	7:40	0
1004	3:10	0.003	1005	7:50	0
1004	3:20	0.002	1005	8:00	0
1004	3:30	0.001	1005	8:10	0
1004	3:40	0.001	1005	8:20	0
1004	3:50	0.001	1005	8:30	0
1004	4:00	0.001	1005	8:40	0
1004	4:10	0.001	1005	8:50	0
1004	4:20	0.001	1005	9:00	0
1004	4:30	0.001	1005	9:10	0
1004	4:40	0.001	1005	9:20	0
1004	4:50	0.001	1005	9:30	0
1004	5:00	0.001	1005	9:40	0
1004	5:10	0	1005	9:50	0
1004	5:20	0	1005	10:00	0
1004	5:30	0	1005	10:10	0
1004	5:40	0	1005	10:20	0
1004	5:50	0	1005	10:30	0
1004	6:00	0	1005	10:40	0
1004	6:10	0	1005	10:50	0
1004	6:20	0	1005	11:00	0
1004	6:30	0	1005	11:10	0
1004	6:40	0	1005	11:20	0
1004	6:50	0	1005	11:30	0
1004	7:00	0	1005	11:40	0
1004	7:10	0	1005	11:50	0
1004	7:20	0	1005	12:00	0
1004	7:30	0	1005	12:10	0
1004	7:40	0	1005	12:20	0
1004	7:50	0	1005	12:30	0
1004	8:00	0	1005	12:40	0
1004	8:10	0	1005	12:50	0
1004	8:20	0	1005	13:00	0
1004	8:30	0	1005	13:10	0
1004	8:40	0			
1004	8:50	0	1006	0:00	0
1004	9:00	0	1006	0:10	0
1004	9:10	0	1006	0:20	0.004
1004	9:20	0	1006	0:30	0.015
1004	9:30	0	1006	0:40	0.034
1004	9:40	0	1006	0:50	0.095
1004	9:50	0	1006	1:00	0.11
1004	10:00	0	1006	1:10	0.095
1004	10:10	0	1006	1:20	0.063
1004	10:20	0	1006	1:30	0.034
1004	10:30	0	1006	1:40	0.019
1004	10:40	0	1006	1:50	0.017
1004	10:50	0	1006	2:00	0.012
1004	11:00	0	1006	2:10	0.011
1004	11:10	0	1006	2:20	0.01
1004	11:20	0	1006	2:30	0.009
1004	11:30	0	1006	2:40	0.008
1004	11:40	0	1006	2:50	0.007
1004	11:50	0	1006	3:00	0.007
1004	12:00	0	1006	3:10	0.005
1004	12:10	0	1006	3:20	0.004
1004	12:20	0	1006	3:30	0.003
1004	12:30	0	1006	3:40	0.002
1004	12:40	0	1006	3:50	0.002
1004	12:50	0	1006	4:00	0.002
1004	13:00	0	1006	4:10	0.002
1004	13:10	0	1006	4:20	0.002
			1006	4:30	0.001
1005	0:00	0	1006	4:40	0.001
1005	0:10	0	1006	4:50	0.001
1005	0:20	0.001	1006	5:00	0.001
1005	0:30	0.004	1006	5:10	0.001
1005	0:40	0.009	1006	5:20	0.001
1005	0:50	0.024	1006	5:30	0.001
1005	1:00	0.044	1006	5:40	0.001
1005	1:10	0.044	1006	5:50	0.001
1005	1:20	0.034	1006	6:00	0.001
1005	1:30	0.026	1006	6:10	0.001
1005	1:40	0.019	1006	6:20	0.001
1005	1:50	0.015	1006	6:30	0.001
1005	2:00	0.011	1006	6:40	0.001
1005	2:10	0.009	1006	6:50	0.001
1005	2:20	0.008	1006	7:00	0.001
1005	2:30	0.007	1006	7:10	0.001

Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

1006	7:20	0.001	1008	12:00	0
1006	7:30	0.001	1008	12:10	0
1006	7:40	0.001	1008	12:20	0
1006	7:50	0	1008	12:30	0
1006	8:00	0	1008	12:40	0
1006	8:10	0	1008	12:50	0
1006	8:20	0	1008	13:00	0
1006	8:30	0	1008	13:10	0
1006	8:40	0			
1006	8:50	0	1009	0:00	0
1006	9:00	0	1009	0:10	0
1006	9:10	0	1009	0:20	0.001
1006	9:20	0	1009	0:30	0.004
1006	9:30	0	1009	0:40	0.008
1006	9:40	0	1009	0:50	0.044
1006	9:50	0	1009	1:00	0.043
1006	10:00	0	1009	1:10	0.03
1006	10:10	0	1009	1:20	0.016
1006	10:20	0	1009	1:30	0.012
1006	10:30	0	1009	1:40	0.006
1006	10:40	0	1009	1:50	0.006
1006	10:50	0	1009	2:00	0.003
1006	11:00	0	1009	2:10	0.003
1006	11:10	0	1009	2:20	0.003
1006	11:20	0	1009	2:30	0.003
1006	11:30	0	1009	2:40	0.002
1006	11:40	0	1009	2:50	0.002
1006	11:50	0	1009	3:00	0.002
1006	12:00	0	1009	3:10	0.001
1006	12:10	0	1009	3:20	0.001
1006	12:20	0	1009	3:30	0.001
1006	12:30	0	1009	3:40	0.001
1006	12:40	0	1009	3:50	0.001
1006	12:50	0	1009	4:00	0.001
1006	13:00	0	1009	4:10	0.001
1006	13:10	0	1009	4:20	0
			1009	4:30	0
1008	0:00	0	1009	4:40	0
1008	0:10	0	1009	4:50	0
1008	0:20	0.001	1009	5:00	0
1008	0:30	0.006	1009	5:10	0
1008	0:40	0.011	1009	5:20	0
1008	0:50	0.044	1009	5:30	0
1008	1:00	0.066	1009	5:40	0
1008	1:10	0.066	1009	5:50	0
1008	1:20	0.065	1009	6:00	0
1008	1:30	0.053	1009	6:10	0
1008	1:40	0.027	1009	6:20	0
1008	1:50	0.018	1009	6:30	0
1008	2:00	0.011	1009	6:40	0
1008	2:10	0.009	1009	6:50	0
1008	2:20	0.007	1009	7:00	0
1008	2:30	0.007	1009	7:10	0
1008	2:40	0.006	1009	7:20	0
1008	2:50	0.005	1009	7:30	0
1008	3:00	0.005	1009	7:40	0
1008	3:10	0.004	1009	7:50	0
1008	3:20	0.003	1009	8:00	0
1008	3:30	0.003	1009	8:10	0
1008	3:40	0.002	1009	8:20	0
1008	3:50	0.002	1009	8:30	0
1008	4:00	0.002	1009	8:40	0
1008	4:10	0.001	1009	8:50	0
1008	4:20	0.001	1009	9:00	0
1008	4:30	0.001	1009	9:10	0
1008	4:40	0.001	1009	9:20	0
1008	4:50	0.001	1009	9:30	0
1008	5:00	0.001	1009	9:40	0
1008	5:10	0.001	1009	9:50	0
1008	5:20	0.001	1009	10:00	0
1008	5:30	0.001	1009	10:10	0
1008	5:40	0.001	1009	10:20	0
1008	5:50	0.001	1009	10:30	0
1008	6:00	0.001	1009	10:40	0
1008	6:10	0	1009	10:50	0
1008	6:20	0	1009	11:00	0
1008	6:30	0	1009	11:10	0
1008	6:40	0	1009	11:20	0
1008	6:50	0	1009	11:30	0
1008	7:00	0	1009	11:40	0
1008	7:10	0	1009	11:50	0
1008	7:20	0	1009	12:00	0
1008	7:30	0	1009	12:10	0
1008	7:40	0	1009	12:20	0
1008	7:50	0	1009	12:30	0
1008	8:00	0	1009	12:40	0
1008	8:10	0	1009	12:50	0
1008	8:20	0	1009	13:00	0
1008	8:30	0	1009	13:10	0
1008	8:40	0			
1008	8:50	0	1010	0:00	0
1008	9:00	0	1010	0:10	0
1008	9:10	0	1010	0:20	0.001
1008	9:20	0	1010	0:30	0.009
1008	9:30	0	1010	0:40	0.021
1008	9:40	0	1010	0:50	0.052
1008	9:50	0	1010	1:00	0.054
1008	10:00	0	1010	1:10	0.049
1008	10:10	0	1010	1:20	0.047
1008	10:20	0	1010	1:30	0.028
1008	10:30	0	1010	1:40	0.016
1008	10:40	0	1010	1:50	0.013
1008	10:50	0	1010	2:00	0.01
1008	11:00	0	1010	2:10	0.008
1008	11:10	0	1010	2:20	0.007
1008	11:20	0	1010	2:30	0.006
1008	11:30	0	1010	2:40	0.006
1008	11:40	0	1010	2:50	0.005
1008	11:50	0	1010	3:00	0.005

Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

1012A	12:10	0.001	1013	3:10	0.136
1012A	12:20	0.001	1013	3:20	0.114
1012A	12:30	0.001	1013	3:30	0.089
1012A	12:40	0.001	1013	3:40	0.069
1012A	12:50	0.001	1013	3:50	0.054
1012A	13:00	0.001	1013	4:00	0.043
1012A	13:10	0.001	1013	4:10	0.035
			1013	4:20	0.03
1012B	0:00	0	1013	4:30	0.025
1012B	0:10	0	1013	4:40	0.022
1012B	0:20	0	1013	4:50	0.019
1012B	0:30	0	1013	5:00	0.017
1012B	0:40	0.001	1013	5:10	0.015
1012B	0:50	0.005	1013	5:20	0.013
1012B	1:00	0.014	1013	5:30	0.012
1012B	1:10	0.02	1013	5:40	0.011
1012B	1:20	0.02	1013	5:50	0.011
1012B	1:30	0.018	1013	6:00	0.01
1012B	1:40	0.015	1013	6:10	0.01
1012B	1:50	0.013	1013	6:20	0.009
1012B	2:00	0.012	1013	6:30	0.009
1012B	2:10	0.01	1013	6:40	0.008
1012B	2:20	0.009	1013	6:50	0.008
1012B	2:30	0.008	1013	7:00	0.008
1012B	2:40	0.008	1013	7:10	0.008
1012B	2:50	0.007	1013	7:20	0.007
1012B	3:00	0.007	1013	7:30	0.007
1012B	3:10	0.006	1013	7:40	0.007
1012B	3:20	0.006	1013	7:50	0.007
1012B	3:30	0.005	1013	8:00	0.006
1012B	3:40	0.005	1013	8:10	0.006
1012B	3:50	0.005	1013	8:20	0.006
1012B	4:00	0.005	1013	8:30	0.006
1012B	4:10	0.004	1013	8:40	0.006
1012B	4:20	0.004	1013	8:50	0.006
1012B	4:30	0.004	1013	9:00	0.006
1012B	4:40	0.004	1013	9:10	0.005
1012B	4:50	0.004	1013	9:20	0.005
1012B	5:00	0.004	1013	9:30	0.005
1012B	5:10	0.004	1013	9:40	0.005
1012B	5:20	0.003	1013	9:50	0.005
1012B	5:30	0.003	1013	10:00	0.005
1012B	5:40	0.003	1013	10:10	0.005
1012B	5:50	0.003	1013	10:20	0.005
1012B	6:00	0.003	1013	10:30	0.005
1012B	6:10	0.003	1013	10:40	0.004
1012B	6:20	0.003	1013	10:50	0.004
1012B	6:30	0.003	1013	11:00	0.004
1012B	6:40	0.003	1013	11:10	0.004
1012B	6:50	0.003	1013	11:20	0.004
1012B	7:00	0.003	1013	11:30	0.004
1012B	7:10	0.003	1013	11:40	0.004
1012B	7:20	0.002	1013	11:50	0.004
1012B	7:30	0.002	1013	12:00	0.004
1012B	7:40	0.002	1013	12:10	0.004
1012B	7:50	0.002	1013	12:20	0.004
1012B	8:00	0.002	1013	12:30	0.004
1012B	8:10	0.002	1013	12:40	0.004
1012B	8:20	0.002	1013	12:50	0.004
1012B	8:30	0.002	1013	13:00	0.004
1012B	8:40	0.002	1013	13:10	0.003
1012B	8:50	0.002			
1012B	9:00	0.002	1014	0:00	0
1012B	9:10	0.002	1014	0:10	0
1012B	9:20	0.002	1014	0:20	0
1012B	9:30	0.002	1014	0:30	0.002
1012B	9:40	0.002	1014	0:40	0.005
1012B	9:50	0.002	1014	0:50	0.022
1012B	10:00	0.002	1014	1:00	0.022
1012B	10:10	0.002	1014	1:10	0.022
1012B	10:20	0.002	1014	1:20	0.022
1012B	10:30	0.002	1014	1:30	0.022
1012B	10:40	0.002	1014	1:40	0.022
1012B	10:50	0.002	1014	1:50	0.021
1012B	11:00	0.002	1014	2:00	0.02
1012B	11:10	0.002	1014	2:10	0.017
1012B	11:20	0.002	1014	2:20	0.015
1012B	11:30	0.002	1014	2:30	0.013
1012B	11:40	0.002	1014	2:40	0.012
1012B	11:50	0.002	1014	2:50	0.01
1012B	12:00	0.001	1014	3:00	0.009
1012B	12:10	0.001	1014	3:10	0.008
1012B	12:20	0.001	1014	3:20	0.007
1012B	12:30	0.001	1014	3:30	0.006
1012B	12:40	0.001	1014	3:40	0.005
1012B	12:50	0.001	1014	3:50	0.005
1012B	13:00	0.001	1014	4:00	0.004
1012B	13:10	0.001	1014	4:10	0.003
			1014	4:20	0.003
			1014	4:30	0.003
;Design inflow to MH1013 from Oct 2007 Ph1 Subdivision design			1014	4:40	0.002
1013	0:00	0	1014	4:50	0.002
1013	0:10	0	1014	5:00	0.002
1013	0:20	0.001	1014	5:10	0.002
1013	0:30	0.028	1014	5:20	0.001
1013	0:40	0.098	1014	5:30	0.001
1013	0:50	0.315	1014	5:40	0.001
1013	1:00	0.913	1014	5:50	0.001
1013	1:10	1.085	1014	6:00	0.001
1013	1:20	1.05	1014	6:10	0.001
1013	1:30	0.959	1014	6:20	0.001
1013	1:40	0.773	1014	6:30	0.001
1013	1:50	0.563	1014	6:40	0.001
1013	2:00	0.42	1014	6:50	0.001
1013	2:10	0.319	1014	7:00	0.001
1013	2:20	0.252	1014	7:10	0.001
1013	2:30	0.208	1014	7:20	0.001
1013	2:40	0.178	1014	7:30	0
1013	2:50	0.157	1014	7:40	0
1013	3:00	0.146	1014		

Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

1014	7:50	0	1015	12:30	0
1014	8:00	0	1015	12:40	0
1014	8:10	0	1015	12:50	0
1014	8:20	0	1015	13:00	0
1014	8:30	0	1015	13:10	0
1014	8:40	0			
1014	8:50	0	1016	0:00	0
1014	9:00	0	1016	0:10	0
1014	9:10	0	1016	0:20	0.001
1014	9:20	0	1016	0:30	0.006
1014	9:30	0	1016	0:40	0.008
1014	9:40	0	1016	0:50	0.022
1014	9:50	0	1016	1:00	0.044
1014	10:00	0	1016	1:10	0.044
1014	10:10	0	1016	1:20	0.044
1014	10:20	0	1016	1:30	0.044
1014	10:30	0	1016	1:40	0.032
1014	10:40	0	1016	1:50	0.021
1014	10:50	0	1016	2:00	0.012
1014	11:00	0	1016	2:10	0.01
1014	11:10	0	1016	2:20	0.007
1014	11:20	0	1016	2:30	0.007
1014	11:30	0	1016	2:40	0.006
1014	11:40	0	1016	2:50	0.006
1014	11:50	0	1016	3:00	0.006
1014	12:00	0	1016	3:10	0.005
1014	12:10	0	1016	3:20	0.004
1014	12:20	0	1016	3:30	0.003
1014	12:30	0	1016	3:40	0.003
1014	12:40	0	1016	3:50	0.002
1014	12:50	0	1016	4:00	0.002
1014	13:00	0	1016	4:10	0.002
1014	13:10	0	1016	4:20	0.001
			1016	4:30	0.001
1015	0:00	0	1016	4:40	0.001
1015	0:10	0	1016	4:50	0.001
1015	0:20	0.001	1016	5:00	0.001
1015	0:30	0.004	1016	5:10	0.001
1015	0:40	0.011	1016	5:20	0.001
1015	0:50	0.044	1016	5:30	0.001
1015	1:00	0.044	1016	5:40	0.001
1015	1:10	0.044	1016	5:50	0.001
1015	1:20	0.044	1016	6:00	0.001
1015	1:30	0.041	1016	6:10	0.001
1015	1:40	0.036	1016	6:20	0.001
1015	1:50	0.031	1016	6:30	0.001
1015	2:00	0.027	1016	6:40	0
1015	2:10	0.022	1016	6:50	0
1015	2:20	0.018	1016	7:00	0
1015	2:30	0.015	1016	7:10	0
1015	2:40	0.013	1016	7:20	0
1015	2:50	0.011	1016	7:30	0
1015	3:00	0.01	1016	7:40	0
1015	3:10	0.008	1016	7:50	0
1015	3:20	0.007	1016	8:00	0
1015	3:30	0.006	1016	8:10	0
1015	3:40	0.005	1016	8:20	0
1015	3:50	0.004	1016	8:30	0
1015	4:00	0.004	1016	8:40	0
1015	4:10	0.003	1016	8:50	0
1015	4:20	0.003	1016	9:00	0
1015	4:30	0.002	1016	9:10	0
1015	4:40	0.002	1016	9:20	0
1015	4:50	0.002	1016	9:30	0
1015	5:00	0.002	1016	9:40	0
1015	5:10	0.001	1016	9:50	0
1015	5:20	0.001	1016	10:00	0
1015	5:30	0.001	1016	10:10	0
1015	5:40	0.001	1016	10:20	0
1015	5:50	0.001	1016	10:30	0
1015	6:00	0.001	1016	10:40	0
1015	6:10	0.001	1016	10:50	0
1015	6:20	0.001	1016	11:00	0
1015	6:30	0.001	1016	11:10	0
1015	6:40	0.001	1016	11:20	0
1015	6:50	0.001	1016	11:30	0
1015	7:00	0.001	1016	11:40	0
1015	7:10	0.001	1016	11:50	0
1015	7:20	0.001	1016	12:00	0
1015	7:30	0.001	1016	12:10	0
1015	7:40	0.001	1016	12:20	0
1015	7:50	0.001	1016	12:30	0
1015	8:00	0.001	1016	12:40	0
1015	8:10	0.001	1016	12:50	0
1015	8:20	0.001	1016	13:00	0
1015	8:30	0.001	1016	13:10	0
1015	8:40	0			
1015	8:50	0	1017	0:00	0
1015	9:00	0	1017	0:10	0
1015	9:10	0	1017	0:20	0.001
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Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

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Trails West: Cope Drive Units – PCSWMM Input Data – 2007 Subdivision

Design Storm: 100 Year 3-hour Chicago Storm

Node	1048A	ExMH	1015	354193.424	5015586.291
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Node	1051	ExMH	1018	354490.164	5015381.283
Node	1052	ExMH	1019	354568.148	5015451.67
Node	1053	ExMH	1020	354569.153	5015459.707
Node	2000	ExMH	1021	354541.323	5015507.456
Node	2001	ExMH	1022	354375.649	5015393.277
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Link	L133	FutMinor	1053	354008.751	5015655.704
Link	L134	FutMinor	2000	354437.797	5015789.783
Link	L135	FutMinor	2000A	354426.849	5015775.696
Link	L136	FutMinor	2001	354475.916	5015772.151
Link	L138	FutMinor	2002	354454.87	5015727.526
Link	L139	FutMinor	2003	354457.518	5015717.281
Link	L140	ExMinorSystem	2004	354489.999	5015701.021
Link	L141	FutMinor	CB1043G	354188.724	5015635.914
Link	L142	FutMinor			
Link	L31	ExMinorSystem	[VERTICES]		
Link	L32	ExMinorSystem	;;Link	X-Coord	Y-Coord
Link	L33	ExMinorSystem	;;		
Link	L34	ExMinorSystem	-----		
Link	L46	ExMinorSystem	[SYMBOLS]		
Link	L47	ExMinorSystem	;;Gage	X-Coord	Y-Coord
Link	L48	ExMinorSystem	;;		
Link	L49	ExMinorSystem	-----		
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Link	L95	ExMinorSystem			
Link	L96	ExMinorSystem			
Link	L97	ExMinorSystem			
Link	L98	ExMinorSystem			

[MAP]
 DIMENSIONS 353854.75295 5015239.9928 354603.17205
 5015858.5492
 UNITS Meters

[COORDINATES]

;;Node	X-Coord	Y-Coord
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Diversion-1053	354335.15	5015642.12
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1002	354353.33	5015268.109
1004	354300.967	5015328.509
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1006A	354198.87	5015454.32
1007	354427.973	5015327.365
1008	354403.791	5015350.159
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1012A	354212.895	5015533.201
1012B	354193.631	5015516.358
1013	354067.827	5015475.072
1014	354122.432	5015526.35

C.3 Storm Sewer Design Sheet



APPENDIX D



Appendix D Geotechnical Information



May 23, 2023

File: 102128.003

Cavanagh Developments
9094 Cavanagh Road
Ashton, Ontario
K0A 1B0

Attention: Erin O'Connor, MCIP, RPP, General Manager, Land Development

**Re: Cope Drive Blocks 24, 43, 46 and 104
Trail West Subdivision
Kanata, City of Ottawa, Ontario**

INTRODUCTION

It is understood that plans are being prepared to develop Cope Drive, Blocks 24, 43, 46 and 104 at the Trail West subdivision in Kanata South, in the City of Ottawa, Ontario.

The Trail West subdivision was formerly referred to as the Soho West development which has been the subject of previous investigation and reporting by GEMTEC Consulting Engineers and Scientists Limited (GEMTEC) and others. The majority of the blocks within the subdivision have been developed previously and are currently occupied.

This letter, in combination with previous reports which are referenced in this letter, provides geotechnical commentary on the proposed works at the vacant blocks identified along Cope Drive (i.e. 24, 43, 46 and 104). GEMTEC was retained by Cavanagh Developments to prepare this letter. Additional geotechnical investigations at the blocks are not part of the current scope of work.

SUMMARY OF PROPOSED WORKS

A combination of "Back-to-Back" and conventional townhome units will be constructed on the blocks. Details of the units were provided to GEMTEC via a series of drawings which include GP-1, Cope Drive Units Grading Plan prepared by Stantec Consulting Ltd. (Stantec) dated April 2023, and a series of architectural drawings prepared by M David Blakely Architect Inc. Based on these drawings it is understood that:

- The units will be provided with a basement level with conventional below slab and perimeter foundation drainage measures:

- Original ground elevation is estimated to be about 0.5 to 2 metres below existing ground elevation in general, and possibly locally up to 2.5 metres, according to information provided by Stantec.
- The proposed Under Side of Foundation (USF) of the units in the relevant blocks range as indicated below:
 - Block 24: from 95.0 to 95.2 metres, approximately;
 - Block 43: 95.0 metres, approximately;
 - Block 46: from 95.1 to 95.4 metres, approximately;
 - Block 104: 95.3 metres, approximately
- Maximum proposed Finished Grade of the units in the relevant blocks range as indicated below:
 - Block 24: 97.3 metres approximately;
 - Block 43: 96.8 metres, approximately;
 - Block 46: 97.5 metres, approximately;
 - Block 104: 96.7 metres, approximately.

Relatively short surficial access roads and driveways will be constructed to the units. Underground municipal services will also be constructed.

In addition, the structural engineers to Cavanagh Developments have provided GEMTEC information on anticipated foundation loads for the proposed structures under Serviceability Limit State conditions.

SUMMARY OF AVAILABLE INFORMATION

Previous Investigations by GEMTEC

Previous geotechnical investigations have been undertaken in the general area of the proposed blocks at various stages from about 1985. In summary, the soils in the area of the subdivision consist of a combination of fill material (engineered and uncontrolled), over deposits of sensitive silty clay and glacial till which overly bedrock. Previously, peat was present at the site of the subdivision.

GEMTEC, formerly Houle Chevrier Engineering Ltd. (HCEL), has carried out a series of investigations within the general subdivision and also at the location of the particular blocks which are the subject of this letter. GEMTEC (formerly HCEL) has prepared two geotechnical reports which are relevant to the proposed development at the blocks. A summary of the investigations is provided below. Refer to the reports for more detail. The locations of the previous investigation points considered for each of these reports is provided as an attachment to this letter.

GEMTEC / HCEL 2010

Records of previous investigations at the Trail West subdivision are provided in the Houle Chevrier Engineering report titled, “Geotechnical Investigation, Sohwest Area B, Eagleson Road at Fernbank Road, Ottawa, Ontario”. The report, dated May 2010, was prepared by GEMTEC (formerly, HCEL) and submitted to TCK Co-Tenancy. The report included Blocks 43 and 104. Blocks 24 and 46 are adjoining to the areas considered in the report.

GEMTEC 2016

GEMTEC carried out additional investigations at the Cope Road development blocks in 2016 (along with other areas) for an alternative construction approach which, GEMTEC understands, is no longer under consideration. The findings of those investigations were provided in GEMTEC’s report titled, “Geotechnical Investigation, Akerson Road and Cope Drive Development, Ottawa, Ontario”. The report, dated March 2016, was submitted to Thomas Cavanagh Construction Ltd.

As part of the GEMTEC 2016 investigation 4 boreholes and 4 (piezo)Cone Penetration Test (CPTu) probes were advanced at the development blocks, with 1 borehole and 1 probe on each block. The boreholes and CPTu probes encountered a surficial layer of fill material over a relatively thin silty sand layer which in turn overlies sensitive silty clay. The silty clay extends to significant depth and overlies glacial till and bedrock.

These subsurface conditions are consistent with those encountered during previous investigations at the subdivision.

GEOTECHNICAL COMMENTARY

Construction Approach

Please note – the GEMTEC 2016 report is identified herein for the purposes of identifying the subsurface conditions at the development blocks; the report was prepared for an alternative development which is no longer under consideration. The geotechnical guidelines provided in the GEMTEC / HCEL 2010 report should be followed for works at the development blocks, subject to the following additional specific considerations:

- Fill material was noted to be present on the development blocks. The fill material may be variable in composition and thickness and may include blast rock fill in addition to other materials. In addition, borehole 15-5 at Block 24 identified a layer of organic material below the fill material which may be a former (i.e. buried) topsoil layer. All existing fill material and any former topsoil or otherwise unsuitable material should be removed from beneath the footprint of the buildings and from below any engineered fill material on which the units are supported to expose the native subgrade soils.

- Based on assessments carried out by GEMTEC, the Geotechnical Reaction at Serviceability Limit State (SLS) for the structures bearing on the native deposits of silty clay is estimated to be 65 kilopascals. The factored Geotechnical Resistance at Ultimate Limit State is estimated to be 100 kilopascals.
 - It is recommended that the bearing pressure for each block be confirmed at the time of construction by advancing 2 hand auger probes from the foundation level / exposed subgrade below the fill (whichever is lower) to measure the strength of the clay and the thickness of the weathered clay crust present. Based on the results of the hand auger holes it may be necessary to revise the allowable bearing pressure.
- Based on assessments carried out by GEMTEC, the maximum thickness of engineered fill material that can be placed above the original ground level should be limited to 1.0 metre for engineered fill with a maximum bulk unit weight (compacted) of 18 kilonewtons per cubic metre. The use of expanded polystyrene (EPS) blocks may be incorporated to account for the difference where the required thickness of grade raise fill exceeds this limit. Refer to the GEMTEC / HCEL 2010 report for guidance on the zones around and within the structures where EPS fill may be placed.
- The granular bedding in the service trenches could act as a “French Drain”, which could promote groundwater lowering. As such, in order to reduce the potential for groundwater lowering to occur, seepage barriers should be installed along the service trenches at strategic locations. The seepage barriers should begin at subgrade level and extend vertically through the granular pipe bedding and granular surround to within the native backfill materials, and horizontally across the full width of the service trench excavation. The seepage barriers could consist of 1.5-metre-wide dykes of compacted silty clay. The silty clay should be compacted in maximum 300-millimetre-thick lifts to at least 95 percent of the standard Proctor dry density value. The locations of the seepage barriers could be provided as the design progresses.
- As noted in the GEMTEC / HCEL 2010 report, consideration could be given to providing additional reinforcement to the foundations at key locations, (for instance between the garage and adjacent areas) to reduce the potential for cracks to develop in the structures.

Construction Inspections

The engagement of the services of GEMTEC during construction is recommended to inspect the subsurface conditions throughout the proposed excavations, and to confirm the allowable bearing pressures.

The placing and compaction of engineered fill materials should be inspected to ensure that the materials used conform to the grading and compaction specifications.

CLOSING

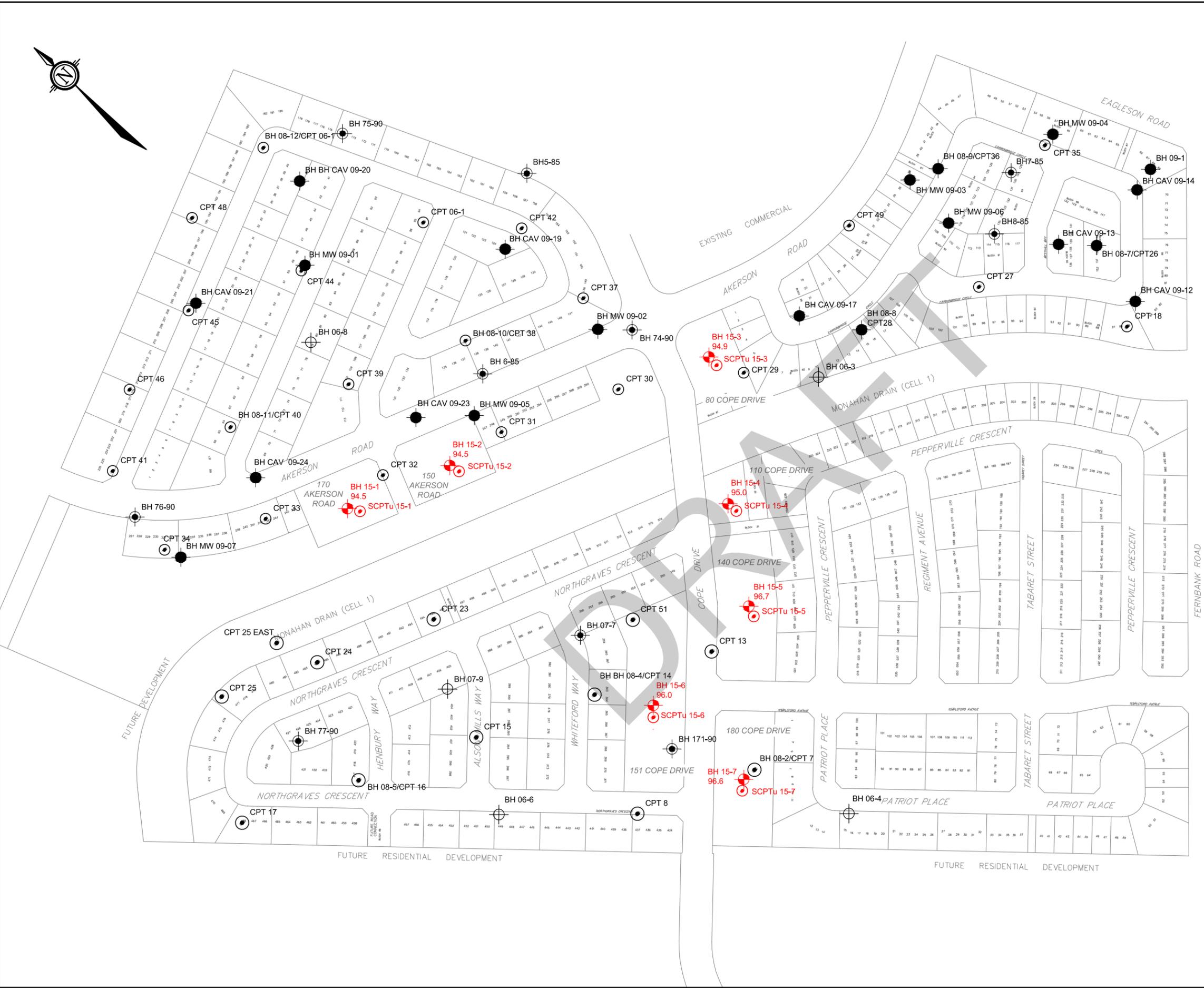
We trust this letter is sufficient for your purposes. Do not hesitate to contact the undersigned should you require additional information.

Daire Cummins, M.Sc.

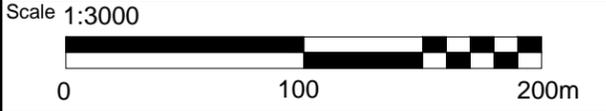
Lauren Ashe, M.A.Sc., P.Eng.
Senior Geotechnical Engineer

N:\Projects\102100\102128.003\Deliverables\Letter 02 Cope\102128.003_LTR02_REV0_2023-05-23.docx

DRAFT



- LEGEND**
- BH 15-1 99.9 BOREHOLE LOCATION IN PLAN
(current investigation by Houle Chevrier Engineering Ltd., 2015)
 - SCPTu GROUND SURFACE ELEVATION IN METRES
GEODETIC DATUM
 - SCPTu CONE PENETRATION TEST LOCATION IN PLAN
(current investigation by Houle Chevrier Engineering Ltd., 2015)
 - BH 09-1 BOREHOLE LOCATION IN PLAN
BH MW09-01 (previous investigation by Houle Chevrier Engineering Ltd., 2009)
BHCPT
 - ⊕ BH 06-1 BOREHOLE LOCATION IN PLAN
BH 07-1 (previous investigation by Houle Chevrier Engineering Ltd., 2006/2007)
 - ⊕ BH 75-90 BOREHOLE LOCATION IN PLAN
(previous investigation by John D. Patterson Associates)
 - ⊕ CPT/BH08 CONE PENETRATION TEST/BOREHOLE
LOCATION IN PLAN
(previous investigation by DST Consulting Engineers)



Houle Chevrier Engineering

Houle Chevrier Engineering Ltd.
32 Steacie Drive
Ottawa, ON
Tel: (613) 836-1422
www.hceng.ca
ottawa@hceng.ca

Client	SOHO WEST DEVELOPMENT	Project	63978.79
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Location	EAGLESON ROAD AT FERNBANK ROAD OTTAWA, ON
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Drwn by	Chkd by	SITE PLAN
P.C.	S.B.	

Date	JULY 2015	Rev.	1	FIGURE 1
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APPENDIX E



Appendix E Drawings



Copyright Reserved

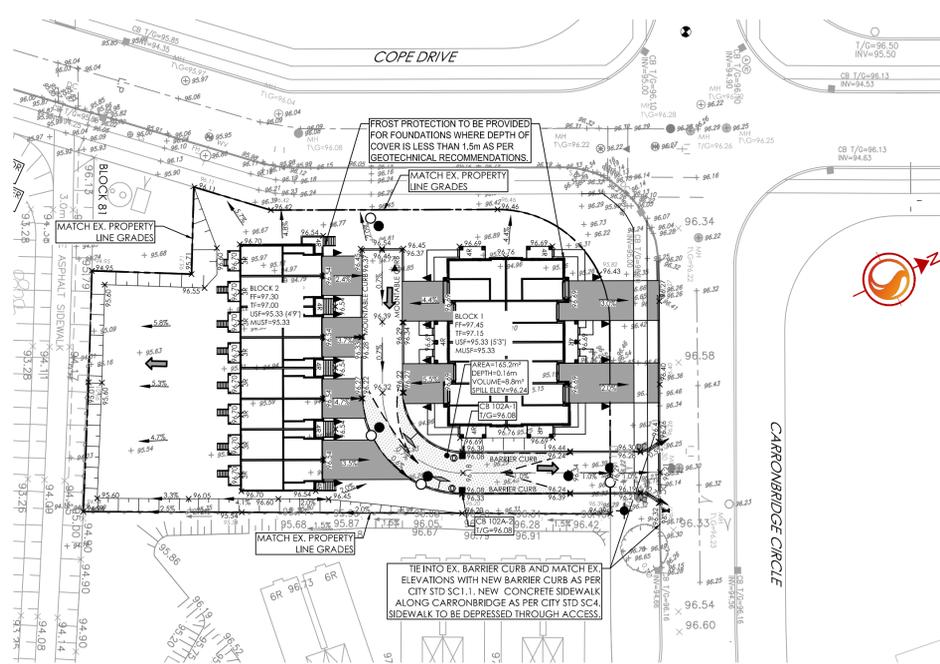
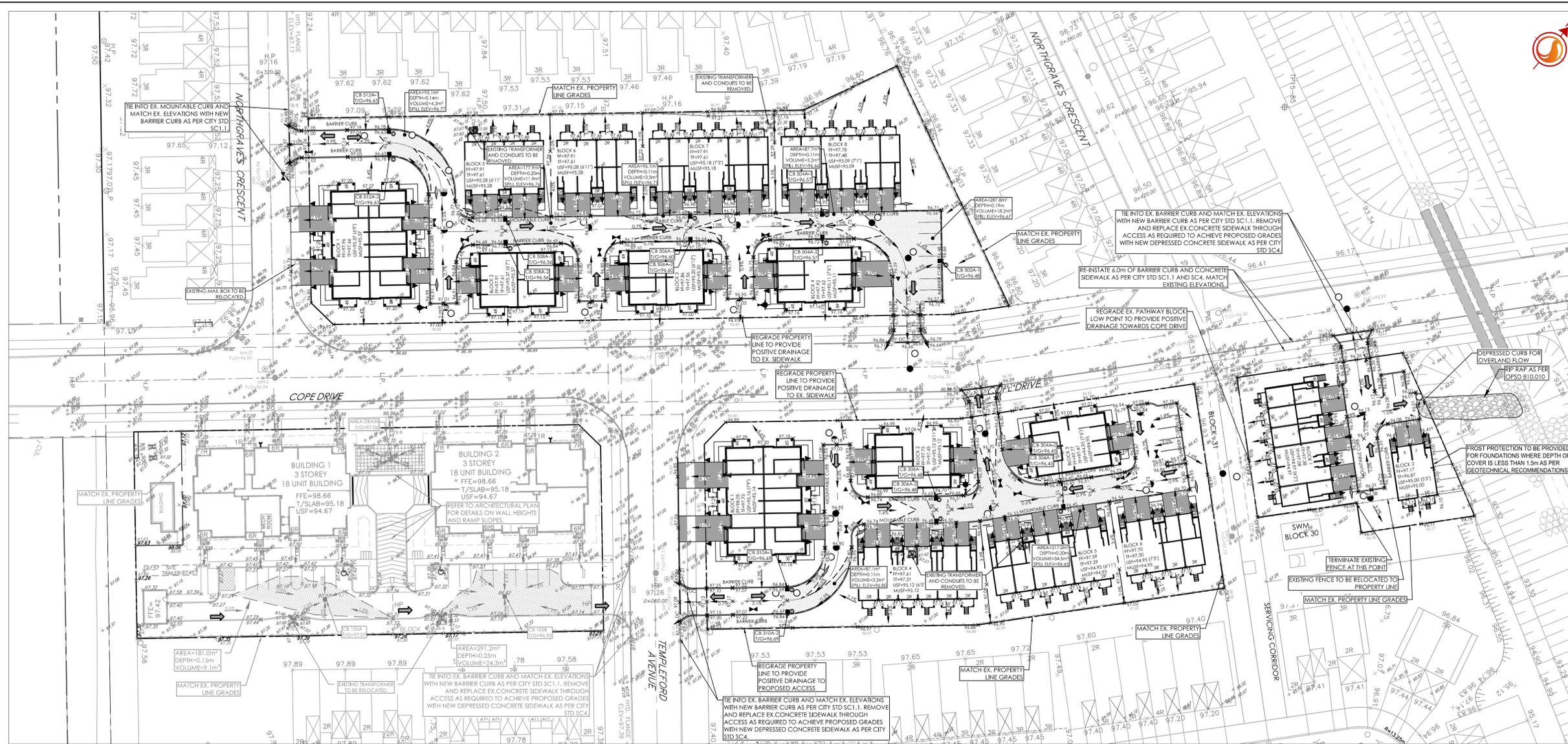
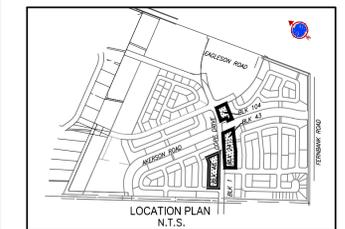
The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.
The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Legend

- 3.0% x 75.98 PROPOSED ELEVATION
- 96.66 PROPOSED LOT CORNER ELEVATION
- FFE=97.15 FINISHED FIRST FLOOR ELEVATION
- USF=97.15 (7') UNDER SIDE OF FOOTING (BASEMENT HEIGHT)
- 7R NUMBER OF RISERS
- 3:1 TERRACING 3:1 SLOPE OR FLATTER (UNLESS OTHERWISE SHOWN)
- PROPOSED FIRE HYDRANT
- PROPOSED CATCH BASIN
- WATS AREA DRAIN OR EQUIVALENT
- DIRECTION OF OVERLAND FLOW
- 96.80 T/W TOP OF RETAINING WALL ELEVATION
- 95.50 B/W BOTTOM OF RETAINING WALL ELEVATION
- ORIGINAL GROUND
- 0.5% LONGITUDINAL SWALES WITH SLOPE AND DIRECTION.
- PROPOSED RETAINING WALL

Notes

- ALL PEAT AND BLAST ROCK INFORMATION TO BE CONFIRMED BY GEOTECHNICAL ENGINEER.
- ALL ENGINEERED FILL AND GRADE RAISES TO BE APPROVED BY GEOTECHNICAL CONSULTANT PRIOR TO PLACING OF MATERIAL.
- ALL PROPOSED BUILDINGS TO BE EQUIPPED WITH BACKWATER VALVES FOR FOUNDATION DRAINAGE.



Revision	By	Appd.	YY.MM.DD
1	WAJ	KJK	23.06.19
ISSUED FOR REVIEW			

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Client/Project	CAVANAUGH CONSTRUCTION LTD.		
	TRAILS WEST		
	Ottawa ON Canada		
Title	COPE DRIVE UNITS GRADING PLAN		
Project No.	Scale	0 5 15 25m	
160401718	1:500		
Drawing No.	Sheet	Revision	
GP-1	2 of 6	1	

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Legend

-  PROPOSED SILT FENCE BOUNDARY AS PER OPSD 219.110
-  PROPOSED STRAW BALE LOCATION AS PER OPSD 219.100
-  PROPOSED MUD MAT LOCATION
-  PROPOSED CATCHBASIN PROTECTION AS PER TERRAFIX SILTSACK DETAIL

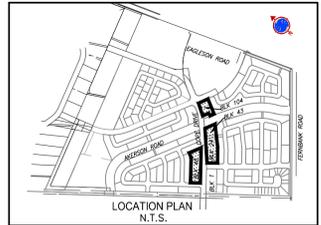
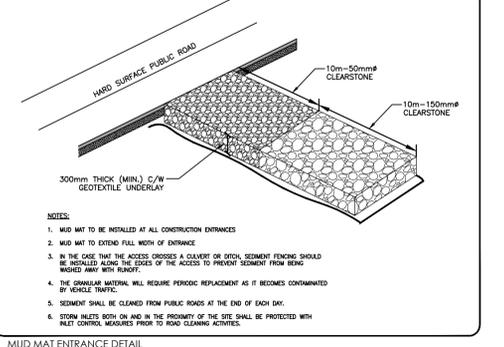
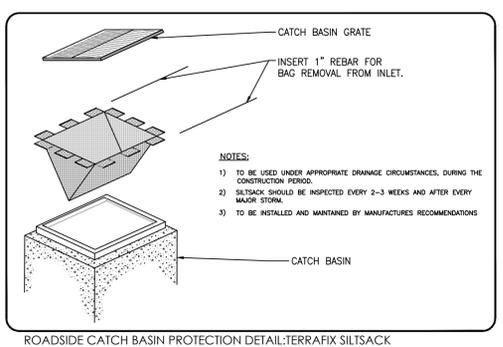
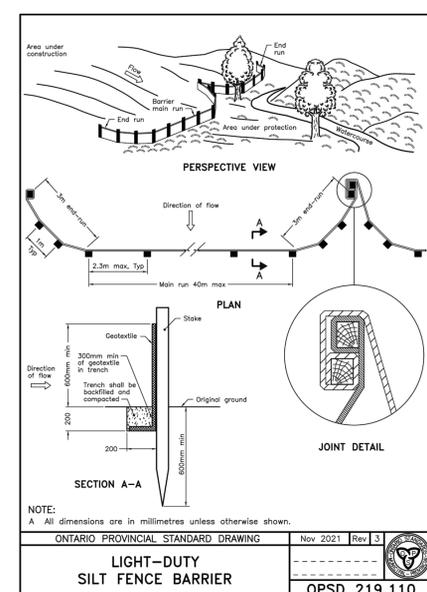
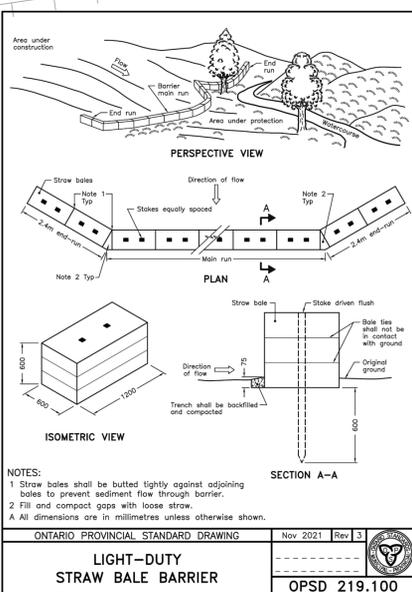
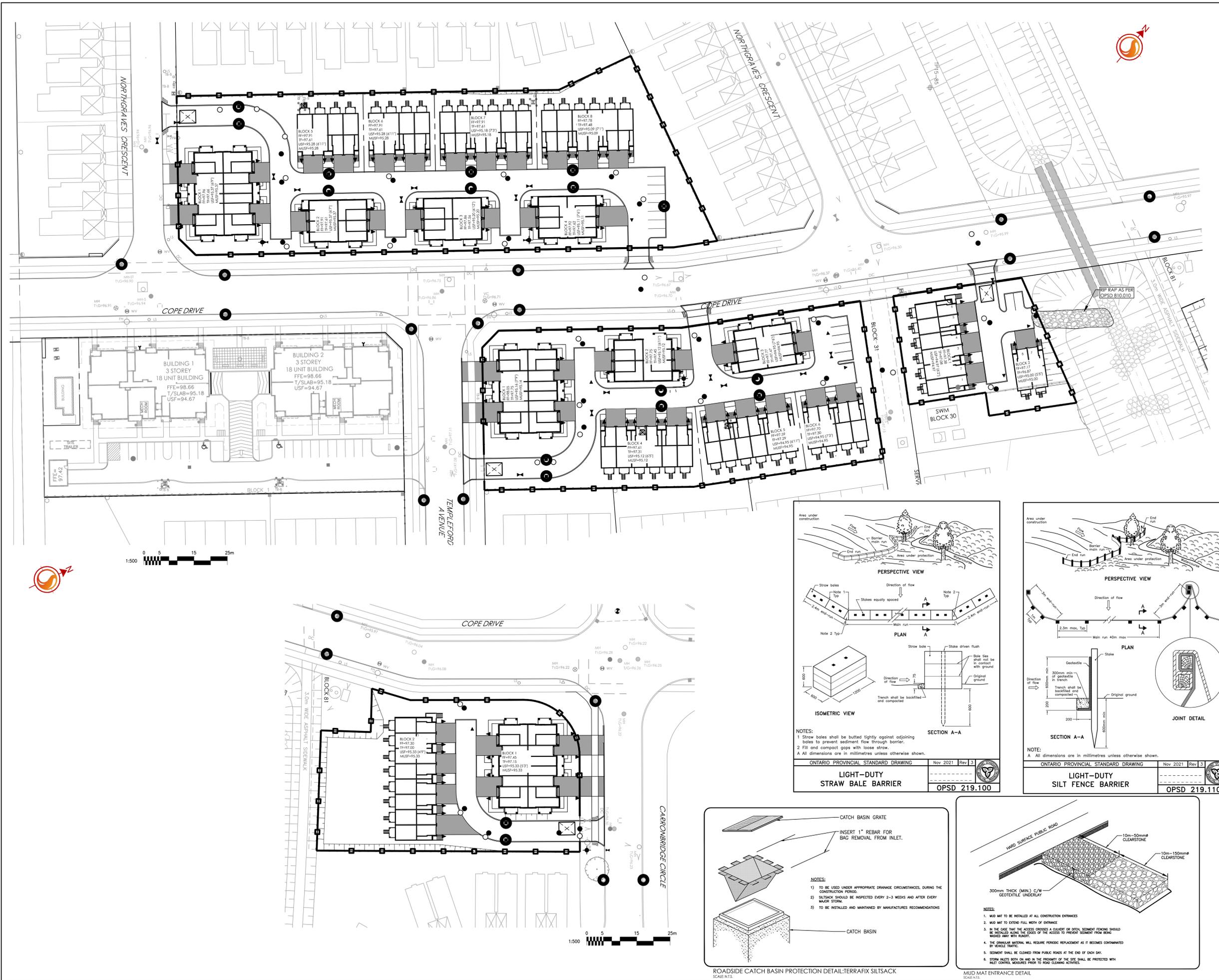
Notes

Best Management Practices

CONTRACTOR TO PROVIDE EROSION AND SEDIMENT CONTROLS (BEST MANAGEMENT PRACTICES) DURING CONSTRUCTION OF THIS PROJECT.

EROSION MUST BE MINIMIZED AND SEDIMENTS MUST BE REMOVED FROM CONSTRUCTION SITE RUN-OFF IN ORDER TO PROTECT DOWNSTREAM AREAS. DURING ALL CONSTRUCTION, EROSION AND SEDIMENTATION SHOULD BE CONTROLLED BY THE FOLLOWING TECHNIQUES:

1. LIMIT THE EXTENT OF EXPOSED SOILS AT ANY GIVEN TIME.
2. REVEGETATE EXPOSED AREAS AND SLOPES AS SOON AS POSSIBLE.
3. MINIMIZE AREA TO BE CLEARED AND GRUBBED.
4. PROTECT EXPOSED SLOPES WITH PLASTIC OR SYNTHETIC MULCHES.
5. INSTALL FILTER CLOTH BETWEEN FRAME AND COVER ON ALL PROPOSED CATCH BASINS AND CATCH BASIN MANHOLES AND ON ALL EXISTING CATCH BASINS THAT WILL RECEIVE RUNOFF FROM THE SITE.
6. A SILT FENCE SHALL BE INSTALLED AROUND THE PERIMETER OF ALL AND ANY STOCKPILES OF MATERIAL TO BE USED OR REMOVED FROM SITE. LOCATION TO BE DETERMINED.
7. A VISUAL INSPECTION SHALL BE DONE DAILY ON SEDIMENT CONTROL MEASURES AND CLEANED OF ANY ACCUMULATED SILT AS REQUIRED. THE DEPOSITS WILL BE EXPOSED OFF SITE AS PER THE REQUIREMENTS OF THE CONTRACT.
8. SEDIMENT CONTROL BARRIERS MAY ONLY BE REMOVED TEMPORARILY WITH APPROVAL OF CONTRACT ADMINISTRATOR TO ACCOMMODATE CONSTRUCTION OPERATIONS. ALL AFFECTED BARRIERS MUST BE REINSTATED AT NIGHT WHEN CONSTRUCTION IS COMPLETED. NO REMOVAL WILL OCCUR IF THERE IS A SIGNIFICANT RAINFALL EVENT ANTICIPATED (>10mm) UNLESS A NEW DEVICE HAS BEEN INSTALLED TO PROTECT THE EXISTING STORM AND SANITARY SEWER SYSTEMS.
9. NO REFUELING OR CLEANING OF EQUIPMENT IS PERMITTED NEAR ANY EXISTING WATERWAY.
10. CONTRACTOR SHALL REMOVE SEDIMENT CONTROL MEASURES WHEN, IN THE OPINION OF THE CONTRACT ADMINISTRATOR, THE MEASURE(S) IS NO LONGER REQUIRED. NO CONTROL MEASURES SHALL BE PERMANENTLY REMOVED WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE CONTRACT ADMINISTRATOR.
11. THE CONTRACTOR SHALL PERIODICALLY OR WHEN REQUESTED BY THE CONTRACT ADMINISTRATOR, CLEAN UP ACCUMULATED SEDIMENTS AS REQUIRED.
12. THE CONTRACTOR SHALL IMMEDIATELY REPORT TO THE ENGINEER ANY ACCIDENTAL DISCHARGES OF SEDIMENT MATERIAL INTO THE WATERCOURSE. APPROPRIATE RESPONSE MEASURES, INCLUDING ANY REPAIRS TO EXISTING CONTROL MEASURES OR THE IMPLEMENTATION OF ADDITIONAL CONTROL MEASURES, SHALL BE CARRIED OUT BY THE CONTRACTOR WITHOUT DELAY.



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Revision		By	Appd. YY.MM.DD
File Name: 160401718-DB	WAJ	KJK	WAJ
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			YY.MM.DD

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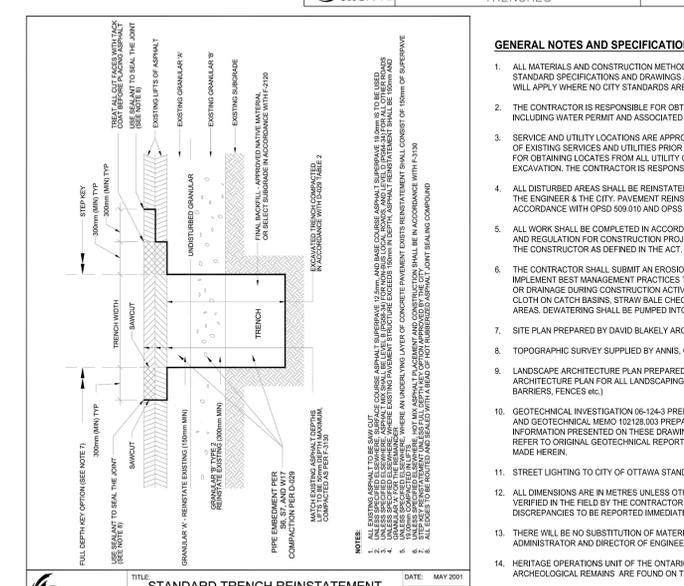
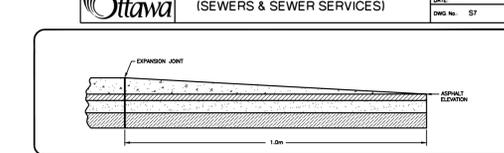
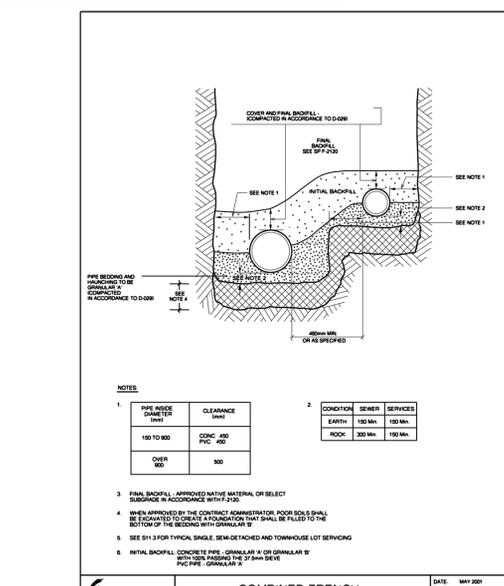
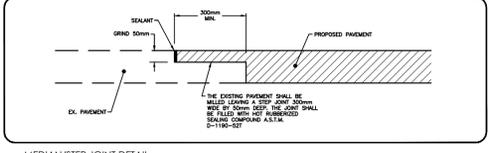
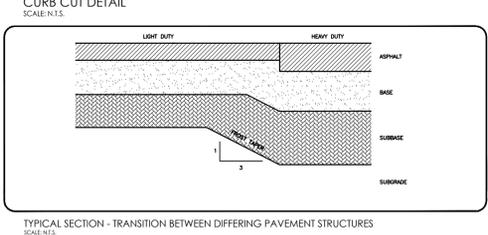
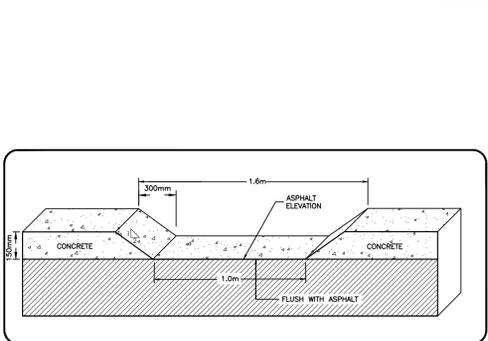
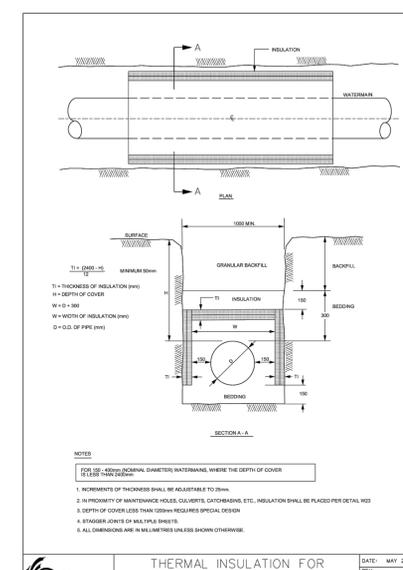
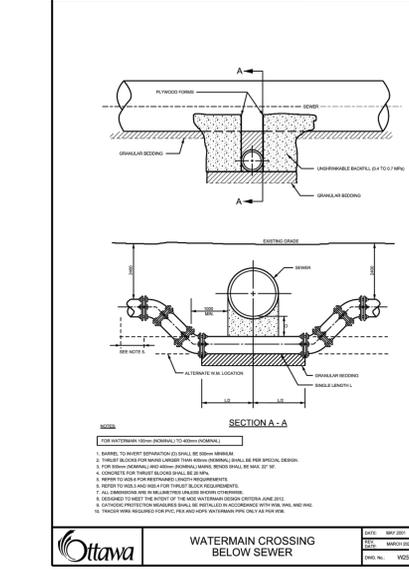
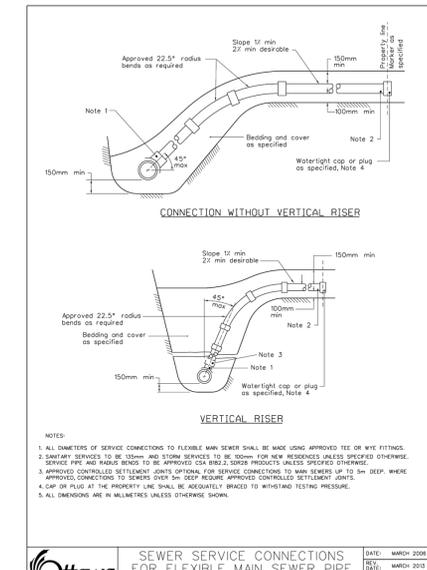
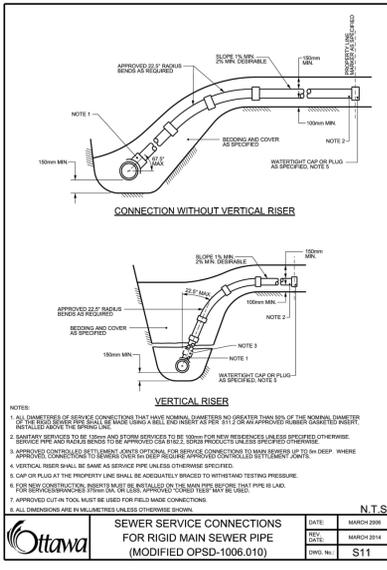
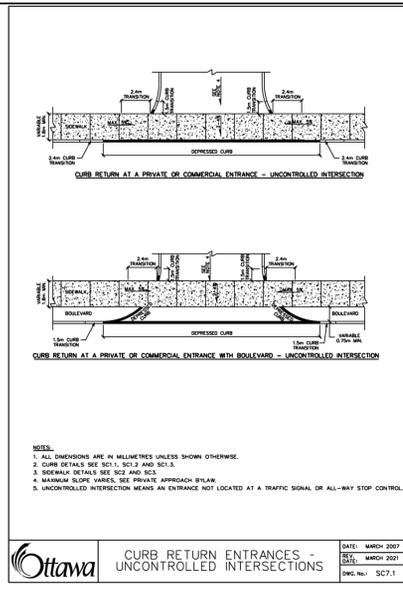
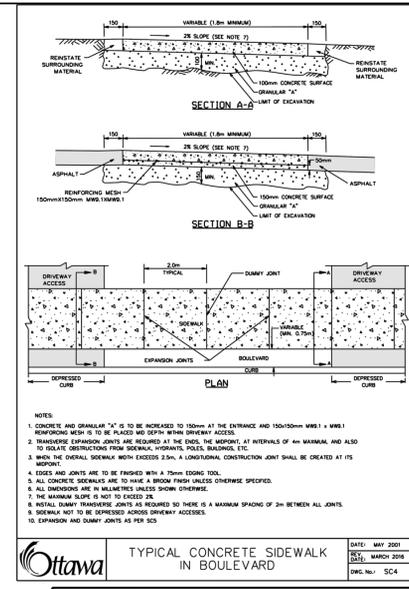
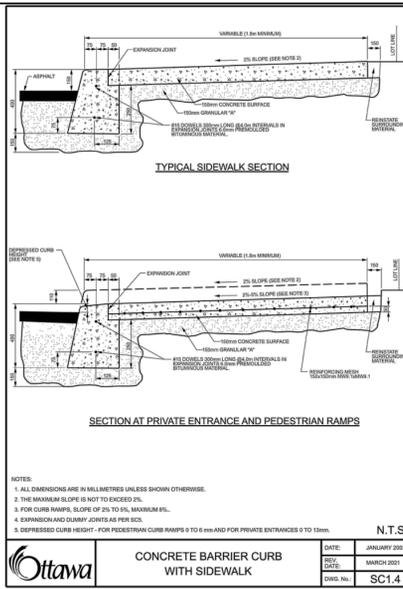
Client/Project
CAVAGH CONSTRUCTION LTD.

TRAILS WEST

Ottawa ON Canada

Title
COPE DRIVE UNITS
EROSION CONTROL PLAN

Project No.	Scale
160401718	AS NOTED
Drawing No.	Sheet
	Revision



GENERAL NOTES AND SPECIFICATIONS

1. ALL MATERIALS AND CONSTRUCTION METHODS TO BE IN ACCORDANCE WITH OPS AND CITY OF OTTAWA STANDARD SPECIFICATIONS AND DRAWINGS AND OPS SUPPLEMENT, ONTARIO PROVINCIAL STANDARDS WILL APPLY WHERE NO CITY STANDARDS ARE AVAILABLE.
2. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED AND BEAR COST OF SAME INCLUDING WATER FORMAL AND ASSOCIATED COSTS.
3. SERVICE AND UTILITY LOCATIONS ARE APPROXIMATE. CONTRACTOR TO VERIFY LOCATION AND ELEVATION OF EXISTING SERVICES AND UTILITIES PRIOR TO CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING LOCATES FROM ALL UTILITY COMPANIES TO LOCATE EXISTING UTILITIES PRIOR TO EXCAVATION. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTION AND REINSTATEMENT.
4. ALL DISTURBED AREAS SHALL BE RESTORED TO EQUAL OR BETTER CONDITION TO THE SATISFACTION OF THE ENGINEER & THE CITY. PAVEMENT REINSTATEMENT FOR SERVICE AND UTILITY CUTS SHALL BE IN ACCORDANCE WITH OPSD 509.010 AND OPS 310.
5. ALL WORK SHALL BE COMPLETED IN ACCORDANCE WITH THE "OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATION FOR CONSTRUCTION PROJECTS". THE GENERAL CONTRACTOR SHALL BE DEEMED TO BE THE CONTRACTOR AS DEFINED IN THE ACT.
6. THE CONTRACTOR SHALL SUBMIT AN EROSION AND SEDIMENTATION CONTROL PLAN THAT WILL IMPLEMENT BEST MANAGEMENT PRACTICES TO PROVIDE PROTECTION FOR RECEIVING STORM SEWERS OR DRAINAGE DURING CONSTRUCTION ACTIVITIES. THIS PLAN SHALL INCLUDE BUT NOT LIMITED TO FILTER CLOTH ON CATCH BASINS, STRAW BALE CHECK DAMS AND TRAPMENTS CONTROLS AROUND ALL DISTURBED AREAS. DEWATERING SHALL BE PUMPED INTO SEDIMENT TRAPS.
7. SITE PLAN PREPARED BY DAVID BLAKELY ARCHITECT, INC.
8. TOPOGRAPHIC SURVEY SUPPLIED BY ANNIS, O'SULLIVAN, VOLLEBECK LTD.
9. LANDSCAPE ARCHITECTURE PLAN PREPARED BY OTHERS. REFER TO ORIGINAL LANDSCAPE ARCHITECTURE PLAN FOR ALL LANDSCAPING FEATURES (e.g. TREES, WALKWAYS, PARK DETAILS, NOISE BARRIERS, FENCES ETC.)
10. GEOTECHNICAL INVESTIGATION 06-124-3 PREPARED BY HOULE CHEVRIER ENGINEERING DATED MAY 2010 AND GEOTECHNICAL MEMO 10212003 PREPARED BY CENTEC JUNE 29, 2003. GEOTECHNICAL INFORMATION PRESENTED ON THESE DRAWINGS MAY BE INTERPOLATED FROM THE ORIGINAL REPORT. REFER TO ORIGINAL GEOTECHNICAL REPORT FOR ADDITIONAL DETAILS AND TO VERIFY ASSUMPTIONS MADE HEREIN.
11. STREET LIGHTING TO CITY OF OTTAWA STANDARDS.
12. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE STATED. DIMENSIONS SHALL BE CHECKED AND VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES TO BE REPORTED IMMEDIATELY TO ENGINEER.
13. THERE WILL BE NO SUBSTITUTION OF MATERIALS UNLESS PRIOR WRITTEN APPROVAL BY THE CONTRACT ADMINISTRATOR AND DIRECTOR OF ENGINEERING HAS BEEN OBTAINED.
14. HERITAGE OPERATIONS UNIT OF THE ONTARIO MINISTRY OF CULTURE TO BE NOTIFIED IF DEEPLY BURIED ARCHAEOLOGICAL REMAINS ARE FOUND ON THE PROPERTY DURING CONSTRUCTION ACTIVITIES.

ROADWORKS

1. ALL TOPSOIL AND ORGANIC MATERIAL TO BE STRIPPED FROM WITHIN THE FULL RIGHT OF WAY PRIOR TO CONSTRUCTION.
 2. SUB-EXCAVATE SOFT AREAS & FILL WITH GRANULAR "B" COMPACTED IN 0.30m LAYERS.
 3. ALL GRANULAR FOR ROADS SHALL BE COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR MAXIMUM DRY DENSITY (SPMD).
 4. ROAD SUBDRAINS SHALL BE CONSTRUCTED AS PER CITY OF OTTAWA STANDARD R1.
 5. ASPHALT WEAR COURSE SHALL NOT BE PLACED UNTIL THE VIDEO INSPECTION OF SEWERS & NECESSARY REPAIRS HAVE BEEN CARRIED OUT TO THE SATISFACTION OF THE CONSULTANT.
 6. CONTRACTOR TO OBTAIN A ROAD OCCUPANCY PERMIT 48 HOURS PRIOR TO COMMENCING ANY WORK WITHIN THE MUNICIPAL ROAD ALLOWANCE IF REQUIRED BY THE MUNICIPALITY. ALL WORK ON THE MUNICIPAL RIGHT OF WAY AND EASEMENTS TO BE INSPECTED BY THE MUNICIPALITY PRIOR TO BACKFILLING.
 7. PAVEMENT REINSTATEMENT FOR SERVICE AND UTILITY CUTS SHALL BE IN ACCORDANCE WITH CITY OF OTTAWA STANDARD R10 AND OPSD 509.010, AND OPS 310.
 8. CONCRETE CURBS SHALL BE CONSTRUCTED AS PER CITY STANDARD SC1.1 AND SC1.3 (BARRIER OR MOUNTABLE CURBS AS SHOWN ON DRAWINGS).
 9. CONCRETE SIDEWALKS SHALL BE CONSTRUCTED AS PER CITY STANDARDS SC3 AND SC1.4.
 10. PAVEMENT CONSTRUCTION AS PER GEOTECHNICAL INVESTIGATION PREPARED BY HOULE CHEVRIER ENGINEERING LTD. DATED MAY 2010.
- AREAS WITH NO BUS TRAFFIC AND WHERE SUBGRADES OF PEAT AND REPLACEMENT WITH BLAST ROCK HAS BEEN CARRIED OUT (MIN. 300mm SUITABLE BLAST ROCK MATERIAL)
- 40mm SUPERPAVE 12.5
 - 50mm SUPERPAVE 19.0
 - 150mm OPSS GRANULAR A BASE
 - 150mm OPSS GRANULAR B TYPE II SUBBASE
- AREAS WITH NO BUS TRAFFIC AND WHERE NATIVE SOILS EXIST AT ROADWAY SUBGRADE ELEVATION
- 40mm SUPERPAVE 12.5
 - 50mm SUPERPAVE 19.0
 - 100mm SUPERPAVE 19.0
 - 150mm OPSS GRANULAR A BASE
 - 450mm OPSS GRANULAR B TYPE II SUBBASE
- AREAS WITH BUS TRUCK TRAFFIC AND WHERE NATIVE SOILS EXIST AT ROADWAY SUBGRADE ELEVATION
- 40mm SUPERPAVE 12.5
 - 50mm SUPERPAVE 19.0
 - 100mm SUPERPAVE 19.0
 - 150mm OPSS GRANULAR A BASE
 - 450mm OPSS GRANULAR B TYPE II SUBBASE

WATER SUPPLY SERVICING

1. THE CONTRACTOR SHALL CONSTRUCT WATERMAIN, WATER SERVICES, CONNECTIONS & APPURTENANCES AS PER CITY OF OTTAWA SPECIFICATIONS & SHALL CO-ORDINATE AND PAY ALL RELATED COSTS INCLUDING THE COST OF CONNECTION, INSPECTION & DISSECTION BY CITY RESIDENTS.
2. WATERMAIN PIPE MATERIAL SHALL BE PVC CL-150 DR16. CONNECTION OF WATERMAIN PIPE IS NOT TO EXCEED 10% OF THAT SPECIFIED BY THE MANUFACTURER. PVC WATERMAINS TO BE INSTALLED WITH TRACER WIRE IN ACCORDANCE WITH CITY OF OTTAWA STANDARD W36.
3. WATER SERVICES TO BE TYPE K SOFT COPPER AS PER CITY OF OTTAWA STANDARD W28 (UNLESS OTHERWISE NOTED); WATER SERVICES WITHIN RIGHT OF WAY TO EXTEND 2.0m BEYOND PROPERTY LINE. STAND POST TO BE INSTALLED AT PROPERTY LINE. WATER SERVICES WITHIN PRIVATE BLOCKS TO EXTEND TO 1.5m FROM PROPOSED FOUNDATIONS. STAND POST TO BE INSTALLED AS SHOWN ON THE SITE SERVICING PLAN (CROSS-SECTION TO BE CONFIRMED PRIOR TO CONSTRUCTION).
4. FIRE HYDRANTS TO BE INSTALLED AS PER CITY OF OTTAWA STANDARDS W18 AND W19.
5. WATER VALVES TO BE INSTALLED AS PER CITY OF OTTAWA STANDARD W4.
6. WATERMAIN TRENCH AND BEDDING SHALL BE IN ACCORDANCE WITH CITY OF OTTAWA STD. W17 UNLESS OTHERWISE SPECIFIED. BEDDING AND COVER MATERIAL TO BE SPECIFIED BY PROJECT GEOTECHNICAL CONSULTANT.
7. SERVICE CONNECTIONS SHALL BE INSTALLED A MINIMUM OF 340mm FROM ANY CATCH BASIN, MANHOLE, OR OBJECT THAT MAY CONTRIBUTE TO FREEZING. THERMAL INSULATION SHALL BE INSTALLED ON ALL PROPOSED CBS ON THE W/M STREET SIDE WHERE 240mm SEPARATION CANNOT BE ACHIEVED (AS PER CITY OF OTTAWA W22 & W23).
8. CATHODIC PROTECTION TO BE SUPPLIED ON METALLIC FITTINGS AS PER CITY OF OTTAWA W40 AND W42.
9. THRUST BLOCKS TO BE INSTALLED AS PER CITY OF OTTAWA STANDARDS W25.3 AND W25.4.
10. WATERMAIN TO HAVE MIN. 2.4m COVER. WHERE WATERMAIN COVER IS LESS THAN 2.4m, INSULATION TO BE SUPPLIED IN ACCORDANCE WITH CITY STANDARD W22.
11. WATERMAIN CROSSINGS ABOVE AND BELOW SEWERS TO BE INSTALLED AS PER CITY OF OTTAWA STANDARD W25 AND W25.2.
12. PRESSURE REDUCING VALVES (PRVs) TO BE INSTALLED AS PER ONTARIO PLUMBING CODE.
13. CONNECTIONS TO EXISTING WATERMAIN ARE TO BE DONE BY CITY FORCES. EXCAVATION AND BACKFILL ARE TO BE PROVIDED BY CONTRACTOR.

STORM AND SANITARY SEWERS

1. SANITARY SEWERS 375mm DIA. OR SMALLER SHALL BE PVC SDR35. SANITARY SEWERS LARGER THAN 375mm SHALL BE CONCRETE CSA A 257.2 CLASS 1000 AS PER OPSD 807.010.
2. STORM SEWERS 375mm DIA. OR SMALLER SHALL BE PVC SDR 35. STORM SEWERS LARGER THAN 375mm DIA. SHALL BE CONCRETE CSA A 257.2 CLASS 1000 AS PER OPSD 807.010.
3. ALL STORM AND SANITARY SEWER BEDDING SHALL BE INSTALLED AS PER CITY OF OTTAWA STANDARDS S6 AND S7. CLASS "B" BEDDING UNLESS OTHERWISE NOTED. SUITABLE BEDDING AND COVER MATERIAL TO BE SPECIFIED BY GEOTECHNICAL CONSULTANT.
4. STORM AND SANITARY MANHOLES SHALL BE 1200mm DIAMETER IN ACCORDANCE WITH OPSD-701.01 (UNLESS OTHERWISE NOTED) w/ FRAME AND COVER AS PER CITY OF OTTAWA S24 AND S25. ALL STORM MANHOLES WITH SEWERS 800mm DIA. SEWERS AND OVER IN GRANULAR SHALL BE FINISHED AS PER CITY OF OTTAWA S24. ALL OTHER STORM MANHOLES SHALL BE COMPLETED WITH 300mm HUMPS AS PER CITY STANDARDS. SANITARY MANHOLES SHALL NOT HAVE HUMPS.
5. ALL SEWERS CONSTRUCTED WITH GRADES 0.50% OR LESS, TO BE INSTALLED WITH LASER AND CHECKED WITH LEVEL INSTRUMENT PRIOR TO BACKFILLING.
6. FOR STORM SEWER INSTALLATION (EXCLUDING OB LEADS) THE MINIMUM DEPTH OF COVER OVER THE CROWN OF THE SEWER IS 2.0m. FOR SANITARY SEWERS THE MINIMUM DEPTH OF COVER IS 2.5m OVER PIPE OBVERT.
7. SAFETY PLATFORMS SHALL BE INSTALLED IN ACCORDANCE WITH OPSD 404.02.
8. DROP STRUCTURES TO BE INSTALLED AS PER CITY OF OTTAWA SPECIFICATIONS AND OPSD 1003.01
9. ALL STORM AND SANITARY SERVICES TO BE EQUIPPED WITH APPROVED BACKWATER VALVES.
10. STORM AND SANITARY SERVICE LATERALS TO BE SDR 28 INSTALLED AT MIN. 1.0% SLOPE. SINGLE STORM SERVICES TO BE 100mm. SINGLE SANITARY SERVICES TO BE 150mm. (SERVICES WITHIN RIGHT OF WAY'S TO EXTEND 2.0m BEYOND PROPERTY LINE. SERVICES WITHIN PRIVATE BLOCKS TO EXTEND TO 1.5m FROM PROPOSED FOUNDATIONS.)
11. CATCH BASINS SHALL BE IN ACCORDANCE WITH CITY STANDARDS w/ FRAME AND GRATE AS PER S20, AND S21 FOR REAR YARDS, AND S3 FOR STREET CBS. PROVIDE 150mm ADJUSTED SPACERS. ALL CATCH BASINS SHALL HAVE HUMPS (600mm DEEP). STREET CATCH BASIN LEADS SHALL BE 200mm DIA (MM) PVC SDR 35 AT 1.0% GRADE WHERE NOT OTHERWISE SHOWN ON PLAN. CATCH BASINS WILL BE INSTALLED WITH INLET CONTROL DEVICES (ICD) AS PER ICD SCHEDULE ON STORM DRAINAGE PLAN.
12. STREET CATCH BASINS TO BE INSTALLED w/ SUBDRAINS 3m LONG IN FOUR OR MORE DIRECTIONS OR LONGITUDINALLY WHEN PLACED ALONG A CURB, AND AT AN ELEVATION OF 300mm BELOW SUBGRADE LEVEL.
13. REAR LOT PERFORATED PIPE TO BE INSTALLED AS PER CITY OF OTTAWA STANDARDS S29. REAR LOT STRUCTURES TO BE INSTALLED AS PER CITY OF OTTAWA STANDARD W30 AND W31.
14. SEEPAGE BARRIERS (AS PER S8) SHOULD BE INSTALLED ALONG THE SERVICE TRENCHES AT STRATEGIC LOCATIONS. THE SEEPAGE BARRIERS SHOULD BEGIN AT SUBGRADE LEVEL AND EXTEND VERTICALLY THROUGH THE GRANULAR PIPE BEDDING AND GRANULAR SURROUND TO WITHIN THE NATIVE BACKFILL MATERIALS AND HORIZONTALLY ACROSS THE FULL WIDTH OF THE SERVICE TRENCH EXCAVATION. THE SEEPAGE BARRIERS SHOULD CONSIST OF 1.5m DEEP DYKES OF COMPACTED SILTY CLAY. THE SILTY CLAY SHOULD BE COMPACTED IN MAXIMUM 300mm THICK LIFTS TO AT LEAST 95% OF THE MATERIALS SPECIFIED. THE CLAY SEALS SHOULD BE PLACED AT THE SITE BOUNDARIES AND AT STRATEGIC LOCATIONS AT NO MORE THAN 60m INTERVALS IN THE SERVICE TRENCHES. FOR DETAILS REFER TO GEOTECHNICAL INVESTIGATION.
15. GRANULAR "A" SHALL BE PLACED TO A MINIMUM THICKNESS OF 300mm AROUND ALL STRUCTURES WITHIN PAVEMENT AREA AND COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY.
16. CONTRACTOR SHALL PERFORM LEAKAGE TESTING, IN THE PRESENCE OF THE CONSULTANT, FOR SANITARY SEWERS IN ACCORDANCE WITH OPS 410 AND OPS 407. CONTRACTOR SHALL PERFORM VIDEO INSPECTION OF ALL STORM AND SANITARY SEWERS. A COPY OF THE VIDEO AND INSPECTION REPORT SHALL BE SUBMITTED TO THE CONSULTANT FOR REVIEW.

- GRADING**
1. ALL GRANULAR BASE & SUB BASE MATERIALS SHALL BE COMPACTED TO 98% STANDARD PROCTOR MAX. DRY DENSITY.
 2. SUB-EXCAVATE SOFT AREAS & FILL WITH GRANULAR "B" COMPACTED IN 0.15m LAYERS.
 3. ALL DISTURBED GRASSED AREAS SHALL BE RESTORED TO ORIGINAL CONDITION OR BETTER, WITH SOD ON MIN. 100mm TOPSOIL. THE RELOCATION OF TREES AND SHRUBS SHALL BE SUBJECT TO APPROVAL BY THE PROJECT LANDSCAPE ARCHITECT OR ENGINEER.
 4. 100 YEAR PONDING DEPTH TO BE 0.30m (MAXIMUM).
 5. EMBANKMENTS TO BE SLOPED AT MIN. 3:1 UNLESS OTHERWISE SPECIFIED.
 6. ALL SWALES TO BE MIN. 0.15m DEEP WITH MAX. 3:1 SIDE SLOPES UNLESS OTHERWISE NOTED. THE MINIMUM LONGITUDINAL SLOPE TO BE 1.5% OR 1.0% WHEN PERFORATED SUBORAN IS INSTALLED.
 7. ALL ROOF DOWNSPOUTS TO DISCHARGE TO THE GROUND ONTO SPLASH PADS AND SHALL NOT BE DIRECTED TO THE STORM SEWER, OR THE BUILDING FOUNDATION DRAIN.
 8. TOP OF GRATE (T.O.G) ELEVATIONS FOR ALL STREET CATCHBASINS SHOWN ON PLANS REFER TO THE ELEVATION AT EDGE OF PAVEMENT, OR OUTLINE WHERE APPLICABLE.
 9. ALL RETAINING WALLS GREATER THAN 1.0m IN HEIGHT ARE TO BE DESIGNED, APPROVED, AND STAMPED BY STRUCTURAL ENGINEER.
 10. FENCES OR RAILINGS ARE REQUIRED FOR RETAINING WALLS GREATER THAN 0.6m IN HEIGHT.
 11. EXCESS EXCAVATED MATERIAL SHALL BE REMOVED FROM THE SITE.
 12. ALL NECESSARY CLEARING AND GRUBBING SHALL BE COMPLETED BY THE CONTRACTOR. REVIEW WITH CONTRACT ADMINISTRATOR AND THE CITY OF OTTAWA PRIOR TO TREE CUTTING.
 13. REFER TO DRAWING EC-1 FOR EROSION AND SEDIMENT CONTROL DETAILS.
 14. FROST PROTECTION TO BE PROVIDED FOR FOUNDATIONS WHERE DEPTH OF COVER IS LESS THAN 1.5m AS PER GEOTECHNICAL RECOMMENDATIONS.

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Legend

- PROPOSED STORM SEWER
- PROPOSED DRAINAGE BOUNDARY
- PROPOSED CATCHBASIN
- ST101A
C=0.38
0.70
- C=COEFFICIENT VALUE
- DRAINAGE AREA (HA)
- DIRECTION OF OVERLAND FLOW
- WATTS AREA DRAIN OR EQUIVALENT
- EXISTING STORM SEWER
- EXISTING CATCHBASIN MANHOLE
- EXISTING CATCHBASIN
- EXISTING SUBDRAIN CATCHBASIN

Notes

- ALL PROPOSED BUILDINGS TO BE EQUIPPED WITH BACKWATER VALVES FOR FOUNDATION DRAINAGE.



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Client/Project
CAVANAUGH CONSTRUCTION LTD.

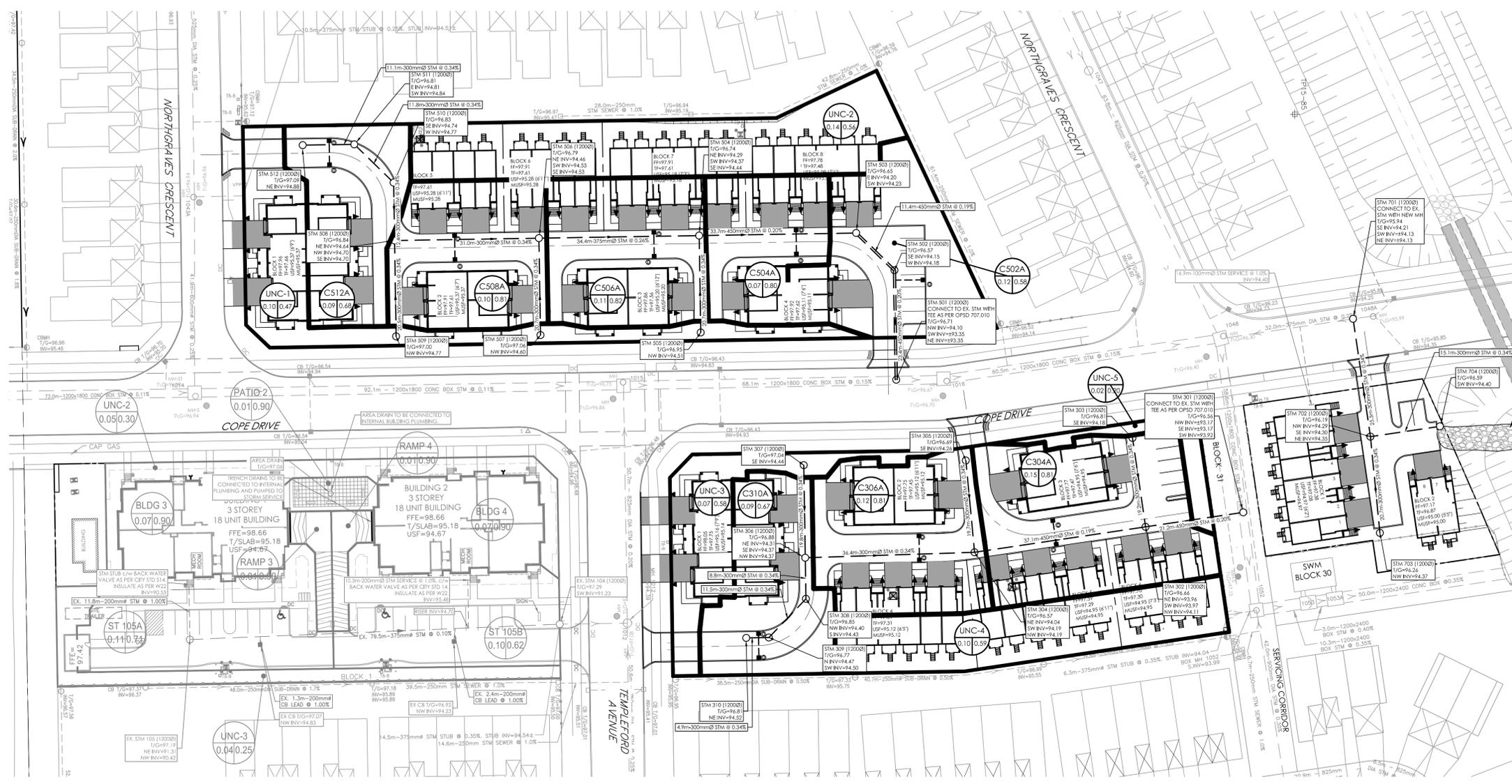
TRAILS WEST

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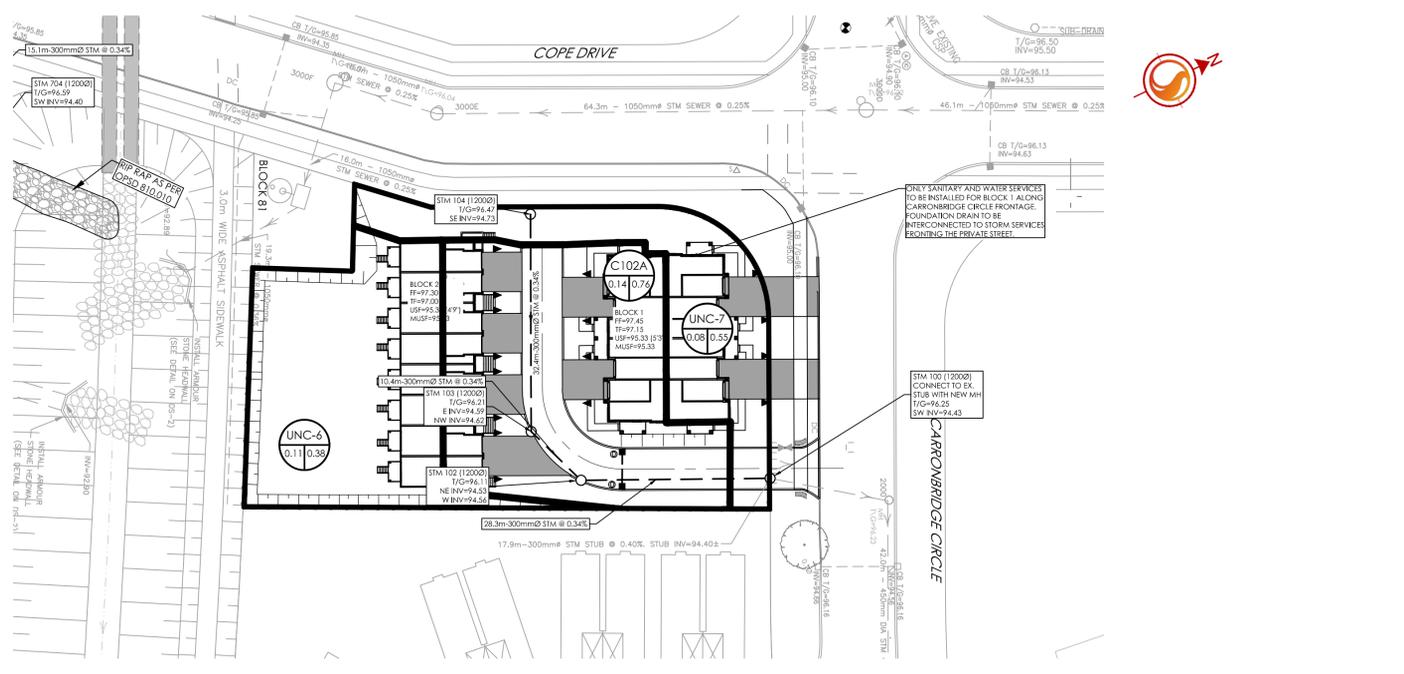
Title
COPE DRIVE UNITS
STORM DRAINAGE PLAN

Project No.	Scale	Sheet	Revision
160401718	1:500	SD-1	1

SD-1 5 of 6



STRUCTURE ID	DRAINAGE AREA ID	ORIFICE SIZE/TYPER	ORIFICE INVERT (m)	100 YEAR HEAD (m)	100 YEAR RELEASE RATE (L/S)
CB102A-1	C102A	100mm IPX TEMPEST HF	94.70	1.51	23
CB102A-2	C102A	100mm IPX TEMPEST HF	94.70	1.51	23
CB304A-1	C304A	85mm IPX TEMPEST HF	95.07	1.53	22
CB304A-2	C304A	85mm IPX TEMPEST HF	95.07	1.53	22
CB306A-1	C306A	83mm IPX TEMPEST HF	95.10	1.51	17
CB306A-2	C306A	83mm IPX TEMPEST HF	95.10	1.51	17
CB310A-1	C310A	83mm IPX TEMPEST HF	95.31	1.47	16
CB310A-2	C310A	83mm IPX TEMPEST HF	95.31	1.47	16
CB502A-1	C502A	100mm IPX TEMPEST HF	95.10	1.50	25
CB504A-1	C504A	75mm IPX TEMPEST HF	95.19	1.47	13
CB504A-2	C504A	75mm IPX TEMPEST HF	95.19	1.47	13
CB506A-1	C506A	83mm IPX TEMPEST HF	95.22	1.51	17
CB506A-2	C506A	83mm IPX TEMPEST HF	95.22	1.51	17
CB508A-1	C508A	75mm IPX TEMPEST HF	95.18	1.52	14
CB508A-2	C502A	83mm IPX TEMPEST HF	95.18	1.52	17
CB512A-1	C512A	75mm IPX TEMPEST HF	95.25	1.52	14
CB512A-2	C512A	75mm IPX TEMPEST HF	95.25	1.52	12



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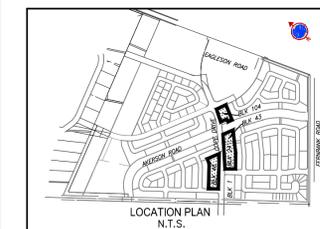
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Legend

- SANITARY DRAINAGE AREA ID#
- POPULATION
- SANITARY DRAINAGE AREA ha.
- SANITARY DRAINAGE AREA
- PROPOSED SANITARY SEWER
- EXISTING SANITARY SEWER

Notes

- ALL PEAT AND BLAST ROCK INFORMATION TO BE CONFIRMED BY GEOTECHNICAL ENGINEER.
- ALL ENGINEERED FILL AND GRADE RAISES TO BE APPROVED BY GEOTECHNICAL CONSULTANT PRIOR TO PLACING OF MATERIAL.



ISSUED FOR REVIEW	WAJ	KJK	23.06.19
Revision	By	Appd.	TY.MM.DD
File Name: 160401718-08	WAJ	KJK	WAJ 21.12.20
Permit-Seal	Dwn.	Chkd.	Dgn. TY.MM.DD



Client/Project
CAVANAUGH CONSTRUCTION LTD.

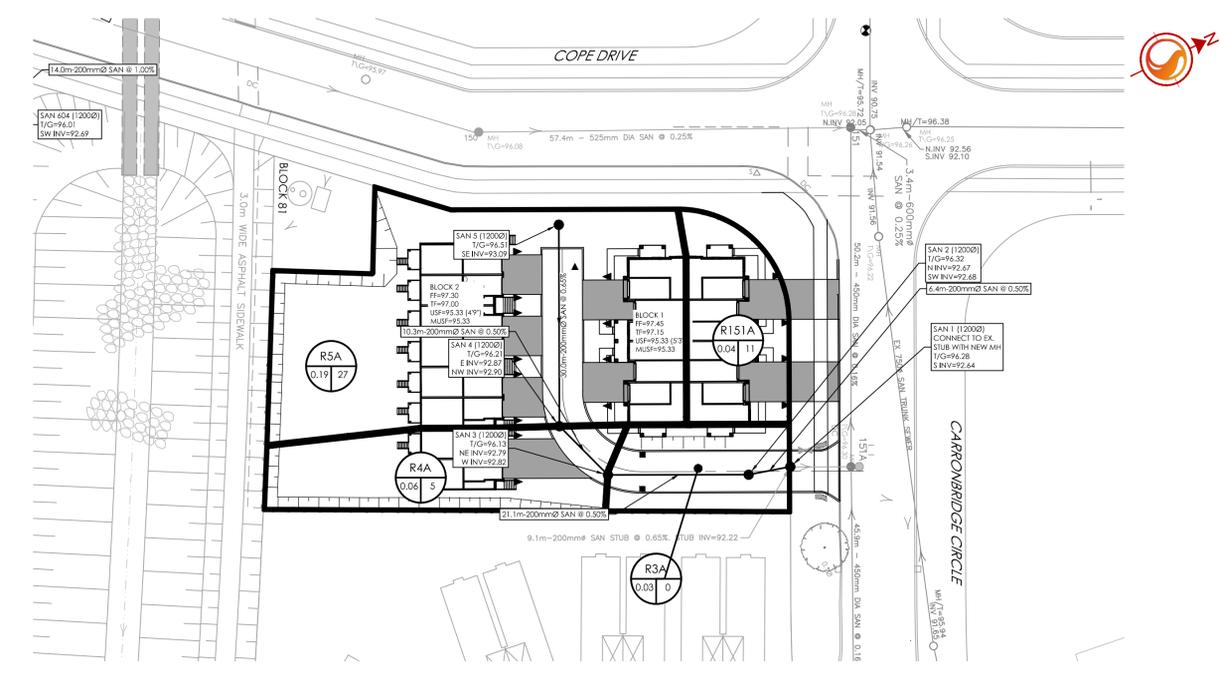
TRAILS WEST

Ottawa ON Canada

Title
COPE DRIVE UNITS
SANITARY DRAINAGE PLAN

Project No. 160401718	Scale 1:500	Sheet SA-1	Revision 1
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SA-1 6 of 6



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