

Hydrogeological Study Proposed Residential Development

4850 Bank Street Ottawa, Ontario

Prepared for Regional Group Report PH5087-REP.01.R1 dated October 17, 2025



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1.0 INTRODUCTION

1.1 Background

Paterson Group (Paterson) was retained by Regional Group to conduct a hydrogeological study for the proposed residential development located at 4850 Bank Street in the City of Ottawa (hereinafter referred to as the "subject site"). The location of the subject site is shown on Drawing PH5087-1 - Site Plan appended to this report. This report incorporates the findings of Paterson Report PG6912-1 prepared concurrently.

1.2 Scope of Work

Paterson has completed this report in accordance with the scope prepared by Paterson. As per the agreed upon scope, the purpose of this study was to:

	Characterize the hydrogeological setting of the subject site. Consideration was given to bedrock and surficial geology, aquifer systems, groundwater levels, hydraulic properties and catchment characteristics. A groundwater impact assessment to determine potential impacts to adjacent infrastructure, well users and the surrounding environment.					
Ass	Additionally, the study was to include the standard components of a Water Budget Assessment as per the City of Ottawa's Water Budget Assessment Terms of Reference, which included the following:					
	Review related higher-level studies. Conduct pre and post-development water budget analyses, including water budget equations, to determine the hydrogeological function of the subject site in order to assess the need for supplemental stormwater management measures.					
	Develop a conceptual model to characterize pre and post-development hydrologic and hydrogeologic site conditions.					
	Identify sensitive hydrologic and hydrogeologic features (if any) within the study area.					
	Identify water budget targets (if applicable) to mitigate post-development hydrologic and hydrogeologic impacts.					
	Identify how climate change projections may impact the water budget.					



2.0 RELATED REPORTS

In addition to a review of the general literature summarized in the following sections and in the 'References' section of this report (MECP water well mapping, available geological and physiographic mapping), Paterson reviewed the following site-specific/related reports:

J	PG6912-1 Rev.1 Geotechnical Investigation - Proposed Residential
	Development & Off-Site Sewer Installation - 4850 Bank Street - prepared by
	Paterson Group - prepared concurrently with this study.
J	S4 Leitrim Urban Expansion Area - Serviceability Report - prepared by Arcadis
	- prepared concurrently with this study.
J	Leitrim West Urban Expansion Area - S4 Environmental Impact Study prepared
	by Arcadis – prepared concurrently with this study.
]	Pathways at Findlay Creek - Constructed Channel Monitoring (Year 5) -
	prepared by Cambium Inc October 30, 2024.
J	PE6336-1 Phase I-II Environmental Site Assessment - 4850 Bank Street -
	prepared by Paterson Group - December 20, 2023.
]	Environmental Management Plan for the Remer and Idone Lands - Ottawa,
	Ontario - Prepared by Golder Associates - February 2015



3.0 METHOD OF INVESTIGATION

3.1 Records Review

A review of available geological, and hydrogeological data was completed as a part of this assessment. However, the literature review and previous reports did not provide site-specific data regarding overburden and bedrock aquifers, recharge and discharge conditions or flow contributions to the nearby water features. Further detail is provided in the following sections.

3.2 Field Program

The geotechnical and hydrogeological field programs were developed to assess geology, groundwater conditions, hydraulic gradients and the overall hydrologic/hydrogeologic function of the subject site. The test holes were advanced to various depths across the site to assess hydrogeological and geotechnical conditions.

Geotechnical field investigations were completed by Paterson at the subject site between December 2023 and July 2025. During this time, 18 boreholes were advanced to a maximum depth of 8.5 m below ground surface (bgs). The location of the test holes are shown on Drawing PG6912-1 - Test Hole Location Plan, included in Appendix 1.

Soil samples were obtained from the boreholes by means of split spoon sampling and the sampling of shallow soils directly from auger flights. Split-spoon samples were taken at approximate 0.76 m intervals. In addition to soil sampling, rock core samples were obtained with the use of a standard diamond drill bit. The depth at which split-spoon, auger samples and rock core samples were obtained from the test holes are shown as "SS", "AU" and "RC", respectively, on the Soil Profile and Test Data sheets, included in Appendix 1.

All samples were classified on site, placed in sealed plastic bags and were transported to our laboratory for further review and testing. Transportation of the samples was completed in accordance with ASTM D4220-95 (2007) - Standard Practice for Preserving and Transporting Soils. Rock core samples were recovered from select boreholes (BH1-25 and BH2-25) drilled during the July 2025 geotechnical investigation using a core barrel and diamond drilling techniques. The bedrock samples were classified on site, placed in hard cardboard core boxes and transported to Paterson's laboratory.



The Standard Penetration Test (SPT) was conducted in conjunction with the recovery of the split-spoon samples. The SPT results are recorded as "N" values on the Soil Profile and Test Data sheets. The "N" value is the number of blows required to drive the split-spoon sampler 300 mm into the ground after an initial penetration of 150 mm using a 63.5 kg hammer falling from a height of 760 mm. This test was done in accordance with ASTM D1586-11 - Standard Method for Penetration Test and Split-Barrel Sampling of Soils.

The recovery value and a Rock Quality Designation (RQD) value were calculated for each drilled section of bedrock and are presented on the borehole logs. The recovery value is the length of the bedrock sample recovered over the length of the drilled section. The RQD value is the total length of intact rock pieces longer than 100 mm over the length of the core run. The values indicate the bedrock quality.

Reference should be made to the Soil Profile and Test Data sheets presented in Appendix 1 for specific details of the soil profiles encountered at the test hole locations.

Drawdown Analysis - Hydraulic Conductivity Testing

Hydraulic conductivity testing (slug testing) was completed at the monitoring wells installed during the December 2023 and July 2025 geotechnical investigations. Falling head and rising head tests (slug tests) were completed in accordance with ASTM Standard Test Method D 4404 - Field Procedure for Instantaneous Change in Head (Slug) Tests for Determining Hydraulic Properties of Aquifers.

Slug testing was completed during the month of July 2025 by Paterson personnel. The general test method consisted of measuring the static water level in the well, followed by inducing a near-instantaneous change of head in the well and subsequent monitoring of water level recovery with an electronic water level meter and a water level data logger. The change in head was induced by the introduction of either an acetal slug, 0.9 m in length and 38 mm in diameter, or a metal slug, 1.0 m in length and 19 mm in diameter, depending on the well diameter. The slug was introduced to raise the groundwater level in the monitoring well, following which the decrease in water level over time was monitored (falling head test). Once the water level had stabilized (or nearly stabilized), the slug was then removed to lower the groundwater level, following which the increase in water level over time was monitored (rising head test).

Following the completion of the slug tests, the test data was analyzed using the AQTESOLV Pro Version 4.5 aquifer analysis software package by HydroSOLVE



Inc. and the results were processed as per the method set out by Hvorslev (1951). Assumptions inherent in the Hvorslev method include a homogeneous aquifer of infinite extent and a screen length significantly greater than the monitoring well diameter. The assumption regarding screen length and well diameter is considered to be met based upon a typical length of 1.52 m and a diameter of 0.03 to 0.05 m.

While the idealized assumptions regarding aquifer extent, homogeneity are not strictly met in this case (or in any real-world situation), it has been our experience that the Hvorslev method produces effective point estimates of hydraulic conductivity in conditions similar to those encountered at the subject site. Hvorslev analysis is based on the line of best fit through the field data (hydraulic head recovery vs. time), plotted on a semi-logarithmic scale.

Infiltration Testing

In-situ infiltration testing was conducted at the subject site during the month of July 2025 using a Pask (Constant Head) Permeameter. At each testing location, up to two in-situ infiltration tests were conducted at depths of approximately 0.5 to 1.0 m below ground surface (bgs). The tests were conducted to provide general coverage of the subject site and are shown on Drawing PG6912-1 - Test Hole Location Plan included in Appendix 1.

At each testing location, an 83 mm diameter hole was excavated using a Riverside/Bucket auger to remove topsoil and other subsoil material until the desired testing elevation. Up to two holes were advanced at each testing location to approximately 0.5 and 1.0 m bgs. All soil from the auger flights was visually inspected and initially classified on site. An aggregated soil sample was gathered at each test location. Each test was conducted by filling the permeameter reservoir with water and inverting it into the hole, ensuring it was relatively vertical and resting at the bottom of the hole. The water level of the reservoir was monitored at 0.5 to 5 minute intervals until the rate of fall out of the permeameter reached equilibrium, known as a quasi "steady state" flow rate. Quasi steady state flow was considered to be obtained once 3 to 5 consecutive rate of fall readings with identical values were measured. The values for the steady state rate of fall were recorded for each test. The steady state rate of fall was converted to a field saturated hydraulic conductivity value (K_{fs}) using the Engineering Technology Canada Ltd. conversion tables. Unfactored infiltration rates were estimated based on the methodology outlined in Appendix C of the Credit Valley Conservation's Low Impact Development Stormwater Management Planning and Design Guide.



3.3 Laboratory Testing

All soil samples were retained for laboratory review following the field portion of the subsurface investigation. The soils were classified in general accordance with ASTM D2488-09a, Standard Practice for Description and Identification of Soils (Visual-Manual Procedure). Based on the soil descriptions across the subject site during the geotechnical investigations, these samples are considered to be sufficiently representative of the site.

3.4 Monitoring Well Installations

As part of the December 2023 and July 2025 geotechnical field programs, monitoring wells were installed in select boreholes to permit the monitoring of groundwater levels and conduct drawdown analyses. The well installations were compliant with ASTM D5092 standards.

3.5 Water Level Measurements

Following the completion of the December 2023 and July 2025 drilling programs, groundwater levels were measured at the monitoring well locations. Water levels were measured using an electronic water level meter relative to the ground surface elevation at each location and are noted on the Soil Profile and Test Data sheets, included in Appendix 1.

In addition to the manual groundwater level measurements, select wells were outfitted with dataloggers to record long-term fluctuations in monitoring well water elevations. The monitoring program began in summer 2025 and will last until spring high water levels are captured in 2026.

3.6 Surveying

The test hole locations and ground surface elevations at each test hole location completed by Paterson were surveyed using a GPS unit with respect to a geodetic datum. The locations and ground surface elevations for each test hole are presented on Drawing PG6912-1 - Test Hole Location Plan, included in Appendix 1.



4.0 REVIEW AND EVALUATION

4.1 Physical Setting

At the time of the field investigations, the subject site was undeveloped and heavily forested with mature trees and grass/shrubs. The subject site is located in the City of Ottawa, Ontario and is bordered by Bank Street to the east, forested land and a driving range to the south, forested land and a wetland to the west and a residential development and commercial properties to the north. The location of the subject site is shown on Drawing PH5087-1 - Site Plan, appended to this report.

Based on mapping provided by the Ministry of Conservation and Parks (MECP) Source Protection Information Atlas, the subject site is located within the Castor River subwatershed. There are several surface water features located within the subject site and within 500 m of the subject site. These include unnamed scratch ditches, drainage ditches and conveyance channels. Also, the Leitrim Wetlands lies west of the subject site. The ground surface at the subject site generally slopes downwards to the east and west from the central portion of the subject site. The site is generally at grade with adjacent roadways and properties. The site slopes from east to west with an elevation difference of approximately 6-8 m.

According to available mapping from the Ontario Geologic Survey (OGS; MRD 228), the majority of the subject site is located in the sand plans physiographic region. The eastern tip of the subject site is located in the limestone plains physiographic region. The sand plains region is characterized by silty sand deposits. The limestone plains region is characterized by limestone bedrock. These are generally consistent with field observations at the subject site.



4.2 Geology

Surficial Geology

Overburden mapping provided by the OGS was reviewed as part of this assessment. Available mapping (MRD 128) indicates that overburden soils throughout the subject site consist of till (stone-poor, sandy silt to silty sand-textured till on paleozoic terrain). Overburden soil mapping is shown on Drawing PH5087-3 - Surficial Geology Plan, appended to this report.

Overburden soils identified during the geotechnical investigations by Paterson between December 2023 and July 2025 were generally consistent with the available mapping. Soils generally consisted of topsoil overlying a glacial till deposit followed by bedrock. Fill material was encountered at one (1) borehole location. The fill material generally consisted of brown silty sand with gravel, crushed stone and trace clay, extending to a maximum observed depth of 0.9 m bgs. The glacial till deposit generally consisted of a silty sand matrix with varying amounts of gravel, cobbles and boulders, extending to a maximum observed depth of 6.7 m bgs.

Specific details are provided on the Soil Profile and Test Data Sheets included in Appendix 1 of this report. More details regarding the overburden soils can be found in Paterson Report PG6912-1 prepared concurrently with this report.

Bedrock Geology

Bedrock was encountered between 5.0 to 5.2 m bgs during Paterson's 2025 geotechnical field investigation and cored to a maximum depth of 8.5 m bgs. The bedrock was observed to vary from poor to excellent quality. Based on available mapping, the overburden drift thickness at the subject site ranges between 3 and 15 m. Bedrock geology mapping is shown on Drawing PH5087-4 - Bedrock Geology Plan, appended to this report.

Karst Features

The term "karst" refers to a geologic formation characterized by the dissolution of carbonate bedrock, such as limestone or dolostone. In order for karstification to occur, precipitation must be able to infiltrate the top of the bedrock, causing dissolution which enlarges previously existing joints and bedding planes. Based on available mapping by the OGS (GRS 005), the subject site is located within an area that does not contain karstic landforms.



4.3 Hydrogeological Setting (Conceptual Model)

Based on the field investigations at the subject site, Paterson used borehole data, existing water well records, topography, monitoring well water levels, hydraulic conductivity and infiltration rates to develop a conceptual model of the transport fate of surface water and groundwater at the subject site. Information related to the conceptual flow model is described below.

Existing Aquifer Systems

Aquifer systems may be defined as geological media, either overburden soils or fractured bedrock, which permit the movement of groundwater under hydraulic gradients. In general, aquifer systems may be present in overburden soils or bedrock. Groundwater was observed within the overburden soils at the subject site and the soils consist of moderate hydraulic conductivities. Given the limited quantity of groundwater within the overburden aquifer, it is not considered an adequate source for water supply wells. If water supply wells are still in use in the vicinity of the subject site, it is anticipated that they are accessing bedrock aquifers.

Based on a review of the MECP water well record database, available geological mapping and field investigations, Paterson has identified two (2) aquifer systems in the vicinity of the study area which consist of the overburden aquifer and underlying bedrock aquifer. The bedrock aquifer system consists of dolostone or dolostone and sandstone of the Oxford or March Formations, respectively. Given the temporal variability between the water well records and lack of geodetic elevation references, we were only able to approximate a potentiometric surface of the deeper bedrock aquifer which was inferred to range between approximately 2 to 15 m bgs. Surrounding water well records are included in Appendix 5.

Groundwater Levels

Piezometers and monitoring wells were placed across the study area for the purpose of monitoring groundwater levels. The piezometers and monitoring wells were installed in the overburden and bedrock. Groundwater levels were observed to be between 0.13 and 1.23 m bgs in the piezometers and between 0.39 and 1.99 m bgs in the monitoring wells. The initial groundwater level measurements are shown on the Soil Profile and Test Data Sheets appended to this report in Appendix 1. Groundwater elevations that were collected on July 3, 2025, were used to determine hydraulic gradients and the general groundwater flow direction at the subject site which is shown on Drawing PH5087-5 - Groundwater Contour Plan appended to this report. The manual measured groundwater levels are presented in Table 1 below.



Table 1 – Summary of Manual Groundwater Level Measurements				
	Ground Manual Groundwater Level			
Borehole ID	Surface	Measu	ırement	Date Recorded
Dolellole ID	Elevation	Depth	Elevation	Date Necolded
	(m asl)	(m bgs)	(m asl)	
BH 1-25*	101.45	1.74	99.71	July 3, 2025
BH 2-25	103.07	2.18	100.89	July 3, 2025
BH 3-25*	109.30	1.99	107.31	July 3, 2025
BH 1-23*	102.56	2.47	100.09	July 3, 2025
BH 2A-23*	102.25	2.09	100.16	July 3, 2025
BH 6C-23*	101.59	1.21	100.38	July 3, 2025
BH 10-23*	103.35	1.25	102.10	July 3, 2025
BH 1-23*	102.56	1.40	101.16	January 29, 2024
BH 2A-23*	102.25	0.94	101.31	January 29, 2024
BH 10-23*	103.35	0.26	103.09	January 29, 2024
BH 1-23*	102.56	1.30	101.26	December 15, 2023
BH 2A-23*	102.25	0.80	101.45	December 15, 2023
BH 3-23	105.46	1.23	104.23	December 15, 2023
BH 4-23	106.70	Dry	-	December 15, 2023
BH 5-23	107.88	0.40	107.48	December 15, 2023
BH 6C-23*	101.59	0.76	100.83	December 15, 2023
BH 7-23	109.28	0.66	108.62	December 15, 2023
BH 8A-23	107.22	0.27	106.95	December 15, 2023
BH 9-23	106.87	0.13	106.74	December 15, 2023
BH 10-23*	103.35	0.39	102.96	December 15, 2023

Note: The ground surface elevation at each borehole location was surveyed by Paterson using a handheld GPS and was referenced to a geodetic datum.

*Borehole instrumented with groundwater monitoring well.

Horizontal Hydraulic Gradients

Due to the nature of the water levels obtained from field work conducted at the subject site (groundwater monitoring wells), the absolute direction of horizontal hydraulic gradients in the vicinity of the subject site was not determined. However, using the available data, it was possible to approximate the horizontal hydraulic gradients in the overburden materials given that the horizontal hydraulic gradient between any 2 points is the slope of the hydraulic head between those points:

$$i=h_2-h_1/L$$

Where: i = horizontal gradient

h = water elevation (m asl)

L = horizontal distance between test hole locations



Using the above noted formula, the horizontal hydraulic gradients within the eastern portion of the site were observed to be in an approximate easterly orientation with a magnitude of approximately 0.022 m/m in the glacial till deposit. The horizontal hydraulic gradients within the western portion of the site were observed to be in an approximate westerly orientation with a magnitude of approximately 0.016 m/m in the glacial till deposit towards the Leitrim Wetlands. The approximate groundwater flow directions are presented on Drawing PH5087-5 - Groundwater Contour Plan, appended to this report. Regional groundwater flow in the overburden is expected to be in a southeasterly orientation towards Castor River which is consistent with available watershed mapping.

Hydraulic Conductivity

Hydraulic conductivity testing (slug testing) was completed by Paterson as part of the field investigations at the subject site. The test data was analyzed as per the method set out by Hvorslev (1951). The testing yielded hydraulic conductivity values of 3.71×10^{-7} to 1.21×10^{-5} m/sec for glacial till and 1.05×10^{-4} to 1.20×10^{-4} m/sec for bedrock. Hydraulic conductivity results are summarized in Table 2 below and have been included in Appendix 3.

Table 2 – Summary of Hydraulic Conductivity (Slug) Testing Results					
Test Hole ID	Ground Surface Elevation (m asl)	Testing Elevation (m asl)	Hydraulic Conductivity (m/sec)	Test Type	Soil Type
BH1-25	101.45	93.17-94.67	1.05 x 10 ⁻⁴	Falling Head	Bedrock
DI11-23	101.45	93.17-94.07	1.20 x 10 ⁻⁴	Rising Head	Dediock
BH3-25	109.30	103.15-104.65	1.33 x 10 ⁻⁶	Falling Head	Glacial Till
DI 13-25		103.15-104.65	2.03 x 10 ⁻⁶	Rising Head	Glaciai IIII
BH1-23	102.56	98.80-100.30	3.71 x 10 ⁻⁷	Falling Head	Glacial Till
BH6C-23	101.59	96.97-98.47	9.00 x 10 ⁻⁶	Falling Head	Glacial Till
		90.97-90.47	1.21 x 10 ⁻⁵	Rising Head	Giaciai IIII
DU10 22	103.35	09 27 00 77	4.28 x 10 ⁻⁶	Falling Head	Glacial Till
BH10-23		98.27-99.77	5.01 X 10 ⁻⁶	Rising Head	Giaciai IIII

In-Situ Infiltration Testing

In-situ infiltration testing was conducted by Paterson using a Pask (Constant Head Well) Permeameter across the subject site to determine the field saturated hydraulic conductivities (K_{fs}) and their respective unfactored infiltration rates of the unsaturated overburden soils. The tests were conducted in a manor to provide general coverage across the subject site. Estimated unfactored infiltration rates varied between 24 and 82 mm/hr for the glacial till unsaturated overburden soils. The variations in the infiltration rates are dependent on the



composition/compaction of the glacial till at a given location. In-situ infiltration testing results can be found below in Table 3.

Table 3 – Summary of Field Saturated Hydraulic Conductivity Testing Results and Estimated Infiltration Rates					
Test Completed Adjacent to Borehole ID	Ground Surface Elevation (m asl)	Infiltration Testing Elevation (m asl)	K _{fs} (m/sec)	Unfactored Infiltration Rate (mm/hr)	Material
PT1-25	101.67	101.17	2.7 x 10 ⁻⁷	33	Glacial Till
P11-25	101.67	100.67	9.3 x 10 ⁻⁸	24	Glacial Till
PT2-25	105.82	105.32	5.3 x 10 ⁻⁶	72	Glacial Till
F12-25	105.62	104.82	8.5 x 10 ⁻⁶	82	Glacial Till
PT3-25	107.29	106.79	2.7 x 10 ⁻⁷	33	Glacial Till
P13-25	107.29	106.49	1.4 x 10 ⁻⁷	27	Glacial Till
PT4-25	109.49	108.99	2.1 x 10 ⁻⁶	56	Glacial Till
F14-25		108.54	1.1 x 10 ⁻⁶	47	Glacial Till
PT5-25	106.11	105.56	1.4 x 10 ⁻⁷	27	Glacial Till

^{*}Field saturated hydraulic conductivity (Kfs)

Groundwater Recharge and Discharge

In general, groundwater will follow the path of least resistance from areas of higher hydraulic head to areas of lower hydraulic head. Upward and downward hydraulic gradients are typically indicative of areas of discharge and recharge, respectively.

It is our interpretation that there is some recharge occurring at the subject site due to the soils identified at the time of the field investigations, and the infiltration testing results at the subject site. Recharge of the shallow overburden aquifer will occur as precipitation infiltrates the subsoils where it will flow vertically downward through the unsaturated surficial soils before intercepting the overburden aquifer. However, it should be noted that the site is not mapped as a significant groundwater recharge area (SGRA) by the MECP.

With regards to discharge zones, neither the topographical nor geological conditions are suitable for discharge to be occurring on a large scale at the subject site.

The subject site intersects one subwatershed as previously mentioned. However, within the confines of the subject site, shallow groundwater was found to have flow generally travelling in an eastward direction within the eastern portion of the subject site, and in a western direction within the western portion of the subject site, given the topographic relief.

^{**}The infiltration rates do not include a safety correction factor. Based on our testing results, a minimum safety correction factor of 2.5 should be applied to the values.



It should be noted that the subject site is not identified by the MECP as a drinking water protection zone.

Catchment Areas

The subject site is located within the Castor River subwatershed. As shown on Drawing PH5087-5 - Groundwater Contour Plan, the groundwater flows in an eastward direction within the eastern portion of the subject site, and in a western direction towards the Leitrim Wetlands within the western portion of the subject site. Therefore, it is Paterson's opinion that the subject site is predominantly characterized by two catchment areas.

Detailed servicing plans were unavailable at the time of report preparation. Based on discussions with the civil design team, the site will continue to be characterized by two catchment areas under post-development which will function similarly to pre-development conditions. However, it is anticipated that the size of the western catchment area will increase to allow for increased hydration to the adjacent Leitrim Wetlands as per recommendations by the Rideau Valley Conservation Authority (RVCA).

Groundwater Inflow/Dewatering Requirements

Three (3) potential sources of dewatering have been identified at the subject site. The sources consist of the excavation footprints related to the building foundations, servicing trenches and stormwater management pond (SWMP). Details regarding the excavation footprints and depths for each potential dewatering source were unavailable at the time of report preparation. Therefore, the building, servicing and SWMP excavations are assumed to encompass an area of approximately 750 m², 125 m² and 5,000 m², respectively, for a preliminary dewatering assessment.

Based on available conceptual drawings for the site, excavation sizes were estimated based on proposed building footprints, typical servicing excavation sizes based on previous experience at similar sites and the proposed SWMP footprint.

The infiltration rates provided for the following sources were calculated using the Dupuit Forchheimer method:

 $Q = \pi k((h_0^2 - h_p^2)/ln(R/r))$

 \square k = hydraulic conductivity (m/sec)

 \Box h_0 = thickness of the aquifer (m)



h_p = thickness of the aquifer from the base of the excavation to the
base of the aquifer (m)
R = effective drawdown radius for the excavation (m)
r = equivalent radius of the excavation (m)

The groundwater infiltration calculations for the excavation footprints are provided in Appendix 3 of this report.

The stratigraphy within the anticipated saturated depth of the building excavations generally consists of a glacial till deposit. Specific design details were not available at the time of report preparation. For the purpose of this study, it has been assumed the buildings will consist of slab-on-grade construction or one basement level with a maximum excavation depth of 3 m bgs. Calculations are based on an excavation size of approximately 750 m² and a saturated depth of 2 m. Using a representative hydraulic conductivity of 3.03 x 10⁻⁶ m/sec (geometric mean of the calculated hydraulic conductivities), the steady state volume of groundwater anticipated is approximately 22,000 L/day, per excavation.

The stratigraphy within the anticipated saturated depth of the servicing excavations generally consists of a glacial till deposit. Specific design details were not available at the time of report preparation. For the purpose of this study, it has been assumed the servicing trenches will have a maximum excavation depth of 5 m bgs. Calculations are based on an excavation size of approximately 125 m² and a saturated depth of 4 m. Using a representative hydraulic conductivity of 3.03 x 10⁻⁶ m/sec (geometric mean of the calculated hydraulic conductivities), the steady state volume of groundwater anticipated is approximately 12,000 L/day, per excavation.

The stratigraphy within the anticipated saturated depth of the SWMP excavation generally consists of a glacial till deposit. Specific design details were not available at the time of report preparation. For the purpose of this study, it has been assumed the SWMP excavation will have a maximum excavation depth of 5 m bgs. Calculations are based on an excavation size of approximately 5,000 m² and a saturated depth of 4 m. Using a representative hydraulic conductivity of 3.03 x 10⁻⁶ m/sec (geometric mean of the calculated hydraulic conductivities), the steady state volume of groundwater anticipated is approximately 44,000 L/day.

It is recommended that source specific dewatering calculations be completed once more specific development details are available.



5.0 SITE SPECIFIC WATER BUDGET ASSESSMENT

The site-specific water budget assessment (SSWB) was conducted to determine the hydrogeological function of the subject site, to identify infiltration potential and to identify opportunities for supplemental stormwater management measures. At the time of the field investigations the study area mainly consisted of mature trees, grass and shrubs. The pre and post-development terrain compositions are illustrated on Drawings PH5087-6 - Pre-Development Terrain Composition Plan and PH5087-7 - Post-Development Terrain Composition Plan, appended to this report. It should be noted that the future Earl Armstrong extension was excluded from the water budget study area as it is not a component to the current serviceability study for the development.

5.1 Calculations

Thornthwaite and Mather Water Balance Calculations

When falling precipitation intercepts the ground, three possible outcomes arise. The water can either evaporate/transpire back into the atmosphere (evapotranspiration), infiltrate into the surface soils (infiltration) or leave the area as runoff.

The method employed by Thornthwaite and Mather (1957) was used along with modelling software by Environment Canada's Engineering Climate Services Unit (EC-ECS) to determine the partitioning of water throughout various portions of the hydrologic cycle. Inputs into the modelling program included monthly temperature, precipitation, water holding capacities and site latitude. Using the long-term averages of these variables, it was possible to calculate annual potential and actual evapotranspiration, change in soil moisture storage and the water surplus.

The formula employed by Thornthwaite and Mather is as follows:

$$S = R + I = P - ET$$

Where: S = surplus (mm/year)

R = annual runoff (mm/year)I = annual infiltration (mm/year)P = annual precipitation (mm/year)

ET = annual evapotranspiration (mm/year).



Shallow unsaturated soils within the study area generally consisted of topsoil overlying a glacial till deposit. Given the similar soil profiles across the entire study area, the above noted calculations were carried out for the soil moisture holding capacity of a fine sandy loam.

Based on the location of the site within the Ottawa area, climatic data was obtained from the climate station located at the McDonald-Cartier International Airport covering the period of January 1939 to December 2022. The information was provided by Environment Canada's Engineering Climate Services Unit and is presented in Appendix 2 of the report.

Table 4, below, displays the soil types present within the study area and their associated water holding capacities (WHC) as well as the actual evapotranspiration (AET) and surplus data. For the purposes of this study, AET values were used as they account for accumulated soil moisture deficit. This deficit represents the volume of water retained within the available pore spaces of the soil and is subtracted from the potential evapotranspiration (PET) value to more accurately calculate the water surplus. The monthly/annual water balance data is presented in Tables 6 and 7 in Appendix 2 of this report. For the purpose of this study, 70% of the detached homes property footprints are considered to be impervious surfaces (100% of surplus will result in runoff) and 30% are considered to be urban lawns.

Table 4 - Site Specific Water Surplus Information			
Land Use Unit	Water Holding Capacity (mm)	Actual Evapotranspiration (mm/year)	Surplus Water (mm/year)
Impervious Surfaces	N/A	145*	759
Urban Lawn/Shallow Rooted Crops (Fine Sandy Loam)	75	525	378
Mature Forests (Fine Sandy Loam)	300	605	298

Table reproduced using WHC values from MOE (2003) - Stormwater Management Planning and Design Manual and modelling data from Environment Canada's Engineering Climate Services Unit.

*Values based on evaporation information for urban areas (16% of precipitation) included in the Eastern Ontario Water Resources Management Study prepared by CH2M HILL Canada Limited (March 30, 2001).

Infiltration Factors

In order to break down the surplus water values for the various materials into infiltration and runoff, various factors must be considered. The MOE Stormwater Management Planning and Design Manual (2003) lists three main factors that



contribute to surface water infiltration rates.

The first factor is topography, which is broken down further into three sections: flat and average slope, rolling land and hilly land. Flat and average slope provides the greatest potential for infiltration and has the largest infiltration factor applied to it (0.3), while the other two have progressively lower infiltration factors (rolling land is 0.2 and hilly land is 0.1).

The second factor is soil, which is also broken down further into three sections: tight impervious clay, medium combinations of clay and loam and open sandy loam. Open sandy loam provides the greatest potential for infiltration (infiltration factor of 0.4) while the other two have progressively lower potential for infiltration to occur (infiltration factor for medium combinations of clay and loam is 0.2 and for tight impervious clay is 0.1).

The final factor the MOE manual uses to partition infiltration from runoff is land cover. It is broken down into two sections: open fields/cultivated lands and woodlands. Woodlands have greater infiltration potential and an infiltration factor of 0.2. Open fields and cultivated lands have lower potential and with an infiltration factor of 0.1. A summary of the MOE manual's descriptors and their associated infiltration factors is shown below in Table 5.

Table 5 - MOE (2003) Infiltration Factors			
Description of Area/Development Site	Value of Infiltration Factor		
Topography			
Flat and average slope (<0.6 m/km)	0.30		
Rolling land (slope of 2.8-3.8 m/km)	0.20		
Hilly land (slope of 28-47 m/km)	0.10		
Soil			
Tight impervious clay	0.10		
Medium combinations of clay and loam	0.20		
Open sandy loam	0.40		
Cover			
Open fields/cultivated lands	0.10		
Woodlands	0.20		
Table reproduced from MOE (2003) - Stormwater Management Planning and Design Manual.			

The topography of the study area is classified as hilly land (slope of 28-47 m/km throughout the subject site) Therefore, a pre-development topography infiltration factor of 0.1 was given for the materials analysed on this property. In order for development to proceed, it is expected that alterations will be made to the topography of the site. In general, it is expected that the overall slope of the site will be reduced to accommodate buildings and parking areas. Therefore, the



topography of the subject site under post-development conditions will consist of a mix rolling and hilly land and was therefore assigned a post-development topography infiltration factor of 0.15. An infiltration factor of 0 was assigned to the impervious surfaces due to its negligible infiltration capacity.

As previously discussed, soils within the study area generally consisted of topsoil overlying a glacial till deposit. Therefore, a pre-development soil infiltration factor of 0.3 was given for the materials analysed on this property. Under post-development conditions, the majority of the site will consist of either landscaped areas or impervious surfaces, with soil infiltration factors ranging from 0.3 for fine sandy loam to 0 for impervious surfaces.

At the time of the field investigations, the subject site generally consisted of mature forest. A pre-development vegetation infiltration factor of 0.2 was therefore used for the site. Post-development, it is expected the majority of the trees remaining on site will be removed to accommodate buildings, parking areas and roadways. As such, a post-development vegetation infiltration factor of 0.1 was assigned to the site, except for impervious surfaces, which were given an infiltration factor of 0 due to its negligible potential to benefit from vegetation cover.

The pre and post-development infiltration factors for all materials considered are included in the water budget calculations provided in Table 8 and Table 9 included in Appendix 2 of this report.

5.2 Pre and Post-Development Water Budget

The pre-development water budget analysis conducted for the study area determined that an estimated 21,992,400 L/year of surplus water currently infiltrates the surface soils. The remaining estimated 14,661,600 L/year of surplus leaves the site as runoff.

The post-development water budget analysis determined that an estimated 6,168,925 L/year of surplus water will infiltrate the surface soils and approximately 72,011,680 L/year will leave the site as runoff. These values equate to an approximate decrease in infiltration of 72% and an increase in runoff of 391%.

The main variable that changed from pre-development conditions to post-development conditions was the addition of approximately 8.8 hectares of impervious surfaces. This results in reducing the area of pervious materials throughout the subject site, therefore, reducing the overall infiltration potential of the subject site. The remaining areas that are not being converted to impervious surfaces will become landscaped surfaces characterized by urban lawn (fine sandy



loam) material. Also, it should be noted that the SWMP area was excluded from the post-development water budget analysis given that this area is designed to manage stormwater runoff and does not contribute to the infiltration or runoff potential of the site.

It is important to note that the post-development water budget analysis for the subject site does not consider any potential infiltration of the impervious surfaces (100% runoff was taken as a conservative approach). In reality, some portion of surface water that lands on impervious surfaces infiltrates (asphalt is not 100% impervious) or is diverted to grassed areas where additional infiltration may occur. As such, the post-development runoff volumes should be considered a conservative estimate and not expected to definitively represent future conditions.

Details of the pre and post-development water budget analyses are presented in Tables 8 and 9, included in Appendix 2 of this report. Additional water budget calculations were completed for each catchment area and are also included in Appendix 2.



6.0 GROUNDWATER IMPACT ASSESSMENT

6.1 Impact of Proposed Development on Surrounding Infrastructure

As previously discussed, soils within the subject site generally consisted of topsoil, overlying a glacial till deposit. The expected groundwater infiltration will be encountered within the glacial till deposit.

The steady-state radius of influence calculations completed were based upon the Sichardt equation as shown below. The assumed setting for the analytical solution was one in which open cut trenches were used to install the services at the subject site, creating an unconfined condition which would allow use of the equation to determine the radius of influence.

$$R = 3000 * \Delta h(K^{0.5})$$

Where: R = radius of influence (m)

 Δh = expected groundwater drawdown (m)

K = hydraulic conductivity (m/sec).

For the purposes of completing the calculations, the following values were used in the analysis for the glacial till:

 \Box $\Delta h = 2 \text{ to } 4 \text{ m}$

 \square K = 3.71 x 10⁻⁷ to 1.21 x 10⁻⁵ m/sec, based on site specific hydraulic conductivity values of the glacial till.

Using the above equation and assumptions, a radius of influence of 5 to 42 m will develop as a steady state condition within the glacial till, extending from the edge of the excavation, in the area of the subject site, depending on the groundwater levels and hydraulic properties of the specific soils encountered.

The surrounding area consists of mostly low-rise residential homes and commercial properties. The buildings located within the theoretical radius of influence are generally expected to be founded on the glacial till deposit. The majority of the groundwater infiltration is expected to occur within the glacial till with minimal compressibility. Furthermore, water takings are also expected to be short term in duration, given the nature of the development. As such, adverse effects to surrounding infrastructure related to dewatering activities at the subject site are expected to be negligible.



6.2 Impact of Proposed Development on Existing Well Users

A search of the Ontario Water Well Records online mapping database indicates there are a number of wells within 500 m of the site as depicted on Drawing PH5087-2 - MECP Water Well Location Plan, appended to this report. However, it is expected that the majority of these wells are either no longer in use due to their installation dates and developed nature of the region or are monitoring well installations. Additionally, the majority of properties surrounding the site are serviced by municipal water supplies. Any wells that may still be in use are cased well below the anticipated excavation depths associated with the proposed development and are accessing the deeper bedrock aquifer. Furthermore, the properties that reside on the eastern boundary of the subject site will have the opportunity to connect to municipal services once the development proceeds. Therefore, it is anticipated that the existing wells will either no longer be in use or have adequate vertical and horizontal separation from proposed construction activities. Therefore, dewatering activities at the subject site are not expected to cause any interference to the water supply of surrounding properties or other negative impacts.

As the potential to interfere with the water quality/quantity of existing well users in the area is negligible, a water well monitoring program is not recommended for the aforementioned water takings. If wells are found to remain in existence on the subject site, they should be decommissioned in accordance with Ontario Regulation 903.

If construction activities are shown to cause negative impacts to the water supplies of existing well users, the contractor shall take action to make available a supply of water equivalent in quality and quantity of their typical takings or shall compensate those affected for reasonable costs for doing so, or shall reduce water taking amounts to alleviate the negative impacts. The contractor shall provide temporary water supplies, to those affected, to meet their typical takings or compensate such persons for reasonable costs associated to do so until permanent restoration of the affected water supply or an equivalent source.

6.3 Impact of Proposed Development on the Environment

A search of the MECP Environmental Site Registry for Records Site Condition (RSCs) was conducted as part of the assessment of the site, neighbouring properties and the general area. No RSCs were identified within the 500 m of the subject site.



A Phase II Environmental Site Assessment (ESA) was completed by Paterson for the subject site. All soil samples were found to be in compliance with MECP Table 2 Residential standards. All groundwater samples were found to be in compliance with MECP Table 2 Potable standards.

There are several surface water features located within the subject site and within 500 m of the subject site. These include unnamed scratch ditches, drainage ditches and conveyance channels. Also, the Leitrim Wetlands lies west of the subject site.

As per the Environmental Impact Study (EIS) prepared by Arcadis, select drainage features located within the subject site are expected to be infilled/removed as part of the proposed development. However, removal of these features is not anticipated to impact the overall hydrogeologic or hydrologic function of the site, as the stormwater management infrastructure will replicate pre-development discharge conditions by directing flows from the western catchment area toward the wetland. Furthermore, the stormwater management infrastructure will direct flows to the adjacent wetland by the means of conventional stormwater management measures, thus increasing hydration to this natural feature. Additionally, it is anticipated that water directed towards the wetland will infiltrate the subsoils offsite, helping to maintain the overall hydrogeologic function of the area, despite the localized infiltration deficit within the subject site itself. Therefore, given that the EIS did not identify impacts to features outside the development area and the stormwater management measures discussed above, impacts to the surrounding natural environment as a result of the proposed development are anticipated to be negligible.

6.4 Adjacent PTTW/EASRs/ECAs

A search of the MECP Permit to Take Water (PTTW) database provided one (1) PTTW within a 500 m radius of the subject site.

PTTW 2014-BAQMK2 is registered to 4840 Bank St. Ltd. and is located within the residential development currently under construction to the north of the subject site. The above noted permit contains 2 sources (Services Trenches and Miscellaneous Ponded Areas) with a maximum taking of 5,200,000 L/day.

A search of the MECP Environmental Activity and Sector Registry (EASR) database provided one (1) active EASR within a 500 m radius of the subject site.

EASR R-009-7232525387 is registered to RON EASTERN CONSTRUCTION LTD. and is located within the residential development currently under construction



to the north of the subject site. The above noted permit contains 2 sources (Building Excavation and Servicing Excavation) with a maximum taking of 400,000 L/day, per source.

Based on available mapping (GeoOttawa), it is expected that the servicing has been completed for the adjacent residential development at 4840 Bank Street. Therefore, it is unlikely that dewatering between the two sites would be taking place concurrently. Cumulative impacts related to anticipated dewatering activities for the proposed development from adjacent water taking permits are not anticipated.

With respect to Environmental Compliance Approvals (ECAs), given the nature of the development in the area (residential and commercial), there are several ECAs that exist for various purposes in the areas bordering the site. Eight (8) ECAs were found to be in relation to existing stormwater management systems in the area. Upon review of the aforementioned ECAs, the ECAs generally relate to installations of new sanitary and storm sewers for new residential developments along with stormwater management strategies on the respective sites. As the ECAs relate to municipal stormwater infrastructure, no concerns were found from a hydrogeological perspective in relation to the subject site.



7.0 ASSESSMENT AND RECOMMENDITIONS

7.1 Sources of Contamination

Based on the soil and groundwater samples collected at the subject site as part of the Phase II ESA investigation, all soil and groundwater samples were found to be in compliance with MECP Table 2 Residential standards and MECP Table 2 Potable standards, respectively.

Prior to and during site development, it is recommended that construction best management practices with respect to fuels and chemical handling, spill prevention, and erosion and sediment control be followed. This will minimize the potential for the introduction of contaminants to the soil, surface water, or groundwater at the subject site.

It is anticipated that the material on site will be disposed of as per Ontario Regulation 406/19 – On-site and Excess Soil Management.

With respect to stormwater runoff quality, it is recommended that best management practices with respect to operational standards be maintained for any stormwater management facilities constructed for the proposed development. It is also recommended that adherence to the City of Ottawa Salt Management Plan - Appendix A (October, 2011) included in Appendix 4 is enforced to ensure that chloride levels in stormwater runoff are minimized.

7.2 Surface Water Features

There are several surface water features located within the subject site and within 500 m of the subject site. These include unnamed scratch ditches, drainage ditches and conveyance channels. Also, the Leitrim Wetlands lies west of the subject site. However, as previously discussed, the stormwater management strategy will employ measures to increase hydration to adjacent surface water features. Therefore, by implementing the proposed stormwater management strategy and as per the Environmental Impact Study prepared by Arcadis, no significant impacts are expected from the proposed development on the overall hydrologic function of the area or sensitive features.

With respect to water discharge, water that is pumped from on site excavations must be managed in an appropriate manner. The contractor will be required to implement a water management program to dispose of the pumped water. If the discharge point for the pumped water is directed to overland drainage, it is



expected that a multi-barrier approach (such as hay bales, geosocks, silt fence, etc.) to a non-frozen, well vegetated area will be utilized in order to promote reinfiltration prior to reaching a watercourse. Furthermore, if the discharged water is to be directed to overland drainage within 30 m of a water body/watercourse, the turbidity of the water shall not exceed 8 NTU above background levels of the nearest water body. The contractor will be required to maintain appropriate BMPs with respect to sediment and erosion control to ensure negative effects to the surrounding environment are minimized.

7.3 Existing Wells

Any wells within the subject site must be decommissioned prior to construction in accordance with Ontario Regulation 903.

If construction activities are shown to cause negative impacts to the water supplies of existing well users, the contractor shall take action to make available a supply of water equivalent in quality and quantity of their typical takings, or shall compensate those affected for reasonable costs for doing so, or shall reduce water taking amounts to alleviate the negative impacts. The contractor shall provide temporary water supplies, to those affected, to meet their typical takings or compensate such persons for reasonable costs associated to do so until permanent restoration of the affected water supply or an equivalent source. As the potential to interfere with the water quality/quantity of existing well users in the area is negligible, a water well monitoring program is not recommended for the proposed development.

7.4 Water Taking Permitting Requirements

If water taking volumes are greater than 50,000 L/day, a MECP water taking Environmental Activity Sector Registry (EASR) or Permit to Take Water (PTTW) will be required. Depending on the nature of the proposed water takings, an additional hydrogeological investigation may be required.

7.5 Infiltration Potential

As previously discussed, surficial soils within the study area generally consisted of topsoil overlying a glacial till deposit. With regards to infiltration rates for the soils found on-site, site-specific testing varied from 24 to 82 mm/hr for the glacial till. The variations in the infiltration rates are dependent on the composition/compaction of the glacial till at a given location. A minimum safety correction factor of 2.5 will need to be applied to the estimated infiltration rates



noted above prior to consideration in the stormwater management design. Additional infiltration testing may be required once additional development details become available.

As noted above, the results of the water budget analyses completed at the subject site indicated that 21,992,400 L/year of infiltration and 14,661,600 L/year of surface runoff are occurring under pre-development conditions. Under post development conditions, it is expected that there will be a 72% infiltration deficit and a 391% increase in runoff. Therefore, it will likely be necessary to incorporate various stormwater management measures into the design of the development. It should be noted that Paterson's water budget assessment is based on mean water budget values for the soil types at the subject site that were calculated by modeling conducted by EC-ECS. The EC-ECS model is calibrated to historical climate data and does not account for climate change predictions. Therefore, based on the National Capital Commission and City of Ottawa climate change predictions, the stormwater management design team could consider potential seasonal changes (longer spring and shorter winter) and increases in temperature and precipitation when developing their stormwater management strategy.

The stormwater management strategy should target methods best suited to mitigate the impacts that may arise due to the post-development decrease in infiltration and increase in runoff at the subject site, while maintaining consideration of site constraints as defined by the City of Ottawa's Technical Bulletin IWSTB-2024-04 and the MECP's Consolidated Linear Infrastructure Environmental Compliance Approval documentation. Based on the site constraints observed at the subject site (i.e., shallow water table), infiltration based Low Impact Development (LID) measures are not recommended at the time of report preparation. Therefore, it is understood that the stormwater management strategy will include conventional stormwater management measures (i.e., end of pipe quality and quantity control) as well as the implementation of Best Management Practices to mitigate impacts related to the post-development water balance. This will be accomplished by directing flows to the adjacent wetland, thereby increasing hydration and maintaining natural function of surrounding features. Despite the post-development infiltration deficit, the majority of runoff will continue to be directed to areas within the same subwatershed, where infiltration will occur offsite, thereby maintaining the natural function of the hydrologic and hydrogeologic systems.



8.0 CLOSURE

The client should be aware that any information pertaining to soils and all test hole logs are furnished as a matter of general information only, and test hole descriptions or logs are not to be interpreted as descriptive of conditions at locations other than those of the test holes.

A hydrogeological review of this nature is a limited sampling of a site. The recommendations are based on information gathered at the specific test locations and can only be extrapolated to an undefined limited area around the test locations. Should any conditions at the site be encountered which differ from those at the test locations, we request notification immediately in order to permit reassessment of our recommendations.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Regional Group or their agent(s) is not authorized without review by Paterson Group for the applicability of our recommendations to the altered use of the report.

PRACTISING MEMBER

Paterson Group Inc.

Zavian Buchanan, E.I.T.

Oliver Blume, P.Geo.



9.0 REFERENCES

Government of Ontario. Provincial Policy Statement 2020 Under the Planning Act.

Engineering Technologies Canada Ltd, "ETC Pask (Constant Head Well) Permeameter – User Guide", dated March 2016.

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"Karst of Southern Ontario and Manitoulin Island (GRS005)" Prepared by the Ontario Geological Survey, 2008

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"Physiography of Southern Ontario (MRD228)", Prepared by the Ontario Geological Survey, 2007

Chapman, L.J., and Putnam, D. F. "The Physiography of Southern Ontario, Third Edition". Ontario Ministry of Natural Resources, 1984.

Freeze, R.A., and Cherry, J.A. "Groundwater". Prentice-Hall, Inc., 1979.



FIGURES

DRAWING PH5087-1 - SITE PLAN

DRAWING PH5087-2 - MECP WATER WELL LOCATION PLAN

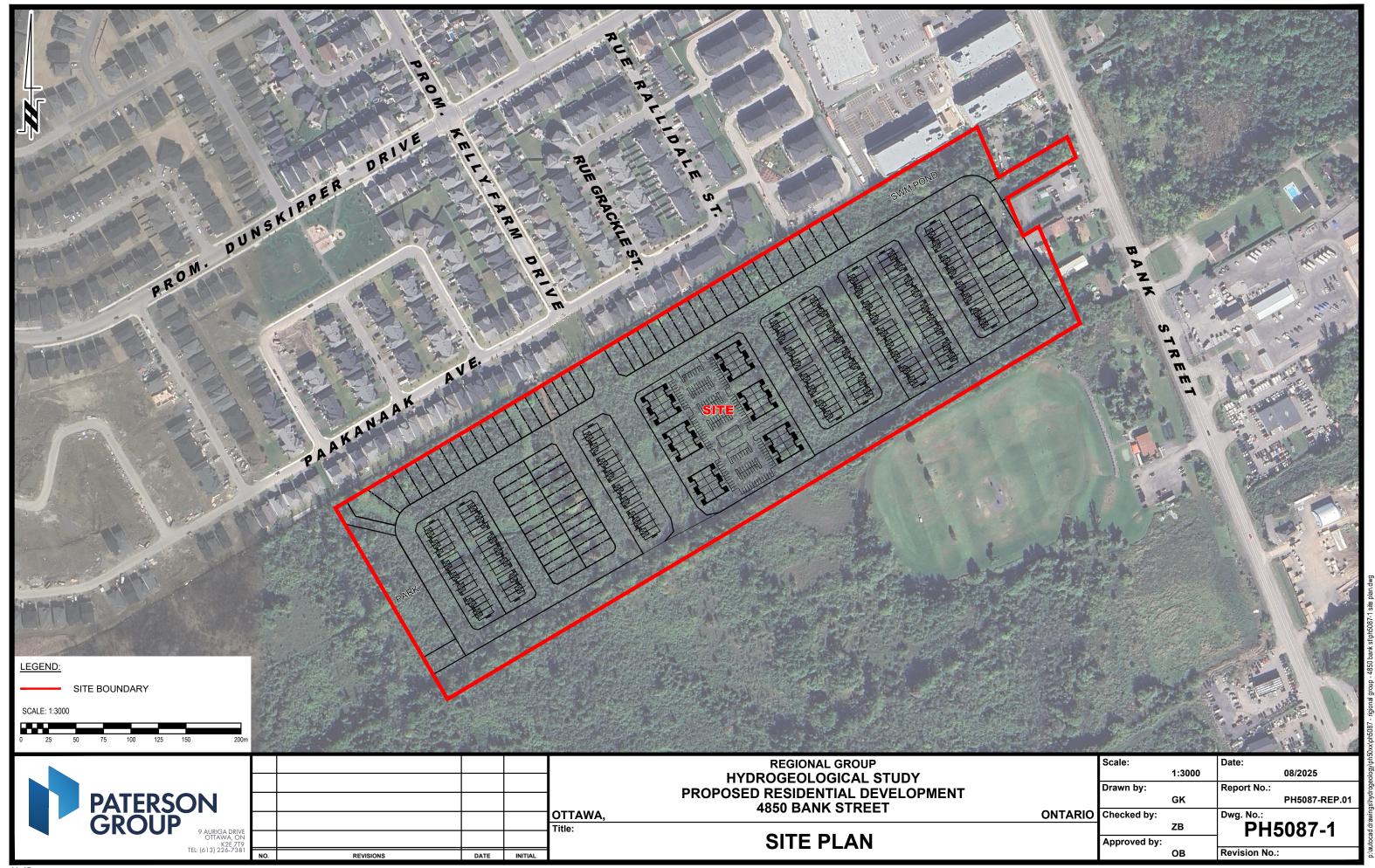
DRAWING PH5087-3 - SURFICIAL GEOLOGY PLAN

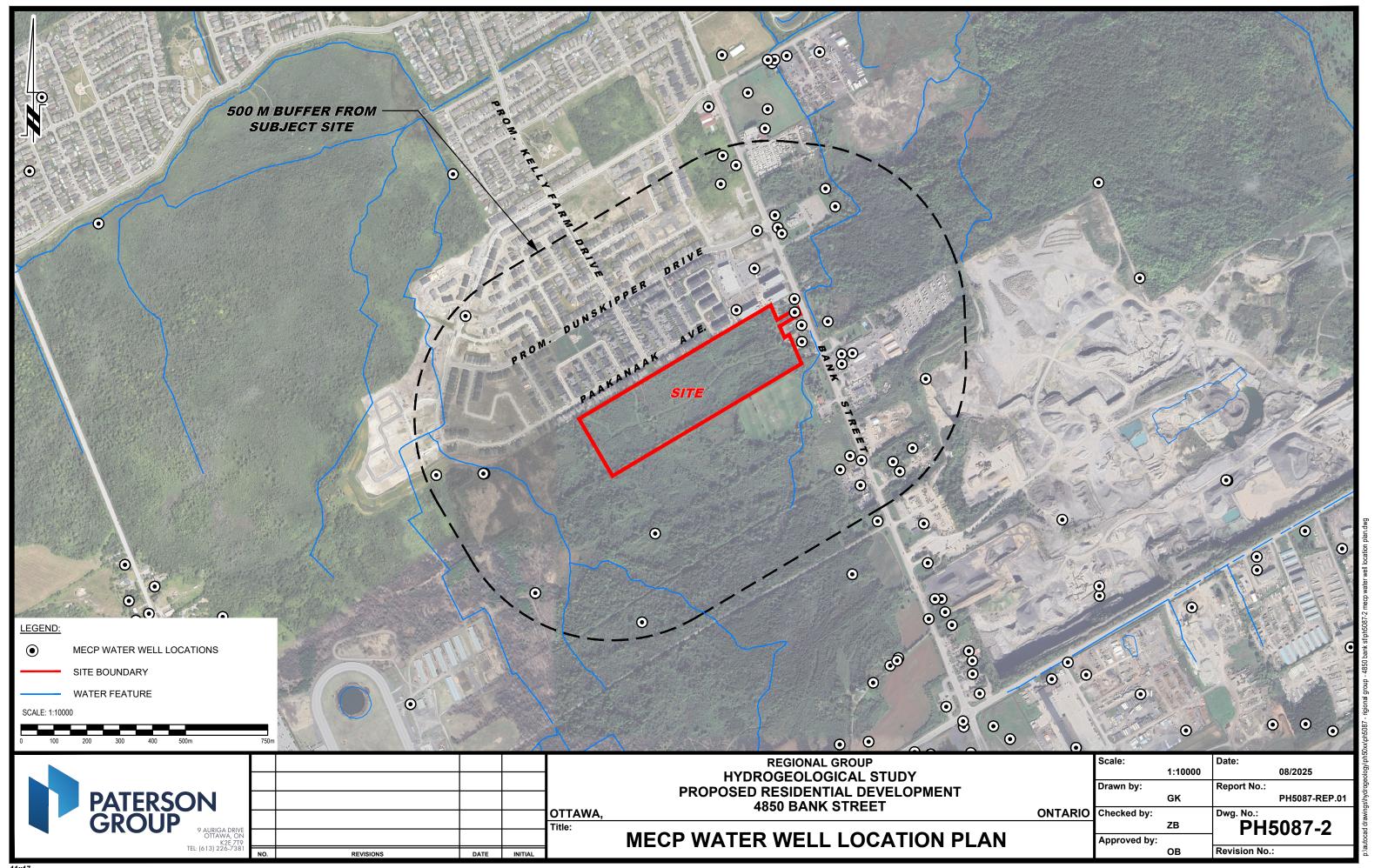
DRAWING PH5087-4 - BEDROCK GEOLOGY PLAN

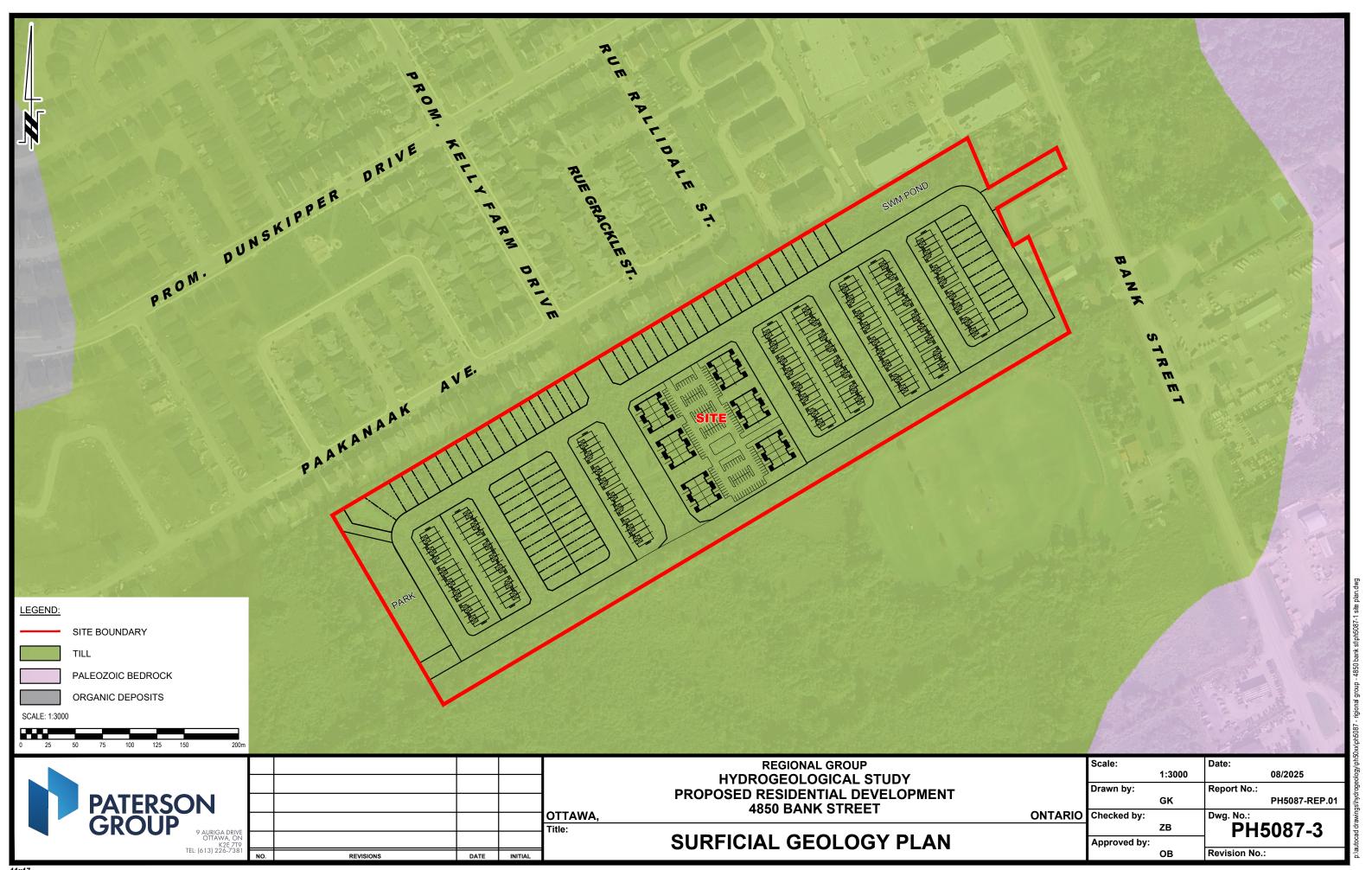
DRAWING PH5087-5 - GROUNDWATER CONTOUR PLAN

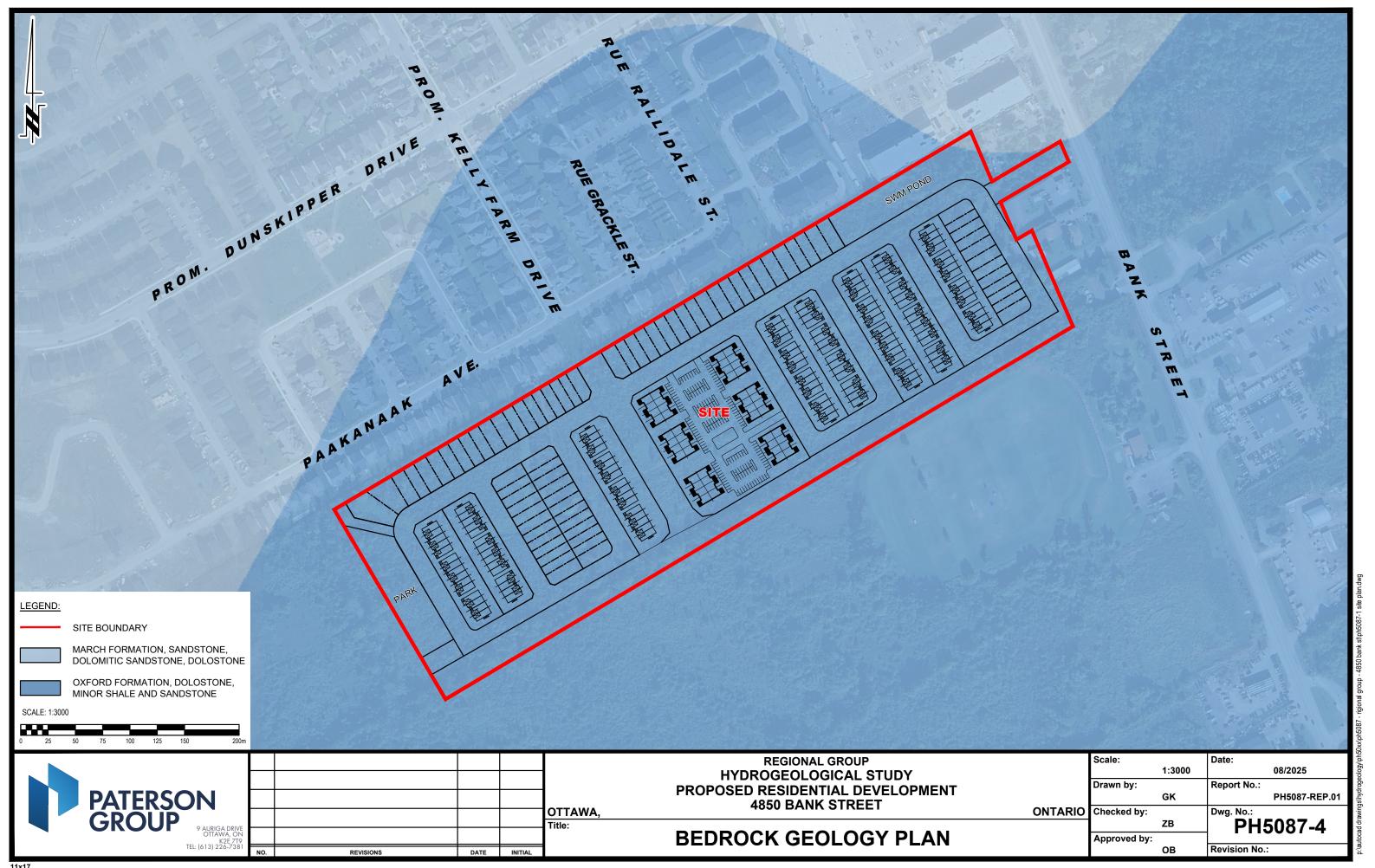
DRAWING PH5087-6 - PRE-DEVELOPMENT TERRAIN COMPOSITION PLAN

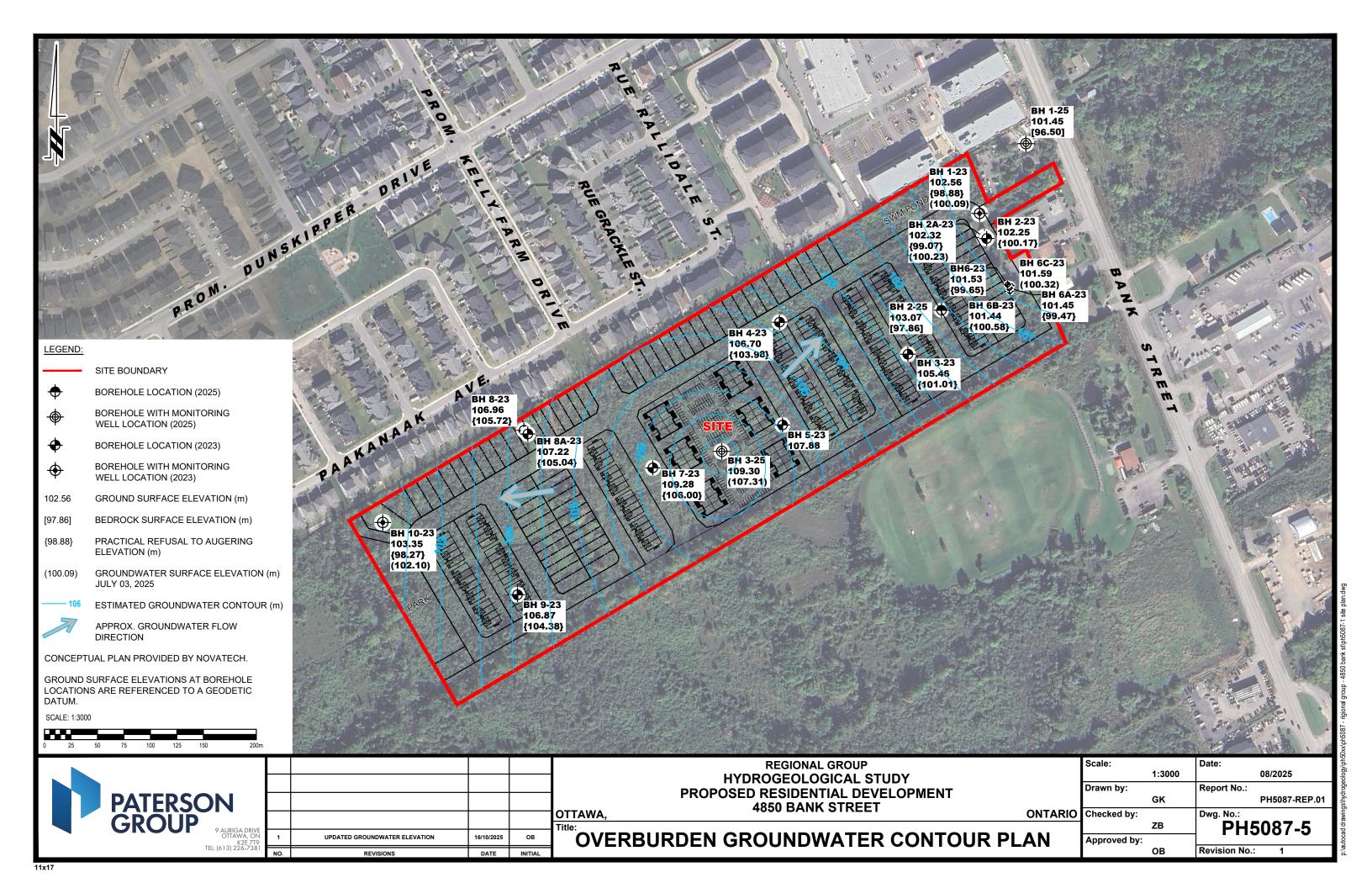
DRAWING PH5087-7 - POST-DEVELOPMENT TERRAIN COMPOSITION PLAN



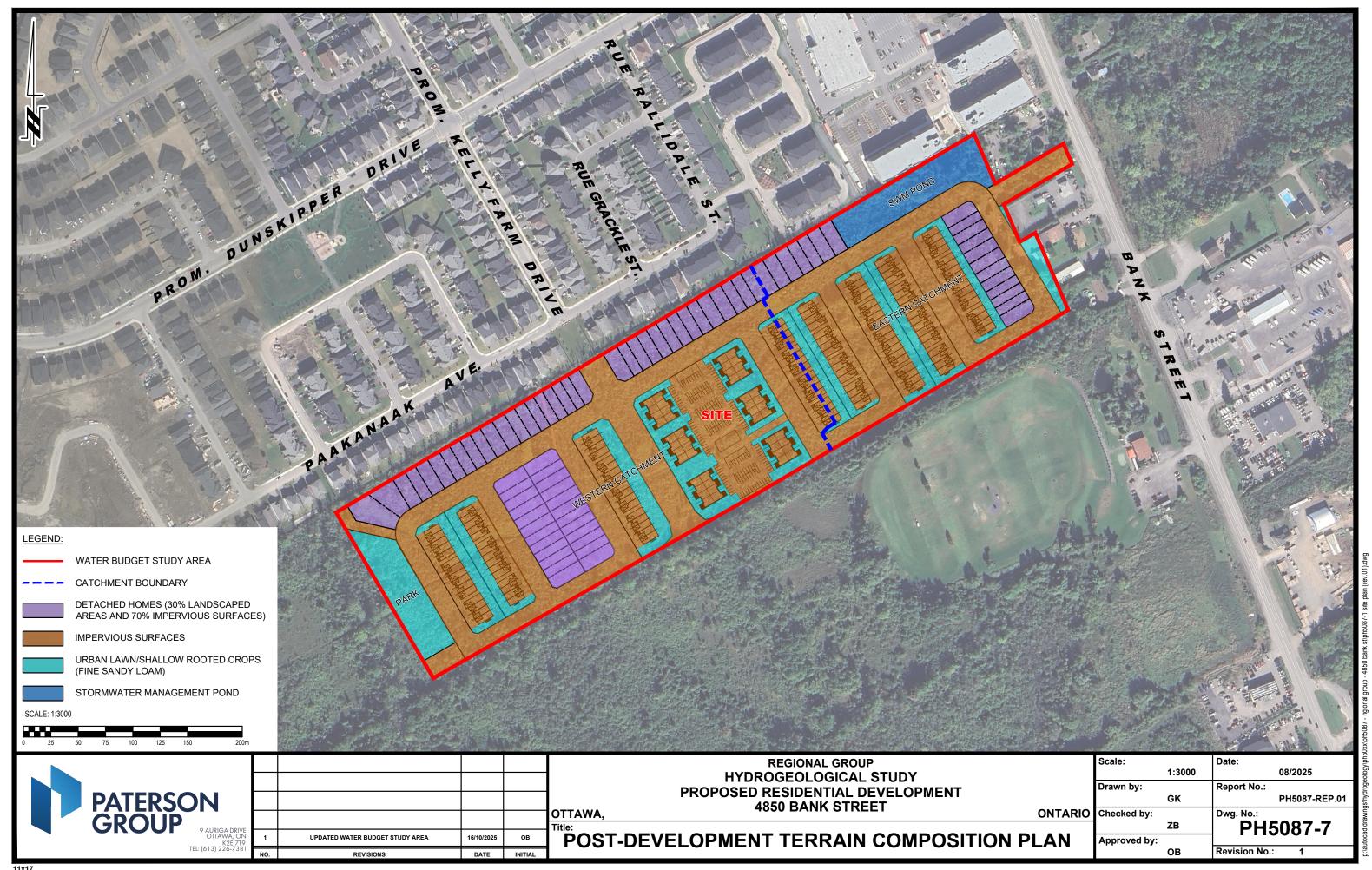














APPENDIX 1

SOIL PROFILE AND TEST DATA SHEETS

DRAWING PG6912-1 - TEST HOLE LOCATION PLAN



FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376393.34 **NORTHING:** 5019077.45 **ELEVATION:** 101.45

PROJECT: Proposed Residential Development

ADVANCED BY: CME-55 Low Clearance Drill

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REMARKS: DATE: June 18, 2025 HOLE NO.: BH 1-25

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DISCLAIMER: THE DATA PRESENTED IN THIS SHEET IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHOM IT WAS PRODUCED. THIS SHEET SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376313.61 **NORTHING:** 5018920.17 **ELEVATION:** 103.07

PROJECT: Proposed Residential Development

ADVANCED BY: CME-55 Low Clearance Drill

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REMARKS: DATE: June 18, 2025 HOLE NO.: BH 2-25

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1.98m [101.09m	1] \[\times \qq \qq \qq \qq \qq \qq \qq \qq \qq \q	_ =			50/0.05							
GLACIAL TILL: Very dense, grey silty fine sand with		2-] __									101-
gravel, cobbles and boulders	A A A A	-	SS 4	76	32-50-/-/			<u>.</u>				
	A A A A		1		50/0.05							
	A A A A	3-	SS 5		50-/-/-/			<u>.</u>				100-
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	-			50/0.03							100
	A A A A	-	RC 1	89				<u>:</u>				
	A A A A											
	\times \t	4-										99-
	A A A A	-										
	~ ~ ~ ~ ~		RC 2	100	DOD 57		:			· · · · · · · · · · · · · · · · · · ·		
	A A A A	5-		100	RQD 57							98-
BEDROCK: Excellent quality bedrock		-										90
BEDROCK. Excellent quality bedrock		-						<u> </u>				
		-					:					
		6-	, m	,								97 -
		-	RC 3	100	RQD 100							
		-										
7.06m [96.01m		7-										96 -
End of Borehole		-					:					90
		-	1					<u> </u>				
		-	1				:			· · · · · · · · · · · · · · · · · · ·		
		8-	1									95-
		-					:					
		-										
		9-	1					<u>.</u>				0.4
			1									94 -
		:	1									
		40	1				:					
		10 -					<u> </u>	: : : : : : : : : : : : : : : : : : :	<u>: : : : : : : : : : : : : : : : : : : </u>			

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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **ELEVATION:** 109.30 **EASTING: 376106.17 NORTHING:** 5018786.90

PROJECT: Proposed Residential Development

ADVANCED BY: CME-55 Low Clearance Drill

P:/AutoCAD Drawings/Test Hole Data Files/PE69xx/PG6912/data.sqlite_2025-07-10, 12:08 Paterson_Template_AE

REMARKS:					DATE: Ju	ıne 20	, 202	25			НО	LE NO.	BH 3-25		
				S	SAMPLE				[CPT (50mm	BLOWS/0	NE)	_	
SAMPLE DESCRIPTION	STRATA PLOT	DЕРТН (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	R CONTENT (%)	Δ.	REI UN	IDRA 20	LDED SINED S	SHEAI 0	R STREN	80 GTH (kPa) GTH (kPa) 80	MONITORING WELL CONSTRUCTION	ELEVATION (m)
GROUND SURFACE	STRA	DEPT	TYPE	REC	N OR	WATER (%)		PL (%	%) 20	WATE	R CO	NTENT (%	6) LL (%)	MONI	ELEV
TOPSOIL: with organics 0.20m [109.10m]	/ 0000								20	- 4	0	- 00			
GLACIAL TILL: Very dense, brown silty fine sand,	$\begin{array}{cccccccccccccccccccccccccccccccccccc$]	¥ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\						<u>.</u>						109-
some gravel, cobbles and boulders	A A A A	_	\/ a												
	~ ~ ~ ~ ~]	X SS	90	12-15-51-50 66/0.05										108-
	~ ~ ~ ~ ~	- - -	SS 3	66	19-50-/-/				<u>.</u>						100
	^ ^ ^ V	2	Δ Ø	00	50/0.08										-
	A A A A	2 -	4	00	40.50.77										107-
	A A A A A A A A A A A A A A A A A A A	1	SS 4	90	49-50-/-/ 50/0.03				<u>.</u>						
	\(\times \times \	3—	5					: : :							
	\(\times \delta \delta \delta \delta \delta \delta \delta		SS	109	50-/-/-/ 50/0.08							:			106-
	A A A A	=						:	<u>.</u>			<u>:</u>			
	A A A A	4	\times 88	66					ļ						:
	~ ~ ~ ~ ~	=			50/0.08										105
- Grey by 4.57 m depth	~ ~ ~ ~ ~	-	SS 7	60	42-50-/-/			1	 !			· · · · · · · · · · · · · · · · · · ·		4.6	3m]
	^ ~ ~ ~ V	5 -			50/0.05			ļ.,,,,,	į.,.,						
- Cobbles and boulders decreasing with depth	A A A A	=													104
	A A A A	=	88.8	8	15-24-18-17				 !						
- Silty fine to medium sand by 6.10 m depth	A A A A A A A A A A A A A A A A A	6			42				ļ.,						
only mo to modium dana by one in dopar	\(\times \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta \delta	Í	88.9	92	10-16-21-20									6.1	103
6.71m [102.59m]	A A A A]			37										
End of Borehole		7-							<u>.</u>						
		=													102
		=													:
		8=							: :						:
		=							<u>.</u>						101-
		=						:	:						:
		9							ļ !			· · · · · · · · · · · · · · · · · · ·			
		=													100
		10													:
		10 -						:	:	<u>: :</u>	: :	: : : : : : : : : : : : : : : : : : : :	<u>: : : : : : : : : : : : : : : : : : : </u>		

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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING**: 376203.99 **NORTHING**: 5019601.65 **ELEVATION**: 96.09

PROJECT: Proposed Residential Development **ADVANCED BY:** CME-55 Low Clearance Drill

REMARKS: DATE: July 7, 2025 HOLE NO.: BH 4-25

REMARKS:					DATE: Ju	uly 7, 2	2025	5		HO	LE NO. :	BH 4-	25		
				s	AMPLE			= F	PEN. RES	IST. (I	BLOWS/0.3 DIA. CONE	m)			
SAMPLE DESCRIPTION GROUND SURFACE	STRATA PLOT	DEРТН (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)			40 ULDED S AINED S 40 WATER	HEAR HEAR CON	60 R STRENG R STRENG 60 NTENT (%)	80 FH (kPa) FH (kPa) 80 LL (%)		PIEZOMETER CONSTRUCTION	ELEVATION (m)
FILL: Brown silty sand with gravel and crushed stone 0.69m [95.40m]			AU X	_	_			20	40		60	80			96-
FILL: Compact, brown silty sand with gravel, trace clay 1.52m [94.57m]		1-	SS 2	50	2-8-7-5 15										95
TOPSOIL: with organics, trace sand 1.83m[94.26m] GLACIAL TILL: Very dense, brown silty fine sand, occasional cobbles	V V V V	2	SS 3	58	5-5-5-11 10										94
2.97m [93.12m]	A A A A A A A A A A A A A A A A A A A A	3	SS 4	83	14-25-24-30 49										
GLACIAL TILL: Very dense, grey silty sand with gravel, cobbles and boulders 3.23m [92.86m] End of Borehole	v v v		SS 5	73	15-50-/-/ 50/0.13										93-
Practical refusal to augering at 3.23 m depth on inferred boulders		4-													92
		5-													91—
		6													90
		7—													
		' =													89— - - -
		8-													88-
		9—													87
		10													- - - - - -

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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376242.76 **NORTHING:** 5019509.68 **ELEVATION:** 96.49

PROJECT: Proposed Residential Development

ADVANCED BY: CME-55 Low Clearance Drill

P:/AutoCAD Drawings/Test Hole Data Files/PE69xx/PG6912/data.sqlite_2025-07-10, 12:08 Paterson_Template_AE

REMARKS: DATE: July 7, 2025 HOLE NO.: BH 5-25

REMARKS:					DATE: Ju	ıly 7, 2	025			НО	LE NO. :	BH	1 5-25		
				s	AMPLE			= F			BLOWS/0. DIA. CON				
SAMPLE DESCRIPTION	STRATA PLOT	1 (m)	TYPE AND NO.	RECOVERY (%)	RaD	WATER CONTENT (%)	Δ		ULDED	0 SHEAI SHEAF	60 R STRENG 60	80 STH (k	Pa)	PIEZOMETER CONSTRUCTION	ELEVATION (m)
GROUND SURFACE	STRAI	DEPTH (m)	TYPE	RECO	N OR RQD	WATE		PL (%)	WATE		NTENT (%		L _. (%)	PIEZO CONS	ELEVA
FILL: Compact, brown silty sand with gravel and crushed stone			A F			-		20	4			O	J		96—
TOPSOIL: with organics with sand, trace grass 1.37m[95.42m]		1-	SS 2	33	3-8-4-3 12	,									- - - -
GLACIAL TILL: Compact to dense, brown silty fine sand with gravel, occasional cobbles, trace clay	A A A A A A A A A A A A A A A A A A A	2	SS 3	83	2-10-14-9 24										95 — - - -
	A A A A A A A A A A A A A A A A A A A		SS 4	92	19-22-19-22 41										94-
3.15m [93.34m] End of Borehole	<u> </u>	3-1	SS 5	0	50-/-/-/ 50/0.05										93
Practical refusal to augering at 3.15 m depth on inferred boulders		4-				,									- - - - -
		5													92-
															91-
		6-													- - - - -
		7-													90-
															89-
		8-													- - - - -
		9—													88 — - - -
															87
		10									:		* * *		

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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376283.28 **NORTHING:** 5019413.06 **ELEVATION:** 97.68

PROJECT: Proposed Residential Development **ADVANCED BY:** CME-55 Low Clearance Drill

REMARKS: DATE: July 7, 2025 HOLE NO.: BH 6-25

REMARKS:					DATE: Ju	uly 7, 2	2025			HOLE NO. :	BH 6-25		
				5	SAMPLE			= ;		SIST. (BLOWS/0. 50mm DIA. CON			
						5		20		10 60	E) 80	_	
SAMPLE DESCRIPTION	6		Š	8		R CONTENT (%)	Δ	REMO		SHEAR STRENG		ୗ ୷ହା	Œ
SAMIFLE DESCRIPTION	STRATA PLOT	Ē	TYPE AND NO.	RECOVERY (%)	a	S S				SHEAR STRENG			ELEVATION (m)
	AT	DEPTH (m)	Щ.		N OR RQD	WATER (%		20 PL (%)		10 60 ER CONTENT (%	80) LL (%)	SOM	VAT
GROUND SURFACE	STR	当	≟	RE	0 2	\ ₩		20		10 60	80	PIEZOMETER CONSTRUCTION	出
ASPHALT 0.05m [97.63m] /			X -										=
FILL: Granular, crushed stone, some sand		-	₹					ii.					=
\\\0.61m[97.07m]/		-											97 _
FILL: Compact, brown silty sand with gravel		1-	SS 2	42	5-7-3-6								=
TOPSOIL: with organics, sand, trace clay and gravel		-	N w	"-	10				:				Ξ
1.52m [96.16m]	~ ~ ~ ~	=						<u>:</u>					-
GLACIAL TILL: Dense to very dense, brown silty fine	A A A A A A A A	-	SS 3	67	7-16-14-13								96 —
sand with gravel, cobbles and boulders	$ \begin{picture}(20,0) \put(0,0){\line(1,0){10}} \put(0,$	2-	4		30								=
	$ \begin{array}{c c} \triangle & \triangle & \triangle & \triangle \\ \hline \triangle & \triangle & \triangle & \triangle \\ \hline \end{array} $	-	SS 2	85									Ξ
	A A A A	-			50/0.13				i				95 _
		3-						<u>.</u>		: : : : : : : : : : : : : : : : : : :			=
	A A A A	-	35	02	30-48-51-42								Ξ
3.51m [94.17m]	\(\times \)	-	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	92	99								=
GLACIAL TILL: Very dense, grey silty fine sand with	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	-							i				94 —
gravel, cobbles and boulders		4-	SS 6	79	20-26-21-50								=
4.50m [93.18m]		-	/		47/0.13								=
End of Borehole		-											93 —
		5-											=
Practical refusal to augering at 4.50 m depth		-							:				Ξ
		-											
		-											92 —
		6-											Ξ
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		-											91-
		7-											=
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		-	1					<u>.</u>		<u> </u>			=
		-											90 —
		8-											=
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		9-	1										=
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		40											88 –
		10 -						<u>: : : : : : : : : : : : : : : : : : : </u>	:	1 1 1			

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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376306.21 **NORTHING:** 5019357.57 **ELEVATION:** 98.42

PROJECT: Proposed Residential Development

ADVANCED BY: CME-55 Low Clearance Drill

REMARKS: DATE: July 7, 2025 HOLE NO.: BH 7-25

REMARKS:						DATE: Ju	ıly 7, 2	025			НО	LE NO. :	BH 7-	25		
					S	AMPLE						BLOWS/0.				
					_		Ä.		20		40	60	80		z	_
SAMPLE DESCRIPTION	Lo Lo		2	2	۲ (%)		ONTE	△				R STRENG R STRENG			띪	E N
	¥	E			VER	RQD	% Ω (%		20		40	60	80		TRU	ATIO
GROUND SURFACE	STRATA PLOT	DEPTH (m)	TVDE AND NO		RECOVERY (%)	N OR RQD	WATER CONTENT (%)		PL (%)		ER CO 40	NTENT (%) 60			PIEZOMETER CONSTRUCTION	ELEVATION (m)
ASPHALT 0.05m [98.37m]		-				_				•	40	60	80 '			
FILL: Granular, crushed stone and gravel, some		-	X	AU 1					<u>.</u>							98
and 0.61m[97.81m],		-														
FILL: Dense, brown silty sand with gravel, trace		1-	X	SS 2	42	16-29-18-10										
opsoil and clay		-	\square	0,		47										97
1.83m [96.59m]		-	M	33	400	40045										
GLACIAL TILL: Compact to very dense, brown silty	0 0 0 0 0 0 0 0	2-	\mathbb{N}	SS	100	4-8-6-15 14			ļ <u>i</u>							
ine sand with gravel, cobbles and boulders		=														96
		- -	X	SS 4	67	13-30-28-42										30
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	3-		2		58			ļļ							
	A A A A	-	X	SS	54	14-50-/-/ 50/0.13										
	A A A A A A A A A A A A A A A A A A A	=				30/0.13			<u></u>							95
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	4-		9 SS	77	26-52-50-/			i. i.							
4.14m [94.28m] End of Borehole	7 7 7 7	-		တ		102/0.03										
		=							.ii				:			94
ractical refusal to augering at 4.14 m depth		_ =														
		5-														
		-														93
		-														
		6-														
		-							<u>.</u>							92
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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376361.10 **NORTHING:** 5019231.96 **ELEVATION:** 100.48

PROJECT: Proposed Residential Development **ADVANCED BY:** CME-55 Low Clearance Drill

REMARKS: DATE: July 7, 2025 HOLE NO.: BH 8-25

REMARKS:					DATE: J	uly /, 2	2025)		110	LL NO	ВП 0-23		
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						_		20		50mm 0	DIA. CONE	80 80		
			<u>o</u>	8		띹	Δ				R STRENG		_ 8	ε
SAMPLE DESCRIPTION	STRATA PLOT	_	TYPE AND NO.	RECOVERY (%)		WATER CONTENT (%)	_				R STRENGT		PIEZOMETER CONSTRUCTION	ELEVATION (m)
	≰	Ë	¥		<u>8</u>	జ క		20	4	10	60	80	N K	ĕ
	<u>R</u>	DEPTH (m)	YE		N OR RQD	ATE		PL (%)	WATE	ER CO	NTENT (%)	LL (%)	ON EZ	Ē
GROUND SURFACE	S S		<u> </u>	~	z	>		20	4	10	60	80	<u> </u>	ш
ASPHALT 0.05m [100.43m]		- =	¥ \						:					
FILL: Granular, crushed stone, some sand		=	XX					<u> </u>						100
0.61m[99.87m]/		=												
FILL: Dense, brown silty sand, trace gravel		1_	SS 2	33	8-26-21-17									
1.45m [99.03m]		=			47									
GLACIAL TILL: Dense, brown silty fine sand with	V V V V	=								} <u>{</u>				99
gravel, cobbles and boulders	$ \begin{picture}(20,0) \put(0,0){\line(1,0){10}} \put(0,$	_	SS 3	75	8-12-24-29									
graver, copples and boulders	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	2			36					ļ., <u>i</u>				
2.34m [98.14m]	<u> </u>	_	SS 4	0	50-/-/-/									
End of Borehole		=	0,		50/0.03					:				98
		=												
Practical refusal to augering at 2.34 m depth		3_								1000				
		=												
		-												97
		=												
		4 –								1000				
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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING: 376401.58 ELEVATION**: 101.22 **NORTHING:** 5019136.66

PROJECT: Proposed Residential Development ADVANCED BY: CME-55 Low Clearance Drill

HOLE NO.: BH 9-25 **REMARKS: DATE:** July 7, 2025

NEMATO.				s	AMPLE	,							LOWS/		1)		
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)		WATER CONTENT (%)	Δ Δ	RE UI PL (20 MO NDR 20 %)	ULDE AINE	40 ED SH D SH 40 TER	IEAR EAR	STREN 60 STREN 60 TENT (9	IGTH GTH	(kPa) 80 LL (%	PIEZOMETER CONSTRUCTION	ELEVATION (m)
GROUND SURFACE ASPHALT 0.05m [4101.47m]/	<i>w</i>			12	2	>			20	:	40	-	60	:	80	F 0	
FILL: Granular, crushed stone, some sand FILL: Compact, brown silty sand with gravel, trace clay and topsoil 1.45m[99.77m]		1-	SS AU1	33	8-5-4-9 9												101-
GLACIAL TILL: Dense, brown silty fine sand with gravel, cobbles and boulders 2.11m [99.11m]	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2	SS 3	66	12-16-24-50 40/0.05												
End of Borehole		-															99-
Practical refusal to augering at 2.11 m depth		3-															98-
		4-															97-
		5-															96-
		6															95-
		7-															94-
		8-															93-
		9-															92-
		10 -															

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FILE NO.:

Geotechnical Investigation 4850 Bank Street, Ottawa, ON

PG6912

COORD. SYS.: MTM ZONE 9 **EASTING:** 376435.21 **NORTHING:** 5019057.96 **ELEVATION:** 101.23

PROJECT: Proposed Residential Development **ADVANCED BY:** CME-55 Low Clearance Drill

REMARKS: DATE: July 7, 2025 HOLE NO.: BH10-25

REWARNS.					DAIE. J	uly 1, 2	023							
					SAMPLE			= F			BLOWS/0.3 DIA. CONE	:)		
						WATER CONTENT (%)		20	40		60	80	_	
SAMPLE DESCRIPTION	6		9	8		뿔	Δ	REMO	ULDED S	SHEAR	STRENG	TH (kPa)	유	E
5: <u></u>	STRATA PLOT	Œ	TYPE AND NO.	RECOVERY (%)	გ	୍ପ ଚ	•	UNDR	RAINED S	HEAR	STRENGT			ELEVATION (m)
	AT	王	Щ	8	N N	HH (20 PL (%)	40 WATE	P CON	60 ITENT (%)	80 LL (%)	ZON	¥
GROUND SURFACE	STR	DEPTH (m)	₹	H	N OR RQD	¥		20	40		60 60	80	PIEZOMETER CONSTRUCTION	
FILL: Compact, brown silty sand with gravel			$\overline{\mathcal{M}}$					20	- 40	:	00	00		
FILE. Compact, brown sitty saild with graver		1	X \{ \{ \}											101-
0.61m [100.62m]		‡	젚 ~	•										
FILL: Compact, brown silty sand with asphalt, some		ŧ												
gravel		1-	X SS 2	33	8-7-25-14									
1.45m [99.78m]					32									100-
FILL: Loose to compact, brown silty sand with		4												
gravel, trace clay and topsoil		3	88.3	42	4-4-4-6					-				
2.21m [99.02m]		2-	/\\ ⁰		8					******				
Compact, grey, medium SAND , some gravel		1												99-
\ 2.44m [98.79m]	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	=	XS 4	58	6-9-26-32									
GLACIAL TILL: Dense, grey silty fine sand with	A A A 4	1)	35									
gravel 2.95m [98.28m] /	//	3								*****				
End of Borehole		=												98-
End of Borenoic		=												
Proctical refusal to augaring at 2.05 m depth		3												
Practical refusal to augering at 2.95 m depth		4												
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PAGE: 1/1

P:/AutoCAD Drawings/Test Hole Data Files/PE69xx/PG6912/data.sqlite_2025-07-10, 12:08 Paterson_Template_AE



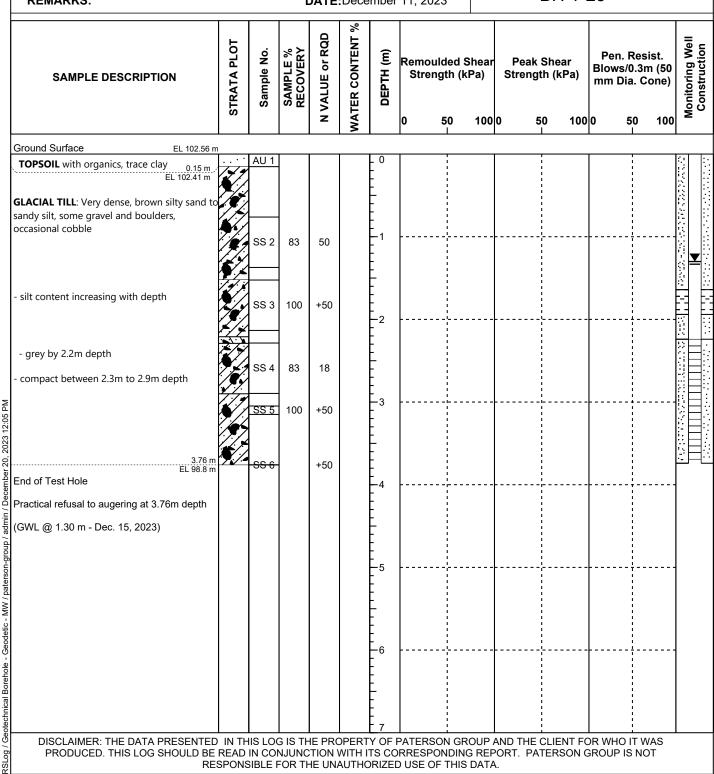
GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 376352.245 NORTHING: 5019009.799 **ELEVATION: 102.56 PROJECT: Proposed Development**

FILE NO. **PG6912** BORINGS BY: CME 55 Track-Mounted Mechanical Auger

HOLE NO. BH 1-23 **REMARKS:** DATE: December 11, 2023





GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 376358.876 NORTHING: 5018986.132 **ELEVATION: 102.25 PROJECT: Proposed Development** FILE NO. **PG6912** BORINGS BY: CME 55 Track-Mounted Mechanical Auger HOLE NO. BH 2-23 **REMARKS:** DATE: December 11, 2023 N VALUE or RQD **NATER CONTENT** Monitoring Well Construction STRATA PLOT SAMPLE % RECOVERY Sample No. $\widehat{\mathbf{E}}$ Pen. Resist. Remoulded Shear **Peak Shear** Blows/0.3m (50 DEPTH Strength (kPa) Strength (kPa) **SAMPLE DESCRIPTION** mm Dia. Cone) 1000 1000 50 100 50 50 Ground Surface EL 102.25 m **GLACIAL TILL**: Brown clayey silt with gravel and occasional cobble, trace organics GLACIAL TILL: Dense, brown silty sand to sandy silt with gravel and boulders, occasiona cobble SS 2 63 48 - very dense by 1.2m depth SS₃ 76 +50 -2 2.08 m EL 100.17 m End of Test Hole Practical refusal to augering at 2.08m depth RSLog / Geotechnical Borehole - Geodetic - MW / paterson-group / admin / December 20, 2023 12:05 PM



GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 376356.813 NORTHING: 5018986.889 **ELEVATION: 102.32 PROJECT: Proposed Development** FILE NO. **PG6912** BORINGS BY: CME 55 Track-Mounted Mechanical Auger HOLE NO. BH 2A-23 **REMARKS:** DATE: December 11, 2023 N VALUE or RQD **NATER CONTENT** Monitoring Well Construction STRATA PLOT SAMPLE % RECOVERY Sample No. $\widehat{\mathbf{E}}$ Pen. Resist. Remoulded Shear **Peak Shear** Blows/0.3m (50 DEPTH Strength (kPa) Strength (kPa) **SAMPLE DESCRIPTION** mm Dia. Cone) 1000 1000 50 100 50 50 Ground Surface EL 102.32 m Overburden Augered to 2.18 m depth -2 GLACIAL TILL: Very dense, grey silty sand to sandy silt with gravel and boulders, occasional SS₁ 86 +50 cobble -3 100 +50 - trace clay by 3.0m depth 3.25 m EL 99.07 m End of Test Hole RSLog / Geotechnical Borehole - Geodetic - MW / paterson-group / admin / December 20, Practical refusal to augering at 3.25m depth (GWL @ 0.80 m - Dec. 15, 2023)



GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 376284.113 NORTHING: 5018877.194 ELEVATION: 105.46 m PROJECT: **Proposed Development** FILE NO. PG6912

BORINGS BY: CME 55 Track-Mounted Mechanical Auger

SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH (m)		th (kPa	Peak Sh Strength (50		Blows mm D	Resist	t. (50 ne)	Piezometer
Ground Surface EL 105.46 r	n													
OPSOIL with organics 0.25 m	.W					- 0							}	8
0.25 m EL 105.21 m ELACIAL TILL: Compact, brown silty sand to andy silt, trace gravel		AU 1				- - - -						1		
		SS 2	42	12		-1 -1 -		- - - - - - -	 				}	\ \
no gravel by 1.5m depth		SS 3	83	19		- - - - - -2								
2.21 m EL 103.25 m ILACIAL TILL : Very dense, grey silty sand to	* />	SS 4	75	+50		-							<u> </u>	XXX -
andy silt with gravel and boulders, occasiona obble		SS 5	100	+50		- - -3 -		 	 			 		
trace clay by 3.8m depth		SS 6	78	+50		- - - - - - -		 						
4.45 m EL 101.01 m						- ' - - - -							1313 222-031	
Practical refusal to augering at 4.45m depth						-			1					
GWL @ 1.23 m - Dec. 15, 2023)						-5 - - - -		. 	 			- 		
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RSLog / Geotechnical Test Pit - Geodetic / paterson-group / admin / December 20, 2023 09:56 AM

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING: 376162.967** NORTHING: 5018907.818 **ELEVATION: 106.7 m PROJECT: Proposed Development** FILE NO. **PG6912** BORINGS BY: CME 55 Track-Mounted Mechanical Auger HOLE NO. BH 4-23 **REMARKS:** DATE: December 11, 2023 N VALUE or RQD **NATER CONTENT** Piezometer Construction STRATA PLOT SAMPLE % RECOVERY Sample No. $\widehat{\mathbf{E}}$ Pen. Resist. Remoulded Shear **Peak Shear** Blows/0.3m (50 DEPTH Strength (kPa) Strength (kPa) **SAMPLE DESCRIPTION** mm Dia. Cone) 1000 1000 100 50 50 50 Ground Surface EL 106.7 m **TOPSOIL** with organics 0.1 m EL 106.6 m GLACIAL TILL: Brown clayey silt, some gravel and boulders, occasional cobble 0.46 m EL 106.24 m SS 2 100 46 GLACIAL TILL: Dense to very dense, brown silty sand to sandy silt with gravel and boulders, occasional cobble SS 3 88 +50 -2 67 +50 2.72 m EL 103.98 m End of Test Hole Practical refusal to augering at 2.72m depth (Dried Borehole - Dec. 15, 2023) DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS

PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 376165.452 **NORTHING:** 5018810.785 ELEVATION: 107.88 m

PROJECT: **Proposed Development** FILE NO. PG6912

BORINGS BY: CME 55 Track-Mounted Mechanical Auger

HOLE NO. BH 5-23

SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH (m)	Remoul Stren			Stren	k She igth (I	Blows mm D	. Res s/0.3n Dia. C	n (50	Piezometer
Ground Surface EL 107.8	I 8 m			l							·	l	ı		
FOPSOIL with organics	3 m					- 0 -							-		X
EL 107.5	3 m // 🏏	AU 1				Ė							-		X <u>*</u>
GLACIAL TILL : Brown clayey silt with grav occasional cobble 0.61	177					[-		X
Occasional cobble 0.61 EL 107.27	m 1 /2/2					-					i		i		X
GLACIAL TILL: Dense to very dense, brown		SS 2	83	43		Ε'						 [$\langle X \rangle$
ilty sand to sandy silt with gravel and oulders, occasional cobble	9 / ₂					-							i		$\langle \rangle$
,		SS 3	100	+50		Ē					-		1		8
			100			<u> </u>							-		8
						-2 -		-				 [8
		SS 4	100	+50		-							-		\otimes
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grey by 3.0m depth		00.5				-3 - -						 			X
		SS 5	100	+50		-		1			-		-		X
						E					i		i		X
						- 4		1			-		-		==
		SS 6	100	+50		-						 [
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dense by 4.6m depth		SS 7	100	46		- - -5		į							
						Ė		į			į		į		
		SS 8	75	34		-							-		
5.9 EL 101.9	4 m					- - -6		į				 			<u>::</u> [
End of Test Hole						F ~									
GWL @ 0.40 m - Dec. 15, 2023)						Ė					į		į		
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						7		i					-		



BORINGS BY: CME 55 Track-Mounted Mechanical Auger

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING: 376381.101** NORTHING: 5018941.64 **ELEVATION: 101.53**

PROJECT: Proposed Development FILE NO. **PG6912**

HOLE NO. BH 6-23 **REMARKS:** DATE: December 13, 2023

N VALUE or RQD **WATER CONTENT** Monitoring Well Construction STRATA PLOT SAMPLE % RECOVERY Sample No. $\widehat{\mathbf{E}}$ Pen. Resist. Remoulded Shear **Peak Shear** Blows/0.3m (50 DEPTH Strength (kPa) Strength (kPa) **SAMPLE DESCRIPTION** mm Dia. Cone) 1000 1000 50 100 50 50 Ground Surface EL 101.53 m TOPSOIL with organics EL 101.28 m GLACIAL TILL: Brown clayey silt, trace gravel 0.61 m EL 100.92 m AU 1 **GLACIAL TILL**: Compact, brown silty sand to sandy silt with gravel and boulders, occasional SS₂ 75 19 cobble SS 3 89 +50 very dense by 1.5m depth 1.88 m EL 99.65 m -2 End of Test Hole Practical refusal to augering at 1.88m depth -3 RSLog / Geotechnical Borehole - Geodetic - MW / paterson-group / admin / December 20, 2023 12:05 PM



GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

	DATUM: Geodetic EASTING:	37638	0		NO	RTHI	NG : 5	018939.186		ELEVATIO	N: 101.45	
	PROJECT: Proposed Developr								FILE	NO. PG69	12	
	BORINGS BY: CME 55 Track-Mou REMARKS:	inted	Mech				mber	13, 2023	HOLE	NO. BH 6	\ -23	
	SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH	Remoulded S Strength (ki		Peak Shear Strength (kPa) 50 100	Pen. Resist. Blows/0.3m (50 mm Dia. Cone)	Monitoring Well Construction
	Ground Surface EL 101.45 m											
	Overburden						- 0	-		i		
	Augered to 1.98 m depth						- - - - - - - - 1 - - -					
	1.98 m EL 99.47 m End of Test Hole Practical refusal to augering at 1.98m depth						- - - - - - - - - -					
mber 20, 2023 12:05 PM							- -3 - - - - - - - -					
RSLog / Geotechnical Borehole - Geodetic - MW / paterson-group / admin / December 20, 2023 12:05 PM							- - - - - - - - - - - - -					
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eotechnical Borehole							- - - - - - - 7					
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GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

EASTING: 376380.758 **DATUM:** Geodetic NORTHING: 5018936.262 **ELEVATION: 101.44 PROJECT: Proposed Development** FILE NO. **PG6912** BORINGS BY: CME 55 Track-Mounted Mechanical Auger HOLE NO. BH 6B-23 **REMARKS:** DATE: December 13, 2023 N VALUE or RQD Monitoring Well Construction **WATER CONTENT** STRATA PLOT SAMPLE % RECOVERY Sample No. Ξ Pen. Resist. Remoulded Shear **Peak Shear** Blows/0.3m (50 DEPTH Strength (kPa) Strength (kPa) **SAMPLE DESCRIPTION** mm Dia. Cone) 1000 1000 50 100 50 50 Ground Surface EL 101.44 m Overburden Augered to 0.86 m depth 0.86 m EL 100.58 m End of Test Hole Practical refusal to augering at 0.86m depth -2

RSLog / Geotechnical Borehole - Geodetic - MW / paterson-group / admin / December 20, 2023 12:05 PM



GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic EASTING:	376378.	444	NO	RTHIN	IG : 5	018942.565		ELEVA	TION: 10)1.59	
PROJECT: Proposed Developr							FILE N	io. PG	6912		
BORINGS BY: CME 55 Track-Mou REMARKS:	ınted Me		_		mber	13, 2023	HOLE	ио. ВН	6C-2	3	
SAMPLE DESCRIPTION	STRATA PLOT	Sample No. SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH (m)	Remoulded S Strength (k		Peak Sheatrength (k	ar Ba) Blo	en. Resist. ws/0.3m (t n Dia. Cond	50 B 5
Ground Surface EL 101.59 m				1	_						1
TOPSOIL with organics 0.25 m EL 101.34 m GLACIAL TILL: Brown clayey silt, trace gravel and boulders 0.61 m EL 100.98 m GLACIAL TILL: Compacto to dense, brown silty sand with gravel, cobble and boulders					- 0 						
- grey by 2.1m depth EL 99.38 m GLACIAL TILL: Dense to very dense, grey silty sand with gravel, cobble and boulders, trace clay - boulders by 3.4m depth	S	S 1 67 S 2 0 C 1 35	43 +50								
# 4.62 m EL 96.97 m End of Test Hole (GWL @ 0.76 m - Dec. 15, 2023) DISCLAIMER: THE DATA PRESENTED PRODUCED. THIS LOG SHOULD BE RESERVED					5 6 7						
DISCLAIMER: THE DATA PRESENTED PRODUCED. THIS LOG SHOULD BE RES	READ IN	CONJUNG	CTION	WITH I	Y OF I		G REPOR				



BORINGS BY: CME 55 Track-Mounted Mechanical Auger

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 376042.873 NORTHING: 5018770.725 ELEVATION: 109.28 m

PROJECT: **Proposed Development** FILE NO. PG6912

HOLE NO. BH 7-23 **REMARKS:** DATE: December 13, 2023

SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT	DEPTH	Remou Stren		Strer	ık She ıgth (I	Blows mm D	. Resi s/0.3m)ia. Co 50	า (50	Piezometer
Fround Surface EL 109.28 m					>									
TORCOU with annualisa	1/. •					- 0		-		i		į		\boxtimes
GLACIAL TILL: Brown clayey silt with gravel 0.51 m EL 108.77 m	P	AU 1				- - - -						1 1 1 1 1 1		
ILACIAL TILL: Dense, brown silty sand to andy silt with gravel and boulders, occasional obble		SS 2	75	30		- 1 1 			 		 			
very dense by 1.5m depth	S	SS 3		+50		_ - - - - -2			 		 			
		SS 4	50	+50		- - - - -								
3.28 m EL 106 m nd of Test Hole	\$ 5	SS 5	100	+50		- -3 - -			 		 			
ractical refusal to augering at 3.28m depth						-								
						- - 1			 		 			1
GWL @ 0.66 m - Dec. 15, 2023)														
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GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

PROJECT: Proposed Development
BORINGS BY: CME 55 Track-Mounted Mechanical Auger
REMARKS: DATE: December 13, 2023

BORINGS BY: DATE: December 13, 2023

ELEVATION: 106.96 m

FILE NO. PG6912

HOLE NO. BH 8-23

SAMPLE DESCRIPTION	STRATA PLOT	Sample No.	SAMPLE % RECOVERY	N VALUE or RQD	WATER CONTENT %	DEPTH	Streng	ded Shear th (kPa) 50 100	Streng	Shear th (kPa)	Blows/0 mm Dia	Resist. J.3m (50 J. Cone)	Piezometer Construction
Ground Surface EL 106.96 m			1		1		1	1	1				
TOPSOIL with organics 0.15 m EL 106.81 m						- 0 -							
GLACIAL TILL: Brown clayey silt with gravel		AU 1				E							
0.69 m EL 106.27 m						Ē							
GLACIAL TILL : Very dense, brown silty sand to sandy silt with gravel and boulders,		SS 2	100	+50		- -1				<u> </u> 			
occasional cobble, trace clay EL 105.72 m			1			-		!		!			
EL 105.72 m End of Test Hole						-		į					
Practical refusal to augering at 1.24m depth										!			
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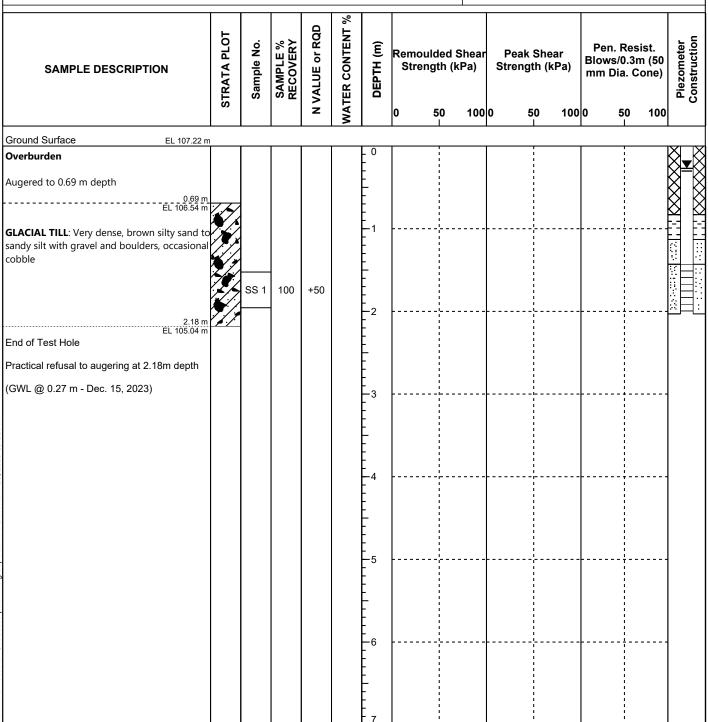


GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING: 375924.947** NORTHING: 5018803.173 ELEVATION: 107.22 m **PROJECT: Proposed Development** FILE NO. **PG6912** BORINGS BY: CME 55 Track-Mounted Mechanical Auger HOLE NO. BH 8A-23

REMARKS: DATE: December 13, 2023



DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.

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GEOTECHNICAL INVESTIGATION

4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 375914.663 **NORTHING:** 5018651.441 **ELEVATION:** 106.87 m

PROJECT: Proposed Development FILE NO. PG6912

BORINGS BY: CME 55 Track-Mounted Mechanical Auger

HOLE NO. BH 9-23 **REMARKS:** DATE: December 13, 2023 N VALUE or RQD **NATER CONTENT** Piezometer Construction STRATA PLOT SAMPLE % RECOVERY Sample No. $\widehat{\mathbf{E}}$ Pen. Resist. Remoulded Shear **Peak Shear** Blows/0.3m (50 DEPTH Strength (kPa) Strength (kPa) **SAMPLE DESCRIPTION** mm Dia. Cone) 1000 1000 100 50 50 50 Ground Surface EL 106.87 m **TOPSOIL** with organics 0.13 m EL 106.74 m **GLACIAL TILL**: Brown clayey silt with gravel 0.48 m EL 106.39 m 100 SS 2 +50 GLACIAL TILL: Very dense, brown silty sand to sandy silt with gravel and boulders, occasional cobble SS₃ 100 +50 -2 80 +50 - grey by 2.2m depth 2.49 m EL 104.38 m End of Test Hole Practical refusal to augering at 2.49m depth -3 (GWL @ 0.13 m - Dec. 15, 2023)

DISCLAIMER: THE DATA PRESENTED IN THIS LOG IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHO IT WAS PRODUCED. THIS LOG SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.

RSLog / Geotechnical Test Pit - Geodetic / paterson-group / admin / December 20, 2023 09:57 AM



BORINGS BY: CME 55 Track-Mounted Mechanical Auger

SOIL PROFILE AND TEST DATA

GEOTECHNICAL INVESTIGATION

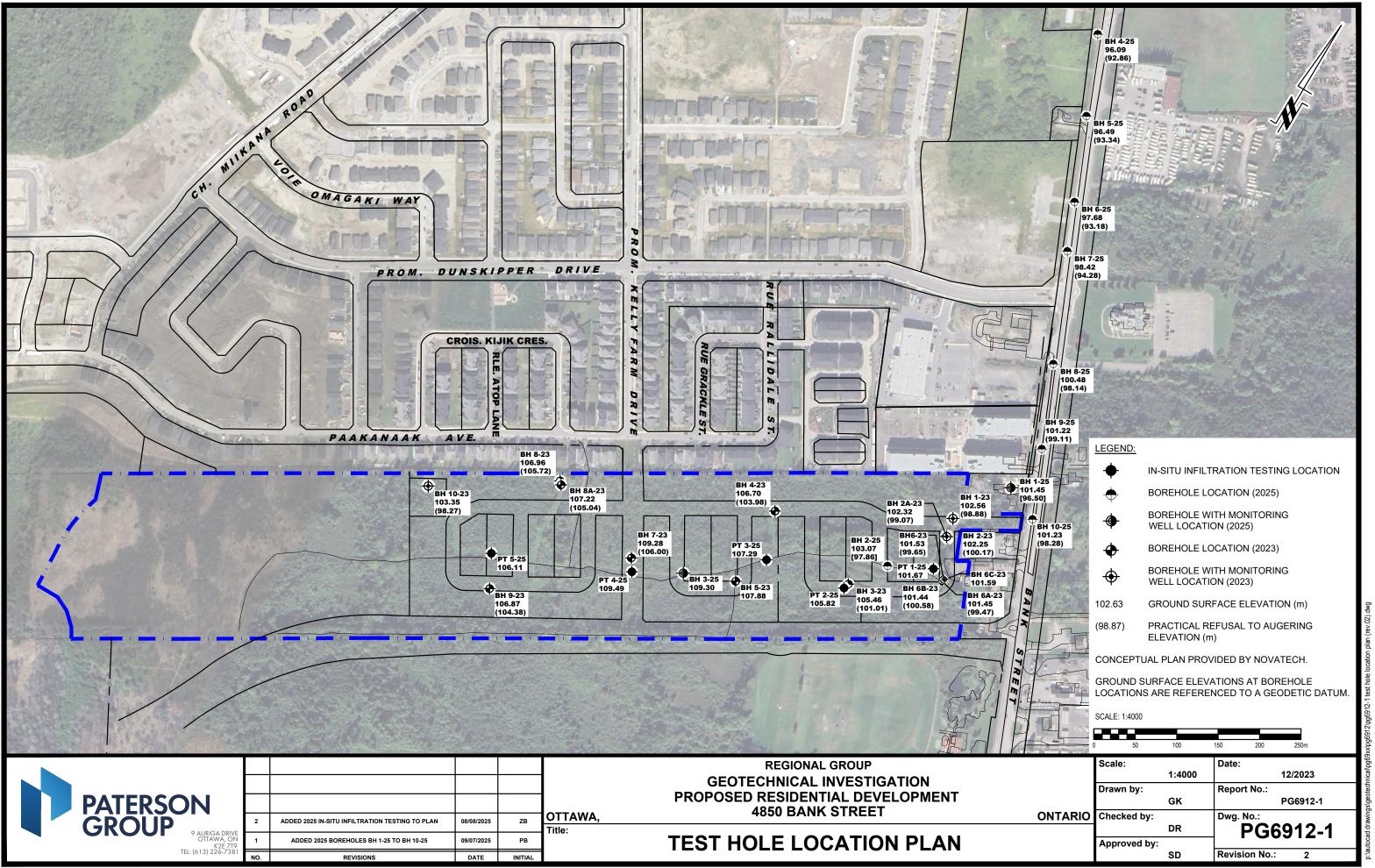
4850 Bank Street, Ottawa, Ontario

DATUM: Geodetic **EASTING:** 375787.732 NORTHING: 5018720.138 **ELEVATION: 103.35**

PROJECT: **Proposed Development** FILE NO. PG6912

HOLE NO. BH10-23 REMARKS. DATE: December 13, 2023

Ground Surface OPSOIL with organics OPSOIL with organics O.33 m EL 103.02 m EL 103.02 m GLACIAL TILL: Compact, brown silty sand to andy silt with gravel, occasional cobble 2.21 m EL 101.14 m EL 101.14 m GLACIAL TILL: Dense to very dense, grey silty and with gravel, cobble, and boulders	AU SS				- 0								
0.33 m EL 103.02 m SIACIAL TILL: Compact, brown silty sand to andy silt with gravel, occasional cobble 2.21 m EL 101.14 m SIACIAL TILL: Dense to very dense, grey silty	AU				- ⁰		i						
EL 103.02 m GLACIAL TILL: Compact, brown silty sand to andy silt with gravel, occasional cobble 2.21 m EL 101.14 m GLACIAL TILL: Dense to very dense, grey silty	SS					1	i						
2.21 m EL 101.14 m		2 46			<u> </u>						-		. ¥
2.21 m EL 101.14 m GLACIAL TILL: Dense to very dense, grey silty		2 46			Ē);; []
iLACIAL TILL: Dense to very dense, grey silty			10		-1				 ÷	 			
iLACIAL TILL: Dense to very dense, grey silty		_			-								
iLACIAL TILL: Dense to very dense, grey silty	6				-						; ; ;		
iLACIAL TILL: Dense to very dense, grey silty	~ ∕∫ >>	3 67	24		-		-						
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5.08 m EL 98.27 m					_5 -		÷	+	 	 	- 		
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ractical refusal to augering on inferred oulder or bedrock at 5.08m depth					-								
GWL @ 0.39 m - Dec. 15, 2023)					- - -6				 <u> </u>		 		
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DISCLAIMER: THE DATA PRESENTED IN			<u> </u>	<u> </u>	7	<u> </u>	1		 1	 	<u> </u>		





APPENDIX 2

- TABLE 6 MONTHLY WATER BALANCE FOR SOIL WITH 75 mm WATER HOLDING CAPACITY AT THE OTTAWA INTERNATIONAL AIRPORT
 - TABLE 7 MONTHLY WATER BALANCE FOR SOIL WITH 300 mm WATER HOLDING CAPACITY AT THE OTTAWA INTERNATIONAL AIRPORT
 - TABLE 8 PRE-DEVELOPMENT ANNUAL WATER BUDGET CALCULATIONS
 - TABLE 9 POST-DEVELOPMENT ANNUAL WATER BUDGET CALCULATIONS
 - TABLE 10 WESTERN CATCHMENT PRE-DEVELOPMENT ANNUAL WATER BUDGET CALCULATIONS
 - TABLE 11 WESTERN CATCHMENT POST-DEVELOPMENT ANNUAL WATER BUDGET CALCULATIONS
 - TABLE 12 EASTERN CATCHMENT PRE-DEVELOPMENT ANNUAL WATER BUDGET CALCULATIONS
 - TABLE 13 EASTERN CATCHMENT POST-DEVELOPMENT ANNUAL WATER BUDGET CALCULATIONS

Month	Temperature (°C)	Total Precipitation (mm)	Actual Evapotranspiration (mm)	Water Surplus (mm)
January	-10.6	62	0	25
February	-9.0	56	1	26
March	-2.8	65	6	103
April	5.7	73	31	110
May	13.1	76	80	14
June	18.3	85	107	4
July	20.9	88	104	3
August	19.7	85	84	1
September	14.8	82	65	3
October	8.3	76	36	14
November	1.3	77	10	38
December	-6.8	79	1	37
Annual	6	904	525	378

Month	Temperature (°C)	Total Precipitation (mm)	Actual Evapotranspiration (mm)	Water Surplus (mm)
January	-10.6	62	0	17
February	-9.0	56	1	21
March	-2.8	65	6	91
April	5.7	73	31	105
May	13.1	76	80	14
June	18.3	85	116	4
July	20.9	89	135	3
August	19.7	84	114	1
September	14.8	82	73	2
October	8.3	76	37	6
November	1.3	77	10	16
December	-6.8	80	1	18
Annual	6	904	605	298



File: PH5087

		Table	8 - Pre-Develo	pment An	nual Water B	udget Calcu	ulations				
Land Use Unit	Area (m²)	Water Surplus (mm)	Topography Factor	Soil Factor	Vegetation Factor	Infiltration Factor	Runoff Factor	Total Infiltration (mm/year)	Total Infiltration (L/year)	Total Runoff (mm/year)	Total Runoff (L/year)
Mature Forest (Fine Sandy Loam)	123,000	298	0.1	0.3	0.2	0.6	0.4	178.8	21,992,400	119.2	14,661,600
Total	123,000								21,992,400		14,661,600

		Table	9 - Post-Devel	opment Aı	nnual Water I	Budget Calc	ulations				
Land Use Unit	Area (m²)	Water Surplus (mm)	Topography Factor	Soil Factor	Vegetation Factor	Infiltration Factor	Runoff Factor	Total Infiltration (mm/year)	Total Infiltration (L/year)	Total Runoff (mm/year)	Total Runoff (L/year)
Impervious Surfaces	88,227	759	0	0	0	0	1	0	0	759.0	66,964,378
Urban Lawn/Shallow Rooted Crops (Fine Sandy Loam)	29,673	378	0.15	0.3	0.1	0.55	0.45	207.9	6,168,925	170.1	5,047,302
Stormwater Management Pond	5,100	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Totals	123,000								6,168,925		72,011,680
Difference (L/year)									-15,823,475		57,350,080
Percentage Variation									-72%		391%



		Table	e 10 - Weste	rn Catchment	Pre-Devel	opment Annı	ual Water Bu	ıdget Cald	culations			
Land Use Unit		Area (m²)	Water Surplus (mm)	Topography Factor	Soil Factor	Vegetation Factor	Infiltration Factor	Runoff Factor	Total Infiltration (mm/year)	Total Infiltration (L/year)	Total Runoff (mm/year)	Total Runoff (L/year)
Mature Forest (Fine Sandy L	oam)	48,000	298	0.1	0.3	0.2	0.6	0.4	178.8	8,582,400	119.2	5,721,600
Total		48,000	•							8,582,400		5,721,600

	Table	e 11 - Weste	rn Catchment	Post-Deve	lopment Ann	ual Water B	udget Cal	culations			
Land Use Unit	Area (m²)	Water Surplus (mm)	Topography Factor	Soil Factor	Vegetation Factor	Infiltration Factor	Runoff Factor	Total Infiltration (mm/year)	Total Infiltration (L/year)	Total Runoff (mm/year)	Total Runoff (L/year)
Impervious Surfaces	57,234	759	0	0	0	0	1	0	0	759.0	43,440,315
Urban Lawn/Shallow Rooted Crops (Fine Sandy Loam)	21,766	378	0.15	0.3	0.1	0.55	0.45	207.9	4,525,165	170.1	3,702,407
Totals	79,000								4,525,165		47,142,722
Difference (L/year)									-4,057,235		41,421,122
Percentage Variation									-47%		724%

	Tabl	le 12 - Easte	rn Catchment	Pre-Devel	opment Annı	ual Water Bu	ıdget Calc	ulations			
Land Use Unit	Area (m²)	Water Surplus (mm)	Topography Factor	Soil Factor	Vegetation Factor	Infiltration Factor	Runoff Factor	Total Infiltration (mm/year)	Total Infiltration (L/year)	Total Runoff (mm/year)	Total Runoff (L/year)
Mature Forest (Fine Sandy Loam)	75,000	298	0.1	0.3	0.2	0.6	0.4	178.8	13,410,000	119.2	8,940,000
Total	75,000								13,410,000		8,940,000

Table 13 - Eastern Catchment Post-Development Annual Water Budget Calculations											
Land Use Unit	Area (m²)	Water Surplus (mm)	Topography Factor	Soil Factor	Vegetation Factor	Infiltration Factor	Runoff Factor	Total Infiltration (mm/year)	Total Infiltration (L/year)	Total Runoff (mm/year)	Total Runoff (L/year)
Impervious Surfaces	30,953	759	0	0	0	0	1	0	0	759.0	23,493,688
Urban Lawn/Shallow Rooted Crops (Fine Sandy Loam)	7,946	378	0.15	0.3	0.1	0.55	0.45	207.9	1,652,074	170.1	1,351,697
Stormwater Management Pond	5,100	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Totals	44,000								1,652,074		24,845,385
Difference (L/year)									-11,757,926		15,905,385
Percentage Variation									-88%		178%

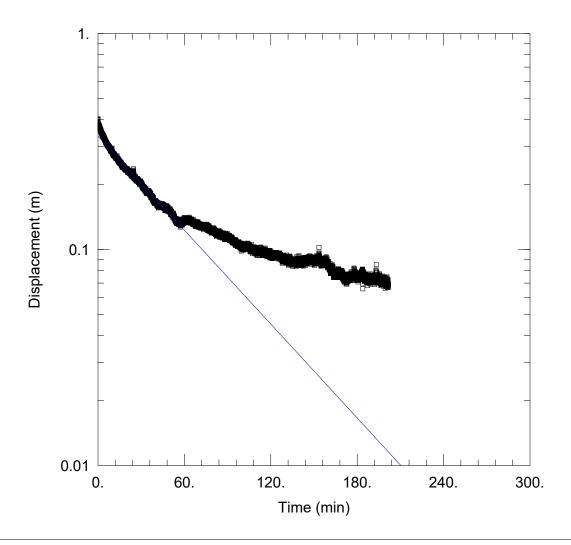




APPENDIX 3

HYDRAULIC CONDUCTIVITY RESULTS - FALLING AND RISING HEAD TESTS

SAMPLE CALCULATIONS - DUPUIT FORCHHEIMER



BH1-23 - FALLING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH1-23 Test Date: July 3, 2025

WELL DATA (BH1-23)

Initial Displacement: 0.403 m Static Water Column Height: 1.28 m

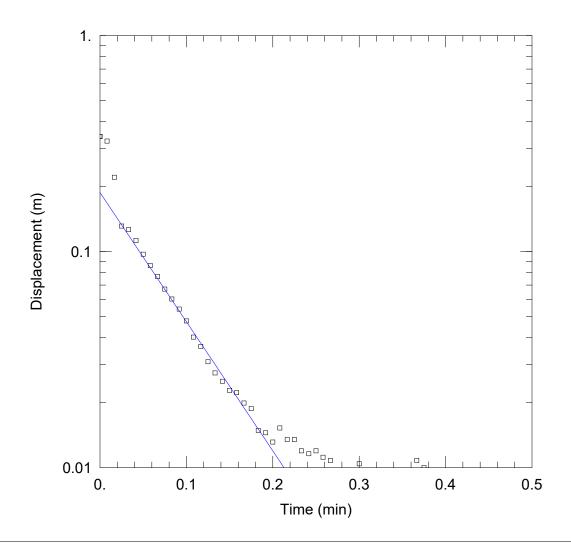
Total Well Penetration Depth: 1.28 m Screen Length: 1.28 m Casing Radius: 0.0254 m

Well Radius: 0.1045 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 3.714E-7 m/secy0 = 0.3347 m



BH1-25 - FALLING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH1-25 Test Date: July 3, 2025

WELL DATA (BH1-25)

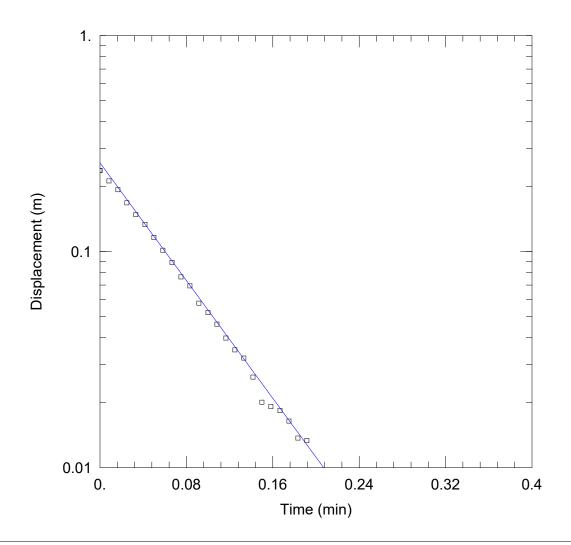
Initial Displacement: 0.341 m Static Water Column Height: 6.54 m

Total Well Penetration Depth: 6.54 m Screen Length: 1.524 m Casing Radius: 0.01588 m Well Radius: 0.0381 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 0.000105 m/sec y0 = 0.1876 m



BH1-25 - RISING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH1-25 Test Date: July 3, 2025

WELL DATA (BH1-25)

Initial Displacement: 0.237 m Static Water Column Height: 6.54 m

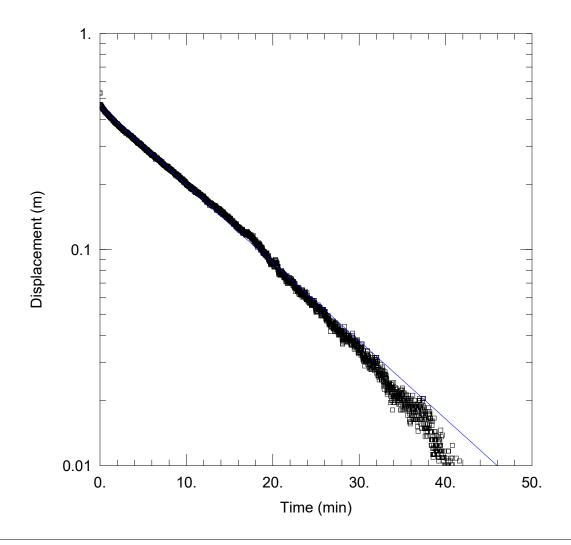
Total Well Penetration Depth: 6.54 m Screen Length: 1.524 m Casing Radius: 0.01588 m

Well Radius: 0.0381 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 0.0001196 m/secy0 = 0.2575 m



BH3-25 - FALLING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH3-25 Test Date: July 3, 2025

WELL DATA (BH3-25)

Initial Displacement: 0.531 m Static Water Column Height: 4.17 m

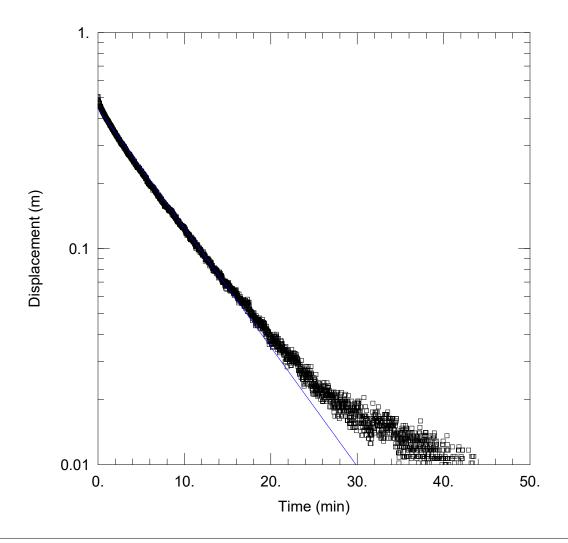
Total Well Penetration Depth: 4.17 m Screen Length: 1.524 m Casing Radius: 0.0254 m Well Radius: 0.1045 m

Well Radius. <u>0.1045</u>

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 1.327E-6 m/sec y0 = 0.4572 m



BH3-25 - RISING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH3-25 Test Date: July 3, 2025

WELL DATA (BH3-25)

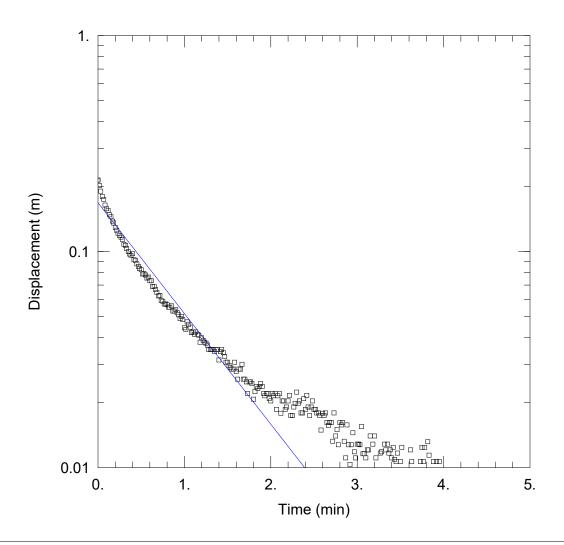
Initial Displacement: 0.506 m Static Water Column Height: 4.16 m

Total Well Penetration Depth: 4.17 m Screen Length: 1.524 m Casing Radius: 0.0254 m Well Radius: 0.1045 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 2.029E-6 m/sec y0 = 0.4448 m



BH6C-23 - FALLING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH6C-23
Test Date: July 3, 2025

WELL DATA (BH6C-23)

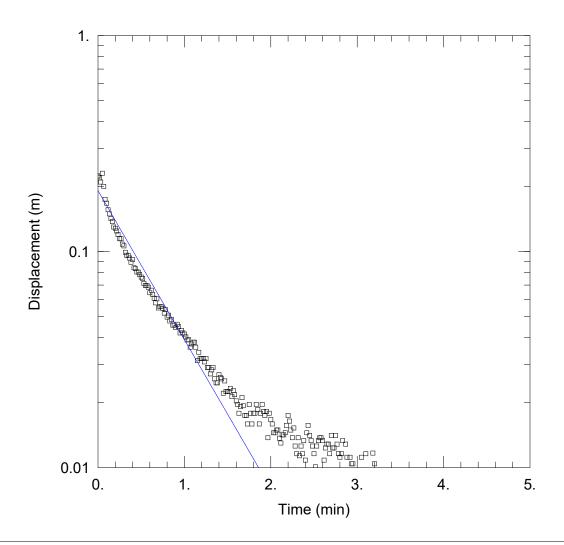
Initial Displacement: 0.214 m Static Water Column Height: 3.41 m

Total Well Penetration Depth: 3.41 m Screen Length: 1.524 m Casing Radius: 0.01588 m Well Radius: 0.0381 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 9.002E-6 m/sec y0 = 0.1683 m



BH6C-23 - RISING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH6C-23
Test Date: July 3, 2025

WELL DATA (BH6C-23)

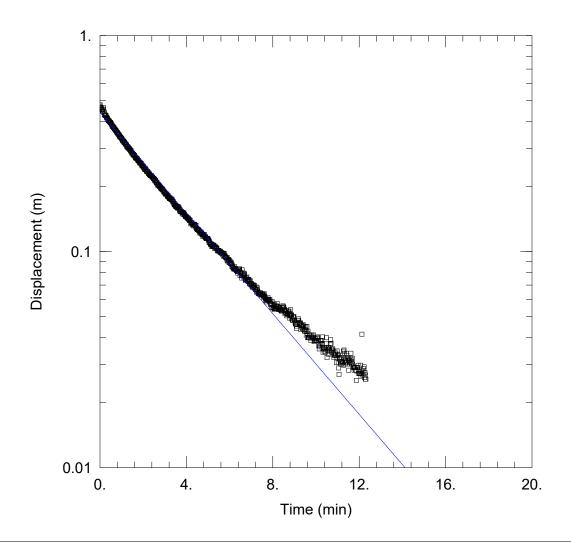
Initial Displacement: 0.224 m Static Water Column Height: 3.41 m

Total Well Penetration Depth: 3.41 m Screen Length: 1.524 m Casing Radius: 0.01588 m Well Radius: 0.0381 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 1.211E-5 m/sec y0 = 0.1914 m



BH10-23 - FALLING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH10-23 Test Date: July 3, 2025

WELL DATA (BH10-23)

Initial Displacement: 0.477 m Static Water Column Height: 3.84 m

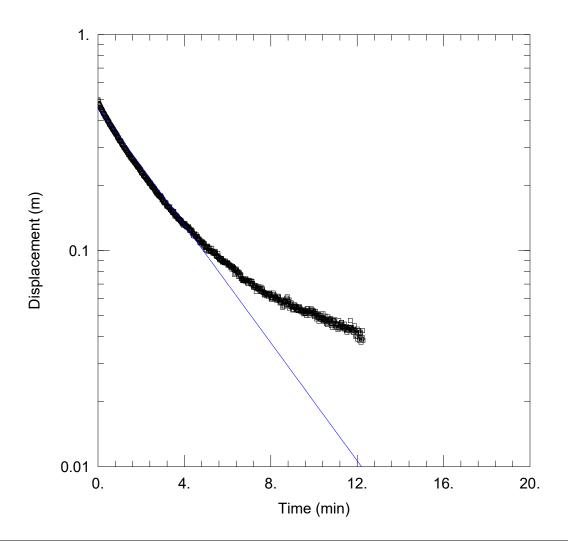
Total Well Penetration Depth: 3.84 m Screen Length: 1.524 m Casing Radius: 0.0254 m

Well Radius: 0.1045 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 4.275E-6 m/secy0 = 0.4396 m



BH10-23 - RISING HEAD 1 OF 1

PROJECT INFORMATION

Company: Paterson Group Inc.

Client: Regional Group

Project: PH5087

Location: 4850 Bank Street

Test Well: BH10-23 Test Date: July 3, 2025

WELL DATA (BH10-23)

Initial Displacement: 0.498 m Static Water Column Height: 3.84 m

Total Well Penetration Depth: 3.84 m Screen Length: 1.524 m Casing Radius: 0.0254 m

Well Radius: 0.1045 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev

K = 5.011E-6 m/secy0 = 0.4612 m

Estimated Groundwater Inflow

Regional Group - 4850 Bank Street - Building Excavation

Dupuit-Forchheimer Equation

 $Q = \pi K((h_0^2 - h_p^2)/In(R/r))$

Equivalent Radius of Excavation = A+B=Pi*r

K (m/sec) = 3.03E-06

h0 (m) = 4 hp (m) = 2 r (m) = 17.35

56.35

58.35

60.35

62.35

64.35

66.35

68.35

70.35

72.35

74.35

76.35

Excavation Width (A) = 26.5 m Excavation Length (B) = 28 m Perimeter Length = 109 m Equivalent Radius (r) = 17.35 m

	Distance to edge of
R	excavation (D)
18.35	1.00
20.35	3.00
22.35	5.00
24.35	7.00
26.35	9.00
28.35	11.00
30.35	13.00
32.35	15.00
34.35	17.00
36.35	19.00
38.35	21.00
40.35	23.00
42.35	25.00
44.35	27.00
46.35	29.00
48.35	31.00
50.35	33.00
52.35	35.00
54.35	37.00

39.00

41.00

43.00

45.00

47.00

49.00

51.00

53.00

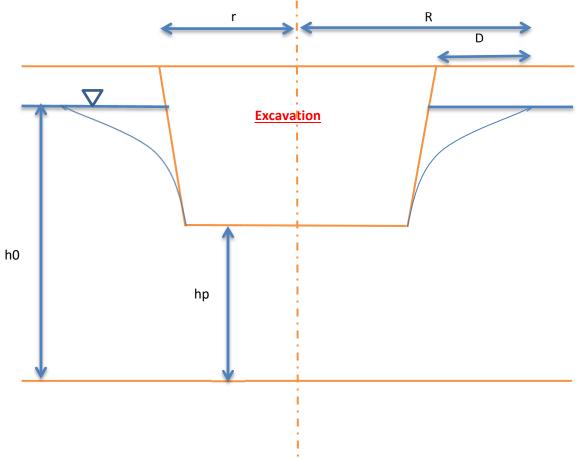
55.00

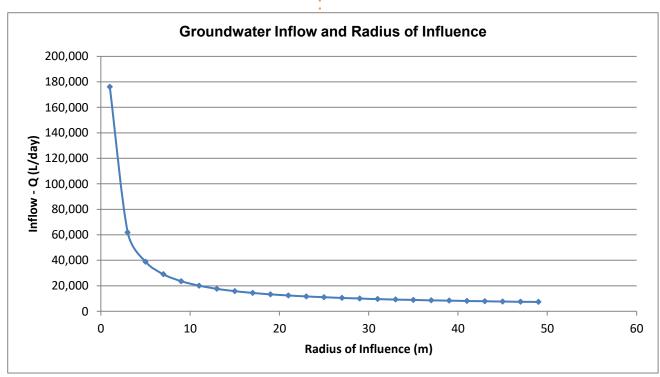
57.00

59.00

Q (m^3/s)	Q (m^3/day)	Q (L/day)
0.0020	176.1	176,101
0.0007	61.9	61,874
0.0005	39.0	38,969
0.0003	29.1	29,115
0.0003	23.6	23,615
0.0002	20.1	20,097
0.0002	17.6	17,647
0.0002	15.8	15,840
0.0002	14.4	14,448
0.0002	13.3	13,343
0.0001	12.4	12,442
0.0001	11.7	11,693
0.0001	11.1	11,059
0.0001	10.5	10,515
0.0001	10.0	10,043
0.0001	9.6	9,629
0.0001	9.3	9,263
0.0001	8.9	8,936
0.0001	8.6	8,643
0.0001	8.4	8,378
0.0001	8.1	8,137
0.0001	7.9	7,917
0.0001	7.7	7,715
0.0001	7.5	7,529
0.0001	7.4	7,357
0.0001	7.2	7,198
0.0001	7.0	7,050
0.0001	6.9	6,911
0.0001	6.8	6,782
0.0001	6.7	6,660









Estimated Groundwater Inflow

Regional Group - 4850 Bank Street - Servicing Excavation

Dupuit-Forchheimer Equation

 $Q = \pi K((h_0^2 - h_p^2)/ln(R/r))$

Equivalent Radius of Excavation = A+B=Pi*r

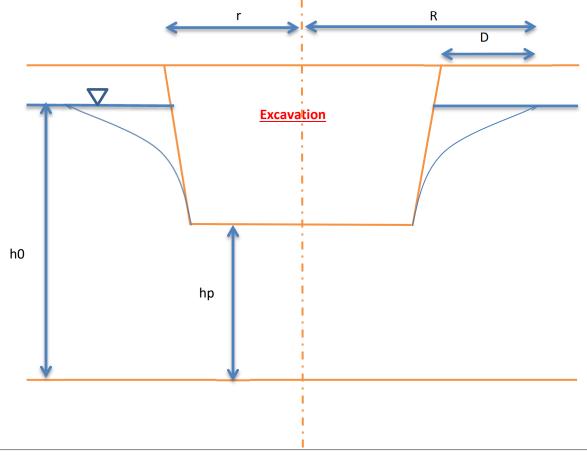
K (m/sec) = 3.03E-06

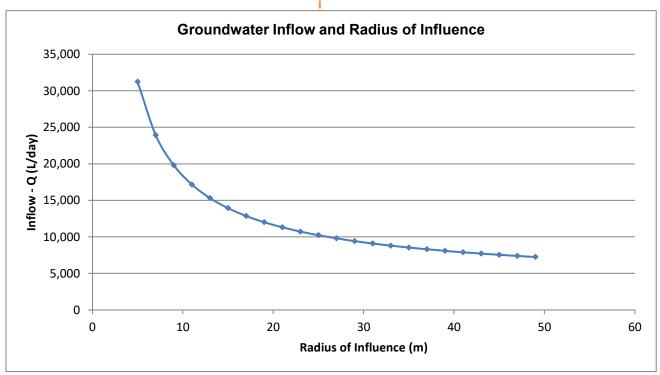
h0 (m) = 4 hp (m) = 0 r (m) = 9.55 Excavation Width (X) = 5 mExcavation Length (Y) = 25 mPerimeter Length = 60 mEquivalent Radius (r) = 9.55 m

	Distance to edge of
R	excavation (D)
14.55	5.00
16.55	7.00
18.55	9.00
20.55	11.00
22.55	13.00
24.55	15.00
26.55	17.00
28.55	19.00
30.55	21.00
32.55	23.00
34.55	25.00
36.55	27.00
38.55	29.00
40.55	31.00
42.55	33.00
44.55	35.00
46.55	37.00
48.55	39.00
50.55	41.00
52.55	43.00
54.55	45.00
56.55	47.00
58.55	49.00
60.55	51.00
62.55	53.00
64.55	55.00
66.55	57.00
68.55	59.00
70.55	61.00
72.55	63.00

Q (m^3/s)	Q (m^3/day)	Q (L/day)
0.0004	31.3	31,251
0.0003	23.9	23,931
0.0002	19.8	19,819
0.0002	17.2	17,171
0.0002	15.3	15,315
0.0002	13.9	13,937
0.0001	12.9	12,869
0.0001	12.0	12,016
0.0001	11.3	11,316
0.0001	10.7	10,731
0.0001	10.2	10,233
0.0001	9.8	9,804
0.0001	9.4	9,430
0.0001	9.1	9,100
0.0001	8.8	8,807
0.0001	8.5	8,544
0.0001	8.3	8,307
0.0001	8.1	8,092
0.0001	7.9	7,896
0.0001	7.7	7,717
0.0001	7.6	7,551
0.0001	7.4	7,398
0.0001	7.3	7,257
0.0001	7.1	7,125
0.0001	7.0	7,001
0.0001	6.9	6,886
0.0001	6.8	6,778
0.0001	6.7	6,676
0.0001	6.6	6,580
0.0001	6.5	6,489









Estimated Groundwater Inflow

Regional Group - 4850 Bank Street - SWMP Excavation

Dupuit-Forchheimer Equation

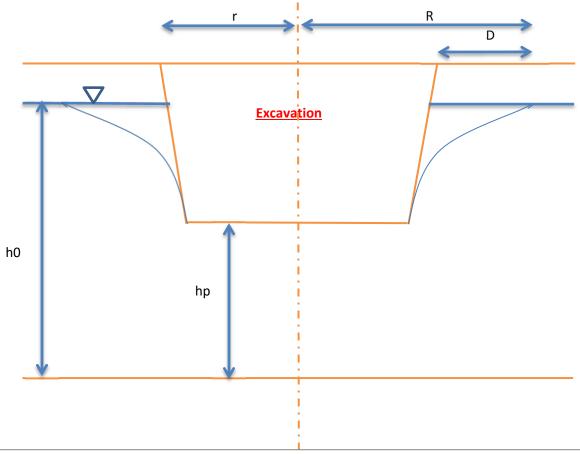
$Q= \pi K((h0^2-hp^2)/ln(R/r))$

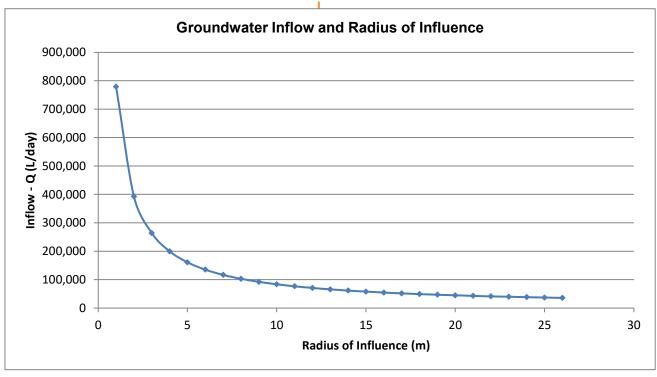
		Equivalent Radius of Excavation =	A+B=Pi + r
K (m/sec) =	3.03E-06		
h0 (m) =	4	Excavation Width (X) =	32.5 m
hp (m) =	0	Excavation Length (Y) =	152 m
r (m) =	58.73	Perimeter Length =	369 m
		Fauivalent Radius (r) =	58 73 m

	Distance to edge of		
R	excavation (D)		
59.73	1.00		
60.73	2.00		
61.73	3.00		
62.73	4.00		
63.73	5.00		
64.73	6.00		
65.73	7.00		
66.73	8.00		
67.73	9.00		
68.73	10.00		
69.73	11.00		
70.73	12.00		
71.73	13.00		
72.73	14.00		
73.73	15.00		
74.73	16.00		
75.73	17.00		
76.73	18.00		
77.73	19.00		
78.73	20.00		
79.73	21.00		
80.73	22.00		
81.73	23.00		
82.73	24.00		
83.73	25.00		
84.73	26.00		
85.73	27.00		
86.73	28.00		
87.73	29.00		
88.73	30.00		

Q (m^3/s)	Q (m^3/day)	Q (L/day)
0.0090	779.4	779,371
0.0045	392.9	392,948
0.0031	264.1	264,128
0.0023	199.7	199,710
0.0019	161.1	161,052
0.0016	135.3	135,275
0.0014	116.9	116,858
0.0012	103.0	103,041
0.0011	92.3	92,291
0.0010	83.7	83,688
0.0009	76.6	76,647
0.0008	70.8	70,777
0.0008	65.8	65,807
0.0007	61.5	61,546
0.0007	57.9	57,851
0.0006	54.6	54,616
0.0006	51.8	51,760
0.0006	49.2	49,221
0.0005	46.9	46,947
0.0005	44.9	44,899
0.0005	43.0	43,045
0.0005	41.4	41,359
0.0005	39.8	39,818
0.0004	38.4	38,405
0.0004	37.1	37,104
0.0004	35.9	35,902
0.0004	34.8	34,788
0.0004	33.8	33,753
0.0004	32.8	32,789
0.0004	31.9	31,889











APPENDIX 4

CITY OF OTTAWA - SALT MANAGEMENT PLAN - APPENDIX A - OCTOBER 171, 2011



City of Ottawa

Public Works and Services Department Surface Operations Branch

Salt Management Plan Appendix A

MATERIAL APPLICATION POLICY

CONTENT

Maintenance Quality Standards – Snow and Ice Control on Roads
General Information
Use of Liquid Chemicals
Material Application Guideline and Policy – Bare Pavement Roads
Material Application Guideline and Policy – Centre-Bare Roads
Material Application Guideline and Policy – Snow Packed Roads
Blast Policy

The Surface Operations Branch District Managers, Area Managers and Zone Supervisors have been consulted through the development of this document.



REVISION INFO

Rev	Date	Ву	Description
3.1	Jan 10 2007		
3.2	Oct 31 2011	D Vander Wal	• Removed 50/50 mix per Dan O'Keefe.
			 Removed specific references to Sodium and Calcium Chloride as new product for 2011 is a Multi-Chloride Brine. Changed liquid application rate from 46 (6%) to 39L/tonne (5%). Removed Dry and Wet salt rates for pavement temperatures below -18C.
			 Updated Epoke rates to match Appendix B and added wet rates to obtain 20% reduction when pre-wetting. Removed separate rate table for Hwy 174 Epoke spreaders since the resulting lane-km rates are the same as other bare pavement.



COUNCIL APPROVED MAINTENANCE QUALITY STANDARDS

For snow clearing, resources are to be deployed and snow clearing completed as defined in the Table below. If the depth of snow accumulation is less than the minimum for deployment, then resources may be deployed subject to road conditions resulting from previous snow accumulations or from forecasted weather conditions.

For treating icy roads, resources are to be deployed as soon as practicable after becoming aware of the icy conditions. Icy roads are to be treated within the times defined in the Table below after becoming aware of the icy conditions.

	MAINTENANCE QUALITY STANDARD SNOW AND ICE CONTROL ON ROADS								
			Minimum Depth of	Time to Clear Snow Accumulation From the End	Treatment Standard				
Main	Road itenance Class	Road Type	Snow Accumulation for Deployment of Resources (Depth as per MMSMH) Accumulation From th of Snow Accumulation From the original From the original From		Bare		Snow Packed		
1	A	High Priority Roads		2 h (3-4 h)	√				
1	В	Tilgii i Hority Roads			2 H (3-4 H)	√			
2	A	Most Arterials	As accumulation begins (2.5-8 cm depending on class) 3 h (3-6 h)		3 h (3-6 h)	$\sqrt{}$			
2	В	Wost Arterials		$\sqrt{}$					
3	A	Most Major		1 h (8 12 h)	$\sqrt{}$				
3	В	Collectors	Collectors 4 h (8-12 h)	4 II (0-12 II)	$\sqrt{}$				
	A				√				
4	В	Most Minor Collectors	5 cm (8 cm)	6 h (12-16 h)		√			
	С	Concetors					√		
-	A, C	Residential Roads	7 cm (10 cm)	10 h (16-24 h)			√		
3	B and Lanes		10 cm (not defined)	16 h (not defined)			√		

Note - MMSMH refers to Ontario Regulation 239/02, Minimum Maintenance Standards for Municipal Highways shown for comparison purposes.

- Bare Pavement: requires that snow and ice be controlled, cleared and/or prevented for the full traveled road pavement width, including flush medians of 2 m width or less, paved shoulders and/or adjacent cycling lanes. It does not include parking lanes.
- **Centre-Bare:** requires that snow and ice be controlled, cleared and/or prevented in a strip down the middle of the road pavement width for a minimum width of 2.5 m on each side of centre-line.
- Snow-Packed: requires that snow and ice be cleared and that ruts and/or potholes that may cause poor
 vehicle control be leveled off. Abrasive or deicing materials are applied at intersections, hills and sharp
 curves.



LIQUID CHEMICALS

Application Rates and Reductions

USE OF LIQUID CHEMICALS						
Chemical	Use	Application Ratio	Chemical Concentration	Application Rate	Dry Salt Reduction	
CaCl, MgCl, or Multi- Chloride	Pre-Wetting	5% by weight	Varies (28%-35%)	39L/t	20%1	
CaCl, MgCl, or Multi- Chloride	Straight Liquid Application	N/A	Varies	60 to 100L/ lane-km	-	

¹ The Epoke controller does not support setting a separate reduction percentage – the rate will only be reduced by the set liquid application ratio (5%).

Pre-Wetted Salt

- Pre-wetting salt is a recommended practice to enhance the performance of the road salt.
- When salt is pre-wet, the brine solution is formed quicker than dry salt and more material is
 retained on the road surface. It is the brine solution that prevents or breaks the bond between the
 road surface and snow/ice.
- The enhanced performance of the salt as well as the retention of salt on the road surface facilitates
 achieving a bare road more quickly and maintains bare pavement longer. As a result, a reduction
 in salt application rates can achieve the same effectiveness as dry salt application at traditional
 rates.

Practical temperature ranges for Pre-Wetted Salt (WET SALT)

- Sodium Chloride Brine (NaCl):
 - o From 0 to −9°C (0 to -12°C as per pre-wetting practices in urban areas)
- Calcium Chloride (CaCl₂), Magnesium Chloride (MgCl), and Multi-Chloride Brines with a minimum eutectic temperature of -30°C:
 - \circ From 0 to -15°C (0 to -18°C as per pre-wetting practices in urban areas)

Direct Liquid Applications (DLA)

- Anti-icing by Direct Liquid Application is a recommended practice to treat frost and black ice conditions in the shoulder seasons at pavement temperatures between 0 and -10°C.
- Liquid should be applied to treat forecasted conditions at the following rates:

Winter Event	Litres / LaneKm	mL/m ² (at 3m width)
Frost	60	20
Light Snow	60 to 80	25
Moderate to Heavy Snow, Freezing Rain	80 to 100	30

- DLA should be applied:
 - As close to the beginning of the winter event as possible
 - When the air and pavement temperatures are both below +5°C currently and forecasted to remain below +5°C within the next 12 hours.
 - When the air and pavement temperatures are a minimum of 10°C above the eutectic temperature of the DLA liquid and forecasted to do so for the next 24 hours.
- DLA should NOT be applied:





October 31, 2011

- When the relative humidity is below 60% and the air and pavement temperatures are between 0°C and +5°C.
- More than once in a three-day period unless a Winter Event (frost, snow, freezing rain or rain) has removed the product from the pavement. Note that DLA liquid can remain on the pavement up to several days after the initial application.

GENERAL INFORMATION

When the Pavement Temperature is below -18°C

- Below –18°C, the salt melting action is close to none.
- Below –18°C, the use of salt shall be discontinued and replaced by an abrasive.
- Multiple factors can affect the performance of de-icing chemicals and abrasives below pavement temperature of -18°C. Under such conditions, supervisors shall select the most appropriate material based on the current and expected traffic volume, current and forecasted weather and road conditions.

Abrasives

- Accepted abrasives are Sand and Grit
- Straight abrasive does contain salt to prevent the stockpile from freezing. The goal is to minimize the amount of salt mixed with the abrasives. The objective is to use an engineered abrasive of 5% salt / 95% sand or grit by volume. The following interim abrasive ratios are accepted (where the engineered ratio cannot be achieved due to equipment and material storage constraints)
 - 10% salt / 90% sand or grit by volume

Rush Hours and Forecasted Conditions

Supervisors are responsible for making timely material application calls based on forecasted conditions and expected traffic peak hours.

Freezing Rain

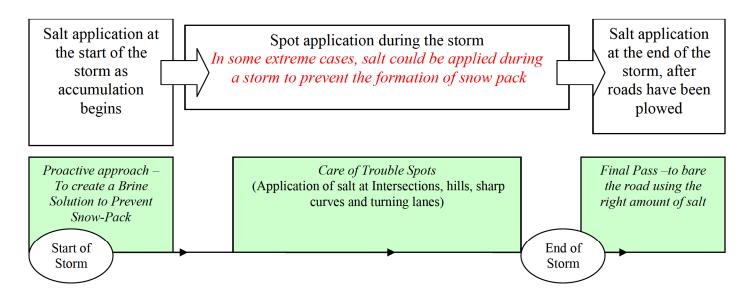
- When Freezing Rain occurs, abrasive materials (sand or grit) will be applied on snow packed roads on a continuous basis (to the full Road Width).
- Snow Packed Roads where available, graders with ice blades shall drag the roads to aid traction.



MATERIAL APPLICATION POLICY					
		BARE PAV	EMENT		
Pavement Temperature °C	Material	Frost and Black Ice <i>Kg/2-lane</i> <i>km</i>	Light Snow <1cm/hr Kg/2-lane km	Heavy Snow >1cm/hr Kg/2-lane km	Freezing Rain Kg/2-lane km
0.4 500	DRY SALT	70	100	140	230
0 to -5°C	WET SALT	55	80	110	185
-5 to -10°C	DRY SALT	85	140	180	230
-5 to -10 C	WET SALT	70	110	145	185
-10 to -18°C	DRY SALT	85	180	230	230
-10 to -18 C	WET SALT	70	145	185	185
<-18°C*	ABRASIVE	350	350	350	-

^{*} Refer to the General Information Section for additional information when the Pavement Temperature is below –18°C. When forecasted warming conditions are expected, dry/wet rates of 180/145, and 230/185 may provide some baring-off benefit.

Timing of Application – BARE PAVEMENT ROADS



Start of the Storm

Salt shall be spread just at the beginning of the icy precipitation.

End of Storm

Salt shall not be spread once bare pavement is achieved and when no further precipitation is forecasted.

^{*} Note: Use wet rates where pre-wetting capable spreaders and liquid supply is available.



MATERIAL APPLICATION POLICY BARE PAVEMENT (EPOKE SPREADERS)

Darramant	Matarial	Even		Light			Cnorr	Engarine	- Dain	
Pavement	Material	I	t and	Light		Heavy		Freezing	g Kam	
Temperature		Blac	k Ice	<1cm/hr		>1cm/hr				
°C		g/m2	Width	g/m2	Width	g/m2	Width	g/m2	Width	
		70kg/2	2ln-km	100kg/2	2ln-km	140kg/2	ln-km	230kg/2	ln-km	
	DRY Salt	35 (30)	2m	50 (43)	2m	70 (60)	2m	115 (98)	2m	
0 to -5°C	(WET Salt)*	23 (20)	3m	35 (30)	3m	45 (38)	3m	77 (65)	3m	
	(WEI Sail)	17 (14)	4m	23 (20)	4m	35 (30)	4m	58 (49)	4m	
		17 (14)	5m	20 (17)	5m	28 (24)	5m	45 (38)	5m	
		85kg/2	2ln-km	140kg/2	140kg/2ln-km		180kg/2ln-km		230kg/2ln-km	
	DRY Salt (WET Salt)*	45 (38)	2m	70 (60)	2m	90 (77)	2m	115 (98)	2m	
-5 to -10°C		28 (24)	3m	45 (38)	3m	58 (49)	3m	77 (65)	3m	
		20 (17)	4m	35 (30)	4m	45 (38)	4m	58 (49)	4m	
		17 (14)	5m	28 (24)	5m	35 (30)	5m	45 (38)	5m	
		85kg/2ln-km		180kg/2ln-km		230kg/2	ln-km	230kg/2	ln-km	
	DDV Cale	45 (38)	2m	90 (77)	2m	115 (98)	2m	115 (98)	2m	
-10 to -18°C	DRY Salt	28 (24)	3m	58 (49)	3m	77 (65)	3m	77 (65)	3m	
	(WET Salt)*	20 (17)	4m	45 (38)	4m	58 (49)	4m	58 (49)	4m	
		17 (14)	5m	35 (30)	5m	45 (38)	5m	45 (38)	5m	
			2ln-km	350kg/2	2ln-km	350kg/2ln-km		-		
		175	2m	175	2m	175	2m			
< -18°C†	ABRASIVE	115	3m	115	3m	115	3m	_	_	
		88	4m	88	4m	88	4m	_	_	
		70	5m	70	5m	70	5m			

^{*} When the pre-wetting system is engaged, the dry material output is reduced. The Epoke controller does not support setting a separate reduction percentage – the rate is only reduced by the set liquid application ratio (5%). Material 2 was therefore configured with rates reduced by 15%.

Notes

There are 2 variables affecting the material output on an Epoke salt spreader:

- -Material Application Rate in **g/m²** AND Application Width in **m**. Examples:
- For a rate of 100kg/2ln-km, the Epoke Setup would be 25g/m² at a Width of 4m. **OR** a rate of 50g/m² at a Width of 2m.
- For a rate of 170kg/2ln-km, the Epoke Setup would be 42g/m² at a Width of 4m. **OR** a rate of 57g/m² at a Width of 3m.

^{*} Use wet rates where pre-wetting capable spreaders and liquid supply is available.

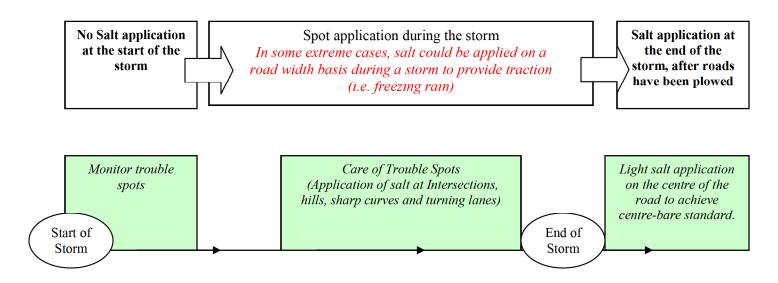
[†] Refer to the General Information Section for additional information when the Pavement Temperature is below –18°C. When forecasted warming conditions are expected, dry/wet rates of 180/145, and 230/185 may provide some baring-off benefit.



MATERIAL APPLICATION POLICY CENTRE-BARE PAVEMENT									
Pavement Temperature °C	Material	Frost and Black Ice	Snow	Freezing Rain					
		Kg/2-lane km	Kg/2-lane km	Kg/2-lane km					
0.4- 500	DRY SALT	70	100	230					
0 to -5°C	WET SALT	55	80	185					
54- 100C	DRY SALT	85	140	230					
-5 to -18°C	WET SALT	70	110	185					
<-18°C	ABRASIVE	350	350	-					

Note: Use wet rates where pre-wetting capable spreaders and liquid supply is available.

Timing of Application – CENTRE-BARE PAVEMENT ROADS



Start of the Storm

No Salt application at the start of the storm. Monitor trouble spots.

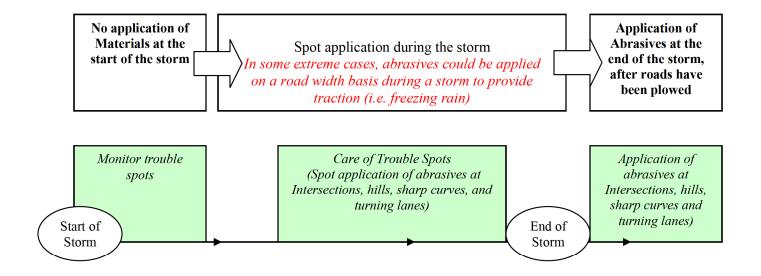
End of Storm

Salt shall not be spread once centre-bare pavement is achieved and when no further precipitation is forecasted.



MATERIAL APPLICATION POLICY (Intersections, Hills and Sharp Curves) SNOW PACKED							
Pavement Temperature °C	Material	Frost and Black Ice Kg/2-lane km	Snow Kg/2-lane km	Freezing Rain Kg/2-lane km			
0 to -30°C and below	ABRASIVE	350	350	500			

Timing of Application – SNOW PACKED ROADS



Start of the Storm

No application of abrasives at the start of the storm. Monitor trouble spots.

End of Storm

Abrasives shall not be spread once traction is provided.



BLASTING POLICY

The On-Board Electronic Controller's Blast function is an important tool for roadway deicing operations. It allows operational staff to timely increase the amount of spread material at trouble locations such as steep hills and sharp curves. Although the blast function is indispensable, it should be used with care as it its liberal use can lead to significant increases in salt consumption and environmental impacts.

- Supervisory staff shall work toward minimizing the amount of salt being spread using the Blast function to achieve the required maintenance quality standard.
- Many variables come into play during a winter weather event. As such, the call to allow the use of the Blast Function during a winter event is left to the judgment of the supervisory staff, as the first priority is the safety of the traveling public.

The Blast function shall only be used at the following locations:

- Steep Hills
- Elevated Curves
- Intersections (within 30m of the stop line on the approach side only)
- Shade areas
- Right and Left Turning Lanes
- Bus Bays
- Railways (within 30m of the railway crossing on the approach side only)
- Bridge Decks

Caution: when blasting salt on a bridge deck. Rock salt needs heat to dissolve. Spreading salt on a bridge deck could lower its surface temperature to a point where the brine solution will refreeze.

Application:

- The Blast function shall only be used under severe winter conditions
- The Blast function shall not be used during light winter weather events such as light snow, frost, etc.
- The blast function shall not be used while clearing the roads (stripping) at the end of a storm.

On-Board Electronic Controller's Blast function

- The Epoke controllers will blast at the highest material calibration setting.
- The CS-230 controller will blast to its maximum hydraulic power (which can be adjusted if too high)
- The CS-440 controller can be calibrated at a defined Blast rate for each material.
 - o The Blast Rate for Salt is to be set at 300kg/2 lane-km
 - O The Blast Rate for Abrasive is to be set at 500kg/2 lane-km. Note: Suburban/Rural District has a requirement to Blast Abrasives on gravel roads at a rate of 700kg/2 lane-km. To achieve this rate, the spreaders need to be calibrated using two gate settings. The District will provide, every fall, a list of spreaders requiring this specific calibration.



APPENDIX 5

SURROUNDING WATER WELL RECORDS



No

Elev. 4 R 0131310

The Water-well Drillers Act, 1954 Department of Mines

GROUND WATER BRANCH MAY 20 1957 ONTARIO WATER

in Village, Town or City).....

Static level

Depth(s)

at which

water(s)

found

60

Address 40 Farrance It Mana

Pumping Test

Water Record

No. of feet

water rises

Basin |2|5| |2|1 Water-Well Record COMMISSION Carleton Township, Village, Town or City Slove County or Territorial District. (month) (day) (year) Pipe and Casing Record Casing diameter(s) Length(s) Type of screen Length of screen Well Log From Overburden and Bedrock Record ft. Sand 20 60 For what purpose(s) is the water to be used? touse Is water clear or cloudy? Is well on upland, in valley, or on hillside?.... Kipland Drilling firm F. R. Const. Address 2 Bareline RO City diese Name of Driller Z. R. f. J. H. Address Licence Number 3 7 3

> I certify that the foregoing statements of fact are true.

Date 24 any 14/50 St R Constit

Pumping rate 800 J. P. f/ Duration of test 2 hr

Kind of water

(fresh, salty, or sulphur)

Jonest

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.

A Month Tohnston Cors CSC.ST

Form 5

316/50 The second UTM | /|8|z | 4|5|3|0|/|0|E 9R 5101/16191910N



The Water-well Drillers Act, 1954 Department of Mines

GAPEND WAYER BRANCH AUG 19 1957 ONTARIO WATER RESOURCES COMMISSION

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County or Territorial District	-antita	≠ M Towr	nghin	Village, Town or C	City Co	ucestar
Country of Territorial Distriction	ere i i i i i i i i i i i i i i i i i i		n Vi	illage, Town or G	ity)	
			Addı	illage, Town or Ciress	1.00.JJ 33 c.	ridge
(day)	(month)	(year)			0	
Pipe and Casir	g Record				Pumping Test	
Control disposition (a)			Ctat	ic level	13	
Casing diameter(s)	1		Pum	ping rate	240 9-	PH
Type of screen			Pum	ping level	Ofeel	
Length of screen				ation of test	Thous	
					377 / 3 2	
Well Lo	S				Water Record	
Overburden and Bedrock Record	From ft.	To ft.		Depth(s) at which water(s) found	No. of feet water rises	Kind of wate (fresh, salty or sulphur)
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Albourders						
	10	50				
Ley hard line		00				
For what purpose(s) is the wate	r to be used?	l			,	A
house he			,		eation of Well	
Is water clear or cloudy?				In diagram below road and lot lipe		
Is well on upland, in valley, or or	n hillside?				South Glo	
upland	?			36	'	
Drilling firm	••••••	•••••				
Address	•••••••••••			* X	70 feet	-> well
Name of Driller amlo	ReTH	10				
Address	and in the	6		16		
7.1.4.1.055	0					
Licence Number 537	•••					
I certify that the	foregoing					
statements of fac	t are true.					
Date Gugs 5 James	Signature of Licen	see		1.		

CSS.58

UTM 2/8 2 4.57.3181610 E		GRO	OUND WATER	15 Nº	21
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Con 4 R F Lot P. T. 22	ate co	()	ıay	month	year)
	lress	28 Cla	rence St	• Ottawa	2, Ont.
Casing and Screen Record		,	Pumping	Test	
Inside diameter of casing 6 3/16	Stat	ic level			a T
Total length of casing 211	Tes	t-pumping rat	e		G.P. A .
Type of screen	Pur	mping level	/0	1 hm	
Length of screen	Dui	ration of test p	umping	clea	ar
Depth to top of screen	Wa	ter clear or clo	udy at end of t	est80	ar H G.P. M .
Diameter of finished hole 6"	Re	commended pi	imping rate 80		w ground surface
	wit	h pump setting	g of		r Record
Well Log				Depth(s) at	Kind of water
Overburden and Bedrock Record		From ft.	To ft.	which water(s) found	(fresh, salty, sulphur)
Till and Wilder rest. Grey hard lime a	ton	9	16-	85	fresh
and sand stone	PERSONAL SPENSING	25	89		
SAN A TO THE PROPERTY OF THE PARTY OF THE PA					
BOULDER TILL		0	16		
HARD GREY LIMESTONE		16	25		
5 ANDS Tone		25	89	85	FBESH.
	1				
For what purpose(s) is the water to be used?		In diagrai	Location n below show	distances of we	ell from
Co-operative Valley		road and	lot line. Ind	licate north by	arrow.
Is well on upland, in valley, or on hillside? Drilling or Boring Firm J. B. Dufresne Co. Ltd.			14		
•			150'	2	
Address Ottawa, Ontario.	. Standard	,	, —	~	
		/	Pini	I	
Licence Number 194	· · · · · · · · · · · · · · · · · · ·	·			and the second s
Name of Driller or Borer W. Roy	-	PALICE TO THE PARICE TO THE PA			
Address Hull			ا نہ		
Date Oct 10/6		Jallan Mark	املغا		
(Signature of Licensed Drilling or Boring Contractor)		John W			
\~~ ```) t	(4)			
Form 7 15M Sets 60-5930					

316/52 GROUND WATER BRANC Ontario Water Resources Commission Act ONTARIO WATER RESOURCES COMMISSION TER WELL MLE Tony Township, Village, Town or City G-Love Es TER Date completed 29 June 6/ ddress BILLINGS BRIDGE **Pumping Test** Casing and Screen Record Inside diameter of casing Test-pumping rate G.P.M. Total length of casing // / Pumping level Type of screen Duration of test pumping / HR Length of screen Water clear or cloudy at end of test ZCEAR Depth to top of screen Recommended pumping rate 4 G.P.M. Diameter of finished hole with pump setting of ______ feet below ground surface **Water Record** Well Loa Kind of water Depth(s) at \mathbf{From} (fresh, salty, sulphur) which water(s) Overburden and Bedrock Record found LOAM 0 FAEY Lomest ME 55 Location of Well For what purpose(s) is the water to be used? In diagram below show distances of well from road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? Drilling or Boring Firm IN MEAGHER OTTAND Licence Number 245 Name of Driller or Borer 5 Am &

OWRC COPY

Form 7 15M Sets 60-5930

C50.03

GROUND WATER BRANCH UTM 118 2 41513181010 E 510117151310 N Ontario Water Resources Commission Act ONTARIO WATER ER WELL RECORDINGES COMMISSIONTownship, Village, Town or City.... Date completed 26 BILLINGS BRIDGE **Pumping Test** Casing and Screen Record Static level Inside diameter of casing Test-pumping rate Total length of casing Pumping level Type of screen Duration of test pumping Length of screen Water clear or cloudy at end of test Depth to top of screen Recommended pumping rate Diameter of finished hole with pump setting of. feet below ground surface Water Record Well Log Kind of water Depth(s) at From To ft. which water(s) (fresh, salty, Overburden and Bedrock Record found sulphur) CLAY LIMESTON 46 Location of Well For what purpose(s) is the water to be used? In diagram below show distances of well from road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? otrana Address Licence Number Name of Driller or Borer Address Form 7 10M-62-1152 CSS.58 OWRC COPY



Elevi d 4 R | 6 1301744 |
Basin 2 157 | 1 |

Lot 23

The Water-well Drillers Act, 1954

Department of Mines



Water-Well Record

	Tami akan	en	ip, Village, Town or	City Gloucest	er
			Village, Town or (City)	••••
			ddress Johnst	ons Corners	••••••
Date completed	(month)	(year)	_		
Pipe and Casing	g Record			Pumping Test	
Casing diameter(s)	5"		Static level50	*	
I anoth (g)	85'		Pumping rate40	o gpu	
Type of screen	Nor	1É	Pumping level2.2	**************************************	********************************
Length of screen				hr.	
Well Log				Water Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
sand & boulders	0	85			
limestone	85	116	116	66	fresh
	_				<u> </u>
		_			
		_			
For what purpose(s) is the water	to be used?		I	ocation of Well	
		<i>y.</i>		w show distances of	
Is water clear or cloudy? farm			road and lot lin	ne. Indicate north	by arrow.
Is well on upland, in valley, or or upland	n hillside?				
Drilling firm F.A. McLean	& Son			//	
Address				/	
				/	
Name of Driller A. Scharf				•	

Form 5

Date.....Jan I

Licence Number.....

statements

I certify that the foregoing

UTM 1/18 Z 41512171810 E



5 R 5 0 1 5 9 1 10 N Ontario Water Resources Commission Act

J451 N862

GROUND WATER BRANCH

ONTARIO WATER

ONTARIO WATER RESOURCES COMMISSION

Elev. 4 R 013141	WATER	WELL	RECORD
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Basin 💄	215.	Chal	eTon		Township, Village	Town or City	6/ouces	1er
County	or District	2 🗢	. 			<i>q</i>	Mari	1912
Con	/V	n-	Lot	ζ 3	Date completed	(day	month	year)
					e/o Mc	• -	4ssociates	
					dress	N. 0 5 / / E . C /	7330077	9 0 V / A

Pumping Test Casing and Screen Record Static level 7' 65/8" Inside diameter of casing..... Test-pumping rate /00 G.P.M. Total length of casing..... Pumping level /5-/ Type of screen none Duration of test pumping 24 hrs Length of screen Water clear or cloudy at end of test c/ear Depth to top of screen Recommended pumping rate /60 G.P.M. Diameter of finished hole with pump setting of 15 feet below ground surface

Well Log			Water	Water Kecord		
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)		
(SANDY LOOM	0	3				
7. 5.4	.3	36				
Fine Sand SandsTone	36	57/	47	Fresh		
		1				

For what purpose(s) is the water to be used? ARETARCH

Domestic & OTher Uses

Is well on upland, in valley, or on hillside? Upland

Drilling or Boring Firm

Drilling or Boring Firm

McLean Water Supply Ltd.

Address 1532 Raven Ave

OTTawa Ont.

Licence Number 196

Name of Driller or Borer H, Sally

Date May 10, 1962

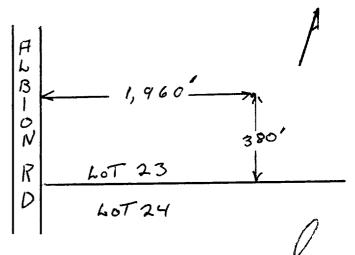
(Signature of Licensed Drilling or Boring Contractor)

Form 7 15M Sets 60-5930

OWRC COPY

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow. V



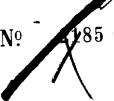
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CSS,58



DEC 12 1952

GEOLOGICAL BRANCH



The Well Drillers Act Department of Mines, Province of DEPARTMENT of MINES

County or District	Tp.,	glowester of	Leughet Acres	F t. LUt	
		_)		
Pipe and Cas	sing Kecoru		Pumping Test		
Casing diameter(s). Length(s) of casing(s). Length of screen. Type of screen. Capacity of pump. Depth of pump setting.	8 fut	Developed Capacity Duration of Test Pumping Rate Drawdown Static level of compl	A Auter eted well . I. 4 . Auter type?	ut.	
		Water Record			1
Kind (fresh or mineral) Quality (hard, soft, contains Appearance (clear, cloudy, clear what purpose(s) is the vertical contains	s iron, sulphur etc.)	clar	Depth(s) to Water Horizon(s)	Kind of Water	No. of Fee Water Rise
What is source of contamina	ation?	een made of water			
What is source of contaminate Enclose a copy of any mine Drift and Bedre	ral analysis that has b		Loca		
What is source of contamina Enclose a copy of any mine	ral analysis that has b	een made of water	Loca	w show dista	
What is source of contamina Enclose a copy of any mine	ral analysis that has b	een made of water	In diagram belofrom road and lo	w show dista	

Licence Number . 3. 6.5.

15 No

W.

			ll Record		n de la compania de la compania de la compania de la compania de la compania de la compania de la compania de La compania de la co
County or Territorial District	arthan	zTown	ship, Village, Town or C Village, Town or Ci	ty)	
			Village, Town or Ci	ngs Bildy	L.C.
(day)	(month)	(year)			
Pipe and Casir	ng Record			Pumping Test	
a. ii da H	Hunch	1	Static level	SAL	
Casing diameter(s)Length(s)	100		Pumping rate	12/990	e PER-HB
Type of screen			Pumping level	feet	
Length of screen			Duration of test	1 Av	
hength of screen					
Well Lo	g			Water Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
RA L Cuan				6210×	
Black Gum				7	fush
boulders	0	6			
Hoch hail mer lene		335	3 3 Fred		
	6				
hard gry lime store	35,	60			
DOLOMITE	60	10	70		
				-	
					
For what purpose(s) is the water				cation of Well	f wall from
Is water clear or cloudy?	Glear		_	. Indicate north	`
Is well on upland, in valley, or o	on hillside?			JOHNSON SORS	
Drilling firm	(-)				. cide
Address		l			1 200.
1				13	
Name of Driller	¥		.	:	و المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة المعالمة
Address			_ 3/ ===	Marine Company	→ <u> </u>
Licence Number 337	•••••		~	X.	
Licence Number 337	•••••				
I cortify that th					

Form 5

statements of fact are true.

Signature of Licensee



GROUND WATER BRANCH

JUN 15

ONTARIO WATER RESOURCES COMMISSION

| 5 R | 5 0 | 1 | 5 | 6 | 1 | 0 | N The Ontario Water Resources Commission Act Elev. 4 R 0131615

Basin 2	ا کا ا r District	Carl	eTon	 Townsh	iip, Village,	Town or Cit
				Date co		

y GlovcesTen Date completed

idress Tome Bastie & DSSO eMTES

Casing and Screen Record	Pumping Test
Inside diameter of casing 6 578 "	Static level 30'
Total length of casing 5/	Test-pumping rate SG.P.M.
Type of screen ———————————————————————————————————	Pumping level 43
Length of screen	Duration of test pumping 4 hrs
Depth to top of screen	Water clear or cloudy at end of test
Diameter of finished hole 6 /2 "	Recommended pumping rate S.P.M.
	with pump setting of 43 feet below ground surface
Well Log	Water Record

Well Log			Water	Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
sandy loam	0	3		
onavel & houlders	3	47		
sandy loam gravel & boulders sandslone	47	60	60	Fresh
				•

For what purpose(s) is the water to be used? RACE TINACH. Domestic & Other uses Is well on upland, in valley, or on hillside? upland Drilling or Boring Firm.

McLean Water Supply Liteli Address 1532 Ruven Hue OTTawa

Licence Number 196 Name of Driller or Borer H. Sqlly

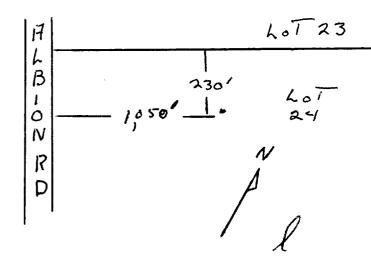
(Signature of Licensed Drilling or Boring Contractor)

Form 7 15M Sets 60-5930

OWRC COPY

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



CSS.53

UTM) 1/18 Z 415141	/ / O E			
[5 R 5 6 1 6]	VATFR	Water Resources	Commission Act	

WATER RESOURCES

DIVISION

15 No. 2250

JAN 1 9 1965

ONTARIO WATER
RESOURCES COMMISSION

40'	\sim \supset	~~~				9			
Basin J 2	Bistrict C	urleTon)	Townsl	nip, Village	e, Town or	City G/	ouces7	ei
Con.	V B	∠ Lot	23		ompleted	14	170		64
					•	(day	mont	h yea	ar)
					p	254	RDI	(37)	

Casing and Screen Record		Pump	oing Test	
Inside diameter of casing 5"	Static level	2	01	
Total length of casing. 10 '	Test-pumping	rate	4	G.P.M
Type of screen none	Pumping leve	1	65-1	
Length of screen	Duration of te	est pumping	1/2 hrs	.
Depth to top of screen	Water clear or	r cloudy at end	of test Clo	vdy
Diameter of finished hole 5"	Recommende	d pumping rat	te 4	G.P.M
	with pump se	tting of 75	feet belo	w ground surface
Well Log			Wate	r Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
	0	2		
Hard SandsTone Red GraniTe	2 65	65	60 - 79	P 4 = 1=
Nett Branife	63		60 - 79	fi-es/7
For what purpose(s) is the water to be used?		Location	n of Well	TAG I I
house	į.	ram below sho	w distances of wel	ll from
Is well on upland, in valley, or on hillside? hillside	road a	nd lot line. I	ndicate north by	arrow.
Drilling or Boring Firm			n I hatwee	217
Mchean Water Supply LTd			Road between	

Is well on upland, in valley, or on hillside? hillside

Drilling or Boring Firm

Mchean Warer- Supply LTcl

Address /532 Raven Ave

OTTawa

Licence Number /328

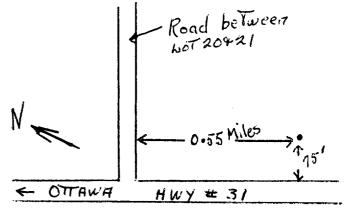
Name of Driller or Borer H. Sally

Address

964 Lean

(Signature of Licensed Drilling or Boring Contractor)

Form 7 15M-60-4138



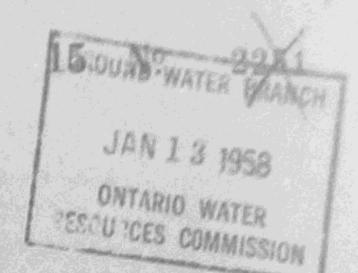
C85.83

18 41514131610 E 15 R 5 01/16131/10 N Elev. 4 R 61410121



The Water-well Drillers Act, 1954

Department of Mines



Water-Well Record

County or Territorial District. Con. Lot. 29 Owner Date completed	Street and N	Number (if	ship, Village, Town or in Village, Town or (Address	City)	angerman ang Karagas
Pipe and Casing	Record			Pumping Test	
Casing diameter(s)	••••••••••••••••••••		Static level	, J	****************
Well Log				Water Record	
Overburden and Bedrock Record	From 2t.	To st.	Depth (s) at which water (s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
Drog limestore	3	101	92	76	Brech
For what purpose(s) is the water t					
Is water clear or cloudy?	oregoing are true.	Caracad Caraca	In diagram below road and lot line	show distances of a lindicate north	그 경우 아무슨 아무슨 아무는 아무는 아무슨 것이 없는데 그 때문에 다른데 다른데 다른데 다른데 다른데 다른데 다른데 다른데 다른데 다른데

Basin 213 WATI	o Water Resc	ELL F	ission Act, 1957 RECORI Village, Town or oleted O John	GROUND WATER FEB 23 ONTARIO WATER RESOURCES CON	1961 VATER AMISSION
Casing and Screen Record				nping Test	
Inside diameter of casing. 6 " Total length of casing. 7 Type of screen. Length of screen. Depth to top of screen. Diameter of finished hole / 00 '		Test-pun Pumping Duration Water cl	g level	end of test C	ur.
Well Log				ter Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, sulphur)
Thosp Innections	22	100	98	68	- I
For what purpose(s) is the water to be used? Is well on upland, in valley, or on hillside? I pland: Drilling Firm I. I folkson Orull Address 1340 Banks Sha Atlanta Atlanta Licence Number 47.0 Name of Driller Baks Andario Date 7.0 20, 1966 Signature of Licensed Drilling Contractor On Diagonal Contractor Con Di	ling Co-St	r	n diagram below oad and lot line	. Indicate nort	

OS5.58

Form 5

SURFACE WATER BRANCH Rideau Front 316/50 UTM 118 2 41514131210 E FEB156 1909 5 R 5 0 1 6 6 6 6 9 Ontario Water Resources Commission Act ONTARIO WATER RESOURCES COMMISSION Elev. 4 R O131810 WATER WELL RECOR Basinty of District 1 1756 & 7 cm Township, Village, Town or City 6 Love & 7 cm Date completed 7 JAM 63 **Pumping Test** Casing and Screen Record Static level Inside diameter of casing Test-pumping rate G.P.M. Total length of casing Pumping level / 2 Type of screen Duration of test pumping /Hr Length of screen Water clear or cloudy at end of test CCEAA Depth to top of screen Recommended pumping rate 3 G.P.M. Diameter of finished hole with pump setting of 70 feet below ground surface Water Record Well Log Kind of water Depth(s) at From To which water(s) (fresh, salty, Overburden and Bedrock Record found sulphur) 10 of man 70 /0 6/m257 ne For what purpose(s) is the water to be used? Location of Well In diagram below show distances of well from 6 BEEN HOUSES road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? Drilling or Boring Firm My MEDGHER Address Taup Licence Number 6/8 Name of Driller or Borer 5 177116 66726 (Signature of Licensed Drilling of Form 7 10M-62-1152 Css.sg OWRC COPY

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The Ontario Water Resources Commission Act

WATER WELL RECORD

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County	or	District		سيمير		رج
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٧		- %	- 1 <i>V</i> .	_(\\'1	∡ot	

Township, Village, Town or City

Date completed (day

month year)

Casing and Screen Record	Pu
Inside diameter of casing	Static level
Total length of casing	Test-pumping rate
	Pumping level
27PS 22 22	Duration of test pumping
Length of screen	Water clear or cloudy at e
Depth to top of screen	Recommended pumping
Diameter of finished hole " " inch	Recommended part 1

Static level

Test-pumping rate

Duration of test pumping

Water clear or cloudy at end of test

Recommended pumping rate

G.P.M.

G.P.M.

G.P.M.

G.P.M.

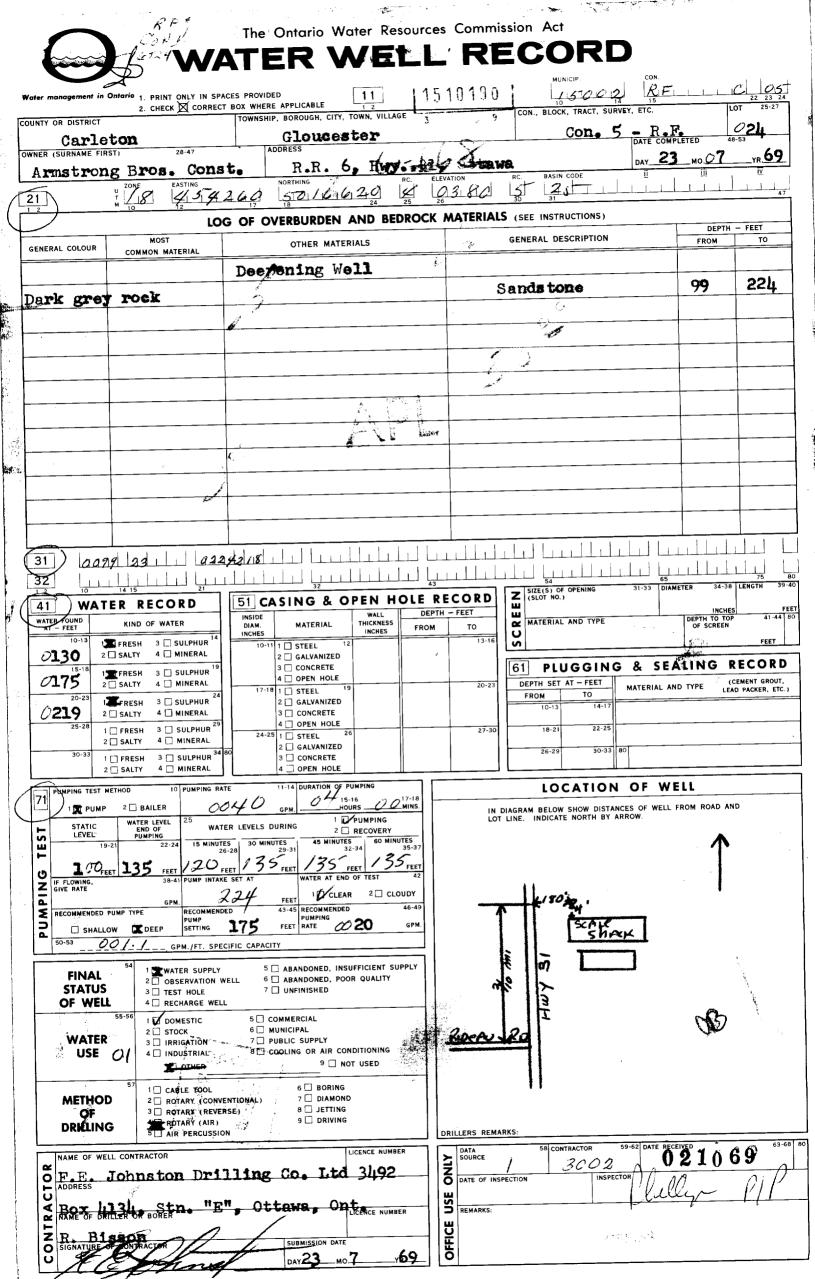
	with party 3			
AM II I			Water	Record
Well Log Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	sulphur)
Trown sand fing sand	8	3	20	fresh
Lord and grown	60	\$ 29		
grey limestone	7-7-			

 Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.

To matacket Hack #31

Form 7 15M-60-4138



OWRC COPY



The Ontario Water Resources Commission Act

WATER WELL RECORD

	ntario 1. PRINT ONLY IN SP 2. CHECK ⊠ CORREC	T BOX WHERE APPLICABLE TOWNSHIP, BOROUGH, CIT	1 1 2	151	0717 1500 CON., BROCK, TRACT, SUF	RVEX ETC.	22 23 24 LOT 25-27
CARLE	DON		COSTEX	3	9	DATE COMPLETED	023
		3/	HIGHWI	ay L	FIT RUM RC. BASIN CODE	DAY /5 MO. #	OUT YR 70
1 1 2	10 12	1 NG	9 20 4	0,342	4 25 I		47
		G OF OVERBURDEN	AND BEDRO	CK MATERIA		DEP	TH - FEET
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MAT	ERIALS		GENERAL DESCRIPTION	FROM	то
BROWN	RUBBLE	(FILLED IN	LOT 7	O HIGHWI	AY GRADE)	0	6
GREY	LIMESTON					6	52
31 QQQQQ 32 10 10 10 10 10 10 10 1		32/5			SIZE(S) OF OPENING (SLOT NO.)	65 31-33 DIAMETER 34-	75 80 38 LENGTH 39-40
WATER FOUND AT - FEET	KIND OF WATER FRESH 3 SULPHUR 14 SALTY 4 MINERAL	INSIDE DIAM. MATERIAL INCHES MATERIAL 21 GALVANIZED	THICKNESS FRO	ртн – FEET ом то 0020	MATERIAL AND TYPE	DEPTH TO OF SCREE	TOP 41-44 80
20-23 1	FRESH 3 SULPHUR 24 FRESH 3 SULPHUR 24 SALTY 4 MINERAL FRESH 3 SULPHUR 29 SALTY 4 MINERAL FRESH 3 SULPHUR 348 FRESH 3 SULPHUR 348 FRESH 3 SULPHUR 348 FRESH 4 MINERAL	3 CONCRETE 4 OPEN HOLE 17-18 1 STEEL 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE 24-25 STEEL	./88 O	20-23 20-23 20-52 27-30	61 PLUGGING DEPTH SET AT - FEET FROM TO 10-13 14-17 18-21 22-25 26-29 30-33 6		RECORD (CEMENT GROUT, LEAD PACKER, ETC.)
PUMPING TEST METH 1 PUMP STATIC LEVEL 19-21 V FEET RECOMMENDED PUM STATIC LEVEL 19-21 PALER WATER LEVEL END OF PUMPING 22-24 15 MINUTE 26 38-41 PUMP INTAKE GPM.	R LEVELS DURING 1 AS MINUTES 29-31 FEET AT 3 O FEET 45 MINUTES 45 MINUTES 45 MINUTES 45 MINUTES 45 MINUTES 47 MATER AT EN 47 MATER AT EN 48 PEDMPING PET PUMPING TRECOVERY SO MINUTES 33-34 FEET D OF TEST AZ CLOUDY	IN I	DIAGRAM BELOW SHOW DISTANGLINE. INDICATE NORTH BY AF	CES OF WELL FROM ROAD	AND		
WATER	water supply by observation we constructed by the supply construction of the supply construction of the supply constructed by the supply constructed	7 UNFINISHED 5 COMMERCIAL 6 MUNICIPAL 7 PUBLIC SUPPLY	OR QUALITY	<u> </u>			/
METHOD OF DRILLING	4 INDUSTRIAL OTHER 57 ACABLE TOOL CONVEN 3 ROTARY (REVERS 4 ROTARY (AIR) 5 AIR PERCUSSION	6 BORING (TIONAL) 7 DIAMON (SE) 8 JETTING 9 DRIVING	OT USED	LOT			
NAME OF WELL CONTROL OF CONTROL O	FISHER OLOUGHN FISHER OR BORER MOLOUGH ONTHACTOR	NE SUBMISSION DATE	LICENCE NUMBER 3701 LICENCE NUMBER	DATA SOURCE DATE OF INSP REMARKS:	3701	9-62 DATE RECEIVED 23027	P W I

The Ontario Water Resources Commission Act
WATER WELL RECORD

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NTY OR DISTRICT	Litaren Cana	TOWNSHIP, BOROUGH, CITY, TOWN,	VILLAGE	3	R.F.		1	5 21
g Mun U	ttawa-Carleto		- 27 04	-t		DATE COMPLET	MO 06	53 VR
		6, HWY	RC. EL	EVATION A	Ontario	DAY 20	111	ΙV
2	" <u> </u>	1/655/2		26	30 31			
		G OF OVERBURDEN AND	BEDROCK	MATERIAL			DEPTH -	
ENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	·		GENERAL DESCRIPTION		FROM	то
Brown	sand	loam			dy loam		0	1
grey	limestone			med	lium		1	
								<u> </u>
		,					Ö	
11) 1200	1162802 1 1007	22/5					1111	
2	14 15	32	43		54	65 31-33 DIAMETEI	R 34-38	75 LENGTH
WAT	ER RECORD	51 CASING & OPEN		ECORD H - FEET	SIZE(S) OF OPENING (SLOT NO.)		INCHES	
AT FEET	KIND OF WATER	DIAM. MATERIAL THICK	NESS FROM	TO 13-16	MATERIAL AND TYPE	D	EPTH TO TOP OF SCREEN	41-
243 ²¹	SALTY 4 MINERAL	10-11 1 STEEL 12 2 GALVANIZED 3 CONCRETE	L88 o	0020	61 PLUGGING	O CEAL	ING DI	FEET
	FRESH 3 SULPHUR SALTY 4 MINERAL	3 CONCRETE 4 OPEN HOLE 17-18 1 STEEL 19	LBB O	0072	DEPTH SET AT - FEET	MATERIAL AND T	VDE (CE	MENT GRO
	RESH 3 SULPHUR 24	6 2 GALVANIZED 3 CONCRETE	20	72	FROM TO 10-13 14-17		LEAD	, HOREN, C
	RESH 3 SULPHUR 29 SALTY 4 MINERAL	4 OPEN HOLE 24-25 1 STEEL 26		27-30	18-21 22-25			
	☐ FRESH 3 ☐ SULPHUR 34	2 GALVANIZED 3 CONCRETE 4 OPEN HOLE			26-29 30-33 80			
PUMPING TEST M		E 11-14 DURATION OF PUMPING			LOCATION	OF WEL	L	
PUMP	2 □ BAILER 00	GPM. DIMBI	00 17-18 MINS.	IN LOT	DIAGRAM BELOW SHOW DISTANC LINE. INDICATE NORTH BY AR	ES OF WELL FROI	M ROAD AND	
STATIC LEVEL 019	END OF WATE	R LEVELS DURING PRECON	/ERY				/	14
1	78 FEET 21 7 1	19 2 18 11 T	0/9 ³⁵⁻³⁷				ı	·
IF FLOWING, GIVE RATE	38-41 PUMP INTAKE	CLEAR 2	T 42					
RECOMMENDED P	UMP TYPE RECOMMENDE	D 43-45 RECOMMENDED ·	46-49		week -	(-95'-)	T.	
50-53	DOO: 3 GPM./FT. SPEC		GPM.			y Smi	} 3	
FINAL	54 WATER SUPPLY	5 ABANDONED, INSUFFICIE			15 m	,510	I	
STATUS OF WELL	2 OBSERVATION W 3 TEST HOLE	7 UNFINISHED	LITY	R	provision Ro			
OF WELL	55-56 DOMESTIC	5 COMMERCIAL			26			
WATER USE	2 ☐ STOCK 3 ☐ IRRIGATION	6 MUNICIPAL 7 PUBLIC SUPPLY 8 COOLING OR AIR CONDITION	ING					
USE	4 INDUSTRIAL OTHER	9 - NOT USE						
METHOD	57 1 CABLE TOOL 2 ROTARY (CONVE	6 ☐ BORING NTIONAL) 7 ☐ DIAMOND			e de la companya de l		l	
OF DRILLING	3 D ROTARY (REVER ROTARY (AIR)	8 ☐ JETTING 9 ☐ DRIVING			A Commence of the Commence of	ļ	l	
	5 ☐ AIR PERCUSSION	LICENCE		DATA		9-62 DATE RECEIVED		
	contractor Dril	i i	002	SOURCE	1 3002	2.7	0771	01
ADDRESS			11	O DATE OF INS	PECTION INSPECTO	Km	<u>.</u> (
NAME OF DRI	LLER OR BORER	Ottawa Ontar	NUMBER	REMARKS:	inc.			7
Z R.W.	Remuick	SUBMISSION DATE		OFFICE	,	JSS.34		
ŭ a	HX/ self	Z 22 40 7	yr. 71	\circ	{	2.75 () 2.4 () 13		



The Ontario Water Resources Commission Act
WATER WELL RECORD

316/50 ·

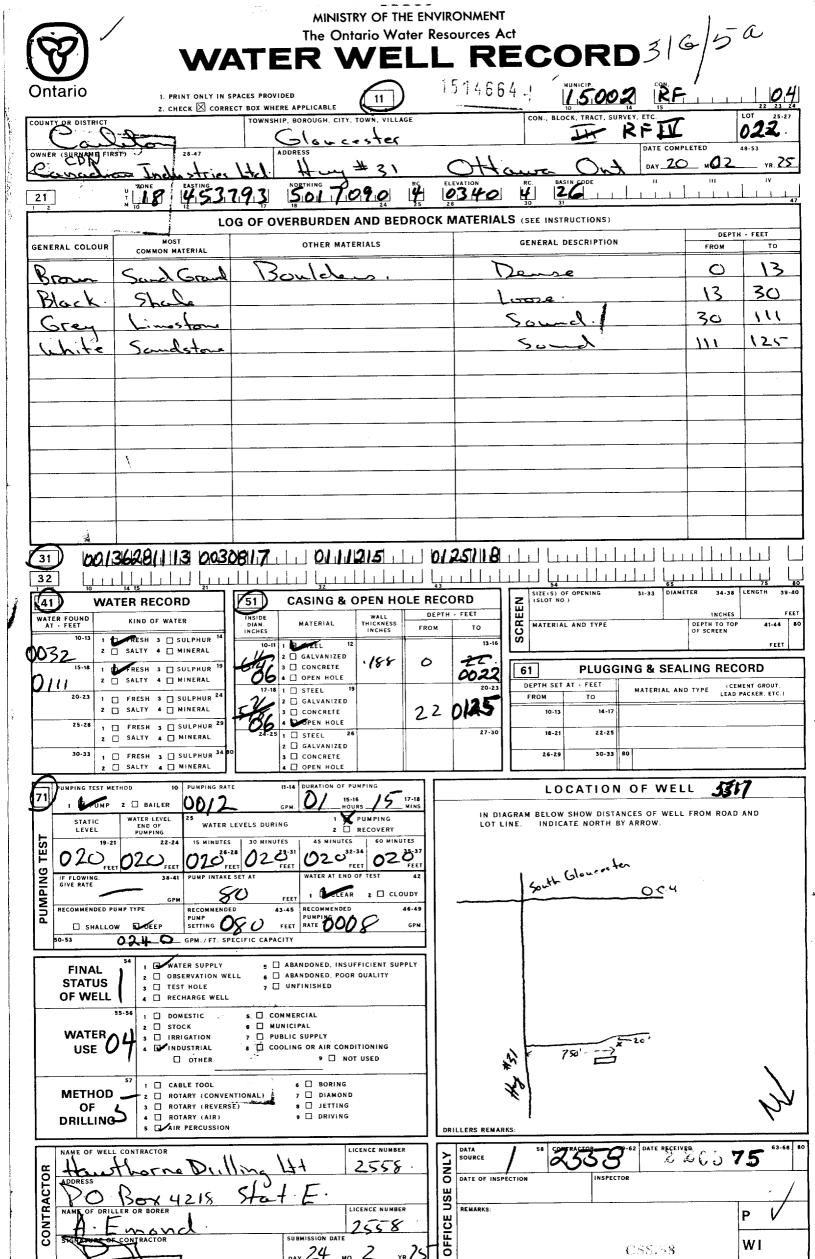
	Water management in C	Ontario 1. PRINT ONLY IN S 2. CHECK 🔀 CORRE	PACES PROVIDED CT BOX WHERE APPLICABLE TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	ie į	151226		CON. ACT, SURVEY, ETC.	F	22 23 24 LOT 25-27
	Carl	eton	Gloucester			5 /	F DATE COI	S He	1£022 8-53
					EVATION	RC. BASIN CODE	DAY_ 2	<u>4</u> мо. Моч	<u>VR. 72</u>
Į				25 2	0336	30 31			47
•	GENERAL COLOUR	MOST	G OF OVERBURDEN AND BED OTHER MATERIALS	ROCK		(SEE INSTRUCTION OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF T		DEPTH	- FEET
		COMMON MATERIAL						FROM	то
	Brown	Clay	Sand & Stones		1	Clay & Si ray limes		3	<u>48</u>

							··· · · · ·		
							0.00		
	31) 00030	6 ast 28/12 lao4	a.2/15t			 	+	<u> </u>	<u> </u>
٦	32	4 15 21	32	43		54	65		75 80
	WATER JOUND	R RECORD	51 CASING & OPEN HO	LE RE		·	31-33 DIAMI	ETER 34-38 L	ENGTH 39-40 FEET
	10:13	RESH 3 SULPHUR	INCHES HICKNESS INCHES 10-11 STEEL 12	FROM	TO 13-16	MATERIAL AND TY	PE	DEPTH TO TOP OF SCREEN	41-44 80
4	15-18 1 FF	RESH 3 SULPHUR 19	06 2	ን . ሩ የት	0012		NG & SEA	LING RE	CORD
*.	20-23 1 FF	RESH 3 SULPHUR	17-18 1 STEEL 19 2 GALVANIZED	J+0	20-23	DEPTH SET AT - FEE	MATERIAL AND		MENT GROUT, PACKER, ETC.)
	25-28 1		3 ☐ CONCRETE 4 ☐ OPEN HOLE 24-25 1 ☐ STEEL 26		0048		-25		
-,	30-33 1 F	RESH 3 SULPHUR 34 80	2 GALVANIZED 3 CONCRETE			26-29 30	-33 80		
1	MIMPING TEST METHO		4 OPEN HOLE	7 [LOCATIO	ON OF WE		
V	STATIC	BAILER WATER LEVEL 25 WATER	1 PUMPING	S.	LOT LINE		STANCES OF WELL FR		
	LEVEL 19-21	22-24 15 MINUTES	2 RECOVERY	· 	20 LOI LINE				
	Z IF FLOWING,	1.007	FEET FEET FEET		21 7		/~		
	RECOMMENDED PUMP	GPM. TYPE RECOMMENDED	8 FEET CLEAR 2 CLOUDY 43-45 RECOMMENDED 46-4		-515		/		
	SHALLOW 50-53	DEEP SETTING C	30 FEET PUMPING OOO8 GPN		i June	11.04	102		
[FINAL 54	WATER SUPPLY	5 🗆 ABANDONED, INSUFFICIENT SUPPLY	<u> </u>	•	Hwy-	x	/	
	STATUS OF WELL	OBSERVATION WELL TEST HOLE CHAPTER WELL	6 ABANDONED, POOR QUALITY 7 UNFINISHED	1		3/	1/20	4'	
Ì	55-56		5 COMMERCIAL	1			_ _	LOT LI	NE_
	WATER USE O	3 IRRIGATION 4 INDUSTRIAL	7 DPUBLIC SUPPLY 8 COOLING OR AIR CONDITIONING				- Erozan	_ Eu	-
	57	1 CABLE TOOL	9	-	:				
	METHOD OF	2 ROTARY (CONVENTI 3 ROTARY (REVERSE)	ONAL) 7 🗌 DIAMOND 8 🗋 JETTING		ļ				
إ	DRILLING	ROTARY (AIR)	9 DRIVING	DRILL	LERS REMARKS:				
1	NAME OF WELL CON	tractor hnston Dril	ling Co. 3002	NE S	DATA SOURCE DATE OF INSPECTION	58 CONTRACTOR 3002	59-62 DATE RECEIVE	50173	63-68 80
	ADDRESS PA		HE" Ottawa, Ont		ATE OF INSPECTION	INSI	PECTOR (
	E OF DRILLER C	OR BORER	LICENCE NUMBER	1 1	REMARKS:	1	3	Р	K
			SURMISSION DATE DAY MO YR.	OFFICE			For the second	W	' I
	AC C	OPY	1	للا					Δ

MINISTRY OF THE ENVIRONMENT The Ontario Water Resources Act TER WELL RECORD 1513436 - Culita LETRIM Ottan GLOUCESTER ONT UNITED CO - OF OF CATARIO R. R. #6 OFFAWA. ONTARIO. BASIN CODE 4.53.850 " [ZONE LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS) GENERAL DESCRIPTION OTHER MATERIALS GENERAL COLOUR FROM то Soft 0 4 Top Seil Brown 4 12 Brown Soil Boulder Hard 12 Soft Porous 16 Grey Limestone Clay 50 Medium Hard 16 White Limestone Grey Limestone 1000HG92 11 DOV CHG122/31 1 DOV 6/21/5/05 1 Dasid/V5 11 11 11 11 11 11 11 11 11 31 SIZE(S) OF OPENING CASING & OPEN HOLE RECORD WATER RECORD [51] (41) WATER FOUND KIND OF WATER WALL THICKNESS MATERIAL FROM 1 T FRESH 3 SULPHUR 2 SALTY 4 MINERAL 00 48 22 13-16 STEEL 2 GALVANIZED -188 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL 3 CONCRETE 61 **PLUGGING & SEALING RECORD** 0022 4 OPEN HOLE DEPTH SET AT - FEET 1 | STEEL 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL 2 GALVANIZED 3 T CONCRETE 4 OPEN HOLE 1 | FRESH 3 | SULPHUR 4 MINERAL 1 🗆 STEEL Z SALTY 2 GALVANIZED 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL 3 CONCRETE LOCATION OF WELL 1 | PUMP IN DIAGRAM BELOW SHOW LISTANCES OF WELL FROM BOAD AND LOT LINE. INDICATE NORTH BY ARROW. PUMPING 2 | RECOVER WATER LEVEL END OF PUMPING WATER LEVELS DURING 15 MINUTES 30 MINUTES 29-22-24 MINUTES (30 FEET $030_{\frac{\text{feet}}{}}$ **30** FEET 014 0.35 m 2 CLOUDY RECOMMENDED PUMP SETTING 30 RECOMMENDED PUMP TYPE FEET RATE UOS PARKING 5 ABANDONED, INSUFFICIENT SUPPLY WATER SUPPLY FINAL OBSERVATION WELL 6 ABANDONED, POOR QUALITY LOT. **STATUS** 3 TEST HOLE 7 UNFINISHED OF WELL 1 DOMESTIC 2 STOCK 6 MUNICIPAL WATER IRRIGATION PUBLIC SUPPLY USE ().\ 8 COOLING OR AIR CONDITIONING 4 | INDUSTRIAL ☐ OTHER 9 🗌 NOT USED CABLE TOOL 6 T BORING **METHOD** ROTARY (CONVENTIONAL) 7 DIAMOND 2 🔲 OF 3 🗍 ROTARY (REVERSE) 8 | JETTING **DRILLING** 5 AIR PERCUSSION DATA SOURCE ONLY 2557 28 HAWTHORNE DRILLING LIMITED ... DATE OF INSPECTIO OFFICE USE Box 4218 STATION FIET OTTAWA ONTARIO REMARKS (188.138

MINISTRY OF THE ENVIRONMENT COPY

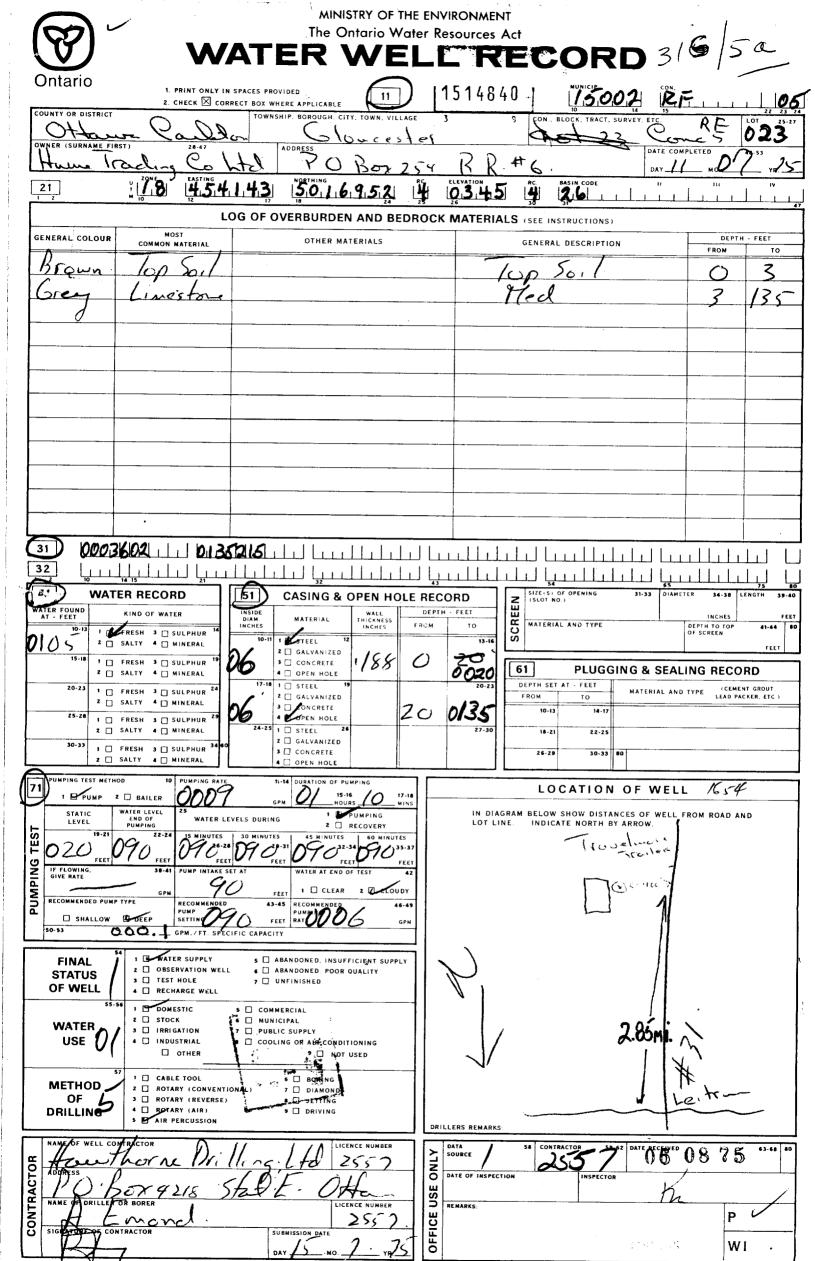
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The Ontario Water Resources Act WATER WELL RECORD

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OWNER (SURNAME FIL	RST) 28 47	TOWNSHIP, BOROUGH, CITY, TOWN. VILLA	AGE		CON., BLOCK, TRACT, SUR	VEY, ETC.	LETED	22 ·
Canad	an Industries	Hel. Huy # 31	. (HC	aura Ont	DAY_2C		vn.25
21	ZONE EASTING	NORTHING 0	RC EL	EVATION	RC BASIN CODE	<u> </u>	. 1
	LC	OG OF OVERBURDEN AND BE	DROCK M	MATERIA	LS (SEE INSTRUCTIONS)			
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS			GENERAL DESCRIPTION		FROM	TO TO
Brown	Sand Grand	Boulders.		7	Dense		<u> </u>	13
Black.	Shala			<u> </u>	Loose.		_13_	30
Grey	Limostone				Sound		<u>30</u>	111
white	Sandstone				کی سیک		_///	125
	1		-					
31							1]] 1	
32	14 15 21		سبها لسا		54 SIZE(S) OF OPENING	55 31-33 DIAME	ER 34-38	LENGTH 39
WATER FOUND	TER RECORD	51 CASING & OPEN HO	DEPTH		Z (SLOT NO)		INCHES	F
10-13	FRESH 3 SULPHUR 14	INCHES INCHES	FROM	TO 13 -16	MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN	41-44 FEET
15-18 1 (TRESH 3 SULPHUR 19	64 GALVANIZED 188	0	22.	61 PLUGGI	NG & SEAL	ING REC	ORD
20-23 1	SALTY 4 MINERAL FRESH 3 SULPHUR 24	17-18 1		20-23	DEPTH SET AT - FEET FROM TO	MATERIAL AND		PACKER, ETC.)
l	SALTY 4 MINERAL FRESH 3 SULPHUR 29	5/8 3 CONCRETE	22	125	10:13 14:17			
	☐ SALTY 4 ☐ MINERAL ☐ FRESH 3 ☐ SULPHUR 34 11	24-25 I STEEL 26 2 GALVANIZED 3 CONCRETE		27.30	18-21 22-25 26-29 30-33 8	10	· · · · · · · · · · · · · · · · · · ·	
	SALTY 4 MINERAL ETHOD 10 PUMPING RAT	4 G OPEN HOLE						
71 PUMPING TEST MI	2 D BAILER 12		17-18 MINS		LOCATION			
STATIC LEVEL	PUMPING	LEVELS DURING 1 PUMPING 2 RECOVERY 1 30 MINUTES 45 MINUTES 60 MINU			AGRAM BELOW SHOW DISTAN LINE. INDICATE NORTH BY		FHOM ROAD	ANU .e.
20,	20 10	$\frac{28}{2}$ $\frac{20^{31}}{2}$ $\frac{20^{32-34}}{2}$ $\frac{20^{32-34}}{2}$	Ö37		۱.	_		
IF FLOWING. GIVE RATE RECOMMENDED P	38-41 PUMP INTAKE	SET AT WATER AT END OF TEST	42		South Gloureste	004		
RECOMMENDED P	UMP TYPE RECOMMENDE		46-49					
SO-53		ECIFIC CAPACITY	GPM.					
FINAL STATUS OF WELL	1 WATER SUPPLY 2 OBSERVATION WE 3 TEST HOLE	5 ABANDONED. INSUFFICIENT SUP 6 ABANDONED. POOR QUALITY 7 UNFINISHED	PLY					
	55-56 DOMESTIC	S COMMERCIAL 6 MUNICIPAL						
WATER USE	3 ☐ IRRIGATION 4	7 DUBLIC SUPPLY B COOLING OR AIR CONDITIONING 9 NOT USED			AN 180' -	-20'		`
METHOD OF DRILLING	3 ROTARY (REVERS		DR	ILLERS REMA	Ap)			R
1 1 1	L CONTRACTOR	LICENCE NUMBER		DATA SOURCE		DATE RECEIVE کے رک		61-6
ADDRESS	Thorne Dil	1mg 4 2558	- ē	DATE OF INS				
NAME OF DRIE	SOX 4218	Statiti		REMARKS:		1		P
DDRESS DAMA OF DRILL	CONTRACTOR	SUBMISSION DATE	25					
		DAY 24 MO. 2	<u>، 2</u> 5 ا			11:30 . HŠ		WI
MINISTR	RY OF THE ENVI	RONMENT COPY					FOR	M 7 MOE 0



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FORM 7 MOE 07-091

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MINISTRY OF THE ENVIRONMENT The Ontario Water Resources Act WATER WELL RECORD

Ontario •	1. PRINT ONLY IN 2. CHECK 🗵 CORF	RECT BOX WHERE	APPLICABLE			1 6 05 2	-		2		, <u>o</u>
COUNTY OR DISTRICT	_		BOROUGH, CITY,	TOWN, VILL	AGE 3	9	CON	BLOCK, TRACT. S	URYEY, ETC	T	022
Carleto	ST) 28-47	ADD	rester ORESS				,-	· 3		TE COMPLETED	48-53 7 YR 77
LIETCO	Investors Corp		934 5a		res.	Dttawa. ぶるでん	Ont	K2B 5H7		.y 13 Ø	
	M 10 12	17	18	24	DDOCK	26	1 7	31			41
GENERAL COLOUR	MOST	JG OF OVE	OTHER MATE			WIATERIA		INSTRUCTIONS)	N		DEPTH - FEET
	COMMON MATERIAL							THE BESCHIT TO		FRO	м то
brown	sand	cla	y & boul	Lders		fill	-			0	7
black	muck		ulders			soft				7	26
grey	hardpan limestone	- Du	ATGGT#			pack medi				2	
grey	sandstons					hard				4	
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32	14 15							54	لبلنا	65	75 80
WATER FOUND	ER RECORD	(51) C	ASING & O	n		ORD H - FEET	Size (SLO	SI OF OPENING T NO. I	31-33		1-38 LENGTH 39-40
AT - FEET	KIND OF WATER FRESH 3 SULPHUR 14	DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	FROM	то		ERIAL AND TYPE		DEPTH TO OF SCREE	TOP 41-44 80
13	SALTY 4 MINERAL FRESH 3 SULPHUR 19	6 1 2	STEEL 12 GALVANIZED CONCRETE	188	0	00 28 ¹³⁻¹⁶					FEET
2 🗆	SALTY 4 MINERAL	06 ↓3 17-18 1 □	-005N-H000		28	20.23		SET AT - FEET			(CEMENT GROUT,
2 🗆	FRESH 3 SULPHUR 24 SALTY 4 MINERAL	06 30	GALVANIZED CONCRETE OPEN HOLE			0178	FROM	0-13 14-17			EAD PACKER, ETC.)
20	FRESH 3 SULPHUR 29 SALTY 4 MINERAL	24-25 1 2	STEEL 26			27-30	1	8-21 22-25			
1 '	FRESH 3 SULPHUR 34 60 SALTY 4 MINERAL		CONCRETE OPEN HOLE				26	5-29 30-33	80		
71 JUMPING TEST METH	10 PUMPING RATE 2 BAILER 001		DURATION OF PUM		7-18		L	OCATION	OF V	VELL	
STATIC LEVEL	WATER LEVEL 25 END OF WATER LI	GPM GPM EVELS DURING	1 火 P	UMPING ECOVERY	IINS	IN DIA LOT L		OW SHOW DISTA		WELL FROM RO	IAD AND
19-21	PUNPING 22-24 15 MINUTES 26-2	30 MINUTES 8 29-31	45 MINUTES 32-3	60 MINUT	ES 5-37		R	DEAU	RI	۵.	
	065 FEET 065 FEE		P65 FEE		42 42						
IF FLOWING. GIVE RATE RECOMMENDED PUMP	GPM RECOMMENDED	FEET 43-45	1 CLEAR	2 CLOU	DY 6-49	_					
SHALLOW	DEEP SETTING	Ø 75 FEET	PUMPING RATE	_	БРМ					. 7%	
	54 WATER SUPPLY	CIFIC CAPACITY	WD0W50 ANGUS		-	4				3	_
FINAL STATUS	2 OBSERVATION WEL		NDONED, INSUFF NDONED, POOR Q INISHED			\bigvee				, -	# 3
OF WELL	4 RECHARGE WELL	5 COMMER	CIAL		$-\parallel$	•			4 /	430,	\$ J
WATER	2 STOCK 3 IRRIGATION	6 MUNICIP 7 PUBLIC:	PAL Supply						. ;	1	乏
USE 01	4 INDUSTRIAL OTHER	8 COOLING	OR AIR CONDITI					·γς.			
METHOD	S7 : CABLE TOOL 2 ROTARY (CONVENT		BORING						ì		
OF #	5 ROTARY (REVERSE)	, a	☐ JETTING ☐ DRIVING								
NAME OF WELL CO	S AIR PERCUSSION		I. re-	NCE NUMBER		ILLERS REMARK		<u></u>			-
1	Water Supply	Ltd.	1	558	_ \ \ \ \ \ \ \ \ \ \ \ \ \	DATA SOURCE	1	5 1558	y-62 DATE R	ECE IVO 80	877"
How Agn					SE 0	DATE OF INSPE	CTION	INSPECTO	OR	Kn.	
Capital NAME OF DRILLER NAME OF DRILLER	Stittsville,	/	LICE	NCE NUMBER	7 >	REMARKS:					Р
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FORM NO. 0506-4-77

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WATER	WEL	L RE	C	OR	C

Environment 1517076 15002 TOWNSHIP, BOROUGH, CITY, TOWN COUNTY OR DISTRICT RF Con. cester DATE COMPLETED Hey 31 Ottawa, Ontario 26 516599 4 5360 4 LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS) DEPTH - FEET GENERAL DESCRIPTION MOST COMMON MATERIAL OTHER MATERIALS GENERAL COLOUR Brown sandy clay with 0. 6.5 Sand pebbles Clay Brown 6.5 92 Grey sandstone Grey and whitich Sandstone Grey 921 106. EXECUTE XX Med. grey limestone Limestone Grey 185' 106' Grey sandstone Grey and whitigh Sandstone Grey 185 187 Mud seam Sand stones Clay Brown 245 187' Grey sandstone Grey and whitigh Sandstone Grey . . 00076052812 0092218 0006215 0092818 00876051803 0245218 51 CASING & OPEN HOLE RECORD SCREEN 41 **WATER RECORD** DEPTH - FEET ATER FOUND KIND OF WATER WALL THICKNESS INCHES MATERIAL FRESH 3 SULPHUR
2 SALTY 4 MINERAL 01 **5167°** GALVANIZED 61 3 CONCRETE 0. @150. FRESH 3 SULPHUR 1.25 .188 **PLUGGING & SEALING RECORD** 4 OPEN HOLE SALTY -4 | MINERAL - FEET 7198* steel coment and FRESH 3 CONCRETE Grouting in pla 216 4 | MINERAL SALTY FRESH 3 SULPHUR
2 SALTY 4 MINERAL 4 🗍 OPEN HOLE D238* 24-25 1 - STEEL 22.25 2 GALVANIZED 30-33 1 | FRESH 3 | SULPHUR
2 | SALTY 4 | MINERAL 4 - OPEN HOLE LOCATION OF WELL 71 15-16 HOURS 00 PUMP 2 D BAILER 0010 IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE INDICATE NORTH BY ARROW. WATER LEVEL END OF PUMPING WATER LEVELS DURING 049 049 049 049 049 49.23:49.87: 49.42: 49.35: EET 49.23: 1 CLEAR RECOMMENDED
PUMP
SETTING 080 43-45 RECOMMENDED PUMPING PEET RATE "*0*0 10 SHALLOW X DEEP 210' X 125' I WATER SUPPLY

OBSERVATION WELL I 5 ABANDONED, INSUFFICIENT SUPPLY FINAL 6 ABANDONED, POOR QUALITY **STATUS** 7 UNFINISHED 3 TEST HOLE OF WELL 4 🔲 RECHARGE WELL I DOMESTIC COMMERCIAL MUNICIPAL 2 STOCK **WATER** 3 | IRRIGATION T PUBLIC SUPPLY USE OS 4 | INDUSTRIAL COOLING OR AIR CONDITIONING 9 🗆 NOT USED OTHER to Ottawa 6 D BORING CABLE TOOL ROTARY (CONVENTIONAL)

ROTARY (REVERSE) METHOD 7 DIAMOND OF DRILLING 2 4 | ROTARY (AIR)
5 | AIR PERCUSSION 9 / DRIVING DRILLERS REMARKS 4006 Olympic Drilling Co. Ltd. 4006 DATE OF INSPECTION INSPECTOR USE ox 9180 Terminal "1" Ottawa, REMARKS SE 4006 C85.58

DAY 14

MINISTRY OF THE ENVIRONMENT COPY



The Ontario Water Resources Act WATER WELL RECORD

County or Distric	/ 1 I	Township/Borough/Cit Address	y/Town/Village		Con block tract	à	23
		4/E	37 A	bion Rd			onth ye
2	M 10	12 17 18	24 25	FIC Elevation FIC	Basin Code	11 111	iv
	LC	OG OF OVERBURDEN AND BE	DROCK MAT	ERIALS (see instru	ctions)	, De-	Ab 6A
General colour	Most common material	Other materials		Gener	ral description	From	th - feet To
Brown	sond	store				0	42
Black	shale					42	51
Grey_	Limestone	_			12 Tr 12	51	12
Grey	Lime stone	w sordstone sandstone	layers			123	168
White	sondstene	sandstone	e			168	aa
Soun	Grande	grance				200	210
		J					
1							
	14 15 21	32	43	54		65	75
ater found		CASING & OPEN HOI side Wall am Material thickness	Depth - f	(01-11)	f opening 31-33 Dia	ameter 34-38 Length	
- feet	☐ Fresh ³ ☐ Sulphur ¹⁴ inc	thes inches	From	To Material Material	and type	Depth at top o	fee f screen 3
	Salty 6 Gas ☐ Fresh 3 ☐ Sulphur 19	2 Galvanized 3 Concrete 4 X Open hole		42 5			feet
123 2	☐ Salty 6 ☐ Gas	5 Plastic 17-18 1 Steel 19		20-23 61	PLUGGING & SI	EALING RECORE)
147 2	☐ Fresh 3 ☐ Sulphur 24 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2 Galvanized 3 Concrete	+2	Down the control		☐ Abandonme	
ال المحافظ الله	☐ Fresh ³ ☐ Sulphur ²⁹	5 ☐ Plastic	, &	42 Depth set at From	To Material and to	ype (Cement grout, ber	
	Salty 6 Minerals 6 Gas Gas Go Fresh 3 Sulphur 34 60	24-25 1 Steel 28 2 Galvanized 3 Concrete	42	18-21	22-25	val 10 b	ays
2 [☐ Salty 6 ☐ Gas	U 4 ☐ Open hole 5 ☐ Plastic	42 0	210 28-29	30-33 80		
Pumping test m		11-14 Duration of pumping 17-18			CATION OF WELL		4
	Water level 25	GPM Hours Mins	- - in	diagram below show	distances of well fr		ne. N
19-21 e	end of pumping vvaler levels during 22-24 15 minutes 30 mir	outes 45 minutes 60 minutes	In	ndicate north by arrow	·	-	
feet	feet feet	29-31 32-34 35-37 feet feet feet) Practice Track	
feet If flowing give ra		Water at end of test ⁴² feet □ Clear □ Cloudy				Track	
Recommended		43-45 Recommended 46-49 pump rate	1	1/2 mile /	Canco		
☐ Shallow 50-53	☐ Deep	feet GPM		[- fence		
NAL STATUS			<u> </u>				
 ¹ ☐ Water sup ² ☐ Observati ³ ☐ Test hole 	ion well 6 🗍 Abandoned, poor	ficient supply 9					
4 ☐ Recharge		,			, \		
ATER USE	55-56 Commercial	9 □ Not used					1
² ☐ Stock ³ ☐ Irrigation	6 ☐ Municipal 7 ☐ Public supply	10 🗆 Other mondering					
4 🗌 Industrial	8 G Cooling & air con	ditioning					6
ETHOD OF C	CONSTRUCTION 57 DI 5 Air percussion	9 🛘 Driving		Baca	siderood		
² ☐ Rotary (co ³ ☐ Rotary (re	onventional) 6 Boring everse) 7 Diamond	Digging Other				2017	กรโ
4 Rotary (ai	ir)					LO11	
ame of Well Contra	actor	Well Contractor's Licence No.	Data source	58 Contracctor		ate received	63-68 80
OLYM,	pic Wrilling Co 20 Scrivers Dr	UK 4006	O Date of i	nspection (0 6	AUG 0 9 19	99
230	10 Scrivers Dr	Hefealfe	1 і ш		·F •		
ame of Well Techn	, ,	Well Technician's Licence No.	Remarks	<u> </u>		CSS.ES0	
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ignature of Technic	cian/Confactor	Submission date	NIN NIN NIN NIN NIN NIN NIN NIN NIN NIN				

The Ontario Water Resources Act WATER WELL RECORD

0506 (07/94) Front Form 9

Print only in spaces provided. 1530654 Mark correct box with a checkmark, where applicable. 11 15002 RF County or District Township/Borough/City/Town/Village Con block tract survey, etc. Glouceste Cadeton 1111 Albier completed 18 للللبا LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions) Depth - feet General colour Other materials Most common material General description From To stone 5and Drown Brown stone 20 sand 0 NWID HW25 -2 PYC Bensed Benseal Phy 10' native backfull Native padul 40 3 Benseal 1 benseal Clifer Flike otone Stone 32 WATER RECORD 51 **CASING & OPEN HOLE RECORD** 41 Inside diam inches Wall thickness inches Water found at - feet Kind of water From То Material and type Depth at top of screen 30 NUID 1 Steel 2 Galvanized 3 Concrete 4 Deopen hole 5 Plastic 54 0 **PLUGGING & SEALING RECORD** 1 Galvanized
2 Galvanized
3 Concrete ¹ ☐ Fresh ³ ☐ Sulphur ² ☐ Salty 6 ☐ Gas ☐ Abandonment ☐ Annular space 20 From ¹ ☐ Fresh ¹ ☐ Fresh 3 ☐ Sulphur 2 ☐ Salty 5 ☐ Gas Duration of pumping Pumping test method Pumping rate **LOCATION OF WELL** ☐ Pump 2 ☐ Bailer In diagram below show distances of well from road and lot line. Water level Static level Water levels during 1 Pumping 2 Recovery end of pumping Indicate north by arrow. 45 minutes 32-34 30 minutes 29-31 19-21 22-24 15 minutes 26-26 175' feet If flowing give rate Pump intake set at Water at end of test ☐ Cloudy 46-49 GPM ☐ Clear Recommended pump setting Recommended pump rate Recommended pump type ☐ Shallow ☐ Deep Practice **GPM** tracel FINAL STATUS OF WELL Water supply
 Observation well
 Test hole
 Recharge well WATER USE 1 Domestic
2 Stock
3 Irrigation
4 Industrial 5 SK Commercial
6 Municipal
7 Public supply
8 Cooling & air conditioning s ☐ Not used METHOD OF CONSTRUCTION 9 Driving BARA SIde (Oac) Digging
Other... 20170**4** ympic Drilling G. Ltd ONLY 4006 4006 AUG 0 9 1999 Date of inspection USE MINISTRY Remarks 10-327 CSS.ES0

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The Ontario Water Resources Act WATER WELL RECORD

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Print only in spaces provided. 1530655 Mark correct box with a checkmark, where applicable. 11 15002 County or District Township/Borough/City/Town/Village Con block tract survey, etc. Lot Carleten SHa 6 buceste 9 Date completed Basin Code LLLL 1_1_ LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions) Depth - feet General colour Other materials Most common material General description From To HW# 2 58 **plonu** sand. sterie Benseal ص Phony notive. moterial 27 Bensealping 30 FILE Scione 53 1 32 **CASING & OPEN HOLE RECORD** 41 WATER RECORD 51 Water found at - feet Inside Wall thickness diam inches Material and type Depth at top of screen Steel
Galvanized
Concrete
Copen hole
Plastic 6 50 **PLUGGING & SEALING RECORD** Steel

Galvanized

Concrete

Open hole

Plastic ☐ Sulphur ☐ Minerals ☐ Gas 20-23 ☐ Abandonment ¹ ☐ Fresh Depth set at - feet ² Galty From 10-13 Material and type (Cement grout, bentonite, etc.) ☐ Sulphur ☐ Minerals ☐ Gas ¹ 🛘 Fresh 1 Steel 2
2 Galvanized
3 Concrete
4 Open hole
5 Plastic ² Salty 27-30 18-21 1 Fresh 4 2 Salty 6 Sulphur Minerals Gas Pumping test method

Pump 2 Bailer LOCATION OF WELL In diagram below show distances of well from road and lot line. Indicate north by arrow. Static level Water level 1 🗌 Pumping Water levels during ² ☐ Recovery end of pumping 45 minutes 32-34 15 minutes 30 minutes 29-31 19-21 N PUMPING Y4 mile If flowing give rate Pump intake set at Water at end of test GPM feet ☐ Clear ☐ Cloudy Recommended pump type Recommended Recommended pump rate pump setting ☐ Shallow ☐ Deep GPM 1514 **FINAL STATUS OF WELL** Bandoned, insufficient supply □ Unfinished
□ Abandoned, poor quality □ Replacement well
□ Abandoned (Other)
□ Dewatering WATER USE 1 Domestic
2 Stock
3 Irrigation 9 🗌 Not used 10 | Other .. Practice ☐ Industrial METHOD OF CONSTRUCTION 9 🗀 Driving 10 ☐ Digging
11 ☐ Other ... 201705 Well Contractor's Licence No **USE ONLY** AUG 0 9 1999 source umpic Drilling Co. Ltd 4006 4000 Date of inspection MINISTRY CSS.ES0

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The Ontario Water Resources Act WATER WELL RECORD

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Print only in spaces provided. 1534131 15002 Mark correct box with a checkmark, where applicable. 11 RF Township/Borough/City/Town/Village Con block survey, etc. Lot County or District 72 6/ Ducester 1003 completed / day reele 21 \Box LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions) Depth - feet General description General colour Most common material From S grave OP disc meston 31 32 CASING & OPEN HOLE RECORD Sizes of opening (Slot No.) WATER RECORD Inside diam inches Wall thickness inches Water found Kind of water То ☐ Fresh ³ ☐ Sulphur Material and type Depth at top of screen 1 Steel 2 Galvanized 2 Gas 29 0 -188 64 **PLUGGING & SEALING RECORD** 2 🗆 Salty ☐ Steel ☐ Galva ☐ Abandonmer Galvanized Concrete Open hole Plastic Sulphur Minerals Depth set at - feet Material and type (Cement grout, bentonite, etc.) 27 2 - Salty 62 Gas From 6 Sulphur Minerals 1 Fresh
2 Salty 27 O Steel
Galvanized
Concrete
Open hole
Plastic Gas Sulphur Minerals 1 Fresh
2 Salty 30-33 **LOCATION OF WELL** 1 ump 2 🗆 Baile In diagram below show distances of well from road and lot line. Water level Water levels during Indicate north by arrow end of pumping 30 minutes 15 minutes 50 19Km If flowing give rate Cloudy 46 ☐ Clear **GPM** Recommended pump setting 56 43-45 Recommended pump type pump rate Deep ☐ Shallow 1.1Km **FINAL STATUS OF WELL** 5 ☐ Abandoned, insufficient supply
6 ☐ Abandoned, poor quality
7 ☐ Abandoned (Other)
8 ☐ Dewatering RdauRo WATER USE 5 □ Commercial
6 □ Municipal
7 □ Public supply
8 □ Cooling & air conditioning 9 Not use
10 Other 1 Domestic
2 Stock
3 Irrigation
4 Industrial METHOD OF CONSTRUCTION 57 5 Air percussion
6 Boring
7 Diamond
8 Jetting □ Cable tool
 □ Rotary (conventional)
 □ Rotary (reverse)
 □ Rotary (air) ☐ Driving 10 Digging
11 Other 265638 ONLY 2 3 2003 119 OCT Date of inspection USE (MINISTRY CSS.ES3 MO ON S

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The Ontario Water Resources Act WATER WELL RECORD

0506 (06/02) Front Form 9

Print only in spaces provided. 1534133 Mark correct box with a checkmark, where applicable. ISOOZ RF 11 County or District Township/Borough/City/Town/Village Con block tract survey, etc. Gloucester Date completed 26 090 Ottawa RC Basin Code 21 للا Щ LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions) Depth - feet General colour Most common material From que 101 sandstone 31 Sizes of op (Slot No.) CASING & OPEN HOLE RECORD WATER RECORD 51 Inside diam inches SCREEN Depth Water found Kind of water inches feet at - feet From То Depth at top of screen Material and type 3 🗆 Sulphur Steel
Concrete
Copen hole
Plastic 1 Fresh 3 Sulpl 2 Mine 2 Gas 69 1 | Trest 3 | Sulprise 2 | Salty 5 | Thing all □ Sulphur .188 83 **PLUGGING & SEALING RECORD** 61 ☐ Steel
☐ Galvanized
☐ Concrete
☐ Open hole Annular space 20-23 3 ☐ Sulphur 4 ☐ Minerals 1 🗆 Fresh Depth s 67 Material and type (Cement grout, bentonite, etc.) 2 🗆 Salty 101 ☐ Gas Fro 6 5 | Plastic 25-28 3 ☐ Sulphur 4 ☐ Minerals 1 🗆 Fresh 67 bentonito 1 Steel 2
2 Galvanized
3 Concrete
4 Open hole
5 Plastic 2 🗆 Salty ☐ Gas 3 Sulphur
4 Minerals
6 Gas 30-33 1 🗆 Fresh 2 Salty Pumping test method f pump 15-16 Hours 11-14 GPM **LOCATION OF WELL** 17-18 Mins ı **Æ**Pump ₂ □ Baile In diagram below show distances of well from road and lot line. Water level end of pumping Water levels during 1 Pumping Indicate north by arrow. 45 minutes 32-34 44 44 If flowing give rate Pump intake set at Water at end of test 200, Upou ☐ Clear **GPM** Recommended pump type Recommended Recommended pump setting 90 pump rate ☐ Shallow Deep feet GPM FINAL STATUS OF WELL 1 ☐ Water supply
2 ☐ Observation well
3 ☐ Test hole
4 ☐ Recharge well 5 ☐ Abandoned, insufficient supply
6 ☐ Abandoned, poor quality
7 ☐ Abandoned (Other)
8 ☐ Dewatering WATER USE 55-56 1 ☐ Domestic 2 ☐ Stock METHOD OF CONSTRUCTION 57 1 ☐ Cable tool
2 ☐ Rotary (conventional)
3 ☐ Rotary (reverse)
4 ☐ Rotary (air) 5 Air percussion
6 Boring
7 Diamond
8 Jetting 9 Drivina 10 Digging 265605 ONLY OCT 2 3 2003 19 source Date of inspection USE MINISTRY CSS.ES3 **%**D

		1 13						
(8) Ontario	Ministry of	Well Tag Number (Place	e sticker and print number below)		Well Record			
	the Environment		Regulation 903 Ontario Water Resource					
Instructions for Compl	eting Form	<u> Α οιφ</u>	1015	_	page <u>2</u> of <u>2</u>			
				Please retain for future reference and explanations are available o				
 Questions regarding of 	ompleting this applicati	ion can be directed to	the Water Well Manage	ment Coordinator at 416-23	5-6203.			
 All metre measurem Please print clearly in 	ents shall be reported blue or black ink only.	to 1/10 th of a metre.		Ministry Use Only				
Well Owner's Informati	on and Location of V	Vell Information		ON RT	OF LOT A3			
First Name	Last Name	Ma	uiling Address (Street Numb	er/Name, RR,Lot,Concession)				
County/District/Municipality	Township	/City/Town/Village	Province Post	al Code Telephone N	lumber (include area code)			
Ottawa - Cor Address of Well Location (Co	e ton Gu	oucester Tou	Ontario Kompania	1 x 167	Concession			
Ottawa- Ca	ar le 4 on		Gloucester	23	5			
RR#/Street Number/Name	k stree	+	City/Town/Village	Site/Compartment/l	Block/Tract etc.			
GPS Reading NAD	Zone Easting	Northing	Unit Make/Model Mod	e of Operation: Undifferentiated	L3			
8 3 Log of Overburden and		5017092 see instructions)	Etrey 4	Differentiated,	specify			
General Colour Most comr	non material	Other Materials	Gener	al Description	Depth Metres From To			
Grey Lines	tone		Layered		0 18.0			
Grey Sand	store		Layered	dan da da da da da da da da da da da da da	18.0 18.8			
Grey Line	stone		Layered		18,8 21.3			
, , , , , , , , , , , , , , , , , , ,								
		I A A A A A A A A A A A A A A A A A A A	TARREST MARKET M	A A A A A A A A A A A A A A A A A A A				
		· · · · · · · · · · · · · · · · · · ·		Addition value of the second s				
Hole Diameter		Construction Reco	ord	Test of We	II Yield			
Depth Metres Diame	I Inside	Wall	Depth Metres	III amping toot mounts	Down Recovery ater Level Time Water Level			
From To Centime	tres diam Mate	thickness centimetres	From To	min	Metres min Metres			
0 21.3 15		Casing		Purito intake set at - Static (metres) Level	2			
	Steel	··		Pumping rate - 1	1 /			
↑ Water Record	Plastic Galvanize	U I D	0 3.0	Duration of humping 2	2			
Water found at Metres Kind of Water		Fibreglass		hrs + min Final water level end 3				
m Fresh Sulp		Concrete		of pumping metres	3			
Gal Salty Mine	Galvanize	ed		Recommended pump 4	4			
m Fresh Sulp	hur Plastic	Concrete		Recommended pump 5	5			
Gas Salty Mine	Galvanize	·		depthmetres				
Gas Sulty Mine		Screen		Recommended pump 70 rate. (litres/min) 15	10			
Other:	diam	Fibreglass Slot No.		If flowing give rate 20	20			
After test of well yield, water wa	Galvanize	- I		(litres/min) 25 If pumping discontin- 30	25 30			
Other, specify		No Casing or Scre	een	ued, give reason.	40			
Chlorinated Yes No	Open hol	е	3.0 21.3	50	50			
V	d Sealing Record	Annular space At	pandonment	Location of Well				
Depth set at - Metres Material a	nd type (bentonite slurry, neat or	amont clurary etc. Volum	e Placed In diagram belo	ow show distances of well from road	, lot line, and building.			
From To	No.	(cuc)	marodio nonti	street	1			
				1	1/			
				+225m > 1				
				1	1 San			
Cable Too	Method of Construct	ion Diamond] Digging	905n				
Rotary (conventional) Ai	percussion	Jetting	Other	V				
Rotary (reverse) Bo	ring Water Use	Driving		Rideau	0 1			
	ustrial	Public Supply	Other	rideau	road			
[inicipal 🔲	Not used Cooling & air conditioning	Audit No.	10079 Date Well 0	Completed MM DD			
	Final Status of We		oned, (Other) Was the well of	owner's information Date Delive	2004 5 26 red			
	ned, insufficient supply	Dewatering	package delive	JWHEI S IIIIOITIAUOIT				
Test Hole Abando	ned, poor quality Contractor/Technician	Replacement well		Ministry Use Only				
Name of Well Contractor BA		Well Contractor's I	Licence No. Data Source	Contractor	4609			
Business Address (street name,	number, city etc.)	4609	Date Received	YYYY MM DD Date of Insp	pection YYYY MM DD			
1 / n = hn	4.4	Well Technician's		0 8 2004 Well Recor	d Number			
Name of Well Technician (last na		T-170	2 Remarks					
Signature de le no an/Contrac	tor	Date Submitted YYYY	Y MM DD	15	534752			
0506E (09/03)	Contractor's C	opy Ministry's Copy	☐ Well Owner's Copy ☐	Cette formule	est disponible en français			
					i			
	i	I						

A 066507 Ministry of Well Record rint Below) the Environment Regulation 903 Ontario Water Resources Act 06650 NImperial | Measurements recorded in: Metric Page Concession RAI City/Town/Village Province Postal Code Ontario Other Municipal A Plan and Sublot Number Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form) Depth (m/ft) General Colour -Most Common Material Other Materials General Description From Results of Well Yield Testing **Annular Space** After test of well yield, water was Draw Down Depth Set at (m/ft) Type of Sealant Used Volume Placed Recovery Clear and sand fr (Material and Type) (m^3/ft^3) Time | Water Level | Time | Water Level (min) (m/ft) Static If pumping discontinued, give reason: Pump intake set at (nd/ft)) Pumping rate (I/min (GPM) Well Use Method of Construction Diamond Cable Tool Public Commercial Not used Duration of pumping Dewatering Rotary (Conventional) Jetting ✓ Domestic Municipal hrs + O min Rotary (Reverse) Monitoring Driving Livestock Test Hole Final water level end of pumping (m/ft) Boring Irrigation Cooling & Air Conditioning Digging Air percussion Industrial Other, specify Other, specify If flowing give rate (Vmin+ GPM) Construction Record - Casing Status of Well 20 Depth (m/ft) Recommended pump depth (n/ft) Water Supply Open Hole OR Material Inside Wall (Galvanized, Fibreglass, Diameter Thickness Replacement Well 25 From To (cm/in) Concrete, Plastic, Steel) (cm/in) Test Hole Recommended pump rate 30 188 Recharge Well (I/min / GPM) Dewatering Well 40 Observation and/or Well production (I/min GPM) Monitoring Hale 50 Alteration Disinfected? (Construction) 60 No Yes Abandoned, Insufficient Supply Map of Well Location Construction Record - Screen Abandoned, Poor Please provide a map below following instructions on the bask Water Quality Outside Depth (m/ft) Material Diameter Slot No. Abandoned, other, (Plastic, Galvanized, Steel) From (cm/in) specify Other, specify **Water Details** Hole Diameter Depth (m/ft) Water found at Depth Kind of Water: Fresh Intested Diameter (cm/in) From Gas Other, specify Water found at Depth Kind of Water: Fresh Untested (m/ft) Gas Other, specify Water found at Depth Kind of Water: Fresh Untested Other, specify Gas Well Contractor and Well Technician Information Business Name of Well Contractor Well Contractor's Licence No. Business Address (Street Number/Name) Municipality Comments: Postal Code Business E-mail Address KOA220 Ministry Use Only Well owner's Date Package Delivered information Bus.Telephone No. (inc. area code) Name of Well Technician (Last Name, First Name) Audit No. package delivered Yes Date Work Completed of Technician and/or Contractor Date Submitted No @ Queen's Printer for Ontario, 2007 Ministry's Copy



Ministry of the Environment

Well Tag No. for Master Well (Place Sticker and/or Print Below)



Master Well Record for Cluster Well Construction

Regulation 903 Ontario Water Resources Act

Master V First Name		and Owner's Infor	Name	PQ			E-mail Ado	u Iress	Page	• C	T
Mailing Ad	aress (Street Number	7Name, RR)	Municipality Rock Lo	سم ما		Provi	nce	Postal Code	•	one No. <i>(inc. a</i>	- 1
		າ of the Master We	II in the Cluster	unci.			JN	INIMIKII	M66113	3141416	<u># </u>
Address of		Number/Name, RR)	Towns	ship				Lot	Conces	ssion	
County/Dis	strict/Municipality		City/To	own/Villag				<u> </u>	Province Ontario	Postal	Code
UTM Coord	18 3 1 8 4 5	41195501	6848 Barn	it Make ∩i Y\	Model Ety	e×	Mode of Op	peration:	Undifferentiate	d 🔀 Ave	raged
Overb General	ourden and Bedrock Most Common	Materials (see insti Other	ructions on the back General		orm) (Metres)	Depth	(Metres)	Hole	Details Diam	ıeter	
Colour	Material ()	Materials	Description	From	To ·	From	То		(Centin	netres)	
rownish	Sandylan	Cravel GII	dia	0.0	0.20	[5,79	<u> 2</u> C)		
grey black	Sandy Ioam Sand	gravelfill gravelfsilt	dry/moist		0,69	·····					
dark grey	Silt	sand	moist	0,69	1,80						······································
grey	Limestone		110191		5.79			Wat	er Use		
J						Public Domes	_	********	Not used Dewatering	Other	, specify
	***					Livesto	ock 🗍 Mu	ınicipal 🔀	Monitoring Cooling & Air Co	onditioning	"
									Construction	-	
						· · · · · · · · · · · · · · · · · · ·	(Conventiona	Air Pe	nd 🔲 i	Digging Boring	
						☐ Rotary☐ Rotary	(Reverse) (Air)	☐ Jetting ☐ Driving		Other, <i>specify</i> HSA	
									s of Well		
		· · · · · · · · · · · · · · · · · · ·	***************************************			i —	ement Well		oned, Insufficien oned, Poor Wate		
						Dewate	-	☐ Other, tion) ☐ Aband	specify oned, other, spe	cîfy	
			·					reen Used	Static W	ater Level 1	est
		Construction Det	ails			Open Hole	Yes X N	·····	<u> </u>	Metres	
Inside Dian	}	Material fibreglass, concrete, ga	Wall Wall Thickness		<i>Metres)</i> To	☐ Galvan	ized []St		eglass Cor	icrete XF	lastic
5.1		PVC	SCHED 40	2.98	5,79	1	ameter (Cen		Slot No.	1	
							/	Water De			
w <u> </u>		•					nd at Depth Metres	1	f Water shSalty _	Sulphur	Minerals
Street de la constant						. ,	nd at Depth	Kind o	f Water shSalty		
Depth Set a	nt (<i>Metres</i>)	Space/Abandonmen Type of Sealant U	sed	Volume] · · · · · · · · · · · · · · · · · · ·	Metres nd at Depth	Kind o	f Water		
From	1.30 F	Material and Type) کے مطعہ م	e)	(Cubic I		<u> </u>		Gas Free	sh Salty C	Sulphur [
1.01	1,30	<u>Sentonite</u>		201	VCl		nitorino		(3/3/2)	y/mm/dd)	·
			· · · · · · · · · · · · · · · · · · ·			Cluster I	nformation	√ (Please also i	fill out the addi	CONOT	r Well
							on for Well Is in Cluster		Please indicate	e Number of (Cluster Well
						Total Wel	ے <u>ک</u> Is on this Pro	operty	Information Lo	g Sheets Sub 	mitted
·····						Luy	<u> 1Know</u>		Well Cluster		
				······································	·	(8.5" x 14'), Sketches	provided as a are not allowe	n attachment no		
				·		Consent t	o release ac or upon req	dditional info	mation concer	ning the clu	ster to
Carteria de la constant				ver ver und de ver ver ver ver ver ver ver ver ver ve			(-)			(yyyy/mm/dd)	_
_	ame of Well Contractor		Well Contra	actor's Lice	nce No.		ell Owner's/	Land Owner's	s consent to us	***************************************	orm
GCOYOC Business Ad	Downing Forders (Street No./Name	Estate Drillinge, number, RR)	a Ltd 8	3 4		Signature	-	Dur	~ 	: (yyyy/mm/dd, 712/01/11	
1110	UP ON OCI C	pale	<u> </u>	surta-	Rouge	A di Ni			Use Only		
a .			Address ACUK, I q Cast Name, First Ne	s.ne	<u> </u>	Audit No.	1 087	19	Well Contractor	NO.	
Bus.Telephor	ne No. (inc. area code) N 니기니니니	Name of Well Technicia	in (Last Name, First Na 31/11/1	ıme)		Date Recei	ved (yyyzmn	5°2012	Date of Inspection	n <i>(yyyy/mm/d</i>	Ŋ
Well Technica	an's Licence No. Signa	erre of Technician	Date Subr	nitted (yyy)	y/mm/dd)	Remarks			t t d d t d d d d d d d d d d d d d d d	······································	
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Ministry of the Environment

Well Tag No. for Master Well (Print Well Tag No.) A110683

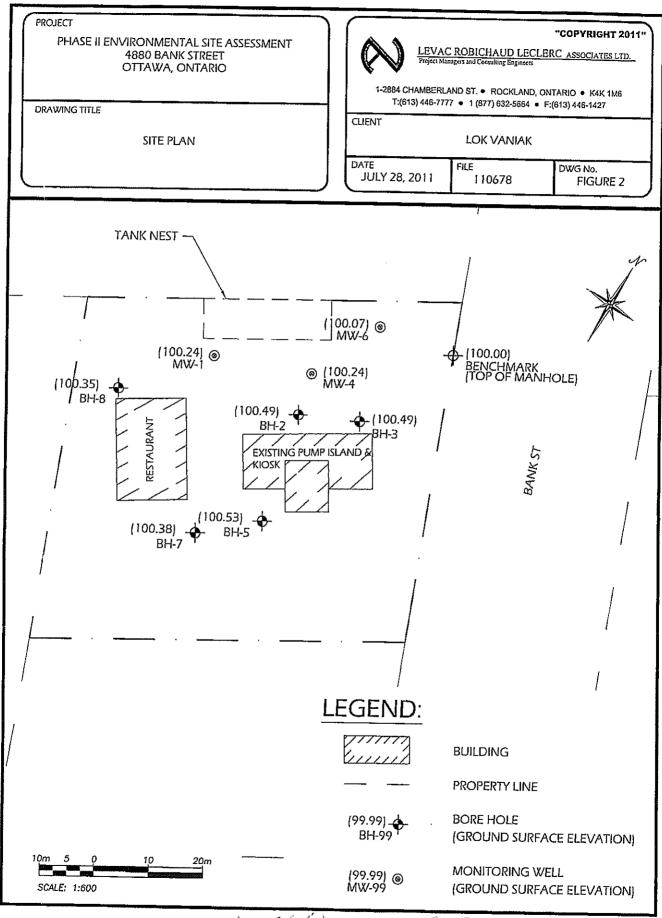
Cluster Well Information for Cluster Well Construction

Regulation 903 Ontario Water Resources Act

				<u> </u>		······································	·.···	<u> </u>					Page	of
Prop	erty Owner's Information											Consent		
	,	Last Name	1>	•	-	ddress (Street N		•	Munic	•	1	Property Owner's Con	sent to use clus	
Provir		<u>. Associa</u>		ail Address	11-28	384 Char	nperla	nd S	Telephone i	OCKION(No. (inc. area) a code)	Signature		Data (man/man/dal)
	ON K	411	M/6						i i	3 4 4	•			
4800406064884	ter Well Information				•							Consent to release add	litional informat	ion to the Director
	ss of Well Location (Street Number/Nam	e, RR)	Lot		Concession	Township			County	y/District/Mur	nicipality	Signature of Technician	Contractor	Date (yyyy/mm/dd)
	own/Village	Province	Postal Code	10	GPS Unit Make	Model	Unit Mod	le of Oper	ation Und	differentiated			_	2012/01/0
	Ottawa 1	Ontario			1 Garmin	Etrex	☐ Differ	entiated, s	specify:			1 Sune /h		201210110
Well # on Sketch	UTM Coordinates Zone Easting Northing	Full Dept Hole (met	h of Hole Diameter res) (cm)	r Method o Construction	, J	rial Casing Length (metres)	Screen Inte	erval (metres)	Annular Space Sealant Used	Static Water Level (metres)	Abandonment Sealant Used	Commen	S	Date of Completion (yyyy/mm/dd)
mω	1184151411815510116181	34 5.60	20	HSA	PVC	2,72	2,72	5,60	Bentonite			· · · · · · · · · · · · · · · · · · ·		2011/07/2
mw 1	1184514111614510116181	117 5;60	20	HSA	PVC	2.74	2,74	5,60	Bentonite					2011/07/2
				-										

								1						
						·								
					÷.`									
Well	Contractor and Well Technicia	n Informati	on									Date 1st Well in Cluster Cor	(www/mm/c	idal)
Δ	ess Name of Well Contractor				s (Street Number/	•		Municipa	•		Province	201107		2011/07/21
_GPO Postal	code Downing Estate	Unling one No. (inc. %)	rea code)	Well Contract	ON NCIDALE ctor's Licence No. E	Business F-mail	Address	Grenv	ille-sur-la	<u>r-Kouge</u>	Q C	Ministry Use Only Date Received (yyyy/mm		spected (yyyy/mm/dd)
J (of Well Technician (First Name, Last Nar	2/4/2/	0 4 6 9)	Cian's Licence No. [<u> </u>	auk.	igs net	·		JAN 2 6 20	12	
Rn	120 Downing	,		2 1	1713	2012/01/	i	Julian	an iconilicially	Lun	~	Audit No. 1369(Remark	s //::[-[-[-]
1991 (1	1/2006)				1 1 1		linietn <i>i</i> 'e	Conv					© Queer	n's Printer for Ontario, 2006

Ministry's Copy



Ministry of the Environment Well Record Well Tag No. (Place Sticker and/or Print Below) and Climate Change Tag#:A 225648 Regulation 903 Ontario Water Resources Act easurements recorded in: 🗌 Metric 🔭 Merial Well Owner's Information ast Name / Organization E-mail Address ☐ Well Constructed First Name RIDEAU CADLETON RACGUAY by Well Owner Mailing Address (Street Number/Name Province No. (inc. area code) KIX1A36138222211 4837 ONT ROAN GLOUCESTER ALBION Well Location Address of Well Location (Street Number/Name) Concession 23 483 ALBION 0772AWA ROAD Postal Code Province County/District/Municipality City/Town/Villa GLOVERSTGE UTM Coordinates Zone Easting Ontario KIXIA3 Other Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form) Depth (m/ft) Most Common Material General Description General Colour Other Materials From SAUDY LOAM 2 36 FWE SAND 36 5 SMDSTONE WELL CONTRUCTION ORIGIONAL WELL RECORD, UN TAGED WATER SUPPLY COUCH CROSSING OF WELL YIELD YESTIN Annular space Draw Down Recovery Depth Set at (m/ft) From | To Time Other, specify Water Level Time | Water Level (m/ft) (m/ft) (min) Static i pumping discentinued, give reason: 1 Pump intake set at (m/ft) 2 2 3 3 Pumping rate (Vmin / GPM) Method of Construction Well Use 4 Δ ☐ Not used Cable Tool Diamond Commercial Public Duration of pumping Domestic Rotary (Conventional) Municipal Dewatering Jetting 5 5 hrs + min Test Hole Driving Livestock Rotary (Reverse) Final water level end of pumping (m/ft) ☐ Boring ☐ Imigation Cooling & Air Conditioning Digging 10 10 Air percussion Industrial 15 Other, specify Other, specify 15 If flowing give rate (Vmin / GPM) Construction Record - Casing Status of Well 20 20 **⊠**Water Supply Recommended pump depth (m/ft) Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel) Depth (m/ft) Inside Wall Thickness (cm/in) Diamete (cm/in) Replacement Well 25 To From Test Hole Recommended pump rate 30 30 3/16 37 Recharge Well (Vmin / GPM) STEEL Dewatering Well 40 40 Observation and/or Well production (Vmin / GPM) Monitoring Hole 50 50 Alteration Disinfected? (Construction) 60 60 Abandoned, Insufficient Supply Map of Well Location Construction Record - Screen Abandoned, Poor Please provide a map below following instructions on the back. Water Quality Outside Material (Plastic, Galvanized, Steel) Depth (m/ft) Abandoned, other, Slot No. Τn (cm/in) specify NONE Other, specify Hole Diameter Water Details Water found at Depth Kind of Water: ▶ Fresh Untested (mft) Gas Other, specify Depth (*m/ft*) Diameter Water found at Depth Kind of Water: Fresh Untested ALBION (m/ft) Gas Other, specify Water found at Depth Kind of Water: Fresh Untested Other, specify Well Contractor and Well Technician Information ne of Well Contractor 37 THE RUMP HOUSE INC (a) 5 Municipality Comments: CLYDE Business E-mail Address INFO @ THEPUMPHOUSE.CA Ministry Use Only Well owner's Date Package Delivered information Audit No. **2**25689 20117 19813G of Well Technician (Last Name, First Name) package delivered 6137224226 MACZUTYRE Date Work Completed Technician and/or Contractor Date Submitted Yes 017198136 20170830 ଟ ☐ No © Queen's Printer for Ontario, 2014 Ministry's Copy E (2014/11)

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Measurements r Well Owner's		(Metric ☐ Imp	репат		var eti medikanan (a.	455479500s			P	age	of <u>/</u>
First Name	1 Secti	Last Name / Org	anizațion	677	WA GI	De	E-mail Address	TINS	TDP WITH		Constructed
Mailing Address (Street Number/N	ame)	<u> </u>	Municina	lity		Province 1	Postal Code	Terepho	ne No. (inc. a	II Owner area code)
A	CATE	RIVE	www.	OTT	Mill		au	K256	K Alais	JETE &	A.
Well Location Address of Well L	ocation (Street N	umber/Name)		Township	ه د د د د د سر			Lot	Conces	ssion	
40084	858 BA	by sta	A) 	AUG	65	102	Z	Davis	4CK	<u>/</u>
County/District/M	unicipality	SITTE	Z ().	City/Towr		PU	6 .		Province Ontario	Postal	
UTM Coordinates		North	ning	Municipa	Plan and Suble	ot Nun	ber		Other		
NAD 8 3 Overburden an		erials/Abandonn	nent Sealing F	Record (see	instructions on th	ne baci	of this form)		(6 - 630 / Co. 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 / 155 /		
General Colour	Most Cor	mmon Material		Other Mate	rials		Ge	neral Description	natary.	Dept From	th (<i>m/ft</i>) To
WELL	10/20-	STATCW	ABR	<u>CASIL</u>	SOTZKY		<u>675/0</u>	Mring)		1	<i>P14</i>)_
		Let			<u> </u>						
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K-20R	30.	0.60	1/5	245	7907		5011	1929	<u> </u>	1)182	530
10219		- Vago					3018				1000
		Annular Sp	and the second s					Results of W		1100 - 1100,000 001101	
Depth Set at (n	n/ft) [o	Type of Sealar (Material and 1		, Vo	lume Placed (m³/ft³)		er test of well yiel Clear and sand	•	11	Level Time	ecovery/ Water Level
Told)	or the	Mig and	terfl	40	603	11	Other, specify	ued, give reason:	(min) (min) Static	fft) (min)	(m/ft)
	D	Aloute	904]]" "	arriping discondin	aea, give reason.	Level 1		
			/			Pur	np intake set at	m/ft)	2	2	
]			3 /	3	
	of Construction	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ll Use] Pur	nping rate (I/min)	GPM)	4	4	
Cable Tool Rotary (Conver	☐ Diamo ntional) ☐ Jetting	. =	stic 🔲 Mu	•	Not used Dewatering	Dur	ation of pumping			5	· · · · · · · · · · · · · · · · · · ·
Rotary (Reverse	e) 🔲 Driving 🗌 Diggin			st Hole oling & Air Cor	Monitoring additioning	Fina	hrs + al water level end	min d of pumping (m/fg	10		
Air percussion Other, specify	_ 33	☐ Industr	rial							10	
	Construction	Record - Casing		Sta	atus of Well	∐ If flo	wing give rate (i	min / GPM)	15	15	
Inside Ope Diameter (Ga	en Hole OR Material Ivanized, Fibreglass	Wall Thickness	Depth (m/ft)	-	ater Supply	Red	commended pun	np depth (m/ft)	20	20	
(cm/in) Cor	ncrete, Plastic, Steel)	(cm/in)	From To	Tes	placement Well st Hole	Red	commended pan	np rate	25	25	
				1 —	charge Well watering Well		in / GPM)	•	30	30	
				₩ Ob	servation and/or onitoring Hole	We	Il production (I/mi	n/GPM)	40	40	1
					eration onstruction)	Disj	flected?		50	50	
					andoned, sufficient Supply		Yes No		60	60	
Outside	Construction Material	Record - Scree	n Depth (m/ft)	Ab	andoned, Poor ater Quality	Ple	ase provide a m	Map of W nap below followi	ell Location ng instructions	on the back	- 4.1//
Diameter (cm/in) (Plas	tic, Galvanized, Stee	Slot No.	From To		andoned, other,			_	~ ~		
				,	VSTULTO)	相		1 4	in Skipp	e/ D	
					2019		My,				- Kink
	Water D	August 1 - Properties Contract Contract		Hole Dia			N.	// u Q	H. Oa.	į il	
(m/n)	epth Kind of War Gas ☐ Other, s		Untested Fro	Depth (<i>m/ft</i>)	Diameter (cm/in)		N X		MILLER	<u> </u>	- 11 3
	epth Kind of Wa	ter Fresh 🔲	Untested	laktr	ی) Ju	ndivisi	in	
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First Name			ast Name / Or	Temple c	£ (OHOW Godel unicipality	n hindu Province	4	gm	104	by We	ell Owner area code)	
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Well Loc	ation	tion (Street Nun	nber/Name)		To	ownship		Lot		Concessi	on		
4835	Rich	5-1	_ .		- - C	ity/Town/Village	· ·		Provin		Postal	Code ~ .	
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_ 0 0	Fr10"		illa z	a/6/		7.0			1		1		
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Other, sp	pecify	inalistamento, incidellyllante y Physikistatifile.	_ Dthe	er, specify	salit alemiatika	al spinosimus da suca cincachinada de se suca sucantino	If flowing give r	ate (Vmin / GPM)	15		15		
Inside Diameter	Open Ho	ole OR Material zed, Fibreglass,	Wall Thickness	ng Depth (<i>m/fi)</i>		☐ Water Supply	Recommended	d pump depth (m/ft)	20		20		
(cm/in) 2.067	Concrete	e, Plastic, Steel)	(cm/in)		io 11	Replacement Well Test Hole Recharge Well	Recommended	d pump rate	25 30		25 30		
2.267	puz	-	2.134	0' 9'10	0	Dewatering Well	(I/min / GPM)		40		40		
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-						(Construction)	Disinfected?	No	60	-	60		
Outside		onstruction R Material	ecord - Scre	en Depth (<i>m/ft</i>)	adothly a flyter	Insufficient Supply Abandoned, Poor Water Quality	Please provide	Map of We e a map below following					
Diameter (cm/in)	(Plastic, G	Salvanized, Steel)	Slot No.	From T	o	Abandoned, other, specify	<u></u>	 _					
2.3%	p	<u> </u>	3	9104 19	'w"	☐ Other, specify		Benk 56					
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	nd at Depth m/ft)	Kind of Water		-	and the state of the state	h (m/ft) Diameter To (cm/in)		-)				
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Water foun		Kind of Water	r: Fresh	Untested				Tample					
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	lame of Wo	ell Contractor	22-millionario Carretto adecidentamilio 143	and the second of the second o	We	Contractor's Licence No.	'						
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_a /	'	Postal Code	یما این	E-mail Address						ingingarisan 1200 cm	Skila i Svatine species	Miles Apple Obsession over the	
Bus.Teleph	one No. (in	c. area code) N	ا ا ا الالاري ame of Well Te	echnician (Last N	G/ lante,	Ou P · CC/ First Name)	Well owner's information package	Date Package Deliver	1		istry Us Z2 Q	6383	
6 [1 [3] Well Technic		5 2 2 タ ce No. Signatur	Seymov of Technician	and/or Contract		γ te Submitted	delivered	Date Work Completed	~ /				
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measurements recorded in.								d/or Print Below 247970		Regulation 903 Ontario Water Resources Ad					
Well Owner's First Name Mailing Address 4835	(Street N	nation L Number/Nam	ast Name / Or 1111 16 e)	ganization MIL A	- Others	Calletoni unicipality Otherwa	<u>/</u>	E-mail Addr Lyd d Province	ess enthe others p Postal Code KIVI	39.M 640	<i>lan</i>	by We	Constructed ell Owner area code)		
Address of Well	t Location	(Street Num				ownship			Lot		Concession	on	•		
County/District/N	<u>√h ≮ S</u> Municipal	ity				ity/Town/Villag	e			Provin Onta		Postal	Code		
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General Colour	CONTROLOGICACION	material and a superior of the	als/Abandon non Material	ment Sea	race in the contract of the co	rd <i>(see Instruct</i> er Materials	ions on the	back of this form	General Description	n		Dep From	th (<i>m/ft</i>) 10		
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Depth Set at (То	t . /	Type of Seala (Material and		15 %	Volume P (m³/ft		After test of well Clear and s Other, spec		Time (min)	aw Down Water Lev (m/ft)		ecovery Water Level (m/ft)		
ר יר	131	<u> </u>	liasu	nd Ing	15 h	TOTAL TOTAL	2943	If pumping disco	ntinued, give reason	Level					
								Pump intake set	at (m/ft)	1 2		1 2			
	AF CSIGN	ituction			Moulle	B		Pumping rate (V/	nin / GPM)	3		3			
Cable Tool	Micribalifically distribution	Diamond	Publi	-	Commer Commer	cial No	ot used	Duration of pum	oina	4		4			
Rotary (Rever		☐ Jetting☐ Driving☐ Digging	Lives	tock	☐ Municipa ☐ Test Hole ☐ Cooling		ewatering onitoring	hrs +	min end of pumping (m/f	5		5	•		
Air percussion Other, specify			☐ Indus			37 th O0110100(III)				10		10			
Inside Or			ecord Casi	i g Depth	(m#)	Status of	anac maramanananan	If flowing give rai		20		20			
Diameter (G	alvanized,	OR Material Fibreglass, astic, Steel)	Wall Thickness (cm/in)	From	To	Replaceme			oump depth (m/ft)	25		25			
2.667	DUL		0.154	0'	81	Recharge '		Recommended (I/min / GPM)	oump rate	30		30			
						Deservation Monitoring	on and/or	Well production	Vmin / GPM)	40		40			
						Alteration (Constructi		Disinfected?	_	50 60		50 60			
	Соля	struction R	ecord - Scree	en		☐ Abandoned Insufficient ☐ Abandoned	Supply	Yes N	Map of V	ال	ation				
Outside Diameter (cm/in) (Pla	Mate stic, Galva	erial anized, Steel)	Slot No.	Depth From	(<i>m/ft</i>) To	Water Qua	· .	Please provide	a map below follow			the back	L=		
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Business Name	of Well C	Contractor	Section of settings in the setting of the setting	o Grandac martineature.	describiblished to the production of the second section of the second se	Contractor's Lid	cence No.								
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44-262) 6 Province 21)		stall Code		-mail Add	ress			Well owner's	Date Package Delive	red	Min	stry Use	е Олју		
Bus. Telephone N	No. (inc. ai	rea code) Na	me of Well Te	,	~	First Name)		information package delivered	Y Y Y Y M M	ם ם	Audit No.	z 28	6385		
Well Technician's			of Technician		hcon () ntractor Da	te Submitted مسما تعرید	الايادان	□ ⊁és [Date Work Complete	۸. ۸	Received				
3 3 1 0506E (2014/11)	80		<u></u>		10-	Ø ∤ ∤7 Ø ⁴ Ministry'			ALFIR IR MIN	Ø [P]			ZUIJ or Ontario, 2014		

Measurement	ntario and Clir	of the Environment mate Change	Well Tag	No. (Place Sticker and Tag#: A2479		Regulation	903 O			ecord
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Well Loc	ation Well Location (Street Nun	abor/Nome)		ownship		Lot		Concession		
Address or	S Bunk S	преглапте)	'	ownship		,				
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Overburd General C	en and Bestrock Maten	als/Abandonment \ non Material	design to a supplied of the su	r d <i>(see instructions on th</i> er Materials	and the second season of second second second second	General Description			_ Dep	th (<i>m/ft)</i>
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Depth Se From	et at (m/ft) To	Type of Sealant Use (Material and Type)	d	Volume Placed (m²/ft³)	After test of well ☐ Clear and s	yield, water was: and free		aw Down Water Leve		ecovery Water Level
01	7' Bear	ton de		25ª	Other, spec	cify	(min) Static	(m/ft)	(min)	(m/ft)
71	13:30 5:75	e. 5,00		343	If pumping disco	ntinued, give reason:	Level			
		u uses				-	1	l 	1	
					Pump intake set	at (m/ft)	2		2	
Wet	hed of Construction		Well Us	8	Pumping rate (Vr	min / GPM)	3		3	
Cable To	ool Diamono	Public	☐ Commer	rcial	Duration of pum		4		4	
☐ Rotary (0	Conventional)	☐ Domestic ☐ Livestock	☐ Municipa ☐ Test Hole		hrs +	min	5		5	
☐ Boring ☐ Air percu	☐ Digging	☐ Imigation☐ Industrial	Cooling	& Air Conditioning	Final water level	end of pumping (m/ft)	10		10	
Other, sp		Other, specify	<u>/</u>		If flowing give rat	ie (Vmin / GPM)	15		15	
Inside	Construction R Open Hole OR Material	ecord - Casing		Status of Well			20		20	
Diameter (cm/in)	(Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall De Thickness (cm/in) From	epth (<i>m/ft)</i> To	☐ Water Supply ☐ Replacement Well	Recommended	pump depth (m/ft)	25		25	
2.067	PUZ	0-154 0	8/34	_	Recommended (I/min / GPM)	pump rate	30		30	
	702	3 ,3 , 3	0)	Dewatering Well	,		40		40	
			-	Observation and/or Monitoring Hole	Well production	(Vmin / GPM)	50	_	50	
			_	Alteration (Construction)	Disinfected?		60		60	
	Construction R	in in the same this Insurant sortion with a fill of		Abandoned, Insufficient Supply	Yes N				<u> </u>	Here the same same same
Outside	Material	De	epth (<i>m/ft</i>)	Abandoned, Poor Water Quality	Please provide	a map below following		ation uctions on t		handand hands and the amplified
Diameter (cm/in)	(Plastic, Galvanized, Steel)	Clof Na		Abandoned, other, specify		a. V		,		
ス・シンド	3 PUC 3 8'3"		" 13'3"	Other energic		Zinh	Bunk 51			_
	-	, ·		Other, specify						
	Water De	APOND APPOINTMENT OF THE PROPERTY OF THE PROPE	complete affected by the complete and the complete of the comp	ole Diameter	il [B	7
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	d at Depth Kind of Water		ed Ou	13311 811		مر				
	n/ft) ☐ Gas ☐ Other, spe id at Depth Kind of Water					1/10	npl			
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		or and Well Technic								
LCL.	lame of Well Contractor 6eOleChnical & ddress (Street Number/Na	and Consection ments	1 mal/s We	Contractor's Licence No. フーラーター オ						
Business A	ddress (Street Number/Na	ame)	Mu Mu	nicipality	Comments:					
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		<u> </u>								
Well Owner's Inform	consideration and a second commence of the contract of the con	Seconization			E-mail Address	0		T =		
First Name	Last Name / 0	2		A FRANCE	ECCONO	Resident Be all	Up com		onstructed	
Mailing Address (Street N	L/CO/Q/	101 AE	O MI	unicipality,	Province ,	Postal Code		ne No. (inc.		
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Well Location	WHIRE LIKIUK		6	2 11270007		MANA	VIVIOLVIS	الماراحا		
Address of Well Location	(Street Number/Name)		То	wnship		Lot	Conces	sion		
HRHA BANI	K Stout			OllowA						
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NAD 8 3 / 8	45351195	0/63	68							
Overburden and Bedr	ock Materials/Abando	nment Seali	ng Recor	d (see instructions on the	back of this form)					
General Colour Most Common Material			Other Materials			General Description			h (<i>m/ft</i>) │ To	
	c! of (7 1	·	1210 36in	1 Nia V 12 F1 1-07			From		
Dean mis.	1000 01 01	one Du	g 4	124 Jein	ELL DAFFOL T	<u>/ ~ / /-</u>	1000111	-6	in E	
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	America	Canac				Docube of M	ell Yield Testi		l .	
Depth Set at (m/ft)	Annular Type of Sea			Volume Placed	After test of well yield,	este à l'avent à l'acter l'auteur les litrés à contrains	Draw Dow	Acres - Est collaboration Decision No.	covery	
From To	(Material ar			(m³/ft³)	☐ Clear and sand f		Time Water I		Water Level	
	······································	,		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Other, specify		(min) (m/l		(m/ft)	
					If pumping discontinue	d. give reason:	Static			
						-, g	Level			
							1	1		
	•				Pump intake set at (m/	ft)	2	2		
Method of Cons	4-vation		Well Use		Pumping rate (Vmin / G	PM)	3	3		
Cable Tool	Diamond Pu		Commercial				4	4		
Rotary (Conventional)	Jetting Do	_] Commerci] Municipal	====	Duration of pumping		<u> </u>			
Rotary (Reverse)	Driving Liv		Test Hole		hrs + n	nin	5	5	•	
☐ Boring	Z-Digging Imig	gation	Cooling &	Air Conditioning	Final water level end o	f pumping (m/ft)	10	10		
Air percussion	☐ Ind							+ +		
Other, specify		er, specify			If flowing give rate (I/mi	n/GPM)	15	15		
Cons	truction Record - Cas	ing	NE SEE SEE SEE	Status of Well		,	20	20		
Inside Open Hole C Diameter (Galvanized.		Depth (n	n/ft)	Water Supply	Recommended pump	depth (m/ft)	<u> </u>			
(cm/in) Concrete, Pla	f, Fibreglass, Thickness From From		To Replacement Well				25	25		
				Recharge Well	Recommended pump (I/min / GPM)	rate	30	30		
				Dewatering Well	(I/IIIII/ GFINI)		 			
				Observation and/or	Well production (Vmin /	GPM)	40	40		
		 		Monitoring Hole ☐ Alteration	. ,		50	50		
				(Construction)	Disinfected?					
				Abandoned,	Yes No		60	60		
Cons	truction Record - Scr	een		Insufficient Supply Abandoned, Poor		Map of W	ell Location			
Outside		Depth (n		Water Quality	Please provide a mag			on the back		
Diameter (cm/in) Mate (Plastic, Galva		From	To	Attandoned, other,				/h .	ę.	
(GIVII)			-	specify NOT in USE				1 1	{	
				<i>y</i>	7	/ *	a T	•	1	
				Other, specify	Dung	SKIPPE	1 5/			
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	Water Details		2525/12112/06/2010/04/2010/04	ole Diameter	4	175 17			()	
Water found at Depth K		Untested	Depth From	(m/ft) Diameter					17	
	Other, specify		710111	(6//////	3				15	
Water found at Depth K		Untested								
(m/ft) ☐ Gas ☐ Other, specify									17	
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Wel					. 1					
Business Name of Well C					N					
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Business Address (Street	Comments:									
130×18, 147/										
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FOAJLOVEBROSSE-UT HOLMA, I. Com					Well owner's Date P	ackage Deliver	25930050903339990	inistry Use	*Only	
Bus.Telephone No. (inc. area code) Name of Well Technician (Last Name, First Name)					package yy	Y Y M M	D D Audit N	^{6,} Z 30	/113	
6139131824 LAymond SACques					delivered Date V	Vork Completed				
Well Technician's Licence No. Signature of Technician and/or Contractor Date Submitted				$ \cup ^{\text{Yes}} $						
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Well Owner's Information First Name	ast Name / 2003 Pration	· iv ch	and Ethalk Address A	9-7-000	<u>, </u>	Well Constructed	
LETIFING DOLL			WWW.	Poetal Code		by Well Owner	
Mailing Address (Street Number/Nan		Municipality OTTAMA	Province	KITGO	456	90 322	
Well Location				V-13-3-4			
Address of Well Location (Street Nur	nber/Name)	Township	TER 14	Lot ZI	Concession	4	
County/District/Municipality	TAINA	City/Town/Village	m.A			Postal Code	
UTM Coordinates Zone , Easting	Northing &	Municipal Plan and Sublo	t Number	Oth	itario er		
NAD 8 3 EXCEPTION	rbe below.	'					
Overburden and Bedrock Materi General Colour Most Com	ials/Abandonment Sealing mon Material	Record (see Instructions on the Other Materials	I	eral Description		Depth (<i>m/f<u>t</u></i>)	
IOF7/ NA	STATE DATES	GPS EASTIN		NRIHINY	780	P714	
mm /mm	1FUFT In	CFSC DIV	90)	<i>W/11/1</i>		2011-1	
13-12 a 3B	3,09	18453100	501	7309		0-3.7.1	
11 1 38	347	""		11		0-6.91	
B/a 50	1.28	18 453/56	501	17343	-	0-2.0	
1316 50.	Zlo	1843154	50	<i>43</i> 43	-	04-1	
			,				
		<u></u>					
					<u> </u>		
Donth Set at (m/ff)	Annular Space Type of Sealant Used	Volume Placed	After test of well vield.	Results of Well Y	ield Testing Draw Down	Recovery	
Depth Set at (m/ft) From To	(Material and Type)	(m³/ft³)	Clear and sand	'	ne Water Level	Time Water Level	
Total Hillips	ug /1,00-1/14	Volls_	☐ Other, specify	ed give reason: Sta	tic	(min) (my)	
oone te	vank good.			Lev		1	
	<i>V</i>		Pump intake set at	<u>2</u> /ft) 2	2	2	
			Director and Waste /	3		3	
Method of Construction ☐ Cable Tool ☐ Diamon	and the algebraic and the first section of the sect	ell.Use ommercial Not used	Pumping rate (Vmin / 0	3-101)		4	
Rotary (Conventional)	Domestic M	lunicipal Dewatering	Duration of pumping hrs +	min 5		5	
☐ Rotary (Reverse) ☐ Driving ☐ Boring ☐ Digging	1 = =	est Hole Monitoring ooling & Air Conditioning	Final water level end	N	0	10	
☐ Air percussion ☐ Other, specify	☐ Industrial ☐ Other, specify		If flowing give rate (I/n	nin / GPM)	5	15	
Construction F	Record - Casing	Status of Well	I til llowing give rate (in	2	+	20	
Inside Open Hole OR Material Diameter (Galvanized, Fibreglass,	Wall Depth (m/ft) Thickness	☐ Water Supply ☐ Replacement Well	Recommended pump	depth (m/ft)	5	25	
(cm/in) Concrete, Plastic, Steel)	(cm/in) From	☐ Test Hole	Recommended pump			80	
Th		Recharge Well Dewatering Well	(l/min / GPM)	4		40	
		Observation and/or Monitoring Hole	Well production (I/min	/GPM) 5		50	
		Alteration (Construction)	Disinfected?	6		60	
		Abandoned, Insufficient Supply	Yes No			Halver	
Outside Mariel	Record - Screen Depth (m/ft)		Please provide a ma	Map of Well L ap below following in		ie ba ck	
Diameter (Plastic Galvanized, Steel)	. I Slof No 🤼 💮 🗀	Abandoned, other,	.	\sim		41\	
		Construct			\ _	- /`.	
		- 2020.	/ 12/	16 B1		al X	
Water De	the and the control of the control of the second of the second of the control of	Hole Diameter	1 4	4 7	}		
Water found at Depth Kind of Water (m/n) ☐ Gas ☐ Other, sp		Depth (m/ft) Diameter rom To (cm/in)		1	\sim	- IIA	
Water found at Depth Kind of Water	T. Fresh Untested		\parallel \vee \wedge	$\wedge \mathcal{N}$	/	M	
(m/ft) ☐ Gas ☐ Other, sp Water found at Depth Kind of Water			0.76			Stepper	
(m/ft) ☐ Gas ☐ Other, sp				456	,	4611	
Ruciness Name of Well Contractor	tor and Well Technician Info	ormation Well Contractor's Licence No.	4	J		1	
STANTON DE	WING INC	1875					
Business Address (Street Number/N	lame)	Municipality DOV FIVE	Comments:	olden B-	121-008	365	
Province Postal Code	Business E-mail Address	TIME TIME	Comments: 10 G	le site	Plan		
OU KUND	(1) Stanion and	in Cathernel	Well owner's Date information	Package Delivered	Minist	try Use Only	
Rus Telephone No. (inc. area code)	lame of Well Technician (Last)	yaine riist name)	package y y	Y Y M M D		32288 3	
Well Technician's Licence No. Signatur	of of point of Alfo or Sortrac	tor Date Submitted	Yes Date	2011	2 10	R 0 8 2020	
0506E (2018/12)	my woon	Ministry's Copy		WW WY DY		Printer for Ontario, 2018	



Well Record - Regulation 903 Ontario Water Resources Act

Notice of Collection of Personal Information

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Most Common Material

Well Depth *

General Colour

Personal information contained on this form is collected pursuant to sections 35-50 and 75(2) of the Ontario Water Resources Act and section 16.3 of the Wells Regulation. This information will be used for the purpose of maintaining a public record of wells in Ontario. This form and the information contained on the form will be stored in the Ministry's

well record database and made Well Customer Service Represe 1-888-396-9355 or <u>wellshelpdes</u>	ntative a	t the Wells Help						
Fields marked with an asterisk (*) a	re manda	atory.						
					Well Tag Nu	ımber *		
					A 219864			
Type *								
✓ Construction	ment							
Measurement recorded in: *								
☐ Metric								
1. Well Owner's Information								
Last Name and First Name, or Orga	anization	is mandatory. *						
Last Name			First Na	me				
Organization Aecon Construction and Materia	ıls Limite	d	Email Ad	ddress				
Current Address			•					
Unit Number Street Number 4949		eet Name * nk Street			City/Town/Village Gloucester			
Country Canada	•	Province Ontario			Postal Code K1X 1G7 Telephone Nu			
2. Well Location								
Address of Well Location								
Unit Number Street Number * 4949	Street N Bank S				Township Gloucester			
Lot	Conces	sion		County/Dist Carleton	rict/Municipality			
City/Town Gloucester				Province Ontario		Postal Code K1X 1G7		
UTM Coordinates Zone * Easting	*	Northing *			Municipal Plan and	Sublot Number		
NAD 83 18 45437	4	5016659	Test U	TM in Map				
Other BH2-23			,					
3. Overburden and Bedrock M	aterial *							

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General Description

Depth From

Depth To

Other Materials

(ft)

			(ft)	(ft)
Grey	Gravel	Sand	0	5
Grey	Limestone		5	50

. Annular Sp	pace *									
Depth From	Depth To	Type of Sealant Used (Material and Type)	Volume	Placed					
(ft)	(ft)			(cubic	feet)					
0	49	Hole P	Hole Plug 4.54							
49	50	Sand	Sand 0.07							
5. Method of	Construction '	•								
Cable Tool	Rotary (C	onventional)	e) Boring Air pe	rcussion Dia	amond					
Jetting	ing ☐ Driving ☐ Digging ☑ Rotary (Air) ☑ Augering ☐ Direct Push									
Other (spec	ify)									
6. Well Use *										
Public	Indu	ustrial Cooling & Air (Conditioning							
Domestic Commercial Not Used										
Livestock	Mur	☐ Municipal ✓ Monitoring								
Irrigation	Test Hole Dewatering									
Other (spec	ify)									
7. Status of V	Vell *									
Water Supp	ly [Replacement Well	Test Hole							
Recharge W	Vell [Dewatering Well	✓ Observation and/or Mo	nitoring Hole						
Alteration (C	Construction)	Abandoned, Insufficient Supply	Abandoned, Poor Water	r Quality						
Abandoned,	, other (specify)									
Other (spec	ify)									
3. Constructi	on Record - C	asing * (use negative number(s)	to indicate depth above grou	nd surface)						
Inside Diamete		Hole or Material (Galvanized, Fibreg Concrete, Plastic, Steel)	lass, Wall Thickness	Depth From	Depth To					
(in)		(ft) (ft)								
2		2 Plastic 0.25 0 40								

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Slot

Number

10

Depth From

(ft)

40

Depth To

(ft)

50

Material (Plastic, Galvanized, Steel)

Plastic

Outside

Diameter

(in)

2

10. Water Det	tails														
Water found at	Depth 2		(ft)	Gas	Kind of	wat	er [Fres	h 🔲 l	Jntested) D	ther			
Water found at	Depth 40)		Gas	Kind of	wat	er [Fres	h 🔲 l	Jntested	l 🗌 0	ther			
			•												
11. Hole Diam	neter														
De	epth Fror	n			Deptl	h To						Diamete	er		
	(ft)				(ft	t)						(in)			
0					5	5						8			
	5				50	0						4			
12. Results o	f Well Y	ield Te	esting												
Pumping Dis	scontinue	ed													
Explain															
If flowing give ra	ate														
Flowing _					((GPI	/ I)								
Draw down								_		,	,		,		
Time (min)	Static Level	1	2	3	4		5	10	15	20	25	30	40	50	60
Water Level (ft)															
Recovery														,	
Time (mir	۱)	1	2	3	4	5	;	10	15	20	25	30	40	50	60
Water Lev (ft)	rel el														
After test of wel	l yield, w	ater wa	s								•		ļ.	'	
Clear and sa	and free	Oth	ner (spe	cify)											
Pump intake se	t at Pun	nping ra	ate Duration of pumping Final water level end of pumping Disinfected?					? *							
	(ft)		(GPM)		hrs +	•		min				(ft)		Yes 🔽	No
Recommended	pump de	epth	Recom	mended	pump ra	te	Wel	II produc	ction						
		(ft)			(GPI	M)				(GPM)					
13 Man of W	ell I oca	tion *													

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Map 1. Please Click the map area below to import an image file to use as the map.

Make map area bigger



14. Information	on							
Well owner's in	formation packaoู ว	ge delive	ered	Date Package Delivered (yyyy/r	, ,	Date Work Cor 2023/03/29	mpleted (yyyy/mm/dd) *
Comments						,		
15. Well Cont	tractor and We	II Tech	ınician	Information				
Business Name of Well Contractor * Aardvark Drilling Inc.						Well Co 7675	ntractor's Licen	se Number *
Business Add	ress					•		
Unit Number C	Street Number Street Name * Lewis Road							
City/Town/Villag			Prov	vince		Postal Code * N1H 1E9		
Business Telep 519-826-9340				Address drillinginc.com	'			
Last Name of V Tabbert	Vell Technician *	I		First Name of Well Techn Devin	ician *		Well Technic 4557	cian's License Number *
16. Declaration	on *							
✓ I hereby cor and accurat		e persor	n who co	nstructed the well and I he	reby c	onfirm th	at the informati	ion on the form is correct
Last Name First Name Matthe						Email A		drillinginc.com
Signature						Date Su	ıbmitted (yyyy/r	nm/dd)
Matt En	v signed by Matt England 023.04.12 09:20:13 -04'00'			2023	/04/12			
17. Ministry U	Jse Only							

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Audit Number UJW8 VM4D



Well Record - Regulation 903

Ontario Water Resources Act

Notice of Collection of Personal Information

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Most Common Material

Well Depth *

General Colour

Personal information contained on this form is collected pursuant to sections 35-50 and 75(2) of the Ontario Water Resources Act and section 16.3 of the Wells Regulation. This information will be used for the purpose of maintaining a public record of wells in Ontario. This form and the information contained on the form will be stored in the Ministry's

	r Service l	Represer	ntative a	it the Wells Help				rected to the Water Ontario M9P 3V6, at		
Fields marked v	vith an aste	erisk (*) ar	e manda	atory.						
							Well Tag N	lumber *		
							A 219865			
Type *										
✓ Construction	n 🔲 ,	Abandonn	nent							
Measurement :	recorded i	n: *								
Metric	✓ I	mperial								
1. Well Own	er's Info	mation								
Last Name and	First Name	e, or Orga	nization	is mandatory. *						
Last Name					First N	Name				
Organization Aecon Constru	uction and	Material	s Limite	ed	Email	Address				
Current Addre	ss				·					
Unit Number	Street 4949	Number	II.	eet Name * nk Street			City/Town/Village Gloucester			
Country Canada	•		•	Province Ontario			Postal Code Telephone Nu K1X 1G7			
2. Well Loca	tion									
Address of We	II Locatio	า								
Unit Number	Street Nui 4949	mber *	Street N Bank S				Township Gloucester			
Lot			Conces	sion		County/Dist Carleton	rict/Municipality			
City/Town Gloucester						Province Ontario		Postal Code K1X 1G8		
UTM Coordinate	es Zone *	Easting	*	Northing *			Municipal Plan an	d Sublot Number		
NAD 83	18	454398	3	5016664	Test	UTM in Map				
Other MW5-23	•	•		•						
3. Overburde	n and Be	drock Ma	aterial *							

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General Description

Depth From

Depth To

Other Materials

(ft)

			(ft)	(ft)
Grey	Gravel	Sand	0	2
Grey	Limestone		2	45

. Annular Sp	pace *									
Depth From	Depth To	Ту	pe of Sealant Used (I	Materia	al and Type)		Volume	Placed		
(ft)	(ft)		(cubic feet)							
0	34		Hole Plug 2.77							
34	45		Sand 0.73							
. Method of	Construction '	k								
Cable Tool	Rotary (C	onventional)	Rotary (Reverse) [Boring	Air perci	ussion Dia	mond		
Jetting	Driving	Digging	✓ Rotary (Air)	✓	Augering	Direct P	ush			
Other (spec	ify)									
. Well Use *										
Public	Indu	ustria l	Cooling & Air C	Conditi	oning					
Domestic	Con	nmercial	■ Not Used							
Livestock	Mur	nicipal	Monitoring							
Irrigation	Tes	t Hole	Dewatering							
Other (spec	ify)									
. Status of V	Vell *									
Water Supp	ly [Replaceme	ent Well	T	est Hole					
Recharge V	Vell [Dewatering	ı Well	√ 0	bservation and	or Monit	oring Hole			
Alteration (0	Construction)	Abandoned	l, Insufficient Supply	□ A	bandoned, Poc	r Water (Quality			
Abandoned	, other (specify)									
Other (spec	ify)									
. Constructi	on Record - C	asing * (use	e negative number(s) t	o indic	ate depth abov	re ground	l surface)			
Inside			al (Galvanized, Fibreg	lass,	Wall		Depth From	Depth To		
Diamete	1	Concrete	, Plastic, Steel)		Thicknes	55	(ft)	(ft)		
(in)	1									

9. Construction Record - Screen										
Outside Diameter	Material (Plastic, Galvanized, Steel)	Slot Number	Depth From	Depth To						
(in)	(Flastic, Galvanizeu, Steel)	Number	(ft)	(ft)						
2	Plastic	10	35	45						

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10. Water Det	tails																
Water found at	Depth 1.5	5	(ft)	Gas	Kind of wa	ater	Fres	h 🔲 l	Jntested	1 🗌 O	ther						
Water found at	Depth 33			Gas	Kind of wa	ater	Fres	h 🔲 l	Jntested	l 🗌 0	ther						
			·														
11. Hole Dian	neter																
De	epth From	า			Depth 7	Го					Diamete	r	Disinfected? *				
	(ft)				(ft)						(in)	40 50 60					
	0				2						8						
-	2				45						4						
12. Results o	f Well Yi	eld Te	esting														
Pumping Dis	scontinue	d															
Explain																	
If flowing give ra	ate																
Flowing _					(GF	PM)											
Draw down		_							_	_	_			_			
Time (min)	Static Level	1	2	3	4	5	10	15	20	25	30	40	50	60			
Water Level (ft)																	
Recovery		•							•		•	•					
Time (mir	۱)	1	2	3	4	5	10	15	20	25	30	40	50	60			
Water Lev (ft)	'el																
After test of wel	l yield, wa	ater wa	S		<u> </u>	'							1				
Clear and sa	and free	Oth	ner (spe	cify)													
Pump intake se	t at Pum	ping ra	ite	Duratio	n of pumpir	ng		Final w	ater leve	el end of	pumping	g Di	sinfected	? *			
	(ft)		(GPM)		hrs +		min				(ft)		Yes 🔽	✓ No			
Recommended	pump de	pth	Recom	mended	pump rate	We	ll produ	ction									
		(ft)			(GPM)				(GPM)								
13. Map of W	ell Locat	tion *															

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Map 1. Please Click the map area below to import an image file to use as the map.

Make map area bigger



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Audit Number 085E RJ9U

14. Information	on								
Well owner's in Yes No	formation packa o	ge delive	red	Date Package Delivered (y	yyyy/m	nm/dd)	Date Work Con 2023/03/28	npleted (yyyy/mm/dd) *	
Comments									
15. Well Conf	tractor and We	ell Tech	nician	Information					
Business Name Aardvark Drill	e of Well Contracting Inc.	ctor *			Well Co 7675	ontractor's Licen	se Number *		
Business Add	ress								
Unit Number C									
City/Town/Villa Guelph	ge *				Prov ON	ince	ince Postal Code * N1H 1E9		
Business Telep 519-826-9340				Address drillinginc.com	•				
Last Name of V Tabbert	Vell Technician *			First Name of Well Techni Devin	ician *		Well Technic 4557	ian's License Number *	
16. Declaration	on *								
✓ I hereby cor and accurat		e person	who co	nstructed the well and I her	reby co	onfirm th	nat the information	on on the form is correct	
Last Name England	ame W		Email A	.ddress and@aardvark	drillinginc.com				
Signature						Date Su	ubmitted (yyyy/m	nm/dd)	
Matt En	gland	v signed by Matt England 023.04.12 09:17:58 -04'00'			2023	/04/12			

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Well Record - Regulation 903

Ontario Water Resources Act

Notice of Collection of Personal Information

General Colour

Most Common Material

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	er Service F	Represent	tative at	the Wells Help				directed to the Water to Ontario M9P 3V6, at
Fields marked	with an aste	erisk (*) are	mandate	ory.				
							Well Tag	Number *
							A21986	6
Type *								
Constructio	n 🗌 A	Abandonm	ent					
Measurement	recorded i	n: *						
Metric	✓ I	mperial						
1. Well Own	er's Infor	rmation						
Last Name and	l First Name	e, or Organ	ization is	mandatory. *	1			
Last Name					First	Name		
Organization Aecon Constr	ruction and	l Materials	Limited		Ema	il Address		
Current Addre	ess		1					
Unit Number	Street 4949	: Number *	Street Name * City/Town/Village Bank Street Gloucester					
Country Canada	·		·	Province Ontario			Postal Code K1X 1G7	Telephone Number
2. Well Loca	ation							
Address of W	ell Location	1						
Unit Number	Street Nur 4949		Street Na <mark>Bank Str</mark>				Township Gloucester	
Lot		(Concessi	on		County/Dis	trict/Municipality	
City/Town Gloucester		,				Province Ontario		Postal Code K1X 1G8
UTM Coordina	tes Zone *	Easting *		Northing *		!	Municipal Plan	and Sublot Number
NAD 83	18	454374		5016605	Tes	st UTM in Map		
Other MW1-23								
3. Overburde	en and Bed	drock Ma	terial *					
Well Depth *		66		(ft)				

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General Description

Depth From

Depth To

Other Materials

			(ft)	(ft)
Brown	Gravel	Sand	0	8
Grey	Limestone		8	66

Depth From	Depth To Type of Sealant Used (Material and Type) Volum									
(ft)	· (ft)	(cubic feet)								
0	53		Hole Plug 5.59							
53	55		Pellet Plug 0.13							
55	66		Sand 0.73							
						1				
. Method of (Construction *									
Cable Tool	Rotary (C	onventional)	Rotary (Reverse	e) 🔲 Bori	ng Air perd	cussion Dia	amond			
_ Jetting	 Driving	Digging	— ✓ Rotary (Air)	— ✓ Aug	ering Direct F	 Push				
Other (speci	fy)									
. Well Use *										
Public	Indu	strial	Cooling & Air	Conditioning						
Domestic	Con	nmercial	Not Used							
Livestock	Mur	icipal	✓ Monitoring							
Irrigation	Tes	: Hole	Dewatering							
] Other (speci	fy)									
. Status of W	/ell *									
Water Suppl	у Г	Replaceme	ent Well	Test Ho	ole					
 ☐ Recharge W	_	 ີ Dewatering		✓ Observ	ation and/or Moni	itoring Hole				
Trecharge VV	_		d, Insufficient Supply		ned, Poor Water	_				
	onstruction)	_	, , , ,		,	•				
 Alteration (C	, <u> </u>									
 Alteration (C	other (specify)									
Alteration (C Abandoned, Other (speci	other (specify)	asing * (use	e negative number(s)	to indicate de	epth above groun	d surface)				

9. Construction Record - Screen									
Outside Diameter (in)	Material (Plastic, Galvanized, Steel)	Slot Number	Depth From (ft)	Depth To (ft)					
2	Plastic	10	56	66					

Plastic

(in)

2

(ft)

56

(ft)

0

0.25

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10. Water Det	tails															
Water found at	Depth 5		(ft)	Gas	Kind of	wate	er [Fres	h 🗌	Untested	d 🗌 0	ther				
Water found at	Depth 57	7		Gas Kind of water Fresh					h 🗌	Untested	J 🗌 O	ther				
			!													
11. Hole Dian	neter															
Depth From				Depth To					Diameter							
	(ft)				(ft	:)						(in)				
	0				8							8				
	8				66	 6						4				
12. Results o	f Well Y	ield Te	esting													
Pumping Dis	scontinue	 ed														
Explain																
If flowing give ra	ate															
Flowing _					((3PN	/ I)									
Draw down																
Time (min)	Static Level	1	2	3	4		5	10	15	20	25	30	4	10	50	60
Water Level (ft)																
Recovery		•							•							
Time (mir	۱)	1	2	3	4	5		10	15	20	25	30	4	0	50	60
Water Lev (ft)	/el															
After test of wel	ll yield, w	ater wa	s	ı	<u> </u>					1		ı				ı
Clear and sa	and free	Oth	ner (spe	cify)												
Pump intake set at Pumping rate Duration of pumping Final water level end of pumping Disinfecte						infected	? *									
	(ft)		(GPM)		hrs +			min				(ft)			Yes 🔽	No
Recommended	pump de	epth	Recom	mended	l pump rat	te	Wel	ll produc	ction							
		(ft)			(GPI	M)				(GPM)						
13. Map of W	ell I oca	tion *														

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Make map area bigger

Map 1. Please Click the map area below to import an image file to use as the map.



17. Ministry Use Only

Audit Number YTCY BLDC

14. Informati	on									
Well owner's in	iformation packa o	ge deli	vered				Date Work Completed (yyyy/mm/dd) * 2023/03/28			
Comments										
15. Well Con	tractor and We	ell Tec	chnician	Information						
Business Name Aardvark Drill	e of Well Contracting Inc.	ctor *		Well Contractor's 7675			ontractor's Licen	s License Number *		
Business Add	ress				•					
Unit Number C	nit Number Street Number Street Name * Lewis Road									
City/Town/Village * Guelph				Province ON				Postal Code * N1H 1E9		
Business Telep 519-826-9340			ness Email Daardvarl	Address drillinginc.com	'			-		
Last Name of Well Technician * Tabbert				First Name of Well Technician * Devin			Well Technician's License Number * 4557			
16. Declaration	on *									
✓ I hereby con		e perso	on who co	nstructed the well and I he	reby co	onfirm th	nat the informati	on on the form is correct		
Last Name First Name England Matthe						Email Address mengland@aardvarkdrillinginc.com				
Signature			-			Date Su	ubmitted (yyyy/n	nm/dd)		
Matt En	gland		/ signed by Matt England 023.04.12 09:19:07 -04'00'			2023/04/12				

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