

# Fastfrate

## Site Servicing and Stormwater Management Report

Fastfrate Ottawa Warehouse and Distribution Facility

Client Project Number : GA18-0631-01




CIMA+ file number: A001083  
December 15, 2021 – Revision 2

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## Executive Summary

This Site Servicing and Stormwater Management Report presents the proposed potable water, sanitary and storm servicing for the Fastfrate Ottawa Warehouse and Distribution Facility. This report will be used in support of the Site Plan Approval process.

Sanitary servicing of the site will be achieved with an on-site wastewater treatment system. This system consists of a sewer, septic tank, pumping chamber, Level IV treatment unit, shallow-buried trench system and mantle. It is anticipated that an Environmental Compliance Approval (ECA) from the MECP will be required, as the system will treat over 10,000 L/d of sanitary sewage.

Potable water will be supplied to the site by a new drinking water well, with sufficient capacity to service the intended development. Since the site is not serviced by municipal watermains, and since the proposed drinking water well will not have the capacity required to provide fire protection, the fire protection volumes will be provided from the permanent pool of the proposed stormwater management wet pond. The fire protection system consists of two (2) dry hydrants, a Siamese connection, and a building sprinkler system.

The stormwater management (SWM) for the Fastfrate site is subject to the overall SWM of the Hawthorne Industrial Park, as presented in the Hawthorne Industrial Park Stormwater Management Report (HIP SWM report), prepared by J.L. Richards & Associates, and dated May 2009. This report also demonstrates how the proposed SWM strategy conforms to the requirements of the HIP SWM report and of the regulatory authorities. Overall, the SWM strategy will be achieved with a system of ditches, culverts, and a wet pond which will provide stormwater quality and quantity control for the site.

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## Table of involved resources

In addition to the signatories of this report, the following individuals have also been involved in the study and writing of the report as technical experts within the project team:

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# 1. Introduction

CIMA+ was retained by CIVITAS & Fastfrate to prepare a Site Servicing and Stormwater Management Report for the proposed construction of a warehouse containing cross-docks and office building, at 301 Somme Street in Ottawa, Ontario.

The purpose of this assessment is to confirm that the proposed development will be serviced adequately by the proposed water supply well, septic system and stormwater management. This assessment shall be used in support of the application for Site Plan Approval.

The detailed design of sediment and erosion control measures, site servicing (storm, sanitary, water) and grading, as well as measures for the control of stormwater runoff, are considered in this report, in general accordance with the Ottawa Sewer Design Guidelines (2012), the Ottawa Design Guidelines – Water Distribution (2010) and associated Technical Bulletins.

## 1.1 Site Description and Proposed Development

The Site is located near the intersection of Rideau Road and Somme Street. The subject site is currently vacant and measures approximately 4.05 ha. The site is bounded by Somme Street to the south and west, by Rideau Road and Christie Creek to the north and by vacant land to the east. The proposed development is a 76,505 sq. ft. warehouse building with associated loading dock areas and employee parking stalls. Refer to the project drawings for the site plan of the proposed development (prepared by CIVITAS).



*Figure 1-1 : Site Location & Key Plan*

The objective of this study is to assess current site servicing conditions through the review of available background documents and to present detailed concepts, calculations, and results to provide adequate site servicing for the new building and associated parking lot.

## 1.2 Existing Infrastructure

The proposed site is part of the Hawthorne Industrial Park (HIP) which is currently serviced by roads and an existing open ditch system and SWM facility that convey stormwater and provide SWM quantity control for the entire HIP. The site is not serviced by municipal sewers or municipal watermain.

## 1.3 Summary of Applicable Background Documents

- + MOE SWM Manual (2003)
- + 2012 Ottawa Sewer Design Guidelines, as amended by technical bulletins
- + 2010 Ottawa Design Guidelines for Water Supply, as amended by technical bulletins
- + Existing Master SWM Report (prepared by J.L. Richards Associates Ltd., May 2009)
- + Hydrogeological Assessment Report (prepared by GHD, 2021)
- + Septic Assessment Report (prepared by GHD, 2021)
- + Environmental Impact Study (prepared by GHD, 2021)

### 1.3.1 STORMWATER MANAGEMENT REPORT, HAWTHORNE INDUSTRIAL PARK BY J.L. RICHARDS & ASSOCIATES LIMITED – MAY 2009.

This report addresses stormwater management within the Hawthorne Industrial Park (**Appendix A – JL Richards SWM Plan**). The contents of this report are discussed in more detail in **Section 4**.

### 1.3.2 HYDROGEOLOGICAL ASSESSMENT REPORT BY GHD, 2021.

This report addresses the hydrogeological characteristics of the site and assessing the capacity of the on-site well (GHD, 2021a).

### 1.3.3 SEPTIC ASSESSMENT REPORT BY GHD, 2021.

This report addresses the percolation rate of the site and assessing the capacity of the on-site septic system (GHD, 2021b).

### 1.3.4 ENVIRONMENTAL IMPACT STUDY BY GHD, 2021.

A scoped environmental impact study was prepared for this project. This report summarised the investigations of potential environmental impacts and required mitigation measures, & setbacks to be respected during construction of this project.

## 1.4 Consultation and Permits

In response to the pre-consultation requirements defined in the City's Development Servicing Study Checklist, the following agencies were consulted in support of the preparation of this report. The Development Servicing Study Checklist as well as all relevant correspondence with the consulted agencies can be found in **Appendix G**.

### City of Ottawa

A Pre-Application Consultation meeting was done with the City of Ottawa. The meeting discussions revolved around planning, engineering, and transportation requirements. Details of this consultation are included in **Appendix G**.

CIMA+ had a second meeting with Harry Alvey from the City of Ottawa on May 18, 2021. The discussion was mostly about SWM strategies and fire protection. Details of this consultation are included in **Appendix G**.

### South Nation Conservation Authority (SNCA)

The subject site falls under the jurisdiction of the South Nation Conservation Authority (SNCA). CIMA+ contacted James Holland from the SNCA to identify the any Natural Heritage/Hazards features that may impact the development as well as any Storm Water Management Criteria for the site and required approvals/permits. Correspondence with James Holland has been included in **Appendix G**.

### Ministry of the Environment, Conservation and Parks (MECP)

CIMA+ expects that the proposed development will require an Environmental Compliance Approval (ECA) as the development requires an on-site wastewater treatment system treating over 10,000 L/d.

It is expected that the application can be submitted directly to the MECP, and not through the City of Ottawa's Transfer of Review (ToR) Program. The correspondence with the City project manager has been provided in **Appendix G**.

## 2. Sanitary Servicing

### 2.1 Existing Conditions

The HIP and the subject site are not serviced by municipal sanitary sewers.

### 2.2 Sanitary Sewer

#### Design Criteria

The design criteria for determining the sanitary peak flow rates for the proposed development follow the parameters outlined in the City of Ottawa Sewer Design Guidelines, 2012 as amended by all applicable Technical Bulletins. Namely, the following parameters have been used in determining the peak sanitary flow rates:

*Table 2-1: Sanitary Peak Flow Determination Design Criteria*

Design Criterion	Commercial Areas
Base Flow	2.80 L/m <sup>2</sup> /day
Peaking Factor	1.5
Total Infiltration Allowance	0.33 L/s/effective gross hectare (for all areas)

#### Proposed Sanitary Peak Flows for Sanitary Sewer Sizing

The estimated peak flows from the proposed development based on the design criteria listed in **Table 2-1** are outlined in the following Table.

*Table 2-2: Peak Sanitary Flows – Sanitary Sewer Sizing*

Flow Type	Total Flow Rate (L/s)
Average Dry Weather Flow Rate	0.23
Peak Dry Weather Flow Rate	0.35
Peak Wet Weather Flow Rate	0.35

Detailed calculations for peak sanitary flows for sanitary sewer sizing are presented in **Appendix F**.

#### Sanitary Sewer Sizing

The flows indicated above will be directed from the building to the onsite wastewater disposal system through a new 200mm diameter PVC sanitary sewer. This sewer sizing is acceptable per the calculations and sewer design sheets (refer to **Appendix F**).



## 2.3 Onsite Wastewater Disposal System

### 2.3.1 Daily Design Sewage Flow

Onsite wastewater treatment systems are regulated under the Ontario Regulation 332/12, the Building Code Act (1992) (OBC), Part 8 of Division B provides the information required the design, construction, installation, operation, and maintenance of these system. The Fastfrate warehouse facility requires a Class 4 system to accept both greywater and human waste.

The proposed Fastfrate facility will be developed with a maximum of 41 loading bays and will be provided with a total of 7 water closets. The daily design sewage flow for the Fastfrate facility was calculated to be 12,800 L/d in accordance with Table 8.2.1.3.3.B of the OBC. For non-residential occupancies, the septic tank working capacity shall be three times the daily design sanitary sewage flow. Therefore, the septic tank must have a minimum working volume of 38,400 L. A summary of the daily sewage design flow calculations are provided in **Table 2-3** below.

*Table 2-3: Daily Design Sewage Flow Rate and Septic Tank Volume*

Parameter as per OBC	Volume (L) as per OBC	Design Basis for Fastfrate	Flow (L/d) <sup>(1)</sup>
<b>Warehouse</b>			
a) Per water closet, <b>and</b>	950	7	6,650
b) Per loading bay	150	41	6,150
<b>Total Daily Design Flow</b>			<b>12,800</b>
<b>Minimum Septic Tank Volume (3x the Daily Design Flow) (L)</b>			<b>38,400</b>
Notes:			
1. Column 2 x Column 3 = Column 4 (e.g., 950 L x 7 = 6,650 L/d)			

### 2.3.2 System Design

A Class 4 septic system typically consists of a septic tank and leaching bed. Depending on the system, a pumping chamber to dose the leaching bed and/or a level IV treatment unit may be required. The design of the septic system is based on the following two factors:

- + Daily sewage design flowrate
- + Percolation Time of the native soil (T-Time)

The percolation time (T-Time) of the native soil is defined as the amount of time it takes for water to travel 1 cm. Typical T-times of soils ranges from 1 to 50 minutes, with some soils up to 125 minutes. GHD limited (GHD) was retained to excavate test pits to help determine soil stratigraphy and the T-time. Five test pits were advanced to depths ranging from 2.4 to 3.4 m within the proposed septic system area and SWM pond. The soil stratigraphy consisted of fill at each location and described as gravelly sand with silt trace clay to a silty sand with gravel and clay. Fill was observed to the bottom of each test pit. Refer to GHD's septic assessment (GHD, 2021b) for more information. Groundwater seepage was encountered at each test pit and was observed between 1.8 and 2.4 m below ground surface. GHD estimated the T-time to have an average value of 12 to 20 min/cm, based upon gradation test results only. As a conservative approach, a Design T-time of 20 min/cm was selected for sizing the leaching bed for this site.

There are 5 types of leaching beds regulated in Ontario under the OBC:

1. Conventional Leaching Bed
2. Sand Filter Bed
3. Shallow Buried Trench (SBT)
4. Type A Dispersal Bed
5. Type B Dispersal Bed

For the Fastfrate site, a raised SBT leaching bed was selected as it would meet all space and site constraints. The footprint of the SBT system is smaller than a conventional absorption trench system such as a conventional leaching or sand filter bed because the soil is not relied upon for any significant portion of the treatment.

A SBT is an alternative to a conventional leaching bed and are always used in conjunction with a treatment unit capable of consistently providing effluent with 10 mg/L five-day carbonaceous biochemical oxygen demand (cBOD<sub>5</sub>) and 10 mg/L suspended solids (SS). A SBT leaching bed is a pressurized distribution system which delivers regular timed doses of effluent to small diameter laterals (typically 25 mm PVC pipe) supported inside of a plastic chamber. The laterals are perforated at regular intervals on the top of the pipe with an adequate number of orifices on the bottom to provide self-drainage to prevent freezing during cold weather. When the dosing pump starts, effluent is forced along the entire length of the lateral and sprayed upwards where it hits the chamber and trickles down into the soil. The pump is sized to account for friction losses, static losses, and a residual pressure head of at least 600 mm at the furthest point from the pump. This ensures the entire footprint of the leaching bed is utilized and provides a more efficient distribution and use of the soil absorption system. For soils with T-times of up to 50 min/cm, hourly dosing is generally sufficient to allow the ponded water in the trench to infiltrate into the soil.

## Septic Tank, Pumping Chamber & Level IV Treatment Unit Clearances

As per Section 8.2.1.6.(1), the septic tank, level IV treatment unit and the pumping chamber will meet the minimum clearances for treatment unit listed in the OBC Table 8.2.1.6.A. In addition, as per 8.7.4.0.(11), the distances set out in column 2 of Table 8.2.1.6.B. shall be increased by twice the height that the leaching bed is raised above the original grade. The current grade at the site where the septic system will be installed is 90.950 meters above sea level (m ASL). The SBT will be raised with a sand mantle below the SBT. The top of grade of the SBT at the highest elevation is 91.6 m. Therefore, the minimum clearances must be increased by 1.3m. A summary of the clearances required for the treatment units (septic system, pumping chamber, and level IV treatment unit) and the SBT leaching bed at the Fastfrate facility septic system is given in **Table 2-4** and **Table 2-5** below, respectively.

It is noted that there will be a SWM facility located east of the septic system, which will be considered as a pond for establishing minimum separation requirements.

Table 2-4: Minimum Clearances for Treatment Units

Object <sup>(1)</sup>	Treatment Units Minimum Clearance, m <sup>(1)</sup>	Additional Clearance required for the Treatment Units at Fastfrate, m <sup>(2)</sup>	Total Clearance required for the Treatment Units at Fastfrate, m <sup>(3)</sup>
Structure	1.5	1.3	2.8
Well	15	1.3	16.3
Lake	15	1.3	16.3
Pond	15	1.3	16.3
Reservoir	15	1.3	16.3
River	15	1.3	16.3
Spring	15	1.3	16.3
Stream	15	1.3	16.3
Property Line	3	1.3	4.3
Notes: 1. Columns 1 and 2 are taken from OBC Table 8.2.4.6.A 2. [SBT Top of Grade (91.6 m) - Original ground elevation (90.95 m)] x 2 = 1.3 m 3. Total Clearances required for the Treatment Units for the Fastfrate facility			

*Table 2-5: Minimum Clearances for Distribution Piping and Leaching Chambers*

Object <sup>(1)</sup>	Distribution Piping and Leaching Chambers Minimum Clearance, m <sup>(1)</sup>	Additional Clearance required for the SBT leaching bed at Fastfrate, m <sup>(2)</sup>	Total Clearance required for the SBT leaching bed at Fastfrate <sup>(3)</sup>
Structure	5	1.3	6.3
Well with a watertight casing to a depth of at least 6 m	15	1.3	16.3
Any other well	30	1.3	31.3
Lake	15	1.3	16.3
Pond	15	1.3	16.3
Reservoir	15	1.3	16.3
River	15	1.3	16.3
Spring not used as a source of potable water	15	1.3	16.3
Stream	15	1.3	16.3
Property Line	3	1.3	4.3
Notes: 1. Columns 1 and 2 is taken from OBC Table 8.2.4.6.B 2. [SBT Top of Grade (91.6 m) - Original ground elevation (90.95 m)] x 2 = 1.3 m 3. Total Clearances required for the Treatment Units for the Fastfrate facility			

### Pumping Chamber

In accordance with sentence 8.7.6.1(3) of the OBC, the pump chamber should have a volume between 50% and 75% of the daily design capacity is recommended. Therefore, it is recommended the pump chamber have a minimum working capacity of 19,200 L.

### Submersible Pumps

Wastewater will flow by gravity to the septic tank, and then by gravity to the pumping chamber. The discharge from the pumping chamber and the rest of the system will be pressurized and require submersible pumps. Submersible, readily available and replaceable pumps are wired and rated for an effluent with 3 mm to 20 mm solids handling capacity. An alternating duplex pump configuration is recommended to allow time for service in the event of a pump failure. The specified pump must have a capacity equal to or greater than the calculated maximum pressure requirement as per the SBT design at the design flow. Five submersible pumps will be required:

- + Two pumps for the pumping chamber discharge which will operate in a duty / standby configuration with rotation on stop, time, and failure
- + Two pumps for the level IV treatment discharge which will operate in a duty / standby configuration with rotation on stop, time, and failure
- + One pump for the level IV treatment discharge that will recycle effluent upstream of the septic tank.



The submersible pumps will be provided by the level IV treatment unit supplier, Waterloo Biofilter. Waterloo Biofilter typically specifies Little Giant WS Effluent Series submersible pumps. As per item 8.6.1.3.(4), when a pump or siphon is required the pump or siphon shall be designed to discharge a dose of at least 75% of the internal volume of the distribution pipe within a time period not exceeding fifteen minutes. Therefore, the volume required to dose 75% of 175 m of 50 mm diameter schedule 40 PVC pipe is approximately 64.5 L within 15 minutes, or a required pump flow rate of 4.30 L/min (0.07 L/s). Sentence 8.7.6.1.(2) requires residual pressure (minimum 600 mm as per sentence 8.7.6.1.(2) at the furthest lateral) to ensure the entire bed is dosed.

The Little Giant WS Effluent Series provides include submersible pumps capable of dosing 1.70 L/s to 9.5 L/s, depending on the model. With a minimum flow rate of 0.07 L/s, the Little Giant submersible pumps will provide more than the minimum required dosing flowrate. There are several Little Giant WS Effluent Series submersible pump models. The Hazen William formula was used to calculate the theoretical total dynamic head (TDH) in meters of each of the three pumping scenarios and plotted against the different Little Giant submersible pump curves to find the theoretical operating flowrate. A summary of the results in listed in Table 2-6 below. Refer to **Appendix E** for the pump system curves and calculations.

Table 2-6: Theoretical Pumping Flow Rates

System	Recommended Pump Model	Theoretical Operating Point
Pumping Chamber Discharge	WS50HM-12-20	3.2 L/s at 12.8 m TDH
Level IV Treatment Discharge to SBT	WS100HM-12-20	2.2 L/s at 23.8 m TDH
Level IV Treatment Discharge Recycle Line	WS50M-20	5.7 L/s at 3.1 m TDH

### Level IV Treatment Unit

A Level IV Treatment is required for SBT type leaching beds. The Waterloo Biofilter level IV treatment unit will be designed to meet the level IV treatment effluent requirements of 10 mg/L for both SS and cBOD<sub>5</sub>, as listed in Table 2-7 (adapted from OBC Table 8.6.2.2.).

Table 2-7: OBC Treatment Unit Levels and Required Effluent Concentrations

Item	Column 1 Classification of Treatment Unit <sup>(1)</sup>	Column 2 Suspended Solids <sup>(2)</sup>	Column 3 CBOD <sub>5</sub> <sup>(2)</sup>
1.	Level II	30	25
2.	Level III	15	15
3.	Level IV	10	10
Notes: 1. The classifications of <i>treatment units</i> specified in Column 1 correspond to the levels of treatment described in CAN/BNQ 3680-600, "Onsite Residential Wastewater Treatment Technologies". 2. Maximum concentration in mg/L based on a 30-day average.			

The level IV treatment unit must be certified to CAN/BNQ 3680-600 “Onsite Residential Water Treatment Technologies”. The treatment units installed in Ontario typically either use aeration or a filter media to provide treatment. Aeration treatment units have higher operation and maintenance costs and effort as blowers are required in addition to pumps. Filter media type treatment units do not require blowers and require the filter media to be replaced approximately every 10+ years or to the manufacturer’s recommendation. A filter media type level IV treatment unit such as a Waterloo Biofilter is recommended for this application. The sanitary waste from the warehouse will flow by gravity to the septic tank, where settling will occur, and the effluent will flow by gravity to a pumping chamber. The pumping chamber will consist of 2 pumps (duty/standby configuration with frequent rotation via an alternating timer), which will pump the effluent to the level IV treatment unit to evenly dose the filter media. The filtered water will then be either pumped to the shallow buried trench by one of two pumps (duty / standby configuration with frequent rotation on an alternating timer) or recycled to the inlet of the septic tank by a third dedicated pump. All pumps will be controlled and monitored by a common control panel for remote monitoring, control, and data logging over a stable cellular network to Waterloo Biofilter who will contact personnel from the Fastfrate facility. Alarms include high water, float failure and pump failure from the Waterloo Smart Panel. A flow schematic of the system is given in **Figure 2-1** below.

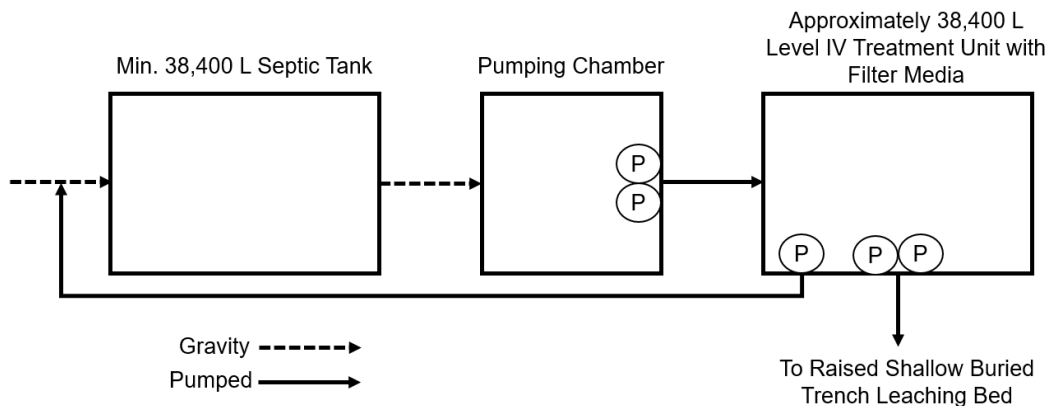


Figure 2-1: Septic System Process Flow Schematic

## Shallow Buried Trench Leaching Bed

Due to the shallow groundwater seepage observed at 1.8 to 2.4 m below the surface and the requirement that the bottom of the leaching bed must be a minimum of 900 mm above the top of the high ground water table, the leaching bed must be raised. Due to the size constraint of the system, a SBT with a sand mantle is recommended. The sand mantle will be approximately 15 m in total length with the last 3 meters of the mantle changing direction slightly more north-west than the first 12 m of the mantle. Even with the irregular shape of the mantle, effluent will flow through the mantle as the T-time of the sand mantle will be imported sand fill with a percolation rate of 6 to 10 minutes/cm and have a maximum 5% if fines passing through a No. 200 sieve.

The length of the SBT distribution pipe laterals is calculated based on the T-time and the Table 8.7.3.1 in the OBC. The percolation tests of the native soil in the area of the proposed septic bed yield 12 to 20 minutes/cm according to the GHD report. As per Table 8.7.3.1 in the OBC, a percolation between 1 to 20 minutes/cm corresponds to the following formula to calculate the length of distribution pipe required:

$$L = \frac{Q}{75}$$

Where:

$L$  = The length of distribution pipe in m

$Q$  = Total Daily Design Flow Rate (12,800 L/d for the Fastfrate Facility)

Therefore, the SBT must have a minimum distribution pipe length of 171 m (rounded up to the nearest meter). The OBC stipulates the maximum length of a SBT distribution run is 30 m as specified in clause 8.7.3.2(2)(a). To accommodate the clearances for the SWM pond and property line, 7 distribution pipe runs of 25 m (175 m total) is recommended.

Each lateral shall include a test port at the end of each line. Each test port will have a long radius sweep bend at the end, equipped with a normally closed ball valve and a removal plug with a drilled orifice the same diameter as the lateral spray orifices. The test ports are intended to allow individual line squirt testing and testing of all lines at once. The plugs will be removable to allow line flushing and cleaning as necessary.

The spray orifice size is important in the flow/pressure calculation, and it is recommended that 3 mm sizing be used as a default. OOWA best practices recommends orifices are spaced between 0.6 to 1.2 m along the lateral for even distribution of effluent. The orifices for the Fastfrate facility are specified to be spaced 0.6 m apart.

In addition to the spray orifices, drain orifices are recommended to be evenly spaced, facing downward, on each lateral to allow for drain-out and prevent freezing between pump cycles. It is recommended to have a drain orifice every 2 to 4 spray orifices, offset from the spray orifices and having orifice shields installed to prevent erosion of the trench base. The drain orifices will be spaced every 3 m apart and will be offset from the spray orifices.

OOWA Best Practices recommends the manifold should be at least one trade size larger than the laterals, typically between 32 mm (1.25" nominal) and 50 mm (2" nominal). The distribution laterals will be 25 mm diameter Schedule 40 PVC, and the manifold will be 50 mm diameter Schedule 40 PVC. Each lateral will include a ball valve for isolation and a 50 mm to 25 mm reducer. The components of the SBT leaching bed are given in the section below.

Fill will be required for the raised SBT system. The contact area at the base of the fill system was carefully considered. The contact area between the fill and the native receiving soils is important in order to safely transition treated effluent from the fill to the native soils without causing environmental risks. Due to inconsistent native soil type at the site and as a precaution, a sand mantle is recommended.

The mantle for the Fastfrate septic system was designed according to Option 2 of the Ontario Onsite Wastewater Association (OOWA) Best Practices: Shallow Buried Trench Guidance Document:

The contact area between the native soils and the fill material is which the SBT bed and mantle area should be at least equal to the following formula:

$$A = \frac{Q \times T}{850}$$

Where:

$A$  = Contact Area ( $m^2$ )

$T$  = The T-time of the receiving soils (a conservative T-time of 20 minutes/cm was used)

$Q$  = Total Daily Design Flow Rate (12,800 L/d for the Fastfrate facility)

Therefore, the minimum recommended mantle area is 302  $m^2$ . The total mantle surface area provided (extended and beneath the SBT) has an approximate contact surface area of 660  $m^2$  and is over double the minimum surface area as calculated by the OOWA Best Practices.

Each lateral shall include a test port at the end of each line this may be an individual access port at the end of each lateral. Each test port will have a long radius sweep bend at each test port equipped with a normal closed ball valve and a removal plug with a drilled orifice the same diameter as the lateral spray orifices. The test ports are intended to allow individual line squirt testing and testing of all lines at once. The plugs will be removable to allow line flushing and cleaning as necessary.

The orifice size is important in the flow/pressure calculation, and it is recommended that 3 mm sizing be used as a default. OOWA Best Practices recommends orifices are spaced between 0.6 to 1.2 m along the later for even distribution of effluent. The orifices for the Fastfrate facility septic system are specified to be spaced 0.6 m apart.

The drain orifices are evenly spaced, facing downward, on each lateral to allow for drain-out and prevent freezing during pump cycles. It is recommended to have a drain orifice every 2 to 4 spray orifices, offset from the spray orifices and having orifice shields installed to prevent erosion of the trench base. The drain orifices will be spaced every 3 m apart and will be offset from the spray orifices.



OOWA Best Practices recommends the manifold should be at least one trade size larger than the laterals, typically between 32 mm (1.25" nominal) and 50 mm (2" nominal). The distribution laterals will be 25 mm diameter Schedule 40 PVC pipe, and the manifold will be 50 mm diameter Schedule 40 PVC pipe. Each lateral will include a ball valve for isolation and a 50 mm to 25 mm reducer. To summarize, the components of the SBT system for the Fastfrate facility include:

- + Treatment Unit certified to Level IV CAN/BNQ 3680-600 "Onsite Residential Wastewater Treatment Technologies"
- + Dosing pump chamber and pumps equipped with timer controls.
- + Forcemain from dosing chamber to distribution manifold which typically is PVC schedule 40
- + Manifold (header) assembly, consisting of 50 mm (2") pressure pipe (PVC Schedule 40)
- + Laterals in the leaching bed consisting of 25 mm (1") pressure pipe (PVC Schedule 40) with 3 mm orifice holes spaced evenly along the top of the pipe and 3 mm drain holes on the bottom
- + Pipe support to keep the lateral off the bottom of the trench
- + Leaching chamber covering the laterals. Large diameter pipe cut in half is not acceptable, as the footprint of the sidewalls is not sufficient to prevent settling of the chambers over time. Chambers with a wide resting foot are preferred.
- + Filter cloth over the chambers
- + "Sweep 90" fitting extending within 10 cm of the finished grade at the end of each lateral. The vertical piece may be equipped with a ball valve if desired, and terminate with a threaded cap.

### Ground Water Elevation and Native Fill

The septic, pump chamber, and level IV treatment unit tanks will require to be wrapped in a waterproof material to prevent groundwater infiltration. Due to the inconsistency of the fill material observed and the shallow groundwater seepage encountered by GHD, the leaching bed will be required to be raised. The 100-year flood elevation is 90.1 m ASL, therefore the SBT leaching bed and sand mantle have been designed to be above this elevation as not to flood out the septic system during a 100-year storm event. It is recommended prior to placement of the imported fill that any surficial organics are to be removed from the tile bed and mantle area. Additionally, the existing fill material is recommended to be compacted to ensure uneven settlement does not occur.

## 2.4 Sanitary Servicing Summary and Conclusions

The sanitary servicing design for the proposed development conforms to the requirements of the City of Ottawa Sewer Design Guidelines, 2012, as amended by all applicable Technical Bulletins.

The on-site wastewater disposal system (Septic Tank, Level IV treatment unit and shallow-buried trench system) conform to the requirements of the Ontario Building Code part 8. However, due to the Total Daily Design Sewage Flow being >10,000L, and ECA from the MECP will be required for this system.

### 3. Potable Water Servicing

#### 3.1 Existing Conditions

The site is currently undeveloped and is not serviced by municipal watermain. As such potable water for this site will be provided by a groundwater supply well. Refer to the GHD's Hydrogeological Assessment (GHD, 2021a) for more information.

#### 3.2 Building Water Demands (Domestic and Fire Protection)

##### 3.2.1 Potable Water Quantity Requirements

Based on design flows from the OBC, the average daily water use for the facility is **8.9 L/min (Table 3-1)**. Considering a peak demand of 35.6 L/min (average demand \* 4), the well discharge of 60 L/min in the Hydrogeological Report will sufficiently meet the water demand requirements of the facility.

*Table 3-1 Potable Water Design Flows*

Parameter as per OBC	Volume (L) as per OBC	Design Basis for Fastfrate	Flow (L/d) <sup>(1)</sup>
<b>Warehouse</b>			
a) Per water closet, and	950	7	6,650
b) Per loading bay	150	41	6,150
<b>Total Daily Design Flow</b>			<b>12,800</b>
Notes:			
1. Column 2 x Column 3 = Column 4 (e.g., 950 L x 7 = 6,650 L/d)			

Water demands were also determined per the City of Ottawa Design Guidelines for comparison purposes. The peak water demand obtained using this method is **0.62 L/s (37.2 L/min)**. This value is also within well discharge capacity. (**Table 3-2**).

*Table 3-2 Potable Water Design Flows – City of Ottawa Design Guidelines*

Demand Type	Average Daily Demand (L/s)	Maximum Daily Demand (L/s)	Maximum (Peak) Hour Demand (L/s)
Residential	0.00	0.00	0.00
Commercial	0.23	0.35	0.62
<b>Total</b>	<b>0.23</b>	<b>0.35</b>	<b>0.62</b>

### 3.2.2 Fire Protection Quantity Requirements

The facility is not connected to a municipal water supply and will therefore require other means of fire protection. The fire protection volumes to be provided and a description of the proposed fire protection system are presented in this section.

#### 3.2.2.1 Fire Protection Volume – Building Mechanical Fire Protection Requirements

The required volume of water available for fire protection shall be calculated based on NFPA13 requirements:

$$\left[ \left( 0.2 \frac{\text{gpm}}{\text{ft}^2} \right) * (1500 \text{ ft}^2) + 250 \text{ gpm} \right] * 60 \text{ min} = 33,000 \text{ US Gal.} = \sim 123.9 \text{ m}^3$$

Where:

250gpm = Hose Allowance Requirement (NFPA13)

60min = Duration Requirement (NFPA13)

#### 3.2.2.2 Fire Protection Volume – FUS requirements

The FUS method was used to determine the Fire Protection Volume required for this site.

The resulting fire protection volume required is of 480 m<sup>3</sup>, for 1 hr of fire protection @ 8000 L/min (**Appendix D**).

#### 3.2.2.3 Fire Protection System

The proposed SWM wet pond shall be used for storing water for fire protection. Refer to **Section 4.5** for more information on the design of the proposed SWM pond.

A fire pump located in a 2-hour fire rated mechanical room in the building shall serve the Fire Protection system. The fire pump inlet shall be connected to an 8m deep sump, to be hydraulically connected to the pond via an intake pipe at the base of Pond.

To ensure that the fire protection volumes are adequate during winter conditions, the maximum ice thickness on the permanent pool of the SWM wet pond was determined based the Annual Freezing Degree Days method. Based on an Ice cover condition coefficient of 2.4 and the Annual Freezing Degree Days value 785 °C-day for 2019, the ice thickness of 67.24 cm was obtained. Based on this calculation, the design ice thickness used is of 69 cm. Detailed calculations are presented in **Appendix D**.

In the permanent pool of the proposed SWM pond, fire protection volumes of 520.3 m<sup>3</sup> and 987.9 m<sup>3</sup> with and without ice cover respectively. These volumes satisfy the FUS and NFPA 13 requirements, and will supply the building fire protection intake, and two (2) dry hydrants.

A free-standing Siamese connection will be located outside the front entrance and would be used to supply the sprinkler system if the pump within the shaft were unable to draw water from the fire protection pond.

The large volume provided in the permanent pool is required to satisfy the minimum depth of water above the building fire protection and dry hydrant intakes, per City of Ottawa detail W53.

To prevent exfiltration and maintain the water level of the permanent pool, the SWM pond will be constructed with a liner. In the event the water level in the sump & pond drops below the minimum level, makeup water will be provided to the sump and pond from the well to mitigate losses due to infiltration and evaporation. Alarm indicators will monitor the levels in the sump & pond, and will control the supply of makeup water to the pond and sump from the well.

The building fire protection system requires 250 US gal. per minute (15.8 L/s) per NFPA 13. As such, the building fire protection intake was sized as a 300mm pipe, slopes at 0.1% with a capacity of 33 L/s under gravity free flow conditions (Factor of safety = 1.90). An intake screen capacity of 64 L/s is also specified for the building fire protection intake (Factor of safety = 4.05).

### 3.3 Proposed Water Supply Well

#### 3.3.1 Well Quality

Samples tested from an existing water supply well confirmed that there were no health-related parameters in exceedance of the Ontario Drinking Water Standards (ODWS). There were several parameters that exceeded their respective ODWS for aesthetic objectives including hardness, total dissolved solids, turbidity, manganese, and iron. These parameters will require commercially available treatment equipment (for example a water softener for treatment of hardness). The treatment systems will be determined later in the design process. A detailed breakdown of test results is presented in GHD's Hydrogeological Assessment (GHD, 2021a).

As a proactive measure, it is recommended that bacteriological treatment (i.e., ultraviolet treatment) be used at a minimum. It is anticipated that the well system will be regulated and will require treatment to meet appropriate standards to ensure potable water is available to employees and visitors. A water treatment specialist should be retained for treatment and a qualified engineer should review the final treatment system before use.

#### 3.3.2 Well Quantity

The water supply well referred to as TW-2 in the Hydrogeological Assessment is capable of providing long-term quantities of groundwater at a pumping rate of 60 L/min based upon the pumping test completed (GHD, 2021a). After 6 hours of pumping, the well drawdown was 1.15 m with 23.9 m of available drawdown remaining. A total of 21,600 L was pumped from the well during the testing.

Based upon the septic total daily design values of 12,800 L/day, the well exceeds the daily design quantities estimated. The actual water volume required for the development on a daily basis is expected to be much less than 10,000 L/day. The water supply well and the aquifer that it is drilled into can safely provide the long-term quantities required for this development based upon the testing completed without significant interference to future and existing neighbouring wells.

### 3.4 Conclusion – Potable Water Servicing

The proposed well will provide sufficient potable water supply for the development, while the proposed SWM pond permanent pool will provide sufficient fire protection volume for the development.

## 4. Storm Water Management

### 4.1 Background

As previously mentioned, the subject site is currently vacant and is part of the Hawthorne Industrial Park (HIP). The site is generally flat and slopes towards the North-East corner before it reaches the 6m tall embankment and reaches Christie Creek on Rideau Road. There is a fill layer of approx. 6m thick across most of the site.

The HIP sector and the Fastfrate site are subject to the HIP Stormwater Management Report and associated drawings (**Appendix A**), developed by J.L. Richards and dated May 2009. This report established the Stormwater Management design for the HIP, which was then used as the design basis for the roads, open ditch system, and HIP SWM facility (refer to Drawings issued for MOE Approval; **Appendix A**).

The HIP SWM facility, located east of the industrial site, only provides stormwater quantity control for the HIP sector. The HIP SWM facility controls storm events up to the 2 - year post-development peak flow to 50% of the 2-year pre-development peak flow; and controls post-development peak flows to pre-development levels for storm events ranging from the 2-year to the 100-year recurrence. The HIP SWM report specifies that individual parcels of the HIP must provide stormwater quality control.

### 4.2 Stormwater Management Strategy

#### 4.2.1 Deviations from the HIP SWM Report & Drainage Plan

The proposed SWM strategy for this site deviates from that of the HIP SWM report.

The drainage plan for the HIP divides the drainage of the Fastfrate site between two outlets. Part of the site drains to Christie Creek while the remainder drains to the HIP SWM facility via the open ditch system along Somme Street. (**Figure 4-1**).

To simplify the SWM strategy the drainage distribution between both outlets has been altered from what was presented in the HIP SWM report, redirecting more runoff towards the HIP SWM facility (**Figure 4-1**). This simplifies the site grading and allows all quality control measures to be in a single location. Therefore, the proposed conditions require quantity control (through on-site retention) to respect the allowable release flowrates up to the 100-year storm stipulated in the HIP SWM report.

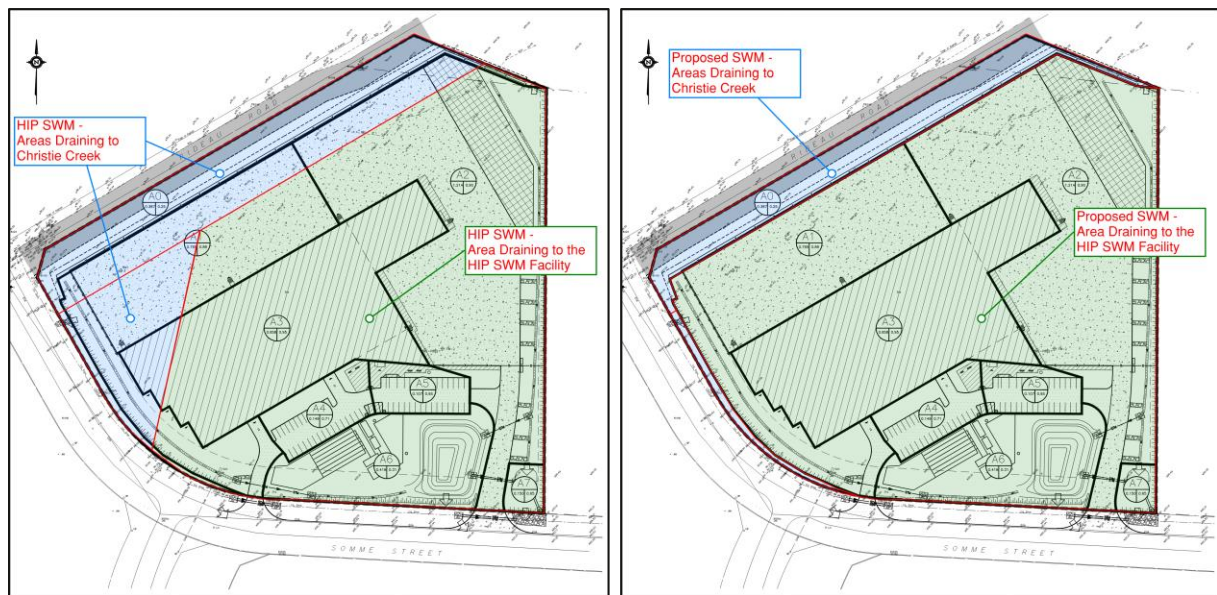


Figure 4-1 SWM Drainage Area from HIP SWM (left), and from Proposed SWM (right)

The original drainage plans and sewer design sheets for the HIP sector, as well as the proposed SWM plan for the Fastfrate site are provided in **Appendix B**.

#### 4.2.2 Allowable Post Development Flow Rates

The allowable release rate was determined based on parameters of the HIP SWM report, Sewer Design sheets and SWM plans as summarized in **Table 4-1**.

Table 4-1: Post-development Allowable 100-year Release Flows – HIP SWM Facility

Catchment ID	Catchment area (ha)	Runoff Coefficient (factored)	Time of Concentration (minutes)	Rainfall Intensity (mm/hr)	Release Rate (L/s)
Fastfrate Site – HIP SWM Report	3.06	0.88	19.43	122.15	906.87

Based on this calculation, the storm runoff under post-development conditions for the site area draining to the HIP SWM facility must be controlled to the allowable release rate of **906.9 L/s**, up to and including the 100-year storm event.

Using this allowable release rate, the resulting unit release rates (as L/s/ha) were determined for the Fastfrate site, assuming an identical time of concentration for the proposed Site SWM (Table 4-2; **Appendix C, pages 2- 4**).



Table 4-2: Post-development Allowable 100-year Release Rates – HIP SWM Facility

Catchment ID	Catchment area (ha)	Runoff Coefficient (factored)	Allowable Release Flow – 100-year (L/s)	Allowable Release Rate – 100-year (L/s/ha)
Fastfrate Site – HIP SWM Report	3.06	0.88	906.9*	296.89
Fastfrate Site – Proposed SWM	3.66	0.88	906.9	247.78

### 4.3 Design Criteria and Assumptions

- + Quality control requirements: 80% TSS Removal must be provided for our site as required by the South Nation Conservation Authority (SNCA).
- + Per the HIP SWM report, the existing open ditch system is designed to the 100-year event, and the existing culverts are designed to the 10-year event.
- + The current site plan deviates from the HIP SWM report. To conform with the original SWM, the 100-year allowable release rate to the SWM facility must remain at 906.9 L/s (refer to **Section 4.2.2**).

### 4.4 Proposed Storm Servicing

All detailed SWM calculations are presented in **Appendix C**.

#### 4.4.1 Stormwater Quality Control

As specified in the HIP SWM report, the HIP SWM facility was not designed to provide quality control. It was anticipated that each individual parcel was to provide its own quality control and achieve the normal level of protection (70% TSS Removal).

Through consultation with the South Nation Conservation Authority (SNCA, refer to **Appendix G**) the quality control requirements for the HIP parcels have been revised to the enhanced level of protection (80% TSS removal).

The portion of the site that naturally drains into Christie Creek will not require quality treatment since this area will remain undeveloped and vegetated. Therefore, only the developed portion of the site draining towards the Somme Street ditches and to the existing HIP SWM facility will be treated for quality.

The quality control requirements will be achieved using a combination of grassed swales and a wet pond, operating as a “treatment train”. The grassed swales, which are sloped to promote infiltration and low channel velocities (<0.5 m/s) will provide the required pre-treatment for the wet pond.

The wet pond was designed based on the volumetric water quality criteria, as presented in Table 3.2 of the MECP SWM guidelines (2003). The wet pond requires a total water Quality Storage of 824m<sup>3</sup>. In the pond dimensioning, at least 677 m<sup>3</sup> will be provided in the permanent pool and at least 146m<sup>3</sup> will be provided as extended detention (**Table 4-3**).



For this facility, the extended detention volume will be retained for a period of 12 hours, as per the MECP SWM Guidelines on wet ponds with < 8 ha of drainage area.

*Table 4-3: Wet Pond Volume Calculations – 70% Impervious; 80% TSS Removal*

Control Area	Storage Volume (m <sup>3</sup> /ha)	Catchment Area (ha)	Required Storage Volume (m <sup>3</sup> )
Permanent Pool	185	3.66	677.1
Extended Detention	40		146.4
Total	225	3.66	823.5

#### 4.4.2 Stormwater Quantity Control

The anticipated post-development flow rates and required storage when controlled to the allowable post-development release rate are summarized in the table below.

*Table 4-4: Post-development Flowrate and Storage Summary*

Control Area	100-year Release Rate (L/s)	Available Storage Volume (m <sup>3</sup> )	100-year Storage Volume (m <sup>3</sup> )
Roof Areas	212.6	137.4	115.1
SWM Pond	906.9	729.2	280.51
Total	906.9	866.6	395.61

For the warehouse and office building, the proposed release rate for roof runoff is **212.6 L/s**. This release rate generates **115 m<sup>3</sup>** of roof storage. This value is conservative with respect to the maximum available (**Table 4-4**).

To restrict stormwater discharge to the allowable release rate of **906.9 L/s**, a storage volume of **281 m<sup>3</sup>** is proposed in the SWM pond and a storage volume of **115 m<sup>3</sup>** is proposed on roofs for a total of **396 m<sup>3</sup>** (**Table 4-4**). These volumes do not account for surface storage within swales, storm sewers, and culvert sections. Refer to **Appendix C** for detailed stormwater storage calculations.

The proposed SWM system will be equipped with a backflow preventer and enough storage capacity on site to ensure the site SWM is not overwhelmed in the event of prolonged surcharging of the receiving open ditch system during the 100-year event.

#### 4.4.3 Municipal Ditch and Culverts

The two entrances to the site cross the existing open ditch system and require installation of culverts. The sizing of the culverts was determined with consideration of the upstream municipal culverts since the SWM system outlet for stormwater is situated downstream of these culverts. Culvert sizing suitability calculations can be found in **Appendix C**.

#### 4.4.4 Site Ditches and Culverts

The site's swales and culverts were sized based on capacity to convey the 100-year peak flow under free flow conditions of the site's storm outlet. Culverts were sized using a constant tailwater elevation.

*Table 4-5: Culvert Sizing Summary*

Culvert	Size	Q (L/s)	HW/D	HW elevation	TW elevation
East Ditch	1x CSPA 910x660	405	1.13	90.160	89.800
West Ditch	1x CSPA 910x660	231	0.93	90.09	89.800
STM Pond Transfer Culvert	2x CSPA 1030x740	907	0.81	89.820	89.510

Detailed calculations supporting the culvert sizing are available under **Appendix C**.

#### 4.4.5 Building Service Connection

A 600 mm storm sewer service connection will be provided on the south side of the proposed building and will be directed towards the SWM pond. The storm sewer will convey controlled runoff from the roof and uncontrolled runoff from catchments A4 and A5 (refer to **Appendix B – SWM plan**).

#### 4.4.6 Deviations from the Sewer Design Guidelines – Swale Minimum Slope

The slope of the swales conveying stormwater for this site are inferior to the minimum slope specified in section 6.4.1 of the Sewer Design guidelines.

The grassed swales are intended to contribute to runoff quality control, operating with the proposed wet pond as a “treatment train”. The reduced slope of grassed swales promotes infiltration and low channel velocities ( $<0.5$  m/s). This improves the effectiveness of grassed swales for runoff quality control (LID SWM Planning and Design Manual).

Based on the interpretation from percolation tests for this site, the soil infiltration rate can be estimated to range between 30 to 50mm/hr. With dry swales, an underdrain is typically recommended if the soil infiltration rate is  $<15$  mm/hr.

As such, the risk of prolonged ponding of water in the ditches is mitigated by the soil infiltration rate and presence of on-site existing fill and well draining soil.

## 4.5 Proposed SWM Pond Sizing

A summary of the required volumes to be provided in the Wet Pond is presented in the table below.

*Table 4-6: Summary of Required SWM Pond Volumes*

Parameter	Required Volume (m <sup>3</sup> )	Source
Retention Volume	280.51	Table 4-4
Extended Detention	146.4	Table 4-3
Fire Protection Volume	480	Section 3.2.2.2
Permanent Pool for Quality Control	677.1	Table 4-3
Sediment Accumulation Volume (25 years)	208	Section 4.6.1

A summary table of the pond volumes is presented below (**Table 4-7**).

*Table 4-7: Summary of Provided SWM Pond Volumes*

Control Volumes			Bottom Elevation	Top Elevation	Depth	Provided Volume	Required Volume
			(m ASL)	(m ASL)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )
Freeboard to Overflow			90.100	90.150	0.050	50.2	-
Retention Volume			89.500	90.100	0.60	560.10	280.51
Extended Detention			89.300	89.500	0.200	169.1	146.4
Permanent Pool (PP)	Fire Protection Volume	With Ice Cover	87.700	88.610	0.690	520.3	480
		Normal	87.700	89.300	1.60	987.9	
	Depth of Fire Protection Intake		87.100	87.700	0.600	243.4	-
	Sediment Accumulation Volume		86.100	87.100	1.0	229.9	205
	Total PP Volume		86.100	89.300	3.2	1510	677.1

## 4.6 Calculations

### 4.6.1 Sediment Accumulation Volume

Based on the MECP SWM planning and design guidelines, a conservative estimate of the sediment accumulation volume required for a duration of 25 years is 205 m<sup>3</sup> assuming an annual TSS loading of 2.84 m<sup>3</sup>/ha/year and a removal efficiency of 80%.

### 4.6.2 Pond Controls

As defined in the City of Ottawa Sewer Design Guidelines (2012), the Rational Method is a valid approach to determination of peak flows and pipe capacity for drainage areas of less than 40 ha in size. Thus, the Rational Method has been used in the determination of required storage volumes to store the 100-year storm events to the pre-determined allowable release rates.

#### 4.6.2.1 Extended Detention Control (Quality)

The wet pond will use a 200mm reverse pipe with **one 80 mm dia. orifice plate** to control the detention time to the minimum detention time of 12h, per MOE Guidelines for drainage areas less than 8 ha.

Using equation 4.10 from the MECP SWM guidelines resulted in a drawdown time of 15.53 hours.

$$t = \frac{2 A_p}{C A_o (2g)^{0.5}} \left( h_1^{0.5} - h_2^{0.5} \right) \quad \text{Equation 4.10: Drawdown Time}$$

Where:

$t$  = drawdown time in seconds

$A_p$  = surface area of pond (m<sup>2</sup>)

$C$  = discharge coefficient

$A_o$  = cross-sectional area of the orifice (m<sup>2</sup>)

$g$  = gravitational acceleration constant

$h_1$  = starting water elevation above the orifice (m)

$h_2$  = ending water elevation above the orifice (m)

$$t = \frac{2A_p}{CA_o(2g)^{0.5}} (h_1^{0.5} - h_2^{0.5})$$

$$t = \frac{2(876.75)}{(0.63)(0.005)(2 * 9.81)^{0.5}} (0.2^{0.5} - 0^{0.5})$$

$$t = 55906 \text{ s} = 15.53 \text{ hours}$$

#### 4.6.2.2 Release Rate Control (Quantity)

The release rate control, under free flow conditions, will be achieved by **one 600x1040mm rectangular orifice** set at an invert elevation of 89.500 m ASL. Under free flow conditions, this opening will act as a weir, and will control the 100-year release rate to 904.6 L/s on average.

*Table 4-8 Resulting Release Flow with Proposed Controls*

Release Rate Control Flow condition	Average Release Flow (L/s)	Max. Water Surface Elevation at pond outlet (m ASL)
Free Flow Condition	904.6	9.100

## 4.7 SWM Conclusions

The storm servicing design for the proposed development generally conforms to the requirements of the City of Ottawa Sewer Design Guidelines, 2012, as amended by all applicable Technical Bulletins. The storm servicing design also conforms to the HIP SWM report (J.L. Richards ,2009). Justifications have been provided where deviations were proposed by the SWM strategy.

The allowable release rate for the site post-development was calculated to be **906.9 L/s**. It is expected that this can be achieved via roof storage and the proposed SWM wet pond.

A Roof Flow Control Declaration will be provided upon completion of the Mechanical and Structural design.

## 5. Sediment and Erosion Control

Appropriate measures must be taken to control erosion and sedimentation during the construction process for the proposed development. Sediment will be trapped on site, implementing the Ontario Ministry of Natural Resources and Forestry's (MNRF) "Guidelines on Erosion and Sediment Control for Urban Construction Sites," to assure proper control measures are upheld. Furthermore, the following measures must be considered:

- + Supply and install silt fences (as per OPSD 219.110) along the perimeter of the impacted lands, including borrow and stockpile areas resulting from topsoil stripping or excavating activities; locations determined during field grading operations;
- + Catch basin inserts must be used within the limits of the project and must remain in place until project completion. The inserts must also be inspected regularly and corrected as deemed necessary;
- + A dewatering system, such as a sedimentation basin or approved equivalent, shall be implemented to filter sediments from an excavated trench should dewatering and pumping operations become necessary, all in accordance with the City of Ottawa Sewer Use By-Law 2003-514.

All control measures will be carried out in accordance with the following documents:

- + "Guidelines on Erosion and Sediment Control for Urban Construction Sites" published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs and Housing, and Transportation and Communication, Association of Construction Authorities of Ontario, and Urban Development Institute, Ontario, May 1987.
- + "Erosion and Sediment Control" Training Manual by Ministry of the Environment, Spring 1998.
- + Applicable Regulations and Guidelines of the Ministry of Natural Resources and Forestry.

Refer to the following project drawings for additional information: Sediment and Erosion Control Plan (C004) and Notes Plans (C005 and C006).



## 6. Conclusion

The current study demonstrates how the proposed servicing of the site will be achieved, in that the proposed SWM strategy conforms to the existing SWM plan and that the proposed Potable Water, Fire Protection and Sanitary Servicing works will be sufficient to service the proposed development.

Within the site, all services have been designed in keeping with the City of Ottawa design requirements and the requirements of the HIP SWM Report.

We trust this site servicing and stormwater management report is to your satisfaction. If you have any questions regarding this report, please do not hesitate to contact the undersigned.

## 7. References

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# A

## Appendix A - J.L. Richards Storm Water Management Plan

**STORMWATER MANAGEMENT REPORT**  
**HAWTHORNE INDUSTRIAL PARK**

February 2009  
(Revised April 2009)  
(Revised May 2009)

Prepared for:

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JLR 20983

# STORMWATER MANAGEMENT REPORT

## HAWTHORNE INDUSTRIAL PARK

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# STORMWATER MANAGEMENT REPORT

## HAWTHORNE INDUSTRIAL PARK

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### 1.0 INTRODUCTION

#### 1.1 Background

In 1999, J.L. Richards & Associates Limited (JLR) completed a Stormwater Management Study, on behalf of Beaver Road Builders Ltd., for the development of a proposed area previously referred to as the Hawthorne Road Industrial Subdivision. The main objective of the 1999 Study was to develop a conceptual storm servicing alternative (including stormwater management) that would support the proposed development without adversely affecting the hydrological regimes of receiving streams. The 1999 Study provided a conceptual design of the conveyance system and on-site storage requirements for the proposed development in order to satisfy the regulatory agencies of the time, namely the Region of Ottawa-Carleton, the City of Gloucester and the South Nation Conservation Authority (SNC).

The current landowner, R.W. Tomlinson Limited (Tomlinson), now wishes to complete the development of the subject land, herein referred to as the Hawthorne Industrial Park (HIP).

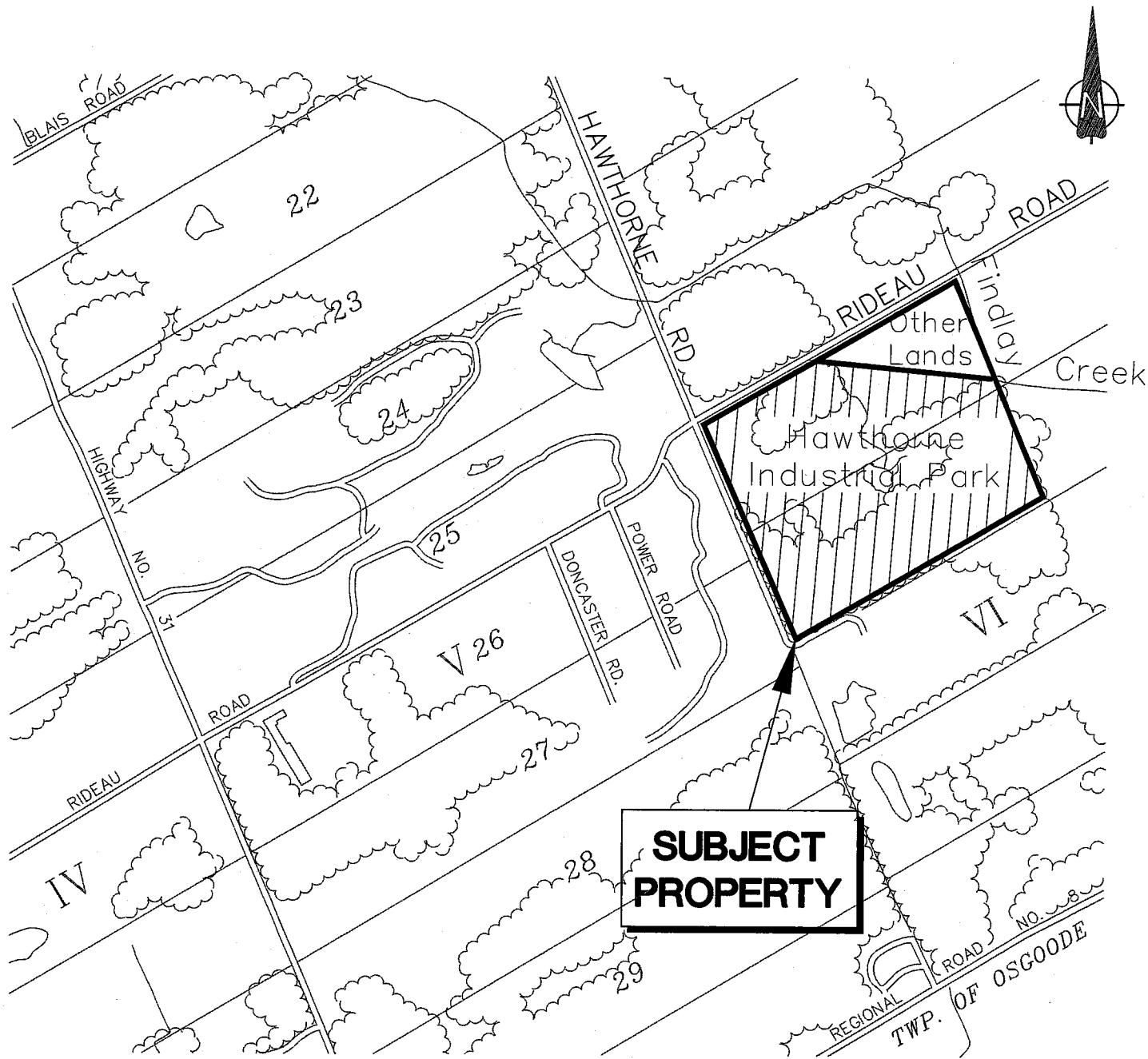
#### 1.2 General

The proposed 70 hectare (ha) site is located immediately southeast of the Hawthorne Road/ Rideau Road intersection (refer to Figure 1) in the City of Ottawa (formerly in the City of Gloucester) and is expected to service future industrial operations varying in size. Over the past decade, the site has been used to dispose of fill materials resulting from Tomlinson's construction activities. The fill material has been placed in areas where fill was required for the construction of the proposed HIP.

Currently, Orgaworld Canada Ltd. (Orgaworld), has leased approximately 10 ha within HIP, which will house the source separated organics program being implemented by the City of Ottawa in 2009. The Orgaworld site includes a Stormwater Management Facility with a capacity of 15,994 m<sup>3</sup> providing on-site water quantity and quality control.

In addition, a permanent facility within the above subject lands is a total suspended solids (TSS) treatment facility. Consisting of three (3) ponds, this facility was designed





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NOT TO SCALE

PROJECT:

**HAWTHORNE  
INDUSTRIAL PARK**

DRAWING:

**KEY PLAN**



**J.L. Richards**

ENGINEERS-ARCHITECTS-PLANNERS

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DRAWING NO.:

**FIGURE 1**

JLR NO:

20983

to provide aggregate wash water management to Tomlinson's existing quarry operations on the west side of Hawthorne Road (refer to Appendix 'I' for a copy of the Ministry of the Environment (MOE) Certificate of Approval (C of A) related to these works). In addition to the existing aggregate wash treatment facility, it is proposed to construct separate stormwater management facilities to service water quantity and quality requirements for the HIP.

### 1.3 Objectives

This Stormwater Management Report (SWMR) was prepared to demonstrate that the subject lands can be developed as an Industrial Park Subdivision in compliance with the current surface water objectives of the watershed. Since the subject lands drain to Findlay Creek, which is tributary to the North Castor River, storm runoff criteria for this development must be in accordance with the recommendations of the document entitled "Shield's Creek Subwatershed Study, Totten Sims Hubicki Associates, June, 2004", referred throughout this Report as SCSS. More specifically, the above Report provided the following design criteria with regard to stormwater:

#### Water Quantity

- |                     |   |
|---------------------|---|
| <u>Peak Flow</u>    | Post-development peak flows must be controlled to pre-development levels for storm events ranging from a 1:2 year to a 1:100 year recurrence. |
| <u>Infiltration</u> | Section 5.5 of the SCSS recommends that the quantity and quality of groundwater infiltration be maintained to pre-development rates.          |
| <u>Erosion</u>      | The stormwater management strategy for the proposed HIP must be developed to maintain the erosion potential to current levels.                |

#### Water Quality

The proposed stormwater management strategy for HIP must be developed to meet a Normal Level of Protection (as per the MOE's publication entitled "Stormwater Management Planning and Design Manual, March, 2003", referred throughout this Report as SWMPDM, which corresponds to a standard approach used in urban development to obtain a targeted total suspended solids (TSS) removal rate of 70%.

## 2.0 STORM DRAINAGE

### 2.1 General

Storm servicing for the HIP was designed using the dual drainage concept, also known as the minor/major drainage system. The minor drainage system is mainly comprised of an on-site open ditch and culvert system. The minor system was designed to capture and convey runoff during frequent storm events up to a 1:10 year recurrence. The major system formed by swales/ditches, streets, etc. was sized to accommodate runoff during storm events exceeding 1:10 year up to the 1:100 year recurrence.

The open ditches, culverts and swales were sized using the Rational Method. An inlet time of 15 minutes and runoff coefficients (C-factors) ranging from 0.20 to 0.90 were used in the sizing of the conveyance systems. It should be noted, however, that C-factors used were increased by 10% for the 1:25 year peak flow calculations and by 25% for the 1:100 year recurrence, as per Section 5.4.5.2.1 of the City of Ottawa's Sewer Design Guidelines (November 2004). Rainfall intensities (i.e., Intensity-Duration-Frequency curves (IDF)) required by the Rational Method were also extracted from the City of Ottawa's Sewer Design Guidelines. Peak flow rates for the HIP and Hawthorne Road and Rideau Road are summarized in Table 1 (refer to Appendix 'A' for copies of the Rational Method Design Sheets for the 1:10 year and 1:100 year storm events).

**Table 1 - Summary of Peak Flow Rates**

Description	Peak Flows (L/s)	
	10 Year	100 Year
Hawthorne Industrial Park (HIP)	5,422	12,814
Hawthorne Road / Rideau Road	3,192	5,417

### 2.2 Design Criteria

The municipal infrastructure associated with the HIP was designed using the following criteria:

- The HIP open ditch system was sized with sufficient capacity to convey, under free-flowing conditions, the 1:100 year peak flow rate, as calculated by the Rational Method (refer to Appendix 'A' for a copy of the 1:100 year Design Sheet).
- The Hawthorne Road open ditch system was sized with sufficient capacity to convey, under free-flowing conditions, the 1:100 year peak flow rate, as calculated by the Rational Method (refer to Appendix 'A' for a copy of the 1:100 year Design Sheet).
- The existing downstream ditch system along Rideau Road was evaluated to ensure sufficient capacity to convey, under free-flowing conditions, the 1:100 year peak flow rate, as calculated by the Rational Method (refer to Appendix 'A' for a copy of the 1:100 year Design Sheet).
- The culverts included in the HIP and along Hawthorne Road/Rideau Road were sized with sufficient capacity to convey the 1:10 year peak flow rate without overtopping the roadway embankment (refer to Appendix 'A' for a copy of the 1:10 year Design Sheet).
- Given that the receiving watercourse was found to shelter fisheries, the SCSS recommended that a "normal" level of protection be achieved for quality control. To fulfill this requirement, industrial sites must direct runoff to an appropriately sized oil/grit separator unit before stormwater can be conveyed off site to the open roadside ditch/culvert system. To achieve quality control for the internal roads, it is proposed to provide infiltration storage volume in the roadside open ditch system, as per the requirements presented in Table 3.2 of the SWMPDM.
- The SCSS recommended that the erosion potential be maintained to current levels for the receiving water course. To fulfill the above requirement, the two year post-development peak flow will be controlled to 50% of the pre-development peak flow rate.
- Storage volume is to be implemented for the control of the post-development peak flows to pre-development levels for storm events ranging from a 1:2 year to a 1:100 year recurrence to comply with the recommendations of the SCSS.

This Stormwater Management Report (SWMR) has been written to demonstrate that the subject land could be developed in compliance with the above surface water criteria and also prepared in accordance with the SWMPDM. The proposed stormwater management strategy for the HIP was developed to meet a "normal" level of protection, which corresponds to a standard approach used in land development to obtain a targeted TSS removal rate of 70%.

### **3.0 STORM SERVICING**

#### **3.1 General**

Peak flow estimation is an important task that is carried out for any proposed development. There are several reasons that explain why flood flow rates are computed as part of site development. The main purpose of these calculations, however, is to allow for the proper configuration and sizing of the proposed conveyance systems to minimize the risk of flooding.

Drainage works are designed for a real or hypothetical storm event that may or may not happen during the lifetime of the facilities. At the onset of the design process, design criteria are adopted that may vary with the type of project, in recognition of the impacts of failure. For this particular project, the level of protection adopted (storm events up to a 1:100 year recurrence) was based on design storm characteristics of an infrequent storm event having a low probability to occur.

#### **3.2 Description of Conveyance Systems and Design Basis**

Flowing water can be conveyed to an outlet by either open-channel flow or pipe flow. Storm runoff generated by the subject lands is to be collected and conveyed by a roadside ditch/culvert system before discharging to Findlay Creek via an end-of-pipe stormwater management facility (SWMF).

Sizing of the conveyance systems was carried out using various levels of service. The open ditch system was sized with sufficient capacity to convey, under free-flowing conditions, storm runoff up to the 1:100 year recurrence, while roadway culverts were sized to provide conveyance of the 1:10 year peak flow rates without overtopping the roadway embankments.

As part of this sizing exercise, Storm Drainage Area Plans were prepared and included in this Report (refer to Drawing D-ST1 for the HIP and Drawing D-ST2 for Hawthorne and Rideau Road) that show the delineated area for each of the conveyance segments (i.e., from node location to node location), along with its assigned runoff coefficient (C-factor) based on the type of surface. Since the final development of Hawthorne Industrial Park is unknown at this time, a conservative on-site runoff coefficient (C-factor) of 0.70 was used. Table 2 illustrates the breakdown of a typical site that would generate a weighted runoff coefficient of 0.70.

**Table 2 - Typical Potential Land Use Breakdown**

Type of Surface	Area (%)	C-Factor
Building	10	1.0
Asphalt Parking	35	0.90
Gravel	35	0.70
Grass	20	0.20
Overall	100	<b>0.70</b>

It should be noted that the C-factors shown on the Storm Drainage Area Plans denote those associated with 1:10 year peak flow calculations. As recommended in Section 5.4.5.2.1 of the City of Ottawa's Sewer Design Guidelines, C-factors shown on drawings were increased by 10% and 25% for the 1:25 year and 1:100 year peak flow calculations, respectively (refer to Appendix 'A' for copies of the Rational Method Design Sheets).

### **3.2.1 Open Ditch System**

An open ditch channel is a conduit used to convey flowing water from one location to another, with a free surface. A channel can be classified as either artificial (i.e., manmade) or natural. Artificial channels are those constructed or developed as a result of human activity. This type of conveyance system is usually implemented as a long and mild-sloped channel built in the ground, which provides conveyance of water between two points, with sections of regular geometry and shape. An open ditch system is generally designed to follow site topography and the vertical profile of the adjacent roadway. The most commonly used shapes for open channel ditches are trapezoidal and triangular, with the latter shape utilized mainly for ditches servicing small drainage areas.

The open ditches associated with the HIP and Hawthorne Road were sized with sufficient capacity to convey 1:100 year peak flow rates. As previously noted, the Rational Method Design Sheets (refer to Appendix 'A' for copy of the 1:100 year design sheet) were used to quantify the 1:100 year peak flow rates. The open ditch configuration was carried out utilizing Manning's relationship, along with the proposed geometry and slope of the channel. Two Storm Drainage Area Plans were prepared (refer to Drawings D-ST1 and D-ST2) showing proposed ditch inverts that match those shown on the Rational Method Design Sheets. Based on the ditch sizing exercise, it was determined that triangular shape ditches with 3:1 side slopes and variable depths provided the necessary conveyance of the 1:100 year peak flow rate. The Site Servicing and Grading Plan (refer to Drawing SG) was developed to provide the configuration of open ditch segments.

The existing open ditches along Rideau Road were also evaluated to ensure sufficient capacity was able to convey the 1:100 year peak flow rates resulting from upstream construction works (i.e., construction of Hawthorne Road). The Rational Method Design Sheets (refer to Appendix 'A' for copy of the 1:100 year design sheet) were used to quantify the 1:100 year peak flow rates. An existing 900 mm diameter culvert crossing under Hawthorne Road conveys flow along the north side of Rideau Road (refer to Drawing D-ST2). The capacity of this existing culvert was estimated at 1,400 L/s under a 1.5 m headwater (refer to Appendix 'B' for Culvert Design Summary Table). Upon the review of existing topography, any headwater depths greater than 1.5 m resulted in runoff being directed northerly along Hawthorne Road towards Findlay Creek. In light of the above, the existing open ditches along Rideau Road were evaluated using a conservative plug flow of 1,400 L/s in addition to surface runoff generated by the contributing areas.

### **3.2.2 Culvert System**

The principal function of a culvert is to convey water through an embankment while, at the same time, supporting the weight of the overlying fill and vehicular movement. Culverts can be made of many different materials; steel, polyvinylchloride (PVC), high density polyethylene (HDPE) and concrete. Culverts selected for the HIP and Hawthorne Road are made of corrugated steel, in either round or arch shape. Field observations have shown that there are two major types of culvert flow conditions: inlet control and outlet control.



### 1. Flow Under Inlet Control

Flow with inlet control means that the discharge capacity of a culvert is controlled at the culvert entrance by the depth of headwater and by the entrance geometry, including the barrel shape, cross sectional area and the type of inlet edge. The roughness and length of the culvert barrel, and the outlet conditions are not factors in determining the culvert capacity. The longitudinal slope reduces headwater only to a small degree and can normally be neglected for conventional culverts flowing in inlet control.

### 2. Flow Under Outlet Control

Flow with outlet control means that the discharge capacity of a culvert is controlled by the depth of tailwater, including the velocity head within the barrel, the entrance and friction losses. The roughness, length of the culvert barrel, and slope are factors in determining the culvert capacity; the inlet geometry is of lesser importance.

To avoid having to conduct detailed hydraulic computations that would determine the type of flow under which a culvert will probably operate, the procedure recommended by the MTO (refer to MTO's Drainage Management Manual) was utilized. This methodology, referred to as the Conventional Culvert Design procedure, requires that MTO's Design Charts and Design Nomographs be used for both inlet and outlet control conditions. The higher headwater depth that is calculated from those two operating conditions would indicate the type of control and would provide the governing headwater depth. This methodology was utilized to size each culvert crossing, along with the 1:10 year peak flow rates calculated by the Rational Method Design Sheets (refer to Appendix 'A') for each of the conveyance segments. Furthermore, this calculation sheet also provides proposed culvert sizes, along with the type of control and governing depth found when using the conventional culvert design procedure. A summary of the various parameters estimated using MTO's nomographs at each of the culverts has been tabulated using MTO's Form D4-I (refer to Appendix 'B' for Conventional Culvert Design Sheet). This analysis shows that the proposed culvert crossings within the HIP and along Hawthorne Road are capable of conveying the 1:10 year peak flow rates as a minimum, without overtopping any of the roadway embankments. The hydraulic calculations were carried out assuming a roughness coefficient of 0.024 for any of the CSP and CSPA culverts. The Site Servicing and Grading Plan (Drawing SG) shows proposed culvert sizes, lengths and invert elevations at each of the crossings.

The proposed 1030 x 740 mm CSPA culvert crossing under the entrance of the pond access road was of concern due to the high flow rate during the 1:100 year storm event.

There was a possibility that the excess flow overtopping this culvert could short circuit into SWMF via the pond access road. Therefore, an analysis of the flow overtopping the proposed entrance culvert was conducted and the results confirmed that the residual flow would indeed be contained within the right-of-way corridor (refer to Appendix 'J' for desktop calculation).

#### **4.0 WATER BALANCE**

Water balance analyses are typically carried out to assess any changes in infiltration to subsurface water-bearing zones as a result of the urbanization (i.e., increase of hard surfaces) of land. The SCSS has identified the need to maintain a necessary level of quantity and quality groundwater recharge via infiltration. Groundwater recharge is required to maintain subsurface base flow to streams and wetlands in addition to maintaining groundwater levels for private and municipal wells. The Hydrogeological Study completed by Golder Associates Limited in 2008 for the HIP identified the site as being underlain by a shallow and deep aquifer separated by an impermeable rock layer. The upper aquifer provided subsurface groundwater flow to streams, while the lower aquifer was the main source for well water supply. Therefore, groundwater recharge for this site was intended to provide subsurface base flow into the receiving Findlay Creek.

Construction fill operations have been active for the HIP since 1994. The results of the geotechnical field investigation conducted by Inspec-Sol Incorporated in 2008 indicates that as much as 5.5 m of fill material (MW7-08) has been placed on parts of the site. The non-native heterogenous fill material is comprised mainly of silty clay and contains trace amounts of road and construction materials. Although the soil component of the fill material exhibits the characteristics of silty clay, the varying composition and density of the remaining portion of the fill affects its permeability in localized areas. Given the above existing conditions, it is difficult to determine how groundwater recharge will behave as subsurface flow in the existing fill matrix, particularly from individual sites within the HIP. The MOE expressed concerns about the use of infiltration strategies on the individual sites given the past history as a construction fill site. Furthermore, the MOE SWMPDM does not endorse the use of infiltration basins on lands zoned for industrial use as there is an increased risk of groundwater contamination should a spill occur on site.

An option was considered to provide infiltration for the entire site at the base of the end-of-pipe Dry Pond facility. Upon further investigation, the geotechnical report indicated

that there was a high groundwater table at the proposed pond location. In addition, in-situ soils in the area exhibited poor drainage properties which would have resulted in long retention times at the base of the pond, making it difficult to meet the water balance deficit requirements for the entire site while attempting to mimic the pre-development hydrological cycle.

Representatives from the City and SNC were consulted, and it was concluded that the SCSS groundwater balance targets for this site would be difficult to meet. It was also recognized that on-site infiltration strategies for this industrial subdivision could have a detrimental effect on groundwater quality and jeopardize the natural ecological integrity of receiving waters. In light of the above, it was decided by the approval authorities that the requirement for the water balance would be waived for the HIP development.

## **5.0 WATER QUALITY**

### **5.1 General**

Urbanization has been found to modify the hydrological regime of a receiving stream if inadequate stormwater management measures are implemented. The potential impacts associated with runoff arise primarily from the amount of urban area that is impervious to rain and snowmelt water. These impervious surfaces increase the amount of direct surface runoff that is generated and is conveyed more efficiently to the receiving stream. As part of the SCSS, fisheries resources have been inventoried along this watercourse, along with its associated tributaries. Given that the receiving watercourses were found to shelter fisheries, the approved document recommended that a "normal" level of protection be achieved. To fulfil this requirement, it is proposed that each individual site provide an oil/grit separator and infiltration storage be provided within the roadside open ditch system, as per the requirements presented in the SWMPDM.

### **5.2 Water Quality Requirement**

Stormwater servicing for the HIP has been developed in accordance with the water quality recommendations of the SCSS (70% TSS removal). To fulfil this requirement, individual sites will be required to provide an oil/grit separator be installed to provide quality treatment (i.e., 70% TSS removal) of surface runoff before entering the roadside open ditch/culvert system. In addition, the oil/grit separator will be able to capture and contain hydrocarbons in the event of an on-site accidental spill.

To fulfill the water quality objectives for the paved portion of the HIP internal roads, it is proposed to provide infiltration within the open roadside ditch system to meet the storage volume requirements presented in Table 3.2 of the SWMPDM. Based on the normal level of service required and an imperviousness of 100% for the internal roads, Table 3.2 yields an extrapolated storage volume requirement of 35 m<sup>3</sup>/ha. To achieve this storage volume, a clear stone envelope complete with a 200 mm diameter perforated pipe will be installed at the base of the roadside ditches to meet the required storage volume (Refer to Appendix C for calculations).

The following table presents the calculated infiltration volume required for water quality control and those provided by the roadside open ditch system to meet the recommended MOE Design Guidelines.

**Table 3 - Water Quality Infiltration Requirements**

Phase	Area (ha)	Infiltration Volume Requirement (m <sup>3</sup> )	Infiltration Method	Length of 200 mm diameter Perf. Pipe (m)	Infiltration Volume Provided (m <sup>3</sup> )
1	1.58	55.1	Open Ditch	1760	55.3
2	0.21	7.4	Open Ditch	240	7.5
Total	1.79	62.5	Open Ditch	2000	62.8

As shown in the above Table, the infiltration volume provided by the proposed open roadside ditch network (62.8 m<sup>3</sup>) exceeds that obtained from Table 3.2 (62.5 m<sup>3</sup>) of the SWMPDM. It should be noted that additional storage within the void space of the clear stone envelope was not accounted for and would increase the actual infiltration storage volume shown in Table 3.

## 6.0 HYDROLOGICAL ANALYSIS

### 6.1 General

To satisfy the surface water objectives presented in Subsections 1.3 and 2.2, a hydrological analysis was carried out to quantify peak flow rate variations resulting from the development of the proposed HIP. To quantify this variation, the SWMHYMO Stormwater Management Hydrological Model (Version 4.02, July, 1999) was utilized to calculate peak flows during severe storm events.

To carry out the hydrological analysis, three storm drainage plans were developed; one representing the pre-development drainage conditions, one representing the post-development conditions for the current study area, Phase 1, and the other for the post-development drainage conditions, including future development, Phase 2. For each of these plans, subwatershed boundaries were delineated based on existing topography of the site and the proposed overland flow direction following development of the site (refer to Figures 2, 3 and 4 for details).

## **6.2 Synthetic Design Storm Simulation and Hydrological Parameters**

Peak runoff rates were calculated for both pre- and post-development conditions using synthetic design storm event modelling. Peak flow rates were estimated using the 3-hour Chicago Design Storm Event, as this synthetic storm event has been recognized as the most critical event for urban runoff applications (refer to Section 5.4.3.1 of the City of Ottawa's Sewer Design Guidelines). The design storm analysis was completed using volumes derived from the Intensity-Duration-Frequency (IDF) curve equation shown in Section 5.4.2 of the City of Ottawa Sewer Design Guidelines compiled using data from 1967 to 1997.

A SWMHYMO data file was developed to represent both pre- and post-development conditions of the subject area. Simulation of surficial runoff generated from undeveloped subwatersheds was carried out using the "DESIGN NASHYD" command along with the SCS procedure to compute rainfall losses. The SCS procedure uses the Curve Number (CN) method to compute rainfall losses and the Nash unit hydrograph to simulate the hydrological response from undeveloped watersheds. To simulate surface runoff from urban subwatersheds, the "CALIB STANDHYD" command was utilized. Hydrological parameter selection and methodology is described below:

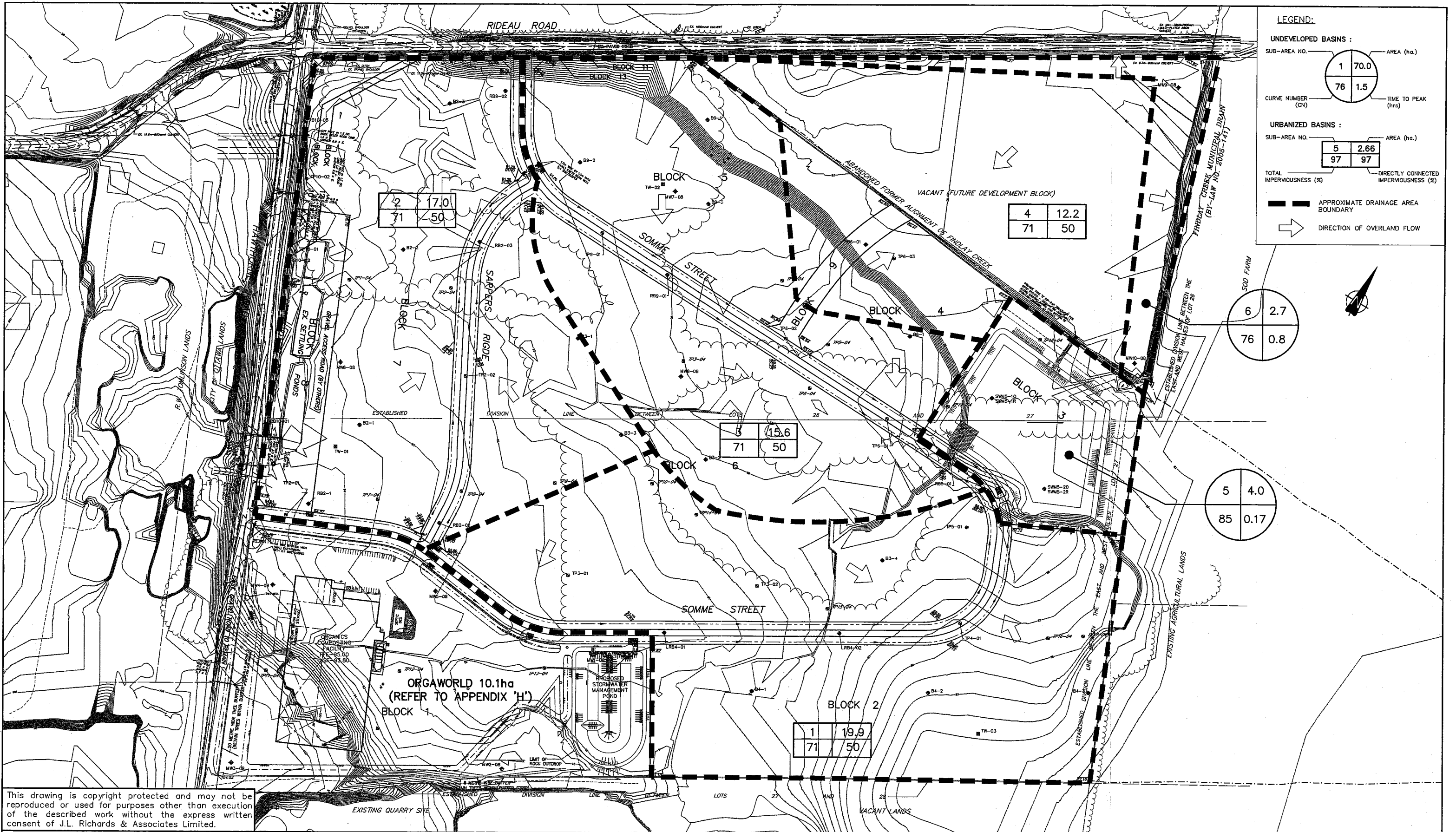
### **Curve Number (CN)**

In order to estimate a Curve Number that represents pre-development conditions, the geotechnical investigation completed by Inspec-Sol, entitled "Geotechnical Study Subdivision Plan, Hawthorne Industrial Park, Lots 26 and 27 Concession 6, Southeast of Hawthorne and Rideau Roads, Ottawa, Ontario" dated December 19, 2008 was used. At the time of this investigation, large amounts of fill material were encountered over the majority of the site, which does not reflect the pre-development conditions. As such, only native soils encountered below fill material were used to establish pre-development condition Curve Numbers. The review of the geotechnical investigation shows native









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PROJECT:  
**HAWTHORNE  
INDUSTRIAL PARK**

DRAWING:  
**POST DEVELOPMENT – PHASE 2  
STORM DRAINAGE AREA  
PLAN**

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DRAWN: ARM  
CHECKED: G.F.  
PLOTTED: Apr 30, 2009

DRAWING NO.:  
**FIGURE 4**  
JLR NO:  
20983



soils ranging from silty sand in Blocks 4 and 5, to silty clay in Blocks 3, 5, 7 and 8, to sandstone and limestone in parts of Blocks 2 and 3. These soils have been classified by Inspec-Sol as being associated with hydrologic soil groups (HSG), ranging from "B" to "D" for silty sand to silty clay, respectively. Areas where rock was encountered (i.e., Sandstone and Limestone) were classified as "Rockland." Based on this information and current land usage, as interpreted from aerial photography, a pre-development Curve Number (CN) of 76 has been calculated using the Ministry of Transportation of Ontario (MTO) Chart H2-8. Detailed calculations for the HIP have been included in Appendix 'D'.

Under post-development conditions, it is proposed to provide sufficient grade differential to allow for positive drainage to meet City of Ottawa Design Standards. As the subject lands are to be developed as an Industrial Park with a significant increase in hard surfaces (i.e., buildings, asphalt and gravel), the post-development conditions were, therefore, analysed taking into consideration the low potential of these surfaces to infiltrate storm runoff.

### **Imperviousness**

Surface runoff under post-development conditions is greatly impacted by the imperviousness of its tributary area. Since the final development of the HIP is unknown, a conservative assumption for typical surfaces encountered in similar industrial parks was developed, as illustrated in Table 2. To determine the imperviousness based on the assumed breakdown presented in Table 2, an imperviousness calculation was carried out and is presented in Appendix 'D'. The imperviousness calculation was based on the following assumptions:

- an imperviousness of 100% was assigned for building footprints;
- an imperviousness of 100% was assigned for all asphalt parking surfaces.
- an imperviousness of 70% was assigned for all gravel surfaces; and
- it was assumed that 50% of the total imperviousness (TIMP) 50 % was modelled as directly connected imperviousness (XIMP).

Based on the above, a total imperviousness of 70% was calculated, which is equivalent to a runoff coefficient of 0.7. The hydrological analysis was, therefore, carried out using

a total imperviousness of 70%, consistent with the runoff coefficient used for sizing the open ditch/culvert system.

### **Time to Peak ( $T_p$ )**

Time to peak calculations were carried out under pre-development conditions. Time of concentration was first estimated using the Uplands Method Chart based on the various flow paths. Once calculated, the times to peak were set to 67% (i.e., 2/3) of the time of concentration ( $T_c$ ). Under pre-development conditions, a 90 minute time to peak was calculated (refer to Appendix 'D' for calculations). When modelling post-development conditions, the "CALIB STANDHYD" command was used to calculate the time to peak associated with the proposed site surfaces and grades (refer to Appendix 'E' for SWMHYMO outputs).

### **6.3 Simulation of Pre- and Post-Development (Uncontrolled) Conditions**

The hydrological analysis was carried over the entire HIP under both the pre- and post-development conditions. As stated in Section 6.1, two post-development conditions were investigated, namely, Phase 1 and Phase 2. Phase 1 evaluates servicing for the current Study area, while Phase 2 includes the current Study area along with servicing of an additional 11.2 ha of land to the north east, shown on drawings as "Future Development Block."

Peak flow rates were computed with SWMHYMO using the procedure and parameters described in Subsection 6.2. Table 4 presents the simulated peak runoff rates under a 3 hour Chicago design storm event for both the pre- and post- (uncontrolled) development conditions for the HIP (refer to Appendix 'E' for SWMHYMO data input and output files), along with those under a 4 hour - 25 mm storm.

**Table 4 - SWMHYMO Simulation Results**

Return Period or Storm Depth	Peak Flow Rates (L/s)		
	Pre-Development	Phase 1 Post-Development (Uncontrolled)	Phase 2 Post-Development (Uncontrolled)
25 mm	252	1,941	2,231
2	467	3,077	3,548
5	826	4,812	5,554
10	1,097	6,135	7,029
25	1,468	7,772	9,013
50	1,767	9,240	10,588
100	2,093	10,662	12,132

Simulation results presented in the above table show that uncontrolled post-development peak flows substantially exceed those obtained under pre-development conditions. Based on the design criterion for water quantity (refer to Subsections 1.3 and 2.2 for details), post-development peak flows should be maintained to their pre-development levels for storm events ranging from a 1:5 year to a 1:100 year recurrence. In addition, the 2-year post-development peak flow should be controlled to 50% of the 2-year pre-development peak flow to satisfy the erosion criterion. Water quantity control measures were, therefore, found to be necessary for the development of this site. Details and stormwater servicing approaches proposed to fulfil the design criteria listed in Subsections 1.3 and 2.2 are presented in the following Subsections.

#### **6.4 Simulation of Phase 1 Post-Development (Controlled) Conditions**

Development of the subject lands (i.e., 70 ha, as illustrated on Figure 3) will increase the imperviousness of the subject area. To achieve the surface water objectives listed in Subsections 1.3 and 2.2, it is proposed that an end-of-pipe facility be constructed that would provide storage volume for retention of runoff.

The stormwater management criteria for the development of the HIP consist of maintaining erosion potential and peak flow rates at the pre-development levels. Storm servicing of the Subdivision was, therefore, developed such that all of these requirements were fulfilled, along with the achievement of a "normal" protection level. It

is proposed to implement the following stormwater management servicing approach for the development of the HIP:

### **End-of-Pipe SWMF (Block 3)**

Based on the proposed grading, the end-of-pipe facility was found to generate a volume of 37,240 m<sup>3</sup> (3.25 m depth). A low flow ditch sized for 2 year storm events was also included in the bottom of the end-of-pipe facility to convey flows to the outlet structure. The configuration of the outlet structure would be as follows:

- 1 x 150 mm diameter orifice within a 200 mm diameter Polyvinyl Chloride (PVC) pipe at elevation 82.90 m, which serves as outlet to the facility;
- 2 x 600 mm diameter Corrugated Steel Pipe culvert at elevation 84.80 m, which also serves as outlet to the facility;
- One (1) emergency overflow spillway (6.0 m wide) at elevation 86.15 m, which serves as outlet to the facility during a storm event greater than 1:100 year.

The above configuration was used to develop a Stage-Storage-Discharge relationship that relates the storativity and outlet capabilities of the proposed facility at various geodetic elevations (refer to Appendix 'F' for copy of this Table). This data (storage-discharge table) was then used as input to the SWMHYMO's ROUTE RESERVOIR command.

A SWMHYMO file, representing the post-development controlled conditions of the HIP, was developed incorporating the storage volume and the outflow capability of the proposed end-of-pipe facility. The following table presents the simulated peak runoff rates for the three (3) hour Chicago design storm under the post-development controlled conditions (refer to Appendix 'G' for SWMHYMO data input and output files), along with those under the four (4) hour - 25 mm storm.

**Table 5 - SWMHYMO Simulation Results  
(Post-Development - Phase 1 Controlled Conditions)**

Return Period or Storm Depth	Peak Flow Rates (L/s)	
	Pre-Development	Phase 1 Post-Development (Controlled) <sup>(1)</sup>
25 mm	252	127
2 year	467	194 <sup>(2)</sup>
5 year	826	359
10 year	1,097	589
25 year	1,468	939
50 year	1,767	1,191
100 year	2,093	1,531

Note: (1) Post-development flow is the sum of flows from the end-of-pipe facility and two uncontrolled Sub-Areas totalling 12.1 ha.

(2) 2 year post-development peak flow less than half the 2-year pre-development peak flow (233 L/s).

Simulation results presented in Table 5 show that the Phase 1 post-development controlled peak flows will be maintained below pre-development levels for the HIP. Consequently, the water quantity objective defined in Subsections 1.3 and 2.2 will be met under Phase 1.

#### **6.5 Simulation of Phase 2 Post-Development (Controlled) Conditions**

Development of Phase 2, as depicted on Figure 4, includes the Future Development Block located in the northeast corner of the HIP. This additional land could be serviced by the previously proposed end-of-pipe <sup>facility</sup>, without any modifications to facility size or outlet structure. However, a second inlet would be required in the northeast corner of the facility, which could be designed during the detailed design stage of the Future Development Block.

A SWMHYMO file, representing the Phase 2 post-development controlled conditions of the HIP, was developed incorporating the storage volume and the outflow capability of the proposed end-of-pipe facility. The following table presents the simulated peak runoff rates for the three (3) hour Chicago design storm under the Phase 2 post-development

controlled conditions (refer to Appendix 'H' for SWMHYMO data input and output files), along with those under the four (4) hour - 25 mm storm.

**Table 6 - SWMHYMO Simulation Results  
(Post-Development - Phase 2 Controlled Conditions)**

Return Period or Storm Depth	Peak Flow Rates (L/s)	
	Pre-Development	Phase 2 Post-Development (Controlled) <sup>(1)</sup>
25 mm	252	73
2 year	467	156 <sup>(2)</sup>
5 year	826	457
10 year	1,097	729
25 year	1,468	1,051
50 year	1,767	1,348
100 year	2,093	1,515

Note: (1) Post-development flow is the sum of flows from the end-of-pipe facility and one uncontrolled Sub-Area totalling 2.7 ha.

(2) 2-year post-development peak flow less than half the 2 year pre-development peak flow (233 L/s).

Simulation results presented in Table 6 show that the Phase 2 post-development controlled peak flows will be maintained below pre-development levels for the HIP. Consequently, the water quantity objective defined in Subsections 1.3 and 2.2 will also be met under Phase 2.

## **6.6 Simulation of the July 1, 1979 Historical Storm Event and Flood Potential**

### **6.6.1 Simulation of the July 1, 1979 Historical Storm Event**

In addition to designing the major drainage system to convey the 1:100 year storm event, the performance of both the open ditch system and SWMF was also assessed under the July 1, 1979 historical storm event. This historical storm event is defined as a high volume / low intensity storm event (when compared to the 1:100 year event) which

occurred mostly over a three hour period (refer to Table 5.6 in the Ottawa Sewer Design Guidelines). As shown in Table 5.6, the maximum intensity of 106.7 mm/hr only occurred for a 10 minute period (i.e., between the 85 to 95 minute time interval). The 1:100 year storm event intensities used to size the open ditch system were found to exceed the highest intensity of 106.7 mm/hr (refer to Appendix 'A' for 1:100 year Rational Method Sheet) with the exception of the most downstream ditch section (i.e., from Node 19 to Pond) where an intensity of 101.69 mm/hr was rather utilized. If an intensity of 106.7 mm/hr was used, the overall peak flow would increase from 12,814 L/s to 13,430 L/s substantially less than the free-flowing capacity of 52,735 L/s for the proposed ditch configuration. Consequently, the proposed open ditch system has the ability to convey flows generated by the July 1, 1979 storm event.

To supplement the above open ditch analysis, a hydrological analysis was also conducted to assess the performance of the SWMF under the July 1, 1979 storm event. A SWMHYMO file was, therefore, developed for the controlled Phase 2 post-development conditions of the HIP. Simulation results show that the Phase 2 post-development runoff during the July 1, 1979 storm event will be contained within the SWMF with all three of the outlet culverts flowing full in addition to approximately 210 mm of flow depth over the emergency overflow channel (refer to Appendix 'K' for SWMHYMO data input and output files). Therefore, the outlet of the SWMF has sufficient capacity to convey the July 1, 1979 historical storm event via the designated overland flow route without overtopping the banks.

### 6.6.2 Flood Potential

Draft approval Condition 12 of the draft subdivision conditions by the former Region of Ottawa-Carleton requires that "The owner shall complete a study indicating the extent of potential flooding on the property from Findlay Creek. The study including all models and assumptions shall be to the satisfaction of the South Nation River Conservation Authority." This condition was included as part of the original February 10, 1998 draft conditions (Gloucester File: S-RU-94-03).

Many changes have occurred on-site and adjacent to the site since Condition 12 was included in the draft approval for this site. Improvements to the roadside ditch were made along Rideau Road, immediately adjacent to the site. Surface runoff generated by the lands north of Rideau Road and conveyed to the small tributary located within the HIP site has now been re-directed toward the northeast corner of the site where the existing 3.8 m wide x 2.8 m high multi plate arch culvert crosses Rideau Road. A

municipal drainage report was prepared by Stantec Consulting in 2004 for this section of Findlay Creek which assessed the overall geomorphological conditions and provided recommendations for future maintenance. In addition, the SCSS conducted a flood hazard analysis. The 100 year flows from the Stantec model were plotted along the creeks modelled. Floodlines were shown in Figure 6.2.3 of the report. No floodlines were indicated for the section of Findlay Creek adjacent to the HIP site.

As indicated previously in the Section 4 of this Report, as much as 5.5 m of construction fill has been added to the site since 1994. The placed fill material on the site has eliminated the natural low lying areas and raised the site grade approximately 4.5 m above the top of creek bank. The current site grades will be maintained as a minimum for the development of the HIP subdivision. Therefore, we have no concerns about flooding on the property from Findlay Creek given the above changes to the site and improvements to the adjacent drainage network. Consequently, Condition 12 of the draft approval should be considered as being satisfied on the basis that this condition is out of date based on the current site conditions.

## **7.0 EROSION AND SEDIMENT CONTROL MEASURES DURING CONSTRUCTION**

During construction of the roadway, the collection systems (i.e., ditches, culverts, sewers, etc.) and end-of-pipe facility, appropriate erosion and sediment control measures, as outlined in MNR's "Guidelines on Erosion and Sediment Control for Urban Construction Sites," will be implemented to trap sediment on site. To ensure proper implementation, the proposed measures have been incorporated onto Drawing ESC (Drawing entitled "Erosion and Sedimentation Control Plan"). The measures shown on this Drawing were developed based on topography and site constraints. As a minimum, the following measures will be implemented during construction:

- Supply and installation of straw bale flow check dams (as per OPSD 219.180) at the upstream end of each culvert. Proposed locations of straw bale barriers are indicated on Drawing ESC.
- Supply and installation of topsoil and hydroseed along the entire open ditch system once grading has been completed for a section. Mulching will be carried out immediately after hydroseeding. This will allow for immediate bank stabilization of the system and will prevent sediment laden from occurring from exposed ditch surfaces.



- Supply and installation of light duty silt fences (as per OPSD 219.110) at the toe of slope surrounding the proposed stormwater management pond (refer to Drawing ESC for details). It is recommended that silt fences also be used to enclose borrow and stockpile areas resulting from topsoil stripping activities or any excavating activities; locations to be determined in the field during grading operations.
- If dewatering and pumping operations become necessary, filtration is proposed using sediment dewatering bags prior to discharge off-site.

All control measures will be carried out in accordance with the following documents:

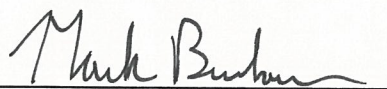
- i) "Guidelines on Erosion and Sediment Control for Urban Construction Sites" published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs and Housing, and Transportation and Communication, Association of Construction Authorities of Ontario, and Urban Development Institute, Ontario, May 1987.
- ii) "Erosion and Sediment Control" Training Manual by Ministry of Environment, Spring 1998.
- iii) Applicable Regulations and Guidelines of the Ministry of Natural Resources. As a minimum, during the construction of the conveyance systems, the following Stormwater Management Practices will be used:

Any stockpiled material will be kept on flat areas during construction, well away from any natural flow paths. In the event that the stockpile is placed in other areas where potential washoff to the conveyance system is expected, silt fences will be installed to enclose the materials and prevent any washoff to the conveyance system.

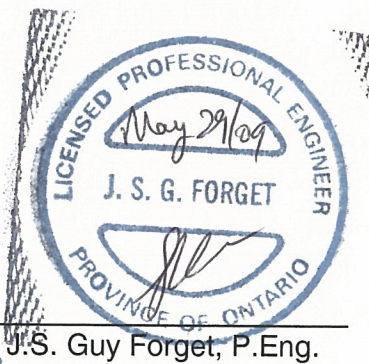
## 8.0 SUMMARY AND CONCLUSION

1. This Stormwater Management Report has been prepared to present a complete approach in achieving the stormwater criteria developed as part of the approved document entitled "Shields Creek Subwatershed Study."
2. Stormwater servicing for the proposed HIP has been designed using the dual drainage concept. Storm servicing will be carried out with the use of an open ditch/culvert system. The open ditch system has been designed to convey the 1:00 year peak flow rates. Similarly, the culverts have been sized to convey the 1:10 year flow without any overtopping.
3. To fulfil the design criteria associated with water quality (as per the SCSS), it is proposed to provide both on-site oil/grit separators and infiltration storage volume within the roadside open ditch system. As per the requirements set out in Table 3.2 of the MOE SWMPDM, a total infiltration volume of 62.5 m<sup>3</sup> is required under Phase 2 to achieve a "normal" level of protection (i.e., TSS removal of 70%).
4. Water balance and infiltration requirements were not implemented due to existing site conditions and proposed industrial use development.
5. The 2-year post-development peak flow will be controlled to 50% of the 2-year pre-development peak flow. Therefore, meeting the SCSS recommendations associated with erosion potential.
6. Simulation results presented in Tables 5 and 6 show that proposed infrastructure will maintain peak flows below pre-development levels for both Phase 1 and Phase 2 of the HIP. Consequently, this design criterion (peak flow control) will be fulfilled.
7. A detailed Erosion and Sedimentation Control Plan has been prepared to reduce the impact of construction activities on Findlay Creek.

Prepared by:



Mark Buchanan, E.I.T.



Reviewed by:

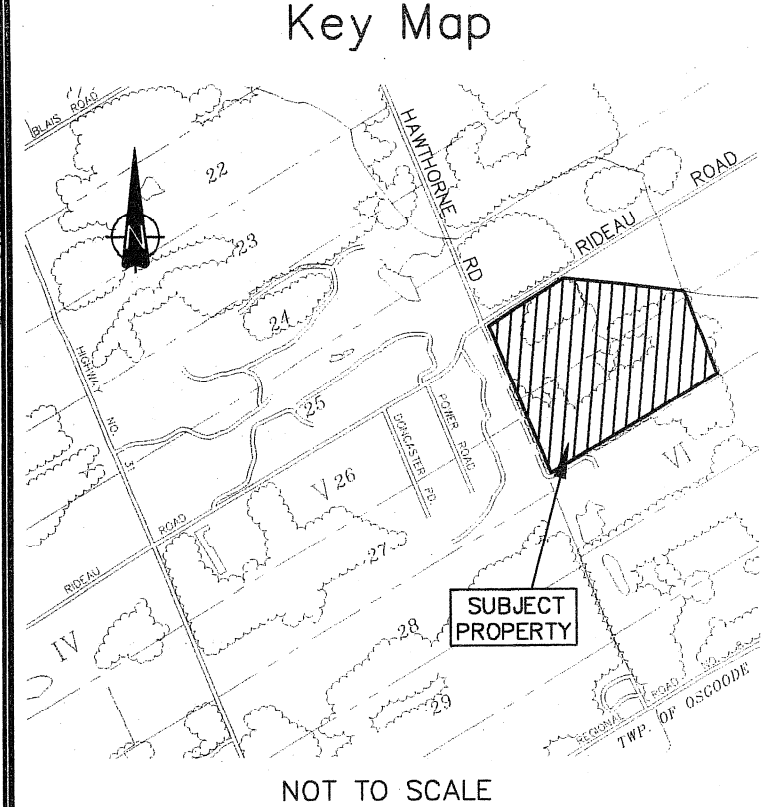
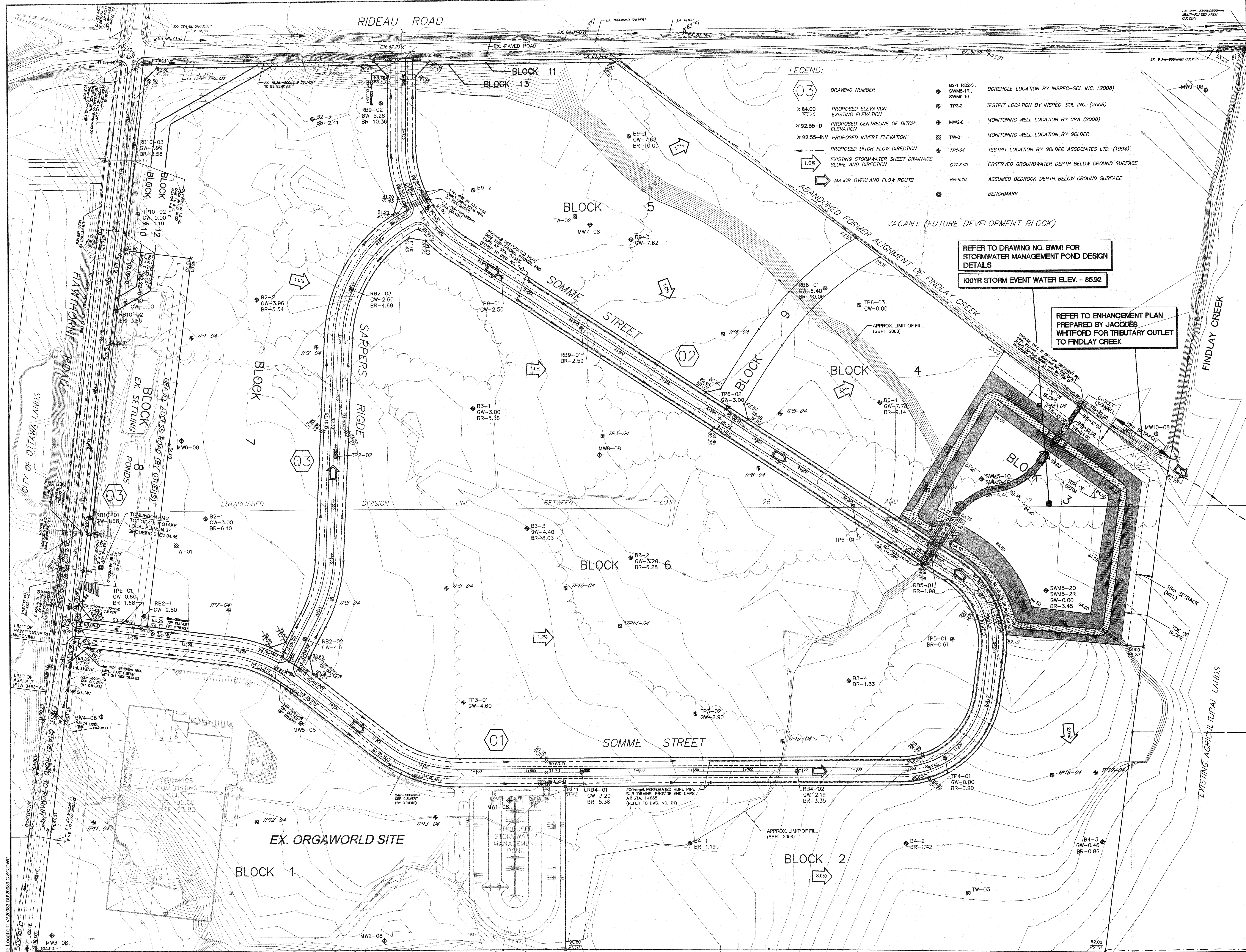
J.S. Guy Forget, P.Eng.



Reviewed by:

Derrick Upton, P.Eng.

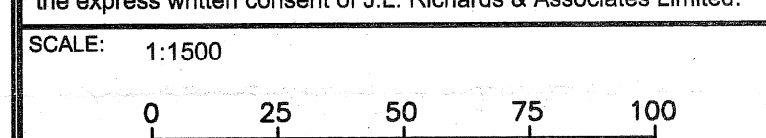




GENERAL NOTES:  
1. HAWTHORNE INDUSTRIAL PARK DRAWINGS TO BE READ IN CONJUNCTION WITH THE GEOTECHNICAL INVESTIGATION REPORT No. T020556-A1 PREPARED BY INSPEC-SOL DATED JANUARY 30, 2009.  
2. A GEOTECHNICAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO IS TO INSPECT ALL SUBGRADE SURFACES FOR FOOTINGS AND PAVEMENT STRUCTURES PRIOR TO CONSTRUCTION.  
3. ALL MATERIAL AND CONSTRUCTION METHODS TO BE IN ACCORDANCE WITH ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS (OPSS) AND CITY OF OTTAWA GUIDELINES.

NO.	ISSUE	DATE
3	ISSUED FOR M.O.E. APPROVAL	28/05/09
2	REVISED PER CITY COMMENTS	30/04/09
1	ISSUED FOR CITY APPROVAL	12/02/09

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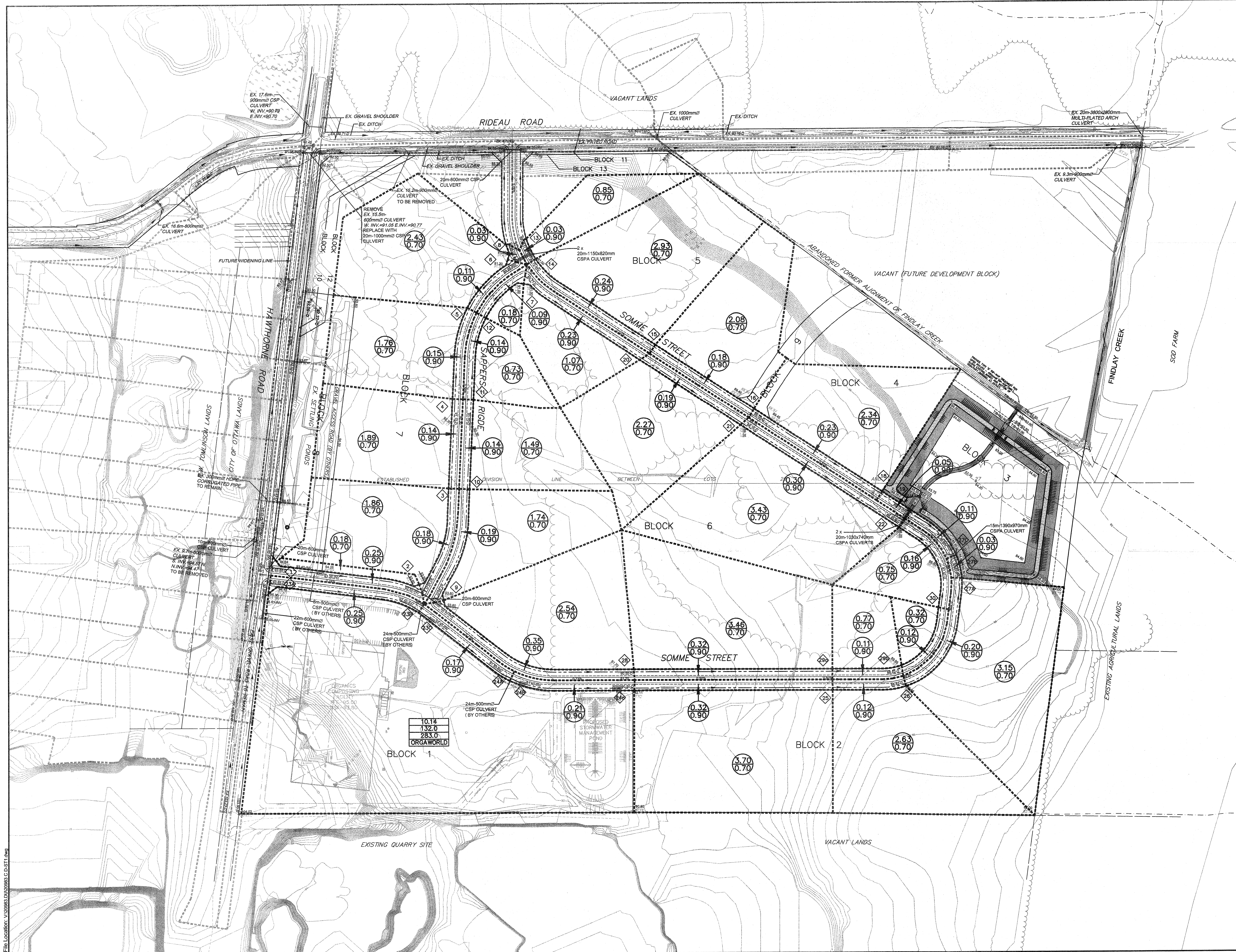
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LICENSED PROFESSIONAL ENGINEER  
D. P. UPTON  
PROVINCE OF ONTARIO

PROJECT: **HAWTHORNE INDUSTRIAL PARK**

DRAWING: **SITE SERVICING AND GRADING**

DESIGN: M.B.	DRAWING NO.: <b>SG</b>
DRAWN: T.S.	JLR NO.: 20983
CHECKED: D.U.	
PLOTTED: May 28, 2009	





Key Map

NOT TO SCALE

**LEGEND**

----- DRAINAGE BOUNDARY

2.91  
0.70

AREA IN HECTARES  
\* RUNOFF COEFFICIENT (C)

10.14  
132.0  
283.0  
ORGAWORLD

DRAINAGE AREA (ha)  
10 YEAR PEAK FLOW (l/s)  
100 YEAR PEAK FLOW (l/s)  
ORGAWORLD SITE

28

NODE LOCATION NUMBER

PROPOSED DITCH AND FLOW DIRECTION

NOTE: RUNOFF COEFFICIENT (C) FOR DEVELOPMENT AREA IS BASED ON A WEIGHTED AVERAGE OF 0.70, WHILE ROADWAYS ARE 0.90.

03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

NO.	ISSUE	DATE
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01	ISSUED FOR CITY APPROVAL	12/02/09

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0 25 50 100 150

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Tel: 613 544 1424  
Fax: 613 544 5679

PROFESSIONAL STAMP

PROJECT NORTH

PROJECT:

**HAWTHORNE INDUSTRIAL PARK**

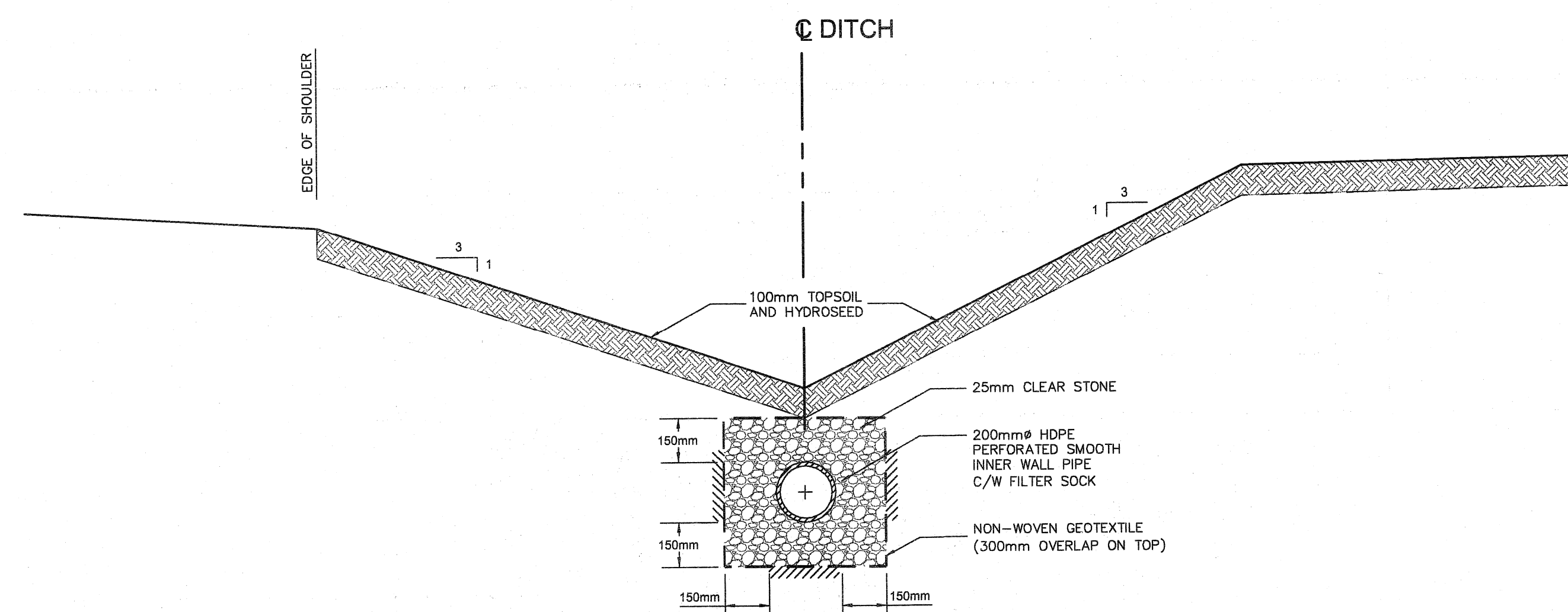
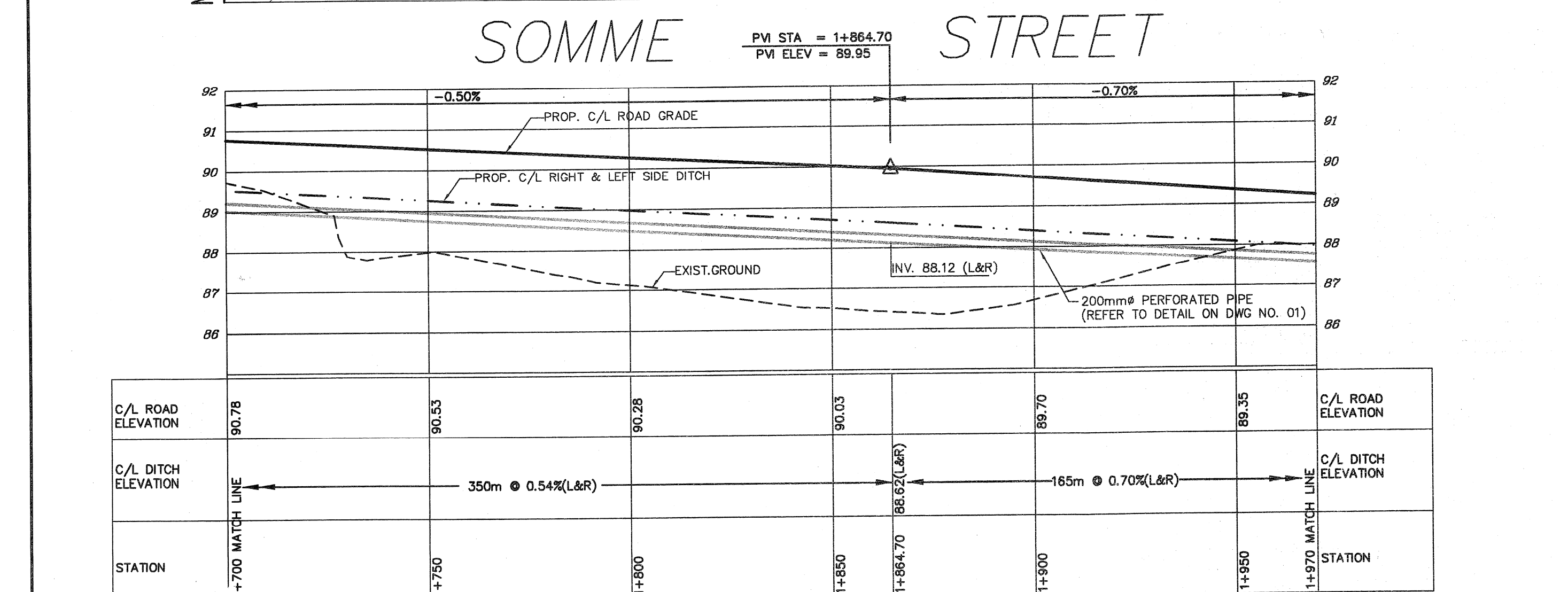
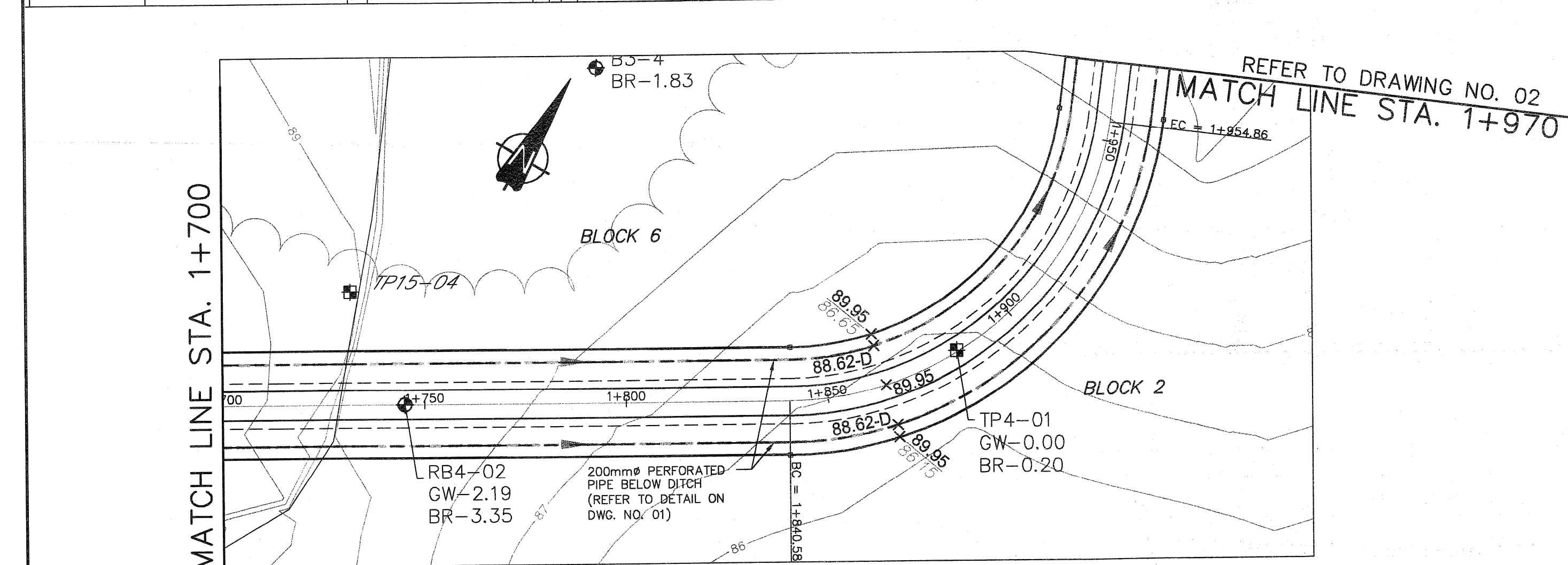
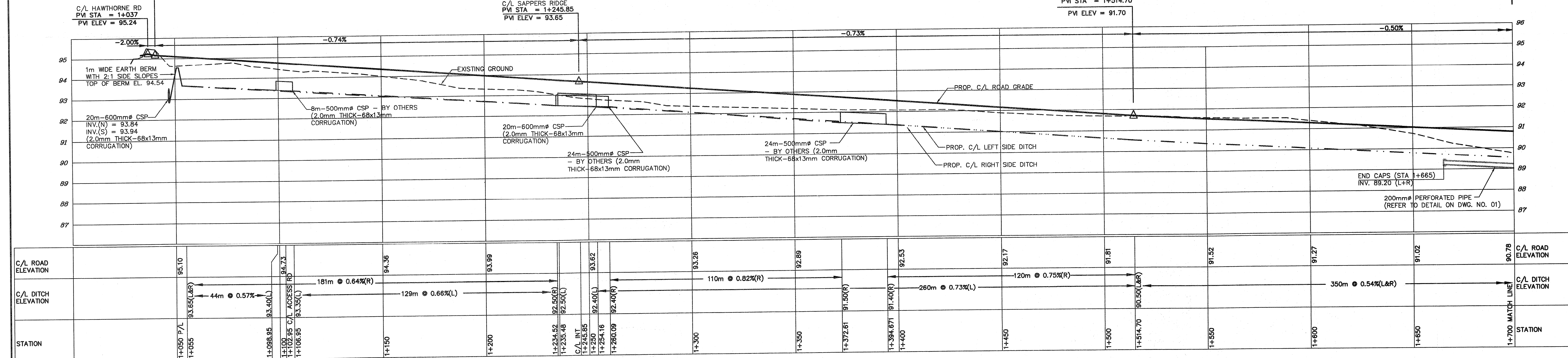
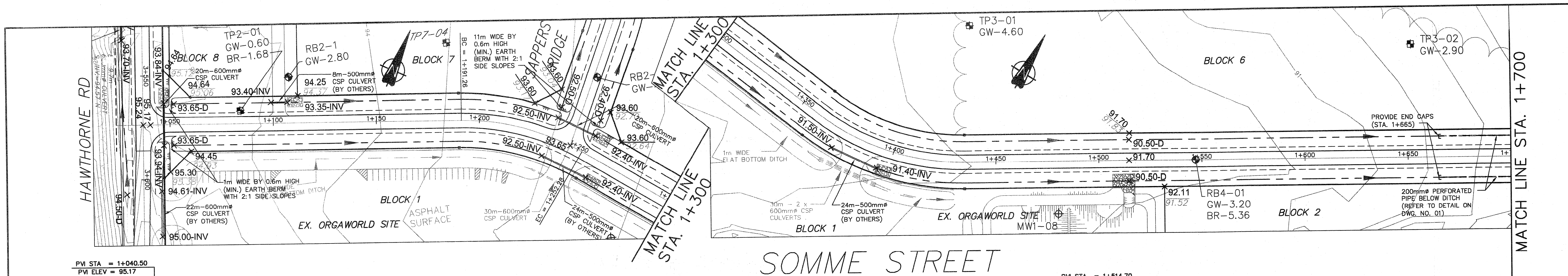
DRAWING:

**STORM DRAINAGE AREA PLAN**









ROADSIDE DITCH PERFORATED PIPE INSTALLATION (TYPICAL)  
N.T.S.

NOTE:  
1. SUB-DRAIN TO BE NON-PERFORATED PIPE UNDER CULVERTS.  
2. WORKS TO BE CONSTRUCTED AS PER CITY OF OTTAWA DETAIL DRAWING NO.s S9 AND S26.

- LEGEND:**
- PROPOSED DITCH AND FLOW DIRECTION
  - PROPOSED CULVERT
  - PROPOSED RIP-RAP
  - PROPOSED C/L ELEVATION
  - PROPOSED DITCH ELEVATION
  - PROPOSED INVERT
  - PROPOSED ELEVATION EXISTING ELEVATION
  - BOREHOLE LOCATION BY INSPEC-SOL INC. (2008)
  - TEST PIT LOCATION BY INSPEC-SOL INC. (2008)
  - MONITORING WELL LOCATION BY CRA (2008)
  - MONITORING WELL LOCATION BY GOLDER ASSOCIATES LTD. (1994)
  - TEST PIT LOCATION BY GOLDER ASSOCIATES LTD. (1994)
  - OBSERVED GROUNDWATER DEPTH BELOW GROUND SURFACE
  - ASSUMED BEDROCK DEPTH BELOW GROUND SURFACE

3	ISSUED FOR M.O.E. APPROVAL	28/05/09
2	REVISED PER CITY COMMENTS	30/04/09
1	ISSUED FOR CITY APPROVAL	12/02/09

NO. ISSUE DATE

SCALE:  
1:1000 (HORIZ.)  
1:100 (VERT.)

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LICENSED PROFESSIONAL ENGINEER  
D. P. UPTON  
PROVINCE OF ONTARIO

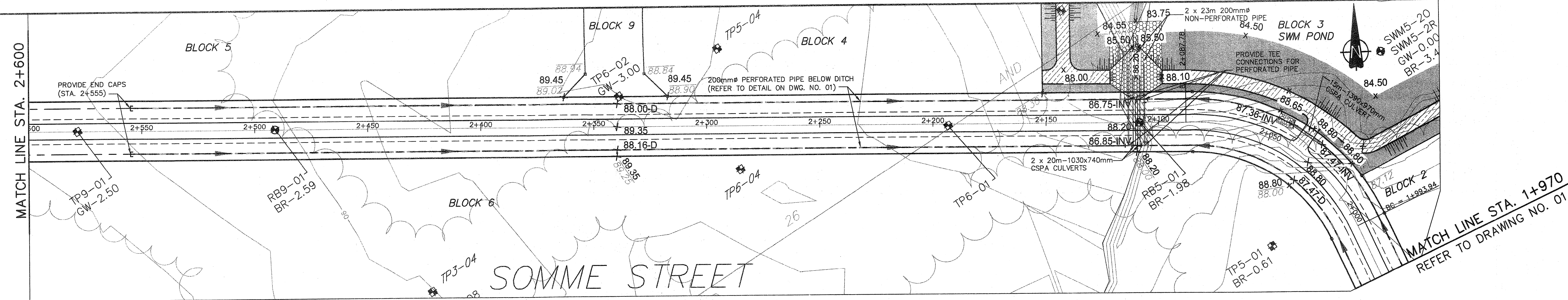
PROJECT: **HAWTHORNE INDUSTRIAL PARK**

DRAWING: **PLAN & PROFILE SOMME STREET HAWTHORNE RD TO STA. 1+970**

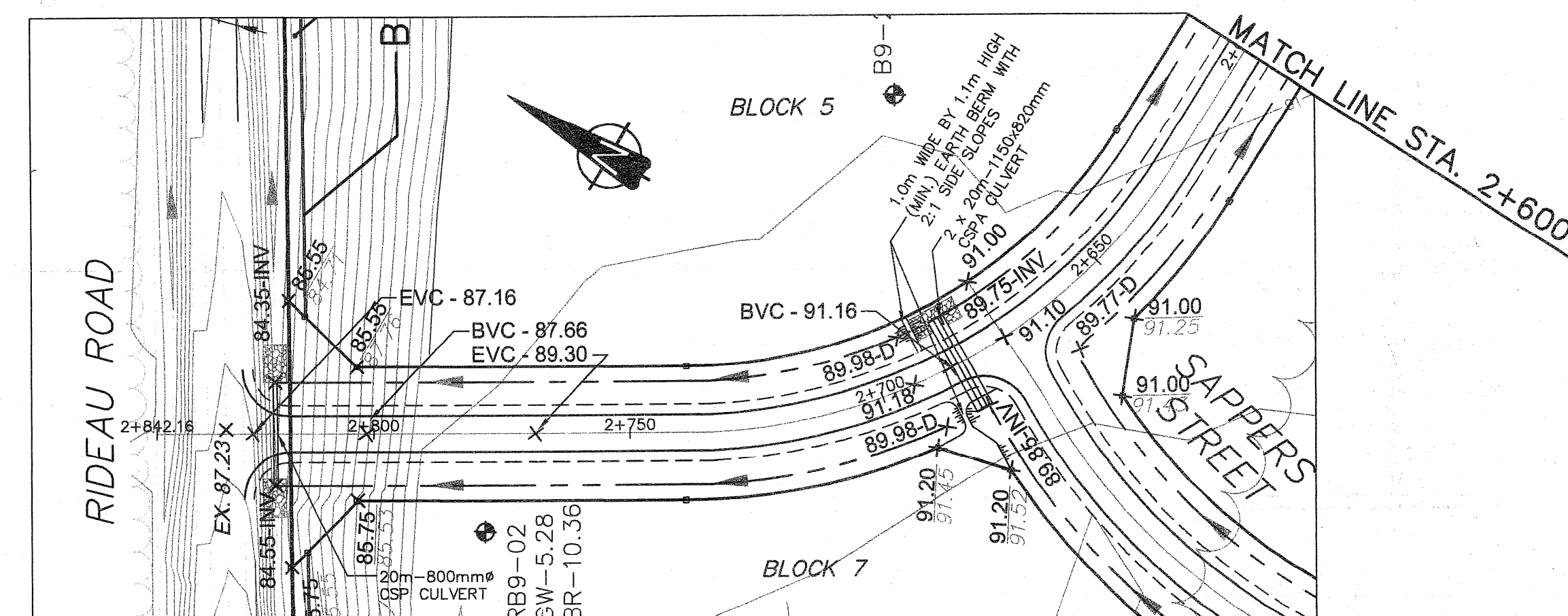
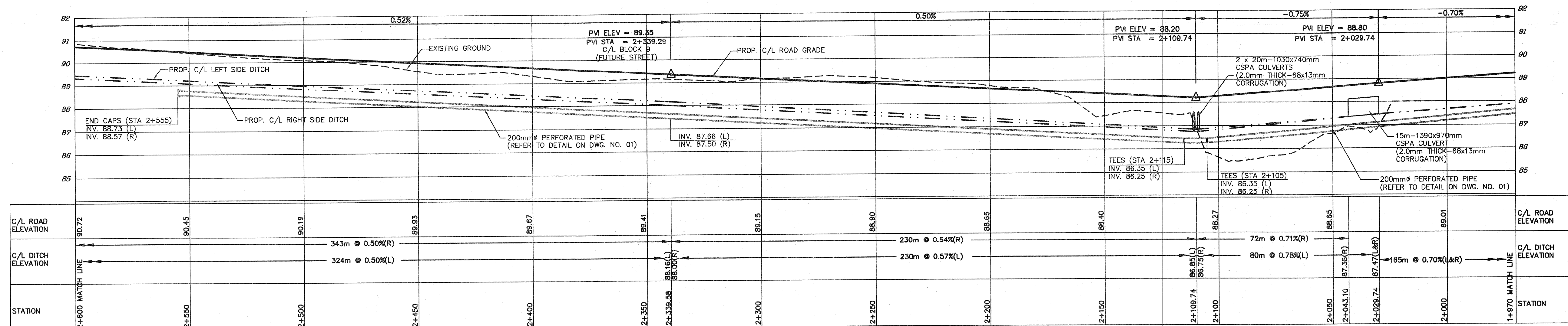
DESIGN: M.B.  
DRAWN: T.S.  
CHECKED: D.U.  
PLOTTED: May 28, 2009

DRAWING NO.: **01**  
JLR NO: 20983

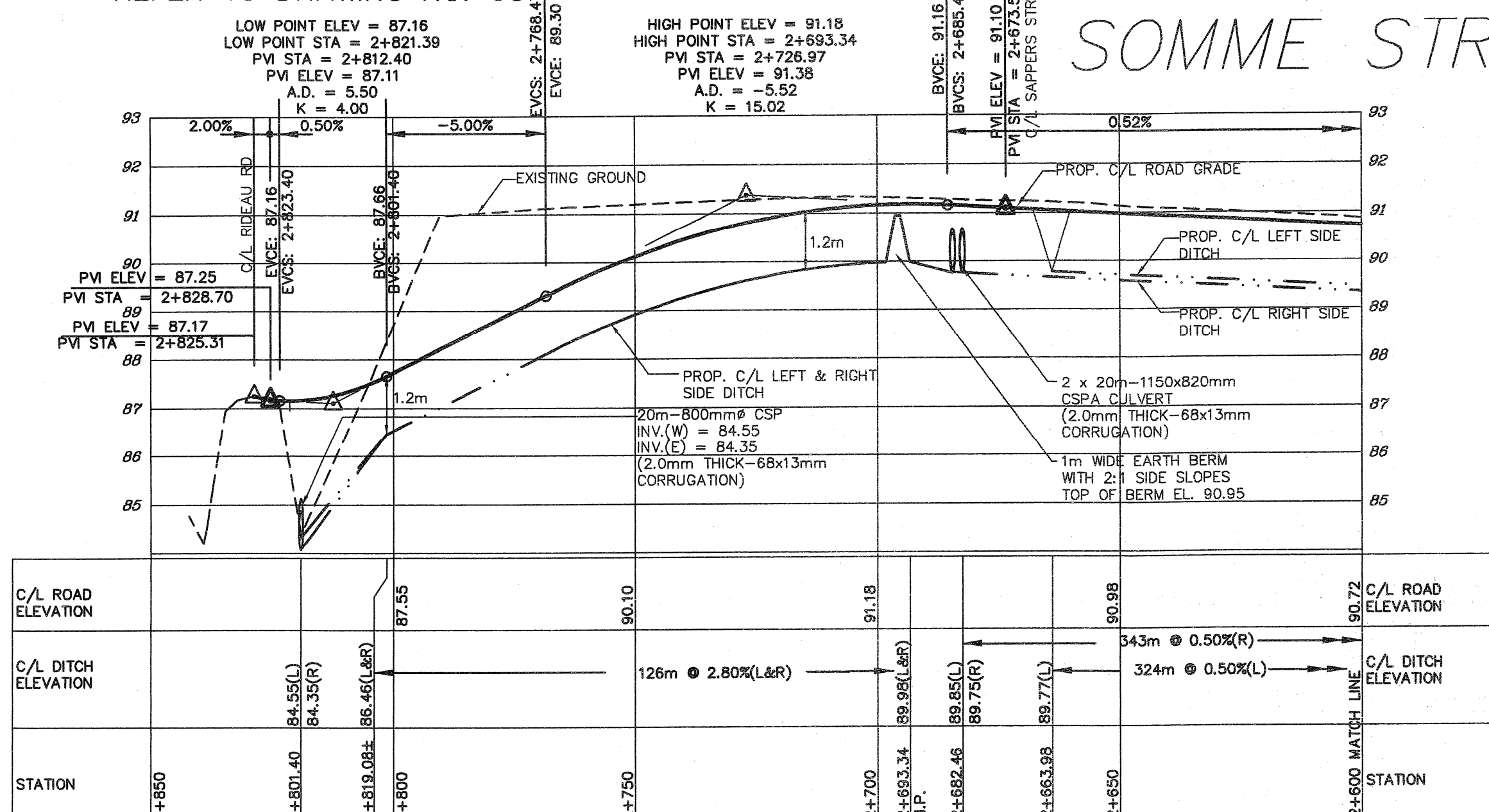




MATCH LINE STA. 1+970  
REFER TO DRAWING NO. 01



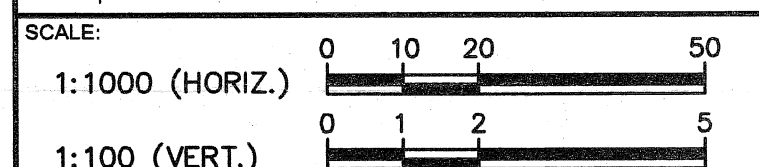
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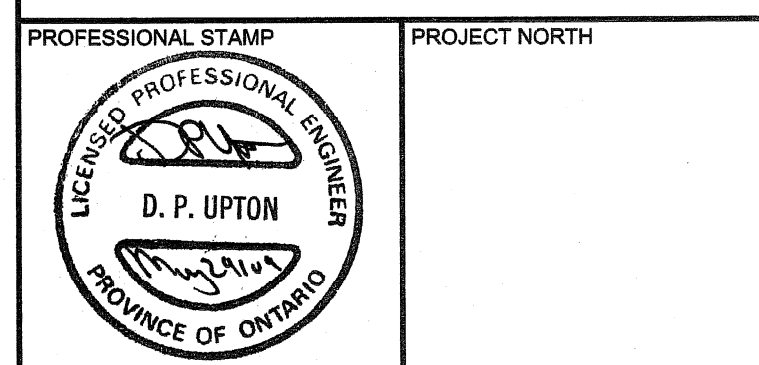
- LEGEND:**
- PROPOSED DITCH AND FLOW DIRECTION
  - PROPOSED CULVERT
  - PROPOSED RIP-RAP
  - PROPOSED C/L ELEVATION
  - PROPOSED DITCH ELEVATION
  - PROPOSED INVERT
  - PROPOSED ELEVATION EXISTING ELEVATION
  - B21, R82-3, SWM5-1R, SWM5-10 BOREHOLE LOCATION BY INSPEC-SOL INC. (2008)
  - TP3-2 TEST PIT LOCATION BY INSPEC-SOL INC. (2008)
  - MW2-8 MONITORING WELL LOCATION BY CRA (2008)
  - TW-3 MONITORING WELL LOCATION BY GOLDER ASSOCIATES LTD.
  - TP1-04 TEST PIT LOCATION BY GOLDER ASSOCIATES LTD. (1994)
  - GW-3.00 OBSERVED GROUNDWATER DEPTH BELOW GROUND SURFACE
  - BR-8.10 ASSUMED BEDROCK DEPTH BELOW GROUND SURFACE

3	ISSUED FOR M.O.E. APPROVAL	28/05/09
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1	ISSUED FOR CITY APPROVAL	12/02/09
NO.	ISSUE	DATE

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Fax: 613 728 6012



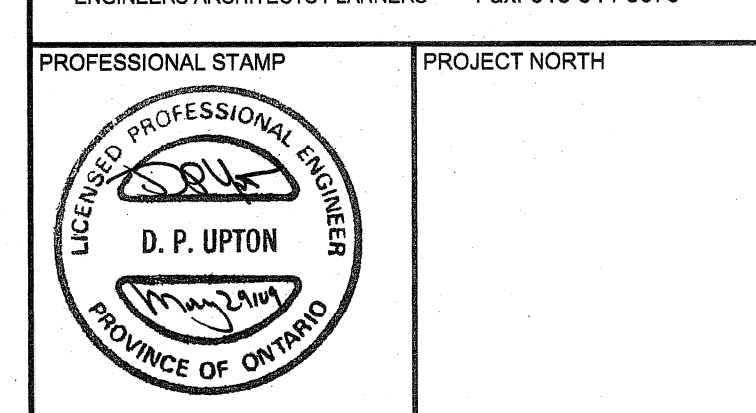
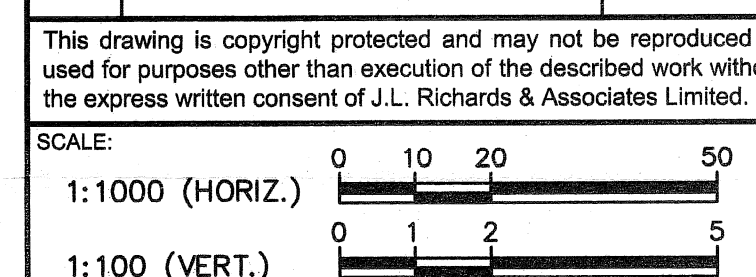
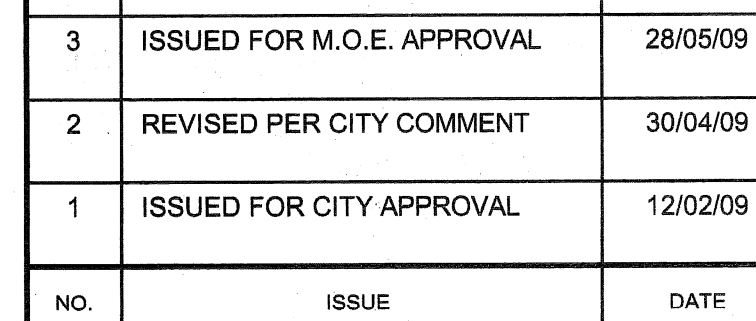
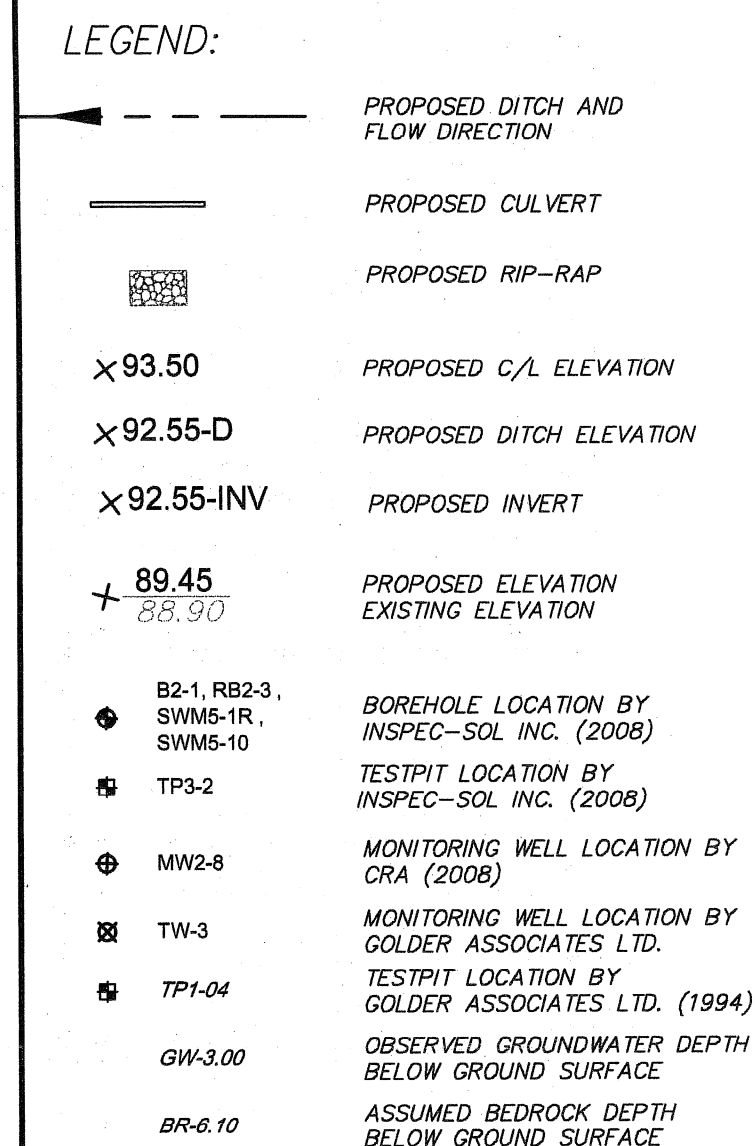
PROJECT: HAWTHORNE INDUSTRIAL PARK

DRAWING: PLAN & PROFILE SOMME STREET STA. 1+970 TO RIDEAU ROAD

DESIGN: M.B.  
DRAWN: T.S.  
CHECKED: D.U.  
PLOTTED: May 28, 2009

DRAWING NO: 02  
JLR NO: 20983





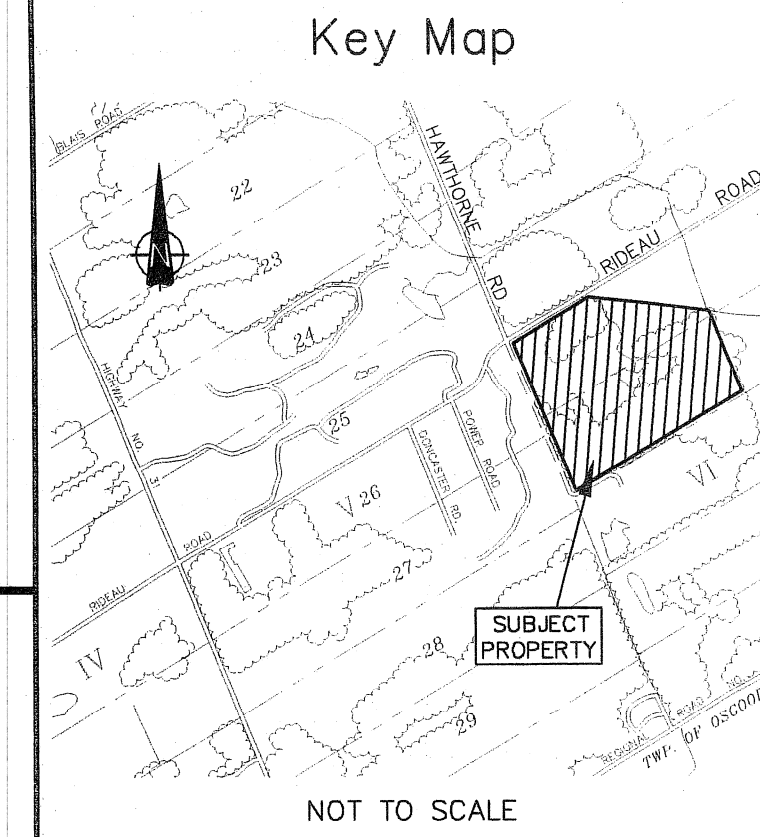
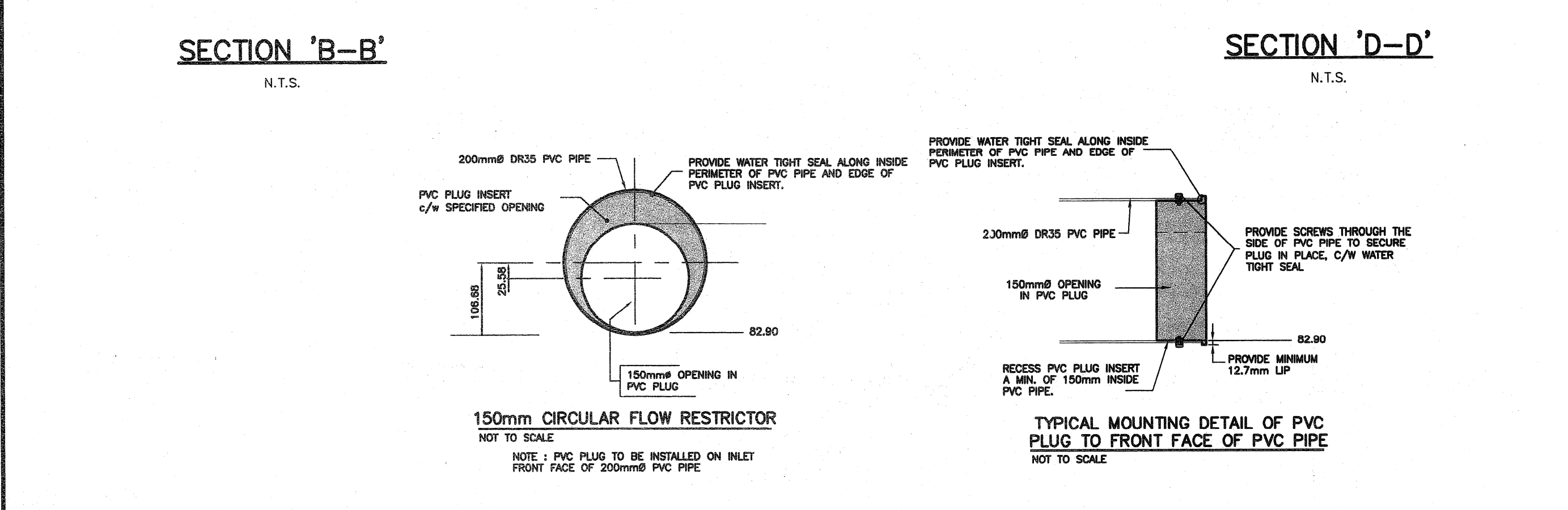
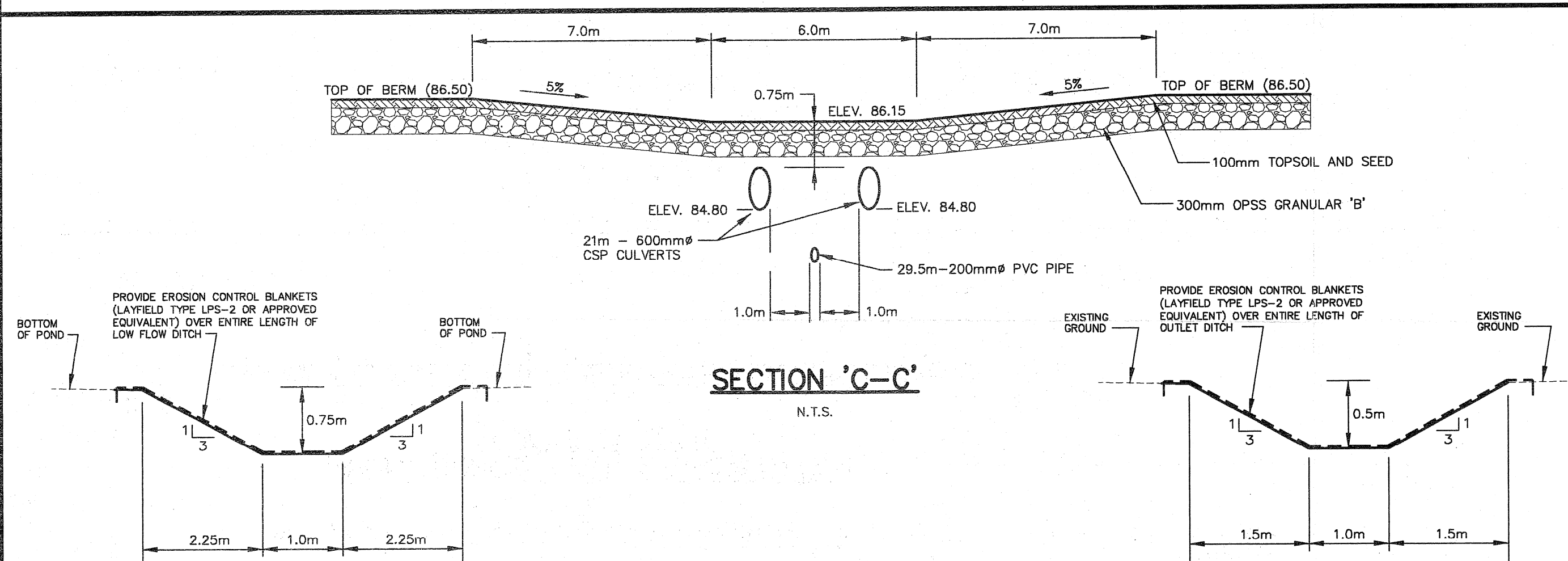
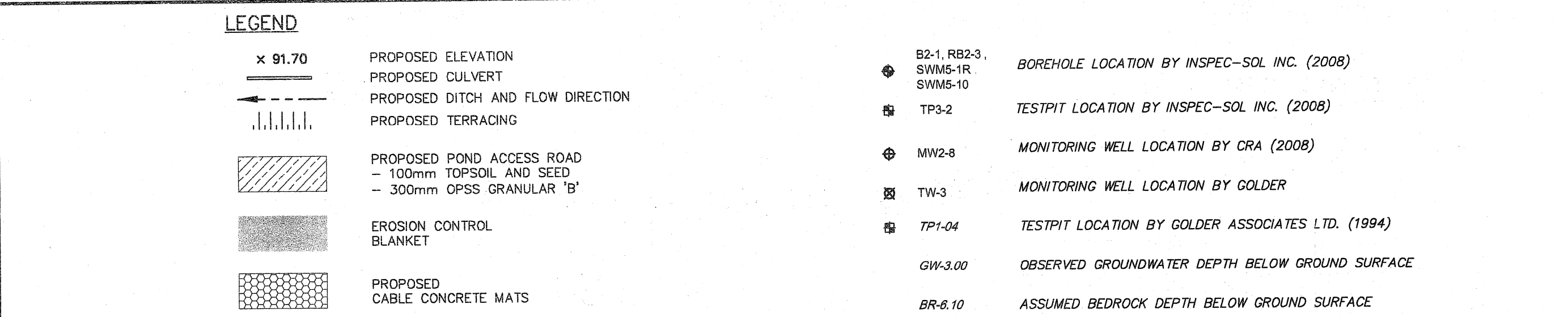
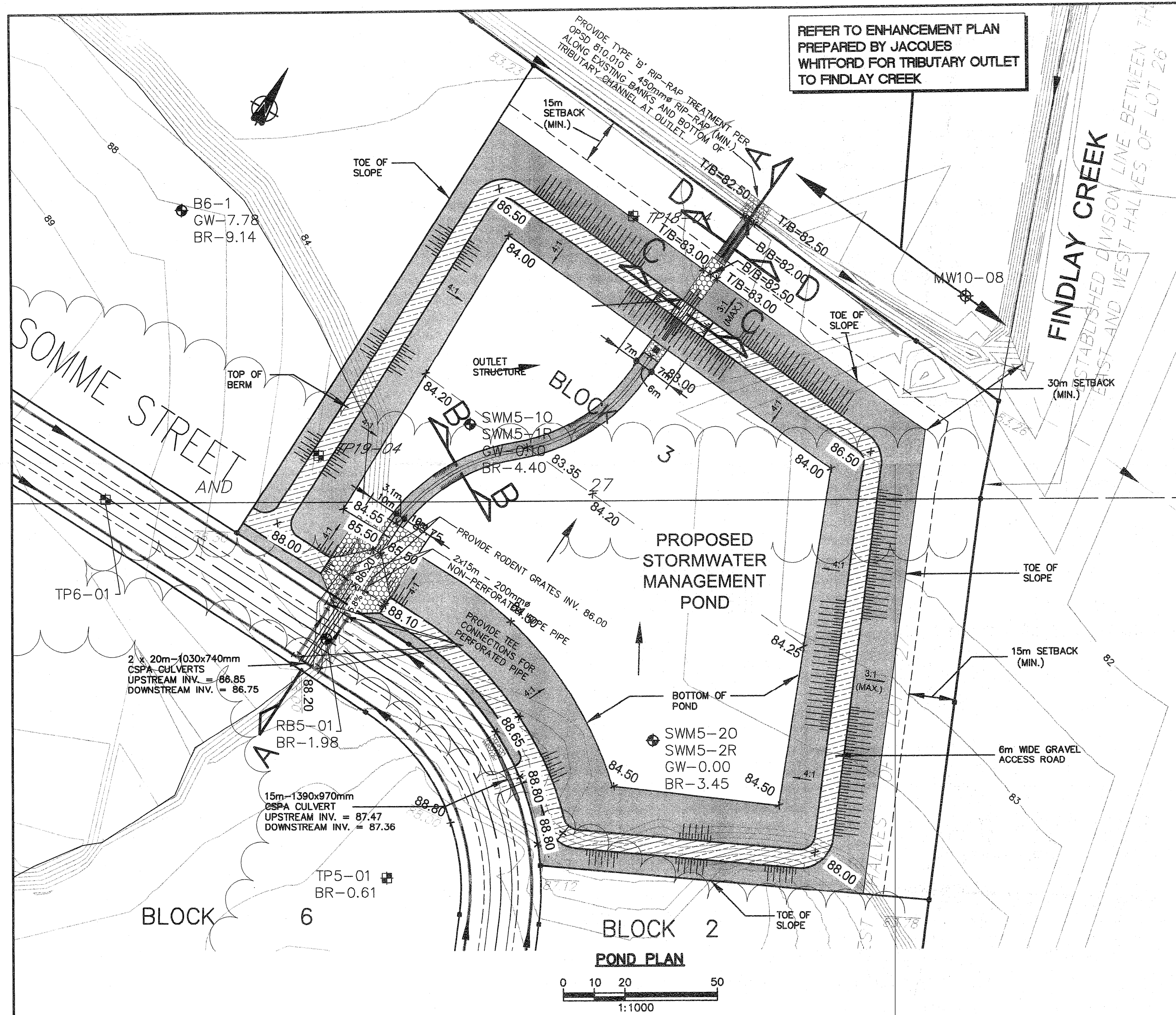
PROJECT:

HAWTHORNE  
INDUSTRIAL  
PARK

DRAWING: PLAN & PROFILE  
SAPPERS RIDGE  
AND  
HAWTHORNE ROAD EXTENSION

DESIGN: M.B.	DRAWING NO.: <b>03</b>
DRAWN: T.S.	
CHECKED: D.U.	JLR NO:
PLOTTED:	20082

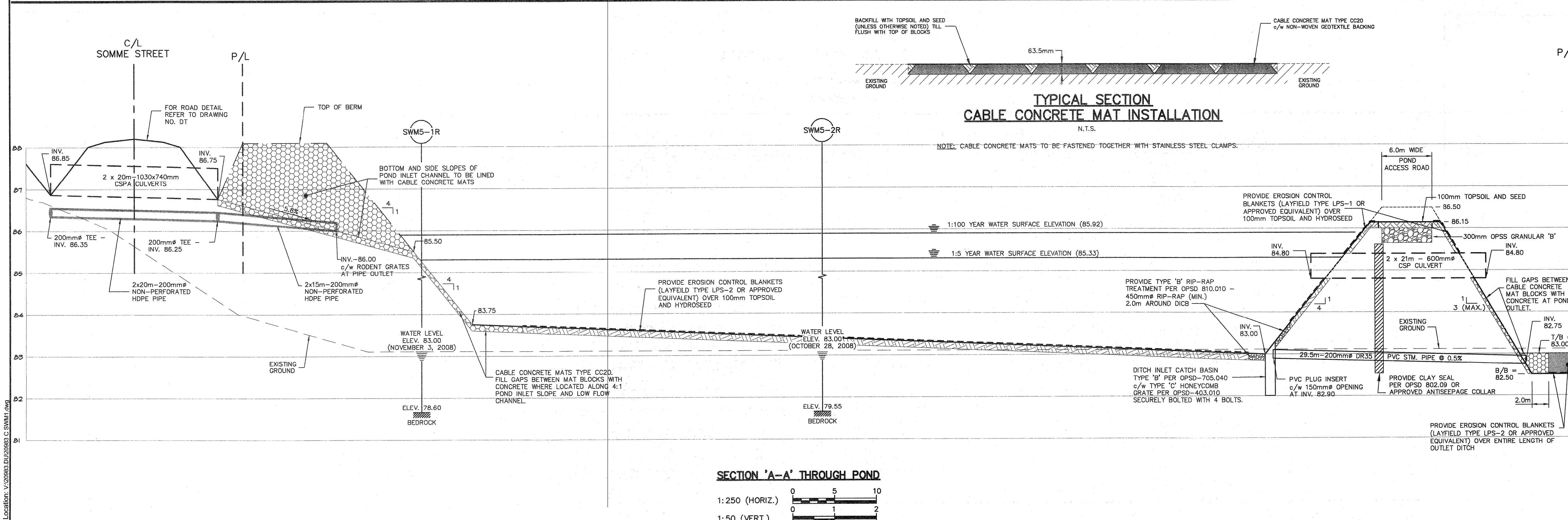




3	ISSUED FOR M.O.E. APPROVAL	28/05/09
2	REVISED PER CITY COMMENTS	30/04/09
1	ISSUED FOR CITY APPROVAL	12/02/09
NO.	ISSUE	DATE

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SCALE: AS SHOWN



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**J.L. Richards**  
 ENGINEERS ARCHITECTS PLANNERS

**PROFESSIONAL STAMP**  
 D. P. UPTON  
 PROVINCE OF ONTARIO

**PROJECT NORTH**

**HAWTHORNE INDUSTRIAL PARK**

**STORMWATER MANAGEMENT POND PLAN AND SECTIONS**

DESIGN: M.B.  
 DRAWN: T.S.  
 CHECKED: D.U.  
 PLOTTED: May 28, 2009

DRAWING NO.: **SWM1**  
 JLR NO: 20983



#### GENERAL NOTES FOR EROSION AND SEDIMENTATION CONTROL MEASURES DURING CONSTRUCTION :

During construction activities appropriate erosion and sediment control measures, as outlined in MNR's "Guidelines on Erosion and Sediment Control for Urban Construction Sites", shall be implemented to trap sediment on-site.

As a minimum, the following erosion and sedimentation control measures will be provided during construction:

- supply and install straw bale flow check dams (per OPSD 219.180) upstream of all culvert installations at locations shown on Drawing NO. ESC. Do not remove straw bale barriers until the upstream vegetation has been established;
- supply and install silt fence barrier (per OPSD 219.110) at locations shown on Drawing NO. ESC; and

- supply and install silt fence barrier (per OPSD 219.110) to enclose all borrow and stockpile areas resulting from topsoil stripping activities or any excavating activities (i.e. exact location to be determined during construction).

Furthermore, if dewatering and pumping operations become necessary, sediment dewatering bags shall be used to filter sediment prior to releasing groundwater into the receiving stream.

All control measures will be carried out in accordance with the following documents:

- 1) "Guidelines on Erosion and Sediment Control for Urban Construction Sites" published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs, and Transportation & Communication, Association of Construction Authorities of Ontario and Urban Development Institute, Ontario, May 1987.
- 2) "Erosion and Sediment Control" Training Manual by Ministry of Environment, Spring 1998.
- 3) Applicable Regulations and guidelines of the Ministry of Natural Resources. As a minimum, during the construction of municipal services, the following Stormwater Management Practices will be used:

- Any stockpiled material will be kept on flat areas during construction, well away from any natural flow paths. In the event that the stockpile is placed in other areas where potential washoff to the conveyance system is expected, silt fences (per OPSD 219.110) will be installed to enclose the materials and prevent any washoff to the conveyance system.
- All pumped stormwater/groundwater will be filtered through sediment dewatering bags prior to its release to the receiving stream.

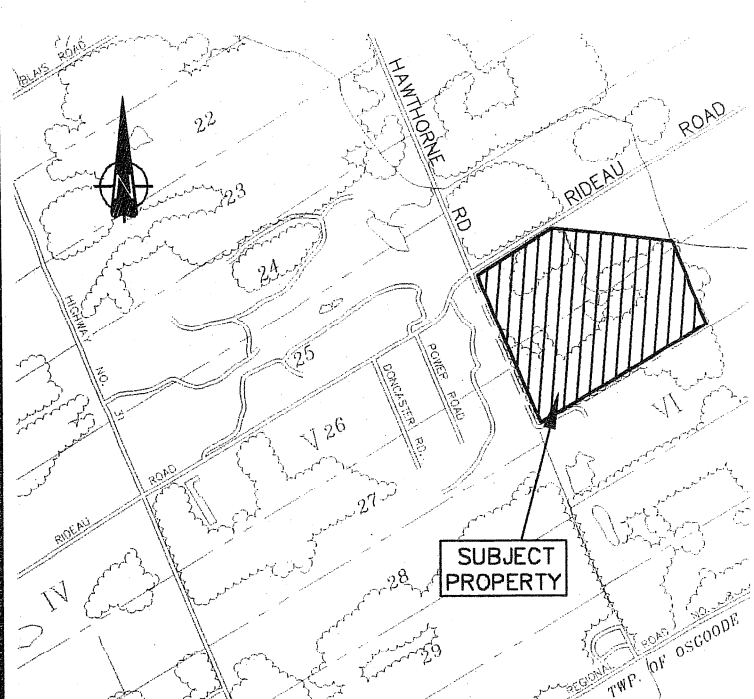
Sediment and Erosion control measures shall be implemented prior to work and maintained during the work phase by the general contractor to prevent entry of sediment into the receiving stream. All sediment and erosion control measures shall be inspected daily by the general contractor to ensure that they are functioning properly and are being maintained and/or upgraded as required. If the sediment and erosion control measures are not functioning properly, no further work shall occur until the problem has been addressed and rectified.

All materials and equipment used for the purpose of site preparation and project completion shall be operated and stored in a manner that prevents any deleterious substances (i.e. petroleum products, silt, etc.) from entering the receiving stream. Vehicle and equipment re-fueling and maintenance shall be conducted away from drainage channels. Any part of equipment entering drainage channels shall be free of fluid leaks and externally cleaned/degreased to prevent any deleterious substances from entering the receiving stream.

#### GENERAL NOTES :

1. ALL STOCKPILED EXCAVATED MATERIAL IS TO BE LOCATED A MINIMUM OF 10m FROM ALL WATERCOURSES AND IS TO BE ENCLOSED WITH A SILT SCREEN (PER OPSD 219.110) OR LIGHT-DUTY STRAW BALE BARRIER (PER OPSD 219.100).
2. ALL SILT CONTROL MEASURES ARE TO BE INSPECTED ONCE PER MONTH AND AFTER EACH SIGNIFICANT RAINFALL EVENT TOTALLING 10mm OR GREATER. GENERAL CONTRACTOR TO REPAIR AS REQUIRED.
3. HYDROSEEDING OF ALL DITCHES IS TO BE PROVIDED IMMEDIATELY FOLLOWING FINAL SHAPING/GRADING.
4. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTING BEST MANAGEMENT PRACTICES TO PROVIDE PROTECTION OF THE RECEIVING WATERCOURSE DURING ALL PHASES OF CONSTRUCTION.
5. SEDIMENT AND EROSION CONTROL MEASURES MAY BE MODIFIED IN THE FIELD AT THE DISCRETION OF THE CITY OF OTTAWA SITE INSPECTOR AND/OR THE LOCAL CONSERVATION AUTHORITY.

#### Key Map



NOT TO SCALE

#### LEGEND

- x 91.70 PROPOSED ELEVATION
- PROPOSED CULVERT
- PROPOSED DITCH AND FLOW DIRECTION
- PROPOSED TERRACING
- LIGHT-DUTY SILT FENCE BARRIER TO OPSD-219.110
- PROPOSED EROSION CONTROL BLANKET
- PROPOSED CABLE CONCRETE MATS
- PROPOSED LOCATION OF STRAW BALE FLOW CHECK DAM TO OPSD-219.180
- PROPOSED RIP-RAP TREATMENT TYPE 'B' TO OPSD-810.010

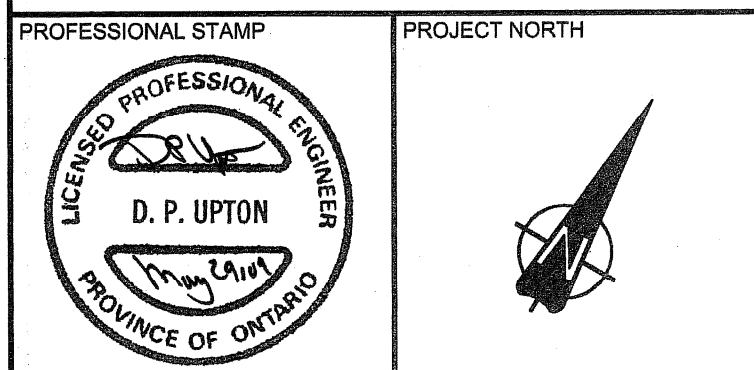
NO.	ISSUE	DATE
03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

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SCALE: 1:2000



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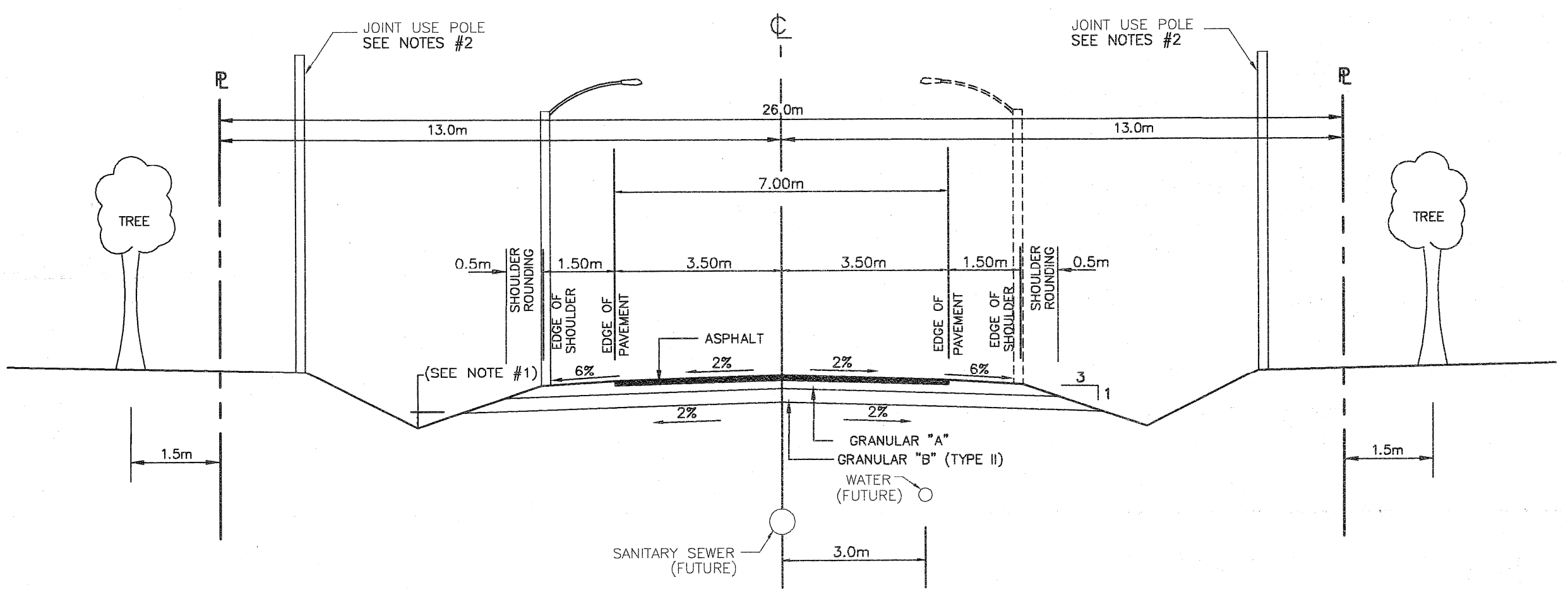
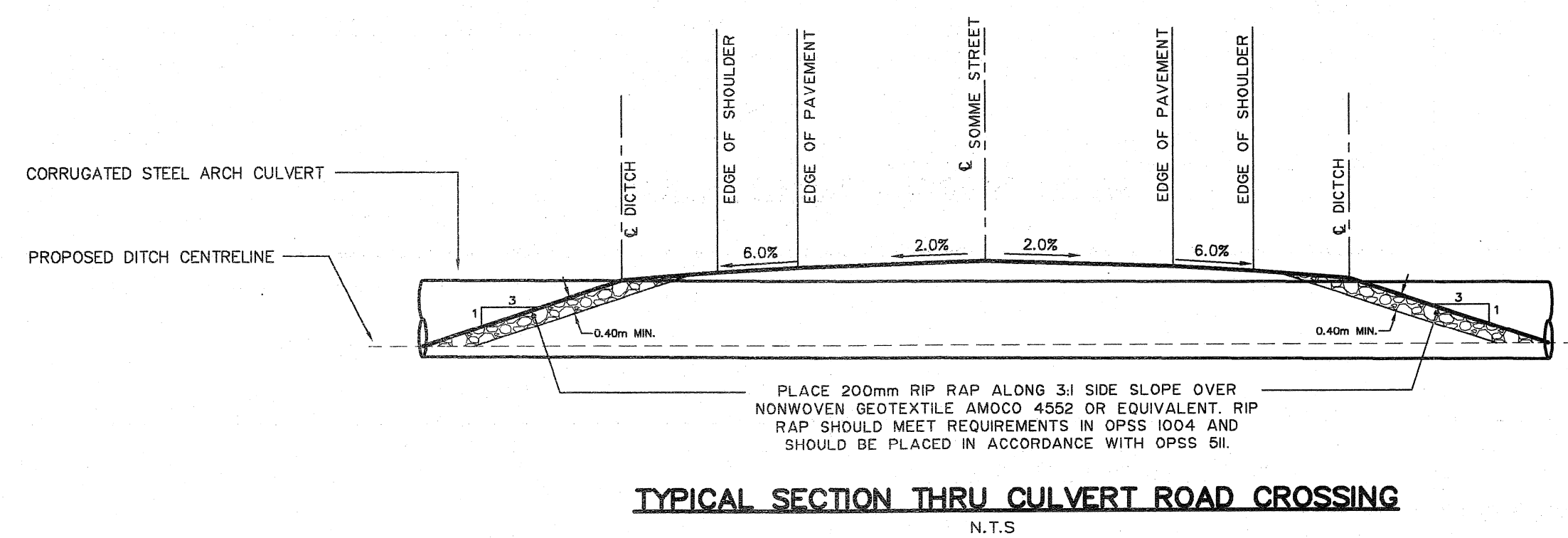
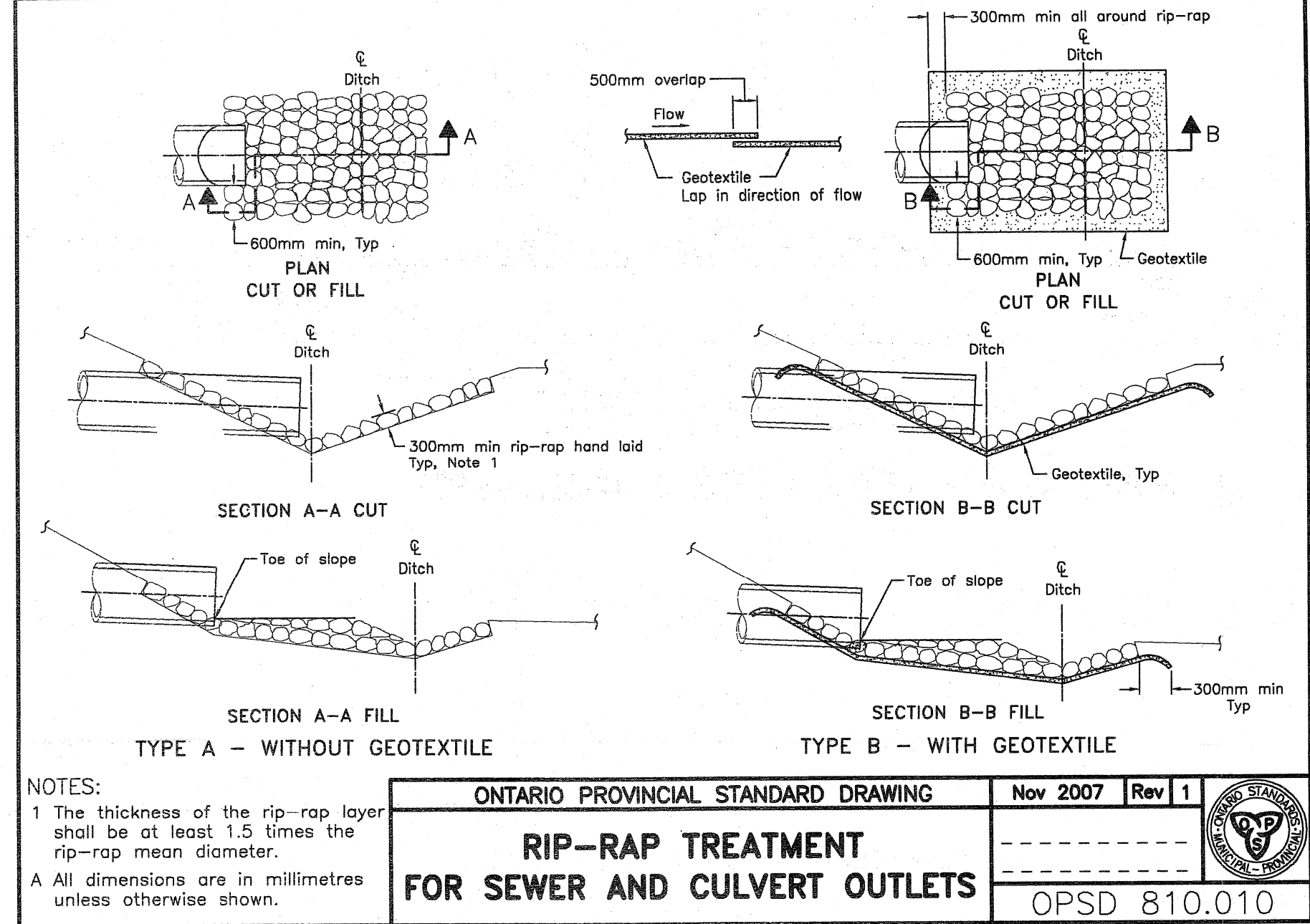
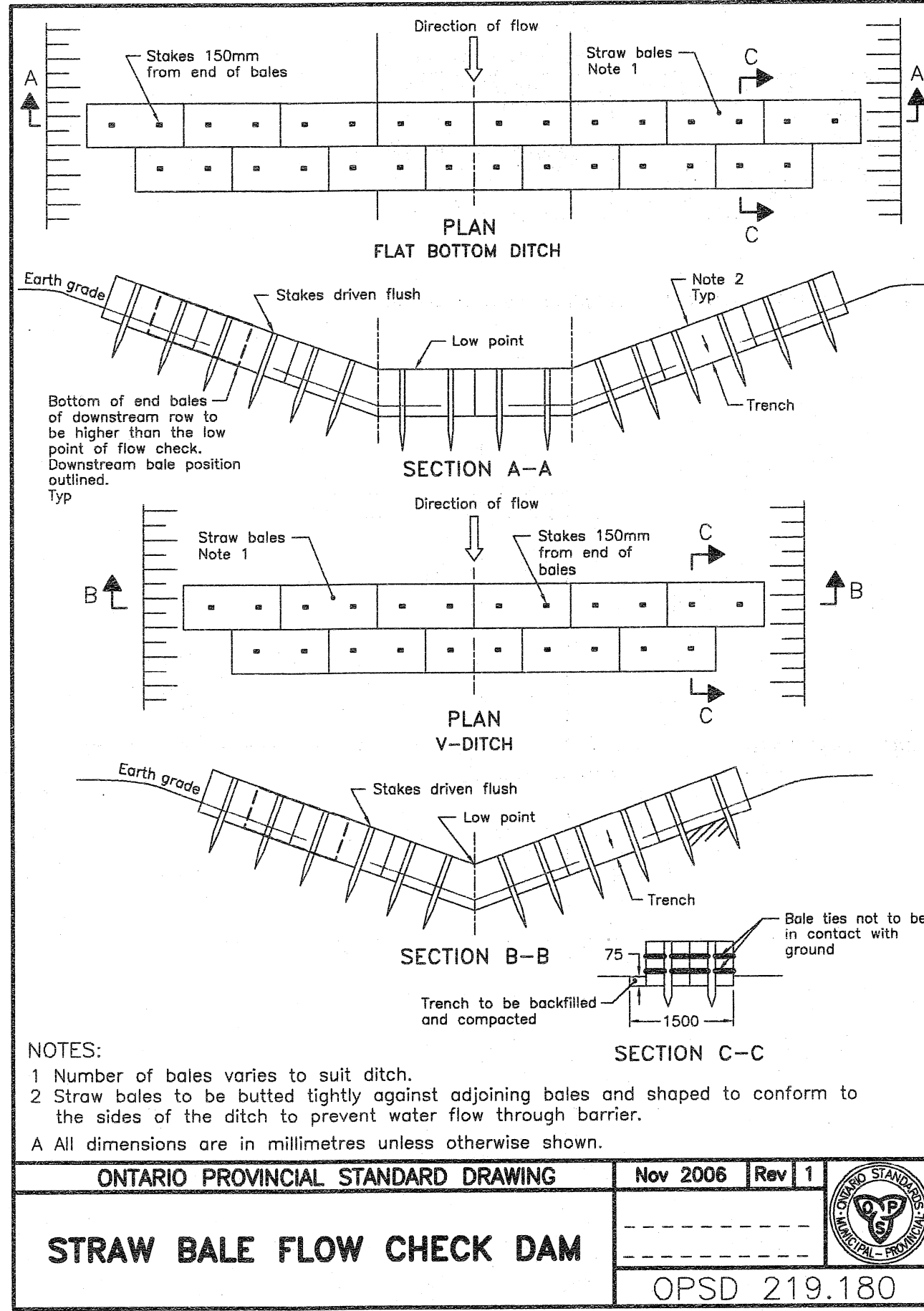
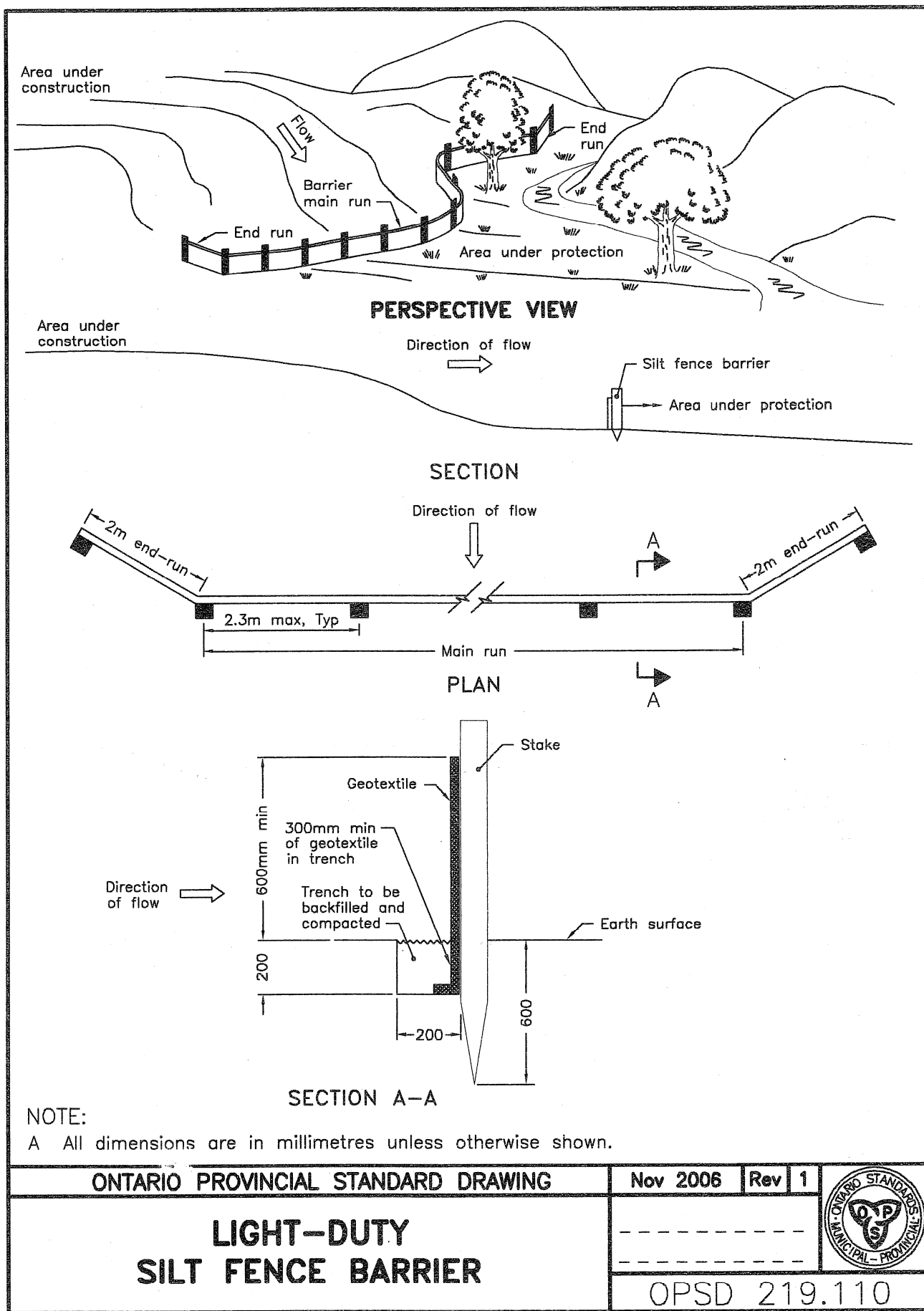


PROJECT: **HAWTHORNE INDUSTRIAL PARK**

DRAWING: **EROSION AND SEDIMENT CONTROL PLAN**

DESIGN: M.B.	DRAWING NO.: <b>ESC</b>
DRAWN: T.S.	JLR NO:
CHECKED: D.U.	
PLOTTED: May 28, 2009	20983





**26.0 METER ROAD ALLOWANCE RURAL SECTION**

**PAVEMENT STRUCTURE SCHEDULE**

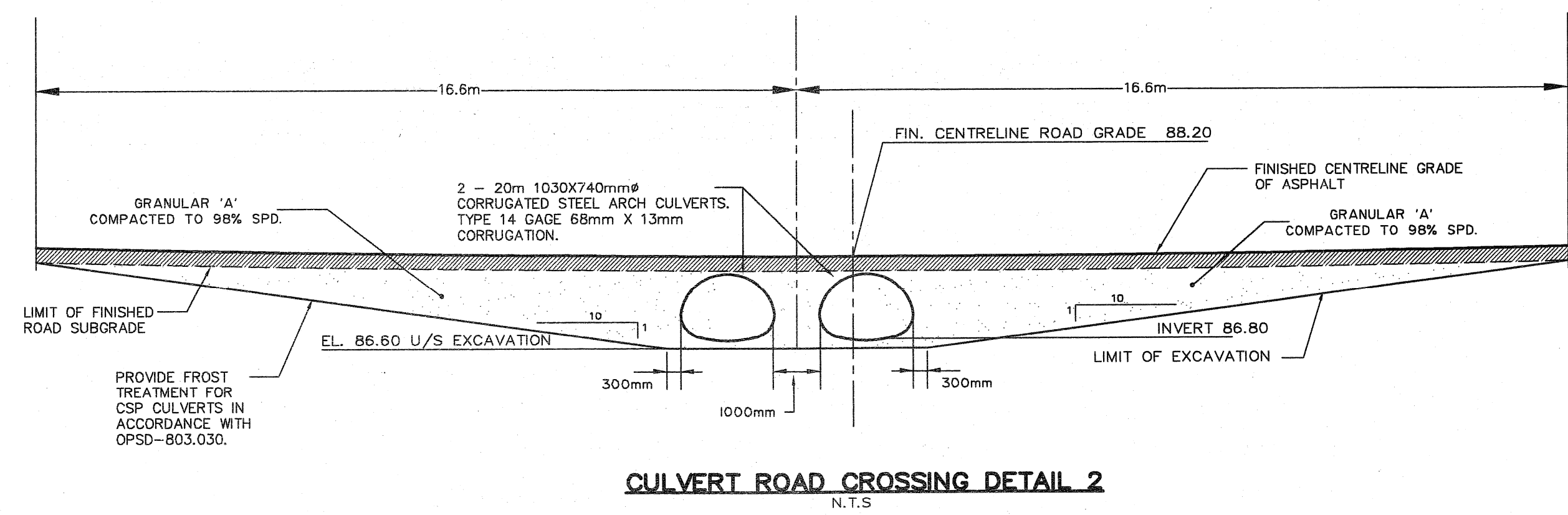
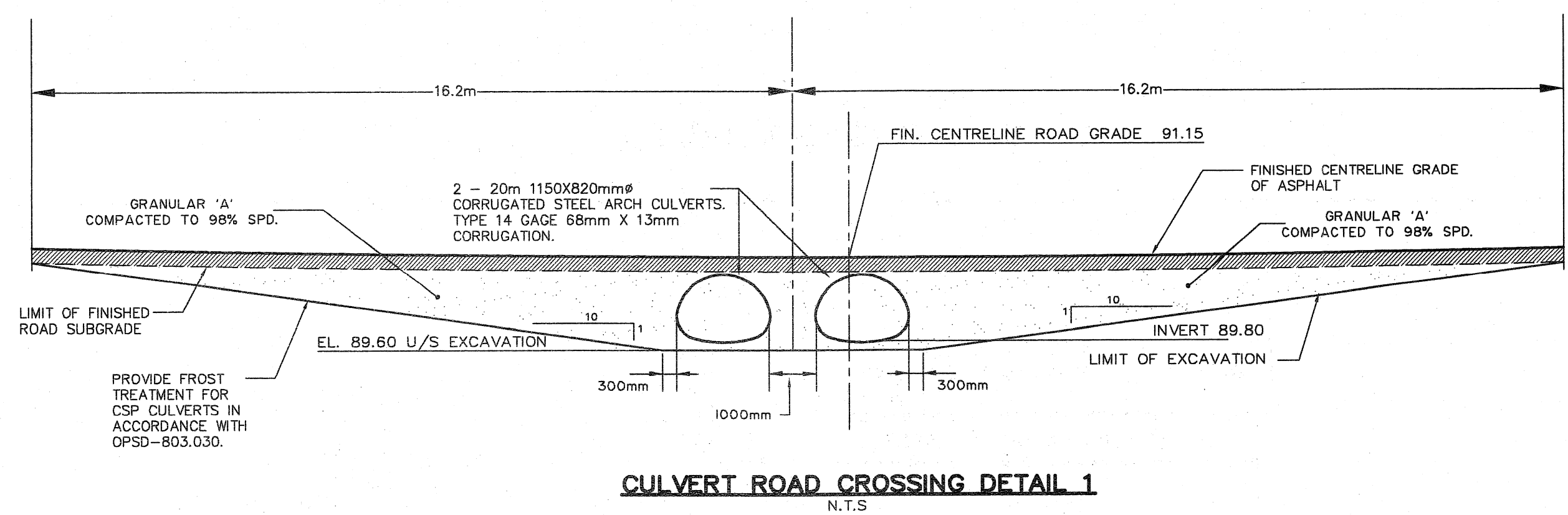
**HAWTHORNE ROAD:**

- 50mm HL3 SURFACE COURSE (SUPERPAVE 12.5mm - PG58-34 LEVEL 3)
- 100mm HL8 BINDER COURSE (SUPERPAVE 19.0mm - PG58-34 LEVEL 3)
- 150mm OPSS GRANULAR "A" BASE
- 300mm OPSS GRANULAR "B" TYPE II SUB-BASE

**INTERNAL ACCESS ROADS:**

- 50mm HL3 SURFACE COURSE (SUPERPAVE 12.5mm - PG58-34 LEVEL 2)
- 75mm HL8 BINDER COURSE (SUPERPAVE 19.0mm - PG58-34 LEVEL 2)
- 150mm OPSS GRANULAR "A" BASE
- 300mm OPSS GRANULAR "B" TYPE II SUB-BASE

- NOTES:**
- DITCHES SHALL BE CONSTRUCTED TO A MINIMUM OF 500mm BELOW SUBGRADE ELEVATION.
  - JOINT USE POLES WILL BE USED FOR OVERHEAD UTILITIES. THE POLES SHALL BE LOCATED 1.0m FROM PROPERTY LINE.
  - SHOULDER ON COLLECTOR STREET TO BE SURFACE TREATED, WHERE REQUIRED BY CITY ENGINEER.
  - SUB-EXCAVATE SOFT AREAS IN SUBBASE AND FILL WITH GRANULAR "B" COMPACTED IN 0.15m LAYERS.
  - ALL MATERIALS TO BE SUPPLIED AND PLACED AS PER O.P.S.S. STANDARDS AND SPECIFICATIONS.
  - DEPTH OF GRANULAR "B" TO BE INCREASED AS REQUIRED BY SOIL CONDITIONS.
  - AREA FROM THE EDGE OF SHOULDER TO THE PROPERTY LINE IS TO BE SODDED OR SEEDED.
  - ALL SERVICES INDICATED MAY NOT NECESSARILY APPLY AT THIS TIME.
  - LIGHT STANDARDS TO BE LOCATED 1.5m FROM EDGE OF ASPHALT.
  - TYPE II GRANULAR "B" IS CRUSHED ROCK.
  - ALL DRIVEWAY CULVERTS TO BE 500mm DIA CSP UNLESS OTHERWISE NOTED.
  - ALL INTERSECTION RADII TO BE PAVED PER OPSS 304.01.
  - ROADWAY CONSTRUCTION IS TO BE AS PER THE GEOTECHNICAL RECOMMENDATIONS PROVIDED BY INSPEC-SOL INC. (REPORT NO. T020556-A1 DATED JAN. 30, 2009)



NO.	ISSUE	DATE
03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

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SCALE: N.T.S.

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**PROFESSIONAL STAMP**

**PROJECT NORTH**

**D. P. UPTON**  
LICENSED PROFESSIONAL ENGINEER  
PROVINCE OF ONTARIO

**PROJECT:**

**HAWTHORNE INDUSTRIAL PARK**

**DRAWING:**

DESIGN: M.B.	DRAWING NO.: <b>DT</b>
DRAWN: T.S.	JLR NO:
CHECKED: D.U.	
PLOTTED: May 29, 2009	20983

# **APPENDIX 'A'**

## **RATIONAL METHOD DESIGN SHEETS (1:10 year and 1:100 year Design Sheets)**

Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA								CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)				
			Area at C of		SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL				HW 1:10 (m)			
	0.70 (ha)	0.90 (ha)																															
NORTHERN CATCHMENT AREA																																	
WEST SIDE SAPPERS RIDGE	2	3	1.86	0.18	2.04	1.46	1.46	4.07	4.07	15.00	97.85	398.2	0.00	0.42	1.20	3.00	0.50	424.2	6973.0	0.80	136.80							2.84	92.50	91.82			
WEST SIDE SAPPERS RIDGE	3	4	1.89	0.14	2.03	1.45	2.92	4.04	8.11	17.84	88.22	715.4	0.00	0.51	1.20	3.00	0.80	904.2	8856.1	1.16	111.00							1.60	91.82	90.93			
WEST SIDE SAPPERS RIDGE	4	5	1.76	0.15	1.91	1.36	4.28	3.79	11.90	19.44	83.68	995.9	0.00	0.58	1.20	3.00	0.51	1011.3	7029.1	1.00	112.85							1.88	90.93	90.36			
WEST SIDE SAPPERS RIDGE	5	6	2.43	0.11	2.54	1.80	6.08	5.00	16.90	21.32	78.96	1334.4	0.00	0.65	1.20	3.00	0.62	1513.4	7762.6	1.19	82.79							1.16	90.36	89.85			
										22.47																							
NORTH ENTRANCE TO SOMME STREET	8	6		0.03	0.03	0.03	0.03	0.08	0.08	15.00	97.85	7.3	0.00	0.20	1.20	3.00	1.30	94.9	11276.7	0.79	10.00							0.21	89.98	89.85			
										15.21																							
CULVERT CROSSING	6	14		0.00	0.00	0.00	6.11	0.00	16.97	22.47	76.34	1295.8					0.50				20.00	2	----	1.15 x 0.82	NO	YES	0.75	0.38	89.85	89.75			
										22.85																							
NORTH PORTION SOMME STREET	13	14	0.85	0.03	0.88	0.62	0.62	1.73	1.73	15.00	97.85	169.2	0.00	0.30	1.20	3.00	2.30	372.0	14999.4	1.38	10.00							0.12	89.98	89.75			
										15.12																							
NORTH PORTION SOMME STREET	14	15	2.93	0.24	3.17	2.27	8.99	6.30	25.00	22.85	75.52	1888.2	0.00	0.74	1.20	3.00	0.50	1926.6	6992.8	1.17	184.04							2.62	89.75	88.83			
NORTH PORTION SOMME STREET	15	16	2.08	0.18	2.26	1.62	10.61	4.50	29.50	25.47	70.36	2075.4	0.00	0.77	1.20	3.00	0.57	2291.4	7480.8	1.29	145.08							1.88	88.83	88.00			
NORTH PORTION SOMME STREET	16	18	2.34	0.23	2.57	1.85	12.46	5.13	34.63	27.35	67.11	2323.9	0.00	0.80	1.20	3.00	0.51	2399.6	7074.8	1.25	185.66							2.48	88.00	87.05			
NORTH PORTION SOMME STREET	18	19	0.00	0.05	0.05	0.05	12.50	0.13	34.75	29.82	63.30	2199.9	0.00	0.76	1.20	3.00	0.72	2476.8	8372.8	1.43	41.86							0.49	87.05	86.75			
										30.31																							
EAST SIDE SAPPERS RIDGE	9	10	1.74	0.19	1.93	1.39	1.39	3.86	3.86	15.00	97.85	378.0	0.00	0.41	1.20	3.00	0.50	399.2	6996.6	0.79	147.87							3.11	92.40	91.66			
EAST SIDE SAPPERS RIDGE	10	11	1.49	0.14	1.63	1.17	2.56	3.25	7.11	18.11	87.42	622.0	0.00	0.49	1.20	3.00	0.66	735.9	8019.2	1.02	111.04							1.81	91.66	90.93			
EAST SIDE SAPPERS RIDGE	11	12	0.73	0.14	0.87	0.64	3.20	1.77	8.88	19.92	82.40	732.0	0.00	0.52	1.20	3.00	0.55	785.5	7304.8	0.97	104.49							1.80	90.93	90.36			
EAST SIDE SAPPERS RIDGE	12	7	0.18	0.09	0.27	0.21	3.40	0.58	9.46	21.72	78.02	738.2	0.00	0.49	1.20	3.00	0.81	818.5	8919.0	1.14	72.55							1.06	90.36	89.77			
NORTH PORTION SOMME STREET	7	20	1.07	0.23	1.30	0.96	4.36	2.66	12.12	22.79	75.66	916.9	0.00	0.57	1.20	3.00	0.50	956.8	6966.1	0.98	177.39							3.01	89.77	88.89			
NORTH PORTION SOMME STREET	20	21	2.27	0.19	2.46	1.76	6.12	4.89	17.01	25.80	69.76	1186.8	0.00	0.62	1.20	3.00	0.50	1200.1	6981.9	1.04	147.49							2.36	88.89	88.16			
NORTH PORTION SOMME STREET	21	22	3.43	0.30	3.73	2.67	8.79	7.43	24.44	28.16	65.80	1608.1	0.00	0.70	1.20	3.00	0.56	1759.0	7404.4	1.20	232.84							3.24	88.16	86.85			
										31.40																							
SOUTHERN CATCHMENT AREA																																	
SOUTH PORTION SOMME STREET	23A	23B	0.00	0.25	0.25	0.23	0.23	0.63	0.63	15.00	97.85	61.2	0.00	0.20	1.20	3.00	0.64	66.3	7883.5	0.55	181.00							5.46	93.65	92.50			
CULVERT CROSSING	23B	23C		0.00	0.00	0.00	0.23	0.00	0.63	20.46	81.05	50.7					0.42				24.00	1	500	----	NO	YES	0.33	1.55	92.50	92.40			
SOUTH PORTION SOMME STREET	23C	24A	0.00	0.17	0.17	0.15	0.38	0.43	1.05	22.00	77.38	81.3	0.00	0.22	1.20	3.00	0.82	97.0	8946.1	0.67	110.00							2.74	92.40	91.50			
CULVERT CROSSING	24A	24B		0.00	0.00	0.00	0.38	0.00	1.05	24.75	71.70	75.3					0.42				24.00	1	500	----	NO	YES	0.34	1.04	91.50	91.40			
SOUTH PORTION SOMME STREET	24B	24C	0.00	0.21	0.21	0.19	0.57	0.53	1.58	25.79	69.78	110.0	0.00	0.25	1.20	3.00	0.70	126.0	8258.2	0.67	142.00							3.52	91.40	90.41			
ORGAWORLD - SITE	U/S	24C	1:10 year peak flow = 132 L/s, see Table 4 of Orgaworld Stormwater Site Management Plan, Sept. 2008										132.0																				
SOUTH PORTION SOMME STREET	24C	25	3.70	0.32	4.02	2.88	3.44	8.00	9.58	29.31	64.05	745.3	0.00	0.52	1.20	3.00	0.54	783.8	7289.5	0.97	244.84							4.22	90.41	89.08			
SOUTH PORTION SOMME STREET	25	26	2.63	0.12	2.75	1.95	5.39	5.42	14.99	33.53	58.41	1007.7	0.00	0.58	1.20	3.00	0.51	1013.1	7041.5	1.00	90.75							1.51	89.08	88.62			
SOUTH PORTION SOMME STREET	26	27A	3.15	0.20	3.35	2.39	7.78	6.63	21.63	35.04	56.65	1357.2	0.00	0.62	1.20	3.00	0.65	1370.0	7970.4	1.19	157.06							2.20	88.62	87.60			
SOUTH PORTION SOMME STREET	27A	27B	0.00	0.03	0.03	0.03	7.81	0.08	21.70	37.24	54.29	1310.1	0.00	0.61	1.20	3.00	0.65	1312.4	7973.8	1.18	20.00							0.28	87.60	87.47			
CULVERT CROSSING	27B	27C		0.00	0.00	0.00	7.81	0.00	21.70	37.53	54.00	1303.8					0.73				15.00	1	----	1.39 X 0.97	YES	NO	0.87	0.20	87.47	87.36			
CORNER OF POND	27C	19	0.00	0.11	0.11	0.10	7.88	0.28	21.98	37.73	53.79	1314.2	0.00	0.65	1.20	3.00	0.71	1622.9	8324.0	1.28	72.00							0.94	87.36	86.85			
										38.67																							

Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA									CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)	
			Area at C of		SUM(A)	SUM(A*C)	TOTAL A°C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL	HW 1:10 (m)				
	0.70 (ha)	0.90 (ha)																													
SW ENTRANCE TO SOMME STREET	1	2	0.18	0.25	0.43	0.35	0.35	0.97	0.97	15.00	97.85	94.6	0.00	0.32	1.20	3.00	0.61	226.9	7702.7	0.74	189.60								4.28	93.65	92.50
CULVERT CROSSING	2	9		0.00	0.00	0.00	0.35	0.00	0.97	19.28	84.12	81.3					0.50				20.00	1	600	-----	NO	YES	0.52	1.16	92.50	92.40	
SOUTH PORTION SOMME STREET	9	28	2.54	0.35	2.89	2.10	2.44	5.83	6.80	20.44	81.10	551.2	0.00	0.47	1.20	3.00	0.73	694.0	8450.7	1.05	272.58								4.34	92.40	90.41
SOUTH PORTION SOMME STREET	28	29A	3.46	0.32	3.78	2.71	5.15	7.53	14.33	24.77	71.65	1026.7	0.00	0.61	1.20	3.00	0.54	1198.8	7283.5	1.07	245.24								3.81	90.41	89.08
SOUTH PORTION SOMME STREET	29A	29B	0.77	0.11	0.88	0.64	5.79	1.78	16.11	28.58	65.15	1049.5	0.00	0.62	1.20	3.00	0.53	1239.6	7212.0	1.07	86.51								1.34	89.08	88.62
SOUTH PORTION SOMME STREET	29B	30	0.32	0.12	0.44	0.33	6.13	0.92	17.03	29.92	63.16	1075.8	0.00	0.58	1.20	3.00	0.70	1191.6	8282.1	1.18	94.12								1.33	88.62	87.96
SOUTH PORTION SOMME STREET	30	22	0.75	0.16	0.91	0.67	6.80	1.86	18.89	31.25	61.31	1158.5	0.00	0.58	1.20	3.00	0.97	1402.6	9748.4	1.39	124.55								1.49	87.96	86.75
										32.74																					
CULVERT CROSSING	22	19		0.00	0.00	0.00	15.59	0.00	43.33	32.74	59.38	2573.1					0.50				20.00	2	-----	1.03 X 0.74	YES	NO	1.30	0.08	86.85	86.75	
										32.82																					
POND INLET	19	POND		0.00	0.00	0.00	35.97	0.00	100.06	38.67	52.87	5422.6	3.09	0.38	1.20	3.00	5.68	5629.1	13135.2	3.50	22.00								0.10	86.75	85.50
POND OUTLET DITCH	POND	DITCH	1:10 year controlled post development peak flow = 696 l/s, see SWMHYMO output of this Report									696.0	1.00	0.27	0.38	3.00	2.08	750.9	1506.6	1.54	24.00								0.26	82.50	82.00

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030



Hawthorne Industrial Park

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City of Ottawa

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JLR 20983

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1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA						CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)		
			Area at C of		SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A°C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL				OUTLET CONTROL	
	0.70 (ha)	0.90 (ha)																										
NORTHERN CATCHMENT AREA																												
WEST SIDE SAPPERS RIDGE	2	3	1.86	0.18	2.04	1.81	1.81	5.02	5.02	15.00	142.89	718.0		0.00	1.20	3.00	0.50	6973.0	1.61	136.80					1.41	92.50	91.82	
WEST SIDE SAPPERS RIDGE	3	4	1.89	0.14	2.03	1.80	3.61	5.00	10.02	16.41	135.47	1357.9		0.00	1.20	3.00	0.80	8856.1	2.05	111.00					0.90	91.82	90.93	
WEST SIDE SAPPERS RIDGE	4	5	1.76	0.15	1.91	1.69	5.29	4.69	14.71	17.31	131.16	1929.7		0.00	1.20	3.00	0.51	7029.1	1.63	112.85					1.16	90.93	90.36	
WEST SIDE SAPPERS RIDGE	5	6	2.43	0.11	2.54	2.23	7.53	6.21	20.92	18.47	126.06	2637.5		0.00	1.20	3.00	0.62	7762.6	1.80	82.79					0.77	90.36	89.85	
										19.24																		
NORTH ENTRANCE TO SOMME STREET	8	6		0.03	0.03	0.03	0.03	0.08	0.08	15.00	142.89	11.9		0.00	1.20	3.00	1.30	11276.7	2.61	10.00					0.06	89.98	89.85	
										15.06																		
CULVERT CROSSING	6	14		0.00	0.00	0.00	7.56	0.00	21.01	19.24	122.91	2581.8					0.50			20.00	2	----	1.15 x 0.82	NO	YES	0.19	89.85	89.75
										19.43																		
NORTH PORTION SOMME STREET	13	14	0.85	0.03	0.88	0.77	0.77	2.15	2.15	15.00	142.89	307.4		0.00	1.20	3.00	2.30	14999.4	3.47	10.00					0.05	89.98	89.75	
										15.05																		
NORTH PORTION SOMME STREET	14	15	2.93	0.24	3.17	2.80	11.13	7.79	30.95	19.43	122.15	3780.5		0.00	1.20	3.00	0.50	6992.8	1.62	184.04					1.89	89.75	88.83	
NORTH PORTION SOMME STREET	15	16	2.08	0.18	2.26	2.00	13.13	5.56	36.51	21.32	115.16	4204.4		0.00	1.20	3.00	0.57	7480.8	1.73	145.08					1.40	88.83	88.00	
NORTH PORTION SOMME STREET	16	18	2.34	0.23	2.57	2.28	15.41	6.33	42.84	22.72	110.55	4736.0		0.00	1.20	3.00	0.51	7074.8	1.64	185.66					1.89	88.00	87.05	
NORTH PORTION SOMME STREET	18	19	0.00	0.05	0.05	0.05	15.46	0.14	42.98	24.61	104.93	4509.7		0.00	1.20	3.00	0.72	8372.8	1.94	41.86					0.36	87.05	86.75	
										24.97																		
EAST SIDE SAPPERS RIDGE	9	10	1.74	0.19	1.93	1.71	1.71	4.76	4.76	15.00	142.89	680.4		0.00	1.20	3.00	0.50	6996.6	1.62	147.87					1.52	92.40	91.66	
EAST SIDE SAPPERS RIDGE	10	11	1.49	0.14	1.63	1.44	3.16	4.02	8.78	16.52	134.93	1184.3		0.00	1.20	3.00	0.66	8019.2	1.86	111.04					1.00	91.66	90.93	
EAST SIDE SAPPERS RIDGE	11	12	0.73	0.14	0.87	0.78	3.94	2.16	10.94	17.52	130.23	1424.7		0.00	1.20	3.00	0.55	7304.8	1.69	104.49					1.03	90.93	90.36	
EAST SIDE SAPPERS RIDGE	12	7	0.18	0.09	0.27	0.25	4.18	0.69	11.63	18.55	125.73	1462.2		0.00	1.20	3.00	0.81	8919.0	2.06	72.55					0.59	90.36	89.77	
NORTH PORTION SOMME STREET	7	20	1.07	0.23	1.30	1.17	5.35	3.24	14.87	19.13	123.33	1834.1		0.00	1.20	3.00	0.50	6966.1	1.61	177.39					1.83	89.77	88.89	
NORTH PORTION SOMME STREET	20	21	2.27	0.19	2.46	2.18	7.53	6.05	20.92	20.97	116.41	2435.6		0.00	1.20	3.00	0.50	6981.9	1.62	147.49					1.52	88.89	88.16	
NORTH PORTION SOMME STREET	21	22	3.43	0.30	3.73	3.30	10.83	9.18	30.10	22.49	111.29	3350.0		0.00	1.20	3.00	0.56	7404.4	1.71	232.84					2.26	88.16	86.85	
										24.75																		
SOUTHERN CATCHMENT AREA																												
SOUTH PORTION SOMME STREET	23A	23B	0.00	0.25	0.25	0.25	0.25	0.70	0.70	15.00	142.89	99.3		0.00	1.20	3.00	0.64	7883.5	1.82	181.00					1.65	93.65	92.50	
CULVERT CROSSING	23B	23C		0.00	0.00	0.00	0.25	0.00	0.70	16.65	134.29	93.3					0.42			24.00	1	500	----	NO	YES	0.84	92.50	92.40
SOUTH PORTION SOMME STREET	23C	24A	0.00	0.17	0.17	0.17	0.42	0.47	1.17	17.49	130.34	152.2		0.00	1.20	3.00	0.82	8946.1	2.07	110.00					0.89	92.40	91.50	
CULVERT CROSSING	24A	24B		0.00	0.00	0.00	0.42	0.00	1.17	18.38	126.45	147.6					0.42			24.00	1	500	----	NO	YES	0.53	91.50	91.40
SOUTH PORTION SOMME STREET	24B	24C	0.00	0.21	0.21	0.21	0.63	0.58	1.75	18.91	124.24	217.6		0.00	1.20	3.00	0.70	8258.2	1.91	142.00					1.24	91.40	90.41	
ORGAWORLD - SITE	U/S	24C	1:100 year peak flow = 283 l/s, see Table 4 of Orgaworld Stormwater Site Management Plan, Sept. 2008																									
SOUTH PORTION SOMME STREET	24C	25	3.70	0.32	4.02	3.56	4.19	9.89	11.64	20.15	119.40	1672.8		0.00	1.20	3.00	0.54	7289.5	1.69	244.84					2.42	90.41	89.08	
SOUTH PORTION SOMME STREET	25	26	2.63	0.12	2.75	2.42	6.61	6.73	18.37	22.57	111.05	2323.0		0.00	1.20	3.00	0.51	7041.5	1.63	90.75					0.93	89.08	88.62	
SOUTH PORTION SOMME STREET	26	27A	3.15	0.20	3.35	2.96	9.57	8.22	26.59	23.49	108.17	3159.5		0.00	1.20	3.00	0.65	7970.4	1.84	157.06					1.42	88.62	87.60	
SOUTH PORTION SOMME STREET	27A	27B	0.00	0.03	0.03	0.03	9.60	0.08	26.67	24.91	104.09	3059.5		0.00	1.20	3.00	0.65	7973.8	1.85	20.00					0.18	87.60	87.47	
CULVERT CROSSING	27B	27C		0.00	0.00	0.00	9.60	0.00	26.67	25.09	103.59	3046.2					0.73			15.00	1	----	1.39 X 0.97	YES	NO	0.09	87.47	87.36
CORNER OF POND	27C	19	0.00	0.11	0.11	0.11	9.71	0.31	26.98	25.18	103.36	3071.7		0.00	1.20	3.00	0.71	8324.0	1.93	72.00					0.62	87.36	86.85	
										25.80																		



Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

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JLR 20983

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1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION				
	FROM	TO	Area at C of		SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s
			0.70 (ha)	0.90 (ha)								
SW ENTRANCE TO SOMME STREET	1	2	0.18	0.25	0.43	0.40	0.40	1.12	1.12	15.00	142.89	160.5
CULVERT CROSSING	2	9		0.00	0.00	0.00	0.40	0.00	1.12	16.77	133.71	150.2
SOUTH PORTION SOMME STREET	9	28	2.54	0.35	2.89	2.58	2.98	7.16	8.29	17.40	130.77	1083.6
SOUTH PORTION SOMME STREET	28	29A	3.46	0.32	3.78	3.35	6.33	9.31	17.59	19.72	121.01	2128.9
SOUTH PORTION SOMME STREET	29A	29B	0.77	0.11	0.88	0.79	7.11	2.19	19.78	22.15	112.40	2223.0
SOUTH PORTION SOMME STREET	29B	30	0.32	0.12	0.44	0.40	7.51	1.11	20.89	23.01	109.65	2290.7
SOUTH PORTION SOMME STREET	30	22	0.75	0.16	0.91	0.82	8.33	2.27	23.16	23.83	107.18	2482.3
										24.75		
CULVERT CROSSING	22	19		0.00	0.00	0.00	19.16	0.00	53.26	24.75	104.53	5567.5
										24.79		
POND INLET	19	POND		0.00	0.00	0.00	44.32	0.00	123.22	25.80	101.69	12813.8
POND OUTLET DITCH	POND	DITCH	1:100 year controlled post development peak flow = 1,432 l/s, see SWMHYMO output of this Report									1432.0

OPEN DITCH/SWALE DATA							CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW	U/S	D/S
BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL	TIME (min)	Inv (m)	Inv (m)
0.00	1.20	3.00	0.61	7702.7	1.78	189.60						1.77	93.65	92.50
			0.50			20.00	1	600	----	NO	YES	0.63	92.50	92.40
0.00	1.20	3.00	0.73	8450.7	1.96	272.58						2.32	92.40	90.41
0.00	1.20	3.00	0.54	7283.5	1.69	245.24						2.42	90.41	89.08
0.00	1.20	3.00	0.53	7212.0	1.67	86.51						0.86	89.08	88.62
0.00	1.20	3.00	0.70	8282.1	1.92	94.12						0.82	88.62	87.96
0.00	1.20	3.00	0.97	9748.4	2.26	124.55						0.92	87.96	86.75
			0.50			20.00	2	----	1.03 X 0.74	YES	NO	0.04	86.85	86.75
3.09	0.55	5.00	5.68	13135.2	4.09	22.00						0.09	86.75	85.50
1.00	0.38	3.00	2.08	1506.6	1.85	24.00						0.22	82.50	82.00

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA									CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)	
			AREA (A) at C of				SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL	HW 1:10 (m)				
	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																													
WEST CATCHMENT AREA																																	
EAST SIDE HAWTHORNE ROAD	1	2		2.46		0.14	2.60	0.86	0.86	2.40	2.40	15.00	97.85	235.0	0.00	0.41	0.50	3.00	0.20	250.1	424.5	0.50	112.00								3.76	103.22	103.00
WEST SIDE HAWTHORNE ROAD	2	3		1.60		0.06	1.66	0.53	1.40	1.48	3.89	18.76	85.54	332.5	0.00	0.25	0.50	3.00	5.00	337.3	2141.9	1.80	50.00							0.46	103.00	100.50	
WEST SIDE HAWTHORNE ROAD	3	4		1.58		0.06	1.64	0.53	1.93	1.47	5.35	19.23	84.26	451.1	0.00	0.27	0.50	3.00	7.00	490.1	2534.3	2.24	50.00							0.37	100.50	97.00	
WEST SIDE HAWTHORNE ROAD	4	5		1.58		0.06	1.64	0.53	2.45	1.47	6.82	19.60	83.26	568.0	0.00	0.34	0.50	3.00	5.00	765.9	2141.9	2.21	50.00							0.38	97.00	94.50	
EAST SIDE HAWTHORNE ROAD	5	6a		1.95		0.10	2.05	0.68	3.13	1.88	8.70	19.98	82.27	715.6	0.00	0.45	0.65	3.00	1.07	747.0	1991.5	1.23	75.00							1.02	94.50	93.70	
CULVERT CROSSING	6a	6b				0.00	0.00	0.00	3.13	0.00	8.70	20.99	79.73	693.6					1.00				10.00	1	800	-----	YES	NO	0.84	0.12	93.70	93.60	
WEST SIDE HAWTHORNE ROAD	6b	7		1.20		0.03	1.23	0.39	3.52	1.08	9.77	21.11	79.45	776.5	0.00	0.53	1.15	3.00	0.53	817.1	6447.9	0.97	15.00							0.26	93.60	93.52	
EAST SIDE HAWTHORNE ROAD	7	8		1.58		0.06	1.64	0.53	4.04	1.47	11.24	21.37	78.83	886.3	0.00	0.56	1.15	3.00	0.50	916.3	6243.2	0.97	50.00							0.86	93.52	93.27	
EAST SIDE HAWTHORNE ROAD	8	9		1.58		0.06	1.64	0.53	4.57	1.47	12.71	22.23	76.88	977.2	0.00	0.58	1.15	3.00	0.50	1006.2	6243.2	1.00	50.00							0.84	93.27	93.02	
WEST SIDE HAWTHORNE ROAD	9	10		1.58		0.06	1.64	0.53	5.10	1.47	14.18	23.06	75.07	1064.4	0.00	0.60	1.15	3.00	0.50	1101.4	6243.2	1.02	50.00							0.82	93.02	92.77	
WEST SIDE HAWTHORNE ROAD	10	11		1.58		0.06	1.64	0.53	5.63	1.47	15.65	23.88	73.39	1148.3	0.00	0.62	1.15	3.00	0.50	1202.1	6243.2	1.04	50.00							0.80	92.77	92.52	
EAST SIDE HAWTHORNE ROAD	11	12		1.48		0.06	1.54	0.50	6.13	1.38	17.03	24.68	71.83	1223.3	0.00	0.63	1.15	3.00	0.50	1254.5	6243.2	1.05	50.00							0.79	92.52	92.27	
EAST SIDE HAWTHORNE ROAD	12	13		1.34		0.06	1.40	0.46	6.58	1.27	18.30	25.47	70.35	1287.3	0.00	0.64	1.15	3.00	0.50	1308.3	6243.2	1.06	50.00							0.78	92.27	92.02	
EAST SIDE HAWTHORNE ROAD	13	14b		1.54		0.21	1.75	0.65	7.23	1.81	20.11	26.25	68.96	1386.6	0.00	0.64	1.15	3.00	0.61	1449.7	6918.0	1.18	158.00							2.23	92.02	91.05	
												28.49																					
SW RIDEAU & HAWTHORNE	14a	14b		0.64		0.18	0.82	0.35	0.35	0.98	0.98	15.00	97.85	96.3	0.00	0.20	1.30	3.00	4.06	167.6	24661.5	1.40	140.00							1.67	96.73	91.05	
												16.67																					
CULVERT CROSSING	14b	23				0.00	0.00	0.00	7.59	0.00	21.09	28.49	65.29	1377.2					1.40				20.00	1	1000	-----	YES	NO	1.14	0.19	91.05	90.77	
												28.68																					
EAST CATCHMENT AREA																																	
EAST SIDE HAWTHORNE ROAD	15	16				0.33	0.33	0.30	0.30	0.83	0.83	15.00	97.85	80.8	0.00	0.25	0.30	3.00	0.45	101.7	165.4	0.54	110.00							3.38	103.80	103.30	
EAST SIDE HAWTHORNE ROAD	16	17				0.14	0.14	0.13	0.42	0.35	1.18	18.38	86.64	101.9	0.00	0.16	0.30	3.00	6.20	114.3	610.8	1.49	100.00							1.12	103.30	97.10	
EAST SIDE HAWTHORNE ROAD	17	18				0.04	0.04	0.04	0.46	0.10	1.28	19.50	83.52	106.6	0.00	0.16	1.20	3.00	6.36	115.8	24949.6	1.51	33.00							0.36	97.10	95.00	
CULVERT CROSSING	18	19				0.00	0.00	0.00	0.46	0.00	1.28	19.86	82.56	105.3					1.77				22.00	1	600	-----	YES	NO	0.30	0.98	95.00	94.61	
EAST SIDE HAWTHORNE ROAD	19	20				0.06	0.06	0.05	0.51	0.15	1.43	20.85	80.08	114.2	0.00	0.21	0.70	3.00	2.79	158.3	3925.7	1.20	24.00							0.33	94.61	93.94	
CULVERT CROSSING	20	21				0.00	0.00	0.00	0.51	0.00	1.43	21.18	79.28	113.1					0.50				20.00	1	600	-----	NO	YES	0.37	0.83	93.94	93.84	
EAST SIDE HAWTHORNE ROAD	21	22a	0.21			0.16	0.37	0.19	0.70	0.52	1.94	22.02	77.35	150.3	0.00	0.29	0.80	3.00	0.50	158.5	2372.0	0.63	82.00							2.18	93.84	93.43	
EAST SIDE HAWTHORNE ROAD	22a	22b	0.61			0.29	0.90	0.38	1.08	1.06	3.01	24.19	72.77	218.9	0.00	0.33	1.17	3.00	0.52	228.1	6666.4	0.70	175.00							4.18	93.43	92.52	
EAST SIDE HAWTHORNE ROAD	22b	23	0.93			0.34	1.27	0.49	1.57	1.37	4.38	28.37	65.47	286.5	0.00	0.35	1.17	3.00	0.70	309.6	7734.6	0.84	260.00							5.14	92.59	90.77	
												33.51																					
SOUTH CATCHMENT AREA																																	
SOUTH SIDE RIDEAU ROAD	23	24	0.73			0.28	1.01	0.40	9.56	1.11	26.57	33.51	58.43	1552.8	0.00	0.51	1.74	3.00	2.65	1642.9	43339.8	2.11	235.00							1.86	90.77	84.55	
												35.37																					
WEST SIDE SOMME STREET	25	24			0.42	0.12	0.54	0.40	0.40	1.12	1.12	15.00	97.85	109.4	0.00	0.18	1.20	3.00	2.80	105.1	16548.0	1.08	125.74							1.94	89.98	86.46	
												16.94																					
CULVERT CROSSING	24	26				0.00	0.00	0.00	9.96	0.00	27.69	35.37	56.28	1558.5					1.00				20.00	1	800	-----	NO	YES	2.31	0.11	84.55	84.35	
												35.48																					
EAST SIDE SOMME STREET	27	26			0.32	0.11	0.43	0.32	0.32	0.90	0.90	15.00	97.85	87.9	0.00	0.17	1.20	3.00	2.80	90.3	16548.0	1.04	125.74							2.01	89.98	86.46	
												17.01																					
SOUTH SIDE RIDEAU ROAD																																	

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA								CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)		
			AREA (A) at C of				SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL				HW 1:10 (m)	
	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																													
NORTH CATCHMENT AREA																																	
			Existing 900 mm dia. culvert capacity before ditch flows to Findlay Creek																														
NORTH SIDE RIDEAU ROAD	31	32	6.66				0.52	7.18	1.80	1.80	5.00	5.00	20.00	97.26		0.00	0.58	1.50	3.00	1.93	1974.3	24880.1	1.96	400.00							3.41	90.71	83.01
													23.41																				
	33	32	0.87				0.10	0.97	0.26	0.26	0.73	0.73	15.00	115.83		0.00	0.40	1.50	3.00	0.16	213.3	7240.8	0.44	92.00							3.45	83.16	83.01
													18.45																				
EXISTING CULVERT CROSSING	32	28					0.00	0.00	0.00	2.06	0.00	5.74	23.41	87.93						-0.15				20.00	1	1000					0.14	83.01	83.04
													23.55																				
SOUTH CATCHMENT AREA																																	
SOUTH SIDE RIDEAU ROAD	28	29	0.90				0.33	1.23	0.48	13.16	1.33	36.58	37.84	53.68	3363.5	0.00	1.17	2.20	3.00	0.14	3437.1	18513.7	0.84	347.24							6.91	83.04	82.56
SOUTH SIDE RIDEAU ROAD	29	30	0.48				0.31	0.79	0.38	13.53	1.04	37.62	44.76	47.64	3192.1	0.00	0.90	2.20	3.00	0.51	3287.0	35640.2	1.35	236.20							2.91	82.56	81.35

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA							CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)		
			AREA (A) at C of				SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL					
	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																											
WEST CATCHMENT AREA																															
WEST SIDE HAWTHORNE ROAD	1	2		2.46		0.14	2.60	1.06	1.06	2.95	2.95	15.00	142.89	422.1	0.00	0.50	3.00	0.20	424.5	0.57	112.00							3.30	103.22	103.00	
WEST SIDE HAWTHORNE ROAD	2	3		1.60		0.06	1.66	0.66	1.72	1.83	4.79	18.30	126.80	607.2	0.00	0.50	3.00	5.00	2141.9	2.86	50.00							0.29	103.00	100.50	
WEST SIDE HAWTHORNE ROAD	3	4		1.58		0.06	1.64	0.65	2.38	1.81	6.60	18.59	125.56	829.0	0.00	0.50	3.00	7.00	2534.3	3.38	50.00							0.25	100.50	97.00	
WEST SIDE HAWTHORNE ROAD	4	5		1.58		0.06	1.64	0.65	3.03	1.81	8.42	18.84	124.54	1048.2	0.00	0.50	3.00	5.00	2141.9	2.86	50.00							0.29	97.00	94.50	
WEST SIDE HAWTHORNE ROAD	5	6A		1.95		0.10	2.05	0.83	3.86	2.31	10.73	19.13	123.35	1323.2	0.00	0.65	3.00	1.07	1991.5	1.57	75.00							0.80	94.50	93.70	
CULVERT CROSSING	6A	6B				0.00	0.00	0.00	3.86	0.00	10.73	19.92	120.24	1289.9				1.00			10.00	1	800	-----	YES	NO	0.06	93.70	93.60		
WEST SIDE HAWTHORNE ROAD	6B	7		1.20		0.03	1.23	0.48	4.34	1.33	12.06	19.99	119.99	1447.3	0.00	1.15	3.00	0.53	6447.9	1.63	15.00							0.15	93.60	93.52	
WEST SIDE HAWTHORNE ROAD	7	8		1.58		0.06	1.64	0.65	4.99	1.81	13.88	20.14	119.42	1657.0	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	93.52	93.27	
WEST SIDE HAWTHORNE ROAD	8	9		1.58		0.06	1.64	0.65	5.64	1.81	15.69	20.67	117.47	1843.0	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	93.27	93.02	
WEST SIDE HAWTHORNE ROAD	9	10		1.58		0.06	1.64	0.65	6.30	1.81	17.50	21.20	115.59	2023.3	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	93.02	92.77	
WEST SIDE HAWTHORNE ROAD	10	11		1.58		0.06	1.64	0.65	6.95	1.81	19.32	21.73	113.78	2197.9	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	92.77	92.52	
WEST SIDE HAWTHORNE ROAD	11	12		1.48		0.06	1.54	0.62	7.56	1.71	21.03	22.26	112.03	2355.6	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	92.52	92.27	
WEST SIDE HAWTHORNE ROAD	12	13		1.34		0.06	1.40	0.56	8.13	1.56	22.59	22.79	110.34	2492.6	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	92.27	92.02	
WEST SIDE HAWTHORNE ROAD	13	14B		1.54		0.21	1.75	0.79	8.91	2.19	24.78	23.32	108.70	2693.6	0.00	1.15	3.00	0.61	6918.0	1.74	158.00							1.51	92.02	91.05	
												24.83																			
												15.48																			
SW RIDEAU & HAWTHORNE	14A	14B		0.64		0.18	0.82	0.42	0.42	1.17	1.17	15.00	142.89	166.8	0.00	1.30	3.00	4.06	24661.5	4.86	140.00							0.48	96.73	91.05	
												15.48																			
												24.83																			
CULVERT CROSSING	14B	23				0.00	0.00	0.00	9.33	0.00	25.95	24.83	104.32	2706.8				1.40			20.00	1	1000	-----	YES	NO	0.10	91.05	90.77		
												24.93																			
EAST CATCHMENT AREA																															
EAST SIDE HAWTHORNE ROAD	15	16				0.33	0.33	0.33	0.33	0.92	0.92	15.00	142.89	131.1	0.00	0.30	3.00	0.45	165.4	0.61	110.00							2.99	103.80	103.30	
EAST SIDE HAWTHORNE ROAD	16	17				0.14	0.14	0.14	0.47	0.39	1.31	17.99	128.11	167.4	0.00	0.30	3.00	6.20	610.8	2.26	100.00							0.74	103.30	97.10	
EAST SIDE HAWTHORNE ROAD	17	18				0.04	0.04	0.04	0.51	0.11	1.42	18.73	124.98	177.2	0.00	1.20	3.00	6.36	24949.6	5.78	33.00							0.10	97.10	95.00	
CULVERT CROSSING	18	19				0.00	0.00	0.00	0.51	0.00	1.42	18.82	124.58	176.6				1.77			22.00	1	600	-----	YES	NO	0.59	95.00	94.61		
EAST SIDE HAWTHORNE ROAD	19	20				0.06	0.06	0.06	0.57	0.17	1.58	19.41	122.22	193.7	0.00	0.70	3.00	2.79	3925.7	2.67	24.00							0.15	94.61	93.94	
CULVERT CROSSING	20	21				0.00	0.00	0.00	0.57	0.00	1.58	19.56	121.63	192.7				0.50			20.00	1	600	-----	NO	YES	0.49	93.94	93.84		
EAST SIDE HAWTHORNE ROAD	21	22A	0.21			0.16	0.37	0.21	0.78	0.59	2.18	20.05	119.76	260.5	0.00	0.80	3.00	0.50	2372.0	1.24	82.00							1.11	93.84	93.43	
EAST SIDE HAWTHORNE ROAD	22A	22B	0.61			0.29	0.90	0.44	1.23	1.23	3.41	21.16	115.75	394.2	0.00	1.17	3.00	0.52	6666.4	1.62	175.00							1.80	93.43	92.52	
EAST SIDE HAWTHORNE ROAD	22B	23	0.93			0.34	1.27	0.57	1.80	1.59	5.00	22.95	109.83	548.8	0.00	1.17	3.00	0.70	7734.6	1.88	260.00							2.30	92.59	90.77	
												25.25																			
SOUTH CATCHMENT AREA																															
SOUTH SIDE RIDEAU ROAD	23	24	0.73			0.28	1.01	0.46	11.59	1.29	32.23	25.25	103.15	3324.7	0.00	1.74	3.00	2.65	43339.8	4.77	235.00							0.82	90.77	84.55	
												26.08																			
WEST SIDE SOMME STREET	25	24			0.42	0.12	0.54	0.49	0.49	1.36	1.36	15.00	142.89	193.7	0.00	1.20	3.00	2.80	16548.0	3.83	125.74							0.55	89.98	86.46	
												15.55																			
CULVERT CROSSING	24	26				0.00	0.00	0.00	12.08	0.00	33.59	26.08	100.99	3391.7				1.00			20.00	1	800	-----	NO	YES	0.05	84.55	84.35		
												26.12																			
EAST SIDE SOMME STREET	27	26			0.32	0.11	0.43	0.39	0.39	1.08	1.08	15.00	142.89	154.9	0.00	1.20	3.00	2.80	16548.0	3.83	125.74							0.55	89.98	86.46	
												15.55																			
												</																			

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA							CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)			
			AREA (A) at C of				SUM(A)	SUM(A*1.25°C) 25% increase in C factor	TOTAL A°C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL						
	FROM	TO	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																										
NORTH CATCHMENT AREA																																
			Existing 900 mm dia. Culvert Capacity before ditch flows to Findlay Creek																													
NORTH SIDE RIDEAU ROAD	31	32	6.66			0.52	7.18	2.19	2.19	6.07	6.07	20.00	119.95	2128.6	0.00	1.50	3.00	1.93	24880.1	3.69	400.00								1.81	90.71	83.01	
												21.81																				
NORTH SIDE RIDEAU ROAD	33	32	0.87			0.10	0.97	0.32	0.32	0.88	0.88	15.00	142.89	126.1	0.00	1.50	3.00	0.16	7240.8	1.07	92.00								1.43	83.16	83.01	
												16.43																				
EXISTING CULVERT CROSSING	32	28				0.00	0.00	0.00	2.50	0.00	6.96	21.81	113.52	2189.7				-0.15			20.00	1	1000					0.12	83.01	83.04		
												21.93																				
SOUTH CATCHMENT AREA																																
SOUTH SIDE RIDEAU ROAD	28	29	0.90			0.33	1.23	0.56	15.91	1.54	44.24	27.18	98.22	5745.1	0.00	2.20	3.00	0.14	18513.7	1.28	347.24								4.54	83.04	82.56	
SOUTH SIDE RIDEAU ROAD	29	30	0.48			0.31	0.79	0.43	16.34	1.20	45.44	31.72	88.42	5417.3	0.00	2.20	3.00	0.51	35640.2	2.45	236.20								1.60	82.56	81.35	

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030

HAWTHORNE INDUSTRIAL PARK

1:10 YEAR ROADSIDE CULVERT DESIGN

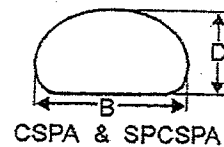
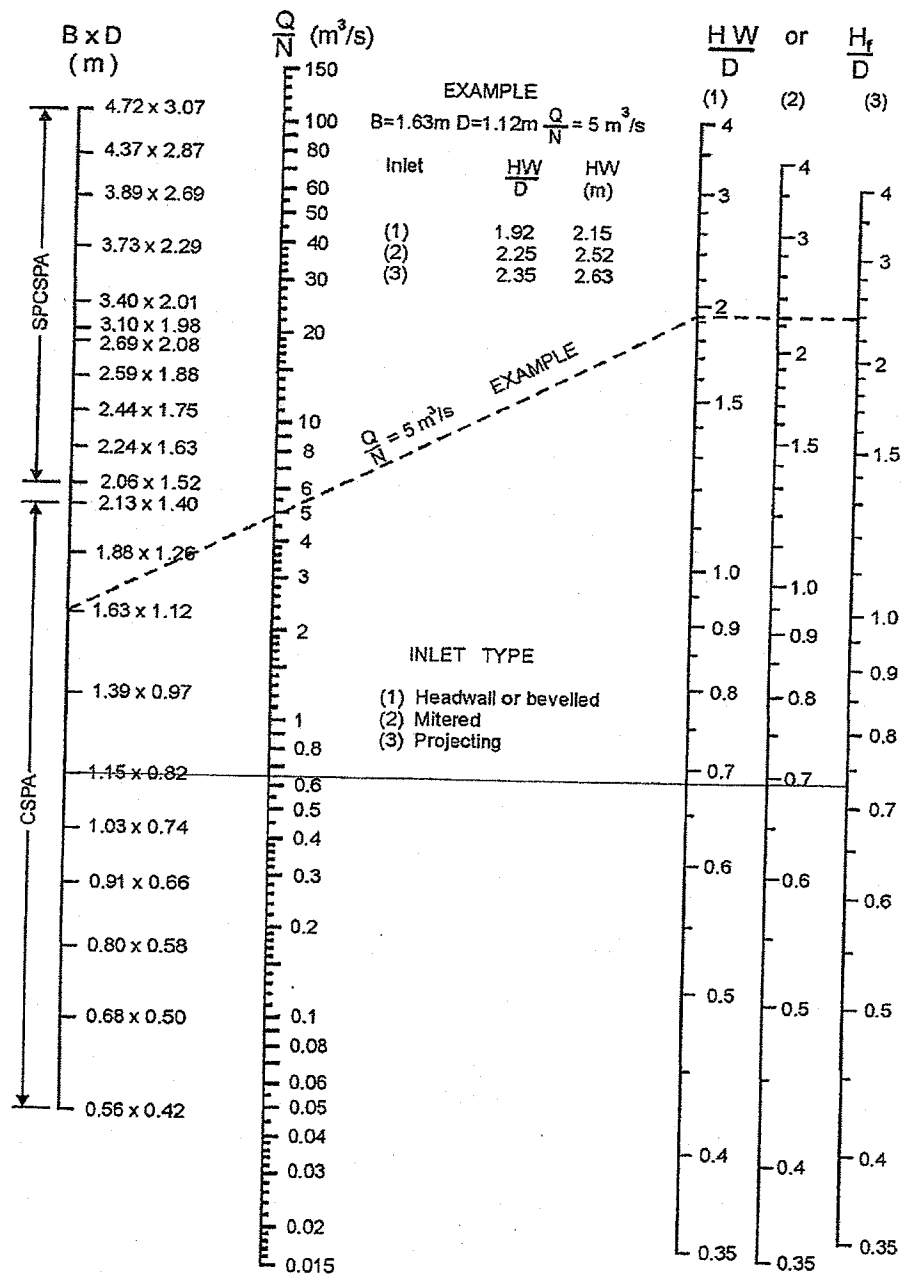
CONVENTIONAL CULVERT DESIGN

Prepared by: Mark Buchanan, E.I.T.  
Reviewed by: Guy Forget, P.Eng.  
Date: February 2009

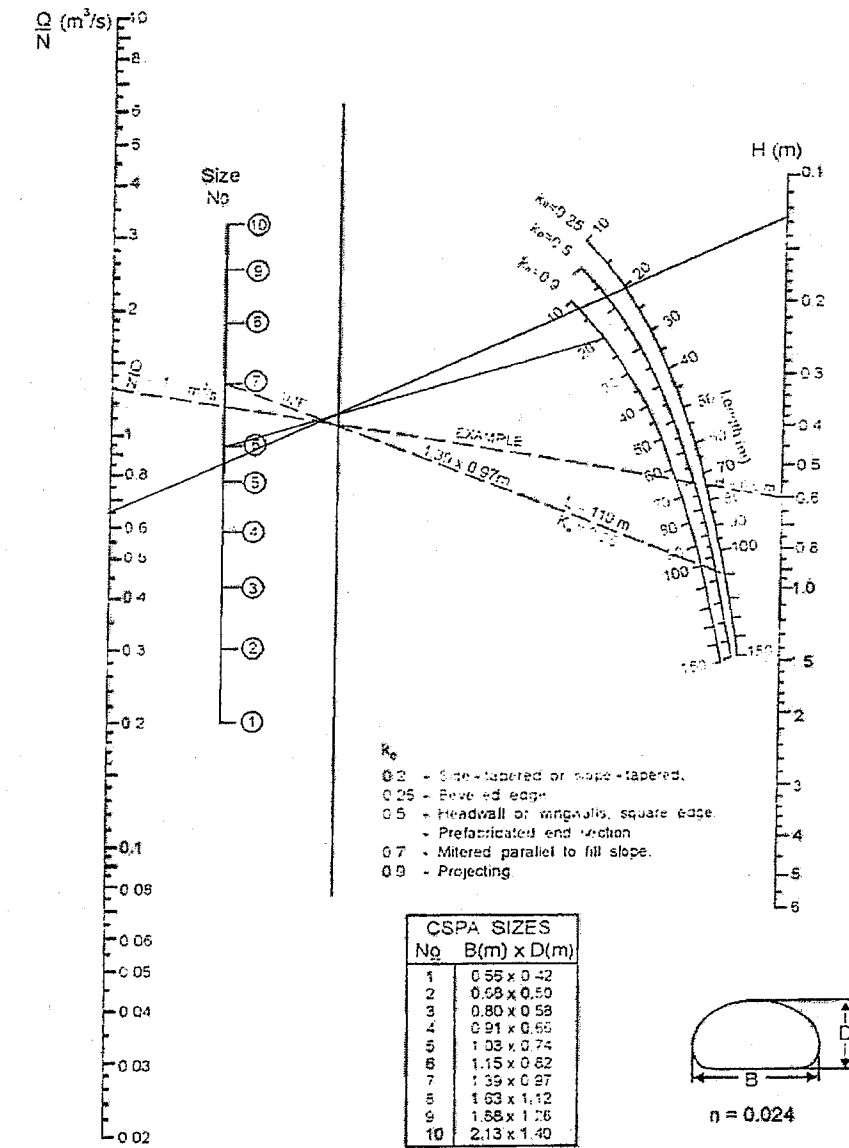
Station	DESIGN DATA							CULVERT DATA					INLET CONTROL			OUTLET CONTROL							GOVERNING HW	VEL V <sub>o</sub>		
	Q	d	d <sub>e</sub>	AHW	Skew No.	L	S	Description	B	D or H	N	Q/N	A (each)	Q/NB	HW/D	HW	K <sub>e</sub>	H	d <sub>e</sub>	(d <sub>e</sub> + D)/2	TW	h <sub>o</sub>			LS	HW
	(m³/s)	(m)	(m)	(m)		(m)	(m/m)		(m)	(m)		(m³/s)	(m²)	(m³/s/m)		(m)		(m)	(m)	(m)	(m)	(m)			(m)	(m)
1	2	3	4	5	6	7	8	9	10a	10b	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26
6 to 14	1.296	0.67	0.05	1.1	0	20.0	0.005	CSPA 6	1.15	0.82	2	0.648	0.74	---	0.73	0.60	0.9	0.13	0.33	0.58	0.72	0.72	0.10	0.75	0.75	
23B to 23C	0.051	0.22	0.05	1.15	0	24.0	0.004	CSP 500	N/A	0.5	1	0.051	0.20	----	0.50	0.25	0.9	0.1	0.15	0.33	0.27	0.33	0.10	0.33	0.33	
24A to 24B	0.075	0.25	0.05	1.15	0	24.0	0.004	CSP 500	N/A	0.5	1	0.075	0.20	----	0.54	0.27	0.9	0.1	0.18	0.34	0.30	0.34	0.10	0.34	0.34	
2 to 9	0.081	0.47	0.05	1.15	0	20.0	0.005	CSP 600	N/A	0.6	1	0.081	0.28	----	0.50	0.30	0.9	0.1	0.19	0.40	0.52	0.52	0.10	0.52	0.52	
27B to 27C	1.304	0.61	0.05	1.23	0	15.0	0.007	CSPA 7	1.39	0.97	1	1.304	1.06	----	0.90	0.87	0.9	0.22	0.45	0.71	0.66	0.71	0.11	0.82	0.87	
22 to 19	2.573	0.38	0.05	1.35	0	20.0	0.005	CSPA 5	1.03	0.74	2	1.287	0.61	----	1.75	1.30	0.9	0.74	0.51	0.63	0.43	0.63	0.10	1.27	1.30	
2 From Form PH-D-533, col. 12 3 Flood Depth 4 Embedment below channel invert 5 Col. 3 + col. 4 + allowable backwater 7 Allowance for skew if applicable 8 Culvert Slope 10a/b D (circular) or B x H (arch) 11 Number of Barrels 13 Area per barrel 14 For box only 15 Charts D5-1A to C and E to J 16 HW = col. 15 x D (col. 10) 17 Chart D5-8 18 Charts D5-2A to G 19 Charts D5-3A to F: (d <sub>e</sub> > D) 21 Col. 3 + col. 4 22 H <sub>o</sub> = larger of cols. 20 and 21 23 Col. 7 x col. 8 24 HW = col. 18 + col. 22 - col. 23 25 Larger of cols 16 and 24 26 Outlet velocity if required (Subsection 3.2.3)																										

Culvert Crossing  $\diamond 6-14 \diamond 2 \times 1.15 \text{ m} \times 0.82 \text{ m}$

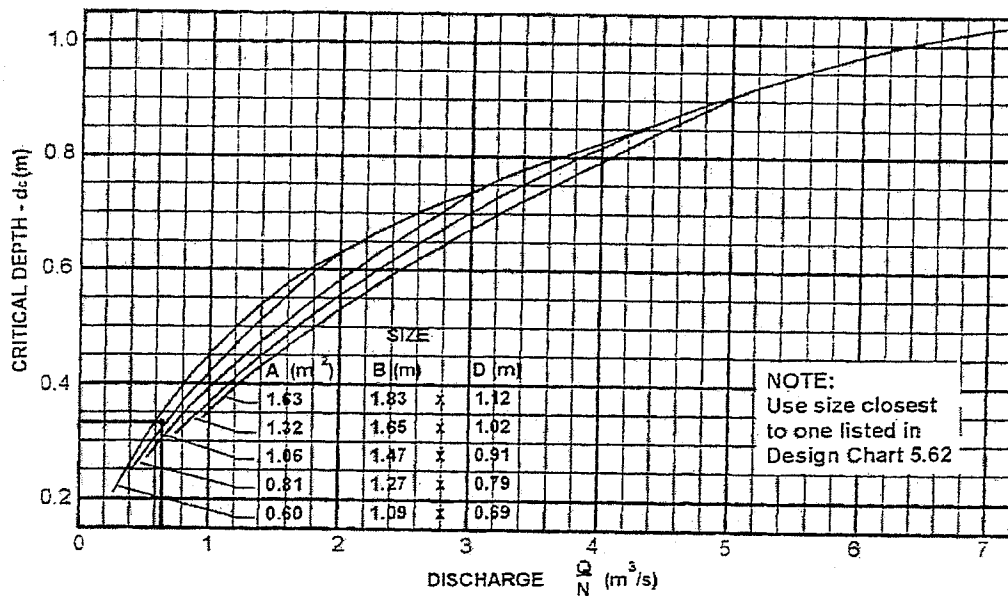
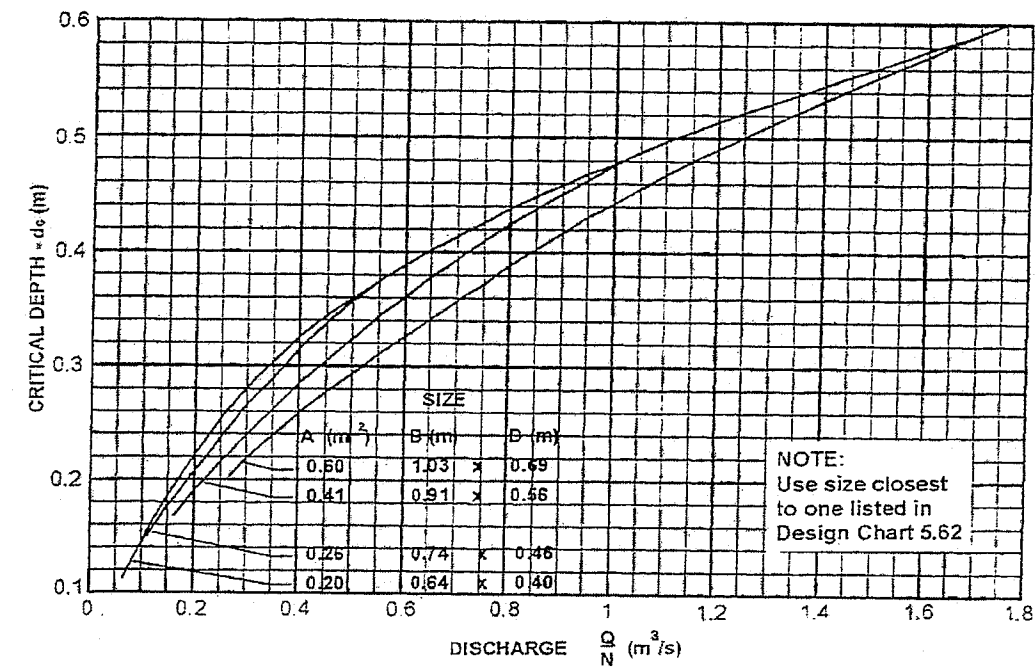
**Design Chart 5.43: Inlet Control: Steel Pipe Arch Culverts**



Source: Herr (1977)

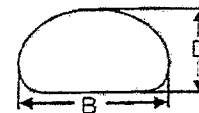
**Design Chart 5.47: Outlet Control: Pipe Arch CSP Culvert - Flowing Full**




**Design Chart 5.53: CSP Pipe Arch Culverts**

$$(d_c \geq D)$$

A = Cross-sectional area per barrel  
interpolated for other sizes

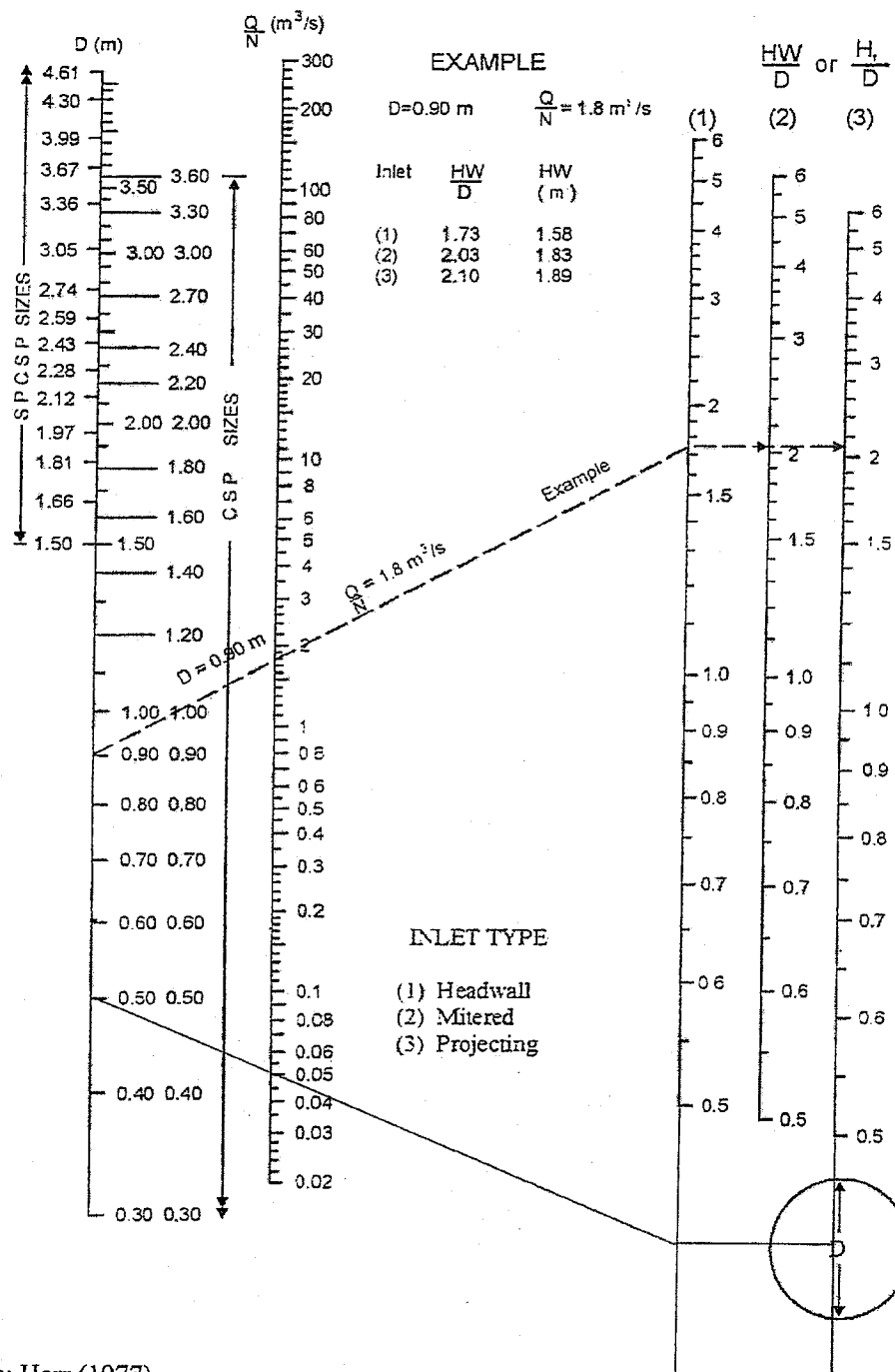


Source: Herr (1977)

Culvert Crossing 23b to 23c 500mm $\varnothing$

MTO Drainage Management Manual

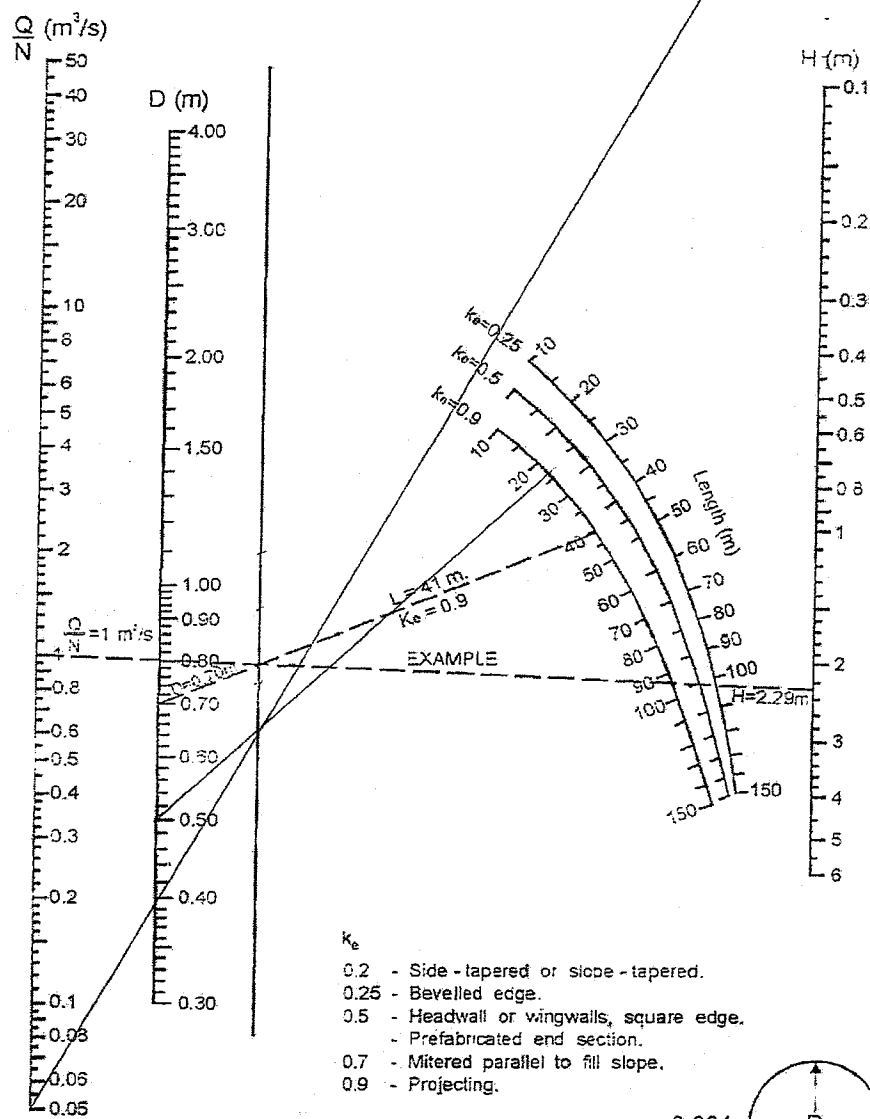
**Design Chart 2.32: Inlet Control: Circular CSP and SPCSP Culverts**



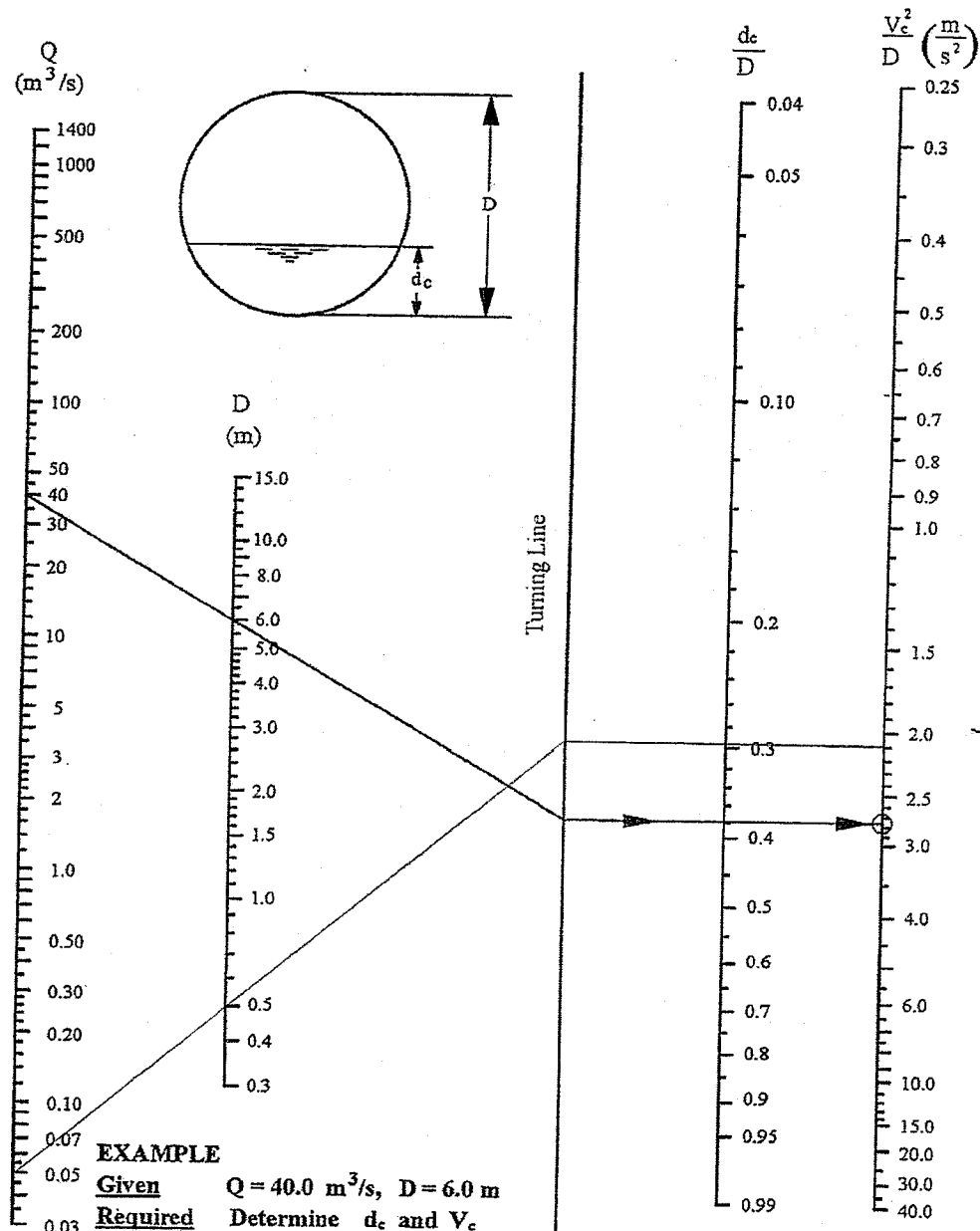
Source: Herr (1977)

Design Chart 2.35: Outlet Control: CSP Culvert - Flowing Full

$$H < 0.1 \text{ m}$$



Source: Herr (1977)

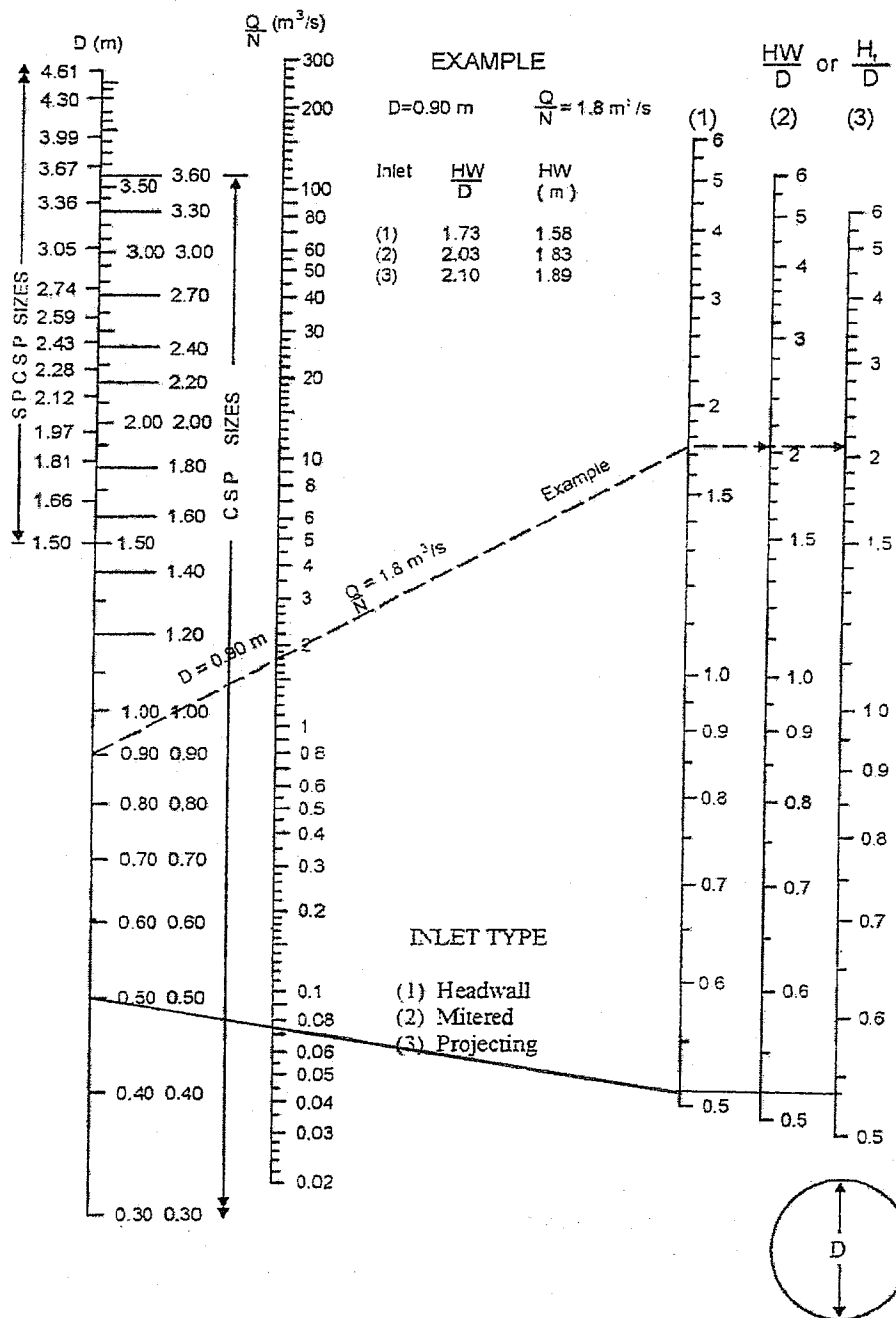
**Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes**

Source: American Iron and Steel Institute

Culvert Crossing 24a to 24b 500 mm  $\phi$

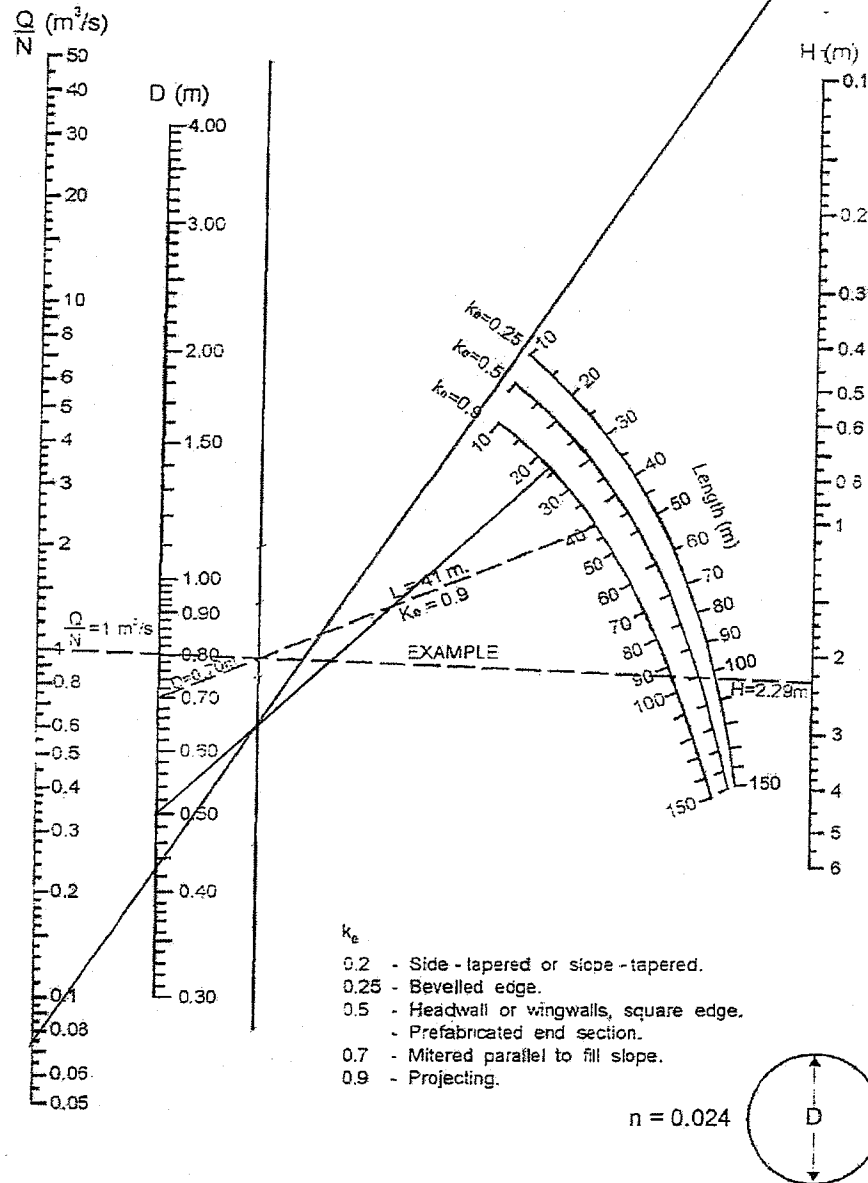
MTO Drainage Management Manual

**Design Chart 2.32: Inlet Control: Circular CSP and SPCSP Culverts**

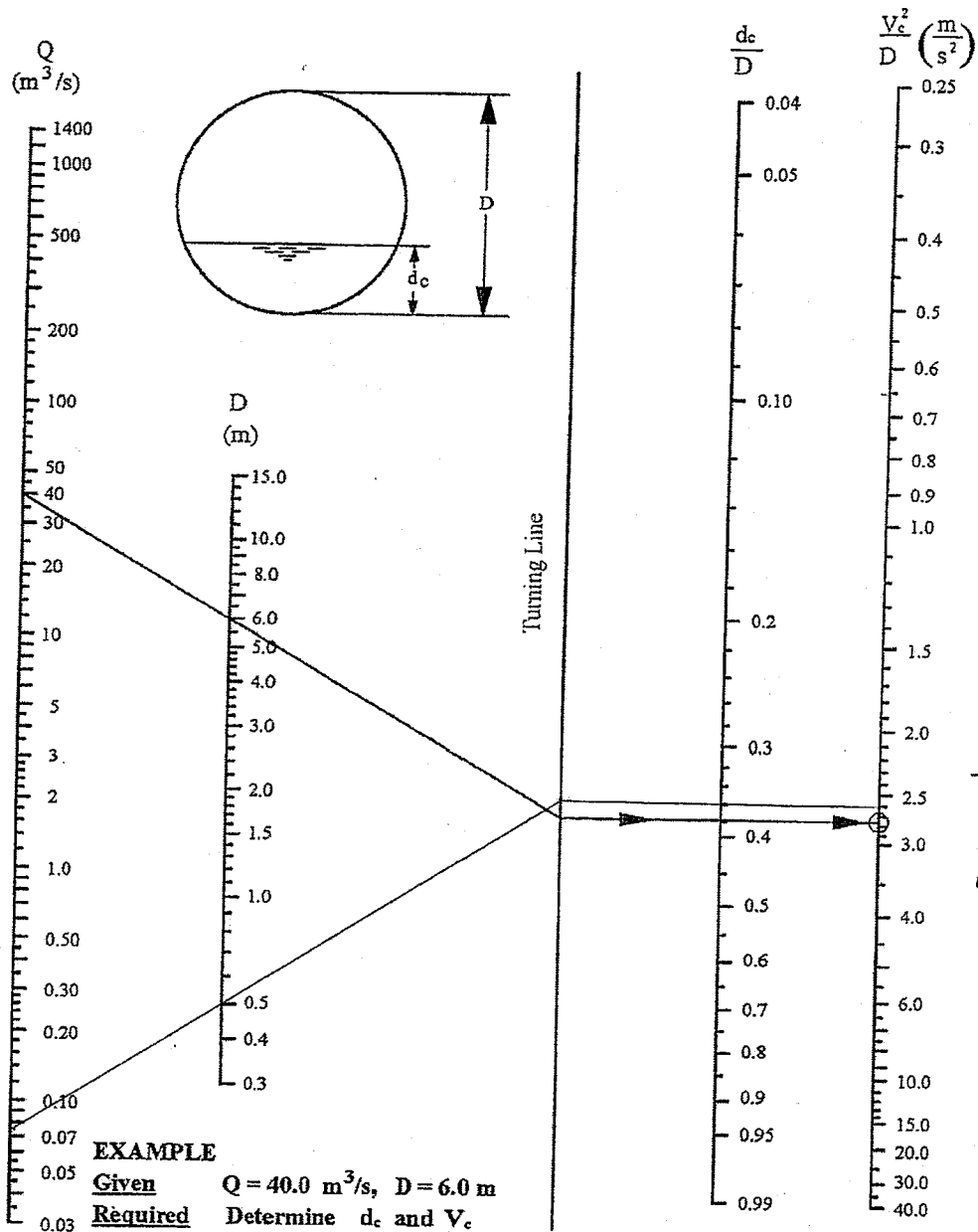


Source: Herr (1977)

Design Chart 2.35: Outlet Control: CSP Culvert - Flowing Full

 $H < 0.1m$ 

Source: Herr (1977)

**Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes**

$$\frac{d_c}{D} = 0.36$$

$$d_c = 0.36 \times 0.5$$

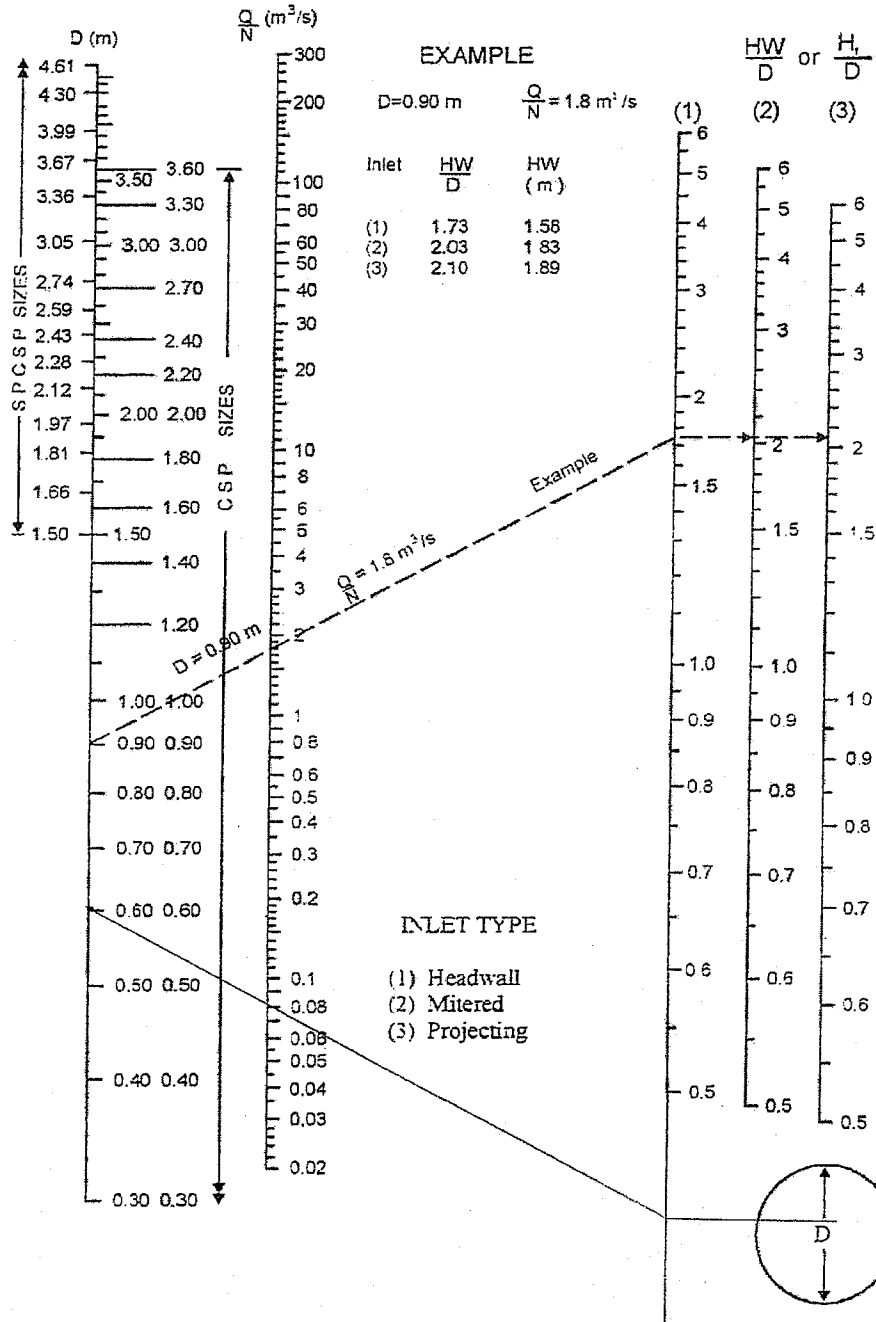
$$= 0.18$$

Source: American Iron and Steel Institute

# Culvert Crossing 2 - 9 600 mm $\phi$

MTO Drainage Management Manual

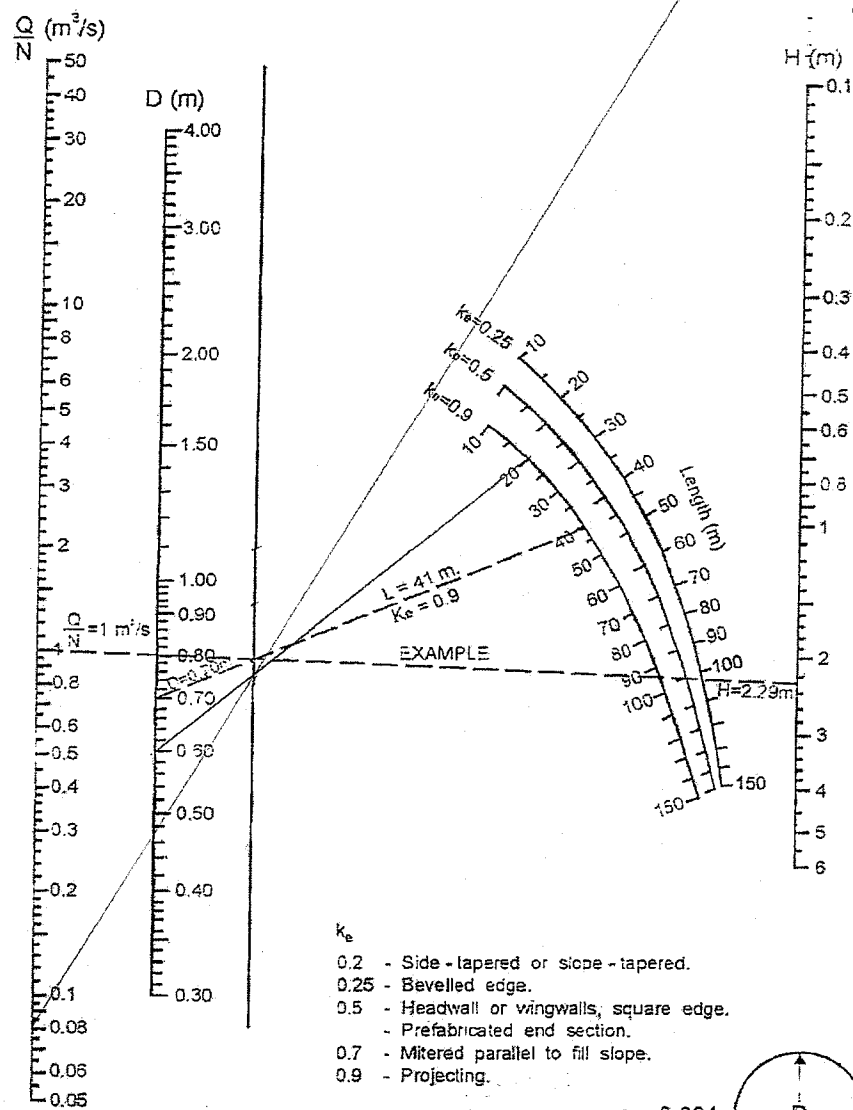
## Design Chart 2.32: Inlet Control: Circular CSP and SPCSP Culverts



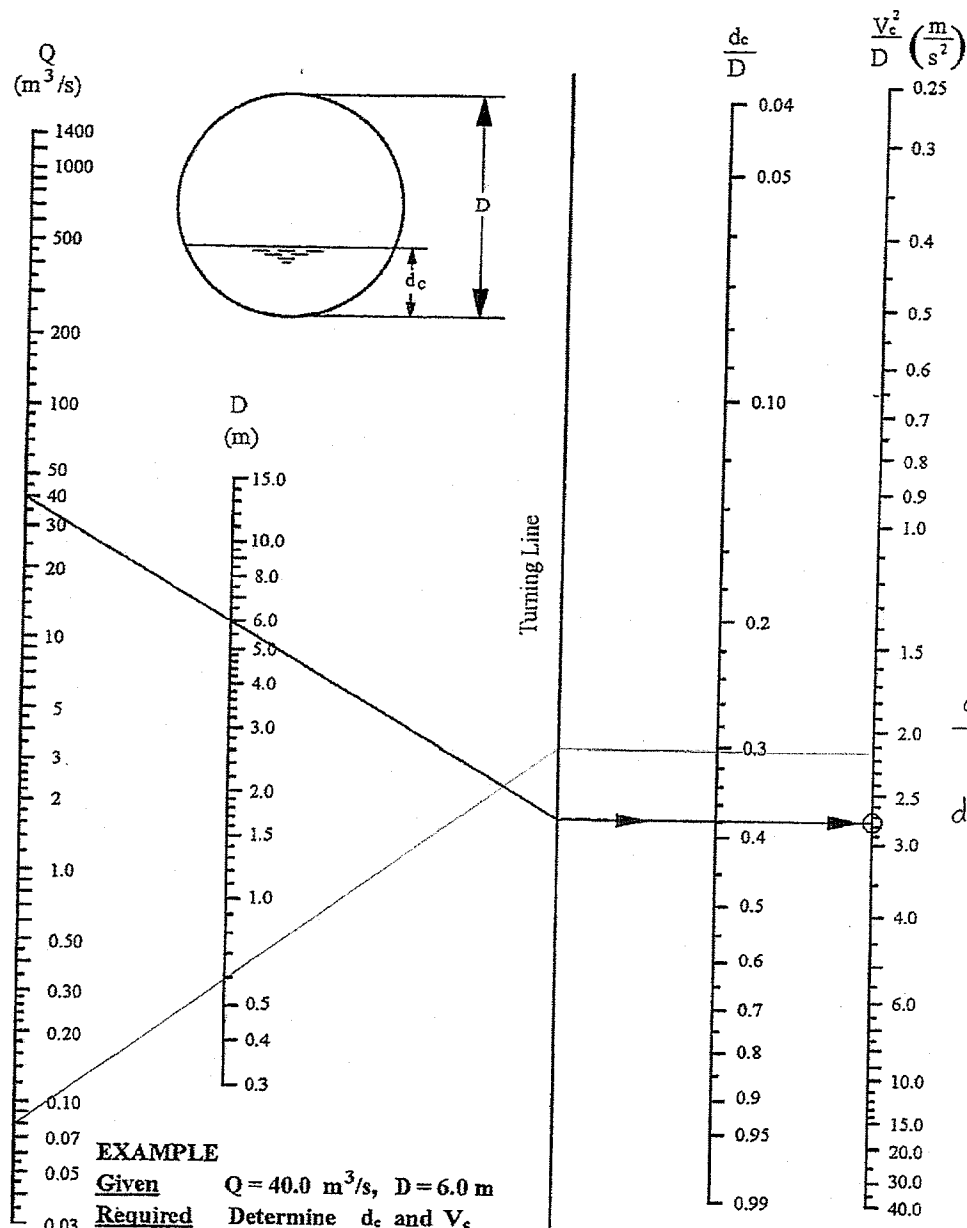
Source: Herr (1977)



Design Chart 2.35: Outlet Control: CSP Culvert - Flowing Full



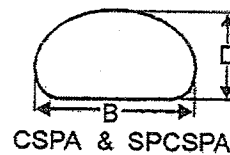
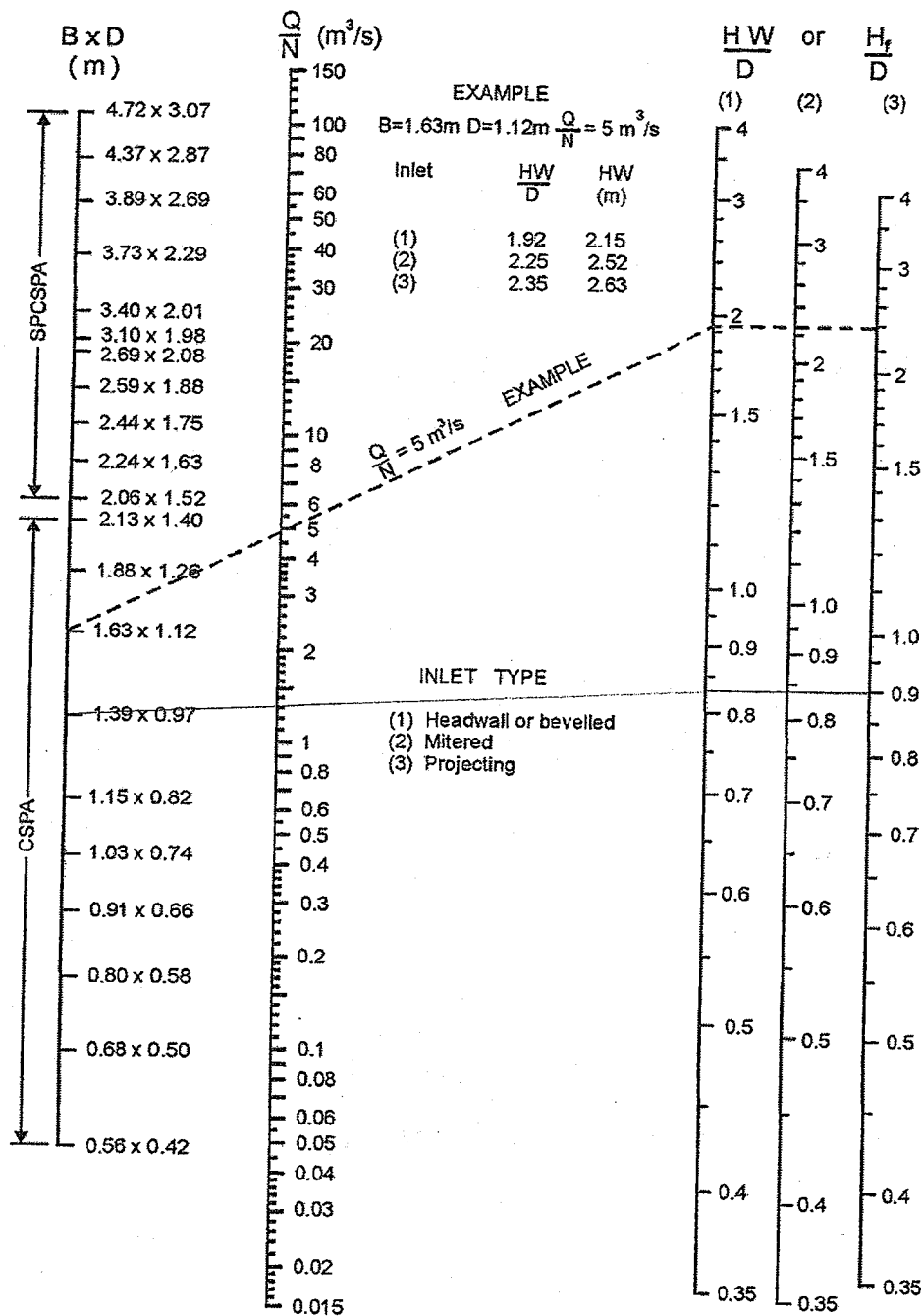
Source: Herr (1977)

**Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes**

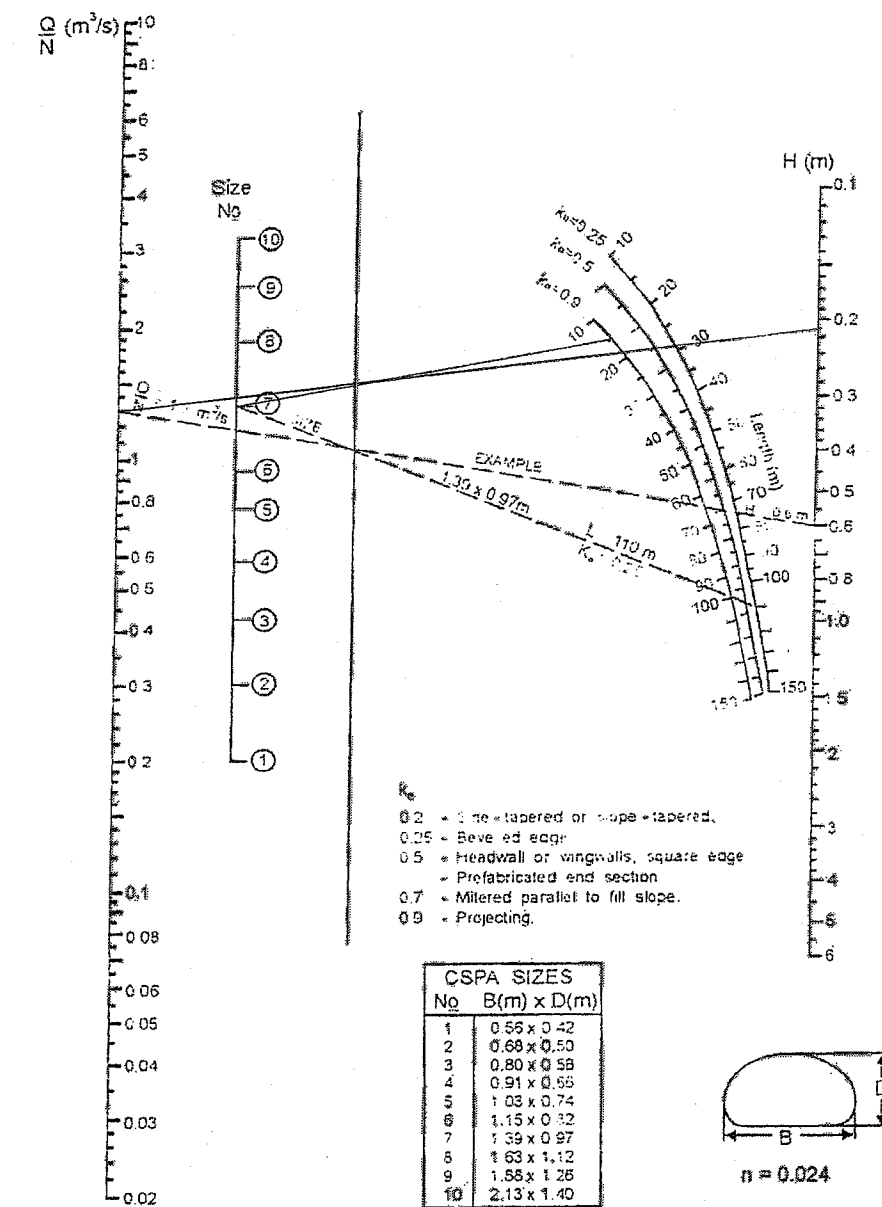
Source: American Iron and Steel Institute

Culvert Crossing 27b to 27c 1.39 x 0.97 m

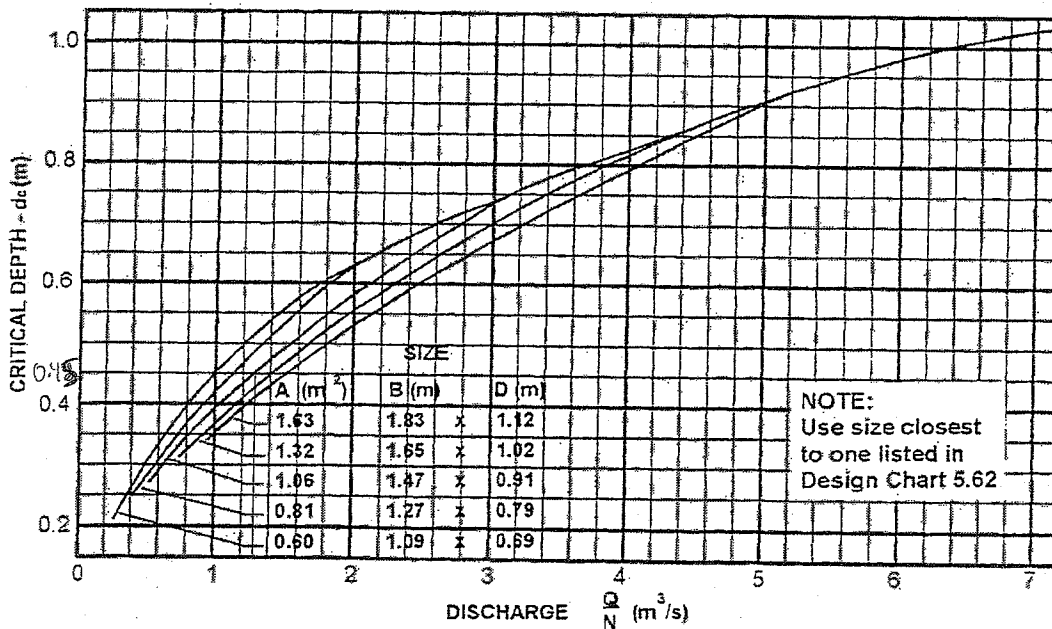
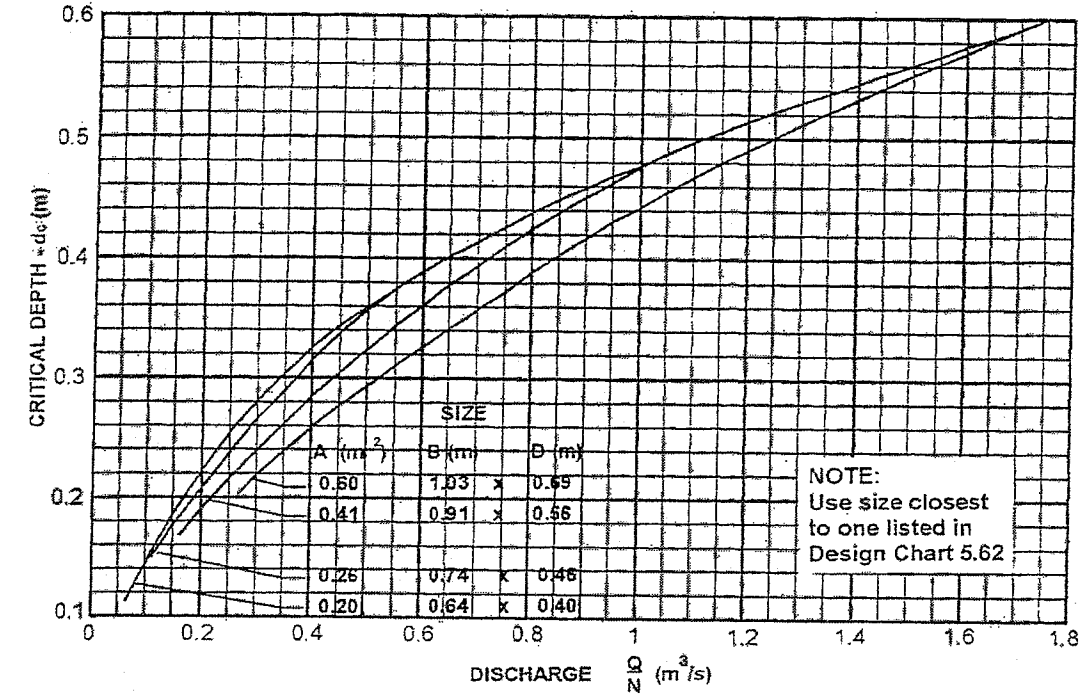
**Design Chart 5.43: Inlet Control: Steel Pipe Arch Culverts**



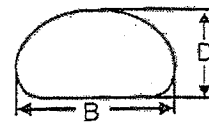
Source: Herr (1977)

**Design Chart 5.47: Outlet Control: Pipe Arch CSP Culvert - Flowing Full**


Source: Herr (1977)

**Design Chart 5.53: CSP Pipe Arch Culverts**


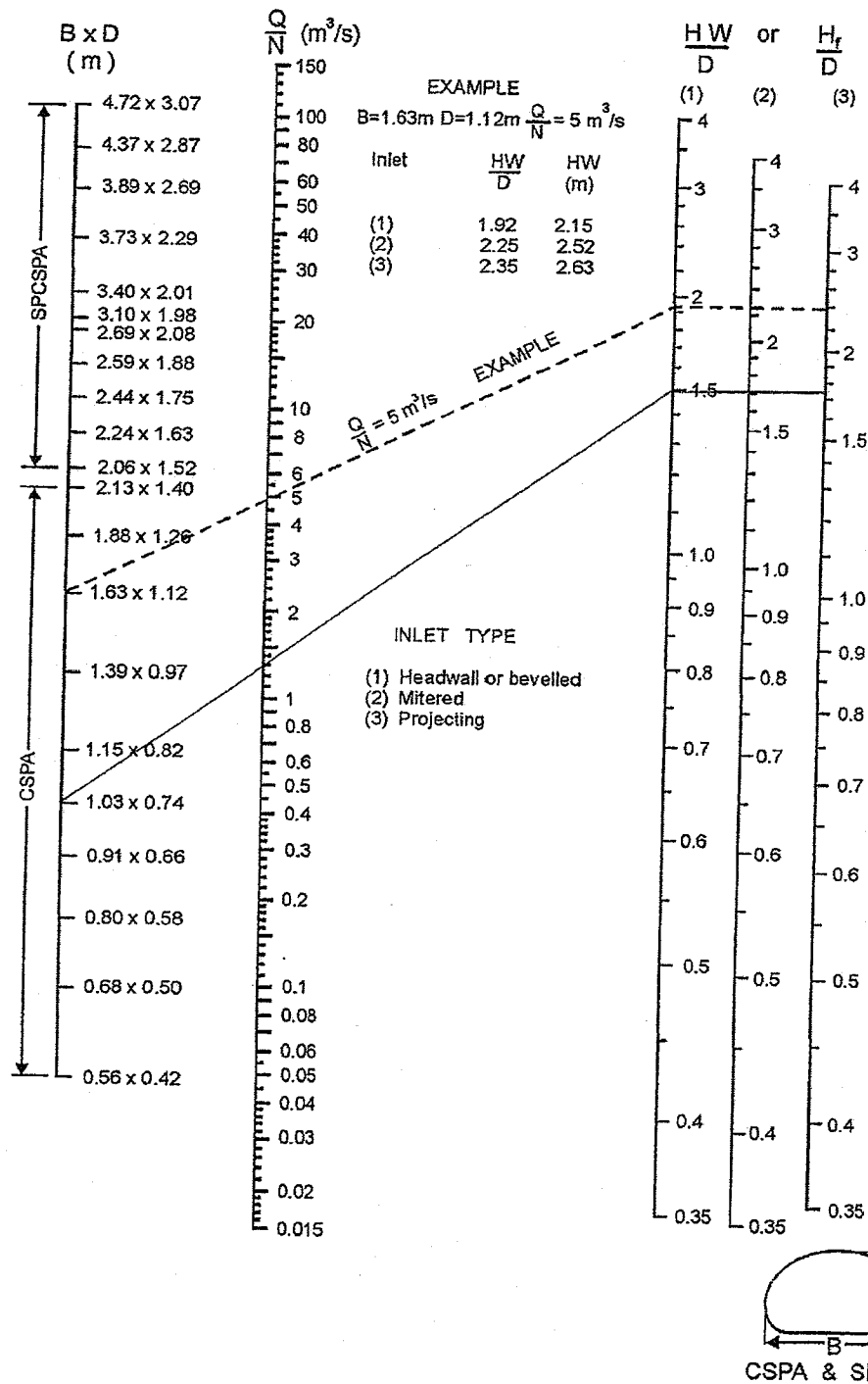
( $d_c \neq D$ )  
 $A$  = Cross-sectional area per barrel  
 interpolated for other sizes



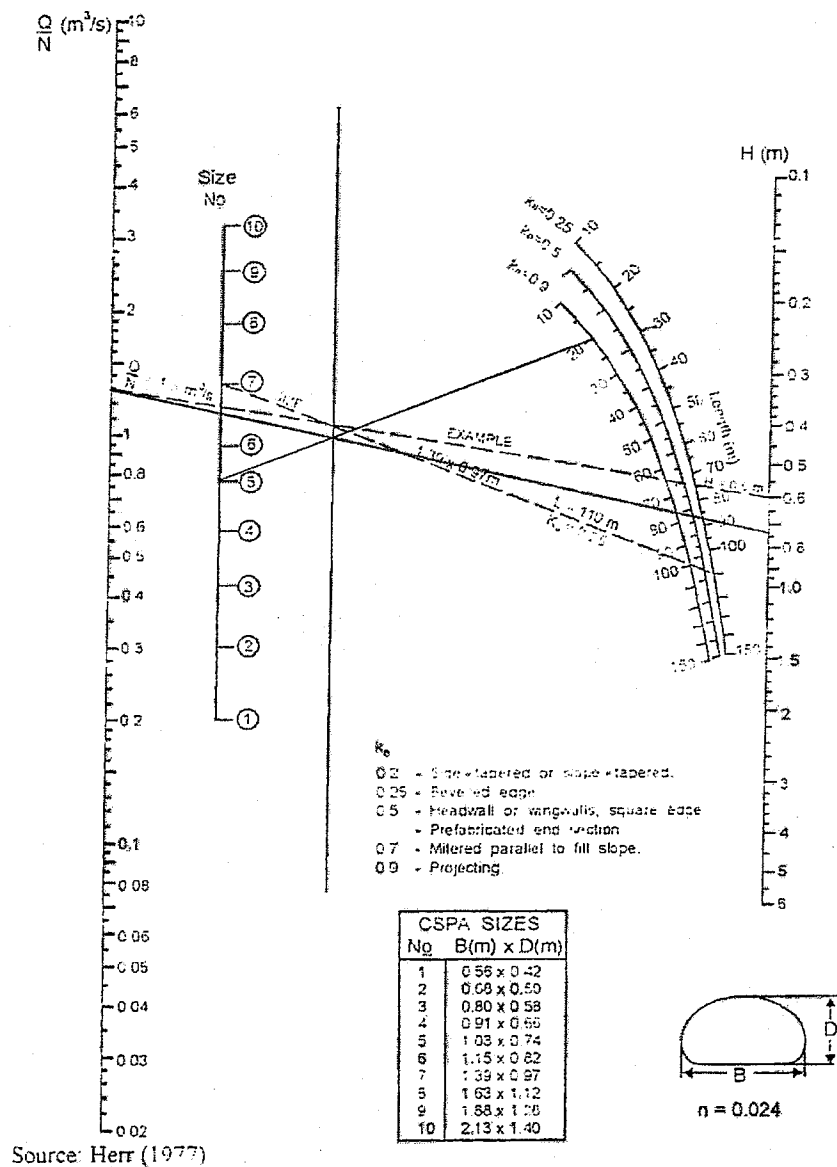
Source: Herr (1977)

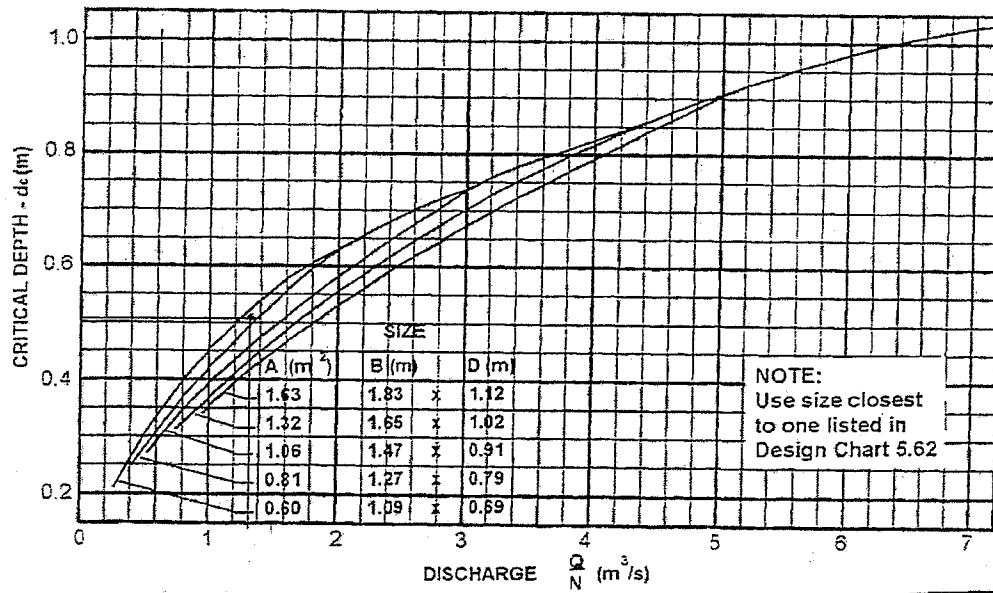
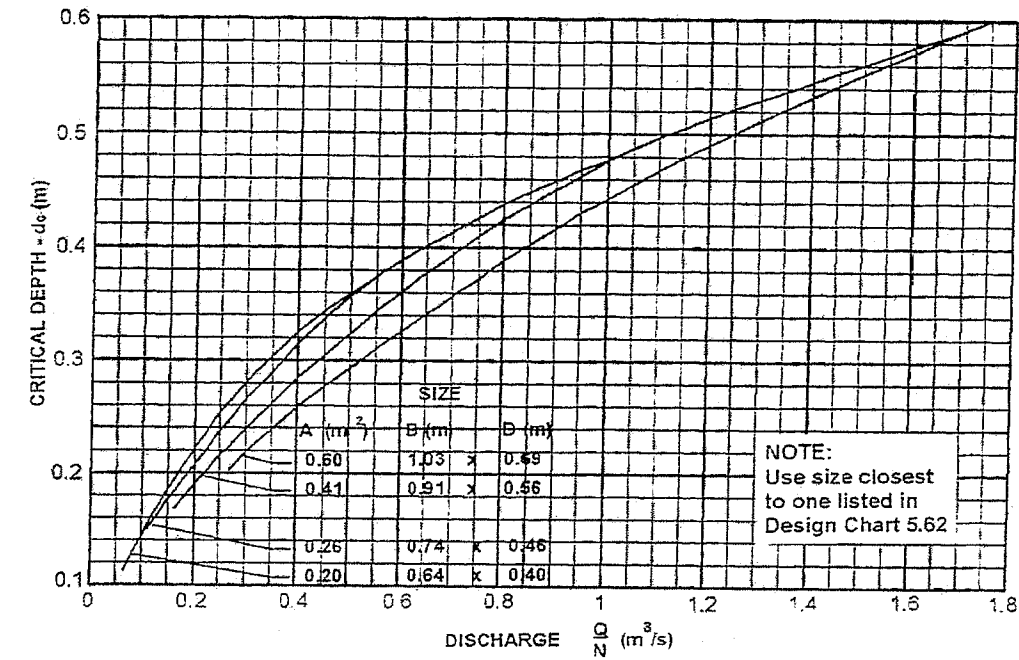
Culvert Crossing 22 - 79  $2 \times 1.03\text{m} \times 0.74\text{m}$

**Design Chart 5.43: Inlet Control: Steel Pipe Arch Culverts**



Source: Herr (1977)

**Design Chart 5.47: Outlet Control: Pipe Arch CSP Culvert - Flowing Full**


**Design Chart 5.53: CSP Pipe Arch Culverts**

 $(d_c \neq D)$ 

A = Cross-sectional area per barrel  
interpolated for other sizes



Source: Herr (1977)



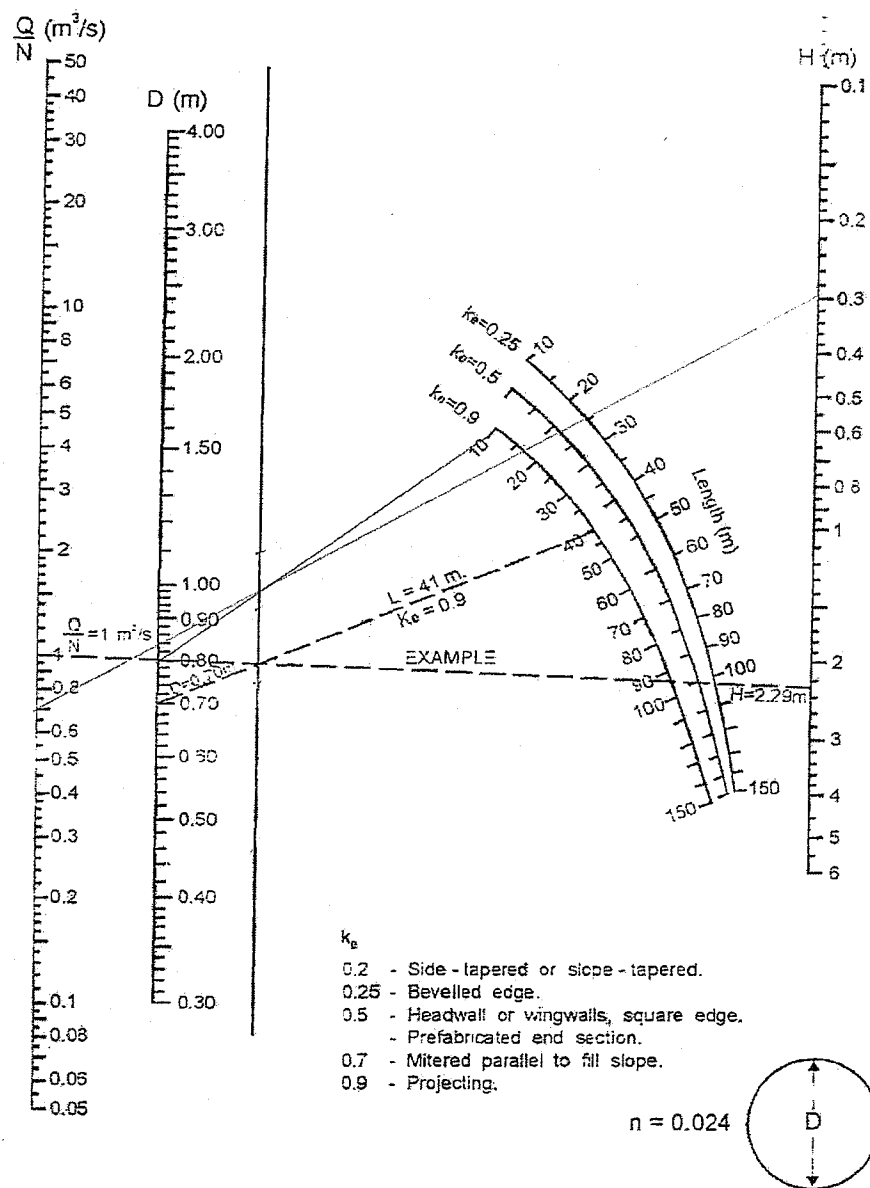
**A P P E N D I X 'B'**

**CONVENTIONAL CULVERT DESIGN SHEET**

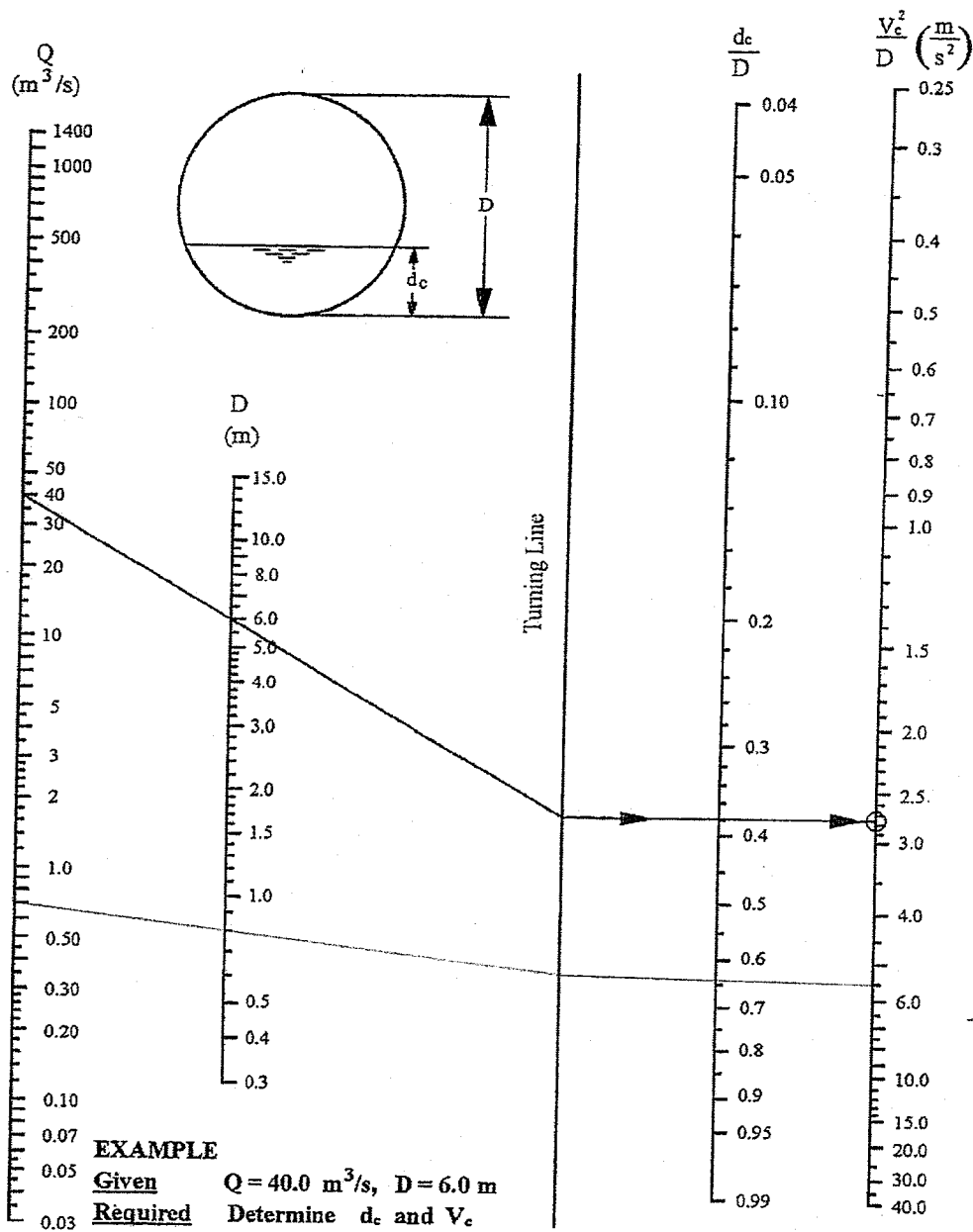
## CONVENTIONAL CULVERT DESIGN

2 From Form PH-D-533, col. 12	8 Culvert Slope	15 Charts D5-1A to C and E to J	21 Col. 3 + col. 4	26 Outlet velocity if required (Subsection 3.2.3)
3 Flood Depth	10 D (circular) or B x D (other)	16 HW = col. 15 x D (col. 10)	22 $H_o$ = larger of cols. 20 and 21	
4 Embedment below channel invert	11 Number of Barrels	17 Chart D5-8	23 Col. 7 x col. 8	
5 Col. 3 + col. 4 + allowable backwater	13 Area per barrel	18 Charts D5-2A to G	24 HW = col. 18 + col. 22 - col. 23	
7 Allowance for skew if applicable	14 For box only	19 Charts D5-3A to F: ( $d_o > D$ )	25 Larger of cols 16 and 24	

Design Chart 2.35: Outlet Control: CSP Culvert - Flowing Full



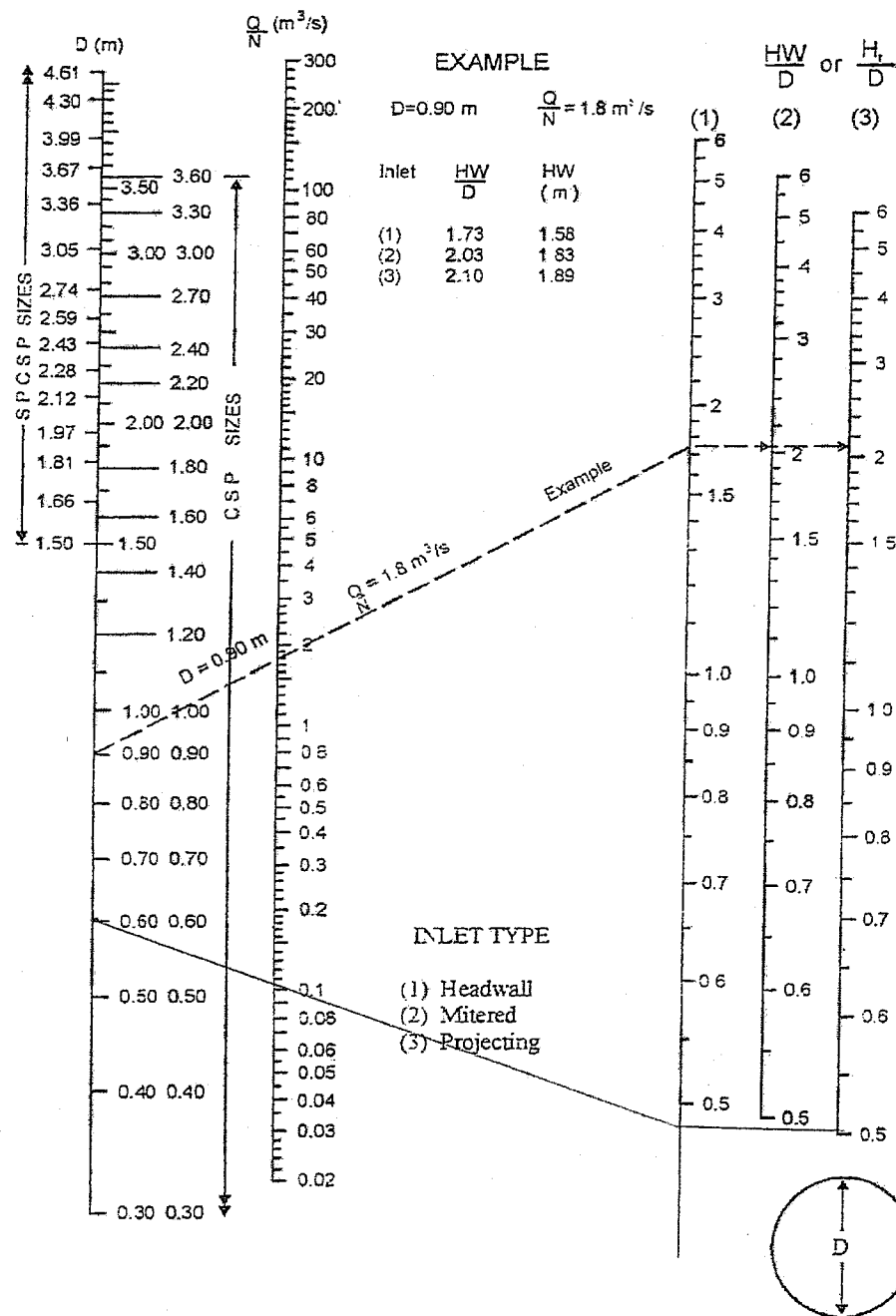
Source: Herr (1977)

**Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes**

Source: American Iron and Steel Institute

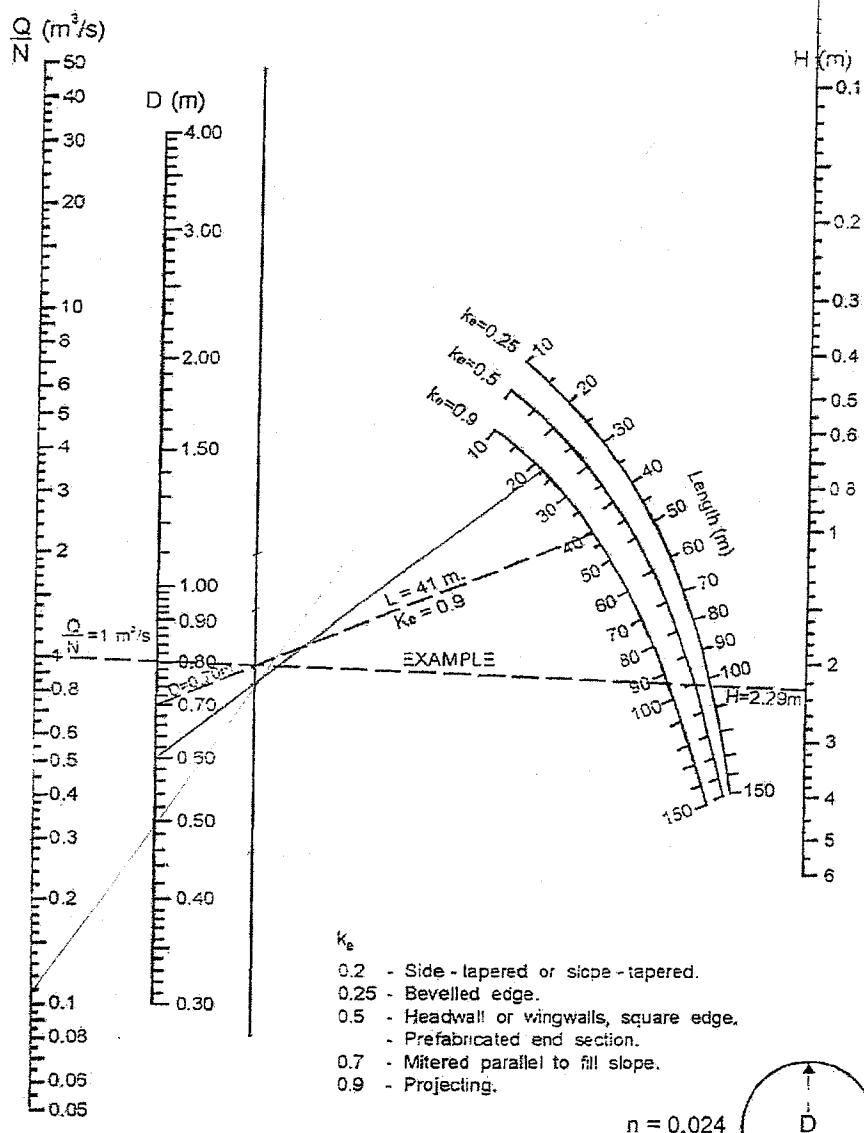
Culvert Crossing  $\diamond 18$  to  $\diamond 19$  600 mm  $\phi$

**Design Chart 2.32: Inlet Control: Circular CSP and SPCSP Culverts**



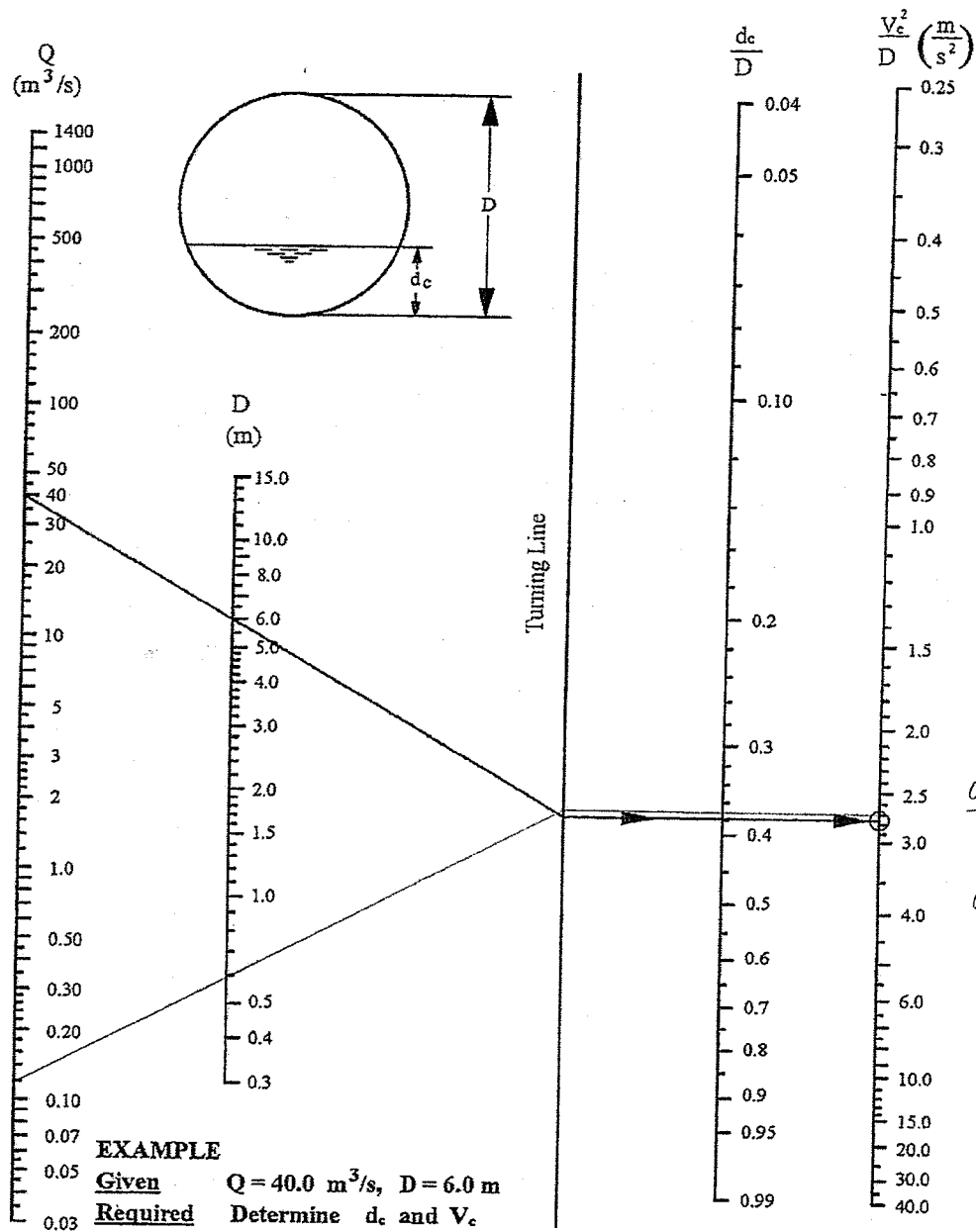
Source: Herr (1977)

Design Chart 2.35: Outlet Control: CSP Culvert - Flowing Full



Source: Herr (1977)

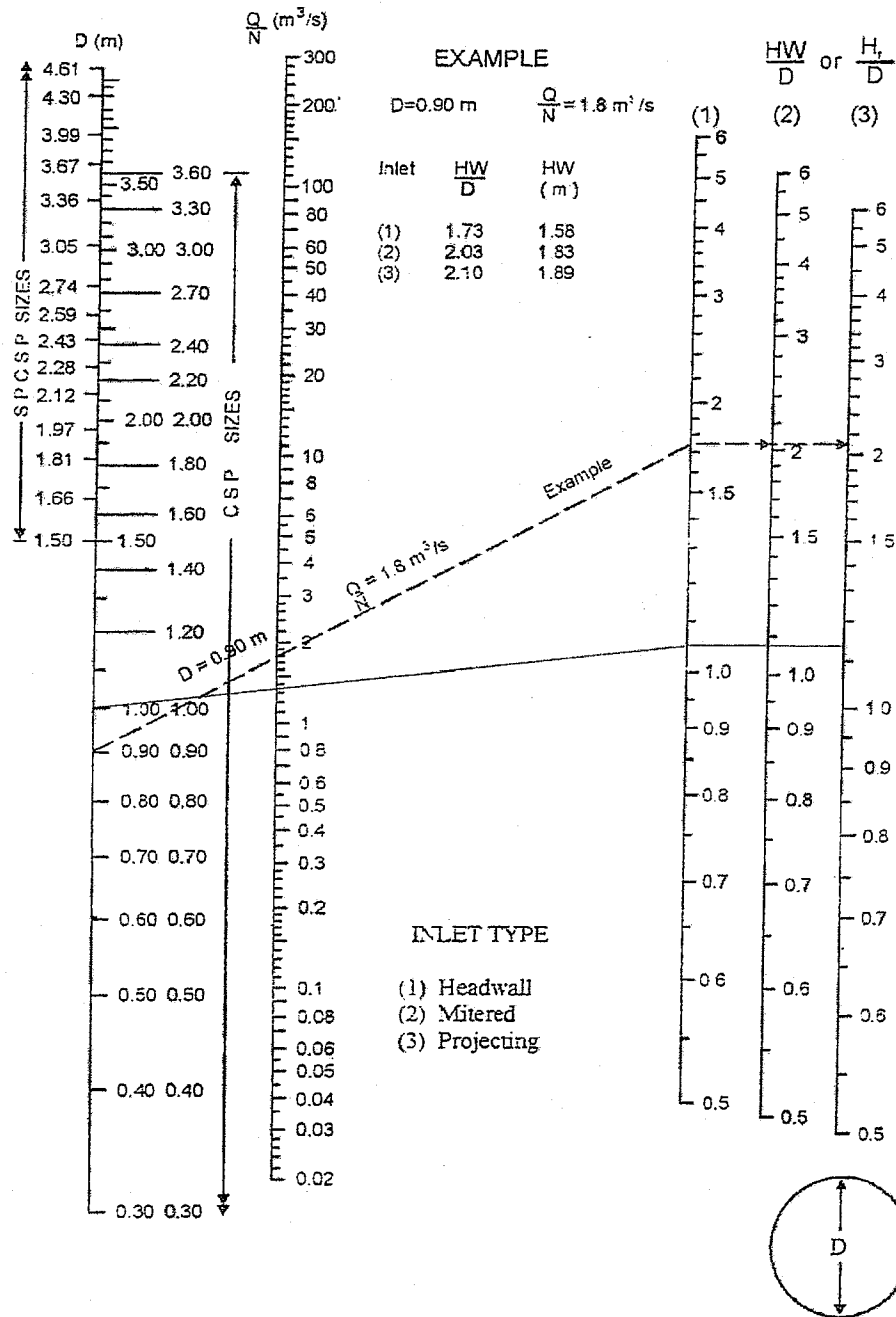
## Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes

**EXAMPLE****Given**  $Q = 40.0 \text{ m}^3/\text{s}$ ,  $D = 6.0 \text{ m}$ **Required** Determine  $d_c$  and  $V_c$ **Solution** Join  $Q = 40.0 \text{ m}^3/\text{s}$  to  $D = 6.0 \text{ m}$  and extend to turning line.Draw a horizontal line perpendicular to the turning line to intersect  $d_c/D = 0.38$  and  $V_c^2/D = 2.76$ Calculate  $d_c = 0.38 \times 6.0 = 2.28 \text{ m}$ .  
 $V_c = (2.76 \times 6.0)^{0.5} = 4.07 \text{ m/s}$ 

Source: American Iron and Steel Institute

Culvert Crossing 14 to 23 1000 mm Ø

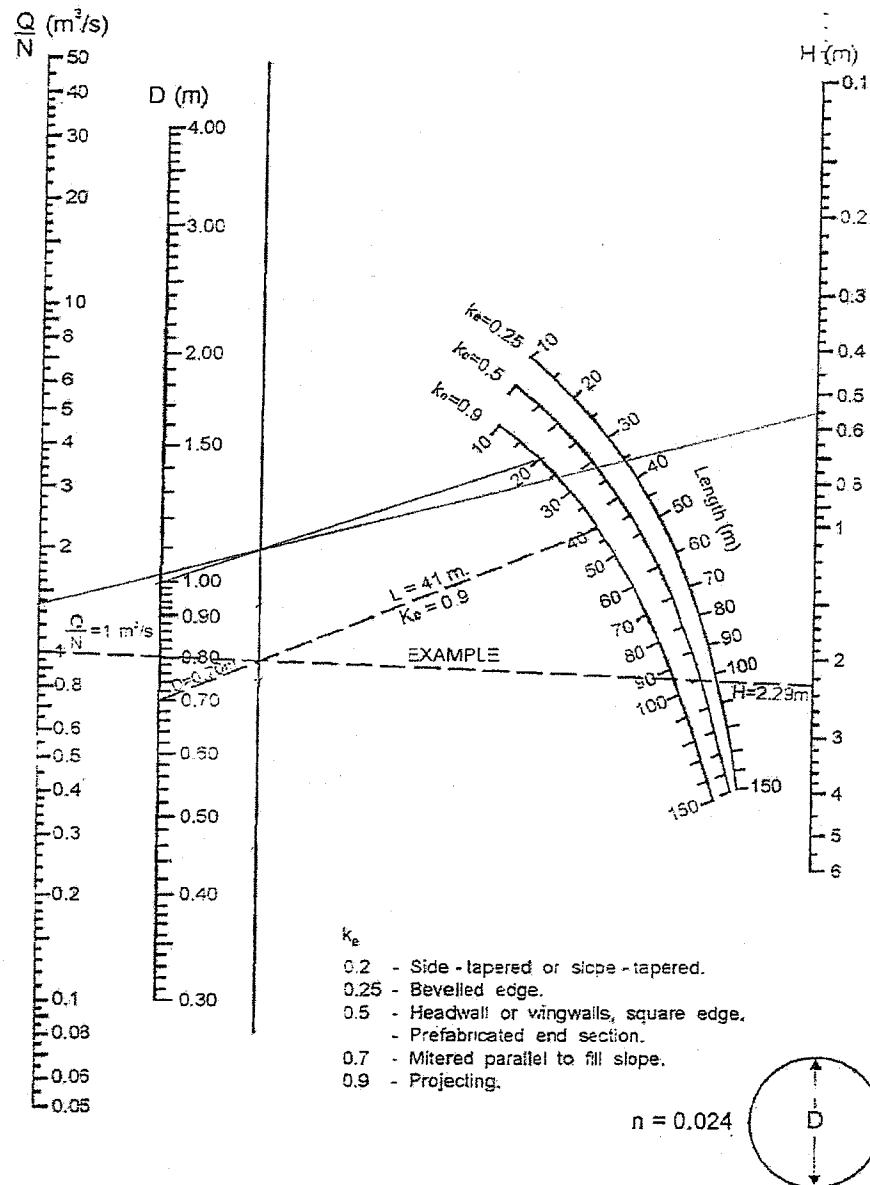
**Design Chart 2.32: Inlet Control: Circular CSP and SPCSP Culverts**



Source: Herr (1977)

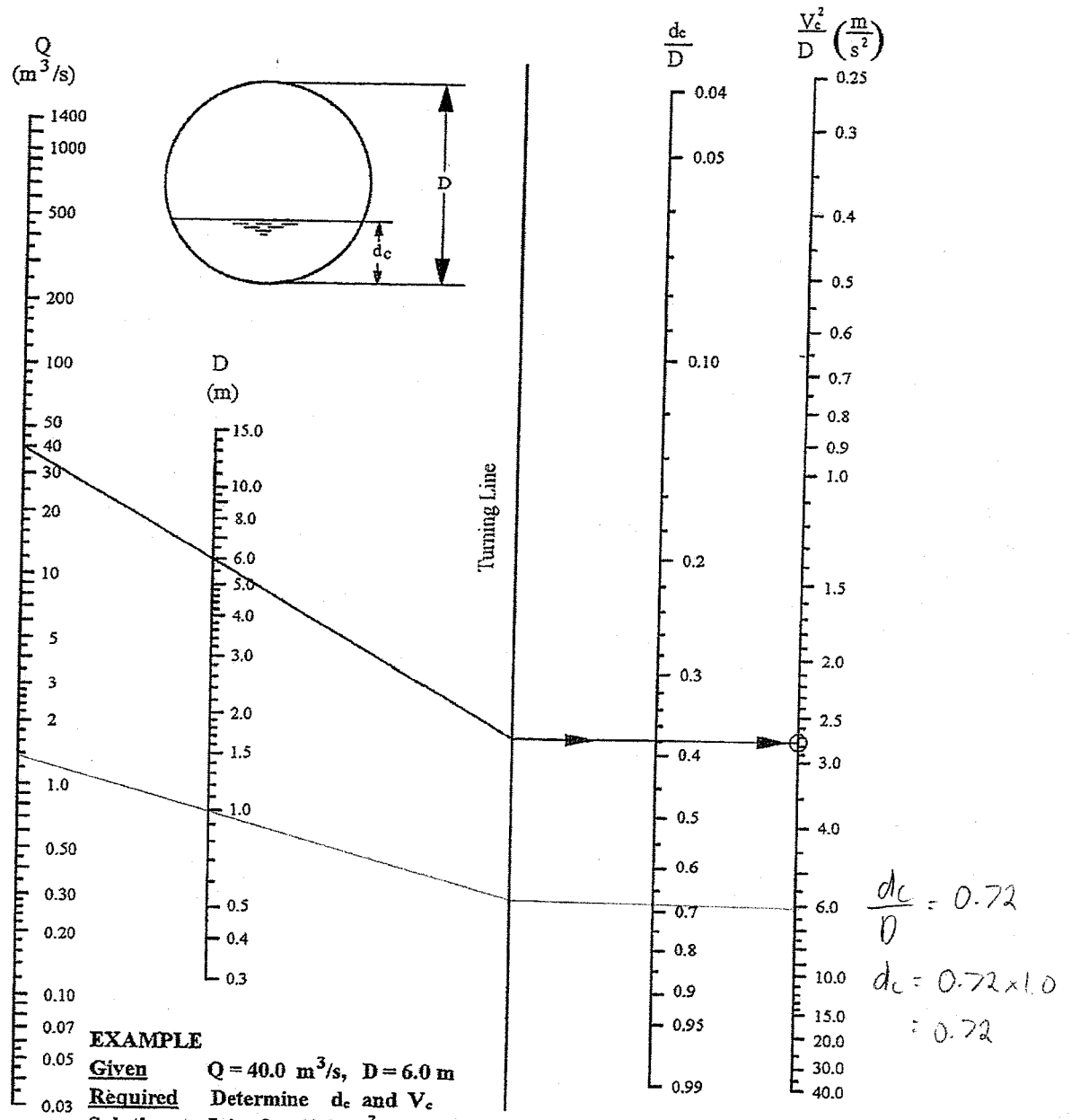


Design Chart 2.35: Outlet Control: CSP Culvert - Flowing Full

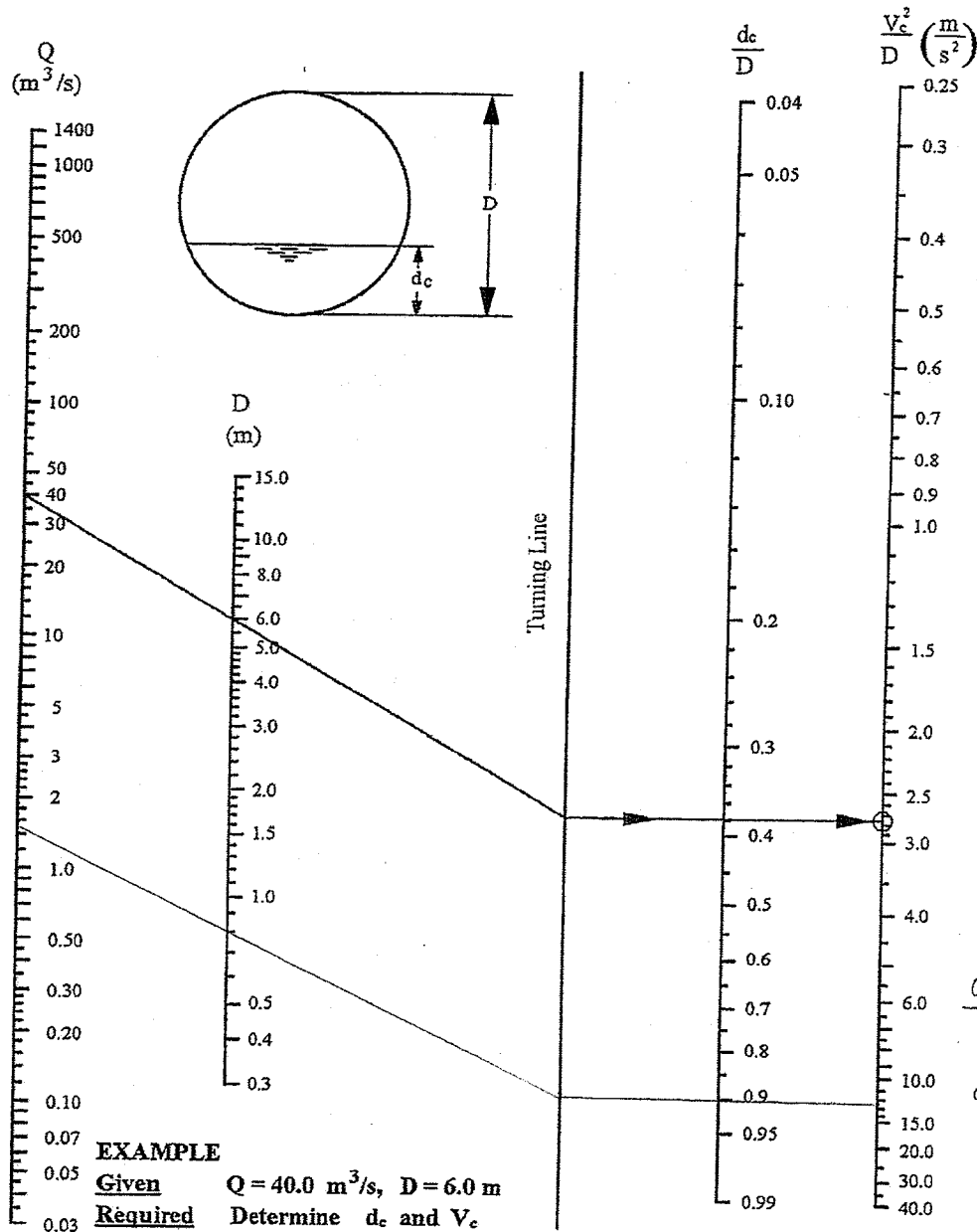


Source: Herr (1977)

## Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes



Source: American Iron and Steel Institute

**Design Chart 2.38: Critical Depth - Velocity relationships: Circular Pipes****EXAMPLE****Given**  $Q = 40.0 \text{ m}^3/\text{s}$ ,  $D = 6.0 \text{ m}$ **Required** Determine  $d_c$  and  $V_c$ **Solution** Join  $Q = 40.0 \text{ m}^3/\text{s}$  to  $D = 6.0 \text{ m}$  and extend to turning line.Draw a horizontal line perpendicular to the turning line to intersect  $d_c/D = 0.38$  and  $V_c^2/D = 2.76$ Calculate  $d_c = 0.38 \times 6.0 = 2.28 \text{ m}$ . $V_c = (2.76 \times 6.0)^{0.5} = 4.07 \text{ m/s}$ 

Source: American Iron and Steel Institute

## **APPENDIX 'C'**

### **WATER QUALITY - INFILTRATION CALCULATION**

JOB NO. 20983

PROJECT Hawthorne Industrial Park

Length of Perforated Pipe in Ditches

BY MB DATE Apr 14/09

Level of Service

Normal 70% TSS removal

Imperviousness 100% for internal roads

Extrapolating from Table 3.2 SWMPDM

water quality infiltration requirement =  $35 \text{ m}^3/\text{ha}$

Area of Asphalt

Phase 1

$$\begin{array}{l} \text{Length} = 2250 \text{ m} \\ \text{width} = \frac{7 \text{ m}}{15750 \text{ m}^2} \end{array}$$

Required Storage

$$= 1.575 \text{ ha} \times 35 \frac{\text{m}^3}{\text{ha}}$$

$$= 55.1 \text{ m}^3$$

Phase 2

$$\begin{array}{l} 300 \text{ m} \\ 7 \text{ m} \\ \hline 2100 \text{ m}^2 \end{array}$$

$$= 0.21 \text{ ha} \times 35 \frac{\text{m}^3}{\text{ha}}$$

$$= 7.35 \text{ m}^3$$

Required Length of 200mm  $\phi$  Perforated Pipe

$$\text{Length} = \frac{55.1 \text{ m}^3}{\pi (0.1)^2 \text{ m}^2}$$

$$= \underline{\underline{1755 \text{ m}}}$$

$$= \frac{7.35 \text{ m}^3}{\pi (0.1)^2 \text{ m}^2}$$

$$= \underline{\underline{234 \text{ m}}}$$



## **A P P E N D I X 'D'**

### **HYDROLOGICAL PARAMETERS**

**(CN<sub>pre</sub>, Imperviousness Calculation, Time to Peak Calculation)**

JOB NO. 20983

PROJECT Hawthorne Industrial Park

% Impervious Calculation

BY MB DATE Jan 22/09



Typical Site Development with  $C=0.7$

Building Footprint 10%

Asphalt Parking 35%

Gravel 35%

Grass 20%

100%

Building Foot print = 100% Impervious

Asphalt Parking = 100% Impervious

Gravel = 70% Impervious

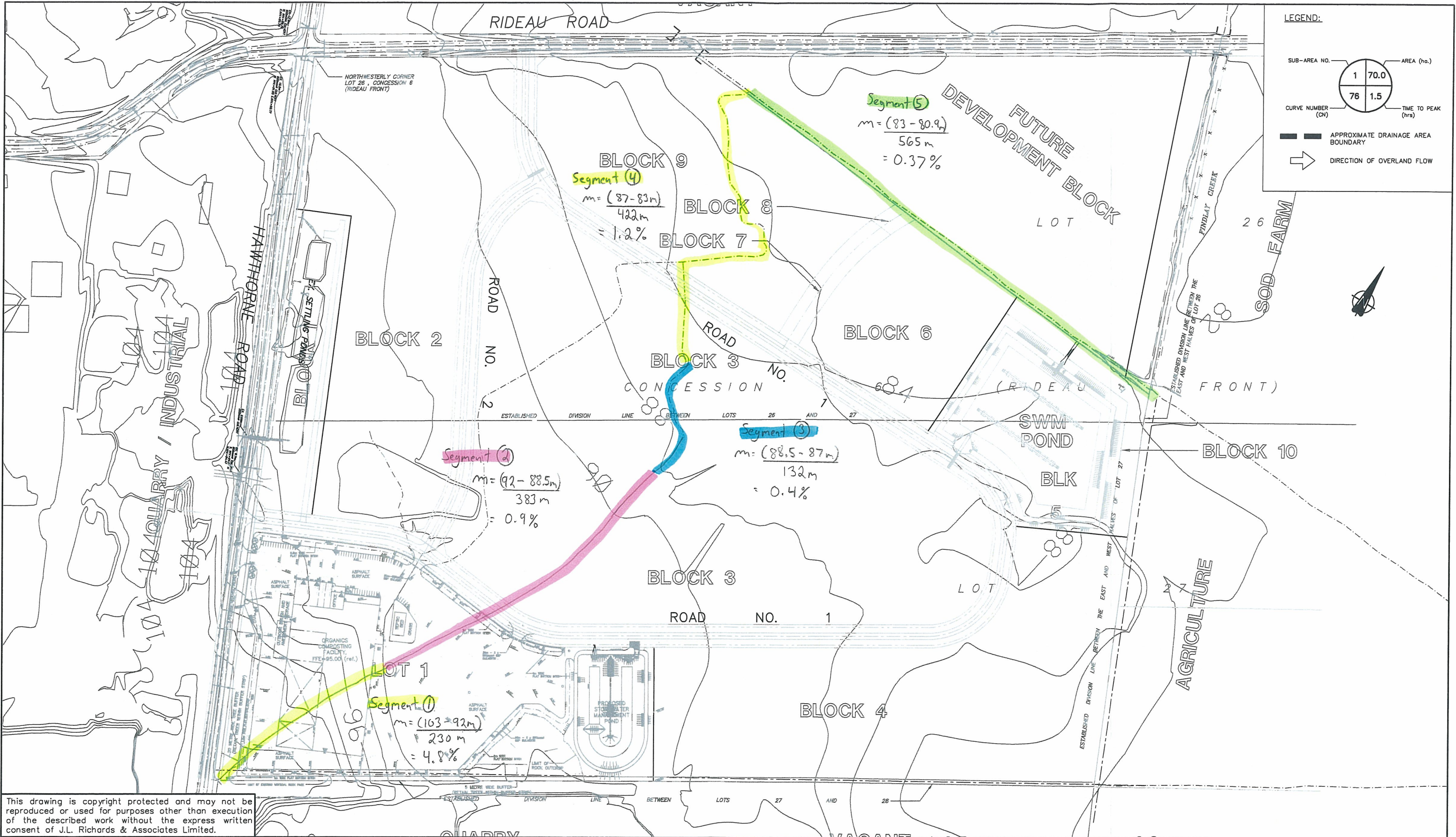
Grass = 0% Impervious

$$\% \text{ Imp.} = 10\% \times 1 + 35\% \times 1 + 35\% \times 0.7 + 20\% \times 0$$

$$= 70\%$$







V:\20983.DU\20983 C FIGURE 2A.dwg

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JOB NO. 20983

PROJECT Hawthorne Industrial Park

Time of Concentration - Pre-development

BY MB DATE Jan 22/09

Segment ①

$$\text{slope} = \frac{(103 - 92) \text{ m}}{230 \text{ m}}$$

$$= 4.8\%$$

Uplands Method Curve B - Woodland

$$\text{Velocity} = 0.32 \text{ m/s}$$

$$\text{Time} = \frac{230 \text{ m}}{0.32 \text{ m/s}}$$

$$= 719 \text{ sec}$$

Segment ②

$$\text{slope} = \frac{(92 - 88.5) \text{ m}}{383 \text{ m}}$$

$$= 0.9\%$$

Uplands Method Curve C - Pasture

$$\text{Velocity} = 0.21 \text{ m/s}$$

$$\text{Time} = \frac{383 \text{ m}}{0.21 \text{ m/s}}$$

$$= 1824 \text{ sec}$$



JOB NO. 20983

PROJECT Hawthorne Industrial Park

Time of Concentration - Pre-development

BY MB DATE Jan 22/09

Segment (3)

$$\text{slope} = \frac{(88.5 - 87) \text{ m}}{132 \text{ m}}$$

$$= 0.4 \%$$

Uplands Method Curve A - Forest (heavy litter)

$$\text{Velocity} = 0.05 \text{ m/s}$$

$$\text{Time} = \frac{132 \text{ m}}{0.05 \text{ m/s}}$$

$$= 2640 \text{ sec.}$$

Segment (4)

$$\text{slope} = \frac{(87 - 83) \text{ m}}{422 \text{ m}}$$

$$= 1.2 \%$$

Uplands Method Curve F - Grassed waterway

$$\text{Velocity} = 0.47 \text{ m/s}$$

$$\text{Time} = \frac{422 \text{ m}}{0.47 \text{ m/s}}$$

$$= 898 \text{ sec}$$



JOB NO. 20983

PROJECT Hawthorne Industrial Park

Time of Concentration - Pre-Development

BY MB DATE Jan 22/09

Segment (5)

$$\text{slope} = \frac{(83 - 80.9) \text{ m}}{565 \text{ m}}$$

$$= 0.37\%$$

Uplands Method Curve F - Grassed waterway

$$\text{Velocity} = 0.28 \text{ m/s}$$

$$\text{Time} = \frac{565 \text{ m}}{0.28 \text{ m/s}}$$

$$= 2018 \text{ sec}$$

$$\begin{aligned} \text{Total Time} &= \textcircled{1} + \textcircled{2} + \textcircled{3} + \textcircled{4} + \textcircled{5} \\ &= 719 + 1824 + 2640 + 898 + 2018 \\ &= 8099 \text{ sec} \end{aligned}$$

$$\text{Time to Peak} = \frac{2}{3} \times 8099 \text{ sec}$$

$$= 5399 \text{ sec}$$

$$= 90 \text{ min}$$



## **APPENDIX 'E'**

### **SWMHYMO INPUT AND OUTPUT FILES (Pre - and Uncontrolled Post-Development Conditions)**

```

00001> 2 Metric units
00002> *****
00003> * Project Name : Hawthorne Industrial Park Project Number: [20983] *
00004> * Date : April, 2009 *
00005> * Rev: 1 *
00006> * Developed by : Mark Buchanan, E.I.T. *
00007> * Reviewed by : Guy Forget, P.Eng. *
00008> * Company : J.L. Richards & Associates Limited *
00009> * License # : 4418403 *
00010> *****
00011> *
00012> *
00013> *****
00014> * FILENAME: V:\20983.DU\ENG\SWM\HYMO\20983PST.DAT *
00015> * FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00016> * OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00017> *****
00018> *
00019> *****
00020> * SWM HYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE *
00021> * PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00022> *****
00023> *
00024> *****
00025> * HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00026> * FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00027> *****
00028> *
00029> *****
00030> * POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00031> *****
00032> *
00033> *****
00034> * CALCULATION OF 4 HR 25 MM STORM EVENT *
00035> *****
00036> *
00037> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00038> * [ ] <- storm filename, one per line for NSTORM time
00039> READ STORM STORM_FILENAME=[ "4HR25-15 STM" ]
00040> *
00041> DEFAULT VALUES ICASEDef=[1], read and print values
00042> * DEFVAL_FILENAME=[V:\22973.DU\ENG\SWM\HYMO\ORGA.VAL]
00043> *
00044> *
00045> *****
00046> * ORGAWORLD FILE *
00047> *****
00048> *
00049> * SUB-AREA No.1 *
00050> *
00051> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00052> * XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00053> * SCS curve number CN=[81],
00054> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00055> * LGP=[20] (m), MNP=[ 0.25 ], SCP=[0.0] (mi)
00056> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00057> * LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00058> * RAINFALL=[ , , , ] (mm/hr), END=-1
00059> *
00060> *
00061> * SUB-AREA No.2 *
00062> *
00063> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00064> * XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00065> * SCS curve number CN=[81],
00066> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00067> * LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00068> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00069> * LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00070> * RAINFALL=[ , , , ] (mm/hr), END=-1
00071> *
00072> *
00073> * SUB-AREA No.3 *
00074> *
00075> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[ 1.4 ] (ha),
00076> * XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00077> * SCS curve number CN=[81],
00078> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00079> * LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00080> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00081> * LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
00082> * RAINFALL=[ , , , ] (mm/hr), END=-1
00083> *
00084> * ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00085> *
00086> * ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00087> *
00088> *
00089> * SUB-AREA No.4 *
00090> *
00091> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00092> * XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00093> * SCS curve number CN=[81],
00094> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00095> * LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00096> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00097> * LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00098> * RAINFALL=[ , , , ] (mm/hr), END=-1
00099> *
00100> *
00101> * SUB-AREA No.5 *
00102> *
00103> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[ 2.66 ] (ha),
00104> * XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00105> * SCS curve number CN=[81],
00106> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00107> * LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (mi)
00108> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00109> * LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00110> * RAINFALL=[ , , , ] (mm/hr), END=-1
00111> *
00112> * ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00113> *
00114> * ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00115> *
00116> *
00117> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00118> * RDT=[1.0] (min),
00119> * TABLE of OUTFLOW-STORAGE values
00120> * (cms) - (ha-m)
00121> * [ 0.000, 0.0000 ]
00122> * [ 0.008, 0.0656 ]
00123> * [ 0.017, 0.1311 ]
00124> * [ 0.093, 0.2831 ]
00125> * [ 0.233, 0.3971 ]
00126> * [ 0.337, 0.4731 ]
00127> * [ 0.465, 0.5491 ]
00128> * [ 0.531, 0.5871 ]
00129> * [ 0.599, 0.6251 ]
00130> * [ 0.654, 0.6631 ]
00131> * [ 0.797, 0.7391 ]
00132> * [ 0.950, 0.8274 ]
00133> * [ 1.304, 0.9157 ]
00134> * [ 1.800, 1.0040 ]
00135> * [ 2.577, 1.0923 ]

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00136> [ -1, -1 ] (max twenty pts)
00137> *****
00138> * Remaining Hawthorne Industrial Park *
00139> *****
00140> *
00141> *
00142> * SUB-AREA No.1 *
00143> *
00144> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00145> * XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00146> * SCS curve number CN=[81],
00147> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00148> * LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
00149> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00150> * LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00151> * RAINFALL=[ , , , ] (mm/hr), END=-1
00152> *
00153> * ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00154> *
00155> *
00156> * SUB-AREA No.2 *
00157> *
00158> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00159> * XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00160> * SCS curve number CN=[81],
00161> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00162> * LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
00163> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00164> * LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00165> * RAINFALL=[ , , , ] (mm/hr), END=-1
00166> *
00167> *
00168> * SUB-AREA No.3 *
00169> *
00170> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00171> * XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00172> * SCS curve number CN=[81],
00173> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00174> * LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
00175> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00176> * LGI=[500] (m), MNI=[0.03], SCI=[0.0] (min)
00177> * RAINFALL=[ , , , ] (mm/hr), END=-1
00178> *
00179> * ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00180> *
00181> *
00182> * SUB-AREA No.4 *
00183> *
00184> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00185> * DWF=[ 0 ] (cms), CNC=[ 85 ], TP=[0.17] hrs,
00186> * RAINFALL=[ , , , ] (mm/hr), END=-1
00187> *
00188> *
00189> *
00190> * ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00191> *
00192> *
00193> * SUB-AREA No.5 *
00194> *
00195> DESIGN NASHYD ID = [10], NHYD=["A2"], DT=[2.5] min, AREA=[6.8] (ha),
00196> * DWF=[0] (cms), CNC=[76], TP=[0.37] hrs,
00197> * RAINFALL=[ , , , ] (mm/hr), END=-1
00198> *
00199> *
00200> * SUB-AREA No.4 *
00201> *
00202> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[5.3] (ha),
00203> * DWF=[0] (cms), CNC=[76], TP=[0.804] hrs,
00204> * RAINFALL=[ , , , ] (mm/hr), END=-1
00205> *
00206> * ADD HYD IDsum=[2], NHYD=["0020"], IDs to add=[7+10+1]
00207> *
00208> *
00209> *****
00210> * CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00211> *****
00212> *
00213> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00214> * [ ] <- storm filename, one per line for NSTORM time
00215> *
00216> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00217> * ICASEDef=[1],
00218> * A=[732.951], B=[6.199], and C=[0.810],
00219> *
00220> DEFAULT VALUES ICASEDef=[1], read and print values
00221> * DEFVAL_FILENAME=[V:\22973.DU\ENG\SWM\HYMO\ORGA.VAL]
00222> *
00223> *
00224> *****
00225> * ORGAWORLD FILE *
00226> *****
00227> *
00228> * SUB-AREA No.1 *
00229> *
00230> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00231> * XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00232> * SCS curve number CN=[81],
00233> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00234> * LGP=[20] (m), MNP=[ 0.25 ], SCP=[0.0] (mi)
00235> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00236> * LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00237> * RAINFALL=[ , , , ] (mm/hr), END=-1
00238> *
00239> *
00240> * SUB-AREA No.2 *
00241> *
00242> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00243> * XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00244> * SCS curve number CN=[81],
00245> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00246> * LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00247> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00248> * LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00249> * RAINFALL=[ , , , ] (mm/hr), END=-1
00250> *
00251> *
00252> * SUB-AREA No.3 *
00253> *
00254> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00255> * XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00256> * SCS curve number CN=[81],
00257> * Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00258> * LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00259> * Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[ 0.51 ] (%),
00260> * LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
00261> * RAINFALL=[ , , , ] (mm/hr), END=-1
00262> *
00263> * ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00264> *
00265> * ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00266> *
00267> *
00268> * SUB-AREA No.4 *
00269> *
00270> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),

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00271> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00272> SCS curve number CN=[81],
00273> Pervious surfaces: IAPER=[4.67](mm), SLPP=[0.7](%),
00274> LGP=[40](m), MNP=[0.25], SCP=[0.0](min)
00275> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.93](%),
00276> LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
00277> RAINFALL=[ , , , ](mm/hr), END=-1
00278> *
00279> * SUB-AREA No.5
00280>
00281> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
00282> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00283> SCS curve number CN=[81],
00284> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00285> LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min)
00286> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.61](%),
00287> LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
00288> RAINFALL=[ , , , ](mm/hr), END=-1
00289> *
00290> *
00291> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00292> *
00293> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00294> *
00295>
00296> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00297> RDT=[1.0](min),
00298> TABLE of ( OUTFLOW-STORAGE ) values
00299> (cms) - (ha-m)
00300> [ 0.000, 0.0000]
00301> [ 0.008, 0.0656]
00302> [ 0.017, 0.1311]
00303> [ 0.093, 0.2831]
00304> [ 0.233, 0.3971]
00305> [ 0.337, 0.4731]
00306> [ 0.465, 0.5491]
00307> [ 0.531, 0.5871]
00308> [ 0.593, 0.6251]
00309> [ 0.654, 0.6631]
00310> [ 0.797, 0.7391]
00311> [ 0.950, 0.8274]
00312> [ 1.304, 0.9157]
00313> [ 1.880, 1.0040]
00314> [ 2.577, 1.0923]
00315> [ -1, -1 ] (max twenty pts)
00316>
00317> *****
00318> * Remaining Hawthorne Industrial Park *
00319> *****
00320> *
00321> * SUB-AREA No.1
00322>
00323> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5](min), AREA=[19.9](ha),
00324> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00325> SCS curve number CN=[81],
00326> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00327> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
00328> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.6](%),
00329> LGI=[580](m), MNI=[0.03], SCI=[0.0]
00330> RAINFALL=[ , , , ](mm/hr), END=-1
00331> *
00332> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00333> *
00334> *
00335> * SUB-AREA No.2
00336>
00337> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5](min), AREA=[17](ha),
00338> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00339> SCS curve number CN=[81],
00340> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00341> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
00342> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.65](%),
00343> LGI=[450](m), MNI=[0.03], SCI=[0.0]
00344> RAINFALL=[ , , , ](mm/hr), END=-1
00345> *
00346> *
00347> * SUB-AREA No.3
00348>
00349> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5](min), AREA=[18.1](ha),
00350> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00351> SCS curve number CN=[81],
00352> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00353> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
00354> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.5](%),
00355> LGI=[600](m), MNI=[0.03], SCI=[0.0]
00356> RAINFALL=[ , , , ](mm/hr), END=-1
00357> *
00358> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00359> *
00360> *
00361> * SUB-AREA No.4
00362>
00363> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5]min, AREA=[4.0](ha),
00364> DWF=[0](cms), CN/C=[ 85 ], TP=[0.17]hrs,
00365> RAINFALL=[ , , , ](mm/hr), END=-1
00366> *
00367> *
00368>
00369> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00370> *
00371> *
00372> * SUB-AREA No. 5
00373>
00374> DESIGN NASHYD ID = [10], NHYD=["A2"], DT=[2.5]min, AREA=[6.8](ha),
00375> DWF=[0](cms), CN/C=[76], TP=[0.37]hrs,
00376> RAINFALL=[ , , , ](mm/hr), END=-1
00377> *
00378> *
00379> * SUB-AREA No.4
00380>
00381> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5]min, AREA=[5.3](ha),
00382> DWF=[0](cms), CN/C=[76], TP=[0.804]hrs,
00383> RAINFALL=[ , , , ](mm/hr), END=-1
00384> *
00385> ADD HYD IDsum=[2], NHYD=["0020"], IDs to add=[7+10+1]
00386> *
00387> *
00388> *****
00389> *****
00390> * CALCULATION OF 3HR - 1:5 YEAR STORM EVENT *
00391> *****
00392>
00393> START TZERO=[0.0], MZERO=[-2], NSTORM=[0], NRUN=[0]
00394> * [ ] <- storm filename, one per line for NSTORM time
00395> *
00396> CHICAGO STORM IUNITS=[2], TD=[3.0](hrs), TPRAT=[0.333], CSPT=[10.0](min)
00397> ICASEC=[1],
00398> A=[999.071], B=[6.053], and C=[0.814],
00399> *
00400> DEFAULT VALUES ICASEDef=[1], read and print values
00401> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHGYMO\ORGA.VAL]
00402> *
00403> *****
00404> *****
00405> * ORGAWORLD FILE *

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00406> *****
00407>
00408> * SUB-AREA No.1
00409>
00410> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5](min), AREA=[ 2.07 ](ha),
00411> XIMP=[0.84], TIMP=[0.84], DWF=[0.0](cms), LOSS=[2],
00412> SCS curve number CN=[81],
00413> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
00414> LGP=[20](m), MNP=[0.25], SCP=[0.0](min)
00415> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.52](%),
00416> LGI=[204.72](m), MNI=[0.03], SCI=[0.0]
00417> RAINFALL=[ , , , ](mm/hr), END=-1
00418> *
00419> *
00420> * SUB-AREA No.2
00421>
00422> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[ 1.54 ](ha),
00423> XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
00424> SCS curve number CN=[81],
00425> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
00426> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
00427> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.50](%),
00428> LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
00429> RAINFALL=[ , , , ](mm/hr), END=-1
00430> *
00431> *
00432> * SUB-AREA No.3
00433>
00434> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
00435> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00436> SCS curve number CN=[81],
00437> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
00438> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
00439> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.51](%),
00440> LGI=[ 225.63 ](m), MNI=[0.03], SCI=[0.0]
00441> RAINFALL=[ , , , ](mm/hr), END=-1
00442> *
00443> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00444> *
00445> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00446> *
00447> *
00448> * SUB-AREA No.4
00449>
00450> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
00451> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00452> SCS curve number CN=[81],
00453> Pervious surfaces: IAPER=[4.67](mm), SLPP=[0.7](%),
00454> LGP=[40](m), MNP=[0.25], SCP=[0.0](min)
00455> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.93](%),
00456> LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
00457> RAINFALL=[ , , , ](mm/hr), END=-1
00458> *
00459> *
00460> * SUB-AREA No.5
00461>
00462> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
00463> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00464> SCS curve number CN=[81],
00465> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00466> LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min)
00467> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.61](%),
00468> LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
00469> RAINFALL=[ , , , ](mm/hr), END=-1
00470> *
00471> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00472> *
00473> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00474> *
00475>
00476> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00477> RDT=[1.0](min),
00478> TABLE of ( OUTFLOW-STORAGE ) values
00479> (cms) - (ha-m)
00480> [ 0.000, 0.0000]
00481> [ 0.008, 0.0656]
00482> [ 0.017, 0.1311]
00483> [ 0.093, 0.2831]
00484> [ 0.233, 0.3971]
00485> [ 0.337, 0.4731]
00486> [ 0.465, 0.5491]
00487> [ 0.531, 0.5871]
00488> [ 0.593, 0.6251]
00489> [ 0.654, 0.6631]
00490> [ 0.797, 0.7391]
00491> [ 0.950, 0.8274]
00492> [ 1.304, 0.9157]
00493> [ 1.880, 1.0040]
00494> [ 2.577, 1.0923]
00495> [ -1, -1 ] (max twenty pts)
00496>
00497> *****
00498> * Remaining Hawthorne Industrial Park *
00499> *****
00500> *
00501> * SUB-AREA No.1
00502>
00503> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5](min), AREA=[19.9](ha),
00504> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00505> SCS curve number CN=[81],
00506> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00507> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
00508> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.6](%),
00509> LGI=[580](m), MNI=[0.03], SCI=[0.0]
00510> RAINFALL=[ , , , ](mm/hr), END=-1
00511> *
00512> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00513> *
00514> *
00515> * SUB-AREA No.2
00516>
00517> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5](min), AREA=[17](ha),
00518> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00519> SCS curve number CN=[81],
00520> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00521> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
00522> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.65](%),
00523> LGI=[450](m), MNI=[0.03], SCI=[0.0]
00524> RAINFALL=[ , , , ](mm/hr), END=-1
00525> *
00526> *
00527> * SUB-AREA No.3
00528>
00529> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5](min), AREA=[18.1](ha),
00530> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00531> SCS curve number CN=[81],
00532> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
00533> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
00534> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.5](%),
00535> LGI=[600](m), MNI=[0.03], SCI=[0.0]
00536> RAINFALL=[ , , , ](mm/hr), END=-1
00537> *
00538> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00539> *
00540> *

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Page 2

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00811> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[0.7] (%),
00812> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00813> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
00814> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00815> RAINFALL=[ , , , ] (mm/hr), END=-1
00816> *%-----
00817> * SUB-AREA No.5
00818>
00819>
00820> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00821> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00822> SCS curve number CN=[81],
00823> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00824> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
00825> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00826> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00827> RAINFALL=[ , , , ] (mm/hr), END=-1
00828> *%-----
00829> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00830> *%-----
00831> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00832> *%-----
00833>
00834> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00835> RDT=[1.0] (min),
00836> TABLE of ( OUTFLOW-STORAGE ) values
00837> (cms) - (ha-m)
00838> [ 0.000, 0.0000]
00839> [ 0.008, 0.0656]
00840> [ 0.017, 0.1311]
00841> [ 0.093, 0.2831]
00842> [ 0.233, 0.3971]
00843> [ 0.337, 0.4731]
00844> [ 0.465, 0.5491]
00845> [ 0.531, 0.5871]
00846> [ 0.593, 0.6251]
00847> [ 0.654, 0.6631]
00848> [ 0.797, 0.7391]
00849> [ 0.950, 0.8274]
00850> [ 1.304, 0.9157]
00851> [ 1.880, 1.0040]
00852> [ 2.577, 1.0923]
00853> [ -1, -1 ] (max twenty pts)
00854>
00855> *****
00856> * Remaining Hawthorne Industrial Park *
00857> *****
00858>
00859> * SUB-AREA No.1
00860>
00861> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00862> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00863> SCS curve number CN=[81],
00864> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00865> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
00866> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00867> LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00868> RAINFALL=[ , , , ] (mm/hr), END=-1
00869> *%-----
00870> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00871> *%-----
00872>
00873> * SUB-AREA No.2
00874>
00875> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00876> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00877> SCS curve number CN=[81],
00878> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00879> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
00880> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00881> LGI=[450] (m), MNI=[0.03], SCI=[0.0]
00882> RAINFALL=[ , , , ] (mm/hr), END=-1
00883> *%-----
00884>
00885> * SUB-AREA No.3
00886>
00887> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00888> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00889> SCS curve number CN=[81],
00890> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00891> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
00892> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
00893> LGI=[600] (m), MNI=[0.03], SCI=[0.0]
00894> RAINFALL=[ , , , ] (mm/hr), END=-1
00895> *%-----
00896> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00897> *%-----
00898>
00899> * SUB-AREA No.4
00900>
00901> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00902> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00903> RAINFALL=[ , , , ] (mm/hr), END=-1
00904> *%-----
00905>
00906>
00907> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00908> *%-----
00909>
00910> * SUB-AREA No. 5
00911>
00912> DESIGN NASHYD ID = [10], NHYD=["A2"], DT=[2.5] min, AREA=[6.8] (ha),
00913> DWF=[0] (cms), CNC=[76], TP=[0.37] hrs,
00914> RAINFALL=[ , , , ] (mm/hr), END=-1
00915> *%-----
00916>
00917> * SUB-AREA No 4
00918>
00919> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[5.3] (ha),
00920> DWF=[0] (cms), CNC=[76], TP=[0.80] hrs,
00921> RAINFALL=[ , , , ] (mm/hr), END=-1
00922> *%-----
00923> ADD HYD IDsum=[2], NHYD=["0020"], IDs to add=[7+10+1]
00924> *%-----
00925>
00926> *****
00927> CALCULATION OF 3HR - 1:50 YEAR STORM EVENT *
00928> *****
00929>
00930>
00931> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00932> *%-----
00933> [ ] <- storm filename, one per line for NSTORM time
00934> CHICAGO STORM IUNIT=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00935> ICASRCS=[1],
00936> A=[1569.580], B=[6.014], and C=[0.820],
00937> *%-----
00938> DEFAULT VALUES ICASEDEF=[1], read and print values
00939> DEVAL_FILENAME=[V:\22973.DU\ENG\SWM\HYMO\ORGA.VAL]
00940> *%-----
00941>
00942> *****
00943> * ORGAWORLD FILE *
00944> *****
00945>
00946> * SUB-AREA No.1
00947>
00948> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00949> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00950> SCS curve number CN=[81],
00951> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00952> LGP=[20] (m), MNP=[ 0.25 ], SCP=[0.0] (min)
00953> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
00954> LGI=[204.72] (m), MNI=[0.03 ], SCI=[0.0]
00955> RAINFALL=[ , , , ] (mm/hr), END=-1
00956> *%-----
00957>
00958> * SUB-AREA No.2
00959>
00960> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00961> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00962> SCS curve number CN=[81],
00963> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00964> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min)
00965> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
00966> LGI=[244.34] (m), MNI=[0.03 ], SCI=[0.0]
00967> RAINFALL=[ , , , ] (mm/hr), END=-1
00968> *%-----
00969>
00970> * SUB-AREA No.3
00971>
00972> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00973> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00974> SCS curve number CN=[81],
00975> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00976> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min)
00977> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
00978> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
00979> RAINFALL=[ , , , ] (mm/hr), END=-1
00980> *%-----
00981> ADD HYD IDsum=[ 4 ], NHYD=["040"], IDs to add=[1+2]
00982> *%-----
00983> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00984> *%-----
00985>
00986> * SUB-AREA No.4
00987>
00988> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00989> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00990> SCS curve number CN=[81],
00991> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[0.7] (%),
00992> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00993> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
00994> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00995> RAINFALL=[ , , , ] (mm/hr), END=-1
00996> *%-----
00997>
00998> * SUB-AREA No.5
00999>
01000> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
01001> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01002> SCS curve number CN=[81],
01003> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01004> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
01005> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01006> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
01007> RAINFALL=[ , , , ] (mm/hr), END=-1
01008> *%-----
01009> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
01010> *%-----
01011> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
01012> *%-----
01013>
01014> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
01015> RDT=[1.0] (min),
01016> TABLE of ( OUTFLOW-STORAGE ) values
01017> (cms) - (ha-m)
01018> [ 0.000, 0.0000]
01019> [ 0.008, 0.0656]
01020> [ 0.017, 0.1311]
01021> [ 0.093, 0.2831]
01022> [ 0.233, 0.3971]
01023> [ 0.337, 0.4731]
01024> [ 0.465, 0.5491]
01025> [ 0.531, 0.5871]
01026> [ 0.593, 0.6251]
01027> [ 0.654, 0.6631]
01028> [ 0.797, 0.7391]
01029> [ 0.950, 0.8274]
01030> [ 1.304, 0.9157]
01031> [ 1.880, 1.0040]
01032> [ 2.577, 1.0923]
01033> [ -1, -1 ] (max twenty pts)
01034>
01035> *****
01036> * Remaining Hawthorne Industrial Park *
01037> *****
01038>
01039> * SUB-AREA No.1
01040>
01041> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01042> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01043> SCS curve number CN=[81],
01044> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01045> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01046> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01047> LGI=[580] (m), MNI=[0.03], SCI=[0.0]
01048> RAINFALL=[ , , , ] (mm/hr), END=-1
01049> *%-----
01050> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
01051> *%-----
01052>
01053> * SUB-AREA No.2
01054>
01055> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01056> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01057> SCS curve number CN=[81],
01058> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01059> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01060> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01061> LGI=[450] (m), MNI=[0.03], SCI=[0.0]
01062> RAINFALL=[ , , , ] (mm/hr), END=-1
01063> *%-----
01064>
01065> * SUB-AREA No.3
01066>
01067> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
01068> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01069> SCS curve number CN=[81],
01070> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01071> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01072> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
01073> LGI=[600] (m), MNI=[0.03], SCI=[0.0]
01074> RAINFALL=[ , , , ] (mm/hr), END=-1
01075> *%-----
01076> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01077> *%-----
01078>
01079> * SUB-AREA No.4
01080>

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01081> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5]min, AREA=[4.0] (ha),
01082> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17]hrs,
01083> RAINFALL=[ , , , ] (mm/hr), END=-1
01084> *%-----
01085>
01086>
01087> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
01088> *%-----
01089>
01090> * SUB-AREA No. 5
01091>
01092> DESIGN NASHYD ID = [10], NHYD=["A2"], DT=[2.5]min, AREA=[6.8] (ha),
01093> DWF=[0] (cms), CNC=[76], TP=[0.37]hrs,
01094> RAINFALL=[ , , , ] (mm/hr), END=-1
01095> *%-----
01096>
01097> * SUB-AREA No. 4
01098>
01099> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5]min, AREA=[5.3] (ha),
01100> DWF=[0] (cms), CNC=[76], TP=[0.804]hrs,
01101> RAINFALL=[ , , , ] (mm/hr), END=-1
01102> *%-----
01103> ADD HYD IDsum=[2], NHYD=["0020"], IDs to add=[7+10+1]
01104> *%-----
01105>
01106> *****
01107> * CALCULATION OF SHR - 1:100 YEAR STORM EVENT *
01108> *****
01109>
01110> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01111> * [ ] <- storm filename, one per line for NSTORM time
01112> *%-----
01113> CHICAGO STORM UNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSPT=[10.0] (min)
01114> ICASE=[1],
01115> A=[1735.688], B=[6.014], and C=[0.820].
01116> *%-----
01117> DEFAULT VALUES ICASEDef=[1], read and print values
01118> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHMYMO\ORGA.VAL"]
01119> *%-----
01120>
01121> *****
01122> * ORGAWORLD FILE *
01123> *****
01124>
01125> * SUB-AREA No.1
01126>
01127> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
01128> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
01129> SCS curve number CN=[81],
01130> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
01131> LGP=[20] (m), MNP=[ 0.25 ], SCP=[0.0] (mi)
01132> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01133> LGI=[204.72] (m), MNI=[0.03 ], SCI=[0.0]
01134> RAINFALL=[ , , , ] (mm/hr), END=-1
01135> *%-----
01136>
01137> * SUB-AREA No.2
01138>
01139> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
01140> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
01141> SCS curve number CN=[81],
01142> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
01143> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01144> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
01145> LGI=[244.34] (m), MNI=[0.03 ], SCI=[0.0]
01146> RAINFALL=[ , , , ] (mm/hr), END=-1
01147> *%-----
01148>
01149> * SUB-AREA No.3
01150>
01151> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
01152> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01153> SCS curve number CN=[81],
01154> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
01155> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01156> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
01157> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
01158> RAINFALL=[ , , , ] (mm/hr), END=-1
01159> *%-----
01160> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
01161> *%-----
01162> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
01163> *%-----
01164>
01165> * SUB-AREA No.4
01166>
01167> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
01168> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01169> SCS curve number CN=[81],
01170> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[0.7] (%),
01171> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
01172> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
01173> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
01174> RAINFALL=[ , , , ] (mm/hr), END=-1
01175> *%-----
01176>
01177> * SUB-AREA No.5
01178>
01179> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
01180> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01181> SCS curve number CN=[81],
01182> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01183> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01184> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01185> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
01186> RAINFALL=[ , , , ] (mm/hr), END=-1
01187> *%-----
01188> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
01189> *%-----
01190> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
01191> *%-----
01192>
01193> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
01194> RDT=[1.0] (min),
01195> TABLE of ( OUTFLOW-STORAGE ) values
01196> (cms) - (ha-m)
01197> [ 0.000, 0.0000 ]
01198> [ 0.008, 0.0656 ]
01199> [ 0.017, 0.1311 ]
01200> [ 0.025, 0.2831 ]
01201> [ 0.233, 0.3971 ]
01202> [ 0.337, 0.4731 ]
01203> [ 0.465, 0.5491 ]
01204> [ 0.531, 0.5871 ]
01205> [ 0.593, 0.6251 ]
01206> [ 0.654, 0.6631 ]
01207> [ 0.797, 0.7391 ]
01208> [ 0.950, 0.8274 ]
01209> [ 1.304, 0.9157 ]
01210> [ 1.880, 1.0040 ]
01211> [ 2.577, 1.0923 ]
01212> [ -1, -1 ] (max twenty pts)
01213>
01214> *****
01215> * Remaining Hawthorne Industrial Park *
01216> *****
01217>
01218> * SUB-AREA No.1
01219>
01220> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01221> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01222> SCS curve number CN=[81],
01223> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01224> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
01225> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01226> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
01227> RAINFALL=[ , , , ] (mm/hr), END=-1
01228> *%-----
01229> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
01230> *%-----
01231>
01232> * SUB-AREA No.2
01233>
01234> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01235> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01236> SCS curve number CN=[81],
01237> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01238> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
01239> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
01240> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
01241> RAINFALL=[ , , , ] (mm/hr), END=-1
01242> *%-----
01243>
01244> * SUB-AREA No.3
01245>
01246> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
01247> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01248> SCS curve number CN=[81],
01249> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01250> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
01251> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
01252> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
01253> RAINFALL=[ , , , ] (mm/hr), END=-1
01254> *%-----
01255> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01256> *%-----
01257>
01258> * SUB-AREA No.4
01259>
01260> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5]min, AREA=[4.0] (ha),
01261> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17]hrs,
01262> RAINFALL=[ , , , ] (mm/hr), END=-1
01263> *%-----
01264>
01265> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
01266> *%-----
01267>
01268> * SUB-AREA No. 5
01269>
01270> DESIGN NASHYD ID = [10], NHYD=["A2"], DT=[2.5]min, AREA=[6.8] (ha),
01271> DWF=[0] (cms), CNC=[76], TP=[0.37]hrs,
01272> RAINFALL=[ , , , ] (mm/hr), END=-1
01273> *%-----
01274>
01275> * SUB-AREA No. 4
01276>
01277> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5]min, AREA=[5.3] (ha),
01278> DWF=[0] (cms), CNC=[76], TP=[0.804]hrs,
01279> RAINFALL=[ , , , ] (mm/hr), END=-1
01280> *%-----
01281>
01282> ADD HYD IDsum=[2], NHYD=["0020"], IDs to add=[7+10+1]
01283> *%-----
01284>
01285> FINISH
01286>
01287> *****Rough Pond 1.87 ha x 1.4 m deep*****
01288> TABLE of ( OUTFLOW-STORAGE ) values
01289> (cms) - (ha-m)
01290> [ 0.0, 0.0 ]
01291> [ 0.10, 0.374 ]
01292> [ 0.25, 0.748 ]
01293> [ 0.50, 1.122 ]
01294> [ 0.85, 1.496 ]
01295> [ 1.20, 1.870 ]
01296> [ 1.30, 2.244 ]
01297> [ 1.50, 2.618 ]
01298> [ -1, -1 ]
01299>
01300>
01301> *****Rough Pond 150x150 x 1.4 m deep*****
01302> (cms) - (ha-m)
01303> [ 0.0, 0.0 ]
01304> [ 0.16, 0.45 ]
01305> [ 0.31, 0.900 ]
01306> [ 0.60, 1.350 ]
01307> [ 0.95, 1.800 ]
01308> [ 1.40, 2.25 ]
01309> [ 1.45, 2.700 ]
01310> [ 1.50, 3.150 ]
01311> [ -1, -1 ] (max twenty pts)
01312>
01313>
01314>
01315>
01316>
01317>
01318>
01319>
01320>
01321>
01322>
01323>
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01347>
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01349>
01350>

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00001>
00002>
00003> SSSSS W W M M H H Y Y M M O O 999 999
00004> S W W M M M H H Y Y M M O O 9 9 9
00005> SSSSS W W M M M H H H Y Y M M O O ## 9 9 9 Ver. 4.02
00006> S W W M M M H H Y Y M M O O 9999 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9
00008>
00009> Stormwater Management Hydrologic Model 999 999
00010>
00011>
00012>
00013> ***** SWHYMO-99 Ver/4.02 *****
00014> ***** A single event and continuous hydrologic simulation model *****
00015> ***** based on the principles of HYMO and its successors *****
00016> ***** OTTHYMO-83 and OTTHYMO-89. *****
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 727-5199 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhymo@fsa.com *****
00021>
00022>
00023>
00024> ***** Licensed user: J. L. Richards & Associates Limited *****
00025> ***** Ottawa SERIAL#:4418403 *****
00026>
00027>
00028>
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033>
00034>
00035>
00036> ***** DETAILED OUTPUT *****
00037>
00038> * DATE: 2009-04-21 TIME: 10:30:14 RUN COUNTER: 000173
00039>
00040> * Input filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\PSTPH1.dat
00041> * Output filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\PSTPH1.out
00042> * Summary filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\PSTPH1.sum
00043> * User comments:
00044> * 1:
00045> * 2:
00046> * 3:
00047>
00048>
00049>
00050> 001:0001-
00051> *
00052> * Project Name: Hawthorne Industrial Park Project Number: [20983] *
00053> * Date: April, 2009 *
00054> * Revised: W/A *
00055> * Developed by: Mark Buchanan, R.I.T. *
00056> * Reviewed by: Guy Forget, P.Eng. *
00057> * Company: J.L. Richards & Associates Limited *
00058> * License #: 4418403 *
00059> *
00060>
00061>
00062> *
00063> * FILENAME: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\20983PST.DAT
00064> * FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00065> * OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00066> *
00067>
00068>
00069> * SWHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE
00070> * PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00071> *
00072>
00073> * HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00074> * FOR DESIGN STORMS OF 12, 5, 10, 25, 50, AND 100 YR *
00075> *
00076>
00077> * POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00078> *
00079>
00080> * CALCULATION OF 4 HR 25 MM STORM EVENT *
00081> *
00082>
00083> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\
00084> | METOUT= 2 (output = METRIC) | Rainfall dir.: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\
00085> | TZERO = .00 hrs on 0
00086> | NRUN = 001
00087> | NSTORM = 0
00088>
00089>
00090> 001:0002-
00091>
00092> | READ STORM | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\4HR25-1
00093> | Ptotal= 25.00 mm | Comments: 4hr-15 min 25 MM STORM EVENT (CHICAGO DI
00094>
00095>
00096>
00097>
00098>
00099>
01000>
01001>
01002>
01003> 001:0003-
01004>
01005> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\ORGA.VAL
01006> | ICASedv = 1 (read and print data)
01007> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
01008> | Horton's infiltration equation parameters:
01009> | [Pc= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01010> | Parameters for Pervious surfaces in STANDHYD:
01011> | [Iaper= 4.67 mm] [LGR=40.00 m] [MNP= .250]
01012> | Parameters for IMPERVIOUS surfaces in STANDHYD:
01013> | [Iimp= 1.57 mm] [CLi= 1.50] [MNI= .035]
01014> | Parameters used in NASHYD:
01015> | [Ia= 4.67 mm] [N= 3.00]
01016>
01017>
01018> 001:0004-
01019>
01020> * ORGAWORLD FILE *
01021> *****
01022> * SUB-AREA No.1
01023>
01024> | CALIB STANDHYD | Area (ha)= 2.07
01025> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01026>
01027>
01028>
01029>
01030>
01031>
01032>
01033>
01034>
01035>

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00136> Storage Coeff. (min)= 10.80 (ii) 29.27 (ii)
00137> Unit Hyd. Tpeak (min)= 10.00 30.00
00138> Unit Hyd. peak (cms)= .11 .04
00139>
00140>
00141>
00142>
00143>
00144>
00145>
00146> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00147> CN* = 81.0 Ia = Dep. Storage (Above)
00148> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00149> THAN THE STORAGE COEFFICIENT.
00150> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00151>
00152>
00153> 001:0005-
00154>
00155> * SUB-AREA No.2
00156>
00157> | CALIB STANDHYD | Area (ha)= 1.54
00158> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
00159>
00160>
00161>
00162>
00163>
00164>
00165>
00166>
00167>
00168>
00169>
00170>
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00175>
00176>
00177>
00178>
00179>
00180>
00181>
00182>
00183>
00184>
00185>
00186> 001:0006-
00187>
00188> * SUB-AREA No.3
00189>
00190> | CALIB STANDHYD | Area (ha)= 1.40
00191> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00192>
00193>
00194>
00195>
00196>
00197>
00198>
00199>
00200>
00201>
00202>
00203>
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00216>
00217>
00218>
00219>
00220>
00221> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00222> | (ha) (cms) (hrs) (mm) (cms)
00223> | ID1 01:010 2.07 .158 1.29 20.51 .000
00224> | +ID2 02:020 1.54 .121 1.33 21.97 .000
00225>
00226> SUM 04:040 3.61 .278 1.33 21.13 .000
00227>
00228> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00229>
00230>
00231> 001:0008-
00232>
00233> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00234> | (ha) (cms) (hrs) (mm) (cms)
00235> | ID1 03:030 1.40 .118 1.33 22.88 .000
00236> | +ID2 04:040 3.61 .278 1.33 21.13 .000
00237>
00238> SUM 05:050 5.01 .396 1.33 21.62 .000
00239>
00240>
00241>
00242>
00243> 001:0009-
00244>
00245> * SUB-AREA No.4
00246>
00247> | CALIB STANDHYD | Area (ha)= .89
00248> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00249>
00250>
00251>
00252>
00253>
00254>
00255>
00256>
00257>
00258>
00259>
00260>
00261>
00262>
00263>
00264>
00265>
00266>
00267>
00268>
00269>
00270>

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00271> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00272> THAN THE STORAGE COEFFICIENT.
00273> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00274>
00275>
00276> 001:0010-----
00277> * SUB-AREA No.5
00278>
00279>
00280> CALIB STANDHYD | Area (ha)= 2.66
00281> 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00282>
00283> IMPERVIOUS PERVIOUS (i)
00284> Surface Area (ha)= 2.58 .08
00285> Dep. Storage (mm)= 1.57 4.67
00286> Average Slope (%)= .61 1.50
00287> Length (m)= 207.25 20.00
00288> Mannings n = .030 .250
00289>
00290> Max.eff.Inten.(mm/hr)= 45.63 5.66
00291> over (min) 10.00 27.50
00292> Storage Coeff. (min)= 10.37 (ii) 26.38 (iii)
00293> Unit Hyd. Tpeak (min)= 10.00 27.50
00294> Unit Hyd. peak (cms)= .11 .04
00295>
00296> PEAK FLOW (cms)= .24 .00 *TOTALS*
00297> TIME TO PEAK (hrs)= 1.29 1.67 1.292
00298> RUNOFF VOLUME (mm)= 23.43 5.17 22.882
00299> TOTAL RAINFALL (mm)= 25.00 25.00 24.999
00300> RUNOFF COEFFICIENT = .94 .21 .915
00301>
00302> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00303> CN* = 81.0 Ia = Dep. Storage (Above)
00304> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00305> THAN THE STORAGE COEFFICIENT.
00306> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00307>
00308>
00309> 001:0011-----
00310>
00311> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00312> | (ha) (cms) (hrs) (mm) (cms)
00313> ID1 05:060 .089 1.25 22.88 .000
00314> ID2 07:070 2.66 .238 1.29 22.88 .000
00315>
00316> SUM 08:080 3.55 .327 1.29 22.88 .000
00317>
00318> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00319>
00320>
00321> 001:0012-----
00322>
00323> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00324> | (ha) (cms) (hrs) (mm) (cms)
00325> ID1 05:050 5.01 .396 1.33 21.62 .000
00326> ID2 08:080 3.55 .327 1.29 22.88 .000
00327>
00328> SUM 09:090 8.56 .716 1.29 22.14 .000
00329>
00330> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00331>
00332>
00333> 001:0013-----
00334>
00335> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00336> | IN:09:(090 ) |
00337> | OUT:10:(POND ) |
00338>
00339> ===== OUTFLOW STORAGE TABLE =====
00340> OUTFLOW STORAGE OUTFLOW STORAGE
00341> (cms) (ha.m.) (cms) (ha.m.)
00342> .000 .0000E+00 .593 .6251E+00
00343> .008 .6560E-01 .654 .6631E+00
00344> .017 .1311E+00 .797 .7391E+00
00345> .093 .2831E+00 .950 .8274E+00
00346> .233 .3971E+00 1.304 .9157E+00
00347> .337 .4731E+00 1.880 .1004E+01
00348> .465 .5491E+00 2.577 .1092E+01
00349> .531 .5871E+00 .000 .0000E+00
00350>
00351> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00352> (ha) (cms) (hrs) (mm)
00353> INFLOW >09:(090 ) 8.56 .716 1.292 22.143
00354> OUTFLOW<10:(POND ) 8.56 .032 3.875 22.141
00355>
00356> PEAK FLOW REDUCTION [Qout/Qin] (%) = 4.470
00357> TIME SHIFT OF PEAK FLOW (min) = 155.00
00358> MAXIMUM STORAGE USED (ha.m.) = .1611E+00
00359>
00360> 001:0014-----
00361> *****
00362> * Remaining Hawthorne Industrial Park
00363> *****
00364> * SUB-AREA No.1
00365>
00366> CALIB STANDHYD | Area (ha)= 19.90
00367> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00368>
00369> IMPERVIOUS PERVIOUS (i)
00370> Surface Area (ha)= 14.13 5.77
00371> Dep. Storage (mm)= 1.57 4.67
00372> Average Slope (%)= .60 1.50
00373> Length (m)= 580.00 100.00
00374> Mannings n = .030 .250
00375>
00376> Max.eff.Inten.(mm/hr)= 34.39 11.90
00377> over (min) 22.50 52.50
00378> Storage Coeff. (min)= 21.64 (ii) 52.88 (ii)
00379> Unit Hyd. Tpeak (min)= 22.50 52.50
00380> Unit Hyd. peak (cms)= .05 .02
00381>
00382> PEAK FLOW (cms)= .60 .11 *TOTALS*
00383> TIME TO PEAK (hrs)= 1.50 2.13 1.542
00384> RUNOFF VOLUME (mm)= 23.43 8.74 16.085
00385> TOTAL RAINFALL (mm)= 25.00 25.00 24.999
00386> RUNOFF COEFFICIENT = .94 .35 .643
00387>
00388> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00389> CN* = 81.0 Ia = Dep. Storage (Above)
00390> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00391> THAN THE STORAGE COEFFICIENT.
00392> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00393>
00394>
00395> 001:0015-----
00396>
00397> | ADD HYD (H1P02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00398> | (ha) (cms) (hrs) (mm) (cms)
00399> ID1 10:POND 9.56 .032 3.88 22.14 .000
00400> ID2 01:H1P01 19.90 .642 1.54 16.08 .000
00401>
00402> SUM 02:H1P02 28.46 .655 1.54 17.91 .000
00403>
00404> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00405>

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00406>
00407> 001:0016-----
00408>
00409> * SUB-AREA No.2
00410>
00411> CALIB STANDHYD | Area (ha)= 17.00
00412> 03:H1P03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00413>
00414> IMPERVIOUS PERVIOUS (i)
00415> Surface Area (ha)= 12.07 4.93
00416> Dep. Storage (mm)= 1.57 4.67
00417> Average Slope (%)= .65 1.50
00418> Length (m)= 450.00 100.00
00419> Mannings n = .030 .250
00420>
00421> Max.eff.Inten.(mm/hr)= 40.81 12.73
00422> over (min) 17.50 47.50
00423> Storage Coeff. (min)= 16.94 (ii) 47.35 (ii)
00424> Unit Hyd. Tpeak (min)= 17.50 47.50
00425> Unit Hyd. peak (cms)= .07 .02
00426>
00427> PEAK FLOW (cms)= .60 .10 *TOTALS*
00428> TIME TO PEAK (hrs)= 1.42 2.00 1.458
00429> RUNOFF VOLUME (mm)= 23.43 8.74 16.085
00430> TOTAL RAINFALL (mm)= 25.00 25.00 24.999
00431> RUNOFF COEFFICIENT = .94 .35 .643
00432>
00433> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00434> CN* = 81.0 Ia = Dep. Storage (Above)
00435> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00436> THAN THE STORAGE COEFFICIENT.
00437> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00438>
00439>
00440> 001:0017-----
00441>
00442> * SUB-AREA No.3
00443>
00444> CALIB STANDHYD | Area (ha)= 18.10
00445> 04:H1P04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00446>
00447> IMPERVIOUS PERVIOUS (i)
00448> Surface Area (ha)= 12.85 5.25
00449> Dep. Storage (mm)= 1.57 4.67
00450> Average Slope (%)= .50 1.50
00451> Length (m)= 600.00 100.00
00452> Mannings n = .030 .250
00453>
00454> Max.eff.Inten.(mm/hr)= 34.39 11.54
00455> over (min) 22.50 55.00
00456> Storage Coeff. (min)= 23.33 (ii) 54.95 (ii)
00457> Unit Hyd. Tpeak (min)= 22.50 55.00
00458> Unit Hyd. peak (cms)= .05 .02
00459>
00460> PEAK FLOW (cms)= .53 .09 *TOTALS*
00461> TIME TO PEAK (hrs)= 1.50 2.17 1.542
00462> RUNOFF VOLUME (mm)= 23.43 8.74 16.085
00463> TOTAL RAINFALL (mm)= 25.00 25.00 24.999
00464> RUNOFF COEFFICIENT = .94 .35 .643
00465>
00466> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00467> CN* = 81.0 Ia = Dep. Storage (Above)
00468> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00469> THAN THE STORAGE COEFFICIENT.
00470> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00471>
00472>
00473> 001:0018-----
00474>
00475> | ADD HYD (H1P05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00476> | (ha) (cms) (hrs) (mm) (cms)
00477> ID1 03:H1P03 17.00 .625 1.46 16.08 .000
00478> ID2 04:H1P04 18.10 .562 1.54 16.08 .000
00479>
00480> SUM 05:H1P05 35.10 1.166 1.46 16.08 .000
00481>
00482> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00483>
00484>
00485> 001:0019-----
00486>
00487> *SUB-AREA No.4
00488>
00489> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
00490> 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
00491> U.H. Tp(hrs)= .170
00492>
00493> Unit Hyd Qpeak (cms)= .899
00494>
00495> PEAK FLOW (cms)= .077 (i)
00496> TIME TO PEAK (hrs)= 1.375
00497> RUNOFF VOLUME (mm)= 6.343
00498> TOTAL RAINFALL (mm)= 24.999
00499> RUNOFF COEFFICIENT = .254
00500>
00501> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00502>
00503>
00504> 001:0020-----
00505>
00506> | ADD HYD (H1P06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00507> | (ha) (cms) (hrs) (mm) (cms)
00508> ID1 02:H1P02 28.46 .655 1.54 17.91 .000
00509> ID2 05:H1P05 35.10 1.166 1.46 16.08 .000
00510> ID3 06:Pond-B 4.00 .077 1.38 6.34 .000
00511>
00512> SUM 07:H1P06 67.56 1.887 1.50 16.28 .000
00513>
00514> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00515>
00516>
00517> 001:0021-----
00518>
00519> * SUB-AREA No. 5
00520>
00521> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00
00522> 10:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
00523> U.H. Tp(hrs)= .370
00524>
00525> Unit Hyd Qpeak (cms)= .702
00526>
00527> PEAK FLOW (cms)= .053 (i)
00528> TIME TO PEAK (hrs)= 1.708
00529> RUNOFF VOLUME (mm)= 4.111
00530> TOTAL RAINFALL (mm)= 24.999
00531> RUNOFF COEFFICIENT = .164
00532>
00533> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00534>
00535>
00536> 001:0022-----
00537>
00538> * SUB-AREA No.4
00539>
00540> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00
00541> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
00542> U.H. Tp(hrs)= .804

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00541> Unit Hyd Qpeak (cms) = .252
00542>
00543>
00544> PEAK FLOW (cms) = .025 (i)
00545> TIME TO PEAK (hrs) = 2.333
00546> RUNOFF VOLUME (mm) = 4.110
00547> TOTAL RAINFALL (mm) = 24.999
00548> RUNOFF COEFFICIENT = .164
00549>
00550> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00551>
00552>
00553> 001:0023-----
00554>
00555> | ADD HYD (0020 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00556> | (ha) (cms) (hrs) (mm) (cms)
00557> | ID1 07:HIP06 67.56 1.887 1.50 16.28 .000
00558> | ID2 10:A2 6.80 .053 1.71 4.11 .000
00559> | ID3 01:A3 5.30 .025 2.33 4.11 .000
00560>
00561> SUM 02:0020 79.66 1.941 1.50 14.43 .000
00562>
00563> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00564>
00565>
00566> 001:0024-----
00567> *****
00568> CALCULATION OF 3HR - 1:2 YEAR STORM EVENT
00569> *****
00570>
00571> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SWHMYMO\
00572> | Rainfall dir.: V:\20983.DU\ENG\3RDSUB-1\SWHMYMO\
00573> | TZERO = .00 hrs on 0
00574> | METOUT = 2 (output = METRIC)
00575> | NRUN = 001
00576> | NSTORM = 0
00577>
00578> 001:0002-----
00579>
00580> | CHICAGO STORM | IDF curve parameters: A= 732.951
00581> | Ptotal= 31.86 mm | B= 6.199
00582> | C= .810
00583>
00584> used in: INTENSITY = A / (t + B) ^ C
00585>
00586> Duration of storm = 3.00 hrs
00587> Storm time step = 10.00 min
00588> Time to peak ratio = .33
00589>
00590> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00591> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00592> .17 2.815 | 1.00 76.805 | 1.83 5.095 | 2.67 2.684
00593> .33 3.498 | 1.17 24.079 | 2.00 4.291 | 2.83 2.463
00594> .50 4.687 | 1.33 12.364 | 2.17 3.718 | 3.00 2.279
00595> .67 7.305 | 1.50 8.324 | 2.33 3.286 |
00596> .83 18.209 | 1.67 6.303 | 2.50 2.953 |
00597>
00598> 001:0003-----
00599>
00600> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHMYMO\ORGA.VAL
00601> | ICASEdv = 1 (read and print data)
00602>
00603> Filetitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
00604>
00605> Horton's infiltration equation parameters:
00606> [F= 50.00 mm/hr] [F= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
00607> Parameters for Pervious surfaces in STANDHYD:
00608> [IAper= 4.67 mm] [LGP=40.00 m] [MNP= .250]
00609> Parameters for IMPervious surfaces in STANDHYD:
00610> [IAimp= 1.57 mm] [CL= 1.50] [MNI= .035]
00611> Parameters used in NASHYD:
00612> [IA= 4.67 mm] [N= 3.00]
00613> 001:0004-----
00614> *****
00615> * ORGAWORLD FILE *
00616> *****
00617> * SUB-AREA No.1
00618>
00619> | CALIB STANDHYD | Area (ha)= 2.07
00620> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
00621>
00622> IMPERVIOUS PERVIOUS (i)
00623> Surface Area (ha)= 1.74 .33
00624> Dep. Storage (mm)= 1.57 4.67
00625> Average Slope (%)= .52 1.00
00626> Length (m)= 204.72 20.00
00627> Mannings n = .030 .250
00628>
00629> Max.eff.Inten.(mm/hr)= 76.81 11.88
00630> over (min)= 10.00 22.50
00631> Storage Coeff. (min)= 8.77 (ii) 22.21 (ii)
00632> Unit Hyd. Tpeak (min)= 10.00 22.50
00633> Unit Hyd. peak (cms)= .12 .05
00634>
00635> *TOTALS*
00636> PEAK FLOW (cms)= .24 .01 .245 (iii)
00637> TIME TO PEAK (hrs)= 1.08 1.38 1.083
00638> RUNOFF VOLUME (mm)= 30.29 8.52 26.807
00639> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00640> RUNOFF COEFFICIENT = .95 .27 .841
00641>
00642> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
00643> CN* = 81.0 Ia = Dep. Storage (Above)
00644> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00645> THAN THE STORAGE COEFFICIENT.
00646> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00647>
00648> 001:0005-----
00649> *
00650> * SUB-AREA No.2
00651>
00652> | CALIB STANDHYD | Area (ha)= 1.54
00653> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
00654>
00655> IMPERVIOUS PERVIOUS (i)
00656> Surface Area (ha)= 1.42 .12
00657> Dep. Storage (mm)= 1.57 4.67
00658> Average Slope (%)= .50 1.00
00659> Length (m)= 244.34 5.00
00660> Mannings n = .030 .030
00661>
00662> Max.eff.Inten.(mm/hr)= 76.81 15.07
00663> over (min)= 10.00 12.50
00664> Storage Coeff. (min)= 9.87 (ii) 11.36 (ii)
00665> Unit Hyd. Tpeak (min)= 10.00 12.50
00666> Unit Hyd. peak (cms)= .11 .10
00667>
00668> *TOTALS*
00669> PEAK FLOW (cms)= .19 .00 .192 (iii)
00670> TIME TO PEAK (hrs)= 1.08 1.17 1.083
00671> RUNOFF VOLUME (mm)= 30.29 8.52 28.548
00672> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00673> RUNOFF COEFFICIENT = .95 .27 .896
00674>
00675> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
00676> CN* = 81.0 Ia = Dep. Storage (Above)

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00677> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00678> THAN THE STORAGE COEFFICIENT.
00679> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00680>
00681> 001:0006-----
00682> *
00683> * SUB-AREA No.3
00684>
00685> | CALIB STANDHYD | Area (ha)= 1.40
00686> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00687>
00688> IMPERVIOUS PERVIOUS (i)
00689> Surface Area (ha)= 1.36 .04
00690> Dep. Storage (mm)= 1.57 4.67
00691> Average Slope (%)= .51 1.00
00692> Length (m)= 225.63 5.00
00693> Mannings n = .030 .030
00694>
00695> Max.eff.Inten.(mm/hr)= 76.81 16.59
00696> over (min)= 10.00 10.00
00697> Storage Coeff. (min)= 9.35 (ii) 10.79 (ii)
00698> Unit Hyd. Tpeak (min)= 10.00 10.00
00699> Unit Hyd. peak (cms)= .12 .11
00700>
00701> *TOTALS*
00702> PEAK FLOW (cms)= .18 .00 .186 (iii)
00703> TIME TO PEAK (hrs)= 1.08 1.13 1.083
00704> RUNOFF VOLUME (mm)= 30.29 8.52 29.637
00705> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00706> RUNOFF COEFFICIENT = .95 .27 .930
00707>
00708> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
00709> CN* = 81.0 Ia = Dep. Storage (Above)
00710> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00711> THAN THE STORAGE COEFFICIENT.
00712> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00713>
00714> 001:0007-----
00715>
00716> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00717> | (ha) (cms) (hrs) (mm) (cms)
00718> | ID1 01:010 2.07 .245 1.08 26.81 .000
00719> | ID2 02:020 1.54 .192 1.08 28.55 .000
00720>
00721> SUM 04:040 3.61 .436 1.08 27.55 .000
00722>
00723> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00724>
00725>
00726> 001:0008-----
00727>
00728> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00729> | (ha) (cms) (hrs) (mm) (cms)
00730> | ID1 03:030 1.40 .186 1.08 29.64 .000
00731> | ID2 04:040 3.61 .436 1.08 27.55 .000
00732>
00733> SUM 05:050 5.01 .623 1.08 28.13 .000
00734>
00735> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00736>
00737>
00738> 001:0009-----
00739> *
00740> * SUB-AREA No.4
00741>
00742> | CALIB STANDHYD | Area (ha)= .89
00743> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00744>
00745> IMPERVIOUS PERVIOUS (i)
00746> Surface Area (ha)= .86 .03
00747> Dep. Storage (mm)= 1.57 4.67
00748> Average Slope (%)= .93 .70
00749> Length (m)= 164.82 40.00
00750> Mannings n = .030 .250
00751>
00752> Max.eff.Inten.(mm/hr)= 76.81 10.24
00753> over (min)= 7.50 30.00
00754> Storage Coeff. (min)= 6.47 (ii) 30.53 (ii)
00755> Unit Hyd. Tpeak (min)= 7.50 30.00
00756> Unit Hyd. peak (cms)= .16 .04
00757>
00758> *TOTALS*
00759> PEAK FLOW (cms)= .14 .00 .139 (iii)
00760> TIME TO PEAK (hrs)= 1.04 1.54 1.042
00761> RUNOFF VOLUME (mm)= 30.29 8.52 29.637
00762> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00763> RUNOFF COEFFICIENT = .95 .27 .930
00764>
00765> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
00766> CN* = 81.0 Ia = Dep. Storage (Above)
00767> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00768> THAN THE STORAGE COEFFICIENT.
00769> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00770>
00771> 001:0010-----
00772> *
00773> * SUB-AREA No.5
00774>
00775> | CALIB STANDHYD | Area (ha)= 2.66
00776> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00777>
00778> IMPERVIOUS PERVIOUS (i)
00779> Surface Area (ha)= 2.58 .08
00780> Dep. Storage (mm)= 1.57 4.67
00781> Average Slope (%)= .61 1.50
00782> Length (m)= 207.25 20.00
00783> Mannings n = .030 .250
00784>
00785> Max.eff.Inten.(mm/hr)= 76.81 12.71
00786> over (min)= 7.50 20.00
00787> Storage Coeff. (min)= 8.42 (ii) 20.00 (ii)
00788> Unit Hyd. Tpeak (min)= 7.50 20.00
00789> Unit Hyd. peak (cms)= .14 .06
00790>
00791> *TOTALS*
00792> PEAK FLOW (cms)= .38 .00 .379 (iii)
00793> TIME TO PEAK (hrs)= 1.04 1.33 1.042
00794> RUNOFF VOLUME (mm)= 30.29 8.52 29.637
00795> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00796> RUNOFF COEFFICIENT = .95 .27 .930
00797>
00798> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
00799> CN* = 81.0 Ia = Dep. Storage (Above)
00800> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00801> THAN THE STORAGE COEFFICIENT.
00802> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00803>
00804> 001:0011-----
00805>
00806> | ADD HYD (080 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00807> | (ha) (cms) (hrs) (mm) (cms)
00808> | ID1 06:060 .89 .139 1.04 29.64 .000
00809> | ID2 07:070 2.66 .379 1.04 29.64 .000
00810>

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00811> SUM 08:080 3.55 .518 1.04 29.64 .000  
 00812>  
 00813> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00814>  
 00815>  
 00816> 001:0012-----  
 00817> | ADD HYD (090 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 00818> | (ha) (cms) (hrs) (mm) (cms) (cms)  
 00819> ID1 05:050 5.01 .623 1.08 28.13 .000  
 00820> +ID2 08:080 3.55 .518 1.04 29.64 .000  
 00821> SUM 09:090 8.56 1.118 1.08 28.76 .000  
 00822>  
 00823> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00824>  
 00825>  
 00826>  
 00827> 001:0013-----  
 00828> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 00829> | IN>09: (090 ) |  
 00830> | OUT<10: (POND ) |  
 00831>  
 00832> ===== OUTFLOW STORAGE TABLE =====  
 00833> OUTFLOW STORAGE OUTFLOW STORAGE  
 00834> (cms) (ha.m.) (cms) (ha.m.)  
 00835> .000 .0000E+00 | .593 .6251E+00  
 00836> .008 .6560E-01 | .654 .6631E+00  
 00837> .017 .1311E+00 | .797 .7391E+00  
 00838> .093 .2831E+00 | .950 .8274E+00  
 00839> .233 .3971E+00 | 1.304 .9157E+00  
 00840> .337 .4731E+00 | 1.880 .1004E+01  
 00841> .465 .5491E+00 | 2.577 .1092E+01  
 00842> .531 .5871E+00 | .000 .0000E+00  
 00843>  
 00844> ROUTING RESULTS AREA OPEAK TPEAK R.V.  
 00845> (ha) (cms) (hrs) (mm)  
 00846> INFLOW >09: (090 ) 8.56 1.118 1.083 28.757  
 00847> OUTFLOW<10: (POND ) 8.56 .056 3.000 28.754  
 00848>  
 00849> PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.030  
 00850> TIME SHIFT OF PEAK FLOW (min) = 115.00  
 00851> MAXIMUM STORAGE USED (ha.m.) = .2095E+00  
 00852>  
 00853>  
 00854> 001:0014-----  
 00855> \*\*\*\*\*  
 00856> \* Remaining Hawthorne Industrial Park \*  
 00857> \*\*\*\*\*  
 00858> \* SUB-AREA No.1  
 00859>  
 00860> 00861> | CALIB STANDHYD | Area (ha) = 19.90  
 00862> | 01:HIP01 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 00863>  
 00864> IMPERVIOUS PERVIOUS (i)  
 00865> Surface Area (ha) = 14.13 5.77  
 00866> Dep. Storage (mm) = 1.57 4.67  
 00867> Average Slope (%) = .60 1.50  
 00868> Length (m) = 580.00 100.00  
 00869> Mannings n = .030 .250  
 00870>  
 00871> Max.eff.Inten.(mm/hr) = 54.21 23.06  
 00872> over (min) = 17.50 42.50  
 00873> Storage Coeff. (min) = 18.04 (ii) 42.02 (ii)  
 00874> Unit Hyd. Tpeak (min) = 17.50 42.50  
 00875> Unit Hyd. peak (cms) = .06 .03  
 00876>  
 00877> PEAK FLOW (cms) = .95 .21 \*TOTALS\*  
 00878> TIME TO PEAK (hrs) = 1.21 1.71 1.250  
 00879> RUNOFF VOLUME (mm) = 31.29 21.814  
 00880> TOTAL RAINFALL (mm) = 31.86 31.86 31.860  
 00881> RUNOFF COEFFICIENT = .95 .42 .685  
 00882>  
 00883> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00884> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00885> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00886> THAN THE STORAGE COEFFICIENT.  
 00887> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00888>  
 00889>  
 00890> 001:0015-----  
 00891> | ADD HYD (HIP02 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 00892> | (ha) (cms) (hrs) (mm) (cms) (cms)  
 00893> ID1 10:POND 8.56 .056 3.00 28.75 .000  
 00894> +ID2 01:HIP01 19.90 1.020 1.25 21.81 .000  
 00895> SUM 02:HIP02 28.46 1.039 1.25 23.90 .000  
 00896>  
 00897> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00898>  
 00899>  
 00900>  
 00901>  
 00902> 001:0016-----  
 00903> \* SUB-AREA No.2  
 00904>  
 00905> 00906> | CALIB STANDHYD | Area (ha) = 17.00  
 00907> | 03:HIP03 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 00908>  
 00909> IMPERVIOUS PERVIOUS (i)  
 00910> Surface Area (ha) = 12.07 4.93  
 00911> Dep. Storage (mm) = 1.57 4.67  
 00912> Average Slope (%) = .65 1.50  
 00913> Length (m) = 450.00 100.00  
 00914> Mannings n = .030 .250  
 00915>  
 00916> Max.eff.Inten.(mm/hr) = 59.23 25.04  
 00917> over (min) = 15.00 37.50  
 00918> Storage Coeff. (min) = 14.60 (ii) 37.80 (ii)  
 00919> Unit Hyd. Tpeak (min) = 15.00 37.50  
 00920> Unit Hyd. peak (cms) = .08 .03  
 00921>  
 00922> PEAK FLOW (cms) = .91 .19 \*TOTALS\*  
 00923> TIME TO PEAK (hrs) = 1.17 1.63 1.167  
 00924> RUNOFF VOLUME (mm) = 30.29 13.34 21.814  
 00925> TOTAL RAINFALL (mm) = 31.86 31.86 31.860  
 00926> RUNOFF COEFFICIENT = .95 .42 1.685  
 00927>  
 00928> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00929> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00930> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00931> THAN THE STORAGE COEFFICIENT.  
 00932> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00933>  
 00934>  
 00935> 001:0017-----  
 00936> \* SUB-AREA No.3  
 00937>  
 00938> 00939> | CALIB STANDHYD | Area (ha) = 18.10  
 00940> | 04:HIP04 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 00941>  
 00942> IMPERVIOUS PERVIOUS (i)  
 00943> Surface Area (ha) = 12.85 5.25  
 00944> Dep. Storage (mm) = 1.57 4.67  
 00945> Average Slope (%) = .50 1.50

00946> Length (m) = 600.00 100.00  
 00947> Mannings n = .030 .250  
 00948>  
 00949> Max.eff.Inten.(mm/hr) = 50.44 22.17  
 00950> over (min) = 20.00 45.00  
 00951> Storage Coeff. (min) = 20.01 (ii) 44.37 (ii)  
 00952> Unit Hyd. Tpeak (min) = 20.00 45.00  
 00953> Unit Hyd. peak (cms) = .06 .03  
 00954>  
 00955> PEAK FLOW (cms) = .80 .18 \*TOTALS\*  
 00956> TIME TO PEAK (hrs) = 1.25 1.79 1.292  
 00957> RUNOFF VOLUME (mm) = 30.29 13.34 21.814  
 00958> TOTAL RAINFALL (mm) = 31.86 31.86 31.860  
 00959> RUNOFF COEFFICIENT = .95 .42 .685  
 00960>  
 00961> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00962> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00963> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00964> THAN THE STORAGE COEFFICIENT.  
 00965> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00966>  
 00967>  
 00968> 001:0018-----  
 00969> | ADD HYD (HIP05 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 00970> | (ha) (cms) (hrs) (mm) (cms) (cms)  
 00971> ID1 03:HIP03 17.00 .978 1.17 21.81 .000  
 00972> +ID2 04:HIP04 18.10 .874 1.29 21.81 .000  
 00973> SUM 05:HIP05 35.10 1.814 1.21 21.81 .000  
 00974>  
 00975> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00976>  
 00977>  
 00978>  
 00979>  
 00980> 001:0019-----  
 00981> \* SUB-AREA No.4  
 00982>  
 00983> 00984> | DESIGN NASHYD | Area (ha) = 4.00 Curve Number (CN)=85.00  
 00985> | 06:Pond-B DT= 2.50 | Ia (mm) = 4.570 # of Linear Res. (N)= 3.00  
 00986> U.H. Tp(hrs) = .170  
 00987>  
 00988> Unit Hyd Opeak (cms) = .899  
 00989>  
 00990> PEAK FLOW (cms) = .145 (i)  
 00991> TIME TO PEAK (hrs) = 1.167  
 00992> RUNOFF VOLUME (mm) = 10.266  
 00993> TOTAL RAINFALL (mm) = 31.860  
 00994> RUNOFF COEFFICIENT = .322  
 00995>  
 00996> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00997>  
 00998>  
 00999>  
 01000> 001:0020-----  
 01001> | ADD HYD (HIP06 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 01002> | (ha) (cms) (hrs) (mm) (cms) (cms)  
 01003> ID1 02:HIP02 28.46 1.039 1.25 23.90 .000  
 01004> +ID2 05:HIP05 35.10 1.814 1.21 21.81 .000  
 01005> +ID3 06:Pond-B 4.00 .145 1.17 10.27 .000  
 01006> SUM 07:HIP06 67.56 2.992 1.21 22.01 .000  
 01007>  
 01008> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 01009>  
 01010>  
 01011>  
 01012> 001:0021-----  
 01013> \* SUB-AREA No. 5  
 01014>  
 01015> | DESIGN NASHYD | Area (ha) = 6.80 Curve Number (CN)=76.00  
 01016> | 10:A2 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00  
 01017> U.H. Tp(hrs) = .370  
 01018>  
 01019> Unit Hyd Opeak (cms) = .702  
 01020>  
 01021> PEAK FLOW (cms) = .102 (i)  
 01022> TIME TO PEAK (hrs) = 1.458  
 01023> RUNOFF VOLUME (mm) = 6.883  
 01024> TOTAL RAINFALL (mm) = 31.860  
 01025> RUNOFF COEFFICIENT = .216  
 01026>  
 01027> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01028>  
 01029>  
 01030> 001:0022-----  
 01031> \* SUB-AREA NO 4  
 01032>  
 01033> | DESIGN NASHYD | Area (ha) = 5.30 Curve Number (CN)=76.00  
 01034> | 01:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00  
 01035> U.H. Tp(hrs) = .804  
 01036>  
 01037> Unit Hyd Opeak (cms) = .252  
 01038>  
 01039> PEAK FLOW (cms) = .048 (i)  
 01040> TIME TO PEAK (hrs) = 2.083  
 01041> RUNOFF VOLUME (mm) = 6.883  
 01042> TOTAL RAINFALL (mm) = 31.860  
 01043> RUNOFF COEFFICIENT = .216  
 01044>  
 01045> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01046>  
 01047>  
 01048> 001:0023-----  
 01049> | ADD HYD (0020 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 01050> | (ha) (cms) (hrs) (mm) (cms) (cms)  
 01051> ID1 07:HIP06 67.56 2.992 1.21 22.01 .000  
 01052> +ID2 10:A2 6.80 .102 1.46 6.88 .000  
 01053> +ID3 01:A3 5.30 .048 2.08 6.88 .000  
 01054> SUM 02:0020 79.66 3.077 1.21 19.71 .000  
 01055>  
 01056> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 01057>  
 01058>  
 01059>  
 01060>  
 01061> 001:0024-----  
 01062> \*\*\*\*\*  
 01063> \* CALCULATION OF 3HR - 1:5 YEAR STORM EVENT \*  
 01064> \*\*\*\*\*  
 01065>  
 01066> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SMHGHYMO\  
 01067> Rainfall dir.: V:\20983.DU\ENG\3RDSUB-1\SMHGHYMO\  
 01068> TZERO = .00 hrs on 0  
 01069> METOUT= 2 (output = METRIC)  
 01070> NRUN = 001  
 01071> NSTORM = 0  
 01072>  
 01073> 001:0002-----  
 01074> | CHICAGO STORM | IDF curve parameters: A= 998.071  
 01075> B= 6.053  
 01076> | Ptotal= 42.51 mm | C= .814  
 01077> used in: INTENSITY = A / (t + B)^C  
 01078>  
 01079> Duration of storm = 3.00 hrs  
 01080>

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01081> Storm time step = 10.00 min
01082> Time to peak ratio = .33
01083>
01084>
01085>
01086>
01087>
01088>
01089>
01090>
01091>
01092>
01093> 001:0003-----
01094>
01095> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\ORGA.VAL
01096> | ICASEdv = 1 (read and print data)
01097> | FileTitle=----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
01098> |----- PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----|
01099> Horton's infiltration equation parameters:
01100> [F= 50.00 mm/hr] [F= 7.50 mm/hr] [DCAL= 2.00 /hr] [F= .00 mm]
01101> Parameters for PERVIOUS surfaces in STANDHYD:
01102> [Laper= 4.67 mm] [LGP=40.00 mm] [MNP=.250]
01103> Parameters for IMPVIOUS surfaces in STANDHYD:
01104> [Iaimp= 1.57 mm] [CLie=1.50] [MNI=.035]
01105> Parameters used in NASHYD:
01106> [Ia= 4.67 mm] [N= 3.00]
01107>
01108> 001:0004-----
01109> *-----
01110> * ORGAWORLD FILE
01111> *-----
01112> * SUB-AREA No.1
01113>
01114> | CALIB STANDHYD | Area (ha)= 2.07
01115> | C1:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01116>
01117>
01118> IMPVIOUS PERVIOUS (i)
01119> Surface Area (ha)= 1.74 .33
01120> Dep. Storage (mm)= 1.57 4.67
01121> Average Slope (%)= .52 1.00
01122> Length (m)= 204.72 20.00
01123> Mannings n = .030 .250
01124>
01125> Max.eff.Inten.(mm/hr)= 104.19 24.26
01126> over (min)= 7.50 17.50
01127> Storage Coeff. (min)= 7.76 (ii) 17.86 (ii)
01128> Unit Hyd. Tpeak (min)= 7.50 17.50
01129> Unit Hyd. peak (cms)= .15 .06
01130>
01131> PEAK FLOW (cms)= .36 .01 *TOTALS*
01132> TIME TO PEAK (hrs)= 1.04 1.25 1.362 (iii)
01133> RUNOFF VOLUME (mm)= 40.94 14.70 36.745
01134> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01135> RUNOFF COEFFICIENT = .96 .35 .864
01136>
01137> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01138> CN* = 81.0 Ia = Dep. Storage (Above)
01139> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01140> THAN THE STORAGE COEFFICIENT.
01141> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01142>
01143> 001:0005-----
01144> *
01145> * SUB-AREA No.2
01146>
01147> | CALIB STANDHYD | Area (ha)= 1.54
01148> | O2:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
01149>
01150>
01151> IMPVIOUS PERVIOUS (i)
01152> Surface Area (ha)= 1.42 .12
01153> Dep. Storage (mm)= 1.57 4.67
01154> Average Slope (%)= .50 1.00
01155> Length (m)= 244.34 5.00
01156> Mannings n = .030 .030
01157>
01158> Max.eff.Inten.(mm/hr)= 104.19 31.02
01159> over (min)= 7.50 10.00
01160> Storage Coeff. (min)= 8.73 (ii) 9.85 (ii)
01161> Unit Hyd. Tpeak (min)= 7.50 10.00
01162> Unit Hyd. peak (cms)= .14 .11
01163>
01164> PEAK FLOW (cms)= .28 .01 *TOTALS*
01165> TIME TO PEAK (hrs)= 1.04 1.13 1.042
01166> RUNOFF VOLUME (mm)= 40.94 14.70 38.845
01167> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01168> RUNOFF COEFFICIENT = .96 .35 .914
01169>
01170> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01171> CN* = 81.0 Ia = Dep. Storage (Above)
01172> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01173> THAN THE STORAGE COEFFICIENT.
01174> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01175>
01176> 001:0006-----
01177> *
01178> * SUB-AREA No.3
01179>
01180> | CALIB STANDHYD | Area (ha)= 1.40
01181> | O3:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01182>
01183>
01184> IMPVIOUS PERVIOUS (i)
01185> Surface Area (ha)= 1.36 .04
01186> Dep. Storage (mm)= 1.57 4.67
01187> Average Slope (%)= .51 1.00
01188> Length (m)= 225.63 5.00
01189> Mannings n = .030 .030
01190>
01191> Max.eff.Inten.(mm/hr)= 104.19 31.02
01192> over (min)= 7.50 10.00
01193> Storage Coeff. (min)= 8.28 (ii) 9.39 (ii)
01194> Unit Hyd. Tpeak (min)= 7.50 10.00
01195> Unit Hyd. peak (cms)= .14 .12
01196>
01197> PEAK FLOW (cms)= .27 .00 *TOTALS*
01198> TIME TO PEAK (hrs)= 1.04 1.13 1.042
01199> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
01200> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01201> RUNOFF COEFFICIENT = .96 .35 .945
01202>
01203> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01204> CN* = 81.0 Ia = Dep. Storage (Above)
01205> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01206> THAN THE STORAGE COEFFICIENT.
01207> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01208>
01209> 001:0007-----
01210> *
01211> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01212> (ha) (cms) (hrs) (mm) (cms)
01213> ID1 01:010 2.07 .362 1.04 36.75 .000
01214> +ID2 02:020 1.54 .283 1.04 38.84 .000
01215>

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01216> SUM 04:040 3.61 .645 1.04 37.64 .000
01217>
01218> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01219>
01220>
01221> 001:0008-----
01222>
01223> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01224> (ha) (cms) (hrs) (mm) (cms)
01225> ID1 03:030 1.40 .274 1.04 40.16 .000
01226> +ID2 04:040 3.61 .645 1.04 37.64 .000
01227>
01228> SUM 05:050 5.01 .918 1.04 38.34 .000
01229>
01230>
01231> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01232>
01233> 001:0009-----
01234> *
01235> * SUB-AREA No.4
01236>
01237> | CALIB STANDHYD | Area (ha)= .89
01238> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01239>
01240>
01241> IMPVIOUS PERVIOUS (i)
01242> Surface Area (ha)= .86 .03
01243> Dep. Storage (mm)= 1.57 4.67
01244> Average Slope (%)= .93 .70
01245> Length (m)= 164.82 40.00
01246> Mannings n = .030 .250
01247>
01248> Max.eff.Inten.(mm/hr)= 104.19 20.32
01249> over (min)= 5.00 25.00
01250> Storage Coeff. (min)= 5.72 (ii) 24.02 (ii)
01251> Unit Hyd. Tpeak (min)= 5.00 25.00
01252> Unit Hyd. peak (cms)= .20 .05
01253>
01254> PEAK FLOW (cms)= .20 .00 *TOTALS*
01255> TIME TO PEAK (hrs)= 1.00 1.38 1.000
01256> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
01257> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01258> RUNOFF COEFFICIENT = .96 .35 .945
01259>
01260> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01261> CN* = 81.0 Ia = Dep. Storage (Above)
01262> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01263> THAN THE STORAGE COEFFICIENT.
01264> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01265>
01266> 001:0010-----
01267> *
01268> * SUB-AREA No.5
01269>
01270> | CALIB STANDHYD | Area (ha)= 2.66
01271> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01272>
01273>
01274> IMPVIOUS PERVIOUS (i)
01275> Surface Area (ha)= 2.58 .08
01276> Dep. Storage (mm)= 1.57 4.67
01277> Average Slope (%)= .51 1.50
01278> Length (m)= 207.25 20.00
01279> Mannings n = .030 .250
01280>
01281> Max.eff.Inten.(mm/hr)= 104.19 24.26
01282> over (min)= 7.50 17.50
01283> Storage Coeff. (min)= 7.45 (ii) 16.40 (ii)
01284> Unit Hyd. Tpeak (min)= 7.50 17.50
01285> Unit Hyd. peak (cms)= .15 .07
01286>
01287> PEAK FLOW (cms)= .54 .00 *TOTALS*
01288> TIME TO PEAK (hrs)= 1.04 1.25 1.042
01289> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
01290> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01291> RUNOFF COEFFICIENT = .96 .35 .945
01292>
01293> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01294> CN* = 81.0 Ia = Dep. Storage (Above)
01295> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01296> THAN THE STORAGE COEFFICIENT.
01297> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01298>
01299> 001:0011-----
01300> *
01301> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01302> (ha) (cms) (hrs) (mm) (cms)
01303> ID1 06:060 .89 .205 1.00 40.16 .000
01304> +ID2 07:070 2.66 .538 1.04 40.16 .000
01305>
01306> SUM 08:080 3.55 .733 1.04 40.16 .000
01307>
01308> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01309>
01310>
01311> 001:0012-----
01312> *
01313> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01314> (ha) (cms) (hrs) (mm) (cms)
01315> ID1 05:050 5.01 .918 1.04 38.34 .000
01316> +ID2 08:080 3.55 .733 1.04 40.16 .000
01317>
01318> SUM 09:090 8.56 1.651 1.04 39.10 .000
01319>
01320>
01321> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01322>
01323> 001:0013-----
01324> *
01325> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01326> | INP:09: (090 ) |
01327> | OUT:10: (POND ) |
01328>
01329> ===== OUTFLOW STORAGE TABLE =====
01330> OUTFLOW STORAGE OUTFLOW STORAGE
01331> (cms) (ha.m.) (cms) (ha.m.)
01332> .000 .0000E+00 .593 .6251E+00
01333> .008 .6560E-01 .654 .6631E+00
01334> .017 .1311E+00 .797 .7391E+00
01335> .093 .2831E+00 .950 .8274E+00
01336> .233 .3971E+00 1.304 .9157E+00
01337> .337 .4731E+00 1.880 .1004E+01
01338> .465 .5491E+00 2.577 .1092E+01
01339> .531 .5871E+00 .000 .0000E+00
01340>
01341> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01342> (ha) (cms) (hrs) (mm)
01343> INFLOW >09: (090 ) 8.56 1.651 1.042 39.096
01344> OUTFLOW<10: (POND ) 8.56 .089 2.625 39.093
01345>
01346> PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.413
01347> TIME SHIFT OF PEAK FLOW (min) = 95.00
01348> MAXIMUM STORAGE USED (ha.m.) = .2758E+00
01349>
01350> 001:0014-----
01351> *****

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01351> * Remaining Hawthorne Industrial Park *
01352> *****
01353> *
01354> * SUB-AREA No.1
01355>
01356> | CALIB STANDHYD | Area (ha)= 19.90
01357> | 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01358>
01359> IMPERVIOUS PERVIOUS (i)
01360> Surface Area (ha)= 14.13 5.77
01361> Dep. Storage (mm)= 1.57 4.67
01362> Average Slope (%)= .60 1.50
01363> Length (m)= 580.00 100.00
01364> Mannings n = .030 .250
01365>
01366> Max.eff.Inten.(mm/hr)= 80.14 42.65
01367> over (min)= 15.00 35.00
01368> Storage Coeff. (min)= 15.43 (ii) 34.18 (ii)
01369> Unit Hyd. Tpeak (min)= 15.00 35.00
01370> Unit Hyd. peak (cms)= .07 .03
01371>
01372> PEAK FLOW (cms)= 1.41 .40 1.572 (iii)
01373> TIME TO PEAK (hrs)= 1.17 1.54 1.208
01374> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01375> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01376> RUNOFF COEFFICIENT = .96 .50 .732
01377>
01378> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01379> CN* = 81.0 Ia = Dep. Storage (Above)
01380> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01381> THAN THE STORAGE COEFFICIENT.
01382> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01383>
01384>
01385> 001:0015-----
01386>
01387> | ADD HYD (H1P02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01388> | 10:POND 8.56 .089 2.63 39.09 .000
01389> +ID2 01:H1P01 19.90 1.572 1.21 31.13 .000
01390>
01391> SUM 02:H1P02 28.46 1.615 1.21 33.52 .000
01392>
01393> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01394>
01395> 001:0016-----
01396>
01397> * SUB-AREA No.2
01398>
01400> | CALIB STANDHYD | Area (ha)= 17.00
01401> | 03:H1P03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01402>
01403> IMPERVIOUS PERVIOUS (i)
01404> Surface Area (ha)= 12.07 4.93
01405> Dep. Storage (mm)= 1.57 4.67
01406> Average Slope (%)= .65 1.50
01407> Length (m)= 450.00 100.00
01408> Mannings n = .030 .250
01409>
01410> Max.eff.Inten.(mm/hr)= 89.76 47.48
01411> over (min)= 12.50 30.00
01412> Storage Coeff. (min)= 12.36 (ii) 30.32 (ii)
01413> Unit Hyd. Tpeak (min)= 12.50 30.00
01414> Unit Hyd. peak (cms)= .09 .04
01415>
01416> PEAK FLOW (cms)= 1.36 .37 1.504 (iii)
01417> TIME TO PEAK (hrs)= 1.13 1.46 1.167
01418> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01419> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01420> RUNOFF COEFFICIENT = .96 .50 .732
01421>
01422> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01423> CN* = 81.0 Ia = Dep. Storage (Above)
01424> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01425> THAN THE STORAGE COEFFICIENT.
01426> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01427>
01428>
01429> 001:0017-----
01430>
01431> * SUB-AREA No.3
01432>
01433> | CALIB STANDHYD | Area (ha)= 18.10
01434> | 04:H1P04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01435>
01436> IMPERVIOUS PERVIOUS (i)
01437> Surface Area (ha)= 12.85 5.25
01438> Dep. Storage (mm)= 1.57 4.67
01439> Average Slope (%)= .50 1.50
01440> Length (m)= 600.00 100.00
01441> Mannings n = .030 .250
01442>
01443> Max.eff.Inten.(mm/hr)= 73.27 42.65
01444> over (min)= 17.50 35.00
01445> Storage Coeff. (min)= 17.24 (ii) 35.98 (ii)
01446> Unit Hyd. Tpeak (min)= 17.50 35.00
01447> Unit Hyd. peak (cms)= .07 .03
01448>
01449> PEAK FLOW (cms)= 1.19 .35 1.364 (iii)
01450> TIME TO PEAK (hrs)= 1.21 1.54 1.250
01451> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01452> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01453> RUNOFF COEFFICIENT = .96 .50 .732
01454>
01455> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01456> CN* = 81.0 Ia = Dep. Storage (Above)
01457> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01458> THAN THE STORAGE COEFFICIENT.
01459> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01460>
01461>
01462> 001:0018-----
01463>
01464> | ADD HYD (H1P05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01465> | 03:H1P03 17.00 1.504 1.17 31.13 .000
01466> +ID2 04:H1P04 18.10 1.364 1.25 31.13 .000
01467>
01468> SUM 05:H1P05 35.10 2.800 1.17 31.13 .000
01469>
01470> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01471>
01472> 001:0019-----
01473>
01474> * SUB-AREA No.4
01475>
01476> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
01477> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
01478> U.H. Tp(hrs)= .170
01479>
01480> Unit Hyd Qpeak (cms)= .899
01481>
01482> PEAK FLOW (cms)= .260 (i)

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01486> TIME TO PEAK (hrs)= 1.167
01487> RUNOFF VOLUME (mm)= 17.325
01488> TOTAL RAINFALL (mm)= 42.514
01489> RUNOFF COEFFICIENT = .408
01490>
01491> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01492>
01493> 001:0020-----
01494>
01495> | ADD HYD (H1P06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01496> | 02:H1P02 28.46 1.615 1.21 33.52 .000
01497> +ID2 03:H1P05 35.10 2.800 1.17 31.13 .000
01498> +ID3 06:Pond-B 4.00 .260 1.17 17.32 .000
01499>
01500> SUM 07:H1P06 67.56 4.661 1.17 31.32 .000
01501>
01502> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01503>
01504> 001:0021-----
01505>
01506> * SUB-AREA NO. 5
01507>
01508> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00
01509> | 10:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
01510> U.H. Tp(hrs)= .370
01511>
01512> Unit Hyd Qpeak (cms)= .702
01513>
01514> PEAK FLOW (cms)= .187 (i)
01515> TIME TO PEAK (hrs)= 1.458
01516> RUNOFF VOLUME (mm)= 12.131
01517> TOTAL RAINFALL (mm)= 42.514
01518> RUNOFF COEFFICIENT = .285
01519>
01520> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01521>
01522> 001:0022-----
01523>
01524> * SUB-AREA NO 4
01525>
01526> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00
01527> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
01528> U.H. Tp(hrs)= .804
01529>
01530> Unit Hyd Qpeak (cms)= .252
01531>
01532> PEAK FLOW (cms)= .086 (i)
01533> TIME TO PEAK (hrs)= 2.042
01534> RUNOFF VOLUME (mm)= 12.131
01535> TOTAL RAINFALL (mm)= 42.514
01536> RUNOFF COEFFICIENT = .285
01537>
01538> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01539>
01540> 001:0023-----
01541>
01542> | ADD HYD (0020 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01543> | 07:H1P06 67.56 4.661 1.17 31.32 .000
01544> +ID2 10:A2 6.80 .187 1.46 12.13 .000
01545> +ID3 01:A3 5.30 .086 2.04 12.13 .000
01546>
01547> SUM 02:0020 79.66 4.812 1.21 28.40 .000
01548>
01549> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01550>
01551> 001:0024-----
01552>
01553> * CALCULATION OF 3HR - 1:10 YEAR STORM EVENT
01554>
01555>
01556> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\
01557> | ZERO = .00 hrs on 0
01558> | METOUT= 2 (output = METRIC)
01559> | NRUN = 001
01560> | NSTORM= 0
01561>
01562> 001:0002-----
01563>
01564> | CHICAGO STORM | IDF curve parameters: A=1174.184
01565> | Ptotal= 49.50 mm | B= 6.014
01566> used in: INTENSITY = A / (t + B)^C
01567> C= .816
01568> Duration of storm = 3.00 hrs
01569> Storm time step = 10.00 min
01570> Time to peak ratio = .33
01571>
01572> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01573> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01574> .17 4.248 | 1.00 122.142 | 1.83 7.733 | 2.67 4.049
01575> .33 5.290 | 1.17 37.285 | 2.00 6.502 | 2.83 3.714
01576> .50 7.108 | 1.33 18.954 | 2.17 5.625 | 3.00 3.434
01577> .67 11.130 | 1.50 12.700 | 2.33 4.969 |
01578> .83 28.100 | 1.67 9.588 | 2.50 4.458 |
01579>
01580> 001:0003-----
01581>
01582> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHYMO\ORGA.VAL
01583> | ICASEd= 1 (read and print data)
01584> FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
01585> PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----
01586> Horton's infiltration equation parameters:
01587> [P= 50.00 mm/hr] [P= 7.50 mm/hr] [ICAY= 2.00 /hr] [P= .00 mm]
01588> Parameters for PERVIOUS surfaces in STANDHYD:
01589> [IAPER= 4.67 mm] [LGP=40.00 m] [MNP= .250]
01590> Parameters for IMPERVIOUS surfaces in STANDHYD:
01591> [IAMP= 1.57 mm] [CLI= 1.50] [MNI= .035]
01592> Parameters used in NASHYD:
01593> [Ia= 4.67 mm] [N= 3.00]
01594>
01595> 001:0004-----
01596>
01597> * SUB-AREA No.1
01598>
01599> | CALIB STANDHYD | Area (ha)= 2.07
01600> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01601>
01602> IMPERVIOUS PERVIOUS (i)
01603> Surface Area (ha)= 1.74 .33
01604> Dep. Storage (mm)= 1.57 4.67
01605> Average Slope (%)= .52 1.00
01606> Length (m)= 204.72 20.00
01607> Mannings n = .030 .250
01608>
01609> Max.eff.Inten.(mm/hr)= 122.14 34.69
01610> over (min)= 7.50 15.00

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01621> Storage Coeff. (min)= 7.28 (ii) 16.04 (ii)  
 01622> Unit Hyd. Tpeak (min)= 7.50 15.00  
 01623> Unit Hyd. peak (cms)= .15 .07  
 01624> \*TOTALS\*  
 01625> PEAK FLOW (cms)= .43 .02 .437 (iii)  
 01626> TIME TO PEAK (hrs)= 1.04 1.21 1.042  
 01627> RUNOFF VOLUME (mm)= 47.93 19.25 43.345  
 01628> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01629> RUNOFF COEFFICIENT = .97 .39 .876

01630> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01631> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01632> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01633> THAN THE STORAGE COEFFICIENT.  
 01634> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01635>  
 01636>

01637> 001:0005-----  
 01638> \* SUB-AREA No.2  
 01639>  
 01640> CALIB STANDHYD | Area (ha)= 1.54  
 01641> 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00  
 01642>  
 01643> IMPERVIOUS PERVIOUS (i)  
 01644> Surface Area (ha)= 1.42 .12  
 01645> Dep. Storage (mm)= 1.57 4.67  
 01646> Average Slope (%)= .50 1.00  
 01647> Length (m)= 244.34 5.00  
 01648> Mannings n = .030 .030  
 01649>  
 01650> Max.eff.Inten.(mm/hr)= 122.14 42.32  
 01651> over (min)= 7.50 10.00  
 01652> Storage Coeff. (min)= 8.20 (ii) 9.18 (ii)  
 01653> Unit Hyd. Tpeak (min)= 7.50 10.00  
 01654> Unit Hyd. peak (cms)= .14 .12  
 01655> \*TOTALS\*  
 01656> PEAK FLOW (cms)= .33 .01 .341 (iii)  
 01657> TIME TO PEAK (hrs)= 1.04 1.13 1.042  
 01658> RUNOFF VOLUME (mm)= 47.93 19.25 45.640  
 01659> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01660> RUNOFF COEFFICIENT = .97 .39 .922

01661> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01662> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01663> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01664> THAN THE STORAGE COEFFICIENT.  
 01665> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01666>  
 01667>

01668> 001:0006-----  
 01669> \* SUB-AREA No.3  
 01670>  
 01671> CALIB STANDHYD | Area (ha)= 1.40  
 01672> 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 01673>  
 01674> IMPERVIOUS PERVIOUS (i)  
 01675> Surface Area (ha)= 1.36 .04  
 01676> Dep. Storage (mm)= 1.57 4.67  
 01677> Average Slope (%)= .51 1.00  
 01678> Length (m)= 225.63 5.00  
 01679> Mannings n = .030 .030  
 01680>  
 01681> Max.eff.Inten.(mm/hr)= 122.14 48.18  
 01682> over (min)= 7.50 7.50  
 01683> Storage Coeff. (min)= 7.77 (ii) 8.70 (ii)  
 01684> Unit Hyd. Tpeak (min)= 7.50 7.50  
 01685> Unit Hyd. peak (cms)= .15 .14  
 01686> \*TOTALS\*  
 01687> PEAK FLOW (cms)= .33 .00 .329 (iii)  
 01688> TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 01689> RUNOFF VOLUME (mm)= 47.93 19.25 47.074  
 01690> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01691> RUNOFF COEFFICIENT = .97 .39 .951

01692> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01693> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01694> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01695> THAN THE STORAGE COEFFICIENT.  
 01696> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01697>  
 01698>

01699> 001:0007-----  
 01700> \* SUB-AREA No.4  
 01701>  
 01702> CALIB STANDHYD | Area (ha)= .89  
 01703> 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 01704>  
 01705> IMPERVIOUS PERVIOUS (i)  
 01706> Surface Area (ha)= .86 .03  
 01707> Dep. Storage (mm)= 1.57 4.67  
 01708> Average Slope (%)= .93 .70  
 01709> Length (m)= 166.82 40.00  
 01710> Mannings n = .030 .250  
 01711>  
 01712> Max.eff.Inten.(mm/hr)= 122.14 31.19  
 01713> over (min)= 5.00 20.00  
 01714> Storage Coeff. (min)= 5.37 (ii) 20.78 (ii)  
 01715> Unit Hyd. Tpeak (min)= 5.00 20.00  
 01716> Unit Hyd. peak (cms)= .21 .06  
 01717> \*TOTALS\*  
 01718> PEAK FLOW (cms)= .24 .00 .245 (iii)  
 01719> TIME TO PEAK (hrs)= 1.00 1.29 1.000  
 01720> RUNOFF VOLUME (mm)= 47.93 19.25 47.074  
 01721> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01722> RUNOFF COEFFICIENT = .97 .39 .951

01723> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01724> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01725> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01726> THAN THE STORAGE COEFFICIENT.  
 01727> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01728>  
 01729>

01730> 001:0008-----  
 01731> \* SUB-AREA No.5  
 01732>  
 01733> CALIB STANDHYD | Area (ha)= 19.90  
 01734> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01735>  
 01736> IMPERVIOUS PERVIOUS (i)  
 01737> Surface Area (ha)= 14.13 5.77  
 01738> Dep. Storage (mm)= 1.57 4.67  
 01739> Average Slope (%)= .60 1.50  
 01740> Length (m)= 580.00 100.00  
 01741> Mannings n = .030 .250  
 01742>  
 01743> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01744> over (min)= 15.00 30.00  
 01745> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01746> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01747> Unit Hyd. peak (cms)= .08 .04  
 01748> \*TOTALS\*  
 01749> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01750> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01751> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01752> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01753> RUNOFF COEFFICIENT = .97 .54 .756

01754> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01755> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01756> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01757> THAN THE STORAGE COEFFICIENT.  
 01758> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01759>  
 01760>

01761> 001:0009-----  
 01762> \* SUB-AREA No.6  
 01763>  
 01764> CALIB STANDHYD | Area (ha)= 19.90  
 01765> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01766>  
 01767> IMPERVIOUS PERVIOUS (i)  
 01768> Surface Area (ha)= 14.13 5.77  
 01769> Dep. Storage (mm)= 1.57 4.67  
 01770> Average Slope (%)= .60 1.50  
 01771> Length (m)= 580.00 100.00  
 01772> Mannings n = .030 .250  
 01773>  
 01774> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01775> over (min)= 15.00 30.00  
 01776> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01777> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01778> Unit Hyd. peak (cms)= .08 .04  
 01779> \*TOTALS\*  
 01780> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01781> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01782> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01783> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01784> RUNOFF COEFFICIENT = .97 .54 .756

01785> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01786> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01787> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01788> THAN THE STORAGE COEFFICIENT.  
 01789> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01790>  
 01791>

01792> 001:0010-----  
 01793> \* SUB-AREA No.7  
 01794>  
 01795> CALIB STANDHYD | Area (ha)= 19.90  
 01796> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01797>  
 01798> IMPERVIOUS PERVIOUS (i)  
 01799> Surface Area (ha)= 14.13 5.77  
 01800> Dep. Storage (mm)= 1.57 4.67  
 01801> Average Slope (%)= .60 1.50  
 01802> Length (m)= 580.00 100.00  
 01803> Mannings n = .030 .250  
 01804>  
 01805> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01806> over (min)= 15.00 30.00  
 01807> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01808> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01809> Unit Hyd. peak (cms)= .08 .04  
 01810> \*TOTALS\*  
 01811> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01812> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01813> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01814> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01815> RUNOFF COEFFICIENT = .97 .54 .756

01816> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01817> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01818> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01819> THAN THE STORAGE COEFFICIENT.  
 01820> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01821>  
 01822>

01823> 001:0011-----  
 01824> \* SUB-AREA No.8  
 01825>  
 01826> CALIB STANDHYD | Area (ha)= 19.90  
 01827> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01828>  
 01829> IMPERVIOUS PERVIOUS (i)  
 01830> Surface Area (ha)= 14.13 5.77  
 01831> Dep. Storage (mm)= 1.57 4.67  
 01832> Average Slope (%)= .60 1.50  
 01833> Length (m)= 580.00 100.00  
 01834> Mannings n = .030 .250  
 01835>  
 01836> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01837> over (min)= 15.00 30.00  
 01838> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01839> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01840> Unit Hyd. peak (cms)= .08 .04  
 01841> \*TOTALS\*  
 01842> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01843> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01844> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01845> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01846> RUNOFF COEFFICIENT = .97 .54 .756

01847> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01848> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01849> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01850> THAN THE STORAGE COEFFICIENT.  
 01851> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01852>  
 01853>

01854> 001:0012-----  
 01855> \* SUB-AREA No.9  
 01856>  
 01857> CALIB STANDHYD | Area (ha)= 19.90  
 01858> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01859>  
 01860> IMPERVIOUS PERVIOUS (i)  
 01861> Surface Area (ha)= 14.13 5.77  
 01862> Dep. Storage (mm)= 1.57 4.67  
 01863> Average Slope (%)= .60 1.50  
 01864> Length (m)= 580.00 100.00  
 01865> Mannings n = .030 .250  
 01866>  
 01867> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01868> over (min)= 15.00 30.00  
 01869> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01870> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01871> Unit Hyd. peak (cms)= .08 .04  
 01872> \*TOTALS\*  
 01873> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01874> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01875> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01876> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01877> RUNOFF COEFFICIENT = .97 .54 .756

01878> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01879> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01880> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01881> THAN THE STORAGE COEFFICIENT.  
 01882> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01883>  
 01884>

01756> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01757> THAN THE STORAGE COEFFICIENT.  
 01758> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01759>

01760> 001:0010-----  
 01761> \* SUB-AREA No.5  
 01762>  
 01763> CALIB STANDHYD | Area (ha)= 2.66  
 01764> 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 01765>  
 01766> IMPERVIOUS PERVIOUS (i)  
 01767> Surface Area (ha)= 2.58 .08  
 01768> Dep. Storage (mm)= 1.57 4.67  
 01769> Average Slope (%)= .61 1.50  
 01770> Length (m)= 207.25 20.00  
 01771> Mannings n = .030 .250  
 01772>  
 01773> Max.eff.Inten.(mm/hr)= 122.14 34.69  
 01774> over (min)= 7.50 15.00  
 01775> Storage Coeff. (min)= 7.00 (ii) 14.75 (ii)  
 01776> Unit Hyd. Tpeak (min)= 7.50 15.00  
 01777> Unit Hyd. peak (cms)= .16 .08  
 01778> \*TOTALS\*  
 01779> PEAK FLOW (cms)= .64 .00 .645 (iii)  
 01780> TIME TO PEAK (hrs)= 1.04 1.21 1.042  
 01781> RUNOFF VOLUME (mm)= 47.93 19.25 47.074  
 01782> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01783> RUNOFF COEFFICIENT = .97 .39 .951

01784> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01785> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01786> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01787> THAN THE STORAGE COEFFICIENT.  
 01788> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01789>  
 01790>

01791> 001:0011-----  
 01792> \* SUB-AREA No.6  
 01793>  
 01794> CALIB STANDHYD | Area (ha)= 19.90  
 01795> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01796>  
 01797> IMPERVIOUS PERVIOUS (i)  
 01798> Surface Area (ha)= 14.13 5.77  
 01799> Dep. Storage (mm)= 1.57 4.67  
 01800> Average Slope (%)= .60 1.50  
 01801> Length (m)= 580.00 100.00  
 01802> Mannings n = .030 .250  
 01803>  
 01804> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01805> over (min)= 15.00 30.00  
 01806> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01807> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01808> Unit Hyd. peak (cms)= .08 .04  
 01809> \*TOTALS\*  
 01810> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01811> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01812> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01813> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01814> RUNOFF COEFFICIENT = .97 .54 .756

01815> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01816> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01817> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01818> THAN THE STORAGE COEFFICIENT.  
 01819> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01820>  
 01821>

01822> 001:0012-----  
 01823> \* SUB-AREA No.7  
 01824>  
 01825> CALIB STANDHYD | Area (ha)= 19.90  
 01826> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01827>  
 01828> IMPERVIOUS PERVIOUS (i)  
 01829> Surface Area (ha)= 14.13 5.77  
 01830> Dep. Storage (mm)= 1.57 4.67  
 01831> Average Slope (%)= .60 1.50  
 01832> Length (m)= 580.00 100.00  
 01833> Mannings n = .030 .250  
 01834>  
 01835> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01836> over (min)= 15.00 30.00  
 01837> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01838> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01839> Unit Hyd. peak (cms)= .08 .04  
 01840> \*TOTALS\*  
 01841> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01842> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01843> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01844> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01845> RUNOFF COEFFICIENT = .97 .54 .756

01846> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01847> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01848> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01849> THAN THE STORAGE COEFFICIENT.  
 01850> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01851>  
 01852>

01853> 001:0013-----  
 01854> \* SUB-AREA No.8  
 01855>  
 01856> CALIB STANDHYD | Area (ha)= 19.90  
 01857> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01858>  
 01859> IMPERVIOUS PERVIOUS (i)  
 01860> Surface Area (ha)= 14.13 5.77  
 01861> Dep. Storage (mm)= 1.57 4.67  
 01862> Average Slope (%)= .60 1.50  
 01863> Length (m)= 580.00 100.00  
 01864> Mannings n = .030 .250  
 01865>  
 01866> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01867> over (min)= 15.00 30.00  
 01868> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01869> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01870> Unit Hyd. peak (cms)= .08 .04  
 01871> \*TOTALS\*  
 01872> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01873> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01874> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01875> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01876> RUNOFF COEFFICIENT = .97 .54 .756

01877> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01878> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01879> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01880> THAN THE STORAGE COEFFICIENT.  
 01881> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01882>  
 01883>

01884> 001:0014-----  
 01885> \* SUB-AREA No.9  
 01886>  
 01887> CALIB STANDHYD | Area (ha)= 19.90  
 01888> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01889>  
 01890> IMPERVIOUS PERVIOUS (i)  
 01891> Surface Area (ha)= 14.13 5.77  
 01892> Dep. Storage (mm)= 1.57 4.67  
 01893> Average Slope (%)= .60 1.50  
 01894> Length (m)= 580.00 100.00  
 01895> Mannings n = .030 .250  
 01896>  
 01897> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01898> over (min)= 15.00 30.00  
 01899> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)  
 01900> Unit Hyd. Tpeak (min)= 15.00 30.00  
 01901> Unit Hyd. peak (cms)= .08 .04  
 01902> \*TOTALS\*  
 01903> PEAK FLOW (cms)= 1.70 .55 1.983 (iii)  
 01904> TIME TO PEAK (hrs)= 1.17 1.46 1.208  
 01905> RUNOFF VOLUME (mm)= 47.93 26.92 37.426  
 01906> TOTAL RAINFALL (mm)= 49.50 49.50 49.505  
 01907> RUNOFF COEFFICIENT = .97 .54 .756

01908> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01909> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01910> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01911> THAN THE STORAGE COEFFICIENT.  
 01912> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01913>  
 01914>

01915> 001:0015-----  
 01916> \* SUB-AREA No.10  
 01917>  
 01918> CALIB STANDHYD | Area (ha)= 19.90  
 01919> 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01920>  
 01921> IMPERVIOUS PERVIOUS (i)  
 01922> Surface Area (ha)= 14.13 5.77  
 01923> Dep. Storage (mm)= 1.57 4.67  
 01924> Average Slope (%)= .60 1.50  
 01925> Length (m)= 580.00 100.00  
 01926> Mannings n = .030 .250  
 01927>  
 01928> Max.eff.Inten.(mm/hr)= 93.86 60.56  
 01929> over (min)= 15.00 30.00



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01891>
01892> 001:0016-----
01893> *
01894> * SUB-AREA No.2
01895>
01896> | CALIB STANDHYD | Area (ha)= 17.00
01897> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01898>
01899> IMPERVIOUS PERVIOUS (i)
01900> Surface Area (ha)= 12.07 4.93
01901> Dep. Storage (mm)= 1.57 4.67
01902> Average Slope (%)= .65 1.50
01903> Length (m)= 450.00 100.00
01904> Mannings n = .030 .250
01905>
01906> Max.eff.Inten.(mm/hr)= 105.17 63.81
01907> over (min) 12.50 27.50
01908> Storage Coeff. (min)= 11.60 (ii) 27.56 (ii)
01909> Unit Hyd. Tpeak (min)= 12.50 27.50
01910> Unit Hyd. peak (cms)= .09 .04
01911>
01912> PEAK FLOW (cms)= 1.63 .51 *TOTALS*
01913> TIME TO PEAK (hrs)= 1.13 1.42 1.167
01914> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
01915> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01916> RUNOFF COEFFICIENT = .97 .54 .756
01917>
01918> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01919> CN* = 81.0 Ia = Dep. Storage (Above)
01920> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01921> THAN THE STORAGE COEFFICIENT.
01922> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01923>
01924>
01925> 001:0017-----
01926> *
01927> * SUB-AREA No.3
01928>
01929> | CALIB STANDHYD | Area (ha)= 18.10
01930> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01931>
01932> IMPERVIOUS PERVIOUS (i)
01933> Surface Area (ha)= 12.85 5.25
01934> Dep. Storage (mm)= 1.57 4.67
01935> Average Slope (%)= .50 1.50
01936> Length (m)= 600.00 100.00
01937> Mannings n = .030 .250
01938>
01939> Max.eff.Inten.(mm/hr)= 93.86 57.19
01940> over (min) 15.00 32.50
01941> Storage Coeff. (min)= 15.61 (ii) 32.28 (ii)
01942> Unit Hyd. Tpeak (min)= 15.00 32.50
01943> Unit Hyd. peak (cms)= .07 .03
01944>
01945> PEAK FLOW (cms)= 1.49 .48 *TOTALS*
01946> TIME TO PEAK (hrs)= 1.17 1.50 1.208 (iii)
01947> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
01948> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01949> RUNOFF COEFFICIENT = .97 .54 .756
01950>
01951> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01952> CN* = 81.0 Ia = Dep. Storage (Above)
01953> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01954> THAN THE STORAGE COEFFICIENT.
01955> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01956>
01957>
01958> 001:0018-----
01959>
01960> | ADD HYD (HIPO5) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01961> | | | (ha) (cms) (hrs) (mm) (cms)
01962> | ID1 03:HIP03 | 17.00 1.865 1.17 37.43 .000
01963> | +ID2 04:HIP04 | 18.10 1.723 1.21 37.43 .000
01964>
01965> SUM 05:HIP05 35.10 3.572 1.17 37.43 .000
01966>
01967> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01968>
01969>
01970> 001:0019-----
01971> *
01972> * SUB-AREA No.4
01973>
01974> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
01975> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
01976> | U.H. Tp(hrs)= .170
01977>
01978> Unit Hyd Qpeak (cms)= .899
01979>
01980> PEAK FLOW (cms)= .345 (i)
01981> TIME TO PEAK (hrs)= 1.167
01982> RUNOFF VOLUME (mm)= 22.420
01983> TOTAL RAINFALL (mm)= 49.505
01984> RUNOFF COEFFICIENT = .453
01985>
01986> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01987>
01988>
01989> 001:0020-----
01990>
01991> | ADD HYD (HIPO6) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01992> | | | (ha) (cms) (hrs) (mm) (cms)
01993> | ID1 02:HIP02 | 28.46 2.044 1.21 39.98 .000
01994> | +ID2 05:HIP05 | 35.10 3.572 1.17 37.43 .000
01995> | +ID3 06:Pond-B | 4.00 .345 1.17 22.42 .000
01996>
01997> SUM 07:HIP06 67.56 5.939 1.17 37.61 .000
01998>
01999> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02000>
02001>
02002> 001:0021-----
02003> * SUB-AREA No. 5
02004>
02005> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00
02006> | 10:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
02007> | U.H. Tp(hrs)= .370
02008>
02009> Unit Hyd Qpeak (cms)= .702
02010>
02011> PEAK FLOW (cms)= .252 (i)
02012> TIME TO PEAK (hrs)= 1.417
02013> RUNOFF VOLUME (mm)= 16.075
02014> TOTAL RAINFALL (mm)= 49.505
02015> RUNOFF COEFFICIENT = .325
02016>
02017> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02018>
02019>
02020> 001:0022-----
02021> * SUB-AREA No 4
02022>
02023> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00
02024> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
02025> | U.H. Tp(hrs)= .804

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02026>
02027> Unit Hyd Qpeak (cms)= .252
02028>
02029> PEAK FLOW (cms)= .115 (i)
02030> TIME TO PEAK (hrs)= 2.000
02031> RUNOFF VOLUME (mm)= 16.075
02032> TOTAL RAINFALL (mm)= 49.505
02033> RUNOFF COEFFICIENT = .325
02034>
02035> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02036>
02037>
02038> 001:0023-----
02039>
02040> | ADD HYD (0020) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
02041> | | | (ha) (cms) (hrs) (mm) (cms)
02042> | ID1 07:HIP06 | 67.56 5.939 1.17 37.61 .000
02043> | +ID2 10:A2 | 6.80 .252 1.42 16.08 .000
02044> | +ID3 01:A3 | 5.30 .115 2.00 16.08 .000
02045>
02046> SUM 02:0020 79.66 6.135 1.17 34.34 .000
02047>
02048> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02049>
02050>
02051> 001:0024-----
02052> *****
02053> * CALCULATION OF 3HR - 1.25 YEAR STORM EVENT *
02054> *****
02055>
02056> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SWHMYM\
02057> | Rainfall dir.: V:\20983.DU\ENG\3RDSUB-1\SWHMYM\
02058> | TZERO = .00 hrs on 0
02059> | METOUT= 2 (output = METRIC)
02060> | NRUN = 001
02061> | NSTORM= 0
02062>
02063> 001:0002-----
02064>
02065> | CHICAGO STORM | IDF curve parameters: A=1402.884
02066> | Total= 58.23 mm | B= 6.018
02067> | | C= .819
02068> | used in: INTENSITY = A / (t + B)^C
02069>
02070> Duration of storm = 3.00 hrs
02071> Storm time step = 10.00 min
02072> Time to peak ratio = .33
02073>
02074> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
02075> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
02076> .17 4.934 | 1.00 144.693 | 1.83 9.014 | 2.67 4.701
02077> .33 6.152 | 1.17 43.904 | 2.00 7.571 | 2.83 4.310
02078> .50 8.282 | 1.33 22.224 | 2.17 6.544 | 3.00 3.983
02079> .67 13.006 | 1.50 14.852 | 2.33 5.776 |
02080> .83 33.041 | 1.67 11.192 | 2.50 5.179 |
02081>
02082>
02083> 001:0003-----
02084>
02085> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHMYM\ORGA.VAL
02086> | ICSAdv = 1 (read and print data)
02087> | FileTitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
02088> | ----- PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
02089>
02090> Horton's infiltration equation parameters:
02091> [P= 50.00 mm/hr] [T= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
02092> Parameters for PERVIOUS surfaces in STANDHYD:
02093> [IaPer= 4.67 mm] [LGP=40.00 m] [MNP=.250]
02094> Parameters for IMPERVIOUS surfaces in STANDHYD:
02095> [IaImp= 1.57 mm] [CLI= 1.50] [MNI=.035]
02096> Parameters used in NASHYD:
02097> [Ia= 4.67 mm] [N= 3.00]
02098>
02099> 001:0004-----
02100> *****
02101> * ORGAWORLD FILE *****
02102>
02103> * SUB-AREA No.1
02104>
02105> | CALIB STANDHYD | Area (ha)= 2.07
02106> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
02107>
02108> IMPERVIOUS PERVIOUS (i)
02109> Surface Area (ha)= 1.74 .33
02110> Dep. Storage (mm)= 1.57 4.67
02111> Average Slope (%)= .52 1.00
02112> Length (m)= 204.72 20.00
02113> Mannings n = .030 .250
02114>
02115> Max.eff.Inten.(mm/hr)= 144.69 47.07
02116> over (min) 7.50 15.00
02117> Storage Coeff. (min)= 6.81 (ii) 14.56 (ii)
02118> Unit Hyd. Tpeak (min)= 7.50 15.00
02119> Unit Hyd. peak (cms)= .16 .08
02120>
02121> PEAK FLOW (cms)= .52 .03 *TOTALS*
02122> TIME TO PEAK (hrs)= 1.04 1.21 .532 (iii)
02123> RUNOFF VOLUME (mm)= 56.66 25.35 51.647
02124> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02125> RUNOFF COEFFICIENT = .97 .44 .887
02126>
02127> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02128> CN* = 81.0 Ia = Dep. Storage (Above)
02129> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02130> THAN THE STORAGE COEFFICIENT.
02131> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02132>
02133> 001:0005-----
02134> *
02135> * SUB-AREA No.2
02136>
02137> | CALIB STANDHYD | Area (ha)= 1.54
02138> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
02139>
02140> IMPERVIOUS PERVIOUS (i)
02141> Surface Area (ha)= 1.42 .12
02142> Dep. Storage (mm)= 1.57 4.67
02143> Average Slope (%)= .50 1.00
02144> Length (m)= 244.34 5.00
02145> Mannings n = .030 .030
02146>
02147> Max.eff.Inten.(mm/hr)= 144.69 65.19
02148> over (min) 7.50 7.50
02149> Storage Coeff. (min)= 7.66 (ii) 8.49 (ii)
02150> Unit Hyd. Tpeak (min)= 7.50 7.50
02151> Unit Hyd. peak (cms)= .15 .14
02152>
02153> PEAK FLOW (cms)= .40 .01 *TOTALS*
02154> TIME TO PEAK (hrs)= 1.04 1.08 .418 (iii)
02155> RUNOFF VOLUME (mm)= 56.66 25.35 54.152
02156> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02157> RUNOFF COEFFICIENT = .97 .44 .930
02158>
02159> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02160> CN* = 81.0 Ia = Dep. Storage (Above)

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02161> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02162> THAN THE STORAGE COEFFICIENT.  
 02163> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02164>  
 02165>  
 02166> 001:0006-----  
 02167> \*  
 02168> \* SUB-AREA No.3  
 02169>  
 02170> | CALIB STANDHYD | Area (ha)= 1.40  
 02171> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 02172>  
 02173> IMPERVIOUS PERVIOUS (i)  
 02174> Surface Area (ha)= 1.36 .04  
 02175> Dep. Storage (mm)= 1.57 4.67  
 02176> Average Slope (%)= .51 1.00  
 02177> Length (m)= 225.63 5.00  
 02178> Mannings n = .030 .030  
 02179>  
 02180> Max. eff. Inten. (mm/hr)= 144.69 65.19  
 02181> over (min) 7.50 7.50  
 02182> Storage Coeff. (min)= 7.26 (ii) 8.09 (ii)  
 02183> Unit Hyd. Tpeak (min)= 7.50 7.50  
 02184> Unit Hyd. peak (cms)= .15 .14  
 02185>  
 02186> PEAK FLOW (cms)= .40 .00 \*TOTALS\*  
 02187> TIME TO PEAK (hrs)= 1.04 1.08 1.042 (iii)  
 02188> RUNOFF VOLUME (mm)= 56.66 25.35 55.717  
 02189> TOTAL RAINFALL (mm)= 58.23 58.23 58.226  
 02190> RUNOFF COEFFICIENT = .97 .44 .957  
 02191>  
 02192> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02193> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02194> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02195> THAN THE STORAGE COEFFICIENT.  
 02196> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02197>  
 02198> 001:0007-----  
 02199> \*  
 02200> \* SUB-AREA No.4  
 02201> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02202> (ha) (cms) (hrs) (mm) (cms)  
 02203> ID1 01:010 2.07 .532 1.04 51.65 .000  
 02204> +ID2 02:020 1.54 .418 1.04 54.15 .000  
 02205>  
 02206> SUM 04:040 3.61 .950 1.04 52.72 .000  
 02207>  
 02208> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02209>  
 02210> 001:0008-----  
 02211> \*  
 02212> \* SUB-AREA No.5  
 02213> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02214> (ha) (cms) (hrs) (mm) (cms)  
 02215> ID1 03:030 1.40 .400 1.04 55.72 .000  
 02216> +ID2 04:040 3.61 .950 1.04 52.72 .000  
 02217>  
 02218> SUM 05:050 5.01 1.350 1.04 53.55 .000  
 02219>  
 02220> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02221>  
 02222> 001:0009-----  
 02223> \*  
 02224> \* SUB-AREA No.4  
 02225> | CALIB STANDHYD | Area (ha)= .89  
 02226> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 02227>  
 02228> IMPERVIOUS PERVIOUS (i)  
 02229> Surface Area (ha)= .86 .03  
 02230> Dep. Storage (mm)= 1.57 4.67  
 02231> Average Slope (%)= .93 .70  
 02232> Length (m)= 169.82 40.00  
 02233> Mannings n = .030 .250  
 02234>  
 02235> Max. eff. Inten. (mm/hr)= 144.69 44.12  
 02236> over (min) 5.00 17.50  
 02237> Storage Coeff. (min)= 5.02 (ii) 18.44 (ii)  
 02238> Unit Hyd. Tpeak (min)= 5.00 17.50  
 02239> Unit Hyd. peak (cms)= .22 .06  
 02240>  
 02241> PEAK FLOW (cms)= .30 .00 \*TOTALS\*  
 02242> TIME TO PEAK (hrs)= 1.00 1.25 .296 (iii)  
 02243> RUNOFF VOLUME (mm)= 56.66 25.35 55.717  
 02244> TOTAL RAINFALL (mm)= 58.23 58.23 58.226  
 02245> RUNOFF COEFFICIENT = .97 .44 .957  
 02246>  
 02247> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02248> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02249> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02250> THAN THE STORAGE COEFFICIENT.  
 02251> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02252>  
 02253> 001:0010-----  
 02254> \*  
 02255> \* SUB-AREA No.5  
 02256> | CALIB STANDHYD | Area (ha)= 2.66  
 02257> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 02258>  
 02259> IMPERVIOUS PERVIOUS (i)  
 02260> Surface Area (ha)= 2.58 .08  
 02261> Dep. Storage (mm)= 1.57 4.67  
 02262> Average Slope (%)= .61 1.50  
 02263> Length (m)= 207.25 20.00  
 02264> Mannings n = .030 .250  
 02265>  
 02266> Max. eff. Inten. (mm/hr)= 144.69 51.33  
 02267> over (min) 7.50 12.50  
 02268> Storage Coeff. (min)= 6.54 (ii) 13.16 (ii)  
 02269> Unit Hyd. Tpeak (min)= 7.50 12.50  
 02270> Unit Hyd. peak (cms)= .16 .09  
 02271>  
 02272> PEAK FLOW (cms)= .78 .01 \*TOTALS\*  
 02273> TIME TO PEAK (hrs)= 1.04 1.17 1.042 (iii)  
 02274> RUNOFF VOLUME (mm)= 56.66 25.35 55.717  
 02275> TOTAL RAINFALL (mm)= 58.23 58.23 58.226  
 02276> RUNOFF COEFFICIENT = .97 .44 .957  
 02277>  
 02278> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02279> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02280> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02281> THAN THE STORAGE COEFFICIENT.  
 02282> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02283>  
 02284> 001:0011-----  
 02285> \*  
 02286> \* SUB-AREA No.3  
 02287> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02288> (ha) (cms) (hrs) (mm) (cms)  
 02289> ID1 06:060 .89 .296 1.00 55.72 .000  
 02290> +ID2 07:070 2.66 .783 1.04 55.72 .000  
 02291>  
 02292> SUM 08:080 3.55 1.060 1.04 55.72 .000  
 02293>  
 02294> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02295>

02296> SUM 08:080 3.55 1.060 1.04 55.72 .000  
 02297>  
 02298> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02299>  
 02300> 001:0012-----  
 02301> \*  
 02302> \* SUB-AREA No.3  
 02303> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02304> (ha) (cms) (hrs) (mm) (cms)  
 02305> ID1 05:050 5.01 1.350 1.04 53.55 .000  
 02306> +ID2 08:080 3.55 1.060 1.04 55.72 .000  
 02307>  
 02308> SUM 09:090 8.56 2.410 1.04 54.45 .000  
 02309>  
 02310> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02311>  
 02312> 001:0013-----  
 02313> \*  
 02314> \* ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 02315> | IN:09: (090 ) |  
 02316> | OUT:10: (POND ) |  
 02317>  
 02318> OUTFLOW STORAGE | OUTFLOW STORAGE TABLE  
 02319> (cms) (ha.m.) | (cms) (ha.m.)  
 02320> .000 .0000E+00 | .593 .6251E+00  
 02321> .008 .6560E-01 | .654 .6631E+00  
 02322> .017 .1311E+00 | .797 .7391E+00  
 02323> .093 .2831E+00 | .950 .8274E+00  
 02324> .233 .3971E+00 | 1.304 .9157E+00  
 02325> .337 .4731E+00 | 1.880 .1004E+01  
 02326> .465 .5491E+00 | 2.577 .1092E+01  
 02327> .531 .5871E+00 | .000 .0000E+00  
 02328>  
 02329> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 02330> (ha) (cms) (hrs) (mm)  
 02331> INFLOW >09: (090 ) 8.56 2.410 1.042 54.451  
 02332> OUTFLOW <10: (POND ) 8.56 .189 2.056 54.449  
 02333>  
 02334> PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.838  
 02335> TIME SHIFT OF PEAK FLOW (min) = 60.83  
 02336> MAXIMUM STORAGE USED (ha.m.) = .3612E+00  
 02337>  
 02338> 001:0014-----  
 02339> \*  
 02340> \* Remaining Hawthorne Industrial Park  
 02341> \*  
 02342> \* SUB-AREA No.1  
 02343> | CALIB STANDHYD | Area (ha)= 19.90  
 02344> | 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 02345>  
 02346> IMPERVIOUS PERVIOUS (i)  
 02347> Surface Area (ha)= 14.13 5.77  
 02348> Dep. Storage (mm)= 1.57 4.67  
 02349> Average Slope (%)= .60 1.50  
 02350> Length (m)= 580.00 100.00  
 02351> Mannings n = .030 .250  
 02352>  
 02353> Max. eff. Inten. (mm/hr)= 124.54 81.98  
 02354> over (min) 12.50 27.50  
 02355> Storage Coeff. (min)= 12.93 (ii) 27.37 (ii)  
 02356> Unit Hyd. Tpeak (min)= 12.50 27.50  
 02357> Unit Hyd. peak (cms)= .09 .04  
 02358>  
 02359> PEAK FLOW (cms)= 2.16 .77 \*TOTALS\*  
 02360> TIME TO PEAK (hrs)= 1.13 1.42 1.167  
 02361> RUNOFF VOLUME (mm)= 56.66 34.22 45.437  
 02362> TOTAL RAINFALL (mm)= 58.23 58.23 58.226  
 02363> RUNOFF COEFFICIENT = .97 .59 .780  
 02364>  
 02365> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02366> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02367> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02368> THAN THE STORAGE COEFFICIENT.  
 02369> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02370>  
 02371> 001:0015-----  
 02372> \*  
 02373> \* SUB-AREA No.2  
 02374> | ADD HYD (H1P02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02375> (ha) (cms) (hrs) (mm) (cms)  
 02376> ID1 10:POND 8.56 .189 2.06 54.45 .000  
 02377> +ID2 01:H1P01 19.90 2.548 1.17 45.44 .000  
 02378>  
 02379> SUM 02:H1P02 28.46 2.622 1.17 48.15 .000  
 02380>  
 02381> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02382>  
 02383> 001:0016-----  
 02384> \*  
 02385> \* SUB-AREA No.2  
 02386> | CALIB STANDHYD | Area (ha)= 17.00  
 02387> | 03:H1P03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 02388>  
 02389> IMPERVIOUS PERVIOUS (i)  
 02390> Surface Area (ha)= 12.07 4.93  
 02391> Dep. Storage (mm)= 1.57 4.67  
 02392> Average Slope (%)= .65 1.50  
 02393> Length (m)= 450.00 100.00  
 02394> Mannings n = .030 .250  
 02395>  
 02396> Max. eff. Inten. (mm/hr)= 144.69 87.13  
 02397> over (min) 10.00 25.00  
 02398> Storage Coeff. (min)= 10.21 (ii) 24.30 (ii)  
 02399> Unit Hyd. Tpeak (min)= 10.00 25.00  
 02400> Unit Hyd. peak (cms)= .11 .05  
 02401>  
 02402> PEAK FLOW (cms)= 2.10 .71 \*TOTALS\*  
 02403> TIME TO PEAK (hrs)= 1.08 1.38 1.125  
 02404> RUNOFF VOLUME (mm)= 56.66 34.22 45.437  
 02405> TOTAL RAINFALL (mm)= 58.23 58.23 58.226  
 02406> RUNOFF COEFFICIENT = .97 .59 .780  
 02407>  
 02408> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02409> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02410> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02411> THAN THE STORAGE COEFFICIENT.  
 02412> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02413>  
 02414> 001:0017-----  
 02415> \*  
 02416> \* SUB-AREA No.3  
 02417> | CALIB STANDHYD | Area (ha)= 18.10  
 02418> | 04:H1P04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 02419>  
 02420> IMPERVIOUS PERVIOUS (i)  
 02421> Surface Area (ha)= 12.85 5.25  
 02422> Dep. Storage (mm)= 1.57 4.67  
 02423> Average Slope (%)= .50 1.50

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02431> Length (m) = 600.00 100.00
02432> Mannings n = .030 .250
02433>
02434> Max.eff.Inten.(mm/hr) = 111.10 77.71
02435> over (min) = 15.00 30.00
02436> Storage Coeff. (min) = 14.59 (ii) 29.34 (ii)
02437> Unit Hyd. Tpeak (min) = 15.00 30.00
02438> Unit Hyd. peak (cms) = .08 .04
02439>
02440> PEAK FLOW (cms) = 1.82 .67
02441> TIME TO PEAK (hrs) = 1.17 1.46
02442> RUNOFF VOLUME (mm) = 56.66 34.22
02443> TOTAL RAINFALL (mm) = 58.23 58.23
02444> RUNOFF COEFFICIENT = .97 .59
02445>
02446> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02447> CN* = 81.0 Ia = Dep. Storage (Above)
02448> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02449> THAN THE STORAGE COEFFICIENT.
02450> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02451>
02452>
02453> 001:0018-----
02454>
02455> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02456> | (ha) (cms) (hrs) (mm) (cms)
02457> | ID1 03:HIP03 17.00 2.398 1.13 45.44 .000
02458> | +ID2 04:HIP04 18.10 2.180 1.21 45.44 .000
02459> |
02460> | SUM 05:HIP05 35.10 4.439 1.13 45.44 .000
02461>
02462> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02463>
02464>
02465> 001:0019-----
02466> *
02467> *SUB-AREA No.4
02468>
02469> | DESIGN NASHYD | Area (ha) = 4.00 Curve Number (CN)=85.00
02470> | 06:Pond-B DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
02471> | U.H. Tp(hrs) = .170
02472>
02473> Unit Hyd Qpeak (cms) = .899
02474>
02475> PEAK FLOW (cms) = .459 (i)
02476> TIME TO PEAK (hrs) = 1.167
02477> RUNOFF VOLUME (mm) = 29.155
02478> TOTAL RAINFALL (mm) = 58.226
02479> RUNOFF COEFFICIENT = .501
02480>
02481> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02482>
02483>
02484> 001:0020-----
02485>
02486> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02487> | (ha) (cms) (hrs) (mm) (cms)
02488> | ID1 02:HIP02 28.46 2.622 1.17 48.15 .000
02489> | +ID2 05:HIP05 35.10 4.439 1.13 45.44 .000
02490> | +ID3 06:Pond-B 4.00 .459 1.17 29.15 .000
02491> |
02492> | SUM 07:HIP06 67.56 7.499 1.17 45.61 .000
02493>
02494> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02495>
02496>
02497> 001:0021-----
02498> * SUB-AREA NO. 5
02499>
02500> | DESIGN NASHYD | Area (ha) = 6.80 Curve Number (CN)=76.00
02501> | 10:A2 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
02502> | U.H. Tp(hrs) = .370
02503>
02504> Unit Hyd Qpeak (cms) = .702
02505>
02506> PEAK FLOW (cms) = .343 (i)
02507> TIME TO PEAK (hrs) = 1.417
02508> RUNOFF VOLUME (mm) = 21.442
02509> TOTAL RAINFALL (mm) = 58.226
02510> RUNOFF COEFFICIENT = .368
02511>
02512> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02513>
02514>
02515> 001:0022-----
02516> * SUB-AREA NO 4
02517>
02518> | DESIGN NASHYD | Area (ha) = 5.30 Curve Number (CN)=76.00
02519> | 01:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
02520> | U.H. Tp(hrs) = .804
02521>
02522> Unit Hyd Qpeak (cms) = .252
02523>
02524> PEAK FLOW (cms) = .155 (i)
02525> TIME TO PEAK (hrs) = 2.000
02526> RUNOFF VOLUME (mm) = 21.442
02527> TOTAL RAINFALL (mm) = 58.226
02528> RUNOFF COEFFICIENT = .368
02529>
02530> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02531>
02532>
02533> 001:0023-----
02534>
02535> | ADD HYD (0020 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02536> | (ha) (cms) (hrs) (mm) (cms)
02537> | ID1 07:HIP06 67.56 7.499 1.17 45.61 .000
02538> | +ID2 10:A2 6.80 .343 1.42 21.44 .000
02539> | +ID3 01:A3 5.30 .155 2.00 21.44 .000
02540> |
02541> | SUM 02:0020 79.66 7.772 1.17 41.94 .000
02542>
02543> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02544>
02545>
02546> 001:0024-----
02547> *****
02548> CALCULATION OF 3HR - 1:50 YEAR STORM EVENT *****
02549> *****
02550>
02551> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SWM\HYMO\
02552> | Rainfall dir.: V:\20983.DU\ENG\3RDSUB-1\SWM\HYMO\
02553> | TZERO = .00 hrs on 0
02554> | METOUT= 2 (output = METRIC)
02555> | NRUN = 001
02556> | NSTORM= 0
02557>
02558> 001:0002-----
02559>
02560> | CHICAGO STORM | IDF curve parameters: A=1569.580
02561> | Ptotal= 64.81 mm | B= 6.014
02562> | C= .820
02563> | used in: INTENSITY = A / (t + B)^C
02564>
02565> Duration of storm = 3.00 hrs

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02566> Storm time step = 10.00 min
02567> Time to peak ratio = .33
02568>
02569> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
02570> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
02571> .17 5.467 | 1.00 161.471 | 1.83 10.000 | 2.67 5.209
02572> .33 6.820 | 1.17 48.876 | 2.00 8.397 | 2.83 4.774
02573> .50 9.187 | 1.33 24.704 | 2.17 7.256 | 3.00 4.412
02574> .67 14.441 | 1.50 16.495 | 2.33 6.403 |
02575> .83 36.764 | 1.67 12.422 | 2.50 5.740 |
02576>
02577>
02578> 001:0003-----
02579>
02580> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWM\HYMO\ORGA.VAL
02581> | ICASEdv = 1 (read and print data)
02582> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
02583> | Horton's infiltration equation parameters:
02584> | [Fo= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
02585> | Parameters for PERVIOUS surfaces in STANDHYD:
02586> | [IaPer= 4.67 mm] [LGP=40.00 m] [MNP= .250]
02587> | Parameters for IMPERVIOUS surfaces in STANDHYD:
02588> | [IaImp= 1.57 mm] [CLi= 1.50] [MNI= .035]
02589> | Parameters used in NASHYD:
02590> | [Ia= 4.67 mm] [N= 3.00]
02591>
02592>
02593> 001:0004-----
02594> *****
02595> * ORGAWORLD FILE *
02596> *****
02597> * SUB-AREA No.1
02598>
02599> | CALIB STANDHYD | Area (ha) = 2.07
02600> | 01:010 DT= 2.50 | Total Imp(%) = 84.00 Dir. Conn.(%) = 84.00
02601>
02602> IMPERVIOUS PERVIOUS (i)
02603> Surface Area (ha) = 1.74 .33
02604> Dep. Storage (mm) = 1.57 4.67
02605> Average Slope (%) = .52 1.00
02606> Length (m) = 204.72 20.00
02607> Mannings n = .030 .250
02608>
02609> Max.eff.Inten.(mm/hr) = 161.47 62.27
02610> over (min) = 7.50 12.50
02611> Storage Coeff. (min) = 6.51 (ii) 13.44 (ii)
02612> Unit Hyd. Tpeak (min) = 7.50 12.50
02613> Unit Hyd. peak (cms) = .16 .09
02614>
02615> PEAK FLOW (cms) = .59 .03
02616> TIME TO PEAK (hrs) = 1.04 1.17
02617> RUNOFF VOLUME (mm) = 63.24 30.21
02618> TOTAL RAINFALL (mm) = 64.81 64.81
02619> RUNOFF COEFFICIENT = .98 .47
02620>
02621> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02622> CN* = 81.0 Ia = Dep. Storage (Above)
02623> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02624> THAN THE STORAGE COEFFICIENT.
02625> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02626>
02627>
02628> 001:0005-----
02629> *
02630> * SUB-AREA No.2
02631>
02632> | CALIB STANDHYD | Area (ha) = 1.54
02633> | 02:020 DT= 2.50 | Total Imp(%) = 92.00 Dir. Conn.(%) = 92.00
02634>
02635> IMPERVIOUS PERVIOUS (i)
02636> Surface Area (ha) = 1.42 .12
02637> Dep. Storage (mm) = 1.57 4.67
02638> Average Slope (%) = .50 1.00
02639> Length (m) = 244.34 5.00
02640> Mannings n = .030 .030
02641>
02642> Max.eff.Inten.(mm/hr) = 161.47 78.73
02643> over (min) = 7.50 7.50
02644> Storage Coeff. (min) = 7.33 (ii) 8.10 (ii)
02645> Unit Hyd. Tpeak (min) = 7.50 7.50
02646> Unit Hyd. peak (cms) = .15 .14
02647>
02648> PEAK FLOW (cms) = .46 .02
02649> TIME TO PEAK (hrs) = 1.04 1.08
02650> RUNOFF VOLUME (mm) = 63.24 30.21
02651> TOTAL RAINFALL (mm) = 64.81 64.81
02652> RUNOFF COEFFICIENT = .98 .47
02653>
02654> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02655> CN* = 81.0 Ia = Dep. Storage (Above)
02656> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02657> THAN THE STORAGE COEFFICIENT.
02658> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02659>
02660>
02661> 001:0006-----
02662> *
02663> * SUB-AREA No.3
02664>
02665> | CALIB STANDHYD | Area (ha) = 1.40
02666> | 03:030 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00
02667>
02668> IMPERVIOUS PERVIOUS (i)
02669> Surface Area (ha) = 1.36 .04
02670> Dep. Storage (mm) = 1.57 4.67
02671> Average Slope (%) = .51 1.00
02672> Length (m) = 225.63 5.00
02673> Mannings n = .030 .030
02674>
02675> Max.eff.Inten.(mm/hr) = 161.47 78.73
02676> over (min) = 7.50 7.50
02677> Storage Coeff. (min) = 6.95 (ii) 7.72 (ii)
02678> Unit Hyd. Tpeak (min) = 7.50 7.50
02679> Unit Hyd. peak (cms) = .16 .15
02680>
02681> PEAK FLOW (cms) = .45 .01
02682> TIME TO PEAK (hrs) = 1.04 1.08
02683> RUNOFF VOLUME (mm) = 63.24 30.21
02684> TOTAL RAINFALL (mm) = 64.81 64.81
02685> RUNOFF COEFFICIENT = .98 .47
02686>
02687> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02688> CN* = 81.0 Ia = Dep. Storage (Above)
02689> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02690> THAN THE STORAGE COEFFICIENT.
02691> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02692>
02693>
02694> 001:0007-----
02695>
02696> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02697> | (ha) (cms) (hrs) (mm) (cms)
02698> | ID1 01:010 2.07 .609 1.04 57.95 .000
02699> | +ID2 02:020 1.54 .475 1.04 60.59 .000
02700>

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02701> SUM 04:040 3.61 1.084 1.04 59.08 .000
02702>
02703> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02704>
02705>
02706> 001:0008-----
02707>
02708> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02709> | 06:060 DT= 2.50 | (ha) (cms) (hrs) (mm) (cms)
02710> ID1 03:030 1.40 .454 1.04 62.25 .000
02711> +ID2 04:040 3.61 1.084 1.04 59.08 .000
02712>
02713> SUM 05:050 5.01 1.538 1.04 59.96 .000
02714>
02715> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02716>
02717>
02718> 001:0009-----
02719> * SUB-AREA No.4
02720>
02721> | CALIB STANDHYD | Area (ha)= .89
02722> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02723>
02724> IMPERVIOUS PERVIOUS (i)
02725> Surface Area (ha)= .86 .03
02726> Dep. Storage (mm)= 1.57 4.67
02727> Average Slope (%)= .93 .70
02728> Length (m)= 164.82 40.00
02729> Mannings n = .030 .250
02730>
02731> Max.eff.Inten.(mm/hr)= 161.47 53.28
02732> over (min)= 5.00 17.50
02733> Storage Coeff. (min)= 4.80 (ii) 17.24 (ii)
02734> Unit Hyd. Tpeak (min)= 5.00 17.50
02735> Unit Hyd. peak (cms)= .23 .07
02736>
02737> PEAK FLOW (cms)= .33 .00 *TOTALS*
02738> TIME TO PEAK (hrs)= 1.00 1.25 .335 (iii)
02739> RUNOFF VOLUME (mm)= 63.24 30.21 62.245
02740> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
02741> RUNOFF COEFFICIENT = .98 .47 .960
02742>
02743> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02744> CN* = 81.0 Ia = Dep. Storage (Above)
02745> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02746> THAN THE STORAGE COEFFICIENT.
02747> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02748>
02749>
02750> 001:0010-----
02751> * SUB-AREA No.5
02752>
02753> | CALIB STANDHYD | Area (ha)= 2.66
02754> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02755>
02756> IMPERVIOUS PERVIOUS (i)
02757> Surface Area (ha)= 2.58 .08
02758> Dep. Storage (mm)= 1.57 4.67
02759> Average Slope (%)= .61 1.50
02760> Length (m)= 207.25 20.00
02761> Mannings n = .030 .250
02762>
02763> Max.eff.Inten.(mm/hr)= 161.47 62.27
02764> over (min)= 7.50 12.50
02765> Storage Coeff. (min)= 6.26 (ii) 12.39 (ii)
02766> Unit Hyd. Tpeak (min)= 7.50 12.50
02767> Unit Hyd. peak (cms)= .17 .09
02768>
02769> PEAK FLOW (cms)= .88 .01 *TOTALS*
02770> TIME TO PEAK (hrs)= 1.04 1.17 .886 (iii)
02771> RUNOFF VOLUME (mm)= 63.24 30.21 62.245
02772> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
02773> RUNOFF COEFFICIENT = .98 .47 .960
02774>
02775> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02776> CN* = 81.0 Ia = Dep. Storage (Above)
02777> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02778> THAN THE STORAGE COEFFICIENT.
02779> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02780>
02781>
02782> 001:0011-----
02783>
02784> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02785> | 06:060 DT= 2.50 | (ha) (cms) (hrs) (mm) (cms)
02786> ID1 06:060 .89 .335 1.00 62.25 .000
02787> +ID2 07:070 2.66 .886 1.04 62.25 .000
02788>
02789> SUM 08:080 3.55 1.197 1.04 62.25 .000
02790>
02791> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02792>
02793>
02794> 001:0012-----
02795>
02796> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02797> | 06:060 DT= 2.50 | (ha) (cms) (hrs) (mm) (cms)
02798> ID1 05:050 5.01 1.538 1.04 59.96 .000
02799> +ID2 08:080 3.55 1.197 1.04 62.25 .000
02800>
02801> SUM 09:090 8.56 2.735 1.04 60.91 .000
02802>
02803> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02804>
02805>
02806> 001:0013-----
02807>
02808> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
02809> | IN>09: (090 ) |
02810> | OUT<10: (POND ) |
02811>
02812> ===== OUTFLOW STORAGE TABLE =====
02813> OUTFLOW STORAGE | OUTFLOW STORAGE
02814> (cms) (ha.m.) | (cms) (ha.m.)
02815> .000 .0000E+00 | .523 .6251E+00
02816> .008 .6560E-01 | .654 .6631E+00
02817> .017 .1311E+00 | .797 .7391E+00
02818> .093 .2831E+00 | .950 .8274E+00
02819> .233 .3971E+00 | 1.304 .9157E+00
02820> .337 .4731E+00 | 1.880 .1004E+01
02821> .465 .5491E+00 | 2.577 .1092E+01
02822> .531 .5871E+00 | .000 .0000E+00
02823>
02824> ROUTING RESULTS
02825> AREA QPEAK TPEAK R.V.
02826> (ha) (cms) (hrs) (mm)
02827> INFLOW>09: (090 ) 8.56 2.735 1.042 60.910
02828> OUTFLOW<10: (POND ) 8.56 .233 1.944 60.908
02829>
02830> PEAK FLOW REDUCTION [Qout/Qin] (%)= 8.503
02831> TIME SHIFT OF PEAK FLOW (min)= 54.17
02832> MAXIMUM STORAGE USED (ha.m.)=.3967E+00
02833>
02834> 001:0014-----
02835>

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02836> * Remaining Hawthorne Industrial Park *
02837> *****
02838> * SUB-AREA No.1
02839>
02840>
02841> | CALIB STANDHYD | Area (ha)= 19.90
02842> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
02843>
02844> IMPERVIOUS PERVIOUS (i)
02845> Surface Area (ha)= 14.13 5.77
02846> Dep. Storage (mm)= 1.57 4.67
02847> Average Slope (%)= .60 1.50
02848> Length (m)= 580.00 100.00
02849> Mannings n = .030 .250
02850>
02851> Max.eff.Inten.(mm/hr)= 138.95 102.13
02852> over (min)= 12.50 25.00
02853> Storage Coeff. (min)= 12.38 (ii) 25.60 (ii)
02854> Unit Hyd. Tpeak (min)= 12.50 25.00
02855> Unit Hyd. peak (cms)= .09 .04
02856>
02857> PEAK FLOW (cms)= 2.46 .95 *TOTALS*
02858> TIME TO PEAK (hrs)= 1.13 1.38 1.167
02859> RUNOFF VOLUME (mm)= 63.24 39.90 51.566
02860> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
02861> RUNOFF COEFFICIENT = .98 .62 .796
02862>
02863> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02864> CN* = 81.0 Ia = Dep. Storage (Above)
02865> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02866> THAN THE STORAGE COEFFICIENT.
02867> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02868>
02869> 001:0015-----
02870>
02871> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02872> | 01:HIP02 DT= 2.50 | (ha) (cms) (hrs) (mm) (cms)
02873> ID1 10:POND 8.56 .233 1.94 60.91 .000
02874> +ID2 01:HIP01 19.90 3.001 1.17 51.57 .000
02875>
02876> SUM 02:HIP02 28.46 3.092 1.17 54.37 .000
02877>
02878> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02879>
02880>
02881> 001:0016-----
02882> * SUB-AREA No.2
02883>
02884> | CALIB STANDHYD | Area (ha)= 17.00
02885> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
02886>
02887> IMPERVIOUS PERVIOUS (i)
02888> Surface Area (ha)= 12.07 4.93
02889> Dep. Storage (mm)= 1.57 4.67
02890> Average Slope (%)= .65 1.50
02891> Length (m)= 450.00 100.00
02892> Mannings n = .030 .250
02893>
02894> Max.eff.Inten.(mm/hr)= 161.47 109.61
02895> over (min)= 10.00 22.50
02896> Storage Coeff. (min)= 9.77 (ii) 22.63 (ii)
02897> Unit Hyd. Tpeak (min)= 10.00 22.50
02898> Unit Hyd. peak (cms)= .11 .05
02899>
02900> PEAK FLOW (cms)= 2.38 .88 *TOTALS*
02901> TIME TO PEAK (hrs)= 1.08 1.33 1.125
02902> RUNOFF VOLUME (mm)= 63.24 39.90 51.566
02903> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
02904> RUNOFF COEFFICIENT = .98 .62 .796
02905>
02906> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02907> CN* = 81.0 Ia = Dep. Storage (Above)
02908> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02909> THAN THE STORAGE COEFFICIENT.
02910> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02911>
02912> 001:0017-----
02913>
02914> * SUB-AREA No.3
02915>
02916> | CALIB STANDHYD | Area (ha)= 18.10
02917> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
02918>
02919> IMPERVIOUS PERVIOUS (i)
02920> Surface Area (ha)= 12.85 5.25
02921> Dep. Storage (mm)= 1.57 4.67
02922> Average Slope (%)= .59 1.50
02923> Length (m)= 600.00 100.00
02924> Mannings n = .030 .250
02925>
02926> Max.eff.Inten.(mm/hr)= 138.95 96.02
02927> over (min)= 13.34 (ii) 26.90 (ii)
02928> Storage Coeff. (min)= 12.50 27.50
02929> Unit Hyd. Tpeak (min)= .09 .04
02930> Unit Hyd. peak (cms)= .216 .83 *TOTALS*
02931> PEAK FLOW (cms)= 1.13 1.42 2.596 (iii)
02932> TIME TO PEAK (hrs)= 63.24 39.90 51.566
02933> RUNOFF VOLUME (mm)= 64.81 64.81 64.806
02934> TOTAL RAINFALL (mm)= .98 .62 .796
02935> RUNOFF COEFFICIENT =
02936>
02937> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02938> CN* = 81.0 Ia = Dep. Storage (Above)
02939> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02940> THAN THE STORAGE COEFFICIENT.
02941> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02942>
02943> 001:0018-----
02944>
02945> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02946> | 06:Pond-B DT= 2.50 | (ha) (cms) (hrs) (mm) (cms)
02947> ID1 03:HIP03 17.00 2.819 1.13 51.57 .000
02948> +ID2 04:HIP04 18.10 2.596 1.17 51.57 .000
02949>
02950> SUM 05:HIP05 35.10 5.372 1.13 51.57 .000
02951>
02952> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02953>
02954>
02955> 001:0019-----
02956> * SUB-AREA No.4
02957>
02958> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
02959> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
02960> U.H. Tp(hrs)= .170
02961>
02962> Unit Hyd Tpeak (cms)= .899
02963>
02964> PEAK FLOW (cms)= .551 (i)
02965>

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02971> TIME TO PEAK (hrs)= 1.125  
 02972> RUNOFF VOLUME (mm)= 34.455  
 02973> TOTAL RAINFALL (mm)= 64.806  
 02974> RUNOFF COEFFICIENT = .532

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02979> 001:0020-----  
 02980> | ADD HYD (HIP06) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 02981> | | | (ha) (cms) (hrs) (mm) (cms)  
 02982> ID1 02:HIP02 28.46 3.092 1.17 54.37 .000  
 02983> +ID2 05:HIP05 35.10 5.372 1.13 51.57 .000  
 02984> +ID3 06:Pond-B 4.00 .551 1.13 34.45 .000  
 02985> =====  
 02986> SUM 07:HIP06 67.56 8.958 1.13 51.73 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02992> 001:0021-----  
 02993> \* SUB-AREA NO. 5  
 02994> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00  
 02995> | 10:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02996> | U.H. Tp(hrs)= .370

Unit Hyd Qpeak (cms)= .702

PEAK FLOW (cms)= .417 (i)

TIME TO PEAK (hrs)= 1.417

RUNOFF VOLUME (mm)= 25.767

TOTAL RAINFALL (mm)= 64.806

RUNOFF COEFFICIENT = .398

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

03009> 001:0022-----  
 03010> \* SUB-AREA NO. 4  
 03011> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00  
 03012> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 03013> | U.H. Tp(hrs)= .804

Unit Hyd Qpeak (cms)= .252

PEAK FLOW (cms)= .188 (i)

TIME TO PEAK (hrs)= 2.000

RUNOFF VOLUME (mm)= 25.767

TOTAL RAINFALL (mm)= 64.806

RUNOFF COEFFICIENT = .398

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

03025> 001:0023-----  
 03026> | ADD HYD (0020) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 03027> | | | (ha) (cms) (hrs) (mm) (cms)  
 03028> ID1 07:HIP06 67.56 8.958 1.13 51.73 .000  
 03029> +ID2 10:A2 6.80 .417 1.42 25.77 .000  
 03030> +ID3 01:A3 5.30 .188 2.00 25.77 .000  
 03031> =====  
 03032> SUM 02:0020 79.66 9.240 1.17 47.79 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

03041> 001:0024-----  
 03042> \*\*\*\*\*  
 03043> \* CALCULATION OF 3HR - 1:00 YEAR STORM EVENT \*  
 03044> \*\*\*\*\*  
 03045> | START | Project dir.: V:\20983.DU\ENG\3RDSUB-1\SWHMYO\  
 03046> | | Rainfall dir.: V:\20983.DU\ENG\3RDSUB-1\SWHMYO\  
 03047> | TZERO = .00 hrs on 0  
 03048> | METOUT= 2 (output = METRIC)  
 03049> | NRUN = 001  
 03050> | NSTORM= 0

03052> 001:0002-----  
 03053> | CHICAGO STORM | IDF curve parameters: A=1735.688  
 03054> | Ptotal= 71.66 mm | B= 6.014  
 03055> | | C= .820  
 03056> | | used in: INTENSITY = A / (t + B)^C  
 03057> | | Duration of storm = 3.00 hrs  
 03058> | | Storm time step = 10.00 min  
 03059> | | Time to peak ratio = .33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
.17	6.046	1.00	178.559	1.83	11.059
.33	7.542	1.17	54.049	2.00	9.285
.50	10.159	1.33	27.319	2.17	8.024
.67	15.969	1.50	18.240	2.33	7.080
.83	40.655	1.67	13.737	2.50	6.347

03072> 001:0003-----  
 03073> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\3RDSUB-1\SWHMYO\ORGA.VAL  
 03074> | ICASEdv = 1 (read and print data)  
 03075> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---  
 03076> | Horton's infiltration equation parameters:  
 03077> | [Fo= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]  
 03078> | Parameters for Pervious surfaces in STANDHYD:  
 03079> | [IAper= 4.67 mm] [LGP=40.00 mm] [MNF= .250]  
 03080> | Parameters for IMPERVIOUS surfaces in STANDHYD:  
 03081> | [IImpe= 1.57 mm] [CLi= 1.50] [MNI= .035]  
 03082> | Parameters used in NASHYD:  
 03083> | [Ia= 4.67 mm] [N= 3.00]

03087> 001:0004-----  
 03088> \*\*\*\*\*  
 03089> \* ORGAWORD FILE \*  
 03090> \*\*\*\*\*  
 03091> \* SUB-AREA No.1

03092> 001:0005-----  
 03093> | CALIB STANDHYD | Area (ha)= 2.07  
 03094> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00  
 03095> | | IMPERVIOUS PERVIOUS (i)  
 03096> | Surface Area (ha)= 1.74 .33  
 03097> | Dep. Storage (mm)= 1.57 4.67  
 03098> | Average Slope (%)= 1.52 1.00  
 03099> | Length (m)= 204.72 20.00  
 03100> | Mannings n = .030 .250

03103> Max. eff. Inten. (mm/hr)= 178.56 74.05  
 03104> over (min) 7.50 12.50

03106> Storage Coeff. (min)= 6.26 (ii) 12.72 (ii)  
 03107> Unit Hyd. Tpeak (min)= 7.50 12.50  
 03108> Unit Hyd. peak (cms)= .17 .09  
 03109> \*TOTALS\*  
 03110> PEAK FLOW (cms)= .66 .04  
 03111> TIME TO PEAK (hrs)= 1.04 1.17  
 03112> RUNOFF VOLUME (mm)= 70.09 35.46  
 03113> TOTAL RAINFALL (mm)= 71.66 71.66  
 03114> RUNOFF COEFFICIENT = .98 .49  
 03115> .901

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

03123> 001:0005-----  
 03124> \* SUB-AREA No.2

03125> | CALIB STANDHYD | Area (ha)= 1.54  
 03126> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00  
 03127> | | IMPERVIOUS PERVIOUS (i)  
 03128> | Surface Area (ha)= 1.42 .12  
 03129> | Dep. Storage (mm)= 1.57 4.67  
 03130> | Average Slope (%)= .50 1.00  
 03131> | Length (m)= 244.34 5.00  
 03132> | Mannings n = .030 .030

03133> Max. eff. Inten. (mm/hr)= 178.56 93.23  
 03134> over (min) 7.50 7.50  
 03135> Storage Coeff. (min)= 7.04 (ii) 7.76 (ii)  
 03136> Unit Hyd. Tpeak (min)= 7.50 7.50  
 03137> Unit Hyd. peak (cms)= .16 .15  
 03138> \*TOTALS\*  
 03139> PEAK FLOW (cms)= .51 .02  
 03140> TIME TO PEAK (hrs)= 1.04 1.08  
 03141> RUNOFF VOLUME (mm)= 70.09 35.46  
 03142> TOTAL RAINFALL (mm)= 71.66 71.66  
 03143> RUNOFF COEFFICIENT = .98 .49  
 03144> .939

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

03156> 001:0006-----  
 03157> \* SUB-AREA No.3

03158> | CALIB STANDHYD | Area (ha)= 1.40  
 03159> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 03160> | | IMPERVIOUS PERVIOUS (i)  
 03161> | Surface Area (ha)= 1.36 .04  
 03162> | Dep. Storage (mm)= 1.57 4.67  
 03163> | Average Slope (%)= .51 1.00  
 03164> | Length (m)= 225.63 5.00  
 03165> | Mannings n = .030 .030

03166> Max. eff. Inten. (mm/hr)= 178.56 93.23  
 03167> over (min) 7.50 7.50  
 03168> Storage Coeff. (min)= 6.67 (ii) 7.39 (ii)  
 03169> Unit Hyd. Tpeak (min)= 7.50 7.50  
 03170> Unit Hyd. peak (cms)= .16 .15  
 03171> \*TOTALS\*  
 03172> PEAK FLOW (cms)= .50 .01  
 03173> TIME TO PEAK (hrs)= 1.04 1.08  
 03174> RUNOFF VOLUME (mm)= 70.09 35.46  
 03175> TOTAL RAINFALL (mm)= 71.66 71.66  
 03176> RUNOFF COEFFICIENT = .98 .49  
 03177> .964

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

03187> 001:0007-----  
 03188> | ADD HYD (040) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 03189> | | | (ha) (cms) (hrs) (mm) (cms)  
 03190> ID1 01:010 2.07 .685 1.04 64.55 .000  
 03191> +ID2 02:020 1.54 .534 1.04 67.32 .000  
 03192> =====  
 03193> SUM 04:040 3.61 1.220 1.04 65.74 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

03198> 001:0008-----  
 03199> | ADD HYD (050) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 03200> | | | (ha) (cms) (hrs) (mm) (cms)  
 03201> ID1 03:030 1.40 .509 1.04 69.06 .000  
 03202> +ID2 04:040 3.61 1.220 1.04 65.74 .000  
 03203> =====  
 03204> SUM 05:050 5.01 1.729 1.04 66.66 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

03213> 001:0009-----  
 03214> \* SUB-AREA No.4

03215> | CALIB STANDHYD | Area (ha)= .89  
 03216> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 03217> | | IMPERVIOUS PERVIOUS (i)  
 03218> | Surface Area (ha)= .86 .03  
 03219> | Dep. Storage (mm)= 1.57 4.67  
 03220> | Average Slope (%)= .93 .70  
 03221> | Length (m)= 164.82 40.00  
 03222> | Mannings n = .030 .250

03223> Max. eff. Inten. (mm/hr)= 178.56 67.61  
 03224> over (min) 5.00 15.00  
 03225> Storage Coeff. (min)= 4.62 (ii) 15.92 (ii)  
 03226> Unit Hyd. Tpeak (min)= 5.00 15.00  
 03227> Unit Hyd. peak (cms)= .24 .07  
 03228> \*TOTALS\*  
 03229> PEAK FLOW (cms)= .37 .00  
 03230> TIME TO PEAK (hrs)= 1.00 1.21  
 03231> RUNOFF VOLUME (mm)= 70.09 35.46  
 03232> TOTAL RAINFALL (mm)= 71.66 71.66  
 03233> RUNOFF COEFFICIENT = .98 .49  
 03234> .964

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)

```
03511>
03512> Unit Hyd Qpeak (cms) = .252
03513>
03514> PEAK FLOW (cms) = .223 (i)
03515> TIME TO PEAK (hrs) = 1.958
03516> RUNOFF VOLUME (mm) = 30.490
03517> TOTAL RAINFALL (mm) = 71.665
03518> RUNOFF COEFFICIENT = .425
03519>
03520> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03521>
03522>
03523> 001:0023-----
03524>
03525> | ADD HYD (0020 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03526> |-----|-----|-----|-----|-----|-----|
03527> | ID1 07:H1P06 67.56 10.299 1.13 58.18 .000
03528> | +ID2 10:A2 6.80 .497 1.42 30.49 .000
03529> | +ID3 01:A3 5.30 .223 1.96 30.49 .000
03530> |-----|-----|-----|-----|-----|-----|
03531> | SUM 02:0020 79.66 10.662 1.17 53.97 .000
03532>
03533> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03534>
03535>
03536> 001:0024-----
03537> FINISH
03538>
03539> *****
03540> WARNINGS / ERRORS / NOTES
03541>
03542> Simulation ended on 2009-04-21 at 10:30:17
03543>
03544>
03545>
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00001> 2      Metric units
00002> *****
00003> *# Project Name : Hawthorne Industrial Park      Project Number: [20983] *
00004> *# Date : January, 2009
00005> *# Revised : W/A
00006> *# Developed by : Mark Buchanan, E.I.T.
00007> *# Reviewed by : Guy Forget, P.Eng.
00008> *# Company : J.L. Richards & Associates Limited
00009> *# License : 4418403
00010> *****
00011> *
00012> *
00013> *****
00014> *# FILENAME: V:\20983.DU\ENG\SWMHYMO\20983PST.DAT
00015> *# FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00016> *# OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00017> *****
00018> *
00019> *****
00020> *# SWMHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE
00021> *# PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00022> *****
00023> *
00024> *****
00025> *# HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00026> *# FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00027> *****
00028> *
00029> *****
00030> *# POST-DEVELOPMENT UNCONTROLLED CONDITIONS
00031> *****
00032> *
00033> *****
00034> *# CALCULATION OF 4 HR 25 MM STORM EVENT
00035> *****
00036> *
00037> START      TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00038> *# [ ] <-storm filename, one per line for NSTORM time
00039> READ STORM  STORM_FILENAME=[\"4HR25-15.STM\"]
00040> *#
00041> DEFAULT VALUES
00042> ICASEdef=[1], read and print values
00043> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\\"ORGA.VAL\"]
00044> *#
00045> *****
00046> *# ORGAWORLD FILE
00047> *****
00048> *
00049> * SUB-AREA No.1
00050> *
00051> CALIB STANDHYD ID=[ 1 ], NHYD=[\"010\"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00052> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00053> SCS curve number CN=[81],
00054> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00055> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00056> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00057> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00058> RAINFALL=[ , , , ] (mm/hr), END=-1
00059> *#
00060> *
00061> * SUB-AREA No.2
00062> *
00063> CALIB STANDHYD ID=[ 2 ], NHYD=[\"020\"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00064> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00065> SCS curve number CN=[81],
00066> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00067> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00068> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00069> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00070> RAINFALL=[ , , , ] (mm/hr), END=-1
00071> *#
00072> *
00073> * SUB-AREA No.3
00074> *
00075> CALIB STANDHYD ID=[ 3 ], NHYD=[\"030\"], DT=[2.5] (min), AREA=[ 1.4 ] (ha),
00076> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00077> SCS curve number CN=[81],
00078> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00079> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00080> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00081> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
00082> RAINFALL=[ , , , ] (mm/hr), END=-1
00083> *#
00084> ADD HYD IDsum=[4], NHYD=[\"040\"], IDs to add=[1+2]
00085> *#
00086> ADD HYD IDsum=[5], NHYD=[\"050\"], IDs to add=[3+4]
00087> *#
00088> *
00089> * SUB-AREA No.4
00090> *
00091> CALIB STANDHYD ID=[6], NHYD=[\"060\"], DT=[2.5] (min), AREA=[0.89] (ha),
00092> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00093> SCS curve number CN=[81],
00094> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00095> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min),
00096> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00097> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00098> RAINFALL=[ , , , ] (mm/hr), END=-1
00099> *#
00100> *
00101> * SUB-AREA No.5
00102> *
00103> CALIB STANDHYD ID=[ 7 ], NHYD=[\"070\"], DT=[2.5] (min), AREA=[2.66] (ha),
00104> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00105> SCS curve number CN=[81],
00106> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00107> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (m)
00108> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00109> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00110> RAINFALL=[ , , , ] (mm/hr), END=-1
00111> *#
00112> ADD HYD IDsum=[8], NHYD=[\"080\"], IDs to add=[6+7]
00113> *#
00114> ADD HYD IDsum=[9], NHYD=[\"090\"], IDs to add=[5+8]
00115> *#
00116> *
00117> ROUTE RESERVOIR IDout=[10], NHYD=[\"POND\"], IDin=[9],
00118> RDT=[1.0] (min),
00119> TABLE of ( OUTFLOW-STORAGE ) values
00120> (cms) (ha-s)
00121> [ 0.000, 0.0000 ]
00122> [ 0.008, 0.0656 ]
00123> [ 0.017, 0.1311 ]
00124> [ 0.093, 0.2831 ]
00125> [ 0.233, 0.3971 ]
00126> [ 0.337, 0.4731 ]
00127> [ 0.465, 0.5491 ]
00128> [ 0.531, 0.5871 ]
00129> [ 0.593, 0.6251 ]
00130> [ 0.654, 0.6631 ]
00131> [ 0.797, 0.7391 ]
00132> [ 0.950, 0.8274 ]
00133> [ 1.304, 0.9157 ]
00134> [ 1.880, 1.0040 ]
00135> [ 2.577, 1.0923 ]
00136> [ -1 , -1 ] (max twenty pts)
00137> *****
00138> *****
00139> *# Remaining Hawthorne Industrial Park
00140> *****
00141> *
00142> * SUB-AREA No.1
00143> *
00144> CALIB STANDHYD ID=[ 1 ], NHYD=[\"HIP01\"], DT=[2.5] (min), AREA=[19.9] (ha),
00145> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00146> SCS curve number CN=[81],
00147> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00148> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00149> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00150> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00151> RAINFALL=[ , , , ] (mm/hr), END=-1
00152> *#
00153> ADD HYD IDsum=[ 2 ], NHYD=[\"HIP02\"], IDs to add=[10+1]
00154> *#
00155> *
00156> * SUB-AREA No.2
00157> *
00158> CALIB STANDHYD ID=[ 3 ], NHYD=[\"HIP03\"], DT=[2.5] (min), AREA=[17] (ha),
00159> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00160> SCS curve number CN=[81],
00161> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00162> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00163> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00164> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00165> RAINFALL=[ , , , ] (mm/hr), END=-1
00166> *#
00167> *
00168> * SUB-AREA No.3
00169> *
00170> CALIB STANDHYD ID=[ 4 ], NHYD=[\"HIP04\"], DT=[2.5] (min), AREA=[15.6] (ha),
00171> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00172> SCS curve number CN=[81],
00173> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00174> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00175> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00176> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
00177> RAINFALL=[ , , , ] (mm/hr), END=-1
00178> *#
00179> ADD HYD IDsum=[ 5 ], NHYD=[\"HIP05\"], IDs to add=[3+4]
00180> *#
00181> ADD HYD IDsum=[ 6 ], NHYD=[\"HIP06\"], IDs to add=[5+2]
00182> *#
00183> *
00184> * SUB-AREA No.4
00185> *
00186> CALIB STANDHYD ID=[ 7 ], NHYD=[\"HIP07\"], DT=[2.5] (min), AREA=[12.2] (ha),
00187> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00188> SCS curve number CN=[81],
00189> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00190> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00191> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.7] (%),
00192> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
00193> RAINFALL=[ , , , ] (mm/hr), END=-1
00194> *#
00195> *
00196> *
00197> * SUB-AREA No.5
00198> *
00199> DESIGN NASHYD ID=[ 8 ], NHYD=[\"Pond-Block\"], DT=[2.5] min, AREA=[4.0] (ha),
00200> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00201> RAINFALL=[ , , , ] (mm/hr), END=-1
00202> *#
00203> *
00204> *
00205> ADD HYD IDsum=[ 9 ], NHYD=[\"HIP08\"], IDs to add=[6+7+8]
00206> *#
00207> *
00208> * SUB-AREA No. 6
00209> *
00210> DESIGN NASHYD ID = [1], NHYD=[\"A3\"], DT=[2.5] min, AREA=[2.7] (ha),
00211> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
00212> RAINFALL=[ , , , ] (mm/hr), END=-1
00213> *#
00214> *
00215> ADD HYD IDsum=[2], NHYD=[\"Ultimate\"], IDs to add=[9+1]
00216> *#
00217> *
00218> *****
00219> *****
00220> *****
00221> *****
00222> *****
00223> START      TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00224> *# [ ] <-storm filename, one per line for NSTORM time
00225> *#
00226> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPAAT=[0.333], CSDT=[10.0] (min)
00227> ICASEcs=[1],
00228> A=[732.951], B=[6.199], and C=[0.810]
00229> *#
00230> DEFAULT VALUES
00231> ICASEdef=[1], read and print values
00232> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\\"ORGA.VAL\"]
00233> *#
00234> *****
00235> *# ORGAWORLD FILE
00236> *****
00237> *
00238> * SUB-AREA No.1
00239> *
00240> CALIB STANDHYD ID=[ 1 ], NHYD=[\"010\"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00241> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00242> SCS curve number CN=[81],
00243> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00244> LGP=[20] (m), MNP=[ 0.25 ], SCP=[0.0] (m)
00245> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00246> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00247> RAINFALL=[ , , , ] (mm/hr), END=-1
00248> *#
00249> *
00250> * SUB-AREA No.2
00251> *
00252> CALIB STANDHYD ID=[ 2 ], NHYD=[\"020\"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00253> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00254> SCS curve number CN=[81],
00255> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00256> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00257> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00258> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00259> RAINFALL=[ , , , ] (mm/hr), END=-1
00260> *#
00261> *
00262> * SUB-AREA No.3
00263> *
00264> CALIB STANDHYD ID=[ 3 ], NHYD=[\"030\"], DT=[2.5] (min), AREA=[ 1.4 ] (ha),
00265> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00266> SCS curve number CN=[81],
00267> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00268> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00269> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[ 0.51 ] (%),
00270> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]

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00271> RAINFALL=[ , , , ] (mm/hr) , END=-1
00272> *%-----
00273> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00274> *%-----
00275> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00276> *%-----
00277> *
00278> * SUB-AREA No.4
00279> *
00280> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00281> RAINFALL=[ , , , ] (mm/hr) , END=-1
00282> *%-----
00283> *
00284> * SUB-AREA No.5
00285> *
00286> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00287> RAINFALL=[ , , , ] (mm/hr) , END=-1
00288> *%-----
00289> *
00290> * SUB-AREA No.6
00291> *
00292> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00293> RAINFALL=[ , , , ] (mm/hr) , END=-1
00294> *%-----
00295> *
00296> * SUB-AREA No.7
00297> *
00298> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00299> RAINFALL=[ , , , ] (mm/hr) , END=-1
00300> *%-----
00301> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00302> *%-----
00303> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00304> *%-----
00305> *
00306> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
RDT=[1.0] (min),
TABLE of ( OUTFLOW-STORAGE ) values
(cms) - (ha-m)
[ 0.000, 0.0000]
[ 0.008, 0.0656]
[ 0.017, 0.1311]
[ 0.093, 0.2831]
[ 0.233, 0.3971]
[ 0.337, 0.4731]
[ 0.465, 0.5491]
[ 0.531, 0.5871]
[ 0.593, 0.6251]
[ 0.654, 0.6631]
[ 0.797, 0.7391]
[ 0.950, 0.8274]
[ 1.304, 0.9157]
[ 1.880, 1.0040]
[ 2.577, 1.0923]
[ -1, -1 ] (max twenty pts)
00307> *%-----
00308> *
00309> *
00310> *
00311> *
00312> *
00313> *
00314> *
00315> *
00316> *
00317> *
00318> *
00319> *
00320> *
00321> *
00322> *
00323> *
00324> *
00325> *
00326> *
00327> *
00328> * Remaining Hawthorne Industrial Park *
00329> *%-----
00330> *
00331> * SUB-AREA No.1
00332> *
00333> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00334> RAINFALL=[ , , , ] (mm/hr) , END=-1
00335> *%-----
00336> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00337> *%-----
00338> *
00339> * SUB-AREA No.2
00340> *
00341> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00342> RAINFALL=[ , , , ] (mm/hr) , END=-1
00343> *%-----
00344> *
00345> * SUB-AREA No.3
00346> *
00347> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00348> RAINFALL=[ , , , ] (mm/hr) , END=-1
00349> *%-----
00350> *
00351> * SUB-AREA No.4
00352> *
00353> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00354> RAINFALL=[ , , , ] (mm/hr) , END=-1
00355> *%-----
00356> *
00357> * SUB-AREA No.5
00358> *
00359> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00360> RAINFALL=[ , , , ] (mm/hr) , END=-1
00361> *%-----
00362> *
00363> * SUB-AREA No.6
00364> *
00365> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00366> RAINFALL=[ , , , ] (mm/hr) , END=-1
00367> *%-----
00368> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00369> *%-----
00370> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00371> *%-----
00372> *
00373> * SUB-AREA No.7
00374> *
00375> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00376> RAINFALL=[ , , , ] (mm/hr) , END=-1
00377> *%-----
00378> *
00379> * SUB-AREA No.8
00380> *
00381> CALIB STANDHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] (min), AREA=[4.0] (ha),
DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] (hrs),
RAINFALL=[ , , , ] (mm/hr) , END=-1
00382> *%-----
00383> *
00384> *
00385> *
00386> * SUB-AREA No.9
00387> *
00388> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] (min), AREA=[4.0] (ha),
DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] (hrs),
RAINFALL=[ , , , ] (mm/hr) , END=-1
00389> *%-----
00390> *
00391> *
00392> *
00393> *
00394> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00395> *%-----
00396> *
00397> * SUB-AREA No.10
00398> *
00399> DESIGN NASHYD ID=[ 1 ], NHYD=["A3"], DT=[2.5] (min), AREA=[2.7] (ha),
DWF=[0] (cms), CN/C=[76], TP=[0.80] (hrs),
RAINFALL=[ , , , ] (mm/hr) , END=-1
00400> *%-----
00401> *
00402> *
00403> *
00404> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[9+1]
00405> *%-----
00406> *%-----
00407> *%-----
00408> * CALCULATION OF 3HR - 1.5 YEAR STORM EVENT *
00409> *%-----
00410> *
00411> START TZERO=[0.0], METOUT=[2], NSTORM=[0], WRHM=[0]
00412> *%-----
00413> * [ ] <- storm filename, one per line for NSTORM time
00414> CHICAGO STORM IUNIT=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00415> ICASECS=[1],
A=[396.071], B=[6.053], and C=[0.814],
00416> *%-----
00417> *
00418> DEFAULT VALUES ICASEDEF=[1], read and print values
00419> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWM\HYMO\ORGA.VAL]
00420> *%-----
00421> *%-----
00422> *%-----
00423> * ORGAWORLD FILE *
00424> *%-----
00425> *
00426> * SUB-AREA No.1
00427> *
00428> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
LGP=[20] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00429> RAINFALL=[ , , , ] (mm/hr) , END=-1
00430> *%-----
00431> *
00432> * SUB-AREA No.2
00433> *
00434> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00435> RAINFALL=[ , , , ] (mm/hr) , END=-1
00436> *%-----
00437> *
00438> * SUB-AREA No.3
00439> *
00440> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
00441> RAINFALL=[ , , , ] (mm/hr) , END=-1
00442> *%-----
00443> *
00444> * SUB-AREA No.4
00445> *
00446> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
00447> RAINFALL=[ , , , ] (mm/hr) , END=-1
00448> *%-----
00449> *
00450> * SUB-AREA No.5
00451> *
00452> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
00453> RAINFALL=[ , , , ] (mm/hr) , END=-1
00454> *%-----
00455> *
00456> * SUB-AREA No.6
00457> *
00458> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00459> RAINFALL=[ , , , ] (mm/hr) , END=-1
00460> *%-----
00461> *
00462> * SUB-AREA No.7
00463> *
00464> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00465> RAINFALL=[ , , , ] (mm/hr) , END=-1
00466> *%-----
00467> *
00468> * SUB-AREA No.8
00469> *
00470> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00471> RAINFALL=[ , , , ] (mm/hr) , END=-1
00472> *%-----
00473> *
00474> * SUB-AREA No.9
00475> *
00476> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00477> RAINFALL=[ , , , ] (mm/hr) , END=-1
00478> *%-----
00479> *
00480> * SUB-AREA No.10
00481> *
00482> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00483> RAINFALL=[ , , , ] (mm/hr) , END=-1
00484> *%-----
00485> *
00486> * SUB-AREA No.11
00487> *
00488> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00489> RAINFALL=[ , , , ] (mm/hr) , END=-1
00490> *%-----
00491> *
00492> * SUB-AREA No.12
00493> *
00494> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
RDT=[1.0] (min),
TABLE of ( OUTFLOW-STORAGE ) values
(cms) - (ha-m)
[ 0.000, 0.0000]
[ 0.008, 0.0656]
[ 0.017, 0.1311]
[ 0.093, 0.2831]
[ 0.233, 0.3971]
[ 0.337, 0.4731]
[ 0.465, 0.5491]
[ 0.531, 0.5871]
[ 0.593, 0.6251]
[ 0.654, 0.6631]
[ 0.797, 0.7391]
[ 0.950, 0.8274]
[ 1.304, 0.9157]
[ 1.880, 1.0040]
[ 2.577, 1.0923]
[ -1, -1 ] (max twenty pts)
00495> *%-----
00496> *
00497> *
00498> *
00499> *
00500> *
00501> *
00502> *
00503> *
00504> *
00505> *
00506> *
00507> *
00508> *
00509> *
00510> *
00511> *
00512> *
00513> *
00514> *
00515> *
00516> * Remaining Hawthorne Industrial Park *
00517> *%-----
00518> *
00519> * SUB-AREA No.1
00520> *
00521> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00522> RAINFALL=[ , , , ] (mm/hr) , END=-1
00523> *%-----
00524> *
00525> * SUB-AREA No.2
00526> *
00527> CALIB STANDHYD ID=[ 2 ], NHYD=["HIP02"], DT=[2.5] (min), AREA=[17] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00528> RAINFALL=[ , , , ] (mm/hr) , END=-1
00529> *%-----
00530> *
00531> * SUB-AREA No.3
00532> *
00533> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00534> RAINFALL=[ , , , ] (mm/hr) , END=-1
00535> *%-----
00536> *
00537> * SUB-AREA No.4
00538> *
00539> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
LGI=[580] (m), MNI=[0.03], SCI=[0.0]
00540> RAINFALL=[ , , , ] (mm/hr) , END=-1

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00541> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min
00542> RAINFALL=[ , , , ] (mm/hr), END=-1
00543> *%-----
00544> *
00545> * SUB-AREA No.3
00546>
00547> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00548> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00549> SCS curve number CN=[81],
00550> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00551> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00552> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
00553> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min
00554> RAINFALL=[ , , , ] (mm/hr), END=-1
00555> *%-----
00556> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00557> *%-----
00558> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00559> *%-----
00560> *
00561> * SUB-AREA No.4
00562>
00563> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00564> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00565> SCS curve number CN=[81],
00566> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00567> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00568> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
00569> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min
00570> RAINFALL=[ , , , ] (mm/hr), END=-1
00571> *%-----
00572> *%-----
00573> *
00574> * SUB-AREA No.5
00575>
00576> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00577> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00578> RAINFALL=[ , , , ] (mm/hr), END=-1
00579> *%-----
00580>
00581>
00582> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00583> *%-----
00584> *
00585> * SUB-AREA No. 6
00586>
00587> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00588> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
00589> RAINFALL=[ , , , ] (mm/hr), END=-1
00590> *%-----
00591>
00592> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[9+1]
00593> *%-----
00594>
00595> *****
00596> * CALCULATION OF 3HR - 1:10 YEAR STORM EVENT *
00597> *****
00598>
00599> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00600> *%-----
00601> * [ ] <- storm filename, one per line for NSTORM time
00602> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00603> ICASDef=[1],
00604> A=[1174.184], B=[6.014], and C=[0.816],
00605> *%-----
00606> DEFAULT VALUES ICASDef=[1], read and print values
00607> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00608> *%-----
00609>
00610> *****
00611> * ORGAWORLD FILE *
00612> *****
00613>
00614> * SUB-AREA No.1
00615>
00616> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00617> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00618> SCS curve number CN=[81],
00619> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00620> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (m)
00621> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
00622> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min
00623> RAINFALL=[ , , , ] (mm/hr), END=-1
00624> *%-----
00625> *
00626> * SUB-AREA No.2
00627>
00628> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00629> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00630> SCS curve number CN=[81],
00631> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00632> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00633> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
00634> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0] (min
00635> RAINFALL=[ , , , ] (mm/hr), END=-1
00636> *%-----
00637> *
00638> * SUB-AREA No.3
00639>
00640> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00641> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00642> SCS curve number CN=[81],
00643> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00644> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00645> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
00646> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0] (min
00647> RAINFALL=[ , , , ] (mm/hr), END=-1
00648> *%-----
00649> ADD HYD IDsum=[4], NHYD=[ "040"], IDs to add=[1+2]
00650> *%-----
00651> ADD HYD IDsum=[5], NHYD=[ "050"], IDs to add=[3+4]
00652> *%-----
00653> *
00654> * SUB-AREA No.4
00655>
00656> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00657> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00658> SCS curve number CN=[81],
00659> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00660> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00661> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
00662> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min
00663> RAINFALL=[ , , , ] (mm/hr), END=-1
00664> *%-----
00665> *
00666> * SUB-AREA No.5
00667>
00668> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00669> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00670> SCS curve number CN=[81],
00671> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00672> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (m)
00673> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00674> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min
00675> RAINFALL=[ , , , ] (mm/hr), END=-1
00676> *%-----
00677> *%-----
00678> *
00679> ADD HYD IDsum=[8], NHYD=[ "080"], IDs to add=[6+7]
00680> *%-----
00681> *
00682> ROUTE RESERVOIR Idout=[10], NHYD=["POND"], Idin=[9],
00683> RDT=[1.0] (min),
00684>
00685> TABLE of ( OUTFLOW-STORAGE ) values
00686> ( cms ) ( ha-m )
00687> [ 0.000, 0.0000 ]
00688> [ 0.008, 0.0656 ]
00689> [ 0.017, 0.1311 ]
00690> [ 0.092, 0.2831 ]
00691> [ 0.233, 0.3971 ]
00692> [ 0.337, 0.4731 ]
00693> [ 0.465, 0.5491 ]
00694> [ 0.531, 0.5871 ]
00695> [ 0.593, 0.6251 ]
00696> [ 0.654, 0.6631 ]
00697> [ 0.797, 0.7391 ]
00698> [ 0.950, 0.8274 ]
00699> [ 1.304, 0.9157 ]
00700> [ 1.880, 1.0040 ]
00701> [ 2.577, 1.0923 ]
00702> [ -1, -1 ] (max twenty pts)
00703> *****
00704> * Remaining Hawthorne Industrial Park *
00705> *****
00706> *
00707> * SUB-AREA No.1
00708>
00709> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00710> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00711> SCS curve number CN=[81],
00712> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00713> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00714> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.6] (%),
00715> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min
00716> RAINFALL=[ , , , ] (mm/hr), END=-1
00717> *%-----
00718> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00719> *%-----
00720> *
00721> * SUB-AREA No.2
00722>
00723> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00724> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00725> SCS curve number CN=[81],
00726> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00727> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00728> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
00729> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min
00730> RAINFALL=[ , , , ] (mm/hr), END=-1
00731> *%-----
00732> *
00733> * SUB-AREA No.3
00734>
00735> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00736> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00737> SCS curve number CN=[81],
00738> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00739> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00740> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
00741> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min
00742> RAINFALL=[ , , , ] (mm/hr), END=-1
00743> *%-----
00744> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00745> *%-----
00746> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00747> *%-----
00748> *
00749> * SUB-AREA No.4
00750>
00751> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00752> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00753> SCS curve number CN=[81],
00754> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00755> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00756> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
00757> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min
00758> RAINFALL=[ , , , ] (mm/hr), END=-1
00759> *%-----
00760> *
00761> *
00762> * SUB-AREA No.5
00763>
00764> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00765> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00766> RAINFALL=[ , , , ] (mm/hr), END=-1
00767> *%-----
00768>
00769>
00770> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00771> *%-----
00772> *
00773> * SUB-AREA No. 6
00774>
00775> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00776> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
00777> RAINFALL=[ , , , ] (mm/hr), END=-1
00778> *%-----
00779>
00780> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[9+1]
00781> *%-----
00782>
00783> *****
00784> * CALCULATION OF 3HR - 1:25 YEAR STORM EVENT *
00785> *****
00786>
00787> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00788> *%-----
00789> * [ ] <- storm filename, one per line for NSTORM time
00790> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00791> ICASDef=[1],
00792> A=[1402.884], B=[6.018], and C=[0.819],
00793> *%-----
00794> DEFAULT VALUES ICASDef=[1], read and print values
00795> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00796> *%-----
00797>
00798> *****
00799> * ORGAWORLD FILE *
00800> *****
00801>
00802> * SUB-AREA No.1
00803>
00804> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00805> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00806> SCS curve number CN=[81],
00807> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00808> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (m)
00809> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
00810> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min

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00811> RAINFALL=[ , , , ](mm/hr) , END=-1
00812> *
00813> *
00814> * SUB-AREA No.2
00815>
00816> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[ 1.54 ](ha),
XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.50](%),
LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00823> *
00824> *
00825> *
00826> * SUB-AREA No.3
00827>
00828> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.51](%),
LGI=[ 225.63 ](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00836> *
00837> ADD HYD IDsum=[ 4 ], NHYD=[ "040" ], IDs to add=[1+2]
00838> *
00839> ADD HYD IDsum=[ 5 ], NHYD=[ "050" ], IDs to add=[3+4]
00840> *
00841> *
00842> * SUB-AREA No.4
00843>
00844> CALIB STANDHYD ID=[ 5 ], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[0.7](%),
LGP=[40](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.93](%),
LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00852> *
00853> *
00854> * SUB-AREA No.5
00855>
00856> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00864> *
00865> ADD HYD IDsum=[ 8 ], NHYD=[ "080" ], IDs to add=[6+7]
00866> *
00867> ADD HYD IDsum=[ 9 ], NHYD=[ "090" ], IDs to add=[5+8]
00868> *
00869>
00870> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
RDT=[1.0](min),
TABLE of ( OUTFLOW-STORAGE ) values
(cms) - (ha-m)
[ 0.008, 0.0000 ]
[ 0.008, 0.0656 ]
[ 0.017, 0.1311 ]
[ 0.093, 0.2831 ]
[ 0.233, 0.3971 ]
[ 0.337, 0.4731 ]
[ 0.465, 0.5491 ]
[ 0.531, 0.5871 ]
[ 0.593, 0.6251 ]
[ 0.654, 0.6631 ]
[ 0.797, 0.7391 ]
[ 0.950, 0.8274 ]
[ 1.304, 0.9157 ]
[ 1.880, 1.0040 ]
[ 2.577, 1.0923 ]
[ -1, -1 ] (max twenty pts)
00891> *****
00892> * Remaining Hawthorne Industrial Park *
00893> *****
00894> *
00895> * SUB-AREA No.1
00896>
00897> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5](min), AREA=[19.9](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.6](%),
LGI=[580](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00904> *
00905> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00907> *
00908> *
00909> * SUB-AREA No.2
00910>
00911> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5](min), AREA=[17](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
LGI=[450](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00920> *
00921> * SUB-AREA No.3
00922>
00923> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5](min), AREA=[15.6](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.5](%),
LGI=[600](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00931> *
00932> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00933> *
00934> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00935> *
00936> *
00937> * SUB-AREA No.4
00938>
00939> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5](min), AREA=[12.2](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.7](%),
LGI=[210](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00946> RAINFALL=[ , , , ](mm/hr) , END=-1
00947>
00948> *
00949> *
00950> * SUB-AREA No.5
00951>
00952> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5](min), AREA=[4.0](ha),
DWF=[ 0 ](cms), CN/C=[ 85 ], TP=[0.17]hrs,
RAINFALL=[ , , , ](mm/hr) , END=-1
00954> *
00955> *
00956> *
00957>
00958> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00959> *
00960> *
00961> * SUB-AREA No. 6
00962>
00963> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5](min), AREA=[2.7](ha),
DWF=[0](cms), CN/C=[76], TP=[0.80]hrs,
RAINFALL=[ , , , ](mm/hr) , END=-1
00965> *
00966> *
00967>
00968> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[9+1]
00969> *
00970> *
00971> *****
00972> * CALCULATION OF 3HR - 1:50 YEAR STORM EVENT *
00973> *****
00974>
00975> START TZERO=[0.0], MEZOUT=[2], NSTORM=[0], NRUN=[0]
[ ] <-- storm filename, one per line for NSTORM time
00976> *
00977> *
00978> CHICAGO STORM IUNITS=[2], TD=[3.0](hrs), TPRAT=[0.333], CSDT=[10.0](min)
ICASEC=[1],
A=[1569.580], B=[6.014], and C=[0.820],
00981> *
00982> DEFAULT VALUES ICASEDef=[1], read and print values
00983> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00984> *
00985> *****
00986> *****
00987> * ORGAWORLD FILE *
00988> *****
00989>
00990> * SUB-AREA No.1
00991>
00992> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5](min), AREA=[ 2.07 ](ha),
XIMP=[0.84], TIMP=[0.84], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[20](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.52](%),
LGI=[204.72](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
00999> *
01000> *
01001> *
01002> * SUB-AREA No.2
01003>
01004> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[ 1.54 ](ha),
XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.50](%),
LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
01013> *
01014> * SUB-AREA No.3
01015>
01016> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.51](%),
LGI=[ 225.63 ](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
01024> *
01025> ADD HYD IDsum=[ 4 ], NHYD=[ "040" ], IDs to add=[1+2]
01026> *
01027> ADD HYD IDsum=[ 5 ], NHYD=[ "050" ], IDs to add=[3+4]
01028> *
01029> *
01030> * SUB-AREA No.4
01031>
01032> CALIB STANDHYD ID=[ 6 ], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[0.7](%),
LGP=[40](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.93](%),
LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
01041> *
01042> * SUB-AREA No.5
01043>
01044> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
RAINFALL=[ , , , ](mm/hr) , END=-1
01052> *
01053> ADD HYD IDsum=[ 8 ], NHYD=[ "080" ], IDs to add=[6+7]
01054> *
01055> ADD HYD IDsum=[ 9 ], NHYD=[ "090" ], IDs to add=[5+8]
01056> *
01057>
01058> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
RDT=[1.0](min),
TABLE of ( OUTFLOW-STORAGE ) values
(cms) - (ha-m)
[ 0.008, 0.0000 ]
[ 0.008, 0.0656 ]
[ 0.017, 0.1311 ]
[ 0.093, 0.2831 ]
[ 0.233, 0.3971 ]
[ 0.337, 0.4731 ]
[ 0.465, 0.5491 ]
[ 0.531, 0.5871 ]
[ 0.593, 0.6251 ]
[ 0.654, 0.6631 ]
[ 0.797, 0.7391 ]
[ 0.950, 0.8274 ]
[ 1.304, 0.9157 ]
[ 1.880, 1.0040 ]
[ 2.577, 1.0923 ]
[ -1, -1 ] (max twenty pts)
01079> *****
01080> * Remaining Hawthorne Industrial Park *

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012165 *%-----
012170 *%-----
012180 * SUB-AREA No.4
012190
012200 CALIB STANDHYD ID=[ 6 ], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
012210 SCS curve number CN=[81],
012220 Pervious surfaces: IAperv=[4.67] (mm), SLPp=[0.7] (%),
012230 LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
012240 Impervious surfaces: IAImp=[1.57] (mm), SLPi=[0.61] (%),
012250 LGI=[64.82] (m), MNI=[0.03], SCI=[0.0] (min)
012260 RAINFALL=[ , , , ] (mm/hr), END=-1
012270 *%-----
012280 *%-----
012290 * SUB-AREA No.5
012300
012310 CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
012320 SCS curve number CN=[81],
012330 Pervious surfaces: IAperv=[4.67] (mm), SLPp=[1.5] (%),
012340 LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
012350 Impervious surfaces: IAImp=[1.57] (mm), SLPi=[0.61] (%),
012360 LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min)
012370 RAINFALL=[ , , , ] (mm/hr), END=-1
012380 *%-----
012390 *%-----
012400 ADD HYD IDsum=[ 8 ], NHYD=[ "080" ], IDs to add=[6+7]
012410 *%-----
012420 *%-----
012430 ADD HYD IDsum=[ 9 ], NHYD=[ "090" ], IDs to add=[5+8]
012440 *%-----
012450 *%-----
012460 ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
012470 RDT=[1.0] (min),
012480 TABLE of ( OUTFLOW-STORAGE ) values
012490 ( cms ) - ( ha-m )
012500 [ 0.000, 0.00000 ]
012510 SCS curve number CN=[81],
012520 [ 0.008, 0.0656 ]
012530 [ 0.017, 0.1311 ]
012540 [ 0.093, 0.2831 ]
012550 [ 0.233, 0.3971 ]
012560 [ 0.337, 0.4731 ]
012570 [ 0.465, 0.5491 ]
012580 [ 0.531, 0.5871 ]
012590 [ 0.592, 0.6251 ]
012600 [ 0.654, 0.6631 ]
012610 [ 0.797, 0.7391 ]
012620 [ 0.950, 0.8274 ]
012630 [ 1.304, 0.9157 ]
012640 [ 1.880, 1.0040 ]
012650 [ 2.577, 1.0923 ]
012660 [ -1 , -1 ] (max twenty pts)
012670 *****
012680 * Remaining Hawthorne Industrial Park *
012690 *****
012700 *%-----
012710 * SUB-AREA No.1
012720
012730 CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
012740 SCS curve number CN=[81],
012750 Pervious surfaces: IAperv=[4.67] (mm), SLPp=[1.5] (%),
012760 LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
012770 Impervious surfaces: IAImp=[1.57] (mm), SLPi=[0.6] (%),
012780 LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
012790 RAINFALL=[ , , , ] (mm/hr), END=-1
012800 *%-----
012810 *%-----
012820 ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
012830 *%-----
012840 *%-----
012850 * SUB-AREA No.2
012860
012870 CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
012880 SCS curve number CN=[81],
012890 Pervious surfaces: IAperv=[4.67] (mm), SLPp=[1.5] (%),
012900 LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
012910 Impervious surfaces: IAImp=[1.57] (mm), SLPi=[0.65] (%),
012920 LGI=[500] (m), MNI=[0.03], SCI=[0.0] (min)
012930 RAINFALL=[ , , , ] (mm/hr), END=-1
012940 *%-----
012950 *%-----
012960 * SUB-AREA No.3
012970
012980 CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
012990 SCS curve number CN=[81],
013000 Pervious surfaces: IAperv=[4.67] (mm), SLPp=[1.5] (%),
013010 LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
013020 Impervious surfaces: IAImp=[1.57] (mm), SLPi=[0.5] (%),
013030 LGI=[500] (m), MNI=[0.03], SCI=[0.0] (min)
013040 RAINFALL=[ , , , ] (mm/hr), END=-1
013050 *%-----
013060 *%-----
013070 ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
013080 *%-----
013090 *%-----
013100 ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
013110 *%-----
013120 *%-----
013130 * SUB-AREA No.4
013140
013150 CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
013160 SCS curve number CN=[81],
013170 Pervious surfaces: IAperv=[4.67] (mm), SLPp=[1.5] (%),
013180 LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
013190 Impervious surfaces: IAImp=[1.57] (mm), SLPi=[0.7] (%),
013200 LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
013210 RAINFALL=[ , , , ] (mm/hr), END=-1
013220 *%-----
013230 *%-----
013240 * SUB-AREA No.5
013250
013260 *%-----
013270 *%-----
013280 DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), CN/CN=[ , ], TP=[0.17] hrs,
013290 RAINFALL=[ , , , ] (mm/hr), END=-1
013300 *%-----
013310 *%-----
013320 *%-----
013330 *%-----
013340 ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
013350 *%-----
013360 *%-----
013370 * SUB-AREA No. 6
013380
013390 DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
XIMP=[0.50], CN/CN=[76], TP=[0.80] hrs,
013400 RAINFALL=[ , , , ] (mm/hr), END=-1
013410 *%-----
013420 *%-----
013430 *%-----
013440 ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[9+1]
013450 *%-----
013460 *%-----
013470 FINISH
013480
013490
013500 ***** 600 mm culverts @ 84.5 + 2 600 mm culverts @ 85.25 10% Pond reduction*****

```

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00001> =====
00002>
00003> SSSSS W W W M M H H Y Y M M M O O O 999 999 =====
00004> S W W W M M H H Y Y M M M O O O 9 9 9 9 Ver. 4.02
00005> SSSSS W W W M M M H H H H Y Y M M M O O O 9999 9999 July 1999
00006> S W W M M M H H Y Y M M O O 9 9 9 9
00007> SSSSS W W M M M H H Y Y M M O O O 9 9 9 9
00008>
00009> StormWater Management HYdrologic Model 999 999 =====
00010>
00011> ***** SWHYMO-99 Ver/4.02 *****
00012> ***** A single event and continuous hydrologic simulation model *****
00013> ***** based on the principles of HMO and its successors *****
00014> ***** OTHYMO-93 and OTHYMO-99 *****
00015> *****
00016> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00017> ***** Ottawa, Ontario: (613) 727-5199 *****
00018> ***** Gatineau, Quebec: (819) 243-6858 *****
00019> ***** E-Mail: sumhymo@jfasa.com *****
00020> *****
00021> *****
00022>
00023> *****
00024> ***** Licensed user: J. L. Richards & Associates Limited *****
00025> ***** Ottawa SERIAL# 4418403 *****
00026> *****
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00036> ***** DETAILED OUTPUT *****
00037> *****
00038> ***** DATE: 2009-02-09 TIME: 14:59:31 RUN COUNTER: 000154 *****
00039> *****
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00271> (i) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00272> THAN THE STORAGE COEFFICIENT.  
 00273> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00274>  
 00275>  
 00276> 001:001C  
 00277> \* SUB-AREA No.5  
 00278>  
 00279> CALIB STANDHYD | Area (ha)= 2.66  
 00280> | 07:07C DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 00281>  
 00282>  
 00283>  
 00284> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)  
 00285> Dep. Storage (mm)= 2.58 .08  
 00286> Average Slope (m)= 1.57 4.67  
 00287> Length (m)= .61 1.50  
 00288> Mannings n = 207.25 20.00  
 00289> Max. eff. Inten. (mm/hr)= .030 .250  
 00290> over (min)= 45.63 5.66  
 00291> Storage Coeff. (min)= 10.00 27.50  
 00292> Unit Hyd. Tpeak (min)= 10.37 (ii) 26.38 (ii)  
 00293> Unit Hyd. peak (cms)= 11.00 27.50  
 00294> PEAK FLOW (cms)= .11 .04  
 00295> TIME TO PEAK (hrs)= .24 .00  
 00296> RUNOFF VOLUME (mm)= 1.29 1.67  
 00297> TOTAL RAINFALL (mm)= 23.43 5.17  
 00298> RUNOFF COEFFICIENT = 25.00 24.999  
 00299> \*TOTALS\*  
 00300> .94 .21 .915  
 00301>  
 00302> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00303> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00304> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00305> THAN THE STORAGE COEFFICIENT.  
 00306> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00307>  
 00308>  
 00309> 001:0011  
 00310>  
 00311> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00312> | ID1 06:060 | (ha) (cms) (hrs) (mm) (cms)  
 00313> +ID2 07:070 2.89 .089 1.25 22.88 .000  
 00314> SUM 08:080 3.55 .327 1.29 22.88 .000  
 00315>  
 00316>  
 00317>  
 00318> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00319>  
 00320>  
 00321> 001:0012  
 00322> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00323> | ID1 05:050 | (ha) (cms) (hrs) (mm) (cms)  
 00324> +ID2 08:080 5.01 .396 1.33 21.62 .000  
 00325> SUM 09:090 8.56 .716 1.29 22.14 .000  
 00326>  
 00327>  
 00328>  
 00329>  
 00330> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00331>  
 00332>  
 00333> 001:0013  
 00334>  
 00335> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 00336> | IN>09: (090 ) |  
 00337> | OUT<10: (POND ) |  
 00338>  
 00339>  
 00340>  
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 00358>  
 00359> 001:0014  
 00360>  
 00361> \* Remaining Hawthorne Industrial Park \*  
 00362>  
 00363>  
 00364> \* SUB-AREA No.1  
 00365>  
 00366> CALIB STANDHYD | Area (ha)= 19.90  
 00367> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00368>  
 00369>  
 00370> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)  
 00371> Dep. Storage (mm)= 14.13 5.77  
 00372> Average Slope (m)= 1.57 4.67  
 00373> Length (m)= .60 1.50  
 00374> Mannings n = 580.00 100.00  
 00375> Max. eff. Inten. (mm/hr)= .030 .250  
 00376> over (min)= 34.39 11.90  
 00377> Storage Coeff. (min)= 22.50 52.50  
 00378> Unit Hyd. Tpeak (min)= 21.64 (ii) 52.88 (ii)  
 00379> Unit Hyd. peak (cms)= 22.50 52.50  
 00380> PEAK FLOW (cms)= .05 .02  
 00381> TIME TO PEAK (hrs)= .60 .11  
 00382> RUNOFF VOLUME (mm)= 1.50 2.13  
 00383> TOTAL RAINFALL (mm)= 23.43 8.74  
 00384> RUNOFF COEFFICIENT = 25.00 24.999  
 00385> \*TOTALS\*  
 00386> .94 .35 .643  
 00387>  
 00388> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00389> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00390> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00391> THAN THE STORAGE COEFFICIENT.  
 00392> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00393>  
 00394>  
 00395> 001:0015  
 00396>  
 00397> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00398> | ID1 10:POND | (ha) (cms) (hrs) (mm) (cms)  
 00399> +ID2 01:HIP01 8.56 .032 3.88 22.14 .000  
 00400> SUM 02:HIP02 19.90 .655 1.54 17.91 .000  
 00401>  
 00402>  
 00403>  
 00404> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00405>

00406>  
 00407> 001:0016  
 00408> \* SUB-AREA No.2  
 00409>  
 00410>  
 00411> CALIB STANDHYD | Area (ha)= 17.00  
 00412> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00413>  
 00414>  
 00415> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)  
 00416> Dep. Storage (mm)= 12.07 4.93  
 00417> Average Slope (m)= 1.57 4.67  
 00418> Length (m)= .65 1.50  
 00419> Mannings n = 450.00 100.00  
 00420> Max. eff. Inten. (mm/hr)= .030 .250  
 00421> over (min)= 40.81 12.73  
 00422> Storage Coeff. (min)= 16.94 (ii) 47.35 (ii)  
 00423> Unit Hyd. Tpeak (min)= 17.50 47.50  
 00424> Unit Hyd. peak (cms)= .07 .02  
 00425> PEAK FLOW (cms)= .60 .10  
 00426> TIME TO PEAK (hrs)= 1.42 2.00  
 00427> RUNOFF VOLUME (mm)= 23.43 8.74  
 00428> TOTAL RAINFALL (mm)= 25.00 25.00  
 00429> RUNOFF COEFFICIENT = .94 .35  
 00430> \*TOTALS\*  
 00431> .625 (iii)  
 00432>  
 00433> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00434> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00435> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00436> THAN THE STORAGE COEFFICIENT.  
 00437> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00438>  
 00439>  
 00440> 001:0017  
 00441> \* SUB-AREA No.3  
 00442>  
 00443> CALIB STANDHYD | Area (ha)= 15.60  
 00444> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00445>  
 00446>  
 00447> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)  
 00448> Dep. Storage (mm)= 11.08 4.52  
 00449> Average Slope (m)= 1.57 4.67  
 00450> Length (m)= .60 1.50  
 00451> Mannings n = 600.00 100.00  
 00452> Max. eff. Inten. (mm/hr)= .030 .250  
 00453> over (min)= 34.39 11.54  
 00454> Storage Coeff. (min)= 22.50 55.00  
 00455> Unit Hyd. Tpeak (min)= 23.33 (ii) 54.95 (ii)  
 00456> Unit Hyd. peak (cms)= 22.50 55.00  
 00457> PEAK FLOW (cms)= .45 .08  
 00458> TIME TO PEAK (hrs)= 1.50 2.17  
 00459> RUNOFF VOLUME (mm)= 23.43 8.74  
 00460> TOTAL RAINFALL (mm)= 25.00 25.00  
 00461> RUNOFF COEFFICIENT = .94 .35  
 00462> \*TOTALS\*  
 00463> .484 (iii)  
 00464>  
 00465> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00466> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00467> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00468> THAN THE STORAGE COEFFICIENT.  
 00469> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00470>  
 00471>  
 00472>  
 00473> 001:0018  
 00474>  
 00475> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00476> | ID1 03:HIP03 | (ha) (cms) (hrs) (mm) (cms)  
 00477> +ID2 04:HIP04 17.00 .625 1.46 16.08 .000  
 00478> SUM 05:HIP05 15.60 .484 1.54 16.08 .000  
 00479>  
 00480>  
 00481>  
 00482> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00483>  
 00484>  
 00485> 001:0019  
 00486>  
 00487> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00488> | ID1 05:HIP05 | (ha) (cms) (hrs) (mm) (cms)  
 00489> +ID2 02:HIP02 32.60 1.091 1.46 16.08 .000  
 00490> SUM 06:HIP06 28.46 .655 1.54 17.91 .000  
 00491>  
 00492>  
 00493>  
 00494>  
 00495>  
 00496>  
 00497> 001:0020  
 00498>  
 00499> \* SUB-AREA No.4  
 00500>  
 00501> CALIB STANDHYD | Area (ha)= 12.20  
 00502> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00503>  
 00504>  
 00505> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)  
 00506> Dep. Storage (mm)= 8.66 3.54  
 00507> Average Slope (m)= 1.57 4.67  
 00508> Length (m)= .70 1.50  
 00509> Mannings n = 210.00 100.00  
 00510> Max. eff. Inten. (mm/hr)= .030 .250  
 00511> over (min)= 45.63 14.15  
 00512> Storage Coeff. (min)= 10.00 40.00  
 00513> Unit Hyd. Tpeak (min)= 10.03 (ii) 39.18 (ii)  
 00514> Unit Hyd. peak (cms)= 10.00 40.00  
 00515> PEAK FLOW (cms)= .11 .03  
 00516> TIME TO PEAK (hrs)= .60 .11  
 00517> RUNOFF VOLUME (mm)= 1.50 2.13  
 00518> TOTAL RAINFALL (mm)= 23.43 8.74  
 00519> RUNOFF COEFFICIENT = 25.00 24.999  
 00520> \*TOTALS\*  
 00521> .94 .35 .643  
 00522>  
 00523> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00524> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00525> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00526> THAN THE STORAGE COEFFICIENT.  
 00527> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00528>  
 00529>  
 00530> 001:0021  
 00531>  
 00532> \* SUB-AREA No.5  
 00533>  
 00534> DESIGN NASHYD | Area (ha)= 4.00  
 00535> | 08:Pond-B DT= 2.50 | Total Imp(%)= 4.670 Dir. Conn.(%)= 85.00  
 00536> U.H. Tp(hrs)= 1.70  
 00537>  
 00538>  
 00539> Unit Hyd. Tpeak (cms)= .899  
 00540> PEAK FLOW (cms)= .077 (i)

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00541> TIME TO PEAK (hrs)= 1.375
00542> RUNOFF VOLUME (mm)= 6.343
00543> TOTAL RAINFALL (mm)= 24.999
00544> RUNOFF COEFFICIENT = .254
00545>
00546> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00547>
00548>
00549> 001:0022-----
00550>
00551> | ADD HYD (HIPO8) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00552> | | (ha) (cms) (hrs) (mm) (cms)
00553> | ID1 06:HIPO8 61.06 1.740 1.50 16.33 .000
00554> | ID2 07:HIPO8 12.20 .585 1.29 16.08 .000
00555> | ID3 08:Pond-B 4.00 .077 1.38 6.34 .000
00556> | SUM 09:HIPO8 77.26 2.227 1.46 16.25 .000
00557>
00558>
00559> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00560>
00561>
00562> 001:0023-----
00563> *
00564> | SUB-AREA No. 6
00565> *
00566>
00567> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
00568> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
00569> | U.H. Tp (hrs)= .800
00570>
00571> Unit Hyd. Qpeak (cms)= .129
00572>
00573> PEAK FLOW (cms)= .013 (i)
00574> TIME TO PEAK (hrs)= 2.292
00575> RUNOFF VOLUME (mm)= 4.110
00576> TOTAL RAINFALL (mm)= 24.999
00577> RUNOFF COEFFICIENT = .164
00578>
00579> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00580>
00581>
00582> 001:0024-----
00583>
00584> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00585> | | (ha) (cms) (hrs) (mm) (cms)
00586> | ID1 09:HIPO8 77.26 2.227 1.46 16.25 .000
00587> | ID2 01:A3 2.70 .013 2.29 4.11 .000
00588> | SUM 02:Ultima 79.96 2.231 1.46 15.84 .000
00589>
00590>
00591> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00592>
00593>
00594> 001:0025-----
00595> *****
00596> * CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00597> *****
00598>
00599> | START | Project dir.: V:\20983.DU\ENG\SWHMYMO\
00600> | Rainfall dir.: V:\20983.DU\ENG\SWHMYMO\
00601> | TZERO = .00 hrs on 0
00602> | METOUT = 2 (output = METRIC)
00603> | NRUN = 001
00604> | NSTORM = 0
00605>
00606> 001:0002-----
00607>
00608> | CHICAGO STORM | IDF curve parameters: A= 732.951
00609> | Ptotal= 31.86 mm | B= 6.199
00610> | | C= .810
00611> | used in: INTENSITY = A / (t + B)^C
00612>
00613> | Duration of storm = 3.00 hrs
00614> | Storm time step = 10.00 min
00615> | Time to peak ratio = .33
00616>
00617> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00618> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00619> .17 2.815 | 1.00 76.805 | 1.83 5.095 | 2.87 2.684
00620> .33 3.498 | 1.17 24.079 | 2.00 4.291 | 2.83 2.463
00621> .50 4.687 | 1.33 12.364 | 2.17 3.718 | 3.00 2.279
00622> .67 7.305 | 1.50 8.324 | 2.33 3.288 |
00623> .83 18.209 | 1.67 6.303 | 2.50 2.953 |
00624>
00625>
00626> 001:0003-----
00627>
00628> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\SWHMYMO\ORGA.VAL
00629> | ICASEdv = 1 (read and print data)
00630> | FileTitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
00631> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----D
00632> | Horton's infiltration equation parameters:
00633> | Fp= 50.00 mm/hr [Pc= 7.50 mm/hr] [ICAY= 2.00 /hr] [P= .00 mm]
00634> | Parameters for PERVIOUS surfaces in STANDHYD:
00635> | [IAper= 4.67 mm] [LGP=40.00 m] [MNP=.250]
00636> | Parameters for IMPERVIOUS surfaces in STANDHYD:
00637> | [IAimp= 1.57 mm] [CLI= 1.50] [MNI=.035]
00638> | Parameters used in NASHYD:
00639> | [Ia= 4.67 mm] [N= 3.00]
00640>
00641> 001:0004-----
00642> *****
00643> * ORGAWORLD FILE *****
00644> *****
00645> * SUB-AREA No.1
00646>
00647> | CALIB STANDHYD | Area (ha)= 2.07
00648> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
00649>
00650>
00651> IMPERVIOUS PERVIOUS (i)
00652> Surface Area (ha)= 1.74 .33
00653> Dep. Storage (mm)= 1.57 4.67
00654> Average Slope (%)= .52 1.00
00655> Length (m)= 204.72 20.00
00656> Mannings n = .030 .250
00657>
00658> Max. eff. Inten. (mm/hr)= 76.81 11.88
00659> over (min)= 10.00 22.50
00660> Storage Coeff. (min)= 8.77 (ii) 22.21 (ii)
00661> Unit Hyd. Tpeak (min)= 10.00 22.50
00662> Unit Hyd. peak (cms)= .12 .05
00663>
00664> *TOTALS*
00665> PEAK FLOW (cms)= .24 .01 .245 (iii)
00666> TIME TO PEAK (hrs)= 1.08 1.38 1.083
00667> RUNOFF VOLUME (mm)= 30.29 8.52 26.807
00668> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00669> RUNOFF COEFFICIENT = .95 .27 .841
00670>
00671> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00672> CN* = 81.0 Ia = Dep. Storage (Above)
00673> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00674> THAN THE STORAGE COEFFICIENT.
00675> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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00676> 001:0005-----
00677> *
00678> | SUB-AREA No.2
00679> *
00680> | CALIB STANDHYD | Area (ha)= 1.54
00681> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
00682>
00683>
00684> IMPERVIOUS PERVIOUS (i)
00685> Surface Area (ha)= 1.42 .12
00686> Dep. Storage (mm)= 1.57 4.67
00687> Average Slope (%)= .50 1.00
00688> Length (m)= 244.34 5.00
00689> Mannings n = .030 .030
00690>
00691> Max. eff. Inten. (mm/hr)= 76.81 15.07
00692> over (min)= 10.00 12.50
00693> Storage Coeff. (min)= 9.87 (ii) 11.36 (ii)
00694> Unit Hyd. Tpeak (min)= 10.00 12.50
00695> Unit Hyd. peak (cms)= .11 .10
00696>
00697> *TOTALS*
00698> PEAK FLOW (cms)= .19 .00 .192 (iii)
00699> TIME TO PEAK (hrs)= 1.08 1.17 1.083
00700> RUNOFF VOLUME (mm)= 30.29 8.52 28.548
00701> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00702> RUNOFF COEFFICIENT = .95 .27 .896
00703>
00704> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00705> CN* = 81.0 Ia = Dep. Storage (Above)
00706> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00707> THAN THE STORAGE COEFFICIENT.
00708> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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00811> Mannings n = .030 .250
00812>
00813> Max. eff. Inten. (mm/hr) = 76.81 12.71
00814> over (min) = 7.50 20.00
00815> Storage Coeff. (min) = 8.42 (ii) 20.00 (ii)
00816> Unit Hyd. Tpeak (min) = 7.50 20.00
00817> Unit Hyd. peak (cms) = .14 .06
00818>
00819> PEAK FLOW (cms) = .38 .00 *TOTALS*
00820> TIME TO PEAK (hrs) = 1.04 1.33 .379 (iii)
00821> RUNOFF VOLUME (mm) = 30.29 8.52 29.637
00822> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00823> RUNOFF COEFFICIENT = .95 .27 .930
00824>

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00825> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00826> CN* = 81.0 Ia = Dep. Storage (Above)
00827> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00828> THAN THE STORAGE COEFFICIENT.
00829> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00830>
00831>

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00832> 001:0011-----
00833> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00834> | (ha) (cms) (hrs) (mm) (cms)
00835> ID1 06:060 .89 .139 1.04 29.64 .000
00836> +ID2 07:070 2.66 .379 1.04 29.64 .000
00837>
00838> SUM 08:080 3.55 .518 1.04 29.64 .000
00839>

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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00844> 001:0012-----
00845> | ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00846> | (ha) (cms) (hrs) (mm) (cms)
00847> ID1 05:050 5.01 .623 1.08 28.13 .000
00848> +ID2 08:080 3.55 .518 1.04 29.64 .000
00849>
00850> SUM 09:090 8.56 1.118 1.08 28.76 .000
00851>

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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00856> 001:0013-----
00857> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00858> | IN:09:090 |
00859> | OUT:10:100 |
00860>
00861> ===== OUTFLOW STORAGE TABLE =====
00862> OUTFLOW STORAGE OUTFLOW STORAGE
00863> (cms) (ha.m.) (cms) (ha.m.)
00864> .000 .0000E+00 .593 .6251E+00
00865> .008 .6560E-01 .654 .6631E+00
00866> .017 .1311E+00 .797 .7391E+00
00867> .093 .2831E+00 .950 .8274E+00
00868> .233 .3971E+00 1.304 .9157E+00
00869> .337 .4731E+00 1.880 .1004E+01
00870> .465 .5491E+00 2.577 .1092E+01
00871> .531 .5871E+00 .000 .0000E+00
00872>

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00872> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00873> Dep. Storage (mm) 8.56 1.118 1.08 28.757
00874> INFLOW:09: (090) 8.56 .056 3.000 28.754
00875> OUTFLOW:10: (POND) 8.56 .056 3.000 28.754
00876>
00877> PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.030
00878> TIME SHIFT OF PEAK FLOW (min) = 115.00
00879> MAXIMUM STORAGE USED (ha.m.) = .2095E+00
00880>

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00882> 001:0014-----
00883> ***** Remaining Hawthorne Industrial Park *****
00884> *****
00885> *****
00886> *
00887> * SUB-AREA No.1
00888>

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00889> | CALIB STANDHYD | Area (ha) = 19.90
00890> | 01:H1P01 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
00891>

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00892> Surface Area (ha) = 14.13 IMPERVIOUS 5.77 PERVIOUS (i)
00893> Dep. Storage (mm) = 1.57 4.67
00894> Average Slope (%) = .60 1.50
00895> Length (m) = 580.00 100.00
00896> Mannings n = .030 .250
00897>
00898> Max. eff. Inten. (mm/hr) = 54.21 23.06
00899> over (min) = 17.50 42.50
00900> Storage Coeff. (min) = 18.04 (ii) 42.02 (ii)
00901> Unit Hyd. Tpeak (min) = 17.50 42.50
00902> Unit Hyd. peak (cms) = .06 .03
00903>
00904> PEAK FLOW (cms) = .95 .21 *TOTALS*
00905> TIME TO PEAK (hrs) = 1.21 1.71 1.020 (iii)
00906> RUNOFF VOLUME (mm) = 30.29 13.34 21.814
00907> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00908> RUNOFF COEFFICIENT = .95 .42 .685
00909>

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00910> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00911> CN* = 81.0 Ia = Dep. Storage (Above)
00912> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00913> THAN THE STORAGE COEFFICIENT.
00914> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00915>
00916>

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00917> 001:0015-----
00918> | ADD HYD (H1P02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00919> | (ha) (cms) (hrs) (mm) (cms)
00920> ID1 10:10ND 8.56 .056 3.00 28.75 .000
00921> +ID2 01:H1P01 19.90 1.020 1.25 21.81 .000
00922>
00923> SUM 02:H1P02 28.46 1.039 1.25 23.90 .000
00924>

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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00930> 001:0016-----
00931> *
00932> * SUB-AREA No.2
00933>

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00934> | CALIB STANDHYD | Area (ha) = 17.00
00935> | 03:H1P03 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
00936>

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00937> Surface Area (ha) = 12.07 IMPERVIOUS 4.93 PERVIOUS (i)
00938> Dep. Storage (mm) = 1.57 4.67
00939> Average Slope (%) = .60 1.50
00940> Length (m) = 450.00 100.00
00941> Mannings n = .030 .250
00942>
00943> Max. eff. Inten. (mm/hr) = 59.23 25.04
00944> over (min) = 15.00 37.50
00945>

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00946> Storage Coeff. (min) = 14.60 (ii) 37.80 (ii)
00947> Unit Hyd. Tpeak (min) = 15.00 37.50
00948> Unit Hyd. peak (cms) = .08 .03
00949>
00950> PEAK FLOW (cms) = .91 .19 *TOTALS*
00951> TIME TO PEAK (hrs) = 1.17 1.63 1.978 (iii)
00952> RUNOFF VOLUME (mm) = 30.29 13.34 21.814
00953> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00954> RUNOFF COEFFICIENT = .95 .42 .685
00955>

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00956> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00957> CN* = 81.0 Ia = Dep. Storage (Above)
00958> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00959> THAN THE STORAGE COEFFICIENT.
00960> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00961>
00962>

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00963> 001:0017-----
00964> * SUB-AREA No.3
00965>

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00966> | CALIB STANDHYD | Area (ha) = 15.60
00967> | 04:H1P04 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
00968>

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00969> Surface Area (ha) = 11.08 IMPERVIOUS 4.52 PERVIOUS (i)
00970> Dep. Storage (mm) = 1.57 4.67
00971> Average Slope (%) = .50 1.50
00972> Length (m) = 600.00 100.00
00973> Mannings n = .030 .250
00974>
00975> Max. eff. Inten. (mm/hr) = 50.44 22.17
00976> over (min) = 20.00 45.00
00977> Storage Coeff. (min) = 20.01 (ii) 44.37 (ii)
00978> Unit Hyd. Tpeak (min) = 20.00 45.00
00979> Unit Hyd. peak (cms) = .06 .03
00980>
00981> PEAK FLOW (cms) = .69 .16 *TOTALS*
00982> TIME TO PEAK (hrs) = 1.25 1.79 .753 (iii)
00983> RUNOFF VOLUME (mm) = 30.29 13.34 21.814
00984> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00985> RUNOFF COEFFICIENT = .95 .42 .685
00986>

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00987> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00988> CN* = 81.0 Ia = Dep. Storage (Above)
00989> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00990> THAN THE STORAGE COEFFICIENT.
00991> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00992>
00993>

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00994> 001:0018-----
00995> | ADD HYD (H1P05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00996> | (ha) (cms) (hrs) (mm) (cms)
00997> ID1 03:H1P03 17.00 .978 1.17 21.81 .000
00998> +ID2 04:H1P04 15.60 .753 1.29 21.81 .000
00999>
01000> SUM 05:H1P05 32.60 1.698 1.21 21.81 .000
01001>

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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01002> 001:0019-----
01003> | ADD HYD (H1P06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01004> | (ha) (cms) (hrs) (mm) (cms)
01005> ID1 05:H1P05 32.60 1.698 1.21 21.81 .000
01006> +ID2 02:H1P02 28.46 1.039 1.25 23.90 .000
01007>
01008> SUM 06:H1P06 61.06 2.733 1.21 22.79 .000
01009>

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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01010> 001:0020-----
01011> *
01012> * SUB-AREA No.4
01013>

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01014> | CALIB STANDHYD | Area (ha) = 12.20
01015> | 07:H1P07 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
01016>

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01017> Surface Area (ha) = 8.66 IMPERVIOUS 3.54 PERVIOUS (i)
01018> Dep. Storage (mm) = 1.57 4.67
01019> Average Slope (%) = .70 1.50
01020> Length (m) = 210.00 100.00
01021> Mannings n = .030 .250
01022>
01023> Max. eff. Inten. (mm/hr) = 76.81 29.02
01024> over (min) = 7.50 30.00
01025> Storage Coeff. (min) = 8.15 (ii) 30.01 (ii)
01026> Unit Hyd. Tpeak (min) = 7.50 30.00
01027> Unit Hyd. peak (cms) = .14 .04
01028>
01029> PEAK FLOW (cms) = .91 .16 *TOTALS*
01030> TIME TO PEAK (hrs) = 1.04 1.50 .941 (iii)
01031> RUNOFF VOLUME (mm) = 30.29 13.34 21.814
01032> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
01033> RUNOFF COEFFICIENT = .95 .42 .685
01034>

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01035> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01036> CN* = 81.0 Ia = Dep. Storage (Above)
01037> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01038> THAN THE STORAGE COEFFICIENT.
01039> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01040>
01041>

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01042> 001:0021-----
01043> * SUB-AREA No.5
01044>

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01045> | DESIGN NASHYD | Area (ha) = 4.00 Curve Number (CN) = 85.00
01046> | 08:Pond-B DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00
01047> | U.H. Tp (hrs) = .170
01048>
01049> Unit Hyd Tpeak (cms) = .899
01050>
01051> PEAK FLOW (cms) = .145 (i)
01052> TIME TO PEAK (hrs) = 1.167
01053> RUNOFF VOLUME (mm) = 10.266
01054> TOTAL RAINFALL (mm) = 31.860
01055> RUNOFF COEFFICIENT = .322
01056>

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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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01057> 001:0022-----
01058> | ADD HYD (H1P08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01059> | (ha) (cms) (hrs) (mm) (cms)
01060> ID1 06:H1P06 61.06 2.733 1.21 22.79 .000
01061> +ID2 07:H1P07 12.20 .941 1.04 21.81 .000
01062> +ID3 08:Pond-B 4.00 .145 1.17 10.27 .000
01063>
01064> SUM 09:H1P08 77.26 3.542 1.21 21.98 .000
01065>

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01081> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01082>
01083>
01084>
01085> 001:0023-
01086> *
01087> *SUB-AREA No. 6
01088> *
01089>
01090> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
01091> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01092> | U.H. Tp(hrs)= .800
01093>
01094> Unit Hyd. Peak (cms)= .129
01095>
01096> PEAK FLOW (cms)= .024 (i)
01097> TIME TO PEAK (hrs)= 2.083
01098> RUNOFF VOLUME (mm)= 6.883
01099> TOTAL RAINFALL (mm)= 31.860
01100> RUNOFF COEFFICIENT = .216
01101>
01102> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01103>
01104>
01105> 001:0024-
01106>
01107> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01108> | ID1 09:HIP08 77.26 3.542 1.21 21.98 .000
01109> | +ID2 01:A3 2.70 .024 2.08 6.88 .000
01110> | SUM 02:Ultima 79.96 3.548 1.21 21.47 .000
01111>
01112>
01113>
01114> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01115>
01116>
01117> 001:0025-
01118> *****
01119> * CALCULATION OF 3HR - 1.5 YEAR STORM EVENT
01120> *****
01121>
01122> | START | Project dir.: V:\20983.DU\ENG\SWHMYMO\
01123> | TZERO = .00 hrs Rainfall dir.: V:\20983.DU\ENG\SWHMYMO\
01124> | METOUT= 2 (output= METRIC)
01125> | NRUN = 001
01126> | NSTORM= 0
01127>
01128>
01129> 001:0002-
01130>
01131> | CHICAGO STORM | IDF curve parameters: A= 998.071
01132> | Ptotal= 42.51 mm | B= 6.053
01133> | C= .814
01134> used in: INTENSITY = A / (t + B)^C
01135>
01136> Duration of storm = 3.00 hrs
01137> Storm time step = 10.00 min
01138> Time to peak ratio = .33
01139>
01140> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01141> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01142> .17 3.682 | 1.00 104.193 | 1.83 6.689 | 2.67 3.510
01143> .33 4.582 | 1.17 32.037 | 2.00 5.628 | 2.83 3.220
01144> .50 5.151 | 1.33 16.337 | 2.17 4.872 | 3.00 2.978
01145> .67 9.614 | 1.50 10.965 | 2.33 4.305 |
01146> .83 24.170 | 1.67 8.287 | 2.50 3.864 |
01147>
01148>
01149> 001:0003-
01150>
01151> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\SWHMYMO\ORGA.VAL
01152> | ICASEdv = 1 (read and print data)
01153> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
01154> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---
01155> | Horton's infiltration equation parameters:
01156> | [F0= 50.00 mm/hr] [F0= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01157> | Parameters for PERVIOUS surfaces in STANDHYD:
01158> | [IAPer= 4.67 mm] [LGP=40.00 m] [MSP= .250]
01159> | Parameters for IMPVIOUS surfaces in STANDHYD:
01160> | [IAImp= 1.57 mm] [CLI= 1.50] [MNI= .035]
01161> | Parameters used in NASHYD:
01162> | [Ia= 4.67 mm] [N= 3.00]
01163>
01164> 001:0004-
01165> *****
01166> * ORGAWORLD FILE
01167> *****
01168> * SUB-AREA No.1
01169>
01170> | CALIB STANDHYD | Area (ha)= 2.07
01171> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01172>
01173>
01174> IMPVIOUS PERVIOUS (i)
01175> Surface Area (ha)= 1.74 .33
01176> Dep. Storage (mm)= 1.57 4.67
01177> Average Slope (%)= .52 1.00
01178> Length (m)= 204.72 20.00
01179> Mannings n = .030 .250
01180>
01181> Max. eff. Inten. (mm/hr)= 104.19 24.26
01182> over (min)= 7.50 17.50
01183> Storage Coeff. (min)= 7.76 (ii) 17.86 (ii)
01184> Unit Hyd. Tpeak (min)= 7.50 17.50
01185> Unit Hyd. peak (cms)= .15 .06
01186>
01187> PEAK FLOW (cms)= .36 .01
01188> TIME TO PEAK (hrs)= 1.04 1.25
01189> RUNOFF VOLUME (mm)= 40.94 36.74
01190> TOTAL RAINFALL (mm)= 42.51 42.51
01191> RUNOFF COEFFICIENT = .96 .35
01192>
01193> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01194> CN* = 81.0 Ia = Dep. Storage (Above)
01195> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01196> THAN THE STORAGE COEFFICIENT.
01197> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01198>
01199> 001:0005-
01200> *
01201> * SUB-AREA No.2
01202>
01203> | CALIB STANDHYD | Area (ha)= 1.54
01204> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
01205>
01206>
01207> IMPVIOUS PERVIOUS (i)
01208> Surface Area (ha)= 1.42 .12
01209> Dep. Storage (mm)= 1.57 4.67
01210> Average Slope (%)= .50 1.00
01211> Length (m)= 244.34 5.00
01212> Mannings n = .030 .030
01213>
01214> Max. eff. Inten. (mm/hr)= 104.19 31.02
01215> over (min)= 7.50 10.00
01216> Storage Coeff. (min)= 8.73 (ii) 9.85 (ii)
01217>
01218>
01219>
01220>
01221>
01222>
01223>
01224>
01225> Unit Hyd. Tpeak (min)= 7.50 10.00
01226> Unit Hyd. peak (cms)= .14 .11
01227>
01228> PEAK FLOW (cms)= .28 .01
01229> TIME TO PEAK (hrs)= 1.04 1.13
01230> RUNOFF VOLUME (mm)= 40.94 14.70
01231> TOTAL RAINFALL (mm)= 42.51 42.51
01232> RUNOFF COEFFICIENT = .96 .35
01233>
01234>
01235> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01236> CN* = 81.0 Ia = Dep. Storage (Above)
01237> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01238> THAN THE STORAGE COEFFICIENT.
01239> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01240>
01241>
01242> 001:0006-
01243> *
01244> * SUB-AREA No.3
01245>
01246> | CALIB STANDHYD | Area (ha)= 1.40
01247> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01248>
01249>
01250> IMPVIOUS PERVIOUS (i)
01251> Surface Area (ha)= 1.36 .04
01252> Dep. Storage (mm)= 1.57 4.67
01253> Average Slope (%)= .51 1.00
01254> Length (m)= 225.63 5.00
01255> Mannings n = .030 .030
01256>
01257> Max. eff. Inten. (mm/hr)= 104.19 31.02
01258> over (min)= 7.50 10.00
01259> Storage Coeff. (min)= 8.28 (ii) 9.39 (ii)
01260> Unit Hyd. Tpeak (min)= 7.50 10.00
01261> Unit Hyd. peak (cms)= .14 .12
01262>
01263> PEAK FLOW (cms)= .27 .00
01264> TIME TO PEAK (hrs)= 1.04 1.13
01265> RUNOFF VOLUME (mm)= 40.94 14.70
01266> TOTAL RAINFALL (mm)= 42.51 42.51
01267> RUNOFF COEFFICIENT = .96 .35
01268>
01269>
01270> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01271> CN* = 81.0 Ia = Dep. Storage (Above)
01272> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01273> THAN THE STORAGE COEFFICIENT.
01274> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01275>
01276>
01277> 001:0007-
01278>
01279> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01280> | ID1 01:010 2.07 .362 1.04 36.75 .000
01281> | +ID2 02:020 1.54 .283 1.04 38.84 .000
01282> | SUM 04:040 3.61 .645 1.04 37.64 .000
01283>
01284>
01285> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01286>
01287>
01288>
01289> 001:0008-
01290>
01291> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01292> | ID1 03:030 1.40 .274 1.04 40.16 .000
01293> | +ID2 04:040 3.61 .645 1.04 37.64 .000
01294> | SUM 05:050 5.01 .918 1.04 38.34 .000
01295>
01296>
01297> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01298>
01299>
01300> 001:0009-
01301> * SUB-AREA No.4
01302>
01303> | CALIB STANDHYD | Area (ha)= .89
01304> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01305>
01306>
01307> IMPVIOUS PERVIOUS (i)
01308> Surface Area (ha)= .86 .03
01309> Dep. Storage (mm)= 1.57 4.67
01310> Average Slope (%)= .70 .70
01311> Length (m)= 164.82 40.00
01312> Mannings n = .030 .250
01313>
01314> Max. eff. Inten. (mm/hr)= 104.19 20.32
01315> over (min)= 5.00 25.00
01316> Storage Coeff. (min)= 5.72 (ii) 24.02 (ii)
01317> Unit Hyd. Tpeak (min)= 5.00 25.00
01318> Unit Hyd. peak (cms)= .20 .05
01319>
01320> PEAK FLOW (cms)= .20 .00
01321> TIME TO PEAK (hrs)= 1.00 1.39
01322> RUNOFF VOLUME (mm)= 40.94 14.70
01323> TOTAL RAINFALL (mm)= 42.51 42.51
01324> RUNOFF COEFFICIENT = .96 .35
01325>
01326>
01327> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01328> CN* = 81.0 Ia = Dep. Storage (Above)
01329> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01330> THAN THE STORAGE COEFFICIENT.
01331> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01332>
01333>
01334> 001:0010-
01335> * SUB-AREA No.5
01336>
01337> | CALIB STANDHYD | Area (ha)= 2.66
01338> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01339>
01340>
01341> IMPVIOUS PERVIOUS (i)
01342> Surface Area (ha)= 2.58 .08
01343> Dep. Storage (mm)= 1.57 4.67
01344> Average Slope (%)= .61 1.50
01345> Length (m)= 207.25 20.00
01346> Mannings n = .030 .250
01347>
01348> Max. eff. Inten. (mm/hr)= 104.19 24.26
01349> over (min)= 7.50 17.50
01350> Storage Coeff. (min)= 7.45 (ii) 16.40 (ii)
01351> Unit Hyd. Tpeak (min)= 7.50 17.50
01352> Unit Hyd. peak (cms)= .15 .07
01353>
01354> PEAK FLOW (cms)= .54 .00
01355> TIME TO PEAK (hrs)= 1.04 1.25
01356> RUNOFF VOLUME (mm)= 40.94 14.70
01357> TOTAL RAINFALL (mm)= 42.51 42.51
01358> RUNOFF COEFFICIENT = .96 .35
01359>
01360>
01361> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01362> CN* = 81.0 Ia = Dep. Storage (Above)
01363> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01364> THAN THE STORAGE COEFFICIENT.
01365>

```

01351> THAN THE STORAGE COEFFICIENT.  
 01352> (i.i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01353>  
 01354>  
 01355> 001:0011-  
 01357> | ADD H\*XD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01358> (ha) (cms) (hrs) (mm) (cms)  
 01359> ID1 06:060 .89 .205 1.00 40.16 .000  
 01360> +ID2 07:070 2.66 .538 1.04 40.16 .000  
 01361> SUM 08:080 3.55 .733 1.04 40.16 .000

01362>  
 01363> NOTE : PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01364>  
 01365>  
 01366> 001:0012-  
 01368> | ADD H\*XD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01369> (ha) (cms) (hrs) (mm) (cms)  
 01370> ID1 05:050 5.01 .918 1.04 38.34 .000  
 01371> +ID2 08:080 3.55 .733 1.04 40.16 .000  
 01372> SUM 09:090 8.56 1.651 1.04 39.10 .000

01373>  
 01374> NOTE : PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01375>  
 01376>  
 01377>  
 01378> 001:0013-  
 01380>  
 01381> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 01382> | IN>09: (090 ) |  
 01383> | OUT<10: (POND ) |

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
01385>	.000	.0000E+00	.593	.6251E+00
01386>	.008	.6560E-01	.654	.6631E+00
01387>	.017	.1311E+00	.797	.7381E+00
01388>	.093	.2831E+00	.950	.8274E+00
01389>	.233	.3971E+00	1.304	.9157E+00
01390>	.337	.4731E+00	1.880	.1004E+01
01391>	.465	.5491E+00	2.577	.1092E+01
01392>	.531	.5871E+00	.000	.0000E+00

01393>  
 01394>  
 01395> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 01396> (ha) (cms) (hrs) (mm)  
 01397> INFLOW>09: (090 ) 8.56 1.651 1.042 39.096  
 01398> OUTFLOW<10: (POND ) 8.56 .089 2.625 39.093

01399>  
 01400> PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.413  
 01401> TIME SHIFT OF PEAK FLOW (min) = 95.00  
 01402> MAXIMUM STORAGE USED (ha.m.) = .2758E+00

01403>  
 01404>  
 01405> 001:0014-  
 01406> \*\*\*\*\*  
 01407> Remaining Hawthorne Industrial Park  
 01408> \*  
 01409> \*  
 01410> \* SUB-AREA No.1  
 01411>  
 01412> | CALIB STANDHYD | Area (ha)= 19.90  
 01413> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01416>	14.13	5.77
01417>	1.57	4.67
01418>	1.50	1.50
01419>	580.00	100.00
01420>	.030	.250

01421>  
 01422> Max. eff. Inten. (mm/hr)= 80.14 42.65  
 01423> over (min)= 15.00 35.00  
 01424> Storage Coeff. (min)= 15.43 (ii) 34.18 (ii)  
 01425> Unit Hyd. Tpeak (min)= 15.00 35.00  
 01426> Unit Hyd. peak (cms)= .07 .03

01427>  
 01428> PEAK FLOW (cms)= 1.41 .40 \*TOTALS\*  
 01429> TIME TO PEAK (hrs)= 1.17 1.54 1.572 (iii)  
 01430> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01431> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01432> RUNOFF COEFFICIENT = .96 .50 .732

01433>  
 01434> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01435> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01436> (i.i) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01437> THAN THE STORAGE COEFFICIENT.  
 01438> (i.i.i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01439>  
 01440>  
 01441> 001:0015-  
 01442>  
 01443> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01444> (ha) (cms) (hrs) (mm) (cms)  
 01445> ID1 10:POND 8.56 .089 2.63 39.09 .000  
 01446> +ID2 01:HIP01 19.90 1.572 1.21 31.13 .000  
 01447> SUM 02:HIP02 28.46 1.615 1.21 33.52 .000

01448>  
 01449>  
 01450> NOTE : PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01451>  
 01452>  
 01453> 001:0016-  
 01454> \*  
 01455> \* SUB-AREA No.2  
 01456>  
 01457> | CALIB STANDHYD | Area (ha)= 17.00  
 01458> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01461>	12.07	4.93
01462>	1.57	4.67
01463>	.65	1.50
01464>	450.00	100.00
01465>	.030	.250

01466>  
 01467> Max. eff. Inten. (mm/hr)= 89.76 47.48  
 01468> over (min)= 12.50 30.00  
 01469> Storage Coeff. (min)= 12.36 (ii) 30.32 (ii)  
 01470> Unit Hyd. Tpeak (min)= 12.50 30.00  
 01471> Unit Hyd. peak (cms)= .09 .04

01472>  
 01473> PEAK FLOW (cms)= 1.36 .37 \*TOTALS\*  
 01474> TIME TO PEAK (hrs)= 1.13 1.46 1.267 (iii)  
 01475> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01476> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01477> RUNOFF COEFFICIENT = .96 .50 .732

01478>  
 01479> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01480> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01481> (i.i) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01482> THAN THE STORAGE COEFFICIENT.  
 01483> (i.i.i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01484>  
 01485>

01486> 001:0017-  
 01487> \*  
 01488> \* SUB-AREA No.3  
 01489>  
 01490> | CALIB STANDHYD | Area (ha)= 15.60  
 01491> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01493>	11.08	4.52
01494>	1.57	4.67
01495>	.50	1.50
01496>	600.00	100.00
01497>	.030	.250

01498>  
 01499> Max. eff. Inten. (mm/hr)= 73.27 42.65  
 01500> over (min)= 17.50 35.00  
 01501> Storage Coeff. (min)= 17.24 (ii) 35.98 (ii)  
 01502> Unit Hyd. Tpeak (min)= 17.50 35.00  
 01503> Unit Hyd. peak (cms)= .07 .03

01504>  
 01505> PEAK FLOW (cms)= 1.03 .30 \*TOTALS\*  
 01506> TIME TO PEAK (hrs)= 1.21 1.54 1.176 (iii)  
 01507> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01508> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01509> RUNOFF COEFFICIENT = .96 .50 .732

01510>  
 01511> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01512> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01513> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01514> THAN THE STORAGE COEFFICIENT.  
 01515> (i.i.i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01516>  
 01517>  
 01518> 001:0018-  
 01519>  
 01520>  
 01521> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01522> (ha) (cms) (hrs) (mm) (cms)  
 01523> ID1 03:HIP03 17.00 1.504 1.17 31.13 .000  
 01524> +ID2 04:HIP04 15.60 1.176 1.25 31.13 .000  
 01525> SUM 05:HIP05 32.60 2.621 1.17 31.13 .000

01526>  
 01527> NOTE : PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01528>  
 01529>  
 01530>  
 01531> 001:0019-  
 01532>  
 01533> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01534> (ha) (cms) (hrs) (mm) (cms)  
 01535> ID1 05:HIP05 32.60 2.621 1.17 31.13 .000  
 01536> +ID2 02:HIP02 28.46 1.615 1.21 33.52 .000  
 01537> SUM 06:HIP06 61.06 4.222 1.17 32.24 .000

01538>  
 01539> NOTE : PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01540>  
 01541>  
 01542>  
 01543> 001:0020-  
 01544> \*  
 01545> \* SUB-AREA No.4  
 01546>  
 01547> | CALIB STANDHYD | Area (ha)= 12.20  
 01548> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01551>	8.66	3.54
01552>	1.57	4.67
01553>	.70	1.50
01554>	210.00	100.00
01555>	.030	.250

01556>  
 01557> Max. eff. Inten. (mm/hr)= 104.19 52.96  
 01558> over (min)= 7.50 25.00  
 01559> Storage Coeff. (min)= 7.21 (ii) 24.40 (ii)  
 01560> Unit Hyd. Tpeak (min)= 7.50 25.00  
 01561> Unit Hyd. peak (cms)= .15 .05

01562>  
 01563> PEAK FLOW (cms)= 1.28 .31 \*TOTALS\*  
 01564> TIME TO PEAK (hrs)= 1.04 1.38 1.375 (iii)  
 01565> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01566> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01567> RUNOFF COEFFICIENT = .96 .50 .732

01568>  
 01569> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 01570> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01571> (i.i) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01572> THAN THE STORAGE COEFFICIENT.  
 01573> (i.i.i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01574>  
 01575>  
 01576> 001:0021-  
 01577> \*  
 01578> \* SUB-AREA No.5  
 01579>  
 01580> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 01581> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 01582> U.H. Tp(hrs)= .170

01583>  
 01584> Unit Hyd Qpeak (cms)= .899  
 01585>  
 01586> PEAK FLOW (cms)= .260 (i)  
 01587> TIME TO PEAK (hrs)= 1.167  
 01588> RUNOFF VOLUME (mm)= 17.325  
 01589> TOTAL RAINFALL (mm)= 42.514  
 01590> RUNOFF COEFFICIENT = .408

01591>  
 01592> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01593>  
 01594>  
 01595> 001:0022-  
 01596>  
 01597> | ADD HYD (HIP08 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01598> (ha) (cms) (hrs) (mm) (cms)  
 01599> ID1 06:HIP06 61.06 4.222 1.17 32.24 .000  
 01600> +ID2 07:HIP07 12.20 1.375 1.04 31.13 .000  
 01601> +ID3 08:Pond-B 4.00 .260 1.17 17.32 .000  
 01602> SUM 09:HIP08 77.26 5.854 1.17 31.29 .000

01603>  
 01604> NOTE : PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01605>  
 01606>  
 01607>  
 01608> 001:0023-  
 01609> \*  
 01610> \* SUB-AREA No. 6  
 01611> \*  
 01612>  
 01613> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 01614> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 01615> U.H. Tp(hrs)= .800

01616>  
 01617> Unit Hyd Qpeak (cms)= .129  
 01618>  
 01619> PEAK FLOW (cms)= .044 (i)  
 01620> TIME TO PEAK (hrs)= 2.042

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01621> RUNOFF VOLUME (mm) = 12.131
01622> TOTAL RAINFALL (mm) = 42.514
01623> RUNOFF COEFFICIENT = .285
01624>
01625> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01626>
01627>
01628> 001:002 4-----
01629>
01630> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01631> | (ha) (cms) (hrs) (mm) (cms)
01632> ID1 09:HIF08 77.26 5.545 1.17 31.29 .000
01633> +ID2 01:A3 2.70 .044 2.04 12.13 .000
01634>
01635> SUM 02:Ultima 79.96 5.554 1.17 30.65 .000
01636>
01637> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01638>
01639>
01640> 001:002 5-----
01641> *****
01642> * CALCULATION OF 3HR - 1:10 YEAR STORM EVENT *
01643> *****
01644>
01645> | START | Project dir.: V:\20983.DU\ENG\SWHYMO\
01646> | Rainfall dir.: V:\20983.DU\ENG\SWHYMO\
01647> TZERO = .00 hrs on 0
01648> METCUT = 2 (output = METRIC)
01649> NRUN = 001
01650> NSTORM = 0
01651>
01652> 001:0002-----
01653>
01654> | CHICAGO STORM | IDF curve parameters: A=1174.184
01655> | Ptotal= 49.50 mm | C= 6.014
01656> | C= 816
01657> used in: INTENSITY = A / (t + B)^C
01658>
01659> Duration of storm = 3.00 hrs
01660> Storm time step = 10.00 min
01661> Time to peak ratio = .33
01662>
01663> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01664> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01665> .17 4.248 | 1.00 122.142 | 1.83 7.733 | 2.67 4.045
01666> .33 5.290 | 1.17 37.285 | 2.00 6.502 | 2.83 3.714
01667> .50 7.108 | 1.33 18.954 | 2.17 5.625 | 3.00 3.434
01668> .67 11.130 | 1.50 12.700 | 2.33 4.969 |
01669> .83 28.100 | 1.67 9.588 | 2.50 4.458 |
01670>
01671>
01672> 001:0003-----
01673>
01674> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\SWHYMO\ORGA.VAL
01675> | ICASEdv = 1 (read and print data)
01676> FileTitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
01677> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
01678> Horton's infiltration equation parameters:
01679> [F0= 50.00 mm/hr] [F0= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01680> Parameters for PERVIOUS surfaces in STANDHYD:
01681> [Laper= 4.67 mm] [LGP=40.00 mm] [MNP= 250]
01682> Parameters for IMPERVIOUS surfaces in STANDHYD:
01683> [Lalmp= 1.57 mm] [CLI= 1.50] [MNI= .035]
01684> Parameters used in NASHVD:
01685> [Lac= 4.67 mm] [N= 3.00]
01686>
01687> 001:0004-----
01688> *****
01689> * ORGAWORLD FILE *
01690> *****
01691> * SUB-AREA No.1
01692>
01693> | CALIB STANDHYD | Area (ha)= 2.07
01694> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01695>
01696> IMPERVIOUS PERVIOUS (i)
01697> Surface Area (ha)= 1.74 .33
01698> Dep. Storage (mm)= 1.57 4.67
01699> Average Slope (%)= .52 1.00
01700> Length (m)= 204.72 20.00
01701> Mannings n = .030 .250
01702>
01703> Max. eff. Inten. (mm/hr)= 122.14 34.69
01704> over (min)= 7.50 15.00
01705> Storage Coeff. (min)= 7.28 (ii) 16.04 (ii)
01706> Unit Hyd. Tpeak (min)= 7.50 15.00
01707> Unit Hyd. peak (cms)= .15 .07
01708>
01709> PEAK FLOW (cms)= .43 .02
01710> TIME TO PEAK (hrs)= 1.04 1.21
01711> RUNOFF VOLUME (mm)= 47.93 19.25
01712> TOTAL RAINFALL (mm)= 49.50 49.50
01713> RUNOFF COEFFICIENT = .97 .39
01714>
01715> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01716> CN* = 81.0 Ia = Dep. Storage (Above)
01717> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01718> THAN THE STORAGE COEFFICIENT.
01719> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01720>
01721>
01722> 001:0005-----
01723> *
01724> * SUB-AREA No.2
01725>
01726> | CALIB STANDHYD | Area (ha)= 1.54
01727> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
01728>
01729> IMPERVIOUS PERVIOUS (i)
01730> Surface Area (ha)= 1.42 .12
01731> Dep. Storage (mm)= 1.57 4.67
01732> Average Slope (%)= .50 1.00
01733> Length (m)= 244.34 5.00
01734> Mannings n = .030 .030
01735>
01736> Max. eff. Inten. (mm/hr)= 122.14 42.32
01737> over (min)= 7.50 10.00
01738> Storage Coeff. (min)= 8.20 (ii) 9.18 (ii)
01739> Unit Hyd. Tpeak (min)= 7.50 10.00
01740> Unit Hyd. peak (cms)= .14 .12
01741>
01742> PEAK FLOW (cms)= .33 .01
01743> TIME TO PEAK (hrs)= 1.04 1.13
01744> RUNOFF VOLUME (mm)= 47.93 19.25
01745> TOTAL RAINFALL (mm)= 49.50 49.50
01746> RUNOFF COEFFICIENT = .97 .39
01747>
01748> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01749> CN* = 81.0 Ia = Dep. Storage (Above)
01750> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01751> THAN THE STORAGE COEFFICIENT.
01752> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01753>
01754>
01755> 001:0006-----

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01756> *
01757> * SUB-AREA No.3
01758>
01759> | CALIB STANDHYD | Area (ha)= 1.40
01760> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01761>
01762> IMPERVIOUS PERVIOUS (i)
01763> Surface Area (ha)= 1.36 .04
01764> Dep. Storage (mm)= 1.57 4.67
01765> Average Slope (%)= .51 1.00
01766> Length (m)= 225.63 5.00
01767> Mannings n = .030 .030
01768>
01769> Max. eff. Inten. (mm/hr)= 122.14 48.18
01770> over (min)= 7.50 7.50
01771> Storage Coeff. (min)= 7.77 (ii) 8.70 (ii)
01772> Unit Hyd. Tpeak (min)= 7.50 7.50
01773> Unit Hyd. peak (cms)= .15 .14
01774>
01775> PEAK FLOW (cms)= .33 .00
01776> TIME TO PEAK (hrs)= 1.04 1.08
01777> RUNOFF VOLUME (mm)= 47.93 19.25
01778> TOTAL RAINFALL (mm)= 49.50 49.50
01779> RUNOFF COEFFICIENT = .97 .39
01780>
01781> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01782> CN* = 81.0 Ia = Dep. Storage (Above)
01783> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01784> THAN THE STORAGE COEFFICIENT.
01785> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01786>
01787>
01788> 001:0007-----
01789>
01790> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01791> | (ha) (cms) (hrs) (mm) (cms)
01792> ID1 01:010 2.07 .437 1.04 43.35 .000
01793> +ID2 02:020 1.54 .341 1.04 45.64 .000
01794>
01795> SUM 04:040 3.61 .778 1.04 44.32 .000
01796>
01797> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01798>
01799>
01800> 001:0008-----
01801>
01802> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01803> | (ha) (cms) (hrs) (mm) (cms)
01804> ID1 03:030 1.40 .329 1.04 47.07 .000
01805> +ID2 04:040 3.61 .778 1.04 44.32 .000
01806>
01807> SUM 05:050 5.01 1.107 1.04 45.09 .000
01808>
01809> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01810>
01811>
01812> 001:0009-----
01813> *
01814> * SUB-AREA No.4
01815>
01816> | CALIB STANDHYD | Area (ha)= .89
01817> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01818>
01819> IMPERVIOUS PERVIOUS (i)
01820> Surface Area (ha)= .86 .03
01821> Dep. Storage (mm)= 1.57 4.67
01822> Average Slope (%)= .93 .70
01823> Length (m)= 164.82 40.00
01824> Mannings n = .030 .250
01825>
01826> Max. eff. Inten. (mm/hr)= 122.14 31.19
01827> over (min)= 5.00 20.00
01828> Storage Coeff. (min)= 5.37 (ii) 20.78 (ii)
01829> Unit Hyd. Tpeak (min)= 5.00 20.00
01830> Unit Hyd. peak (cms)= .21 .06
01831>
01832> PEAK FLOW (cms)= .24 .00
01833> TIME TO PEAK (hrs)= 1.00 1.29
01834> RUNOFF VOLUME (mm)= 47.93 19.25
01835> TOTAL RAINFALL (mm)= 49.50 49.50
01836> RUNOFF COEFFICIENT = .97 .39
01837>
01838> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01839> CN* = 81.0 Ia = Dep. Storage (Above)
01840> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01841> THAN THE STORAGE COEFFICIENT.
01842> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01843>
01844>
01845> 001:0010-----
01846>
01847> * SUB-AREA No.5
01848>
01849> | CALIB STANDHYD | Area (ha)= 2.66
01850> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01851>
01852> IMPERVIOUS PERVIOUS (i)
01853> Surface Area (ha)= 2.58 .08
01854> Dep. Storage (mm)= 1.57 4.67
01855> Average Slope (%)= .61 1.50
01856> Length (m)= 207.25 20.00
01857> Mannings n = .030 .250
01858>
01859> Max. eff. Inten. (mm/hr)= 122.14 34.69
01860> over (min)= 7.50 15.00
01861> Storage Coeff. (min)= 7.00 (ii) 14.75 (ii)
01862> Unit Hyd. Tpeak (min)= 7.50 15.00
01863> Unit Hyd. peak (cms)= .16 .08
01864>
01865> PEAK FLOW (cms)= .64 .00
01866> TIME TO PEAK (hrs)= 1.04 1.21
01867> RUNOFF VOLUME (mm)= 47.93 19.25
01868> TOTAL RAINFALL (mm)= 49.50 49.50
01869> RUNOFF COEFFICIENT = .97 .39
01870>
01871> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01872> CN* = 81.0 Ia = Dep. Storage (Above)
01873> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01874> THAN THE STORAGE COEFFICIENT.
01875> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01876>
01877>
01878> 001:0011-----
01879>
01880> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01881> | (ha) (cms) (hrs) (mm) (cms)
01882> ID1 06:060 .89 .245 1.00 47.07 .000
01883> +ID2 07:070 2.66 .645 1.04 47.07 .000
01884>
01885> SUM 08:080 3.55 .876 1.04 47.07 .000
01886>
01887> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01888>
01889>
01890> 001:0012-----

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02161>
02162>
02163> 001:0025-----
02164> *****
02165> * CALCULATION OF 3HR - 125 YEAR STORM EVENT *
02166> *****
02167>
02168> | START | Project dir.: V:\20983.DU\ENG\SWHMYMO\
02169> | Rainfall dir.: V:\20983.DU\ENG\SWHMYMO\
02170> | TZERO = .00 hrs on 0
02171> | METOUT= 2 (output = METRIC)
02172> | NRUN = 001
02173> | NSTORM= 0
02174>
02175> 001:0002-----
02176>
02177> | CHICAGO STORM | IDF curve parameters: A=1402.884
02178> | Ptotal= 58.23 mm | B= 6.018
02179> | C= .819
02180> | used in: INTENSITY = A / (t + B)^C
02181>
02182> | Duration of storm = 3.00 hrs
02183> | Storm time step = 10.00 min
02184> | Time to peak ratio = .33
02185>
02186>
02187>
02188>
02189>
02190>
02191>
02192>
02193>
02194>
02195> 001:0003-----
02196>
02197> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\SWHMYMO\ORGA.VAL
02198> | ICASEdv = 1 (read and print data)
02199> | FileTitle=
02200> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---|
02201> | Horton's infiltration equation parameters:
02202> | [P= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
02203> | Parameters for Pervious surfaces in STANHYD:
02204> | [Ia= 4.67 mm] [LCP= 4.00 m] [MNP= .250]
02205> | Parameters for IMPERVIOUS surfaces in STANHYD:
02206> | [Ia= 1.57 mm] [CLI= 1.50] [MNI= .035]
02207> | Parameters used in NASHYD:
02208> | [Ia = 4.67 mm] [N= 3.00]
02209>
02210> 001:0004-----
02211> *****
02212> * ORGAWORLD FILE *
02213> *****
02214> * SUB-AREA No.1
02215>
02216> | CALIB STANDHYD | Area (ha)= 2.07
02217> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
02218>
02219>
02220>
02221>
02222>
02223>
02224>
02225>
02226>
02227>
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02233>
02234>
02235>
02236>
02237>
02238>
02239>
02240>
02241>
02242>
02243>
02244>
02245> 001:0005-----
02246> *
02247> * SUB-AREA No.2
02248>
02249> | CALIB STANDHYD | Area (ha)= 1.54
02250> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
02251>
02252>
02253>
02254>
02255>
02256>
02257>
02258>
02259>
02260>
02261>
02262>
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02274>
02275>
02276>
02277>
02278> 001:0006-----
02279>
02280> * SUB-AREA No.3
02281>
02282> | CALIB STANDHYD | Area (ha)= 1.40
02283> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02284>
02285>
02286>
02287>
02288>
02289>
02290>
02291>
02292>
02293>
02294>
02295>

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02296> Unit Hyd. peak (cms)= .15 .14
02297>
02298>
02299>
02300>
02301>
02302>
02303>
02304>
02305>
02306>
02307>
02308>
02309>
02310>
02311> 001:0007-----
02312>
02313> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02314> | (ha) (cms) (hrs) (mm) (cms)
02315> | ID1 01:010 2.07 .532 1.04 51.65 .000
02316> | +ID2 02:020 1.54 .418 1.04 54.15 .000
02317> | SUM 04:040 3.61 .950 1.04 52.72 .000
02318>
02319>
02320>
02321>
02322>
02323> 001:0008-----
02324>
02325> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02326> | (ha) (cms) (hrs) (mm) (cms)
02327> | ID1 03:030 1.40 .400 1.04 55.72 .000
02328> | +ID2 04:040 3.61 .950 1.04 52.72 .000
02329> | SUM 05:050 5.01 1.350 1.04 53.55 .000
02330>
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02365>
02366>
02367>
02368> 001:0010-----
02369>
02370> * SUB-AREA No.5
02371>
02372> | CALIB STANDHYD | Area (ha)= 2.66
02373> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02374>
02375>
02376>
02377>
02378>
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02380>
02381>
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02383>
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02400>
02401> 001:0011-----
02402>
02403> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02404> | (ha) (cms) (hrs) (mm) (cms)
02405> | ID1 06:060 .89 .296 1.00 55.72 .000
02406> | +ID2 07:070 2.66 .783 1.04 55.72 .000
02407> | SUM 08:080 3.55 1.060 1.04 55.72 .000
02408>
02409>
02410>
02411>
02412>
02413> 001:0012-----
02414>
02415> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02416> | (ha) (cms) (hrs) (mm) (cms)
02417> | ID1 05:050 5.01 1.350 1.04 53.55 .000
02418> | +ID2 08:080 3.55 1.060 1.04 55.72 .000
02419> | SUM 09:090 8.56 2.410 1.04 54.45 .000
02420>
02421>
02422>
02423>
02424>
02425> 001:0013-----
02426>
02427> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
02428> | IN>09: (090 ) |
02429> | OUT<10: (POND ) |
02430>

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	(cms)	(ha.m.)	(cms)	(ha.m.)
02431>	.000	.0000E+00	.593	.6251E+00
02432>	.008	.6560E-01	.654	.6631E+00
02433>	.017	.1311E+00	.797	.7391E+00
02434>	.093	.2831E+00	.950	.8274E+00
02435>	.233	.3971E+00	1.304	.9157E+00
02436>	.337	.4731E+00	1.880	.1004E+01
02437>	.465	.5491E+00	2.577	.1092E+01
02438>	.531	.5871E+00	.000	.0000E+00

ROUTING RESULTS		AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)	
INFLOW >09: (090 )	8.56	2.410	1.042	54.451	
OUTFLOW >10: (POND )	8.56	.189	2.056	54.449	

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.838  
 TIME SHIFT OF PEAK FLOW (min) = 60.83  
 MAXIMUM STORAGE USED (ha.m.) = .3612E+00

001:0014

\*\*\*\*\* Remaining Hawthorne Industrial Park \*

\*\*\*\*\* SUB-AREA No.1

CALIB STANDHYD	Area (ha)	IMP	PERV	Dir. Conn. (%)
01:HIP01 DT= 2.50	19.90	71.00		50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	14.13	5.77
Dep. Storage (mm)	1.57	4.67
Average Slope (%)	.60	1.50
Length (m)	580.00	100.00
Mannings n	.030	.250
Max. eff. Inten. (mm/hr)	124.54	81.98
over (min)	12.50	27.50
Storage Coeff. (min)	12.93 (ii)	27.37 (ii)
Unit Hyd. Tpeak (min)	12.50	27.50
Unit Hyd. peak (cms)	.09	.04

\*TOTALS\*

	(cms)	(ha.m.)	(cms)	(ha.m.)
PEAK FLOW	2.16	.77	2.548 (iii)	
TIME TO PEAK (hrs)	1.13	1.42	1.167	
RUNOFF VOLUME (mm)	56.66	34.22	45.437	
TOTAL RAINFALL (mm)	58.23	58.23	58.226	
RUNOFF COEFFICIENT	.97	.59	.780	

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0015

CALIB STANDHYD	Area (ha)	QPEAK	TPEAK	R.V.	DWF
ADD HYD (HIP02)   ID: NHYD					
ID1 10:POND	8.56	.189	2.06	54.45	.000
+ID2 01:HIP01	19.90	2.548	1.17	45.44	.000
SUM 02:HIP02	28.46	2.622	1.17	48.15	.000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0016

CALIB STANDHYD	Area (ha)	IMP	PERV	Dir. Conn. (%)
03:HIP03 DT= 2.50	17.00	71.00		50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	12.07	4.93
Dep. Storage (mm)	1.57	4.67
Average Slope (%)	.65	1.50
Length (m)	450.00	100.00
Mannings n	.030	.250
Max. eff. Inten. (mm/hr)	144.69	87.13
over (min)	10.00	25.00
Storage Coeff. (min)	10.21 (ii)	24.30 (ii)
Unit Hyd. Tpeak (min)	10.00	25.00
Unit Hyd. peak (cms)	.11	.05

\*TOTALS\*

	(cms)	(ha.m.)	(cms)	(ha.m.)
PEAK FLOW	2.10	.71	2.398 (iii)	
TIME TO PEAK (hrs)	1.08	1.38	1.125	
RUNOFF VOLUME (mm)	56.66	34.22	45.437	
TOTAL RAINFALL (mm)	58.23	58.23	58.226	
RUNOFF COEFFICIENT	.97	.59	.780	

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0017

CALIB STANDHYD	Area (ha)	IMP	PERV	Dir. Conn. (%)
04:HIP04 DT= 2.50	15.60	71.00		50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	11.08	4.52
Dep. Storage (mm)	1.57	4.67
Average Slope (%)	.50	1.50
Length (m)	600.00	100.00
Mannings n	.030	.250
Max. eff. Inten. (mm/hr)	111.10	77.71
over (min)	15.00	30.00
Storage Coeff. (min)	14.59 (ii)	29.34 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	.08	.04

\*TOTALS\*

	(cms)	(ha.m.)	(cms)	(ha.m.)
PEAK FLOW	1.57	.57	1.879 (iii)	
TIME TO PEAK (hrs)	1.17	1.46	1.208	
RUNOFF VOLUME (mm)	56.66	34.22	45.437	
TOTAL RAINFALL (mm)	58.23	58.23	58.226	
RUNOFF COEFFICIENT	.97	.59	.780	

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0018

ADD HYD (HIP05)   ID: NHYD	AREA	QPEAK	TPEAK	R.V.	DWF
ID1 03:HIP03	17.00	2.398	1.13	45.44	.000
+ID2 04:HIP04	15.60	1.879	1.21	45.44	.000
SUM 05:HIP05	32.60	4.157	1.13	45.44	.000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0019

ADD HYD (HIP06)   ID: NHYD	AREA	QPEAK	TPEAK	R.V.	DWF
ID1 05:HIP05	32.60	4.157	1.13	45.44	.000
+ID2 02:HIP02	28.46	2.622	1.17	48.15	.000
SUM 06:HIP06	61.06	6.741	1.17	46.70	.000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0020

CALIB STANDHYD	Area (ha)	IMP	PERV	Dir. Conn. (%)
07:HIP07 DT= 2.50	12.20	71.00		50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	8.66	3.54
Dep. Storage (mm)	1.57	4.67
Average Slope (%)	.70	1.50
Length (m)	210.00	100.00
Mannings n	.030	.250
Max. eff. Inten. (mm/hr)	144.69	101.36
over (min)	7.50	20.00
Storage Coeff. (min)	6.32 (ii)	19.58 (ii)
Unit Hyd. Tpeak (min)	7.50	20.00
Unit Hyd. peak (cms)	.17	.06

\*TOTALS\*

	(cms)	(ha.m.)	(cms)	(ha.m.)
PEAK FLOW	1.86	.59	2.109 (iii)	
TIME TO PEAK (hrs)	1.04	1.29	1.042	
RUNOFF VOLUME (mm)	56.66	34.22	45.437	
TOTAL RAINFALL (mm)	58.23	58.23	58.226	
RUNOFF COEFFICIENT	.97	.59	.780	

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0021

DESIGN NASHYD	Area (ha)	IMP	PERV	Dir. Conn. (%)	Curve Number (CN)
08:Pond-B DT= 2.50	4.00				85.00
U.H. Tp (hrs)	4.670				# of Linear Res. (N) = 3.00

	(cms)	(ha.m.)	(cms)	(ha.m.)
Unit Hyd Qpeak	.899			
PEAK FLOW	.459 (i)			
TIME TO PEAK (hrs)	1.167			
RUNOFF VOLUME (mm)	29.155			
TOTAL RAINFALL (mm)	58.226			
RUNOFF COEFFICIENT	.501			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0022

ADD HYD (HIP08)   ID: NHYD	AREA	QPEAK	TPEAK	R.V.	DWF
ID1 06:HIP06	61.06	6.741	1.17	46.70	.000
+ID2 07:HIP07	12.20	2.109	1.04	45.44	.000
+ID3 08:Pond-B	4.00	.459	1.17	29.15	.000
SUM 09:HIP08	77.26	8.998	1.13	45.59	.000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0023

DESIGN NASHYD	Area (ha)	IMP	PERV	Dir. Conn. (%)	Curve Number (CN)
01:A3 DT= 2.50	2.70				76.00
U.H. Tp (hrs)	4.670				# of Linear Res. (N) = 3.00

	(cms)	(ha.m.)	(cms)	(ha.m.)
Unit Hyd Qpeak	.129			
PEAK FLOW	.079 (i)			
TIME TO PEAK (hrs)	2.000			
RUNOFF VOLUME (mm)	21.442			
TOTAL RAINFALL (mm)	58.226			
RUNOFF COEFFICIENT	.368			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0024

ADD HYD (Ultima)   ID: NHYD	AREA	QPEAK	TPEAK	R.V.	DWF
ID1 09:HIP08	77.26	8.998	1.13	45.59	.000
+ID2 01:A3	2.70	.079	2.00	21.44	.000
SUM 02:Ultima	79.96	9.013	1.13	44.78	.000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0025

\*\*\*\*\* CALCULATION OF 3HR - 1:50 YEAR STORM EVENT \*\*\*\*\*  
 02690> \*\*\*\*\*  
 02691> | START | Project dir.: V:\20983.DU\ENG\SWHMYMO\  
 02692> | TZERO = .00 hrs on | Rainfall dir.: V:\20983.DU\ENG\SWHMYMO\  
 02693> | METOUT= 2 (output = METRIC)  
 02694> | NRUN = 001  
 02695> | NSTORM= 0  
 02696> \*\*\*\*\*  
 02697> 001:0002  
 02698> | CHICAGO STORM | IDF curve parameters: A=1569.580

02701> | Ptotal = 64.81 mm | B= 6.014  
 02702> | C= .820  
 02703> | used in: INTENSITY = A / (t + B)<sup>C</sup>  
 02704> |  
 02705> | Duration of storm = 3.00 hrs  
 02706> | Storm time step = 10.00 min  
 02707> | Time to peak ratio = .33  
 02708> |  
 02709> |  
 02710> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN  
 02711> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr  
 02712> | .17 5.467 | 1.00 161.471 | 1.83 10.000 | 2.67 5.209  
 02713> | .33 6.820 | 1.17 48.876 | 2.00 8.397 | 2.83 4.774  
 02714> | .50 9.187 | 1.33 24.704 | 2.17 7.256 | 3.00 4.412  
 02715> | .67 14.441 | 1.50 16.495 | 2.33 6.403 |  
 02716> | .83 36.764 | 1.67 12.422 | 2.50 5.740 |  
 02717> |  
 02718> | 001:0003  
 02719> |  
 02720> | | DEFAULT VALUES | Filename: V:\20983.DU\ENG\SWHYMO\ORGA.VAL  
 02721> | | CASE# = 1 (read and print data)  
 02722> | | Filetitle = ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE  
 02723> | | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60  
 02724> | | Horton's infiltration equation parameters:  
 02725> | | [F= 50.00 mm/hr] [fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]  
 02726> | | Parameters for PERVIOUS surfaces in STANDHYD:  
 02727> | | [IAPER= 4.67 mm] [LGP= 40.00 mm] [MNI= .250]  
 02728> | | Parameters for IMPERVIOUS surfaces in STANDHYD:  
 02729> | | [IIMP= 1.57 mm] [CLI= 1.50] [MNI= .035]  
 02730> | | Parameters used in NASHYD:  
 02731> | | [Ia= 4.67 mm] [N= 3.00]  
 02732> | |  
 02733> | 001:0004  
 02734> | \*\*\*\*\*  
 02735> | \* ORGAWORLD FILE  
 02736> | \*\*\*\*\*  
 02737> | \* SUB-AREA No.1  
 02738> | |  
 02739> | | CALIB STANDHYD | Area (ha)= 2.07  
 02740> | | 01:010 DT= 2.50 | Total Imp(h)= 84.00 Dir. Conn.(%)= 84.00  
 02741> | |  
 02742> | | IMPERVIOUS PERVIOUS (i)  
 02743> | | Surface Area (ha)= 1.74 .33  
 02744> | | Dep. Storage (mm)= 1.57 4.67  
 02745> | | Average Slope (ft)= .52 1.00  
 02746> | | Length (m)= 204.72 20.00  
 02747> | | Mannings n = .030 .250  
 02748> | |  
 02749> | | Max. eff. Inten. (mm/hr)= 161.47 62.27  
 02750> | | over (min)= 7.50 12.50  
 02751> | | Storage Coeff. (min)= 6.51 (ii) 13.44 (ii)  
 02752> | | Unit Hyd. Tpeak (min)= 7.50 12.50  
 02753> | | Unit Hyd. peak (cms)= .16 .09  
 02754> | |  
 02755> | | PEAK FLOW (cms)= .59 .03 \*TOTALS\*  
 02756> | | TIME TO PEAK (hrs)= 1.04 1.17 1.609 (iii)  
 02757> | | RUNOFF VOLUME (mm)= 63.24 30.21 57.952  
 02758> | | TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02759> | | RUNOFF COEFFICIENT = .98 .47 .894  
 02760> | |  
 02761> | | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02762> | | CN\* = 81.0 Ia = Dep. Storage (Above)  
 02763> | | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02764> | | THAN THE STORAGE COEFFICIENT.  
 02765> | | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02766> | |  
 02767> | |  
 02768> | 001:0005  
 02769> | \*  
 02770> | \* SUB-AREA No.2  
 02771> | |  
 02772> | | CALIB STANDHYD | Area (ha)= 1.54  
 02773> | | 02:020 DT= 2.50 | Total Imp(h)= 92.00 Dir. Conn.(%)= 92.00  
 02774> | |  
 02775> | | IMPERVIOUS PERVIOUS (i)  
 02776> | | Surface Area (ha)= 1.42 .12  
 02777> | | Dep. Storage (mm)= 1.57 4.67  
 02778> | | Average Slope (ft)= .50 1.00  
 02779> | | Length (m)= 244.34 5.00  
 02780> | | Mannings n = .030 .030  
 02781> | |  
 02782> | | Max. eff. Inten. (mm/hr)= 161.47 78.73  
 02783> | | over (min)= 7.50 7.50  
 02784> | | Storage Coeff. (min)= 7.33 (ii) 8.10 (ii)  
 02785> | | Unit Hyd. Tpeak (min)= 7.50 7.50  
 02786> | | Unit Hyd. peak (cms)= .15 .14  
 02787> | |  
 02788> | | PEAK FLOW (cms)= .46 .02 \*TOTALS\*  
 02789> | | TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 02790> | | RUNOFF VOLUME (mm)= 63.24 30.21 60.594  
 02791> | | TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02792> | | RUNOFF COEFFICIENT = .98 .47 .935  
 02793> | |  
 02794> | | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02795> | | CN\* = 81.0 Ia = Dep. Storage (Above)  
 02796> | | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02797> | | THAN THE STORAGE COEFFICIENT.  
 02798> | | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02799> | |  
 02800> | |  
 02801> | 001:0006  
 02802> | \*  
 02803> | \* SUB-AREA No.3  
 02804> | |  
 02805> | | CALIB STANDHYD | Area (ha)= 1.40  
 02806> | | 03:030 DT= 2.50 | Total Imp(h)= 97.00 Dir. Conn.(%)= 97.00  
 02807> | |  
 02808> | | IMPERVIOUS PERVIOUS (i)  
 02809> | | Surface Area (ha)= 1.36 .04  
 02810> | | Dep. Storage (mm)= 1.57 4.67  
 02811> | | Average Slope (ft)= .51 1.00  
 02812> | | Length (m)= 225.63 5.00  
 02813> | | Mannings n = .030 .030  
 02814> | |  
 02815> | | Max. eff. Inten. (mm/hr)= 161.47 78.73  
 02816> | | over (min)= 7.50 7.50  
 02817> | | Storage Coeff. (min)= 6.95 (ii) 7.72 (ii)  
 02818> | | Unit Hyd. Tpeak (min)= 7.50 7.50  
 02819> | | Unit Hyd. peak (cms)= .16 .15  
 02820> | |  
 02821> | | PEAK FLOW (cms)= .45 .01 \*TOTALS\*  
 02822> | | TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 02823> | | RUNOFF VOLUME (mm)= 63.24 30.21 62.245  
 02824> | | TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02825> | | RUNOFF COEFFICIENT = .98 .47 .960  
 02826> | |  
 02827> | | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02828> | | CN\* = 81.0 Ia = Dep. Storage (Above)  
 02829> | | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02830> | | THAN THE STORAGE COEFFICIENT.  
 02831> | | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02832> | |  
 02833> | |  
 02834> | 001:0007  
 02835> | |

02836> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02837> | (ha) (cms) (hrs) (mm) (cms)  
 02838> | ID1 01:010 2.07 .609 1.04 57.95 .000  
 02839> | +ID2 02:020 1.54 .475 1.04 60.59 .000  
 02840> |  
 02841> | SUM 04:040 3.61 1.084 1.04 59.08 .000  
 02842> |  
 02843> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02844> |  
 02845> |  
 02846> | 001:0008  
 02847> | |  
 02848> | | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02849> | | (ha) (cms) (hrs) (mm) (cms)  
 02850> | | ID1 03:030 1.40 .454 1.04 62.25 .000  
 02851> | | +ID2 04:040 3.61 1.084 1.04 59.08 .000  
 02852> | |  
 02853> | | SUM 05:050 5.01 1.538 1.04 59.96 .000  
 02854> | |  
 02855> | | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02856> | |  
 02857> | |  
 02858> | 001:0009  
 02859> | \*  
 02860> | \* SUB-AREA No.4  
 02861> | |  
 02862> | | CALIB STANDHYD | Area (ha)= .89  
 02863> | | 06:060 DT= 2.50 | Total Imp(h)= 97.00 Dir. Conn.(%)= 97.00  
 02864> | |  
 02865> | | IMPERVIOUS PERVIOUS (i)  
 02866> | | Surface Area (ha)= .86 .03  
 02867> | | Dep. Storage (mm)= 1.57 4.67  
 02868> | | Average Slope (ft)= .93 .70  
 02869> | | Length (m)= 164.82 40.00  
 02870> | | Mannings n = .030 .250  
 02871> | |  
 02872> | | Max. eff. Inten. (mm/hr)= 161.47 53.28  
 02873> | | over (min)= 5.00 17.50  
 02874> | | Storage Coeff. (min)= 4.80 (ii) 17.24 (ii)  
 02875> | | Unit Hyd. Tpeak (min)= 5.00 17.50  
 02876> | | Unit Hyd. peak (cms)= .23 .07  
 02877> | |  
 02878> | | PEAK FLOW (cms)= .33 .00 \*TOTALS\*  
 02879> | | TIME TO PEAK (hrs)= 1.00 1.25 .335 (iii)  
 02880> | | RUNOFF VOLUME (mm)= 63.24 30.21 62.245  
 02881> | | TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02882> | | RUNOFF COEFFICIENT = .98 .47 .960  
 02883> | |  
 02884> | | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02885> | | CN\* = 81.0 Ia = Dep. Storage (Above)  
 02886> | | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02887> | | THAN THE STORAGE COEFFICIENT.  
 02888> | | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02889> | |  
 02890> | |  
 02891> | 001:0010  
 02892> | \*  
 02893> | \* SUB-AREA No.5  
 02894> | |  
 02895> | | CALIB STANDHYD | Area (ha)= 2.66  
 02896> | | 07:070 DT= 2.50 | Total Imp(h)= 97.00 Dir. Conn.(%)= 97.00  
 02897> | |  
 02898> | | IMPERVIOUS PERVIOUS (i)  
 02899> | | Surface Area (ha)= 2.58 .08  
 02900> | | Dep. Storage (mm)= 1.57 4.67  
 02901> | | Average Slope (ft)= .61 1.50  
 02902> | | Length (m)= 207.25 20.00  
 02903> | | Mannings n = .030 .250  
 02904> | |  
 02905> | | Max. eff. Inten. (mm/hr)= 161.47 62.27  
 02906> | | over (min)= 7.50 12.50  
 02907> | | Storage Coeff. (min)= 6.26 (ii) 12.39 (ii)  
 02908> | | Unit Hyd. Tpeak (min)= 7.50 12.50  
 02909> | | Unit Hyd. peak (cms)= .17 .09  
 02910> | |  
 02911> | | PEAK FLOW (cms)= .88 .01 \*TOTALS\*  
 02912> | | TIME TO PEAK (hrs)= 1.04 1.17 .886 (iii)  
 02913> | | RUNOFF VOLUME (mm)= 63.24 30.21 62.245  
 02914> | | TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02915> | | RUNOFF COEFFICIENT = .98 .47 .960  
 02916> | |  
 02917> | | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02918> | | CN\* = 81.0 Ia = Dep. Storage (Above)  
 02919> | | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02920> | | THAN THE STORAGE COEFFICIENT.  
 02921> | | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02922> | |  
 02923> | |  
 02924> | 001:0011  
 02925> | \*  
 02926> | | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02927> | | (ha) (cms) (hrs) (mm) (cms)  
 02928> | | ID1 06:060 .89 .335 1.00 62.25 .000  
 02929> | | +ID2 07:070 2.66 .886 1.04 62.25 .000  
 02930> | |  
 02931> | | SUM 08:080 3.55 1.197 1.04 62.25 .000  
 02932> | |  
 02933> | | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02934> | |  
 02935> | |  
 02936> | 001:0012  
 02937> | \*  
 02938> | | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02939> | | (ha) (cms) (hrs) (mm) (cms)  
 02940> | | ID1 05:050 5.01 1.538 1.04 59.96 .000  
 02941> | | +ID2 06:060 3.55 1.197 1.04 62.25 .000  
 02942> | |  
 02943> | | SUM 09:090 8.56 2.735 1.04 60.91 .000  
 02944> | |  
 02945> | | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02946> | |  
 02947> | |  
 02948> | 001:0013  
 02949> | \*  
 02950> | | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 02951> | | IN>09: (090 ) |  
 02952> | | OUT<10: (POND ) |  
 02953> | |  
 02954> | |  
 02955> | |  
 02956> | |  
 02957> | |  
 02958> | |  
 02959> | |  
 02960> | |  
 02961> | |  
 02962> | |  
 02963> | |  
 02964> | | ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 02965> | | (ha) (cms) (hrs) (mm)  
 02966> | | INFLOW >09: (090 ) 8.56 2.735 1.042 60.910  
 02967> | | OUTFLOW <10: (POND ) 8.56 .233 1.944 60.908  
 02968> | |  
 02969> | | PEAK FLOW REDUCTION [Qout/Qin] (%)= 8.503  
 02970> | | TIME SHIFT OF PEAK FLOW (min)= 54.17

02971> MAXIMUM STORAGE USED (ha.m.) = 3967E+00  
 02972>  
 02973>  
 02974> 001:0014-----  
 02975> \*\*\*\*\*  
 02976> \* Remaining Hawthorne Industrial Park \*  
 02977> \*\*\*\*\*  
 02978> \*  
 02979> \* SUB-AREA No.1  
 02980> | CALIB STANDHYD | Area (ha) = 19.90  
 02981> | 01:HIP01 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 02982>  
 02983>  
 02984> IMPERVIOUS PERVIOUS (i)  
 02985> Surface Area (ha) = 14.13 5.77  
 02986> Dep. Storage (mm) = 1.57 4.67  
 02987> Average Slope (%) = .60 1.50  
 02988> Length (m) = 580.00 100.00  
 02989> Mannings n = .030 .250  
 02990>  
 02991> Max. eff. Inten. (mm/hr) = 138.95 102.13  
 02992> over (min) 12.50 25.00  
 02993> Storage Coeff. (min) = 12.38 (ii) 25.60 (ii)  
 02994> Unit Hyd. Tpeak (min) = 12.50 25.00  
 02995> Unit Hyd. peak (cms) = .09 .04  
 02996>  
 02997> PEAK FLOW (cms) = 2.46 .95 \*TOTALS\*  
 02998> TIME TO PEAK (hrs) = 1.13 1.38 3.001 (iii)  
 02999> RUNOFF VOLUME (mm) = 63.24 39.90 51.566  
 03000> TOTAL RAINFALL (mm) = 64.81 64.81 64.806  
 03001> RUNOFF COEFFICIENT = .98 .62 .796  
 03002>  
 03003> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03004> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03005> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03006> THAN THE STORAGE COEFFICIENT.  
 03007> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03008>  
 03009> 001:0015-----  
 03010> | ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03011> (ha) (cms) (hrs) (mm) (cms)  
 03012> ID1 10:POND 8.56 .233 1.94 60.91 .000  
 03013> +ID2 01:HIP01 19.90 3.001 1.17 51.57 .000  
 03014>  
 03015> SUM 02:HIP02 28.46 3.092 1.17 54.37 .000  
 03016>  
 03017> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03018>  
 03019>  
 03020>  
 03021> 001:0016-----  
 03022> \* SUB-AREA No.2  
 03023>  
 03024> | CALIB STANDHYD | Area (ha) = 17.00  
 03025> | 03:HIP03 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 03026>  
 03027>  
 03028>  
 03029> IMPERVIOUS PERVIOUS (i)  
 03030> Surface Area (ha) = 12.07 4.93  
 03031> Dep. Storage (mm) = 1.57 4.67  
 03032> Average Slope (%) = .65 1.50  
 03033> Length (m) = 450.00 100.00  
 03034> Mannings n = .030 .250  
 03035>  
 03036> Max. eff. Inten. (mm/hr) = 161.47 109.61  
 03037> over (min) 10.00 22.50  
 03038> Storage Coeff. (min) = 9.77 (ii) 22.63 (ii)  
 03039> Unit Hyd. Tpeak (min) = 10.00 22.50  
 03040> Unit Hyd. peak (cms) = .11 .05  
 03041>  
 03042> PEAK FLOW (cms) = 2.38 .88 \*TOTALS\*  
 03043> TIME TO PEAK (hrs) = 1.08 1.33 1.125  
 03044> RUNOFF VOLUME (mm) = 63.24 39.90 51.566  
 03045> TOTAL RAINFALL (mm) = 64.81 64.81 64.806  
 03046> RUNOFF COEFFICIENT = .98 .62 .796  
 03047>  
 03048> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03049> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03050> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03051> THAN THE STORAGE COEFFICIENT.  
 03052> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03053>  
 03054> 001:0017-----  
 03055> \* SUB-AREA No.3  
 03056>  
 03057> | CALIB STANDHYD | Area (ha) = 15.60  
 03058> | 04:HIP04 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 03059>  
 03060>  
 03061>  
 03062> IMPERVIOUS PERVIOUS (i)  
 03063> Surface Area (ha) = 11.08 4.52  
 03064> Dep. Storage (mm) = 1.57 4.67  
 03065> Average Slope (%) = .50 1.50  
 03066> Length (m) = 600.00 100.00  
 03067> Mannings n = .030 .250  
 03068>  
 03069> Max. eff. Inten. (mm/hr) = 138.95 96.02  
 03070> over (min) 12.50 27.50  
 03071> Storage Coeff. (min) = 13.34 (ii) 26.90 (ii)  
 03072> Unit Hyd. Tpeak (min) = 12.50 27.50  
 03073> Unit Hyd. peak (cms) = .09 .04  
 03074>  
 03075> PEAK FLOW (cms) = 1.86 .72 \*TOTALS\*  
 03076> TIME TO PEAK (hrs) = 1.13 1.42 1.167  
 03077> RUNOFF VOLUME (mm) = 63.24 39.90 51.566  
 03078> TOTAL RAINFALL (mm) = 64.81 64.81 64.806  
 03079> RUNOFF COEFFICIENT = .98 .62 .796  
 03080>  
 03081> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03082> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03083> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03084> THAN THE STORAGE COEFFICIENT.  
 03085> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03086>  
 03087> 001:0018-----  
 03088> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03089> (ha) (cms) (hrs) (mm) (cms)  
 03090> ID1 03:HIP03 17.00 2.819 1.13 51.57 .000  
 03091> +ID2 04:HIP04 15.60 2.237 1.17 51.57 .000  
 03092>  
 03093> SUM 05:HIP05 32.60 5.019 1.13 51.57 .000  
 03094>  
 03095> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03096>  
 03097>  
 03098>  
 03099> 001:0019-----  
 03100> | ADD HYD (HIP06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03101> (ha) (cms) (hrs) (mm) (cms)  
 03102> ID1 05:HIP05 32.60 5.019 1.13 51.57 .000  
 03103> +ID2 02:HIP02 28.46 3.092 1.17 54.37 .000  
 03104>  
 03105>

03106> SUM 06:HIP06 61.06 8.054 1.13 52.87 .000  
 03107>  
 03108> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03109>  
 03110>  
 03111>  
 03112> 001:0020-----  
 03113> \* SUB-AREA No.4  
 03114>  
 03115> | CALIB STANDHYD | Area (ha) = 12.20  
 03116> | 07:HIP07 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00  
 03117>  
 03118>  
 03119> IMPERVIOUS PERVIOUS (i)  
 03120> Surface Area (ha) = 8.66 3.54  
 03121> Dep. Storage (mm) = 1.57 4.67  
 03122> Average Slope (%) = .70 1.50  
 03123> Length (m) = 210.00 100.00  
 03124> Mannings n = .030 .250  
 03125>  
 03126> Max. eff. Inten. (mm/hr) = 161.47 126.32  
 03127> over (min) 5.00 17.50  
 03128> Storage Coeff. (min) = 6.05 (ii) 18.19 (ii)  
 03129> Unit Hyd. Tpeak (min) = 5.00 17.50  
 03130> Unit Hyd. peak (cms) = .20 .06  
 03131>  
 03132> PEAK FLOW (cms) = 2.19 .73 \*TOTALS\*  
 03133> TIME TO PEAK (hrs) = 1.00 1.25 1.042 (iii)  
 03134> RUNOFF VOLUME (mm) = 63.24 39.90 51.566  
 03135> TOTAL RAINFALL (mm) = 64.81 64.81 64.806  
 03136> RUNOFF COEFFICIENT = .98 .62 .796  
 03137>  
 03138> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03139> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03140> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03141> THAN THE STORAGE COEFFICIENT.  
 03142> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03143>  
 03144> 001:0021-----  
 03145> \* SUB-AREA No.5  
 03146>  
 03147> | DESIGN NASHYD | Area (ha) = 4.00 Curve Number (CN) = 85.00  
 03148> | 08:Pond-B DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00  
 03149> U.H. Tp (hrs) = .170  
 03150>  
 03151> Unit Hyd. Tpeak (cms) = .899  
 03152>  
 03153> PEAK FLOW (cms) = .551 (i)  
 03154> TIME TO PEAK (hrs) = 1.125  
 03155> RUNOFF VOLUME (mm) = 34.455  
 03156> TOTAL RAINFALL (mm) = 64.806  
 03157> RUNOFF COEFFICIENT = .532  
 03158>  
 03159> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03160>  
 03161>  
 03162> 001:0022-----  
 03163> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03164> (ha) (cms) (hrs) (mm) (cms)  
 03165> ID1 06:HIP06 61.06 8.054 1.13 52.87 .000  
 03166> +ID2 07:HIP07 12.20 2.470 1.04 51.57 .000  
 03167> +ID3 08:Pond-B 4.00 .551 1.13 34.45 .000  
 03168>  
 03169> SUM 09:HIP08 77.26 10.570 1.13 51.71 .000  
 03170>  
 03171> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03172>  
 03173>  
 03174> 001:0023-----  
 03175> \* SUB-AREA No. 6  
 03176>  
 03177> | DESIGN NASHYD | Area (ha) = 2.70 Curve Number (CN) = 76.00  
 03178> | 01:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00  
 03179> U.H. Tp (hrs) = .800  
 03180>  
 03181> Unit Hyd. Tpeak (cms) = .129  
 03182>  
 03183> PEAK FLOW (cms) = .096 (i)  
 03184> TIME TO PEAK (hrs) = 1.958  
 03185> RUNOFF VOLUME (mm) = 25.767  
 03186> TOTAL RAINFALL (mm) = 64.806  
 03187> RUNOFF COEFFICIENT = .398  
 03188>  
 03189> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03190>  
 03191>  
 03192> 001:0024-----  
 03193> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03194> (ha) (cms) (hrs) (mm) (cms)  
 03195> ID1 09:HIP08 77.26 10.570 1.13 51.71 .000  
 03196> +ID2 01:A3 2.70 .096 1.96 25.77 .000  
 03197>  
 03198> SUM 02:Ultima 79.96 10.588 1.13 50.84 .000  
 03199>  
 03200> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03201>  
 03202>  
 03203>  
 03204>  
 03205>  
 03206>  
 03207>  
 03208> 001:0025-----  
 03209> \*\*\*\*\*  
 03210> \* CALCULATION OF 3HR - 1:100 YEAR STORM EVENT \*  
 03211> \*\*\*\*\*  
 03212>  
 03213>  
 03214> | START | Project dir.: V:\20983.DU\ENG\SWHYMO\  
 03215> Rainfall dir.: V:\20983.DU\ENG\SWHYMO\  
 03216> TZERO = .00 hrs on 0  
 03217> METOUT = 4 (output = METRIC)  
 03218> NRUN = 001  
 03219> NSTORM = 0  
 03220>  
 03221> 001:0002-----  
 03222> | CHICAGO STORM | IDF curve parameters: A=1735.688  
 03223> | Ptotal= 71.66 mm | B= 6.014  
 03224> C= .820  
 03225>  
 03226> used in: INTENSITY = A / (t + B) ^ C  
 03227>  
 03228> Duration of storm = 3.00 hrs  
 03229> Storm time step = 10.00 min  
 03230> Time to peak ratio = .33  
 03231>  
 03232> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN  
 03233> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr  
 03234> .17 6.046 | 1.00 178.559 | 1.83 11.059 | 2.67 5.760  
 03235> .33 7.542 | 1.17 54.049 | 2.00 9.285 | 2.83 5.280  
 03236> .67 10.159 | 1.33 27.319 | 2.17 8.024 | 3.00 4.879  
 03237> .83 15.969 | 1.50 18.240 | 2.33 7.080 |  
 03238> .83 40.655 | 1.67 13.737 | 2.50 6.347 |  
 03239>  
 03240>

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03241> 001:0003-----
03242>
03243> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\SWH\HYMO\ORGA.VAL
03244> | ICASEddy = 1 (read and print data)
03245> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
03246> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----|
03247> Horton's infiltration equation parameters:
03248> [Fo= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
03249> Parameters for PERVIOUS surfaces in STANDHYD:
03250> [I.Aper= 4.67 mm] [I.Cp=40.00 mm] [MNP=.250]
03251> Parameters for PERVIOUS surfaces in STANDHYD:
03252> [I.AImp= 1.57 mm] [CL= 1.50] [MNI=.035]
03253> Parameters used in NASHYD:
03254> [Ia= 4.67 mm] [N= 3.00]
03255>
03256> 001:0004-----
03257> *****
03258> * ORGAWORLD FILE *
03259> *****
03260> * SUB-AREA No.1
03261>
03262> | CALIB STANDHYD | Area (ha)= 2.07
03263> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
03264>
03265> IMPERVIOUS PERVIOUS (i)
03266> Surface Area (ha)= 1.74 .33
03267> Dep. Storage (mm)= 1.57 4.67
03268> Average Slope (%)= .52 1.00
03269> Length (m)= 204.72 20.00
03270> Mannings n = .030 .250
03271>
03272> Max.eff.Inten.(mm/hr)= 178.56 74.05
03273> over (min) 7.50 12.50
03274> Storage Coeff. (min)= 6.26 (ii) 12.72 (ii)
03275> Unit Hyd. Tpeak (min)= 7.50 12.50
03276> Unit Hyd. peak (cms)= .17 .09
03277>
03278> PEAK FLOW (cms)= .66 .04 *TOTALS*
03279> TIME TO PEAK (hrs)= 1.04 1.17 .685 (iii)
03280> RUNOFF VOLUME (mm)= 70.09 35.46 64.553
03281> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03282> RUNOFF COEFFICIENT = .98 .49 .901
03283>
03284> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03285> CN* = 81.0 Ia = Dep. Storage (Above)
03286> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03287> THAN THE STORAGE COEFFICIENT.
03288> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03289>
03290>
03291> 001:0005-----
03292> *
03293> * SUB-AREA No.2
03294>
03295> | CALIB STANDHYD | Area (ha)= 1.54
03296> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
03297>
03298> IMPERVIOUS PERVIOUS (i)
03299> Surface Area (ha)= 1.42 .12
03300> Dep. Storage (mm)= 1.57 4.67
03301> Average Slope (%)= 1.50 1.00
03302> Length (m)= 244.34 5.00
03303> Mannings n = .030 .030
03304>
03305> Max.eff.Inten.(mm/hr)= 178.56 93.23
03306> over (min) 7.50 12.50
03307> Storage Coeff. (min)= 7.04 (ii) 7.76 (ii)
03308> Unit Hyd. Tpeak (min)= 7.50 7.50
03309> Unit Hyd. peak (cms)= .16 .15
03310>
03311> PEAK FLOW (cms)= .51 .02 *TOTALS*
03312> TIME TO PEAK (hrs)= 1.04 1.08 1.042 (iii)
03313> RUNOFF VOLUME (mm)= 70.09 35.46 67.324
03314> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03315> RUNOFF COEFFICIENT = .98 .49 .939
03316>
03317> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03318> CN* = 81.0 Ia = Dep. Storage (Above)
03319> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03320> THAN THE STORAGE COEFFICIENT.
03321> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03322>
03323>
03324> 001:0006-----
03325> *
03326> * SUB-AREA No.3
03327>
03328> | CALIB STANDHYD | Area (ha)= 1.40
03329> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03330>
03331> IMPERVIOUS PERVIOUS (i)
03332> Surface Area (ha)= 1.36 .04
03333> Dep. Storage (mm)= 1.57 4.67
03334> Average Slope (%)= .51 1.00
03335> Length (m)= 225.63 5.00
03336> Mannings n = .030 .030
03337>
03338> Max.eff.Inten.(mm/hr)= 178.56 93.23
03339> over (min) 7.50 7.50
03340> Storage Coeff. (min)= 6.67 (ii) 7.39 (ii)
03341> Unit Hyd. Tpeak (min)= 7.50 7.50
03342> Unit Hyd. peak (cms)= .16 .15
03343>
03344> PEAK FLOW (cms)= .50 .01 *TOTALS*
03345> TIME TO PEAK (hrs)= 1.04 1.08 1.042 (iii)
03346> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
03347> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03348> RUNOFF COEFFICIENT = .98 .49 .964
03349>
03350> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03351> CN* = 81.0 Ia = Dep. Storage (Above)
03352> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03353> THAN THE STORAGE COEFFICIENT.
03354> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03355>
03356>
03357> 001:0007-----
03358> *
03359> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03360> | (ha) (cms) (hrs) (mm) (cms)
03361> ID1 01:010 2.07 .685 1.04 64.55 .000
03362> +ID2 02:020 1.54 .534 1.04 67.32 .000
03363>
03364> SUM 04:040 3.61 1.220 1.04 65.74 .000
03365>
03366> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03367>
03368>
03369> 001:0008-----
03370> *
03371> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03372> | (ha) (cms) (hrs) (mm) (cms)
03373> ID1 03:030 1.40 .509 1.04 69.06 .000
03374> +ID2 04:040 3.61 1.220 1.04 65.74 .000
03375>

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03376> SUM 05:050 5.01 1.729 1.04 66.66 .000
03377>
03378> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03379>
03380>
03381> 001:0009-----
03382> *
03383> * SUB-AREA No.4
03384>
03385> | CALIB STANDHYD | Area (ha)= .89
03386> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03387>
03388> IMPERVIOUS PERVIOUS (i)
03389> Surface Area (ha)= .86 .03
03390> Dep. Storage (mm)= 1.57 4.67
03391> Average Slope (%)= .93 .70
03392> Length (m)= 164.82 40.00
03393> Mannings n = .030 .250
03394>
03395> Max.eff.Inten.(mm/hr)= 178.56 67.61
03396> over (min) 5.00 15.00
03397> Storage Coeff. (min)= 4.62 (ii) 15.92 (ii)
03398> Unit Hyd. Tpeak (min)= 5.00 15.00
03399> Unit Hyd. peak (cms)= .24 .07
03400>
03401> PEAK FLOW (cms)= .37 .00 *TOTALS*
03402> TIME TO PEAK (hrs)= 1.00 1.21 1.000
03403> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
03404> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03405> RUNOFF COEFFICIENT = .98 .49 .964
03406>
03407> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03408> CN* = 81.0 Ia = Dep. Storage (Above)
03409> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03410> THAN THE STORAGE COEFFICIENT.
03411> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03412>
03413>
03414> 001:0010-----
03415> *
03416> * SUB-AREA No.5
03417>
03418> | CALIB STANDHYD | Area (ha)= 2.66
03419> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03420>
03421> IMPERVIOUS PERVIOUS (i)
03422> Surface Area (ha)= 2.58 .08
03423> Dep. Storage (mm)= 1.57 4.67
03424> Average Slope (%)= .61 1.50
03425> Length (m)= 207.25 20.00
03426> Mannings n = .030 .250
03427>
03428> Max.eff.Inten.(mm/hr)= 178.56 74.05
03429> over (min) 5.00 12.50
03430> Storage Coeff. (min)= 6.01 (ii) 11.73 (ii)
03431> Unit Hyd. Tpeak (min)= 5.00 12.50
03432> Unit Hyd. peak (cms)= .20 .09
03433>
03434> PEAK FLOW (cms)= 1.03 .01 *TOTALS*
03435> TIME TO PEAK (hrs)= 1.00 1.17 1.000
03436> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
03437> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03438> RUNOFF COEFFICIENT = .98 .49 .964
03439>
03440> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03441> CN* = 81.0 Ia = Dep. Storage (Above)
03442> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03443> THAN THE STORAGE COEFFICIENT.
03444> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03445>
03446>
03447> 001:0011-----
03448> *
03449> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03450> | (ha) (cms) (hrs) (mm) (cms)
03451> ID1 06:060 .89 .374 1.00 69.06 .000
03452> +ID2 07:070 2.66 1.034 1.00 69.06 .000
03453>
03454> SUM 08:080 3.55 1.408 1.00 69.06 .000
03455>
03456> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03457>
03458>
03459> 001:0012-----
03460> *
03461> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03462> | (ha) (cms) (hrs) (mm) (cms)
03463> ID1 05:050 5.01 1.729 1.04 66.66 .000
03464> +ID2 08:080 3.55 1.408 1.00 69.06 .000
03465>
03466> SUM 09:090 8.56 3.067 1.04 67.66 .000
03467>
03468> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03469>
03470>
03471> 001:0013-----
03472> *
03473> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03474> | IN>09: (090 ) |
03475> | OUT<10: (POND ) |
03476>
03477> ===== OUTFLOW STORAGE TABLE =====
03478> OUTFLOW STORAGE | OUTFLOW STORAGE
03479> (cms) (ha.m.) | (cms) (ha.m.)
03480> .000 .0000E+00 | .593 .6251E+00
03481> .008 .6560E-01 | .654 .6631E+00
03482> .017 .1311E+00 | .797 .7391E+00
03483> .093 .2831E+00 | .950 .8274E+00
03484> .233 .3971E+00 | 1.304 .9157E+00
03485> .337 .4731E+00 | 1.880 .1004E+01
03486> .465 .5491E+00 | 2.577 .1092E+01
03487> .531 .5871E+00 | .000 .0000E+00
03488>
03489> ROUTING RESULTS AREA QPEAK TPEAK R.V.
03490> (ha) (cms) (hrs) (mm)
03491> INFLOW >09: (090 ) 8.56 3.067 1.042 67.655
03492> OUTFLOW<10: (POND ) 8.56 .283 1.861 67.653
03493>
03494> PEAK FLOW REDUCTION (Qout/Qin)(%)= 9.214
03495> TIME SHIFT OF PEAK FLOW (min)= 49.17
03496> MAXIMUM STORAGE USED (ha.m.)=.4333E+00
03497>
03498> 001:0014-----
03499> *
03500> * Remaining Hawthorne Industrial Park *
03501> *
03502> * SUB-AREA No.1
03503>
03504> | CALIB STANDHYD | Area (ha)= 19.90
03505> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03506>
03507> IMPERVIOUS PERVIOUS (i)
03508> Surface Area (ha)= 14.13 5.77
03509> Dep. Storage (mm)= 1.57 4.67
03510> Average Slope (%)= .60 1.50

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03511> Length (m) = 580.00 100.00
03512> Mannings n = .030 .250
03513>
03514> Max. eff. Inten. (mm/hr) = 153.66 117.89
03515> over (min) = 12.50 25.00
03516> Storage Coeff. (min) = 11.89 (ii) 24.37 (ii)
03517> Unit Hyd. Tpeak (min) = 12.50 25.00
03518> Unit Hyd. peak (cms) = .09 .05
03519>
03520> PEAK FLOW (cms) = 2.77 1.13 3.419 (iii)
03521> TIME TO PEAK (hrs) = 1.13 1.38 1.167
03522> RUNOFF VOLUME (mm) = 70.09 45.94 58.015
03523> TOTAL RAINFALL (mm) = 71.66 71.66 71.665
03524> RUNOFF COEFFICIENT = .98 .64 .810
03525>
03526> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
03527> CN* = 81.0 Ia = Dep. Storage (Above)
03528> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03529> THAN THE STORAGE COEFFICIENT.
03530> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03531>
03532> -----
03533> 001:0015 -----
03534>
03535> | ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03536> | (ha) (cms) (hrs) (mm) (cms)
03537> | ID1 10:POND 8.56 .283 1.86 67.65 .000
03538> | +ID2 01:HIP01 19.90 3.419 1.17 58.02 .000
03539> |-----|
03540> | SUM 02:HIP02 28.46 3.554 1.17 60.91 .000
03541>
03542> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03543>
03544> -----
03545> 001:0016 -----
03546> *
03547> * SUB-AREA No.2
03548>
03549> | CALIB STANDHYD | Area (ha) = 17.00
03550> | 03:HIP03 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00
03551>
03552> IMPERVIOUS PERVIOUS (i)
03553> Surface Area (ha) = 12.07 4.93
03554> Dep. Storage (mm) = 1.57 4.67
03555> Average Slope (%) = .65 1.50
03556> Length (m) = 450.00 100.00
03557> Mannings n = .030 .250
03558>
03559> Max. eff. Inten. (mm/hr) = 178.56 126.60
03560> over (min) = 10.00 22.50
03561> Storage Coeff. (min) = 9.39 (ii) 21.52 (ii)
03562> Unit Hyd. Tpeak (min) = 10.00 22.50
03563> Unit Hyd. peak (cms) = .12 .05
03564>
03565> PEAK FLOW (cms) = 2.68 1.05 3.203 (iii)
03566> TIME TO PEAK (hrs) = 1.08 1.33 1.125
03567> RUNOFF VOLUME (mm) = 70.09 45.94 58.015
03568> TOTAL RAINFALL (mm) = 71.66 71.66 71.665
03569> RUNOFF COEFFICIENT = .98 .64 .810
03570>
03571> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
03572> CN* = 81.0 Ia = Dep. Storage (Above)
03573> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03574> THAN THE STORAGE COEFFICIENT.
03575> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03576>
03577> -----
03578> 001:0017 -----
03579> *
03580> * SUB-AREA No.3
03581>
03582> | CALIB STANDHYD | Area (ha) = 15.60
03583> | 04:HIP04 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00
03584>
03585> IMPERVIOUS PERVIOUS (i)
03586> Surface Area (ha) = 11.08 4.52
03587> Dep. Storage (mm) = 1.57 4.67
03588> Average Slope (%) = .50 1.50
03589> Length (m) = 600.00 100.00
03590> Mannings n = .030 .250
03591>
03592> Max. eff. Inten. (mm/hr) = 153.66 117.89
03593> over (min) = 12.50 25.00
03594> Storage Coeff. (min) = 12.82 (ii) 25.30 (ii)
03595> Unit Hyd. Tpeak (min) = 12.50 25.00
03596> Unit Hyd. peak (cms) = .09 .04
03597>
03598> PEAK FLOW (cms) = 2.10 .87 2.612 (iii)
03599> TIME TO PEAK (hrs) = 1.13 1.38 1.167
03600> RUNOFF VOLUME (mm) = 70.09 45.94 58.015
03601> TOTAL RAINFALL (mm) = 71.66 71.66 71.665
03602> RUNOFF COEFFICIENT = .98 .64 .810
03603>
03604> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
03605> CN* = 81.0 Ia = Dep. Storage (Above)
03606> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03607> THAN THE STORAGE COEFFICIENT.
03608> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03609>
03610> -----
03611> 001:0018 -----
03612>
03613> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03614> | (ha) (cms) (hrs) (mm) (cms)
03615> | ID1 03:HIP03 17.00 3.203 1.13 58.02 .000
03616> | +ID2 04:HIP04 15.60 2.612 1.17 58.02 .000
03617> |-----|
03618> | SUM 05:HIP05 32.60 5.767 1.13 58.02 .000
03619>
03620> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03621>
03622> -----
03623> 001:0019 -----
03624>
03625> | ADD HYD (HIP06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03626> | (ha) (cms) (hrs) (mm) (cms)
03627> | ID1 05:HIP05 32.60 5.767 1.13 58.02 .000
03628> | +ID2 02:HIP02 28.46 3.554 1.17 60.91 .000
03629> |-----|
03630> | SUM 06:HIP06 61.06 9.239 1.13 59.36 .000
03631>
03632> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03633>
03634> -----
03635> 001:0020 -----
03636> *
03637> * SUB-AREA No.4
03638>
03639> | CALIB STANDHYD | Area (ha) = 12.20
03640> | 07:HIP07 DT= 2.50 | Total Imp(%) = 71.00 Dir. Conn.(%) = 50.00
03641>
03642> IMPERVIOUS PERVIOUS (i)
03643> Surface Area (ha) = 8.66 3.54
03644> Dep. Storage (mm) = 1.57 4.67
03645> Average Slope (%) = .70 1.50

```

```

03646> Length (m) = 210.00 100.00
03647> Mannings n = .030 .250
03648>
03649> Max. eff. Inten. (mm/hr) = 178.56 146.17
03650> over (min) = 5.00 17.50
03651> Storage Coeff. (min) = 5.81 (ii) 17.27 (ii)
03652> Unit Hyd. Tpeak (min) = 5.00 17.50
03653> Unit Hyd. peak (cms) = .20 .07
03654>
03655> PEAK FLOW (cms) = 2.46 .87 2.793 (iii)
03656> TIME TO PEAK (hrs) = 1.00 1.25 1.042
03657> RUNOFF VOLUME (mm) = 70.09 45.94 58.015
03658> TOTAL RAINFALL (mm) = 71.66 71.66 71.665
03659> RUNOFF COEFFICIENT = .98 .64 .810
03660>
03661> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
03662> CN* = 81.0 Ia = Dep. Storage (Above)
03663> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03664> THAN THE STORAGE COEFFICIENT.
03665> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03666>
03667> -----
03668> 001:0021 -----
03669> *
03670> * SUB-AREA No.5
03671>
03672> | DESIGN NASHYD | Area (ha) = 4.00 Curve Number (CN)=85.00
03673> | 08:Pond-B DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
03674> | U.H. Tp(hrs) = .170
03675>
03676> Unit Hyd Qpeak (cms) = .899
03677>
03678> PEAK FLOW (cms) = .649 (i)
03679> TIME TO PEAK (hrs) = 1.125
03680> RUNOFF VOLUME (mm) = 40.139
03681> TOTAL RAINFALL (mm) = 71.665
03682> RUNOFF COEFFICIENT = .560
03683>
03684> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03685>
03686> -----
03687> 001:0022 -----
03688>
03689> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03690> | (ha) (cms) (hrs) (mm) (cms)
03691> | ID1 06:HIP06 61.06 9.239 1.13 59.36 .000
03692> | +ID2 07:HIP07 12.20 2.793 1.04 58.02 .000
03693> | +ID3 08:Pond-B 4.00 .649 1.13 40.14 .000
03694> |-----|
03695> | SUM 09:HIP08 77.26 12.109 1.13 58.16 .000
03696>
03697> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03698>
03699> -----
03700> 001:0023 -----
03701> *
03702> * SUB-AREA No. 6
03703>
03704> | DESIGN NASHYD | Area (ha) = 2.70 Curve Number (CN)=76.00
03705> | 01:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
03706> | U.H. Tp(hrs) = .800
03707>
03708> Unit Hyd Qpeak (cms) = .129
03709>
03710> PEAK FLOW (cms) = .114 (i)
03711> TIME TO PEAK (hrs) = 1.958
03712> RUNOFF VOLUME (mm) = 30.490
03713> TOTAL RAINFALL (mm) = 71.665
03714> RUNOFF COEFFICIENT = .425
03715>
03716> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03717>
03718> -----
03719> 001:0024 -----
03720>
03721> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03722> | (ha) (cms) (hrs) (mm) (cms)
03723> | ID1 09:HIP08 77.26 12.109 1.13 58.16 .000
03724> | +ID2 01:A3 2.70 .114 1.96 30.49 .000
03725> |-----|
03726> | SUM 02:Ultima 79.96 12.132 1.13 57.22 .000
03727>
03728> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03729>
03730> -----
03731> 001:0025 -----
03732> FINISH
03733>
03734> *****
03735> WARNINGS / ERRORS / NOTES
03736>
03737> Simulation ended on 2009-02-09 at 14:59:34
03738>
03739>
03740>
03741>
03742>

```

## **APPENDIX 'F'**

### **STAGE-STORAGE-DISCHARGE TABLE**



## Hawthorne Industrial Park Configuration of Storage Facility

	RESTRICTOR FLOW (L/S)	RESTRICTOR FLOW (L/S)	WEIR FLOW (L/S)	TOTAL OUTFLOW (L/S)	Storage Cell Configuration		
					SWMHYMO DATA		
					AREA m <sup>2</sup>	VOLUME m <sup>3</sup>	VOLUME ha-m
Invert Elevation (m):	82.90	84.80	86.15				
Dia. or Width (mm):	150	600	6000				
# of restrictors/weirs:	1	2	1				
Discharge Coeff. (C <sub>d</sub> ):	0.61	0.61	1.87				
ELEV. (m)	DISCH. (L/S)	DISCH. (L/S)	DISCH. (L/S)				
82.900	0.0	0.0	0.0	0	0	0	0.0000
84.000	48.3	0.0	0.0	48	3093	574	0.0574
84.250	53.9	0.0	0.0	54	11192	2434	0.2434
84.500	59.0	0.0	0.0	59	16913	5834	0.5834
84.650	61.8	0.0	0.0	62	17299	8400	0.8400
84.800	64.5	0.0	0.0	64	17684	11024	1.1024
84.950	67.1	80.0	0.0	147	18070	13705	1.3705
85.100	69.6	210.0	0.0	280	18456	16444	1.6444
85.250	72.0	400.0	0.0	472	18842	19242	1.9242
85.400	74.3	650.0	0.0	724	19227	22097	2.2097
85.550	76.6	860.0	0.0	937	19613	25010	2.5010
85.700	78.8	1183.3	0.0	1262	19999	27981	2.7981
85.850	80.9	1323.0	0.0	1404	20384	31009	3.1009
86.000	83.0	1449.3	0.0	1532	20770	34096	3.4096
86.150	85.1	1565.4	0.0	1650	21156	37240	3.7240
86.300	87.1	1673.5	648.6	2409	21541	40442	4.0442
86.450	89.0	1775.0	1825.2	3689	21927	43702	4.3702

Note: Restrictor flows estimated by MTO Design Chart 2.32: Inlet Control for elevations  $\leq 85.55$  for double 600 mm culverts.

## **A P P E N D I X 'G'**

### **SWMHYMO INPUT AND OUTPUT FILES (Post-Development Controlled Phase 1 Conditions)**

```

00001> 2 Metric units
00002> *****
00003> * Project Name : Hawthorne Industrial Park Project Number: [20983] *
00004> * Date : January, 2005 *
00005> * Revisd : N/A *
00006> * Developed by : Mark Buchanan, E.I.T. *
00007> * Reviewed by : Guy Forget, P.Eng. *
00008> * Company : J.L. Richards & Associates Limited *
00009> * License # : 4418403 *
00010> *****
00011> *
00012> *
00013> *****
00014> * FILENAME: V:\20983.DU\ENG\SWHYMO\20983PST.DAT *
00015> * FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00016> * OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00017> *****
00018> *
00019> *****
00020> * SWHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE *
00021> * PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00022> *****
00023> *
00024> *****
00025> * HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00026> * FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00027> *****
00028> *
00029> *****
00030> * POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00031> *****
00032> *
00033> *****
00034> * CALCULATION OF 4 HR 25 MM STORM EVENT *
00035> *****
00036> *
00037> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00038> * [ ] <-storm filename, one per line for NSTORM time
00039> READ STORM STORM_FILENAME=[4HR25-15.STM]
00040> *
00041> DEFAULT VALUES ICASEDef=[1], read and print values
00042> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00043> *
00044> *****
00045> *
00046> * ORGAWORLD FILE *
00047> *****
00048> *
00049> * SUB-AREA No.1 *
00050> *
00051> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00052> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00053> SCS curve number CN=[81],
00054> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00055> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
00056> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00057> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00058> RAINFALL=[ , , , ] (mm/hr), END=-1
00059> *
00060> *
00061> * SUB-AREA No.2 *
00062> *
00063> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00064> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00065> SCS curve number CN=[81],
00066> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00067> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00068> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00069> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00070> RAINFALL=[ , , , ] (mm/hr), END=-1
00071> *
00072> *
00073> * SUB-AREA No.3 *
00074> *
00075> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[ 1.4 ] (ha),
00076> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00077> SCS curve number CN=[81],
00078> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00079> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00080> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00081> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
00082> RAINFALL=[ , , , ] (mm/hr), END=-1
00083> *
00084> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00085> *
00086> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00087> *
00088> *
00089> * SUB-AREA No.4 *
00090> *
00091> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00092> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00093> SCS curve number CN=[81],
00094> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00095> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00096> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00097> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00098> RAINFALL=[ , , , ] (mm/hr), END=-1
00099> *
00100> *
00101> * SUB-AREA No.5 *
00102> *
00103> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00104> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00105> SCS curve number CN=[81],
00106> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00107> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (mi)
00108> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00109> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00110> RAINFALL=[ , , , ] (mm/hr), END=-1
00111> *
00112> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00113> *
00114> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00115> *
00116> *
00117> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00118> RDT=[1.0] (min),
00119> TABLE of ( OUTFLOW-STORAGE ) values
00120> (cms) (ha-m)
00121> [ 0.000, 0.0000 ]
00122> [ 0.008, 0.0656 ]
00123> [ 0.017, 0.1311 ]
00124> [ 0.053, 0.2831 ]
00125> [ 0.233, 0.3971 ]
00126> [ 0.337, 0.4731 ]
00127> [ 0.465, 0.5491 ]
00128> [ 0.531, 0.5871 ]
00129> [ 0.593, 0.6251 ]
00130> [ 0.654, 0.6631 ]
00131> [ 0.797, 0.7391 ]
00132> [ 0.950, 0.8274 ]
00133> [ 1.304, 0.9157 ]
00134> [ 1.880, 1.0040 ]
00135> [ 2.577, 1.0923 ]
00136> [ -1, -1 ] (max twenty pts)
00137> *****
00138> *
00139> * Remaining Hawthorne Industrial Park *
00140> *****
00141> *
00142> * SUB-AREA No.1 *
00143> *
00144> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00145> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00146> SCS curve number CN=[81],
00147> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00148> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00149> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00150> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00151> RAINFALL=[ , , , ] (mm/hr), END=-1
00152> *
00153> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00154> *
00155> *
00156> * SUB-AREA No.2 *
00157> *
00158> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00159> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00160> SCS curve number CN=[81],
00161> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00162> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00163> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00164> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00165> RAINFALL=[ , , , ] (mm/hr), END=-1
00166> *
00167> *
00168> * SUB-AREA No.3 *
00169> *
00170> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00171> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00172> SCS curve number CN=[81],
00173> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00174> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00175> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00176> LGI=[500] (m), MNI=[0.03], SCI=[0.0] (min)
00177> RAINFALL=[ , , , ] (mm/hr), END=-1
00178> *
00179> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00180> *
00181> *
00182> * SUB-AREA No.4 *
00183> *
00184> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00185> DWF=[0] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00186> RAINFALL=[ , , , ] (mm/hr), END=-1
00187> *
00188> *
00189> *
00190> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00191> *
00192> *
00193> ROUTE RESERVOIR IDout=[ 8 ], NHYD=["HIP-POND"], IDin=[ 7 ],
00194> RDT=[1.0] (min),
00195> TABLE of ( OUTFLOW-STORAGE ) values
00196> (cms) (ha-m)
00197> [ 0.0, 0.0 ]
00198> [ 0.048, 0.0574 ]
00199> [ 0.054, 0.2434 ]
00200> [ 0.059, 0.5834 ]
00201> [ 0.062, 0.8400 ]
00202> [ 0.064, 1.1024 ]
00203> [ 0.147, 1.3705 ]
00204> [ 0.280, 1.6444 ]
00205> [ 0.472, 1.9242 ]
00206> [ 0.724, 2.2097 ]
00207> [ 0.937, 2.5010 ]
00208> [ 1.262, 2.7981 ]
00209> [ 1.404, 3.1009 ]
00210> [ 1.532, 3.4096 ]
00211> [ 1.650, 3.7240 ]
00212> [ 2.409, 4.0442 ]
00213> [ 3.689, 4.3702 ]
00214> [ -1, -1 ] (max twenty pts)
00215> *
00216> *
00217> *
00218> *
00219> * SUB-AREA No. 5 *
00220> *
00221> DESIGN NASHYD ID = [9], NHYD=["A2"], DT=[2.5] min, AREA=[6.8] (ha),
00222> DWF=[0] (cms), CN/C=[76], TP=[0.37] hrs,
00223> RAINFALL=[ , , , ] (mm/hr), END=-1
00224> *
00225> *
00226> * SUB-AREA No. 6 *
00227> *
00228> DESIGN NASHYD ID = [10], NHYD=["A3"], DT=[2.5] min, AREA=[5.3] (ha),
00229> DWF=[0] (cms), CN/C=[76], TP=[0.804] hrs,
00230> RAINFALL=[ , , , ] (mm/hr), END=-1
00231> *
00232> ADD HYD IDsum=[1], NHYD=["Interim"], IDs to add=[8+9+10]
00233> *
00234> *
00235> *****
00236> *
00237> * CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00238> *****
00239> *
00240> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00241> * [ ] <-storm filename, one per line for NSTORM time
00242> *
00243> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSST=[10.0] (min)
00244> ICASEcs=[1],
00245> A=[732.951], B=[6.199], and C=[0.810],
00246> *
00247> DEFAULT VALUES ICASEDef=[1], read and print values
00248> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00249> *
00250> *
00251> *
00252> * ORGAWORLD FILE *
00253> *****
00254> *
00255> * SUB-AREA No.1 *
00256> *
00257> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00258> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00259> SCS curve number CN=[81],
00260> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00261> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
00262> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00263> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00264> RAINFALL=[ , , , ] (mm/hr), END=-1
00265> *
00266> *
00267> * SUB-AREA No.2 *
00268> *
00269> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00270> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],

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00271> SCS curve number CN=[81],
00272> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00273> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00274> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
00275> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00276> RAINFALL=[ , , , ] (mm/hr), END=-1
00277> *%-----
00278> *
00279> * SUB-AREA No.3
00280>
00281> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00282> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00283> SCS curve number CN=[81],
00284> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00285> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00286> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
00287> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
00288> RAINFALL=[ , , , ] (mm/hr), END=-1
00289> *%-----
00290> * ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00291> *%-----
00292> * ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00293> *%-----
00294> *
00295> * SUB-AREA No.4
00296>
00297> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00298> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00299> SCS curve number CN=[81],
00300> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[0.7] (%),
00301> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min),
00302> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
00303> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00304> RAINFALL=[ , , , ] (mm/hr), END=-1
00305> *%-----
00306> *
00307> * SUB-AREA No.5
00308>
00309> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00310> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00311> SCS curve number CN=[81],
00312> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00313> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min),
00314> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00315> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00316> RAINFALL=[ , , , ] (mm/hr), END=-1
00317> *%-----
00318> * ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00319> *%-----
00320> * ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00321> *%-----
00322> *
00323> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00324> RDT=[1.0] (min),
00325> TABLE of ( OUTFLOW-STORAGE ) values
00326> (cms) - (ha-m)
00327> [ 0.000, 0.0000 ]
00328> [ 0.008, 0.0656 ]
00329> [ 0.017, 0.1311 ]
00330> [ 0.093, 0.2831 ]
00331> [ 0.233, 0.3971 ]
00332> [ 0.337, 0.4731 ]
00333> [ 0.465, 0.5491 ]
00334> [ 0.531, 0.5871 ]
00335> [ 0.593, 0.6251 ]
00336> [ 0.654, 0.6631 ]
00337> [ 0.797, 0.7391 ]
00338> [ 0.950, 0.8274 ]
00339> [ 1.304, 0.9157 ]
00340> [ 1.880, 1.0040 ]
00341> [ 2.577, 1.0923 ]
00342> [ -1, -1 ] (max twenty pts)
00343> *%-----
00344> *
00345> * Remaining Hawthorne Industrial Park *
00346> *%-----
00347> *
00348> * SUB-AREA No.1
00349>
00350> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00351> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00352> SCS curve number CN=[81],
00353> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00354> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min),
00355> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00356> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min),
00357> RAINFALL=[ , , , ] (mm/hr), END=-1
00358> *%-----
00359> * ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00360> *%-----
00361> *
00362> * SUB-AREA No.2
00363>
00364> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00365> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00366> SCS curve number CN=[81],
00367> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00368> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min),
00369> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
00370> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min),
00371> RAINFALL=[ , , , ] (mm/hr), END=-1
00372> *%-----
00373> *
00374> * SUB-AREA No.3
00375>
00376> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00377> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00378> SCS curve number CN=[81],
00379> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00380> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min),
00381> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
00382> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min),
00383> RAINFALL=[ , , , ] (mm/hr), END=-1
00384> *%-----
00385> * ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00386> *%-----
00387> *
00388> * SUB-AREA No.4
00389>
00390> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00391> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00392> RAINFALL=[ , , , ] (mm/hr), END=-1
00393> *%-----
00394> *
00395>
00396> * ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00397> *%-----
00398>
00399> ROUTE RESERVOIR IDout=[ 8 ], NHYD=["HIP-POND"], IDin=[ 7 ],
00400> RDT=[1.0] (min),
00401> TABLE of ( OUTFLOW-STORAGE ) values
00402> (cms) - (ha-m)
00403> [ 0.0, 0.0 ]
00404> [ 0.048, 0.0574 ]
00405> [ 0.054, 0.2434 ]
00406> [ 0.059, 0.5834 ]
00407> [ 0.062, 0.8400 ]
00408> [ 0.064, 1.1024 ]
00409> [ 0.147, 1.3705 ]
00410> [ 0.280, 1.6444 ]
00411> [ 0.472, 1.9242 ]
00412> [ 0.724, 2.2097 ]
00413> [ 0.937, 2.5010 ]
00414> [ 1.262, 2.7981 ]
00415> [ 1.404, 3.1009 ]
00416> [ 1.532, 3.4096 ]
00417> [ 1.650, 3.7240 ]
00418> [ 2.409, 4.0442 ]
00419> [ 3.689, 4.3702 ]
00420> [ -1, -1 ] (max twenty pts)
00421> *%-----
00422> *
00423> *
00424> * SUB-AREA No. 5
00425> *
00426> DESIGN NASHYD ID = [9], NHYD=["A2"], DT=[2.5] min, AREA=[6.8] (ha),
00427> DWF=[0] (cms), CN=[76], TP=[0.37] hrs,
00428> RAINFALL=[ , , , ] (mm/hr), END=-1
00429> *%-----
00430> *
00431> * SUB-AREA No. 6
00432> *
00433> DESIGN NASHYD ID = [10], NHYD=["A3"], DT=[2.5] min, AREA=[5.3] (ha),
00434> DWF=[0] (cms), CN=[76], TP=[0.804] hrs,
00435> RAINFALL=[ , , , ] (mm/hr), END=-1
00436> *%-----
00437> * ADD HYD IDsum=[1], NHYD=["Interim"], IDs to add=[8+9+10]
00438> *%-----
00439> *
00440>
00441> *****
00442> * CALCULATION OF 3HR - 1:5 YEAR STORM EVENT *
00443> *****
00444>
00445> * START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00446> [ ] <- storm filename, one per line for LOSS time
00447> *%-----
00448> * CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00449> ICASEcs=[1],
00450> R=[998.071], B=[6.053], and C=[0.814],
00451> *%-----
00452> * DEFAULT VALUES ICASDef=[1], read and print values
00453> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00454> *%-----
00455> *
00456> *****
00457> * ORGAWORLD FILE *
00458> *****
00459>
00460> * SUB-AREA No.1
00461>
00462> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00463> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00464> SCS curve number CN=[81],
00465> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00466> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (min),
00467> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
00468> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00469> RAINFALL=[ , , , ] (mm/hr), END=-1
00470> *%-----
00471> *
00472> * SUB-AREA No.2
00473>
00474> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00475> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00476> SCS curve number CN=[81],
00477> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00478> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00479> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
00480> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00481> RAINFALL=[ , , , ] (mm/hr), END=-1
00482> *%-----
00483> *
00484> * SUB-AREA No.3
00485>
00486> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00487> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00488> SCS curve number CN=[81],
00489> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00490> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00491> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
00492> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
00493> RAINFALL=[ , , , ] (mm/hr), END=-1
00494> *%-----
00495> * ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00496> *%-----
00497> * ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00498> *%-----
00499> *
00500> * SUB-AREA No.4
00501>
00502> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00503> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00504> SCS curve number CN=[81],
00505> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[0.7] (%),
00506> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min),
00507> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
00508> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00509> RAINFALL=[ , , , ] (mm/hr), END=-1
00510> *%-----
00511> *
00512> * SUB-AREA No.5
00513>
00514> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00515> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00516> SCS curve number CN=[81],
00517> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00518> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min),
00519> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00520> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00521> RAINFALL=[ , , , ] (mm/hr), END=-1
00522> *%-----
00523> * ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00524> *%-----
00525> * ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00526> *%-----
00527> *
00528> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00529> RDT=[1.0] (min),
00530> TABLE of ( OUTFLOW-STORAGE ) values
00531> (cms) - (ha-m)
00532> [ 0.000, 0.0000 ]
00533> [ 0.008, 0.0656 ]
00534> [ 0.017, 0.1311 ]
00535> [ 0.093, 0.2831 ]
00536> [ 0.233, 0.3971 ]
00537> [ 0.337, 0.4731 ]
00538> [ 0.465, 0.5491 ]
00539> [ 0.531, 0.5871 ]
00540> [ 0.593, 0.6251 ]

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00541> [ 0.654, 0.6631]
00542> [ 0.797, 0.7391]
00543> [ 0.950, 0.8274]
00544> [ 1.304, 0.8157]
00545> [ 1.880, 1.0040]
00546> [ 2.577, 1.0923]
00547> [ -1, -1 ] (max twenty pts)
00548>
00549> *****
00550> * Remaining Hawthorne Industrial Park *
00551> *****
00552> *
00553> * SUB-AREA No.1
00554>
00555> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00556> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00557> SCS curve number CN=[81],
00558> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00559> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00560> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00561> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00562> RAINFALL=[ , , , ] (mm/hr), END=-1
00563>
00564> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00565> *
00566> *
00567> * SUB-AREA No.2
00568>
00569> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00570> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00571> SCS curve number CN=[81],
00572> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00573> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00574> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00575> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00576> RAINFALL=[ , , , ] (mm/hr), END=-1
00577> *
00578> *
00579> * SUB-AREA No.3
00580>
00581> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00582> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00583> SCS curve number CN=[81],
00584> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00585> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00586> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00587> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
00588> RAINFALL=[ , , , ] (mm/hr), END=-1
00589> *
00590> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00591> *
00592> *
00593> * SUB-AREA No.4
00594>
00595> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00596> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00597> RAINFALL=[ , , , ] (mm/hr), END=-1
00598> *
00599> *
00600>
00601> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00602> *
00603>
00604> ROUTE RESERVOIR IDout=[ 8 ], NHYD=["HIP-POND"], IDin=[ 7 ],
00605> RDT=[1.0] (min),
00606> TABLE of ( OUTFLOW-STORAGE ) values
00607> (cms) - (ha-m)
00608> [ 0.0, 0.0 ]
00609> [ 0.048, 0.0574 ]
00610> [ 0.054, 0.2434 ]
00611> [ 0.059, 0.5834 ]
00612> [ 0.062, 0.8400 ]
00613> [ 0.064, 1.1024 ]
00614> [ 0.147, 1.3705 ]
00615> [ 0.280, 1.6444 ]
00616> [ 0.472, 1.9242 ]
00617> [ 0.724, 2.2097 ]
00618> [ 0.937, 2.5010 ]
00619> [ 1.262, 2.7981 ]
00620> [ 1.404, 3.1009 ]
00621> [ 1.532, 3.4096 ]
00622> [ 1.650, 3.7240 ]
00623> [ 2.409, 4.0442 ]
00624> [ 3.689, 4.3702 ]
00625> [ -1, -1 ] (max twenty pts)
00626> *
00627> *
00628> *
00629> * SUB-AREA No. 5
00630>
00631> DESIGN NASHYD ID = [9], NHYD=["A2"], DT=[2.5] min, AREA=[6.8] (ha),
00632> DWF=[0] (cms), CN/C=[76], TP=[0.37] hrs,
00633> RAINFALL=[ , , , ] (mm/hr), END=-1
00634> *
00635> *
00636> * SUB-AREA No. 6
00637>
00638> DESIGN NASHYD ID = [10], NHYD=["A3"], DT=[2.5] min, AREA=[5.3] (ha),
00639> DWF=[0] (cms), CN/C=[76], TP=[0.804] hrs,
00640> RAINFALL=[ , , , ] (mm/hr), END=-1
00641> *
00642> ADD HYD IDsum=[1], NHYD=["Interim"], IDs to add=[8+9+10]
00643> *
00644>
00645> *****
00646> * CALCULATION OF 3HR - 1:10 YEAR STORM EVENT *
00647> *****
00648>
00649> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUM=[0]
00650> [ ] <- storm filename, one per line for NSTORM time
00651> *
00652> CHICAGO STORM UNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00653> ICASRcs=[1],
00654> A=[1174.184], B=[0.014], and C=[0.816],
00655> *
00656> DEFAULT VALUES ICASDef=[1], read and print values
00657> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYM\ORGA.VAL]
00658> *
00659> *
00660> *****
00661> * ORGAWORLD FILE *
00662> *****
00663> *
00664> * SUB-AREA No.1
00665>
00666> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00667> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00668> SCS curve number CN=[81],
00669> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00670> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (m)
00671> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00672> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min)
00673> RAINFALL=[ , , , ] (mm/hr), END=-1
00674> *
00675> *
00676> * SUB-AREA No.2
00677>
00678> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00679> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00680> SCS curve number CN=[81],
00681> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00682> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00683> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00684> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0] (min)
00685> RAINFALL=[ , , , ] (mm/hr), END=-1
00686> *
00687> *
00688> * SUB-AREA No.3
00689>
00690> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00691> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00692> SCS curve number CN=[81],
00693> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00694> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00695> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00696> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0] (min)
00697> RAINFALL=[ , , , ] (mm/hr), END=-1
00698> *
00699> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00700> *
00701> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00702> *
00703> *
00704> * SUB-AREA No.4
00705>
00706> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00707> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00708> SCS curve number CN=[81],
00709> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00710> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00711> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00712> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min)
00713> RAINFALL=[ , , , ] (mm/hr), END=-1
00714> *
00715> *
00716> * SUB-AREA No.5
00717>
00718> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00719> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00720> SCS curve number CN=[81],
00721> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00722> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
00723> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00724> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min)
00725> RAINFALL=[ , , , ] (mm/hr), END=-1
00726> *
00727> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00728> *
00729> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00730> *
00731>
00732> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00733> RDT=[1.0] (min),
00734> TABLE of ( OUTFLOW-STORAGE ) values
00735> (cms) - (ha-m)
00736> [ 0.000, 0.0000 ]
00737> [ 0.008, 0.0656 ]
00738> [ 0.017, 0.1311 ]
00739> [ 0.093, 0.2831 ]
00740> [ 0.233, 0.3971 ]
00741> [ 0.337, 0.4731 ]
00742> [ 0.465, 0.5491 ]
00743> [ 0.531, 0.5871 ]
00744> [ 0.593, 0.6251 ]
00745> [ 0.654, 0.6631 ]
00746> [ 0.797, 0.7391 ]
00747> [ 0.950, 0.8274 ]
00748> [ 1.304, 0.8157 ]
00749> [ 1.880, 1.0040 ]
00750> [ 2.577, 1.0923 ]
00751> [ -1, -1 ] (max twenty pts)
00752> *
00753> *****
00754> * Remaining Hawthorne Industrial Park *
00755> *****
00756> *
00757> * SUB-AREA No.1
00758>
00759> CALIB STANDHYD ID=[ 2 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00760> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00761> SCS curve number CN=[81],
00762> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00763> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00764> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00765> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00766> RAINFALL=[ , , , ] (mm/hr), END=-1
00767> *
00768> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00769> *
00770> *
00771> * SUB-AREA No.2
00772>
00773> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00774> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00775> SCS curve number CN=[81],
00776> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00777> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00778> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00779> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00780> RAINFALL=[ , , , ] (mm/hr), END=-1
00781> *
00782> *
00783> * SUB-AREA No.3
00784>
00785> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00786> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00787> SCS curve number CN=[81],
00788> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00789> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00790> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00791> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
00792> RAINFALL=[ , , , ] (mm/hr), END=-1
00793> *
00794> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00795> *
00796> *
00797> * SUB-AREA No.4
00798>
00799> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00800> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00801> RAINFALL=[ , , , ] (mm/hr), END=-1
00802> *
00803> *
00804>
00805> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
00806> *
00807> *
00808> ROUTE RESERVOIR IDout=[ 8 ], NHYD=["HIP-POND"], IDin=[ 7 ],
00809> RDT=[1.0] (min),
00810> TABLE of ( OUTFLOW-STORAGE ) values

```

```

00811> (cms) - (ha-m)
00812> [ 0.0 , 0.0 ]
00813> [ 0.048 , 0.0574 ]
00814> [ 0.054 , 0.2434 ]
00815> [ 0.059 , 0.5834 ]
00816> [ 0.062 , 0.8400 ]
00817> [ 0.064 , 1.1024 ]
00818> [ 0.147 , 1.3705 ]
00819> [ 0.280 , 1.6444 ]
00820> [ 0.472 , 1.9242 ]
00821> [ 0.724 , 2.2097 ]
00822> [ 0.937 , 2.5010 ]
00823> [ 1.262 , 2.7981 ]
00824> [ 1.404 , 3.1009 ]
00825> [ 1.532 , 3.4096 ]
00826> [ 1.650 , 3.7240 ]
00827> [ 2.409 , 4.0442 ]
00828> [ 3.689 , 4.3702 ]
00829> [ -1 , -1 ] (max twenty pts)
00830>
00831> *%-----|-----|
00832> *
00833> *SUB-AREA No. 5
00834> *
00835> DESIGN NASHYD ID = [9], NHYD=["A2"], DT=[2.5]min, AREA=[6.8] (ha),
00836> DWF=[0] (cms), CNC=[76], TP=[0.37]hrs,
00837> RAINFALL=[ , , , ] (mm/hr), END=-1
00838> *%-----|-----|
00839> *
00840> *SUB-AREA No. 6
00841> *
00842> DESIGN NASHYD ID = [10], NHYD=["A3"], DT=[2.5]min, AREA=[5.3] (ha),
00843> DWF=[0] (cms), CNC=[76], TP=[0.804]hrs,
00844> RAINFALL=[ , , , ] (mm/hr), END=-1
00845> *%-----|-----|
00846> ADD HYD IDsum=[1], NHYD=["Interim"], IDs to add=[8+9+10]
00847> *%-----|-----|
00848>
00849> *****
00850> * CALCULATION OF 3HR - 1.25 YEAR STORM EVENT *
00851> *****
00852>
00853> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00854> *%-----|-----|
00855> * [ ] <- storm filename, one per line for NSTORM time
00856> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00857> ICASES=[1],
00858> A=[1402.884], B=[6.018], and C=[0.819],
00859> *%-----|-----|
00860> DEFAULT VALUES ICASEDef=[1], read and print values
00861> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00862> *%-----|-----|
00863>
00864> *****
00865> * ORGAWORLD FILE *
00866> *****
00867> *
00868> *SUB-AREA No.1
00869>
00870> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00871> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00872> SCS curve number CN=[81],
00873> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00874> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
00875> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00876> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00877> RAINFALL=[ , , , ] (mm/hr), END=-1
00878> *%-----|-----|
00879>
00880> *SUB-AREA No.2
00881>
00882> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00883> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00884> SCS curve number CN=[81],
00885> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00886> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00887> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00888> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00889> RAINFALL=[ , , , ] (mm/hr), END=-1
00890> *%-----|-----|
00891>
00892> *SUB-AREA No.3
00893>
00894> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00895> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00896> SCS curve number CN=[81],
00897> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00898> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00899> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00900> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
00901> RAINFALL=[ , , , ] (mm/hr), END=-1
00902> *%-----|-----|
00903> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00904> *%-----|-----|
00905> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00906> *%-----|-----|
00907>
00908> *SUB-AREA No.4
00909>
00910> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00911> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00912> SCS curve number CN=[81],
00913> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00914> LGP=[4] (m), MNP=[0.25], SCP=[0.0] (min)
00915> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00916> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (0.0)
00917> RAINFALL=[ , , , ] (mm/hr), END=-1
00918> *%-----|-----|
00919>
00920> *SUB-AREA No.5
00921>
00922> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00923> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00924> SCS curve number CN=[81],
00925> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00926> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (mi)
00927> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00928> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00929> RAINFALL=[ , , , ] (mm/hr), END=-1
00930> *%-----|-----|
00931> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00932> *%-----|-----|
00933> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00934> *%-----|-----|
00935>
00936> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00937> RDT=[1.0] (min),
00938> TABLE OF ( OUTFLOW-STORAGE ) values
00939> (cms) - (ha-m)
00940> [ 0.000 , 0.0000 ]
00941> [ 0.008 , 0.0656 ]
00942> [ 0.017 , 0.1311 ]
00943> [ 0.093 , 0.2831 ]
00944> [ 0.233 , 0.3971 ]
00945> [ 0.337 , 0.4731 ]
00946> [ 0.465 , 0.5491 ]
00947> [ 0.531 , 0.5871 ]
00948> [ 0.593 , 0.6251 ]
00949> [ 0.654 , 0.6631 ]
00950> [ 0.797 , 0.7391 ]
00951> [ 0.950 , 0.8274 ]
00952> [ 1.304 , 0.9157 ]
00953> [ 1.880 , 1.0040 ]
00954> [ 2.577 , 1.0923 ]
00955> [ -1 , -1 ] (max twenty pts)
00956>
00957> *****
00958> * Remaining Hawthorne Industrial Park *
00959> *****
00960> *
00961> *SUB-AREA No.1
00962>
00963> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00964> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00965> SCS curve number CN=[81],
00966> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00967> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00968> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00969> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00970> RAINFALL=[ , , , ] (mm/hr), END=-1
00971> *%-----|-----|
00972> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00973> *%-----|-----|
00974>
00975> *SUB-AREA No.2
00976>
00977> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00978> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00979> SCS curve number CN=[81],
00980> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00981> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00982> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00983> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00984> RAINFALL=[ , , , ] (mm/hr), END=-1
00985> *%-----|-----|
00986>
00987> *SUB-AREA No.3
00988>
00989> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
00990> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00991> SCS curve number CN=[81],
00992> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00993> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00994> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00995> LGI=[500] (m), MNI=[0.03], SCI=[0.0] (min)
00996> RAINFALL=[ , , , ] (mm/hr), END=-1
00997> *%-----|-----|
00998> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00999> *%-----|-----|
01000>
01001> *SUB-AREA No.4
01002>
01003> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5]min, AREA=[4.0] (ha),
01004> DWF=[ 0 ] (cms), CNC/[ 85 ], TP=[0.17]hrs,
01005> RAINFALL=[ , , , ] (mm/hr), END=-1
01006> *%-----|-----|
01007>
01008> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
01009> *%-----|-----|
01010>
01011> ROUTE RESERVOIR IDout=[ 8 ], NHYD=["HIP-POND"], IDin=[ 7 ],
01012> RDT=[1.0] (min),
01013> TABLE OF ( OUTFLOW-STORAGE ) values
01014> (cms) - (ha-m)
01015> [ 0.0 , 0.0 ]
01016> [ 0.048 , 0.0574 ]
01017> [ 0.054 , 0.2434 ]
01018> [ 0.059 , 0.5834 ]
01019> [ 0.062 , 0.8400 ]
01020> [ 0.064 , 1.1024 ]
01021> [ 0.147 , 1.3705 ]
01022> [ 0.280 , 1.6444 ]
01023> [ 0.472 , 1.9242 ]
01024> [ 0.724 , 2.2097 ]
01025> [ 0.937 , 2.5010 ]
01026> [ 1.262 , 2.7981 ]
01027> [ 1.404 , 3.1009 ]
01028> [ 1.532 , 3.4096 ]
01029> [ 1.650 , 3.7240 ]
01030> [ 2.409 , 4.0442 ]
01031> [ 3.689 , 4.3702 ]
01032> [ -1 , -1 ] (max twenty pts)
01033>
01034>
01035> *SUB-AREA No. 5
01036> *
01037> DESIGN NASHYD ID = [9], NHYD=["A2"], DT=[2.5]min, AREA=[6.8] (ha),
01038> DWF=[0] (cms), CNC=[76], TP=[0.37]hrs,
01039> RAINFALL=[ , , , ] (mm/hr), END=-1
01040> *%-----|-----|
01041>
01042> *SUB-AREA No. 6
01043> *
01044> DESIGN NASHYD ID = [10], NHYD=["A3"], DT=[2.5]min, AREA=[5.3] (ha),
01045> DWF=[0] (cms), CNC=[76], TP=[0.804]hrs,
01046> RAINFALL=[ , , , ] (mm/hr), END=-1
01047> *%-----|-----|
01048>
01049> ADD HYD IDsum=[1], NHYD=["Interim"], IDs to add=[8+9+10]
01050> *%-----|-----|
01051>
01052> *****
01053> *****
01054> * CALCULATION OF 3HR - 1.50 YEAR STORM EVENT *
01055> *****
01056>
01057> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01058> *%-----|-----|
01059> * [ ] <- storm filename, one per line for NSTORM time
01060> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
01061> ICASES=[1],
01062> A=[1569.580], B=[6.014], and C=[0.820],
01063> *%-----|-----|
01064> DEFAULT VALUES ICASEDef=[1], read and print values
01065> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
01066> *%-----|-----|
01067>
01068> *****
01069> * ORGAWORLD FILE *
01070> *****
01071> *
01072> *SUB-AREA No.1
01073>
01074> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
01075> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
01076> SCS curve number CN=[81],
01077> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01078> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
01079> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
01080> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]

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01081> RAINFALL=[ , , , ](mm/hr) , END=-1
01082> *%-----
01083> * SUB-AREA No.2
01084> *
01085>
01086> CALIB STANDHYD ID=[ 2 ], NYHD=["020"], DT=[2.5](min), AREA=[1.54](ha),
01087> XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
01088> SCS curve number CN=[81],
01089> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
01090> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
01091> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.51](%),
01092> LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
01093> RAINFALL=[ , , , ](mm/hr) , END=-1
01094> *%-----
01095> *
01096> * SUB-AREA No.3
01097> *
01098> CALIB STANDHYD ID=[ 3 ], NYHD=["030"], DT=[2.5](min), AREA=[1.4](ha),
01099> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
01100> SCS curve number CN=[81],
01101> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
01102> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
01103> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.51](%),
01104> LGI=[225.63](m), MNI=[0.03], SCI=[0.0]
01105> RAINFALL=[ , , , ](mm/hr) , END=-1
01106> *%-----
01107> ADD HYD IDsum=[4], NYHD=["040"], IDs to add=[1+2]
01108> *%-----
01109> ADD HYD IDsum=[5], NYHD=["050"], IDs to add=[3+4]
01110> *%-----
01111> *
01112> * SUB-AREA No.4
01113> *
01114> CALIB STANDHYD ID=[6], NYHD=["060"], DT=[2.5](min), AREA=[0.89](ha),
01115> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
01116> SCS curve number CN=[81],
01117> Pervious surfaces: IAPER=[4.67](mm), SLPP=[0.7](%),
01118> LGP=[40](m), MNP=[0.25], SCP=[0.0](min),
01119> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.93](%),
01120> LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
01121> RAINFALL=[ , , , ](mm/hr) , END=-1
01122> *%-----
01123> *
01124> * SUB-AREA No.5
01125> *
01126> CALIB STANDHYD ID=[ 7 ], NYHD=["070"], DT=[2.5](min), AREA=[2.66](ha),
01127> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
01128> SCS curve number CN=[81],
01129> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
01130> LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min),
01131> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.61](%),
01132> LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
01133> RAINFALL=[ , , , ](mm/hr) , END=-1
01134> *%-----
01135> ADD HYD IDsum=[8], NYHD=["080"], IDs to add=[6+7]
01136> *%-----
01137> ADD HYD IDsum=[9], NYHD=["090"], IDs to add=[5+8]
01138> *%-----
01139> *
01140> ROUTE RESERVOIR IDout=[10], NYHD=["POND"], IDin=[9],
01141> RDT=[1.0](min),
01142> TABLE of ( OUTFLOW-STORAGE ) values
01143> (cms) - (ha-m)
01144> [ 0.000, 0.0000 ]
01145> [ 0.008, 0.0656 ]
01146> [ 0.017, 0.1311 ]
01147> [ 0.093, 0.2831 ]
01148> [ 0.233, 0.3971 ]
01149> [ 0.337, 0.4731 ]
01150> [ 0.465, 0.5491 ]
01151> [ 0.531, 0.5871 ]
01152> [ 0.593, 0.6251 ]
01153> [ 0.654, 0.6631 ]
01154> [ 0.797, 0.7391 ]
01155> [ 0.950, 0.8271 ]
01156> [ 1.304, 0.9157 ]
01157> [ 1.880, 1.0040 ]
01158> [ 2.577, 1.0923 ]
01159> [ -1, -1 ] (max twenty pts)
01160>
01161> *****
01162> * Remaining Hawthorne Industrial Park *
01163> *****
01164> *
01165> * SUB-AREA No.1
01166> *
01167> CALIB STANDHYD ID=[ 1 ], NYHD=["HIP01"], DT=[2.5](min), AREA=[19.9](ha),
01168> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
01169> SCS curve number CN=[81],
01170> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
01171> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
01172> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.61](%),
01173> LGI=[580](m), MNI=[0.03], SCI=[0.0]
01174> RAINFALL=[ , , , ](mm/hr) , END=-1
01175> *%-----
01176> ADD HYD IDsum=[ 2 ], NYHD=["HIP02"], IDs to add=[10+1]
01177> *%-----
01178> *
01179> * SUB-AREA No.2
01180> *
01181> CALIB STANDHYD ID=[ 3 ], NYHD=["HIP03"], DT=[2.5](min), AREA=[17](ha),
01182> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
01183> SCS curve number CN=[81],
01184> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
01185> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
01186> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.61](%),
01187> LGI=[450](m), MNI=[0.03], SCI=[0.0]
01188> RAINFALL=[ , , , ](mm/hr) , END=-1
01189> *%-----
01190> *
01191> * SUB-AREA No.3
01192> *
01193> CALIB STANDHYD ID=[ 4 ], NYHD=["HIP04"], DT=[2.5](min), AREA=[18.1](ha),
01194> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
01195> SCS curve number CN=[81],
01196> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
01197> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
01198> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.5](%),
01199> LGI=[600](m), MNI=[0.03], SCI=[0.0]
01200> RAINFALL=[ , , , ](mm/hr) , END=-1
01201> *%-----
01202> ADD HYD IDsum=[ 5 ], NYHD=["HIP05"], IDs to add=[3+4]
01203> *%-----
01204> *
01205> * SUB-AREA No.4
01206> *
01207> DESIGN NASHYD ID=[ 6 ], NYHD=["Pond-Block"], DT=[2.5]min, AREA=[4.0](ha),
01208> DWF=[ 0 ](cms), CN/C=[ 85 ], TP=[0.17]hrs,
01209> RAINFALL=[ , , , ](mm/hr) , END=-1
01210> *%-----
01211> *
01212> *
01213> ADD HYD IDsum=[ 7 ], NYHD=["HIP06"], IDs to add=[2+5+6]
01214> *%-----
01215> *
01216> ROUTE RESERVOIR IDout=[ 8 ], NYHD=["HIP-POND"], IDin=[ 7 ],
01217> RDT=[1.0](min),
01218> TABLE of ( OUTFLOW-STORAGE ) values
01219> (cms) - (ha-m)
01220> [ 0.0, 0.0 ]
01221> [ 0.048, 0.0574 ]
01222> [ 0.054, 0.2434 ]
01223> [ 0.059, 0.5834 ]
01224> [ 0.062, 0.8400 ]
01225> [ 0.064, 1.1024 ]
01226> [ 0.147, 1.3706 ]
01227> [ 0.280, 1.6444 ]
01228> [ 0.472, 1.9242 ]
01229> [ 0.724, 2.2097 ]
01230> [ 0.937, 2.5010 ]
01231> [ 1.262, 2.7981 ]
01232> [ 1.404, 3.1009 ]
01233> [ 1.532, 3.4096 ]
01234> [ 1.650, 3.7240 ]
01235> [ 2.409, 4.0442 ]
01236> [ 3.689, 4.3702 ]
01237> [ -1, -1 ] (max twenty pts)
01238> *%-----
01239> *
01240> * SUB-AREA No. 5
01241> *
01242> *
01243> DESIGN NASHYD ID = [9], NYHD=["A2"], DT=[2.5]min, AREA=[6.8](ha),
01244> DWF=[0](cms), CNC=[76], TP=[0.37]hrs,
01245> RAINFALL=[ , , , ](mm/hr), END=-1
01246> *%-----
01247> *
01248> * SUB-AREA No. 6
01249> *
01250> DESIGN NASHYD ID = [10], NYHD=["A3"], DT=[2.5]min, AREA=[5.3](ha),
01251> DWF=[0](cms), CNC=[76], TP=[0.804]hrs,
01252> RAINFALL=[ , , , ](mm/hr), END=-1
01253> *%-----
01254> ADD HYD IDsum=[1], NYHD=["Interim"], IDs to add=[8+9+10]
01255> *%-----
01256> *
01257> *****
01258> * CALCULATION OF 3HR - 1:100 YEAR STORM EVENT *
01259> *****
01260> *
01261> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01262> *%-----
01263> * [ ] <-- storm filename, one per line for NSTORM time
01264> CHICAGO STORM IUNIT5=[2], TD=[3.0](hrs), TPRAT=[0.333], CSDT=[10.0](min)
01265> ICASE5=[1],
01266> A=[1735.68], B=[6.014], and C=[0.820]
01267> *%-----
01268> DEFAULT VALUES ICASDef=[1], read and print values
01269> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
01270> *%-----
01271> *
01272> *****
01273> * ORGAWORLD FILE *
01274> *****
01275> *
01276> * SUB-AREA No.1
01277> *
01278> CALIB STANDHYD ID=[ 1 ], NYHD=["010"], DT=[2.5](min), AREA=[2.07](ha),
01279> XIMP=[0.84], TIMP=[0.84], DWF=[0.0](cms), LOSS=[2],
01280> SCS curve number CN=[81],
01281> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
01282> LGP=[20](m), MNP=[0.25], SCP=[0.0](min),
01283> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.52](%),
01284> LGI=[204.72](m), MNI=[0.03], SCI=[0.0]
01285> RAINFALL=[ , , , ](mm/hr) , END=-1
01286> *%-----
01287> *
01288> * SUB-AREA No.2
01289> *
01290> CALIB STANDHYD ID=[ 2 ], NYHD=["020"], DT=[2.5](min), AREA=[1.54](ha),
01291> XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
01292> SCS curve number CN=[81],
01293> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
01294> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
01295> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.51](%),
01296> LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
01297> RAINFALL=[ , , , ](mm/hr) , END=-1
01298> *%-----
01299> *
01300> * SUB-AREA No.3
01301> *
01302> CALIB STANDHYD ID=[ 3 ], NYHD=["030"], DT=[2.5](min), AREA=[1.4](ha),
01303> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
01304> SCS curve number CN=[81],
01305> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.0](%),
01306> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
01307> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.51](%),
01308> LGI=[225.63](m), MNI=[0.03], SCI=[0.0]
01309> RAINFALL=[ , , , ](mm/hr) , END=-1
01310> *%-----
01311> ADD HYD IDsum=[4], NYHD=["040"], IDs to add=[1+2]
01312> *%-----
01313> ADD HYD IDsum=[5], NYHD=["050"], IDs to add=[3+4]
01314> *%-----
01315> *
01316> * SUB-AREA No.4
01317> *
01318> CALIB STANDHYD ID=[6], NYHD=["060"], DT=[2.5](min), AREA=[0.89](ha),
01319> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
01320> SCS curve number CN=[81],
01321> Pervious surfaces: IAPER=[4.67](mm), SLPP=[0.7](%),
01322> LGP=[40](m), MNP=[0.25], SCP=[0.0](min),
01323> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.93](%),
01324> LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
01325> RAINFALL=[ , , , ](mm/hr) , END=-1
01326> *%-----
01327> *
01328> * SUB-AREA No.5
01329> *
01330> CALIB STANDHYD ID=[ 7 ], NYHD=["070"], DT=[2.5](min), AREA=[2.66](ha),
01331> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
01332> SCS curve number CN=[81],
01333> Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.5](%),
01334> LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min),
01335> Impervious surfaces: IAIMP=[1.57](mm), SLPI=[0.61](%),
01336> LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
01337> RAINFALL=[ , , , ](mm/hr) , END=-1
01338> *%-----
01339> ADD HYD IDsum=[8], NYHD=["080"], IDs to add=[6+7]
01340> *%-----
01341> ADD HYD IDsum=[9], NYHD=["090"], IDs to add=[5+8]
01342> *%-----
01343> *
01344> ROUTE RESERVOIR IDout=[10], NYHD=["POND"], IDin=[9],
01345> RDT=[1.0](min),
01346> TABLE of ( OUTFLOW-STORAGE ) values
01347> (cms) - (ha-m)
01348> [ 0.000, 0.0000 ]
01349> [ 0.008, 0.0656 ]
01350> [ 0.017, 0.1311 ]

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01351> [ 0.093, 0.2831]
01352> [ 0.233, 0.3971]
01353> [ 0.337, 0.4731]
01354> [ 0.465, 0.5491]
01355> [ 0.531, 0.5871]
01356> [ 0.593, 0.6251]
01357> [ 0.654, 0.6631]
01358> [ 0.797, 0.7331]
01359> [ 0.950, 0.8274]
01360> [ 1.304, 0.9157]
01361> [ 1.880, 1.0040]
01362> [ 2.577, 1.0923]
01363> [ -1, -1 ] (max twenty pts)
01364>
01365> *****
01366> * Remaining Hawthorne Industrial Park *
01367> *****
01368> *
01369> * SUB-AREA No.1
01370>
01371> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01372> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01373> SCS curve number CN=[81],
01374> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01375> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
01376> Impervious surfaces: IAimp=[1.57] (mm), SLPI=[0.6] (%),
01377> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
01378> RAINFALL=[ , , , ] (mm/hr), END=-1
01379> *%-----|
01380> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
01381> *%-----|
01382> *
01383> * SUB-AREA No.2
01384>
01385> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01386> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01387> SCS curve number CN=[81],
01388> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01389> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
01390> Impervious surfaces: IAimp=[1.57] (mm), SLPI=[0.65] (%),
01391> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
01392> RAINFALL=[ , , , ] (mm/hr), END=-1
01393> *%-----|
01394> *
01395> * SUB-AREA No.3
01396>
01397> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[18.1] (ha),
01398> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01399> SCS curve number CN=[81],
01400> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
01401> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
01402> Impervious surfaces: IAimp=[1.57] (mm), SLPI=[0.5] (%),
01403> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
01404> RAINFALL=[ , , , ] (mm/hr), END=-1
01405> *%-----|
01406> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01407> *%-----|
01408> *
01409> * SUB-AREA No.4
01410>
01411> DESIGN NASHYD ID=[ 6 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
01412> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
01413> RAINFALL=[ , , , ] (mm/hr), END=-1
01414> *%-----|
01415> *
01416>
01417> ADD HYD IDsum=[ 7 ], NHYD=["HIP06"], IDs to add=[2+5+6]
01418> *%-----|
01419> *
01420> ROUTE RESERVOIR IDout=[ 8 ], NHYD=["HIP-POND"], IDin=[ 7 ],
01421> RDT=[1.0] (min),
01422> TABLE of ( OUTFLOW-STORAGE ) values
01423> (cms) - (ha-m)
01424> [ 0.0, 0.0 ]
01425> [ 0.048, 0.0574 ]
01426> [ 0.054, 0.2434 ]
01427> [ 0.059, 0.5834 ]
01428> [ 0.062, 0.8400 ]
01429> [ 0.064, 1.1024 ]
01430> [ 0.147, 1.3705 ]
01431> [ 0.280, 1.6444 ]
01432> [ 0.472, 1.9242 ]
01433> [ 0.724, 2.2097 ]
01434> [ 0.937, 2.5010 ]
01435> [ 1.262, 2.7981 ]
01436> [ 1.404, 3.1009 ]
01437> [ 1.532, 3.4096 ]
01438> [ 1.650, 3.7240 ]
01439> [ 2.409, 4.0442 ]
01440> [ 3.689, 4.3702 ]
01441> [ -1, -1 ] (max twenty pts)
01442>
01443> *%-----|
01444> *
01445> * SUB-AREA No. 5
01446> *
01447> DESIGN NASHYD ID = [9], NHYD=["A2"], DT=[2.5] min, AREA=[6.8] (ha),
01448> DWF=[0] (cms), CNC=[76], TP=[0.37] hrs,
01449> RAINFALL=[ , , , ] (mm/hr), END=-1
01450> *%-----|
01451> *
01452> * SUB-AREA No. 6
01453> *
01454> DESIGN NASHYD ID = [10], NHYD=["A3"], DT=[2.5] min, AREA=[5.3] (ha),
01455> DWF=[0] (cms), CNC=[76], TP=[0.804] hrs,
01456> RAINFALL=[ , , , ] (mm/hr), END=-1
01457> *%-----|
01458> ADD HYD IDsum=[1], NHYD=["Interim"], IDs to add=[8+9+10]
01459> *%-----|
01460>
01461>
01462> FINISH

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00136s	over (min)	10.00	30.00						
00137s	Storage Coeff. (min)	= 10.80 (ii)	29.27 (ii)						
00138s	Unit Hyd. Tpeak (min)	= 10.00	30.00						
00139s	Unit Hyd. peak (cms)	= .11	.04						
00140s									*TOTALS*
00141s	PEAK FLOW (cms)	= .16	.00						.158 (iii)
00142s	TIME TO PEAK (hrs)	= 1.29	1.75						1.292
00143s	RUNOFF VOLUME (mm)	= 23.43	5.17						20.508
00144s	TOTAL RAINFALL (mm)	= 25.00	25.00						24.999
00145s	RUNOFF COEFFICIENT	= .94	.21						.820
00147s	(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:								
00148s	CN* = 81.0 Ia = Dep. Storage (Above)								
00149s	(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL								
00150s	THAN THE STORAGE COEFFICIENT.								
00151s	(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.								
00152s									
00153s									
00154s	001:0005-----								
00155s	* SUB-AREA No.2								
00156s	CALIB STANDHYD	Area (ha)	= 1.54						
00159s	02:020 DT= 2.50	Total Imp(%)	= 92.00	Dir. Conn.(%)	= 92.00				
00160s									
00161s		IMPERVIOUS	PERVIOUS (i)						
00162s	Surface Area (ha)	= 1.42	.12						
00163s	Dep. Storage (mm)	= 1.57	4.67						
00164s	Average Slope (%)	= .50	1.00						
00165s	Length (m)	= 244.34	5.00						
00166s	Mannings n	= .030	.030						
00167s									
00168s	Max. eff. Inten. (mm/hr)	= 45.63	7.24						
00169s	over (min)	= 12.50	15.00						
00170s	Storage Coeff. (min)	= 12.15 (ii)	14.15 (ii)						
00171s	Unit Hyd. Tpeak (min)	= 12.50	15.00						
00172s	Unit Hyd. peak (cms)	= .09	.08						
00173s									*TOTALS*
00174s	PEAK FLOW (cms)	= .12	.00						.121 (iii)
00175s	TIME TO PEAK (hrs)	= 1.33	1.46						1.333
00176s	RUNOFF VOLUME (mm)	= 23.43	5.17						21.969
00177s	TOTAL RAINFALL (mm)	= 25.00	25.00						24.999
00178s	RUNOFF COEFFICIENT	= .94	.21						.879
00179s									
00180s	(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:								
00181s	CN* = 81.0 Ia = Dep. Storage (Above)								
00182s	(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL								
00183s	THAN THE STORAGE COEFFICIENT.								
00184s	(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.								
00185s									
00186s									
00187s	001:0006-----								
00188s	* SUB-AREA No.3								
00189s	CALIB STANDHYD	Area (ha)	= 1.40						
00190s	03:030 DT= 2.50	Total Imp(%)	= 97.00	Dir. Conn.(%)	= 97.00				
00191s									
00192s		IMPERVIOUS	PERVIOUS (i)						
00193s	Surface Area (ha)	= 1.36	.04						
00194s	Dep. Storage (mm)	= 1.57	4.67						
00195s	Average Slope (%)	= .51	1.00						
00196s	Length (m)	= 225.63	5.00						
00197s	Mannings n	= .030	.030						
00198s									
00199s	Max. eff. Inten. (mm/hr)	= 45.63	7.97						
00200s	over (min)	= 12.50	12.50						
00201s	Storage Coeff. (min)	= 11.52 (ii)	13.44 (ii)						
00202s	Unit Hyd. Tpeak (min)	= 12.50	12.50						
00203s	Unit Hyd. peak (cms)	= .10	.09						
00204s									*TOTALS*
00205s	PEAK FLOW (cms)	= .12	.00						.118 (iii)
00206s	TIME TO PEAK (hrs)	= 1.33	1.42						1.333
00207s	RUNOFF VOLUME (mm)	= 23.43	5.17						22.881
00208s	TOTAL RAINFALL (mm)	= 25.00	25.00						24.99

00271> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00272> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00273> THAN THE STORAGE COEFFICIENT.  
 00274> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00275>  
 00276>  
 00277> 001:0010  
 00278> \* SUB-AREA No.5  
 00279> | CALIB STANDHYD | Area (ha)= 2.66  
 00280> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 00281>  
 00282> IMPERVIOUS PERVIOUS (i)  
 00283> Surface Area (ha)= 2.58 0.08  
 00284> Dep. Storage (mm)= 1.57 4.67  
 00285> Average Slope (%)= .61 1.50  
 00286> Length (m)= 207.25 20.00  
 00287> Mannings n = .030 .250  
 00288>  
 00289> Max.eff.Inten.(mm/hr)= 45.63 5.66  
 00290> over (min)= 10.00 27.50  
 00291> Storage Coeff. (min)= 10.37 (ii) 26.38 (ii)  
 00292> Unit Hyd. Tpeak (min)= 10.00 27.50  
 00293> Unit Hyd. peak (cms)= .11 .04  
 00294>  
 00295> \*TOTALS\*  
 00296> PEAK FLOW (cms)= .24 .00 .238 (iii)  
 00297> TIME TO PEAK (hrs)= 1.29 1.67 1.292  
 00298> RUNOFF VOLUME (mm)= 23.43 5.17 22.862  
 00299> TOTAL RAINFALL (mm)= 25.00 25.00 24.999  
 00300> RUNOFF COEFFICIENT = .94 .21 .915  
 00301>  
 00302> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00303> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00304> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00305> THAN THE STORAGE COEFFICIENT.  
 00306> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00307>  
 00308>  
 00309>

00310> 001:0011  
 00311>  
 00312> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00313> | | (ha) (cms) (hrs) (mm) (cms)  
 00314> ID1 06:060 .89 .089 1.25 22.88 .000  
 00315> +ID2 07:070 2.66 .238 1.29 22.88 .000  
 00316> =====  
 00317> SUM 08:080 3.55 .327 1.29 22.88 .000  
 00318>  
 00319> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00320>  
 00321>  
 00322> 001:0012  
 00323>  
 00324> | ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00325> | | (ha) (cms) (hrs) (mm) (cms)  
 00326> ID1 05:050 5.01 .396 1.33 21.62 .000  
 00327> +ID2 08:080 3.55 .327 1.29 22.88 .000  
 00328> =====  
 00329> SUM 09:090 8.56 .716 1.29 22.14 .000  
 00330>  
 00331> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00332>  
 00333>

00334> 001:0013  
 00335>  
 00336> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 00337> | IN>09:(090) |  
 00338> | OUT<10:(POND) |  
 00339>  
 00340> ===== OUTFLOW STORAGE TABLE =====  
 00341> OUTFLOW STORAGE | OUTFLOW STORAGE  
 00342> (cms) (ha.m.) | (cms) (ha.m.)  
 00343> .000 .0000E+00 | .593 .6251E+00  
 00344> .008 .6560E-01 | .654 .6631E+00  
 00345> .017 .1311E+00 | .797 .7391E+00  
 00346> .093 .2831E+00 | .950 .8274E+00  
 00347> .233 .3071E+00 | 1.804 .9157E+00  
 00348> .337 .4731E+00 | 1.880 .1004E+01  
 00349> .465 .5491E+00 | 2.577 .1092E+01  
 00350> .531 .5871E+00 | .000 .0000E+00  
 00351>  
 00352> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 00353> (ha) (cms) (hrs) (mm)  
 00354> INFLOW >09: (090) 8.56 .716 1.292 22.143  
 00355> OUTFLOW<10: (POND) 8.56 .032 3.875 22.141  
 00356>  
 00357> PEAK FLOW REDUCTION (Qout/Qin)(%)= 4.470  
 00358> TIME SHIFT OF PEAK FLOW (min)= 155.00  
 00359> MAXIMUM STORAGE USED (ha.m.)=.1611E+00  
 00360>  
 00361> 001:0014  
 00362> \*\*\*\*\* Remaining Hawthorne Industrial Park \*\*\*\*\*  
 00363> \*\*\*\*\*  
 00364> \*  
 00365> \* SUB-AREA No.1  
 00366>  
 00367> | CALIB STANDHYD | Area (ha)= 19.90  
 00368> | 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00369>  
 00370> IMPERVIOUS PERVIOUS (i)  
 00371> Surface Area (ha)= 14.13 5.77  
 00372> Dep. Storage (mm)= 1.57 4.67  
 00373> Average Slope (%)= .60 1.50  
 00374> Length (m)= 580.00 100.00  
 00375> Mannings n = .030 .250  
 00376>  
 00377> Max.eff.Inten.(mm/hr)= 34.39 11.90  
 00378> over (min)= 22.50 52.50  
 00379> Storage Coeff. (min)= 21.64 (ii) 52.88 (ii)  
 00380> Unit Hyd. Tpeak (min)= 22.50 52.50  
 00381> Unit Hyd. peak (cms)= .05 .02  
 00382>  
 00383> \*TOTALS\*  
 00384> PEAK FLOW (cms)= .60 .11 .642 (iii)  
 00385> TIME TO PEAK (hrs)= 1.50 2.13 1.542  
 00386> RUNOFF VOLUME (mm)= 23.43 5.17 22.862  
 00387> TOTAL RAINFALL (mm)= 25.00 25.00 24.999  
 00388> RUNOFF COEFFICIENT = .94 .35 .643  
 00389>  
 00390> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00391> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00392> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00393> THAN THE STORAGE COEFFICIENT.  
 00394> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00395>  
 00396> 001:0015  
 00397>  
 00398> | ADD HYD (H1P02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00399> | | (ha) (cms) (hrs) (mm) (cms)  
 00400> ID1 10:POND 8.56 .032 3.88 22.14 .000  
 00401> +ID2 01:H1P01 19.90 .642 1.54 16.08 .000  
 00402> =====  
 00403> SUM 02:H1P02 28.46 .655 1.54 17.91 .000  
 00404>  
 00405> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00406>  
 00407>  
 00408> 001:0016  
 00409>  
 00410> \* SUB-AREA No.2  
 00411>  
 00412> | CALIB STANDHYD | Area (ha)= 17.00  
 00413> | 03:H1P03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00414>  
 00415> IMPERVIOUS PERVIOUS (i)  
 00416> Surface Area (ha)= 12.07 4.93  
 00417> Dep. Storage (mm)= 1.57 4.67  
 00418> Average Slope (%)= .65 1.50  
 00419> Length (m)= 450.00 100.00  
 00420> Mannings n = .030 .250  
 00421>  
 00422> Max.eff.Inten.(mm/hr)= 40.81 12.73  
 00423> over (min)= 17.50 47.50  
 00424> Storage Coeff. (min)= 16.94 (ii) 47.35 (ii)  
 00425> Unit Hyd. Tpeak (min)= 17.50 47.50  
 00426> Unit Hyd. peak (cms)= .07 .02  
 00427>  
 00428> \*TOTALS\*  
 00429> PEAK FLOW (cms)= .60 .10 .625 (iii)  
 00430> TIME TO PEAK (hrs)= 1.42 2.00 1.458  
 00431> RUNOFF VOLUME (mm)= 23.43 5.17 22.862  
 00432> TOTAL RAINFALL (mm)= 25.00 25.00 24.999  
 00433> RUNOFF COEFFICIENT = .94 .35 .643  
 00434>  
 00435> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00436> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00437> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00438> THAN THE STORAGE COEFFICIENT.  
 00439> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00440>  
 00441> 001:0017  
 00442> \*  
 00443> \* SUB-AREA No.3  
 00444>  
 00445> | CALIB STANDHYD | Area (ha)= 18.10  
 00446> | 04:H1P04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00447>  
 00448> IMPERVIOUS PERVIOUS (i)  
 00449> Surface Area (ha)= 12.85 5.25  
 00450> Dep. Storage (mm)= 1.57 4.67  
 00451> Average Slope (%)= .65 1.50  
 00452> Length (m)= 600.00 100.00  
 00453> Mannings n = .030 .250  
 00454>  
 00455> Max.eff.Inten.(mm/hr)= 34.39 11.54  
 00456> over (min)= 22.50 55.00  
 00457> Storage Coeff. (min)= 23.33 (ii) 54.95 (ii)  
 00458> Unit Hyd. Tpeak (min)= 22.50 55.00  
 00459> Unit Hyd. peak (cms)= .05 .02  
 00460>  
 00461> \*TOTALS\*  
 00462> PEAK FLOW (cms)= .53 .09 .562 (iii)  
 00463> TIME TO PEAK (hrs)= 1.50 2.17 1.542  
 00464> RUNOFF VOLUME (mm)= 23.43 5.17 22.862  
 00465> TOTAL RAINFALL (mm)= 25.00 25.00 24.999  
 00466> RUNOFF COEFFICIENT = .94 .35 .643  
 00467>  
 00468> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00469> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00470> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00471> THAN THE STORAGE COEFFICIENT.  
 00472> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00473>  
 00474> 001:0018  
 00475>  
 00476> | ADD HYD (H1P05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00477> | | (ha) (cms) (hrs) (mm) (cms)  
 00478> ID1 03:H1P03 17.00 .625 1.46 16.08 .000  
 00479> +ID2 04:H1P04 18.10 .562 1.54 16.08 .000  
 00480> =====  
 00481> SUM 05:H1P05 35.10 1.166 1.46 16.08 .000  
 00482>  
 00483> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00484>  
 00485>

00486> 001:0019  
 00487> \*  
 00488> \*SUB-AREA No.4  
 00489>  
 00490> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 00491> | 06:Pond-B DT= 2.50 | Ia = 4.670 # of Linear Res. (N)= 3.00  
 00492> U.H. Tp(hrs)= .170  
 00493>  
 00494> Unit Hyd Qpeak (cms)= .899  
 00495>  
 00496> PEAK FLOW (cms)= .077 (i)  
 00497> TIME TO PEAK (hrs)= 1.375  
 00498> RUNOFF VOLUME (mm)= 6.343  
 00499> TOTAL RAINFALL (mm)= 24.999  
 00500> RUNOFF COEFFICIENT = .294  
 00501>  
 00502> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00503>  
 00504>  
 00505> 001:0020  
 00506>  
 00507> | ADD HYD (H1P06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00508> | | (ha) (cms) (hrs) (mm) (cms)  
 00509> ID1 02:H1P02 28.46 .655 1.54 17.91 .000  
 00510> +ID2 05:H1P05 35.10 1.166 1.46 16.08 .000  
 00511> +ID3 06:Pond-B 4.00 .077 1.38 6.34 .000  
 00512> =====  
 00513> SUM 07:H1P06 67.56 1.887 1.50 16.28 .000  
 00514>  
 00515> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00516>  
 00517>  
 00518> 001:0021  
 00519>  
 00520> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 00521> | IN>07:(H1P06) |  
 00522> | OUT<08:(H1P-PO) |  
 00523>  
 00524> ===== OUTFLOW STORAGE TABLE =====  
 00525> OUTFLOW STORAGE | OUTFLOW STORAGE  
 00526> (cms) (ha.m.) | (cms) (ha.m.)  
 00527> .000 .0000E+00 | .724 .2210E+01  
 00528> .048 .5740E-01 | .937 .2501E+01  
 00529> .054 .2434E+00 | 1.262 .2798E+01  
 00530> .059 .5834E+00 | 1.404 .3101E+01  
 00531> .062 .8400E+00 | 1.532 .3410E+01  
 00532> .064 .1102E+01 | 1.650 .3724E+01  
 00533> .147 .1370E+01 | 2.409 .4044E+01  
 00534> .280 .1644E+01 | 3.689 .4370E+01  
 00535> .472 .1924E+01 | .000 .0000E+00  
 00536>  
 00537> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 00538> (ha) (cms) (hrs) (mm)  
 00539> INFLOW >07: (H1P06) 67.56 1.887 1.500 16.275  
 00540> OUTFLOW<08: (H1P-PO) 67.56 .062 5.417 16.275  
 00541>  
 00542> PEAK FLOW REDUCTION (Qout/Qin)(%)= 3.289

00541> TIME SHIFT OF PEAK FLOW (min)= 235.00  
 00542> MAXIMUM STORAGE USED (ha.m.)=8484E+00  
 00543>  
 00544>  
 00545> 001:0022-----  
 00546> \*  
 00547> \*SUB-AREA No. 5  
 00548>  
 00549>  
 00550> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00  
 00551> | 09:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 00552> | U.H. Tp (hrs)= .370  
 00553>  
 00554> Unit Hyd. Peak (cms)= .702  
 00555>  
 00556> PEAK FLOW (cms)= .053 (i)  
 00557> TIME TO PEAK (hrs)= 1.708  
 00558> RUNOFF VOLUME (mm)= 4.111  
 00559> TOTAL RAINFALL (mm)= 24.999  
 00560> RUNOFF COEFFICIENT = .164  
 00561>  
 00562> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00563>  
 00564>  
 00565> 001:0023-----  
 00566> \*  
 00567> \*SUB-AREA No. 6  
 00568>  
 00569>  
 00570> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00  
 00571> | 10:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 00572> | U.H. Tp (hrs)= .804  
 00573>  
 00574> Unit Hyd. Peak (cms)= .252  
 00575>  
 00576> PEAK FLOW (cms)= .025 (i)  
 00577> TIME TO PEAK (hrs)= 2.333  
 00578> RUNOFF VOLUME (mm)= 4.110  
 00579> TOTAL RAINFALL (mm)= 24.999  
 00580> RUNOFF COEFFICIENT = .164  
 00581>  
 00582> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00583>  
 00584>  
 00585> 001:0024-----  
 00586> | ADD HYD (Interi) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00587> | ID: NHYD (ha) (cms) (hrs) (mm) (cms)  
 00588> ID1 08:H1P-PO 67.56 .052 5.42 16.28 .000  
 00589> ID2 09:A2 6.80 .053 1.71 4.11 .000  
 00590> ID3 10:A3 5.30 .025 2.33 4.11 .000  
 00591> SUM 01:Interi 79.66 .127 1.83 14.43 .000  
 00592>  
 00593> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00594>  
 00595>  
 00596>  
 00597>  
 00598> 001:0025-----  
 00599> \*\*\*\*\*  
 00600> \* CALCULATION OF 3HR - 1.2 YEAR STORM EVENT \*  
 00601> \*\*\*\*\*  
 00602>  
 00603> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWMMHYM-1\  
 00604> | Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMMHYM-1\  
 00605> TZERO = .00 hrs on 0  
 00606> METOUT= 2 (output= METRIC)  
 00607> NRUN = 001  
 00608> NSTORM = 0  
 00609>  
 00610> 001:0002-----  
 00611>  
 00612> | CHICAGO STORM | IDF curve parameters: A= 732.951  
 00613> | Ptotal= 31.86 mm | B= 6.199  
 00614> | C= .810  
 00615> used in: INTENSITY = A / (t + B)^C  
 00616>  
 00617> Duration of storm = 3.00 hrs  
 00618> Storm time step = 10.00 min  
 00619> Time to peak ratio = .33  
 00620>  
 00621> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN  
 00622> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr  
 00623> .17 2.815 | 1.00 76.805 | 1.83 5.095 | 2.67 2.684  
 00624> .33 3.498 | 1.17 24.079 | 2.00 4.291 | 2.83 2.463  
 00625> .50 4.687 | 1.33 12.364 | 2.17 3.718 | 3.00 2.279  
 00626> .67 7.305 | 1.50 8.324 | 2.33 3.288  
 00627> .83 18.209 | 1.67 6.303 | 2.50 2.953 |  
 00628>  
 00629>  
 00630> 001:0003-----  
 00631>  
 00632> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWMMHYM-1\ORGA.VAL  
 00633> | ICASEdv = 1 (read and print data)  
 00634> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---  
 00635> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----  
 00636> Horton's infiltration equation parameters:  
 00637> [F0= 50.00 mm/hr] [F0= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]  
 00638> Parameters for PERVIOUS surfaces in STANDHYD:  
 00639> [Ia= 4.67 mm] [L0=40.00 mm] [DHF= .250]  
 00640> Parameters for IMPERVIOUS surfaces in STANDHYD:  
 00641> [Ia= 1.57 mm] [CLI= 1.50] [MNI= .035]  
 00642> Parameters used in NASHYD:  
 00643> [Ia= 4.67 mm] [N= 3.00]  
 00644>  
 00645> 001:0004-----  
 00646> \*\*\*\*\*  
 00647> \* ORGAWORLD FILE \*  
 00648> \*\*\*\*\*  
 00649>  
 00650> \* SUB-AREA No.1  
 00651>  
 00652> | CALIB STANDHYD | Area (ha)= 2.07  
 00653> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00  
 00654>  
 00655>  
 00656> IMPERVIOUS PERVIOUS (i)  
 00657> Surface Area (ha)= 1.74 .33  
 00658> Dep. Storage (mm)= 1.57 4.67  
 00659> Average Slope (%)= .52 1.00  
 00660> Length (m)= 204.72 20.00  
 00661> Mannings n = .030 .250  
 00662>  
 00663> Max. eff. Inten. (mm/hr)= 76.81 11.88  
 00664> over (min)= 10.00 22.50  
 00665> Storage Coeff. (min)= 8.77 (ii) 22.21 (ii)  
 00666> Unit Hyd. Tpeak (min)= 10.00 22.50  
 00667> Unit Hyd. peak (cms)= .12 .05  
 00668>  
 00669> PEAK FLOW (cms)= .24 .01 \*TOTALS\*  
 00670> TIME TO PEAK (hrs)= 1.08 1.38 .139 (iii)  
 00671> RUNOFF VOLUME (mm)= 30.29 8.52 26.807  
 00672> TOTAL RAINFALL (mm)= 31.86 31.86 31.860  
 00673> RUNOFF COEFFICIENT = .95 .27 .841  
 00674>  
 00675> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00676> CN\* = 81.0 Ia = Dep. Storage (Above)

00676> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00677> THAN THE STORAGE COEFFICIENT.  
 00678> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00679>  
 00680>  
 00681> 001:0005-----  
 00682> \*  
 00683> \* SUB-AREA No.2  
 00684>  
 00685> | CALIB STANDHYD | Area (ha)= 1.54  
 00686> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00  
 00687>  
 00688>  
 00689> IMPERVIOUS PERVIOUS (i)  
 00690> Surface Area (ha)= 1.42 .12  
 00691> Dep. Storage (mm)= 1.57 4.67  
 00692> Average Slope (%)= .50 1.00  
 00693> Length (m)= 244.34 5.00  
 00694> Mannings n = .030 .030  
 00695>  
 00696> Max. eff. Inten. (mm/hr)= 76.81 15.07  
 00697> over (min)= 10.00 12.50  
 00698> Storage Coeff. (min)= 9.87 (ii) 11.36 (ii)  
 00699> Unit Hyd. Tpeak (min)= 10.00 12.50  
 00700> Unit Hyd. peak (cms)= .11 .10  
 00701>  
 00702> PEAK FLOW (cms)= .19 .00 \*TOTALS\*  
 00703> TIME TO PEAK (hrs)= 1.08 1.17 .192 (iii)  
 00704> RUNOFF VOLUME (mm)= 30.29 8.52 28.548  
 00705> TOTAL RAINFALL (mm)= 31.86 31.86 31.860  
 00706> RUNOFF COEFFICIENT = .95 .27 .896  
 00707>  
 00708> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00709> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00710> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00711> THAN THE STORAGE COEFFICIENT.  
 00712> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00713>  
 00714>  
 00715> 001:0006-----  
 00716> \*  
 00717> \* SUB-AREA No.3  
 00718>  
 00719> | CALIB STANDHYD | Area (ha)= 1.40  
 00720> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 00721>  
 00722>  
 00723> IMPERVIOUS PERVIOUS (i)  
 00724> Surface Area (ha)= 1.36 .04  
 00725> Dep. Storage (mm)= 1.57 4.67  
 00726> Average Slope (%)= .51 1.00  
 00727> Length (m)= 225.63 5.00  
 00728> Mannings n = .030 .030  
 00729>  
 00730> Max. eff. Inten. (mm/hr)= 76.81 16.59  
 00731> over (min)= 10.00 10.00  
 00732> Storage Coeff. (min)= 9.35 (ii) 10.79 (ii)  
 00733> Unit Hyd. Tpeak (min)= 10.00 10.00  
 00734> Unit Hyd. peak (cms)= .12 .11  
 00735>  
 00736> PEAK FLOW (cms)= .18 .00 \*TOTALS\*  
 00737> TIME TO PEAK (hrs)= 1.13 1.08 .186 (iii)  
 00738> RUNOFF VOLUME (mm)= 30.29 8.52 29.637  
 00739> TOTAL RAINFALL (mm)= 31.86 31.86 31.860  
 00740> RUNOFF COEFFICIENT = .95 .27 .930  
 00741>  
 00742> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00743> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00744> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00745> THAN THE STORAGE COEFFICIENT.  
 00746> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00747>  
 00748> 001:0007-----  
 00749> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00750> | ID: NHYD (ha) (cms) (hrs) (mm) (cms)  
 00751> ID1 01:010 2.07 .245 1.08 26.81 .000  
 00752> ID2 02:020 1.54 .192 1.08 28.55 .000  
 00753> SUM 04:040 3.61 .436 1.08 27.55 .000  
 00754>  
 00755> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00756>  
 00757>  
 00758>  
 00759>  
 00760> 001:0008-----  
 00761> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00762> | ID: NHYD (ha) (cms) (hrs) (mm) (cms)  
 00763> ID1 03:030 1.40 .186 1.08 29.64 .000  
 00764> ID2 04:040 3.61 .436 1.08 27.55 .000  
 00765> SUM 05:050 5.01 .623 1.08 28.13 .000  
 00766>  
 00767> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00768>  
 00769>  
 00770>  
 00771> 001:0009-----  
 00772> \*  
 00773> \* SUB-AREA No.4  
 00774>  
 00775> | CALIB STANDHYD | Area (ha)= .89  
 00776> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 00777>  
 00778>  
 00779> IMPERVIOUS PERVIOUS (i)  
 00780> Surface Area (ha)= .86 .03  
 00781> Dep. Storage (mm)= 1.57 4.67  
 00782> Average Slope (%)= .93 .70  
 00783> Length (m)= 164.82 40.00  
 00784> Mannings n = .030 .250  
 00785>  
 00786> Max. eff. Inten. (mm/hr)= 76.81 10.24  
 00787> over (min)= 7.50 30.00  
 00788> Storage Coeff. (min)= 6.47 (ii) 30.53 (ii)  
 00789> Unit Hyd. Tpeak (min)= 7.50 30.00  
 00790> Unit Hyd. peak (cms)= .16 .04  
 00791>  
 00792> PEAK FLOW (cms)= .14 .00 \*TOTALS\*  
 00793> TIME TO PEAK (hrs)= 1.04 1.54 .139 (iii)  
 00794> RUNOFF VOLUME (mm)= 30.29 8.52 29.637  
 00795> TOTAL RAINFALL (mm)= 31.86 31.86 31.860  
 00796> RUNOFF COEFFICIENT = .95 .27 .930  
 00797>  
 00798> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00799> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00800> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00801> THAN THE STORAGE COEFFICIENT.  
 00802> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00803>  
 00804> 001:0010-----  
 00805> \*  
 00806> \* SUB-AREA No.5  
 00807>  
 00808> | CALIB STANDHYD | Area (ha)= 2.66  
 00809> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 00810>

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00811> Surface Area (ha)= 2.58 IMPERVIOUS PERVIOUS (i)
00812> Dep. Storage (mm)= 1.57 4.67
00813> Average Slope (%)= .61 1.50
00815> Length (m)= 207.25 20.00
00816> Mannings n = .030 .250
00817> Max. eff. Inten. (mm/hr)= 76.81 12.71
00819> over (min)= 7.50 20.00
00820> Storage Coeff. (min)= 8.42 (ii) 20.00 (ii)
00821> Unit Hyd. Tpeak (min)= 7.50 20.00
00822> Unit Hyd. peak (cms)= .14 .06
00823>
00824> PEAK FLOW (cms)= .38 .00 *TOTALS*
00825> TIME TO PEAK (hrs)= 1.04 1.33 1.042 (iii)
00826> RUNOFF VOLUME (mm)= 30.29 8.52 29.637
00827> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00828> RUNOFF COEFFICIENT = .95 .27 .930
00829>
00830> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00831> CN* = 81.0 Ia = Dep. Storage (Above)
00832> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00833> THAN THE STORAGE COEFFICIENT.
00834> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00835>
00836> -----
00837> 001:0011-----
00838> | ADD HYD (080 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00839> | (ha) (cms) (hrs) (mm) (cms)
00840> ID1 06:060 .89 .139 1.04 29.64 .000
00842> +ID2 07:070 2.66 .379 1.04 29.64 .000
00843>
00844> SUM 08:080 3.55 .518 1.04 29.64 .000
00845>
00846> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00847>
00848> -----
00849> 001:0012-----
00850> | ADD HYD (090 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00851> | (ha) (cms) (hrs) (mm) (cms)
00852> ID1 05:050 5.01 .623 1.08 28.13 .000
00854> +ID2 08:080 3.55 .518 1.04 29.64 .000
00855>
00856> SUM 09:090 8.56 1.118 1.08 28.76 .000
00857>
00858> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00859>
00860> -----
00861> 001:0013-----
00862> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00863> | IN:09 (POND ) |
00865> | OUT:10 (POND ) |
00866>
00867> ===== OUTFLOW STORAGE TABLE =====
00868> OUTFLOW STORAGE OUTFLOW STORAGE
00869> (cms) (ha.m.) (cms) (ha.m.)
00870> .000 .0000E+00 .593 .6231E+00
00871> .008 .6560E-01 .654 .6631E+00
00872> .017 .1311E+00 .797 .7391E+00
00873> .093 .2831E+00 .950 .8274E+00
00874> .233 .3971E+00 1.304 .9157E+00
00875> .337 .4731E+00 1.880 .1004E+01
00876> .465 .5491E+00 2.577 .1092E+01
00877> .531 .5871E+00 .000 .0000E+00
00878>
00879> ROUTING RESULTS AREA OPEAK TPEAK R.V.
00880> (ha) (cms) (hrs) (mm)
00881> INFLOW >09: (090 ) 8.56 1.118 1.08 28.757
00882> OUTFLOW<10: (POND ) 8.56 .056 3.000 28.754
00883>
00884> PEAK FLOW REDUCTION [Qout/Qin] (%)= 5.030
00885> TIME SHIFT OF PEAK FLOW (min)= 115.00
00886> MAXIMUM STORAGE USED (ha.m.)= 2095E+00
00887>
00888> -----
00889> 001:0014-----
00890> * SUB-AREA No.1
00891>
00892> * CALIB STANDHYD | Area (ha)= 19.90
00893> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00894>
00895>
00896>
00897> Surface Area (ha)= 14.13 IMPERVIOUS PERVIOUS (i)
00898> Dep. Storage (mm)= 1.57 4.67
00899> Average Slope (%)= .60 1.50
00900> Length (m)= 580.00 100.00
00901> Mannings n = .030 .250
00902>
00903> Max. eff. Inten. (mm/hr)= 54.21 23.06
00904> over (min)= 17.50 42.50
00905> Storage Coeff. (min)= 18.04 (ii) 42.02 (ii)
00906> Unit Hyd. Tpeak (min)= 17.50 42.50
00907> Unit Hyd. peak (cms)= .06 .03
00908>
00909> PEAK FLOW (cms)= .95 .21 *TOTALS*
00910> TIME TO PEAK (hrs)= 1.21 1.71 1.250 (iii)
00911> RUNOFF VOLUME (mm)= 30.29 13.34 21.814
00912> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00913> RUNOFF COEFFICIENT = .95 .42 .685
00914>
00915> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00916> CN* = 81.0 Ia = Dep. Storage (Above)
00917> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00918> THAN THE STORAGE COEFFICIENT.
00919> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00920>
00921> -----
00922> 001:0015-----
00923> | ADD HYD (HIP02 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00924> | (ha) (cms) (hrs) (mm) (cms)
00925> ID1 10:POND 8.56 .056 3.00 28.75 .000
00926> +ID2 01:HIP01 19.90 1.020 1.25 21.81 .000
00927>
00928> SUM 02:HIP02 28.46 1.039 1.25 23.90 .000
00929>
00930> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00931>
00932> -----
00933> 001:0016-----
00934> * SUB-AREA No.2
00935>
00936> * CALIB STANDHYD | Area (ha)= 17.00
00937> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00938>
00939>
00940>
00941> Surface Area (ha)= 12.07 IMPERVIOUS PERVIOUS (i)
00942> Dep. Storage (mm)= 1.57 4.67
00943> Average Slope (%)= .65 1.50
00944>
00945>

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00946> Length (m)= 450.00 100.00
00947> Mannings n = .030 .250
00948>
00949> Max. eff. Inten. (mm/hr)= 59.23 25.04
00950> over (min)= 15.00 37.50
00951> Storage Coeff. (min)= 14.60 (ii) 37.80 (ii)
00952> Unit Hyd. Tpeak (min)= 15.00 37.50
00953> Unit Hyd. peak (cms)= .08 .03
00954>
00955> PEAK FLOW (cms)= .91 .19 *TOTALS*
00956> TIME TO PEAK (hrs)= 1.17 1.63 1.167 (iii)
00957> RUNOFF VOLUME (mm)= 30.29 13.34 21.814
00958> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00959> RUNOFF COEFFICIENT = .95 .42 .685
00960>
00961> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00962> CN* = 81.0 Ia = Dep. Storage (Above)
00963> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00964> THAN THE STORAGE COEFFICIENT.
00965> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00966>
00967> -----
00968> 001:0017-----
00969> * SUB-AREA No.3
00970>
00971> * CALIB STANDHYD | Area (ha)= 18.10
00972> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00973>
00974>
00975> Surface Area (ha)= 12.85 IMPERVIOUS PERVIOUS (i)
00976> Dep. Storage (mm)= 1.57 4.67
00977> Average Slope (%)= .50 1.50
00978> Length (m)= 600.00 100.00
00979> Mannings n = .030 .250
00980>
00981> Max. eff. Inten. (mm/hr)= 50.44 22.17
00982> over (min)= 20.00 45.00
00983> Storage Coeff. (min)= 20.01 (ii) 44.37 (ii)
00984> Unit Hyd. Tpeak (min)= 20.00 45.00
00985> Unit Hyd. peak (cms)= .06 .03
00986>
00987> PEAK FLOW (cms)= .80 .18 *TOTALS*
00988> TIME TO PEAK (hrs)= 1.25 1.79 .874 (iii)
00989> RUNOFF VOLUME (mm)= 30.29 13.34 21.814
00990> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00991> RUNOFF COEFFICIENT = .95 .42 .685
00992>
00993> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00994> CN* = 81.0 Ia = Dep. Storage (Above)
00995> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00996> THAN THE STORAGE COEFFICIENT.
00997> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00998>
00999> -----
01000> 001:0018-----
01001> | ADD HYD (HIP05 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01002> | (ha) (cms) (hrs) (mm) (cms)
01003> ID1 03:HIP03 17.00 .978 1.17 21.81 .000
01004> +ID2 04:HIP04 18.10 .874 1.29 21.81 .000
01005>
01006> SUM 05:HIP05 35.10 1.814 1.21 21.81 .000
01007>
01008> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01009>
01010> -----
01011> 001:0019-----
01012> * SUB-AREA No.4
01013>
01014> * DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
01015> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01016> | U.H. Tp(hrs)= .170
01017>
01018>
01019> Unit Hyd. Tpeak (cms)= .899
01020>
01021> PEAK FLOW (cms)= .145 (i)
01022> TIME TO PEAK (hrs)= 1.167
01023> RUNOFF VOLUME (mm)= 10.266
01024> TOTAL RAINFALL (mm)= 31.860
01025> RUNOFF COEFFICIENT = .322
01026>
01027> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01028>
01029> -----
01030> 001:0020-----
01031> | ADD HYD (HIP06 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01032> | (ha) (cms) (hrs) (mm) (cms)
01033> ID1 02:HIP02 28.46 1.039 1.25 23.90 .000
01034> +ID2 05:HIP05 35.10 1.814 1.21 21.81 .000
01035> +ID3 06:Pond-B 4.00 .145 1.17 10.27 .000
01036>
01037> SUM 07:HIP06 67.56 2.992 1.21 22.01 .000
01038>
01039> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01040>
01041> -----
01042> 001:0021-----
01043> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01044> | IN:07 (HIP06 ) |
01045> | OUT:08 (HIP-PO) |
01046>
01047> ===== OUTFLOW STORAGE TABLE =====
01048> OUTFLOW STORAGE OUTFLOW STORAGE
01049> (cms) (ha.m.) (cms) (ha.m.)
01050> .000 .0000E+00 .724 .2210E+01
01051> .048 .5740E-01 .937 .2501E+01
01052> .054 .2434E+00 1.262 .2798E+01
01053> .059 .5834E+00 1.404 .3101E+01
01054> .062 .8400E+00 1.532 .3410E+01
01055> .064 .1102E+01 1.650 .3724E+01
01056> .147 .1370E+01 2.409 .4044E+01
01057> .280 .1644E+01 3.689 .4370E+01
01058> .472 .1924E+01 .000 .0000E+00
01059>
01060>
01061> ROUTING RESULTS AREA OPEAK TPEAK R.V.
01062> (ha) (cms) (hrs) (mm)
01063> INFLOW >07: (HIP06 ) 67.56 2.992 1.208 22.009
01064> OUTFLOW<08: (HIP-PO) 67.56 .093 4.444 22.009
01065>
01066> PEAK FLOW REDUCTION [Qout/Qin] (%)= 3.122
01067> TIME SHIFT OF PEAK FLOW (min)= 194.17
01068> MAXIMUM STORAGE USED (ha.m.)= .1197E+01
01069>
01070> -----
01071> 001:0022-----
01072> * SUB-AREA No. 5
01073>
01074> * DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00
01075> | 09:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01076> | U.H. Tp(hrs)= .370
01077>
01078>
01079>
01080>

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01081> Unit Hyd Qpeak (cms) = .702  
 01082> PEAK FLOW (cms) = .102 (i)  
 01083> TIME TO PEAK (hrs) = 1.458  
 01084> RUNOFF VOLUME (mm) = 6.883  
 01085> TOTAL RAINFALL (mm) = 31.860  
 01086> RUNOFF COEFFICIENT = .216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01092> 001:0023  
 01093> \* SUB-AREA No. 6  
 01094> \*  
 01095> \*  
 01096> \*  
 01097> | DESIGN NASHYD | Area (ha) = 5.30 Curve Number (CN) = 76.00  
 01098> | 10:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00  
 01099> | U.H. Tp(hrs) = .804

01100> Unit Hyd Qpeak (cms) = .252  
 01101> PEAK FLOW (cms) = .048 (i)  
 01102> TIME TO PEAK (hrs) = 2.083  
 01103> RUNOFF VOLUME (mm) = 6.883  
 01104> TOTAL RAINFALL (mm) = 31.860  
 01105> RUNOFF COEFFICIENT = .216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01112> 001:0024  
 01113> | ADD HYD (Interi) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 01114> | ID1 08:HYP-PO | 67.56 0.093 4.44 22.01 .000  
 01115> | +ID2 09:A2 | 6.80 .102 1.46 6.88 .000  
 01116> | +ID3 10:A3 | 5.30 .048 2.08 6.88 .000  
 01117> SUM 01:Interi 79.66 .194 1.58 19.71 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01122> 001:0025  
 01123> \*\*\*\*\*  
 01124> \* CALCULATION OF 3HR - 1.5 YEAR STORM EVENT \*  
 01125> \*\*\*\*\*  
 01126> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWMMHYM-1\  
 01127> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMMHYM-1\  
 01128> | METOUT = 2 (output = METRIC)  
 01129> | NRUN = 001  
 01130> | NSTORM = 0

01137> 001:0002  
 01138> | CHICAGO STORM | IDF curve parameters: A= 998.071  
 01139> | Ptotal = 42.51 mm B= 6.053  
 01140> | C= .814  
 01141> used in: INTENSITY = A / (t + B)^C

01142> Duration of storm = 3.00 hrs  
 01143> Storm time step = 10.00 min  
 01144> Time to peak ratio = .33  
 01145> TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
 01146> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
 01147> .17 3.684 1.00 104.193 1.83 6.689 2.67 3.510  
 01148> .33 4.582 1.17 32.037 2.00 5.628 2.83 3.220  
 01149> .50 6.151 1.33 16.337 2.17 4.872 3.00 2.978  
 01150> .67 9.614 1.50 10.965 2.33 4.305  
 01151> .83 24.170 1.67 8.287 2.50 3.864

01156> 001:0003  
 01157> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWMMHYM-1\ORGA.VAL  
 01158> | FileTitle = | CASEEv = 1 (read and print data)  
 01159> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---  
 01160> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---  
 01161> Horton's infiltration equation parameters:  
 01162> [F= 50.00 mm/hr] [F= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]  
 01163> Parameters for PERVIOUS surfaces in STANDHYD:  
 01164> [IAPER= 4.67 mm] [LGP=40.00 mm] [MNI= .250]  
 01165> Parameters for IMPVIOUS surfaces in STANDHYD:  
 01166> [IAPER= 1.57 mm] [CLI= 1.50] [MNI= .035]  
 01167> Parameters used in NASHYD:  
 01168> [Ia= 4.67 mm] [N= 3.00]

01172> 001:0004  
 01173> \*\*\*\*\*  
 01174> \* ORGAWORLD FILE \*  
 01175> \*\*\*\*\*

01176> \* SUB-AREA No. 1  
 01177> \*  
 01178> \*  
 01179> | CALIB STANDHYD | Area (ha) = 2.07  
 01180> | 01:010 DT= 2.50 | Total Imp(%) = 84.00 Dir. Conn.(%) = 84.00  
 01181> \*  
 01182> IMPVIOUS PERVIOUS (i)  
 01183> Surface Area (ha) = 1.74 .33  
 01184> Dep. Storage (mm) = 1.57 4.67  
 01185> Average Slope (%) = .52 1.00  
 01186> Length (m) = 204.72 20.00  
 01187> Mannings n = .030 .250  
 01188> Max. eff. Inten. (mm/hr) = 104.19 24.26  
 01189> over (min) = 7.50 17.50  
 01190> Storage Coeff. (min) = 7.76 (ii) 17.96 (ii)  
 01191> Unit Hyd. Tpeak (min) = 7.50 17.50  
 01192> Unit Hyd. peak (cms) = .15 .06  
 01193> \*TOTALS\*  
 01194> PEAK FLOW (cms) = .36 .01 .362 (iii)  
 01195> TIME TO PEAK (hrs) = 1.04 1.25 1.042  
 01196> RUNOFF VOLUME (mm) = 40.94 14.70 36.745  
 01197> TOTAL RAINFALL (mm) = 42.51 42.51 42.514  
 01198> RUNOFF COEFFICIENT = .96 .35 .864

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01208> 001:0005  
 01209> \* SUB-AREA No. 2  
 01210> \*  
 01211> \*  
 01212> | CALIB STANDHYD | Area (ha) = 1.54  
 01213> | 02:020 DT= 2.50 | Total Imp(%) = 92.00 Dir. Conn.(%) = 92.00  
 01214> \*  
 01215> IMPVIOUS PERVIOUS (i)

01216> Surface Area (ha) = 1.42 .12  
 01217> Dep. Storage (mm) = 1.57 4.67  
 01218> Average Slope (%) = .50 1.00  
 01219> Length (m) = 244.34 5.00  
 01220> Mannings n = .030 .030  
 01221> Max. eff. Inten. (mm/hr) = 104.19 31.02  
 01222> over (min) = 7.50 10.00  
 01223> Storage Coeff. (min) = 8.73 (ii) 9.85 (ii)  
 01224> Unit Hyd. Tpeak (min) = 7.50 10.00  
 01225> Unit Hyd. peak (cms) = .14 .11  
 01226> \*TOTALS\*  
 01227> PEAK FLOW (cms) = .28 .01 .283 (iii)  
 01228> TIME TO PEAK (hrs) = 1.04 1.13 1.042  
 01229> RUNOFF VOLUME (mm) = 40.94 14.70 38.845  
 01230> TOTAL RAINFALL (mm) = 42.51 42.51 42.514  
 01231> RUNOFF COEFFICIENT = .96 .35 .914

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0006  
 01243> \* SUB-AREA No. 3  
 01244> \*  
 01245> | CALIB STANDHYD | Area (ha) = 1.40  
 01246> | 02:030 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00  
 01247> \*  
 01248> IMPVIOUS PERVIOUS (i)  
 01249> Surface Area (ha) = 1.36 .04  
 01250> Dep. Storage (mm) = 1.57 4.67  
 01251> Average Slope (%) = .51 1.00  
 01252> Length (m) = 225.63 5.00  
 01253> Mannings n = .030 .030  
 01254> Max. eff. Inten. (mm/hr) = 104.19 31.02  
 01255> over (min) = 7.50 10.00  
 01256> Storage Coeff. (min) = 8.28 (ii) 9.39 (ii)  
 01257> Unit Hyd. Tpeak (min) = 7.50 10.00  
 01258> Unit Hyd. peak (cms) = .14 .12  
 01259> \*TOTALS\*  
 01260> PEAK FLOW (cms) = .27 .00 .274 (iii)  
 01261> TIME TO PEAK (hrs) = 1.04 1.13 1.042  
 01262> RUNOFF VOLUME (mm) = 40.94 14.70 40.157  
 01263> TOTAL RAINFALL (mm) = 42.51 42.51 42.514  
 01264> RUNOFF COEFFICIENT = .96 .35 .945

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0007  
 01275> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 01276> | ID1 01:010 | 2.07 .362 1.04 36.75 .000  
 01277> | +ID2 02:020 | 1.54 .283 1.04 38.84 .000  
 01278> SUM 04:040 3.61 .645 1.04 37.64 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0008  
 01288> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 01289> | ID1 03:030 | 1.40 .274 1.04 40.16 .000  
 01290> | +ID2 04:040 | 3.61 .645 1.04 37.64 .000  
 01291> SUM 05:050 5.01 .918 1.04 38.34 .000

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

001:0009  
 01298> \* SUB-AREA No. 4  
 01299> \*  
 01300> \*  
 01301> \*  
 01302> | CALIB STANDHYD | Area (ha) = .89  
 01303> | 06:060 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00  
 01304> \*  
 01305> IMPVIOUS PERVIOUS (i)  
 01306> Surface Area (ha) = .86 .03  
 01307> Dep. Storage (mm) = 1.57 4.67  
 01308> Average Slope (%) = .93 .70  
 01309> Length (m) = 164.82 40.00  
 01310> Mannings n = .030 .250  
 01311> Max. eff. Inten. (mm/hr) = 104.19 20.32  
 01312> over (min) = 5.00 25.00  
 01313> Storage Coeff. (min) = 5.72 (ii) 24.02 (ii)  
 01314> Unit Hyd. Tpeak (min) = 5.00 25.00  
 01315> Unit Hyd. peak (cms) = .20 .05  
 01316> \*TOTALS\*  
 01317> PEAK FLOW (cms) = .20 .00 .205 (iii)  
 01318> TIME TO PEAK (hrs) = 1.00 1.38 1.000  
 01319> RUNOFF VOLUME (mm) = 40.94 14.70 40.157  
 01320> TOTAL RAINFALL (mm) = 42.51 42.51 42.514  
 01321> RUNOFF COEFFICIENT = .96 .35 .945

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0010  
 01333> \* SUB-AREA No. 5  
 01334> \*  
 01335> | CALIB STANDHYD | Area (ha) = 2.66  
 01336> | 07:070 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00  
 01337> \*  
 01338> IMPVIOUS PERVIOUS (i)  
 01339> Surface Area (ha) = 2.58 .08  
 01340> Dep. Storage (mm) = 1.57 4.67  
 01341> Average Slope (%) = .61 1.50  
 01342> Length (m) = 207.25 20.00  
 01343> Mannings n = .030 .250  
 01344> Max. eff. Inten. (mm/hr) = 104.19 24.26  
 01345> over (min) = 7.50 17.50  
 01346> Storage Coeff. (min) = 7.45 (ii) 16.40 (ii)  
 01347> Unit Hyd. Tpeak (min) = 7.50 17.50  
 01348> Unit Hyd. peak (cms) = .15 .07  
 01349> \*TOTALS\*  
 01350> PEAK FLOW (cms) = .36 .01 .362 (iii)  
 01351> TIME TO PEAK (hrs) = 1.04 1.25 1.042  
 01352> RUNOFF VOLUME (mm) = 40.94 14.70 36.745  
 01353> TOTAL RAINFALL (mm) = 42.51 42.51 42.514  
 01354> RUNOFF COEFFICIENT = .96 .35 .864

01351> PEAK FLOW (cms)= .54 .00 .538 (iii)  
 01352> TIME TO PEAK (hrs)= 1.04 1.25 1.042  
 01353> RUNOFF VOLUME (mm)= 40.94 14.70 40.157  
 01354> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01355> RUNOFF COEFFICIENT = .96 .35 .945  
 01356>

01357> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 01358> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01359> THAN THE STORAGE COEFFICIENT.  
 01360> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01361>  
 01362>

01363> 001:0011-----  
 01364> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01365> (ha) (cms) (hrs) (mm) (cms)  
 01366> ID1 06:060 .89 .205 1.00 40.16 .000  
 01367> +ID2 07:070 2.66 .538 1.04 40.16 .000  
 01368> SUM 08:080 3.55 .733 1.04 40.16 .000  
 01369>

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01370> 001:0012-----  
 01371> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01372> (ha) (cms) (hrs) (mm) (cms)  
 01373> ID1 05:050 5.01 .918 1.04 36.34 .000  
 01374> +ID2 08:080 3.55 .733 1.04 40.16 .000  
 01375> SUM 09:090 8.56 1.651 1.04 39.10 .000  
 01376>

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01377> 001:0013-----  
 01378> | ROUTE RESERVOIR Requested routing time step = 1.0 min.  
 01379> | IN>09: (090 ) |  
 01380> | OUT<10: (POND ) |

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
01381>	.000	.0000E+00	.593	.6251E+00
01382>	.008	.6560E-01	.654	.6631E+00
01383>	.017	.1311E+00	.797	.7391E+00
01384>	.093	.2831E+00	.950	.8274E+00
01385>	.233	.3971E+00	1.304	.9157E+00
01386>	.337	.4731E+00	1.880	.1004E+01
01387>	.465	.5491E+00	2.577	.1092E+01
01388>	.531	.5871E+00	.000	.0000E+00

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
01403>	8.56	1.651	1.042	39.096
01404>	8.56	.089	2.625	39.093

01405> PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.413  
 01406> TIME SHIFT OF PEAK FLOW (min) = 95.00  
 01407> MAXIMUM STORAGE USED (ha.m.) = 2758E+00  
 01408>

01409> 001:0014-----  
 01410> \*\*\*\*\* Remaining Hawthorne Industrial Park \*\*\*\*\*  
 01411> \*\*\*\*\*  
 01412> \* SUB-AREA No.1

01413> 01414> | CALIB STANDHYD | Area (ha)= 19.90  
 01415> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01416>

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01424>	14.13	5.77
01425>	14.13	5.77
01426>	14.13	5.77
01427>	14.13	5.77
01428>	14.13	5.77
01429>	14.13	5.77
01430>	14.13	5.77

	Max. eff. Inten. (mm/hr) over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
01431>	80.14	15.00	15.00	.07
01432>	80.14	15.00	15.00	.07
01433>	80.14	15.00	15.00	.07
01434>	80.14	15.00	15.00	.07
01435>	80.14	15.00	15.00	.07

01436> \*TOTALS\*  
 01437> PEAK FLOW (cms)= 1.41 .40 1.572 (iii)  
 01438> TIME TO PEAK (hrs)= 1.17 1.54 1.208  
 01439> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01440> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01441> RUNOFF COEFFICIENT = .96 .50 .732  
 01442>

01443> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 01444> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01445> THAN THE STORAGE COEFFICIENT.  
 01446> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01447>  
 01448>

01449> 001:0015-----  
 01450> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01451> (ha) (cms) (hrs) (mm) (cms)  
 01452> ID1 10: POND 8.56 .089 2.63 39.09 .000  
 01453> +ID2 01:HIP01 19.90 1.572 1.21 31.13 .000  
 01454> SUM 02:HIP02 28.46 1.615 1.21 33.52 .000  
 01455>

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01456> 001:0016-----  
 01457> \* SUB-AREA No.2

01458> 01459> | CALIB STANDHYD | Area (ha)= 17.00  
 01460> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01461>

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01469>	12.07	4.93
01470>	12.07	4.93
01471>	12.07	4.93
01472>	12.07	4.93
01473>	12.07	4.93
01474>	12.07	4.93
01475>	12.07	4.93

	Max. eff. Inten. (mm/hr) over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
01476>	89.76	12.50	12.50	.09
01477>	89.76	12.50	12.50	.09
01478>	89.76	12.50	12.50	.09
01479>	89.76	12.50	12.50	.09
01480>	89.76	12.50	12.50	.09

01481> \*TOTALS\*  
 01482> PEAK FLOW (cms)= 1.36 .37 1.504 (iii)  
 01483> TIME TO PEAK (hrs)= 1.13 1.46 1.167  
 01484> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01485> TOTAL RAINFALL (mm)= 42.51 42.51 42.514

01486> RUNOFF COEFFICIENT = .96 .50 .732

01487> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 01488> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01489> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01490> THAN THE STORAGE COEFFICIENT.  
 01491> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01492>  
 01493>

01494> 001:0017-----  
 01495> \* SUB-AREA No.3  
 01496> | CALIB STANDHYD | Area (ha)= 18.10  
 01497> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 01498>

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
01502>	12.85	5.25
01503>	12.85	5.25
01504>	12.85	5.25
01505>	12.85	5.25
01506>	12.85	5.25
01507>	12.85	5.25

	Max. eff. Inten. (mm/hr) over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
01508>	73.27	17.50	17.50	.07
01509>	73.27	17.50	17.50	.07
01510>	73.27	17.50	17.50	.07
01511>	73.27	17.50	17.50	.07
01512>	73.27	17.50	17.50	.07

01513> \*TOTALS\*  
 01514> PEAK FLOW (cms)= 1.19 .35 1.364 (iii)  
 01515> TIME TO PEAK (hrs)= 1.21 1.54 1.250  
 01516> RUNOFF VOLUME (mm)= 40.94 21.31 31.126  
 01517> TOTAL RAINFALL (mm)= 42.51 42.51 42.514  
 01518> RUNOFF COEFFICIENT = .96 .50 .732  
 01519>

01520> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 01521> CN\* = 81.0 Ia = Dep. Storage (Above)  
 01522> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 01523> THAN THE STORAGE COEFFICIENT.  
 01524> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 01525>  
 01526>

01527> 001:0018-----  
 01528> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01529> (ha) (cms) (hrs) (mm) (cms)  
 01530> ID1 03:HIP03 17.00 1.504 1.17 31.13 .000  
 01531> +ID2 04:HIP04 18.10 1.364 1.25 31.13 .000  
 01532> SUM 05:HIP05 35.10 2.800 1.17 31.13 .000  
 01533>

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01534> 001:0019-----  
 01535> \*SUB-AREA No.4  
 01536> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 01537> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 01538> U.H. Tp(hrs)= .170

	Unit Hyd Qpeak (cms)
01539>	.899

01540> PEAK FLOW (cms)= .260 (i)  
 01541> TIME TO PEAK (hrs)= 1.167  
 01542> RUNOFF VOLUME (mm)= 17.326  
 01543> TOTAL RAINFALL (mm)= 42.514  
 01544> RUNOFF COEFFICIENT = .408  
 01545>

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01546> 001:0020-----  
 01547> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 01548> (ha) (cms) (hrs) (mm) (cms)  
 01549> ID1 02:HIP02 28.46 1.615 1.21 33.52 .000  
 01550> +ID2 05:HIP05 35.10 2.800 1.17 31.13 .000  
 01551> +ID3 06:Pond-B 4.00 .260 1.17 17.32 .000  
 01552> SUM 07:HIP06 67.56 4.661 1.17 31.32 .000  
 01553>

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

01554> 001:0021-----  
 01555> | ROUTE RESERVOIR Requested routing time step = 1.0 min.  
 01556> | IN>07: (HIP06 ) |  
 01557> | OUT<08: (HIP-PO) |

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
01578>	.000	.0000E+00	.724	.2210E+01
01579>	.048	.5740E-01	.937	.2501E+01
01580>	.054	.2434E+00	1.262	.2798E+01
01581>	.059	.5834E+00	1.404	.3101E+01
01582>	.062	.8400E+00	1.532	.3410E+01
01583>	.064	.1102E+01	1.650	.3724E+01
01584>	.147	.1370E+01	2.409	.4044E+01
01585>	.280	.1644E+01	3.689	.4370E+01
01586>	.472	.1924E+01	.000	.0000E+00

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
01587>	67.56	4.661	1.167	31.317
01588>	67.56	.288	3.597	31.317

01589> PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.182  
 01590> TIME SHIFT OF PEAK FLOW (min) = 145.83  
 01591> MAXIMUM STORAGE USED (ha.m.) = 1656E+01  
 01592>

01593> 001:0022-----  
 01594> \* SUB-AREA No.5

01595> 01596> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00  
 01597> | 09:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 01598> U.H. Tp(hrs)= .370

	Unit Hyd Qpeak (cms)
01600>	.702

01601> PEAK FLOW (cms)= .187 (i)  
 01602> TIME TO PEAK (hrs)= 1.458  
 01603> RUNOFF VOLUME (mm)= 12.131  
 01604> TOTAL RAINFALL (mm)= 42.514  
 01605> RUNOFF COEFFICIENT = .285  
 01606>

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

01607> 001:0023-----  
 01608> \* SUB-AREA No.6

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01621> *SUB-AREA No. 6
01622> *
01623> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00
01624> | 10:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01625> | U.H. Tp(hrs)= .804
01626>
01627> Unit Hyd Qpeak (cms)= .252
01628>
01629>
01630> PEAK FLOW (cms)= .086 (i)
01631> TIME TO PEAK (hrs)= 2.042
01632> RUNOFF VOLUME (mm)= 12.131
01633> TOTAL RAINFALL (mm)= 42.514
01634> RUNOFF COEFFICIENT = .285
01635>
01636> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01637>
01638>
01639> 001:0024-----
01640>
01641> | ADD HYD (Interi) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01642> | (ha) (cms) (hrs) (mm) (cms)
01643> | ID1 08:HIP-PO 67.56 .288 3.60 31.32 .000
01644> | +ID2 09:A2 6.80 .187 1.46 12.13 .000
01645> | +ID3 10:A3 5.30 .086 2.04 12.13 .000
01646> | SUM 01:Interi 79.66 .359 3.08 28.40 .000
01647>
01648> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01649>
01650>
01651> 001:0025-----
01652> *****
01653> *****
01654> *****
01655> *****
01656> *****
01657> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHWHY-1\
01658> | TERO = .00 hrs on 0
01659> | METOUT= 2 (output = METRIC)
01660> | NRUN = 001
01661> | NSTORM= 0
01662>
01663>
01664> 001:0002-----
01665>
01666> | CHICAGO STORM | IDF curve parameters: A=1174.184
01667> | Ptotal= 49.50 mm | B= 6.014
01668> | C= .816
01669>
01670> used in: INTENSITY = A / (t + B)^C
01671>
01672> Duration of storm = 3.00 hrs
01673> Storm time step = 10.00 min
01674> Time to peak ratio = .33
01675>
01676>
01677> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01678> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01679> .17 4.248 | 1.00 122.142 | 1.83 7.733 | 2.67 4.049
01680> .33 5.290 | 1.17 37.285 | 2.00 6.502 | 2.83 3.714
01681> .50 7.108 | 1.33 18.954 | 2.17 5.625 | 3.00 3.434
01682> .67 11.130 | 1.50 12.700 | 2.33 4.969 |
01683> .83 28.100 | 1.67 9.588 | 2.50 4.458 |
01684>
01685>
01686> 001:0003-----
01687>
01688> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHWHY-1\ORGA.VAL
01689> | ICASEdv = 1 (read and print data)
01690> | FileTitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
01691> | Horton's infiltration equation parameters:
01692> | [P= 50.00 mm/hr] [Pc= 7.50 mm/hr] [DCAV= 2.00 /hr] [F= .00 mm]
01693> | Parameters for Pervious surfaces in STANDHYD:
01694> | [IAPER= 4.67 mm] [LGP=40.00 m] [MNP= .250]
01695> | Parameters for IMPervious surfaces in STANDHYD:
01696> | [IADIMP= 1.57 mm] [CLi= 1.50] [MWI= .035]
01697> | Parameters used in NASHYD:
01698> | [Ia= 4.67 mm] [N= 3.00]
01699>
01700> 001:0004-----
01701> *****
01702> *****
01703> *****
01704> * SUB-AREA No.1
01705>
01706> | CALIB STANDHYD | Area (ha)= 2.07
01707> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01708>
01709>
01710> IMPERVIOUS PERVIOUS (i)
01711> Surface Area (ha)= 1.74 .23
01712> Dep. Storage (mm)= 1.57 4.67
01713> Average Slope (ft)= .52 1.00
01714> Length (m)= 204.72 20.00
01715> Mannings n = .030 .250
01716>
01717> Max. eff. Inten. (mm/hr)= 122.14 34.69
01718> over (min)= 7.50 15.00
01719> Storage Coeff. (min)= 7.28 (ii) 16.04 (ii)
01720> Unit Hyd. Tpeak (min)= 7.50 15.00
01721> Unit Hyd. peak (cms)= .15 .07
01722>
01723> PEAK FLOW (cms)= .43 .02
01724> TIME TO PEAK (hrs)= 1.04 1.21
01725> RUNOFF VOLUME (mm)= 47.93 19.25
01726> TOTAL RAINFALL (mm)= 49.50 49.50
01727> RUNOFF COEFFICIENT = .97 .39
01728>
01729> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
01730> CN* = 81.0 Ia = Dep. Storage (Above)
01731> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01732> THAN THE STORAGE COEFFICIENT.
01733> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01734>
01735> 001:0005-----
01736> *****
01737> *****
01738> *****
01739> * SUB-AREA No.2
01740>
01741> | CALIB STANDHYD | Area (ha)= 1.54
01742> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
01743>
01744>
01745> IMPERVIOUS PERVIOUS (i)
01746> Surface Area (ha)= 1.42 .12
01747> Dep. Storage (mm)= 1.57 4.67
01748> Average Slope (ft)= .50 1.00
01749> Length (m)= 244.34 5.00
01750> Mannings n = .030 .030
01751>
01752> Max. eff. Inten. (mm/hr)= 122.14 42.32
01753> over (min)= 7.50 10.00
01754> Storage Coeff. (min)= 8.20 (ii) 9.18 (ii)
01755> Unit Hyd. Tpeak (min)= 7.50 10.00
01756> Unit Hyd. peak (cms)= .14 .12
01757>
01758> PEAK FLOW (cms)= .33 .01
01759>
01760> *TOTALS*
01761> .341 (iii)

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01756> TIME TO PEAK (hrs)= 1.04 1.13 1.042
01757> RUNOFF VOLUME (mm)= 47.93 19.25 45.640
01758> TOTAL RAINFALL (mm)= 49.50 49.50 45.505
01759> RUNOFF COEFFICIENT = .97 .39 .922
01760>
01761> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
01762> CN* = 81.0 Ia = Dep. Storage (Above)
01763> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01764> THAN THE STORAGE COEFFICIENT.
01765> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01766>
01767>
01768> 001:0006-----
01769> *****
01770> *****
01771> * SUB-AREA No.3
01772>
01773> | CALIB STANDHYD | Area (ha)= 1.40
01774> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01775>
01776>
01777> IMPERVIOUS PERVIOUS (i)
01778> Surface Area (ha)= 1.36 .04
01779> Dep. Storage (mm)= 1.57 4.67
01780> Average Slope (ft)= .51 1.00
01781> Length (m)= 225.63 5.00
01782> Mannings n = .030 .030
01783>
01784> Max. eff. Inten. (mm/hr)= 122.14 48.18
01785> over (min)= 7.50 7.50
01786> Storage Coeff. (min)= 7.77 (ii) 8.70 (ii)
01787> Unit Hyd. Tpeak (min)= 7.50 7.50
01788> Unit Hyd. peak (cms)= .15 .14
01789>
01790> PEAK FLOW (cms)= .33 .00
01791> TIME TO PEAK (hrs)= 1.04 1.08
01792> RUNOFF VOLUME (mm)= 47.93 19.25
01793> TOTAL RAINFALL (mm)= 49.50 49.50
01794> RUNOFF COEFFICIENT = .97 .39
01795>
01796> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
01797> CN* = 81.0 Ia = Dep. Storage (Above)
01798> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01799> THAN THE STORAGE COEFFICIENT.
01800> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01801>
01802>
01803> 001:0007-----
01804> *****
01805> *****
01806> *****
01807> | ADD HYD (040) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01808> | (ha) (cms) (hrs) (mm) (cms)
01809> | ID1 01:010 2.07 .437 1.04 43.35 .000
01810> | +ID2 02:020 1.54 .341 1.04 45.64 .000
01811> | SUM 04:040 3.61 .778 1.04 44.32 .000
01812>
01813> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01814>
01815> 001:0008-----
01816> *****
01817> *****
01818> *****
01819> | ADD HYD (050) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01820> | (ha) (cms) (hrs) (mm) (cms)
01821> | ID1 03:030 1.40 .329 1.04 47.07 .000
01822> | +ID2 04:040 3.61 .778 1.04 44.32 .000
01823> | SUM 05:050 5.01 1.107 1.04 45.09 .000
01824>
01825> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01826>
01827> 001:0009-----
01828> *****
01829> *****
01830> *****
01831> * SUB-AREA No.4
01832>
01833> | CALIB STANDHYD | Area (ha)= .89
01834> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01835>
01836>
01837> IMPERVIOUS PERVIOUS (i)
01838> Surface Area (ha)= .86 .03
01839> Dep. Storage (mm)= 1.57 4.67
01840> Average Slope (ft)= .53 .70
01841> Length (m)= 164.82 40.00
01842> Mannings n = .030 .250
01843>
01844> Max. eff. Inten. (mm/hr)= 122.14 31.19
01845> over (min)= 5.00 20.00
01846> Storage Coeff. (min)= 5.37 (ii) 20.78 (ii)
01847> Unit Hyd. Tpeak (min)= 5.00 20.00
01848> Unit Hyd. peak (cms)= .21 .06
01849>
01850> PEAK FLOW (cms)= .24 .00
01851> TIME TO PEAK (hrs)= 1.00 1.29
01852> RUNOFF VOLUME (mm)= 47.93 19.25
01853> TOTAL RAINFALL (mm)= 49.50 49.50
01854> RUNOFF COEFFICIENT = .97 .39
01855>
01856> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
01857> CN* = 81.0 Ia = Dep. Storage (Above)
01858> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01859> THAN THE STORAGE COEFFICIENT.
01860> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01861>
01862>
01863> 001:0010-----
01864> *****
01865> *****
01866> *****
01867> * SUB-AREA No.5
01868>
01869> | CALIB STANDHYD | Area (ha)= 2.66
01870> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01871>
01872>
01873> IMPERVIOUS PERVIOUS (i)
01874> Surface Area (ha)= 2.58 .08
01875> Dep. Storage (mm)= 1.57 4.67
01876> Average Slope (ft)= .61 1.50
01877> Length (m)= 207.25 20.00
01878> Mannings n = .030 .250
01879>
01880> Max. eff. Inten. (mm/hr)= 122.14 34.69
01881> over (min)= 7.50 15.00
01882> Storage Coeff. (min)= 7.00 (ii) 14.75 (ii)
01883> Unit Hyd. Tpeak (min)= 7.50 15.00
01884> Unit Hyd. peak (cms)= .16 .08
01885>
01886> PEAK FLOW (cms)= .64 .00
01887> TIME TO PEAK (hrs)= 1.04 1.21
01888> RUNOFF VOLUME (mm)= 47.93 19.25
01889> TOTAL RAINFALL (mm)= 49.50 49.50
01890> RUNOFF COEFFICIENT = .97 .39
01891>
01892> (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
01893> CN* = 81.0 Ia = Dep. Storage (Above)
01894> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01895> THAN THE STORAGE COEFFICIENT.
01896> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01897>
01898>
01899>
01900>

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01891> 001:0011-----
01892>
01893> | ADD HYD (080 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01894> | ID1 06:060 | (ha) (cms) (hrs) (mm) (cms)
01895> | +ID2 07:070 | 2.66 .645 1.04 47.07 .000
01896> | SUM 08:080 | 3.55 .876 1.04 47.07 .000
01897>
01898>
01899>
01900> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01901>
01902>
01903> 001:0012-----
01904>
01905> | ADD HYD (090 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01906> | ID1 05:050 | (ha) (cms) (hrs) (mm) (cms)
01907> | +ID2 08:080 | 5.01 1.107 1.04 45.09 .000
01908> | SUM 09:090 | 3.55 .876 1.04 47.07 .000
01909>
01910>
01911>
01912> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01913>
01914>
01915> 001:0013-----
01916>
01917> ROUTE RESERVOIR Requested routing time step = 1.0 min.
01918> | IN>09: (090 ) |
01919> | OUT<10: (POND ) |
01920>
01921> ===== OUTFLOW STORAGE TABLE =====
01922> OUTFLOW STORAGE OUTFLOW STORAGE
01923> (cms) (ha.m.) (cms) (ha.m.)
01924> .000 .0000E+00 | .593 .6251E+00
01925> .008 .6560E-01 | .654 .6631E+00
01926> .017 .1311E+00 | .797 .7391E+00
01927> .093 .2831E+00 | .950 .8274E+00
01928> .233 .3971E+00 | 1.304 .9157E+00
01929> .337 .4731E+00 | 1.880 .1004E+01
01930> .465 .5491E+00 | 2.577 .1092E+01
01931> .531 .5871E+00 | .000 .0000E+00
01932>
01933> ROUTING RESULTS AREA OPEAK TPEAK R.V.
01934> | INFLW >09: (090 ) | (ha) (cms) (hrs) (mm)
01935> | OUTFLOW<10: (POND ) | 8.56 1.984 1.042 45.914
01936> | PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.440
01937> | TIME SHIFT OF PEAK FLOW (min) = 74.17
01938> | MAXIMUM STORAGE USED (ha.m.) = .3148E+00
01939>
01940>
01941> 001:0014-----
01942>
01943> * Remaining Hawthorne Industrial Park *
01944>
01945> *
01946> * SUB-AREA No.1
01947>
01948> | CALIB STANDHYD | Area (ha)= 19.90
01949> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01950>
01951> IMPERVIOUS PERVIOUS (i)
01952> Surface Area (ha)= 14.13 5.77
01953> Dep. Storage (mm)= 1.57 4.67
01954> Average Slope (ft)= 1.50 1.50
01955> Length (m)= 580.00 100.00
01956> Mannings n = .030 .250
01957>
01958> Max.eff.Inten.(mm/hr)= 93.86 60.56
01959> over (min)= 15.00 30.00
01960> Storage Coeff. (min)= 14.48 (ii) 30.78 (ii)
01961> Unit Hyd. Tpeak (min)= 15.00 30.00
01962> Unit Hyd. peak (cms)= .08 .04
01963>
01964> PEAK FLOW (cms)= 1.70 .55 *TOTALS*
01965> TIME TO PEAK (hrs)= 1.17 1.55 1.983 (iii)
01966> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
01967> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01968> RUNOFF COEFFICIENT = .97 .54 .756
01969>
01970> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01971> CN* = 81.0 Ia = Dep. Storage (Above)
01972> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01973> THAN THE STORAGE COEFFICIENT.
01974> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01975>
01976>
01977> 001:0015-----
01978>
01979> | ADD HYD (HIP02 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01980> | ID1 10:POND | (ha) (cms) (hrs) (mm) (cms)
01981> | +ID2 01:HIP01 | 8.56 .132 2.28 45.91 .000
01982> | SUM 02:HIP02 | 19.90 1.983 1.21 37.43 .000
01983>
01984>
01985>
01986> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01987>
01988>
01989> 001:0016-----
01990>
01991> * SUB-AREA No.2
01992>
01993> | CALIB STANDHYD | Area (ha)= 17.00
01994> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01995>
01996> IMPERVIOUS PERVIOUS (i)
01997> Surface Area (ha)= 12.07 4.93
01998> Dep. Storage (mm)= 1.57 4.67
01999> Average Slope (ft)= 1.50 1.50
02000> Length (m)= 450.00 100.00
02001> Mannings n = .030 .250
02002>
02003> Max.eff.Inten.(mm/hr)= 105.17 63.81
02004> over (min)= 12.50 27.50
02005> Storage Coeff. (min)= 11.60 (ii) 27.56 (ii)
02006> Unit Hyd. Tpeak (min)= 12.50 27.50
02007> Unit Hyd. peak (cms)= .09 .04
02008>
02009> PEAK FLOW (cms)= 1.63 .51 *TOTALS*
02010> TIME TO PEAK (hrs)= 1.13 1.42 1.167
02011> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
02012> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
02013> RUNOFF COEFFICIENT = .97 .54 .756
02014>
02015> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02016> CN* = 81.0 Ia = Dep. Storage (Above)
02017> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02018> THAN THE STORAGE COEFFICIENT.
02019> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02020>
02021>
02022> 001:0017-----
02023>
02024> * SUB-AREA No.3
02025>

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02026> | CALIB STANDHYD | Area (ha)= 18.10
02027> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
02028>
02029> IMPERVIOUS PERVIOUS (i)
02030> Surface Area (ha)= 12.85 5.25
02031> Dep. Storage (mm)= 1.57 4.67
02032> Average Slope (ft)= 1.50 1.50
02033> Length (m)= 600.00 100.00
02034> Mannings n = .030 .250
02035>
02036> Max.eff.Inten.(mm/hr)= 93.86 57.19
02037> over (min)= 15.00 32.50
02038> Storage Coeff. (min)= 15.61 (ii) 32.28 (ii)
02039> Unit Hyd. Tpeak (min)= 15.00 32.50
02040> Unit Hyd. peak (cms)= .07 .03
02041>
02042> PEAK FLOW (cms)= 1.49 .48 *TOTALS*
02043> TIME TO PEAK (hrs)= 1.17 1.50 1.208
02044> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
02045> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
02046> RUNOFF COEFFICIENT = .97 .54 .756
02047>
02048> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02049> CN* = 81.0 Ia = Dep. Storage (Above)
02050> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02051> THAN THE STORAGE COEFFICIENT.
02052> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02053>
02054>
02055> 001:0018-----
02056>
02057> | ADD HYD (HIP05 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
02058> | ID1 03:HIP03 | (ha) (cms) (hrs) (mm) (cms)
02059> | +ID2 04:HIP04 | 17.00 1.865 1.17 37.43 .000
02060> | SUM 05:HIP05 | 18.10 1.723 1.21 37.43 .000
02061>
02062>
02063>
02064> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02065>
02066>
02067> 001:0019-----
02068>
02069> * SUB-AREA No.4
02070>
02071> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
02072> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
02073> | U.H. Tp(hrs)= .170
02074>
02075> Unit Hyd Opeak (cms)= .899
02076>
02077> PEAK FLOW (cms)= .345 (i)
02078> TIME TO PEAK (hrs)= 1.167
02079> RUNOFF VOLUME (mm)= 22.420
02080> TOTAL RAINFALL (mm)= 49.505
02081> RUNOFF COEFFICIENT = .453
02082>
02083> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02084>
02085>
02086> 001:0020-----
02087>
02088> | ADD HYD (HIP06 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
02089> | ID1 02:HIP02 | (ha) (cms) (hrs) (mm) (cms)
02090> | +ID2 05:HIP05 | 28.46 2.044 1.21 39.98 .000
02091> | +ID3 06:Pond-B | 35.10 3.572 1.17 37.43 .000
02092> | SUM 07:HIP06 | 4.00 .345 1.17 22.42 .000
02093>
02094>
02095>
02096> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02097>
02098>
02099> 001:0021-----
02100>
02101> ROUTE RESERVOIR Requested routing time step = 1.0 min.
02102> | IN>07: (HIP06 ) |
02103> | OUT<08: (HIP-PO) |
02104>
02105> ===== OUTFLOW STORAGE TABLE =====
02106> OUTFLOW STORAGE OUTFLOW STORAGE
02107> (cms) (ha.m.) (cms) (ha.m.)
02108> .000 .0000E+00 | .724 .2210E+01
02109> .048 .5740E-01 | .937 .2501E+01
02110> .054 .2434E+00 | 1.262 .2798E+01
02111> .059 .5834E+00 | 1.404 .3101E+01
02112> .062 .8400E+00 | 1.532 .3410E+01
02113> .064 .1102E+01 | 1.650 .3724E+01
02114> .147 .1370E+01 | 2.409 .4044E+01
02115> .280 .1644E+01 | 3.689 .4370E+01
02116> .472 .1924E+01 | .000 .0000E+00
02117>
02118> ROUTING RESULTS AREA OPEAK TPEAK R.V.
02119> | INFLW >07: (HIP06 ) | (ha) (cms) (hrs) (mm)
02120> | OUTFLOW<08: (HIP-PO) | 67.56 5.939 1.167 37.611
02121> | PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.200
02122> | TIME SHIFT OF PEAK FLOW (min) = 131.67
02123> | MAXIMUM STORAGE USED (ha.m.) = .1941E+01
02124>
02125>
02126> 001:0022-----
02127>
02128> * SUB-AREA No. 5
02129>
02130>
02131> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00
02132> | 09:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
02133> | U.H. Tp(hrs)= .370
02134>
02135> Unit Hyd Opeak (cms)= .702
02136>
02137> PEAK FLOW (cms)= .252 (i)
02138> TIME TO PEAK (hrs)= 1.417
02139> RUNOFF VOLUME (mm)= 16.075
02140> TOTAL RAINFALL (mm)= 49.505
02141> RUNOFF COEFFICIENT = .325
02142>
02143> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02144>
02145>
02146> 001:0023-----
02147>
02148> * SUB-AREA No. 6
02149>
02150>
02151> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00
02152> | 10:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
02153> | U.H. Tp(hrs)= .804
02154>
02155> Unit Hyd Opeak (cms)= .252
02156>
02157> PEAK FLOW (cms)= .115 (i)
02158> TIME TO PEAK (hrs)= 2.000
02159> RUNOFF VOLUME (mm)= 16.075
02160> TOTAL RAINFALL (mm)= 49.505

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02161> RUNOFF COEFFICIENT = .325
02162>
02163> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02164>
02165>
02166> 001:0024-----
02167>
02168> | ADD HYD (Interi) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02169> |-----|
02170> | ID1 08:HIP-PO 67.56 .487 3.36 37.61 .000
02171> | +ID2 09:A2 6.80 .252 1.42 16.08 .000
02172> | +ID3 10:A3 5.30 .115 2.00 16.08 .000
02173> |-----|
02174> | SUM 01:Interi 79.66 .589 3.04 34.34 .000
02175>
02176> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02177>
02178>
02179> 001:0025-----
02180> *****
02181> * CALCULATION OF 3HR - 1:25 YEAR STORM EVENT *
02182> *****
02183>
02184> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWMHY-1\
02185> | Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMHY-1\
02186> | TZERO = .00 hrs on 0
02187> | METOUT= 2 (output = METRIC)
02188> | NRUN = 001
02189> | NSTORM= 0
02190>
02191> 001:0002-----
02192>
02193> | CHICAGO STORM | IDF curve parameters: A=1402.884
02194> | Ptotal= 58.23 mm | C= .819
02195> | used in: INTENSITY = A / (t + B)^C
02196> | Duration of storm = 3.00 hrs
02197> | Storm time step = 10.00 min
02198> | Time to peak ratio = .33
02199>
02200>
02201>
02202> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
02203> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
02204> .17 4.934 | 1.00 144.593 | 1.93 9.014 | 2.67 4.702
02205> .33 6.152 | 1.17 43.904 | 2.00 7.571 | 2.83 4.310
02206> .50 8.282 | 1.33 22.224 | 2.17 6.544 | 3.00 3.983
02207> .67 13.006 | 1.50 14.852 | 2.33 5.776 |
02208> .83 33.041 | 1.67 11.192 | 2.50 5.179 |
02209>
02210>
02211> 001:0003-----
02212>
02213> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWMHY-1\ORGA.VAL
02214> | ICRSEdy = 1 (read and print data)
02215> | Filetitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
02216> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
02217> | Horton's infiltration equation parameters:
02218> | [F0= 50.00 mm/hr] [F1= 7.50 mm/hr] [DCAY= 2.00 /hr] [F2= .00 mm]
02219> | Parameters for Pervious surfaces in STANDHYD:
02220> | [IAper= 4.67 mm] [LGP=40.00 mm] [MNP= .250]
02221> | Parameters for IMPERVIOUS surfaces in STANDHYD:
02222> | [IAimp= 1.57 mm] [CLI= 1.50] [MNI= .035]
02223> | Parameters used in NASHYD:
02224> | [Iaw 4.67 mm] [Nw 3.00]
02225>
02226> 001:0004-----
02227> *****
02228> * ORGAWORD FILE *****
02229> *****
02230> *
02231> * SUB-AREA No.1
02232>
02233> | CALIB STANDHYD | Area (ha)= 2.07
02234> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
02235>
02236> IMPERVIOUS PERVIOUS (i)
02237> Surface Area (ha)= 1.74 .33
02238> Dep. Storage (mm)= 1.57 4.67
02239> Average Slope (%)= .52 1.00
02240> Length (m)= 204.72 20.00
02241> Mannings n = .030 .250
02242>
02243> Max.eff.Inten.(mm/hr)= 144.69 47.07
02244> over (min)= 7.50 15.00
02245> Storage Coeff. (min)= 6.81 (ii) 14.56 (ii)
02246> Unit Hyd. Tpeak (min)= 7.50 15.00
02247> Unit Hyd. peak (cms)= .16 .08
02248>
02249> PEAK FLOW (cms)= .52 .03 *TOTALS*
02250> TIME TO PEAK (hrs)= 1.04 1.21 .532 (iii)
02251> RUNOFF VOLUME (mm)= 56.66 25.35 51.647
02252> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02253> RUNOFF COEFFICIENT = .97 .44 .887
02254>
02255> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02256> CN* = 81.0 Ia = Dep. Storage (Above)
02257> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02258> THAN THE STORAGE COEFFICIENT.
02259> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02260>
02261>
02262> 001:0005-----
02263> *
02264> * SUB-AREA No.2
02265>
02266> | CALIB STANDHYD | Area (ha)= 1.54
02267> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
02268>
02269> IMPERVIOUS PERVIOUS (i)
02270> Surface Area (ha)= 1.42 .12
02271> Dep. Storage (mm)= 1.57 4.67
02272> Average Slope (%)= .50 1.00
02273> Length (m)= 244.34 5.00
02274> Mannings n = .030 .030
02275>
02276> Max.eff.Inten.(mm/hr)= 144.69 65.19
02277> over (min)= 7.50 7.50
02278> Storage Coeff. (min)= 7.66 (ii) 8.49 (ii)
02279> Unit Hyd. Tpeak (min)= 7.50 7.50
02280> Unit Hyd. peak (cms)= .15 .14
02281>
02282> PEAK FLOW (cms)= .40 .01 *TOTALS*
02283> TIME TO PEAK (hrs)= 1.04 1.08 1.042
02284> RUNOFF VOLUME (mm)= 56.66 25.35 54.152
02285> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02286> RUNOFF COEFFICIENT = .97 .44 .930
02287>
02288> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02289> CN* = 81.0 Ia = Dep. Storage (Above)
02290> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02291> THAN THE STORAGE COEFFICIENT.
02292> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02293>
02294>
02295> 001:0006-----

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02296> *
02297> * SUB-AREA No.3
02298>
02299> | CALIB STANDHYD | Area (ha)= 1.40
02300> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02301>
02302> IMPERVIOUS PERVIOUS (i)
02303> Surface Area (ha)= 1.36 .04
02304> Dep. Storage (mm)= 1.57 4.67
02305> Average Slope (%)= .51 1.00
02306> Length (m)= 225.63 5.00
02307> Mannings n = .030 .030
02308>
02309> Max.eff.Inten.(mm/hr)= 144.69 65.19
02310> over (min)= 7.50 7.50
02311> Storage Coeff. (min)= 7.26 (ii) 8.09 (ii)
02312> Unit Hyd. Tpeak (min)= 7.50 7.50
02313> Unit Hyd. peak (cms)= .15 .14
02314>
02315> PEAK FLOW (cms)= .40 .00 *TOTALS*
02316> TIME TO PEAK (hrs)= 1.04 1.08 1.042
02317> RUNOFF VOLUME (mm)= 56.66 25.35 55.717
02318> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02319> RUNOFF COEFFICIENT = .97 .44 .957
02320>
02321> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02322> CN* = 81.0 Ia = Dep. Storage (Above)
02323> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02324> THAN THE STORAGE COEFFICIENT.
02325> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02326>
02327>
02328> 001:0007-----
02329>
02330> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02331> |-----|
02332> | ID1 01:010 2.07 .532 1.04 51.65 .000
02333> | +ID2 02:020 1.54 .418 1.04 54.15 .000
02334> |-----|
02335> | SUM 04:040 3.61 .950 1.04 52.72 .000
02336>
02337> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02338>
02339>
02340> 001:0008-----
02341>
02342> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02343> |-----|
02344> | ID1 03:030 1.40 .400 1.04 55.72 .000
02345> | +ID2 04:040 3.61 .950 1.04 52.72 .000
02346> |-----|
02347> | SUM 05:050 5.01 1.350 1.04 53.55 .000
02348>
02349> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02350>
02351>
02352> 001:0009-----
02353> *
02354> * SUB-AREA No.4
02355>
02356> | CALIB STANDHYD | Area (ha)= .89
02357> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02358>
02359> IMPERVIOUS PERVIOUS (i)
02360> Surface Area (ha)= .86 .03
02361> Dep. Storage (mm)= 1.57 4.67
02362> Average Slope (%)= .93 .70
02363> Length (m)= 164.82 40.00
02364> Mannings n = .030 .250
02365>
02366> Max.eff.Inten.(mm/hr)= 144.69 44.12
02367> over (min)= 5.00 17.50
02368> Storage Coeff. (min)= 5.02 (ii) 18.44 (ii)
02369> Unit Hyd. Tpeak (min)= 5.00 17.50
02370> Unit Hyd. peak (cms)= .22 .06
02371>
02372> PEAK FLOW (cms)= .30 .00 *TOTALS*
02373> TIME TO PEAK (hrs)= 1.00 1.25 1.006
02374> RUNOFF VOLUME (mm)= 56.66 25.35 55.717
02375> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02376> RUNOFF COEFFICIENT = .97 .44 .957
02377>
02378> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02379> CN* = 81.0 Ia = Dep. Storage (Above)
02380> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02381> THAN THE STORAGE COEFFICIENT.
02382> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02383>
02384>
02385> 001:0010-----
02386> *
02387> * SUB-AREA No.5
02388>
02389> | CALIB STANDHYD | Area (ha)= 2.66
02390> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02391>
02392> IMPERVIOUS PERVIOUS (i)
02393> Surface Area (ha)= 2.58 .08
02394> Dep. Storage (mm)= 1.57 4.67
02395> Average Slope (%)= .61 1.50
02396> Length (m)= 207.25 20.00
02397> Mannings n = .030 .250
02398>
02399> Max.eff.Inten.(mm/hr)= 144.69 51.33
02400> over (min)= 7.50 12.50
02401> Storage Coeff. (min)= 6.54 (ii) 13.16 (ii)
02402> Unit Hyd. Tpeak (min)= 7.50 12.50
02403> Unit Hyd. peak (cms)= .16 .09
02404>
02405> PEAK FLOW (cms)= .78 .01 *TOTALS*
02406> TIME TO PEAK (hrs)= 1.04 1.17 1.042
02407> RUNOFF VOLUME (mm)= 56.66 25.35 55.717
02408> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02409> RUNOFF COEFFICIENT = .97 .44 .957
02410>
02411> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02412> CN* = 81.0 Ia = Dep. Storage (Above)
02413> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02414> THAN THE STORAGE COEFFICIENT.
02415> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02416>
02417>
02418> 001:0011-----
02419>
02420> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02421> |-----|
02422> | ID1 06:060 2.89 .296 1.00 55.72 .000
02423> | +ID2 07:070 1.66 .783 1.04 55.72 .000
02424> |-----|
02425> | SUM 08:080 3.55 1.060 1.04 55.72 .000
02426>
02427> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02428>
02429>
02430> 001:0012-----

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02431>-----  
 02432> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02433> | | | (ha) (cms) (hrs) (mm) (cms)  
 02434> | ID1 05:050 | 5.01 1.350 1.04 55.55 .000  
 02435> | +ID2 08:090 | 3.55 1.060 1.04 55.72 .000  
 02436>-----  
 02437> | SUM 09:090 | 8.56 2.410 1.04 54.45 .000  
 02438>-----  
 02439>-----  
 02440>-----

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02441>-----  
 02442> 001:0013-----  
 02443> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 02444> | IN-09: (090 ) |  
 02445> | OUT<10: (POND ) |  
 02446>-----  
 02447>-----  
 02448>-----  
 02449>-----  
 02450>-----  
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 02465>-----  
 02466>-----  
 02467>-----

## ROUTING RESULTS

02458>-----  
 02459>-----  
 02460>-----  
 02461>-----  
 02462>-----  
 02463>-----  
 02464>-----  
 02465>-----  
 02466>-----  
 02467>-----

02468> 001:0014-----  
 02469> \*\*\*\*\*  
 02470> \* Remaining Hawthorne Industrial Park  
 02471> \*\*\*\*\*  
 02472> \*  
 02473> \* SUB-AREA No.1  
 02474>-----  
 02475> | CALIB STANDHYD | Area (ha)= 19.90  
 02476> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 02477>-----  
 02478>-----  
 02479>-----  
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 02498>-----  
 02499>-----  
 02500>-----  
 02501>-----  
 02502>-----  
 02503>-----  
 02504>-----  
 02505>-----

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02506> 001:0015-----  
 02507> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02508> | | | (ha) (cms) (hrs) (mm) (cms)  
 02509> | ID1 10:POND | 8.56 .189 2.06 54.45 .000  
 02510> | +ID2 01:HIP01 | 19.90 2.548 1.17 45.44 .000  
 02511>-----  
 02512> | SUM 02:HIP02 | 28.46 2.622 1.17 48.15 .000  
 02513>-----  
 02514>-----  
 02515>-----

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02516> 001:0016-----  
 02517> \*  
 02518> \* SUB-AREA No.2  
 02519>-----  
 02520> | CALIB STANDHYD | Area (ha)= 17.00  
 02521> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 02522>-----  
 02523>-----  
 02524>-----  
 02525>-----  
 02526>-----  
 02527>-----  
 02528>-----  
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 02563>-----  
 02564>-----  
 02565>-----

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02549> 001:0017-----  
 02550> \*  
 02551> \* SUB-AREA No.3  
 02552>-----  
 02553> | CALIB STANDHYD | Area (ha)= 18.10  
 02554> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 02555>-----  
 02556>-----  
 02557>-----  
 02558>-----  
 02559>-----  
 02560>-----  
 02561>-----  
 02562>-----  
 02563>-----  
 02564>-----  
 02565>-----

Storage Coeff. (min)= 14.59 (ii) 29.34 (iii)

02566> Unit Hyd. Tpeak (min)= 15.00 30.00  
 02567> Unit Hyd. peak (cms)= .08 .04  
 02568>-----  
 02569>-----  
 02570>-----  
 02571>-----  
 02572>-----  
 02573>-----  
 02574>-----  
 02575>-----  
 02576>-----  
 02577>-----  
 02578>-----  
 02579>-----  
 02580>-----

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02581>-----  
 02582> 001:0018-----  
 02583>-----  
 02584> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02585> | | | (ha) (cms) (hrs) (mm) (cms)  
 02586> | ID1 03:HIP03 | 17.00 2.398 1.13 45.44 .000  
 02587> | +ID2 04:HIP04 | 18.10 2.180 1.21 45.44 .000  
 02588>-----  
 02589> | SUM 05:HIP05 | 35.10 4.439 1.13 45.44 .000  
 02590>-----  
 02591>-----  
 02592>-----  
 02593>-----

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02594> 001:0019-----  
 02595> \*  
 02596> \* SUB-AREA No.4  
 02597>-----  
 02598> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 02599> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02600> | U.H. Tp(hrs)= .170  
 02601>-----  
 02602>-----  
 02603>-----  
 02604>-----  
 02605>-----  
 02606>-----  
 02607>-----  
 02608>-----  
 02609>-----  
 02610>-----  
 02611>-----

Unit Hyd Opeak (cms)= .899

02604>-----  
 02605>-----  
 02606>-----  
 02607>-----  
 02608>-----  
 02609>-----  
 02610>-----  
 02611>-----

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02612>-----  
 02613> 001:0020-----  
 02614> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02615> | | | (ha) (cms) (hrs) (mm) (cms)  
 02616> | ID1 02:HIP02 | 28.46 2.622 1.17 48.15 .000  
 02617> | +ID2 05:HIP05 | 35.10 4.439 1.13 45.44 .000  
 02618> | +ID3 06:Pond-B | 4.00 .459 1.17 29.15 .000  
 02619>-----  
 02620> | SUM 07:HIP06 | 67.56 7.499 1.17 45.61 .000  
 02621>-----  
 02622>-----  
 02623>-----  
 02624>-----  
 02625>-----

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02626> 001:0021-----  
 02627> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 02628> | IN-07: (HIP06 ) |  
 02629> | OUT<08: (HIP-PO) |  
 02630>-----  
 02631>-----  
 02632>-----  
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 02634>-----  
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 02636>-----  
 02637>-----  
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 02652>-----

## ROUTING RESULTS

02644>-----  
 02645>-----  
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 02650>-----  
 02651>-----  
 02652>-----

02653> 001:0022-----  
 02654> \*  
 02655> \* SUB-AREA No. 5  
 02656>-----  
 02657>-----  
 02658> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00  
 02659> | 09:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02660> | U.H. Tp(hrs)= .370  
 02661>-----  
 02662>-----  
 02663>-----  
 02664>-----  
 02665>-----  
 02666>-----  
 02667>-----  
 02668>-----  
 02669>-----  
 02670>-----  
 02671>-----

Unit Hyd Opeak (cms)= .702

02664>-----  
 02665>-----  
 02666>-----  
 02667>-----  
 02668>-----  
 02669>-----  
 02670>-----  
 02671>-----

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02672> 001:0023-----  
 02673> \*  
 02674> \* SUB-AREA No. 6  
 02675>-----  
 02676>-----  
 02677>-----  
 02678> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00  
 02679> | 10:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02680> | U.H. Tp(hrs)= .804  
 02681>-----  
 02682>-----  
 02683>-----  
 02684>-----  
 02685>-----  
 02686>-----  
 02687>-----  
 02688>-----  
 02689>-----  
 02690>-----  
 02691>-----  
 02692>-----  
 02693>-----  
 02694>-----

Unit Hyd Opeak (cms)= .252

02684>-----  
 02685>-----  
 02686>-----  
 02687>-----  
 02688>-----  
 02689>-----  
 02690>-----  
 02691>-----  
 02692>-----  
 02693>-----  
 02694>-----

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02695> 001:0024-----  
 02696> | ADD HYD (Interi) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 02697> | | | (ha) (cms) (hrs) (mm) (cms)  
 02698> | ID1 08:HIP-PO | 67.56 .773 3.18 45.61 .000  
 02699> | +ID2 09:A2 | 6.80 .343 1.42 21.44 .000  
 02700> | +ID3 10:A3 | 5.30 .155 2.00 21.44 .000  
 02701>-----



02701> SUM 01:Interi 79.66 .939 2.60 41.94 .000

02702> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02703> 02704> 02705> 02706> 001:0025-----

02707> \*\*\*\*\* CALCULATION OF 3HR - 1:50 YEAR STORM EVENT \*\*\*\*\*

02708> \* 02709> \*\*\*\*\*

02710> 02711> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\

02712> Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\

02713> TZERO = .00 hrs on 0

02714> METOUT= 2 (output = METRIC)

02715> NRUM = 001

02716> NSTORM= 0

02717> 02718> 001:0002-----

02719> 02720> | CHICAGO STORM | IDF curve parameters: A=1569.580

02721> | Ptotal= 64.81 mm | B= 6.014

02722> C= .820

02723> used in: INTENSITY =  $A / (t + B)^C$

02724> Duration of storm = 3.00 hrs

02725> Storm time step = 10.00 min

02726> Time to peak ratio = .33

02727> 02728> 02729> 02730> 02731> 02732> 02733> 02734> 02735> 02736>

02737> 02738> 001:0003-----

02739> 02740> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\ORGA.VAL

02741> | ICASEdv = 1 (read and print data)

02742> | Filetitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----

02743> | Parameter values must be entered after column 60 ---->

02744> | Horton's infiltration equation parameters:

02745> | [F= 50.00 mm/hr] [F= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]

02746> | Parameters for Pervious surfaces in STANDHYD:

02747> | [Iaper= 4.67 mm] [LGP=40.00 m] [HWP=.250]

02748> | Parameters for IMPervious surfaces in STANDHYD:

02749> | [Kimp= 1.57 mm] [CLi= 1.50] [HMI=.035]

02750> | Parameters used in WASHYD:

02751> | [Ia= 4.67 mm] [N= 3.00]

02752> 02753> 001:0004-----

02754> \*\*\*\*\* ORGAWORLD FILE \*\*\*\*\*

02755> \* 02756> \*\*\*\*\*

02757> \* 02758> \* SUB-AREA No.1

02759> 02760> | CALIB STANDHYD | Area (ha)= 2.07

02761> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

02762> 02763> 02764> 02765> 02766> 02767> 02768> 02769>

02770> 02771> 02772> 02773> 02774> 02775> 02776> 02777> 02778> 02779> 02780> 02781>

02782> 02783> 02784> 02785> 02786> 02787>

02788> 02789> 001:0005-----

02790> \* 02791> \* SUB-AREA No.2

02792> 02793> | CALIB STANDHYD | Area (ha)= 1.54

02794> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00

02795> 02796> 02797> 02798> 02799> 02800> 02801> 02802>

02803> 02804> 02805> 02806> 02807> 02808> 02809> 02810> 02811> 02812> 02813>

02814> 02815> 02816> 02817> 02818> 02819> 02820>

02821> 02822> 001:0006-----

02823> \* 02824> \* SUB-AREA No.3

02825> 02826> | CALIB STANDHYD | Area (ha)= 1.40

02827> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02828> 02829> 02830> 02831> 02832> 02833> 02834> 02835>

02836> 02837> 02838> 02839> 02840> 02841> 02842> 02843> 02844> 02845> 02846> 02847>

02848> 02849> 02850> 02851> 02852> 02853> 02854> 02855>

02856> 02857> 02858> 02859> 02860> 02861> 02862> 02863> 02864> 02865> 02866> 02867>

02868> 02869> 02870> 02871> 02872> 02873> 02874> 02875>

02876> 02877> 02878> 02879> 001:0009-----

02880> \* 02881> \* SUB-AREA No.4

02882> 02883> | CALIB STANDHYD | Area (ha)= .89

02884> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02885> 02886> 02887> 02888> 02889> 02890> 02891> 02892>

02893> 02894> 02895> 02896> 02897> 02898> 02899> 02900> 02901> 02902> 02903>

02904> 02905> 02906> 02907> 02908> 02909> 02910> 02911>

02912> 02913> 001:0010-----

02914> \* 02915> \* SUB-AREA No.5

02916> 02917> | CALIB STANDHYD | Area (ha)= 2.66

02918> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02919> 02920> 02921> 02922> 02923> 02924> 02925>

02926> 02927> 02928> 02929> 02930> 02931> 02932> 02933> 02934> 02935> 02936> 02937>

02938> 02939> 02940> 02941> 02942> 02943>

02944> 02945> 001:0011-----

02946> 02947> | ADD HYD (080 ) | ID: NHYD

02948> | ID1 06:060 | Area (ha)= .89 QPEAK (cms) TPEAK (hrs) R.V. (mm) DWF (cms)

02949> | +ID2 07:070 | 2.66 .886 1.04 62.25 .000

02950> 02951> 02952> 02953> 02954> 02955>

02956> 02957> 001:0012-----

02958> 02959> | ADD HYD (090 ) | ID: NHYD

02960> | ID1 05:050 | Area (ha)= 5.01 QPEAK (cms) TPEAK (hrs) R.V. (mm) DWF (cms)

02961> | +ID2 08:080 | 3.55 1.197 1.04 62.25 .000

02962> 02963> 02964> 02965> 02966> 02967> 02968> 02969> 001:0013-----

02970> 02971> 02972> 02973> 02974> 02975> 02976> 02977> 02978> 02979>

02980> 02981> 02982> 02983> 02984> 02985> 02986> 02987> 02988> 02989> 02990> 02991>

02992> 02993> 02994> 02995> 02996> 02997> 02998> 02999>

03000> 03001> 03002> 03003> 03004> 03005> 03006> 03007> 03008> 03009> 03010> 03011>

03012> 03013> 03014> 03015> 03016> 03017> 03018> 03019> 03020>

03021> 03022> 03023> 03024> 03025> 03026> 03027> 03028> 03029> 03030>

03031> 03032> 03033> 03034> 03035>

02836> Max.eff.Inten.(mm/hr)= 161.47 78.73

02837> over (min)= 7.50 7.50

02838> Storage Coeff. (min)= 6.95 (ii) 7.72 (ii)

02839> Unit Hyd. Tpeak (min)= 7.50 7.50

02840> Unit Hyd. peak (cms)= .16 .15

02841> 02842> 02843> 02844> 02845> 02846> 02847>

02848> 02849> 02850> 02851> 02852> 02853> 02854> 02855>

02856> 02857> 02858> 02859> 02860> 02861> 02862> 02863> 02864> 02865> 02866> 02867>

02868> 02869> 02870> 02871> 02872> 02873> 02874> 02875>

02876> 02877> 02878> 02879> 001:0009-----

02880> \* 02881> \* SUB-AREA No.4

02882> 02883> | CALIB STANDHYD | Area (ha)= .89

02884> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02885> 02886> 02887> 02888> 02889> 02890> 02891> 02892>

02893> 02894> 02895> 02896> 02897> 02898> 02899> 02900> 02901> 02902> 02903>

02904> 02905> 02906> 02907> 02908> 02909> 02910> 02911>

02912> 02913> 001:0010-----

02914> \* 02915> \* SUB-AREA No.5

02916> 02917> | CALIB STANDHYD | Area (ha)= 2.66

02918> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02919> 02920> 02921> 02922> 02923> 02924> 02925>

02926> 02927> 02928> 02929> 02930> 02931> 02932> 02933> 02934> 02935> 02936> 02937>

02938> 02939> 02940> 02941> 02942> 02943>

02944> 02945> 001:0011-----

02946> 02947> | ADD HYD (080 ) | ID: NHYD

02948> | ID1 06:060 | Area (ha)= .89 QPEAK (cms) TPEAK (hrs) R.V. (mm) DWF (cms)

02949> | +ID2 07:070 | 2.66 .886 1.04 62.25 .000

02950> 02951> 02952> 02953> 02954> 02955>

02956> 02957> 001:0012-----

02958> 02959> | ADD HYD (090 ) | ID: NHYD

02960> | ID1 05:050 | Area (ha)= 5.01 QPEAK (cms) TPEAK (hrs) R.V. (mm) DWF (cms)

02961> | +ID2 08:080 | 3.55 1.197 1.04 62.25 .000

02962> 02963> 02964> 02965> 02966> 02967> 02968> 02969> 001:0013-----

02970> 02971> 02972> 02973> 02974> 02975> 02976> 02977> 02978> 02979>

02980> 02981> 02982> 02983> 02984> 02985> 02986> 02987> 02988> 02989> 02990> 02991>

02992> 02993> 02994> 02995> 02996> 02997> 02998> 02999>

03000> 03001> 03002> 03003> 03004> 03005> 03006> 03007> 03008> 03009> 03010> 03011>

03012> 03013> 03014> 03015> 03016> 03017> 03018> 03019> 03020>

03021> 03022> 03023> 03024> 03025> 03026> 03027> 03028> 03029> 03030>

03031> 03032> 03033> 03034> 03035>

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02971> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
02972> | IN>09: (090 ) |
02973> | OUT<10: (POND ) |
02974> ===== OUTFLOW STORAGE TABLE =====
02975> OUTFLOW STORAGE | OUTFLOW STORAGE
02976> (cms) (ha.m.) | (cms) (ha.m.)
02977> .000 .0000E+00 | .593 .6251E+00
02978> .008 .6560E-01 | .654 .6631E+00
02979> .017 .1311E+00 | .797 .7391E+00
02980> .093 .2631E+00 | 1.304 .9157E+00
02981> .233 .3971E+00 | 1.880 .1004E+01
02982> .337 .4731E+00 | 2.577 .1092E+01
02983> .465 .5491E+00 | .000 .0000E+00
02984> .531 .5871E+00 |
02985>
02986> ROUTING RESULTS AREA QPEAK TPEAK R.V.
02987> (ha) (cms) (hrs) (mm)
02988> INFLOW >09: (090 ) 8.56 2.735 1.042 60.910
02989> OUTFLOW<10: (POND ) 8.56 .233 1.944 60.908
02990>
02991> PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.503
02992> TIME SHIFT OF PEAK FLOW (min) = 54.17
02993> MAXIMUM STORAGE USED (ha.m.) = .3967E+00
02994>
02995> 001:0014-----
02996> *****
02997> * Remaining Hawthorne Industrial Park *
02998> *****
02999> *
03000> * SUB-AREA No.1
03001>
03002> | CALIB STANDHYD | Area (ha) = 19.90
03003> | 01:HIP01 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
03004>
03005> IMPERVIOUS PERVIOUS (i)
03006> Surface Area (ha) = 14.13 5.77
03007> Dep. Storage (mm) = 1.57 4.67
03008> Average Slope (%) = .60 1.50
03009> Length (m) = 580.00 100.00
03010> Mannings n = .030 .250
03011>
03012> Max. eff. Inten. (mm/hr) = 138.95 102.13
03013> over (min) 12.50 25.00
03014> Storage Coeff. (min) = 12.38 (ii) 25.60 (ii)
03015> Unit Hyd. Tpeak (min) = 12.50 25.00
03016> Unit Hyd. peak (cms) = .09 .04
03017>
03018> PEAK FLOW (cms) = 2.46 .95 *TOTALS*
03019> TIME TO PEAK (hrs) = 1.13 1.38
03020> RUNOFF VOLUME (mm) = 63.24 39.90
03021> TOTAL RAINFALL (mm) = 64.81 64.81 64.806
03022> RUNOFF COEFFICIENT = .98 .62 .796
03023>
03024> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03025> CN* = 81.0 Ia = Dep. Storage (Above)
03026> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03027> THAN THE STORAGE COEFFICIENT.
03028> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03029>
03030>
03031> 001:0015-----
03032>
03033> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03034> (ha) (cms) (hrs) (mm) (cms)
03035> ID1 10:POND 8.56 .233 1.94 60.91 .000
03036> +ID2 01:HIP01 19.90 3.001 1.17 51.57 .000
03037>
03038> SUM 02:HIP02 28.46 3.092 1.17 54.37 .000
03039>
03040> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03041>
03042>
03043> 001:0016-----
03044> *
03045> * SUB-AREA No.2
03046>
03047> | CALIB STANDHYD | Area (ha) = 17.00
03048> | 03:HIP03 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
03049>
03050> IMPERVIOUS PERVIOUS (i)
03051> Surface Area (ha) = 12.07 4.93
03052> Dep. Storage (mm) = 1.57 4.67
03053> Average Slope (%) = .65 1.50
03054> Length (m) = 450.00 100.00
03055> Mannings n = .030 .250
03056>
03057> Max. eff. Inten. (mm/hr) = 161.47 109.61
03058> over (min) 10.00 22.50
03059> Storage Coeff. (min) = 10.90 (ii) 22.63 (ii)
03060> Unit Hyd. Tpeak (min) = 10.90 22.50
03061> Unit Hyd. peak (cms) = .11 .05
03062>
03063> PEAK FLOW (cms) = 2.38 .88 *TOTALS*
03064> TIME TO PEAK (hrs) = 1.08 1.33
03065> RUNOFF VOLUME (mm) = 64.24 39.90
03066> TOTAL RAINFALL (mm) = 64.81 64.81 64.806
03067> RUNOFF COEFFICIENT = .98 .62 .796
03068>
03069> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03070> CN* = 81.0 Ia = Dep. Storage (Above)
03071> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03072> THAN THE STORAGE COEFFICIENT.
03073> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03074>
03075>
03076> 001:0017-----
03077> *
03078> * SUB-AREA No.3
03079>
03080> | CALIB STANDHYD | Area (ha) = 18.10
03081> | 04:HIP04 DT= 2.50 | Total Imp (%) = 71.00 Dir. Conn. (%) = 50.00
03082>
03083> IMPERVIOUS PERVIOUS (i)
03084> Surface Area (ha) = 12.85 5.25
03085> Dep. Storage (mm) = 1.57 4.67
03086> Average Slope (%) = .50 1.50
03087> Length (m) = 600.00 100.00
03088> Mannings n = .030 .250
03089>
03090> Max. eff. Inten. (mm/hr) = 138.95 96.02
03091> over (min) 12.50 27.50
03092> Storage Coeff. (min) = 13.34 (ii) 26.90 (ii)
03093> Unit Hyd. Tpeak (min) = 12.50 27.50
03094> Unit Hyd. peak (cms) = .09 .04
03095>
03096> PEAK FLOW (cms) = 2.16 .83 *TOTALS*
03097> TIME TO PEAK (hrs) = 1.13 1.42
03098> RUNOFF VOLUME (mm) = 63.24 39.90
03099> TOTAL RAINFALL (mm) = 64.81 64.81 64.806
03100> RUNOFF COEFFICIENT = .98 .62 .796
03101>
03102> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03103> CN* = 81.0 Ia = Dep. Storage (Above)
03104> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03105> THAN THE STORAGE COEFFICIENT.

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03106> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03107>
03108>
03109> 001:0018-----
03110>
03111> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03112> (ha) (cms) (hrs) (mm) (cms)
03113> ID1 03:HIP03 17.00 2.819 1.13 51.57 .000
03114> +ID2 04:HIP04 18.10 2.596 1.17 51.57 .000
03115>
03116> SUM 05:HIP05 35.10 5.372 1.13 51.57 .000
03117>
03118> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03119>
03120>
03121> 001:0019-----
03122> *
03123> * SUB-AREA No.4
03124>
03125> | DESIGN NASHYD | Area (ha) = 4.00 Curve Number (CN) = 85.00
03126> | 06:Pond-B DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00
03127> U.H. Tp (hrs) = .170
03128>
03129> Unit Hyd Qpeak (cms) = .899
03130>
03131> PEAK FLOW (cms) = .551 (i)
03132> TIME TO PEAK (hrs) = 1.125
03133> RUNOFF VOLUME (mm) = 34.455
03134> TOTAL RAINFALL (mm) = 64.806
03135> RUNOFF COEFFICIENT = .532
03136>
03137> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03138>
03139>
03140> 001:0020-----
03141>
03142> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03143> (ha) (cms) (hrs) (mm) (cms)
03144> ID1 02:HIP02 28.46 3.092 1.17 54.37 .000
03145> +ID2 05:HIP05 35.10 5.372 1.13 51.57 .000
03146> +ID3 06:Pond-B 4.00 .551 1.13 34.45 .000
03147>
03148> SUM 07:HIP06 67.56 8.958 1.13 51.73 .000
03149>
03150> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03151>
03152>
03153> 001:0021-----
03154>
03155> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03156> | IN>07: (HIP06 ) |
03157> | OUT<08: (HIP-PO) |
03158> ===== OUTFLOW STORAGE TABLE =====
03159> OUTFLOW STORAGE | OUTFLOW STORAGE
03160> (cms) (ha.m.) | (cms) (ha.m.)
03161> .000 .0000E+00 | .724 .2210E+01
03162> .048 .5740E-01 | .937 .2501E+01
03163> .054 .2434E+00 | 1.262 .2798E+01
03164> .059 .5834E+00 | 1.404 .3101E+01
03165> .062 .8400E+00 | 1.532 .3410E+01
03166> .064 .1102E+01 | 1.650 .3724E+01
03167> .147 .1370E+01 | 2.409 .4044E+01
03168> .280 .1644E+01 | 3.689 .4370E+01
03169> .472 .1924E+01 | .000 .0000E+00
03170>
03171> ROUTING RESULTS AREA QPEAK TPEAK R.V.
03172> (ha) (cms) (hrs) (mm)
03173> INFLOW >07: (HIP06 ) 67.56 8.958 1.125 51.735
03174> OUTFLOW<08: (HIP-PO) 67.56 .973 3.097 51.735
03175>
03176> PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.864
03177> TIME SHIFT OF PEAK FLOW (min) = 118.33
03178> MAXIMUM STORAGE USED (ha.m.) = .2534E+01
03179>
03180>
03181> 001:0022-----
03182> *
03183> * SUB-AREA No. 5
03184>
03185> | DESIGN NASHYD | Area (ha) = 6.80 Curve Number (CN) = 76.00
03186> | 09:A2 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00
03187> U.H. Tp (hrs) = .370
03188>
03189> Unit Hyd Qpeak (cms) = .702
03190>
03191> PEAK FLOW (cms) = .417 (i)
03192> TIME TO PEAK (hrs) = 1.417
03193> RUNOFF VOLUME (mm) = 25.767
03194> TOTAL RAINFALL (mm) = 64.806
03195> RUNOFF COEFFICIENT = .398
03196>
03197> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03198>
03199>
03200> 001:0023-----
03201> *
03202> * SUB-AREA No. 6
03203>
03204>
03205> | DESIGN NASHYD | Area (ha) = 5.30 Curve Number (CN) = 76.00
03206> | 10:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N) = 3.00
03207> U.H. Tp (hrs) = .804
03208>
03209> Unit Hyd Qpeak (cms) = .252
03210>
03211> PEAK FLOW (cms) = .188 (i)
03212> TIME TO PEAK (hrs) = 2.000
03213> RUNOFF VOLUME (mm) = 25.767
03214> TOTAL RAINFALL (mm) = 64.806
03215> RUNOFF COEFFICIENT = .398
03216>
03217> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03218>
03219>
03220> 001:0024-----
03221>
03222> | ADD HYD (Interi ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03223> (ha) (cms) (hrs) (mm) (cms)
03224> ID1 08:HIP-PO 67.56 .973 3.10 51.73 .000
03225> +ID2 09:A2 6.80 .417 1.42 25.77 .000
03226> +ID3 10:A3 5.30 .188 2.00 25.77 .000
03227>
03228> SUM 01:Interi 79.66 1.191 2.31 47.79 .000
03229>
03230> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03231>
03232>
03233> 001:0025-----
03234> *****
03235> * CALCULATION OF 3HR - 1:100 YEAR STORM EVENT *
03236> *****
03237>
03238> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\
03239> | TZERO = .00 hrs on | Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\
03240>

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03241> METOUT= 2 (output = METRIC)
03242> NRUN = 001
03243> NSTORM= 0
03244>
03245> 001:0002-----
03246>
03247> CHICAGO STORM | IDF curve parameters: A=1735.688
03248> Ptotal= 71.66 mm | B= 6.014
03249> | C= .820
03250> used in: INTENSITY = A / (t + B)^C
03251>
03252> Duration of storm = 3.00 hrs
03253> Storm time step = 10.00 min
03254> Time to peak ratio = .33
03255>
03256> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
03257> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
03258> .17 6.046 | 1.00 178.559 | 1.83 11.059 | 2.67 5.760
03259> .33 7.542 | 1.17 54.049 | 2.00 9.285 | 2.83 5.280
03260> .50 10.159 | 1.33 27.319 | 2.17 8.024 | 3.00 4.879
03261> .67 15.969 | 1.50 18.240 | 2.33 7.080 |
03262> .83 40.655 | 1.67 13.737 | 2.50 6.347 |
03263>
03264>
03265> 001:0003-----
03266>
03267> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\ORGA.VAL
03268> | ICASdy = 1 (read and print data)
03269> | Filetitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
03270> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---
03271> Horton's infiltration equation parameters:
03272> [Fo= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
03273> Parameters for Pervious surfaces in STANDHYD:
03274> [Iapex= 4.67 mm] [LGP=10.00 mm] [DMP= .250]
03275> Parameters for Imperious surfaces in STANDHYD:
03276> [Iaimp= 1.57 mm] [CLI= 1.50] [MWI= .035]
03277> Parameters used in NASHYD:
03278> [Ia= 4.67 mm] [N= 3.00]
03279>
03280> 001:0004-----
03281> *****
03282> * ORGAWORLD FILE *
03283> *****
03284> *
03285> * SUB-AREA No.1
03286>
03287> | CALIB STANDHYD | Area (ha)= 2.07
03288> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
03289>
03290> IMPERVIOUS PERVIOUS (i)
03291> Surface Area (ha)= 1.74 .33
03292> Dep. Storage (mm)= 1.57 4.67
03293> Average Slope (%)= .52 1.00
03294> Length (m)= 204.72 20.00
03295> Mannings n = .030 .250
03296>
03297> Max.eff.Inten.(mm/hr)= 178.56 74.05
03298> over (min) 7.50 12.50
03299> Storage Coeff. (min)= 6.26 (ii) 12.72 (ii)
03300> Unit Hyd. Tpeak (min)= 7.50 12.50
03301> Unit Hyd. peak (cms)= .17 .09
03302>
03303> PEAK FLOW (cms)= .66 .04 *TOTALS*
03304> TIME TO PEAK (hrs)= 1.04 1.17 .685 (iii)
03305> RUNOFF VOLUME (mm)= 70.09 35.46 64.553
03306> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03307> RUNOFF COEFFICIENT = .98 .49 .901
03308>
03309> (i) CN PROCEDURE SELECTED FOR Pervious Losses:
03310> CN* = 81.0 Ia = Dep. Storage (Above)
03311> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03312> THAN THE STORAGE COEFFICIENT.
03313> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03314>
03315>
03316> 001:0005-----
03317> *
03318> * SUB-AREA No.2
03319>
03320> | CALIB STANDHYD | Area (ha)= 1.54
03321> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
03322>
03323> IMPERVIOUS PERVIOUS (i)
03324> Surface Area (ha)= 1.42 .12
03325> Dep. Storage (mm)= 1.57 4.67
03326> Average Slope (%)= .50 1.00
03327> Length (m)= 244.34 5.00
03328> Mannings n = .030 .030
03329>
03330> Max.eff.Inten.(mm/hr)= 178.56 93.23
03331> over (min) 7.50 7.50
03332> Storage Coeff. (min)= 7.04 (ii) 7.76 (ii)
03333> Unit Hyd. Tpeak (min)= 7.50 7.50
03334> Unit Hyd. peak (cms)= .16 .15
03335>
03336> PEAK FLOW (cms)= .51 .02 *TOTALS*
03337> TIME TO PEAK (hrs)= 1.04 1.08 1.042 (iii)
03338> RUNOFF VOLUME (mm)= 70.09 35.46 67.324
03339> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03340> RUNOFF COEFFICIENT = .98 .49 .939
03341>
03342> (i) CN PROCEDURE SELECTED FOR Pervious Losses:
03343> CN* = 81.0 Ia = Dep. Storage (Above)
03344> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03345> THAN THE STORAGE COEFFICIENT.
03346> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03347>
03348>
03349> 001:0006-----
03350> *
03351> * SUB-AREA No.3
03352>
03353> | CALIB STANDHYD | Area (ha)= 1.40
03354> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03355>
03356> IMPERVIOUS PERVIOUS (i)
03357> Surface Area (ha)= 1.36 .04
03358> Dep. Storage (mm)= 1.57 4.67
03359> Average Slope (%)= .51 1.00
03360> Length (m)= 225.63 5.00
03361> Mannings n = .030 .030
03362>
03363> Max.eff.Inten.(mm/hr)= 178.56 93.23
03364> over (min) 7.50 7.50
03365> Storage Coeff. (min)= 6.67 (ii) 7.39 (ii)
03366> Unit Hyd. Tpeak (min)= 7.50 7.50
03367> Unit Hyd. peak (cms)= .16 .15
03368>
03369> PEAK FLOW (cms)= .50 .01 *TOTALS*
03370> TIME TO PEAK (hrs)= 1.04 1.08 1.042 (iii)
03371> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
03372> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03373> RUNOFF COEFFICIENT = .98 .49 .964
03374>
03375> (i) CN PROCEDURE SELECTED FOR Pervious Losses:

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03376> CN* = 81.0 Ia = Dep. Storage (Above)
03377> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03378> THAN THE STORAGE COEFFICIENT.
03379> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03380>
03381>
03382> 001:0007-----
03383>
03384> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03385> | (ha) (cms) (hrs) (mm) (cms)
03386> ID1 01:010 2.07 .685 1.04 64.55 .000
03387> +ID2 02:020 1.54 .534 1.04 67.32 .000
03388>
03389> SUM 04:040 3.61 1.220 1.04 65.74 .000
03390>
03391> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03392>
03393>
03394> 001:0008-----
03395>
03396> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03397> | (ha) (cms) (hrs) (mm) (cms)
03398> ID1 03:030 1.40 .509 1.04 69.06 .000
03399> +ID2 04:040 3.61 1.220 1.04 65.74 .000
03400>
03401> SUM 05:050 5.01 1.729 1.04 66.66 .000
03402>
03403> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03404>
03405>
03406> 001:0009-----
03407> *
03408> * SUB-AREA No.4
03409>
03410> | CALIB STANDHYD | Area (ha)= .89
03411> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03412>
03413> IMPERVIOUS PERVIOUS (i)
03414> Surface Area (ha)= 1.86 .03
03415> Dep. Storage (mm)= 1.57 4.67
03416> Average Slope (%)= .52 1.00
03417> Length (m)= 164.82 40.00
03418> Mannings n = .030 .250
03419>
03420> Max.eff.Inten.(mm/hr)= 178.56 67.61
03421> over (min) 5.00 15.00
03422> Storage Coeff. (min)= 4.62 (ii) 15.92 (ii)
03423> Unit Hyd. Tpeak (min)= 5.00 15.00
03424> Unit Hyd. peak (cms)= .24 .07
03425>
03426> PEAK FLOW (cms)= .37 .00 *TOTALS*
03427> TIME TO PEAK (hrs)= 1.00 1.21 1.000 (iii)
03428> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
03429> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03430> RUNOFF COEFFICIENT = .98 .49 .964
03431>
03432> (i) CN PROCEDURE SELECTED FOR Pervious Losses:
03433> CN* = 81.0 Ia = Dep. Storage (Above)
03434> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03435> THAN THE STORAGE COEFFICIENT.
03436> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03437>
03438>
03439> 001:0010-----
03440> *
03441> * SUB-AREA No.5
03442>
03443> | CALIB STANDHYD | Area (ha)= 2.66
03444> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03445>
03446> IMPERVIOUS PERVIOUS (i)
03447> Surface Area (ha)= 2.58 .08
03448> Dep. Storage (mm)= 1.57 4.67
03449> Average Slope (%)= .61 1.50
03450> Length (m)= 207.25 20.00
03451> Mannings n = .030 .250
03452>
03453> Max.eff.Inten.(mm/hr)= 178.56 74.05
03454> over (min) 5.00 12.50
03455> Storage Coeff. (min)= 6.01 (ii) 11.73 (ii)
03456> Unit Hyd. Tpeak (min)= 5.00 12.50
03457> Unit Hyd. peak (cms)= .20 .09
03458>
03459> PEAK FLOW (cms)= 1.03 .01 *TOTALS*
03460> TIME TO PEAK (hrs)= 1.00 1.17 1.034 (iii)
03461> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
03462> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03463> RUNOFF COEFFICIENT = .98 .49 .964
03464>
03465> (i) CN PROCEDURE SELECTED FOR Pervious Losses:
03466> CN* = 81.0 Ia = Dep. Storage (Above)
03467> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03468> THAN THE STORAGE COEFFICIENT.
03469> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03470>
03471>
03472> 001:0011-----
03473>
03474> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03475> | (ha) (cms) (hrs) (mm) (cms)
03476> ID1 06:060 .89 .374 1.00 69.06 .000
03477> +ID2 07:070 2.66 1.034 1.00 69.06 .000
03478>
03479> SUM 08:080 3.55 1.408 1.00 69.06 .000
03480>
03481> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03482>
03483>
03484> 001:0012-----
03485>
03486> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03487> | (ha) (cms) (hrs) (mm) (cms)
03488> ID1 05:050 5.01 1.729 1.04 66.66 .000
03489> +ID2 08:080 3.55 1.408 1.00 69.06 .000
03490>
03491> SUM 09:090 8.56 3.067 1.04 67.66 .000
03492>
03493> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03494>
03495>
03496> 001:0013-----
03497>
03498> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03499> | IN>09: (090 ) |
03500> | OUT<10: (POND ) |
03501>
03502> ===== OUTFLOW STORAGE TABLE =====
03503> OUTFLOW STORAGE | OUTFLOW STORAGE
03504> (cms) (ha.m.) | (cms) (ha.m.)
03505> .000 .0000E+00 | .593 .6251E+00
03506> .008 .6560E-01 | .654 .6631E+00
03507> .017 .1311E+00 | .797 .7391E+00
03508> .093 .2831E+00 | .950 .8274E+00
03509> .233 .3971E+00 | 1.304 .9157E+00
03510> .337 .4731E+00 | 1.880 .1004E+01
03511> .465 .5491E+00 | 2.577 .1092E+01
03512> .531 .5871E+00 | .000 .0000E+00

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03511> ROUTING RESULTS
03512> AREA QPEAK TPEAK R.V.
03513> (ha) (cms) (hrs) (mm)
03514> INFLOW >09: (090 ) 8.56 3.067 1.042 67.655
03515> OUTFLOW<10: (POND ) 8.56 .283 1.861 67.653
03516>
03517> PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.214
03518> TIME SHIFT OF PEAK FLOW (min) = 49.17
03519> MAXIMUM STORAGE USED (ha.m.) = .4333E+00
03520>
03521>
03522> 001:0014-----
03523> *****
03524> * Remaining Hawthorne Industrial Park *
03525> *****
03526> *
03527> * SUB-AREA No.1
03528>
03529> | CALIB STANDHYD | Area (ha)= 19.90
03530> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03531>
03532> IMPERVIOUS PERVIOUS (i)
03533> Surface Area (ha)= 14.13 5.77
03534> Dep. Storage (mm)= 1.57 4.67
03535> Average Slope (%)= .60 1.50
03536> Length (m)= 580.00 100.00
03537> Mannings n = .030 .250
03538>
03539> Max. eff. Inten. (mm/hr)= 153.66 117.89
03540> over (min) 12.50 25.00
03541> Storage Coeff. (min)= 11.89 (ii) 24.37 (ii)
03542> Unit Hyd. Tpeak (min)= 12.50 25.00
03543> Unit Hyd. peak (cms)= .09 .05
03544>
03545> PEAK FLOW (cms)= 2.77 1.13 *TOTALS*
03546> TIME TO PEAK (hrs)= 1.13 1.38 1.167
03547> RUNOFF VOLUME (mm)= 70.09 45.94 58.015
03548> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03549> RUNOFF COEFFICIENT = .98 .64 .810
03550>
03551> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03552> CN* = 81.0 Ia = Dep. Storage (Above)
03553> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03554> THAN THE STORAGE COEFFICIENT.
03555> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03556>
03557>
03558> 001:0015-----
03559>
03560> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03561> (ha) (cms) (hrs) (mm) (cms)
03562> ID1 10:POND 8.56 .283 1.86 67.65 .000
03563> +ID2 01:HIP01 19.90 3.419 1.17 58.02 .000
03564>
03565> SUM 02:HIP02 28.46 3.554 1.17 60.91 .000
03566>
03567> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03568>
03569>
03570> 001:0016-----
03571> *
03572> * SUB-AREA No.2
03573>
03574> | CALIB STANDHYD | Area (ha)= 17.00
03575> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03576>
03577> IMPERVIOUS PERVIOUS (i)
03578> Surface Area (ha)= 12.07 4.93
03579> Dep. Storage (mm)= 1.57 4.67
03580> Average Slope (%)= .65 1.50
03581> Length (m)= 450.00 100.00
03582> Mannings n = .030 .250
03583>
03584> Max. eff. Inten. (mm/hr)= 178.56 126.60
03585> over (min) 10.00 22.50
03586> Storage Coeff. (min)= 9.39 (ii) 21.52 (ii)
03587> Unit Hyd. Tpeak (min)= 10.00 22.50
03588> Unit Hyd. peak (cms)= .12 .05
03589>
03590> PEAK FLOW (cms)= 2.68 1.05 3.203 (iii)
03591> TIME TO PEAK (hrs)= 1.08 1.33 1.125
03592> RUNOFF VOLUME (mm)= 70.09 45.94 58.015
03593> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03594> RUNOFF COEFFICIENT = .98 .64 .810
03595>
03596> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03597> CN* = 81.0 Ia = Dep. Storage (Above)
03598> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03599> THAN THE STORAGE COEFFICIENT.
03600> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03601>
03602>
03603> 001:0017-----
03604> *
03605> * SUB-AREA No.3
03606>
03607> | CALIB STANDHYD | Area (ha)= 18.10
03608> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03609>
03610> IMPERVIOUS PERVIOUS (i)
03611> Surface Area (ha)= 12.85 5.25
03612> Dep. Storage (mm)= 1.57 4.67
03613> Average Slope (%)= .50 1.50
03614> Length (m)= 600.00 100.00
03615> Mannings n = .030 .250
03616>
03617> Max. eff. Inten. (mm/hr)= 153.66 117.89
03618> over (min) 12.50 25.00
03619> Storage Coeff. (min)= 12.92 (ii) 25.30 (ii)
03620> Unit Hyd. Tpeak (min)= 12.50 25.00
03621> Unit Hyd. peak (cms)= .09 .04
03622>
03623> PEAK FLOW (cms)= 2.43 1.01 3.031 (iii)
03624> TIME TO PEAK (hrs)= 1.13 1.38 1.167
03625> RUNOFF VOLUME (mm)= 70.09 45.94 58.015
03626> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03627> RUNOFF COEFFICIENT = .98 .64 .810
03628>
03629> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03630> CN* = 81.0 Ia = Dep. Storage (Above)
03631> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03632> THAN THE STORAGE COEFFICIENT.
03633> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03634>
03635>
03636> 001:0018-----
03637>
03638> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03639> (ha) (cms) (hrs) (mm) (cms)
03640> ID1 03:HIP03 17.00 3.203 1.13 58.02 .000
03641> +ID2 04:HIP04 18.10 3.031 1.17 58.02 .000
03642>
03643> SUM 05:HIP05 35.10 6.178 1.13 58.02 .000
03644>
03645> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03646>
03647>
03648> 001:0019-----
03649>
03650> * SUB-AREA No.4
03651>
03652> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
03653> | 06:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03654> | U.H. Tp(hrs)= .170
03655>
03656> Unit Hyd Qpeak (cms)= .899
03657>
03658> PEAK FLOW (cms)= .649 (i)
03659> TIME TO PEAK (hrs)= 1.125
03660> RUNOFF VOLUME (mm)= 40.139
03661> TOTAL RAINFALL (mm)= 71.665
03662> RUNOFF COEFFICIENT = .560
03663>
03664> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03665>
03666>
03667> 001:0020-----
03668>
03669> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03670> (ha) (cms) (hrs) (mm) (cms)
03671> ID1 02:HIP02 28.46 3.554 1.17 60.91 .000
03672> +ID2 05:HIP05 35.10 6.178 1.13 58.02 .000
03673> +ID3 06:Pond-B 4.00 .649 1.13 40.14 .000
03674>
03675> SUM 07:HIP06 67.56 10.299 1.13 58.18 .000
03676>
03677> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03678>
03679>
03680> 001:0021-----
03681>
03682> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03683> | IN>07: (HIP06 ) |
03684> | OUT<08: (HIP-PO) |
03685>
03686> OUTFLOW STORAGE TABLE
03687> (cms) (ha.m.) (cms) (ha.m.)
03688> .000 .000E+00 .724 .2210E+01
03689> .048 .5740E+01 .937 .2501E+01
03690> .054 .2434E+00 1.262 .2798E+01
03691> .059 .5834E+00 1.404 .3101E+01
03692> .062 .8400E+00 1.532 .3410E+01
03693> .064 .1102E+01 1.650 .3724E+01
03694> .147 .1370E+01 2.409 .4044E+01
03695> .280 .1644E+01 3.689 .4370E+01
03696> .472 .1924E+01 .000 .0000E+00
03697>
03698> ROUTING RESULTS AREA QPEAK TPEAK R.V.
03699> (ha) (cms) (hrs) (mm)
03700> INFLOW >07: (HIP06 ) 67.56 10.299 1.125 58.176
03701> OUTFLOW<08: (HIP-PO) 67.56 1.246 2.958 58.176
03702>
03703> PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.102
03704> TIME SHIFT OF PEAK FLOW (min) = 110.00
03705> MAXIMUM STORAGE USED (ha.m.) = .2784E+01
03706>
03707> 001:0022-----
03708> *
03709> * SUB-AREA No. 5
03710>
03711>
03712> | DESIGN NASHYD | Area (ha)= 6.80 Curve Number (CN)=76.00
03713> | 09:A2 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03714> | U.H. Tp(hrs)= .370
03715>
03716> Unit Hyd Qpeak (cms)= .702
03717>
03718> PEAK FLOW (cms)= .497 (i)
03719> TIME TO PEAK (hrs)= 1.417
03720> RUNOFF VOLUME (mm)= 30.490
03721> TOTAL RAINFALL (mm)= 71.665
03722> RUNOFF COEFFICIENT = .425
03723>
03724> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03725>
03726>
03727> 001:0023-----
03728> *
03729> * SUB-AREA No. 6
03730>
03731>
03732> | DESIGN NASHYD | Area (ha)= 5.30 Curve Number (CN)=76.00
03733> | 10:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03734> | U.H. Tp(hrs)= .804
03735>
03736> Unit Hyd Qpeak (cms)= .252
03737>
03738> PEAK FLOW (cms)= .223 (i)
03739> TIME TO PEAK (hrs)= 1.958
03740> RUNOFF VOLUME (mm)= 30.490
03741> TOTAL RAINFALL (mm)= 71.665
03742> RUNOFF COEFFICIENT = .425
03743>
03744> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03745>
03746>
03747> 001:0024-----
03748>
03749> | ADD HYD (Interi) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03750> (ha) (cms) (hrs) (mm) (cms)
03751> ID1 08:HIP-PO 67.56 1.246 2.96 58.18 .000
03752> +ID2 09:A2 6.80 .497 1.42 30.49 .000
03753> +ID3 10:A3 5.30 .223 1.96 30.49 .000
03754>
03755> SUM 01:Interi 79.66 1.531 2.39 53.97 .000
03756>
03757> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03758>
03759>
03760> 001:0025-----
03761> FINISH
03762>
03763> *****
03764> WARNINGS / ERRORS / NOTES
03765>
03766> Simulation ended on 2009-05-15 at 08:57:05
03767>
03768>
03769>

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## **A P P E N D I X 'H'**

### **SWMHYMO INPUT AND OUTPUT FILES (Post-Development Controlled Phase 2 Conditions)**

```

00001> 2 Metric units
00002> *****
00003> # Project Name : Hawthorne Industrial Park Project Number: [20983] *
00004> # Date : January, 2009
00005> # Revised : WJA *
00006> # Developed by : Mark Buchanan, E.I.T.
00007> # Reviewed by : Guy Forget, P.Eng.
00008> # Company : J.L. Richards & Associates Limited
00009> # License # : 4418403
00010> *****
00011>
00012> #*****
00013> #*****
00014> # FILENAME: V:\20983.DU\ENG\SWMHYMO\20983PST.DAT *
00015> # FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00016> # OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00017> #*****
00018>
00019>
00020> # SWMHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE
00021> # PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00022> #*****
00023>
00024>
00025> # HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00026> # FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00027> #*****
00028>
00029> #*****
00030> # POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00031> #*****
00032>
00033>
00034> # CALCULATION OF 4 HR 25 MM STORM EVENT *
00035> #*****
00036>
00037> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00038> # [ ] <- storm filename, one per line for NSTORM time
00039> READ STORM STORM_FILENAME=[4HR25-15.STM]
00040> #-----
00041> # DEFAULT VALUES ICASEdef=[1], read and print values
00042> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00043> #-----
00044>
00045>
00046> # ORGAWORLD FILE *
00047> #*****
00048>
00049> # SUB-AREA No.1
00050>
00051> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00052> XIMP=[0.64], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00053> SCS curve number CN=[81],
00054> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00055> LGP=[20] (m), MNP=[0.25 ], SCP=[0.0] (m)
00056> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00057> LGI=[204.72] (m), MNI=[0.03 ], SCI=[0.0]
00058> RAINFALL=[ , , , ] (mm/hr), END=-1
00059> #-----
00060>
00061> # SUB-AREA No.2
00062>
00063> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00064> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00065> SCS curve number CN=[81],
00066> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00067> LGP=[5] (m), MNP=[0.03 ], SCP=[0.0] (min),
00068> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00069> LGI=[244.34] (m), MNI=[0.03 ], SCI=[0.0]
00070> RAINFALL=[ , , , ] (mm/hr), END=-1
00071> #-----
00072>
00073> # SUB-AREA No.3
00074>
00075> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[ 1.4 ] (ha),
00076> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00077> SCS curve number CN=[81],
00078> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00079> LGP=[5] (m), MNP=[0.03 ], SCP=[0.0] (min),
00080> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51 ] (%),
00081> LGI=[225.63 ] (m), MNI=[0.03 ], SCI=[0.0]
00082> RAINFALL=[ , , , ] (mm/hr), END=-1
00083> #-----
00084> ADD HYD IDsum=[4], NHYD= "040", IDs to add=[1+2]
00085> #-----
00086> ADD HYD IDsum=[5], NHYD= "050", IDs to add=[3+4]
00087> #-----
00088>
00089> # SUB-AREA No.4
00090>
00091> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00092> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00093> SCS curve number CN=[81],
00094> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00095> LGP=[40] (m), MNP=[0.25 ], SCP=[0.0] (min)
00096> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00097> LGI=[164.82] (m), MNI=[0.03 ], SCI=[0.0]
00098> RAINFALL=[ , , , ] (mm/hr), END=-1
00099> #-----
00100>
00101> # SUB-AREA No.5
00102>
00103> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[ 2.66 ] (ha),
00104> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00105> SCS curve number CN=[81],
00106> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00107> LGP=[20.0] (m), MNP=[0.25 ], SCP=[0.0] (m)
00108> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00109> LGI=[207.25] (m), MNI=[0.03 ], SCI=[0.0]
00110> RAINFALL=[ , , , ] (mm/hr), END=-1
00111> #-----
00112> ADD HYD IDsum=[8], NHYD= "080", IDs to add=[6+7]
00113> #-----
00114> ADD HYD IDsum=[9], NHYD= "090", IDs to add=[5+8]
00115> #-----
00116>
00117> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00118> RDT=[1.0] (min),
00119>
00120> TABLE of ( OUTFLOW-STORAGE ) values
00121> (cms) - (ha-m)
00122> [ 0.000, 0.0000 ]
00123> [ 0.008, 0.0656 ]
00124> [ 0.017, 0.1311 ]
00125> [ 0.093, 0.2831 ]
00126> [ 0.233, 0.3971 ]
00127> [ 0.337, 0.4731 ]
00128> [ 0.465, 0.5491 ]
00129> [ 0.531, 0.5871 ]
00130> [ 0.593, 0.6251 ]
00131> [ 0.654, 0.6631 ]
00132> [ 0.797, 0.7391 ]
00133> [ 0.950, 0.8274 ]
00134> [ 1.304, 0.9157 ]
00135> [ 1.880, 1.0040 ]
00136> [ 2.577, 1.0923 ]

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```

00136> [ -1 , -1 ] (max twenty pts)
00137>
00138> *****
00139> # Remaining Hawthorne Industrial Park *
00140> *****
00141>
00142> # SUB-AREA No.1
00143>
00144> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00145> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00146> SCS curve number CN=[81],
00147> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00148> LGP=[100.0] (m), MNP=[0.25 ], SCP=[0.0] (m)
00149> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00150> LGI=[580] (m), MNI=[0.03 ], SCI=[0.0] (min)
00151> RAINFALL=[ , , , ] (mm/hr), END=-1
00152> #-----
00153> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00154> #-----
00155>
00156> # SUB-AREA No.2
00157>
00158> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00159> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00160> SCS curve number CN=[81],
00161> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00162> LGP=[100.0] (m), MNP=[0.25 ], SCP=[0.0] (m)
00163> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00164> LGI=[450] (m), MNI=[0.03 ], SCI=[0.0] (min)
00165> RAINFALL=[ , , , ] (mm/hr), END=-1
00166> #-----
00167>
00168> # SUB-AREA No.3
00169>
00170> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00171> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00172> SCS curve number CN=[81],
00173> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00174> LGP=[100.0] (m), MNP=[0.25 ], SCP=[0.0] (m)
00175> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00176> LGI=[600] (m), MNI=[0.03 ], SCI=[0.0] (min)
00177> RAINFALL=[ , , , ] (mm/hr), END=-1
00178> #-----
00179> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00180> #-----
00181> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00182> #-----
00183>
00184> # SUB-AREA No.4
00185>
00186> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00187> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00188> SCS curve number CN=[81],
00189> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00190> LGP=[100.0] (m), MNP=[0.25 ], SCP=[0.0] (m)
00191> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.7] (%),
00192> LGI=[210] (m), MNI=[0.03 ], SCI=[0.0] (min)
00193> RAINFALL=[ , , , ] (mm/hr), END=-1
00194> #-----
00195>
00196> # SUB-AREA No.5
00197>
00198> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00199> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00200> RAINFALL=[ , , , ] (mm/hr), END=-1
00201> #-----
00202>
00203>
00204> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00205> #-----
00206>
00207> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
00208> RDT=[1.0] (min),
00209>
00210> TABLE of ( OUTFLOW-STORAGE ) values
00211> (cms) - (ha-m)
00212> [ 0.048, 0.0574 ]
00213> [ 0.054, 0.2434 ]
00214> [ 0.059, 0.5834 ]
00215> [ 0.062, 0.8400 ]
00216> [ 0.064, 1.1024 ]
00217> [ 0.147, 1.3705 ]
00218> [ 0.280, 1.6444 ]
00219> [ 0.472, 1.9242 ]
00220> [ 0.724, 2.2097 ]
00221> [ 0.937, 2.5010 ]
00222> [ 1.262, 2.7981 ]
00223> [ 1.404, 3.1009 ]
00224> [ 1.532, 3.4096 ]
00225> [ 1.650, 3.7240 ]
00226> [ 2.409, 4.0442 ]
00227> [ 3.689, 4.3702 ]
00228> [ -1 , -1 ] (max twenty pts)
00229>
00230> #-----
00231>
00232> # SUB-AREA No. 6
00233>
00234> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00235> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
00236> RAINFALL=[ , , , ] (mm/hr), END=-1
00237> #-----
00238>
00239> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00240> #-----
00241>
00242>
00243> *****
00244> # CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00245> *****
00246>
00247> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00248> # [ ] <- storm filename, one per line for NSTORM time
00249> #-----
00250> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00251> ICASEcs=[1],
00252> A=[732.951], B=[6.199], and C=[0.810],
00253> #-----
00254> DEFAULT VALUES ICASEdef=[1], read and print values
00255> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00256> #-----
00257>
00258> *****
00259> # ORGAWORLD FILE *
00260> *****
00261>
00262> # SUB-AREA No.1
00263>
00264> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00265> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00266> SCS curve number CN=[81],
00267> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00268> LGP=[20] (m), MNP=[0.25 ], SCP=[0.0] (m)
00269> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00270> LGI=[204.72] (m), MNI=[0.03 ], SCI=[0.0]

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00271> RAINFALL=[ , , , ](mm/hr) , END=-1
00272> *%-----
00273> *
00274> * SUB-AREA No.2
00275> *
00276> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[1.54](ha),
00277> XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
00278> SCS curve number CN=[81],
00279> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
00280> LGP=[20.0](m), MNP=[0.03], SCP=[0.0](min),
00281> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.50](%),
00282> LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
00283> RAINFALL=[ , , , ](mm/hr) , END=-1
00284> *%-----
00285> *
00286> * SUB-AREA No.3
00287> *
00288> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
00289> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00290> SCS curve number CN=[81],
00291> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
00292> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
00293> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.51](%),
00294> LGI=[225.63](m), MNI=[0.03], SCI=[0.0]
00295> RAINFALL=[ , , , ](mm/hr) , END=-1
00296> *%-----
00297> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00298> *%-----
00299> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00300> *%-----
00301> *
00302> * SUB-AREA No.4
00303> *
00304> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
00305> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00306> SCS curve number CN=[81],
00307> Pervious surfaces: IAPer=[4.67](mm), SLPP=[0.7](%),
00308> LGP=[40](m), MNP=[0.25], SCP=[0.0](min),
00309> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.93](%),
00310> LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
00311> RAINFALL=[ , , , ](mm/hr) , END=-1
00312> *%-----
00313> *
00314> * SUB-AREA No.5
00315> *
00316> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
00317> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00318> SCS curve number CN=[81],
00319> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
00320> LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min),
00321> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
00322> LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
00323> RAINFALL=[ , , , ](mm/hr) , END=-1
00324> *%-----
00325> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00326> *%-----
00327> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00328> *%-----
00329> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00330> RDT=[1.0](min)
00331> TABLE of ( OUTFLOW-STORAGE ) values
00332> (cms) - (ha-m)
00333> [ 0.000, 0.0000]
00334> [ 0.008, 0.0056]
00335> [ 0.017, 0.1311]
00336> [ 0.093, 0.2831]
00337> [ 0.233, 0.3971]
00338> [ 0.337, 0.4731]
00339> [ 0.465, 0.5491]
00340> [ 0.531, 0.5871]
00341> [ 0.593, 0.6251]
00342> [ 0.654, 0.6631]
00343> [ 0.797, 0.7391]
00344> [ 0.950, 0.8274]
00345> [ 1.304, 0.9157]
00346> [ 1.880, 1.0040]
00347> [ 2.577, 1.0923]
00348> [ -1, -1 ] (max twenty pts)
00349> *%-----
00350> *
00351> *****
00352> * Remaining Hawthorne Industrial Park *
00353> *****
00354> *
00355> * SUB-AREA No.1
00356> *
00357> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5](min), AREA=[19.9](ha),
00358> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00359> SCS curve number CN=[81],
00360> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
00361> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
00362> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.6](%),
00363> LGI=[580](m), MNI=[0.03], SCI=[0.0]
00364> RAINFALL=[ , , , ](mm/hr) , END=-1
00365> *%-----
00366> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00367> *%-----
00368> *
00369> * SUB-AREA No.2
00370> *
00371> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5](min), AREA=[17](ha),
00372> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00373> SCS curve number CN=[81],
00374> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
00375> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
00376> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.65](%),
00377> LGI=[450](m), MNI=[0.03], SCI=[0.0]
00378> RAINFALL=[ , , , ](mm/hr) , END=-1
00379> *%-----
00380> *
00381> * SUB-AREA No.3
00382> *
00383> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5](min), AREA=[15.6](ha),
00384> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00385> SCS curve number CN=[81],
00386> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
00387> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
00388> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.5](%),
00389> LGI=[600](m), MNI=[0.03], SCI=[0.0]
00390> RAINFALL=[ , , , ](mm/hr) , END=-1
00391> *%-----
00392> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00393> *%-----
00394> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00395> *%-----
00396> *
00397> * SUB-AREA No.4
00398> *
00399> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5](min), AREA=[12.2](ha),
00400> XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
00401> SCS curve number CN=[81],
00402> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
00403> LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min),
00404> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.7](%),
00405> LGI=[210](m), MNI=[0.03], SCI=[0.0]

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00406> RAINFALL=[ , , , ](mm/hr) , END=-1
00407> *%-----
00408> *
00409> * SUB-AREA No.5
00410> *
00411> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5](min), AREA=[4.0](ha),
00412> DWF=[0.0](cms), CN/C=[ 85 ], TP=[0.17]hrs,
00413> RAINFALL=[ , , , ](mm/hr) , END=-1
00414> *%-----
00415> *
00416> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00417> *%-----
00418> *
00419> ROUTE RESERVOIR IDout=[10], NHYD=["HIP-POND"], IDin=[ 9 ],
00420> RDT=[1.0](min)
00421> TABLE of ( OUTFLOW-STORAGE ) values
00422> (cms) - (ha-m)
00423> [ 0.0, 0.0 ]
00424> [ 0.048, 0.0574 ]
00425> [ 0.054, 0.2434 ]
00426> [ 0.059, 0.5834 ]
00427> [ 0.062, 0.8400 ]
00428> [ 0.064, 1.1024 ]
00429> [ 0.147, 1.3705 ]
00430> [ 0.280, 1.6444 ]
00431> [ 0.472, 1.9242 ]
00432> [ 0.724, 2.2097 ]
00433> [ 0.937, 2.5010 ]
00434> [ 1.262, 2.7981 ]
00435> [ 1.404, 3.1009 ]
00436> [ 1.532, 3.4096 ]
00437> [ 1.650, 3.7240 ]
00438> [ 2.409, 4.0442 ]
00439> [ 3.689, 4.3702 ]
00440> [ -1, -1 ] (max twenty pts)
00441> *%-----
00442> *
00443> * SUB-AREA No.6
00444> *
00445> *
00446> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5](min), AREA=[2.7](ha),
00447> DWF=[0](cms), CN=[76], TP=[0.80]hrs,
00448> RAINFALL=[ , , , ](mm/hr) , END=-1
00449> *%-----
00450> *
00451> *
00452> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00453> *%-----
00454> *
00455> *****
00456> *****
00457> *****
00458> *****
00459> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00460> [ ] <- storm filename, one per line for NSTORM time
00461> *%-----
00462> CHICAGO STORM IUNITS=[2], TD=[3.0](hrs), TPRAT=[0.333], CSDT=[10.0](min)
00463> ICASECS=[1],
00464> A=[998.071], B=[6.053], and C=[0.814]
00465> *%-----
00466> DEFAULT VALUES ICASEDEF=[1], read and print values
00467> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00468> *%-----
00469> *
00470> *****
00471> *****
00472> *****
00473> *
00474> * SUB-AREA No.1
00475> *
00476> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5](min), AREA=[2.07](ha),
00477> XIMP=[0.84], TIMP=[0.84], DWF=[0.0](cms), LOSS=[2],
00478> SCS curve number CN=[81],
00479> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
00480> LGP=[20](m), MNP=[0.25], SCP=[0.0](min),
00481> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.52](%),
00482> LGI=[204.72](m), MNI=[0.03], SCI=[0.0]
00483> RAINFALL=[ , , , ](mm/hr) , END=-1
00484> *%-----
00485> *
00486> * SUB-AREA No.2
00487> *
00488> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[1.54](ha),
00489> XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
00490> SCS curve number CN=[81],
00491> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
00492> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
00493> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.50](%),
00494> LGI=[244.34](m), MNI=[0.03], SCI=[0.0]
00495> RAINFALL=[ , , , ](mm/hr) , END=-1
00496> *%-----
00497> *
00498> * SUB-AREA No.3
00499> *
00500> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
00501> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00502> SCS curve number CN=[81],
00503> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
00504> LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
00505> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.51](%),
00506> LGI=[225.63](m), MNI=[0.03], SCI=[0.0]
00507> RAINFALL=[ , , , ](mm/hr) , END=-1
00508> *%-----
00509> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00510> *%-----
00511> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00512> *%-----
00513> *
00514> * SUB-AREA No.4
00515> *
00516> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
00517> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00518> SCS curve number CN=[81],
00519> Pervious surfaces: IAPer=[4.67](mm), SLPP=[0.7](%),
00520> LGP=[40](m), MNP=[0.25], SCP=[0.0](min),
00521> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.93](%),
00522> LGI=[164.82](m), MNI=[0.03], SCI=[0.0]
00523> RAINFALL=[ , , , ](mm/hr) , END=-1
00524> *%-----
00525> *
00526> * SUB-AREA No.5
00527> *
00528> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
00529> XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
00530> SCS curve number CN=[81],
00531> Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
00532> LGP=[20.0](m), MNP=[0.25], SCP=[0.0](min),
00533> Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
00534> LGI=[207.25](m), MNI=[0.03], SCI=[0.0]
00535> RAINFALL=[ , , , ](mm/hr) , END=-1
00536> *%-----
00537> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00538> *%-----
00539> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00540> *%-----

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00541>
00542> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00543> RDT=[1.0] (min),
00544> TABLE of ( OUTFLOW-STORAGE ) values
00545> (cms) - (ha-m)
00546> [ 0.000, 0.0000 ]
00547> [ 0.008, 0.0656 ]
00548> [ 0.017, 0.1311 ]
00549> [ 0.093, 0.2831 ]
00550> [ 0.233, 0.3971 ]
00551> [ 0.337, 0.4731 ]
00552> [ 0.465, 0.5491 ]
00553> [ 0.531, 0.5871 ]
00554> [ 0.593, 0.6251 ]
00555> [ 0.654, 0.6631 ]
00556> [ 0.797, 0.7391 ]
00557> [ 0.950, 0.8274 ]
00558> [ 1.304, 0.9157 ]
00559> [ 1.880, 1.0040 ]
00560> [ 2.577, 1.0923 ]
00561> [ -1, -1 ] (max twenty pts)
00562>
00563> *****
00564> * Remaining Hawthorne Industrial Park *
00565> *****
00566> *
00567> * SUB-AREA No.1
00568>
00569> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00570> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00571> SCS curve number CN=[81],
00572> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00573> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00574> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.6] (%),
00575> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00576> RAINFALL=[ , , , ] (mm/hr), END=-1
00577> *
00578> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00579> *
00580> *
00581> * SUB-AREA No.2
00582>
00583> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00584> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00585> SCS curve number CN=[81],
00586> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00587> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00588> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
00589> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00590> RAINFALL=[ , , , ] (mm/hr), END=-1
00591> *
00592> *
00593> * SUB-AREA No.3
00594>
00595> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00596> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00597> SCS curve number CN=[81],
00598> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00599> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00600> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
00601> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
00602> RAINFALL=[ , , , ] (mm/hr), END=-1
00603> *
00604> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00605> *
00606> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00607> *
00608> *
00609> * SUB-AREA No.4
00610>
00611> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00612> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00613> SCS curve number CN=[81],
00614> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00615> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00616> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
00617> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
00618> RAINFALL=[ , , , ] (mm/hr), END=-1
00619> *
00620> *
00621> * SUB-AREA No.5
00622>
00623>
00624> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00625> DWF=[0] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00626> RAINFALL=[ , , , ] (mm/hr), END=-1
00627> *
00628>
00629> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00630> *
00631> *
00632> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
00633> RDT=[1.0] (min),
00634> TABLE of ( OUTFLOW-STORAGE ) values
00635> (cms) - (ha-m)
00636> [ 0.0, 0.0 ]
00637> [ 0.048, 0.0574 ]
00638> [ 0.054, 0.2434 ]
00639> [ 0.059, 0.5834 ]
00640> [ 0.062, 0.8400 ]
00641> [ 0.064, 1.1024 ]
00642> [ 0.147, 1.3705 ]
00643> [ 0.280, 1.6444 ]
00644> [ 0.472, 1.9242 ]
00645> [ 0.724, 2.2097 ]
00646> [ 0.937, 2.5010 ]
00647> [ 1.262, 2.7981 ]
00648> [ 1.404, 3.1009 ]
00649> [ 1.532, 3.4096 ]
00650> [ 1.650, 3.7240 ]
00651> [ 2.409, 4.0442 ]
00652> [ 3.689, 4.3702 ]
00653> [ -1, -1 ] (max twenty pts)
00654>
00655> *
00656> *
00657> * SUB-AREA No. 6
00658>
00659> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00660> DWF=[0] (cms), CNC=[76], TP=[0.80] hrs,
00661> RAINFALL=[ , , , ] (mm/hr), END=-1
00662> *
00663>
00664> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00665> *
00666> *
00667> *****
00668> * CALCULATION OF 3HR - 1:10 YEAR STORM EVENT *
00669> *****
00670>
00671> TZERO=[0.0], MFCOUT=[2], NSTORM=[0], NRUN=[0]
00672> *
00673> * C-storm filename, one per line for NSTORM time
00674> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00675> ICASEcs=[1],

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00676>
00677> A=[1174.184], B=[6.014], and C=[0.816],
00678> *****
00679> DEFAULT VALUES ICASEdef=[1], read and print values
00680> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00681> *****
00682> *****
00683> * ORGAWORLD FILE *
00684> *****
00685> *
00686> * SUB-AREA No.1
00687>
00688> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00689> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00690> SCS curve number CN=[81],
00691> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00692> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (m)
00693> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
00694> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min)
00695> RAINFALL=[ , , , ] (mm/hr), END=-1
00696> *
00697> *
00698> * SUB-AREA No.2
00699>
00700> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00701> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00702> SCS curve number CN=[81],
00703> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00704> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min)
00705> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
00706> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0] (min)
00707> RAINFALL=[ , , , ] (mm/hr), END=-1
00708> *
00709> *
00710> * SUB-AREA No.3
00711>
00712> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00713> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00714> SCS curve number CN=[81],
00715> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.0] (%),
00716> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min)
00717> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
00718> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0] (min)
00719> RAINFALL=[ , , , ] (mm/hr), END=-1
00720> *
00721> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00722> *
00723> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00724> *
00725> *
00726> * SUB-AREA No.4
00727>
00728> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00729> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00730> SCS curve number CN=[81],
00731> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[0.7] (%),
00732> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00733> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
00734> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min)
00735> RAINFALL=[ , , , ] (mm/hr), END=-1
00736> *
00737> *
00738> * SUB-AREA No.5
00739>
00740> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00741> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00742> SCS curve number CN=[81],
00743> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00744> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (m)
00745> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
00746> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min)
00747> RAINFALL=[ , , , ] (mm/hr), END=-1
00748> *
00749> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00750> *
00751> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00752> *
00753>
00754> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00755> RDT=[1.0] (min),
00756> TABLE of ( OUTFLOW-STORAGE ) values
00757> (cms) - (ha-m)
00758> [ 0.000, 0.0000 ]
00759> [ 0.008, 0.0656 ]
00760> [ 0.017, 0.1311 ]
00761> [ 0.093, 0.2831 ]
00762> [ 0.233, 0.3971 ]
00763> [ 0.337, 0.4731 ]
00764> [ 0.465, 0.5491 ]
00765> [ 0.531, 0.5871 ]
00766> [ 0.593, 0.6251 ]
00767> [ 0.654, 0.6631 ]
00768> [ 0.797, 0.7391 ]
00769> [ 0.950, 0.8274 ]
00770> [ 1.304, 0.9157 ]
00771> [ 1.880, 1.0040 ]
00772> [ 2.577, 1.0923 ]
00773> [ -1, -1 ] (max twenty pts)
00774>
00775> *****
00776> * Remaining Hawthorne Industrial Park *
00777> *****
00778> *
00779> * SUB-AREA No.1
00780>
00781> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00782> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00783> SCS curve number CN=[81],
00784> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00785> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00786> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.6] (%),
00787> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00788> RAINFALL=[ , , , ] (mm/hr), END=-1
00789> *
00790> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00791> *
00792> *
00793> * SUB-AREA No.2
00794>
00795> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00796> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00797> SCS curve number CN=[81],
00798> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),
00799> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00800> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
00801> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00802> RAINFALL=[ , , , ] (mm/hr), END=-1
00803> *
00804> *
00805> * SUB-AREA No.3
00806>
00807> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00808> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00809> SCS curve number CN=[81],
00810> Pervious surfaces: IAPer=[4.67] (mm), SLPP=[1.5] (%),

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00811> Impervious surfaces: LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00812> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.5] (%),
00813> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min
00814> RAINFALL=[ , , , ] (mm/hr), END=-1
00815> *%-----
00816> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00817> *%-----
00818> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00819> *%-----
00820> *
00821> * SUB-AREA No.4
00822>
00823> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00824> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00825> SCS curve number CN=[81],
00826> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00827> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00828> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.7] (%),
00829> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min
00830> RAINFALL=[ , , , ] (mm/hr), END=-1
00831>
00832> *%-----
00833> *
00834> * SUB-AREA No.5
00835>
00836> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00837> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00838> RAINFALL=[ , , , ] (mm/hr), END=-1
00839> *%-----
00840>
00841> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00842> *%-----
00843>
00844> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
00845> RDT=[1.0] (min),
00846> TABLE of ( OUTFLOW-STORAGE ) values
00847> (cms) - (ha-m)
00848> [ 0.0, 0.0 ]
00849> [ 0.048, 0.0574 ]
00850> [ 0.054, 0.2434 ]
00851> [ 0.059, 0.5834 ]
00852> [ 0.062, 0.8400 ]
00853> [ 0.064, 1.1024 ]
00854> [ 0.147, 1.3705 ]
00855> [ 0.280, 1.6444 ]
00856> [ 0.472, 1.9242 ]
00857> [ 0.724, 2.2097 ]
00858> [ 0.937, 2.5010 ]
00859> [ 1.262, 2.7981 ]
00860> [ 1.404, 3.1009 ]
00861> [ 1.532, 3.4096 ]
00862> [ 1.650, 3.7240 ]
00863> [ 2.409, 4.0442 ]
00864> [ 3.689, 4.3702 ]
00865> [ -1, -1 ] (max twenty pts)
00866>
00867> *%-----
00868> *
00869> * SUB-AREA No. 6
00870>
00871> DESIGN NASHYD ID=[ 1 ], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00872> DWF=[0] (cms), CNC=[76], TP=[0.80] hrs,
00873> RAINFALL=[ , , , ] (mm/hr), END=-1
00874> *%-----
00875>
00876> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00877> *%-----
00878>
00879>
00880> ***** CALCULATION OF 3HR - 1.25 YEAR STORM EVENT *****
00881> *****
00882>
00883>
00884> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00885> *%-----
00886> * [ ] <- storm filename, one per line for NSTORM time
00887> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00888> ICASECS=[1],
00889> A=[1402.884], B=[6.018], and C=[0.819],
00890>
00891> DEFAULT VALUES ICASEDEF=[1], read and print values
00892> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00893> *%-----
00894>
00895> ***** ORGA WORLD FILE *****
00896> *
00897> *
00898> *
00899> * SUB-AREA No.1
00900>
00901> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00902> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00903> SCS curve number CN=[81],
00904> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00905> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00906> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.52] (%),
00907> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min
00908> RAINFALL=[ , , , ] (mm/hr), END=-1
00909> *%-----
00910> *
00911> * SUB-AREA No.2
00912>
00913> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00914> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00915> SCS curve number CN=[81],
00916> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00917> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00918> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.50] (%),
00919> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0] (min
00920> RAINFALL=[ , , , ] (mm/hr), END=-1
00921> *%-----
00922> *
00923> * SUB-AREA No.3
00924>
00925> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00926> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00927> SCS curve number CN=[81],
00928> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00929> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00930> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.51] (%),
00931> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0] (min
00932> RAINFALL=[ , , , ] (mm/hr), END=-1
00933> *%-----
00934> ADD HYD IDsum=[4], NHYD=[ "040"], IDs to add=[1+2]
00935> *%-----
00936> ADD HYD IDsum=[5], NHYD=[ "050"], IDs to add=[3+4]
00937> *%-----
00938> *
00939> * SUB-AREA No.4
00940>
00941> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00942> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00943> SCS curve number CN=[81],
00944> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00945> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min
00946> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.93] (%),
00947> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min
00948> RAINFALL=[ , , , ] (mm/hr), END=-1
00949> *%-----
00950> *
00951> * SUB-AREA No.5
00952>
00953> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00954> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00955> SCS curve number CN=[81],
00956> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00957> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min
00958> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.61] (%),
00959> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min
00960> RAINFALL=[ , , , ] (mm/hr), END=-1
00961> *%-----
00962> ADD HYD IDsum=[8], NHYD=[ "080"], IDs to add=[6+7]
00963> *%-----
00964> ADD HYD IDsum=[9], NHYD=[ "090"], IDs to add=[5+8]
00965> *%-----
00966>
00967> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00968> RDT=[1.0] (min),
00969> TABLE of ( OUTFLOW-STORAGE ) values
00970> (cms) - (ha-m)
00971> [ 0.000, 0.0000 ]
00972> [ 0.008, 0.0656 ]
00973> [ 0.017, 0.1311 ]
00974> [ 0.093, 0.2831 ]
00975> [ 0.233, 0.3971 ]
00976> [ 0.337, 0.4731 ]
00977> [ 0.465, 0.5491 ]
00978> [ 0.531, 0.5871 ]
00979> [ 0.593, 0.6251 ]
00980> [ 0.654, 0.6631 ]
00981> [ 0.797, 0.7391 ]
00982> [ 0.950, 0.8274 ]
00983> [ 1.304, 0.9157 ]
00984> [ 1.880, 1.0040 ]
00985> [ 2.577, 1.0923 ]
00986> [ -1, -1 ] (max twenty pts)
00987>
00988> ***** Remaining Hawthorne Industrial Park *****
00989> *
00990> *****
00991> *
00992> * SUB-AREA No.1
00993>
00994> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00995> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00996> SCS curve number CN=[81],
00997> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00998> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00999> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.6] (%),
01000> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min
01001> RAINFALL=[ , , , ] (mm/hr), END=-1
01002> *%-----
01003> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
01004> *%-----
01005> *
01006> * SUB-AREA No.2
01007>
01008> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01009> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01010> SCS curve number CN=[81],
01011> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01012> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
01013> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.65] (%),
01014> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min
01015> RAINFALL=[ , , , ] (mm/hr), END=-1
01016> *%-----
01017> *
01018> * SUB-AREA No.3
01019>
01020> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
01021> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01022> SCS curve number CN=[81],
01023> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01024> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
01025> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.5] (%),
01026> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min
01027> RAINFALL=[ , , , ] (mm/hr), END=-1
01028> *%-----
01029> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01030> *%-----
01031> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
01032> *%-----
01033> *
01034> * SUB-AREA No.4
01035>
01036> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
01037> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01038> SCS curve number CN=[81],
01039> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01040> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
01041> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.7] (%),
01042> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min
01043> RAINFALL=[ , , , ] (mm/hr), END=-1
01044> *%-----
01045> *
01046> * SUB-AREA No.5
01047>
01048> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
01049> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
01050> RAINFALL=[ , , , ] (mm/hr), END=-1
01051> *%-----
01052> *
01053>
01054> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
01055> *%-----
01056>
01057> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
01058> RDT=[1.0] (min),
01059> TABLE of ( OUTFLOW-STORAGE ) values
01060> (cms) - (ha-m)
01061> [ 0.0, 0.0 ]
01062> [ 0.048, 0.0574 ]
01063> [ 0.054, 0.2434 ]
01064> [ 0.059, 0.5834 ]
01065> [ 0.062, 0.8400 ]
01066> [ 0.064, 1.1024 ]
01067> [ 0.147, 1.3705 ]
01068> [ 0.280, 1.6444 ]
01069> [ 0.472, 1.9242 ]
01070> [ 0.724, 2.2097 ]
01071> [ 0.937, 2.5010 ]
01072> [ 1.262, 2.7981 ]
01073> [ 1.404, 3.1009 ]
01074> [ 1.532, 3.4096 ]
01075> [ 1.650, 3.7240 ]
01076> [ 2.409, 4.0442 ]
01077> [ 3.689, 4.3702 ]
01078> [ -1, -1 ] (max twenty pts)
01079>
01080> *%-----

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01081> *
01082> *SUB-AREA No. 6
01083>
01084> DESIGN NASHYD ID = [ 1 ], NHYD=["A3"], DT=[2.5]min, AREA=[2.7] (ha),
01085> DWF=[0] (cms), CNC=[76], TP=[0.80]hrs,
01086> RAINFALL=[ , , , ] (mm/hr), END=-1
01087> *%-----
01088>
01089> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
01090> *%-----
01091>
01092> *****
01093> * CALCULATION OF 3HR - 1:50 YEAR STORM EVENT *
01094> *****
01095>
01096> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01097> *%-----
01098> * [ ] <- storm filename, one per line for NSTORM time
01099>
01099> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
01100> ICASEdef=[1],
01101> A=[1569.580], B=[6.014], and C=[0.820],
01102> *%-----
01103> DEFAULT VALUES ICASEdef=[1], read and print values
01104> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
01105> *%-----
01106>
01107> *****
01108> * ORGAWORLD FILE *
01109> *****
01110>
01111> * SUB-AREA No.1
01112>
01113> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
01114> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
01115> SCS curve number CN=[81],
01116> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01117> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
01118> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01119> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01120> RAINFALL=[ , , , ] (mm/hr), END=-1
01121> *%-----
01122>
01123> * SUB-AREA No.2
01124>
01125> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
01126> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
01127> SCS curve number CN=[81],
01128> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01129> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01130> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
01131> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01132> RAINFALL=[ , , , ] (mm/hr), END=-1
01133> *%-----
01134>
01135> * SUB-AREA No.3
01136>
01137> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
01138> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01139> SCS curve number CN=[81],
01140> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01141> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01142> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
01143> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
01144> RAINFALL=[ , , , ] (mm/hr), END=-1
01145> *%-----
01146> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
01147> *%-----
01148> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
01149> *%-----
01150>
01151> * SUB-AREA No.4
01152>
01153> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
01154> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01155> SCS curve number CN=[81],
01156> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
01157> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
01158> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
01159> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
01160> RAINFALL=[ , , , ] (mm/hr), END=-1
01161> *%-----
01162>
01163> * SUB-AREA No.5
01164>
01165> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
01166> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01167> SCS curve number CN=[81],
01168> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01169> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01170> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01171> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01172> RAINFALL=[ , , , ] (mm/hr), END=-1
01173> *%-----
01174> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
01175> *%-----
01176> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
01177> *%-----
01178>
01179> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
01180> RDT=[1.0] (min),
01181> TABLE of ( OUTFLOW-STORAGE ) values
01182> ( cms ) - ( ha-m )
01183> [ 0.000, 0.0000 ]
01184> [ 0.008, 0.0656 ]
01185> [ 0.017, 0.1311 ]
01186> [ 0.093, 0.2831 ]
01187> [ 0.233, 0.3971 ]
01188> [ 0.337, 0.4731 ]
01189> [ 0.465, 0.5491 ]
01190> [ 0.531, 0.5871 ]
01191> [ 0.593, 0.6251 ]
01192> [ 0.654, 0.6631 ]
01193> [ 0.797, 0.7391 ]
01194> [ 0.950, 0.8274 ]
01195> [ 1.304, 0.9157 ]
01196> [ 1.880, 1.0040 ]
01197> [ 2.577, 1.0923 ]
01198> [ -1, -1 ] (max twenty pts)
01199>
01200> *****
01201> * Remaining Hawthorne Industrial Park *
01202> *****
01203>
01204> * SUB-AREA No.1
01205>
01206> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01207> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01208> SCS curve number CN=[81],
01209> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01210> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01211> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01212> LGI=[580] (m), MNI=[0.03], SCI=[0.0]
01213> RAINFALL=[ , , , ] (mm/hr), END=-1
01214> *%-----
01215> ADD HYD IDsum=[2], NHYD=["HIP02"], IDs to add=[10+1]
01216>
01217>
01218> * SUB-AREA No.2
01219>
01220> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01221> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01222> SCS curve number CN=[81],
01223> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01224> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01225> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01226> LGI=[450] (m), MNI=[0.03], SCI=[0.0]
01227> RAINFALL=[ , , , ] (mm/hr), END=-1
01228> *%-----
01229>
01230> * SUB-AREA No.3
01231>
01232> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
01233> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01234> SCS curve number CN=[81],
01235> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01236> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01237> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01238> LGI=[500] (m), MNI=[0.03], SCI=[0.0]
01239> RAINFALL=[ , , , ] (mm/hr), END=-1
01240> *%-----
01241> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01242> *%-----
01243> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
01244> *%-----
01245>
01246> * SUB-AREA No.4
01247>
01248> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
01249> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01250> SCS curve number CN=[81],
01251> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01252> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01253> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
01254> LGI=[210] (m), MNI=[0.03], SCI=[0.0]
01255> RAINFALL=[ , , , ] (mm/hr), END=-1
01256> *%-----
01257>
01258>
01259> * SUB-AREA No.5
01260>
01261> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5]min, AREA=[4.0] (ha),
01262> DWF=[ 0 ] (cms), CNC=[ 85 ], TP=[0.17]hrs,
01263> RAINFALL=[ , , , ] (mm/hr), END=-1
01264> *%-----
01265>
01266> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
01267> *%-----
01268>
01269> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
01270> RDT=[1.0] (min),
01271> TABLE of ( OUTFLOW-STORAGE ) values
01272> ( cms ) - ( ha-m )
01273> [ 0.0, 0.0 ]
01274> [ 0.048, 0.0574 ]
01275> [ 0.054, 0.2434 ]
01276> [ 0.059, 0.5834 ]
01277> [ 0.062, 0.8400 ]
01278> [ 0.064, 1.1024 ]
01279> [ 0.147, 1.3705 ]
01280> [ 0.280, 1.6444 ]
01281> [ 0.472, 1.9242 ]
01282> [ 0.724, 2.2097 ]
01283> [ 0.937, 2.5010 ]
01284> [ 1.262, 2.7981 ]
01285> [ 1.404, 3.1009 ]
01286> [ 1.532, 3.4096 ]
01287> [ 1.650, 3.7240 ]
01288> [ 2.409, 4.0442 ]
01289> [ 3.689, 4.3702 ]
01290> [ -1, -1 ] (max twenty pts)
01291>
01292> *%-----
01293>
01294> * SUB-AREA No. 6
01295>
01296> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5]min, AREA=[2.7] (ha),
01297> DWF=[0] (cms), CNC=[76], TP=[0.80]hrs,
01298> RAINFALL=[ , , , ] (mm/hr), END=-1
01299> *%-----
01300>
01301> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
01302> *%-----
01303>
01304> *****
01305> * CALCULATION OF 3HR - 1:100 YEAR STORM EVENT *
01306> *****
01307>
01308> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01309> *%-----
01310> * [ ] <- storm filename, one per line for NSTORM time
01311>
01312> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
01313> ICASEdef=[1],
01314> A=[1735.688], B=[6.014], and C=[0.820],
01315> *%-----
01316> DEFAULT VALUES ICASEdef=[1], read and print values
01317> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
01318> *%-----
01319>
01320> *****
01321> * ORGAWORLD FILE *
01322> *****
01323>
01324> * SUB-AREA No.1
01325>
01326> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
01327> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
01328> SCS curve number CN=[81],
01329> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01330> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
01331> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01332> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01333> RAINFALL=[ , , , ] (mm/hr), END=-1
01334> *%-----
01335>
01336> * SUB-AREA No.2
01337>
01338> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
01339> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
01340> SCS curve number CN=[81],
01341> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01342> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01343> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
01344> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01345> RAINFALL=[ , , , ] (mm/hr), END=-1
01346> *%-----
01347>
01348> * SUB-AREA No.3
01349>
01350> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
01351> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],

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01351> SCS curve number CN=[81],
01352> Pervious surfaces: IAPER=[4.67] (mm), SLP=[1.0] (%),
01353> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01354> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
01355> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0] (min)
01356> RAINFALL=[ , , , ] (mm/hr), END=-1
01357> *%-----
01358> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
01359> *%-----
01360> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
01361> *%-----
01362> *
01363> * SUB-AREA No.4
01364>
01365> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
01366> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01367> SCS curve number CN=[81],
01368> Pervious surfaces: IAPER=[4.67] (mm), SLP=[0.7] (%),
01369> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
01370> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
01371> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min)
01372> RAINFALL=[ , , , ] (mm/hr), END=-1
01373> *%-----
01374> *
01375> * SUB-AREA No.5
01376>
01377> CALIB STANDHYD ID=[7], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
01378> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01379> SCS curve number CN=[81],
01380> Pervious surfaces: IAPER=[4.67] (mm), SLP=[1.5] (%),
01381> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
01382> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01383> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min)
01384> RAINFALL=[ , , , ] (mm/hr), END=-1
01385> *%-----
01386> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
01387> *%-----
01388> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
01389> *%-----
01390>
01391> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
01392> RDT=[1.0] (min),
01393> TABLE of ( OUTFLOW-STORAGE ) values
01394> (cms) - (ha-m)
01395> [ 0.000, 0.0000]
01396> [ 0.008, 0.0656]
01397> [ 0.017, 0.1311]
01398> [ 0.093, 0.2831]
01399> [ 0.233, 0.3971]
01400> [ 0.337, 0.4731]
01401> [ 0.465, 0.5491]
01402> [ 0.531, 0.5871]
01403> [ 0.593, 0.6251]
01404> [ 0.654, 0.6631]
01405> [ 0.797, 0.7391]
01406> [ 0.950, 0.8274]
01407> [ 1.304, 0.9157]
01408> [ 1.880, 1.0040]
01409> [ 2.577, 1.0923]
01410> [ -1, -1 ] (max twenty pts)
01411>
01412> *****
01413> * Remaining Hawthorne Industrial Park *
01414> *****
01415> *
01416> * SUB-AREA No.1
01417>
01418> CALIB STANDHYD ID=[1], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01419> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01420> SCS curve number CN=[81],
01421> Pervious surfaces: IAPER=[4.67] (mm), SLP=[1.5] (%),
01422> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01423> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.6] (%),
01424> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
01425> RAINFALL=[ , , , ] (mm/hr), END=-1
01426> *%-----
01427> ADD HYD IDsum=[2], NHYD=["HIP02"], IDs to add=[10+1]
01428> *%-----
01429> *
01430> * SUB-AREA No.2
01431>
01432> CALIB STANDHYD ID=[3], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01433> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01434> SCS curve number CN=[81],
01435> Pervious surfaces: IAPER=[4.67] (mm), SLP=[1.5] (%),
01436> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01437> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
01438> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
01439> RAINFALL=[ , , , ] (mm/hr), END=-1
01440> *%-----
01441> *
01442> * SUB-AREA No.3
01443>
01444> CALIB STANDHYD ID=[4], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
01445> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01446> SCS curve number CN=[81],
01447> Pervious surfaces: IAPER=[4.67] (mm), SLP=[1.5] (%),
01448> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01449> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
01450> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
01451> RAINFALL=[ , , , ] (mm/hr), END=-1
01452> *%-----
01453> ADD HYD IDsum=[5], NHYD=["HIP05"], IDs to add=[3+4]
01454> *%-----
01455> ADD HYD IDsum=[6], NHYD=["HIP06"], IDs to add=[5+2]
01456> *%-----
01457> *
01458> * SUB-AREA No.4
01459>
01460> CALIB STANDHYD ID=[7], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
01461> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01462> SCS curve number CN=[81],
01463> Pervious surfaces: IAPER=[4.67] (mm), SLP=[1.5] (%),
01464> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01465> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
01466> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
01467> RAINFALL=[ , , , ] (mm/hr), END=-1
01468> *%-----
01469> *
01470> *
01471> * SUB-AREA No.5
01472>
01473> DESIGN NASHYD ID=[8], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
01474> DWF=[0] (cms), CN/C=[85], TP=[0.17] hrs,
01475> RAINFALL=[ , , , ] (mm/hr), END=-1
01476> *%-----
01477> *
01478> ADD HYD IDsum=[9], NHYD=["HIP08"], IDs to add=[6+7+8]
01479> *%-----
01480>
01481> ROUTE RESERVOIR IDout=[10], NHYD=["HIP-POND"], IDin=[9],
01482> RDT=[1.0] (min),
01483> TABLE of ( OUTFLOW-STORAGE ) values
01484> (cms) - (ha-m)
01485> [ 0.0, 0.0 ]

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01486> [ 0.048, 0.0574 ]
01487> [ 0.054, 0.2434 ]
01488> [ 0.059, 0.5834 ]
01489> [ 0.062, 0.8400 ]
01490> [ 0.064, 1.1024 ]
01491> [ 0.147, 1.3705 ]
01492> [ 0.280, 1.6444 ]
01493> [ 0.472, 1.9242 ]
01494> [ 0.724, 2.2097 ]
01495> [ 0.937, 2.5010 ]
01496> [ 1.262, 2.7981 ]
01497> [ 1.404, 3.1009 ]
01498> [ 1.532, 3.4096 ]
01499> [ 1.650, 3.7240 ]
01500> [ 2.409, 4.0442 ]
01501> [ 3.689, 4.3702 ]
01502> [ -1, -1 ] (max twenty pts)
01503>
01504> *%-----
01505> *
01506> * SUB-AREA No. 6
01507>
01508> DESIGN NASHYD ID=[1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
01509> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
01510> RAINFALL=[ , , , ] (mm/hr), END=-1
01511> *%-----
01512> *
01513> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
01514> *%-----
01515>
01516>
01517>
01518>
01519> FINISH

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00001>
00002>
00003> SSSSS W W M M H H Y Y M M O O 999 999
00004> S W W W M M M H H H Y Y M M O O 9 9 9 9
00005> SSSSS W W M M M H H H Y Y M M O O 9 9 9 9 Ver. 4.02
00006> S W W M M M H H Y Y M M O O 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9 9 # 418403
00008>
00009> StormWater Management Hydrologic Model 999 999
00010>
00011>
00012> ***** SWHYMO-99 Ver/4.02 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHMO-83 and OTTHMO-89 *****
00016>
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 727-5199 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhyo@jfsa.com *****
00021>
00022>
00023> ++++++ Licensed user: J. L. Richards & Associates Limited ++++++
00024> ++++++ Ottawa SERIAL#418403 ++++++
00025>
00026>
00027>
00028> *****
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033> *****
00034>
00035> ***** DETAILED OUTPUT *****
00036>
00037>
00038> * DATE: 2009-05-15 TIME: 08:45:21 RUN COUNTER: 000198 *
00039>
00040> * Input filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\SWM-ALL.dat *
00041> * Output filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\SWM-ALL.out *
00042> * Summary filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\SWM-ALL.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047>
00048>
00049>
00050> 001:0001-----
00051> * Project Name : Hawthorne Industrial Park Project Number: [20983] *
00052> * Date : January, 2009 *
00053> * Revised : N/A *
00054> * Developed by : Mark Buchanan, E.I.T. *
00055> * Reviewed by : Guy Forget, P.Eng. *
00056> * Company : J.L. Richards & Associates Limited *
00057> * License # : 4418403 *
00058>
00059>
00060>
00061>
00062>
00063> * FILENAME: V:\20983.DU\ENG\SWHYMO\20983PST.DAT *
00064> * FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00065> * OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00066>
00067>
00068>
00069> * SWHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE *
00070> * PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00071>
00072>
00073> * HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00074> * FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00075>
00076>
00077> * POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00078>
00079>
00080> * CALCULATION OF 4 HR 25 MM STORM EVENT *
00081>
00082>
00083> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\
00084> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\
00085> | METOUT = 2 (output = METRIC)
00086> | NRUN = 001
00087> | NSTORM = 0
00088>
00089>
00090> 001:0002-----
00091>
00092> | READ STORM | Filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\4HR25
00093> | Ptotal= 25.00 mm | Comments: 4hr-15 min 25 MM STORM EVENT (CHICAGO DI
00094>
00095>
00096>
00097>
00098>
00099>
00100>
00101>
00102>
00103> 001:0003-----
00104>
00105> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\ORGA.VAL
00106> | FileTitle= ICASEdv = 1 (read and print data)
00107> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE
00108> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----
00109> | Horton's infiltration equation parameters:
00110> | [P= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
00111> | Parameters for Pervious surfaces in STANDHYD:
00112> | [Ia= 4.67 mm] [LGP= 40.00 mm] [MNI= .250]
00113> | Parameters for Impervious surfaces in STANDHYD:
00114> | [Ia= 1.57 mm] [CLI= 1.50] [MNI= .035]
00115> | Parameters used in NASHYD:
00116> | [Ia= 4.67 mm] [N= 3.00]
00117>
00118> 001:0004-----
00119>
00120> * ORGAWORLD FILE *
00121>
00122>
00123> * SUB-AREA No.1
00124>
00125> | CALIB STANDHYD | Area (ha)= 2.07
00126> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
00127>
00128>
00129>
00130>
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00132>
00133>
00134>
00135>

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00136> over (min) 10.00 30.00
00137> Storage Coeff. (min)= 10.00 (ii) 29.27 (ii)
00138> Unit Hyd. Tpeak (min)= 10.00 30.00
00139> Unit Hyd. peak (cms)= .11 .04
00140>
00141> PEAK FLOW (cms)= .16 .00 .158 (iii)
00142> TIME TO PEAK (hrs)= 1.29 1.75 1.292
00143> RUNOFF VOLUME (mm)= 23.43 5.17 20.508
00144> TOTAL RAINFALL (mm)= 25.00 25.00 24.999
00145> RUNOFF COEFFICIENT = .94 .21 .820
00146>
00147> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
00148> CN* = 81.0 Ia = Dep. Storage (Above)
00149> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00150> THAN THE STORAGE COEFFICIENT.
00151> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00152>
00153>
00154> 001:0005-----
00155>
00156> * SUB-AREA No.2
00157>
00158> | CALIB STANDHYD | Area (ha)= 1.54
00159> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
00160>
00161>
00162>
00163>
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00271> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00272> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00273> THAN THE STORAGE COEFFICIENT.  
 00274> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00275>  
 00276>

00277> 001:0010-----  
 00278> \* SUB-AREA No.5  
 00280> CALIB STANDHYD | Area (ha)= 2.66  
 00281> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 00283>

	IMPERVIOUS	PERVIOUS (i)
00285> Surface Area (ha)=	2.58	.08
00286> Dep. Storage (mm)=	1.57	4.67
00287> Average Slope (%)=	.61	1.50
00288> Length (m)=	207.25	20.00
00289> Mannings n =	.030	.250
00290> Max.eff.Inten.(mm/hr)=	45.63	5.66
00291> over (min)	10.00	27.50
00292> Storage Coeff. (min)=	10.37 (ii)	26.38 (ii)
00293> Unit Hyd. Tpeak (min)=	10.00	27.50
00294> Unit Hyd. peak (cms)=	.11	.04
00295> *TOTALS*		
00296> PEAK FLOW (cms)=	.24	.00
00297> TIME TO PEAK (hrs)=	1.29	1.67
00298> RUNOFF VOLUME (mm)=	23.43	5.17
00299> TOTAL RAINFALL (mm)=	25.00	25.00
00300> RUNOFF COEFFICIENT =	.94	.21

00301> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00302> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00303> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00304> THAN THE STORAGE COEFFICIENT.  
 00305> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00306>  
 00307>  
 00308>  
 00309>

00310> 001:0011-----  
 00311> ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00312> | (ha) (cms) (hrs) (mm) (cms)  
 00313> ID1 06:060 .89 .089 1.25 22.88 .000  
 00314> +ID2 07:070 2.66 .238 1.29 22.88 .000  
 00315> SUM 08:080 3.55 .327 1.29 22.88 .000  
 00316>  
 00317> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00318>  
 00319>  
 00320>

00321> 001:0012-----  
 00322> ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00323> | (ha) (cms) (hrs) (mm) (cms)  
 00324> ID1 05:050 5.01 .396 1.33 21.62 .000  
 00325> +ID2 08:080 3.55 .327 1.29 22.88 .000  
 00326> SUM 09:090 8.56 .716 1.29 22.14 .000  
 00327>  
 00328> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00329>  
 00330>  
 00331>

00332> 001:0013-----  
 00333> ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 00334> IN-09: (090 )  
 00335> OUT-10: (POND )  
 00336> ===== OUTFLOW STORAGE TABLE =====  
 00337> OUTFLOW STORAGE OUTFLOW STORAGE  
 00338> (cms) (ha.m.) (cms) (ha.m.)  
 00339> 000 0000E+00 | .593 .625E+00  
 00340> 008 .650E+01 | .654 .663E+00  
 00341> 017 .131E+00 | .797 .739E+00  
 00342> 093 .283E+00 | .950 .827E+00  
 00343> 233 .397E+00 | 1.304 .915E+00  
 00344> 337 .473E+00 | 1.880 .100E+01  
 00345> 465 .549E+00 | 2.577 .109E+01  
 00346> 531 .587E+00 | .000 .000E+00  
 00347>  
 00348> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 00349> (ha) (cms) (hrs) (mm)  
 00350> INFLOW>09: (090 ) 8.56 .716 1.29 22.14  
 00351> OUTFLOW>10: (POND ) 8.56 .032 3.875 22.14  
 00352>  
 00353> PEAK FLOW REDUCTION [Qout/qin] (%) = 4.470  
 00354> TIME SHIFT OF PEAK FLOW (min) = 155.00  
 00355> MAXIMUM STORAGE USED (ha.m.) = .161E+00  
 00356>  
 00357>  
 00358>  
 00359>

00360> 001:0014-----  
 00361> \*\*\*\*\*  
 00362> \* Remaining Hawthorne Industrial Park \*  
 00363> \*\*\*\*\*  
 00364> \*  
 00365> \* SUB-AREA No.1  
 00366>

00367> CALIB STANDHYD | Area (ha)= 19.90  
 00368> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00369>

	IMPERVIOUS	PERVIOUS (i)
00370> Surface Area (ha)=	14.13	5.77
00371> Dep. Storage (mm)=	1.57	4.67
00372> Average Slope (%)=	.60	1.50
00373> Length (m)=	580.00	100.00
00374> Mannings n =	.030	.250
00375> Max.eff.Inten.(mm/hr)=	34.39	11.90
00376> over (min)	22.50	52.50
00377> Storage Coeff. (min)=	21.64 (ii)	52.88 (ii)
00378> Unit Hyd. Tpeak (min)=	22.50	52.50
00379> Unit Hyd. peak (cms)=	.05	.02
00380> *TOTALS*		
00381> PEAK FLOW (cms)=	.60	.11
00382> TIME TO PEAK (hrs)=	1.50	2.13
00383> RUNOFF VOLUME (mm)=	23.43	8.74
00384> TOTAL RAINFALL (mm)=	25.00	25.00
00385> RUNOFF COEFFICIENT =	.94	.35

00386> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00387> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00388> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00389> THAN THE STORAGE COEFFICIENT.  
 00390> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00391>  
 00392>  
 00393>  
 00394>  
 00395>

00396> 001:0015-----  
 00397> ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00398> | (ha) (cms) (hrs) (mm) (cms)  
 00399> ID1 10:POND 8.56 .032 3.88 22.14 .000  
 00400> +ID2 01:HIP01 19.90 .642 1.54 16.08 .000  
 00401> SUM 02:HIP02 28.46 .655 1.54 17.91 .000  
 00402>  
 00403> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00404>  
 00405>

00406>  
 00407>  
 00408> 001:0016-----  
 00409> \* SUB-AREA No.2  
 00410>  
 00411> CALIB STANDHYD | Area (ha)= 17.00  
 00412> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00413>  
 00414>

	IMPERVIOUS	PERVIOUS (i)
00415> Surface Area (ha)=	12.07	4.93
00416> Dep. Storage (mm)=	1.57	4.67
00417> Average Slope (%)=	.65	1.50
00418> Length (m)=	450.00	100.00
00419> Mannings n =	.030	.250
00420> Max.eff.Inten.(mm/hr)=	40.81	12.73
00421> over (min)	17.50	47.50
00422> Storage Coeff. (min)=	16.94 (ii)	47.35 (ii)
00423> Unit Hyd. Tpeak (min)=	17.50	47.50
00424> Unit Hyd. peak (cms)=	.07	.02
00425> *TOTALS*		
00426> PEAK FLOW (cms)=	.60	.10
00427> TIME TO PEAK (hrs)=	1.42	2.00
00428> RUNOFF VOLUME (mm)=	23.43	8.74
00429> TOTAL RAINFALL (mm)=	25.00	25.00
00430> RUNOFF COEFFICIENT =	.94	.35

00431> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00432> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00433> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00434> THAN THE STORAGE COEFFICIENT.  
 00435> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00436>  
 00437>  
 00438>  
 00439>

00440> 001:0017-----  
 00441> \* SUB-AREA No.3  
 00442>  
 00443> CALIB STANDHYD | Area (ha)= 15.60  
 00444> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00445>  
 00446>

	IMPERVIOUS	PERVIOUS (i)
00447> Surface Area (ha)=	11.08	4.52
00448> Dep. Storage (mm)=	1.57	4.67
00449> Average Slope (%)=	.50	1.50
00450> Length (m)=	600.00	100.00
00451> Mannings n =	.030	.250
00452> Max.eff.Inten.(mm/hr)=	34.39	11.54
00453> over (min)	22.50	55.00
00454> Storage Coeff. (min)=	23.33 (ii)	54.95 (ii)
00455> Unit Hyd. Tpeak (min)=	22.50	55.00
00456> Unit Hyd. peak (cms)=	.05	.02
00457> *TOTALS*		
00458> PEAK FLOW (cms)=	.45	.08
00459> TIME TO PEAK (hrs)=	1.50	2.17
00460> RUNOFF VOLUME (mm)=	23.43	8.74
00461> TOTAL RAINFALL (mm)=	25.00	25.00
00462> RUNOFF COEFFICIENT =	.94	.35

00463> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00464> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00465> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00466> THAN THE STORAGE COEFFICIENT.  
 00467> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00468>  
 00469>  
 00470>  
 00471>

00472> 001:0018-----  
 00473> ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00474> | (ha) (cms) (hrs) (mm) (cms)  
 00475> ID1 03:HIP03 17.00 .625 1.46 16.08 .000  
 00476> +ID2 04:HIP04 15.60 .484 1.54 16.08 .000  
 00477> SUM 05:HIP05 32.60 1.091 1.46 16.08 .000  
 00478>  
 00479> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00480>  
 00481>  
 00482>

00483> 001:0019-----  
 00484> ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 00485> | (ha) (cms) (hrs) (mm) (cms)  
 00486> ID1 05:HIP05 32.60 1.091 1.46 16.08 .000  
 00487> +ID2 02:HIP02 28.46 .655 1.54 17.91 .000  
 00488> SUM 06:HIP06 61.06 1.740 1.50 16.93 .000  
 00489>  
 00490> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 00491>  
 00492>  
 00493>

00494> 001:0020-----  
 00495> \* SUB-AREA No.4  
 00496>  
 00497> CALIB STANDHYD | Area (ha)= 12.20  
 00498> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 00499>  
 00500>

	IMPERVIOUS	PERVIOUS (i)
00501> Surface Area (ha)=	8.66	3.54
00502> Dep. Storage (mm)=	1.57	4.67
00503> Average Slope (%)=	.70	1.50
00504> Length (m)=	210.00	100.00
00505> Mannings n =	.030	.250
00506> Max.eff.Inten.(mm/hr)=	45.63	14.15
00507> over (min)	10.00	40.00
00508> Storage Coeff. (min)=	10.03 (ii)	39.18 (ii)
00509> Unit Hyd. Tpeak (min)=	10.00	40.00
00510> Unit Hyd. peak (cms)=	.11	.03
00511> *TOTALS*		
00512> PEAK FLOW (cms)=	.57	.08
00513> TIME TO PEAK (hrs)=	1.29	1.88
00514> RUNOFF VOLUME (mm)=	23.43	8.74
00515> TOTAL RAINFALL (mm)=	25.00	25.00
00516> RUNOFF COEFFICIENT =	.94	.35

00517> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 00518> CN\* = 81.0 Ia = Dep. Storage (Above)  
 00519> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 00520> THAN THE STORAGE COEFFICIENT.  
 00521> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 00522>  
 00523>  
 00524>  
 00525>

00526> 001:0021-----  
 00527> \* SUB-AREA No.5  
 00528>  
 00529> DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 00530> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 00531> U.H. Tp(hrs)= .170  
 00532>  
 00533> Unit Hyd Qpeak (cms)= .899  
 00534>  
 00535>  
 00536>  
 00537>  
 00538>  
 00539>  
 00540>



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00541> PEAK FLOW (cms) = .077 (i)
00542> TIME TO PEAK (hrs) = 1.375
00543> RUNOFF VOLUME (mm) = 6.343
00544> TOTAL RAINFALL (mm) = 24.999
00545> RUNOFF COEFFICIENT = .254
00546>
00547> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00548>
00549>
00550> 001:0022-----
00551>
00552> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00553> | (ha) (cms) (hrs) (mm) (cms)
00554> ID1 06:HIP06 61.06 1.740 1.50 16.93 .000
00555> +ID2 07:HIP07 12.20 .585 1.29 16.08 .000
00556> +ID3 08:POND-B 4.00 .077 1.38 6.34 .000
00557>
00558> SUM 09:HIP08 77.26 2.227 1.46 16.25 .000
00559>
00560> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00561>
00562>
00563> 001:0023-----
00564>
00565> | ROUTE RESERVOIR Requested routing time step = 1.0 min.
00566> | IN:09:(HIP08) |
00567> | OUT:10:(HIP-PO) |
00568>
00569> OUTFLOW STORAGE TABLE
00570> (cms) (ha.m.) (cms) (ha.m.)
00571> .000 .0000E+00 .724 .2210E+01
00572> .048 .5740E-01 .937 .2501E+01
00573> .054 .2434E+00 1.262 .2798E+01
00574> .059 .5834E+00 1.404 .3101E+01
00575> .062 .8400E+00 1.532 .3410E+01
00576> .064 .1102E+01 1.650 .3724E+01
00577> .147 .1370E+01 2.409 .4044E+01
00578> .280 .1644E+01 3.689 .4370E+01
00579> .472 .1924E+01 .000 .0000E+00
00580>
00581> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00582> (ha) (cms) (hrs) (mm)
00583> INFLOW>09: (HIP08) 77.26 2.227 1.458 16.251
00584> OUTFLOW>10: (HIP-PO) 77.26 .063 5.431 16.251
00585>
00586> PEAK FLOW REDUCTION [Qout/Qin] (%) = 2.839
00587> TIME SHIFT OF PEAK FLOW (min) = 238.33
00588> MAXIMUM STORAGE USED (ha.m.) = .1001E+01
00589>
00590> 001:0024-----
00591> *
00592> *SUB-AREA No. 6
00593>
00594> | DESIGN NASHYD | Area (ha) = 2.70 Curve Number (CN)=76.00
00595> | 01:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
00596> | U.H. Tp (hrs) = .800
00597>
00598> Unit Hyd Qpeak (cms) = .129
00599>
00600> PEAK FLOW (cms) = .013 (i)
00601> TIME TO PEAK (hrs) = 2.292
00602> RUNOFF VOLUME (mm) = 4.110
00603> TOTAL RAINFALL (mm) = 24.999
00604> RUNOFF COEFFICIENT = .164
00605>
00606> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00607>
00608>
00609> 001:0025-----
00610>
00611> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00612> | (ha) (cms) (hrs) (mm) (cms)
00613> ID1 10:HIP-PO 77.26 .063 5.43 16.25 .000
00614> +ID2 01:A3 2.70 .013 2.29 4.11 .000
00615>
00616> SUM 02:Ultima 79.96 .073 2.50 15.84 .000
00617>
00618> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00619>
00620>
00621> 001:0026-----
00622> *****
00623> * CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00624> *****
00625>
00626> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
00627> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
00628> | METOUT= 2 (output = METRIC)
00629> | NRUN = 001
00630> | NSTORM= 0
00631>
00632> 001:0002-----
00633>
00634> | CHICAGO STORM IDF curve parameters: A= 732.951
00635> | Protal= 31.86 mm B= 6.199
00636> C= .810
00637> used in: INTENSITY = A / (t + B)^C
00638>
00639> Duration of storm = 3.00 hrs
00640> Storm time step = 10.00 min
00641> Time to peak ratio = .33
00642>
00643>
00644> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00645> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00646> .17 2.815 | 1.00 76.805 | 1.83 5.095 | 2.67 2.684
00647> .33 3.498 | 1.17 24.079 | 2.00 4.291 | 2.83 2.463
00648> .50 4.687 | 1.33 12.364 | 2.17 3.718 | 3.00 2.279
00649> .67 7.305 | 1.50 8.324 | 2.33 3.288 |
00650> .83 16.209 | 1.67 6.303 | 2.50 2.953 |
00651>
00652> 001:0003-----
00653>
00654> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
00655> | ICSHEDV = 1 (read and print data)
00656> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
00657> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---
00658>
00659> Horton's infiltration equation parameters:
00660> [F0= 50.00 mm/hr] [F0= 7.50 mm/hr] [DCAX= 2.00 /hr] [F= .00 mm]
00661> Parameters for PERVIOUS surfaces in STANDHYD:
00662> [Iaper= 4.67 mm] [LGP= 40.00 mm] [MNI= .250]
00663> Parameters for IMPERVIOUS surfaces in STANDHYD:
00664> [Iaimp= 1.57 mm] [CLI= 1.50] [MNI= .035]
00665> Parameters used in NASHYD:
00666> [Ia= 4.67 mm] [N= 3.00]
00667>
00668> 001:0004-----
00669> *****
00670> | ORGAWORLD FILE *
00671> *****
00672> *
00673> * SUB-AREA No.1
00674> | CALIB STANDHYD | Area (ha) = 2.07
00675>
00676> 01:010 DT= 2.50 | Total Imp(%) = 84.00 Dir. Conn.(%) = 84.00
00677>
00678> IMPERVIOUS PERVIOUS (i)
00679> Surface Area (ha) = 1.74 .33
00680> Dep. Storage (mm) = 1.57 4.67
00681> Average Slope (%) = .52 1.00
00682> Length (m) = 204.72 20.00
00683> Mannings n = .030 .250
00684>
00685> Max.eff.Inten.(mm/hr) = 76.81 11.88
00686> over (min) = 10.00 22.50
00687> Storage Coeff. (min) = 8.77 (ii) 22.21 (ii)
00688> Unit Hyd. Tpeak (min) = 10.00 22.50
00689> Unit Hyd. peak (cms) = .12 .05
00690>
00691> PEAK FLOW (cms) = .24 .01 *TOTALS*
00692> TIME TO PEAK (hrs) = 1.08 1.38 .245 (iii)
00693> RUNOFF VOLUME (mm) = 30.29 8.52 1.083
00694> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00695> RUNOFF COEFFICIENT = .95 .27 .841
00696>
00697> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00698> CN* = 81.0 Ia = Dep. Storage (Above)
00699> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00700> THAN THE STORAGE COEFFICIENT.
00701> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00702>
00703>
00704> 001:0005-----
00705> * SUB-AREA No.2
00706>
00707> | CALIB STANDHYD | Area (ha) = 1.54
00708> | 02:020 DT= 2.50 | Total Imp(%) = 92.00 Dir. Conn.(%) = 92.00
00709>
00710> IMPERVIOUS PERVIOUS (i)
00711> Surface Area (ha) = 1.42 .12
00712> Dep. Storage (mm) = 1.57 4.67
00713> Average Slope (%) = .50 1.00
00714> Length (m) = 244.34 5.00
00715> Mannings n = .030 .030
00716>
00717> Max.eff.Inten.(mm/hr) = 76.81 15.07
00718> over (min) = 10.00 12.50
00719> Storage Coeff. (min) = 9.87 (ii) 11.36 (ii)
00720> Unit Hyd. Tpeak (min) = 10.00 12.50
00721> Unit Hyd. peak (cms) = .11 .10
00722>
00723> PEAK FLOW (cms) = .19 .00 *TOTALS*
00724> TIME TO PEAK (hrs) = 1.08 1.17 .192 (iii)
00725> RUNOFF VOLUME (mm) = 30.29 8.52 1.083
00726> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00727> RUNOFF COEFFICIENT = .95 .27 .896
00728>
00729> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00730> CN* = 81.0 Ia = Dep. Storage (Above)
00731> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00732> THAN THE STORAGE COEFFICIENT.
00733> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00734>
00735>
00736> 001:0006-----
00737> * SUB-AREA No.3
00738>
00739> | CALIB STANDHYD | Area (ha) = 1.40
00740> | 03:030 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00
00741>
00742> IMPERVIOUS PERVIOUS (i)
00743> Surface Area (ha) = 1.36 .04
00744> Dep. Storage (mm) = 1.57 4.67
00745> Average Slope (%) = .51 1.00
00746> Length (m) = 225.63 5.00
00747> Mannings n = .030 .030
00748>
00749> Max.eff.Inten.(mm/hr) = 76.81 16.59
00750> over (min) = 10.00 10.00
00751> Storage Coeff. (min) = 9.35 (ii) 10.79 (ii)
00752> Unit Hyd. Tpeak (min) = 10.00 10.00
00753> Unit Hyd. peak (cms) = .12 .11
00754>
00755> PEAK FLOW (cms) = .18 .00 *TOTALS*
00756> TIME TO PEAK (hrs) = 1.08 1.13 1.083
00757> RUNOFF VOLUME (mm) = 30.29 8.52 29.637
00758> TOTAL RAINFALL (mm) = 31.86 31.86 31.860
00759> RUNOFF COEFFICIENT = .95 .27 .930
00760>
00761> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00762> CN* = 81.0 Ia = Dep. Storage (Above)
00763> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00764> THAN THE STORAGE COEFFICIENT.
00765> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00766>
00767>
00768> 001:0007-----
00769>
00770> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00771> | (ha) (cms) (hrs) (mm) (cms)
00772> ID1 01:010 2.07 .245 1.08 26.81 .000
00773> +ID2 02:020 1.54 .192 1.08 28.55 .000
00774>
00775> SUM 04:040 3.61 .436 1.08 27.55 .000
00776>
00777>
00778> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00779>
00780>
00781> 001:0008-----
00782>
00783> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00784> | (ha) (cms) (hrs) (mm) (cms)
00785> ID1 03:030 1.40 .186 1.08 29.64 .000
00786> +ID2 04:040 3.61 .436 1.08 27.55 .000
00787>
00788> SUM 05:050 5.01 .623 1.08 28.13 .000
00789>
00790> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00791>
00792>
00793> 001:0009-----
00794> * SUB-AREA No.4
00795>
00796> | CALIB STANDHYD | Area (ha) = .89
00797> | 06:060 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00
00798>
00799> IMPERVIOUS PERVIOUS (i)
00800> Surface Area (ha) = .86 .03
00801> Dep. Storage (mm) = 1.57 4.67
00802> Average Slope (%) = .93 .70
00803> Length (m) = 164.82 40.00
00804> Mannings n = .030 .250
00805>
00806> Max.eff.Inten.(mm/hr) = 76.81 10.24
00807> over (min) = 7.50 30.00
00808> Storage Coeff. (min) = 6.47 (ii) 30.53 (ii)
00809>
00810>

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00811> Unit Hyd. Tpeak (min)= 7.50 30.00
00812> Unit Hyd. peak (cms)= .16 .04
00813>
00814> PEAK FLOW (cms)= .14 .00 *TOTALS*
00815> TIME TO PEAK (hrs)= 1.04 1.54 1.39 (iii)
00816> RUNOFF VOLUME (mm)= 30.29 8.52 29.637
00817> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00818> RUNOFF COEFFICIENT = .95 .27 .930
00819>
00820> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00821> CN* = 81.0 Ia = Dep. Storage (Above)
00822> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00823> THAN THE STORAGE COEFFICIENT.
00824> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00825>
00826>
00827> 001:0010-----
00828> * SUB-AREA No.5
00829>
00830>
00831> | CALIB STANDHYD | Area (ha)= 2.66
00832> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00833>
00834> IMPERVIOUS PERVIOUS (i)
00835> Surface Area (ha)= 2.58 .08
00836> Dep. Storage (mm)= 1.57 4.67
00837> Average Slope (%)= .61 1.50
00838> Length (m)= 207.25 20.00
00839> Mannings n = .030 .250
00840>
00841> Max.eff.Inten.(mm/hr)= 76.81 12.71
00842> over (min) 7.50 20.00
00843> Storage Coeff. (min)= 8.42 (ii) 20.00 (ii)
00844> Unit Hyd. Tpeak (min)= 7.50 20.00
00845> Unit Hyd. peak (cms)= .14 .06
00846>
00847> PEAK FLOW (cms)= .38 .00 *TOTALS*
00848> TIME TO PEAK (hrs)= 1.04 1.33 1.042
00849> RUNOFF VOLUME (mm)= 31.29 8.52 29.637
00850> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00851> RUNOFF COEFFICIENT = .95 .27 .930
00852>
00853> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00854> CN* = 81.0 Ia = Dep. Storage (Above)
00855> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00856> THAN THE STORAGE COEFFICIENT.
00857> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00858>
00859>
00860> 001:0011-----
00861>
00862> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00863> | 01:06:060 | (ha) (cms) (hrs) (mm) (cms)
00864> | +ID2 07:070 | 2.66 .139 1.04 29.64 .000
00865> | SUM 08:080 | 3.55 .518 1.04 29.64 .000
00866>
00867> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00868>
00869>
00870>
00871>
00872> 001:0012-----
00873> | ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00874> | 01:05:050 | (ha) (cms) (hrs) (mm) (cms)
00875> | +ID2 08:080 | 3.55 .518 1.04 29.64 .000
00876> | SUM 09:090 | 8.56 1.118 1.08 28.76 .000
00877>
00878> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00879>
00880>
00881>
00882>
00883> 001:0013-----
00884>
00885>
00886> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00887> | IN>09: (090) |
00888> | OUT<10: (POND) |
00889>
00890> OUTFLOW STORAGE TABLE
00891> (cms) (ha.m.) (cms) (ha.m.)
00892> .000 .0000E+00 .593 .6251E+00
00893> .008 .6560E-01 .654 .6631E+00
00894> .017 .1311E+00 .797 .7391E+00
00895> .093 .2831E+00 .950 .8274E+00
00896> .233 .3971E+00 1.304 .9157E+00
00897> .337 .4731E+00 1.880 .1004E+01
00898> .465 .5491E+00 2.577 .1092E+01
00899> .531 .5871E+00 .000 .0000E+00
00900>
00901> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00902> (ha) (cms) (hrs) (mm)
00903> INFLOW >09: (090) 8.56 1.118 1.083 28.757
00904> OUTFLOW<10: (POND) 8.56 .056 3.000 28.754
00905>
00906> PEAK FLOW REDUCTION [Qout/Qin](%)= 5.030
00907> TIME SHIFT OF PEAK FLOW (min)= 115.00
00908> MAXIMUM STORAGE USED (ha.m.)= .2095E+00
00909>
00910> 001:0014-----
00911> *****
00912> * Remaining Hawthorne Industrial Park *
00913> *****
00914>
00915> * SUB-AREA No.1
00916>
00917> | CALIB STANDHYD | Area (ha)= 19.90
00918> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00919>
00920> IMPERVIOUS PERVIOUS (i)
00921> Surface Area (ha)= 14.13 5.77
00922> Dep. Storage (mm)= 1.57 4.67
00923> Average Slope (%)= .60 1.50
00924> Length (m)= 580.00 100.00
00925> Mannings n = .030 .250
00926>
00927> Max.eff.Inten.(mm/hr)= 54.21 23.06
00928> over (min) 17.50 42.50
00929> Storage Coeff. (min)= 18.04 (ii) 42.02 (ii)
00930> Unit Hyd. Tpeak (min)= 17.50 42.50
00931> Unit Hyd. peak (cms)= .06 .03
00932>
00933> PEAK FLOW (cms)= .95 .21 *TOTALS*
00934> TIME TO PEAK (hrs)= 1.21 1.71 1.250
00935> RUNOFF VOLUME (mm)= 30.29 13.34 21.814
00936> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00937> RUNOFF COEFFICIENT = .95 .42 .685
00938>
00939> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00940> CN* = 81.0 Ia = Dep. Storage (Above)
00941> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00942> THAN THE STORAGE COEFFICIENT.
00943> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00944>
00945>

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00946> 001:0015-----
00947>
00948> | ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00949> | 01:10:POND | (ha) (cms) (hrs) (mm) (cms)
00950> | +ID2 01:HIP01 | 8.56 .056 3.00 28.75 .000
00951> | SUM 02:HIP02 | 19.90 1.020 1.25 21.81 .000
00952>
00953> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00954>
00955>
00956>
00957>
00958> 001:0016-----
00959>
00960> * SUB-AREA No.2
00961>
00962> | CALIB STANDHYD | Area (ha)= 17.00
00963> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00964>
00965> IMPERVIOUS PERVIOUS (i)
00966> Surface Area (ha)= 12.07 4.93
00967> Dep. Storage (mm)= 1.57 4.67
00968> Average Slope (%)= .65 1.50
00969> Length (m)= 450.00 100.00
00970> Mannings n = .030 .250
00971>
00972> Max.eff.Inten.(mm/hr)= 59.23 25.04
00973> over (min) 15.00 37.50
00974> Storage Coeff. (min)= 14.60 (ii) 37.80 (ii)
00975> Unit Hyd. Tpeak (min)= 15.00 37.50
00976> Unit Hyd. peak (cms)= .08 .03
00977>
00978> PEAK FLOW (cms)= .91 .19 *TOTALS*
00979> TIME TO PEAK (hrs)= 1.17 1.63 .978 (iii)
00980> RUNOFF VOLUME (mm)= 30.29 13.34 1.167
00981> TOTAL RAINFALL (mm)= 31.86 31.86 21.814
00982> RUNOFF COEFFICIENT = .95 .42 31.860
00983>
00984> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00985> CN* = 81.0 Ia = Dep. Storage (Above)
00986> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00987> THAN THE STORAGE COEFFICIENT.
00988> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00989>
00990>
00991> 001:0017-----
00992>
00993> * SUB-AREA No.3
00994>
00995> | CALIB STANDHYD | Area (ha)= 15.60
00996> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00997>
00998> IMPERVIOUS PERVIOUS (i)
00999> Surface Area (ha)= 11.08 4.52
01000> Dep. Storage (mm)= 1.57 4.67
01001> Average Slope (%)= .50 1.50
01002> Length (m)= 600.00 100.00
01003> Mannings n = .030 .250
01004>
01005> Max.eff.Inten.(mm/hr)= 50.44 22.17
01006> over (min) 20.00 45.00
01007> Storage Coeff. (min)= 20.01 (ii) 44.37 (ii)
01008> Unit Hyd. Tpeak (min)= 20.00 45.00
01009> Unit Hyd. peak (cms)= .06 .03
01010>
01011> PEAK FLOW (cms)= .69 .16 *TOTALS*
01012> TIME TO PEAK (hrs)= 1.25 1.79 .753 (iii)
01013> RUNOFF VOLUME (mm)= 30.29 13.34 1.292
01014> TOTAL RAINFALL (mm)= 31.86 31.86 21.814
01015> RUNOFF COEFFICIENT = .95 .42 31.860
01016>
01017> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01018> CN* = 81.0 Ia = Dep. Storage (Above)
01019> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01020> THAN THE STORAGE COEFFICIENT.
01021> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01022>
01023>
01024> 001:0018-----
01025>
01026> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01027> | 01:03:HIP03 | (ha) (cms) (hrs) (mm) (cms)
01028> | +ID2 04:HIP04 | 17.00 .978 1.17 21.81 .000
01029> | SUM 05:HIP05 | 15.60 .753 1.29 21.81 .000
01030>
01031> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01032>
01033>
01034>
01035>
01036> 001:0019-----
01037>
01038> | ADD HYD (HIP06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01039> | 01:05:HIP05 | (ha) (cms) (hrs) (mm) (cms)
01040> | +ID2 02:HIP02 | 32.60 1.698 1.21 21.81 .000
01041> | SUM 06:HIP06 | 28.46 1.039 1.25 23.90 .000
01042>
01043> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01044>
01045>
01046>
01047>
01048> 001:0020-----
01049>
01050> * SUB-AREA No.4
01051>
01052> | CALIB STANDHYD | Area (ha)= 12.20
01053> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01054>
01055> IMPERVIOUS PERVIOUS (i)
01056> Surface Area (ha)= 8.26 3.54
01057> Dep. Storage (mm)= 1.57 4.67
01058> Average Slope (%)= .70 1.50
01059> Length (m)= 210.00 100.00
01060> Mannings n = .030 .250
01061>
01062> Max.eff.Inten.(mm/hr)= 76.81 29.02
01063> over (min) 7.50 30.00
01064> Storage Coeff. (min)= 8.15 (ii) 30.01 (ii)
01065> Unit Hyd. Tpeak (min)= 7.50 30.00
01066> Unit Hyd. peak (cms)= .14 .04
01067>
01068> PEAK FLOW (cms)= .91 .16 *TOTALS*
01069> TIME TO PEAK (hrs)= 1.04 1.50 .941 (iii)
01070> RUNOFF VOLUME (mm)= 30.29 13.34 1.042
01071> TOTAL RAINFALL (mm)= 31.86 31.86 21.814
01072> RUNOFF COEFFICIENT = .95 .42 31.860
01073>
01074> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01075> CN* = 81.0 Ia = Dep. Storage (Above)
01076> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01077> THAN THE STORAGE COEFFICIENT.
01078> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01079>
01080>

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01081> 001:0021-----
01082> *
01083> *SUB-AREA No.5
01084>
01085> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
01086> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01087> | U.H. Tp(hrs)= .170
01088>
01089> Unit Hyd Qpeak (cms)= .899
01090>
01091> PEAK FLOW (cms)= .145 (i)
01092> TIME TO PEAK (hrs)= 1.167
01093> RUNOFF VOLUME (mm)= 10.266
01094> TOTAL RAINFALL (mm)= 31.860
01095> RUNOFF COEFFICIENT = .322
01096>
01097> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01098>
01099>
01100> 001:0022-----
01101>
01102> | ADD HYD (HIP08 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01103> | | (ha) (cms) (hrs) (mm) (cms)
01104> | ID1 06:HIP06 | 61.06 2.733 1.21 22.79 .000
01105> | +ID2 07:HIP07 | 12.20 .941 1.04 21.81 .000
01106> | +ID3 08:Pond-B | 4.00 .145 1.17 10.27 .000
01107>
01108> SUM 09:HIP08 77.26 3.542 1.21 21.98 .000
01109>
01110> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01111>
01112>
01113> 001:0023-----
01114>
01115> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01116> | IN:09: (HIP08 ) |
01117> | OUT:10: (HIP-PO) |
01118>
01119> ===== OUTFLOW STORAGE TABLE =====
01120> OUTFLOW STORAGE OUTFLOW STORAGE
01121> (cms) (ha.m.) (cms) (ha.m.)
01122> .000 .000E+00 .724 .2210E+01
01123> .049 .5740E+01 .937 .2501E+01
01124> .054 .2434E+00 1.262 .2798E+01
01125> .059 .5834E+00 1.404 .3101E+01
01126> .062 .8400E+00 1.532 .3410E+01
01127> .064 .1102E+01 1.650 .3724E+01
01128> .147 .1370E+01 2.409 .4044E+01
01129> .280 .1644E+01 3.689 .4370E+01
01130> .472 .1924E+01 .000 .0000E+00
01131>
01132> ROUTING RESULTS AREA OPEAK TPEAK R.V.
01133> (ha) (cms) (hrs) (mm)
01134> INFLOW >09: (HIP08 ) 77.26 3.542 1.208 21.985
01135> OUTFLOW <10: (HIP-PO) 77.26 .148 4.014 21.985
01136>
01137> PEAK FLOW REDUCTION [Qout/Qin] (%) = 4.179
01138> TIME SHIFT OF PEAK FLOW (min) = 168.33
01139> MAXIMUM STORAGE USED (ha.m.) = 1373E+01
01140>
01141> 001:0024-----
01142> *
01143> *SUB-AREA No. 6
01144>
01145> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
01146> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01147> | U.H. Tp(hrs)= .800
01148>
01149> Unit Hyd Qpeak (cms)= .129
01150>
01151> PEAK FLOW (cms)= .024 (i)
01152> TIME TO PEAK (hrs)= 2.083
01153> RUNOFF VOLUME (mm)= 6.883
01154> TOTAL RAINFALL (mm)= 31.860
01155> RUNOFF COEFFICIENT = .216
01156>
01157> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01158>
01159>
01160> 001:0025-----
01161> | ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01162> | | (ha) (cms) (hrs) (mm) (cms)
01163> | ID1 10:HIP-PO | 77.26 .148 4.01 21.98 .000
01164> | +ID2 01:A3 | 2.70 .024 2.08 6.88 .000
01165>
01166> SUM 02:Ultima 79.96 .156 3.65 21.47 .000
01167>
01168> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01169>
01170>
01171> 001:0026-----
01172> ***** CALCULATION OF 3HR - 1.5 YEAR STORM EVENT *****
01173> *
01174>
01175> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01176> | Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01177>
01178> TZERO = .00 hrs on 0
01179> METOUT= 2 (output = METRIC)
01180> NRUN = 001
01181> NSTORM= 0
01182>
01183> 001:0002-----
01184>
01185> | CHICAGO STORM | IDF curve parameters: A= 998.071
01186> | Ptotal= 42.51 mm | B= 6.053
01187> | C= .814
01188> used in: INTENSITY = A / (t + B)^C
01189>
01190> Duration of storm = 3.00 hrs
01191> Storm time step = 10.00 min
01192> Time to peak ratio = .33
01193>
01194> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01195> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01196> .17 3.682 | 1.00 104.193 | 1.83 6.689 | 2.67 3.510
01197> .33 4.582 | 1.17 32.037 | 2.00 5.628 | 2.83 3.220
01198> .50 6.251 | 1.33 16.337 | 2.17 4.872 | 3.00 2.978
01199> .67 9.614 | 1.50 10.965 | 2.33 4.305 |
01200> .83 24.170 | 1.67 8.287 | 2.50 3.864 |
01201>
01202>
01203> 001:0003-----
01204>
01205> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
01206> | ICASEdv (read and print data)
01207> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE
01208> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---
01209> Horton's infiltration equation parameters:
01210> [F= 50.00 mm/hr] [P= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01211> Parameters for PERVIOUS surfaces in STANDHYD:
01212> [Ia= 4.67 mm] [LGP= 40.00 mm] [MNI= .250]
01213> Parameters for IMPVIOUS surfaces in STANDHYD:
01214> [Ia= 1.57 mm] [CLI= 1.50] [MNI= .035]
01215> Parameters used in NASHYD:

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01216> [Ia= 4.67 mm] [N= 3.00]
01217>
01218> 001:0004-----
01219> ***** ORGAWORD FILE *****
01220> *
01221> *****
01222> *
01223> * SUB-AREA No.1
01224>
01225> | CALIB STANDHYD | Area (ha)= 2.07
01226> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
01227>
01228> IMPVIOUS PERVIOUS (i)
01229> Surface Area (ha)= 1.74 .33
01230> Dep. Storage (mm)= 1.57 4.67
01231> Average Slope (s)= .52 1.00
01232> Length (m)= 204.72 20.00
01233> Mannings n = .030 .250
01234>
01235> Max.eff.Inten.(mm/hr)= 104.19 24.26
01236> over (min)= 7.50 17.50
01237> Storage Coeff. (min)= 7.76 (ii) 17.86 (ii)
01238> Unit Hyd. Tpeak (min)= 7.50 17.50
01239> Unit Hyd. peak (cms)= .15 .06
01240>
01241> PEAK FLOW (cms)= .36 .01
01242> TIME TO PEAK (hrs)= 1.04 1.25
01243> RUNOFF VOLUME (mm)= 40.94 14.70
01244> TOTAL RAINFALL (mm)= 42.51 42.51
01245> RUNOFF COEFFICIENT = .96 .35
01246>
01247> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01248> CN* = 81.0 Ia = Dep. Storage (Above)
01249> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01250> THAN THE STORAGE COEFFICIENT.
01251> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01252>
01253>
01254> 001:0005-----
01255> *
01256> *SUB-AREA No.2
01257>
01258> | CALIB STANDHYD | Area (ha)= 1.54
01259> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
01260>
01261> IMPVIOUS PERVIOUS (i)
01262> Surface Area (ha)= 1.42 .12
01263> Dep. Storage (mm)= 1.57 4.67
01264> Average Slope (s)= .50 1.00
01265> Length (m)= 244.34 5.00
01266> Mannings n = .030 .030
01267>
01268> Max.eff.Inten.(mm/hr)= 104.19 31.02
01269> over (min)= 7.50 10.00
01270> Storage Coeff. (min)= 8.73 (ii) 9.85 (ii)
01271> Unit Hyd. Tpeak (min)= 7.50 10.00
01272> Unit Hyd. peak (cms)= .14 .11
01273>
01274> PEAK FLOW (cms)= .28 .01
01275> TIME TO PEAK (hrs)= 1.04 1.13
01276> RUNOFF VOLUME (mm)= 40.94 14.70
01277> TOTAL RAINFALL (mm)= 42.51 42.51
01278> RUNOFF COEFFICIENT = .96 .35
01279>
01280> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01281> CN* = 81.0 Ia = Dep. Storage (Above)
01282> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01283> THAN THE STORAGE COEFFICIENT.
01284> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01285>
01286>
01287> 001:0006-----
01288> *
01289> * SUB-AREA No.3
01290>
01291> | CALIB STANDHYD | Area (ha)= 1.40
01292> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01293>
01294> IMPVIOUS PERVIOUS (i)
01295> Surface Area (ha)= 1.36 .04
01296> Dep. Storage (mm)= 1.57 4.67
01297> Average Slope (s)= .51 1.00
01298> Length (m)= 225.63 5.00
01299> Mannings n = .030 .030
01300>
01301> Max.eff.Inten.(mm/hr)= 104.19 31.02
01302> over (min)= 7.50 10.00
01303> Storage Coeff. (min)= 8.28 (ii) 9.39 (ii)
01304> Unit Hyd. Tpeak (min)= 7.50 10.00
01305> Unit Hyd. peak (cms)= .14 .12
01306>
01307> PEAK FLOW (cms)= .27 .00
01308> TIME TO PEAK (hrs)= 1.04 1.13
01309> RUNOFF VOLUME (mm)= 40.94 14.70
01310> TOTAL RAINFALL (mm)= 42.51 42.51
01311> RUNOFF COEFFICIENT = .96 .35
01312>
01313> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01314> CN* = 81.0 Ia = Dep. Storage (Above)
01315> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01316> THAN THE STORAGE COEFFICIENT.
01317> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01318>
01319>
01320> 001:0007-----
01321>
01322> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01323> | | (ha) (cms) (hrs) (mm) (cms)
01324> | ID1 01:010 | 2.07 .362 1.04 36.75 .000
01325> | +ID2 02:020 | 1.54 .283 1.04 38.94 .000
01326>
01327> SUM 04:040 3.61 .645 1.04 37.64 .000
01328>
01329> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01330>
01331>
01332> 001:0008-----
01333>
01334> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01335> | | (ha) (cms) (hrs) (mm) (cms)
01336> | ID1 03:030 | 1.40 .274 1.04 40.16 .000
01337> | +ID2 04:040 | 3.61 .645 1.04 37.64 .000
01338>
01339> SUM 05:050 5.01 .918 1.04 38.34 .000
01340>
01341> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01342>
01343>
01344> 001:0009-----
01345> *
01346> * SUB-AREA No.4
01347>
01348> | CALIB STANDHYD | Area (ha)= .89
01349> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01350>

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01351> Surface Area (ha)= 8.6 IMPERVIOUS PERVIOUS (i)
01352> Dep. Storage (mm)= 1.57 4.67
01353> Average Slope (%)= 1.93 7.70
01354> Length (m)= 164.82 40.00
01355> Mannings n = .030 .250
01357>
01358> Max.eff.Inten.(mm/hr)= 104.19 20.32
01359> over (min) 5.00 25.00
01360> Storage Coeff. (min)= 5.72 (ii) 24.02 (ii)
01361> Unit Hyd. Tpeak (min)= 5.72 25.00
01362> Unit Hyd. peak (cms)= .20 .05
01363>
01364> PEAK FLOW (cms)= .20 .00 *TOTALS*
01365> TIME TO PEAK (hrs)= 1.00 1.38 1.000 (iii)
01366> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
01367> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01368> RUNOFF COEFFICIENT = .96 .35 .945
01369>
01370> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01371> CN* = 81.0 Ia = Dep. Storage (Above)
01372> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01373> THAN THE STORAGE COEFFICIENT.
01374> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01375>
01376>
01377> 001:0010-----
01378> * SUB-AREA No.5
01379>
01380> CALIB STANDHYD | Area (ha)= 2.66
01381> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
01382>
01383> IMPERVIOUS PERVIOUS (i)
01384> Surface Area (ha)= 2.58 .09
01385> Dep. Storage (mm)= 1.57 4.67
01386> Average Slope (%)= .61 1.50
01387> Length (m)= 207.25 20.00
01388> Mannings n = .030 .250
01389>
01390> Max.eff.Inten.(mm/hr)= 104.19 24.26
01391> over (min) 7.50 17.50
01392> Storage Coeff. (min)= 7.45 (ii) 16.40 (ii)
01393> Unit Hyd. Tpeak (min)= 7.50 17.50
01394> Unit Hyd. peak (cms)= .15 .07
01395>
01396> PEAK FLOW (cms)= .54 .00 *TOTALS*
01397> TIME TO PEAK (hrs)= 1.04 1.25 1.042 (iii)
01398> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
01399> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01400> RUNOFF COEFFICIENT = .96 .35 .945
01401>
01402> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01403> CN* = 81.0 Ia = Dep. Storage (Above)
01404> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01405> THAN THE STORAGE COEFFICIENT.
01406> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01407>
01408>
01409> 001:0011-----
01410> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01411> | (ha) (cms) (hrs) (mm) (cms)
01412> ID1 06:060 .89 .205 1.00 40.16 .000
01413> +ID2 07:070 2.66 .538 1.04 40.16 .000
01414>
01415> SUM 08:080 3.55 .733 1.04 40.16 .000
01416>
01417> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01418>
01419>
01420> 001:0012-----
01421> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01422> | (ha) (cms) (hrs) (mm) (cms)
01423> ID1 05:050 5.01 .918 1.04 38.34 .000
01424> +ID2 08:080 3.55 .733 1.04 40.16 .000
01425>
01426> SUM 09:090 8.56 1.651 1.04 39.10 .000
01427>
01428> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01429>
01430>
01431> 001:0013-----
01432> ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01433> | IN>09: (090 ) |
01434> | OUT<10: (POND ) |
01435>
01436> ===== OUTFLOW STORAGE TABLE =====
01437> OUTFLOW STORAGE OUTFLOW STORAGE
01438> (cms) (ha.m.) (cms) (ha.m.)
01439> .000 .0000E+00 .593 .6251E+00
01440> .008 .6560E-01 .654 .6631E+00
01441> .017 .1311E+00 .797 .7391E+00
01442> .093 .2831E+00 .950 .8274E+00
01443> .233 .3971E+00 1.304 .9157E+00
01444> .337 .4731E+00 1.880 .1004E+01
01445> .465 .5491E+00 2.577 .1092E+01
01446> .531 .5871E+00 .000 .0000E+00
01447>
01448> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01449> (ha) (cms) (hrs) (mm)
01450> INFLOW>09: (POND ) 8.56 1.651 1.042 39.096
01451> OUTFLOW<10: (POND ) 8.56 .089 2.625 39.093
01452>
01453> PEAK FLOW REDUCTION [Qout/Qin] (%)= 5.413
01454> TIME SHIFT OF PEAK FLOW (min)= 95.00
01455> MAXIMUM STORAGE USED (ha.m.)=.2758E+00
01456>
01457>
01458> 001:0014-----
01459> * Remaining Hawthorne Industrial Park *
01460> * SUB-AREA No.1
01461>
01462> CALIB STANDHYD | Area (ha)= 19.90
01463> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01464>
01465> IMPERVIOUS PERVIOUS (i)
01466> Surface Area (ha)= 14.13 5.77
01467> Dep. Storage (mm)= .57 4.67
01468> Average Slope (%)= .60 1.50
01469> Length (m)= 580.00 100.00
01470> Mannings n = .030 .250
01471>
01472> Max.eff.Inten.(mm/hr)= 80.14 42.65
01473> over (min) 15.00 35.00
01474> Storage Coeff. (min)= 15.43 (ii) 34.18 (ii)
01475> Unit Hyd. Tpeak (min)= 15.00 35.00
01476> Unit Hyd. peak (cms)= .07 .03
01477>
01478> PEAK FLOW (cms)= 1.41 .40 *TOTALS*
01479> TIME TO PEAK (hrs)= 1.17 1.54 1.208 (iii)
01480> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01481>
01482>

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01486> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01487> RUNOFF COEFFICIENT = .96 .50 .732
01488>
01489> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01490> CN* = 81.0 Ia = Dep. Storage (Above)
01491> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01492> THAN THE STORAGE COEFFICIENT.
01493> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01494>
01495> 001:0015-----
01496> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01497> | (ha) (cms) (hrs) (mm) (cms)
01498> ID1 10:POND 8.56 .089 2.63 39.09 .000
01499> +ID2 01:HIP01 19.90 1.572 1.21 31.13 .000
01500>
01501> SUM 02:HIP02 28.46 1.615 1.21 33.52 .000
01502>
01503> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01504>
01505> 001:0016-----
01506> * SUB-AREA No.2
01507>
01508> CALIB STANDHYD | Area (ha)= 17.00
01509> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01510>
01511> IMPERVIOUS PERVIOUS (i)
01512> Surface Area (ha)= 12.07 4.93
01513> Dep. Storage (mm)= 1.57 4.67
01514> Average Slope (%)= .65 1.50
01515> Length (m)= 450.00 100.00
01516> Mannings n = .030 .250
01517>
01518> Max.eff.Inten.(mm/hr)= 89.76 47.48
01519> over (min) 12.50 30.00
01520> Storage Coeff. (min)= 12.36 (ii) 30.32 (ii)
01521> Unit Hyd. Tpeak (min)= 12.50 30.00
01522> Unit Hyd. peak (cms)= .09 .04
01523>
01524> PEAK FLOW (cms)= 1.36 .37 *TOTALS*
01525> TIME TO PEAK (hrs)= 1.13 1.46 1.167 (iii)
01526> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01527> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01528> RUNOFF COEFFICIENT = .96 .50 .732
01529>
01530> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01531> CN* = 81.0 Ia = Dep. Storage (Above)
01532> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01533> THAN THE STORAGE COEFFICIENT.
01534> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01535>
01536> 001:0017-----
01537> * SUB-AREA No.3
01538>
01539> CALIB STANDHYD | Area (ha)= 15.60
01540> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01541>
01542> IMPERVIOUS PERVIOUS (i)
01543> Surface Area (ha)= 11.08 4.52
01544> Dep. Storage (mm)= 1.57 4.67
01545> Average Slope (%)= .50 1.50
01546> Length (m)= 600.00 100.00
01547> Mannings n = .030 .250
01548>
01549> Max.eff.Inten.(mm/hr)= 73.27 42.65
01550> over (min) 17.50 35.00
01551> Storage Coeff. (min)= 17.24 (ii) 35.38 (ii)
01552> Unit Hyd. Tpeak (min)= 17.50 35.00
01553> Unit Hyd. peak (cms)= .07 .03
01554>
01555> PEAK FLOW (cms)= 1.03 .30 *TOTALS*
01556> TIME TO PEAK (hrs)= 1.21 1.54 1.176 (iii)
01557> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01558> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01559> RUNOFF COEFFICIENT = .96 .50 .732
01560>
01561> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01562> CN* = 81.0 Ia = Dep. Storage (Above)
01563> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01564> THAN THE STORAGE COEFFICIENT.
01565> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01566>
01567> 001:0018-----
01568> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01569> | (ha) (cms) (hrs) (mm) (cms)
01570> ID1 03:HIP03 17.00 1.504 1.17 31.13 .000
01571> +ID2 04:HIP04 15.60 1.176 1.25 31.13 .000
01572>
01573> SUM 05:HIP05 32.60 2.621 1.17 31.13 .000
01574>
01575> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01576>
01577> 001:0019-----
01578> | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01579> | (ha) (cms) (hrs) (mm) (cms)
01580> ID1 05:HIP05 32.60 2.621 1.17 31.13 .000
01581> +ID2 02:HIP02 28.46 1.615 1.21 33.52 .000
01582>
01583> SUM 06:HIP06 61.06 4.222 1.17 32.24 .000
01584>
01585> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01586>
01587> 001:0020-----
01588> * SUB-AREA No.4
01589>
01590> CALIB STANDHYD | Area (ha)= 12.20
01591> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01592>
01593> IMPERVIOUS PERVIOUS (i)
01594> Surface Area (ha)= 8.66 3.54
01595> Dep. Storage (mm)= 1.57 4.67
01596> Average Slope (%)= .70 1.50
01597> Length (m)= 210.00 100.00
01598> Mannings n = .030 .250
01599>
01600> Max.eff.Inten.(mm/hr)= 104.19 52.96
01601> over (min) 7.50 25.00
01602> Storage Coeff. (min)= 7.21 (ii) 24.40 (ii)
01603> Unit Hyd. Tpeak (min)= 7.50 25.00
01604> Unit Hyd. peak (cms)= .15 .05
01605>
01606> PEAK FLOW (cms)= 1.28 .31 *TOTALS*
01607> TIME TO PEAK (hrs)= 1.04 1.38 1.042 (iii)
01608> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
01609>
01610>

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01621> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01622> RUNOFF COEFFICIENT = .96 .50 .732
01623>
01624> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01625> CN* = 81.0 Ia = Dep. Storage (Above)
01626> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01627> THAN THE STORAGE COEFFICIENT.
01628> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01629>
01630>
01631> 001:0021-----
01632> *
01633> *SUB-AREA No.5
01634>
01635> | DESIGN NASHYD | Area (ha)= 4.09 Curve Number (CN)=85.00
01636> | 08:Pcond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01637> | U.H. Tp(hrs)= .170
01638>
01639> Unit Hyd Qpeak (cms)= .899
01640>
01641> PEAK FLOW (cms)= .260 (i)
01642> TIME TO PEAK (hrs)= 1.167
01643> RUNOFF VOLUME (mm)= 17.325
01644> TOTAL RAINFALL (mm)= 42.514
01645> RUNOFF COEFFICIENT = .408
01646>
01647> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01648>
01649>
01650> 001:0022-----
01651>
01652> | ADD HYD (HIP08 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01653> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01654> | IN-09: (HIP08 ) |
01655> | OUT<10: (HIP-PO) |
01656>
01657> ID1 06:HIP06 61.06 4.222 1.17 32.24 .000
01658> +ID2 07:HIP07 12.20 1.375 1.04 31.13 .000
01659> +ID3 08:Pcond-B 4.00 .260 1.17 17.32 .000
01660>
01661> SUM 09:HIP08 77.26 5.545 1.17 31.29 .000
01662>
01663> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01664>
01665> 001:0023-----
01666>
01667> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01668> | IN-09: (HIP08 ) |
01669> | OUT<10: (HIP-PO) |
01670>
01671> ===== OUTFLOW STORAGE TABLE =====
01672> OUTFLOW STORAGE OUTFLOW STORAGE
01673> (cms) (ha.m.) (cms) (ha.m.)
01674> .000 .0000E+00 .724 .2210E+01
01675> .048 .3740E+01 .937 .2501E+01
01676> .054 .2434E+00 1.262 .2798E+01
01677> .059 .5834E+00 1.404 .3101E+01
01678> .062 .8400E+00 1.532 .3410E+01
01679> .064 .1102E+01 1.650 .3724E+01
01680> .147 .1370E+01 2.409 .4044E+01
01681> .280 .1644E+01 3.689 .4370E+01
01682> .472 .1924E+01 .000 .0000E+00
01683>
01684> ROUTING RESULTS AREA OPEAK TPEAK R.V.
01685> (ha) (cms) (hrs) (mm)
01686> INFLOW>09: (HIP08 ) 77.26 5.545 1.167 31.292
01687> OUTFLOW<10: (HIP-PO) 77.26 .435 3.389 31.292
01688>
01689> PEAK FLOW REDUCTION [Qout/qin] (%) = 7.850
01690> TIME SHIFT OF PEAK FLOW (min) = 133.33
01691> MAXIMUM STORAGE USED (ha.m.)=.1871E+01
01692>
01693> 001:0024-----
01694> *
01695> *SUB-AREA No. 6
01696>
01697> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
01698> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01699> | U.H. Tp(hrs)= .800
01700>
01701> Unit Hyd Qpeak (cms)= .129
01702>
01703> PEAK FLOW (cms)= .044 (i)
01704> TIME TO PEAK (hrs)= 2.042
01705> RUNOFF VOLUME (mm)= 12.131
01706> TOTAL RAINFALL (mm)= 42.514
01707> RUNOFF COEFFICIENT = .285
01708>
01709> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01710>
01711> 001:0025-----
01712>
01713> | ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01714> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01715> | IN-10:HIP-PO |
01716> | +ID2 01:A3 |
01717>
01718> ID1 10:HIP-PO 77.26 .435 3.39 31.29 .000
01719> +ID2 01:A3 2.70 .044 2.04 12.13 .000
01720>
01721> SUM 02:Ultima 79.96 .457 3.29 30.65 .000
01722>
01723> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01724>
01725> 001:0026-----
01726> *
01727> * CALCULATION OF 3HR = 1:10 YEAR STORM EVENT
01728>
01729> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01730> | TZERO = .00 hrs on Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01731> | METOUT= 2 (output = METRIC)
01732> | NRUN = 001
01733> | NSTORM= 0
01734>
01735> 001:0002-----
01736>
01737> | CHICAGO STORM | IDF curve parameters: A=1174.184
01738> | Ptotal= 49.50 mm | B= 6.414
01739> | C= .816
01740>
01741> used in: INTENSITY = A / (t + B)^C
01742>
01743> Duration of storm = 3.00 hrs
01744> Storm time step = 10.00 min
01745> Time to peak ratio = .33
01746>
01747> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
01748> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
01749> .17 4.248 1.00 122.142 1.83 7.733 2.67 4.049
01750> .33 5.290 1.17 37.288 2.00 6.502 2.83 3.714
01751> .50 7.108 1.33 18.954 2.17 5.625 3.00 3.434
01752> .67 11.130 1.50 12.700 2.33 4.969
01753> .83 28.100 1.67 9.588 2.50 4.458
01754>
01755> 001:0003-----
01756>
01757> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL

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01756> ICASEdv = 1 (read and print data)
01757> FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
01758> ----- PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
01759> Horton's infiltration equation parameters:
01760> [Fw= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01761> Parameters for PERVIOUS surfaces in STANDHYD:
01762> [IAPER= 4.67 mm] [LGP=40.00 m] [MNI= .250]
01763> Parameters for IMPERVIOUS surfaces in STANDHYD:
01764> [IAlmp= 1.57 mm] [CLI= 1.50] [MNI= .035]
01765> Parameters used in NASHYD:
01766> [Ia= 4.67 mm] [N= 3.00]
01767>
01768> 001:0004-----
01769> *
01770> * ORGAWORLD FILE
01771> *
01772> *
01773> * SUB-AREA No.1
01774>
01775> | CALIB STANDHYD | Area (ha)= 2.07 Dir. Conn. (%) = 84.00
01776> | 01:010 DT= 2.50 | Total Imp (%) =
01777>
01778> IMPERVIOUS PERVIOUS (i)
01779> Surface Area (ha)= 1.74 .33
01780> Dep. Storage (mm)= 1.57 4.67
01781> Average Slope (%) = 5.2 1.00
01782> Length (m) = 204.72 20.00
01783> Mannings n = .030 .250
01784>
01785> Max.eff.Inten.(mm/hr)= 122.14 34.69
01786> over (min) 7.50 15.00
01787> Storage Coeff. (min)= 7.28 (ii) 16.04 (ii)
01788> Unit Hyd. Tpeak (min)= 7.50 15.00
01789> Unit Hyd. peak (cms)= .15 .07
01790>
01791> PEAK FLOW (cms)= .43 .02 *TOTALS*
01792> TIME TO PEAK (hrs)= 1.04 1.21 .437 (iii)
01793> RUNOFF VOLUME (mm)= 47.93 19.25 43.345
01794> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01795> RUNOFF COEFFICIENT = .97 .39 .876
01796>
01797> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01798> CN* = 81.0 Ia = Dep. Storage (Above)
01799> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01800> THAN THE STORAGE COEFFICIENT.
01801> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01802>
01803> 001:0005-----
01804> *
01805> * SUB-AREA No.2
01806>
01807> | CALIB STANDHYD | Area (ha)= 1.54 Dir. Conn. (%) = 92.00
01808> | 02:020 DT= 2.50 | Total Imp (%) =
01809>
01810> IMPERVIOUS PERVIOUS (i)
01811> Surface Area (ha)= 1.42 .12
01812> Dep. Storage (mm)= 1.57 4.67
01813> Average Slope (%) = .50 1.00
01814> Length (m) = 244.34 5.00
01815> Mannings n = .030 .030
01816>
01817> Max.eff.Inten.(mm/hr)= 122.14 42.32
01818> over (min) 7.50 10.00
01819> Storage Coeff. (min)= 8.20 (ii) 9.18 (ii)
01820> Unit Hyd. Tpeak (min)= 7.50 10.00
01821> Unit Hyd. peak (cms)= .14 .12
01822>
01823> PEAK FLOW (cms)= .33 .01 *TOTALS*
01824> TIME TO PEAK (hrs)= 1.04 1.13 .341 (iii)
01825> RUNOFF VOLUME (mm)= 47.93 19.25 45.640
01826> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01827> RUNOFF COEFFICIENT = .97 .39 .922
01828>
01829> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01830> CN* = 81.0 Ia = Dep. Storage (Above)
01831> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01832> THAN THE STORAGE COEFFICIENT.
01833> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01834>
01835> 001:0006-----
01836> *
01837> * SUB-AREA No.3
01838>
01839> | CALIB STANDHYD | Area (ha)= 1.40 Dir. Conn. (%) = 97.00
01840> | 03:030 DT= 2.50 | Total Imp (%) =
01841>
01842> IMPERVIOUS PERVIOUS (i)
01843> Surface Area (ha)= 1.36 .04
01844> Dep. Storage (mm)= 1.57 4.67
01845> Average Slope (%) = .53 1.00
01846> Length (m) = 225.63 5.00
01847> Mannings n = .030 .030
01848>
01849> Max.eff.Inten.(mm/hr)= 122.14 48.18
01850> over (min) 7.50 7.50
01851> Storage Coeff. (min)= 7.77 (ii) 8.70 (ii)
01852> Unit Hyd. Tpeak (min)= 7.50 7.50
01853> Unit Hyd. peak (cms)= .15 .14
01854>
01855> PEAK FLOW (cms)= .33 .00 *TOTALS*
01856> TIME TO PEAK (hrs)= 1.04 1.08 .329 (iii)
01857> RUNOFF VOLUME (mm)= 47.93 19.25 47.074
01858> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01859> RUNOFF COEFFICIENT = .97 .39 .951
01860>
01861> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01862> CN* = 81.0 Ia = Dep. Storage (Above)
01863> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01864> THAN THE STORAGE COEFFICIENT.
01865> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01866>
01867> 001:0007-----
01868> *
01869> *
01870> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01871> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01872> | IN-01:010 |
01873> | +ID2 02:020 |
01874>
01875> ID1 01:010 2.07 .437 1.04 43.35 .000
01876> +ID2 02:020 1.54 .341 1.04 45.64 .000
01877>
01878> SUM 04:040 3.61 .778 1.04 44.32 .000
01879>
01880> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01881>
01882> 001:0008-----
01883>
01884> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01885> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01886> | IN-03:030 |
01887> | +ID2 04:040 |
01888>
01889> ID1 03:030 1.40 .329 1.04 47.07 .000
01890> +ID2 04:040 3.61 .778 1.04 44.32 .000
01891>
01892> SUM 05:050 5.01 1.107 1.04 45.09 .000
01893>

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02161> Max.eff.inten.(mm/hr)= 122.14 72.53
02162> over (min)= 7.50 22.50
02163> Storage Coeff. (min)= 6.77 (ii) 21.93 (ii)
02164> Unit Hyd. Tpeak (min)= 7.50 22.50
02165> Unit Hyd. peak (cms)= .16 .05
02166>
02167> *TOTALS*
02168> PEAK FLOW (cms)= 1.54 .42 1.687 (iii)
02169> TIME TO PEAK (hrs)= 1.04 1.33 1.042
02170> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
02171> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
02172> RUNOFF COEFFICIENT = .97 .54 .756
02173>
02174> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02175> CN* = 81.0 Ia = Dep. Storage (Above)
02176> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02177> THAN THE STORAGE COEFFICIENT.
02178> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02179>
02180> 001:0021-----
02181> *SUB-AREA No.5
02182>
02183> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
02184> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
02185> | U.H. Tp(hrs)= .170
02186>
02187> Unit Hyd Qpeak (cms)= .899
02188>
02189> PEAK FLOW (cms)= .345 (i)
02190> TIME TO PEAK (hrs)= 1.167
02191> RUNOFF VOLUME (mm)= 22.420
02192> TOTAL RAINFALL (mm)= 49.505
02193> RUNOFF COEFFICIENT = .453
02194>
02195> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02196>
02197> 001:0022-----
02198>
02199> | ADD HYD (HIP08 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02200> | 05:HIP06 | (ha) (cms) (hrs) (mm) (cms)
02201> +ID2 07:HIP07 61.06 5.358 1.17 38.61 .000
02202> +ID3 08:Pond-B 4.00 1.687 1.04 37.43 .000
02203> +ID3 08:Pond-B 4.00 .345 1.17 22.42 .000
02204>
02205> SUM 09:HIP08 77.26 7.016 1.17 37.59 .000
02206>
02207> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02208>
02209> 001:0023-----
02210>
02211> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
02212> | IN:09: (HIP08 ) |
02213> | OUT:10: (HIP-PO) |
02214>
02215> ===== OUTFLOW STORAGE TABLE =====
02216> OUTFLOW STORAGE OUTFLOW STORAGE
02217> (cms) (ha.m.) (cms) (ha.m.)
02218> .000 .000E+00 724 .2210E+01
02219> .048 .5740E-01 .937 .2501E+01
02220> .054 .2434E+00 1.262 .2798E+01
02221> .059 .5830E+00 1.404 .3101E+01
02222> .062 .8400E+00 1.532 .3410E+01
02223> .064 .1102E+01 1.650 .3724E+01
02224> .147 .1370E+01 2.409 .4044E+01
02225> .280 .1644E+01 3.689 .4370E+01
02226> .472 .1924E+01 .000 .0000E+00
02227>
02228> ROUTING RESULTS AREA QPEAK TPEAK R.V.
02229> (ha) (cms) (hrs) (mm)
02230> INFLOW>09: (HIP08 ) 77.26 7.016 1.167 37.588
02231> OUTFLOW<10: (HIP-PO) 77.26 .696 3.208 37.588
02232>
02233> PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.919
02234> TIME SHIFT OF PEAK FLOW (min) = 122.50
02235> MAXIMUM STORAGE USED (ha.m.) = .2178E+01
02236>
02237> 001:0024-----
02238>
02239> *SUB-AREA No. 6
02240>
02241> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
02242> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
02243> | U.H. Tp(hrs)= .800
02244>
02245> Unit Hyd Qpeak (cms)= .129
02246>
02247> PEAK FLOW (cms)= .059 (i)
02248> TIME TO PEAK (hrs)= 2.000
02249> RUNOFF VOLUME (mm)= 16.075
02250> TOTAL RAINFALL (mm)= 49.505
02251> RUNOFF COEFFICIENT = .325
02252>
02253> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02254>
02255> 001:0025-----
02256>
02257> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02258> | 10:HIP-PO | (ha) (cms) (hrs) (mm) (cms)
02259> +ID2 01:A3 77.26 .696 3.21 37.59 .000
02260> +ID2 01:A3 2.70 .059 2.00 16.08 .000
02261>
02262> SUM 02:Ultima 79.96 .729 3.15 36.86 .000
02263>
02264> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02265>
02266> 001:0026-----
02267>
02268> ***** CALCULATION OF 3HR - 1:25 YEAR STORM EVENT *****
02269>
02270> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
02271> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
02272> | METOUT= 2 (output = METRIC)
02273> | NRUN = 001
02274> | NSTORM= 0
02275>
02276> 001:0002-----
02277>
02278> | CHICAGO STORM | IDF curve parameters: A=1402.884
02279> | Ptotal= 58.23 mm | B= 6.018
02280> | C= .819
02281> used in: INTENSITY = A / (t + B)^C
02282>
02283> Duration of storm = 3.00 hrs
02284> Storm time step = 10.00 min
02285> Time to peak ratio = .33
02286>
02287> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
02288> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
02289>
02290>
02291>
02292>
02293>
02294>
02295>

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02296> .17 4.934 | 1.00 144.693 | 1.83 9.014 | 2.67 4.701
02297> .33 6.152 | 1.17 43.904 | 2.00 7.571 | 2.83 4.310
02298> .50 8.282 | 1.33 22.224 | 2.17 6.544 | 3.00 3.983
02299> .67 13.006 | 1.50 14.852 | 2.33 5.776 |
02300> .83 33.041 | 1.67 11.192 | 2.50 5.179 |
02301>
02302>
02303> 001:0003-----
02304>
02305> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
02306> | ICASEd= 1 (read and print data)
02307> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
02308> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
02309>
02310> Horton's infiltration equation parameters:
02311> [F= 50.00 mm/hr] [F= 7.50 mm/hr] [DCA= 2.00 /hr] [F= .00 mm]
02312> Parameters for PERVIOUS surfaces in STANDHYD:
02313> [IAPER= 4.67 mm] [LGP=40.00 m] [MNP=.250]
02314> Parameters for IMPERVIOUS surfaces in STANDHYD:
02315> [IAMP= 1.57 mm] [CL=1.50] [MNI=.035]
02316> Parameters used in NASHYD:
02317> [Ia= 4.67 mm] [N= 3.00]
02318>
02319> 001:0004-----
02320>
02321> ***** ORGAWORLD FILE *****
02322>
02323> * SUB-AREA No.1
02324>
02325> | CALIB STANDHYD | Area (ha)= 2.07
02326> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
02327>
02328> IMPERVIOUS PERVIOUS (i)
02329> Surface Area (ha)= 1.74 .33
02330> Dep. Storage (mm)= 1.57 4.67
02331> Average Slope (%)= .52 1.00
02332> Length (m)= 204.72 20.00
02333> Mannings n = .030 .250
02334>
02335> Max.eff.inten.(mm/hr)= 144.69 47.07
02336> over (min)= 7.50 15.00
02337> Storage Coeff. (min)= 6.81 (ii) 14.56 (ii)
02338> Unit Hyd. Tpeak (min)= 7.50 15.00
02339> Unit Hyd. peak (cms)= .16 .08
02340>
02341> PEAK FLOW (cms)= .52 .03 *TOTALS*
02342> TIME TO PEAK (hrs)= 1.04 1.21 1.532 (iii)
02343> RUNOFF VOLUME (mm)= 56.66 25.35 1.042
02344> TOTAL RAINFALL (mm)= 58.23 58.23 51.647
02345> RUNOFF COEFFICIENT = .97 .44 58.226
02346>
02347> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02348> CN* = 81.0 Ia = Dep. Storage (Above)
02349> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02350> THAN THE STORAGE COEFFICIENT.
02351> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02352>
02353> 001:0005-----
02354>
02355> * SUB-AREA No.2
02356>
02357> | CALIB STANDHYD | Area (ha)= 1.54
02358> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
02359>
02360> IMPERVIOUS PERVIOUS (i)
02361> Surface Area (ha)= 1.42 .12
02362> Dep. Storage (mm)= 1.57 4.67
02363> Average Slope (%)= .50 1.00
02364> Length (m)= 244.34 5.00
02365> Mannings n = .030 .030
02366>
02367> Max.eff.inten.(mm/hr)= 144.69 65.19
02368> over (min)= 7.50 7.50
02369> Storage Coeff. (min)= 7.66 (ii) 8.49 (ii)
02370> Unit Hyd. Tpeak (min)= 7.50 7.50
02371> Unit Hyd. peak (cms)= .15 .14
02372>
02373> PEAK FLOW (cms)= .40 .01 *TOTALS*
02374> TIME TO PEAK (hrs)= 1.04 1.08 1.042
02375> RUNOFF VOLUME (mm)= 56.66 25.35 54.152
02376> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02377> RUNOFF COEFFICIENT = .97 .44 .930
02378>
02379> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02380> CN* = 81.0 Ia = Dep. Storage (Above)
02381> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02382> THAN THE STORAGE COEFFICIENT.
02383> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02384>
02385> 001:0006-----
02386>
02387> * SUB-AREA No.3
02388>
02389> | CALIB STANDHYD | Area (ha)= 1.40
02390> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02391>
02392> IMPERVIOUS PERVIOUS (i)
02393> Surface Area (ha)= 1.36 .04
02394> Dep. Storage (mm)= 1.57 4.67
02395> Average Slope (%)= .51 1.00
02396> Length (m)= 225.63 5.00
02397> Mannings n = .030 .030
02398>
02399> Max.eff.inten.(mm/hr)= 144.69 65.19
02400> over (min)= 7.50 7.50
02401> Storage Coeff. (min)= 7.26 (ii) 8.09 (ii)
02402> Unit Hyd. Tpeak (min)= 7.50 7.50
02403> Unit Hyd. peak (cms)= .15 .14
02404>
02405> PEAK FLOW (cms)= .40 .00 *TOTALS*
02406> TIME TO PEAK (hrs)= 1.04 1.08 1.400 (iii)
02407> RUNOFF VOLUME (mm)= 56.66 25.35 55.717
02408> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02409> RUNOFF COEFFICIENT = .97 .44 .957
02410>
02411> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02412> CN* = 81.0 Ia = Dep. Storage (Above)
02413> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02414> THAN THE STORAGE COEFFICIENT.
02415> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02416>
02417> 001:0007-----
02418>
02419> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02420> | 01:010 | (ha) (cms) (hrs) (mm) (cms)
02421> +ID2 02:020 2.07 .532 1.04 51.65 .000
02422> +ID2 02:020 1.54 .418 1.04 54.15 .000
02423>
02424> SUM 04:040 3.61 .950 1.04 52.72 .000
02425>
02426> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02427>
02428>
02429>
02430>

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02431> 001:0008-----

02432> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02433> | ID1 03:030 | (ha) (cms) (hrs) (mm) (mm) (cms)

02434> | +ID2 04:040 | 1.40 .400 1.04 55.72 .000

02435> | SUM 05:050 | 3.61 .950 1.04 52.72 .000

02436> | | 5.01 1.350 1.04 53.55 .000

02437> | | | | | | |

02438> | | | | | | |

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02441> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02442> | | | | | | |

02443> | | | | | | |

02444> 001:0009-----

02445> \* SUB-AREA No.4

02446> | CALIB STANDHYD | Area (ha)= .89

02447> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02448> | | IMPERVIOUS PERVIOUS (i)

02449> | Surface Area (ha)= .86 .03

02450> | Dep. Storage (mm)= 1.57 4.67

02451> | Average Slope (%)= .93 .70

02452> | Length (m)= 164.82 40.00

02453> | Mannings n = .030 .250

02454> | Max. eff. Inten. (mm/hr)= 144.69 44.12

02455> | over (min)= 5.00 17.50

02456> | Storage Coeff. (min)= 5.02 (ii) 18.44 (ii)

02457> | Unit Hyd. Tpeak (min)= 5.00 17.50

02458> | Unit Hyd. peak (cms)= .22 .06

02459> | | | | | | |

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02463> | | | | | | |

02464> | PEAK FLOW (cms)= .30 .00 \*TOTALS\* (iii)

02465> | TIME TO PEAK (hrs)= 1.00 1.25

02466> | RUNOFF VOLUME (mm)= 56.66 25.35 55.717

02467> | TOTAL RAINFALL (mm)= 58.23 58.23 58.226

02468> | RUNOFF COEFFICIENT = .97 .44 .957

02469> | | | | | | |

02470> | | | | | | |

02471> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

02472> | CN\* = 81.0 Ia = Dep. Storage (Above)

02473> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

02474> | THAN THE STORAGE COEFFICIENT.

02475> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02476> | | | | | | |

02477> 001:0010-----

02478> \* SUB-AREA No.5

02479> | CALIB STANDHYD | Area (ha)= 2.66

02480> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02481> | | IMPERVIOUS PERVIOUS (i)

02482> | Surface Area (ha)= 2.58 .08

02483> | Dep. Storage (mm)= 1.57 4.67

02484> | Average Slope (%)= .61 1.50

02485> | Length (m)= 207.25 20.00

02486> | Mannings n = .030 .250

02487> | Max. eff. Inten. (mm/hr)= 144.69 51.33

02488> | over (min)= 7.50 12.50

02489> | Storage Coeff. (min)= 6.54 (ii) 13.16 (ii)

02490> | Unit Hyd. Tpeak (min)= 7.50 12.50

02491> | Unit Hyd. peak (cms)= .16 .09

02492> | | | | | | |

02493> | | | | | | |

02494> | | | | | | |

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02496> | | | | | | |

02497> | PEAK FLOW (cms)= .78 .01 \*TOTALS\* (iii)

02498> | TIME TO PEAK (hrs)= 1.04 1.17 1.042

02499> | RUNOFF VOLUME (mm)= 56.66 25.35 55.717

02500> | TOTAL RAINFALL (mm)= 58.23 58.23 58.226

02501> | RUNOFF COEFFICIENT = .97 .44 .957

02502> | | | | | | |

02503> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

02504> | CN\* = 81.0 Ia = Dep. Storage (Above)

02505> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

02506> | THAN THE STORAGE COEFFICIENT.

02507> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02508> | | | | | | |

02509> | | | | | | |

02510> 001:0011-----

02511> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02512> | ID1 06:060 | (ha) (cms) (hrs) (mm) (mm) (cms)

02513> | +ID2 07:070 | .89 .296 1.00 55.72 .000

02514> | SUM 08:080 | 2.66 .783 1.04 55.72 .000

02515> | | 3.55 1.060 1.04 55.72 .000

02516> | | | | | | |

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02521> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02522> 001:0012-----

02523> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02524> | ID1 05:050 | (ha) (cms) (hrs) (mm) (mm) (cms)

02525> | +ID2 08:080 | 5.01 1.350 1.04 55.72 .000

02526> | SUM 09:090 | 3.55 1.060 1.04 55.72 .000

02527> | | 8.56 2.410 1.04 54.45 .000

02528> | | | | | | |

02529> | | | | | | |

02530> | | | | | | |

02531> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02532> | | | | | | |

02533> | | | | | | |

02534> 001:0013-----

02535> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.

02536> | IN:09: (090 ) |

02537> | OUT<10: (POND ) |

02538> | | | | | | |

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02550> | ROUTING RESULTS | AREA QPEAK TPEAK R.V.

02551> | INFLOW >09: (090 ) | (ha) (cms) (hrs) (mm) (mm) (cms)

02552> | | 8.56 2.410 1.04 54.451

02553> | OUTFLOW<10: (POND ) | 8.56 .189 2.056 54.449

02554> | | | | | | |

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02559> | | | | | | |

02560> 001:0014-----

02561> | Remaining Hawthorne Industrial Park |

02562> | | | | | | |

02563> | | | | | | |

02564> | | | | | | |

02565> \* SUB-AREA No.1

02566> 001:0015-----

02567> | CALIB STANDHYD | Area (ha)= 19.90

02568> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

02569> | | IMPERVIOUS PERVIOUS (i)

02570> | Surface Area (ha)= 14.13 5.77

02571> | Dep. Storage (mm)= 1.57 4.67

02572> | Average Slope (%)= .60 1.50

02573> | Length (m)= 580.00 100.00

02574> | Mannings n = .030 .250

02575> | Max. eff. Inten. (mm/hr)= 124.54 81.98

02576> | over (min)= 12.50 27.50

02577> | Storage Coeff. (min)= 12.93 (ii) 27.37 (ii)

02578> | Unit Hyd. Tpeak (min)= 12.50 27.50

02579> | Unit Hyd. peak (cms)= .09 .04

02580> | | | | | | |

02581> | | | | | | |

02582> | | | | | | |

02583> | PEAK FLOW (cms)= 2.16 .77 \*TOTALS\* (iii)

02584> | TIME TO PEAK (hrs)= 1.13 1.42 1.167

02585> | RUNOFF VOLUME (mm)= 56.66 34.22 45.437

02586> | TOTAL RAINFALL (mm)= 58.23 58.23 58.226

02587> | RUNOFF COEFFICIENT = .97 .59 .780

02588> | | | | | | |

02589> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

02590> | CN\* = 81.0 Ia = Dep. Storage (Above)

02591> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

02592> | THAN THE STORAGE COEFFICIENT.

02593> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02594> | | | | | | |

02595> | | | | | | |

02596> 001:0016-----

02597> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02598> | ID1 10:POND | (ha) (cms) (hrs) (mm) (mm) (cms)

02599> | +ID2 01:HIP01 | 8.56 .189 2.06 54.45 .000

02600> | SUM 02:HIP02 | 19.90 2.548 1.17 45.44 .000

02601> | | 28.46 2.622 1.17 48.15 .000

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02612> | CALIB STANDHYD | Area (ha)= 17.00

02613> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

02614> | | IMPERVIOUS PERVIOUS (i)

02615> | Surface Area (ha)= 12.07 4.93

02616> | Dep. Storage (mm)= 1.57 4.67

02617> | Average Slope (%)= .65 1.50

02618> | Length (m)= 450.00 100.00

02619> | Mannings n = .030 .250

02620> | Max. eff. Inten. (mm/hr)= 144.69 87.13

02621> | over (min)= 10.00 25.00

02622> | Storage Coeff. (min)= 10.21 (ii) 24.30 (ii)

02623> | Unit Hyd. Tpeak (min)= 10.00 25.00

02624> | Unit Hyd. peak (cms)= .11 .05

02625> | | | | | | |

02626> | | | | | | |

02627> | | | | | | |

02628> | PEAK FLOW (cms)= 2.10 .71 \*TOTALS\* (iii)

02629> | TIME TO PEAK (hrs)= 1.08 1.38 1.125

02630> | RUNOFF VOLUME (mm)= 56.66 34.22 45.437

02631> | TOTAL RAINFALL (mm)= 58.23 58.23 58.226

02632> | RUNOFF COEFFICIENT = .97 .59 .780

02633> | | | | | | |

02634> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

02635> | CN\* = 81.0 Ia = Dep. Storage (Above)

02636> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

02637> | THAN THE STORAGE COEFFICIENT.

02638> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02639> | | | | | | |

02640> | | | | | | |

02641> 001:0017-----

02642> \* SUB-AREA No.3

02643> | CALIB STANDHYD | Area (ha)= 15.60

02644> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

02645> | | IMPERVIOUS PERVIOUS (i)

02646> | Surface Area (ha)= 11.08 4.52

02647> | Dep. Storage (mm)= 1.57 4.67

02648> | Average Slope (%)= .60 1.50

02649> | Length (m)= 600.00 100.00

02650> | Mannings n = .030 .250

02651> | Max. eff. Inten. (mm/hr)= 111.10 77.71

02652> | over (min)= 15.00 30.00

02653> | Storage Coeff. (min)= 14.59 (ii) 29.34 (ii)

02654> | Unit Hyd. Tpeak (min)= 15.00 30.00

02655> | Unit Hyd. peak (cms)= .08 .04

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02701> CALIB STANDHYD | Area (ha)= 12.20  
 02702> 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

02703> IMPERVIOUS PERVIOUS (i)  
 02704> Surface Area (ha)= 8.66 3.54  
 02705> Dep. Storage (mm)= 1.57 4.67  
 02706> Average Slope (%)= .70 1.50  
 02707> Length (m)= 210.00 100.00  
 02708> Mannings n = .030 .250

02709> Max. eff. Inten. (mm/hr)= 144.69 101.36  
 02710> over (min)= 7.50 20.00  
 02711> Storage Coeff. (min)= 6.32 (ii) 19.58 (ii)  
 02712> Unit Hyd. Tpeak (min)= 7.50 20.00  
 02713> Unit Hyd. peak (cms)= .17 .06

02714> PEAK FLOW (cms)= 1.86 .59 \*TOTALS\*  
 02715> TIME TO PEAK (hrs)= 1.04 1.29 1.042  
 02716> RUNOFF VOLUME (mm)= 56.66 34.22 45.437  
 02717> TOTAL RAINFALL (mm)= 58.23 58.23 58.226  
 02718> RUNOFF COEFFICIENT = .97 .59 .780

02719> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02720> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02721> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02722> THAN THE STORAGE COEFFICIENT.  
 02723> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02724> 001:0021-----  
 02725> \*SUB-AREA No.5

02726> DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 02727> 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02728> U.H. Tp(hrs)= .170

02729> Unit Hyd. Qpeak (cms)= .899  
 02730> PEAK FLOW (cms)= .459 (i)  
 02731> TIME TO PEAK (hrs)= 1.167  
 02732> RUNOFF VOLUME (mm)= 29.155  
 02733> TOTAL RAINFALL (mm)= 58.226  
 02734> RUNOFF COEFFICIENT = .501

02735> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02736> 001:0022-----  
 02737> ADD HYD (HIP08) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 02738> ID1 06:HIP06 61.06 6.741 1.17 46.70 .000  
 02739> +ID2 07:HIP07 12.20 2.109 1.04 45.44 .000  
 02740> +ID3 08:Pond-B 4.00 .459 1.17 29.15 .000  
 02741> SUM 09:HIP08 77.26 8.998 1.13 45.59 .000

02742> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02743> 001:0023-----  
 02744> ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 02745> IN-09: (HIP08) |  
 02746> OUT-10: (HIP-PO) |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.00	0.000E+00	.724	.2210E+01
.048	.5740E+01	1.937	.2501E+01
.054	.2434E+00	1.262	.2798E+01
.059	.5834E+00	1.404	.3101E+01
.062	.8400E+00	1.532	.3410E+01
.064	.1102E+01	1.650	.3724E+01
.147	.1370E+01	2.409	.4048E+01
.280	.1644E+01	3.689	.4370E+01
.472	.1924E+01	.000	.0000E+00

02747> ROUTING RESULTS AREA OPEAK TPEAK R.V.  
 02748> (ha) (cms) (hrs) (mm)  
 02749> INFLOW>09: (HIP08) 77.26 8.998 1.125 45.591  
 02750> OUTFLOW>10: (HIP-PO) 77.26 1.004 3.083 45.591

02751> PEAK FLOW REDUCTION [Qout/Qin] (%)= 11.160  
 02752> TIME SHIFT OF PEAK FLOW (min)= 117.50  
 02753> MAXIMUM STORAGE USED (ha.m.)= .2562E+01

02754> 001:0024-----  
 02755> \*SUB-AREA No. 6

02756> DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 02757> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02758> U.H. Tp(hrs)= .800

02759> Unit Hyd. Qpeak (cms)= .129  
 02760> PEAK FLOW (cms)= .079 (i)  
 02761> TIME TO PEAK (hrs)= 2.000  
 02762> RUNOFF VOLUME (mm)= 21.442  
 02763> TOTAL RAINFALL (mm)= 58.226  
 02764> RUNOFF COEFFICIENT = .368

02765> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02766> 001:0025-----  
 02767> ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF  
 02768> ID1 10:HIP-PO 77.26 1.004 3.08 45.59 .000  
 02769> +ID2 01:A3 2.70 .079 2.00 21.44 .000  
 02770> SUM 02:Ultima 79.96 1.051 3.01 44.78 .000

02771> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 02772> 001:0026-----  
 02773> \*SUB-AREA No. 7

02774> DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 02775> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02776> U.H. Tp(hrs)= .800

02777> Unit Hyd. Qpeak (cms)= .129  
 02778> PEAK FLOW (cms)= .079 (i)  
 02779> TIME TO PEAK (hrs)= 2.000  
 02780> RUNOFF VOLUME (mm)= 21.442  
 02781> TOTAL RAINFALL (mm)= 58.226  
 02782> RUNOFF COEFFICIENT = .368

02783> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02784> 001:0027-----  
 02785> \*SUB-AREA No. 8

02786> DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 02787> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02788> U.H. Tp(hrs)= .800

02789> Unit Hyd. Qpeak (cms)= .129  
 02790> PEAK FLOW (cms)= .079 (i)  
 02791> TIME TO PEAK (hrs)= 2.000  
 02792> RUNOFF VOLUME (mm)= 21.442  
 02793> TOTAL RAINFALL (mm)= 58.226  
 02794> RUNOFF COEFFICIENT = .368

02795> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02796> 001:0028-----  
 02797> \*SUB-AREA No. 9

02798> DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 02799> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02800> U.H. Tp(hrs)= .800

02801> Unit Hyd. Qpeak (cms)= .129  
 02802> PEAK FLOW (cms)= .079 (i)  
 02803> TIME TO PEAK (hrs)= 2.000  
 02804> RUNOFF VOLUME (mm)= 21.442  
 02805> TOTAL RAINFALL (mm)= 58.226  
 02806> RUNOFF COEFFICIENT = .368

02807> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02808> 001:0029-----  
 02809> \*SUB-AREA No. 10

02810> DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 02811> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 02812> U.H. Tp(hrs)= .800

02813> Unit Hyd. Qpeak (cms)= .129  
 02814> PEAK FLOW (cms)= .079 (i)  
 02815> TIME TO PEAK (hrs)= 2.000  
 02816> RUNOFF VOLUME (mm)= 21.442  
 02817> TOTAL RAINFALL (mm)= 58.226  
 02818> RUNOFF COEFFICIENT = .368

02819> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 02820> 001:0030-----  
 02821> \*SUB-AREA No. 11

02836> | Total= 64.81 mm | B= 6.014  
 02837> C= 820  
 02838> used in: INTENSITY = A / (t + B)^C

02839> Duration of storm = 3.00 hrs  
 02840> Storm time step = 10.00 min  
 02841> Time to peak ratio = .33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
.17	5.467	1.00	161.471	1.83	10.000	2.67	5.209
.33	6.820	1.17	48.876	2.00	8.397	2.83	4.774
.50	9.187	1.33	24.704	2.17	7.256	3.00	4.412
.67	14.441	1.50	16.495	2.33	6.403		
.83	36.764	1.67	12.422	2.50	5.740		

02842> 001:0003-----  
 02843> \*SUB-AREA No. 1

02844> DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORCA.VAL  
 02845> ICASRev = 1 (read and print data)  
 02846> FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---  
 02847> PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----

02848> Horton's infiltration equation parameters:  
 02849> [P= 50.00 mm/hr] [Pc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]  
 02850> Parameters for PERVIOUS surfaces in STANDHYD:  
 02851> [Iaper= 4.67 mm] [LGP=40.00 mm] [MNI= .250]  
 02852> Parameters for IMPERVIOUS surfaces in STANDHYD:  
 02853> [Iaimp= 1.57 mm] [CLI= 1.50] [MNI= .035]  
 02854> Parameters used in NASHYD:  
 02855> [Ia= 4.67 mm] [N= 3.00]

02856> 001:0004-----  
 02857> \*SUB-AREA No. 2

02858> CALIB STANDHYD | Area (ha)= 2.07  
 02859> 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

02860> IMPERVIOUS PERVIOUS (i)  
 02861> Surface Area (ha)= 1.74 .33  
 02862> Dep. Storage (mm)= 1.57 4.67  
 02863> Average Slope (%)= .52 1.00  
 02864> Length (m)= 204.72 20.00  
 02865> Mannings n = .030 .250

02866> Max. eff. Inten. (mm/hr)= 161.47 62.27  
 02867> over (min)= 7.50 12.50  
 02868> Storage Coeff. (min)= 6.51 (ii) 13.44 (ii)  
 02869> Unit Hyd. Tpeak (min)= 7.50 12.50  
 02870> Unit Hyd. peak (cms)= .16 .09

02871> PEAK FLOW (cms)= .59 .03 \*TOTALS\*  
 02872> TIME TO PEAK (hrs)= 1.04 1.17 1.609 (iii)  
 02873> RUNOFF VOLUME (mm)= 63.24 30.21 57.952  
 02874> TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02875> RUNOFF COEFFICIENT = .98 .47 .894

02876> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02877> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02878> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02879> THAN THE STORAGE COEFFICIENT.  
 02880> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02881> 001:0005-----  
 02882> \*SUB-AREA No. 3

02883> CALIB STANDHYD | Area (ha)= 1.54  
 02884> 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00

02885> IMPERVIOUS PERVIOUS (i)  
 02886> Surface Area (ha)= 1.42 .12  
 02887> Dep. Storage (mm)= 1.57 4.67  
 02888> Average Slope (%)= .50 1.00  
 02889> Length (m)= 244.34 5.00  
 02890> Mannings n = .030 .030

02891> Max. eff. Inten. (mm/hr)= 161.47 78.73  
 02892> over (min)= 7.50 7.50  
 02893> Storage Coeff. (min)= 7.33 (ii) 8.10 (ii)  
 02894> Unit Hyd. Tpeak (min)= 7.50 7.50  
 02895> Unit Hyd. peak (cms)= .15 .14

02896> PEAK FLOW (cms)= .46 .02 \*TOTALS\*  
 02897> TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 02898> RUNOFF VOLUME (mm)= 63.24 30.21 60.594  
 02899> TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02900> RUNOFF COEFFICIENT = .98 .47 .935

02901> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02902> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02903> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02904> THAN THE STORAGE COEFFICIENT.  
 02905> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02906> 001:0006-----  
 02907> \*SUB-AREA No. 4

02908> CALIB STANDHYD | Area (ha)= 1.40  
 02909> 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02910> IMPERVIOUS PERVIOUS (i)  
 02911> Surface Area (ha)= 1.36 .04  
 02912> Dep. Storage (mm)= 1.57 4.67  
 02913> Average Slope (%)= .51 1.00  
 02914> Length (m)= 225.63 5.00  
 02915> Mannings n = .030 .030

02916> Max. eff. Inten. (mm/hr)= 161.47 78.73  
 02917> over (min)= 7.50 7.50  
 02918> Storage Coeff. (min)= 6.95 (ii) 7.72 (ii)  
 02919> Unit Hyd. Tpeak (min)= 7.50 7.50  
 02920> Unit Hyd. peak (cms)= .16 .15

02921> PEAK FLOW (cms)= .45 .01 \*TOTALS\*  
 02922> TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 02923> RUNOFF VOLUME (mm)= 63.24 30.21 62.245  
 02924> TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 02925> RUNOFF COEFFICIENT = .98 .47 .960

02926> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 02927> CN\* = 81.0 Ia = Dep. Storage (Above)  
 02928> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 02929> THAN THE STORAGE COEFFICIENT.  
 02930> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02931> 001:0007-----  
 02932> \*SUB-AREA No. 5

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02971 | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02972 | ID1 03:010 (ha) (cms) (hrs) (mm) (cms)
02973 | +ID2 02:020 1.54 .475 1.04 60.59 .000
02974 | SUM 04:040 3.61 1.084 1.04 59.08 .000

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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02981 | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02982 | ID1 03:030 (ha) (cms) (hrs) (mm) (cms)
02983 | +ID2 04:040 3.61 1.084 1.04 59.08 .000
02984 | SUM 05:050 5.01 1.538 1.04 59.96 .000

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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02991 | SUB-AREA No. 4
02992 | CALIB STANDHYD | Area (ha)= .89 Dir. Conn.(%)= 97.00
02993 | 06:060 DT= 2.50 | Total Imp(%)= 97.00

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03001 | IMPERVIOUS PERVIOUS (i)
03002 | Surface Area (ha)= .86 .03
03003 | Dep. Storage (mm)= 1.57 4.67
03004 | Average Slope (ft)= .93 .70
03005 | Length (m)= 164.82 42.00
03006 | Mannings n = .030 .250
03007 |
03008 | Max.eff.Inten.(mm/hr)= 161.47 53.28
03009 | over (min)= 5.00 17.50
03010 | Storage Coeff. (min)= 4.80 (ii) 17.24 (ii)
03011 | Unit Hyd. Tpeak (min)= 5.00 17.50
03012 | Unit Hyd. peak (cms)= .23 .07
03013 |
03014 | PEAK FLOW (cms)= .33 .00 *TOTALS*
03015 | TIME TO PEAK (hrs)= 1.00 1.25
03016 | RUNOFF VOLUME (mm)= 63.24 30.21 62.245
03017 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03018 | RUNOFF COEFFICIENT = .98 .47 .960

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(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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03021 | SUB-AREA No. 5
03022 | CALIB STANDHYD | Area (ha)= 2.66 Dir. Conn.(%)= 97.00
03023 | 07:070 DT= 2.50 | Total Imp(%)= 97.00

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03031 | IMPERVIOUS PERVIOUS (i)
03032 | Surface Area (ha)= 2.58 .08
03033 | Dep. Storage (mm)= 1.57 4.67
03034 | Average Slope (ft)= .61 1.50
03035 | Length (m)= 207.25 20.00
03036 | Mannings n = .030 .250
03037 |
03038 | Max.eff.Inten.(mm/hr)= 161.47 62.27
03039 | over (min)= 7.50 12.50
03040 | Storage Coeff. (min)= 6.26 (ii) 12.39 (ii)
03041 | Unit Hyd. Tpeak (min)= 7.50 12.50
03042 | Unit Hyd. peak (cms)= .17 .09
03043 |
03044 | PEAK FLOW (cms)= .88 .01 *TOTALS*
03045 | TIME TO PEAK (hrs)= 1.04 1.17 1.042
03046 | RUNOFF VOLUME (mm)= 63.24 30.21 62.245
03047 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03048 | RUNOFF COEFFICIENT = .98 .47 .960

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(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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03051 | SUB-AREA No. 1
03052 | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03053 | ID1 06:060 (ha) (cms) (hrs) (mm) (cms)
03054 | +ID2 07:070 2.66 .886 1.04 62.25 .000
03055 | SUM 08:080 3.55 1.197 1.04 62.25 .000

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03061 | SUB-AREA No. 2
03062 | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03063 | ID1 05:050 (ha) (cms) (hrs) (mm) (cms)
03064 | +ID2 08:080 3.55 1.197 1.04 62.25 .000
03065 | SUM 09:090 8.56 2.735 1.04 60.91 .000

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03071 | SUB-AREA No. 3
03072 | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03073 | IN>09: (090 ) |
03074 | OUT<10: (POND ) |

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03081 | OUTFLOW STORAGE TABLE
03082 | OUTFLOW STORAGE (cms) (ha.m.) | OUTFLOW STORAGE (cms) (ha.m.)
03083 | 0.00 0.000E+00 | 593 6251E+00
03084 | 0.08 6560E-01 | .654 6631E+00
03085 | 0.17 1311E+00 | .797 7391E+00
03086 | 0.26 2831E+00 | .950 8274E+00
03087 | 0.35 3971E+00 | 1.304 9157E+00
03088 | 0.44 4731E+00 | 1.880 1004E+01
03089 | 0.53 5491E+00 | 2.577 1092E+01
03090 | 0.62 5871E+00 | .000 0.000E+00

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03101 | ROUTING RESULTS AREA QPEAK TPEAK R.V.
03102 | INFLOW >09: (090 ) 8.56 2.735 1.042 60.910
03103 | OUTFLOW <10: (POND ) 8.56 .233 1.944 60.908
03104 |
03105 | PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.503

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TIME SHIFT OF PEAK FLOW (min)= 54.17  
 MAXIMUM STORAGE USED (ha.m.)= .3967E+00

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03106 | SUB-AREA No. 1
03107 | CALIB STANDHYD | Area (ha)= 19.90 Dir. Conn.(%)= 50.00
03108 | 01:HIP01 DT= 2.50 | Total Imp(%)= 50.00

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03111 | IMPERVIOUS PERVIOUS (i)
03112 | Surface Area (ha)= 14.13 5.77
03113 | Dep. Storage (mm)= 1.57 4.67
03114 | Average Slope (ft)= .60 1.50
03115 | Length (m)= 580.00 100.00
03116 | Mannings n = .030 .250
03117 |
03118 | Max.eff.Inten.(mm/hr)= 138.95 102.13
03119 | over (min)= 12.50 25.00
03120 | Storage Coeff. (min)= 12.38 (ii) 25.60 (ii)
03121 | Unit Hyd. Tpeak (min)= 12.50 25.00
03122 | Unit Hyd. peak (cms)= .09 .04
03123 |
03124 | PEAK FLOW (cms)= 2.46 .95 *TOTALS*
03125 | TIME TO PEAK (hrs)= 1.13 1.38 3.001 (iii)
03126 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03127 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03128 | RUNOFF COEFFICIENT = .98 .62 .796

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(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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03131 | SUB-AREA No. 2
03132 | CALIB STANDHYD | Area (ha)= 17.00 Dir. Conn.(%)= 50.00
03133 | 03:HIP03 DT= 2.50 | Total Imp(%)= 50.00

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03141 | IMPERVIOUS PERVIOUS (i)
03142 | Surface Area (ha)= 12.07 4.93
03143 | Dep. Storage (mm)= 1.57 4.67
03144 | Average Slope (ft)= .65 1.50
03145 | Length (m)= 450.00 100.00
03146 | Mannings n = .030 .250
03147 |
03148 | Max.eff.Inten.(mm/hr)= 161.47 109.61
03149 | over (min)= 10.00 22.50
03150 | Storage Coeff. (min)= 9.77 (ii) 22.63 (ii)
03151 | Unit Hyd. Tpeak (min)= 10.00 22.50
03152 | Unit Hyd. peak (cms)= .11 .05
03153 |
03154 | PEAK FLOW (cms)= 2.38 .88 *TOTALS*
03155 | TIME TO PEAK (hrs)= 1.08 1.33 2.819 (iii)
03156 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03157 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03158 | RUNOFF COEFFICIENT = .98 .62 .796

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

03161 | SUB-AREA No. 3
03162 | CALIB STANDHYD | Area (ha)= 15.60 Dir. Conn.(%)= 50.00
03163 | 04:HIP04 DT= 2.50 | Total Imp(%)= 50.00

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03171 | IMPERVIOUS PERVIOUS (i)
03172 | Surface Area (ha)= 11.08 4.52
03173 | Dep. Storage (mm)= 1.57 4.67
03174 | Average Slope (ft)= .50 1.50
03175 | Length (m)= 600.00 100.00
03176 | Mannings n = .030 .250
03177 |
03178 | Max.eff.Inten.(mm/hr)= 138.95 96.02
03179 | over (min)= 12.50 27.50
03180 | Storage Coeff. (min)= 13.34 (ii) 26.90 (ii)
03181 | Unit Hyd. Tpeak (min)= 12.50 27.50
03182 | Unit Hyd. peak (cms)= .09 .04
03183 |
03184 | PEAK FLOW (cms)= 1.86 .72 *TOTALS*
03185 | TIME TO PEAK (hrs)= 1.13 1.42 2.237 (iii)
03186 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03187 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03188 | RUNOFF COEFFICIENT = .98 .62 .796

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

03191 | SUB-AREA No. 4
03192 | CALIB STANDHYD | Area (ha)= 15.60 Dir. Conn.(%)= 50.00
03193 | 04:HIP04 DT= 2.50 | Total Imp(%)= 50.00

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03201 | IMPERVIOUS PERVIOUS (i)
03202 | Surface Area (ha)= 11.08 4.52
03203 | Dep. Storage (mm)= 1.57 4.67
03204 | Average Slope (ft)= .50 1.50
03205 | Length (m)= 600.00 100.00
03206 | Mannings n = .030 .250
03207 |
03208 | Max.eff.Inten.(mm/hr)= 138.95 96.02
03209 | over (min)= 12.50 27.50
03210 | Storage Coeff. (min)= 13.34 (ii) 26.90 (ii)
03211 | Unit Hyd. Tpeak (min)= 12.50 27.50
03212 | Unit Hyd. peak (cms)= .09 .04
03213 |
03214 | PEAK FLOW (cms)= 1.86 .72 *TOTALS*
03215 | TIME TO PEAK (hrs)= 1.13 1.42 2.237 (iii)
03216 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03217 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03218 | RUNOFF COEFFICIENT = .98 .62 .796

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 81.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

03241> +ID2 02:HIP02 28.46 3.092 1.17 54.37 .000  
 03242> SUM 06:HIP06 61.06 8.054 1.13 52.87 .000  
 03243>  
 03244>  
 03245> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03246>  
 03247>

03248> 001:0020-----  
 03249> \*  
 03250> \* SUB-AREA No. 4  
 03251>

03252> | CALIB STANDHYD | Area (ha)= 12.20  
 03253> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 03254>  
 03255>

03256> IMPERVIOUS PERVIOUS (i)  
 03257> Surface Area (ha)= 8.66 3.54  
 03258> Dep. Storage (mm)= 1.57 4.67  
 03259> Average Slope (%)= .70 1.50  
 03260> Length (m)= 210.00 100.00  
 03261> Mannings n = .030 .250

03262> Max. eff. Inten. (mm/hr)= 161.47 126.32  
 03263> over (min)= 5.00 17.50  
 03264> Storage Coeff. (min)= 6.05 (ii) 18.19 (ii)  
 03265> Unit Hyd. Tpeak (min)= 5.00 17.50  
 03266> Unit Hyd. peak (cms)= .20 .06

03267> \*TOTALS\*  
 03268> PEAK FLOW (cms)= 2.19 .73 2.470 (iii)  
 03269> TIME TO PEAK (hrs)= 1.00 1.042  
 03270> RUNOFF VOLUME (mm)= 63.24 39.90  
 03271> TOTAL RAINFALL (mm)= 64.81 64.81 64.806  
 03272> RUNOFF COEFFICIENT = .98 .62 .796

03273> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03274> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03275> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03276> THAN THE STORAGE COEFFICIENT.  
 03277> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03278>  
 03279>

03280> 001:0021-----  
 03281> \*  
 03282> \* SUB-AREA No. 5  
 03283>

03284> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00  
 03285> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 03286> U.H. Tp (hrs)= .170  
 03287>  
 03288>

03289> Unit Hyd Qpeak (cms)= .899  
 03290>  
 03291> PEAK FLOW (cms)= .551 (i)  
 03292> TIME TO PEAK (hrs)= 1.125  
 03293> RUNOFF VOLUME (mm)= 34.455  
 03294> TOTAL RAINFALL (mm)= 64.806  
 03295> RUNOFF COEFFICIENT = .532

03296> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03297>  
 03298>  
 03299>  
 03300> 001:0022-----

03301> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03302> (ha) (cms) (hrs) (mm) (cms)  
 03303> ID1 06:HIP06 61.06 8.054 1.13 52.87 .000  
 03304> +ID2 07:HIP07 12.20 2.470 1.04 51.57 .000  
 03305> +ID3 08:Pond-B 4.00 .551 1.13 34.45 .000  
 03306> SUM 09:HIP08 77.26 10.570 1.13 51.71 .000  
 03307>  
 03308>  
 03309>  
 03310> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03311>  
 03312>

03313> 001:0023-----  
 03314> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 03315> | IN>09: (HIP08) |  
 03316> | OUT<10: (HIP-PO) |

03317> ===== OUTFLOW STORAGE TABLE =====  
 03318> OUTFLOW STORAGE OUTFLOW STORAGE  
 03319> (cms) (ha.m.) (cms) (ha.m.)  
 03320> .000 .0000E+00 1.724 .2210E+01  
 03321> .048 .5740E-01 .937 .2501E+01  
 03322> .054 .2434E+00 1.262 .2798E+01  
 03323> .059 .5834E+00 1.404 .3101E+01  
 03324> .062 .8400E+00 1.532 .3410E+01  
 03325> .064 .1102E+01 1.650 .3724E+01  
 03326> .147 .1370E+01 2.409 .4044E+01  
 03327> .280 .1644E+01 3.689 .4370E+01  
 03328> .472 .1924E+01 1.000 .0000E+00

03329> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 03330> (ha) (cms) (hrs) (mm)  
 03331> INFLOW >09: (HIP08) 77.26 10.570 1.125 51.714  
 03332> OUTFLOW<10: (HIP-PO) 77.26 1.280 2.917 51.714  
 03333>  
 03334> PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.106  
 03335> TIME SHIFT OF PEAK FLOW (min) = 107.50  
 03336> MAXIMUM STORAGE USED (ha.m.) = .2836E+01  
 03337>  
 03338>

03339> 001:0024-----  
 03340> \*  
 03341> \* SUB-AREA No. 6  
 03342>

03343> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00  
 03344> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00  
 03345> U.H. Tp (hrs)= .800  
 03346>  
 03347>

03348> Unit Hyd Qpeak (cms)= .129  
 03349>  
 03350> PEAK FLOW (cms)= .096 (i)  
 03351> TIME TO PEAK (hrs)= 1.958  
 03352> RUNOFF VOLUME (mm)= 25.767  
 03353> TOTAL RAINFALL (mm)= 64.806  
 03354> RUNOFF COEFFICIENT = .398

03355> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03356>  
 03357>  
 03358>

03359> 001:0025-----  
 03360> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03361> (ha) (cms) (hrs) (mm) (cms)  
 03362> ID1 10:HIP-PO 77.26 1.280 2.92 51.71 .000  
 03363> +ID2 01:A3 2.70 .096 1.96 25.77 .000  
 03364> SUM 02:Ultima 79.96 1.348 2.63 50.84 .000  
 03365>  
 03366>  
 03367>

03368> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03369>  
 03370>  
 03371> 001:0026-----

03372> \*\*\*\*\*  
 03373> \* CALCULATION OF 3HR - 1:100 YEAR STORM EVENT \*  
 03374> \*\*\*\*\*  
 03375>

03376> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\  
 03377> Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\  
 03378> TEERO = .00 hrs on 0  
 03379> METOUT= 2 (output = METRIC)  
 03380> NRUM = 001  
 03381> NSTORM= 0  
 03382>  
 03383> 001:0002-----

03384> | CHICAGO STORM | IDF curve parameters: A=1735.688  
 03385> | Total= 17.66 mm | B= 6.014  
 03386> C= .820  
 03387> used in: INTENSITY = A / (t + B)^C  
 03388>  
 03389> Duration of storm = 3.00 hrs  
 03390> Storm time step = 10.00 min  
 03391> Time to peak ratio = .33

03392>  
 03393>  
 03394> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN  
 03395> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr  
 03396> .17 6.04E-01 1.00 178.559 1.83 11.059 1.27 5.700  
 03397> .33 7.542 1.17 54.049 2.00 9.285 2.83 5.280  
 03398> .50 10.159 1.33 27.319 2.17 8.024 3.00 4.879  
 03399> .67 15.969 1.50 18.240 2.33 7.080  
 03400> .83 40.655 1.67 13.737 2.50 6.347

03401>  
 03402> 001:0003-----  
 03403>  
 03404> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\ORGA.VAL  
 03405> | CASEDEV = (read and print data)  
 03406> FileTitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ----  
 03407> ----- PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----  
 03408>  
 03409> Horton's infiltration equation parameters:  
 03410> [Pw= 50.00 mm/hr] [Pc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]  
 03411> Parameters for PERVIOUS surfaces in STANDHYD:  
 03412> [IAPER= 4.67 mm] [LGP=40.00 m] [MNP= .250]  
 03413> Parameters for IMPERVIOUS surfaces in STANDHYD:  
 03414> [IAlmp= 1.57 mm] [CLI= 1.50] [MNI= .035]  
 03415> Parameters used in NASHYD:  
 03416> [Ia= 4.67 mm] [N= 3.00]  
 03417>

03418> 001:0004-----  
 03419> \*\*\*\*\*  
 03420> ORGAWORLD FILE \*  
 03421> \*\*\*\*\*  
 03422> \*

03423> \* SUB-AREA No. 1  
 03424>  
 03425> | CALIB STANDHYD | Area (ha)= 2.07  
 03426> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00  
 03427>  
 03428>

03429> IMPERVIOUS PERVIOUS (i)  
 03430> Surface Area (ha)= 1.74 .33  
 03431> Dep. Storage (mm)= 1.57 4.67  
 03432> Average Slope (%)= .52 1.00  
 03433> Length (m)= 204.72 20.00  
 03434> Mannings n = .030 .250

03435> Max. eff. Inten. (mm/hr)= 178.56 74.05  
 03436> over (min)= 7.50 12.50  
 03437> Storage Coeff. (min)= 6.26 (ii) 12.72 (ii)  
 03438> Unit Hyd. Tpeak (min)= 7.50 12.50  
 03439> Unit Hyd. peak (cms)= .17 .09

03440> \*TOTALS\*  
 03441> PEAK FLOW (cms)= .66 .04  
 03442> TIME TO PEAK (hrs)= 1.04 1.17 1.042  
 03443> RUNOFF VOLUME (mm)= 70.09 35.46 64.553  
 03444> TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03445> RUNOFF COEFFICIENT = .98 .49 .901

03446> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03447> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03448> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03449> THAN THE STORAGE COEFFICIENT.  
 03450> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03451>  
 03452>

03453> 001:0005-----  
 03454> \*  
 03455> \* SUB-AREA No. 2  
 03456>

03457> | CALIB STANDHYD | Area (ha)= 1.54  
 03458> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00  
 03459>  
 03460>

03461> IMPERVIOUS PERVIOUS (i)  
 03462> Surface Area (ha)= 1.42 .12  
 03463> Dep. Storage (mm)= 1.57 4.67  
 03464> Average Slope (%)= .50 1.00  
 03465> Length (m)= 244.34 5.00  
 03466> Mannings n = .030 .030

03467> Max. eff. Inten. (mm/hr)= 178.56 93.23  
 03468> over (min)= 7.50 7.50  
 03469> Storage Coeff. (min)= 7.04 (ii) 7.76 (ii)  
 03470> Unit Hyd. Tpeak (min)= 7.50 7.50  
 03471> Unit Hyd. peak (cms)= .16 .15

03472> \*TOTALS\*  
 03473> PEAK FLOW (cms)= .51 .02 .534 (iii)  
 03474> TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 03475> RUNOFF VOLUME (mm)= 70.09 35.46 67.324  
 03476> TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03477> RUNOFF COEFFICIENT = .98 .49 .939

03478> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03479> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03480> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03481> THAN THE STORAGE COEFFICIENT.  
 03482> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03483>  
 03484>

03485> 001:0006-----  
 03486> \*  
 03487> \* SUB-AREA No. 3  
 03488>

03489> | CALIB STANDHYD | Area (ha)= 1.40  
 03490> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 03491>  
 03492>

03493> IMPERVIOUS PERVIOUS (i)  
 03494> Surface Area (ha)= 1.36 .04  
 03495> Dep. Storage (mm)= 1.57 4.67  
 03496> Average Slope (%)= .51 1.00  
 03497> Length (m)= 225.63 5.00  
 03498> Mannings n = .030 .030

03499> Max. eff. Inten. (mm/hr)= 178.56 93.23  
 03500> over (min)= 7.50 7.50  
 03501> Storage Coeff. (min)= 6.67 (ii) 7.39 (ii)  
 03502> Unit Hyd. Tpeak (min)= 7.50 7.50  
 03503> Unit Hyd. peak (cms)= .16 .15

03504> \*TOTALS\*  
 03505> PEAK FLOW (cms)= .50 .01 .509 (iii)  
 03506> TIME TO PEAK (hrs)= 1.04 1.08 1.042  
 03507> RUNOFF VOLUME (mm)= 70.09 35.46 69.056  
 03508> TOTAL RAINFALL (mm)= 71.66 71.66 71.665

03511> RUNOFF COEFFICIENT = .98 .49 .964  
 03512>  
 03513> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03514> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03515> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03516> THAN THE STORAGE COEFFICIENT.  
 03517> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03518>  
 03519>  
 03520> 001:0007-----  
 03521>  
 03522> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03523> | | (ha) (cms) (hrs) (mm) (cms)  
 03524> | ID1 01:010 2.07 .685 1.04 64.55 .000  
 03525> | +ID2 02:020 1.54 .534 1.04 67.32 .000  
 03526> |-----  
 03527> | SUM 04:040 3.61 1.220 1.04 65.74 .000  
 03528>  
 03529> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03530>  
 03531>  
 03532> 001:0008-----  
 03533>  
 03534> | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03535> | | (ha) (cms) (hrs) (mm) (cms)  
 03536> | ID1 03:030 1.40 .509 1.04 69.06 .000  
 03537> | +ID2 04:040 3.61 1.220 1.04 65.74 .000  
 03538> |-----  
 03539> | SUM 05:050 5.01 1.729 1.04 66.66 .000  
 03540>  
 03541> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03542>  
 03543>  
 03544> 001:0009-----  
 03545> \* SUB-AREA No.4  
 03546> | CALIB STANDHYD | Area (ha)= .89  
 03547> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 03548> |-----  
 03549> | IMPERVIOUS PERVIOUS (i)  
 03550> | Surface Area (ha)= .86 .03  
 03551> | Dep. Storage (mm)= 1.57 4.67  
 03552> | Average Slope (%)= .93 .70  
 03553> | Length (m)= 164.82 40.00  
 03554> | Mannings n = .030 .250  
 03555> |-----  
 03556> | Max. eff. Inten. (mm/hr)= 178.56 67.61  
 03557> | over (min)= 5.00 15.00  
 03558> | Storage Coeff. (min)= 4.62 (ii) 15.92 (ii)  
 03559> | Unit Hyd. Tpeak (min)= 5.00 15.00  
 03560> | Unit Hyd. peak (cms)= .24 .07  
 03561> |-----  
 03562> | PEAK FLOW (cms)= .37 .00 \*TOTALS\*  
 03563> | TIME TO PEAK (hrs)= 1.00 1.21 1.000  
 03564> | RUNOFF VOLUME (mm)= 70.09 35.46 69.056  
 03565> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03566> | RUNOFF COEFFICIENT = .98 .49 .964  
 03567>  
 03568> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03569> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03570> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03571> THAN THE STORAGE COEFFICIENT.  
 03572> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03573>  
 03574>  
 03575>  
 03576>  
 03577> 001:0010-----  
 03578> \* SUB-AREA No.5  
 03579> | CALIB STANDHYD | Area (ha)= 2.66  
 03580> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00  
 03581> |-----  
 03582> | IMPERVIOUS PERVIOUS (i)  
 03583> | Surface Area (ha)= 2.58 .08  
 03584> | Dep. Storage (mm)= 1.57 4.67  
 03585> | Average Slope (%)= .61 1.50  
 03586> | Length (m)= 207.25 20.00  
 03587> | Mannings n = .030 .250  
 03588> |-----  
 03589> | Max. eff. Inten. (mm/hr)= 178.56 74.05  
 03590> | over (min)= 5.00 12.50  
 03591> | Storage Coeff. (min)= 6.01 (ii) 11.73 (ii)  
 03592> | Unit Hyd. Tpeak (min)= 5.00 12.50  
 03593> | Unit Hyd. peak (cms)= .20 .09  
 03594> |-----  
 03595> | PEAK FLOW (cms)= 1.03 .01 \*TOTALS\*  
 03596> | TIME TO PEAK (hrs)= 1.00 1.17 1.000  
 03597> | RUNOFF VOLUME (mm)= 70.09 35.46 69.056  
 03598> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03599> | RUNOFF COEFFICIENT = .98 .49 .964  
 03600>  
 03601> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03602> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03603> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03604> THAN THE STORAGE COEFFICIENT.  
 03605> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03606>  
 03607>  
 03608>  
 03609>  
 03610> 001:0011-----  
 03611>  
 03612> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03613> | | (ha) (cms) (hrs) (mm) (cms)  
 03614> | ID1 06:060 2.89 .374 1.00 69.06 .000  
 03615> | +ID2 07:070 2.66 1.034 1.00 69.06 .000  
 03616> |-----  
 03617> | SUM 08:080 3.55 1.408 1.00 69.06 .000  
 03618>  
 03619> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03620>  
 03621>  
 03622> 001:0012-----  
 03623>  
 03624> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03625> | | (ha) (cms) (hrs) (mm) (cms)  
 03626> | ID1 05:050 5.01 1.729 1.04 66.66 .000  
 03627> | +ID2 08:080 3.55 1.408 1.00 69.06 .000  
 03628> |-----  
 03629> | SUM 09:090 8.56 3.067 1.04 67.66 .000  
 03630>  
 03631> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03632>  
 03633>  
 03634> 001:0013-----  
 03635>  
 03636> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.  
 03637> | IN>09: (090 ) |  
 03638> | OUT<10: (POND ) |  
 03639> |-----  
 03640> | OUTFLOW STORAGE | OUTFLOW STORAGE  
 03641> | (cms) (ha.m.) | (cms) (ha.m.)  
 03642> | .000 .0000E+00 | .593 .6281E+00  
 03643> | .008 .6500E-01 | .654 .6631E+00  
 03644> | .017 .1311E+00 | .797 .7391E+00  
 03645> | .093 .2831E+00 | .950 .8274E+00  
 03646> | .233 .3971E+00 | 1.304 .9157E+00

03646> .337 .4731E+00 | 1.880 .1004E+01  
 03647> .465 .5491E+00 | 2.577 .1092E+01  
 03648> .531 .5871E+00 | .000 .0000E+00  
 03649>  
 03650> ROUTING RESULTS AREA QPEAK TPEAK R.V.  
 03651> (ha) (cms) (hrs) (mm)  
 03652> INFLOW >09: (090 ) 8.56 3.067 1.042 67.655  
 03653> OUTFLOW<10: (POND ) 8.56 .283 1.861 67.653  
 03654>  
 03655> PEAK FLOW REDUCTION [Out/In] (%) = 9.214  
 03656> TIME SHIFT OF PEAK FLOW (min) = 49.17  
 03657> MAXIMUM STORAGE USED (ha.m.) = .4333E+00  
 03658>  
 03659>  
 03660> 001:0014-----  
 03661> \*\*\*\*\*  
 03662> \* Remaining Hawthorne Industrial Park \*  
 03663> \*\*\*\*\*  
 03664> \* SUB-AREA No.1  
 03665> | CALIB STANDHYD | Area (ha)= 19.90  
 03666> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 03667> |-----  
 03668> | IMPERVIOUS PERVIOUS (i)  
 03669> | Surface Area (ha)= 14.13 5.77  
 03670> | Dep. Storage (mm)= 1.57 4.67  
 03671> | Average Slope (%)= .60 1.50  
 03672> | Length (m)= 580.00 100.00  
 03673> | Mannings n = .030 .250  
 03674> |-----  
 03675> | Max. eff. Inten. (mm/hr)= 153.66 117.89  
 03676> | over (min)= 12.50 25.00  
 03677> | Storage Coeff. (min)= 11.89 (ii) 24.37 (ii)  
 03678> | Unit Hyd. Tpeak (min)= 12.50 25.00  
 03679> | Unit Hyd. peak (cms)= .09 .05  
 03680> |-----  
 03681> | PEAK FLOW (cms)= 2.77 1.13 \*TOTALS\*  
 03682> | TIME TO PEAK (hrs)= 1.13 1.38 3.419 (iii)  
 03683> | RUNOFF VOLUME (mm)= 70.09 45.94 58.015  
 03684> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03685> | RUNOFF COEFFICIENT = .98 .64 .810  
 03686>  
 03687> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03688> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03689> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03690> THAN THE STORAGE COEFFICIENT.  
 03691> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03692>  
 03693>  
 03694>  
 03695> 001:0015-----  
 03696> | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03697> | | (ha) (cms) (hrs) (mm) (cms)  
 03698> | ID1 10:POND 8.56 .283 1.86 67.65 .000  
 03699> | +ID2 01:HIP01 19.90 3.419 1.17 58.02 .000  
 03700> |-----  
 03701> | SUM 02:HIP02 28.46 3.554 1.17 60.91 .000  
 03702>  
 03703> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 03704>  
 03705>  
 03706>  
 03707>  
 03708> 001:0016-----  
 03709> \* SUB-AREA No.2  
 03710> | CALIB STANDHYD | Area (ha)= 17.00  
 03711> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 03712> |-----  
 03713> | IMPERVIOUS PERVIOUS (i)  
 03714> | Surface Area (ha)= 12.07 4.93  
 03715> | Dep. Storage (mm)= 1.57 4.67  
 03716> | Average Slope (%)= .65 1.50  
 03717> | Length (m)= 450.00 100.00  
 03718> | Mannings n = .030 .250  
 03719> |-----  
 03720> | Max. eff. Inten. (mm/hr)= 178.56 126.60  
 03721> | over (min)= 10.00 22.50  
 03722> | Storage Coeff. (min)= 9.39 (ii) 21.52 (ii)  
 03723> | Unit Hyd. Tpeak (min)= 10.00 22.50  
 03724> | Unit Hyd. peak (cms)= .12 .05  
 03725> |-----  
 03726> | PEAK FLOW (cms)= 2.68 1.05 \*TOTALS\*  
 03727> | TIME TO PEAK (hrs)= 1.08 1.33 3.203 (iii)  
 03728> | RUNOFF VOLUME (mm)= 70.09 45.94 58.015  
 03729> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03730> | RUNOFF COEFFICIENT = .98 .64 .810  
 03731>  
 03732> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03733> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03734> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03735> THAN THE STORAGE COEFFICIENT.  
 03736> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03737>  
 03738>  
 03739>  
 03740>  
 03741> 001:0017-----  
 03742> \* SUB-AREA No.3  
 03743> | CALIB STANDHYD | Area (ha)= 15.60  
 03744> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00  
 03745> |-----  
 03746> | IMPERVIOUS PERVIOUS (i)  
 03747> | Surface Area (ha)= 11.08 4.52  
 03748> | Dep. Storage (mm)= 1.57 4.67  
 03749> | Average Slope (%)= .50 1.50  
 03750> | Length (m)= 600.00 100.00  
 03751> | Mannings n = .030 .250  
 03752> |-----  
 03753> | Max. eff. Inten. (mm/hr)= 153.66 117.89  
 03754> | over (min)= 12.50 25.00  
 03755> | Storage Coeff. (min)= 12.82 (ii) 25.30 (ii)  
 03756> | Unit Hyd. Tpeak (min)= 12.50 25.00  
 03757> | Unit Hyd. peak (cms)= .09 .04  
 03758> |-----  
 03759> | PEAK FLOW (cms)= 2.10 .87 \*TOTALS\*  
 03760> | TIME TO PEAK (hrs)= 1.13 1.38 2.612 (iii)  
 03761> | RUNOFF VOLUME (mm)= 70.09 45.94 58.015  
 03762> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665  
 03763> | RUNOFF COEFFICIENT = .98 .64 .810  
 03764>  
 03765> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 03766> CN\* = 81.0 Ia = Dep. Storage (Above)  
 03767> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 03768> THAN THE STORAGE COEFFICIENT.  
 03769> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 03770>  
 03771>  
 03772>  
 03773>  
 03774> 001:0018-----  
 03775> | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF  
 03776> | | (ha) (cms) (hrs) (mm) (cms)  
 03777> | ID1 03:HIP03 17.00 3.203 1.13 58.02 .000  
 03778> | +ID2 04:HIP04 15.60 2.612 1.17 58.02 .000  
 03779> |-----  
 03780> | SUM 04:HIP04 32.60 5.815 1.17 58.02 .000

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03781> SUM 05:HIP05 32.60 5.767 1.13 58.02 .000
03782>
03783> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03784>
03785>
03786> 001:0019-----
03787>
03788> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03789> | (ha) (cms) (hrs) (mm) (cms)
03790> ID1 05:HIP05 32.60 5.767 1.13 58.02 .000
03791> +ID2 02:HIP02 28.46 3.554 1.17 60.91 .000
03792>
03793> SUM 06:HIP06 61.06 9.239 1.13 59.36 .000
03794>
03795> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03796>
03797>
03798> 001:0020-----
03799> *
03800> * SUB-AREA No. 4
03801>
03802> | CALIB STANDHYD | Area (ha)= 12.20
03803> | 07:HIP07 DT= 2.50 | Total Imp (%)= 71.00 Dir. Conn. (%)= 50.00
03804>
03805> IMPERVIOUS PERVIOUS (i)
03806> Surface Area (ha)= 8.66 3.54
03807> Dep. Storage (mm)= 1.57 4.67
03808> Average Slope (%)= .70 1.50
03809> Length (m)= 210.00 100.00
03810> Mannings n = .030 .250
03811>
03812> Max. eff. Inten. (mm/hr)= 178.56 146.17
03813> over (min)= 5.00 17.50
03814> Storage Coeff. (min)= 5.81 (ii) 17.27 (ii)
03815> Unit Hyd. Tpeak (min)= 5.00 17.50
03816> Unit Hyd. peak (cms)= .20 .07
03817>
03818> PEAK FLOW (cms)= 2.46 .87 *TOTALS*
03819> TIME TO PEAK (hrs)= 1.00 1.25 2.793 (iii)
03820> RUNOFF VOLUME (mm)= 70.09 45.94 58.015
03821> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03822> RUNOFF COEFFICIENT = .98 .64 .810
03823>
03824> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03825> CN* = 81.0 Ia = Dep. Storage (Above)
03826> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03827> THAN THE STORAGE COEFFICIENT.
03828> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03829>
03830>
03831> 001:0021-----
03832> *
03833> * SUB-AREA No. 5
03834>
03835> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
03836> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03837> | U.H. Tp (hrs)= .170
03838>
03839> Unit Hyd Qpeak (cms)= .899
03840>
03841> PEAK FLOW (cms)= .649 (i)
03842> TIME TO PEAK (hrs)= 1.125
03843> RUNOFF VOLUME (mm)= 40.139
03844> TOTAL RAINFALL (mm)= 71.665
03845> RUNOFF COEFFICIENT = .560
03846>
03847> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03848>
03849>
03850> 001:0022-----
03851>
03852> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03853> | (ha) (cms) (hrs) (mm) (cms)
03854> ID1 06:HIP06 61.06 9.239 1.13 59.36 .000
03855> +ID2 07:HIP07 12.20 2.793 1.04 58.02 .000
03856> +ID3 08:Pond-B 4.00 .649 1.13 40.14 .000
03857>
03858> SUM 09:HIP08 77.26 12.109 1.13 58.16 .000
03859>
03860> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03861>
03862>
03863> 001:0023-----
03864>
03865> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03866> | IN>09: (HIP08) |
03867> | OUT<10: (HIP-PO) |
03868>
03869>
03870> ===== OUTFLOW STORAGE TABLE =====
03871> OUTFLOW STORAGE | OUTFLOW STORAGE
03872> (cms) (ha.m.) | (cms) (ha.m.)
03873> .000 .0000E+00 | .724 .2210E+01
03874> .048 .5740E-01 | .937 .2501E+01
03875> .054 .2434E+00 | 1.262 .2798E+01
03876> .059 .5834E+00 | 1.404 .3101E+01
03877> .062 .8400E+00 | 1.532 .3410E+01
03878> .064 .1102E+01 | 1.650 .3724E+01
03879> .147 .1370E+01 | 2.409 .4044E+01
03880> .280 .1644E+01 | 3.689 .4370E+01
03881> .472 .1924E+01 | .000 .0000E+00
03882>
03883> ROUTING RESULTS AREA QPEAK TPEAK R.V.
03884> (ha) (cms) (hrs) (mm)
03885> INFLOW >09: (HIP08) 77.26 12.109 1.125 58.156
03886> OUTFLOW <10: (HIP-PO) 77.26 1.432 2.889 58.156
03887>
03888> PEAK FLOW REDUCTION [Qout/Qin] (%)= 11.826
03889> TIME SHIFT OF PEAK FLOW (min)= 105.83
03890> MAXIMUM STORAGE USED (ha.m.)= .3168E+01
03891>
03892> 001:0024-----
03893> *
03894> * SUB-AREA No. 6
03895>
03896> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
03897> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03898> | U.H. Tp (hrs)= .800
03899>
03900> Unit Hyd Qpeak (cms)= .129
03901>
03902> PEAK FLOW (cms)= .114 (i)
03903> TIME TO PEAK (hrs)= 1.958
03904> RUNOFF VOLUME (mm)= 30.490
03905> TOTAL RAINFALL (mm)= 71.665
03906> RUNOFF COEFFICIENT = .425
03907>
03908> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03909>
03910>
03911> 001:0025-----
03912>
03913> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03914> | (ha) (cms) (hrs) (mm) (cms)
03915> ID1 10:HIP-PO 77.26 1.432 2.89 58.16 .000
03916> +ID2 01:A3 2.70 .114 1.96 30.49 .000
03917>

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03916> SUM 02:Ultima 79.96 1.515 2.57 57.22 .000
03917>
03918> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03919>
03920>
03921> 001:0026-----
03922> FINISH
03923>
03924> *****
03925> WARNINGS / ERRORS / NOTES
03926>
03927> Simulation ended on 2009-05-15 at 08:45:24
03928>
03929>
03930>

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## **APPENDIX 'I'**

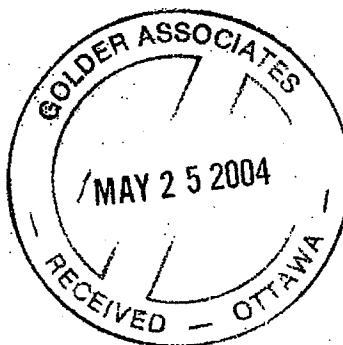
**MINISTRY OF THE ENVIRONMENT  
CERTIFICATE OF APPROVAL  
EXISTING SETTLING PONDS**

UK



Ministry  
of the  
Environment

Ministère  
de  
l'Environnement



CERTIFICATE OF APPROVAL  
INDUSTRIAL SEWAGE WORKS  
NUMBER 6924-5YWQ3U

R. W. Tomlinson Limited  
5597 Power Road, R.R. No. 6  
Gloucester, Ontario  
K1G 3N4

Site Location: Tomlinson Property, east side of Hawthorne Road  
Lot 26 & 27, Concession VI  
Ottawa City

*You have applied in accordance with Section 53 of the Ontario Water Resources Act for approval of:*

the establishment of sewage works for the collection, transmission, treatment and disposal of excess wash plant wash water, consisting of the following:

- 410 millimeter pipeline extending from the wash plant, located on the Rideau Road Quarry #1 site, to the settling ponds;
- three (3) settling ponds, in series, Cell #1 having an effective volume of 3,275 cubic metres (and an operating freeboard of 0.6 metres), Cell #2 having an effective volume of 2,347 cubic metres (and an operating freeboard of 0.6 metres) and Cell #3 having an effective volume of 1,154 cubic metres (and an operating freeboard of 0.6 metres), including temporary floating pumping station in Cell #1, floating recycle pumping station in Cell #2, baffle in Cell #2 and mixing manhole between Cell #2 and Cell #3 (if required), with an overflow discharge from Cell #3 to the roadside ditch along Hawthorne Road;
- all other controls, electrical equipment, instrumentation, piping, pumps, valves and appurtenances essential for the proper operation of the aforementioned sewage works;

all in accordance with the following submitted supporting documents:

1. Application for Approval of Industrial Sewage Works submitted by Ronald Tomlinson of R. W. Tomlinson Limited dated March 8, 2004;
2. Report on Application for Industrial Sewage Works Approval under Section 53 of the Ontario Water Resources Act, R.W. Tomlinson Limited, Aggregate Wash Water Management Associated with Rideau Road Quarry No. 1, Geographic City of Gloucester, City of Ottawa, Ontario prepared by Golder Associates, dated March 2004; and



3. Letter and attachments dated May 11, 2004 from Nural Kuyucak and K. Marentette of Golder Associates to Randy Chin of the Ministry of the Environment.

*For the purpose of this Certificate of Approval and the terms and conditions specified below, the following definitions apply:*

"Certificate" means this entire certificate of approval document, issued in accordance with Section 53 of the *Ontario Water Resources Act*, and includes any schedules;

"Director" means any Ministry employee appointed by the Minister pursuant to section 5 of the *Ontario Water Resources Act*;

"District Manager" means the District Manager of the Ottawa District Office of the Ministry;

"Ministry" means the Ontario Ministry of the Environment;

"Owner" means R. W. Tomlinson Limited and includes its successors and assignees; and

"works" means the sewage works described in the Owner's application, this certificate and in the supporting documentation referred to herein, to the extent approved by this certificate.

*You are hereby notified that this approval is issued to you subject to the terms and conditions outlined below:*

## **TERMS AND CONDITIONS**

### **1. GENERAL CONDITION**

(1) Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the works in accordance with the description given in this Certificate, the application for approval of the works and the submitted supporting documents and plans and specifications as listed in this Certificate.

(2) Where there is a conflict between a provision of any submitted document referred to in this Certificate and the Conditions of this Certificate, the Conditions in this Certificate shall take precedence, and where there is a conflict between the listed submitted documents, the document bearing the most recent date shall prevail.

### **2. CHANGE OF OWNER**

(1) The Owner shall notify the District Manager and the Director, in writing, of any of the following changes within 30 days of the change occurring:

(a) change of Owner or operating authority, or both;

(b) change of address of Owner or operating authority or address of new owner or operating

authority;

(c) change of partners where the Owner or operating authority is or at any time becomes a partnership, and a copy of the most recent declaration filed under the *Partnerships Registration Act*; and

(d) change of name of the corporation where the Owner or operator is or at any time becomes a corporation, and a copy of the most current "Initial Notice or Notice of Change" (Form 1, 2 or 3 of O. Reg. 189, R.R.O. 1980, as amended from time to time), filed under the *Corporations Informations Act* shall be included in the notification to the District Manager.

(2) In the event of any change in ownership of the works, the Owner shall notify in writing the succeeding owner of the existence of this certificate, and a copy of such notice shall be forwarded to the District Manager.

(3) The Owner shall ensure that all communications made pursuant to this condition will refer to this certificate's number.

### 3. OPERATIONS MANUAL

(1) The Owner shall prepare an operations manual prior to the commencement of operation of the sewage works, that includes, but not necessarily limited to, the following information:

(a) operating procedures for routine operation of the works;

(b) inspection programs, including frequency of inspection, for the works and the methods or tests employed to detect when maintenance is necessary;

(c) repair and maintenance programs, including the frequency of repair and maintenance for the works;

(d) contingency plans and procedures for dealing with potential spill, bypasses and any other abnormal situations and for notifying the District Manager; and

(e) complaint procedures for receiving and responding to public complaints.

(2) The Owner shall maintain the operations manual up to date through revisions undertaken from time to time and retain a copy at the location of the sewage works. Upon request, the Owner shall make the manual available for inspection and copying by Ministry personnel.

### 4. CLOSED LOOP OPERATION

(1) The Owner shall ensure that the works are normally operated as a closed loop system with treated water being recycled back to the wash plant.

(2) In the event that excess accumulation of water occurs and a discharge is necessary, the Owner shall undertake the monitoring outlined in Condition 6 and shall adhere to the effluent limits in Condition 5.

5. **EFFLUENT LIMITS**

(1) The Owner shall design, construct and operate the works such that the concentration of Total Suspended Solids does not exceed 25 milligrams per litre in the effluent from the works.

(2) For the purposes of determining compliance with and enforcing subsection (1), non-compliance with respect to the Total Suspended Solids concentration limit is deemed to have occurred when any single sample (along with a follow-up confirmation sample collected within 7 days of the receipt of the original sample result that indicated that an exceedance had occurred) analyzed for Total Suspended Solids is greater than the corresponding maximum concentration set out in subsection (1).

6. **EFFLUENT MONITORING AND RECORDING**

The Owner shall, upon commencement of operation of the sewage works, carry out the following monitoring program:

(1) All samples and measurements taken for the purposes of this certificate are to be taken at a time and in a location characteristic of the quality and quantity of the effluent stream over the time period being monitored.

(2) Samples shall be collected of the discharge from Cell #3 to the Hawthorne Road ditch and analyzed, at the sampling frequencies and using the sample type specified for each parameter listed:

<b>Table 1 - Effluent Monitoring</b>	
<b>Frequency</b>	Once each Month During Periods of Effluent Discharge
<b>Sample Type</b>	Grab
<b>Parameters</b>	Total Suspended Solids

(3) The methods and protocols for sampling, analysis, and recording shall conform, in order of precedence, to the methods and protocols specified in the following:

(a) the Ministry's publication "Protocol for the Sampling and Analysis of Industrial/Municipal Wastewater" (August 1994), ISBN 0-7778-1880-9, as amended from time to time by more recently published editions; and

(b) the publication "Standard Methods for the Examination of Water and Wastewater" (17th edition) as amended from time to time by more recently published editions.

(4) The Owner shall measure, record and calculate the flowrate from Cell #3 to the Hawthorne Road ditch daily (during periods of discharge), within an accuracy of plus or minus 15 per cent of the actual flowrate.

(5) The Owner shall retain for a minimum of three (3) years from the date of their creation, all records and information related to or resulting from the monitoring activities required by this certificate.

## 7. REPORTING

(1) The Owner shall report to the District Manager or designate, of any exceedence of any parameter specified in Conditions 5 orally, as soon as reasonably possible, and in writing within seven (7) days of the exceedence.

*The reasons for the imposition of these terms and conditions are as follows:*

1. Condition 1 is imposed to ensure that the works are built and operated in the manner in which they were described for review and upon which approval was granted. This condition is also included to emphasize the precedence of Conditions in the Certificate and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
2. Condition 2 is included to ensure that the Ministry records are kept accurate and current with respect to approved works and to ensure that subsequent owners of the works are made aware of the certificate and continue to operate the works in compliance with it.
3. Condition 3 is included to ensure that a comprehensive operations manual governing all significant areas of operation, maintenance and repair is prepared, implemented and kept up-to-date by the owner and made available to the Ministry. Such a manual is an integral part of the operation of the works. Its compilation and use should assist the owner in staff training, in proper plant operation and in identifying and planning for contingencies during possible abnormal conditions. The manual will also act as a benchmark for Ministry staff when reviewing the owner's operation of the work.
4. Condition 4 is included to ensure that the works are operated as designed.
5. Condition 5 is imposed to ensure that the effluent discharged from the works meets the Ministry's effluent quality requirements thus minimizing environmental impact on the receiver.
6. Conditions 6 and 7 are included to require the owner to demonstrate on a continual basis that the quality of the effluent from the approved works is consistent with the effluent limits specified in the certificate and that the approved works does not cause any impairment to the receiving watercourse.

*In accordance with Section 100 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, as amended, you may by written notice served upon me and the Environmental Review Tribunal and in accordance with Section 47 of the Environmental Bill of Rights, S.O. 1993, Chapter 28, the Environmental Commissioner, within 15 days after receipt of this Notice, require a hearing by the Tribunal. The Environmental Commissioner will place notice of your appeal on the Environmental Registry. Section 101 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, provides that the Notice requiring the hearing shall state:*

1. The portions of the approval or each term or condition in the approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

*The Notice should also include:*

3. The name of the appellant;
4. The address of the appellant;
5. The Certificate of Approval number;
6. The date of the Certificate of Approval;
7. The name of the Director;
8. The municipality within which the works are located;

*And the Notice should be signed and dated by the appellant.*

*This Notice must be served upon:*

The Secretary\*  
Environmental Review Tribunal  
2300 Yonge St., 12th Floor  
P.O. Box 2382  
Toronto, Ontario  
M4P 1E4

AND

The Environmental Commissioner  
1075 Bay Street, 6th Floor  
Suite 605  
Toronto, Ontario  
M5S 2B1

AND

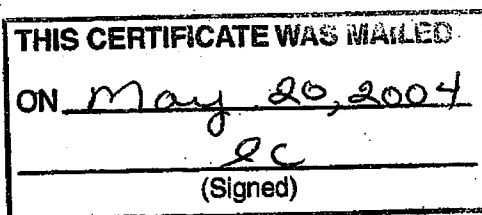
The Director  
Section 53, *Ontario Water Resources Act*  
Ministry of the Environment  
2 St. Clair Avenue West, Floor 12A  
Toronto, Ontario  
M4V 1L5

\* Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 314-4600, Fax: (416) 314-4506 or [www.ert.gov.on.ca](http://www.ert.gov.on.ca)

*This instrument is subject to Section 38 of the Environmental Bill of Rights, that allows residents of Ontario to seek leave to appeal the decision on this instrument. Residents of Ontario may seek leave to appeal within 15 days from the date this decision is placed on the Environmental Registry. By accessing the Environmental Registry at [www.ene.gov.on.ca](http://www.ene.gov.on.ca), you can determine when the leave to appeal period ends.*

*The above noted sewage works are approved under Section 53 of the Ontario Water Resources Act.*

DATED AT TORONTO this 19th day of May, 2004



Mohamed Dhalla, P.Eng.  
Director  
Section 53, *Ontario Water Resources Act*

RC/

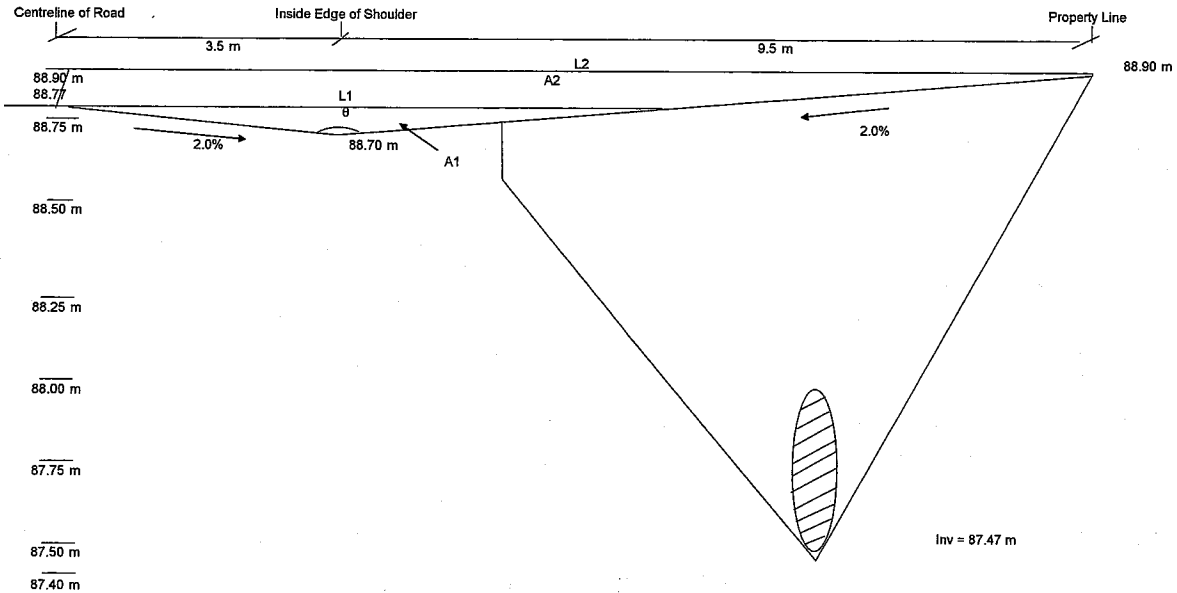
c: District Manager, MOE Ottawa  
Nural Kuyucak, Golder Associates Ltd. ✓

## **A P P E N D I X ' J '**

### **ASSESSMENT OF CULVERT CROSSING DURING AN EXTREME STORM EVENT**

**ENTRANCE TO POND ACCESS ROAD - OPEN DITCH/CULVERT CONFIGURATION**

Typical open ditch/culvert configuration: 1390x970mm CSPA culvert, invert approx. 1.43 m below elevation at property line. Proposed Terrace Elevation is approx. 0.13 m above road centreline.



A1:	0.24 m2	L1:	7.000 m
A2:	1.30 m2	L2:	13.000 m
θ:	178 Degrees		

FLOW ABOVE CULVERT THRU A1:	FLOW ABOVE CULVERT THRU A2:
<p>Since <math>\theta</math> is equal to approx. 180 degrees            Use the Rectangular Weir Equation to Estimate the Flow Thru A1:</p> $Q = C \times L \times H^{1.5}$ $C = 1.84$ $L' = L_1 - (0.1 \times n \times h)$ <p style="text-align: right;">, where <math>n</math> = no. of end contractions</p> $\text{use } h = 88.77 - 88.7 = 0.07 \text{ m}$ $h = 0.07 \text{ m}$ $L' = 6.99 \text{ m}$	<p>Using the Rectangular Weir Equation to Estimate the Flow Thru A2:</p> $Q = C \times L \times H^{1.5}$ $C = 1.84$ $L' = L_3 - (0.1 \times n \times h)$ <p style="text-align: right;">, where <math>n</math> = no. of end contractions</p> $\text{use } h = 88.9 - 88.77 = 0.13 \text{ m}$ $h = 0.13 \text{ m}$ $L_3 = (L_1 + L_2) / 2 = 10 \text{ m} \quad (\text{Avg. Length})$ $L' = 9.97 \text{ m}$
$Q_{A1} = 0.24 \text{ m}^3/\text{s}$	$Q_{A2} = 0.86 \text{ m}^3/\text{s}$

1:100 year Peak Flow Rate of 3.0 m<sup>3</sup>/s (From Storm Design Sheet : 100 Year Flow 27B-27C)

Flow through the 1390 x 970 mm CSPA Culvert under inlet Control Conditions = 1.9 m<sup>3</sup>/s (From Culvert Sizing Nomograph 27B-27C)

$$\text{Total flow above culvert} = Q_{A1} + Q_{A2} = 0.24 \text{ m}^3/\text{s} + 0.86 \text{ m}^3/\text{s} = 1.10 \text{ m}^3/\text{s}$$

Therefore, Total Flow =  $1.9 \text{ m}^3/\text{s} + 1.1 \text{ m}^3/\text{s}$

$$= 3.0 \text{ m}^3/\text{s}$$

=1:100 year Peak Flow Rate



## **A P P E N D I X 'K'**

### **SWMHYMO INPUT AND OUTPUT FILES (July 1, 1979 Historical Storm Event)**

```

00001> 2 Metric units
00002> *****
00003> # Project Name : Hawthorne Industrial Park Project Number: [20983] *
00004> # Date : January, 2009 *
00005> # Revised : N/A *
00006> # Developed by : Mark Buchanan, E.I.T. *
00007> # Reviewed by : Guy Forget, P.Eng. *
00008> # Company : J.L. Richards & Associates Limited *
00009> # License # : 4618403 *
00010> *****
00011> *
00012> *
00013> *****
00014> # FILENAME: V:\20983.DU\ENG\SWMHYMO\20983PST.DAT
00015> # FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00016> # OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00017> *****
00018> *
00019> *****
00020> # SWMHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE
00021> # PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00022> *****
00023> *
00024> *****
00025> # HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00026> # FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00027> *****
00028> *****
00029> # CALCULATION OF JULY 1st 1979 STORM EVENT
00030> *****
00031> *
00032> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00033> # [ ] <-- storm filename, one per line for NSTORM time
00034> STORM_FILENAME=[ "JUL_1_79.STM" ]
00035> *****
00036> # DEFAULT VALUES
00037> ICASEDEF=[1], read and print values
00038> DEFWAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00039> *****
00040> *
00041> # ORGAWORLD FILE
00042> *****
00043> *
00044> # SUB-AREA No.1
00045> *
00046> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00047> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00048> SCS curve number CN=[81],
00049> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00050> LGP=[20] (m), MNP=[ 0.25 ], SCP=[0.0] (m)
00051> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00052> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
00053> RAINFALL=[ , , , ] (mm/hr), END=-1
00054> *****
00055> *
00056> # SUB-AREA No.2
00057> *
00058> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00059> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00060> SCS curve number CN=[81],
00061> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00062> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00063> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00064> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0]
00065> RAINFALL=[ , , , ] (mm/hr), END=-1
00066> *****
00067> *
00068> # SUB-AREA No.3
00069> *
00070> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00071> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00072> SCS curve number CN=[81],
00073> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00074> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00075> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00076> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
00077> RAINFALL=[ , , , ] (mm/hr), END=-1
00078> *****
00079> # ADD HYD
00080> IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00081> *****
00082> # ADD HYD
00083> IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00084> *****
00085> *
00086> # SUB-AREA No.4
00087> *
00088> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00089> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00090> SCS curve number CN=[81],
00091> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00092> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00093> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00094> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
00095> RAINFALL=[ , , , ] (mm/hr), END=-1
00096> *****
00097> *
00098> # SUB-AREA No.5
00099> *
00100> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00101> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00102> SCS curve number CN=[81],
00103> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00104> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (m)
00105> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00106> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
00107> RAINFALL=[ , , , ] (mm/hr), END=-1
00108> *****
00109> # ADD HYD
00110> IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00111> *****
00112> # ADD HYD
00113> IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00114> *****
00115> *
00116> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00117> RDT=[1.0] (min),
00118> TABLE of ( OUTFLOW-STORAGE ) values
00119> (cms) - (ha-m)
00120> [ 0.000, 0.0000 ]
00121> [ 0.008, 0.0656 ]
00122> [ 0.017, 0.1311 ]
00123> [ 0.093, 0.2831 ]
00124> [ 0.233, 0.3971 ]
00125> [ 0.337, 0.4731 ]
00126> [ 0.465, 0.5491 ]
00127> [ 0.531, 0.5871 ]
00128> [ 0.593, 0.6251 ]
00129> [ 0.654, 0.6631 ]
00130> [ 0.797, 0.7391 ]
00131> [ 0.950, 0.8274 ]
00132> [ 1.304, 0.9157 ]
00133> [ 1.880, 1.0040 ]
00134> [ 2.577, 1.0923 ]
00135> [ -1, -1 ] (max twenty pts)

```

```

00136> *
00137> # SUB-AREA No.1
00138> *
00139> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00140> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00141> SCS curve number CN=[81],
00142> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00143> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00144> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00145> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00146> RAINFALL=[ , , , ] (mm/hr), END=-1
00147> *****
00148> # ADD HYD
00149> IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00150> *****
00151> # SUB-AREA No.2
00152> *
00153> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00154> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00155> SCS curve number CN=[81],
00156> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00157> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00158> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00159> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00160> RAINFALL=[ , , , ] (mm/hr), END=-1
00161> *****
00162> *
00163> # SUB-AREA No.3
00164> *
00165> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00166> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00167> SCS curve number CN=[81],
00168> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00169> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00170> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00171> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
00172> RAINFALL=[ , , , ] (mm/hr), END=-1
00173> *****
00174> # ADD HYD
00175> IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00176> *****
00177> # ADD HYD
00178> IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00179> *****
00180> *
00181> # SUB-AREA No.4
00182> *
00183> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00184> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00185> SCS curve number CN=[81],
00186> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00187> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00188> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00189> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
00190> RAINFALL=[ , , , ] (mm/hr), END=-1
00191> *****
00192> *
00193> # SUB-AREA No.5
00194> *
00195> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00196> DWF=[0.0] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00197> RAINFALL=[ , , , ] (mm/hr), END=-1
00198> *****
00199> # ADD HYD
00200> IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00201> *****
00202> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
00203> RDT=[1.0] (min),
00204> TABLE of ( OUTFLOW-STORAGE ) values
00205> (cms) - (ha-m)
00206> [ 0.0, 0.0 ]
00207> [ 0.048, 0.0574 ]
00208> [ 0.054, 0.2434 ]
00209> [ 0.059, 0.5834 ]
00210> [ 0.062, 0.8400 ]
00211> [ 0.064, 1.1024 ]
00212> [ 0.147, 1.3705 ]
00213> [ 0.280, 1.6444 ]
00214> [ 0.472, 1.9242 ]
00215> [ 0.724, 2.2097 ]
00216> [ 0.937, 2.5010 ]
00217> [ 1.262, 2.7981 ]
00218> [ 1.404, 3.1009 ]
00219> [ 1.532, 3.4096 ]
00220> [ 1.650, 3.7240 ]
00221> [ 2.409, 4.0442 ]
00222> [ 3.689, 4.3702 ]
00223> [ -1, -1 ] (max twenty pts)
00224> *****
00225> *
00226> # SUB-AREA No.6
00227> *
00228> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00229> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
00230> RAINFALL=[ , , , ] (mm/hr), END=-1
00231> *****
00232> # ADD HYD
00233> IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00234> *****
00235> *
00236> FINISH
00237> *****
00238> *
00239> *
00240> *
00241> *
00242> *

```

```

00001 *****
00002 *****
00003 SSSSS W W M M H H Y Y M M M 000 999 999 *****
00004 S W W W M M M H H Y Y M M M 0 0 9 9 9 9 *****
00005 SSSSS W W W M M M H H H Y Y M M M 0 0 ## 9 9 9 9 Ver. 4.02
00006 S W W M M M H H Y Y M M M 0 0 9999 9999 July 1999
00007 SSSSS W W M M H H Y Y M M M 000 9 9 9 *****
00008 9 9 9 9 # 418403
00009 StormWater Management Hydrologic Model 999 999 *****
00010 *****
00011 *****
00012 *****
00013 *****
00014 ***** A single event and principles of HYMO simulation model *****
00015 ***** based on the principles of HYMO and its successors *****
00016 ***** OTHYMO-83 and OTHYMO-89 *****
00017 ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018 ***** Ottawa, Ontario: (613) 727-5199 *****
00019 ***** Gatineau, Quebec: (819) 243-6858 *****
00020 ***** E-Mail: swmhymodiffa.com *****
00021 *****
00022 *****
00023 *****
00024 ***** Licensed user: J. L. Richards & Associates Limited *****
00025 ***** Ottawa SERIAL: 418403 *****
00026 *****
00027 *****
00028 *****
00029 ***** PROGRAM ARRAY DIMENSIONS *****
00030 ***** Maximum Value for ID number: 10 *****
00031 ***** Max. number of rainfall points: 15000 *****
00032 ***** Max. number of flow points: 15000 *****
00033 *****
00034 *****
00035 *****
00036 ***** DETAILED OUTPUT *****
00037 *****
00038 ***** DATE: 2009-05-15 TIME: 09:03:53 RUN COUNTER: 000200 *****
00039 *****
00040 ***** Input filename: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\July1979.dat *****
00041 ***** Output filename: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\July1979.out *****
00042 ***** Summary filename: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\July1979.sum *****
00043 ***** User comments: *****
00044 ***** 1: *****
00045 ***** 2: *****
00046 ***** 3: *****
00047 *****
00048 *****
00049 *****
00050 001:0001 *****
00051 *****
00052 ***** Project Name: Hawthorne Industrial Park Project Number: [20983] *****
00053 ***** Date: January, 2009 *****
00054 ***** Revised: N/A *****
00055 ***** Developed by: Mark Buchanan, E.I.T. *****
00056 ***** Reviewed by: Guy Forget, P.Eng. *****
00057 ***** Company: J.L. Richards & Associates Limited *****
00058 ***** License #: 4418403 *****
00059 *****
00060 *****
00061 *****
00062 *****
00063 ***** FILENAME: V:\20983.DU\ENG\SWMHYM\20983PST.DAT *****
00064 ***** FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *****
00065 ***** OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *****
00066 *****
00067 *****
00068 *****
00069 ***** SWMHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE *****
00070 ***** PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *****
00071 *****
00072 *****
00073 ***** HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *****
00074 ***** FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *****
00075 *****
00076 *****
00077 ***** CALCULATION OF JULY 1st 1979 STORM EVENT *****
00078 *****
00079 *****
00080 ***** START Project dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\ *****
00081 ***** Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWMHYM-1\ *****
00082 ***** TZERO = .00 hrs on 0 *****
00083 ***** METOUT = 2 (output = METRIC) *****
00084 ***** NRUN = 001 *****
00085 ***** NSTORM = 0 *****
00086 *****
00087 *****
00088 *****
00089 ***** READ STORM *****
00090 ***** Total= 88.86 mm *****
00091 ***** Comments: HISTORICAL STORM - JULY 1, 1979 *****
00092 *****
00093 *****
00094 *****
00095 *****
00096 *****
00097 *****
00098 *****
00099 *****
00100 *****
00101 *****
00102 *****
00103 *****
00104 *****
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00128 *****
00129 *****
00130 *****
00131 *****
00132 *****
00133 *****
00134 *****
00135 *****

```

```

00271> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00272> CN* = 81.0 Ia = Dep. Storage (Above)
00273> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00274> THAN THE STORAGE COEFFICIENT.
00275> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00276>
00277>
00278>
00279> 001:0010-----
00280> * SUB-AREA No.5
00281>
00282> | CALIB STANDHYD | Area (ha)= 2.66 Dir. Conn.(%)= 97.00
00283> | 07:070 DT= 2.50 | Total Imp(%)= 97.00
00284>
00285> IMPERVIOUS PERVIOUS (i)
00286> Surface Area (ha)= 2.58 .08
00287> Dep. Storage (mm)= 1.57 4.67
00288> Average Slope (%)= .61 1.50
00289> Length (m)= 207.25 20.00
00290> Mannings n = .030 .250
00291>
00292> Max.eff.Inten.(mm/hr)= 106.70 70.39
00293> over (min)= 7.50 12.50
00294> Storage Coeff. (min)= 7.39 (ii) 13.23 (ii)
00295> Unit Hyd. Tpeak (min)= 7.50 12.50
00296> Unit Hyd. peak (cms)= .15 .09
00297>
00298> PEAK FLOW (cms)= .65 .01 *TOTALS*
00299> TIME TO PEAK (hrs)= 1.54 1.67 .665 (iii)
00300> RUNOFF VOLUME (mm)= 87.29 49.30 86.147
00301> TOTAL RAINFALL (mm)= 88.86 88.86 88.857
00302> RUNOFF COEFFICIENT = .98 .55 .970
00303>
00304> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00305> CN* = 81.0 Ia = Dep. Storage (Above)
00306> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00307> THAN THE STORAGE COEFFICIENT.
00308> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00309>
00310>
00311>
00312> 001:0011-----
00313> | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00314> | (ha) (cms) (hrs) (mm) (cms)
00315> ID1 06:060 .89 .235 1.50 86.15 .000
00316> +ID2 07:070 2.66 .665 1.54 86.15 .000
00317>
00318> SUM 08:080 3.55 .896 1.54 86.15 .000
00319>
00320> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00321>
00322>
00323>
00324> 001:0012-----
00325> | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00326> | (ha) (cms) (hrs) (mm) (cms)
00327> ID1 05:050 5.01 1.188 1.54 83.52 .000
00328> +ID2 08:000 3.55 .896 1.54 86.15 .000
00329>
00330> SUM 09:090 8.56 2.084 1.54 84.61 .000
00331>
00332> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00333>
00334>
00335>
00336> 001:0013-----
00337> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00338> | IN:09: (090 ) |
00339> | OUT:10: (POND ) |
00340>
00341> ===== OUTFLOW STORAGE TABLE =====
00342> OUTFLOW STORAGE OUTFLOW STORAGE
00343> (cms) (ha.m.) (cms) (ha.m.)
00344> .000 .0000E+00 .593 .6251E+00
00345> .009 .6560E-01 .634 .6631E+00
00346> .017 .1331E+00 .797 .7391E+00
00347> .093 .2833E+00 .950 .8274E+00
00348> .233 .3971E+00 1.304 .9157E+00
00349> .337 .4731E+00 1.880 .1004E+01
00350> .465 .5491E+00 2.577 .1092E+01
00351> .531 .5871E+00 .000 .0000E+00
00352>
00353> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00354> INFLOW-09: (090 ) (ha) (cms) (hrs) (mm)
00355> OUTFLOW-10: (POND ) 8.56 2.084 1.54 84.61
00356>
00357> PEAK FLOW REDUCTION (Qout/Qin) (%) = 23.815
00358> TIME SHIFT OF PEAK FLOW (min) = 35.000
00359> MAXIMUM STORAGE USED (ha.m.) = .5671E+00
00360>
00361>
00362> 001:0014-----
00363> ***** Remaining Hawthorn Industrial Park *****
00364> *
00365>
00366> * SUB-AREA No.1
00367>
00368> | CALIB STANDHYD | Area (ha)= 19.90
00369> | 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00370>
00371> IMPERVIOUS PERVIOUS (i)
00372> Surface Area (ha)= 14.13 5.77
00373> Dep. Storage (mm)= 1.57 4.67
00374> Average Slope (%)= .60 1.50
00375> Length (m)= 580.00 100.00
00376> Mannings n = .030 .250
00377>
00378> Max.eff.Inten.(mm/hr)= 96.53 119.96
00379> over (min)= 15.00 27.50
00380> Storage Coeff. (min)= 14.32 (ii) 26.72 (ii)
00381> Unit Hyd. Tpeak (min)= 15.00 27.50
00382> Unit Hyd. peak (cms)= .08 .04
00383>
00384> PEAK FLOW (cms)= 2.14 1.33 *TOTALS*
00385> TIME TO PEAK (hrs)= 1.67 1.92 3.264 (iii)
00386> RUNOFF VOLUME (mm)= 87.29 61.48 74.386
00387> TOTAL RAINFALL (mm)= 88.86 88.86 88.857
00388> RUNOFF COEFFICIENT = .98 .69 .837
00389>
00390> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00391> CN* = 81.0 Ia = Dep. Storage (Above)
00392> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00393> THAN THE STORAGE COEFFICIENT.
00394> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00395>
00396>
00397>
00398> 001:0015-----
00399> | ADD HYD (H1P02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00400> | (ha) (cms) (hrs) (mm) (cms)
00401> ID1 10:POND 8.56 .496 2.13 84.61 .000
00402> +ID2 01:H1P01 19.90 3.264 1.71 74.39 .000
00403>
00404> SUM 02:H1P02 28.46 3.642 1.75 77.46 .000
00405>

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00406> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00407>
00408>
00409>
00410> 001:0016-----
00411> * SUB-AREA No.2
00412>
00413> | CALIB STANDHYD | Area (ha)= 17.00
00414> | 03:H1P03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00415>
00416> IMPERVIOUS PERVIOUS (i)
00417> Surface Area (ha)= 12.07 4.93
00418> Dep. Storage (mm)= 1.57 4.67
00419> Average Slope (%)= .65 1.50
00420> Length (m)= 450.00 100.00
00421> Mannings n = .030 .250
00422>
00423> Max.eff.Inten.(mm/hr)= 100.60 125.35
00424> over (min)= 12.50 25.00
00425> Storage Coeff. (min)= 11.81 (ii) 23.99 (ii)
00426> Unit Hyd. Tpeak (min)= 12.50 25.00
00427> Unit Hyd. peak (cms)= .09 .05
00428>
00429> PEAK FLOW (cms)= 1.92 1.20 *TOTALS*
00430> TIME TO PEAK (hrs)= 1.63 1.88 2.923 (iii)
00431> RUNOFF VOLUME (mm)= 87.29 61.48 74.386
00432> TOTAL RAINFALL (mm)= 88.86 88.86 88.857
00433> RUNOFF COEFFICIENT = .98 .69 .837
00434>
00435> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00436> CN* = 81.0 Ia = Dep. Storage (Above)
00437> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00438> THAN THE STORAGE COEFFICIENT.
00439> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00440>
00441>
00442>
00443> 001:0017-----
00444> * SUB-AREA No.3
00445>
00446> | CALIB STANDHYD | Area (ha)= 15.60
00447> | 04:H1P04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00448>
00449> IMPERVIOUS PERVIOUS (i)
00450> Surface Area (ha)= 11.08 4.52
00451> Dep. Storage (mm)= 1.57 4.67
00452> Average Slope (%)= .50 1.50
00453> Length (m)= 600.00 100.00
00454> Mannings n = .030 .250
00455>
00456> Max.eff.Inten.(mm/hr)= 96.53 119.96
00457> over (min)= 15.00 27.50
00458> Storage Coeff. (min)= 15.44 (ii) 27.83 (ii)
00459> Unit Hyd. Tpeak (min)= 15.00 27.50
00460> Unit Hyd. peak (cms)= .07 .04
00461>
00462> PEAK FLOW (cms)= 1.64 1.03 *TOTALS*
00463> TIME TO PEAK (hrs)= 1.67 1.92 2.519 (iii)
00464> RUNOFF VOLUME (mm)= 87.29 61.48 74.386
00465> TOTAL RAINFALL (mm)= 88.86 88.86 88.857
00466> RUNOFF COEFFICIENT = .98 .69 .837
00467>
00468> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00469> CN* = 81.0 Ia = Dep. Storage (Above)
00470> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00471> THAN THE STORAGE COEFFICIENT.
00472> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00473>
00474>
00475>
00476> 001:0018-----
00477> | ADD HYD (H1P05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00478> | (ha) (cms) (hrs) (mm) (cms)
00479> ID1 03:H1P03 17.00 2.923 1.67 74.39 .000
00480> +ID2 04:H1P04 15.60 2.519 1.75 74.39 .000
00481>
00482> SUM 05:H1P05 32.60 5.435 1.71 74.39 .000
00483>
00484> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00485>
00486>
00487>
00488> 001:0019-----
00489> | ADD HYD (H1P06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00490> | (ha) (cms) (hrs) (mm) (cms)
00491> ID1 05:H1P05 32.60 5.435 1.71 74.39 .000
00492> +ID2 02:H1P02 28.46 3.642 1.75 77.46 .000
00493>
00494> SUM 06:H1P06 61.06 9.050 1.74 75.82 .000
00495>
00496>
00497> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00498>
00499>
00500>
00501> 001:0020-----
00502> * SUB-AREA No.4
00503>
00504> | CALIB STANDHYD | Area (ha)= 12.20
00505> | 07:H1P07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00506>
00507> IMPERVIOUS PERVIOUS (i)
00508> Surface Area (ha)= 8.66 3.54
00509> Dep. Storage (mm)= 1.57 4.67
00510> Average Slope (%)= .70 1.50
00511> Length (m)= 210.00 100.00
00512> Mannings n = .030 .250
00513>
00514> Max.eff.Inten.(mm/hr)= 106.70 131.04
00515> over (min)= 7.50 20.00
00516> Storage Coeff. (min)= 7.14 (ii) 19.11 (ii)
00517> Unit Hyd. Tpeak (min)= 7.50 20.00
00518> Unit Hyd. peak (cms)= .15 .06
00519>
00520> PEAK FLOW (cms)= 1.56 .95 *TOTALS*
00521> TIME TO PEAK (hrs)= 1.54 1.79 2.287 (iii)
00522> RUNOFF VOLUME (mm)= 87.29 61.48 74.386
00523> TOTAL RAINFALL (mm)= 88.86 88.86 88.857
00524> RUNOFF COEFFICIENT = .98 .69 .837
00525>
00526> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00527> CN* = 81.0 Ia = Dep. Storage (Above)
00528> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00529> THAN THE STORAGE COEFFICIENT.
00530> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00531>
00532>
00533> 001:0021-----
00534> * SUB-AREA No.5
00535>
00536> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
00537> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
00538> U.H. Tp (hrs)= .170
00539>
00540>

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00541> Unit Hyd Qpeak (cms)= .899
00542>
00543> PEAK FLOW (cms)= .721 (i)
00544> TIME TO PEAK (hrs)= 1.667
00545> RUNOFF VOLUME (mm)= 54.937
00546> TOTAL RAINFALL (mm)= 88.857
00547> RUNOFF COEFFICIENT = .618
00548>
00549> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00550>
00551> -----
00552> 001:0022-----
00553>
00554> | ADD HYD (HIP08 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00555> |-----|-----|-----|-----|-----|-----|
00556> | ID1 06:HIP06 61.06 9.050 1.74 75.82 .000
00557> | +ID2 07:HIP07 12.20 2.287 1.58 74.39 .000
00558> | +ID3 08:Pond-B 4.00 .721 1.67 54.94 .000
00559> |-----|-----|-----|-----|-----|-----|
00560> | SUM 09:HIP08 77.26 11.944 1.71 74.51 .000
00561>
00562> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00563>
00564> -----
00565> 001:0023-----
00566>
00567> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00568> | IN>09: (HIP08 ) |
00569> | OUT<10: (HIP-PO) |
00570> |-----|-----|-----|-----|-----|-----|
00571> | OUTFLOW STORAGE | OUTFLOW STORAGE |
00572> | (cms) (ha.m.) | (cms) (ha.m.) |
00573> | .000 .0000E+00 | .724 .2210E+01 |
00574> | .048 .5740E+01 | .937 .2501E+01 |
00575> | .054 .2454E+00 | 1.262 .3798E+01 |
00576> | .059 .5834E+00 | 1.404 .3101E+01 |
00577> | .062 .8400E+00 | 1.532 .3410E+01 |
00578> | .064 .1102E+01 | 1.650 .3724E+01 |
00579> | .147 .1370E+01 | 2.409 .4044E+01 |
00580> | .280 .1644E+01 | 3.689 .4370E+01 |
00581> | .472 .1924E+01 | .000 .0000E+00 |
00582>
00583> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00584> | IN>09: (HIP08 ) 77.26 11.944 1.708 74.508
00585> | OUTFLOW<10: (HIP-PO) 77.26 2.666 2.625 74.508
00586>
00587> PEAK FLOW REDUCTION [Qout/qin] (%)= 22.321
00588> TIME SHIFT OF PEAK FLOW (min)= 55.00
00589> MAXIMUM STORAGE USED (ha.m.)=.4310E+01
00590>
00591> -----
00592> 001:0024-----
00593> *
00594> *SUB-AREA No. 6
00595>
00596> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
00597> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res.(N)= 3.00
00598> | U.H. Tp(hrs)= .800
00599>
00600> Unit Hyd Qpeak (cms)= .129
00601>
00602> PEAK FLOW (cms)= .180 (i)
00603> TIME TO PEAK (hrs)= 2.333
00604> RUNOFF VOLUME (mm)= 43.111
00605> TOTAL RAINFALL (mm)= 88.857
00606> RUNOFF COEFFICIENT = .485
00607>
00608> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00609>
00610> -----
00611> 001:0025-----
00612>
00613> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00614> |-----|-----|-----|-----|-----|-----|
00615> | ID1 10:HIP-PO 77.26 2.666 2.63 74.51 .000
00616> | +ID2 01:A3 2.70 .180 2.33 43.11 .000
00617> |-----|-----|-----|-----|-----|-----|
00618> | SUM 02:Ultima 79.96 2.830 2.61 73.45 .000
00619>
00620> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00621>
00622> -----
00623> 001:0026-----
00624> FINISH
00625>
00626> *****
00627> WARNINGS / ERRORS / NOTES
00628>
00629> Simulation ended on 2009-05-15 at 09:03:53
00630>
00631>
00632>

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**CERTIFICATE OF APPROVAL**  
**MUNICIPAL AND PRIVATE SEWAGE WORKS**  
NUMBER 4660-7UNPRJ  
Issue Date: November 9, 2009

Tomlinson Development Corporation  
5597 Power Rd  
Ottawa, Ontario K1G 3N4

Site Location: Hawthorne Industrial Park (HIP) - Phase 1  
Lot 26 and 27, Concession 6 (R.F.)  
City of Ottawa, Ontario

*You have applied in accordance with Section 53 of the Ontario Water Resources Act for approval of:*

the establishment of sewage works for the collection, transmission, treatment and disposal of stormwater runoff from a catchment area of approximately 70 hectares, servicing the Hawthorne Industrial Park, located immediately southeast of the Hawthorne Road/Rideau Road intersection in the City of Ottawa, to provide partial water quality protection (Normal Protection Level) and to attenuate post-development peak flows to pre-development levels, discharging to Findlay Creek, which is a tributary to the North Castor River, for all storm events up to and including the 100 year return storm, consisting of the following stormwater works:

**Stormwater Management System**Outlet No. 1, HIP to a dry pond facility (Service area of 69.81 ha):

- A dry pond facility to provide quantity control by attenuating post development peak flows to pre-development levels for all storm events up to and including the 100 year return storm, having a design minimum liquid retention volume of approximately 37,240 m<sup>3</sup> at elevation 86.15 m (0.23 m above 100-year surface pond elevation), with side slopes of 4:1, and servicing approximately 69.81 hectares, which includes Orgaworld Canada Ltd's stormwater treated effluent (10.14 ha). The SWM pond is designed to provide a controlled maximum discharge flow rate of 1,531 L/s for the 100-year storm event, discharging to Findlay Creek; and equipped with:
  - An outlet structure consisting of a 150 mm diameter orifice within a 200 mm diameter polyvinyl chloride (PVC) pipe at an invert elevation of 82.90 m, which serves as outlet to the facility;
  - Two (2) 600 mm diameter corrugated steel pipe (CSP) culvert placed at an invert elevation of 84.80 m, which also serves as an outlet to the facility; and
  - An emergency spillway of 0.35 m deep with a 6.0 m wide base to convey surface flow toward the



receiving channel during extreme storm events.

- The simulated modelling estimate and drainage pattern draining to Outlet No.1 is as follows:

<b>Storm Events</b> (catchment for Outlet #1 – 70 ha)	<b>2-year</b>	<b>5-year</b>	<b>25-year</b>	<b>100-year</b>
Existing flows, pre-development (m <sup>3</sup> /s.)	0.467	0.826	1.468	2.093
Post-development flows (m <sup>3</sup> /s)	3.077	4.812	7.772	10.662
Post-development attenuated flows (m <sup>3</sup> /s)	0.194	0.359	0.939	1.531

- A new roadside ditch system draining to the dry pond facility, equipped with CSP culverts and approximately 1,755 m of 200 mm diameter HDPE perforated pipe sub-drains and clear stone bedding wrapped in geotextile located at the base of the ditches to meet a Normal water quality Protection Level (70% Total Suspended Solids removal) for the contributing catchment area of 1.58 ha which includes the paved portion of the industrial park road network located within the subdivision right-of-way as per the SWM Report (J.L.Richards, 2009).
- The requirement for quality protection for the remaining 68.23 ha is provided by the individual industrial lots within HIP as per the following Certificates of Approval (this list will be amended as future CofAs for other lots within HIP are developed, as per Condition 7 of this Certificate):
  - CofA # 9465-7NVRWT, issued on September 16, 2009, providing Normal water quality Protection Level for 10.14 ha.

Outlet No.2, to Findlay Creek (Service area of 39.16 ha):

- A new roadside ditch system draining to Findlay Creek via an existing roadside ditch located adjacent to Rideau Road, servicing a catchment area along the Hawthorne Road extension and includes the Tomlinson Quarry, as per the SWM Report (J.L.Richards, 2009). This service area is not part of the HIP site.

All including erosion/sedimentation control measures during construction and all other controls and appurtenances essential for the proper operation of the aforementioned *Works* ;

all in accordance with the following supporting documents:

1. Application for Approval of Industrial Sewage Works submitted by Domenic Idone, P.Eng., Planning Engineer of Tomlinson Development Corporation, dated March 12, 2009, and received on June 8 , 2009;
2. Stormwater Management Report - Hawthorne Industrial Park, dated February 2009 (revised May 2009), and prepared by J.L Richards & Associates Limited.
3. Geotechnical Study Subdivision Plan - Hawthorne Industrial Park, Lots 26 and 27, Concession 6, Southeast of Hawthorne and Rideau Roads, Ottawa, dated May 4, 2009, and prepared by



Inspec-Sol Inc.

4. Certificate of Approval 6924-5YWQ3U, issued on May 19, 2004, for R.W. Tomlinson Limited for a lagoon system to treat sewage from the Tomlinson Quarry.
5. s.53 OWRA Certificate of Approval, Orgaworld Canada Ltd. (9465-7NVRWT, issued on September 16, 2009).
6. Revised Fish Habitat Enhancement Strategy - Hawthorne Industrial Park Stormwater Management Pond, prepared by Stantec (Jacques Whitford Stantec Limited), dated May 13, 2009.
7. Clearance Letter from the South Nation Conservation dated May 26, 2009, issued to the City of Ottawa for the Tomlinson / Hawthorne Industrial Park Subdivision.
8. Emails from Derrick P. Upton, P.Eng., of J.L. Richards & Associates Limited to Edgardo Tovilla, P.Eng., of the MOE, dated August 7 & 11, 2009, with additional information requested.
9. Letter from Derrick P. Upton, P.Eng., of J.L. Richards & Associates Limited to Edgardo Tovilla, P.Eng., of the MOE, dated August 31, 2009, with additional information requested.
10. Email from Tim Chadder of J.L. Richards & Associates Limited to Edgardo Tovilla, P.Eng., of the MOE, dated October 9, 2009, with final comments to the CofA.

*For the purpose of this Certificate of Approval and the terms and conditions specified below, the following definitions apply:*

"*Certificate* " means this entire certificate of approval document, issued in accordance with Section 53 of the Ontario Water Resources Act, and includes any schedules;

"*Director* " means any *Ministry* employee appointed by the Minister pursuant to section 5 of the Ontario Water Resources Act;

"*District Manager* " means the District Manager of the Ottawa District Office of the *Ministry* ;

"*Ministry* " means the Ontario Ministry of the Environment;

"*Owner* " means Tomlinson Development Corporation and includes its successors and assignees; and

"*Works* " means the sewage works described in the *Owner* 's application, this *Certificate* and in the supporting documentation referred to herein, to the extent approved by this *Certificate* .

*You are hereby notified that this approval is issued to you subject to the terms and conditions outlined below:*

## TERMS AND CONDITIONS

### 1. GENERAL PROVISIONS

(1) Except as otherwise provided by these Conditions, the *Owner* shall design, build, install, operate and maintain the *Works* in accordance with the description given in this *Certificate*, the application for approval of the works and the submitted supporting documents and plans and specifications as listed in this *Certificate*.

(2) Where there is a conflict between a provision of any submitted document referred to in this *Certificate* and the Conditions of this *Certificate*, the Conditions in this *Certificate* shall take precedence, and where there is a conflict between the listed submitted documents, the document bearing the most recent date shall prevail.

(3) Where there is a conflict between the listed submitted documents, and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.

### 2. EXPIRY OF APPROVAL

The approval issued by this *Certificate* will cease to apply to those parts of the *Works* which have not been constructed within five (5) years of the date of this *Certificate*.

### 3. CHANGE OF OWNER

The *Owner* shall notify the *District Manager* and the *Director*, in writing, of any of the following changes within thirty (30) days of the change occurring:

(a) change of *Owner* ;

(b) change of address of the *Owner* ;

(c) change of partners where the *Owner* is or at any time becomes a partnership, and a copy of the most recent declaration filed under the Business Names Act, R.S.O. 1990, c.B17 shall be included in the notification to the *District Manager* ; and

(d) change of name of the corporation where the *Owner* is or at any time becomes a corporation, and a copy of the most current information filed under the Corporations Information Act, R.S.O. 1990, c. C39 shall be included in the notification to the *District Manager* .

### 4. OPERATION AND MAINTENANCE.

(1) The *Owner* shall ensure that the design minimum liquid retention volume(s) is maintained at all times.

(2) The *Owner* shall inspect the *Works* at least once a year and, if necessary, clean and maintain the



*Works* to prevent the excessive build-up of sediments and/or vegetation.

(3) The *Owner* shall maintain a logbook to record the results of these inspections and any cleaning and maintenance operations undertaken, and shall keep the logbook at the Owner's office for inspection by the *Ministry*. The logbook shall include the following:

(a) the name of the *Works* ;

(b) the date and results of each inspection, maintenance, monitoring reports and cleaning, including an estimate of the quantity of any materials removed; and

(c) the date of each spill within the catchment area, including follow-up actions / remedial measures undertaken.

(4) The *Owner* shall operate the *Works* with an objective of achieving Normal water quality Protection Level (70% long-term Total Suspended Solids removal) for the portion of the land being treated with the proposed Works.

## 5. MONITORING AND RECORDING

The *Owner* shall, upon commencement of operation of the *Works* , carry out the following monitoring program:

(1) All samples and measurements taken for the purposes of this *Certificate* are to be taken at a time and in a location characteristic of the quality and quantity of the effluent stream over the time period being monitored.

(2) For the purposes of this condition, Semi-annually means once twice per year;

(3) Samples shall be collected at the following sampling points, at the frequency specified, by means of the specified sample type and analyzed for each parameter listed and all results recorded:

<b>Table 1 - Surface Water Monitoring</b>	
Sample location: at the inlet of the dry pond facility	
<b>Frequency</b>	Semi-annually; at least once being for the snowmelt freshets and another being 72 hours after the fall of precipitation of more than 25 mm.
<b>Sample Type</b>	Grab
<b>Parameters</b>	CBOD5 , Total Suspended Solids, Total Phosphorus, <i>E. Coli</i> , pH, Temperature, Acute Lethality.

(4) The methods and protocols for sampling, analysis and recording shall conform, in order of precedence, to the methods and protocols specified in the following:

(a) the Ministry's Procedure F-10-1, "Procedures for Sampling and Analysis Requirements for Municipal and Private Sewage Treatment Works (Liquid Waste Streams Only), as amended from

time to time by more recently published editions;

(b) the Ministry's publication "Protocol for the Sampling and Analysis of Industrial/Municipal Wastewater" (January 1999), ISBN 0-7778-1880-9, as amended from time to time by more recently published editions;

(c) the publication "Standard Methods for the Examination of Water and Wastewater" (21st edition), as amended from time to time by more recently published editions;

(d) the Environment Canada publications "Biological Test Method: Reference Method for Determining Acute Lethality of Effluents to Rainbow Trout" (July 1990) and "Biological Test Method: Reference Method for Determining Acute Lethality of Effluents to Daphnia magna" (July 1990), as amended from time to time by more recently published editions; and,

(6) The measurement frequencies and the overall monitoring program specified in subsection (3) are minimum requirements which may, after three (3) years of monitoring in accordance with this Condition or after a minimum 75% build-up of the site, whichever occurs first, be modified by the *District Manager* in writing from time to time.

(7) The *Owner* shall retain for a minimum of three (3) years from the date of their creation, all records and information related to or resulting from the monitoring activities required by this *Certificate* .

(8) The *Owner* shall enter into an agreement with the owner of the composting facility located within HIP, located at Part of Lot 27, Concession 6, 5123 Hawthorne Road, for the long-term access to private wells for its operation, maintenance and testing to ensure that the provisions of a groundwater monitoring program can be administered. A copy of such Agreement shall be provided to the *District Manager* prior to the commencement of operation of the *Works* .

## 6. RECORD KEEPING

The *Owner* shall retain for a minimum of five (5) years from the date of their creation, all records and information related to or resulting from the operation and maintenance and activities required by this *Certificate* .

## 7. SPECIAL CONDITION

(1) The *Owner* shall ensure through the Site Plan Approval process that individual lots developed within the industrial park will obtain a approval, in accordance with section 53 of the OWRA, before discharging into the roadside ditches and ultimately to the dry pond facility.

(2) The *Owner* shall not approve any additional flow from storm sewers, catchbasin leads, and storm service drains to the individual industrial plots to connect with the dry pond unless this Certificate of Approval is amended with adequate quality treatment proposed via provision of additional sewage treatment works, best management practices and hydraulic capacity servicing them has been designed and reviewed by the Ministry concluding that the additional quality of stormwater will not overload the



downstream collection system, pond and/or alter the stormwater quality of effluent discharged to the receiver of this *Certificate*.

*The reasons for the imposition of these terms and conditions are as follows:*

1. Condition 1 is imposed to ensure that the *Works* are built and operated in the manner in which they were described for review and upon which approval was granted. This condition is also included to emphasize the precedence of Conditions in the *Certificate* and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
2. Condition 2 is included to ensure that the *Works* are constructed in a timely manner so that standards applicable at the time of Approval of the *Works* are still applicable at the time of construction, to ensure the ongoing protection of the environment
3. Condition 3 is included to ensure that the *Ministry* records are kept accurate and current with respect to approved works and to ensure that subsequent owners of the works are made aware of the certificate and continue to operate the works in compliance with it.
4. Condition 4 is included to require that the *Works* be properly operated and maintained such that the environment is protected .
5. Conditions 5 and 7 are included to enable the *Owner* to evaluate and demonstrate the performance of the *Works* , on a continual basis, so that the *Works* are properly operated and maintained at a level which is consistent with the design objectives specified in the *Certificate* and that the *Works* does not cause any impairment to the receiving watercourse.
6. Condition 6 is included to require that all records are retained for a sufficient time period to adequately evaluate the long-term operation and maintenance of the *Works* .

*In accordance with Section 100 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, as amended, you may by written notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 101 of the Ontario Water Resources Act , R.S.O. 1990, Chapter 0.40, provides that the Notice requiring the hearing shall state:*

1. The portions of the approval or each term or condition in the approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

*The Notice should also include:*

3. The name of the appellant;
4. The address of the appellant;
5. The Certificate of Approval number;
6. The date of the Certificate of Approval;
7. The name of the Director;
8. The municipality within which the works are located;

*And the Notice should be signed and dated by the appellant.*

*This Notice must be served upon:*

The Secretary\*  
Environmental Review Tribunal  
655 Bay Street, 15th Floor  
Toronto, Ontario  
M5G 1E5

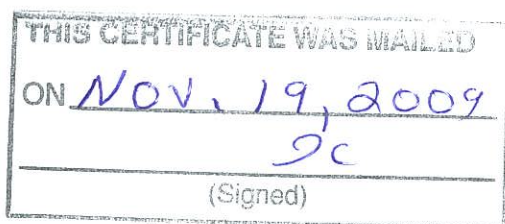
AND

The Director  
Section 53, *Ontario Water Resources Act*  
Ministry of the Environment  
2 St. Clair Avenue West, Floor 12A  
Toronto, Ontario  
M4V 1L5

\* Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 314-4600, Fax: (416) 314-4506 or [www.ert.gov.on.ca](http://www.ert.gov.on.ca)

*The above noted sewage works are approved under Section 53 of the Ontario Water Resources Act.*

DATED AT TORONTO this 9th day of November, 2009



\_\_\_\_\_  
Mansoor Mahmood, P.Eng.  
Director  
Section 53, *Ontario Water Resources Act*

ET/

c: District Manager, MOE Ottawa District Office  
Derrick Upton, P.Eng., J.L. Richards & Associates Limited ✓

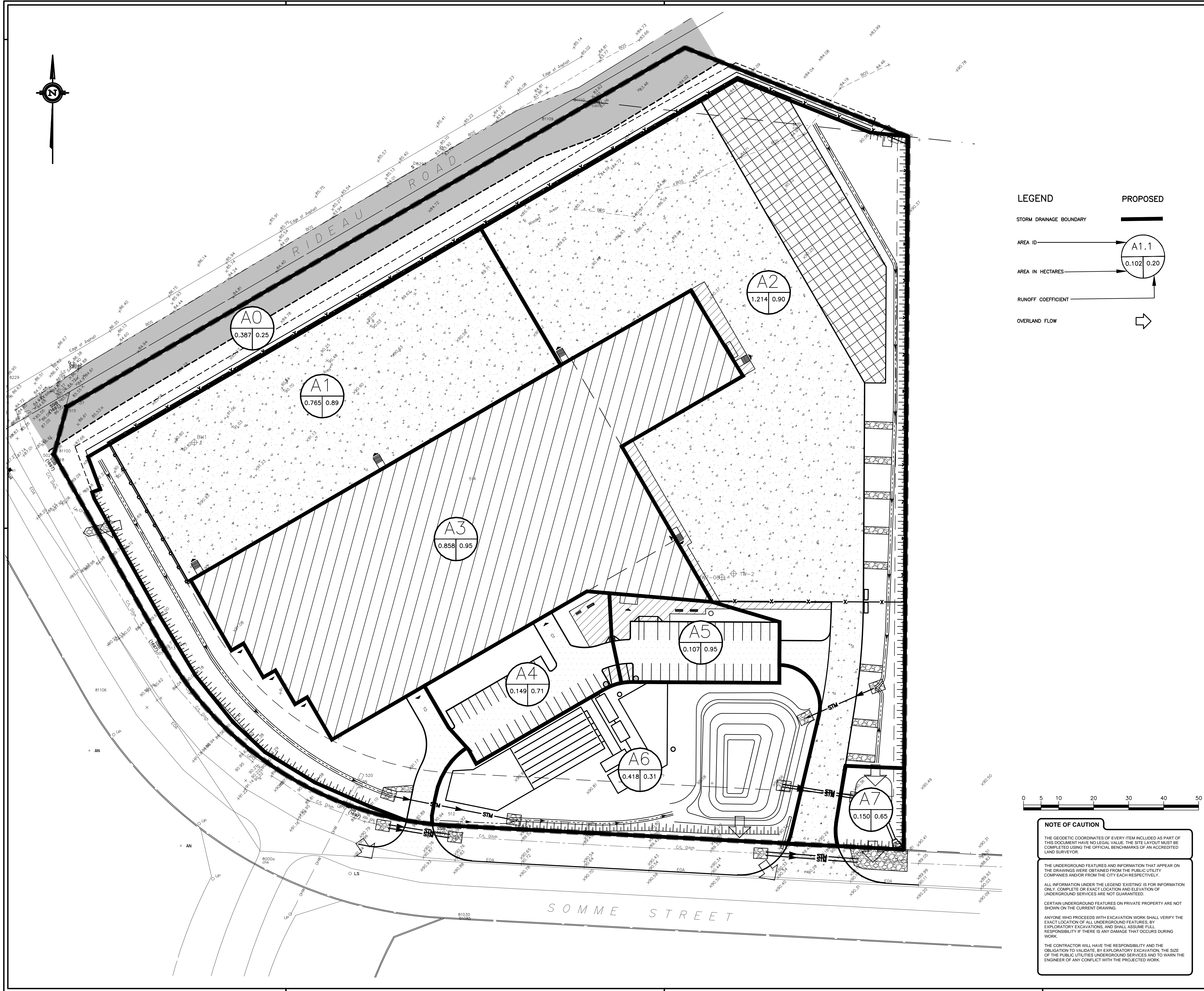
RECEIVED  
NOV 23 2009

J.L. Richards & Associates Limited  
OTTAWA OFFICE

# B

## Appendix B - Stormwater Management Plan





**LEGEND**

STORM DRAINAGE BOUNDARY

AREA ID

AREA IN HECTARES

RUNOFF COEFFICIENT

OVERLAND FLOW

**PROPOSED**

A1.1

0.102 0.20

⇒



**NOTE OF CAUTION**

THE GEODETIC COORDINATES OF EVERY ITEM INCLUDED AS PART OF THIS DOCUMENT HAVE NO LEGAL VALUE. THE SITE LAYOUT MUST BE COMPLETED USING THE OFFICIAL BENCHMARKS OF AN ACCREDITED LAND SURVEYOR.

THE UNDERGROUND FEATURES AND INFORMATION THAT APPEAR ON THE DRAWINGS WERE OBTAINED FROM THE PUBLIC UTILITY COMPANIES AND/OR FROM THE CITY EACH RESPECTIVELY.

ALL INFORMATION UNDER THE LEGEND 'EXISTING' IS FOR INFORMATION ONLY. COMPLETE OR EXACT LOCATION AND ELEVATION OF UNDERGROUND SERVICES ARE NOT GUARANTEED.

CERTAIN UNDERGROUND FEATURES ON PRIVATE PROPERTY ARE NOT SHOWN ON THE CURRENT DRAWINGS.

ANYONE WHO PROCEEDS WITH EXCAVATION WORK SHALL VERIFY THE EXACT LOCATION OF ALL UNDERGROUND FEATURES, BY EXPLORATORY EXCAVATIONS, AND SHALL ASSUME FULL RESPONSIBILITY IF THERE IS ANY DAMAGE THAT OCCURS DURING WORK.

THE CONTRACTOR WILL HAVE THE RESPONSIBILITY AND THE OBLIGATION TO VALIDATE, BY EXPLORATORY EXCAVATION, THE SIZE OF THE PUBLIC UTILITIES UNDERGROUND SERVICES AND TO WARN THE ENGINEER OF ANY CONFLICT WITH THE PROJECTED WORK.

**Key Plan**

**1**

no.	date	revision/issue	by
1	JULY 26, 2021	ISSUED FOR REVIEW	

**CIVITAS GROUP**  
ARCHITECTURE & LANDSCAPE ARCHITECTURE

consultant

**CIMA+**

northpoint

professional stamp

project title

**SOMME STREET, OTTAWA, ONTARIO  
FASTRATE FACILITY**

drawing title

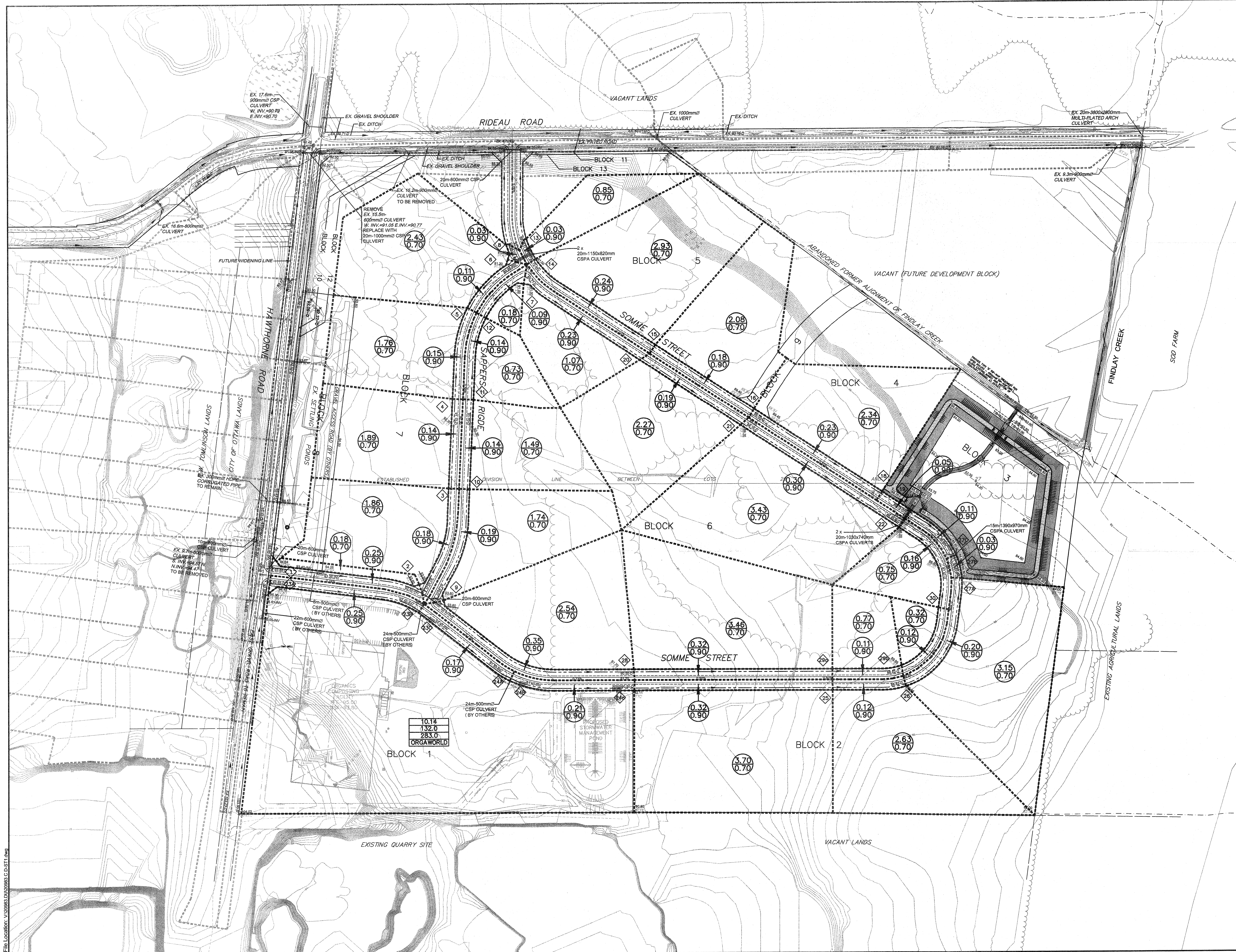
**STORMWATER  
MANAGEMENT PLAN**

date	MARCH 08, 2021	job no.	A001083
scale	1 : 500	drawn	D.CANN
approved	J.SAUVÉ	plot date	1/13/2021 3:31:05 PM

**SWM**

1. DO NOT SCALE FROM THIS DRAWING  
2. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE WORK COMMENCES  
3. THIS DRAWING TO BE READ IN CONJUNCTION WITH THE FOLLOWING DRAWINGS: STRUCTURAL, MECHANICAL, ELECTRICAL





Key Map

NOT TO SCALE

**LEGEND**

----- DRAINAGE BOUNDARY

2.91  
0.70

AREA IN HECTARES

\* RUNOFF COEFFICIENT (C)

10.14  
132.0  
283.0

DRAINAGE AREA (ha)

10 YEAR PEAK FLOW (l/s)

100 YEAR PEAK FLOW (l/s)

ORGAWORLD SITE

28

NODE LOCATION NUMBER

PROPOSED DITCH AND FLOW DIRECTION

NOTE: RUNOFF COEFFICIENT (C) FOR DEVELOPMENT AREA IS BASED ON A WEIGHTED AVERAGE OF 0.70, WHILE ROADWAYS ARE 0.90.

03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

NO.	ISSUE	DATE
03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

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SCALE: 1:2000

0 25 50 100 150

**J.L. Richards & Associates Limited**  
ENGINEERS ARCHITECTS PLANNERS

J.L. Richards & Associates Limited  
203-863 Princess Street  
Kingston, ON Canada  
K7L 5N4  
Tel: 613 544 1424  
Fax: 613 544 5679

PROFESSIONAL STAMP

PROJECT NORTH

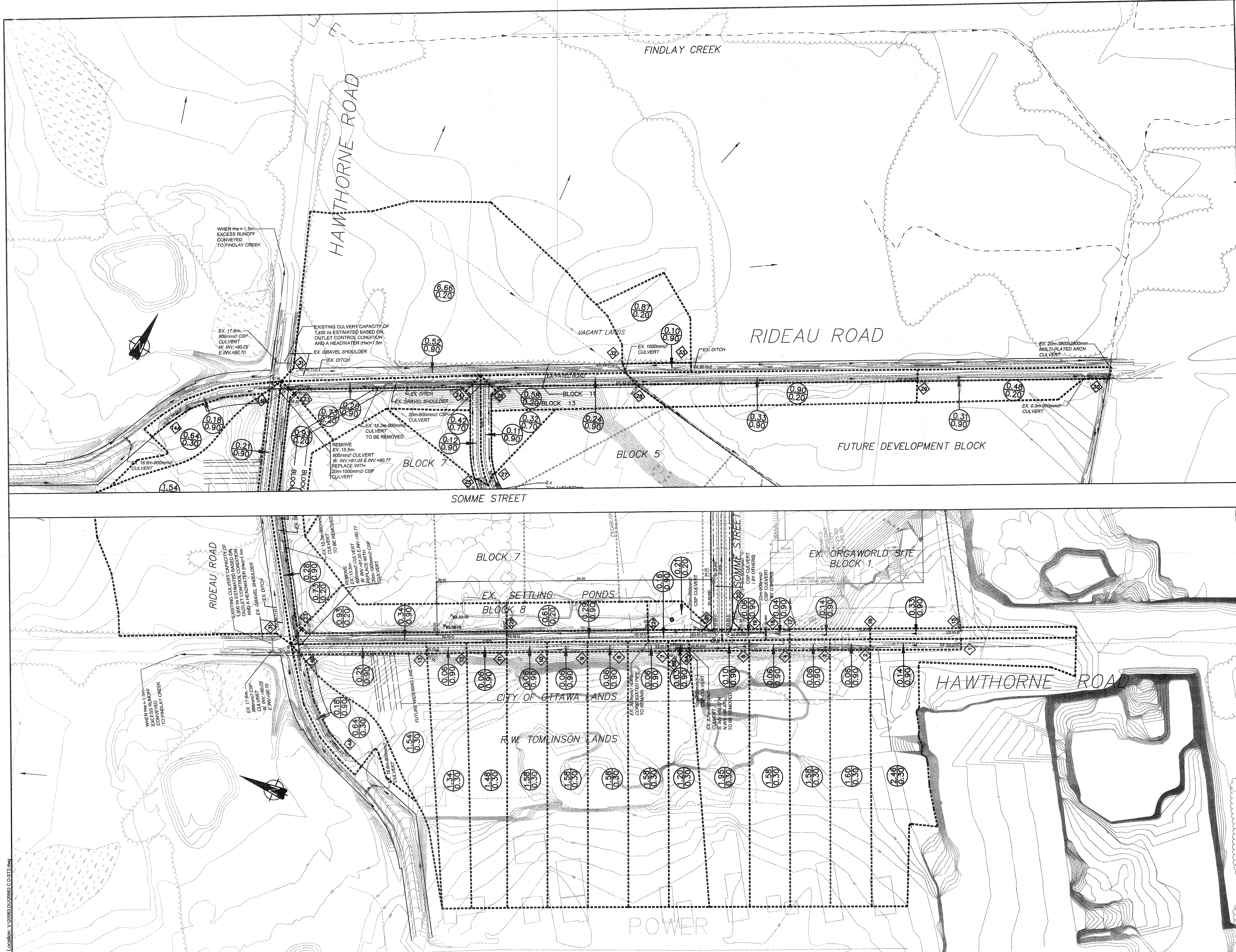
PROJECT:

**HAWTHORNE INDUSTRIAL PARK**

DRAWING:

**STORM DRAINAGE AREA PLAN**





**Key Map**

NOT TO SCALE

**LEGEND**

- DRAINAGE BOUNDARY
- 0.91 / 0.70 AREA IN HECTARES \* RUNOFF COEFFICIENT (C)
- 28 NODE LOCATION NUMBER
- PROPOSED DITCH AND FLOW DIRECTION
- EXISTING SURFACE FLOW DIRECTION

\* NOTE: RUNOFF COEFFICIENT (C) FOR DEVELOPMENT AREA IS BASED ON A WEIGHTED AVERAGE OF 0.70, WHILE ROADWAYS ARE 0.90.

NO.	ISSUE	DATE
03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
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NO. ISSUE DATE

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SCALE: 1:2000

**J.L. Richards & Associates Limited**  
 203-863 Princess Street  
 Kingston, ON Canada  
 K7L 5N4  
 Tel: 613 544 1424  
 Fax: 613 544 5679

**PROFESSIONAL STAMP**

**PROJECT NORTH**

**PROJECT:**

**HAWTHORNE INDUSTRIAL PARK**

**DRAWING:**

**STORM DRAINAGE AREA PLAN**

DESIGN: M.B.	DRAWING NO.: <b>D-ST2</b>
DRAWN: T.S.	JLR NO:
CHECKED: D.U.	20983
PLOTTED: May 28, 2009	



Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA								CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)				
			Area at C of		SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL				HW 1:10 (m)			
	0.70 (ha)	0.90 (ha)																															
NORTHERN CATCHMENT AREA																																	
WEST SIDE SAPPERS RIDGE	2	3	1.86	0.18	2.04	1.46	1.46	4.07	4.07	15.00	97.85	398.2	0.00	0.42	1.20	3.00	0.50	424.2	6973.0	0.80	136.80							2.84	92.50	91.82			
WEST SIDE SAPPERS RIDGE	3	4	1.89	0.14	2.03	1.45	2.92	4.04	8.11	17.84	88.22	715.4	0.00	0.51	1.20	3.00	0.80	904.2	8856.1	1.16	111.00							1.60	91.82	90.93			
WEST SIDE SAPPERS RIDGE	4	5	1.76	0.15	1.91	1.36	4.28	3.79	11.90	19.44	83.68	995.9	0.00	0.58	1.20	3.00	0.51	1011.3	7029.1	1.00	112.85							1.88	90.93	90.36			
WEST SIDE SAPPERS RIDGE	5	6	2.43	0.11	2.54	1.80	6.08	5.00	16.90	21.32	78.96	1334.4	0.00	0.65	1.20	3.00	0.62	1513.4	7762.6	1.19	82.79							1.16	90.36	89.85			
										22.47																							
NORTH ENTRANCE TO SOMME STREET	8	6		0.03	0.03	0.03	0.03	0.08	0.08	15.00	97.85	7.3	0.00	0.20	1.20	3.00	1.30	94.9	11276.7	0.79	10.00							0.21	89.98	89.85			
										15.21																							
CULVERT CROSSING	6	14		0.00	0.00	0.00	6.11	0.00	16.97	22.47	76.34	1295.8					0.50				20.00	2	----	1.15 x 0.82	NO	YES	0.75	0.38	89.85	89.75			
										22.85																							
NORTH PORTION SOMME STREET	13	14	0.85	0.03	0.88	0.62	0.62	1.73	1.73	15.00	97.85	169.2	0.00	0.30	1.20	3.00	2.30	372.0	14999.4	1.38	10.00							0.12	89.98	89.75			
										15.12																							
NORTH PORTION SOMME STREET	14	15	2.93	0.24	3.17	2.27	8.99	6.30	25.00	22.85	75.52	1888.2	0.00	0.74	1.20	3.00	0.50	1926.6	6992.8	1.17	184.04							2.62	89.75	88.83			
NORTH PORTION SOMME STREET	15	16	2.08	0.18	2.26	1.62	10.61	4.50	29.50	25.47	70.36	2075.4	0.00	0.77	1.20	3.00	0.57	2291.4	7480.8	1.29	145.08							1.88	88.83	88.00			
NORTH PORTION SOMME STREET	16	18	2.34	0.23	2.57	1.85	12.46	5.13	34.63	27.35	67.11	2323.9	0.00	0.80	1.20	3.00	0.51	2399.6	7074.8	1.25	185.66							2.48	88.00	87.05			
NORTH PORTION SOMME STREET	18	19	0.00	0.05	0.05	0.05	12.50	0.13	34.75	29.82	63.30	2199.9	0.00	0.76	1.20	3.00	0.72	2476.8	8372.8	1.43	41.86							0.49	87.05	86.75			
										30.31																							
EAST SIDE SAPPERS RIDGE	9	10	1.74	0.19	1.93	1.39	1.39	3.86	3.86	15.00	97.85	378.0	0.00	0.41	1.20	3.00	0.50	399.2	6996.6	0.79	147.87							3.11	92.40	91.66			
EAST SIDE SAPPERS RIDGE	10	11	1.49	0.14	1.63	1.17	2.56	3.25	7.11	18.11	87.42	622.0	0.00	0.49	1.20	3.00	0.66	735.9	8019.2	1.02	111.04							1.81	91.66	90.93			
EAST SIDE SAPPERS RIDGE	11	12	0.73	0.14	0.87	0.64	3.20	1.77	8.88	19.92	82.40	732.0	0.00	0.52	1.20	3.00	0.55	785.5	7304.8	0.97	104.49							1.80	90.93	90.36			
EAST SIDE SAPPERS RIDGE	12	7	0.18	0.09	0.27	0.21	3.40	0.58	9.46	21.72	78.02	738.2	0.00	0.49	1.20	3.00	0.81	818.5	8919.0	1.14	72.55							1.06	90.36	89.77			
NORTH PORTION SOMME STREET	7	20	1.07	0.23	1.30	0.96	4.36	2.66	12.12	22.79	75.66	916.9	0.00	0.57	1.20	3.00	0.50	956.8	6966.1	0.98	177.39							3.01	89.77	88.89			
NORTH PORTION SOMME STREET	20	21	2.27	0.19	2.46	1.76	6.12	4.89	17.01	25.80	69.76	1186.8	0.00	0.62	1.20	3.00	0.50	1200.1	6981.9	1.04	147.49							2.36	88.89	88.16			
NORTH PORTION SOMME STREET	21	22	3.43	0.30	3.73	2.67	8.79	7.43	24.44	28.16	65.80	1608.1	0.00	0.70	1.20	3.00	0.56	1759.0	7404.4	1.20	232.84							3.24	88.16	86.85			
										31.40																							
SOUTHERN CATCHMENT AREA																																	
SOUTH PORTION SOMME STREET	23A	23B	0.00	0.25	0.25	0.23	0.23	0.63	0.63	15.00	97.85	61.2	0.00	0.20	1.20	3.00	0.64	66.3	7883.5	0.55	181.00							5.46	93.65	92.50			
CULVERT CROSSING	23B	23C		0.00	0.00	0.00	0.23	0.00	0.63	20.46	81.05	50.7					0.42				24.00	1	500	----	NO	YES	0.33	1.55	92.50	92.40			
SOUTH PORTION SOMME STREET	23C	24A	0.00	0.17	0.17	0.15	0.38	0.43	1.05	22.00	77.38	81.3	0.00	0.22	1.20	3.00	0.82	97.0	8946.1	0.67	110.00							2.74	92.40	91.50			
CULVERT CROSSING	24A	24B		0.00	0.00	0.00	0.38	0.00	1.05	24.75	71.70	75.3					0.42				24.00	1	500	----	NO	YES	0.34	1.04	91.50	91.40			
SOUTH PORTION SOMME STREET	24B	24C	0.00	0.21	0.21	0.19	0.57	0.53	1.58	25.79	69.78	110.0	0.00	0.25	1.20	3.00	0.70	126.0	8258.2	0.67	142.00							3.52	91.40	90.41			
ORGAWORLD - SITE	U/S	24C	1:10 year peak flow = 132 L/s, see Table 4 of Orgaworld Stormwater Site Management Plan, Sept. 2008										132.0																				
SOUTH PORTION SOMME STREET	24C	25	3.70	0.32	4.02	2.88	3.44	8.00	9.58	29.31	64.05	745.3	0.00	0.52	1.20	3.00	0.54	783.8	7289.5	0.97	244.84							4.22	90.41	89.08			
SOUTH PORTION SOMME STREET	25	26	2.63	0.12	2.75	1.95	5.39	5.42	14.99	33.53	58.41	1007.7	0.00	0.58	1.20	3.00	0.51	1013.1	7041.5	1.00	90.75							1.51	89.08	88.62			
SOUTH PORTION SOMME STREET	26	27A	3.15	0.20	3.35	2.39	7.78	6.63	21.63	35.04	56.65	1357.2	0.00	0.62	1.20	3.00	0.65	1370.0	7970.4	1.19	157.06							2.20	88.62	87.60			
SOUTH PORTION SOMME STREET	27A	27B	0.00	0.03	0.03	0.03	7.81	0.08	21.70	37.24	54.29	1310.1	0.00	0.61	1.20	3.00	0.65	1312.4	7973.8	1.18	20.00							0.28	87.60	87.47			
CULVERT CROSSING	27B	27C		0.00	0.00	0.00	7.81	0.00	21.70	37.53	54.00	1303.8					0.73				15.00	1	----	1.39 X 0.97	YES	NO	0.87	0.20	87.47	87.36			
CORNER OF POND	27C	19	0.00	0.11	0.11	0.10	7.88	0.28	21.98	37.73	53.79	1314.2	0.00	0.65	1.20	3.00	0.71	1622.9	8324.0	1.28	72.00							0.94	87.36	86.85			
										38.67																							

Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA									CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)	
			Area at C of		SUM(A)	SUM(A*C)	TOTAL A°C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL	HW 1:10 (m)				
	0.70 (ha)	0.90 (ha)																													
SW ENTRANCE TO SOMME STREET	1	2	0.18	0.25	0.43	0.35	0.35	0.97	0.97	15.00	97.85	94.6	0.00	0.32	1.20	3.00	0.61	226.9	7702.7	0.74	189.60								4.28	93.65	92.50
CULVERT CROSSING	2	9		0.00	0.00	0.00	0.35	0.00	0.97	19.28	84.12	81.3					0.50				20.00	1	600	-----	NO	YES	0.52	1.16	92.50	92.40	
SOUTH PORTION SOMME STREET	9	28	2.54	0.35	2.89	2.10	2.44	5.83	6.80	20.44	81.10	551.2	0.00	0.47	1.20	3.00	0.73	694.0	8450.7	1.05	272.58								4.34	92.40	90.41
SOUTH PORTION SOMME STREET	28	29A	3.46	0.32	3.78	2.71	5.15	7.53	14.33	24.77	71.65	1026.7	0.00	0.61	1.20	3.00	0.54	1198.8	7283.5	1.07	245.24								3.81	90.41	89.08
SOUTH PORTION SOMME STREET	29A	29B	0.77	0.11	0.88	0.64	5.79	1.78	16.11	28.58	65.15	1049.5	0.00	0.62	1.20	3.00	0.53	1239.6	7212.0	1.07	86.51								1.34	89.08	88.62
SOUTH PORTION SOMME STREET	29B	30	0.32	0.12	0.44	0.33	6.13	0.92	17.03	29.92	63.16	1075.8	0.00	0.58	1.20	3.00	0.70	1191.6	8282.1	1.18	94.12								1.33	88.62	87.96
SOUTH PORTION SOMME STREET	30	22	0.75	0.16	0.91	0.67	6.80	1.86	18.89	31.25	61.31	1158.5	0.00	0.58	1.20	3.00	0.97	1402.6	9748.4	1.39	124.55								1.49	87.96	86.75
										32.74																					
CULVERT CROSSING	22	19		0.00	0.00	0.00	15.59	0.00	43.33	32.74	59.38	2573.1					0.50				20.00	2	-----	1.03 X 0.74	YES	NO	1.30	0.08	86.85	86.75	
										32.82																					
POND INLET	19	POND		0.00	0.00	0.00	35.97	0.00	100.06	38.67	52.87	5422.6	3.09	0.38	1.20	3.00	5.68	5629.1	13135.2	3.50	22.00								0.10	86.75	85.50
POND OUTLET DITCH	POND	DITCH	1:10 year controlled post development peak flow = 696 l/s, see SWMHYMO output of this Report									696.0	1.00	0.27	0.38	3.00	2.08	750.9	1506.6	1.54	24.00								0.26	82.50	82.00

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030

Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA						CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)	
			Area at C of		SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A°C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL				OUTLET CONTROL
	0.70 (ha)	0.90 (ha)																									
NORTHERN CATCHMENT AREA																											
WEST SIDE SAPPERS RIDGE	2	3	1.86	0.18	2.04	1.81	1.81	5.02	5.02	15.00	142.89	718.0	0.00	1.20	3.00	0.50	6973.0	1.61	136.80					1.41	92.50	91.82	
WEST SIDE SAPPERS RIDGE	3	4	1.89	0.14	2.03	1.80	3.61	5.00	10.02	16.41	135.47	1357.9	0.00	1.20	3.00	0.80	8856.1	2.05	111.00					0.90	91.82	90.93	
WEST SIDE SAPPERS RIDGE	4	5	1.76	0.15	1.91	1.69	5.29	4.69	14.71	17.31	131.16	1929.7	0.00	1.20	3.00	0.51	7029.1	1.63	112.85					1.16	90.93	90.36	
WEST SIDE SAPPERS RIDGE	5	6	2.43	0.11	2.54	2.23	7.53	6.21	20.92	18.47	126.06	2637.5	0.00	1.20	3.00	0.62	7762.6	1.80	82.79					0.77	90.36	89.85	
										19.24																	
NORTH ENTRANCE TO SOMME STREET	8	6		0.03	0.03	0.03	0.03	0.08	0.08	15.00	142.89	11.9	0.00	1.20	3.00	1.30	11276.7	2.61	10.00					0.06	89.98	89.85	
										15.06																	
CULVERT CROSSING	6	14		0.00	0.00	0.00	7.56	0.00	21.01	19.24	122.91	2581.8				0.50			20.00	2	----	1.15 x 0.82	NO	YES	0.19	89.85	89.75
										19.43																	
NORTH PORTION SOMME STREET	13	14	0.85	0.03	0.88	0.77	0.77	2.15	2.15	15.00	142.89	307.4	0.00	1.20	3.00	2.30	14999.4	3.47	10.00					0.05	89.98	89.75	
										15.05																	
NORTH PORTION SOMME STREET	14	15	2.93	0.24	3.17	2.80	11.13	7.79	30.95	19.43	122.15	3780.5	0.00	1.20	3.00	0.50	6992.8	1.62	184.04					1.89	89.75	88.83	
NORTH PORTION SOMME STREET	15	16	2.08	0.18	2.26	2.00	13.13	5.56	36.51	21.32	115.16	4204.4	0.00	1.20	3.00	0.57	7480.8	1.73	145.08					1.40	88.83	88.00	
NORTH PORTION SOMME STREET	16	18	2.34	0.23	2.57	2.28	15.41	6.33	42.84	22.72	110.55	4736.0	0.00	1.20	3.00	0.51	7074.8	1.64	185.66					1.89	88.00	87.05	
NORTH PORTION SOMME STREET	18	19	0.00	0.05	0.05	0.05	15.46	0.14	42.98	24.61	104.93	4509.7	0.00	1.20	3.00	0.72	8372.8	1.94	41.86					0.36	87.05	86.75	
										24.97																	
EAST SIDE SAPPERS RIDGE	9	10	1.74	0.19	1.93	1.71	1.71	4.76	4.76	15.00	142.89	680.4	0.00	1.20	3.00	0.50	6996.6	1.62	147.87					1.52	92.40	91.66	
EAST SIDE SAPPERS RIDGE	10	11	1.49	0.14	1.63	1.44	3.16	4.02	8.78	16.52	134.93	1184.3	0.00	1.20	3.00	0.66	8019.2	1.86	111.04					1.00	91.66	90.93	
EAST SIDE SAPPERS RIDGE	11	12	0.73	0.14	0.87	0.78	3.94	2.16	10.94	17.52	130.23	1424.7	0.00	1.20	3.00	0.55	7304.8	1.69	104.49					1.03	90.93	90.36	
EAST SIDE SAPPERS RIDGE	12	7	0.18	0.09	0.27	0.25	4.18	0.69	11.63	18.55	125.73	1462.2	0.00	1.20	3.00	0.81	8919.0	2.06	72.55					0.59	90.36	89.77	
NORTH PORTION SOMME STREET	7	20	1.07	0.23	1.30	1.17	5.35	3.24	14.87	19.13	123.33	1834.1	0.00	1.20	3.00	0.50	6966.1	1.61	177.39					1.83	89.77	88.89	
NORTH PORTION SOMME STREET	20	21	2.27	0.19	2.46	2.18	7.53	6.05	20.92	20.97	116.41	2435.6	0.00	1.20	3.00	0.50	6981.9	1.62	147.49					1.52	88.89	88.16	
NORTH PORTION SOMME STREET	21	22	3.43	0.30	3.73	3.30	10.83	9.18	30.10	22.49	111.29	3350.0	0.00	1.20	3.00	0.56	7404.4	1.71	232.84					2.26	88.16	86.85	
										24.75																	
SOUTHERN CATCHMENT AREA																											
SOUTH PORTION SOMME STREET	23A	23B	0.00	0.25	0.25	0.25	0.25	0.70	0.70	15.00	142.89	99.3	0.00	1.20	3.00	0.64	7883.5	1.82	181.00					1.65	93.65	92.50	
CULVERT CROSSING	23B	23C		0.00	0.00	0.00	0.25	0.00	0.70	16.65	134.29	93.3				0.42			24.00	1	500	----	NO	YES	0.84	92.50	92.40
SOUTH PORTION SOMME STREET	23C	24A	0.00	0.17	0.17	0.17	0.42	0.47	1.17	17.49	130.34	152.2	0.00	1.20	3.00	0.82	8946.1	2.07	110.00					0.89	92.40	91.50	
CULVERT CROSSING	24A	24B		0.00	0.00	0.00	0.42	0.00	1.17	18.38	126.45	147.6				0.42			24.00	1	500	----	NO	YES	0.53	91.50	91.40
SOUTH PORTION SOMME STREET	24B	24C	0.00	0.21	0.21	0.21	0.63	0.58	1.75	18.91	124.24	217.6	0.00	1.20	3.00	0.70	8258.2	1.91	142.00					1.24	91.40	90.41	
ORGAWORLD - SITE	U/S	24C	1:100 year peak flow = 283 l/s, see Table 4 of Orgaworld Stormwater Site Management Plan, Sept. 2008									283.0															
SOUTH PORTION SOMME STREET	24C	25	3.70	0.32	4.02	3.56	4.19	9.89	11.64	20.15	119.40	1672.8	0.00	1.20	3.00	0.54	7289.5	1.69	244.84					2.42	90.41	89.08	
SOUTH PORTION SOMME STREET	25	26	2.63	0.12	2.75	2.42	6.61	6.73	18.37	22.57	111.05	2323.0	0.00	1.20	3.00	0.51	7041.5	1.63	90.75					0.93	89.08	88.62	
SOUTH PORTION SOMME STREET	26	27A	3.15	0.20	3.35	2.96	9.57	8.22	26.59	23.49	108.17	3159.5	0.00	1.20	3.00	0.65	7970.4	1.84	157.06					1.42	88.62	87.60	
SOUTH PORTION SOMME STREET	27A	27B	0.00	0.03	0.03	0.03	9.60	0.08	26.67	24.91	104.09	3059.5	0.00	1.20	3.00	0.65	7973.8	1.85	20.00					0.18	87.60	87.47	
CULVERT CROSSING	27B	27C		0.00	0.00	0.00	9.60	0.00	26.67	25.09	103.59	3046.2				0.73			15.00	1	----	1.39 X 0.97	YES	NO	0.09	87.47	87.36
CORNER OF POND	27C	19	0.00	0.11	0.11	0.11	9.71	0.31	26.98	25.18	103.36	3071.7	0.00	1.20	3.00	0.71	8324.0	1.93	72.00					0.62	87.36	86.85	
										25.80																	

Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION				
	FROM	TO	Area at C of		SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s
			0.70 (ha)	0.90 (ha)								
SW ENTRANCE TO SOMME STREET	1	2	0.18	0.25	0.43	0.40	0.40	1.12	1.12	15.00	142.89	160.5
CULVERT CROSSING	2	9		0.00	0.00	0.00	0.40	0.00	1.12	16.77	133.71	150.2
SOUTH PORTION SOMME STREET	9	28	2.54	0.35	2.89	2.58	2.98	7.16	8.29	17.40	130.77	1083.6
SOUTH PORTION SOMME STREET	28	29A	3.46	0.32	3.78	3.35	6.33	9.31	17.59	19.72	121.01	2128.9
SOUTH PORTION SOMME STREET	29A	29B	0.77	0.11	0.88	0.79	7.11	2.19	19.78	22.15	112.40	2223.0
SOUTH PORTION SOMME STREET	29B	30	0.32	0.12	0.44	0.40	7.51	1.11	20.89	23.01	109.65	2290.7
SOUTH PORTION SOMME STREET	30	22	0.75	0.16	0.91	0.82	8.33	2.27	23.16	23.83	107.18	2482.3
										24.75		
CULVERT CROSSING	22	19		0.00	0.00	0.00	19.16	0.00	53.26	24.75	104.53	5567.5
										24.79		
POND INLET	19	POND		0.00	0.00	0.00	44.32	0.00	123.22	25.80	101.69	12813.8
POND OUTLET DITCH	POND	DITCH	1:100 year controlled post development peak flow = 1,432 l/s, see SWMHYMO output of this Report									1432.0

OPEN DITCH/SWALE DATA							CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW	U/S	D/S
BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL	TIME (min)	Inv (m)	Inv (m)
0.00	1.20	3.00	0.61	7702.7	1.78	189.60						1.77	93.65	92.50
			0.50			20.00	1	600	----	NO	YES	0.63	92.50	92.40
0.00	1.20	3.00	0.73	8450.7	1.96	272.58						2.32	92.40	90.41
0.00	1.20	3.00	0.54	7283.5	1.69	245.24						2.42	90.41	89.08
0.00	1.20	3.00	0.53	7212.0	1.67	86.51						0.86	89.08	88.62
0.00	1.20	3.00	0.70	8282.1	1.92	94.12						0.82	88.62	87.96
0.00	1.20	3.00	0.97	9748.4	2.26	124.55						0.92	87.96	86.75
			0.50			20.00	2	----	1.03 X 0.74	YES	NO	0.04	86.85	86.75
3.09	0.55	5.00	5.68	13135.2	4.09	22.00						0.09	86.75	85.50
1.00	0.38	3.00	2.08	1506.6	1.85	24.00						0.22	82.50	82.00

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030



Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA									CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)	
			AREA (A) at C of				SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL	HW 1:10 (m)				
	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																													
WEST CATCHMENT AREA																																	
EAST SIDE HAWTHORNE ROAD	1	2		2.46		0.14	2.60	0.86	0.86	2.40	2.40	15.00	97.85	235.0	0.00	0.41	0.50	3.00	0.20	250.1	424.5	0.50	112.00								3.76	103.22	103.00
WEST SIDE HAWTHORNE ROAD	2	3		1.60		0.06	1.66	0.53	1.40	1.48	3.89	18.76	85.54	332.5	0.00	0.25	0.50	3.00	5.00	337.3	2141.9	1.80	50.00							0.46	103.00	100.50	
WEST SIDE HAWTHORNE ROAD	3	4		1.58		0.06	1.64	0.53	1.93	1.47	5.35	19.23	84.26	451.1	0.00	0.27	0.50	3.00	7.00	490.1	2534.3	2.24	50.00							0.37	100.50	97.00	
WEST SIDE HAWTHORNE ROAD	4	5		1.58		0.06	1.64	0.53	2.45	1.47	6.82	19.60	83.26	568.0	0.00	0.34	0.50	3.00	5.00	765.9	2141.9	2.21	50.00							0.38	97.00	94.50	
EAST SIDE HAWTHORNE ROAD	5	6a		1.95		0.10	2.05	0.68	3.13	1.88	8.70	19.98	82.27	715.6	0.00	0.45	0.65	3.00	1.07	747.0	1991.5	1.23	75.00							1.02	94.50	93.70	
CULVERT CROSSING	6a	6b				0.00	0.00	0.00	3.13	0.00	8.70	20.99	79.73	693.6					1.00				10.00	1	800	-----	YES	NO	0.84	0.12	93.70	93.60	
WEST SIDE HAWTHORNE ROAD	6b	7		1.20		0.03	1.23	0.39	3.52	1.08	9.77	21.11	79.45	776.5	0.00	0.53	1.15	3.00	0.53	817.1	6447.9	0.97	15.00							0.26	93.60	93.52	
EAST SIDE HAWTHORNE ROAD	7	8		1.58		0.06	1.64	0.53	4.04	1.47	11.24	21.37	78.83	886.3	0.00	0.56	1.15	3.00	0.50	916.3	6243.2	0.97	50.00							0.86	93.52	93.27	
EAST SIDE HAWTHORNE ROAD	8	9		1.58		0.06	1.64	0.53	4.57	1.47	12.71	22.23	76.88	977.2	0.00	0.58	1.15	3.00	0.50	1006.2	6243.2	1.00	50.00							0.84	93.27	93.02	
WEST SIDE HAWTHORNE ROAD	9	10		1.58		0.06	1.64	0.53	5.10	1.47	14.18	23.06	75.07	1064.4	0.00	0.60	1.15	3.00	0.50	1101.4	6243.2	1.02	50.00							0.82	93.02	92.77	
WEST SIDE HAWTHORNE ROAD	10	11		1.58		0.06	1.64	0.53	5.63	1.47	15.65	23.88	73.39	1148.3	0.00	0.62	1.15	3.00	0.50	1202.1	6243.2	1.04	50.00							0.80	92.77	92.52	
EAST SIDE HAWTHORNE ROAD	11	12		1.48		0.06	1.54	0.50	6.13	1.38	17.03	24.68	71.83	1223.3	0.00	0.63	1.15	3.00	0.50	1254.5	6243.2	1.05	50.00							0.79	92.52	92.27	
EAST SIDE HAWTHORNE ROAD	12	13		1.34		0.06	1.40	0.46	6.58	1.27	18.30	25.47	70.35	1287.3	0.00	0.64	1.15	3.00	0.50	1308.3	6243.2	1.06	50.00							0.78	92.27	92.02	
EAST SIDE HAWTHORNE ROAD	13	14b		1.54		0.21	1.75	0.65	7.23	1.81	20.11	26.25	68.96	1386.6	0.00	0.64	1.15	3.00	0.61	1449.7	6918.0	1.18	158.00							2.23	92.02	91.05	
												28.49																					
SW RIDEAU & HAWTHORNE	14a	14b		0.64		0.18	0.82	0.35	0.35	0.98	0.98	15.00	97.85	96.3	0.00	0.20	1.30	3.00	4.06	167.6	24661.5	1.40	140.00							1.67	96.73	91.05	
												16.67																					
CULVERT CROSSING	14b	23				0.00	0.00	0.00	7.59	0.00	21.09	28.49	65.29	1377.2					1.40				20.00	1	1000	-----	YES	NO	1.14	0.19	91.05	90.77	
												28.68																					
EAST CATCHMENT AREA																																	
EAST SIDE HAWTHORNE ROAD	15	16				0.33	0.33	0.30	0.30	0.83	0.83	15.00	97.85	80.8	0.00	0.25	0.30	3.00	0.45	101.7	165.4	0.54	110.00							3.38	103.80	103.30	
EAST SIDE HAWTHORNE ROAD	16	17				0.14	0.14	0.13	0.42	0.35	1.18	18.38	86.64	101.9	0.00	0.16	0.30	3.00	6.20	114.3	610.8	1.49	100.00							1.12	103.30	97.10	
EAST SIDE HAWTHORNE ROAD	17	18				0.04	0.04	0.04	0.46	0.10	1.28	19.50	83.52	106.6	0.00	0.16	1.20	3.00	6.36	115.8	24949.6	1.51	33.00							0.36	97.10	95.00	
CULVERT CROSSING	18	19				0.00	0.00	0.00	0.46	0.00	1.28	19.86	82.56	105.3					1.77				22.00	1	600	-----	YES	NO	0.30	0.98	95.00	94.61	
EAST SIDE HAWTHORNE ROAD	19	20				0.06	0.06	0.05	0.51	0.15	1.43	20.85	80.08	114.2	0.00	0.21	0.70	3.00	2.79	158.3	3925.7	1.20	24.00							0.33	94.61	93.94	
CULVERT CROSSING	20	21				0.00	0.00	0.00	0.51	0.00	1.43	21.18	79.28	113.1					0.50				20.00	1	600	-----	NO	YES	0.37	0.83	93.94	93.84	
EAST SIDE HAWTHORNE ROAD	21	22a	0.21			0.16	0.37	0.19	0.70	0.52	1.94	22.02	77.35	150.3	0.00	0.29	0.80	3.00	0.50	158.5	2372.0	0.63	82.00							2.18	93.84	93.43	
EAST SIDE HAWTHORNE ROAD	22a	22b	0.61			0.29	0.90	0.38	1.08	1.06	3.01	24.19	72.77	218.9	0.00	0.33	1.17	3.00	0.52	228.1	6666.4	0.70	175.00							4.18	93.43	92.52	
EAST SIDE HAWTHORNE ROAD	22b	23	0.93			0.34	1.27	0.49	1.57	1.37	4.38	28.37	65.47	286.5	0.00	0.35	1.17	3.00	0.70	309.6	7734.6	0.84	260.00							5.14	92.59	90.77	
												33.51																					
SOUTH CATCHMENT AREA																																	
SOUTH SIDE RIDEAU ROAD	23	24	0.73			0.28	1.01	0.40	9.56	1.11	26.57	33.51	58.43	1552.8	0.00	0.51	1.74	3.00	2.65	1642.9	43339.8	2.11	235.00							1.86	90.77	84.55	
												35.37																					
WEST SIDE SOMME STREET	25	24			0.42	0.12	0.54	0.40	0.40	1.12	1.12	15.00	97.85	109.4	0.00	0.18	1.20	3.00	2.80	105.1	16548.0	1.08	125.74							1.94	89.98	86.46	
												16.94																					
CULVERT CROSSING	24	26				0.00	0.00	0.00	9.96	0.00	27.69	35.37	56.28	1558.5					1.00				20.00	1	800	-----	NO	YES	2.31	0.11	84.55	84.35	
												35.48																					
EAST SIDE SOMME STREET	27	26			0.32	0.11	0.43	0.32	0.32	0.90	0.90	15.00	97.85	87.9	0.00	0.17	1.20	3.00	2.80	90.3	16548.0	1.04	125.74							2.01	89.98	86.46	
												17.01																					
SOUTH SIDE RIDEAUH																																	

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA								CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)		
			AREA (A) at C of				SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D <sub>10yr</sub> m	D <sub>max</sub> m	SS X:1	SLOPE %	Q <sub>10yr</sub> l/s	Q <sub>100yr</sub> l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL				HW 1:10 (m)	
	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																													
NORTH CATCHMENT AREA																																	
			Existing 900 mm dia. culvert capacity before ditch flows to Findlay Creek																														
NORTH SIDE RIDEAU ROAD	31	32	6.66				0.52	7.18	1.80	1.80	5.00	5.00	20.00	97.26		0.00	0.58	1.50	3.00	1.93	1974.3	24880.1	1.96	400.00							3.41	90.71	83.01
													23.41																				
	33	32	0.87				0.10	0.97	0.26	0.26	0.73	0.73	15.00	115.83		0.00	0.40	1.50	3.00	0.16	213.3	7240.8	0.44	92.00							3.45	83.16	83.01
													18.45																				
EXISTING CULVERT CROSSING	32	28					0.00	0.00	0.00	2.06	0.00	5.74	23.41	87.93						-0.15				20.00	1	1000					0.14	83.01	83.04
													23.55																				
SOUTH CATCHMENT AREA																																	
SOUTH SIDE RIDEAU ROAD	28	29	0.90				0.33	1.23	0.48	13.16	1.33	36.58	37.84	53.68	3363.5	0.00	1.17	2.20	3.00	0.14	3437.1	18513.7	0.84	347.24							6.91	83.04	82.56
SOUTH SIDE RIDEAU ROAD	29	30	0.48				0.31	0.79	0.38	13.53	1.04	37.62	44.76	47.64	3192.1	0.00	0.90	2.20	3.00	0.51	3287.0	35640.2	1.35	236.20							2.91	82.56	81.35

ote: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA							CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)		
			AREA (A) at C of				SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL					
	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																											
WEST CATCHMENT AREA																															
WEST SIDE HAWTHORNE ROAD	1	2		2.46		0.14	2.60	1.06	1.06	2.95	2.95	15.00	142.89	422.1	0.00	0.50	3.00	0.20	424.5	0.57	112.00							3.30	103.22	103.00	
WEST SIDE HAWTHORNE ROAD	2	3		1.60		0.06	1.66	0.66	1.72	1.83	4.79	18.30	126.80	607.2	0.00	0.50	3.00	5.00	2141.9	2.86	50.00							0.29	103.00	100.50	
WEST SIDE HAWTHORNE ROAD	3	4		1.58		0.06	1.64	0.65	2.38	1.81	6.60	18.59	125.56	829.0	0.00	0.50	3.00	7.00	2534.3	3.38	50.00							0.25	100.50	97.00	
WEST SIDE HAWTHORNE ROAD	4	5		1.58		0.06	1.64	0.65	3.03	1.81	8.42	18.84	124.54	1048.2	0.00	0.50	3.00	5.00	2141.9	2.86	50.00							0.29	97.00	94.50	
WEST SIDE HAWTHORNE ROAD	5	6A		1.95		0.10	2.05	0.83	3.86	2.31	10.73	19.13	123.35	1323.2	0.00	0.65	3.00	1.07	1991.5	1.57	75.00							0.80	94.50	93.70	
CULVERT CROSSING	6A	6B				0.00	0.00	0.00	3.86	0.00	10.73	19.92	120.24	1289.9				1.00			10.00	1	800	-----	YES	NO	0.06	93.70	93.60		
WEST SIDE HAWTHORNE ROAD	6B	7		1.20		0.03	1.23	0.48	4.34	1.33	12.06	19.99	119.99	1447.3	0.00	1.15	3.00	0.53	6447.9	1.63	15.00							0.15	93.60	93.52	
WEST SIDE HAWTHORNE ROAD	7	8		1.58		0.06	1.64	0.65	4.99	1.81	13.88	20.14	119.42	1657.0	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	93.52	93.27	
WEST SIDE HAWTHORNE ROAD	8	9		1.58		0.06	1.64	0.65	5.64	1.81	15.69	20.67	117.47	1843.0	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	93.27	93.02	
WEST SIDE HAWTHORNE ROAD	9	10		1.58		0.06	1.64	0.65	6.30	1.81	17.50	21.20	115.59	2023.3	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	93.02	92.77	
WEST SIDE HAWTHORNE ROAD	10	11		1.58		0.06	1.64	0.65	6.95	1.81	19.32	21.73	113.78	2197.9	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	92.77	92.52	
WEST SIDE HAWTHORNE ROAD	11	12		1.48		0.06	1.54	0.62	7.56	1.71	21.03	22.26	112.03	2355.6	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	92.52	92.27	
WEST SIDE HAWTHORNE ROAD	12	13		1.34		0.06	1.40	0.56	8.13	1.56	22.59	22.79	110.34	2492.6	0.00	1.15	3.00	0.50	6243.2	1.57	50.00							0.53	92.27	92.02	
WEST SIDE HAWTHORNE ROAD	13	14B		1.54		0.21	1.75	0.79	8.91	2.19	24.78	23.32	108.70	2693.6	0.00	1.15	3.00	0.61	6918.0	1.74	158.00							1.51	92.02	91.05	
												24.83																			
SW RIDEAU & HAWTHORNE	14A	14B		0.64		0.18	0.82	0.42	0.42	1.17	1.17	15.00	142.89	166.8	0.00	1.30	3.00	4.06	24661.5	4.86	140.00							0.48	96.73	91.05	
												15.48																			
CULVERT CROSSING	14B	23				0.00	0.00	0.00	9.33	0.00	25.95	24.83	104.32	2706.8				1.40			20.00	1	1000	-----	YES	NO	0.10	91.05	90.77		
												24.93																			
EAST CATCHMENT AREA																															
EAST SIDE HAWTHORNE ROAD	15	16				0.33	0.33	0.33	0.33	0.92	0.92	15.00	142.89	131.1	0.00	0.30	3.00	0.45	165.4	0.61	110.00							2.99	103.80	103.30	
EAST SIDE HAWTHORNE ROAD	16	17				0.14	0.14	0.14	0.47	0.39	1.31	17.99	128.11	167.4	0.00	0.30	3.00	6.20	610.8	2.26	100.00							0.74	103.30	97.10	
EAST SIDE HAWTHORNE ROAD	17	18				0.04	0.04	0.04	0.51	0.11	1.42	18.73	124.98	177.2	0.00	1.20	3.00	6.36	24949.6	5.78	33.00							0.10	97.10	95.00	
CULVERT CROSSING	18	19				0.00	0.00	0.00	0.51	0.00	1.42	18.82	124.58	176.6				1.77			22.00	1	600	-----	YES	NO	0.59	95.00	94.61		
EAST SIDE HAWTHORNE ROAD	19	20				0.06	0.06	0.06	0.57	0.17	1.58	19.41	122.22	193.7	0.00	0.70	3.00	2.79	3925.7	2.67	24.00							0.15	94.61	93.94	
CULVERT CROSSING	20	21				0.00	0.00	0.00	0.57	0.00	1.58	19.56	121.63	192.7				0.50			20.00	1	600	-----	NO	YES	0.49	93.94	93.84		
EAST SIDE HAWTHORNE ROAD	21	22A	0.21			0.16	0.37	0.21	0.78	0.59	2.18	20.05	119.76	260.5	0.00	0.80	3.00	0.50	2372.0	1.24	82.00							1.11	93.84	93.43	
EAST SIDE HAWTHORNE ROAD	22A	22B	0.61			0.29	0.90	0.44	1.23	1.23	3.41	21.16	115.75	394.2	0.00	1.17	3.00	0.52	6666.4	1.62	175.00							1.80	93.43	92.52	
EAST SIDE HAWTHORNE ROAD	22B	23	0.93			0.34	1.27	0.57	1.80	1.59	5.00	22.95	109.83	548.8	0.00	1.17	3.00	0.70	7734.6	1.88	260.00							2.30	92.59	90.77	
												25.25																			
SOUTH CATCHMENT AREA																															
SOUTH SIDE RIDEAU ROAD	23	24	0.73			0.28	1.01	0.46	11.59	1.29	32.23	25.25	103.15	3324.7	0.00	1.74	3.00	2.65	43339.8	4.77	235.00							0.82	90.77	84.55	
												26.08																			
WEST SIDE SOMME STREET	25	24			0.42	0.12	0.54	0.49	0.49	1.36	1.36	15.00	142.89	193.7	0.00	1.20	3.00	2.80	16548.0	3.83	125.74							0.55	89.98	86.46	
												15.55																			
CULVERT CROSSING	24	26				0.00	0.00	0.00	12.08	0.00	33.59	26.08	100.99	3391.7				1.00			20.00	1	800	-----	NO	YES	0.05	84.55	84.35		
												26.12																			
EAST SIDE SOMME STREET	27	26			0.32	0.11	0.43	0.39	0.39	1.08	1.08	15.00	142.89	154.9	0.00	1.20	3.00	2.80	16548.0	3.83	125.74							0.55	89.98	86.46	
												15.55																			
															</																

Hawthorne Road & Rideau Road

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983  
February 2009

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0% up C = 1.0

DETAILS	NODES		DRAINAGE AREA							PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA							CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)	
			AREA (A) at C of				SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL				
	FROM	TO	0.20 (ha)	0.30 (ha)	0.70 (ha)	0.90 (ha)																								
NORTH CATCHMENT AREA																														
			Existing 900 mm dia. Culvert Capacity before ditch flows to Findlay Creek																											
NORTH SIDE RIDEAU ROAD	31	32	6.66			0.52	7.18	2.19	2.19	6.07	6.07	20.00	119.95	2128.6	0.00	1.50	3.00	1.93	24880.1	3.69	400.00							1.81	90.71	83.01
												21.81																		
NORTH SIDE RIDEAU ROAD	33	32	0.87			0.10	0.97	0.32	0.32	0.88	0.88	15.00	142.89	126.1	0.00	1.50	3.00	0.16	7240.8	1.07	92.00							1.43	83.16	83.01
												16.43																		
EXISTING CULVERT CROSSING	32	28				0.00	0.00	0.00	2.50	0.00	6.96	21.81	113.52	2189.7				-0.15			20.00	1	1000				0.12	83.01	83.04	
												21.93																		
SOUTH CATCHMENT AREA																														
SOUTH SIDE RIDEAU ROAD	28	29	0.90			0.33	1.23	0.56	15.91	1.54	44.24	27.18	98.22	5745.1	0.00	2.20	3.00	0.14	18513.7	1.28	347.24						4.54	83.04	82.56	
SOUTH SIDE RIDEAU ROAD	29	30	0.48			0.31	0.79	0.43	16.34	1.20	45.44	31.72	88.42	5417.3	0.00	2.20	3.00	0.51	35640.2	2.45	236.20						1.60	82.56	81.35	

Note: Conveyance Capacities for the Open Ditch/Swale were calculated based on a Manning's Roughness Coefficient (n) of 0.030

HAWTHORNE INDUSTRIAL PARK

1:10 YEAR ROADSIDE CULVERT DESIGN

CONVENTIONAL CULVERT DESIGN

Prepared by: Mark Buchanan, E.I.T.  
Reviewed by: Guy Forget, P.Eng.  
Date: February 2009

Station	DESIGN DATA							CULVERT DATA					INLET CONTROL			OUTLET CONTROL							GOVERNING HW	VEL V <sub>o</sub>			
	Q	d	d <sub>c</sub>	AHW	Skew No.	L	S	Description	B	D or H	N	Q/N	A (each)	Q/NB	HW/D	HW	K <sub>e</sub>	H	d <sub>c</sub>	(d <sub>c</sub> + D)/2	TW	h <sub>o</sub>			LS	HW	
	(m³/s)	(m)	(m)	(m)		(m)	(m/m)		(m)	(m)		(m³/s)	(m²)	(m³/s/m)		(m)		(m)	(m)	(m)	(m)	(m)			(m)	(m)	
1	2	3	4	5	6	7	8	9	10a	10b	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	
6 to 14	1.296	0.67	0.05	1.1	0	20.0	0.005	CSPA 6	1.15	0.82	2	0.648	0.74	---	0.73	0.60	0.9	0.13	0.33	0.58	0.72	0.72	0.10	0.75	0.75		
23B to 23C	0.051	0.22	0.05	1.15	0	24.0	0.004	CSP 500	N/A	0.5	1	0.051	0.20	----	0.50	0.25	0.9	0.1	0.15	0.33	0.27	0.33	0.10	0.33	0.33		
24A to 24B	0.075	0.25	0.05	1.15	0	24.0	0.004	CSP 500	N/A	0.5	1	0.075	0.20	----	0.54	0.27	0.9	0.1	0.18	0.34	0.30	0.34	0.10	0.34	0.34		
2 to 9	0.081	0.47	0.05	1.15	0	20.0	0.005	CSP 600	N/A	0.6	1	0.081	0.28	----	0.50	0.30	0.9	0.1	0.19	0.40	0.52	0.52	0.10	0.52	0.52		
27B to 27C	1.304	0.61	0.05	1.23	0	15.0	0.007	CSPA 7	1.39	0.97	1	1.304	1.06	----	0.90	0.87	0.9	0.22	0.45	0.71	0.66	0.71	0.11	0.82	0.87		
22 to 19	2.573	0.38	0.05	1.35	0	20.0	0.005	CSPA 5	1.03	0.74	2	1.287	0.61	----	1.75	1.30	0.9	0.74	0.51	0.63	0.43	0.63	0.10	1.27	1.30		

# C

## Appendix C - Stormwater Management and Storm Sewer Design Calculations

## EVALUATION OF RUNOFF COEFFICIENTS

**Client:** Fastfrate (Ottawa) Holdings Inc.  
**Project:** Fastfrate Warehouse Development  
**Location:** Ottawa, Ontario  
**Project #:** A001083  
**Project Status:**



Area	Grassed Area (m²)	Runoff Coefficient	Hard Surface Area (m²)	Runoff Coefficient	Total Area (m²)	Runoff Coefficient (10-year event)	Runoff Coefficient (100-year)
A0	3869	0.20	0	0.90	3869	0.20	0.25
TOTAL - Christie Creek	3869		0		3869	0.20	0.25
A1	2073	0.20	5573	0.90	7646	0.71	0.89
A2	2121	0.20	10017	0.83	12138	0.72	0.90
A3	0	0.20	8582	0.90	8582	0.90	0.95
A4	705	0.20	781	0.90	1486	0.57	0.71
A5	0	0.20	1069	0.90	1069	0.90	0.95
A6	3917	0.20	266	0.90	4183	0.24	0.31
A7	820	0.20	676	0.90	1496	0.52	0.65
TOTAL - Somme Street SWMF	9636		26964		36600	0.70	0.87

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
 PEO No.: 100530467

Date: 2021-07-20

Verified by: Christian Lavoie-Lebel, P.Eng.  
 PEO No.: 100067842

Date: 2021-07-20

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**PROJECT NAME:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**CIMA+ PROJECT NUMBER:** A001083  
**CLIENT:** Fastrate  
**PROJECT STATUS:** Detailed Design

## STORM POST-DEVELOPMENT FLOW (UNCONTROLLED) Proposed Stormwater Management

### APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

### PRE-DEVELOPMENT FLOW DETERMINATION:

#### DESIGN CRITERIA:

Design Storm (year):	10	
IDF Regression Constants: (a)	1174.184	
(b)	6.014	
(c)	0.816	
IDF Curve Equation (mm/hr):	$I = a / (\text{Time in min} + b)^c$	
Rational Formula (L/s):	$Q = 2.78C \cdot I \cdot A$	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area

### ALLOWABLE RELEASE RATE - SUMMARY:

Catchment ID	Area (A) ha	Runoff Coefficient (C)	Time of Concentration (tc) min	Intensity (I) mm/hr	Allowable Release Rate (Q) L/s	Release Flow Per Unit Area (Q/ha) L/s/ha
A1	0.76	0.71	22.85	75.52	113.92	149.00
A2	1.21	0.72	22.85	75.52	183.32	151.03
A3	0.86	0.90	22.85	75.52	162.04	188.81
A4	0.15	0.57	22.85	75.52	17.70	119.14
A5	0.11	0.90	22.85	75.52	20.18	188.81
A6	0.49	0.24	22.85	75.52	24.47	50.35
A7	0.15	0.52	22.85	75.52	16.32	109.09
Total	3.73				537.956	144.31

### NOTES:

- Time of concentration taken from SWM report (JL Richards, 2009). It is assumed that the resulting time of concentration is identical to JL Richards SWM report.
- IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 20, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 20, 2021



PROJECT NAME: Fastfrate Warehouse Development  
Industrial/Commercial Development

CIMA+ PROJECT NUMBER: A001083

CLIENT: Fastfrate

PROJECT STATUS: Detailed Design

**STORM POST-DEVELOPMENT FLOW (CONTROLLED)**  
Per Master Stormwater Management Report (J.L. Richards, 2009)

**APPLICABLE DESIGN GUIDELINES:**

1. City of Ottawa Sewer Design Guidelines, 2012

**PRE-DEVELOPMENT FLOW DETERMINATION:**

**DESIGN CRITERIA:**

Design Storm (year):	10	
IDF Regression Constants: (a)	1174.184	
(b)	6.014	
(c)	0.816	
IDF Curve Equation (mm/hr):	$I = a / (\text{Time in min} + b)^c$	
Rational Formula (L/s):	$Q = 2.78C \cdot I \cdot A$	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area

**ALLOWABLE RELEASE RATE - SUMMARY:**

Catchment ID	Area (A) ha	Runoff Coefficient (C)	Time of Concentration (tc) min	Intensity (I) mm/hr	Allowable Release Rate (Q) L/s	Release Flow Per Unit Area (Q/ha) L/s/ha
Total Site Area Draining to SWMF per JLR 2009 SWM	3.05	0.70	22.85	75.52	448.57	146.85
Total	3.05				448.567	146.85
Revised Total Area	3.73				448.567	120.33

**NOTES:**

1. Time of concentration taken from SWM report (JL Richards, 2009).
2. Runoff coefficients taken from SWM report (JL Richards, 2009).
3. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 20, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 20, 2021



**PROJECT NAME:** Fastrate Warehouse Development  
Industrial/Commercial Development

**CIMA+ PROJECT NUMBER:** A001083

**CLIENT:** Fastrate

**PROJECT STATUS:** Detailed Design

## STORM POST-DEVELOPMENT FLOW (UNCONTROLLED)

### Proposed Stormwater Management

#### APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

#### PRE-DEVELOPMENT FLOW DETERMINATION:

##### DESIGN CRITERIA:

Design Storm (year):	100	
IDF Regression Constants: (a)	1735.688	
(b)	6.014	
(c)	0.820	
IDF Curve Equation (mm/hr):	$I = a / (\text{Time in min} + b)^c$	
Rational Formula (L/s):	$Q = 2.78C \cdot I \cdot A$	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area

#### ALLOWABLE RELEASE RATE - SUMMARY:

Catchment ID	Area (A) ha	Runoff Coefficient (C) (factored)	Time of Concentration (tc) min	Intensity (I) mm/hr	Allowable Release Rate (Q) L/s	Release Flow Per Unit Area (Q/ha) L/s/ha
A1	0.76	0.89	19.43	122.15	230.315	301.22
A2	1.21	0.90	19.43	122.15	370.618	305.34
A3	0.86	0.95	19.43	122.15	276.631	322.34
A4	0.15	0.71	19.43	122.15	35.792	240.86
A5	0.11	0.95	19.43	122.15	34.458	322.34
A6	0.42	0.31	19.43	122.15	43.999	105.18
A7	0.15	0.65	19.43	122.15	32.994	220.55
Total	3.66				1024.808	280.00

#### NOTES:

1. Time of concentration taken from SWM report (JL Richards, 2009). It is assumed that the resulting time of concentration is identical to JL Richards SWM report.
2. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)
3. Runoff coefficients are increased by 25% for the 100y storm per City of Ottawa Sewer Design Guidelines.

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Date: July 20, 2021

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Date: July 20, 2021



**PROJECT NAME:** Fastfrate Warehouse Development  
Industrial/Commercial Development  
**CIMA+ PROJECT NUMBER:** A001083  
**CLIENT:** Fastfrate  
**PROJECT STATUS:** Detailed Design

**STORM POST-DEVELOPMENT FLOW (CONTROLLED)**  
**Per Master Stormwater Management Report (J.L. Richards, 2009)**

**APPLICABLE DESIGN GUIDELINES:**

1. City of Ottawa Sewer Design Guidelines, 2012

**PRE-DEVELOPMENT FLOW DETERMINATION:**

**DESIGN CRITERIA:**

Design Storm (year):	100	
IDF Regression Constants: (a)	1735.688	
(b)	6.014	
(c)	0.820	
IDF Curve Equation (mm/hr):	$I = a / (\text{Time in min} + b)^c$	
Rational Formula (L/s):	$Q = 2.78C \cdot I \cdot A$	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area

**ALLOWABLE RELEASE RATE - SUMMARY:**

Catchment ID	Area (A) ha	Runoff Coefficient (C) (factored)	Time of Concentration (tc) min	Intensity (I) mm/hr	Allowable Release Rate (Q) L/s	Release Flow Per Unit Area (Q/ha) L/s/ha
Total Site Area Draining to SWMF per JLR 2009 SWM	3.05	0.70	19.43	122.15	906.87	296.89
Total	3.05				906.867	296.89
Revised Total Area	3.66				906.867	247.78

**NOTES:**

1. Time of concentration taken from SWM report (JL Richards, 2009).
2. Runoff coefficients taken from SWM report (JL Richards, 2009).
3. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)
4. Runoff coefficients are increased by 25% for the 100y storm per City of Ottawa Sewer Design Guidelines.

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PEO No.: 100067842

Date: July 20, 2021



PROJECT NAME: Fastrate Warehouse Development  
Industrial/Commercial Development

CIMA+ PROJECT NUMBER: A001083

CLIENT: Fastrate

PROJECT STATUS: Detailed Design

**STORM POST-DEVELOPMENT FLOW (CONTROLLED)**  
**Per Master Stormwater Management Report (J.L. Richards, 2009)**

**APPLICABLE DESIGN GUIDELINES:**

1. City of Ottawa Sewer Design Guidelines, 2012

**PRE-DEVELOPMENT FLOW DETERMINATION:**

**DESIGN CRITERIA:**

Design Storm (year):	100	
IDF Regression Constants: (a)	1735.688	
(b)	6.014	
(c)	0.820	
IDF Curve Equation (mm/hr):	$I = a / (\text{Time in min} + b)^c$	
Rational Formula (L/s):	$Q = 2.78C \cdot I \cdot A$	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area

**ALLOWABLE RELEASE RATE - SUMMARY:**

Catchment ID	Area (A) ha	Runoff Coefficient (C) (factored)	Time of Concentration (tc) min	Intensity (I) mm/hr	Allowable Release Rate (Q) L/s	Release Flow Per Unit Area (Q/ha) L/s/ha
East Side Somme Street	0.32	0.88	15.00	142.89	111.140	347.31
South Side Rideau Road	0.58	0.25	26.12	100.87	40.628	70.05
East Side Somme Street (Revised)	0.00	0.88	15.00	142.89	0.000	#DIV/0!
South Side Rideau Road (Revised)	0.26	0.25	26.12	100.87	18.072	70.05
<b>Total</b>	<b>0.90</b>				<b>151.768</b>	<b>168.63</b>
<b>Revised Total Area</b>	<b>0.26</b>				<b>Actual Release Rate: Residual Release Rate:</b>	<b>18.072 133.695</b>

**NOTES:**

1. Time of concentration taken from SWM report (JL Richards, 2009).
2. Runoff coefficients taken from SWM report (JL Richards, 2009).
3. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)
4. Runoff coefficients are increased by 25% for the 100y storm per City of Ottawa Sewer Design Guidelines.

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Date: July 21, 2021



Fastfrate Warehouse Development  
Industrial/Commercial Development  
A001083 (360)

STORM WATER MANAGEMENT - SUMMARY - FULL RELEASE RATE

Rainfall event		100 years													
Sub-Area	Total Area	Capacity Area	Catchbasin Elev.	Max. Elev.	Y <sub>max</sub>	V <sub>max</sub>	V <sub>rain</sub>	Difference	V <sub>acc</sub>	Y <sub>rain</sub>	Elev <sub>rain</sub>	A <sub>rain</sub>	Q <sub>ave</sub>	Drawdown Time	Comments
	(m <sup>2</sup> )	(m <sup>2</sup> )	(m)	(m)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	(m)	(m)	(m <sup>2</sup> )	(L/s)	(min)	
A1	7646	2294	10.000	10.001	0.001	0.76	90.96	-90.19	0.76	0.00	10.001	2294	184.958	0	
A2	12138	3641	10.000	10.001	0.001	1.21	148.91	-147.69	1.21	0.00	10.001	3641	293.620	0	
A3 - Building	8582	8582	10.000	10.050	0.050	143.03	115.04	27.99	115.04	0.04	10.045	7697	211.136	9	
A4	1486	446	10.000	10.001	0.001	0.15	10.63	-10.48	0.15	0.00	10.001	446	35.947	0	
A5	1069	321	10.000	10.001	0.001	0.11	14.71	-14.60	0.11	0.00	10.001	321	25.859	0	
A6	4860	1458	10.000	10.001	0.001	0.49	6.50	-6.02	0.49	0.00	10.001	1458	117.564	0	
A7	1497	449	10.000	10.001	0.001	0.15	8.82	-8.67	0.15	0.00	10.001	449	36.213	0	
Total	37278	17191				145.90	395.55	-249.65	117.91						

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Legend:

- NC = Non-controlled areas (no storage available)
- Capacity Area = Area of water accumulated in sub-area at Max. Elev.
- Catchbasin Elev. = Elevation of catchbasin inlet (top of grate).
- Max. Elev. = Maximum elevation of water that may be accumulated within sub-area.
- Y<sub>max</sub> = Maximum depth of water that may be accumulated within the sub-area.
- V<sub>max</sub> = Maximum volume of water (capacity) that may be accumulated within the sub-area.
- V<sub>rain</sub> = Volume of water generated by rainfall.
- Difference = Difference between V<sub>max</sub> and V<sub>rain</sub> (remaining capacity of sub-area)
- V<sub>acc</sub> = Total volume of water accumulated within the sub-area in the event of a specific rainfall.
- Y<sub>rain</sub> = Depth of water generated by rainfall.
- Elev<sub>rain</sub> = Elevation of water generated by rainfall.
- A<sub>rain</sub> = Area of water generated by rainfall.
- Q<sub>ave</sub> = Average flow (for drawdown time calculation).
- Drawdown Time = Time required for the total volume of water accumulated within sub-area to evacuate (following rainfall event).

Design Criteria:

- 1) Maximum Allowable Release Rate = 247.78 L/s/ha
- 2) ~~Pipe size for 10 years~~
- 3) Rainfall event of 100 years
- 4) ~~Pre-development flow (5 year) = \_\_\_\_ L/s (or \_\_\_\_ L/s/ha)~~

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Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 22, 2021



Catchment ID	Release Rate	Specified Flow rate	Calculated area
	L/s/ha	L/s	(mm <sup>2</sup> )
A1	241.93	184.98	50482
A2	241.93	293.66	80140
A3 - Building	247.78	212.65	57299
A4	241.93	35.95	9811
A5	241.93	25.86	7058
A6	241.93	117.58	32088
A7	241.93	36.22	9884
Total Flowrate		906.90	

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Préparé par: Guillaume LeBlond, M.A.Sc., EIT  
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Date: July 22, 2021

Vérifié par: Christian Lavoie-Lebel, P.Eng.  
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Date: July 22, 2021



## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
 Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station:** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File:** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210719\_Storm Water Management - Storage and Drawdown\_full RR.xlsx\A1  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 241.9332667 L/s/ha

**Area :** A1 0.7646 ha  
**Runoff Coefficient C (unfactored)** 0.71  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.8875  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.184982176 m³/s  
**Discharge Factor K :** 1

**Design Volume:** 90.96 m³

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.810	0.810	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.820	0.820	0.820	0.820

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
 PEO No.: 100530467

Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
 PEO No.: 100067842

Date: July 22, 2021

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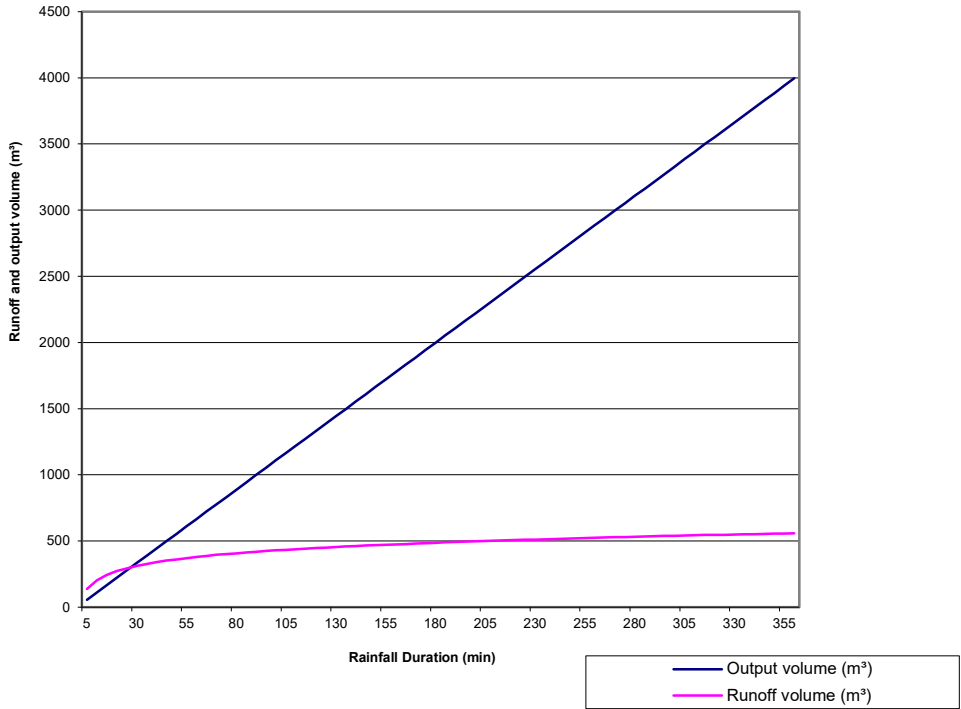
<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIA T</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	137.25	55.4946527	81.75
10.0	178.56	201.95	110.989305	90.96
15.0	142.89	242.41	166.483958	75.93
20.0	119.95	271.32	221.978611	49.34
25.0	103.85	293.62	277.473264	16.15
30.0	91.87	311.70	332.967916	-21.27
35.0	82.58	326.88	388.462569	-61.58
40.0	75.15	339.95	443.957222	-104.01
45.0	69.05	351.42	499.451874	-148.03
50.0	63.95	361.65	554.946527	-193.30
55.0	59.62	370.88	610.44118	-239.56
60.0	55.89	379.29	665.935833	-286.64
65.0	52.65	387.02	721.430485	-334.41
70.0	49.79	394.17	776.925138	-382.75
75.0	47.26	400.83	832.419791	-431.59
80.0	44.99	407.07	887.914443	-480.85
85.0	42.95	412.93	943.409096	-530.48
90.0	41.11	418.46	998.903749	-580.45
95.0	39.43	423.70	1054.3984	-630.70
100.0	37.90	428.67	1109.89305	-681.22
105.0	36.50	433.41	1165.38771	-731.97
110.0	35.20	437.94	1220.88236	-782.94
115.0	34.01	442.28	1276.37701	-834.10
120.0	32.89	446.44	1331.87167	-885.43
125.0	31.86	450.44	1387.36632	-936.93
130.0	30.90	454.28	1442.86097	-988.58
135.0	30.00	458.00	1498.35562	-1040.36
140.0	29.15	461.58	1553.85028	-1092.27
145.0	28.36	465.05	1609.34493	-1144.30
150.0	27.61	468.40	1664.83958	-1196.44
155.0	26.91	471.66	1720.33423	-1248.68
160.0	26.24	474.81	1775.82889	-1301.01
165.0	25.61	477.88	1831.32354	-1353.44
170.0	25.01	480.87	1886.81819	-1405.95
175.0	24.44	483.77	1942.31285	-1458.54
180.0	23.90	486.60	1997.8075	-1511.21
185.0	23.39	489.35	2053.30215	-1563.95
190.0	22.90	492.04	2108.7968	-1616.76
195.0	22.43	494.67	2164.29146	-1669.63
200.0	21.98	497.23	2219.78611	-1722.56
205.0	21.55	499.74	2275.28076	-1775.54
210.0	21.14	502.19	2330.77541	-1828.59
215.0	20.75	504.59	2386.27007	-1881.68
220.0	20.37	506.94	2441.76472	-1934.83
225.0	20.01	509.24	2497.25937	-1988.02
230.0	19.66	511.50	2552.75402	-2041.26
235.0	19.33	513.71	2608.24868	-2094.54
240.0	19.01	515.88	2663.74333	-2147.87

245.0	18.69	518.01	2719.23798	-2201.23
250.0	18.39	520.10	2774.73264	-2254.63
255.0	18.11	522.15	2830.22729	-2308.07
260.0	17.83	524.17	2885.72194	-2361.55
265.0	17.56	526.16	2941.21659	-2415.06
270.0	17.29	528.11	2996.71125	-2468.60
275.0	17.04	530.03	3052.2059	-2522.18
280.0	16.80	531.92	3107.70055	-2575.78
285.0	16.56	533.78	3163.1952	-2629.42
290.0	16.33	535.61	3218.68986	-2683.08
295.0	16.11	537.41	3274.18451	-2736.78
300.0	15.89	539.18	3329.67916	-2790.50
305.0	15.68	540.93	3385.17382	-2844.24
310.0	15.48	542.66	3440.66847	-2898.01
315.0	15.28	544.36	3496.16312	-2951.81
320.0	15.09	546.03	3551.65777	-3005.62
325.0	14.90	547.69	3607.15243	-3059.47
330.0	14.72	549.32	3662.64708	-3113.33
335.0	14.54	550.93	3718.14173	-3167.21
340.0	14.37	552.52	3773.63638	-3221.12
345.0	14.20	554.08	3829.13104	-3275.05
350.0	14.04	555.63	3884.62569	-3328.99
355.0	13.88	557.16	3940.12034	-3382.96
360.0	13.72	558.67	3995.615	-3436.94
<b>Max Volume (V max):</b>				90.96
<b>Design Volume (V design) :</b>				<b>90.96</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

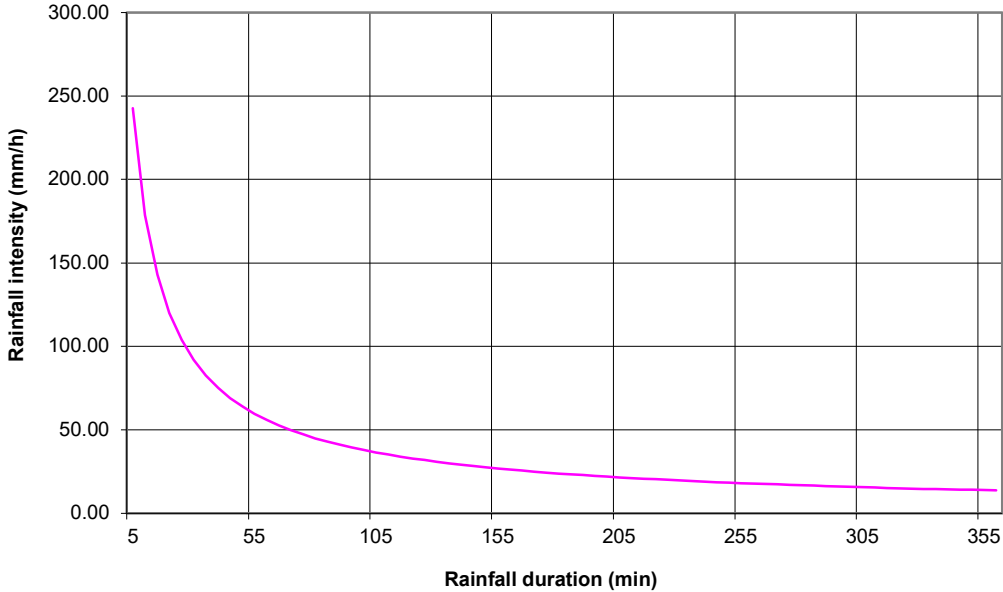
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Storage volume sizing with the rational method



Rainfall I-D-F curve

**100** yea







## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

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**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 241.9332667 L/s/ha

**Area** : A2 1.2138 ha  
**Runoff Coefficient C (unfactored)** 0.72  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.9  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.293658599 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 148.91 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 22, 2021

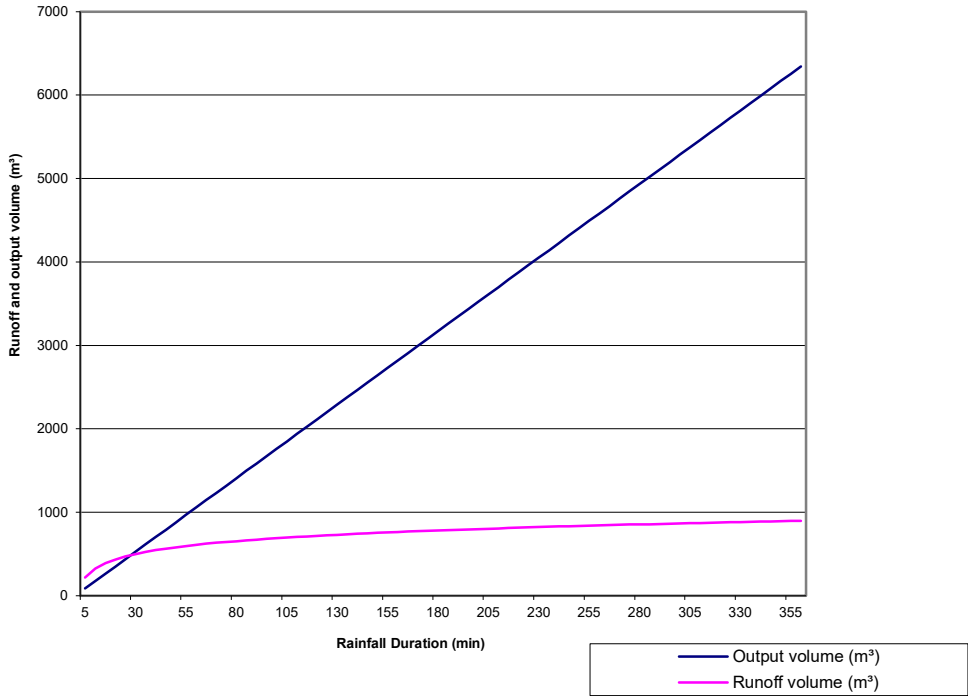
<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIAT</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	220.95	88.0975797	132.85
10.0	178.56	325.10	176.195159	148.91
15.0	142.89	390.25	264.292739	125.96
20.0	119.95	436.79	352.390319	84.40
25.0	103.85	472.69	440.487899	32.20
30.0	91.87	501.79	528.585478	-26.79
35.0	82.58	526.23	616.683058	-90.46
40.0	75.15	547.27	704.780638	-157.51
45.0	69.05	565.74	792.878218	-227.14
50.0	63.95	582.21	880.975797	-298.77
55.0	59.62	597.06	969.073377	-372.01
60.0	55.89	610.60	1057.17096	-446.57
65.0	52.65	623.05	1145.26854	-522.22
70.0	49.79	634.56	1233.36612	-598.80
75.0	47.26	645.29	1321.4637	-676.18
80.0	44.99	655.32	1409.56128	-754.24
85.0	42.95	664.75	1497.65886	-832.91
90.0	41.11	673.66	1585.75644	-912.10
95.0	39.43	682.09	1673.85402	-991.77
100.0	37.90	690.10	1761.95159	-1071.85
105.0	36.50	697.73	1850.04917	-1152.32
110.0	35.20	705.02	1938.14675	-1233.12
115.0	34.01	712.00	2026.24433	-1314.24
120.0	32.89	718.70	2114.34191	-1395.64
125.0	31.86	725.14	2202.43949	-1477.30
130.0	30.90	731.33	2290.53707	-1559.20
135.0	30.00	737.31	2378.63465	-1641.33
140.0	29.15	743.08	2466.73223	-1723.65
145.0	28.36	748.66	2554.82981	-1806.17
150.0	27.61	754.06	2642.92739	-1888.87
155.0	26.91	759.30	2731.02497	-1971.72
160.0	26.24	764.38	2819.12255	-2054.74
165.0	25.61	769.32	2907.22013	-2137.90
170.0	25.01	774.12	2995.31771	-2221.19
175.0	24.44	778.80	3083.41529	-2304.62
180.0	23.90	783.35	3171.51287	-2388.16
185.0	23.39	787.79	3259.61045	-2471.82
190.0	22.90	792.12	3347.70803	-2555.59
195.0	22.43	796.34	3435.80561	-2639.46
200.0	21.98	800.47	3523.90319	-2723.43
205.0	21.55	804.50	3612.00077	-2807.50
210.0	21.14	808.45	3700.09835	-2891.65
215.0	20.75	812.31	3788.19593	-2975.88
220.0	20.37	816.10	3876.29351	-3060.20
225.0	20.01	819.80	3964.39109	-3144.59
230.0	19.66	823.43	4052.48867	-3229.05
235.0	19.33	827.00	4140.58625	-3313.59
240.0	19.01	830.49	4228.68383	-3398.19

245.0	18.69	833.92	4316.78141	-3482.86
250.0	18.39	837.29	4404.87899	-3567.59
255.0	18.11	840.59	4492.97657	-3652.38
260.0	17.83	843.84	4581.07415	-3737.23
265.0	17.56	847.04	4669.17173	-3822.13
270.0	17.29	850.18	4757.26931	-3907.09
275.0	17.04	853.27	4845.36689	-3992.10
280.0	16.80	856.31	4933.46447	-4077.16
285.0	16.56	859.30	5021.56205	-4162.26
290.0	16.33	862.25	5109.65962	-4247.41
295.0	16.11	865.15	5197.7572	-4332.61
300.0	15.89	868.01	5285.85478	-4417.85
305.0	15.68	870.82	5373.95236	-4503.13
310.0	15.48	873.60	5462.04994	-4588.45
315.0	15.28	876.34	5550.14752	-4673.81
320.0	15.09	879.04	5638.2451	-4759.21
325.0	14.90	881.70	5726.34268	-4844.65
330.0	14.72	884.32	5814.44026	-4930.12
335.0	14.54	886.91	5902.53784	-5015.62
340.0	14.37	889.47	5990.63542	-5101.16
345.0	14.20	892.00	6078.733	-5186.74
350.0	14.04	894.49	6166.83058	-5272.34
355.0	13.88	896.95	6254.92816	-5357.98
360.0	13.72	899.38	6343.02574	-5443.64
<b>Max Volume (V max):</b>				<b>148.91</b>
<b>Design Volume (V design) :</b>				<b>148.91</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

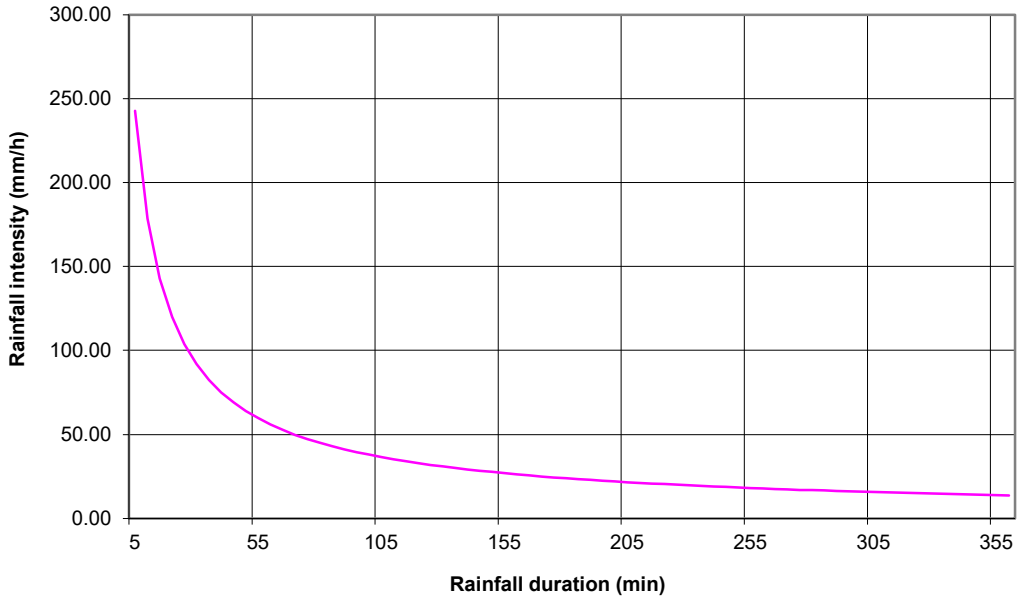
Sub-bassin **A2**

Storage volume sizing with the rational method



Rainfall I-D-F curve

100 yea





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
 Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210719\_Storm Water Management - Storage and Drawdown\_full  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 247.7840805 L/s/ha

**Area** : A3 - Building 0.8582 ha  
**Runoff Coefficient C (unfactored):** 0.9  
**C<sub>runoff</sub> factor:** -  
**Runoff Coefficient C :** 0.95  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.212648298 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 115.04 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
 PEO No.: 100530467

Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
 PEO No.: 100067842

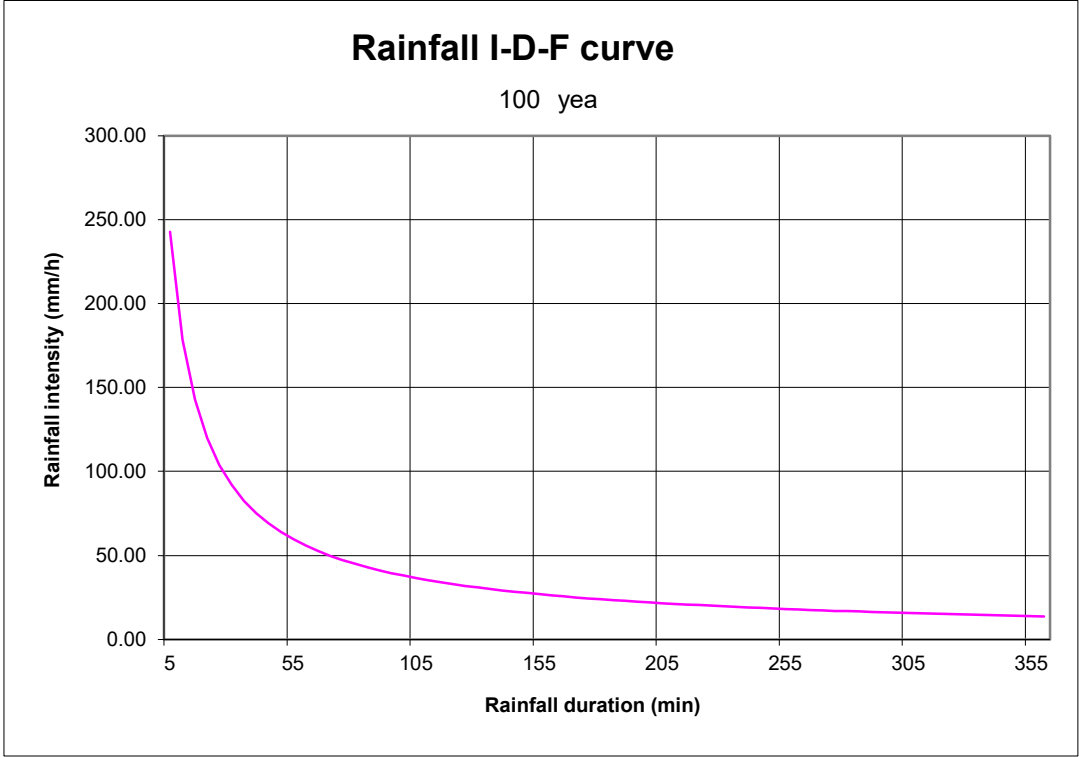
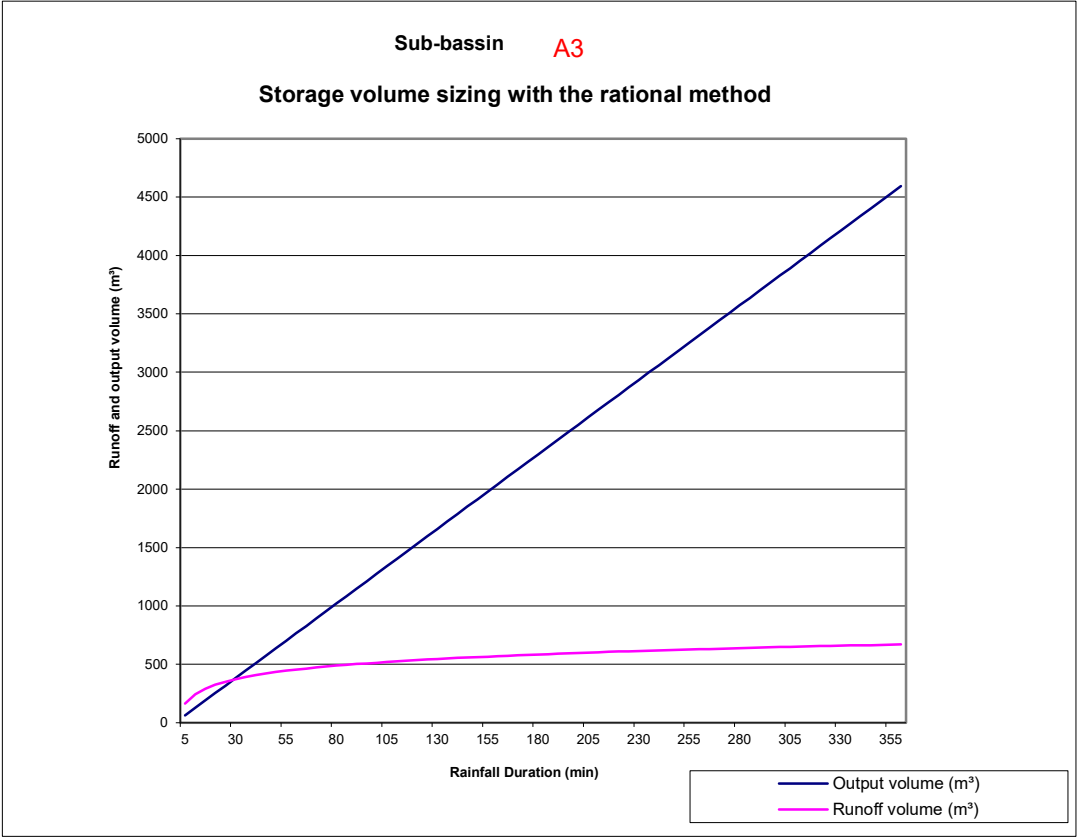
Date: July 22, 2021

<b>Rainfall Duration (min) <i>T</i> (1)</b>	<b>Rainfall Intensity (mm/h) <i>I</i> (2)</b>	<b>Runoff Volume (m<sup>3</sup>) <i>CIA T</i> (4)</b>	<b>Output Volume (m<sup>3</sup>) <i>kQT</i> (5)</b>	<b>Retention Volume (m<sup>3</sup>) (4)-(5) (6)</b>
5.0	242.70	164.90	63.7944894	101.10
10.0	178.56	242.63	127.588979	115.04
15.0	142.89	291.25	191.383468	99.87
20.0	119.95	325.98	255.177957	70.80
25.0	103.85	352.77	318.972447	33.80
30.0	91.87	374.50	382.766936	-8.27
35.0	82.58	392.73	446.561426	-53.83
40.0	75.15	408.43	510.355915	-101.92
45.0	69.05	422.22	574.150404	-151.93
50.0	63.95	434.51	637.944894	-203.44
55.0	59.62	445.60	701.739383	-256.14
60.0	55.89	455.70	765.533872	-309.83
65.0	52.65	464.99	829.328362	-364.34
70.0	49.79	473.58	893.122851	-419.54
75.0	47.26	481.59	956.91734	-475.33
80.0	44.99	489.08	1020.71183	-531.64
85.0	42.95	496.12	1084.50632	-588.39
90.0	41.11	502.76	1148.30081	-645.54
95.0	39.43	509.05	1212.0953	-703.04
100.0	37.90	515.03	1275.88979	-760.86
105.0	36.50	520.73	1339.68428	-818.95
110.0	35.20	526.17	1403.47877	-877.31
115.0	34.01	531.38	1467.27326	-935.89
120.0	32.89	536.38	1531.06774	-994.69
125.0	31.86	541.18	1594.86223	-1053.68
130.0	30.90	545.80	1658.65672	-1112.85
135.0	30.00	550.26	1722.45121	-1172.19
140.0	29.15	554.57	1786.2457	-1231.67
145.0	28.36	558.74	1850.04019	-1291.30
150.0	27.61	562.77	1913.83468	-1351.07
155.0	26.91	566.68	1977.62917	-1410.95
160.0	26.24	570.47	2041.42366	-1470.95
165.0	25.61	574.16	2105.21815	-1531.06
170.0	25.01	577.74	2169.01264	-1591.27
175.0	24.44	581.23	2232.80713	-1651.58
180.0	23.90	584.63	2296.60162	-1711.98
185.0	23.39	587.94	2360.39611	-1772.46
190.0	22.90	591.17	2424.1906	-1833.02
195.0	22.43	594.32	2487.98509	-1893.66
200.0	21.98	597.40	2551.77957	-1954.38
205.0	21.55	600.41	2615.57406	-2015.16
210.0	21.14	603.36	2679.36855	-2076.01
215.0	20.75	606.24	2743.16304	-2136.92
220.0	20.37	609.07	2806.95753	-2197.89
225.0	20.01	611.83	2870.75202	-2258.92
230.0	19.66	614.54	2934.54651	-2320.00
235.0	19.33	617.20	2998.341	-2381.14
240.0	19.01	619.81	3062.13549	-2442.33



245.0	18.69	622.37	3125.92998	-2503.56
250.0	18.39	624.88	3189.72447	-2564.84
255.0	18.11	627.35	3253.51896	-2626.17
260.0	17.83	629.77	3317.31345	-2687.54
265.0	17.56	632.16	3381.10794	-2748.95
270.0	17.29	634.50	3444.90243	-2810.40
275.0	17.04	636.81	3508.69691	-2871.89
280.0	16.80	639.08	3572.4914	-2933.41
285.0	16.56	641.31	3636.28589	-2994.98
290.0	16.33	643.51	3700.08038	-3056.57
295.0	16.11	645.67	3763.87487	-3118.20
300.0	15.89	647.81	3827.66936	-3179.86
305.0	15.68	649.91	3891.46385	-3241.55
310.0	15.48	651.98	3955.25834	-3303.28
315.0	15.28	654.02	4019.05283	-3365.03
320.0	15.09	656.04	4082.84732	-3426.81
325.0	14.90	658.02	4146.64181	-3488.62
330.0	14.72	659.98	4210.4363	-3550.45
335.0	14.54	661.92	4274.23079	-3612.31
340.0	14.37	663.83	4338.02528	-3674.20
345.0	14.20	665.71	4401.81977	-3736.11
350.0	14.04	667.57	4465.61426	-3798.04
355.0	13.88	669.41	4529.40874	-3860.00
360.0	13.72	671.22	4593.20323	-3921.98
<b>Max Volume (V max):</b>				115.04
<b>Design Volume (V design) :</b>				<b>115.04</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

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**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 241.9332667 L/s/ha

**Area** : A4 0.1486 ha  
**Runoff Coefficient C (unfactored)** 0.57  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.7125  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.035951283 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 10.63 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 22, 2021

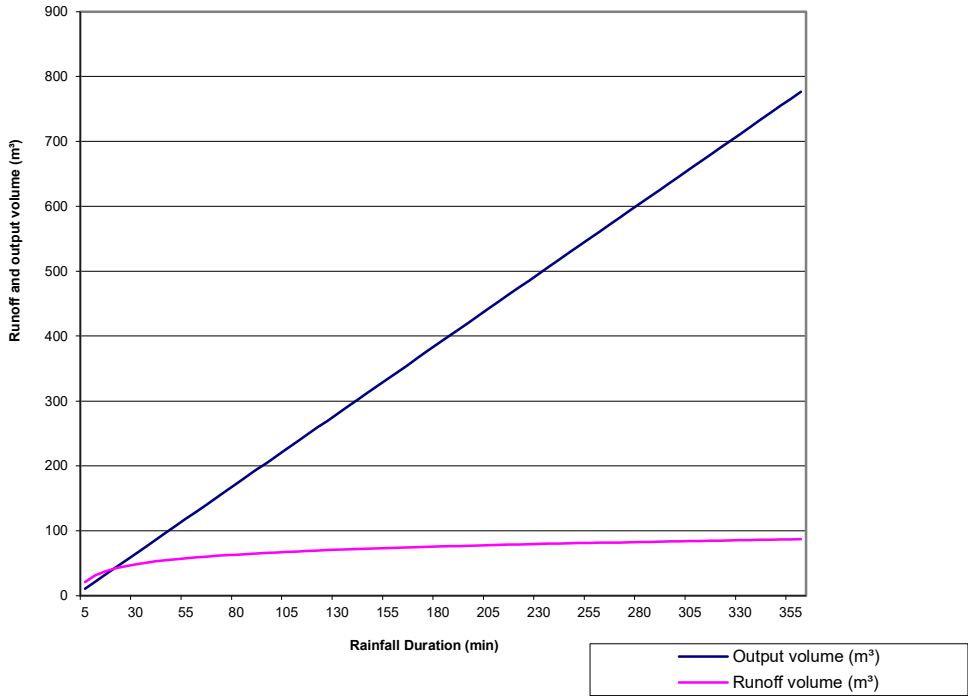
<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIAT</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	21.41	10.785385	10.63
10.0	178.56	31.51	21.5707701	9.94
15.0	142.89	37.82	32.3561551	5.47
20.0	119.95	42.33	43.1415401	-0.81
25.0	103.85	45.81	53.9269251	-8.11
30.0	91.87	48.63	64.7123102	-16.08
35.0	82.58	51.00	75.4976952	-24.50
40.0	75.15	53.04	86.2830802	-33.24
45.0	69.05	54.83	97.0684653	-42.24
50.0	63.95	56.43	107.85385	-51.43
55.0	59.62	57.87	118.639235	-60.77
60.0	55.89	59.18	129.42462	-70.24
65.0	52.65	60.39	140.210005	-79.82
70.0	49.79	61.50	150.99539	-89.49
75.0	47.26	62.54	161.780775	-99.24
80.0	44.99	63.51	172.56616	-109.05
85.0	42.95	64.43	183.351546	-118.92
90.0	41.11	65.29	194.136931	-128.85
95.0	39.43	66.11	204.922316	-138.81
100.0	37.90	66.88	215.707701	-148.82
105.0	36.50	67.62	226.493086	-158.87
110.0	35.20	68.33	237.278471	-168.95
115.0	34.01	69.01	248.063856	-179.06
120.0	32.89	69.66	258.849241	-189.19
125.0	31.86	70.28	269.634626	-199.35
130.0	30.90	70.88	280.420011	-209.54
135.0	30.00	71.46	291.205396	-219.75
140.0	29.15	72.02	301.990781	-229.97
145.0	28.36	72.56	312.776166	-240.22
150.0	27.61	73.08	323.561551	-250.48
155.0	26.91	73.59	334.346936	-260.76
160.0	26.24	74.08	345.132321	-271.05
165.0	25.61	74.56	355.917706	-281.35
170.0	25.01	75.03	366.703091	-291.67
175.0	24.44	75.48	377.488476	-302.01
180.0	23.90	75.92	388.273861	-312.35
185.0	23.39	76.35	399.059246	-322.71
190.0	22.90	76.77	409.844631	-333.07
195.0	22.43	77.18	420.630016	-343.45
200.0	21.98	77.58	431.415401	-353.83
205.0	21.55	77.97	442.200786	-364.23
210.0	21.14	78.36	452.986171	-374.63
215.0	20.75	78.73	463.771556	-385.04
220.0	20.37	79.10	474.556941	-395.46
225.0	20.01	79.46	485.342326	-405.89
230.0	19.66	79.81	496.127711	-416.32
235.0	19.33	80.15	506.913096	-426.76
240.0	19.01	80.49	517.698481	-437.21

245.0	18.69	80.82	528.483866	-447.66
250.0	18.39	81.15	539.269251	-458.12
255.0	18.11	81.47	550.054637	-468.58
260.0	17.83	81.79	560.840022	-479.05
265.0	17.56	82.10	571.625407	-489.53
270.0	17.29	82.40	582.410792	-500.01
275.0	17.04	82.70	593.196177	-510.50
280.0	16.80	82.99	603.981562	-520.99
285.0	16.56	83.28	614.766947	-531.48
290.0	16.33	83.57	625.552332	-541.98
295.0	16.11	83.85	636.337717	-552.49
300.0	15.89	84.13	647.123102	-563.00
305.0	15.68	84.40	657.908487	-573.51
310.0	15.48	84.67	668.693872	-584.02
315.0	15.28	84.93	679.479257	-594.54
320.0	15.09	85.20	690.264642	-605.07
325.0	14.90	85.45	701.050027	-615.60
330.0	14.72	85.71	711.835412	-626.13
335.0	14.54	85.96	722.620797	-636.66
340.0	14.37	86.21	733.406182	-647.20
345.0	14.20	86.45	744.191567	-657.74
350.0	14.04	86.69	754.976952	-668.28
355.0	13.88	86.93	765.762337	-678.83
360.0	13.72	87.17	776.547722	-689.38
<b>Max Volume (V max):</b>				10.63
<b>Design Volume (V design) :</b>				<b>10.63</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

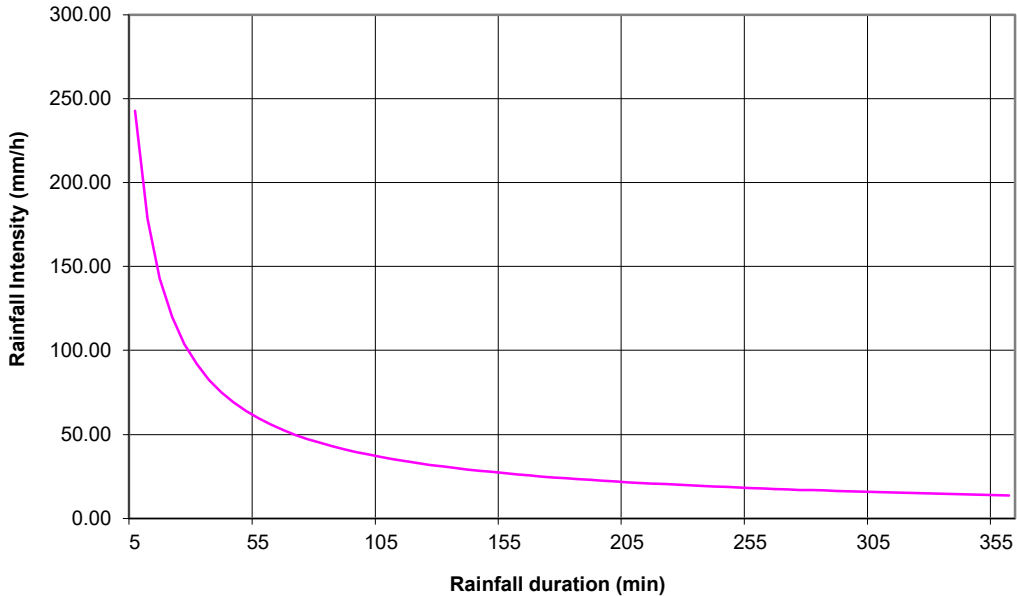
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Storage volume sizing with the rational method



Rainfall I-D-F curve

100 ya







## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

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**Location:** - - - - -

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 241.9332667 L/s/ha

**Area** : A5 0.1069 ha  
**Runoff Coefficient C (unfactored** 0.9  
**C\_runoff factor:** -  
**Runoff Coefficient C :** 0.95  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.025862666 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 14.71 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 22, 2021

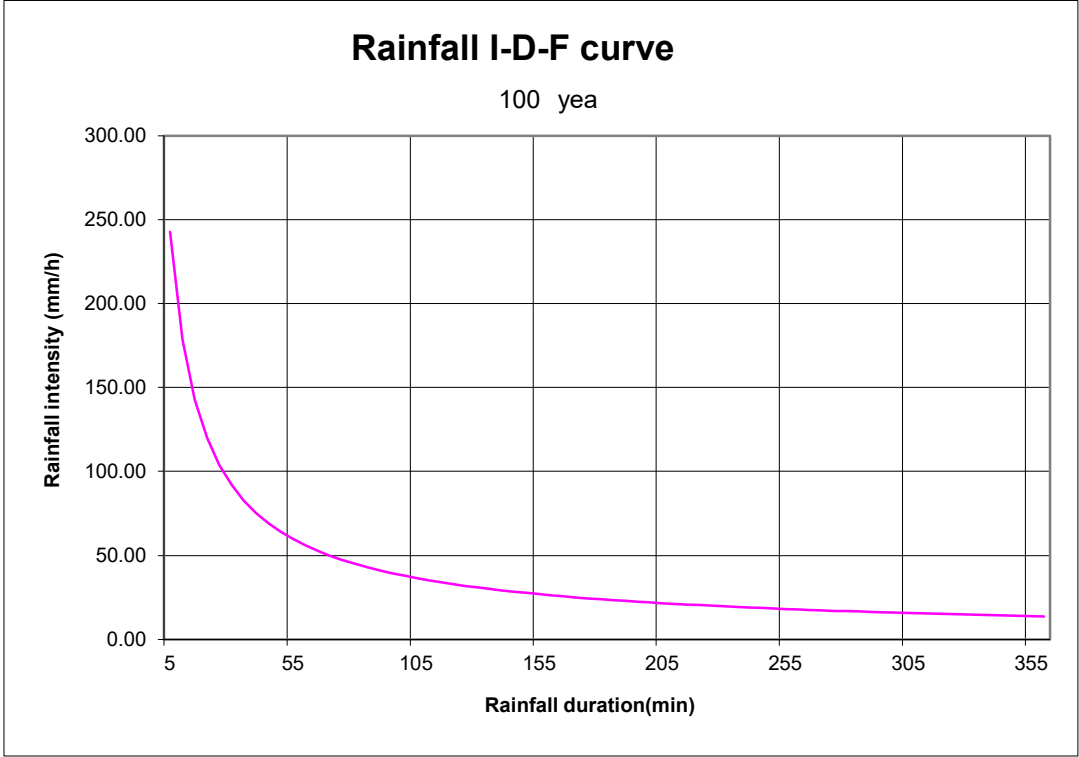
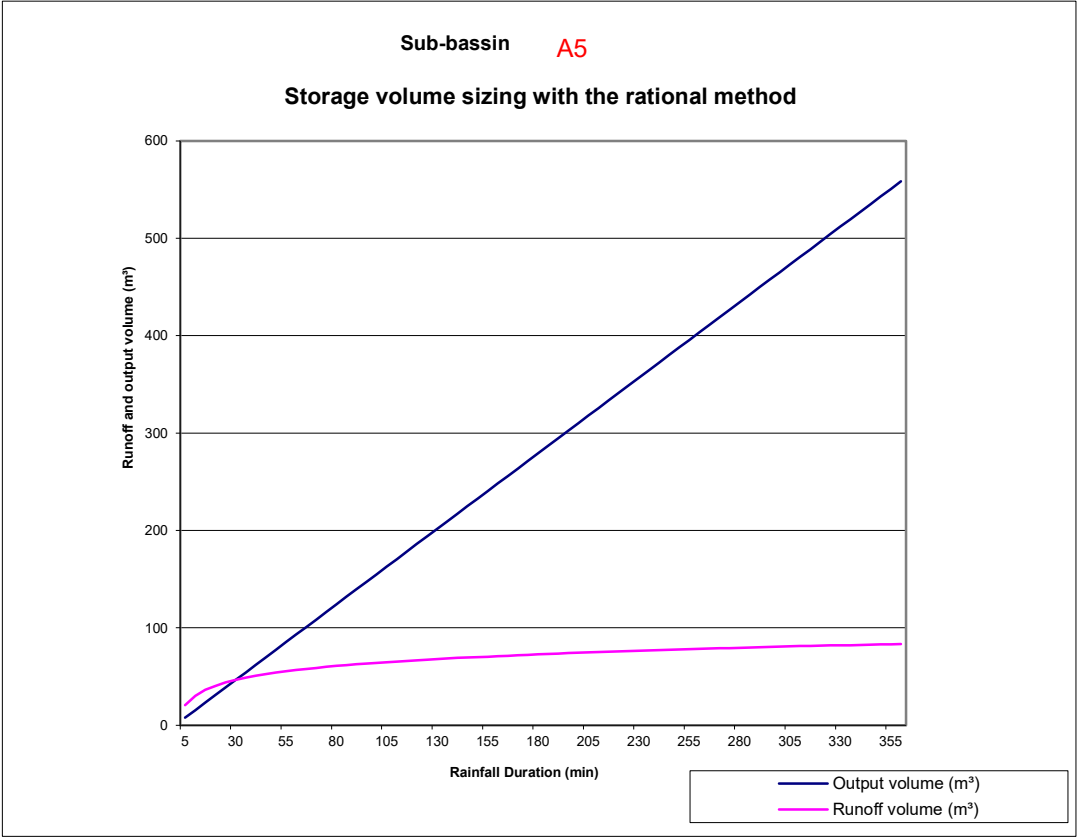
Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 22, 2021

<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIAT</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	20.54	7.75879986	12.78
10.0	178.56	30.22	15.5175997	14.71
15.0	142.89	36.28	23.2763996	13.00
20.0	119.95	40.61	31.0351995	9.57
25.0	103.85	43.94	38.7939993	5.15
30.0	91.87	46.65	46.5527992	0.10
35.0	82.58	48.92	54.311599	-5.39
40.0	75.15	50.88	62.0703989	-11.19
45.0	69.05	52.59	69.8291988	-17.24
50.0	63.95	54.12	77.5879986	-23.46
55.0	59.62	55.51	85.3467985	-29.84
60.0	55.89	56.76	93.1055984	-36.34
65.0	52.65	57.92	100.864398	-42.94
70.0	49.79	58.99	108.623198	-49.63
75.0	47.26	59.99	116.381998	-56.39
80.0	44.99	60.92	124.140798	-63.22
85.0	42.95	61.80	131.899598	-70.10
90.0	41.11	62.63	139.658398	-77.03
95.0	39.43	63.41	147.417197	-84.01
100.0	37.90	64.15	155.175997	-91.02
105.0	36.50	64.86	162.934797	-98.07
110.0	35.20	65.54	170.693597	-105.15
115.0	34.01	66.19	178.452397	-112.26
120.0	32.89	66.81	186.211197	-119.40
125.0	31.86	67.41	193.969997	-126.56
130.0	30.90	67.99	201.728796	-133.74
135.0	30.00	68.54	209.487596	-140.95
140.0	29.15	69.08	217.246396	-148.17
145.0	28.36	69.60	225.005196	-155.41
150.0	27.61	70.10	232.763996	-162.66
155.0	26.91	70.59	240.522796	-169.94
160.0	26.24	71.06	248.281596	-177.22
165.0	25.61	71.52	256.040395	-184.52
170.0	25.01	71.97	263.799195	-191.83
175.0	24.44	72.40	271.557995	-199.16
180.0	23.90	72.82	279.316795	-206.49
185.0	23.39	73.24	287.075595	-213.84
190.0	22.90	73.64	294.834395	-221.20
195.0	22.43	74.03	302.593195	-228.56
200.0	21.98	74.41	310.351995	-235.94
205.0	21.55	74.79	318.110794	-243.32
210.0	21.14	75.16	325.869594	-250.71
215.0	20.75	75.52	333.628394	-258.11
220.0	20.37	75.87	341.387194	-265.52
225.0	20.01	76.21	349.145994	-272.93
230.0	19.66	76.55	356.904794	-280.36
235.0	19.33	76.88	364.663594	-287.78
240.0	19.01	77.21	372.422393	-295.22

245.0	18.69	77.52	380.181193	-302.66
250.0	18.39	77.84	387.939993	-310.10
255.0	18.11	78.14	395.698793	-317.55
260.0	17.83	78.45	403.457593	-325.01
265.0	17.56	78.74	411.216393	-332.47
270.0	17.29	79.04	418.975193	-339.94
275.0	17.04	79.32	426.733992	-347.41
280.0	16.80	79.61	434.492792	-354.89
285.0	16.56	79.88	442.251592	-362.37
290.0	16.33	80.16	450.010392	-369.85
295.0	16.11	80.43	457.769192	-377.34
300.0	15.89	80.69	465.527992	-384.84
305.0	15.68	80.95	473.286792	-392.33
310.0	15.48	81.21	481.045592	-399.83
315.0	15.28	81.47	488.804391	-407.34
320.0	15.09	81.72	496.563191	-414.85
325.0	14.90	81.97	504.321991	-422.36
330.0	14.72	82.21	512.080791	-429.87
335.0	14.54	82.45	519.839591	-437.39
340.0	14.37	82.69	527.598391	-444.91
345.0	14.20	82.92	535.357191	-452.43
350.0	14.04	83.15	543.11599	-459.96
355.0	13.88	83.38	550.87479	-467.49
360.0	13.72	83.61	558.63359	-475.02
<b>Max Volume (V max):</b>				14.71
<b>Design Volume (V design) :</b>				<b>14.71</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210719\_Storm Water Management - Storage and Drawdown\_full  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 241.9332667 L/s/ha

**Area** : A6 0.486 ha  
**Runoff Coefficient C (unfactored)** 0.34  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.425  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.117579568 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 6.50 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

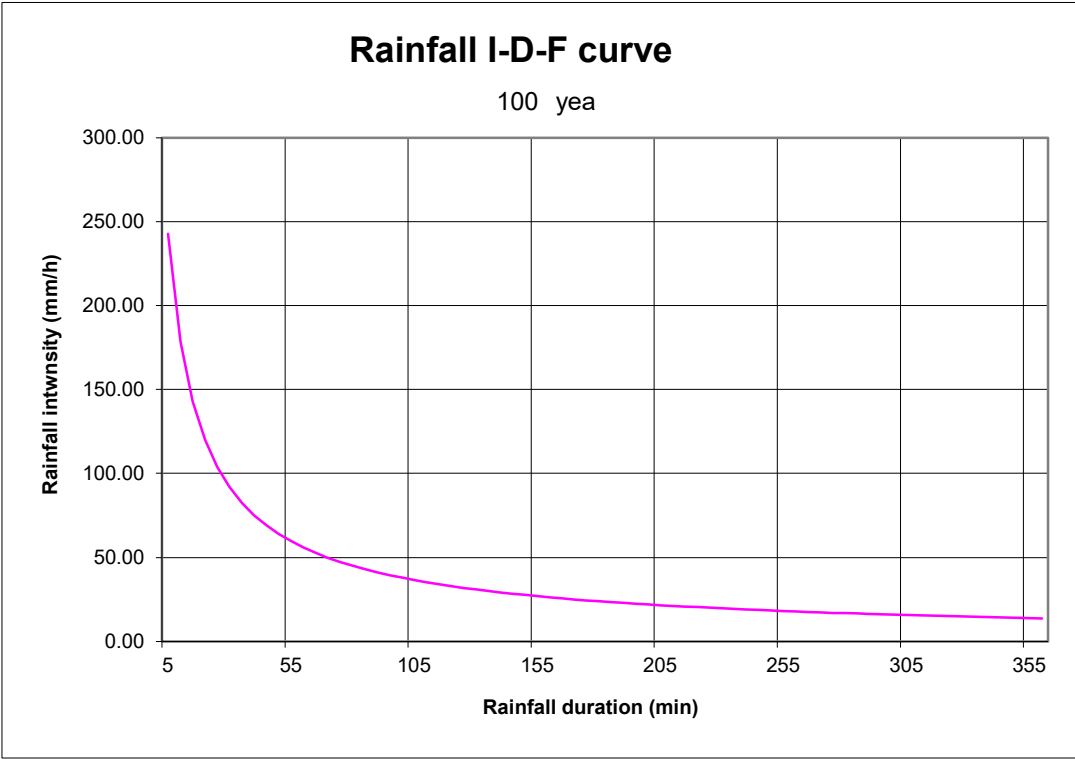
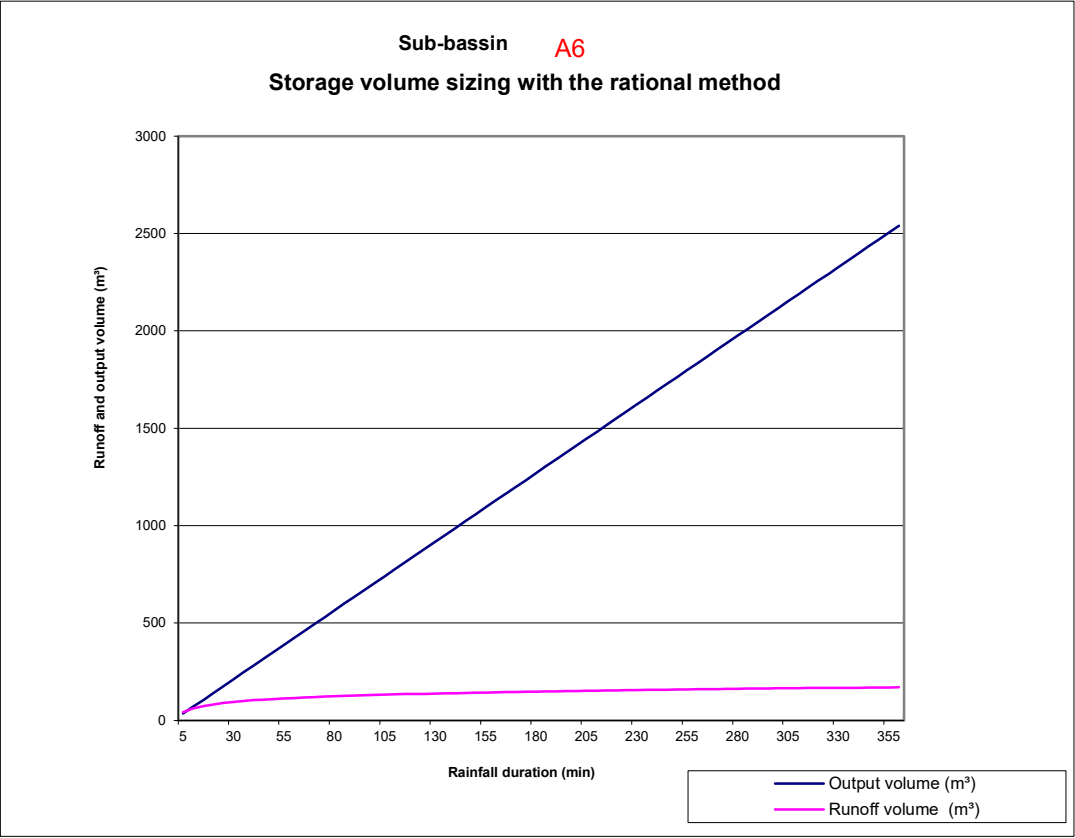
Date: July 22, 2021

<b>Rainfall Duration (min) <math>T</math> (1)</b>	<b>Rainfall Intensity (mm/h) <math>I</math> (2)</b>	<b>Runoff Volume (m<sup>3</sup>) <math>CIAT</math> (4)</b>	<b>Output Volume (m<sup>3</sup>) <math>kQT</math> (5)</b>	<b>Retention Volume (m<sup>3</sup>) (4)-(5) (6)</b>
5.0	242.70	41.78	35.2738703	6.50
10.0	178.56	61.47	70.5477406	-9.08
15.0	142.89	73.79	105.821611	-32.03
20.0	119.95	82.59	141.095481	-58.51
25.0	103.85	89.37	176.369351	-87.00
30.0	91.87	94.88	211.643222	-116.77
35.0	82.58	99.50	246.917092	-147.42
40.0	75.15	103.48	282.190962	-178.72
45.0	69.05	106.97	317.464833	-210.50
50.0	63.95	110.08	352.738703	-242.66
55.0	59.62	112.89	388.012573	-275.12
60.0	55.89	115.45	423.286443	-307.84
65.0	52.65	117.80	458.560314	-340.76
70.0	49.79	119.98	493.834184	-373.85
75.0	47.26	122.01	529.108054	-407.10
80.0	44.99	123.91	564.381925	-440.48
85.0	42.95	125.69	599.655795	-473.97
90.0	41.11	127.37	634.929665	-507.56
95.0	39.43	128.97	670.203535	-541.24
100.0	37.90	130.48	705.477406	-575.00
105.0	36.50	131.92	740.751276	-608.83
110.0	35.20	133.30	776.025146	-642.72
115.0	34.01	134.62	811.299017	-676.68
120.0	32.89	135.89	846.572887	-710.68
125.0	31.86	137.11	881.846757	-744.74
130.0	30.90	138.28	917.120627	-778.84
135.0	30.00	139.41	952.394498	-812.99
140.0	29.15	140.50	987.668368	-847.17
145.0	28.36	141.55	1022.94224	-881.39
150.0	27.61	142.57	1058.21611	-915.64
155.0	26.91	143.57	1093.48998	-949.92
160.0	26.24	144.53	1128.76385	-984.24
165.0	25.61	145.46	1164.03772	-1018.58
170.0	25.01	146.37	1199.31159	-1052.94
175.0	24.44	147.25	1234.58546	-1087.33
180.0	23.90	148.11	1269.85933	-1121.75
185.0	23.39	148.95	1305.1332	-1156.18
190.0	22.90	149.77	1340.40707	-1190.64
195.0	22.43	150.57	1375.68094	-1225.11
200.0	21.98	151.35	1410.95481	-1259.61
205.0	21.55	152.11	1446.22868	-1294.12
210.0	21.14	152.86	1481.50255	-1328.64
215.0	20.75	153.59	1516.77642	-1363.19
220.0	20.37	154.30	1552.05029	-1397.75
225.0	20.01	155.00	1587.32416	-1432.32
230.0	19.66	155.69	1622.59803	-1466.91
235.0	19.33	156.36	1657.8719	-1501.51
240.0	19.01	157.03	1693.14577	-1536.12



245.0	18.69	157.67	1728.41964	-1570.75
250.0	18.39	158.31	1763.69351	-1605.38
255.0	18.11	158.94	1798.96738	-1640.03
260.0	17.83	159.55	1834.24125	-1674.69
265.0	17.56	160.15	1869.51513	-1709.36
270.0	17.29	160.75	1904.789	-1744.04
275.0	17.04	161.33	1940.06287	-1778.73
280.0	16.80	161.91	1975.33674	-1813.43
285.0	16.56	162.47	2010.61061	-1848.14
290.0	16.33	163.03	2045.88448	-1882.85
295.0	16.11	163.58	2081.15835	-1917.58
300.0	15.89	164.12	2116.43222	-1952.31
305.0	15.68	164.65	2151.70609	-1987.05
310.0	15.48	165.18	2186.97996	-2021.80
315.0	15.28	165.69	2222.25383	-2056.56
320.0	15.09	166.20	2257.5277	-2091.32
325.0	14.90	166.71	2292.80157	-2126.09
330.0	14.72	167.20	2328.07544	-2160.87
335.0	14.54	167.69	2363.34931	-2195.66
340.0	14.37	168.18	2398.62318	-2230.45
345.0	14.20	168.65	2433.89705	-2265.24
350.0	14.04	169.13	2469.17092	-2300.04
355.0	13.88	169.59	2504.44479	-2334.85
360.0	13.72	170.05	2539.71866	-2369.67
<b>Max Volume (V max):</b>				<b>6.50</b>
<b>Design Volume (V design) :</b>				<b>6.50</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210719\_Storm Water Management - Storage and Drawdown\_full RR.xlsx]A7  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 241.9332667 L/s/ha

**Area** : A7 0.1497 ha  
**Runoff Coefficient C (unfactored)** 0.52  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.65  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.03621741 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 8.82 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 22, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 22, 2021

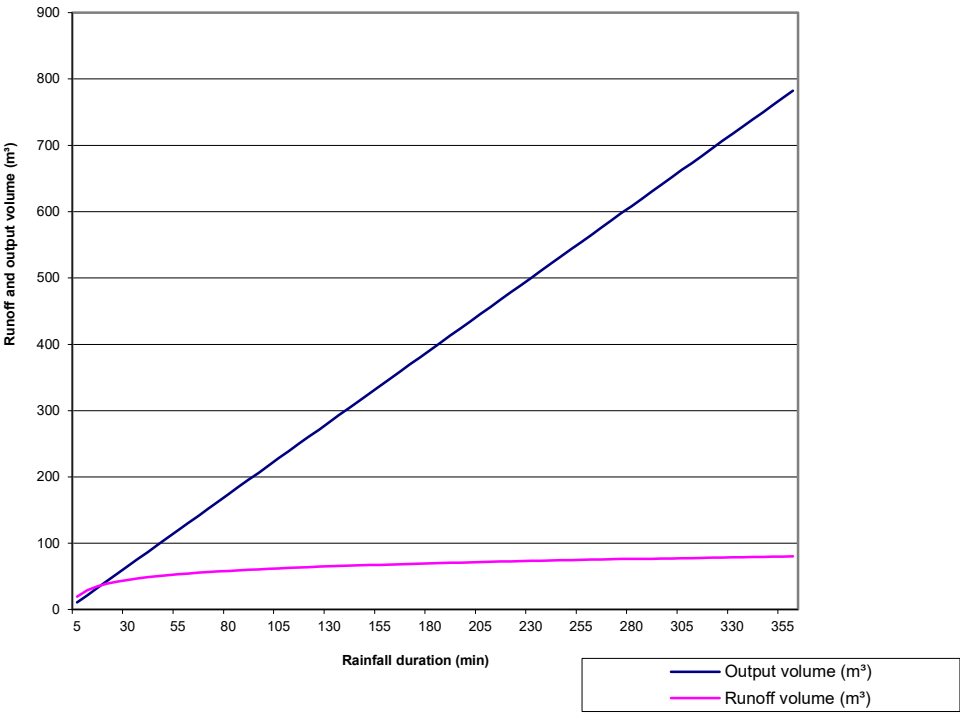
<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIA T</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	19.68	10.865223	8.82
10.0	178.56	28.96	21.730446	7.23
15.0	142.89	34.76	32.595669	2.17
20.0	119.95	38.91	43.460892	-4.55
25.0	103.85	42.10	54.326115	-12.22
30.0	91.87	44.70	65.191338	-20.50
35.0	82.58	46.87	76.0565611	-29.18
40.0	75.15	48.75	86.9217841	-38.18
45.0	69.05	50.39	97.7870071	-47.39
50.0	63.95	51.86	108.65223	-56.79
55.0	59.62	53.18	119.517453	-66.34
60.0	55.89	54.39	130.382676	-75.99
65.0	52.65	55.50	141.247899	-85.75
70.0	49.79	56.52	152.113122	-95.59
75.0	47.26	57.48	162.978345	-105.50
80.0	44.99	58.37	173.843568	-115.47
85.0	42.95	59.21	184.708791	-125.50
90.0	41.11	60.00	195.574014	-135.57
95.0	39.43	60.76	206.439237	-145.68
100.0	37.90	61.47	217.30446	-155.84
105.0	36.50	62.15	228.169683	-166.02
110.0	35.20	62.80	239.034906	-176.24
115.0	34.01	63.42	249.900129	-186.48
120.0	32.89	64.02	260.765352	-196.75
125.0	31.86	64.59	271.630575	-207.04
130.0	30.90	65.14	282.495798	-217.35
135.0	30.00	65.67	293.361021	-227.69
140.0	29.15	66.19	304.226244	-238.04
145.0	28.36	66.69	315.091467	-248.41
150.0	27.61	67.17	325.95669	-258.79
155.0	26.91	67.63	336.821913	-269.19
160.0	26.24	68.09	347.687136	-279.60
165.0	25.61	68.53	358.552359	-290.03
170.0	25.01	68.95	369.417582	-300.46
175.0	24.44	69.37	380.282805	-310.91
180.0	23.90	69.78	391.148028	-321.37
185.0	23.39	70.17	402.013251	-331.84
190.0	22.90	70.56	412.878474	-342.32
195.0	22.43	70.93	423.743697	-352.81
200.0	21.98	71.30	434.60892	-363.31
205.0	21.55	71.66	445.474143	-373.81
210.0	21.14	72.01	456.339366	-384.33
215.0	20.75	72.36	467.204589	-394.85
220.0	20.37	72.69	478.069812	-405.38
225.0	20.01	73.02	488.935035	-415.91
230.0	19.66	73.35	499.800258	-426.45
235.0	19.33	73.66	510.665481	-437.00

240.0	19.01	73.97	521.530704	-447.56
245.0	18.69	74.28	532.395927	-458.12
250.0	18.39	74.58	543.26115	-468.68
255.0	18.11	74.87	554.126373	-479.25
260.0	17.83	75.16	564.991596	-489.83
265.0	17.56	75.45	575.856819	-500.41
270.0	17.29	75.73	586.722042	-510.99
275.0	17.04	76.00	597.587265	-521.58
280.0	16.80	76.27	608.452488	-532.18
285.0	16.56	76.54	619.317711	-542.78
290.0	16.33	76.80	630.182934	-553.38
295.0	16.11	77.06	641.048157	-563.99
300.0	15.89	77.32	651.91338	-574.60
305.0	15.68	77.57	662.778603	-585.21
310.0	15.48	77.81	673.643826	-595.83
315.0	15.28	78.06	684.509049	-606.45
320.0	15.09	78.30	695.374272	-617.08
325.0	14.90	78.54	706.239495	-627.70
330.0	14.72	78.77	717.104719	-638.34
335.0	14.54	79.00	727.969942	-648.97
340.0	14.37	79.23	738.835165	-659.61
345.0	14.20	79.45	749.700388	-670.25
350.0	14.04	79.67	760.565611	-680.89
355.0	13.88	79.89	771.430834	-691.54
360.0	13.72	80.11	782.296057	-702.19
<b>Max Volume (V max):</b>				<b>8.82</b>
<b>Design Volume (V design) :</b>				<b>8.82</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

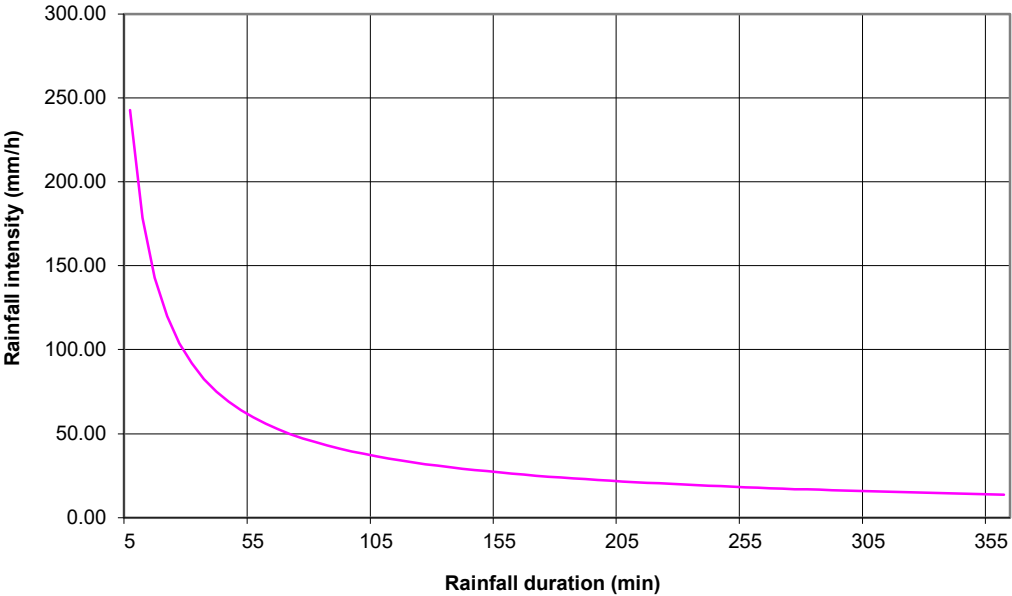
Sub-bassin **A7**

Storage volume sizing with the rational method



Rainfall I-D-F curve

100 ya







Date: 2021-09-21

Fastfrate Warehouse Development  
Industrial/Commercial Development  
A001083 (360)

STORM WATER MANAGEMENT - SUMMARY - HALF RELEASE RATE

Rainfall event		100 years													
Sub-Area	Total Area	Capacity Area	Catchbasin Elev.	Max. Elev.	Y <sub>max</sub>	V <sub>max</sub>	V <sub>rain</sub>	Difference	V <sub>acc</sub>	Y <sub>rain</sub>	Elev <sub>rain</sub>	A <sub>rain</sub>	Q <sub>ave</sub>	Drawdown Time	Comments
	(m <sup>2</sup> )	(m <sup>2</sup> )	(m)	(m)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	(m)	(m)	(m <sup>2</sup> )	(L/s)	(min)	
A1	7646	2294	10.000	10.001	0.001	0.76	197.16	-196.39	0.76	0.00	10.001	2294	64.300	0	
A2	12138	3641	10.000	10.001	0.001	1.21	319.55	-318.34	1.21	0.00	10.001	3641	102.076	0	
A3 - Building	8582	8582	10.000	10.050	0.050	143.03	115.04	27.99	115.04	0.04	10.045	7697	211.132	9	
A4	1486	446	10.000	10.001	0.001	0.15	27.34	-27.19	0.15	0.00	10.001	446	12.497	0	
A5	1069	321	10.000	10.001	0.001	0.11	30.46	-30.36	0.11	0.00	10.001	321	8.990	0	
A6	4860	1458	10.000	10.001	0.001	0.49	37.00	-36.51	0.49	0.00	10.001	1458	40.871	0	
A7	1497	449	10.000	10.001	0.001	0.15	23.80	-23.65	0.15	0.00	10.001	449	12.589	0	
Total	37278	17191					145.90	750.35	-604.44	117.91					

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Legend:

- NC = Non-controlled areas (no storage available)
- Capacity Area = Area of water accumulated in sub-area at Max. Elev.
- Catchbasin Elev. = Elevation of catchbasin inlet (top of grate).
- Max. Elev. = Maximum elevation of water that may be accumulated within sub-area.
- Y<sub>max</sub> = Maximum depth of water that may be accumulated within the sub-area.
- V<sub>max</sub> = Maximum volume of water (capacity) that may be accumulated within the sub-area.
- V<sub>rain</sub> = Volume of water generated by rainfall.
- Difference = Difference between V<sub>max</sub> and V<sub>rain</sub> (remaining capacity of sub-area)
- V<sub>acc</sub> = Total volume of water accumulated within the sub-area in the event of a specific rainfall.
- Y<sub>rain</sub> = Depth of water generated by rainfall.
- Elev<sub>rain</sub> = Elevation of water generated by rainfall.
- A<sub>rain</sub> = Area of water generated by rainfall.
- Q<sub>ave</sub> = Average flow (for drawdown time calculation).
- Drawdown Time = Time required for the total volume of water accumulated within sub-area to evacuate (following rainfall event).

Design Criteria:

- 1) Maximum Allowable Release Rate = 124.04 L/s/ha
- 2) ~~Pipe size for 10 years~~
- 3) Rainfall event of 100 years
- 4) ~~Pre-development flow (5 year) = \_\_\_\_ L/s (or \_\_\_\_ L/s/ha)~~

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 23, 2021

Catchment ID	Release Rate	Specified Flow rate	Calculated area
	L/s/ha	L/s	(mm <sup>2</sup> )
A1	84.11	64.31	17550
A2	84.11	102.09	27861
A3 - Building	247.78	212.64	57298
A4	84.11	12.50	3411
A5	84.11	8.99	2454
A6	84.11	40.88	11155
A7	84.11	12.59	3436
Total Flowrate		454.00	

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Préparé par: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 23, 2021

Vérifié par: Christian Lavoie 2) Pipe size for 10 years  
PEO No.: 100067842

Date: July 23, 2021



## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
 Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station:** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File:** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210723\_Storm Water Management - Storage and Drawdown\_half

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 84.10757773 L/s/ha

**Area :** A1 0.7646 ha  
**Runoff Coefficient C (unfactored)** 0.71  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.8875  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.064308654 m³/s  
**Discharge Factor K :** 1

**Design Volume:** 197.16 m³

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.810	0.810	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.820	0.820	0.820	0.820

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
 PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
 PEO No.: 100067842

Date: July 23, 2021

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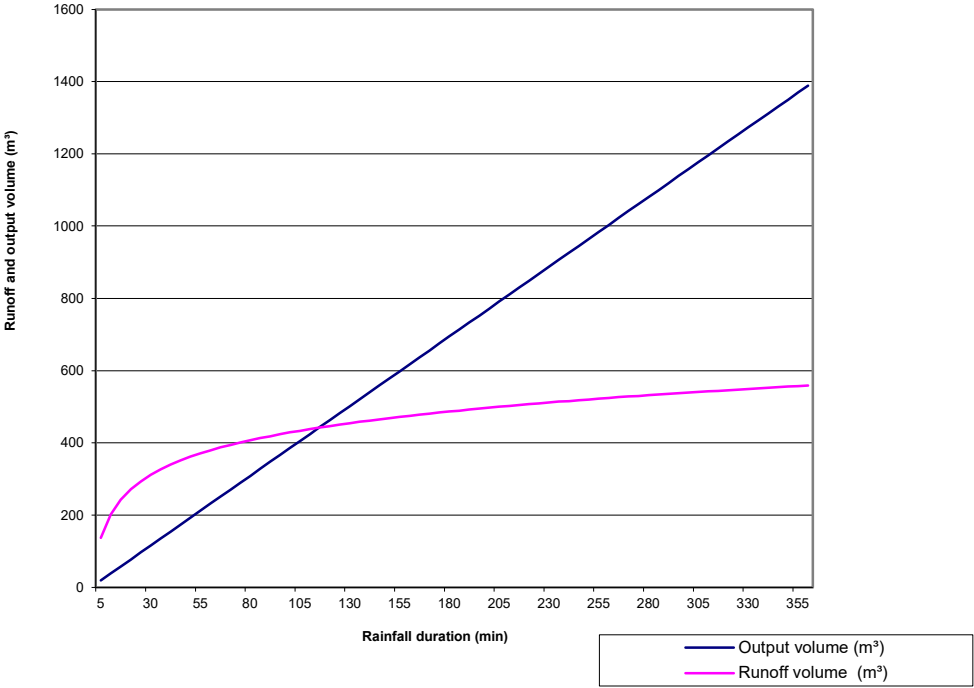
<b>Rainfall Duration (min) <math>T</math> (1)</b>	<b>Rainfall Intensity (mm/h) <math>I</math> (2)</b>	<b>Runoff Volume (m<sup>3</sup>) <math>CIAT</math> (4)</b>	<b>Output Volume (m<sup>3</sup>) <math>kQT</math> (5)</b>	<b>Retention Volume (m<sup>3</sup>) (4)-(5) (6)</b>
5.0	242.70	137.25	19.2925962	117.95
10.0	178.56	201.95	38.5851924	163.36
15.0	142.89	242.41	57.8777885	184.54
20.0	119.95	271.32	77.1703847	194.15
25.0	103.85	293.62	96.4629809	197.16
30.0	91.87	311.70	115.755577	195.95
35.0	82.58	326.88	135.048173	191.83
40.0	75.15	339.95	154.340769	185.61
45.0	69.05	351.42	173.633366	177.79
50.0	63.95	361.65	192.925962	168.73
55.0	59.62	370.88	212.218558	158.66
60.0	55.89	379.29	231.511154	147.78
65.0	52.65	387.02	250.80375	136.22
70.0	49.79	394.17	270.096347	124.08
75.0	47.26	400.83	289.388943	111.45
80.0	44.99	407.07	308.681539	98.39
85.0	42.95	412.93	327.974135	84.95
90.0	41.11	418.46	347.266731	71.19
95.0	39.43	423.70	366.559327	57.14
100.0	37.90	428.67	385.851924	42.82
105.0	36.50	433.41	405.14452	28.27
110.0	35.20	437.94	424.437116	13.51
115.0	34.01	442.28	443.729712	-1.45
120.0	32.89	446.44	463.022308	-16.58
125.0	31.86	450.44	482.314904	-31.88
130.0	30.90	454.28	501.607501	-47.32
135.0	30.00	458.00	520.900097	-62.90
140.0	29.15	461.58	540.192693	-78.61
145.0	28.36	465.05	559.485289	-94.44
150.0	27.61	468.40	578.777885	-110.37
155.0	26.91	471.66	598.070482	-126.41
160.0	26.24	474.81	617.363078	-142.55
165.0	25.61	477.88	636.655674	-158.77
170.0	25.01	480.87	655.94827	-175.08
175.0	24.44	483.77	675.240866	-191.47
180.0	23.90	486.60	694.533462	-207.94
185.0	23.39	489.35	713.826059	-224.47
190.0	22.90	492.04	733.118655	-241.08
195.0	22.43	494.67	752.411251	-257.75
200.0	21.98	497.23	771.703847	-274.47
205.0	21.55	499.74	790.996443	-291.26
210.0	21.14	502.19	810.28904	-308.10
215.0	20.75	504.59	829.581636	-324.99
220.0	20.37	506.94	848.874232	-341.94
225.0	20.01	509.24	868.166828	-358.93
230.0	19.66	511.50	887.459424	-375.96
235.0	19.33	513.71	906.75202	-393.04
240.0	19.01	515.88	926.044617	-410.17

245.0	18.69	518.01	945.337213	-427.33
250.0	18.39	520.10	964.629809	-444.53
255.0	18.11	522.15	983.922405	-461.77
260.0	17.83	524.17	1003.215	-479.04
265.0	17.56	526.16	1022.5076	-496.35
270.0	17.29	528.11	1041.80019	-513.69
275.0	17.04	530.03	1061.09279	-531.06
280.0	16.80	531.92	1080.38539	-548.47
285.0	16.56	533.78	1099.67798	-565.90
290.0	16.33	535.61	1118.97058	-583.37
295.0	16.11	537.41	1138.26317	-600.86
300.0	15.89	539.18	1157.55577	-618.37
305.0	15.68	540.93	1176.84837	-635.92
310.0	15.48	542.66	1196.14096	-653.48
315.0	15.28	544.36	1215.43356	-671.08
320.0	15.09	546.03	1234.72616	-688.69
325.0	14.90	547.69	1254.01875	-706.33
330.0	14.72	549.32	1273.31135	-723.99
335.0	14.54	550.93	1292.60394	-741.68
340.0	14.37	552.52	1311.89654	-759.38
345.0	14.20	554.08	1331.18914	-777.10
350.0	14.04	555.63	1350.48173	-794.85
355.0	13.88	557.16	1369.77433	-812.61
360.0	13.72	558.67	1389.06692	-830.39
<b>Max Volume (V max):</b>				197.16
<b>Design Volume (V design) :</b>				<b>197.16</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

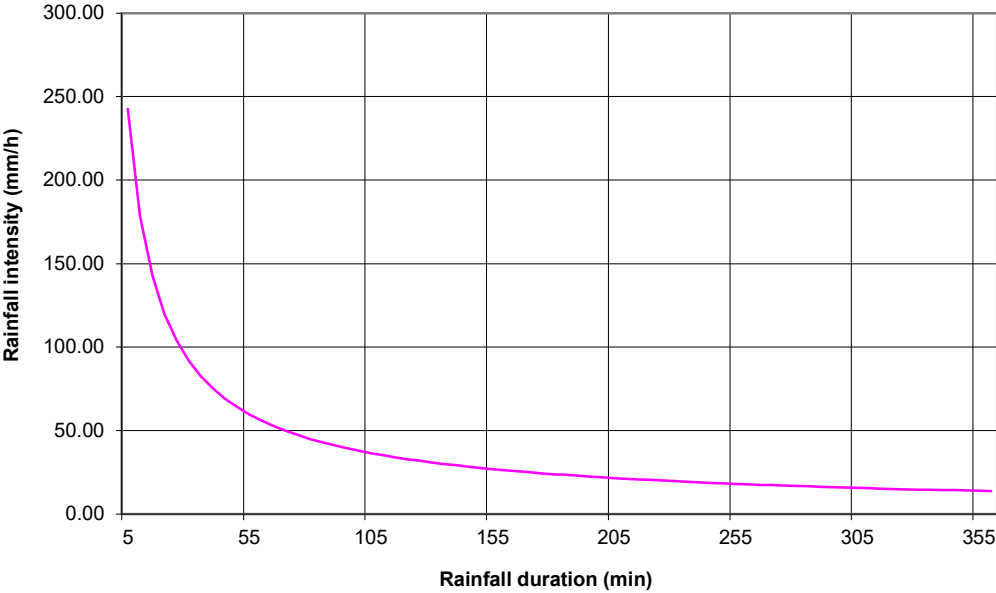
Sub-bassin **A1**

Storage volume sizing with the rational method



Rainfall I-D-F curve

**100** yea





## **STORAGE VOLUME CALCULATIONS**

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210723\_Storm Water Management - Storage and Drawdown\_half  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 84.10757773 L/s/ha

**Area** : A2 1.2138 ha  
**Runoff Coefficient C (unfactored)** 0.72  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.9  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.102089778 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 319.55 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

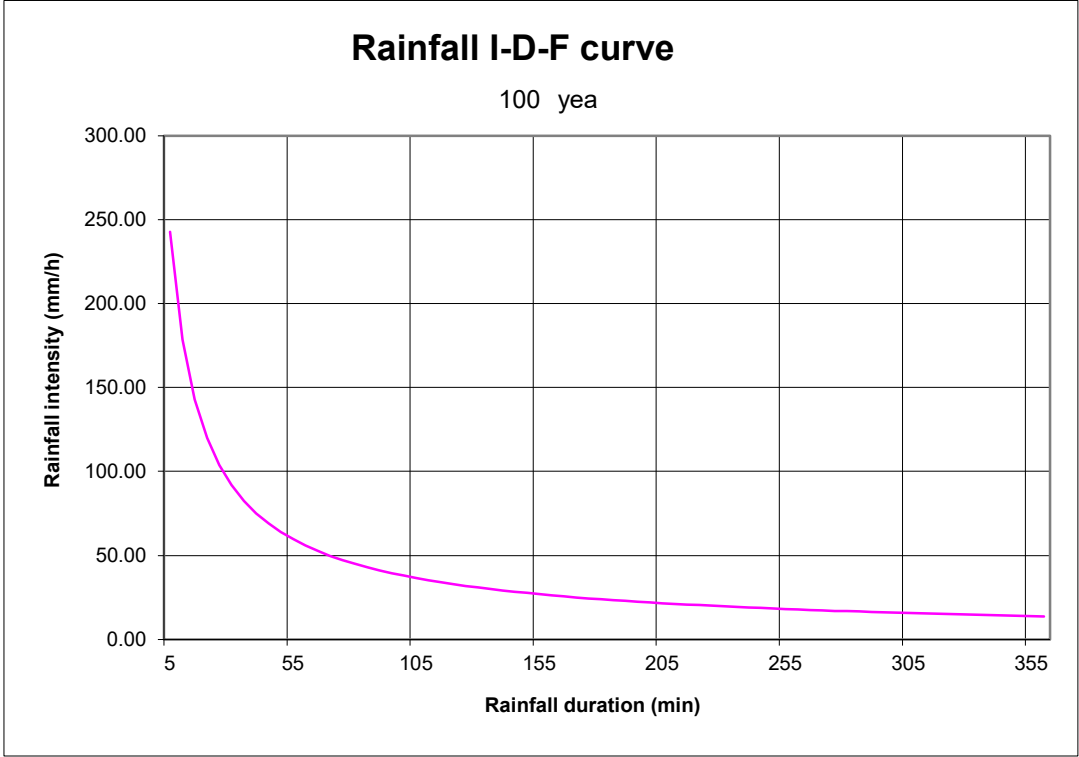
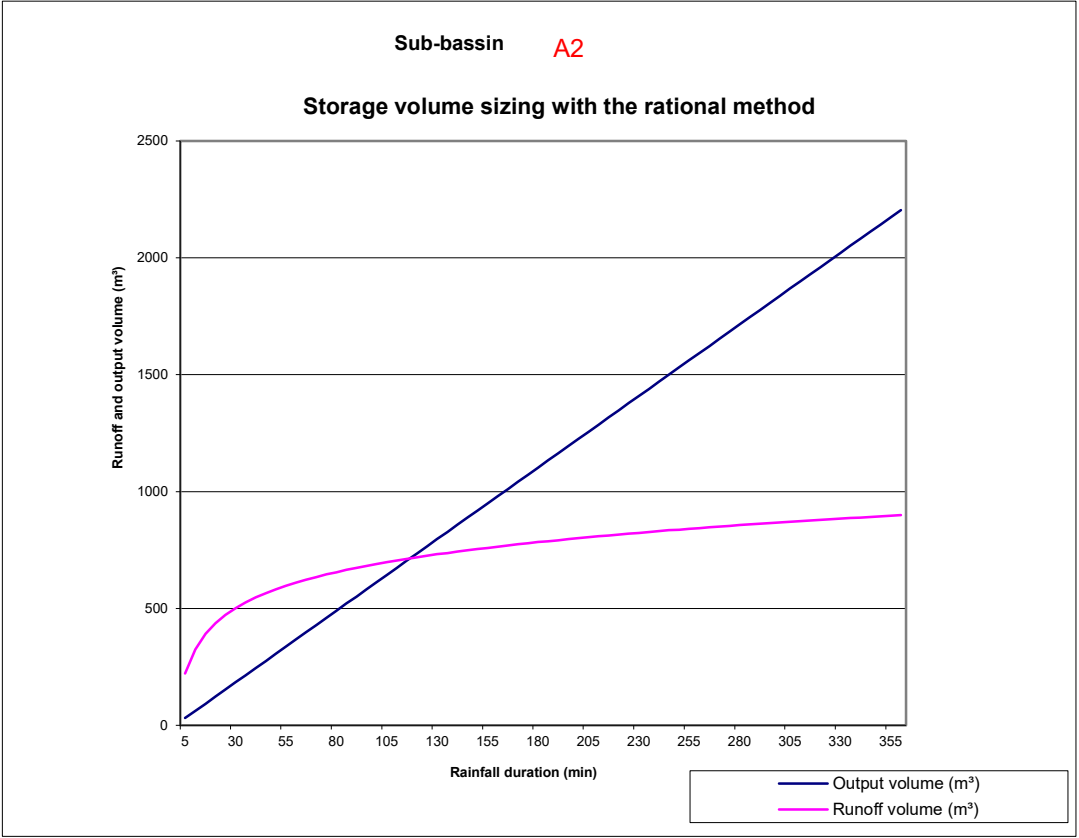
Date: July 23, 2021



<b>Rainfall Duration (min) <math>T</math> (1)</b>	<b>Rainfall Intensity (mm/h) <math>I</math> (2)</b>	<b>Runoff Volume (m<sup>3</sup>) <math>CIAT</math> (4)</b>	<b>Output Volume (m<sup>3</sup>) <math>kQT</math> (5)</b>	<b>Retention Volume (m<sup>3</sup>) (4)-(5) (6)</b>
5.0	242.70	220.95	30.6269334	190.32
10.0	178.56	325.10	61.2538667	263.85
15.0	142.89	390.25	91.8808001	298.37
20.0	119.95	436.79	122.507733	314.28
25.0	103.85	472.69	153.134667	319.55
30.0	91.87	501.79	183.7616	318.03
35.0	82.58	526.23	214.388533	311.84
40.0	75.15	547.27	245.015467	302.25
45.0	69.05	565.74	275.6424	290.10
50.0	63.95	582.21	306.269334	275.94
55.0	59.62	597.06	336.896267	260.17
60.0	55.89	610.60	367.5232	243.08
65.0	52.65	623.05	398.150134	224.90
70.0	49.79	634.56	428.777067	205.79
75.0	47.26	645.29	459.404	185.88
80.0	44.99	655.32	490.030934	165.29
85.0	42.95	664.75	520.657867	144.10
90.0	41.11	673.66	551.2848	122.37
95.0	39.43	682.09	581.911734	100.18
100.0	37.90	690.10	612.538667	77.56
105.0	36.50	697.73	643.1656	54.57
110.0	35.20	705.02	673.792534	31.23
115.0	34.01	712.00	704.419467	7.58
120.0	32.89	718.70	735.046401	-16.35
125.0	31.86	725.14	765.673334	-40.54
130.0	30.90	731.33	796.300267	-64.97
135.0	30.00	737.31	826.927201	-89.62
140.0	29.15	743.08	857.554134	-114.48
145.0	28.36	748.66	888.181067	-139.52
150.0	27.61	754.06	918.808001	-164.75
155.0	26.91	759.30	949.434934	-190.13
160.0	26.24	764.38	980.061867	-215.68
165.0	25.61	769.32	1010.6888	-241.37
170.0	25.01	774.12	1041.31573	-267.19
175.0	24.44	778.80	1071.94267	-293.15
180.0	23.90	783.35	1102.5696	-319.22
185.0	23.39	787.79	1133.19653	-345.41
190.0	22.90	792.12	1163.82347	-371.71
195.0	22.43	796.34	1194.4504	-398.11
200.0	21.98	800.47	1225.07733	-424.61
205.0	21.55	804.50	1255.70427	-451.20
210.0	21.14	808.45	1286.3312	-477.88
215.0	20.75	812.31	1316.95813	-504.64
220.0	20.37	816.10	1347.58507	-531.49
225.0	20.01	819.80	1378.212	-558.41
230.0	19.66	823.43	1408.83893	-585.40
235.0	19.33	827.00	1439.46587	-612.47
240.0	19.01	830.49	1470.0928	-639.60

245.0	18.69	833.92	1500.71973	-666.80
250.0	18.39	837.29	1531.34667	-694.06
255.0	18.11	840.59	1561.9736	-721.38
260.0	17.83	843.84	1592.60053	-748.76
265.0	17.56	847.04	1623.22747	-776.19
270.0	17.29	850.18	1653.8544	-803.68
275.0	17.04	853.27	1684.48133	-831.21
280.0	16.80	856.31	1715.10827	-858.80
285.0	16.56	859.30	1745.7352	-886.43
290.0	16.33	862.25	1776.36213	-914.11
295.0	16.11	865.15	1806.98907	-941.84
300.0	15.89	868.01	1837.616	-969.61
305.0	15.68	870.82	1868.24293	-997.42
310.0	15.48	873.60	1898.86987	-1025.27
315.0	15.28	876.34	1929.4968	-1053.16
320.0	15.09	879.04	1960.12373	-1081.09
325.0	14.90	881.70	1990.75067	-1109.05
330.0	14.72	884.32	2021.3776	-1137.05
335.0	14.54	886.91	2052.00453	-1165.09
340.0	14.37	889.47	2082.63147	-1193.16
345.0	14.20	892.00	2113.2584	-1221.26
350.0	14.04	894.49	2143.88533	-1249.40
355.0	13.88	896.95	2174.51227	-1277.56
360.0	13.72	899.38	2205.1392	-1305.76
<b>Max Volume (V max):</b>				<b>319.55</b>
<b>Design Volume (V design) :</b>				<b>319.55</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
 Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

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**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 247.7801153 L/s/ha

**Area** : A3 - Building 0.8582 ha  
**Runoff Coefficient C (unfactored):** 0.9  
**C<sub>runoff</sub> factor:** -  
**Runoff Coefficient C :** 0.95  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.212644895 m<sup>3</sup>/s  
**Discharge Factor K :** 1

<b>Design Volume:</b>	<b>115.04 m<sup>3</sup></b>
-----------------------	-----------------------------

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816

Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
 PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
 PEO No.: 100067842

Date: July 23, 2021

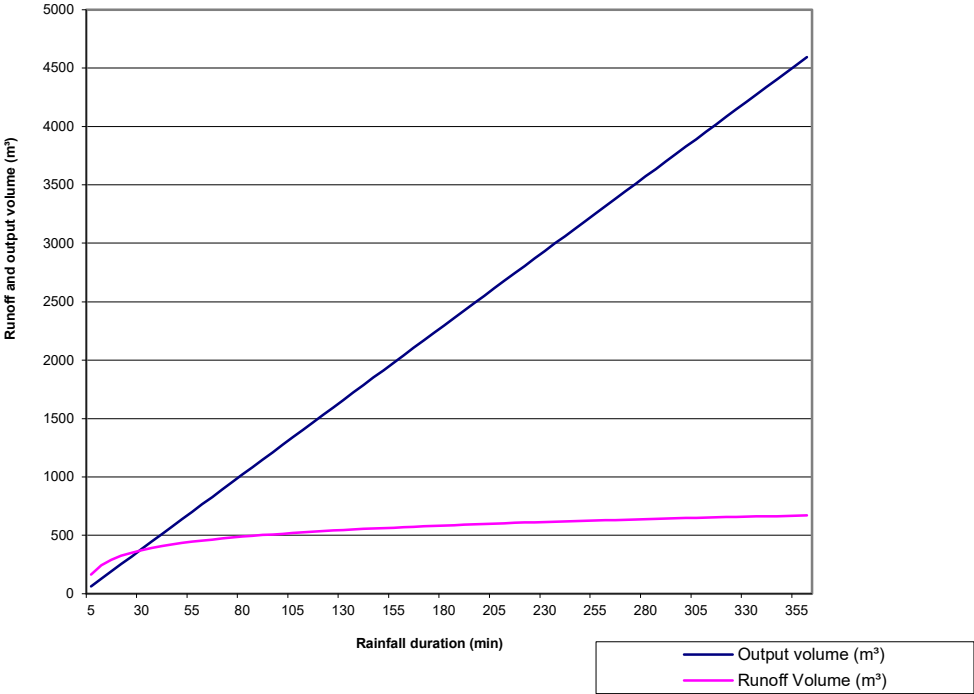
<b>Rainfall Duration (min) <i>T</i> (1)</b>	<b>Rainfall Intensity (mm/h) <i>I</i> (2)</b>	<b>Runoff Volume (m³) <i>CIA T</i> (4)</b>	<b>Output Volume (m³) <i>kQT</i> (5)</b>	<b>Retention Volume (m³) (4)-(5) (6)</b>
5.0	242.70	164.90	63.7934685	101.10
10.0	178.56	242.63	127.586937	115.04
15.0	142.89	291.25	191.380405	99.87
20.0	119.95	325.98	255.173874	70.81
25.0	103.85	352.77	318.967342	33.81
30.0	91.87	374.50	382.760811	-8.26
35.0	82.58	392.73	446.554279	-53.82
40.0	75.15	408.43	510.347748	-101.91
45.0	69.05	422.22	574.141216	-151.92
50.0	63.95	434.51	637.934685	-203.43
55.0	59.62	445.60	701.728153	-256.13
60.0	55.89	455.70	765.521622	-309.82
65.0	52.65	464.99	829.31509	-364.32
70.0	49.79	473.58	893.108559	-419.52
75.0	47.26	481.59	956.902027	-475.32
80.0	44.99	489.08	1020.6955	-531.62
85.0	42.95	496.12	1084.48896	-588.37
90.0	41.11	502.76	1148.28243	-645.52
95.0	39.43	509.05	1212.0759	-703.02
100.0	37.90	515.03	1275.86937	-760.84
105.0	36.50	520.73	1339.66284	-818.93
110.0	35.20	526.17	1403.45631	-877.29
115.0	34.01	531.38	1467.24978	-935.87
120.0	32.89	536.38	1531.04324	-994.67
125.0	31.86	541.18	1594.83671	-1053.66
130.0	30.90	545.80	1658.63018	-1112.83
135.0	30.00	550.26	1722.42365	-1172.16
140.0	29.15	554.57	1786.21712	-1231.65
145.0	28.36	558.74	1850.01059	-1291.27
150.0	27.61	562.77	1913.80405	-1351.04
155.0	26.91	566.68	1977.59752	-1410.92
160.0	26.24	570.47	2041.39099	-1470.92
165.0	25.61	574.16	2105.18446	-1531.03
170.0	25.01	577.74	2168.97793	-1591.24
175.0	24.44	581.23	2232.7714	-1651.54
180.0	23.90	584.63	2296.56487	-1711.94
185.0	23.39	587.94	2360.35833	-1772.42
190.0	22.90	591.17	2424.1518	-1832.98
195.0	22.43	594.32	2487.94527	-1893.62
200.0	21.98	597.40	2551.73874	-1954.34
205.0	21.55	600.41	2615.53221	-2015.12
210.0	21.14	603.36	2679.32568	-2075.97
215.0	20.75	606.24	2743.11914	-2136.88
220.0	20.37	609.07	2806.91261	-2197.85
225.0	20.01	611.83	2870.70608	-2258.87
230.0	19.66	614.54	2934.49955	-2319.96
235.0	19.33	617.20	2998.29302	-2381.09
240.0	19.01	619.81	3062.08649	-2442.28

245.0	18.69	622.37	3125.87996	-2503.51
250.0	18.39	624.88	3189.67342	-2564.79
255.0	18.11	627.35	3253.46689	-2626.12
260.0	17.83	629.77	3317.26036	-2687.49
265.0	17.56	632.16	3381.05383	-2748.90
270.0	17.29	634.50	3444.8473	-2810.35
275.0	17.04	636.81	3508.64077	-2871.83
280.0	16.80	639.08	3572.43424	-2933.36
285.0	16.56	641.31	3636.2277	-2994.92
290.0	16.33	643.51	3700.02117	-3056.51
295.0	16.11	645.67	3763.81464	-3118.14
300.0	15.89	647.81	3827.60811	-3179.80
305.0	15.68	649.91	3891.40158	-3241.49
310.0	15.48	651.98	3955.19505	-3303.21
315.0	15.28	654.02	4018.98851	-3364.96
320.0	15.09	656.04	4082.78198	-3426.74
325.0	14.90	658.02	4146.57545	-3488.55
330.0	14.72	659.98	4210.36892	-3550.38
335.0	14.54	661.92	4274.16239	-3612.24
340.0	14.37	663.83	4337.95586	-3674.13
345.0	14.20	665.71	4401.74933	-3736.04
350.0	14.04	667.57	4465.54279	-3797.97
355.0	13.88	669.41	4529.33626	-3859.93
360.0	13.72	671.22	4593.12973	-3921.91
<b>Max Volume (V max):</b>				115.04
<b>Design Volume (V design) :</b>				<b>115.04</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

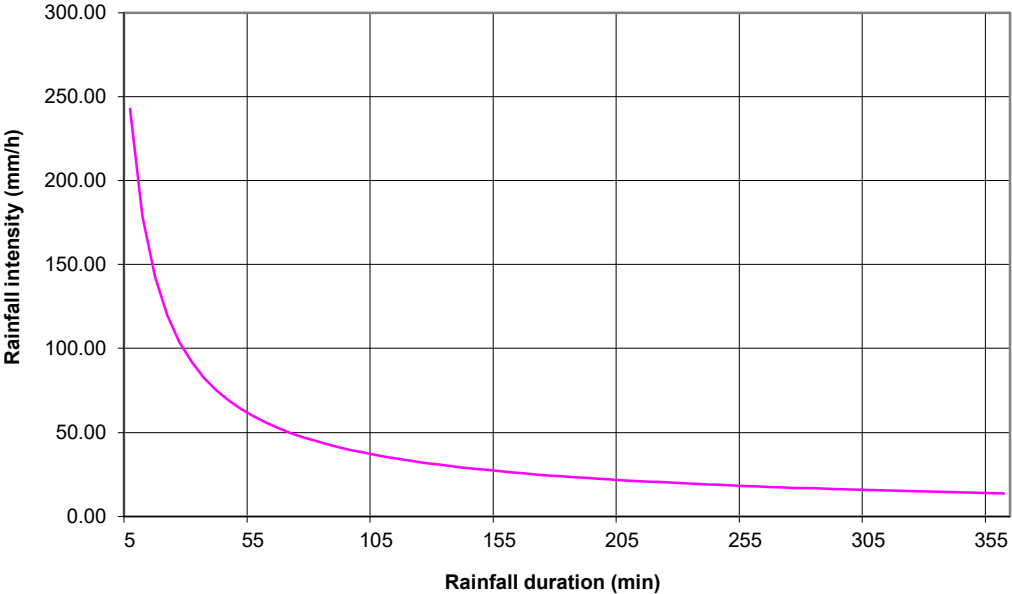
Sub-bassin **A3**

Storage volume sizing with the rational method



Rainfall I-D-F curve

100 yea







## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210723\_Storm Water Management - Storage and Drawdown\_half  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 84.10757773 L/s/ha

**Area** : A4 0.1486 ha  
**Runoff Coefficient C (unfactored)** 0.57  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.7125  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.012498386 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 27.34 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 23, 2021

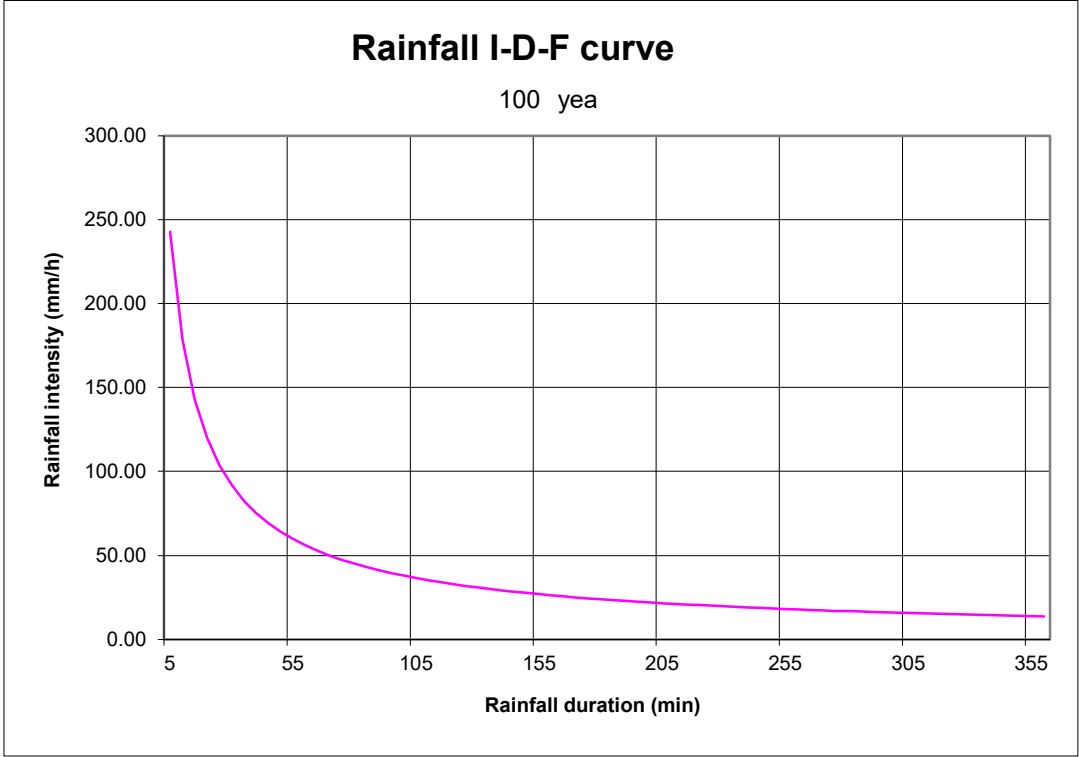
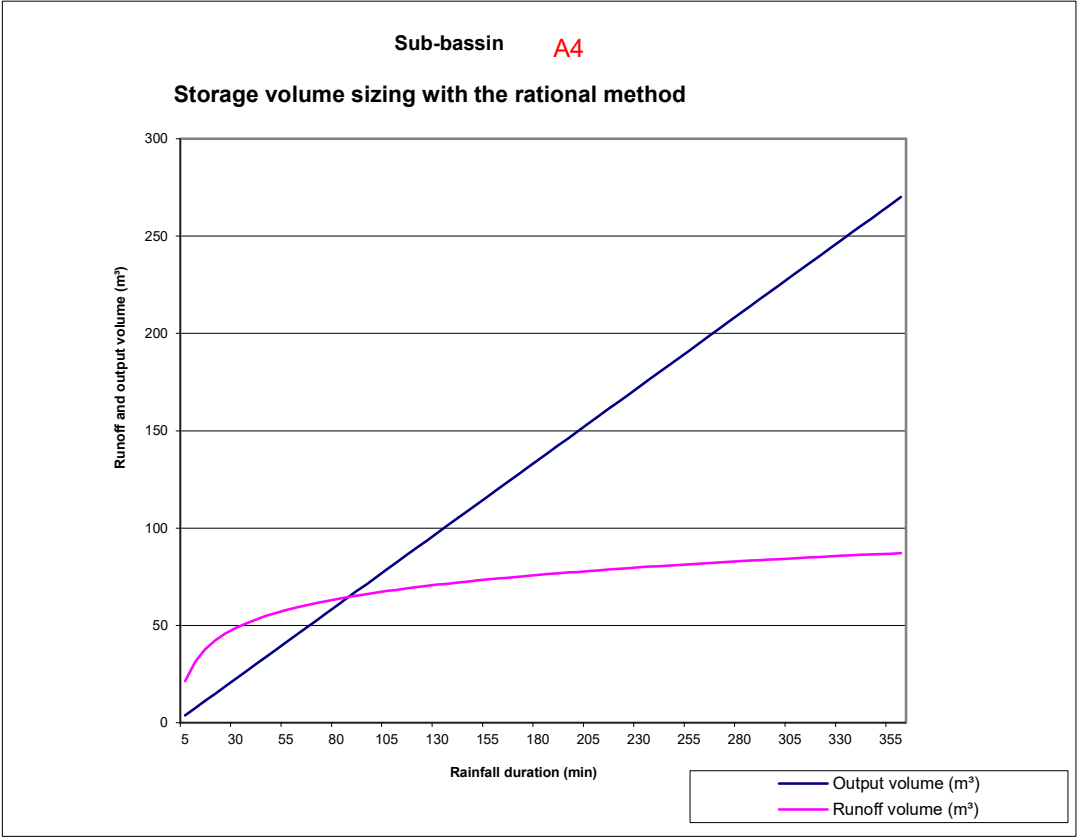
Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 23, 2021

<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIAT</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	21.41	3.74951582	17.66
10.0	178.56	31.51	7.49903163	24.01
15.0	142.89	37.82	11.2485474	26.57
20.0	119.95	42.33	14.9980633	27.34
25.0	103.85	45.81	18.7475791	27.07
30.0	91.87	48.63	22.4970949	26.14
35.0	82.58	51.00	26.2466107	24.76
40.0	75.15	53.04	29.9961265	23.05
45.0	69.05	54.83	33.7456423	21.09
50.0	63.95	56.43	37.4951582	18.93
55.0	59.62	57.87	41.244674	16.62
60.0	55.89	59.18	44.9941898	14.19
65.0	52.65	60.39	48.7437056	11.64
70.0	49.79	61.50	52.4932214	9.01
75.0	47.26	62.54	56.2427372	6.30
80.0	44.99	63.51	59.992253	3.52
85.0	42.95	64.43	63.7417689	0.69
90.0	41.11	65.29	67.4912847	-2.20
95.0	39.43	66.11	71.2408005	-5.13
100.0	37.90	66.88	74.9903163	-8.11
105.0	36.50	67.62	78.7398321	-11.12
110.0	35.20	68.33	82.4893479	-14.16
115.0	34.01	69.01	86.2388637	-17.23
120.0	32.89	69.66	89.9883796	-20.33
125.0	31.86	70.28	93.7378954	-23.46
130.0	30.90	70.88	97.4874112	-26.61
135.0	30.00	71.46	101.236927	-29.78
140.0	29.15	72.02	104.986443	-32.97
145.0	28.36	72.56	108.735959	-36.18
150.0	27.61	73.08	112.485474	-39.40
155.0	26.91	73.59	116.23499	-42.64
160.0	26.24	74.08	119.984506	-45.90
165.0	25.61	74.56	123.734022	-49.17
170.0	25.01	75.03	127.483538	-52.46
175.0	24.44	75.48	131.233054	-55.75
180.0	23.90	75.92	134.982569	-59.06
185.0	23.39	76.35	138.732085	-62.38
190.0	22.90	76.77	142.481601	-65.71
195.0	22.43	77.18	146.231117	-69.05
200.0	21.98	77.58	149.980633	-72.40
205.0	21.55	77.97	153.730148	-75.76
210.0	21.14	78.36	157.479664	-79.12
215.0	20.75	78.73	161.22918	-82.50
220.0	20.37	79.10	164.978696	-85.88
225.0	20.01	79.46	168.728212	-89.27
230.0	19.66	79.81	172.477727	-92.67
235.0	19.33	80.15	176.227243	-96.07
240.0	19.01	80.49	179.976759	-99.49

245.0	18.69	80.82	183.726275	-102.90
250.0	18.39	81.15	187.475791	-106.33
255.0	18.11	81.47	191.225307	-109.75
260.0	17.83	81.79	194.974822	-113.19
265.0	17.56	82.10	198.724338	-116.63
270.0	17.29	82.40	202.473854	-120.07
275.0	17.04	82.70	206.22337	-123.52
280.0	16.80	82.99	209.972886	-126.98
285.0	16.56	83.28	213.722401	-130.44
290.0	16.33	83.57	217.471917	-133.90
295.0	16.11	83.85	221.221433	-137.37
300.0	15.89	84.13	224.970949	-140.84
305.0	15.68	84.40	228.720465	-144.32
310.0	15.48	84.67	232.469981	-147.80
315.0	15.28	84.93	236.219496	-151.28
320.0	15.09	85.20	239.969012	-154.77
325.0	14.90	85.45	243.718528	-158.26
330.0	14.72	85.71	247.468044	-161.76
335.0	14.54	85.96	251.21756	-165.26
340.0	14.37	86.21	254.967075	-168.76
345.0	14.20	86.45	258.716591	-172.26
350.0	14.04	86.69	262.466107	-175.77
355.0	13.88	86.93	266.215623	-179.28
360.0	13.72	87.17	269.965139	-182.80
<b>Max Volume (V max):</b>				<b>27.34</b>
<b>Design Volume (V design) :</b>				<b>27.34</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210723\_Storm Water Management - Storage and Drawdown\_half  
**Location:** - - - - -

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 84.10757773 L/s/ha

**Area** : A5 0.1069 ha  
**Runoff Coefficient C (unfactored** 0.9  
**C\_runoff factor:** -  
**Runoff Coefficient C :** 0.95  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.0089911 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 30.46 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

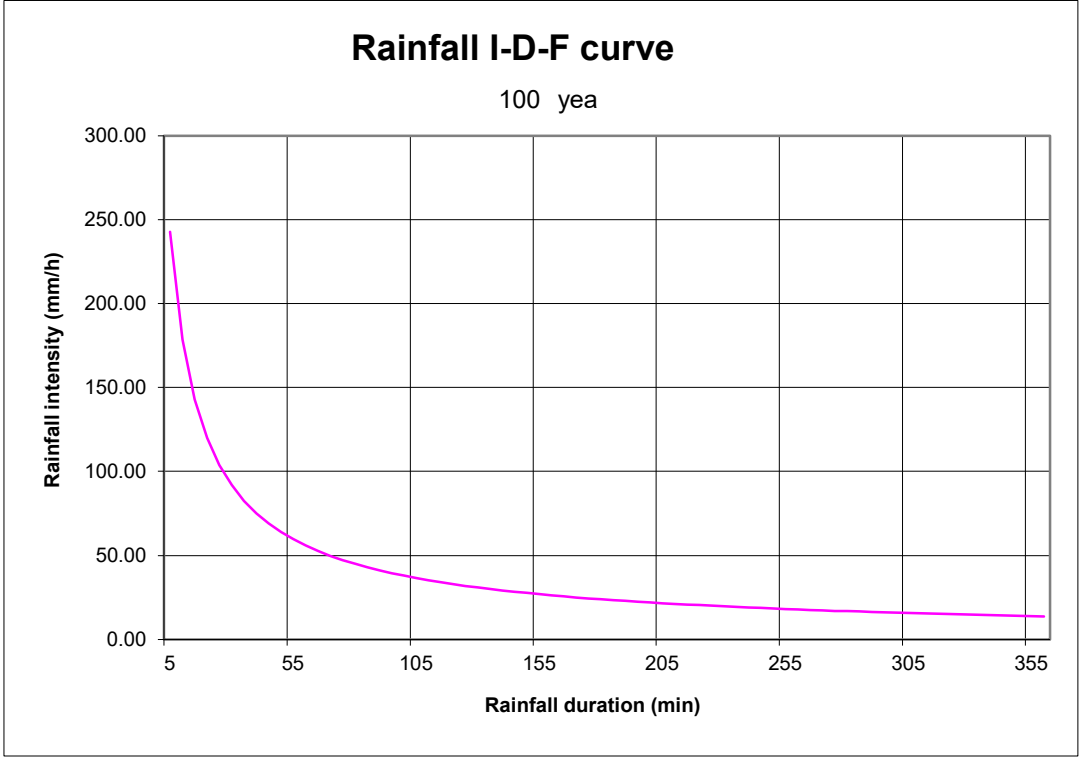
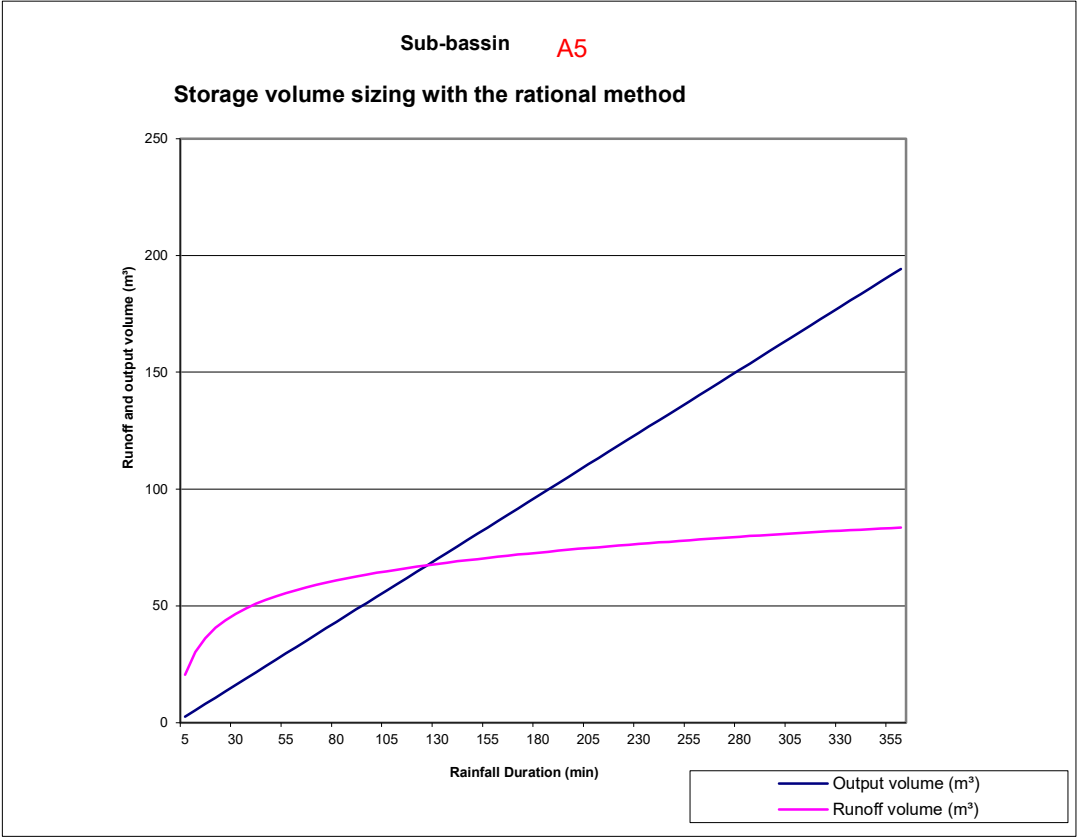
Date: July 23, 2021

<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIAT</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	20.54	2.69733002	17.84
10.0	178.56	30.22	5.39466004	24.83
15.0	142.89	36.28	8.09199005	28.19
20.0	119.95	40.61	10.7893201	29.82
25.0	103.85	43.94	13.4866501	30.46
30.0	91.87	46.65	16.1839801	30.46
35.0	82.58	48.92	18.8813101	30.04
40.0	75.15	50.88	21.5786401	29.30
45.0	69.05	52.59	24.2759702	28.32
50.0	63.95	54.12	26.9733002	27.15
55.0	59.62	55.51	29.6706302	25.83
60.0	55.89	56.76	32.3679602	24.40
65.0	52.65	57.92	35.0652902	22.86
70.0	49.79	58.99	37.7626202	21.23
75.0	47.26	59.99	40.4599503	19.53
80.0	44.99	60.92	43.1572803	17.76
85.0	42.95	61.80	45.8546103	15.94
90.0	41.11	62.63	48.5519403	14.07
95.0	39.43	63.41	51.2492703	12.16
100.0	37.90	64.15	53.9466004	10.21
105.0	36.50	64.86	56.6439304	8.22
110.0	35.20	65.54	59.3412604	6.20
115.0	34.01	66.19	62.0385904	4.15
120.0	32.89	66.81	64.7359204	2.08
125.0	31.86	67.41	67.4332504	-0.02
130.0	30.90	67.99	70.1305805	-2.14
135.0	30.00	68.54	72.8279105	-4.29
140.0	29.15	69.08	75.5252405	-6.45
145.0	28.36	69.60	78.2225705	-8.62
150.0	27.61	70.10	80.9199005	-10.82
155.0	26.91	70.59	83.6172306	-13.03
160.0	26.24	71.06	86.3145606	-15.25
165.0	25.61	71.52	89.0118906	-17.49
170.0	25.01	71.97	91.7092206	-19.74
175.0	24.44	72.40	94.4065506	-22.01
180.0	23.90	72.82	97.1038806	-24.28
185.0	23.39	73.24	99.8012107	-26.57
190.0	22.90	73.64	102.498541	-28.86
195.0	22.43	74.03	105.195871	-31.17
200.0	21.98	74.41	107.893201	-33.48
205.0	21.55	74.79	110.590531	-35.80
210.0	21.14	75.16	113.287861	-38.13
215.0	20.75	75.52	115.985191	-40.47
220.0	20.37	75.87	118.682521	-42.82
225.0	20.01	76.21	121.379851	-45.17
230.0	19.66	76.55	124.077181	-47.53
235.0	19.33	76.88	126.774511	-49.89
240.0	19.01	77.21	129.471841	-52.27

245.0	18.69	77.52	132.169171	-54.65
250.0	18.39	77.84	134.866501	-57.03
255.0	18.11	78.14	137.563831	-59.42
260.0	17.83	78.45	140.261161	-61.81
265.0	17.56	78.74	142.958491	-64.22
270.0	17.29	79.04	145.655821	-66.62
275.0	17.04	79.32	148.353151	-69.03
280.0	16.80	79.61	151.050481	-71.45
285.0	16.56	79.88	153.747811	-73.86
290.0	16.33	80.16	156.445141	-76.29
295.0	16.11	80.43	159.142471	-78.72
300.0	15.89	80.69	161.839801	-81.15
305.0	15.68	80.95	164.537131	-83.58
310.0	15.48	81.21	167.234461	-86.02
315.0	15.28	81.47	169.931791	-88.46
320.0	15.09	81.72	172.629121	-90.91
325.0	14.90	81.97	175.326451	-93.36
330.0	14.72	82.21	178.023781	-95.81
335.0	14.54	82.45	180.721111	-98.27
340.0	14.37	82.69	183.418441	-100.73
345.0	14.20	82.92	186.115771	-103.19
350.0	14.04	83.15	188.813101	-105.66
355.0	13.88	83.38	191.510431	-108.13
360.0	13.72	83.61	194.207761	-110.60
<b>Max Volume (V max):</b>				30.46
<b>Design Volume (V design) :</b>				<b>30.46</b>



Fastfrate Warehouse Development  
Industrial/Commercial Development





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210723\_Storm Water Management - Storage and Drawdown\_half  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 84.10757773 L/s/ha

**Area** : A6 0.486 ha  
**Runoff Coefficient C (unfactored)** 0.34  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.425  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.040876283 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 37.00 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: July 23, 2021

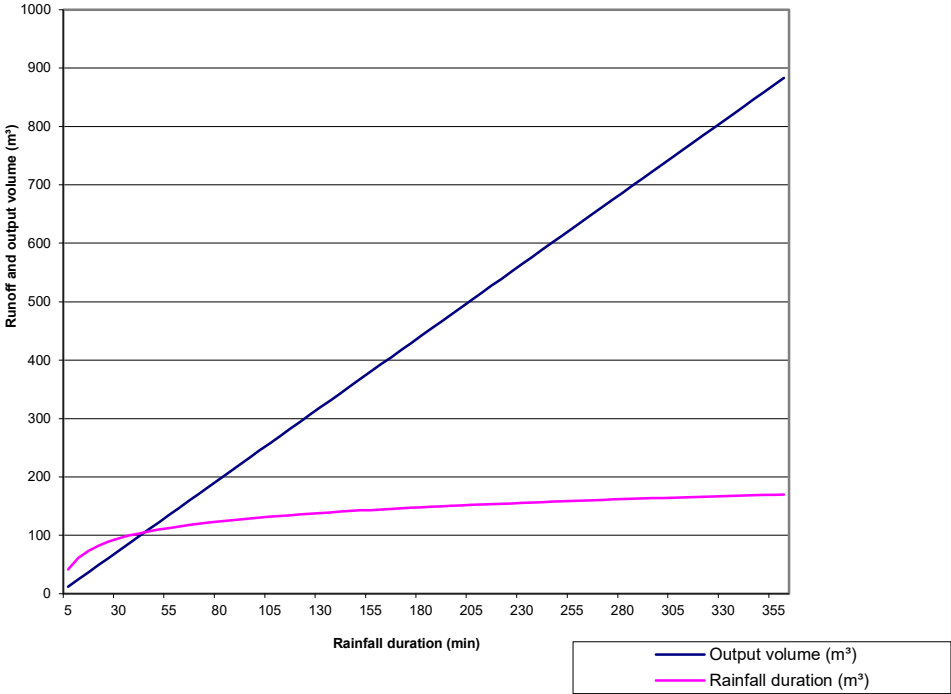
<b>Rainfall Duration (min) <math>T</math> (1)</b>	<b>Rainfall Intensity (mm/h) <math>I</math> (2)</b>	<b>Runoff Volume (m<sup>3</sup>) <math>CIAT</math> (4)</b>	<b>Output Volume (m<sup>3</sup>) <math>kQT</math> (5)</b>	<b>Retention Volume (m<sup>3</sup>) (4)-(5) (6)</b>
5.0	242.70	41.78	12.2628848	29.51
10.0	178.56	61.47	24.5257697	36.94
15.0	142.89	73.79	36.7886545	37.00
20.0	119.95	82.59	49.0515393	33.53
25.0	103.85	89.37	61.3144242	28.06
30.0	91.87	94.88	73.577309	21.30
35.0	82.58	99.50	85.8401938	13.66
40.0	75.15	103.48	98.1030787	5.37
45.0	69.05	106.97	110.365963	-3.40
50.0	63.95	110.08	122.628848	-12.55
55.0	59.62	112.89	134.891733	-22.00
60.0	55.89	115.45	147.154618	-31.70
65.0	52.65	117.80	159.417503	-41.61
70.0	49.79	119.98	171.680388	-51.70
75.0	47.26	122.01	183.943272	-61.94
80.0	44.99	123.91	196.206157	-72.30
85.0	42.95	125.69	208.469042	-82.78
90.0	41.11	127.37	220.731927	-93.36
95.0	39.43	128.97	232.994812	-104.03
100.0	37.90	130.48	245.257697	-114.78
105.0	36.50	131.92	257.520581	-125.60
110.0	35.20	133.30	269.783466	-136.48
115.0	34.01	134.62	282.046351	-147.42
120.0	32.89	135.89	294.309236	-158.42
125.0	31.86	137.11	306.572121	-169.47
130.0	30.90	138.28	318.835006	-180.56
135.0	30.00	139.41	331.09789	-191.69
140.0	29.15	140.50	343.360775	-202.86
145.0	28.36	141.55	355.62366	-214.07
150.0	27.61	142.57	367.886545	-225.31
155.0	26.91	143.57	380.14943	-236.58
160.0	26.24	144.53	392.412315	-247.89
165.0	25.61	145.46	404.675199	-259.22
170.0	25.01	146.37	416.938084	-270.57
175.0	24.44	147.25	429.200969	-281.95
180.0	23.90	148.11	441.463854	-293.35
185.0	23.39	148.95	453.726739	-304.78
190.0	22.90	149.77	465.989624	-316.22
195.0	22.43	150.57	478.252508	-327.68
200.0	21.98	151.35	490.515393	-339.17
205.0	21.55	152.11	502.778278	-350.67
210.0	21.14	152.86	515.041163	-362.18
215.0	20.75	153.59	527.304048	-373.72
220.0	20.37	154.30	539.566933	-385.26
225.0	20.01	155.00	551.829817	-396.83
230.0	19.66	155.69	564.092702	-408.40
235.0	19.33	156.36	576.355587	-419.99
240.0	19.01	157.03	588.618472	-431.59

245.0	18.69	157.67	600.881357	-443.21
250.0	18.39	158.31	613.144242	-454.83
255.0	18.11	158.94	625.407126	-466.47
260.0	17.83	159.55	637.670011	-478.12
265.0	17.56	160.15	649.932896	-489.78
270.0	17.29	160.75	662.195781	-501.45
275.0	17.04	161.33	674.458666	-513.13
280.0	16.80	161.91	686.721551	-524.81
285.0	16.56	162.47	698.984435	-536.51
290.0	16.33	163.03	711.24732	-548.22
295.0	16.11	163.58	723.510205	-559.93
300.0	15.89	164.12	735.77309	-571.65
305.0	15.68	164.65	748.035975	-583.38
310.0	15.48	165.18	760.29886	-595.12
315.0	15.28	165.69	772.561744	-606.87
320.0	15.09	166.20	784.824629	-618.62
325.0	14.90	166.71	797.087514	-630.38
330.0	14.72	167.20	809.350399	-642.15
335.0	14.54	167.69	821.613284	-653.92
340.0	14.37	168.18	833.876169	-665.70
345.0	14.20	168.65	846.139053	-677.48
350.0	14.04	169.13	858.401938	-689.28
355.0	13.88	169.59	870.664823	-701.07
360.0	13.72	170.05	882.927708	-712.88
<b>Max Volume (V max):</b>				<b>37.00</b>
<b>Design Volume (V design) :</b>				<b>37.00</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

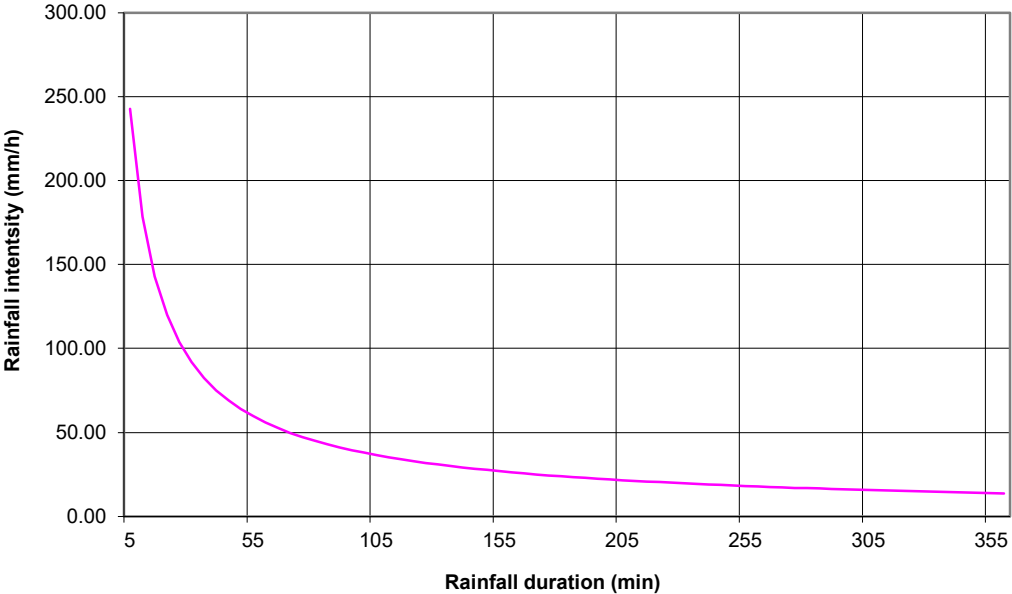
Sub-bassin **A6**

Storage volume sizing with the rational method



Rainfall I-D-F curve

100 ya





## STORAGE VOLUME CALCULATIONS

**Project:** Fastrate Warehouse Development  
 Industrial/Commercial Development  
**Project #:** A001083 (360)  
**Station** OTTAWA SEWER DESIGN GUIDELINES  
**Date:** 2021-09-21 14:40

**File** Z:\Cima-C10\Ott\_Projects\A\A001000-A001499\A001083\_Fastrate Warehouse Development\300\360\_Civil\01-SWM\210921\_Fully Translated Spreadsheets\210723\_Storm Water Management - Storage and Drawdown\_half RR.xlsx]A7  
**Location:**

**Description:** Storage volume calculations with the rational method

**Specified Release Rate:** 84.10757773 L/s/ha

**Area** : A7 0.1497 ha  
**Runoff Coefficient C (unfactored)** 0.52  
**C\_runoff factor:** 1.25  
**Runoff Coefficient C :** 0.65  
**Rainfall Event :** 100 year  
**Discharge Flow Q :** 0.012590904 m<sup>3</sup>/s  
**Discharge Factor K :** 1

**Design Volume:** 23.80 m<sup>3</sup>

Rainfall Pluviometry Coefficients	2 year		5 year		10 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	732.951	732.951	998.071	998.071	1174.184	1174.184
B	6.199	6.199	6.053	6.053	6.014	6.014
C	0.81	0.81	0.814	0.814	0.816	0.816
Rainfall Pluviometry Coefficients	25 year		50 year		100 year	
	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.	30 min. or less	Over 30 min.
A	1402.884	1402.884	1569.58	1569.58	1735.688	1735.688
B	6.018	6.018	6.014	6.014	6.014	6.014
C	0.819	0.819	0.82	0.82	0.82	0.82

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
 PEO No.: 100530467

Date: July 23, 2021

Verified by: Christian Lavoie-Lebel, P.Eng.  
 PEO No.: 100067842

Date: July 23, 2021

<b>Rainfall Duration</b> (min) <i>T</i> (1)	<b>Rainfall Intensity</b> (mm/h) <i>I</i> (2)	<b>Runoff Volume</b> (m³) <i>CIA T</i> (4)	<b>Output Volume</b> (m³) <i>kQT</i> (5)	<b>Retention Volume</b> (m³) (4)-(5) (6)
5.0	242.70	19.68	3.77727132	15.90
10.0	178.56	28.96	7.55454263	21.40
15.0	142.89	34.76	11.3318139	23.43
20.0	119.95	38.91	15.1090853	23.80
25.0	103.85	42.10	18.8863566	23.22
30.0	91.87	44.70	22.6636279	22.03
35.0	82.58	46.87	26.4408992	20.43
40.0	75.15	48.75	30.2181705	18.53
45.0	69.05	50.39	33.9954418	16.40
50.0	63.95	51.86	37.7727132	14.09
55.0	59.62	53.18	41.5499845	11.63
60.0	55.89	54.39	45.3272558	9.06
65.0	52.65	55.50	49.1045271	6.39
70.0	49.79	56.52	52.8817984	3.64
75.0	47.26	57.48	56.6590697	0.82
80.0	44.99	58.37	60.4363411	-2.07
85.0	42.95	59.21	64.2136124	-5.00
90.0	41.11	60.00	67.9908837	-7.99
95.0	39.43	60.76	71.768155	-11.01
100.0	37.90	61.47	75.5454263	-14.08
105.0	36.50	62.15	79.3226976	-17.17
110.0	35.20	62.80	83.0999689	-20.30
115.0	34.01	63.42	86.8772403	-23.46
120.0	32.89	64.02	90.6545116	-26.64
125.0	31.86	64.59	94.4317829	-29.84
130.0	30.90	65.14	98.2090542	-33.07
135.0	30.00	65.67	101.986326	-36.31
140.0	29.15	66.19	105.763597	-39.58
145.0	28.36	66.69	109.540868	-42.86
150.0	27.61	67.17	113.318139	-46.15
155.0	26.91	67.63	117.095411	-49.46
160.0	26.24	68.09	120.872682	-52.79
165.0	25.61	68.53	124.649953	-56.12
170.0	25.01	68.95	128.427225	-59.47
175.0	24.44	69.37	132.204496	-62.83
180.0	23.90	69.78	135.981767	-66.21
185.0	23.39	70.17	139.759039	-69.59
190.0	22.90	70.56	143.53631	-72.98
195.0	22.43	70.93	147.313581	-76.38
200.0	21.98	71.30	151.090853	-79.79
205.0	21.55	71.66	154.868124	-83.21
210.0	21.14	72.01	158.645395	-86.63
215.0	20.75	72.36	162.422667	-90.07
220.0	20.37	72.69	166.199938	-93.51
225.0	20.01	73.02	169.977209	-96.96
230.0	19.66	73.35	173.754481	-100.41
235.0	19.33	73.66	177.531752	-103.87

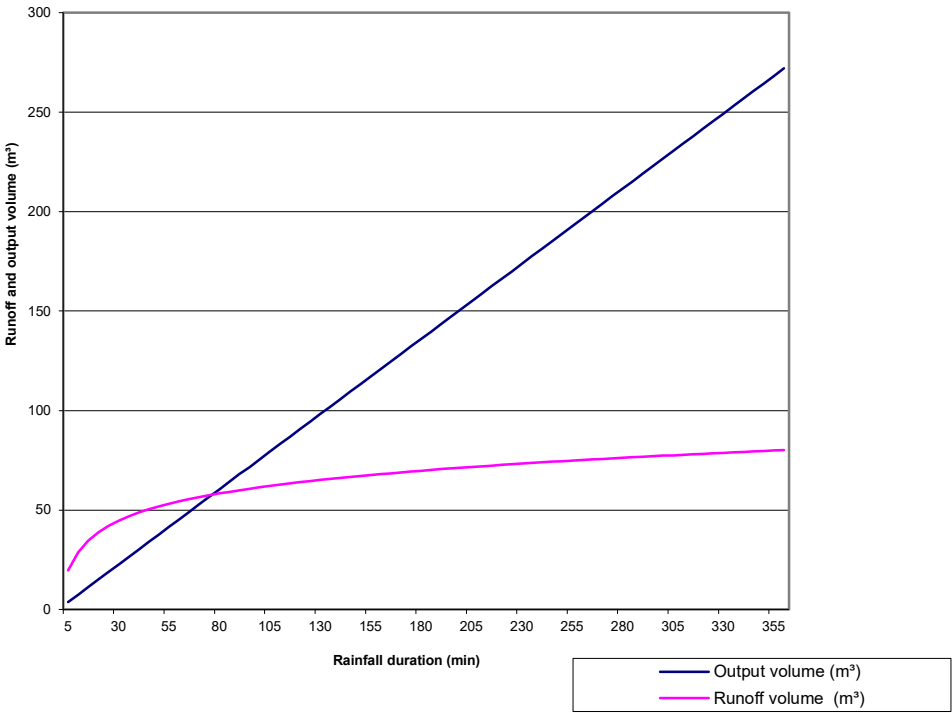


240.0	19.01	73.97	181.309023	-107.33
245.0	18.69	74.28	185.086294	-110.81
250.0	18.39	74.58	188.863566	-114.28
255.0	18.11	74.87	192.640837	-117.77
260.0	17.83	75.16	196.418108	-121.25
265.0	17.56	75.45	200.19538	-124.75
270.0	17.29	75.73	203.972651	-128.24
275.0	17.04	76.00	207.749922	-131.75
280.0	16.80	76.27	211.527194	-135.25
285.0	16.56	76.54	215.304465	-138.76
290.0	16.33	76.80	219.081736	-142.28
295.0	16.11	77.06	222.859008	-145.80
300.0	15.89	77.32	226.636279	-149.32
305.0	15.68	77.57	230.41355	-152.85
310.0	15.48	77.81	234.190822	-156.38
315.0	15.28	78.06	237.968093	-159.91
320.0	15.09	78.30	241.745364	-163.45
325.0	14.90	78.54	245.522636	-166.99
330.0	14.72	78.77	249.299907	-170.53
335.0	14.54	79.00	253.077178	-174.08
340.0	14.37	79.23	256.854449	-177.63
345.0	14.20	79.45	260.631721	-181.18
350.0	14.04	79.67	264.408992	-184.73
355.0	13.88	79.89	268.186263	-188.29
360.0	13.72	80.11	271.963535	-191.85
<b>Max Volume (V max):</b>				23.80
<b>Design Volume (V design) :</b>				<b>23.80</b>

Fastfrate Warehouse Development  
Industrial/Commercial Development

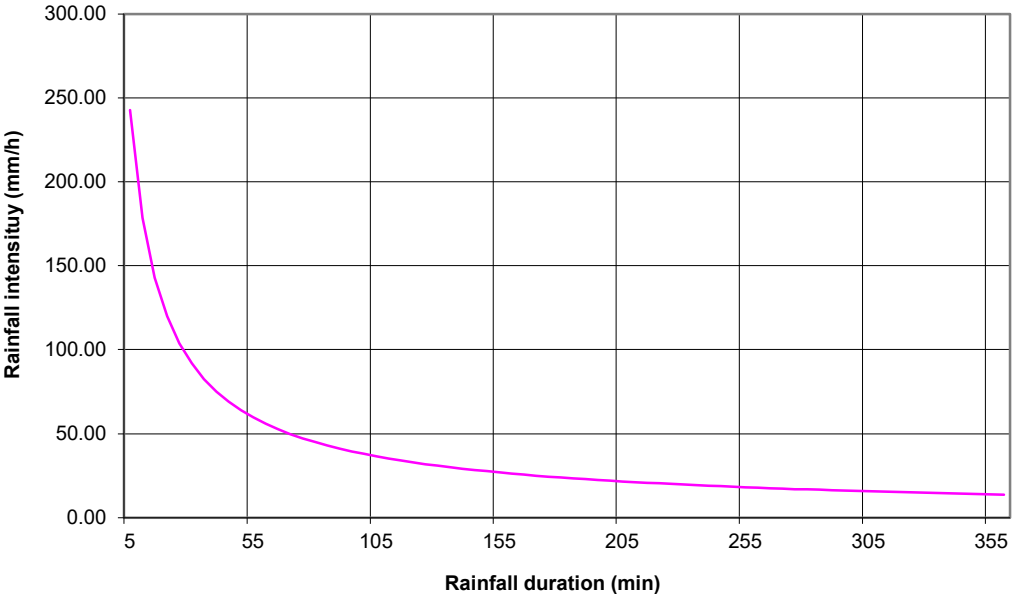
Sub-bassin **A7**

Storage volume sizing with the rational method



Courbe I-D-F de la pluie

100 yea

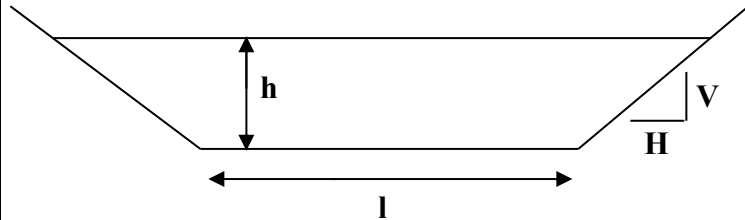


# FASTFRATE

A001083 (360)

## CHANNEL CHECK AT DITCH ON SOMME STREET (100-YEAR)

Bed Length (l)	m	0.000		
Side Slopes (H:V)	H/V	3.0000	1.0000	
Slope (S)	m/m	0.0050	%	0.50
Roughness Coefficient	n	0.0300		
Flow (Q)	m <sup>3</sup> /s	3.857	l/s	3,857
Velocity (V)	m/s	1.395	cm/s	140
Hydraulic Radius (R <sub>h</sub> )	m	0.455		
Wetted Area	m <sup>2</sup>	2.765		
Wetted Perimeter	m	6.072		
Height of water (h)	m	0.960		



### Notes:

The ditch on Somme street at which our site is connecting will have a headwater height of 0.96m during the 100-year storm event. The bottom of the ditch at that location is 89.110 which means the hydraulic grade line within the ditch will be at 90.07.

Prepared by: Julien Sauvé, P.Eng  
100200100

Date: July 20, 2021

Verified by: Julien Sauvé, P.Eng  
PEO No.: 100200100

Date: July 20, 2021





## Calculation sheet

Location: Outfall  
Road: n.a.  
Initial water level (m):  
Initial velocity (m/s):  
Manning number:

89.11
0.3
0.013

Initial EGLs (m):  
Initial HGL (m):

### Graphic

[illegible]

**Comment:**









PROJECT NAME: Fastrate (Ottawa) Warehouse Development  
CIMA+ PROJECT NUMBER: A001083  
CLIENT: Fastrate (Ottawa) Holdings Inc.  
PROJECT STATUS: 90 % Design (Site plan Approval)

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: August 9, 2021

#### Numerical Analysis; Orifice sizing

#### Extended Detention Control

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: August 9, 2021

#### Extended Detention Orifice

Control Type Circular Orifice plate  
Elevation Range (m) 89.3 to 89.5  
Base elevation (m)  
Initial head over Orifice  
Orifice Diameter (mm)  
  
No. of orifices  
Gravitational Acceleration, g (m/s<sup>2</sup>)  
Discharge Coefficient, C<sub>d</sub>

#### Weir Equation Comparison

	Values	Notes
89.3 Weir Elevation (m)	89.3	
0 Head over weir, H <sub>w</sub> (m)	0.20	
80 Weir Discharge Coeff., C <sub>w</sub>	0.61	
1 Weir Length, L <sub>w</sub> (m):	0.1	
9.81 Weir Flow, q <sub>w</sub> (m <sup>3</sup> /s) – Peak Flow	0.02 (2/3*C <sub>w</sub> *L <sub>w</sub> *sqrt(2*g)*H <sub>w</sub> <sup>(3/2)</sup> )	
0.63 Weir Flow, q <sub>w</sub> (L/s) – Peak Flow	16.11	

Water Elevation (m)	Head over Orifice, hf (m)	Head differential, dh (m)	Pond Area "A" (m <sup>2</sup> )	Orifice Area "a" (m <sup>2</sup> )	Qea="C"*sqrt(2*g*hf) (m <sup>3</sup> /s)	Time differential, dt (s)	Q=2/3*C <sub>w</sub> *L <sub>w</sub> *sqrt(2*g)*h <sub>w</sub> <sup>(3/2)</sup> (m <sup>3</sup> /s)	Time differential, dt (s)
89.30	0.00	0		846.29	5.03E-03	1.00E-06	0	0.00E+00
89.31	0.01	0.01	0.01	849.30	5.03E-03	1.40E-03	6055	1.80E-04
89.32	0.02	0.02	0.01	852.32	5.03E-03	1.98E-03	4297	5.09E-04
89.33	0.03	0.03	0.01	855.34	5.03E-03	2.43E-03	3521	9.36E-04
89.34	0.04	0.04	0.01	858.37	5.03E-03	2.81E-03	3060	1.44E-03
89.35	0.05	0.05	0.01	861.40	5.03E-03	3.14E-03	2746	2.01E-03
89.36	0.06	0.06	0.01	864.44	5.03E-03	3.44E-03	2516	2.65E-03
89.37	0.07	0.07	0.01	867.48	5.03E-03	3.71E-03	2338	3.34E-03
89.38	0.08	0.08	0.01	870.53	5.03E-03	3.97E-03	2194	4.08E-03
89.39	0.09	0.09	0.01	873.59	5.03E-03	4.21E-03	2076	4.86E-03
89.40	0.10	0.10	0.01	876.65	5.03E-03	4.44E-03	1976	5.70E-03
89.41	0.11	0.11	0.01	879.71	5.03E-03	4.65E-03	1891	6.57E-03
89.42	0.12	0.12	0.01	882.78	5.03E-03	4.86E-03	1817	7.49E-03
89.43	0.13	0.13	0.01	885.86	5.03E-03	5.06E-03	1752	8.44E-03
89.44	0.14	0.14	0.01	888.94	5.03E-03	5.25E-03	1694	9.44E-03
89.45	0.15	0.15	0.01	892.03	5.03E-03	5.43E-03	1642	1.05E-02
89.46	0.16	0.16	0.01	895.12	5.03E-03	5.61E-03	1595	1.15E-02
89.47	0.17	0.17	0.01	898.22	5.03E-03	5.78E-03	1553	1.26E-02
89.48	0.18	0.18	0.01	901.32	5.03E-03	5.95E-03	1515	1.38E-02
89.49	0.19	0.19	0.01	904.43	5.03E-03	6.11E-03	1479	1.49E-02
89.50	0.20	0.20	0.01	907.55	5.03E-03	6.27E-03	1447	1.61E-02

#### Numerical Results:

Parameter	Value	Units
Peak Flowrate (L/s)	6.27	L/s
Average Flowrate (L/s)	4.12	L/s
Water Quality Volume (m <sup>3</sup> )	175.65	m <sup>3</sup>
Drawdown Time (h)	13.1	h
90% Drawdown Time (h)	11.4	h

#### MOE Equation 4.10 Results:

Parameter	Value	Units
Area of Pond	878.2696766	m <sup>2</sup>
Orifice Discharge Coeff. C	0.63	unls.
Orifice Area, A <sub>o</sub>	5.03E-03	m <sup>2</sup>
g	9.81	m/s <sup>2</sup>
h1	0.2	m
h2	0.0	m
Drawdown Time, t	5.6E+04	s
Drawdown Time, t	15.6	h

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: August 9, 2021

Retention Control - Freeflow condition

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: August 9, 2021

Retention Control Orifice

Control Type	Rectangular Orifice			
Elevation Range (m)	89.5 - 89.85	Weir Equation Comparison	Values	Notes
Base elevation (m)		89.5 Weir Elevation (m)	89.5	
Initial head over Orifice		0 Head over weir, H_w (m)	0.60	
Orifice Depth (mm)	600	Weir Discharge Coeff., C_w	0.61	
Orifice Width (mm)	1040			
No. of orifices	1	Weir Length, L_w (m): 3x 780mm	1040	
Gravitational Acceleration, g (m/s <sup>2</sup> )	9.81	Weir Flow, q_w (m <sup>3</sup> /s)	870.66 (2/3*C_w*L_w*sqrt(2*g)*H_w^(3/2))	
Orifice Discharge Coeff., C_d	0.63	Weir Flow, q_w (L/s)	870659.40	

Water Elevation (m)	Head over Orifice, hf (m)	Head differential, dh (m)	Pond Area "A" (m2)	Orifice Area "a" (m2)	Q=ca*C_d*sqrt(2*g*hf) (m3/s)	Time differential, dt (s)
89.50	0.00	0	907.55	6.24E-01	1.00E+06	0.00
89.51	0.01	0.01	910.67	6.24E-01	1.74E-01	52.30
89.52	0.02	0.01	913.79	6.24E-01	2.46E-01	37.11
89.53	0.03	0.01	916.93	6.24E-01	3.02E-01	30.40
89.54	0.04	0.01	920.06	6.24E-01	3.48E-01	26.42
89.55	0.05	0.01	923.21	6.24E-01	3.89E-01	23.71
89.56	0.06	0.01	926.35	6.24E-01	4.27E-01	21.72
89.57	0.07	0.01	929.51	6.24E-01	4.61E-01	20.18
89.58	0.08	0.01	932.67	6.24E-01	4.93E-01	18.94
89.59	0.09	0.01	935.83	6.24E-01	5.22E-01	17.91
89.60	0.10	0.01	939.00	6.24E-01	5.51E-01	17.05
89.61	0.11	0.01	942.18	6.24E-01	5.78E-01	16.31
89.62	0.12	0.01	945.36	6.24E-01	6.03E-01	15.67
89.63	0.13	0.01	948.54	6.24E-01	6.28E-01	15.11
89.64	0.14	0.01	951.73	6.24E-01	6.52E-01	14.61
89.65	0.15	0.01	954.93	6.24E-01	6.74E-01	14.16
89.66	0.16	0.01	958.13	6.24E-01	6.97E-01	13.76
89.67	0.17	0.01	961.34	6.24E-01	7.18E-01	13.39
89.68	0.18	0.01	964.56	6.24E-01	7.39E-01	13.06
89.69	0.19	0.01	967.78	6.24E-01	7.59E-01	12.75
89.70	0.20	0.01	971.00	6.24E-01	7.79E-01	12.47
89.71	0.21	0.01	974.23	6.24E-01	7.98E-01	12.21
89.72	0.22	0.01	977.47	6.24E-01	8.17E-01	11.97
89.73	0.23	0.01	980.71	6.24E-01	8.35E-01	11.74
89.74	0.24	0.01	983.95	6.24E-01	8.53E-01	11.53
89.75	0.25	0.01	987.21	6.24E-01	8.71E-01	11.34
89.76	0.26	0.01	990.46	6.24E-01	8.88E-01	11.16
89.77	0.27	0.01	993.73	6.24E-01	9.05E-01	10.98
89.78	0.28	0.01	997.00	6.24E-01	9.21E-01	10.82
89.79	0.29	0.01	1000.27	6.24E-01	9.38E-01	10.67
89.80	0.30	0.01	1003.55	6.24E-01	9.54E-01	10.52
89.81	0.31	0.01	1006.84	6.24E-01	9.70E-01	10.38
89.82	0.32	0.01	1010.13	6.24E-01	9.85E-01	10.25
89.83	0.33	0.01	1013.42	6.24E-01	1.00E+00	10.13
89.84	0.34	0.01	1016.72	6.24E-01	1.02E+00	10.01
89.85	0.35	0.01	1020.03	6.24E-01	1.03E+00	9.90
89.86	0.36	0.01	1023.34	6.24E-01	1.04E+00	9.79
89.87	0.37	0.01	1026.66	6.24E-01	1.06E+00	9.69
89.88	0.38	0.01	1029.99	6.24E-01	1.07E+00	9.60
89.89	0.39	0.01	1033.32	6.24E-01	1.09E+00	9.50
89.90	0.40	0.01	1036.65	6.24E-01	1.10E+00	9.41
89.91	0.41	0.01	1039.99	6.24E-01	1.11E+00	9.33
89.92	0.42	0.01	1043.34	6.24E-01	1.13E+00	9.25
89.93	0.43	0.01	1046.69	6.24E-01	1.14E+00	9.17
89.94	0.44	0.01	1050.04	6.24E-01	1.16E+00	9.09
89.95	0.45	0.01	1053.41	6.24E-01	1.17E+00	9.02
89.96	0.46	0.01	1056.77	6.24E-01	1.18E+00	8.95
89.97	0.47	0.01	1060.15	6.24E-01	1.19E+00	8.88
89.98	0.48	0.01	1063.53	6.24E-01	1.21E+00	8.82
89.99	0.49	0.01	1066.91	6.24E-01	1.22E+00	8.75
90.00	0.50	0.01	1070.30	6.24E-01	1.23E+00	8.69
90.01	0.51	0.01	1073.70	6.24E-01	1.24E+00	8.63
90.02	0.52	0.01	1077.10	6.24E-01	1.26E+00	8.58
90.03	0.53	0.01	1080.50	6.24E-01	1.27E+00	8.52
90.04	0.54	0.01	1083.91	6.24E-01	1.28E+00	8.47
90.05	0.55	0.01	1087.33	6.24E-01	1.29E+00	8.42
90.06	0.56	0.01	1090.75	6.24E-01	1.30E+00	8.37
90.07	0.57	0.01	1094.18	6.24E-01	1.31E+00	8.32
90.08	0.58	0.01	1097.62	6.24E-01	1.33E+00	8.28
90.09	0.59	0.01	1101.05	6.24E-01	1.34E+00	8.23
90.10	0.60	0.01	1104.50	6.24E-01	1.35E+00	8.19

Numerical Results:

Average Flowrate - Quantity Control Orifice	894.9 L/s
Average Flowrate - Extended Detention Orifice	9.6 L/s
Total Average Flowrate	904.6 L/s
Allowable Flowrate	906.9 L/s

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: August 9, 2021

**Retention Control - Freeflow condition**

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: August 9, 2021

**Extended Detention Orifice**

Control Type: Circular Orifice plate  
Elevation Range (m): 89.5 - 89.85  
Base elevation (m): 89.5  
Initial head over Orifice: 0.2  
Orifice Diameter (mm): 80  
  
No. of orifices: 1  
Gravitational Acceleration, g (m/s<sup>2</sup>): 9.81  
Discharge Coefficient, C<sub>d</sub>: 0.63

Water Elevation (m)	Head over Orifice, hf (m)	Head differential, dh (m)	Pond Area "A" (m <sup>2</sup> )	Orifice Area "a" (m <sup>2</sup> )	Qea*"C*sqrt(2*g*hf) (m <sup>3</sup> /s)	Time differential, dt (s)
89.50	0.20	0	907.55	5.03E-03	1.00E-06	0.00
89.51	0.21	0.01	910.67	5.03E-03	6.43E-03	1416.74
89.52	0.22	0.01	913.79	5.03E-03	6.58E-03	1388.92
89.53	0.23	0.01	916.93	5.03E-03	6.73E-03	1363.05
89.54	0.24	0.01	920.06	5.03E-03	6.87E-03	1338.91
89.55	0.25	0.01	923.21	5.03E-03	7.01E-03	1316.34
89.56	0.26	0.01	926.35	5.03E-03	7.15E-03	1295.18
89.57	0.27	0.01	929.51	5.03E-03	7.29E-03	1275.30
89.58	0.28	0.01	932.67	5.03E-03	7.42E-03	1256.57
89.59	0.29	0.01	935.83	5.03E-03	7.55E-03	1238.90
89.60	0.30	0.01	939.00	5.03E-03	7.68E-03	1222.21
89.61	0.31	0.01	942.18	5.03E-03	7.81E-03	1206.40
89.62	0.32	0.01	945.36	5.03E-03	7.93E-03	1191.41
89.63	0.33	0.01	948.54	5.03E-03	8.06E-03	1177.17
89.64	0.34	0.01	951.73	5.03E-03	8.18E-03	1163.63
89.65	0.35	0.01	954.93	5.03E-03	8.30E-03	1150.74
89.66	0.36	0.01	958.13	5.03E-03	8.42E-03	1138.45
89.67	0.37	0.01	961.34	5.03E-03	8.53E-03	1126.72
89.68	0.38	0.01	964.56	5.03E-03	8.65E-03	1115.52
89.69	0.39	0.01	967.78	5.03E-03	8.76E-03	1104.80
89.70	0.40	0.01	971.00	5.03E-03	8.87E-03	1094.53
89.71	0.41	0.01	974.23	5.03E-03	8.98E-03	1084.70
89.72	0.42	0.01	977.47	5.03E-03	9.09E-03	1075.27
89.73	0.43	0.01	980.71	5.03E-03	9.20E-03	1066.22
89.74	0.44	0.01	983.95	5.03E-03	9.30E-03	1057.52
89.75	0.45	0.01	987.21	5.03E-03	9.41E-03	1049.16
89.76	0.46	0.01	990.46	5.03E-03	9.51E-03	1041.12
89.77	0.47	0.01	993.73	5.03E-03	9.62E-03	1033.38
89.78	0.48	0.01	997.00	5.03E-03	9.72E-03	1025.92
89.79	0.49	0.01	1000.27	5.03E-03	9.82E-03	1018.73
89.80	0.50	0.01	1003.55	5.03E-03	9.92E-03	1011.80
89.81	0.51	0.01	1006.84	5.03E-03	1.00E-02	1005.11
89.82	0.52	0.01	1010.13	5.03E-03	1.01E-02	998.65
89.83	0.53	0.01	1013.42	5.03E-03	1.02E-02	992.41
89.84	0.54	0.01	1016.72	5.03E-03	1.03E-02	986.39
89.85	0.55	0.01	1020.03	5.03E-03	1.04E-02	980.56
89.86	0.56	0.01	1023.34	5.03E-03	1.05E-02	974.92
89.87	0.57	0.01	1026.66	5.03E-03	1.06E-02	969.46
89.88	0.58	0.01	1029.99	5.03E-03	1.07E-02	964.18
89.89	0.59	0.01	1033.32	5.03E-03	1.08E-02	959.06
89.90	0.60	0.01	1036.65	5.03E-03	1.09E-02	954.11
89.91	0.61	0.01	1039.99	5.03E-03	1.10E-02	949.30
89.92	0.62	0.01	1043.34	5.03E-03	1.10E-02	944.65
89.93	0.63	0.01	1046.69	5.03E-03	1.11E-02	940.13
89.94	0.64	0.01	1050.04	5.03E-03	1.12E-02	935.75
89.95	0.65	0.01	1053.41	5.03E-03	1.13E-02	931.49
89.96	0.66	0.01	1056.77	5.03E-03	1.14E-02	927.36
89.97	0.67	0.01	1060.15	5.03E-03	1.15E-02	923.36
89.98	0.68	0.01	1063.53	5.03E-03	1.16E-02	919.46
89.99	0.69	0.01	1066.91	5.03E-03	1.17E-02	915.68
90.00	0.70	0.01	1070.30	5.03E-03	1.17E-02	912.00
90.01	0.71	0.01	1073.70	5.03E-03	1.18E-02	908.43
90.02	0.72	0.01	1077.10	5.03E-03	1.19E-02	904.96
90.03	0.73	0.01	1080.50	5.03E-03	1.20E-02	901.58
90.04	0.74	0.01	1083.91	5.03E-03	1.21E-02	898.29
90.05	0.75	0.01	1087.33	5.03E-03	1.21E-02	895.10
90.06	0.76	0.01	1090.75	5.03E-03	1.22E-02	891.99
90.07	0.77	0.01	1094.18	5.03E-03	1.23E-02	888.96
90.08	0.78	0.01	1097.62	5.03E-03	1.24E-02	886.02
90.09	0.79	0.01	1101.05	5.03E-03	1.25E-02	883.15
90.10	0.80	0.01	1104.50	5.03E-03	1.25E-02	880.36

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: August 9, 2021

**Retention Control - Surcharged condition**

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: August 9, 2021

**Retention Control Orifice**

Control Type	Rectangular Orifice
Elevation Range (m)	90.07-90.15
Base elevation (m)	90.07
Initial net head over Orifice	0
Orifice Depth (mm)	600
Orifice Width (mm)	1040
No. of orifices	1
Gravitational Acceleration, g (m/s <sup>2</sup> )	9.81
Discharge Coefficient, C <sub>d</sub>	0.63
Weir Discharge Coeff., C <sub>w</sub>	0.61

Water Elevation (m)	Head over Orifice, hf (m)	Head differential, dh (m)	Pond Area "A" (m <sup>2</sup> )	Orifice Area "a" (m <sup>2</sup> )	$Q=a*C_d*\sqrt{2*g*h_f}$ (m <sup>3</sup> /s)	Time differential, dt (s)
90.07	0.00	0	1094.18	6.24E-01	1.00E-06	0.00
90.08	0.01	0.01	1097.62	6.24E-01	1.74E-01	63.03
90.09	0.02	0.01	1101.05	6.24E-01	2.46E-01	44.71
90.10	0.03	0.01	1104.50	6.24E-01	3.02E-01	36.62
90.11	0.04	0.01	1107.95	6.24E-01	3.48E-01	31.81
90.12	0.05	0.01	1111.40	6.24E-01	3.89E-01	28.54
90.13	0.06	0.01	1114.87	6.24E-01	4.27E-01	26.14
90.14	0.07	0.01	1118.33	6.24E-01	4.61E-01	24.27
90.15	0.08	0.01	1121.80	6.24E-01	4.93E-01	22.78

**Numerical Results:**

Maximum Flowrate - Quantity Control Orifice	492.52 L/s
Maximum Flowrate - Extended Detention Orifice	3.97 L/s
Total Flowrate	496.5 L/s
Allowable Flowrate	906.9 L/s

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: August 9, 2021

**Retention Control - Surcharged condition**

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: August 9, 2021

**Extended Detention Orifice**

Control Type	Circular Orifice plate
Elevation Range (m)	90.07-90.15
Base elevation (m)	90.07
Initial net head over Orifice	0
Orifice Diameter (mm)	80
No. of orifices	1
Gravitational Acceleration, g (m/s <sup>2</sup> )	9.81
Discharge Coefficient, C <sub>d</sub>	0.63

Water Elevation (m)	Head over Orifice, hf (m)	Head differential, dh (m)	Pond Area "A" (m <sup>2</sup> )	Orifice Area "a" (m <sup>2</sup> )	$Q=a*C*\sqrt{2*g*hf}$ (m <sup>3</sup> /s)	Time differential, dt (s)
90.07	0.00	0	1094.18	5.03E-03	1.00E-06	0
90.08	0.01	0.01	1097.62	5.03E-03	1.40E-03	7825
90.09	0.02	0.01	1101.05	5.03E-03	1.98E-03	5551
90.10	0.03	0.01	1104.50	5.03E-03	2.43E-03	4546
90.11	0.04	0.01	1107.95	5.03E-03	2.81E-03	3949
90.12	0.05	0.01	1111.40	5.03E-03	3.14E-03	3543
90.13	0.06	0.01	1114.87	5.03E-03	3.44E-03	3245
90.14	0.07	0.01	1118.33	5.03E-03	3.71E-03	3013
90.15	0.08	0.01	1121.80	5.03E-03	3.97E-03	2828

# HY-8 Culvert Analysis Report

## Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 45.7678 cfs

Maximum Flow: 45.7678 cfs

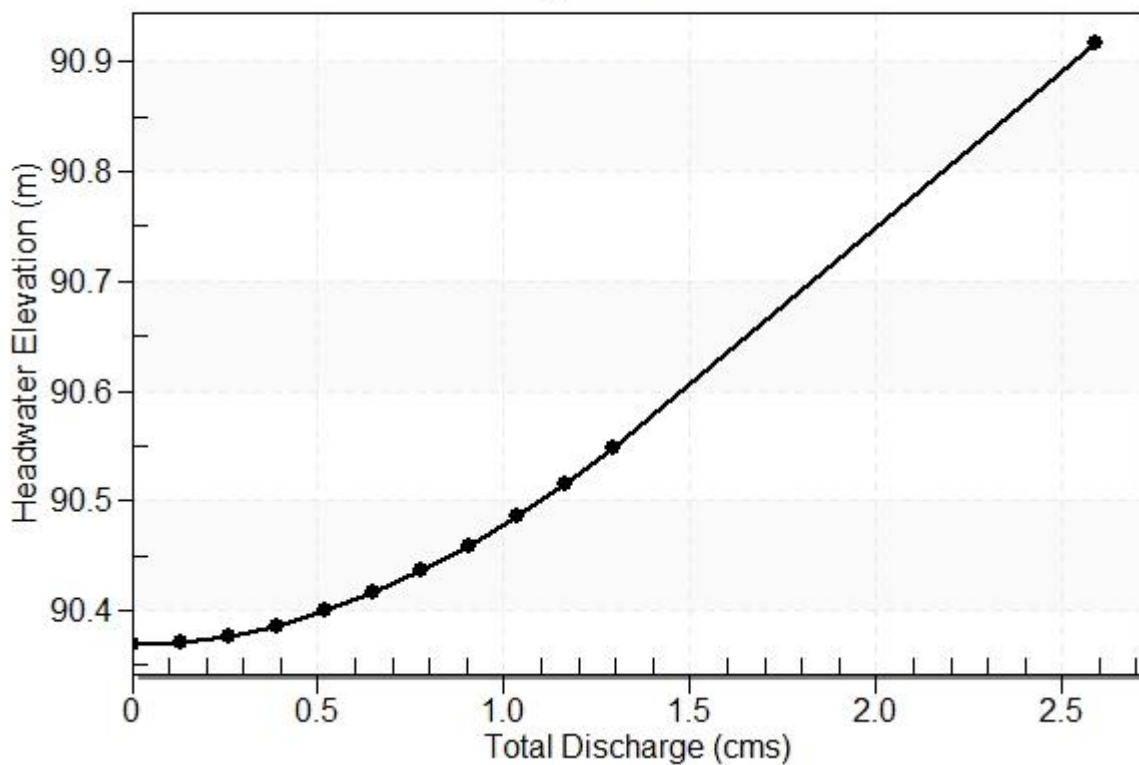
**Table 1 - Summary of Culvert Flows at Crossing: West Entrance**

Headwater Elevation (m)	Total Discharge (cms)	West Entrance Road Culvert Discharge (cms)	Roadway Discharge (cms)	Iterations
90.37	0.00	0.00	0.00	1
90.37	0.13	0.13	0.00	1
90.38	0.26	0.26	0.00	1
90.38	0.39	0.39	0.00	1
90.40	0.52	0.52	0.00	1
90.42	0.65	0.65	0.00	1
90.44	0.78	0.78	0.00	1
90.46	0.91	0.91	0.00	1
90.49	1.04	1.04	0.00	1
90.52	1.17	1.17	0.00	1
90.55	1.30	1.30	0.00	1
90.87	2.23	2.23	0.00	Overtopping

## Rating Curve Plot for Crossing: West Entrance

### Total Rating Curve

Crossing: West Entrance





**Table 2 - Culvert Summary Table: West Entrance Road Culvert**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	90.37	0.000	0.730	0-NF	0.000	0.000	0.740	0.740	0.000	0.000
0.13	0.13	90.37	0.118	0.732	6-FFt	0.250	0.070	0.740	0.740	0.084	0.000
0.26	0.26	90.38	0.185	0.737	6-FFt	0.395	0.110	0.740	0.740	0.169	0.000
0.39	0.39	90.38	0.240	0.745	6-FFt	0.544	0.144	0.740	0.740	0.253	0.000
0.52	0.52	90.40	0.289	0.760	6-FFt	0.758	0.173	0.740	0.740	0.338	0.000
0.65	0.65	90.42	0.334	0.776	6-FFt	0.758	0.200	0.740	0.740	0.422	0.000
0.78	0.78	90.44	0.376	0.796	6-FFt	0.758	0.224	0.740	0.740	0.506	0.000
0.91	0.91	90.46	0.420	0.819	6-FFt	0.758	0.247	0.740	0.740	0.591	0.000
1.04	1.04	90.49	0.463	0.846	6-FFt	0.758	0.269	0.740	0.740	0.675	0.000
1.17	1.17	90.52	0.507	0.875	6-FFt	0.758	0.290	0.740	0.740	0.759	0.000
1.30	1.30	90.55	0.551	0.908	6-FFt	0.758	0.310	0.740	0.740	0.844	0.000

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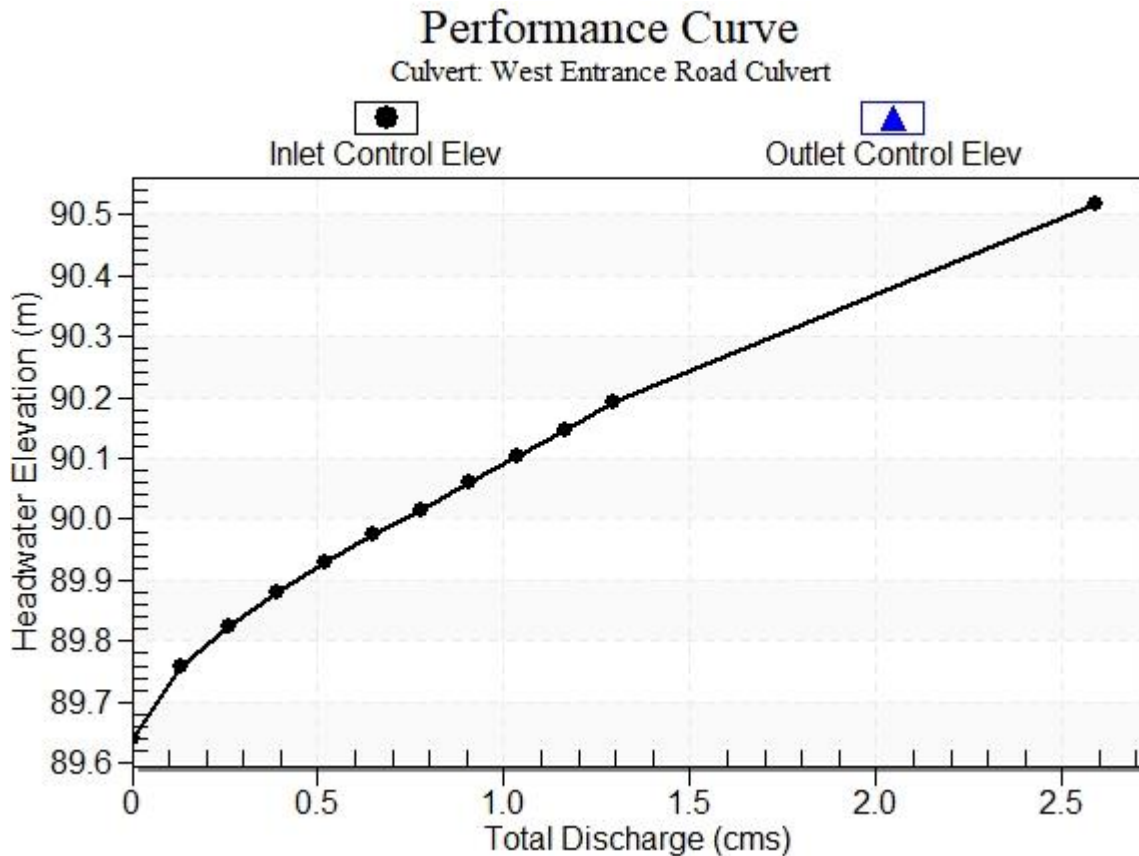
Straight Culvert

Inlet Elevation (invert): 89.64 m, Outlet Elevation (invert): 89.63 m

Culvert Length: 18.90 m, Culvert Slope: 0.0005

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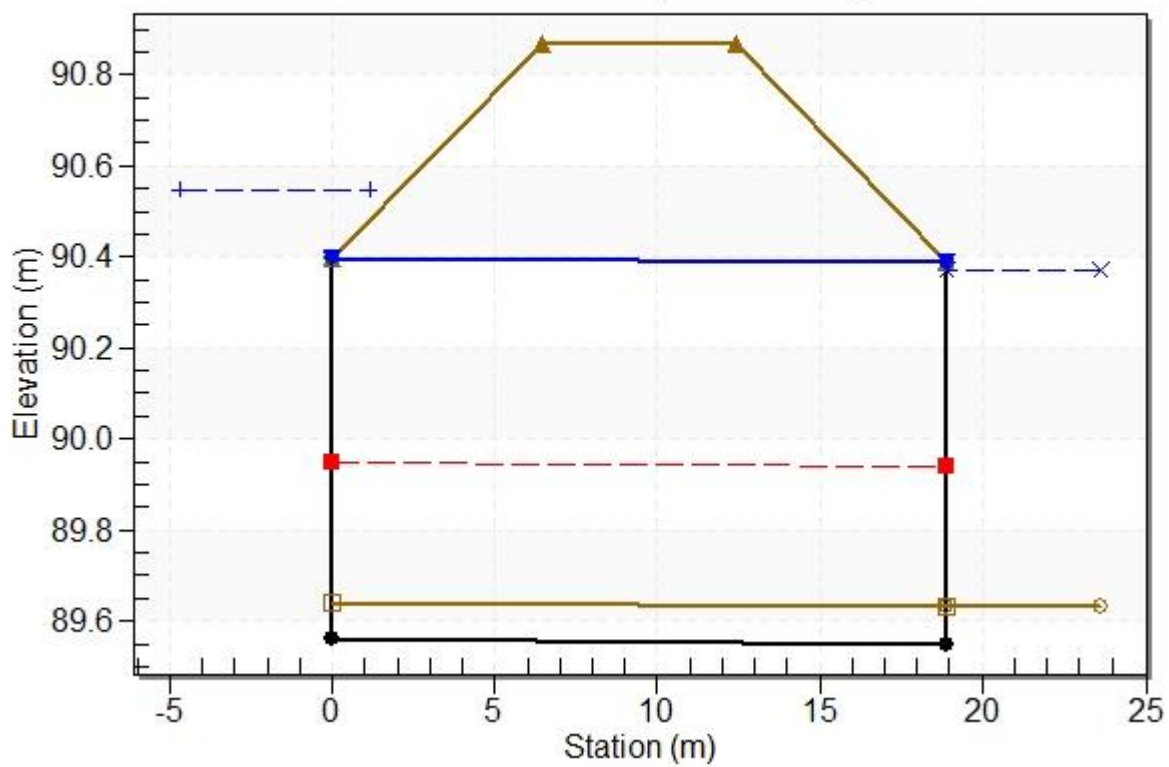
**Culvert Performance Curve Plot: West Entrance Road Culvert**



# Water Surface Profile Plot for Culvert: West Entrance Road Culvert

Crossing - West Entrance, Design Discharge - 1.30 cms

Culvert - West Entrance Road Culvert, Culvert Discharge - 1.30 cms



### **Site Data - West Entrance Road Culvert**

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.56 m

Outlet Station: 18.90 m

Outlet Elevation: 89.55 m

Number of Barrels: 2

### **Culvert Data Summary - West Entrance Road Culvert**

Barrel Shape: Pipe Arch

Barrel Span: 1244.60 mm

Barrel Rise: 838.20 mm

Barrel Material: Steel or Aluminum

Embedment: 80.00 mm

Barrel Manning's n: 0.0240 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None

**Table 3 - Downstream Channel Rating Curve (Crossing: West Entrance)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	90.37	0.74
4.58	90.37	0.74
9.15	90.37	0.74
13.73	90.37	0.74
18.31	90.37	0.74
22.88	90.37	0.74
27.46	90.37	0.74
32.04	90.37	0.74
36.61	90.37	0.74
41.19	90.37	0.74
45.77	90.37	0.74

**Tailwater Channel Data - West Entrance**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 90.37 m

**Roadway Data for Crossing: West Entrance**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 14.60 m

Crest Elevation: 90.87 m

Roadway Surface: Paved

Roadway Top Width: 6.00 m

**Crossing Discharge Data**

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

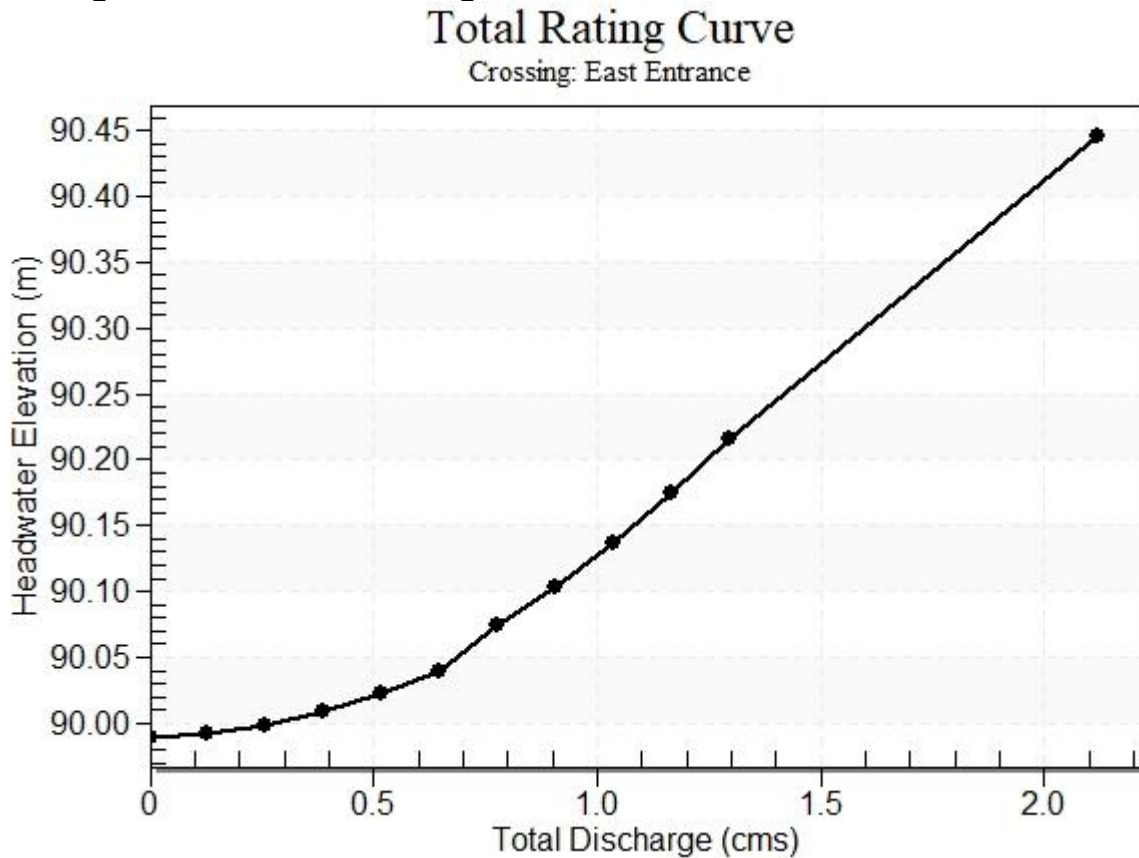
Minimum Flow: 0 cfs

Design Flow: 45.7325 cfs

Maximum Flow: 45.7325 cfs

**Table 4 - Summary of Culvert Flows at Crossing: East Entrance**

Headwater Elevation (m)	Total Discharge (cms)	East Entrance Road Culvert Discharge (cms)	Roadway Discharge (cms)	Iterations
89.99	0.00	0.00	0.00	1
89.99	0.13	0.13	0.00	1
90.00	0.26	0.26	0.00	1
90.01	0.39	0.39	0.00	1
90.02	0.52	0.52	0.00	1
90.04	0.65	0.65	0.00	1
90.07	0.78	0.78	0.00	1
90.10	0.91	0.91	0.00	1
90.14	1.04	1.04	0.00	1
90.17	1.17	1.17	0.00	1
90.22	1.29	1.29	0.00	1
90.40	1.77	1.77	0.00	Overtopping

**Rating Curve Plot for Crossing: East Entrance**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	89.99	0.000	0.710	0-NF	0.000	0.000	0.740	0.740	0.000	0.000
0.13	0.13	89.99	0.118	0.712	6-FFt	0.199	0.070	0.740	0.740	0.084	0.000
0.26	0.26	90.00	0.185	0.719	6-FFt	0.308	0.110	0.740	0.740	0.169	0.000
0.39	0.39	90.01	0.240	0.730	6-FFt	0.406	0.143	0.740	0.740	0.253	0.000
0.52	0.52	90.02	0.289	0.743	6-FFt	0.507	0.173	0.740	0.740	0.337	0.000
0.65	0.65	90.04	0.334	0.760	6-FFt	0.636	0.200	0.740	0.740	0.422	0.000
0.78	0.78	90.07	0.376	0.795	6-FFt	0.758	0.224	0.740	0.740	0.506	0.000
0.91	0.91	90.10	0.419	0.824	6-FFt	0.758	0.247	0.740	0.740	0.590	0.000
1.04	1.04	90.14	0.463	0.858	6-FFt	0.758	0.269	0.740	0.740	0.674	0.000
1.17	1.17	90.17	0.507	0.895	6-FFt	0.758	0.290	0.740	0.740	0.759	0.000
1.29	1.29	90.22	0.550	0.936	6-FFt	0.758	0.310	0.740	0.740	0.843	0.000

**Table 5 - Culvert Summary Table: East Entrance Road Culvert**

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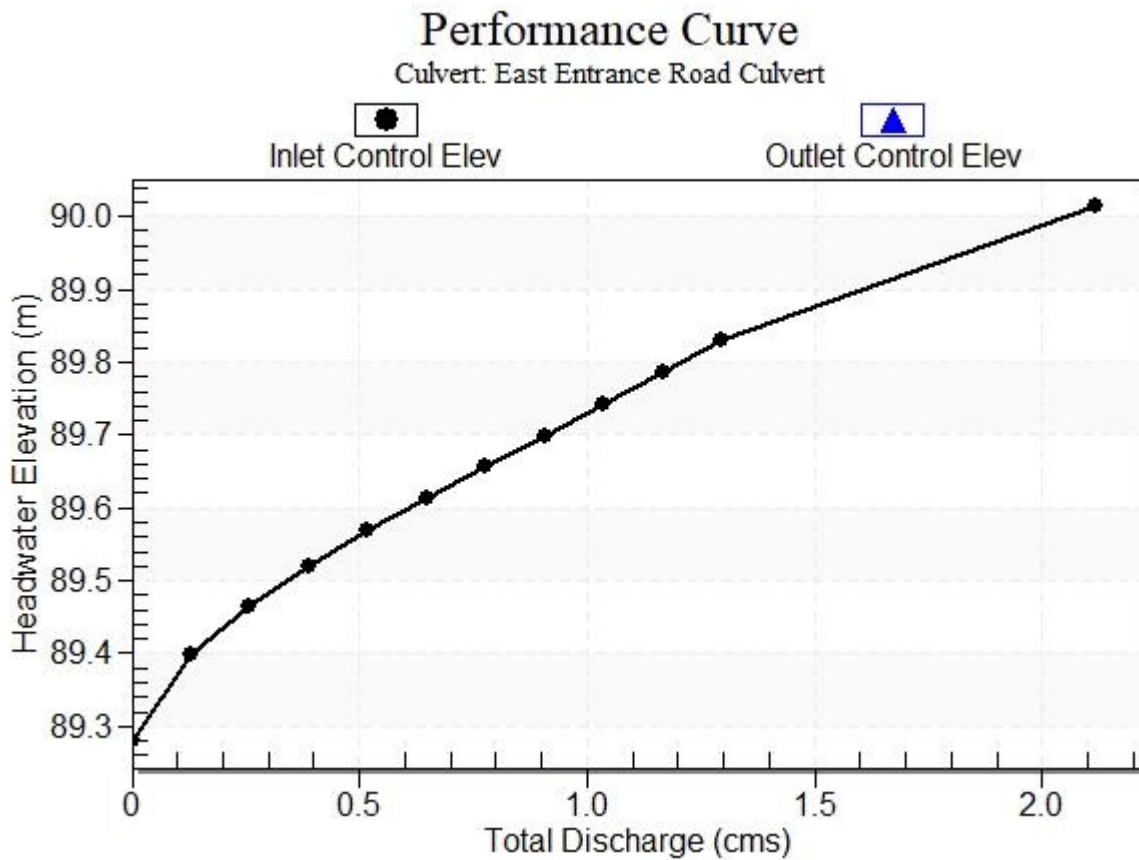
Straight Culvert

Inlet Elevation (invert): 89.28 m,    Outlet Elevation (invert): 89.25 m

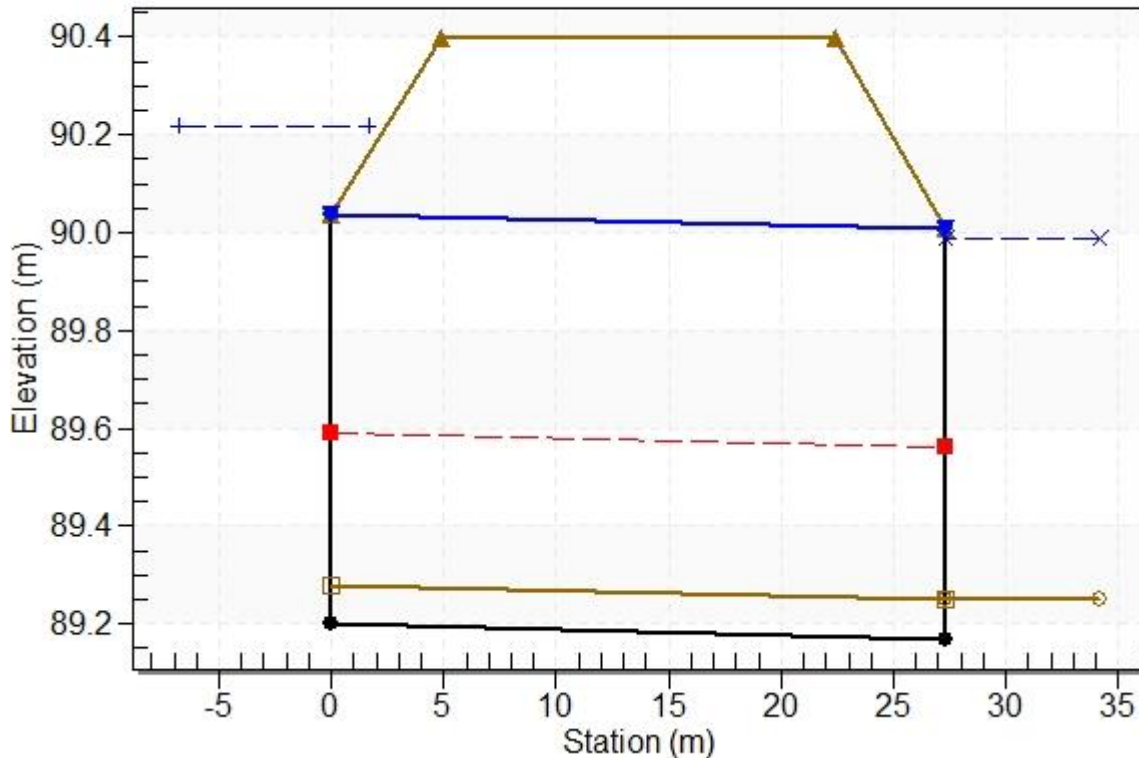
Culvert Length: 27.30 m,    Culvert Slope: 0.0011

\*\*\*\*\*

**Culvert Performance Curve Plot: East Entrance Road Culvert**



**Water Surface Profile Plot for Culvert: East Entrance Road Culvert**  
**Crossing - East Entrance, Design Discharge - 1.29 cms**  
Culvert - East Entrance Road Culvert, Culvert Discharge - 1.29 cms



**Site Data - East Entrance Road Culvert**

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.20 m

Outlet Station: 27.30 m

Outlet Elevation: 89.17 m

Number of Barrels: 2

**Culvert Data Summary - East Entrance Road Culvert**

Barrel Shape: Pipe Arch

Barrel Span: 1244.60 mm

Barrel Rise: 838.20 mm

Barrel Material: Steel or Aluminum

Embedment: 80.00 mm

Barrel Manning's n: 0.0240 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None



**Table 6 - Downstream Channel Rating Curve (Crossing: East Entrance)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	89.99	0.74
4.57	89.99	0.74
9.15	89.99	0.74
13.72	89.99	0.74
18.29	89.99	0.74
22.87	89.99	0.74
27.44	89.99	0.74
32.01	89.99	0.74
36.59	89.99	0.74
41.16	89.99	0.74
45.73	89.99	0.74

**Tailwater Channel Data - East Entrance**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 89.99 m

**Roadway Data for Crossing: East Entrance**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 14.60 m

Crest Elevation: 90.40 m

Roadway Surface: Paved

Roadway Top Width: 17.45 m

# HY-8 Culvert Analysis Report

## Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 4.02587 cfs

Maximum Flow: 8.15769 cfs

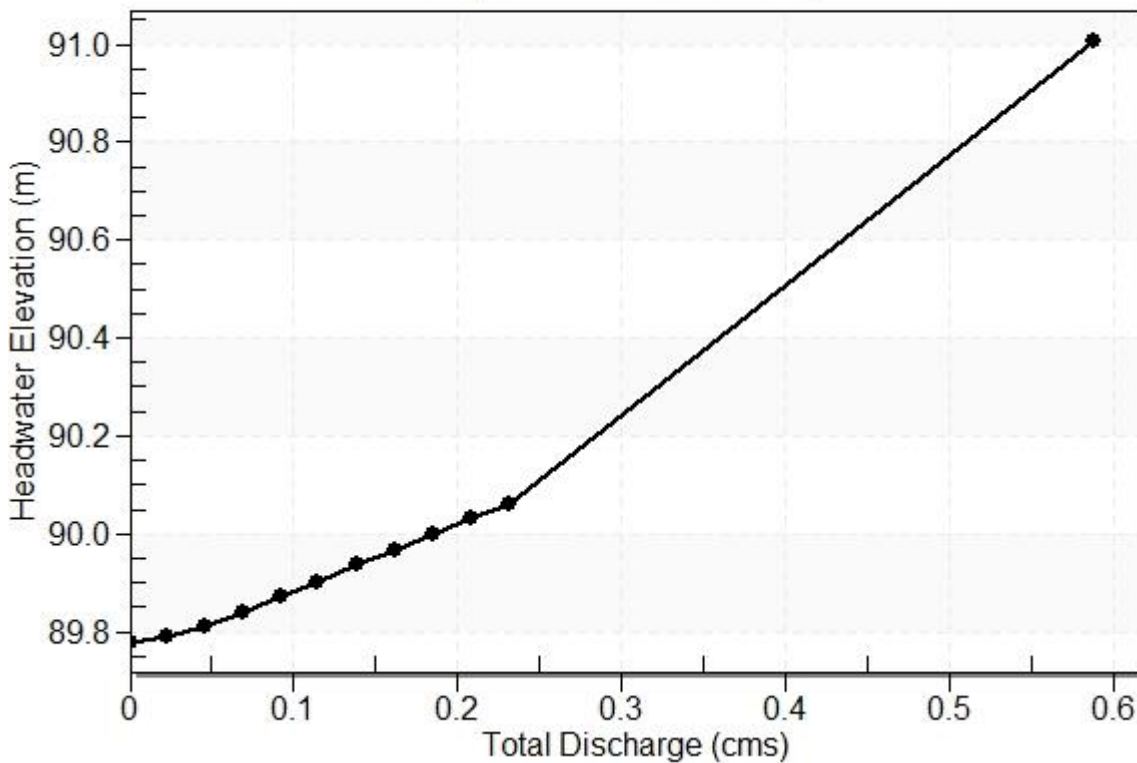
**Table 1 - Summary of Culvert Flows at Crossing: West Ditch Site Culvert 10y**

Headwater Elevation (m)	Total Discharge (cms)	West Ditch Site Culvert 10y Discharge (cms)	Roadway Discharge (cms)	Iterations
89.78	0.00	0.00	0.00	1
89.79	0.02	0.02	0.00	1
89.81	0.05	0.05	0.00	1
89.84	0.07	0.07	0.00	1
89.87	0.09	0.09	0.00	1
89.90	0.11	0.11	0.00	1
89.94	0.14	0.14	0.00	1
89.97	0.16	0.16	0.00	1
90.00	0.18	0.18	0.00	1
90.03	0.21	0.21	0.00	1
90.06	0.23	0.23	0.00	1
91.00	0.57	0.57	0.00	Overtopping

## Rating Curve Plot for Crossing: West Ditch Site Culvert 10y

### Total Rating Curve

Crossing: West Ditch Site Culvert 10y



**Table 2 - Culvert Summary Table: West Ditch Site Culvert 10y**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	89.78	0.000	0.195	0-NF	0.000	0.000	0.236	0.240	0.000	0.000
0.02	0.02	89.79	0.075	0.204	3-M1t	0.129	0.044	0.236	0.240	0.114	0.000
0.05	0.05	89.81	0.119	0.227	3-M1t	0.199	0.069	0.236	0.240	0.228	0.000
0.07	0.07	89.84	0.154	0.256	3-M2t	0.261	0.090	0.236	0.240	0.341	0.000
0.09	0.09	89.87	0.186	0.287	3-M2t	0.321	0.109	0.236	0.240	0.455	0.000
0.11	0.11	89.90	0.213	0.317	3-M2t	0.382	0.125	0.236	0.240	0.561	0.000
0.14	0.14	89.94	0.242	0.351	3-M2t	0.480	0.141	0.236	0.240	0.683	0.000
0.16	0.16	89.97	0.267	0.382	3-M2t	0.545	0.156	0.236	0.240	0.796	0.000
0.18	0.18	90.00	0.290	0.414	3-M2t	0.545	0.170	0.236	0.240	0.910	0.000
0.21	0.21	90.03	0.311	0.445	3-M2t	0.545	0.183	0.236	0.240	1.024	0.000
0.23	0.23	90.06	0.333	0.477	3-M2t	0.545	0.196	0.236	0.240	1.138	0.000

\*\*\*\*\*

Straight Culvert

Inlet Elevation (invert): 89.58 m, Outlet Elevation (invert): 89.54 m

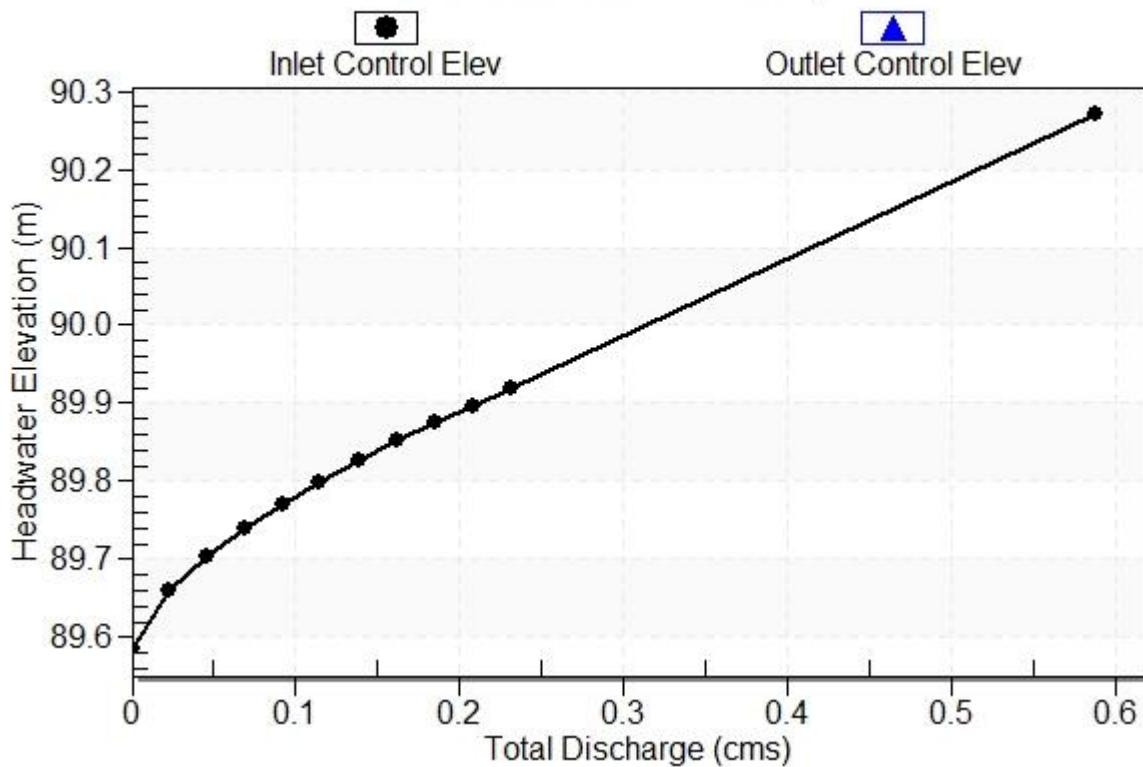
Culvert Length: 36.00 m, Culvert Slope: 0.0011

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**Culvert Performance Curve Plot: West Ditch Site Culvert 10y**

Performance Curve

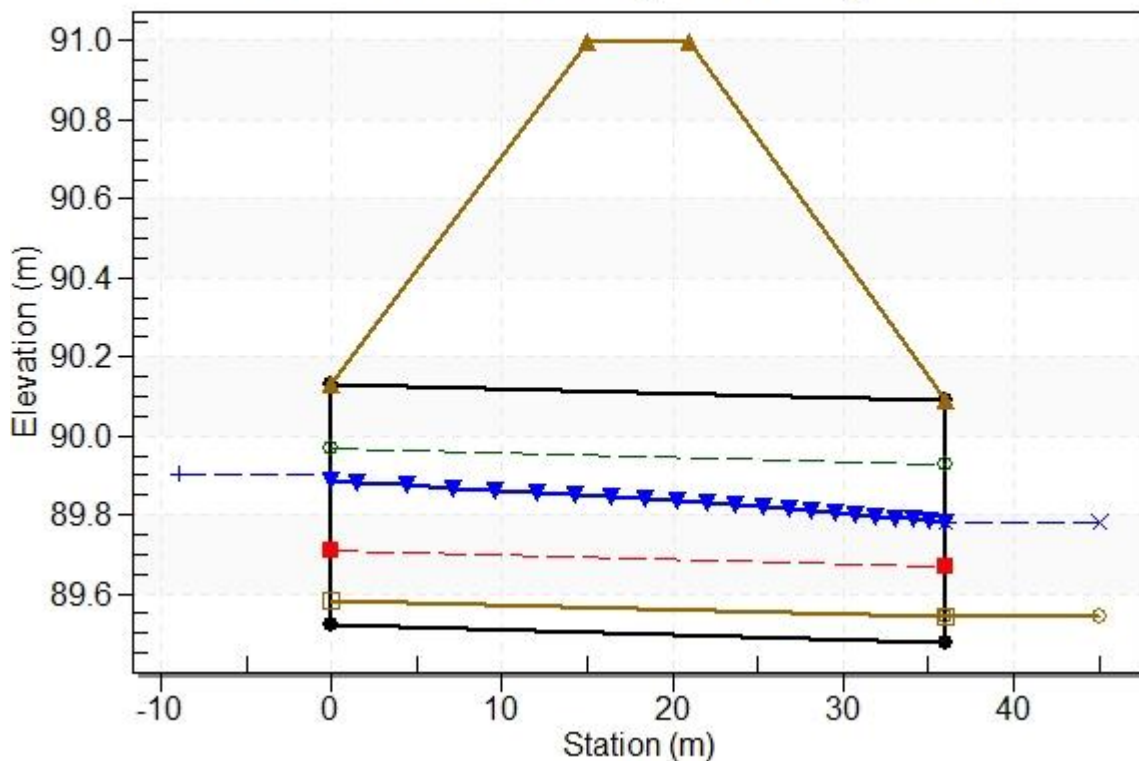
Culvert: West Ditch Site Culvert 10y



### Water Surface Profile Plot for Culvert: West Ditch Site Culvert 10y

Crossing - West Ditch Site Culvert 10y, Design Discharge - 0.11 cms

Culvert - West Ditch Site Culvert 10y, Culvert Discharge - 0.11 cms



### Site Data - West Ditch Site Culvert 10y

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.52 m

Outlet Station: 36.00 m

Outlet Elevation: 89.48 m

Number of Barrels: 1

### Culvert Data Summary - West Ditch Site Culvert 10y

Barrel Shape: Pipe Arch

Barrel Span: 889.00 mm

Barrel Rise: 609.60 mm

Barrel Material: Steel or Aluminum

Embedment: 65.00 mm

Barrel Manning's n: 0.0250 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None

**Table 3 - Downstream Channel Rating Curve (Crossing: West Ditch Site Culvert 10y)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	89.78	0.24
0.82	89.78	0.24
1.63	89.78	0.24
2.45	89.78	0.24
3.26	89.78	0.24
4.03	89.78	0.24
4.89	89.78	0.24
5.71	89.78	0.24
6.53	89.78	0.24
7.34	89.78	0.24
8.16	89.78	0.24

**Tailwater Channel Data - West Ditch Site Culvert 10y**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 89.78 m

**Roadway Data for Crossing: West Ditch Site Culvert 10y**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 14.00 m

Crest Elevation: 91.00 m

Roadway Surface: Paved

Roadway Top Width: 6.00 m

## **Crossing Discharge Data**

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

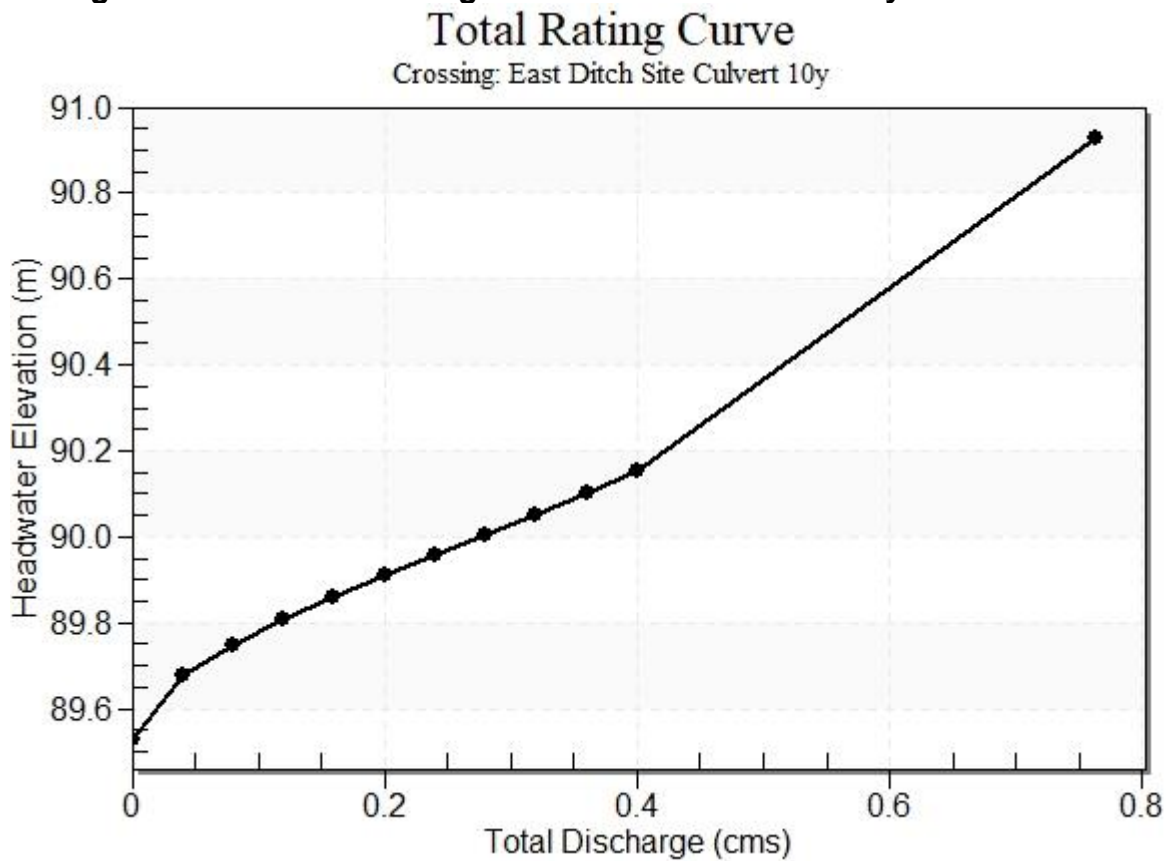
Minimum Flow: 0 cfs

Design Flow: 7.09825 cfs

Maximum Flow: 14.1259 cfs

**Table 4 - Summary of Culvert Flows at Crossing: East Ditch Site Culvert 10y**

Headwater Elevation (m)	Total Discharge (cms)	East Ditch Site Culvert 10y Discharge (cms)	Roadway Discharge (cms)	Iterations
89.53	0.00	0.00	0.00	1
89.68	0.04	0.04	0.00	1
89.75	0.08	0.08	0.00	1
89.81	0.12	0.12	0.00	1
89.86	0.16	0.16	0.00	1
89.91	0.20	0.20	0.00	1
89.96	0.24	0.24	0.00	1
90.00	0.28	0.28	0.00	1
90.05	0.32	0.32	0.00	1
90.10	0.36	0.36	0.00	1
90.15	0.40	0.40	0.00	1
90.92	0.74	0.74	0.00	Overtopping

**Rating Curve Plot for Crossing: East Ditch Site Culvert 10y**



**Table 5 - Culvert Summary Table: East Ditch Site Culvert 10y**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	89.53	0.000	0.000	0-NF	0.000	0.000	0.000	0.000	0.000	0.000
0.04	0.04	89.68	0.108	0.147	2-M2c	0.156	0.063	0.063	0.000	0.774	0.000
0.08	0.08	89.75	0.169	0.219	2-M2c	0.245	0.099	0.099	0.000	0.966	0.000
0.12	0.12	89.81	0.220	0.278	2-M2c	0.331	0.129	0.129	0.000	1.100	0.000
0.16	0.16	89.86	0.265	0.330	2-M2c	0.433	0.155	0.155	0.000	1.209	0.000
0.20	0.20	89.91	0.305	0.381	2-M2c	0.550	0.180	0.180	0.000	1.307	0.000
0.24	0.24	89.96	0.342	0.427	2-M2c	0.550	0.201	0.201	0.000	1.392	0.000
0.28	0.28	90.00	0.379	0.474	2-M2c	0.550	0.222	0.222	0.000	1.470	0.000
0.32	0.32	90.05	0.417	0.521	2-M2c	0.550	0.242	0.242	0.000	1.542	0.000
0.36	0.36	90.10	0.453	0.570	7-M2c	0.550	0.260	0.260	0.000	1.611	0.000
0.40	0.40	90.15	0.490	0.623	7-M2c	0.550	0.278	0.278	0.000	1.680	0.000

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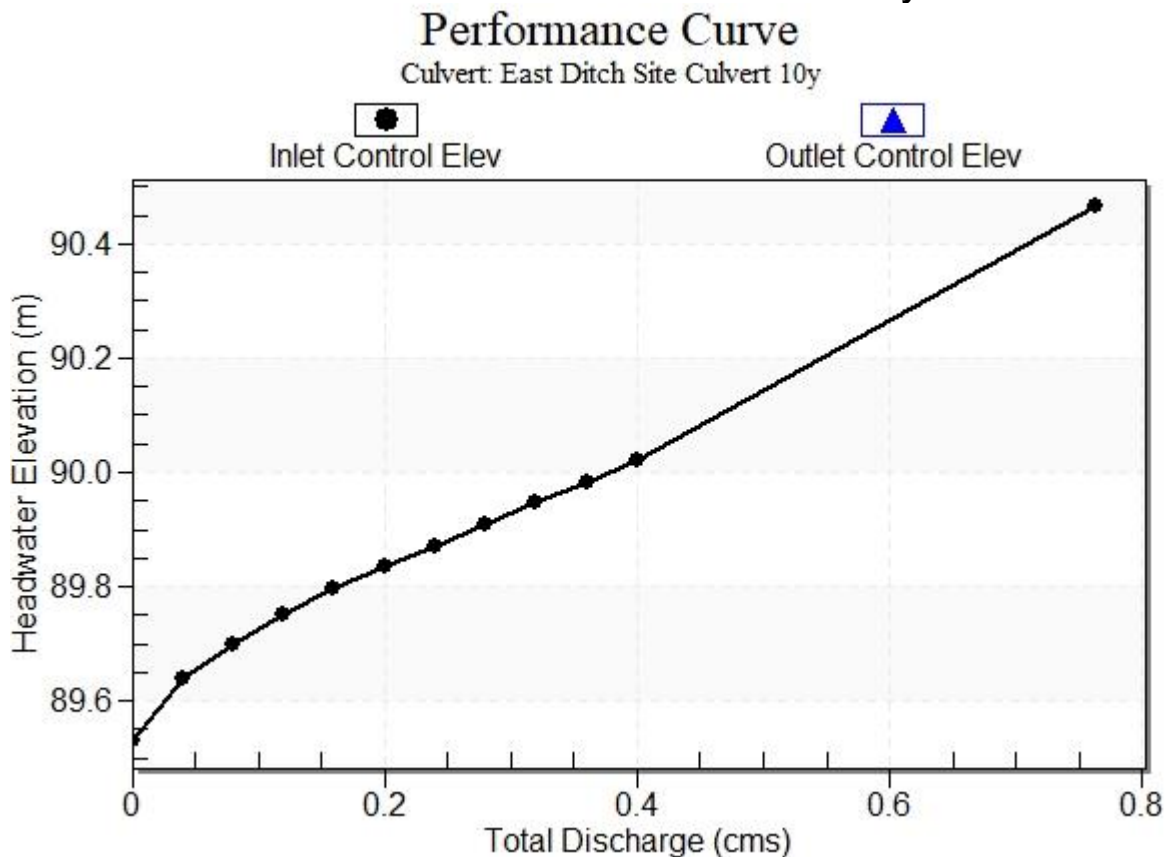
Straight Culvert

Inlet Elevation (invert): 89.53 m, Outlet Elevation (invert): 89.50 m

Culvert Length: 21.55 m, Culvert Slope: 0.0014

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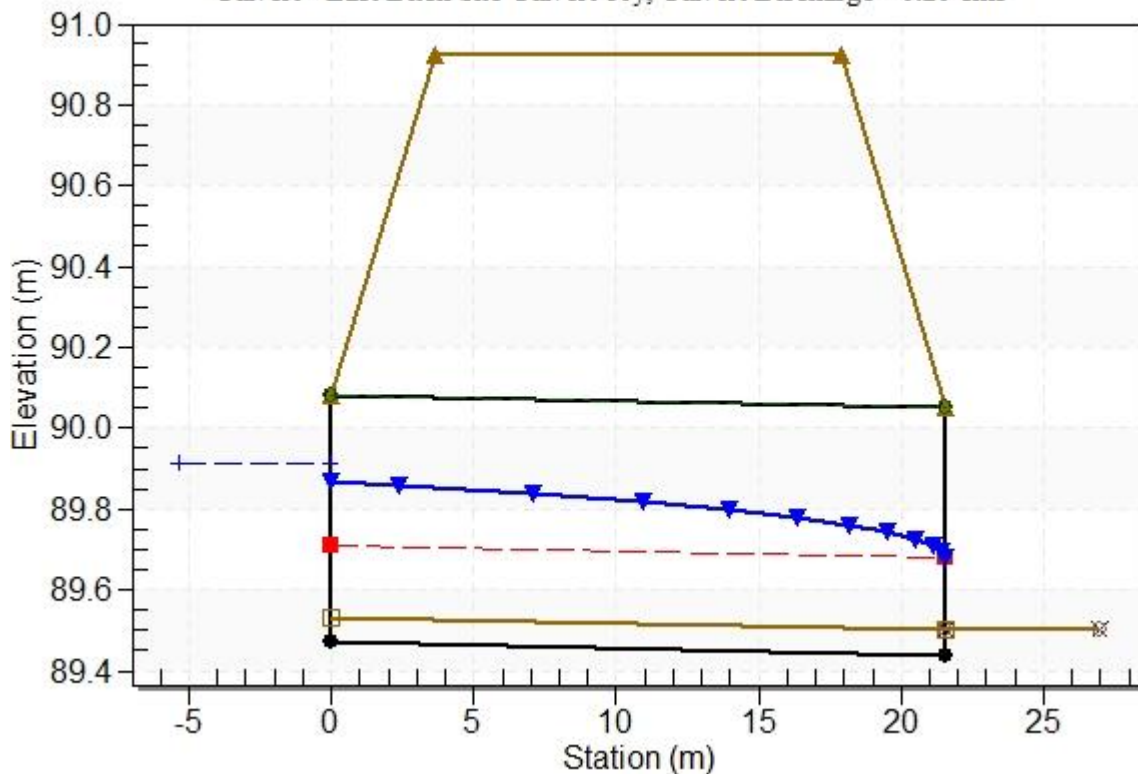
**Culvert Performance Curve Plot: East Ditch Site Culvert 10y**



## Water Surface Profile Plot for Culvert: East Ditch Site Culvert 10y

Crossing - East Ditch Site Culvert 10y, Design Discharge - 0.20 cms

Culvert - East Ditch Site Culvert 10y, Culvert Discharge - 0.20 cms



## Site Data - East Ditch Site Culvert 10y

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.47 m

Outlet Station: 21.55 m

Outlet Elevation: 89.44 m

Number of Barrels: 1

## Culvert Data Summary - East Ditch Site Culvert 10y

Barrel Shape: Pipe Arch

Barrel Span: 889.00 mm

Barrel Rise: 609.60 mm

Barrel Material: Steel or Aluminum

Embedment: 60.00 mm

Barrel Manning's n: 0.0250 (top and sides)

Manning's n: 0.0300 (bottom)

Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None

**Table 6 - Downstream Channel Rating Curve (Crossing: East Ditch Site Culvert 10y)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	89.50	0.00
1.41	89.50	0.00
2.83	89.50	0.00
4.24	89.50	0.00
5.65	89.50	0.00
7.10	89.50	0.00
8.48	89.50	0.00
9.89	89.50	0.00
11.30	89.50	0.00
12.71	89.50	0.00
14.13	89.50	0.00

**Tailwater Channel Data - East Ditch Site Culvert 10y**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 89.50 m

**Roadway Data for Crossing: East Ditch Site Culvert 10y**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 25.00 m

Crest Elevation: 90.92 m

Roadway Surface: Paved

Roadway Top Width: 14.20 m

## Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 8.15769 cfs

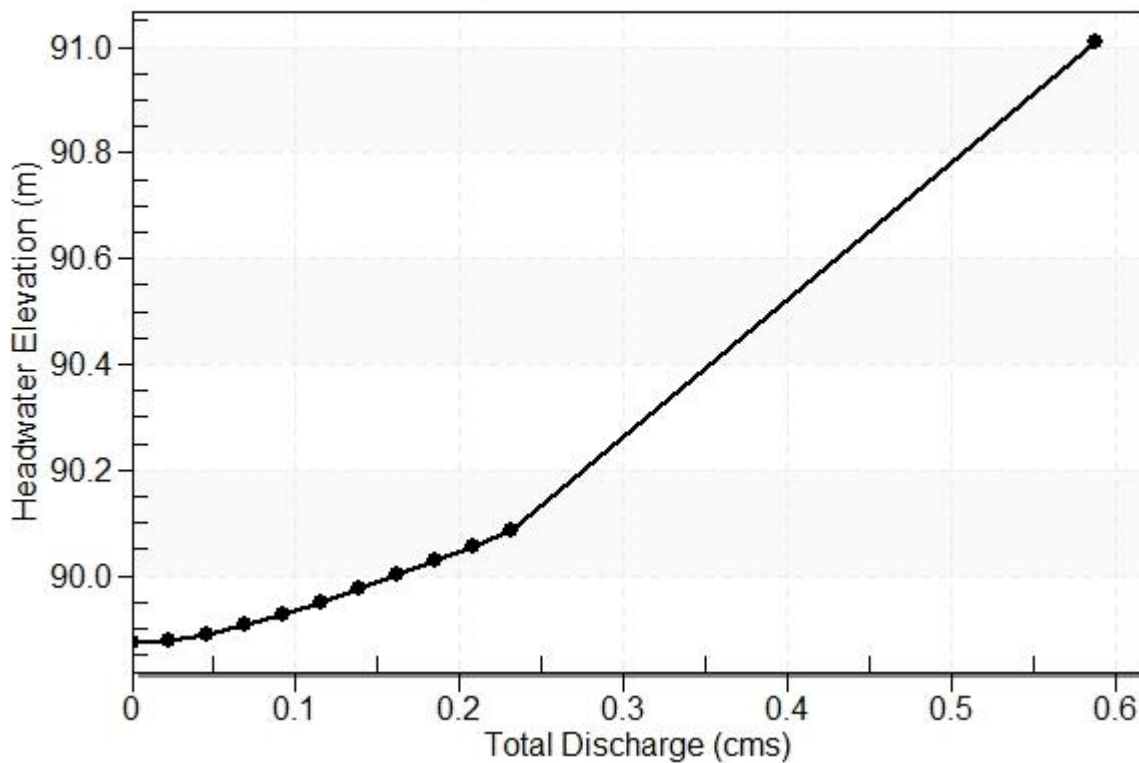
Maximum Flow: 8.15769 cfs

**Table 7 - Summary of Culvert Flows at Crossing: West Ditch Site Culvert 100y**

Headwater Elevation (m)	Total Discharge (cms)	West Ditch Site Culvert 100y Discharge (cms)	Roadway Discharge (cms)	Iterations
89.88	0.00	0.00	0.00	1
89.88	0.02	0.02	0.00	1
89.89	0.05	0.05	0.00	1
89.91	0.07	0.07	0.00	1
89.93	0.09	0.09	0.00	1
89.95	0.12	0.12	0.00	1
89.97	0.14	0.14	0.00	1
90.00	0.16	0.16	0.00	1
90.03	0.18	0.18	0.00	1
90.06	0.21	0.21	0.00	1
90.09	0.23	0.23	0.00	1
91.00	0.56	0.56	0.00	Overtopping

## Rating Curve Plot for Crossing: West Ditch Site Culvert 100y

**Total Rating Curve**  
Crossing: West Ditch Site Culvert 100y



**Table 8 - Culvert Summary Table: West Ditch Site Culvert 100y**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	89.88	0.000	0.282	0-NF	0.000	0.000	0.323	0.335	0.000	0.000
0.02	0.02	89.88	0.075	0.286	3-M1t	0.128	0.044	0.323	0.335	0.084	0.000
0.05	0.05	89.89	0.119	0.296	3-M1t	0.198	0.069	0.323	0.335	0.169	0.000
0.07	0.07	89.91	0.154	0.312	3-M1t	0.260	0.090	0.323	0.335	0.253	0.000
0.09	0.09	89.93	0.186	0.333	3-M1t	0.322	0.109	0.323	0.335	0.337	0.000
0.12	0.12	89.95	0.215	0.356	3-M2t	0.389	0.125	0.323	0.335	0.421	0.000
0.14	0.14	89.97	0.242	0.381	3-M2t	0.537	0.141	0.323	0.335	0.506	0.000
0.16	0.16	90.00	0.267	0.407	3-M2t	0.537	0.155	0.323	0.335	0.590	0.000
0.18	0.18	90.03	0.290	0.435	3-M2t	0.537	0.169	0.323	0.335	0.674	0.000
0.21	0.21	90.06	0.312	0.464	3-M2t	0.537	0.182	0.323	0.335	0.758	0.000
0.23	0.23	90.09	0.334	0.494	3-M2t	0.537	0.195	0.323	0.335	0.843	0.000

\*\*\*\*\*

#### Straight Culvert

Inlet Elevation (invert): 89.59 m, Outlet Elevation (invert): 89.55 m

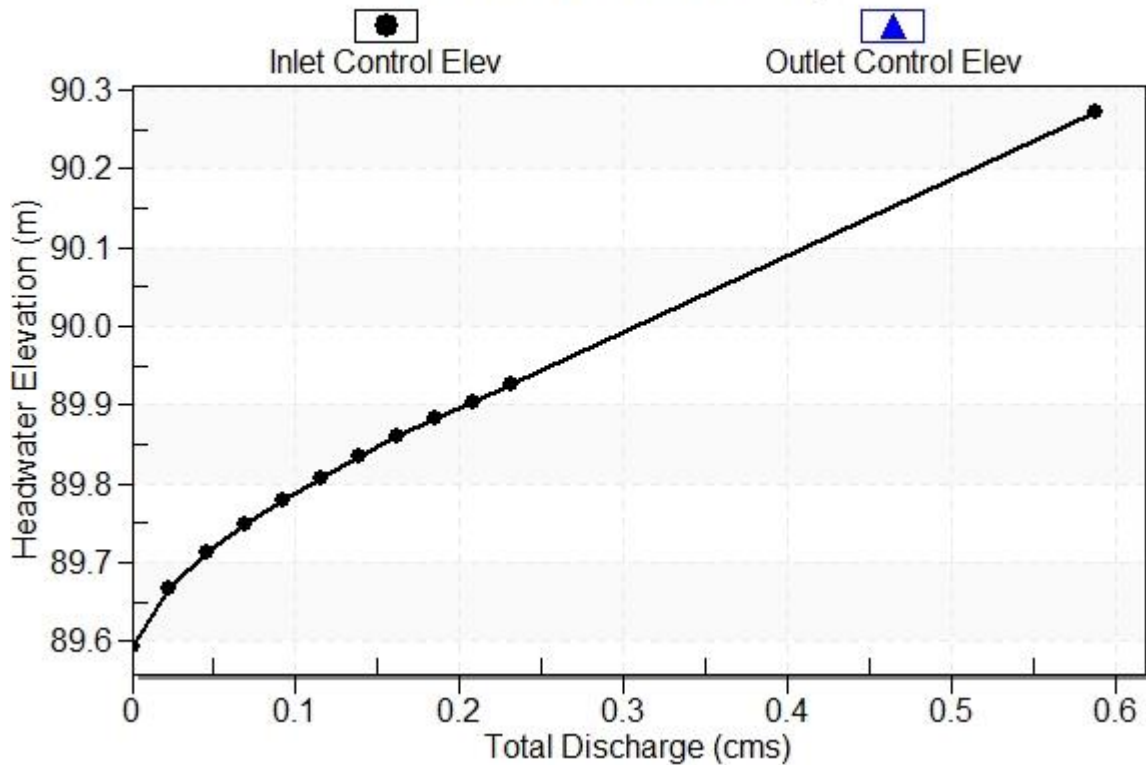
Culvert Length: 36.00 m, Culvert Slope: 0.0011

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### Culvert Performance Curve Plot: West Ditch Site Culvert 100y

#### Performance Curve

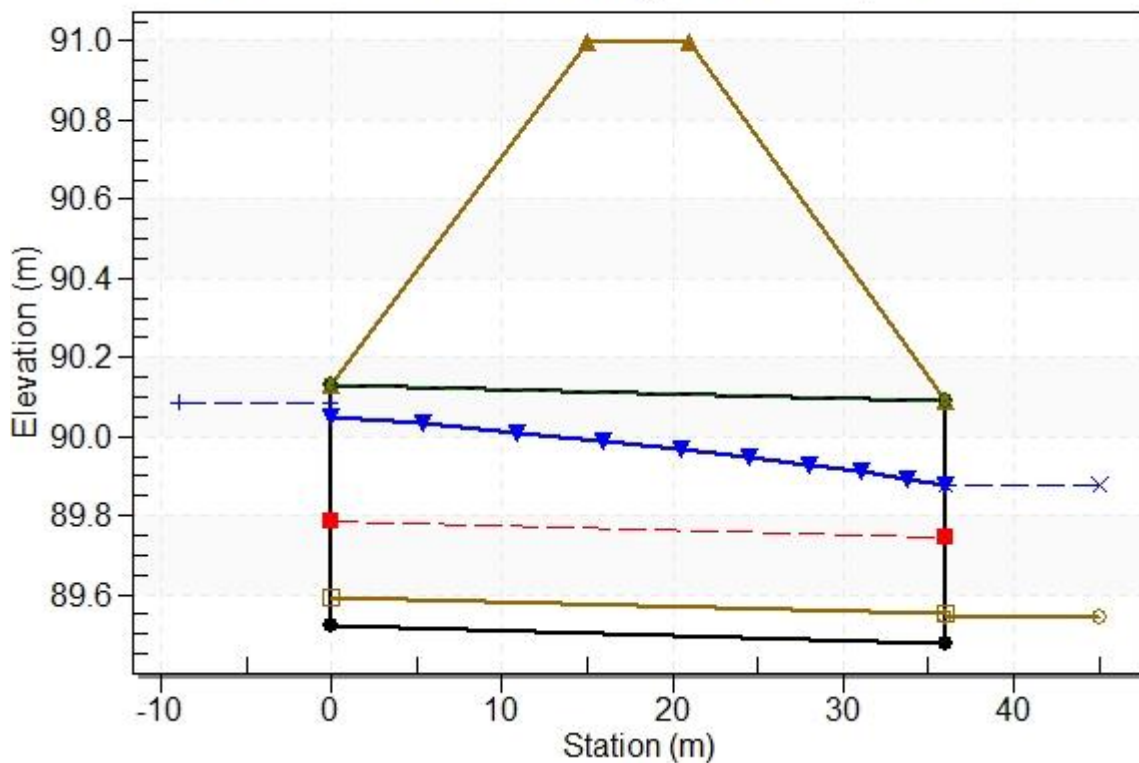
Culvert: West Ditch Site Culvert 100y



### Water Surface Profile Plot for Culvert: West Ditch Site Culvert 100y

Crossing - West Ditch Site Culvert 100y, Design Discharge - 0.23 cms

Culvert - West Ditch Site Culvert 100y, Culvert Discharge - 0.23 cms



### Site Data - West Ditch Site Culvert 100y

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.52 m

Outlet Station: 36.00 m

Outlet Elevation: 89.48 m

Number of Barrels: 1

### Culvert Data Summary - West Ditch Site Culvert 100y

Barrel Shape: Pipe Arch

Barrel Span: 889.00 mm

Barrel Rise: 609.60 mm

Barrel Material: Steel or Aluminum

Embedment: 73.00 mm

Barrel Manning's n: 0.0250 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None

**Table 9 - Downstream Channel Rating Curve (Crossing: West Ditch Site Culvert 100y)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	89.88	0.34
0.82	89.88	0.34
1.63	89.88	0.34
2.45	89.88	0.34
3.26	89.88	0.34
4.08	89.88	0.34
4.89	89.88	0.34
5.71	89.88	0.34
6.53	89.88	0.34
7.34	89.88	0.34
8.16	89.88	0.34

**Tailwater Channel Data - West Ditch Site Culvert 100y**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 89.88 m

**Roadway Data for Crossing: West Ditch Site Culvert 100y**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 14.00 m

Crest Elevation: 91.00 m

Roadway Surface: Paved

Roadway Top Width: 6.00 m



## Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 14.3024 cfs

Maximum Flow: 14.3024 cfs

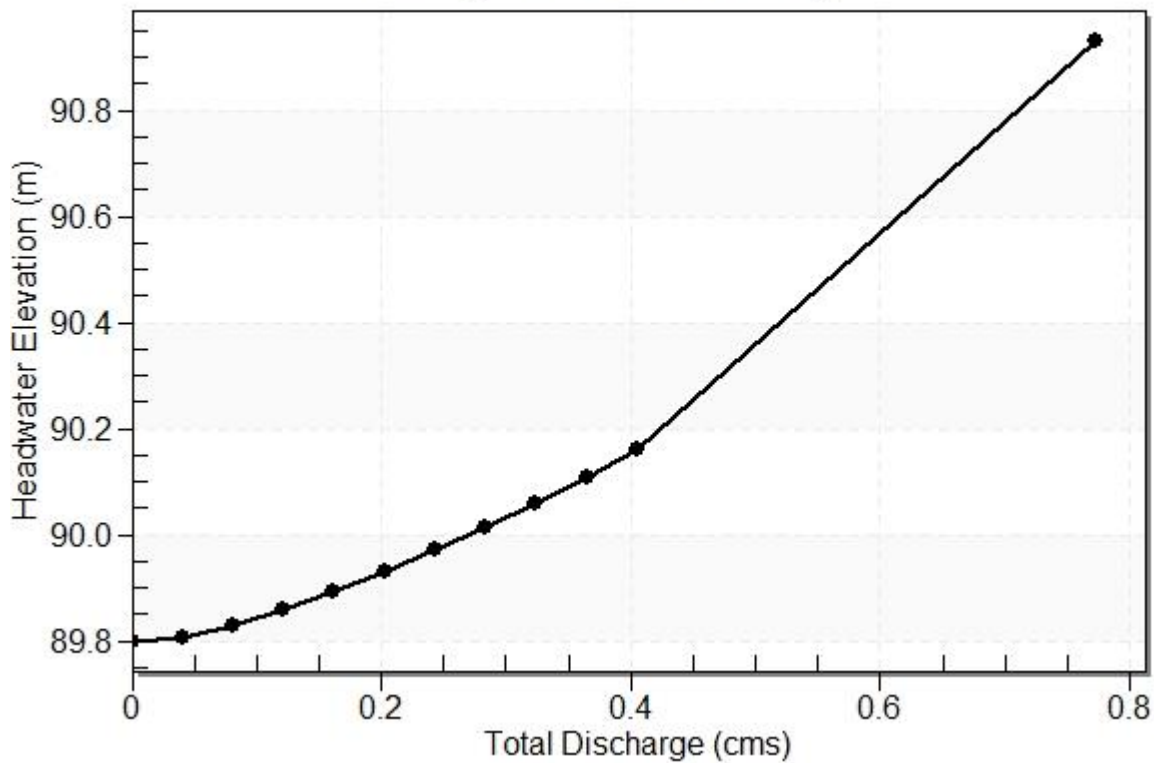
**Table 10 - Summary of Culvert Flows at Crossing: East Ditch Site Culvert 100y**

Headwater Elevation (m)	Total Discharge (cms)	East Ditch Site Culvert 100y Discharge (cms)	Roadway Discharge (cms)	Iterations
89.80	0.00	0.00	0.00	1
89.81	0.04	0.04	0.00	1
89.83	0.08	0.08	0.00	1
89.86	0.12	0.12	0.00	1
89.89	0.16	0.16	0.00	1
89.93	0.20	0.20	0.00	1
89.97	0.24	0.24	0.00	1
90.02	0.28	0.28	0.00	1
90.06	0.32	0.32	0.00	1
90.11	0.36	0.36	0.00	1
90.16	0.40	0.40	0.00	1
90.92	0.74	0.74	0.00	Overtopping

### Rating Curve Plot for Crossing: East Ditch Site Culvert 100y

#### Total Rating Curve

Crossing: East Ditch Site Culvert 100y



**Table 11 - Culvert Summary Table: East Ditch Site Culvert 100y**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	89.80	0.000	0.270	0-NF	0.000	0.000	0.300	0.300	0.000	0.000
0.04	0.04	89.81	0.109	0.277	3-M1t	0.157	0.064	0.300	0.300	0.158	0.000
0.08	0.08	89.83	0.171	0.298	3-M1t	0.247	0.100	0.300	0.300	0.316	0.000
0.12	0.12	89.86	0.222	0.327	3-M2t	0.334	0.130	0.300	0.300	0.474	0.000
0.16	0.16	89.89	0.267	0.362	3-M2t	0.440	0.157	0.300	0.300	0.632	0.000
0.20	0.20	89.93	0.306	0.401	3-M2t	0.550	0.180	0.300	0.300	0.790	0.000
0.24	0.24	89.97	0.344	0.442	3-M2t	0.550	0.203	0.300	0.300	0.948	0.000
0.28	0.28	90.02	0.383	0.485	3-M2t	0.550	0.223	0.300	0.300	1.106	0.000
0.32	0.32	90.06	0.421	0.530	3-M2t	0.550	0.243	0.300	0.300	1.264	0.000
0.36	0.36	90.11	0.457	0.577	3-M2t	0.550	0.262	0.300	0.300	1.422	0.000
0.40	0.40	90.16	0.494	0.631	3-M2t	0.550	0.280	0.300	0.300	1.580	0.000

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Straight Culvert

Inlet Elevation (invert): 89.53 m,      Outlet Elevation (invert): 89.50 m

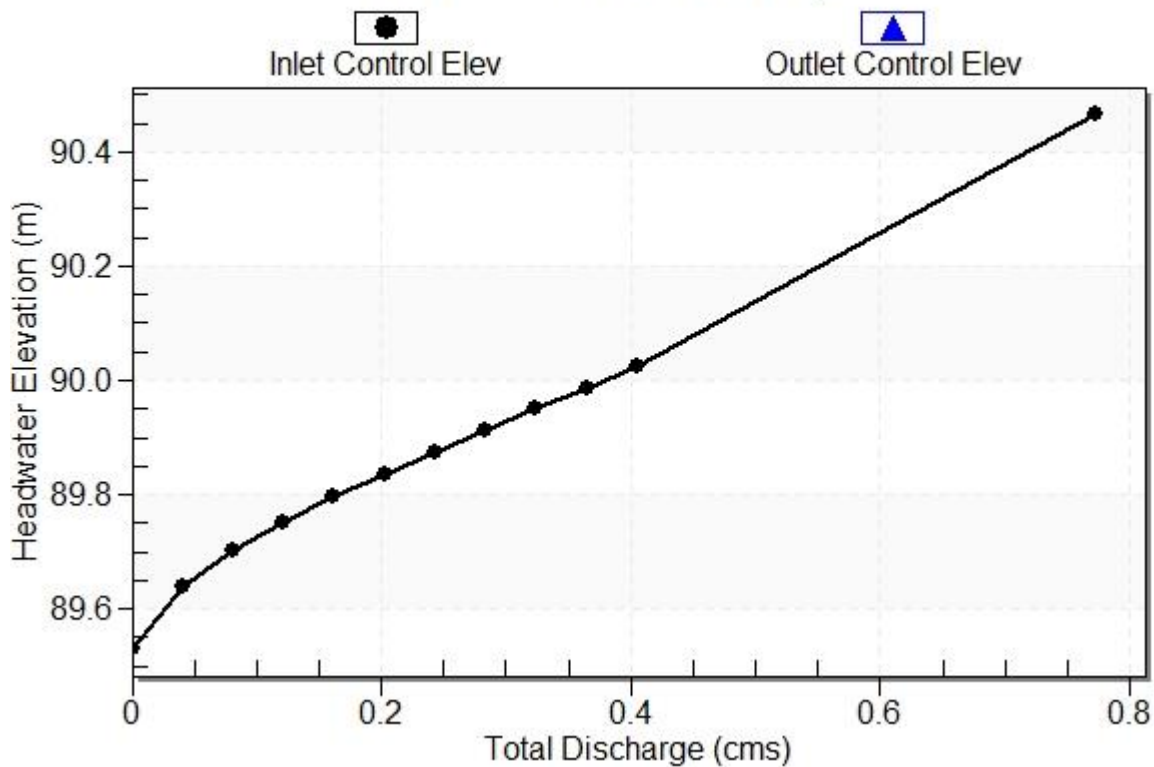
Culvert Length: 21.55 m,      Culvert Slope: 0.0014

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**Culvert Performance Curve Plot: East Ditch Site Culvert 100y**

### Performance Curve

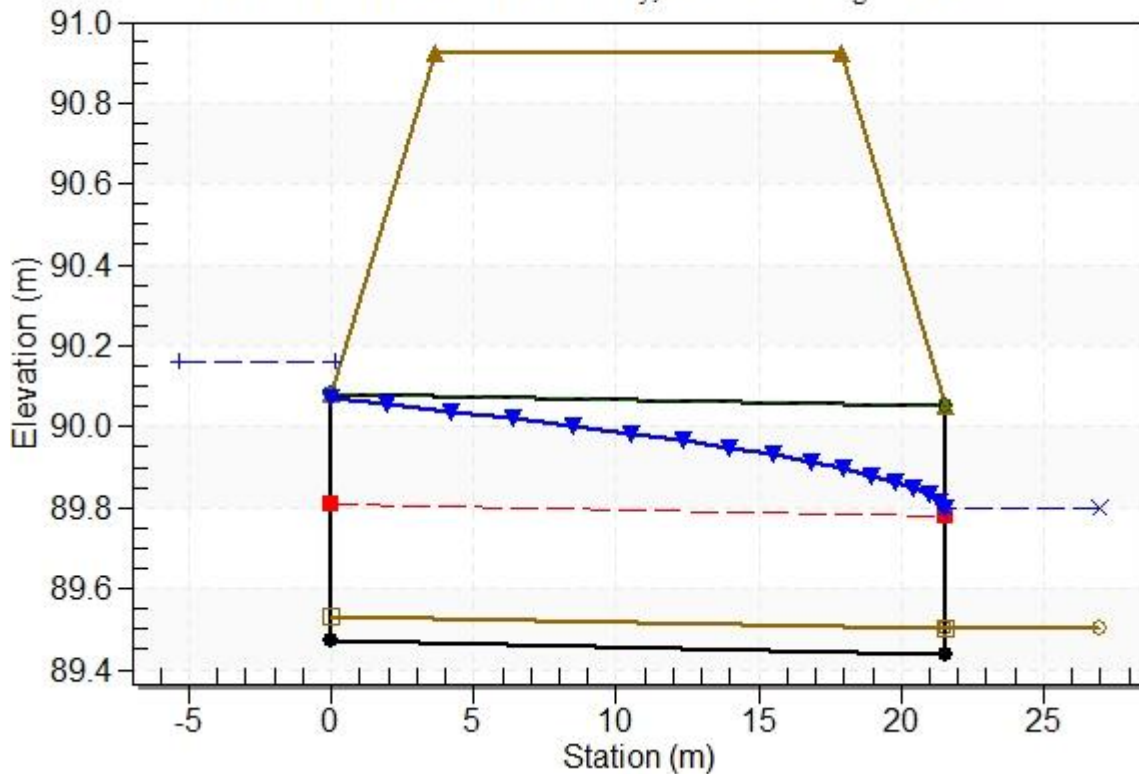
Culvert: East Ditch Site Culvert 100y



### Water Surface Profile Plot for Culvert: East Ditch Site Culvert 100y

Crossing - East Ditch Site Culvert 100y, Design Discharge - 0.40 cms

Culvert - East Ditch Site Culvert 100y, Culvert Discharge - 0.40 cms



### Site Data - East Ditch Site Culvert 100y

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.47 m

Outlet Station: 21.55 m

Outlet Elevation: 89.44 m

Number of Barrels: 1

### Culvert Data Summary - East Ditch Site Culvert 100y

Barrel Shape: Pipe Arch

Barrel Span: 889.00 mm

Barrel Rise: 609.60 mm

Barrel Material: Steel or Aluminum

Embedment: 60.00 mm

Barrel Manning's n: 0.0250 (top and sides)

Manning's n: 0.0300 (bottom)

Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None

**Table 12 - Downstream Channel Rating Curve (Crossing: East Ditch Site Culvert 100y)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	89.80	0.30
1.43	89.80	0.30
2.86	89.80	0.30
4.29	89.80	0.30
5.72	89.80	0.30
7.15	89.80	0.30
8.58	89.80	0.30
10.01	89.80	0.30
11.44	89.80	0.30
12.87	89.80	0.30
14.30	89.80	0.30

**Tailwater Channel Data - East Ditch Site Culvert 100y**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 89.80 m

**Roadway Data for Crossing: East Ditch Site Culvert 100y**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 25.00 m

Crest Elevation: 90.92 m

Roadway Surface: Paved

Roadway Top Width: 14.20 m

## Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 32.0304 cfs

Maximum Flow: 32.0304 cfs

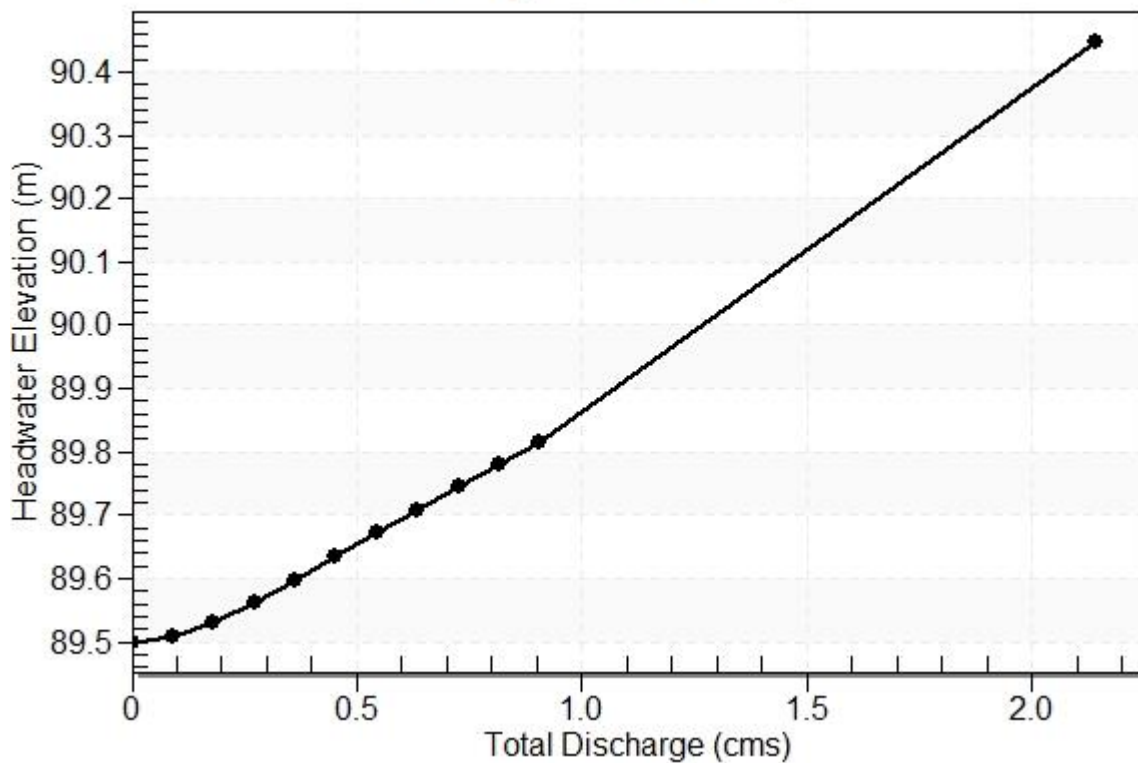
**Table 13 - Summary of Culvert Flows at Crossing: Transfer Culvert 100y**

Headwater Elevation (m)	Total Discharge (cms)	Transfer Culvert 100y Discharge (cms)	Roadway Discharge (cms)	Iterations
89.50	0.00	0.00	0.00	1
89.51	0.09	0.09	0.00	1
89.53	0.18	0.18	0.00	1
89.56	0.27	0.27	0.00	1
89.60	0.36	0.36	0.00	1
89.63	0.45	0.45	0.00	1
89.67	0.54	0.54	0.00	1
89.71	0.63	0.63	0.00	1
89.74	0.73	0.73	0.00	1
89.78	0.82	0.82	0.00	1
89.82	0.91	0.91	0.00	1
90.43	2.01	2.01	0.00	Overtopping

## Rating Curve Plot for Crossing: Transfer Culvert 100y

### Total Rating Curve

Crossing: Transfer Culvert 100y



**Table 14 - Culvert Summary Table: Transfer Culvert 100y**

Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)	Tailwater Velocity (m/s)
0.00	0.00	89.50	0.000	0.200	0-NF	0.000	0.000	0.325	0.325	0.000	0.000
0.09	0.09	89.51	0.105	0.208	3-M1t	0.096	0.061	0.325	0.325	0.135	0.000
0.18	0.18	89.53	0.165	0.231	3-M1t	0.148	0.096	0.325	0.325	0.271	0.000
0.27	0.27	89.56	0.214	0.262	3-M1t	0.191	0.125	0.325	0.325	0.406	0.000
0.36	0.36	89.60	0.258	0.297	3-M1t	0.229	0.151	0.325	0.325	0.541	0.000
0.45	0.45	89.63	0.298	0.334	3-M1t	0.266	0.174	0.325	0.325	0.676	0.000
0.54	0.54	89.67	0.336	0.371	3-M1t	0.301	0.195	0.325	0.325	0.812	0.000
0.63	0.63	89.71	0.368	0.407	3-M2t	0.336	0.215	0.325	0.325	0.947	0.000
0.73	0.73	89.74	0.400	0.444	3-M2t	0.371	0.234	0.325	0.325	1.082	0.000
0.82	0.82	89.78	0.432	0.479	3-M2t	0.407	0.253	0.325	0.325	1.218	0.000
0.91	0.91	89.82	0.465	0.515	3-M2t	0.446	0.270	0.325	0.325	1.353	0.000

\*\*\*\*\*

#### Straight Culvert

Inlet Elevation (invert): 89.30 m, Outlet Elevation (invert): 89.18 m

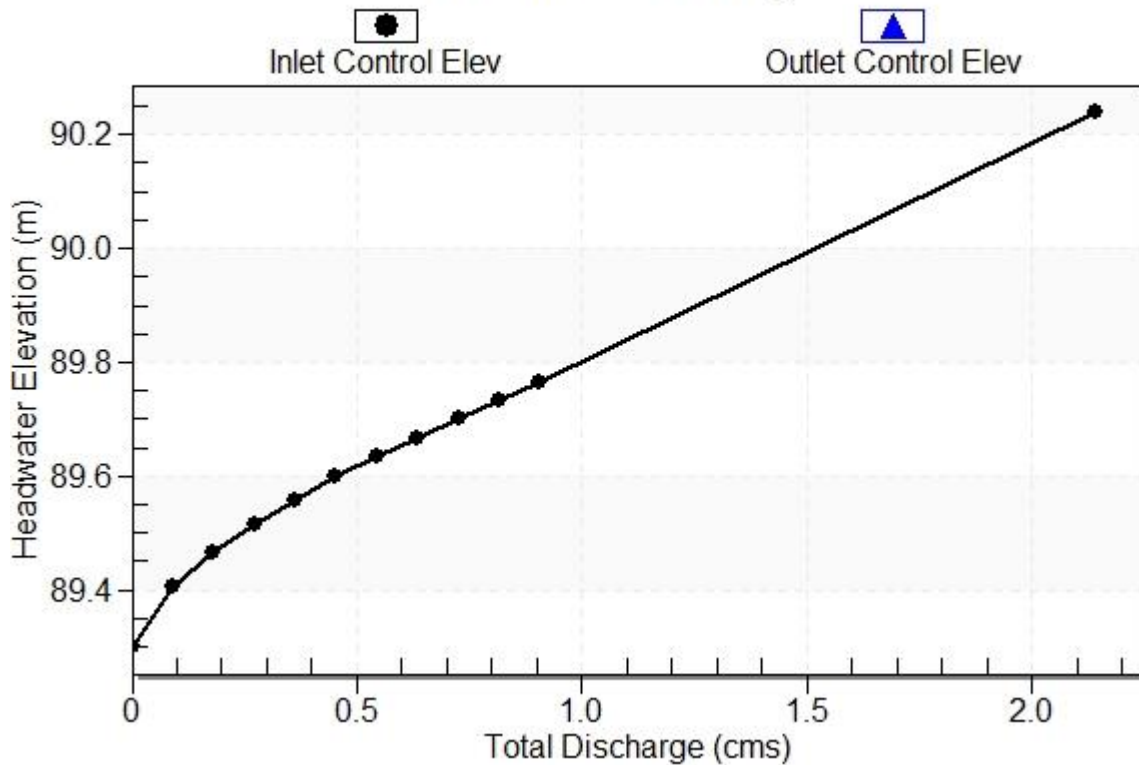
Culvert Length: 22.00 m, Culvert Slope: 0.0057

\*\*\*\*\*

### Culvert Performance Curve Plot: Transfer Culvert 100y

#### Performance Curve

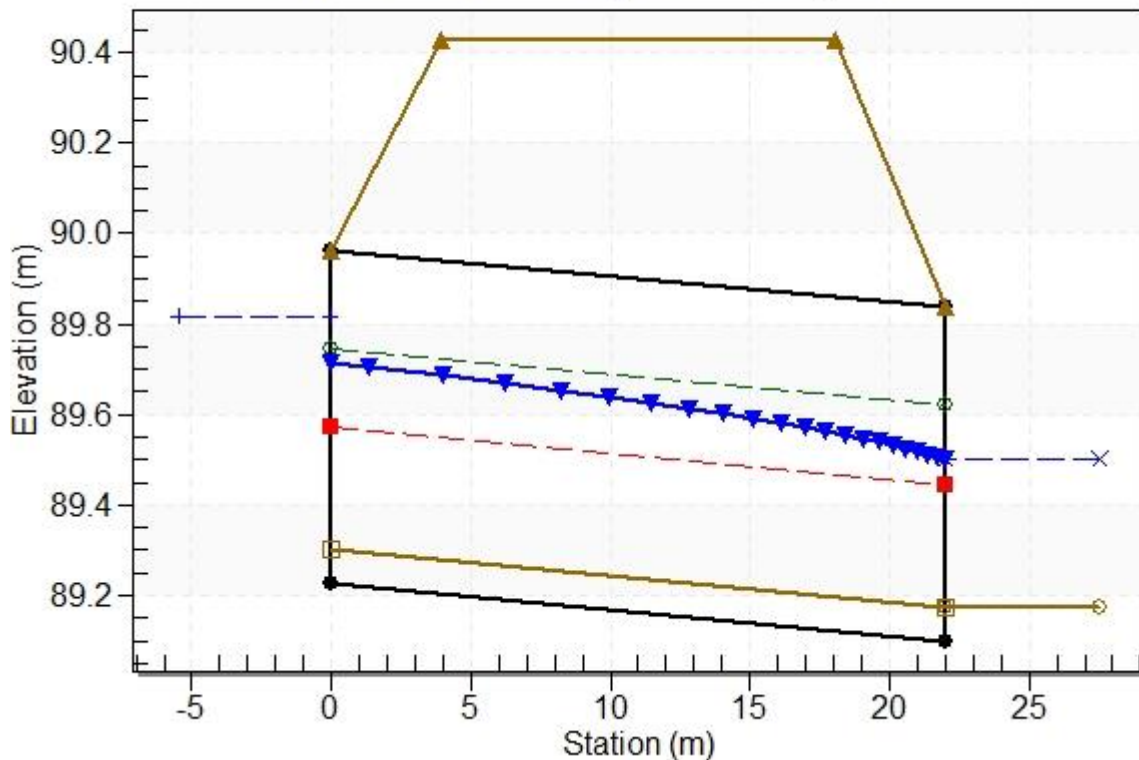
Culvert: Transfer Culvert 100y



## Water Surface Profile Plot for Culvert: Transfer Culvert 100y

Crossing - Transfer Culvert 100y, Design Discharge - 0.91 cms

Culvert - Transfer Culvert 100y, Culvert Discharge - 0.91 cms



### Site Data - Transfer Culvert 100y

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 89.23 m

Outlet Station: 22.00 m

Outlet Elevation: 89.10 m

Number of Barrels: 2

### Culvert Data Summary - Transfer Culvert 100y

Barrel Shape: Pipe Arch

Barrel Span: 1066.80 mm

Barrel Rise: 736.60 mm

Barrel Material: Steel or Aluminum

Embedment: 75.00 mm

Barrel Manning's n: 0.0250 (top and sides)

Manning's n: 0.0300 (bottom)

Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None



**Table 15 - Downstream Channel Rating Curve (Crossing: Transfer Culvert 100y)**

Flow (cms)	Water Surface Elev (m)	Depth (m)
0.00	89.50	0.33
3.20	89.50	0.33
6.41	89.50	0.33
9.61	89.50	0.33
12.81	89.50	0.33
16.02	89.50	0.33
19.22	89.50	0.33
22.42	89.50	0.33
25.62	89.50	0.33
28.83	89.50	0.33
32.03	89.50	0.33

**Tailwater Channel Data - Transfer Culvert 100y**

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 89.50 m

**Roadway Data for Crossing: Transfer Culvert 100y**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 25.00 m

Crest Elevation: 90.43 m

Roadway Surface: Paved

Roadway Top Width: 14.20 m

# Hydraulic Analysis Report

## Project Data

Project Title: A001103 - Fastfrate Swales

Designer:

Project Date: Wednesday, June 2, 2021

Project Units: SI Units (Metric)

Notes:

## Channel Analysis: Channel West\_100y

Notes:

## Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.5000 m/m

Side Slope 2 (Z2): 3.0000 m/m

Channel Width: 1.0000 m

Longitudinal Slope: 0.0010 m/m

Manning's n: 0.0300

Flow: 0.2310 cms

## Result Parameters

Depth: 0.3050 m

Area of Flow: 0.6537 m<sup>2</sup>

Wetted Perimeter: 3.3702 m

Hydraulic Radius: 0.1940 m

Average Velocity: 0.3534 m/s

Top Width: 3.2872 m

Froude Number: 0.2529

Critical Depth: 0.1455 m

Critical Velocity: 1.0269 m/s

Critical Slope: 0.0190 m/m

Critical Top Width: 2.09 m

Calculated Max Shear Stress: 2.9893 N/m<sup>2</sup>

Calculated Avg Shear Stress: 1.9013 N/m<sup>2</sup>

## Channel Analysis: Channel West\_10y

Notes:

### Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.5000 m/m

Side Slope 2 (Z2): 3.0000 m/m

Channel Width: 1.0000 m

Longitudinal Slope: 0.0010 m/m

Manning's n: 0.0300

Flow: 0.1140 cms

### Result Parameters

Depth: 0.2158 m

Area of Flow: 0.3904 m<sup>2</sup>

Wetted Perimeter: 2.6770 m

Hydraulic Radius: 0.1458 m

Average Velocity: 0.2920 m/s

Top Width: 2.6183 m

Froude Number: 0.2414

Critical Depth: 0.0967 m

Critical Velocity: 0.8655 m/s

Critical Slope: 0.0212 m/m

Critical Top Width: 1.73 m

Calculated Max Shear Stress: 2.1151 N/m<sup>2</sup>

Calculated Avg Shear Stress: 1.4294 N/m<sup>2</sup>

## Channel Analysis: Channel East\_100y

Notes:

### Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 3.0000 m/m

Side Slope 2 (Z2): 3.0000 m/m

Channel Width: 1.0000 m

Longitudinal Slope: 0.0010 m/m

Manning's n: 0.0300

Flow: 0.4000 cms

### Result Parameters

Depth: 0.4165 m

Area of Flow: 0.9368 m<sup>2</sup>

Wetted Perimeter: 3.6340 m

Hydraulic Radius: 0.2578 m

Average Velocity: 0.4270 m/s

Top Width: 3.4988 m

Froude Number: 0.2634

Critical Depth: 0.2052 m

Critical Velocity: 1.2066 m/s

Critical Slope: 0.0173 m/m

Critical Top Width: 2.23 m

Calculated Max Shear Stress: 4.0823 N/m<sup>2</sup>

Calculated Avg Shear Stress: 2.5269 N/m<sup>2</sup>

## Channel Analysis: Channel East\_10y

Notes:

### Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 3.0000 m/m

Side Slope 2 (Z2): 3.0000 m/m

Channel Width: 1.0000 m

Longitudinal Slope: 0.0010 m/m

Manning's n: 0.0300

Flow: 0.2010 cms

### Result Parameters

Depth: 0.2984 m

Area of Flow: 0.5656 m<sup>2</sup>

Wetted Perimeter: 2.8874 m

Hydraulic Radius: 0.1959 m

Average Velocity: 0.3554 m/s

Top Width: 2.7906 m

Froude Number: 0.2520

Critical Depth: 0.1386 m

Critical Velocity: 1.0247 m/s

Critical Slope: 0.0192 m/m

Critical Top Width: 1.83 m

Calculated Max Shear Stress: 2.9253 N/m<sup>2</sup>

Calculated Avg Shear Stress: 1.9201 N/m<sup>2</sup>

## Channel Analysis: Channel West\_B\_100y

Notes:

### Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.5000 m/m

Side Slope 2 (Z2): 3.0000 m/m

Channel Width: 1.0000 m

Longitudinal Slope: 0.0010 m/m

Manning's n: 0.0300

Flow: 0.2750 cms

### Result Parameters

Depth: 0.3314 m

Area of Flow: 0.7433 m<sup>2</sup>

Wetted Perimeter: 3.5758 m

Hydraulic Radius: 0.2079 m

Average Velocity: 0.3700 m/s

Top Width: 3.4856 m

Froude Number: 0.2557

Critical Depth: 0.1605 m

Critical Velocity: 1.0695 m/s

Critical Slope: 0.0185 m/m

Critical Top Width: 2.20 m

Calculated Max Shear Stress: 3.2486 N/m<sup>2</sup>

Calculated Avg Shear Stress: 2.0376 N/m<sup>2</sup>

## Channel Analysis: Channel West\_B\_10y

Notes:

### Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.5000 m/m

Side Slope 2 (Z2): 3.0000 m/m

Channel Width: 1.0000 m

Longitudinal Slope: 0.0010 m/m

Manning's n: 0.0300

Flow: 0.1390 cms

### Result Parameters

Depth: 0.2382 m

Area of Flow: 0.4511 m<sup>2</sup>

Wetted Perimeter: 2.8516 m

Hydraulic Radius: 0.1582 m

Average Velocity: 0.3081 m/s

Top Width: 2.7868 m

Froude Number: 0.2445

Critical Depth: 0.1086 m

Critical Velocity: 0.9091 m/s

Critical Slope: 0.0206 m/m

Critical Top Width: 1.81 m

Calculated Max Shear Stress: 2.3353 N/m<sup>2</sup>

Calculated Avg Shear Stress: 1.5506 N/m<sup>2</sup>





PROJECT NAME: Warehouse Development  
CIMA+ PROJECT NUMBER: A001083  
CLIENT: Fastfrate (Ottawa) Holdings Inc.  
PROJECT STATUS: 90 % Design (Site Plan Approval)

July 25, 2021

### HYDRAULIC CALCULATIONS FOR STORM SEWERS

#### APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012
2. City of Ottawa Technical Bulletins up to and including ISTB-2018-01

#### DESIGN BASIS:

Manning Coefficient : 0.013  
Maximum permitted velocity : 3.00 m/s  
Minimum permitted velocity : 0.80 m/s

Section	Dia. mm	Length m	Slope %	Invert upstream m	Invert downstream m	Capacity (full) m³/s	Velocity (full) m/s	Flow m³/s	Velocity (actual) m/s	% Full
Building Service Connection / STM 1	600	29.3	1.00%	89.750	89.460	0.614	2.17	0.213	1.96	35%
STM 2	600	21.9	0.50%	89.430	89.320	0.435	1.54	0.283	1.64	65%
STM 3	600	13.2	0.50%	87.765	87.700	0.435	1.54	0.283	1.64	65%
Outlet				87.700						

#### Remarks

The data in green has been calculated or modified by the designer  
The data in blue has been calculated using formulas inserted by the designer

#### Notes :

1. Storm Sewer Peak Flow Determined per Roof Restricted flow of 213 L/s; and uncontrolled flow from Catchments A4 of 35.792 L/s and from Catchment A5 of 34.458 L/s.

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: 2021-07-25

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 1E+08

Date: 2021-07-25

# D

## Appendix D - Potable Water & Fire Protection Calculations



**PROJECT NAME:** Fastfrate Warehouse Development  
**CIMA+ PROJECT NUMBER:** A001083  
**CLIENT:** Fastfrate (Ottawa) Holdings Inc.  
**PROJECT STATUS:** 90 % Design (Site Plan Approval)

## WATER CONSUMPTION CALCULATIONS

### APPLICABLE DESIGN GUIDELINES:

1. Ottawa Design Guidelines - Water Distribution (2010)
2. City of Ottawa Technical Bulletin ISTB-2018-02, ISDTB-2014-02 and ISD-2010-02
3. MOE Design Guidelines for Drinking-Water Systems

### RESIDENTIAL AND COMMERCIAL WATER DEMANDS:

#### RESIDENTIAL DESIGN CRITERIA:

Residential Average Day Demand: 350 L/c/day  
Maximum Day Peaking Factor: 3.9 x Average Daily Demand  
Maximum (Peak Hour) Peaking Factor: 5.8 x Average Daily Demand

Per Unit Populations:

Table 4.1 Per Unit Populations

Unit Type	Persons Per Unit
Single Family	3.4
Semi-detached	2.7
Duplex	2.3
Townhouse (row)	2.7
<b>Apartments:</b>	
Bachelor	1.4
1 Bedroom	1.4
2 Bedroom	2.1
3 Bedroom	3.1
Average Apt.	1.8

#### EQUIVALENT POPULATION :

Unit Type	Number of Units	Persons Per Unit	Population
Studio Apartments	0	1.4	0
1 Bedroom Apartments	0	1.4	0
1 Bedroom + Den Apartments	0	1.4	0
2 Bedroom Apartments	0	2.1	0
<b>Total</b>	<b>0</b>		<b>0</b>

#### COMMERCIAL DESIGN CRITERIA:

Contributing Commercial Area: 0.711 gross ha (including amenity areas, party room and gym)  
Commercial Average Day Demand: 28,000 L/gross ha/d  
Maximum Day Peaking Factor: 1.5 x Average Daily Demand  
Maximum (Peak Hour) Peaking Factor: 1.8 x Maximum Daily Demand

#### WATER DEMANDS:

Demand Type	Average Daily Demand (L/s)	Maximum Daily Demand (L/s)	Maximum (Peak) Hour Demand (L/s)
Residential	0.00	0.00	0.00
Commercial	0.23	0.35	0.62
<b>Total</b>	<b>0.23</b>	<b>0.35</b>	<b>0.62</b>

### NOTES:

1. Maximum Day and Maximum Hour residential peaking factors determined using Table 3-3 of the MOE Design Guidelines for Drinking-Water System for 0 to 500 persons.
2. Given basic day demand greater than 50 m3/day (0.57 L/s), two connections, separated by an isolation valve required. Furthermore given location on corner lot, City will not support the addition of an isolation valve on the main line, thus one connection to Richmond Rd and one connection to Roosevelt Ave. required.

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Date: 2021-07-26

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO# 100173201

Date: 2021-07-26



PROJECT NAME: Fastfrate Warehouse Development

CIMA+ PROJECT NUMBER: A001083

CLIENT: Fastfrate (Ottawa) Holdings Inc.

PROJECT STATUS: 90 % Design (Site Plan Approval)

## FIRE FLOW ASSESSMENT

### APPLICABLE DESIGN GUIDELINES:

1. Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection, 1999
2. Ottawa Design Guidelines - Water Distribution (2010) including Appendix H per ISTB-2018-02
3. City of Ottawa Technical Bulletin ISTB-2018-02
4. MOE Design Guidelines for Drinking-Water Systems

### STEP A - DETERMINE THE TYPE OF CONSTRUCTION

Type of Construction	Coefficient (C)	Value Selected (C)
Fire-resistive Construction (> 3 hours)	0.6	0.6
Non-combustible Construction	0.8	
Ordinary Construction	1	
Wood Frame Construction	1.5	

### STEP B - DETERMINE THE FLOOR AREA

Floor/Level	Floor Area Per Level (sq. ft.)	Floor Area Per Level (m2)	Fire Resistive Building	Protected Openings (one hour rating)	Area of Structure Considered (m2)
Gross Floor Area (GFA) Ground Level:	92,376	8,582	YES	YES	8,582
<b>TOTAL FLOOR AREA (A):</b>	<b>92,376</b>	<b>8,582</b>			<b>8,582</b>

### STEP C - DETERMINE THE HEIGHT IN STOREYS

Floor/Level	Number of Storeys	Percent of Floor Area Considered
Ground Level:	1	100%
<b>HEIGHT IN STOREYS:</b>	<b>1</b>	

### STEP D - DETERMINE BASE FIRE FLOW (ROUND TO NEAREST 1,000 L/min)

$$F = 220C\sqrt{A}$$

Where:

F is the required fire flow in L/min

C is the coefficient related to the type of construction, and;

A is the total floor area of the building in m<sup>2</sup>

Coefficient Related to Type of Construction (C) = 0.6  
Floor Area Considered (A) = 8,582 m<sup>2</sup>

**REQUIRED (BASE) FIRE FLOW (F) = 12000 L/min (Rounded to Nearest 1,000 L/min)**

### STEP E - DETERMINE THE INCREASE OR DECREASE FOR OCCUPANCY AND APPLY TO STEP D (STEP D x STEP E, DO NOT ROUND)

Occupancy Class	Occupancy Factor	Value Selected (C)
Non-combustible	0.75	1.00
Limited combustible	0.85	
<b>Combustible</b>	<b>1.00</b>	
Free burning	1.15	
Rapid burning	1.25	

**REQUIRED (BASE) FIRE FLOW (F) = 12000 L/min (Not rounded)**



PROJECT NAME: Fastfrate Warehouse Development

CIMA+ PROJECT NUMBER: A001083

CLIENT: Fastfrate (Ottawa) Holdings Inc.

PROJECT STATUS: 90 % Design (Site Plan Approval)

#### FIRE FLOW ASSESSMENT

##### STEP F - DETERMINE THE DECREASE, IF ANY, FOR AUTOMATIC SPRINKLER PROTECTION AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

Sprinkler System Design	Sprinkler Design Charge	Value Selected (C)	Total Charge
Automatic sprinkler system conforming to NFPA standards	-30%	Yes	-30%
Standard water supply	-10%	No	0%
Fully supervised system	-10%	No	0%
<b>TOTAL CHARGE FOR SPRINKLER SYSTEM</b>			<b>-30%</b>

DECREASE FOR SPRINKLER PROTECTION = **-3600** L/min (Not rounded)

##### STEP G - DETERMINE THE TOTAL INCREASE FOR EXPOSURES AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

Façade	Separation Distance (m)	Length-height Factor of Exposed Wall (m-storeys)	Assumed Construction of Exposed Wall of Adjacent	Total Charge
North Façade	>45	N/A	N/A	0%
East Façade (fire/party wall)	>45	N/A	N/A	0%
South Façade	>45	N/A	N/A	0%
West Façade	>45	N/A	N/A	0%
<b>TOTAL CHARGE FOR EXPOSURES</b>				<b>0%</b>

INCREASE FOR EXPOSURES = **0** L/min (Not rounded)

##### STEP H - DETERMINE FIRE FLOW INCLUDING ALL INCREASES AND REDUCTIONS ((STEP E + STEP F + STEP G, ROUND TO NEAREST 1,000 L/min)

TOTAL REQUIRED FIRE FLOW (RFF) = **8000** L/min (Rounded to Nearest 1,000 L/min)

**133.3333333** L/s

**2113** USGPM



**PROJECT NAME:** Fastfrate Warehouse Development

**CIMA+ PROJECT NUMBER:** A001083

**CLIENT:** Fastfrate (Ottawa) Holdings Inc.

**PROJECT STATUS:** 90 % Design (Site Plan Approval)

## FIRE FLOW ASSESSMENT

### NOTES/COMMENTS:

#### STEP A - DETERMINE THE TYPE OF CONSTRUCTION

1. No notes or comments

#### STEP B - DETERMINE THE FLOOR AREA

1. Assumed vertical openings and exterior vertical communications are properly protected (one hour rating), thus only the area of the largest floor plus 25% of each of the two immediately adjoining floors accounted for per Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection, 1999
2. Per the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection, 1999, Note E: Fire Walls - In determining floor areas, a fire wall that meets or exceeds the requirements of the current edition of the National Building Code of Canada (provided this necessitates a fire resistance rating of 2 or more hours) may be deemed to subdivide the building into more than one area or may, as a party wall, separate the building from an adjoining building. It is assumed that the party wall to the east will have a fire-resistance rating of at least two hours.

#### STEP C - DETERMINE THE HEIGHT IN STOREYS

1. Two levels of underground parking not considered as they are at least 50% below grade (note F of Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection, 1999)

#### STEP D - DETERMINE BASE FIRE FLOW (ROUND TO NEAREST 1,000 L/min)

1. No notes or comments.

#### STEP E - DETERMINE THE INCREASE OR DECREASE FOR OCCUPANCY AND APPLY TO STEP D (STEP D x STEP E, DO NOT ROUND)

1. Occupancy selected assuming commercial establishment will fall under C-3 occupancy type.

#### STEP F - DETERMINE THE DECREASE, IF ANY, FOR AUTOMATIC SPRINKLER PROTECTION AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

1. Assumes sprinkler system will not be fully supervised.

#### STEP G - DETERMINE THE TOTAL INCREASE FOR EXPOSURES AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

1. Assumes adjoining wall to east is an unpierced party wall considered to form a boundary when determining floor areas warranting a 10% exposure charge per Note E of the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection, 1999

#### STEP H - DETERMINE FIRE FLOW INCLUDING ALL INCREASES AND REDUCTIONS ((STEP E + STEP F + STEP G, ROUND TO NEAREST 1,000 L/min)

1. No notes or comments.

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PEO# 100200100

Date: 2020-07-26

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO# 100067842

Date: 2020-07-26



PROJECT NAME: Warehouse Development  
CIMA+ PROJECT NUMBER: A001083  
CLIENT: Fastrate (Ottawa) Holdings Inc.  
PROJECT STATUS: 90 % Design (Site Plan Approval)

July 25, 2021

### HYDRAULIC CALCULATIONS FOR GRAVITY FIRE PROTECTION WATERMAIN

#### APPLICABLE DESIGN GUIDELINES:

NFPA 13

#### DESIGN BASIS:

Manning Coefficient : 0.013  
Maximum permitted velocity : 3.00 m/s  
Minimum permitted velocity : 0.60 m/s

Section	Dia. mm	Length m	Slope %	Invert upstream m	Invert downstream m	Capacity (full) m <sup>3</sup> /s	Velocity (full) m/s	Flow m <sup>3</sup> /s	Velocity (actual) m/s	% Full	F.S.
Fire Protection WM	300	60.1	0.10%	86.485	86.425	0.030	0.43	0.015800	0.43	53%	1.90

#### Remarks

The data in green has been calculated or modified by the designer  
The data in blue has been calculated using formulas inserted by the designer

#### Notes :

1. Slope of 3.00% has been assumed for all building connections.

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
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Date: 2021-07-25

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: 2021-07-25





**PROJECT NAME:** Fastfrate (Ottawa) Warehouse Development  
**NUMBER:** A001083  
**CLIENT:** Fastfrate (Ottawa) Holdings Inc.  
**PROJECT STATUS:** 90 % Design (Site Plan Approval)

$$AFDD = \sum_{day=1}^n FDD_{day}$$

AFDD 785 °C.day

$$Thickness (cm) = \alpha \sqrt{AFDD}$$

$\alpha$	2.4
T (cm)	67.24 cm
T (ft)	2.21 ft
T (ft, in)	2'3"

$\alpha$	1.7
T (cm)	47.63 cm
T (ft)	1.56 ft
T (ft, in)	1'7"

$\alpha$	2.7
T (cm)	75.65 cm
T (ft)	2.48 ft
T (ft, in)	2'6"

Only temperatures from winter (Dec 21 – March 21) are used for calculation.

Freezing Degree Days (FDD) are computed with this simple formula:

$$FDD = 0^{\circ}\text{C} - T_{(\text{daily mean})}$$

AFDD is the sum of daily FDD over the season

— used to estimate river ice thickness

$$Thickness (cm) = \alpha \sqrt{AFDD}$$

Ice Cover Condition	$\alpha$
Windy lake, no snow	2.7
Average lake with snow	1.7-2.4
Average river with snow	0.4-0.5
Sheltered small river	0.7-1.4

Prepared by Jaymeson Adams, EIT Date: 2020-11-25

Verified by: Christian Lavoie-Lebel, P.Eng. Date: 2020-11-25

# E

## Appendix E - Septic System Detailed Calculations

Project:	Fastfrate Warehouse
Task:	Saniatry Sewage Flows per OBC
Project Number:	A0001083
Created By:	Kayla Schmidt
Date:	19-Jul-21

Notes: Hazen Williams was used to calculate the TDH. There are 6 pumps total (2 for the Pumping Chamber, 2 for the Level IV treatment, and 1 for the recycle line).

**Table 1: Dosing Criteria**

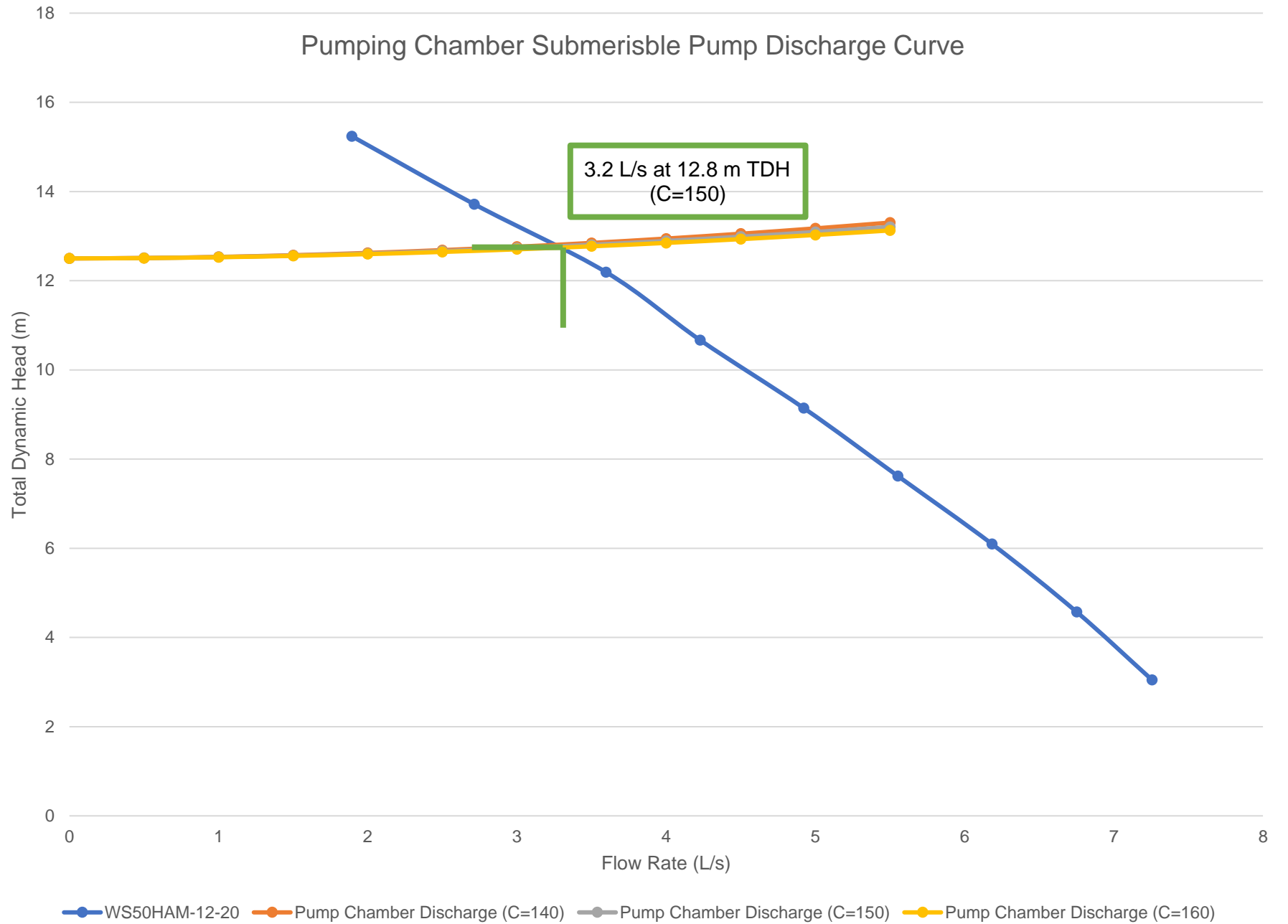
Parameter	Value	Unit
Daily Design Flow Rate	12,800	L/d
Required Dosing per day	24	times
Time for each dosing	15	minutes
Hourly Design Flow Rate	533.3	L/hr
Design Flow Rate	8.9	L/min
Design Flow Rate	0.15	L/s
Assumed Pump Chamber Volume	17,578	L
Where a pump or siphon is required, the pump or siphon shall be designed to discharge a dose of at least 75% of the internal volume of the <i>distribution pipe</i> within a time period not exceeding fifteen minutes.		

Table 2: Dosing Requirements			
Parameter	Value	Unit	Notes
Length of Each Distribution Pipe	25	m	
Number of Distribution Pipes	7		
Total Length	175	m	
Diameter	0.025	m	
Cross Sectional Area	0.000490874	m <sup>2</sup>	
Total Volume of Distribution Pipe	0.085902924	m <sup>3</sup>	
Total Volume of Distribution Pipe	85.90	L	
75% of Volume of Distribution Pipe	64.43	L	
Max time	15	minutes	
Flow Rate Required	4.30	L/min	
Flow Rate Required	0.07	L/s	
Daily Volume for Flow Rate	2061.67	L/d	
Minimum Required Flow Rate per hour	533.33	L/hr	
Flow Rate require for 15 minute time frame	35.56	L/min (per 15 minutes)	
Flow Rate require for 15 minute time frame	0.59	L/s (per 15 minutes)	
Check	12800	L/d	
Pump Design Flow Rate	1	L/s	
Daily Flow Rate	21600	L/d	

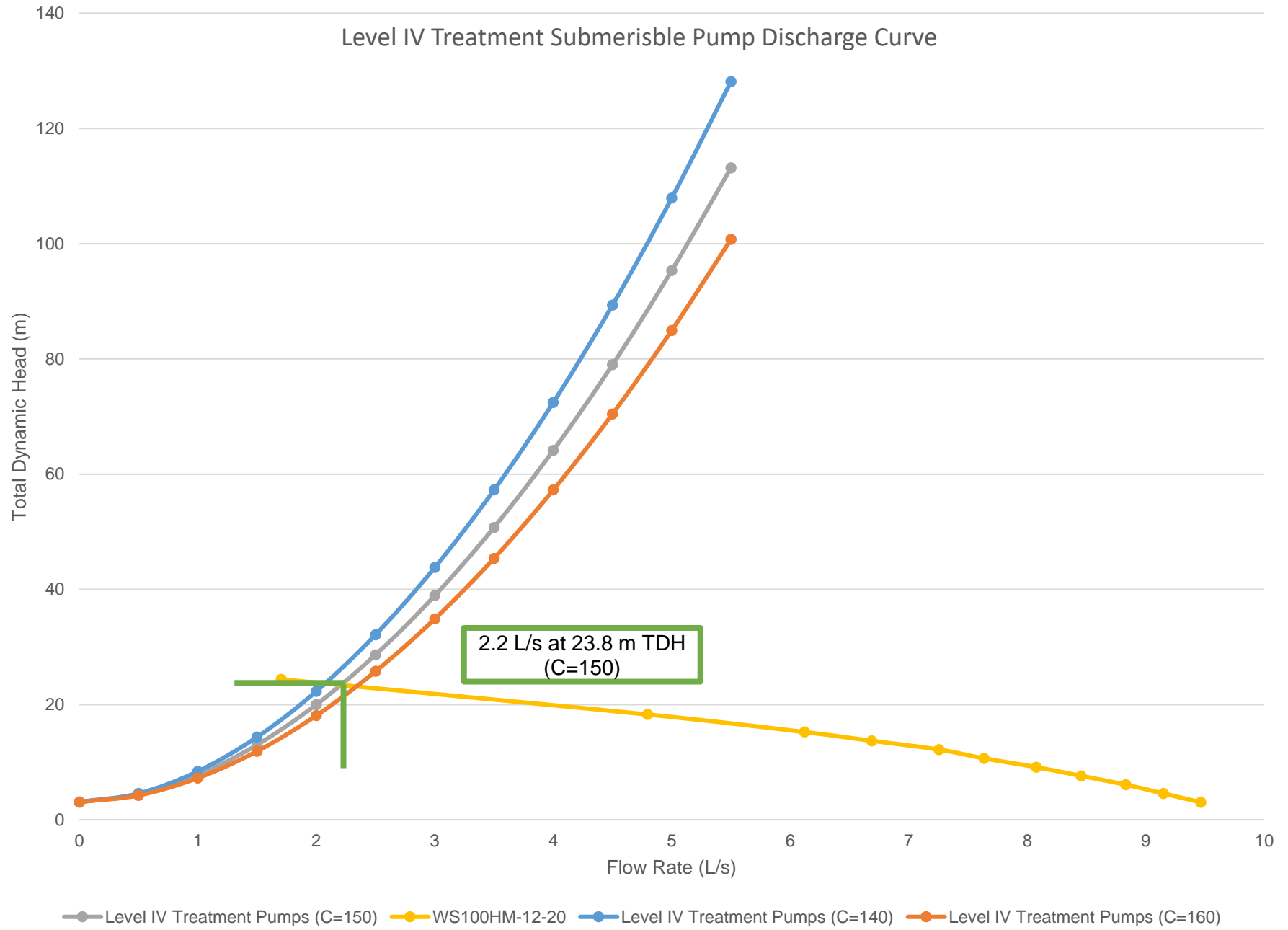


Recycle Line Pump (from Level IV Treatment to Upstream of the Septic System)															
Parameter	Value	Unit	Notes	Flow		Velocity m/s	Fitting Loss (K*V^2/2*g) m	Pipe Friction Losses Friction Coefficient (C) in m			Static Head m	Pressure to be dosed m	Total Dynamic Head Loss (m)		
				L/s	m3/s			140	150	160			140	150	160
Low Water Level	86.712	mASL													
Top of Pipe	89.212	mASL		0	0.0000	0.0E+00	0.0E+00	0.00	0.00	0.00	2.5	0.6	3.10	3.10	3.10
Static Head	2.5	m		0.5	0.0005	9.8E-07	7.1E-13	0.03	0.00	0.00	2.5	0.6	3.13	3.10	3.10
Pipe Diameter	0.05	m		1	0.0010	2.0E-06	2.9E-12	0.12	0.00	0.00	2.5	0.6	3.22	3.10	3.10
Pipe Area	0.001963495	m2		1.5	0.0015	2.9E-06	6.4E-12	0.26	0.00	0.00	2.5	0.6	3.36	3.10	3.10
Pipe Length	18	m		2	0.0020	3.9E-06	1.1E-11	0.44	0.00	0.00	2.5	0.6	3.54	3.10	3.10
Pressure at end	0.6	m		2.5	0.0025	4.9E-06	1.8E-11	0.67	0.00	0.00	2.5	0.6	3.77	3.10	3.10
				3	0.0030	5.9E-06	2.6E-11	0.94	0.00	0.00	2.5	0.6	4.04	3.10	3.10
Fittings	K Value	Qty	Total	3.5	0.0035	6.9E-06	3.5E-11	1.25	0.00	0.00	2.5	0.6	4.35	3.10	3.10
90 degree elbows	0.81	3	2.43	4	0.0040	7.9E-06	4.6E-11	1.60	0.00	0.00	2.5	0.6	4.70	3.10	3.10
Check Valve	10.8	1	10.8	4.5	0.0045	8.8E-06	5.8E-11	1.99	0.00	0.00	2.5	0.6	5.09	3.10	3.10
Ball Valve	0.08	1	0.08	5	0.0050	9.8E-06	7.1E-11	2.42	0.00	0.00	2.5	0.6	5.52	3.10	3.10
Subtotal			13.31	5.5	0.0055	1.1E-05	8.6E-11	2.89	0.00	0.00	2.5	0.6	5.99	3.10	3.10
Safety Factor			1.2	6	0.0060	1.2E-05	1.0E-10	3.40	0.00	0.00	2.5	0.6	6.50	3.10	3.10
Total			14.51	6.5	0.0065	1.3E-05	1.2E-10	3.94	0.00	0.00	2.5	0.6	7.04	3.10	3.10
				7	0.0070	1.4E-05	1.4E-10	4.52	0.00	0.00	2.5	0.6	7.62	3.10	3.10
				7.5	0.0075	1.5E-05	1.6E-10	5.14	0.00	0.00	2.5	0.6	8.24	3.10	3.10
				8	0.0080	1.6E-05	1.8E-10	5.79	0.00	0.00	2.5	0.6	8.89	3.10	3.10
				8.5	0.0085	1.7E-05	2.1E-10	6.48	0.00	0.00	2.5	0.6	9.58	3.10	3.10

Pumping Chamber Submersible Pump Discharge Curve

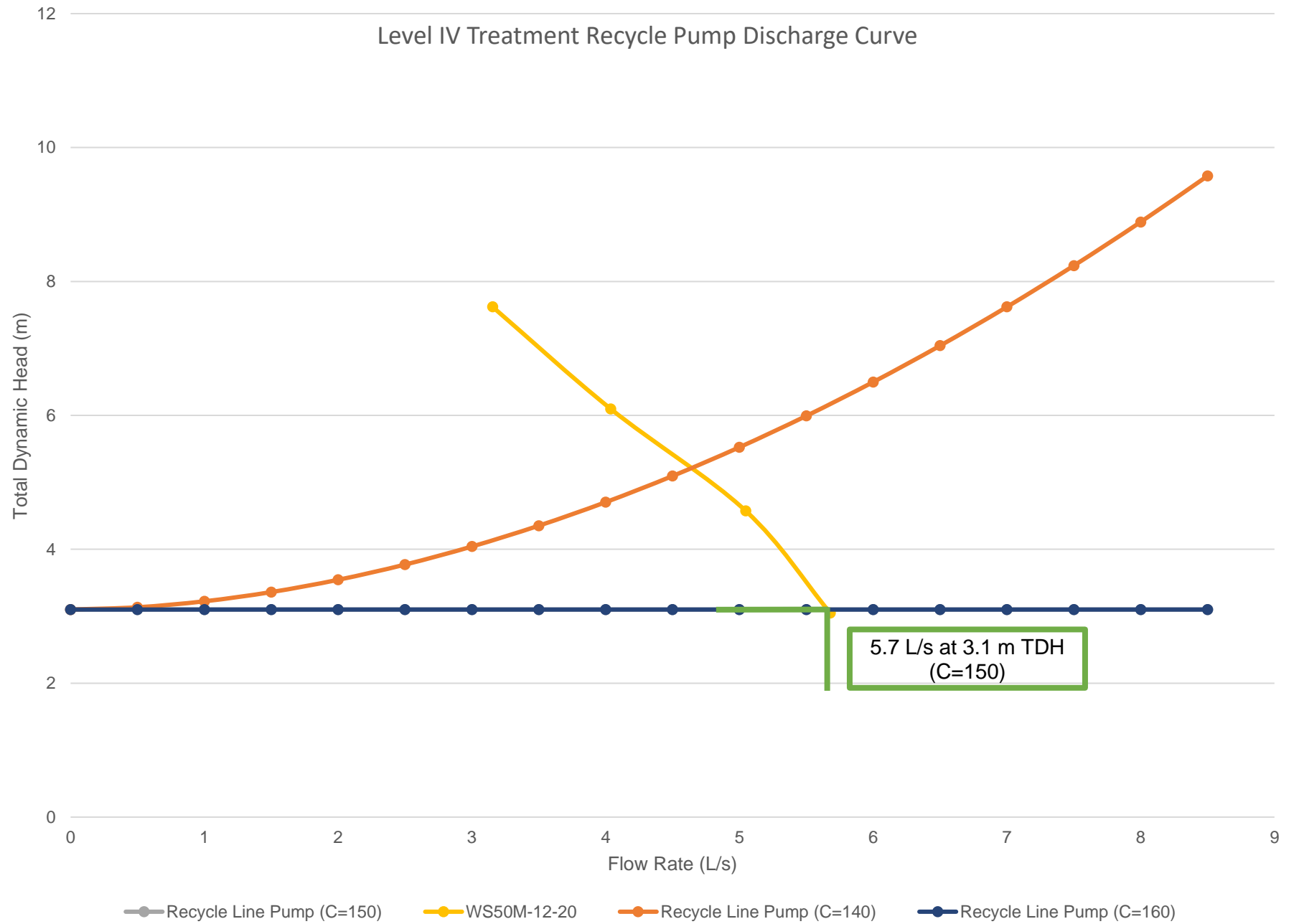


Level IV Treatment Submersible Pump Discharge Curve



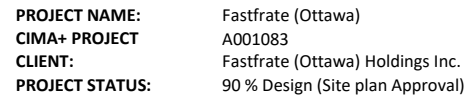


Level IV Treatment Recycle Pump Discharge Curve



# F

## Appendix F - Sanitary Servicing Calculations

**APPLICABLE DESIGN GUIDELINES:**

- DOMESTIC CONTRIBUTIONS:**

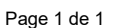
Base Flow:	2.8 L/m <sup>2</sup> /d
Peaking factor:	1.5 unitless
Extreuous Flows + Infiltration:	0.33 L/s/ha
OBC Baseflow:	12800 L/d
	0.148 L/s

<b>Commercial Peak factor:</b>	1.5 if commercial contribution >20%; otherwise use 1.0
<b>Institutional Peak factor:</b>	1.5 if institutional contribution >20%; otherwise use 1.0
<b>Industrial Peak Factor:</b>	Per Figure in Appendix 4-B

Buildings	Building Area ft <sup>2</sup>	Building Area m <sup>2</sup>	Proportional Area ha	Average Base Flow (L/s)	Peaking Factor	Peak Flow (L/s)	Extraneous Flow (L/s)	Maximum Flow (L/s)
Warehouse - Ottawa Sewer Design Guidelines	76503	7107	0.003	0.23	1.50	0.35	0.00	0.35
Warehouse - Ontario Building Code	76503	7107	0.003	0.15	1.50	0.22	0.00	0.22
Note: the value obtained from the City of Ottawa Sewer Design Guidelines for maximum flow was used since it is more conservative.								
<b>Total</b>	76503	7107				Qmax - Total (L/s) =		<b>0.35</b>

EXTRANEOUS FLOWS (Typical values for <b>Partially Separated Sewers</b> ):	
Local Street Level Analysis (less than or equal to 10 ha):	
Wet Weather Extraneous Flow:	5.0 L/s/gross ha (rare event) Annual event to be determined at design
Neighborhood Level Analysis (between 10 ha and 100 ha):	
Wet Weather Extraneous Flow:	3.0 L/s/gross ha (rare event) Annual event to be determined at design
Large Drainage area – Collector Level Analysis (greater than 100 ha):	
Wet Weather Extraneous Flow:	2.0 L/s/gross ha (rare event) Annual event to be determined at design

Date: July 20 2021





PROJECT NAME: Warehouse Development  
CIMA+ PROJECT NUMBER: A001083  
CLIENT: Fastfrate (Ottawa) Holdings Inc.  
PROJECT STATUS: 90 % Design (Site Plan Approval)

July 25, 2021

### HYDRAULIC CALCULATIONS FOR SANITARY SEWERS

#### APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012
2. City of Ottawa Technical Bulletin ISTB-2018-01

#### DESIGN BASIS:

Manning Coefficient : 0.013  
Maximum permitted velocity : 3.00 m/s  
Minimum permitted velocity : 0.60 m/s

Section	Dia. mm	Length m	Slope %	Invert upstream m	Invert downstream m	Capacity (full) m <sup>3</sup> /s	Velocity (full) m/s	Flow m <sup>3</sup> /s	Velocity (actual) m/s	% Full
Building to SAN #1	200	9.2	3.00%	89.850	89.574	0.057	1.81	0.000350	0.50	1%
SAN #1 to Septic tank	200	18.1	1.46%	89.564	89.300	0.040	1.26	0.000350	0.39	1%
Outlet				89.300						

#### Remarks

The data in green has been calculated or modified by the designer  
The data in blue has been calculated using formulas inserted by the designer

#### Notes :

1. Slope of 3.00% has been assumed for all building connections.

Prepared by: Guillaume LeBlond, M.A.Sc., EIT  
PEO No.: 100530467

Date: 2021-07-20

Verified by: Christian Lavoie-Lebel, P.Eng.  
PEO No.: 100067842

Date: 2021-07-20

# G

## Appendix G - Correspondence



**APPLICANT’S STUDY AND PLAN IDENTIFICATION LIST**

Legend: **S** indicates that the study or plan is required with application submission.  
**A** indicates that the study or plan may be required to satisfy a condition of approval/draft approval.

For information and guidance on preparing required studies and plans refer to:

<http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans>

S/A	Number of copies	ENGINEERING		S/A	Number of copies
S	5	1. Site Servicing Plan	2. Assessment of Adequacy of Servicing	S	5
S	5	3. Grade Control and Drainage Plan	4. Geotechnical Study	S	5
		5. Composite Utility Plan	6. Groundwater Impact Study		
		7. Servicing Options Report	8. Wellhead Protection Study		
S	5	9. Transportation Impact Study	10.Erosion and Sediment Control Plan	S	5
S	5	11.Storm water Management Plan	12.Hydrogeological and Terrain Analysis	S	5
		13.Hydraulic Water main Analysis	14.Noise / Vibration Study	S	5
		15.Roadway Modification Design Plan	16.Confederation Line Proximity Study		

S/A	Number of copies	PLANNING / DESIGN / SURVEY		S/A	Number of copies
		17.Draft Plan of Subdivision	18.Plan Showing Layout of Parking Garage		
		19.Draft Plan of Condominium	20.Planning Rationale	S	3
S	5	21.Site Plan ( <i>can be combined with Landscape Plan</i> )	22.Minimum Distance Separation (MDS)		
		23.Concept Plan Showing Proposed Land Uses and Landscaping	24.Agrology and Soil Capability Study		
		25.Concept Plan Showing Ultimate Use of Land	26.Cultural Heritage Impact Statement		
S	5	27.Landscape Plan ( <i>can be combined with Site Plan</i> )	28.Archaeological Resource Assessment Requirements: <b>S</b> (site plan) <b>A</b> (subdivision, condo)		
S	3	29.Survey Plan	30.Shadow Analysis		
S	5	31.Architectural Building Elevation Drawings (dimensioned) - Concept	32.Design Brief (*should be a part of the Planning Rationale)	S	*
		33.Wind Analysis			

S/A	Number of copies	ENVIRONMENTAL		S/A	Number of copies
		34.Phase 1 Environmental Site Assessment	35.Impact Assessment of Adjacent Waste Disposal/Former Landfill Site		
		36.Phase 2 Environmental Site Assessment	37.Assessment of Landform Features		
		38.Record of Site Condition	39.Mineral Resource Impact Assessment		
S	3	40.Tree Conservation Report ( <i>Include in EIS</i> )	41.Environmental Impact Statement ( <i>please contact the SNC</i> )	S	3
		42.Mine Hazard Study / Abandoned Pit or Quarry Study	43.Integrated Environmental Review (Draft, as part of Planning Rationale)		

Meeting Date: December 17, 2020  
 File Lead (Assigned Planner): Krishon Walker  
 Site Address (Municipal Address): 301 Somme Street

Application Type: Site Plan Control, Complex  
 Infrastructure Approvals Project Manager: Harry Alvey  
 \*Preliminary Assessment: 1 ☐ 2 ☐ 3 ☐ 4 ☐ 5 ☐

\*One (1) indicates that considerable major revisions are required before a planning application is submitted, while five (5) suggests that proposal appears to meet the City's key land use policies and guidelines. **This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.**

*It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, the Planning, Infrastructure and Economic Development Department will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the Planning, Infrastructure and Economic Development Department.*

# Pre-Application Consultation

## Site Plan Control (Complex)

301 Somme Street

<b>Applicant:</b>	Douglas Rancier, Civitas Group	<b>Owner:</b>	Rod Pierce, R. W. Tomlinson Limited
<b>Ward</b>	20 - Osgoode	<b>Councillor</b>	George Darouze
<b>Proposal Summary:</b>	Development of a 4,645.15 square metre (50,000 sq. ft.) warehouse on the western portion of the subject site, an 1,858.06 square metre (20,000 sq. ft.) cross deck that would connect to the warehouse, and a 278.71 square metre (3,000 sq. ft.) office space.		
<b>Attendees:</b>	Krishon Walker, Planner, PIEDD, City of Ottawa Harry Alvey, Infrastructure Project Manager, PIEDD, City of Ottawa		
<b>Regrets:</b>	Mike Giampa, Transportation Project Manager, PIEDD, City of Ottawa Matthew Hayley, Environmental Planner, PIEDD, City of Ottawa Michel Kearney, Project Manager, Hydrogeologist, PIEDD, City of Ottawa James Holland, Watershed Planner, South Nation Conservation Authority		

## Meeting Notes

### Planning Comments (Provided by Krishon Walker, Planner)

- As per Schedule A of the Official Plan, the site is designated Rural Employment Area. The Rural Employment Area is intended to support and encourage clustering of primarily industrial uses not suitable in the Urban Area or General Rural Area. Uses permitted in this designation includes but is not limited to new; heavy and light industrial uses, transportation uses, and warehouse and storage operations. The proposed use is consistent with the policies of the Official Plan.  
  
Development within the Rural Employment Area triggers Site Plan Control. Particular attention will be given to the physical design of the building(s) and site, including signage, buffering, landscaping and fencing.
- As per the City's Zoning By-law, the site is zoned as Rural Heavy Industrial Zone (RH).  
  
The Zoning By-law defines a warehouse as "*a building used for the storage and distribution of goods and equipment including self-storage units and mini-warehouses and may include one accessory dwelling unit for a facility manager*".  
  
Please ensure that your proposal complies with all applicable provisions of the Zoning By-law.  
  
Additionally, please ensure that the proposed parking complies with the provisions of Part 4 of the Zoning By-law. Parking areas should be screened from the street.  
  
If any aspect of the proposal does not comply with the zoning provisions of the applicable zone, a Minor Variance may be required through the Committee of Adjustment. If a Minor Variance is required, please note approval from the Committee of Adjustment would be required before a decision is made on the Site Plan Control application.
- Cash-in-Lieu of Parkland was to be collected through the Plan of Subdivision (15-94-0505) application. As the proposed site development is the same as anticipated in the subdivision agreement, we would not request any additional CIL or land at this time.

- There is a 30cm reserve along the frontage of the property. A lifting of a reserve application will also be required. The reserve was put in place during the establishment of the subdivision and, as per clause 18 of Schedule F, Section D, of the Subdivision Agreement, can only be lifted:

*'when certification of the proposed on-site well has been provided by a Professional Engineer or professional geoscientist licensed in the Province of Ontario that the well construction is in accordance with Ontario Regulation 903 and the recommendations contained in the report titled "Hydrogeological Investigation, Terrain Analysis & Impact Assessment, Proposed Industrial Subdivision" prepared by Golder Associates; Dated December 2008; Project No. 08-1122-0215 and the supporting letter "Tomlinson Industrial Subdivision – City of Ottawa File Number D07-16-15-94-0505; response to South nation Conservation Authority"; Golder Associates; Dated April 17, 2009; Project No. 08-1122-0215. This certification must be to the satisfaction of the General Manager, Planning and Growth Management.'*

- As the property is located within 500 metres of a Bedrock Resource Area, the Planning Rationale must speak to this designation and provide a discussion on how the proposal will impact (if at all) the Bedrock Resource Area.
- Please note that, as per Table 221 of the RH zone, any proposed outdoor storage is not permitted within the front yard and must be screened from the public street by an opaque screen at least 1.8 metres in height from finished grade.
- Please contact the South Nation Conservation Authority (SNC), amongst other federal and provincial departments/agencies, to identify all the necessary permits and approvals required to facilitate the development. Responsibility rests with the developer and their consultant for obtaining all external agency approvals. The address shall be in good standing with all approval agencies. Copies of confirmation of correspondence will be required by the City of Ottawa from all approval agencies that a form of assent is given. No construction shall commence until after a commence work notification is given.
- Please ensure that the Site Plan shows the full extent of the property and that a complete zoning table is provided. The Site Plan should also clearly show the dimensions of all proposed buildings, roads, radii of turns, overhead clearances, parking areas with defined parking spaces, steps, terraces, fences, walks, aisles and private approaches.
- Please show the location for snow storage on both the Site Plan and Landscape Plan. Storage shall not interfere with approved grading and drainage patterns or servicing. If snow is to be removed from the site, then please make a note of that on the Site Plan and include where the snow will be placed in the interim. Temporary snow storage areas should not conflict with utility box, landscaping, required parking, and site circulation.
- Be sure to follow the City's guide to preparing plans and studies (see [link below](#)) to ensure a high quality of your submission.

Feel free to contact Krishon Walker at [Krishon.Walker@ottawa.ca](mailto:Krishon.Walker@ottawa.ca), for follow-up questions.

### **Engineering Comments (Provided by Harry Alvey, Infrastructure Project Manager)**

- This site is part of the Hawthorne Industrial Park that was approved in 2009. A stormwater management pond was constructed as part as the development of this park. This stormwater management pond provides stormwater management for 75% of Hawthorne Industrial Park and includes the proposed development in that service area. The pond was designed to provide 70% TSS removal. The current requirement is to provide 80% TSS removal, which will require this proposed development to meet the new enhanced requirement. It is suggested that the consultant procure a copy of the stormwater management report for Hawthorne



Industrial Park for coordination. The stormwater management report was prepared by J.L. Richards & Associates Limited (J.L.R. *Project #*: JLR 20983; *City Index #*: R-2973; *City Old Tag #*: W09-04-1713) Revision date May 2009.

- The site appears to cover two adjacent drainage areas. There should be a comprehensive discussion of how the SWM will be handled in each of the drainage areas.
- Provide Pre- and Post-Drainage Area Maps with Pre- based on existing site conditions.
- The conceptual plan provided indicated there would possibly be several stormwater management ponds provided on site. These stormwater management facilities could be used to achieve the required 80% TSS removal now required. During the pre-consultation meeting, the design team indicated that the ponds along with underground water tanks will be needed to provide the required fire protection and sprinkler system for the proposed warehouse and truck docks. Information will need to be provided during the design process discussing how both the stormwater management objectives and the fire flow conditions will be met jointly form these ponds.
- Information will need to be provided for fire siamese connections to the building for the sprinklers. These will need to be accessible from fire lanes for fire trucks.
- Provide fire flow computations based on FSU method and information on interior fire sprinkler system.
- This site has been filled with uncontrolled fill. The geotechnical report will should provide an analysis of these soils and their ability to provide adequate bearing capacity for the traffic and proposed structures on site.
- The geotechnical report will need to include a section on slope stability for the slopes along Rideau Road and Somme Street.
- Percolations tests should be provided to indicate that an appropriate infiltration rate can be achieved for the needed septic discharge. This should be provided in the hydrogeological report.
- Truck traffic maneuvers for the proposed trucks, fire trucks and garbage trucks should be modeled in AutoTurn for onsite to show there is adequate access/space for these vehicles to maneuver safely. This analysis should also show proposed location of proposed well if it is in or adjacent to the pavement.
- For onsite design of pavement provide the ESAL's expected for the site, the CBR or Mr of the subgrade soils, frost heave potential and proposed pavement design.
- The stormwater management will require a direct submission of the ECA to the MECP. The current turnaround times for these ECA applications are approximately 11 to 12 months.

Feel free to contact Harry Alvey at [Harry.Alvey@ottawa.ca](mailto:Harry.Alvey@ottawa.ca), for follow-up questions.

#### **Transportation Comments (Provided by Mike Giampa, Transportation Project Manager)**

- A Transportation Impact Assessment (TIA) is warranted, please proceed to scoping.
- The application will not be deemed complete until the submission of the draft step 1-4, including the functional draft RMA package (*if applicable*) and/or monitoring report (*if applicable*).
- Although a full review of the TIA Strategy report (*Step 4*) is not required prior to an application, it is strongly recommended.

- Right-of-way protection on Rideau is 26 metres and the sight triangle at Somme/Rideau: 5 metre x 5 metres
- A Road Noise Impact Study is required for the proposed office use.

Feel free to contact Mike Giampa at [Mike.Giampa@ottawa.ca](mailto:Mike.Giampa@ottawa.ca), for follow-up questions.

#### **Enviromental Comments (Provided by Matthew Hayley, Environmental Planner)**

- The lot was created as part of a subdivision (15-94-0505) and in 2008 a “Tree Preservation and Protection Plan, Proposed Industrial Subdivision (Excluding Orgaworld site)...” was prepared by Golder Associates; dated October 15, 2008 as part of the final approval of the subdivision. This document will need to be followed.
- The site plan will need to have a Tree Conservation Report (TCR) to implement the previously approved tree preservation and protection plan. The TCR will also need to reflect current requirements regarding butternuts and other Official Plan policies. The proposal to add parking within the wooded area will not be supported if this area is identified from preservation in the approved tree preservation and protection plan.
- Please note that a watercourse is mapped along Rideau Road and the South Nation Conservation Authority should be consulted as the proposed parking lot may be within 30 m of this mapped feature. You will need to support this location for the parking lot as per the Official Plan and the Shields Creek Subwatershed study.

Feel free to contact Matthew Hayley at [Matthew.Hayley@ottawa.ca](mailto:Matthew.Hayley@ottawa.ca), for follow-up questions.

#### **Hydrogeological Comments (Provided by Michel Kearney, Hydrogeologist)**

- A Hydrogeological and Terrain Analysis report is required, in accordance with Procedures D-5-4 and D-5-5 of the Ministry of the Environment, Conservation and Parks. This will include the siting, drilling and testing of the production well (*i.e. not just a test well*).
- It appears that there are thin soils (*defined as 2 m or less*) on the subject site. Enough test pits and boreholes are to be put down in the area of the leaching bed and in the surrounding area to assess the risk to the onsite well and any existing or future offsite wells. The report is to document the fieldwork and provide an opinion on the level of risk.
- Depending on the findings of the fieldwork, mitigation measures may be required in order to reduce the risk to the water supply. These may include a longer casing length for the well, a deeper aquifer source, an advanced (*Level 4 or beyond*) sewage treatment system and ensuring the well is upgradient from the sewage system. Discussion with the City’s technical reviewers is encouraged, as the study progresses.
- The well must be located in a landscaped area, away from traffic and potential sources of contamination, a minimum distance of 3 m from property lines and buildings, as well as the minimum distance to the sewage system as prescribed in the Ontario Building Code. Grades are to be provided on the Grading Plan for the top of casing, the ground at the well and 3 m away from the well, to demonstrate drainage away from the well in accordance with the Regulation (O.Reg. 903).

Feel free to contact Michel Kearney at [Michel.Kearney@ottawa.ca](mailto:Michel.Kearney@ottawa.ca), for follow-up questions.

## **Conservation Authority Comments (Provided by James Holland, Watershed Planner, SNC)**

### **Natural Heritage**

- A watercourse flows along Rideau Road towards the Findlay Creek Municipal Drain, approximately 70m downstream. Findlay Creek is a permanent feature watercourse known to contain sensitive aquatic species.
- To prevent soil erosion and impacts to surface water, development and site alteration should be set back 30 metres from the high water mark of the watercourse, or 15 metres from the existing top of bank, whichever is greater. This is consistent with Section 4.7.3 of the City of Ottawa's Official Plan and Section 69 of the Zoning By-law.
- For any development within the setback area, an EIS should be completed demonstrating that the development will have no negative impacts on the feature or its functions.

### **Stormwater Management**

- Stormwater management must conform to the design for the Hawthorn Industrial Park and meet the current standards.
- Water quality should be managed so that post-runoff equals pre runoff volumes for the 1 or 5 and the 100 year event.
- Water quality should achieve 80% TSS removal.
- The stormwater design should include, at a minimum, a grading and drainage plan, sediment and erosion control plan and a supporting report with calculations demonstrating how the standards have been met.

### **Conservation Authority Regulations**

- Any interference with a watercourse, including a roadside ditch, may require a permit under O. Regulation 170/06, and restrictions may apply.

### **Private Servicing**

- The applicant should contact the Ottawa Septic Service Office for input on the design of private servicing.

Feel free to contact Planner, James Holland, at [jholland@nation.on.ca](mailto:jholland@nation.on.ca), for follow-up questions.

### Application Submission Information

Applications Type: **Site Plan Control, Complex.**

Application processing timeline generally depends on the quality of the submission. For more information on standard processing timelines, please visit: <https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/development-application-forms#site-plan-control>

Prior to submitting a formal application, it is recommended that you pre-consult with the Ward Councillor.

For information on application fees, please visit: <https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/fees-and-funding-programs/development-application-fees>

To request City of Ottawa plan(s) or report information please contact the City of Ottawa Information Centre: [InformationCentre@ottawa.ca](mailto:InformationCentre@ottawa.ca) or (613) 580-2424 ext. 44455

### Application Submission Requirements

For information on the preparation of Studies and Plans and the City's requirements, please visit: <https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans>

**Please provide hard copies and electronic copy (PDF) of all plans and studies required.**

**All plans and drawings must be produced on A1-sized paper and folded to 21.6 cm x 27.9 cm (8½"x 11").**

**Note that many of the plans and studies collected with this application must be signed, sealed and dated by a qualified engineer, architect, surveyor, planner or designated specialist.**

## Julien Sauvé

---

**From:** Julien Sauvé  
**Sent:** Wednesday, May 19, 2021 9:19 AM  
**To:** Alvey, Harry; Brown, Adam  
**Cc:** Christian Lavoie-Lebel; Tim Kennedy  
**Subject:** 301 Somme Street. Fastfrate Meeting Minutes

Hi Harry,

Thanks a lot again for meeting with us. The following is a brief summary of our discussion.

**Date of Meeting:** May 18, 2021  
**Attendees:** Harry Alvey – City of Ottawa  
Adam Brown – City of Ottawa  
Julien Sauvé – CIMA+  
Tim Kennedy – CIMA+

**Notes:**

1. City will look to see if it can provide to CIMA+ a copy of the Appendices for the SWM Report by J.L. Richards. CIMA+ will refer to this report in the design development and append it to their report.
2. CIMA+ will refer to the SWM Report prepared by J.L. Richards for allowable release rate to the existing pond which accounts for a release of the entire site even though the site appears to cover two adjacent drainage areas. Any uncontrolled area will be accounted for in this allowable release rate. Pre and post development drainage maps would no longer be applicable in this instance.
3. CIMA+ discussed how on site pond and grassed swales would provide for quality control (80% TSS) and quantity control would be available in the existing downstream pond per J.L. Richards SWM Report.
  - a. On-site pond would also provide quantity for sprinklers and firefighting.
4. City recommended having a free standing Siamese connection closer to the Fire Route (within 3-6 m and perpendicular to adjacent parked fire truck).
5. City noted that dry Fire Hydrants need to be 3-6m from fire route and cannot be behind a parking stall.
6. CIMA+ to show Autoturn simulation for fire trucks positioned at hydrants and Siamese.
7. City provided the contact for Fire Service Allan Evans and noted he would be the best reference for questions regarding dry hydrant flow and firefighting requirements, etc.
8. City noted the retaining wall would require design by a structural engineer prior to approval. The design must include a cross section and the highest point of the wall as well as a force diagram and a load diagram as it is over 1m in height.
9. City noted that minimum slope of swale without subdrains is 0.5%. However, they are open to looking at the possibility of having low slope swale of 0.1% assuming CIMA+ can provide justification. CIMA+ to demonstrate adequate percolation (prior to and after vegetation) of water during frequent (smaller) storms and confirm it can still convey the larger storms at a reasonable velocity.
10. City noted that septic system to be design in accordance with DS55 and DS54.
11. City noted OSSO (Ottawa Septic System Office) would govern septic design where flows are less than 10 000 L, while the MECP would govern for over 10 000L. Correspondence is to be provide in the Servicing Report by CIMA+.
  - a. City confirmed OSSO operates out of RVCA's offices.
12. CIMA+ and City briefly discussed potential for Limited Commence Work Order given current long turnaround times for ECA approvals of 11-12 months. City confirmed this can be further discussed closer to the time of Site Plan Approval.
  - a. City confirmed they will not have ToR for the Industrial use ECA or the septic ECA.

Please let us know if there is anything we have missed or misrepresented in this summary.

Regards,

---

**JULIEN SAUVÉ, P.Eng.**  
Engineer / Infrastructure  
Ingénieur / Infrastructure

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## Julien Sauvé

---

**From:** James Holland <jholland@nation.on.ca>  
**Sent:** Tuesday, May 4, 2021 11:35 AM  
**To:** Julien Sauvé  
**Subject:** FW: Fastrate Site Water Quality Requirements  
**Attachments:** FW\_ South Nation Conservation Property Inquiry Letters \_ (Roll\_ 061460008029995.msg; 200608 2009 05 Hawthorne Industrial Park-SWM REPORT FEB09.pdf

**Follow Up Flag:** Follow up  
**Flag Status:** Flagged

### EXTERNAL EMAIL

Hi Julien,

Thanks for confirming with the Conservation Authority; this question has come up for every property in the subdivision. The current standard is 80% TSS removal.

The pre-constitution for the site plan focussed on the adjacent watercourse and encroachment into the 30m setback. Our review will look to confirm that the stormwater management design implements the recommendations of an environmental impact statement that addresses this issue. We have not received a study so I cannot provide any additional information.

Feel free to contact me if there are any other questions about the site plan application.

Regards,  
James

---

**From:** Julien Sauvé <[Julien.Sauve@cima.ca](mailto:Julien.Sauve@cima.ca)>  
**Sent:** May 3, 2021 3:33 PM  
**To:** Laura Crites <[lcrites@nation.on.ca](mailto:lcrites@nation.on.ca)>  
**Cc:** Christian Lavoie-Lebel <[Christian.Lavoie-Lebel@cima.ca](mailto:Christian.Lavoie-Lebel@cima.ca)>; Douglas Rancier <[drancier@civitasgroup.ca](mailto:drancier@civitasgroup.ca)>  
**Subject:** Fastrate Site Water Quality Requirements

**External email** - if you don't know or can't confirm the identity of the sender, please exercise caution and do not open links or attachments.

Hi Laura,

My name is Julien and I am working with Fastrate to help design their new facility at the intersection of Rideau road and Somme Street. Refer to attached email for previous correspondence about the subject site.



The reason we are contacting you is to get confirmation on the water quality requirements. The attached SWM report 2009 for the Hawthorne Industrial site (see attached) states that individual site will need to fulfil the normal level of protection (TSS 70% removal). Can you confirm if this requirement is still valid? Refer to section 5 p. 14 of 30.

Please advise us on the water quality requirement and let us know if you have any questions.

Regards,

---

**JULIEN SAUVÉ, P.Eng.**  
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**From:** Uzoechina Ukeje <uukeje@gwal.com>  
**Sent:** July 8, 2021 1:23 PM  
**To:** Guillaume LeBlond  
**Cc:** Christian Lavoie-Lebel; Peter Chan; Tim Kennedy; Julien Sauvé  
**Subject:** RE: [EXTERNAL]RE: A001083 - CBRE Fastfrate - Building Stormwater Management

**EXTERNAL EMAIL**

Hi Guillaume,

The architectural drawings we have on hand do not show any roof drain positions.

However, if we are to assume a horizontal roof with no adjacent walls, the **total** release rate will be **173.45L/s**.

- 1) With a 6in capacity Rain Water Leader, a total of 13 Roof drains will be required (each having a release rate of 14L/s)
- 2) With an 8in capacity Rain Water Leader, a total of 6 Roof drains will be required (each having a release rate of 30L/s)

Let me know if you have further questions.

Thank you

---

**From:** Guillaume LeBlond <[Guillaume.LeBlond@cima.ca](mailto:Guillaume.LeBlond@cima.ca)>  
**Sent:** July-08-21 11:53 AM  
**To:** Uzoechina Ukeje <[uukeje@gwal.com](mailto:uukeje@gwal.com)>  
**Cc:** Christian Lavoie-Lebel <[Christian.Lavoie-Lebel@cima.ca](mailto:Christian.Lavoie-Lebel@cima.ca)>; Peter Chan <[pchan@gwal.com](mailto:pchan@gwal.com)>; Tim Kennedy <[Tim.Kennedy@cima.ca](mailto:Tim.Kennedy@cima.ca)>; Julien Sauvé <[Julien.Sauve@cima.ca](mailto:Julien.Sauve@cima.ca)>  
**Subject:** [EXTERNAL]RE: A001083 - CBRE Fastfrate - Building Stormwater Management

Hi Uzo,

Just to clarify what I need from my last email:

I need the number of roof drains as well as the flowrate per drain .

Hope this clears up any confusion.

Thanks,

---

**GUILLAUME LEBLOND**, M.A.Sc., EIT  
EIT / Infrastructures  
EIT / Infrastructure



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Employeur  
CANADA 2019

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**From:** Guillaume LeBlond

**Sent:** July 8, 2021 10:44 AM

**To:** Uzoechina Ukeje <[uukeje@gwal.com](mailto:uukeje@gwal.com)>

**Cc:** Christian Lavoie-Lebel <[Christian.Lavoie-Lebel@cima.ca](mailto:Christian.Lavoie-Lebel@cima.ca)>; [pchan@gwal.com](mailto:pchan@gwal.com); Tim Kennedy <[Tim.Kennedy@cima.ca](mailto:Tim.Kennedy@cima.ca)>; Julien Sauvé <[Julien.Sauve@cima.ca](mailto:Julien.Sauve@cima.ca)>

**Subject:** A001083 - CBRE Fastrate - Building Stormwater Management

Good morning Uzo,

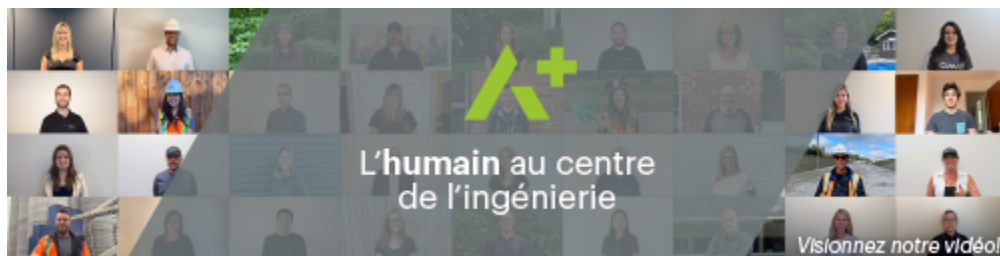
I work with Julien Sauvé and Christian Lavoie-Lebel on the Fastrate project and we are currently finalizing the stormwater management design for the site.

Could you please provide us with the release rates of the building roof drains? We are looking for both the 10 year and 100 year rainfall.

Thank you,

---

**GUILLAUME LEBLOND**, M.A.Sc., EIT  
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## Servicing Study Guidelines for Development Applications

### 4. Development Servicing Study Checklist

#### 4.1 General Content

Required Content	Reference Location
<input type="checkbox"/> Executive Summary (for larger reports only).	N/A
<input checked="" type="checkbox"/> Date and revision number of the report.	Cover Sheet
<input checked="" type="checkbox"/> Location map and plan showing municipal address, boundary, and layout of proposed development.	Report Figures, Appendix
<input checked="" type="checkbox"/> Plan showing the site and location of all existing services.	Project Drawings - Under separate cover
<input checked="" type="checkbox"/> Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Section 1.1
<input checked="" type="checkbox"/> Summary of Pre-consultation Meetings with City and other approval agencies.	Section 1.4, Appendix L
<input checked="" type="checkbox"/> Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	Section 1.3 & 4.3.2
<input checked="" type="checkbox"/> Statement of objectives and servicing criteria.	Section 1 , 2.2.1, 3.2 & 4.2
<input checked="" type="checkbox"/> Identification of existing and proposed infrastructure available in the immediate area.	Section 1.2 & Appendix B
<input type="checkbox"/> Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Section 1.1
<input type="checkbox"/> Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Project Drawings - Under separate cover
<input type="checkbox"/> Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	Geotechnical, Hydrogeological, and septic assessment - Under separate cover
<input type="checkbox"/> Proposed phasing of the development, if applicable.	N/A
<input type="checkbox"/> Reference to geotechnical studies and recommendations concerning servicing.	Section 7. References
<input type="checkbox"/> All preliminary and formal site plan submissions should have the following information: <ul style="list-style-type: none"> <li>- Metric scale;</li> <li>- North Arrow (including construction North);</li> <li>- Key Plan;</li> <li>- Name and contact information of applicant and property owner;</li> <li>- Property limits including bearings and dimensions;</li> <li>- Existing and proposed structures and parking areas;</li> <li>- Easements, road widening and rights-of-way;</li> <li>- Adjacent street names.</li> </ul>	Project Drawings - Under separate cover

#### 4.2 Development Servicing Report: Water

Required Content	Reference Location
<input type="checkbox"/> Confirm consistency with Master Servicing Study, if available	N/A
<input checked="" type="checkbox"/> Availability of public infrastructure to service proposed development	Section 1.2 & 3.1
<input checked="" type="checkbox"/> Identification of system constraints	
<input checked="" type="checkbox"/> Identify boundary conditions	Geotechnical, Hydrogeological, and septic assessment - Under separate cover
<input checked="" type="checkbox"/> Confirmation of adequate domestic supply and pressure	Section 3.2 & 3.3
<input checked="" type="checkbox"/> Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Section 3.2.2
<input type="checkbox"/> Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	N/A
<input type="checkbox"/> Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A
<input checked="" type="checkbox"/> Address reliability requirements such as appropriate location of shut-off valves	Project Drawings - Under separate cover

## Servicing Study Guidelines for Development Applications

<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification.	N/A
<input checked="" type="checkbox"/>	Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Section 3.3 & Geotechnical, Hydrogeological, and septic assessment - Under separate cover
<input type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	N/A
<input type="checkbox"/>	Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input checked="" type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Section 3.2, Appendix D
<input type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N/A

### 4.3 Development Servicing Report: Wastewater

Required Content		Reference Location
<input checked="" type="checkbox"/>	Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Section 2.2
<input type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input checked="" type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development	N/A
<input checked="" type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	N/A
<input checked="" type="checkbox"/>	Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Section 2.2 & Appendix F
<input checked="" type="checkbox"/>	Description of proposed sewer network including sewers, pumping stations, and forcemains.	Section 2.2
<input type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/>	Special considerations such as contamination, corrosive environment etc.	N/A

### 4.4 Development Servicing Report: Stormwater Checklist

Required Content		Reference Location
<input checked="" type="checkbox"/>	Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)	Section 4.1
<input checked="" type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	Section 4.1, 4.3
<input checked="" type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Appendix A, B
<input checked="" type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 4.2
<input checked="" type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 4.2
<input checked="" type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 4.3, 4.4 & Appendix C
<input type="checkbox"/>	Set-back from private sewage disposal systems.	Project Drawings - Under separate cover

## Servicing Study Guidelines for Development Applications

<input type="checkbox"/>	Watercourse and hazard lands setbacks.	Project Drawings - Under separate cover
<input checked="" type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Section 1.4 & Appendix G
<input type="checkbox"/>	Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	Section 4
<input checked="" type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).	Section 4.3 & Project Drawings - Under separate cover
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Section 4
<input checked="" type="checkbox"/>	Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 4.1 & 4.3
<input type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	Section 4.2, Appendix B
<input type="checkbox"/>	Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Project Drawings - Under separate cover
<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.	N/A
<input type="checkbox"/>	Identification of potential impacts to receiving watercourses	Section 1.3.4
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A
<input checked="" type="checkbox"/>	Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 4.3 and 4.4
<input type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Project Drawings - Under separate cover
<input type="checkbox"/>	Inclusion of hydraulic analysis including hydraulic grade line elevations.	Appendix C
<input type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Section 5
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

### 4.5 Approval and Permit Requirements: Checklist

Required Content	Reference Location
<input type="checkbox"/> Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/A
<input type="checkbox"/> Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A
<input type="checkbox"/> Changes to Municipal Drains.	N/A
<input type="checkbox"/> Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A

### 4.6 Conclusion Checklist

Required Content	Reference Location
<input checked="" type="checkbox"/> Clearly stated conclusions and recommendations	Section 6
<input type="checkbox"/> Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	
<input type="checkbox"/> All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	