

Functional Serviceability Report

Official Plan Amendment and Zoning By-law Amendment Proposed Industrial Development 322 Waverly Street, Ottawa ON

Prepared for:

Serco Realty Group 9 Capella Court Unit 200, Ottawa ON. K2E 8A7

Attention: Loredena Parcari

LRL File No.: 190523 May 13, 2021

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1 Introduction and Site Description

LRL. Associates LTD. was retained by Serco Realty Group to prepare a functional serviceability report to support the Official Plan Amendment and Zoning By-law amendment application for the property located at 322 Waverly Street within the City of Ottawa.

The subject site is within the Somerset Ward, located on the south side of Waverly street and has an approximate area of 0.06 ha, east of O'Connor street and west of Metcalf street. The land is currently occupied by a two-storey brick dwelling and a large asphalt parking area with a small area of short grass in front of the dwelling. The subject site can be seen below in figure 1.



Figure 1: Arial View of Subject Lands

Under the City of Ottawa Zoning by-law, the existing land is currently split zoned with the west portion of the site zoned Residential Fourth Density, Subzone UD, Exception 479 (R4UD, [479]) and the east portion of the property zoned Residential Fourth Density, Subzone UD, Exception 478 (R4UD, [478]). This servicing study has been prepared to support an amendment to rezone to Residential Fifth Density which is further expanded in in the planning documentation accompanying the submission.

The serviceability review summarized in this document has been completed to further investigate the potential for this property to be redeveloped to accommodate a six-storey residential building with approximately 32 units. The concept site plan prepared by Chmiel Architects, illustrating the proposed development can be found in appendix A at the back of this report.

Following the Official Plan Amendment and Zoning By-law amendment application, a detailed design will advance with intentions for full Site Plan Control application submission.

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2 EXISTING SITE AND AVAILABLE SERVICES

J. D. Barnes Limited prepared a topographic survey of the subject property in September of 2020 which has been included in appendix B for reference. Based on the topography information available, the general elevation of the land is slightly higher in the rear of the property with an existing retaining wall located at the rear property line.

Utilizing the GeoOttawa mapping portal as well as plan and profile drawings provided by the City of Ottawa indicate the following services are running along Waverly Street within the right-of-way in front of the property boundary.

- 200mm diameter PVC watermain
- 900mm diameter Combined Concrete Sewer @ 0.25% Slope
- 750mm x 900mm Combined Brick Sewer @ 0.25% Slope

The existing dwelling located central on the site is currently serviced to meet the domestic demands of the home; however, these will be abandoned to accommodate the proposed development of a greater population.

3 WATER SUPPLY SERVICING

The site is intended to be services through a new 150mm diameter water service connecting to the existing 200mm PVC watermain within Waverly Street.

Bases on the location and available data form the city, the subject property lies within the 1W pressure zone of the water distribution system.

3.1 Domestic Water Demands

Proposed populations have been interpreted from the architectural drawings. Based on the number of residential units within the building combined with the City of Ottawa design guidelines for population projection, the number of units translates to approximately 58 residents. Table 1 below summarizes the proposed population count as interpreted using table 4.1 of the City of Ottawa design guidelines.

Table 1: Development Residential Population Estimate

| Proposed Unit Type | Persons Per Unit | Number of Units | Total Population |
|----------------------------------|------------------|-----------------|------------------|
| Residential Average Apartment | 1.8 | 32 | 58 |

Table 2 below summarizes the water supply guidelines which must be employed during detailed design and sizing of the service to the building.

Table 2: Water Supply Guidelines

| Design Parameter | Value |
|--|------------------|
| Residential Average Dailey Demand | 280 L/person/Day |
| Maximum Dailey Peaking Factor (As per MOE Table 3-3) | 7.3 |
| Peak Hour Factor (As per MOE Table 3-3) | 10.9 |
| Minimum Depth of Cover | 2.4m |

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| Desired operating pressure range during normal operating conditions | 350 kPa and 480 kPa |
|---|---------------------|
| During normal operating conditions pressure must not drop below | 275 kPa |
| During normal operating conditions pressure shall not exceed | 552 kPa |
| During fire flow operating conditions pressure must not drop below | 140 kPa |

The required water supply requirements for the residential units in the proposed building have been calculated using the following formula:

Where:

$$Q = (q \times P \times M)$$

q = average water consumption (L/capita/day)

P = design population (capita)

M = Peak factor

Using a calculated Maximum Day Factor and Peak Hour factor of 7.3 and 10.9 respectively as per Table 3-3 in the MOE Design Guidelines, anticipated demands were calculated as follows:

- Average daily domestic water demand is 0.19 L/s,
- Maximum daily demand is 1.35 L/s, and
- o Maximum hourly demand is 14.76 L/s.

Based on maximum hourly rate of 14.76 L/s a minimum of 150 mm dia. servicing is required. Refer to *Appendix C* for water demand calculations.

During the detailed design which will take place during the site plan control process, it is recommended that the City of Ottawa is contacted to obtain boundary conditions associated with the final calculated water demands. At that time, further review will take place to ensure that pressures of the water network remain within the pressure ranges outlined in Table 2 above.

3.2 Fire Protection

The estimated fire flow for the proposed buildings was calculated in accordance with *ISTB-2018-02*. The following parameters were assumed during this functional serviceability review, with detailed input from the building Architect to be requested during the Site Plan Design stage.

- Type of construction Wood Frame Construction.
- Occupancy type Limited Combustibility; and
- Sprinkler Protection Standard Sprinkler System.

The estimated fire flow demand was estimated to be 10,000 L/min, see Appendix C for details

There are three (3) existing fire hydrants in close proximity to the proposed buildings that are available to provide fire flow demands of approximately 17 000 L/min. Refer to *Appendix C* for fire

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hydrant locations. Table 3 below summarizes the aggregate fire flow of the contributing hydrants in close proximity to the proposed development based on Table 18.5.4.3 of *ISTB-2018-02*.

Table 3: Fire Protection Summary Table

| Building | Fire Flow Demand (L/min) | Fire Hydrants(s) within 75m | Available Combined Fire Flow (L/min) |
|-------------------------------|---|--------------------------------|--|
| Proposed 6 Storey Building | TBD During Detailed Design (assumed 10 000 L/min) | 3 | (3 x 5678) = 17 000 |

The total available fire flow from contributing hydrants is equal to approximately 17,000 L/min which is sufficient to provide adequate fire flow for the proposed development. A certified fire protection system specialist will need to be employed to design the building's fire suppression system and confirm the actual fire flow demand.

4 SANITARY SERVICE

There is an existing 900mm combined sewer and a 750mm x 900mm Combined Sewer within Waverly Street running across the frontage of the subject site ultimately conveying flow in the easterly direction.

The post development total sanitary effluent was calculated to be is **0.77 L/s** as a result of proposed residential population and a small portion of infiltration. Refer to Appendix *D* for further information on the calculated sanitary flows.

Based on existing as-built information (Refer to Appendix E), the existing 900mm dia. Combined sewer is sloped at 0.25% and is calculated to have a maximum capacity of approximately 905 L/s. The proposed increase in total wastewater flow from the increased population represents less than 0.1% of existing maximum capacity in the single combined sewer. Therefore, it is anticipated that the existing local sewer network has sufficient capacity to accommodate the proposed development.

5 STORMWATER MANAGEMENT

The existing site has a large portion of impervious area which will be slightly increased in post development conditions.

At the time of detailed design, stormwater quantity will be dealt with to ensure that the post-development runoff discharge from the site is controlled to no exceed the 2-year storm release rate. During the pre consultation meeting, it was commented on that strict stormwater management principles will be required for this site.

In order to control the quantity of runoff from the site, storage within the building (cistern) or the rooftop is intended.

Additionally, it is noted that the proposed site plan results in only a slightly higher runoff coefficient than that in the pre development conditions. This is summarized in table 4 below.

Table 4: Pre-Development and Post-Development Runoff Coefficients

| Pre-D | evelopment Condi | tions | Post-Deve | elopment Condition | ns |
|--------------------------|-------------------------------------|---------------------|--------------------|-------------------------------------|---------------------|
| Grass Area (C=0.2) | Building Area/Asphalt (C=0.9) | Combined C Value | Grass Area (C=0.2) | Building Area/Asphalt (C=0.9) | Combined C Value |
| 55 sqm | 556 sqm | 0.837 | 25 sqm | 586 | 0.871 |

6 CONCLUSIONS AND SERVICEABILITY CONSIDERATIONS

This evaluation is limited to assessing the technical feasibility of servicing the site described within this document to support an Official Plan Amendment and Zoning By-law Amendment.

Based on the forgoing the conclusions in relation to the serviceability of the site are as follows:

Water:

- Domestic demands from the proposed 32-unit apartment building are expected to be in the range of **0.19** L/s for the Average daily demand, 1.36 L/s for the maximum daily and 14.76 L/s for maximum hourly.
- 3 fire hydrants within 75m are expected to provide the required fire flow.
- During detailed design, pressures available along Waverly street are to be investigated as it will be serviced from the existing 200mm PVC watermain.

Sanitary Sewage:

- The post development total sanitary effluent was calculated to be is 0.77 L/s because
 of proposed residential population and a small portion of infiltration.
- The flow from this development would make up a very small percentage (Less than 0.1%) of the flow capacity of the existing 900mm combined sewer in Waverly street.

Stormwater

- The small increase in impervious area along with strict quantity control requirements outlined by the City of Ottawa will require the site to implement a stormwater quantity management system.
- Storage could be achieved via a cistern or rooftop storage on site to ensure flow is regulated to not release more than the allowable release rate.

7 CLOSURE

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Prepared by:

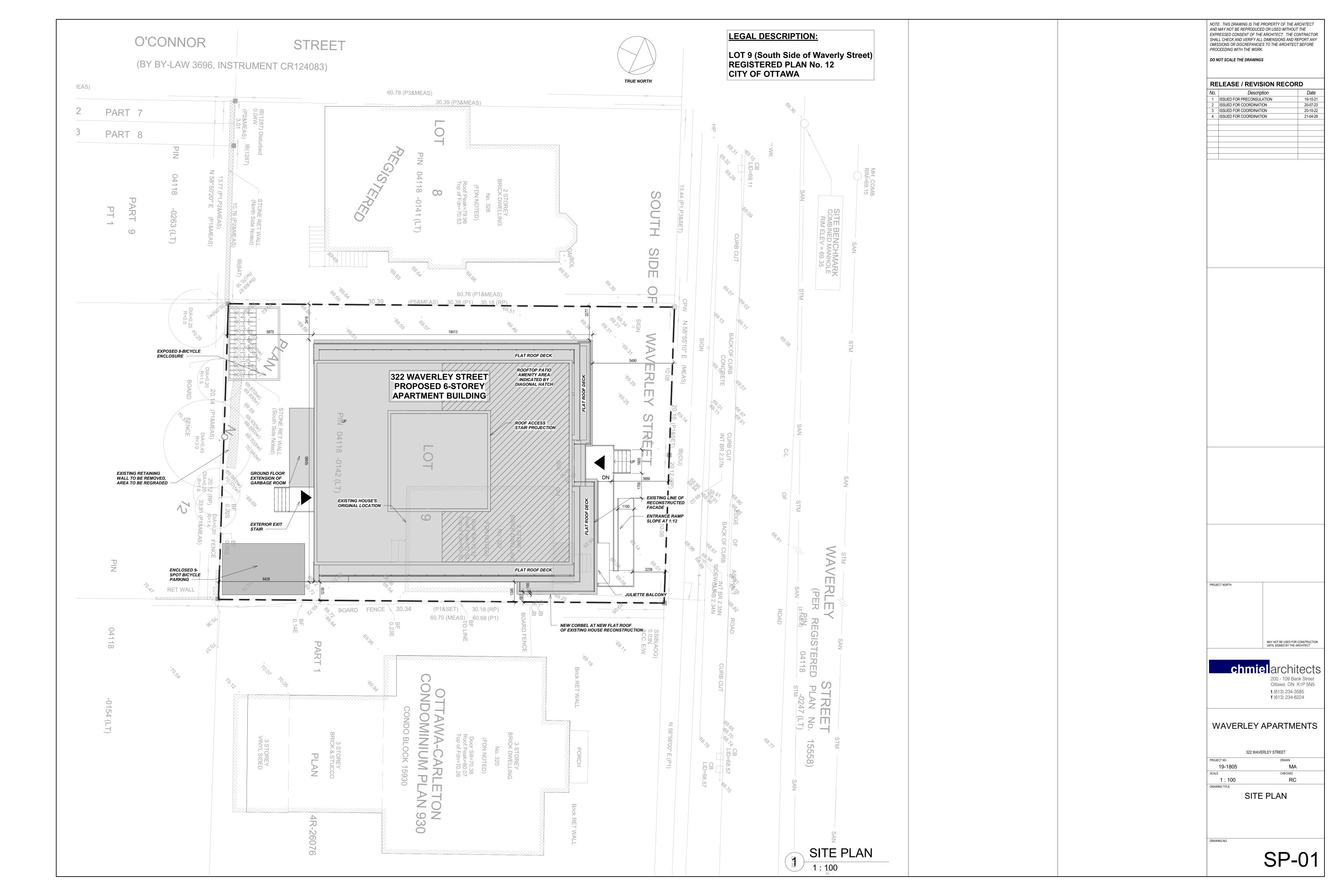
LRL Associates Ltd.

Virginia Johnson, P. Eng.



APPENDIX A Concept Site Plan

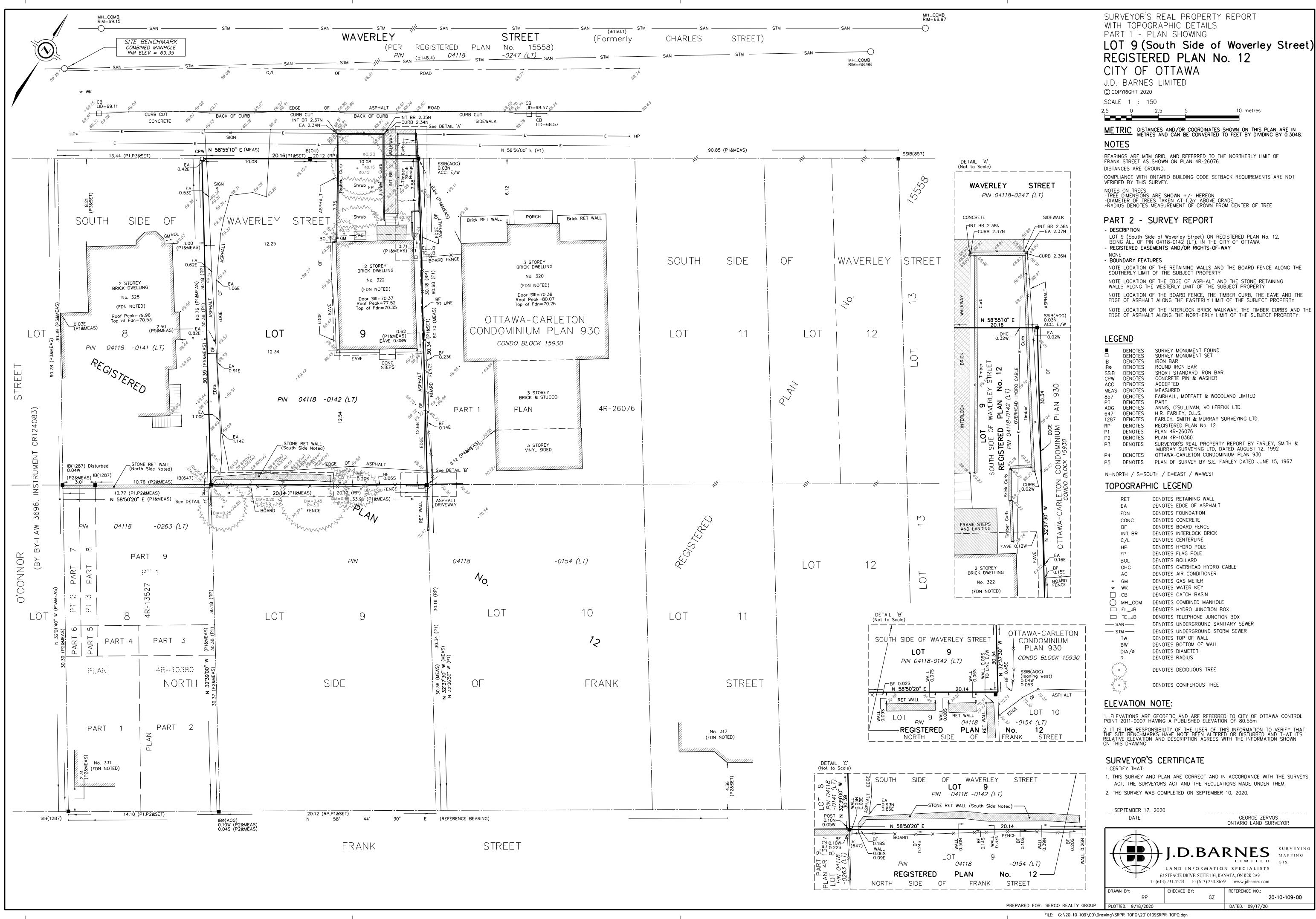




APPENDIX B

Topography Survey

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APPENDIX C Water Servicing Calculations





Water Supply Calculations

LRL File No. 190523 Date May 10, 2021 Virginia Johnson Prepared by

Residential Demand based on the City of Ottawa Design Guidelines-Water Distribution, 2010

| Unit Type | Persons Per Unit | Number of Units | Population |
|-------------------------------|------------------|-----------------|------------|
| Average Residential Apartment | 1.8 | 32 | 57.6 |
| - | - | | |
| | Total | 32 | 57.6 |

Average Water Consumption Rate 280 L/c/d

Average Day Demand 16,128 L/d 0.19 L/s Maximum Day Factor 7.3 (MOE Table 3-3) **Maximum Daily Demand** 116,966 L/d 1.35 L/s (MOE Table 3-3) Peak Hour Factor 10.9 **Maximum Hour Demand** 1,275,636 L/d 14.76 L/s

Water Service Pipe Sizing

Q = VAWhere: V = velocity

A = area of pipe

Q = flow rate

Assuming a maximum velocity of 1.8m/s, the diameter of pipe is calculated as:

Minimum pipe diameter (d) = $(4Q/\pi V)^{1/2}$

> 0.102 = m 102 mm

Proposed pipe diameter (d) = 150 mm

> 6 Inches



Fire Flow Calculations

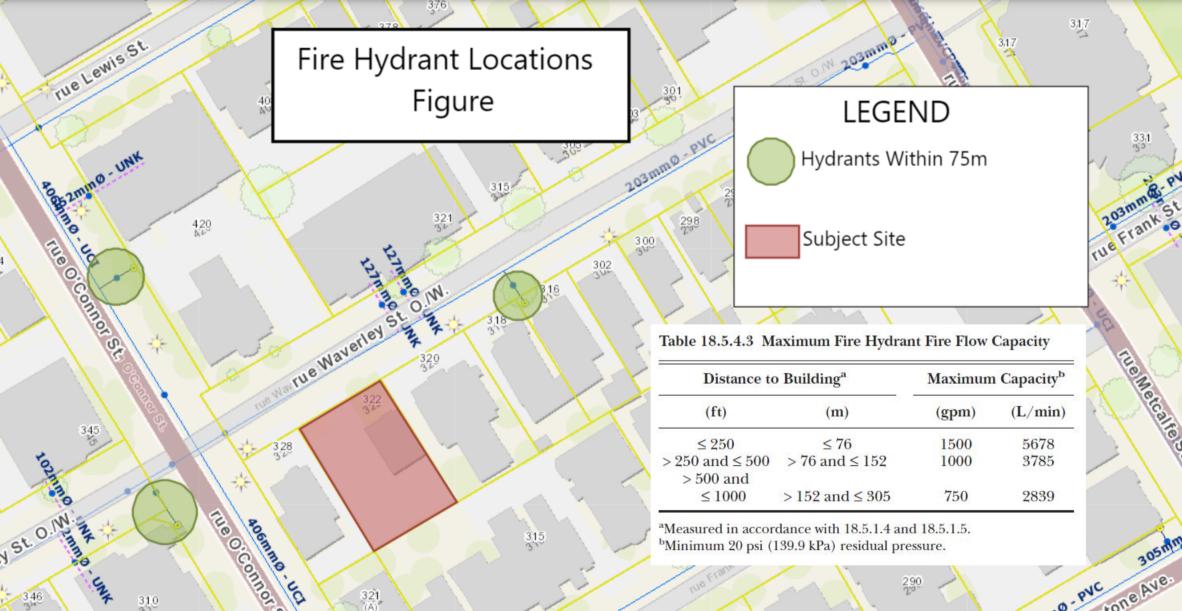
LRL File No. 190523

Date May 10th, 2021

Method Fire Underwriters Survey (FUS)

Prepared by Virginia Johnson

| Step | Task | Term | Options | Multiplier | Choose: | Value | Unit | Fire Flow | | | | |
|--|------------------------------------|---|---|-------------|--|--------------|----------|-----------|--|--|--|--|
| | | | Structural Framing M | aterial | | | | | | | | |
| | | | Wood Frame | 1.5 | | | | | | | | |
| | Choose frame used for building | Coefficient C | Ordinary Construction | 1.0 | | | | | | | | |
| 1 | | related to the type of | Non-combustible construction | 0.8 | Ordinary Construction | 1 | | | | | | |
| | building | construction | Fire resistive construction <2 hrs | 0.7 | | | | | | | | |
| | | | Fire resistive construction >2 hrs | 0.6 | | | | | | | | |
| Floor Space Area (A) | | | | | | | | | | | | |
| 2 | | Total area (ASSUMED BASED ON PRELIMINARY ARCH DWGS) 1,200 | | | | | | | | | | |
| 3 | Obtain fire flow before reductions | Required fire flow Fire Flow = 220 x C x A ^{0.5} | | | | | | 7,621 | | | | |
| Reductions or surcharge due to factors affecting burning | | | | | | | | | | | | |
| | Choose combustibility of contents | | Non-combustible | -25% | | | | | | | | |
| | | 0 | Limited combustible | -15% | | | | | | | | |
| 4 | | Occupancy hazard reduction or surcharge | Combustible | 0% | Limited combustible | -15% | L/min | 6,478 | | | | |
| | | Teduction of surcharge | Free burning | 15% | | | | | | | | |
| | | | Rapid burning | 25% | | | | | | | | |
| | | | Full automatic sprinklers | -30% | False | 0% | | | | | | |
| 5 | Choose reduction for sprinklers | Sprinkler reduction | Water supply is standard for both the system and fire department hose lines | -10% | True | -10% | L/min | 5,830 | | | | |
| | | | Fully supervised system | -10% | False | 0% | | | | | | |
| | | | North side | 20.1 to 30m | 10% | | | | | | | |
| 6 | Choose separation | Exposure distance | East side | 3.1 to 10m | 20% | | L/min | 9,911 | | | | |
| 0 | Choose separation | between units | South side | 3.1 to 10m | 20% | | L/111111 | 9,911 | | | | |
| | | | West side | 3.1 to 10m | 20% | 70% | | | | | | |
| | | | Net required fire fl | ow | | | | | | | | |
| | Obtain fire flow, | | | Minimum | required fire flow rate (rounded to near | rest 1000) | L/min | 10,000 | | | | |
| 7 | duration, and volume | | | | Minimum required fire | e flow rate | L/s | 166.7 | | | | |
| | | | | | Required duration of | of fire flow | hr | 2 | | | | |



APPENDIX D Sanitary Service Calculations





LRL File No. 190523

Project: Apartment Building
Location: 322 Waverly
Date: May 10, 2021

Sanitary Design Parameters

Industrial Peak Factor = as per Appendix 4-B = 7

Extraneous Flow = 0.33L/s/gross ha

Pipe Design Parameters

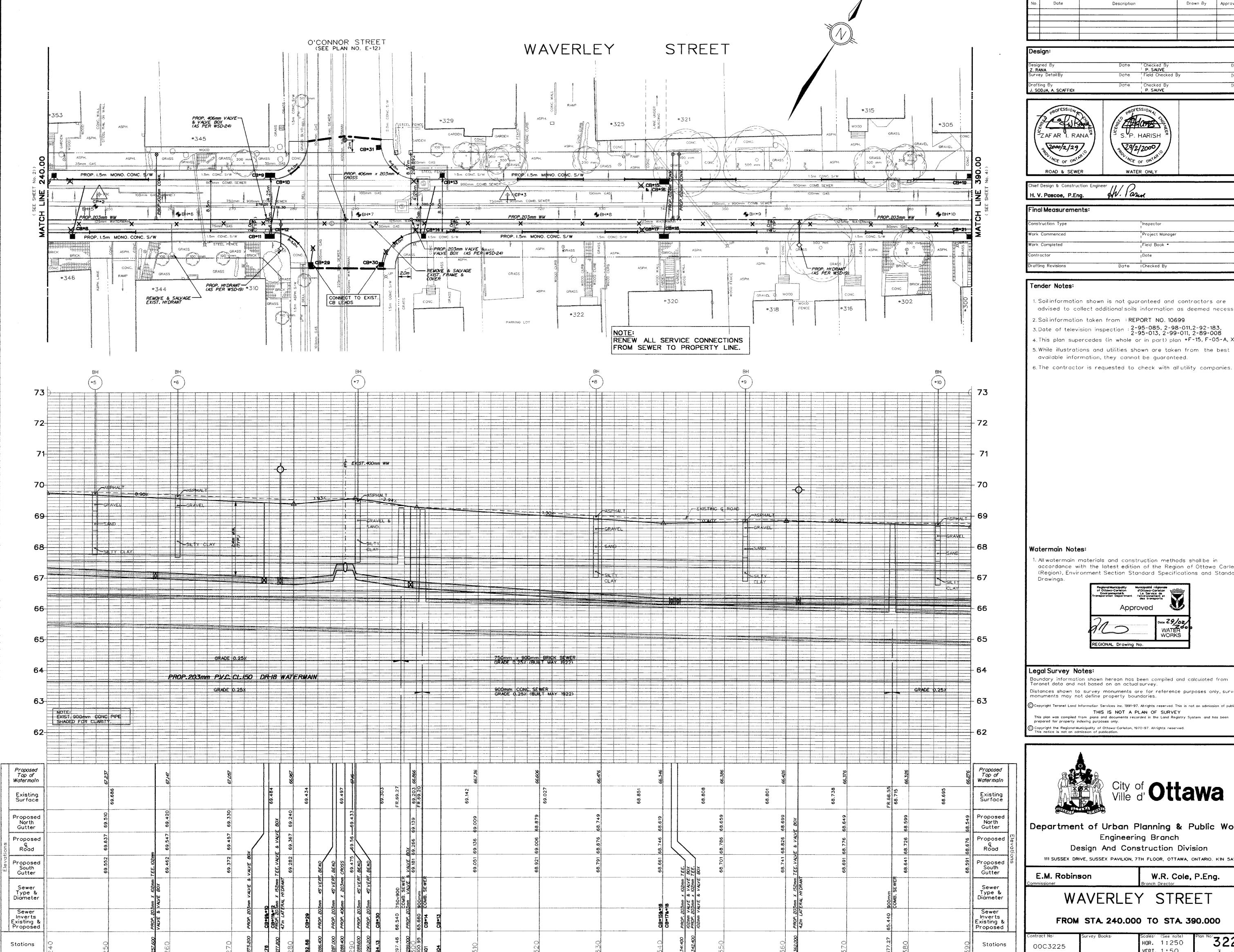
Minimum Velocity = 0.60 m/s Manning's n = 0.013

Average Daily Flow = 280 L/p/day
Commercial & Institutional Flow = 50000 L/ha/day
Light Industrial Flow = 35000 L/ha/day
Heavy Industrial Flow = 55000 L/ha/day
Maximum Residential Peak Factor = 4.0
Commercial & Institutional Peak Factor = 1.5

| | LOCATION | | | RESIDEN | TIAL AREA | AND POPL | JLATION | | COMM | ERCIAL | I | NDUSTRIA | ۸L | INSTITU | JTIONAL | C+I+I | INFILTRATION PIPE TOTAL | | | | | | | | | | |
|--------|--------------------|---------------|------------|---------------|--------------|--------------|---------|-------|-------|-----------|---------|----------|------------|-----------|---------|-------|-------------------------|------|---------|--------|---------|---------|--------------|----------|--------|--------|---|
| | | | AREA | | CUMM | ULATIVE | PEAK | PEAK | AREA | ACCU. | AREA | ACCU. | PEAK | AREA | ACCU. | PEAK | TOTAL | | INFILT. | | LENGTH | DIA. | SLOPE | | CAP. | VEL. | |
| STREET | FROM MH | TO MH | (Ha) | POP. | AREA | POP. | FACT. | FLOW | (Ha) | AREA | (Ha) | AREA | FACT. | (Ha) | AREA | FLOW | AREA | AREA | FLOW | (l/s) | (m) | (mm) | (%) | MATERIAL | (FULL) | (FULL) | |
| | | | (ι ια) | | (Ha) | FOF. | 17.01. | (l/s) | (Πα) | (Ha) | (114) | (Ha) | 17.01. | (Πα) | (Ha) | (l/s) | (Ha) | (Ha) | (l/s) | (,, 0) | (111) | (11111) | (70) | | (l/s) | (m/s) | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| SITE | PROP. BLDG | EX. SAN | 0.060 | 58.0 | 0.06 | 58.0 | 4.0 | 0.75 | 0.000 | 0.000 | 0.00 | 0.00 | 7.0 | 0.0 | 0.0 | 0.00 | 0.06 | 0.06 | 0.02 | 0.77 | 10.0 | 150 | 2.00% | PVC | 21.54 | 1.22 | |
| | | | | | | | | | | | | _ | | Designed: | : | | | | | | | PR | OJECT: | | | | |
| NOTES | Existing inverts a | and slopes ar | e estimate | d. They are t | o be confirr | ned on-site. | | | | | | | | | VJ | | | | | | | Apartm | ent Building | 9 | | | |
| | | | | | | | | | | | | _ | | Checked: | | | | | | | | LOC | CATION: | | | | |
| | | | | | | | | | | | | | VJ | | | | | | | 322 | Waverly | | | | | | |
| | | | | | | | | | | Dwg. Refe | erence: | | File Ref.: | | | | Date: | | | | | Shee | No. | | | | |
| | | | | | | | | | | | | | | | N/A | | | 190 | 523 | | | 202 | 1-05-10 | | | 1 o | 1 |

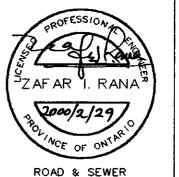
APPENDIX E Waverly Street Plan and Profile Drawing

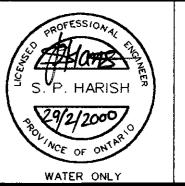
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Checked By
P. SAUVE
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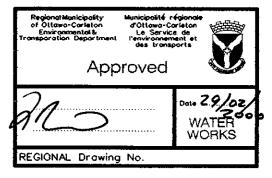
Chief Design & Construction Engineer W. Pascoe, P.Eng.

| Final Measurements: | | | |
|---------------------|------|-----------------|--|
| Construction Type | | Inspector | |
| Work Commenced | | Project Manager | |
| Work Completed | | Field Book • | |
| Contractor | | Date | |
| Drafting Revisions | Date | I Checked By | |

Tender Notes:

. Soil information shown is not guaranteed and contractors are advised to collect additional soils information as deemed necessary. 2. Soil information taken from : REPORT NO. 10699 4. This plan supercedes (in whole or in part) plan #F-15, F-05-A, X-03B 5. While illustrations and utilities shown are taken from the best available information, they cannot be guaranteed.

. All watermain materials and construction methods shall be in accordance with the latest edition of the Region of Ottawa Carleton (Region), Environment Section Standard Specifications and Standard



Legal Survey Notes:

Boundary information shown hereon has been compiled and calculated from

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City of Ottawa

W.R. Cole, P.Eng.

Department of Urban Planning & Public Works Engineering Branch

Design And Construction Division 111 SUSSEX DRIVE, SUSSEX PAVILION, 7TH FLOOR, OTTAWA, ONTARIO. KIN 5A1

E.M. Robinson

WAVERLEY STREET

FROM STA. 240.000 TO STA. 390.000

HOR. 1:250 00C3225 VERT. 1:50