



**PATERSON
GROUP**

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Geotechnical Engineering
Environmental Engineering
Hydrogeology
Materials Testing
Building Science
Rural Development Design
Temporary Shoring Design
Retaining Wall Design
Noise and Vibration Studies

June 25, 2024
PG6557-LET.01

Myers Automotive Group

1200 Baseline Road
Ottawa, Ontario
K2C 0A6

Attention: **David Johnston**

patersongroup.ca

Subject: **Preliminary Geotechnical Investigation
Proposed Commercial Development
2175 Prince of Wales Drive – Ottawa, Ontario**

Dear David Johnston,

Further to your request, Paterson Group (Paterson) completed a preliminary geotechnical investigation at the aforementioned site. The current letter report presents the results of the geotechnical investigation and provides preliminary foundation design information and construction recommendations from a geotechnical perspective.

Paterson understands that the proposed development will consist of two commercial low-rise buildings of slab-on-grade constructions. It is also understood that associated asphaltic parking areas, access lanes and hardscaped areas are also anticipated as part of the proposed development. It is expected that the proposed buildings will be municipally serviced.

1.0 Field Observations

Field Investigation

The field program for the investigation was conducted on May 11, 2017. At that time, a total of ten (10) test pits were excavated to a maximum depth 3.0 m below the existing ground surface. Previous investigations were completed by Paterson between November 2002 and May 2003 and consisted of advancing five (5) boreholes and four (4) hand augered test pits to maximum depths of 16.7 and 7.9 m below ground surface, respectively.



A previous historical geotechnical investigation for the subject site was also completed on December 30, 1983, and consisted of advancing three (3) boreholes to a maximum depth of 12.2 m below ground surface. The test hole locations were distributed in a manner to provide general coverage of the subject site and taking into consideration underground utilities and site features. The approximate locations of all test holes are shown on Drawing PG6557-1 - Test Hole Location Plan attached to the present letter report.

The test pit procedure consisted of excavating using a rubber-tired backhoe at the selected locations and sampling the overburden. The boreholes of the previous investigation were drilled using a portable drill rig operated by a two-person crew. The drilling procedure consisted of augering to the required depths at the selected depths and sampling the overburden. All fieldwork was reviewed in the field by Paterson personnel under the direction of a senior engineer from the geotechnical division.

Surface Conditions

The subject site is undeveloped and generally covered with grass. The subject site is bordered by a treed ravine around a tributary to the Rideau River to the north, the Rideau River to the east, Waterbend Lane followed by residential dwellings to the south and Prince of Wales Drive to the west. The ground surface slopes gradually downward to the east towards Rideau River.

Paterson conducted a slope stability analysis for the slopes along the ravine and Rideau River as part of a previous study and determined the geotechnical limit of hazard lands for the subject site. The results of our slope stability analysis are presented under letter report PG1887-LET-01 Revision 3 dated February 28, 2017, and attached to the present letter report.

Subsurface Soil Profile

The subsurface profile encountered at the test hole locations consisted of a thin layer of topsoil underlain by interbedded layers of silty sand, sandy silt and silty clay. A predominance of sand was observed to occur along the watercourse while an increase in clay content was encountered towards the central portion of the subject site and at a greater distance from the river and its tributary.

The interbedded layers of silty sand to silty clay were observed to consist of compact silty sand or very stiff to stiff silty clay with varying amounts of clay, silty and/or sand, respectively. Trace amounts of gravel, cobbles or boulders were also observed occasionally throughout the subject site. The interbedded layers were observed to be brown and weathered up to depths ranging between 2.7 and 8.8 m below ground elevation. Details of the soil profile at each test hole location are presented on the Soil Profile and Test Data sheets appended to this letter report.

Bedrock

Based on available geological mapping, the bedrock surface in this area is encountered at depths varying between 15 to 25 m and consists of dolomite of the Oxford formation.

Groundwater

Groundwater infiltration levels were observed within the open test pit locations. The groundwater infiltration levels are presented in the Soil Profile and Test Data sheets attached. Long-term groundwater levels can also be estimated based on the observed colour, moisture content and consistency of the recovered soil samples. Based on our observations, the long-term groundwater level is located approximately **4 to 6 m** below existing ground surface. It should be noted that groundwater levels are subject to fluctuations, therefore, the groundwater levels could vary at the time of construction.

2.0 Geotechnical Discussion and Construction Precautions

Geotechnical Assessment

From a geotechnical perspective, the subject site is considered suitable for the proposed development. It is expected that the proposed commercial buildings will consist of slab-on-grade construction and be supported by conventional spread footing foundations placed on an undisturbed compact silty sand or stiff silty clay bearing surface.

Due to the presence of the silty clay deposit, a permissible grade raise restriction is required for the subject site.

The above and other considerations are discussed in the following paragraphs.

Site Grading and Preparation

Topsoil and any deleterious fill, containing organics and/or construction debris, should be stripped from under any buildings, paved areas, pipe bedding and other settlement sensitive structures. Care should be taken not to disturb adequate bearing surfaces during site preparation activities.

Fill Placement

Engineered fill used for grading beneath the proposed building, where required, should consist of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. This material should be tested and approved prior to delivery to the site.

The fill should be placed in a maximum 300 mm thick loose lifts and compacted using suitable compaction equipment for the lift thickness. Fill placed beneath the building should be compacted to at least 98% of the material's Standard Proctor Maximum Dry Density (SPMDD).

Site-excavated soil can be used as general landscaping fill where settlement of the ground surface is of minor concern. The soil should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids and approved by Paterson personnel. If the soil is to be used to build up the subgrade level for areas to be paved, it should be compacted in thin lifts to a minimum 95% of its SPMDD. The material should be placed under dry conditions and above freezing temperatures.

Non-specified existing fill and site-excavated soils are not suitable for use as backfill against foundation walls unless used in conjunction with a composite drainage membrane.

Foundation Design

Strip footings, up to 3 m wide, and pad footings, up to 6 m wide, founded on an undisturbed stiff silty clay bearing surface can be designed using a bearing resistance value at serviceability limit states (SLS) of **150 kPa** and a factored bearing resistance value at ultimate limit states (ULS) of **225 kPa** incorporating a geotechnical resistance factor of 0.5 at ULS.

Footings placed on an undisturbed, compact silty sand bearing surface can be designed using a bearing resistance value at SLS of **150 kPa** and a factored bearing resistance value at ultimate limit states (ULS) of **225 kPa**.

An undisturbed soil bearing surface consists of one from which all topsoil and deleterious materials, such as loose, frozen, or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

The bearing resistance value at SLS will be subjected to potential post-construction total and differential settlements of 25 and 20 mm, respectively.

Permissible Grade Raise Restrictions

Due to the presence of the underlying silty clay layer, a permissible grade raise restriction of **2.0 m** is recommended in the immediate area of settlement sensitive structures. A post-development groundwater lowering of 0.5 m was considered in our permissible grade raise restriction calculations. If higher than permissible grade raises are required, preloading with or without a surcharge, lightweight fill and/or other measures should be investigated to reduce the risks of unacceptable long-term post construction total and differential settlements.

Lateral Support

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels.

Above the groundwater level, adequate lateral support is provided to a stiff silty clay or compact silty sand bearing medium when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V passes only through in situ soil of the same or higher capacity as the bearing medium soil.

Design For Earthquakes

The site class for seismic site response can be taken as **Class D** for the foundations considered at this site as per Table 4.1.8.4.A of the Ontario Building Code (OBC) 2012. The soils underlying the subject site are not susceptible to liquefaction. Reference should be made to the latest revision of the OBC 2012 for a full discussion of the earthquake design requirements.

Slab-On-Grade Construction

With the removal of all topsoil and deleterious materials within the footprint of the proposed buildings, the native soil surface, approved by Paterson personnel at the time of construction, will be considered an acceptable subgrade on which to commence backfilling for floor slab construction.

The upper 200 mm of sub-slab fill is recommended to consist of OPSS Granular A crushed stone. All backfill material within the footprints of the proposed buildings should be placed in maximum 300 mm thick loose lifts and compacted to a minimum of 98% of its SPMDD.

Any soft areas should be removed and backfilled with OPSS Granular B Type II, with a maximum particle size of 50 mm and compacted to 98% of the material's SPMDD.

Pavement Structure

If required, the pavement structure for car only parking, access lanes and heavy truck parking is presented in Table 1 and Table 2 below.

Table 1 - Recommended Pavement Structure - Driveways and Car Only Parking	
Thickness (mm)	Material Description
50	Wear Course - HL-3 or Superpave 12.5 Asphaltic Concrete
150	BASE - OPSS Granular A Crushed Stone
300	SUBBASE - OPSS Granular B Type II
SUBGRADE - Either approved fill, in situ soil, or OPSS Granular B Type I or II material placed over in situ soil or fill.	

Table 2 - Recommended Pavement Structure - Access Lane and Heavy Truck Parking Areas	
Thickness (mm)	Material Description
40	Wear Course - HL-3 or Superpave 12.5 Asphaltic Concrete
50	Binder Course - HL-8 or Superpave 19 Asphaltic Concrete
150	BASE - OPSS Granular A Crushed Stone
400	SUBBASE - OPSS Granular B Type II
SUBGRADE - Either approved fill, in situ soil, or OPSS Granular B Type I or II material placed over in situ soil or fill.	

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type I or II material. The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment.

Pavement Structure Drainage

Satisfactory performance of the pavement structure is largely dependent on keeping the contact zone between the subgrade material and the base stone in a dry condition. Failure to provide adequate drainage under conditions of heavy wheel loading can result in the fine subgrade soil being pumped into the voids in the stone subbase, thereby reducing its load carrying capacity. Where silty clay is anticipated at subgrade level, consideration should be given to installing subdrains during the pavement construction. These drains should extend in four orthogonal directions or longitudinally when placed along a curb. The clear crushed stone surrounding the drainage lines or the pipe, should be wrapped with suitable filter cloth. The subdrain inverts should be approximately 300 mm below subgrade level. The subgrade surface should be shaped to promote water flow to the drainage lines. Discharge of the subdrains should be directed by gravity to storm sewers or deeper drainage ditches.

Foundation Drainage

Since the buildings will consist of a slab-on-grade construction, a perimeter foundation drainage system is considered optional throughout the landscaped portions of the proposed building footprints. In areas where hardscaping or pavement structures will abut the building footprints, it is recommended to implement a foundation drainage system. The system should consist of a minimum 100 to 150 mm diameter perforated corrugated plastic pipe wrapped in a geosock and surrounded on all sides by 150 mm of minimum 10 mm clear crushed stone. The clear stone should be wrapped in a non-woven geotextile. The pipe should have a positive outlet, such as a gravity connection to the storm sewer.

The pipe should be placed at the footing level around the exterior perimeter of the structure if the backfill between the founding depth and will consist of crushed stone fill or site-generated soil backfill in conjunction with a composite foundation drainage board, such as CCW MiraDRAIN 2000 or Delta-Teraxx. Alternatively, the perimeter drainage pipe may be placed up to 1 m below proposed finished grade and against the building footprint upon approved soil backfill to ensure adequate drainage of the granular fill layer is provided from precipitation events and/or spring meltwater. In this configuration, provided the backfill overlying the pipe consists of crushed stone fill associated with the pavement structure, a composite foundation drainage board will not be required in these areas.

Foundation Backfill

Backfill against the exterior sides of the foundation walls should consist of free-draining, non-frost susceptible imported crushed stone or clean sand fill. Alternatively, consideration may be given to placing site-generated soil fill as backfill against the foundation walls provided the material is compacted in 300 mm thick loose lifts and provided the foundation wall is covered with a composite foundation drainage board layer and associated perimeter drainage pipe with a gravity outlet. If the building's perimeter drainage pipe is located at footing level, a composite foundation drainage board should be placed against the foundation walls to ensure satisfactory drainage of the backfill layer to the perimeter drainage pipe.

All fill placed as foundation backfill should be placed in maximum 300 mm thick loose lifts, compacted using suitable compaction equipment (suitably sized smooth-drum roller for crushed stone fill, sheepsfoot roller for soil fill) and tested for compaction efforts at the time of construction by Paterson personnel.

Protection of Footings Against Frost Action

Perimeter footings of heated structures are required to be insulated against the deleterious effect of frost action. A minimum of 1.5 m thick soil cover should be provided for adequate frost protection of heated structures, or an equivalent combination of soil cover and foundation insulation.

Other exterior unheated footings, such as those for isolated exterior piers are more prone to deleterious movement associated with frost action than the exterior walls of the structure proper and require additional protection. These should be provided with a minimum 2.1 m thick soil cover or a combination of soil cover and foundation insulation.

Excavation Side Slopes

The side slopes of excavations in the soil and fill overburden materials should be either cut back to acceptable slopes or should be retained by shoring systems from the start of the excavation until the structure is backfilled. It is expected that sufficient room will be available for the greater part of the excavation to be undertaken by open-cut methods (i.e. unsupported excavations).

Unsupported Excavations

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be cut back to 1H:1V or flatter. The flatter slope is required for excavation below groundwater level. The subsurface soils are considered to be Type 2 and Type 3 soil according to the Occupational Health and Safety Act and Regulations.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should be kept away from the excavation sides. Slopes in excess of 3 m in height should be periodically inspected by Paterson field personnel in order to detect if the slopes are exhibiting signs of distress.

Excavation side slopes around the building excavation should be protected from erosion by surface water and rainfall events by the use of secured tarpaulins spanning the length of the side slopes, or other means of erosion protection along their footprint. Efforts should also be made to maintain dry surfaces at the bottom of the excavation footprints and along the bottom of side slopes. Additional measures may be recommended at the time of construction by the geotechnical consultant.

A trench box is recommended to protect personnel working in trenches with steep or vertical sides. It is expected that services will be installed by “cut and cover” methods and excavations will not be left open for extended periods of time.

Winter Construction

If winter construction is considered for this project, precautions should be provided for frost protection. The subsurface soil conditions mainly consist of frost susceptible materials. In the presence of water and freezing conditions, ice could form within the soil mass. Heaving and settlement upon thawing could occur.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the installation of straw, propane heaters and tarpaulins or other suitable means. The excavation base should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Trench excavations and pavement construction are also difficult activities to complete during freezing conditions without introducing frost in the subgrade or in the excavation walls and bottoms. Precautions should be taken if such activities are to be carried out during freezing conditions. Additional information could be provided, if required.

Corrosion Potential and Sulphate

The results of analytical testing show that the sulphate content is less than 0.1%. This result is indicative that Type 10 Portland cement (normal cement) would be appropriate for this site. The chloride content and the pH of the sample indicate that they are not significant factors in creating a corrosive environment for exposed ferrous metals at this site, whereas the resistivity is indicative of a non-aggressive to slightly aggressive corrosive environment.

Geotechnical Limit of Hazard Lands

Reference should be made to Paterson's previous letter report PG1887-LET-01 Revision 3 dated February 28, 2017 which is attached to the present report and documents our slope stability analysis and determination of the geotechnical Limit of Hazard Lands designation line for the subject site. The Limit of Hazard Lands determined at that time is considered applicable for the proposed development and the subject site. The associated Limit of Hazard Lands and location of our cross-sections studied as part of our slope stability analysis are depicted on Drawing PG6557-1 – Test Hole Location Plan attached to the present letter report.

In summary, the Limit of Hazard Lands is formed of a combination of setbacks considering the stable slope allowance (varies between 9.5 and 11.6 m), toe erosion allowance (considered as 8 m) and an erosion access allowance (considered as 6 m). The Limit of Hazard Lands is based on the results of our analysis which were undertaken in accordance with the City of Ottawa's *Slope Stability Guidelines for Development Application in the City of Ottawa*.



3.0 Recommendations

A materials testing and observation services program is a requirement for the provided foundation design recommendations to be applicable. The following aspects of the program should be performed by Paterson:

- A full geotechnical investigation should be completed once conceptual details of the proposed development are available.
- Review of the grading and servicing plans from a geotechnical perspective.
- Sampling and testing of the concrete and fill materials used.
- Observation of all bearing surfaces prior to the placement of concrete (within 24 hours).
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable
- Observation of the placement of the foundation insulation, if applicable.
- Observation of all subgrades prior to backfilling.
- Field density tests to determine the level of compaction achieved.
- Sampling and testing of bituminous concrete including mix design reviews.

A report confirming that the construction has been conducted in general accordance with Paterson's recommendations could be issued upon the completion of a satisfactory inspection program by the Paterson consultant.

All excess soils should be handled as per *Ontario Regulation 406/19: On-Site and Excess Soil Management*.

4.0 Statement of Limitations

The recommendations provided in the report are in accordance with Paterson's present understanding of the project. Paterson requests permission to review the recommendations when the drawings and specifications are completed.

The client should be aware that any information pertaining to soils and all test pit logs are furnished as a matter of general information only and test hole descriptions or logs are not to be interpreted as descriptive of conditions at locations other than those of the test holes.

A soils investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from the test locations, Paterson requests immediate notification to permit reassessment of the recommendations.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Myers Automotive Group or their agents, is not authorized without review by Paterson Group Inc. for the applicability of our recommendations to the altered use of the report.

We trust that the current submission meets your immediate requirements.

Best Regards,

Paterson Group Inc.



Drew Petahtegoose, P.Eng.



David J. Gilbert, P.Eng.

Attachments

- Soil Profile and Test Data Sheets
- Symbols and Terms
- Figure 1 - Key Plan
- Drawing PG6557-1 - Test Hole Location Plan
- Report PG1887-LET-01 Revision 3 dated February 28, 2017

Report Distribution

- Myers Automotive Group (e-mail copy)
- Paterson Group (1 copy)

DATUM

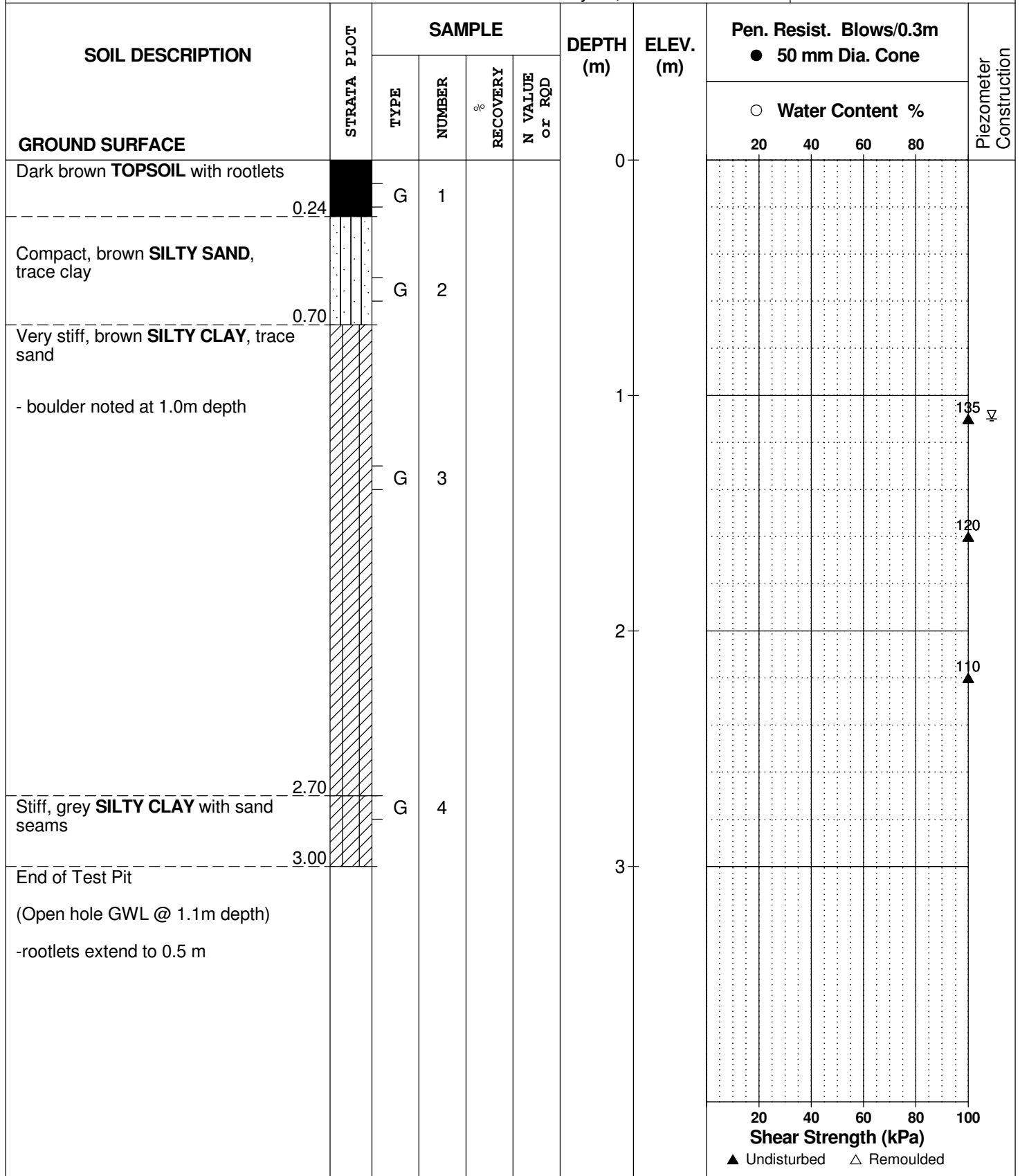
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BORINGS BY Rubber Tired Backhoe

DATE May 11, 2017

FILE NO. **PG4120**

HOLE NO. **TP 1-17**



DATUM

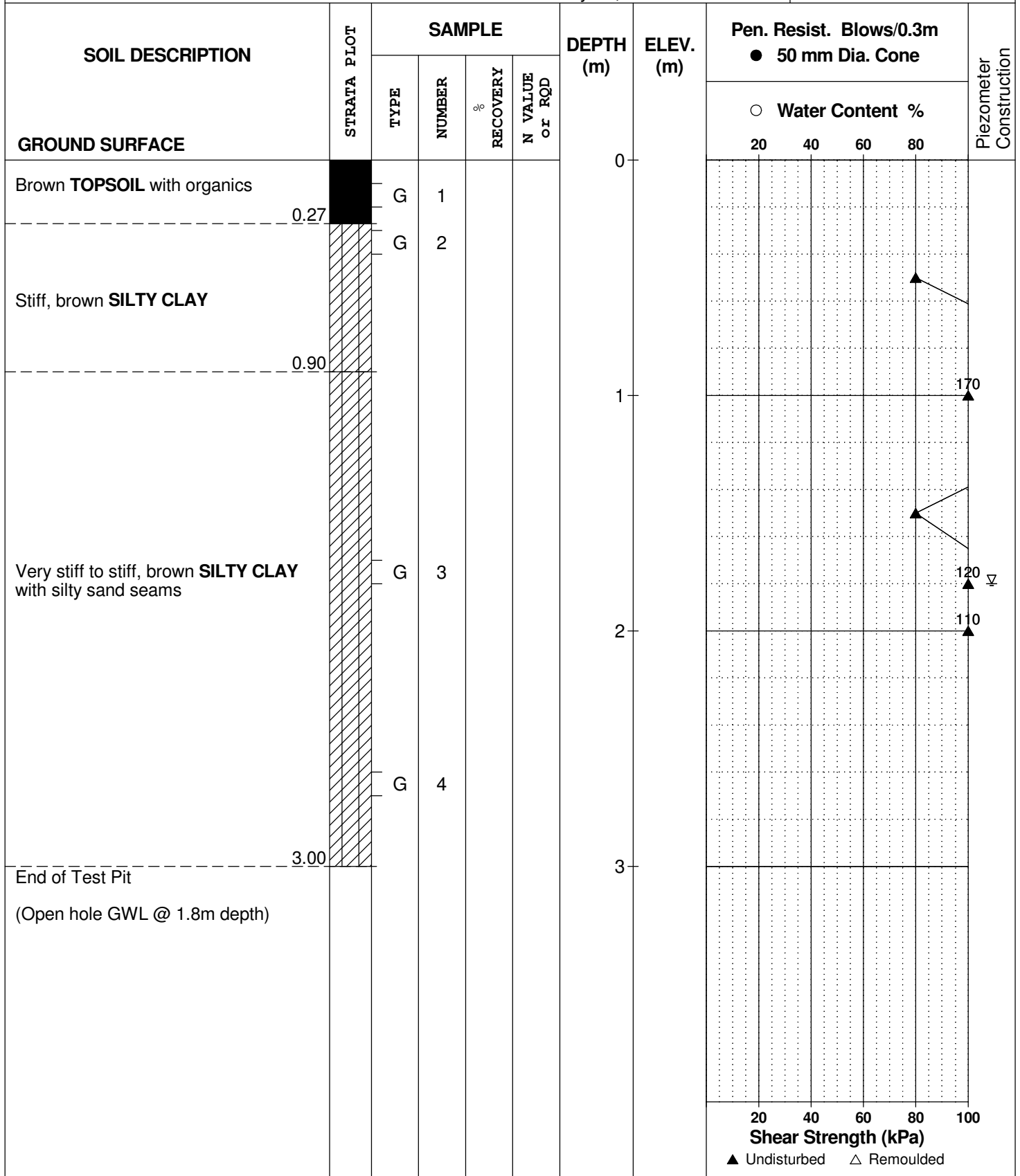
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BORINGS BY Rubber Tired Backhoe

DATE May 11, 2017

FILE NO. **PG4120**

HOLE NO. **TP 3-17**



DATUM

REMARKS Co-ordinates: 45.332538N, 75.698942W

BORINGS BY Rubber Tired Backhoe

DATE May 11, 2017

FILE NO. **PG4120**

HOLE NO. **TP 4-17**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
GROUND SURFACE													
Brown TOPSOIL with rootlets		G	1			0							
Dense, brown SILTY SAND , some clay		G	2			0.20							
Very stiff, brown SILTY CLAY with sand seams		G	3			0.50							
		G	4			1.40							▲ 170
Compact, brown SILTY SAND		G	5										
End of Test Pit (TP dry upon completion)						3.00							

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM

REMARKS Co-ordinates: 45.332822N, 75.699174W

BORINGS BY Rubber Tired Backhoe

DATE May 11, 2017

FILE NO. **PG4120**

HOLE NO. **TP 5-17**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80	
GROUND SURFACE						0						
Brown TOPSOIL with rootlets		G	1			0.25						
Compact to dense, brown SILTY SAND , some clay, trace cobbles		G	2			0.90						
Compact, brown SILTY SAND , trace gravel and boulders		G	3			1.40						
Very stiff, brown SILTY CLAY with sand, trace cobbles		G	4			2.20						130
Compact, brown SILTY SAND , some clay		G	5			3.00						∇
End of Test Pit (Open hole GWL @ 2.7m depth)												

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM

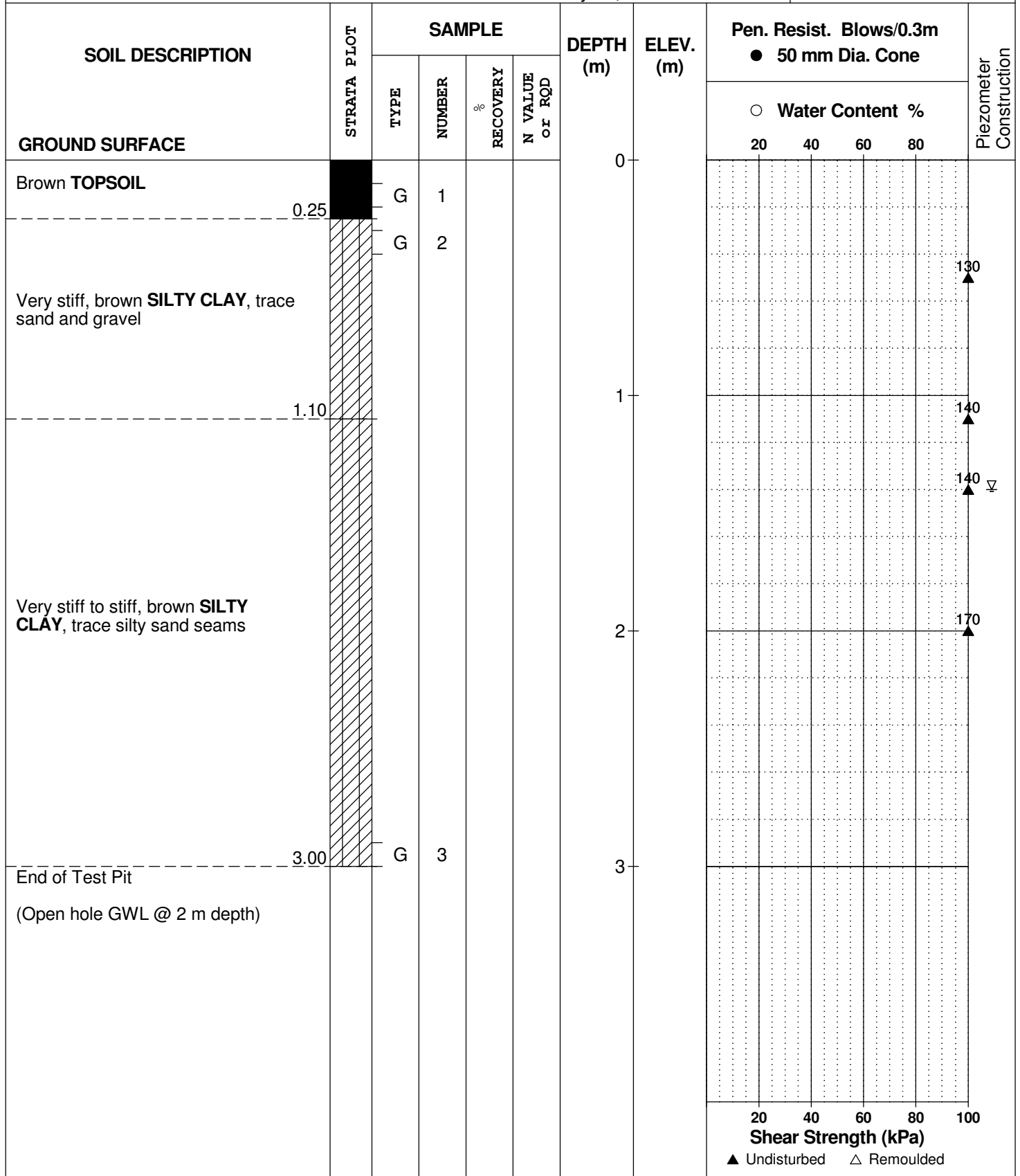
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BORINGS BY Rubber Tired Backhoe

DATE May 11, 2017

FILE NO. **PG4120**

HOLE NO. **TP 8-17**



DATUM

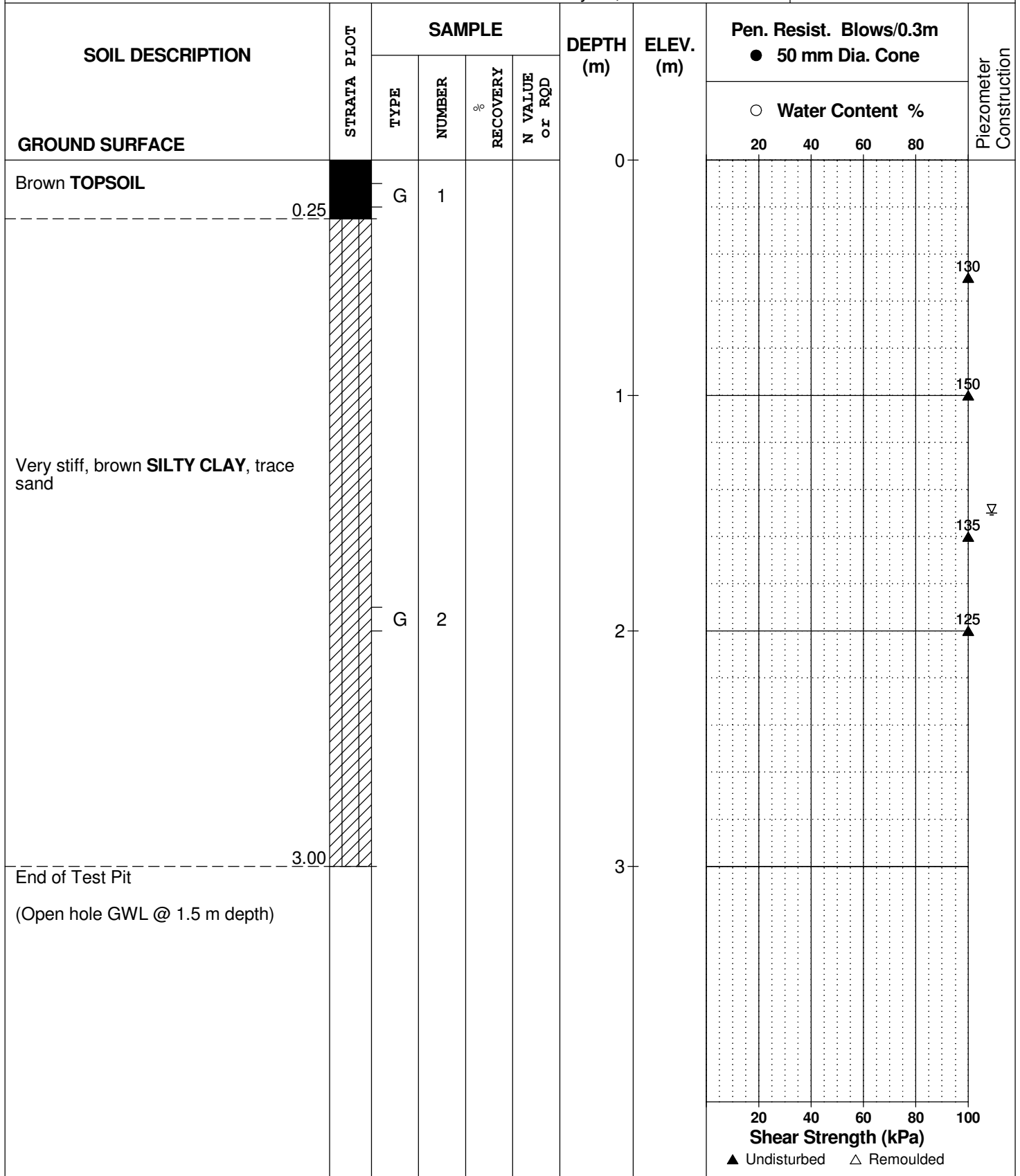
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BORINGS BY Rubber Tired Backhoe

DATE May 11, 2017

FILE NO. **PG4120**

HOLE NO. **TP 9-17**





JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers

28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment

Waterbend Lane

Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

FILE NO.

G8824

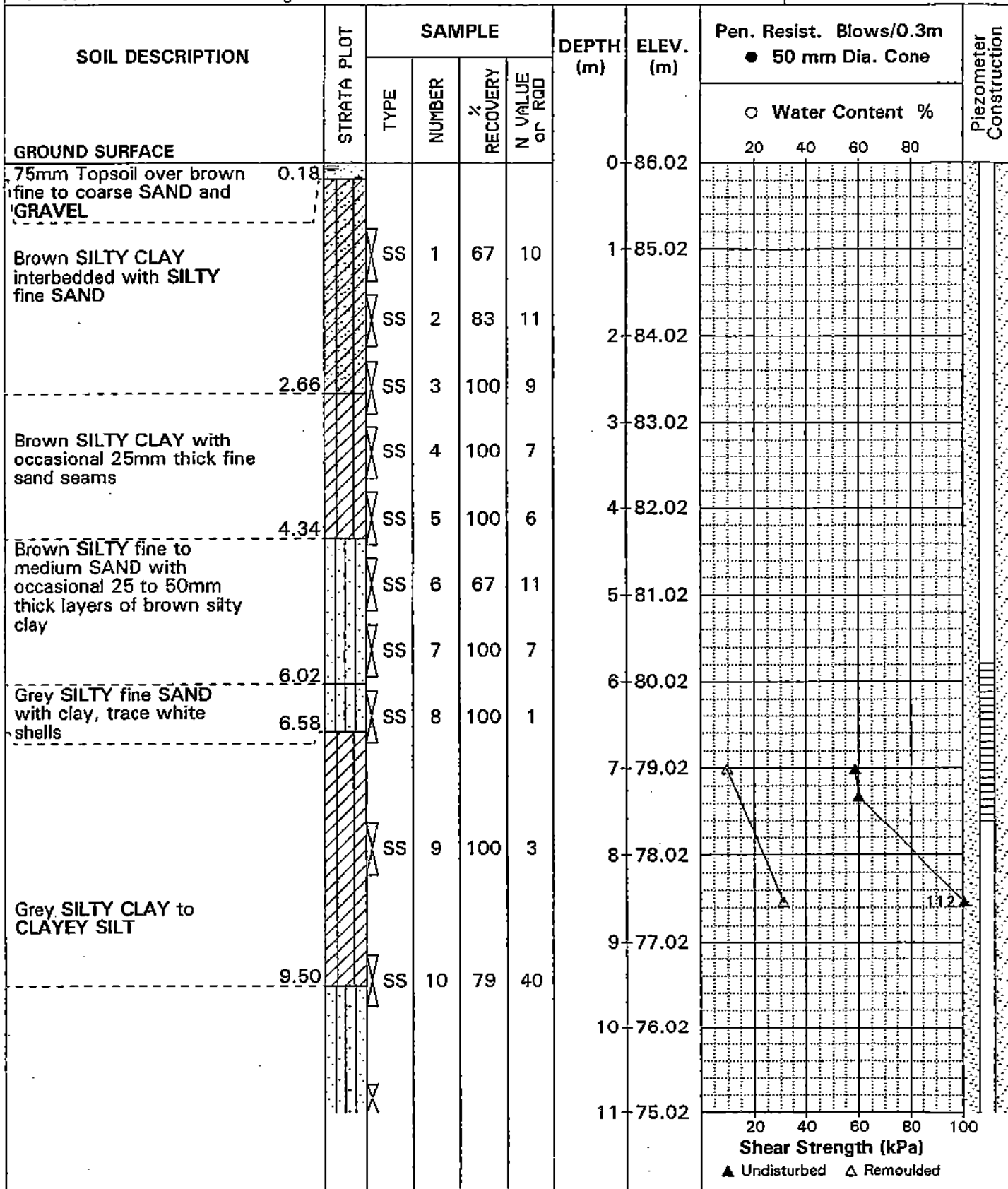
REMARKS

HOLE NO.

BH 1

BORINGS BY CME 55 Power Auger

DATE 12 DEC 02





JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers
28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment
Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

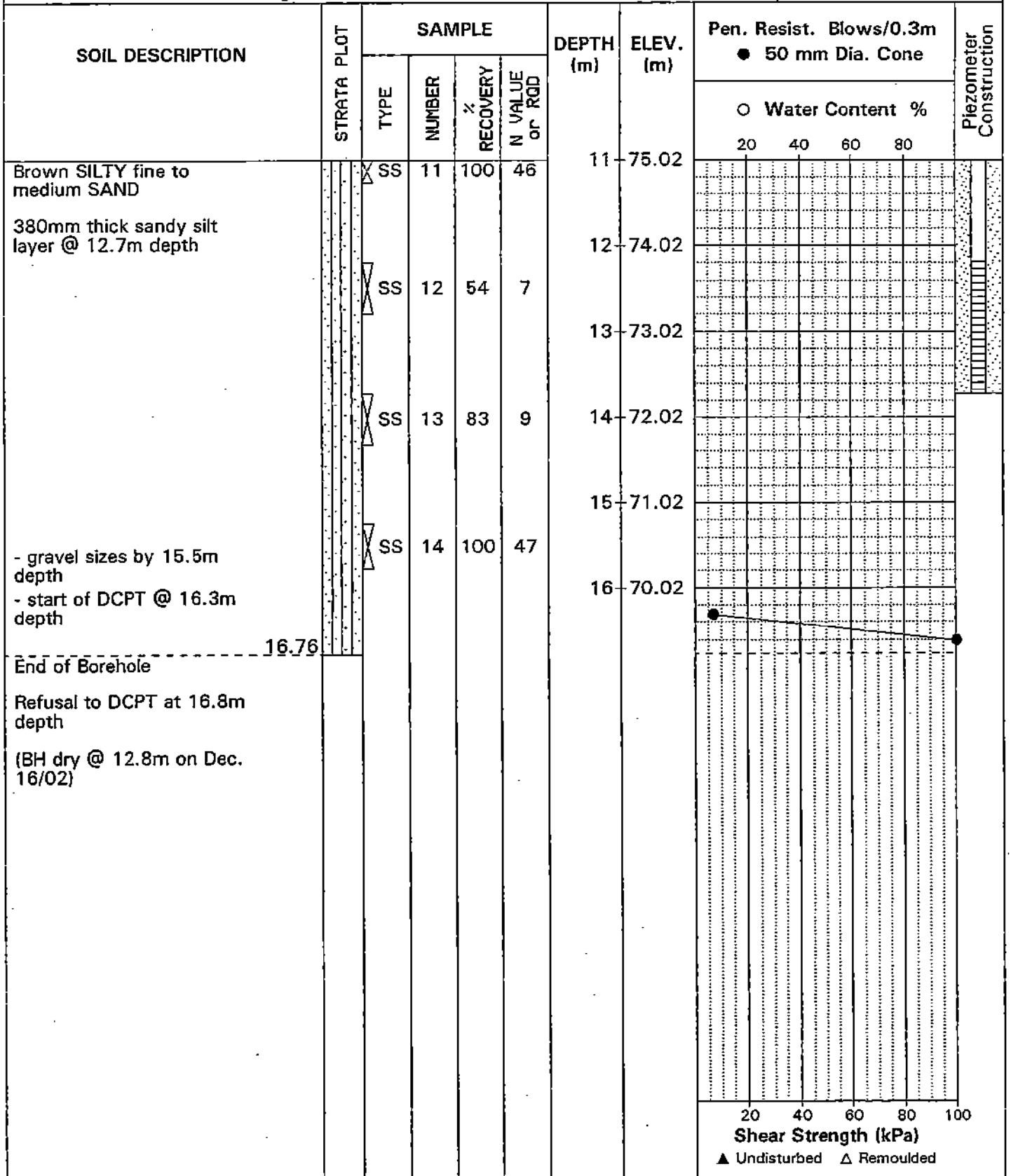
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REMARKS

HOLE NO. **BH 1**

BORINGS BY CME 55 Power Auger

DATE 12 DEC 02





JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers

28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment

Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

FILE NO.

G8824

REMARKS

HOLE NO.

BH 2-03

BORINGS BY Portable Drill

DATE 21 MAR 03

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	% RECOVERY	N VALUE or RGD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE													
Brown SILTY fine SAND	0.30	G	1			0	74.93						
Brown SILTY CLAY, some sand	1.20	SS	2	71	2	1	73.93						
SS		SS	3	58	13								
Compact, brown SILTY fine SAND		SS	4	100	11	2	72.93						
- 25mm thick silty clay seam at 2.12m depth		SS	5	62	14								
- 50mm thick grey silt, trace clay @ 2.46m depth	2.74	SS	6	100	12	3	71.93						
Compact, grey SILTY fine SAND		SS	7	100	12								
- 10mm thick silty clay seam by 4.2m depth	4.57	SS	8	100	13	4	70.93						
End of Borehole													
(Open hole GWL @ 0.2m depth)													

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded



JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers

28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment

Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

FILE NO.

G8824

REMARKS

HOLE NO.

BH 3-03

BORINGS BY Portable Drill

DATE 21 MAR 03

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE													
Brown SILTY fine to medium SAND	0.15	G	1			0	74.89						#4
		SS	2	67	4								
Loose, grey SILTY fine SAND with thin grey silty clay layers		SS	3	100	5	1	73.89						
		SS	4	100	5								
	2.13	SS	5	100	11	2	72.89						
		SS	6	100	8								
Loose to compact, grey to grey-brown SILTY fine to coarse SAND		SS	7	100	13	3	71.89						
		SS	8	92	22	4	70.89						
End of Test Pit (Open hole GWL @ 0.3m depth)	4.57												

20 40 60 80 100

Shear Strength (kPa)

▲ Undisturbed Δ Remoulded



JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers
28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment
Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

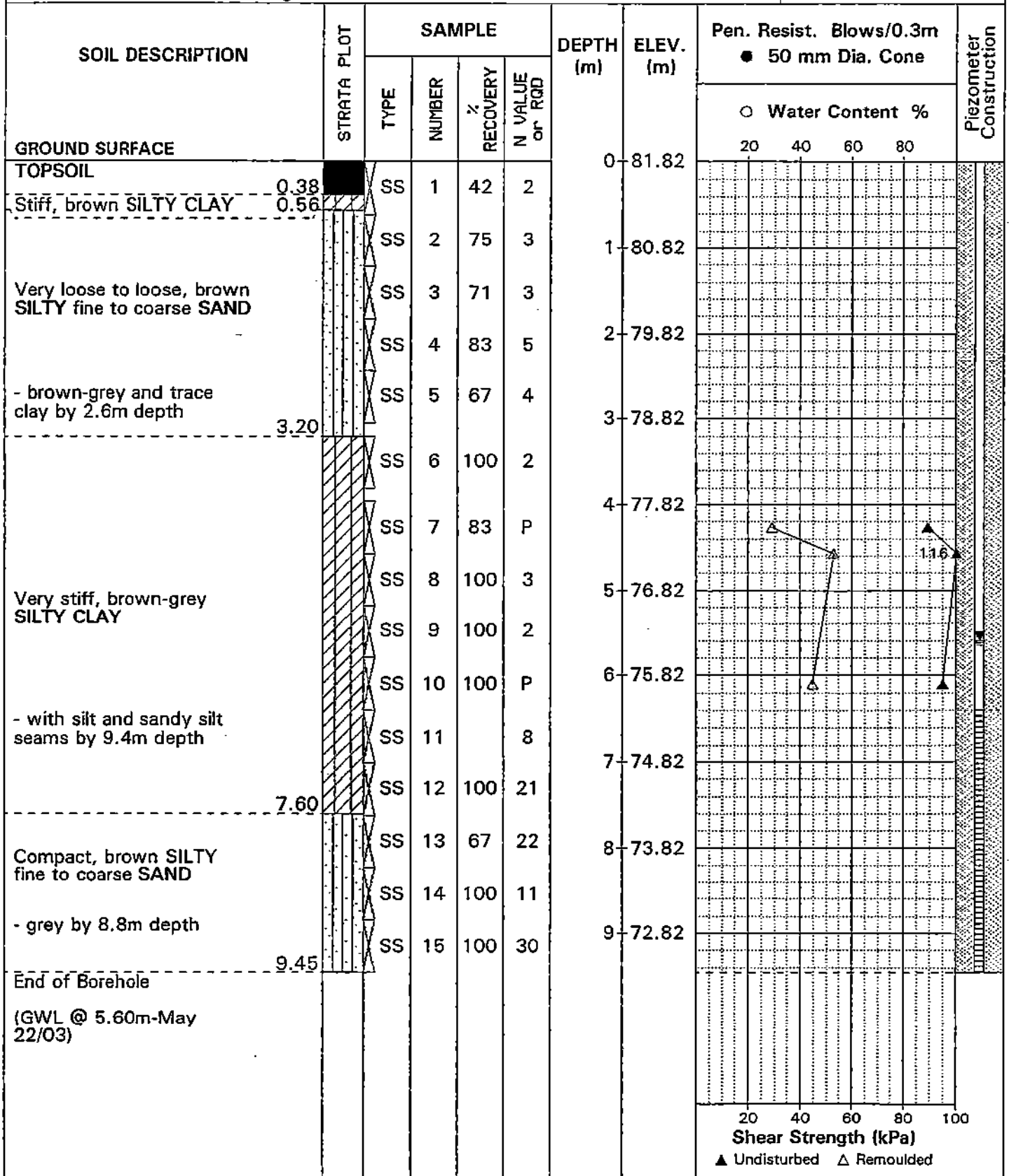
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REMARKS

HOLE NO. **BH 4-03**

BORINGS BY CME 55 Power Auger

DATE 16 MAY 03





JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers

28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment

Waterbend Lane

Ottawa, Ontario

DATUM Geodetic, estimated.

FILE NO.

G8824

REMARKS

HOLE NO.

BH 5-03

BORINGS BY Portable Drill

DATE 8 MAY 03

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
GROUND SURFACE								20	40	60	80	
TOPSOIL	0.44	SS	1	48	3	0	85.66					
Very loose to loose, brown SILTY fine to medium SAND	1.47	SS	2		3	1	84.66					
Very stiff, brown-grey SILTY CLAY - occasional silty sand layers by 2.7m depth - grey by 3.1m depth		SS	3		6	2	83.66					
		SS	4		10	3	82.66					
		SS	5	62	14	4	81.66					
		SS	6	88	10	5	80.66					
		SS	7	58	7	6	79.66					
Very loose to loose, brown-grey SILTY SAND, trace clay - 0.3m thick silty clay at 6.1m depth	5.00	SS	8	71	10	7	78.66					
		SS	9	75	7	8	77.66					
		SS	10	96	8	9	76.66					
		SS	11	54	4	10	75.66					
		SS	12	100	3	11	74.66					
Firm to stiff, grey SILTY CLAY, trace sand	8.80	SS	13	100	2	12						
		SS	14	100	2	13						
		SS	15		P	14						
		SS	16	100	3	15						
		SS	17	100	6	16						

Shear Strength (kPa)
▲ Undisturbed △ Remoulded



JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers
28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment
Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, estimated.

FILE NO. **G8824**

REMARKS

HOLE NO. **BH 5-03**

BORINGS BY Portable Drill

DATE 8 MAY 03

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Firm to stiff, grey SILTY CLAY, trace sand		SS	18	100	3	11	74.66					
		SS	19	100	10	12	73.66					
End of Borehole	12.34											
(GWL @ 10.5m-May 22/03)												

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded



JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers
28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment
Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

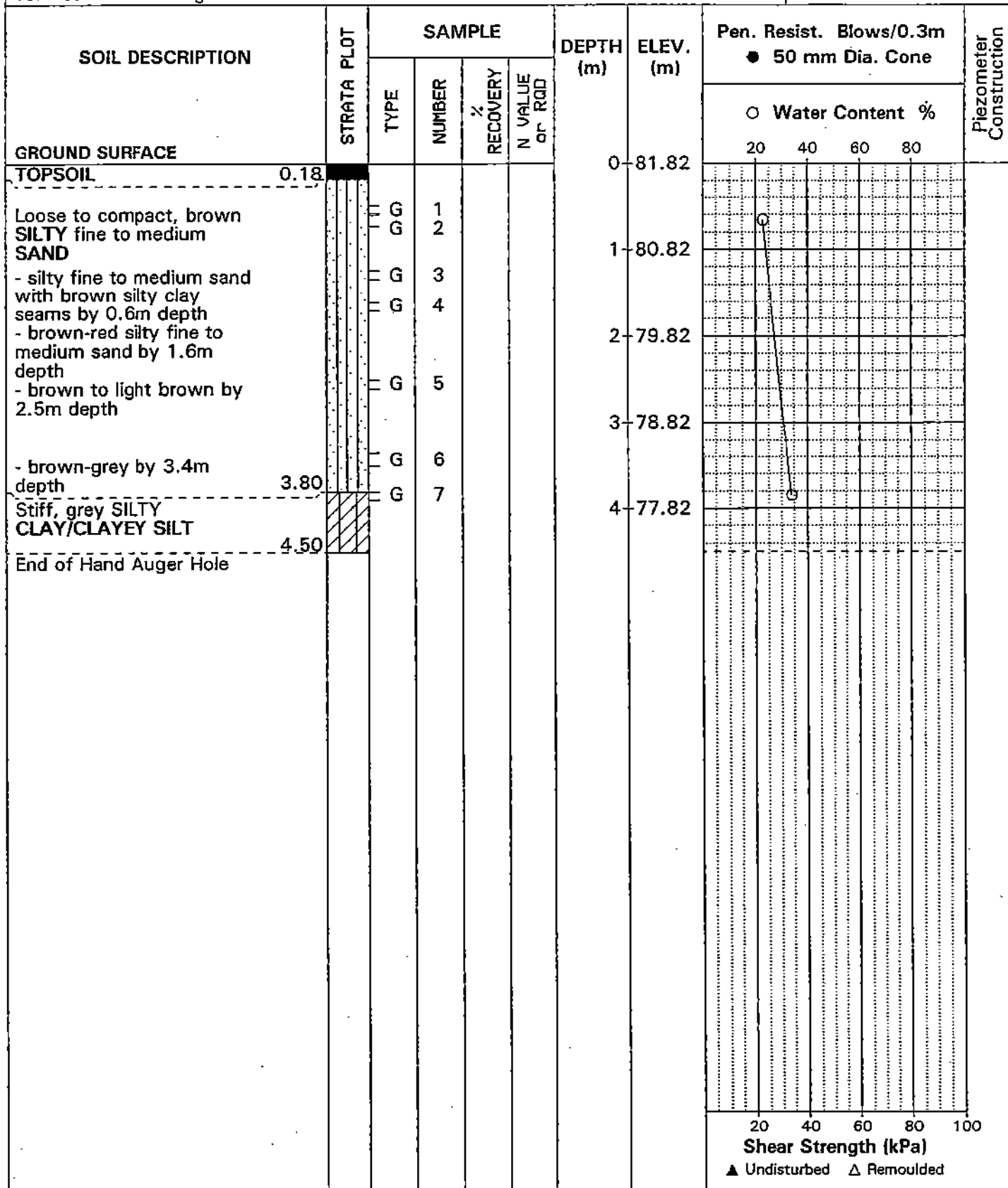
FILE NO. **G8824**

REMARKS

HOLE NO. **HA 1**

BORINGS BY Hand Auger

DATE 28 NOV 02





JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers
28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment
Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

FILE NO. **G8824**

REMARKS

HOLE NO. **HA 2**

BORINGS BY Hand Auger

DATE 28 NOV 02

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone		Piezometer Construction
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %		
GROUND SURFACE						0	74.93			
SILTY fine SAND	0.05									
Stiff, grey SILTY CLAY, trace sand	0.70	G	8							
		G	9			1	73.93			
Compact, grey SANDY SILT, trace clay		G	10			2	72.93			
End of Hand Auger Hole	2.49									
(Open hole GWL @ 2.2m depth)										

Shear Strength (kPa)	
20	40 60 80 100
▲ Undisturbed	△ Remoulded



JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers

28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment

Waterbend Lane

Ottawa, Ontario

DATUM Geodetic, estimated.

FILE NO.

G8824

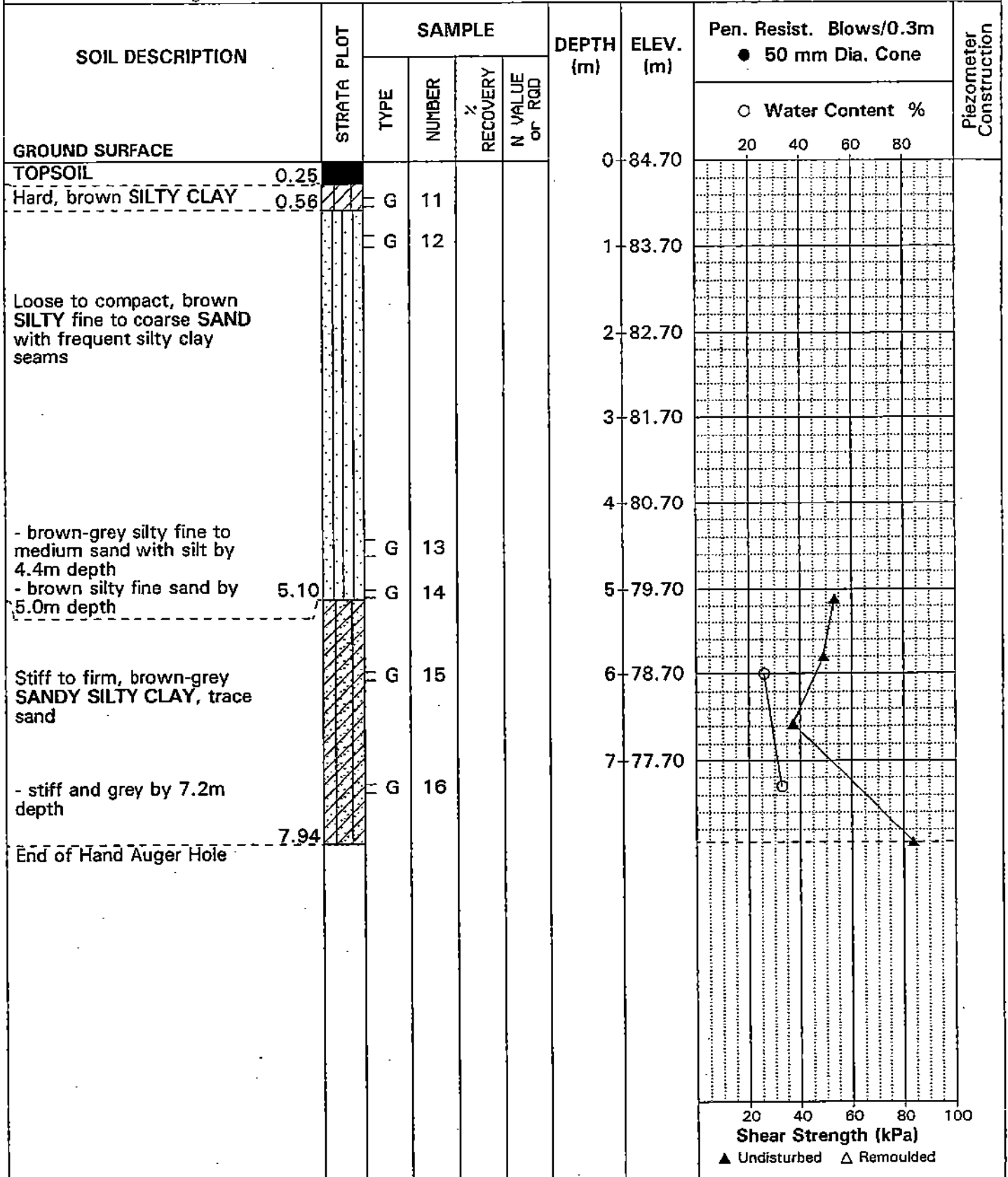
REMARKS

HOLE NO.

HA 3

BORINGS BY Hand Auger

DATE 28 NOV 02





JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers

28 Concourse Gate, Nepean, Ont. K2E 7T7

SOIL PROFILE & TEST DATA

Riverbank Failure Assessment

Waterbend Lane
Ottawa, Ontario

DATUM Geodetic, as provided by Cumming Cockburn Limited.

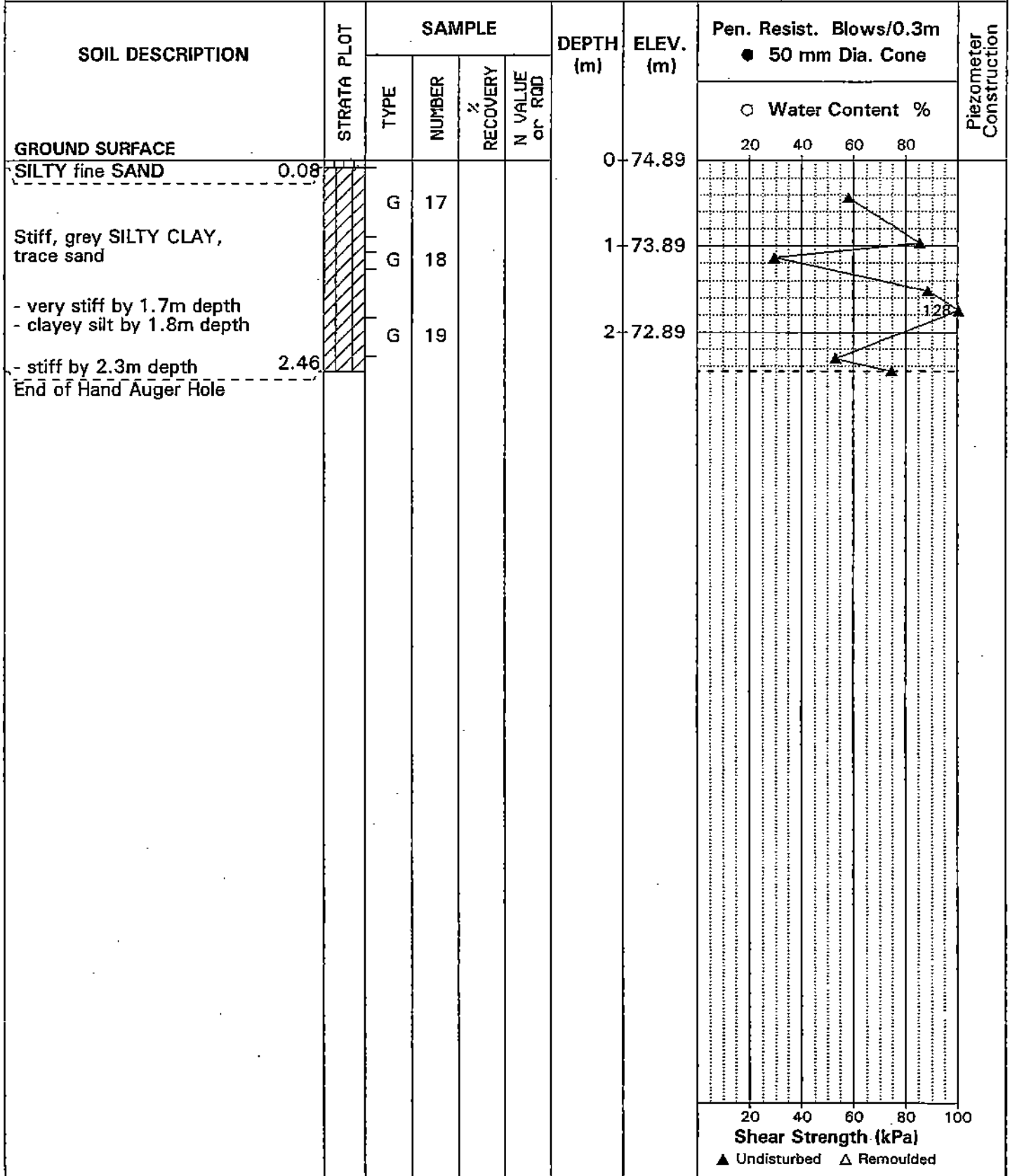
FILE NO. **G8824**

REMARKS

HOLE NO. **HA 4**

BORINGS BY Hand Auger

DATE 29 NOV 02



20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

SOIL PROFILE AND TEST DATA			JOHN D. PATERSON & ASSOCIATES LTD. Consulting Engineers & Geologists Soil Investigations Inspection & Testing Services Damage Claims				Offices & Laboratory 1479 Laperriere Ave. Ottawa, Canada K1Z 7S8 Telephone (613) 728-3505		SHEET NO. <u>1</u> OF <u>3</u> HOLE NO. <u>BH 1</u>																		
Proposed Residential Subdivision South 1/2 Lot 26, Concession "A", R.F. Nepean, Ontario							GROUND SURFACE <u>86.04</u> BOTTOM HOLE <u>73.84</u>		BEDROCK _____ GROUNDWATER <u>DRY</u>																		
DESCRIPTION	LEGEND	SAMPLE TYPE	SAMPLE NUMBER	ELEV. DEPTH	WATER CONTENT %								UNIT WEIGHT kN/m ³				SHEAR STRENGTH (kPa)						STANDARD (N) PENETRATION TEST RESISTANCE				WATER LEVEL
					10	20	30	40	50	60	70	80	5	10	15	20	20	40	60	80	100	120	140	20	40	60	
Ground Surface				86.04																							
250 mm TOPSOIL over a compact grey interbedded FINE SAND and SANDY SILT 08		G	1	0.00																							
Very stiff to stiff fissured olive grey SILTY CLAY with pinkish grey banding containing fine sand seams 3.6		SS	2	0.80																							
		TW	3	1.60																							
		TW	4	2.40																							
		TW	5	2.40																							
		TW	6	3.20																							
		TW	6	3.20																							
Compact brown SILTY FINE SAND containing clayey silt seams approximately 5 mm thick 5.5		SS	7	82.04																							
		SS	8	4.00																							
		SS	9	4.80																							
Very dense grey SANDY SILT containing silty fine sand seams 6.7		SS	10	5.60																							
		SS	11	6.40																							
Dense pale grey FINE SAND with some hairlike black banding. Becoming coarser with depth. 12.2		SS	12																								
		SS	13	7.20																							
		SS	14	78.04																							
		SS	14	8.00																							
		SS	15	8.80																							
		SS	16	9.60																							
		SS	17																								
		SS	18	10.40																							
		SS	19	11.20																							
		SS	20	74.04																							
Borehole terminated in sand				12.00																							
				12.80																							
												(psf)		1000	2000	3000	BLOWS / 0.3m.										

SOIL PROFILE AND TEST DATA		JOHN D. PATERSON & ASSOCIATES LTD.				SHEET NO. 3 OF 3																						
Proposed Residential Subdivision South 1/2 Lot 26, Concession "A", R.F. Nepean, Ontario		Consulting Engineers & Geologists		Offices & Laboratory		HOLE NO. BH 3																						
		Soil Investigations		1479 Laperriere Ave.		GROUND SURFACE 85.53 BOTTOM HOLE 73.33																						
		Inspection & Testing Services		Ottawa, Canada K1Z 7S8		BEDROCK _____ GROUNDWATER DRY																						
		Damage Claims		Telephone (613) 728-3505																								
DESCRIPTION	LEGEND	SAMPLE TYPE	SAMPLE NUMBER	ELEV.	WATER CONTENT								UNIT WEIGHT				SHEAR STRENGTH (kPa)				STANDARD (N) PENETRATION TEST				WATER LEVEL			
				DEPTH	%								kN/m ³				▲ UNDISTURBED △ REMOULDED				○ PENETRATION RESISTANCE							
Ground Surface				85.53	10	20	30	40	50	60	70	80	5	10	15	20	20	40	60	80	100	120	140	20	40	60	80	
250 mm TOPSOIL over a loose brown SANDY SILT interbedded with clayey silt & sand		G	35	0.00																								
		SS	36	0.80																								
Stiff olive grey fissured SILTY CLAY containing brown fine sand seams at 50 mm ± intervals		SS	37	1.60																								
		TW	38	1.60																								
		TW	39	2.40																								
		SS	40	3.20																								
		SS	41	81.53																								
		SS	42	4.00																								
		SS	43	4.80																								
Compact brown SILTY FINE SAND containing clayey silt seams		SS	44	5.60																								
		SS	45	6.40																								
		TW	46	7.20																								
		TW	47	77.53																								
		TW	48	8.00																								
Firm to stiff grey fissured SILTY CLAY with occasional fine sand lenses and containing fine sand seams		TW	49	8.80																								
		TW	50	9.60																								
		TW	51	10.40																								
STRATIFIED SILT: grey compact layers of silty sand, sandy silt and stiff silty clay		TW	52	11.20																								
		TW	53	73.53																								
				12.00																								
Borehole terminated in silt																												
																			</									

SYMBOLS AND TERMS

SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

SYMBOLS AND TERMS (continued)

SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their "sensitivity". The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called "mechanical breaks") are easily distinguishable from the normal in situ fractures.

RQD %	ROCK QUALITY
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube
PS	-	Piston sample
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.

SYMBOLS AND TERMS (continued)

GRAIN SIZE DISTRIBUTION

MC%	-	Natural moisture content or water content of sample, %
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)
PL	-	Plastic limit, % (water content above which soil behaves plastically)
PI	-	Plasticity index, % (difference between LL and PL)
Dxx	-	Grain size which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size
D10	-	Grain size at which 10% of the soil is finer (effective grain size)
D60	-	Grain size at which 60% of the soil is finer
Cc	-	Concavity coefficient = $(D_{30})^2 / (D_{10} \times D_{60})$
Cu	-	Uniformity coefficient = D_{60} / D_{10}

Cc and Cu are used to assess the grading of sands and gravels:

Well-graded gravels have: $1 < Cc < 3$ and $Cu > 4$

Well-graded sands have: $1 < Cc < 3$ and $Cu > 6$

Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.

Cc and Cu are not applicable for the description of soils with more than 10% silt and clay (more than 10% finer than 0.075 mm or the #200 sieve)

CONSOLIDATION TEST

p'_o	-	Present effective overburden pressure at sample depth
p'_c	-	Preconsolidation pressure of (maximum past pressure on) sample
Ccr	-	Recompression index (in effect at pressures below p'_c)
Cc	-	Compression index (in effect at pressures above p'_c)
OC Ratio		Overconsolidation ratio = p'_c / p'_o
Void Ratio		Initial sample void ratio = volume of voids / volume of solids
Wo	-	Initial water content (at start of consolidation test)

PERMEABILITY TEST

k	-	Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.
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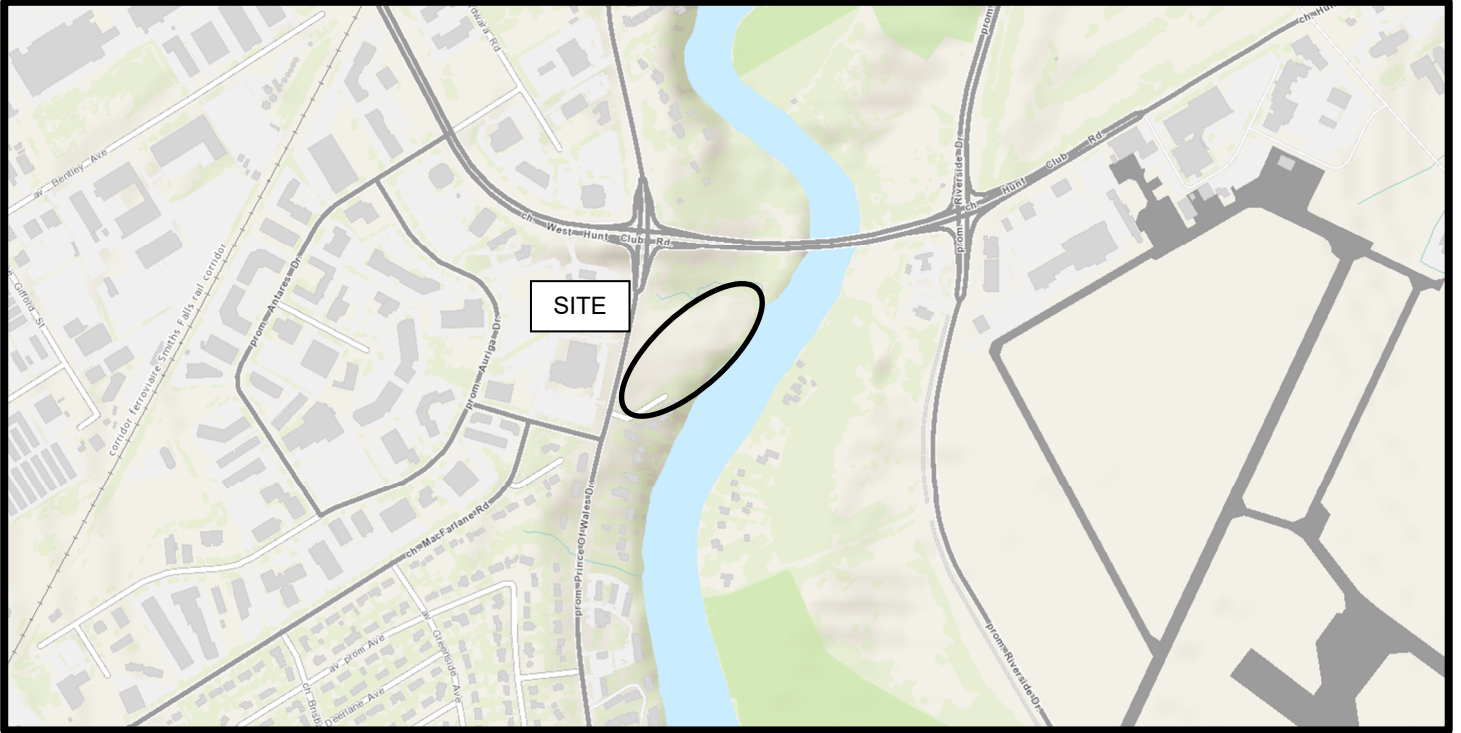
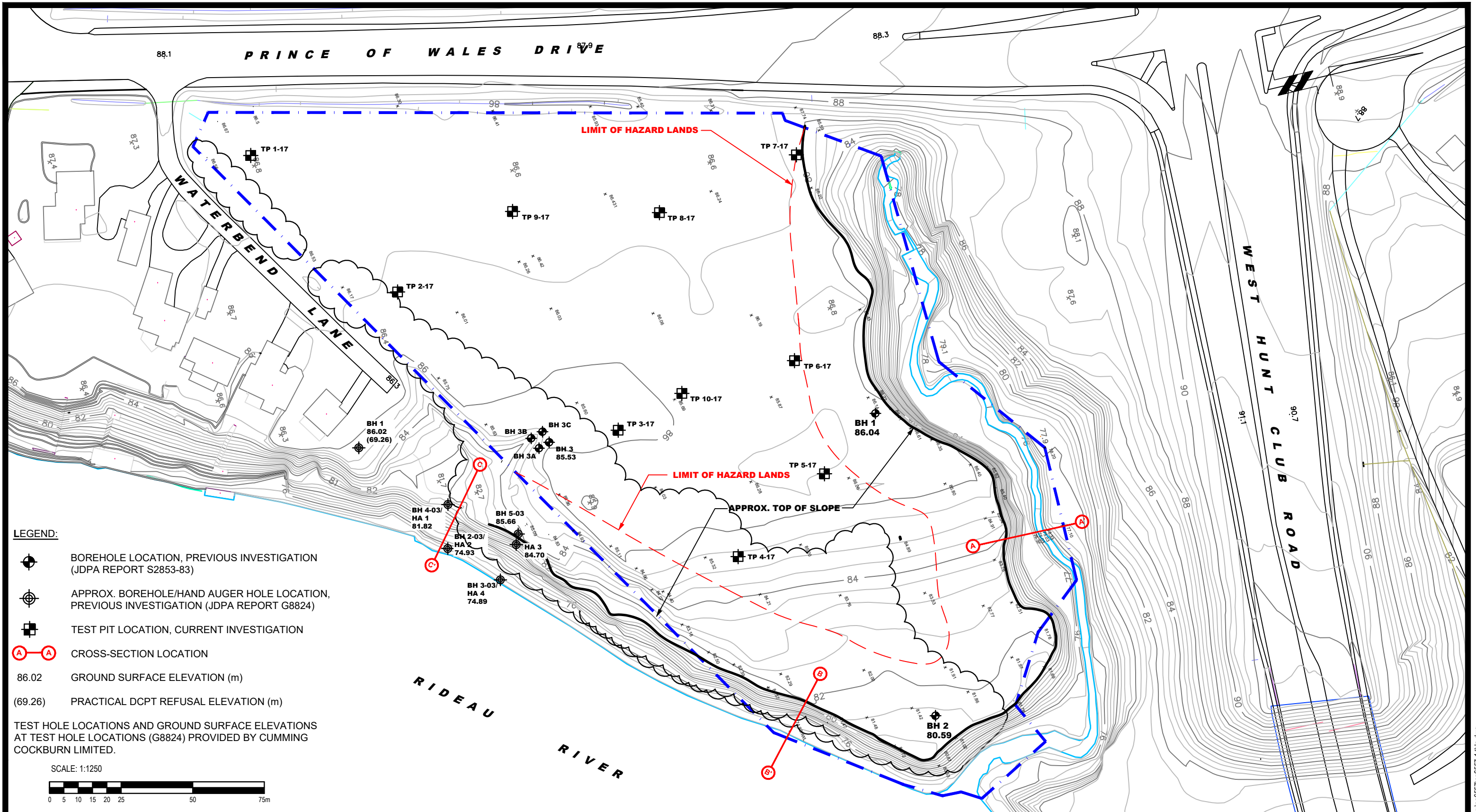






FIGURE 1

KEY PLAN



LEGEND:

-  BOREHOLE LOCATION, PREVIOUS INVESTIGATION (JDPA REPORT S2853-83)
-  APPROX. BOREHOLE/HAND AUGER HOLE LOCATION, PREVIOUS INVESTIGATION (JDPA REPORT G8824)
-  TEST PIT LOCATION, CURRENT INVESTIGATION
-  CROSS-SECTION LOCATION
- 86.02 GROUND SURFACE ELEVATION (m)
- (69.26) PRACTICAL DCPT REFUSAL ELEVATION (m)

TEST HOLE LOCATIONS AND GROUND SURFACE ELEVATIONS AT TEST HOLE LOCATIONS (G8824) PROVIDED BY CUMMING COCKBURN LIMITED.

SCALE: 1:1250




PATERSON GROUP
 9 AURIGA DRIVE
 OTTAWA, ON
 K2E 7T9
 TEL: (613) 226-7381

NO.	REVISIONS	DATE	INITIAL

MYERS AUTOMOTIVE GROUP
PRELIMINARY GEOTECHNICAL INVESTIGATION
2175 PRINCE OF WALES DRIVE
 OTTAWA, ONTARIO

Title: **TEST HOLE LOCATION PLAN**

Scale:	1:1250	Date:	06/2024
Drawn by:	NFRV	Report No.:	PG6557-1
Checked by:	FC	Dwg. No.:	PG6557-1
Approved by:	DP	Revision No.:	

154 Colonnade Road South
Ottawa, Ontario
Canada, K2E 7J5
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Fax: (613) 226-6344

February 28, 2017
File: PG1887-LET.01 Revision 3

Mr. Scott Thomson
3 Lemon Point Lane
Prescott, Ontario
K0E 1T0

Geotechnical Engineering
Environmental Engineering
Archaeological Services
Hydrogeology
Geological Engineering
Materials Testing
Building Science

www.patersongroup.ca

Subject: **Slope Stability Analysis**
2175 Prince of Wales Drive
Ottawa, Ontario

Dear Sir,

Further to your request, Paterson Group (Paterson) has conducted a slope stability analysis and determined the limit of hazard lands for the aforementioned site. The limit of hazard lands for the subject site extends along the west side of the Rideau River and along the south side of a ravine containing a tributary watercourse to the Rideau River. The present letter summarizes our findings from a geotechnical perspective.

The subject site is presently undeveloped and has an approximate area of 3.23 hectares. The majority of the subject site is grassed covered and slopes gradually downward to the west towards the Rideau River. The subject site is bordered by a ravine to the north, the Rideau River to the east, Waterbend Lane followed by residential housing to the south and Prince of Wales Drive to the west. A topographic survey was completed by Paterson to provide spot grade elevations across the subject site and three (3) slope cross sections were completed for our slope stability analysis. One slope cross section was completed for the area that has undergone a slope remedial repair after slope toe erosion activities have caused slip failures.

A previous geotechnical investigation was completed by John D. Paterson and Associates (JDPA) for the subject site with the findings presented under cover Report S2853-83 dated December 30, 1983.

1.0 Existing Slope Conditions and Soils Information

The south valley corridor wall of the drainage ravine along the north property boundary was noted to be vegetated with small brush and signs of erosion occurring at several localized outbends in the watercourse/creek channel. A 2 to 3 m wide watercourse was noted to meander throughout the valley corridor. The water depth was noted to vary between approximately 0.2 to 0.3 m.

Along the east property boundary, the west valley corridor wall of the Rideau River is undergoing active erosion within several areas, the slope was noted to have been undercut at the toe. It is expected that historical erosional activities have resulted in currently observed steep back scarp slope. Currently, the majority of the bank was vegetated with small brush and full grown trees (mainly deciduous). A previous slope slip failure due to toe erosion activities had occurred at the south property boundary of the subject site (Section C in Drawing PG1887-2 - Site Plan). A slope remedial program was initiated in Summer 2003 and consisted of modifying the existing slope and reinstating with blast rock fill.

The subsurface soil profile used for the slope stability analysis was based on existing test hole information and available geological mapping in the immediate area of the subject site. Generally, the soil profile at the test hole locations placed within the subject site, consists of a thin layer of topsoil overlying a sandy silt layer followed by a 1 to 3 m thick very stiff brown silty clay deposit. The silty clay layer was underlain by a sandy silt to silty sand deposit extending beyond a 12 m depth. Based on nearby borehole locations, glacial till was encountered at 18 to 20 m followed by bedrock at 25 to 30 m below ground surface. Based on available geological mapping, the bedrock surface in this area is encountered at depths varying between 15 to 25 m and consists of dolomite of the Oxford formation.

2.0 Slope Stability Analysis

The slope stability analysis was completed using the topographical survey, as well as, a current slope condition review by Paterson field personnel. Two (2) slope cross-sections (Section A and Section B) were studied as the worst case scenarios. Due to the proximity of the former slope failure located at the south property boundary, a third cross-section (Section C) was analysed during a recent site visit and using topographic mapping from before and after the remedial program. The cross section locations are presented on Drawing PG1887-2 - Site Plan attached to the present letter.

The analysis of the stability of the slope was carried out using SLIDE, a computer program which permits a two-dimensional slope stability analysis using several methods including the Bishop's method, which is a widely used and accepted analysis method. The program calculates a factor of safety, which represents the ratio of the forces resisting failure to those favoring failure. Theoretically, a factor of safety of 1.0 represents a condition where the slope is stable. However, due to intrinsic limitations of the calculation methods and the variability of the subsoil and groundwater conditions, a factor of safety greater than one is usually required to ascertain the risks of failure are acceptable. A minimum factor of safety of 1.5 is generally recommended for conditions where the failure of the slope would endanger permanent structures.

Subsoil conditions at the cross-sections were inferred based on the findings at nearby borehole locations and general knowledge of the area's geology.

The results for the existing slope conditions under static loading at Sections A, B and C are shown in Figures 1, 2 and 3, respectively, attached to the present letter. The overall slope stability factors of safety for the subject sections were found to be less than 1.5, except at Section C. The stable slope allowance from top of slope required for a slope with a minimum factor of safety of 1.5 is identified for each profile in the attached figures.

Seismic Loading Analysis

An analysis considering seismic loading was also completed. A horizontal seismic acceleration, K_h , of 0.16G was considered for the analyzed sections. A factor of safety of 1.1 is considered to be satisfactory for stability analyses including seismic loading.

The results of the analyses including seismic loading are shown in Figures 2, 4 and 6 for the slope sections. Where the minimum factor of safety is less than 1.1, the stable slope allowance from top of slope required for the slope section is identified in the attached figures.

3.0 Limit of Hazard Lands

The limit of hazard lands includes a stable slope allowance taken from top of slope. The limit of hazard lands also includes a toe erosion and a 6 m erosion access allowance. The various allowances and the overall limit of hazard lands for the subject site are indicated on Drawing PG1887-2 - Site plan attached to the present letter.

The toe erosion allowance for the slopes was based on the nature of the soils, the observed current erosional activities and the width and location of the current watercourse. Signs of erosion were noted in areas where the existing watercourse has meandered in close proximity to the toe of the corridor wall of the north neighbouring tributary watercourse. It is considered that a toe erosion allowance of 5 m is appropriate for the tributary watercourse.

Some erosional activities were noted along the toe of the subject valley corridor wall for the Rideau River. It is considered that a toe erosion allowance of 8 m is appropriate for the subject slope along the Rideau River.

Based on the location of the limit of hazard lands line within the subject site, a total of 5.14 acres of developable land is available from a geotechnical perspective within the subject site.

4.0 Recommendations

The existing vegetation on the slope face should not be removed as it contributes to the stability of the slope and reduces erosion. If the existing vegetation needs to be removed, it is recommended that 100 to 150 mm of topsoil mixed with a hardy seed or an erosional control blanket be placed across the exposed slope face.

5.0 Statement of Limitations

The information gathered for this report is based on a soils investigation, which is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test hole locations, we request that we be notified immediately in order to permit reassessment of our recommendations.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Mr. Scott Thomson or their agent(s) is not authorized without review by this firm for the applicability of our recommendations to the altered use of the report.

We trust that this letter satisfies your requirements.

Sincerely,

Paterson Group Inc.



Richard Groniger, C. Tech.



David J. Gilbert, P.Eng.

Attachments:

- Figures 1 to 6 - Slope Stability Analysis
- Soil and Profile Test Data sheets (JDPA)
- Drawing PG1887-2 - Site Plan

Figure 1 - Section A - Static Conditions

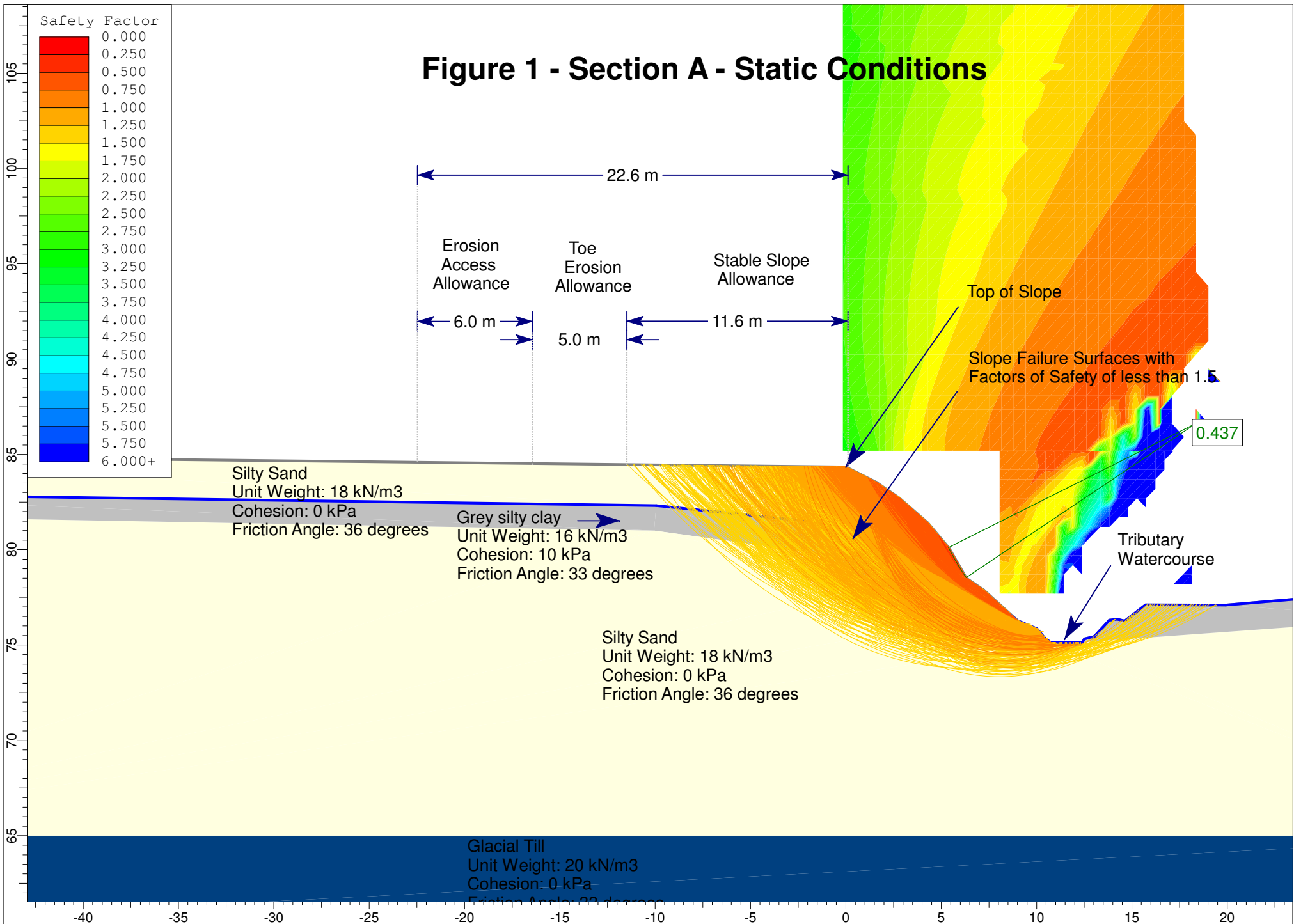


Figure 2 - Section A - Seismic Loading

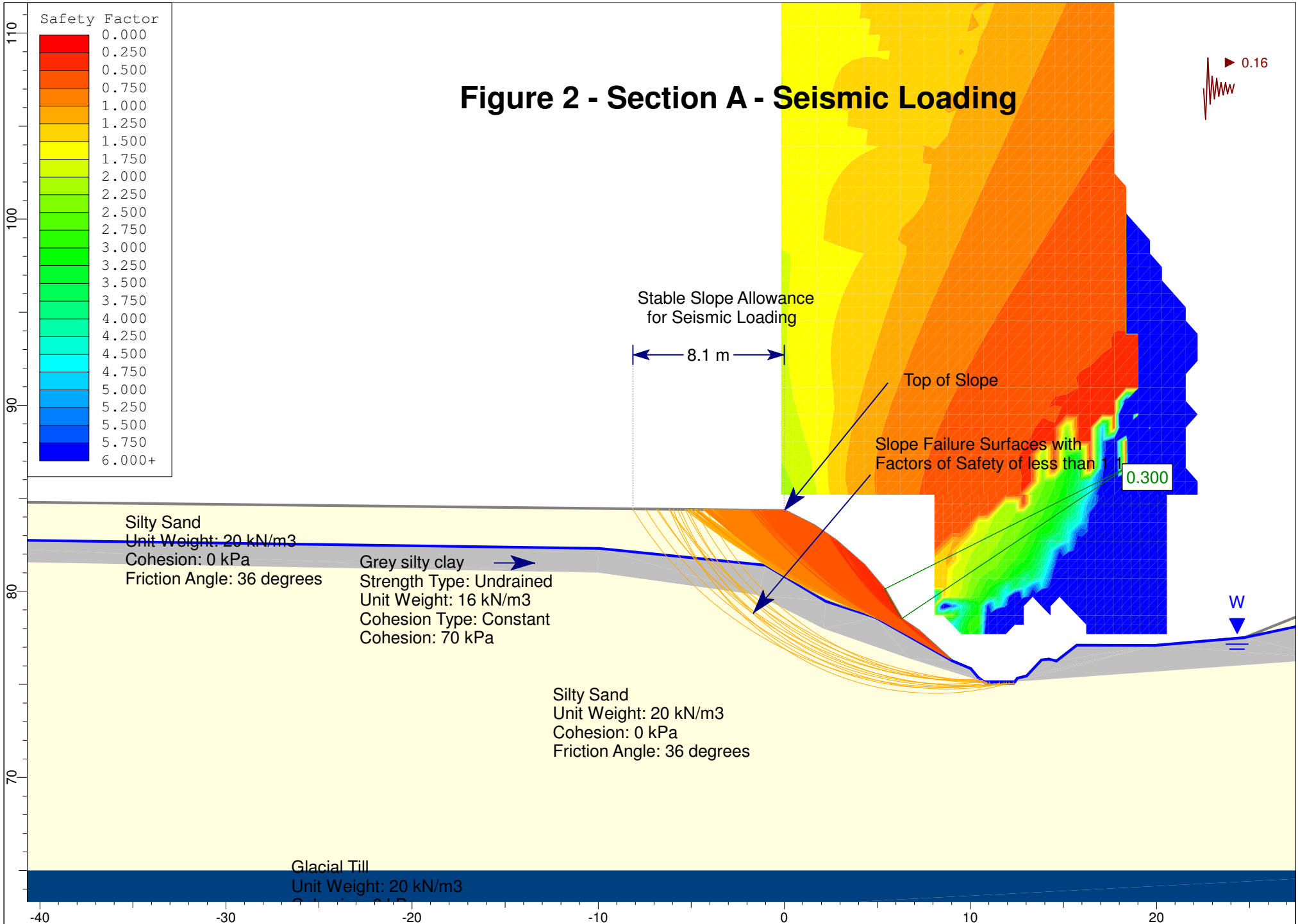


Figure 3 - Section B - Static Conditions

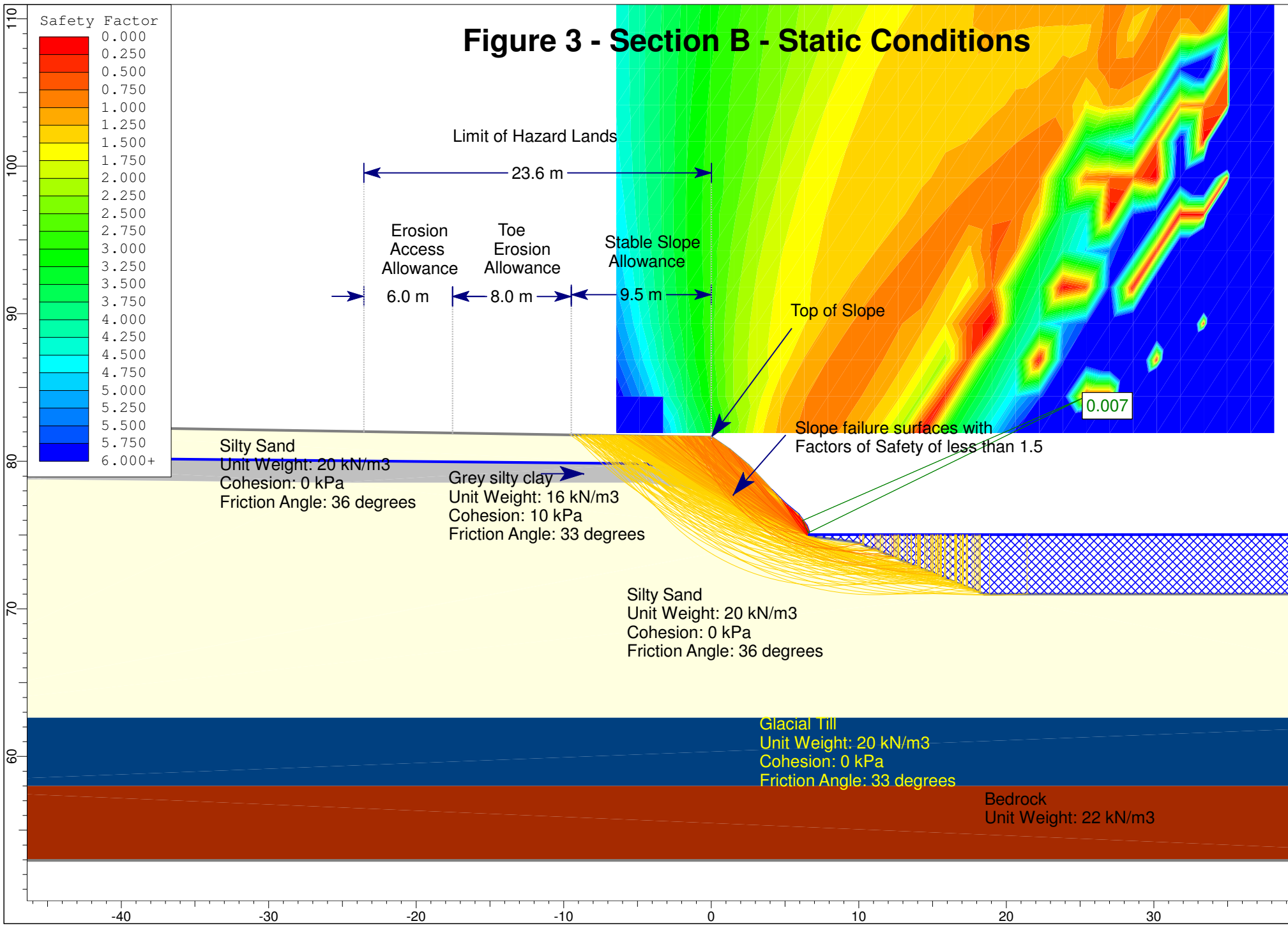


Figure 4 - Section B - Seismic Loading

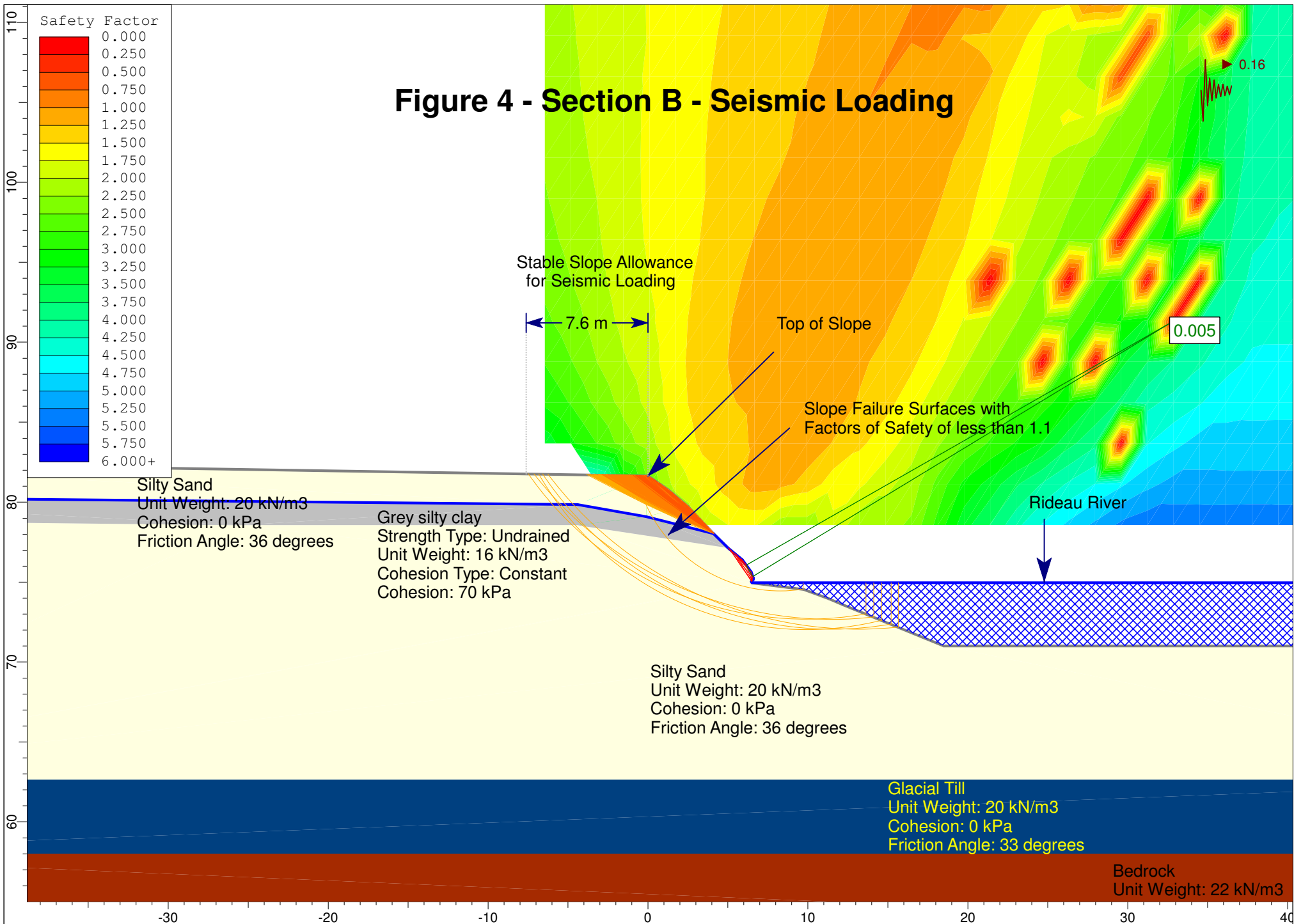


Figure 5 - Section C - Static Conditions

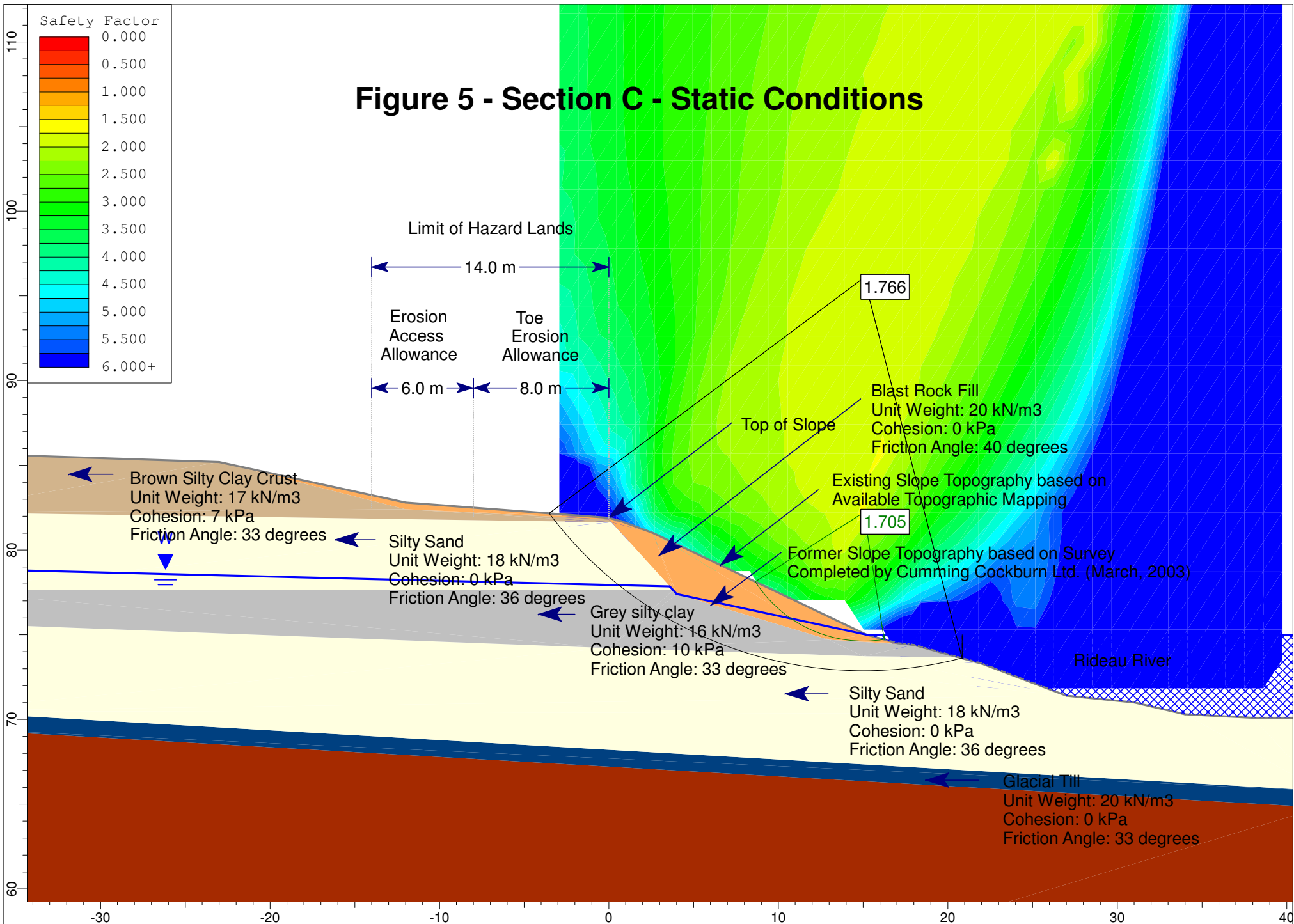
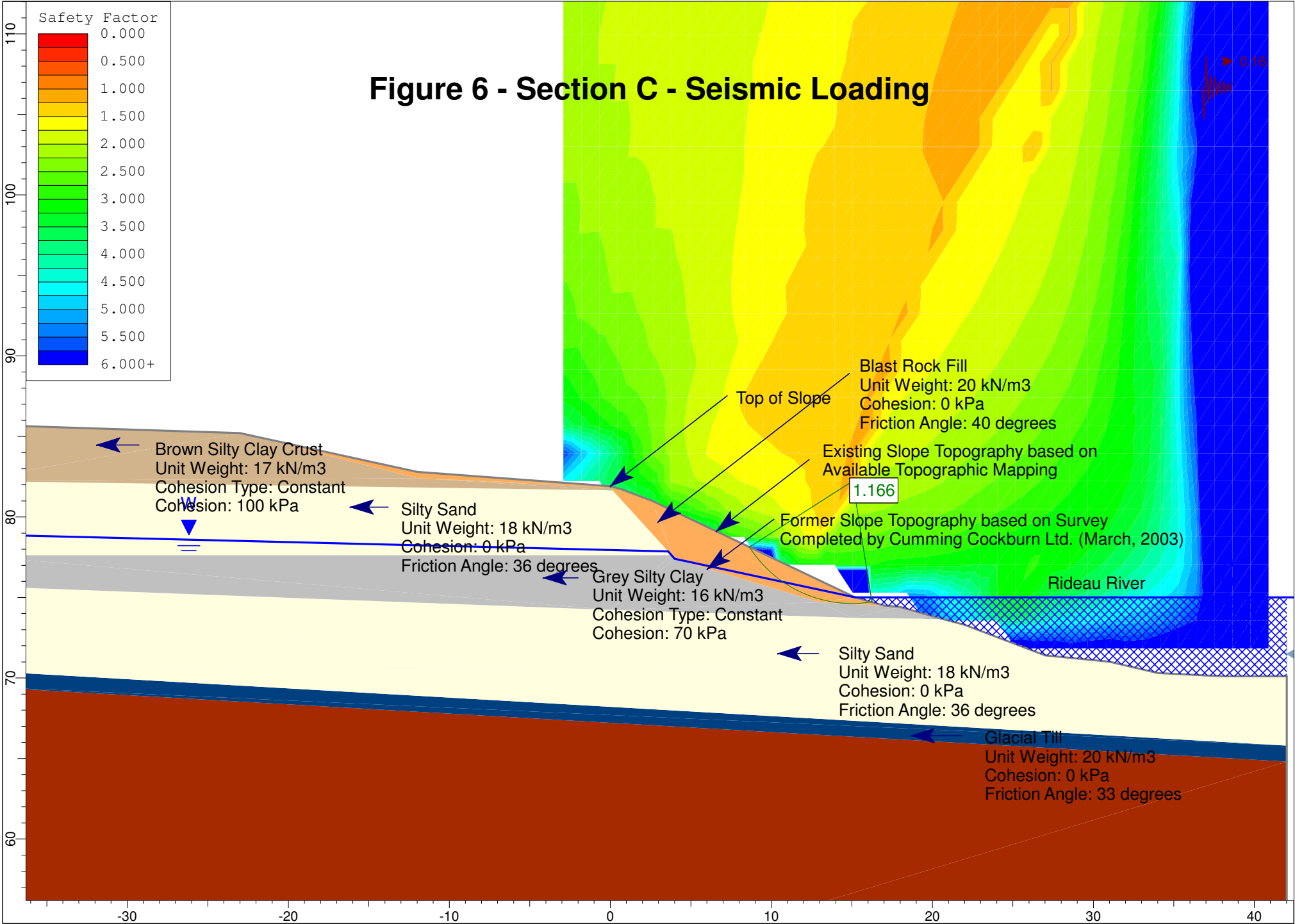
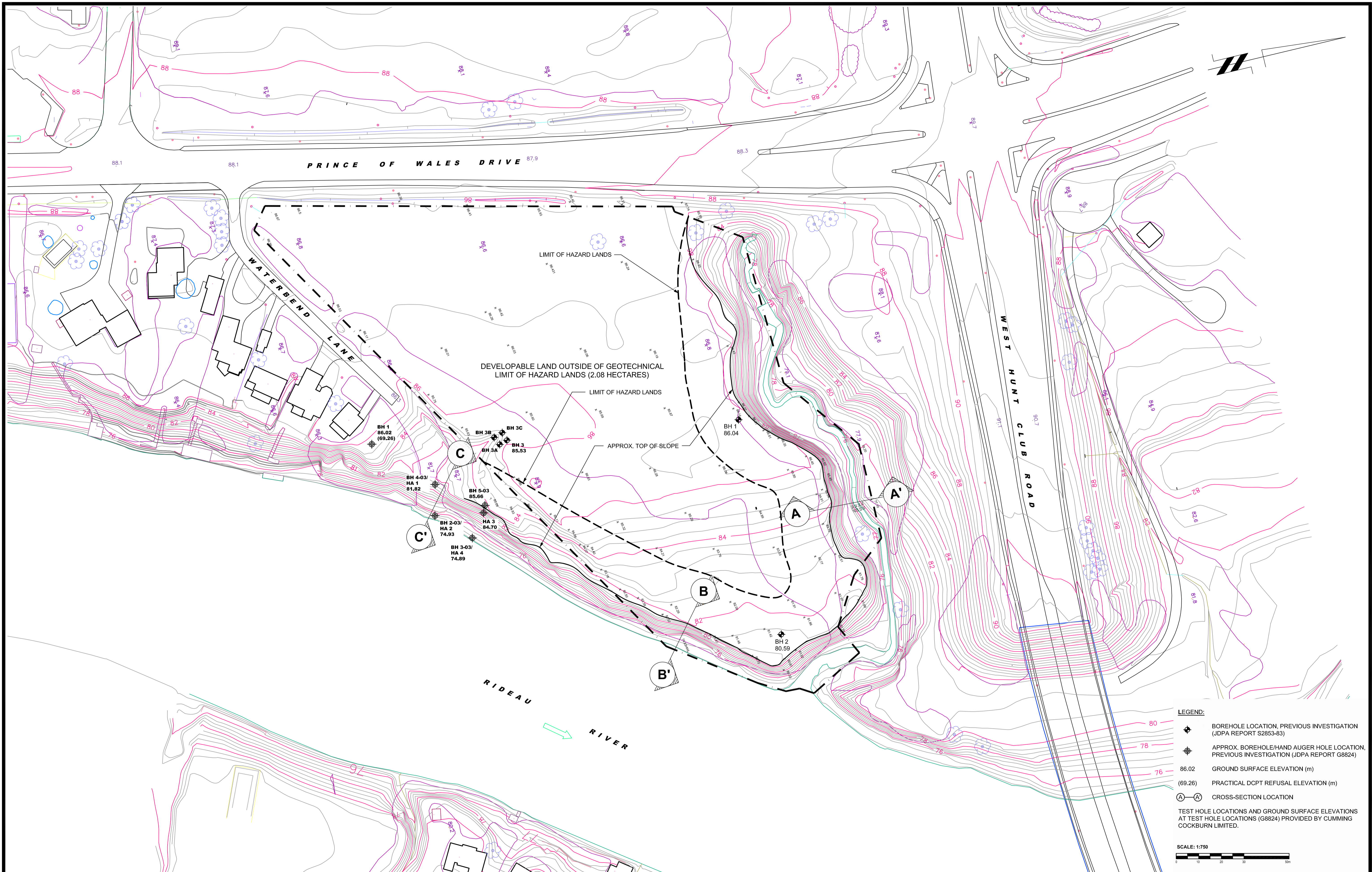


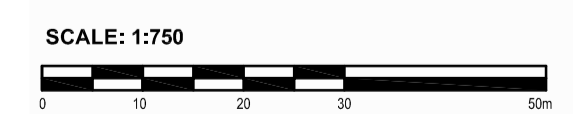
Figure 6 - Section C - Seismic Loading



SOIL PROFILE AND TEST DATA		JOHN D. PATERSON & ASSOCIATES LTD.				SHEET NO. 3 OF 3																						
Proposed Residential Subdivision South 1/2 Lot 26, Concession "A", R.F. Nepean, Ontario		Consulting Engineers & Geologists		Offices & Laboratory		HOLE NO. BH 3																						
		Soil Investigations		1479 Laperriere Ave.		GROUND SURFACE 85.53 BOTTOM HOLE 73.33																						
		Inspection & Testing Services		Ottawa, Canada K1Z 7S8		BEDROCK _____ GROUNDWATER DRY																						
		Damage Claims		Telephone (613) 728-3505																								
DESCRIPTION	LEGEND	SAMPLE TYPE	SAMPLE NUMBER	ELEV.	WATER CONTENT								UNIT WEIGHT				SHEAR STRENGTH (kPa)				STANDARD (N) PENETRATION TEST				WATER LEVEL			
				DEPTH	%								kN/m ³				▲ UNDISTURBED △ REMOULDED				○ PENETRATION RESISTANCE							
Ground Surface				85.53	10	20	30	40	50	60	70	80	5	10	15	20	20	40	60	80	100	120	140	20	40	60	80	
250 mm TOPSOIL over a loose brown SANDY SILT interbedded with clayey silt & sand		G	35	0.00																								
		SS	36	0.80																								
Stiff olive grey fissured SILTY CLAY containing brown fine sand seams at 50 mm ± intervals		SS	37	1.60																								
		TW	38	1.60																								
		TW	39	2.40																								
		SS	40	3.20																								
		SS	41	81.53																								
		SS	42	4.00																								
		SS	43	4.80																								
Compact brown SILTY FINE SAND containing clayey silt seams		SS	44	5.60																								
		SS	45	6.40																								
		TW	46	7.20																								
		TW	47	77.53																								
		TW	48	8.00																								
Firm to stiff grey fissured SILTY CLAY with occasional fine sand lenses and containing fine sand seams		TW	49	8.80																								
		TW	50	9.60																								
		TW	51	10.40																								
STRATIFIED SILT: grey compact layers of silty sand, sandy silt and stiff silty clay		TW	52	11.20																								
		TW	53	73.53																								
				12.00																								
Borehole terminated in silt																												
																			</									



- LEGEND:**
- BOREHOLE LOCATION, PREVIOUS INVESTIGATION (JDKA REPORT S2853-83)
 - APPROX. BOREHOLE/HAND AUGER HOLE LOCATION, PREVIOUS INVESTIGATION (JDKA REPORT G8824)
 - 86.02 GROUND SURFACE ELEVATION (m)
 - (69.26) PRACTICAL DCPT REFUSAL ELEVATION (m)
 - CROSS-SECTION LOCATION
- TEST HOLE LOCATIONS AND GROUND SURFACE ELEVATIONS AT TEST HOLE LOCATIONS (G8824) PROVIDED BY CUMMING COCKBURN LIMITED.



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NO.	REVISIONS	DATE	INITIAL

Title:

MR. SCOTT THOMSON
SLOPE STABILITY ANALYSIS
2175 PRINCE OF WALES DRIVE
OTTAWA, ONTARIO

SITE PLAN

Stamp:

Scale:	1:750	Report No.:	PG1887-2
Drawn by:	MPG	Drawing No.:	
Checked by:	RG		
Approved by:	DJG		
Date:	01/2017	Revision No.:	0

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