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Merivale Medical Imaging Clinic 1545A Merivale Road Ottawa, Ontario

Servicing Design Brief



Engineering excellence.

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**MERIVALE MEDICAL IMAGING CLINIC
1545A MERIVALE ROAD
OTTAWA, ONTARIO**

**SERVICING DESIGN BRIEF
IN SUPPORT OF A
SITE PLAN CONTROL APPLICATION**

Prepared For:

1545A Merivale Inc.

Prepared By:



NOVATECH
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
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December 23, 2022

Novatech File: 122098
Ref: R-2022-207

December 23, 2022

City of Ottawa
Infrastructure Services and Community Sustainability
110 Laurier Avenue West, 4th Floor
Ottawa, ON K1P 1J1

Attention: Colette Gorni, Planner II

**Reference: Merivale Medical Imaging Clinic
1545A Merivale Road
Servicing Design Brief
Our File No.: 122098**

Enclosed for your review and approval is the Servicing Design Brief for the proposed Merivale Medical Imaging Clinic at 1545A Merivale Road in support of the site plan control application.

If you have any questions or comments, please do not hesitate to contact us.

Sincerely,

NOVATECH



Trevor McKay, E.I.T.
Project Manager

cc: Dr. Keyur Shah, MMI
Dr. Nirav Patal, MMI

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1.0 INTRODUCTION

1.1 Background

Novatech has been retained to prepare a Servicing Design Brief for the proposed Merivale Medical Imaging Clinic, located at 1545A Merivale Road in the City of Ottawa. The site will be developed by 1545A Merivale Inc. This report is being submitted in support of a Site Plan Control application.

1545A Merivale Inc. is proposing to construct a one-storey medical imaging facility at 1545A Merivale Road in Ottawa, Ontario. The site is located approximately 1.1 kilometers southwest of the intersection of Merivale Road and Baseline Road, along the arterial of Merivale Road in the former municipality of Nepean. **Figure 1** shows the location of the proposed development.

The site is approximately 6,905 square meters (0.69 ha) in area and is irregularly shaped. A narrow front lot line approximately 4.7 metres in width provides access to the site from Merivale Road, in conjunction with a right-of-way over an adjacent site (1543 Merivale Road). There is currently a 1-storey building on the site which was constructed during the 1950s. The immediate surroundings of the site are other commercial uses.

This Servicing Design Brief provides information on the considerations and approach by which Novatech has analyzed the existing site information for the Merivale Medical Imaging Clinic development, and details how the development lands will be serviced while meeting the City of Ottawa requirements and all other relevant regulations.

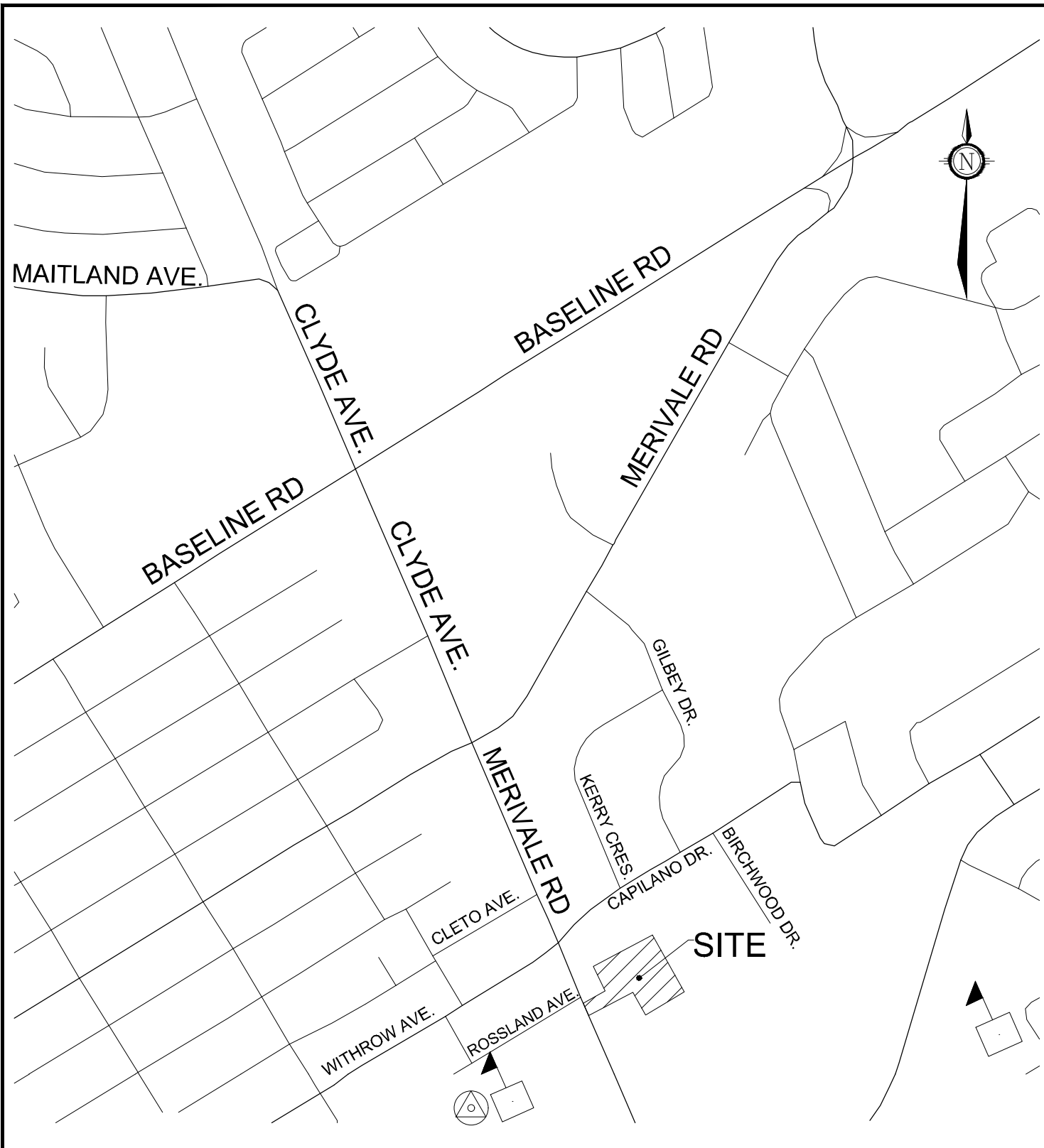
This report should be read in conjunction with the following:

- Geotechnical Investigation, Proposed Commercial Building, 1545 Merivale Road, Ottawa, Ontario, prepared by Paterson Group, dated August 3, 2022 (Report: PG6288-1). **[1]**

1.2 Proposed Development

The site will consist of a new one-storey medical imaging facility with underground parking. The proposed Site Plan is shown in **Figure 2**. Site modifications associated with the proposed development include:

- Demolition of the existing building;
- Construction of a 1-storey medical imaging building of approximately 2,481 square meters;
- Creation of a surface parking lot containing 72 vehicle spaces, including 2 accessible spaces;
- Construction of one level of underground parking containing 57 spaces, to be accessed via a ramp at the rear of the building;
- Installation of a 1.5-meter sidewalk to provide safe pedestrian access to the main building entrance from Merivale Road;
- Provision of bicycle parking at the front of the building;
- Landscaping elements interspersed throughout the parking lot and around the edges of the site, along with a landscaped outdoor amenity area for staff at the rear of the building;
- Installation of water, sanitary and storm infrastructure to service the new building.



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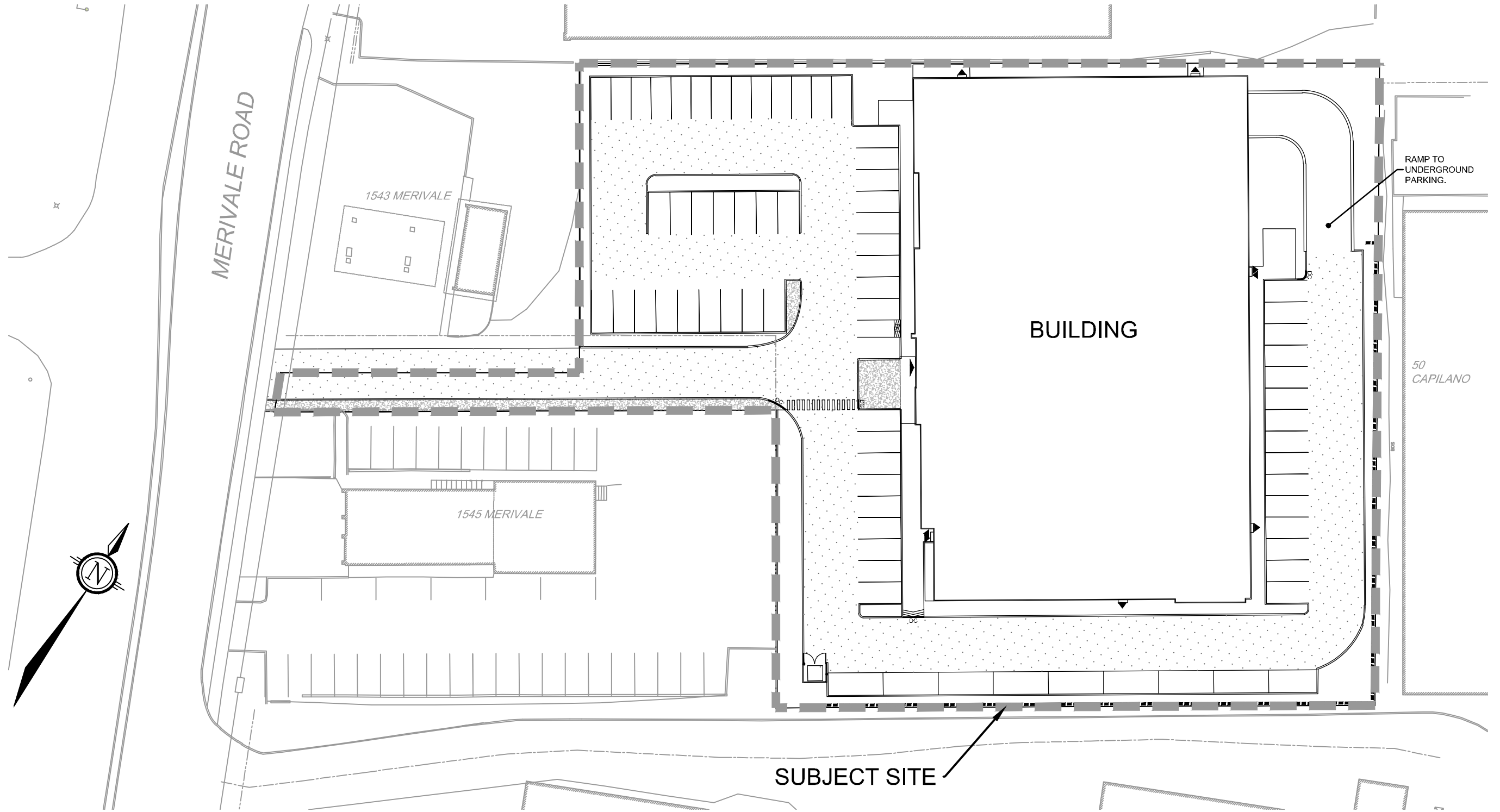
CITY of OTTAWA
 MERIVALE MEDICAL IMAGING CLINIC
 (1545A MERIVALE ROAD)

KEY PLAN

N.T.S

| | | |
|----------|--------|--------|
| DATE | JOB | FIGURE |
| DEC 2022 | 122098 | FIG-1 |

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SITE PLAN

SCALE 1 : 500

| | | |
|----------|--------|--------|
| DATE | JOB | FIGURE |
| DEC 2022 | 122098 | FIG-2 |

1.3 Pre-Consultation Information

A pre-consultation meeting was held with the City of Ottawa on June 16, 2022, at which time the client was advised of the general submission requirements. The Rideau Valley Conservation Authority (RVCA) was also consulted regarding the proposed development. Refer to **Appendix A** for a summary of the correspondence related to the proposed development.

2.0 SITE GRADING

2.1 Existing Conditions

The site has an existing vacant commercial building. The remainder of the site is primarily composed of asphalt-paved parking areas with some gravel towards the rear (east) of the site. The site slopes to the east (back) of the site. The site is partially graded to direct flows to two existing catch basins in the middle of the site. These catchbasins outlet to an offsite maintenance hole located on the property to the south of the subject site.

A geotechnical investigation was carried out by Paterson Group [1] indicating varying bedrock elevations across the site. Inferred bedrock depths varied from 3.1-4.6m below existing grade. The groundwater elevation varied between 1.71-2.89m below existing grade in Boreholes 2-22 and 3-22. No groundwater elevation was reported for Borehole 1-22.

2.2 Proposed Conditions

The design grades will tie into existing back of sidewalk elevations along Merivale Road at the entrance and the existing grades on the south side of the right of way along the adjacent property (1543 Merivale Road). Elevations along the north side of the site to the existing commercial plaza, the west of the site (to the rear of 1543 & 1545 Merivale Road) and the south side of the access driveway along the adjacent site (1545 Merivale Road) will terrace down to existing elevations. Retaining walls will be required along the south and east sides of the site abutting 1547 Merivale Road and 50 Capilano Drive, respectively, to tie into existing grades at property line. For detailed grading refer to drawing **122098-GR (Appendix F)**.

The proposed grading will fall within these ranges:

- Landscaped Areas: Minimum 2% - Maximum 6%
- Parking Areas: 0.5% - 5%
- Maximum Terracing Grade of 3H:1V

2.3 Emergency Overland Flow Route

In the case of a major rainfall event exceeding the 100-year design storm provided for, the site will be graded to provide an overland flow route for the stormwater to leave the site. The major system flow route from the subject site will overflow towards Merivale Road. There are several small landscaped areas along the north and northeast portions of the property that are sloped towards the adjacent properties, consistent with the existing drainage conditions. The finished floor elevation of the building is a minimum of 0.30m above the major system overflow points. The emergency overland flow route is shown on drawing **122098-GR (Appendix F)**.

3.0 EROSION AND SEDIMENT CONTROL

Erosion and sediment control measures will be implemented during construction in accordance with the “Guidelines on Erosion and Sediment Control for Urban Construction Sites” (Government of Ontario, May 1987). Typical erosion and sediment control measures recommended include, but are not limited to, the use of silt fences around perimeter of site, filter fabric or inserts under catch basin/maintenance hole lids, heavy duty silt fence barrier, straw bale check dams, rock check dams, turbidity curtain, dewatering trap, temporary water passage system, riprap, mud mats, silt bags for dewatering operations, topsoil and sod to disturbed areas and natural grassed waterways. Dewatering and sediment control techniques will be developed for the individual situations based on the above guidelines and utilizing typical measures to ensure erosion and sediment control is controlled in an acceptable manner and there is no negative impact to adjacent lands, water bodies or water treatment/conveyance facilities.

The following erosion and sediment control measures will be implemented during construction. Details are provided on the Erosion and Sediment Control Plan.

- All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.
- A qualified inspector should conduct daily visits during construction to ensure that the contractor is working in accordance with the design drawings and that mitigation measures are being implemented as specified.
 - A light duty silt fence barrier is to be installed in the locations shown on the Erosion and Sediment Control & Removals Plan (**122098-ESC, Appendix F**).
 - Catchbasin inlet protection measures are to be established for all proposed and existing catchbasins and storm sewer structures.
 - After complete build-out, all sewers are to be inspected and cleaned and all sediment and construction fencing is to be removed.
- The contractor shall ensure that proper dust control is provided with the application of water (and if required, calcium chloride) during dry periods.
- The contractor shall immediately report to the engineer or inspector any accidental discharges of sediment material into any ditch or sewer system. Appropriate response measures shall be carried out by the contractor without delay.
- The contractor acknowledges that failure to implement erosion and sediment control measures may result in penalties imposed by any applicable regulatory agency.

Temporary erosion and sediment control measures would be implemented both prior to commencement and during construction in accordance with the “Guidelines on Erosion and Sediment Control for Urban Construction Sites”, (Government of Ontario, May 1987).

4.0 SANITARY SEWERS

4.1 Existing Conditions

There is an existing 250mm diameter concrete sanitary sewer in Merivale Road. The existing sanitary service lateral on the property was unable to be located. The existing service will be located during the demolition of the existing building and will be abandoned in accordance with City of Ottawa standards.

4.2 Proposed Conditions

The peak design flow parameters in **Table 4-1** have been used in the sewer capacity analysis. Design parameters are specified in the City of Ottawa Sewer Design Guidelines [2].

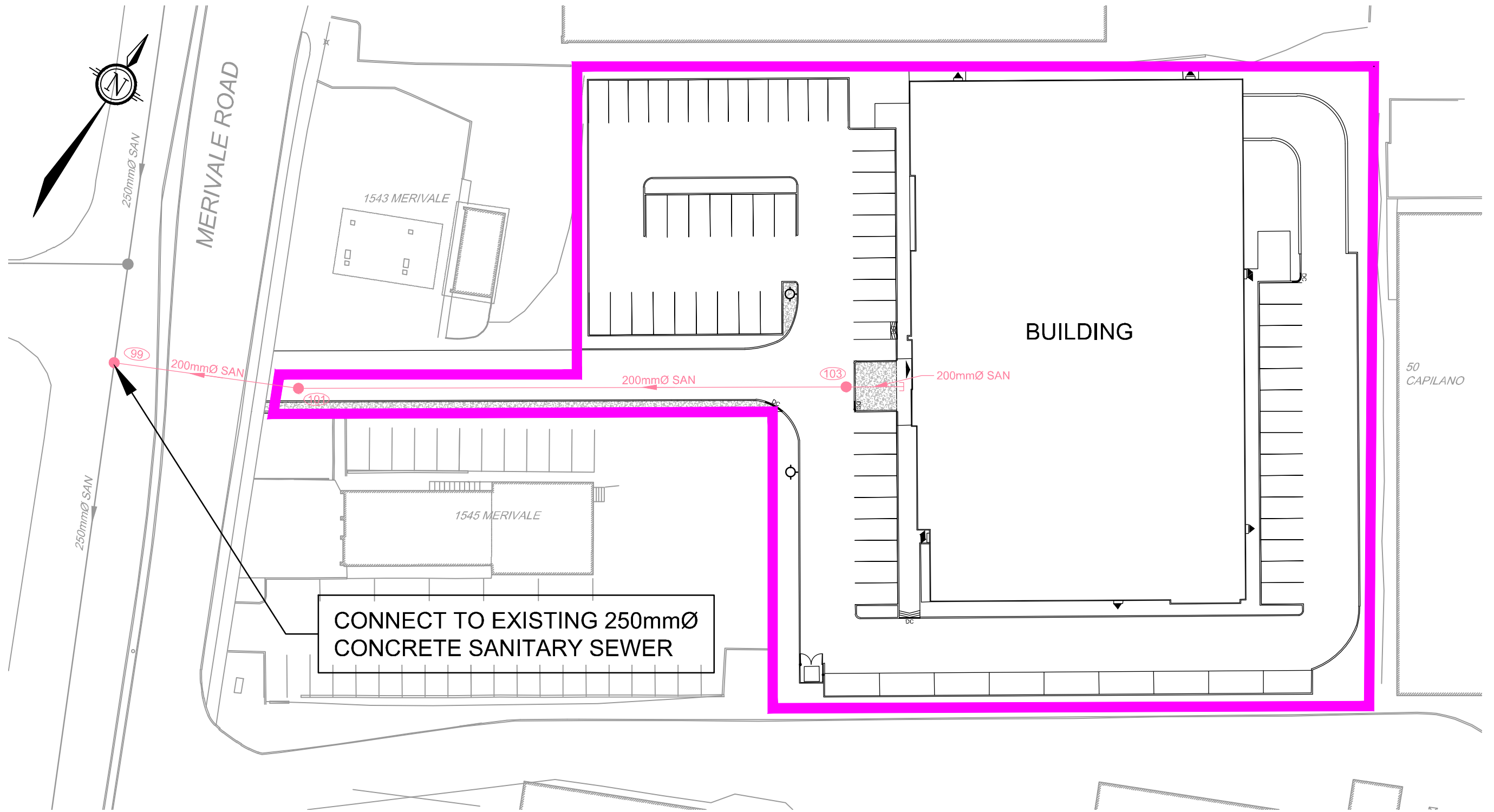
Under post-development conditions, the proposed development will be serviced by a new 200mm diameter sanitary sewer connected to the existing municipal 250mm diameter concrete sanitary sewer located in Merivale Road. A new maintenance hole will be required to be installed on the existing sanitary sewer to facilitate the connection to the existing 250mm diameter rigid pipe sanitary sewer. The sanitary sewer layout is shown in **Figure 3** and the design sheet is attached in **Appendix B**. Sanitary flows from the site will outlet at MH 99 with a peak design flow of 0.83 L/s.




Due to the irregular lot shape, the proposed development only has 4.7m of frontage onto Merivale Road. To provide municipal services to the site, it is proposed to provide 1m (centre to centre) separation between each of the proposed sewers and the watermain. The proposed separation distance is less than the preferred separation distances for mainline sewers (1.5m) and watermains (3m), however the provided distance is greater than the minimum distance required for services (approximately 0.5m) as outlined in the City of Ottawa specifications.

Table 4-1: Sanitary Sewer Design Parameters

| Parameter | Design Parameter |
|---------------------------|-----------------------|
| Gross Area | 0.69 ha |
| Commercial Average Flow | 50,000 L/gross ha/day |
| Commercial Peaking Factor | 1.5 |
| Infiltration Rate | 0.33 L/s/ha |
| Minimum Pipe Size | 200 mm |
| Minimum Velocity | 0.6 m/s |
| Maximum Velocity | 3.0 m/s |

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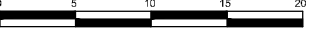


| LEGEND | |
|---|-------------------------|
|  | PROPOSED SANITARY SEWER |
|  | EXISTING SANITARY SEWER |
|  | SITE BOUNDARY |

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SANITARY SEWER NETWORK

SCALE 1 : 500 

DATE DEC 2022 JOB 122098 FIGURE FIG-3

5.0 STORM DRAINAGE & STORMWATER MANAGEMENT

Storm servicing for the proposed development at 1545A Merivale Road will be provided using a dual drainage system. Runoff will be stored and conveyed by underground storm sewers (minor system), while flows from large storm events which exceed the capacity of the minor system will be conveyed overland along defined overland flow routes (major system). Water quality treatment will ultimately be provided by the existing Nepean Creek Stormwater Facility, located at the outlet of the Merivale Road storm sewer.

5.1 Existing Conditions

The site is partially graded to direct flows to two existing catch basins in the middle of the site which outlets to an existing storm maintenance hole on the property to the south of the subject site (1547 Merivale Road). The remainder of the site sheet drains to the adjacent properties to the north (1541 Merivale Road), east (50 Capilano Drive) and south (1547 Merivale Road) of the site. These properties all have storm sewer (minor system) inlets which capture the existing runoff from the site. Based on existing topography, most of the overland drainage leaving the site that is not captured by the adjacent minor system inlets is directed through the adjacent properties to Capilano Drive. Portions of the existing property to the northeast (1543 Merivale Road) currently sheet drain onto the 1545A Merivale Road property.

5.2 Stormwater Management Criteria

The following stormwater management criteria for the proposed development were prepared in accordance with the City of Ottawa Sewer Design Guidelines (October 2012), and Technical Bulletins PIEDTB-2016-01, ISTB-2018-01, ISTB-2018-02, and ISTB-2018-03 [2].

5.2.1 *Minor System (Storm Sewers)*

- Storm sewers are to be designed using the Rational Method for a 2-year return period;
- A maximum minor system release rate calculated based on the pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less, with a time of concentration no less than 10 minutes.

5.2.2 *Major System*

- Provide on-site storage for storm runoff which exceeds the allowable minor system release rate from the site up to and including the 100-year design event;
- Ponding depths are not to exceed 0.35m (static + dynamic) and are not to be within 0.30m (vertical) to the nearest building opening;
- No surface ponding for storms up to and including the 2-year event.

5.2.3 *Water Quality*

- Provide an *Enhanced* level (80% long-term TSS removal) of water quality control;
- Where possible, implement lot-level and conveyance best management practices to maximize the potential for water quality treatment.

5.3 Storm Sewer Design (Minor System)

The proposed storm sewers have been designed using the Rational Method to convey peak flows associated with the 2-year return period. The storm sewer layout is shown in **Figure 4** and the design sheet is provided in **Appendix C**. The corresponding Storm Drainage Area Plan (**122098-STM**) is provided in **Appendix F**.

Table 5-1: Storm Sewer Design Parameters

| Parameter | Design Criteria |
|---|---|
| Private Site | 2 Year Return Period |
| Storm Sewer Design | Rational Method/PCSWMM |
| IDF Rainfall Data | Ottawa Sewer Design Guidelines [2] |
| Initial Time of Concentration (T _c) | 10 min |
| Minimum Velocity | 0.8 m/s |
| Maximum Velocity | 3.0 m/s |
| Minimum Diameter | 250 mm |

Table 5-2: Runoff Coefficients

| Land Use | Runoff Coefficient |
|--------------|--------------------|
| Hard Surface | 0.90 |
| Soft Surface | 0.20 |

5.3.1 Allowable Release Rate

As noted above in the design criteria, the allowable release rate for the site is based on using either the pre-development runoff coefficient, or a C=0.5, whichever is less. Under existing conditions, the site consists of a parking lot, a flat-roofed building, and a small strip of grass which would give an approximate runoff coefficient of C=0.85. As such, a runoff coefficient of C=0.5 has been used in the Rational Method calculation for the allowable release rate. Calculations as follows:

$$Q_{\text{allowable}} = 2.78CiA \text{ where;}$$

$$C = 0.5$$

$$i = 76.81 \text{ mm/hr (for a 2-year event and } T_c \text{ of 10 mins)}$$

$$A = 0.77 \text{ ha (refer to } \mathbf{122098-STM} \text{ for total drainage area tributary to the outlet)}$$

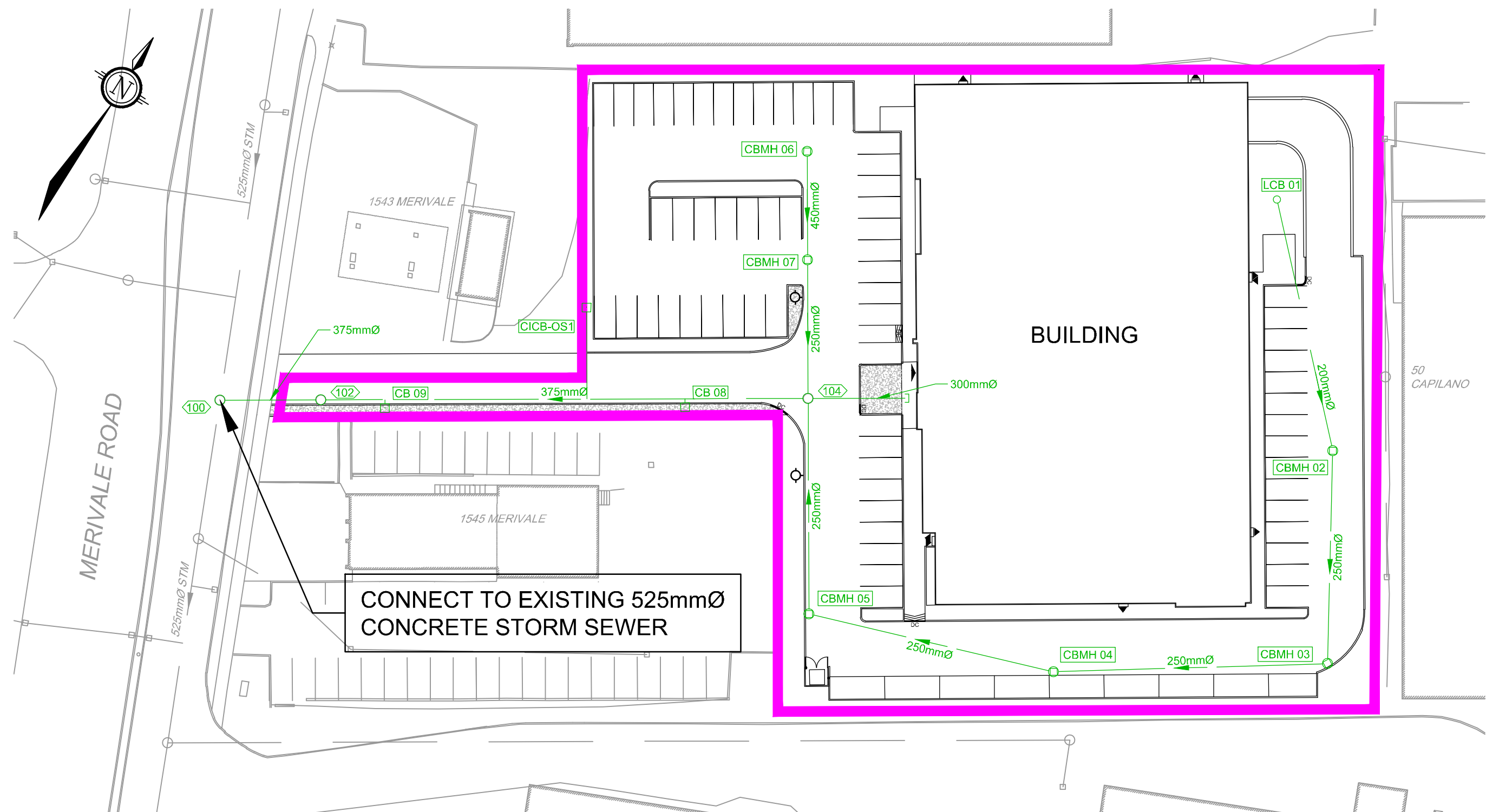
$$Q_{\text{allowable}} = 2.78 * 0.5 * 76.81 * 0.77$$

$$Q_{\text{allowable}} = \mathbf{82.2 \text{ L/s}}$$

5.3.2 Outlet Orifice Controls

Outflows to the proposed water quality unit and ultimately the Merivale Road storm sewers will be controlled using orifices sized to restrict the flow from the proposed site to the allowable release rate for each storm event. This will result in some surface ponding in the parking areas during larger storm events (greater than the 2-year return period). Orifice controls are to be installed in CBMH05, CBMH07, CB08, and CB09 to control the runoff from the parking areas.

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**CONNECT TO EXISTING 525mmØ
CONCRETE STORM SEWER**

LEGEND

- PROPOSED STORM SEWER
- EXISTING STORM SEWER
- SITE BOUNDARY

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STORM SEWER NETWORK

SCALE 1 : 500

DATE DEC 2022 JOB 122098 FIGURE FIG-4

5.3.3 Water Quality Controls

Water quality control for the site will be provided by the existing Nepean Creek Stormwater Facility, located approximately 1.8km downstream from the site outlet to the Merivale Road storm sewer. Through discussions with City and RVCA staff, it has been determined that this facility will provide the requisite water quality control for the site. The facility was designed to provide quality control at a minimum design of 70% long-term TSS removal, however pond performance has been shown to range between 70-90% long-term TSS removal. Refer to **Appendix A** for details and correspondence with City and RVCA staff.

5.3.4 Upstream Drainage Areas

There are two small areas from the property to the northeast of the site which are tributary to catchbasins on the subject site. These areas have been included in the calculation of the allowable release rate from the site, and runoff from them will be controlled accordingly.

5.4 On-Site Storage

A total storage volume of approximately 250m³ can be provided across the entire 0.77ha site by a combination of surface storage, rooftop storage, and underground storage (superpipe).

5.4.1 Overland Flow and Surface Storage (Major System)

The parking areas have been designed to store runoff from storms that exceed the capacity of the storm sewers and superpipe at each catchbasin/ catchbasin maintenance hole inlet. The site has been graded to ensure that ponding is confined within the parking areas to a maximum depth of 0.35m (static ponding + dynamic flow). An overland flow path has been provided to ensure that runoff from extreme storm events that exceed the available storage can be safely directed onto the adjacent roadway (Merivale Road).

The available storage available at each low-point above the catchbasins was calculated using the Autodesk Civil 3D software, based on the proposed surface shown on the Grading Plan (**122098-GR**). Based on these calculations, approximately 152m³ of storage (static) is available within the low points of the parking area.

5.4.2 Rooftop Storage and Controls

Runoff from the building roof will be attenuated by the use of seven (7) Watts flow control roof drains, and storage for runoff which exceeds the flow rate of these drains will be provided on the roof surface. Manufacturer specifications for the proposed roof drains have been provided in **Appendix D**. The Mechanical and Structural Engineer's will be required to ensure that the proposed roof ponding volumes and total release rates from the roof are incorporated and accounted for during the detailed design of the building.

5.4.3 Underground Storage (Superpipe)

A single run of storm sewers, between CBMH06 and CBMH07 has been over-sized to provide additional storage upstream of the flow-control orifice located in CBMH07. Based on Rational method calculations the required sewer size to convey flows was 250mm, but the pipe has been upsized to 450mm to provide additional storage volume to prevent surface ponding during the 2-year storm event.

5.5 Hydrologic & Hydraulic Modeling

The City of Ottawa Sewer Design Guidelines [2] require hydrologic modeling for all dual drainage systems. The performance of the proposed storm drainage system for the site was evaluated using the *PCWMM* hydrologic/hydraulic modeling software.

Design Storms

The hydrologic analysis was completed using the following synthetic design storms and historical storms. The IDF parameters used to generate the design storms were taken from the Sewer Design Guidelines [2].

Chicago Storms:

25mm 4hr Chicago storm
2-year 3hr Chicago storm
5-year 3hr Chicago storm
100-year 3hr Chicago storm

12 Hour SCS Storms:

2-year 12-hr SCS storm
5-year 24hr Chicago storm
100-year 24hr Chicago storm

The 3-hour Chicago distribution generates the highest peak flows for both the minor and major systems and was determined to be the critical storm distribution for the design of the storm drainage system.

The proposed drainage system has also been stress tested using a 3-hour Chicago design storm that has a 20% higher intensity and total volume compared to the 100-year event.

Model Development

The PCSWMM model accounts for both minor and major system flows (*dual drainage*), including the routing of flows through the storm sewer network (*minor system*), and overland along the road network (*major system*). The results of the analysis were used to;

- Determine the total major and minor system runoff from the site;
- Size the ICDs to ensure the allowable release rate from the site is not exceeded;
- Calculate the storm sewer hydraulic gradeline for the 100-year storm event; and
- Evaluate the overland flow depths and ponding volumes during the 100-year event.

The model is capable of accounting for both static and dynamic storage within the private roadways and parking areas, including the overland flow across all high points. The 100-year flow depths computed by the model represent the total (static + dynamic) ponding depths at low points for areas in road and parking sags.

Storm Drainage Area Plan & Subcatchment Parameters

The development has been divided into subcatchments based on the drainage areas tributary to each inlet of the proposed storm sewer system. The catchment areas are shown on the Storm Drainage Area Plan (**122098-STM**) in **Appendix F**.

The hydrologic parameters for each subcatchment were developed based on the Site Plan (**Figure 2**) and the Storm Drainage Area Plan specified above. Subcatchment parameters are outlined in **Table 5-3**.

Table 5-3: Subcatchment Model Parameters

| Area ID | Catchment Area (ha) | Runoff Coefficient (C) | Percent Impervious (%) | No Depression (%) | Flow Path Length (m) | Equivalent Width (m) | Average Slope (%) |
|---------|------------------------|---------------------------|---------------------------|----------------------|-------------------------|-------------------------|----------------------|
| A-01 | 0.009 | 0.28 | 11% | 0% | 12.49 | 7.21 | 0.5% |
| A-02 | 0.046 | 0.89 | 99% | 0% | 32.56 | 14.13 | 1.3% |
| A-03 | 0.032 | 0.90 | 100% | 0% | 19.03 | 16.82 | 1.0% |
| A-04 | 0.039 | 0.90 | 100% | 0% | 33.09 | 11.79 | 1.0% |
| A-05 | 0.057 | 0.77 | 81% | 0% | 30.59 | 18.63 | 0.5% |
| A-06 | 0.056 | 0.82 | 89% | 0% | 38.00 | 14.74 | 1.5% |
| A-07 | 0.072 | 0.90 | 100% | 0% | 38.43 | 18.74 | 1.5% |
| A-08 | 0.046 | 0.83 | 90% | 0% | 16.28 | 28.25 | 1.5% |
| A-09 | 0.023 | 0.90 | 100% | 0% | 7.15 | 32.18 | 1.5% |
| B-01 | 0.257 | 0.90 | 100% | 100% | 62.27 | 41.27 | 1.5% |
| B-02 | 0.015 | 0.90 | 100% | 0% | 23.70 | 6.33 | 3.0% |
| DR-01 | 0.018 | 0.20 | 0% | 0% | 69.44 | 2.59 | 0.5% |
| DR-02 | 0.010 | 0.20 | 0% | 0% | 67.30 | 1.49 | 0.5% |
| DR-03 | 0.018 | 0.26 | 9% | 0% | 74.72 | 2.41 | 0.5% |
| DR-04 | 0.004 | 0.27 | 10% | 0% | 25.97 | 1.54 | 0.5% |
| OS-01 | 0.049 | 0.48 | 40% | 0% | 32.86 | 14.91 | 0.5% |
| OS-02 | 0.014 | 0.90 | 100% | 0% | 7.59 | 18.45 | 0.5% |

Infiltration

Infiltration losses for all catchment areas were modeled using Horton's infiltration equation, which defines the infiltration capacity of the soil over the duration of a precipitation event using a decay function that ranges from an initial maximum infiltration rate to a minimum rate as the storm progresses. The default values for the City of Ottawa were used for all catchments.

$$\text{Horton's Equation: } f(t) = f_c + (f_o - f_c)e^{-k(t)}$$

$$\begin{aligned} \text{Initial infiltration rate: } & f_o = 76.2 \text{ mm/hr} \\ \text{Final infiltration rate: } & f_c = 13.2 \text{ mm/hr} \\ \text{Decay Coefficient: } & k = 4.14/\text{hr} \end{aligned}$$

Depression Storage

The default values for depression storage in the City of Ottawa were used for all catchments. Residential rooftops were assumed to provide no depression storage.

- Depression Storage (pervious areas): 4.67 mm
- Depression Storage (impervious areas): 1.57 mm

Equivalent Width

'Equivalent Width' refers to the width of the sub-catchment flow path. This parameter is calculated as described in the Sewer Design Guidelines [2], Section 5.4.5.6. The flow paths used to calculate the equivalent widths are shown on the PCSWMM schematics provided in **Appendix D**.

Impervious Values

Runoff coefficients for each subcatchment area were calculated based on the proposed Site Plan (**Figure 2**). Refer to the Storm Drainage Area Plan (**122098-STM**) for details. Percent impervious values were calculated using the following equation:

$$\%imp = \frac{C - 0.2}{0.7}$$

5.6 Minor System Design and Analysis

The following sections outline the model parameters and results of the PCSWMM model pertaining to the minor system (storm sewers).

5.6.1 Orifice Controls

Inflows to the storm sewer were modeled based on the characteristics of each inlet. All the catchbasins in the parking areas and roadway are located at low points. Inflows to the storm sewer are based on the ICD specified for the inlet and the maximum depth of ponding. ICDs have been sized to limit the ultimate outlet peak flows to the allowable release rate of 82 L/s.

Per the Storm Sewer Design Guidelines [2], “ICDs shall not be used in series (i.e. where the backwater from one device affect the next upstream device) unless a dynamic model is used to assess their performance and to compute the corresponding upstream water elevation and storage requirements”. As such, ICDs have been installed in the downstream catchbasin maintenance hole to limit peak flows from the upstream series of inlets, as well as take advantage of the storage provided by the upstream storm sewers. Details are outlined as follows in **Table 5-4**. ICD information is indicated on the General Plan of Services (**122098-GP, Appendix F**). Roof drain calculations have been included in **Appendix D**.

Table 5-4: Inlet Control Devices & Design Flows

| Structure | ICD Size | Max Head* (m) | Calculated Max. Capture Rate** (L/s) | 100-yr Head*** (m) | 100-year Approach Flow*** (L/s) | 100-year Capture Rate*** (L/s) |
|-----------|----------|------------------|---|-----------------------|------------------------------------|-----------------------------------|
| CBMH05 | LMF75 | 2.12 | 7.84 | 1.99 | 39.80 | 7.45 |
| CBMH07 | 83mm | 2.13 | 21.68 | 1.92 | 53.50 | 19.80 |
| CB08 | LMF95 | 1.37 | 9.09 | 1.11 | 22.57 | 8.09 |
| CB09 | LMF85 | 1.37 | 7.64 | 1.12 | 18.35 | 6.83 |

*Max head is calculated as the T/G elevation plus 0.35m

** Max capture rate is calculated using the orifice equation, based on the max head

*** From PCSWMM 100-yr 3-hour Chicago Storm event

5.6.2 Roof Drains

The building rooftop was simulated in PCSWMM based on an outlet rating curve for the proposed roof drains and using a storage node to represent the available storage provided by the roof surface. It has been assumed that the building roof will have seven (7) drains each set to fully open, giving flow rates outlined in **Table 5-5** for a single drain (converted from inches and gallons per minute). For modelling purposes, a single outlet link for the roof has been used, with the flow rates below multiplied by seven (7) to give the total flow from all drains.

Table 5-5: Roof Drain Rating Curve

| Head (m) | Single Drain - Controlled Flow Rate* (L/s) | | | | |
|----------|--|----------|----------|----------|--------------|
| | Fully Open | 3/4 Open | 1/2 Open | 1/4 Open | Fully Closed |
| 0.000 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 0.025 | 0.32 | 0.32 | 0.32 | 0.32 | 0.32 |
| 0.051 | 0.63 | 0.63 | 0.63 | 0.63 | 0.63 |
| 0.076 | 0.95 | 0.87 | 0.79 | 0.71 | 0.63 |
| 0.102 | 1.26 | 1.10 | 0.95 | 0.79 | 0.63 |
| 0.127 | 1.58 | 1.34 | 1.10 | 0.87 | 0.63 |
| 0.150 | 1.89 | 1.58 | 1.26 | 0.95 | 0.63 |
| 1.000 | 1.89 | 1.58 | 1.26 | 0.95 | 0.63 |

*Watts Flow Control Roof Drains Rating Curve (single drain)

The available storage and flow rating curve for the roof drains has been multiplied by the number of drains and the storage lumped into a single storage node. Approximately 96m³ of storage can be provided by the building rooftop. **Table 5-6** and **Table 5-7** summarize the controlled post-development design flows from the building rooftop, the maximum anticipated ponding depths, storage volumes required, and the storage volumes provided for the 5-year and 100-year storm events.

Table 5-6: 5-year Roof Storage & Peak Flows

| Area ID | Static Ponding Area (m ²) | Drainage Area (ha) | Uncontrolled Peak Flow (L/s) | Controlled Peak Flow (L/s) | Flow Depth (m) | Storage Required (m ³) | Storage Available (m ³) |
|--------------|---------------------------------------|--------------------|------------------------------|----------------------------|----------------|------------------------------------|-------------------------------------|
| R-01 | 299 | 0.038 | 9.8 | 1.26 | 0.12 | 6.99 | 14.97 |
| R-02 | 300 | 0.043 | 11.1 | 1.26 | 0.12 | 8.32 | 14.98 |
| R-03 | 339 | 0.045 | 11.7 | 1.26 | 0.12 | 8.94 | 16.95 |
| R-04 | 305 | 0.045 | 11.8 | 1.26 | 0.13 | 9.03 | 15.23 |
| R-05 | 271 | 0.037 | 9.7 | 1.26 | 0.12 | 6.82 | 13.54 |
| R-06 | 205 | 0.023 | 5.9 | 1.26 | 0.10 | 3.31 | 10.25 |
| R-07 | 205 | 0.023 | 5.9 | 1.26 | 0.10 | 3.31 | 10.24 |
| TOTAL | | 0.253 | 66.0 | 8.82 | | 46.73 | 96.16 |

Table 5-7: 100-year Roof Storage & Peak Flows

| Area ID | Static Ponding Area (m ²) | Drainage Area (ha) | Uncontrolled Peak Flow (L/s) | Controlled Peak Flow (L/s) | Flow Depth (m) | Storage Required (m ³) | Storage Available (m ³) |
|--------------|---------------------------------------|--------------------|------------------------------|----------------------------|----------------|------------------------------------|-------------------------------------|
| R-01 | 299 | 0.038 | 18.7 | 1.89 | 0.15 | 14.97 | 14.97 |
| R-02 | 300 | 0.043 | 21.2 | 1.89 | 0.15 | 14.98 | 14.98 |
| R-03 | 339 | 0.045 | 22.3 | 1.89 | 0.15 | 16.95 | 16.95 |
| R-04 | 305 | 0.045 | 22.5 | 1.89 | 0.15 | 15.23 | 15.23 |
| R-05 | 271 | 0.037 | 18.4 | 1.89 | 0.15 | 13.54 | 13.54 |
| R-06 | 205 | 0.023 | 11.3 | 1.89 | 0.15 | 10.25 | 10.25 |
| R-07 | 205 | 0.023 | 11.3 | 1.89 | 0.15 | 10.24 | 10.24 |
| TOTAL | | 0.253 | 125.8 | 13.23 | | 96.16 | 96.16 |

As shown in the above tables, the building roof will provide sufficient storage for all storm events up to the 100-year event, with some overflow through the scuppers during the 100-year event and larger. Flows exceeding the available storage will overflow through the scuppers and onto the ground surface below and will be conveyed to storm sewer inlets via the major system flow routes. Detailed calculations for each of the roof drains have been provided in **Appendix D**.

5.6.3 Hydraulic Grade Line

The results of the analysis were used to determine if there would be any surcharging from the storm sewer system during the 100-year storm event. **Table 5-8** provides a summary of the 100-year HGL elevation at each storm maintenance hole within the proposed development, as well as a summary of the HGL elevations for a 20% increase (rainfall intensity and total precipitation) in the 100-year design event.

Table 5-8: 100-year HGL Elevations (m)

| Maintenance Hole ID | MH Invert Elevation (m) | T/G Elevation (m) | HGL Elevation: 100yr-3hr (m) | HGL Elevation: 100yr-3hr +20% (m) | Clearance from T/G (100yr) (m) | Clearance from T/G (100yr+20%) (m) |
|---------------------|-------------------------|-------------------|------------------------------|-----------------------------------|--------------------------------|------------------------------------|
| MH102 | 94.14 | 96.64 | 94.50 | 94.51 | 2.14 | 2.13 |
| MH104 | 94.42 | 96.68 | 94.61 | 94.63 | 2.07 | 2.05 |

5.6.4 Upstream Drainage Areas

Two areas from the property to the northwest of the site are tributary to the proposed storm infrastructure. Area OS-1 (0.049ha) is tributary to a curb-inlet catchbasin (CICB-OS1) to be installed in the grassed area between the site and the adjacent property. Runoff from this area will flow uncontrolled into the storm sewer. Area OS-2 (0.014ha) is tributary to CB09 and runoff from this area will pond above this CB with inflows controlled by the ICD located within CB09.

5.7 Major System Design and Analysis

Catchbasin maintenance holes were modeled as storage nodes to account for the surface storage provided by the parking areas of the proposed development, and the storage provided by the structure itself. The storage nodes are interconnected using short rectangular open channels to simulate the flows cascading over high points when the available static storage is exceeded. A total volume of approximately 152 m³ is provided by the low points in the parking areas and roadway, as shown in **Table 5-9**.

Table 5-9: Ponding Volumes (m³)

| STM Area ID | CB ID | Ponding Area (m ²) | Available Static Ponding Volume (m ³) |
|-------------|--------|--------------------------------|---|
| A-02 | CBMH02 | 170.25 | 11.39 |
| A-03 | CBMH03 | 223.38 | 14.70 |
| A-04 | CBMH04 | 235.95 | 16.19 |
| A-05 | CBMH05 | 289.22 | 21.42 |
| A-06 | CBMH06 | 369.86 | 38.15 |
| A-07 | CBMH07 | 460.55 | 36.72 |
| A-08 | CB08 | 200.44 | 9.90 |
| A-09 | CB09 | 69.50 | 3.21 |

The major system network was evaluated using the PCSWMM model to ensure that the ponding depths conform to City standards. A summary of ponding depths at each inlet for the 100-year event is provided in **Table 5-10**. There will be no ponding during the 2-year event, and ponding which occurs for larger storm events will be less than 0.35m.

Table 5-10: 100-year Event Ponding Depths

| Structure | T/G (m) | Max. Static Ponding (Spill Depth) | | 100-yr Event (3hr) | | | |
|-----------|---------|-----------------------------------|-----------|--------------------|-----------|-----------------|-------------------|
| | | Elev. (m) | Depth (m) | Elev. (m) | Depth (m) | Cascading Flow? | Cascade Depth (m) |
| CBMH02 | 96.50 | 96.70 | 0.20 | 96.66 | 0.16 | N | 0.00 |
| CBMH03 | 96.50 | 96.70 | 0.20 | 96.65 | 0.15 | N | 0.00 |
| CBMH04 | 96.50 | 96.70 | 0.20 | 96.65 | 0.15 | N | 0.00 |
| CBMH05 | 96.45 | 96.70 | 0.25 | 96.65 | 0.20 | N | 0.00 |
| CBMH06 | 96.45 | 96.70 | 0.25 | 96.55 | 0.10 | N | 0.00 |
| CBMH07 | 96.45 | 96.70 | 0.25 | 96.55 | 0.10 | N | 0.00 |
| CB08 | 96.55 | 96.70 | 0.15 | 96.61 | 0.06 | N | 0.00 |
| CB09 | 96.55 | 96.69 | 0.14 | 96.62 | 0.07 | N | 0.00 |

An expanded table of the ponding depths at low points in the parking lots (including the stress-test event) is provided in **Appendix D**. Based on these results, the proposed storm drainage system will not experience any adverse flooding even with a 20% increase to the 100-year event.

5.8 Peak Flows

For all storm events, the allowable release rate is 82 L/s. Peak flows for each storm event are outlined in the following table:

Table 5-11: Peak Flows (L/s)

| Storm Distribution-> | 3hr Chicago | | | | | 12hr SCS | | |
|-------------------------|-------------|-----|-----|-------|------------|----------|-----|-------|
| | 25mm | 2yr | 5yr | 100yr | 100yr +20% | 2yr | 5yr | 100yr |
| Offsite - North | 0 | 1 | 1 | 3 | 4 | 0 | 1 | 2 |
| Offsite - East | 0 | 0 | 0 | 2 | 3 | 0 | 0 | 2 |
| STM to Merivale | 42 | 50 | 59 | 76 | 83 | 37 | 49 | 66 |
| Maj. System to Merivale | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 42 | 51 | 60 | 81 | 90 | 37 | 50 | 71 |

As outlined in the above table, peak flows for all storm events up to and including the 100-year event will be controlled to the allowable release rate of 82 L/s. There will be no major overland flow directed to Merivale Road for all storm events up to and including the stress test event (100-year + 20%).

6.0 WATER

6.1 Existing Conditions

The subject site is located within the City of Ottawa 2W2C watermain pressure zone. There is an existing 300mm watermain in Merivale Road to the west of the site.

The existing watermain boundary conditions that were provided for the site by the City of Ottawa (October 2022) are listed in Table 6-1, and are included in **Appendix E**.

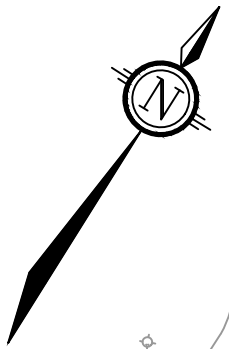
Table 6-1: Watermain Boundary Conditions

| Location | Pressure (m) | | |
|---------------|--------------|----------------------------------|-----------|
| | Maximum HGL | Max Daily and Fire Flow (183L/s) | Peak Hour |
| Merivale Road | 134.1 | 123.0 | 125.4 |

6.2 Proposed Conditions

The site will be serviced with approximately 13m of 150mm diameter watermain and 85m of 250mm diameter watermain connecting to the existing 300mm watermain on Merivale Road. The building will be fully sprinklered. Shut-off valves will be provided on the proposed watermain at the property line and on the building service.

The proposed watermain will service the commercial building and two hydrants. It will provide sufficient capacity to maintain appropriate pressures for the high pressure and peak hour demand conditions. **Figure 5** provides a high-level schematic of the proposed water distribution system. Due to the distance from the proposed building to the existing fire hydrants located on Merivale Road, two on-site fire hydrants will be required to provide the required fire flow rate.



MERIVALE ROAD

1543 MERIVALE

1545 MERIVALE

50
CAPILANO

BUILDING

R1

N2

N1

P2

P1

HYD
T/F=96.78

HYD
T/F=96.89

CONNECT TO EXISTING
300mmØ WATERMAIN

SUBJECT SITE

LEGEND

- SITE BOUNDARY
- PROPOSED 250mmØ WATERMAIN
- PROPOSED 150mmØ WATERMAIN
- PROPOSED WM PIPE NUMBER
- PROPOSED WATERMAIN NODE
- EXISTING 300mmØ WATERMAIN

NOVATECH

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CITY of OTTAWA
MERIVALE MEDICAL IMAGING CLINIC
(1545A MERIVALE ROAD)

WATERMAIN LAYOUT

SCALE 1 : 500

DATE DEC 2022 JOB 122098 FIGURE FIG-5

M:\2022\122098\CAD\Civil\Figures\Servicing\122098-WM.dwg, FIG5-WM, Dec 13, 2022, 4:12pm, jroy

The water demand was calculated using City of Ottawa Design Guidelines – Water Distribution (2010) [3] and the fire flow demands were calculated as per the Fire Underwriter’s Survey (2020) guidelines. The design criteria used are outlined in **Table 6-2**. Refer to **Appendix E** for fire flow calculations.

Table 6-2: Watermain Design Criteria

| Design Parameter | Design Criteria |
|-------------------------------|---|
| Commercial Site Area (gross) | 0.69 ha |
| Average Day Commercial Demand | 28,000 L/gross ha/d |
| Maximum Day Demand | 1.5 x Average Day |
| Peak Hour Demand | 1.8 x Maximum Day |
| Fire Demand | Fire Underwriters Survey (2020) |
| Maximum Pressure | 690 kPa (100psi) unoccupied areas |
| Maximum Pressure | 552 kPa (80psi) occupied areas outside of ROW |
| Minimum Pressure | 275 kPa (40 psi) except during fire flow |
| Minimum Pressure | 140 kPa (20 psi) fire flow conditions |

The water demand for the site was calculated and is summarized in **Table 6-3** for high pressure, maximum daily and fire flow, and peak hour demand conditions for the site.

Table 6-3: Water Demand Summary

| Commercial Space (ha) | Average Day Commercial Demand (L/gross ha/d) | Demand (L/s) | | | |
|-----------------------|--|--------------|-------------|-----------|-----------|
| | | Average Day | Maximum Day | Peak Hour | Fire Flow |
| 0.69 | 28,000 | 0.22 | 0.34 | 0.60 | 183 |

The proposed watermain was modeled using EPANET 2 for the watermain layout per **122098-GP (Appendix F)**. The results of the modelling are presented in **Table 6-4**.

Table 6-4: Water Operating Conditions

| Condition | Demand (L/s) | Minimum/Maximum Allowable Pressure (kPa/psi) | Operating Pressure (kPa/psi) |
|--|---------------|--|------------------------------|
| High Pressure | 0.22 | 552/80 (max) | 365.4/53.0 |
| Maximum Day Demand (c/w Max Fire Flow) | 0.34 (183.34) | 140/20 (min) | 200.52/29.08 |
| Peak Hour Demand | 0.60 | 275/40 (min) | 278.6/40.4 |

These results confirm that proposed watermain layout is adequate to maintain hydraulic pressures within the normal operating range meeting the City of Ottawa Guidelines [3]. Full model results are included in **Appendix E**. This report concludes the proposed water design will adequately service the site.

7.0 GEOTECHNICAL INVESTIGATION

A geotechnical investigation has been completed by Paterson Group Inc. for the proposed project. Refer to the corresponding Geotechnical Report [1] for subsurface conditions, construction recommendations and geotechnical inspection requirements.

8.0 CONCLUSIONS AND RECOMMENDATIONS

This report has been prepared in support of Site Plan Control application for the proposed Merivale Medical Imaging Clinic. The conclusions are as follows:

- The proposed building will be serviced by the municipal watermain, sanitary and storm sewers within the right-of-way for Merivale Road.
- The building will be sprinklered and supplied with a fire department siamese connection. The siamese connection will be located within 45m of the proposed on-site fire hydrants.
- The post-development site stormwater flow will be approximately 51 L/s during the 2-year design, 60 L/s during the 5-year design event and 81 L/s during the 100-year event, all less than the target control rate of 82 L/s. Post-development flows are being controlled by inlet control devices, utilizing a combination of underground and surface storage (roof and parking lots).
- Water quality control for the site will be provided by the existing end of pipe stormwater management facility (Nepean Creek Stormwater Facility).

It is recommended that the proposed site servicing and stormwater management design be approved for implementation. Please contact the undersigned should you have questions or require additional information.

Sincerely,

NOVATECH

Prepared By:



Trevor McKay, E.I.T.
Project Manager, Land Development

Prepared By:



Kallie Auld, P.Eng.
Project Manager, Water Resources

Reviewed/Approved by:



Melanie Riddell, P.Eng.
Director, Land Development



References

1. "Geotechnical Investigation, Proposed Commercial Building, 1545 Merivale Road, Ottawa, Ontario prepared by Paterson Group (Report: PG6288-1). [August 3, 2022]
2. "Sewer Design Guidelines", Department of Public Works and Services, City of Ottawa [October 2012] as ammended by Technical Bulletins PIEDTB-2016-01, ISTB-2018-01, ISTB-2018-03, ISTB-2018-04, and ISTB-2019-02
3. "Ottawa Design Guidelines – Water Distribution", Infrastructure Services Department, City of Ottawa [July 2010] as ammended by Technical Bulletins ISTB-2014-02, ISTB-2018-02, ISTB-2018-03, and ISTB-2021-03.

APPENDIX A

Correspondence

Pre-Application Consultation Meeting Minutes

Property Address: 1545(A) Merivale Road

Location: Virtual – Microsoft Teams

Meeting Date: June 16, 2022

Attendees: Colette Groni – Planner (File Lead), City of Ottawa
Patrick McMahon – Transportation Project Manager, City of Ottawa
Ann O'Connor – Urban Design Planner, City of Ottawa
Louise Cerveny – Parks Planner, City of Ottawa
Alexa Giannini – Transportation Co-op Student, City of Ottawa
Steven Payne – Planning Co-op Student, City of Ottawa
Philippe Doyle – Lalande + Doyle Architects Inc
Louise Lalande – Lalande + Doyle Architects Inc
Melanie Riddell – Novatech

Regrets: Sami Rehman – Planner (Environment), City of Ottawa
Julie Candow – Infrastructure Project Manager, City of Ottawa
Mark Richardson – Planning Forester, City of Ottawa
Eric Lalande – Planner, RVCA

Applicant Comments

1. The applicant is proposing a new one-storey medical facility with a gross floor area of 2,300 m². The proposed building is located at the rear of the site with surface parking in front; 103 vehicle parking spaces are currently proposed.
2. The owners are seeking to relocate their medical imaging business to the new facility and have expressed that a one-storey building is preferable due to operational considerations for the business.
3. The neighbouring site, 1545 Merivale Road, is under the same ownership. It has approximately 36m of frontage along Merivale Road and is currently occupied by a vacant restaurant building. The owner's current intention is to develop/sell this parcel in the future as a separate development.
4. The neighbouring site, 1543 Merivale Road, is under separate ownership. However, there is an existing agreement in place between the owner of 1543 Merivale Road and the subject site allowing for a portion of the site to be used as a part of the access to subject site. It is currently occupied by a gas station.

Planning

1. Confirm ownership situation for 1545 (A) Merivale Road and 1545 Merivale Road (currently occupied by vacant restaurant). Please note that if both properties are

under the same ownership than they will have merged on title, which may have implications for how zoning is applied.

2. Please note that Merivale Road is considered the front lot line for the purposes of zoning, as per Section 186(10)(a) of the Zoning By-law.
3. Please note that a rear yard setback of 7.5m is required as per Section 186(10)(d)(ii). The provided plan currently shows a rear yard setback of approximately 1.5m.
4. Vehicle parking is to be provided at the rates specified for Area C on Schedule 1A in Table 101 of the Zoning By-law:
 - a. Medical facility – 4 per 100 m² of gross floor area (~103 spaces required)
5. Bicycle parking is to be provided at the rates specified in Table 111A of the Zoning By-law:
 - a. Medical facility – 1 per 1000 m² of gross floor area (~2-3 spaces required)
6. Please note that the minimum required width of a landscaped buffer around a parking lot containing 100 or more spaces is 3m. Based on the provided plan, a total of 104 parking spaces are proposed, therefore 3m landscaped buffers are required.
7. Consider including additional landscaped islands throughout the parking lot to address the urban heat island effect.
8. Staff have concerns with the current location of the proposed building. Consider shifting the building towards Merivale Road and having parking in behind. Please note that if the building is brought forward to Merivale Road, that it will need to comply with the requirements set out in Section 186(10)(e)-(h)
9. Based on the current proposal, relief is required from Section 186(10)(b)(i) of the Zoning By-law, which requires at least 50% of the frontage along the front lot line (Merivale) must be occupied by building walls located within 3m of the frontage for non-residential buildings. This can be achieved by way of a Minor Zoning By-law Amendment or through a minor variance application to the Committee of Adjustment. Please note that staff have concerns with providing relief from this provision as the proposed development does not align with applicable policies, and it appears that the provision could be met by bring the building forward to Merivale Road.
10. Confirm whether paid parking is proposed on site. Please note that paid parking is considered to be a separate use (i.e., “parking lot”), not accessory to the medical facility use. It would also not contribute to the minimum required parking spaces. Further, as “parking lot” is not identified as a permitted use in the AM10 zone, a major zoning by-law amendment would be required to permit it.

11. Required applications:

- a. Minor Zoning By-law Amendment or Minor Variances. Please note that staff believe that a minor rezoning would be more appropriate.
 - i. More information on Zoning By-law Amendment applications can be found [here](#).
 - ii. Please note that Minor Variances are handled by the Committee of Adjustment. The Planning Department provides comments on Committee of Adjustment applications; however, the Committee of Adjustment makes the decision. For more information on the Committee of Adjustment, including application forms and fees, please visit: <https://ottawa.ca/en/city-hall/planning-and-development/committee-adjustment>. For questions pertaining to forms and fees, please contact the Committee of Adjustment directly at cofa@ottawa.ca or at (613)-580-2436.
- b. The proposed development is subject to Site Plan Control, and will be a “New – Site Plan Control – Complex” application. Application, timeline and fees can be found [here](#).

Feel free to contact Colette Gorni, Planner (File Lead), at Colette.Gorni@ottawa.ca for follow-up questions.

Transportation

1. Follow Traffic Impact Assessment Guidelines
 - a. Submit a screening form alongside the Scoping report. Start this process as soon as possible.
 - b. The application will not be deemed complete until the submission of the draft step 1-4, including the functional draft RMA package. An RMA may be required to restrict access through the median break. Use of the existing median depression for this site should be evaluated within the TIA, this movement is not guaranteed.
 - c. Collaboration and communication between development proponents and City staff are required at the end of every step of the TIA process.
2. Noise Impact Study is recommended for the following:
 - a. Road (proximity to Merivale Road); and,
 - b. Aircraft (within the 25 NEF/NEP line).
3. The clear throat requirement for general office of less than 5,000m² accessing an arterial is 15m.
4. On site plan:

- a. Show all details of the roads abutting the site up to and including the opposite curb; include such items as pavement markings, accesses and/or sidewalks.
 - b. Turning templates will be required for all accesses showing the largest vehicle to access the site; required for internal movements and at all access (entering and exiting and going in both directions).
 - c. Show all curb radii measurements; ensure that all curb radii are reduced as much as possible
 - d. Sidewalk is to be continuous across access as per City Specification 7.1.
5. Merivale Road has a protected right of way of 44.5m, it appears that a widening is required. Show this line on the site plan.
 6. Consider how pedestrians are going to reach the office from Merivale Road and provide a connection, preferably a separated sidewalk.
 7. Ensure that appropriate access aisles are provided for the accessible parking spaces.

Feel free to contact Patrick McMahon, Transportation Project Manager, at Patrick.McMahon@ottawa.ca.

Urban Design

1. A Design Brief is required. A terms of reference is attached.
2. The property is within a Design Priority Area. Please ensure the design brief addresses how the development meets Design Priority Area policies in Section 2.5.1 of the Official Plan (2008, as amended) and in the New OP.
3. Urban Design staff have some concerns with the current proposal of a large 1-storey building located at the rear of the property located on an Arterial Mainstreet. Explore the option of consolidating the subject site with the existing restaurant site and bringing the building up to Merivale Road to establish a strong street edge along the Arterial Mainstreet and provide active street frontage. Explore a multi-storey building. This re-design would align with the direction the City has for development along Arterial Mainstreets / Main Corridors. Below are examples of such policies:
 - a) Urban Design Guidelines for Development along Arterial Mainstreets:
 - Guideline 1: Locate new buildings along the public street edge
 - Guideline 4: Use buildings, landscaping and other streetscape elements to create continuous streetscapes
 - Guideline 6: Set new buildings 0 to 3.0 metres back from the front property line...

- Guideline 12: Design the built form in relation to the adjacent properties to create coherent streetscapes.

b) Merivale Road Secondary Plan

- 2.1 Goals – (1)(a)(i) strengthen the visual character of the area; (ii) Improve the quality and consistency of pedestrian amenities in the public and private realm; (vi) Provide, to the maximum extent possible, for a safe and efficient environment for all users – transit riders, pedestrians, cyclists and rivers travelling within and through the study area.
- 3.1.1.2 (1)(a)(vi) Parking on sites with multiple retail and commercial uses should be directed to the interior of the block with access between frontage buildings or at the rear
- 3.1.1.3 The streetscape treatment along Merivale Road should be consistent with an environment that supports and encourages pedestrian activity
- 3.1.2.1 (1)(a)(i) All public sidewalks should be connected to sidewalks on adjacent private retail and commercial properties

c) Official Plan (2008, as amended)

- Section 3.6.3 – Mainstreets, Policy 9: On Arterial Mainstreets, the location of surface parking will be evaluated in the context of Section 2.5.1 and Section 4.11
- Section 2.5.1 Design Objectives (2) To define quality public and private spaces through development / (3) To create places that are safe, accessible and are easy to get to, and move through
- Section 4.11 Policy 6: The City will require that all applications for new development: (a) Orient the principal façade and entrance(s) of main building(s) to the street.

4. If the building will not have frontage on Merivale Road, provide a landscaped promenade connecting the building to Merivale Road. This promenade should include:

- a) A continuous 2.0m wide concrete continuous sidewalk connecting the public sidewalk on Merivale Road to the proposed new entrance to the building. Where there are breaks required in the sidewalk for drive aisles, clear pavement markings and signage should be included to indicate pedestrian priority.
- b) Landscaping alongside the connecting sidewalk, providing visual cues to lead people from Merivale Road into the site.

- c) Align the front door of the building to be the viewpoint terminus at the end of the promenade. The front door should be visible from Merivale Road.
5. Consider the width of the access needed off of Merivale Road. It appears the subject site has only 4.5 metres of lot frontage on Merivale Road, which is insufficient to provide vehicular access as well as a pedestrian sidewalk and landscaping. Provide clarity if a lot line adjustment will be pursued and/or provide clarity on the existing or proposed easements for access.
6. Consider increasing the width of the setback from the eastern rear property line.
7. Consider accessibility needs when designing access, the surface parking lot and the building itself.
8. Include a sidewalk around the building.
9. Provide more information on the gate/controlled access to the parking lot.
10. Google streetview shows a large sign with directional arrow close to the entrance off of Merivale Road; however, this is not illustrated on the concept plan. Clarify if updated wayfinding measures will be put in place.
11. Please adhere to and address the policies in the Merivale Road Secondary Plan and the Urban Design Guidelines for Development along Arterial Mainstreets.

Feel free to contact Ann O'Connor, Urban Design Planner, for follow-up questions at Ann.OConnor@ottawa.ca.

Engineering

1. The Servicing Study Guidelines for Development Applications are available at the following address: <https://ottawa.ca/en/planning-development-and-construction/developing-property/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#servicing-study-guidelines-development-applications>
2. Servicing and site works shall be in accordance with the following documents:
 - Ottawa Sewer Design Guidelines (October 2012)
 - Ottawa Design Guidelines – Water Distribution (2010)
 - Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
 - City of Ottawa Slope Stability Guidelines for Development Applications (revised 2012)
 - City of Ottawa Environmental Noise Control Guidelines (January, 2016)
 - City of Ottawa Park and Pathway Development Manual (2012)
 - City of Ottawa Accessibility Design Standards (2012)
 - Ottawa Standard Tender Documents (latest version)
 - Ontario Provincial Standards for Roads & Public Works (2013)

3. Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at geoinformation@ottawa.ca or by phone at (613) 580-2424 x.44455).
4. The Stormwater Management Criteria, for the subject site, is to be based on the following:
 - i. The 2-yr storm event using the IDF information derived from the Meteorological Services of Canada rainfall data, taken from the MacDonald Cartier Airport, collected 1966 to 1997 (separated sewer system built pre-1970 on Merivale Road).
 - ii. The pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less (§ 8.3.7.3).
 - iii. A calculated time of concentration (Cannot be less than 10 minutes).
 - iv. Flows to the storm sewer in excess of the 2-year storm release rate, up to and including the 100-year storm event, must be detained on site.
5. Deep Services (Storm, Sanitary & Water Supply)
 - i. Existing deep services on Merivale Road; 525mm dia. storm, 300mm dia. watermain, 250mm dia. sanitary sewer. Please confirm the location and size of the existing service laterals onsite. If the existing service laterals are to be re-used, please provide CCTV inspection reports (if possible) to confirm the condition of the existing services. New services must be grouped in a common trench to minimize the number of road cuts.
 - ii. A monitoring maintenance hole for storm and sanitary sewers shall be required just inside the property line for all non-residential building connections from a private sewer to a public sewer. See the sewer use by-law for details.
 - iii. Sewer connections to be made above the springline of the sewermain as per:
 - a. Std Dwg S11.1 for flexible main sewers – *connections made using approved tee or wye fittings.*
 - b. Std Dwg S11 (For rigid main sewers) – *lateral must be less than 50% the diameter of the sewermain,*
 - c. Std Dwg S11.2 (for rigid main sewers using bell end insert method) – *for larger diameter laterals where manufactured inserts are not available; lateral must be less than 50% the diameter of the sewermain,*

- d. Connections to manholes permitted when the connection is to rigid main sewers where the lateral exceeds 50% the diameter of the sewermain. – Connect obvert to obvert with the outlet pipe unless pipes are a similar size.
 - e. No submerged outlet connections.
6. Water Boundary condition requests must include the location of the service (map or plan with connection location(s) indicated) and the expected loads required by the proposed development, including calculations. Please provide the following information:
- i. Location of service
 - ii. Type of development and the amount of fire flow required (as per FUS 2020).
 - iii. Average daily demand: ___ l/s.
 - iv. Maximum daily demand: ___ l/s.
 - v. Maximum hourly daily demand: ___ l/s.

7. MECP ECA Requirements

An MECP Environmental Compliance Approval is not anticipated to be required for this application assuming the proposed development meets the following criteria:

- i. Is designed to service one lot or parcel of land;
 - ii. Discharges into a storm sewer that is not a combined sewer;
 - iii. Does not service industrial land or a structure located on industrial land; and
 - iv. Is not located on industrial land. O.Reg. 525/98, s. 3; O.Reg. 40/15, s. 4.
8. Phase 1 ESAs and Phase 2 ESAs must conform to clause 4.8.4 of the Official Plan that requires that development applications conform to Ontario Regulation 153/04.

Should you have any questions or require additional information, please contact me directly at Julie.candow@ottawa.ca.

Parks

1. Pursuant to Section 3 and Section 10 Parkland Dedication By-law 2009-05, as amended, cash-in-lieu (CIL) of parkland shall be paid at the time of Site Plan Control approval.
2. The land valuation shall be determined as of the day before Site Plan Control approval and shall be at the cost of the Owner.
3. Parks and Facilities Planning is currently undertaking a legislated review for the replacement of the Parkland Dedication By-law, with the new by-law to be considered by City Council in early July 2022. To ensure you are aware of parkland dedication requirements for your proposed development, we encourage you to familiarize yourself with the [existing Parkland Dedication By-law](#) and to sign up for project notifications on the [Engage Ottawa project page](#) or by emailing the project lead at Kersten.Nitsche@ottawa.ca

Feel free to contact Louise Cervený, Parks Planner, at

Forestry

TCR requirements:

1. A Tree Conservation Report (TCR) must be supplied for review along with the suite of other plans/reports required by the City
 - a. An approved TCR is a requirement of Site Plan approval.
 - b. The TCR may be combined with the LP provided all information is supplied
2. Any removal of privately-owned trees 10cm or larger in diameter, or city-owned trees of any diameter requires a tree permit issued under the Tree Protection Bylaw (Bylaw 2020 – 340); the permit will be based on an approved TCR and made available at or near plan approval.
3. The Planning Forester from Planning and Growth Management as well as foresters from Forestry Services will review the submitted TCR
 - a. If tree removal is required, both municipal and privately-owned trees will be addressed in a single permit issued through the Planning Forester
 - b. Compensation may be required for city owned trees – if so, it will need to be paid prior to the release of the tree permit
4. The TCR must list all trees on site, as well as off-site trees if the CRZ extends into the developed area, by species, diameter and health condition

5. Please identify trees by ownership – private onsite, private on adjoining site, city owned, co-owned (trees on a property line)
6. If trees are to be removed, the TCR must clearly show where they are, and document the reason they cannot be retained
7. All retained trees must be shown, and all retained trees within the area impacted by the development process must be protected as per City guidelines available at [Tree Protection Specification](#) or by searching Ottawa.ca
 - a. The location of tree protection fencing must be shown on the plan
 - b. Show the critical root zone of the retained trees
 - c. If excavation will occur within the critical root zone, please show the limits of excavation
8. The City encourages the retention of healthy trees; if possible, please seek opportunities for retention of trees that will contribute to the design/function of the site.

For more information on the process or help with tree retention options, contact Mark Richardson mark.richardson@ottawa.ca or on [City of Ottawa](#)

LP tree planting requirements:

For additional information on the following please contact adam.palmer@Ottawa.ca

9. Minimum Setbacks
 - a. Maintain 1.5m from sidewalk or MUP/cycle track or water service laterals.
 - b. Maintain 2.5m from curb
 - c. Coniferous species require a minimum 4.5m setback from curb, sidewalk or MUP/cycle track/pathway.
 - d. Maintain 7.5m between large growing trees, and 4m between small growing trees. Park or open space planting should consider 10m spacing, except where otherwise approved in naturalization / afforestation areas. Adhere to Ottawa Hydro's planting guidelines (species and setbacks) when planting around overhead primary conductors.
10. Tree specifications
 - a. Minimum stock size: 50mm tree caliper for deciduous, 200cm height for coniferous.
 - b. Maximize the use of large deciduous species wherever possible to maximize future canopy coverage

- c. Tree planting on city property shall be in accordance with the City of Ottawa’s Tree Planting Specification; and include watering and warranty as described in the specification (can be provided by Forestry Services).
- d. Plant native trees whenever possible
- e. No root barriers, dead-man anchor systems, or planters are permitted.
- f. No tree stakes unless necessary (and only 1 on the prevailing winds side of the tree)

11. Hard surface planting

- a. Curb style planter is highly recommended
- b. No grates are to be used and if guards are required, City of Ottawa standard (which can be provided) shall be used.
- c. Trees are to be planted at grade

12. Soil Volume

- a. Please document on the LP that adequate soil volumes can be met:

| Tree Type/Size | Single Tree Soil Volume (m3) | Multiple Tree Soil Volume (m3/tree) |
|----------------|------------------------------|-------------------------------------|
| Ornamental | 15 | 9 |
| Columnar | 15 | 9 |
| Small | 20 | 12 |
| Medium | 25 | 15 |
| Large | 30 | 18 |
| Conifer | 25 | 15 |

*Please note that these soil volumes are not applicable in cases with Sensitive Marine Clay.

13. Sensitive Marine Clay

- a. Please follow the City’s 2017 Tree Planting in Sensitive Marine Clay guidelines

14. Tree Canopy Cover

- a. The landscape plan shall show how the proposed tree planting will replace and increase canopy cover on the site over time, to support the City’s 40% urban forest canopy cover target.

- b. At a site level, efforts shall be made to provide as much canopy cover as possible, through tree planting and tree retention, with an aim of 40% canopy cover at 40 years, as appropriate.
- c. Indicate on the plan the projected future canopy cover at 40 years for the site.

Environment

1. No environmental concerns with the proposed development.

Feel free to contact Sami Rehman, Environmental Planner, at Sami.Rehman@ottawa.ca for follow-up questions.

Rideau Valley Conservation Authority (RVCA)

1. Enhanced Water quality protection be detailed through the SWM report. Either through best management practices and on-site controls or confirmation that the downstream outlet pond provides quality protection.

Feel free to contact Eric Lalande, RVCA Planner, at eric.lalande@rvca.ca for follow-up questions.

City Surveyor

1. The determination of property boundaries, minimum setbacks and other regulatory constraints are a critical component of development. An Ontario Land Surveyor (O.L.S.) needs to be consulted at the outset of a project to ensure properties are properly defined and can be used as the geospatial framework for the development.
2. Topographic details may also be required for a project and should be either carried out by the O.L.S. that has provided the Legal Survey or done in consultation with the O.L.S. to ensure that the project is integrated to the appropriate control network.

Questions regarding the above requirements can be directed to the City's Surveyor, Bill Harper, at Bill.Harper@ottawa.ca

Other

1. Plans are to be standard A1 size (594 mm x 841 mm) or Arch D size (609.6 mm x 914.4 mm) sheets, dimensioned in metric and utilizing an appropriate Metric scale (1:200, 1:250, 1:300, 1:400 or 1:500).
2. All PDF submitted documents are to be unlocked and flattened.
3. You are encouraged to contact the Ward Councillor, Councillor Keith Egli, at Keith.Egli@ottawa.ca about the proposal.

Samantha Bennett

From: Candow, Julie <julie.candow@ottawa.ca>
Sent: Thursday, December 1, 2022 12:51 PM
To: Trevor McKay
Subject: RE: 1545A Merivale Road - Existing Services

Hi Trevor,

I agree with your statement below, that will be acceptable for SPA submission.

"...the existing service will be located during building demolition and blanked at the property line as per City of Ottawa standards."

Thanks

Julie Candow, P.Eng
Project Manager
Planning, Real Estate and Economic Development Department - West Branch
City of Ottawa
110 Laurier Avenue West Ottawa, ON
613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Trevor McKay <t.mckay@novatech-eng.com>
Sent: December 01, 2022 12:27 PM
To: Candow, Julie <julie.candow@ottawa.ca>
Subject: 1545A Merivale Road - Existing Services

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Julie,

The site owners have engaged a utility locator to trace the existing services for the property as noted in the pre-consultation notes. They have been unable to locate the existing sanitary lateral.

We are not planning on reusing the existing lateral (we expect that it is likely not large enough and is likely not aligned with current property boundaries given the age and history of the property) and are proposing a new sanitary service.

Is it a requirement of the site plan application to show the location of the existing services, or would it be all right to note that the existing service will be located during building demolition and blanked at the property line as per City of Ottawa standards?

Thank you again for all your help on this project.

Trevor McKay, B.Eng., E.I.T., Project Manager | Land Development Engineering

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 291 | Cell: 613.263.9113

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From: Candow, Julie <julie.candow@ottawa.ca>
Sent: Tuesday, November 22, 2022 11:30 AM
To: Trevor McKay
Subject: FW: 15435A Merivale Road - PC2022-0137

Hi Trevor,

Please see below from Chris Roy regarding the DMA chamber for 1545A Merivale Road. The City will not be imposing a DMA chamber at this location. I am also okay with the reduced separation for the services entering the property. As long as you provide rationale within your Servicing Study for the reduced separation (small road frontage), and the solution is reasonable, will can accept it.

Happy to still meet tomorrow at 9am if required.
Thanks,

Julie Candow, P.Eng
Project Manager
Planning, Real Estate and Economic Development Department - West Branch
City of Ottawa
110 Laurier Avenue West Ottawa, ON
613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Roy, Christopher <Christopher.Roy@ottawa.ca>
Sent: November 22, 2022 11:02 AM
To: Candow, Julie <julie.candow@ottawa.ca>
Cc: Hannewyk, Joseph <Joseph.Hannewyk@ottawa.ca>
Subject: RE: 15435A Merivale Road - PC2022-0137

Hi Julie,

A DMA chamber is not required here since the private watermain length is approximately 100m. Although this additional detail is not in the design guidelines, 150m of private watermain has been the length cut off of when a DMA chamber is required or not.

Thanks,

Chris

Chris Roy, P.Eng.
Program Engineer, Water Loss Program
Infrastructure & Water Services Department
613.797-1484
Christopher.Roy@ottawa.ca

From: Candow, Julie <julie.candow@ottawa.ca>

Sent: 2022/11/21 1:45 PM

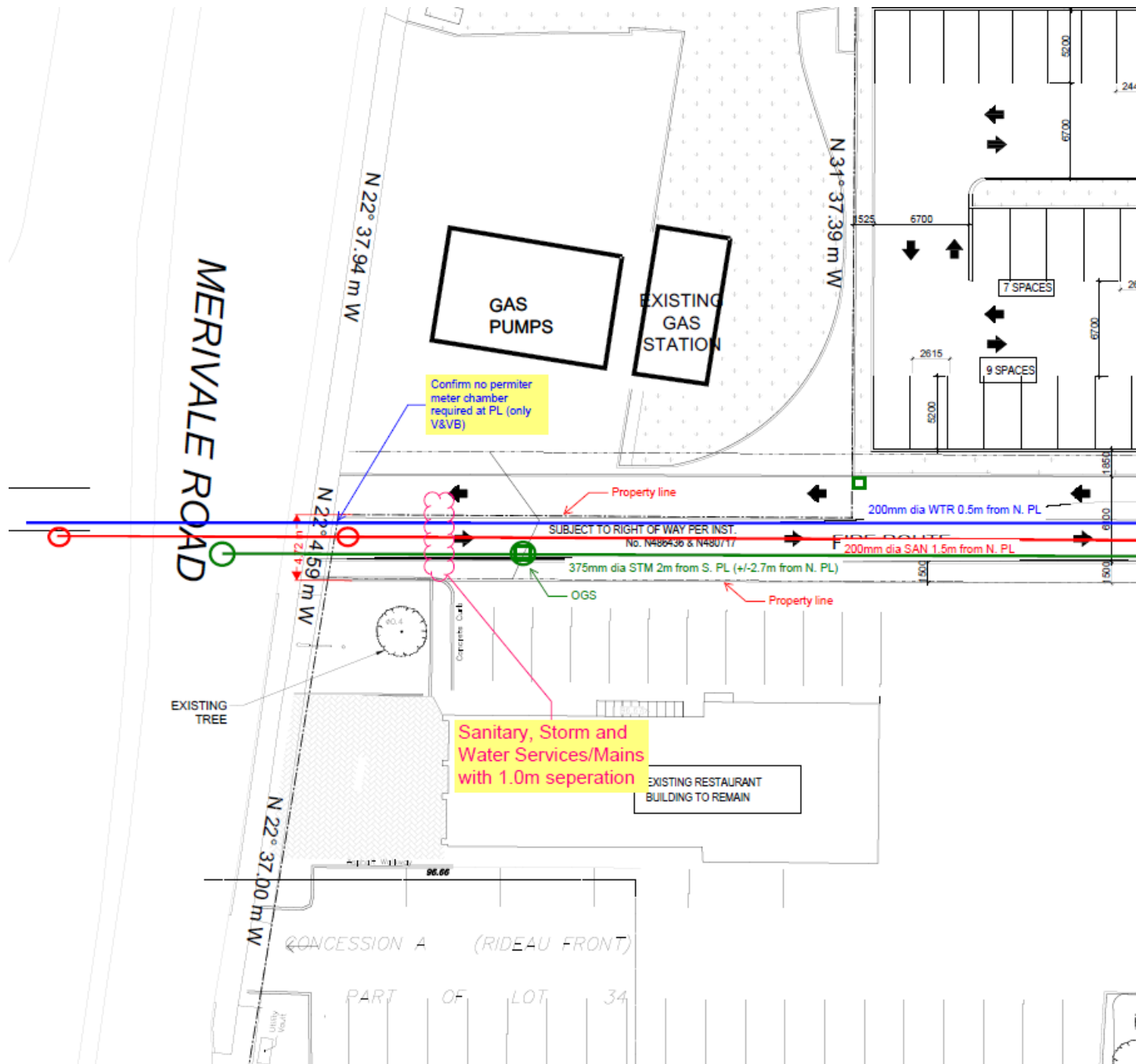
To: Hannewyk, Joseph <Joseph.Hannewyk@ottawa.ca>; Roy, Christopher <Christopher.Roy@ottawa.ca>

Subject: FW: 15435A Merivale Road - PC2022-0137

Hi Joseph and Chris,

I was advised to reach out to you both regarding the need for a District Metering Area chamber at a private site (1545A Merivale Road) for a proposed medical imaging building. They have not submitted a formal application yet but Novatech has inquired as to whether a DMA chamber will be requested. As per the water design guidelines, a single DMA chamber is required for private developments when serviced by a connection of 150mm or larger. They have a very small frontage on Merivale Road, +/- 4.7m and do not see any way of accommodating a DMA chamber at the Merivale Road property line if one were to be requested. Please see attached site plan and also below snip of proposed services. Can you please confirm if a DMA chamber will be enforced by the City on this site plan.

Thank you both, and let me know if any additional information is required.



Julie Candow, P.Eng
 Project Manager
 Planning, Real Estate and Economic Development Department - West Branch
 City of Ottawa
 110 Laurier Avenue West Ottawa, ON
 613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Trevor McKay <t.mckay@novatech-eng.com>
Sent: November 21, 2022 10:39 AM
To: Candow, Julie <julie.candow@ottawa.ca>
Cc: McGirr, Emily [NN-CA] <emily.mcgirr@parsons.com>
Subject: 15435A Merivale Road - PC2022-0137

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Julie,

I hope you are doing well. I'm reaching out regarding the upcoming site plan application for the 1545A Merivale Road property.

I know that everyone at the City is very busy right now, however I was wondering if you would be able to spare 30 minutes or less in the next day or two to review the proposed servicing alignment for the site. In particular, I would like to get your thoughts on the proposed +/-1.0m separation between the storm, sanitary and water mains as they enter the site. The reasoning behind the decreased separation is that there is only +/- 4.7m of site frontage on Merivale Road so we are limited in the available space.

The second question I have is whether a perimeter water meter chamber is going to be requested for this site. Given the lack of space at the property line, we do not see any way to accommodate one at the Merivale Road property line if it is requested.

I've attached the current site plan concept with a pdf markup of the proposed servicing layout and the pre-consultation meeting notes. I'm busy 12-1pm today, however I can make myself available any other time over the next few days.

613-263-9113 or teams/zoom.

Thanks,

Trevor McKay, B.Eng., E.I.T., Project Manager | Land Development Engineering

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 291 | Cell: 613.263.9113

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,

Kallie Auld

From: Candow, Julie <julie.candow@ottawa.ca>
Sent: Tuesday, November 29, 2022 12:29 PM
To: Eric Lalande; Kallie Auld
Cc: Trevor McKay
Subject: RE: 1545A Merivale Road water quality criteria
Attachments: R-5572.pdf

Hi Kallie,

I apologize for the delay. Based on the attached Nepean Creek Stormwater Facility 2005 Annual Report, it appears that the criteria for this pond is 70% TSS removal. That said, I have reached out to our Asset Management branch to try and locate the original SWM Report and/or a more recent monitoring report. The attached is the only report I could find in our digital records.

I will keep you undated with any further details provided by our Asset Management branch. From there, I will let RVCA decide if additional TSS removal will be required for this site.

Thanks,

Julie Candow, P.Eng
Project Manager
Planning, Real Estate and Economic Development Department - West Branch
City of Ottawa
110 Laurier Avenue West Ottawa, ON
613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Eric Lalande <eric.lalande@rvca.ca>
Sent: November 29, 2022 10:03 AM
To: Kallie Auld <k.auld@novatech-eng.com>; Candow, Julie <julie.candow@ottawa.ca>
Cc: Trevor McKay <t.mckay@novatech-eng.com>
Subject: RE: 1545A Merivale Road water quality criteria

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Hi Kallie,

I'll defer to the City, as don't have access to the pond design criteria, currently. Based on memory I believe it does, but I would look to confirm that formally.

Thanks,

Eric Lalande, MCIP, RPP

Planner, RVCA

613-692-3571 x1137

From: Kallie Auld <k.auld@novatech-eng.com>

Sent: Tuesday, November 29, 2022 8:47 AM

To: Eric Lalande <eric.lalande@rvca.ca>; Julie.candow@ottawa.ca

Cc: Trevor McKay <t.mckay@novatech-eng.com>

Subject: RE: 1545A Merivale Road water quality criteria

Good morning Eric/ Julie,

I'm just following up on my email below.

Thanks very much,

Kallie Auld, P.Eng. (she/her), Project Manager | Water Resources

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x 294 | Fax: 613.254.5867 | Email: k.auld@novatech-eng.com

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Kallie Auld

Sent: Wednesday, November 23, 2022 1:29 PM

To: eric.lalande@rvca.ca; Julie.candow@ottawa.ca

Cc: Trevor McKay <t.mckay@novatech-eng.com>

Subject: 1545A Merivale Road water quality criteria

Good afternoon Eric/ Julie,

I'm emailing you to check about the water quality requirements for our site located at 1545A Merivale Road. In the preconsult meeting minutes it states that *"Enhanced Water quality protection be detailed through the SWM report. Either through best management practices and on-site controls or confirmation that the downstream outlet pond provides quality protection."*

Per geoOttawa, outflows from 1545A Merivale Road outlet to the storm sewers on Merivale Road which outlet approximately 1.8 km downstream to Nepean Creek, which then flows into an existing SWM facility, which ultimately outlets to the Rideau River (please see attached screenshot).

I was wondering if either of you have any information about this existing SWM facility and if it would provide the required water quality protection for our site, or if you think on-site controls would be required.

Thanks very much,

Kallie Auld, P.Eng. (she/her), Project Manager | Water Resources

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x 294 | Fax: 613.254.5867 | Email: k.auld@novatech-eng.com

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Kallie Auld

From: Eric Lalande <eric.lalande@rvca.ca>
Sent: Wednesday, November 30, 2022 10:08 AM
To: Kallie Auld; Julie.candow@ottawa.ca
Cc: Trevor McKay
Subject: RE: 1545A Merivale Road water quality criteria

Hi Kallie,

With some follow up through City staff, they were able to confirm the design parameters of the SWM Facility. It was designed to provide quality control (albeit at a minimum design of 70% TSS). That being said the pond performance varies between 70-90%.

The RVCA will not require any additional on-site quality control measures, however, encourage best management practices to be implemented where feasible.

Thank you,

Eric Lalande, MCIP, RPP
Planner, RVCA
613-692-3571 x1137

From: Kallie Auld <k.auld@novatech-eng.com>
Sent: Tuesday, November 29, 2022 8:47 AM
To: Eric Lalande <eric.lalande@rvca.ca>; Julie.candow@ottawa.ca
Cc: Trevor McKay <t.mckay@novatech-eng.com>
Subject: RE: 1545A Merivale Road water quality criteria

Good morning Eric/ Julie,

I'm just following up on my email below.

Thanks very much,

Kallie Auld, P.Eng. (she/her), Project Manager | Water Resources

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x 294 | Fax: 613.254.5867 | Email: k.auld@novatech-eng.com

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Kallie Auld
Sent: Wednesday, November 23, 2022 1:29 PM
To: eric.lalande@rvca.ca; julie.candow@ottawa.ca
Cc: Trevor McKay <t.mckay@novatech-eng.com>
Subject: 1545A Merivale Road water quality criteria

Good afternoon Eric/ Julie,

I'm emailing you to check about the water quality requirements for our site located at 1545A Merivale Road. In the preconsult meeting minutes it states that *"Enhanced Water quality protection be detailed through the SWM report. Either through best management practices and on-site controls or confirmation that the downstream outlet pond provides quality protection."*

Per geoOttawa, outflows from 1545A Merivale Road outlet to the storm sewers on Merivale Road which outlet approximately 1.8 km downstream to Nepean Creek, which then flows into an existing SWM facility, which ultimately outlets to the Rideau River (please see attached screenshot).

I was wondering if either of you have any information about this existing SWM facility and if it would provide the required water quality protection for our site, or if you think on-site controls would be required.

Thanks very much,

Kallie Auld, P.Eng. (she/her), Project Manager | Water Resources

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x 294 | Fax: 613.254.5867 | Email: k.auld@novatech-eng.com

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APPENDIX B

Sanitary Sewer Design Sheet

Merivale Medical Imaging Clinic - 1545A Merivale Road: Sanitary Sewer Design Sheet

| AREA | | | COMMERCIAL | | | | INFILTRATION | | | Total Flow (l/s) | PIPE | | | | | | | |
|------|--------------|------------|------------|-------------|------------------|-----------------|-----------------|------------------|-------------------|------------------|-----------|-----------|------------|----------------|----------------------|-------------------|-------------------------|-------|
| ID | From | To | GROSS HA | PEAK FACTOR | Accum. Area (ha) | Peak Flow (L/s) | Total Area (ha) | Accum. Area (ha) | Infil. Flow (l/s) | | Size (mm) | Slope (%) | Length (m) | Capacity (L/s) | Full Flow Vel. (m/s) | Actual Vel. (m/s) | Q/Q _{full} (%) | d/D |
| | BUILD | 103 | 0.69 | 1.5 | 0.69 | 0.60 | 0.69 | 0.69 | 0.23 | 0.83 | 200 | 2.00 | 7.0 | 48.4 | 1.49 | 0.49 | 1.7% | 0.077 |
| | 103 | 101 | 0.00 | 1.5 | 0.69 | 0.60 | 0.00 | 0.69 | 0.23 | 0.83 | 200 | 0.40 | 66.9 | 21.6 | 0.67 | 0.26 | 3.8% | 0.132 |
| | 101 | 99 | 0.00 | 1.5 | 0.69 | 0.60 | 0.00 | 0.69 | 0.23 | 0.83 | 200 | 1.00 | 22.7 | 34.2 | 1.06 | 0.36 | 2.4% | 0.108 |
| | | | | | | | | | | | | | | | | | | |

Design Parameters:

Commercial average flow 50000 l/gross ha/day
 Infiltration = 0.33 l/s/ha
 Pipe Friction n = 0.013
 Peaking Factor = 1.5

Project: 1545A Merivale Road (122098)

Designed: SAB

Checked: TJM

Date: December 23, 2022



APPENDIX C

Storm Sewer Design Sheet

Merivale Medical Imaging Clinic - 1545A Merivale Road: Storm Sewer Design Sheet (Rational Method)

| LOCATION | | | AREA | | | | FLOW | | | | | | PROPOSED SEWER | | | | | | | |
|----------------------------|-----------|---------|--------------|--------------|------------|-----------------------------|----------------|---------------|-----------------------|-------------------------|-----------|---------------------|----------------|------|-------|--------|----------|--------------------|--------------|---------|
| Location | From Node | To Node | Hard Surface | Soft Surface | Total Area | Weighted Runoff Coefficient | Indivi 2.78 AR | Accum 2.78 AR | Time of Concentration | Rain Intensity (2 year) | Peak Flow | Total Peak Flow (Q) | Pipe | Size | Grade | Length | Capacity | Full Flow Velocity | Time of Flow | Q/Qfull |
| | | | 0.90 | 0.20 | (ha) | | | | (min) | (mm/hr) | (L/s) | (L/s) | Type | (mm) | (%) | (m) | (l/s) | (m/s) | (min.) | (%) |
| A-1 | LCB-1 | CBMH2 | 0.001 | 0.008 | 0.009 | 0.28 | 0.01 | 0.01 | 10.00 | 76.81 | 0.6 | 0.6 | PVC | 200 | 1.00 | 31.4 | 34.2 | 1.06 | 0.50 | 1.6% |
| A-2 | CBMH2 | CBMH3 | 0.046 | 0.001 | 0.046 | 0.89 | 0.12 | 0.12 | 10.50 | 74.95 | 9.2 | 9.2 | PVC | 250 | 0.50 | 26.0 | 43.9 | 0.87 | 0.50 | 20.9% |
| A-3 | CBMH3 | CBMH4 | 0.032 | 0.000 | 0.032 | 0.90 | 0.08 | 0.20 | 11.00 | 73.18 | 14.9 | 14.9 | PVC | 250 | 0.50 | 33.5 | 43.9 | 0.87 | 0.64 | 33.9% |
| A-4 | CBMH4 | CBMH5 | 0.039 | 0.000 | 0.039 | 0.90 | 0.10 | 0.30 | 11.64 | 71.03 | 21.4 | 21.4 | PVC | 250 | 0.50 | 30.7 | 43.9 | 0.87 | 0.59 | 48.7% |
| A-5 | CBMH5 | 104 | 0.046 | 0.011 | 0.057 | 0.77 | 0.12 | 0.42 | 12.23 | 69.18 | 29.2 | 29.2 | PVC | 250 | 0.50 | 26.3 | 43.9 | 0.87 | 0.51 | 66.5% |
| A-6 | CBMH6 | CBMH7 | 0.049 | 0.007 | 0.056 | 0.82 | 0.13 | 0.13 | 10.00 | 76.81 | 9.7 | 9.7 | PVC | 450 | 1.00 | 13.2 | 297.4 | 1.81 | 0.12 | 3.3% |
| A-7 | CBMH7 | 104 | 0.071 | 0.000 | 0.072 | 0.90 | 0.18 | 0.31 | 10.12 | 76.34 | 23.3 | 23.3 | PVC | 250 | 0.50 | 16.9 | 43.9 | 0.87 | 0.33 | 53.1% |
| B-1, B-2 | BLDG | 104 | 0.272 | 0.000 | 0.272 | 0.90 | 0.68 | 0.68 | 10.00 | 76.81 | 52.2 | 52.2 | PVC | 300 | 2.00 | 12.3 | 142.7 | 1.96 | 0.10 | 36.6% |
| A-8, A-9, OS-1, OS-2, DR-4 | 104 | 102 | 0.099 | 0.038 | 0.137 | 0.71 | 0.27 | 1.68 | 12.74 | 67.68 | 113.4 | 113.4 | PVC | 375 | 0.45 | 59.5 | 122.7 | 1.08 | 0.92 | 92.4% |
| - | 102 | CONN | 0.000 | 0.000 | 0.000 | - | 0.00 | 1.68 | 13.66 | 65.12 | 109.1 | 109.1 | PVC | 375 | 1.00 | 12.3 | 182.9 | 1.60 | 0.13 | 59.7% |

Project: Merivale Medical Imaging Clinic - 1545A Merivale Road (122098)

Q = 2.78 AIR

WHERE : Q = PEAK FLOW IN LITRES PER SECOND (L/s)

A = AREA IN HECTARES (ha)

I = RAINFALL INTENSITY IN MILLIMETERS PER HOUR (mm/hr)

R = WEIGHTED RUNOFF COEFFICIENT

$Q = (1/n) A R^{2/3} S_0^{1/2}$

WHERE : Q = CAPACITY (L/s)

n = MANNING COEFFICIENT OF ROUGHNESS (0.013)

A = FLOW AREA (m²)

Designed: SAB

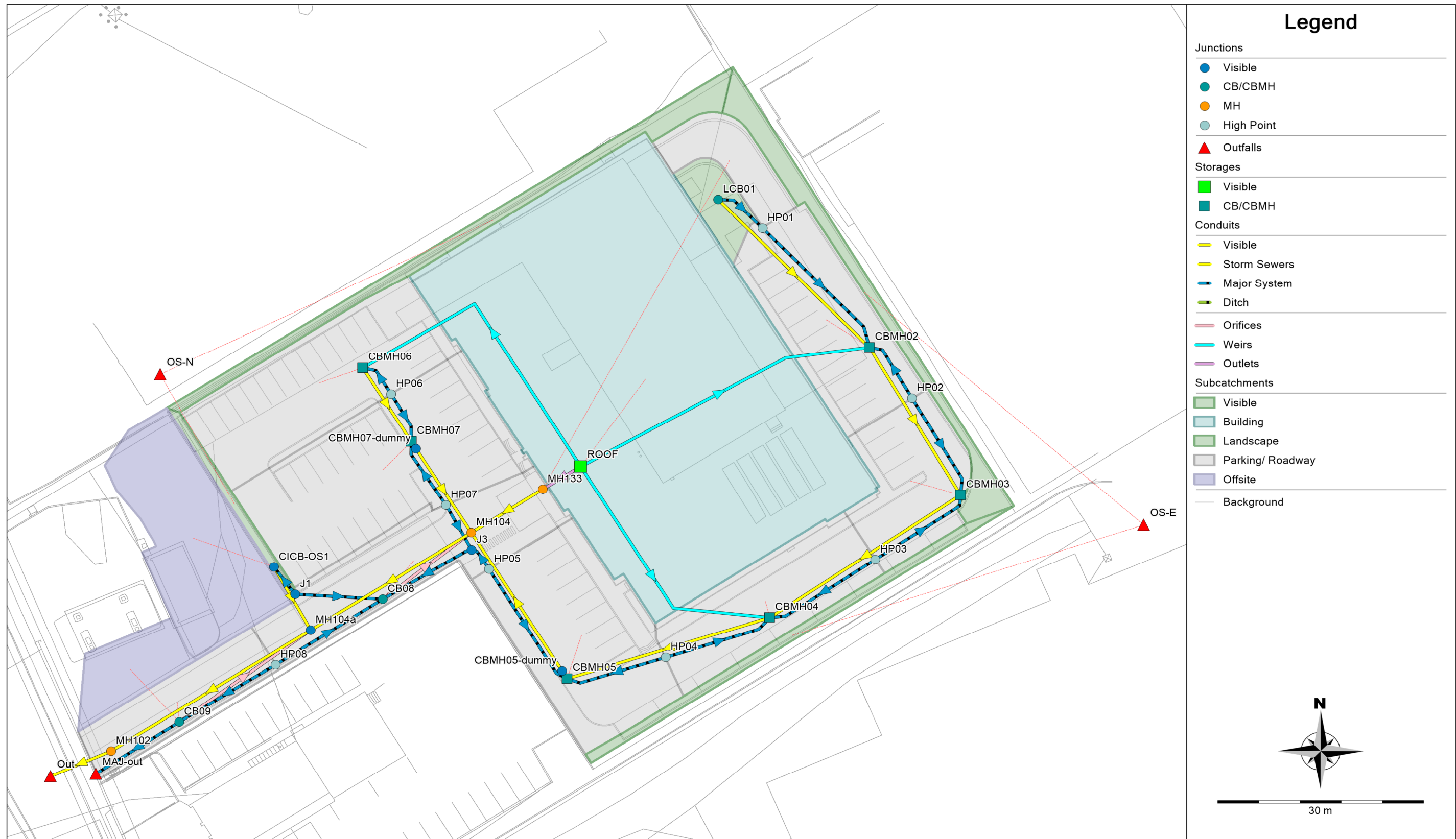
Checked: TJM

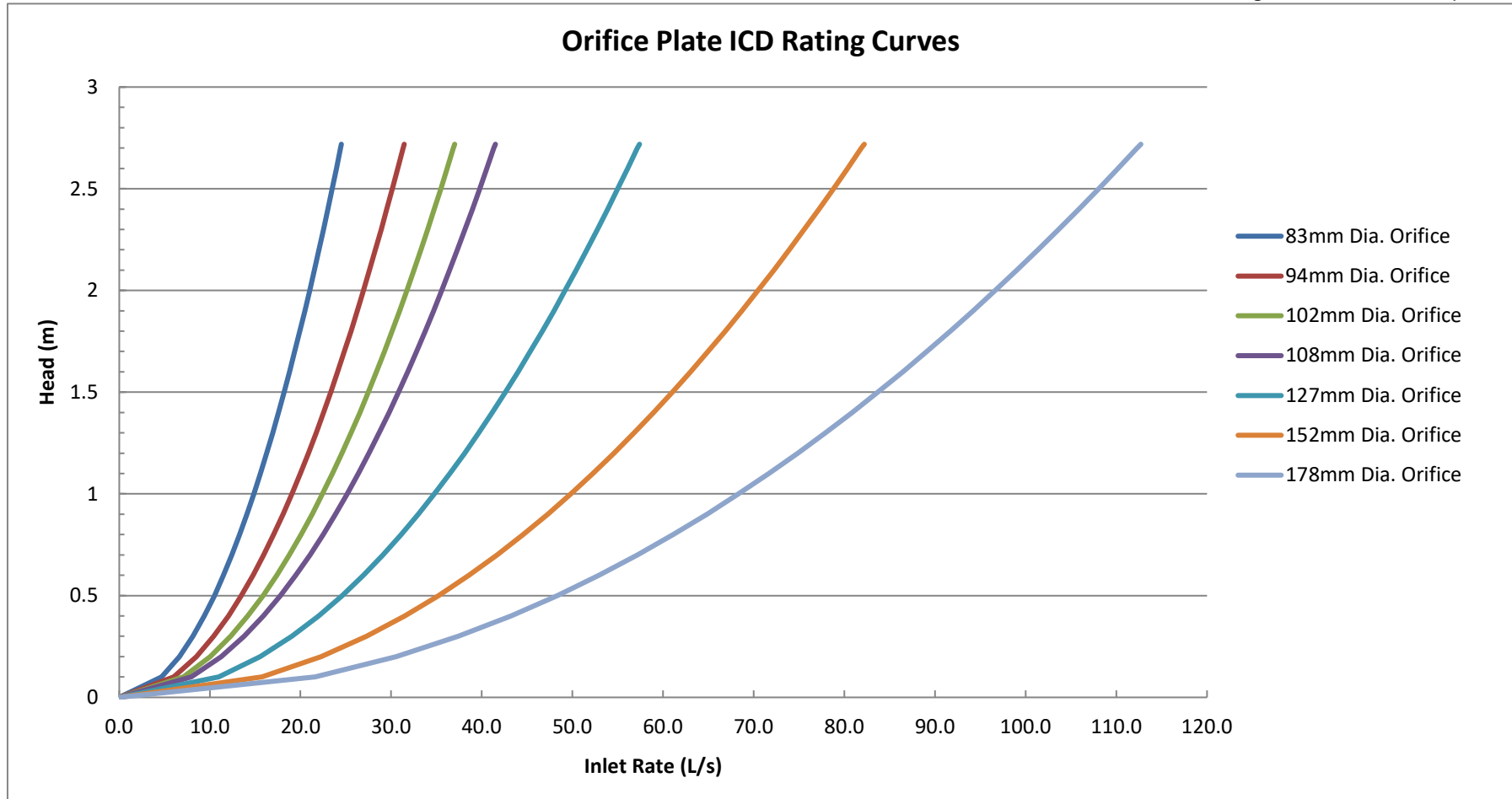
Date: December 23, 2022

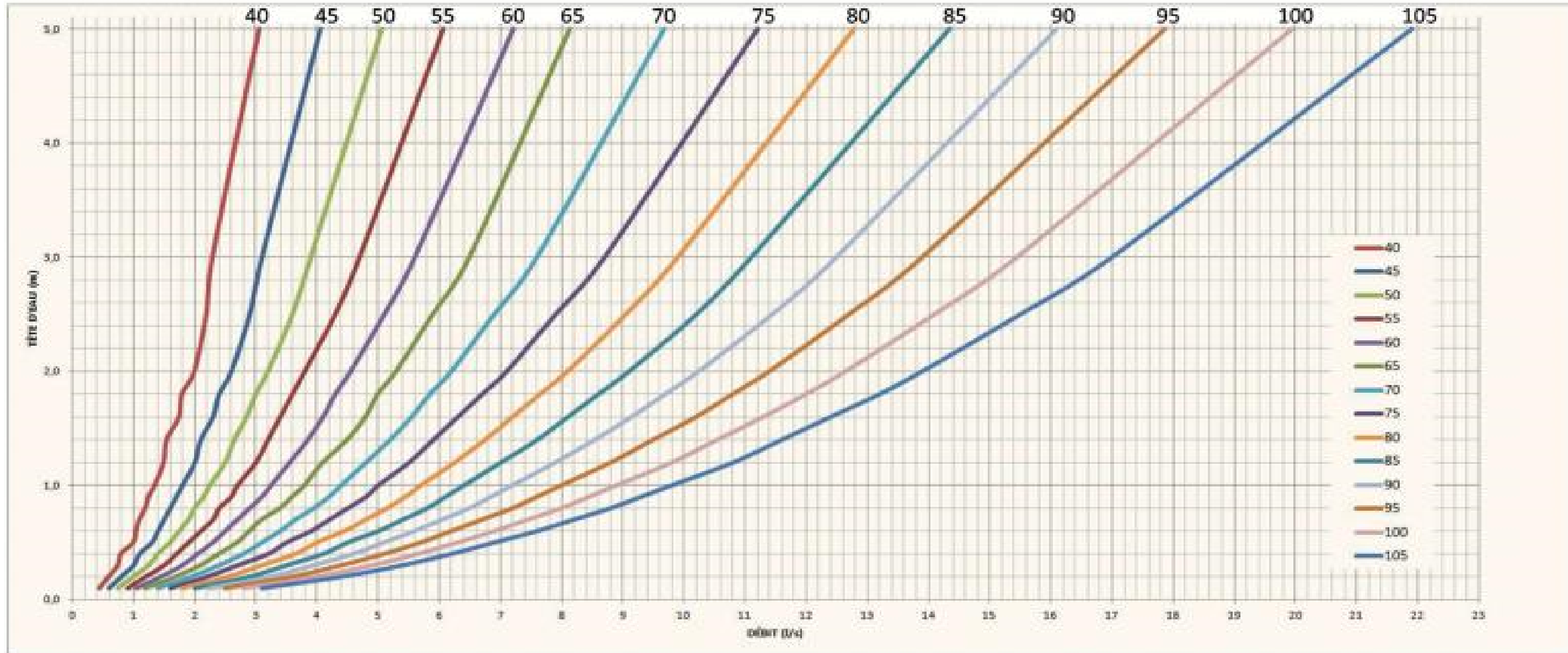


APPENDIX D

SWM Calculations







1545 Merivale Road
ROW Ponding Depths

| Structure | T/G (m) | Max. Static Ponding (Spill Depth) | | 2-yr Event (3hr) | | | | 5-yr Event (3hr) | | | | 100-yr Event (3hr) | | | | 100-yr Event (+20%) (3hr) | | | |
|-----------|------------|--------------------------------------|--------------|------------------|--------------|--------------------|----------------------|------------------|--------------|--------------------|----------------------|--------------------|--------------|--------------------|----------------------|---------------------------|--------------|--------------------|----------------------|
| | | Elev. (m) | Depth (m) | Elev. (m) | Depth (m) | Cascading Flow? | Cascade Depth (m) | Elev. (m) | Depth (m) | Cascading Flow? | Cascade Depth (m) | Elev. (m) | Depth (m) | Cascading Flow? | Cascade Depth (m) | Elev. (m) | Depth (m) | Cascading Flow? | Cascade Depth (m) |
| CBMH02 | 96.50 | 96.70 | 0.20 | 96.34 | 0.00 | N | 0.00 | 96.54 | 0.04 | N | 0.00 | 96.66 | 0.16 | N | 0.00 | 96.70 | 0.20 | N | 0.00 |
| CBMH03 | 96.50 | 96.70 | 0.20 | 96.34 | 0.00 | N | 0.00 | 96.54 | 0.04 | N | 0.00 | 96.65 | 0.15 | N | 0.00 | 96.70 | 0.20 | N | 0.00 |
| CBMH04 | 96.50 | 96.70 | 0.20 | 96.34 | 0.00 | N | 0.00 | 96.54 | 0.04 | N | 0.00 | 96.65 | 0.15 | N | 0.00 | 96.70 | 0.20 | N | 0.00 |
| CBMH05 | 96.45 | 96.70 | 0.25 | 96.33 | 0.00 | N | 0.00 | 96.53 | 0.08 | N | 0.00 | 96.65 | 0.20 | N | 0.00 | 96.69 | 0.24 | N | 0.00 |
| CBMH06 | 96.45 | 96.70 | 0.25 | 96.08 | 0.00 | N | 0.00 | 96.48 | 0.03 | N | 0.00 | 96.55 | 0.10 | N | 0.00 | 96.58 | 0.13 | N | 0.00 |
| CBMH07 | 96.45 | 96.70 | 0.25 | 96.08 | 0.00 | N | 0.00 | 96.48 | 0.03 | N | 0.00 | 96.55 | 0.10 | N | 0.00 | 96.58 | 0.13 | N | 0.00 |
| CB08 | 96.55 | 96.70 | 0.15 | 96.55 | 0.00 | N | 0.00 | 96.57 | 0.02 | N | 0.00 | 96.61 | 0.06 | N | 0.00 | 96.65 | 0.10 | N | 0.00 |
| CB09 | 96.55 | 96.69 | 0.14 | 96.55 | 0.00 | N | 0.00 | 96.57 | 0.02 | N | 0.00 | 96.62 | 0.07 | N | 0.00 | 96.65 | 0.10 | N | 0.00 |

1545A Merivale
Major Flow

| Location | 100-year | | | | | 100-year +20% | | | |
|----------------------------------|----------------------------------|-------------------|---------------------|--|---|----------------------------------|-------------------|--------------------|---|
| | Peak Flow (m ³ /s) | Velocity (m/s) | Static Depth (m) | Total Depth (static + dynamic) (m) | Velocity x Depth (m ² /s) | Peak Flow (m ³ /s) | Velocity (m/s) | Total Depth (m) | Velocity x Depth (m ² /s) |
| Catchbasins at Low Points | | | | | | | | | |
| CBMH02 | 0.027 | 0.00 | 0.20 | 0.16 | 0.00 | 0.039 | 0.01 | 0.20 | 0.00 |
| CBMH03 | 0.027 | 0.00 | 0.20 | 0.15 | 0.00 | 0.032 | 0.01 | 0.20 | 0.00 |
| CBMH04 | 0.030 | 0.00 | 0.20 | 0.15 | 0.00 | 0.034 | 0.00 | 0.20 | 0.00 |
| CBMH05 | 0.039 | 0.00 | 0.25 | 0.20 | 0.00 | 0.044 | 0.00 | 0.24 | 0.00 |
| CBMH06 | 0.027 | 0.00 | 0.25 | 0.10 | 0.00 | 0.036 | 0.00 | 0.13 | 0.00 |
| CBMH07 | 0.051 | 0.00 | 0.25 | 0.10 | 0.00 | 0.055 | 0.00 | 0.13 | 0.00 |
| CB08 | 0.023 | 0.00 | 0.15 | 0.06 | 0.00 | 0.027 | 0.00 | 0.10 | 0.00 |
| CB09 | 0.018 | 0.00 | 0.14 | 0.07 | 0.00 | 0.022 | 0.00 | 0.10 | 0.00 |
| High Points | | | | | | | | | |
| HP01 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.000 | 0.00 | 0.00 | 0.00 |
| HP02 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.003 | 0.01 | 0.01 | 0.00 |
| HP03 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.001 | 0.00 | 0.01 | 0.00 |
| HP04 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.000 | 0.02 | 0.00 | 0.00 |
| HP05 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.000 | 0.00 | 0.00 | 0.00 |
| HP06 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.000 | 0.00 | 0.00 | 0.00 |
| HP07 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.000 | 0.00 | 0.00 | 0.00 |
| HP08 | 0.000 | 0.00 | - | 0.00 | 0.00 | 0.000 | 0.00 | 0.00 | 0.00 |

1545A Merivale Road
Design Storm Time Series Data
3-hour Chicago Design Storms



| C25mm-3.stm | | C2-3.stm | | C5-3.stm | |
|-------------|-----------|----------|-----------|----------|-----------|
| Duration | Intensity | Duration | Intensity | Duration | Intensity |
| min | mm/hr | min | mm/hr | min | mm/hr |
| 0:00 | 0 | 0:00 | 0 | 0:00 | 0 |
| 0:10 | 2.21 | 0:10 | 2.81 | 0:10 | 3.68 |
| 0:20 | 2.75 | 0:20 | 3.5 | 0:20 | 4.58 |
| 0:30 | 3.68 | 0:30 | 4.69 | 0:30 | 6.15 |
| 0:40 | 5.73 | 0:40 | 7.3 | 0:40 | 9.61 |
| 0:50 | 14.29 | 0:50 | 18.21 | 0:50 | 24.17 |
| 1:00 | 60.28 | 1:00 | 76.81 | 1:00 | 104.19 |
| 1:10 | 18.9 | 1:10 | 24.08 | 1:10 | 32.04 |
| 1:20 | 9.7 | 1:20 | 12.36 | 1:20 | 16.34 |
| 1:30 | 6.53 | 1:30 | 8.32 | 1:30 | 10.96 |
| 1:40 | 4.94 | 1:40 | 6.3 | 1:40 | 8.29 |
| 1:50 | 3.99 | 1:50 | 5.09 | 1:50 | 6.69 |
| 2:00 | 3.37 | 2:00 | 4.29 | 2:00 | 5.63 |
| 2:10 | 2.92 | 2:10 | 3.72 | 2:10 | 4.87 |
| 2:20 | 2.58 | 2:20 | 3.29 | 2:20 | 4.3 |
| 2:30 | 2.32 | 2:30 | 2.95 | 2:30 | 3.86 |
| 2:40 | 2.1 | 2:40 | 2.68 | 2:40 | 3.51 |
| 2:50 | 1.93 | 2:50 | 2.46 | 2:50 | 3.22 |
| 3:00 | 1.79 | 3:00 | 2.28 | 3:00 | 2.98 |

1545A Merivale Road
Design Storm Time Series Data
3-hour Chicago Design Storms



| C100-3.stm | | C100-3+20%.stm | |
|------------|-----------|----------------|-----------|
| Duration | Intensity | Duration | Intensity |
| min | mm/hr | min | mm/hr |
| 0:00 | 0 | 0:00 | 0 |
| 0:10 | 6.05 | 0:10 | 6.14 |
| 0:20 | 7.54 | 0:20 | 9.05 |
| 0:30 | 10.16 | 0:30 | 12.19 |
| 0:40 | 15.97 | 0:40 | 19.16 |
| 0:50 | 40.65 | 0:50 | 48.78 |
| 1:00 | 178.56 | 1:00 | 214.27 |
| 1:10 | 54.05 | 1:10 | 64.86 |
| 1:20 | 27.32 | 1:20 | 32.78 |
| 1:30 | 18.24 | 1:30 | 21.89 |
| 1:40 | 13.74 | 1:40 | 16.49 |
| 1:50 | 11.06 | 1:50 | 13.27 |
| 2:00 | 9.29 | 2:00 | 11.15 |
| 2:10 | 8.02 | 2:10 | 9.62 |
| 2:20 | 7.08 | 2:20 | 8.5 |
| 2:30 | 6.35 | 2:30 | 7.62 |
| 2:40 | 5.76 | 2:40 | 6.91 |
| 2:50 | 5.28 | 2:50 | 6.34 |
| 3:00 | 4.88 | 3:00 | 5.86 |

1545A Merivale Road
Design Storm Time Series Data
SCS Design Storms



| S2-12.stm | | S5-12.stm | | S100-12.stm | |
|-----------|-----------|-----------|-----------|-------------|-----------|
| Duration | Intensity | Duration | Intensity | Duration | Intensity |
| min | mm/hr | min | mm/hr | min | mm/hr |
| 0:00 | 0.00 | 0:00 | 0 | 0:00 | 0 |
| 0:30 | 1.27 | 0:30 | 1.69 | 0:30 | 2.82 |
| 1:00 | 0.59 | 1:00 | 0.79 | 1:00 | 1.31 |
| 1:30 | 1.10 | 1:30 | 1.46 | 1:30 | 2.44 |
| 2:00 | 1.10 | 2:00 | 1.46 | 2:00 | 2.44 |
| 2:30 | 1.44 | 2:30 | 1.91 | 2:30 | 3.19 |
| 3:00 | 1.27 | 3:00 | 1.69 | 3:00 | 2.82 |
| 3:30 | 1.69 | 3:30 | 2.25 | 3:30 | 3.76 |
| 4:00 | 1.69 | 4:00 | 2.25 | 4:00 | 3.76 |
| 4:30 | 2.29 | 4:30 | 3.03 | 4:30 | 5.07 |
| 5:00 | 2.88 | 5:00 | 3.82 | 5:00 | 6.39 |
| 5:30 | 4.57 | 5:30 | 6.07 | 5:30 | 10.14 |
| 6:00 | 36.24 | 6:00 | 48.08 | 6:00 | 80.38 |
| 6:30 | 9.23 | 6:30 | 12.25 | 6:30 | 20.47 |
| 7:00 | 4.06 | 7:00 | 5.39 | 7:00 | 9.01 |
| 7:30 | 2.71 | 7:30 | 3.59 | 7:30 | 6.01 |
| 8:00 | 2.37 | 8:00 | 3.15 | 8:00 | 5.26 |
| 8:30 | 1.86 | 8:30 | 2.47 | 8:30 | 4.13 |
| 9:00 | 1.95 | 9:00 | 2.58 | 9:00 | 4.32 |
| 9:30 | 1.27 | 9:30 | 1.69 | 9:30 | 2.82 |
| 10:00 | 1.02 | 10:00 | 1.35 | 10:00 | 2.25 |
| 10:30 | 1.44 | 10:30 | 1.91 | 10:30 | 3.19 |
| 11:00 | 0.93 | 11:00 | 1.24 | 11:00 | 2.07 |
| 11:30 | 0.85 | 11:30 | 1.12 | 11:30 | 1.88 |
| 12:00 | 0.85 | 12:00 | 1.12 | 12:00 | 1.88 |

Roof Drain Calculations Summary

5-Year

| Area ID | Static Ponding Area (m ²) | Drainage Area (ha) | Runoff Coef. (5-year) | Time-of-Conc. (min) | Rainfall Intensity mm/hr | Uncontrolled Peak Flow (L/s) | Roof Drain Flow Control System | Setting | Controlled Peak Flow (L/s) | Flow Depth (m) | Storage Required (m ³) | Storage Available (m ³) |
|--------------|---------------------------------------|--------------------|-----------------------|---------------------|--------------------------|------------------------------|--------------------------------|------------|----------------------------|----------------|------------------------------------|-------------------------------------|
| R-01 | 299 | 0.038 | 0.90 | 10.00 | 104.19 | 9.8 | Watts Flow Control | Fully Open | 1.26 | 0.12 | 6.99 | 14.97 |
| R-02 | 300 | 0.043 | 0.90 | 10.00 | 104.19 | 11.1 | Watts Flow Control | Fully Open | 1.26 | 0.12 | 8.32 | 14.98 |
| R-03 | 339 | 0.045 | 0.90 | 10.00 | 104.19 | 11.7 | Watts Flow Control | Fully Open | 1.26 | 0.12 | 8.94 | 16.95 |
| R-04 | 305 | 0.045 | 0.90 | 10.00 | 104.19 | 11.8 | Watts Flow Control | Fully Open | 1.26 | 0.13 | 9.03 | 15.23 |
| R-05 | 271 | 0.037 | 0.90 | 10.00 | 104.19 | 9.7 | Watts Flow Control | Fully Open | 1.26 | 0.12 | 6.82 | 13.54 |
| R-06 | 205 | 0.023 | 0.90 | 10.00 | 104.19 | 5.9 | Watts Flow Control | Fully Open | 1.26 | 0.10 | 3.31 | 10.25 |
| R-07 | 205 | 0.023 | 0.90 | 10.00 | 104.19 | 5.9 | Watts Flow Control | Fully Open | 1.26 | 0.10 | 3.31 | 10.24 |
| TOTAL | | 0.253 | | | | 66.0 | | | 8.82 | | 46.73 | 96.16 |

100-Year

| Area ID | Static Ponding Area (m ²) | Drainage Area (ha) | Runoff Coef. (100-year) | Time-of-Conc. (min) | Rainfall Intensity mm/hr | Uncontrolled Peak Flow (L/s) | Roof Drain Flow Control System | Setting | Controlled Peak Flow (L/s) | Flow Depth (m) | Storage Required (m ³) | Storage Available (m ³) |
|--------------|---------------------------------------|--------------------|-------------------------|---------------------|--------------------------|------------------------------|--------------------------------|------------|----------------------------|----------------|------------------------------------|-------------------------------------|
| R-01 | 299 | 0.038 | 1.00 | 10.00 | 178.56 | 18.7 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 14.97 | 14.97 |
| R-02 | 300 | 0.043 | 1.00 | 10.00 | 178.56 | 21.2 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 14.98 | 14.98 |
| R-03 | 339 | 0.045 | 1.00 | 10.00 | 178.56 | 22.3 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 16.95 | 16.95 |
| R-04 | 305 | 0.045 | 1.00 | 10.00 | 178.56 | 22.5 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 15.23 | 15.23 |
| R-05 | 271 | 0.037 | 1.00 | 10.00 | 178.56 | 18.4 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 13.54 | 13.54 |
| R-06 | 205 | 0.023 | 1.00 | 10.00 | 178.56 | 11.3 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 10.25 | 10.25 |
| R-07 | 205 | 0.023 | 1.00 | 10.00 | 178.56 | 11.3 | Watts Flow Control | Fully Open | 1.89 | 0.15 | 10.24 | 10.24 |
| TOTAL | | 0.253 | | | | 125.8 | | | 13.23 | | 96.16 | 96.16 |

1545A Merivale Road
Roof Drain Calculations

| Area ID R-01 | | | | |
|--------------------|-------------------|------------------------------|------------|-----------------------|
| 5-Year Event | | Static Ponding Area = 299 m2 | | |
| Area = 0.038 ha ha | | Qcontrolled = 1.26 L/s | | |
| C = 0.90 | | Vol(req.) = 6.99 m3 | | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 141.18 | 13.33 | 12.07 | 3.62 |
| 10 | 104.19 | 9.84 | 8.58 | 5.15 |
| 15 | 83.56 | 7.89 | 6.63 | 5.97 |
| 20 | 70.25 | 6.63 | 5.37 | 6.45 |
| 25 | 60.90 | 5.75 | 4.49 | 6.74 |
| 30 | 53.93 | 5.09 | 3.83 | 6.90 |
| 35 | 48.52 | 4.58 | 3.32 | 6.97 |
| 40 | 44.18 | 4.17 | 2.91 | 6.99 |
| 45 | 40.63 | 3.84 | 2.58 | 6.96 |
| 50 | 37.65 | 3.56 | 2.30 | 6.89 |
| 55 | 35.12 | 3.32 | 2.06 | 6.79 |
| 60 | 32.94 | 3.11 | 1.85 | 6.66 |
| 65 | 31.04 | 2.93 | 1.67 | 6.52 |
| 70 | 29.37 | 2.77 | 1.51 | 6.36 |
| 75 | 27.89 | 2.63 | 1.37 | 6.18 |
| 80 | 26.56 | 2.51 | 1.25 | 5.99 |
| 85 | 25.37 | 2.40 | 1.14 | 5.79 |
| 90 | 24.29 | 2.29 | 1.03 | 5.58 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
 Qnet = Q - Qallow

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.04 | 0.02 |
| 12 | 0.12 | 0.03 |
| 21 | 0.28 | 0.04 |
| 33 | 0.55 | 0.05 |
| 48 | 0.96 | 0.06 |
| 65 | 1.52 | 0.07 |
| 85 | 2.27 | 0.08 |
| 108 | 3.23 | 0.09 |
| 133 | 4.43 | 0.10 |
| 161 | 5.90 | 0.11 |
| 192 | 7.66 | 0.12 |
| 225 | 9.74 | 0.13 |
| 261 | 12.17 | 0.14 |
| 299 | 14.97 | 0.15 |

| Linear Interpolation | | | | |
|----------------------|------|------|--|-------------------------------|
| 0.12 | H | 0.11 | | H = 0.116 m |
| 7.66 | 6.99 | 5.90 | | Q _{allow} = 1.26 L/s |

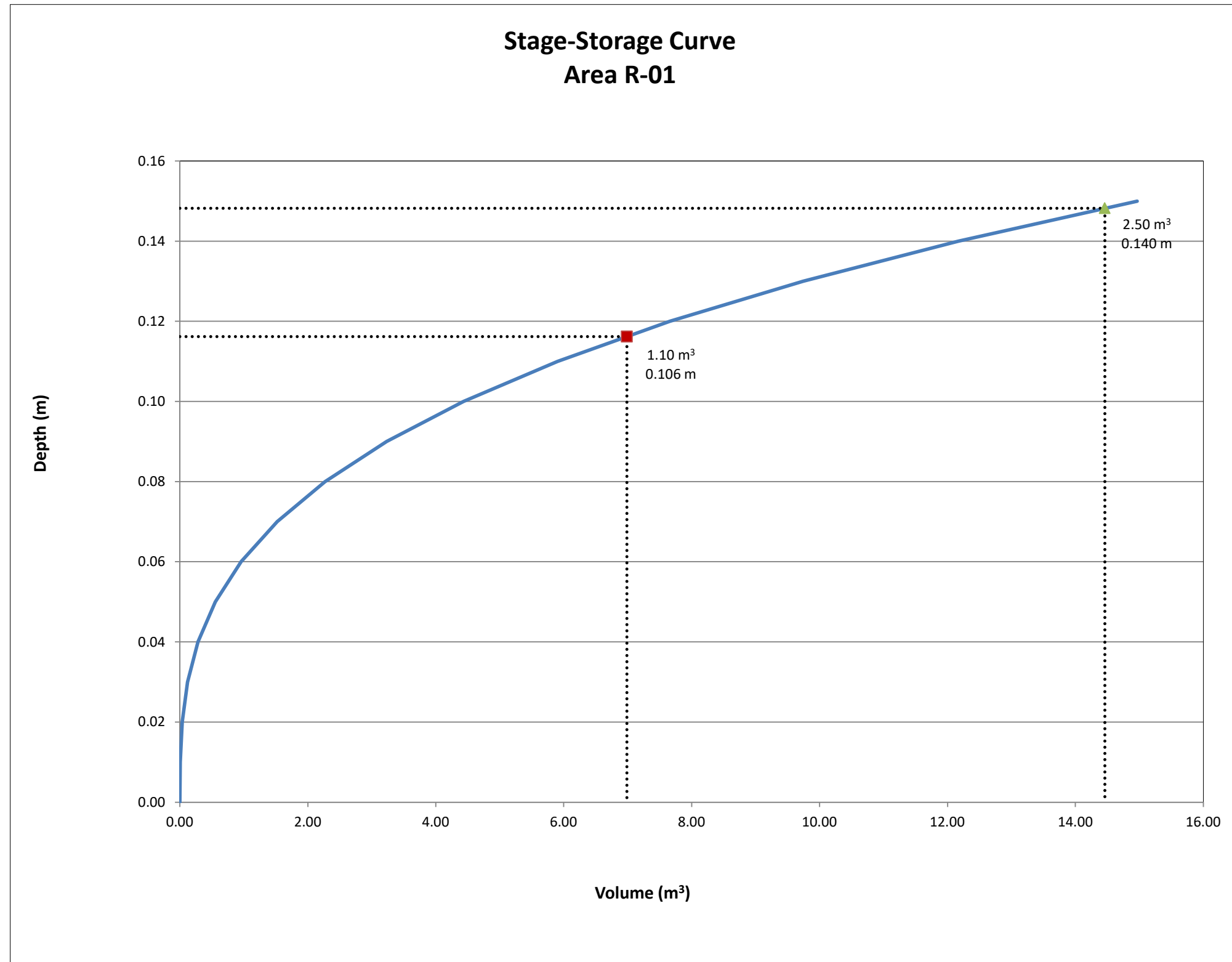
| Area ID R-01 | | | | |
|--------------------|-------------------|------------------------------|------------|-----------------------|
| 100-Year Event | | Static Ponding Area = 299 m2 | | |
| Area = 0.038 ha ha | | Qcontrolled = 1.89 L/s | | |
| C = 1.00 | | Vol(req.) = 14.46 m3 | | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 242.70 | 25.46 | 23.57 | 7.07 |
| 10 | 178.56 | 18.73 | 16.84 | 10.11 |
| 15 | 142.89 | 14.99 | 13.10 | 11.79 |
| 20 | 119.95 | 12.58 | 10.69 | 12.83 |
| 25 | 103.85 | 10.90 | 9.01 | 13.51 |
| 30 | 91.87 | 9.64 | 7.75 | 13.95 |
| 35 | 82.58 | 8.66 | 6.77 | 14.23 |
| 40 | 75.15 | 7.88 | 5.99 | 14.39 |
| 45 | 69.05 | 7.24 | 5.35 | 14.46 |
| 50 | 63.95 | 6.71 | 4.82 | 14.46 |
| 55 | 59.62 | 6.26 | 4.37 | 14.41 |
| 60 | 55.89 | 5.86 | 3.97 | 14.31 |
| 65 | 52.65 | 5.52 | 3.63 | 14.17 |
| 70 | 49.79 | 5.22 | 3.33 | 14.00 |
| 75 | 47.26 | 4.96 | 3.07 | 13.81 |
| 80 | 44.99 | 4.72 | 2.83 | 13.59 |
| 85 | 42.95 | 4.51 | 2.62 | 13.34 |
| 90 | 41.11 | 4.31 | 2.42 | 13.09 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
 Qnet = Q - Qallow

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.04 | 0.02 |
| 12 | 0.12 | 0.03 |
| 21 | 0.28 | 0.04 |
| 33 | 0.55 | 0.05 |
| 48 | 0.96 | 0.06 |
| 65 | 1.52 | 0.07 |
| 85 | 2.27 | 0.08 |
| 108 | 3.23 | 0.09 |
| 133 | 4.43 | 0.10 |
| 161 | 5.90 | 0.11 |
| 192 | 7.66 | 0.12 |
| 225 | 9.74 | 0.13 |
| 261 | 12.17 | 0.14 |
| 299 | 14.97 | 0.15 |

| Linear Interpolation | | | | |
|----------------------|-------|-------|--|-------------------------------|
| 0.15 | H | 0.14 | | H = 0.148 m |
| 14.97 | 14.46 | 12.17 | | Q _{allow} = 1.89 L/s |



1545A Merivale Road
Roof Drain Calculations

| Area ID R-02 | | | | |
|--------------------|-------------------|------------------------------|------------|-----------------------|
| 5-Year Event | | Static Ponding Area = 300 m2 | | |
| Area = 0.043 ha ha | | Qcontrolled = 1.26 L/s | | |
| C = 0.90 | | Vol(req.) = 8.32 m3 | | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 141.18 | 15.09 | 13.83 | 4.15 |
| 10 | 104.19 | 11.14 | 9.88 | 5.93 |
| 15 | 83.56 | 8.93 | 7.67 | 6.90 |
| 20 | 70.25 | 7.51 | 6.25 | 7.50 |
| 25 | 60.90 | 6.51 | 5.25 | 7.87 |
| 30 | 53.93 | 5.76 | 4.50 | 8.11 |
| 35 | 48.52 | 5.19 | 3.93 | 8.24 |
| 40 | 44.18 | 4.72 | 3.46 | 8.31 |
| 45 | 40.63 | 4.34 | 3.08 | 8.32 |
| 50 | 37.65 | 4.02 | 2.76 | 8.29 |
| 55 | 35.12 | 3.75 | 2.49 | 8.23 |
| 60 | 32.94 | 3.52 | 2.26 | 8.14 |
| 65 | 31.04 | 3.32 | 2.06 | 8.03 |
| 70 | 29.37 | 3.14 | 1.88 | 7.89 |
| 75 | 27.89 | 2.98 | 1.72 | 7.74 |
| 80 | 26.56 | 2.84 | 1.58 | 7.58 |
| 85 | 25.37 | 2.71 | 1.45 | 7.40 |
| 90 | 24.29 | 2.60 | 1.34 | 7.21 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
Qnet = Q - Qallow

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.04 | 0.02 |
| 12 | 0.12 | 0.03 |
| 21 | 0.28 | 0.04 |
| 33 | 0.55 | 0.05 |
| 48 | 0.96 | 0.06 |
| 65 | 1.52 | 0.07 |
| 85 | 2.27 | 0.08 |
| 108 | 3.23 | 0.09 |
| 133 | 4.44 | 0.10 |
| 161 | 5.91 | 0.11 |
| 192 | 7.67 | 0.12 |
| 225 | 9.75 | 0.13 |
| 261 | 12.18 | 0.14 |
| 300 | 14.98 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.13 | H | 0.12 | | H = | 0.123 m |
| 9.75 | 8.32 | 7.67 | | Q _{allow} = | 1.26 L/s |

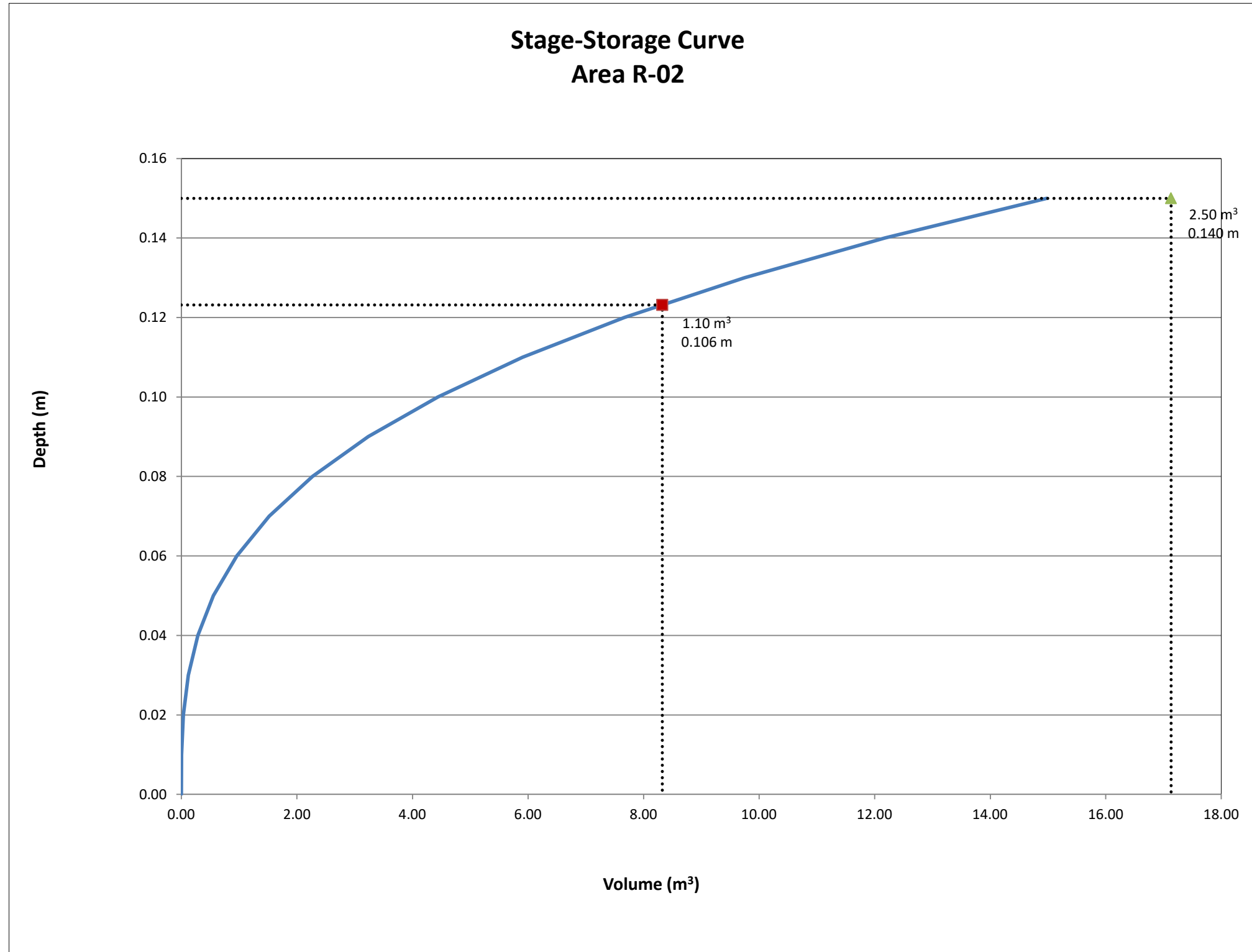
| Area ID R-02 | | | | |
|--------------------|-------------------|------------------------------|------------|-----------------------|
| 100-Year Event | | Static Ponding Area = 300 m2 | | |
| Area = 0.043 ha ha | | Qcontrolled = 1.89 L/s | | |
| C = 1.00 | | Vol(req.) = 17.13 m3 | | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 242.70 | 28.82 | 26.93 | 8.08 |
| 10 | 178.56 | 21.21 | 19.32 | 11.59 |
| 15 | 142.89 | 16.97 | 15.08 | 13.57 |
| 20 | 119.95 | 14.25 | 12.36 | 14.83 |
| 25 | 103.85 | 12.33 | 10.44 | 15.66 |
| 30 | 91.87 | 10.91 | 9.02 | 16.24 |
| 35 | 82.58 | 9.81 | 7.92 | 16.63 |
| 40 | 75.15 | 8.92 | 7.03 | 16.88 |
| 45 | 69.05 | 8.20 | 6.31 | 17.04 |
| 50 | 63.95 | 7.60 | 5.71 | 17.12 |
| 55 | 59.62 | 7.08 | 5.19 | 17.13 |
| 60 | 55.89 | 6.64 | 4.75 | 17.09 |
| 65 | 52.65 | 6.25 | 4.36 | 17.01 |
| 70 | 49.79 | 5.91 | 4.02 | 16.90 |
| 75 | 47.26 | 5.61 | 3.72 | 16.75 |
| 80 | 44.99 | 5.34 | 3.45 | 16.58 |
| 85 | 42.95 | 5.10 | 3.21 | 16.38 |
| 90 | 41.11 | 4.88 | 2.99 | 16.16 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
Qnet = Q - Qallow

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.04 | 0.02 |
| 12 | 0.12 | 0.03 |
| 21 | 0.28 | 0.04 |
| 33 | 0.55 | 0.05 |
| 48 | 0.96 | 0.06 |
| 65 | 1.52 | 0.07 |
| 85 | 2.27 | 0.08 |
| 108 | 3.23 | 0.09 |
| 133 | 4.44 | 0.10 |
| 161 | 5.91 | 0.11 |
| 192 | 7.67 | 0.12 |
| 225 | 9.75 | 0.13 |
| 261 | 12.18 | 0.14 |
| 300 | 14.98 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|-------|-------|--|----------------------|----------|
| 0.16 | H | 0.15 | | H = | 0.150 m |
| 14.98 | 17.13 | 14.98 | | Q _{allow} = | 1.89 L/s |



1545A Merivale Road
Roof Drain Calculations

| Area ID R-03 | | | | |
|-----------------|-------------------|---------|--|-----------------------|
| 5-Year Event | | | | |
| Area = 0.045 ha | | | Static Ponding Area = 339 m ² | |
| C = 0.90 | | | Q _{controlled} = 1.26 L/s | |
| | | | Vol(req.) = 8.94 m ³ | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Q _{net} (L/s) | Vol (m ³) |
| 5 | 141.18 | 15.89 | 14.63 | 4.39 |
| 10 | 104.19 | 11.73 | 10.47 | 6.28 |
| 15 | 83.56 | 9.40 | 8.14 | 7.33 |
| 20 | 70.25 | 7.91 | 6.65 | 7.98 |
| 25 | 60.90 | 6.85 | 5.59 | 8.39 |
| 30 | 53.93 | 6.07 | 4.81 | 8.66 |
| 35 | 48.52 | 5.46 | 4.20 | 8.82 |
| 40 | 44.18 | 4.97 | 3.71 | 8.91 |
| 45 | 40.63 | 4.57 | 3.31 | 8.94 |
| 50 | 37.65 | 4.24 | 2.98 | 8.93 |
| 55 | 35.12 | 3.95 | 2.69 | 8.89 |
| 60 | 32.94 | 3.71 | 2.45 | 8.81 |
| 65 | 31.04 | 3.49 | 2.23 | 8.71 |
| 70 | 29.37 | 3.31 | 2.05 | 8.59 |
| 75 | 27.89 | 3.14 | 1.88 | 8.45 |
| 80 | 26.56 | 2.99 | 1.73 | 8.30 |
| 85 | 25.37 | 2.85 | 1.59 | 8.13 |
| 90 | 24.29 | 2.73 | 1.47 | 7.96 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Q_{net} x time
Q_{net} = Q - Q_{allow}

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 2 | 0.01 | 0.01 |
| 6 | 0.04 | 0.02 |
| 14 | 0.14 | 0.03 |
| 24 | 0.32 | 0.04 |
| 38 | 0.63 | 0.05 |
| 54 | 1.08 | 0.06 |
| 74 | 1.72 | 0.07 |
| 96 | 2.57 | 0.08 |
| 122 | 3.66 | 0.09 |
| 151 | 5.02 | 0.10 |
| 182 | 6.69 | 0.11 |
| 217 | 8.68 | 0.12 |
| 255 | 11.03 | 0.13 |
| 295 | 13.78 | 0.14 |
| 339 | 16.95 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.13 | H | 0.12 | | H = | 0.121 m |
| 11.03 | 8.94 | 8.68 | | Q _{allow} = | 1.26 L/s |

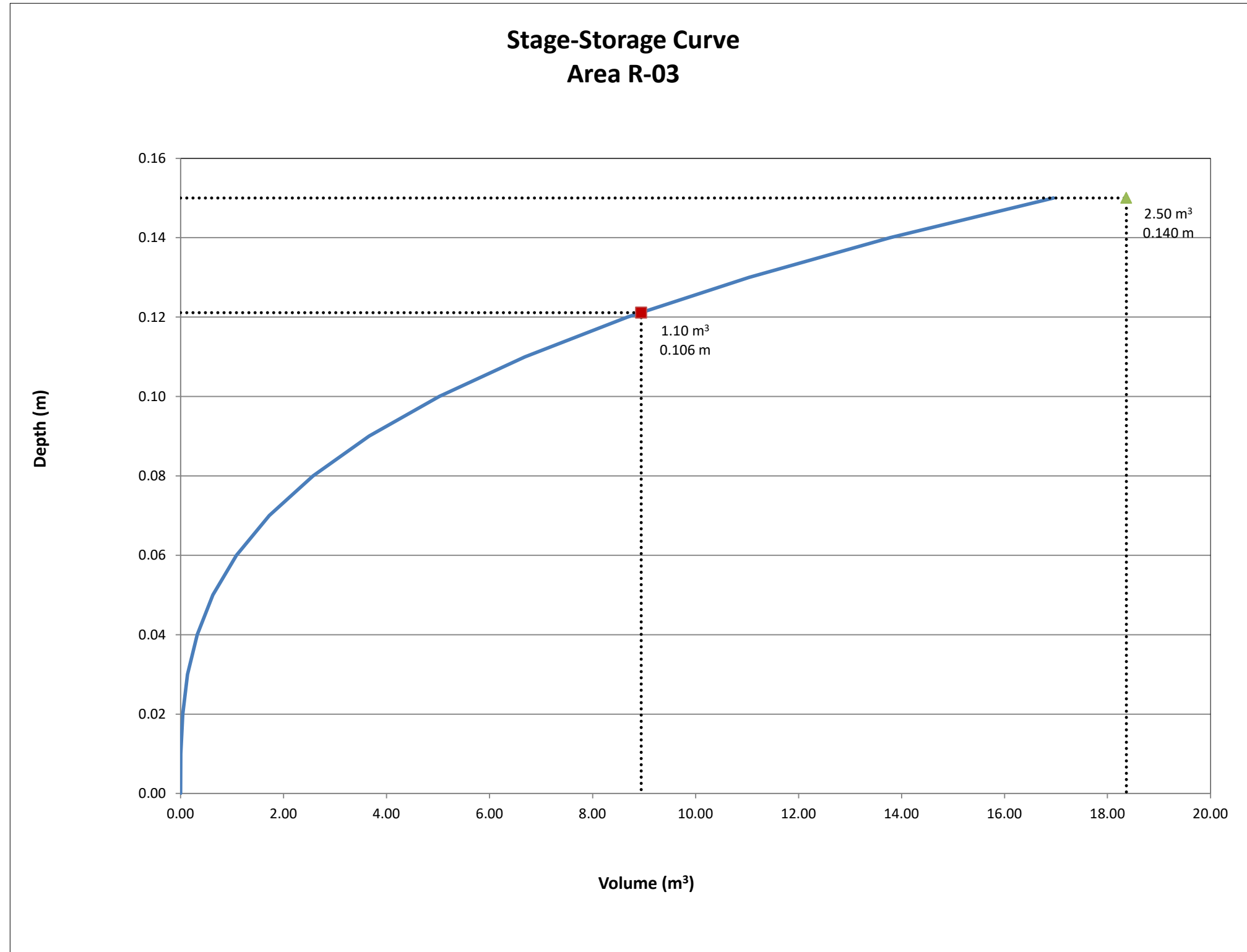
| Area ID R-03 | | | | |
|-----------------|-------------------|---------|--|-----------------------|
| 100-Year Event | | | | |
| Area = 0.045 ha | | | Static Ponding Area = 339 m ² | |
| C = 1.00 | | | Q _{controlled} = 1.89 L/s | |
| | | | Vol(req.) = 18.37 m ³ | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Q _{net} (L/s) | Vol (m ³) |
| 5 | 242.70 | 30.35 | 28.46 | 8.54 |
| 10 | 178.56 | 22.33 | 20.44 | 12.26 |
| 15 | 142.89 | 17.87 | 15.98 | 14.38 |
| 20 | 119.95 | 15.00 | 13.11 | 15.73 |
| 25 | 103.85 | 12.99 | 11.10 | 16.64 |
| 30 | 91.87 | 11.49 | 9.60 | 17.28 |
| 35 | 82.58 | 10.33 | 8.44 | 17.72 |
| 40 | 75.15 | 9.40 | 7.51 | 18.02 |
| 45 | 69.05 | 8.63 | 6.74 | 18.21 |
| 50 | 63.95 | 8.00 | 6.11 | 18.32 |
| 55 | 59.62 | 7.46 | 5.57 | 18.37 |
| 60 | 55.89 | 6.99 | 5.10 | 18.36 |
| 65 | 52.65 | 6.58 | 4.69 | 18.30 |
| 70 | 49.79 | 6.23 | 4.34 | 18.21 |
| 75 | 47.26 | 5.91 | 4.02 | 18.09 |
| 80 | 44.99 | 5.63 | 3.74 | 17.93 |
| 85 | 42.95 | 5.37 | 3.48 | 17.75 |
| 90 | 41.11 | 5.14 | 3.25 | 17.55 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Q_{net} x time
Q_{net} = Q - Q_{allow}

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 2 | 0.01 | 0.01 |
| 6 | 0.04 | 0.02 |
| 14 | 0.14 | 0.03 |
| 24 | 0.32 | 0.04 |
| 38 | 0.63 | 0.05 |
| 54 | 1.08 | 0.06 |
| 74 | 1.72 | 0.07 |
| 96 | 2.57 | 0.08 |
| 122 | 3.66 | 0.09 |
| 151 | 5.02 | 0.10 |
| 182 | 6.69 | 0.11 |
| 217 | 8.68 | 0.12 |
| 255 | 11.03 | 0.13 |
| 295 | 13.78 | 0.14 |
| 339 | 16.95 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|-------|-------|--|----------------------|----------|
| 0.16 | H | 0.15 | | H = | 0.150 m |
| 16.95 | 18.37 | 16.95 | | Q _{allow} = | 1.89 L/s |



**1545A Merivale Road
Roof Drain Calculations**

| Area ID R-04 | | | | |
|--|-------------------|---------|------------------------------------|-----------------------|
| 5-Year Event | | | | |
| Static Ponding Area = 305 m ² | | | | |
| Area = 0.045 ha | | ha | Q _{controlled} = 1.26 L/s | |
| C = 0.90 | | | Vol(req.) = 9.03 m ³ | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Q _{net} (L/s) | Vol (m ³) |
| 5 | 141.18 | 16.00 | 14.74 | 4.42 |
| 10 | 104.19 | 11.81 | 10.55 | 6.33 |
| 15 | 83.56 | 9.47 | 8.21 | 7.39 |
| 20 | 70.25 | 7.96 | 6.70 | 8.04 |
| 25 | 60.90 | 6.90 | 5.64 | 8.46 |
| 30 | 53.93 | 6.11 | 4.85 | 8.73 |
| 35 | 48.52 | 5.50 | 4.24 | 8.90 |
| 40 | 44.18 | 5.01 | 3.75 | 8.99 |
| 45 | 40.63 | 4.60 | 3.34 | 9.03 |
| 50 | 37.65 | 4.27 | 3.01 | 9.02 |
| 55 | 35.12 | 3.98 | 2.72 | 8.98 |
| 60 | 32.94 | 3.73 | 2.47 | 8.90 |
| 65 | 31.04 | 3.52 | 2.26 | 8.81 |
| 70 | 29.37 | 3.33 | 2.07 | 8.69 |
| 75 | 27.89 | 3.16 | 1.90 | 8.55 |
| 80 | 26.56 | 3.01 | 1.75 | 8.40 |
| 85 | 25.37 | 2.87 | 1.61 | 8.23 |
| 90 | 24.29 | 2.75 | 1.49 | 8.06 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Q_{net} x time
Q_{net} = Q - Q_{allow}

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.04 | 0.02 |
| 12 | 0.12 | 0.03 |
| 22 | 0.29 | 0.04 |
| 34 | 0.56 | 0.05 |
| 49 | 0.97 | 0.06 |
| 66 | 1.55 | 0.07 |
| 87 | 2.31 | 0.08 |
| 110 | 3.29 | 0.09 |
| 135 | 4.51 | 0.10 |
| 164 | 6.01 | 0.11 |
| 195 | 7.80 | 0.12 |
| 229 | 9.92 | 0.13 |
| 265 | 12.38 | 0.14 |
| 305 | 15.23 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.13 | H | 0.12 | | H = | 0.126 m |
| 9.92 | 9.03 | 7.80 | | Q _{allow} = | 1.26 L/s |

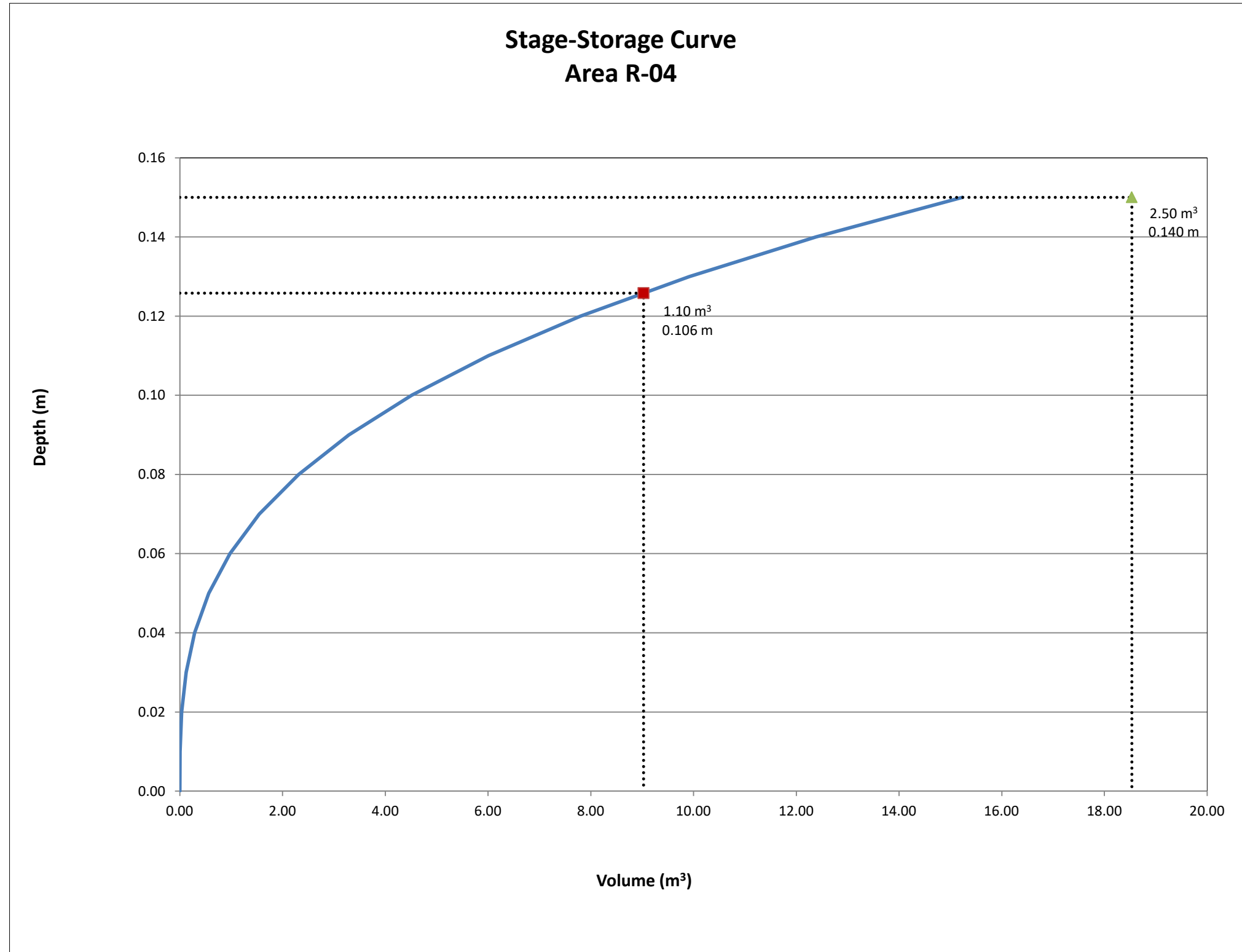
| Area ID R-04 | | | | |
|--|-------------------|---------|------------------------------------|-----------------------|
| 100-Year Event | | | | |
| Static Ponding Area = 305 m ² | | | | |
| Area = 0.045 ha | | ha | Q _{controlled} = 1.89 L/s | |
| C = 1.00 | | | Vol(req.) = 18.54 m ³ | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Q _{net} (L/s) | Vol (m ³) |
| 5 | 242.70 | 30.56 | 28.67 | 8.60 |
| 10 | 178.56 | 22.48 | 20.59 | 12.36 |
| 15 | 142.89 | 17.99 | 16.10 | 14.49 |
| 20 | 119.95 | 15.10 | 13.21 | 15.86 |
| 25 | 103.85 | 13.07 | 11.18 | 16.78 |
| 30 | 91.87 | 11.57 | 9.68 | 17.42 |
| 35 | 82.58 | 10.40 | 8.51 | 17.87 |
| 40 | 75.15 | 9.46 | 7.57 | 18.17 |
| 45 | 69.05 | 8.69 | 6.80 | 18.37 |
| 50 | 63.95 | 8.05 | 6.16 | 18.49 |
| 55 | 59.62 | 7.51 | 5.62 | 18.54 |
| 60 | 55.89 | 7.04 | 5.15 | 18.53 |
| 65 | 52.65 | 6.63 | 4.74 | 18.48 |
| 70 | 49.79 | 6.27 | 4.38 | 18.39 |
| 75 | 47.26 | 5.95 | 4.06 | 18.27 |
| 80 | 44.99 | 5.66 | 3.77 | 18.12 |
| 85 | 42.95 | 5.41 | 3.52 | 17.94 |
| 90 | 41.11 | 5.18 | 3.29 | 17.75 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Q_{net} x time
Q_{net} = Q - Q_{allow}

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.04 | 0.02 |
| 12 | 0.12 | 0.03 |
| 22 | 0.29 | 0.04 |
| 34 | 0.56 | 0.05 |
| 49 | 0.97 | 0.06 |
| 66 | 1.55 | 0.07 |
| 87 | 2.31 | 0.08 |
| 110 | 3.29 | 0.09 |
| 135 | 4.51 | 0.10 |
| 164 | 6.01 | 0.11 |
| 195 | 7.80 | 0.12 |
| 229 | 9.92 | 0.13 |
| 265 | 12.38 | 0.14 |
| 305 | 15.23 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|-------|-------|--|----------------------|----------|
| 0.16 | H | 0.15 | | H = | 0.150 m |
| 15.23 | 18.54 | 15.23 | | Q _{allow} = | 1.89 L/s |



**1545A Merivale Road
Roof Drain Calculations**

| Area ID R-05 | | | | |
|-----------------|-------------------|--|------------------------|-----------------------|
| 5-Year Event | | Static Ponding Area = 271 m ² | | |
| Area = 0.037 ha | | Q _{controlled} = 1.26 L/s | | |
| C = 0.90 | | Vol(req.) = 6.82 m ³ | | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Q _{net} (L/s) | Vol (m ³) |
| 5 | 141.18 | 13.10 | 11.84 | 3.55 |
| 10 | 104.19 | 9.67 | 8.41 | 5.05 |
| 15 | 83.56 | 7.75 | 6.49 | 5.84 |
| 20 | 70.25 | 6.52 | 5.26 | 6.31 |
| 25 | 60.90 | 5.65 | 4.39 | 6.59 |
| 30 | 53.93 | 5.00 | 3.74 | 6.74 |
| 35 | 48.52 | 4.50 | 3.24 | 6.81 |
| 40 | 44.18 | 4.10 | 2.84 | 6.82 |
| 45 | 40.63 | 3.77 | 2.51 | 6.78 |
| 50 | 37.65 | 3.49 | 2.23 | 6.70 |
| 55 | 35.12 | 3.26 | 2.00 | 6.60 |
| 60 | 32.94 | 3.06 | 1.80 | 6.47 |
| 65 | 31.04 | 2.88 | 1.62 | 6.32 |
| 70 | 29.37 | 2.73 | 1.47 | 6.16 |
| 75 | 27.89 | 2.59 | 1.33 | 5.98 |
| 80 | 26.56 | 2.46 | 1.20 | 5.78 |
| 85 | 25.37 | 2.35 | 1.09 | 5.58 |
| 90 | 24.29 | 2.25 | 0.99 | 5.37 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Q_{net} x time
Q_{net} = Q - Q_{allow}

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.03 | 0.02 |
| 11 | 0.11 | 0.03 |
| 19 | 0.26 | 0.04 |
| 30 | 0.50 | 0.05 |
| 43 | 0.87 | 0.06 |
| 59 | 1.38 | 0.07 |
| 77 | 2.05 | 0.08 |
| 97 | 2.92 | 0.09 |
| 120 | 4.01 | 0.10 |
| 146 | 5.34 | 0.11 |
| 173 | 6.93 | 0.12 |
| 203 | 8.81 | 0.13 |
| 236 | 11.01 | 0.14 |
| 271 | 13.54 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.12 | H | 0.11 | | H = | 0.119 m |
| 6.93 | 6.82 | 5.34 | | Q _{allow} = | 1.26 L/s |

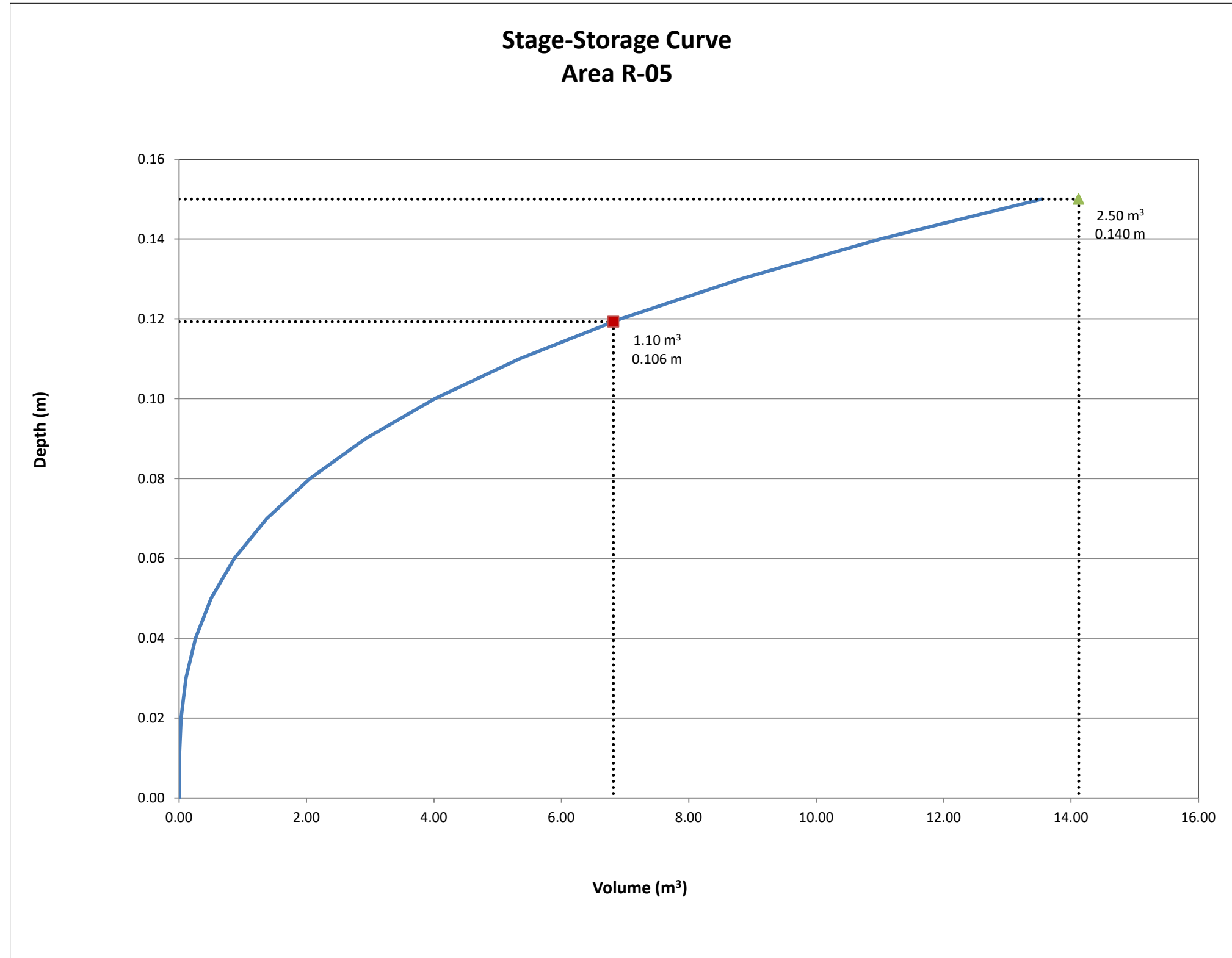
| Area ID R-05 | | | | |
|-----------------|-------------------|--|------------------------|-----------------------|
| 100-Year Event | | Static Ponding Area = 271 m ² | | |
| Area = 0.037 ha | | Q _{controlled} = 1.89 L/s | | |
| C = 1.00 | | Vol(req.) = 14.12 m ³ | | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Q _{net} (L/s) | Vol (m ³) |
| 5 | 242.70 | 25.03 | 23.14 | 6.94 |
| 10 | 178.56 | 18.41 | 16.52 | 9.91 |
| 15 | 142.89 | 14.73 | 12.84 | 11.56 |
| 20 | 119.95 | 12.37 | 10.48 | 12.57 |
| 25 | 103.85 | 10.71 | 8.82 | 13.23 |
| 30 | 91.87 | 9.47 | 7.58 | 13.65 |
| 35 | 82.58 | 8.51 | 6.62 | 13.91 |
| 40 | 75.15 | 7.75 | 5.86 | 14.06 |
| 45 | 69.05 | 7.12 | 5.23 | 14.12 |
| 50 | 63.95 | 6.59 | 4.70 | 14.11 |
| 55 | 59.62 | 6.15 | 4.26 | 14.05 |
| 60 | 55.89 | 5.76 | 3.87 | 13.94 |
| 65 | 52.65 | 5.43 | 3.54 | 13.80 |
| 70 | 49.79 | 5.13 | 3.24 | 13.62 |
| 75 | 47.26 | 4.87 | 2.98 | 13.42 |
| 80 | 44.99 | 4.64 | 2.75 | 13.20 |
| 85 | 42.95 | 4.43 | 2.54 | 12.95 |
| 90 | 41.11 | 4.24 | 2.35 | 12.68 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Q_{net} x time
Q_{net} = Q - Q_{allow}

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 5 | 0.03 | 0.02 |
| 11 | 0.11 | 0.03 |
| 19 | 0.26 | 0.04 |
| 30 | 0.50 | 0.05 |
| 43 | 0.87 | 0.06 |
| 59 | 1.38 | 0.07 |
| 77 | 2.05 | 0.08 |
| 97 | 2.92 | 0.09 |
| 120 | 4.01 | 0.10 |
| 146 | 5.34 | 0.11 |
| 173 | 6.93 | 0.12 |
| 203 | 8.81 | 0.13 |
| 236 | 11.01 | 0.14 |
| 271 | 13.54 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|-------|-------|--|----------------------|----------|
| 0.16 | H | 0.15 | | H = | 0.150 m |
| 13.54 | 14.12 | 13.54 | | Q _{allow} = | 1.89 L/s |



1545A Merivale Road
Roof Drain Calculations

| Area ID R-06 | | | | |
|-----------------|-------------------|---------|------------------------------|-----------------------|
| 5-Year Event | | | | |
| Area = 0.023 ha | | | Static Ponding Area = 205 m2 | |
| C = 0.90 | | | Qcontrolled = 1.26 L/s | |
| | | | Vol(req.) = 3.31 m3 | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 141.18 | 8.04 | 6.78 | 2.03 |
| 10 | 104.19 | 5.94 | 4.68 | 2.81 |
| 15 | 83.56 | 4.76 | 3.50 | 3.15 |
| 20 | 70.25 | 4.00 | 2.74 | 3.29 |
| 25 | 60.90 | 3.47 | 2.21 | 3.31 |
| 30 | 53.93 | 3.07 | 1.81 | 3.26 |
| 35 | 48.52 | 2.76 | 1.50 | 3.16 |
| 40 | 44.18 | 2.52 | 1.26 | 3.02 |
| 45 | 40.63 | 2.31 | 1.05 | 2.85 |
| 50 | 37.65 | 2.15 | 0.89 | 2.66 |
| 55 | 35.12 | 2.00 | 0.74 | 2.45 |
| 60 | 32.94 | 1.88 | 0.62 | 2.22 |
| 65 | 31.04 | 1.77 | 0.51 | 1.98 |
| 70 | 29.37 | 1.67 | 0.41 | 1.74 |
| 75 | 27.89 | 1.59 | 0.33 | 1.48 |
| 80 | 26.56 | 1.51 | 0.25 | 1.22 |
| 85 | 25.37 | 1.45 | 0.19 | 0.94 |
| 90 | 24.29 | 1.38 | 0.12 | 0.67 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
Qnet = Q - Qallow

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 4 | 0.02 | 0.02 |
| 8 | 0.08 | 0.03 |
| 15 | 0.19 | 0.04 |
| 23 | 0.38 | 0.05 |
| 33 | 0.66 | 0.06 |
| 45 | 1.04 | 0.07 |
| 58 | 1.55 | 0.08 |
| 74 | 2.21 | 0.09 |
| 91 | 3.04 | 0.10 |
| 110 | 4.04 | 0.11 |
| 131 | 5.25 | 0.12 |
| 154 | 6.67 | 0.13 |
| 179 | 8.33 | 0.14 |
| 205 | 10.25 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.11 | H | 0.10 | | H = | 0.103 m |
| 4.04 | 3.31 | 3.04 | | Q _{allow} = | 1.26 L/s |

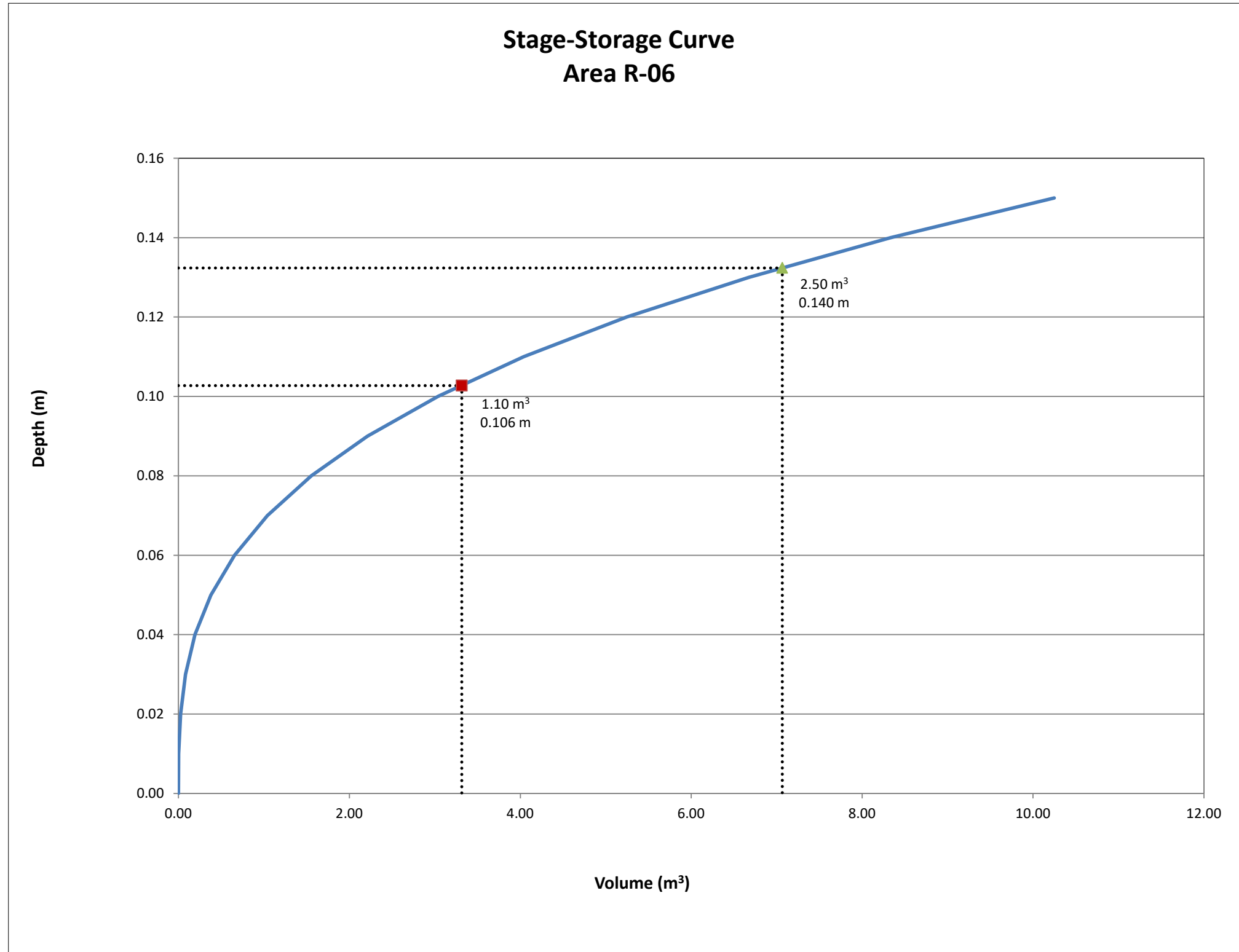
| Area ID R-06 | | | | |
|-----------------|-------------------|---------|------------------------------|-----------------------|
| 100-Year Event | | | | |
| Area = 0.023 ha | | | Static Ponding Area = 205 m2 | |
| C = 1.00 | | | Qcontrolled = 1.89 L/s | |
| | | | Vol(req.) = 7.07 m3 | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 242.70 | 15.36 | 13.47 | 4.04 |
| 10 | 178.56 | 11.30 | 9.41 | 5.65 |
| 15 | 142.89 | 9.05 | 7.16 | 6.44 |
| 20 | 119.95 | 7.59 | 5.70 | 6.84 |
| 25 | 103.85 | 6.57 | 4.68 | 7.03 |
| 30 | 91.87 | 5.82 | 3.93 | 7.07 |
| 35 | 82.58 | 5.23 | 3.34 | 7.01 |
| 40 | 75.15 | 4.76 | 2.87 | 6.88 |
| 45 | 69.05 | 4.37 | 2.48 | 6.70 |
| 50 | 63.95 | 4.05 | 2.16 | 6.48 |
| 55 | 59.62 | 3.77 | 1.88 | 6.22 |
| 60 | 55.89 | 3.54 | 1.65 | 5.93 |
| 65 | 52.65 | 3.33 | 1.44 | 5.63 |
| 70 | 49.79 | 3.15 | 1.26 | 5.30 |
| 75 | 47.26 | 2.99 | 1.10 | 4.96 |
| 80 | 44.99 | 2.85 | 0.96 | 4.60 |
| 85 | 42.95 | 2.72 | 0.83 | 4.23 |
| 90 | 41.11 | 2.60 | 0.71 | 3.85 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
Qnet = Q - Qallow

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 4 | 0.02 | 0.02 |
| 8 | 0.08 | 0.03 |
| 15 | 0.19 | 0.04 |
| 23 | 0.38 | 0.05 |
| 33 | 0.66 | 0.06 |
| 45 | 1.04 | 0.07 |
| 58 | 1.55 | 0.08 |
| 74 | 2.21 | 0.09 |
| 91 | 3.04 | 0.10 |
| 110 | 4.04 | 0.11 |
| 131 | 5.25 | 0.12 |
| 154 | 6.67 | 0.13 |
| 179 | 8.33 | 0.14 |
| 205 | 10.25 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.14 | H | 0.13 | | H = | 0.132 m |
| 8.33 | 7.07 | 6.67 | | Q _{allow} = | 1.89 L/s |



**1545A Merivale Road
Roof Drain Calculations**

| Area ID R-07 | | | | |
|--------------------|-------------------|---------|------------------------------|-----------------------|
| 5-Year Event | | | | |
| Area = 0.023 ha ha | | | Static Ponding Area = 205 m2 | |
| C = 0.90 | | | Qcontrolled = 1.26 L/s | |
| | | | Vol(req.) = 3.31 m3 | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 141.18 | 8.04 | 6.78 | 2.03 |
| 10 | 104.19 | 5.93 | 4.67 | 2.80 |
| 15 | 83.56 | 4.76 | 3.50 | 3.15 |
| 20 | 70.25 | 4.00 | 2.74 | 3.29 |
| 25 | 60.90 | 3.47 | 2.21 | 3.31 |
| 30 | 53.93 | 3.07 | 1.81 | 3.26 |
| 35 | 48.52 | 2.76 | 1.50 | 3.16 |
| 40 | 44.18 | 2.52 | 1.26 | 3.01 |
| 45 | 40.63 | 2.31 | 1.05 | 2.84 |
| 50 | 37.65 | 2.14 | 0.88 | 2.65 |
| 55 | 35.12 | 2.00 | 0.74 | 2.44 |
| 60 | 32.94 | 1.88 | 0.62 | 2.22 |
| 65 | 31.04 | 1.77 | 0.51 | 1.98 |
| 70 | 29.37 | 1.67 | 0.41 | 1.73 |
| 75 | 27.89 | 1.59 | 0.33 | 1.48 |
| 80 | 26.56 | 1.51 | 0.25 | 1.21 |
| 85 | 25.37 | 1.44 | 0.18 | 0.94 |
| 90 | 24.29 | 1.38 | 0.12 | 0.66 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
Qnet = Q - Qallow

| Ponding Depth (5-Year Storm) | | |
|------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 4 | 0.02 | 0.02 |
| 8 | 0.08 | 0.03 |
| 15 | 0.19 | 0.04 |
| 23 | 0.38 | 0.05 |
| 33 | 0.66 | 0.06 |
| 45 | 1.04 | 0.07 |
| 58 | 1.55 | 0.08 |
| 74 | 2.21 | 0.09 |
| 91 | 3.03 | 0.10 |
| 110 | 4.04 | 0.11 |
| 131 | 5.24 | 0.12 |
| 154 | 6.67 | 0.13 |
| 178 | 8.33 | 0.14 |
| 205 | 10.24 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.11 | H | 0.10 | | H = | 0.103 m |
| 4.04 | 3.31 | 3.03 | | Q _{allow} = | 1.26 L/s |

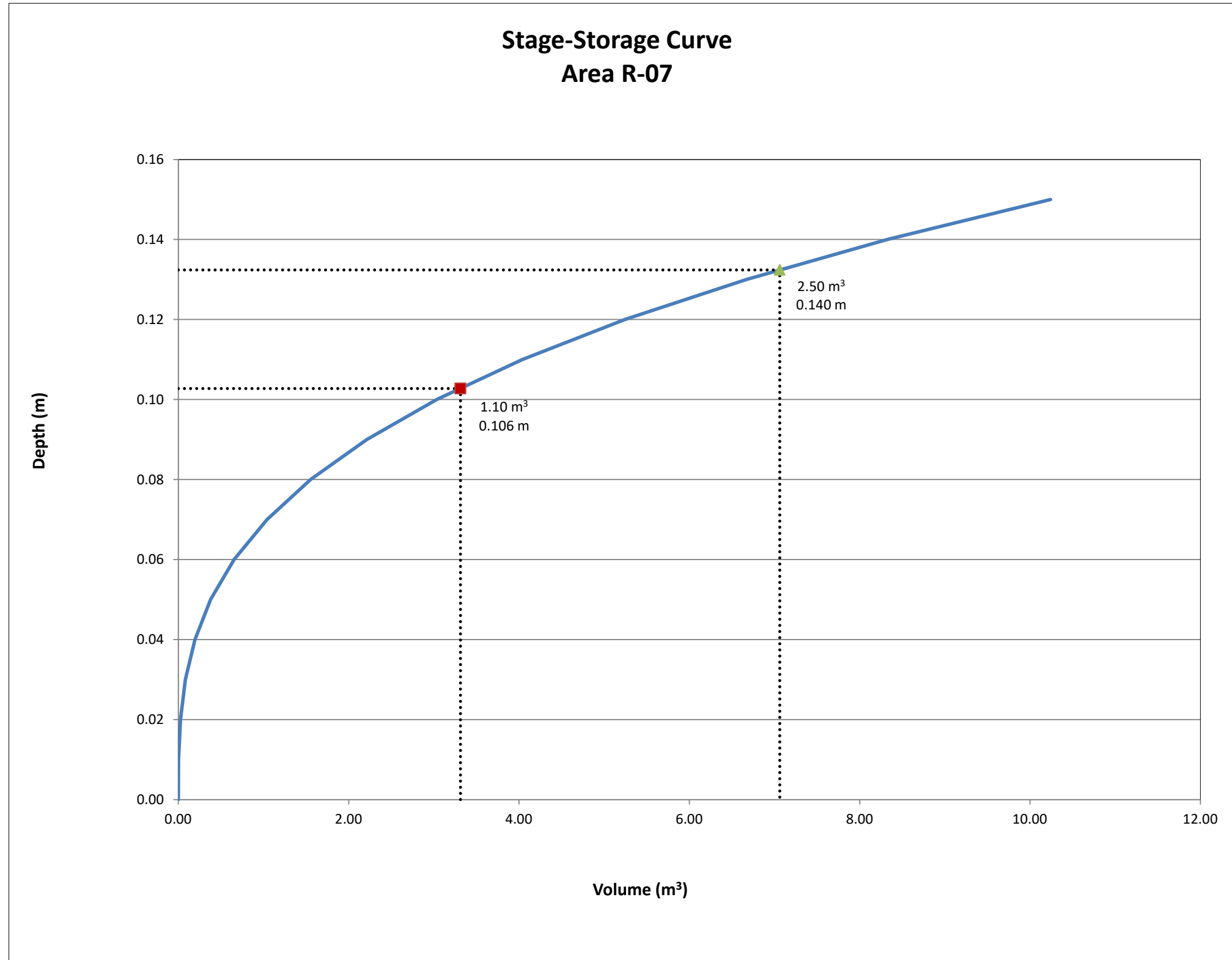
| Area ID R-07 | | | | |
|--------------------|-------------------|---------|------------------------------|-----------------------|
| 100-Year Event | | | | |
| Area = 0.023 ha ha | | | Static Ponding Area = 205 m2 | |
| C = 1.00 | | | Qcontrolled = 1.89 L/s | |
| | | | Vol(req.) = 7.06 m3 | |
| Time (min) | Intensity (mm/hr) | Q (L/s) | Qnet (L/s) | Vol (m ³) |
| 5 | 242.70 | 15.36 | 13.47 | 4.04 |
| 10 | 178.56 | 11.30 | 9.41 | 5.64 |
| 15 | 142.89 | 9.04 | 7.15 | 6.44 |
| 20 | 119.95 | 7.59 | 5.70 | 6.84 |
| 25 | 103.85 | 6.57 | 4.68 | 7.02 |
| 30 | 91.87 | 5.81 | 3.92 | 7.06 |
| 35 | 82.58 | 5.22 | 3.33 | 7.00 |
| 40 | 75.15 | 4.75 | 2.86 | 6.88 |
| 45 | 69.05 | 4.37 | 2.48 | 6.69 |
| 50 | 63.95 | 4.05 | 2.16 | 6.47 |
| 55 | 59.62 | 3.77 | 1.88 | 6.21 |
| 60 | 55.89 | 3.54 | 1.65 | 5.93 |
| 65 | 52.65 | 3.33 | 1.44 | 5.62 |
| 70 | 49.79 | 3.15 | 1.26 | 5.29 |
| 75 | 47.26 | 2.99 | 1.10 | 4.95 |
| 80 | 44.99 | 2.85 | 0.96 | 4.59 |
| 85 | 42.95 | 2.72 | 0.83 | 4.22 |
| 90 | 41.11 | 2.60 | 0.71 | 3.84 |

*City of Ottawa IDF Data (Sewer Design Guidelines, Oct. 2012)

Notes: Vol = Qnet x time
Qnet = Q - Qallow

| Ponding Depth (100-Year Storm) | | |
|--------------------------------|------------------|-------------|
| Area m ² | V m ³ | H m |
| 0 | 0.00 | 0.00 |
| 1 | 0.00 | 0.01 |
| 4 | 0.02 | 0.02 |
| 8 | 0.08 | 0.03 |
| 15 | 0.19 | 0.04 |
| 23 | 0.38 | 0.05 |
| 33 | 0.66 | 0.06 |
| 45 | 1.04 | 0.07 |
| 58 | 1.55 | 0.08 |
| 74 | 2.21 | 0.09 |
| 91 | 3.03 | 0.10 |
| 110 | 4.04 | 0.11 |
| 131 | 5.24 | 0.12 |
| 154 | 6.67 | 0.13 |
| 178 | 8.33 | 0.14 |
| 205 | 10.24 | 0.15 |

| Linear Interpolation | | | | | |
|----------------------|------|------|--|----------------------|----------|
| 0.14 | H | 0.13 | | H = | 0.132 m |
| 8.33 | 7.06 | 6.67 | | Q _{allow} = | 1.89 L/s |



APPENDIX E

Water Boundary Conditions & Hydraulic Calculations

From: Candow, Julie <julie.candow@ottawa.ca>
Sent: Wednesday, October 26, 2022 10:16 AM
To: Trevor McKay
Cc: Melanie Riddell; Samantha Bennett
Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request
Attachments: [1545A Merivale Road October 2022.pdf](#)

Good morning all,

The following are boundary conditions, HGL, for hydraulic analysis at 1545A Merivale Road (zone 2W2C) assumed to be connected to the 305 mm on Merivale Road (see attached PDF for location).

Minimum HGL: 125.4 m

Maximum HGL: 134.1 m

Max Day + FF (183 L/s): 123.0 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Julie Candow, P.Eng

Project Manager

Planning, Real Estate and Economic Development Department - West Branch

City of Ottawa

110 Laurier Avenue West Ottawa, ON

613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Trevor McKay <t.mckay@novatech-eng.com>

Sent: October 04, 2022 10:09 AM

To: Candow, Julie <julie.candow@ottawa.ca>

Cc: Melanie Riddell <m.riddell@novatech-eng.com>; Samantha Bennett <s.bennett@novatech-

[eng.com](#)>

Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Julie,

Revised as requested.

Thanks,

Trevor McKay, B.Eng., E.I.T., Project Coordinator | Engineering/Contract Administration

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 291 | Cell: 613.263.9113 | Fax: 613.254.5867

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Candow, Julie <julie.candow@ottawa.ca>

Sent: Tuesday, October 4, 2022 10:04 AM

To: Trevor McKay <t.mckay@novatech-eng.com>

Cc: Melanie Riddell <m.riddell@novatech-eng.com>; Samantha Bennett <s.bennett@novatech-eng.com>

Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

Hi Trevor,

Apologies for the back and forth, I received the below from AM:

"They need to resubmit using 2020 FUS & also referencing the 2020 FUS guidelines. Their submissions states 1999 FUS. Please ask them for a re-submission."

Julie Candow, P.Eng

Project Manager

Planning, Real Estate and Economic Development Department - West Branch

City of Ottawa

110 Laurier Avenue West Ottawa, ON

613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Trevor McKay <t.mckay@novatech-eng.com>

Sent: October 04, 2022 9:00 AM

To: Candow, Julie <julie.candow@ottawa.ca>

Cc: Melanie Riddell <m.riddell@novatech-eng.com>; Samantha Bennett <s.bennett@novatech-eng.com>

Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

You are correct the calculations do not change between the 1999 FUS and the 2020 FUS. We can confirm that the fire flow boundary conditions requested are consistent with the 2020 FUS method.

Do you require a resubmission, or is this sufficient to move forward?

Thanks,

Trevor McKay, B.Eng., E.I.T., Project Coordinator | Engineering/Contract Administration

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 291 | Cell: 613.263.9113 | Fax: 613.254.5867

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Candow, Julie <julie.candow@ottawa.ca>

Sent: Monday, October 3, 2022 1:45 PM

To: Trevor McKay <t.mckay@novatech-eng.com>

Cc: Melanie Riddell <m.riddell@novatech-eng.com>; Samantha Bennett <s.bennett@novatech-eng.com>

Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

Hi Trevor,

Asset Management noted that in your 'notes' the 1999 Fire Underwriter's Survey Guidelines are referenced. As you may be aware the 2020 Fire Underwriter's Survey Guidelines were released and should be used. I don't believe there are any changes to your calculations but can you please confirm that the 2020 FUS Guidelines were referenced.

Thanks,

Julie Candow, P.Eng

Project Manager

Planning, Real Estate and Economic Development Department - West Branch

City of Ottawa

110 Laurier Avenue West Ottawa, ON

613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Trevor McKay <t.mckay@novatech-eng.com>
Sent: September 29, 2022 2:04 PM
To: Candow, Julie <julie.candow@ottawa.ca>
Cc: Melanie Riddell <m.riddell@novatech-eng.com>; Samantha Bennett <s.bennett@novatech-eng.com>
Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

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Good catch. Please see revised boundary conditions request.

We'll reach out to Doug for 50 Capilano. Appreciate your help.

Thank you,

Trevor McKay, B.Eng., E.I.T., Project Coordinator | Engineering/Contract Administration

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 291 | Cell: 613.263.9113 | Fax: 613.254.5867

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From: Candow, Julie <julie.candow@ottawa.ca>
Sent: Thursday, September 29, 2022 1:48 PM
To: Trevor McKay <t.mckay@novatech-eng.com>
Cc: Melanie Riddell <m.riddell@novatech-eng.com>; colette.groni@ottawa.ca; Samantha Bennett <s.bennett@novatech-eng.com>
Subject: RE: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

Hi Trevor,

For the boundary condition request, the water demands for the commercial site area should be calculated using 28,000 L/gross ha/day. The entire hectarage of the site shall be used in the calculation, versus the floor area of the commercial building. Alternatively, Appendix 4-A of the Sewer Design Guidelines can be used to estimate the Daily Volume in Liters based on the use of the commercial space. Please update the calculations accordingly.

For 50 Capilano Drive, I was able to locate the design drawings (missing the SWM plan), the Geotech Report and the Phase I ESA but for some reason the Site Servicing Report and SWM Report is missing. I

can see from the drawings that the site plan application was done by D.B. Gray Engineering (Douglas Gray d.gray@dbgrayengineering.com). I suggest you reach out to Doug to determine if he has a copy of the report.

If you need a copy of the Site Plan, Grading Plan and Servicing Plan for 50 Capilano I can pull those from records and provide them to you.

Hope that helps,

Julie Candow, P.Eng

Project Manager

Planning, Real Estate and Economic Development Department - West Branch

City of Ottawa

110 Laurier Avenue West Ottawa, ON

613.580.2424 ext. 13850

Please take note that due to the current COVID situation, I am working remotely and phone communication may not be reliable at this time. The best way to reach me is by email.

From: Trevor McKay <t.mckay@novatech-eng.com>

Sent: September 29, 2022 8:09 AM

To: Candow, Julie <julie.candow@ottawa.ca>

Cc: Melanie Riddell <m.riddell@novatech-eng.com>; colette.groni@ottawa.ca; Samantha Bennett <s.bennett@novatech-eng.com>

Subject: 1545A Merivale Road Site Plan Application - Water Boundary Condition Request

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We have been retained to provide the site servicing and stormwater management design services for the pending site plan application for 1545A Merivale Road. There was a pre-application consultation meeting held in June (meeting notes attached for reference – PC2022-0137).

We would like to request watermain boundary conditions for the project. Please see attached boundary condition request documents (20220928-122098-BC-Request). If there is any additional information required to accommodate this request, please let us know.

Secondly, we were hopeful that you may be able to provide a stormwater management plan/report for 50 Capilano Drive (estimated to be circa 2010-2014). We had included this information as part of our records request from the City Geoinformation centre, however their search didn't provide any information. Based on a preliminary review of the available site information/survey, it appears that a significant portion of the current storm drainage from the 1545A Merivale Road property is directed to

several catchbasins on the adjacent City View curling club property (50 Capilano Drive). It appears that the curling rink was built in +/-2014. Any information you may be able to provide would be helpful.

If you have any questions, please don't hesitate to reach out. We appreciate your time in processing these requests.

Trevor McKay, B.Eng., E.I.T., Project Coordinator | Engineering/Contract Administration

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 291 | Cell: 613.263.9113 | Fax: 613.254.5867

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Boundary Conditions for 1545A Merivale Road



Legend

- PRIVATE
- PUBLIC

1545A MERIVALE RD POST-DEVELOPMENT WATER DEMANDS

DOMESTIC WATER DEMANDS

| Commercial Space / Amenity Use | |
|---|----------------------|
| Commercial Space | 6,900 m ² |
| Average Day Demand (28,000 L/ha/day) | 0.22 L/s |
| Maximum Day Demand (1.5 x avg. day) | 0.34 L/s |
| Peak Hour Demand (1.8 x max. day) | 0.60 L/s |
| TOTALS | |
| Average Day Demand | 0.22 L/s |
| Maximum Day Demand | 0.34 L/s |
| Peak Hour Demand | 0.60 L/s |

Required Fire Flow (as per FUS)

183L/s

Notes:

- 1) Water demand based on City of Ottawa Design Guidelines - Water Distribution (2010, as ammended)
- 2) Fireflows calculated as per 2020 Fire Underwriter's Survey Guidelines.

FUS - Fire Flow Calculations

As per 2020 Fire Underwriter's Survey Guidelines



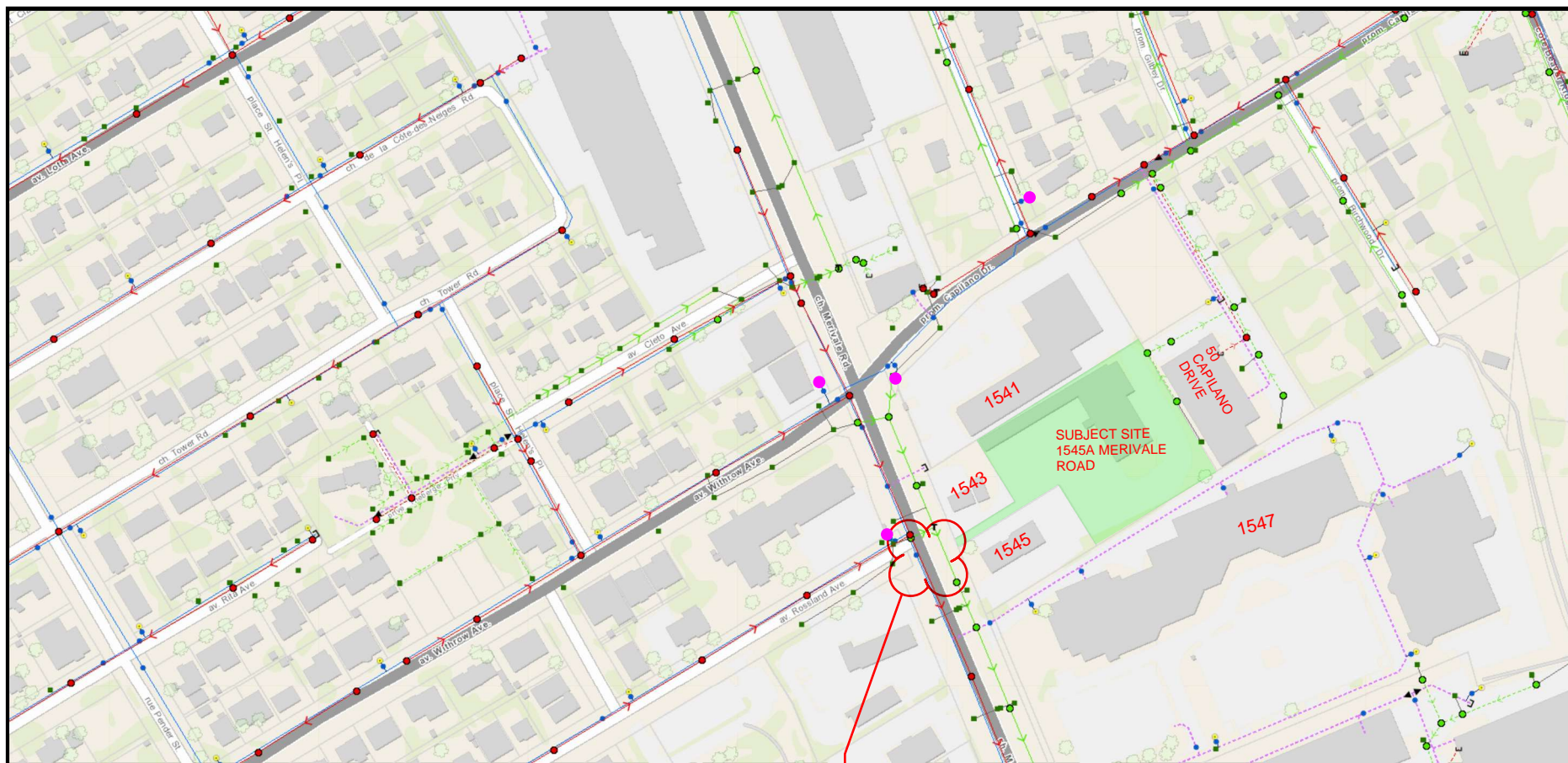
Novatech Project #: 122098
 Project Name: 1545A MERIVALE ROAD
 Date: 9/28/2022
 Input By: Samantha Bennett
 Reviewed By: Trevor McKay

Legend

Input by User
 No Information or Input Required

Building Description: Commercial 1 Storey + underground parking garage
 Non-combustible construction

| Step | Input | | Value Used | Total Fire Flow (L/min) | | |
|---------------------------------------|--|---|----------------------------|-------------------------|---------------|--------------|
| Base Fire Flow | | | | | | |
| 1 | Construction Material | | Multiplier | | | |
| | Coefficient related to type of construction C | Wood frame | | 1.5 | 0.8 | |
| | | Ordinary construction | | 1 | | |
| | | Non-combustible construction | Yes | 0.8 | | |
| | | Modified Fire resistive construction (2 hrs) | | 0.6 | | |
| Fire resistive construction (> 3 hrs) | | | 0.6 | | | |
| 2 | Floor Area | | | | | |
| | A | Building Footprint (m ²) | 2566 | | | |
| | | Number of Floors/Storeys | 1 | | | |
| | | Area of structure considered (m ²) | | 2,566 | | |
| F | Base fire flow without reductions | | 9,000 | | | |
| | $F = 220 C (A)^{0.5}$ | | | | | |
| Reductions or Surcharges | | | | | | |
| 3 | Occupancy hazard reduction or surcharge | | Reduction/Surcharge | | | |
| | (1) | Non-combustible | | -25% | 0% | |
| | | Limited combustible | | -15% | | |
| | | Combustible | Yes | 0% | | |
| | | Free burning | | 15% | | |
| Rapid burning | | | 25% | | | |
| 4 | Sprinkler Reduction | | Reduction | | | |
| | (2) | Adequately Designed System (NFPA 13) | Yes | -30% | -30% | |
| | | Standard Water Supply | | -10% | | |
| | | Fully Supervised System | | -10% | | |
| Cumulative Total | | | -30% | | | |
| 5 | Exposure Surcharge (cumulative %) | | Surcharge | | | |
| | (3) | North Side | 3.1 - 10 m | 20% | 4,500 | |
| | | East Side | 10.1 - 20 m | 15% | | |
| | | South Side | 20.1 - 30 m | 10% | | |
| | | West Side | 30.1 - 45 m | 5% | | |
| Cumulative Total | | | 50% | | | |
| Results | | | | | | |
| 6 | (1) + (2) + (3) | Total Required Fire Flow, rounded to nearest 1000L/min | | L/min | 11,000 | |
| | | (2,000 L/min < Fire Flow < 45,000 L/min) | | or | L/s | 183 |
| | | | | or | USGPM | 2,906 |
| 7 | Storage Volume | Required Duration of Fire Flow (hours) | | Hours | 2 | |
| | | Required Volume of Fire Flow (m ³) | | m ³ | 1320 | |



PROPOSED CONNECTION TO EX. 300mm DIA.
WATERMAIN ON MERIVALE ROAD.
+/-15m SOUTH OF ROSSLAND AVE.

● EXISTING PUBLIC HYDRANT

| Node | Elevation | Consumption Rates (L/s) | | |
|------------------------------|-----------|-------------------------|---------------|----------------|
| | | Average Daily | Maximum Daily | Maximum Hourly |
| N1 (Building) | 97.00 | 0.22 | 0.34 | 0.60 |
| N2 (Cross for Hydrant Leads) | 96.85 | 0.00 | 0.00 | 0.00 |
| | | 0.22 | 0.34 | 0.60 |

Water Demand Parameters

1. Commercial Space

| | |
|--------------------|----------------------|
| Average Day Demand | 28,000 L/ha/day |
| Commercial Area | 6,900 m ² |
| Maximum Day Demand | 1.5 x avg. day |
| Peak Hour Demand | 1.8 x max. day |

2. Required Fire Flow =183L/s (at N2)

Notes:

- 1) Water demand based on City of Ottawa Design Guidelines - Water Distribution (2010, as amended)
- 2) Fireflows calculated as per 2020 Fire Underwriter's Survey Guidelines.

Junction Report

| Node ID | Elevation | Demand | Total Head | Pressure | Pressure | Pressure | Age |
|---------|-----------|--------|------------|----------|----------|----------|-------|
| | m | LPS | m | m | kPa | psi | hours |
| R1 | 134.10 | -0.22 | 134.10 | 0.00 | 0.00 | 0.00 | 0.0 |
| N1 | 97.00 | 0.22 | 134.10 | 37.10 | 363.95 | 52.79 | 5.6 |
| N2 | 96.85 | 0.00 | 134.10 | 37.25 | 365.42 | 53.00 | 5.3 |

Maximum Pressure
 Maximum Age

Pipe Report

| Link ID | Length m | Diameter mm | Roughness | Flow LPS | Velocity m/s | Headloss m/km | Friction Factor |
|---------|-------------|----------------|-----------|-------------|-----------------|------------------|--------------------|
| Pipe 1 | 13.00 | 150 | 100 | 0.22 | 0.01 | 0.00 | 0.082 |
| Pipe 2 | 85.00 | 250 | 110 | 0.22 | 0.00 | 0.00 | 0.053 |

Junction Report

| Node ID | Elevation m | Demand LPS | Total Head m | Pressure m | Pressure kPa | Pressure psi |
|---------|----------------|---------------|-----------------|---------------|-----------------|-----------------|
| R1 | 125.40 | -0.60 | 125.40 | 0.00 | 0.00 | 0.00 |
| N1 | 97.00 | 0.60 | 125.40 | 28.40 | 278.60 | 40.41 |
| N2 | 96.85 | 0.00 | 125.40 | 28.55 | 280.08 | 40.62 |

 Minimum Pressure

Pipe Report

| Link ID | Length m | Diameter mm | Roughness | Flow LPS | Velocity m/s | Headloss m/km | Friction Factor |
|---------|-------------|----------------|-----------|-------------|-----------------|------------------|--------------------|
| Pipe 1 | 13.00 | 150 | 100 | 0.60 | 0.03 | 0.020 | 0.060 |
| Pipe 2 | 85.00 | 250 | 110 | 0.60 | 0.01 | 0.000 | 0.054 |

Junction Report

| Node ID | Elevation m | Demand LPS | Total Head m | Pressure m | Pressure kPa | Pressure psi |
|---------|----------------|---------------|-----------------|---------------|-----------------|-----------------|
| R1 | 123.00 | -183.34 | 123.00 | 0.00 | 0.00 | 0.00 |
| N1 | 97.00 | 0.34 | 117.44 | 20.44 | 200.52 | 29.08 |
| N2 | 96.85 | 183.00 | 117.44 | 20.59 | 201.99 | 29.30 |

 Minimum Pressure

Pipe Report

| Link ID | Length m | Diameter mm | Roughness | Flow LPS | Velocity m/s | Headloss m/km | Friction Factor |
|---------|-------------|----------------|-----------|-------------|-----------------|------------------|--------------------|
| Pipe 1 | 13.00 | 150 | 100 | 0.34 | 0.020 | 0.010 | 0.068 |
| Pipe 2 | 85.00 | 200 | 110 | 183.34 | 3.730 | 65.400 | 0.023 |

APPENDIX F

Drawings

122098-GP
122098-GR
122098-STM
122098-ESC

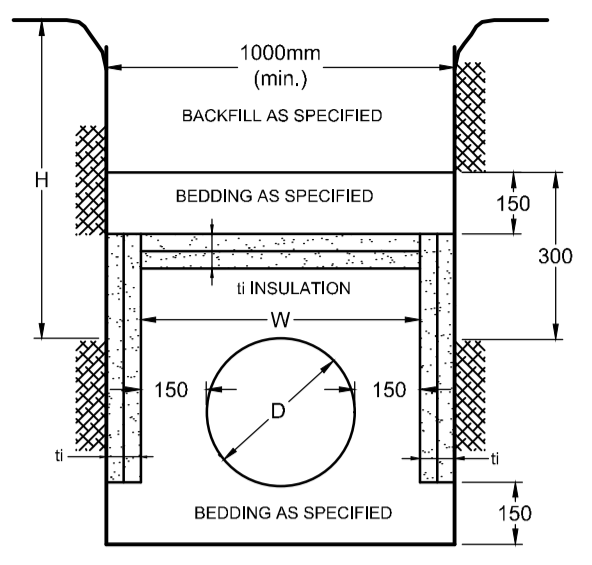
| SAN MANHOLE TABLE | | |
|-------------------|----------|----------------------------------|
| MANHOLE ID | T/G ELEV | INVERT |
| 99 | 96.58 | NE=94.50 NW=93.80 SE=93.80 |
| 101 | 96.68 | NE=94.76 SW=94.73 |
| 103 | 96.74 | SW=95.03 NE=95.06 |

| STM MANHOLE TABLE | | |
|-------------------|----------|--|
| MANHOLE ID | T/G ELEV | INVERT |
| 100 | 96.55 | NE=94.02 NW=93.50 SE=93.80 |
| 102 | 96.64 | NE=94.15 SW=94.14 |
| 104 | 96.68 | SW=94.42 NE=94.49 SE=94.53 NW=94.55 |

| CATCHBASIN MANHOLE TABLE | | | | | | |
|--------------------------|-----------|--------------|----------------------|--------------|------------------------|--|
| CBMH ID | SIZE (mm) | T/G ELEV (m) | INVERT (m) | ICD DIA (mm) | 2yr CAPTURE RATE (L/s) | |
| CBMH 02 | 1200mm Ø | 96.50 | NW=95.29 SE=95.26 | - | - | |
| CBMH 03 | 1200mm Ø | 96.50 | NW=95.13 SW=95.07 | - | - | |
| CBMH 04 | 1200mm Ø | 96.50 | NE=94.90 W=94.87 | - | - | |
| CBMH 05 | 1200mm Ø | 96.45 | E=94.72 NW=94.66 | LMF-75 | 6.76 | |
| CBMH 06 | 1200mm Ø | 96.45 | SE=94.77 | - | - | |
| CBMH 07 | 1200mm Ø | 96.45 | NW=94.64 SE=94.63 | 83mm | 17.11 | |

| CATCHBASIN TABLE | | | | | |
|------------------|--------------|------------|--------------|------------------------|--|
| CB ID | T/G ELEV (m) | INVERT (m) | ICD DIA (mm) | 2yr CAPTURE RATE (L/s) | |
| CB 08 | 96.55 | 95.50 | LMF-95 | 7.85 | |
| CB 09 | 96.55 | 95.50 | LMF-85 | 6.81 | |
| CICB-OS1 | 96.54 | 95.49 | - | - | |
| LCB 01 | 96.80 | 95.60 | - | - | |

| WATERMAIN TABLE | | | | |
|-----------------|---------------|------------------|------------------------|--|
| Station | F/G ELEVATION | TOP OF WATERMAIN | DESCRIPTION | |
| 0+007.19 | 96.59 | ±95.15 | 250 x 300 TEE | |
| 0+009.76 | 96.56 | 94.56 | 45° VERTICAL BEND | |
| 0+010.26 | 96.56 | 94.14 | 45° VERTICAL BEND | |
| 0+019.30 | 96.54 | 94.14 | 45° VERTICAL BEND | |
| 0+020.20 | 96.54 | 95.04 | 45° VERTICAL BEND | |
| 0+022.92 | 96.53 | 95.04 | 45° VERTICAL BEND | |
| 0+023.82 | 96.53 | 94.13 | 45° VERTICAL BEND | |
| 0+028.98 | 96.69 | 94.29 | V&VB | |
| 0+031.50 | 96.69 | 94.29 | 11.25° HORIZONTAL BEND | |
| 0+091.49 | 96.67 | 94.27 | 250 x 150 CROSS | |
| 0+094.37 | 96.71 | 94.31 | 250 x 150 REDUCER | |
| 0+095.43 | 96.72 | 94.32 | V&VB | |
| 0+105.31 | 96.98 | 94.50 | CAP | |

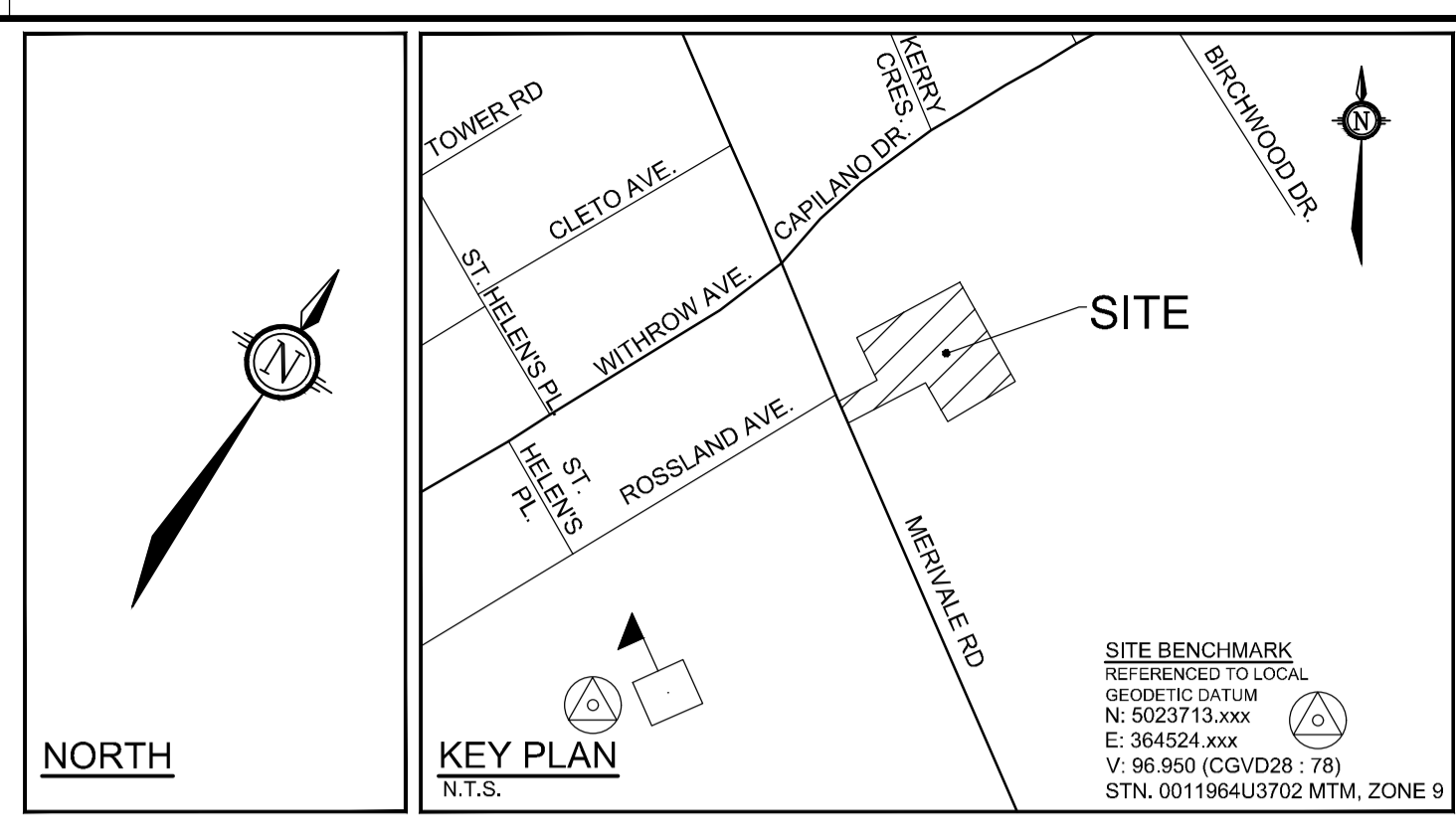


| COVER (mm) | INSULATION THICKNESS (mm) |
|------------|---------------------------|
| 1800-1500 | 50 |
| 1500-1200 | 75 |
| 1200-900 | 100 |
| 900-600 | 125 |

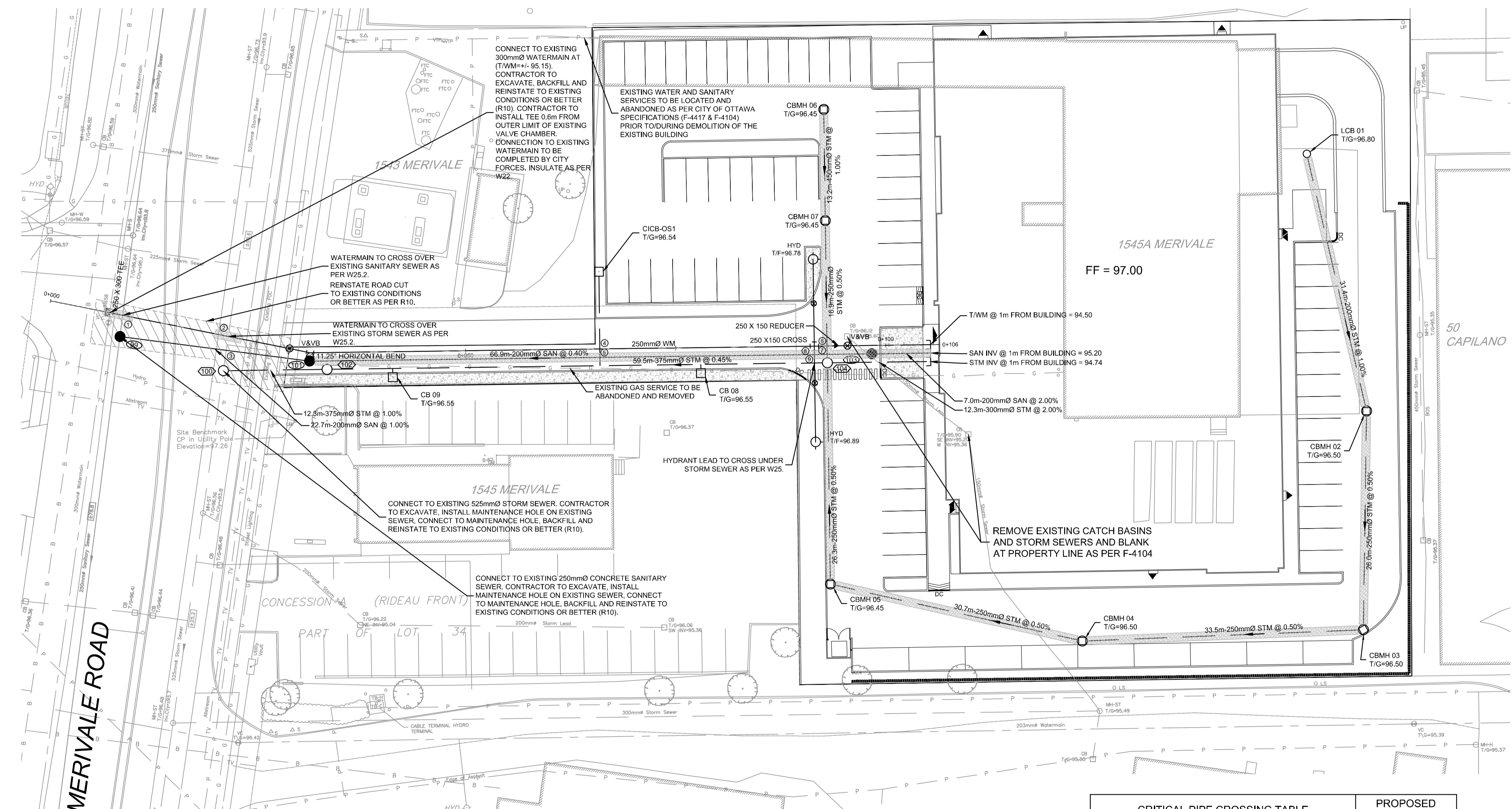
t = THICKNESS OF INSULATION (mm)
 h = DEPTH OF COVER
 W = D + 300 (1000 min.)
 W = WIDTH OF INSULATION (mm)
 D = O.D. OF PIPE (mm)

- NOTES:
- INSULATE ALL SEWER PIPES THAT ARE LESS THAN 600mmØ AND HAVE LESS THAN 1.8m COVER WITH EXPANDED POLYSTYRENE INSULATION AS SHOWN.
 - THE THICKNESS OF INSULATION SHALL BE THE EQUIVALENT OF 25mm FOR EVERY 300mm REDUCTION IN THE REQUIRED DEPTH OF COVER (SEE TABLE).

INSULATION DETAIL FOR SHALLOW SEWERS ONLY
NOT TO SCALE



SOURCE REFERENCE:
 TOPOGRAPHIC PLAN OF SURVEY OF PART OF LOT 16 REGISTERED PLAN 353 AND PART OF LOT 34 CONCESSION A (RIDEAU FRONT), PREPARED BY FARLEY, SMITH & DENIS SURVEYING LTD. 2021 ON OCTOBER 1st, 2021
 TOPOGRAPHIC INFORMATION:
 HORIZONTAL DATUM: NAD 83 (ORIGINAL, MTM - ZONE 9)
 VERTICAL DATUM: CGVD 1928-1978
 1. CITY OF OTTAWA 1:1000 MAPPING
 2. FARLEY, SMITH & DENIS SURVEYING LTD'S TOPOGRAPHIC PLAN OF SURVEY



- GENERAL NOTES:
- DIMENSIONS AND LAYOUT INFORMATION SHALL BE CONFIRMED PRIOR TO START OF CONSTRUCTION.
 - THE ORIGINAL TOPOGRAPHY AND GROUND ELEVATIONS, SERVICING AND SURVEY INFORMATION SHOWN ON THIS PLAN ARE SUPPLIED FOR INFORMATION PURPOSES ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE ACCURACY OF ALL INFORMATION OBTAINED FROM THIS PLAN, PRIOR TO COMMENCING ANY ON SITE SERVICING. THE CONTRACTOR SHALL VERIFY THE ELEVATIONS OF THE EXISTING SEWERS, WATERMAINS AND UTILITIES IN THE MERIVALE ROAD RIGHT OF WAY.
 - CO-ORDINATE AND SCHEDULE ALL WORK WITH OTHER TRADES AND CONTRACTORS.
 - BEFORE COMMENCING CONSTRUCTION, PROVIDE PROOF OF COMPREHENSIVE ALL RISK AND OPERATIONAL LIABILITY INSURANCE INCLUDING BLASTING. INSURANCE POLICY TO NAME THE OWNER, ENGINEER AND THE CITY AS CO-INSURED.
 - CONNECT TO EXISTING SYSTEMS AS DETAILED, INCLUDING ALL RESTORATION WORK NECESSARY TO REINSTATE SURFACES TO EXISTING CONDITIONS OR BETTER.
 - DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME RESPONSIBILITY FOR ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON THESE DRAWINGS.
 - OBTAIN AND PAY FOR ALL NECESSARY PERMITS AND APPROVALS BEFORE COMMENCING CONSTRUCTION.
 - RESTORE ALL TRENCHES AND SURFACE FEATURES TO EXISTING CONDITIONS OR BETTER AND TO THE SATISFACTION OF MUNICIPAL AUTHORITIES.
 - REMOVE FROM SITE ALL DEBRIS AND EXCESS EXCAVATED MATERIAL UNLESS OTHERWISE INSTRUCTED BY THE ENGINEER.
 - ALL ELEVATIONS ARE GEODETIC AND UTILIZE METRIC UNITS.
 - REFER TO STRUCTURAL PLANS FOR UNDERSIDE OF FOOTING AND TOP OF FOUNDATION INFORMATION.
 - REFER TO GEOTECHNICAL INVESTIGATION P66288-1 (DATED AUGUST 3, 2022), PREPARED BY PATERSON GROUP INC. FOR SUBSURFACE CONDITIONS AND CONSTRUCTION RECOMMENDATIONS.
 - PERFORATED PIPE SUB-DRAINS TO BE PROVIDED 300mm BELOW SUBGRADE LEVEL EXTENDING FROM THE ROADSIDE CATCH-BASIN FOR A DISTANCE OF 3.0m, PARALLEL TO THE CURB IN TWO DIRECTIONS AS PER CITY OF OTTAWA DETAIL R1.
 - PERFORATED PIPE SUB-DRAINS TO BE PROVIDED 300mm BELOW SUBGRADE LEVEL EXTENDING FROM ALL PARKING LOT CATCHBASIN AND CATCH BASIN MAINTENANCE HOLES FOR A DISTANCE OF 3.0m, PARALLEL AND PERPENDICULAR TO THE CURB IN FOUR DIRECTIONS. CLEAR STONE PIPE SURROUND AND GEOTEXTILE WRAP TO BE PROVIDED AS PER CITY OF OTTAWA DETAIL R1.
 - CONTRACTOR TO PROVIDE ALL LINE PAINTING AND PARKING LOT MARKINGS.
 - CONTRACTOR TO PROVIDE THE CONSULTANT WITH A GENERAL PLAN OF SERVICES INDICATING ALL SERVICING AS-BUILT INFORMATION SHOWN ON THIS PLAN. AS-BUILT INFORMATION MUST INCLUDE: PIPE MATERIAL, SIZES, LENGTHS, SLOPES, INVERT AND T/G ELEVATIONS, STRUCTURE LOCATIONS, VALVE AND HYDRANT LOCATIONS, T/W/M ELEVATIONS AND ANY ALIGNMENT CHANGES, ETC.
 - ALL WORK TO BE CONSTRUCTED TO CITY OF OTTAWA AND ONTARIO PROVINCIAL STANDARDS.

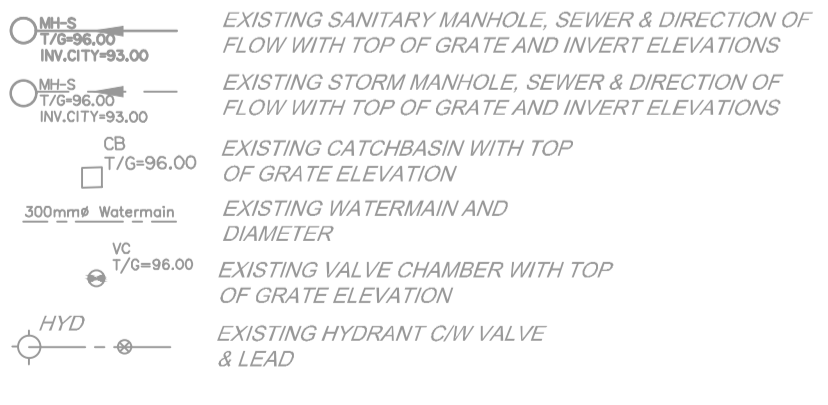
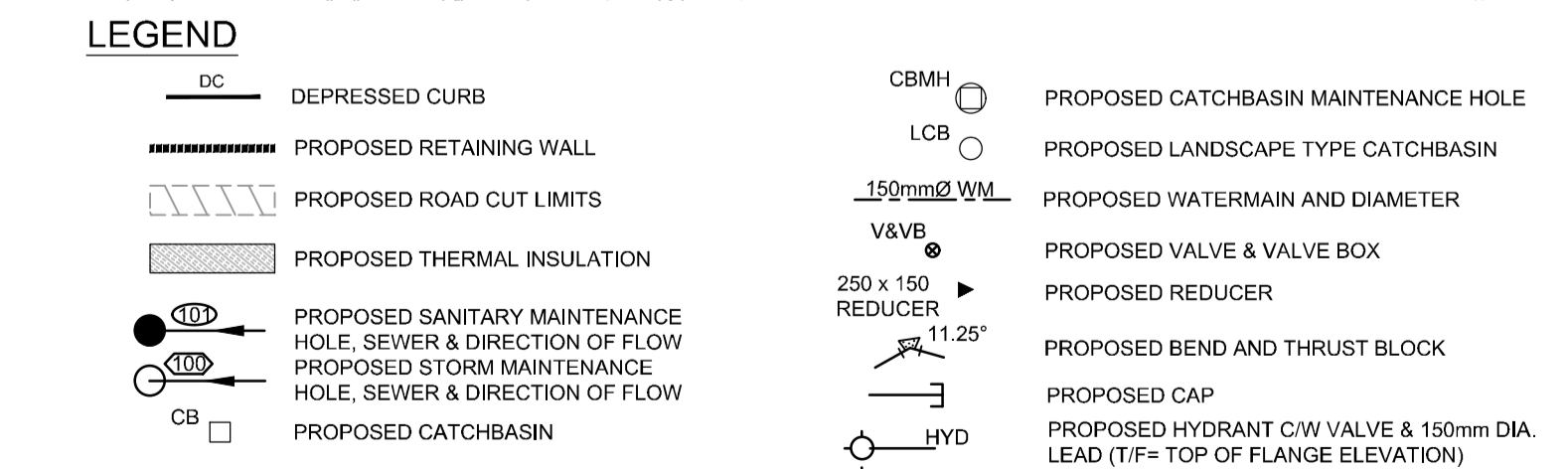
- SEWER NOTES:
- SPECIFICATIONS:

| ITEM | SPEC. No. | REFERENCE |
|---|--------------------------|----------------|
| CATCHBASIN (600x600mm) | 705.010 | OPSD |
| CATCHBASIN MAINTENANCE HOLE (12000) | 701.010 | OPSD |
| STORM / SANITARY MAINTENANCE HOLE (12000) | 701.010 | OPSD |
| CURB INLET CB, FRAME & COVER | S3 & S22 | CITY OF OTTAWA |
| CBMH FRAME & COVER | S2 & S28.1 | CITY OF OTTAWA |
| STORM / SANITARY MH FRAME & COVER | S24.1 / S24 & S25 | CITY OF OTTAWA |
| STORM SEWER | PVC DR 35 OR 100-D CONC. | CITY OF OTTAWA |
| SANITARY SEWER | PVC DR 35 | CITY OF OTTAWA |
| CATCHBASIN LEAD | PVC DR 35 | CITY OF OTTAWA |
| LANDSCAPE CB | S31 | CITY OF OTTAWA |
| SEWER TRENCH | S6 | CITY OF OTTAWA |
 - ALL CATCHBASIN LEADS ARE TO BE 200mm DIA. PVC SDR 35 AT 2% SLOPE UNLESS OTHERWISE SPECIFIED ON THE DRAWING.
 - INSULATE ALL PIPES (SAN/STM) THAT HAVE LESS THAN 1.8m COVER AS PER THE INSULATION DETAIL FOR SHALLOW SEWERS.
 - SERVICES ARE TO BE CONSTRUCTED TO 1.0m FROM BUILDING FACE AT 2.0% SLOPE (1.0% MINIMUM). SERVICES TO BE CONNECTED TO MAINLINE SEWER AS PER CITY OF OTTAWA S11.1.
 - PIPE BEDDING AND COVER ARE TO BE COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY. WHERE THE BEDDING IS LOCATED WITHIN FIRM TO SOFT GREY SILTY CLAY, THE THICKNESS OF THE BEDDING MATERIAL SHOULD BE INCREASED TO A MINIMUM OF 300mm. THE COVER MATERIAL SHALL CONSIST OF OPS GRANULAR 'A' AND SHOULD EXTEND FROM THE SPRING LINE OF THE PIPE TO AT LEAST 300mm ABOVE THE OVERTOP OF THE PIPE.
 - THE SITE SERVICING CONTRACTOR SHALL PERFORM FIELD TESTS FOR QUALITY CONTROL OF ALL SANITARY SEWERS. LEAKAGE TESTING SHALL BE COMPLETED IN ACCORDANCE WITH OPS 410.07.16 AND 407.07.24. DYE TESTING IS TO BE COMPLETED ON ALL SANITARY SERVICES TO CONFIRM PROPER CONNECTION TO THE SANITARY SEWER MAIN. THE FIELD TESTS SHALL BE PERFORMED IN THE PRESENCE OF THE ENGINEER.
 - STORM MAINTENANCE HOLES AND CBMH'S SHALL HAVE 300mm SUMPS UNLESS OTHERWISE INDICATED.
 - CONTRACTOR TO TELEVIEW ALL PROPOSED SEWERS, 200mmØ OR GREATER PRIOR TO BASE COURSE ASPHALT. UPON COMPLETION OF CONTRACT, THE CONTRACTOR IS RESPONSIBLE TO FLUSH AND CLEAN ALL SEWERS & APPURTENANCES.

- WATERMAIN NOTES:
- GENERAL:

| ITEM | DETAIL No. | REFERENCE |
|---|-------------|----------------|
| WATERMAIN TRENCHING | W17 | CITY OF OTTAWA |
| THERMAL INSULATION IN SHALLOW TRENCHES | W22 | CITY OF OTTAWA |
| WATERMAIN CROSSING BELOW SEWER / OVER SEWER | W25 / W25.2 | CITY OF OTTAWA |
| HYDRANT INSTALLATION | W19 | CITY OF OTTAWA |
 - THE WATERMAIN SHALL BE PVC DR 18 IN ACCORDANCE WITH MATERIAL SPECIFICATION MW-18.1, UNLESS OTHERWISE INDICATED.
 - SUPPLY AND CONSTRUCT ALL WATERMAINS AND APPURTENANCES IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARDS AND SPECIFICATIONS. EXCAVATION, INSTALLATION, BACKFILL AND RESTORATION OF ALL WATERMAINS BY THE CONTRACTOR. CONNECTIONS AND SHUT-OFFS AT THE MAIN AND CHLORINATION OF THE WATER SYSTEM SHALL BE PERFORMED BY CITY OFFICIALS.
 - WATERMAIN SHALL BE MINIMUM 2.4m DEPTH BELOW GRADE UNLESS OTHERWISE INDICATED. CONTRACTOR TO SUPPLY AND INSTALL INSULATION AS PER W22 FOR ALL WATERMAIN LESS THAN 2.4m BELOW GRADE.
 - PROVIDE MINIMUM CLEARANCE BETWEEN OUTSIDE OF PIPES AT ALL CROSSINGS PER W25 (0.50m) AND W25.2 (0.25m).
 - WATER SERVICES ARE TO BE CAPPED 1.0m FROM BUILDING FACE.
 - UPON COMPLETION, CONTRACTOR TO PROVIDE PRIVATE FIRE HYDRANT FLOW AND PRESSURE TEST REPORTS TO THE CONSULTANT. ALL PRIVATE HYDRANTS (ON-SITE) ARE TO BE RED. ALL PAINT SHALL BE ALKYD ENAMEL AND MEET THE MANUFACTURERS REQUIREMENTS. THE BONNET, PUMPER AND HOSE OUTLET CAPS SHALL ALL BE PAINTED ACCORDING TO THE FOLLOWING TABLE:
 CAP COLOUR - MEASURED FLOW
 BLUE - 1500 GPM (5700L/min)
 GREEN - 1000-1499 GPM (3800-5699L/min)
 ORANGE - 500-999 GPM (1800-3799L/min)

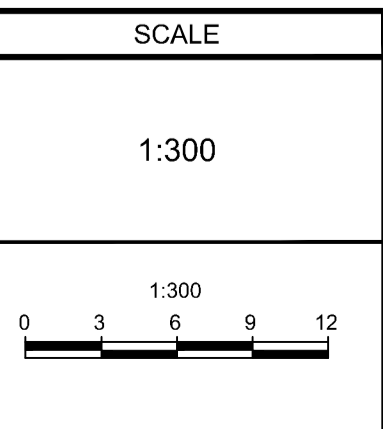
| CRITICAL PIPE CROSSING TABLE | | | PROPOSED SEPARATION |
|------------------------------|----------------------|----------------------|---------------------|
| ① | 250mmØ WM INV=94.36 | 250mmØ SAN OBV=94.06 | WM-SAN=0.30m |
| ② | 250mmØ WM INV=94.79 | 525mmØ STM OBV=94.54 | WM-STM=0.25m |
| ③ | 200mmØ SAN INV=94.63 | 525mmØ STM OBV=94.54 | SAN-STM=0.09m |
| ④ | 200mmØ STM INV=95.32 | 250mmØ WM OBV=94.28 | STM-WM=1.04m |
| ⑤ | 200mmØ STM INV=95.30 | 200mmØ SAN OBV=95.10 | STM-SAN=0.20m |
| ⑥ | 250mmØ STM INV=94.56 | 250mmØ WM OBV=94.29 | STM-WM=0.27m |
| ⑦ | 200mmØ SAN INV=95.01 | 250mmØ STM OBV=94.81 | SAN-STM=0.20m |
| ⑧ | 200mmØ SAN INV=95.00 | 150mmØ WM OBV=94.21 | SAN-WM=0.79m |
| ⑨ | 375mmØ STM INV=94.41 | 150mmØ WM OBV=93.91 | STM-WM=0.50m |



NOTE:
 THE POSITION OF ALL POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.



| No. | REVISION | DATE | BY |
|-----|-------------------------------|-----------|-----|
| 2. | ISSUED FOR SITE PLAN APPROVAL | DEC 23/22 | TJM |
| 1. | ISSUED FOR COORDINATION | NOV 16/22 | TJM |



FOR REVIEW ONLY

| | |
|----------|-----|
| DESIGN | JMR |
| CHECKED | SAB |
| DRAWN | JMR |
| CHECKED | SAB |
| APPROVED | TJM |

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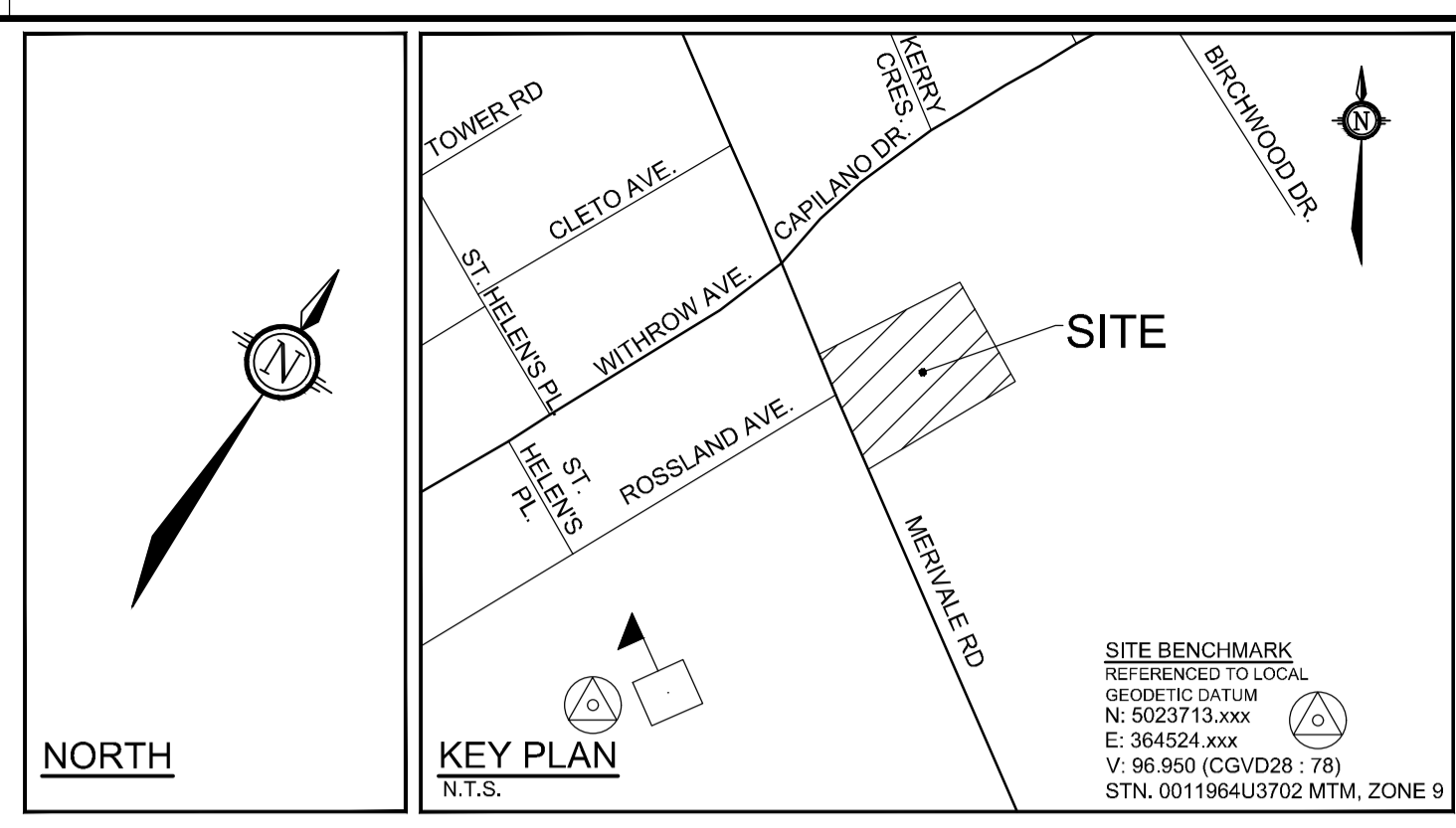
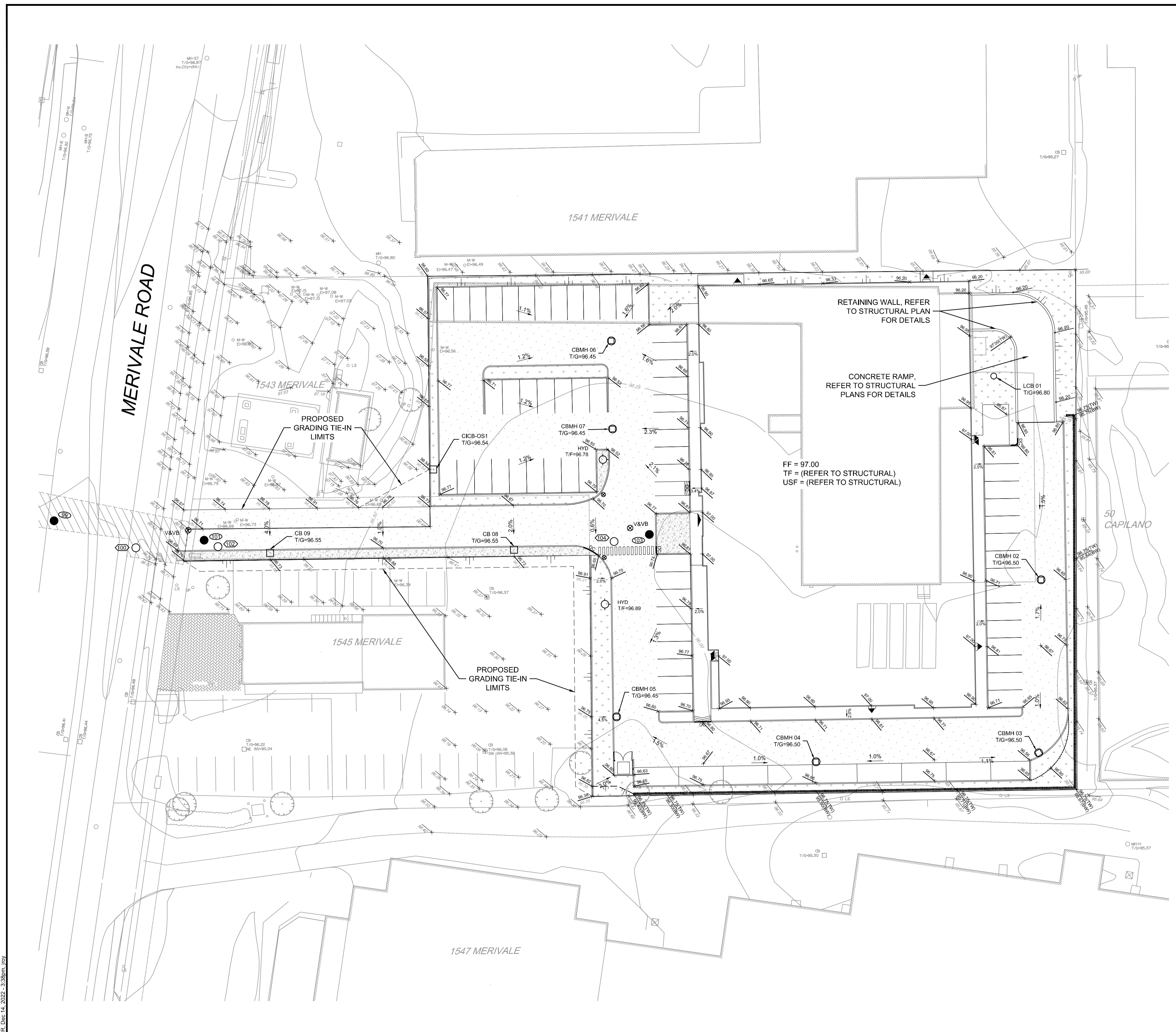
LOCATION
 CITY OF OTTAWA
 1545A MERIVALE ROAD

DRAWING NAME
 GENERAL PLAN OF SERVICES

PROJECT No.
 122098

REV # 2

DRAWING No.
 122098-GP



SOURCE REFERENCE:
 TOPOGRAPHIC PLAN OF SURVEY OF PART OF LOT 16 REGISTERED PLAN 353 AND PART OF LOT 34 CONCESSION A (RIDEAU FRONT), PREPARED BY FARLEY, SMITH & DENIS SURVEYING LTD. 2021 ON OCTOBER 1st, 2021

TOPOGRAPHIC INFORMATION:
 HORIZONTAL DATUM: NAD 83 (ORIGINAL), MTM - ZONE 9
 VERTICAL DATUM: CGVD 1928-1978
 1. CITY OF OTTAWA 1:1000 MAPPING
 2. FARLEY, SMITH & DENIS SURVEYING LTD'S TOPOGRAPHIC PLAN OF SURVEY

LEGEND

| | | | |
|-----------|--|------|---|
| 97.00 | PROPOSED ELEVATION | ⊙ | PROPOSED SANITARY MAINTENANCE HOLE |
| 96.50 | EXISTING ELEVATION | ⊙ | PROPOSED STORM MAINTENANCE HOLE |
| 97.00(TW) | PROPOSED TOP OF WALL ELEVATION | CB | PROPOSED CATCHBASIN |
| 97.00(BW) | PROPOSED ELEVATION AT FRONT FACE OF RETAINING WALL (TO MATCH EXISTING GRADE) | CBMH | PROPOSED CATCHBASIN MAINTENANCE HOLE |
| - | EXISTING CONTOUR AND ELEVATION | LCB | PROPOSED LANDSCAPE TYPE CATCHBASIN |
| - | TERRACE TO EXISTING (3:1 MAX) | VB | PROPOSED VALVE & VALVE BOX LOCATION |
| - | PROPOSED RETAINING WALL | HYD | PROPOSED HYDRANT WITH TOP OF FLANGE ELEVATION |
| - | PROPOSED GRADING TIE-IN LIMITS | DC | DEPRESSED CURB |
| - | PROPOSED BERM LIMITS | | |

- GRADING AND PAVEMENT NOTES:**
- ALL TOPSOIL, ORGANIC OR DELETERIOUS MATERIAL MUST BE ENTIRELY REMOVED FROM BENEATH THE PROPOSED HARD SURFACE (i.e. PAVEMENT, CURB, SIDEWALK, ETC.) AREAS AS DIRECTED BY THE SITE ENGINEER OR GEOTECHNICAL ENGINEER.
 - EXPOSED SUBGRADES IN PROPOSED PAVED AREAS SHOULD BE HEAVILY PROOF ROLLED WITH A LARGE (10 TON) VIBRATORY STEEL DRUM ROLLER UNDER DRY CONDITIONS AND INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF GRANULARS.
 - ANY SOFT AREAS EVIDENT FROM THE PROOF ROLLING SHOULD BE SUB-EXCAVATED AND REPLACED WITH SUITABLE MATERIAL THAT IS FROST COMPATIBLE WITH THE EXISTING SOILS AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
 - THE GRANULAR BASE SHOULD BE PLACED IN MAXIMUM 300mm LIFTS AND COMPACTED TO AT LEAST 98% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE. ANY ADDITIONAL GRANULAR FILL USED BELOW THE PROPOSED PAVEMENT SHOULD BE PLACED IN MAXIMUM 300mm LIFTS AND COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE.
 - ROADWAY SUBGRADE TO BE INSPECTED BY THE GEOTECHNICAL ENGINEER AT THE TIME OF CONSTRUCTION TO REVIEW IF A WOVEN GEOTEXTILE IS REQUIRED BELOW THE GRANULAR MATERIALS, AND TO CONFIRM THE DEPTH AND COMPACTION OF GRANULAR B.
 - PRIOR TO PLACEMENT OF WEAR COURSE ASPHALT, THE CONTRACTOR SHALL ADJUST ALL STRUCTURES TO FINAL GRADE PER CITY OF OTTAWA STANDARDS.
 - MINIMUM OF 2% GRADE FOR ALL GRASSED AREAS UNLESS OTHERWISE NOTED.
 - MAXIMUM TERRACING GRADE TO BE 3:1 UNLESS OTHERWISE NOTED.
 - ALL GRADES BY CURBS ARE EDGE OF PAVEMENT GRADES UNLESS OTHERWISE INDICATED.
 - ALL CURBS SHALL BE BARRIER CURB UNLESS OTHERWISE NOTED AND CONSTRUCTED PER CITY OF OTTAWA STANDARD (SC1).
 - ALL SIDEWALKS ARE TO HAVE 2% CROSSFALL UNLESS OTHERWISE NOTED. CROSSFALL IS TO BE DIRECTED AWAY FROM BUILDINGS AND PROPERTY LINES UNLESS OTHERWISE NOTED.
 - BARRIER FREE CURB RAMP TO BE PROVIDED AS PER CITY OF OTTAWA STANDARD (SC6).
 - REFER TO LANDSCAPE PLAN FOR PLANTING AND OTHER LANDSCAPE FEATURE DETAILS.
 - CONTRACTOR TO SUBMIT SHOP DRAWINGS OF ALL TWS/S TO THE CONSULTANT.
 - ALL RETAINING WALLS TO BE DESIGNED BY STRUCTURAL ENGINEER. ANY RETAINING WALL WITH EXPOSED FACE >0.6m WILL REQUIRE A GUARDRAIL AS PER OBC.

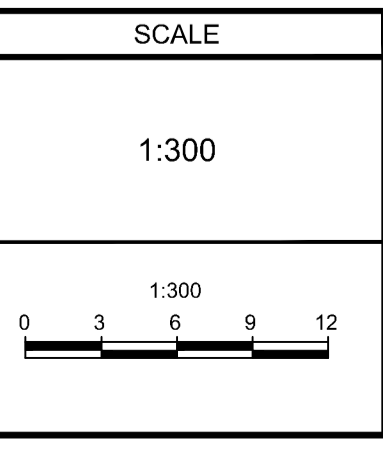
PAVEMENT STRUCTURE:

| | |
|-------|----------------------|
| 40mm | ASPHALT SP12.5 |
| 50mm | ASPHALT SP19.0 |
| 150mm | GRANULAR "A" |
| 450mm | GRANULAR "B" TYPE II |
| 690mm | TOTAL DEPTH |
| | |
| 50mm | ASPHALT SP12.5 |
| 150mm | GRANULAR "A" |
| 300mm | GRANULAR "B" TYPE II |
| 500mm | TOTAL DEPTH |

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| 1. | ISSUED FOR COORDINATION | NOV 16/22 | TJM |



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| CHECKED | SAB |
| APPROVED | TJM |

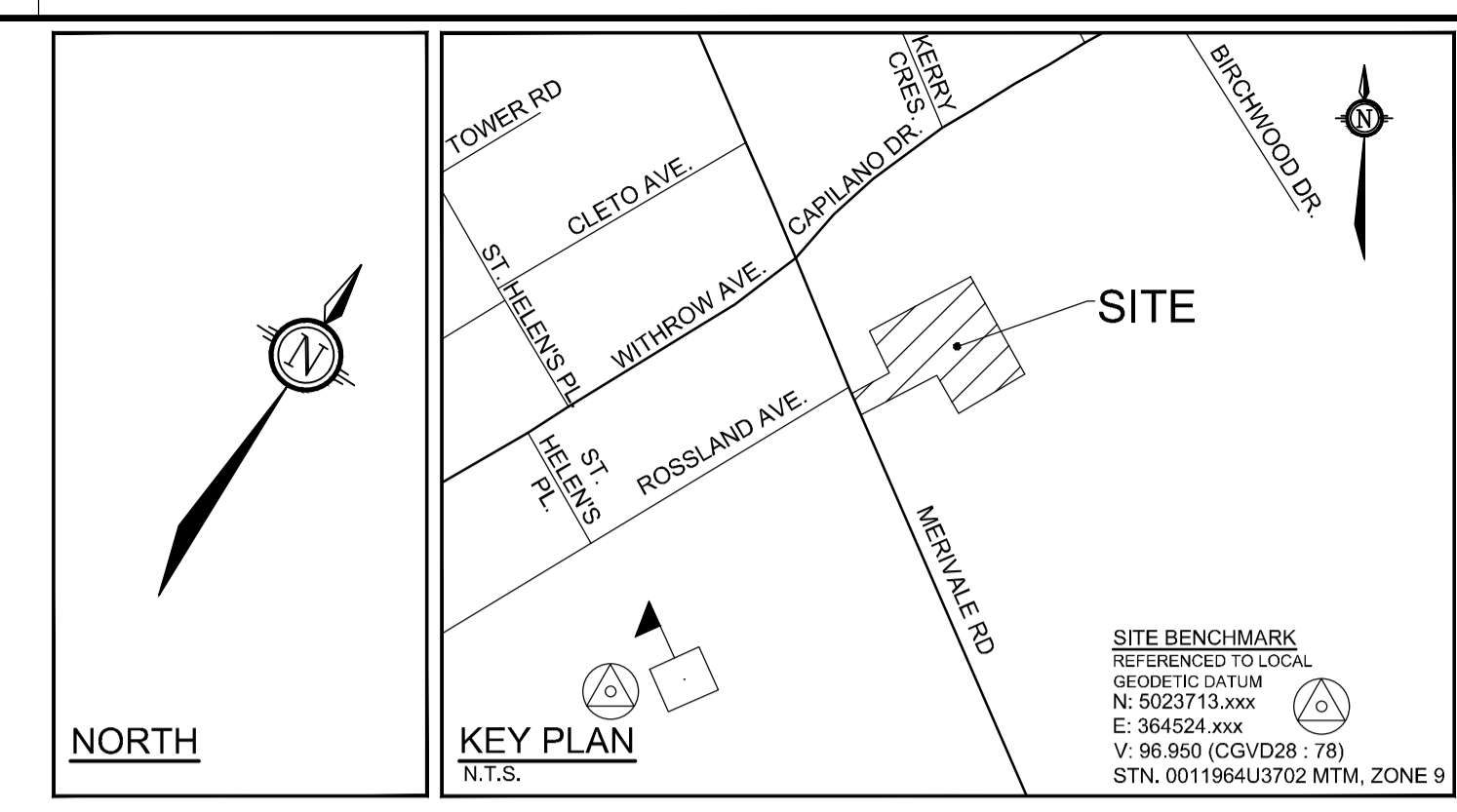


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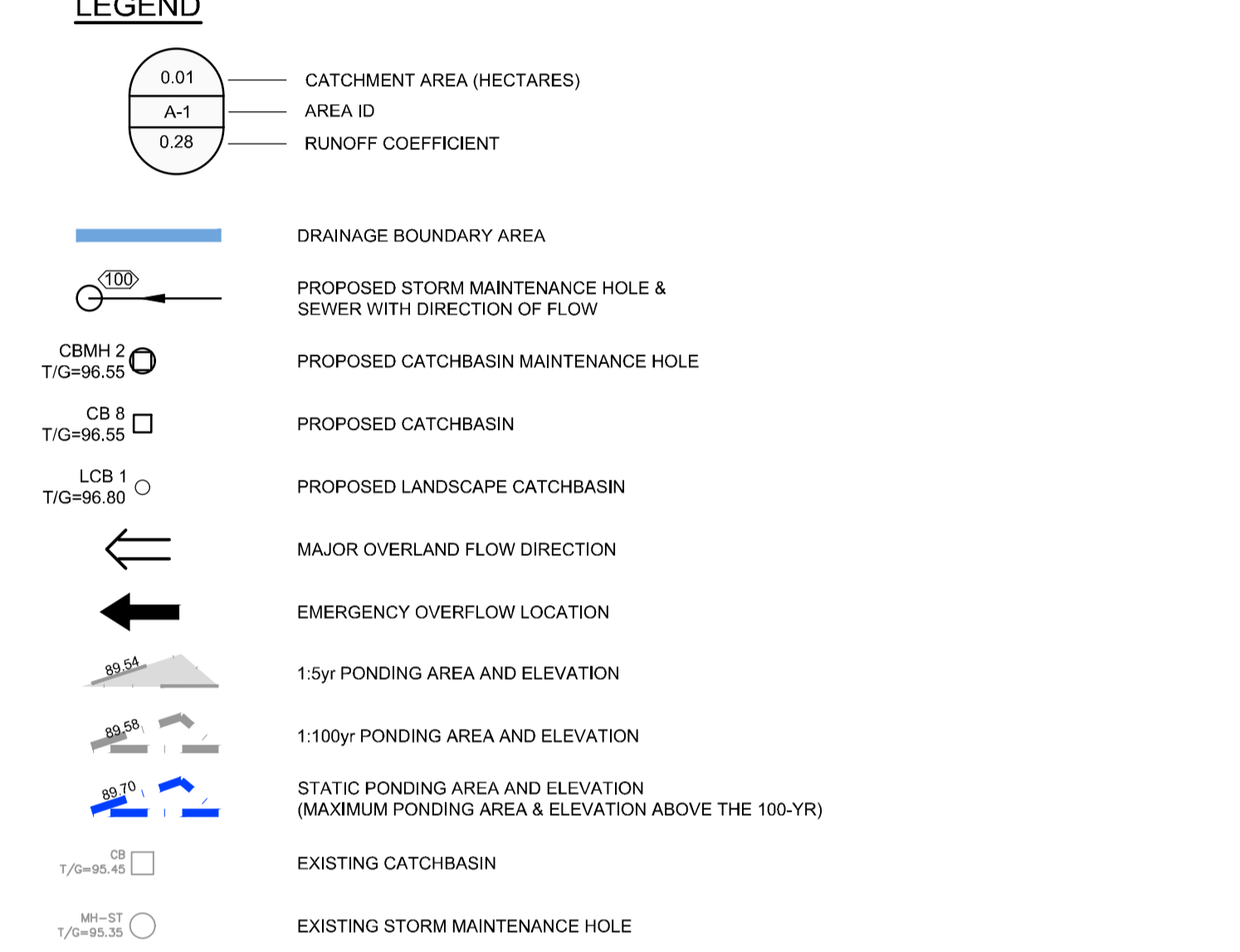
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|---|--------------------------|
| LOCATION CITY OF OTTAWA 1545A MERIVALE ROAD | PROJECT No. 122098 |
| DRAWING NAME GRADING PLAN | REV # REV # 2 |
| | DRAWING No. 122098-GR |

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SOURCE REFERENCE:
TOPOGRAPHIC PLAN OF SURVEY OF PART OF LOT 16 REGISTERED PLAN 353 AND PART OF LOT 34 CONCESSION A (RIEUAU FRONT), PREPARED BY FARLEY, SMITH & DENIS SURVEYING LTD. 2021 ON OCTOBER 1st, 2021

TOPOGRAPHIC INFORMATION:
HORIZONTAL DATUM: NAD 83 (ORIGINAL), MTM - ZONE 9
VERTICAL DATUM: CGVD 1928-1978
1. CITY OF OTTAWA 1:1000 MAPPING
2. FARLEY, SMITH & DENIS SURVEYING LTD'S TOPOGRAPHIC PLAN OF SURVEY



PROPOSED ROOF DRAIN TABLE: AREA B-1 (ROOF DRAINS 1 to 7)

| AREA ID | ROOF DRAIN No. (WATTS MODEL) | ROOF DRAIN SETTING | 1.5 YEAR RELEASE RATE | APPROX. 5-YR PONDING DEPTH | 1:100 YEAR RELEASE RATE | APPROX. 100-YR PONDING DEPTH |
|---------|------------------------------|--------------------|-----------------------|----------------------------|-------------------------|------------------------------|
| B-1 | RD 1 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 12 cm | 1.89 L/s | 15 cm |
| B-1 | RD 2 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 12 cm | 1.89 L/s | 15 cm |
| B-1 | RD 3 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 12 cm | 1.89 L/s | 15 cm |
| B-1 | RD 4 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 13 cm | 1.89 L/s | 15 cm |
| B-1 | RD 5 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 12 cm | 1.89 L/s | 15 cm |
| B-1 | RD 6 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 10 cm | 1.89 L/s | 15 cm |
| B-1 | RD 7 (RD-100-A-ADJ) | FULLY OPEN | 1.26 L/s | 10 cm | 1.89 L/s | 15 cm |

* REFER TO THE 'SERVICING DESIGN BRIEF' (R-2022-207) PREPARED BY NOVATECH FOR STORMWATER MANAGEMENT DETAILS.
** ALL CONTROLLED FLOW ROOF DRAINS FOR THE PROPOSED BUILDING TO BE WATTS 'ADJUSTABLE ACCUTROL' ROOF DRAINS.

INLET CONTROL DEVICE DATA TABLE - CBMH05

| DESIGN EVENT | ICD TYPE | DIAMETER OF OUTLET PIPE (mm) | DESIGN FLOW (L/s) | DESIGN HEAD (m) | WATER ELEVATION (m) | VOLUME (m³) | AVAILABLE STORAGE |
|--------------|----------------------------|------------------------------|-------------------|-----------------|---------------------|-------------|-------------------|
| 1:2 YR | IPEX TEMPEST VORTEX LMF 75 | 250mmØ PVC | 6.82 | 1.67 | 96.33 | 0.00 | |
| 1:5 YR | | | 7.23 | 1.87 | 96.53 | 12.80* | 75.02 m³ |
| 1:100 YR | | | 7.45 | 1.99 | 96.65 | 67.25* | |

* COMBINED VOLUME FOR CBMH02, CBMH03, CBMH04 & CBMH05.

INLET CONTROL DEVICE DATA TABLE - CBMH07

| DESIGN EVENT | ICD TYPE | DIAMETER OF OUTLET PIPE (mm) | DESIGN FLOW (L/s) | DESIGN HEAD (m) | WATER ELEVATION (m) | VOLUME (m³) | AVAILABLE STORAGE |
|--------------|---|------------------------------|-------------------|-----------------|---------------------|-------------|-------------------|
| 1:2 YR | SLIDE-IN-PLATE ICD c/w 83mmØ ORIFICE @ OUTLET PIPE INVERT | 250mmØ PVC | 17.11 | 1.45 | 96.08 | 2.10* | |
| 1:5 YR | | | 19.41 | 1.85 | 96.48 | 7.29** | 80.93 m³** |
| 1:100 YR | | | 19.80 | 1.92 | 96.55 | 22.30** | |

* VOLUME IN 450mmØ DIA. PIPE BETWEEN CBMH06 & CBMH07 (NO SURFACE PONDING).
** COMBINED VOLUME FOR CBMH06 & CBMH07.

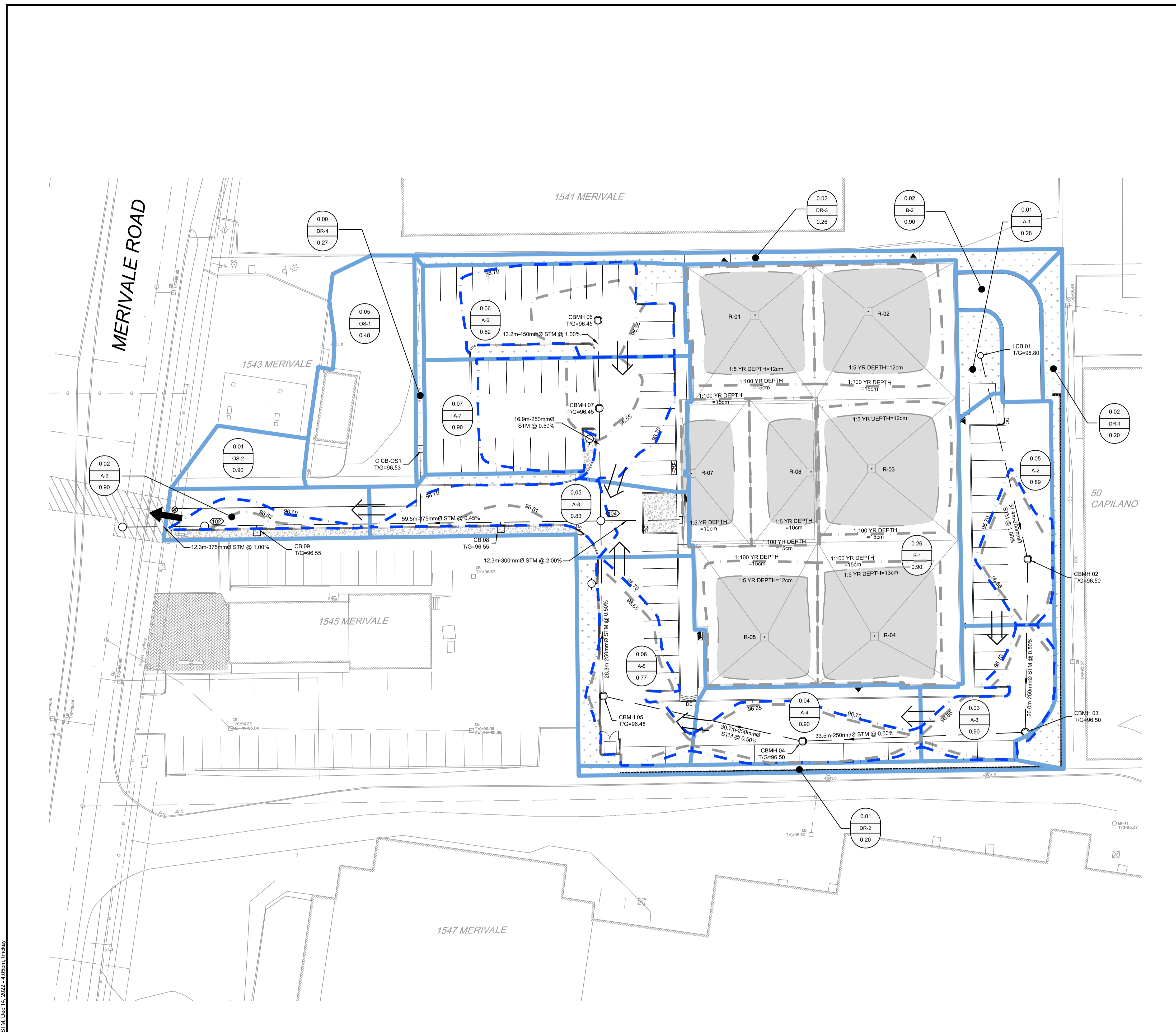
INLET CONTROL DEVICE DATA TABLE - CB08

| DESIGN EVENT | ICD TYPE | DIAMETER OF OUTLET PIPE (mm) | DESIGN FLOW (L/s) | DESIGN HEAD (m) | WATER ELEVATION (m) | VOLUME (m³) | AVAILABLE STORAGE |
|--------------|----------------------------|------------------------------|-------------------|-----------------|---------------------|-------------|-------------------|
| 1:2 YR | IPEX TEMPEST VORTEX LMF 95 | 200mmØ PVC | 7.85 | 1.05 | 96.55 | 0.00 | |
| 1:5 YR | | | 7.90 | 1.07 | 96.57 | 0.02 | 10.28 m³ |
| 1:100 YR | | | 8.09 | 1.11 | 96.61 | 0.43 | |

INLET CONTROL DEVICE DATA TABLE - CB09

| DESIGN EVENT | ICD TYPE | DIAMETER OF OUTLET PIPE (mm) | DESIGN FLOW (L/s) | DESIGN HEAD (m) | WATER ELEVATION (m) | VOLUME (m³) | AVAILABLE STORAGE |
|--------------|----------------------------|------------------------------|-------------------|-----------------|---------------------|-------------|-------------------|
| 1:2 YR | IPEX TEMPEST VORTEX LMF 85 | 200mmØ PVC | 6.61 | 1.05 | 96.55 | 0.00 | |
| 1:5 YR | | | 6.66 | 1.07 | 96.57 | 0.01 | 3.59 m³ |
| 1:100 YR | | | 6.83 | 1.12 | 96.62 | 0.38 | |

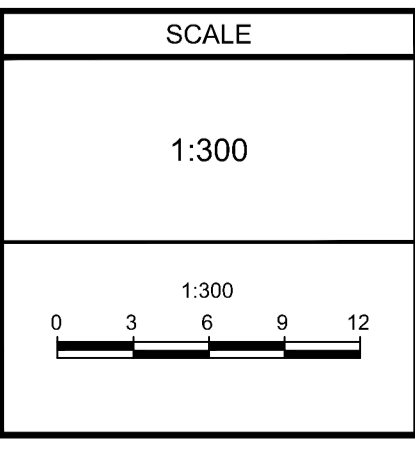
NOTE: AVAILABLE STORAGE FOR EACH CONTROLLED INLET INCLUDES ALL STRUCTURE AND PIPE STORAGE UPSTREAM OF THE INLET CONTROL DEVICE IN ADDITION TO ALL AVAILABLE STATIC PONDING ON THE SURFACE.



NOTE:
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DESIGN: SAB
CHECKED: TJM
DRAWN: SAB
CHECKED: TJM
APPROVED: MER

LICENSED PROFESSIONAL ENGINEER
M.E. RIDDELL
100040125
PROVINCE OF ONTARIO

NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6
Telephone: (613) 254-9643
Facsimile: (613) 254-5867
Website: www.novatech-eng.com

LOCATION
CITY OF OTTAWA
1545A MERIVALE ROAD

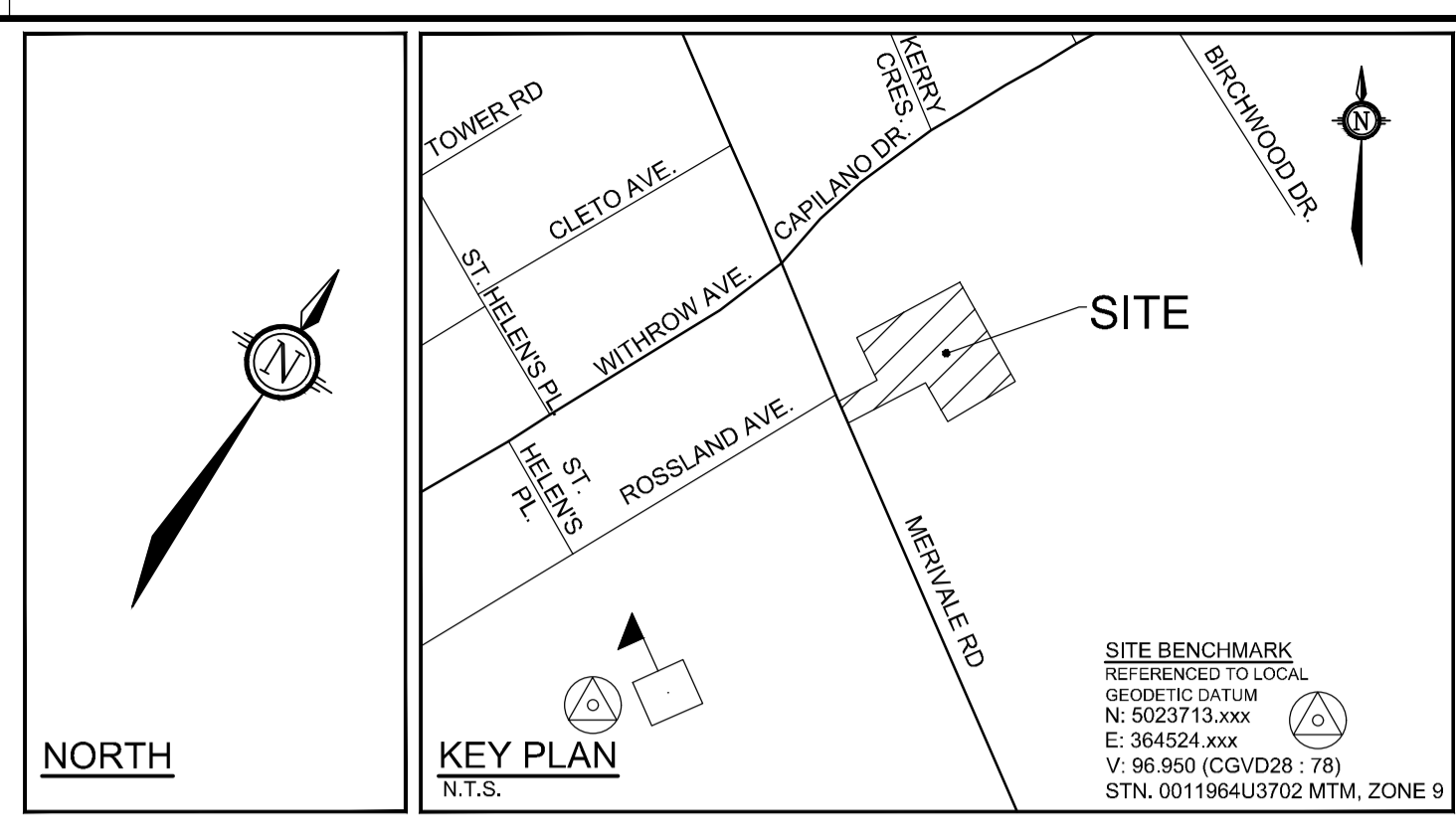
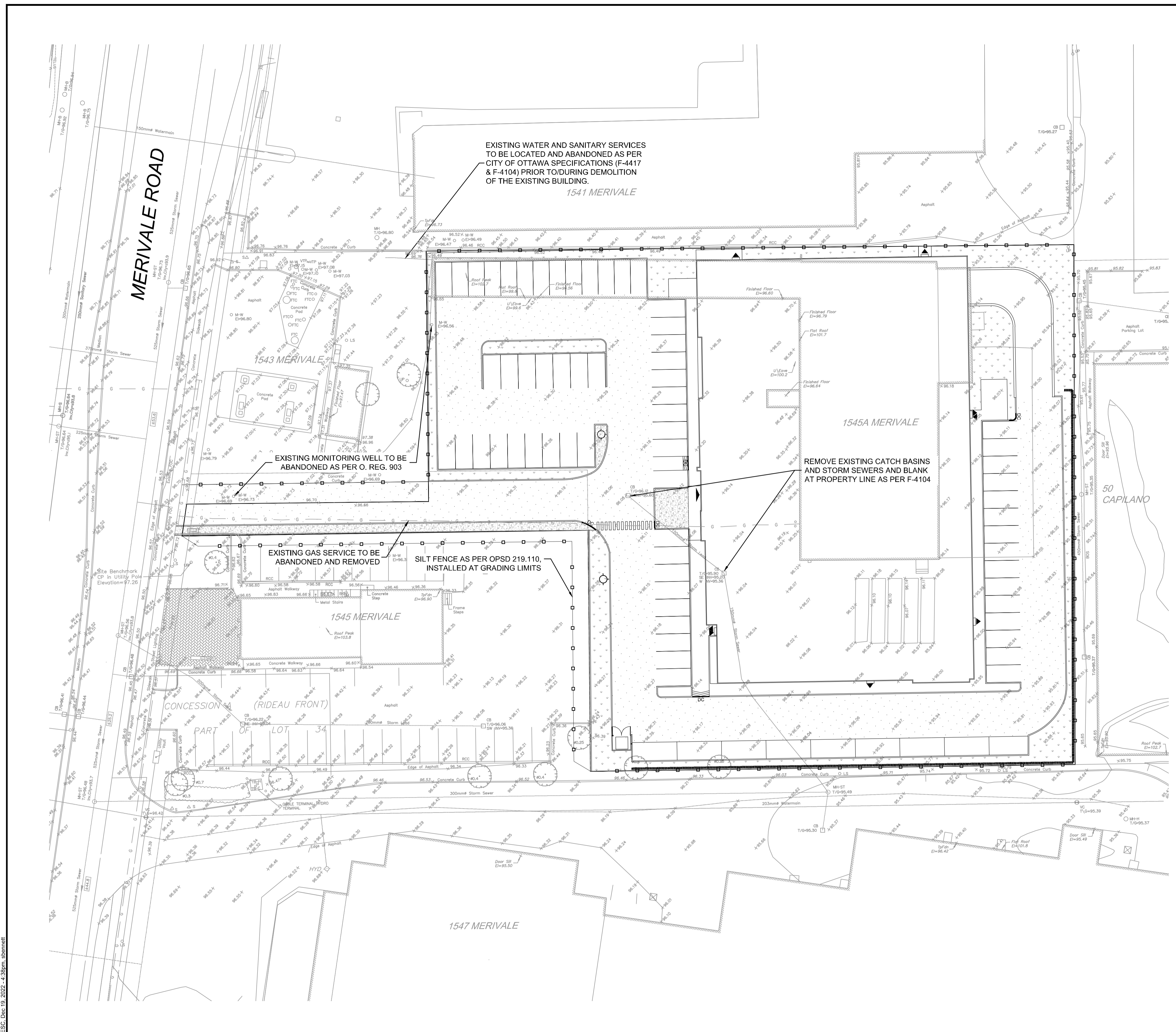
DRAWING NAME
STORM DRAINAGE AREA PLAN

PROJECT No.
122098

REV
REV # 1

DRAWING No.
122098-STM

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SITE BENCHMARK
 REFERENCED TO LOCAL
 GEODETIC DATUM
 N: 5023713.000
 E: 984524.000
 V: 96.950 (CGVD28 : 78)
 STN. 0011964U3702 MTM, ZONE 9
 N.T.S.

SOURCE REFERENCE:
 TOPOGRAPHIC PLAN OF SURVEY OF PART OF LOT 16 REGISTERED PLAN 353 AND PART OF LOT 34 CONCESSION A (RIDEAU FRONT), PREPARED BY FARLEY, SMITH & DENIS SURVEYING LTD. 2021 ON OCTOBER 1st, 2021
TOPOGRAPHIC INFORMATION:
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 2. FARLEY, SMITH & DENIS SURVEYING LTD'S TOPOGRAPHIC PLAN OF SURVEY

LEGEND

- SILTY FENCE PER OPSD 219.110
- MONITORING WELL (WITH ELEVATION)
- EXISTING CATCHBASIN (WITH TOP OF GRATE)
- EXISTING CONTOUR AND ELEVATION
- PROPOSED RETAINING WALL
- DEPRESSED CURB

EROSION AND SEDIMENT CONTROL NOTES :

1. ALL EROSION AND SEDIMENT CONTROLS ARE TO BE INSTALLED TO THE SATISFACTION OF THE ENGINEER, THE MUNICIPALITY AND THE CONSERVATION AUTHORITY. THEY ARE TO BE APPROPRIATE TO THE SITE CONDITIONS, PRIOR TO UNDERTAKING ANY SITE ALTERATIONS (FILLING, GRADING, REMOVAL OF VEGETATION, ETC.) AND DURING ALL PHASES OF SITE PREPARATION AND CONSTRUCTION. THESE PRACTICES ARE TO BE IMPLEMENTED IN ACCORDANCE WITH THE CURRENT BEST MANAGEMENT PRACTICES FOR EROSION AND SEDIMENT CONTROL AND SHOULD INCLUDE AS A MINIMUM THOSE MEASURES INDICATED ON THE PLAN.
2. TO PREVENT SURFACE EROSION FROM ENTERING THE STORM SYSTEM DURING CONSTRUCTION, SILT SOX OR SILT SACKS, OR APPROVED EQUIVALENTS, WILL BE PLACED AROUND/UNDER GRATES OF ALL PROPOSED AND EXISTING CATCHBASINS AND STRUCTURES. A LIGHT DUTY SILT FENCE BARRIER WILL ALSO BE INSTALLED IN SELECTED LOCATIONS SHOWN ON THIS PLAN, AND STRAW BALE BARRIERS WILL BE INSTALLED WITHIN THE OUTLET DITCHES. THESE CONTROL MEASURES WILL REMAIN IN PLACE UNTIL VEGETATION HAS BEEN ESTABLISHED AND CONSTRUCTION COMPLETE.
3. THE SEDIMENT CONTROL MEASURES SHALL ONLY BE REMOVED WHEN, IN THE OPINION OF THE ENGINEER, THE MEASURES ARE NO LONGER REQUIRED. NO CONTROL MEASURES MAY BE PERMANENTLY REMOVED WITHOUT PRIOR AUTHORIZATION FROM THE ENGINEER.
4. THE CONTRACTOR SHALL IMMEDIATELY REPORT TO THE ENGINEER ANY ACCIDENTAL DISCHARGES OF SEDIMENT MATERIAL INTO ANY DITCH OR STORM SEWER SYSTEM. APPROPRIATE RESPONSE MEASURES, INCLUDING ANY REPAIRS TO EXISTING CONTROL MEASURES OR THE IMPLEMENTATION OF ADDITIONAL CONTROL MEASURES, SHALL BE CARRIED OUT BY THE CONTRACTOR WITHOUT DELAY.
5. THE CONTRACTOR ACKNOWLEDGES THAT FAILURE TO IMPLEMENT EROSION AND SEDIMENT CONTROL MEASURES MAY BE SUBJECT TO PENALTIES IMPOSED BY ANY APPLICABLE REGULATORY AGENCY.
6. THE CONTRACTOR SHALL ENSURE PROPER DUST CONTROL IS PROVIDED WITH THE APPLICATION OF WATER (AND IF REQUIRED, CALCIUM CHLORIDE) DURING DRY PERIODS.
7. THE CONTRACTOR SHALL PROTECT ALL SURVEY MONUMENTS.
8. ALL TOPSOIL AND ANY SOFT, WET OR DELETERIOUS MATERIAL SHALL BE REMOVED FROM IMPROVED AREAS UNLESS OTHERWISE DIRECTED BY ENGINEER.

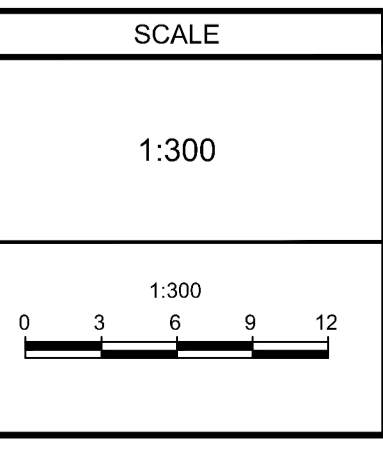
PAVEMENT STRUCTURE:

- ACCESS LANES
 - 40mm ASPHALT SP12.5
 - 50mm ASPHALT SP19.0
 - 150mm GRANULAR "A"
 - 450mm GRANULAR "B" TYPE II
 - 690mm TOTAL DEPTH
- CAR ONLY PARKING AREAS
 - 50mm ASPHALT SP12.5
 - 150mm GRANULAR "A"
 - 300mm GRANULAR "B" TYPE II
 - 500mm TOTAL DEPTH

NOTE:
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| DESIGN | JMR |
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| CHECKED | SAB |
| DRAWN | JMR |
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| | | |
|---|--|---------------------------|
| LOCATION CITY OF OTTAWA 1545A MERIVALE ROAD | | PROJECT No. 122098 |
| DRAWING NAME EROSION AND SEDIMENT CONTROL PLAN | | REV # REV # 2 |
| | | DRAWING No. 122098-ESC |

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APPENDIX G

Development Servicing Study Checklist

Development Servicing Study Checklist

| 4.1 General Content | Addressed (Y/N/NA) | Section | Comments |
|--|--------------------|-------------------------|--|
| Executive Summary (for larger reports only). | NA | | |
| Date and revision number of the report. | Y | Cover | |
| Location map and plan showing municipal address, boundary, and layout of proposed development. | Y | Fig 1-2 | |
| Plan showing the site and location of all existing services. | Y | 122098-ESC 122098-GP | Appendix F |
| Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere. | N | | Refer to Planning Rationale |
| Summary of Pre-consultation Meetings with City and other approval agencies. | Y | Appendix A | |
| Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria. | N | | No higher level plans referenced in pre-construction meeting note design criteria. |
| Statement of objectives and servicing criteria. | Y | 4.0-6.0 | |
| Identification of existing and proposed infrastructure available in the immediate area. | Y | | Figures 3 to 5 |
| Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available). | N/A | | |
| Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighboring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths. | Y | 12209-GR | Appendix F |

Development Servicing Study Checklist

| 4.1 General Content | Addressed (Y/N/NA) | Section | Comments |
|--|--------------------|-----------|-------------------------------------|
| Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts. | NA | | |
| Proposed phasing of the development, if applicable. | NA | | |
| Reference to geotechnical studies and recommendations concerning servicing. | Y | 1.1 & 7.0 | |
| All preliminary and formal site plan submissions should have the following information: | | | |
| Metric scale | Y | | |
| North arrow (including construction North) | Y | | |
| Key plan | Y | | |
| Name and contact information of applicant and property owner | Y | | |
| Property limits including bearings and dimensions | Y | | Refer to Topographic Plan of Survey |
| Existing and proposed structures and parking areas | Y | | |
| Easements, road widening and rights-of- | Y | | |
| Adjacent street names | Y | | |

Development Servicing Study Checklist

| 4.2 Water | Addressed (Y/N/NA) | Section | Comments |
|---|--------------------|---------|-------------------------|
| Confirm consistency with Master Servicing Study, if available. | N/A | | |
| Availability of public infrastructure to service proposed development. | Y | 6.1 | |
| Identification of system constraints. | Y | 6.1 | |
| Identify boundary conditions. | Y | 6.1 | And Appendix E |
| Confirmation of adequate domestic supply and pressure. | Y | 6.2 | And Appendix E |
| Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. | Y | 6.2 | And Appendix E |
| Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves. | Y | 6.2 | And Appendix E |
| Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design. | NA | | |
| Address reliability requirements such as appropriate location of shut-off valves. | Y | 6.2 | |
| Check on the necessity of a pressure zone boundary modification. | NA | | |
| Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range. | Y | 6.2 | |
| Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions. | Y | 6.2 | |
| Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation. | NA | | |
| Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines. | Y | 6.2 | |
| Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference. | Y | 6.0 | Figure 5 and Appendix E |

Development Servicing Study Checklist

| 4.3 Wastewater | Addressed (Y/N/NA) | Section | Comments |
|--|--------------------|---------|--|
| Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure). | Y | 4.0 | |
| Confirm consistency with Master Servicing Study and/or justifications for deviations. | N/A | | |
| Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers. | N/A | | |
| Description of existing sanitary sewer available for discharge of wastewater from proposed development. | Y | 5.1 | |
| Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable) | N/A | | No constraints identified in Pre-consultation meeting notes. |
| Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format. | Y | | Appendix B |
| Description of proposed sewer network including sewers, pumping stations, and forcemains. | Y | 4.0 | Figure 3 |
| Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality). | NA | | |
| Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development. | NA | | |
| Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity. | NA | | |
| Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding. | NA | | |
| Special considerations such as contamination, corrosive environment etc. | NA | | |

Development Servicing Study Checklist

| 4.4 Stormwater | Addressed (Y/N/NA) | Section | Comments |
|--|--------------------|---------|---|
| Description of drainage outlets and downstream constraints including legality of outlet (i.e. municipal drain, right-of-way, watercourse, or private property). | Y | 5.0 | |
| Analysis of the available capacity in existing public infrastructure. | NA | | No constraints identified in Pre-consultation meeting notes. Allowable release rate calculated based on |
| A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns and proposed drainage patterns. | Y | | 122098-STM & 122098-GR, Appendix F |
| Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects. | Y | 5.2 | |
| Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements. | Y | 5.2 | |
| Description of stormwater management concept with facility locations and descriptions with references and supporting information. | Y | 5.3.3 | |
| Set-back from private sewage disposal systems. | NA | | |
| Watercourse and hazard lands setbacks. | NA | | |
| Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed. | NA | | |
| Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists. | NA | | |
| Storage requirements (complete with calcs) and conveyance capacity for 5 yr and 100 yr events. | Y | 5.0 | Appendix C & D |
| Identification of watercourse within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals. | NA | | |
| Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions. | Y | 5.0 | Appendix C & D |
| Any proposed diversion of drainage catchment areas from one outlet to another. | NA | | |
| Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and SWM facilities. | Y | 5.0 | Appendix C & D |

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| 4.4 Stormwater (Continued) | Addressed (Y/N/NA) | Section | Comments |
|---|---------------------------|----------------|-----------------|
| If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event. | NA | | |
| Identification of municipal drains and related approval requirements. | NA | | |
| Description of how the conveyance and storage capacity will be achieved for the development. | Y | 5 | |
| 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading. | Y | 5 | Appendix C |
| Inclusion of hydraulic analysis including HGL elevations. | Y | 5 | Appendix C |
| Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors. | Y | 3 | |
| Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions. | NA | | |
| Identification of fill constrains related to floodplain and geotechnical investigation. | NA | | |

| 4.5 Approval and Permit Requirements | Addressed (Y/N/NA) | Section | Comments |
|--|---------------------------|----------------|-----------------|
| Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act. | NA | | |
| Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. | NA | | |
| Changes to Municipal Drains. | NA | | |
| Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) | NA | | |

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| 4.6 Conclusion | Addressed (Y/N/NA) | Section | Comments |
|---|--------------------|---------|----------|
| Clearly stated conclusions and recommendations. | Y | 8.0 | |
| Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency. | NA | | |
| All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario. | Y | 8.0 | |