

July 2, 2026
PG8011-LET.01 Revision 3



Forum Investment and Development Corporation

181 Bay Street
East Podium – 2nd Floor
Toronto, Ontario
M5J 2T3

Attention: Stuart Cooper

Subject: Geotechnical Desktop Slope Stability Assessment
Proposed Residential Development
530 Brisebois Cres. & Part of 265 Centrum Blvd.
Ottawa, Ontario

Consulting Engineers

9 Auriga Drive
Ottawa, Ontario
K2E 7T9
Tel: (613) 226-7381

Geotechnical Engineering
Environmental Engineering
Hydrogeology
Materials Testing
Building Science
Rural Development Design
Temporary Shoring Design
Retaining Wall Design
Noise and Vibration Studies

patersongroup.ca

Dear Stuart,

Further to your request and authorization, Paterson Group (Paterson) prepared this letter report to provide our geotechnical desktop slope stability assessment of the existing escarpment slope south of St. Joseph Boulevard, in support of the proposed residential development at the subject site. In particular, the primary purpose of the geotechnical review is to assess the stability of the existing slope at the subject site in relation to the proposed shoreline rehabilitation works to be completed along the northeastern property boundary.

1.0 Background Information

The desktop slope stability analysis described in the present report was undertaken in response to the April 2026 Guidance letter (referred to as “Guidance letter” in this report) provided by the City of Ottawa outlining requirements for assessing the presence of clay soils susceptible to retrogressive landslides within the nearby escarpment slope. The present report was prepared to investigate the potential exemption for escarpment slopes, particularly the existing escarpment slope located along the southern side of St. Joseph Boulevard, approximately 180 m south of the proposed residential development.

In summary, the Guidance letter recommends that the following criteria be determined prior to confirming whether an escarpment slope may be deemed exempt from the minimum field and laboratory requirements outlined in the Guidance letter: *demonstrates that the escarpment slope meets the minimum factors of safety as follows:*

- ☐ ≥ 1.5 under static conditions
- ☐ ≥ 1.1 under seismic conditions
- ☐ ≥ 1.3 under post-seismic conditions



The following sections present the methodologies, assumptions, and existing data used in the analysis, along with a summary and discussion of the results of the desktop slope stability analysis completed by Paterson. The assessment was prepared based on the methodology outlined in the Guidance letter and based on consultations with the City of Ottawa Development Review and Rideau Valley Conservation Authority (RVCA) staff.

2.0 Proposed Development

Based on our review of available reports and site plan documents, it is understood that the proposed development will consist of a multi-storey community centre building and several mid- and high-rise residential buildings. It is understood that the proposed buildings will be provided with one level of underground parking. The approximate site location is shown in the attached Figure 1 – Key Plan.

3.0 Existing Geotechnical Information

3.1 Surface Conditions and Topography

Escarpment Slope – South Side of St. Joseph Boulevard

Paterson field personnel completed a walkthrough and visual assessment of the subject escarpment slope along the south side of St. Joseph Boulevard on April 28, 2026. At the time of the site visit, the escarpment slope profile was observed to generally contain dense, mature, medium-sized trees, with some fallen trees noted throughout the area. The trees were generally observed to extend vertically from the existing ground surface with no evidence of curving or bending at their bases. Such curvature is commonly associated with gradual slope movement or instability, where trees tilt as the slope slowly displaces and subsequently correct their vertical growth, resulting in a curved trunk base. Gravel pedestrian pathways were noted throughout, providing access from the roadways located at the top of escarpment slope to St. Joseph Boulevard below.

Several segmental block retaining walls were noted at the base of the escarpment slope, while a stacked armor stone retaining wall was noted further up the slope along the south side of Prestone Drive. It should be noted that the majority of the segmental block retaining wall located at the base of the escarpment slope between Vieux-Silo Street and Tenth Line Road was noted to be founded over a limestone bedrock outcrop. The limestone bedrock outcrop was observed extending eastward through and beyond the intersection of Tenth Line Road and St. Joseph Boulevard.

Paterson completed a review of the general topography of the subject escarpment slope based on available LiDAR topographic data (2015). The steepest sections of the subject escarpment slope were noted to generally extend from top of slope to bottom of slope geodetic elevation 86 m to a bottom of slope geodetic elevation 72 m. The inclination was generally noted to range between 9° and 16.5° (16% and 30%) throughout the unsupported sections of the escarpment slope.



The existing average ground surface elevation across the top of the escarpment slope was noted to be relatively flat and at-grade with the surrounding areas and streets, ranging between approximate geodetic elevations 87 to 88 m. The ground surface elevation across the bottom of the escarpment slope was noted to slope gently east to west along St. Joseph Boulevard between approximate geodetic elevations 75 to 70 m, respectively.

North of Escarpment Slope – North Side of St. Joseph Boulevard & Subject Site

The existing average ground surface elevation north of the subject escarpment slope (towards and including the subject site), were noted to downwards slope gently northward from St. Joseph Boulevard between approximate geodetic elevations 72 to 65 m. The properties to the north of the subject escarpment slope were noted to include one- and two-storey commercial buildings, multi storey residential and municipal buildings, parking lots, and landscaped areas.

The subject site parcels, 530 Brisebois Crescent and Part of 265 Centrum Boulevard, were noted to be vacant. The 530 Brisebois Crescent parcel as noted to consist of vacant grassed land with the southern half of the parcel containing dense medium sized trees, and was noted to border Highway 174 to the immediate north. The Part of 265 Centrum Boulevard parcel was noted to consist of grassed land with several medium to large sized trees sparsely placed throughout.

3.2 Escarpment Slope Subsurface Conditions

Overburden

Paterson completed a review of several borehole log reports completed historically by Paterson and by others that were previously advanced within the top side of the subject escarpment slope (south of St. Joseph Boulevard). Further, Paterson completed a review of the borehole log reports advanced by others north of the escarpment slope and within the subject site boundary. Generally, based on our review and our knowledge of the area, the subsurface soil profile typically consists of topsoil underlain by a layer of desiccated brown silty clay crust, further underlain by a layer of unweathered grey silty clay, further underlain by a glacial till deposit and underlying bedrock formation.

Bedrock

Based on available geological mapping, the bedrock below the subject escarpment slope consists of limestone of the Bobcaygeon formation with an approximate overburden drift thickness ranging between 0 to 15 m. *It should be noted that geological mapping is approximate and published data may vary from in-situ conditions.* Further, it should be noted that the slope stability analysis presented herein does not rely on the bedrock depths from the above-noted available geological mapping. Reference should be made to the slope stability section of this report for more information and clarification.



Groundwater

Groundwater levels were previously measured in piezometers and monitoring wells within the boreholes from the surrounding investigations completed at nearby properties along the high side of the subject escarpment slope. The long-term groundwater level can also be estimated based on moisture levels, consistency, and coloring of the recovered samples. Based on the existing groundwater information and our knowledge of the groundwater within the nearby properties, the long-term groundwater level is estimated to be between **3 and 5 m** below the existing ground surface. However, for slope stability analysis purposes, the long-term groundwater level has been conservatively considered to be **1.5 m** below the existing ground surface. Reference should be made to the slope stability analysis section of the present report.

4.0 Geotechnical Desktop Slope Stability Analysis

4.1 Considerations and Assumptions

Background Information

The revised slope stability analysis presented herein was completed following discussions with the RVCA, during which the technical review comments and report contents were discussed in further detail. The revised analysis considers the feedback provided by each party with respect to the relevant discussion items.

Existing Slope Profile Topography

The slope stability analysis was completed using the existing slope topography derived from available LiDAR topographic data (2015). To verify the accuracy and suitability of the LiDAR data for the assessment, Paterson completed topographic survey shots during our site visit on April 29, 2026, using a high-precision GPS unit. The field survey observations and spot check survey shots generally confirmed the accuracy of the LiDAR-derived slope profile relied upon in the analysis.

Subsurface Information

The slope stability analysis was completed using available nearby and surrounding subsurface investigation data, including historical borehole logs and in-situ shear strength testing results, to assess the anticipated stratigraphy, soil strength parameters, and bedrock profile along the analyzed slope cross-section. As intrusive field investigations were not completed at the exact analyzed location, the subsurface profile and associated soil strength parameters incorporated into the analysis were selected conservatively to provide a reasonable worst-case assessment of slope stability conditions.



Paterson completed a review of several historical borehole investigation reports from test holes previously advanced south of the high side of the subject escarpment slope. The reviewed borehole data generally demonstrated consistent subsurface conditions and soil strength characteristics throughout the local silty clay deposit, with no significant stratigraphic discontinuities identified between investigation locations. Based on this consistency, Paterson considers the available nearby subsurface information to be representative and suitable for desktop slope stability assessment purposes.

The analysis approach adopted herein incorporates conservative assumptions regarding both soil strength and bedrock geometry to account for the limitations associated with the absence of site-specific intrusive investigations at the analyzed cross-section. Reference should be made to Drawings *PG8011-1 – Test Hole Location Plan* and *PG8011-2 Extrapolated Borehole Data Cross-Section* attached to the present report for the approximate borehole locations considered in our assessment.

Subsoil Stratigraphy

Brown Silty Clay

Based on the reviewed nearby borehole data, the upper brown silty clay crust was noted to exhibit a hard to very stiff consistency, with Standard Penetration Test (SPT) N-values generally ranging from approximately 12 to 16 blows per 300 mm and undrained shear strength values ranging between approximately 100 and 240 kPa. This layer was noted to generally extend from the existing ground surface to an approximate depth of 2 m below the existing ground surface. Beneath the crust, a transitional very stiff to stiff brown silty clay layer was noted, generally characterized by SPT N-values below 10 and undrained shear strength values ranging between approximately 50 and 210 kPa. This layer was noted to extend from approximately 2 to 3 m below the existing ground surface.

Grey Silty Clay

The underlying grey silty clay deposit was noted to exhibit a stiff to firm consistency with undrained shear strength values generally ranging between approximately 25 and 97 kPa. This layer was noted to extend from approximately 3 to 5 m below the existing ground surface. Beneath this layer, a firm grey silty clay was noted, generally characterized by undrained shear strength values ranging between approximately 22 and 52 kPa. This layer was noted to extend from approximately 5 to 11 m below the existing ground surface.

Glacial Till and Bedrock

Glacial till deposits were identified within portions of the reviewed historical test hole and well record datasets and were intentionally omitted from the modeled stratigraphy to maintain a conservative approach and to avoid incorporating additional stabilizing benefit associated with the presence of higher-strength materials.



Strength Parameter Selection Background Information

The soil strength parameter selection was completed conservatively to reflect potential variability in subsurface conditions across the subject silty clay deposit. The undrained shear strength profile was developed by grouping the undrained shear strength testing results into geologically consistent layers based on the observed changes in physical characteristics and measured shear strength values. For each layer, the average undrained shear strength was determined from the available nearby field vane data.

To provide a conservative approach that reflects the observed variability within each layer, the design undrained shear strength was selected by subtracting the standard deviation from the calculated mean strength for each layer. Further, given the absence of undrained shear strength values deeper than 11 m, the undrained shear strength of the grey silty clay below 11 m was assumed to not increase with depth but rather was assigned the same value as the deepest available undrained shear strength data (i.e. the same as the firm grey silty clay layer). This approach provides a representative lower-bound estimate of the available undrained shear strength data while accounting for potential variability throughout the deposit.

The post-seismic slope stability assessment was completed using a strain-softened undrained shear strength approach to account for the potential cyclic degradation of the silty clay soils following seismic loading. The assessment methodology considers the post-cyclic degradation approach presented by Ajmera (2015)¹, while considering its applicability based on published cyclic loading studies and literature on sensitive Champlain Sea clays in eastern Ontario and Quebec, including Mitchell and King (1977)² and LeBoeuf et al. (2016)³.

The Canadian Foundation Engineering Manual (CFEM) (2023) criterion regarding the potential use of remoulded shear strength was also considered. Although the subject clay exhibits high sensitivity, remoulded shear strength represents a fully de-structured soil condition associated with large strain deformation and substantial breakdown of the natural clay fabric. The available Canadian-based literature indicates that cyclic degradation of Champlain Sea clay is progressive and governed by stress level, pore pressure response, and the accumulation of cyclic strains, rather than an immediate transition to a fully remoulded condition.

Of particular relevance, LeBoeuf et al. (2016) reported that Rigaud clay (located roughly 100km east of Ottawa within the historical Champlain Sea) subjected to approximately 150 loading cycles at a cyclic deviatoric stress equal to approximately 80% of the sample's undrained shear strength did not exhibit a significant reduction in subsequent peak undrained shear strength.

¹ Ajmera, Beena. (2015). Factors Influencing the Post-Earthquake Shear Strength.

² Mitchell and King. (1977). Cyclic loading of an Ottawa area Champlain Sea Clay.

³ LeBoeuf, Denis. (1977). Cyclic Softening and Failures in Sensitive Clays and Silts.



Considering the available cyclic loading research, the limited number of significant stress cycles anticipated during a design earthquake in eastern Ontario, and the calculated seismic factor of safety exceeding 1.1 (as discussed in subsequent sections of the present report), widespread deformation and the large strains required to fully de-structure the clay deposit are not anticipated. As such, the use of fully remoulded shear strength for the post-seismic assessment is not considered representative of the anticipated in-situ response for the subject escarpment slope.

Accordingly, a post-seismic degradation ratio of $\delta = 0.67$ was adopted and was considered to provide a conservative representation of a degraded, but not fully remoulded, undrained shear strength condition for the post-seismic slope stability assessment.

Strength Parameters

The soil strength parameters presented in Table 1, 2 and 3 were considered for the slope stability analysis under Static, Seismic, and Post-Seismic conditions, respectively.

Table 1 – Effective Stress Soil Parameters (Static Analysis)			
Soil Layer	Unit Weight (kN/m ³)	Friction Angle (degrees)	Cohesion (kPa)
Brown Silty Clay Crust	17	33	5
Brown Silty Clay	17	33	5
Stiff to Firm Grey Silty Clay	16	33	10
Firm Grey Silty Clay	16	33	10
Bedrock	21	-	-

Table 2 – Total Stress Soil Parameters (Seismic Analysis)			
Soil Layer	Unit Weight (kN/m ³)	Friction Angle (degrees)	Cohesion (kPa)
Brown Silty Clay Crust	17	0	157
Brown Silty Clay	17	0	75
Stiff to Firm Grey Silty Clay	16	0	32
Firm Grey Silty Clay	16	0	28
Bedrock	21	-	-

**Table 3 – Residual Total Stress Soil Parameters (Post-Seismic Analysis)**

Soil Layer	Unit Weight (kN/m ³)	Friction Angle (degrees)	Cohesion (kPa)
Brown Silty Clay Crust	17	0	105
Brown Silty Clay	17	0	50
Stiff to Firm Grey Silty Clay	16	0	22
Firm Grey Silty Clay	16	0	19
Bedrock	21	-	-

Bedrock Profile

The slope stability analysis considers a bedrock surface elevation tapering from north to south between approximate elevation 66.9 m beyond the toe of slope (towards the north), and 56.6 m beyond the top of slope (towards the south). These interpolated bedrock surface elevations are based on nearby borehole rock coring data obtained from borehole BH25-08 completed by others at 265 Centrum Boulevard (located within the subject site to the north of the subject escarpment slope toe) and nearby Dynamic Cone Penetration Testing (DCPT) refusal obtained from borehole BH 5-23 previously completed to the south of the subject escarpment slope by Paterson. Nearby Ontario Well Record data was reviewed as to confirm the general bedrock surface depth to support the suitability of the DCPT refusal depth.

It should be noted that evidence of localized upward undulations in the bedrock surface is present based on visible bedrock outcrops along St. Joseph Boulevard between Vieux-Silo Road and Tenth Line Road, as well as south of the St. Joseph Boulevard and Tenth Line Road intersection along Tenth Line Road. Similar indications of shallower bedrock conditions were also identified within nearby Ontario Geological Survey (OGS) borehole 804200 located east of the analyzed escarpment near the intersection of Tenth Line Road and St. Joseph Boulevard.

Despite these observations, a relatively uniform and deeper bedrock profile was assumed within the analysis to maintain a conservative assessment and eliminate any potential benefit associated with shallower bedrock conditions. Visible bedrock outcrops and well records locations considered in the analysis are presented on Drawing PG8011-1 – Test Hole Location Plan attached to the present report.

Groundwater

The groundwater table depth incorporated into the slope stability analysis was assigned a depth limited by foundation drainage systems and hardscaping and considered as **1.5 m** below the existing ground surface, which is considered the minimum founding depth below ground surface for heated occupied structures to be founded below the local depth of frost migration.



4.2 Slope Stability Analysis

Analysis Methodology

One (1) cross-section (Section A) was studied as a worst-case scenario. The location of Section A was selected to model the slope in its unsupported state rather than relying on the potential stability increase that the various existing retaining walls along the subject escarpment slope toe would provide. The cross-section location is presented on Drawing PG8011-1 – Test Hole Location Plan, attached to the present report.

The slope stability analysis was carried out using SLIDE, a computer program which permits a two-dimensional slope stability analysis using several methods including Bishop's method, which is a widely used and accepted analysis method. The program calculates a factor of safety, which represents the ratio of the forces resisting failure to those favoring failure.

Theoretically, a factor of safety of 1.0 represents a condition where the slope is stable. However, due to intrinsic limitations of the calculation methods and variability of the subsoil and groundwater conditions, a factor of safety greater than one is usually required to ascertain the risks of failure is acceptable. A minimum factor of safety of 1.5 is generally recommended under static conditions where the failure of the slope would endanger permanent structures. A factor of safety of 1.1 is considered to be satisfactory for stability analysis considering seismic loading.

Analysis Results

Static Loading

The results of the existing escarpment slope under static loading conditions at Section A are shown in Figure 1A attached to the present report. The overall slope stability factor of safety for the subject section was found to be greater than 1.5. As such, a stable slope allowance setback is not required for the subject escarpment slope under static conditions.

Seismic Loading

An analysis considering seismic loading was also completed as part of the slope stability analysis. A horizontal seismic loading acceleration, K_h , of 0.2g was considered for the analyzed section. The results of the existing escarpment slope under seismic loading conditions at Section A are shown in Figure 1B attached to the present report. The overall slope stability factor of safety for the subject section was found to be greater than 1.1. As such, a stable slope allowance setback is not required for the subject escarpment slope under seismic conditions.



Post-Seismic Loading

An analysis considering post-seismic loading was also completed as part of the slope stability analysis. Reduced undrained shear strength values were considered for the analyzed section, as discussed in previous sections of the present report. The results of the existing escarpment slope under post-seismic loading conditions at Section A are shown in Figure 1C attached to the present report. The overall slope stability factor of safety for the subject section was found to be greater than 1.3. As such, a stable slope allowance setback is not required for the subject escarpment slope under post-seismic conditions.

Sensitivity Analysis

A series of sensitivity analyses were completed as part of the revised slope stability assessment to evaluate the influence of uncertainty associated with the interpreted subsurface conditions, groundwater levels and adopted undrained shear strength parameters. These analyses were completed to support the conservative approach of the slope stability model, recognizing that subsurface conditions were interpolated from the available borehole data in the vicinity of the analyzed cross-section.

High-Groundwater Scenario

A high groundwater sensitivity analysis was completed with the groundwater table placed at the existing ground surface to assess the potential influence of adverse groundwater conditions that may occur during periods of significant snowmelt or precipitation. The results of the analyses completed using both groundwater conditions indicated that the calculated factors of safety remained above the minimum values required by the Guidance letter for the static, seismic, and post-seismic loading conditions.

Undrained Shear Strength

Sensitivity analyses were also completed to assess the influence of the adopted undrained shear strength profile. As mentioned in previous sections of this report, the undrained shear strength values were developed from a statistical interpretation of the available field vane test data and were conservatively selected as the mean value less one standard deviation for each interpreted layer of the deposit. To further assess the reliability of the stability results, an additional sensitivity analysis was completed using an additional 10% reduction in the already conservatively selected undrained shear strength values for the seismic and post-seismic loading conditions. The results of this analysis indicated that the calculated factors of safety for each loading case remained above the minimum required factors of safety of 1.1 and 1.3 for seismic and post-seismic loading conditions, respectively.



The sensitivity analyses demonstrate that the conclusions of the slope stability assessment are not significantly affected by reasonable variations in the interpreted groundwater conditions or undrained shear strength parameters. The results further support that the adopted subsurface model and input parameters provide a conservative assessment of the stability of the subject escarpment slope.

Concluding Remarks

Based on our review and analysis, the subject escarpment slope was found to satisfy and exceed the minimum factors of safety of 1.5, 1.1, and 1.3 under static, seismic, and post-seismic conditions, respectively. Based on the results of the assessment, a stable slope allowance setback is not considered necessary for the subject escarpment slope. Furthermore, given the absence of an active watercourse or evidence of significant toe erosion processes at the base of the slope, neither toe erosion nor erosion access setbacks are considered warranted for the subject site.

Based on the above, Paterson considers the subject escarpment slope exempt from the minimum field investigation and laboratory testing requirements outlined within the aforementioned Guidance letter. Accordingly, Paterson finds the subject site parcels located north of the escarpment slope suitable for the proposed development from a geotechnical and slope stability perspective.



5.0 Statement of Limitations

The recommendations made in this report are in accordance with our present understanding of the project. A desktop investigation is a limited investigation of a site. Should any conditions at the site be encountered which differ from those at the test locations, we request that we be notified immediately in order to permit reassessment of our recommendations.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Forum Investment and Development Corporation or their agents, without review by this firm for the applicability of our recommendations to the altered use of the report.

We trust that the current submission meets your immediate requirements.

Best Regards,

Paterson Group Inc.

Nicholas F. R. Versolato, P.Eng., ing.

Drew Petahtegoose, P.Eng.



Attachments:

- ☐ Soil Profile and Test Data Sheets.
- ☐ Atterberg Limits Results.
- ☐ Well Records.
- ☐ Figure 1 – Key Plan.
- ☐ Figures 1A to 1C – Slope Stability Analysis.
- ☐ Drawing PG8011-1 – Test Hole Location Plan.
- ☐ Drawing PG8011-2 - Extrapolated Borehole Data Cross-Section.

Report Distribution

- ☐ Forum Investment and Development Corporation (e-mail copy)
- ☐ Paterson Group (1 copy)



Log of Borehole BH 25-08



Project No: OTT-22022218-A0

Project: High-Rise Commercial And Residential Development

Location: 530 Brisebois Cres. Part of 265 Centrum Blvd. (Forum Lands), Ottawa, ON

Figure No. 10

Page. 1 of 1

Date Drilled: 'August 14, 2025

Drill Type: CME-55LC Rubber Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: MZ Checked by: SMP

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☒

Shear Strength by
Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at
% Strain at Failure ☐

Shear Strength by
Penetrometer Test ☒

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLING	Natural Unit Wt. kN/m³
					20 40 60 80				250 500 750				
					Shear Strength				Natural Moisture Content % Atterberg Limits (% Dry Weight)				
					50 100 150 200 kPa				20 40 60				
		TOPSOIL ~ 50 mm thick	69.52	0	22					X			SS1
		FILL Mixture of silty sand and gravel, cobbles and boulders, trace clay, brown and grey, damp, no odours, no stains, (compact to very dense)	69.5										
		Auger grinding on possible cobbles and boulders from ground surface to 2.3 m depth	67.2	1			60			X			SS2
				2	36					X			SS3
		HIGHLY WEATHERED LIMESTONE BEDROCK Grey	66.9		15 the 50 for 125 mm					X			SS4
		LIMESTONE BEDROCK		3									RUN1
		With shale partings, grey, fair to excellent quality											
				4									RUN2
		With clay seams from 3.0 m to 4.5 m depths											
				5									
			63.72										
			63.3	6									RUN3
		Borehole Terminated at 6.2 m Depth											

NOTES:

- Borehole data requires interpretation by EXP before use by others
- A 19 mm diameter standpipe was installed to 6.2 m.
- Field work was supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-22022218-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)
September 26, 2025	5.8	

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %
1	2.6 - 3	93	93
2	3 - 4.5	85	56
3	4.5 - 6.2	95	70

LOG OF BOREHOLE BH-LOGS - 265 CENTRUM BLVD.GPJ TROW OTTAWA.GDT 12/2/25



**PATERSON
GROUP**

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

1445 rue Duford, Orléans, Ontario

DATUM: Geodetic **EASTING:** 382392.361 **NORTHING:** 5037474.207 **ELEVATION:** 87.13 m

PROJECT: Proposed Modular Classroom Addition - La Source School

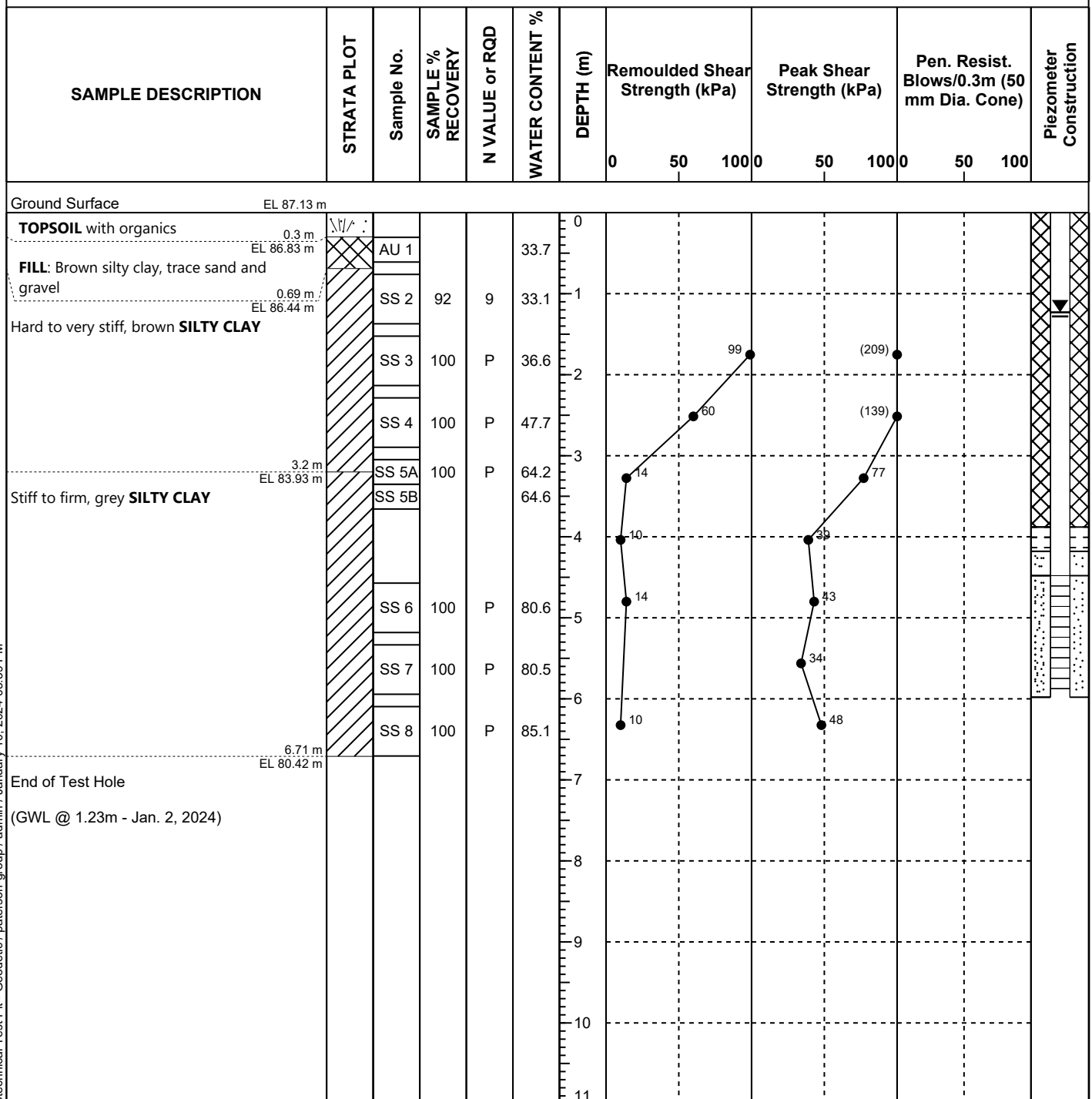
FILE NO. **PG6942**

BORINGS BY: CME 55 Low Clearance Power Auger

REMARKS:

DATE: December 20, 2023

HOLE NO. BH 1-23



DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



**PATERSON
GROUP**

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

1445 rue Duford, Orléans, Ontario

DATUM: Geodetic **EASTING:** 382396.669 **NORTHING:** 5037462.085 **ELEVATION:** 87.06 m

PROJECT: Proposed Modular Classroom Addition - La Source School

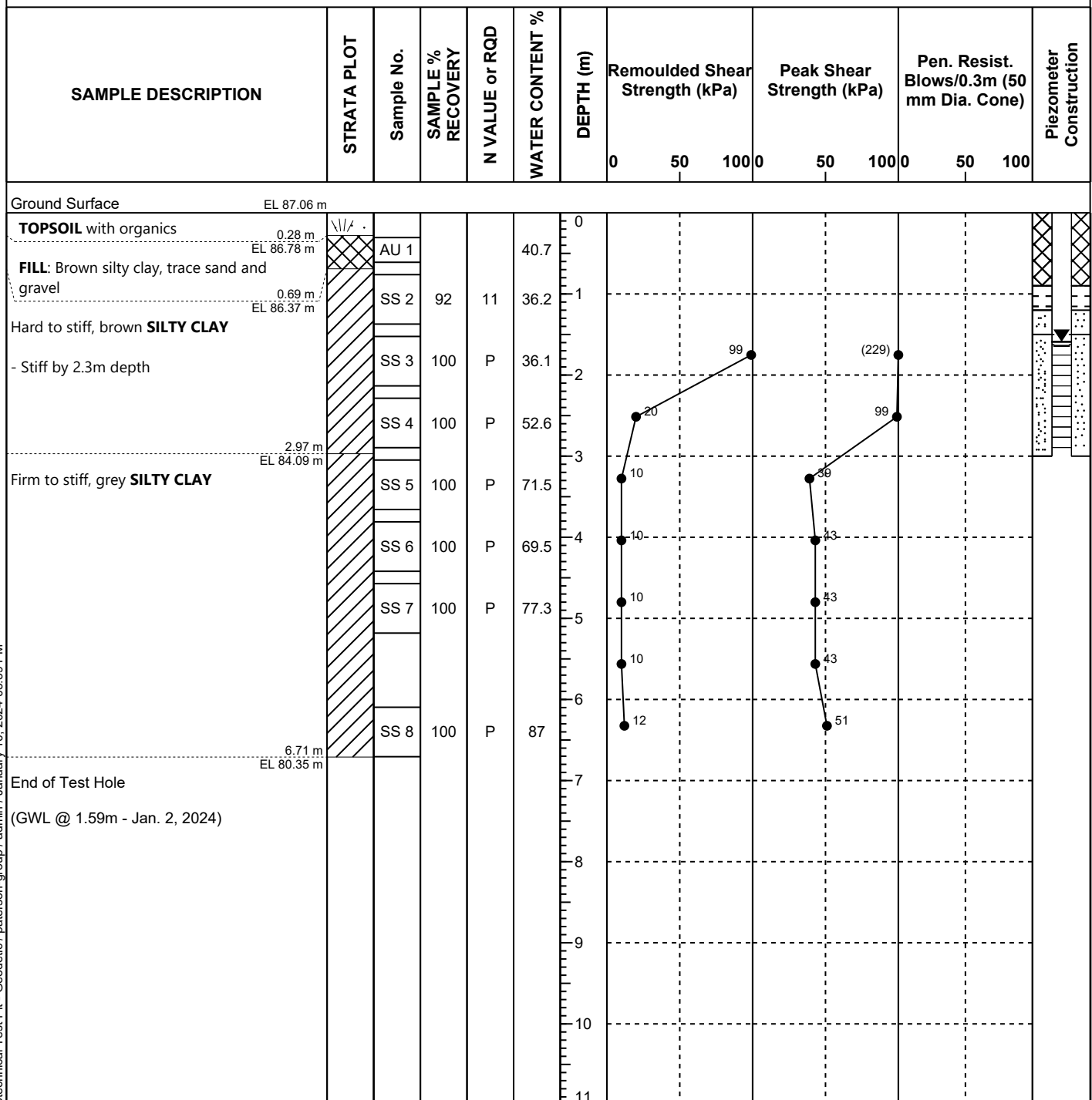
FILE NO. PG6942

BORINGS BY: CME 55 Low Clearance Power Auger

REMARKS:

DATE: December 20, 2023

HOLE NO. BH 2-23



DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



1445 rue Duford, Orléans, Ontario

FILE NO. PG6942

HOLE NO. BH 3-23

REMARKS: DATE: December 20, 2023

DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



**PATERSON
GROUP**

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

1445 rue Duford, Orléans, Ontario

DATUM: Geodetic **EASTING:** 382372.19 **NORTHING:** 5037465.851 **ELEVATION:** 87.34 m

PROJECT: Proposed Modular Classroom Addition - La Source School

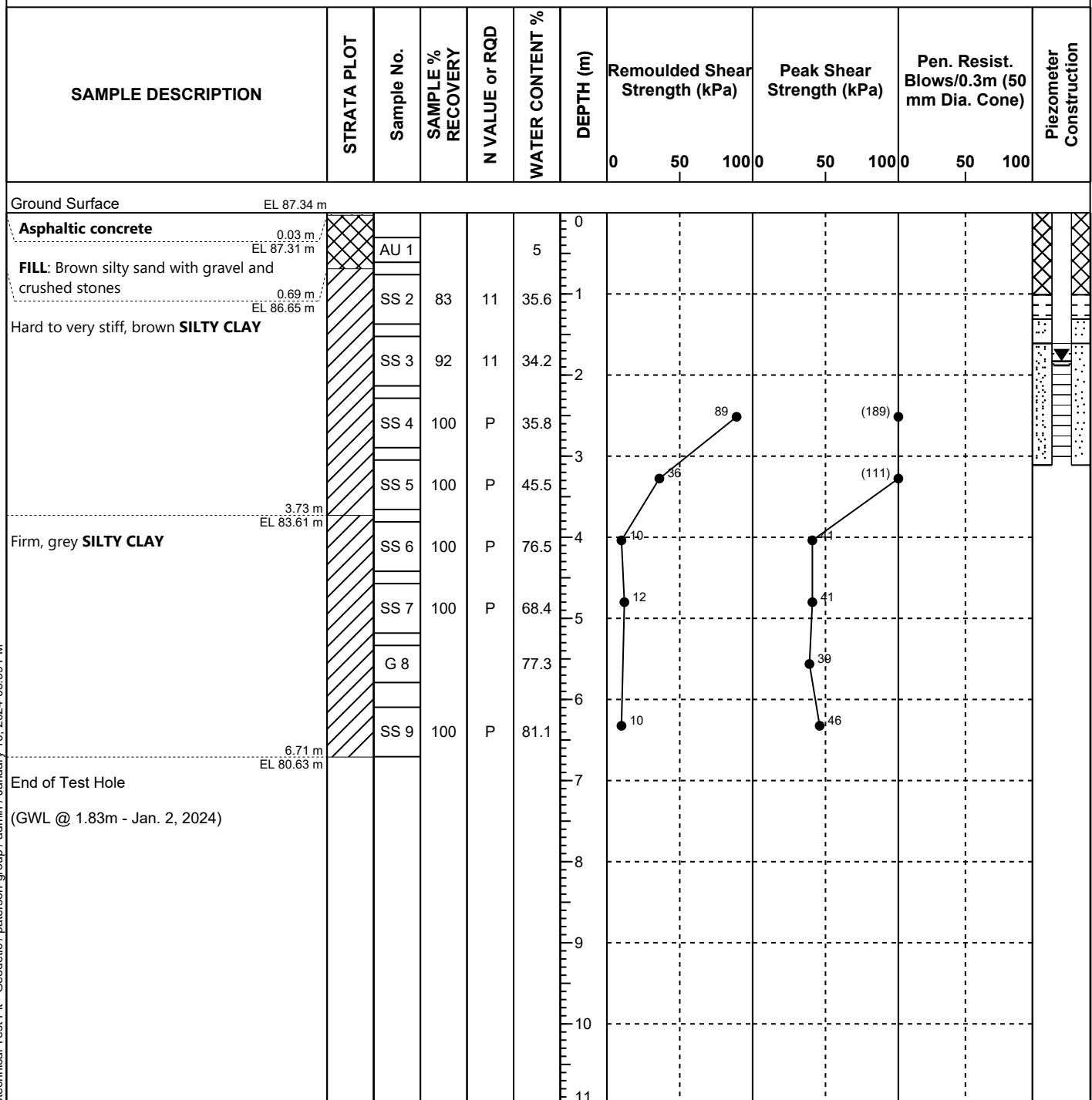
FILE NO. PG6942

BORINGS BY: CME 55 Low Clearance Power Auger

REMARKS:

DATE: December 20, 2023

HOLE NO. BH 4-23



DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



1445 rue Duford, Orléans, Ontario

FILE NO. PG6942

HOLE NO. BH 5-23

REMARKS: DATE: December 20, 2023

DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



**PATERSON
GROUP**

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

1445 rue Duford, Orléans, Ontario

DATUM: Geodetic **EASTING:** 382383.793 **NORTHING:** 5037464.073 **ELEVATION:** 87.05 m

PROJECT: Proposed Modular Classroom Addition - La Source School

FILE NO. **PG6942**

BORINGS BY: CME 55 Low Clearance Power Auger

HOLE NO. BH 5-23

REMARKS: **DATE:** December 20, 2023

SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH (m)	Remoulded Shear Strength (kPa)			Peak Shear Strength (kPa)			Pen. Resist. Blows/0.3m (50 mm Dia. Cone)			Piezometer Construction
							0	50	100	0	50	100	0	50	100	
(continued) Dynamic Cone Penetration Test commenced at 6.71m depth. The cone was pushed to 30.48m depth.						11										
						12										
						13										
						14										
						15										
						16										
						17										
						18										
						19										
						20										
						21										
						22										

DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



**PATERSON
GROUP**

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

1445 rue Duford, Orléans, Ontario

DATUM: Geodetic **EASTING:** 382383.793 **NORTHING:** 5037464.073 **ELEVATION:** 87.05 m

PROJECT: Proposed Modular Classroom Addition - La Source School

FILE NO. **PG6942**

BORINGS BY: CME 55 Low Clearance Power Auger

HOLE NO. BH 5-23

REMARKS: **DATE:** December 20, 2023

SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH (m)	Remoulded Shear Strength (kPa)			Peak Shear Strength (kPa)			Pen. Resist. Blows/0.3m (50 mm Dia. Cone)			Piezometer Construction
							0	50	100	0	50	100	0	50	100	
(continued) Dynamic Cone Penetration Test commenced at 6.71m depth. The cone was pushed to 30.48m depth. 30.48 m EL 56.57 m End of Test Hole DCPT terminated at 30.48m depth (Borehole blocked - Jan. 2, 2024)						22										
						23										
						24										
						25										
						26										
						27										
						28										
						29										
						30										
						31										
						32										
						33										

DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



Log of Borehole: MW1

Project #: 296551.001

Logged By: WT

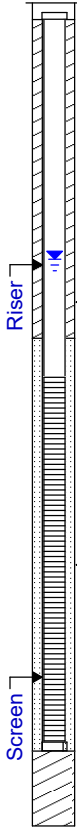
Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 8, 2021

Project Manager: WT

SUBSURFACE PROFILE					SAMPLE									
Depth (m)	Symbol	Description	Elevation (m)	Monitoring Well Details	Sample Type	Sampler #	Recovery (%)	SPT N-values	SPT N-values		Lab Analysis	Moisture (%)	Plasticity Index	
									20 40 60	Shear Strength kPa 50 100 150 200				
0		Ground Surface	87.43											
		Organics ~ 100 mm			SS	1	30	9						
		Clayey Silt Grey clayey silt, trace sand, stiff, damp	86.67		SS	2	100	18						
1		Very stiff	85.90		SS	3	80	7						
2		Firm	85.14		SS	4	80	3						
3		Soft, moist			SS	5	100	3						
4														
5		Very soft, wet	82.86		SS	6	100	1						
6						FV	1	N/A	N/A					
						SS	7	100	0					
7		End of Borehole	80.72	Groundwater level = 2.09 mbgs, as measured on October 27, 2021.										
		Borehole terminated at 6.7 mbgs in very soft clayey silt.												
8														
9														

Contractor: Marathon Underground

Grade Elevation: 87.43 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: 88.32 m

Well Casing Size: 5.08 cm

Sheet: 1 of 1



Log of Borehole: BH2

Project #: 296551.001

Logged By: WT

Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 8, 2021

Project Manager: WT

SUBSURFACE PROFILE					SAMPLE									
Depth (m)	Symbol	Description	Elevation (m)	Monitoring Well Details	Sample Type	Sampler #	Recovery (%)	SPT N-values	SPT N-values		Lab Analysis	Moisture (%)	Plasticity Index	
									20	40	60			
									Shear Strength kPa					
									50	100	150	200		
0		Ground Surface	87.59											
		Organics ~ 100 mm			SS	1	60	9						
		Clayey Silt Grey clayey silt, very stiff, damp	86.83		SS	2	80	18						
1		Hard			SS	3	80	7						
2		Firm	85.30		SS	4	80	3						
3		Soft, moist	84.54		SS	5	100	3						
		End of Borehole	83.93											
4		Borehole terminated at 3.7 mbgs in soft clayey silt. At drilling completion, no free groundwater was encountered.												
5														
6														
7														
8														
9														

Contractor: Marathon Underground

Grade Elevation: 87.59 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: N/A

Well Casing Size: N/A

Sheet: 1 of 1



Log of Borehole: MW3

Project #: 296551.001

Logged By: WT

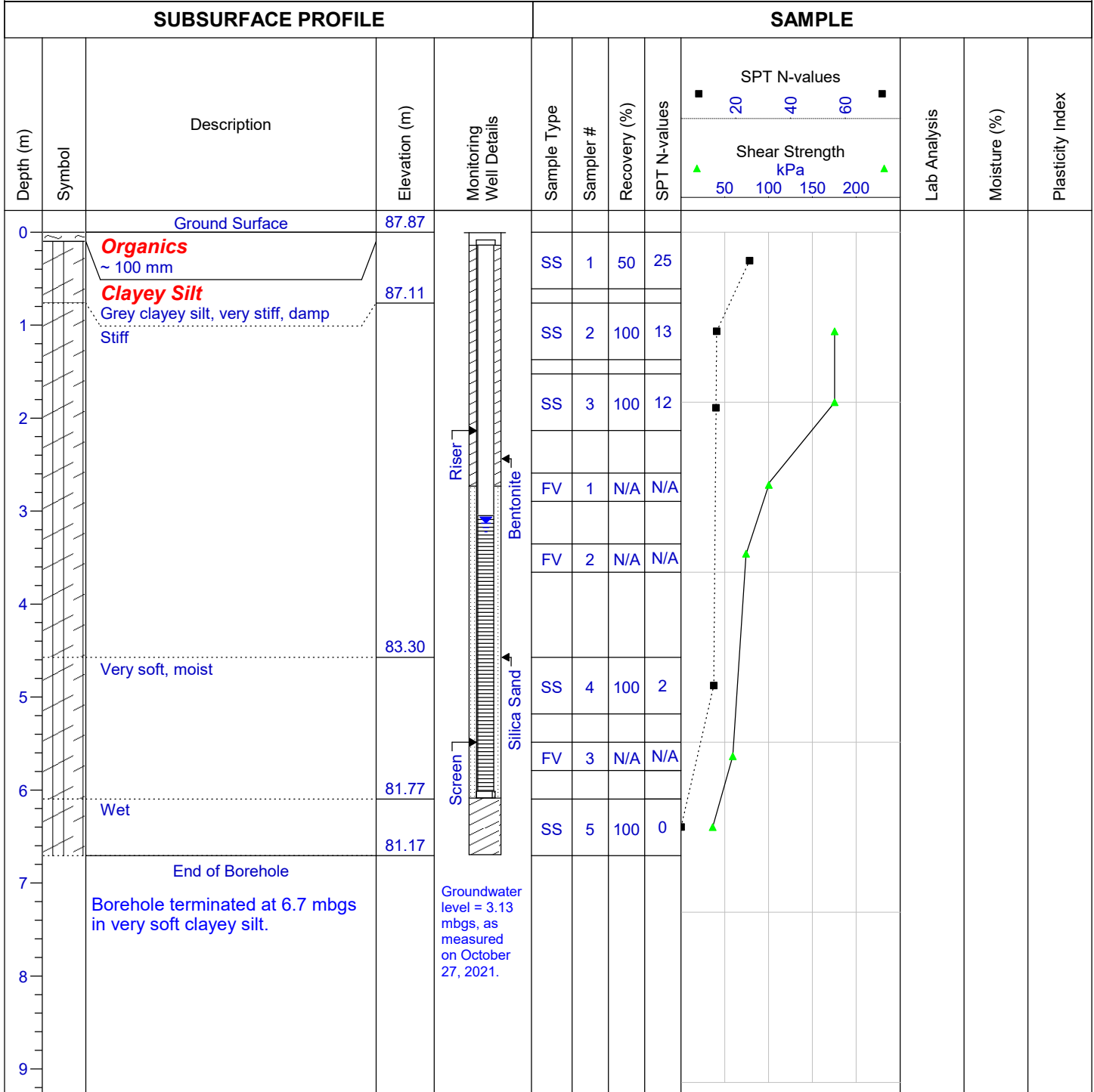
Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 8, 2021

Project Manager: WT



Contractor: Marathon Underground

Grade Elevation: 87.87 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: 88.75 m

Well Casing Size: 5.08 cm

Sheet: 1 of 1



Log of Borehole: MW4

Project #: 296551.001

Logged By: WT

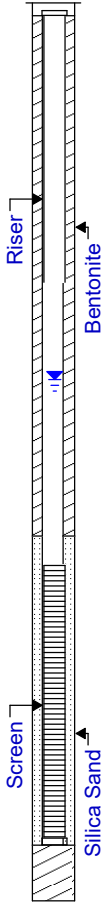
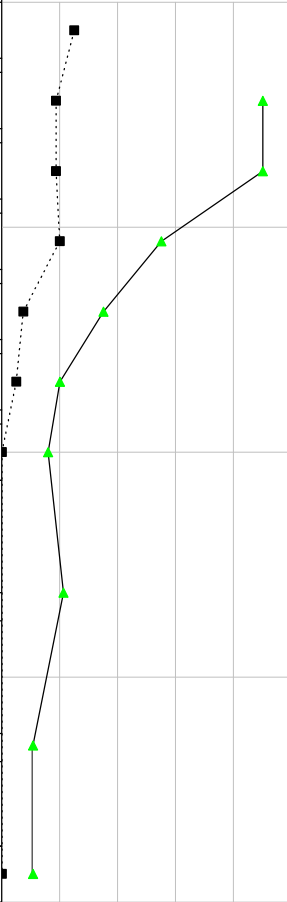
Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 9, 2021

Project Manager: WT

SUBSURFACE PROFILE					SAMPLE									
Depth (m)	Symbol	Description	Elevation (m)	Monitoring Well Details	Sample Type	Sampler #	Recovery (%)	SPT N-values	SPT N-values		Lab Analysis	Moisture (%)	Plasticity Index	
									20 40 60	Shear Strength kPa 50 100 150 200				
0		Ground Surface	87.62		SS	1	60	20						
		Organics ~ 100 mm	86.86		SS	2	80	15						
1		Clayey Silt Grey clayey silt, trace gravel, trace sand, very stiff, damp	86.10		SS	3	100	15						
2		Hard No sand or gravel	85.33		SS	4	100	16						
3		Stiff	84.57		SS	5	100	6						
4		Firm	83.81		SS	6	100	4						
5		Soft	83.05		SS	7	N/A	0						
6		Very soft												
7														
8														
9			77.87											
10		End of Borehole		Groundwater level = 4.09 mbgs, as measured on October 27, 2021.										
		Borehole terminated at 9.8 mbgs in very soft clayey silt.												
11														
12														

Contractor: Marathon Underground

Grade Elevation: 87.62 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: 88.48 m

Well Casing Size: 5.08 cm

Sheet: 1 of 1



Log of Borehole: MW5

Project #: 296551.001

Logged By: WT

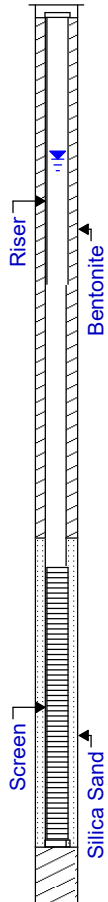
Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 9, 2021

Project Manager: WT

SUBSURFACE PROFILE					SAMPLE											
Depth (m)	Symbol	Description	Elevation (m)	Monitoring Well Details	Sample Type	Sampler #	Recovery (%)	SPT N-values	SPT N-values 20 40 60				Lab Analysis	Moisture (%)	Plasticity Index	
									Shear Strength kPa 50 100 150 200							
0		Ground Surface	86.80													
		Organics ~ 100 mm	86.04		SS	1	80	11								
1		Clayey Silt Grey clayey silt, stiff, damp Hard			SS	2	100	15								
2		Stiff, moist	84.51		SS	3	100	11								
3		Soft	83.75		SS	4	100	9								
4		Wet	82.99		SS	5	100	4								
5					SS	6	100	2								
6					FV	1	N/A	N/A								
7					FV	2	N/A	N/A								
8					FV	3	N/A	N/A								
9				FV	4	N/A	N/A									
10		End of Borehole		Groundwater level = 1.68 mbgs, as measured on October 27, 2021.												
		Borehole terminated at 9.8 mbgs in very soft clayey silt.	77.05													
11																
12																

Contractor: Marathon Underground

Grade Elevation: 86.80 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: 87.55 m

Well Casing Size: 5.08 cm

Sheet: 1 of 1



Log of Borehole: MW6

Project #: 296551.001

Logged By: WT

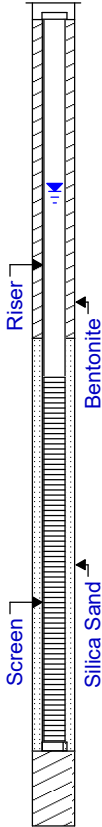
Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 9, 2021

Project Manager: WT

SUBSURFACE PROFILE					SAMPLE									
Depth (m)	Symbol	Description	Elevation (m)	Monitoring Well Details	Sample Type	Sampler #	Recovery (%)	SPT N-values	SPT N-values 20 40 60		Lab Analysis	Moisture (%)	Plasticity Index	
									Shear Strength kPa 50 100 150 200					
0		Ground Surface	88.00											
		Organics ~ 100 mm			SS	1	60	11						
		Clayey Silt Grey clayey silt, stiff, damp	87.24		SS	2	80	17						
1		Hard			SS	3	100	17						
2		Stiff	85.71		SS	4	100	11						
3		Moist	84.95		SS	5	100	9						
4														
5						FV	1	N/A	N/A					
6		Very soft	81.90											
			81.29			SS	6	100	0					
7		End of Borehole Borehole terminated at 6.7 mbgs in very soft clayey silt.		Groundwater level = 1.54 mbgs, as measured on October 27, 2021.										
8														
9														

Contractor: Marathon Underground

Grade Elevation: 88.00 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: 88.72 m

Well Casing Size: 5.08 cm

Sheet: 1 of 1



Log of Borehole: MW7

Project #: 296551.001

Logged By: WT

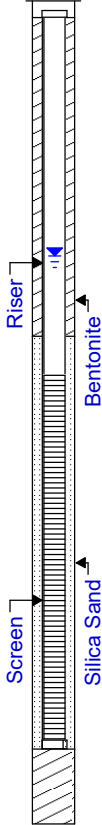
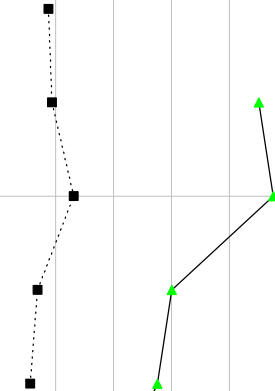
Project: Geotechnical Investigation

Client: United Property Resource Corporation

Location: 360 Kennedy Line East, Orleans, Ontario

Drill Date: September 9, 2021

Project Manager: WT

SUBSURFACE PROFILE					SAMPLE									
Depth (m)	Symbol	Description	Elevation (m)	Monitoring Well Details	Sample Type	Sampler #	Recovery (%)	SPT N-values	SPT N-values		Lab Analysis	Moisture (%)	Plasticity Index	
									20	40				60
									Shear Strength kPa					
									50	100	150	200		
0		Ground Surface	87.78		SS	1	60	14						
		Organics ~ 100 mm												
		Clayey Silt Grey clayey silt, stiff, damp	87.02		SS	2	80	15						
1		Hard												
2		Stiff	85.49		SS	3	100	21						
3					SS	4	100	11						
4					SS	5	100	9						
5					FV	1	N/A	N/A						
6														
7		End of Borehole	81.07		FV	2	N/A	N/A						
		Borehole terminated at 6.7 mbgs in very soft clayey silt.												
8														
9														

Contractor: Marathon Underground

Grade Elevation: 87.78 m

Drilling Method: Hollow Stem Augers / Split Spoon

Top of Casing Elevation: 88.52 m

Well Casing Size: 5.08 cm

Sheet: 1 of 1

JOHN D. PATERSON & ASSOCIATES LTD. Consulting Engineers & Geologists 1479 LAPERRIERE AVE. OTTAWA, CANADA K1Z7S8						SOIL PROFILE & TEST DATA						
Datum T.B.M: Top of manhole southeast of borehole one. (Elev. 87.50 m) Remarks						Proposed Residential Development, Prestone Drive and Kennedy Lane, Township of Cumberland						
						Sheet no. 1 of 5						
Borings by CME 55 Power Auger						Date March 19, 1984						
						Hole no. BH 1						
SOIL DESCRIPTION	STRAT.	SAMPLE		N	DEPTH (m)	ELEV. (m)	Penetration Resistance Blows/.3m — 50 mm Diameter Cone					
		TYPE	NO.				Moisture Content %					
Ground Surface					0.00	-87.09	20	40	60	80	100	
FILL: Grey silty clay with traces of gravel and sand.	x	G	1				Dry to 6.1 metres 6/4/84					
	x	SS	2	6	1.00							
	x	SS	3	2								
	x	SS	4	3	2.00	-85.09	Poly Tubing					
	x	SS	5	2								
	x	SS	6	3	3.00							
	x	SS	7	3	4.00	-83.09						
	x	SS	8	1								
	x	SS	9	1	5.00							
	x	SS	10	1	6.00	-81.09	Bentonite Seal					
	x	SS	11	2								
	x	SS	12	3	7.00							
	x	SS	13	5								
FILL: Brown sand-gravel.	x	SS	14	8	8.00	-79.09						
	x	SS	15	4	9.00							
	x	SS	16	5								
	x	SS	17	1	10.00	-77.09	Piez-ometer					
Firm grey SILTY CLAY.		TW	18									
					11.00							
Borehole terminated in clay.					12.00	-75.09						
							40	80	120			
							Shear Strength (kPa)					
							▲ Undisturbed △ Remolded					

JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers & Geologists

1479 LAPERRIERE AVE. OTTAWA, CANADA K1Z7S8

SOIL PROFILE & TEST DATA

Proposed Residential Development,
Prestone Drive and Kennedy Lane,
Township of Cumberland

Datum T.B.M: Top of manhole southeast of borehole one. (Elev. 87.50 m)

Remarks

Sheet no. 3 of 5

Hole no.

Borings by CME 55 Power Auger

Date March 20, 1984

BH 3

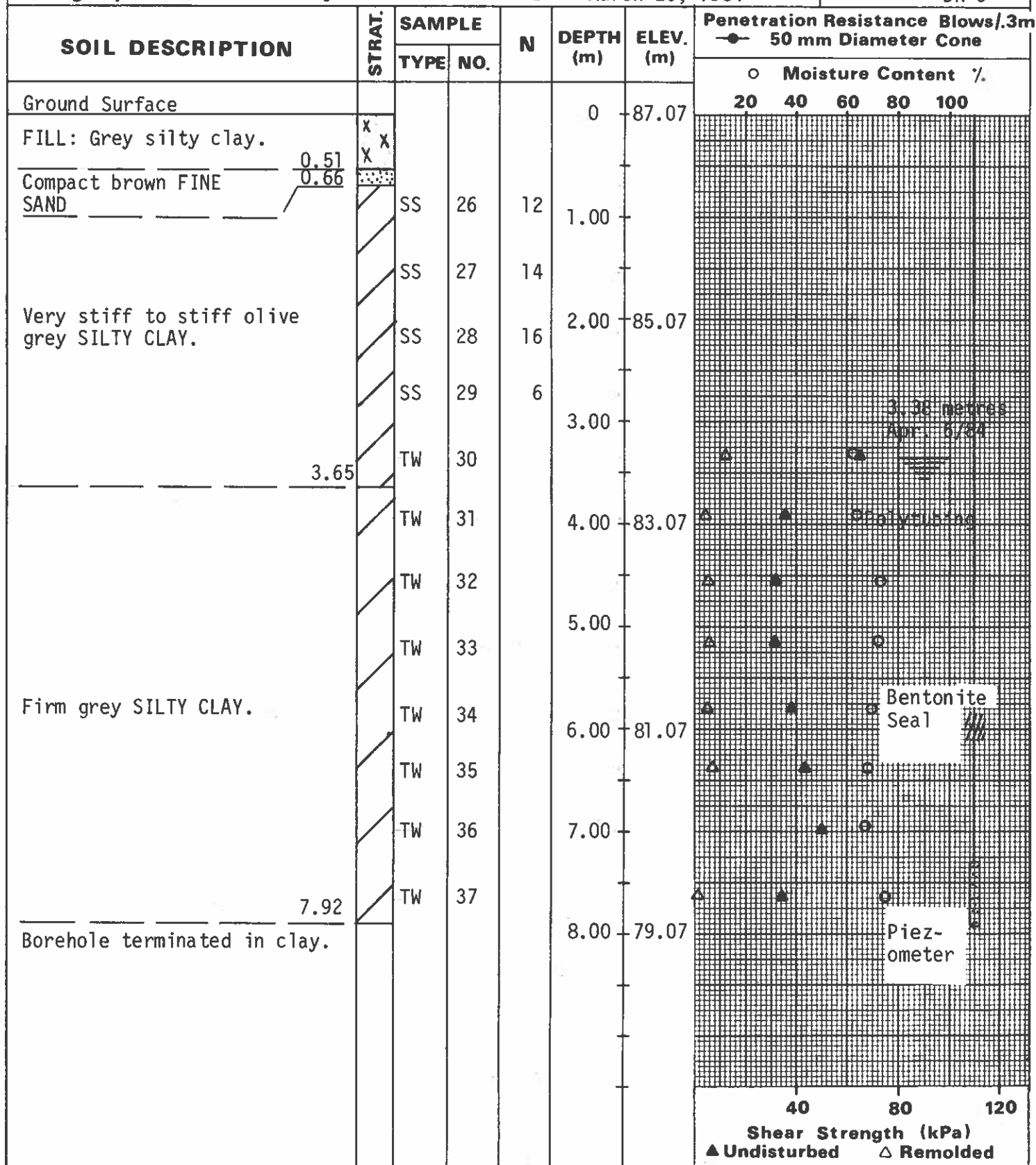


TABLE 2
SUMMARY OF LABORATORY TEST DATA
PRESTONE DRIVE AND KENNEDY LANE
TOWNSHIP OF CUMBERLAND

Hole No.	Sample No.	Depth (m)		Shear Strength (kPa)		% Water Content	Unit Weight (KN/m ³)
		From	To	Undisturbed	Remoulded		
1	TW 18	10.36	10.97	41.1	16.3	69.1	15.8
3	TW 30	3.05	3.66	65.5	11.6	63.5	15.9
	TW 31	3.66	4.27	35.8	4.3	63.8	15.9
	TW 32	4.27	4.88	30.6	6.3	72.6	15.5
	TW 33	4.88	5.49	30.7	6.3	70.9	15.8
	TW 34	5.49	6.10	39.0	5.1	70.0	16.2
	TW 35	6.10	6.71	43.7	7.5	68.6	15.8
	TW 36	6.71	7.32	49.9	-	67.8	15.8
	TW 37	7.32	7.93	32	1.2	75.0	16.6

CLIENT:	Pinchin	FILE NO.:	PM41784
PROJECT:	296551.001	DATE SAMPLED:	18-Oct-21
LOCATION:	BH1 @ 5' - 7'	DATE REPORTED:	26-Oct-21

LIQUID LIMIT DETERMINATION

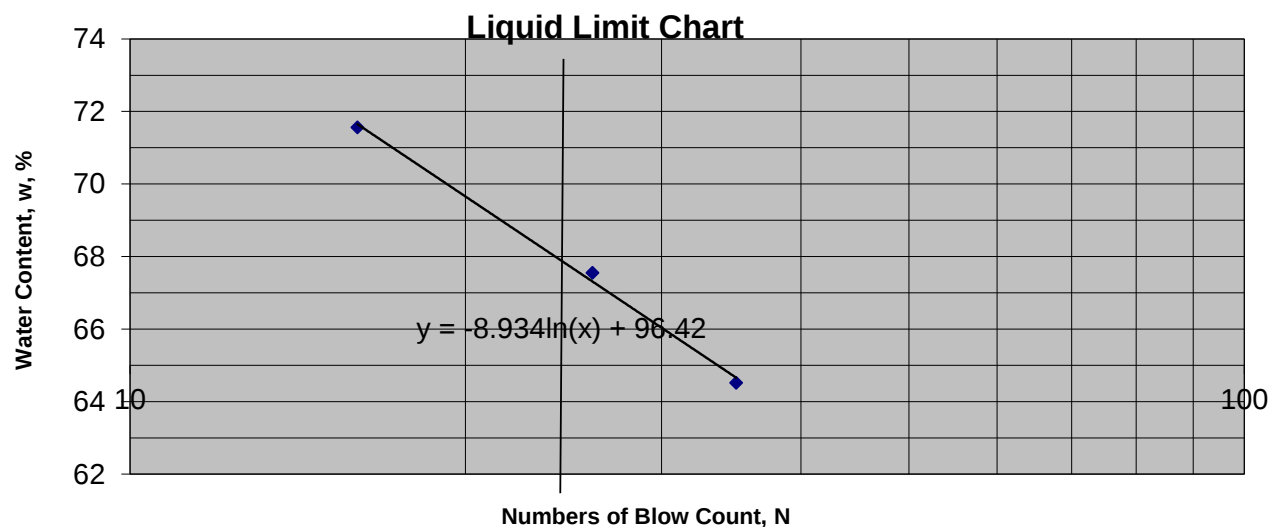
CAN NO.	2	3	4				
WT. OF CAN	8.67	8.67	8.68				
WT. OF SOIL & CAN	23.51	20.65	19.85				
WT. OF DRY SOIL & CAN	17.32	15.82	15.47				
WT. OF MOISTURE	6.19	4.83	4.38				
WT. OF DRY SOIL & CAN	8.65	7.15	6.79				
WATER CONTENT, w, %	71.56	67.55	64.51				
NO. OF BLOWS, N	16	26	35				

PLASTIC LIMIT DETERMINATION

CAN NO.	10	11
WT. OF CAN	19.81	20.01
WT. OF SOIL & CAN	26.75	27.01
WT. OF DRY SOIL & CAN	25.14	25.38
WT. OF MOISTURE	1.61	1.63
WT. OF DRY SOIL & CAN	5.33	5.37
WATER CONTENT, w, %	30.21	30.35

RESULTS

LIQUID LIMIT	68
PLASTIC LIMIT	30
PLASTICITY INDEX	38



TECHNICIAN:CS		C. Beadow	J. Forsyth, P. Eng.
	REVIEWED BY:	<i>[Signature]</i>	<i>[Signature]</i>

CLIENT:	Pinchin	FILE NO.:	PM4184
PROJECT:	296551.001	DATE SAMPLED:	18-Oct-21
LOCATION:	BH3 @ 20' - 22'	DATE REPORTED:	26-Oct-21

LIQUID LIMIT DETERMINATION

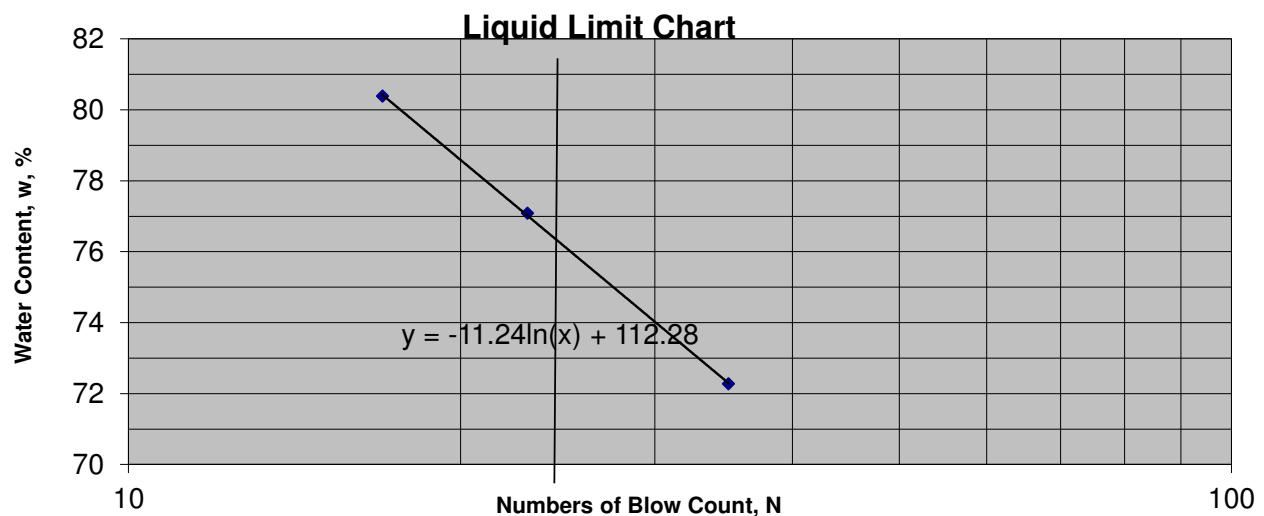
CAN NO.	33	34	35				
WT. OF CAN	4.36	4.36	4.42				
WT. OF SOIL & CAN	12.82	13.71	13.24				
WT. OF DRY SOIL & CAN	9.05	9.64	9.54				
WT. OF MOISTURE	3.77	4.07	3.70				
WT. OF DRY SOIL & CAN	4.69	5.28	5.12				
WATER CONTENT, w, %	80.38	77.08	72.27				
NO. OF BLOWS, N	17	23	35				

PLASTIC LIMIT DETERMINATION

CAN NO.	1	2
WT. OF CAN	19.86	19.96
WT. OF SOIL & CAN	26.59	26.78
WT. OF DRY SOIL & CAN	25.10	25.24
WT. OF MOISTURE	1.49	1.54
WT. OF DRY SOIL & CAN	5.24	5.28
WATER CONTENT, w, %	28.44	29.17

RESULTS

LIQUID LIMIT	77
PLASTIC LIMIT	29
PLASTICITY INDEX	48



TECHNICIAN:CS		C. Beadow	J. Forsyth, P. Eng.
	REVIEWED BY:	<i>[Signature]</i>	<i>[Signature]</i>

CLIENT:	Pinchin	FILE NO.:	PM4184
PROJECT:	296551.001	DATE SAMPLED:	18-Oct-21
LOCATION:	BH5 @ 12.5' - 14.5'	DATE REPORTED:	16-Oct-21

LIQUID LIMIT DETERMINATION

CAN NO.	30	31	32				
WT. OF CAN	4.37	4.35	4.40				
WT. OF SOIL & CAN	15.61	14.66	14.61				
WT. OF DRY SOIL & CAN	10.44	10.00	10.06				
WT. OF MOISTURE	5.17	4.66	4.55				
WT. OF DRY SOIL & CAN	6.07	5.65	5.66				
WATER CONTENT, w, %	85.17	82.48	80.39				
NO. OF BLOWS, N	15	23	32				

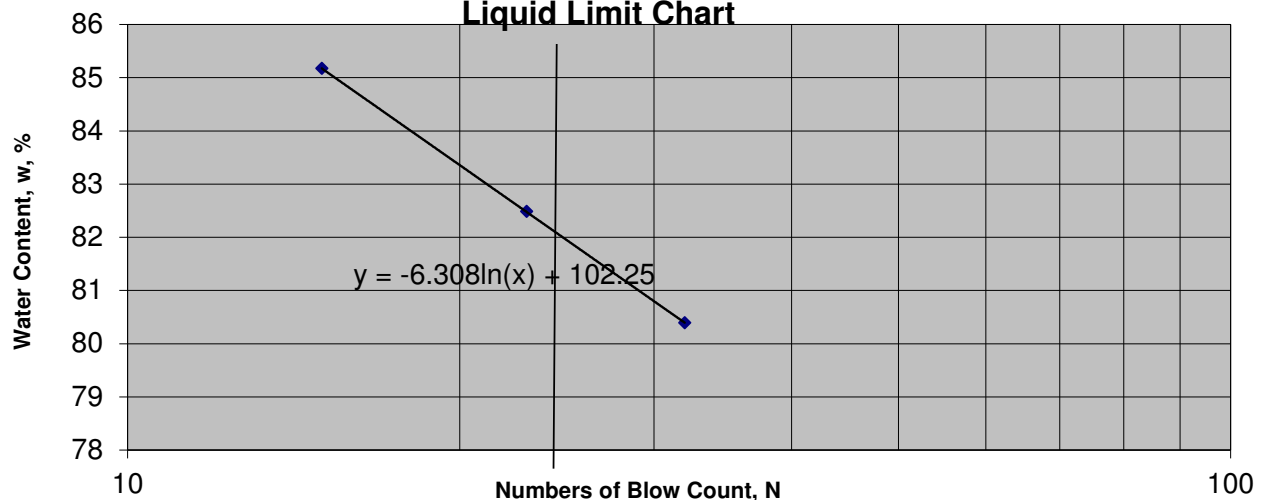
PLASTIC LIMIT DETERMINATION

CAN NO.	14	15
WT. OF CAN	20.08	19.91
WT. OF SOIL & CAN	27.47	27.30
WT. OF DRY SOIL & CAN	25.62	25.43
WT. OF MOISTURE	1.85	1.87
WT. OF DRY SOIL & CAN	5.54	5.52
WATER CONTENT, w, %	33.39	33.88

RESULTS

LIQUID LIMIT	83
PLASTIC LIMIT	34
PLASTICITY INDEX	49

Liquid Limit Chart



TECHNICIAN:CS		C. Beadow	J. Forsyth, P. Eng.
	REVIEWED BY:	<i>[Signature]</i>	<i>[Signature]</i>

CLIENT:	Pinchin	FILE NO.:	PM4184
PROJECT:	296551.001	DATE SAMPLED:	18-Oct-21
LOCATION:	BH7 @ 10' - 12'	DATE REPORTED:	26-Oct-21

LIQUID LIMIT DETERMINATION

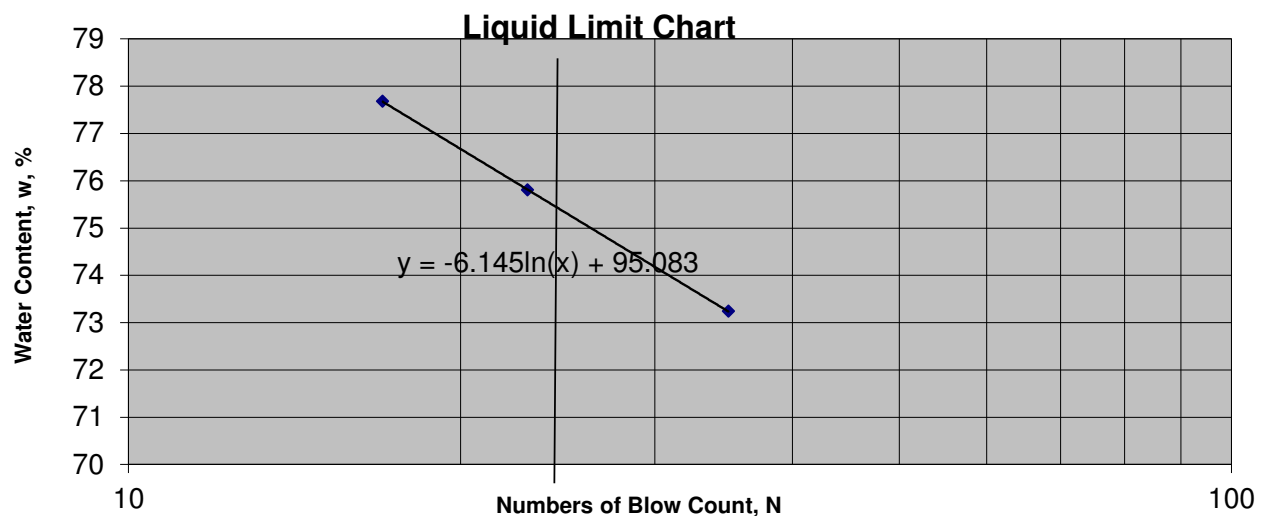
CAN NO.	67	70	87				
WT. OF CAN	7.27	7.15	7.24				
WT. OF SOIL & CAN	17.7	17.54	17.34				
WT. OF DRY SOIL & CAN	13.14	13.06	13.07				
WT. OF MOISTURE	4.56	4.48	4.27				
WT. OF DRY SOIL & CAN	5.87	5.91	5.83				
WATER CONTENT, w, %	77.68	75.8	73.24				
NO. OF BLOWS, N	17	23	35				

PLASTIC LIMIT DETERMINATION

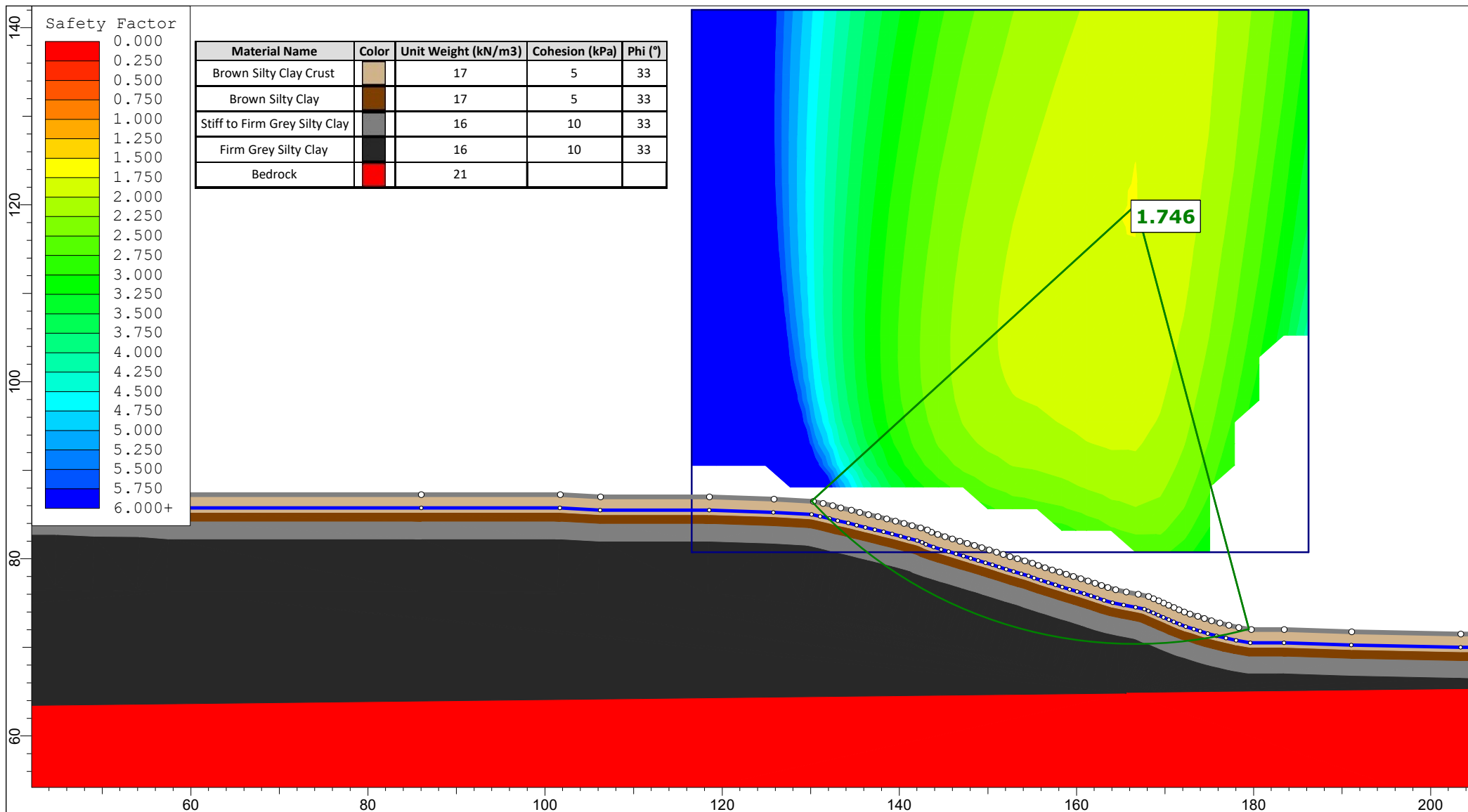
CAN NO.	9	18
WT. OF CAN	19.35	20.01
WT. OF SOIL & CAN	26.59	26.88
WT. OF DRY SOIL & CAN	24.90	25.32
WT. OF MOISTURE	1.69	1.56
WT. OF DRY SOIL & CAN	5.55	5.31
WATER CONTENT, w, %	30.45	29.38

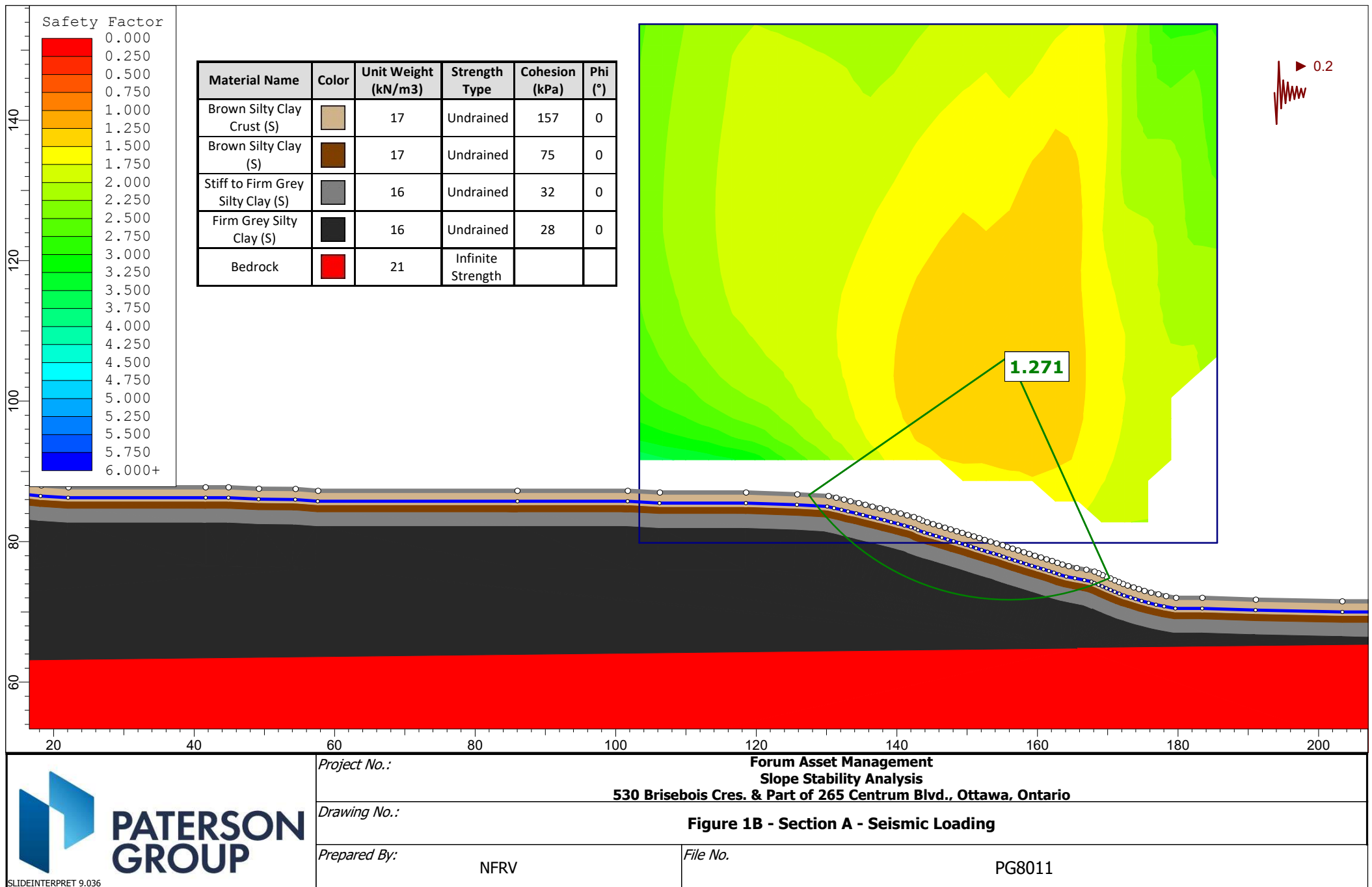
RESULTS

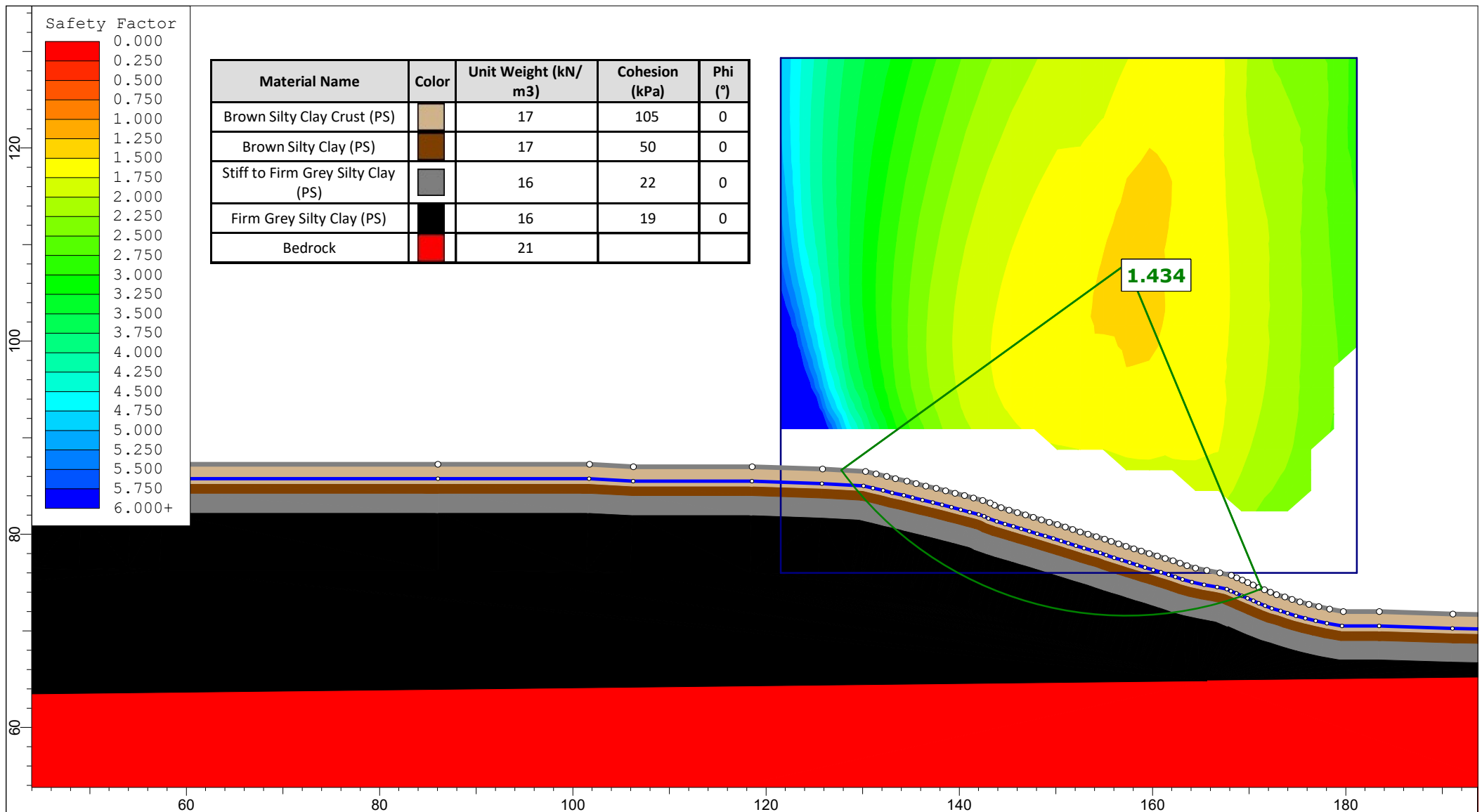
LIQUID LIMIT	76
PLASTIC LIMIT	30
PLASTICITY INDEX	46

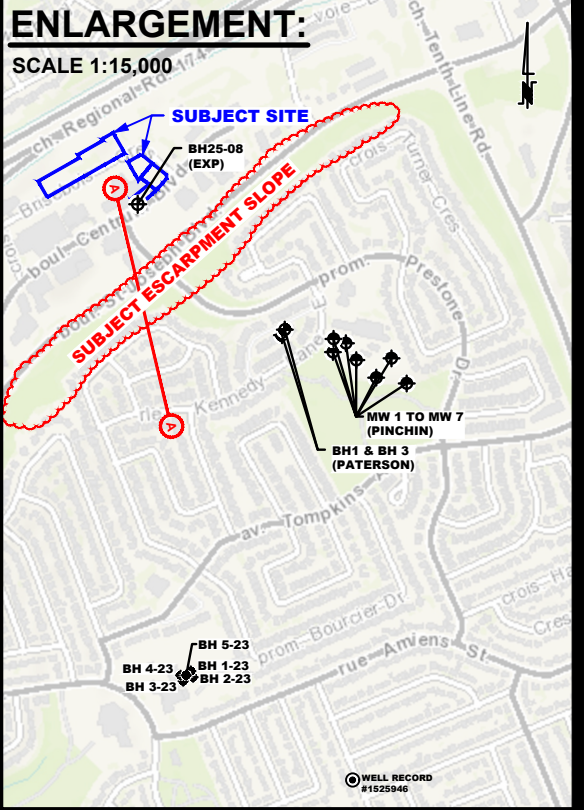
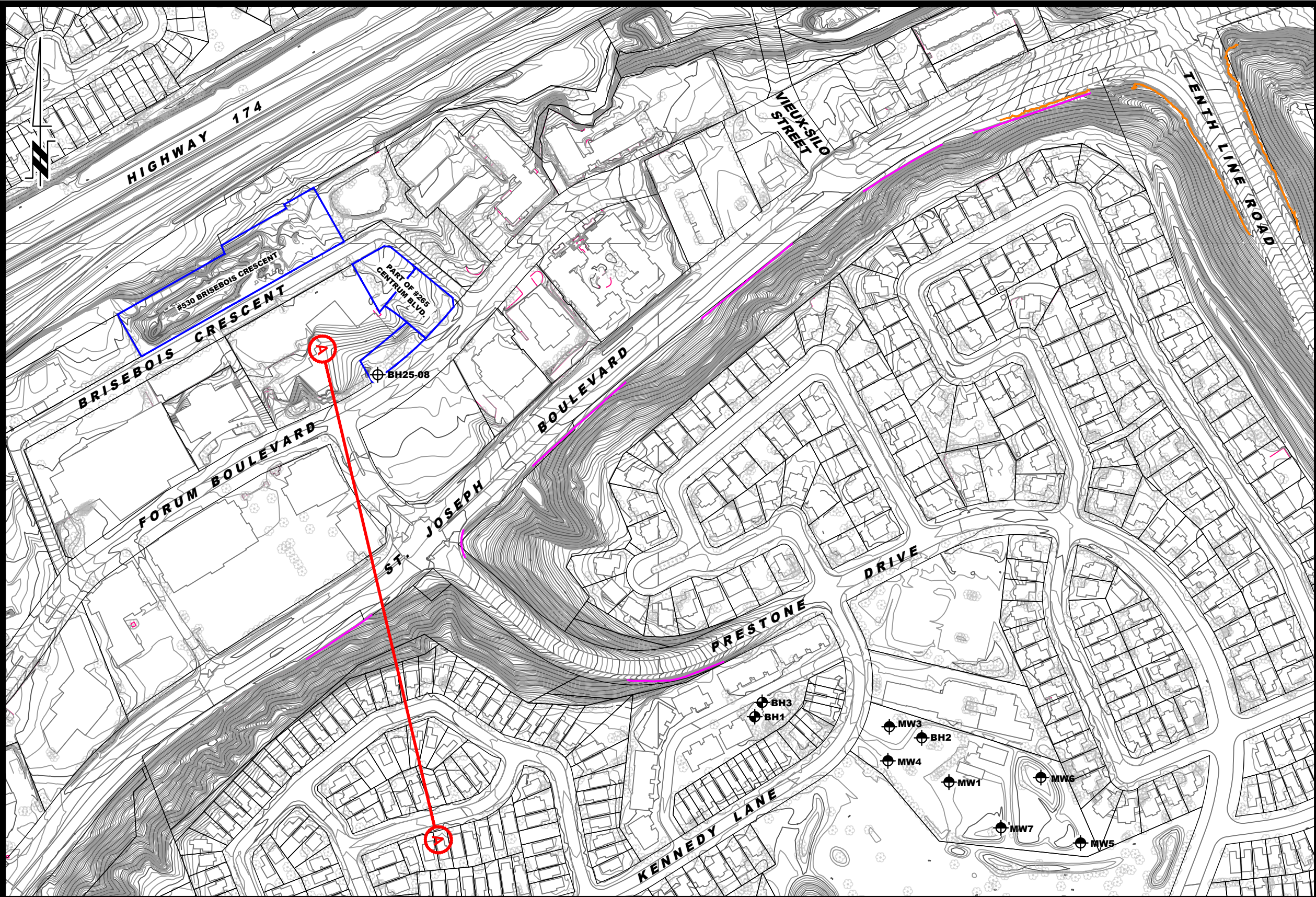


TECHNICIAN:CS		C. Beadow	J. Forsyth, P. Eng.
	REVIEWED BY:	<i>[Signature]</i>	<i>[Signature]</i>

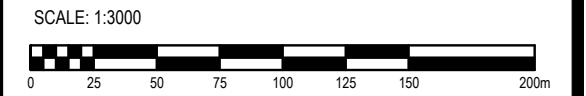








- LEGEND:**
- EXISTING BOREHOLE LOCATION BY OTHERS (EXP, 2025)
 - EXISTING BOREHOLE LOCATION (PATERSON GROUP, 2023)
 - EXISTING BOREHOLE LOCATION BY OTHERS (PINCHIN, 2021)
 - EXISTING BOREHOLE LOCATION (PATERSON GROUP, 1984)
 - WELL RECORD BORING LOCATION
 - SLOPE STABILITY CROSS-SECTION LOCATION
 - EXISTING RETAINING WALL
 - BEDROCK OUTCROP
 - SUBJECT SITE BOUNDARY
- NOTE: TOPOGRAPHIC CONTOUR DATA DEPICTED HEREIN ARE BASED ON 1:1000 SCALE LIDAR DIGITAL MAPPING DATA (2015)



9 AURIGA DRIVE
OTTAWA, ON
K2E 7T9
TEL: (613) 226-7381

1	AS PER ADDITIONAL BOREHOLE COVERAGE	29/06/2026	NFRV
NO.	REVISIONS	DD/MM/YYYY	INITIAL

FORUM INVESTMENT AND DEVELOPMENT CORPORATION
GEOTECHNICAL DESKTOP SLOPE STABILITY ASSESSMENT
530 BRISEBOIS CRES. & PART OF 265 FORUM BLVD.

OTTAWA, ONTARIO

Title: **TEST HOLE LOCATION PLAN**

Scale: 1:3000

Drawn by: NFRV

Checked by: NFRV

Approved by: DP

Date: 05/2026

Report No.: PG8011-LET.01

Dwg. No.: **PG8011-1**

Revision No.: 1

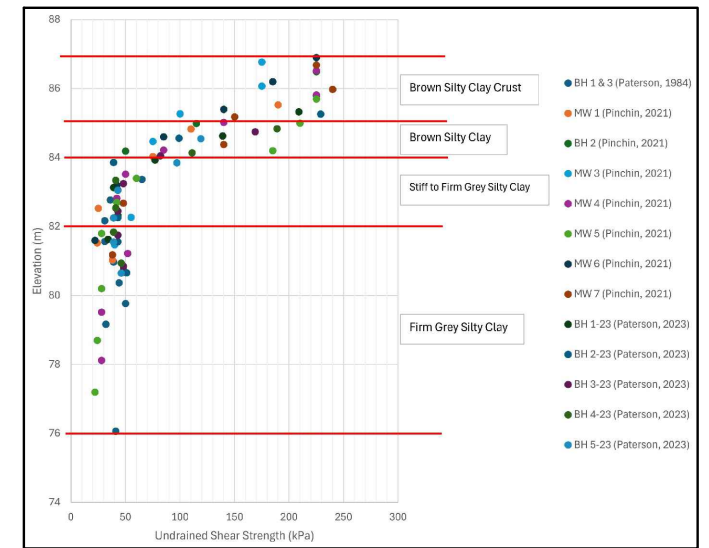
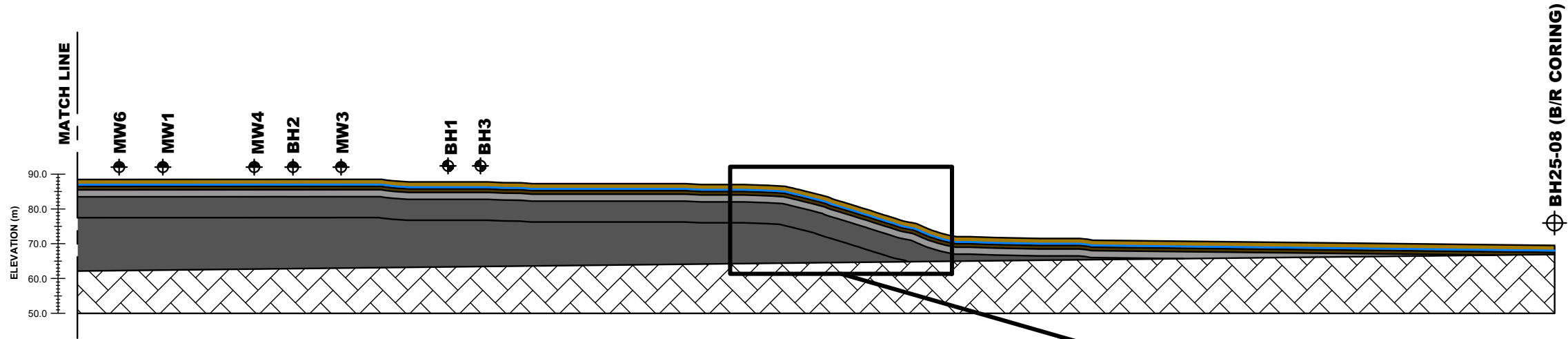
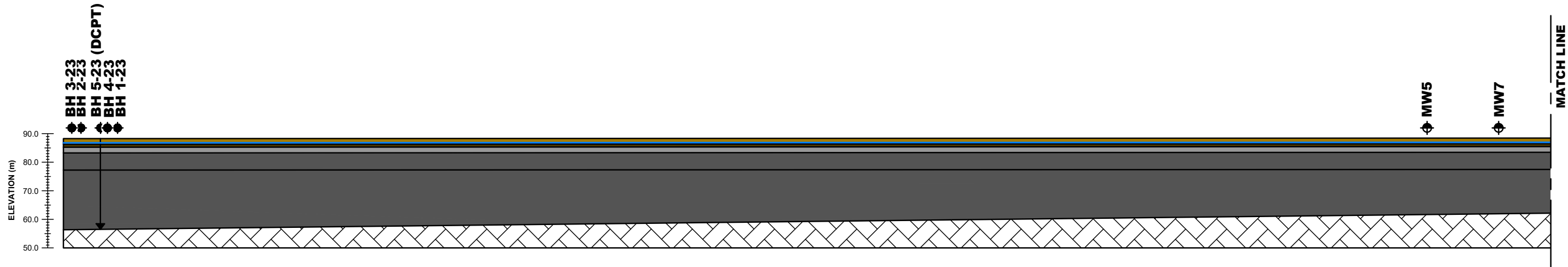


FIGURE 1: UNDRAINED SHEAR STRENGTH vs. ELEVATION

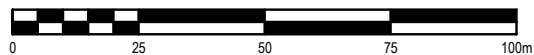
GENERAL NOTES:

1. THE GRAPHICAL REPRESENTATION ABOVE OF THE UNDRAINED SHEAR STRENGTH VALUES (S_u) RECOVERED THROUGHOUT THE LOCAL SILTY CLAY DEPOSIT WAS CONSIDERED WHEN EXTRAPOLATING THE SUBSURFACE CONDITIONS.
2. THE GROUNDWATER TABLE WAS CONSIDERED AT A DEPTH OF 1.5m FROM THE EXISTING GROUND SURFACE BASED ON THE SHALLOWEST RECOVERED GROUNDWATER LEVEL MEASUREMENT (PINCHIN, MW6, 2021).
3. THE BEDROCK SURFACE WAS EXTRAPOLATED CONSIDERING THE BEDROCK CORING DATA PROVIDED FROM BH25-08 (EXP) AND THE REFUSAL TO DCPT FROM BH 5-26 (PATERSON)

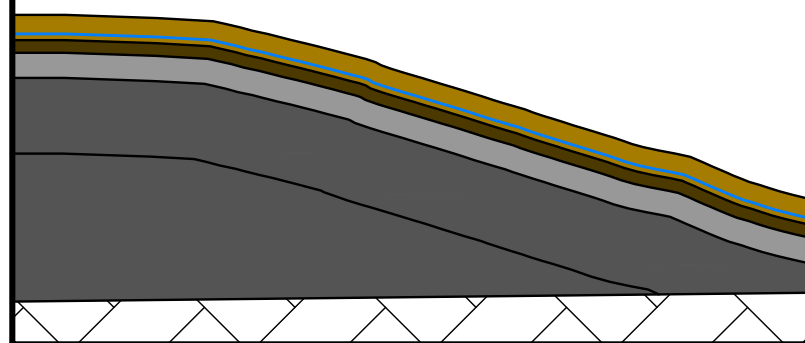
LEGEND:

- EXISTING BOREHOLE LOCATION BY OTHERS (EXP, 2025)
- EXISTING BOREHOLE LOCATION (PATERSON GROUP, 2023)
- EXISTING BOREHOLE LOCATION BY OTHERS (PINCHIN, 2021)
- EXISTING BOREHOLE LOCATION (PATERSON GROUP, 1984)
- BROWN SILTY CLAY CRUST
- BROWN SILTY CLAY
- STIFF TO FIRM GREY SILTY CLAY
- FIRM GREY SILTY CLAY
- BEDROCK
- LONG-TERM GROUNDWATER TABLE (1.5m DEEP)

SCALE: 1:1500



ENLARGEMENT (SCALE 1:600)



1	AS PER ADDITIONAL BOREHOLE COVERAGE	29/06/2026	NFRV
NO.	REVISIONS	DD/MM/YYYY	INITIAL

Scale:	1:1500	Date:	06/2026
Drawn by:	NFRV	Report No.:	PG8011-LET.01
Checked by:	NFRV	Dwg. No.:	PG8011-2
Approved by:	DP	Revision No.:	1