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Commercial & Institutional

Environmental Restoration

Proposed Residential Development 1900-1920 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard, and 2990-3000 Conroy Road, Ottawa

Transportation Impact Assessment



Proposed Residential Development 1900-1920 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard, and 2990-3000 Conroy Road

Transportation Impact Assessment

Prepared By:

NOVATECH

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario K2M 1P6

> Dated: October 2022 Revised: September 2023

Novatech File: 122040 Ref: R-2022-109



September 29, 2023

City of Ottawa Planning and Growth Management Department 110 Laurier Ave. W., 4th Floor, Ottawa. Ontario K1P 1J1

Attention: Mr. Mike Giampa

Senior Engineer, Infrastructure Applications

Dear Mr. Giampa:

Reference: 1900-1920 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard,

and 2990-3000 Conroy Road

Revised Transportation Impact Assessment

Novatech File No. 122040

We are pleased to submit the following revised Transportation Impact Assessment (TIA), in support of Zoning By-Law Amendment, Site Plan Control, and Draft Plan of Condominium applications at 1900-1920 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard, and 2990-3000 Conroy Road, for your review and signoff. The structure and format of this report is in accordance with the City of Ottawa's *Transportation Impact Assessment Guidelines* (June 2017).

The original TIA prepared in support of this development was submitted in October 2022. This revised TIA reflects changes in the Site Plan and addresses City comments.

If you have any questions or comments regarding this report, please feel free to contact Brad Byvelds, or the undersigned.

Yours truly,

NOVATECH

Joshua Audia, P.Eng.

Project Engineer | Transportation



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check $\sqrt{\text{appropriate field(s)}}$] is either transportation engineering \square or transportation planning \square .
- 1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Fax: 613-560-6006

Ottawa (City)	this <u>29th</u> day of	September	, 2023
Name:	Brad Byve (Please	elds, P.Eng. Print)	
Professional Title:	Project Manager	r, Transportation	
	B. Byveld	4	
Signature of Ir	dividual certifier that s/he m	eats the above four crite	rio

Office Contact Information (Please Print)				
Address:	240 Michael Cowpland Drive, Suite 200			
City / Postal Code:	Ottawa, ON, K2M 1P6			
Telephone / Extension:	613-254-9643 x 286			
E-Mail Address:	b.byvelds@novatech-eng.com			

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EXECUTIVE SUMMARY

This Transportation Impact Assessment (TIA) has been prepared in support of Zoning By-Law Amendment, Site Plan Control, and Draft Plan of Condominium applications for the property located at 1900-1920 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard, and 2990-3000 Conroy Road (referred to as 2510 St. Laurent Boulevard in this TIA). The subject site is approximately 5.89 hectares in size and is currently vacant land. The subject site is surrounded by the following:

- Walkley Road, followed by parkland to the north,
- St. Laurent Boulevard, followed by commercial uses to the south,
- · Conroy Road, followed by commercial uses to the east, and
- Don Reid Drive, followed by commercial uses to the west.

The proposed development will consist of 160 townhouse dwellings. Access to the townhouses will be provided via two driveways to St. Laurent Boulevard and one driveway to Don Reid Drive. The development will be constructed in a single phase, with a buildout year of 2024.

The proponent owns additional lands north of the proposed development which includes the subject site's entire frontage to Walkley Road. These lands are anticipated to include a retirement home with approximately 150 units and an apartment building with approximately 100 units. Access to the development block is anticipated to occur through a right-in/right-out (RIRO) access to Walkley Road and an all-movement access through the adjacent signalized commercial access to Walkley Road. In total, 410 dwellings are proposed for the entire subject site. The boundaries of the subject site currently include most of the westernmost commercial driveway to 1950 Walkley Road and the southernmost commercial driveway to 2980 Conroy Road. These driveways are intended to continue serving the adjacent commercial plaza and conveying the affected lands to the proper addresses will be resolved through this application process.

The boundaries of the subject site currently include most of the westernmost commercial driveway to 1950 Walkley Road and the southernmost commercial driveway to 2980 Conroy Road. These driveways are intended to continue serving the adjacent commercial plaza and conveying the affected lands to the proper addresses will be resolved through this application process.

The subject site is located within the 'Evolving Neighbourhood' overlay, and is designated as 'Corridor – Minor' (Walkley Road) and 'Neighbourhood' on Schedule B3 of the City of Ottawa's Official Plan (2021, Council Adopted). The current zoning for the property is 'General Mixed Use' (GM[1327]), and the site is not located within any Community Design Plan or Secondary Plan areas. A Zoning By-Law Amendment is required to remove the site and use-specific zone provisions of Exception 1327. The proposed residential uses are permitted by the parent GM zone. The retirement home and apartment building located at the northwest corner of the proponent's lands will be subject to a separate Site Plan Control application in the future, but is included in the current Zoning By-Law Amendment application. As such, this TIA will consider the traffic generated by the future retirement home/apartments, and the future Site Plan Control application for the proposed retirement home/apartments will include details on the on-site design aspects (such as access locations, development design, and parking provisions).

The study area for this report includes the boundary roadways Walkley Road, Conroy Road, St. Laurent Boulevard, and Don Reid Drive, as well as the following intersections:

- Walkley Road/Don Reid Drive/Ryder Street
- Walkley Road/160m West of Conroy Road
- Walkley Road/Conroy Road
- St. Laurent Boulevard/Conroy Road
- St. Laurent Boulevard/Don Reid Drive

The selected time periods for the analysis are the weekday AM and PM peak hours, as they represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2024 build-out year and 2029 horizon year.

The conclusions and recommendations of this TIA can be summarized as follows:

Forecasting

 The proposed development is estimated to generate 170 person trips (including 67 vehicle trips) during the AM peak hour, and 193 person trips (including 79 vehicle trips) during the PM peak hour.

Development Design and Parking

- In general, the proposed development includes a pavement width of 6.5m to 6.7m for on-site roadways with perpendicular parking spaces or no on-street parking. Parallel parking spaces are provided on the south side of Street 1 (adjacent to the public park), the east side of Street 1 (adjacent to 2500 St. Laurent Boulevard), and on the north side of Street 3 (adjacent to the commercial access serving 1950 Walkley Road and 2980 Conroy Road). These parallel parking spaces are provided as lay-bys, to maintain a narrower pavement width outside of these spaces and reduce the operating speed of vehicles on-site.
- On-site concrete sidewalks will be provided along the south side of Street 1 between Don Reid Drive and Street 3, the east side of Street 1, the south side of Street 3, and the east side of the additional lands to the north. Midblock pathways will also be provided between the proposed public park and Street 2, and between Street 2 and Street 1 at Street 3. These sidewalks will connect the proposed development to the proposed parkland fronting Don Reid Drive, and to the existing sidewalks along Conroy Road, St. Laurent Boulevard, and Don Reid Drive.
- Any required TDM-supportive design and infrastructure measures in the TDM checklist that are relevant to townhouse developments have been met.
- Garbage collection will take place curbside in front of the proposed dwellings. The on-site fire route will include all private roadways within the subject site.
- The minimum parking requirements will be met. As every proposed dwelling will include their own garage, the ZBL does not identify any minimum bicycle parking requirements.

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Boundary Streets

- The results of the segment multi-modal level of service (MMLOS) analysis can be summarized as follows:
 - No boundary street meets the target pedestrian level of service (PLOS);
 - No boundary street meets the target bicycle level of service (BLOS);
 - Conroy Road meets the target transit level of service (TLOS), while Walkley Road does not:
 - All boundary streets meet the target truck level of service (TkLOS).
- Both sides of Walkley Road and Conroy Road do not meet the target PLOS C. Walkley Road can achieve the target PLOS C and Conroy Road can achieve a PLOS D by implementing sidewalks with a minimum width of 2.0m and a minimum boulevard width of 2.0m. This is identified for the City's consideration.
- The south side of St. Laurent Boulevard and west side of Don Reid Drive do not meet the target PLOS C, as sidewalks are only provided on one side of each roadway. Implementing curbside sidewalks with a minimum width of 1.8m are sufficient to achieve the target PLOS. This is identified for the City's consideration. The existing sidewalks on St. Laurent Boulevard and Don Reid Drive meet the target PLOS C, and therefore no recommendations for these sidewalks are identified. Any sidewalks that need to be reconstructed as a result of the proposed development will be reinstated to a width of 1.8m.
- Walkley Road does not meet the target BLOS B. The target BLOS B can only be achieved through the implementation of physically separated bikeways along Walkley Road. This is identified for the City's consideration.
- St. Laurent Boulevard and Don Reid Drive do not meet the target BLOS B. The target BLOS B can be achieved by providing curbside bike lanes with a minimum width of 1.5m, and reducing the operating speed to 50 km/h.
- Walkley Road does not meet the target TLOS B, which is achieved by providing bus lanes with no or limited parking/driveway friction. It is anticipated that this target will be met upon completion of the Baseline/Heron/Walkley/St. Laurent BRT project, which is anticipated to occur beyond 2031.

Access Intersections

- The proposed development includes two full-movement accesses to St. Laurent Boulevard and one full-movement access to Don Reid Drive. Depressed curbs and continuous sidewalks are proposed along the entirety of each access, in accordance with City standards. The design of each access meets the relevant provisions of the City's *Private* Approach By-Law.
- The proposed access to Don Reid Drive will have clear sightlines to Walkley Road to the
 north and St. Laurent Boulevard to the south. The proposed accesses to St. Laurent
 Boulevard are located on the inside of a slight curve, but will still achieve the sightlines
 recommended by the Transportation Association of Canada (TAC), provided that any
 vegetation within the ROW of St. Laurent Boulevard is trimmed and maintained. Therefore,
 no sightline concerns are anticipated.

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 The proposed accesses to St. Laurent Boulevard and Don Reid Drive are anticipated to operate with an acceptable vehicular level of service for the buildout year 2024 and horizon year 2029.

Transportation Demand Management

- A review of the City's *TDM Measures Checklist* has been conducted by the proponent, who has committed to providing the following TDM measures at the sales centre:
 - o Provide local area maps with walking/cycling access routes and key destinations;
 - Provide relevant transit schedules and route maps;
 - o Provide a multimodal travel option information package.

Neighbourhood Traffic Management

 The peak hour and daily NTM thresholds for both St. Laurent Boulevard and Don Reid Drive are exceeded by the existing traffic volumes. Since St. Laurent Boulevard and Don Reid Drive primarily serve industrial, commercial, or office uses, no neighbourhood traffic management measures have been recommended as part of this proposed development.

Transit

• The proposed development is anticipated to generate 52 AM peak hour transit trips, (including 35 boarding and 17 alighting), and 55 PM peak hour transit trips (including 25 boarding and 30 alighting). These additional transit trips are not anticipated to require more frequent service at any stops within the study area.

Intersection MMLOS

- The results of the intersection MMLOS analysis can be summarized as follows:
 - No signalized intersections meet the target PLOS;
 - No signalized intersections meet the target BLOS;
 - Walkley Road/160m West of Conroy Road and St. Laurent Boulevard/Conroy Road meets the target TLOS, while Walkley Road/Don Reid Drive/Ryder Street and Walkley Road/Conroy Road do not;
 - Walkley Road/Conroy Road meets the target TkLOS, while Walkley Road/Don Reid Drive/Ryder Street, Walkley Road/160m West of Conroy Road, and St. Laurent Boulevard/Conroy Road do not.
- All approaches at the study area intersections do not meet the target PLOS C. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or removing right turn channels where applicable. The south and east approaches at Walkley Road/Conroy Road meet the City's vehicle/pedestrian conflict threshold to consider zebra-striped crosswalks.
- For approaches with failing BLOS based on left turn characteristics, the target BLOS can be achieved by implementing two-stage, left-turn cycling facilities. Implementing bike boxes would also require restricting right turns on red (RTOR). This is identified for the City's consideration.

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- The south approach of Walkley Road/160m West of Conroy Road, the south and west approaches of Walkley Road/Conroy Road, and the east and west approaches of St. Laurent Boulevard/Conroy Road do not meet the target BLOS based on right turn characteristics. The provision of separated cycling facilities on Walkley Road and the east side of Conroy Road, and bike lanes on St. Laurent Boulevard is identified for the City's consideration.
- The north and west approaches of Walkley Road/Don Reid Drive/Ryder Street and all approaches of Walkley Road/Conroy Road do not meet the target TLOS B. It is anticipated that the target TLOS will be met on Walkley Road upon completion of the Baseline/Heron/Walkley/St. Laurent BRT project, and on Conroy Road with the implementation of isolated transit priority measures. No recommendations are identified for Ryder Street (i.e. a local roadway with no transit priority designation).
- Any approaches that do not meet the target TkLOS represent right turns into private approaches or onto local/collector roadways with no truck route designation, and therefore no recommendations are identified.

Existing Traffic Operations

- The eastbound through and westbound left turn movements at Walkley Road/Conroy Road operate at an Auto LOS E during the PM peak hour.
- During the AM peak hour, the Synchro analysis identifies that the maximum (95th-percentile) queue lengths of the westbound through movements at Walkley Road/Don Reid Drive/Ryder Street and Walkley Road/160m West of Conroy Road extend into the upstream intersections on Walkley Road.
- During the PM peak hour, the Synchro analysis identified that the maximum queue length
 of the northbound left turn movement at Walkley Road/Don Reid Drive/Ryder Street exceeds
 the storage length of the auxiliary northbound left turn, but is contained within the taper. The
 maximum queue length of the eastbound through movement at Walkley Road/Conroy Road
 extends into the upstream intersection on Walkley Road.

Background Traffic Operations

- Compared to the existing conditions, improvements in some movements is due to differences in the Peak Hour Factor parameter (0.9 in existing conditions and 1.0 in future conditions, per the 2017 TIA Guidelines).
- The eastbound through movement at Walkley Road/Conroy Road operates at an Auto LOS
 E during the PM peak hour. Increasing the green time for the eastbound-westbound phases
 has been reviewed, and the analysis indicates that this mitigation allows the eastbound
 through movement to operate at the target Auto LOS D.

Total Traffic Operations

- Compared to the future background traffic conditions, site-generated traffic is anticipated to have marginal impacts on traffic operations within the study area.
- Based on the foregoing, the proposed development is recommended from a transportation perspective.

1.0 SCREENING

1.1 Introduction

This Transportation Impact Assessment (TIA) has been prepared in support of Zoning By-Law Amendment, Site Plan Control, and Draft Plan of Condominium applications for the property located at 1900-1920 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard, and 2990-3000 Conroy Road (referred to as 2510 St. Laurent Boulevard in this TIA). The subject site is approximately 5.89 hectares in size and is currently vacant land.

The subject site is surrounded by the following:

- Walkley Road, followed by parkland to the north,
- St. Laurent Boulevard, followed by commercial uses to the south,
- Conroy Road, followed by commercial uses to the east, and
- Don Reid Drive, followed by commercial uses to the west.

An aerial of the vicinity around the subject site is provided in **Figure 1**.



1.2 Proposed Development

The proposed development will consist of 160 townhouse dwellings. Access to the townhouses will be provided via two driveways to St. Laurent Boulevard and one driveway to Don Reid Drive. The development will be constructed in a single phase, with a buildout year of 2024.

The proponent owns additional lands north of the proposed development which includes the subject site's entire frontage to Walkley Road. These lands are anticipated to include a retirement home with approximately 150 units and an apartment building with approximately 100 units. Access to the development block is anticipated to occur through a right-in/right-out (RIRO) access to Walkley Road and an all-movement access through the adjacent signalized commercial access to Walkley Road. In total, 410 dwellings are proposed for the entire subject site. The boundaries of the subject site currently include most of the westernmost commercial driveway to 1950 Walkley Road and the southernmost commercial driveway to 2980 Conroy Road. These driveways are intended to continue serving the adjacent commercial plaza and conveying the affected lands to the proper addresses will be resolved through this application process.

The subject site is located within the 'Evolving Neighbourhood' overlay, and is designated as 'Corridor – Minor' (Walkley Road) and 'Neighbourhood' on Schedule B3 of the City of Ottawa's Official Plan. The current zoning for the property is 'General Mixed Use' (GM[1327]), and the site is not located within any Community Design Plan or Secondary Plan areas. A Zoning By-Law Amendment is required to remove the site and use-specific zone provisions of Exception 1327. The proposed residential uses are permitted by the parent GM zone. The retirement home and apartment building located at the northwest corner of the proponent's lands will be subject to a separate Site Plan Control application in the future, but is included in the current Zoning By-Law Amendment application. As such, this TIA will consider the traffic generated by the future retirement home/apartments, and the future Site Plan Control application for the proposed retirement home/apartments will include details on the on-site design aspects (such as access locations, development design, and parking provisions).

A copy of the proposed site plan is included in **Appendix A**.

1.3 Screening Form

The City's 2017 TIA Guidelines identify three triggers for completing a TIA report, including trip generation, location, and safety. The criteria for each trigger are outlined in the City's TIA Screening Form, which is included in **Appendix B**. The trigger results are as follows:

- Trip Generation Trigger The development is anticipated to generate over 60 peak hour person trips; further assessment is **required** based on this trigger.
- Location Triggers The development does not propose a new connection to a boundary street designated in the City's Transit Priority, Rapid Transit, or Spine Bicycle Networks, and the development is not located in a Design Priority Area or Transit-Oriented Development zone; further assessment is **not required** based on this trigger.
- Safety Triggers The proposed development will include a driveway within the area of influence of a signalized intersection, and will be located within the auxiliary lane of an intersection; further assessment is **required** based on this trigger.

2.0 SCOPING

2.1 Existing Conditions

2.1.1 Roadways

All roadways within the study area fall under the jurisdiction of the City of Ottawa.

Walkley Road is an arterial roadway that generally runs on an east-west alignment between Riverside Drive and Ramsayville Road. Within the study area, Walkley Road has a four-lane divided urban cross-section, concrete sidewalks on both sides of the roadway, and a posted speed limit of 50 km/h. The roadway is classified as a truck route, allowing full loads. On-street parking is not permitted. The City's Official Plan identifies a right-of-way (ROW) protection of 44.5m for Walkley Road between Heron Road and the Greenbelt boundary. A ROW widening may be required.

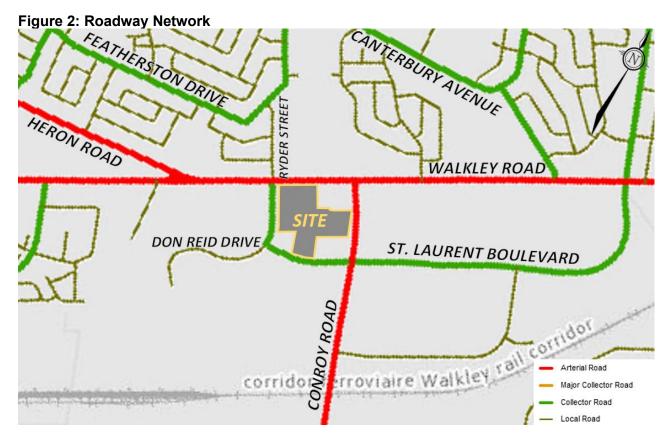
Conroy Road is an arterial roadway that generally runs on a north-south alignment between Walkley Road and Bank Street. Within the study area, Conroy Road has a five-lane divided urban cross-section, a concrete sidewalk on the east side of the roadway, an asphalt multi-use pathway on the west side of the roadway, and a posted speed limit of 60 km/h. The roadway is classified as a truck route, allowing full loads. On-street parking is not permitted. The City's Official Plan identifies a ROW protection of 44.5m for Conroy Road between Walkley Road and the Greenbelt boundary. A ROW widening is not required.

St. Laurent Boulevard is a curvilinear collector roadway that runs between Don Reid Drive and Russell Road. Within the study area, St. Laurent Boulevard has a two-lane undivided urban cross-section, a concrete sidewalk on the north side of the roadway, and a posted speed limit of 50 km/h. The roadway is not classified as a truck route. On-street parking is not permitted. The City's Official Plan does not identify a ROW protection for this section of St. Laurent Boulevard, and therefore no ROW widening is required.

Don Reid Drive is a curvilinear roadway that starts at Walkley Road and terminates approximately 830m southwest of the intersection of Walkley Road/Don Reid Drive/Ryder Street. Don Reid Drive is classified as a collector roadway between Walkley Road and St. Laurent Boulevard, and as a local roadway south of St. Laurent Boulevard. Within the study area, Don Reid Drive has a two-lane undivided urban cross-section, a concrete sidewalk on the east side of the roadway, and an unposted speed limit of 50 km/h. The roadway is not classified as a truck route. On-street parking is not permitted. The City's Official Plan does not identify a ROW protection for Don Reid Drive, and therefore no ROW widening is required.

Ryder Street is a local roadway that generally runs on a north-south alignment between Walkley Road and Featherston Drive. Within the study area, Ryder Street has a two-lane undivided urban cross-section, a concrete sidewalk on the east side of the roadway, and an unposted speed limit of 50 km/h. The roadway is not classified as a truck route. On-street parking is permitted in select areas on the west side of the roadway from April 1 to November 30.

The roadways of the greater area surrounding the subject site is illustrated in **Figure 2**.



2.1.2 Intersections

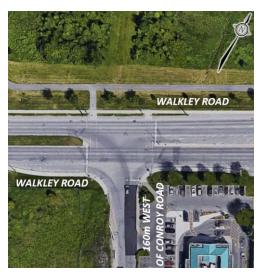
Walkley Road/Don Reid Drive/Ryder Street

- Signalized four-legged intersection
- North Approach (Ryder Street): one left turn lane and one shared through/right turn lane
- South Approach (Don Reid Drive): one left turn lane and one right turn lane (northbound through movement is restricted, except for authorized vehicles and cyclists)
- East/West Approaches (Walkley Road): one left turn lane, one through lane, and one shared through/right turn lane
- Standard crosswalks are provided on all approaches



Walkley Road/160m West of Conroy Road

- Signalized three-legged intersection
- South Approach (Access to 1950 Walkley Road): one left turn lane and one right turn lane
- East Approach (Walkley Road): one left turn lane and two through lanes
- West Approach (Walkley Road): two through lanes and one right turn lane
- Standard crosswalks are provided on all approaches
- Pocket bike lane is provided on west approach



Walkley Road/Conroy Road

- Signalized three-legged intersection
- South Approach (Conroy Road): two left turn lanes and one channelized right turn lane
- East Approach (Walkley Road): two left turn lanes and two through lanes
- West Approach (Walkley Road): two through lanes and one channelized right turn lane
- Standard crosswalks are provided on all approaches
- Pocket bike lanes are provided on south and west approaches



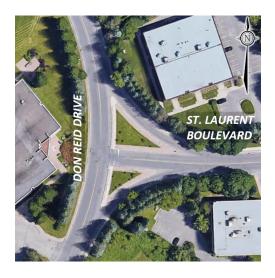
St. Laurent Boulevard/Conroy Road

- Signalized four-legged intersection
- North Approach (Conroy Road): one slotted left turn lane, one through lane, and one shared through/right turn lane
- South Approach (Conroy Road): one left turn lane, two through lanes, and one shared through/right turn lane
- East Approach (St. Laurent Boulevard): one left turn lane, one through lane, and one right turn lane
- West Approach (St. Laurent Boulevard): one left turn lane, one through lanes, and one channelized right turn lane
- Standard crosswalks are provided on all approaches
- Bike lanes are provided on north and south approaches



St. Laurent Boulevard/Don Reid Drive

- Unsignalized three-legged intersection
- Stop-controlled on St. Laurent Boulevard
- North Approach (Don Reid Drive): one shared left turn/through lane
- South Approach (Don Reid Drive): one through lane and one channelized right turn lane
- East Approach (St. Laurent Boulevard): one left turn lane and one channelized right turn lane
- Standard crosswalk is provided on east approach



2.1.3 Driveways

A review of the existing adjacent driveways (i.e. accesses within 200m of the subject site, per the 2017 TIA Guidelines) along the boundary roads are provided below.

Walkley Road, North Side

11 driveway to residences at 1845, 1847, 1849, 1853, 1855, 1857, 1859, 1863, 1865, 1867, 1869, 1871, 1873, 1875, 1877, 1883, 1885, 1897, & 1899 Walkley Road

Conroy Road, East Side

 One driveway to commercial uses at 2020 Walkley Road

St. Laurent Boulevard, North Side

- One driveway to commercial uses at 2490 and 2500 St. Laurent Boulevard
- One driveway to commercial uses at 2520
 St. Laurent Boulevard

Don Reid Drive, East Side

 One driveway to commercial/industrial uses at 2455 Don Reid Drive

Walkley Road, South Side

 Three driveways to commercial uses and a gas station at 1950 and 1970 Walkley Road

Conroy Road, West Side

- Two driveways to commercial uses and a gas station at 1970 Walkley Road and 2980 Conroy Road
- One driveway to commercial uses at 2490 and 2500 St. Laurent Boulevard

St. Laurent Boulevard, South Side

 Four driveways to commercial/industrial uses or government offices at 2505, 2507, 2515, & 2525 St. Laurent Boulevard and 3030 Conroy Road

Don Reid Drive, West Side

- One driveway to an early years centre at 2330
 Don Reid Drive
- Two driveways to commercial/industrial uses at 2410 and 2420 Don Reid Drive

2.1.4 Pedestrian and Cycling Facilities

Sidewalks are provided on both sides of Walkley Road, on the north side of St. Laurent Boulevard, and on the east side of Conroy Road, Don Reid Drive, and Ryder Street. A National Capital Commission (NCC) asphalt multi-use pathway (MUP) is provided on the north side of Walkley Road, which continues as a City of Ottawa MUP on the west side of Conroy Road. This pathway generally runs on a north-south alignment between Smyth Road at Roger Guindon Avenue (north of the study area) and Conroy Road at Hunt Club Road (south of the study area).

In the City's primary cycling network, Walkley Road and Conroy Road are classified as Spine Routes, while St. Laurent Boulevard, Ryder Street, and Don Reid Drive between Walkley Road and St. Laurent Boulevard are classified as Local Routes. The MUPs described above are shown in the cycling network as a Major Pathway. Bike lanes are provided along Conroy Road.

The NCC MUP connecting Smyth Road and Hunt Club, the entirety of the City's MUP along Conroy Road, and the section of Walkley Road between Heron Road and Conroy Road are all designated in the City's Crosstown Bikeway network.

The pedestrian and cycling network of the greater area surrounding the subject site is illustrated in **Figure 3**.

2.1.5 Area Traffic Management

There are no Area Traffic Management (ATM) studies within the study area that have been completed or are currently in progress.

Signage on Ryder Street indicates that the residential neighbourhood north of Walkley Road is a 'traffic calmed neighbourhood.' Flex posts and painted islands have been implemented in select curbside locations on Ryder Street. North of the study area, speed humps, bulbouts, and speed boards have been implemented on Featherston Drive north of Ryder Street.

2.1.6 Transit

A summary of the various OC Transpo routes which serve the study area is included in **Table 1**. The locations of bus stops in the vicinity of the subject site are described in **Table 2**, and are shown in **Figure 4**.

Detailed route information and an excerpt from the OC Transpo System Map are included in **Appendix C**.

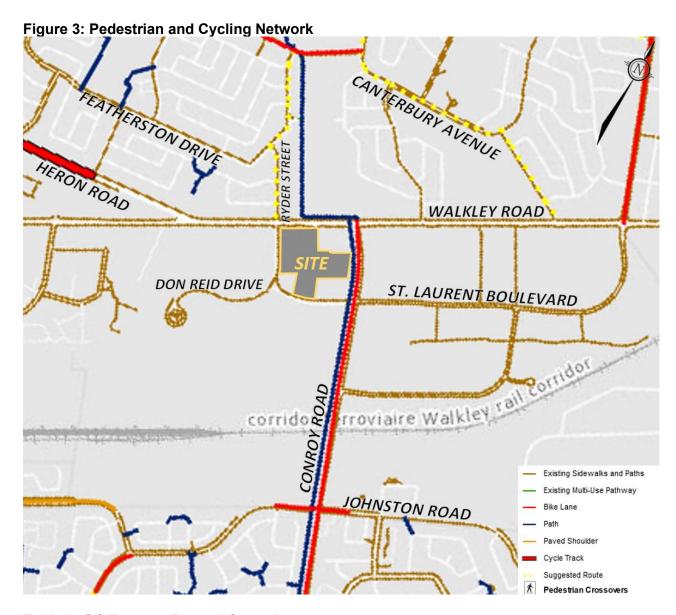


Table 1: OC Transpo Route Information

Route	From ↔ To	Frequency
40	St. Laurent ↔	All day service, seven days a week;
40	Greenboro	15- to 30-minute headways
46	Hurdman ↔	All day service, seven days a week;
40	Billings Bridge	30-minute headways
140	Heron Park ↔	Limited service during the day, Monday to Saturday;
140	Billings Bridge	30-minute headways
291	Hurdman ↔	Peak period service, weekdays only;
291	Herongate	30-minute headways in peak direction only
644	Canterbury ↔	Service at select times on school days only
044	Greenboro	Service at select times on school days only
649	Hillcrest ↔	Service at select times on school days only
049	Greenboro	Service at select times on school days only
689	Omer-Deslauriers ↔	Sorvice at coloct times on school days only
009	Billings Bridge	Service at select times on school days only

Table 2: OC Transpo Transit Stops

Stop	Location	Routes Serviced
#1321	East side of Conroy Road, south of St. Laurent Boulevard	40, 644, 649
#1899	South side of Walkley Road, east of Harding Road	46, 644, 649
#2344	West side of Conroy Road, south of St. Laurent Boulevard	40, 644, 649
#4307	North side of St. Laurent Boulevard, east of Conroy Road	40
#4311	South side of St. Laurent Boulevard, east of Conroy Road	40
#6927	South side of Walkley Road, midblock between Don Reid Drive and 160m West of Conroy Road	46, 689
#7200	South side of Walkley Road, west of Holly Lane	46, 689
#7202	North side of Walkley Road, west of Holly Lane	46, 140, 291, 689
#7281	North side of Walkley Road, midblock between Conroy Road and 160m West of Conroy Road	46, 689
#7282	South side of Walkley Road, east of Conroy Road	46, 644, 649
#7283	North side of Walkley Road, midblock between Heron Road and Ryder Street	46, 140, 291, 689
#8324	North side of Walkley Road, west of Harding Road	46, 644, 649
#8388	South side of Walkley Road, midblock between Heron Road and Don Reid Drive	46, 291, 689
#8391	North side of Walkley Road, west of Ryder Street	46, 140, 291, 689
#8398	East side of Ryder Street, north of Walkley Road	291





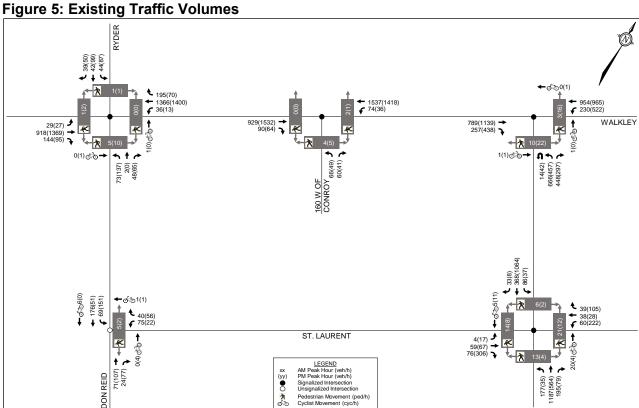
Existing Traffic Volumes 2.1.7

Weekday traffic counts completed by the City of Ottawa or coordinated by Novatech were used to determine the existing pedestrian, cyclist, and vehicular traffic volumes at the study area intersections. These counts were completed on the dates listed below:

•	Walkley Road/Don Reid Drive/Ryder Street	November 29, 2016	(City)
•	Walkley Road/160m West of Conroy Road	January 8, 2019	(City)
•	Walkley Road/Conroy Road	February 22, 2018	(City)
•	St. Laurent Boulevard/Conroy Road	June 1, 2017	(City)
•	St. Laurent Boulevard/Don Reid Drive	June 8, 2022	(Nova)

It is noted that the City has traffic count data from January 2022 for Walkley Road/Don Reid Drive/ Ryder Street, which was collected at a time when restrictions related to the COVID-19 pandemic were in place. Comparing the 2022 data to the 2016 data, peak hour volumes at all approaches have decreased by approximately 20% to 40%, and are not consistent with the traffic volumes observed at the other study area intersections. Therefore, the 2016 count data at Walkley Road/Don Reid Drive/Ryder Street has been considered in this TIA.

All traffic count data previously discussed are included in **Appendix D**. Traffic volumes within the study area are shown in Figure 5.



Based on the traffic count data above, the approximate average annual daily traffic (AADT) volumes for the boundary streets can be summarized as follows:

Walkley Road (Don Reid Drive/Ryder Street to Conroy Road): 38,710 vpd;
 Conroy Road (Walkley Road to St. Laurent Boulevard): 18,580 vpd;
 St. Laurent Boulevard (Don Reid Drive to Conroy Road): 3,930 vpd;
 Don Reid Drive (Walkley Road to St. Laurent Boulevard): 4,330 vpd.

2.1.8 Collision Records

Historical collision data from the last five years was obtained from the City's Public Works and Service Department for the study area intersections and midblock segments. Copies of the collision summary reports are included in **Appendix E**.

The collision data has been evaluated to determine if there are any identifiable collision patterns, which are defined in the *2017 TIA Guidelines* as 'more than six collisions in five years' for any one movement. The number of collisions at each intersection from January 1, 2016 to December 31, 2020 is summarized in **Table 3**.

Table 3: Reported Collisions

Intersection or		Impact Types					
Street Segment	Approach	Angle	Rear End	Sideswipe	Turning Mvmt	SMV ⁽¹⁾ / Other	Total
Walkley Road/ Don Reid Drive/Ryder Street	1	17	13	3	1	2	37
Walkley Road/ 160m West of Conroy Road	-	-	5	-	4	2	11
Walkley Road/ Conroy Road	-	4	51	11	4	4	74
St. Laurent Boulevard/ Conroy Road	-	7	10	3	2	-	22
St. Laurent Boulevard/ Don Reid Drive	-	1	-	-	-	-	1
Walkley Road btwn Don Reid Drive & 160m West of Conroy Road	-	-	4	1	-	-	5
Walkley Road btwn Conroy Road & 160m West of Conroy Road	-	2	4	-	1	-	7
Conroy Road btwn Walkley Road & St. Laurent Boulevard	-	-	1	1	-	1	3
St. Laurent Boulevard btwn Conroy Road & Don Reid Drive	-	1	-	-	1	-	2
Don Reid Drive btwn Walkley Road & St. Laurent Boulevard	-	-	-	-	-	-	0

^{1.} SMV = Single Motor Vehicle

Walkley Road/Don Reid Drive/Ryder Street

A total of 37 collisions were reported at this intersection over the last five years, of which there were one approaching impact, 17 angle impacts, 13 rear-end impacts, three sideswipe impacts, one turning movement impact, and two single-vehicle/other impacts. Ten of the 37 collisions resulted in injuries, but none caused fatalities. Thirteen of the collisions occurred in poor driving conditions. Two collisions involved cyclists, and no collisions involved pedestrians.

Of the 17 angle impacts, two involved a northbound vehicle and a westbound cyclist, and 15 involved a southbound vehicle and a westbound vehicle. Four of the 17 collisions occurred in poor driving conditions. The 15 southbound-westbound angle impacts meet the threshold to be considered a collision pattern. The apparent driver action in 14 of the 15 impacts was the westbound driver disobeying the traffic control. While the intersection appears to have standard geometry and no appreciable changes in grade, it is possible that driver sightlines are obscured at the northeast corner of the intersection, where there is currently fencing and tall vegetation. It should be noted that daylight triangles are not provided at the northwest or northeast corners of the intersection.

Of the 13 rear-end impacts, one involved southbound vehicles, five involved eastbound vehicles, and seven involved westbound vehicles. Three of the 13 collisions occurred in poor driving conditions. The seven westbound rear-end impacts meet the collision pattern threshold. It is anticipated that the number of rear-end impacts at this intersection is a function of high volumes on Walkley Road.

The two collisions involving cyclists included a northbound right-turning vehicle and a westbound through cyclist, suggesting that these cyclists were riding on the south sidewalk of Walkley Road.

Walkley Road/160m West of Conroy Road

A total of 11 collisions were reported at this intersection over the last five years, of which there were five rear-end impacts, four turning movement impacts, and two single vehicle/other impacts. Seven of the 11 collisions resulted in injuries, but none caused fatalities. Three of the collisions occurred in poor driving conditions. No collisions involved cyclists or pedestrians.

Walkley Road/Conroy Road

A total of 74 collisions were reported at this intersection over the last five years, of which there were four angle impacts, 51 rear-end impacts, 11 sideswipe impacts, four turning movement impacts, and four single vehicle/other impacts. Eight of the 74 collisions resulted in injuries, but none caused fatalities. Twenty-nine of the collisions occurred in poor driving conditions. One collision involved a pedestrian, and no collisions involved cyclists.

Of the 51 rear-end impacts, 16 involved northbound vehicles, 20 involved eastbound vehicles, and 15 involved westbound vehicles. Fifteen of the 51 collisions occurred in poor driving conditions. Each approach of this intersection meets the collision pattern threshold. It is anticipated that the number of rear-end impacts at this intersection is a function of the high traffic volumes observed on both Walkley Road and Conroy Road.

Of the 11 sideswipe impacts, three involved northbound vehicles, two involved eastbound vehicles, and six involved westbound vehicles. Three of the 11 collisions occurred in poor driving conditions.

The collision involving a pedestrian occurred in dark driving conditions, and involved a westbound through vehicle. The collision records identify that the vehicle was being driven properly, suggesting that the pedestrian may not have crossed during the appropriate 'walk' phase.

St. Laurent Boulevard/Conroy Road

A total of 22 collisions were reported at this intersection over the last five years, of which there were seven angle impacts, ten rear-end impacts, three sideswipe impacts, and two single vehicle/other impacts. Five of the 22 collisions resulted in injuries, but none caused fatalities. Seven of the collisions occurred in poor driving conditions. No collisions involved cyclists or pedestrians.

Of the seven angle impacts, five involved a northbound vehicle and a westbound vehicle, and two involved a southbound vehicle and a westbound vehicle. Three of the seven collisions occurred in poor driving conditions.

Of the ten rear-end impacts, two involved northbound vehicles, three involved southbound vehicles, three involved eastbound vehicles, and two involved westbound vehicles. One of the ten collisions occurred in poor driving conditions.

St. Laurent Boulevard/Don Reid Drive

One collision has been reported at this intersection over the last five years, which was an angle impact in poor driving conditions. This collision did not result in injuries, and did not involve cyclists or pedestrians.

Walkley Road between Don Reid Drive and 160m West of Conroy Road

A total of five collisions were reported along this segment over the last five years, of which there were four rear-end impacts and one sideswipe impact. All impacts involved eastbound vehicles. Three of the five collisions occurred in poor driving conditions, and no collisions resulted in injuries. No collisions involved cyclists or pedestrians.

Walkley Road between Conroy Road and 160m West of Conroy Road

A total of seven collisions were reported along this segment over the last five years, of which there were two angle impacts, four rear-end impacts, and one turning movement impact. Four of the seven collisions occurred in poor driving conditions, and no collisions resulted in injuries. No collisions involved cyclists or pedestrians.

Conroy Road between Walkley Road and St. Laurent Boulevard

A total of three collisions were reported along this segment over the last five years, of which there was one rear-end impact, one sideswipe impact, and one single vehicle/other impact. Two of the three collisions occurred in poor driving conditions, and the single-vehicle impact resulted in injuries. No collisions involved cyclists or pedestrians.

St. Laurent Boulevard between Conroy Road and Don Reid Drive

A total of two collisions were reported along this segment over the last five years, of which there was one angle impact and one single vehicle/other impact. One of the two collisions occurred in poor driving conditions, and neither collision resulted in injuries. The collisions did not involve cyclists or pedestrians.

2.2 Planned Conditions

2.2.1 Planned Transportation Projects

The 2013 Ottawa Cycling Plan and 2013 Ottawa Pedestrian Plan do not identify any improvements within the study area.

The City's 2013 Transportation Master Plan (TMP) identifies a Rapid Transit and Transit Priority (RTTP) project within the study area. The Baseline/Heron/Walkley/St. Laurent Bus Rapid Transit (BRT) project will provide high-quality transit access to employment, commercial, and institutional land uses along the corridor. In the 2031 Affordable Network, at-grade BRT is planned to run from Baseline Station to Heron Station (i.e. west of the study area). In the 2031 Network Concept, at-grade BRT will connect from Bayshore Station to St. Laurent Station. The 2031 Network Concept will not be implemented prior to 2031.

Conroy Road is identified in the City's TMP for transit signal priority and queue jump lanes between Walkley Road and Hunt Club Road. This corridor is part of the City's 2031 RTTP Network Concept, but will not be implemented prior to 2031.

The Alta Vista Transportation Corridor is identified in the 2031 Roadway Network Concept, as a new four-lane roadway (including two peak-period bus lanes) between the Ottawa Health Services Centre and Walkley Road at Conroy Road. The roadway will also include transit signal priority and queue jump lanes, and will improve transit access to the Ottawa Hospital, CHEO, and the Canadian Forces Health Care Centre. The roadway is anticipated to address capacity deficiencies and the Environmental Assessment (EA) is complete. While this corridor is part of the City's 2031 Network Concept, it will not be implemented prior to 2031.

2.2.2 Other Area Developments

In proximity of the subject site, there are multiple other developments that are under construction, approved, or in the approval process. In the list below, only development applications significant enough to necessitate transportation studies are included.

Timbercreek Heron Gate

The proposed redevelopment is located at 2848, 2851, 2881, and 2898 Baycrest Drive, and 2820 and 2831 Cedarwood Drive. In seven blocks, the redevelopment will consist of 118 low-rise dwellings, 2,047 mid-rise dwellings, and 2,874 high-rise dwellings. A TIA was prepared in April 2021 by CGH, in support of Zoning By-Law Amendment and Official Plan Amendment applications, and notes that subsequent TIAs will be required as Site Plan Control applications for each block as the project moves forward. Per the 2021 TIA, the anticipated buildout year for this development is 2040, and therefore analysis was conducted for an interim year 2030 and ultimate buildout year 2040.

2020 Walkley Road and 2935 Conroy Road

The proposed redevelopment will consist of three single-storey warehouses with a total gross floor area (GFA) of approximately 262,715 ft². A TIA was prepared in August 2021 by Novatech, in support of Zoning By-Law Amendment and Site Plan Control applications. Per the 2021 TIA, the anticipated buildout year of the development is 2023, and analysis was conducted for the buildout year 2023 and horizon year 2028.

2500 St. Laurent Boulevard

The now-constructed development consists of two two-storey office buildings with a total GFA of approximately 68,134 ft². A Transportation Brief was prepared in October 2017 by Stantec. Per the brief, the anticipated buildout year was 2021 (i.e. after traffic count data was collected at all signalized study area intersections), and analysis was conducted for the buildout year 2021 and horizon year 2026.

2190 Halifax Drive

The proposed development will consist of 202 additional high-rise dwellings. A TIA was prepared in July 2019 by Dillon, in support of a Site Plan Control application. Per the 2019 TIA, the anticipated buildout year was 2021, and analysis was conducted for the buildout year 2021 and horizon year 2026. For the purposes of this TIA, buildout of this development is anticipated to occur by 2024 (i.e. the assumed buildout year for this application).

2.3 Study Area and Time Periods

The study area for this report includes the boundary roadways Walkley Road, Conroy Road, St. Laurent Boulevard, and Don Reid Drive, as well as the following intersections:

- Walkley Road/Don Reid Drive/Ryder Street
 St. Laurent Boulevard/Conroy Road
- Walkley Road/160m West of Conroy Road
- St. Laurent Boulevard/Don Reid Drive
- Walkley Road/Conroy Road

The selected time periods for the analysis are the weekday AM and PM peak hours, as they represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2024 build-out year and 2029 horizon year.

2.4 **Exemptions Review**

This module reviews possible exemptions from the final Transportation Impact Assessment, as outlined in the 2017 TIA Guidelines. The applicable exemptions for this site are shown in Table 4.

Table 4: TIA Exemptions

Module	Element	Exemption Criteria	Status
Design Review	Component		
4.1 Development	4.1.2 Circulation and Access	Only required for site plans	Not Exempt
Design	4.1.3 New Street Networks	Only required for plans of subdivision	Not Exempt
4.2	4.2.1 Parking Supply	Only required for site plans	Not Exempt
Parking	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Exempt
Network Impact	Component		
4.5 Transportation Demand Management	All elements	Not required for non-residential site plans expected to have fewer than 60 employees and/or students on location at any given time	Not Exempt
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Not Exempt
4.8 Network Concept	All elements	Only required when proposed development generates more than 200 person-trips during the peak hour in excess of the equivalent volume permitted by the established zoning	Exempt

Based on the foregoing, the following modules will be included in the TIA report:

Design Review Component

- Module 4.1: Development Design
- Module 4.2: Parking
- Module 4.3: Boundary Streets
- Module 4.4: Access Design

Network Impact Component

- Module 4.5: Transportation Demand Management
- Module 4.6: Neighbourhood Traffic Management
- Module 4.7: Transit
- Module 4.9: Intersection Design

3.0 FORECASTING

3.1 Development-Generated Travel Demand

3.1.1 Trip Generation

Trips generated by the proposed townhouse/apartment dwellings and proposed retirement dwellings have been estimated separately, as described below.

Proposed Townhouse/Apartment Dwellings

The number of person trips generated by the proposed townhouse and apartment dwellings have been estimated using the *TRANS Trip Generation Manual Summary Report*, which was prepared in October 2020 by WSP. The *TRANS Trip Generation Manual* presents peak period trip generation rates and mode shares for different types of housing for the AM and PM peak periods, including the Low-Rise (one to two storeys) and High-Rise (three or more storeys) Multifamily Housing land uses. The process of converting the trip generation estimates from peak period to peak hour is discussed below. Relevant excerpts of the *TRANS Trip Generation Manual* are included in **Appendix F**.

The TRANS Trip Generation Manual identifies the subject site as being located within the Alta Vista district, which has the following observed mode shares for low-rise and high-rise multifamily housing during the peak hours:

ak.
ak.
ak.

One set of mode shares have been assumed for both peak hours and for both townhouse and apartment residents. These mode shares are generally based on the above mode shares (i.e. 40% driver, 15% passenger, 30% transit, 5% cyclist, 10% pedestrian).

For the Multifamily Housing land uses, the process of converting the trip generation estimates from peak period to peak hour is shown in the following tables. The estimated number of person trips generated by the proposed townhouse dwellings for the AM and PM peak periods are shown in **Table 5**. A breakdown of these trips by modal share is shown in **Table 6**.

Table 5: Proposed Townhouses and Apartments – Peak Period Trip Generation

Land Use	TRANS Rate	Units	AM Peak Period (ppp ⁽¹⁾)		PM Peak Period (ppp)		l (ppp)	
Lanu USE	TRANS Rate	Units	IN	OUT	TOT	IN	OUT	ТОТ
Low-Rise Multifamily Housing	AM: 1.35 PM: 1.58	160	65	151	216	141	111	252
High-Rise Multifamily Housing	AM: 0.80 PM: 0.90	100	25	55	80	52	38	90
		Total	90	206	296	193	149	342

1. ppp: Person Trips per Peak Period

Table 6: Proposed Townhouses and Apartments – Peak Period Trips by Mode Share

Travel Mode	Mode Share	Al	I l Peak Peri	od	PM Peak Period			
Traver Mode	Wode Share	IN	OUT	TOT	IN	OUT	TOT	
Peak Period Person Trips		90	206	296	193	149	342	
Auto Driver	40%	36	82	118	77	59	136	
Auto Passenger	15%	13	31	44	29	23	52	
Transit	30%	27	62	89	58	44	102	
Cyclist	5%	5	10	15	10	8	18	
Pedestrian	10%	9	21	30	19	15	34	

Table 4 of the *TRANS Trip Generation Manual* includes adjustment factors to convert the estimated number of trips generated for each mode from peak period to peak hour. A breakdown of the peak hour trips by mode is shown in **Table 7**.

Table 7: Proposed Townhouses and Apartments – Peak Hour Trips by Mode Share

Travel Mode	Adj. Factor		Α	M Peak Ho	ur	PM Peak Hour			
Traver mede	AM	PM	IN	OUT	TOT	IN	OUT	TOT	
Auto Driver	0.48	0.44	17	40	57	34	26	60	
Auto Passenger	0.48	0.44	6	15	21	13	10	23	
Transit	0.55	0.47	15	34	49	27	21	48	
Cyclist	0.58	0.48	3	6	9	5	4	9	
Pedestrian	0.58	0.52	5	12	17	10	8	18	
Peak Hour Person Trips			46	107	153	89	69	158	

Proposed Retirement Dwellings

The number of person trips generated by the proposed retirement dwellings have been estimated using the trip generation rates outlined in the *ITE Trip Generation Manual*, 11th Edition, corresponding to the Congregate Care Facility land use (code 253). Trips estimated using the *ITE Trip Generation Manual* have been converted to person trips using an adjustment factor of 1.28, consistent with the City's 2017 TIA Guidelines.

Mode shares for the proposed retirement dwellings have been estimated using data outlined in the 2011 TRANS O-D Survey Report, based on all trips from/within the Alta Vista district during the AM peak hour and all trips to/within Alta Vista during the PM peak hour.

The estimated number of person trips generated by the proposed retirement dwellings for the AM and PM peak hours are shown in **Table 8**. A breakdown of these trips by modal share is shown in **Table 9**.

Table 8: Proposed Retirement – Peak Hour Trip Generation

Land Use	ITE Code	Units	AM Pe	ak Hour (pph ⁽¹⁾)	PM Peak Hour (pph)		
Land USe			IN	OUT	ТОТ	IN	OUT	TOT
Congregate Care	253	150	10	7	17	17	18	35

1. pph: Person Trips per Hour

Travel Mode	Mode Share	Α	M Peak Ho	ur	PM Peak Hour			
Travel Mode	Wode Share	IN	OUT	TOT	IN	OUT	TOT	
Peak Hour Person Trips		10	7	17	17	18	35	
Auto Driver	55%	6	4	10	9	10	19	
Auto Passenger	15%	1	1	2	3	3	6	
Transit	20%	2	1	3	3	4	7	
Cyclist	0%	ı	-	•	-	-	-	
Pedestrian	10%	1	1	2	2	1	3	

The peak hour trip generation estimates by mode share for the entire proposed development (shown in **Table 7** and **Table 9**) have been added together, and are shown in **Table 10**.

Table 10: Entire Proposed Development – Total Peak Hour Trips

Travel Mode	Į.	M Peak Hou	ır	PM Peak Hour			
Travel Wiode	IN	OUT	TOT	IN	OUT	TOT	
Peak Hour Person Trips	56	114	170	106	87	193	
Auto Driver	23	44	67	43	36	79	
Auto Passenger	7	16	23	16	13	29	
Transit	17	35	52	30	25	55	
Cyclist	3	6	9	5	4	9	
Pedestrian	6	13	19	12	9	21	

As shown in the previous table, the proposed development is estimated to generate 170 person trips (including 67 vehicle trips) during the AM peak hour, and 193 person trips (including 79 vehicle trips) during the PM peak hour.

3.1.2 Trip Distribution

The assumed distribution of trips generated by the proposed development have been derived from existing traffic patterns associated with the typical commute (i.e. outbound AM trips and inbound PM trips), and can be summarized as follows:

- 15% to/from the south via Conroy Road;
- 10% to/from the east via St. Laurent Boulevard;
- 35% to/from the east via Walkley Road;
- 40% to/from the west via Walkley Road.

3.1.3 Trip Assignment

For the purposes of this TIA, all trips to/from the retirement home and apartment building have been assigned to a future RIRO access on Walkley Road or to Walkley Road/160m West of Conroy Road. Trips arriving from the west and departing to the south or east have been assigned equally between both access locations, while all other trips have been assigned to the intersection at Walkley Road/160m West of Conroy Road.

Based on the layout of the proposed development, vehicle trips generated by the proposed townhouses have been assigned to the proposed accesses as follows:

Eastern Access to St. Laurent Boulevard

- 60% of trips to/from the south via Conroy Road;
- 60% of trips to/from the east via St. Laurent Boulevard;
- 45% of trips to/from the east via Walkley Road;
- 5% of trips to/from the west via Walkley Road.

Western Access to St. Laurent Boulevard

- 30% of trips to/from the south via Conroy Road;
- 30% of trips to/from the east via St. Laurent Boulevard;
- 15% of trips to/from the east via Walkley Road;
- 5% of trips to/from the west via Walkley Road.

Access to Don Reid Drive

- 10% of trips to/from the south via Conroy Road;
- 10% of trips to/from the east via St. Laurent Boulevard:
- 40% of trips to/from the east via Walkley Road;
- 90% of trips to/from the west via Walkley Road.

3.2 Background Traffic

3.2.1 General Background Growth Rate

A review of snapshots of the City's *Strategic Long-Range Model* and *Intersection Traffic Growth Rates (2000-2016)* has been conducted. Both resources are included in **Appendix H**. Comparing snapshots of the 2011 and 2031 AM peak hour traffic volumes, the *Strategic Long-Range Model* generally suggests positive growth between 0% and 1% per annum on Walkley Road and negative growth between 0% and -1% per annum on Conroy Road. The *Intersection Traffic Growth Rates* figures, which determine growth rates based on total vehicular volumes entering the intersection, identify growth rates for the following study area intersections.

- Walkley Road/Conroy Road
 - o AM Peak Hour: negative growth (between -0.2% and -2.0% per annum);
 - PM Peak Hour: negative growth (between -0.2% and -2.0% per annum).
- St. Laurent Boulevard/Conroy Road
 - AM Peak Hour: positive growth (between +0.2% and +2.0% per annum):
 - o PM Peak Hour: no growth (between -0.2% and +0.2% per annum).

In the interest of maintaining a conservative analysis, an annual background growth rate assumption of 1% has been applied to the arterial roadways Walkley Road and Conroy Road in this TIA.

3.2.2 Other Area Developments

As first discussed in Section 2.2.2, traffic generated by the following developments in proximity of the subject site have been considered in the 2024 and 2029 background volumes. Relevant excerpts of the traffic studies in support of these developments are included in **Appendix G**.

Timbercreek Heron Gate

The proposed redevelopment is located at 2848, 2851, 2881, and 2898 Baycrest Drive, and 2820 and 2831 Cedarwood Drive. The redevelopment will ultimately consist of seven blocks, with 118 low-rise dwellings, 2,047 mid-rise dwellings, and 2,874 high-rise dwellings. Per the 2021 TIA, the anticipated buildout year for this development is 2040, and therefore analysis was conducted for an interim year 2030 and ultimate buildout year 2040. For the purposes of this TIA, traffic generated by the interim year 2030 has been added to the 2029 background traffic volumes.

2020 Walkley Road and 2935 Conroy Road

The proposed redevelopment will consist of three single-storey warehouses with a total gross floor area (GFA) of approximately 262,715 ft². Per the 2021 TIA, the anticipated buildout year of the development is 2023, and analysis was conducted for the buildout year 2023 and horizon year 2028. For the purposes of this TIA, traffic generated by this development has been added to the 2024 and 2029 background traffic volumes.

2500 St. Laurent Boulevard

The now-constructed development consists of two two-storey office buildings with a total GFA of approximately 68,134 ft². A Transportation Brief was prepared in October 2017 by Stantec. Since traffic count data was collected at all signalized study area intersections prior to completion of the development, site-generated traffic has been added to the 2024 and 2029 background traffic volumes within the study area.

2190 Halifax Drive

The proposed development will consist of 202 additional high-rise dwellings. Per the 2019 TIA, the anticipated buildout year was 2021, and analysis was conducted for the buildout year 2021 and horizon year 2026. For the purposes of this TIA, traffic generated by this development has been added to the 2024 and 2029 background traffic volumes.

3.3 Future Traffic Conditions

The figures below present the following future traffic conditions:

- Proposed site-generated traffic volumes are shown in Figure 6;
- Other area development-generated traffic volumes in 2024 are shown in Figure 7;
- Other area development-generated traffic volumes in 2029 are shown in Figure 8;
- Background traffic volumes in 2024 are shown in **Figure 9**:
- Background traffic volumes in 2029 are shown in Figure 10;
- Total traffic volumes in 2024 are shown in Figure 11;
- Total traffic volumes in 2029 are shown in Figure 12.

Figure 6: Site-Generated Traffic Volumes

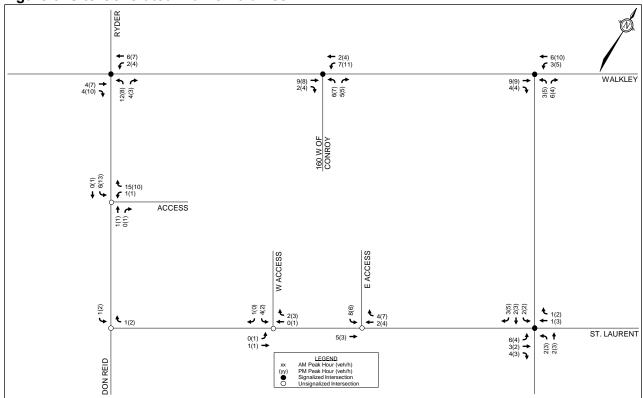
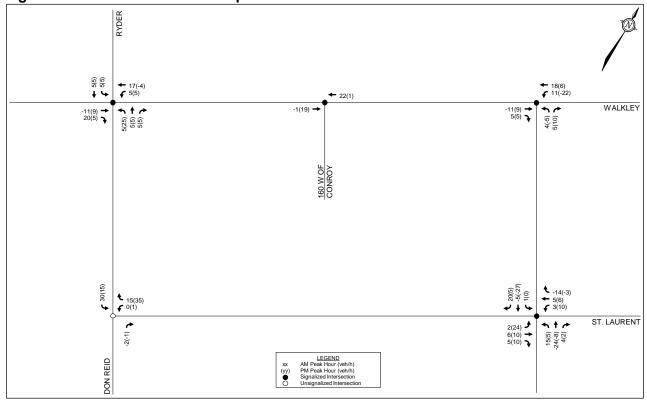


Figure 7: 2024 Other Area Development-Generated Traffic Volumes



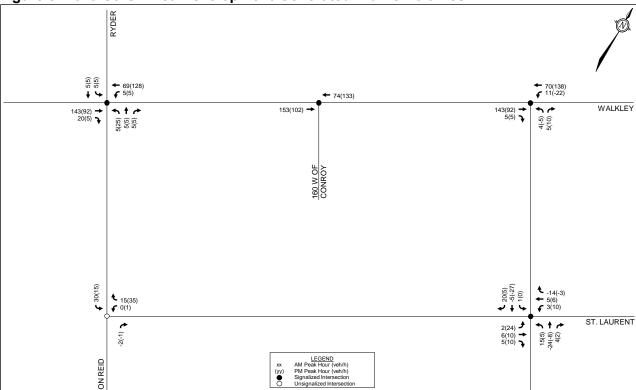


Figure 8: 2029 Other Area Development-Generated Traffic Volumes



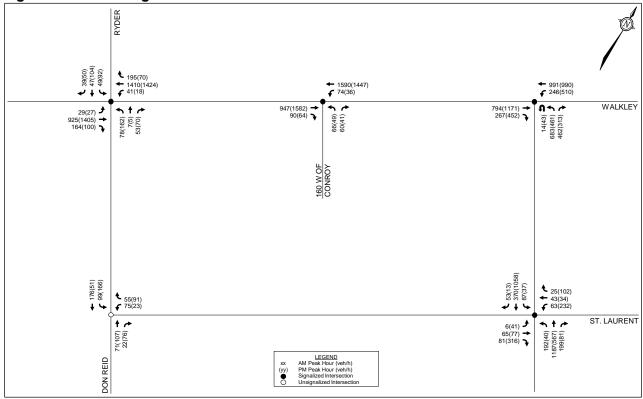


Figure 10: 2029 Background Traffic Volumes

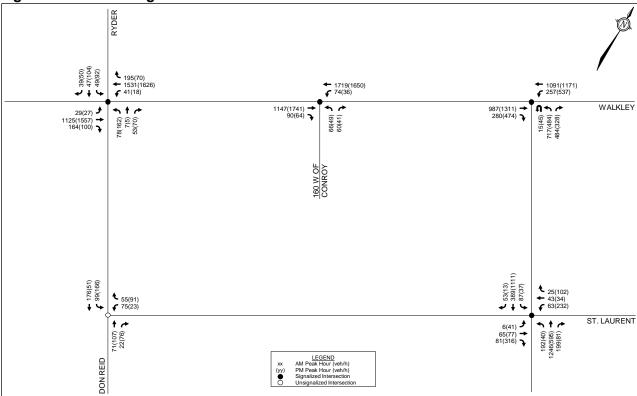
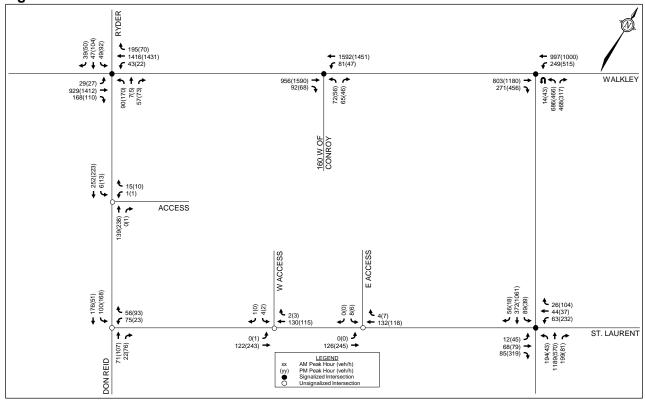


Figure 11: 2024 Total Traffic Volumes



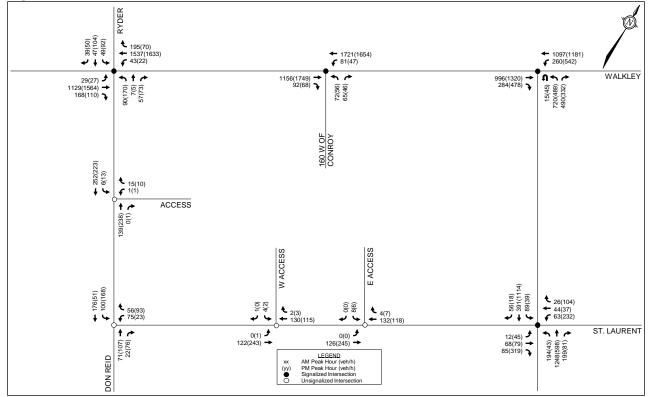


Figure 12: 2029 Total Traffic Volumes

3.4 Demand Rationalization

A review of the existing and background intersection operations has been conducted using Synchro 11, to determine if and when traffic volumes exceed capacity within the study area. The intersection parameters used in the analysis are consistent with the *2017 TIA Guidelines* (Saturated Flow Rate: 1,800 vphpl, Peak Hour Factor: 0.9 in existing conditions and 1.0 in future conditions).

Per Exhibit 22 of the *Multi-Modal Level of Service (MMLOS) Guidelines*, the target vehicular level of service (Auto LOS) at all study area intersections is an Auto LOS D, which equates to a vehicle-to-capacity (v/c) ratio of 0.90 at signalized intersections, and a maximum delay of 35 seconds at unsignalized intersections. Signal timing plans were obtained from the City, and are included in **Appendix I**.

3.4.1 Existing Intersection Operations

Intersection capacity analysis has been conducted for the existing traffic conditions. The results of the analysis are summarized in **Table 11** and **Table 12** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix J**.

Table 11: Existing Traffic Operations

January Landson Grant Cope		Critic	al Move	ements	lr	ntersectio	n
Intersection	Period	Max v/c or Delay	Los	Mvmt	v/c	Delay	LOS
Walkley Road/	AM	0.71	С	WBT/R	0.67	9 sec	В
Don Reid Drive/Ryder Street(1)	PM	0.81	D	NBL	0.70	16 sec	В
Walkley Road/	AM	0.67	В	WBT	0.66	8 sec	В
160m West of Conroy Road(1)	PM	0.64	В	EBT	0.63	6 sec	В
Wellder Deed	AM	0.88	D	NBL	0.73	29 sec	С
Walkley Road/ Conroy Road ⁽¹⁾	PM	0.94	Е	EBT	0.91	24 000	Е
Corney Road	FIVI	0.91	Е	WBL	0.91	31 sec	_
St. Laurent Boulevard/	AM	0.57	Α	SBL	0.46	13 sec	Α
Conroy Road ⁽¹⁾	PM	0.79	С	EBR	0.73	25 sec	С
St. Laurent Boulevard/	AM	12 sec	В	WBL		•	
Don Reid Drive ⁽²⁾	PM	11 sec	В	WBL		-	

^{1.} Signalized intersection

Table 12: Existing Queues

				AM Peak			PM Peak	
Intersection	Mvmt	Storage/ Spacing ⁽¹⁾	v/c [LOS]	50 th %	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Walkley Rd/Don	NBL	35m	0.46 [A]	14	22	0.81 [D]	29	46
Reid Dr/Ryder St	WBT/R	140m	0.71 [C]	29	#193	0.71 [C]	58	51
Walkley Rd/160m West of Conroy Rd	WBT	130m	0.67 [B]	60	140	0.58 [A]	72	130
Walkley Rd/	EBT	130m	0.62 [B]	77	102	0.94 [E]	89	#169
Conroy Rd	WBL	200m	0.66 [B]	28	39	0.91 [E]	58	#87

^{1.} Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

#: volume for the 95th percentile cycle exceeds capacity

From the previous tables, the eastbound through and westbound left turn movements at Walkley Road/Conroy Road operate at an Auto LOS E during the PM peak hour. An alternate scenario where the cycle length is increased from 110 seconds to 120 seconds (with all additional green time allocated to the eastbound-westbound phases) has been reviewed, and indicate that this mitigation allows both movements to operate at the target Auto LOS D. Detailed reports of this alternate scenario are included in **Appendix J**.

During the AM peak hour, the Synchro analysis identifies that the maximum (95th-percentile) queue lengths of the westbound through movements at Walkley Road/Don Reid Drive/Ryder Street and Walkley Road/160m West of Conroy Road extend into the upstream intersections on Walkley Road.

During the PM peak hour, the Synchro analysis identified that the maximum queue length of the northbound left turn movement at Walkley Road/Don Reid Drive/Ryder Street exceeds the storage length of the auxiliary northbound left turn, but is contained within the taper. The maximum queue length of the eastbound through movement at Walkley Road/Conroy Road extends into the upstream intersection on Walkley Road.

^{2.} Unsignalized intersection

3.4.2 **2024 Background Intersection Operations**

Intersection capacity analysis has been conducted for the 2024 background traffic conditions. The results of the analysis are summarized in Table 13 and Table 14 for the weekday AM and PM peak hours. Detailed reports are included in **Appendix K**.

Table 13: 2024 Background Traffic Operations

		Critic	al Move	ements	Intersection			
Intersection	Period	Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS	
Walkley Road/	AM	0.66	В	WBT/R	0.63	8 sec	В	
Don Reid Drive/Ryder Street(1)	PM	0.81	D	NBL	0.65	16 sec	В	
Walkley Road/	AM	0.63	В	WBT	0.61	7 sec	В	
160m West of Conroy Road(1)	PM	0.60	Α	EBT	0.58	5 sec	Α	
Walkley Road/	AM	0.84	D	NBL	0.68	27 sec	В	
Conroy Road ⁽¹⁾	PM	0.84	D	WBL	0.82	27 sec	D	
St. Laurent Boulevard/	AM	0.43	Α	SBL	0.41	12 sec	Α	
Conroy Road ⁽¹⁾	PM	0.76	O	EBR	0.66	22 sec	В	
St. Laurent Boulevard/	AM	12 sec	В	WBL		•		
Don Reid Drive ⁽²⁾	PM	11 sec	В	WBL		-		

^{1.} Signalized intersection

Table 14: 2024 Background Queues

		Storage/		AM Peak		PM Peak			
Intersection	Intersection Mvmt Spacin		v/c	50 th %	95 th %	v/c	50 th %	95 th %	
		•	[LOS]	Queue (m)	Queue (m)	[LOS]	Queue (m)	Queue (m)	
Walkley Rd/Don	NBL	35m	0.44 [A]	13	21	0.81 [D]	31	49	
Reid Dr/Ryder St	WBT/R	140m	0.66 [B]	27	30	0.65 [B]	55	51	
Walkley Rd/160m	WBT	120m	0 62 ID1	F2	120	0.50 [4]	60	111	
West of Conroy Rd	WDI	130m	0.63 [B]	52	120	0.53 [A]	60	111	
Walkley Rd/	EBT	130m	0.55 [A]	66	90	0.83 [D]	67	#134	
Conroy Rd	WBL	200m	0.65 [B]	27	37	0.84 [D]	49	#71	

^{1.} Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

From the previous tables, all intersections operate at the target Auto LOS D or better. Compared to the existing conditions, improvements in some movements is due to differences in the Peak Hour Factor parameter (0.9 in existing conditions and 1.0 in future conditions, per the 2017 TIA Guidelines).

3.4.3 **2029 Background Intersection Operations**

Intersection capacity analysis has been conducted for the 2029 background traffic conditions. The results of the analysis are summarized in Table 15 and Table 16 for the weekday AM and PM peak hours. Detailed reports are included in **Appendix K**.

^{2.} Unsignalized intersection

^{#:} volume for the 95th percentile cycle exceeds capacity

Table 15: 2029 Background Traffic Operations

		Critic	al Move	ements	Intersection			
Intersection	Period	Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS	
Walkley Road/	AM	0.70	В	WBT/R	0.66	9 sec	В	
Don Reid Drive/Ryder Street(1)	PM	0.81	D	NBL	0.72	16 sec	С	
Walkley Road/	AM	0.68	В	WBT	0.66	7 sec	В	
160m West of Conroy Road(1)	PM	0.66	В	EBT	0.64	6 sec	В	
Walkley Road/	AM	0.87	D	NBL	0.75	29 sec	С	
Conroy Road ⁽¹⁾	PM	0.95	Е	EBT	0.90	30 sec	D	
St. Laurent Boulevard/	AM	0.46	Α	SBL	0.43	12 sec	Α	
Conroy Road ⁽¹⁾	PM	0.76	C	EBR	0.68	23 sec	В	
St. Laurent Boulevard/	AM	12 sec	В	WBL				
Don Reid Drive ⁽²⁾	PM	11 sec	В	WBL		-		

^{1.} Signalized intersection

Table 16: 2029 Background Queues

	Storage/			AM Peak		PM Peak			
Intersection	Mvmt	Spacing ⁽¹⁾	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	
Walkley Rd/Don	NBL	35m	0.44 [A]	13	21	0.81 [D]	31	48	
Reid Dr/Ryder St	WBT/R	140m	0.70 [B]	28	#191	0.74 [C]	57	50	
Walkley Rd/160m West of Conroy Rd	WBT	130m	0.68 [B]	61	142	0.61 [B]	77	141	
Walkley Rd/	EBT	130m	0.69 [B]	91	120	0.95 [E]	~104	#179	
Conroy Rd	WBL	200m	0.67 [B]	28	39	0.87 [D]	53	#77	

^{1.} Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

From the previous tables, the eastbound through movement at Walkley Road/Conroy Road operates at an Auto LOS E during the PM peak hour. The alternate timing discussed in Section 3.4.1 (increasing the green time for the eastbound-westbound phases) has been reviewed, and the analysis indicates that this mitigation allows the eastbound through movement to operate at the target Auto LOS D. Detailed reports of this alternate scenario are included in **Appendix K**.

4.0 ANALYSIS

4.1 Development Design

Further review of the proposed retirement and apartment block will be conducted in a future Site Plan Control application. The Development Design, Parking, Access Intersections, and Transportation Demand Management modules below have only been conducted for the proposed townhouses. The preliminary site plan is included in **Appendix A**.

^{2.} Unsignalized intersection

^{#:} volume for the 95th percentile cycle exceeds capacity

^{~:} approach is above capacity

4.1.1 Design for Sustainable Modes

In general, the proposed development includes a pavement width of 6.5m to 6.7m for on-site roadways with perpendicular parking spaces or no on-street parking. Parallel parking spaces are provided on the south side of Street 1 (adjacent to the public park), the east side of Street 1 (adjacent to 2500 St. Laurent Boulevard), and on the north side of Street 3 (adjacent to the commercial access serving 1950 Walkley Road and 2980 Conroy Road). These parallel parking spaces are provided in the form of lay-bys, to maintain a narrower pavement width outside of these spaces and reduce the operating speed of vehicles on-site.

On-site concrete sidewalks with a width of 1.8m will be provided in the following locations:

- Along the south side of Street 1 between Don Reid Drive and Street 3 (providing a direct connection to the public park),
- Along the east side of Street 1 (providing a direct and generally straight connection through the site from St. Laurent Boulevard to the additional owned lands to the north),
- Along the south side of Street 3 (providing a direct and straight connection from Street 1 to Conroy Road), and
- Along the east side of the additional lands to the north (providing a direct and straight connection from Street 1 to Walkley Road).

Midblock pathways with a width of 2.0m will also be provided between the proposed public park and Street 2, and between Street 2 and Street 1 at Street 3. These sidewalks and pathways will connect the development to Conroy Road, St. Laurent Boulevard, Don Reid Drive, and the proposed parkland fronting Don Reid Drive. It is anticipated that the sidewalk will be extended along the eastern frontage of the future development and connect to Walkley Road as part of a future Site Plan Control application.

The nearest bus stops are discussed in Section 2.1.5. OC Transpo's service design guidelines for peak period service is to provide service within a five-minute (400m) walk of home, work, or school, for 95% of urban residents. Residents or visitors of any proposed dwelling will be within 400m of at least one bus stop shown in **Figure 4**.

A review of the City's *Transportation Demand Management (TDM)-Supportive Development Design and Infrastructure Checklist* has been conducted. Any required TDM-supportive design and infrastructure measures in the TDM checklist that are relevant to townhouse developments have been met. A copy of this checklist is included in **Appendix L**. In addition to the required measures, the proposed development also meets the following 'basic' or 'better' measures as defined in the *TDM-Supportive Development Design and Infrastructure Checklist*:

- Locate buildings close to the street, and do not locate parking areas between the street and building entrances;
- Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations;
- Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort;
- Provide safe, direct, and attractive walking routes from building entrances to nearby transit stops:
- Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h:

 Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists.

4.1.2 Circulation and Access

Garbage collection will take place curbside in front of the proposed dwellings. The on-site fire route will include all private roadways within the subject site.

4.2 Parking

The subject site is located in Area C of Schedule 1 and Schedule 1A of the City's ZBL. Minimum vehicle parking rates for the proposed townhouses are identified in Sections 101 and 102 of the ZBL, and are summarized in **Table 17**.

Table 17: Required and Proposed Parking

	and a mile i i o o o o a i animig			
Land Use	Rate	Units	Required	Provided
Minimum Vehicl	e Parking (Townhouse Dwellings)			
Dwelling,	1.0 per dwelling unit (residents)	160 units	160	320
Townhouse	0.2 per dwelling unit (visitors)	100 units	32	320
	Additional page	arking on p	rivate streets	45
		Total	192	365

Based on the previous table, the minimum parking requirements will be met. As every proposed dwelling will include their own garage, the ZBL does not identify any minimum bicycle parking requirements. A review of the parking requirements for the future development block will be conducted as part of a future Site Plan Control application.

4.3 Boundary Streets

This section provides a review of the boundary streets Walkley Road, Conroy Road, St. Laurent Boulevard, and Don Reid Drive, using complete streets principles. The *Multi-Modal Level of Service* (*MMLOS*) Guidelines, produced by IBI Group in October 2015, were used to evaluate the levels of service for each alternative mode of transportation on the boundary streets, based on existing conditions. Since each boundary street is located within both the General Urban Area and Urban Employment Area (per Schedule B of the City's previous Official Plan, which is referenced by the *MMLOS Guidelines*), whichever target is more stringent has been considered.

A detailed segment MMLOS review of the boundary streets is included in **Appendix M**. A summary of the segment MMLOS analysis is provided below in **Table 18**.

Table 18: Segment MMLOS Summary

Segment	PLOS		BLOS		TL	os	TkLOS	
Segment	Actual	Target	Actual	Target	Actual	Target	Actual	Target
Walkley Road	E	С	F	В	D	В	Α	В
Conroy Road	E	С	Е	В	D	D	Α	В
St. Laurent Boulevard	F	С	F	В	-	-	В	D
Don Reid Drive	F	С	F	В	-	-	В	D

The results of the segment MMLOS analysis can be summarized as follows:

- No boundary street meets the target pedestrian level of service (PLOS);
- No boundary street meets the target bicycle level of service (BLOS);
- Conroy Road meets the target transit level of service (TLOS), while Walkley Road does not;
- All boundary streets meet the target truck level of service (TkLOS).

Pedestrian Level of Service

Both sides of Walkley Road and Conroy Road do not meet the target PLOS C. Per Exhibit 4 of the *MMLOS Guidelines*, Walkley Road can achieve the target PLOS C and Conroy Road can achieve a PLOS D by implementing sidewalks with a minimum width of 2.0m and a minimum boulevard width of 2.0m. This is identified for the City's consideration.

The south side of St. Laurent Boulevard and west side of Don Reid Drive do not meet the target PLOS C, as sidewalks are only provided on one side of each roadway. Per Exhibit 4 of the *MMLOS Guidelines*, implementing curbside sidewalks with a minimum width of 1.8m are sufficient to achieve the target PLOS. This is identified for the City's consideration. The existing sidewalks on St. Laurent Boulevard and Don Reid Drive meet the target PLOS C, and therefore no recommendations for these sidewalks are identified. Any sidewalks that need to be reconstructed as a result of the proposed development will be reinstated to a width of 1.8m.

Bicycle Level of Service

Walkley Road does not meet the target BLOS B. Per Exhibit 9 of the *MMLOS Guidelines*, the target BLOS B can only be achieved through the implementation of physically separated bikeways along Walkley Road. This is identified for the City's consideration.

A mixed-use pathway is provided on the west side of Conroy Road, which achieves the bestpossible BLOS A. For the purposes of this review, the existing curbside bike lanes on both sides of Conroy Road have also been evaluated, and these bike lanes achieve a BLOS E.

St. Laurent Boulevard and Don Reid Drive do not meet the target BLOS B. Per Exhibit 9 of the *MMLOS Guidelines*, the target BLOS B can be achieved by providing curbside bike lanes with a minimum width of 1.5m, and reducing the operating speed to 50 km/h (i.e. a posted speed limit of 40 km/h).

Transit Level of Service

Walkley Road does not meet the target TLOS B, which is achieved by providing bus lanes with no or limited parking/driveway friction. It is anticipated that this target will be met upon completion of the Baseline/Heron/Walkley/St. Laurent BRT project, which is anticipated to occur beyond 2031.

4.4 Access Intersections

4.4.1 Access Design

The proposed development includes two full-movement accesses to St. Laurent Boulevard and one full-movement access to Don Reid Drive. Depressed curbs and continuous sidewalks are proposed along the entirety of each access, in accordance with City standards. The design of each access has been evaluated using the relevant provisions of the City's *Private Approach By-Law* (PABL).

Section 25(a) of the PABL identifies that, for sites with 46m or more of frontage to a given roadway, two two-way private approaches to that roadway are permitted. Therefore, the two-way private approaches to St. Laurent Boulevard and Don Reid Drive meet this requirement.

Section 25(c) of the PABL identifies a maximum width requirement of 9.0m for any two-way private approach, as measured at the street line. Since each private approach is approximately 7.2m to 7.7m in width at the street line, this requirement is met.

Section 25(g) of the PABL identifies a minimum separation requirement of 9.0m between a two-way private approach and any other private approach to the same property, measuring nearest edge to nearest edge at the street line. Since the proposed accesses to St. Laurent Boulevard are approximately 55m apart, this requirement is met.

Section 25(m)(ii) of the PABL identifies that, for a property that abuts or is within 46m of an arterial roadway, there are minimum distance requirements between a private approach and the nearest intersecting street line, and between any two private approaches to the same property. The minimum distance is determined by the land use and number of parking spaces provided. For the purposes of this review, the proposed residences will be treated as apartment dwellings, as the section does not directly reference requirements for townhouse developments. Per Section 25(m)(ii) of the PABL, the minimum separation between accesses to the same property is 30m, when 100 to 199 parking spaces are accessed. Although the proposed development will include more than 200 parking spaces, this range of parking spaces was selected as the parking spaces are distributed throughout the entire subject site. The 30m requirement is met, as a distance of approximately 55m is proposed between the nearest edges of the accesses to St. Laurent Boulevard.

Section 25(p) of the PABL identifies a minimum separation requirement of 3.0m between the edge of any private approach and the nearest property line, as measured at the street line. This requirement is met by all proposed accesses.

Section 25(u) of the PABL identifies a requirement that any private approach serving a parking area with more than 50 parking spaces shall not have a grade exceeding 2% for the first 9m inside the property line. This requirement is met by all proposed accesses.

The Transportation Association of Canada (TAC)'s *Geometric Design Guide for Canadian Roads* identifies minimum intersection sight distance (ISD) and stopping sight distance (SSD) requirements, based on the roadway grade and design speed (taken as the speed limit plus 10 km/h). Assuming level grade and a design speed of 60 km/h for St. Laurent Boulevard and Don Reid Drive, the ISD requirements are 130m for left-turning vehicles and 110m for right-turning vehicles, and the SSD requirement is 85m.

The proposed accesses to Don Reid Drive will have clear sightlines to Walkley Road to the north and St. Laurent Boulevard to the south. The proposed accesses to St. Laurent Boulevard are located on the inside of a slight curve, but will still achieve the TAC-recommended sightlines, provided that any vegetation within the ROW of St. Laurent Boulevard is trimmed and maintained. Therefore, no sightline concerns are anticipated. Intersection sight triangles for outbound drivers at each access are shown in **Figure 13** through **Figure 15**.

Engineers, Planners & Landscape Architects

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6

Telephone Facsimile Website (613) 254-9643 (613) 254-5867 www.novatech-eng.com 2510 ST. LAURENT BLVD.

SIGHT DISTANCE DON REID DR & STREET 1

1: 1250° 10 20 30 40 50 SEP 2023 122040 FIGURE 13

Engineers, Planners & Landscape Architects

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6

Telephone Facsimile Website (613) 254-9643 (613) 254-5867 www.novatech-eng.com 2510 ST. LAURENT BLVD.

SIGHT DISTANCE ST. LAURENT BLVD & STREET 2

1: 1250 10 20 30 40 50 SEP 2023 122040 FIGURE 14

SHT8X11.DWG - 216mmx279mm

Engineers, Planners & Landscape Architects

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6

Telephone Facsimile Website (613) 254-9643 (613) 254-5867 www.novatech-eng.com 2510 ST. LAURENT BLVD.

SIGHT DISTANCE ST. LAURENT BLVD & STREET 1

1: 1250 10 20 30 40 50 SEP 2023 122040 FIGURE 15

4.4.2 Access Operations

Analysis of the access intersection operations has been conducted in Synchro, with the results summarized in **Table 19**. The intersection parameters used in the analysis are consistent with the 2017 TIA Guidelines (Saturated Flow Rate: 1,800 vphpl, Peak Hour Factor: 1.0 in future conditions).

Table 19: 2024/2029 Access Intersection Operations

A	A	M Peak Ho	ur	PM Peak Hour			
Access	Delay	LOS	Mvmt	Delay	LOS	Mvmt	
St. Laurent Boulevard – East Access	10 sec	Α	SBL/R	11 sec	В	SBL/R	
St. Laurent Boulevard – West Access	10 sec	Α	SBL/R	11 sec	В	SBL/R	
Don Reid Drive – Access	9 sec	Α	WBL/R	10 sec	Α	WBL/R	

Based on the foregoing, the proposed accesses to St. Laurent Boulevard and Don Reid Drive are anticipated to operate with an acceptable vehicular level of service for the buildout year 2024 and horizon year 2029.

4.5 Transportation Demand Management

4.5.1 Context for TDM

The proposed development will consist of 160 townhouses. A detailed TDM review of the retirement/apartment block, which at this time is anticipated to include 150 retirement home units and 100 apartment units, will be conducted as part of a future Site Plan Control application.

4.5.2 Need and Opportunity

Per Schedule B, the subject site is located in the General Urban Area, and surrounded completely by General Urban Area or Major Open Space north of Walkley Road, and Urban Employment Area south of Walkley Road. As first discussed in Section 3.1.1, the mode share targets for the proposed development are assumed to be generally consistent with the observed multifamily housing mode shares for the Alta Vista region, as outlined in the *TRANS Trip Generation Manual*. These target shares include a 40% driver share.

Failure to meet the already observed driver shares for the Alta Vista region are not anticipated, due to the proximity of the subject site to places of employment to the south, east, and west, as well as commercial areas immediately north of the site and west of Heron Road. Regardless, failure to meet the proposed mode share targets are anticipated to marginally increase congestion within the study area.

4.5.3 TDM Program

A review of the City's *TDM Measures Checklist* has been conducted by the proponent, who has committed to providing the following TDM measures at the sales centre for this development. A copy of the checklist is included in **Appendix L.**

- Provide local area maps with walking/cycling access routes and key destinations;
- Provide relevant transit schedules and route maps;
- Provide a multimodal travel option information package.

4.6 Neighbourhood Traffic Management

The 2017 TIA Guidelines identify two-way peak hour traffic volume thresholds for considering when a Neighbourhood Traffic Management (NTM) plan should be developed, whenever a site relies on local or collector roadways for access. The NTM two-way volume thresholds are as follows:

- Local Roadways: 120 vehicles during the peak hour, or 1,000 vehicles per day;
- Collector Roadways: 300 vehicles during the peak hour, or 2,500 vehicles per day;
- Major Collector Roadways: 600 vehicles during the peak hour, or 5,000 vehicles per day.

The proposed development will rely on the collector roadways St. Laurent Boulevard and Don Reid Drive for direct access. As shown in Section 2.1.7 and **Figure 5**, the peak hour and daily NTM thresholds for both St. Laurent Boulevard and Don Reid Drive are exceeded by the existing traffic volumes.

Since St. Laurent Boulevard and Don Reid Drive primarily serve industrial, commercial, or office uses, no neighbourhood traffic management measures have been recommended as part of this proposed development.

4.7 Transit

Based on the trip generation estimates presented in Section 3.1.1, the proposed development is anticipated to generate the following number of transit trips:

AM Peak Hour: 52 transit trips, including 35 boarding and 17 alighting;
 PM Peak Hour: 55 transit trips, including 25 boarding and 30 alighting.

The distribution of transit trips to/from the development has been estimated using the same trip distribution assumptions outlined in Section 3.1.2, which are summarized as follows:

- 15% to/from the south via Conroy Road;
- 10% to/from the east via St. Laurent Boulevard;
- 35% to/from the east via Walkley Road;
- 40% to/from the west via Walkley Road.

Winter 2020 (January 5 to March 7) transit utilization data within the study area was obtained from OC Transpo, and is included in **Appendix C**. This period is considered the most recent 'normal' ridership period, before ridership was impacted by the ongoing COVID-19 pandemic. Average peak period (6:00am to 9:00am and 3:00pm to 6:00pm) boarding, alighting, and bus load at departure information was provided by City staff for stops #4307, #4311, #6927, #8391, and #8398.

Existing and projected boarding and alighting information is summarized in **Table 20**. Any zero (0) values in the table indicate a measured average boarding or alighting value of zero, rather than an absence of data. Peak period boarding and alighting data have been converted to peak hour boardings and alightings, using factors of 0.55 for the AM peak hour and 0.47 for the PM peak hour (per the *TRANS Trip Generation Manual*).

Table 20: Existing and Projected Transit Utilization

Stop	Location	Ro	ute	Воа	arding (tp	h) ⁽¹⁾	Alig	ghting (tp	h) ⁽¹⁾
Stop	Location	(Dire	ction)	Existing	Site	Total	Existing	Site	Total
AM Pea	nk Hour								
#4307	St. Laurent/Conroy	40	SB	1	5	6	31	1	32
#4311	St. Laurent/Conroy	40	NB	2	4	6	7	3	10
#6927	Walkley/Don Reid	46	EB	3	12	15	9	7	16
#8391	Walkley/Ryder	46	WB	2	7	9	2	6	8
#8398	Ryder/Walkley	291	IB	5	7	12	1	-	1
PM Pea	k Hour								
#4307	St. Laurent/Conroy	40	SB	5	4	9	9	3	12
#4311	St. Laurent/Conroy	40	NB	23	3	26	1	5	6
#6927	Walkley/Don Reid	46	EB	4	4	8	4	3	7
		46	WB	3	5	8	1	5	6
#8391	Walkley/Ryder	140	EB	0	4	4	0	3	3
#6391	vvaikiey/Rydei	140	WB	0	5	5	0	5	5
		291	OB	0	-	0	1	6	7

A discussion of the site-generated impacts to each route during the weekday peak hours is included below.

Route 40 (to St. Laurent)

At stop #4311, the proposed development is estimated to generate a net addition of four AM boarding trips, three AM alighting trips, three PM boarding trips, and five PM alighting trips. As Route 40 runs on approximately 15-minute intervals during the peak hours, this equates to an addition of one AM boarding trip, AM alighting trip, PM boarding trip, and PM alighting trip per bus during the peak hours. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 40 (to Greenboro)

At stop #4307, the proposed development is estimated to generate a net addition of five AM boarding trips, one AM alighting trip, four PM boarding trips, and three PM alighting trips. As Route 40 runs on approximately 15-minute intervals during the peak hours, this equates to an addition of two AM boarding trips, one AM alighting trip, one PM boarding trip, and one PM alighting trip per bus during the peak hours. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 46 (to Hurdman)

At stop #6927, the proposed development is estimated to generate a net addition of 12 AM boarding trips, seven AM alighting trips, four PM boarding trips, and three PM alighting trips. As Route 46 runs on approximately 30-minute intervals during the peak hours, this equates to an addition of six AM boarding trips, four AM alighting trips, two PM boarding trips, and two PM alighting trips per bus during the peak hours. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 46 (to Billings Bridge)

At stop #8391, the proposed development is estimated to generate a net addition of seven AM boarding trips, six AM alighting trips, five PM boarding trips, and five PM alighting trips. As Route 46 runs on approximately 30-minute intervals during the peak hours, this equates to an addition of four AM boarding trips, three AM alighting trips, three PM boarding trips, and three PM alighting trips per bus during the peak hours. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 140 (to Heron Park)

At stop #8391, the proposed development is estimated to generate a net addition of four PM boarding trips and three PM alighting trips. As Route 140 runs on approximately 30-minute intervals during the peak hours, this equates to an addition of two PM boarding trips and two PM alighting trips per bus during the peak hours. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 140 (to Billings Bridge)

At stop #8391, the proposed development is estimated to generate a net addition of five PM boarding trips and five PM alighting trips. As Route 140 runs on approximately 30-minute intervals during the peak hours, this equates to an addition of three PM boarding trips and three PM alightings trip per bus during the peak hours. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 291 (to Hurdman)

At stop #8398, the proposed development is estimated to generate a net addition of seven AM boarding trips. As Route 291 runs on approximately 30-minute intervals during the peak hours, this equates to an addition of four boarding trips per bus during the AM peak hour. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

Route 291 (to Herongate)

At stop #8391, the proposed development is estimated to generate a net addition of six PM alighting trips. As Route 291 runs on approximately 30-minute intervals during the peak hours, this equates to an addition of three alighting trips per bus during the PM peak hour. Therefore, these additional transit trips are not anticipated to require more frequent service at these stops.

4.8 Intersection Design

4.8.1 Intersection MMLOS Review

This section provides a review of the signalized study area intersections (Walkley Road/Don Reid Drive/Ryder Street, Walkley Road/160m West of Conroy Road, Walkley Road/Conroy Road, and St. Laurent Boulevard/Conroy Road) using complete streets principles. The signalized intersections within the study area have been evaluated for PLOS, BLOS, TLOS, and TkLOS, based on existing conditions. The MMLOS targets considered in this review are associated with those outlined in Exhibit 22 of the *MMLOS Guidelines* for the General Urban Area or Employment Area, whichever targets are stricter.

The full intersection MMLOS analysis is included in **Appendix M**. A summary of the results is shown in **Table 21**.

Table 21: Intersection MMLOS Summary

Intersection	PL	PLOS		BLOS		TLOS		.os
intersection	Actual	Target	Actual	Target	Actual	Target	Actual	Target
Walkley Road/ Don Reid Drive/Ryder Street	F	С	F	В	Е	В	Е	В
Walkley Road/ 160m West of Conroy Road	F	С	F	В	В	В	Е	В
Walkley Road/ Conroy Road	F	С	F	В	F	В	Α	В
St. Laurent Boulevard/ Conroy Road	F	С	F	В	D	D	Е	В

The results of the intersection MMLOS analysis can be summarized as follows:

- No signalized intersections meet the target PLOS;
- No signalized intersections meet the target BLOS;
- Walkley Road/160m West of Conroy Road and St. Laurent Boulevard/Conroy Road meets the target TLOS, while Walkley Road/Don Reid Drive/Ryder Street and Walkley Road/ Conroy Road do not;
- Walkley Road/Conroy Road meets the target TkLOS, while Walkley Road/Don Reid Drive/ Ryder Street, Walkley Road/160m West of Conroy Road, and St. Laurent Boulevard/Conroy Road do not.

Walkley Road/Don Reid Drive/Ryder Street

The intersection does not meet the target PLOS C, BLOS B, TLOS B, or TkLOS B.

All approaches do not meet the target PLOS C, and have cross-sections equivalent to five to nine lanes crossed. Per the *MMLOS Guidelines*, every 3.5m in crossing distance is equivalent to one lane crossed. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks (greater than 400,000 vehicle/pedestrian conflicts over an eight-hour period).

All approaches do not meet the target BLOS B based on left turn characteristics. Per Exhibit 12 of the *MMLOS Guidelines*, the target BLOS can only be achieved by implementing two-stage, left-turn bike boxes. Implementing bike boxes would also require restricting right turns on red (RTOR) for each approach. This is identified for the City's consideration.

The north and west approaches do not meet the target TLOS B. No recommendations are identified for the north approach, which is Ryder Street (i.e. a local roadway with no transit priority designation). It is anticipated that the target TLOS B will be met at the west approach, upon completion of the Baseline/Heron/Walkley/St. Laurent BRT project.

The east and west approaches do not meet the target TkLOS B. The TkLOS at these approaches represent the level of accommodation for trucks turning right from Walkley Road onto Don Reid Drive or Ryder Street (i.e. roadways that are not designated as truck routes with limited heavy vehicle volumes), and therefore no recommendations are identified.

Walkley Road/160m West of Conroy Road

The intersection does not meet the target PLOS C, BLOS B, or TkLOS B.

All approaches do not meet the target PLOS C, and have cross-sections equivalent to six or seven lanes crossed. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks.

The east approach does not meet the target BLOS B based on left turn characteristics, and the south approach does not meet the target BLOS B based on right turn characteristics. The south approach is a private approach to 1950 Walkley Road, and therefore no recommendations are identified. Per Exhibit 12 of the *MMLOS Guidelines*, the target BLOS can only be achieved at the east approach by implementing a jug-handle, crossride, and bicycle traffic signal for cyclists to enter the private approach to 1950 Walkley Road. This is identified for the City's consideration.

The west approach does not meet the target TkLOS B. The TkLOS at this approach represents the level of accommodation for trucks turning right into 1950 Walkley Road (i.e. an existing private approach to commercial/retail uses), and therefore no recommendations are identified.

Walkley Road/Conroy Road

The intersection does not meet the target PLOS C, BLOS B, or TLOS B.

The south and east approaches do not meet the target PLOS C, and have cross-sections equivalent to nine or ten lanes crossed. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or removing the westbound and northbound right turn channels. Both approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. This is identified for the City's consideration. While this would improve the level of comfort for pedestrians, the provision of zebra-striped crosswalks alone will not improve the PLOS for either approach.

The south and east approaches do not meet the target BLOS B based on left turn characteristics, and the south and west approaches do not meet the target BLOS based on right turn characteristics. From a left-turn perspective and per Exhibit 12 of the *MMLOS Guidelines*, the target BLOS can only be achieved by implementing left-turn bike facilities. This would include a bike box for cyclists arriving from the south approach, and a jug-handle, crossride, and bicycle traffic signal for cyclists arriving from the east approach. This is identified for the City's consideration.

From a right-turn perspective, Exhibit 12 of the *MMLOS Guidelines* identifies that the target BLOS B can be achieved with pocket bike lanes, as long as the right turn lane is less than 50m in length, and is introduced to the right of the pocket bike lane. Based on the existing queue lengths of the northbound and eastbound right turn movements, this is not recommended. Therefore, the provision of separated cycling facilities (like the existing multi-use pathway on the west side of Conroy Road) is identified for the City's consideration.

All approaches do not meet the target TLOS. It is anticipated that the target TLOS B will be met at the east and west approaches, upon completion of the Baseline/Heron/Walkley/St. Laurent BRT project. As Conroy Road is designated as a Transit Priority Corridor with Isolated Measures (with a target TLOS D), it is anticipated that the implementation of measures such as queue jump lanes or transit priority signals would improve the TLOS of the south approach to a TLOS D.

St. Laurent Boulevard/Conroy Road

The intersection does not meet the target PLOS C, BLOS B, or TkLOS B.

All approaches do not meet the target PLOS C, and have cross-sections equivalent to six lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or removing the westbound right turn channel. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks.

All approaches do not meet the target BLOS B based on left turn characteristics, and the east and west approaches do not meet the target BLOS based on right turn characteristics. From a left-turn perspective and per Exhibit 12 of the *MMLOS Guidelines*, the target BLOS can only be achieved by implementing two-stage, left-turn bike boxes. Implementing bike boxes would also require RTOR restrictions for the north and south approaches. This is identified for the City's consideration.

From a right-turn perspective, Exhibit 12 of the *MMLOS Guidelines* identifies that the target BLOS B can be achieved by implementing a curbside bike lane for the east approach and a pocket bike lane for the west approach. This is identified for the City's consideration.

The north and south approaches do not meet the target TkLOS B. The TkLOS at these approaches represent the level of accommodation for trucks turning right from Conroy Road onto St. Laurent Boulevard (i.e. a roadway that is not designated as a truck route with limited heavy vehicle volumes), and therefore no recommendations are identified.

4.8.2 2024 Total Intersection Operations

Intersection capacity analysis has been conducted for the 2024 total traffic conditions. The analysis included below is based on an earlier concept plan with a higher number of townhouses, and is therefore conservative. The results of the analysis are summarized in **Table 22** and **Table 23** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix N**.

Table 22: 2024 Total Traffic Operations

		Critic	al Mov	ements	lr	ntersectio	n
Intersection	Period	Max v/c or Delay	LOS	Mvmt	v/c	Delay	Los
Walkley Road/	AM	0.71	С	WBT/R	0.63	9 sec	В
Don Reid Drive/Ryder Street(1)	PM	0.83	D	NBL	0.68	17 sec	В
Walkley Road/	AM	0.62	В	WBT	0.53	7 sec	Α
160m West of Conroy Road ⁽¹⁾	PM	0.59	Α	EBT	0.57	5 sec	А
Walkley Road/	AM	0.84	D	NBL	0.68	27 sec	В
Conroy Road ⁽¹⁾	PM	0.85	D	WBL	0.83	27 sec	D
St. Laurent Boulevard/	AM	0.43	Α	NBT/R	0.41	12 sec	Α
Conroy Road ⁽¹⁾	PM	0.77	С	EBR	0.67	23 sec	В
St. Laurent Boulevard/	AM	12 sec	В	WBL			
Don Reid Drive ⁽²⁾	PM	11 sec	В	WBL		-	

^{1.} Signalized intersection

^{2.} Unsignalized intersection

Table 23: 2024 Total Queues

		Storage/		AM Peak		PM Peak		
Intersection	Mvmt	Spacing ⁽¹⁾	v/c	50 th %	95 th %	v/c	50 th %	95 th %
			[LOS]	Queue (m)	Queue (m)	[LOS]	Queue (m)	Queue (m)
Walkley Rd/Don	NBL	35m	0.52 [A]	17	26	0.83 [D]	34	53
Reid Dr/Ryder St	WBT/R	140m	0.66 [B]	27	30	0.66 [B]	122	50
Walkley Rd/160m West of Conroy Rd	WBT	130m	0.63 [B]	50	121	0.53 [A]	59	112
Walkley Rd/	EBT	130m	0.55 [A]	67	91	0.84 [D]	72	#149
Conroy Rd	WBL	200m	0.66 [B]	27	38	0.85 [D]	50	#73

^{1.} Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

Compared to the 2024 background traffic conditions, site-generated traffic is anticipated to have marginal impacts on traffic operations within the study area.

4.8.3 2029 Total Intersection Operations

Intersection capacity analysis has been conducted for the 2029 total traffic conditions. The results of the analysis are summarized in **Table 24** and **Table 25** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix N**.

Table 24: 2029 Total Traffic Operations

Table 24. 2023 Total Traffic Operations							
		Critic	al Mov	ements	Intersection		
Intersection	Period	Max v/c or Delay	Los	Mvmt	v/c	Delay	LOS
Walkley Road/	AM	0.71	С	WBT/R	0.66	9 sec	В
Don Reid Drive/Ryder Street(1)	PM	0.83	D	NBL	0.73	18 sec	С
Walkley Road/	AM	0.67	В	WBT	0.58	8 sec	Α
160m West of Conroy Road(1)	PM	0.65	В	EBT	0.63	6 sec	В
Walkley Road/	AM	0.87	D	NBL	0.75	29 sec	С
Conroy Road ⁽¹⁾	PM	0.96	Е	EBT	0.91	31 sec	Е
St. Laurent Boulevard/	AM	0.46	Α	SBL	0.43	12 sec	Α
Conroy Road ⁽¹⁾	PM	0.77	C	EBR	0.69	23 sec	В
St. Laurent Boulevard/	AM	12 sec	В	WBL		·	
Don Reid Drive ⁽²⁾	PM	11 sec	В	WBL		-	

^{1.} Signalized intersection

Table 25: 2029 Total Queues

		Storage/		AM Peak		PM Peak		
Intersection	Mvmt	Spacing ⁽¹⁾	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Walkley Rd/Don	NBL	35m	0.51 [A]	16	26	0.83 [D]	34	53
Reid Dr/Ryder St	WBT/R	140m	0.71 [C]	28	#191	0.75 [C]	145	51
Walkley Rd/160m West of Conroy Rd	WBT	130m	0.67 [B]	59	143	0.61 [B]	75	143
Walkley Rd/	EBT	130m	0.70 [B]	92	121	0.96 [E]	~112	#181
Conroy Rd	WBL	200m	0.67 [B]	28	40	0.88 [D]	54	#79

^{1.} Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

^{#:} volume for the 95th percentile cycle exceeds capacity

^{2.} Unsignalized intersection

^{#:} volume for the 95th percentile cycle exceeds capacity

^{~:} approach is above capacity

Compared to the 2029 background traffic conditions, site-generated traffic is anticipated to have marginal impacts on traffic operations within the study area. The alternate timing discussed in Sections 3.4.1 and 3.4.3 (increasing the green time for the eastbound-westbound phases) has been reviewed, and the analysis indicates that this mitigation allows the eastbound through movement to operate at the target Auto LOS D. Detailed reports of this alternate scenario are included in **Appendix N**.

5.0 CONCLUSIONS AND RECOMMENDATIONS

The conclusions and recommendations of this TIA can be summarized as follows:

Forecasting

• The proposed development is estimated to generate 170 person trips (including 67 vehicle trips) during the AM peak hour, and 193 person trips (including 79 vehicle trips) during the PM peak hour.

Development Design and Parking

- In general, the proposed development includes a pavement width of 6.5m to 6.7m for onsite roadways with perpendicular parking spaces or no on-street parking. Parallel parking spaces are provided on the south side of Street 1 (adjacent to the public park), the east side of Street 1 (adjacent to 2500 St. Laurent Boulevard), and on the north side of Street 3 (adjacent to the commercial access serving 1950 Walkley Road and 2980 Conroy Road). These parallel parking spaces are provided as lay-bys, to maintain a narrower pavement width outside of these spaces and reduce the operating speed of vehicles on-site.
- On-site concrete sidewalks will be provided along the south side of Street 1 between Don Reid Drive and Street 3, the east side of Street 1, the south side of Street 3, and the east side of the additional lands to the north. Midblock pathways will also be provided between the proposed public park and Street 2, and between Street 2 and Street 1 at Street 3. These sidewalks will connect the proposed development to the proposed parkland fronting Don Reid Drive, and to the existing sidewalks along Conroy Road, St. Laurent Boulevard, and Don Reid Drive.
- Any required TDM-supportive design and infrastructure measures in the TDM checklist that are relevant to townhouse developments have been met.
- Garbage collection will take place curbside in front of the proposed dwellings. The on-site fire route will include all private roadways within the subject site.
- The minimum parking requirements will be met. As every proposed dwelling will include their own garage, the ZBL does not identify any minimum bicycle parking requirements.

Boundary Streets

- The results of the segment multi-modal level of service (MMLOS) analysis can be summarized as follows:
 - No boundary street meets the target pedestrian level of service (PLOS);
 - No boundary street meets the target bicycle level of service (BLOS);
 - Conroy Road meets the target transit level of service (TLOS), while Walkley Road does not:
 - All boundary streets meet the target truck level of service (TkLOS).

- Both sides of Walkley Road and Conroy Road do not meet the target PLOS C. Walkley Road can achieve the target PLOS C and Conroy Road can achieve a PLOS D by implementing sidewalks with a minimum width of 2.0m and a minimum boulevard width of 2.0m. This is identified for the City's consideration.
- The south side of St. Laurent Boulevard and west side of Don Reid Drive do not meet the target PLOS C, as sidewalks are only provided on one side of each roadway. Implementing curbside sidewalks with a minimum width of 1.8m are sufficient to achieve the target PLOS. This is identified for the City's consideration. The existing sidewalks on St. Laurent Boulevard and Don Reid Drive meet the target PLOS C, and therefore no recommendations for these sidewalks are identified. Any sidewalks that need to be reconstructed as a result of the proposed development will be reinstated to a width of 1.8m.
- Walkley Road does not meet the target BLOS B. The target BLOS B can only be achieved through the implementation of physically separated bikeways along Walkley Road. This is identified for the City's consideration.
- St. Laurent Boulevard and Don Reid Drive do not meet the target BLOS B. The target BLOS B can be achieved by providing curbside bike lanes with a minimum width of 1.5m, and reducing the operating speed to 50 km/h.
- Walkley Road does not meet the target TLOS B, which is achieved by providing bus lanes with no or limited parking/driveway friction. It is anticipated that this target will be met upon completion of the Baseline/Heron/Walkley/St. Laurent BRT project, which is anticipated to occur beyond 2031.

Access Intersections

- The proposed development includes two full-movement accesses to St. Laurent Boulevard and one full-movement access to Don Reid Drive. Depressed curbs and continuous sidewalks are proposed along the entirety of each access, in accordance with City standards. The design of each access meets the relevant provisions of the City's Private Approach By-Law.
- The proposed access to Don Reid Drive will have clear sightlines to Walkley Road to the
 north and St. Laurent Boulevard to the south. The proposed accesses to St. Laurent
 Boulevard are located on the inside of a slight curve, but will still achieve the sightlines
 recommended by the Transportation Association of Canada (TAC), provided that any
 vegetation within the ROW of St. Laurent Boulevard is trimmed and maintained. Therefore,
 no sightline concerns are anticipated.
- The proposed accesses to St. Laurent Boulevard and Don Reid Drive are anticipated to operate with an acceptable vehicular level of service for the buildout year 2024 and horizon year 2029.

Transportation Demand Management

- A review of the City's *TDM Measures Checklist* has been conducted by the proponent, who has committed to providing the following TDM measures at the sales centre:
 - o Provide local area maps with walking/cycling access routes and key destinations;
 - Provide relevant transit schedules and route maps;
 - o Provide a multimodal travel option information package.

Neighbourhood Traffic Management

• The peak hour and daily NTM thresholds for both St. Laurent Boulevard and Don Reid Drive are exceeded by the existing traffic volumes. Since St. Laurent Boulevard and Don Reid Drive primarily serve industrial, commercial, or office uses, no neighbourhood traffic management measures have been recommended as part of this proposed development.

Transit

 The proposed development is anticipated to generate 52 AM peak hour transit trips, (including 35 boarding and 17 alighting), and 55 PM peak hour transit trips (including 25 boarding and 30 alighting). These additional transit trips are not anticipated to require more frequent service at any stops within the study area.

Intersection MMLOS

- The results of the intersection MMLOS analysis can be summarized as follows:
 - No signalized intersections meet the target PLOS;
 - No signalized intersections meet the target BLOS;
 - Walkley Road/160m West of Conroy Road and St. Laurent Boulevard/Conroy Road meets the target TLOS, while Walkley Road/Don Reid Drive/Ryder Street and Walkley Road/Conroy Road do not;
 - Walkley Road/Conroy Road meets the target TkLOS, while Walkley Road/Don Reid Drive/Ryder Street, Walkley Road/160m West of Conroy Road, and St. Laurent Boulevard/Conroy Road do not.
- All approaches at the study area intersections do not meet the target PLOS C. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or removing right turn channels where applicable. The south and east approaches at Walkley Road/Conroy Road meet the City's vehicle/pedestrian conflict threshold to consider zebra-striped crosswalks.
- For approaches with failing BLOS based on left turn characteristics, the target BLOS can be achieved by implementing two-stage, left-turn cycling facilities. Implementing bike boxes would also require restricting right turns on red (RTOR). This is identified for the City's consideration.
- The south approach of Walkley Road/160m West of Conroy Road, the south and west approaches of Walkley Road/Conroy Road, and the east and west approaches of St. Laurent Boulevard/Conroy Road do not meet the target BLOS based on right turn characteristics. The provision of separated cycling facilities on Walkley Road and the east side of Conroy Road, and bike lanes on St. Laurent Boulevard is identified for the City's consideration.
- The north and west approaches of Walkley Road/Don Reid Drive/Ryder Street and all approaches of Walkley Road/Conroy Road do not meet the target TLOS B. It is anticipated that the target TLOS will be met on Walkley Road upon completion of the Baseline/Heron/ Walkley/St. Laurent BRT project, and on Conroy Road with the implementation of isolated transit priority measures. No recommendations are identified for Ryder Street (i.e. a local roadway with no transit priority designation).

 Any approaches that do not meet the target TkLOS represent right turns into private approaches or onto local/collector roadways with no truck route designation, and therefore no recommendations are identified.

Existing Traffic Operations

- The eastbound through and westbound left turn movements at Walkley Road/Conroy Road operate at an Auto LOS E during the PM peak hour.
- During the AM peak hour, the Synchro analysis identifies that the maximum (95th-percentile) queue lengths of the westbound through movements at Walkley Road/Don Reid Drive/Ryder Street and Walkley Road/160m West of Conroy Road extend into the upstream intersections on Walkley Road.
- During the PM peak hour, the Synchro analysis identified that the maximum queue length
 of the northbound left turn movement at Walkley Road/Don Reid Drive/Ryder Street exceeds
 the storage length of the auxiliary northbound left turn, but is contained within the taper. The
 maximum queue length of the eastbound through movement at Walkley Road/Conroy Road
 extends into the upstream intersection on Walkley Road.

Background Traffic Operations

- Compared to the existing conditions, improvements in some movements is due to differences in the Peak Hour Factor parameter (0.9 in existing conditions and 1.0 in future conditions, per the 2017 TIA Guidelines).
- The eastbound through movement at Walkley Road/Conroy Road operates at an Auto LOS
 E during the PM peak hour. Increasing the green time for the eastbound-westbound phases
 has been reviewed, and the analysis indicates that this mitigation allows the eastbound
 through movement to operate at the target Auto LOS D.

Total Traffic Operations

• Compared to the future background traffic conditions, site-generated traffic is anticipated to have marginal impacts on traffic operations within the study area.

Based on the foregoing, the proposed development is recommended from a transportation perspective.

NOVATECH

Prepared by:



Joshua Audia, P.Eng. Project Engineer | Transportation

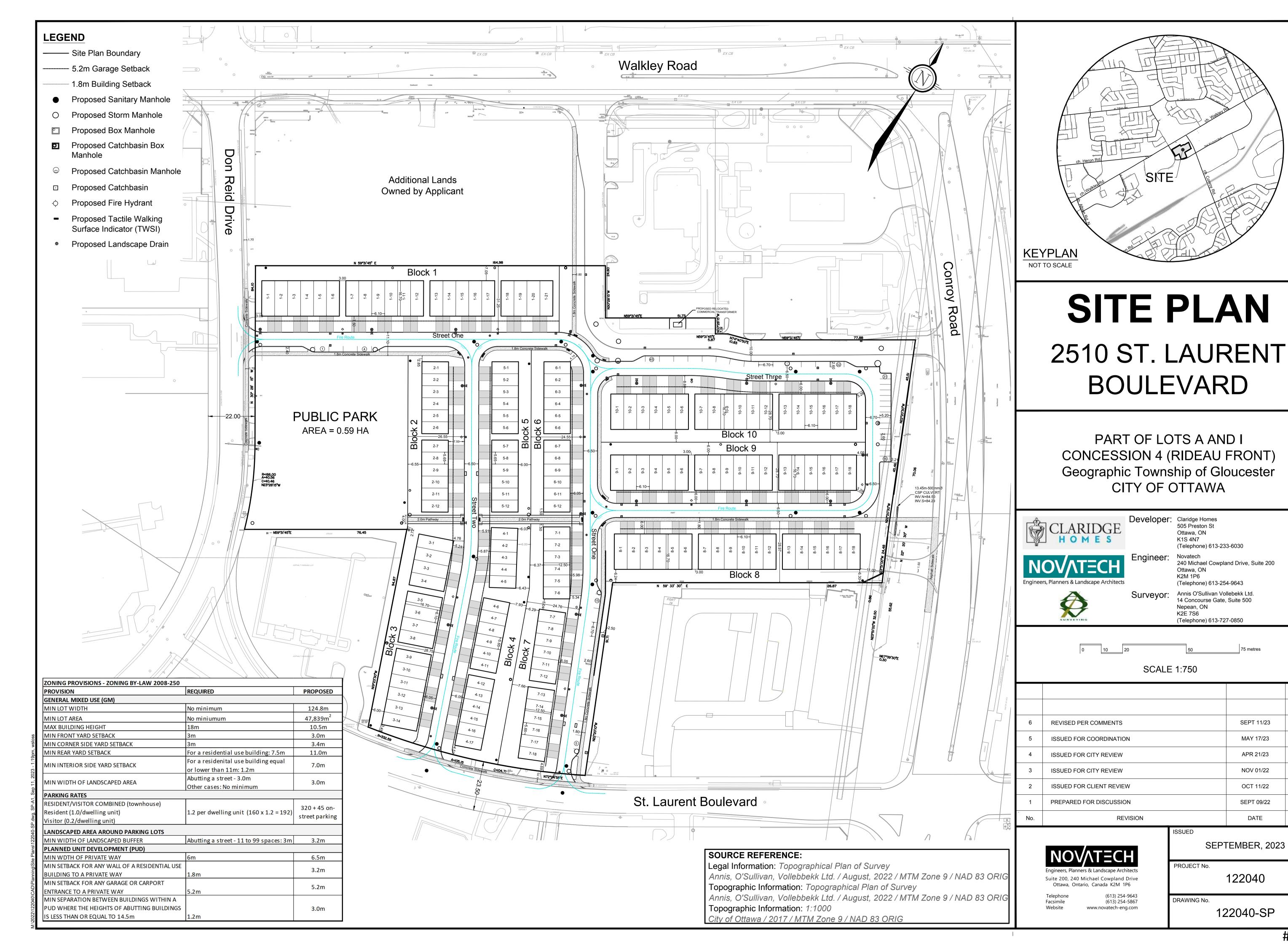
Reviewed by:



Brad Byvelds, P.Eng. Project Manager | Transportation

APPENDIX A

Proposed Site Plan



D07-12-22-0155

APPENDIX B

TIA Screening Form



City of Ottawa 2017 TIA Guidelines Screening Form

1. Description of Proposed Development

Municipal Address	1900-1902 Walkley Road, 2425 Don Reid Drive, 2510 St. Laurent Boulevard, and 3000 Conroy Road
Description of Location	Located north of St. Laurent Boulevard, south of Walkley Road, east of Don Reid Drive, and west of Conroy Road
Land Use Classification	Townhouses
Development Size (units)	160 townhouse dwellings, 150 retirement dwellings, and 100 apartment dwellings
Development Size (m²)	_
Number of Accesses and Locations	Two accesses to St. Laurent Boulevard and one access to Don Reid Drive
Phase of Development	1
Buildout Year	2024

If available, please attach a sketch of the development or site plan to this form.

2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size
Single-family homes	40 units
Townhomes or apartments	90 units
Office	3,500 m ²
Industrial	5,000 m²
Fast-food restaurant or coffee shop	100 m²
Destination retail	1,000 m²
Gas station or convenience market	75 m²

^{*} If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, the Trip Generation Trigger is satisfied.



3. Location Triggers

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?		✓
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*		✓

^{*}DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

4. Safety Triggers

	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		✓
Are there any horizontal/vertical curvatures on a boundary street limiting sight lines at a proposed driveway?		✓
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/suburban conditions)?	✓	
Is the proposed driveway within auxiliary lanes of an intersection?	\checkmark	
Does the proposed driveway make use of an existing median break that serves an existing site?		✓
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?		✓
Does the development include a drive-thru facility?		✓

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

5. Summary

	Yes	No
Does the development satisfy the Trip Generation Trigger?	✓	
Does the development satisfy the Location Trigger?		✓
Does the development satisfy the Safety Trigger?	✓	

If none of the triggers are satisfied, <u>the TIA Study is complete</u>. If one or more of the triggers is satisfied, <u>the TIA Study must continue into the next stage</u> (Screening and Scoping).

APPENDIX C

OC Transpo Route Information

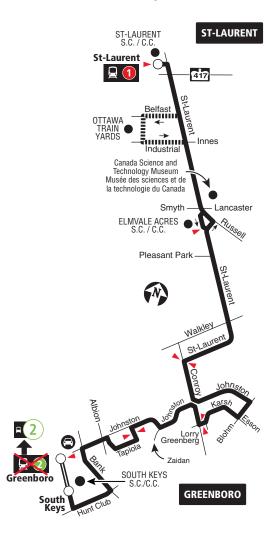




GREENBORO

7 days a week / 7 jours par semaine

All day service Service toute la journée





Transitway & Station

Some trips early morning only / Quelques trajets tôt le matin seulement



Park & Ride / Parc-o-bus

Timepoint / Heures de passage

2021.06





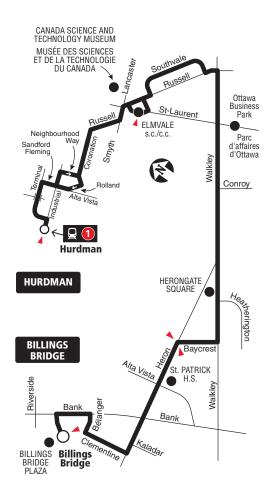
46

BILLINGS BRIDGE HURDMAN

Local

7 days a week / 7 jours par semaine

All day service Service toute la journée





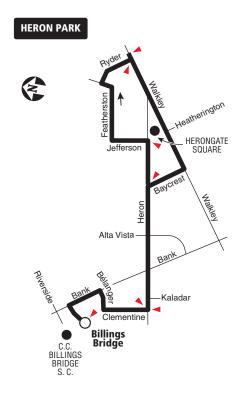




Monday to Saturday / Lundi au samedi

Local

Limited service during the day Service limité pendant la journée







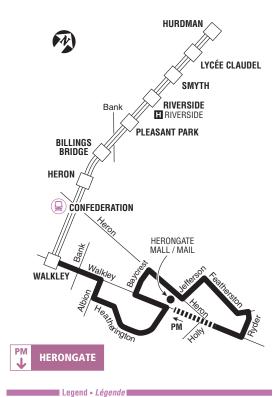




Monday to Friday / Lundi au vendredi

Peak periods only Périodes de pointe seulement





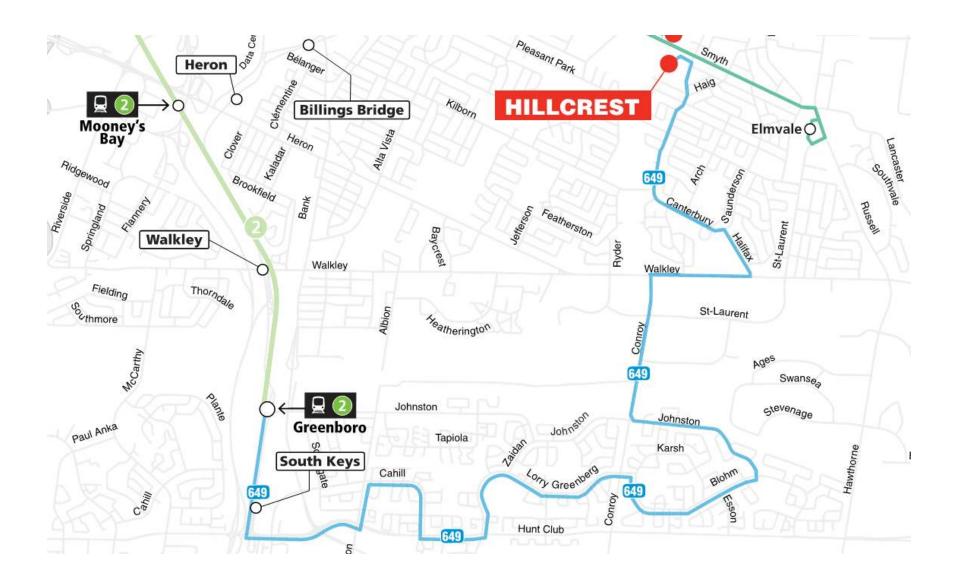
Line 2 – O-Train Trillium Line Ligne 2 - O-Train LigneTrillium

PM only / PM seulement

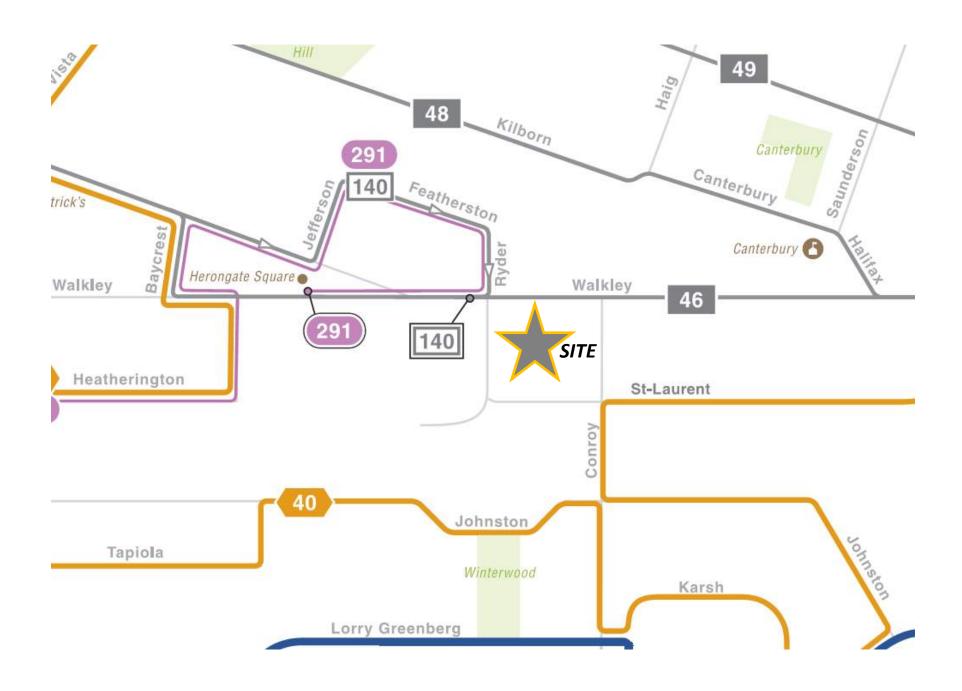
Transitway & Station











Joshua Audia

Subject:

FW: Transit Data Request - Walkley/Conroy area

Hi Josh,

The requested data is below. Note that I pared down the list of stops a bit: what's shown below should provide a suitable representative sample to cover the routes in the area. Please let me know if you need any other data.

Data was sampled from the period of January 5 to March 16 2020, which is the last normal ridership period before pandemic-related impacts began. Note that cells with a zero (0) value indicate a measured average value of zero, based on available APC data, rather than an absence of data. Cells with a dash (-) indicate that that route in question does not service the stop in the given time period.

Winter 2020 (Jan 5 - Mar 16)

		_		, A	AM (6:00 - 9:00)	PI	M (15:00 - 18:0	0)		24-hr	
Stop	Location	(Direc	ute ction)	Boardings	Alightings	Avg Load at Departure	Boardings	Alightings	Avg Load at Departure	Boardings	Alightings	Avg Load at Departure
4307	St-Laurent/ Conroy	40	SB	1	56	3	10	19	8	30	143	6
4311	St-Laurent/ Conroy	40	NB	3	12	7	48	2	6	121	27	7
6927	Walkley/ Don Reid	46	EB	5	16	13	7	9	15	15	40	12
		46	WB	4	3	16	6	2	17	20	10	12
0004	Walkley/	440	EB	-	-	-	0	0	0	0	7	0
8391	Ryder	140	WB	-	-	-	0	0	0	9	0	1
		291	ОВ	-	-	-	0	3	1	0	5	1
8398	Ryder/ Walkley	291	IB	8	1	1	-	-	-	8	1	1

Please let me know if there are any questions, or if any additional information is required.

Best,

Graham Rathwell

Transit Planner, Network Service Design Service Planning Branch Transit Services Department OC Transpo | City of Ottawa

APPENDIX D

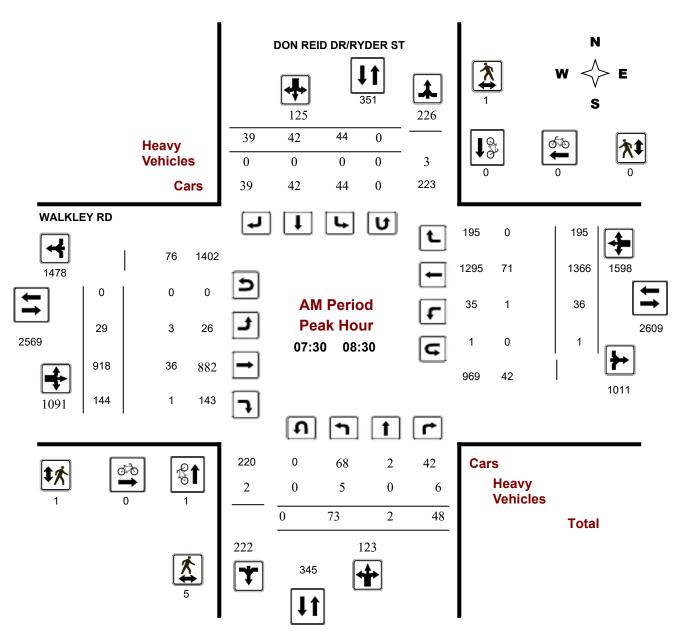
Traffic Count Data



Turning Movement Count - Peak Hour Diagram

WALKLEY RD @ DON REID DR/RYDER ST

Survey Date: Tuesday, November 29, 2016 WO No: 36554
Start Time: 07:00 Device: Miovision



Comments

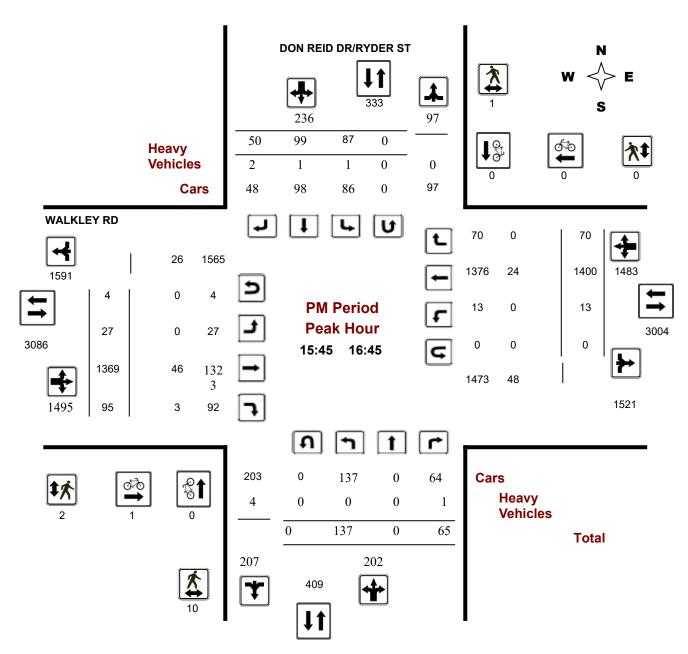
2022-May-13 Page 1 of 9



Turning Movement Count - Peak Hour Diagram

WALKLEY RD @ DON REID DR/RYDER ST





Comments

2022-May-13 Page 2 of 9



Turning Movement Count - Study Results

WALKLEY RD @ DON REID DR/RYDER ST

Survey Date: Tuesday, November 29, 2016 WO No: 36554

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, November 29, 2016 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 1

WALKLEY RD

1.00

Eastbound: 14 Westbound: 3

DON REID DR/RYDER ST

		DOI		יועום								* * * /		1 110					
	Nor	thbou	nd		So	uthbou	ınd			Е	astbou	ınd		V	Vestbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Tota
07:00 08:00	56	0	28	84	35	24	33	92	176	24	845	123	992	38	1159	155	1352	2344	2520
08:00 09:00	66	5	40	111	38	45	35	118	229	34	909	157	1100	30	1345	195	1570	2670	2899
09:00 10:00	51	1	35	87	41	22	33	96	183	19	710	78	807	23	855	60	938	1745	1928
11:30 12:30	78	1	80	159	33	21	30	84	243	30	792	69	891	25	873	56	954	1845	2088
12:30 13:30	58	0	59	117	37	22	37	96	213	18	864	91	973	28	886	48	962	1935	2148
15:00 16:00	87	0	55	142	85	74	47	206	348	27	1291	90	1408	18	1385	79	1482	2890	3238
16:00 17:00	142	1	72	215	76	101	51	228	443	33	1382	100	1515	10	1367	74	1451	2966	3409
17:00 18:00	95	5	34	134	62	62	45	169	303	26	1217	67	1310	6	1178	65	1249	2559	2862
Sub Total	633	13	403	1049	407	371	311	1089	2138	211	8010	775	8996	178	9048	732	9958	18954	21092
U Turns	0			0	0			0	0	0			0	0			0	0	0
Total	633	13	403	1049	407	371	311	1089	2138	211	8010	775	8996	178	9048	732	9958	18954	21092
EQ 12Hr	880	18	560	1458	567	516	432	1515	2973	313	11134	1077	12524	252	12577	1017	13846	26370	29343
Note: These	values ar	e calcul	lated by	y multiply	ying the	totals b	y the a	opropriat	e expans	ion fac	tor.			1.39					
AVG 12Hr	880	18	560	1458	567	516	432	1515	2973	313	11134	1077	12524	252	12577	1017	13846	26370	29343
Note: These	volumes	are calc	culated	by multi	plying th	ne Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			1.00					
AVG 24Hr	1153	24	734	1911	743	676	566	1985	3896	410	14586	1411	16407	330	16476	1332	18138	34545	38441
Note: These	volumes	are calc	culated	by multi	plying th	ne Avera	age Dai	y 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

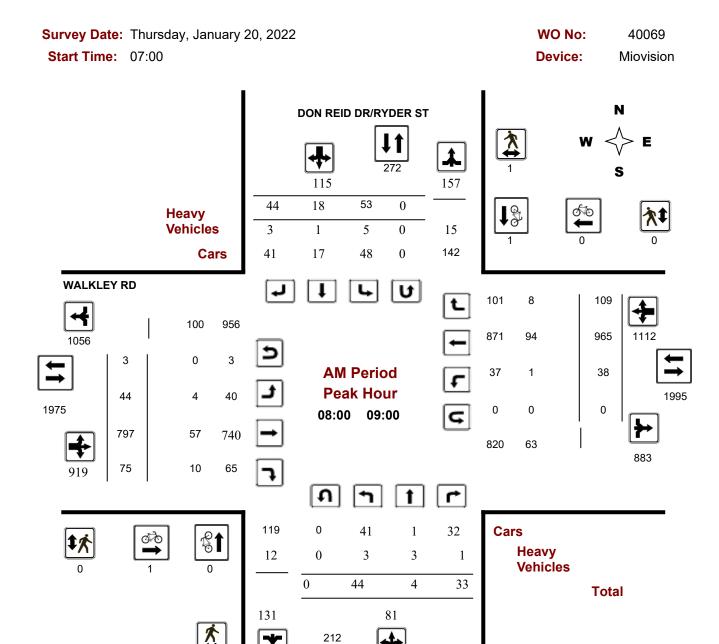
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Peak Hour Diagram

WALKLEY RD @ DON REID DR/RYDER ST



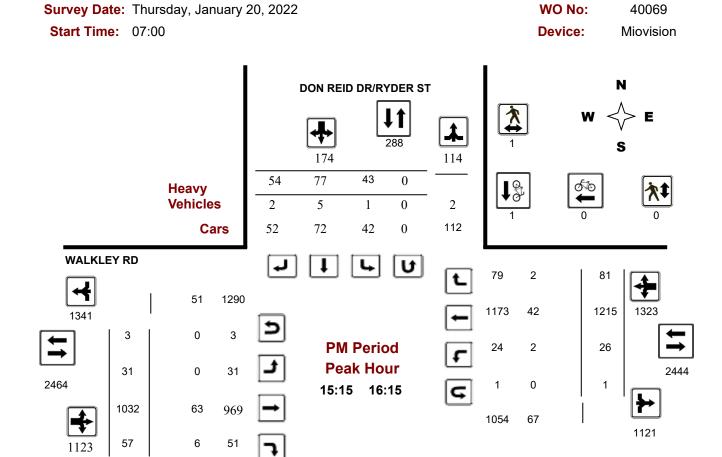
Comments

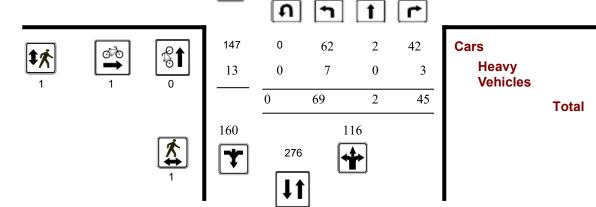
2022-May-13 Page 2 of 9



Turning Movement Count - Peak Hour Diagram

WALKLEY RD @ DON REID DR/RYDER ST





Comments

2022-May-13 Page 1 of 9



Turning Movement Count - Study Results

WALKLEY RD @ DON REID DR/RYDER ST

Survey Date: Thursday, January 20, 2022 WO No: 40069

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, January 20, 2022 Total Observed U-Turns AADT Factor

Northbound: 1 Southbound: 0

1.00

Eastbound: 15 Westbound: 6

		DON	REI	D DR/R	YDER	ST						WA	ALKLE	Y RD					
	Nor	thbou	nd		So	uthbou	ınd			Е	astbou	ınd		V	Vestbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	31	0	22	53	31	17	19	67	120	31	604	83	718	54	704	73	831	1549	1669
08:00 09:00	44	4	33	81	53	18	44	115	196	44	797	75	916	38	965	109	1112	2028	2224
09:00 10:00	43	2	38	83	46	22	35	103	186	27	703	54	784	36	731	57	824	1608	1794
11:30 12:30	66	0	51	117	41	13	32	86	203	20	823	70	913	17	884	54	955	1868	2071
12:30 13:30	37	1	37	75	46	14	31	91	166	20	799	61	880	27	819	55	901	1781	1947
15:00 16:00	62	1	45	108	46	67	50	163	271	27	1026	55	1108	27	1135	84	1246	2354	2625
16:00 17:00	92	3	56	151	54	52	51	157	308	26	1035	64	1125	15	1169	47	1231	2356	2664
17:00 18:00	60	2	59	121	45	22	34	101	222	30	893	44	967	16	964	62	1042	2009	2231
Sub Total	435	13	341	789	362	225	296	883	1672	225	6680	506	7411	230	7371	541	8142	15553	17225
U Turns	1			1	0			0	1	15			15	6			6	21	22
Total	436	13	341	790	362	225	296	883	1673	240	6680	506	7426	236	7371	541	8148	15574	17247
EQ 12Hr	606	18	474	1098	503	313	411	1227	2325	334	9285	703	10322	328	10246	752	11326	21648	23973
Note: These	values ar	e calcu	lated by	y multiply	ying the	totals b	y the ap	opropriat	e expans	ion fac	tor.			1.39					
AVG 12Hr	606	18	474	1098	503	313	411	1227	2325	334	9285	703	10322	328	10246	752	11326	21648	23973
Note: These	volumes	are calc	culated	by multi	plying th	ne Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			1.00					
AVG 24Hr	794	24	621	1439	659	410	538	1607	3046	438	12163	921	13522	430	13422	985	14837	28359	31405
Note: These	volumes	are calc	culated	by multip	plying th	ne Avera	ige Dail	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

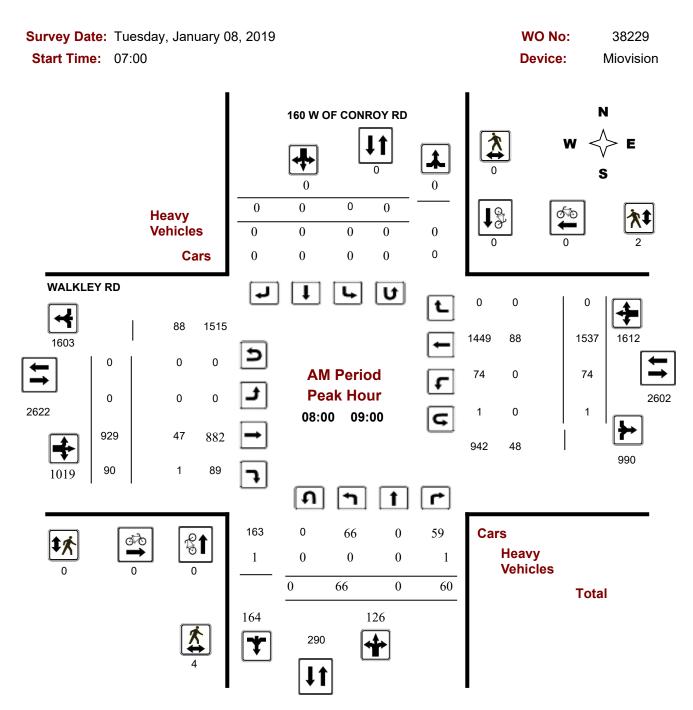
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

May 13, 2022 Page 3 of 8



Turning Movement Count - Peak Hour Diagram

WALKLEY RD @ 160 W OF CONROY RD



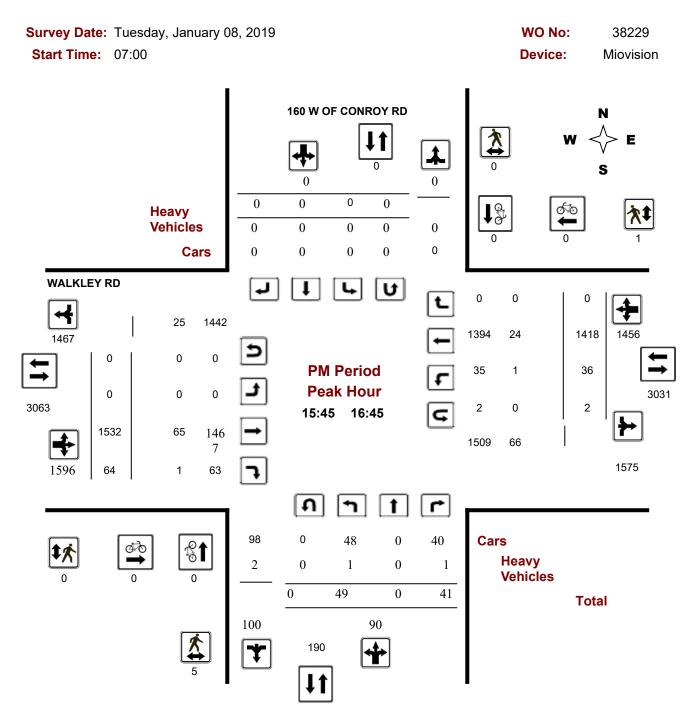
Comments

2022-May-13 Page 1 of 9



Turning Movement Count - Peak Hour Diagram

WALKLEY RD @ 160 W OF CONROY RD



Comments

2022-May-13 Page 2 of 9



Turning Movement Count - Study Results

WALKLEY RD @ 160 W OF CONROY RD

Survey Date: Tuesday, January 08, 2019 WO No: 38229

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, January 08, 2019 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 Eastbound: 0 Westbound: 12

1.10

160 W OF CONROY RD WALKLEY RD

	Nor	thbou	nd		Sou	ıthbou	nd			E	astbou	und		٧	Vestbou	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	59	0	53	112	0	0	0	0	112	0	771	82	853	73	1285	0	1358	2211	2323
08:00 09:00	66	0	60	126	0	0	0	0	126	0	929	90	1019	74	1537	0	1611	2630	2756
09:00 10:00	51	0	43	94	0	0	0	0	94	0	800	67	867	71	988	0	1059	1926	2020
11:30 12:30	66	0	50	116	0	0	0	0	116	0	871	114	985	98	1033	0	1131	2116	2232
12:30 13:30	52	0	53	105	0	0	0	0	105	0	908	98	1006	53	923	0	976	1982	2087
15:00 16:00	52	0	47	99	0	0	0	0	99	0	1376	71	1447	56	1351	0	1407	2854	2953
16:00 17:00	49	0	47	96	0	0	0	0	96	0	1536	65	1601	32	1371	0	1403	3004	3100
17:00 18:00	59	0	35	94	0	0	0	0	94	0	1219	73	1292	54	1245	0	1299	2591	2685
Sub Total	454	0	388	842	0	0	0	0	842	0	8410	660	9070	511	9733	0	10244	19314	20156
U Turns	0			0	0			0	0	0			0	12			12	12	12
Total	454	0	388	842	0	0	0	0	842	0	8410	660	9070	523	9733	0	10256	19326	20168
EQ 12Hr	631	0	539	1170	0	0	0	0	1170	0	11690	917	12607	727	13529	0	14256	26863	28033
Note: These v	/alues ar	e calcul	ated by	/ multiply	ing the	totals by	the ap	propriate	e expansi	on fac	tor.			1.39					
AVG 12Hr	694	0	593	1287	0	0	0	0	1287	0	12859	1009	13868	800	14882	0	15682	29550	30837
Note: These \	olumes :	are calc	ulated	by multip	lying th	e Equiv	alent 12	2 hr. tota	ls by the	AADT	factor.			1.10					
AVG 24Hr	909	0	777	1686	0	0	0	0	1686	0	16845	1322	18167	1048	19495	0	20543	38710	40396
Note: These	olumes :	are calc	ulated	by multip	lying th	e Avera	ge Dail	y 12 hr. 1	totals by	12 to 2	4 expan	sion fa	ctor.	1.31					

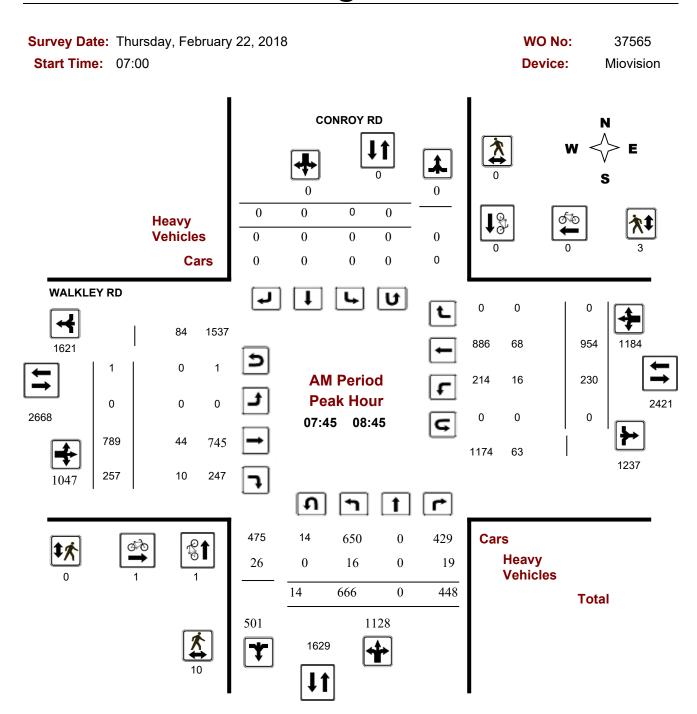
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

May 13, 2022 Page 3 of 8



Turning Movement Count - Peak Hour Diagram

CONROY RD @ WALKLEY RD



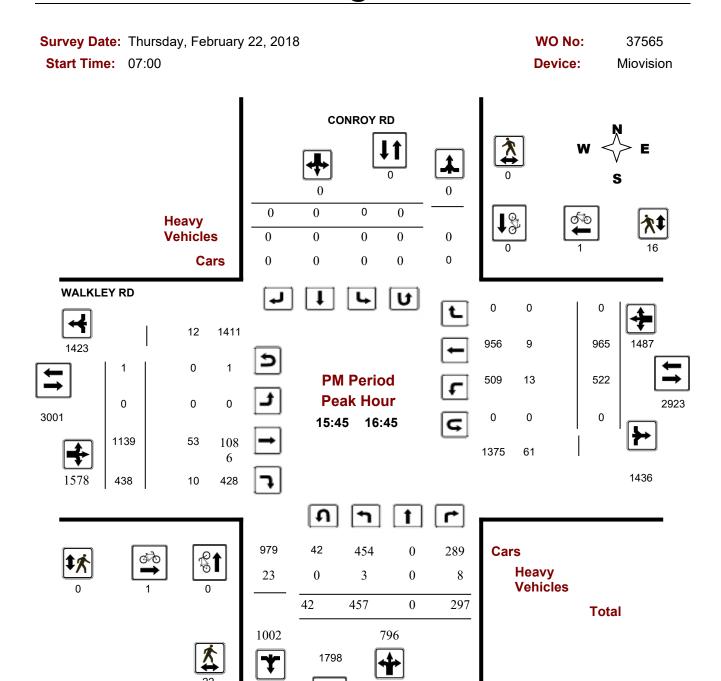
Comments

2019-Apr-08 Page 1 of 4



Turning Movement Count - Peak Hour Diagram

CONROY RD @ WALKLEY RD



Comments

2019-Apr-08 Page 4 of 4



Work Order

37565

Turning Movement Count - Full Study Summary Report

CONROY RD @ WALKLEY RD

Survey Date: Thursday, February 22, 2018

Total Observed U-Turns

AADT Factor

Northbound: 187 Southbound: 0 .90

Eastbound:

Westbound: 3

Full Study

								Г	uli Sti	ıay									
			C	ONRO	Y RD							٧	VALKL	EY RI)				
-	N	lorthb	ound		S	outhbo	ound		_		Eastb	ound			Westbo	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	585	0	379	964	0	0	0	0	964	0	668	174	842	182	895	0	1077	1919	2883
08:00 09:00	641	0	476	1117	0	0	0	0	1117	0	791	257	1048	239	904	0	1143	2191	3308
09:00 10:00	378	0	317	695	0	0	0	0	695	0	671	209	880	227	736	0	963	1843	2538
11:30 12:30	400	0	243	643	0	0	0	0	643	0	798	252	1050	262	740	0	1002	2052	2695
12:30 13:30	342	0	259	601	0	0	0	0	601	0	793	302	1095	241	735	0	976	2071	2672
15:00 16:00	436	0	328	764	0	0	0	0	764	0	1028	360	1388	453	976	0	1429	2817	3581
16:00 17:00	443	0	292	735	0	0	0	0	735	0	1120	419	1539	504	986	0	1490	3029	3764
17:00 18:00	431	0	264	695	0	0	0	0	695	0	945	439	1384	416	824	0	1240	2624	3319
Sub Total	3656	0	2558	6214	0	0	0	0	6214	0	6814	2412	9226	2524	6796	0	9320	18546	24760
U Turns				187				0	187				4				3	7	194
Total	3656	0	2558	6401	0	0	0	0	6401	0	6814	2412	9230	2524	6796	0	9323	18553	24954
EQ 12Hr	5082	0	3556	8897	0	0	0	0	8897	0	9471	3353	12830	3508	9446	0	12959	25789	34686
Note: These	values are	e calcu	ılated b	y multiply	ing the	totals by	/ the ap	opropriat	e expans	ion fac	tor.			1.39					
AVG 12Hr	4574	0	3200	8008	0	0	0	0	8008	0	8524	3017	11547	3158	8502	0	11663	23210	31218
Note: These	volumes a	are cal	culated	by multip	lying th	e Equiv	alent 12	2 hr. tota	ls by the	AADT	factor.			.90					
AVG 24Hr	5991	0	4192	10490	0	0	0	0	10490	0	11167	3953	15126	4136	11137	0	15279	30405	40895
Note: These	volumes a	are cal	culated	by multip	lying th	e Avera	ge Dail	y 12 hr.	totals by	12 to 2	4 expan	sion fa	ctor.	1.31					

Comments:

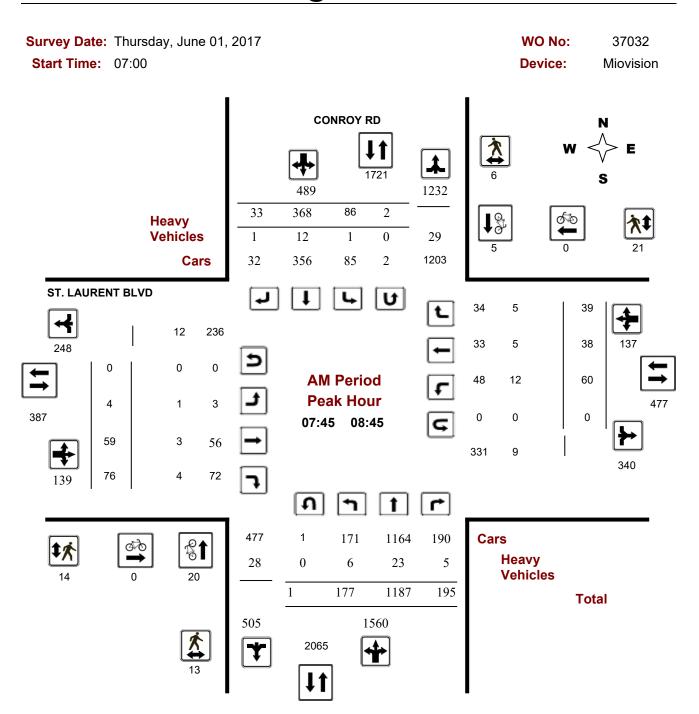
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

2019-Apr-08 Page 1 of 1



Turning Movement Count - Peak Hour Diagram

CONROY RD @ ST. LAURENT BLVD



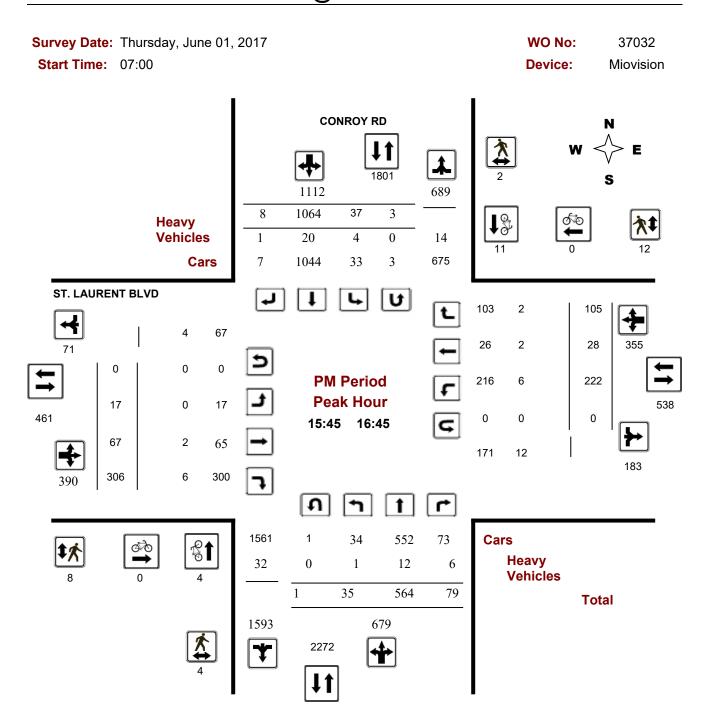
Comments

2019-Apr-17 Page 1 of 4



Turning Movement Count - Peak Hour Diagram

CONROY RD @ ST. LAURENT BLVD



Comments

2019-Apr-17 Page 4 of 4



Work Order

37032

Turning Movement Count - Full Study Summary Report

CONROY RD @ ST. LAURENT BLVD

Survey Date: Thursday, June 01, 2017

Total Observed U-Turns

AADT Factor

Northbound: 12 Eastbound:

Southbound: 36 Westbound: 0

.90

Full Study

								Г	uli Sti	uay									
			C	ONRO	Y RD							ST. L	_AURE	NT BL	.VD				
_		Northb	ound		,	Southb	ound		_		Eastb	ound		,	Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	146	949	149	1244	88	327	33	448	1692	3	28	52	83	44	33	19	96	179	1871
08:00 09:00	153	1186	194	1533	81	387	29	497	2030	4	59	73	136	53	29	46	128	264	2294
09:00 10:00	71	646	114	831	66	427	21	514	1345	10	25	72	107	56	23	47	126	233	1578
11:30 12:30	47	554	70	671	72	521	23	616	1287	13	24	75	112	115	32	128	275	387	1674
12:30 13:30	59	572	113	744	121	522	31	674	1418	8	36	78	122	88	25	99	212	334	1752
15:00 16:00	44	671	68	783	43	873	13	929	1712	13	29	196	238	167	36	109	312	550	2262
16:00 17:00	32	568	76	676	39	1007	9	1055	1731	17	65	314	396	229	23	106	358	754	2485
17:00 18:00	22	598	70	690	54	929	5	988	1678	7	37	207	251	122	16	70	208	459	2137
Sub Total	574	5744	854	7172	564	4993	164	5721	12893	75	303	1067	1445	874	217	624	1715	3160	16053
U Turns				12				36	48				0				0	0	48
Total	574	5744	854	7184	564	4993	164	5757	12941	75	303	1067	1445	874	217	624	1715	3160	16101
EQ 12Hr	798	7984	1187	9986	784	6940	228	8002	17988	104	421	1483	2009	1215	302	867	2384	4393	22381
Note: These	values a	re calcu	ılated b	y multipl	ying the	totals b	y the a	ppropria	te expans	ion fact	or.			1.39					
AVG 12Hr	718	7186	1068	8987	706	6246	205	7202	16189	94	379	1335	1808	1093	271	781	2145	3953	20142
Note: These	volumes	are cal	culated	by multi	plying t	he Equiv	alent 1	2 hr. tota	als by the	AADT	factor.			.90					
AVG 24Hr	941	9413	1400	11773	924	8183	269	9435	21208	123	497	1749	2368	1432	356	1023	2811	5179	26387
Note: These	volumes	are cal	culated	by multi	plying t	he Avera	age Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

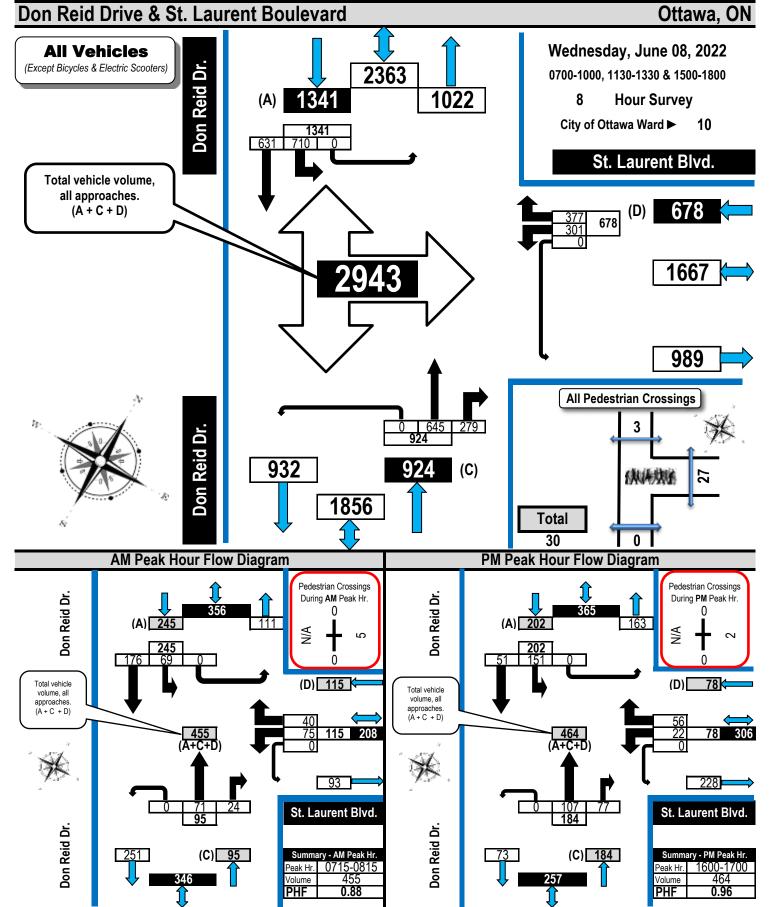
2019-Apr-17 Page 1 of 1



Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams



All Vehicles Except Bicycles





Turning Movement Count Summary Report Including Peak Hours, AADT and Expansion Factors



All Vehicles Except Bicycles

Don Reid Drive & St. Laurent Boulevard

Ottawa, ON

0700 **AADT Factor:** 0.9 **Survey Date:** Wednesday, June 08, 2022 Start Time:

Survey Duration: 8 Hrs. Survey Hours: 0700-1000, 1130-1330 & 1500-1800 Weather AM: Mostly Cloudy 16° C

Weather PM: Mainly Cloudy 23° C Surveyor(s): J. Mousseau

			N/A			St	. Laı	ıren	t Bl	vd.			Don	Rei	d Dr			Don	Rei	d Dr	•		
_		Ea	stbou	ınd			We	stboı	ınd				Noi	thbou	ınd			Sou	uthbo	und		'	
Time	ıт	ST	RT	UT	E/B	LT	ST	RT	UT	W/B	Street	ıт	ST	RT	UT	N/B	ıт	ST	RT	UT	S/B	Street	Grand
Period	_	J I	IXI	5	Tot	_	31	17.1	5	Tot	Total	L	31	IXI	O I	Tot	_	5	IXI	5	Tot	Total	Total
0700-0800						75		29	0	104	104		57	19	0	76	55	157		0	212	288	392
0800-0900						47		66	0	113	113		50	14	0	64	91	94		0	185	249	362
0900-1000						28		57	0	85	85		27	14	0	41	81	69		0	150	191	276
1130-1230						36		39	0	75	75		121	32	0	153	77	89		0	166	319	394
1230-1330						43		49	0	92	92		76	33	0	109	63	75		0	138	247	339
1500-1600						31		42	0	73	73		101	41	0	142	106	44		0	150	292	365
1600-1700						22		56	0	78	78		107	77	0	184	151	51		0	202	386	464
1700-1800						19		39	0	58	58		106	49	0	155	86	52		0	138	293	351
Totals						301		377	0	678	678		645	279	0	924	710	631		0	1341	2265	2943

Equivalent 12 & 24-hour Vehicle Volumes Including the Annual Average Daily Traffic (AADT) Factor **Applicable to the Day and Month of the Turning Movement Count**

Expansion factors are applied exclusively to standard weekday 8-hour turning movement counts conducted during the hours of 0700h - 1000h, 1130h - 1330h and 1500h - 1800h

12 0 897 388 0 1284 987 877 0 0 1864 3148 40 lated by multiplying the equivalent 12-hour totals by the AADT factor of: 0.9
lated by multiplying the equivalent 12-hour totals by the AADT factor of: 0.9
iated by multiplying the equivalent 12-hour totals by the AADT factor of: 0,9
48 0 807 349 0 1156 888 789 0 0 1678 2834 3 0
48 e c

AADT and expansion factors provided by the City of Ottawa

AM Peak Ho	ur Fac	tor •	>	0.	88									Hig	hest	Hourl	y Vehi	icle Vo	lume	Betv	ween (700h &	1000h
AM Peak Hr	LT	ST	RT	UT	Total	LT	ST	RT	UT	Total	Str. Tot.	LT	ST	RT	UT	Total	LT	ST	RT	UT	Total	Str. Tot.	Gr. Tot.
0715-0815	0	0	0	0	0	75	0	40	0	115	115	0	71	24	0	95	69	176	0	0	245	340	455
OFF Peak H	our Fa	ctor	→	0.	84									Hig	hest	Hourl	y Vehi	icle Vo	lume	Betv	ween 1	130h &	1330h
OFF Peak Hr	LT	ST	RT	UT	Total	LT	ST	RT	UT	Total	Str. Tot.	LT	ST	RT	UT	Total	LT	ST	RT	UT	Total	Str. Tot.	Gr. Tot.
1200-1300	0	0	0	0	0	45	0	54	0	99	99	0	120	37	0	157	71	85	0	0	156	313	412
PM Peak Ho	ur Fac	tor 🖣)	0.	96									Hig	hest	Hourl	y Vehi	icle Vo	lume	Betv	ween 1	500h &	1800h
PM Peak Hr	LT	ST	RT	UT	Total	LT	ST	RT	UT	Total	Str. Tot.	LT	ST	RT	UT	Total	LT	ST	RT	UT	Total	Str. Tot.	Gr. Tot.
1600-1700	0	0	0	0	0	22	0	56	0	78	78	0	107	77	0	184	151	51	0	0	202	386	464

Comments:

Buses and school buses comprise 7.94% of the heavy vehicle traffic. Ambulances comprise much of the heavy vehicle volume. The bicycle totals include 3 E-bikes.

Notes:

- 1. Includes all vehicle types except bicycles, electric bicycles, and electric scooters.
- 2. When expansion and AADT factors are applied, the results will differ slightly due to rounding.

APPENDIX E

Collision Records



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ ST. LAURENT BLVD

Traffic Control: Traffic signal Total Collisions: 22

	0								
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Jun-20, Mon,09:52	Clear	Rear end	P.D. only	Dry	South	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jul-18, Mon,18:21	Clear	Rear end	Non-fatal injury	Dry	South	Turning left	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Aug-12, Fri,21:35	Rain	Rear end	P.D. only	Wet	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Oct-21, Fri,13:22	Rain	Angle	Non-fatal injury	Wet	West	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Nov-11, Fri,18:04	Clear	Turning movement	P.D. only	Dry	North	Turning left	Unknown	Other motor vehicle	0
					South	Going ahead	Passenger van	Other motor vehicle	
2016-Nov-11, Fri,18:45	Clear	Angle	P.D. only	Dry	West	Going ahead	Passenger van	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Nov-18, Fri,14:35	Clear	Angle	Non-fatal injury	Dry	North	Going ahead	Passenger van	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jan-17, Tue,15:30	Clear	Rear end	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Pick-up truck	Other motor vehicle	
2017-Feb-05, Sun,21:15	Snow	Sideswipe	P.D. only	Loose snow	South	Slowing or stoppin	g Pick-up truck	Other motor vehicle	0
					South	Stopped	Passenger van	Other motor vehicle	
2017-May-03, Wed,19:07	Clear	Angle	P.D. only	Dry	West	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jun-18, Sun,11:10	Clear	Sideswipe	Non-fatal injury	Dry	South	Unknown	Unknown	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jul-26, Wed,13:14	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	

May 09, 2022 Page 1 of 15



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ ST. LAURENT BLVD

Traffic Control: Traffic signal Total Collisions: 22

	9								
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Jul-26, Wed,14:49	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Aug-11, Fri,13:18	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
2017-Nov-21, Tue,13:45	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	School van	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-08, Mon,10:34	Snow	Angle	P.D. only	Loose snow	South	Going ahead	Police vehicle	Skidding/sliding	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2018-May-03, Thu,07:24	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Oct-09, Tue,07:06	Fog, mist, smoke, dust	, Turning movement	Non-fatal injury	Wet	South	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Mar-29, Fri,15:30	Clear	Rear end	P.D. only	Dry	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2019-Jun-11, Tue,10:44	Clear	Rear end	P.D. only	Dry	North	Going ahead	Delivery van	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Sep-06, Fri,14:40	Clear	Rear end	P.D. only	Dry	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2020-Nov-11, Wed,13:16	Clear	Rear end	P.D. only	Dry	East	Turning right	Unknown	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped

May 09, 2022 Page 2 of 15



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

	•								
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-Jan-17, Sun,16:20	Snow	Rear end	P.D. only	Loose snow	North	Turning right	Pick-up truck	Other motor vehicle	0
					North	Turning right	Pick-up truck	Other motor vehicle	
2016-Feb-09, Tue,10:45	Snow	Rear end	P.D. only	Loose snow	North	Slowing or stoppin	g Pick-up truck	Other motor vehicle	0
					North	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	
2016-Feb-16, Tue,13:00	Snow	Rear end	P.D. only	Packed snow	West	Slowing or stoppin	g Pick-up truck	Other motor vehicle	0
					West	Stopped	Passenger van	Other motor vehicle	
2016-Feb-19, Fri,08:40	Clear	Rear end	P.D. only	Ice	West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Pick-up truck	Other motor vehicle	
2016-Feb-23, Tue,12:31	Clear	Rear end	P.D. only	Dry	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2016-Mar-10, Thu,15:54	Clear	Rear end	Non-fatal injury	Wet	West	Going ahead	Truck - tank	Other motor vehicle	0
					West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	
2016-Mar-29, Tue,18:15	Clear	Sideswipe	P.D. only	Dry	North	Turning left	Unknown	Other motor vehicle	0
					North	Turning left	Pick-up truck	Other motor vehicle	
2016-Apr-02, Sat,13:38	Clear	Rear end	P.D. only	Wet	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Pick-up truck	Other motor vehicle	
2016-Apr-19, Tue,16:43	Clear	Rear end	P.D. only	Dry	West	Going ahead	Passenger van	Other motor vehicle	0
					West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Jun-02, Thu,18:38	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Jul-18, Mon,15:58	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Police vehicle	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

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Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Jul-22, Fri,08:50	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Sep-26, Mon,15:48	Clear	Rear end	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Oct-11, Tue,01:58	Clear	Sideswipe	P.D. only	Dry	North	Overtaking	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Pick-up truck	Other motor vehicle	
2016-Oct-27, Thu,17:45	Snow	Rear end	P.D. only	Wet	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Nov-04, Fri,17:07	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Pick-up truck	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Dec-06, Tue,10:30	Clear	Sideswipe	P.D. only	Wet	West	Changing lanes	Pick-up truck	Other motor vehicle	0
					West	Turning left	Tow truck	Other motor vehicle	
2016-Dec-07, Wed,07:35	Snow	Rear end	P.D. only	Slush	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Dec-19, Mon,06:27	Clear	Angle	P.D. only	Packed snow	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Jan-26, Thu,10:41	Snow	Rear end	P.D. only	Wet	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Apr-25, Tue,08:16	Clear	Rear end	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Pick-up truck	Other motor vehicle	
2017-May-11, Thu,16:15	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Passenger van	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

	0								
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Jun-04, Sun,19:14	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Jun-12, Mon,19:30	Clear	Rear end	P.D. only	Dry	East	Unknown	Unknown	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Jun-15, Thu,18:43	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Passenger van	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Jun-22, Thu,17:14	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Passenger van	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Jul-06, Thu,16:25	Clear	Rear end	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Sep-05, Tue,08:21	Clear	Rear end	Non-fatal injury	Wet	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Sep-26, Tue,13:25	Clear	Rear end	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Oct-11, Wed,16:09	Clear	Rear end	P.D. only	Dry	North	Going ahead	Passenger van	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Nov-08, Wed,15:15	Clear	Rear end	P.D. only	Dry	East	Turning right	Pick-up truck	Other motor vehicle	0
					East	Turning right	Pick-up truck	Other motor vehicle	
2017-Dec-23, Sat,14:25	Snow	Rear end	P.D. only	Loose snow	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-09, Tue,17:40	Clear	Rear end	P.D. only	Slush	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
2018-Jan-26, Fri,12:20	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-Feb-11, Sun,14:18	Rain	Turning movement	P.D. only	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2018-Feb-14, Wed,09:31	Clear	Rear end	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Skidding/sliding	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Apr-23, Mon,16:31	Clear	Rear end	P.D. only	Dry	East	Going ahead	Passenger van	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-May-15, Tue,08:56	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-May-17, Thu,15:00	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-May-29, Tue,08:48	Clear	Other	P.D. only	Dry	West	Reversing	School bus	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-25, Mon,15:50	Clear	Rear end	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2018-Oct-03, Wed,07:44	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Nov-11, Sun,07:45	Clear	Angle	P.D. only	Dry	North	Making "U" turn	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Nov-22, Thu,11:15	Clear	Rear end	P.D. only	Dry	North	Turning right	Pick-up truck	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2019-Jan-16, Wed,14:25	Clear	Rear end	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Jan-21, Mon,09:19	Freezing Rain	Rear end	P.D. only	Ice	East	Going ahead	Unknown	Other motor vehicle	0
					East	Slowing or stopping	g Pick-up truck	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

Trainic Control. Tra	ino oignai						Total Completions.	, , ,	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2019-Feb-02, Sat,16:30	Snow	Rear end	P.D. only	Packed snow	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2019-Feb-11, Mon,20:02	Clear	Other	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Curb	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Mar-07, Thu,16:30	Clear	Rear end	P.D. only	Ice	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
					East	Unknown	Unknown	Other motor vehicle	
2019-Mar-20, Wed,16:49	Rain	Rear end	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Apr-03, Wed,01:21	Clear	SMV other	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Curb	0
2019-May-19, Sun,15:00	Rain	Angle	Non-fatal injury	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2019-May-21, Tue,14:55	Clear	Rear end	P.D. only	Dry	East	Stopped	Pick-up truck	Other motor vehicle	0
					East	Slowing or stoppin	ng Pick-up truck	Other motor vehicle	
2019-Jun-03, Mon,14:47	Clear	Rear end	P.D. only	Dry	East	Slowing or stoppin	ng Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stoppin	ng Automobile, station wagon	Other motor vehicle	
2019-Aug-13, Tue,16:25	Clear	Rear end	P.D. only	Dry	East	Slowing or stoppin	ng Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stoppin	ng Automobile, station wagon	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Sep-18, Wed,16:36	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Oct-02, Wed,08:30	Clear	Rear end	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	

May 09, 2022 Page 7 of 15



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

	•								
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2019-Oct-07, Mon,09:00	Clear	Rear end	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Nov-05, Tue,06:22	Clear	SMV other	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Pedestrian	1
2019-Nov-18, Mon,10:05	Clear	Rear end	P.D. only	Dry	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2019-Nov-22, Fri,07:59	Clear	Rear end	P.D. only	Wet	North	Going ahead	Construction equipment	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Dec-04, Wed,09:33	Snow	Rear end	P.D. only	Loose snow	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Dec-13, Fri,15:49	Clear	Sideswipe	P.D. only	Wet	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Dec-16, Mon,14:29	Clear	Rear end	P.D. only	Ice	North	Slowing or stoppin	g Automobile, station wagon	Skidding/sliding	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Dec-16, Mon,16:23	Clear	Rear end	P.D. only	Dry	West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Pick-up truck	Other motor vehicle	
2019-Dec-27, Fri,11:30	Clear	Rear end	P.D. only	Ice	West	Going ahead	Automobile, station wagon	Skidding/sliding	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2020-Feb-03, Mon,15:35	Clear	Rear end	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2020-Mar-19, Thu,21:45	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Unknown	Automobile, station wagon	Other motor vehicle	
2020-May-12, Tue,11:56	Clear	Sideswipe	P.D. only	Dry	North	Turning left	Pick-up truck	Other motor vehicle	0
					North	Turning left	Passenger van	Other motor vehicle	

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From: January 1, 2016 **To:** December 31, 2020

Location: CONROY RD @ WALKLEY RD

Traffic Control: Traffic signal Total Collisions: 74

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2020-Jun-22, Mon,16:39	Rain	Angle	Non-fatal injury	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2020-Jul-27, Mon,13:55	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Pick-up truck	Other motor vehicle	
					East	Stopped	Passenger van	Other motor vehicle	
2020-Jul-28, Tue,15:56	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2020-Oct-20, Tue,20:55	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Unknown	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2020-Oct-31, Sat,15:48	Clear	Rear end	P.D. only	Dry	North	Turning right	Pick-up truck	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	

Location: CONROY RD btwn ST. LAURENT BLVD & WALKLEY RD

Traffic Control: No control Total Collisions: 3

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Jan-16, Sat,12:02	Snow	Sideswipe	P.D. only	Loose snow	North	Changing lanes	Unknown	Other motor vehicle	0
					North	Going ahead	Pick-up truck	Other motor vehicle	
2017-Sep-23, Sat,18:45	Clear	SMV other	Non-fatal injury	Dry	North	Turning right	Automobile, station wagon	Ran off road	0
2019-Jan-14, Mon,14:54	Clear	Rear end	P.D. only	Dry	South	Unknown	Unknown	Other motor vehicle	0
					South	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	

Location: DON REID DR @ ST. LAURENT BLVD

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped
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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: DON REID DR @ ST. LAURENT BLVD

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-Feb-28, Sun,09:00	Snow	Angle	P.D. only	Loose snow	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: ST. LAURENT BLVD btwn CONROY RD & DON REID DR

Traffic Control: No control

Total Collisions: 2

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2019-Jan-15, Tue,13:02	Clear	SMV other	P.D. only	Wet	East	Going ahead	Automobile, station wagon	Skidding/sliding	0
2020-Feb-12, Wed,16:22	Snow	Angle	P.D. only	Wet	North	Turning right	Pick-up truck	Other motor vehicle	0
					East	Going ahead	Passenger van	Other motor vehicle	

Location: WALKLEY RD @ DON REID DR/RYDER ST

Traffic Control: Traffic signal Total Collisions: 37

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-Apr-10, Sun,13:12	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Jun-19, Sun,07:07	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Jul-15, Fri,11:05	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Bicycle	Other motor vehicle	0
					North	Turning right	Pick-up truck	Cyclist	
2016-Sep-19, Mon,15:35	Clear	Angle	Non-fatal injury	Dry	North	Turning right	Unknown	Cyclist	0
					West	Going ahead	Bicycle	Other motor vehicle	
2016-Dec-12, Mon,15:21	Snow	Angle	Non-fatal injury	Loose snow	West	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Pick-up truck	Other motor vehicle	
2017-Mar-14, Tue,18:18	Snow	SMV other	P.D. only	Loose snow	East	Slowing or stoppin	g Pick-up truck	Ran off road	0
2017-Mar-24, Fri,11:16	Snow	SMV other	P.D. only	Wet	South	Slowing or stoppin	ig Pick-up truck	Pole (utility, power)	0

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: WALKLEY RD @ DON REID DR/RYDER ST

Traffic Control: Traffic signal Total Collisions: 37

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Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2017-May-29, Mon,10:40	Rain	Angle	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Pick-up truck	Other motor vehicle	
2017-Jun-04, Sun,12:37	Clear	Angle	P.D. only	Dry	West	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jun-28, Wed,15:34	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Passenger van	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jun-28, Wed,15:51	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stoppin	g Passenger van	Other motor vehicle	
					East	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	
2017-Aug-08, Tue,07:30	Clear	Rear end	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stoppin	g Pick-up truck	Other motor vehicle	
2017-Sep-02, Sat,16:25	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Sep-26, Tue,23:04	Clear	Rear end	P.D. only	Dry	East	Unknown	Unknown	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Oct-20, Fri,17:15	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Pick-up truck	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Feb-15, Thu,08:51	Freezing Rain	Rear end	P.D. only	Ice	West	Slowing or stoppin	g Automobile, station wagon	Skidding/sliding	0
					West	Turning left	Delivery van	Other motor vehicle	
2018-Mar-11, Sun,18:03	Clear	Sideswipe	P.D. only	Dry	West	Going ahead	Unknown	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Mar-16, Fri,17:52	Clear	Angle	P.D. only	Dry	West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: WALKLEY RD @ DON REID DR/RYDER ST

Traffic Control: Traffic signal Total Collisions: 37

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Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-Sep-23, Sun,15:20	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Nov-13, Tue,15:20	Freezing Rain	Rear end	P.D. only	Wet	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jan-20, Sun,19:26	Snow	Approaching	Non-fatal injury	Loose snow	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Ambulance	Other motor vehicle	
2019-Feb-02, Sat,17:30	Snow	Angle	P.D. only	Wet	West	Going ahead	Passenger van	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Feb-03, Sun,06:57	Snow	Turning movement	P.D. only	Slush	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Pick-up truck	Other motor vehicle	
2019-Feb-15, Fri,16:15	Clear	Rear end	Non-fatal injury	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jun-12, Wed,18:00	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jun-17, Mon,06:45	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Pick-up truck	Other motor vehicle	
2019-Jun-23, Sun,15:09	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Sep-05, Thu,09:44	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Fire vehicle	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Sep-17, Tue,16:30	Clear	Rear end	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Other motor vehicle	
2019-Nov-01, Fri,13:59	Clear	Angle	P.D. only	Dry	West	Turning left	School bus	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: WALKLEY RD @ DON REID DR/RYDER ST

Traffic Control: Traffic signal Total Collisions: 37

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2019-Dec-19, Thu,16:18	Clear	Angle	P.D. only	Wet	West	Going ahead	Fire vehicle	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Dec-24, Tue,12:55	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2020-Feb-04, Tue,15:55 Cl	Clear	Rear end	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2020-Feb-06, Thu,16:40	Clear	Rear end	P.D. only	Dry	West	Going ahead	Passenger van	Other motor vehicle	0
					West	Slowing or stopping	Delivery van	Other motor vehicle	
2020-Apr-10, Fri,20:00	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	
2020-May-02, Sat,16:17	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Pick-up truck	Other motor vehicle	
2020-May-29, Fri,09:20	Clear	Rear end	P.D. only	Dry	West	Going ahead	Construction equipment	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	

Location: WALKLEY RD btwn 160 W OF CONROY RD & CONROY RD

Traffic Control: No control Total Collisions: 7

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2017-Oct-26, Thu,15:41	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Pick-up truck	Other motor vehicle	
2017-Nov-24, Fri,16:41	Clear	Rear end	P.D. only	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Pick-up truck	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: WALKLEY RD btwn 160 W OF CONROY RD & CONROY RD

Traffic Control: No control

Total Collisions: 7

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-Oct-10, Wed,14:20	Clear	Turning movement	P.D. only	Dry	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Feb-02, Sat,15:00 Sno	Snow	Rear end	P.D. only	Loose snow	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Passenger van	Other motor vehicle	
2019-May-08, Wed,05:01	Clear	Angle	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-May-24, Fri,13:04	Clear	Angle	P.D. only	Dry	North	Turning right	Unknown	Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	
2020-Sep-25, Fri,19:26	Clear	Rear end	P.D. only	Dry	West	Changing lanes	Motorcycle	Other motor vehicle	0
					West	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	

Location: WALKLEY RD btwn 160 W OF CONROY RD & DON REID DR/RYDER ST

Traffic Control: No control

Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped
2017-May-13, Sat,12:47	Rain	Rear end	P.D. only	Wet	East	Slowing or stopping Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping Passenger van	Other motor vehicle	
					East	Stopped Automobile, station wagon	Other motor vehicle	
2018-Jan-30, Tue,17:29 Cle	Clear	Rear end	P.D. only	Dry	East	Slowing or stopping Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping Automobile, station wagon	Other motor vehicle	
					East	Slowing or stopping Automobile, station wagon	Other motor vehicle	
2018-Feb-15, Thu,13:43	Clear	Sideswipe	P.D. only	Wet	East	Unknown Unknown	Other motor vehicle	0
					East	Going ahead Truck - closed	Other motor vehicle	
2019-Apr-09, Tue,07:45	Freezing Rain	Rear end	P.D. only	Ice	East	Slowing or stopping Passenger van	Other motor vehicle	0
					East	Stopped Pick-up truck	Other motor vehicle	

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Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: WALKLEY RD btwn 160 W OF CONROY RD & DON REID DR/RYDER ST

Traffic Control: No control

Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2020-Aug-05, Wed,12:41	Clear	Rear end	P.D. only	Dry	East	Going ahead	Truck - tank	Other motor vehicle	0
					East	Stopped	Municipal transit bus	Other motor vehicle	

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No.	Location	Date	Time	Environment	Road_Surface	Traffic_Control	Collision_Location	Light	Collision_Classification	Impact_Type
1	WALKLEY RD @ 160 W OF CONROY RD	5/11/2017	5:42	01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	03 - Dawn	02 - Non-fatal injury	07 - SMV other
2	WALKLEY RD @ 160 W OF CONROY RD (0011391)	6/11/2018	15:36	01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	02 - Non-fatal injury	03 - Rear end
3	WALKLEY RD @ 160 W OF CONROY RD (0011391)	6/12/2018	14:02	01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	02 - Non-fatal injury	05 - Turning movement
4	WALKLEY RD @ 160 W OF CONROY RD (0011391)	1/25/2019	15:46	01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end
5	WALKLEY RD @ 160 W OF CONROY RD (0011391)	1/26/2019	16:37	01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	02 - Non-fatal injury	03 - Rear end
6	WALKLEY RD @ 160 W OF CONROY RD (0011391)	3/29/2019	17:13	01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	02 - Non-fatal injury	05 - Turning movement
7	WALKLEY RD @ 160 W OF CONROY RD (0011391)	4/30/2019	9:06	01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end
8	WALKLEY RD @ 160 W OF CONROY RD (0011391)	12/4/2019	14:35	01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	99 - Other
9	WALKLEY RD @ 160 W OF CONROY RD (0011391)	10/16/2020	10:26	01 - Clear	02 - Wet	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end
10	WALKLEY RD @ 160 W OF CONROY RD (0011391)	10/24/2020	6:08	02 - Rain	02 - Wet	01 - Traffic signal	03 - At intersection	07 - Dark	02 - Non-fatal injury	05 - Turning movement
11	WALKLEY RD @ 160 W OF CONROY RD (0011391)	12/30/2020	11:40	05 - Drifting Snow	03 - Loose snow	01 - Traffic signal	03 - At intersection	01 - Daylight	02 - Non-fatal injury	05 - Turning movement

APPENDIX F Relevant Excerpts of TRANS Trip Generation Manual (WSP, 2020)

3.2 Recommended Residential Trip Generation Rates

A blended trip rate was developed from the three data sources through application of a rank-sum weighting process, considering the strengths and weaknesses of each dataset for the dwelling type in question. The recommended blended **residential person-trip rates** are presented in **Table 3**. All rates represent person-trips per dwelling unit and are to be applied to the **AM or PM peak period**.

ITE Land Use Code	Dwelling Unit Type	Period	Person-Trip Rate
210	Single detected	AM	2.05
210	Single-detached	PM	2.48
220	Multi Unit (Low Pice)	AM	1.35
220	Multi-Unit (Low-Rise)	PM	1.58
221 & 222	Multi-Unit (High-Rise)	AM	0.80
221 & 222	Mulli-Offit (High-Nise)	DМ	0.90

Table 3: Recommended Residential Person-trip Rates

3.3 Adjustment Factors - Peak Period to Peak Hour

The various trip generation data sources require some adjustment to standardize the data for developing robust blended trip rates. The peak period conversion factor in **Table 4** may be used where applicable to develop trip generation rate estimates in the desired format.

Table 4: Adjus	tment Factors	for Residential Trip	Generation Rates

Factor	Application	Apply To	Period	Value
		Person-trip	AM	0.50
	Back wasied to waste back	rates per peak period	PM	0.44
	Peak period to peak hour conversion. Because the 2020	Vehicle trip	AM	0.48
	TRANS Trip Generation Study	rates per peak period	PM	0.44
Peak Period Conversion	reports trip generation rates by peak period, factors must be applied if the practitioner requires peak hour rates. In practice, the conversion to peak hour trip rates should occur after the application of modal shares.	Transit trip	AM	0.55
Factor		rates per peak period	PM	0.47
		Cycling trip	AM	0.58
		rates per peak period	PM	0.48
	application of modal shares.	Walking trip	AM	0.58
		rates per peak period	PM	0.52

Table 7: Residential Mode Share for Low-Rise Multifamily Housing

				Mode		
District	Period	Auto Driver	Auto Pass.	Transit	Cycling	Walking
Ottowa Cantra	AM	27%	9%	25%	9%	30%
Ottawa Centre	PM	31%	10%	20%	9%	30%
Ottown Immor Area	AM	27%	8%	26%	9%	30%
Ottawa Inner Area	PM	31%	9%	20%	9%	31%
Île de Hull	AM	27%	9%	25%	9%	30%
ile de Hull	PM	34%	22%	16%	5%	22%
Ottowa Foot	AM	36%	11%	38%	7%	8%
Ottawa East	PM	39%	16%	29%	5%	11%
Pagan Hill	AM	45%	9%	35%	1%	10%
Beacon Hill	PM	48%	16%	24%	1%	11%
Alta Vista	AM	38%	15%	35%	1%	10%
Alla Vista	PM	38%	19%	31%	2%	10%
Hunt Club	AM	44%	11%	38%	1%	6%
Hulli Club	PM	47%	15%	29%	1%	8%
Maritrala	AM	44%	11%	32%	6%	7%
Merivale	PM	44%	12%	29%	4%	11%
Ottown Most	AM	36%	12%	24%	10%	19%
Ottawa West	PM	35%	12%	16%	10%	27%
Dayahara/Cadamiayy	AM	43%	11%	31%	1%	13%
Bayshore/Cedarview	PM	44%	14%	25%	1%	15%
Liuli Dáninhánia	AM	46%	22%	22%	4%	6%
Hull Périphérie	PM	46%	17%	22%	3%	11%
Orloopo	AM	47%	15%	29%	1%	9%
Orleans	PM	51%	19%	24%	1%	6%
South Gloucester /	AM	59%	20%	16%	1%	4%
Leitrim	PM	62%	18%	17%	1%	3%
Couth Manage	AM	49%	13%	26%	2%	9%
South Nepean	PM	49%	13%	24%	2%	12%
Kanata - Stittsville	AM	52%	14%	22%	0%	11%
Kanata - Stittsville	PM	58%	17%	17%	0%	8%
Dietecu	AM	44%	18%	28%	4%	6%
Plateau	PM	47%	17%	26%	2%	8%
Aulmor	AM	52%	18%	23%	0%	7%
Aylmer	PM	52%	16%	20%	1%	12%
Pointe Gatineau	AM	46%	17%	23%	0%	14%
Pointe Gatineau	PM	52%	16%	19%	1%	12%
Gatineau Est	AM	54%	17%	20%	1%	8%
Gatineau Est	PM	56%	21%	16%	0%	7%
Masson-Angers	AM	60%	15%	21%	4%	1%
Masson-Angers	PM	63%	15%	17%	3%	1%
Other Rural Districts	AM	66%	13%	21%	1%	0%
Other Naral Districts	PM	62%	19%	16%	3%	0%

Table 8: Residential Mode Share for High-Rise Multifamily Housing

			3	Mode	3	
District	Period	Auto Driver	Auto Pass.	Transit	Cycling	Walking
Ottown Contro	AM	18%	2%	26%	1%	52%
Ottawa Centre	PM	17%	9%	21%	1%	52%
Ottowa Immar Area	AM	26%	6%	28%	5%	34%
Ottawa Inner Area	PM	25%	8%	21%	6%	39%
البيال عام ال	AM	27%	3%	37%	12%	21%
Île de Hull	PM	26%	8%	27%	11%	28%
Ottowo Foot	AM	39%	7%	38%	2%	13%
Ottawa East	PM	40%	14%	28%	3%	15%
Danas IIII	AM	48%	9%	30%	3%	10%
Beacon Hill	PM	52%	16%	28%	0%	4%
A14- \ /:-4-	AM	38%	12%	42%	2%	7%
Alta Vista	PM	45%	16%	28%	2%	9%
Lloud Olob	AM	39%	6%	44%	1%	9%
Hunt Club	PM	44%	11%	35%	2%	9%
	AM	41%	6%	42%	2%	8%
Merivale	PM	41%	11%	33%	2%	13%
200	AM	28%	11%	41%	3%	16%
Ottawa West	PM	33%	11%	26%	7%	23%
	AM	40%	12%	38%	2%	8%
Bayshore/Cedarview	PM	40%	15%	33%	1%	11%
	AM	48%	11%	30%	1%	10%
Hull Périphérie	PM	47%	15%	23%	3%	13%
0.1	AM	54%	7%	29%	0%	10%
Orleans	PM	61%	13%	21%	0%	6%
South Gloucester /	AM	50%	15%	25%	1%	9%
Leitrim	PM	53%	17%	21%	1%	9%
	AM	58%	6%	30%	2%	4%
South Nepean	PM	54%	15%	25%	0%	7%
	AM	43%	26%	28%	0%	4%
Kanata - Stittsville	PM	55%	19%	21%	0%	5%
-	AM	53%	9%	35%	3%	1%
Plateau	PM	65%	7%	25%	2%	1%
	AM	45%	17%	25%	0%	13%
Aylmer	PM	31%	21%	23%	4%	20%
	AM	44%	15%	24%	3%	14%
Pointe Gatineau	PM	52%	15%	20%	2%	11%
	AM	53%	10%	25%	0%	12%
Gatineau Est	PM	61%	10%	25%	0%	4%
	AM	63%	15%	19%	0%	3%
Masson-Angers	PM	64%	18%	16%	0%	1%
	AM	63%	15%	19%	0%	3%
Other Rural Districts	PM	64%	18%	16%	0%	1%
	FIVI	0470	1070	1070	U70	1 70

5 RESIDENTIAL DIRECTIONAL SPLITS

After calculating the total person trips generated by the development and applying the appropriate modal shares, directional factors can be applied to estimate the number of inbound and outbound trips by vehicle. The vehicle trip directional splits were developed for both the AM and PM peak periods². The vehicle trip directional splits, as shown in **Table 9**, have been developed for the NCR based on a review of the local trip generator surveys as well as the latest published data in the ITE *Trip Generation Manual* (10th Edition).

Table 9: Recommended Vehicle Trip Directional Splits (Peak Period)

ITE Land Use Code	Dwelling Unit Type	Period	Inbound	Outbound
210	Single-detached	AM	30%	70%
210	Single-detached	PM	62%	38%
220	Multi-Unit (Low-Rise)	AM	30%	70%
220	wuiti-Offit (Low-Rise)	PM	56%	44%
221 & 222	Multi Unit (High Dica)	AM	31%	69%
221 & 222	Multi-Unit (High-Rise)	PM	58%	42%

6 NON-RESIDENTIAL MODE SHARE

Mode shares were developed for three types of non-residential development: schools (elementary and high school); employment generators; and commercial (retail) generators. These mode shares were developed through data provided by the Ville de Gatineau from local school surveys as well as the TRANS Origin-Destination Survey. The non-residential mode shares presented below are limited and do not capture all development types. For data on the travel characteristics associated with colleges and universities, transportation terminals, and sports and entertainment venues in the National Capital Region, practitioners should refer to the various reports for the TRANS *Special Generators Survey* (2013), which are posted on the TRANS website. For other development types, practitioners may need to carry out their own local generator data collection where necessary.

-

² A directional split for active transportation was calculated based on the local generator surveys for low-rise and mid-rise land uses. The splits are mostly in-line with the vehicle directional splits, which could be used as a rough assumption for areas with lower vehicle mode share.

APPENDIX G

Other Area Developments

Timbercreek Heron Gate Official Plan Amendment Transportation Impact Assessment

Step 1 Screening Report
Step 2 Scoping Report
Step 3 Forecasting Report
Step 4 Strategy Report (revision #3)

Prepared for:

Timbercreek Asset Management 25 Price Street Toronto, ON M4W 1Z1

Prepared by:



13 Markham Avenue Ottawa, ON K2G 3Z1

April 2021

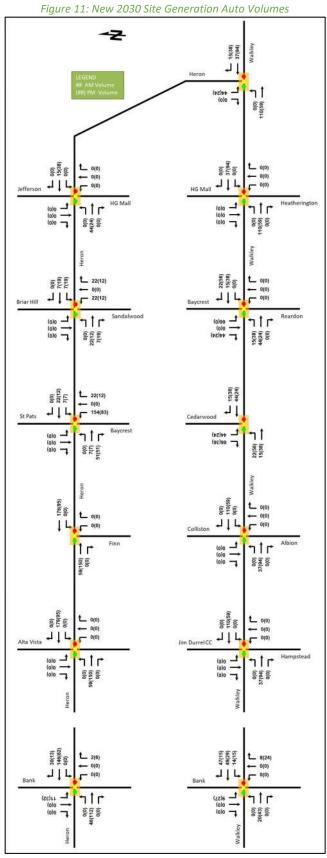
PN: 2018-49



Figure 1: Area Context Plan

Source: http://maps.ottawa.ca/geoOttawa/ Accessed: October 27, 2020







2020 Walkley Road & 2935 Conroy Road Transportation Impact Assessment

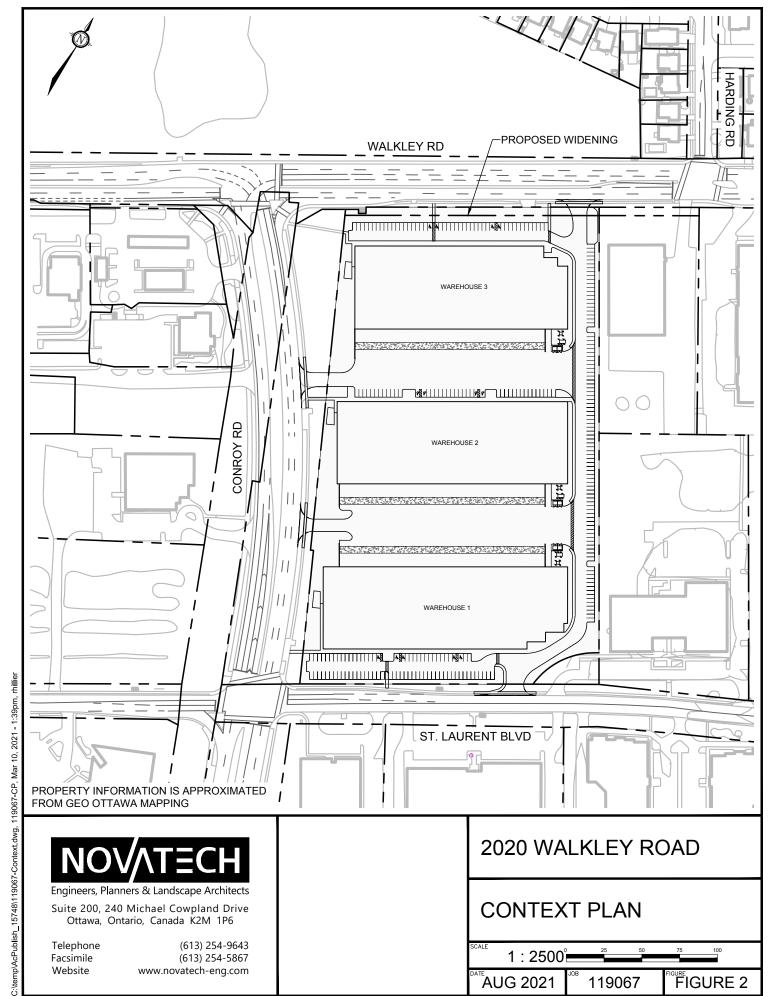
Prepared By:

NOVATECH

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario K2M 1P6

> March 2021 Rev: August 2021

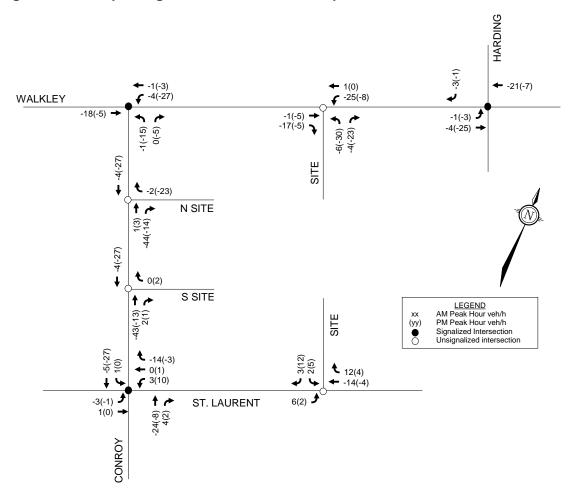
Novatech File: 119067 Ref: R-2020-124



SHT8X11.DWG - 216mmx279mm

The net assignment of trips with site redevelopment (Proposed Site Generated Trips, **Figure 5** less Existing Site Generated Trips, **Figure 6**) is shown in **Figure 7**.

Figure 7: Net Trip Assignment for Site Redevelopment



Truck access is primarily expected to be from / to the east on Walkley and Highway 417. Trucks are expected to travel via Walkley Road, Conroy Road, and St. Laurent Boulevard. Truck departures are expected to primarily be from the Conroy Road driveways due to the connections from the truck courts.

5.2 Background Traffic

5.2.1 Future Background Growth

For the 'Inner Suburbs' area of Ottawa, Exhibit 2.10 of the 2013 TMP projects population and employment growth rates of approximately 0.3% and 1.2% per annum, respectively. A 1% background growth rate has been applied to non-site traffic in this area.

This 1% background growth rate is in line with the annual historical (2000 to 2016) growth rate for this area (-2% to 2%) identified by the City of Ottawa (See **Figure 8**).

Novatech Page 16



September 7, 2017 File: 163601146

Attention: Simon Nehme Conroy Business Park Inc. 1890 Broadmoor Ave Ottawa, Ontario K1H 5B4

Dear Mr. Nehme,

Reference: 2500 St Laurent Blvd Transportation Brief

1.0 INTRODUCTION

Conroy Business Park Inc. is seeking site plan approval for a proposed office development located at 2500 St Laurent Boulevard in the Alta Vista area of the City of Ottawa. Stantec Consulting Ltd. was retained to undertake a Transportation Brief to determine the potential transportation implications of the proposed office development.

This Transportation Brief includes:

- A description of the proposed office development;
- A review of the site plan to confirm site access location;
- An overview of the existing surrounding transportation environment, including an operational assessment of the study area intersections under 2017 existing conditions;
- The volume of site traffic the proposed office development is anticipated to generate during the AM and PM roadway peak hours;
- An operational assessment of the study area intersections under 2021 total future conditions (site build-out); and
- An operational assessment of the study area intersections under 2026 ultimate future conditions (5-years beyond build-out).

Reference: 2500 St Laurent Blvd Transportation Brief

Figure 7 - Auto Trips Generated at the Site during AM Peak Period

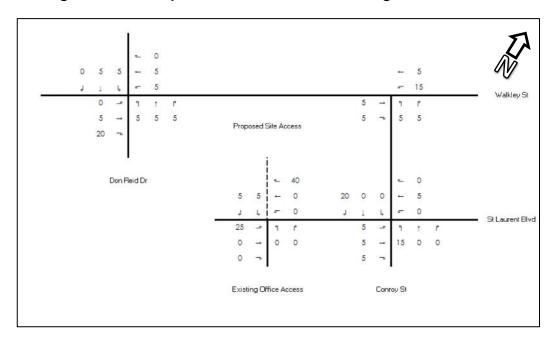
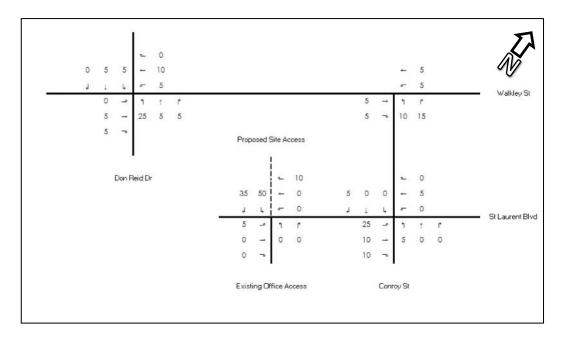
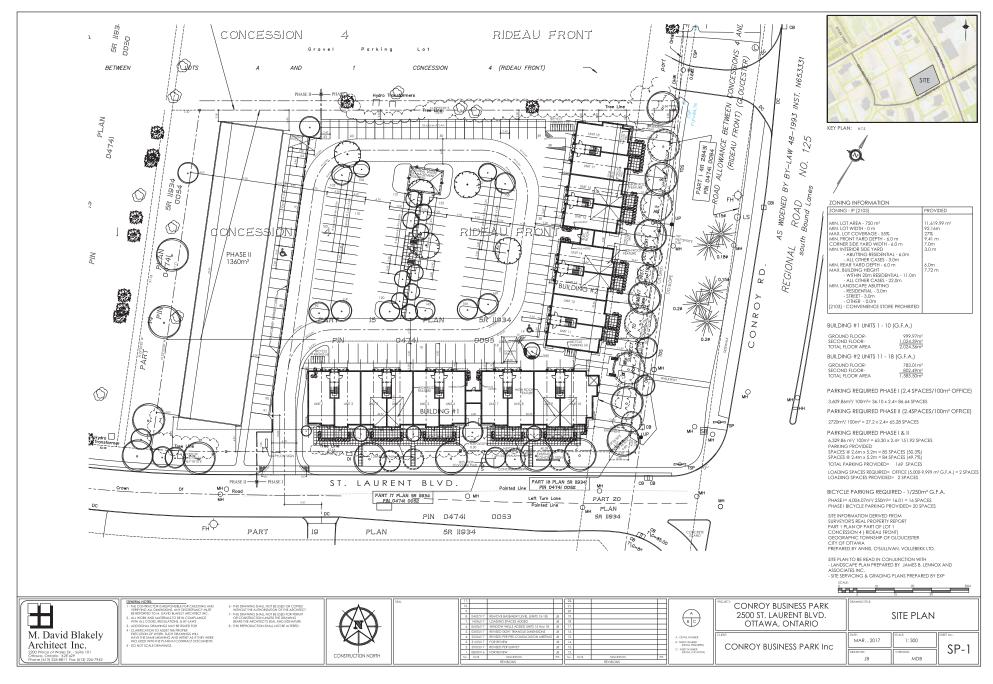


Figure 8 - Auto Trips generated at the Site during PM Peak Period





NSTETLANSTETLAN-17-01-04, Bindbong 7/4/2017 TELE41 AM

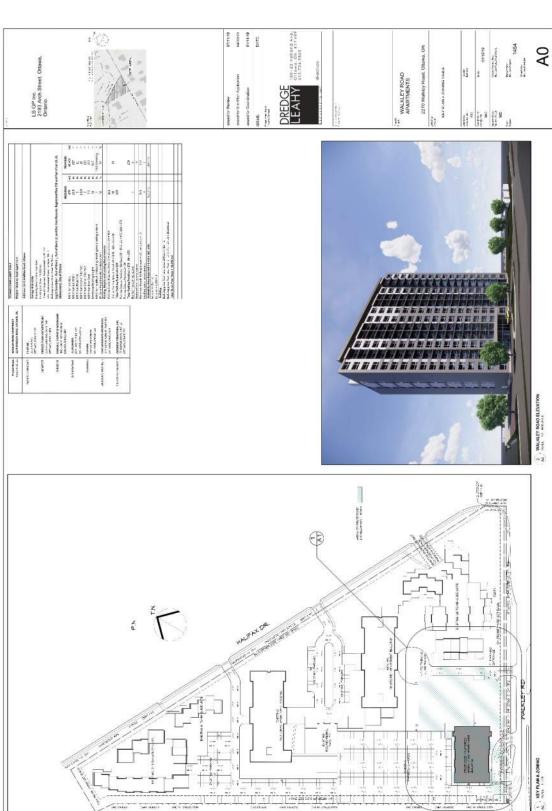


LS GP INC.

Walkley Road Apartments 2190 Halifax Drive

Transportation Impact Assessment

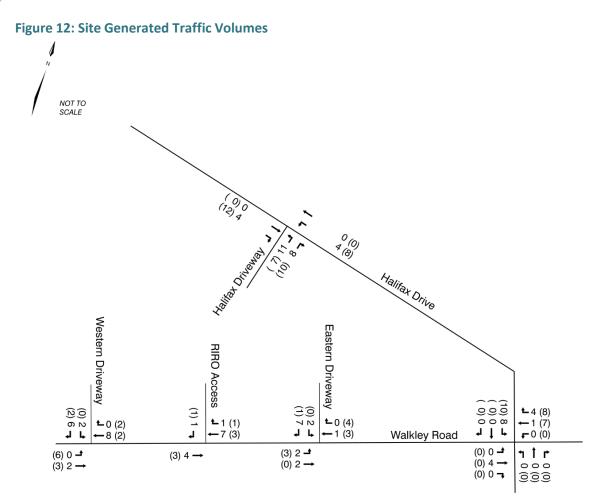
Figure 4: Proposed Site Plan









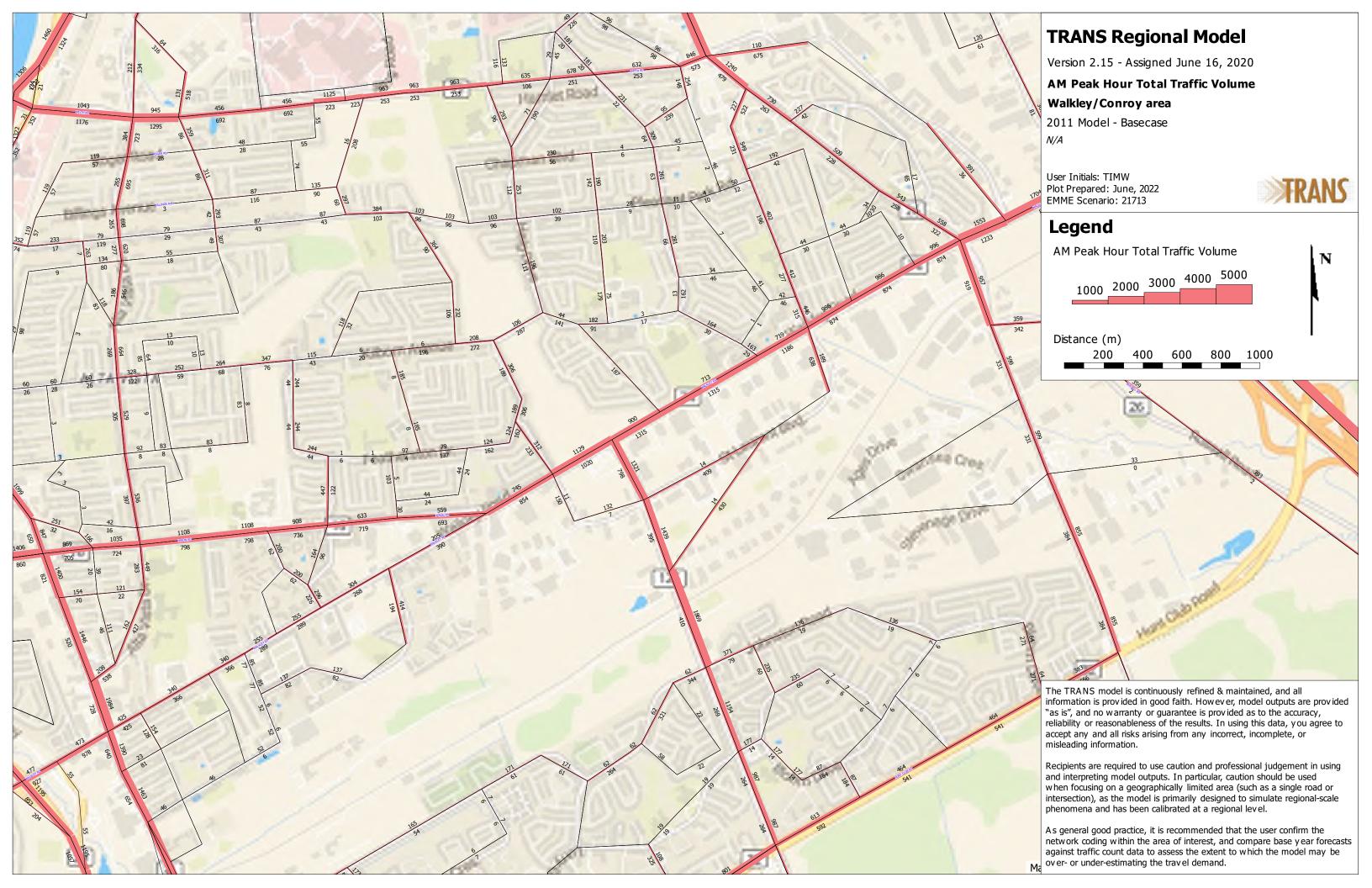


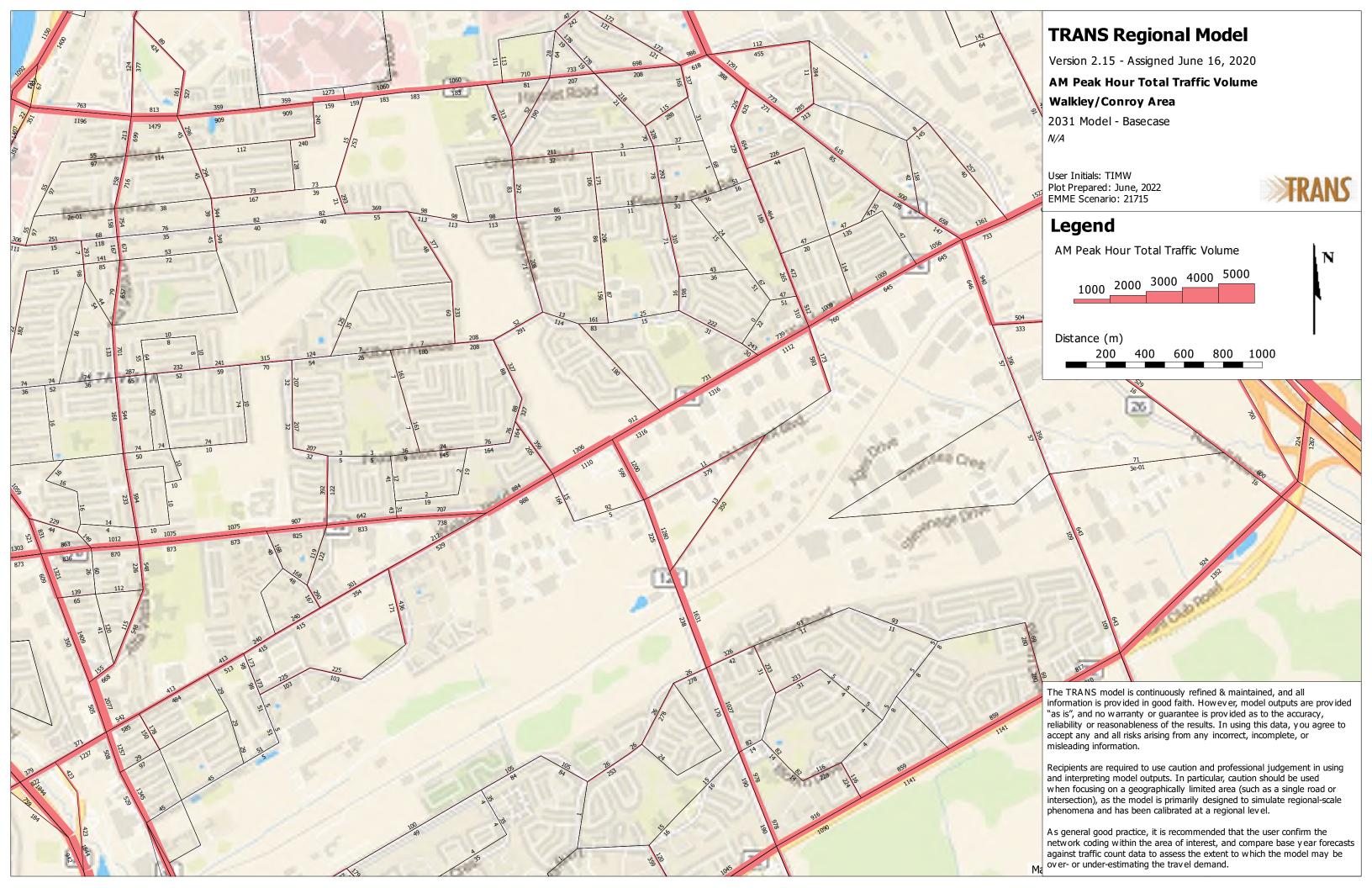
Legend:

- ► AM (PM)
- ← peak hour turning



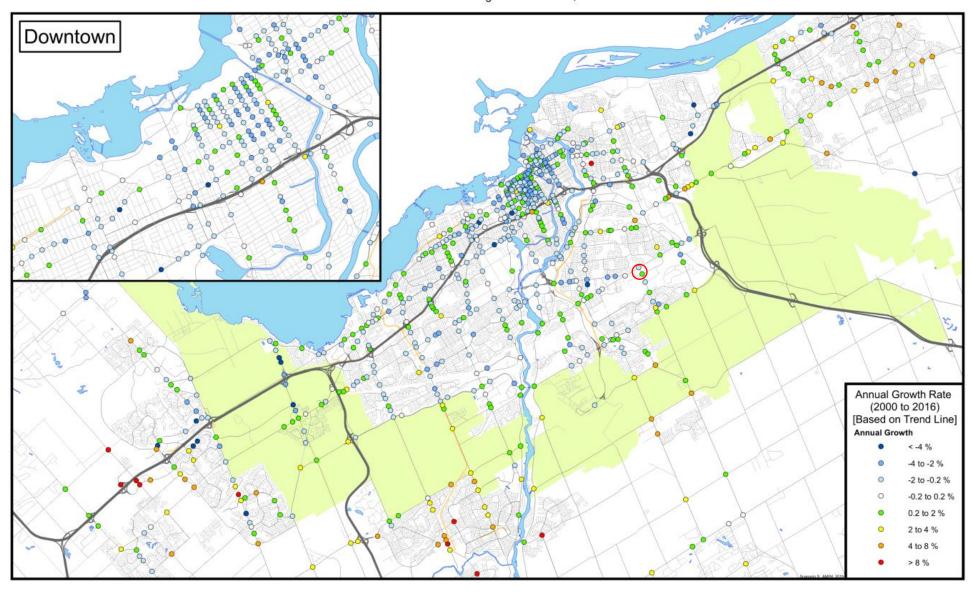
APPENDIX H Strategic Long-Range Model and Intersection Growth Rate Figures





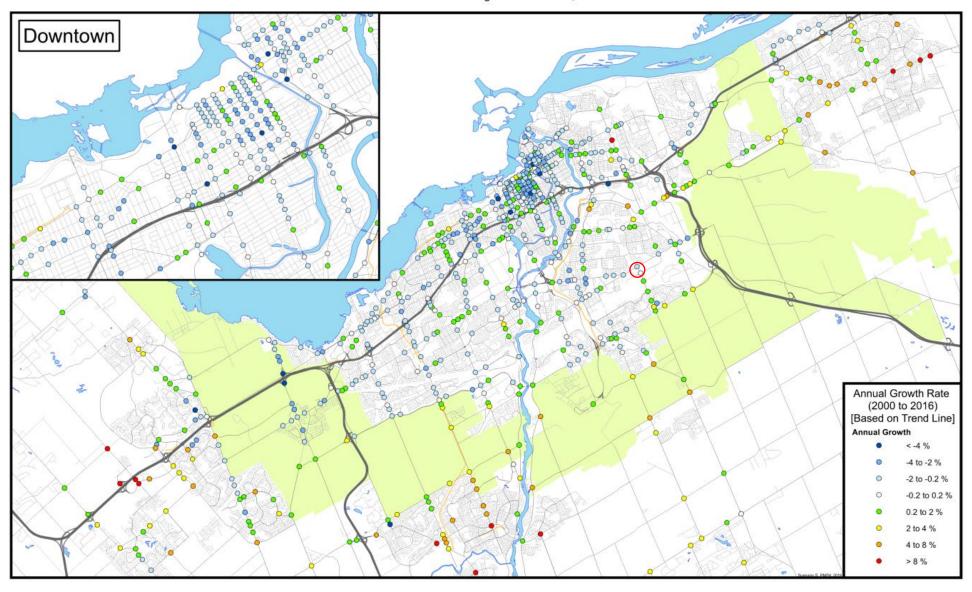
INTERSECTION TRAFFIC GROWTH RATE, AM PEAK PERIOD

Total Vehicular Volume Entering the Intersection, 2000 to 2016



INTERSECTION TRAFFIC GROWTH RATE, PM PEAK PERIOD

Total Vehicular Volume Entering the Intersection, 2000 to 2016



APPENDIX I

Signal Timing Plans

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

Intersection: Main: Walkley Side: Don Reid / Ryder

Controller: MS 3200 TSD: 5424

Author: Matthew Anderson Date: 01-Jun-2022

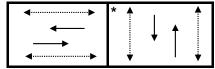
Existing Timing Plans[†]

Plan Ped Minimum Time

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	100	100	110	80	100			
Offset	94	20	20	30	38			
EB Thru	64	64	74	44	64	15	15	3.3+2.6
WB Thru	64	64	74	44	64	15	15	3.3+2.6
NB Thru	36	36	36	36	36	10	20	3.3+2.9
SB Thru	36	36	36	36	36	10	20	3.3+2.9

Phasing Sequence[‡]

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
9:30	2
15:00	3
18:30	2
21:30	4

Saturday

outu. uu,						
Time	Plan					
0:15	4					
6:30	2					
11:00	5					
19:30	2					
22:00	4					

Sunday

Time	Plan
0:15	4
6:30	2
21:00	4

Notes

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

Intersection: *Main:* Walkley *Side:* 160m W of Conroy

Controller: MS 3200 TSD: 6486

Author: Matthew Anderson Date: 01-Jun-2022

Existing Timing Plans[†]

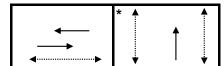
Plan

Ped Minimum Time

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	100	100	110	80	100			
Offset	88	20	25	X	16			
EB Thru	71	71	81	51	71	18	13	3.3+2.7
WB Thru	71	71	81	51	71	-	-	3.3+2.7
NB Thru	29	29	29	29	29	7	16	3.3+2.7

Phasing Sequence[‡]

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
9:30	2
15:00	3
18:30	2
21:30	4

Saturday

Plan
4
2
5
2
4

Sunday

Time	Plan
0:15	4
6:30	2
21:00	4

Notes

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

→ Pedestrian signal

Cost is \$61.16 (\$54.12 + HST)

^{†:} Time for each direction includes amber and all red intervals

^{‡:} Start of first phase should be used as reference point for offset

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

Intersection:Main:WalkleySide:ConroyController:MS 3200TSD:5612

Author: Matthew Anderson Date: 01-Jun-2022

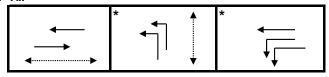
Existing Timing Plans[†]

Plan Ped Minimum Time

	Early AM	Off Peak	PM Peak	Night	Weekend	AM Peak	Walk	DW	A+R
	1	2	3	4	5	10			
Cycle	100	100	110	85	100	120			
Offset	90	47	20	Х	18	43			
EB Thru	44	45	51	36	45	55	10	20	3.7+2.7
WB Thru	65	69	79	54	69	80	-	-	3.7+2.7
NB Left (fp)	35	31	31	31	31	40	7	17	3.7+2.7
WB Left (fp)	21	24	28	18	24	25	-	-	3.3+2.9

Phasing Sequence[‡]

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
7:00	10
9:30	2
15:00	3
18:30	2
21:30	4

Saturday

Plan
4
2
5
2
4

Sunday

Time	Plan
0:15	4
6:30	2
21:00	4

Notes

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

 Intersection:
 Main:
 Conroy
 Side:
 St. Laurent

 Controller:
 ATC 3
 TSD:
 5612

 Author:
 Matthew Anderson
 Date:
 01-Jun-2022

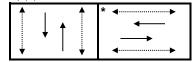
Existing Timing Plans[†]

Plan Ped Minimum Time

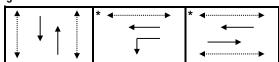
	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	100	90	95	90	90			
Offset	23	50	2	Х	50			
NB Thru	56	46	36	46	46	7	17	3.7+2.6
SB Thru	56	46	36	46	46	7	17	3.7+2.6
WB Left	•	-	15	-	•	•	-	3.3+2.4
EB Thru	44	44	44	44	44	7	30	3.3+3.6
WB Thru	44	44	59	44	44	7	30	3.3+3.6

Phasing Sequence[‡]

Plan: 1,2,4,5



Plan: 3



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
9:30	2
15:00	3
18:30	2
21:30	4

Saturday

Time	Plan
0:15	4
6:30	2
11:00	5
19:30	2
22:00	4

Sunday

Time	Plan
0:15	4
6:30	2
21:00	4

Notes

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

◄······► Pedestrian signal

APPENDIX J

Existing Synchro Analysis

		٠	→	•	•	+	•	4	†	/	/	↓	4
Traffic Volume (uph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (riph)	Lane Configurations	*	∱ %		7	∱ Љ		¥	ĵ.		7	î,	
	Traffic Volume (vph)	29		144	36		195	73		48	44		39
Storage Length (m)	Future Volume (vph)	29	918	144	36	1366	195	73	2	48	44	42	39
Storage Lanes	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Taper Length (m)	Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Lane Util. Factor	Storage Lanes	1		0	1		0	1		0	1		0
Ped Bike Factor	Taper Length (m)	25.0			30.0			25.0			30.0		
Fit Protected 0.980 0.980 0.980 0.980 0.980 0.950 0.	Lane Util. Factor	1.00	0.95	0.95	1.00		0.95	1.00	1.00	1.00	1.00	1.00	1.00
Filt Principated 0.950 0					1.00			1.00					
Satt Flow (proft) 1537 3187 0 1642 3166 0 1580 1347 0 1674 1625 0	Frt		0.980			0.981			0.855			0.928	
FILP Permitted 0.099	Flt Protected												
Satt Flow (perm) 160 3187 0 371 3166 0 1162 1347 0 1271 1625 0 Right Turn on Red 768	Satd. Flow (prot)		3187	0		3166	0		1347	0		1625	0
Right Turn on Red	Flt Permitted												
Satt Flow (RTOR)	Satd. Flow (perm)	160	3187		371	3166		1162	1347	0	1271	1625	-
Link Deped (k/h) 50 50 50 Link Distance (m) 402.0 171.1 100.3 281.0 Travel Time (s) 28.9 12.3 7.2 20.2 Confl. Bikes (#/hr) 1 5 5 1 1				Yes			Yes			Yes			Yes
Link Distance (m)	Satd. Flow (RTOR)												
Travel Time (s)	Link Speed (k/h)												
Confl. Beds. (#hr)	Link Distance (m)												
Confi. Bikes (#hr) Confi.			28.9			12.3			7.2			20.2	
Peak Hour Factor		1		5	5		1	1					1
Heavy Vehicles (%)													
Adj. Flow (vph) 32 1020 160 40 1518 217 81 2 53 49 47 43	Peak Hour Factor												
Shared Lane Traffic (%) Lane Group Flow (vph) 32 1180 0 40 1735 0 81 55 0 49 90 0 0	Heavy Vehicles (%)		4%	1%	3%	5%	1%		1%	12%	1%		
Lane Group Flow (vph) 32 1180 0 40 1735 0 81 55 0 49 90 0	Adj. Flow (vph)	32	1020	160	40	1518	217	81	2	53	49	47	43
Enter Blocked Intersection No No No No No No No													
Lane Alignment	Lane Group Flow (vph)		1180						55	-			-
Median Width(m) 5.0 5.0 3.5 3.5 1.5										No		No	
Link Offset(m) 0.0 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane 1.09		L NA		R NA	Left		Right	L NA		R NA	L NA		R NA
Crosswalk Width(m) 5.0 5.0 5.0 5.0 5.0													
Two way Left Turn Lane Headway Factor 1.09													
Headway Factor 1.09			5.0			5.0			5.0			5.0	
Turning Speed (k/h) 24 14 24 14 24 14 24 14 24 14 1													
Number of Detectors 1 2 1 2 1 2 1 2 Detector Template Left Thru Left Thru Left Thru Left Thru Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0			1.09			1.09			1.09			1.09	
Detector Template				14			14			14			14
Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0		-											
Trailing Detector (m) 0.0	•												
Detector 1 Position(m) 0.0													
Detector 1 Size(m) 6.1 1.8 6.1 1.8 6.1 1.8 6.1 1.8 Detector 1 Type CI+Ex													
Detector 1 Type CI+Ex													
Detector 1 Channel Detector 1 Extend (s) 0.0	\ ,												
Detector 1 Extend (s) 0.0		CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Queue (s) 0.0													
Detector 1 Delay (s) 0.0 1.8													
Detector 2 Position(m) 28.7 28.7 28.7 28.7 Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4		0.0			0.0			0.0			0.0		
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4	()												
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4			Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Permitted Phases 2 6 8 4		Perm			Perm			Perm			Perm		
			2			6			8			4	
Detector Phase 2 2 6 6 8 8 4 4													
	Detector Phase	2	2		6	6		8	8		4	4	

	•	→	\rightarrow	•	←	•	1	†	/	>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	64.0	64.0		64.0	64.0		36.0	36.0		36.0	36.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		36.0%	36.0%		36.0%	36.0%	
Maximum Green (s)	58.1	58.1		58.1	58.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	5	5		5	5		5	5		5	5	
Act Effct Green (s)	77.0	77.0		77.0	77.0		15.4	15.4		15.4	15.4	
Actuated g/C Ratio	0.77	0.77		0.77	0.77		0.15	0.15		0.15	0.15	
v/c Ratio	0.26	0.48		0.14	0.71		0.46	0.22		0.25	0.33	
Control Delay	14.9	7.3		5.6	6.9		44.5	11.1		37.4	26.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.9	7.3		5.6	6.9		44.5	11.1		37.4	26.1	
LOS	В	Α		Α	Α		D	В		D	С	
Approach Delay		7.5			6.9			31.0			30.1	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	1.4	34.1		1.0	29.1		13.8	0.3		8.1	9.6	
Queue Length 95th (m)	10.9	85.7		m2.6	#193.0		22.0	8.1		14.4	18.2	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	123	2459		285	2442		346	438		378	506	
Starvation Cap Reductn	0	0		0	14		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.26	0.48		0.14	0.71		0.23	0.13		0.13	0.18	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Offset: 94 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.71 Intersection Signal Delay: 9.1

Intersection Signal Delay: 9.1
Intersection Capacity Utilization 67.4%

Intersection LOS: A ICU Level of Service C

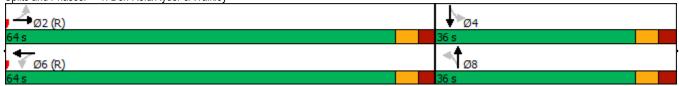
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Don Reid/Ryder & Walkley



	-	•	•	•	4	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	*	EDK	VVDL T	↑ ↑	NDL	NDIN
Traffic Volume (vph)	TT 929	90	74	TT 1537	1 66	60
Future Volume (vph)	929	90	74 74	1537	66	60
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)		20.0	65.0		30.0	0.0
Storage Lanes		1	1		1	1
Taper Length (m)	^ ^=	4.00	25.0	0.0-	30.0	4.00
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	3221	1498	1674	3191	1674	1483
Flt Permitted			0.264		0.950	
Satd. Flow (perm)	3221	1459	465	3191	1674	1462
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		73				67
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	14.0	4	4	14.4	3.5	2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	1%	1%	6%	1%	2%
	1032	100	82	1708	73	2% 67
Adj. Flow (vph)	1032	100	82	1708	13	07
Shared Lane Traffic (%)	4000	400	00	4700	70	07
Lane Group Flow (vph)	1032	100	82	1708	73	67
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
		CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Type	CI+Ex	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Channel	^ ^					
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			Cl+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2	. 0.111	. 0.111	6	8	· Oilli
Permitted Phases	۷.	2	6	U	U	8
	2			G	0	
Detector Phase	2	2	6	6	8	8
Switch Phase						

	→	\rightarrow	•	←	1	/	
ane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0	
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0	
Total Split (s)	71.0	71.0	71.0	71.0	29.0	29.0	
Total Split (%)	71.0%	71.0%	71.0%	71.0%	29.0%	29.0%	
Maximum Green (s)	65.0	65.0	65.0	65.0	23.0	23.0	
fellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7	
ost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	
_ead/Lag							
ead-Lag Optimize?							
/ehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	
Valk Time (s)	18.0	18.0			7.0	7.0	
Flash Dont Walk (s)	13.0	13.0			16.0	16.0	
Pedestrian Calls (#/hr)	5	5			5	5	
Act Effct Green (s)	80.0	80.0	80.0	80.0	11.6	11.6	
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.12	0.12	
/c Ratio	0.40	0.08	0.22	0.67	0.38	0.29	
Control Delay	3.0	0.9	6.6	8.5	44.1	12.2	
Queue Delay	0.0	0.0	0.0	0.5	0.0	0.0	
Total Delay	3.0	0.9	6.6	9.1	44.1	12.2	
.OS	Α	Α	Α	Α	D	В	
Approach Delay	2.8			8.9	28.8		
Approach LOS	A			Α	С		
Queue Length 50th (m)	16.6	0.3	3.0	59.1	12.5	0.0	
Queue Length 95th (m)	22.2	1.1	13.0	140.2	21.3	9.6	
nternal Link Dist (m)	147.1			145.1	104.6		
Turn Bay Length (m)		20.0	65.0		30.0		
Base Capacity (vph)	2576	1181	372	2552	385	387	
Starvation Cap Reductn	0	0	0	405	0	0	
Spillback Cap Reductn	0	0	0	73	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.40	0.08	0.22	0.80	0.19	0.17	
ntaraastian Cummanu							
ntersection Summary	Othor						
Area Type: Cycle Length: 100	Other						
Actuated Cycle Length: 100							
Offset: 88 (88%), Referenced	to nhase 2.⊏	RT and 6·1	MRTI Sta	rt of Green	1		
latural Cycle: 80	to pridat Z.E	ום anu 0.1	TE, Ola	it of Gleef	<u> </u>		
Control Type: Actuated-Coordi	inated						
laximum v/c Ratio: 0.67	inateu						
itersection Signal Delay: 7.6				In	tersection	1 OS: 4	
itersection Capacity Utilizatio	n 60 0%				CU Level of		
nalysis Period (min) 15	11 00.0 /0			- IC	O LEVELUI	OCI VICE D	
maryolo i ollou (IIIII) io							
Splits and Phases: 2: 160m	W of Conroy	& Walkley	/				
™ Ø2 (R)							
₩ WZ (K)							
/15							
₩ Ø6 (R)							★ ⁄Ø8
+ ₽0 (K)							1 20

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	^	7	ሻሻ	**	1,00	ሻሻ	7
Traffic Volume (vph)	789	257	230	954	14	666	448
Future Volume (vph)	789	257	230	954	14	666	448
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
		75.0	200.0			2	1
Storage Lanes			50.0			10.0	I
Taper Length (m)	0.05	1.00		0.05	0.05		1.00
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
Ped Bike Factor		0.98	0.99				0.98
Frt		0.850	0.050			0.050	0.850
Flt Protected	2121	4	0.950	0/0/		0.950	4
Satd. Flow (prot)	3161	1455	3066	3161	0	3186	1455
Flt Permitted			0.950			0.950	
Satd. Flow (perm)	3161	1419	3048	3161	0	3186	1430
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		286					345
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		10	10				3
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	7%	4%	7%	7%	2%	3%	4%
Adj. Flow (vph)	877	286	256	1060	16	740	498
Shared Lane Traffic (%)							
Lane Group Flow (vph)	877	286	256	1060	0	756	498
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0	11111	Lon	9.0	11111	10.5	1 (14) (
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	5.0			5.0		5.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
	1.09	1.09	24	1.09	1.09	24	1.09
Turning Speed (k/h)	0	14	24 1	2			
Number of Detectors	2 Thru	-	-	2 Thru	1	1	1 Diaht
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel	JI: LA			J. L A			
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2	i Giiii	1	6	i Giiii	8	i Giiii
Permitted Phases		2		U	8	0	8
r ennilleu Phases		2			Ŏ		
Detector Phase	2	2	1	6	8	8	8

	-	•	•	←	₹I	1	~
Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	55.0	55.0	25.0	80.0	40.0	40.0	40.0
Total Split (%)	45.8%	45.8%	20.8%	66.7%	33.3%	33.3%	33.3%
Maximum Green (s)	48.6	48.6	18.8	73.6	33.6	33.6	33.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	10	10			5	5	5
Act Effct Green (s)	53.7	53.7	15.1	75.0		32.2	32.2
Actuated g/C Ratio	0.45	0.45	0.13	0.62		0.27	0.27
v/c Ratio	0.62	0.36	0.66	0.54		0.88	0.78
Control Delay	28.7	4.0	58.3	14.2		55.1	21.6
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0
Total Delay	28.7	4.0	58.3	14.2		55.1	21.6
LOS	С	А	Е	В		Е	С
Approach Delay	22.6			22.8		41.8	
Approach LOS	С			С		D	
Queue Length 50th (m)	76.8	0.0	27.6	66.0		79.8	30.5
Queue Length 95th (m)	102.3	14.9	38.9	81.9		#103.7	72.2
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1413	792	480	1974		892	648
Starvation Cap Reductn	0	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.62	0.36	0.53	0.54		0.85	0.77

Other

Area Type: Cycle Length: 120

Actuated Cycle Length: 120

Offset: 43 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

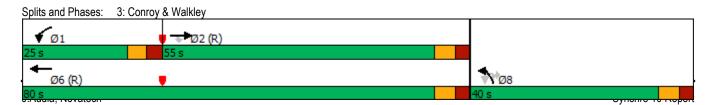
Intersection Signal Delay: 29.1 Intersection Capacity Utilization 68.2%

Intersection LOS: C ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



	۶	→	•	•	+	•	•	†	~	\		✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ		7	*	†	7	ሻ	ተ ተ		ሻ	↑ Љ	
Traffic Volume (vph)	4	59	76	60	38	39	177	1187	195	86	368	33
Future Volume (vph)	4	59	76	60	38	39	177	1187	195	86	368	33
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0	1000	50.0	55.0	1000	55.0	0.0	1000	0.0	110.0	1000	0.0
Storage Lanes	1		1	1		1	1		0.0	1		0.0
Taper Length (m)	40.0		'	40.0			10.0		· ·	25.0		J
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00	1.00	0.98	0.99	1.00	0.98	0.98	0.99	0.51	1.00	0.99	0.50
Frt	1.00		0.850	0.55		0.850	0.50	0.979		1.00	0.988	
Flt Protected	0.950		0.000	0.950		0.000	0.950	0.515		0.950	0.500	
Satd. Flow (prot)	1353	1695	1427	1409	1589	1351	1642	4623	0	1674	3198	0
Flt Permitted	0.730	1033	1421	0.714	1505	1331	0.494	4023	U	0.137	3130	U
	1035	1695	1395	1049	1589	1328	841	4623	0	241	3198	0
Satd. Flow (perm) Right Turn on Red	1035	1090	Yes	1049	1509	Yes	041	4023	Yes	241	3190	Yes
			84			36		45	165		13	168
Satd. Flow (RTOR)		Ε0	04			30						
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2	40	40	19.6	•	4.4	22.4	0.4	0.4	20.9	4.4
Confl. Peds. (#/hr)	6		13	13		6	14		21	21		14
Confl. Bikes (#/hr)												25
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	25%	5%	6%	20%	12%	12%	3%	2%	2%	1%	4%	3%
Adj. Flow (vph)	4	66	84	67	42	43	197	1319	217	96	409	37
Shared Lane Traffic (%)												
Lane Group Flow (vph)	4	66	84	67	42	43	197	1536	0	96	446	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
							_					

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	43.9	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	44.0	44.0	44.0	56.0	56.0		56.0	56.0	
Total Split (%)	44.0%	44.0%	44.0%	44.0%	44.0%	44.0%	56.0%	56.0%		56.0%	56.0%	
Maximum Green (s)	37.1	37.1	37.1	37.1	37.1	37.1	49.7	49.7		49.7	49.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0	30.0	30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	15	15	15	10	10	10	20	20		15	15	
Act Effct Green (s)	21.2	21.2	21.2	21.2	21.2	21.2	70.3	70.3		70.3	70.3	
Actuated g/C Ratio	0.21	0.21	0.21	0.21	0.21	0.21	0.70	0.70		0.70	0.70	
v/c Ratio	0.02	0.18	0.23	0.30	0.13	0.14	0.33	0.47		0.57	0.20	
Control Delay	22.2	28.6	6.9	32.3	27.3	10.1	13.3	11.0		34.4	8.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	22.2	28.6	6.9	32.3	27.3	10.1	13.3	11.0		34.4	8.9	
LOS	С	С	Α	С	С	В	В	В		С	Α	
Approach Delay		16.6			24.6			11.2			13.4	
Approach LOS		В			С			В			В	
Queue Length 50th (m)	0.6	11.0	1.2	11.5	6.9	1.1	9.5	30.0		5.9	9.7	
Queue Length 95th (m)	m2.2	16.4	9.5	16.9	11.2	7.1	40.6	84.6		#42.6	31.8	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	383	628	570	389	589	515	590	3262		169	2251	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.01	0.11	0.15	0.17	0.07	0.08	0.33	0.47		0.57	0.20	

Other

Area Type: Cycle Length: 100

Actuated Cycle Length: 100

Offset: 23 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57 Intersection Signal Delay: 12.8 Intersection Capacity Utilization 69.9%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Conroy & St. Laurent



	•	4	†	/	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1 >			4
Traffic Volume (vph)	75	40	71	24	69	176
Future Volume (vph)	75	40	71	24	69	176
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.953		0.966			
Flt Protected	0.968					0.986
Satd. Flow (prot)	1563	0	1495	0	0	1678
Flt Permitted	0.968					0.986
Satd. Flow (perm)	1563	0	1495	0	0	1678
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)		1				
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	7%	15%	15%	6%	4%
Adj. Flow (vph)	83	44	79	27	77	196
Shared Lane Traffic (%)						
Lane Group Flow (vph)	127	0	106	0	0	273
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
_						
Intersection Summary	Other					
Area Type:	Other					
Control Type: Unsignalized	04 40/			101		0
Intersection Capacity Utilization	on 34.1%			ICI	J Level of	Service A
Analysis Period (min) 15						

Synchro 10 Report J.Audia, Novatech

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	↑ 1≽		ሻ	↑ 1≽		ሻ	ĵ,		7	ĵ.	
Traffic Volume (vph)	27	1369	95	13	1400	70	137	0	65	87	99	50
Future Volume (vph)	27	1369	95	13	1400	70	137	0	65	87	99	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0	1000	0.0	50.0	1000	0.0	35.0	1000	0.0	30.0	1000	0.0
Storage Lanes	1		0.0	1		0.0	1		0.0	1		0.0
Taper Length (m)	25.0		0	30.0		· ·	25.0		· ·	30.0		·
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	0.50	1.00	1.00	0.50	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.990			0.993		1.00	0.850			0.949	
Flt Protected	0.950	0.550		0.950	0.933		0.950	0.000		0.950	0.543	
Satd. Flow (prot)	1674	3242	0	1674	3291	0	1674	1483	0	1674	1648	0
Flt Permitted	0.104	J2 4 2	U	0.105	3231	U	0.562	1400	U	0.710	1040	U
	183	3242	0	185	3291	0	989	1483	0	1251	1648	0
Satd. Flow (perm)	103	3242	Yes	100	3291		909	1403		1201	1040	Yes
Right Turn on Red		12	res		9	Yes		20	Yes		വ	res
Satd. Flow (RTOR)					_			38			23	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
Travel Time (s)		28.9			12.3		_	7.2			20.2	
Confl. Peds. (#/hr)	1		10	10		1	2					2
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	3%	3%	1%	2%	1%	1%	1%	2%	1%	1%	4%
Adj. Flow (vph)	30	1521	106	14	1556	78	152	0	72	97	110	56
Shared Lane Traffic (%)												
Lane Group Flow (vph)	30	1627	0	14	1634	0	152	72	0	97	166	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel		<u> </u>		<u> </u>	<u> </u>		<u> </u>	<u> </u>		<u> </u>	<u> </u>	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7		0.0	28.7		0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			Cl+Ex			CI+Ex	
Detector 2 Channel		OITLX			OITLX			OITEX			OITLX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
	reiiil	2		FEIIII	1NA 6		Fellii	NA 8		Fellii	1NA 4	
Protected Phases	0	2		G	O		0	0		1	4	
Permitted Phases	2	2		6 6			8	8		4		
Detector Phase	2	2		Ö	6		8	Ŏ		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max	(C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	77.1	77.1		77.1	77.1		20.8	20.8		20.8	20.8	
Actuated g/C Ratio	0.70	0.70		0.70	0.70		0.19	0.19		0.19	0.19	
v/c Ratio	0.23	0.72		0.11	0.71		0.81	0.23		0.41	0.50	
Control Delay	13.7	13.4		7.8	10.6		72.5	20.2		42.5	38.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	13.7	13.4		7.8	10.6		72.5	20.2		42.5	38.1	
LOS	В	В		Α	В		Е	С		D	D	
Approach Delay		13.4			10.6			55.7			39.7	
Approach LOS		В			В			Е			D	
Queue Length 50th (m)	1.9	89.5		8.0	58.0		29.0	5.6		17.0	25.4	
Queue Length 95th (m)	8.7	148.7		m1.3	51.4		46.0	15.3		28.6	39.9	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	128	2275		129	2308		267	429		338	463	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.23	0.72		0.11	0.71		0.57	0.17		0.29	0.36	

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

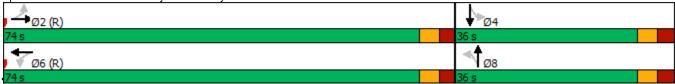
Intersection Signal Delay: 16.5 Intersection Capacity Utilization 76.6% Intersection LOS: B

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Don Reid/Ryder & Walkley



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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				
Lane Configurations	↑ ↑		ሻ	^	ሻ	7
Traffic Volume (vph)	1532	64	36	1418	49	41
Future Volume (vph)	1532	64	36	1418	49	41
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)		20.0	65.0		30.0	0.0
Storage Lanes		1	1		1	1
Taper Length (m)			25.0		30.0	,
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted			0.116		0.950	
Satd. Flow (perm)	3252	1441	200	3316	1658	1463
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		31				37
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	14.0	5	5	14.4	3.0	1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
	4%		3%	2%	2%	2%
Heavy Vehicles (%)		2%				
Adj. Flow (vph)	1702	71	40	1576	54	46
Shared Lane Traffic (%)	4700	7.4	40	4570	- 1	40
Lane Group Flow (vph)	1702	71	40	1576	54	46
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.1	0.0	0.0	0.1	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
\ <i>\</i>						
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2	I GIIII	I GIIII	6	8	1 61111
		2	6	U	U	0
Permitted Phases	^	2	6	^	0	8
Detector Phase	2	2	6	6	8	8
Switch Phase						

	-	\rightarrow	•	←	•	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0	
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0	
Total Split (s)	81.0	81.0	81.0	81.0	29.0	29.0	
Total Split (%)	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%	
Maximum Green (s)	75.0	75.0	75.0	75.0	23.0	23.0	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	0.0	0.0	0.0	0.0	0.0	0.0	
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	
Walk Time (s)	18.0	18.0			7.0	7.0	
Flash Dont Walk (s)	13.0	13.0			16.0	16.0	
Pedestrian Calls (#/hr)	5	5	00.4	00.4	5	5	
Act Effct Green (s)	90.4	90.4	90.4	90.4	11.1	11.1	
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.10	0.10	
v/c Ratio	0.64	0.06	0.24	0.58	0.33	0.26	
Control Delay	2.2	8.0	10.1	7.5	48.5	19.9	
Queue Delay	0.1	0.0	0.0	0.1	0.0	0.0	
Total Delay	2.3	0.8	10.1	7.7	48.5	19.9	
LOS	Α	Α	В	Α	D	В	
Approach Delay	2.3			7.7	35.3		
Approach LOS	Α			Α	D		
Queue Length 50th (m)	16.4	0.3	0.8	70.3	10.3	1.7	
Queue Length 95th (m)	22.8	m0.8	m9.0	130.1	18.8	10.2	
Internal Link Dist (m)	147.1			145.1	104.6		
Turn Bay Length (m)		20.0	65.0		30.0		
Base Capacity (vph)	2673	1190	164	2725	346	335	
Starvation Cap Reductn	85	0	0	283	0	0	
Spillback Cap Reductn	125	0	0	0	0	1	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.67	0.06	0.24	0.65	0.16	0.14	
ntersection Summary							
J	Other						
Cycle Length: 110							
Actuated Cycle Length: 110							
Offset: 25 (23%), Referenced to	o phase 2:E	BT and 6:\	NBTL, Sta	rt of Green	l		
Natural Cycle: 80							
Control Type: Actuated-Coordin	nated						
Maximum v/c Ratio: 0.64							
ntersection Signal Delay: 5.7				In	tersection	LOS: A	
ntersection Capacity Utilization	n 59.4%			IC	U Level of	Service B	
Analysis Period (min) 15							
n Volume for 95th percentile	queue is me	etered by u	ıpstream s	ignal.			
Splits and Phases: 2: 160m	W of Conroy	& Walkley	/				
▼ Ø2 (R)							
B1s							
₩ Ø6 (R)							↑ øs

Lane Configurations
Lane Configurations
Traffic Volume (vph)
Future Volume (vph)
Ideal Flow (ryhpi)
Storage Length (m)
Storage Length (m)
Taper Length (m)
Lane Util. Factor
Ped Bike Factor
Fit
Fit Protected 0.950 0.950 Satd. Flow (prot) 3221 1483 3185 3349 0 3248 1469 1469 1470 1481
Satd. Flow (prot) 3221 1483 3185 3349 0 3248 1469 Flt Permitted 0.950 0.950 0.950 0.950 0.950 0.950 1469 1469 1469 1483 3163 3349 0 3248 1418 Right Furn on Red Yes 2 2 76 2 330 1418 Right Turn on Red Yes 330 1418 Right Turn on Red Yes 330 1418 1418 1418 Right Turn on Red Yes 330 160 140 140 140 140 140 140 140 140 140 140 140 140 140 140 140
Fit Permitted
Satd. Flow (perm) 3221 1430 3163 3349 0 3248 1418 Right Turn on Red Yes Yes 330 348.5 330 348.5 348.
Right Turn on Red Yes Satd. Flow (RTOR) 452 330 Link Speed (k/h) 50 50 60 Link Distance (m) 169.1 271.7 348.5 Travel Time (s) 12.2 19.6 20.9 Confl. Peds. (#/hr) 22 22 16 Confl. Bikes (#/hr) 1 1 1 1 Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 0.90 Heavy Vehicles (%) 5% 2% 3% 1% 1% 1% 3% Adj. Flow (vph) 1266 487 580 1072 47 508 330 Shared Lane Traffic (%) 1266 487 580 1072 47 508 330 Enter Blocked Intersection No No No No No No No
Satid Flow (RTOR) 452 50 60
Link Speed (k/h) 50 50 60 Link Distance (m) 169.1 271.7 348.5 Travel Time (s) 12.2 19.6 20.9 Confl. Peds. (#/hr) 22 22 16 Confl. Bikes (#/hr) 1 1 1 Peak Hour Factor 0.90 <t< td=""></t<>
Link Distance (m)
Travel Time (s)
Confl. Peds. (#/hr)
Confl. Bikes (#/hr)
Peak Hour Factor 0.90 0.
Heavy Vehicles (%) 5% 2% 3% 1% 1% 1% 3% Adj. Flow (vph) 1266 487 580 1072 47 508 330 Shared Lane Traffic (%) Lane Group Flow (vph) 1266 487 580 1072 0 555 330 Enter Blocked Intersection No No No No No No No
Adj. Flow (vph) 1266 487 580 1072 47 508 330 Shared Lane Traffic (%) Lane Group Flow (vph) 1266 487 580 1072 0 555 330 Enter Blocked Intersection No
Shared Lane Traffic (%) Lane Group Flow (vph) 1266 487 580 1072 0 555 330
Lane Group Flow (vph) 1266 487 580 1072 0 555 330 Enter Blocked Intersection No
Enter Blocked Intersection No No <th< td=""></th<>
Lane Alignment Left R NA Left Left R NA L NA R NA Median Width(m) 7.0 9.0 10.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Two way Left Turn Lane Two way Left Turn Lane Turning Speed (k/h) 14 24 14 24 14 Number of Detectors 2 1 1 2 1 <t< td=""></t<>
Median Width(m) 7.0 9.0 10.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00
Median Width(m) 7.0 9.0 10.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00
Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00 1.00
Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00 1.00 1.00 1.00 1.00
Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 1.09 1.09 1.09
Headway Factor 1.09
Turning Speed (k/h) 14 24 14 24 14 Number of Detectors 2 1 1 2 1 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0
Number of Detectors 2 1 1 2 1 1 1 Detector Template Thru Right Left Thru Left Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 <
Detector Template Thru Right Left Thru Left Left Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1
Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0
Trailing Detector (m) 0.0
Detector 1 Position(m) 0.0 1.8 6.1 8.1 9.0 9.0 9.0 9.0
Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 6.1 6.1 6.1 6.1 Detector 1 Type CI+Ex
Detector 1 Type CI+Ex
Detector 1 Channel Detector 1 Extend (s) 0.0
Detector 1 Extend (s) 0.0
Detector 1 Queue (s) 0.0
Detector 1 Delay (s) 0.0
Detector 2 Position(m) 28.7 Detector 2 Size(m) 1.8 Detector 2 Type CI+Ex Detector 2 Channel
Detector 2 Size(m) 1.8 1.8 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel CI+Ex CI+Ex
Detector 2 Type CI+Ex CI+Ex Detector 2 Channel
Detector 2 Channel
Detector 2 Extend (s) 0.0 0.0
Turn Type NA Perm Prot NA Perm Prot Perm
Protected Phases 2 1 6 8
Permitted Phases 2 8 8
Detector Phase 2 2 1 6 8 8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	51.0	51.0	28.0	79.0	31.0	31.0	31.0
Total Split (%)	46.4%	46.4%	25.5%	71.8%	28.2%	28.2%	28.2%
Maximum Green (s)	44.6	44.6	21.8	72.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	46.1	46.1	22.0	74.3		22.9	22.9
Actuated g/C Ratio	0.42	0.42	0.20	0.68		0.21	0.21
v/c Ratio	0.94	0.57	0.91	0.47		0.82	0.59
Control Delay	40.6	7.6	62.9	9.6		52.5	8.8
Queue Delay	0.0	0.2	0.0	0.0		0.0	0.0
Total Delay	40.6	7.8	62.9	9.6		52.5	8.8
LOS	D	Α	Е	Α		D	Α
Approach Delay	31.5			28.3		36.2	
Approach LOS	С			С		D	
Queue Length 50th (m)	89.4	0.0	58.4	50.7		53.3	0.0
Queue Length 95th (m)	#169.0	39.9	#87.2	63.4		70.9	21.8
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1349	861	644	2263		726	573
Starvation Cap Reductn	0	57	0	0		0	0
Spillback Cap Reductn	0	0	0	42		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.94	0.61	0.90	0.48		0.76	0.58

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94 Intersection Signal Delay: 31.2

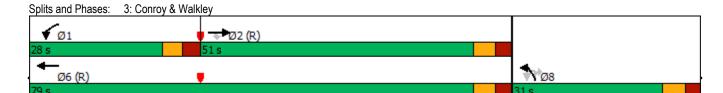
Intersection Capacity Utilization 81.9%

Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	*	7	7	^	7	7	ᡮ ᡮᡗ∌		7	Φβ	
Traffic Volume (vph)	17	67	306	222	28	105	35	564	79	37	1064	8
Future Volume (vph)	17	67	306	222	28	105	35	564	79	37	1064	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	1.00		0.99		0.99		0.99	1.00	
Frt			0.850			0.850		0.982			0.999	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4617	0	1537	3309	0
Flt Permitted	0.737			0.565			0.127			0.349		
Satd. Flow (perm)	1297	1728	1461	974	1618	1463	219	4617	0	560	3309	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			115			68		28			1	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	2		4	4		2	8		12	12		8
Confl. Bikes (#/hr)									4			11
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	3%	2%	3%	10%	2%	3%	2%	8%	10%	2%	12%
Adj. Flow (vph)	19	74	340	247	31	117	39	627	88	41	1182	9
Shared Lane Traffic (%)			0.0		•			V		• • •		
Lane Group Flow (vph)	19	74	340	247	31	117	39	715	0	41	1191	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5		20.0	7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane		0.0										
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	• •
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel	OI · EX	OI · EX	OITEX	OI · EX	OI · LX	OI LX	OI LX	OI LX		OI · EX	OI LX	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7	0.0	0.0	28.7	0.0	0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OI+LX			OITEX			OI*LX			OITEX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	reiiii	NA 4	reilli		NA 8	reiiii	reilli	NA 2		reilli	NA 6	
	A	4	4	3	Ŏ	0	0	2		•	Ö	
Permitted Phases	4	4	4	8	0	8	2	2		6		
Detector Phase	4	4	4	3	8	8	Z	Z		6	6	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	22.3	22.3	22.3	38.5	37.3	37.3	44.5	44.5		44.5	44.5	
Actuated g/C Ratio	0.23	0.23	0.23	0.41	0.39	0.39	0.47	0.47		0.47	0.47	
v/c Ratio	0.06	0.18	0.79	0.54	0.05	0.19	0.38	0.33		0.16	0.77	
Control Delay	23.4	26.4	34.3	22.8	14.4	7.6	37.3	17.5		21.4	27.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	23.4	26.4	34.3	22.8	14.4	7.6	37.3	17.5		21.4	27.8	
LOS	С	С	С	С	В	Α	D	В		С	С	
Approach Delay		32.5			17.6			18.6			27.6	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	2.6	10.2	37.0	29.3	3.3	5.2	4.0	24.9		3.7	82.4	
Queue Length 95th (m)	6.0	16.0	51.8	32.7	5.9	10.8	#20.4	44.7		13.2	#162.8	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	506	674	640	460	887	833	102	2177		262	1550	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.04	0.11	0.53	0.54	0.03	0.14	0.38	0.33		0.16	0.77	

Other

Area Type: Cycle Length: 95

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

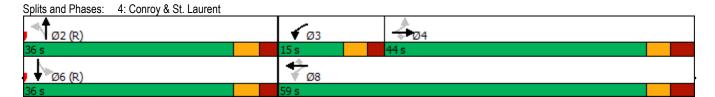
Maximum v/c Ratio: 0.79 Intersection Signal Delay: 24.5 Intersection Capacity Utilization 81.8%

Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1}			ર્ની
Traffic Volume (vph)	22	56	107	77	151	51
Future Volume (vph)	22	56	107	77	151	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.903		0.943			
Flt Protected	0.986					0.964
Satd. Flow (prot)	1479	0	1639	0	0	1588
Flt Permitted	0.986					0.964
Satd. Flow (perm)	1479	0	1639	0	0	1588
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				2	2	
Confl. Bikes (#/hr)		1		4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	24	62	119	86	168	57
Shared Lane Traffic (%)						
Lane Group Flow (vph)	86	0	205	0	0	225
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	<u> </u>	0.0	, i		0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Canacity Litilizat	ion 37 7%			IC	ا ا ا مرا ا	Service A

Intersection Capacity Utilization 37.7% Analysis Period (min) 15

ICU Level of Service A

Synchro 10 Report J.Audia, Novatech

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	♦ %		7	∱ }		ř	f)		7	f)	
Traffic Volume (vph)	27	1369	95	13	1400	70	137	0	65	87	99	50
Future Volume (vph)	27	1369	95	13	1400	70	137	0	65	87	99	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0			30.0			25.0			30.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			1.00		1.00				1.00	
Frt		0.990			0.993			0.850			0.949	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	3242	0	1674	3291	0	1674	1483	0	1674	1648	0
Flt Permitted	0.104			0.105			0.562			0.710		
Satd. Flow (perm)	183	3242	0	185	3291	0	989	1483	0	1251	1648	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12			9			38			23	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
Travel Time (s)		28.9			12.3			7.2			20.2	
Confl. Peds. (#/hr)	1		10	10		1	2					2
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	3%	3%	1%	2%	1%	1%	1%	2%	1%	1%	4%
Adj. Flow (vph)	30	1521	106	14	1556	78	152	0	72	97	110	56
Shared Lane Traffic (%)												
Lane Group Flow (vph)	30	1627	0	14	1634	0	152	72	0	97	166	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	77.1	77.1		77.1	77.1		20.8	20.8		20.8	20.8	
Actuated g/C Ratio	0.70	0.70		0.70	0.70		0.19	0.19		0.19	0.19	
v/c Ratio	0.23	0.72		0.11	0.71		0.81	0.23		0.41	0.50	
Control Delay	13.7	13.4		7.8	8.5		72.5	20.2		42.5	38.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	13.7	13.4		7.8	8.5		72.5	20.2		42.5	38.1	
LOS	В	В		Α	Α		Е	С		D	D	
Approach Delay		13.4			8.5			55.7			39.7	
Approach LOS		В			Α			Е			D	
Queue Length 50th (m)	1.9	89.5		0.6	45.3		29.0	5.6		17.0	25.4	
Queue Length 95th (m)	8.7	148.7		m1.2	50.2		46.0	15.3		28.6	39.9	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	128	2275		129	2308		267	429		338	463	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.23	0.72		0.11	0.71		0.57	0.17		0.29	0.36	

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 15.6 Intersection LOS: B Intersection Capacity Utilization 76.6% ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				
Lane Configurations	↑ ↑		ሻ	^	ሻ	7
Traffic Volume (vph)	1532	64	36	1418	49	41
Future Volume (vph)	1532	64	36	1418	49	41
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)		20.0	65.0		30.0	0.0
Storage Lanes		1	1		1	1
Taper Length (m)			25.0		30.0	,
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted			0.116		0.950	
Satd. Flow (perm)	3252	1441	200	3316	1658	1463
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		31				37
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	14.0	5	5	14.4	3.0	1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
	4%		3%	2%	2%	2%
Heavy Vehicles (%)		2%				
Adj. Flow (vph)	1702	71	40	1576	54	46
Shared Lane Traffic (%)	4700	7.4	40	4570	- 1	40
Lane Group Flow (vph)	1702	71	40	1576	54	46
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.1	0.0	0.0	0.1	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
\ <i>\</i>						
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2	I GIIII	I GIIII	6	8	1 61111
		2	6	U	U	0
Permitted Phases	^	2	6	^	0	8
Detector Phase	2	2	6	6	8	8
Switch Phase						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0
Total Split (s)	81.0	81.0	81.0	81.0	29.0	29.0
Total Split (%)	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%
Maximum Green (s)	75.0	75.0	75.0	75.0	23.0	23.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	0.0	0.0	0.0	0.0	0.0	0.0
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
	18.0	18.0	O-IVIAX	O-IVIAX	7.0	7.0
Walk Time (s) Flash Dont Walk (s)	18.0	13.0			16.0	16.0
()						
Pedestrian Calls (#/hr)	5	5	00.4	00.4	5	5
Act Effct Green (s)	90.4	90.4	90.4	90.4	11.1	11.1
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.10	0.10
v/c Ratio	0.64	0.06	0.24	0.58	0.33	0.26
Control Delay	2.2	0.8	9.2	6.3	48.5	19.9
Queue Delay	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay	2.3	0.8	9.2	6.7	48.5	19.9
LOS	Α	Α	Α	Α	D	В
Approach Delay	2.2			6.8	35.3	
Approach LOS	Α			Α	D	
Queue Length 50th (m)	16.4	0.3	1.5	46.2	10.3	1.7
Queue Length 95th (m)	22.8	m0.8	9.3	110.7	18.8	10.2
Internal Link Dist (m)	147.1			145.1	104.6	
Turn Bay Length (m)		20.0	65.0		30.0	
Base Capacity (vph)	2673	1190	164	2725	346	335
Starvation Cap Reductn	85	0	0	619	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.06	0.24	0.75	0.16	0.14
	0.00	0.00	0.27	0.70	0.10	0.17
Intersection Summary						
Area Type:	Other					
Cycle Length: 110						
Actuated Cycle Length: 110						
Offset: 25 (23%), Referenced to	phase 2:El	BT and 6:\	VBTL, Sta	rt of Green	1	
Natural Cycle: 80	·					
Control Type: Actuated-Coording	nated					
Maximum v/c Ratio: 0.64						
Intersection Signal Delay: 5.3				In	tersection	LOS: A
Intersection Capacity Utilization	59.4%				CU Level of	
Analysis Period (min) 15	1 00.170				201010	COI VICO D
m Volume for 95th percentile	allalla is ma	stared by I	inetroam e	ianal		
iii volume for 55th percentile	queue is inic	stered by t	ipstream s	ngriai.		
Splits and Phases: 2: 160m	W of Conroy	2 Malklo	,			
Spills and Friases. 2. 100m	vv or Cornoy	& walkley	/			
▼ Ø2 (R)						
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* ac (0)						
Ø6 (R)						

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	44	7	ሻሻ	^	.,,,,,	ሻሻ	7
Traffic Volume (vph)	1139	438	522	965	42	457	297
Future Volume (vph)	1139	438	522	965	42	457	297
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		1	2			2	1
Taper Length (m)			50.0			10.0	1
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
Ped Bike Factor	0.53	0.96	0.99	0.95	0.33	0.91	0.96
Frt		0.850	0.99				0.850
		0.000	0.950			0.950	0.000
Fit Protected	2004	1,100		2240	0		1460
Satd. Flow (prot)	3221	1483	3185	3349	0	3248	1469
Flt Permitted	2004	1400	0.950	2240	0	0.950	4445
Satd. Flow (perm)	3221	1426	3161	3349	0	3248	1415
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)	=-	418				20	330
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		22	22				16
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	2%	3%	1%	1%	1%	3%
Adj. Flow (vph)	1266	487	580	1072	47	508	330
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1266	487	580	1072	0	555	330
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	J.J			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	14	24	14
Number of Detectors	2	1	1	2	1	1	1
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
•	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Leading Detector (m)		0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0						
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6	2	8	2
Permitted Phases	_	0			8		8
		,					
Detector Phase	2	2	1	6	8	8	8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	58.0	58.0	31.0	89.0	31.0	31.0	31.0
Total Split (%)	48.3%	48.3%	25.8%	74.2%	25.8%	25.8%	25.8%
Maximum Green (s)	51.6	51.6	24.8	82.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	53.1	53.1	24.2	83.5		23.7	23.7
Actuated g/C Ratio	0.44	0.44	0.20	0.70		0.20	0.20
v/c Ratio	0.89	0.56	0.90	0.46		0.87	0.61
Control Delay	40.2	6.8	65.3	9.1		61.5	9.6
Queue Delay	4.3	0.0	0.0	0.0		0.0	0.0
Total Delay	44.5	6.8	65.3	9.1		61.5	9.6
LOS	D	Α	Е	Α		Е	Α
Approach Delay	34.0			28.8		42.2	
Approach LOS	С			С		D	
Queue Length 50th (m)	133.2	8.8	63.4	50.7		60.0	0.0
Queue Length 95th (m)	#174.6	34.1	#90.2	62.5		#83.0	23.6
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1426	864	659	2331		665	552
Starvation Cap Reductn	108	8	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.96	0.57	0.88	0.46		0.83	0.60

Other

Area Type: Cycle Length: 120

Actuated Cycle Length: 120

Offset: 20 (17%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90 Intersection Signal Delay: 33.7

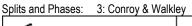
Intersection Capacity Utilization 81.9%

Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	*	7	7	^	7	7	ᡮ ᡮᡗ∌		7	∱ ∱	
Traffic Volume (vph)	17	67	306	222	28	105	35	564	79	37	1064	8
Future Volume (vph)	17	67	306	222	28	105	35	564	79	37	1064	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	1.00		0.99		0.99		0.99	1.00	
Frt			0.850			0.850		0.982			0.999	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4617	0	1537	3309	0
FIt Permitted	0.737			0.565			0.127			0.349		
Satd. Flow (perm)	1297	1728	1461	974	1618	1463	219	4617	0	560	3309	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			115			68		28			1	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	2		4	4		2	8		12	12		8
Confl. Bikes (#/hr)									4			11
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	3%	2%	3%	10%	2%	3%	2%	8%	10%	2%	12%
Adj. Flow (vph)	19	74	340	247	31	117	39	627	88	41	1182	9
Shared Lane Traffic (%)												
Lane Group Flow (vph)	19	74	340	247	31	117	39	715	0	41	1191	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4	-	4	8		8	2			6	-	
Detector Phase	4	4	4	3	8	8	2	2		6	6	
		•	•			•	_	_			•	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	22.3	22.3	22.3	38.5	37.3	37.3	44.5	44.5		44.5	44.5	
Actuated g/C Ratio	0.23	0.23	0.23	0.41	0.39	0.39	0.47	0.47		0.47	0.47	
v/c Ratio	0.06	0.18	0.79	0.54	0.05	0.19	0.38	0.33		0.16	0.77	
Control Delay	23.4	26.4	34.3	22.8	14.4	7.6	37.3	17.5		21.4	27.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	23.4	26.4	34.3	22.8	14.4	7.6	37.3	17.5		21.4	27.8	
LOS	С	С	С	С	В	Α	D	В		С	С	
Approach Delay		32.5			17.6			18.6			27.6	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	2.6	10.2	37.0	29.3	3.3	5.2	4.0	24.9		3.7	82.4	
Queue Length 95th (m)	6.0	16.0	51.8	32.7	5.9	10.8	#20.4	44.7		13.2	#162.8	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	506	674	640	460	887	833	102	2177		262	1550	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.04	0.11	0.53	0.54	0.03	0.14	0.38	0.33		0.16	0.77	

Other

Area Type: Cycle Length: 95

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.79 Intersection Signal Delay: 24.5

Intersection LOS: C Intersection Capacity Utilization 81.8% ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



	•	1	†	~	/	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1>			सी
Traffic Volume (vph)	22	56	107	77	151	51
Future Volume (vph)	22	56	107	77	151	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.903		0.943			
Flt Protected	0.986					0.964
Satd. Flow (prot)	1479	0	1639	0	0	1588
Flt Permitted	0.986					0.964
Satd. Flow (perm)	1479	0	1639	0	0	1588
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				2	2	
Confl. Bikes (#/hr)		1		4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	24	62	119	86	168	57
Shared Lane Traffic (%)						
Lane Group Flow (vph)	86	0	205	0	0	225
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	<u> </u>	0.0	, i		0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Cantral Tunas Hasimadinad						

Control Type: Unsignalized Intersection Capacity Utilization 37.7% Analysis Period (min) 15

ICU Level of Service A

Synchro 10 Report J.Audia, Novatech

APPENDIX K

Background Synchro Analysis

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ተ ኈ		*	∱ Љ		ሻ	ĥ		ሻ	f.	
Traffic Volume (vph)	29	925	164	41	1410	195	78	7	53	49	47	39
Future Volume (vph)	29	925	164	41	1410	195	78	7	53	49	47	39
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0		•	30.0		•	25.0		•	30.0		•
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.99	1.00	1.00	0.99	1.00
Frt		0.977			0.982		1.00	0.867			0.932	
Flt Protected	0.950	0.0		0.950	0.002		0.950	0.00.		0.950	0.002	
Satd. Flow (prot)	1537	3177	0	1642	3169	0	1580	1378	0	1674	1633	0
Flt Permitted	0.121	0111	•	0.241	0100	•	0.701	1010	•	0.718	1000	•
Satd. Flow (perm)	196	3177	0	416	3169	0	1165	1378	0	1265	1633	0
Right Turn on Red	100	0111	Yes	110	0100	Yes	1100	1070	Yes	1200	1000	Yes
Satd. Flow (RTOR)		34	100		26	100		53	100		39	100
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
Travel Time (s)		28.9			12.3			7.2			20.2	
Confl. Peds. (#/hr)	1	20.3	5	5	12.5	1	1	1.2			20.2	1
Confl. Bikes (#/hr)			3	3					1			·
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	10%	4%	1%	3%	5%	1%	7%	1%	12%	1.00	1.00	1.00
Adj. Flow (vph)	29	925	164	41	1410	195	78	7	53	49	47	39
Shared Lane Traffic (%)												
Lane Group Flow (vph)	29	1089	0	41	1605	0	78	60	0	49	86	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6	-		8			4		
Detector Phase	2	2		6	6		8	8		4	4	
_ 5.00.0	_	_										

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	64.0	64.0		64.0	64.0		36.0	36.0		36.0	36.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		36.0%	36.0%		36.0%	36.0%	
Maximum Green (s)	58.1	58.1		58.1	58.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	5	5		5	5		5	5		5	5	
Act Effct Green (s)	77.1	77.1		77.1	77.1		15.2	15.2		15.2	15.2	
Actuated g/C Ratio	0.77	0.77		0.77	0.77		0.15	0.15		0.15	0.15	
v/c Ratio	0.19	0.44		0.13	0.66		0.44	0.24		0.26	0.30	
Control Delay	11.1	6.8		4.5	5.0		44.0	12.8		37.6	23.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	11.1	6.8		4.5	5.1		44.0	12.8		37.6	23.0	
LOS	В	Α		Α	Α		D	В		D	С	
Approach Delay		6.9			5.0			30.4			28.3	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	1.2	29.4		1.0	26.6		13.3	1.1		8.1	7.7	
Queue Length 95th (m)	8.3	75.7		m2.2	29.6		21.3	9.1		14.5	16.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	151	2456		320	2448		347	447		376	514	
Starvation Cap Reductn	0	0		0	18		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.19	0.44		0.13	0.66		0.22	0.13		0.13	0.17	

Other

Area Type: Cycle Length: 100

Actuated Cycle Length: 100

Offset: 94 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

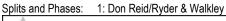
Maximum v/c Ratio: 0.66

Intersection Signal Delay: 7.9 Intersection Capacity Utilization 69.0%

Intersection LOS: A ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.





	-	•	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
		EDR	VVDL		INDL T	INDIX 7
Lane Configurations	^			^		
Traffic Volume (vph)	947	90	74 74	1590	66	60
Future Volume (vph)	947	90	74	1590	66	60
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)		20.0	65.0		30.0	0.0
Storage Lanes		1	1		1	1
Taper Length (m)			25.0	-	30.0	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	3221	1498	1674	3191	1674	1483
Flt Permitted			0.292		0.950	
Satd. Flow (perm)	3221	1459	514	3191	1674	1462
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		72				60
Link Speed (k/h)	50			50	50	- 30
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	14.5	4	4	14.4	3.5	2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
	1.00 5%			1.00	1.00	1.00
Heavy Vehicles (%)		1%	1%			
Adj. Flow (vph)	947	90	74	1590	66	60
Shared Lane Traffic (%)	0.4-	22	- 1	4500	22	22
Lane Group Flow (vph)	947	90	74	1590	66	60
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)						
()	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			Cl+Ex		
Detector 2 Channel	J. L.			J. LA		
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases		I CIIII	I CIIII			I CIIII
	2	0		6	8	0
Permitted Phases		2	6		_	8
Detector Phase	2	2	6	6	8	8
Switch Phase						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0	
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0	
Total Split (s)	71.0	71.0	71.0	71.0	29.0	29.0	
Total Split (%)	71.0%	71.0%	71.0%	71.0%	29.0%	29.0%	
Maximum Green (s)	65.0	65.0	65.0	65.0	23.0	23.0	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	
Walk Time (s)	18.0	18.0			7.0	7.0	
Flash Dont Walk (s)	13.0	13.0			16.0	16.0	
Pedestrian Calls (#/hr)	5	5			5	5	
Act Effct Green (s)	80.2	80.2	80.2	80.2	11.3	11.3	
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.11	0.11	
v/c Ratio	0.37	0.08	0.18	0.62	0.35	0.28	
Control Delay	3.0	0.8	5.8	7.5	43.6	12.5	
Queue Delay	0.0	0.0	0.0	0.4	0.0	0.0	
Total Delay	3.0	0.8	5.8	8.0	43.6	12.5	
LOS	A	A	A	A	D	В	
Approach Delay	2.8	- , ,		7.9	28.8		
Approach LOS	A			A	C		
Queue Length 50th (m)	14.5	0.2	2.6	49.7	11.3	0.0	
Queue Length 95th (m)	21.4	0.9	11.2	120.3	19.6	8.9	
Internal Link Dist (m)	147.1			145.1	104.6		
Turn Bay Length (m)		20.0	65.0		30.0		
Base Capacity (vph)	2583	1184	412	2560	385	382	
Starvation Cap Reductn	0	0	0	444	0	0	
Spillback Cap Reductn	0	0	0	26	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.37	0.08	0.18	0.75	0.17	0.16	
latana atian Communi							
Intersection Summary	Other						
Area Type:	Other						
Cycle Length: 100 Actuated Cycle Length: 100							
Offset: 88 (88%), Referenced	to phase 2.⊏	RT and 6.1	NRTI Sta	rt of Groon	,		
Natural Cycle: 75	to priase z.E	ום מווט ט. /	voil, sia	it of Greet	I 		
Natural Cycle. 75 Control Type: Actuated-Coord	linated						
Control Type: Actuated-Coord Maximum v/c Ratio: 0.62	maleu						
Intersection Signal Delay: 6.9				ln.	tersection	1 OS: V	
ntersection Capacity Utilization	n 61 50/				CU Level of		
ntersection Capacity Utilizatio Analysis Period (min) 15	лг 01.3%			IC	o Level of	Service B	
niaiyəiə i elibu (IIIIII) 10							
Splits and Phases: 2: 160m	W of Conroy	& Walkley	/				
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71 s							
+							4
Ø6 (R)							™ ø8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	^	₹	ሻሻ	*	1100	ሻሻ	₩ M
Traffic Volume (vph)	794	267	246	991	14	683	462
Future Volume (vph)	794	267	246	991	14	683	462
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		1 1	200.0			2	1
Taper Length (m)			50.0			10.0	
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
Ped Bike Factor	0.95	0.98	0.97	0.95	0.95	0.97	0.98
			0.99				0.850
Frt		0.850	0.050			0.050	0.830
Fit Protected	0404	4.455	0.950	0404	^	0.950	4.455
Satd. Flow (prot)	3161	1455	3066	3161	0	3186	1455
Flt Permitted	6464	4440	0.950	0/0/		0.950	4 400
Satd. Flow (perm)	3161	1419	3046	3161	0	3186	1430
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		267					358
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		10	10				3
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	7%	4%	7%	7%	2%	3%	4%
Adj. Flow (vph)	794	267	246	991	14	683	462
Shared Lane Traffic (%)							
Lane Group Flow (vph)	794	267	246	991	0	697	462
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	1.03	24	1.00	1.03	24	1.03
Number of Detectors	2	14	1	2	14	1	14
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0 1.8			0.0 1.8		6.1	0.0
Detector 1 Size(m)		6.1	6.1		6.1		6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel	^ ^	^ ^	^ ^	^ ^	^ ^	^ ^	^ ^
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	
Permitted Phases		2			8		8
Detector Phase	2	2	1	6	8	8	8
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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	55.0	55.0	25.0	80.0	40.0	40.0	40.0
Total Split (%)	45.8%	45.8%	20.8%	66.7%	33.3%	33.3%	33.3%
Maximum Green (s)	48.6	48.6	18.8	73.6	33.6	33.6	33.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	Z.1	0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
()				0.4		0.4	0.4
Lead/Lag Ontimize?	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes	2.0	2.0	2.0	2.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	10	10			5	5	5
Act Effct Green (s)	55.1	55.1	14.8	76.1		31.1	31.1
Actuated g/C Ratio	0.46	0.46	0.12	0.63		0.26	0.26
v/c Ratio	0.55	0.34	0.65	0.49		0.84	0.73
Control Delay	26.3	3.9	58.0	13.1		52.4	16.5
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0
Total Delay	26.3	3.9	58.0	13.1		52.4	16.5
LOS	С	Α	Е	В		D	В
Approach Delay	20.7			22.0		38.1	
Approach LOS	С			С		D	
Queue Length 50th (m)	65.7	0.0	26.5	58.3		72.6	18.2
Queue Length 95th (m)	90.0	14.6	37.4	74.5		92.2	54.7
Internal Link Dist (m)	145.1		• • • • • • • • • • • • • • • • • • • •	247.7		324.5	•
Turn Bay Length (m)	110.1	75.0	200.0			021.0	
Base Capacity (vph)	1450	795	480	2004		892	658
Starvation Cap Reductn	0	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductin	0	0	0	0		0	0
Reduced v/c Ratio	0.55	0.34	0.51	0.49		0.78	0.70
Reduced V/C Ratio	0.55	0.34	0.51	0.49		0.76	0.70
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 120							
Offset: 43 (36%), Referenced	to phase 2:FI	BT and 6·V	VBT Start	of Green			
Natural Cycle: 85	to pridoo 2.El	Di ana o.v	ibi, otari	01 010011			
Control Type: Actuated-Coord	linated						
Maximum v/c Ratio: 0.84	iiiateu						
	0			ln:	toropotion	00.0	
Intersection Signal Delay: 27.					tersection		.
Intersection Capacity Utilization	on 69.2%			IC	U Level of	Service C	•
Analysis Period (min) 15							
0.111 1.101 0.00	0.147.11.1						
Splits and Phases: 3: Conro	oy & Walkley						
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√ Ø1	₩ Ø2	(R)					
25 s	55 s						
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Ø6 (R)	•						

Synchro 10 Report J.Audia, Novatech

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	*	7	7	^	7	7	↑ ↑₽		7	∱ β	
Traffic Volume (vph)	6	65	81	63	43	25	192	1187	199	87	370	53
Future Volume (vph)	6	65	81	63	43	25	192	1187	199	87	370	53
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	0.99		0.98	0.98	0.99		1.00	0.99	
Frt			0.850			0.850		0.978			0.981	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1353	1695	1427	1409	1589	1351	1642	4617	0	1674	3167	0
FIt Permitted	0.729			0.715			0.505			0.166		
Satd. Flow (perm)	1033	1695	1395	1050	1589	1328	859	4617	0	291	3167	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			81			36		46			22	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	6		13	13		6	14		21	21		14
Confl. Bikes (#/hr)	•					-						25
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	25%	5%	6%	20%	12%	12%	3%	2%	2%	1%	4%	3%
Adj. Flow (vph)	6	65	81	63	43	25	192	1187	199	87	370	53
Shared Lane Traffic (%)	· ·	00	O1	00	10	20	102	1101	100	O1	010	00
Lane Group Flow (vph)	6	65	81	63	43	25	192	1386	0	87	423	0
Enter Blocked Intersection	No	No	No	No	No							
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)	LIVA	3.5	IX IN/A	LINA	3.5	IX IN/A	LGIL	7.0	rtigiit	LINA	6.0	IX IN/X
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane		5.0			3.0			0.0			5.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	1.03	14	24	1.03	1.03	24	1.03	1.03	24	1.03	1.03
Number of Detectors	1	2	1	1	2	1	1	2	14	1	2	14
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)												
Detector 1 Size(m)	0.0 6.1	0.0 1.8	0.0 6.1	0.0 6.1	0.0 1.8	0.0 6.1	0.0 6.1	0.0 1.8		0.0 6.1	0.0 1.8	
Detector 1 Type	CI+Ex		CI+Ex	CI+Ex								
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	_	0.0	_	_	0.0	_	_	0.0		_	0.0	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases Detector Phase	4 4	4	4 4	8 8	8	8	2	2		6 6	6	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	43.9	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	44.0	44.0	44.0	56.0	56.0		56.0	56.0	
Total Split (%)	44.0%	44.0%	44.0%	44.0%	44.0%	44.0%	56.0%	56.0%		56.0%	56.0%	
Maximum Green (s)	37.1	37.1	37.1	37.1	37.1	37.1	49.7	49.7		49.7	49.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0	30.0	30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	15	15	15	10	10	10	20	20		15	15	
Act Effct Green (s)	21.1	21.1	21.1	21.1	21.1	21.1	70.4	70.4		70.4	70.4	
Actuated g/C Ratio	0.21	0.21	0.21	0.21	0.21	0.21	0.70	0.70		0.70	0.70	
v/c Ratio	0.03	0.18	0.23	0.29	0.13	0.08	0.32	0.42		0.43	0.19	
Control Delay	23.2	28.5	7.1	31.8	27.4	5.2	13.0	10.4		23.6	8.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	23.2	28.5	7.1	31.8	27.4	5.2	13.0	10.4		23.6	8.6	
LOS	С	С	Α	С	С	Α	В	В		С	Α	
Approach Delay		16.9			25.3			10.7			11.2	
Approach LOS		В			С			В			В	
Queue Length 50th (m)	0.9	10.9	1.1	10.8	7.1	0.0	9.0	25.0		4.5	8.7	
Queue Length 95th (m)	m2.8	16.6	9.2	16.0	11.5	3.5	38.8	73.0		#33.1	29.6	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	383	628	568	389	589	515	604	3262		204	2235	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.02	0.10	0.14	0.16	0.07	0.05	0.32	0.42		0.43	0.19	

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Offset: 23 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.43 Intersection Signal Delay: 12.0

Intersection Signal Delay: 12.0
Intersection Capacity Utilization 70.0%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Conroy & St. Laurent



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1}			ર્ની
Traffic Volume (vph)	75	40	71	22	69	176
Future Volume (vph)	75	40	71	22	69	176
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.953		0.968			
Flt Protected	0.968					0.986
Satd. Flow (prot)	1563	0	1498	0	0	1678
Flt Permitted	0.968					0.986
Satd. Flow (perm)	1563	0	1498	0	0	1678
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)		1				
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	7%	15%	15%	6%	4%
Adj. Flow (vph)	75	40	71	22	69	176
Shared Lane Traffic (%)						
Lane Group Flow (vph)	115	0	93	0	0	245
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	Ţ.	0.0	, i		0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Canacity Litilizat	ion 3/1 1%			IC	III evel of	Sarvica A

Intersection Capacity Utilization 34.1% Analysis Period (min) 15

ICU Level of Service A

Synchro 10 Report J.Audia, Novatech

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Traffic Volume (uph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations	*	∱ %		7	∱ Љ		¥	ĵ.		7	î,	
	Traffic Volume (vph)	27		100	18		70	162		70	92		50
Storage Length (m)	Future Volume (vph)	27	1405	100	18	1424	70	162	5	70	92	104	50
Storage Lanes	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Taper Length (m)	Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Lane Util. Factor	Storage Lanes	1		0	1		0	1		0	1		0
Ped Bike Factor	Taper Length (m)	25.0			30.0			25.0			30.0		
Fit Protected 0.990 0.990 0.990 0.950 0.	Lane Util. Factor	1.00	0.95	0.95	1.00		0.95	1.00	1.00	1.00	1.00	1.00	1.00
Filt Principated 0.950	Ped Bike Factor	1.00			1.00			1.00					
Satt Flow (proft)	Frt		0.990			0.993			0.860			0.951	
FILP Permitted	Flt Protected												
Satt Flow (perm) 227 3242 0 224 3291 0 1045 1502 0 1248 1652 0 Right Turn on Red Yes	Satd. Flow (prot)		3242	0		3291	0		1502	0		1652	0
Right Turn on Red	Flt Permitted										0.708		
Satte Flow (RTOR)		227	3242	~	224	3291	-	1045	1502	0	1248	1652	-
Link Speed (k/h)				Yes			Yes			Yes			Yes
Link Distance (m)													
Travel Time (s)													
Confl. Beds. (#hr)	Link Distance (m)												
Confi. Bikes (#hr)			28.9			12.3			7.2			20.2	
Peak Hour Factor		1		10	10		1	2					2
Heavy Vehicles (%)				-									
Adj. Flow (vph) 27 1405 100 18 1424 70 162 5 70 92 104 50 Shared Lane Traffic (%) Lane Group Flow (vph) 27 1505 0 18 1494 0 162 75 0 92 154 0 Enter Blocked Intersection No	Peak Hour Factor												
Shared Lane Traffic (%) Lane Group Flow (vph) 27 1505 0 18 1494 0 162 75 0 92 154 0 0 1516 0 No No No No No No No	Heavy Vehicles (%)		3%	3%	1%	2%	1%	1%	1%	2%		1%	4%
Lane Group Flow (vph) 27 1505 0 18 1494 0 162 75 0 92 154 0	Adj. Flow (vph)	27	1405	100	18	1424	70	162	5	70	92	104	50
Enter Blocked Intersection No No No No No No No													
Lane Alignment	Lane Group Flow (vph)		1505										-
Median Width(m) 5.0 5.0 3.5 3.5 1.5	Enter Blocked Intersection						No			No		No	
Link Offset(m) 0.0 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane 1.09	Lane Alignment	L NA		R NA	Left		Right	L NA		R NA	L NA		R NA
Crosswalk Width(m) 5.0 5.0 5.0 5.0 5.0													
Two way Left Turn Lane Headway Factor 1.09													
Headway Factor 1.09			5.0			5.0			5.0			5.0	
Turning Speed (k/h) 24 14 24 14 24 14 24 14 24 14 1													
Number of Detectors 1 2 1 2 1 2 1 2 Detector Template Left Thru Left Thru Left Thru Left Thru Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0			1.09			1.09			1.09			1.09	
Detector Template				14			14			14			14
Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0		· · · · · · · · · · · · · · · · · · ·											
Trailing Detector (m) 0.0	·												
Detector 1 Position(m) 0.0													
Detector 1 Size(m) 6.1 1.8 6.1 1.8 6.1 1.8 6.1 1.8 Detector 1 Type CI+Ex								0.0					
Detector 1 Type CI+Ex													
Detector 1 Channel Detector 1 Extend (s) 0.0	\ ,												
Detector 1 Extend (s) 0.0		CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Queue (s) 0.0													
Detector 1 Delay (s) 0.0 1.8													
Detector 2 Position(m) 28.7 28.7 28.7 28.7 Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4		0.0			0.0			0.0			0.0		
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4	()												
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4			Cl+Ex			Cl+Ex			CI+Ex			CI+Ex	
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Permitted Phases 2 6 8 4		Perm			Perm			Perm			Perm		
			2			6			8			4	
Detector Phase 2 2 6 6 8 8 4 4													
	Detector Phase	2	2		6	6		8	8		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	76.7	76.7		76.7	76.7		21.2	21.2		21.2	21.2	
Actuated g/C Ratio	0.70	0.70		0.70	0.70		0.19	0.19		0.19	0.19	
v/c Ratio	0.17	0.66		0.12	0.65		0.81	0.23		0.38	0.46	
Control Delay	11.0	12.3		8.3	10.6		69.5	16.0		41.4	36.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	11.0	12.3		8.3	10.6		69.5	16.0		41.4	36.4	
LOS	В	В		Α	В		Е	В		D	D	
Approach Delay		12.2			10.5			52.6			38.3	
Approach LOS		В			В			D			D	
Queue Length 50th (m)	1.6	78.3		1.1	55.1		30.8	4.1		16.0	23.1	
Queue Length 95th (m)	6.8	127.7		m2.0	50.3		48.4	13.8		27.3	37.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	158	2264		156	2297		283	443		338	463	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.17	0.66		0.12	0.65		0.57	0.17		0.27	0.33	

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 16.0 Intersection Capacity Utilization 79.2%

Intersection LOS: B ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

1: Don Reid/Ryder & Walkley Splits and Phases:



Synchro 10 Report J.Audia, Novatech

	-	•	•	•	•	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	*	EBK	VVDL T	<u>₩</u>	NDL	NDK
Traffic Volume (vph)	TT 1582	64	3 6	TT 1447	1 49	
Future Volume (vph)	1582	64	36	1447	49	41
	1800	1800	1800	1800	1800	1800
Ideal Flow (vphpl)	1800	20.0	65.0	1000	30.0	0.0
Storage Length (m)		20.0	65.0 1		30.0	0.0
Storage Lanes		I	•			
Taper Length (m)	0.05	1.00	25.0	0.05	30.0	4.00
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850	0.050		0.050	0.850
Flt Protected	00-0	4.400	0.950	0040	0.950	1100
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted			0.136		0.950	
Satd. Flow (perm)	3252	1441	235	3316	1658	1463
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		30				41
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)		5	5			1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	2%	3%	2%	2%	2%
Adj. Flow (vph)	1582	64	36	1447	49	41
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1582	64	36	1447	49	41
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane	0.0			0.0	0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.03	1.09	24	1.03	24	1.09
Number of Detectors	2	14	1	2	1	14
Detector Template	Thru		Left	Thru	Left	-
		Right				Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			Cl+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2	. 71111	. 71111	6	8	
Permitted Phases		2	6	<u> </u>	J	8
Detector Phase	2	2	6	6	8	8
Switch Phase			U	U	U	Ü
SWILLII FIIASE						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0
Total Split (s)	81.0	81.0	81.0	81.0	29.0	29.0
Total Split (%)	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%
Maximum Green (s)	75.0%	75.0	75.0	75.0	23.0	23.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7
	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)						
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag						
Lead-Lag Optimize?		2.2	2.2	2.2	2.2	0.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)	18.0	18.0			7.0	7.0
Flash Dont Walk (s)	13.0	13.0			16.0	16.0
Pedestrian Calls (#/hr)	5	5			5	5
Act Effct Green (s)	90.6	90.6	90.6	90.6	10.9	10.9
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.10	0.10
v/c Ratio	0.59	0.05	0.19	0.53	0.30	0.23
Control Delay	2.1	0.7	7.7	6.6	48.0	15.0
Queue Delay	0.0	0.0	0.0	0.1	0.0	0.0
Total Delay	2.1	0.7	7.7	6.7	48.0	15.0
LOS	Α	Α	Α	Α	D	В
Approach Delay	2.1			6.7	32.9	
Approach LOS	A			А	С	
Queue Length 50th (m)	14.5	0.2	0.8	58.0	9.4	0.0
Queue Length 95th (m)	21.3	m0.7	m8.1	111.0	17.5	8.2
Internal Link Dist (m)	147.1			145.1	104.6	٥.٢
Turn Bay Length (m)	171.1	20.0	65.0	1 70.1	30.0	
Base Capacity (vph)	2679	1192	193	2731	346	338
Starvation Cap Reductn	86	0	0	333	0	0
Spillback Cap Reductn	73	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.61	0.05	0.19	0.60	0.14	0.12
Reduced V/C Ratio	0.01	0.05	0.19	0.60	0.14	0.12
Intersection Summary						
Area Type:	Other					
Cycle Length: 110						
Actuated Cycle Length: 110						
Offset: 25 (23%), Referenced to	n nhase 2·F	BT and 6·\	VRTI Sta	rt of Green	<u> </u>	
Natural Cycle: 70	o priaso z.L	Di ana o.v	VDTL, Ola	it or orcor	ı	
Control Type: Actuated-Coordin	natod					
Maximum v/c Ratio: 0.59	ialeu					
Intersection Signal Delay: 5.1				ln	tersection	1 OC: A
	S CO 00/					
Intersection Capacity Utilization	100.8%			IC	CU Level of	Service B
Analysis Period (min) 15						
m Volume for 95th percentile	queue is me	etered by u	ıpstream s	ignal.		
Splits and Dhagas: 2: 160m l	W of Conroy	, 9 \Malklas	,			
Splits and Phases: 2: 160m	vv or Coriroy	α vvaikie	/			
▼ Ø2 (R)						
81 c						
015						
Ø6 (R)						
91 -						

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	^	7	ሻሻ	^	1,00	ሻሻ	7
Traffic Volume (vph)	1171	452	510	990	42	461	313
Future Volume (vph)	1171	452	510	990	42	461	313
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		75.0	200.0			2	1
		l I	50.0			10.0	ı
Taper Length (m)	0.05	1.00		0.05	0.05		1.00
Lane Util. Factor	0.95	1.00 0.96	0.97 0.99	0.95	0.95	0.97	1.00 0.97
Ped Bike Factor			0.99				0.850
Frt		0.850	0.050			0.050	0.830
Fit Protected	0004	4400	0.950	20.40	^	0.950	4.400
Satd. Flow (prot)	3221	1483	3185	3349	0	3248	1469
Flt Permitted	0004	4 400	0.950	00.40	^	0.950	4440
Satd. Flow (perm)	3221	1430	3160	3349	0	3248	1418
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		452					313
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		22	22				16
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	2%	3%	1%	1%	1%	3%
Adj. Flow (vph)	1171	452	510	990	42	461	313
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1171	452	510	990	0	503	313
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	1.03	24	1.03
Number of Detectors	2	14	1	2	14	1	14
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0 1.8			0.0 1.8		6.1	0.0
Detector 1 Size(m)		6.1	6.1		6.1		6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel	^ ^	2.2	^ ^	2.2	^ ^	^ ^	^ ^
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	
Permitted Phases		2			8		8
Detector Phase	2	2	1	6	8	8	8
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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	51.0	51.0	28.0	79.0	31.0	31.0	31.0
Total Split (%)	46.4%	46.4%	25.5%	71.8%	28.2%	28.2%	28.2%
Maximum Green (s)	44.6	44.6	21.8	72.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	48.3	48.3	20.9	75.5		21.7	21.7
Actuated g/C Ratio	0.44	0.44	0.19	0.69		0.20	0.20
v/c Ratio	0.83	0.51	0.84	0.43		0.78	0.59
Control Delay	30.7	6.2	56.4	8.7		50.9	9.0
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0
Total Delay	30.7	6.2	56.4	8.7		50.9	9.0
LOS	С	Α	Е	Α		D	Α
Approach Delay	23.9			25.0		34.9	
Approach LOS	С			С		С	
Queue Length 50th (m)	66.8	0.2	49.4	42.6		48.4	0.0
Queue Length 95th (m)	#134.1	31.5	#71.2	56.8		63.7	21.0
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1414	881	637	2297		726	560
Starvation Cap Reductn	2	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.83	0.51	0.80	0.43		0.69	0.56

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

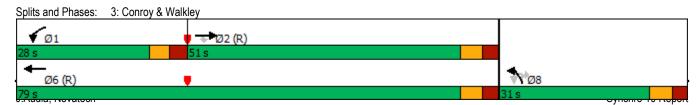
Intersection Signal Delay: 26.6 Intersection Capacity Utilization 82.5%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	•	7	7	^	7	7	ᡮ ᡮᡗ∌		7	ħβ	
Traffic Volume (vph)	41	77	316	232	34	102	40	567	81	37	1058	13
Future Volume (vph)	41	77	316	232	34	102	40	567	81	37	1058	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	1.00		0.99	1.00	0.99		0.99	1.00	
Frt			0.850			0.850		0.981			0.998	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4612	0	1537	3303	0
FIt Permitted	0.735			0.555			0.174			0.385		
Satd. Flow (perm)	1293	1728	1461	957	1618	1463	300	4612	0	617	3303	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			123			88		29			1	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	2		4	4		2	8		12	12		8
Confl. Bikes (#/hr)									4			11
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	2%	3%	10%	2%	3%	2%	8%	10%	2%	12%
Adj. Flow (vph)	41	77	316	232	34	102	40	567	81	37	1058	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	41	77	316	232	34	102	40	648	0	37	1071	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0	•		6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8		8	2			6	-	
Detector Phase	4	4	4	3	8	8	2	2		6	6	
		•	•				_					

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	20.8	20.8	20.8	37.0	35.8	35.8	46.0	46.0		46.0	46.0	
Actuated g/C Ratio	0.22	0.22	0.22	0.39	0.38	0.38	0.48	0.48		0.48	0.48	
v/c Ratio	0.14	0.20	0.76	0.53	0.06	0.17	0.28	0.29		0.12	0.67	
Control Delay	26.5	27.9	31.4	23.6	15.3	4.7	27.0	16.3		19.8	24.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	26.5	27.9	31.4	23.6	15.3	4.7	27.0	16.3		19.8	24.1	
LOS	С	С	С	С	В	Α	С	В		В	С	
Approach Delay		30.3			17.6			16.9			24.0	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	5.8	11.0	31.8	28.4	3.7	1.5	3.7	20.8		3.1	66.5	
Queue Length 95th (m)	10.4	16.5	45.0	30.7	6.4	7.4	15.8	40.0		12.0	#139.4	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	504	674	645	440	887	842	145	2246		298	1598	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.08	0.11	0.49	0.53	0.04	0.12	0.28	0.29		0.12	0.67	

Other

Area Type: Cycle Length: 95

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76

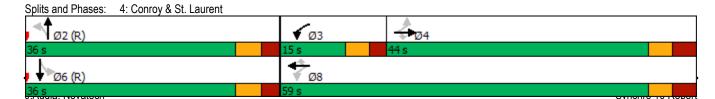
Intersection Signal Delay: 22.3 Intersection Capacity Utilization 82.9%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^			ર્ની
Traffic Volume (vph)	23	56	107	76	151	51
Future Volume (vph)	23	56	107	76	151	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.904		0.944			
Flt Protected	0.986					0.964
Satd. Flow (prot)	1481	0	1641	0	0	1588
Flt Permitted	0.986					0.964
Satd. Flow (perm)	1481	0	1641	0	0	1588
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				2	2	
Confl. Bikes (#/hr)		1		4		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	23	56	107	76	151	51
Shared Lane Traffic (%)						
Lane Group Flow (vph)	79	0	183	0	0	202
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	Ţ.	0.0	, i		0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Canacity Litilizati	ion 37.7%			IC	III evel of	Service A

Intersection Capacity Utilization 37.7% Analysis Period (min) 15

ICU Level of Service A

Synchro 10 Report J.Audia, Novatech

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Traffic Volume (volt)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations	*	↑ 13-		, N	∱ Љ		¥	ĵ.		7	î,	
	Traffic Volume (vph)	29	1125	164	41		195	78		53	49		39
Storage Length (m)	Future Volume (vph)	29	1125	164	41	1531	195	78	7	53	49	47	39
Storage Lanes	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Taper Length (m)	Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Lane Util. Factor	Storage Lanes	1		0	1		0	1		0	1		0
Ped Bike Factor	Taper Length (m)	25.0			30.0			25.0			30.0		
Fit Protected 0.981	Lane Util. Factor	1.00	0.95	0.95	1.00		0.95	1.00	1.00	1.00	1.00	1.00	1.00
Fit Protected 0.950 0.95	Ped Bike Factor		1.00		1.00	1.00		1.00	0.99			0.99	
Satt Flow (proft) 1537 3190 0 1642 3172 0 1580 1378 0 1674 1633 0	Frt		0.981			0.983			0.867			0.932	
Fit Permitted	Flt Protected												
Satd Flow (perm) 163 3190 0 325 3172 0 1165 1378 0 1265 1633 1633 16	Satd. Flow (prot)		3190	0		3172	0		1378	0		1633	0
Right Turn on Red	Flt Permitted												
Satisf Flow (RTOR)		163	3190	_	325	3172		1165	1378	0	1265	1633	-
Link Distance (m)				Yes			Yes			Yes			Yes
Link Distance (m)													
Travel Time (s)													
Confi. Bets. (#hr)	Link Distance (m)												
Confi. Bikes (#hr)			28.9			12.3			7.2			20.2	
Peak Hour Factor		1		5	5		1	1					1
Heavy Vehicles (%)													
Adj. Flow (vph) 29 1125 164 41 1531 195 78 7 53 49 47 39	Peak Hour Factor												
Shared Lane Traffic (%) Lane Group Flow (vph) 29 1289 0 41 1726 0 78 60 0 49 86 0 0 1289 1289 0 1289 1289 0 1289	Heavy Vehicles (%)		4%	1%	3%	5%	1%	7%	1%	12%	1%		
Lane Group Flow (vph) 29 1289 0	Adj. Flow (vph)	29	1125	164	41	1531	195	78	7	53	49	47	39
Enter Blocked Intersection													
Link Alignment	Lane Group Flow (vph)		1289			1726			60	0			-
Median Width(m) 5.0 5.0 3.5 3.5 3.5	Enter Blocked Intersection						No			No		No	
Link Offset(m) 0.0	Lane Alignment	L NA		R NA	Left		Right	L NA		R NA	L NA		R NA
Crosswalk Width(m) 5.0 5.0 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09													
Two way Left Turn Lane Headway Factor 1.09													
Headway Factor 1.09			5.0			5.0			5.0			5.0	
Turning Speed (k/h) 24 14 <td></td>													
Number of Detectors 1 2 1 2 1 2 1 2 Detector Template Left Thru Left Thru Left Thru Left Thru Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0			1.09			1.09			1.09			1.09	
Detector Template				14			14			14			14
Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0		· · · · · · · · · · · · · · · · · · ·			-								
Trailing Detector (m) 0.0	·												
Detector 1 Position(m) 0.0													
Detector 1 Size(m) 6.1 1.8 6.1 1.8 6.1 1.8 Detector 1 Type CI+Ex								0.0					
Detector 1 Type CI+Ex													
Detector 1 Channel Detector 1 Extend (s) 0.0	\ ,												
Detector 1 Extend (s) 0.0		CI+Ex	CI+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Queue (s) 0.0													
Detector 1 Delay (s) 0.0 1.8													
Detector 2 Position(m) 28.7 28.7 28.7 28.7 Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4		0.0			0.0			0.0			0.0		
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4	()												
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4			CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Permitted Phases 2 6 8 4		Perm			Perm			Perm			Perm		
			2			6			8			4	
Detector Phase 2 2 6 6 8 8 4 4													
	Detector Phase	2	2		6	6		8	8		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	64.0	64.0		64.0	64.0		36.0	36.0		36.0	36.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		36.0%	36.0%		36.0%	36.0%	
Maximum Green (s)	58.1	58.1		58.1	58.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	5	5		5	5		5	5		5	5	
Act Effct Green (s)	77.1	77.1		77.1	77.1		15.2	15.2		15.2	15.2	
Actuated g/C Ratio	0.77	0.77		0.77	0.77		0.15	0.15		0.15	0.15	
v/c Ratio	0.23	0.52		0.16	0.70		0.44	0.24		0.26	0.31	
Control Delay	13.4	7.8		6.0	6.8		44.0	12.8		37.6	25.9	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	13.4	7.8		6.0	6.8		44.0	12.8		37.6	25.9	
LOS	В	Α		Α	Α		D	В		D	С	
Approach Delay		7.9			6.8			30.4			30.2	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	1.3	39.0		1.1	28.4		13.3	1.1		8.1	9.1	
Queue Length 95th (m)	9.5	99.2		m2.6	#191.1		21.3	9.1		14.5	17.5	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	125	2464		250	2450		347	447		376	508	
Starvation Cap Reductn	0	0		0	20		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.23	0.52		0.16	0.71		0.22	0.13		0.13	0.17	

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Offset: 94 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70
Intersection Signal Delay: 9.2

Intersection Signal Delay: 9.2
Intersection Capacity Utilization 72.6%

Intersection LOS: A ICU Level of Service C

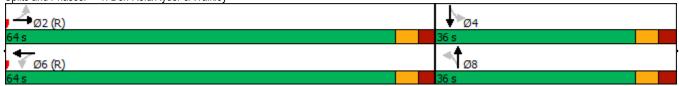
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Don Reid/Ryder & Walkley



	-	•	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
		EDR	VVDL		INDL T	NDK.
Lane Configurations	↑ ↑			↑ ↑		
Traffic Volume (vph)	1147	90	74	1719	66	60
Future Volume (vph)	1147	90	74	1719	66	60
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)		20.0	65.0		30.0	0.0
Storage Lanes		1	1		1	1
Taper Length (m)			25.0		30.0	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	3221	1498	1674	3191	1674	1483
Flt Permitted			0.231		0.950	
Satd. Flow (perm)	3221	1459	407	3191	1674	1462
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		59				60
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	14.0	4	4	14.4	3.0	2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
	5%	1.00	1.00	6%	1.00	2%
Heavy Vehicles (%)						
Adj. Flow (vph)	1147	90	74	1719	66	60
Shared Lane Traffic (%)	444-	00	7.4	4740	00	00
Lane Group Flow (vph)	1147	90	74	1719	66	60
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.1	0.0	0.0	0.1	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
\ <i>\</i>						
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2	I GIIII	I GIIII	6	8	1 51111
		2	6	U	U	0
Permitted Phases	^	2	6	•	0	8
Detector Phase	2	2	6	6	8	8
Switch Phase						

	-	\rightarrow	•	←	~	/		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR		
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0		
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0		
Total Split (s)	71.0	71.0	71.0	71.0	29.0	29.0		
Total Split (%)	71.0%	71.0%	71.0%	71.0%	29.0%	29.0%		
Maximum Green (s)	65.0	65.0	65.0	65.0	23.0	23.0		
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0		
Lead/Lag								
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None		
Walk Time (s)	18.0	18.0			7.0	7.0		
Flash Dont Walk (s)	13.0	13.0			16.0	16.0		
Pedestrian Calls (#/hr)	5	5			5	5		
Act Effct Green (s)	80.2	80.2	80.2	80.2	11.3	11.3		
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.11	0.11		
v/c Ratio	0.44	0.08	0.23	0.67	0.35	0.28		
Control Delay	3.0	1.0	6.9	8.5	43.6	12.5		
Queue Delay	0.0	0.0	0.0	0.6	0.0	0.0		
Total Delay	3.0	1.0	6.9	9.0	43.6	12.5		
LOS	Α	Α	Α	Α	D	В		
Approach Delay	2.9			8.9	28.8			
Approach LOS	Α			Α	С			
Queue Length 50th (m)	18.3	0.3	2.7	58.5	11.3	0.0		
Queue Length 95th (m)	23.7	1.1	12.5	142.2	19.6	8.9		
Internal Link Dist (m)	147.1			145.1	104.6			
Turn Bay Length (m)		20.0	65.0		30.0			
Base Capacity (vph)	2583	1182	326	2560	385	382		
Starvation Cap Reductn	0	0	0	406	0	0		
Spillback Cap Reductn	0	0	0	70	0	0		
Storage Cap Reductn	0	0	0	0	0	0		
Reduced v/c Ratio	0.44	0.08	0.23	0.80	0.17	0.16		
Intersection Summary								
	Other							
Cycle Length: 100	Culci							
Actuated Cycle Length: 100								
Offset: 88 (88%), Referenced to	n nhase 2.F	RT and 6·\	WRTI Sta	rt of Green	1			
Natural Cycle: 80	o priuse z.L	5 1 and 0.1	, D i E, Ola	TO OIGGI	·			
Control Type: Actuated-Coordi	nated							
Maximum v/c Ratio: 0.67	nateu							
ntersection Signal Delay: 7.4				In	tersection	I OS: A		
ntersection Capacity Utilization	n 65 3%				CU Level of			
Analysis Period (min) 15	11 00.070			IC	O FEACUAL	JOI VIOC O		
Allalysis i ellou (Illill) 15								
Splits and Phases: 2: 160m	W of Conroy	& Walkley	/					
71 s								
4							-	k .
₩ Ø6 (R)								ÿ8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	*		NDL FF	↑ ↑	1400	NDL YY	TION.
Traffic Volume (vph)	47 987	280	257	1091	14	717	484
Future Volume (vph)	987	280	257	1091	14	717	484
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		1	2			2	1
Taper Length (m)		•	50.0			10.0	•
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
Ped Bike Factor		0.98	0.99	0.00	0.00	3.01	0.98
Frt		0.850	2.00				0.850
Flt Protected		1.000	0.950			0.950	
Satd. Flow (prot)	3161	1455	3066	3161	0	3186	1455
Flt Permitted			0.950	3.31		0.950	
Satd. Flow (perm)	3161	1419	3050	3161	0	3186	1430
Right Turn on Red		Yes	- 5500	J 10 1		0100	Yes
Satd. Flow (RTOR)		280					332
Link Speed (k/h)	50			50		60	- JUL
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)	,	10	10	10.0		_0.0	3
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	7%	4%	7%	7%	2%	3%	4%
Adj. Flow (vph)	987	280	257	1091	14	717	484
Shared Lane Traffic (%)		200		1301			101
Lane Group Flow (vph)	987	280	257	1091	0	731	484
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	RNA
Median Width(m)	7.0	1,10,1	Lon	9.0	11101	10.5	11171
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	1.03	24	1.03
Number of Detectors	2	1	1	2	1	1	1
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel	OITEA	OITEX	OITEX	OI'LX	OI'LX	OITEX	OI'LX
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s) Detector 2 Position(m)	28.7	0.0	0.0		0.0	0.0	0.0
· ,				28.7			
Detector 2 Size(m)	1.8			1.8 CI+Ex			
Detector 2 Type	CI+Ex			CI+EX			
Detector 2 Channel	0.0			0.0			
Detector 2 Extend (s)	0.0	De	D4	0.0	Dema	Dest	Dem
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2	^	1	6	^	8	^
Permitted Phases		2			8		8
Detector Phase	2	2	1	6	8	8	8

	-	•	•	•	₹ì	1	/
Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	55.0	55.0	25.0	80.0	40.0	40.0	40.0
Total Split (%)	45.8%	45.8%	20.8%	66.7%	33.3%	33.3%	33.3%
Maximum Green (s)	48.6	48.6	18.8	73.6	33.6	33.6	33.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	2.1	0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead	0.7		0.7	U. T
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0	INOHE	O-IVIAX	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	10	10			17.0	17.0	17.0
	54.1		15 1	7E /	ວ	31.8	31.8
Act Effct Green (s)		54.1	15.1 0.13	75.4 0.63		0.26	0.26
Actuated g/C Ratio	0.45	0.45					
v/c Ratio	0.69	0.35	0.67	0.55		0.87	0.78
Control Delay	30.5	3.9	58.4	14.3		53.7	21.7
Queue Delay	0.3	0.0	0.0	0.0		0.0	0.0
Total Delay	30.9	3.9	58.4	14.3		53.7	21.7
LOS	C	Α	Е	В		D	С
Approach Delay	24.9			22.7		40.9	
Approach LOS	С			С		D	
Queue Length 50th (m)	90.9	0.0	27.7	68.9		76.4	30.1
Queue Length 95th (m)	119.8	14.9	39.0	85.5		97.6	70.5
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1424	793	480	1986		892	639
Starvation Cap Reductn	95	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.74	0.35	0.54	0.55		0.82	0.76
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 120							
Offset: 43 (36%), Referenced	to phase 2.F	RT and 6·\	WRT Start	of Green			
Natural Cycle: 85	to priase z.L	Di ana o.v	VD1, Otari	OI OIGGII			
Control Type: Actuated-Coord	linated						
Maximum v/c Ratio: 0.87	ilialeu						
	^			la.	tana asti an	1.00.0	
Intersection Signal Delay: 29.					tersection		
Intersection Capacity Utilization	on /4.4%			IC	U Level of	Service D	
Analysis Period (min) 15							
Splits and Phases: 3: Conre	oy & Walkley						
√ Ø1		(D)					
▼ Ø1 25 s	55 s	(R)					
←							
Ø6 (R)	•						

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	*	7	7	^	7	7	ተ ቀጭ		7	ħβ	
Traffic Volume (vph)	6	65	81	63	43	25	192	1246	199	87	389	53
Future Volume (vph)	6	65	81	63	43	25	192	1246	199	87	389	53
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	0.99		0.98	0.98	0.99		1.00	0.99	
Frt			0.850			0.850		0.979			0.982	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1353	1695	1427	1409	1589	1351	1642	4624	0	1674	3172	0
FIt Permitted	0.729			0.715			0.496			0.154		
Satd. Flow (perm)	1033	1695	1395	1050	1589	1328	844	4624	0	270	3172	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			81			36		43			21	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	6		13	13		6	14		21	21		14
Confl. Bikes (#/hr)												25
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	25%	5%	6%	20%	12%	12%	3%	2%	2%	1%	4%	3%
Adj. Flow (vph)	6	65	81	63	43	25	192	1246	199	87	389	53
Shared Lane Traffic (%)												
Lane Group Flow (vph)	6	65	81	63	43	25	192	1445	0	87	442	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
		•	•				_			•		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	43.9	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	44.0	44.0	44.0	56.0	56.0		56.0	56.0	
Total Split (%)	44.0%	44.0%	44.0%	44.0%	44.0%	44.0%	56.0%	56.0%		56.0%	56.0%	
Maximum Green (s)	37.1	37.1	37.1	37.1	37.1	37.1	49.7	49.7		49.7	49.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0	30.0	30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	15	15	15	10	10	10	20	20		15	15	
Act Effct Green (s)	21.1	21.1	21.1	21.1	21.1	21.1	70.4	70.4		70.4	70.4	
Actuated g/C Ratio	0.21	0.21	0.21	0.21	0.21	0.21	0.70	0.70		0.70	0.70	
v/c Ratio	0.03	0.18	0.23	0.29	0.13	0.08	0.32	0.44		0.46	0.20	
Control Delay	23.2	28.4	6.8	31.8	27.4	5.2	13.1	10.6		26.2	8.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	23.2	28.4	6.8	31.8	27.4	5.2	13.1	10.6		26.2	8.7	
LOS	С	С	Α	С	С	Α	В	В		С	Α	
Approach Delay		16.7			25.3			10.9			11.6	
Approach LOS		В			С			В			В	
Queue Length 50th (m)	0.9	10.9	1.1	10.8	7.1	0.0	9.0	26.7		4.6	9.2	
Queue Length 95th (m)	m2.5	m16.1	8.9	16.0	11.5	3.5	39.2	77.6		#35.0	31.1	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	383	628	568	389	589	515	594	3266		190	2238	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.02	0.10	0.14	0.16	0.07	0.05	0.32	0.44		0.46	0.20	

Other

Area Type: Cycle Length: 100

Actuated Cycle Length: 100

Offset: 23 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.46

Intersection Signal Delay: 12.2

Intersection Capacity Utilization 71.2%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Conroy & St. Laurent



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1}			ર્ન
Traffic Volume (vph)	75	40	71	22	69	176
Future Volume (vph)	75	40	71	22	69	176
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.953		0.968			
Flt Protected	0.968					0.986
Satd. Flow (prot)	1563	0	1498	0	0	1678
Flt Permitted	0.968					0.986
Satd. Flow (perm)	1563	0	1498	0	0	1678
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)		1				
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	7%	15%	15%	6%	4%
Adj. Flow (vph)	75	40	71	22	69	176
Shared Lane Traffic (%)						
Lane Group Flow (vph)	115	0	93	0	0	245
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Canacity Litilizat	ion 3/1 1%			IC	III evel of	Sarvica A

Intersection Capacity Utilization 34.1% Analysis Period (min) 15

ICU Level of Service A

Synchro 10 Report J.Audia, Novatech

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Traffic Volume (uph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations	*	∱ %		7	∱ Љ		¥	ĵ.		7	î,	
	Traffic Volume (vph)	27		100	18		70	162		70	92		50
Storage Length (m)	Future Volume (vph)	27	1557	100	18	1626	70	162	5	70	92	104	50
Storage Lanes	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Taper Length (m)	Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Lane Util. Factor	Storage Lanes	1		0	1		0	1		0	1		0
Ped Bike Factor	Taper Length (m)	25.0			30.0			25.0			30.0		
Fit Protected 0.991	Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Filt Principated 0.950	Ped Bike Factor		1.00			1.00		1.00				1.00	
Satt Flow (proft)	Frt		0.991			0.994			0.860			0.951	
FILP Permitted 0.092	Flt Protected												
Satt Flow (perm) 162 3246 0 174 3294 0 1045 1502 0 1248 1652 0 Right Turn on Red Yes	Satd. Flow (prot)		3246	0		3294	0		1502	0		1652	0
Right Turn on Red	Flt Permitted										0.708		
Satte Flow (RTOR)		162	3246		174	3294	-	1045	1502	0	1248	1652	-
Link Deped (k/h) 50 50 50 Link Distance (m) 402.0 171.1 100.3 281.0 Travel Time (s) 28.9 12.3 7.2 20.2 Confl. Bikes (#/hr) 1 10 10 1 2 20.0 Confl. Bikes (#/hr) 1 10 10 1 2 2 20.2 Confl. Bikes (#/hr) 1 10 1.00				Yes			Yes			Yes			Yes
Link Distance (m)													
Travel Time (s)													
Confl. Beds. (#hr)	Link Distance (m)												
Confi. Bikes (#hr)			28.9			12.3			7.2			20.2	
Peak Hour Factor		1		10	10		1	2					2
Heavy Vehicles (%)				-									
Adj. Flow (vph) 27 1557 100 18 1626 70 162 5 70 92 104 50	Peak Hour Factor												
Shared Lane Traffic (%) Lane Group Flow (vph) 27 1657 0 18 1696 0 162 75 0 92 154 0 0 1612 175 0 92 154 0 0 0 0 0 0 0 0 0	Heavy Vehicles (%)	1%		3%				1%		2%			
Lane Group Flow (vph) 27 1657 0 18 1696 0 162 75 0 92 154 0		27	1557	100	18	1626	70	162	5	70	92	104	50
Enter Blocked Intersection No No No No No No No													
Lane Alignment	Lane Group Flow (vph)		1657										-
Median Width(m) 5.0 5.0 3.5 3.5 1.5										No		No	
Link Offset(m) 0.0 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane 1.09		L NA		R NA	Left		Right	L NA		R NA	L NA		R NA
Crosswalk Width(m) 5.0 5.0 5.0 5.0 5.0													
Two way Left Turn Lane Headway Factor 1.09													
Headway Factor 1.09			5.0			5.0			5.0			5.0	
Turning Speed (k/h) 24 14 24 14 24 14 24 14 24 14 1													
Number of Detectors 1 2 1 2 1 2 1 2 Detector Template Left Thru Left Thru Left Thru Left Thru Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0			1.09			1.09			1.09			1.09	
Detector Template				14			14			14			14
Leading Detector (m) 6.1 30.5 6.1 30.5 6.1 30.5 Trailing Detector (m) 0.0		-											
Trailing Detector (m) 0.0	·												
Detector 1 Position(m) 0.0													
Detector 1 Size(m) 6.1 1.8 6.1 1.8 6.1 1.8 6.1 1.8 Detector 1 Type CI+Ex													
Detector 1 Type CI+Ex													
Detector 1 Channel Detector 1 Extend (s) 0.0	\ ,												
Detector 1 Extend (s) 0.0		CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Queue (s) 0.0													
Detector 1 Delay (s) 0.0 1.8													
Detector 2 Position(m) 28.7 28.7 28.7 28.7 Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Size(m) 1.8 1.8 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4		0.0			0.0			0.0			0.0		
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4	()												
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Detector 2 Extend (s) 0.0 0.0 0.0 0.0 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4			Cl+Ex			Cl+Ex			CI+Ex			Cl+Ex	
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Protected Phases 2 6 8 4 Permitted Phases 2 6 8 4													
Permitted Phases 2 6 8 4		Perm			Perm			Perm			Perm		
			2			6			8			4	
Detector Phase 2 2 6 6 8 8 4 4													
	Detector Phase	2	2		6	6		8	8		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	76.7	76.7		76.7	76.7		21.2	21.2		21.2	21.2	
Actuated g/C Ratio	0.70	0.70		0.70	0.70		0.19	0.19		0.19	0.19	
v/c Ratio	0.24	0.73		0.15	0.74		0.81	0.24		0.38	0.46	
Control Delay	14.9	14.0		8.4	10.8		69.5	21.9		41.4	36.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.9	14.0		8.4	10.8		69.5	21.9		41.4	36.4	
LOS	В	В		Α	В		Е	С		D	D	
Approach Delay		14.0			10.7			54.4			38.3	
Approach LOS		В			В			D			D	
Queue Length 50th (m)	1.7	94.6		1.0	57.3		30.8	6.6		16.0	23.1	
Queue Length 95th (m)	8.2	154.2		m1.5	50.4		48.4	16.3		27.3	37.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	113	2266		121	2298		283	432		338	463	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.24	0.73		0.15	0.74		0.57	0.17		0.27	0.33	

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

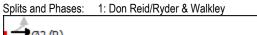
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 16.6 Intersection LOS: B Intersection Capacity Utilization 84.6% ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.





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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	*	EDK	VVDL	↑ ↑	NDL T	NDK
Traffic Volume (vph)	TT 1741	64	36	TT 1650	4 9	41
Future Volume (vph)	1741	64	36	1650	49	41
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	20.0	65.0	1000	30.0	0.0
Storage Lanes		20.0	1		1	1
Taper Length (m)			25.0		30.0	- 1
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor	0.00	0.97	1.00	0.55	1.00	0.99
Frt		0.850	1.00			0.850
Flt Protected		0.000	0.950		0.950	0.000
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted	JZJZ	1400	0.110	3310	0.950	1403
Satd. Flow (perm)	3252	1441	190	3316	1658	1463
Right Turn on Red	3232	Yes	190	3310	1000	Yes
		7 es 28				res 34
Satd. Flow (RTOR)	EΛ	20		FΛ	EΟ	34
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	4.00	5	5	4.00	4.00	1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	2%	3%	2%	2%	2%
Adj. Flow (vph)	1741	64	36	1650	49	41
Shared Lane Traffic (%)				40-0		
Lane Group Flow (vph)	1741	64	36	1650	49	41
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel		J. LA	J. LA	J. LA	∪ ∟ ∧	J.: LA
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7	0.0	0.0	28.7	0.0	0.0
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			Cl+Ex		
Detector 2 Channel	UI+EX			OI+EX		
	0.0			0.0		
Detector 2 Extend (s)	NA	Dorm	Dorm	NA	Drot	Darm
Turn Type		Perm	Perm		Prot	Perm
Protected Phases	2	0	^	6	8	0
Permitted Phases	_	2	6	^	^	8
Detector Phase	2	2	6	6	8	8
Switch Phase						

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	*		77 P	<u></u> ↑↑	1400	NDL NDL	TIDIN
Traffic Volume (vph)	1311	474	537	TT	42	484	328
Future Volume (vph)	1311	474	537	1171	42	484	328
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		75.0	200.0			2	1
		l I	50.0			10.0	ı
Taper Length (m)	0.05	1.00		0.05	0.05		1.00
Lane Util. Factor	0.95	1.00 0.96	0.97 0.99	0.95	0.95	0.97	1.00 0.97
Ped Bike Factor			0.99				0.850
Frt		0.850	0.050			0.050	0.830
Fit Protected	2004	4.400	0.950	22.40	^	0.950	4.400
Satd. Flow (prot)	3221	1483	3185	3349	0	3248	1469
Flt Permitted	0004	4 400	0.950	00.40	^	0.950	4440
Satd. Flow (perm)	3221	1430	3164	3349	0	3248	1418
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		443					328
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		22	22				16
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	2%	3%	1%	1%	1%	3%
Adj. Flow (vph)	1311	474	537	1171	42	484	328
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1311	474	537	1171	0	526	328
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	1.03	24	1.03
Number of Detectors	2	14	2 4 1	2	14	1	14
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
•	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Leading Detector (m)							
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6	2	8	2
Permitted Phases	_	2			8		8
Detector Phase	2	2	1	6	8	8	8
Dolotto i ilase				U	U	0	U

	-	•	•	•	₹I	1	-
Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	51.0	51.0	28.0	79.0	31.0	31.0	31.0
Total Split (%)	46.4%	46.4%	25.5%	71.8%	28.2%	28.2%	28.2%
Maximum Green (s)	44.6	44.6	21.8	72.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	47.3	47.3	21.3	74.8		22.4	22.4
Actuated g/C Ratio	0.43	0.43	0.19	0.68		0.20	0.20
v/c Ratio	0.95	0.55	0.87	0.51		0.80	0.60
Control Delay	41.5	7.7	58.8	9.9		51.2	8.9
Queue Delay	0.0	0.2	0.0	0.0		0.0	0.0
Total Delay	41.5	7.9	58.8	10.0		51.2	8.9
LOS	D	Α	Е	Α		D	Α
Approach Delay	32.6			25.3		35.0	
Approach LOS	С			С		С	
Queue Length 50th (m)	~104.1	0.3	52.6	56.2		50.4	0.0
Queue Length 95th (m)	#179.1	39.9	#77.4	72.2		67.0	21.5
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1385	867	636	2278		726	571
Starvation Cap Reductn	0	57	0	0		0	0
Spillback Cap Reductn	0	0	0	101		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.95	0.59	0.84	0.54		0.72	0.57

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.95 Intersection Signal Delay: 30.2 Intersection Capacity Utilization 87.8%

Intersection LOS: C
ICU Level of Service E

Analysis Period (min) 15

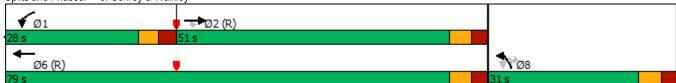
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Conroy & Walkley



	۶	→	•	•	+	4	1	†	/	/	+	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	^	7	7	ᡮ ᡮᡗ∌		7	∱ β	
Traffic Volume (vph)	41	77	316	232	34	102	40	595	81	37	1111	13
Future Volume (vph)	41	77	316	232	34	102	40	595	81	37	1111	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	1.00		0.99		0.99		0.99	1.00	
Frt			0.850			0.850		0.982			0.998	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4619	0	1537	3303	0
Flt Permitted	0.735			0.555			0.155			0.371		
Satd. Flow (perm)	1293	1728	1461	957	1618	1463	268	4619	0	595	3303	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			121			78		27			1	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	2		4	4		2	8		12	12		8
Confl. Bikes (#/hr)									4			11
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	2%	3%	10%	2%	3%	2%	8%	10%	2%	12%
Adj. Flow (vph)	41	77	316	232	34	102	40	595	81	37	1111	13
Shared Lane Traffic (%)	• • • • • • • • • • • • • • • • • • • •		0.0		•				•	•		
Lane Group Flow (vph)	41	77	316	232	34	102	40	676	0	37	1124	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5		20.0	7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane		0.0										
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	• •
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel	OI · LX	OI · EX	OITEX	OI · LX	OI · LX	OI LX	OI LX	OI LX		OI · EX	OI LX	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7	0.0	0.0	28.7	0.0	0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OI+LX			OITEX			OI*LX			OITEX	
		0.0			0.0			0.0			0.0	
Detector 2 Extend (s)	Dorm		Dorm	nmint		Perm	Perm	NA		Dorm	NA	
Turn Type	Perm	NA 4	Perm	pm+pt	NA	reim	reim	NA 2		Perm		
Protected Phases	A	4	4	3	8	0	0	2		•	6	
Permitted Phases	4	4	4	8	0	8	2	0		6		
Detector Phase	4	4	4	3	8	8	2	2		6	6	

	•	→	\rightarrow	•	←	•	•	†	/	/	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	20.9	20.9	20.9	37.1	35.9	35.9	45.9	45.9		45.9	45.9	
Actuated g/C Ratio	0.22	0.22	0.22	0.39	0.38	0.38	0.48	0.48		0.48	0.48	
v/c Ratio	0.14	0.20	0.76	0.53	0.06	0.17	0.31	0.30		0.13	0.70	
Control Delay	26.5	27.8	31.6	23.5	15.3	5.6	29.7	16.5		20.1	25.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	26.5	27.8	31.6	23.5	15.3	5.6	29.7	16.5		20.1	25.0	
LOS	С	С	С	С	В	Α	С	В		С	С	
Approach Delay		30.5			17.8			17.3			24.9	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	5.8	11.0	32.2	28.4	3.7	2.6	3.8	22.1		3.2	71.5	
Queue Length 95th (m)	10.4	16.5	45.3	30.7	6.4	8.2	#18.4	42.1		12.1	#149.8	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	504	674	644	440	887	837	129	2244		287	1595	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.08	0.11	0.49	0.53	0.04	0.12	0.31	0.30		0.13	0.70	

Other

Area Type: Cycle Length: 95

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76

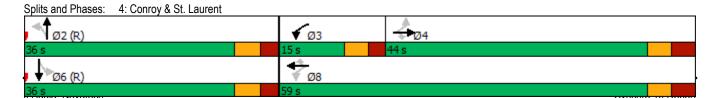
Intersection Signal Delay: 22.8 Intersection Capacity Utilization 84.5%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



	•	•	†	~	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1 2			ની
Traffic Volume (vph)	23	56	107	76	151	51
Future Volume (vph)	23	56	107	76	151	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.904		0.944			
Flt Protected	0.986					0.964
Satd. Flow (prot)	1481	0	1641	0	0	1588
Flt Permitted	0.986					0.964
Satd. Flow (perm)	1481	0	1641	0	0	1588
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				2	2	
Confl. Bikes (#/hr)		1		4		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	23	56	107	76	151	51
Shared Lane Traffic (%)						<u> </u>
Lane Group Flow (vph)	79	0	183	0	0	202
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
	07.70/			10		

Intersection Capacity Utilization 37.7% Analysis Period (min) 15

ICU Level of Service A

Synchro 10 Report J.Audia, Novatech

	۶	→	•	•	+	4	1	†	/	/	↓	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ŧβ		7	∱ β		7	ĵ₃		7	f)	
Traffic Volume (vph)	27	1557	100	18	1626	70	162	5	70	92	104	50
Future Volume (vph)	27	1557	100	18	1626	70	162	5	70	92	104	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0			30.0			25.0			30.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			1.00		1.00				1.00	
Frt		0.991			0.994			0.860			0.951	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	3246	0	1674	3294	0	1674	1502	0	1674	1652	0
Flt Permitted	0.092			0.099			0.594			0.708		
Satd. Flow (perm)	162	3246	0	174	3294	0	1045	1502	0	1248	1652	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11			7			35			22	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
Travel Time (s)		28.9			12.3			7.2			20.2	
Confl. Peds. (#/hr)	1		10	10		1	2					2
Confl. Bikes (#/hr)			1									
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	3%	1%	2%	1%	1%	1%	2%	1%	1%	4%
Adj. Flow (vph)	27	1557	100	18	1626	70	162	5	70	92	104	50
Shared Lane Traffic (%)												
Lane Group Flow (vph)	27	1657	0	18	1696	0	162	75	0	92	154	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	
								-				

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	76.7	76.7		76.7	76.7		21.2	21.2		21.2	21.2	
Actuated g/C Ratio	0.70	0.70		0.70	0.70		0.19	0.19		0.19	0.19	
v/c Ratio	0.24	0.73		0.15	0.74		0.81	0.24		0.38	0.46	
Control Delay	14.9	14.0		8.7	9.1		69.5	21.9		41.4	36.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.9	14.0		8.7	9.1		69.5	21.9		41.4	36.4	
LOS	В	В		Α	Α		Е	С		D	D	
Approach Delay		14.0			9.1			54.4			38.3	
Approach LOS		В			Α			D			D	
Queue Length 50th (m)	1.7	94.6		0.8	46.4		30.8	6.6		16.0	23.1	
Queue Length 95th (m)	8.2	154.2		m1.5	51.3		48.4	16.3		27.3	37.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	113	2266		121	2298		283	432		338	463	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.24	0.73		0.15	0.74		0.57	0.17		0.27	0.33	

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

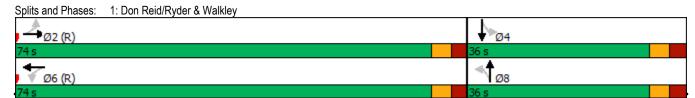
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 15.8 Intersection LOS: B Intersection Capacity Utilization 84.6% ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	*		YVDL T	*	NDL 1	TVDIX
Traffic Volume (vph)	TT 1741	64	36	TT 1650	4 9	41
Future Volume (vph)	1741	64	36	1650	49	41
						1800
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	
Storage Length (m)		20.0	65.0		30.0	0.0
Storage Lanes		1	1		1	1
Taper Length (m)			25.0		30.0	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.97	1.00			0.99
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted			0.110		0.950	
Satd. Flow (perm)	3252	1441	190	3316	1658	1463
Right Turn on Red	OLOL	Yes	100	0010	1300	Yes
Satd. Flow (RTOR)		28				34
	50	20		50	50	34
Link Speed (k/h)						
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3	_	_	12.2	9.3	
Confl. Peds. (#/hr)		5	5			1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	2%	3%	2%	2%	2%
Adj. Flow (vph)	1741	64	36	1650	49	41
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1741	64	36	1650	49	41
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
	3.5	ragni	L INA	5.0	3.5	IX INA
Median Width(m)						
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
				1.8		6.1
Detector 1 Size(m)	1.8	6.1	6.1		6.1	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7		
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			Cl+Ex		
Detector 2 Channel	J/			,		
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
		Fellii	Fellii			Fellil
Protected Phases	2	^		6	8	_
Permitted Phases		2	6			8
Detector Phase	2	2	6	6	8	8
Switch Phase						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR		
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0		
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0		
Total Split (s)	81.0	81.0	81.0	81.0	29.0	29.0		
Total Split (%)	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%		
Maximum Green (s)	75.0	75.0	75.0	75.0	23.0	23.0		
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0		
Lead/Lag	0.0	0.0	0.0	0.0	0.0	0.0		
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None		
Walk Time (s)	18.0	18.0	O-IVIAX	O-IVIAX	7.0	7.0		
Flash Dont Walk (s)	13.0	13.0			16.0	16.0		
. ,	13.0	5			5	5		
Pedestrian Calls (#/hr)			00.0	00.0		10.9		
Act Effct Green (s)	90.6	90.6	90.6	90.6	10.9			
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.10	0.10		
v/c Ratio	0.65	0.05	0.23	0.60	0.30	0.23		
Control Delay	2.2	0.7	9.0	6.5	48.0	19.5		
Queue Delay	0.0	0.0	0.0	0.5	0.0	0.0		
Total Delay	2.3	0.7	9.0	7.1	48.0	19.5		
LOS	Α	A	Α	A	D	В		
Approach Delay	2.2			7.1	35.0			
Approach LOS	Α			Α	D			
Queue Length 50th (m)	16.1	0.1	1.3	49.2	9.4	1.3		
Queue Length 95th (m)	22.3	m0.6	8.3	121.0	17.5	9.4		
Internal Link Dist (m)	147.1			145.1	104.6			
Turn Bay Length (m)		20.0	65.0		30.0			
Base Capacity (vph)	2679	1192	156	2731	346	332		
Starvation Cap Reductn	74	0	0	595	0	0		
Spillback Cap Reductn	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0		
Reduced v/c Ratio	0.67	0.05	0.23	0.77	0.14	0.12		
Intersection Summary								
	Other							
Cycle Length: 110								
Actuated Cycle Length: 110								
Offset: 25 (23%), Referenced to	phase 2:E	BT and 6:\	NBTL, Sta	rt of Green	1			
Natural Cycle: 80								
Control Type: Actuated-Coording	nated							
Maximum v/c Ratio: 0.65								
Intersection Signal Delay: 5.4				In	tersection	LOS: A		
Intersection Capacity Utilization 65.5% ICU Level of Service C								
Analysis Period (min) 15								
m Volume for 95th percentile	queue is me	etered by u	ıpstream s	signal.				
Culita and Dhasses O. 100ms V	N of Common	. 0 \\/= . .						
Splits and Phases: 2: 160m \	N of Conroy	& walkles	/					
▼ Ø2 (R)								
91 c								
018								
- ac (n)								
∮ Ø6 (R)								

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	*	Z Z	ሻሻ	^	1,00	ሻሻ	7
Traffic Volume (vph)	1311	474	537	1171	42	484	328
Future Volume (vph)	1311	474	537	1171	42	484	328
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		1 1	200.0			2	1
Taper Length (m)		- 1	50.0			10.0	
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
	0.95	0.96		0.95	0.95	0.97	0.96
Ped Bike Factor Frt		0.850	0.99				
		0.000	0.050			0.050	0.850
Flt Protected	0004	4.400	0.950	00.40	^	0.950	4400
Satd. Flow (prot)	3221	1483	3185	3349	0	3248	1469
Flt Permitted	2224	4 400	0.950	00.40	^	0.950	4445
Satd. Flow (perm)	3221	1426	3162	3349	0	3248	1415
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		424					328
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		22	22				16
Confl. Bikes (#/hr)		1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	2%	3%	1%	1%	1%	3%
Adj. Flow (vph)	1311	474	537	1171	42	484	328
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1311	474	537	1171	0	526	328
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	14	24	1.09
Number of Detectors	2	14	1	2	14	1	1
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
•	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Leading Detector (m)		0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0						
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	
Permitted Phases		2			8		8
Detector Phase	2	2	1	6	8	8	8
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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	58.0	58.0	31.0	89.0	31.0	31.0	31.0
Total Split (%)	48.3%	48.3%	25.8%	74.2%	25.8%	25.8%	25.8%
Maximum Green (s)	51.6	51.6	24.8	82.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	54.4	54.4	23.5	84.0		23.2	23.2
Actuated g/C Ratio	0.45	0.45	0.20	0.70		0.19	0.19
v/c Ratio	0.90	0.54	0.86	0.50		0.84	0.61
Control Delay	40.6	6.0	61.5	9.4		59.5	9.7
Queue Delay	4.8	0.0	0.0	0.0		0.0	0.0
Total Delay	45.3	6.0	61.5	9.4		59.5	9.7
LOS	D	Α	Е	Α		Е	Α
Approach Delay	34.9			25.8		40.3	
Approach LOS	С			С		D	
Queue Length 50th (m)	141.3	6.3	57.7	57.9		56.2	0.0
Queue Length 95th (m)	#185.3	29.1	#79.5	71.0		74.2	23.3
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1459	877	658	2345		665	550
Starvation Cap Reductn	105	7	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.97	0.54	0.82	0.50		0.79	0.60

Other

Area Type: Cycle Length: 120

Actuated Cycle Length: 120

Offset: 20 (17%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

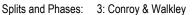
Maximum v/c Ratio: 0.90 Intersection Signal Delay: 32.4 Intersection Capacity Utilization 87.8%

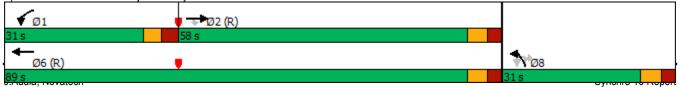
Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	7	^	7	7	ተ ቀኈ		7	∱ β	
Traffic Volume (vph)	41	77	316	232	34	102	40	595	81	37	1111	13
Future Volume (vph)	41	77	316	232	34	102	40	595	81	37	1111	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	1.00		0.98	1.00		0.99		0.99		0.99	1.00	
Frt			0.850			0.850		0.982			0.998	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4619	0	1537	3303	0
Flt Permitted	0.735			0.555			0.155			0.371		
Satd. Flow (perm)	1293	1728	1461	957	1618	1463	268	4619	0	595	3303	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			121			78		27			1	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	2		4	4		2	8		12	12		8
Confl. Bikes (#/hr)									4			11
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	2%	3%	10%	2%	3%	2%	8%	10%	2%	12%
Adj. Flow (vph)	41	77	316	232	34	102	40	595	81	37	1111	13
Shared Lane Traffic (%)	• • • • • • • • • • • • • • • • • • • •		0.0		•				•	•		
Lane Group Flow (vph)	41	77	316	232	34	102	40	676	0	37	1124	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5		20.0	7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane		0.0						0.0				
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	• •
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel	OI LX	OI · EX	OI · LX	OI · EX	OI · LX	OI LX	OI LX	OI · EX		OI · EX	OI LX	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7	0.0	0.0	28.7	0.0	0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		CITEX			OITEX			OITEX			OITEX	
		0.0			0.0			0.0			0.0	
Detector 2 Extend (s) Turn Type	Perm	NA	Perm	nmint	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	rellli	NA 4	reilli	pm+pt	NA 8	reiiii	reilli	NA 2		reilli	NA 6	
	A	4	4	3	Ŏ	0	0	Z			Ö	
Permitted Phases	4	4	4	8	0	8	2	0		6		
Detector Phase	4	4	4	3	8	8	2	2		6	6	

	۶	→	•	•	←	•	4	†	/	/	ļ	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	20.9	20.9	20.9	37.1	35.9	35.9	45.9	45.9		45.9	45.9	
Actuated g/C Ratio	0.22	0.22	0.22	0.39	0.38	0.38	0.48	0.48		0.48	0.48	
v/c Ratio	0.14	0.20	0.76	0.53	0.06	0.17	0.31	0.30		0.13	0.70	
Control Delay	26.5	27.8	31.6	23.5	15.3	5.6	29.7	16.5		20.1	25.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	26.5	27.8	31.6	23.5	15.3	5.6	29.7	16.5		20.1	25.0	
LOS	С	С	С	С	В	Α	С	В		С	С	
Approach Delay		30.5			17.8			17.3			24.9	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	5.8	11.0	32.2	28.4	3.7	2.6	3.8	22.1		3.2	71.5	
Queue Length 95th (m)	10.4	16.5	45.3	30.7	6.4	8.2	#18.4	42.1		12.1	#149.8	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	504	674	644	440	887	837	129	2244		287	1595	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.08	0.11	0.49	0.53	0.04	0.12	0.31	0.30		0.13	0.70	

Other

Area Type: Cycle Length: 95

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76 Intersection Signal Delay: 22.8

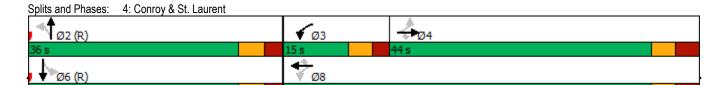
Intersection Capacity Utilization 84.5%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.			ર્ન
Traffic Volume (vph)	23	56	107	76	151	51
Future Volume (vph)	23	56	107	76	151	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.904		0.944			
Flt Protected	0.986					0.964
Satd. Flow (prot)	1481	0	1641	0	0	1588
Flt Permitted	0.986					0.964
Satd. Flow (perm)	1481	0	1641	0	0	1588
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				2	2	
Confl. Bikes (#/hr)		1		4		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	23	56	107	76	151	51
Shared Lane Traffic (%)						
Lane Group Flow (vph)	79	0	183	0	0	202
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	J -	0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizati	ion 37.7%			IC	J Level of	Service A

Intersection Capacity Utili Analysis Period (min) 15

APPENDIX L

Transportation Demand Management

TDM-Supportive Development Design and Infrastructure Checklist:

Residential Developments (multi-family or condominium)

Legend							
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed						
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users						
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance						

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	□ - n/a
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	\square
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	upportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	☑ - Garage is provided for each unit
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	☑ - Garage is provided for each unit
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	☑ - Garage is provided for each unit
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	□ - n/a
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	,
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	_
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	

TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

EASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC *	1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

		TDM	measures: Residential developments	Check if proposed & add descriptions
		3.	TRANSIT	
		3.1	Transit information	
BASIC		3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	\square
BETTER		3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
		3.2	Transit fare incentives	
BASIC	*	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER		3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
		3.3	Enhanced public transit service	
BETTER	*	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
		3.4	Private transit service	
BETTER		3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
		4.	CARSHARING & BIKESHARING	
		4.1	Bikeshare stations & memberships	
BETTER		4.1.1	Contract with provider to install on-site bikeshare station (<i>multi-family</i>)	
BETTER		4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	
		4.2	Carshare vehicles & memberships	
BETTER		4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER		4.2.2	Provide residents with carshare memberships, either free or subsidized	
		5.	PARKING	
		5.1	Priced parking	
BASIC	*	5.1.1	Unbundle parking cost from purchase price	
			(condominium)	

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	TDM	measures: Residential developments	Check if proposed & add descriptions
	6.	TDM MARKETING & COMMUNICATIONS	
	6.1	Multimodal travel information	
BASIC	★ 6.1.1	Provide a multimodal travel option information package to new residents	
	6.2	Personalized trip planning	
BETTER	★ 6.2.1	Offer personalized trip planning to new residents	

APPENDIX M

MMLOS Analysis

Segment MMLOS Analysis

This section provides a review of the boundary streets Walkley Road, Conroy Road, St. Laurent Boulevard, and Don Reid Drive, using complete streets principles. The *Multi-Modal Level of Service (MMLOS) Guidelines*, produced by IBI Group in October 2015, were used to evaluate the levels of service for each alternative mode of transportation on the boundary streets. As each boundary street is located within both the General Urban Area and Urban Employment Area, whichever target is more stringent has been considered.

Exhibit 4 of the *MMLOS Guidelines* has been used to evaluate the segment pedestrian level of service (PLOS) of the boundary streets. Exhibit 22 of the *MMLOS Guidelines* identifies a target PLOS C for all roadways in the General Urban Area or Employment Area. The results of the segment PLOS analysis are summarized in **Table 1**.

Exhibit 11 of the *MMLOS Guidelines* has been used to evaluate the segment bicycle level of service (BLOS) of the boundary streets. In the General Urban Area, Exhibit 22 of the *MMLOS Guidelines* identifies a target BLOS B for roadways with a Crosstown Bikeway designation (Walkley Road, Conroy Road) or Local Route designation (St. Laurent Boulevard, Don Reid Drive). The results of the segment BLOS analysis are summarized in **Table 2**.

Exhibit 15 of the *MMLOS Guidelines* has been used to evaluate the segment transit level of service (TLOS) of the boundary streets. Within the General Urban Area or Employment Area, Exhibit 22 of the *MMLOS Guidelines* identifies a target TLOS B for roadways with a Rapid Transit Corridor designation in the RTTP Network Concept (Walkley Road), and a target TLOS D for roadways with a Transit Priority with Isolated Measures designation (Conroy Road). St. Laurent Boulevard and Don Reid Drive have not been evaluated for segment TLOS. The results of the segment TLOS analysis are summarized in **Table 3**.

Exhibit 20 of the *MMLOS Guidelines* has been used to evaluate the segment truck level of service (TkLOS) of the boundary streets. Within the Employment Area, Exhibit 22 of the *MMLOS Guidelines* identifies a target TkLOS B for arterial roadways with a truck route designation (Walkley Road, Conroy Road), and a target TkLOS D for collector roadways without a truck route designation (St. Laurent Boulevard, Don Reid Drive). The results of the segment TkLOS analysis are summarized in **Table 4**.

Table 1: PLOS Segment Analysis

Sidewalk Width	Boulevard Width	Avg. Daily Curb Lane Traffic Volume		Operating Speed ⁽¹⁾	PLOS
Walkley Road	l (north side, F	Ryder Street to 160m We	est of Conroy Road)		
1.5m	> 2.0m	> 3,000 vpd	No	60 km/h	E
Walkley Road	l (south side, l	Don Reid Drive to 160m	West of Conroy Roa	ad)	
≥ 2.0m	0m	> 3,000 vpd	No	60 km/h	E
Conroy Road	(east side, Wa	alkley Road to St. Laure	nt Boulevard)		
1.5m	> 2.0m	> 3,000 vpd	No	70 km/h	E
Conroy Road	(west side, W	alkley Road to St. Laure	ent Boulevard)		
≥ 2.0m	> 2.0m	> 3,000 vpd	No	70 km/h	D
St. Laurent B	oulevard (nort	h side, Don Reid Drive t	to Conroy Road)		
1.5m	0.5 to 2.0m	<u><</u> 3,000 vpd	No	60 km/h	С
St. Laurent B	oulevard (sou	th side, Don Reid Drive	to Conroy Road)		
No sid	lewalk	<u><</u> 3,000 vpd	No	60 km/h	F
Don Reid Driv	e (east side, \	Walkley Road to St. Lau	rent Boulevard)		
1.5m	0.5 to 2.0m	≤ 3,000 vpd	No	60 km/h	С
Don Reid Driv	ve (west side,	Walkley Road to St. Lau	rent Boulevard)		
No sid	lewalk	≤ 3,000 vpd	No	60 km/h	F

^{1.} Operating speed taken as the speed limit plus 10 km/h.

Table 2: BLOS Segment Analysis

Road Class	Route Type	Bikeway Type	Travel Lanes	Operating Speed	Bike Lane Width	Bike Lane Blockage	BLOS							
Walkley Roa	Walkley Road (Don Reid Drive/Ryder Street to 160m West of Conroy Road)													
Arterial	Crosstown	Mixed	4	60 km/h	N/A	N/A	F							
	Bikeway	Traffic	-		,, .	. 4// 1	•							
Conroy Road	d (Walkley Ro	<u>ad to St. Laur</u>	ent Boule	vard)										
		Curbside 4 to 5		70 km/h	> 1.8m	Rare	Е							
Arterial	Crosstown Bikeway	Bike Lane	7 10 0	7 0 1011/11	<u> </u>	raio	_							
Aiteriai		Mixed-Use Pathway	N/A	N/A	N/A	N/A	Α							
St. Laurent E	Soulevard (Do	n Reid Drive	to Conroy	Road)										
Collector	Local Route	Mixed Traffic	2	60 km/h	N/A	N/A	F							
Don Reid Dri	ive (Walkley F	Road to St. La	urent Bou	levard)										
Collector	Local Route	Mixed Traffic	2	60 km/h	N/A	N/A	F							

Table 3: TLOS Segment Analysis

Facility Type	Exposure to Cong	TLOS										
racility Type	Congestion	Friction	Incident Potential	ILUS								
Walkley Road (Don Reid Dri	Walkley Road (Don Reid Drive/Ryder Street to 160m West of Conroy Road)											
Mixed Traffic – Limited	Yes	Low	Medium	ר								
Parking/Driveway Friction	162	Low	Medium	D								
Conroy Road (Walkley Road	to St. Laurent Bou	ılevard)										
Mixed Traffic – Limited	Yes	Low	Medium	ר								
Parking/Driveway Friction	162	Low	Medium	D								

Table 4: TkLOS Segment Analysis

. and if the second first that yell								
Curb Lane Width	TkLOS							
Walkley Road (Don Reid Drive/Ryder Street to 160m West of Conroy Road)								
> 3.7m	2	Α						
Conroy Road (Walkley Road to	St. Laurent Boulevard)							
3.5 to 3.7m	2 to 3	Α						
St. Laurent Boulevard (Don Re	id Drive to Conroy Road)							
> 3.7m	1	В						
Don Reid Drive (Walkley Road	Don Reid Drive (Walkley Road to St. Laurent Boulevard)							
> 3.7m	1	В						

Intersection MMLOS Analysis

The following is a review of the MMLOS of the signalized intersections within the study area (Walkley Road/Don Reid Drive/Ryder Street, Walkley Road/160m West of Conroy Road, Walkley Road/Conroy Road, and St. Laurent Boulevard/Conroy Road), using complete streets principles. All of these intersections have been evaluated based on existing conditions, using the MMLOS targets for intersections within the General Urban Area or Employment Area, whichever are stricter.

Exhibit 5 of the *Addendum to the MMLOS Guidelines* has been used to evaluate the existing PLOS at the intersections listed above. Exhibit 22 of the *MMLOS Guidelines* identifies a target PLOS C for all roadways in the General Urban Area or Employment Area. The results of the intersection PLOS analysis are summarized in **Table 5** through **Table 8**.

Exhibit 12 of the *MMLOS Guidelines* has been used to evaluate the existing BLOS at the intersections listed above. In the General Urban Area, Exhibit 22 of the *MMLOS Guidelines* identifies a target BLOS B for Crosstown Bikeways (Conroy Road, Walkley Road), and a target BLOS B for Local Cycling Routes (Don Reid Drive, Ryder Street, St. Laurent Boulevard). The results of the intersection BLOS analysis are summarized in **Table 9**.

Exhibit 16 of the *MMLOS Guidelines* has been used to evaluate the existing TLOS at the intersections listed above. Exhibit 22 of the *MMLOS Guidelines* identifies a target TLOS B for Rapid Transit Corridors (Walkley Road) and a target TLOS D for Transit Priority Corridors with Isolated Measures (Conroy Road). The TLOS has been evaluated for every approach that is currently utilized by transit at the study area intersections. The results of the intersection TLOS analysis are summarized in **Table 10**.

Exhibit 21 of the *MMLOS Guidelines* has been used to evaluate the existing TkLOS at the intersections listed above. In the Employment Area, Exhibit 22 of the *MMLOS Guidelines* identifies a target TkLOS B for arterial truck routes (Walkley Road, Conroy Road), a target TkLOS D for collector roadways without a truck route designation (St. Laurent Boulevard, Don Reid Drive), and a target TkLOS E for local roadways without a truck route designation (Ryder Street). The results of the intersection TkLOS analysis are summarized in **Table 11**.

Table 5: PLOS Intersection Analysis – Walkley Road/Don Reid Drive/Ryder Street

CRITERIA	North Approach		South Approach		East Approach		West Approach	
			PETSI SCORE	:				
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	72	No	39	No	39	No	
Lanes Crossed (3.5m Lane Width)	5	/2	7	39	7	39	9	6
SIGNAL PHASING AND TIMING				•				
Left Turn Conflict	Permissive	-8	Permissive	-8	Permissive	-8	Permissive	-8
Right Turn Conflict	Permissive or Yield	-5						
Right Turn on Red	RTOR Allowed	-3						
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS		•				-		•
Parallel Radius	> 10m to 15m	-6						
Parallel Right Turn Channel	No Right Turn Channel	-4						
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Standard	-7	Standard	-7	Standard	-7	Standard	-7
	PETSI SCORE	37		4		4		-29
	LOS	E		F		F		F
			DELAY SCORI	E				
Cycle Length		100		100		110		110
Pedestrian Walk Time		43.1		43.1		10.1		10.1
	DELAY SCORE	16.2		16.2		45.4		45.4
	LOS	В		В		E		E
	OVERALL	E		F		F		F

Table 6: PLOS Intersection Analysis - Walkley Road/160m West of Conroy Road

CRITERIA	North Approach		South Approach		East Approach		West Approach	
STATI ETABLE	itorai Approaen				Zuct / tpp/ cucii		Woot / tpp/ odd//	
			PETSI SCORE					
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	N/A	0	No	55	No	39	No	39
Lanes Crossed (3.5m Lane Width)	N/A	U	6	33	7	55	7	55
SIGNAL PHASING AND TIMING								
Left Turn Conflict	N/A	0	Permissive	-8	No Left Turn/Prohibited	0	Permissive	-8
Right Turn Conflict	N/A	0	Permissive or Yield	-5	Permissive or Yield	-5	No Right Turn/Prohibited	0
Right Turn on Red	N/A	0	RTOR Allowed	-3	N/A	0	RTOR Allowed	-3
Leading Pedestrian Interval	N/A	0	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	N/A	0	> 10m to 15m	-6	> 10m to 15m	-6	No Right Turn	0
Parallel Right Turn Channel	N/A	0	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn	0
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	N/A	0	Standard	-7	Standard	-7	Standard	-7
	PETSI SCORE	-		20		15		19
	LOS	-		F		F		F
	-		DELAY SCORE				•	
Cycle Length		0		100		110		110
Pedestrian Walk Time		0.0		52.0		7.0		7.0
	DELAY SCORE	-		11.5		48.2		48.2
	LOS	-		В		E		E
	OVERALL	-		F		F		F

Table 7: PLOS Intersection Analysis - Walkley Road/Conroy Road

CRITERIA	North Approach		South Approach		East Approach		West Approach	
			PETSI SCORE					
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	N/A	0	No	-10	No	6	N/A	
Lanes Crossed (3.5m Lane Width)	N/A	0	10 +	-10	9	7 ° [N/A	0
SIGNAL PHASING AND TIMING								
Left Turn Conflict	N/A	0	Protected	0	No Left Turn/Prohibited	0	N/A	0
Right Turn Conflict	N/A	0	Permissive or Yield	-5	Permissive or Yield	-5	N/A	0
Right Turn on Red	N/A	0	N/A	0	N/A	0	N/A	0
Leading Pedestrian Interval	N/A	0	No	-2	No	-2	N/A	0
CORNER RADIUS	·			•		•		
Parallel Radius	N/A	0	> 15m to 25m	-8	> 15m to 25m	-8	N/A	0
Parallel Right Turn Channel	N/A	0	Conventional without Receiving	0	Conventional with Receiving	-3	N/A	0
Perpendicular Radius	N/A	0	> 15m to 25m	-8	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	Conventional with Receiving	-3	N/A	0	N/A	0
CROSSING TREATMENT	-							
Treatment	N/A	0	Standard	-7	Standard	-7	N/A	0
	PETSI SCORE	-		-43		-19		-
	LOS	-		F		F		-
			DELAY SCORE					
Cycle Length		0		120		120		0
Pedestrian Walk Time		0.0		28.6		16.6		0.0
	DELAY SCORE	-		34.8	_	44.5	_	-
	LOS	-		D		E		-
	OVERALL			F		F		

Table 8: PLOS Intersection Analysis – St. Laurent Boulevard/Conroy Road

CRITERIA	North Approach		South Approach		East Approach		West Approach						
	PETSI SCORE												
CROSSING DISTANCE CONDITIONS													
Median > 2.4m in Width	No	-10	No	-10	No	55	No	55					
Lanes Crossed (3.5m Lane Width)	10 +	-10	10 +	-10	6		6	55					
SIGNAL PHASING AND TIMING													
Left Turn Conflict	Permissive	-8	Perm + Prot	-8	Permissive	-8	Permissive	-8					
Right Turn Conflict	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5					
Right Turn on Red	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3	N/A	0					
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2					
CORNER RADIUS													
Parallel Radius	> 10m to 15m	-6	> 15m to 25m	-8	> 10m to 15m	-6	> 10m to 15m	-6					
Parallel Right Turn Channel	No Right Turn Channel	-4	Conventional without Receiving	0	No Right Turn Channel	-4	No Right Turn Channel	-4					
Perpendicular Radius	N/A	0	N/A	0	N/A	0	> 15m to 25m	-8					
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	Conventional without Receiving	0					
CROSSING TREATMENT													
Treatment	Standard	-7	Standard	-7	Standard	-7	Standard	-7					
	PETSI SCORE	-45		-43		20		15					
	LOS	F		F		F		F					
			DELAY SCORE										
Cycle Length		100		100		95		95					
Pedestrian Walk Time		7.1		7.1		12.7		12.7					
	DELAY SCORE	43.2		43.2		35.6		35.6					
	LOS	E		E		D		D					
	OVERALL	F		F		F		F					

Table 9: BLOS Intersection Analysis

Approach	Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
Walkley Road/D			Traver Lanes andror Speed	DLOS
Trainiey Noau/D	OII NEIG DINGI	Right Turn Lane		
North Approach	Mixed Traffic	Characteristics	Shared through/right turn lane	Α
		Left Turn Accommodation	One lane crossed; 50 km/h	D
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	Α
South Approach	Wilked Traffic	Left Turn Accommodation	One lane crossed; ≥ 60 km/h	F
East Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	Α
Last Approach	WIIAGG FRAIIIG	Left Turn Accommodation	Two lanes crossed; ≥ 50 km/h	F
West Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	Α
		Left Turn Accommodation	Two lanes crossed; ≥ 50 km/h	F
Walkley Road/1	60m West of Co			
Couth Approach	Mixed Troffic	Right Turn Lane Characteristics	Right turn lane < 50m; turning speed ≤ 25 km/h	D
South Approach	Mixed Traffic	Left Turn Accommodation	One lane crossed; ≤ 40 km/h (private approach, low speed assumed)	В
Foot Approach	Mixed Traffic	Right Turn Lane Characteristics	No right turn	-
East Approach	wiixed Hailic	Left Turn Accommodation	Two lanes crossed; ≥ 50 km/h	F
West Approach	Pocket	Right Turn Lane Characteristics	Right turn lane ≤ 50m, and is introduced to the right	В
	Bike Lane	Left Turn Accommodation	No left turn	-
Walkley Road/C	onroy Road			
-	Pocket	Right Turn Lane Characteristics	Bike lane shifts to the left; turning speed ≤ 25 km/h	D
South Approach	Bike Lane	Left Turn Accommodation	Dual left turn lanes	F
East Approach	Mixed Traffic	Right Turn Lane Characteristics	No right turn	-
East Approach	wiixea Hailic	Left Turn Accommodation	Dual left turn lanes	F
Most Approach Pocket		Right Turn Lane Characteristics	Right turn lane > 50m, and is introduced to the right	D
West Approach	Bike Lane	Left Turn Accommodation	No left turn	-

Approach	Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
St. Laurent Bou	levard/Conroy	Road		
North Approach	Curbside	Right Turn Lane Characteristics	Shared through/right turn lane	Α
Попт Арргоаст	Bike Lane	Left Turn Accommodation	Three lanes crossed; ≥ 50 km/h	F
South Approach	Curbside	Right Turn Lane Characteristics	Shared through/right turn lane	Α
South Approach	Bike Lane	Left Turn Accommodation	Three lanes crossed; > 50 km/h	F
Foot Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane ≤ 50m; turning speed ≤ 25 km/h	D
East Approach	Mixed Framic	Left Turn Accommodation	One lane crossed; ≥ 60 km/h	F
West Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane < 50m; turning speed ≤ 25 km/h	D
West Apploach	WIINEU HAIIIC	Left Turn Accommodation	One lane crossed; ≥ 60 km/h	F

Table 10: TLOS Intersection Analysis

Table 10: 1LOS Intersection	n Anaiysis		
Approach	De	lay ⁽¹⁾	TLOS
Арргоасп	AM Peak Hour	PM Peak Hour	ILUS
Walkley Road/Don Reid Dr	ive/Ryder Street		
North Approach	30 sec	40 sec	E
East Approach	7 sec	10 sec	В
West Approach	8 sec	13 sec	С
Walkley Road/160m West of	of Conroy Road		
East Approach	9 sec	8 sec	В
West Approach	3 sec	2 sec	В
Walkley Road/Conroy Road	d		
South Approach	42 sec	36 sec	F
East Approach	23 sec	28 sec	D
West Approach	23 sec	31 sec	П
St. Laurent Boulevard/Con	roy Road		
North Approach	13 sec	28 sec	О
South Approach	11 sec	19 sec	С
East Approach	25 sec	18 sec	D

^{1.} Delay based on outputs from Synchro analysis of existing conditions

Table 11: TkLOS Intersection Analysis

Approach	Effective Corner Radius	Number of Receiving Lanes Departing Intersection	TkLOS
Walkley Road/Don Re	eid Drive/Ryder Street		
North Approach	10m to 15m	2	В
South Approach	10m to 15m	2	В
East Approach	10m to 15m	1	E
West Approach	10m to 15m	1	E
Walkley Road/160m V	Vest of Conroy Road		
South Approach	10m to 15m	2	В
West Approach	10m to 15m	1	E
Walkley Road/Conroy	/ Road		
South Approach	> 15m	3	Α
West Approach	> 15m	2	Α
St. Laurent Boulevard	d/Conroy Road		
North Approach	10m to 15m	1	E
South Approach	10m to 15m	1	E
East Approach	10m to 15m	3	В
West Approach	> 15m	2	Α

APPENDIX N

Total Synchro Analysis

	۶	→	•	•	←	1	1	†	/	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	↑ 1≽		*	† \$		7	ĵ.		*	^	•
Traffic Volume (vph)	29	925	173	46	1410	195	97	7	60	49	47	39
Future Volume (vph)	29	925	173	46	1410	195	97	7	60	49	47	39
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0			30.0			25.0			30.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.99		1.00	1.00		1.00	0.98		1.00	0.99	
Frt		0.976			0.982			0.866			0.932	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1537	3169	0	1642	3166	0	1580	1367	0	1674	1630	0
Flt Permitted	0.119		•	0.237		•	0.701		-	0.713		•
Satd. Flow (perm)	193	3169	0	408	3166	0	1161	1367	0	1251	1630	0
Right Turn on Red	100	0100	Yes	100	0100	Yes	1101	1001	Yes	1201	1000	Yes
Satd. Flow (RTOR)		37			26			60			39	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
Travel Time (s)		28.9			12.3			7.2			20.2	
Confl. Peds. (#/hr)	5	20.5	10	10	12.0	5	5	1.2	5	5	20.2	5
Confl. Bikes (#/hr)	J		3	10		3	0		3	J		J
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	10%	4%	1%	3%	5%	1%	7%	1%	12%	1%	1%	1.00
Adj. Flow (vph)	29	925	173	46	1410	195	97	7	60	49	47	39
Shared Lane Traffic (%)	29	920	173	40	1410	190	31	I	00	43	41	39
Lane Group Flow (vph)	29	1098	0	46	1605	0	97	67	0	49	86	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
	L NA	Left	R NA	Left	Left		L NA	Left	R NA	L NA	Left	R NA
Lane Alignment	LINA	5.0	RINA	Leit	5.0	Right	LINA	3.5	KINA	LINA	3.5	KINA
Median Width(m) Link Offset(m)		0.0			0.0			0.0			0.0	
		5.0			5.0			5.0			5.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24	_	14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	

	۶	→	•	•	+	•	4	†	_	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	64.0	64.0		64.0	64.0		36.0	36.0		36.0	36.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		36.0%	36.0%		36.0%	36.0%	
Maximum Green (s)	58.1	58.1		58.1	58.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	5	5		5	5		5	5		5	5	
Act Effct Green (s)	76.1	76.1		76.1	76.1		16.2	16.2		16.2	16.2	
Actuated g/C Ratio	0.76	0.76		0.76	0.76		0.16	0.16		0.16	0.16	
v/c Ratio	0.20	0.45		0.15	0.66		0.52	0.25		0.24	0.29	
Control Delay	11.6	7.3		4.8	5.4		46.2	11.9		36.4	22.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	11.6	7.3		4.8	5.4		46.2	11.9		36.4	22.1	
LOS	В	Α		Α	Α		D	В		D	С	
Approach Delay		7.4			5.4			32.2			27.3	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	1.3	32.5		1.3	26.7		16.5	1.1		8.0	7.6	
Queue Length 95th (m)	8.4	76.6		m2.4	29.6		25.8	9.6		14.5	16.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	147	2421		310	2416		345	449		372	513	
Starvation Cap Reductn	0	0		0	18		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.20	0.45		0.15	0.67		0.28	0.15		0.13	0.17	

Intersection Summary

Other

Area Type: Cycle Length: 100

Actuated Cycle Length: 100

Offset: 94 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

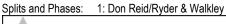
Maximum v/c Ratio: 0.66

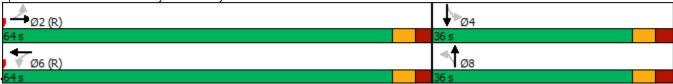
Intersection Signal Delay: 8.5 Intersection Capacity Utilization 72.1%

Intersection LOS: A ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.





Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 10 10 5 5 Confl. Bikes (#/hr) 3 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Heavy Vehicles (%) 5% 1% 1% 6% 1% 2% Adj. Flow (vph) 954 90 74 1595 66 60 Shared Lane Traffic (%) Lane Group Flow (vph) 954 90 74 1595 66 60 Enter Blocked Intersection No No No No No No No Lane Alignment Left Right L NA Left L NA R NA Median Width(m) 3.5 5.0 3.5 Link Offset(m) 0.0 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 1.09 1.09 Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.		→	•	•	←	4	/
Lane Configurations	Lane Group	FRT	FRR	WRI	WRT	NRI	NRR
Traffic Volume (vph) 954 90 74 1595 66 60 Future Volume (vph) 954 90 74 1595 66 60 Educal Flow (vphpl) 1800 1800 1800 1800 1800 1800 Storage Length (m) 20.0 65.0 30.0 0.0 Storage Length (m) 22.0 65.0 30.0 0.0 Storage Lanes 1 1 1 1 1 Taper Length (m) 22.0 30.0 0.9 Lane Util. Factor 0.95 1.00 1.00 0.95 1.00 1.00 Ped Bike Factor 0.96 1.00 0.99 0.99 Fit Protected 0.850 0.950 0.950 Satd. Flow (prot) 3221 1498 1674 3191 1674 1483 Fit Permitted 0.290 0.950 Satd. Flow (prom) 3221 1442 509 3191 1663 1456 Right Turn on Red Yes 71 600 Satd. Flow (RTOR) 71 600 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#hr) 3 12.2 9.3 Confl. Peds. (#hr) 3 10 10 5 5 5 Confl. Peds. (#hr) 3 10 10 10 5 5 5 Shared Lane Traffic (%) 1% 1% 6% 1% 2% Adj. Flow (vph) 954 90 74 1595 66 60 Enter Blocked Intersection Lon No							
Future Volume (vph) 954 90 74 1595 66 60 (deal Flow (vphpl) 1800 1800 1800 1800 1800 0.0 0.0 Storage Lanes 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							
Ideal Flow (vphpt)							
Storage Length (m)							
Storage Lanes		1000			1000		
Taper Length (m)							
Lane Util. Factor 0.95 1.00 1.00 0.95 1.00 1.00 Ped Bike Factor 0.96 1.00 0.99 0.99 0.985 Fit Protected 0.950 0.950 0.950 Satol. Flow (prot) 3221 1498 1674 3191 1674 1483 Fit Permitted 0.290 0.950 0.950 Satd. Flow (perm) 3221 1442 509 3191 1663 1456 Right Turn on Red Yes 71 60							I
Ped Bike Factor		0.05	1.00		0.05		1.00
Fit Protected		0.95			0.95		
Fit Protected 0.950 0.950 0.950 Satd. Flow (prot) 3221 1498 1674 3191 1674 1483 1514 1674 1483 1674 3191 1674 1483 1514 1674 1483 1514 1674 1483 1514 1674 1483 1514 1674 1483 1514 1674 1483 1514 1674 1483 1514 1674 1483 1514 1674 1483 1514 1691 1286 1456 1684 1456 1456 1684 1456 145				1.00		0.99	
Satd. Flow (prot) 3221			0.850	0.050		0.050	0.850
Fit Permitted		2004	4 400		0.10.1		4.400
Satd. Flow (perm) 3221 1442 509 3191 1663 1456 14	,	3221	1498		3191		1483
Right Turn on Red							
Satid. Flow (RTOR) 71 50 50 50 50 50 50 50 5		3221		509	3191	1663	
Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 3 10 10 5 5 Confl. Bikes (#/hr) 3							
Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 10 10 5 5 Confl. Bikes (#/hr) 3 10 1.00	Satd. Flow (RTOR)		71				60
Travel Time (s)	Link Speed (k/h)						
Travel Time (s)	Link Distance (m)	171.1			169.1	128.6	
Confl. Peds. (#/hr) 10 10 5 5 Confl. Bikes (#/hr) 3	Travel Time (s)						
Confl. Bikes (#/hr) 3 Peak Hour Factor 1.00	Confl. Peds. (#/hr)		10	10			5
Peak Hour Factor							
Heavy Vehicles (%)		1.00		1.00	1.00	1.00	1.00
Adj. Flow (vph) 954 90 74 1595 66 60 Shared Lane Traffic (%) Lane Group Flow (vph) 954 90 74 1595 66 60 Enter Blocked Intersection No							
Shared Lane Traffic (%) Lane Group Flow (vph) 954 90 74 1595 66 60							
Lane Group Flow (vph) 954 90 74 1595 66 60		304	30	14	1090	00	00
Enter Blocked Intersection No No <th< td=""><td></td><td>054</td><td>00</td><td>7.1</td><td>1505</td><td>66</td><td>60</td></th<>		054	00	7.1	1505	66	60
Lane Alignment Left Right R NA Left L NA R NA Median Width(m) 3.5 5.0 3.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09	,						
Median Width(m) 3.5 5.0 3.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Leading Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Leading Detector (m) 0.0							
Link Offset(m) 0.0 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 </td <td></td> <td></td> <td>Right</td> <td>LNA</td> <td></td> <td></td> <td>RNA</td>			Right	LNA			RNA
Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Two way Left Turn Lane Headway Factor 1.09 1.00 0.							
Headway Factor		5.0			5.0	5.0	
Turning Speed (k/h) Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel Detector 1 Extend (s) Detector 1 Queue (s) Detector 1 Delay (s) Detector 2 Position(m) 28.7 28.7 Detector 2 Size(m) 1.8 1.8 Detector 2 Type CI+Ex CI+Ex CI+Ex CI	Two way Left Turn Lane						
Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex D.0 0.0	Headway Factor	1.09			1.09		
Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex D.0 0.0	Turning Speed (k/h)		14	24		24	14
Detector Template	Number of Detectors	2	1	1	2	1	1
Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex D.0 0.0 <t< td=""><td>Detector Template</td><td>Thru</td><td>Right</td><td>Left</td><td>Thru</td><td>Left</td><td>Right</td></t<>	Detector Template	Thru	Right	Left	Thru	Left	Right
Trailing Detector (m) 0.0	•						
Detector 1 Position(m) 0.0							
Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex D.0 0.0							
Detector 1 Type CI+Ex							
Detector 1 Channel Detector 1 Extend (s) 0.0 1.8 0.0 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Detector 1 Extend (s) 0.0		UI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Queue (s) 0.0		2.2	0.0	0.0	0.0	0.0	
Detector 1 Delay (s) 0.0							
Detector 2 Position(m) 28.7 28.7 Detector 2 Size(m) 1.8 1.8 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8							
Detector 2 Size(m) 1.8 1.8 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8			0.0	0.0		0.0	0.0
Detector 2 Type CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 Perm Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8	Detector 2 Position(m)						
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 Permitted Phases 2 6 8	Detector 2 Size(m)						
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8	Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Extend (s) 0.0 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8	Detector 2 Channel						
Turn TypeNAPermPermNAProtPermProtected Phases268Permitted Phases268		0.0			0.0		
Protected Phases 2 6 8 Permitted Phases 2 6 8			Perm	Perm		Prot	Perm
Permitted Phases 2 6 8			. 5	. 31111			. 51111
			2	6	- 0	- 0	ρ
Detector 1 1103C		2			6	Q	
	DEIGCIOI FIIASE		Z	U	U	0	0

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0
Total Split (s)	71.0	71.0	71.0	71.0	29.0	29.0
Total Split (%)	71.0%	71.0%	71.0%	71.0%	29.0%	29.0%
Maximum Green (s)	65.0	65.0	65.0	65.0	23.0	23.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)	18.0	18.0			7.0	7.0
Flash Dont Walk (s)	13.0	13.0			16.0	16.0
Pedestrian Calls (#/hr)	5	5			5	5
Act Effct Green (s)	80.2	80.2	80.2	80.2	11.3	11.3
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.11	0.11
v/c Ratio	0.37	0.08	0.18	0.62	0.35	0.28
Control Delay	3.0	0.9	5.9	7.6	43.6	12.6
Queue Delay	0.0	0.0	0.0	0.4	0.0	0.0
Total Delay	3.0	0.9	5.9	8.0	43.6	12.6
LOS	A	A	A	A	D	В
Approach Delay	2.8			7.9	28.8	
Approach LOS	A			A	C	
Queue Length 50th (m)	14.8	0.2	2.6	50.0	11.3	0.0
Queue Length 95th (m)	21.8	0.9	11.2	121.1	19.6	8.9
Internal Link Dist (m)	147.1			145.1	104.6	
Turn Bay Length (m)		20.0	65.0		30.0	
Base Capacity (vph)	2583	1170	408	2560	385	381
Starvation Cap Reductn	0	0	0	443	0	0
Spillback Cap Reductn	0	0	0	26	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.08	0.18	0.75	0.17	0.16
	0.01	0.00	0.10	0.70	V. 17	0.10
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 100						
Offset: 88 (88%), Referenced	to phase 2:E	BT and 6:\	NBTL, Sta	rt of Green		
Natural Cycle: 75						
Control Type: Actuated-Coord	dinated					
Maximum v/c Ratio: 0.62						
Intersection Signal Delay: 6.9				In	tersection	LOS: A
Intersection Capacity Utilization	on 63.0%			IC	U Level of	Service B
Analysis Period (min) 15						
Splits and Phases: 2: 160m	W of Conroy	& Walkley	/			
	•					
● Ø2 (R)						
71s						
4-						
∮ Ø6 (R)						
71 s						

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	*	T.	ሻሻ	^	1,50	ሻሻ	7
Traffic Volume (vph)	801	267	250	996	14	683	471
Future Volume (vph)	801	267	250	996	14	683	471
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		15.0	200.0			2	1
Taper Length (m)			50.0			10.0	- I
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
	0.93	0.97		0.95	0.95	0.97	0.98
Ped Bike Factor Frt		0.850	0.99				
		0.850	0.050			0.050	0.850
Fit Protected	2404	1155	0.950	2404	0	0.950	1455
Satd. Flow (prot)	3161	1455	3066	3161	0	3186	1455
Flt Permitted	0404	1400	0.950	0404	^	0.950	4400
Satd. Flow (perm)	3161	1409	3036	3161	0	3186	1423
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		267					355
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		15	15				5
Confl. Bikes (#/hr)		3					3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	7%	4%	7%	7%	2%	3%	4%
Adj. Flow (vph)	801	267	250	996	14	683	471
Shared Lane Traffic (%)							
Lane Group Flow (vph)	801	267	250	996	0	697	471
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	1.09	24	1.00	14	24	1.09
Number of Detectors	2	1	1	2	14	1	1
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
•	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Leading Detector (m)		0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0						
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	
	-				_		0
		2			X		X
Permitted Phases Detector Phase	2	2	1	6	8	8	8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	55.0	55.0	25.0	80.0	40.0	40.0	40.0
Total Split (%)	45.8%	45.8%	20.8%	66.7%	33.3%	33.3%	33.3%
Maximum Green (s)	48.6	48.6	18.8	73.6	33.6	33.6	33.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	2.1	0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead	0.7		0.7	0.4
Lead-Lag Optimize?	Yes	Yes	Yes				
	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)							
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	10	10			5	5	5
Act Effct Green (s)	55.0	55.0	14.9	76.1		31.1	31.1
Actuated g/C Ratio	0.46	0.46	0.12	0.63		0.26	0.26
v/c Ratio	0.55	0.34	0.66	0.50		0.84	0.75
Control Delay	26.5	3.9	58.3	13.2		52.4	17.9
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0
Total Delay	26.5	3.9	58.3	13.2		52.4	17.9
LOS	С	Α	Е	В		D	В
Approach Delay	20.8			22.2		38.5	
Approach LOS	С			С		D	
Queue Length 50th (m)	66.6	0.0	27.0	58.8		72.6	21.3
Queue Length 95th (m)	90.9	14.6	38.0	75.0		92.2	59.1
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1448	790	480	2004		892	654
Starvation Cap Reductn	0	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.55	0.34	0.52	0.50		0.78	0.72
	0.55	0.07	0.02	0.00		0.70	0.12
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 120							
Offset: 43 (36%), Referenced	to phase 2:E	BT and 6:V	VBT, Start	of Green			
Natural Cycle: 85							
Control Type: Actuated-Coord	dinated						
Maximum v/c Ratio: 0.84							
Intersection Signal Delay: 27.	3			In	tersection	LOS: C	
Intersection Capacity Utilization						Service C	,
Analysis Period (min) 15	55. 176			10	2 20101 01	33,7100 0	
Splits and Phases: 3: Conr	oy & Walkley						
_	1						
ÿ1	♥ 🔝 Ø2	(R)					- 1
25 s	55 s						
4							
Ø6 (R)	•						l
	*						

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	*	7	7	*	7	7	ተተ _ጉ		ň	∱ }	
Traffic Volume (vph)	15	69	88	63	46	25	195	1187	199	87	370	57
Future Volume (vph)	15	69	88	63	46	25	195	1187	199	87	370	57
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	0.99		0.97	0.99		0.98	0.98	0.99		0.99	0.99	
Frt	0.00		0.850	0.00		0.850	0.00	0.978		0.00	0.980	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1353	1695	1427	1409	1589	1351	1642	4610	0	1674	3157	0
Flt Permitted	0.727			0.712			0.503		•	0.166	0.00	•
Satd. Flow (perm)	1027	1695	1391	1044	1589	1322	850	4610	0	291	3157	0
Right Turn on Red	1021	1000	Yes	1011	1000	Yes	000	1010	Yes	201	0101	Yes
Satd. Flow (RTOR)			88			36		46	100		24	100
Link Speed (k/h)		50	00		50	00		60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	10	14.2	15	15	13.0	10	20	22.4	25	25	20.3	20
Confl. Bikes (#/hr)	10		3	10		3	20		3	20		25
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	25%	5%	6%	20%	12%	12%	3%	2%	2%	1.00	4%	3%
Adj. Flow (vph)	15	69	88	63	46	25	195	1187	199	87	370	57
Shared Lane Traffic (%)												
Lane Group Flow (vph)	15	69	88	63	46	25	195	1386	0	87	427	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)	V.V	28.7	0.0	0.0	28.7	0.0	0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OI LX			OI · LX			OI LX			OI · LX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	ı Gilli	4	1 51111	1 51111	8	1 51111	1 (31111	2		1 51111	6	
Permitted Phases	4	4	4	8	U	8	2			6	U	
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Delector Filase	4	4	4	0	0	0	Z	Z		O	0	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	43.9	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	44.0	44.0	44.0	56.0	56.0		56.0	56.0	
Total Split (%)	44.0%	44.0%	44.0%	44.0%	44.0%	44.0%	56.0%	56.0%		56.0%	56.0%	
Maximum Green (s)	37.1	37.1	37.1	37.1	37.1	37.1	49.7	49.7		49.7	49.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0	30.0	30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	15	15	15	10	10	10	20	20		15	15	
Act Effct Green (s)	21.1	21.1	21.1	21.1	21.1	21.1	70.4	70.4		70.4	70.4	
Actuated g/C Ratio	0.21	0.21	0.21	0.21	0.21	0.21	0.70	0.70		0.70	0.70	
v/c Ratio	0.07	0.19	0.24	0.29	0.14	0.08	0.33	0.43		0.43	0.19	
Control Delay	24.3	28.7	7.4	31.8	27.7	5.2	13.1	10.4		23.6	8.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	24.3	28.7	7.4	31.8	27.7	5.2	13.1	10.4		23.6	8.6	
LOS	С	С	Α	С	С	Α	В	В		С	Α	
Approach Delay		17.4			25.5			10.7			11.1	
Approach LOS		В			С			В			В	
Queue Length 50th (m)	2.5	11.7	1.7	10.8	7.6	0.0	9.2	25.0		4.5	8.7	
Queue Length 95th (m)	m5.2	17.2	9.9	16.1	12.1	3.5	39.9	73.0		#33.1	29.8	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	381	628	571	387	589	513	598	3257		204	2228	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.04	0.11	0.15	0.16	0.08	0.05	0.33	0.43		0.43	0.19	

Intersection Summary

Other

Area Type: Cycle Length: 100

Actuated Cycle Length: 100

Offset: 23 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.43 Intersection Signal Delay: 12.1

Intersection Capacity Utilization 71.0%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Conroy & St. Laurent



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.			ની
Traffic Volume (vph)	75	59	71	22	101	176
Future Volume (vph)	75	59	71	22	101	176
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.941		0.968			
Flt Protected	0.973					0.982
Satd. Flow (prot)	1547	0	1498	0	0	1669
Flt Permitted	0.973					0.982
Satd. Flow (perm)	1547	0	1498	0	0	1669
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				10	10	
Confl. Bikes (#/hr)		4		3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	7%	15%	15%	6%	4%
Adj. Flow (vph)	75	59	71	22	101	176
Shared Lane Traffic (%)						
Lane Group Flow (vph)	134	0	93	0	0	277
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	Ĭ	0.0	Ĭ		0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizati	on 37 2%			ICI	I I evel of	Service A

Intersection Capacity Utilization 37.2% Analysis Period (min) 15 ICU Level of Service A

	•	→	←	•	\	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ન	1 >		W	
Traffic Volume (vph)	0	128	135	5	13	0
Future Volume (vph)	0	128	135	5	13	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.995			
Flt Protected					0.950	
Satd. Flow (prot)	0	1618	1615	0	1674	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	1618	1615	0	1674	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		40.4	197.7		90.8	
Travel Time (s)		2.9	14.2		6.5	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	10%	10%	1%	1%	1%
Adj. Flow (vph)	0	128	135	5	13	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	128	140	0	13	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	3.5		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 19.4%

ICU Level of Service A

Analysis Period (min) 15

	•	→	—	•	/	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	ĵ.		W	
Traffic Volume (vph)	0	123	133	2	5	1
Future Volume (vph)	0	123	133	2	5	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.998		0.977	
Flt Protected					0.960	
Satd. Flow (prot)	0	1618	1617	0	1653	0
Flt Permitted					0.960	
Satd. Flow (perm)	0	1618	1617	0	1653	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		137.6	40.4		92.1	
Travel Time (s)		9.9	2.9		6.6	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	10%	10%	1%	1%	1%
Adj. Flow (vph)	0	123	133	2	5	1
Shared Lane Traffic (%)		•				
Lane Group Flow (vph)	0	123	135	0	6	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized Intersection Capacity Utilization 19.2% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		î,			ર્ન
Traffic Volume (vph)	1	15	139	0	6	252
Future Volume (vph)	1	15	139	0	6	252
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.873					
Flt Protected	0.997					0.999
Satd. Flow (prot)	1534	0	1618	0	0	1695
FIt Permitted	0.997					0.999
Satd. Flow (perm)	1534	0	1618	0	0	1695
Link Speed (k/h)	50		50			50
Link Distance (m)	105.3		30.4			100.3
Travel Time (s)	7.6		2.2			7.2
Confl. Peds. (#/hr)				10	10	
Confl. Bikes (#/hr)		3		3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	10%	1%	1%	5%
Adj. Flow (vph)	1	15	139	0	6	252
Shared Lane Traffic (%)						
Lane Group Flow (vph)	16	0	139	0	0	258
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	5.0		0.0			0.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	on 29.1%			IC	U Level of	Service A

Intersection Capacity Utilization 29.1% Analysis Period (min) 15

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ħβ		7	ħβ		¥	ĵ.		7	ĵ.	
Traffic Volume (vph)	27	1405	119	27	1424	70	177	5	78	92	104	50
Future Volume (vph)	27	1405	119	27	1424	70	177	5	78	92	104	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0			30.0			25.0			30.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00		1.00	1.00		1.00	0.98		1.00	0.99	
Frt		0.988			0.993			0.859			0.951	
Flt Protected	0.950	0.000		0.950	0.000		0.950	0.000		0.950	0.00	
Satd. Flow (prot)	1674	3229	0	1674	3289	0	1674	1472	0	1674	1651	0
Flt Permitted	0.126	ULLU	0	0.121	0200	U	0.600	1712	U	0.703	1001	U
Satd. Flow (perm)	222	3229	0	213	3289	0	1053	1472	0	1233	1651	0
Right Turn on Red		UZZU	Yes	210	3203	Yes	1000	1712	Yes	1200	1001	Yes
Satd. Flow (RTOR)		15	163		8	163		48	163		22	163
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
		28.9			12.3							
Travel Time (s)	_	26.9	20	20	12.3	r	r	7.2	r	F	20.2	F
Confl. Peds. (#/hr)	5		20	20		5	5		5	5		5
Confl. Bikes (#/hr)	4.00	4.00	5	4.00	4.00	3	4.00	4.00	3	4.00	4.00	4.00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	3%	1%	2%	1%	1%	1%	2%	1%	1%	4%
Adj. Flow (vph) Shared Lane Traffic (%)	27	1405	119	27	1424	70	177	5	78	92	104	50
Lane Group Flow (vph)	27	1524	0	27	1494	0	177	83	0	92	154	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel		<u> </u>		<u> </u>	<u> </u>		<u> </u>	<u> </u>		<u> </u>	<u> </u>	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7		0.0	28.7		0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OITLX			OI LX			OI · LA			OITEX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
	reiiil	2		Fellii	1NA 6		Fellii	NA 8		Fellii	1NA 4	
Protected Phases	^	Z		•	Ö		0	Ŏ		4	4	
Permitted Phases	2	2		6			8	0		4	4	
Detector Phase	2	2		6	6		8	8		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	75.6	75.6		75.6	75.6		22.3	22.3		22.3	22.3	
Actuated g/C Ratio	0.69	0.69		0.69	0.69		0.20	0.20		0.20	0.20	
v/c Ratio	0.18	0.69		0.18	0.66		0.83	0.25		0.37	0.44	
Control Delay	11.6	13.2		9.8	11.3		71.0	18.1		40.2	35.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	11.6	13.2		9.8	11.3		71.0	18.1		40.2	35.1	
LOS	В	В		Α	В		Е	В		D	D	
Approach Delay		13.2			11.3			54.1			37.0	
Approach LOS		В			В			D			D	
Queue Length 50th (m)	1.7	84.6		1.9	122.0		33.5	5.7		15.7	22.7	
Queue Length 95th (m)	6.9	130.9		m2.9	49.9		52.9	16.2		27.3	37.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	152	2223		146	2263		285	433		334	463	
Starvation Cap Reductn	0	0		0	4		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.18	0.69		0.18	0.66		0.62	0.19		0.28	0.33	

Intersection Summary

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 17.0 Intersection Capacity Utilization 82.3%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Don Reid/Ryder & Walkley



	-	\rightarrow	•	•	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	*	₹	VVDL	<u></u> ↑↑	NDL Š	TADIX
Traffic Volume (vph)	TT 1590	64	36	TT 1456	4 9	41
Future Volume (vph)	1590	64	36	1456	49	41
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	20.0	65.0	1000	30.0	0.0
		20.0	1		30.0	1
Storage Lanes			25.0		30.0	
Taper Length (m)	0.05	1.00		0.05		1.00
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	
Ped Bike Factor		0.95	1.00		0.99	0.98
Frt		0.850	0.050		0.050	0.850
Flt Protected	2052	4.400	0.950	0040	0.950	4.400
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted		4	0.135	06:15	0.950	4
Satd. Flow (perm)	3252	1413	233	3316	1646	1455
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		30				41
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)		15	15		5	5
Confl. Bikes (#/hr)		3				
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	2%	3%	2%	2%	2%
Adj. Flow (vph)	1590	64	36	1456	49	41
Shared Lane Traffic (%)	1000	V I	00	1100	10	
Lane Group Flow (vph)	1590	64	36	1456	49	41
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
	3.5	Rigiil	LINA	5.0	3.5	KINA
Median Width(m)	0.0				0.0	
Link Offset(m)				0.0		
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	31 · LA	J. L.	Ç	J. LA	J. L.	J
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7	0.0	0.0	28.7	0.0	0.0
				1.8		
Detector 2 Size(m)	1.8					
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	2.2			^ ^		
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2			6	8	
Permitted Phases		2	6			8
Detector Phase	2	2	6	6	8	8

	-	•	•	←	•	-				
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR				
Switch Phase		LDIK	,,,,,,,	.,,,,	TOL	HOIT				
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0				
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0				
Total Split (s)	81.0	81.0	81.0	81.0	29.0	29.0				
Total Split (%)	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%				
Maximum Green (s)	75.0	75.0	75.0	75.0	23.0	23.0				
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3				
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7				
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0				
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0				
Lead/Lag	0.0	0.0	0.0	0.0	0.0	0.0				
Lead-Lag Optimize?										
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0				
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None				
			C-IVIAX	C-IVIAX						
Walk Time (s)	18.0	18.0			7.0	7.0				
Flash Dont Walk (s)	13.0	13.0			16.0	16.0				
Pedestrian Calls (#/hr)	5	5	00.0	00.0	5	5				
Act Effct Green (s)	90.6	90.6	90.6	90.6	10.9	10.9				
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.10	0.10				
v/c Ratio	0.59	0.05	0.19	0.53	0.30	0.23				
Control Delay	2.2	0.8	7.8	6.6	48.0	15.0				
Queue Delay	0.0	0.0	0.0	0.1	0.0	0.0				
Total Delay	2.2	8.0	7.8	6.7	48.0	15.0				
LOS	А	Α	Α	Α	D	В				
Approach Delay	2.1			6.8	32.9					
Approach LOS	Α			Α	С					
Queue Length 50th (m)	15.4	0.2	0.8	58.9	9.4	0.0				
Queue Length 95th (m)	22.6	m0.8	m8.1	112.4	17.5	8.2				
Internal Link Dist (m)	147.1			145.1	104.6					
Turn Bay Length (m)		20.0	65.0		30.0					
Base Capacity (vph)	2679	1169	192	2731	346	336				
Starvation Cap Reductn	102	0	0	330	0	0				
Spillback Cap Reductn	79	0	0	0	0	1				
Storage Cap Reductn	0	0	0	0	0	0				
Reduced v/c Ratio	0.62	0.05	0.19	0.61	0.14	0.12				
Intersection Summary	0.02	3.00	3	3.0.	2					
Area Type:	Other									
Cycle Length: 110	Outo									
Actuated Cycle Length: 110										
Offset: 25 (23%), Reference	d to phone 2:E	DT and G·l	MDTI Cto	rt of Croon						
	u to phase z.E	DI allu 0.1	WDTL, Sia	it di Gieei	l					
Natural Cycle: 70										
Control Type: Actuated-Coordinated										
Maximum v/c Ratio: 0.59 Intersection Signal Delay: 5.1 Intersection LOS: A										
Intersection Signal Delay: 5.										
Intersection Capacity Utilizat	tion 62.9%			IC	CU Level of	Service E				
Analysis Period (min) 15										

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: 160m W of Conroy & Walkley



	-	•	•	•	∳ 1	•	-
Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	^		ሻሻ	*	1100	ሻሻ	₩ M
Traffic Volume (vph)	1179	452	518	999	43	461	319
Future Volume (vph)	1179	452	518	999	43	461	319
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
		75.0	200.0			2	1
Storage Lanes			50.0			10.0	I
Taper Length (m)	0.05	1.00		0.05	0.05		1.00
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
Ped Bike Factor		0.95	0.99				0.96
Frt		0.850	0.050			0.050	0.850
Flt Protected	0004	4.400	0.950	2040	^	0.950	4.400
Satd. Flow (prot)	3221	1483	3185	3349	0	3248	1469
Flt Permitted	0004	4444	0.950	00.40		0.950	4 400
Satd. Flow (perm)	3221	1414	3151	3349	0	3248	1406
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		452					319
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		30	30				20
Confl. Bikes (#/hr)		4					4
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	2%	3%	1%	1%	1%	3%
Adj. Flow (vph)	1179	452	518	999	43	461	319
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1179	452	518	999	0	504	319
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	1.03	24	1.00	1.03	24	1.03
Number of Detectors	2	14	1	2	14	1	14
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0 1.8			0.0 1.8		6.1	0.0
Detector 1 Size(m)		6.1	6.1		6.1		6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel	0.0	0.0	^ ^	^ ^	^ ^	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	
Permitted Phases		2			8		8
Detector Phase	2	2	1	6	8	8	8
							-

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	51.0	51.0	28.0	79.0	31.0	31.0	31.0
Total Split (%)	46.4%	46.4%	25.5%	71.8%	28.2%	28.2%	28.2%
Maximum Green (s)	44.6	44.6	21.8	72.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	48.1	48.1	21.2	75.4		21.8	21.8
Actuated g/C Ratio	0.44	0.44	0.19	0.69		0.20	0.20
v/c Ratio	0.84	0.52	0.85	0.44		0.78	0.60
Control Delay	32.2	6.6	56.6	8.8		50.9	9.1
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0
Total Delay	32.2	6.6	56.6	8.8		50.9	9.1
LOS	С	Α	Е	Α		D	Α
Approach Delay	25.1			25.1		34.7	
Approach LOS	С			С		С	
Queue Length 50th (m)	72.1	0.0	50.0	43.5		48.4	0.0
Queue Length 95th (m)	#149.0	31.6	#72.9	57.4		63.9	21.3
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1407	872	640	2296		726	562
Starvation Cap Reductn	0	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.84	0.52	0.81	0.44		0.69	0.57

Intersection Summary

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 27.1

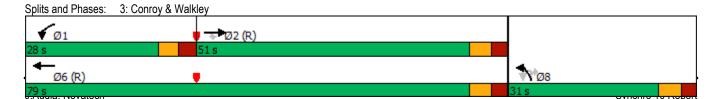
Intersection Capacity Utilization 83.4%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1	7	*	†	7	*	ተተ _ጮ		*	ተ ኈ	
Traffic Volume (vph)	47	81	322	232	38	102	47	567	81	37	1058	21
Future Volume (vph)	47	81	322	232	38	102	47	567	81	37	1058	21
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	0.99		0.98	0.99		0.98	1.00	0.99		0.99	1.00	
Frt			0.850			0.850		0.981			0.997	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4602	0	1537	3296	0
Flt Permitted	0.732	1120	1100	0.555	1010	1100	0.169	1002	•	0.384	0200	J
Satd. Flow (perm)	1281	1728	1452	953	1618	1453	291	4602	0	612	3296	0
Right Turn on Red	1201	1720	Yes	300	1010	Yes	201	7002	Yes	012	0230	Yes
Satd. Flow (RTOR)			122			82		29	103		2	103
Link Speed (k/h)		50	122		50	02		60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	10	14.2	10	10	19.0	10	10	22.4	20	20	20.9	10
Confl. Bikes (#/hr)	10		3	10		3	10		5	20		14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	1.00		2%		1.00	2%		2%	8%		2%	
Heavy Vehicles (%)	47	3%		3%			3%			10% 37		12%
Adj. Flow (vph)	47	81	322	232	38	102	47	567	81	31	1058	21
Shared Lane Traffic (%)	47	0.4	200	000	20	400	47	040	٥	27	4070	0
Lane Group Flow (vph)	47	81	322	232	38	102	47	648	0	37	1079	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	_	14	24	_	14	24	_	14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	*	4	•	3	8		*****	2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	3	8	8	2	2		6	6	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	21.2	21.2	21.2	37.4	36.2	36.2	45.6	45.6		45.6	45.6	
Actuated g/C Ratio	0.22	0.22	0.22	0.39	0.38	0.38	0.48	0.48		0.48	0.48	
v/c Ratio	0.16	0.21	0.77	0.52	0.06	0.17	0.34	0.29		0.13	0.68	
Control Delay	26.7	27.8	32.2	23.2	15.2	5.2	30.5	16.5		20.1	24.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	26.7	27.8	32.2	23.2	15.2	5.2	30.5	16.5		20.1	24.6	
LOS	С	С	С	С	В	Α	С	В		С	С	
Approach Delay		30.8			17.5			17.5			24.5	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	6.6	11.5	32.9	28.1	4.2	2.2	4.5	21.2		3.2	68.2	
Queue Length 95th (m)	11.5	17.2	46.8	30.7	6.9	7.9	#20.7	40.1		12.0	#141.2	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	500	674	641	442	887	833	139	2224		293	1583	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.09	0.12	0.50	0.52	0.04	0.12	0.34	0.29		0.13	0.68	

Intersection Summary

Area Type: Cycle Length: 95 Other

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77 Intersection Signal Delay: 22.7

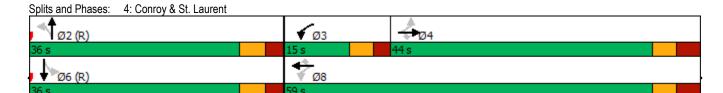
Intersection Capacity Utilization 85.5%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1 >			4
Traffic Volume (vph)	23	96	107	76	172	51
Future Volume (vph)	23	96	107	76	172	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.891		0.944			
Flt Protected	0.990					0.963
Satd. Flow (prot)	1462	0	1641	0	0	1592
FIt Permitted	0.990	,		-		0.963
Satd. Flow (perm)	1462	0	1641	0	0	1592
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)		4		5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	23	96	107	76	172	51
Shared Lane Traffic (%)						
Lane Group Flow (vph)	119	0	183	0	0	223
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Conseity Litilizati	ion 44 00/			IC	I I aval of	Comico A

Intersection Capacity Utilization 41.9% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ની	^		**	
Traffic Volume (vph)	1	249	122	11	9	1
Future Volume (vph)	1	249	122	11	9	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.989		0.986	
Flt Protected					0.957	
Satd. Flow (prot)	0	1695	1682	0	1663	0
Flt Permitted					0.957	
Satd. Flow (perm)	0	1695	1682	0	1663	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		40.4	197.7		90.8	
Travel Time (s)		2.9	14.2		6.5	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	5%	5%	1%	1%	1%
Adj. Flow (vph)	1	249	122	11	9	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	250	133	0	10	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)	20.1	0.0	3.5		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	1.00	1.00	14	24	14
Sign Control		Free	Free		Stop	
					0.00	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 24.7% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્સ	ĵ.		W		
Traffic Volume (vph)	1	247	119	4	3	0	
Future Volume (vph)	1	247	119	4	3	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt			0.996				
Flt Protected					0.950		
Satd. Flow (prot)	0	1695	1691	0	1674	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	1695	1691	0	1674	0	
Link Speed (k/h)		50	50		50		
Link Distance (m)		137.6	40.4		92.1		
Travel Time (s)		9.9	2.9		6.6		
Confl. Peds. (#/hr)	10			10			
Confl. Bikes (#/hr)				3		3	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles (%)	1%	5%	5%	1%	1%	1%	
Adj. Flow (vph)	1	247	119	4	3	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	248	123	0	3	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(m)		0.0	0.0		3.5		
Link Offset(m)		0.0	0.0		0.0		
Crosswalk Width(m)		0.0	0.0		5.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	24			14	24	14	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Interportion Conneity Litilizati	on 24 60/			IC	III aval of	Conside A	

Intersection Capacity Utilization 24.6% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥ /		1}			ની
Traffic Volume (vph)	1	10	238	1	13	223
Future Volume (vph)	1	10	238	1	13	223
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.877		0.999			
Flt Protected	0.995					0.997
Satd. Flow (prot)	1538	0	1694	0	0	1694
FIt Permitted	0.995					0.997
Satd. Flow (perm)	1538	0	1694	0	0	1694
Link Speed (k/h)	50		50			50
Link Distance (m)	105.3		30.4			100.3
Travel Time (s)	7.6		2.2			7.2
Confl. Peds. (#/hr)				10	10	
Confl. Bikes (#/hr)		3		3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	5%	1%	1%	5%
Adj. Flow (vph)	1	10	238	1	13	223
Shared Lane Traffic (%)						
Lane Group Flow (vph)	11	0	239	0	0	236
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	5.0		0.0			0.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
	00.00/			10		

Intersection Capacity Utilization 33.6% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ħβ		7	ħβ		¥	ĵ.		7	ĵ.	
Traffic Volume (vph)	29	1125	173	46	1531	195	96	7	60	49	47	39
Future Volume (vph)	29	1125	173	46	1531	195	96	7	60	49	47	39
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0			30.0			25.0			30.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.99		1.00	1.00		1.00	0.98		1.00	0.99	
Frt		0.980			0.983			0.866			0.932	
Flt Protected	0.950	0.000		0.950	0.000		0.950	0.000		0.950	0.002	
Satd. Flow (prot)	1537	3183	0	1642	3169	0	1580	1367	0	1674	1630	0
Flt Permitted	0.099	0100	U	0.184	0103	0	0.701	1007	0	0.713	1000	U
Satd. Flow (perm)	160	3183	0	317	3169	0	1161	1367	0	1251	1630	0
Right Turn on Red	100	0100	Yes	317	3103	Yes	1101	1001	Yes	1201	1000	Yes
Satd. Flow (RTOR)		29	163		23	163		60	163		30	163
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		402.0			171.1			100.3			281.0	
		28.9			12.3							
Travel Time (s)	_	28.9	10	10	12.3	r	r	7.2	F	F	20.2	F
Confl. Peds. (#/hr)	5		10	10		5	5		5	5		5
Confl. Bikes (#/hr)	4.00	4.00	3	4.00	4.00	3	4.00	4.00	3	4.00	4.00	4.00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	10%	4%	1%	3%	5%	1%	7%	1%	12%	1%	1%	1%
Adj. Flow (vph) Shared Lane Traffic (%)	29	1125	173	46	1531	195	96	7	60	49	47	39
Lane Group Flow (vph)	29	1298	0	46	1726	0	96	67	0	49	86	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7		0.0	28.7			28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OITEX			OI LX			OI · LX			OI · LX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	1 61111	2		1 (31111	6		i Giiii	8		1 51111	4	
Permitted Phases	2			6	U		8	U		4	4	
Detector Phase	2	2		6	6		8	8		4	4	
Delector Fridae		Z		U	U		0	0		4	4	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	64.0	64.0		64.0	64.0		36.0	36.0		36.0	36.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		36.0%	36.0%		36.0%	36.0%	
Maximum Green (s)	58.1	58.1		58.1	58.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	5	5		5	5		5	5		5	5	
Act Effct Green (s)	76.2	76.2		76.2	76.2		16.1	16.1		16.1	16.1	
Actuated g/C Ratio	0.76	0.76		0.76	0.76		0.16	0.16		0.16	0.16	
v/c Ratio	0.24	0.53		0.19	0.71		0.51	0.25		0.24	0.30	
Control Delay	14.2	8.3		6.4	7.2		46.1	11.9		36.4	25.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.2	8.3		6.4	7.2		46.1	11.9		36.4	25.4	
LOS	В	Α		Α	Α		D	В		D	С	
Approach Delay		8.4			7.2			32.1			29.4	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	1.4	43.0		1.4	28.4		16.4	1.1		8.0	9.1	
Queue Length 95th (m)	9.6	100.6		m3.0	#191.4		25.6	9.6		14.5	17.7	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	121	2432		241	2420		345	449		372	506	
Starvation Cap Reductn	0	0		0	20		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.24	0.53		0.19	0.72		0.28	0.15		0.13	0.17	

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Offset: 94 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 9.8
Intersection Capacity Utilization 75.6%

Intersection LOS: A ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Don Reid/Ryder & Walkley



Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 10 10 5 5 Confl. Bikes (#/hr) 3 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Heavy Vehicles (%) 5% 1% 1% 6% 1% 2% Adj. Flow (vph) 1154 90 74 1724 66 60 Shared Lane Traffic (%) Lane Group Flow (vph) 1154 90 74 1724 66 60 Enter Blocked Intersection No No No No No No No Lane Alignment Left Right L NA Left L NA R NA Median Width(m) 3.5 5.0 3.5 Link Offset(m) 0.0 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 1.09 1.09 Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 1.8 6.1 6.1 1.8 6.1 6.1 Trailing Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Desition(m) 28.7		-	\rightarrow	•	•	4	/
Lane Configurations	Lane Group	FRT	FRR	WRI	WRT	NRI	NRR
Traffic Volume (vph)							
Future Volume (vph)							
Ideal Flow (vphpl)							
Storage Length (m)							
Storage Lanes		1000			1000		
Taper Length (m)							
Lane Util. Factor 0.95 1.00 1.00 0.95 1.00 1.00 Ped Bike Factor 0.96 1.00 0.99 0.99 0.985 Fit Protected 0.850 0.950 0.950 Satd. Flow (prot) 3221 1498 1674 3191 1674 1483 Fit Permitted 0.229 0.950 0.950 Satd. Flow (perm) 3221 1442 403 3191 1663 1456 Right Turn on Red Yes 50							
Ped Bike Factor		0.05	1.00		0.05		1.00
Fit Protected		0.95			0.95		
Fit Protected				1.00		0.99	
Satd. Flow (prot) 3221 1498 1674 3191 1674 1483 FIF Permitted 0.229 0.950 0.950 1456 3191 1663 1456 3191 1663 1456 Right Turn on Red Yes Yes Yes Yes Yes Satd. Flow (RTOR) 59 60 60 60 60 60 Link Speed (k/h) 50 50 50 50 50 Link Speed (k/h) 50 50 50 50 50 Link Speed (k/h) 50 50 10 11 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 30 122.9 9.3 Confl. Peds. (#/hr) 30 12.2 9.3 Confl. Peds. (#/hr) 30 12.2 9.3 Confl. Peds. (#/hr) 10 10 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00			0.850	0.050		0.050	0.850
Fit Permitted		2224	4400		0.10.1		4 400
Satd. Flow (perm) 3221 1442 403 3191 1663 1456 Right Turn on Red Yes 59 60 Satd. Flow (RTOR) 59 60 Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 3 10 10 5 5 Confl. Bikes (#/hr) 3 - - 5 5 Peak Hour Factor 1.00	,	3221	1498		3191		1483
Right Turn on Red Yes Yes Satd. Flow (RTOR) 59 60 Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 3 10 10 5 5 Confl. Bikes (#/hr) 3 12.2 9.3 1.00							
Satd. Flow (RTOR) 59 60 Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 3 10 10 5 5 Confl. Bikes (#/hr) 3 7 100 1.00 <td></td> <td>3221</td> <td></td> <td>403</td> <td>3191</td> <td>1663</td> <td></td>		3221		403	3191	1663	
Link Speed (k/h) 50 50 50 Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 3 10 10 15 5 Confl. Bikes (#/hr) 3							
Link Distance (m) 171.1 169.1 128.6 Travel Time (s) 12.3 12.2 9.3 Confl. Peds. (#/hr) 10 10 5 5 Confl. Bikes (#/hr) 3	Satd. Flow (RTOR)		59				60
Travel Time (s)	Link Speed (k/h)						
Travel Time (s)	Link Distance (m)	171.1			169.1	128.6	
Confl. Peds. (#/hr) 10 10 5 5 Confl. Bikes (#/hr) 3	Travel Time (s)						
Confl. Bikes (#/hr) 3 Peak Hour Factor 1.00			10	10			5
Peak Hour Factor							
Heavy Vehicles (%)		1 00		1.00	1 00	1.00	1.00
Adj. Flow (vph) 1154 90 74 1724 66 60 Shared Lane Traffic (%) Lane Group Flow (vph) 1154 90 74 1724 66 60 Enter Blocked Intersection No							
Shared Lane Traffic (%) Lane Group Flow (vph) 1154 90 74 1724 66 60 Enter Blocked Intersection No							
Lane Group Flow (vph) 1154 90 74 1724 66 60 Enter Blocked Intersection No <		1104	30	14	1724	00	00
Enter Blocked Intersection No No <th< td=""><td></td><td>1151</td><td>00</td><td>7.1</td><td>1704</td><td>SS</td><td>60</td></th<>		1151	00	7.1	1704	SS	60
Lane Alignment Left Right L NA Left L NA R NA Median Width(m) 3.5 5.0 3.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09	,						
Median Width(m) 3.5 5.0 3.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Turning Speed (k/h) 1.09 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00							
Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Leading Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Leading Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type Cl+Ex Cl+Ex Cl+Ex Cl+Ex Cl+Ex			Right	L NA			R NA
Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Two way Left Turn Lane Headway Factor 1.09 1.00 0.							
Headway Factor		5.0			5.0	5.0	
Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex D.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Two way Left Turn Lane						
Turning Speed (k/h) 14 24 24 14 Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Channel Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Number of Detectors 2 1 1 2 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(m) 28.7 28.7 28.7 28.7 <	Turning Speed (k/h)						14
Detector Template	Number of Detectors	2			2		
Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex			-	-			-
Trailing Detector (m) 0.0	•						
Detector 1 Position(m) 0.0							
Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 Detector 1 Type CI+Ex D.0 0.0 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Detector 1 Type CI+Ex							
Detector 1 Channel O.0 0.0							
Detector 1 Extend (s) 0.0		CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Queue (s) 0.0							
Detector 1 Delay (s) 0.0							
Detector 2 Position(m) 28.7 28.7 Detector 2 Size(m) 1.8 1.8 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 Perm Turn Type NA Perm Perm NA Prot Permitted Phases 2 6 8 Permitted Phases 2 6 8							
Detector 2 Size(m) 1.8 1.8 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 Perm Perm NA Prot Perm Protected Phases 2 6 8 Permitted Phases 8	Detector 1 Delay (s)		0.0	0.0		0.0	0.0
Detector 2 Type CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 Perm Turn Type NA Perm Perm NA Prot Perm Permitted Phases 2 6 8 8	Detector 2 Position(m)	28.7			28.7		
Detector 2 Type CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 Perm Turn Type NA Perm Perm NA Prot Perm Permitted Phases 2 6 8 8	Detector 2 Size(m)	1.8			1.8		
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8							
Detector 2 Extend (s) 0.0 0.0 Turn Type NA Perm Perm NA Prot Perm Protected Phases 2 6 8 8 Permitted Phases 2 6 8 8							
Turn TypeNAPermPermNAProtPermProtected Phases268Permitted Phases268		0.0			0.0		
Protected Phases 2 6 8 Permitted Phases 2 6 8			Perm	Perm		Prot	Perm
Permitted Phases 2 6 8			1 01111	1 01111			1 01111
			2	6	U	U	0
Detector Phase Z Z 6 6 8		•				0	
	Detector Phase	2	2	Ь	Ь	8	8

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0
Total Split (s)	71.0	71.0	71.0	71.0	29.0	29.0
Total Split (%)	71.0%	71.0%	71.0%	71.0%	29.0%	29.0%
Maximum Green (s)	65.0	65.0	65.0	65.0	23.0	23.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)	18.0	18.0	O MICK	O Max	7.0	7.0
Flash Dont Walk (s)	13.0	13.0			16.0	16.0
Pedestrian Calls (#/hr)	5	5			5	5
Act Effet Green (s)	80.2	80.2	80.2	80.2	11.3	11.3
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.11	0.11
v/c Ratio	0.60	0.00	0.80	0.67	0.11	0.11
Control Delay	3.1	1.1	7.0	8.5	43.6	12.6
Queue Delay	0.0	0.0	0.0	0.6	0.0	0.0
Total Delay	3.1	1.1	7.0	9.1	43.6	12.6
LOS	ا. S	1.1 A	7.0 A	9.1 A	43.0 D	12.0 B
Approach Delay	2.9	А	A	9.0	28.8	Б
	2.9 A			9.0 A	20.0 C	
Approach LOS	18.4	0.3	2.7	58.8	11.3	0.0
Queue Length 50th (m)	24.2	1.1	12.5	143.4	19.6	8.9
Queue Length 95th (m)		1.1	12.5			8.9
Internal Link Dist (m)	147.1	20.0	65.0	145.1	104.6	
Turn Bay Length (m)	0500	20.0	65.0	2500	30.0	204
Base Capacity (vph)	2583	1168	323	2560	385	381
Starvation Cap Reductn	0	0	0	405	0	0
Spillback Cap Reductn	0	0	0	70	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.45	0.08	0.23	0.80	0.17	0.16
Intersection Summary						
Area Type:	Other					
Cycle Length: 100	30101					
Actuated Cycle Length: 100						
Offset: 88 (88%), Referenced	to phase 2.F	RT and 6·\	NRTI Sta	rt of Green		
Natural Cycle: 80	to pridate Z.E.	ום מוע ט.ז	TE, Ola	it of Gleen		
Control Type: Actuated-Coord	linated					
Maximum v/c Ratio: 0.67	mateu					
Intersection Signal Delay: 7.4				ln.	tersection	I 08: V
	on 66 80/					f Service C
Intersection Capacity Utilization	00.0%			IC	O Level 01	Service C
Analysis Period (min) 15						
Califo and Dhases: 0: 100-	M of Comme	, 0 \A/all-l	,			
Splits and Phases: 2: 160m	W of Conroy	& walkle	/			
→ Ø2 (R)						
71 n						
/18						
Ø6 (D)						
√ Ø6 (R)						
112						

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	44	Z Z	ሻሻ	^	1,50	ሻሻ	7
Traffic Volume (vph)	994	280	261	1096	15	717	493
Future Volume (vph)	994	280	261	1096	15	717	493
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		1 1	200.0			2	1
Taper Length (m)			50.0			10.0	I
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
Ped Bike Factor	0.93	0.97	0.99	0.95	0.33	0.91	0.98
Frt		0.850	0.99				0.850
		0.000	0.950			0.950	0.000
Fit Protected	2464	1/55	3066	2464	0		1/55
Satd. Flow (prot)	3161	1455		3161	0	3186	1455
Flt Permitted	2404	1400	0.950	2404	0	0.950	1400
Satd. Flow (perm)	3161	1409	3043	3161	0	3186	1423
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		280				22	330
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		15	15				5
Confl. Bikes (#/hr)		3					3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	7%	4%	7%	7%	2%	3%	4%
Adj. Flow (vph)	994	280	261	1096	15	717	493
Shared Lane Traffic (%)							
Lane Group Flow (vph)	994	280	261	1096	0	732	493
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	14	24	14
Number of Detectors	2	1	1	2	1	1	1
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1	6.1
	0.0	0.0	0.0	0.0	0.0	0.1	0.1
Trailing Detector (m) Detector 1 Position(m)							
	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	
Permitted Phases	_	2			8		8
		,					
Detector Phase	2	2	1	6	8	8	8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	55.0	55.0	25.0	80.0	40.0	40.0	40.0
Total Split (%)	45.8%	45.8%	20.8%	66.7%	33.3%	33.3%	33.3%
Maximum Green (s)	48.6	48.6	18.8	73.6	33.6	33.6	33.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	10	10			5	5	5
Act Effct Green (s)	53.9	53.9	15.2	75.4		31.8	31.8
Actuated g/C Ratio	0.45	0.45	0.13	0.63		0.26	0.26
v/c Ratio	0.70	0.36	0.67	0.55		0.87	0.80
Control Delay	30.8	4.0	58.5	14.3		53.7	23.4
Queue Delay	0.3	0.0	0.0	0.0		0.0	0.0
Total Delay	31.1	4.0	58.5	14.3		53.7	23.4
LOS	С	Α	Е	В		D	С
Approach Delay	25.2			22.8		41.5	
Approach LOS	С			С		D	
Queue Length 50th (m)	92.0	0.0	28.1	69.3		76.5	33.1
Queue Length 95th (m)	121.0	14.9	39.6	86.1		97.6	75.1
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1420	787	480	1985		892	636
Starvation Cap Reductn	93	0	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.75	0.36	0.54	0.55		0.82	0.78
Intersection Summary							
Area Type:	Other						
Cycle Length: 120							
Actuated Cycle Length: 120							

Offset: 43 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green

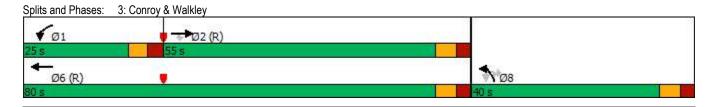
Natural Cycle: 85 Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 29.5 Intersection Capacity Utilization 74.7%

Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	•	7	7	•	7	7	ተ ቀጭ		7	ħβ	
Traffic Volume (vph)	15	69	88	63	46	25	195	1246	199	87	389	57
Future Volume (vph)	15	69	88	63	46	25	195	1246	199	87	389	57
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	0.99		0.97	0.99		0.98	0.98	0.99		1.00	0.99	
Frt			0.850			0.850		0.979			0.981	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1353	1695	1427	1409	1589	1351	1642	4617	0	1674	3162	0
Flt Permitted	0.727			0.712			0.494			0.154		
Satd. Flow (perm)	1027	1695	1391	1044	1589	1322	835	4617	0	270	3162	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			88			36		43			23	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	10		15	15		10	20		25	25		20
Confl. Bikes (#/hr)			3			3			3			25
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	25%	5%	6%	20%	12%	12%	3%	2%	2%	1%	4%	3%
Adj. Flow (vph)	15	69	88	63	46	25	195	1246	199	87	389	57
Shared Lane Traffic (%)												
Lane Group Flow (vph)	15	69	88	63	46	25	195	1445	0	87	446	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0			6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	43.9	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	44.0	44.0	44.0	56.0	56.0		56.0	56.0	
Total Split (%)	44.0%	44.0%	44.0%	44.0%	44.0%	44.0%	56.0%	56.0%		56.0%	56.0%	
Maximum Green (s)	37.1	37.1	37.1	37.1	37.1	37.1	49.7	49.7		49.7	49.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0	30.0	30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	15	15	15	10	10	10	20	20		15	15	
Act Effct Green (s)	21.1	21.1	21.1	21.1	21.1	21.1	70.4	70.4		70.4	70.4	
Actuated g/C Ratio	0.21	0.21	0.21	0.21	0.21	0.21	0.70	0.70		0.70	0.70	
v/c Ratio	0.07	0.19	0.24	0.29	0.14	0.08	0.33	0.44		0.46	0.20	
Control Delay	24.1	28.4	7.0	31.8	27.7	5.2	13.3	10.6		26.2	8.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	24.1	28.4	7.0	31.8	27.7	5.2	13.3	10.6		26.2	8.7	
LOS	С	С	Α	С	С	Α	В	В		С	Α	
Approach Delay		17.1			25.5			10.9			11.5	
Approach LOS		В			С			В			В	
Queue Length 50th (m)	2.5	11.7	1.6	10.8	7.6	0.0	9.2	26.7		4.6	9.3	
Queue Length 95th (m)	m4.9	m16.7	9.4	16.1	12.1	3.5	40.3	77.6		#35.0	31.2	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	381	628	571	387	589	513	587	3261		190	2231	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.04	0.11	0.15	0.16	0.08	0.05	0.33	0.44		0.46	0.20	

Other

Area Type: Cycle Length: 100

Actuated Cycle Length: 100

Offset: 23 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.46 Intersection Signal Delay: 12.3

Intersection Capacity Utilization 72.2%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Conroy & St. Laurent



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f.			4
Traffic Volume (vph)	75	59	71	22	101	176
Future Volume (vph)	75	59	71	22	101	176
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.941		0.968			
Flt Protected	0.973					0.982
Satd. Flow (prot)	1547	0	1498	0	0	1669
Flt Permitted	0.973					0.982
Satd. Flow (perm)	1547	0	1498	0	0	1669
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				10	10	
Confl. Bikes (#/hr)		4		3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	7%	15%	15%	6%	4%
Adj. Flow (vph)	75	59	71	22	101	176
Shared Lane Traffic (%)						
Lane Group Flow (vph)	134	0	93	0	0	277
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	J .	0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
_						
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						_
Intersection Capacity Utilization	on 37.2%			ICI	J Level of	Service A
Analysis Period (min) 15						

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ની	1 >		W	
Traffic Volume (vph)	0	128	135	5	13	0
Future Volume (vph)	0	128	135	5	13	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	, ,					
Frt			0.995			
Flt Protected					0.950	
Satd. Flow (prot)	0	1618	1615	0	1674	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	1618	1615	0	1674	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		40.4	197.7		90.8	
Travel Time (s)		2.9	14.2		6.5	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	10%	10%	1%	1%	1%
Adj. Flow (vph)	0	128	135	5	13	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	128	140	0	13	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	3.5	Ĭ	3.5	Ĭ
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 19.4% Analysis Period (min) 15

ICU Level of Service A

	•	→	←	•	/	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્વ	ĵ,		W	
Traffic Volume (vph)	0	123	133	2	5	1
Future Volume (vph)	0	123	133	2	5	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.998		0.977	
Flt Protected					0.960	
Satd. Flow (prot)	0	1618	1617	0	1653	0
Flt Permitted					0.960	
Satd. Flow (perm)	0	1618	1617	0	1653	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		137.6	40.4		92.1	
Travel Time (s)		9.9	2.9		6.6	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	10%	10%	1%	1%	1%
Adj. Flow (vph)	0	123	133	2	5	1
Shared Lane Traffic (%)		,			-	<u> </u>
Lane Group Flow (vph)	0	123	135	0	6	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	1.00	1.00	14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
	40.00/			10		

Intersection Capacity Utilization 19.2% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1			4
Traffic Volume (vph)	1	15	139	1	6	252
Future Volume (vph)	1	15	139	1	6	252
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.873		0.999			
Flt Protected	0.997		0.000			0.999
Satd. Flow (prot)	1534	0	1618	0	0	1695
Flt Permitted	0.997	<u> </u>	1010	U		0.999
Satd. Flow (perm)	1534	0	1618	0	0	1695
Link Speed (k/h)	50		50			50
Link Opeed (km)	105.3		30.4			100.3
Travel Time (s)	7.6		2.2			7.2
Confl. Peds. (#/hr)	1.0		L. E	10	10	1.2
Confl. Bikes (#/hr)		3		3	.,	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	10%	1%	1%	5%
Adj. Flow (vph)	1 / 1	15	139	1 / 1	6	252
Shared Lane Traffic (%)	' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' '	10	100			202
Lane Group Flow (vph)	16	0	140	0	0	258
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	1 119111	3.5		20.0	3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	5.0		0.0			0.0
Two way Left Turn Lane	0.0		0.0			0.0
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	1.00	14	24	1.00
Sign Control	Stop	1.1	Free	, ,		Free
	Сюр		1100			1100
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 29.1% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	∱ %		ሻ	↑ Љ		ሻ	ĵ,		ሻ	f)	
Traffic Volume (vph)	27	1557	119	27	1626	70	177	5	78	92	104	50
Future Volume (vph)	27	1557	119	27	1626	70	177	5	78	92	104	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	35.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	25.0		· ·	30.0		, and the second	25.0		•	30.0		•
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.98	1.00	1.00	0.99	1.00
Frt		0.989			0.994		1.00	0.859		1.00	0.951	
Flt Protected	0.950	0.000		0.950	0.001		0.950	0.000		0.950	0.001	
Satd. Flow (prot)	1674	3234	0	1674	3293	0	1674	1472	0	1674	1651	0
Flt Permitted	0.089	0204	0	0.093	0230	0	0.600	1712	U	0.703	1001	U
Satd. Flow (perm)	157	3234	0	164	3293	0	1053	1472	0	1233	1651	0
Right Turn on Red	131	3234	Yes	104	3233	Yes	1000	1412	Yes	1233	1031	Yes
Satd. Flow (RTOR)		13	163		7	163		34	163		22	163
		50			50			50			50	
Link Speed (k/h)		402.0			171.1			100.3			281.0	
Link Distance (m)												
Travel Time (s)	_	28.9	20	20	12.3	r	F	7.2	F	F	20.2	F
Confl. Peds. (#/hr)	5		20	20		5	5		5	5		5
Confl. Bikes (#/hr)	4.00	4.00	5	4.00	4.00	3	4.00	4.00	3	4.00	4.00	4.00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	3%	1%	2%	1%	1%	1%	2%	1%	1%	4%
Adj. Flow (vph) Shared Lane Traffic (%)	27	1557	119	27	1626	70	177	5	78	92	104	50
Lane Group Flow (vph)	27	1676	0	27	1696	0	177	83	0	92	154	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		5.0			5.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7		0.0	28.7			28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OIILX			OIILX			OITEX			OIILX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	i Cilil	2		1 61111	6		1 61111	8		1 61111	4	
Permitted Phases	2	Z		6	U		8	0		1	4	
	2	2		6	6		8 8	0		4	1	
Detector Phase	Z	2		0	0		Ŏ	8		4	4	

	۶	→	•	•	+	4	1	†	/	/	↓	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	35.9	35.9		35.9	35.9		36.2	36.2		36.2	36.2	
Total Split (s)	74.0	74.0		74.0	74.0		36.0	36.0		36.0	36.0	
Total Split (%)	67.3%	67.3%		67.3%	67.3%		32.7%	32.7%		32.7%	32.7%	
Maximum Green (s)	68.1	68.1		68.1	68.1		29.8	29.8		29.8	29.8	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.6	2.6		2.6	2.6		2.9	2.9		2.9	2.9	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9		6.2	6.2		6.2	6.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	15.0	15.0		15.0	15.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		20.0	20.0		20.0	20.0	
Pedestrian Calls (#/hr)	10	10		5	5		5	5		5	5	
Act Effct Green (s)	75.6	75.6		75.6	75.6		22.3	22.3		22.3	22.3	
Actuated g/C Ratio	0.69	0.69		0.69	0.69		0.20	0.20		0.20	0.20	
v/c Ratio	0.25	0.75		0.24	0.75		0.83	0.26		0.37	0.44	
Control Delay	16.0	15.1		10.9	11.7		71.0	23.1		40.2	35.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	16.0	15.1		10.9	11.7		71.0	23.1		40.2	35.1	
LOS	В	В		В	В		Е	С		D	D	
Approach Delay		15.1			11.7			55.7			37.0	
Approach LOS		В			В			Е			D	
Queue Length 50th (m)	1.8	102.3		1.7	144.8		33.5	8.0		15.7	22.7	
Queue Length 95th (m)	8.4	158.6		m2.3	50.6		52.9	18.5		27.3	37.3	
Internal Link Dist (m)		378.0			147.1			76.3			257.0	
Turn Bay Length (m)	40.0			50.0			35.0			30.0		
Base Capacity (vph)	107	2226		112	2265		285	423		334	463	
Starvation Cap Reductn	0	0		0	4		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.25	0.75		0.24	0.75		0.62	0.20		0.28	0.33	

Other

Area Type: Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

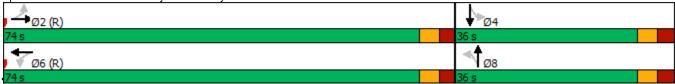
Intersection Signal Delay: 17.7 Intersection Capacity Utilization 87.0%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

1: Don Reid/Ryder & Walkley Splits and Phases:



	-	•	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	EDK 7	VVDL	<u>₩</u>	INDL Š	INDIX 7
Traffic Volume (vph)	TT 1749	6 4	1 36	TT 1659	1 49	41
Future Volume (vph)	1749	64	36	1659	49	41
	1800	1800	1800	1800	1800	1800
Ideal Flow (vphpl)	1800	20.0	65.0	1000	30.0	0.0
Storage Length (m)			05.0		30.0	0.0
Storage Lanes		1			•	T
Taper Length (m)	0.05	1.00	25.0	0.05	30.0	4.00
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Ped Bike Factor		0.95	1.00		0.99	0.98
Frt		0.850	0.050		0.050	0.850
Flt Protected	0.5=5	1.100	0.950	00:5	0.950	
Satd. Flow (prot)	3252	1483	1642	3316	1658	1483
Flt Permitted			0.109		0.950	
Satd. Flow (perm)	3252	1413	188	3316	1646	1455
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		28				32
Link Speed (k/h)	50			50	50	
Link Distance (m)	171.1			169.1	128.6	
Travel Time (s)	12.3			12.2	9.3	
Confl. Peds. (#/hr)	12.0	15	15		5.5	5
Confl. Bikes (#/hr)		3	- 10			
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	4%	2%	3%	2%	2%	2%
	1749	64	36		49	41
Adj. Flow (vph)	1749	04	30	1659	49	41
Shared Lane Traffic (%)	4740	0.4	20	1050	40	. 4.4
Lane Group Flow (vph)	1749	64	36	1659	49	41
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	L NA	Left	L NA	R NA
Median Width(m)	3.5			5.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			5.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.00	14	24	1.00	24	1.03
Number of Detectors	2	14	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
•						
Leading Detector (m)	30.5	6.1	6.1	30.5	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7	0.0	0.0	28.7	0.0	0.0
Detector 2 Size(m)	1.8			1.8		
Detector 2 Type	CI+Ex			Cl+Ex		
Detector 2 Channel	OITEX			OFFLA		
	0.0			0.0		
Detector 2 Extend (s)		D	D		D4	De
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	2			6	8	
Permitted Phases		2	6			8
Detector Phase	2	2	6	6	8	8

	→	•	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	5.0
Minimum Split (s)	37.0	37.0	37.0	37.0	29.0	29.0
Total Split (s)	81.0	81.0	81.0	81.0	29.0	29.0
Total Split (%)	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%
Maximum Green (s)	75.0	75.0	75.0	75.0	23.0	23.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	0.0	0.0	0.0	0.0	0.0	0.0
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)	18.0	18.0	O-IVIAX	O-IVIAX	7.0	7.0
Flash Dont Walk (s)	13.0	13.0			16.0	16.0
Pedestrian Calls (#/hr)	13.0	13.0			16.0	16.0
Act Effct Green (s)	90.6	90.6	90.6	00.6	10.9	10.9
()				90.6		
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.10	0.10
v/c Ratio	0.65	0.05	0.23	0.61	0.30	0.24
Control Delay	2.3	8.0	9.4	7.6	48.0	20.9
Queue Delay	0.1	0.0	0.0	0.1	0.0	0.0
Total Delay	2.4	8.0	9.4	7.7	48.0	20.9
LOS	A	Α	Α	A	D	С
Approach Delay	2.4			7.8	35.6	
Approach LOS	A			Α	D	
Queue Length 50th (m)	16.8	0.1	0.6	75.4	9.4	1.7
Queue Length 95th (m)	23.4	m0.6	m7.5	143.2	17.5	9.7
Internal Link Dist (m)	147.1			145.1	104.6	
Turn Bay Length (m)		20.0	65.0		30.0	
Base Capacity (vph)	2679	1169	155	2731	346	329
Starvation Cap Reductn	89	0	0	267	0	0
Spillback Cap Reductn	200	0	0	14	0	1
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.71	0.05	0.23	0.67	0.14	0.13
Intersection Summary						
Area Type:	Other					
Cycle Length: 110	Culoi					
Actuated Cycle Length: 110						
Offset: 25 (23%), Reference	d to phase 2:F	RT and 6·1	MRTI Sta	rt of Green	,	
Natural Cycle: 80	d to pridate z.L.	Di ana o.v	NDTE, Ola	it of Oleer	•	
Control Type: Actuated-Coo	rdinated					
Maximum v/c Ratio: 0.65	Tullialeu					
Intersection Signal Delay: 5.	7			ln.	tersection	1 OC: 1
,						
Intersection Capacity Utilizate Analysis Period (min) 15	1011 07.5%			IC	CU Level of	i Service C

Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Lane Group
Lane Configurations
Traffic Volume (vph)
Future Volume (vph)
Ideal Flow (vphpl)
Storage Length (m)
Storage Lanes
Taper Length (m)
Lane Util. Factor
Ped Bike Factor
Fith
Fit Protected 1483 3185 3349 0 3248 1469 Satd. Flow (proty) 3221 1483 3185 3349 0 3248 1469 Fit Permitted 0.950 0.950 Satd. Flow (perm) 3221 1414 3157 3349 0 3248 1406 Right Turn on Red Yes
Satd. Flow (prot) 3221 1483 3185 3349 0 3248 1469 Flt Permitted 0.950 0.950 0.950 Satd. Flow (perm) 3221 1414 3157 3349 0 3248 1406 Right Turn on Red Yes Yes Yes Yes Yes Satd. Flow (RTOR) 440 50 60 50 60 Link Speed (k/h) 50 50 60 50 60 Link Distance (m) 169.1 271.7 348.5 7 Travel Time (s) 12.2 19.6 20.9 20 Confl. Peds. (#/hr) 30 30 20
Fit Permitted
Satd. Flow (perm) 3221
Right Turn on Red Yes 334
Satd. Flow (RTOR)
Link Speed (k/h) 50
Link Distance (m) 169.1 271.7 348.5 Travel Time (s) 12.2 19.6 20.9 Confl. Peds. (#/hr) 30 30 20 Confl. Bikes (#/hr) 4 4 4 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Heavy Vehicles (%) 5% 2% 3% 1% 1% 1% 3% Adj. Flow (vph) 1319 474 545 1180 45 484 334 Shared Lane Traffic (%) Lane Group Flow (vph) 1319 474 545 1180 0 529 334 Enter Blocked Intersection No No No No No No No
Travel Time (s)
Confl. Peds. (#/hr)
Confl. Bikes (#/hr)
Peak Hour Factor
Heavy Vehicles (%) 5% 2% 3% 1% 1% 1% 3% Adj. Flow (vph) 1319 474 545 1180 45 484 334 Shared Lane Traffic (%) Lane Group Flow (vph) 1319 474 545 1180 0 529 334 Enter Blocked Intersection No No No No No No No Lane Alignment Left R NA Left Left R NA L NA R NA Median Width(m) 7.0 9.0 10.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 1.09 Turning Speed (k/h) 14 24 14 24 14 Number of Detectors 2 1 1 2 1 1 1 Detector Template Thru Right Left Thru Left Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 6.1 Detector 1 Channel Detector 1 Letend (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(m) 28.7 28.7 Detector 2 Size(m) 1.8 1.8 1.8
Adj. Flow (vph) 1319 474 545 1180 45 484 334 Shared Lane Traffic (%) Lane Group Flow (vph) 1319 474 545 1180 0 529 334 Enter Blocked Intersection No
Shared Lane Traffic (%) Lane Group Flow (vph) 1319 474 545 1180 0 529 334
Lane Group Flow (vph) 1319 474 545 1180 0 529 334 Enter Blocked Intersection No
Enter Blocked Intersection No Lon No Lon No Lon No
Lane Alignment Left R NA Left Left R NA L NA R NA Median Width(m) 7.0 9.0 10.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Two way Left Turn Lane 1.09
Median Width(m) 7.0 9.0 10.5 Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane 1.09
Link Offset(m) 0.0 0.0 0.0 Crosswalk Width(m) 5.0 5.0 Two way Left Turn Lane 1.09
Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00
Crosswalk Width(m) 5.0 5.0 5.0 Two way Left Turn Lane Headway Factor 1.09 1.00
Two way Left Turn Lane Headway Factor 1.09 1.09 1.09 1.09 1.09 1.09 1.09 1.09
Headway Factor 1.09
Turning Speed (k/h) 14 24 14 24 14 24 14 Number of Detectors 2 1 1 2 1 1 1 1 Detector Template Thru Right Left Thru Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0
Number of Detectors 2 1 1 2 1 1 1 Detector Template Thru Right Left Thru Left Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 <
Detector Template Thru Right Left Thru Left Left Right Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Position(m) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 </td
Leading Detector (m) 30.5 6.1 6.1 30.5 6.1 6.1 6.1 Trailing Detector (m) 0.0
Trailing Detector (m) 0.0
Detector 1 Position(m) 0.0 0.1 1.8 6.1 8.1 8.2 8.2 8.2 9.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Detector 1 Size(m) 1.8 6.1 6.1 1.8 6.1 6.1 6.1 Detector 1 Type CI+Ex
Detector 1 Type CI+Ex
Detector 1 Channel Detector 1 Extend (s) 0.0 0
Detector 1 Extend (s) 0.0 0.
Detector 1 Queue (s) 0.0
Detector 1 Delay (s) 0.0
Detector 2 Position(m) 28.7 28.7 Detector 2 Size(m) 1.8 1.8
Detector 2 Size(m) 1.8 1.8
Detector 0 Times OI Fig.
Detector 2 Type CI+Ex CI+Ex
Detector 2 Channel
Detector 2 Extend (s) 0.0 0.0
Turn Type NA Perm Prot NA Perm Prot Perm
Protected Phases 2 1 6 8
Permitted Phases 2 8 8
Detector Phase 2 2 1 6 8 8

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	51.0	51.0	28.0	79.0	31.0	31.0	31.0
Total Split (%)	46.4%	46.4%	25.5%	71.8%	28.2%	28.2%	28.2%
Maximum Green (s)	44.6	44.6	21.8	72.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	47.2	47.2	21.5	74.8		22.4	22.4
Actuated g/C Ratio	0.43	0.43	0.20	0.68		0.20	0.20
v/c Ratio	0.96	0.55	0.88	0.52		0.80	0.61
Control Delay	43.5	8.3	59.4	10.0		51.5	9.1
Queue Delay	0.0	0.2	0.0	0.0		0.0	0.0
Total Delay	43.5	8.5	59.4	10.0		51.5	9.1
LOS	D	Α	Е	В		D	Α
Approach Delay	34.2			25.6		35.1	
Approach LOS	С			С		D	
Queue Length 50th (m)	~112.0	4.2	53.6	56.9		50.8	0.0
Queue Length 95th (m)	#181.2	40.3	#79.3	73.0		67.3	21.9
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1380	857	638	2278		726	573
Starvation Cap Reductn	0	57	0	0		0	0
Spillback Cap Reductn	0	0	0	108		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.96	0.59	0.85	0.54		0.73	0.58

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 20 (18%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.96 Intersection Signal Delay: 31.0

Intersection Signal Delay: 31.0
Intersection Capacity Utilization 88.6%

Analysis Period (min) 15

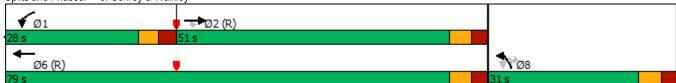
Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Conroy & Walkley



Intersection LOS: C

ICU Level of Service E

	۶	→	•	•	+	4	1	†	/	/	↓	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	•	7	7	^	7	7	ᡮ ᡮᡗ∌		7	Φβ	
Traffic Volume (vph)	47	81	322	232	38	102	47	595	81	37	1111	21
Future Volume (vph)	47	81	322	232	38	102	47	595	81	37	1111	21
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		50.0	55.0		55.0	0.0		0.0	110.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	40.0			40.0			10.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Ped Bike Factor	0.99		0.98	0.99		0.98		0.99		0.99	1.00	
Frt			0.850			0.850		0.982			0.997	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1674	1728	1483	1642	1618	1483	1642	4610	0	1537	3296	0
Flt Permitted	0.732			0.555			0.151			0.370		
Satd. Flow (perm)	1281	1728	1452	953	1618	1453	261	4610	0	590	3296	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			121			73		27			2	
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		197.7			271.8			372.7			348.5	
Travel Time (s)		14.2			19.6			22.4			20.9	
Confl. Peds. (#/hr)	10		10	10		10	10		20	20		10
Confl. Bikes (#/hr)			3			3			5			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	2%	3%	10%	2%	3%	2%	8%	10%	2%	12%
Adj. Flow (vph)	47	81	322	232	38	102	47	595	81	37	1111	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	47	81	322	232	38	102	47	676	0	37	1132	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	R NA	L NA	Left	R NA	Left	Left	Right	L NA	Left	R NA
Median Width(m)		3.5			3.5			7.0	•		6.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	3	8	8	2	2		6	6	
		•	•				_	_		•		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	43.9	43.9	43.9	11.3	43.9	43.9	30.3	30.3		30.3	30.3	
Total Split (s)	44.0	44.0	44.0	15.0	59.0	59.0	36.0	36.0		36.0	36.0	
Total Split (%)	46.3%	46.3%	46.3%	15.8%	62.1%	62.1%	37.9%	37.9%		37.9%	37.9%	
Maximum Green (s)	37.1	37.1	37.1	9.3	52.1	52.1	29.7	29.7		29.7	29.7	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7		3.7	3.7	
All-Red Time (s)	3.6	3.6	3.6	2.4	3.6	3.6	2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.9	6.9	6.9	5.7	6.9	6.9	6.3	6.3		6.3	6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	30.0	30.0	30.0		30.0	30.0	17.0	17.0		17.0	17.0	
Pedestrian Calls (#/hr)	5	5	5		5	5	10	10		10	10	
Act Effct Green (s)	21.2	21.2	21.2	37.4	36.2	36.2	45.6	45.6		45.6	45.6	
Actuated g/C Ratio	0.22	0.22	0.22	0.39	0.38	0.38	0.48	0.48		0.48	0.48	
v/c Ratio	0.16	0.21	0.77	0.52	0.06	0.17	0.38	0.30		0.13	0.72	
Control Delay	26.7	27.8	32.3	23.2	15.2	6.0	33.9	16.8		20.3	25.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	26.7	27.8	32.3	23.2	15.2	6.0	33.9	16.8		20.3	25.5	
LOS	С	С	С	С	В	Α	С	В		С	С	
Approach Delay		30.9			17.7			17.9			25.4	
Approach LOS		С			В			В			С	
Queue Length 50th (m)	6.6	11.5	33.1	28.1	4.1	3.1	4.6	22.4		3.2	73.5	
Queue Length 95th (m)	11.5	17.2	47.0	30.7	6.9	8.7	#22.3	42.1		12.1	#151.6	
Internal Link Dist (m)		173.7			247.8			348.7			324.5	
Turn Bay Length (m)	45.0		50.0	55.0		55.0				110.0		
Base Capacity (vph)	500	674	640	443	887	829	125	2224		282	1581	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.09	0.12	0.50	0.52	0.04	0.12	0.38	0.30		0.13	0.72	

Other

Area Type: Cycle Length: 95

Actuated Cycle Length: 95

Offset: 2 (2%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

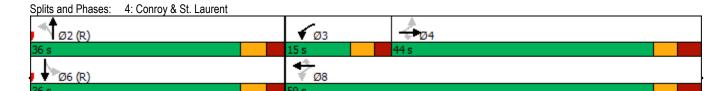
Maximum v/c Ratio: 0.77 Intersection Signal Delay: 23.2

Intersection LOS: C ICU Level of Service E

Intersection Capacity Utilization 87.0% Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.			ની
Traffic Volume (vph)	23	96	107	76	172	51
Future Volume (vph)	23	96	107	76	172	51
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.891		0.944			
Flt Protected	0.990					0.963
Satd. Flow (prot)	1462	0	1641	0	0	1592
Flt Permitted	0.990					0.963
Satd. Flow (perm)	1462	0	1641	0	0	1592
Link Speed (k/h)	50		50			50
Link Distance (m)	137.6		234.0			146.3
Travel Time (s)	9.9		16.8			10.5
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)		4		5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	8%	2%	3%	4%	20%
Adj. Flow (vph)	23	96	107	76	172	51
Shared Lane Traffic (%)						<u> </u>
Lane Group Flow (vph)	119	0	183	0	0	223
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	2.0		10.0			10.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
	44.00/			10		

Intersection Capacity Utilization 41.9% Analysis Period (min) 15

ICU Level of Service A

	•	→	+	4	/	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	1>		W	
Traffic Volume (vph)	1	249	122	11	9	1
Future Volume (vph)	1	249	122	11	9	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.989		0.986	
Flt Protected					0.957	
Satd. Flow (prot)	0	1695	1682	0	1663	0
Flt Permitted					0.957	
Satd. Flow (perm)	0	1695	1682	0	1663	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		40.4	197.7		90.8	
Travel Time (s)		2.9	14.2		6.5	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	5%	5%	1%	1%	1%
Adj. Flow (vph)	1	249	122	11	9	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	250	133	0	10	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	3.5		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 24.7% Analysis Period (min) 15

ICU Level of Service A

	•	→	←	4	/	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ન	ĵ.		W	
Traffic Volume (vph)	1	247	119	4	3	0
Future Volume (vph)	1	247	119	4	3	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.996			
Flt Protected					0.950	
Satd. Flow (prot)	0	1695	1691	0	1674	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	1695	1691	0	1674	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		137.6	40.4		92.1	
Travel Time (s)		9.9	2.9		6.6	
Confl. Peds. (#/hr)	10			10		
Confl. Bikes (#/hr)				3		3
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	5%	5%	1%	1%	1%
Adj. Flow (vph)	1	247	119	4	3	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	248	123	0	3	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary	0.11					
Area Type: Control Type: Unsignalized	Other					

Control Type: Unsignalized Intersection Capacity Utilization 24.6% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥ /		^			ની
Traffic Volume (vph)	1	10	238	1	13	223
Future Volume (vph)	1	10	238	1	13	223
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.877		0.999			
Flt Protected	0.995					0.997
Satd. Flow (prot)	1538	0	1694	0	0	1694
Flt Permitted	0.995					0.997
Satd. Flow (perm)	1538	0	1694	0	0	1694
Link Speed (k/h)	50		50			50
Link Distance (m)	105.3		30.4			100.3
Travel Time (s)	7.6		2.2			7.2
Confl. Peds. (#/hr)				10	10	
Confl. Bikes (#/hr)		3		3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	5%	1%	1%	5%
Adj. Flow (vph)	1	10	238	1	13	223
Shared Lane Traffic (%)						
Lane Group Flow (vph)	11	0	239	0	0	236
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	Ŭ	3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	5.0		0.0			0.0
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14		14	24	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized Intersection Capacity Utilization 33.6% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Lane Configurations	*	Z Z	ሻሻ	^	1,00	ሻሻ	7
Traffic Volume (vph)	1319	474	545	1180	45	484	334
Future Volume (vph)	1319	474	545	1180	45	484	334
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	75.0	200.0	1000	1000	0.0	0.0
Storage Lanes		1 1	200.0			2	1
Taper Length (m)		- 1	50.0			10.0	
Lane Util. Factor	0.95	1.00	0.97	0.95	0.95	0.97	1.00
	0.95	0.95		0.95	0.95	0.97	0.95
Ped Bike Factor			0.99				
Frt		0.850	0.050			0.050	0.850
Flt Protected	2004	4.400	0.950	00.40	^	0.950	4400
Satd. Flow (prot)	3221	1483	3185	3349	0	3248	1469
Flt Permitted	2024	4440	0.950	00.40	^	0.950	4 400
Satd. Flow (perm)	3221	1410	3154	3349	0	3248	1402
Right Turn on Red		Yes					Yes
Satd. Flow (RTOR)		427					334
Link Speed (k/h)	50			50		60	
Link Distance (m)	169.1			271.7		348.5	
Travel Time (s)	12.2			19.6		20.9	
Confl. Peds. (#/hr)		30	30				20
Confl. Bikes (#/hr)		4					4
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	2%	3%	1%	1%	1%	3%
Adj. Flow (vph)	1319	474	545	1180	45	484	334
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1319	474	545	1180	0	529	334
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	R NA	L NA	R NA
Median Width(m)	7.0			9.0		10.5	
Link Offset(m)	0.0			0.0		0.0	
Crosswalk Width(m)	5.0			5.0		5.0	
Two way Left Turn Lane	0.0			0.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	1.03	14	24	1.00	14	24	1.09
Number of Detectors	2	14	1	2	14	1	1
Detector Template	Thru	Right	Left	Thru	Left	Left	Right
•	30.5	6.1	6.1	30.5	6.1	6.1	6.1
Leading Detector (m)				0.0			0.0
Trailing Detector (m)	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	1.8	6.1	6.1	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	28.7			28.7			
Detector 2 Size(m)	1.8			1.8			
Detector 2 Type	CI+Ex			CI+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA	Perm	Prot	NA	Perm	Prot	Perm
Protected Phases	2		1	6		8	2
Permitted Phases		2			8		8
Detector Phase	2	2	1	6	8	8	8
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Lane Group	EBT	EBR	WBL	WBT	NBU	NBL	NBR
Switch Phase							
Minimum Initial (s)	10.0	10.0	5.0	10.0	5.0	5.0	5.0
Minimum Split (s)	36.4	36.4	16.0	24.7	30.4	30.4	30.4
Total Split (s)	59.0	59.0	30.0	89.0	31.0	31.0	31.0
Total Split (%)	49.2%	49.2%	25.0%	74.2%	25.8%	25.8%	25.8%
Maximum Green (s)	52.6	52.6	23.8	82.6	24.6	24.6	24.6
Yellow Time (s)	3.7	3.7	3.3	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.9	2.7	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.4	6.4	6.2	6.4		6.4	6.4
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None	None
Walk Time (s)	10.0	10.0			7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0			17.0	17.0	17.0
Pedestrian Calls (#/hr)	20	20			15	15	15
Act Effct Green (s)	54.7	54.7	23.1	84.0		23.2	23.2
Actuated g/C Ratio	0.46	0.46	0.19	0.70		0.19	0.19
v/c Ratio	0.90	0.54	0.89	0.50		0.84	0.62
Control Delay	40.3	5.8	64.9	9.5		59.6	9.8
Queue Delay	6.3	0.0	0.0	0.0		0.0	0.0
Total Delay	46.7	5.8	64.9	9.5		59.6	9.8
LOS	D	Α	Е	Α		Е	Α
Approach Delay	35.9			27.0		40.3	
Approach LOS	D			С		D	
Queue Length 50th (m)	140.4	5.8	59.4	58.6		56.5	0.0
Queue Length 95th (m)	#184.1	28.0	#84.7	72.0		74.5	23.7
Internal Link Dist (m)	145.1			247.7		324.5	
Turn Bay Length (m)		75.0	200.0				
Base Capacity (vph)	1467	874	631	2343		665	552
Starvation Cap Reductn	120	11	0	0		0	0
Spillback Cap Reductn	0	0	0	0		0	0
Storage Cap Reductn	0	0	0	0		0	0
Reduced v/c Ratio	0.98	0.55	0.86	0.50		0.80	0.61

Area Type: Cycle Length: 120 Other

Actuated Cycle Length: 120

Offset: 20 (17%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90 Intersection Signal Delay: 33.2

Intersection Capacity Utilization 88.6%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

