# 3717 Borrisokane Road Transportation Impact Assessment 

Step 1 Screening Report<br>Step 2 Scoping Report<br>Step 3 Forecasting Report<br>Step 4 Strategy Report (revision 2)

Prepared for:
Caivan Communities
2934 Baseline Road, Suite 302
Ottawa, ON K2H 1B2

Prepared by:

13 Markham Avenue
Ottawa, ON K2G $3 Z 1$

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## 1 Screening

This study has been prepared according to the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines. Accordingly, a Step 1 Screening Form has been completed and is included as Appendix A, along with the Certification Form for TIA Study PM. As shown in the Screening Form, a TIA is required including the Design Review component and the Network Impact component. The application is for a zoning bylaw amendment and plan of subdivision.

## 2 Existing and Planned Conditions

### 2.1 Proposed Development

The proposed residential development, located at 3717 Borrisokane Road, is currently a surrendered sand and gravel pit within the Barrhaven South Urban Expansion Area (UEA). The site is in an area that is currently zoned as a Mineral Extraction Operation Zone (ME2). The current development application would modify the zoning to allow for low-rise residential uses. The proposed residential development will consist of a mixture of 61 detached homes and 589 townhouses. Access to the development lands will be provided to the east of the proposed development along the east-west collector road extension to Dundonald Drive and through the Re-aligned Greenbank Road corridor, and to the north along the planned north-south collector road (Elevation Avenue) to Cambrian Road (at Apolune Way). The development will have active mode connections to the adjacent developments to allow access to shared community services (i.e. parks, schools, etc.). The anticipated full buildout and occupancy horizon is 2024. Figure 1 illustrates the Study Area Context. Figure 2 illustrates the proposed concept plan.


Source: http://maps.ottawa.ca/geoOttawa/ Accessed: May 14, 2020


### 2.2 Existing Conditions

### 2.2.1 Area Road Network

Borrisokane Road: Borrisokane Road is a Ministry of Ontario road with a two-lane rural cross-section and a posted speed limit of $80 \mathrm{~km} / \mathrm{h}$ along the frontage of the site. No sidewalks are provided. North of Cambrian Road, Borrisokane Road is an Arterial Road, and south of Cambrian Road it is a Collector Road. Borrisokane is part of the Veterans Memorial Highway (Highway 416) corridor to the south of Cambrian Road and has a measured 37.5 metre right of way to the north of Cambrian Road.

Cambrian Road: Cambrian Road is a City of Ottawa collector road with a two-lane rural cross-section and a posted speed limit of $70 \mathrm{~km} / \mathrm{h}$ for approximately 700 metres east of Borrisokane Road and $50 \mathrm{~km} / \mathrm{h}$ in the remaining Study Area. To the west of Seeley's Bay Street, Cambrian Road has no sidewalks and to the east of Seeley's Bay Street, Cambrian Road has sidewalks. The Ottawa Official Plan reserves a 37.5 metre right-of-way from Cedarview (now Borrisokane Road) to Jockvale Road.

Dundonald Drive: Dundonald Drive is a City of Ottawa collector road with a two-lane urban cross-section and an unposted speed limit of $50 \mathrm{~km} / \mathrm{h}$. Sidewalks are present on both sides of the road within the Study Area. The measured right-of-way is 24 metres.

### 2.2.2 Existing Intersections

There are no existing signalized intersections within a one-kilometre radius of the proposed development, as the Re-Aligned Greenbank Road corridor has not been constructed and the adjacent developments are in various stages of the planning and development process. Therefore, no intersections will be analyzed for the existing horizon and new and planned intersections will be included in the analysis of future horizons.

### 2.2.3 Existing Driveways

There are no existing driveways within 200 metres of the potential future accesses except for residential driveways along Dundonald Drive, east of Re-Aligned Greenbank Road. These driveways are not expected to provide access to significant traffic generators and would therefore have no impact on this TIA.

### 2.2.4 Cycling and Pedestrian Facilities

No cycling facilities and very limited pedestrian facilities currently exist along Borrisokane Road or Cambrian Road. As Re-Aligned Greenbank Road has not yet been constructed, no cycling or pedestrian facilities currently exist. Approved cycling infrastructure as part of The City of Ottawa's Ultimate Cycling Network includes plans for local cycling routes along Cambrian Road, Borrisokane Road and Apolune Way / Elevation Avenue (north-south development site collector road). A spine route is also planned for the Re-Aligned Greenbank Road. These approved cycling plans are shown in Figure 3.


Source: http://maps.ottawa.ca/geoOttawa/Accessed: December 1, 2020

### 2.2.5 Existing Transit

There is no existing transit service along the boundary roads. East of the subject development, Route 75 and Route 275 run along River Mist Road and Cambrian Road. Figure 4 illustrates the existing transit service and Figure 5 illustrates the existing transit stops.

Figure 4: Existing Transit Service


Source: http://www.octranspo.com/ Accessed: December 1, 2020


Source: http://plan.octranspo.com/plan Accessed: December 1, 2020

### 2.2.6 Existing Area Traffic Management Measures

There are no existing area traffic management measures within the Study Area.

### 2.2.7 Existing Peak Hour Travel Demand

There are no existing intersections in the Study Area that will be examined as outlined in Section 2.2.2 above.

### 2.2.8 Collision Analysis

As illustrated in Figure 6, no significant collisions are noted in the vicinity of the study area. Therefore, no collision analysis has been performed.


Source: https://maps.bikeottawa.ca/collisions/ Accessed: December 1, 2020

### 2.3 Planned Conditions

### 2.3.1 Changes to the Area Transportation Network

The planned development is subject to policies outlined in the City of Ottawa's Master Plan and the Barrhaven South Urban Expansion Area Community Design Plan (CDP). Additionally, Development Charges (DC) outlined in the 2019 City of Ottawa Intersection Control Measures By-Law will impact the planned development.

Expected changes to the subject development as outlined in the City of Ottawa's Master Plan are:

- The Re-Aligned Greenbank Road extension, south of Cambrian Road, is located on the east side of the proposed development. This will provide Arterial Road connectivity to the site. The timing of this extension is unknown as it is not included in the City of Ottawa's Transportation Master Plan 2031 Affordable Road Network and is only indicated as a 'Conceptual Arterial Extension' in the Network Concept Plan. The proposed cross-section of Re-Aligned Greenbank Road can be seen in Figure 7.
- A 'Conceptual Future Transit Corridor'. This is shown along the Re-Aligned portion of Greenbank Road, south of Cambrian Road in the Rapid Transit and Priority 2031 Network Concept Plan. This is not shown on the 2031 Affordable Rapid Transit and Priority Network.
- A cycling spine route indicated along the Re-Aligned portion of Greenbank Road, south of Cambrian Road in the Primary Urban Cycling Network map.

Intersection Control Measures outlined in the 2019 Ottawa Development Charges By-Law are expected to be implemented at the following intersections:

- Cambrian Road and Borrisokane Road
- Cambrian Road and Apolune Way
- Old Greenbank Road and Kilbirnie Drive
- Old Greenbank Road and Barnsdale Road

The intersection modification at Apolune Way and Cambrian Road is underway and is expected that these changes will be complete prior to the proposed development's build-out year of 2024. City of Ottawa staff has indicated that signalization of this intersection is anticipated to be warranted in 2024 or 2025 and signal design will be completed as a City project. The planned intersection design can be seen in Appendix B as an excerpt from the RMA completed at the intersection of Apolune Way and Cambrian Road (Stantec, 2019). Correspondence with City of Ottawa staff confirming this approach to the design and signalization timeline of Cambrian Road at Apolune Way / Elevation has also been included in Appendix B.

The subject development is within the Barrhaven South Urban Expansion Area CDP. As such, it is subject to the planning polices outlined in the CDP. Some of the expected changes outlined are:

- Road Network:
- The subject development proposes the addition of a north-south collector road (Elevation Avenue) and an east-west collector road (Dundonald Drive extension) within the development area which connect to the two development accesses. These collector roads are subject to the City of Ottawa Road Corridor Planning and Design Guidelines. Both collector roads have cycling and pedestrian facilities, and parking facilities and the Elevation Avenue will also include transit.
- The Re-Aligned Greenbank Road extension will provide arterial road connection to the site. The proposed cross-section of the Re-Aligned Greenbank Road can be seen in Figure 7

Figure 7: Re-Aligned Greenbank Road Cross-Section


Source: Barrhaven South Urban Expansion Study Area Community Design Plan Transportation Master Study. Accessed: October 3, 2019

- Pedestrian Network:
- Future sidewalks are proposed on both sides of the Dundonald Drive extension within the proposed development.
- Cycling Network:
- A local cycling route is proposed along Dundonald Drive and its connection to the east-west collector road within the proposed development.
- A cycle track is proposed along Re-Aligned Greenbank Road.
- Transit Network:
- A BRT route is proposed along Re-Aligned Greenbank Road with a BRT station at the intersection of Dundonald Drive and Re-Aligned Greenbank Road.


### 2.3.2 Other Study Area Developments

The Meadows Phase 4
Northeast of the proposed development is Phase 4 of the Meadows Tamarack Development and is expected to be built out during 2019. Phase 4 will have 136 townhouse units and 50 single family units This development is anticipated to produce 142 two-way AM peak period auto trips and 171 two-way PM peak period auto trips. (IBI 2018)

## The Meadows Phase 5

North of the proposed development is Phase 5 of the Tamarack Development of the Meadows and is expected to be built-out during 2022. Phase 5 will have 221 townhouse units and 125 single family units. This development is anticipated to produce 294 two-way AM peak period auto trips and 334 two-way PM peak period auto trips. (IBI 2018)

## 3809 Borrisokane Road

South of the proposed development is the 3809 Borrisokane Road development which is expected to be built-out during 2025. This development will include 590 residential units, split between townhouse units and detached home units. 3717 Borrisokane Road will include a connection to 3809 Borrisokane Road and both developments will share an access to Borrisokane Road as part of an interim phase only. Approximately 300 units will use this connection prior to the full build-out in 2025 at which time the connection to Borrisokane Road will be closed. This development is expected to produce 401 two-way AM peak period auto trips and 457 two-way PM peak period auto trips. (CGH 2019).

## Half Moon Bay West

North of the proposed development is the Mattamy Development of Half Moon Bay West which is expected to be built-out during 2024. This development will include 552 single family homes and 464 townhomes. Construction has not yet commenced on this subdivision. This development is expected to produce 786 two-way AM peak period auto trips and 1193 two-way PM peak period auto trips. (Stantec 2016).

## Citi Gate's Highway 416 Employment Lands

North of the proposed development is the Citi Gate Corporate Campus. This development will include 32,516 square metres allocated towards a shopping centre, 165,600 square metres allocated towards business parks and 105,000 square metres allocated towards car dealerships. The full build-out year is 2029 with an interim development year of 2019. This development is expected to produce 4267 two-way AM peak period auto trips and 4848 two-way PM peak period auto trips. (Novatech 2012).

## Mattamy's Half Moon Bay North Phase 9 (Apartment Block)

North of the proposed development is the Half Moon Bay North Phase 9 development which is expected to be built-out during 2019. This development will consist of 60 stacked townhouses. This development is expected to produce 74 two-way AM peak period auto trips and 80 two-way PM peak period auto trips. (Stantec 2018).

3285 Borrisokane Road

North of the proposed development is 3285 Borrisokane Road which is expected to be built-out during 2020. This development will include 125 single family homes and 75 townhouses. This development is expected to produce 129 two-way AM peak period auto trips and 146 two-way PM peak period auto trips. (Parsons 2018).

## 3713 Borrisokane Road

Directly west of the proposed development an industrial development which is expected to be built-out during 2022. The development will include approximately 3,250 square metres of general office space and 9,385 square metres of industrial buildings. This development is expected to produce 136 two-way AM peak period auto trips and 188 two-way PM peak period auto trips. (CGH 2019).

Barrhaven South Expansion Lands (Quinn's Pointe 2)
To the southeast of the proposed development is the Minto Development of Quinn's Pointe 2. This development will include 536 single-family dwelling units, 493 townhomes, 100 apartment units, and two elementary schools, anticipated over 2 phases of construction for the horizon years of 2022 and 2025. A total of 749 two-way AM peak period auto trips and 813 two-way PM peak period auto trips are expected from this development (Stantec 2018).

## 3387 Borrisokane Road

North of Cambrian Road is the Glenview Development of 3387 Borrisokane Road which is expected to be builtout during 2022. The development is expected to have 179 single family units and 109 townhouses. The development is anticipated to produce 137 two-way AM peak period auto trips and 174 two-way PM peak period auto trips. (Stantec 2016)

## Half Moon Bay South Phase 5

Southeast of the proposed development is the Mattamy Development of Half Moon Bay South which is expected to be built-out during 2020. The development will consist of 164 single detached home units and 97 townhouse units. This development is expected to produce 180 two-way AM peak period auto trips and 207 two-way PM peak period auto trips. (CGH 2019)

## Mattamy's Half Moon Bay North Phases 7,8

North of the proposed development is the Half Moon Bay North Phase 7 and 8 development and is expected to be built-out during 2019. The development will consist of 471 residential units. No TIA is currently available for this development.

## 3 Study Area and Time Periods

### 3.1 Study Area

The subject site does not have any existing boundary roads to consider and Re-Aligned Greenbank Road is beyond the study horizons.

The Study will instead focus on the development access to Cambrian Road and the connection to Dundonald Drive.

### 3.2 Time Periods

As the proposed development is composed entirely of residential units the AM and PM peak hours will be examined.

### 3.3 Horizon Years

The anticipated build-out year is 2024. As a result, the full build-out plus five years horizon year is 2029.

## 4 Exemption Review

Table 1 summarizes the exemptions for this TIA.
Table 1: Exemption Review

| Module | Element | Explanation | Exempt/Required |
| :--- | :--- | :--- | :--- |
| Design Review Component | 4.1.2 Circulation <br> and Access | Only required for site plans | Exempt |
| 4.1 Development <br> Design | 4.2.3 New Street <br> Networks | Only required for plans of subdivision | Required |
| 4.2 Parking | Supply <br> 4.2.1 Parking | Only required for site plans <br> Parking | Only required for site plans where parking <br> supply is 15\% below unconstrained demand |
| Network Impact Comporent | Exempt |  |  |
| 4.5 Transportation <br> Demand <br> Management | All Elements | Not required for site plans expected to have <br> fewer than 60 employees and/or students <br> on location at any given time | Exempt |
| 4.6 Neighbourhood | 4.6.1 Adjacent | Only required when the development relies <br> on local or collector streets for access and <br> total volumes exceed ATM capacity <br> Thresholds | Required |

## 5 Development-Generated Travel Demand

### 5.1 Trip Generation and Mode Shares

This TIA has been prepared using the vehicle and person trips for the residential components using the TRANS Trip Generation Study Report (2009). Table 2 summarizes the person trip rates for the proposed land uses.

Table 2: Trip Generation Person Trip Rates

| Dwelling Type | Land Use <br> Code | Peak <br> Hour | Vehicle Trip <br> Rate | Person Trip <br> Rates |
| :---: | :---: | :---: | :---: | :---: |
|  | 210 | AM | 0.70 | 1.27 |
| Townhouses | (TRANS) | PM | 0.90 | 1.41 |
|  | 224 | AM | 0.54 | 0.98 |
|  | (TRANS) | PM | 0.71 | 1.16 |

Using the above Person Trip rates, the total person trip generation has been estimated. Table 3 below illustrates the total person trip generation by dwelling type.

Table 3: Total Person Trip Rates

| Land Use | Units | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | In | Out | Total | In | Out | Total |
| Single-detached Dwellings | 61 | 22 | 55 | 77 | 52 | 34 | 86 |
| Townhouses | 589 | 213 | 364 | 577 | 362 | 321 | 683 |
|  | Total Person Trips | $\mathbf{2 3 5}$ | $\mathbf{4 1 9}$ | $\mathbf{6 5 4}$ | $\mathbf{4 1 4}$ | $\mathbf{3 5 5}$ | $\mathbf{7 6 9}$ |

Using the most recent National Capital Region Origin-Destination (OD Survey), the existing mode shares for South Nepean have been summarized in Table 4.

Table 4: Mode Share

| Travel Mode | South Nepean Mode Share |
| :---: | :---: |
| Auto Driver | $60 \%$ |
| Auto Passenger | $15 \%$ |
| Transit | $15 \%$ |
| Cyclist | $1 \%$ |
| Pedestrian | $9 \%$ |
| Total | $100 \%$ |

There are no major transit upgrades (i.e. BRT, transit priority measures, etc.) within the Study Area that are planned to be in place by the study horizons that will be examined in this study. Therefore, the existing mode shares will be carried forward.

Using the above mode shares and the person trip rates, the person trips by mode have been projected. Table 5 summarizes the trip generation by mode.

Table 5: Trip Generation by Mode

| Travel Mode | Mode Share | In | Out | Total | In | Out | Total |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Auto Driver | $60 \%$ | 141 | 251 | 392 | 248 | 213 | 462 |
| Auto Passenger | $15 \%$ | 35 | 63 | 99 | 62 | 53 | 115 |
| Transit | $15 \%$ | 35 | 63 | 99 | 62 | 53 | 115 |
| Cyclist | $1 \%$ | 2 | 4 | 7 | 4 | 4 | 8 |
| Pedestrian | $9 \%$ | 21 | 38 | 59 | 37 | 32 | 69 |
| Total | $100 \%$ | 235 | 419 | 654 | 414 | 355 | 769 |

As shown above, 392 AM and 462 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.

### 5.2 Trip Distribution

To understand the travel patterns of the subject development, the OD survey has been reviewed to determine the existing travel patterns that will be applied to the new vehicle trips. Table 6 below summarizes the distribution for South Nepean.
Table 6: OD Survey Existing Mode Share Sout

| To/From | \% of Trips |
| :---: | :---: |
|  | North |
| South | $80 \%$ |
|  | East |
|  | West |
|  | Total |
|  | $10 \%$ |

### 5.3 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the Study Area road network. Figure 8 illustrates the new site generated volumes.

CGH
TRANSPORTATION


## 6 Background Network Travel Demand

### 6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3.1 and are not anticipated to impact to site, trip generation, or distribution.

### 6.2 Background Growth

Surrounding development Traffic Impact Assessments have used a $2 \%$ traffic growth within the Study Area of this report. As such, an annual background growth of $2 \%$ will be used in order to remain consistent with these studies and to capture any growth not already directly considered as discussed in Section 2.3.2.

### 6.3 Other Developments

The background developments explicitly considered in both the 2024 and 2029 background conditions include the Meadows Phase 4, Meadows Phase 5, Half Moon Bay West, Citi Gate's Highway 416 Employment Lands (Interim2019), Half Moon Bay North Phase 9, 3713 Borrisokane Road, 3809 Borrisokane Road, and 3285 Borrisokane Road developments. The Citi Gate's Highway 416 Employment Lands (2029) development is only considered in the 2029 background conditions. All background developments are discussed in Section 2.3.2.

## 7 Demand Rationalization

### 7.12024 Future Background Intersection Operations

Figure 9 illustrates the 2024 future background volumes and Table 7 summarizes the background intersection operations for the study area. Signal warrants have been evaluated at the intersection of Cambrian Road and Apolune Way / Elevation Avenue and are not found to be warranted for the 2024 future background horizon. As
such, the intersection has been assumed to have stop-control on the minor approaches. Signal warrants are provided in Appendix C.

The level of service for unsignalized intersections is based on HCM average delay. The synchro worksheets for the 2024 future background horizon are provided in Appendix D.

Intersection geometry at Apolune Way and Cambrian Road is based on the RMA prepared by Stantec which has been provided in Appendix B.

Figure 9: 2024 Future Background Volumes


| Intersection | Lane | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) |
| Elevation Ave (Site <br> Access \#1) / <br>  <br> Cambrian Rd <br> (Unsignalized) | EBL | A | 0.01 | 8.4 | 0.3 | A | 0.04 | 8.1 | 0.9 |
|  | EBT/R | - | - | - | - | - | - | - | - |
|  | WBL | A | 0.03 | 7.8 | 0.6 | A | 0.07 | 9.0 | 1.6 |
|  | WBT/R | - | - | - | - | - | - | - | - |
|  | NBL | D | 0.48 | 30.1 | 18.7 | F | 0.58 | 52.3 | 21.8 |
|  | NBT/R | A | 0.08 | 9.7 | 2.0 | B | 0.09 | 12.5 | 2.3 |
|  | SBL | D | 0.35 | 26.1 | 11.5 | E | 0.40 | 43.3 | 12.3 |
|  | SBT/R | B | 0.08 | 11.8 | 1.9 | B | 0.04 | 10.2 | 0.8 |
|  | Overall | A | - | 7.0 | - | A | - | 7.3 | - |
| Notes: Saturation f <br>  PHF $=1.00$ | $\text { rate of } 180$ | $\mathrm{h} / \text { lane }$ |  |  |  |  |  |  |  |

The intersection operations for the 2024 future background horizon generally operate satisfactorily during the peak hours with all $\mathrm{v} / \mathrm{c}$ ratios below 1.00 and no high delays noted. The exception to this is the northbound leftturn in the PM peak period that operates with a LOS of F . This is expected to be mitigated by the planned
signalization of this intersection by the City of Ottawa when warranted. As such, no mitigation measures are recommended at this time.

### 7.2 2029 Future Background Intersection Operations

Figure 10 illustrates the 2029 background horizon volumes and Table 8 summarizes the background intersection operations for the study area. Signal warrants have been evaluated at the intersection of Cambrian Road and Apolune Way / Elevation Avenue and have indicated signalization to be warranted for the 2029 future background horizon. Signal warrants are provided in Appendix C.

The level of service for signalized intersections is based on HCM 2010 calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets for the 2029 future background horizon are provided in Appendix E.

The signal timing at the intersection of Cambrian Road and Apolune Way was optimized and Amber Clearance, All Red Clearance, Walk, and Flash Don't Walk times were calculated using the Methodology provided in OTM Book 12-Traffic Signals. The westbound left-turn movement will operate as a protected and permissive turn during the PM peak period. Intersection geometry at Apolune Way and Cambrian Road is based on the RMA prepared by Stantec which has been provided in Appendix B.

Figure 10: 2029 Future Background Volumes


Table 8: 2029 Future Background Intersection Operations

| Intersection | Lane | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) |
| Elevation Ave (Site Access\#1) / Apolune Way \& Cambrian Rd (Signalized) | EBL | A | 0.10 | 13.5 | 4.3 | A | 0.09 | 11.2 | 9.4 |
|  | EBT/R | A | 0.42 | 12.9 | 40.4 | E | 0.95 | 42.5 | \#250.1 |
|  | WBL | A | 0.17 | 13.3 | 12.5 | B | 0.64 | 24.7 | \#27.2 |
|  | WBT/R | D | 0.88 | 32.7 | 126.7 | A | 0.35 | 8.7 | 45.0 |
|  | NBL | A | 0.54 | 25.0 | 63.9 | A | 0.65 | 49.2 | \#68.4 |
|  | NBT/R | A | 0.13 | 0.3 | 0.0 | A | 0.15 | 0.5 | 0.0 |
|  | SBL | A | 0.20 | 19.6 | 22.6 | A | 0.22 | 36.9 | 22.8 |
|  | SBT/R | A | 0.06 | 0.2 | 0.0 | A | 0.03 | 0.1 | 0.0 |
|  | Overall | C | 0.71 | 22.2 | - | D | 0.84 | 31.0 | - |

Notes: Saturation flow rate of 1800 veh/h/lane PHF $=1.00$

The intersection operations for the 2029 future background horizon generally operate satisfactorily during the peak hours with all v/c ratios below 1.00 and no high delays noted. During the PM peak, the shared eastbound through/right queue is expected to extend past the eastbound left-turn lane, the shared westbound through / right-turn queue is expected to block the westbound left-turn lane, and the northbound left-turn queue is expected to extend past the northbound left-turn lane. No other capacity issues are noted.

### 7.3 Future Total Demand

The future total 2024 volumes are illustrated in Figure 11 and the future total 2029 volumes are illustrated in Figure 12.

Figure 11: Future Total 2024 Volumes


Figure 12: Future Total 2029 Volumes


Volumes developed for the future horizons at the intersection of Cambrian Road and Apolune Way / Elevation Avenue were based on the existing volume conditions at the intersection of Cambrian Road and Borrisokane Road. As such, future horizon volumes were developed using a growth rate of $2 \% /$ annum, the background developments listed in Section 6.3, and the proposed development within this report. The trip generation of this development is consistent with the area modal shares as seen in Table 4 and no adjustments are required.

## 8 Development Design

### 8.1 Design for Sustainable Modes

The proposed development is a residential subdivision and therefore auto and bicycle parking areas will be within each resident's home.

As discussed in Section 2.3.1 above, the Barrhaven South Urban Expansion Study proposes a local cycling route along the extension of Dundonald Drive. Mixed traffic conditions were initially considered for the extension of Dundonald Drive, however a layer traffic calming measure approach would be required to reduce operating speeds to satisfy the target cycling LOS. Off-road cycling facilities in the form of a multi-use pathway or cycle tracks are recommended instead as both facilities will satisfy cycling and pedestrian LOS targets. Additionally, given the anticipated connections from cycling facilities on the Dundonald Drive extension to the planned multi-use pathway on Elevation Avenue as well as the cycle tracks on the re-aligned Greenbank Road, off-road cycling facilities are more appropriate than mixed-traffic cycling conditions in this context. Two walkway connections within the development, and one on the northwest corner of the development will provide additional active mode connection to the property to the west as well as within the subdivision.

Figure 13 illustrates the concept active mode network. The plan incorporates the adjacent developments, planned routes on geoOttawa, the extension of the Barrhaven South Urban Expansion Study Area CDP network, and the above cycling facility recommendations.

Figure 13: Concept Pedestrian Network


### 8.2 New Street Networks

The planned street network will include 14.0 metre window roads, 16.5 metre local roadways, and 24.0 metre collector roadways. The local and collector roads will provide parking on one side of the roadway. The local roads are proposed to be posted as $30 \mathrm{~km} / \mathrm{h}$ and the collector roads are proposed to be posted as $40 \mathrm{~km} / \mathrm{h}$. The pedestrian and cycling network are provided in Section 8.1.

To support the pedestrian and cycling connectivity within the subdivision, Figure 14 illustrates the concept traffic calming plan. The plan reduces crossing distances for the pedestrian and cycling network, as well as limits the speed of vehicles entering and exiting the local roads from the collector roads.


The internal road intersections are recommended to be stop-controlled on the minor approaches of all intersections and the intersection of Dundonald Drive and Elevation Avenue operate as an all-way stop-controlled intersection.

## 9 Boundary Street Design

The Re-aligned Greenbank Road corridor will be a future boundary road to the proposed development however its construction is anticipated to occur outside of the future horizons of this TIA. Any MMLOS analysis will be completed as part of the detailed design of the corridor and is beyond the scope of this study.

## 10 Access Intersections Design

### 10.1 Location and Design of Access

Access to the development lands will be provided to the east of the proposed development along the east-west collector road extension to Dundonald Drive and through the Re-aligned Greenbank Road corridor, and to the north along the planned north-south collector road (Elevation Avenue) to Cambrian Road at Apolune Way.

The intersection of Cambrian Road and Apolune Way / Elevation Avenue is considered a study area intersection and is not considered an access intersection.

As the Greenbank corridor is not expected to be re-aligned within the future horizons considered in this study, the MMLOS and capacity analysis will be completed as part of the detailed design of the corridor and is beyond the scope of this study.

### 10.2 Access Intersection Control

No intersections are located on the boundary of the site and access is provided through collector roadways. Assessment of the network intersections is provided in Section 15.

### 10.3 Access Intersection Design

No access intersections are considered in this TIA.

## 11 Transportation Demand Management

### 11.1 Context for TDM

The mode shares used within the TIA represent this area of the City and have not been altered.
The subject site is not within a design priority or transit-oriented design area.
Total bedrooms within the development is subject to owner purchasing preferences. No age restrictions noted.

### 11.2 Need and Opportunity

The subject site has been assumed to rely predominately on auto travel and those assumptions have been carried through the analysis. A decrease in the low transit or non-auto mode shares will result in higher volumes along Cambrian Road. Little opportunity is available to shift these modes until major infrastructure projects, such as the Re-aligned Greenbank Road corridor, are complete to increase the transit and active mode network from South Barrhaven to the rest of the City.

### 11.3 TDM Program

As discussed above, any "suite of post-occupancy TDM measures" are limited in their applicability. It is anticipated that this development will rely predominantly on auto travel and those assumptions have been carried through the analysis. As a result, no TDM measures are recommended at this time beyond those required for zoning and standard subdivision design. The TDM Checklist has been provided in Appendix F.

## 12 Neighbourhood Traffic Management

Given the developing nature of the area, a higher-level approach was undertaken to perform analysis in this section. As an illustrative process to document the impact of the development on the network concept and the TIA guideline AADT thresholds, the percent area of the development in the total influence area has been compared to the trip generation of the development. The TIA guidelines outline a collector road threshold of 2,500 vehicles per day (AADT), or 300 vehicles in a given peak hour for Neighbourhood Traffic Management review. This will give an indication of whether the road network meets or exceeds the theoretical threshold, discuss the implications of the anticipated traffic within the context of the existing/planned road network and outline if any mitigation measures are required for the adjacent development areas.

### 12.1 Elevation Avenue

Table 9 summarizes the AADT in the peak direction on the collector road of Elevation Avenue in both the AM and PM peak periods.

|  | Table 9: Elevation Avenue NTM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Area (Hectares) | Theoretical Threshold |  | Site Trip Generation |  |
|  |  | Percentage of Area | Equivalent Volume per AADT Threshold | AADT Volume Generated AM(PM) | Percentage of Theoretical Threshold |
| Proposed Development | 16.25 | 28\% | 700 | 1970(1980) | 79\% (79\%) |
| Area of Influence | 58.7 | 100\% | 2500 | - | - |
| Note:1. AADT  <br>  2. AADT | roximated usin | :1 ratio of peak hour peak direction volum |  |  |  |

As shown above, using this method the overall trip generation is expected to use $79 \%(79 \%)$ of the theoretical TIA AADT threshold of Elevation Avenue. This indicates that while the overall theoretical TIA AADT threshold of Elevation Avenue will not be exceeded within the proposed development, it is likely that it will be exceeded as Elevation Avenue approaches Cambrian Road.

Within the study area context, the road network planning is consistent with the adjacent collector road and arterial road spacing. As such it is expected to serve as planned for within Barrhaven South, similar to River Mist Road and Kilbirnie Drive. The incorporation of the new collector road guidelines and passive traffic calming measures, as outlined within the TIA guidelines, should ensure that the future Elevation Avenue will operate as intended. No additional measures are recommended to accommodate the projected volumes along the corridor. Section 0 presents the intersection operations at Cambrian Road and potential mitigation for the intersection.

Beyond the horizons of this study, Re-Aligned Greenbank Road will provide the additional capacity and may reduce the reliance on Elevation Avenue by vehicular traffic. The addition of the BRT component of the corridor would also provide an alternative travel mode and decrease the peak hour demands. South of the site, the future Barnsdale Road interchange at Highway 417 may also have larger regional travel impacts and the subsequent redistribution of trips may balance directional demands along Elevation Avenue, reducing northbound trips and increasing southbound trips.

### 12.2 Dundonald Drive

Table 10 summarizes the AADT in the peak direction on the collector road of Dundonald Drive in both the AM and PM peak periods.

Table 10: Dundonald Drive NTM

|  | Area (Hectares) | Theoretical Threshold |  | Site Trip Generation |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Percentage of Area | Equivalent Volume per AADT Threshold | AADT Volume Generated AM(PM) | Percentage of Theoretical Threshold |
| Proposed Development | 4.06 | 13\% | 325 | 490(490) | 20\% (20\%) |
| Area of Influence | 30.79 | 100\% | 2500 | - | - |
| Note:1. AADT  <br>  2. AADT | proximated us lculated as one | :1 ratio of peak hou peak direction volu |  |  |  |

As shown above, using this method the overall trip generation is expected to use $20 \%(20 \%)$ of the theoretical TIA AADT threshold of Dundonald Drive. This indicates that while the overall theoretical TIA AADT threshold of Dundonald Drive will not be exceeded within the proposed development, it may be exceeded as Dundonald Drive approaches the existing Greenbank Road depending on the travel patterns of the neighbourhoods between ReAligned Greenbank Road and Greenbank Road.

Within the study area context, the collector road network serves to distribute traffic to the adjacent arterial roads and facilitate north-south travel beyond Barrhaven South. As Re-Aligned Greenbank Road is beyond the study horizons, Dundonald Drive approaching Greenbank Road will be required to operate near the theoretical TIA AADT threshold. Once Re-Aligned Greenbank Road is constructed, the demands will decrease and likely be within the ascribed threshold. No additional measures are recommended to accommodate the projected volumes along the corridor.

## 13 Transit

### 13.1 Route Capacity

Overall, the forecasted new transit trips would result in approximately one to two buses (single bus, 55-person capacity) being required in the peak direction to accommodate the transit trips generated from the subject site.

While no transit routes currently serve the study area, a transit route along Elevation Avenue from Cambrian Road to Kilbirnie Drive is anticipated to service the development once Elevation Avenue is constructed.

### 13.2 Transit Priority

No transit priority is required/considered for the study area.

## 14 Review of Network Concept

Cambrian Road may potentially approach or exceed a single lane capacity in the peak direction by the 2029 background and total future conditions. For example, in the PM peak period the west approach volumes in the shared through/right-turn lane in the 2029 future background horizon is 888 and in the PM peak period the west approach volumes in the shared through/right-turn lane in the 2029 total future horizon is 1061. These volume projections are dependent on surrounding development growth being realized, Re-Aligned Greenbank Road being constructed beyond the study horizon, and on growth proceeding at the same rate. The likely impact of the interim condition is extended queues along Cambrian Road, between Borrisokane Road and Greenbank Road.

The network concept, as identified within the City of Ottawa's Transportation Master Plan Map 10, illustrates extensive improvements within Barrhaven South:

- New Re-Aligned Greenbank Road, from Chapman Mills Drive to Cambrian Road
- Re-Aligned Greenbank Road extension south of Cambrian Road
- Widening of Cambrian Road from the Re-Aligned Greenbank Road to the existing Greenbank Road
- Widening of Jockvale Road from Cambrian Road to Prince of Wales Drive
- Widening of Barnsdale Road between Highway 416 and Prince of Wales Drive
- New interchange at Barnsdale Road and Highway 416

These planned improvements are expected to address the high volumes experienced along Cambrian Road, therefore no changes to the network concept are required.

## 15 Network Intersection Design

### 15.1 Network Intersection Control

Signal warrants have been evaluated at the intersection of Cambrian Road and Apolune Way / Elevation Avenue and have found signalization to be warranted for the 2024 future total 2029 future background, and the 2029 future total horizons. Signal warrants are provided in Appendix C.

### 15.2 Network Intersection Design

### 15.2.1 2024 Future Total Intersection Operations

The 2024 future total future traffic volumes are illustrated in Figure 9 and the intersection operations are summarized in Table 11.

The level of service for signalized intersections is based on HCM 2010 calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets for the 2024 future total horizon have been provided in Appendix G.

The signal timing at the intersection of Cambrian Road and Apolune Way was optimized and Amber Clearance, All Red Clearance, Walk, and Flash Don't Walk times were calculated using the Methodology provided in OTM Book 12-Traffic Signals. Intersection geometry at Apolune Way and Cambrian Road is based on the RMA prepared by Stantec which has been provided in Appendix B.

Table 11: 2024 Future Total Intersection Operations

| Intersection | Lane | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) |
| Elevation Ave (Site <br> Access\#1) / <br> Apolune Way \& Cambrian Rd (Signalized) | EBL | A | 0.09 | 16.8 | 4.7 | A | 0.10 | 9.6 | 8.0 |
|  | EBT/R | A | 0.53 | 17.7 | 48.7 | E | 0.91 | 31.1 | 148.0 |
|  | WBL | A | 0.18 | 17.6 | 11.5 | C | 0.71 | 47.4 | \#35.6 |
|  | WBT/R | D | 0.82 | 33.9 | 97.5 | A | 0.40 | 11.8 | 44.0 |
|  | NBL | A | 0.51 | 19.2 | 66.1 | A | 0.59 | 31.9 | \#69.1 |
|  | NBT/R | A | 0.08 | 0.1 | 0.0 | A | 0.09 | 0.2 | 0.0 |
|  | SBL | A | 0.16 | 14.2 | 19.5 | A | 0.16 | 23.4 | 17.9 |
|  | SBT/R | A | 0.05 | 0.1 | 0.0 | A | 0.03 | 0.0 | 0.0 |
|  | Overall | B | 0.64 | 21.8 | - | C | 0.78 | 25.4 | - |
| Notes: Saturation flow rate of $1800 \mathrm{veh} / \mathrm{h} / \mathrm{lane}$$\text { PHF = } 1.00$ |  |  |  |  |  |  |  |  |  |

The intersection operations for the 2024 future total horizon generally operate satisfactorily during the peak hours with all v/c ratios below 1.00 and no high delays noted. During the PM peak, the shared westbound through/right movement queue is anticipated to extend past the westbound left-turn lane, and the northbound left-turn queue is anticipated to block the northbound through/ right-turn movement. No other capacity issues are noted.

To further illustrate the need for signalization of this intersection, it has also been analyzed as an unsignalized intersection with stop-control on the minor approaches in Table 12.

Table 12: 2024 Future Total Stop-control Scenario

| Intersection | Lane | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) |
| Elevation Ave (Site Access\#1)/ Apolune Way \& Cambrian Rd (Unsignalized) | EBL | A | 0.01 | 8.4 | 0.0 | A | 0.04 | 8.1 | 0.8 |
|  | EBT/R | - | - | - | - | - | - | - | - |
|  | WBL | A | 0.04 | 8 | 0.8 |  | 0.11 | 9.9 | 3.0 |
|  | WBT/R | - | - | - | - | - | - | - | - |
|  | NBL | F | 1.30 | 202.3 | 118.5 | F | 1.87 | 476.2 | 143.3 |
|  | NBT/R | B | 0.12 | 10.2 | 3.0 | A | 0.15 | 13.9 | 3.8 |
|  | SBL | D | 0.42 | 33.5 | 15.0 | A | 0.56 | 72.3 | 18.5 |
|  | SBT/R | B | 0.08 | 11.8 | 2.3 | A | 0.04 | 10.2 | 0.8 |
|  | Overall | E | - | 46.4 | - | F | - | 73.9 | - |

Notes: $\quad$ Saturation flow rate of 1800 veh/h/lane PHF $=1.00$

As shown above, the unsignalized intersection of Cambrian Road and Apolune Way / Elevation Avenue operates with over capacity movements in the 2024 future total horizon.

As such, the City of Ottawa is encouraged to signalize the intersection of Cambrian Road at Apolune Way / Elevation Avenue in 2024.

### 15.2.2 2029 Future Total Intersection Operations

The 2029 future total future traffic volumes are illustrated in Figure 10 and the intersection operations are summarized in Table 13.

The level of service for signalized intersections is based on HCM 2010 calculations for individual lane movements and HCM $2000 \mathrm{v} / \mathrm{c}$ calculations for the overall intersection. The synchro worksheets for the 2029 future total horizon have been provided in Appendix H .

The signal timing at the intersection of Cambrian Road and Apolune Way was optimized and Amber Clearance, All Red Clearance, Walk, and Flash Don't Walk times were calculated using the Methodology provided in OTM Book 12-Traffic Signals. The westbound left-turn movement will operate as a protected and permissive turn during the PM peak period. Intersection geometry at Apolune Way and Cambrian Road is based on the RMA prepared by Stantec which has been provided in Appendix B.

Table 13: 2029 Future Total Intersection Operations

| Intersection | Lane | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) | LOS | V/C | Delay | Q (95 ${ }^{\text {th }}$ ) |
| Elevation Ave (Site Access\#1) / Apolune Way \& Cambrian Rd | EBL | A | 0.13 | 17.3 | 5.1 | A | 0.09 | 12.4 | 10.3 |
|  | EBT/R | A | 0.55 | 16.3 | 62.2 | F | 1.11 | 91.3 | \#361.4 |
|  | WBL | A | 0.29 | 18.4 | 18.4 | F | 1.03 | 110.4 | \#67.6 |
|  | WBT/R | D | 0.88 | 36.7 | \#159.2 | A | 0.34 | 9.8 | 51.8 |
|  | NBL | C | 0.78 | 32.3 | \#117.6 | F | 1.12 | 128.9 | \#149.5 |
|  | NBT/R | A | 0.15 | 0.3 | 0.0 | A | 0.19 | 0.6 | 0.0 |
|  | SBL | A | 0.18 | 16.0 | 18.9 | A | 0.22 | 38.4 | 23.9 |
|  | SBT/R | A | 0.05 | 0.1 | 0.0 | A | 0.03 | 0.1 | 0.0 |
|  | Overall | D | 0.83 | 25.5 | - | F | 1.11 | 75.2 | - |
| PM Peak Mitigation Measures-Eastbound Right-turn Lane |  |  |  |  |  |  |  |  |  |
| Elevation Ave (Site Access\#1) / Apolune Way \& Cambrian Rd | EBL | A | 0.13 | 18.8 | 5.3 | A | 0.12 | 20.2 | 13.2 |
|  | EBT | A | 0.25 | 16.9 | 32.5 | D | 0.90 | 46.7 | 172.5 |
|  | EBR | A | 0.30 | 3.3 | 12.0 | A | 0.64 | 23.2 | 85.2 |
|  | WBL | A | 0.18 | 16.3 | 16.8 | C | 0.77 | 40.2 | \#37.7 |
|  | WBT/R | E | 0.92 | 43.0 | \#165.4 | A | 0.44 | 17.9 | 70.2 |
|  | $N B L$ | D | 0.82 | 37.2 | \#122.2 | C | 0.72 | 41.3 | \#119.6 |
|  | NBT/R | A | 0.15 | 0.3 | 0.0 | A | 0.15 | 0.4 | 0.0 |
|  | SBL | A | 0.19 | 17.3 | 19.7 | A | 0.14 | 26.1 | 20.4 |
|  | SBT/R | A | 0.06 | 0.1 | 0.0 | A | 0.03 | 0.0 | 0.0 |
|  | Overall | D | 0.87 | 27.3 | - | D | 0.81 | 31.6 | - |
| Notes: Saturation flow rate of 1800 veh/h/lane PHF = 1.00 |  |  |  |  |  |  |  |  |  |

The intersection operations for the 2029 future total horizon generally operate satisfactorily during the peak hours with all $\mathrm{v} / \mathrm{c}$ ratios below 1.00 with the exception of the eastbound through/right-turn, westbound left-turn, and northbound left-turn movement in the PM peak period. High delays in the PM peak period are also noted at these movements. During the AM peak, the shared westbound through/right movement queue is anticipated to extend past the westbound left-turn lane and the northbound left-turn queue is anticipated to block the northbound through/ right-turn movement. During the PM peak, the shared eastbound through/right queue is expected to extend past the eastbound left-turn lane, the westbound left-turn queue is anticipated to block the westbound through / right-turn, and the northbound left-turn queue is anticipated to block the northbound through/rightturn movement. No other capacity issues are noted.

An eastbound right-turn lane will help improve the discussed capacity issues and is shown as a mitigation measure. With the implemented mitigation measure, the intersection operations for the 2029 future total horizon generally operate satisfactorily during the peak hours with all $\mathrm{v} / \mathrm{c}$ ratios below 1.00 and no high delays noted. During the AM peak, the shared westbound through/right movement queue is anticipated to extend past the westbound left-turn lane and the northbound left-turn queue is anticipated to block the northbound through/ right-turn movement. During the PM peak, the westbound left-turn queue will block the westbound through / right-turn movement and the northbound left-turn queue is anticipated to block the northbound through/right-turn movement. No other capacity issues are noted.

### 15.2.3 Network Intersection MMLOS

Table 14 summarizes the MMLOS analysis for the network intersection. The 2024 future, 2029 future and 2029 future with the suggested mitigation measure conditions have been evaluated in three rows. The future configuration of Cambrian Road is based on the RMA plans and the future configuration of Elevation Avenue is in
line with the collector road guidelines and will have a multi-use pathway. The multi-use pathway is expected to travel through the intersection of Cambrian Road, however as the presence of cycling facilities on Apolune Way is currently unknown it has been assumed that Apolune Way will be mixed traffic. Where applicable, AM and PM peak results have been displayed separately (AM(PM)). The MMLOS worksheets have been provided in Appendix I.

Table 14: Study Area Intersection MMLOS Analysis

| Intersection | Pedestrian LOS |  | Bicycle LOS |  | Transit LOS |  | Truck LOS |  | Auto LOS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | PLOS | Target | BLOS | Target | TLOS | Target | TrLOS | Target | ALOS | Target |
| Elevation Ave (Site Access\#1) / Apolune Way \& Cambrian Rd (2024) | C | C | E | B | E(D) | D | - | $\begin{gathered} \text { No } \\ \text { Target } \end{gathered}$ | C(C) | D |
| Elevation Ave (Site Access\#1) / Apolune Way \& Cambrian Rd (2029) | C | C | E | B | E(F) | D | - | $\begin{gathered} \text { No } \\ \text { Target } \end{gathered}$ | C(F) | D |
| Elevation Ave (Site Access\#1) / Apolune Way \& Cambrian Rd (2029 Mitigation Measure) | D | C | E | B | F(E) | D | - | $\begin{gathered} \text { No } \\ \text { Target } \end{gathered}$ | C(C) | D |

The target levels of service in a developing community were used to evaluate the study area intersection. The pedestrian LOS meets the target for the intersection for both the 2024 and 2029 future horizons. With the addition of the eastbound right-turn lane, the pedestrian LOS does not meet the target due to the resulting increase in pedestrian crossing distance. The bicycle LOS is limited due to mixed traffic conditions on the north approach, and high vehicle operating speeds and bike lane configuration on the east and west approaches and as such, does not meet the target. Transit LOS is limited due to signal delays and does not meet the target except during the PM peak in 2024. Auto LOS meets the outlined target in most cases. As no truck LOS target is provided for developing communities, it was not evaluated.

Given the limitations of the MMLOS framework, the pedestrian LOS cannot be achieved with the proposed addition of an eastbound right-turn lane at the intersection of Cambrian Road and Apolune Way / Elevation Avenue. The posted speed limit on Cambrian Road would need to be reduced to less than $40 \mathrm{~km} / \mathrm{h}$ and cycle tracks would need to be provided along Cambrian Road in order to meet the bicycle LOS. No mitigation measures are proposed for the transit LOS.

## 16 Summary of Improvements Indicates and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

## Proposed Site and Screening

- The proposed site includes a mixture of 61 detached homes and 589 townhouses
- Access to the development will be provided to the east of the proposed development along the east-west collector road extension to Dundonald Drive, and to the north along the planned north-south collector, Elevation Avenue
- The development is proposed to be completed as a single phase by 2024
- A TIA is required including the Design Review component and the Network Impact Component as determined by the TIA Screening
- The application for the proposed site is for a Zoning By-Law Amendment and plan of subdivision


## Existing Conditions

- Cambrian Road and Dundonald Drive are both collector roads
- Cambrian Road does not have sidewalks to the west of Seeley's Bay Street and Dundonald Drive has sidewalks on both sides
- On the study area roadways, cycling conditions are classified as mixed traffic conditions
- The existing transit routes \#75 and 275 stop on Cambrian Road, however no existing transit service runs along the boundary roads
- There are no existing intersections in the study area


## Development Generated Travel Demand

- The proposed development is forecasted to generate 654 people two-way trips during the AM peak and 769 people two-way trips during the PM peak
- Based on the area mode shares, a total of 392 two-way vehicle trips will be generated during the AM peak and 462 two-way vehicle trips during the PM peak
- The distribution of the site trips is estimated to be $80 \%$ to the north, $5 \%$ to the south, $10 \%$ to the east, and $5 \%$ to the west


## Background Conditions

- The background developments of The Meadows Phase 4, The Meadows Phase 5, 3809 Borrisokane Road, Half Moon Bay West, Citi Gate’s Highway 416 Employment Lands, Mattamy's Half Moon Bay North Phase 9, 3285 Borrisokane Road and 3713 Borrisokane Road were included within the background conditions, including a $2.0 \%$ background growth
- Large volumes generated by adjacent developments are noted
- Signalization of Cambrian Road and Apolune Way/Elevation Avenue is warranted in the 2029 future background horizon
- The northbound left-turn for the intersection of Cambrian Road and Apolune Way/Elevation Avenue in the PM peak period of the 2024 future background horizon operates with a LOS of $F$
- No operational issues are noted in the 2029 future background horizon for the intersection of Cambrian Road and Apolune Way/Elevation Avenue


## Development Design

- Elevation Avenue will be a collector road provided in the north-south direction and the extension of Dundonald Drive in the east-west direction within the development with 24.0 metre right-of-way and the remaining development roads are 16.5 metre local roads, 14.0 metre window roads, and 8.5 metre lanes
- Sidewalks are proposed on both sides of the collector roads, sidewalk connections are proposed in the vicinity of the park and other primary connections
- A multi-use pathway is proposed on the Elevation Avenue and off-road cycling facilities are proposed on the extension of Dundonald Drive
- Two pedestrian walkway connections within, and one on the northwest corner of the development will provide additional active mode connection to the property to the west as well as within the subdivision
- Traffic calming measures are recommended to reduce pedestrian crossing distances where sidewalks are provided, reduce turning speeds from the collector roads to local roads, and encourage the posted speed limits of $30 \mathrm{~km} / \mathrm{h}$ on local roads and $40 \mathrm{~km} / \mathrm{h}$ on the collector roads


## Boundary Street Design

- The Re-aligned Greenbank Road corridor will be a future boundary road to the proposed development however its construction is anticipated to occur outside of the future horizons of this TIA. As such, there are no existing or future boundary roads that will be considered.


## Access Intersection Design

- No access intersections were considered in this TIA


## TDM

- The lack of supporting infrastructure limits the potential for TDM measures to reduce the auto reliance anticipated for the proposed development
- Beyond the study horizons, the transit network along Re-Aligned Greenbank Road and the associated cycling and pedestrian networks will begin to produce the connectivity required to see a mode shift from the proposed development
- No TDM measures are recommended at this time beyond those required for zoning and standard subdivision design


## Neighbourhood Traffic Management

- The TIA thresholds are met for both Elevation Avenue and Dundonald Drive
- The Re-Aligned Greenbank Road corridor will provide the additional capacity required


## Transit

- No transit service is provided on the boundary road network
- A transit route along Elevation Avenue from Cambrian Road to Kilbirnie Drive will service this development beginning in the 2024 horizon
- To meet minimum area transit use, approximately one to two bus trips, or equivalent capacity, would be required to support the proposed development during the AM and PM peak hours


## Network Intersection Design

- Intersection geometry at Apolune Way and Cambrian Road is based on the RMA prepared by Stantec
- Signalization of Cambrian Road and Apolune Way/Elevation Avenue is warranted in the 2024 and 2029 future total horizons
- No operational issues are noted in the 2024 future total horizon for the intersection of Cambrian Road and Apolune Way/Elevation Avenue
- Based on the volumes at the 2029 future total horizon, an eastbound right-turn lane should be considered for the 2029 horizon year
- The study area intersection will not meet a combination of pedestrian, bicycle, or transit MMLOS targets due to road widths, mixed traffic conditions, high vehicle operating speeds, and intersection delays
- In order to meet the bicycle LOS, the posted speed limit on Cambrian Road would need to be reduced to less than $40 \mathrm{~km} / \mathrm{h}$ and cycle tracks would need to be proposed on Apolune Way


## 17 Conclusion

The proposed residential development at 3717 Borrisokane Road is recommended to proceed from a transportation perspective.

Prepared By:


Robin Marinac, EIT.
Transportation Engineering Intern


## Appendix A

TIA Screening Form and PM Certification Form

City of Ottawa 2017 TIA Guidelines
Step 1 - Screening Form

Date:
Project Number: Project Reference:

| 1.1 Description of Proposed Development |  |
| :--- | :--- |
| Municipal Address | 3717 Borrisokane Road |
| Description of Location | CON 3RF PT LOT 9 RP 5R-6254; PART 2 LESS RP 5R-13374 <br> PTS;9 \& 10 RD WIDENING |
| Land Use Classification | ME2-Mineral Extraction Operation-Pit Only and Urban <br> Expansion Area (Residential Use) |
| Development Size | 589 townhouse units, 61 detached housing units |
| Accesses | Access provided through adjacent properties, Dundonald <br> Drive and new north-south collector road |
| Phase of Development | Single Phase |
| Buildout Year | 2024 |
| TIA Requirement | Full TIA Required |


| 1.2 Trip Generation Trigger |  |  |
| :--- | :---: | :---: |
| Land Use Type | Townhomes or apartments |  |
| Development Size | 589 | Units |
| Trip Generation Trigger | Yes |  |

### 1.3 Location Triggers

Does the development propose a new driveway to a boundary street that is

| designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle | No |
| :--- | :--- |
| Networks? | No |
| Is the development in a Design Priority Area (DPA) or Transit-oriented | No |
| Development (TOD) zone? | Nocation Trigger |

1.4. Safety Triggers

| Are posted speed limits on a boundary street $80 \mathrm{~km} / \mathrm{hr}$ or greater? | Yes |
| :--- | :--- |
| Are there any horizontal/vertical curvatures on a boundary street limits sight <br> lines at a proposed driveway? | No |
| Is the proposed driveway within the area of influence of an adjacent traffic <br> signal or roundabout (i.e. within 300 m of intersection in rural conditions, or <br> within 150 m of intersection in urban/ suburban conditions)? | No |
| Is the proposed driveway within auxiliary lanes of an intersection? | No |
| Does the proposed driveway make use of an existing median break that <br> serves an existing site? | No |
| Is there is a documented history of traffic operations or safety concerns on <br> the boundary streets within 500 m of the development? | No |
| Does the development include a drive-thru facility? | No |
| Safety Trigger | Yes |

## TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

## CERTIFICATION

1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
4. I am either a licensed ${ }^{1}$ or registered ${ }^{2}$ professional in good standing, whose field of expertise [check $\sqrt{ }$ appropriate field(s)] is either transportation engineering $\sqrt{ }$ or transportation planning $\square$.

1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Dated at $\qquad$ (City)
this $\qquad$ day of $\qquad$ , 2018.

Name: Andrew Harte
(Please Print)
Professional Title: $\qquad$ Professional Engineer


Signature of Individarceitmer that s/he meets the above four criteria

| Office Contact Information (Please Print) |
| :--- |
| Address: 13 Markham Avenue |
| City / Postal Code: Ottawa / K2G 3Z1 |
| Telephone / Extension: (613) 697-3797 |
| E-Mail Address: Andrew.Harte@CGHTransportation.com |



## Appendix B

Apolune Way and Cambrian Road RMA




## Appendix C

Signal Warrants

Cambrian Road @ Apolune Way / Delphinus Avenue
2024 FB

## Justification \#7

| Justification | Description | Minimum | equirement | Minimum | equirement |  | mplia |  | Signal |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 Lane Highway |  | 2 or More Lanes |  | Sectional |  | Entire \% |  |
|  |  | Free Flow | Restr. Flow | Free Flow | Restr. Flow | Numerical | \% |  |  |
| 1. Minimum Vehicular Volume | A. Vehicle volume, all approaches (average hour) | 480 | 720 | 600 | 900 | 606 | 126\% | 118\% | No |
|  | B. Vehicle volume, along minor streets (average hour) | 120 | 170 | 120 | 170 | 141 | 118\% |  |  |
| 2. Delay to Cross Traffic | A. Vehicle volumes, major street (average hour) | 480 | 720 | 600 | 900 | 465 | 97\% |  |  |
|  | B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour) | 50 | 75 | 50 | 75 | 94 | 188\% | 97\% | No |

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
. Lowest section percentage governs justification
. Average hourly volumes estimated from peak hour volumes, $\mathrm{AHV}=\mathrm{PM} / 2$ or $(\mathrm{AM}+\mathrm{PM}) / 4$
2. T-intersection factor corrected, applies only to 1 B

Cambrian Road @ Apolune Way / Delphinus Avenue
2029 FB

## Justification \#7

| Justification | Description | Minimum Requirement 1 Lane Highway |  | Minimum Requirement <br> 2 or More Lanes |  | Compliance |  |  | Signal |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Sectional | Entire \% |  |
|  |  | Free Flow | Restr. Flow |  |  |  | Free Flow | Restr. Flow |  | Numerical | \% |
| 1. Minimum Vehicular Volume | A. Vehicle volume, all approaches (average hour) | 480 | 720 | 600 | 900 | 856 | 178\% | 178\% | Yes |
|  | B. Vehicle volume, along minor streets (average hour) | 120 | 170 | 120 | 170 | 228 | 190\% |  |  |
| 2. Delay to Cross Traffic | A. Vehicle volumes, major street (average hour) | 480 | 720 | 600 | 900 | 627 | 131\% | 131\% | No |
|  | B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour) | 50 | 75 | 50 | 75 | 152 | 304\% |  |  |

. Refer to OTM Book 12, pg 88, Nov 2007
. Lowest section percentage governs justification
. Average hourly volumes estimated from peak hour volumes, $\mathrm{AHV}=\mathrm{PM} / 2$ or (AM +PM ) / 4
4. T-intersection factor corrected, applies only to 1 B

Cambrian Road @ Apolune Way / Delphinus Avenue
2024 FT

## Justification \#7

| Justification | Description | Minimum Requirement 1 Lane Highway |  | Minimum Requirement <br> 2 or More Lanes |  | Compliance |  |  | Signal |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Sectional | Entire \% |  |
|  |  | Free Flow | Restr. Flow |  |  |  | Free Flow | Restr. Flow |  | Numerical | \% |
| 1. Minimum Vehicular Volume | A. Vehicle volume, all approaches (average hour) | 480 | 720 | 600 | 900 | 772 | 161\% | 161\% | Yes |
|  | B. Vehicle volume, along minor streets (average hour) | 120 | 170 | 120 | 170 | 231 | 193\% |  |  |
| 2. Delay to Cross Traffic | A. Vehicle volumes, major street (average hour) | 480 | 720 | 600 | 900 | 541 | 113\% | 113\% | No |
|  | B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour) | 50 | 75 | 50 | 75 | 173 | 346\% |  |  |

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes, $\mathrm{AHV}=\mathrm{PM} / 2$ or $(\mathrm{AM}+\mathrm{PM}) / 4$
4. T-intersection factor corrected, applies only to 1 B

Cambrian Road @ Apolune Way / Delphinus Avenue
2029 FT

## Justification \#7

| Justification | Description | Minimum | equirement | Minimum | equirement |  | mplian |  | Signal |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 Lane Highway |  | 2 or More Lanes |  | Sectional |  | Entire \% |  |
|  |  | Free Flow | Restr. Flow | Free Flow | Restr. Flow | Numerical | \% |  |  |
| 1. Minimum Vehicular Volume | A. Vehicle volume, all approaches (average hour) | 480 | 720 | 600 | 900 | 1021 | 213\% | 213\% | Yes |
|  | B. Vehicle volume, along minor streets (average hour) | 120 | 170 | 120 | 170 | 318 | 265\% |  |  |
| 2. Delay to Cross Traffic | A. Vehicle volumes, major street (average hour) | 480 | 720 | 600 | 900 | 703 | 146\% |  |  |
|  | B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour) | 50 | 75 | 50 | 75 | 231 | 462\% | 146\% | No |

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes, $\mathrm{AHV}=\mathrm{PM} / 2$ or $(\mathrm{AM}+\mathrm{PM}) / 4$
4. T-intersection factor corrected, applies only to 1 B

## Appendix D

2024 Future Background Synchro Sheets



## 08-09-2021

## Appendix E

2029 Future Background Synchro Sheets

| Lanes, Volumes, Timings <br> 1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian Rd |  |  |  |  |  |  |  |  |  | $\begin{aligned} & 2029 \text { FB AM } \\ & 3717 \text { Borrisokane Rd } \end{aligned}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | $\geqslant$ |  |  | 4 | 4 | $\dagger$ | $p$ | - | $\downarrow$ | $\checkmark$ |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\stackrel{1}{ }$ |  | ${ }^{7}$ | 1 |  | \% | $\uparrow$ |  | ${ }_{1}$ | ${ }_{\text {F }}$ |  |
| Traffic Volume (vph) | 13 | 184 | 129 | 66 | 649 | 20 | 262 | 0 | 137 | 91 | 0 | 45 |
| Future Volume (vph) | 13 | 184 | 129 | 66 | 649 | 20 | 262 | 0 | 137 | 91 | 0 | 45 |
| Satd. Flow (prot) | 1602 | 1637 | 0 | 1602 | 1738 | 0 | 1565 | 1567 | 0 | 1565 | 1567 | 0 |
| Flt Permitted | 0.171 |  |  | 0.513 |  |  | 0.728 |  |  | 0.669 |  |  |
| Sald. Flow (permi) | 288 | 1637 | 0 | 805 | 1738 | 0 | 1199 | 1567 | 0 | 1102 | 1567 | 0 |
| Satd. Flow (RTOR) |  | 59 |  |  | 3 |  |  | 698 |  |  | 217 |  |
| Lane Group Flow (vph) | 13 | 313 | 0 | 66 | 669 | 0 | 262 | 137 | 0 | 91 | 45 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 2 | 2 |  | 6 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split (s) | 23.7 | 23.7 |  | 23.7 | 23.7 |  | 30.4 | 30.4 |  | 30.4 | 30.4 |  |
| Total Split (s) | 53.0 | 53.0 |  | 53.0 | 53.0 |  | 37.0 | 37.0 |  | 37.0 | 37.0 |  |
| Total Split (\%) | 58.9\% | 58.9\% |  | 58.9\% | 58.9\% |  | 41.1\% | 41.1\% |  | 41.1\% | 41.1\% |  |
| Yellow Time (s) | 4.2 | 4.2 |  | 4.2 | 4.2 |  | 3.3 | 3.3 |  | 3.3 | 3.3 |  |
| All-Red Time (s) | 1.5 | 1.5 |  | 1.5 | 1.5 |  | 2.6 | 2.6 |  | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.7 | 5.7 |  | 5.7 | 5.7 |  | 5.9 | 5.9 |  | 5.9 | 5.9 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recaill Mode | None. | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Effct Green (s) | 33.9 | 33.9 |  | 33.9 | 33.9 |  | 31.5 | 31.5 |  | 31.5 | 31.5 |  |
| Actuated g/C Ratio | 0.44 | 0.44 |  | 0.44 | 0.44 |  | 0.41 | 0.41 |  | 0.41 | 0.41 |  |
| v/c Ratio | 0.10 | 0.42 |  | 0.17 | 0.88 |  | 0.54 | 0.13 |  | 0.20 | 0.06 |  |
| Control Delay | 13.5 | 12.9 |  | 13.3 | 32.7 |  | 25.0 | 0.3 |  | 19.6 | 0.2 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 13.5 | 12.9 |  | 13.3 | 32.7 |  | 25.0 | 0.3 |  | 19.6 | 0.2 |  |
| LOS | B | B |  | B | C |  | C | A |  | B | A |  |
| Approach Delay |  | 13.0 |  |  | 31.0 |  |  | 16.5 |  |  | 13.2 |  |
| Approach LOS |  | B |  |  | C |  |  | B |  |  | B |  |
| Queue Length 50th (m) | 1.1 | 23.4 |  | 5.6 | 84.0 |  | 28.5 | 0.0 |  | 8.4 | 0.0 |  |
| Queue Length 95th (m) | 4.3 | 40.4 |  | 12.5 | 126.7 |  | 63.9 | 0.0 |  | 22.6 | 0.0 |  |
| Internal Link Dist ( m ) |  | 519.1 |  |  | 354.3 |  |  | 415.4 |  |  | 383.2 |  |
| Turn Bay Length (m) | 37.5 |  |  | 37.5 |  |  | 30.0 |  |  | 30.0 |  |  |
| Base Capacity (vph) | 178 | 1039 |  | 537 | 1080 |  | 489 | 1052 |  | 450 | 768 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.07 | 0.30 |  | 0.12 | 0.62 |  | 0.54 | 0.13 |  | 0.20 | 0.06 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 77.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 65 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.88 |  |  |  |  |  |  |  |  |  |  |  |  |
| 08-09-2021 |  |  |  |  |  |  |  |  |  |  | Trans | ration |
| RM |  |  |  |  |  |  |  |  |  |  |  | Page 1 |

Lanes, Volumes, Timings

1. Delphinus Ave/Site Access \# 1/Apolune Way \& Cambrian Rd


| Lanes, Volumes, Timings <br> 1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian R |  |  |  |  |  |  |  |  |  | 2029 FB PM <br> 3717 Borrisokane Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\rangle$ |  | $\geqslant$ | $\downarrow$ | $\leftarrow$ | 4 |  | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 1 |  | ${ }^{*}$ | $\dagger$ |  | ${ }^{*}$ | F |  | ${ }_{1}$ | to |  |
| Traffic Volume (vph) | 45 | 633 | 255 | 131 | 316 | 68 | 193 | 0 | 98 | 62 | 0 | 25 |
| Future Volume (vph) | 45 | 633 | 255 | 131 | 316 | 68 | 193 | 0 | 98 | 62 | 0 | 25 |
| Satd. Flow (prot) | 1602 | 1670 | 0 | 1602 | 1698 | 0 | 1565 | 1567 | 0 | 1565 | 1567 | 0 |
| Flt Permitted | 0.534 |  |  | 0.096 |  |  | 0.741 |  |  | 0.694 |  |  |
| Sald. Flow (perin) | 900 | 1670 | 0 | 162 | 1698 | 0 | 1220 | 1567 | 0 | 1143 | 1567 | 0 |
| Satd. Flow (RTOR) |  | 31 |  |  | 21 |  |  | 343 |  |  | 595 |  |
| Lane Group Flow (vph) | 45 | 888 | 0 | 131 | 384 | 0 | 193 | 98 | 0 | 62 | 25 | 0 |
| Turn Type | Perm | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 2 | 2 |  | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 |  | 5.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split (s) | 23.7 | 23.7 |  | 9.5 | 23.7 |  | 30.4 | 30.4 |  | 30.4 | 30.4 |  |
| Total Split (s) | 69.0 | 69.0 |  | 10.0 | 79.0 |  | 31.0 | 31.0 |  | 31.0 | 31.0 |  |
| Total Split (\%) | 62.7\% | 62.7\% |  | 9.1\% | 71.8\% |  | 28.2\% | 28.2\% |  | 28.2\% | 28.2\% |  |
| Yellow Time (s) | 4.2 | 4.2 |  | 2.0 | 4.2 |  | 3.3 | 3.3 |  | 3.3 | 3.3 |  |
| All-Red Time (s) | 1.5 | 1.5 |  | 1.0 | 1.5 |  | 2.6 | 2.6 |  | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.7 | 5.7 |  | 3.0 | 5.7 |  | 5.9 | 5.9 |  | 5.9 | 5.9 |  |
| Lead/Lag | Lag | Lag |  | Lead |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? | Yes | Yes |  | Yes |  |  |  |  |  |  |  |  |
| Recrall Mode | None | None |  | None. | None |  | Max | Max |  | Max | Max |  |
| Act Efft Green (s) | 57.1 | 57.1 |  | 69.7 | 67.0 |  | 25.3 | 25.3 |  | 25.3 | 25.3 |  |
| Actuatedg/C Ratio | 0.55 | 0.55 |  | 0.67 | 0.64 |  | 0.24 | 0.24 |  | 0.24 | 0.24 |  |
| v/c Ratio | 0.09 | 0.95 |  | 0.64 | 0.35 |  | 0.65 | 0.15 |  | 0.22 | 0.03 |  |
| Control Delay | 11.2 | 42.5 |  | 24.7 | 8.7 |  | 49.2 | 0.5 |  | 36.9 | 0.1 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 11.2 | 42.5 |  | 24.7 | 8.7 |  | 49.2 | 0.5 |  | 36.9 | 0.1 |  |
| LOS | B | D |  | C | A |  | D | A |  | D | A |  |
| Approach Delay |  | 41.0 |  |  | 12.8 |  |  | 32.8 |  |  | 26.3 |  |
| Approach LOS |  | D |  |  | B |  |  | C |  |  | C |  |
| Queue Length 50th (m) | 4.1 | 156.6 |  | 8.5 | 30.1 |  | 38.2 | 0.0 |  | 10.9 | 0.0 |  |
| Queue Length 95th (m) | 9.4 | \#250.1 |  | \#27.2 | 45.0 |  | \#68.4 | 0.0 |  | 22.8 | 0.0 |  |
| Internal Link Dist (m) |  | 519.1 |  |  | 354.3 |  |  | 415.4 |  |  | 383.2 |  |
| Turn Bay Length (m) | 37.5 |  |  | 37.5 |  |  | 30.0 |  |  | 30.0 |  |  |
| Base Capacity (vph) | 551 | 1036 |  | 206 | 1212 |  | 296 | 640 |  | 277 | 831 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Splllback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.08 | 0.86 |  | 0.64 | 0.32 |  | 0.65 | 0.15 |  | 0.22 | 0.03 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 110 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 104 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.95 |  |  |  |  |  |  |  |  |  |  |  |  |
| 08-09-2021 |  |  |  |  |  |  |  |  |  |  | Trans | tation |
| RM |  |  |  |  |  |  |  |  |  |  |  | Page 1 |

Lanes, Volumes, Timings
Intersection Signal Delay 310

Intersection LOS: C

Intersection Capacity Utilization 90.2\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacty, queue may be longer.
Queue shown is maximum after two cycles.


## Appendix F

TDM Checklist

## TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

> | Legend |  |
| :--- | :--- |
| BASIC | $\begin{array}{l}\text { The measure is generally feasible and effective, and in most } \\ \text { cases would benefit the development and its users }\end{array}$ |
| BEITIER | $\begin{array}{l}\text { The measure could maximize support for users of sustainable } \\ \text { modes, and optimize development performance }\end{array}$ |
| $\begin{array}{l}\text { The measure is one of the most dependably effective tools to } \\ \text { encourage the use of sustainable modes }\end{array}$ |  |

| TDM measures: Residential developments |  |  | Check if proposed \& add descriptions |
| :---: | :---: | :---: | :---: |
| 1. TDM PROGRAM MANAGEMENT |  |  |  |
| 1.1 Program coordinator |  |  |  |
| basc | * 1.1.1 | Designate an internal coordinator, or contract with an external coordinator | $\square$ |
| 1.2 Travel surveys |  |  |  |
| beiter | 1.2 .1 | Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress | $\square$ |
| 2. WALKING AND CYCLING |  |  |  |
| 2.1 Information on walking/cycling routes \& destinations |  |  |  |
| easic | 2.1 .1 | Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium) | $\square$ |
|  | 2.2 | Bicycle skills training |  |
| BEItER | 2.2.1 | Offer on-site cycling courses for residents, or subsidize off-site courses | $\square$ |

TDM measures: Residential developments

1. TDM PROGRAM MANAGEMENT
1.1 Program coordinator

1 Designate an internal coordinator, or contract with $\square$
Travel surveys
2.1 Conduct periodic surveys to identify travel-related behaviours, attitudes,
and to track progress

## 2. WALKING AND CYCLING

2.1 Information on walking/cycling routes \& destinations
2.2.1 Offer on-site cycling courses for residents, or subsidize off-site courses

TDM Measures Checklist

## 3. TRANSIT

3.1.1 Display relevant transit schedules and route maps at entrances (multi-family, condominium)
3.1.2 Provide real-time arrival information display at $\quad \square$ entrances (multi-family, condominium)
3.2 Transit fare incentives
3.2.1 Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit
3.2.2 Offer at least one year of free monthly transit passes on residence purchase/move-in
3.3 Enhanced public transit service

BETTER * 3.3.1 Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)
3.4 Private transit service
BETTER
3.4.1 Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)
4. CARSHARING \& BIKESHARING
4.1 Bikeshare stations \& memberships

BETTER 4.1.1 Contract with provider to install on-site bikeshare station (multi-family)
4.1.2 Provide residents with bikeshare memberships, either free or subsidized (multi-family)
4.2 Carshare vehicles \& memberships

BETTER 4.2.1 Contract with provider to install on-site carshare vehicles and promote their use by residents
4.2.2 Provide residents with carshare memberships either free or subsidized

## 5. PARKING

### 5.1 Priced parking

EASIC * 5.1.1 Unbundle parking cost from purchase price (condominium)
5.1.2 Unbundle parking cost from monthly rent (multi-family)

|  | TDM measures: Residential developments |  |  <br> add descriptions |  |
| :--- | :--- | :--- | :--- | :--- |
| 6. |  |  |  | TDM MARKETING \& COMMUNICATIONS |

## Appendix G

2024 Future Total Synchro Sheets

| Lanes, Volumes, Timings <br> 1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian Rd |  |  |  |  |  |  |  |  |  | 2024 FT AM <br> 3717 Borrisokane Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\rangle$ | $\rightarrow$ | $\geqslant$ | $\downarrow$ | $\leftarrow$ | 4 |  | $\dagger$ | $p$ | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\dagger$ |  | \% | $\dagger$ |  | ${ }^{*}$ | F |  | ${ }^{1}$ | $\dagger$ |  |
| Traffic Volume (vph) | 13 | 166 | 163 | 48 | 486 | 20 | 303 | 0 | 94 | 91 | 0 | 45 |
| Future Volume (vph) | 13 | 166 | 163 | 48 | 486 | 20 | 303 | 0 | 94 | 91 | 0 | 45 |
| Satd. Flow (prot) | 1602 | 1616 | 0 | 1602 | 1735 | 0 | 1565 | 1567 | 0 | 1565 | 1567 | 0 |
| Flt Permitted | 0.238 |  |  | 0.452 |  |  | 0.728 |  |  | 0.696 |  |  |
| Sald. Flow (perin) | 401 | 1610 | 0 | 762 | 1735 | 0 | 1199 | 1567 | 0 | 1140 | 1567 | 0 |
| Satd. Flow (RTOR) |  | 73 |  |  | 3 |  |  | 687 |  |  | 275 |  |
| Lane Group Flow (vph) | 13 | 329 | 0 | 48 | 506 | 0 | 303 | 94 | 0 | 91 | 45 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 2 | 2 |  | 6 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split (s) | 23.7 | 23.7 |  | 23.7 | 23.7 |  | 30.4 | 30.4 |  | 30.4 | 30.4 |  |
| Total Split (s) | 47.0 | 47.0 |  | 47.0 | 47.0 |  | 43.0 | 43.0 |  | 43.0 | 43.0 |  |
| Total Split (\%) | 52.2\% | 52.2\% |  | 52.2\% | 52.2\% |  | 47.8\% | 47.8\% |  | 47.8\% | 47.8\% |  |
| Yellow Time (s) | 4.2 | 4.2 |  | 4.2 | 4.2 |  | 3.3 | 3.3 |  | 3.3 | 3.3 |  |
| All-Red Time (s) | 1.5 | 1.5 |  | 1.5 | 1.5 |  | 2.6 | 2.6 |  | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.7 | 5.7 |  | 5.7 | 5.7 |  | 5.9 | 5.9 |  | 5.9 | 5.9 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recrall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Efft Green (s) | 26.9 | 26.9 |  | 26.9 | 26.9 |  | 37.5 | 37.5 |  | 37.5 | 37.5 |  |
| Actuated g/C Ratio | 0.35 | 0.35 |  | 0.35 | 0.35 |  | 0.49 | 0.49 |  | 0.49 | 0.49 |  |
| v/c Ratio | 0.09 | 0.53 |  | 0.18 | 0.82 |  | 0.51 | 0.08 |  | 0.16 | 0.05 |  |
| Control Delay | 16.8 | 17.7 |  | 17.6 | 33.9 |  | 19.2 | 0.1 |  | 14.2 | 0.1 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 16.8 | 17.7 |  | 17.6 | 33.9 |  | 19.2 | 0.1 |  | 14.2 | 0.1 |  |
| LOS | B | B |  | B | C |  | B | A |  | B | A |  |
| Approach Delay |  | 17.7 |  |  | 32.5 |  |  | 14.7 |  |  | 9.5 |  |
| Approach LOS |  | B |  |  | C |  |  | B |  |  | A |  |
| Queue Length 50th (m) | 1.2 | 27.8 |  | 4.6 | 64.1 |  | 27.4 | 0.0 |  | 6.7 | 0.0 |  |
| Queue Length 95th (m) | 4.7 | 48.7 |  | 11.5 | 97.5 |  | 66.1 | 0.0 |  | 19.5 | 0.0 |  |
| Internal Link Dist (m) |  | 519.1 |  |  | 354.3 |  |  | 415.4 |  |  | 383.2 |  |
| Turn Bay Length (m) | 37.5 |  |  | 37.5 |  |  | 30.0 |  |  | 30.0 |  |  |
| Base Capacity (vph) | 219 | 919 |  | 418 | 953 |  | 591 | 1120 |  | 564 | 911 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Splllback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.06 | 0.36 |  | 0.11 | 0.53 |  | 0.51 | 0.08 |  | 0.16 | 0.05 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 76.1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 60 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.82 |  |  |  |  |  |  |  |  |  |  |  |  |
| 08-09-2021 |  |  |  |  |  |  |  |  |  |  | Tranp | tation |
| RM |  |  |  |  |  |  |  |  |  |  |  | Page 1 |

Lanes, Volumes, Timings

1. Delphinus Ave/Site Access \# 1/Apolune Way a Cambrian Rd


| Lanes, Volumes, Timings <br> 1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian R |  |  |  |  |  |  |  |  |  | 2024 FT PM <br> 3717 Borrisokane Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\rangle$ | $\rightarrow$ | $\geqslant$ | $\downarrow$ | $\leftarrow$ | 4 |  | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\dagger$ |  | ${ }^{*}$ | 1 |  | \% | F |  | ${ }_{1}$ | to |  |
| Traffic Volume (vph) | 45 | 491 | 297 | 90 | 285 | 68 | 245 | 0 | 71 | 62 | 0 | 25 |
| Future Volume (vph) | 45 | 491 | 297 | 90 | 285 | 68 | 245 | 0 | 71 | 62 | 0 | 25 |
| Satd. Flow (prot) | 1602 | 1646 | 0 | 1602 | 1694 | 0 | 1565 | 1567 | 0 | 1565 | 1567 | 0 |
| Flt Permitted | 0.500 |  |  | 0.147 |  |  | 0.741 |  |  | 0.711 |  |  |
| Sald. Flow (perin) | 843 | 1640 | 0 | 248 | 1694 | 0 | 1220 | 1567 | 0 | 1171 | 1567 | 0 |
| Satd. Flow (RTOR) |  | 58 |  |  | 23 |  |  | 366 |  |  | 582 |  |
| Lane Group Flow (vph) | 45 | 788 | 0 | 90 | 353 | 0 | 245 | 71 | 0 | 62 | 25 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 2 | 2 |  | 6 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split (s) | 23.7 | 23.7 |  | 23.7 | 23.7 |  | 30.4 | 30.4 |  | 30.4 | 30.4 |  |
| Total Split (s) | 58.0 | 58.0 |  | 58.0 | 58.0 |  | 32.0 | 32.0 |  | 32.0 | 32.0 |  |
| Total Split (\%) | 64.4\% | 64.4\% |  | 64.4\% | 64.4\% |  | 35.6\% | 35.6\% |  | 35.6\% | 35.6\% |  |
| Yellow Time (s) | 4.2 | 4.2 |  | 4.2 | 4.2 |  | 3.3 | 3.3 |  | 3.3 | 3.3 |  |
| All-Red Time (s) | 1.5 | 1.5 |  | 1.5 | 1.5 |  | 2.6 | 2.6 |  | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.7 | 5.7 |  | 5.7 | 5.7 |  | 5.9 | 5.9 |  | 5.9 | 5.9 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recrall Mode | None | None |  | None | None |  | Max | Max |  | Max | Max |  |
| Act Efft Green (s) | 40.0 | 40.0 |  | 40.0 | 40.0 |  | 26.6 | 26.6 |  | 26.6 | 26.6 |  |
| Actuated g/C Ratio | 0.51 | 0.51 |  | 0.51 | 0.51 |  | 0.34 | 0.34 |  | 0.34 | 0.34 |  |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.10 | 0.91 |  | 0.71 | 0.40 |  | 0.59 | 0.09 |  | 0.16 | 0.03 |  |
| Control Delay | 9.6 | 31.1 |  | 47.4 | 11.8 |  | 31.9 | 0.2 |  | 23.4 | 0.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 9.6 | 31.1 |  | 47.4 | 11.8 |  | 31.9 | 0.2 |  | 23.4 | 0.0 |  |
| LOS | A | C |  | D | B |  | C | A |  | C | A |  |
| Approach Delay |  | 30.0 |  |  | 19.0 |  |  | 24.8 |  |  | 16.7 |  |
| Approach LOS |  | C |  |  | B |  |  | C |  |  | B |  |
| Queue Length 50th (m) | 3.2 | 92.3 |  | 9.5 | 27.5 |  | 30.6 | 0.0 |  | 6.5 | 0.0 |  |
| Queue Length 95th (m) | 8.0 | 148.0 |  | \#35.6 | 44.0 |  | \#69.1 | 0.0 |  | 17.9 | 0.0 |  |
| Internal Link Dist (m) |  | 519.1 |  |  | 354.3 |  |  | 415.4 |  |  | 383.2 |  |
| Turn Bay Length (m) | 37.5 |  |  | 37.5 |  |  | 30.0 |  |  | 30.0 |  |  |
| Base Capacity (vph) | 572 | 1136 |  | 168 | 1158 |  | 413 | 773 |  | 397 | 915 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Splllback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.08 | 0.69 |  | 0.54 | 0.30 |  | 0.59 | 0.09 |  | 0.16 | 0.03 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 78.4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 75 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.91 |  |  |  |  |  |  |  |  |  |  |  |  |
| 08-09-2021 |  |  |  |  |  |  |  |  |  |  | Tran | tation |
| RM |  |  |  |  |  |  |  |  |  |  |  | Page 1 |

Lanes, Volumes, Timings
1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian Rd

| Intersection Signal Delay: 25.4 | Intersection LOS: C |  |
| :---: | :---: | :---: |
| Intersection Capacity Utilization 90.1\% | ICU Level of Service E |  |
| Analysis Period (min) 15 |  |  |
| \# 95th percentile volume exceeds capacty, queue may be longer. |  |  |
| Queue shown is maximum after two cycles. |  |  |
| Splits and Phases: 1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian Rd |  |  |
| $\rightarrow{ }_{\square 02}$ |  | - 04 |
| 58 s |  | 32 s |
| 4 |  | 408 |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 46.4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations | \% | F |  | \% | F |  | \% | F |  | 4 | $\stackrel{1}{ }$ |  |  |
| Traffic Vol, veh/h | 13 | 166 | 163 | 48 | 486 | 20 | 303 | 0 | 94 | 91 | 0 | 45 |  |
| Future Vol, veh/h | 13 | 166 | 163 | 48 | 486 | 20 | 303 | 0 | 94 | 91 | 0 | 45 |  |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - |  | None |  | - | None |  |  | None |  |
| Storage Length | 375 |  | - | 375 |  |  | 300 | - |  | 300 |  |  |  |
| Veh in Median Storage, | , \# | 0 |  | - | 0 | - |  | 0 |  | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - |  | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 13 | 166 | 163 | 48 | 486 | 20 | 303 | 0 | 94 | 91 | 0 | 45 |  |
| Major/Minor Ma | Major1 |  |  | Major2 |  |  | Minor1 |  |  | linor2 |  |  |  |
| Conficting Flow All | 506 | 0 | 0 | 329 | 0 | 0 | 889 | 876 | 248 | 913 | 947 | 496 |  |
| Stage 1 | - | - | - | - |  | - | 274 | 274 | - | 592 | 592 | - |  |
| Stage 2 | - | - | - | - | - | - | 615 | 602 | - | 321 | 355 | - |  |
| Critical Hdwy | 412 | - | - | $41 ?$ | - | - | 712 | 652 | 677 | 712 | 65 ? | 672 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Critical Hdwy Stg 2 |  |  | - |  | - |  | 6.12 | 5.52 | - | 6.12 | 5.52 |  |  |
| Follow-up Hdwy 2 | 2.218 | - | - | 2.218 | - |  | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1059 | - | - | 1231 | - |  | - 264 | 287 | 791 | 254 | 261 | 574 |  |
| Stage 1 |  |  |  |  |  |  | 732 | 683 |  | 493 | 494 |  |  |
| Stage 2 | - | - | - | - | - | - | 479 | 489 | - | 691 | 630 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1059 | - | - | 1231 |  |  | ~ 234 | 272 | 791 | 215 | 248 | 574 |  |
| Mov Cap-2 Maneuver |  | - |  |  |  |  | $\sim 234$ | 272 |  | 215 | 248 |  |  |
| Stage 1 | - | - | - | - |  |  | 723 | 675 |  | 487 | 475 |  |  |
| Stage 2 | - | - | - | - | - | - | 424 | 470 | - | 601 | 622 | - |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.3 |  |  | 0.7 |  |  | 156.8 |  |  | 26.3 |  |  |  |
| HCM LOS |  |  |  |  |  |  | F |  |  | D |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 NBLn2 |  | EBL | EBT | EBR | WBL | WBT | WBR SBLn1 SBLn2 |  |  |  |  |
| Capacity (veh/h) |  | 234 | 791 | 1059 | - | - | 1231 | - | - | 215 | 574 |  |  |
| HCM Lane V/C Ratio |  | 1.295 | 0.119 | 0.012 | - |  | 0.039 | - |  | 0.423 | 0.078 |  |  |
| HCM Control Delay (s) |  | 202.3 | 10.2 | 8.4 |  |  | 8 | - |  | 33.5 | 11.8 |  |  |
| HCM Lane LOS |  | F | B | A |  |  | A | - |  | D | B |  |  |
| HCM 95th \%tile Q(veh) |  | 15.8 | 0.4 | 0 | - | - | 0.1 | - | - | 2 | 0.3 |  |  |
| Notes |  | \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity |  |  |  |  |  |  |  |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 73.6 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations | \% | F |  | ${ }^{7}$ | $\hat{}$ |  | ${ }^{7}$ | $\stackrel{\rightharpoonup}{1}$ |  | ${ }^{7}$ | $\dagger$ |  |  |
| Traffic Vol, veh/h | 45 | 491 | 297 | 90 | 285 | 68 | 245 | 0 | 71 | 62 | 0 | 25 |  |
| Future Vol, veh/h | 45 | 491 | 297 | 90 | 285 | 68 | 245 | 0 | 71 | 62 | 0 | 25 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - | - | None |  | - | None | - | - | None |  |
| Storage Length Veh in Median Storage, \# | 375 | - | - | 375 | - | - | 300 | - | - | 300 | - | - |  |
|  | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 | 100 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mumt Flow | 45 | 491 | 297 | 90 | 285 | 68 | 245 | 0 | 71 | 62 | 0 | 25 |  |
| Major/Minor Major | Major1 |  |  | Major2 |  |  | Minor1 |  |  | Minor2 |  |  |  |
| Conficting Flow All 3 | 353 | 0 | 0 | 788 | 0 | 0 | 1242 | 1263 | 640 | 1264 | 1377 | 319 |  |
| Stage 1 | - | - | - |  | - | - | 730 | 730 |  | 499 | 499 | - |  |
| Stage 2 | - | - | - | - | - | - | 512 | 533 | - | 765 | 878 | - |  |
| Critical Hduy | $41 ?$ | - | - | $41 ?$ | - | - | 712 | 6.5 | $6 \geqslant 7$ | 712 | 657 | 672 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Critical Hdwy Stg 2 |  | - | - |  | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Follow-up Hdwy 2.2 | 2.218 |  |  | 2.218 |  |  | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver 12 | 1206 | - | - | 831 | - |  | ~152 | 170 | 475 | 146 | 145 | 722 |  |
| Stage 1 | - | - | - | - | - | - | 414 | 428 | - | 554 | 544 | - |  |
| Stage 2 | - | - | - | - | - | - | 545 | 525 | - | 396 | 366 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1206 | - | - | 831 | - |  | $\sim 131$ | 146 | 475 | 111 | 125 | 722 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - |  | ~131 | 146 | - | 111 | 125 | - |  |
| Stage 1 | - | - | - | - | - | - | 399 | 412 | - | 534 | 485 | - |  |
| Stage 2 | - | - | - | - | - | - | 469 | 468 | - | 324 | 352 | - |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.4 |  |  | 2 |  |  | \$372.3 |  |  | 54.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | F |  |  | F |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | NBLn2 | EBL | EBT | EBR | WBL | WBT | WBR S | BLn1 | BLn2 |  |  |
| Capacity (veh/h) |  | 131 | 475 | 1206 |  | - | 831 | - | - | 111 | 722 |  |  |
| HCM Lane VIC Ratio |  | 1.87 | 0.149 | 0.037 |  |  | 0.108 | - | - | 0.559 | 0.035 |  |  |
| HCM Control Delay (s) |  | 476.2 | 13.9 | 8.1 |  | - | 9.9 |  | - | 72.3 | 10.2 |  |  |
| HCM Lane LOS |  | F | B | A |  | - | A | - | - | F | B |  |  |
| HCM 95th \%tile Q(veh) |  | 19.1 | 0.5 | 0.1 |  | - | 0.4 | - | - | 2.6 | 0.1 |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ :Volume exceeds capa | acity | \$: D | elay exc | ceds 30 |  | +: Com | putation | Not De | efined | * All | major v | volume in | in platoon |

## 08-09-2021

## Appendix H

2029 Future Total Synchro Sheets

| Lanes, Volumes, Timings <br> 1: Delphinus Ave/Site Access \#1/Apolune Way \& Cambrian Rd |  |  |  |  |  |  |  |  | 2029 FT PM-Mitigation |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 1 | $\checkmark$ | $\leftarrow$ |  |  | $\uparrow$ | P |  | $\downarrow$ | $\downarrow$ |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ | * | \% | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  | ${ }^{1}$ | ${ }_{7}$ |  |
| Traffic Volume (vph) | 45 | 633 | 429 | 156 | 316 | 68 | 342 | O | 119 | 62 | 0 | 25 |
| Future Volume (vph) | 45 | 633 | 429 | 156 | 316 | 68 | 342 | 0 | 119 | 62 | 0 | 25 |
| Satd. Flow (prot) | 1602 | 1745 | 1483 | 1602 | 1698 | 0 | 1565 | 1567 | 0 | 1565 | 1567 | 0 |
| Flt Permitted | 0.534 |  |  | 0.129 |  |  | 0.741 |  |  | 0.681 |  |  |
| Sald. Flow (perin) | 900 | 1745 | 1483 | 218 | 1698 | 0 | 1220 | 1567 | 0 | 1122 | 1567 | 0 |
| Satd. Flow (RTOR) |  |  | 112 |  | 14 |  |  | 282 |  |  | 523 |  |
| Lane Group Flow (vph) | 45 | 633 | 429 | 156 | 384 | 0 | 342 | 119 | 0 | 62 | 25 | 0 |
| Turn Type | Perm | NA | Perm | pm+pt | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  | 2 | 6 |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 2 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 5.0 | 10.0 |  | 10.0 | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split (s) | 23.7 | 23.7 | 23.7 | 9.0 | 23.7 |  | 30.4 | 30.4 |  | 30.4 | 30.4 |  |
| Total Split (s) | 61.0 | 61.0 | 61.0 | 11.0 | 72.0 |  | 48.0 | 48.0 |  | 48.0 | 48.0 |  |
| Total Split (\%) | 50.8\% | 50.8\% | 50.8\% | 9.2\% | 60.0\% |  | 40.0\% | 40.0\% |  | 40.0\% | 40.0\% |  |
| Yellow Time (s) | 4.2 | 4.2 | 4.2 | 3.0 | 4.2 |  | 3.3 | 3.3 |  | 3.3 | 3.3 |  |
| All-Red Time (s) | 1.5 | 1.5 | 1.5 | 1.0 | 1.5 |  | 2.6 | 2.6 |  | 2.6 | 2.6 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.7 | 5.7 | 5.7 | 4.0 | 5.7 |  | 5.9 | 5.9 |  | 5.9 | 5.9 |  |
| Lead/Lag | Lag | Lag | Lag | Lead |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None |  | Max | Max |  | Max | Max |  |
| Act Effict Green (s) | 44.3 | 44.3 | 44.3 | 57.1 | 55.4 |  | 42.4 | 42.4 |  | 42.4 | 42.4 |  |
| Actuated g/C Ratio | 0.40 | 0.40 | 0.40 | 0.52 | 0.51 |  | 0.39 | 0.39 |  | 0.39 | 0.39 |  |
| v/c Ratio | 0.12 | 0.90 | 0.64 | 0.77 | 0.44 |  | 0.72 | 0.15 |  | 0.14 | 0.03 |  |
| Control Delay | 20.2 | 46.7 | 23.2 | 40.2 | 17.9 |  | 41.3 | 0.4 |  | 26.1 | 0.0 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 20.2 | 46.7 | 23.2 | 40.2 | 17.9 |  | 41.3 | 0.4 |  | 26.1 | 0.0 |  |
| LOS | C | D | C | D | B |  | D | A |  | C | A |  |
| Approach Delay |  | 36.5 |  |  | 24.3 |  |  | 30.7 |  |  | 18.6 |  |
| Approach LOS |  | D |  |  | C |  |  | C |  |  | B |  |
| Queue Length 50th ( m ) | 5.9 | 123.2 | 53.3 | 16.8 | 48.0 |  | 62.7 | 0.0 |  | 8.7 | 0.0 |  |
| Queue Length 95th ( m ) | 13.2 | 172.5 | 85.2 | \#37.7 | 70.2 |  | \#119.6 | 0.0 |  | 20.4 | 0.0 |  |
| Internal Link Dist (m) |  | 519.1 |  |  | 354.3 |  |  | 415.4 |  |  | 383.2 |  |
| Turn Bay Length ( $m$ ) | 37.5 |  | 15.0 | 37.5 |  |  | 30.0 |  |  | 30.0 |  |  |
| Base Capacity (vph) | 457 | 887 | 809 | 203 | 1041 |  | 472 | 779 |  | 434 | 927 |  |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.10 | 0.71 | 0.53 | 0.77 | 0.37 |  | 0.72 | 0.15 |  | 0.14 | 0.03 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 109.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.90 |  |  |  |  |  |  |  |  |  |  |  |  |
| 08-09-2021 |  |  |  |  |  |  |  |  |  |  | H Trans | ration |
|  |  |  |  |  |  |  |  |  |  |  |  | Page 1 |

Lanes, Volumes, Timings


## Appendix I

MMLOS Worksheets

| INTERSECTIONS |  | Cambrian Road and Apolune Way / Delphinus Ave (2024 AM) |  |  |  | (2024 PM) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Crossing Side | NORTH | SOUTH | EAST | WEST | NORTH | SOUTH | EAST | WEST |
| 든©0000 | Lanes | 3 | 3 | 3 | 3 |  |  |  |  |
|  | Median | No Median - 2.4 m | No Median - 2.4 m | No Median - 2.4 m | No Median - 2.4 m |  |  |  |  |
|  | Conflicting Left Turns | Permissive | Permissive | Permissive | Permissive |  |  |  |  |
|  | Conflicting Right Turns | Permissive or yield control | Permissive or yield control | Permissive or yield control | Permissive or yield control |  |  |  |  |
|  | Right Turns on Red (RToR) ? | RTOR allowed | RTOR allowed | RTOR allowed | RTOR allowed |  |  |  |  |
|  | Ped Signal Leading Interval? | No | No | No | No |  |  |  |  |
|  | Right Turn Channel | No Channel | No Channel | No Channel | No Channel |  |  |  |  |
|  | Corner Radius | 10-15m | 10-15m | 10-15m | 10-15m |  |  |  |  |
|  | Crosswalk Type | Std transverse markings | Std transverse markings | Std transverse markings | Std transverse markings |  |  |  |  |
|  | PETSI Score | 70 | 70 | 70 | 70 |  |  |  |  |
|  | Ped. Exposure to Traffic LoS | c | c | c | c | - | - | - | - |
|  | Cycle Length |  |  |  |  |  |  |  |  |
|  | Effective Walk Time |  |  |  |  |  |  |  |  |
|  | Average Pedestrian Delay |  |  |  |  |  |  |  |  |
|  | Pedestrian Delay LoS | - | - | $\cdot$ | $\cdot$ | - | - | - | - |
|  | Level of Service | C | C | C | C | - | - | - | - |
|  |  | C |  |  |  | - |  |  |  |
|  | Approach From | NORTH | SOUTH | EAST | WEST | NORTH | SOUTH | EAST | WEST |
| $\begin{aligned} & \text { 0 } \\ & \frac{0}{\mathrm{O}} \\ & \hline \mathbf{0} \end{aligned}$ | Bicycle Lane Arrangement on Approach | Mixed Traffic | Curb Bike Lane, Cycletrack or MUP | Curb Bike Lane, Cycletrack or MUP | Curb Bike Lane, Cycletrack or MUP |  |  |  |  |
|  | Right Turn Lane Configuration <br> Right Turning Speed | $\begin{gathered} \leq 50 \mathrm{~m} \\ \leq 25 \mathrm{~km} / \mathrm{h} \end{gathered}$ | Not Applicable <br> Not Applicable | Not Applicable <br> Not Applicable | Not Applicable <br> Not Applicable |  |  |  |  |
|  | Cyclist relative to RT motorists | D | Not Applicable | Not Applicable | Not Applicable | - | - | - | - |
|  | Separated or Mixed Traffic | Mixed Traffic | Separated | Separated | Separated | - | - | - | - |
|  | Left Turn Approach <br> Operating Speed | No lane crossed $>40 \text { to } \leq 50 \mathrm{~km} / \mathrm{h}$ | No lane crossed $>40 \text { to } \leq 50 \mathrm{~km} / \mathrm{h}$ | 1 lane crossed $\geq 60 \mathrm{~km} / \mathrm{h}$ | 1 lane crossed $\geq 60 \mathrm{~km} / \mathrm{h}$ |  |  |  |  |
|  | Left Turning Cyclist | B | B | E | E | - | - | - | - |
|  | Level of Service | D | B | E | E | - | - | - | - |
|  |  | E |  |  |  | - |  |  |  |
| $\begin{aligned} & \frac{\pi}{\omega} \\ & \text { 든 } \\ & \text { NㅡN } \end{aligned}$ | Average Signal Delay | $\leq 10 \mathrm{sec}$ | $\leq 20 \mathrm{sec}$ | $\leq 40 \mathrm{sec}$ | $\leq 20 \mathrm{sec}$ | $\leq 20$ sec | $\leq 30 \mathrm{sec}$ | $\leq 20 \mathrm{sec}$ | $\leq 30 \mathrm{sec}$ |
|  | Level of Service | B | C | E | C | C | D | C | D |
|  |  | E |  |  |  | D |  |  |  |
| $\begin{aligned} & \text { 들 } \\ & \text { 른 } \end{aligned}$ | Effective Corner Radius <br> Number of Receiving Lanes on Departure from Intersection |  |  |  |  |  |  |  |  |
|  | Level of Service | - | - | - | - | - | - | - | - |
|  |  | - |  |  |  | - |  |  |  |
| $\begin{aligned} & \circ \\ & \stackrel{H}{3} \end{aligned}$ | Volume to Capacity Ratio | 0.71-0.80 |  |  |  | 0.71-0.80 |  |  |  |
|  | Level of Service | C |  |  |  | C |  |  |  |


| Cambrian Road and Apolune Way / Delphinus Ave (2029 AM) |  |  |  | (2029 PM) |  |  |  | Cambrian Road and Apolune Way / Delphinus Ave (2029 AM Mitigated) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| NORTH | SOUTH | EAST | WEST | NORTH | SOUTH | EAST | WEST | NORTH | SOUTH | EAST | WEST |
| 3 | 3 | 3 | 3 |  |  |  |  | 3 | 3 | 3 | 4 |
| No Median - 2.4 m | No Median - 2.4 m | No Median - 2.4 m | No Median - 2.4 m |  |  |  |  | No Median - 2.4 m | No Median - 2.4 m | No Median - 2.4 m | No Median - 2.4 m |
| Permissive | Protected/ <br> Permissive | Permissive | Permissive |  |  |  |  | Permissive | Protected/ Permissive | Permissive | Permissive |
| Permissive or yield control | Permissive or yield control | Permissive or yield control | Permissive or yield control |  |  |  |  | Permissive or yield control | Permissive or yield control | Permissive or yield control | Permissive or yield control |
| RTOR allowed | RTOR allowed | RTOR allowed | RTOR allowed |  |  |  |  | RTOR allowed | RTOR allowed | RTOR allowed | RTOR allowed |
| No | No | No | No |  |  |  |  | No | No | No | No |
| No Channel | No Channel | No Channel | No Channel |  |  |  |  | No Channel | No Channel | No Channel | No Channel |
| 10-15m | 10-15m | 10-15m | 10-15m |  |  |  |  | 10-15m | 10-15m | 10-15m | 10-15m |
| Std transverse markings | Std transverse markings | Std transverse markings | Std transverse markings |  |  |  |  | Std transverse markings | Std transverse markings | Std transverse markings | Std transverse markings |
| 70 | 70 | 70 | 70 |  |  |  |  | 70 | 70 | 70 | 53 |
| c | c | c | c | - | - | - | - | c | c | c | D |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
| - | - | - | - | - | - | - | - | - | - | - | - |
| C | C | C | C | - | - | - | - | C | C | C | D |
| C |  |  |  | - |  |  |  | D |  |  |  |
| NORTH | SOUTH | EAST | WEST | NORTH | SOUTH | EAST | WEST | NORTH | South | EAST | WEST |
| Mixed Traffic | Curb Bike Lane, Cycletrack or MUP | Curb Bike Lane, Cycletrack or MUP | Curb Bike Lane, Cycletrack or MUP |  |  |  |  | Mixed Traffic | Curb Bike Lane, Cycletrack or MUP | Curb Bike Lane, Cycletrack or MUP | Curb Bike Lane, Cycletrack or MUP |
| $\leq 50 \mathrm{~m}$ | Not Applicable | Not Applicable | Not Applicable |  |  |  |  | $\leq 50 \mathrm{~m}$ | Not Applicable | Not Applicable | Not Applicable |
| $\leq 25 \mathrm{~km} / \mathrm{h}$ | Not Applicable | Not Applicable | Not Applicable |  |  |  |  | $\leq 25 \mathrm{~km} / \mathrm{h}$ | Not Applicable | Not Applicable | Not Applicable |
| D | Not Applicable | Not Applicable | Not Applicable | - | - | - | - | D | Not Applicable | Not Applicable | Not Applicable |
| Mixed Traffic | Separated | Separated | Separated | - | - | - | - | Mixed Traffic | Separated | Separated | Separated |
| No lane crossed $>40 \text { to } \leq 50 \mathrm{~km} / \mathrm{h}$ | No lane crossed $>40 \text { to } \leq 50 \mathrm{~km} / \mathrm{h}$ | 1 lane crossed $\geq 60 \mathrm{~km} / \mathrm{h}$ | 1 lane crossed $\geq 60 \mathrm{~km} / \mathrm{h}$ |  |  |  |  | No lane crossed $>40 \text { to } \leq 50 \mathrm{~km} / \mathrm{h}$ | No lane crossed $>40 \text { to } \leq 50 \mathrm{~km} / \mathrm{h}$ | 1 lane crossed $\geq 60 \mathrm{~km} / \mathrm{h}$ | 1 lane crossed $\geq 60 \mathrm{~km} / \mathrm{h}$ |
| B | B | E | E | - | - | - | - | B | B | E | E |
| D | B | E | E | - | - | - | - | D | B | E | E |
| E |  |  |  | - |  |  |  | E |  |  |  |
| $\leq 20 \mathrm{sec}$ | $\leq 30 \mathrm{sec}$ | $\leq 40 \mathrm{sec}$ | $\leq 20$ sec | $\leq 30$ sec | $>40 \mathrm{sec}$ | $\leq 40 \mathrm{sec}$ | $>40 \mathrm{sec}$ | $\leq 20 \mathrm{sec}$ | $\leq 30 \mathrm{sec}$ | $>40 \mathrm{sec}$ | $\leq 10 \mathrm{sec}$ |
| C | D | E | C | D | F | E | F | C | D | F | B |
| E |  |  |  | F |  |  |  | F |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
| - | - | - | - | - | - | - | - | - | - | - | - |
| - |  |  |  | - |  |  |  | - |  |  |  |
| 0.71-0.80 |  |  |  | >1.00 |  |  |  | 0.71-0.80 |  |  |  |
| C |  |  |  | F |  |  |  | C |  |  |  |



