Appendix A WATER SUPPLY SERVICING

A.1 DOMESTIC WATER DEMAND ESTIMATE

105-109 Henderson Avenue - Domestic Water Demand Estimates Densities as per City Guidelines:

Phase 1

Building ID	Area (m2)	Population	Daily Rate of Demand	Avg Day Demand [1]		Max Day Demand [2]		Peak Hour Demand [2]	
				(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Residential 1		48	350	11.6	0.19	29.0	0.48	63.9	1.06
Total Site :		**48**		**11.6**	**0.19**	**29.0**	**0.48**	**63.9**	**1.06**

1 Average day water demand for residential areas equal to 350 L/cap/d

2 City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

maximum day demand rate = 2.5 x average day demand rate for residential

maximum hour demand rate = 2.2 x maximum day demand rate for residential

A.2 FIRE FLOW REQUIREMENTS PER FUS

Stantec

FUS Fire Flow Calculation Sheet

Stantec Project #: 160401351
Project Name: 105-109 Henderson Ave
Date: 5/7/2018
Fire Flow Calculation #: 1
Description: Apartment Buildings

Notes:

Step	Task	Notes						Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Ordinary Construction						1	-
2	Determine Ground Floor Area of One Unit	-						307	-
	Determine Number of Adjoining Units	-						1	-
3	Determine Height in Storeys	Does not include floors >50% below grade or open attic space						3	-
4	Determine Required Fire Flow	($F = 220 \times C \times A^{1/2}$). Round to nearest 1000 L/min						-	7000
5	Determine Occupancy Charge	Limited Combustible						-15%	5950
6	Determine Sprinkler Reduction	None						0%	0
		Non-Standard Water Supply or N/A						0%	
		Not Fully Supervised or N/A						0%	
		% Coverage of Sprinkler System						0%	
7	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	-	-
		North	0 to 3	20	3	31-60	Ordinary or Fire-Resistive with Unprotected Openings	22%	3392
		East	10.1 to 20	17.6	3	31-60	Wood Frame or Non-Combustible	13%	
		South	3.1 to 10	18.7	1	0-30	Ordinary or Fire-Resistive with Unprotected Openings	15%	
		West	20.1 to 30	17.5	2	31-60	Ordinary or Fire-Resistive with Unprotected Openings	7%	
8	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							9000
		Total Required Fire Flow in L/s							150.0
		Required Duration of Fire Flow (hrs)							2.00
		Required Volume of Fire Flow (m^3)							1080

A.3 BOUNDARY CONDITIONS

Kilborn, Kris

From: Mottalib, Abdul <Abdul.Mottalib@ottawa.ca>
Sent: Thursday, February 22, 2018 10:02 AM
To: Kilborn, Kris
Cc: McCreight, Andrew; Wu, John; Mottalib, Abdul
Subject: FW: 105-109 Henderson Avenue - Boundary Requests
Attachments: 105-109 Henderson Feb 2018.pdf

Good morning Kris,

Please see the email below as requested.

Thanks,

Abdul Mottalib, P. Eng.

From: ……
Sent: February 22, 2018 8:38 AM
To: Mottalib, Abdul <Abdul.Mottalib@ottawa.ca>
Subject: RE: 105-109 Henderson Avenue - Boundary Requests

The following are boundary conditions, HGL, for hydraulic analysis at 105-109 Henderson (zone 1W) assumed to be connected to the 203mm on Henderson (see attached PDF for location).

Minimum HGL = 106.5m

Maximum HGL = 115.5m

Max Day + FireFlow (150L/s) = 102.7m

Max Day + FireFlow (233L/s) = 97.4m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

From: Kilborn, Kris [mailto:kris.kilborn@stantec.com]
Sent: February 15, 2018 2:00 PM
To: Mottalib, Abdul <Abdul.Mottalib@ottawa.ca>
Cc: McCreight, Andrew <Andrew.McCreight@ottawa.ca>; Odam, Cameron <Cameron.Odam@stantec.com>
Subject: FW: 105-109 Henderson Avenue - Boundary Requests

Good afternoon Abdul

Stantec is working with TC united Group on their 105-109 Henderson Avenue Development.

I am looking for watermain hydraulic boundary conditions for the proposed 105-109 Henderson Avenue – site plan. We anticipate the watermain connection to the proposed site plan as shown in the attached figure. This includes the connection to the 203mm WM along Henderson Avenue - adjacent to the site.

The intended land use is a 3 storey apartment building consisting of two (two storey) 4-bedroom houses and an attached three storey building with two 3-bedroom units, one 2-bedroom unit and a bachelor unit on each floor.

Estimated domestic demands and fire flow requirements for the site are as follows, please provide the results for both fireflow scenarios:

Average Day Demand	- 0.14 L/s
Max Day Demand	- 0.35 L/s
Peak Hour Demand	- 0.78 L/s
Fire Flow Demand Scenario 1 (ordinary construction)	- 150 L/s
Fire Flow Demand Scenario 2 (wood frame)	- 233 L/s

The Fire Flow Requirement is based on 2 scenarios of how the building is built which is reflected in their respective FUS sheet. Scenario 1 is if the building complies with the characteristics of an ordinary construction classification and Scenario 2 where it is built with the characteristics of a wood frame classification.

Information and calculations for each scenario can be found in their respective FUS sheet attached to the email.

Thanks in advance,

Sincerely

Kris Kilborn
Senior Associate, Community Development,
Business Center Sector Leader (BCSL)

Direct: (613) 724-4337
Mobile: (613) 297-0571
Fax: (613) 722-2799

Stantec Consulting Ltd.
400 - 1331 Clyde Avenue
Ottawa ON K2C 3G4 CA

Stantec

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Appendix B WASTEWATER SERVICING

B.1 SANITARY SEWER DESIGN SHEET

Stantec

SUBDIVISION:

105-109 Henderson Avenue

DATE: 5/7/2018
REVISION: 1
DESIGNED BY: TR
CHECKED BY:

SANITARY SEWER DESIGN SHEET
(City of Ottawa)

FILE NUMBER: 160401351

DESIGN PARAMETERS

MAX PEAK FACTOR (RES.)=	4.0	AVG. DAILY FLOW / PERSON	280 l/p/day	MINIMUM VELOCITY	0.60 m/s
MIN PEAK FACTOR (RES.)=	2.0	COMMERCIAL	28,000 l/ha/day	MAXIMUM VELOCITY	3.00 m/s
PEAKING FACTOR (INDUSTRIAL):	2.4	INDUSTRIAL (HEAVY)	55,000 l/ha/day	MANNINGS n	0.013
PEAKING FACTOR (COMM., INST.):	1.5	INDUSTRIAL (LIGHT)	35,000 l/ha/day	BEDDING CLASS	B
PERSONS / BACHELOR APT	1.4	INSTITUTIONAL	28,000 l/ha/day	MINIMUM COVER	2.50 m
PERSONS / 2 BED APT	2.1	INFILTRATION	0.33 l/s/Ha		
PERSONS / 3 BED APT	3.1				

LOCATION			RESIDENTIAL AREA AND POPULATION									COMMERCIAL		INDUSTRIAL (L)		INDUSTRIAL (H)		INSTITUTIONAL		GREEN / UNUSED		C+I+I	INFILTRATION			TOTAL	PIPE							
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (ha)	Bachelor	UNITS 2 BED	3 BED	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	FLOW (l/s)	LENGTH (m)	DIA (mm)	MATERIAL	CLASS	SLOPE (%)	CAP. (FULL) (l/s)	CAP. V PEAK FLOW (%)	VEL. (FULL) (m/s)	VEL. (ACT.) (m/s)
BLDG	BLDG	TEE	0.065	6	4	10	48	0.065	48	4.00	0.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.065	0.065	0.02	0.64	11.8	135	PVC	SDR 28	1.00	11.5	**5.57%**	0.80	0.35
																												250							

Appendix C STORMWATER MANAGEMENT

C.1 STORM SEWER DESIGN SHEET

Stantec

105-109 Henderson Avenue

DATE: 7-May-2018
REVISION: 1
DESIGNED BY: TR
CHECKED BY:

FILE NUMBER: 1604-01351

STORM SEWER DESIGN SHEET
(City of Ottawa)

DESIGN PARAMETERS

$I = a / (t+b)^c$ (As per City of Ottawa Guidelines, 2012)

	1:2 yr	1:100 yr
a =	732.951	1735.688
b =	6.199	6.014
c =	0.810	0.820

MANNING'S n = 0.013
MINIMUM COVER: 2.00 m
TIME OF ENTRY 10 min
BEDDING CLASS = B

LOCATION			DRAINAGE AREA															PIPE SELECTION										
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (2-YEAR) (ha)	AREA (100-YEAR) (ha)	AREA (ROOF) (ha)	C (-)	ACCUM. AREA (2YR) (ha)	A x C (2-YEAR) (ha)	ACCUM. AxC (2YR) (ha)	ACCUM. AREA (100YR (ha)	A x C (100-YEAR) (ha)	ACCUM. AxC (100YR) (ha)	T of C (min)	$I2_{-YEAR}$ (mm/h)	$I_{100-YEAR}$ (mm/h)	$Q_{CONTROL}$ (NOTE 1) (L/s)	ACCUM. $Q_{CONTROL}$ (L/s)	Q_{ACT} (CIA/360) (L/s)	LENGTH (m)	PIPE WIDTH OR DIAMETEI (mm)	PIPE HEIGHT (mm)	PIPE SHAPE (-)	MATERIAL (-)	CLASS (-)	SLOPE %	Q_{CAP} (FULL) (L/s)	% FULL (-)	VEL. (FULL) (m/s)	VEL. (ACT) (m/s)	TIME OF FLOW (min)
SITE	BLDG	MAIN	0.065	0.00	0.00	0.70	0.07	0.046	0.046	0.00	0.000	0.000	10.00	76.81	178.56	0.0	0.0	9.71	11.5	200	200	CIRCULAR	PVC	-	17.10	137.7	**7.05%**	4.33	2.09	0.09
													10.09							675	675								

C.2 RATIONAL METHOD CALCULATIONS

Stormwater Management Calculations

File No: **160401351**
Project: **105-109 Henderson Avenue**
Date: **07-May-18**

SWM Approach:
Post-development to Pre-development flows

Post-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

Runoff Coefficient Table

Catchment Type	Sub-catchment Area ID / Description		Area (ha) "A"		Runoff Coefficient "C"	"A x C"		Overall Runoff Coefficient
Uncontrolled - Tributary	UNC-1	Hard	0.007		0.9	0.006		
		Soft	0.001		0.2	0.000		
		Subtotal		0.008			0.006232	0.82
Uncontrolled - Tributary	UNC-2	Hard	0.000		0.9	0.000		
		Soft	0.002		0.2	0.000		
		Subtotal		0.002			0.00035	0.20
Roof	BLDG 1	Hard	0.024		0.9	0.021		
		Soft	0.000		0.2	0.000		
		Subtotal		0.024			0.0211707	0.90
Roof	BLDG 2	Hard	0.007		0.9	0.006		
		Soft	0.000		0.2	0.000		
		Subtotal		0.007			0.005949	0.90
Controlled - Tributary	CB-1	Hard	0.008		0.9	0.008		
		Soft	0.015		0.2	0.003		
		Subtotal		0.024			0.010575	0.45
Total				**0.063**			**0.044**	
Overall Runoff Coefficient= C:								**0.70**

Total Roof Areas	**0.030 ha**
Total Tributary Surface Areas (Controlled and Uncontrolled)	**0.033 ha**
Total Tributary Area to Outlet	**0.063 ha**
Total Uncontrolled Areas (Non-Tributary)	**0.000 ha**
Total Site	**0.063 ha**

Stormwater Management Calculations

Project #160401351, 105-109 Henderson Avenue
Modified Rational Method Calculatons for Storage

2 yr Intensity
City of Ottawa

$I = a/(t + b)^c$

a =	732.951
b =	6.199
c =	0.81

t (min)	I (mm/hr)
5	103.57
10	76.81
15	61.77
20	52.03
25	45.17
30	40.04
35	36.06
40	32.86
45	30.24
50	28.04
55	26.17
60	24.56

2 YEAR Predevelopment Target Release from Portion of Site

Subdrainage Area: Predevelopment Tributary Area to Outlet
Area (ha): 0.0630
C: 0.40

Typical Time of Concentration

tc (min)	I (2 yr) (mm/hr)	Qtarget (L/s)
10	76.81	**5.38**

2 YEAR Modified Rational Method for Entire Site

Subdrainage Area: UNC-1 — Uncontrolled - Tributary
Area (ha): 0.008
C: 0.82

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)
10	76.81	1.33	1.33		
20	52.03	0.90	0.90		
30	40.04	0.69	0.69		
40	32.86	0.57	0.57		
50	28.04	0.49	0.49		
60	24.56	0.43	0.43		
70	21.91	0.38	0.38		
80	19.83	0.34	0.34		
90	18.14	0.31	0.31		
100	16.75	0.29	0.29		
110	15.57	0.27	0.27		
120	14.56	0.25	0.25		

Subdrainage Area: UNC-2 — 0.00
Area (ha): 0.002
C: 0.20

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)
10	76.81	0.07	0.07		
20	52.03	0.05	0.05		
30	40.04	0.04	0.04		
40	32.86	0.03	0.03		
50	28.04	0.03	0.03		
60	24.56	0.02	0.02		
70	21.91	0.02	0.02		
80	19.83	0.02	0.02		
90	18.14	0.02	0.02		
100	16.75	0.02	0.02		
110	15.57	0.02	0.02		
120	14.56	0.01	0.01		

Subdrainage Area: BLDG 1 — Roof
Area (ha): 0.024 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	76.81	4.52	1.48	3.04	1.82	84.9	0.00
20	52.03	3.06	1.49	1.57	**1.89**	85.9	0.00
30	40.04	2.36	1.46	0.90	1.61	81.6	0.00
40	32.86	1.93	1.42	0.51	1.22	75.7	0.00
50	28.04	1.65	1.36	0.29	0.87	65.6	0.00
60	24.56	1.45	1.30	0.15	0.53	55.6	0.00
70	21.91	1.29	1.21	0.08	0.33	48.0	0.00
80	19.83	1.17	1.11	0.06	0.28	44.0	0.00
90	18.14	1.07	1.02	0.04	0.23	40.6	0.00
100	16.75	0.99	0.95	0.03	0.20	37.7	0.00
110	15.57	0.92	0.89	0.03	0.17	35.3	0.00
120	14.56	0.86	0.84	0.02	0.14	33.2	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
2-year Water Level	85.9	0.09	1.49	1.89	9.45	OK

Subdrainage Area: BLDG 2 — Roof
Area (ha): 0.007 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	76.81	1.27	0.63	0.64	0.38	77.6	0.00
20	52.03	0.86	0.63	0.23	0.28	68.6	0.00
30	40.04	0.66	0.63	0.03	0.06	37.6	0.00
40	32.86	0.54	0.54	0.00	0.01	21.4	0.00
50	28.04	0.46	0.46	0.00	0.01	18.3	0.00
60	24.56	0.41	0.40	0.00	0.01	16.0	0.00
70	21.91	0.36	0.36	0.00	0.01	14.3	0.00
80	19.83	0.33	0.33	0.00	0.01	12.9	0.00
90	18.14	0.30	0.30	0.00	0.01	11.8	0.00
100	16.75	0.28	0.28	0.00	0.01	10.9	0.00
110	15.57	0.26	0.26	0.00	0.01	10.2	0.00
120	14.56	0.24	0.24	0.00	0.00	9.5	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
2-year Water Level	77.6	0.08	0.63	0.38	2.68	OK

Subdrainage Area: CB-1 — Controlled - Tributary
Area (ha): 0.024
C: 0.45

tc (min)	I (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)

Project #160401351, 105-109 Henderson Avenue
Modified Rational Method Calculatons for Storage

100 yr Intensity
City of Ottawa

$I = a/(t + b)^c$

a =	1735.688
b =	6.014
c =	0.820

t (min)	I (mm/hr)
5	242.70
10	178.56
15	142.89
20	119.95
25	103.85
30	91.87
35	82.58
40	75.15
45	69.05
50	63.95
55	59.62
60	55.89

100 YEAR Predevelopment Target Release from Portion of Site

Subdrainage Area: Predevelopment Tributary Area to Outlet
Area (ha): 0.0410
C: 0.40

	Qtarget	
2-Year Pre Development Discharge	5.38	L/s
Less Peak Sanitary Discharge of	0.64	L/s
Target Release Rate	**4.74**	**L/s**

100 YEAR Modified Rational Method for Entire Site

Subdrainage Area: UNC-1 — Uncontrolled - Tributary
Area (ha): 0.008
C: 0.82

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)
10	178.56	3.09	3.09		
20	119.95	2.08	2.08		
30	91.87	1.59	1.59		
40	75.15	1.30	1.30		
50	63.95	1.11	1.11		
60	55.89	0.97	0.97		
70	49.79	0.86	0.86		
80	44.99	0.78	0.78		
90	41.11	0.71	0.71		
100	37.90	0.66	0.66		
110	35.20	0.61	0.61		
120	32.89	0.57	0.57		

Subdrainage Area: UNC-2 — 0.00
Area (ha): 0.002
C: 0.20

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)
10	178.56	0.17	0.17		
20	119.95	0.12	0.12		
30	91.87	0.09	0.09		
40	75.15	0.07	0.07		
50	63.95	0.06	0.06		
60	55.89	0.05	0.05		
70	49.79	0.05	0.05		
80	44.99	0.04	0.04		
90	41.11	0.04	0.04		
100	37.90	0.04	0.04		
110	35.20	0.03	0.03		
120	32.89	0.03	0.03		

Subdrainage Area: BLDG 1 — Roof
Area (ha): 0.024 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	10.51	1.72	8.79	5.27	123.2	0.00
20	119.95	7.06	1.77	5.29	6.35	130.5	0.00
30	91.87	5.41	1.78	3.63	**6.53**	131.7	0.00
40	75.15	4.42	1.77	2.65	6.37	130.6	0.00
50	63.95	3.76	1.76	2.01	6.02	128.5	0.00
60	55.89	3.29	1.74	1.55	5.58	125.7	0.00
70	49.79	2.93	1.71	1.22	5.11	121.6	0.00
80	44.99	2.65	1.68	0.96	4.62	117.1	0.00
90	41.11	2.42	1.66	0.76	4.13	112.4	0.00
100	37.90	2.23	1.63	0.60	3.63	107.8	0.00
110	35.20	2.07	1.60	0.47	3.13	103.1	0.00
120	32.89	1.94	1.56	0.37	2.67	98.0	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	131.7	0.13	1.78	6.53	9.45	OK

Subdrainage Area: BLDG 2 — Roof
Area (ha): 0.007 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	2.95	0.63	2.32	1.39	119.7	0.00
20	119.95	1.98	0.63	1.35	1.62	126.6	0.00
30	91.87	1.52	0.63	0.89	1.60	126.0	0.00
40	75.15	1.24	0.63	0.61	1.47	122.2	0.00
50	63.95	1.06	0.63	0.43	1.28	116.0	0.00
60	55.89	0.92	0.63	0.29	1.06	108.6	0.00
70	49.79	0.82	0.63	0.19	0.81	100.4	0.00
80	44.99	0.74	0.63	0.11	0.54	86.3	0.00
90	41.11	0.68	0.63	0.05	0.26	67.5	0.00
100	37.90	0.63	0.62	0.00	0.01	24.8	0.00
110	35.20	0.58	0.58	0.00	0.01	23.0	0.00
120	32.89	0.54	0.54	0.00	0.01	21.5	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	126.6	0.13	0.63	1.62	2.68	OK

Subdrainage Area: CB-1 — Controlled - Tributary
Area (ha): 0.024
C: 0.45

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)

Stormwater Management Calculations

Project #160401351, 105-109 Henderson Avenue
Modified Rational Method Calculatons for Storage

10	76.81	4.37	1.48	2.89	1.73
20	52.03	3.65	1.48	2.16	2.60
30	40.04	3.27	1.48	1.79	3.21
40	32.86	2.93	1.48	1.44	3.47
50	28.04	2.65	1.48	1.16	**3.48**
60	24.56	2.42	1.48	0.94	3.38
70	21.91	2.22	1.48	0.73	3.08
80	19.83	2.02	1.48	0.54	2.57
90	18.14	1.86	1.48	0.37	2.01
100	16.75	1.72	1.48	0.24	1.42
110	15.57	1.61	1.48	0.12	0.80
120	14.56	1.51	1.48	0.02	0.15

Storage: Above CB

Orifice Size:	LMF 40	
Invert Elevation	67.88	m
Bottom of Tank	67.73	m
Top of Tank	69.08	m
T/G Elevation	69.58	m
Max Ponding Depth	1.02	m
Downstream W/L	65.76	m

	Stage	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Volume Check
2-year Water Level	68.90	1.20	1.48	3.48	16.50	OK

SUMMARY TO OUTLET

			Vrequired	Vavailable*	
Tributary Area	0.063	ha			
Total 2yr Flow to Sewer	1.5	L/s	3.48	16.50 m^3	Ok
Non-Tributary Area	0.008	ha			
Total 5yr Flow Uncontrolled	1.4	L/s			
Total Area	0.071	ha			
Total 2yr Flow	2.9	L/s			
Target	4.7	L/s			

Project #160401351, 105-109 Henderson Avenue
Modified Rational Method Calculatons for Storage

10	178.56	7.60	1.48	6.12	3.67
20	119.95	5.93	1.48	4.44	5.33
30	91.87	5.11	1.48	3.62	6.52
40	75.15	4.61	1.48	3.13	7.50
50	63.95	4.27	1.48	2.78	8.35
60	55.89	4.01	1.48	2.53	9.11
70	49.79	3.81	1.48	2.32	9.76
80	44.99	3.64	1.48	2.15	10.34
90	41.11	3.50	1.48	2.01	10.86
100	37.90	3.37	1.48	1.88	11.29
125	31.86	3.11	1.48	1.63	**12.23**
130	30.90	3.02	1.48	1.53	11.95

Storage: Surface Storage Above CB

Orifice Size:	LMF 40	
Inv. Elev.(orfice)	67.88	m
Bottom of Tank	67.73	m
Top of Tank	69.08	m
T/G Elevation	69.58	m
Max Ponding Depth	1.02	m
Downstream W/L	65.76	m

	Stage	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Volume Check
100-year Water Level	70.60	1.20	1.48	12.23	16.50	OK
					4.27	

SUMMARY TO OUTLET

			Vrequired	Vavailable*	
Tributary Area	0.063	ha			
Total 100yr Flow to Sewer	1.5	L/s	12.23	16.50 m^3	
Non-Tributary Area	0.008	ha			
Total 100yr Flow Uncontrolled	3.3	L/s			
Total Area	0.071	ha			
Total 100yr Flow	4.7	L/s			
Target	4.7	L/s			

Project #160401351, 105-109 Henderson Avenue
Roof Drain Design Sheet, Area BLDG
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0006	0	0.025	5	0	0	0.025
0.050	0.0006	0.0013	0	0.050	21	0	0	0.050
0.075	0.0007	0.0014	1	0.075	47	1	1	0.075
0.100	0.0008	0.0016	3	0.100	84	2	3	0.100
0.125	0.0009	0.0017	5	0.125	131	3	5	0.125
0.150	0.0009	0.0019	9	0.150	189	4	9	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.3	242.7	0.3	0.06741
1.1	585.5	0.8	0.23006
2.8	1026.2	1.6	0.51512
5.4	1538.1	2.7	0.94236
9.4	2103.3	4.0	1.52661

Rooftop Storage Summary

Total Building Area (sq.m)		236.23
Assume Available Roof Area (sq.	80%	188.984
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		9
Estimated 100 Year Drawdown Time (h)		1.1

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m)	L/s Open	75%	50%	**25%**	Closed
0.025	0.3155	0.3155	0.3155	**0.3155**	0.3155
0.050	0.6309	0.6309	0.6309	**0.6309**	0.3155
0.075	0.9464	0.8675	0.7886	**0.7098**	0.3155
0.100	1.2618	1.1041	0.9464	**0.7886**	0.3155
0.125	1.5773	1.3407	1.1041	**0.8675**	0.3155
0.150	1.8927	1.5773	1.2618	**0.9464**	0.3155

Calculation Results	**2yr**	**100yr**	**Available**
Qresult (cu.m/s)	0.001	0.002	-
Depth (m)	0.086	0.132	0.150
Volume (cu.m)	1.9	6.5	9.4
Draintime (hrs)	0.4	1.1	

Project #160401351, 105-109 Henderson Avenue
Roof Drain Design Sheet, Area BLDG
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0006	0	0.025	1	0	0	0.025
0.050	0.0003	0.0006	0	0.050	6	0	0	0.050
0.075	0.0003	0.0006	0	0.075	13	0	0	0.075
0.100	0.0003	0.0006	1	0.100	24	0	1	0.100
0.125	0.0003	0.0006	2	0.125	37	1	2	0.125
0.150	0.0003	0.0006	3	0.150	54	1	3	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.1	137.9	0.1	0.0383
0.3	374.2	0.2	0.14225
0.8	728.7	0.5	0.34467
1.5	1201.4	0.8	0.6784
2.7	1792.3	1.1	1.17626

Rooftop Storage Summary

Total Building Area (sq.m)		67.1
Assume Available Roof Area (sq.	80%	53.68
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		3
Estimated 100 Year Drawdown Time (h)		0.7

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m)	L/s Open	75%	50%	25%	**Closed**
0.025	0.3155	0.3155	0.3155	0.3155	**0.3155**
0.050	0.6309	0.6309	0.6309	0.6309	**0.3155**
0.075	0.9464	0.8675	0.7886	0.7098	**0.3155**
0.100	1.2618	1.1041	0.9464	0.7886	**0.3155**
0.125	1.5773	1.3407	1.1041	0.8675	**0.3155**
0.150	1.8927	1.5773	1.2618	0.9464	**0.3155**

Calculation Results	**2yr**	**100yr**	**Available**
Qresult (cu.m/s)	0.001	0.001	-
Depth (m)	0.078	0.127	0.150
Volume (cu.m)	0.4	1.6	2.7
Draintime (hrs)	0.2	0.7	

C.3 TANK DETAILS

16,500 LITRE PRECAST CONCRETE WATER HOLDING TANK MODEL H16.5L

WILKINSON HEAVY PRECAST LIMITED
DUNDAS, ONTARIO 1-800-263-8503
www.wilkinsonheavyprecast.com

LOW PROFILE

CONSTRUCTION DETAILS *

Concrete: 35 MPa at 28 Days, 5 to 8% Air Entrainment.

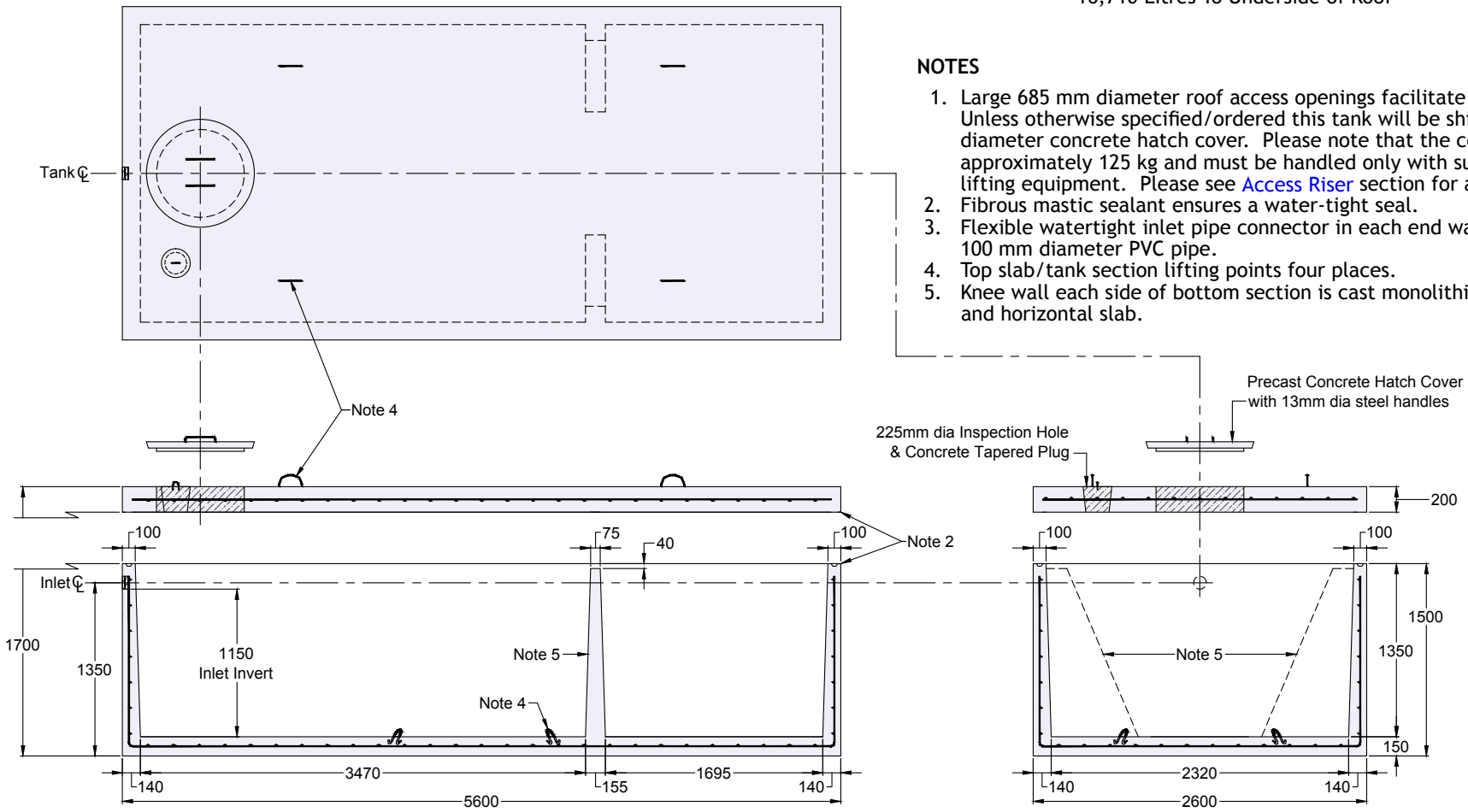
Reinforcing: 10 M bars at 200 mm centres each way in walls and floor.
10 M bars at 200 mm centres each way in top slabs.
Four extra 15 M bars around roof access opening.
Minimum cover over reinforcing steel - 25 mm.

Weight: Top Slab 7100 kg
Tank Section 11,880 kg
Total 18,880 kg

Actual Capacity: 12,378 Litres Per Vertical Metre.
16,710 Litres To Underside of Roof

NOTES

1. Large 685 mm diameter roof access openings facilitate tank maintenance. Unless otherwise specified/ordered this tank will be shipped with 840 mm diameter concrete hatch cover. Please note that the cover weighs approximately 125 kg and must be handled only with suitable mechanical lifting equipment. Please see Access Riser section for available options.
2. Fibrous mastic sealant ensures a water-tight seal.
3. Flexible watertight inlet pipe connector in each end wall accommodates 100 mm diameter PVC pipe.
4. Top slab/tank section lifting points four places.
5. Knee wall each side of bottom section is cast monolithically with the walls and horizontal slab.



Dimensions in mm
N.T.S.

*Product designed for a **Maximum 1 Metre** burial over the top slab in firm soil beneath an area of vehicular traffic.

For recommended installation procedures refer to Wilkinson Installation Guidelines.

WARNING! IMPROPER INSTALLATION ESPECIALLY IN UNSTABLE SOIL CAN RESULT IN THE STRUCTURAL FAILURE OF THIS PRODUCT

April 30th, 2014

Appendix D GEOTECHNICAL INVESTIGATION

105-109 HENDERSON ROAD GEOTECHNICAL REPORT

Project No.: CP-17-0638

Prepared for:

Daniel Boulanger

Director – Planning & Consultation
TC United
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Ottawa, ON K1G 4B8

Prepared by:

McIntosh Perry
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Carp, ON K0A 1L0

DRAFT

March 2018

McINTOSH PERRY

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APPENDICES

GEOTECHNICAL INVESTIGATION and
FOUNDATION DESIGN RECOMMENDATION REPORT
105-109 Henderson Road, Ottawa, Ontario

1.0 INTRODUCTION

This report presents the factual findings obtained from a geotechnical investigation performed at the above-mentioned site, for the proposed construction of three-story apartment building with a basement, in the neighborhood of Sandy Hill in Ottawa, Ontario. It is understood the existing residential homes will remain in place, and the proposed construction will be in the backyard of these properties. The field work was carried out on January 31, 2018 and comprised of two boreholes advanced to a maximum depth of 17.2 m below existing ground surface.

The purpose of the investigation was to explore the subsurface conditions at this site and to provide anticipated geotechnical conditions influencing the design and construction of the proposed building.

McIntosh Perry Consulting Engineers Ltd (McIntosh Perry) carried out the investigation at the request of TC United.

2.0 SITE DESCRIPTION

The property under consideration for proposed development is located at 105 and 109 Henderson Avenue in the Sandy Hill neighbourhood of Ottawa. Henderson Avenue is a southbound one-way avenue containing high density residential properties. The property to the south of 109 Henderson is a Hydro Ottawa building, with multiplexes bordering the East and North property lines of both properties. The properties have very minimal vegetation and the grade is relatively flat. 109 Henderson Avenue has a garage at the rear of the property bordering the fence line with 105 Henderson Avenue. At the south end of Henderson Avenue at Somerset Street, grade drops significantly to the South.

It is understood based on the concept plans provided, the proposed structure will be a 3-story building, with a basement. The proposed building will be surrounded with an asphalt parking lot.

Location of the property is shown on Figure 1, included in Appendix B.

3.0 FIELD PROCEDURES

Staff of McIntosh Perry Consulting Engineers (McIntosh Perry) visited the site before the drilling investigation to mark out the proposed borehole locations. Utility clearance was carried out by USL-1 on behalf of McIntosh Perry. Public and private utility authorities were informed and all utility clearance documents were obtained before the commencement of drilling work.

The equipment used for drilling was owned and operated by George Downing Estate Drilling Ltd. of Hawkesbury, Ontario. Boreholes were advanced using hollow and solid stem augers aided by track-mounted LC-55 drilling rig. Boreholes were advanced to a maximum depth of 17.2 m below the ground level. Soil samples were obtained at 0.75 m intervals of depth in boreholes using a 50 mm outside diameter split spoon sampler in accordance with the Standard Penetration Test (SPT) procedure. MTO 'N' vane tests were taken to measure in-situ shear strength of cohesive material. In boreholes BH18-1, the investigation was advanced beyond the sampled depth with Dynamic Cone Penetration Tests (DCPT) to the termination depth. Boreholes were backfilled with auger cuttings. All boreholes were restored to match the original surface. Borehole locations are shown on Figure 2, included in Appendix B.

4.0 LABORATORY TEST PROCEDURES

Laboratory testing on representative SPT samples was performed at McIntosh Perry geotechnical lab included moisture content, and Atterberg Limit Testing. Atterberg Limit test and moisture content was done on retrieved SPT samples, was tested by LRL Ltd. The laboratory tests to determine index properties were performed in accordance with CCIL test procedures, which follow American Society for Testing Materials (ASTM) test procedures.

The rest of the soil samples recovered will be stored in McIntosh Perry storage facility for a period of one month after submission of the final report. Samples will be disposed after this period of time unless otherwise requested in writing by the Client.

Laboratory tests are included in Appendix C.

5.0 SITE GEOLOGY AND SUBSURFACE CONDITIONS

5.1 Site Geology

Based on published physiography maps of the area (Ontario Geological Survey) the site is located within the Ottawa Valley Clay Plains. Surficial geology maps of southern Ontario identify the property as on older alluvial deposits.

The Ottawa Valley between Pembroke and Hawkesbury, Ontario consists of clay plains interrupted by ridges of rock or sand. It is naturally divided into two parts, above and below Ottawa, Ontario. Within the valley, the bedrock is further faulted so that some of the uplifted blocks appear above the clay beds. The sediments themselves in the valley are deep silty clay. Although the clay deposits are grey in color like the lime stones that underlies them in part, they are only mildly calcareous and likely derived from the more acidic rock of the Canadian Shield.

5.2 Subsurface Conditions

In general, the site stratigraphy consists of a topsoil, underlain by fill material, followed by a silty clay. The soils encountered at this site can be divided into two different zones.

a) Fill
b) Clay

The soils encountered during the course of the investigation, together with the field and laboratory test results are shown on the Record of Borehole sheets included in Appendix C. Description of the strata encountered are given below.

5.2.1 Fill

At the top of both boreholes a layer of topsoil was observed, the thickness of the topsoil was observed to be between 150 and 300 mm. Under the topsoil was silty sand fill, observed to have trace to some clay, and trace gravel. The fill was observed to be loose, brown and moist. SPT 'N' values were observed to be between 3 to 6 blows/300mm. The fill was observed to extend to a depth of 1.5 m.

5.2.2 Clay

The clay was observed to be stiff to firm, moist to wet and grey. Moisture content within the weathered crust was an average of 51%. Within the weathered clay crust SPT 'N' values ranged from 4 to 11 blows/300 mm, below the crust SPT 'N' values ranged from 0 to 2 blows/300 mm, with an average moisture content of 51%. Boreholes BH17-4 and BH17-5, were advanced with DCPT, values were observed to be between 0 and 16 blows/300mm. MTO N-sized vane tests were conducted which estimated the in-situ shear strength of the layer ranged from 38 kPa to 102 kPa (firm to stiff), with an average of 70kPa, and sensitivity ranging between 13 and 3, indicating non-sensitive to highly sensitive clay. Three Atterberg Limit test were conducted on representative samples and found to be clay of high-plasticity (CH). Results showed the liquid limit values range from 80% to 81% and the plastic limit range from 27% to 30%. Test results are shown on Figure 3, included in Appendix B. Moisture content of sample tested below the weathered crust for Atterberg Limits, indicate the natural moisture content of the sample is close to the liquid limit of the sample, indicating the layer is in a sensitive state. The thickness of the clay layer was observed to be 15.7 m, terminating at a depth of 17.2 m from the existing ground surface (El. 82.2 m). Bottom of the clay layer was determined to be at DCPT refusal on probable bedrock.

5.3 Groundwater

Groundwater was not observed in open boreholes. Moisture content of the clay was observed to increase at an approximate depth of 3.5-4.0 m. Groundwater level may be expected to fluctuate due to seasonal changes.

5.4 Chemical Analysis

The chemical test results conducted by Paracel Laboratories in Ottawa, Ontario, to determine the resistivity, pH, sulphate and chloride content of representative soil sample are shown in Table 5-1 below:

Table 5-1: Soil Chemical Analysis Results

Borehole	Sample	Depth / El. (m)	pH	Sulphate (%)	Chloride (%)	Resistivity (Ohm-cm)
BH18-1	SS-3	1.5-2.1	7.25	0.0016	0.0009	9,190

6.0 DISCUSSIONS AND RECOMMENDATIONS

6.1 General

This section of the report provides recommendations for the design of the proposed building behind 105 and 109 Henderson Road in Ottawa, Ontario. The recommendations are based on interpretation of the factual information obtained from the boreholes advanced during the subsurface investigation. The discussions and recommendations presented are intended to provide sufficient information to the designer of the proposed building to select the suitable type of foundation to support the structure.

The comments made on the construction are intended to highlight aspects which could have impact or affect the detailed design of the building, for which special provisions may be required in the Contract Documents. Those who requiring information on construction aspects should make their own interpretation of the factual data presented in the report. Interpretation of the data presented may affect equipment selection, proposed construction methods, and scheduling of construction activities.

6.2 Project Design

6.2.1 Existing Site Condition

Detailed site condition is provided in Section 2. The site contains two existing two-story residential structures and is located in the middle of a residential subdivision. The location of the site is shown on Figure 1 included in Appendix B.

6.2.2 Proposed Development

It is understood that the proposed development will be a three-storey apartment building with a basement, and will likely be a conventional slab on grade with shallow footing foundation.

Finished grade was not provided at the time of this report, it is expected construction will occur at the existing grade and no grade raise are expected.

6.3 Frost Protection

Based on applicable building codes, a minimum earth cover of 1.8 m, or the thermal equivalent of insulation, should be provided for all exterior footings to reduce the effects of frost action.

6.4 Site Classification for Seismic Site Response

Table 4.2 of CHBDC shall be consulted for the purpose of seismic design. Selected spectral responses in the general vicinity of the site for 10% chance of exceedance in 50 years (475 years return period) are as indicated in Table 6-3, shown below;

Table 6-1: Selected Seismic Spectral Responses (10% in 50 Yrs)

Sa(0.2)	Sa(0.5)	Sa(2.0)	PGA	PGV
0.161	0.124	0.021	0.102	0.068

The site can be classified as a Site Class "E" based on the clay consistency for the purposes of site-specific seismic response to earthquakes based on Table 4.1.8.4.A OBC 2012.

6.5 Engineered Fill

It is understood there are no plans for grade raise at this site.

If engineered fill is required, any topsoil or soft and spongy material should be removed before placing the engineered fill. The fill should be placed in horizontal lifts of uniform thickness of no more than 300 mm before compaction. It should be placed at appropriate moisture content and compacted to the specified density. The requirements for fill material and compaction may be addressed with a note on the structural drawing for foundation or grading drawing and/or with a Non-Standard Special Provision (NSSP). In any location where the engineered fill is to support any structural element, including pavement structure, minimum 100% Standard Proctor Maximum Dry Density (SPMDD) should be achieved. In other cases, minimum 96% SPMDD is adequate.

6.6 Slabs-on-Grade

Slabs-on-grade should be supported on minimum 200 mm of Granular A compacted to 100% SPMDD. In case the subgrade needs to be raised Granular B type II or granular A needs to be compacted to minimum 96% SPMDD.

All subgrades should be proof-rolled under the supervision of a geotechnical representative prior to placement of the Granular "A" and slab-on-grade.

6.7 Shallow Foundations

Based on the proposed building concept and architectural sketches, it is the authors' understanding that the building foundation level may fall close to the interface of the weather crust and the soft clay. Based on the in-situ undrained shear test results of the clay and laboratory test results for plasticity index, a pre-consolidation pressure of 150 kPa was considered in settlement calculations.

The structure is expected to be a light-weight wood frame with or without steel or concrete components. Considering the order of structural loads expected at the foundation level, provision of conventional strip footings will be adequate. If necessary, pad footings can be also used in the design, however the dimensions of isolated pad footing shall not exceed 2 m. Footings are expected to be buried to resist overturning and sliding and also to provide protection against frost action.

The excavation should extended to the top of the native clay, care must be taken not to disturb the clay. From the final stage of the excavation to placement of footings, construction traffic over the sensitive clay shall be minimized. Placement of mud-slab immediately after excavation can reduce the risk of subgrade degradation. Excavation into the clay layer should be limited. If adequate frost cover is not provided, the deficit of earth cover should be compensated by application of synthetic insulation material. A minimum of 0.6m of the clay crust should remain intact.

6.7.1 Bearing Capacity

Assuming the strip footings are constructed through excavating the fill and exposing the native clay crust, the following bearing capacity values can be used for structural design;

Factored beading pressure at Ultimate Limit State (ULS): 115 kPa

Serviceability Limit State (SLS): 75 kPa (1 m to 1.5 m wide strip footings)

If strip footings wider than 1.5 m are required, then authors of this report should be informed to verify the compatibility of the design with settlement criteria. Footings narrower than 0.6 m are not recommended due to the risk of punching failure. Following above note recommendations, total settlements are expected to remain between 25 mm to 35 mm. The structural designer shall note that wider strip footings with the same applied pressure will trigger larger settlements. When designing footings on clay, it is the best practice to keep the footing sizes and bearing pressures as similar as possible to reduce the risk of differential settlements.

6.8 Protection of Subgrade

Inspection and approval of the footing subgrade are required. This requirement may be addressed with a note on the structural drawing for foundation and/or with a Non-Standard Special Provision (NSSP). If the

constructor can ensure there won't be any traffics on the subgrade, protection can be done through temporary covering. To limit disturbance, subgrade should be protected from freezing or precipitation.

6.9 Lateral Earth Pressure

Free draining material should be used as backfill material for foundation walls. If the proper drainage is provided "at rest" condition may be assumed for calculation of earth pressure on foundation walls. The following parameters are recommended for the granular backfill.

Table 6-1: Backfill Material Properties

Borehole	Granular "A"	Granular "B"
Effective Internal Friction Angle, ϕ'	35°	30°
Unit Weight, γ (kN/m^3)	22.8	22.8

6.10 Cement Type and Corrosion Potential

Sample from subgrade soil was submitted to Parcel laboratories for testing of chemical properties relevant to exposure of concrete elements to sulphate attacks as well as potential soil corrosivity effects on buried metallic structural element. Test results are presented in Tables 5-1.

The potential for sulphate attack on concrete structures is low. Therefore Type GU Portland cement may be adequate to protect buried concrete elements in the subsurface conditions encountered.

The soil pH is slightly on the basic side, high resistivity and relatively low chloride content determines the environment for buried steel elements is within the non-aggressive range.

7.0 CONSTRUCTION CONSIDERATIONS

Any organic or topsoil material, and existing fill material of any kind, should be removed from the footprint of the footing. If grade raise above the native clay subgrade is required suitable fill material to conform to specifications of OPSS Granular A should be placed over a layer of geotextile.

The founding level is expected above the groundwater level encountered at this site and no dewatering problems are anticipated. However, the excavated subgrade must be kept dry at all time to minimize the disturbance of the subgrade. Groundwater elevation is expected to fluctuate seasonally.

A geotechnical engineer or technician should attend the site to confirm the type of the material and level of compaction.

Foundation walls should be backfilled with free-draining material such as OPSS Granular material. The native clay is not a suitable material for backfilling. Sub-drains with positive drainage to the City sewer should be provided at foundation level.

Based on the proposed site layout there is not adequate room for sloped excavation. The contractor shall retain a professional engineer to provide excavation and shoring design to protect the existing buildings adjacent to the proposed excavation.

Groundwater table is expected to be lower than the proposed excavation (2.0 ± 0.3 m depth below existing ground) and the chance of water draw down due to the proposed excavation is minimal. Since the proposed excavation will be relatively close to the neighboring properties, the contractor should consider the addition of an instrumentation and monitoring program to their excavation plan. A baseline should be established and documented by surveying structural monitoring points and photographing exterior and interior of the adjacent buildings before the start of construction activities.

Given the age of the existing structure, the primary position of its consolidation settlement for the current load should have been achieved. The proposed building will undergo settlements as described in Section 6.7. In order to accommodate the expected varying levels of settlement between the two structures, it is best practice to separate the exiting and the proposed buildings. If there has to be connected structural components such as links or corridors, between the existing and proposed buildings, a provision of an expansion joint will be necessary.

The applied surcharge from the proposed building on the subgrade may also cause some settlement of the existing buildings. The magnitude of this settlement is a function of the distance, depth, and existing in-situ stress under each of the adjacent structures. The above noted instrumentation program can be used to measure or rule-out such effects and to quantify or reject potential claims by the owners of neighboring properties.

8.0 SITE SERVICES

At the subject site, the burial depth of water-bearing utility lines is typically 2.4 m below ground surface. If this depth is not achievable due to design restrictions, equivalent thermal insulation should be provided. The contractor should retain a professional engineer to provide detailed drawings for excavation and temporary support of the excavation walls during construction.

Utilities should be supported on minimum of 150 mm bedding of Granular A compacted to minimum 96% of SPMDD. Since the native subgrade is fine grained, it is recommended to separate the subgrade from the bedding material by a layer of geotextile to prevent cross migration of materials. Utility cover can be Granular A or Granular B type II compacted to 96% SPMDD. All covers are to be compacted to 100% SPMDD if intersecting structural elements.

Cut-off walls should be provided for utility trenches running below the groundwater level to mitigate the settlement risk due to groundwater lowering.

9.0 PAVEMENT STRUCTURE

It is understood the site plan contains an asphalt driveway to include room for two parking spaces. If this parking area is to be part of the new construction, the pavement structure detailed in the table below should be followed. The proposed pavement structure is suitable for construction on native subgrade or raised grade through engineered fill.

Table 9-1: Proposed Pavement Structure for Residential Driveways

Material		Thickness (mm)
Surface	Superpave 12.5, Design Category C, PG 58-34	50
Base	OPSS Granular A	250

10.0 CLOSURE

We trust this geotechnical investigation and foundation design report meets requirements of your project. The "Limitations of Report" presented in Appendix A are an integral part of this report. Please do not hesitate to contact the undersigned should you have any questions or concerns.

McIntosh Perry Consulting Engineers Ltd.

Mary-Ellen Gleeson, M.Eng., EIT.
Geotechnical Engineering Intern

N'eem Tavakkoli, M.Eng., P.Eng.
Senior Geotechnical Engineer

11.0 REFERENCES

Canadian Geotechnical Society, "Canadian Foundation Engineering Manual", 4th Edition, 2006.

Ontario Ministry of Natural Resources (OMNR), Ontario Geological Survey, Special Volume 2, "The Physiography of Southern Ontario", 3rd Edition, 1984.

Google Earth, Google, 2015.

MTO – Pavement Design and Rehabilitation Manual

105 – 109 HENDERSON AVENUE

APPENDIX A
LIMITATIONS OF REPORT

McINTOSH PERRY

LIMITATIONS OF REPORT

McIntosh Perry Consulting Engineers Ltd. (McIntosh Perry) carried out the field work and prepared the report. This document is an integral part of the Foundation Investigation and Design report presented.

The conclusions and recommendations provided in this report are based on the information obtained at the borehole locations where the tests were conducted. Subsurface and groundwater conditions between and beyond the boreholes may differ from those encountered at the specific locations where tests were conducted and conditions may become apparent during construction, which were not detected and could not be anticipated at the time of the site investigation. The benchmark level used and borehole elevations presented in this report are primarily to establish relative differenced in elevations between the borehole locations and should not be used for other purposes such as to establish elevations for grading, depth of excavations or for planning construction.

The recommendations presented in this report for design are applicable only to the intended structure and the project described in the scope of the work, and if constructed in accordance with the details outlined in the report. Unless otherwise noted, the information contained in this report does not reflect on any environmental aspects of either the site or the subsurface conditions.

The comments or recommendation provided in this report on potential construction problems and possible construction methods are intended only to guide the designer. The number of boreholes advanced at this site may not be sufficient or adequate to reveal all the subsurface information or factors that may affect the method and cost of construction. The contractors who are undertaking the construction shall make their own interpretation of the factual data presented in this report and make their conclusions, as to how the subsurface conditions of the site may affect their construction work.

The boundaries between soil strata presented in the report are based on information obtained at the borehole locations. The boundaries of the soil strata between borehole locations are assumed from geological evidences. If differing site conditions are encountered, or if the Client becomes aware of any additional information that differs from or is relevant to the McIntosh Perry findings, the Client agrees to immediately advise McIntosh Perry so that the conclusions presented in this report may be re-evaluated.

Under no circumstances shall the liability of McIntosh Perry for any claim in contract or in tort, related to the services provided and/or the content and recommendations in this report, exceed the extent that such liability is covered by such professional liability insurance from time to time in effect including the deductible therein, and which is available to indemnify McIntosh Perry. Such errors and omissions policies are available for inspection by the Client at all times upon request, and if the Client desires to obtain further insurance to protect it against any risks beyond the coverage provided by such policies, McIntosh Perry will co-operate with the Client to obtain such insurance.

McIntosh Perry prepared this report for the exclusive use of the Client. Any use which a third party makes of this report, or any reliance on or decision to be made based on it, are the responsibility of such third parties. McIntosh Perry accepts no responsibility and will not be liable for damages, if any, suffered by any third party as a result of decisions made or actions taken based on this report.

105 – 109 HENDERSON AVENUE

APPENDIX B
FIGURES

McINTOSH PERRY

450 225 0 450

Scale 1:15,000 Metres

LEGEND

- Site Location
- Local Road
- Major Road
- Wooded Area
- Watercourse
- Waterbody

REFERENCE

GIS data provided by the Ontario Ministry of Natural Resources and Forestry, 2018.

CLIENT: TC UNITED

PROJECT: GEOTECHNICAL INVESTIGATION
105-109 HENDERSON AVE.

TITLE: SITE LOCATION

McINTOSH PERRY
115 Walgreen Road, RR3, Carp, ON K0A1L0
Tel: 613-836-2184 Fax: 613-836-3742
www.mcintoshperry.com

PROJECT NO:	CP-17-0638
Date	Feb., 01, 2018
GIS	JD
Checked By	MG

FIGURE: 1

H:\01 Project - Proposals\2017 Jobs\CP\0CP-17-0638 TCU_20 Unit Apartment Building_105-109 Henderson Avenue\Phase 1 ESA\05 GIS\mxd\0CP-17-0638_Geotechnical\01_SiteLocationHenderson.mxd

7 3.5 0 7

Scale 1:300 Metres

LEGEND

- Borehole Location
- Approximate Property Boundary

REFERENCE

GIS data provided by the Ontario Ministry of Natural Resources and Forestry, 2018.

CLIENT: TC UNITED

PROJECT: GEOTECHNICAL INVESTIGATION
105-109 HENDERSON AVE.

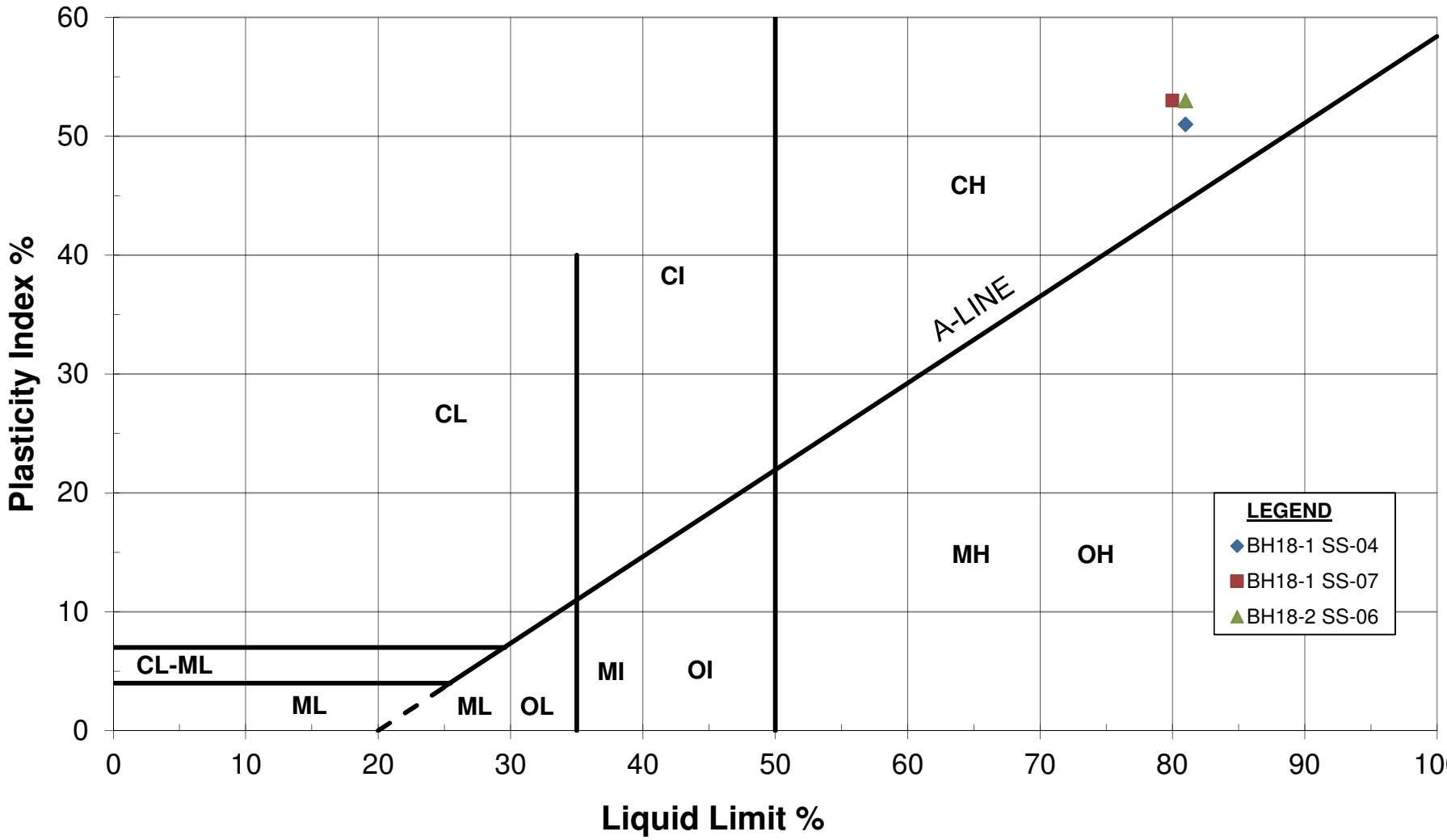
TITLE: BOREHOLE LOCATIONS

McINTOSH PERRY
115 Walgreen Road, RR3, Carp, ON K0A1L0
Tel: 613-836-2184 Fax: 613-836-3742
www.mcintoshperry.com

PROJECT NO:	CP-17-0638
Date	Feb., 01, 2018
GIS	JD
Checked By	MG

FIGURE: 2

H:\01 Project - Proposals\2017 Jobs\CP\0CP-17-0638 TCU_20 Unit Apartment Building_105-109 Henderson Avenue\Phase 1 ESA\05 GIS\mxd\0CP-17-0638_Geotechnical\02_BoreholeLocationsHenderson.mxd



McINTOSH PERRY

PLASTICITY CHART

CLAY

105-109 Henderson Road

Figure No. 3

CP-17-0638

105 – 109 HENDERSON AVENUE

APPENDIX C
BOREHOLE LOGS

McINTOSH PERRY

EXPLANATION OF TERMS USED IN REPORT

N-VALUE: THE STANDARD PENETRATION TEST (SPT) N-VALUE IS THE NUMBER OF BLOWS REQUIRED TO CAUSE A STANDARD 51mm O.D SPLIT BARREL SAMPLER TO PENETRATE 0.3m INTO UNDISTURBED GROUND IN A BOREHOLE WHEN DRIVEN BY A HAMMER WITH A MASS OF 63.5 kg, FALLING FREELY A DISTANCE OF 0.76m. FOR PENETRATIONS OF LESS THAN 0.3m N-VALUES ARE INDICATED AS THE NUMBER OF BLOWS FOR THE PENETRATION ACHIEVED. AVERAGE N-VALUE IS DENOTED THUS $\bar{N}$.

DYNAMIC CONE PENETRATION TEST: CONTINUOUS PENETRATION OF A CONICAL STEEL POINT (51mm O.D. 60° CONE ANGLE) DRIVEN BY 475J IMPACT ENERGY ON 'A' SIZE DRILL RODS. THE RESISTANCE TO CONE PENETRATION IS MEASURED AS THE NUMBER OF BLOWS FOR EACH 0.3m ADVANCE OF THE CONICAL POINT INTO THE UNDISTURBED GROUND.

SOILS ARE DESCRIBED BY THEIR COMPOSITION AND CONSISTENCY OR DENSENESS.

CONSISTENCY: COHESIVE SOILS ARE DESCRIBED ON THE BASIS OF THEIR UNDRAINED SHEAR STRENGTH (c_u) AS FOLLOWS:

C_u (kPa)	0 – 12	12 – 25	25 – 50	50 – 100	100 – 200	>200
	VERY SOFT	*SOFT*	*FIRM*	*STIFF*	*VERY STIFF*	*HARD*

DENSENESS: COHESIONLESS SOILS ARE DESCRIBED ON THE BASIS OF DENSENESS AS INDICATED BY SPT N VALUES AS FOLLOWS:

N (BLOWS/0.3m)	0 – 5	5 – 10	10 – 30	30 – 50	>50
	VERY LOOSE	*LOOSE*	*COMPACT*	*DENSE*	*VERY DENSE*

ROCKS ARE DESCRIBED BY THEIR COMPOSION AND STRUCUTRAL FEATURES AND/OR STRENGTH.

RECOVERY: SUM OF ALL RECOVERED ROCK CORE PIECES FROM A CORING RUN EXPRESSED AS A PERCENT OF THE TOTAL LENGTH OF THE CORING RUN.

MODIFIED RECOVERY: SUM OF THOSE INTACT CORE PIECES, 100mm+ IN LENGTH EXPRESSED AS A PERCENT OF THE LENGTH OF THE CORING RUN. THE ROCK QUALITY DESIGNATION (RQD), FOR MODIFIED RECOVERY IS:

RQD (%)	0 – 25	25 – 50	50 – 75	75 – 90	90 – 100
	VERY POOR	*POOR*	*FAIR*	*GOOD*	*EXCELLENT*

JOINT AND BEDDING:

SPACING	50mm	50 – 300mm	0.3m – 1m	1m – 3m	>3m
JOINTING	*VERY CLOSE*	*CLOSE*	*MOD. CLOSE*	*WIDE*	*VERY WIDE*
BEDDING	*VERY THIN*	*THIN*	*MEDIUM*	*THICK*	*VERY THICK*

ABBREVIATIONS AND SYMBOLS

FIELD SAMPLING

SS	SPLIT SPOON	TP	THINWALL PISTON
WS	WASH SAMPLE	OS	OSTERBERG SAMPLE
ST	SLOTTED TUBE SAMPLE	RC	ROCK CORE
BS	BLOCK SAMPLE	PH	TW ADVANCED HYDRAULICALLY
CS	CHUNK SAMPLE	PM	TW ADVANCED MANUALLY
TW	THINWALL OPEN	FS	FOIL SAMPLE

STRESS AND STRAIN

u_w	kPa	PORE WATER PRESSURE
r_u	1	PORE PRESSURE RATIO
σ	kPa	TOTAL NORMAL STRESS
σ'	kPa	EFFECTIVE NORMAL STRESS
τ	kPa	SHEAR STRESS
$\sigma_1, \sigma_2, \sigma_3$	kPa	PRINCIPAL STRESSES
ε	%	LINEAR STRAIN
$\varepsilon_1, \varepsilon_2, \varepsilon_3$	%	PRINCIPAL STRAINS
E	kPa	MODULUS OF LINEAR DEFORMATION
G	kPa	MODULUS OF SHEAR DEFORMATION
μ	1	COEFFICIENT OF FRICTION

MECHANICALL PROPERTIES OF SOIL

m_v	kPa^{-1}	COEFFICIENT OF VOLUME CHANGE
c_c	1	COMPRESSION INDEX
c_s	1	SWELLING INDEX
c_a	1	RATE OF SECONDARY CONSOLIDATION
c_v	m^2/s	COEFFICIENT OF CONSOLIDATION
H	m	DRAINAGE PATH
T_v	1	TIME FACTOR
U	%	DEGREE OF CONSOLIDATION
σ'_{vo}	kPa	EFFECTIVE OVERBURDEN PRESSURE
σ'_p	kPa	PRECONSOLIDATION PRESSURE
τ_f	kPa	SHEAR STRENGTH
c'	kPa	EFFECTIVE COHESION INTERCEPT
Φ'	$^{-o}$	EFFECTIVE ANGLE OF INTERNAL FRICTION
c_u	kPa	APPARENT COHESION INTERCEPT
Φ_u	$^{-o}$	APPARENT ANGLE OF INTERNAL FRICTION
τ_R	kPa	RESIDUAL SHEAR STRENGTH
τ_r	kPa	REMOULDED SHEAR STRENGTH
S_t	1	SENSITIVITY = c_u / τ_r

PHYSICAL PROPERTIES OF SOIL

ρ_s	kg/m^3	DENSITY OF SOLID PARTICLES	e	1,%	VOID RATIO	e_{min}	1,%	VOID RATIO IN DENSEST STATE
γ_s	kN/m^3	UNIT WEIGHT OF SOLID PARTICLES	n	1,%	POROSITY	I_D	1	DENSITY INDEX = $\frac{e_{max} - e}{e_{max} - e_{min}}$
ρ_w	kg/m^3	DENSITY OF WATER	w	1,%	WATER CONTENT	D	mm	GRAIN DIAMETER
γ_w	kN/m^3	UNIT WEIGHT OF WATER	s_r	%	DEGREE OF SATURATION	D_n	mm	N PERCENT – DIAMETER
ρ	kg/m^3	DENSITY OF SOIL	w_L	%	LIQUID LIMIT	C_u	1	UNIFORMITY COEFFICIENT
γ	kN/m^3	UNIT WEIGHT OF SOIL	w_P	%	PLASTIC LIMIT	h	m	HYDRAULIC HEAD OR POTENTIAL
ρ_d	kg/m^3	DENSITY OF DRY SOIL	w_s	%	SHRINKAGE LIMIT	q	m^3/s	RATE OF DISCHARGE
γ_d	kN/m^3	UNIT WEIGHT OF DRY SOIL	I_P	%	PLASTICITY INDEX = $(W_L - W_L)$	v	m/s	DISCHARGE VELOCITY
ρ_{sat}	kg/m^3	DENSITY OF SATURATED SOIL	I_L	1	LIQUIDITY INDEX = $(W - W_P)/ I_P$	i	1	HYDAULIC GRADIENT
γ_{sat}	kN/m^3	UNIT WEIGHT OF SATURATED SOIL	I_C	1	CONSISTENCY INDEX = $(W_L - W) / 1_P$	k	m/s	HYDRAULIC CONDUCTIVITY
ρ'	kg/m^3	DENSITY OF SUBMERED SOIL	$e_{,max}$	1,%	VOID RATIO IN LOOSEST STATE	j	kN/m^3	SEEPAGE FORCE
γ'	kN/m^3	UNIT WEIGHT OF SUBMERGED SOIL						

McINTOSH PERRY

RECORD OF BOREHOLE No 18-1

DATE:	31/01/2018 - 31/01/2018	**LOCATION:**	105-109 Henderson Avenue ()	**ORIGINATED BY:**	Phil Hulan
ID:	CP-17-0638-HENDERSON	**COORDINATES:**	Lat: 45.423615 , Lon: -75.679798	**COMPILED BY:**	Mary-Ellen Gleeson
CLIENT:	TC United	**DATUM:**	Local	**CHECKED BY:**	N'eem Tavakkoli
ELEVATION:	99.4 m	**REMARK:**	No water observed in open borehole.	**REPORT DATE:**	07/03/2018

DEPTH - feet	DEPTH - meters	SOIL PROFILE: ELEVATION - m / DEPTH - m	SOIL PROFILE: DESCRIPTION	SYMBOL	SAMPLES: TYPE AND NUMBER	SAMPLES: STATE	SAMPLES: RECOVERY	SAMPLES: "N" or RQD	GROUNDWATER CONDITIONS	DYNAMIC CONE PEN. RESISTANCE PLOT / SHEAR STRENGTH (kPa)	WATER CONTENT and LIMITS (%)	REMARKS & GRAIN SIZE DISTRIBUTION (%)
		99.4	Natural ground surface							DYNAMIC CONE PEN. RESISTANCE PLOT 20 40 60 80; SHEAR STRENGTH (kPa) Vane test: ◇ Intact, ◆ Remolded; Lab vane: □ Intact, ■ Remolded; 20 40 60 80 100	W_P W W_L; 25 50 75	G S M C
		0.0 / 99.1	Topsoil									
		0.3	Fill. Silty sand, brown, moist, loose.		SS-01		79	3				
	1				SS-02		54	6				
		97.9										
5		1.5	Silty clay, grey, moist to wet, stiff, weathered.		SS-03		83	7				
	2				SS-04		83	5				
10	3				SS-05		92	3				
		95.6										
		3.8	Silty clay, grey, wet, firm.		SS-06		100	0		5.0; 68.0		
	4											
15												
	5									27.0; 81.0		
					SS-07		100	2				
20	6									8.0; 68.0; 11.0; 59.0		
	7				SS-08		100	0				
25										11.0; 38.0; 7.0; 49.0		
	8											
					SS-09		75	1				
	9											
30												

McINTOSH PERRY

RECORD OF BOREHOLE No 18-1

DATE:	31/01/2018 - 31/01/2018	**LOCATION:**	105-109 Henderson Avenue ()	**ORIGINATED BY:**	Phil Hulan
ID:	CP-17-0638-HENDERSON	**COORDINATES:**	Lat: 45.423615 , Lon: -75.679798	**COMPILED BY:**	Mary-Ellen Gleeson
CLIENT:	TC United	**DATUM:**	Local	**CHECKED BY:**	N'eem Tavakkoli
ELEVATION:	99.4 m	**REMARK:**	No water observed in open borehole.	**REPORT DATE:**	07/03/2018

DEPTH - feet	DEPTH - meters	SOIL PROFILE: ELEVATION - m / DEPTH - m	SOIL PROFILE: DESCRIPTION	SYMBOL	SAMPLES: TYPE AND NUMBER	SAMPLES: STATE	SAMPLES: RECOVERY	SAMPLES: "N" or RQD	GROUNDWATER CONDITIONS	DYNAMIC CONE PEN. RESISTANCE PLOT / SHEAR STRENGTH (kPa)	WATER CONTENT and LIMITS (%)	REMARKS & GRAIN SIZE DISTRIBUTION (%)
35	11											
40	12											
45	13											
50	14											
55	15											
	16											
	17	82.2 17.2	END OF BOREHOLE									
60	18											
	19											

DYNAMIC CONE PEN. RESISTANCE PLOT: 20 40 60 80

SHEAR STRENGTH (kPa): Vane test (◇ Intact, ◆ Remolded); Lab vane (□ Intact, ■ Remolded); 20 40 60 80 100

WATER CONTENT and LIMITS (%): W_P W W_L; 25 50 75

REMARKS & GRAIN SIZE DISTRIBUTION (%): G S M C

\\LICENSES7\Sobek\Geotec80\Style\Log_Borehole_v5.sty

McINTOSH PERRY

RECORD OF BOREHOLE No 18-2

DATE:	31/01/2018 - 31/01/2018	**LOCATION:**	105-109 Henderson Avenue ()	**ORIGINATED BY:**	Phil Hulan
ID:	CP-17-0638-HENDERSON	**COORDINATES:**	Lat: 45.423555 , Lon: -75.679717	**COMPILED BY:**	Mary-Ellen Gleeson
CLIENT:	TC United	**DATUM:**	Local	**CHECKED BY:**	N'eem Tavakkoli
ELEVATION:	99.5 m	**REMARK:**	No water observed in open borehole.	**REPORT DATE:**	07/03/2018

DEPTH - feet	DEPTH - meters	SOIL PROFILE: ELEVATION - m / DEPTH - m	SOIL PROFILE: DESCRIPTION	SYMBOL	SAMPLES: TYPE AND NUMBER	SAMPLES: STATE	SAMPLES: RECOVERY	SAMPLES: "N" or RQD	GROUNDWATER CONDITIONS	DYNAMIC CONE PEN. RESISTANCE PLOT / SHEAR STRENGTH (kPa)	WATER CONTENT and LIMITS (%)	REMARKS & GRAIN SIZE DISTRIBUTION (%)
		99.5	Natural ground surface							DYNAMIC CONE PEN. RESISTANCE PLOT 20 40 60 80; SHEAR STRENGTH (kPa) Vane test: Intact ◇, Remolded ◆; Lab vane: Intact □, Remolded ■; 20 40 60 80 100	W_P W W_L; 25 50 75	G S M C
		0.0	150 mm Topsoil.									
		99.3 / 0.2	Fill. Silty sand, trace to some clay, trace gravel, brown, dry to moist, loose.		SS-01		8	5				
	1				SS-02		54	6				
		98.0										
5		1.5	Silty clay, grey with iron staining, moist, stiff to firm, weathered.		SS-03		83	11				
	2				SS-04		92	7				
10	3	96.1 / 3.4	Silty clay, grey, moist to wet, stiff to firm.		SS-05		100	4		19.0; 102.		
	4									21.0; 72.0		
15					SS-06		100	2				
	5									19.0; 85.0		
		93.7 / 5.8	**END OF BOREHOLE**							18.0; 81.0		
20	6											
	7											
25												
	8											
30	9											

105 – 109 HENDERSON AVENUE

APPENDIX D
LAB RESULTS

McINTOSH PERRY

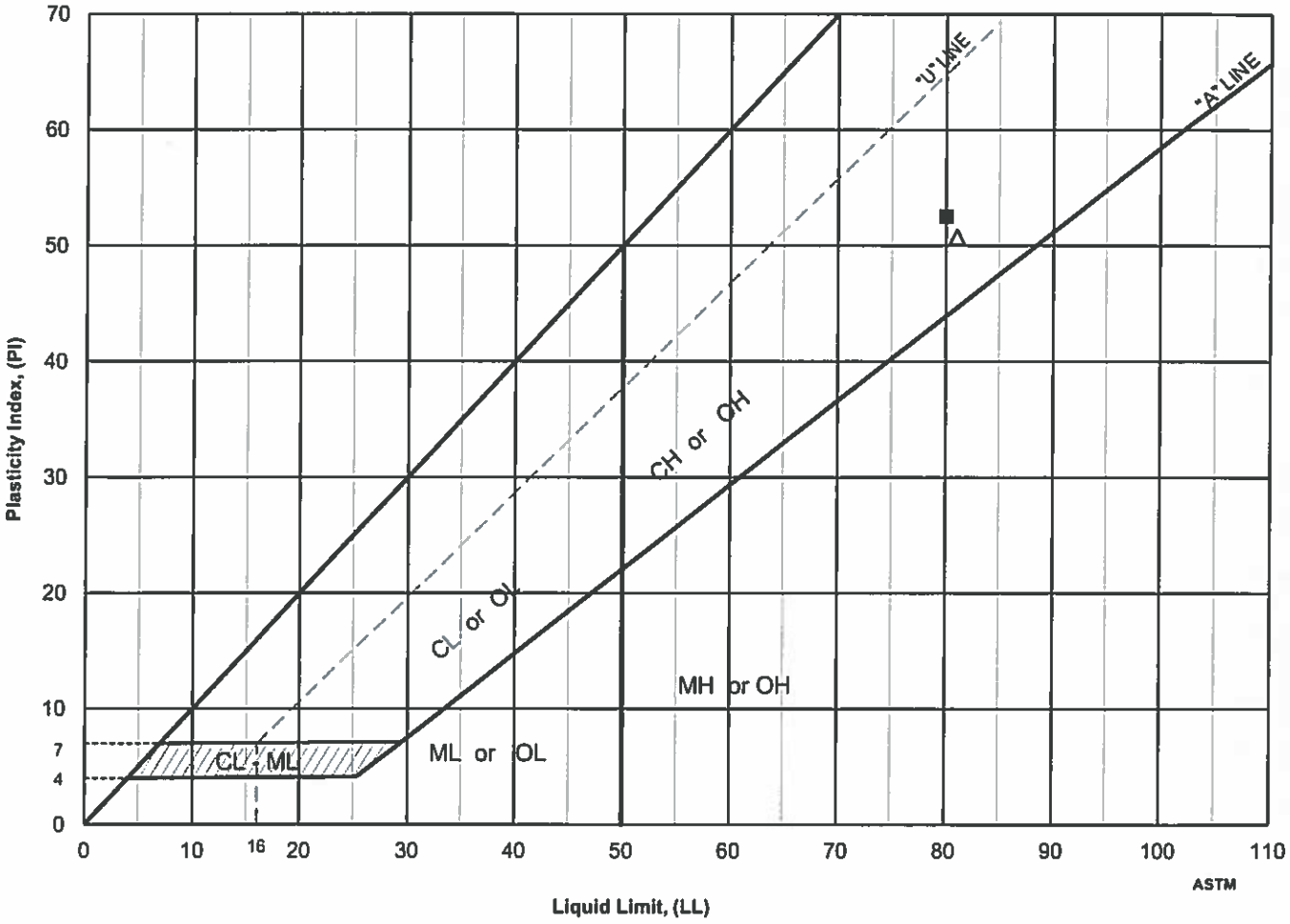
LRL Associates Ltd.

PLASTICITY INDEX

ASTM D 4318 / LS-703/704

Client:	McIntosh Perry Consulting Engineers	**Reference No.:**	CP-17-0638
Project:	Materials Testing	**File No.:**	170496-20
Location:	Henderson	**Report No.:**	1

Plasticity Chart



	Location	Sample	Depth, m	Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Activity Number	USCS
△	BH 18-1	SS-04	2.29 - 2.90	54	81	30	51	0.47	n/d	CH
■	BH 18-1	SS-07	5.33 - 5.94	75	80	27	53	0.90	n/d	CH

Date Issued: February 7, 2018

Reviewed By: W.A.McLaughlin

W.A.McLaughlin, Geo.Tech., C.Tech.

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Certificate of Analysis

McIntosh Perry Consulting Eng. (Carp)

115 Walgreen Road
RR#3 Carp, ON K0A 1L0
Attn: Mary Ellen Gleeson

Client PO: Henderson CP-17-0638
Project: CP-17-0638
Custody: 34160

Report Date: 12-Feb-2018
Order Date: 6-Feb-2018

Order #: 1806215

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1806215-01	CP-17-0638 BH18-1 SS-03

Approved By:

Mark Foto

Mark Foto, M.Sc.
Lab Supervisor

Certificate of Analysis
Client: McIntosh Perry Consulting Eng. (Carp)
Client PO: Henderson CP-17-0638

Report Date: 12-Feb-2018
Order Date: 6-Feb-2018
Project Description: CP-17-0638

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Anions	EPA 300.1 - IC, water extraction	8-Feb-18	9-Feb-18
pH, soil	EPA 150.1 - pH probe @ 25 °C, CaCl buffered ext.	6-Feb-18	7-Feb-18
Resistivity	EPA 120.1 - probe, water extraction	9-Feb-18	10-Feb-18
Solids, %	Gravimetric, calculation	7-Feb-18	7-Feb-18

Certificate of Analysis

Report Date: 12-Feb-2018

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 6-Feb-2018

Client PO: Henderson CP-17-0638

Project Description: CP-17-0638

	Client ID:	CP-17-0638 BH18-1 SS-03	-	-	-
	Sample Date:	31-Jan-18	-	-	-
	Sample ID:	1806215-01	-	-	-
	MDL/Units	Soil	-	-	-
Physical Characteristics					
% Solids	0.1 % by Wt.	75.2	-	-	-
General Inorganics					
pH	0.05 pH Units	7.25	-	-	-
Resistivity	0.10 Ohm.m	91.9	-	-	-
Anions					
Chloride	5 ug/g dry	9	-	-	-
Sulphate	5 ug/g dry	16	-	-	-

Certificate of Analysis
Client: McIntosh Perry Consulting Eng. (Carp)
Client PO: Henderson CP-17-0638

Report Date: 12-Feb-2018
Order Date: 6-Feb-2018
Project Description: CP-17-0638

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	ND	5	ug/g						
Sulphate	ND	5	ug/g						
General Inorganics									
Resistivity	ND	0.10	Ohm.m						

Certificate of Analysis
Client: McIntosh Perry Consulting Eng. (Carp)
Client PO: Henderson CP-17-0638

Report Date: 12-Feb-2018
Order Date: 6-Feb-2018
Project Description: CP-17-0638

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	14.4	5	ug/g dry	17.3			18.5	20	
Sulphate	15.7	5	ug/g dry	15.6			0.6	20	
General Inorganics									
pH	7.84	0.05	pH Units	7.89			0.6	10	
Resistivity	401	0.10	Ohm.m	395			1.4	20	
Physical Characteristics									
% Solids	90.0	0.1	% by Wt.	86.5			3.9	25	

Certificate of Analysis
Client: McIntosh Perry Consulting Eng. (Carp)
Client PO: Henderson CP-17-0638

Report Date: 12-Feb-2018
Order Date: 6-Feb-2018
Project Description: CP-17-0638

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	108	5	ug/g	17.3	90.5	78-113			
Sulphate	118	5	ug/g	15.6	103	78-111			

Qualifier Notes:

None

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable
ND: Not Detected
MDL: Method Detection Limit
Source Result: Data used as source for matrix and duplicate samples
%REC: Percent recovery.
RPD: Relative percent difference.

Soil results are reported on a dry weight basis when the units are denoted with 'dry'.
Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.

Paracel ID: 1806215

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e: paracel@paracellabs.com
www.paracellabs.com

Chain of Custody
(Lab Use Only)
№ 34160

Page 1 of 1

Client Name: McIntosh Perry	Project Reference: Henderson CP-17-0638
Contact Name: Mary-Ellen Gleeson	Quote #
Address: 115 Walgreen Rd, R.R.3 Carp, ON K0A1L0	PO # CP-17-0638
Telephone: 613-836-2184 (ex2279)	Email Address: M.gleeson@mcintoshperry.com

Turnaround Time:
☐ 1 Day ☐ 3 Day
☐ 2 Day ☒ Regular
Date Required: _____

Criteria: ☐ O. Reg. 153/04 (As Amended) Table __ ☐ RSC Filing ☐ O. Reg. 558/00 ☐ PWQO ☐ CCME ☐ SUB (Storm) ☐ SUB (Sanitary) Municipality: _____ ☐ Other: _____

Matrix Type: S (Soil/Sed.) GW (Ground Water) SW (Surface Water) SS (Storm/Sanitary Sewer) P (Paint) A (Air) O (Other)

Required Analyses

Paracel Order Number: 1806215

	Sample ID/Location Name	Matrix	Air Volume	# of Containers	Sample Taken Date	Sample Taken Time	Corrosivity										
1	CP-17-0638 BH18-1 SS-03	S			Jan 31/18		X		2 plastic								
2																	
3																	
4																	
5																	
6																	
7																	
8																	
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Comments:

Method of Delivery: Paracel.

Relinquished By (Sign):	Received by Driver/Depot: A Vecchio	Received at Lab: Rachel subject	Verified By:
Relinquished By (Print): Juli Usney	Date/Time: 06/02/18 2 50	Date/Time: Feb 6 '18	Date/Time: 02/06/18 5:54
Date/Time: Feb 1/2018 15:20	Temperature: ____ °C pH.	Temperature: 18.4 °C 5:47	pH Verified [ ] By:

105 – 109 HENDERSON AVENUE

APPENDIX E
SEISMIC HAZARD CALCULATION

McINTOSH PERRY

2015 National Building Code Seismic Hazard Calculation

INFORMATION: Eastern Canada English (613) 995-5548 français (613) 995-0600 Facsimile (613) 992-8836
Western Canada English (250) 363-6500 Facsimile (250) 363-6565

February 14, 2018

Site: 45.4236 N, 75.6799 W User File Reference: 105-109 Henderson Road

Requested by: , McIntosh Perry

National Building Code ground motions: 2% probability of exceedance in 50 years (0.000404 per annum)

Sa(0.05)	Sa(0.1)	**Sa(0.2)**	Sa(0.3)	**Sa(0.5)**	**Sa(1.0)**	**Sa(2.0)**	**Sa(5.0)**	**Sa(10.0)**	**PGA (g)**	**PGV (m/s)**
0.447	0.523	**0.439**	0.334	**0.237**	**0.118**	**0.056**	**0.015**	**0.0054**	**0.281**	**0.197**

Notes. Spectral (Sa(T), where T is the period in seconds) and peak ground acceleration (PGA) values are given in units of g (9.81 m/s^2). Peak ground velocity is given in m/s. Values are for "firm ground" (NBCC 2015 Site Class C, average shear wave velocity 450 m/s). NBCC2015 and CSAS6-14 values are specified in **bold** font. Three additional periods are provided - their use is discussed in the NBCC2015 Commentary. Only 2 significant figures are to be used. ***These values have been interpolated from a 10-km-spaced grid of points. Depending on the gradient of the nearby points, values at this location calculated directly from the hazard program may vary. More than 95 percent of interpolated values are within 2 percent of the directly calculated values.***

Ground motions for other probabilities:

Probability of exceedance per annum	0.010	0.0021	0.001
Probability of exceedance in 50 years	40%	10%	5%
Sa(0.05)	0.044	0.149	0.247
Sa(0.1)	0.061	0.187	0.300
Sa(0.2)	0.055	0.161	0.255
Sa(0.3)	0.044	0.124	0.195
Sa(0.5)	0.031	0.088	0.138
Sa(1.0)	0.015	0.044	0.070
Sa(2.0)	0.0061	0.021	0.033
Sa(5.0)	0.0012	0.0047	0.0081
Sa(10.0)	0.0006	0.0019	0.0032
PGA	0.033	0.102	0.163
PGV	0.021	0.068	0.111

References

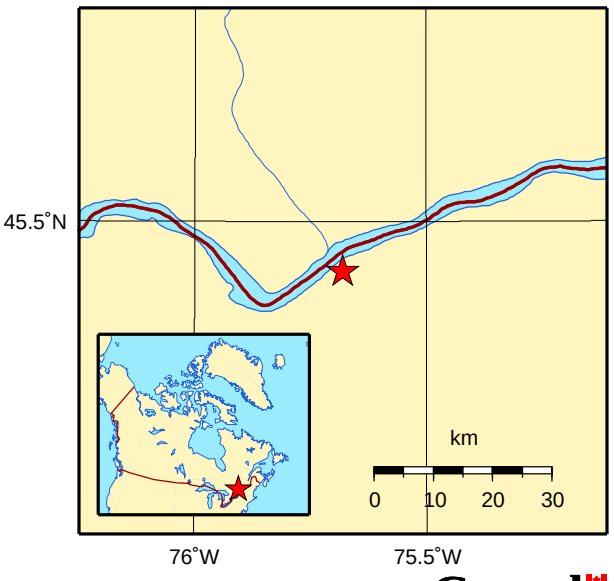
National Building Code of Canada 2015 NRCC no. 56190;
Appendix C: Table C-3, Seismic Design Data for Selected Locations in Canada

User's Guide - NBC 2015, Structural Commentaries NRCC no. xxxxxx (in preparation)
Commentary J: Design for Seismic Effects

Geological Survey of Canada Open File 7893 Fifth Generation Seismic Hazard Model for Canada: Grid values of mean hazard to be used with the 2015 National Building Code of Canada

See the websites *www.EarthquakesCanada.ca* and *www.nationalcodes.ca* for more information

Aussi disponible en français



Natural Resources Canada Ressources naturelles Canada

Canada

Appendix E DRAWINGS