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ADEQUACY OF SERVICES REPORT

FOR

BARRHAVEN CONSERVANCY DEVELOPMENT CORPORATION

BARRHAVEN CONSERVANCY EAST PHASE 5

CITY OF OTTAWA

PROJECT NO.: 20-1180

DECEMBER 2022 – 1ST SUBMISSION

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**ADEQUACY OF SERVICES REPORT
FOR
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**CITY OF OTTAWA
PROJECT NO: 20-1180**

1.0 INTRODUCTION

David Schaeffer Engineering Limited (DSEL) has been retained to update an Adequacy of Services Report (AES) in support of the Barrhaven Conservancy “Phase 5” development area on behalf of Barrhaven Conservancy Development Corporation (BCDC). This area (being referred to as “Phase 5”) is part of a previously approved draft plan of subdivision within the greater Barrhaven Conservancy development area (City file no. D07-16-20-0021).

The overall Conservancy land area is approximately 139.7 ha (all land use components) and is located within the City of Ottawa urban boundary in the Barrhaven ward. As illustrated in **Figure 1**, the site is located north of the Jock River, east of Highway 416, west of Greenbank Road (and the Kennedy-Burnett Stormwater Facility), and south of both McKenna Casey Drive and Strandherd Drive.

The focus of this report is for the **Conservancy East (Phase 5)** draft plan area consisting of vacant land that is located east of the existing Foster Ditch, which bisects the overall BCDC landholdings, and west of Borrisokane Road. The subject lands are an approximately 19.4 ha irregular parcel including parts of 3288, and 3300 Borrisokane Road. Of this, approximately 13.82 ha in area (including right-of-ways environmental areas and open space) are considered in the servicing review with the proposed updated development draft plan **Figure 2B** provided in the **Drawings** section of this report for reference. Also provided is Figure 2A which illustrates the portion of the prior approved draft plan and the “Phase 5” area being revised. The development area is planned to be developed with a mix of detached single homes, townhomes, park blocks, open spaces and a road network.

The Conservancy East Phase 5 development area is outside of the Jock River 100-year limit as confirmed by the Rideau Valley Conservation Authority (RVCA). Refer to the RVCA confirmation letter in **Appendix D**. The 100-year regulatory flood line is demonstrated in Drawing 1 (Grading) and Drawing 3 (Stormwater) in the **Appendix**.

The objective of this report is to provide sufficient detail to demonstrate that the updated development plan area can be supported by municipal services.

1.1 Existing Conditions

The **Conservancy East (Phase 5)** property is relatively flat with the existing elevations ranging from ~91.5 m in the north to 91 m in the south. All existing flows are either overland to the Jock River or conveyed to the Jock River by way of the Foster Ditch and Borrisokane Road ditches which is adjacent to the subject property. The property is within the Jock River watershed and is under the jurisdiction of the RVCA.

1.2 Summary of Pre-Consultation

The following provides a summary of the pre-consultation:

1.2.1 Ministry of the Environment, Conservation and Parks (MECP)

Prior consultations associated with the Conservancy Phase 2-4 development east of Borrisokane Road were previously undertaken for the approval of those phases of the development area.

A pre-consultation with the local MECP office has not yet been completed for the balance of the Conservancy development area until the functional design details and requirements have been finalized with the City of Ottawa.

1.2.2 Rideau Valley Conservation Authority (RVCA)

Multiple consultations, analysis and submissions were coordinated with the RVCA to establish that the development area is outside of the Jock River 100-year limit. See the RVCA documentation in **Appendix D** for reference.

1.3 Existing Permits / Approvals

Key approvals associated with the advancement of development of the Barrhaven Conservancy area, are presented in the following table. The most relevant approvals are the Environmental Compliance Approval (ECA) for the South Nepean Collector sanitary trunk sewer as well as sanitary sewer ECA for the Conservancy development area east of Borrisokane Road. The documents are provided in **Appendix A** for reference.

Table 1A: Existing Permits / Approvals

Agency	Approval Type	Approval Number	Remarks
Ministry of the Environment, Conservation and Parks (MECP)	Environmental Compliance Approval	# 8129-AB7LDF (June 23, 2016)	South Nepean Collector existing approval (sanitary outlet for development area)
(MECP)	Environmental Compliance Approval	# 4357-CHMQEM (Sept. 1, 2022)	Sanitary and storm sewer approvals for Conservancy lands east of Borrisokane Road
MECP	Permit to take Water	#5633-C2RQPL (May 26, 2021)	Water taking from Building Excavation, Site Servicing, SWMW, In-Water Works, Poned Surface Water
Rideau Valley Conservation Authority (RVCA)	RVCA Letter of Permission under O.Reg. 174/06	RV5-4419	Letter of permission related to placement of fill within a regulated area.

1.4 Required Permits / Approvals

The City of Ottawa must approve detailed engineering design drawings and reports prior to future construction of the municipal infrastructure identified in this report. This will occur as part of the Plan of Subdivision application process and detailed design.

Based on pre-consultation with City staff, the additional approvals and permits listed in the following table are expected to be required prior to construction of the municipal infrastructure detailed herein. Please note that other permits and approvals may be required, as detailed in the other studies to be submitted as part of the Plan of Subdivision application (e.g. *Tree Conservation Report, Environmental Impact Statement, Phase 1 Environmental Site Assessment, Headwater Drainage Feature Assessment, etc.*)

Table 1B: Required Permits/Approvals

Agency	Permit/Approval Required	Trigger	Remarks
MECP	Environmental Compliance Approval	Construction of new sanitary and storm sewers throughout the subdivision.	The MECP will review the sanitary and storm sewer design through the City of Ottawa transfer of review process.
MECP	Environmental Compliance Approval	Implementation of oil-grit separator units and LIDs for quality control.	The MECP will review the stormwater management appurtenance design through the City of Ottawa transfer of review process.

MECP	Permit to Take Water	Construction of proposed land uses (e.g. basements for residential homes) and services.	Pumping of groundwater may be required during construction, given groundwater conditions and proposed land uses and on-site/off-site municipal infrastructure.
City of Ottawa	MECP Form 1 – Record of Watermains Authorized as a Future Alteration.	Construction of watermains throughout the subdivision	The City of Ottawa will review the watermains on behalf of the MECP through the Form 1 – Record of Watermains Authorized as a Future Alteration.
RVCA	Permit under Ontario Regulation 174/06, RVCA's Development, Interference with Wetlands and Alterations to Shorelines and Watercourses Regulation	Grading (proposed development & potential temporary access roads) within the subject lands (i.e. crossing of Fraser-Clarke Watercourse)	Supporting applications and documentation as required through consultation with the RVCA.
RVCA	Outlets to Jock River	In conjunction with issuance of MECP applications	Supporting applications and documentation as required through consultation with the RVCA.
RVCA	Alteration to Watercourses	As necessary through consultation with the RVCA	Supporting applications and documentation as required through consultation with the RVCA.
City of Ottawa	Commence Work Notification (CWN)	Construction of new sanitary and storm sewers throughout the subdivision	The City of Ottawa will issue a commence work notification for construction of the sanitary and storm sewers once an approval is issued by the MECP.

2.0 GUIDELINES, PREVIOUS STUDIES, AND REPORTS

2.1 Existing Studies, Guidelines, and Reports

The following studies were utilized in the preparation of this report.

- Ottawa Sewer Design Guidelines, City of Ottawa, *SDG002*, October 2012 (*City Standards*)

- Technical Bulletin ISDTB-2014-01
City of Ottawa, February 5, 2014
(ITSB-2014-01)
- Technical Bulletin PIEDTB-2016-01
City of Ottawa, September 6, 2016
(PIEDTB-2016-01)
- Technical Bulletin ISTB-2018-01
City of Ottawa, March 21, 2018
(ISTB-2018-01)
- Technical Bulletin ISTB-2018-04
City of Ottawa, June 27, 2018
(ISTB-2018-04)
- Ottawa Design Guidelines – Water Distribution
City of Ottawa, July 2010.
(*Water Supply Guidelines*)
 - Technical Bulletin ISD-2010-2
City of Ottawa, December 15, 2010.
(ISD-2010-2)
 - Technical Bulletin ISDTB-2014-2
City of Ottawa, May 27, 2014.
(ISDTB-2014-2)
 - Technical Bulletin ISTB-2018-02 / ISTB-2019-02
City of Ottawa, March 21, 2018 / July 08, 2019
(ISTB-2018-02 / ISTB-2019-02)
- Design Guidelines for Sewage Works,
Ministry of the Environment, Conservation and Parks, 2008. (formerly MOECC)
(*MECP Design Guidelines*)
- Stormwater Planning and Design Manual,
Ministry of the Environment, March 2003.
(*SWMP Design Manual*)
- City of Ottawa Official Plan,
adopted by Council 2003.
(*Official Plan*)
- City of Ottawa Secondary Plan – Former Nepean – South Nepean Urban Area –
Areas 9 and 10,
Adopted by Council 2003.
(*Secondary Plan*)

- South Nepean Collector: Phase 2 Hydraulics Review / Assessment Technical Memorandum
Novatech, August 2015
(*Novatech SNC Memo*)
- South Nepean Collector: Phase 2 Preliminary Design Report,
Novatech, March 2016
(*Novatech SNC Design Report*)
- Strandherd Drive Widening Project, South Nepean Collector: Phase 3 Sanitary Flow Calculations
Novatech, May 2019
(*2019 Novatech SNC Design Report*)
- Hydraulic Potable Water Assessment for Barrhaven Conservancy Development Corporation, March 2021
(*Stantec Hydraulic Analysis*)
- Jock River Reach One Subwatershed Study
Stantec, 2007
(*Jock River SWS*)
- Geotechnical Investigation, Proposed Residential Development, Conservancy Lands East, Ottawa, Ontario
Paterson Group, September 24, 2019 (Project No. PG5036-1)
(*Geotechnical Report*)
- Environmental Impact Statement for Barrhaven Conservancy East
Kilgour & Associates Ltd., July 29, 2020
(*Kilgour EIS*)
- Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River): Water Distribution System Analysis, Stantec, June 2, 2022
(*Stantec Hydraulic Analysis - East*)
- Adequacy of Services Report for Barrhaven Conservancy Development Corporation, Barrhaven Conservancy East
David Schaeffer Engineering Ltd., July 2021
(*DSEL East FSR*)
- Design Brief for Barrhaven Conservancy East – Phase 2, 3, & Jock River
David Schaeffer Engineering Ltd., June 2022
(*DSEL East Design Brief*)

3.0 WATER SUPPLY SERVICING

3.1 Existing Water Supply Services

The subject property is located adjacent to the City of Ottawa's Pressure Zone (PZ) 3SW (previously known as PZ BARR). PZ SUC services the lands that are east of the subject property, as well as south of the Jock River.

The City of Ottawa has recently reconfigured the pressure zones servicing Barrhaven and the South Urban Community (SUC) in order to improve reliability and efficiency and to increase pumping capacity to accommodate for future growth in the area. Work is ongoing. There are three pumping stations servicing Zone 3SW and Zone SUC as follows: the Fallowfield Road Pumping Station (FRPS), the Barrhaven Pumping Station (BPS) and the Ottawa South Pumping Station (OSPS).

There are future trunk watermains proposed in the vicinity of the subject property (i.e. along Greenbank Road) which will provide water service to development lands to the east and south of Conservancy East. These services will be further extended to provide the requisite water supply to the development area.

3.2 Water Supply Servicing Design

Stantec Consulting Limited was retained to perform a hydraulic assessment for the Conservancy East Lands. The ***Hydraulic Potable Water Assessment for Barrhaven Conservancy Development Corporation (Stantec Hydraulic Analysis)*** prepared by Stantec (March 2021) previously supported the advancement of the Conservancy East lands east of Borrisokane road. Subsequently, as part of the detailed design for the approved phases east of Borrisokane Road, Stantec prepared an updated study "***Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River): Water Distribution System Analysis*** (June 2022 – ***Stantec Hydraulic Analysis - East***) which is enclosed in ***Appendix B*** for reference. Note that phasing references have changed for the development area and the "Phase 4" area referenced in the Stantec study represents the "Phase 5" which is the subject of this updated draft plan – See Figure 1-2 of the ***Stantec Hydraulic Analysis – East*** study for reference. As well, the layout analyzed in the Stantec analysis differs slightly but is not expected to impact the serviceability.

The analysis reviewed the system requirements of the development area on the west and east sides of Borrisokane Road but only the detailed design of the areas east of Borrisokane Road were advanced to detailed design.

The proposed water servicing layout is presented in ***Figure 3***.

The following table summarizes the relevant Water Supply Design Criteria which will be employed in the design of the subject property.

Table 2A: Water Supply Design Criteria

Design Parameter	Value
<i>Extracted from Section 4: Ottawa Design Guidelines, Water Distribution (July 2010)</i>	
Residential – Detached Single	3.4 p/unit
Residential – Townhome/ Semi	2.7 p/unit
Residential – Apartment	1.8 p/unit
Minimum Watermain Size	150 mm diameter
Minimum Depth of Cover	2.4 m from top of watermain to finished grade
During normal operating conditions desired operating pressure is within	350 kPa and 480kPa
During fire flow operating pressure must not drop below	140 kPa
<i>Stantec Hydraulic Analysis, Stantec, July 20, 2017 for Population Exceeding 3000 Persons</i>	
Residential – Detached Single	180 L/cap/day
Residential – Rear Lane Town	198 L/cap/day
Residential – Back-to-Back	198 L/cap/day
Outdoor Water Demand	1049 L/unit/day (single detached)
Basic Day	Population x Demand
Max Day	Basic Day + Outdoor Water Demand

3.2.1 Fire Flow Demand

Fire Flow requirements are established in the boundary condition request found in **Appendix B** as prepared by Stantec. Based on anticipated unit configurations and separations the City’s fire flow cap of 10,000 L/min for single dwellings and traditional townhomes as outlined in *ISDTB-2014-02* does not apply and separation of fire areas with units of ordinary construction, as well as architectural elements, are required to meet target fire flows. The fire flows are calculated in accordance with the Fire Underwriters Survey’s Water Supply for Public Fire Protection Guideline (1999). Detailed FUS calculations can be found in the Stantec reporting.

3.2.2 Boundary Conditions

To support the preparation of a hydraulic analysis for the subdivision, boundary conditions were provided by the City of Ottawa for the anticipated water demands and are summarized in the following table. See **Appendix B** for full details of the boundary condition request submitted.

Table 2B: Boundary Conditions (from *Stantec Hydraulic Analysis – East report*)

HGL (m) - Zone SUC Servicing Conditions			
Demand Scenario	Two Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	150.0	150.0	
PKHR	144.2	144.0	
AVDY +FF	138.7	135.1	
MXDY +FF	137.0	133.2	
Demand Scenario	Two Connections with Upgrades ⁽⁴⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	149.5	149.5	
PKHR	144.1	144.1	
AVDY +FF	138.6	139.8	
MXDY +FF	136.8	138.1	
Demand Scenario	Three Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	Connection 3 ⁽³⁾
AVDY	149.5	149.5	149.5
PKHR	144.5	144.4	142.0
AVDY +FF	138.6	135.1	137.4
MXDY +FF	137.1	133.4	134.8

(1) Ground elevation at Connection 1 (Chapman Mills Drive) = 92.80 m
(2) Ground elevation at Connection 2 (Danson Gardens Grv / Darjeeling Ave) 91.80 m
(3) Ground elevation at Connection 3 (Flagstaff Dr) 92.10 m
(4) Upgrades to existing water distribution required to increase HGL at Connection 2; upscale existing 203mm diameter watermain on Danson Gardens Grv to a 305mm watermain
(5) For scenarios where ultimate conditions will include three connections, the boundary conditions for two connections (without upgrades) were used when only connections 1 and 2 are in place (i.e. for modelling Phases 2 and 3.

3.2.3 Water Demand Calculations

A summary of water demands for the subject site is presented in the following table as derived from the criteria above and the *Stantec Hydraulic Analysis* found in **Appendix B**.

Table 2C: Water Demand Estimate

	Unit Count Conservancy East	Pop ⁽¹⁾	AVDY ⁽²⁾ (L/s)	OWD ⁽³⁾ (L/s)	MXDY ⁽⁴⁾ (L/s)	PKHR ⁽⁵⁾ (L/s)
Single Family	782	2,659	8.62	9.49	21.55	47.38
Townhouse	606	1,636	5.30	0	13.25	29.17
Totals	1,388⁽⁶⁾	4,296	13.92	9.49	34.80	76.55

(1) Population per unit is 3.4 for Single Family and 2.7 for Townhomes
(2) AVDY = Average Day
(3) OWD (outdoor water demand) = 1,049 L/unit/day for Singles
(4) MXDY = Maximum Day
(5) PKHR = Peak Hour
(6) Total unit count may vary slightly from final layouts but are estimated to be within +/-2.5%.
(7) See Stantec Hydraulic Analysis in **Appendix B** for details.

3.3 Summary of Hydraulic Modeling Analysis

A watermain analysis has been prepared to confirm that the network is sized adequately, which is the greater of maximum day plus fire and maximum hour. City review comments on the current **Stantec Hydraulic Analysis - East** note that 'Option B' is the preferred system configuration (sizing and layout for three connections) and those results are presented below. For full details of the assessments refer to the **Stantec Hydraulic Analysis - East**, enclosed in **Appendix B**.

System Pressures

The modeling indicates that the development can be adequately serviced by the proposed watermain network. Modeled service pressures for the development are summarized the following table. The detailed pipe and junction tables are contained in the **Stantec Hydraulic Analysis**, enclosed in **Appendix B**.

Table 2D: Summary of Available System Pressures

	AVDY Maximum Pressure		Peak Hour Demand Minimum Pressure	
	kPA	psi	kPA	psi
Option B – 3 Connections	559	81.14 (J55, J60)	482	69.83 (J103)

*Note: See model results in the Appendix D of the **Stantec Hydraulic Analysis** memo (buildout of all phases).*

The generally accepted best practice is to design new water distribution systems to operate between 350 kPa (50 psi) and 480 kPa (70 psi) as outlined in the City of Ottawa Design Guidelines. Where pressures exceed 80psi pressure reducing valves (PRV) shall be implemented as per the Ontario Building Code.

Available Fire Flows

The minimum allowable pressure under fire flow conditions is 140 kPa (20 psi) at the location of the fire. A summary of the available fire flows is presented in the following table. The detailed fire flow reports are found in the **Stantec Hydraulic Analysis - East** enclosed in **Appendix B**.

Table 2E: Summary of Available Fire Flows

	Required Fire Flow (L/s)	Minimum Available Flow (L/s)	Junction ID
Option B – 3 Connections	217	250	J86, J87

*Note: See model results in the Appendix D of the **Stantec Hydraulic Analysis – East** memo (buildout of all phases). Exception is the phase west of Borrisokane Road where the anticipated cul-de-sac fire flow node can be managed by procedures noted in ISDTB-2018-02 (See Section 3.2 of Stantec report)*

As shown in the above table, the model predicts the network will be able to provide all required fire flows. Detailed results are included in the ***Stantec Hydraulic Analysis - East***, enclosed in ***Appendix B***.

System Reliability

Various major watermain failure scenarios were reviewed by Stantec. Some scenarios resulted in potential reliability issues which have been resolved in the updated watermain layout with additional looping in the northwest area of the design. See discussion in Section 3.3 of the ***Stantec Hydraulic Analysis -East***.

3.4 Water Supply Conclusion

The subject lands are have been reviewed by Stantec to confirm that servicing is feasible from the SUC pressure zone. Future watermain extensions from Nepean Town Centre development areas, being constructed as part of Phase 2-4 approvals, will facilitate servicing to the Conservancy East Phase 5 lands via watermain extension along the future Chapman Mills Drive extension and through the Claridge “Burnett Lands” development area. Future modelling at the detailed design stage will confirm phasing of the extensions of trunk watermains and sizing of the local watermain network. The proposed water supply design will conform to all relevant City and MECP Guidelines and Policies.

4.0 WASTEWATER SERVICING

4.1 Existing Wastewater Services

Per the ***South Nepean Collector (SNC) Wastewater Servicing Study and Functional Design Report*** by Dillon in October 2003 (***Dillon SNC Report***), the subject property is tributary to the South Nepean Collector (SNC) sewer as urban development land.

The SNC (previously called the Jock River Collector) sewer operates north of the subject property within Strandherd Drive prior to travelling south down a Chapman Mills Drive (CMD) and then turns eastward within the future CMD right-of-way (ROW).

The ***South Nepean Collector Phase 2: Hydraulics Review / Assessment*** memo was prepared by Novatech Engineering Consultants on August 20, 2015 (***Novatech SNC Memo***) to provide an update to the sanitary design flows for Phase 2 of the South Nepean Collector, as previously documented in the ***South Nepean Collector (SNC) – Functional Design Report and Update*** by Dillon in 2012 (***Dillon SNC Report and Update***).

4.2 South Nepean Collector Phase 3 – Preliminary Design

The 2015 ***Novatech SNC Memo*** contemplated that the Conservancy Phase 1 development area (north of the Fraser-Clarke Watercourse) would be serviced by the 900 mm diameter SNC sewer running adjacent to the property within the future extension of CMD. This is represented by area “A6-E” within the “***Sanitary Drainage Areas and Land Use – Fig.1***” plan within the 2015 Novatech memo (note that the actual tributary area and population varied slightly).

For the Phase 3 extension of the SNC, Novatech has prepared another review of sanitary flows within their technical memorandum titled “***Strandherd Drive Widening Project, South Nepean Collector Phase 3: Sanitary Flow Calculations***” May 30, 2019 (***2019 Novatech SNC Memo***). The memorandum along with the design sheet calculations from the Novatech memo are provided in ***Appendix C*** for reference along with DSEL annotations on key items in the figure and design sheets. The updated “***Sanitary Drainage Areas and Land Use – Fig.1***” (May 2019) plan is essentially reflective of the same tributary information that was provided in the 2015 study (the plan has been marked up to reflect the Conservancy areas as a frame of reference). The associated design sheet also reflects updated City wastewater design criteria that was not accounted for in the 2015 study and is discussed further in the following section.

Report excerpts are provided in ***Appendix C*** for the SNC Phase 2 analysis as well as draft information associated with the Phase 3 extension. The location of the SNC sewer is shown in ***Figure 4***.

4.3 Wastewater Design

The subject property is planned to be serviced by an internal gravity sanitary sewer system that is to generally follow the local road network. The wastewater servicing plan can be seen in **Drawing 4**.

The prior report proposed that the drainage area of the SNC sanitary sewer be expanded to include the entirety of the Conservancy property. The sewer network will connect to the off-site SNC sanitary sewer within the future CMD at existing manhole 'SANMH8' as identified in the Novatech SNC Phase 2 design Drawing No. 20 provided in **Appendix C** for reference (City contract number ISD14-2033). As noted in the prior section, the 2015 **Novatech SNC Memo** was derived flows based on the City guideline parameters of the time (namely 350 L/capita/day, infiltration allowance of 0.28 L/s/ha and commercial properties at 50,000 L/ha/d). The following table summarizes the new City design guidelines and criteria to be applied to the **Conservancy East** sewer design as well for the determination of the projected flows to be tributary to the SNC along the frontage of the Conservancy Phase 1 development area.

Table 3: Wastewater Design Criteria

Design Parameter	Value
Current Design Guidelines	
Residential - Single Family	3.4 p/unit
Residential – Townhome/ Semi	2.7 p/unit
Residential – Apartment	1.8 p/unit
Average Daily Demand	280 L/d/person
Peaking Factor	Harmon's Peaking Factor. Max 4.0, Min 2.0
Commercial / Institutional Flows	28,000 L/ha/day
Commercial / Institutional Peak Factor	1.5
Infiltration and Inflow Allowance	0.33 L/s/ha
Park Flows	28,000 L/ha/d
Park Peaking Factor	1.0
Sanitary sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} AR^{2/3} S^{1/2}$
Minimum Sewer Size	200mm diameter
Minimum Manning's 'n'	0.013
Minimum Depth of Cover	2.5m from crown of sewer to grade
Minimum Full Flowing Velocity	0.6m/s
Maximum Full Flowing Velocity	3.0m/s
<i>Extracted from Sections 4 and 6 of the City of Ottawa Sewer Design Guidelines, October 2012, and associated Technical Bulletins.</i>	

The sanitary design sheet for the lands east of Borrisokane Road is provided in **Appendix C** for reference. Within that design sheet the area and flows from the lands west of Borrisokane Road are highlighted where flows enter that development area at the westward stub from MH10A. That design sheet projected a flow of 77.81 L/s. Based on the updated Phase 5 draft plan, and updated concept plans for the

Conservancy West development area (west of the Foster Ditch), the flows shown at the eastern limit of Phase 5 (see Phase 5 design sheet in **Appendix C**) is now ~68.96 L/s at MH 532A. As such, downstream systems are sufficient and no negative impacts given that flows are lower than the previously projected 77.81 L/s.

4.4 Wastewater Servicing Conclusion

The subject property will be serviced by local sanitary sewers, an on-site trunk sanitary sewer, and the off-site SNC sanitary sewer as defined in previous reports. This AES continues to confirm that the expansion of the drainage areas from the **2019 Novatech SNC Memo** to include the entirety of the subject property has no negative impacts. There is residual capacity in the downstream SNC providing sufficient capacity for the peak sanitary flows for the subject property, including external commercial and community park flows.

5.0 STORMWATER CONVEYANCE

5.1 Existing Stormwater Drainage

The subject property is within the Jock River watershed. Per the existing topography characterized in available City of Ottawa base mapping, as well as site specific survey, all flows from the subject property are ultimately conveyed to the Jock River by a series of watercourses, sheet flow and minor ditches. The Foster Ditch, Borrisokane Road roadside ditches, are the main stormwater conveyances within the Conservancy East Phase 5 property that convey stormwater to the Jock River.

5.2 Proposed Stormwater Management Strategy

As documented in the previous AES, various stormwater strategies were discussed within the Master Infrastructure Review (MIR) prepared in parallel with the AES. Alternatives reviewed were:

Alternative 1 – Oil and Grit Separators & Treatment Train to Naturalized Wetlands*

Alternative 2 – Stormwater Management Wetland Facilities in the Floodplain

Alternative 3 – Stormwater Management Wetland Facilities out of the Floodplain

Alternative 4 – Modified Etobicoke filtration System (MEFS)

For the purposes of this AES update for Phase 5 Alternative 1 continues to be advanced as per the evaluation provided in the MIR and per discussions with the City of Ottawa on July 20, 2021. This alternative:

- A storm sewer system designed to capture at least the minimum design capture events in accordance with the amendment to the storm sewer and stormwater management elements of the Ottawa Design Guidelines – Sewer (Technical Bulletin PIEDTB-2016-01). The stormwater runoff will be treated before ultimately being released into the natural heritage features and the Jock River as per the ***Jock River Reach One Subwatershed Study*** prepared by Stantec in 2007 (***Jock River SWS***).
- All proposed units will be equipped with sump pumps due to local constraints;
- A treatment train approach to attain an Enhanced Level of Protection (80% total suspended solids (TSS) removal) per MECP guidelines consisting of:
 - Deep sump catchbasins;
 - The incorporation of infiltration-type LIDs within the right-of-way extending out from catchbasin locations (see ***Figure 5*** in the ***Figures & Drawings*** section). Future detailed grading will allow for the determination of suitable locations in order to yield optimal benefit from this LID. See Section 5.7 for additional LID discussion.

- Multiple oil and grit separators (OGS) units to provide TSS treatment with outlets that are above the 2-year event summer water levels on the Jock River;
- The storm systems will discharge the treated stormwater at multiple outlets located along the southern natural heritage corridor, connecting via channels. Discharge locations are demonstrated in the **Storm Tributary Area** plan in the **Figures & Drawings** section
- An on-site road network designed to maximize the available storage within right-of-ways for the 100-year design event, where possible; and
- An overland flow route designed to safely convey stormwater runoff flows in excess of the on-site road storage.

The design for the site proposes to have stormwater flows conveyed through the development area of the subject property via an underground sewer network. The stormwater runoff will be treated before ultimately being released into the Jock River as per the **Jock River Reach One Subwatershed Study** prepared by Stantec in 2007 (**Jock River SWS**).

The proposed stormwater design layout is shown on **Drawing 3** with the stormwater management design consisting of (similar to prior phases):

- A storm sewer system designed to capture at least the minimum design capture events in accordance with the amendment to the storm sewer and stormwater management elements of the Ottawa Design Guidelines – Sewer (Technical Bulletin PIEDTB-2016-01);
- All proposed units will be equipped with sump pumps due to local constraints;
- A treatment train approach to attain an Enhanced Level of Protection (80% total suspended solids (TSS) removal) per MECP guidelines consisting of:
 - Deep sump catchbasins to reduce catchbasin sump sediment re-suspension and optimize TSS removal;
 - Multiple oil and grit separators (OGS) units to provide TSS treatment with outlets that are above the 2-year event summer water levels of the Jock River;
 - The incorporation of infiltration-type LIDs within the right-of-way extending out from catchbasin locations (see **Figure 5** in the **Figures & Drawings** section). The future detailed grading will allow for the determination of preferred locations in order to yield optimal benefit from this LID. See Section 5.7 for additional LID discussion;

- The storm systems will discharge the treated stormwater at multiple outlets (2) located along the natural heritage corridor, connecting to the Foster Ditch via channels to support hydration of the wetlands and ultimately outletting to the Jock River. Discharge locations are demonstrated in **Drawing 3**;
- An on-site road network designed to maximize the available storage within right-of-ways for the 100-year design event, where possible, with controlled release of stormwater to the minor storm system; and
- An overland flow route designed to safely convey stormwater runoff flows in excess of the on-site road storage.

Although quantity control has not typically been required for this reach of the Jock River, as per the **Jock River SWS**, the quantity of stormwater runoff exiting from the subject property will be minimized by optimizing on-site storage in the sags of the proposed road network, which in turn minimizes the size of downstream storm sewer infrastructure. It is noted that the RVCA is currently reviewing the SWM requirements within the Jock River Reach 1 area. In consideration of this, J.F. Sabourin and Associates (JFSA) has undertaken a review of the existing quantity control recommendations and the existing, and proposed, development conditions for this area. The findings are presented in the JFSA memorandum *Review of Quantity Control Requirement for Jock River Reach 1 (March 2021)* provided in **Appendix D** which concludes that quantity controls will still not be required for this reach of the Jock River.

5.2.1 Post-Development Stormwater Management Targets

Stormwater management requirements for the proposed alternative Stormwater management scheme have been adopted from the **Jock River SWS**, **City Standards**, and the **MECP SWMP Manual**.

Given the general criteria mentioned above, the following specific standards are anticipated for stormwater management within the subject property:

- Enhanced quality treatment will be provided for stormwater runoff from the subject property, corresponding to a long-term average TSS removal efficiency of 80%, as defined by the MECP prescribed treatment levels;
- Downstream receiving watercourses will be assessed for responses to planned stormwater management outflows, and stabilization mitigation measures will be planned as required;
- Storm sewers on local roads are to be designed to provide at least a 2-year level of service without any ponding per the City's latest Technical Bulletin PIEDTB-2016-01;

- Storm sewers on collector roads are to be designed to provide at least a 5-year level of service without any ponding per the City's latest Technical Bulletin PIEDTB-2016-01;
- For less frequent storms (i.e. larger than 2-year or 5-year), the minor system sewer capture will be restricted with the use of inlet control devices to prevent excessive hydraulic surcharges;
- Under full flow conditions, the allowable velocity in storm sewers is to be no less than 0.80 m/s and no greater than 6.0 m/s;
- For the 100-year storm and for all roads, the maximum depth of water (static and/or dynamic) on streets, rear yards, public space and parking areas shall not exceed 0.35 m at the gutter;
- The major system shall be designed with sufficient capacity to allow the excess runoff of a 100-year storm to be conveyed within the public ROW, or adjacent to the ROW, provided the water level does not touch any part of the building envelope; must remain below all building openings during the stress test event (100-year + 20%); and must maintain 15 cm vertical clearance between spill elevation on the street and the ground elevation at the nearest building envelope;
- Flow across road intersections shall not be permitted for minor storms (generally 5-year or less);
- When catchbasins are installed in rear yards, safe overland flow routes are to be provided to allow the release of excess flows from such areas. A minimum of 30 cm of vertical clearance is required between the rear yard spill elevation and the ground elevation at the adjacent building envelope; and
- The product of the maximum flow depths on streets and maximum flow velocity must be less than 0.60 m²/s on all roads.

5.2.2 Quality Control

Per the **Jock River SWS**, Enhanced quality treatment will be provided for stormwater runoff from the subject property, corresponding to a long-term average TSS removal efficiency of 80%, as described by the MECP prescribed treatment levels. See Section 5.3 for quality control approach and discussion.

5.2.3 Quantity Control

As noted in the **Jock River SWS**, quantity control is not anticipated to be required for outlets to the Jock River, however, some quantity control may be provided by erosion storage, as erosion thresholds for any watercourses/outlets will be respected where required. As noted in Section 5.2, JFSA has reviewed the current/future development conditions contributing to this reach of the Jock River and concludes that quantity

control will still not be required. See “Review of Quantity Control Requirement for Jock River Reach 1 (JFSA March 2021) provided in **Appendix D**.

5.3 Stormwater Management Design

5.3.1 Treatment Train Approach

JFSA previously (June 2021 memo) prepared a review of various potential stormwater quality treatment options that were investigated for the development. These included options, and combinations of options, as summarized in the following updated table. Each of the options has an expected total suspended sediment (TSS) removal capability, varying from 5% to 88%. This review assessed how the required Enhanced Level of Protection (80% TSS removal) could be achieved when the options are used in a treatment train approach, consistent with the expected requirements of the upcoming MECP Consolidated Linear Infrastructure policy.

<u>Selection and comparison of alternatives</u>									
Method	TSS Removal	Alt. 1	Alt. 2	Alt. 3	Alt. 4	Alt. 5	Alt. 6	Alt. 7	Alt. 8
Street Sweeping (Monthly)	5%								
Street Sweeping (Weekly)	10%								
Street Sweeping (Weekly with Elgin Eagle)*	88%	x							
Curb Cut with Grass Swales	75%			X					
Curb Cut with Infiltration Trenches	80%								
Catchbasin Inserts (CB Shield)*	27%		X			X		X	
Deep Sump Catch Basin	25%				X	X	X		X
Infiltration/ Filtration Trenches**	80%				X	X	X		
Infiltration at CBs, per MOE Table 3.2 (22.5m ³ /ha)	70%								X
OGS*	50%			X					X
JellyFish*	85%							X	
SWM Pond (Wet Pond)	80%		X						
Overall Performance		88.0%	85.4%	87.5%	89.1%	85.4%	85.0%	89.1%	88.8%
Treatment Train Overall Performance = 1 - (1- TSS Removal Rate Method 1) x (1- TSS Removal Rate Method 2) x (1- TSS Removal Rate Method 3 x ...)									
*) TSS Removal as documented by ETV Canada									
**) includes the use of Etobicoke infiltration or filtration systems or other permutations of the same									

The above table provides a summary of the TSS removal for the various methods that were considered. An option of infiltration LID measures located at catchbasin locations has been added as a method, and to Alternative 8 (see further discussion regarding this method below). The options, and combinations of options, have been assessed and shown to meet or exceed the required 80% TSS target.

For the development area, the updated Alternative 8 option demonstrates an estimated TSS removal of 88.8% for that particular treatment train approach which has been discussed with City staff for the approval of prior phases and will be the design being

advanced. For the determination of the TSS removal of 70% for the infiltration LID at catchbasins, the Table 3.2 of the MOECC (now MECP) publication entitled “*Stormwater Management Planning and Design Manual, March 2003*” sets the storage volume requirements for infiltration measures to achieve certain TSS removal rates.

Table 3.2 Water Quality Storage Requirements based on Receiving Waters^{1, 2}

Protection Level	SWMP Type	Storage Volume (m ³ /ha) for Impervious Level			
		35%	55%	70%	85%
<i>Enhanced</i> 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
<i>Normal</i> 70% long-term S.S. removal	Infiltration	20	20	25	30
	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
<i>Basic</i> 60% long-term S.S. removal	Infiltration	20	20	20	20
	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240



¹Table 3.2 does not include every available SWMP type. Any SWMP type that can be demonstrated to the approval agencies to meet the required long-term suspended solids removal for the selected protection levels under the conditions of the site is acceptable for water quality objectives. The sizing for these SWMP types is to be determined based on performance results that have been peer-reviewed. The designer and those who review the design should be fully aware of the assumptions and sampling methodologies used in formulating performance predictions and their implications for the design.

²Hybrid Wet Pond/Wetland systems have 50-60% of their permanent pool volume in deeper portions of the facility (e.g., forebay, wet pond).

The required storage volume of 22.5 m³/ha is determined for the development area prorated from the above table based on an overall imperviousness of ~62.5%. Similar to prior phases it is anticipated that the extent of the site area for Conservancy East Phase 5 can be managed with the proposed LID. For prior phases it is noted that approximately 140 lineal meters of LID per hectare of area to be treated was required. With approximately 9.0 ha of area to be treated (which excludes rear yards similar to the prior phase) this equates to 140x9.0= ~1,260 m extent of LID required. Phase 5 has approximately 1,800 m of roadway to incorporate the LID infrastructure therefore sufficient roadway is available for use.

5.3.2 Oil-Grit Separator Units (OGS)

As shown on **Drawing 3**, two (2) OGS units at locations along the southern boundary of the property, discharging to the Jock River via the existing Foster Ditch. By way of an MECP Certificate of Technology Assessment and manufacturer’s design report, the OGS units will demonstrate compliance with Enhanced Level of Protection requirements, with specific drainage area parameters for each area.

The manufacturer’s reported efficiency of TSS removal of the OGS units is expected to be based on a ‘fine distribution’ particle size distribution in conformance with the following table, unless otherwise approved by the City of Ottawa, RVCA, and MECP. The particle size distribution is the generic particle size distribution accepted by the City of Toronto per the *Wet Weather Flow Management Guidelines* (City of Toronto, 2006) as a typical average stormwater particle size distribution, and is an excerpt from Table 3.3 of the *Stormwater Management Practices Planning and Design Manual* (MOECC, 1994).

**Table 4: Typical Stormwater Particle Size Distribution & Settling Velocities
 (Source: *Stormwater Management Practices Planning and Design Manual*,
 MOECC, 1994)**

Particle Size (microns) (NURP 1983)	% of Particle Mass	Average Settling Velocities (m/s)
< 20	0 - 20	0.00000254
20 - 40	20 - 30	0.00001300
40 - 60	30 - 40	0.00002540
60 - 130	40 - 60	0.00012700
130 - 400	60 - 80	0.00059267
400 - 4000	80 - 100	0.00550333

To allow for flexibility as detailed design advances, it is proposed that any OGS unit can be selected, given that it:

- Meets the requirements set out in the preceding sections;
- Ensures no significant negative impact on the upstream storm sewer system – to be determined via hydraulic modelling at detailed design; and
- Demonstrates suitability for meeting Enhanced water quality targets via a MECP Certificate of Technology Assessment.

The preliminary OGS units proposed in the following table have been sized to treat the stormwater runoff for the tributary areas noted in order to meet MECP Enhanced Level of Protection criteria prior to discharge to the Jock River via naturalized wetlands as

shown on **Drawing 3**. The OGS total suspended removal rates and preliminary OGS unit details have been attached for reference in **Appendix D**.

Table 5: OGS Unit ID and Design Characteristics

Area and Unit ID ⁽¹⁾⁽²⁾	Drainage Area Target (ha)	Estimated Weighted C Value	Unit Treatment Capacity (L/s)	Unit Model ⁽¹⁾
Area 9 – OGS9 ⁽³⁾	7.21	0.72	212	CDS Model 4045-8
Area 10 – OGS10 ⁽³⁾	6.61	0.70	212	CDS Model 4045-8
(1) Providing at minimum 80% TSS removal for a Fine Distribution (2) See Drawing 3 for OGS unit locations (3) NOTE: the OGS numbering of OGS9 and OGS10 have been used to maintain consistency with prior functional servicing reports circulated in relation to this development area.				

The above OGS units will achieve required quality controls within the treatment train and, along with other elements, will have additional beneficial TSS mitigation.

5.3.3 Groundwater

Paterson Group has reviewed the anticipated long term groundwater condition for the development area. Paterson drawing PG5036-10A in **Appendix D** demonstrates the long term groundwater elevation across the Conservancy East Phase 5 development area. The lowest elevation of 88.70 is below the trunk sewer profiles shown in **Drawing No. 5** in the **Drawings** section and would be below any infiltration-type LID proposed within the development area.

5.4 Proposed Minor System

The subject property will be serviced by an internal gravity storm sewer system that will generally follow the local road network and proposed servicing easements as required. The drainage will be conveyed within the underground piped sewer system to headwall outlets located along the natural heritage corridor.

Street catchbasins will collect drainage from the streets and front yards, while rear yard catchbasins will capture drainage from backyards. Perforated catch basin leads will be provided in rear yards, except the last segment where they connect to the right-of-way which will be solid pipe, per City standards.

The rational method design of the minor system captures drainage for storm events up to and including the 2-year (local) and 5-year (collector) event within the subject property. The following table summarizes the standards employed in the detailed

design of the storm sewer network. The preliminary drainage area information can be found in **Drawing 3** and rational method design sheets are provided in **Appendix D**.

Table 6: Storm Sewer Design Criteria

Design Parameter	Value
Minor System Design Return Period	1:2 year (PIEDTB-2016-01) for local roads, without ponding 1:5 year (PIEDTB-2016-01) for collector roads, without ponding 1:100 year (PIEDTB-2016-01) for arterial road, without ponding
Major System Design Return Period	1:100 year
Intensity Duration Frequency Curve (IDF) 2-year storm event: A=732.951 B=6.199 C=0.810 5-year storm event: A = 998.071 B = 6.053 C = 0.814	$i = \frac{A}{(t_c + B)^C}$
Minimum Time of Concentration	10 minutes
Rational Method	$Q = CiA$
Storm sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} AR^{2/3} S^{1/2}$
Runoff coefficient for paved and roof areas	0.9
Runoff coefficient for landscaped areas	0.2
Minimum Sewer Size	250 mm diameter
Minimum Manning's 'n' for pipe flow	0.013
Minimum Depth of Cover	1.5 m from crown of sewer to grade
Minimum Full Flowing Velocity	0.8 m/s
Maximum Full Flowing Velocity	6.0 m/s
Clearance from 100-Year Hydraulic Grade Line to Building Opening	0.30 m
Design Parameter	Value
Max. Allowable Flow Depth on Municipal Roads	35 cm above gutter (PIEDTB-2016-01)
Extent of Major System	To be contained within the municipal ROW or adjacent to the ROW provided that the water level must not touch any part of the building envelope and must remain below the lowest building opening during the stress test event (100-year + 20%) and 15cm vertical clearance is maintained between spill elevation on the street and the ground elevation at the nearest building envelope (PIEDTB-2016-01)
Stormwater Management Model	DDSWMM (release 2.1), SWMHYMO (v. 5.02) and XPSWMM (v. 10)

Model Parameters	Fo = 76.2 mm/hr, Fc = 13.2 mm/hr, DCAY = 4.14/hr, D.Stor.Imp. = 1.57 mm, D.Stor.Per. = 4.67 mm
Imperviousness	Based on runoff coefficient (C) where Percent Imperviousness = (C - 0.2) / 0.7 x 100%.
Design Storms	Chicago 3-hour Design Storms and 24-hour SCS Type II Design Storms. Maximum intensity averaged over 10 minutes.
Historical Events	July 1st, 1979, August 4th, 1988 and August 8th, 1996
Climate Change Street Test	20% increase in the 100-year, 3-hour Chicago storm
<i>Extracted from City of Ottawa Sewer Design Guidelines, October 2012, and ISSU, and based on recent residential subdivisions in City of Ottawa.</i>	

The peak design flows are calculated based on an average predicted runoff coefficient (C-value) of 0.67 and 0.80 for the development areas, 0.40 for park areas and 0.25 for grassed areas. As detailed design progresses, the runoff coefficients will be refined to reflect the proposed building envelopes, driveways and other details.

There are several trunk sewers proposed and the peak flows are described for the trunk sewers which correspond to the stormwater management design areas as summarized in the following table:

Table 7: Minor System Trunk Sewer Outlets

Area/Outlet # (from east to west)	Trunk Sewer Outlet Headwall	Peak Flow (L/s)
9 (HW9) ⁽¹⁾	1050 mm diameter @ 0.20%	894
10(HW10) ⁽¹⁾	1050 mm diameter @ 0.11%	739
(1) NOTE: the OGS numbering of OGS9 and OGS10 have been used to maintain consistency with prior functional servicing reports circulated in relation to this development area.		

The storm sewers tributary to the various outlets, and associated peak flows, are detailed in the rational method design sheet, enclosed in **Appendix D**.

The conceptual servicing layout is shown on **Drawing 2** in **Drawings**. As detailed design progresses, alignment and sizing of local storm sewers will be confirmed and additional servicing easements may be required, potentially triggering minor amendments to the proposed lot fabric in the concept plan. The preliminary sanitary and storm trunk plan and profiles are shown on **Drawing 5** in **Drawings**.

5.4.1 Hydraulic Grade Line Analysis

A preliminary hydraulic grade line (HGL) modelling analysis has been completed by JFSA to demonstrate that the HGL will be maintained below the ground surface. See the JFSA memo entitled *BCDC Phase 5 – Preliminary HGL Analysis (December 1, 2022)* in **Appendix D** for details/results. The analysis has been evaluated for various scenarios for the Jock River (as per prior City requirements) in order to assess the appropriate HGL boundary condition:

- 100-year rainfall event on the development and a 5-year spring water level on the Jock River; or
- 5-year rainfall event on the development and a 100-year spring water level on the Jock River (deemed to be the critical event).

The HGL results in JFSA's Table 1 demonstrate that the worst case scenario freeboard to the ground surface ranges from 0.64m to 0.90m for the critical event noted above. The HGL is shown on the profile **Drawing 5** for reference.

An updated HGL analysis will be completed for the proposed system at the detailed design stage, based on the above noted events, including historical design storms and climate change stress test as required. Detailed grading design and storm sewer design will be modified as required to achieve the freeboard requirements (per PIEDTB-2016-01).

5.5 Proposed Major System

Major system conveyance, or overland flow, will be provided to accommodate flows in excess of the minor system capacity. Overland flow is accommodated by generally storing stormwater up to the 100-year design event in road sags then routing additional surface flow along the road network and service easements towards the proposed stormwater outlets, discharging to the Jock River through the natural heritage corridors, as shown on **Drawing 1**. The grading design includes a saw-toothed-road design with 0.10% minimum grade from high point to high point in order to maximize available surface storage for management of flows up to the 100-year design event where possible.

5.6 Foundation Drainage (Sump Pumps)

Due to the grade raise restrictions and the proposed storm and sanitary drainage schemes, the road centerlines do not allow for standard basements with a gravity connection to the storm sewer system. As such, because of the constraints on the subject property, sump pumps are proposed to be installed for all residential blocks and residential lots.

The City of Ottawa issued Technical Bulletin *ISTB-2018-04* and *2019-02* for the amendment of the *Ottawa Design Guidelines – Sewer, Second Edition*, October 2012 with respect to the screening criteria for the use of sump pump systems for foundation drainage in Greenfield developments on sites with clay soils. Similar to the development of Conservancy Phase 1, and Conservancy East (Phase 2-4) this site has also been assessed as meeting the required criteria for the use of sump pumps.

One of the screening criterion is with respect to the hydraulic grade line (HGL) for the development wherein the system should be reviewed to demonstrate that the HGL cannot reasonably be lowered any further due to outlet restrictions. The site grading is constrained by the close proximity of the Jock River, which is the receiver of stormwater outflows, and is also constrained by grade raise restrictions for the property.

For the Barrhaven Conservancy East Phase 5 Lands the grade raise restriction varies between 1.4 m and 1.8 m. Paterson's permissible grade raise plan is contained in **Appendix E** for reference (See Section 6 for discussion). Further investigations on the property and potential surcharging or lightweight fill (LWF) underneath garages could increase the permissible grade raise and will be investigated further as part of the detailed design.

The functional grading plan for the subdivision has been prepared with the grade raise restrictions in mind with grades being kept as low as possible.

The proposed centerline of road grades, and subsequently the house grades, do not allow for standard basements with a gravity connection to the storm sewer system. As such, the subdivision will be serviced entirely by sump pumps due to site constraints imposed by grade raise restrictions, HGL elevations and the proximity to the Jock River stormwater outlet.

5.7 Low Impact Development (LID) - Infiltration

The following general Low Impact Development (LID) techniques could be considered for implementation, where possible, as part of detailed design (noting that they have to be weighed against the objectives of the City's sump pump technical bulletins):

- Rear-yard swales should be designed with minimum grades where possible, to promote infiltration;
- Rear-yard catchbasin leads should be perforated (except for the last segment connecting to the storm sewer within the ROW), to promote infiltration; and,
- Where eavestroughs are provided on residential units, they are to be directed to landscaped surfaces, to promote infiltration.
- Furthermore, the following techniques can be examined as part of detailed landscaping design of the park block; and,
- Micro-grading can be considered to promote infiltration.

Generally, the LID techniques proposed above are most suitable due to the existing clay soils and high groundwater levels. The long term groundwater anticipated is demonstrated on Paterson Drawing *PG5036-10A* in **Appendix D** as previously noted. The proposed LID infiltration measure noted in Section 5.2 will contribute some infiltration benefits as first flush stormwater is conveyed into the filtration trench. The amount of infiltration is dependent upon the surrounding soils, but the proposed design will optimize the potential on the site.

5.8 Existing Watercourses

5.8.1 Foster Ditch

The Foster Ditch borders the western boundary of the Conservancy East Phase 5 development area. It originates south of Fallowfield Road, west of Cedarview Road and flows south until it converges with the Jock River South of McKenna Casey Drive. The ditch is approximately 3200 m long and has been artificially straightened. This non-municipal drain is a fish bearing tributary of the Jock River with approximately 335 ha of catchment area. The surrounding land use is urban and vacant lands. Riparian vegetation is very sparse consisting of mostly grasses with a few shrubs.

As noted in the **Jock River SWS**, to ensure protection of the aquatic habitat north of the Jock River, a development setback should be provided for all of the tributaries. Further studies will determine the development setback, which will be the greater of: 1) regulatory floodplain; 2) meander belt width; and 3) aquatic setback, whichever is greater.

5.9 Floodplain

On November 8th, 2019 the RCVA gave permission to Barrhaven Conservancy Development Corporation to cut and fill on the subject property under permit RV5 44/19 pursuant to review under Section 28 of the Conservation Authorities Act, regulation 174/06. The application and approval by the RVCA was supported by a 2D HEC-RAS model prepared by JFSA. The JFSA model identified the existing and proposed 100-year water levels and permissible extent of fill placement.

The works pursuant to the above-mentioned permit were completed and accepted by the RVCA on May 31st 2020. Options to complete the fill area boundary as set by JFSA included building a structural face of fill (retaining wall) to the limits of the 100-year floodplain boundary, or, building a berm with the toe of slope at the 100-year floodplain boundary. A vertical structural face of fill was not seen as a desirable or practical outcome and a berm was thus constructed. As-builts for the top of berm were subsequently provided and approved by the RVCA, resulting in the May 31st approval noted above and the current 100-year floodplain boundary delineation. The toe of the berm as constructed corresponds to the approved JFSA 100-year floodplain line and the current top of berm corresponds to the as-built top of berm.

5.10 Stormwater Servicing Conclusions

The Phase 5 stormwater runoff is designed to be captured by an internal gravity sewer system that will convey flows to multiple outlet locations equipped with end of line OGS units (two). A proposed treatment train arrangement of 1.0 m deep sump catchbasins, to optimize catchbasin sump retention of solids, as well as select catchbasin locations with connected infiltration-type subdrains will provide the required quality control treatment to achieve the Enhanced Level of protection. Downstream of the storm outlets along the southern development boundary will be channels within the natural heritage corridor where flows will be conveyed to the Foster Ditch prior to discharge to the Jock River. It is anticipated that quantity control is not required for the Jock River. Notwithstanding, some quantity control by means of erosion storage will be included.

A preliminary Hydraulic Grade Line (HGL) modelling analysis has been completed for the Conservancy East (Phase 5) development area at this time and demonstrates that the HGL is maintained below the ground surface with freeboards ranging from 0.64 m to 0.90 m. Further detailed HGL review will be completed for the proposed system at the detailed design stage. Due to the grade raise restrictions, and the proposed storm and sanitary drainage layouts, the road centerlines do not allow for standard basements with a gravity connection to the storm sewer system. As such, because of the constraints on the subject property, sump pumps are proposed to be installed for all residential blocks and residential lots.

The Conservancy East phase 5 development area will be outside of the Jock River's regulatory floodplain area.

Appropriate setbacks from existing watercourse are incorporated into the draft plan based on advancement/finalizing of studies to assess the various determining criteria.

6.0 GRADING

A site grading arrangement has been developed to optimize earthworks and provide major system conveyance to the receiving outlets, and naturalized wetland facilities, which ultimately outlet to the existing Jock River drainage network. The proposed grading can be found in **Drawing 1** in **Drawings**.

The development area is outside of the Jock River regulatory flood plain limits. The site grading will be a minimum of 0.50m above the 100-year regulatory limit event of the Jock River.

6.1 Geotechnical Conditions

Paterson completed a geotechnical investigation for the Conservancy East lands as follows:

- *Geotechnical Investigation – Proposed Residential Development, Conservancy Lands East* (Paterson Group, September 24, 2019);

The existing ground surface across the site is relatively level with approximate ground surface elevation varying between 91.5 m and 91.0 m. The subsurface profile generally consists of an approximate 50 mm to 460 mm thick layer of topsoil underlain by a silty clay deposit.

Due to the presence of a silty clay deposit, permissible grade raise restrictions are recommended for this site. The recommended permissible grade raise varies between 1.4 m in the north area of the phase and 1.8 m in the south. Figure PG5036-2 '*Permissible Grade Raise Plan*' by Paterson is enclosed in **Appendix E** for reference. At the time of detailed design, efforts will be made to mitigate any exceedances and detailed review and signoff by a licensed Geotechnical Engineer will be required. Where grade raises exceed the permissible levels the Engineer will recommend appropriate measures to mitigate where required (i.e. light weight fill or pre-consolidation etc).

The following additional grading criteria and guidelines will be applied to detailed design, per **City of Ottawa Guidelines**:

- Driveway slopes will have a maximum slope of 6%;
- Grading in grassed/landscaped areas to range from 2% to 3:1, with terracing required for slopes larger than 7%;
- Swales are to be 0.15m deep with 3:1 side slopes unless otherwise indicated on the drawings; and,
- Perforated pipe will be required for drainage swales if they are less than 1.5% in slope.

The geotechnical analysis of the site, published under separate cover in support of the development applications, provides additional information about the suitability of the site for the proposed services and grading scheme. At the time of detailed design, detailed review and signoff by a licensed Geotechnical Engineer will be required.

7.0 EROSION AND SEDIMENT CONTROL

Soil erosion occurs naturally and is a function of soil type, climate and topography. The extent of erosion losses is exaggerated during construction where vegetation has been removed and the top layer of soil becomes agitated.

Prior to topsoil stripping, earthworks or underground construction, erosion and sediment controls are implemented and will be maintained throughout any construction phase.

The following specific recommendations to the Contractor will be included in contract documents.

- Limit extent of exposed soils at any given time.
- Re-vegetate exposed areas as soon as possible.
- Minimize the area to be cleared and grubbed.
- Protect exposed slopes with plastic or synthetic mulches.
- Install silt fence to prevent sediment from leaving the site and entering existing watercourses, and clean and maintain throughout construction.
- Install catchbasin inserts during construction to protect from silt entering the storm sewer system.
- Install mud mats in order to prevent mud tracking onto adjacent roadways.
- No refueling or cleaning of equipment near existing watercourses.
- No material stockpiles within 30m of existing watercourses, unless otherwise permitted by RVCA and City of Ottawa.
- Provide sediment traps and basins during dewatering.
- Plan construction at proper time to avoid flooding.
- The Contractor will, at every rainfall, complete inspections to ensure proper performance.
- Erosion and sediment controls will remain in place until the working areas have been stabilized and re-vegetated.

8.0 UTILITIES

Utility services extending to the site may require connections to multiple existing infrastructure points: consultation with Enbridge gas, Hydro Ottawa, Rogers, and Bell is required as part of the development process to confirm the servicing plan for the subject lands.

9.0 CONCLUSION AND RECOMMENDATIONS

This AES provides details on the planned on-site municipal services for the subject property and demonstrates that adequate municipal infrastructure capacity is expected to be available for the planned development area.

- The subject lands are have been reviewed by Stantec to confirm that servicing is feasible from the SUC pressure zone. Future watermain extensions from Nepean Town Centre development areas, being constructed as part of Phase 2-4 approvals, will facilitate servicing to the Conservancy East Phase 5 lands via watermain extensions along the future Chapman Mills Drive extension and through the Claridge “Burnett Lands” development area. Detailed modelling will confirm phasing of the extensions of trunk watermains and verify sizing of the local watermain network.
- The subject property will be serviced by local sanitary sewers, an on-site trunk sanitary sewer, and the off-site SNC sanitary sewer as defined in previous reports. This AES continues to confirm that the expansion of the drainage areas from the **2019 Novatech SNC Memo** to include the entirety of the subject property has no negative impacts. There is residual capacity in the downstream SNC providing sufficient capacity for the peak sanitary flows for the subject property, including external commercial and community park flows.
- Stormwater service is to be provided by capturing stormwater runoff by an internal gravity sewer system that will convey flows to various outlets along the southern boundary to the Foster Ditch which will convey flows to the Jock River. Prior to discharge from the development, any first flush stormwater will have passed through a treatment train of measures in order to provide the appropriate level of Enhanced quality control. The treatment train consists of deep sump catchbasins, LID infiltration trench at catchbasin locations and an end-of-line OGS unit. It is anticipated that quantity control will not be required for discharges to the Jock River.
- A preliminary Hydraulic Grade Line (HGL) modelling analysis has been completed at this time and demonstrates that the HGL is maintained below the ground surface. Another detailed HGL review will be completed for the proposed system at the detailed design level.

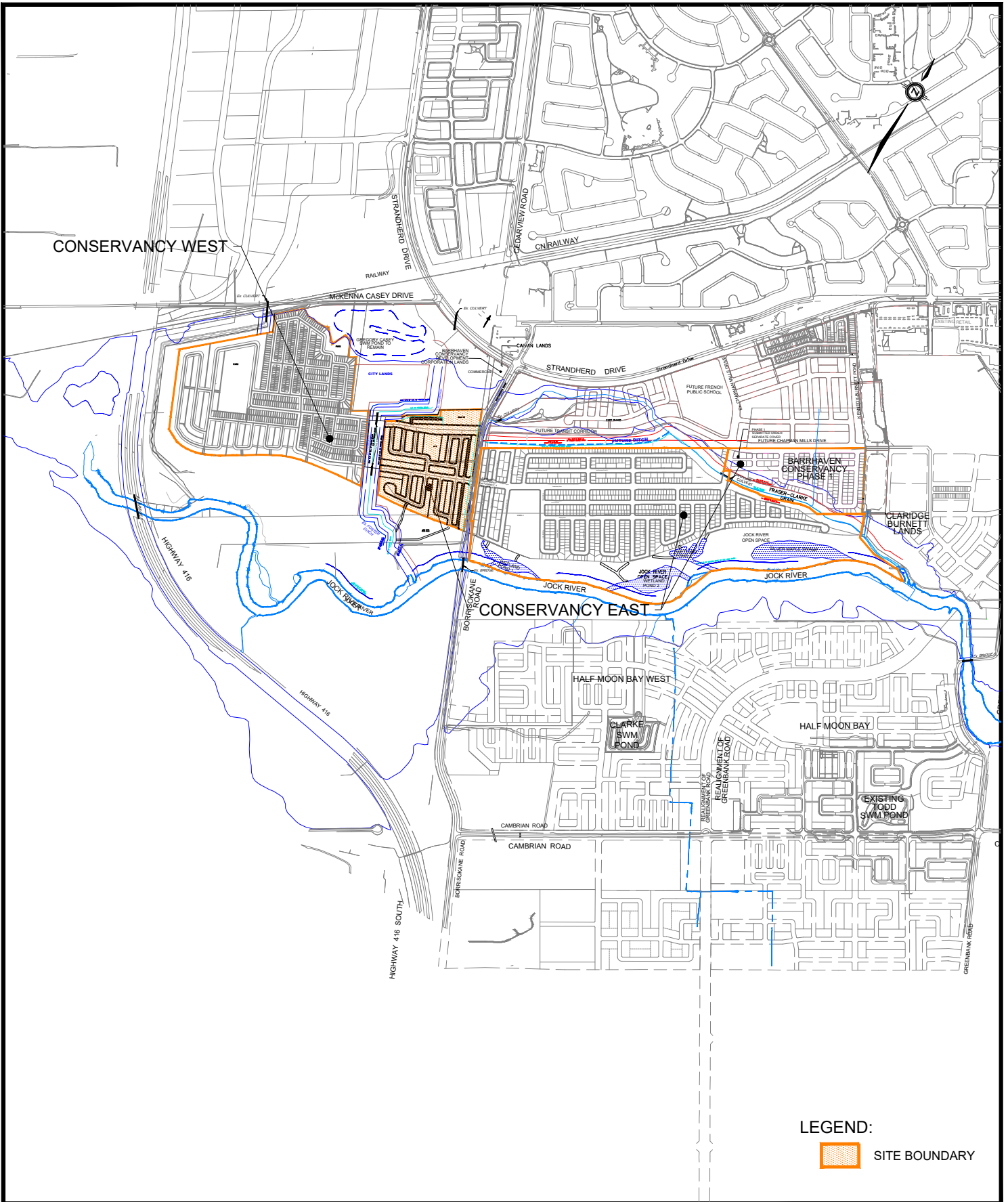
- Sump pumps are proposed to be installed for all units within residential blocks and lots;
- The proposed servicing and grading plans are anticipated to meet all City, RVCA, and MECP requirements as set out in background studies and current standards.
- Prior to detailed design of the infrastructure presented in this report, this AES will require approval under the Planning Act as supporting information for the Plan of Subdivision application. Project-specific approvals are also expected to be required for the infrastructure presented in this report from the City of Ottawa, MECP, and Rideau Valley Conservation Authority, among other agencies.

Prepared by,
David Schaeffer Engineering Ltd.



Per: Kevin L. Murphy, P.Eng.

FIGURES & DRAWINGS



LEGEND:
 SITE BOUNDARY



120 Iber Road, Unit 103
 Stittsville, ON K2S 1E9
 TEL: (613) 836-0856
 FAX: (613) 836-7183
 www.DSEL.ca

**BARRHAVEN CONSERVANCY
 EAST PHASE 5
 KEY PLAN
 CITY OF OTTAWA**

DATE: DECEMBER 2022
 SCALE: 1:20000
 PROJECT No.: 20-1180 A-5
 FIGURE: 1

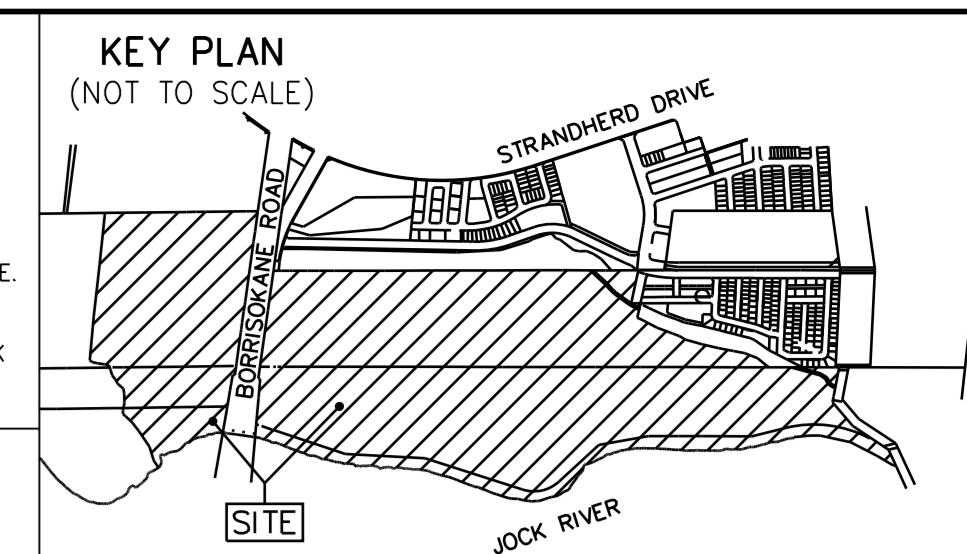
SCHEDULE OF LAND USE		
LAND USE	BLOCK(S)	AREA (Sq. m)
SINGLES (DETACHED)	1 TO 8, (BOTH INCLUSIVE), 20, 21, 24, 25, 41, 43, 45, 47, 48 AND 50 TO 59, (BOTH INCLUSIVE)	219,615.4
TOWNHOME	9 TO 19, (BOTH INCLUSIVE), 22, 23, 26, 27, 31, 32, 35, 36, 42, 44, 46, 49 AND 135	66,745.4
REAR LANE TOWNHOME	100 TO 103, (BOTH INCLUSIVE) AND 106 TO 109, (BOTH INCLUSIVE)	27,698.3
WALKWAYS	104, 105, 110 TO 116, (BOTH INCLUSIVE) AND 134	1,815.1
OPEN SPACE	118, 119 AND 121	4,021.3
BUS LANE	132	8,651.3
FUTURE DEVELOPMENT	120	9,218.2
PARK	117, 122, 123 AND 133	42,832.3
NATURAL HERITAGE SYSTEM	124, 125 AND 126	52,086.5
JOCK RIVER OPEN SPACE	127, 128 AND 129	237,125.6
STREET WIDENING	130 AND 131	5,989.5
STREETS	STREET '7' TO STREET '37', (BOTH INCLUSIVE)	176,962.4
LANES	LANE '1' TO LANE '3', (BOTH INCLUSIVE)	4,890.3
TOTAL:		857,651.6

SUBJECT TO THE CONDITIONS, IF ANY, SET FORTH IN OUR LETTER DATED _____, 2021. THIS DRAFT PLAN IS APPROVED BY THE CITY OF OTTAWA UNDER SECTION 51(17) OF THE PLANNING ACT THIS _____ DAY OF _____, 2021.

SEAN MOORE, MCR, RPP, MANAGER
DEVELOPMENT REVIEW SECTION
PLANNING, INFRASTRUCTURE AND ECONOMIC DEVELOPMENT DEPARTMENT,
CITY OF OTTAWA

ADDITIONAL INFORMATION AS REQUIRED UNDER SECTION 51(17) OF THE PLANNING ACT R.S.O. 2001

(X) - AS SHOWN ON THIS DRAFT AND KEY PLAN
(O) - AS SHOWN ON THIS DRAFT AND KEY PLAN
(S) - LAND TO BE USED IN ACCORDANCE WITH THE SCHEDULE OF LAND USE
(H) - FULL MUNICIPAL SERVICES
(U) - OFFSHORE MARINE DEPOSITS OF CLAY, SILT, CLAY AND SILT, BEDROCK
OTTAWA FORMATION, LIMESTONE

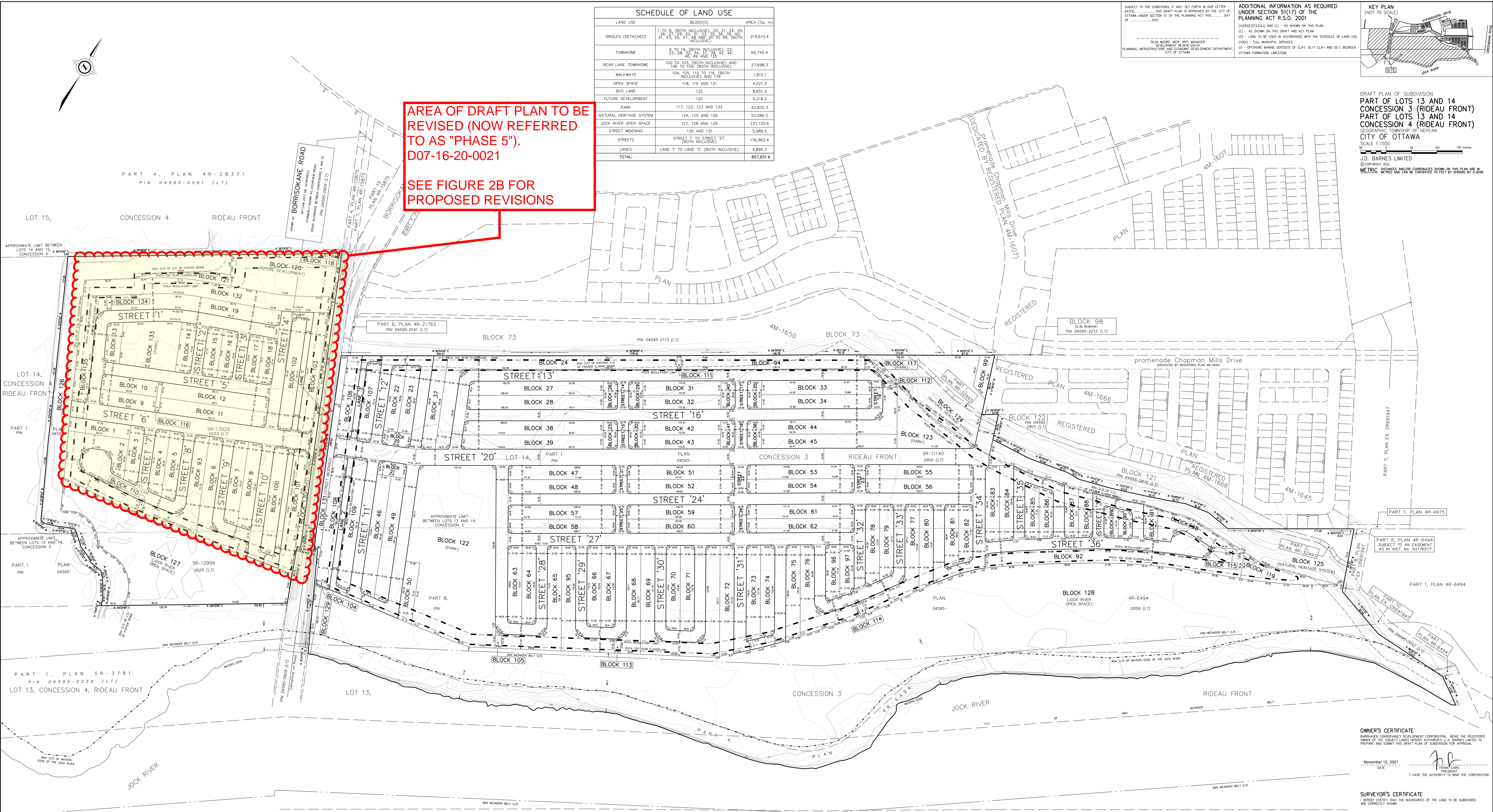


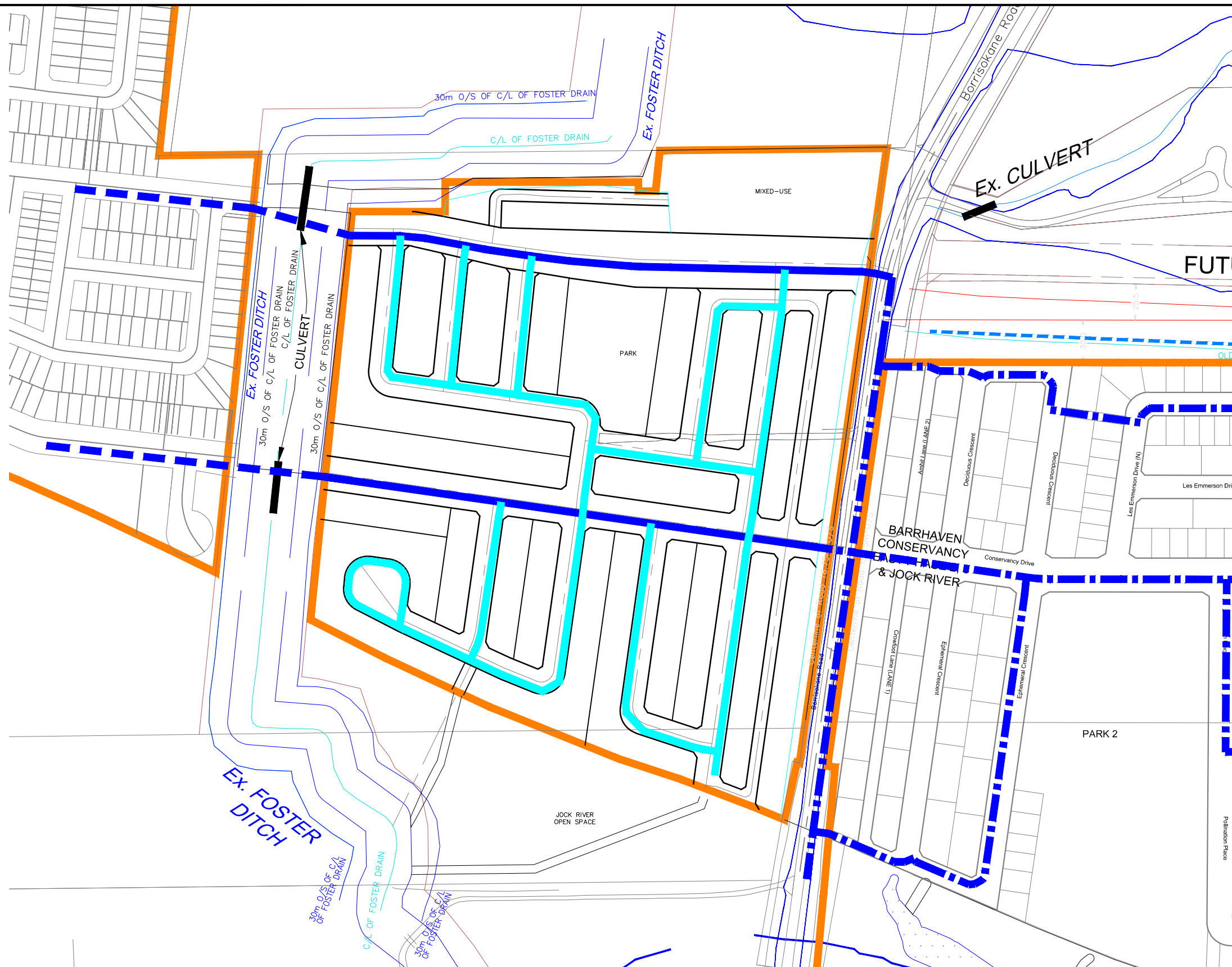
DRAFT PLAN OF SUBDIVISION
PART OF LOTS 13 AND 14
CONCESSION 3 (RIDEAU FRONT)
PART OF LOTS 13 AND 14
CONCESSION 4 (RIDEAU FRONT)
GEOGRAPHIC TOWNSHIP OF NEPEAN
CITY OF OTTAWA
SCALE 1:1500
J.D. BARNES LIMITED
© COPYRIGHT 2021
DISTANCES AND/OR COORDINATES SHOWN ON THIS PLAN ARE IN METRIC UNITS AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048






AREA OF DRAFT PLAN TO BE REVISED (NOW REFERRED TO AS "PHASE 5"). D07-16-20-0021

SEE FIGURE 2B FOR PROPOSED REVISIONS

FIGURE 2A
2021 APPROVED DRAFT PLAN





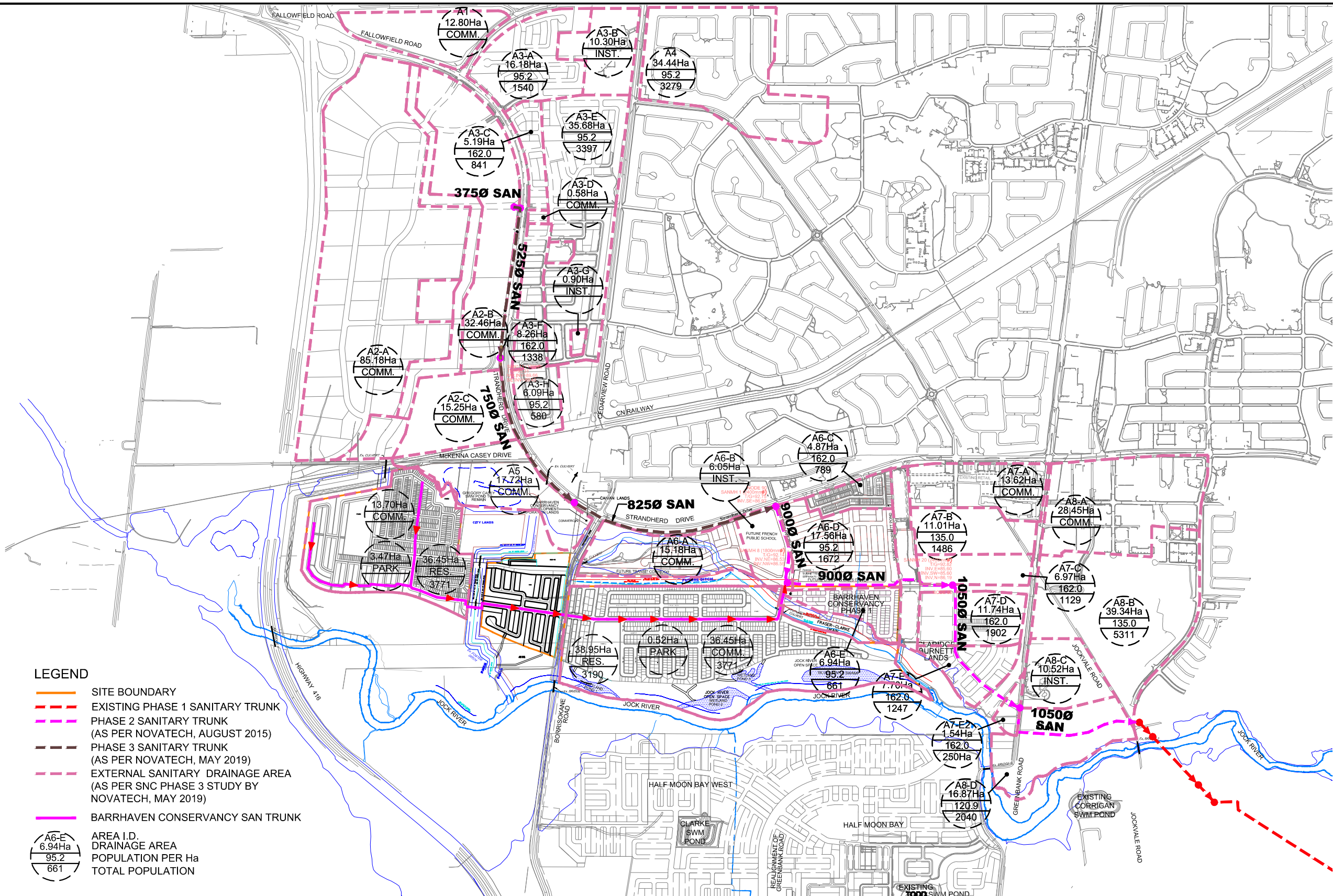
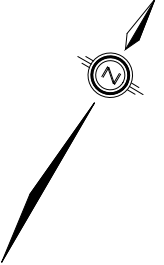
LEGEND	
	SITE BOUNDARY
	PROPOSED 300mm WATERMAIN
	PROPOSED LOCAL WATERMAIN
	EXTERNAL 300mm WATERMAIN
	FUTURE 300mm WATERMAIN



120 Iber Road, Unit 103
 Stittsville, ON K2S 1E9
 TEL: (613) 836-0856
 FAX: (613) 836-7183
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BARRHAVEN CONSERVANCY EAST PHASE 5
WATERMAIN SERVICING PLAN
 CITY OF OTTAWA

PROJECT No.:	20-1180 A-5
SCALE:	1:3000
DATE:	DECEMBER 2022
FIGURE:	3



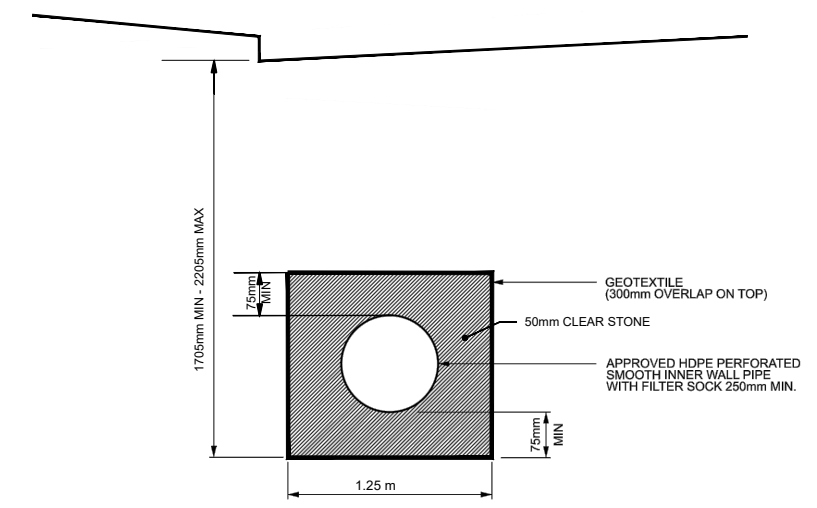
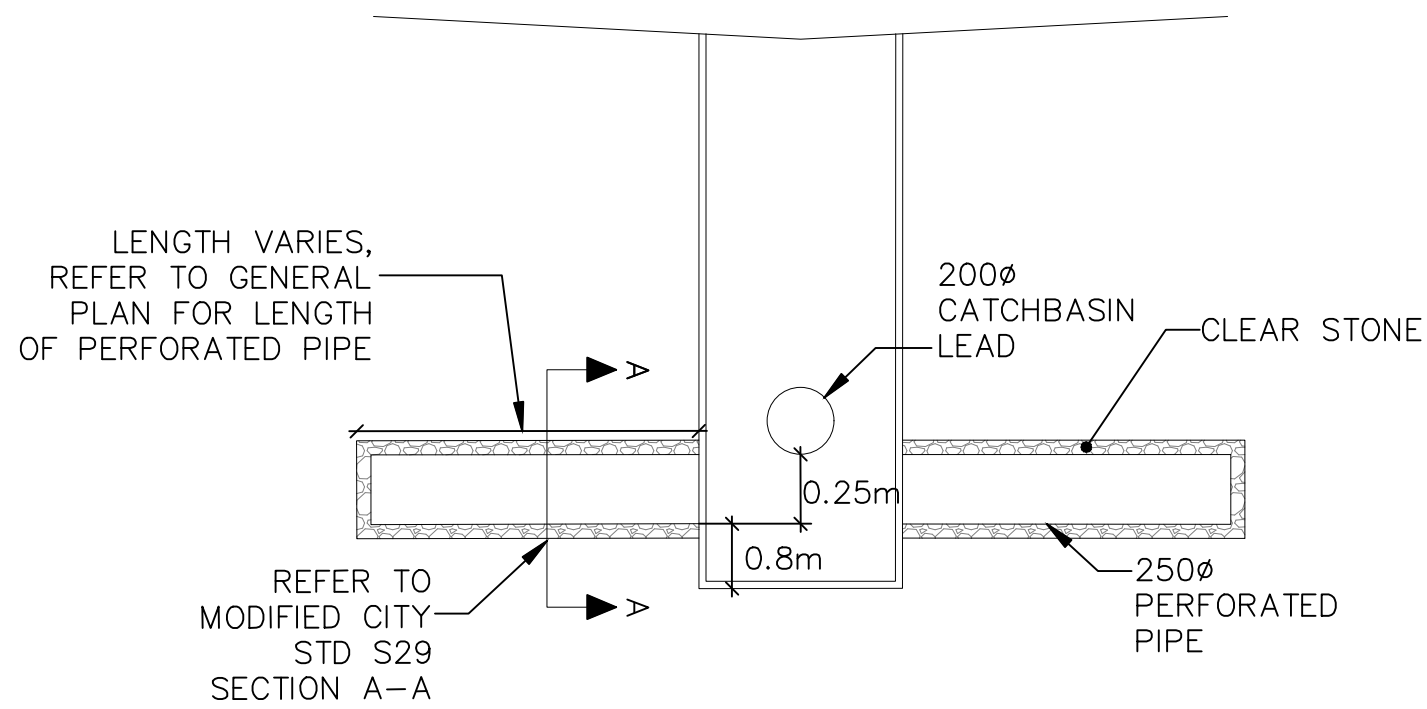
- LEGEND**
- SITE BOUNDARY
 - - - EXISTING PHASE 1 SANITARY TRUNK
 - - - PHASE 2 SANITARY TRUNK (AS PER NOVATECH, AUGUST 2015)
 - - - PHASE 3 SANITARY TRUNK (AS PER NOVATECH, MAY 2019)
 - - - EXTERNAL SANITARY DRAINAGE AREA (AS PER SNC PHASE 3 STUDY BY NOVATECH, MAY 2019)
 - BARRHAVEN CONSERVANCY SAN TRUNK
- A6-E
6.94Ha
95.2
661 AREA I.D.
DRAINAGE AREA
POPULATION PER Ha
TOTAL POPULATION



120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
TEL: (613) 836-0856
FAX: (613) 836-7183
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**BARRHAVEN CONSERVANCY EAST PHASE 5
EXTERNAL SANITARY SERVICING
CITY OF OTTAWA**

PROJECT No.:	20-1180 A-5
SCALE:	1:18000
DATE:	DECEMBER 2022
FIGURE:	4



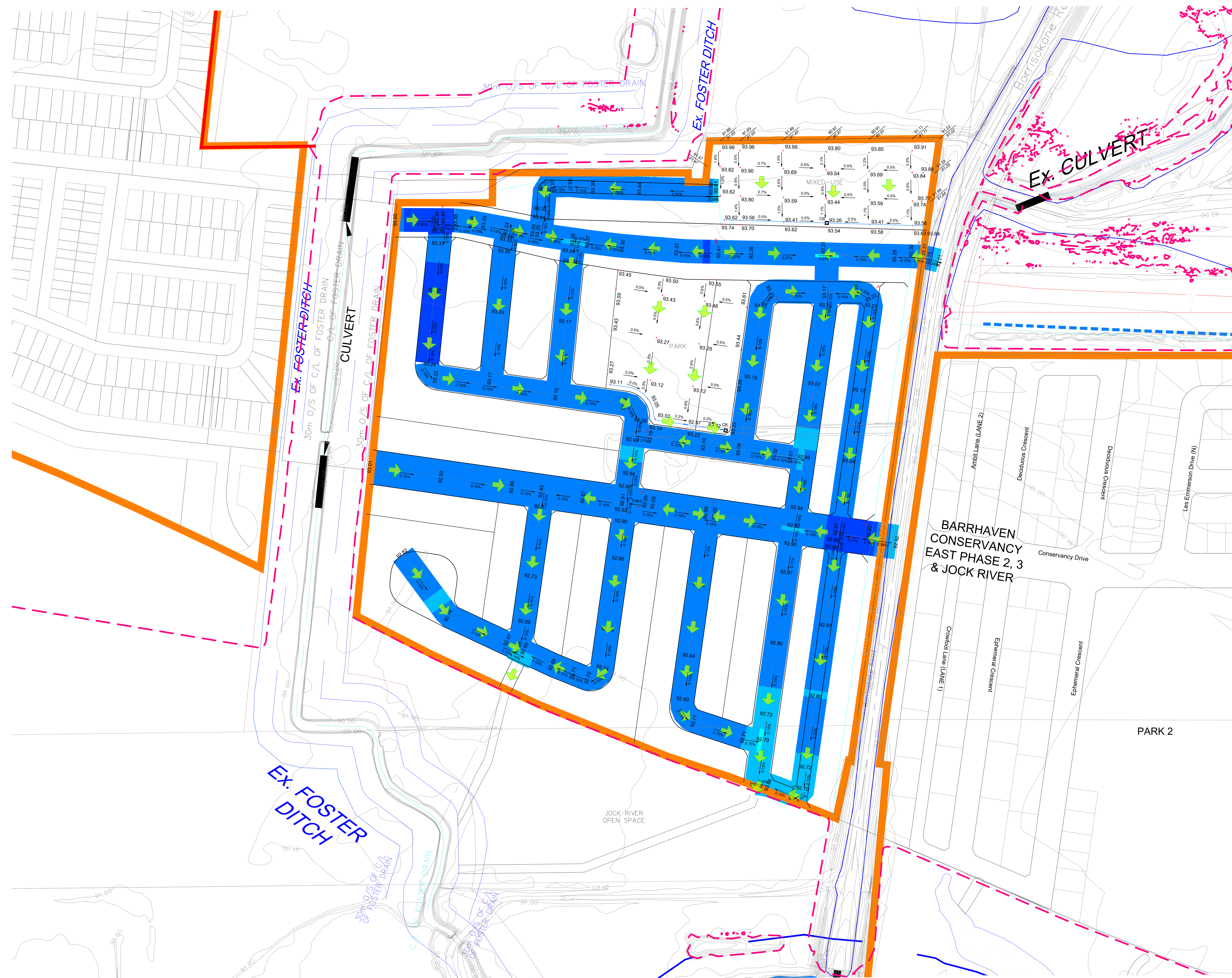
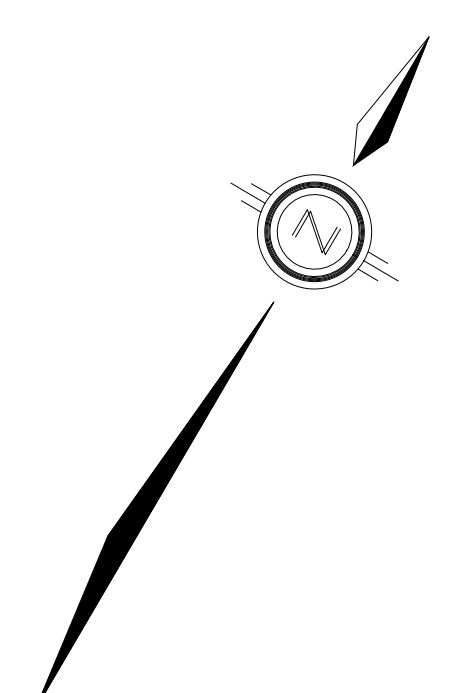
SECTION A-A: MODIFIED CITY STD S29
SCALE: N.T.S.



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FAX: (613) 836-7183
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BARRHAVEN CONSERVANCY EAST PHASE 5
FILTRATION SYSTEM DETAILS
CITY OF OTTAWA

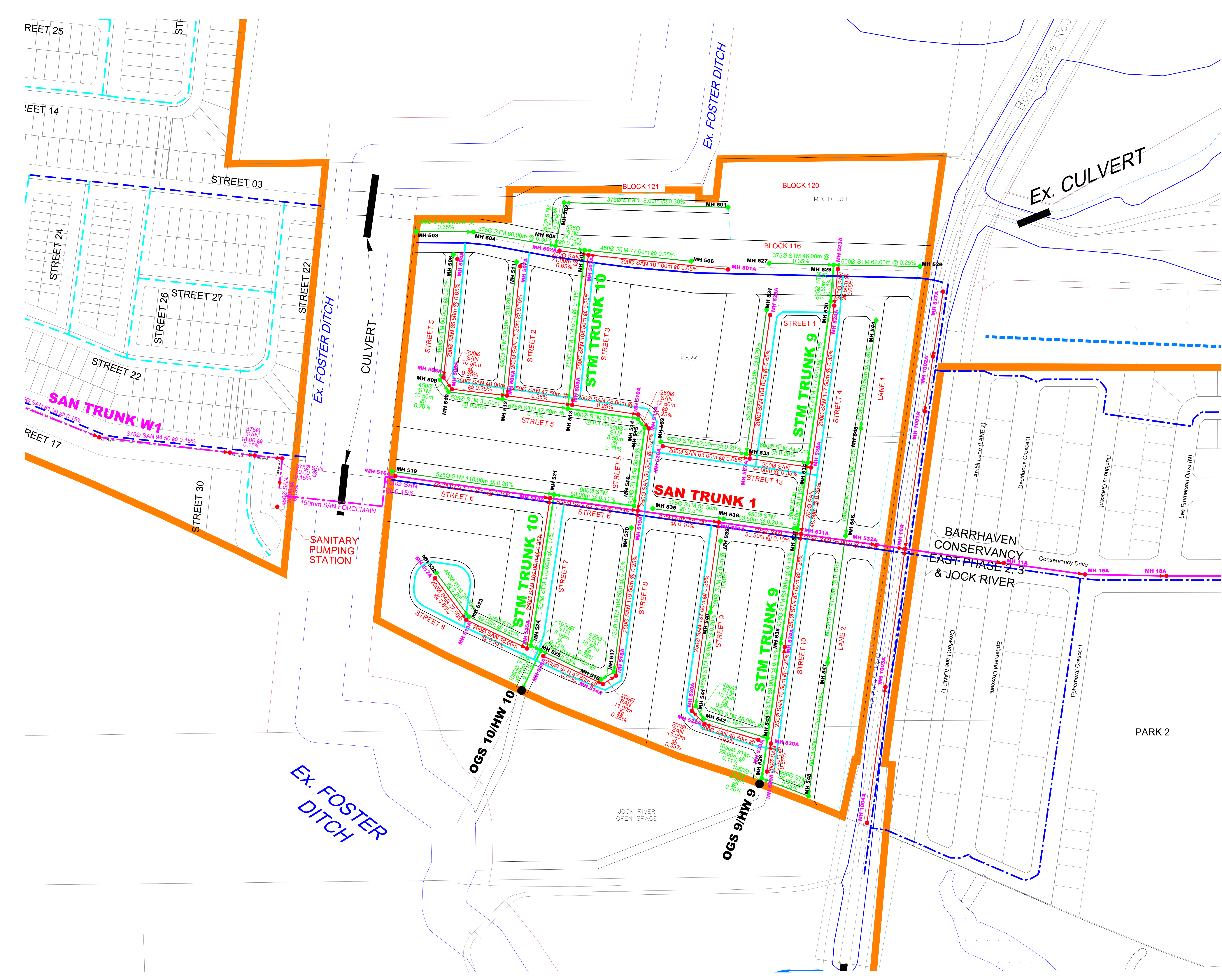
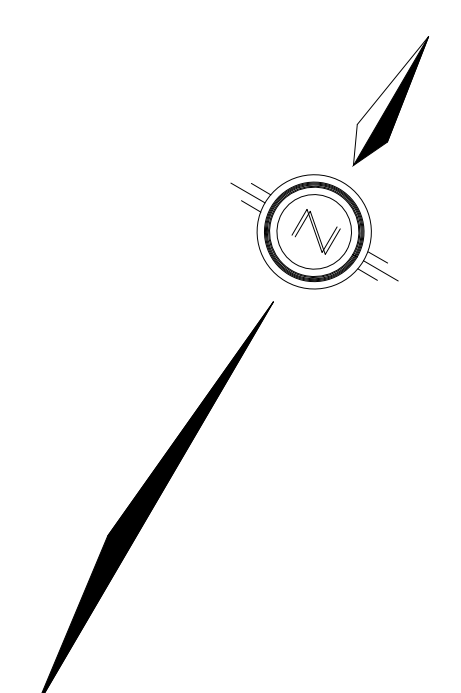
PROJECT No.:	20-1180 A-5
SCALE:	NTS
DATE:	DECEMBER 2022
FIGURE:	5



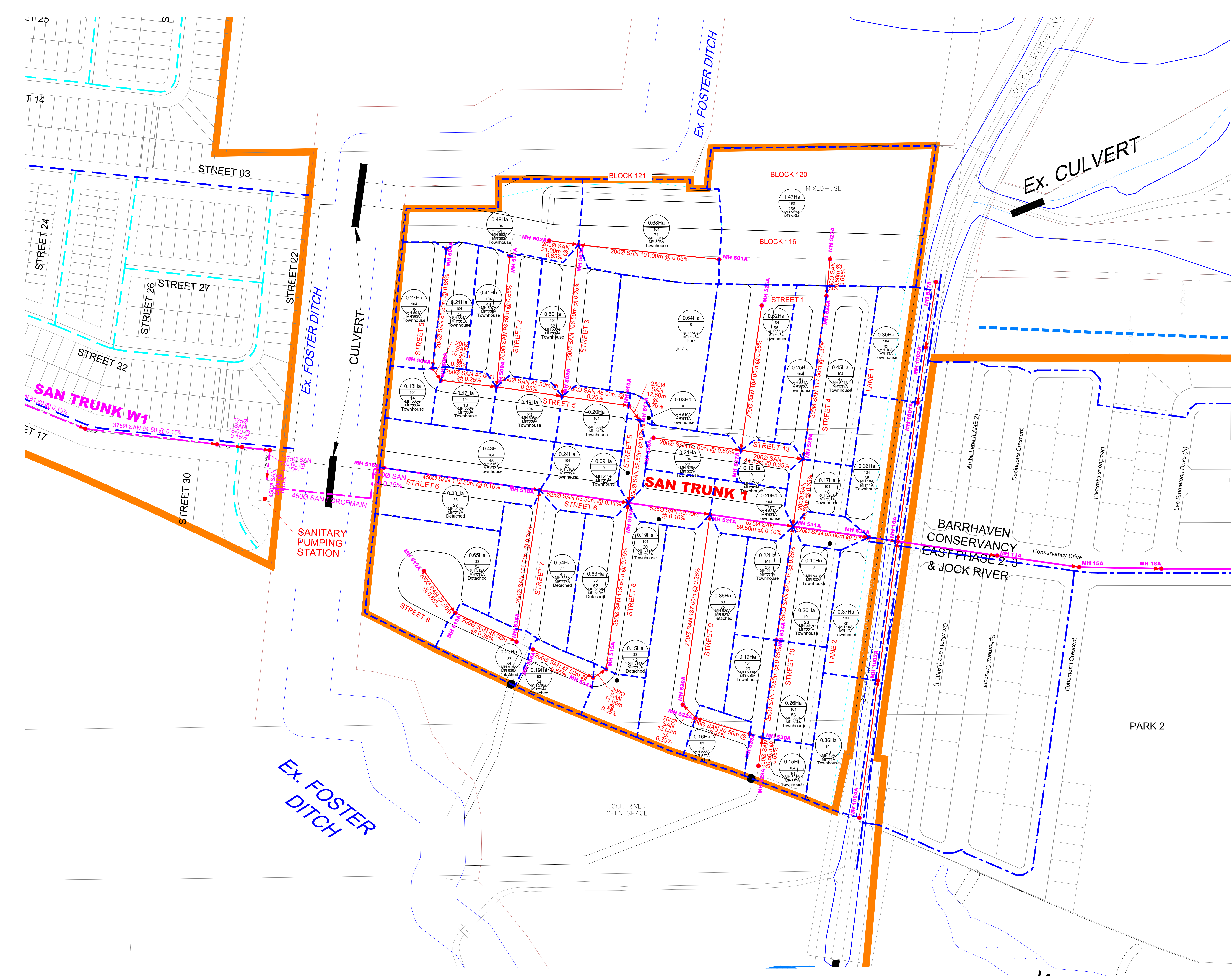
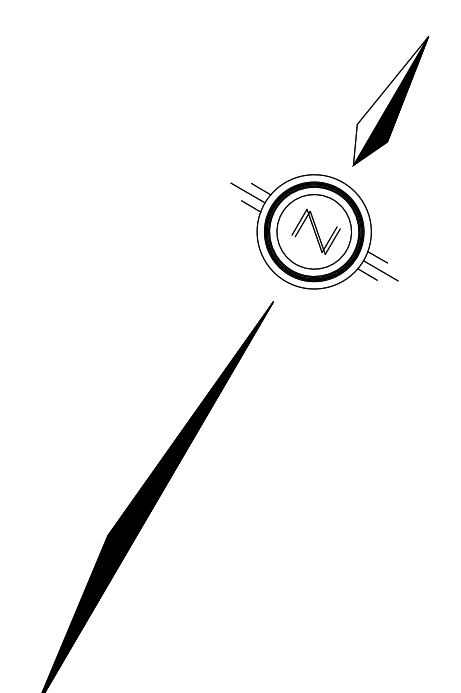
LEGEND

	SITE BOUNDARY		STORM OVERLAND FLOW ARROW
	PROPOSED CENTERLINE ELEVATION		EXISTING ELEVATION
	EXISTING CONTOUR ELEVATION		2019 RVCA APPROVED FLOODLINE BOUNDARY
	PROPOSED FLOODLINE PER MARCH 2021 RVCA APPLICATION		

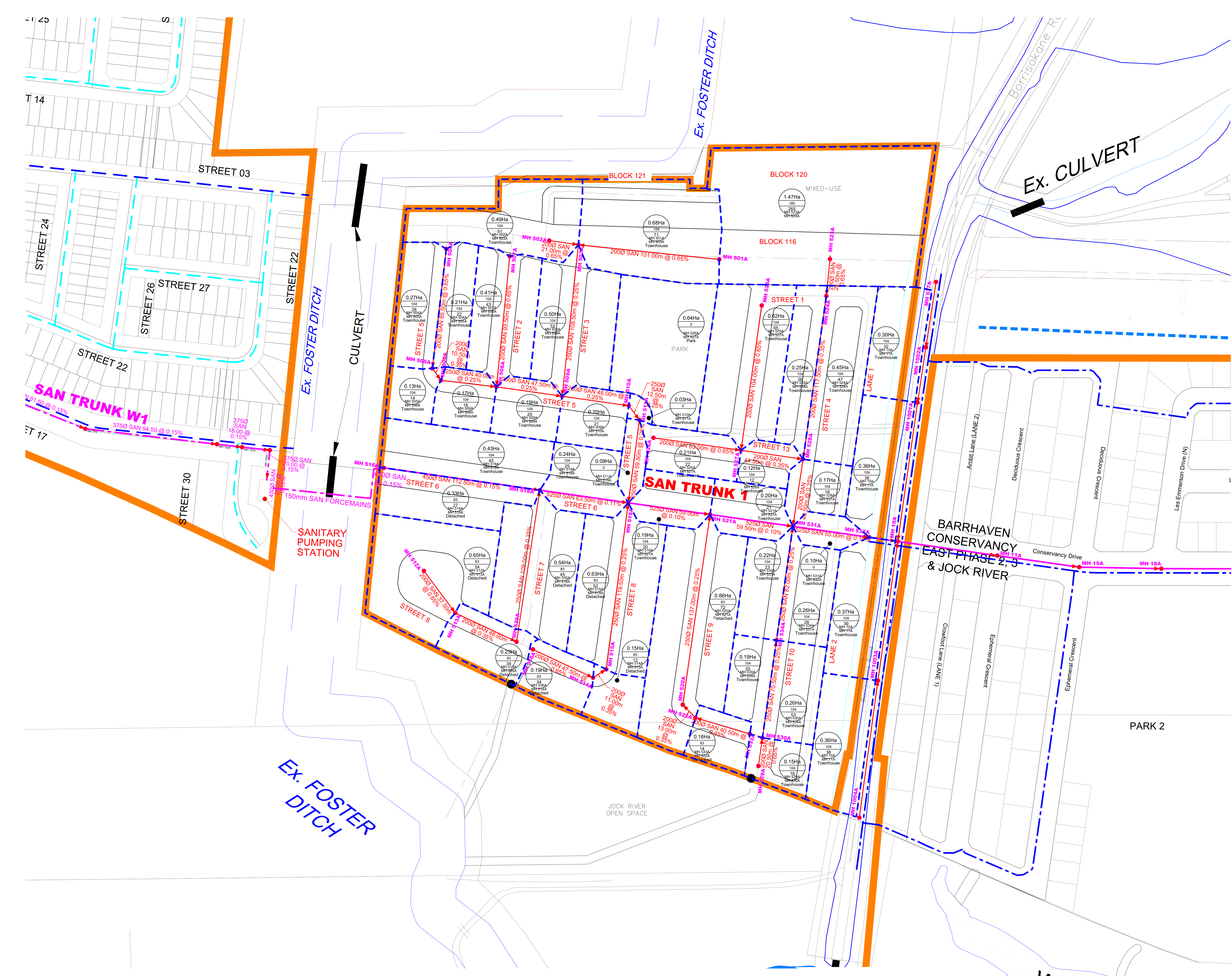
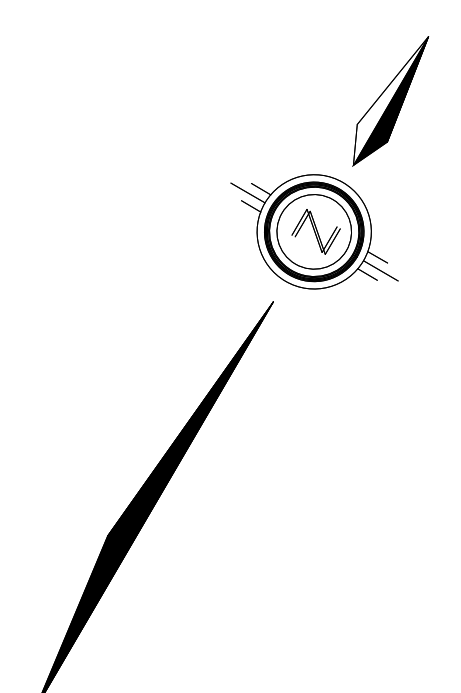
CUT-FILL DEPTH ALONG CENTER LINE:	
CUT DEPTH (m)	FILL DEPTH (m)
0.00~0.50:	0.00~0.50:
0.50~1.00:	0.50~1.00:
1.00~1.50:	1.00~1.50:
1.50~2.00:	1.50~2.00:
>2.00:	>2.00:



LEGEND	
	SITE BOUNDARY
	STORM TRUNK SEWER
	STORM LOCAL SEWER
	SANITARY TRUNK SEWER
	SANITARY LOCAL SEWER
	EXTERNAL SANITARY TRUNK SEWER
	PROPOSED 300mm WATERMAIN
	PROPOSED LOCAL WATERMAIN
	FUTURE 300mm WATERMAIN
	EXTERNAL 300mm WATERMAIN
	STORM MANHOLE
	SANITARY MANHOLE

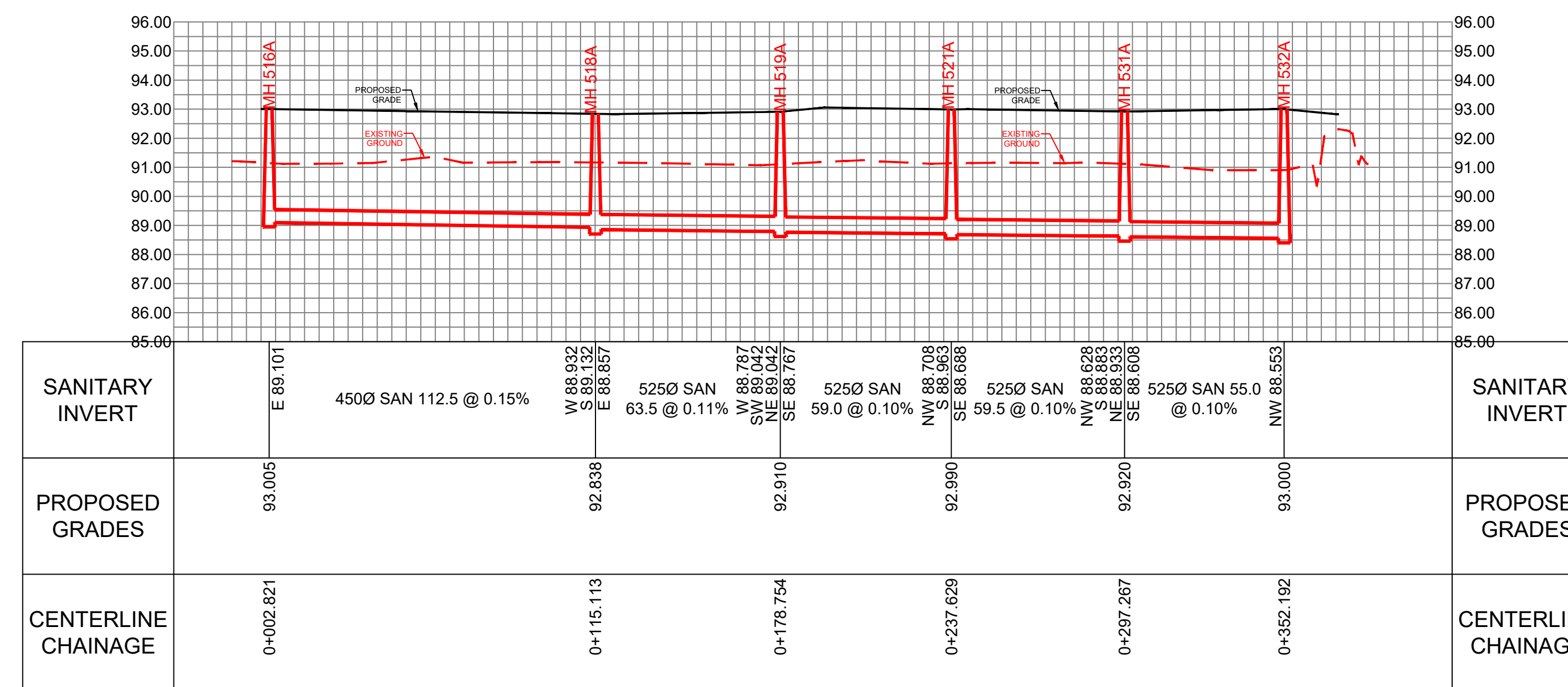


- LEGEND**
- SITE BOUNDARY
 - SANITARY TRUNK SEWER
 - SANITARY LOCAL SEWER
 - EXTERNAL SANITARY TRUNK SEWER
 - SANITARY MANHOLE
 - SANITARY TRIBUTARY AREA
 - 0.19Ha
104
Townhouse AREA IN HECTARES
POPULATION PER HECTARE
POPULATION
AREA TYPE

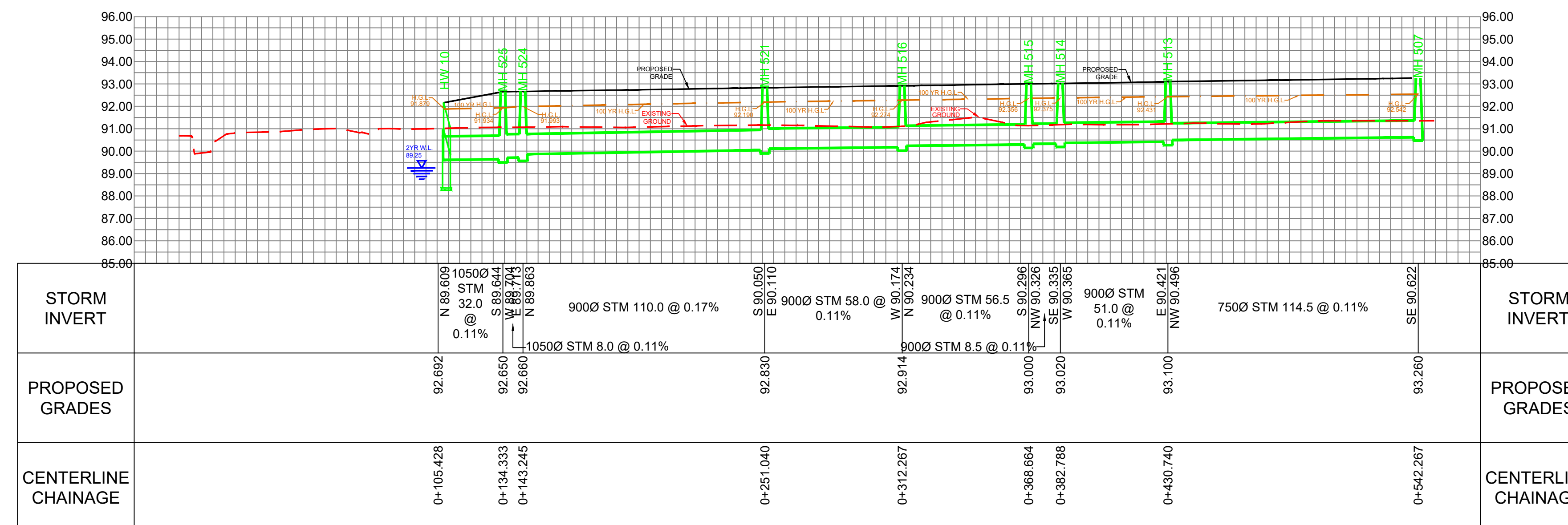


- LEGEND**
- SITE BOUNDARY
 - SANITARY TRUNK SEWER
 - SANITARY LOCAL SEWER
 - EXTERNAL SANITARY TRUNK SEWER
 - SANITARY MANHOLE
 - SANITARY TRIBUTARY AREA
 - AREA IN HECTARES
POPULATION PER HECTARE
POPULATION
 - AREA TYPE

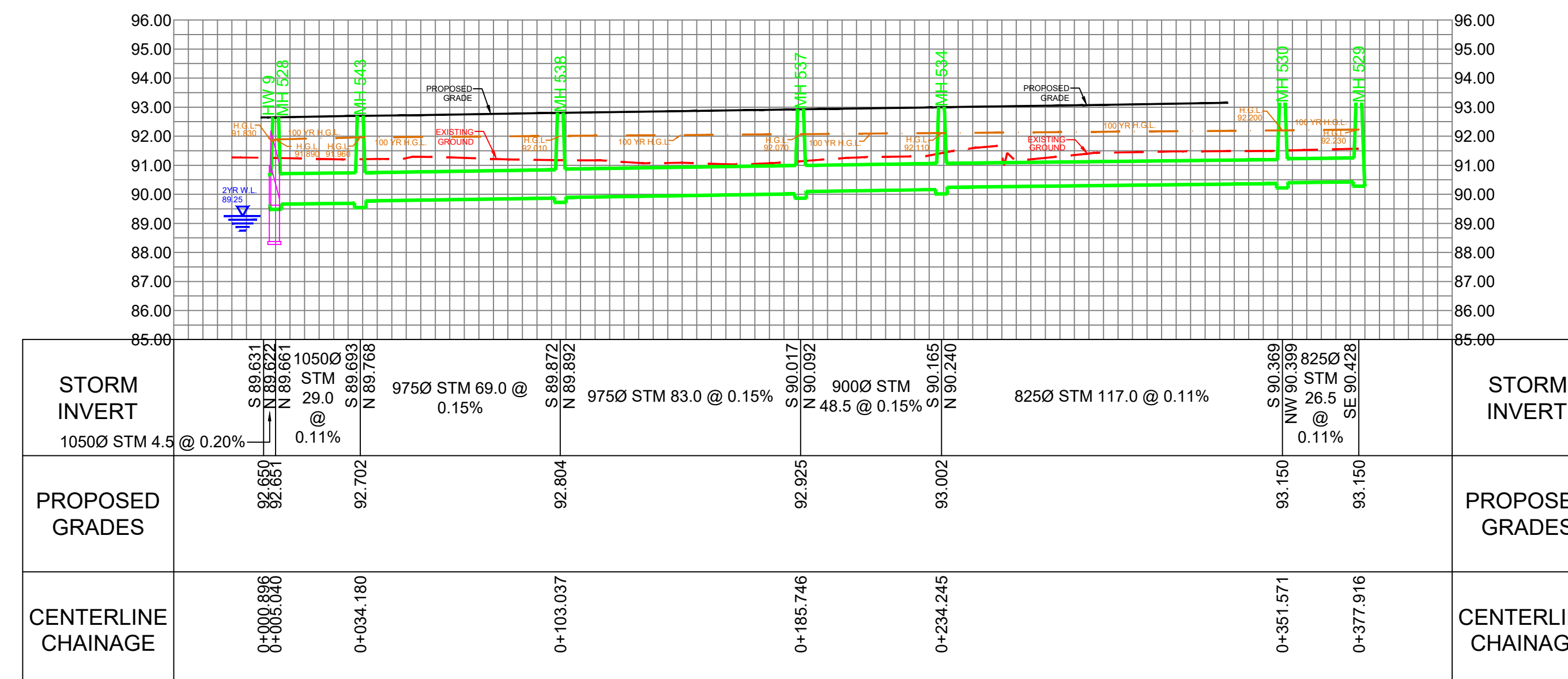
SAN TRUNK 1



STM TRUNK 10



STM TRUNK 9



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Fax. (613) 836-7183
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BARRHAVEN CONSERVANCY EAST PHASE 5
SANITARY AND STORM TRUNK PROFILES
CITY OF OTTAWA

PROJECT No. : 20-1180 A-5
SCALE: 1:1500
DATE: DECEMBER 2022
DRAWING No. 5

APPENDIX A

GENERAL

Content Copy Of Original



Ministry of the Environment and Climate Change
Ministère de l'Environnement et de l'Action en matière de changement
climatique

ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 8129-AB7LDF

Issue Date: June 23, 2016

City of Ottawa
100 Constellation Crescent West, 6th Floor
Ottawa, Ontario
K2G 6J8

Site Location: Jockvale Road and Strandherd Drive
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

sanitary sewers to be constructed in the City of Ottawa, on various vacant development lands (from Station 0+003 to Station 2+517), Greenbank Road (from Station 1+846 to Station 1+947), and Jockvale Road (from Station 2+430 to Station 2+517);

all in accordance with the application form from the City of Ottawa, dated June 22, 2016, including final plans and specifications prepared by Novatech Engineers, Planners and Landscape Architects.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

1. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The environmental compliance approval number;
6. The date of the environmental compliance approval;
7. The name of the Director, and;
8. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500

AND

The Director appointed for the
purposes of Part II.1 of the
Environmental Protection Act

Toronto, Ontario
M5G 1E5

Ministry of the Environment and
Climate Change
135 St. Clair Avenue West, 1st
Floor
Toronto, Ontario
M4V 1P5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca**

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 23rd day of June, 2016

Gregory Zimmer, P.Eng.
Director
appointed for the purposes of Part II.1 of
the *Environmental Protection Act*

AF/
c: District Manager, MOECC Ottawa
Water Supervisor, MOECC, Ottawa
M. Rick O'Connor, City Clerk, City of Ottawa
Luc Marineau, City of Ottawa
Jonathan Knoyle, City of Ottawa
Bob Dowdall, Novatech Engineers, Planners and Landscape Architects
Edson Donnelly, Novatech Engineers, Planners and Landscape Architects

ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 4357-CHMQEM
Issue Date: September 1, 2022

Barrhaven Conservancy Development Corporation
2934 Baseline Road, Suite 302
Ottawa, Ontario
K2H 1B2

Site Location: Barrhaven Conservancy East - Phase 2, 3 & Jock River
Part of Lots 13 & 14 (Rideau Front)
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

the establishment of wastewater infrastructure Works located in the City of Ottawa, consisting of the following:

- **sanitary sewers** on Les Emmerson Drive (N)(from Station 0+000.000 to Station 0+720.000), Les Emmerson Drive (S) (from Station 0+000.000 to Station 0+660.000), Conservancy Drive (from Station 0-010.000 to Station 0+973.545), Peninsula Road (from Station 0+010.058 to Station 0+703.797), Sapling Grove (from Station 0+000.000 to Station 0+528.245), Canoe Street (from Station 0+000.000 to Station 0+491.136), Deciduous Crescent (from Station 0+002.000 to Station 0+328.189), Ephemeral Crescent (from Station 0+000.000 to Station 0+492.987), Mineral Street (from Station 0+242.832 to Station 0+000.000), Pollination Place (from Station 0+002.985 to Station 0+433.904), Gallium Crescent (from Station 0+002.728 to Station 0+321.940), Syringa Court (from Station 0+000.000 to Station 0+332.328), Anemone Mews (from Station 0+242.833 to Station 0+059.755), Ainsworth Crescent (from Station 0+002.715 to Station 0+353.228), Ecology Lane (from Station 0+205.411 to Station 0+007.658), Meander Way (from Station 0+002.747 to Station 0+333.559), Elation Heights (from Station 0+000.000 to Station 0+380.000), Jollity Crescent (from Station 0+001.794 to Station 0+221.612), Euphoria Crescent (from Station 0+000.000 to Station 0+170.000), and on Borrisokane Road (from Station 0+168.736 to Station 0+507.126), all discharging to the existing South Nepean Collector sanitary sewer; and

- **storm sewers** on Les Emmerson Drive (N) (from Station 0+002.919 to Station 0+718.915), Les Emmerson Drive (S) (from Station 0-002.269 to Station 0+676.895), Conservancy Drive (from Station 0+020.468 to Station 0+961.195), Peninsula Road (from Station 0-001.986 to Station 0+705.797), Sapling Grove (from Station 0-010.000 to Station 0+526.245), Canoe Street (from Station 0+000.000 to Station 0+493.136), Deciduous Crescent (from Station 0+004.500 to Station 0+324.827), Ephemeral Crescent (from Station 0-002.063 to Station 0+495.738), Mineral Street (from Station 0+244.847 to Station 0+002.015), Pollination Place (from Station 0+000.000 to Station 0+424.262), Gallium Crescent (from Station 0+000.000 to Station 0+325.307), Syringa Court (from Station 0-001.985 to Station 0+334.348), Anemone Mews (from Station 0+244.843 to Station 0+001.982), Ainsworth Crescent (from Station 0+000.000 to Station 0+354.443), Ecology Lane (from Station 0+207.411 to Station 0+006.523), Meander Way (from Station 0+016.643 to Station 0+335.359), Elation Heights (from Station 0+000.000 to Station 0+381.539), Jollity Crescent (from Station 0+003.277 to Station 0+220.000), Euphoria Crescent (from Station 0+003.400 to Station 0+157.175), Lane 1 (Crowfoot Lane) (from Station 0-002.000 to Station 0+201.525), and on Lane 2 (Ambit Lane) (from Station 0+002.000 to Station 0+127.5060), proposed storm sewers collect flows from the subdivision and discharge to the Jock River and the Fraser-Clarke Watercourse which is an existing tributary to the Jock River;

the modification of a section of the Fraser-Clarke Watercourse to accommodate stormwater outflows from Phase 2 of the Barrhaven Conservancy East Subdivision development, for the collection and transmission of stormwater runoff for all storm events up to and including the 100-year storm event, discharging to the Jock River, consisting of the following:

- **approximately 950 metres long modified channel**, located along the northern boundary of the Barrhaven Conservancy East Phase 2, 3 & Jock River, having a channel gradient of 0.09% and 3:1 side slopes, complete with low flow path and riffle-pool sequences and erosion protection structures, including two (2) 2.4 metre by 1.2 metre box culverts under the future Canoe Street crossing, discharging to the Jock River;

the establishment of stormwater management Works to serve the Barrhaven Conservancy East – Phase 2, 3 & Jock River development, located in the City of Ottawa, for the collection, transmission, treatment and disposal of stormwater runoff from a total catchment area of 44.17 hectares, to provide Enhanced Level protection and to provide on-site retention of 22.5 cubic metres per hectare, discharging to proposed storm sewers, consisting of the following:

- **subsurface infiltration trenches (catchment area 41.81 hectares)**, located on-site within proposed roadways, having a total length of 3,514 metres, a width of 1.75 metres, a base area of 6,150 square metres, a maximum allowable storage depth of 0.40 metres and a maximum available storage volume of 1,087 cubic metres, comprised of a 75 millimetre deep clear stone layer overlying a geotextile non-woven filter fabric, complete with a 250 millimetre diameter perforated storm sub-drain installed in the clear stone layer, installed at select street catch basin manhole locations;

the establishment of stormwater management Works to serve Steeves & Rozema Enterprises Limited, located in the City of Sarnia, consisting of the following:

- **oil and grit separator (catchment area 5.52 hectares):** one (1) oil and grit separator (OGS1), CDS Model PMSU4040-8 or Equivalent Equipment, located within the Canoe Street right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 83%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Canoe Street right-of-way, discharging via a 975 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 5.59 hectares):** one (1) oil and grit separator (OGS2), CDS Model PMSU4040-8 or Equivalent Equipment, located within Servicing Block 767, providing a Predicted Net Annual Load Removal Efficiency of 82.1%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Block 767 and Meander Way right-of-way, discharging via a 975 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 6.77 hectares):** one (1) oil and grit separator (OGS3), CDS Model PMSU4045-8 or Equivalent Equipment, located within the Ainsworth Crescent right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 82.4%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 2,149 litres, a total storage volume of approximately 11,510 litres, and a maximum treatment rate of 212 litres per second, receiving inflow from the storm sewer located within the Ainsworth Crescent right-of-way, discharging via a 975 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 8.42 hectares):** one (1) oil and grit separator (OGS5), CDS Model PMSU5640-10 or Equivalent Equipment, located within Servicing Block 766, providing a Predicted Net Annual Load Removal Efficiency of 81.7%, having a sediment storage capacity of 6,672 litres, an oil storage capacity of 2,869 litres, a total storage volume of approximately 17,070 litres, and a maximum treatment rate of 255 litres per second, receiving inflow from the storm sewer located within Block 766 and the Gallium Crescent right-of-way, discharging via a 1050 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;

- **oil and grit separator (catchment area 5.46 hectares):** one (1) oil and grit separator (OGS6), CDS Model PMSU3035-8 or Equivalent Equipment, located within Servicing Block 765, providing a Predicted Net Annual Load Removal Efficiency of 80.0%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,493 litres, a total storage volume of approximately 10,210 litres, and a maximum treatment rate of 108 litres per second, receiving inflow from the storm sewer located within Block 766 and the Pollination Place right-of-way, discharging via a 900 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 5.05 hectares):** one (1) oil and grit separator (OGS7), CDS Model PMSU4040-8 or Equivalent Equipment, located within the Ephemeral Crescent right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 82.3%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Ephemeral Crescent right-of-way, discharging via a 900 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 4.52 hectares):** one (1) oil and grit separator (OGS8), CDS Model PMSU4040-8 or Equivalent Equipment, located within Servicing Block 744, providing a Predicted Net Annual Load Removal Efficiency of 82.1%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Borrisokane Road right-of-way, discharging via a 1200 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 1.63 hectares):** one (1) oil and grit separator (OGS12), CDS Model PMSU2025-5 or Equivalent Equipment, located within Servicing Block 317, providing a Predicted Net Annual Load Removal Efficiency of 81.1%, having a sediment storage capacity of 1,668 litres, an oil storage capacity of 439 litres, a total storage volume of approximately 3,330 litres, and a maximum treatment rate of 45 litres per second, receiving inflow from the storm sewer located within the Les Emmerson Drive right-of-way, discharging via a 600 millimetre diameter outlet pipe to an outlet channel connecting to the Fraser-Clarke watercourse;
- **oil and grit separator (catchment area 1.21 hectares):** one (1) oil and grit separator (OGS13), CDS Model PMSU2020-5 or Equivalent Equipment, located within the Deciduous Crescent right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 80.1%, having a sediment storage capacity of 1,668 litres, an oil storage capacity of 376 litres, a total storage volume of approximately 3,150 litres, and a maximum treatment rate of 31 litres per second, receiving inflow from the storm sewer located within the Deciduous Crescent right-of-way, discharging via a 600 millimetre diameter outlet pipe to an outlet channel connecting to the Fraser-Clarke watercourse;

including erosion/sedimentation control measures during construction and all other controls and appurtenances essential for the proper operation of the aforementioned Works;

all in accordance with the submitted application and supporting documents listed in Schedule "A" forming part of this Approval.

For the purpose of this environmental compliance approval, the following definitions apply:

1. "Approval" means this entire document and any schedules attached to it, and the application;
2. "Director" means a person appointed by the Minister pursuant to section 5 of the EPA for the purposes of Part II.1 of the EPA;
3. "District Manager" means the District Manager of the appropriate local District Office of the Ministry, where the Works are geographically located;
4. "EPA" means the *Environmental Protection Act*, R.S.O. 1990, c.E.19, as amended;
5. "Equivalent Equipment" means a substituted equipment or like-for-like equipment that meets the required quality and performance standards of the approved named equipment.
6. "Ministry" means the ministry of the government of Ontario responsible for the EPA and OWRA and includes all officials, employees or other persons acting on its behalf;
7. "Owner" means Barrhaven Conservancy Development Corporation, and includes its successors and assignees;
8. "OWRA" means the *Ontario Water Resources Act*, R.S.O. 1990, c. O.40 , as amended;
9. "Works" means the sewage Works described in the Owner's application, and this Approval.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL CONDITIONS

1. The Owner shall ensure that any person authorized to carry out work on or operate any aspect of the Works is notified of this Approval and the conditions herein and shall take all reasonable measures to ensure any such person complies with the same.

2. Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the Works in accordance with the description given in this Approval, and the application for approval of the Works.
3. Where there is a conflict between a provision of any document in the schedule referred to in this Approval and the conditions of this Approval, the conditions in this Approval shall take precedence, and where there is a conflict between the documents in the schedule, the document bearing the most recent date shall prevail.
4. Where there is a conflict between the documents listed in Schedule "A" and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.
5. The conditions of this Approval are severable. If any condition of this Approval, or the application of any requirement of this Approval to any circumstance, is held invalid or unenforceable, the application of such condition to other circumstances and the remainder of this Approval shall not be affected thereby.

2. EXPIRY OF APPROVAL

1. This Approval will cease to apply to those parts of the Works which have not been constructed within five (5) years of the date of this Approval.
2. In the event that completion and commissioning of any portion of the Works is anticipated to be delayed beyond the specified expiry period, the Owner shall submit an application of extension to the expiry period, at least twelve (12) months prior to the end of the period. The application for extension shall include the reason(s) for the delay, whether there is any design change(s) and a review of whether the standards applicable at the time of Approval of the Works are still applicable at the time of request for extension, to ensure the ongoing protection of the environment.

3. CHANGE OF OWNER

1. The Owner shall notify the District Manager and the Director, in writing, of any of the following changes within thirty (30) days of the change occurring:
 - a. change of Owner;
 - b. change of address of the Owner;
 - c. change of partners where the Owner is or at any time becomes a partnership, and a copy of the most recent declaration filed under the *Business Names Act*, R.S.O. 1990, c.B17 shall be included in the notification to the District Manager; or

- d. change of name of the corporation where the Owner is or at any time becomes a corporation, and a copy of the most current information filed under the *Corporations Information Act*, R.S.O. 1990, c. C39 shall be included in the notification to the District Manager.
2. In the event of any change in ownership of the Works, other than a change to a successor municipality, the Owner shall notify in writing the succeeding owner of the existence of this Approval, and a copy of such notice shall be forwarded to the District Manager and the Director.
3. The Owner shall ensure that all communications made pursuant to this condition refer to the number at the top of this Approval.

4. OPERATION AND MAINTENANCE

1. If applicable, any proposed storm sewers or other stormwater conveyance in this Approval can be constructed but not operated until the proposed stormwater management facilities in this Approval or any other Approval that are designed to service the storm sewers or other stormwater conveyance are in operation.
2. The Owner shall make all necessary investigations, take all necessary steps and obtain all necessary approvals so as to ensure that the physical structure, siting and operations of the Works do not constitute a safety or health hazard to the general public.
3. The Owner shall undertake an inspection of the condition of the Works, at least once a year, and undertake any necessary cleaning and maintenance to ensure that sediment, debris and excessive decaying vegetation are removed from the Works to prevent the excessive build-up of sediment, oil/grit, debris and/or decaying vegetation, to avoid reduction of the capacity and/or permeability of the Works, as applicable. The Owner shall also regularly inspect and clean out the inlet to and outlet from the Works to ensure that these are not obstructed.
4. The Owner shall construct, operate and maintain the Works with the objective that the effluent from the Works is essentially free of floating and settleable solids and does not contain oil or any other substance in amounts sufficient to create a visible film, sheen, foam or discoloration on the receiving waters.
5. The Owner shall maintain a logbook to record the results of these inspections and any cleaning and maintenance operations undertaken, and shall keep the logbook at the Owner's administrative office for inspection by the Ministry. The logbook shall include the following:
 - a. the name of the Works; and
 - b. the date and results of each inspection, maintenance and cleaning, including an estimate of the quantity of any materials removed and method of clean-out of the Works.

6. The Owner shall prepare an operations manual prior to the commencement of operation of the Works that includes, but is not necessarily limited to, the following information:
 - a. operating and maintenance procedures for routine operation of the Works;
 - b. inspection programs, including frequency of inspection, for the Works and the methods or tests employed to detect when maintenance is necessary;
 - c. repair and maintenance programs, including the frequency of repair and maintenance for the Works;
 - d. contingency plans and procedures for dealing with potential spills and any other abnormal situations and for notifying the District Manager; and
 - e. procedures for receiving, responding and recording public complaints, including recording any follow-up actions taken.
7. The Owner shall maintain the operations manual current and retain a copy at the Owner's administrative office for the operational life of the Works. Upon request, the Owner shall make the manual available to Ministry staff.

5. TEMPORARY EROSION AND SEDIMENT CONTROL

1. The Owner shall install and maintain temporary sediment and erosion control measures during construction and conduct inspections once every two (2) weeks and after each significant storm event (a significant storm event is defined as a minimum of 25 mm of rain in any 24 hours period). The inspections and maintenance of the temporary sediment and erosion control measures shall continue until they are no longer required and at which time they shall be removed and all disturbed areas reinstated properly.
2. The Owner shall maintain records of inspections and maintenance which shall be made available for inspection by the Ministry, upon request. The record shall include the name of the inspector, date of inspection, and the remedial measures, if any, undertaken to maintain the temporary sediment and erosion control measures.

6. REPORTING

1. One (1) week prior to the start-up of the operation of the Works, the Owner shall notify the District Manager (in writing) of the pending start-up date.
2. The Owner shall, upon request, make all reports, manuals, plans, records, data, procedures and supporting documentation available to Ministry staff.

3. The Owner shall prepare a performance report within ninety (90) days following the end of the period being reported upon, and submit the report(s) to the District Manager when requested. The first such report shall cover the first annual period following the commencement of operation of the Works and subsequent reports shall be prepared to cover successive annual periods following thereafter. The reports shall contain, but shall not be limited to, the following information:
 - a. a description of any operating problems encountered and corrective actions taken;
 - b. a summary of all maintenance carried out on any major structure, equipment, apparatus, mechanism or thing forming part of the Works, including an estimate of the quantity of any materials removed from the Works;
 - c. a summary of any complaints received during the reporting period and any steps taken to address the complaints;
 - d. a summary of all spill or abnormal discharge events; and
 - e. any other information the District Manager requires from time to time.

7. RECORD KEEPING

1. The Owner shall retain for a minimum of five (5) years from the date of their creation, all records and information related to or resulting from the operation, maintenance and monitoring activities required by this Approval.

Schedule "A"

1. Application for Environmental Compliance Approval, dated August 9, 2022 and received on August 17, 2022, submitted by Barrhaven Conservancy Development Corporation;
2. Transfer of Review Letter of Recommendation, dated August 17, 2022 and signed by Jeff Shillington, P.Eng., Senior Project Manager, Development Review, City of Ottawa , including the following supporting documents:
 - a. Final Plans and Specifications prepared by David Schaeffer Engineering Ltd.
 - b. Pipe Data Form - Watermain, Storm Sewer, Sanitary Sewer, and Forcemain Design Supplement to Application for Approval for Water and Sewage Works.
 - c. Hydraulic Design Sheets prepared by David Schaeffer Engineering Ltd.
 - d. Stormwater Management Report prepared by David Schaeffer Engineering Ltd.
 - e. Design brief, calculations and specifications prepared by David Schaeffer Engineering Ltd.
3. Email received on August 25, 2022 from Jeff Shillington, City of Ottawa.

The reasons for the imposition of these terms and conditions are as follows:

1. Condition 1 is imposed to ensure that the Works are constructed and operated in the manner in which they were described and upon which approval was granted. This condition is also included to emphasize the precedence of conditions in the Approval and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
2. Condition 2 is included to ensure that, when the Works are constructed, the Works will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment.
3. Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to the approved Works and to ensure that subsequent owners of the Works are made aware of the Approval and continue to operate the Works in compliance with it.
4. Condition 4 is included as regular inspection and necessary removal of sediment and excessive decaying vegetation from the Works are required to mitigate the impact of sediment, debris and/or decaying vegetation on the treatment capacity of the Works. The Condition also ensures that adequate storage is maintained in the Works at all times as required by the design. Furthermore, this Condition is included to ensure that the Works are operated and maintained to function as designed.
5. Condition 5 is included as installation, regular inspection and maintenance of the temporary sediment and erosion control measures is required to mitigate the impact on the downstream receiving watercourse during construction until they are no longer required.
6. Condition 6 is included to provide a performance record for future references, to ensure that the Ministry is made aware of problems as they arise, and to provide a compliance record for all the terms and conditions outlined in this Approval, so that the Ministry can work with the Owner in resolving any problems in a timely manner.
7. Condition 7 is included to require that all records are retained for a sufficient time period to adequately evaluate the long-term operation and maintenance of the Works.

In accordance with Section 139 of the *Environmental Protection Act*, you may by written notice served upon me and the Ontario Land Tribunal within 15 days after receipt of this notice, require a hearing by the Tribunal. Section 142 of the *Environmental Protection Act* provides that the notice requiring the hearing ("the Notice") shall state:

- a. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
- b. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

1. The name of the appellant;
2. The address of the appellant;
3. The environmental compliance approval number;
4. The date of the environmental compliance approval;
5. The name of the Director, and;
6. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

Registrar*
Ontario Land Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5
OLT.Registrar@ontario.ca

and

The Director appointed for the purposes of
Part II.1 of the *Environmental Protection Act*
Ministry of the Environment,
Conservation and Parks
135 St. Clair Avenue West, 1st Floor
Toronto, Ontario
M4V 1P5

* **Further information on the Ontario Land Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349 or 1 (866) 448-2248, or www.olt.gov.on.ca**

The above noted activity is approved under s.20.3 of Part II.1 of the *Environmental Protection Act*.

DATED AT TORONTO this 1st day of September, 2022



Aziz Ahmed, P.Eng.
Director
appointed for the purposes of Part II.1 of the
Environmental Protection Act

RR/

c: District Manager, MECP Ottawa District Office
Clerk, City of Ottawa (File No. D07-16-20-0021)
Jeff Shillington, P.Eng., Senior Project Manager, Development Review, City of Ottawa
Kevin Murphy, David Shaeffer Engineering Ltd.

APPENDIX B
WATER SUPPLY



**Barrhaven Conservancy East
(Phases 2, 3, 4 & Jock River):
Water Distribution System Analysis**

Final Report

June 2, 2022

Prepared for:

David Schaeffer Engineering Ltd.

Prepared by:

Stantec Consulting Ltd.

Revision	Description	Author		Quality Check		Independent Review	
0	Final	TAW	20211213	JS	20211214	KA	20211216
1	Final	TAW/AMG	20220512	AMG	20220516	AP	20220518
2	Final	TAW/AMG	20220602	AMG	20220602	AP	20220602



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

This document entitled **Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River): Water Distribution System Analysis** was prepared by Stantec Consulting Ltd. ("Stantec") for the account of David Schaeffer Engineering Ltd. (the "Client"). Any reliance on this document by any third party is strictly prohibited. The material in it reflects Stantec's professional judgment in light of the scope, schedule and other limitations stated in the document and in the contract between Stantec and the Client. The opinions in the document are based on conditions and information existing at the time the document was published and do not take into account any subsequent changes. In preparing the document, Stantec did not verify information supplied to it by others. Any use which a third party makes of this document is the responsibility of such third party. Such third party agrees that Stantec shall not be responsible for costs or damages of any kind, if any, suffered by it or any other third party as a result of decisions made or actions taken based on this document.

Prepared by _____
(signature)

Thomas Westwood, M.Eng., P.Eng.

Prepared by _____
(signature)

Alexandre Mineault-Guitard, M.Sc.A., ing., P.Eng.

Reviewed by _____
(signature)

Alexandre Mineault-Guitard, M.Sc.A., ing., P.Eng.

Approved by _____
(signature)

Ana Paerez, P.Eng.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Introduction
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1.0 INTRODUCTION

To support David Schaeffer Engineering Ltd (DSEL) with their conceptual design submission for the Barrhaven Conservancy East development lands (Phases 2, 3, 4 and Jock River), Stantec Consulting Ltd (Stantec) was requested to provide engineering services to complete a water distribution system analysis for this proposed development located within the City of Ottawa's (City) South Urban Community (SUC). The purpose of the analysis is to confirm associated watermain sizing and redundancy needs.

For this assignment, Stantec's scope of work included the following tasks:

- 1) Reviewing background information and establishing updated water demands for the Conservancy East development area based on the most current draft plan;
- 2) Preparing and submitting a boundary condition request to the City;
- 3) Preparing a stand-alone hydraulic model of the distribution system within the Conservancy East lands using boundary conditions provided by the City. The backbone watermain planning model used for previous planning-level analyses will be used as a base;
- 4) Assessing Fire Underwriters Survey (FUS) fire flow requirements;
- 5) Setting up and running model simulations for average day (AVDY), peak hour (PKHR), and maximum day (MXDY) plus fire flow demands to identify watermain sizing and redundancy needs required for the water distribution system within the development lands to meet design criteria; and,
- 6) Documenting the approach used, findings and recommendations from the analysis.

1.1 STUDY AREA

The study area, referred to as the Barrhaven Conservancy East development lands, is located in the City's southwestern suburban neighbourhood of Barrhaven. The lands are situated between Strandherd Dr to the north, the Jock River to the south, Fraser-Clark Drain to the east, and bisected by Borrisokane Rd through the western portion. Based on the current site plan provided by DSEL (dated October 13, 2021) and additional sub-phasing information (dated March 9, 2022), the proposed development is to be subdivided into four (4) phases, which are further described in **Section 1.2**. The proposed development will comprise a total of 782 single family home (SFH) units and 606 townhouse (MLT) units (consisting of a combination of rear-lane, back-to-back and standard townhouse units) for a total estimated population of 4,295 persons.

Based on a previously completed serviceability study for these lands (Stantec Consulting Ltd., 2021), this residential community, which is currently situated adjacent to Pressure Zone 3SW (previously known as



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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Zone BARR), is ultimately planned to be serviced by the future Zone SUC. In 2015, the City embarked on a large initiative to reconfigure the pressure zones servicing Barrhaven and the southern reaches of Ottawa (i.e., SUC). The City has indicated that the pressure zone reconfiguration is planned to be completed by the second quarter (Q2) of 2024. The purpose of the zone reconfiguration was to improve reliability and efficiencies, and to provide increased pumping capacity for future growth. As such, these development lands are to be serviced by two connections to the existing distribution network, both of which are currently part of Zone 3SW and will ultimately be part of Zone SUC. These include the following locations as shown in **Figure 1-1**:

- 1) The existing 305 mm stub extending from Chapman Mills Dr (east of Kennedy-Burnett Pond); and
- 2) The T-junction on the existing 203 mm watermain at Danson Gardens Grv and Darjeeling Ave.

Both connections would require crossing the Kennedy-Burnett Pond and the Fraser-Clarke Drain.

The City has also suggested that a third connection be considered, which is also illustrated in **Figure 1-1**. This potential third connection is located south of the Jock River, at a future 305 mm stub at the intersection of Flagstaff Dr and Borrisokane Rd, and would require crossing the Jock River to service the proposed development lands. The serviceability of the development lands using this third connection is also analyzed herein.

1.2 PHASING OF BARRHAVEN CONSERVANCY EAST

For the purpose of this assessment, development within Barrhaven Conservancy East, as shown in **Figure 1-2**, is assumed to occur in the following phasing order:

- 1) Phase 2 – Comprising 240 SFH units, 98 MLT units and two park areas. The townhouses in this phase are a combination of rear-lane and standard townhouse units. Phase 2 will consist of three (3) subphases:
 - Phase 2A – Comprising 102 SFH units and one park area.
 - Phase 2B – Comprising 129 SFH units.
 - Phase 2C – Comprising 9 SFH units, 98 MLT units and one park area.
- 2) Phase 3 – Comprising 128 SFH units and 197 MLT units. As with Phase 2, these townhouses are a combination of rear-lane and standard townhouse units. Phase 3 will consist of two (2) subphases:
 - Phase 2D – Comprising 42 SFH units and 47 MLT units.
 - Phase 2E – Comprising 86 SFH units and 150 MLT units.



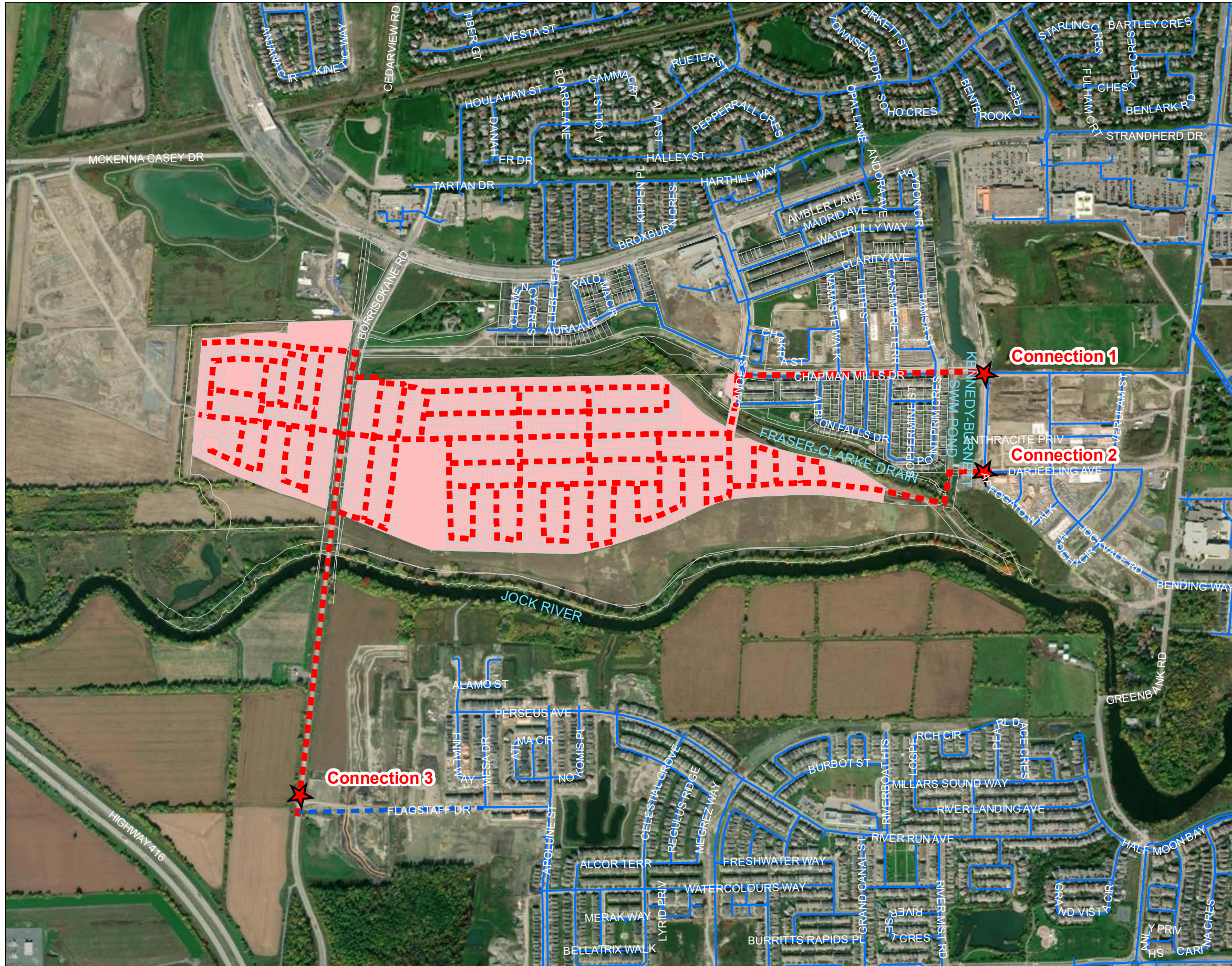
BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Introduction
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- 3) Phase 4 - Comprising 86 SFH units, 311 MLT units and one park area. As with Phase 2 and 3, the townhouses in this phase are a combination of rear-lane and standard townhouse units, with additional blocks of back-to-back townhouses; and,
- 4) Jock River – Comprising 328 SFH units.

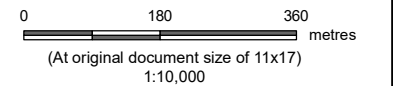
As previously mentioned, the development area will ultimately be serviced by the pressure Zone SUC, once the reconfiguration is complete (planned in Q2 of 2024). As such, the analysis and proposed watermain sizing and layout documented in this report only considers the Zone SUC servicing conditions.



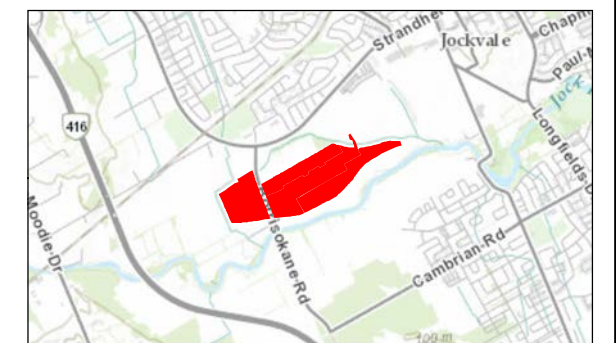


Legend

- Barrhaven Conservancy East Lands
- Property Line
- Existing Distribution Watermain
- Future Distribution Watermain
- Connection Location
- Future Watermain to Service Barrhaven Conservancy East Lands



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community
 Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
 Ottawa, ON

Client/Project
 David Schaeffer Engineering Ltd
 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

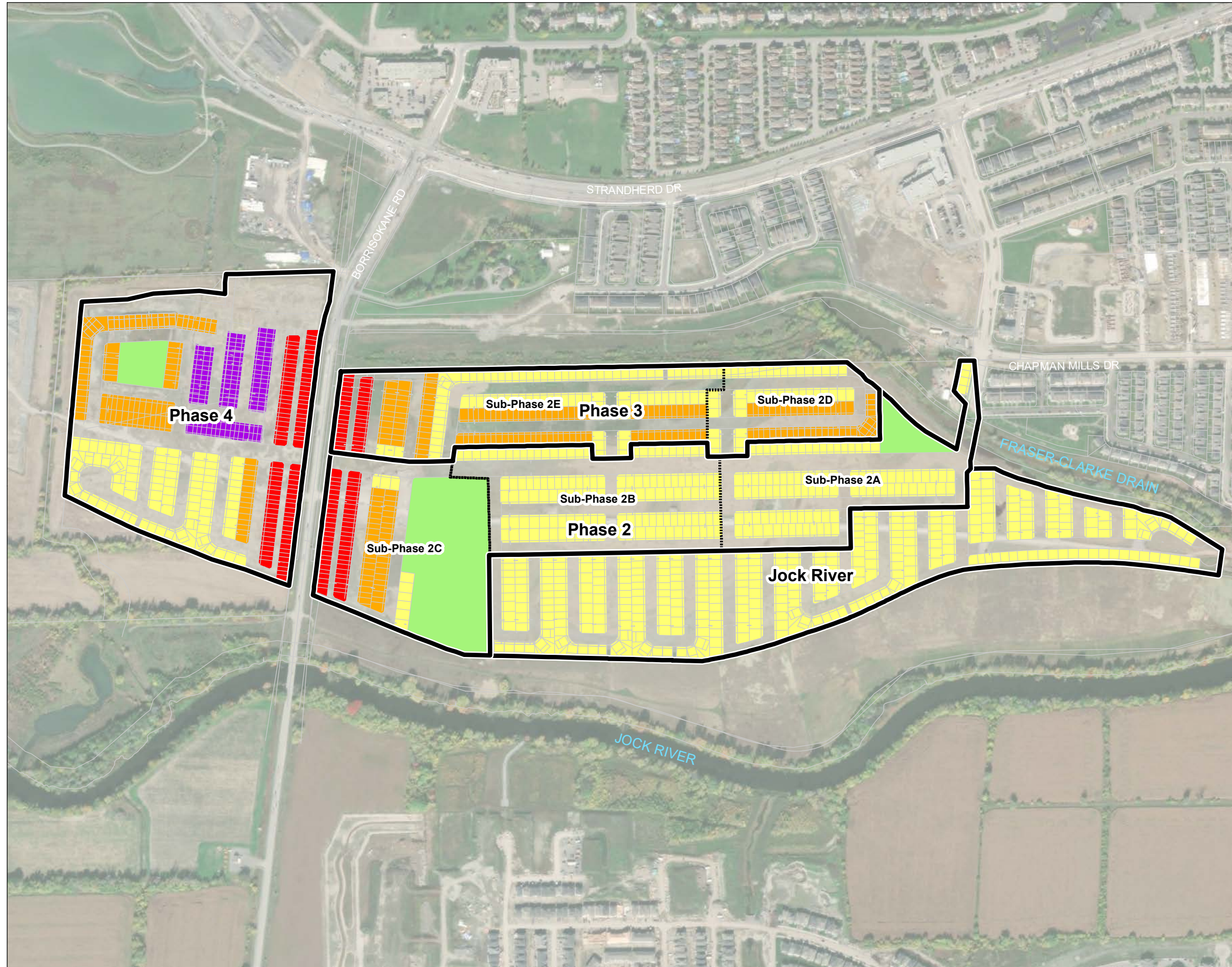
Figure No.

1-1

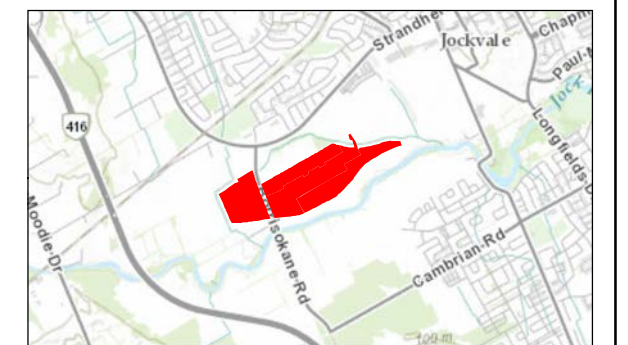
Title
Connections to Existing Water Distribution Network

Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Single Family Home (SFH)
- Standard Townhouse (STND TH)
- Rear-Lane Townhouse (RLTH)
- Back-to-Back Townhouse (B2B)
- Park
- Property Line



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community
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 Water Distribution System Analysis

Figure No.

1-2

Title

Phasing Plan of Barrhaven Conservancy East

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Hydraulic Assessment
June 2, 2022

2.0 HYDRAULIC ASSESSMENT

The City of Ottawa Water Design Guidelines (City of Ottawa, 2010) and criteria outlined in the 2013 Water Master Plan (WMP) were used to establish water demands, level of service and pressure objectives during normal and emergency conditions. As per the City's design guidelines and recently issued Technical Bulletin ISTB-2021-03, since this is a new development involving the design of new watermains, the design shall consider a required fire flow established using the calculation method published by the Fire Underwriters Survey (FUS).

2.1 SERVICEABILITY

2.1.1 System Pressures

As per the City's Water Design Guidelines, the desired range of pressure under average day (AVDY), maximum day (MXDY) and peak hour (PKHR) demands is 345 to 552 kPa (50 to 80 psi) and no less than 276 kPa (40 psi) at ground elevation (i.e., at street level). The maximum pressure at any point in the water distribution system should not exceed 552 kPa (80 psi); pressure reducing measures are required to service areas where pressures greater than 552 kPa (80 psi) are anticipated.

Under emergency fire conditions, the system must be able to supply appropriate fire flow while maintaining a residual pressure of 138 kPa (20 psi).

Figure 2-1 shows the elevations of each model junction based on the site's current grading plan. These range from 92.4 m to 93.5 m.

2.1.2 Fire Flows

The City requires a fire flow assessment to be completed to demonstrate that local watermains can provide the objective fire flows. The detailed FUS Guidelines (long method; 1999 Version) was used to calculate the objective fire flows. Based on site plan information provided, the following characteristics were considered in the FUS calculations:

- All townhouse units will be of typical construction (e.g., wood frame, limited combustible building contents); firewalls are to be added where required to meet the study area's target fire flow;
- Single family home units will generally be of typical (wood frame) construction except where a break in fire area is required to meet the study area's target fire flow. At such locations, units will be of ordinary construction as described in the FUS guidelines.
 - With side yard separation distances of < 3.0 m between SFH units, the current site layout would contain several large blocks of contiguous SFH units if all were to be of wood frame construction. It is our understanding that the current rearyard setback for all SFH



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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products will be 4.5 m, which in most locations throughout the development area results in rearyard separation distances of less than 10 m. As such, the City's cap of 10,000 L/min, as per Technical Bulletin ISDTB-2018-02, does not apply to these areas and measures such as separating fire areas with units of ordinary construction is required to meet the study area's target fire flow;

- All buildings will have 2 stories above grade (with basements more than 50% below ground level);
- Buildings are not sprinklered; and,
- Setbacks between adjacent units are greater than 3.0 m, with the exception of some proposed SFH units.
 - Per the FUS Guidelines, units with setbacks less than 3.0 m and of wood frame construction will be considered a single fire area.

Based on the latest site plan dated October 2021 and subsequent architectural changes, the required fire flow (RFF) for the governing unit design (rear-lane townhouses, RLTH) was calculated to be 13,000 L/min (217 L/s). This is based on the understanding that, as previously noted, ordinary construction SFH units will be used to separate SFH blocks into fire areas that result in RFFs no greater than 13,000 L/min. Similarly, townhouse blocks will also have firewalls to limit fire areas such that the resulting RFFs will be no greater than 13,000 L/min. The local watermains must therefore be able to provide a minimum fire flow of 13,000 L/min at a residual pressure of 20 psi. The FUS fire flow calculations for the governing unit design and to meet the target fire flow are provided in **Appendix A**.

2.1.3 Water Age

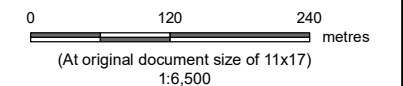
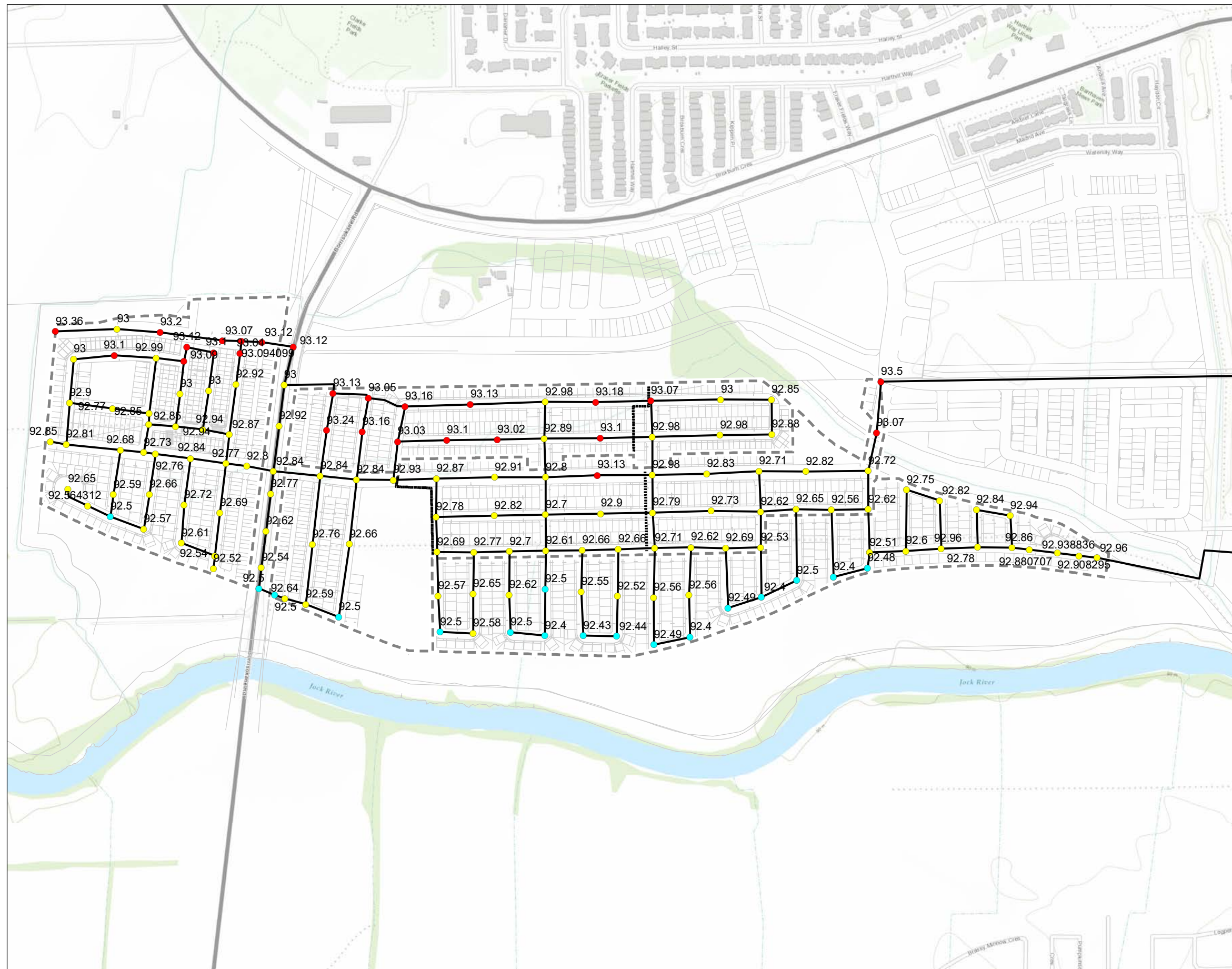
As per the City of Ottawa Design Guidelines, watermains should not be oversized as this may pose water quality degradation, assessed in terms of water age. The Design Guidelines recommend the following:

- A total travel time of 5 days or less during average day demand; and
- A maximum residence time of 8 days.

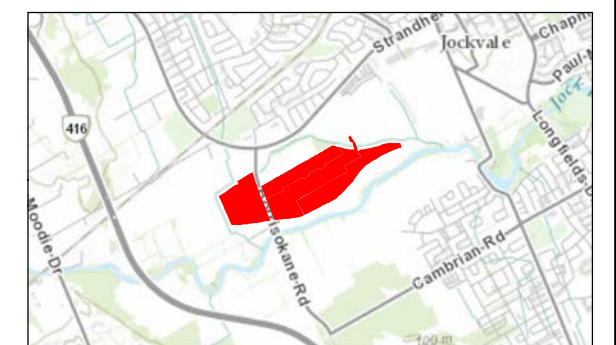


Legend

- Development Phase Boundary
 - Development Sub-Phase Boundary
 - Property Line
 - Future Watermain
- Ground Elevation (m AD)**
- ≤ 92.00
 - 92.01 - 92.50
 - 92.51 - 93.00
 - > 93.00



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.

2-1

Title

Junction Elevations

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Hydraulic Assessment
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2.2 GROWTH PROJECTIONS

The estimated residential population for Barrhaven Conservancy East was estimated based on projected household sizes as per population densities (or persons per unit, PPU) specified in the City's Water Design Guidelines.

Table 2-1 shows the estimated number of units per phase of these development lands and the projected populations based on the distribution of residential types. The total number of units is estimated to be 1,388 with a residential population of 4,295 persons.

Table 2-1: Estimated Unit Counts and Populations for Barrhaven Conservancy East

Phase	Sub Phase	Unit Types	Units	PPU	Population
2	2A	Singles	102	3.4	347
		Towns	0	2.7	0
	2B	Singles	129	3.4	439
		Towns	0	2.7	0
	2C	Singles	9	3.4	31
		Towns	98	2.7	265
<i>Phase 2 Sub-total</i>			338	-	1,081
3	2D	Singles	42	3.4	143
		Towns	47	2.7	127
	2E	Singles	86	3.4	292
		Towns	150	2.7	405
<i>Phase 3 Sub-total</i>			325	-	967
4		Singles	86	3.4	292
		Towns	311	2.7	840
	<i>Phase 4 Sub-total</i>			397	-
Jock River (JR)		Singles	328	3.4	1,115
		Towns	0	2.7	0
	<i>JR Phase Sub-total</i>			328	-
Total			1,388		4,295



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.3 DEMAND PROJECTIONS

Due to the size of the service area, the criteria outlined in the City's Water Design Guidelines and recently issued Technical Bulletin ISTB-2021-03 were followed to establish water demands in Barrhaven Conservancy East. As the buildout population of the proposed development is 4,925 (i.e., greater than 3,000), the City's Water Design Guidelines refer to the MECF Guidelines for consumption rates. The MECF Guidelines provide a consumption rate range of 270 L/cap/day to 450 L/cap/day. The City's Water Design Guidelines consumption rates for subdivisions of 501 to 3,000 persons fall within that range and are therefore applicable. The demand rates and peaking factors from the Water Design Guidelines and Technical Bulletin ISTB-2021-03 were applied to the population projections presented in **Table 2-1** based on land-use.

For residential land-use, SFH and MLT units were assigned an average day (AVDY) consumption rate of 280 L/cap/d. To determine maximum day (MXDY) demands, the AVDY demands were multiplied by a residential peaking factor of 2.5. Peak hour (PKHR) demands were established by multiplying MXDY demands by a residential peaking factor of 2.2. The projected AVDY, MXDY and PKHR demands were distributed to the model nodes by phase (and sub-phase) for the corresponding demand scenario.

Estimated AVDY, MXDY and PKHR demand projections are summarized in **Table 2-2**.

Table 2-2: Estimated Demand Projections for Barrhaven Conservancy East

Phase	Sub-Phase	Unit Types	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
2	2A	Singles	1.12	2.81	6.18
		Towns	0	0	0
	2B	Singles	1.42	3.55	7.82
		Towns	0	0	0
	2C	Singles	0.10	0.25	0.55
		Towns	0.86	2.14	4.72
Phase 2 Sub-total			3.50	8.75	19.26
3	2D	Singles	0.46	1.16	2.55
		Towns	0.41	1.03	2.26
	2E	Singles	0.95	2.37	5.21
		Towns	1.31	3.28	7.22
	Phase 3 Sub-total			3.13	7.84
4	Singles	0.95	2.37	5.21	
	Towns	2.72	6.80	14.97	
	Phase 4 Sub-total			3.67	9.17
Jock River (JR)	Singles	3.61	9.04	19.88	
	Towns	0	0	0	
	JR Phase Sub-total			3.61	9.04
Total			13.92	34.80	76.55



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.4 MODEL DEVELOPMENT

Innovyze’s InfoWater (Suite 12.4, Update #9) was used to create a stand-alone hydraulic model of the water distribution system within the proposed development area for this analysis. The model was developed to reflect the most current site plan, including proposed watermain layout (based on proposed road alignment) and water demands.

Watermains added to the model were assigned Hazen-Williams coefficients (“C-Factors”) in accordance with the City’s Water Design Guidelines. These factors are listed in **Table 2-3**.

Table 2-3: Hazen-Williams Coefficients by Watermain Size

Watermain Diameter (mm)	Coefficient
152	100
203 - 305	110
350 - 600	120
> 600	130

2.4.1 Boundary Conditions

The proposed subdivision has two connection points to the existing water distribution system; the option to add a third connection point is also assessed (see **Section 1.1**). The boundary conditions provided by the City include hydraulic gradeline (HGL) values for Zone SUC servicing conditions. Values are provided in **Appendix B** and summarized in **Table 2-4**, and have been simulated in the hydraulic model using fixed head reservoirs to which HGLs have been applied for the respective demand scenarios.

Differences in HGL between connections 1 and 2 are observed under higher demand scenarios. While under AVDY and PKHR conditions, these differences are small (0.2 m or less), under MXDY+FF and AVDY+FF conditions, these differences increase to approximately 4 m. This is likely due to the fact that the Connection 1 is connected to a 305 mm diameter along Chapman Mills Dr, whereas Connection 2 is to a 203 mm diameter along Darjeeling Ave, and the two connection points are interconnected by a 203 mm diameter watermain along Danson Gardens Grv.

If the existing watermain along Danson Gardens Grv is upgraded to a 305 mm diameter watermain, the discrepancies in HGL between Connection 1 and Connection 2 decrease. A second set of boundary conditions (two connections with upgrades) reflecting these conditions was provided by the City.

Finally, a third set of boundary conditions was provided by the City, reflecting conditions with three connection points to the existing water distribution system. These boundary conditions do not consider any upgrades between Connection 1 and Connection 2, and as such the HGLs differ by 3.5 to 3.7 m under MXDY+FF and AVDY+FF conditions.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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Table 2-4: HGL Boundary Conditions

HGL (m)			
Zone SUC Servicing Conditions			
Demand Scenario	Two Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	150.0	150.0	
PKHR	144.2	144.0	
AVDY +FF	138.7	135.1	
MXDY+FF	137.0	133.2	
Demand Scenario	Two Connections with Upgrades ⁽⁴⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	149.5	149.5	
PKHR	144.1	144.1	
AVDY +FF	138.6	139.8	
MXDY+FF	136.8	138.1	
Demand Scenario	Three Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	Connection 3 ⁽³⁾
AVDY	149.5	149.5	149.5
PKHR	144.5	144.4	142.0
AVDY +FF	138.6	135.1	137.4
MXDY+FF	137.1	133.4	134.8

Notes:

- (1) Ground elevation @ Connection 1 (Chapman Mills Dr) = 92.8 m.
- (2) Ground elevation @ Connection 2 (Danson Gardens Grv / Darjeeling Ave) = 91.8 m.
- (3) Ground elevation @ Connection 3 (Flagstaff Dr) = 92.1 m.
- (4) Upgrades to existing water distribution required to increase HGL at Connection 2; upsize existing 203 mm diameter watermain on Danson Gardens Grv to a 305 mm diameter watermain.
- (5) For scenarios where ultimate conditions will include three connections, the boundary conditions for two connections (without upgrades) were used when only Connections 1 and 2 are in place, i.e., for modelling Phases 2 and 3.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.4.2 Proposed Watermain Sizing & Layout

Two layouts and sizing of the watermains within the proposed development are presented. The first layout (Option A) is required to service the development if only two connections to the water distribution system (Connection 1 and Connection 2) are made. The second layout (Option B) is required to service the development if a third connection (south of the Jock River) is introduced.

2.4.2.1 Option A: Watermain Sizing & Layout for Two Connections

The layout and sizing of the watermains within the proposed development for a scenario with two ultimate connections to the water distribution system are shown in **Figure 2-2**. The same layout and sizing are required with upgrades to the water distribution system along Danson Gardens Grv.

The network is proposed to consist of 152 mm, 203 mm, 305 mm, and 406 mm diameter watermains, with the 305 mm and 406 mm watermains acting as the hydraulic backbone throughout the development lands. The 406 mm diameter watermains run west from connections 1 and 2, interconnect at the east side of the development lands and continue westward across Borrisokane Rd into the Phase 4 lands. The 406 mm diameter watermains can thus serve as backbone if future developments extend further westward. Additional backbone loops added for reliability will be 305 mm watermains. The remaining local watermains will be 152 mm and 203 mm diameter watermains.

The dead-end watermain in the Phase 4 cul-de-sac is proposed to be 203 mm stepping down to 152 mm. Using the traditional “point load assumption” modelling approach to sizing new watermains, the resulting diameter that would be required to provide a fire flow of 13,000 L/min would be larger than the maximum of 152 mm specified in the City’s design guidelines for dead-end watermains. As such, to optimize sizing of this watermain, the alternative procedure outlined in Appendix I (Guidelines on Coordination of Hydrant Placement with Required Fire Flow) of the City’s Technical Bulletin ISDTB-2018-02 was employed. Additional nodes were added to the model network to represent hydrant locations, to which hydrant flows from Table 1 of Appendix I were applied. To achieve a fire flow of 13,000 L/min, two Class AA hydrants within 75 m (each with an assumed maximum flow capacity of 5,700 L/min) and an additional Class AA hydrant between 75 m and 150 m (with an assumed maximum flow capacity of 3,800 L/min) of the furthest unit along the cul-de-sac would provide a total fire flow of 15,200 L/min (i.e., > the RFF of 13,000 L/min). Other hydrant spacing combinations in accordance with the City’s Technical Bulletin ISDTB-2018-02 can also achieve a total fire flow greater than the RFF of 13,000 L/min.

For reliability, the second backbone feed (305 mm) is redirected north then west (i.e., north of the standard and rear-lane townhouse blocks) across to the Phase 4 lands. This alignment provides reliability of service to the lands west of Borrisokane Rd in the event of a failure at the current intersection of the backbone feeds situated within the Borrisokane Rd ROW. System reliability is further discussed in **Section 3.3**. West of Borrisokane Rd, the 305 mm section of backbone will serve as a service connection to future adjacent development. There are no direct Phase 4 property service connections anticipated



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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along this temporary dead-end section of watermain; therefore, it is recommended that this section be isolated until it is required to provide flow to adjacent future lands.

2.4.2.2 Option B: Watermain Sizing & Layout for Three Connections

The layout and sizing of the watermains within the proposed development for a scenario with three ultimate connections to the water distribution system are shown in **Figure 2-3**.

The layout of the watermains is the same as in Option A (with two connections). However, with the third connection, the entire backbone can be reduced to 305 mm diameter watermains at all three connection points and throughout the development lands. The remaining watermain sizes are unchanged from Option A, with local watermains of 152 mm and 203 mm diameter watermains.

Based on the proposed phasing, the implementation of the third connection could be delayed until the development of the Phase 4 lands, west of Borrisokane Rd.

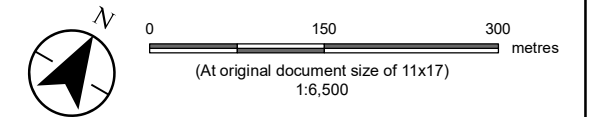


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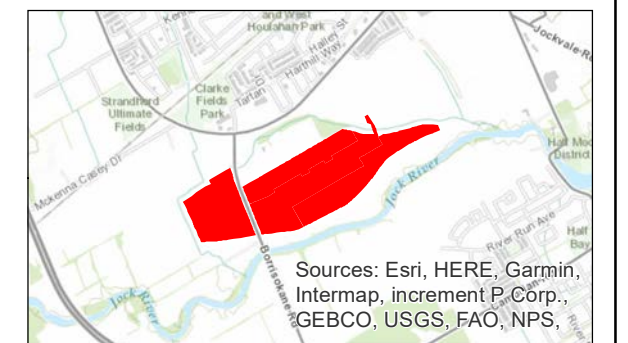
- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line

Proposed Watermain Diameter (mm)

- 152
- 203
- 305
- 406



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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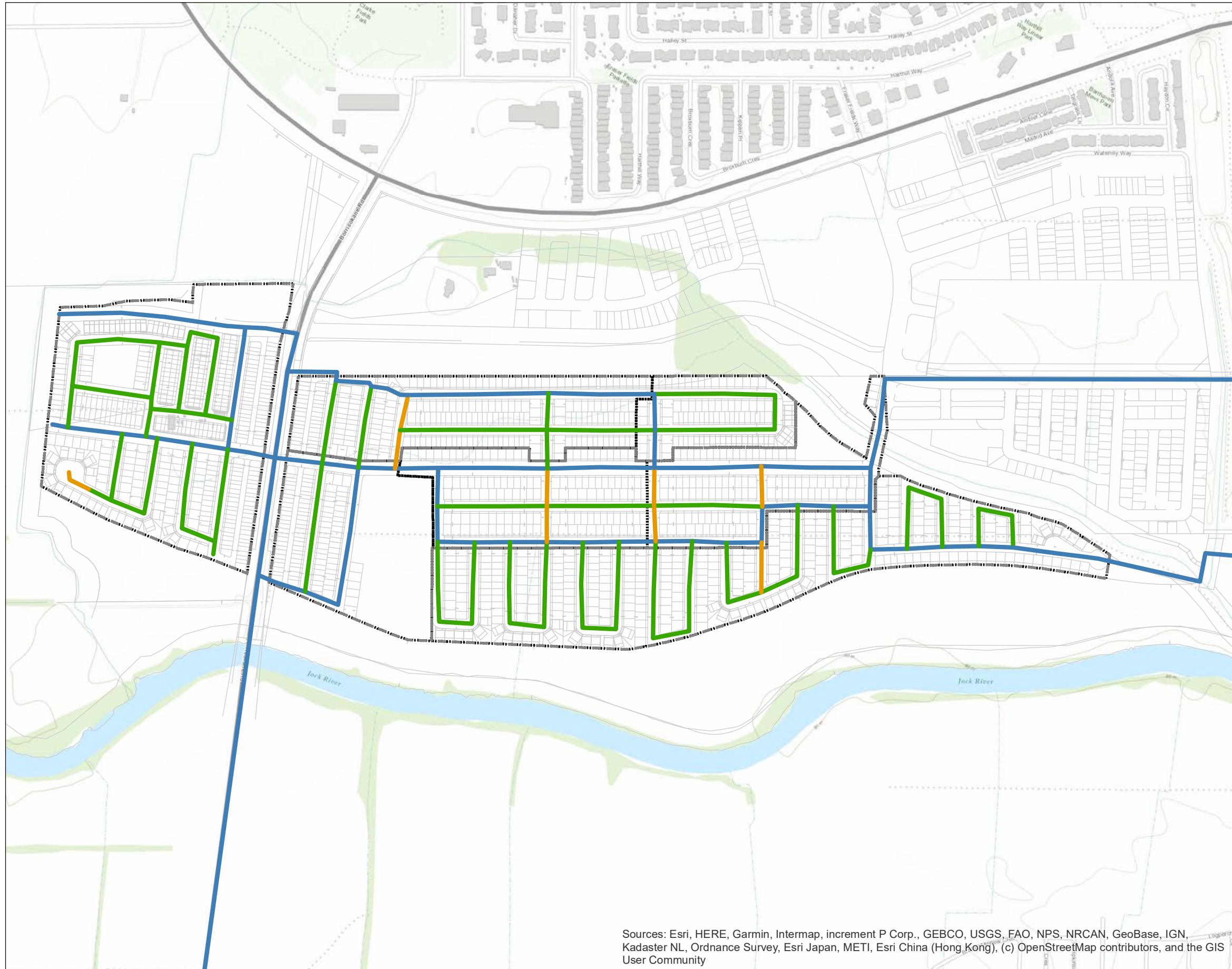
Figure No.

2-2

Title

Proposed Watermain Sizing and Layout - Option A

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

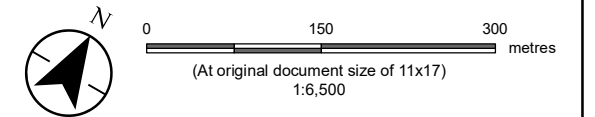


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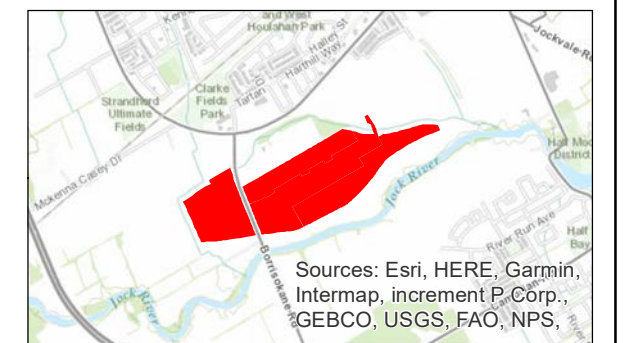
- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line

Proposed Watermain Diameter (mm)

- 152
- 203
- 305



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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 Water Distribution System Analysis

Figure No.

2-3

Title

Proposed Watermain Sizing and Layout - Option B

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Hydraulic Modelling Results
June 2, 2022

3.0 HYDRAULIC MODELLING RESULTS

Hydraulic modelling was completed for interim phasing conditions and ultimate buildout conditions of the development lands, under SUC servicing conditions, to verify how the network would respond. The following sub-sections present the modelling results under AVDY, PKHR, and MXDY+FF demands, plus under emergency conditions in the event of a watermain break at key points within the proposed network. Detailed modelling results for all scenarios are provided in **Appendix D**.

3.1 AVERAGE DAY & PEAK HOUR DEMANDS

Under AVDY demands with two connections to the water distribution system, maximum modelled pressures for each interim phase and buildout conditions are 82 psi. With three connections to the water distribution system, maximum modelled pressures for each interim phase and buildout conditions are 81 to 82 psi. These maximum pressures exceed the City's maximum pressure objective of 80 psi. As per the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 80 psi. Where pressures do exceed 80 psi, pressure control measures such as pressure reducing valves (PRVs) shall be considered.

Under PKHR demands with two connections to the water distribution system, minimum modelled pressures for each interim phase and buildout conditions are 72 psi. With three connections to the water distribution system, minimum modelled pressures for each interim phase and buildout conditions are also 72 psi. These pressures fall within the desired pressure range of 50 to 80 psi.

3.2 MAXIMUM DAY PLUS FIRE FLOW

MXDY+FF demands were applied for the two connections and the three connections scenarios. Each phase was included sequentially to verify network response as the development phases are constructed and occupied.

With two connections to the water distribution system, available fire flow throughout each interim phase and buildout conditions were above the required 13,000 L/min throughout the network. Likewise, with three connections to the water distribution system, available fire flow throughout each interim phase and buildout conditions were above the required 13,000 L/min throughout the network. To optimize the sizing of the dead-end watermain in the Phase 4 cul-de-sac and to reduce potential water quality issues associated with a large-diameter dead-end watermain, the alternative procedure outlined in Appendix I of ISDTB-2018-02 was employed, as described in **Section 2.4.2**. As such, placing two Class AA hydrants within 75 m of the furthest unit along the cul-de-sac (each with an assumed maximum flow capacity of 5,700 L/min), and two Class AA hydrants between 75 m and 150 m (each with an assumed maximum flow capacity of 3,800 L/min), satisfies the fire flow requirement of 13,000 L/min for units along this cul-de-



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Hydraulic Modelling Results
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sac. Other hydrant spacing combinations in accordance with the City's Technical Bulletin ISDTB-2018-02 may be implemented to achieve a total fire flow greater than the RFF of 13,000 L/min.

These results show that the proposed watermain sizing and layout meet serviceability requirements with two connections to the water distribution system without requiring further upstream upgrades along Danson Gardens Grv. Nonetheless, these upgrades would still be beneficial, as they would provide consistent HGLs between the two connection points.

3.3 RELIABILITY

As per the City of Ottawa Design Guidelines, the system must be able to provide average day demand plus fire flow (AVDY+FF) while meeting serviceability requirements during a major failure (i.e., watermain break). To assess reliability and resiliency against major failures, a number of reliability scenarios were completed to confirm sufficient pressure and flow can be achieved during a major failure. These scenarios included the following and are shown in **Figure 3-1** (for Option A, with two connections) and in **Figure 3-2: Reliability Analysis Watermain Break Locations – Option B**

(for Option B, with three connections):

- 1) **Break Scenario 1** – Break in the backbone watermain from Connection 1;
- 2) **Break Scenario 2** – Break in the backbone watermain from Connection 2;
- 3) **Break Scenario 3** – Break in backbone watermain at the northwest edge of the large Phase 2 (Phase 2C) park;
- 4) **Break Scenario 4** – Break along the east-west backbone watermain, immediately west of Borrisokane Rd;
- 5) **Break Scenario 5** – Break in the south backbone watermain through Phase 2 lands, immediately east of Borrisokane Rd;
- 6) **Break Scenario 6** – Break in the north backbone watermain through Phase 2 lands, immediately west of Borrisokane Rd; and,
- 7) **Break Scenario 7** (for Option B only) – Break in the backbone watermain from Connection 3 (crossing the Jock River).

Under break scenario 1, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 2, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Hydraulic Modelling Results
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Under break scenario 3, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections). However, under the current phasing plan, a second feed would still be required to service the Phase 2 service area west of the park area (sub-phase 2C; containing more than 50 properties) in the interim until sufficient looping can be provided through the subsequent Phase 3 (sub-phase 2E) if current phasing plans remain unchanged. It is thus recommended that sub-phase 2C be developed after sub-phase 2E, to provide sufficient looping. However, some alternatives could be considered to proceed with sub-phase 2C in accordance with the City's Design Guidelines. First, if the construction of the rear-lane townhouse units (refer to **Figure 1-2**) under sub-phase 2C are delayed until sufficient looping is provided, 55 units will remain as part of sub-phase 2C. As per the City's Design Guidelines, up to 75 units can be serviced on a temporary basis by a dead-end water (or a single feed in this case), given that all pressure and demand objectives are met, and it will be looped by a future phase within 2 years. As such, the 55 units could be serviced from the single feed, given that a second loop is provided in a timely matter (2 years). Alternatively, delaying the construction of 6 other units, in addition to the rear-lane townhouses, to avoid the creation of a vulnerable service area (i.e., less than 50 units under sub-phase 2C until sufficient looping is provided) could be considered.

Under break scenario 4, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 5, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 6, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 7, all junctions meet their respective required fire flows. This break scenario would only occur if Option B (watermain layout with third connection across the Jock River) is selected.

3.4 WATER AGE

Water age is calculated as the total pipe volume divided by the AVDY demand. For sizing Option A (with two connections), water age is highest (1.56 days) when Phase 2 is built. The water age upon buildout is 0.67 days. For sizing Option B (with three connections), water age is highest (1.05 days) when Phase 2 is built. The water age upon buildout is 0.57 days.

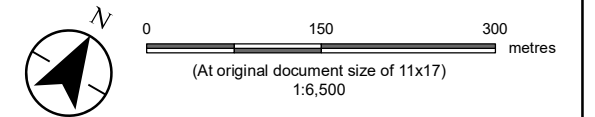
It should be noted that no water age boundary conditions at the connection points were available, therefore the total water age from the source or last point of rechlorination cannot be assessed. Nonetheless, this analysis shows that the residence time of water within the development lands does not exceed the limits per the City's Design Guidelines, and as such water age issues within the development are not anticipated.

Detailed calculations are provided in **Appendix E**.

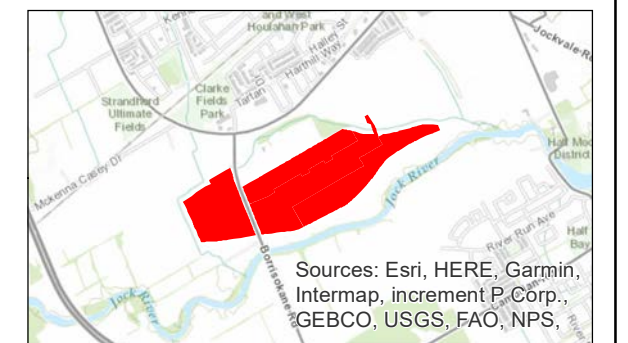


Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line
- Proposed Watermain Diameter (mm)**
- 152
- 203
- 305
- 406
- Watermain Break Locations



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
 Ottawa, ON

Client/Project
 David Schaeffer Engineering Ltd
 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

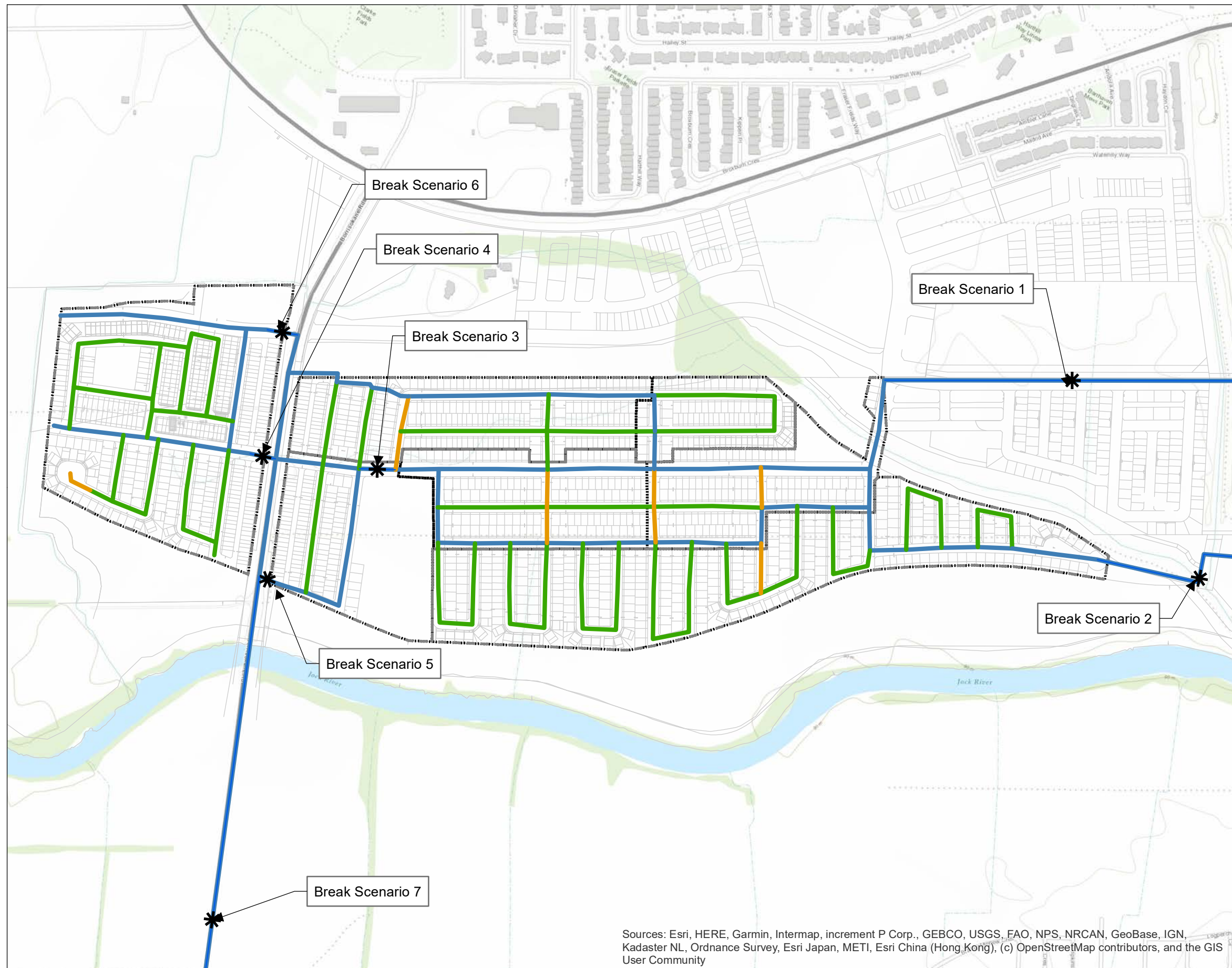
Figure No.

3-1

Title

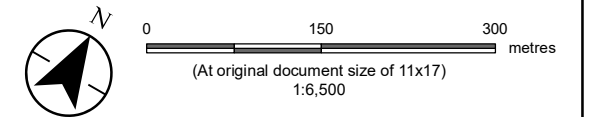
Reliability Analysis Watermain Break Locations - Option A

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

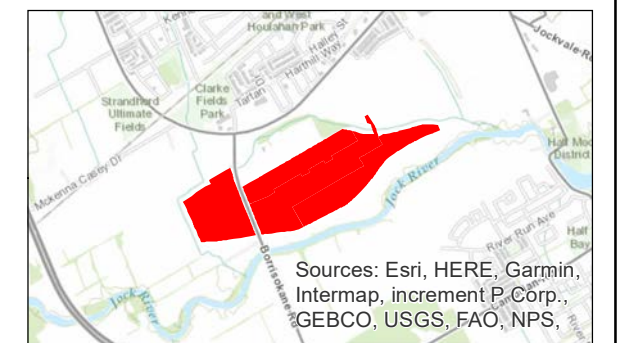


Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line
- Proposed Watermain Diameter (mm)**
- 152
- 203
- 305
- Watermain Break Locations



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
 Ottawa, ON

Client/Project
 David Schaeffer Engineering Ltd
 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.
3-2

Title
Reliability Analysis Watermain Break Locations - Option B

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Conclusion and Recommendations
June 2, 2022

4.0 CONCLUSION AND RECOMMENDATIONS

A water distribution system hydraulic analysis was completed for the Barrhaven Conservancy East development lands. The purpose of this analysis was to confirm associated watermain sizing and redundancy needs. Based on the hydraulic analysis, the following conclusions and recommendations were made:

- Based on the most current site plan layout, the estimated AVDY, MXDY and PKHR demand projections for the development lands are 13.92 L/s, 34.80 L/s, and 76.55 L/s, respectively.
- The required fire flow for the governing unit design (rear-lane townhouses) was calculated to be 13,000 L/min (217 L/s). This is based on the understanding that ordinary construction single-family housing (SFH) units will be used to separate SFH blocks into fire areas that result in required fire flows (RFFs) no greater than 13,000 L/min. Similarly, townhouse blocks will have firewalls to limit fire areas such that the resulting RFFs will be no greater than 13,000 L/min.
 - As the watermain sizing presented herein is based on an RFF of 13,000 L/min, the final design of the units should meet the requirements for this RFF, per the FUS Guidelines.
- Two watermain layout and sizing are proposed:
 - The first option (Option A) would involve two connections to the water distribution system; the proposed sizing is recommended to include 406 mm diameter watermains from connections 1 and 2 as the hydraulic backbone of the network, with 305 mm diameter watermains for backbone looping. Watermains along local right-of-ways would be 152 mm and 203 mm diameter watermains. The dead-end watermain in the cul-de-sac at the western extent of the development would be 152 mm diameter. The proposed layout and sizing is shown in **Figure 2-2**.
 - The second option (Option B) would involve three connections to the water distribution system, with the third connection requiring crossing the Jock River. The proposed layout within the development is similar to Option A; the proposed sizing is recommended to include 305 mm diameter watermains for the hydraulic backbone of the network. Watermains along local right-of-ways would be 152 mm and 203 mm diameter watermains. The dead-end watermain in the cul-de-sac at the western extent of the development would be 152 mm diameter. The proposed layout and sizing is shown in **Figure 2-3**.
- The serviceability of the development lands was analysed, considering that they would be serviced by the pressure zone SUC.
- As part of the currently proposed watermain layout, the backbone watermain is proposed to extend west along the northern edge of the Phase 4 lands to serve as a service connection to future adjacent development. There are no direct Phase 4 property service connections



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Conclusion and Recommendations
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anticipated along this temporary dead-end section of watermain; therefore, it is recommended that this section be isolated until it is required to provide flow to adjacent future lands.

- Under AVDY demand conditions, model results using boundary conditions provided by the City exceed the allowable maximum pressure of 80 psi in accordance with the City of Ottawa Design Guidelines. As per the OBC, the static pressure at any fixture shall not exceed 80 psi, in areas that may be occupied. Where pressures do exceed 80 psi, pressure control measures such as PRVs installed immediately downstream of the isolation valve to the home/building shall be considered.
- Under PKHR demand conditions, the minimum pressures are in accordance with the City's system pressure requirements.
- Under MXDY+FF demand conditions, the target required fire flow of 13,000 L/min can be achieved through the proposed network for all phases when the alternative procedure outlined in the Appendix I of ISDTB-2018-02 is applied to the dead-end watermain in the western extent of the development lands.
- If Option A (servicing with two connections) is selected, watermain upgrades along Danson Gardens Grv are recommended to provide similar HGLs at the two connection points under a fire flow scenario.
- To satisfy and improve system reliability in the event of an emergency break scenario at key points in the network, a second backbone feed was redirected north then west (i.e., north of the standard and rear-lane townhouse blocks) across to the Phase 4 lands. However, under the current phasing plan, a second feed would still be required to service the Phase 2 service area west of the park area (sub-phase 2C, containing more than 50 properties) in the interim until sufficient looping can be provided through the subsequent Phase 3 (sub-phase 2E) if current phasing plans remain unchanged. It is recommended that sub-phase 2C be developed after sub-phase 2E, to provide sufficient looping. However, sub-phase 2C could be serviced on an interim basis, from the single feed, given that the rear-lane townhouses proposed under sub-phase 2C are delayed until sufficient looping is provided. Alternatively, delaying the construction of 6 other units, in addition to the rear-lane townhouses, could be considered for sub-phase 2C, to avoid the creation of a vulnerable service area.
- A water age analysis shows that the residence time of water within the development lands does not exceed the limits per the City's Design Guidelines. No water age boundary conditions at the connection points were available, therefore the total water age from the source or last point of rechlorination cannot be assessed. As such, water age issues within the development are not anticipated.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

References
June 2, 2022

5.0 REFERENCES

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BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix A FUS CALCULATION





FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: November 29, 2021
 Data inputted by: Jasmin Sidhu, P.Eng.
 Data reviewed by:

Fire Flow Calculation #: 1
 Building Type/Description/Name: Residential

*Single family house (SFH) block based on draft site plan dated September 20, 2021. Area assumes largest SFH unit size.
 Minimum spatial separation between the backs of adjacent units is <10m (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).
 Notes: Assumed wood frame construction.
 Target fire flow = 13,000 L/min. Requires a maximum of 2 adjacent consecutive wood frame construction units.*

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method

Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
Framing Material									
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Wood Frame	1.5	Wood Frame	1.5	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
			Fire resistive construction (> 2 hrs)	0.6					
Floor Space Area									
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Single Family	2	Single Family	2	Units		
			Townhouse - indicate # of units	1					
			Other (Comm, Ind, Apt etc.)	1					
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			158	158	Area in Square Meters (m ²)		
					Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			632	632			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						8,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	6,800	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.75	m	5,100	
			Side Yard (Left)	0 to 3.0m	0.25				
			Rear Yard	3.1 to 10.0m	0.2				
			Side Yard (Right)	0 to 3.0m	0.25				
			Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow (above) in L/s:							200
		Required Duration of Fire Flow (hrs)							2.50
		Required Volume of Fire Flow (m³)							1,800



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: November 29, 2021
 Data inputted by: Jasmin Sidhu, P.Eng.
 Data reviewed by:

Fire Flow Calculation #: 2
 Building Type/Description/Name: Residential

Single family house (SFH) block based on draft site plan dated September 20, 2021. Area assumes largest SFH unit size.
 Notes: *Minimum spatial separation between the backs of adjacent units is <10m (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).*
Assumed ordinary construction (i.e., more than 2/3 of the buildings' exterior walls are made of brick or masonry veneer).
Target fire flow = 13,000 L/min. Requires a maximum of 5 adjacent consecutive ordinary construction units.

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method

Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material						
			Wood Frame	1.5	Ordinary construction	1	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area						
			Single Family	5	Single Family	5	Units		
			Townhouse - indicate # of units	1					
			Other (Comm, Ind, Apt etc.)	1					
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			158	158	Area in Square Meters (m ²)		
					Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			1,580	1,580			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.75	m	5,738	
			Side Yard (Left)	0 to 3.0m	0.25				
			Rear Yard	3.1 to 10.0m	0.2				
			Side Yard (Right)	0 to 3.0m	0.25				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						13,000	
		Total Required Fire Flow (above) in L/s:						217	
		Required Duration of Fire Flow (hrs)						2.75	
		Required Volume of Fire Flow (m³)						2,145	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: May 10, 2022
 Data inputted by: Christène Razafimaharo
 Data reviewed by: Jasmin Sidhu, P.Eng.

Fire Flow Calculation #: 3
 Building Type/Description/Name: Residential

*Notes: Maximum GFA for a STND TH fire area to achieve a required fire flow (RFF) of 13,000 L/min.
 Target fire flow = 13,000 L/min. Fire walls required at the back to achieve RFF.*

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method									
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Framing Material							
		Coefficient related to type of construction (C)	Wood Frame	1.5	Wood Frame	1.5	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Floor Space Area							
		Type of Housing	Single Family	1	Townhouse - indicate # of units	5	Units		
			Townhouse - indicate # of units	5					
			Other (Comm, Ind, Apt etc.)	1					
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			82	82	Area in Square Meters (m ²)		
					Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			820	820			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.6	m	4,590	
			Side Yard (Left)	3.1 to 10.0m	0.2				
			Rear Yard	Fire Wall	0.1				
			Side Yard (Right)	3.1 to 10.0m	0.2				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						12,000	
		Total Required Fire Flow (above) in L/s:						200	
		Required Duration of Fire Flow (hrs)						2.50	
		Required Volume of Fire Flow (m³)						1,800	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: September 29, 2021
 Data inputted by: Tom Westwood, P.Eng
 Data reviewed by: Jasmin Sidhu, P.Eng.

Fire Flow Calculation #: 4
 Building Type/Description/Name: Residential

Notes: Governing rear lane townhouse (RLTH) block based on draft site plan dated September 20, 2021. Block consists of 5 RLTH units where minimum spatial separation between the backs of adjacent units is <10m and gross floor area of block is > 600m² (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method									
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material						
			Wood Frame	1.5	Wood Frame	1.5	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area						
			Single Family	1	Townhouse - indicate # of units	5	Units		
			Townhouse - indicate # of units	5					
Other (Comm, Ind, Apt etc.)	1								
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			78	78	Area in Square Meters (m ²)		
		Square Metres (m2)							
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			780	780			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.7	m	5,355	
			Side Yard (Left)	3.1 to 10.0m	0.2				
			Rear Yard	3.1 to 10.0m	0.2				
			Side Yard (Right)	3.1 to 10.0m	0.2				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						13,000	
		Total Required Fire Flow (above) in L/s:						217	
		Required Duration of Fire Flow (hrs)						2.75	
		Required Volume of Fire Flow (m³)						2,145	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: September 29, 2021
 Data inputted by: Tom Westwood, P.Eng
 Data reviewed by: Jasmin Sidhu, P.Eng.

Fire Flow Calculation #: 5
 Building Type/Description/Name: Residential

*Notes: Governing back-to-back townhouse (B2B TH) block based on draft site plan dated September 20, 2021. Block consists of 10 B2B TH units separated by one fire wall, resulting in a fire area comprising 5 units with no spatial separation between the backs of adjacent units and gross floor area of block is > 600m² (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).
 Target Fire Flow = 13,000 L/min. Fire walls required at the back to achieve target fire flow.*

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method								
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Wood Frame	1.5	Wood Frame	1.5	m	
			Ordinary construction	1				
			Non-combustible construction	0.8				
			Fire resistive construction (< 2 hrs)	0.7				
			Fire resistive construction (> 2 hrs)	0.6				
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Single Family	1	Townhouse - indicate # of units	5	Units	
			Townhouse - indicate # of units	5				
			Other (Comm, Ind, Apt etc.)	1				
			2.2	# of Storeys				
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			78	78	Area in Square Meters (m ²)	
		Square Metres (m2)						
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			780	780		
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning						
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650
			Limited combustible	-0.15				
			Combustible	0				
			Free burning	0.15				
			Rapid burning	0.25				
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0
			None	0				
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0
			Water supply is not standard or N/A	0				
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0
			Sprinkler not fully supervised or N/A	0				
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	3.1 to 10.0m	0.2	0.6	m	4,590
			Side Yard (Left)	20.1 to 30.1m				
			Rear Yard	3.1 to 10.0m				
			Side Yard (Right)	20.1 to 30.1m				
				0.1				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						12,000
		Total Required Fire Flow (above) in L/s:						200
		Required Duration of Fire Flow (hrs)						2.50
		Required Volume of Fire Flow (m³)						1,800

**BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION
SYSTEM ANALYSIS**

June 2, 2022

Appendix B BOUNDARY CONDITIONS



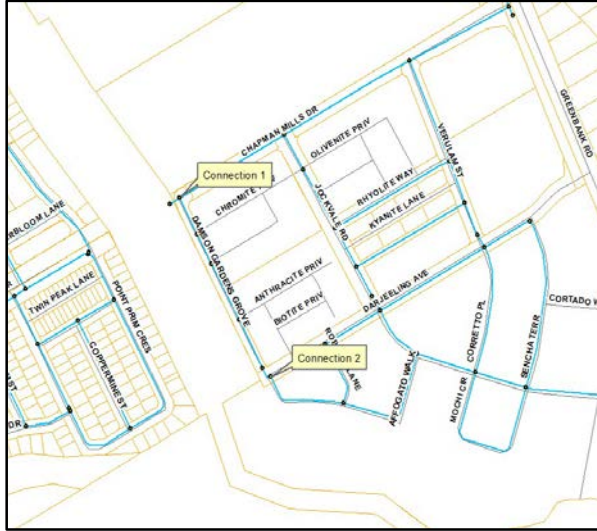
Boundary Conditions Barrhaven Conservancy East

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	832	13.87
Maximum Daily Demand	2,080	34.67
Peak Hour	4,576	76.27
Fire Flow Demand #1	13,000	216.67

Location





Results – Existing Conditions BSDY

Connection 1 – Chapman Mills Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.6	70.7
Basic Day plus Fire 1	131.2	54.5

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	95.1
Peak Hour	142.5	72.1
Basic Day plus Fire 1	127.6	50.9

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.6	94.5
Peak Hour	142.1	71.1
Basic Day plus Fire 1	129.9	53.7

Ground Elevation = 92.1 m

Results – Existing Conditions MXDY

Connection 1 – Chapman Mills Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	91.2
Peak Hour	137.6	63.7
Max Day plus Fire 1	140.1	67.2

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	92.7
Peak Hour	137.6	65.0
Max Day plus Fire 1	136.4	63.4

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	156.8	91.9
Peak Hour	136.9	63.7
Max Day plus Fire 1	137.7	64.8

Ground Elevation = 92.1 m

Results – SUC Zone Reconfiguration BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	80.7
Peak Hour	147.3	77.5
Basic Day plus Fire 1	138.6	65.1

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	82.1
Peak Hour	147.3	78.9
Basic Day plus Fire 1	135.1	61.5

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	81.5
Peak Hour	146.9	77.9
Basic Day plus Fire 1	137.4	64.3

Ground Elevation = 92.1 m

Results – SUC Zone Reconfiguration MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	78.3
Peak Hour	144.5	73.4
Max Day plus Fire 1	137.1	62.9

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	79.7
Peak Hour	144.4	74.7
Max Day plus Fire 1	133.4	59.1

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.5	78.8
Peak Hour	142.0	71.0
Max Day plus Fire 1	134.8	60.6

Ground Elevation = 92.1 m

Notes

1. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
 - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

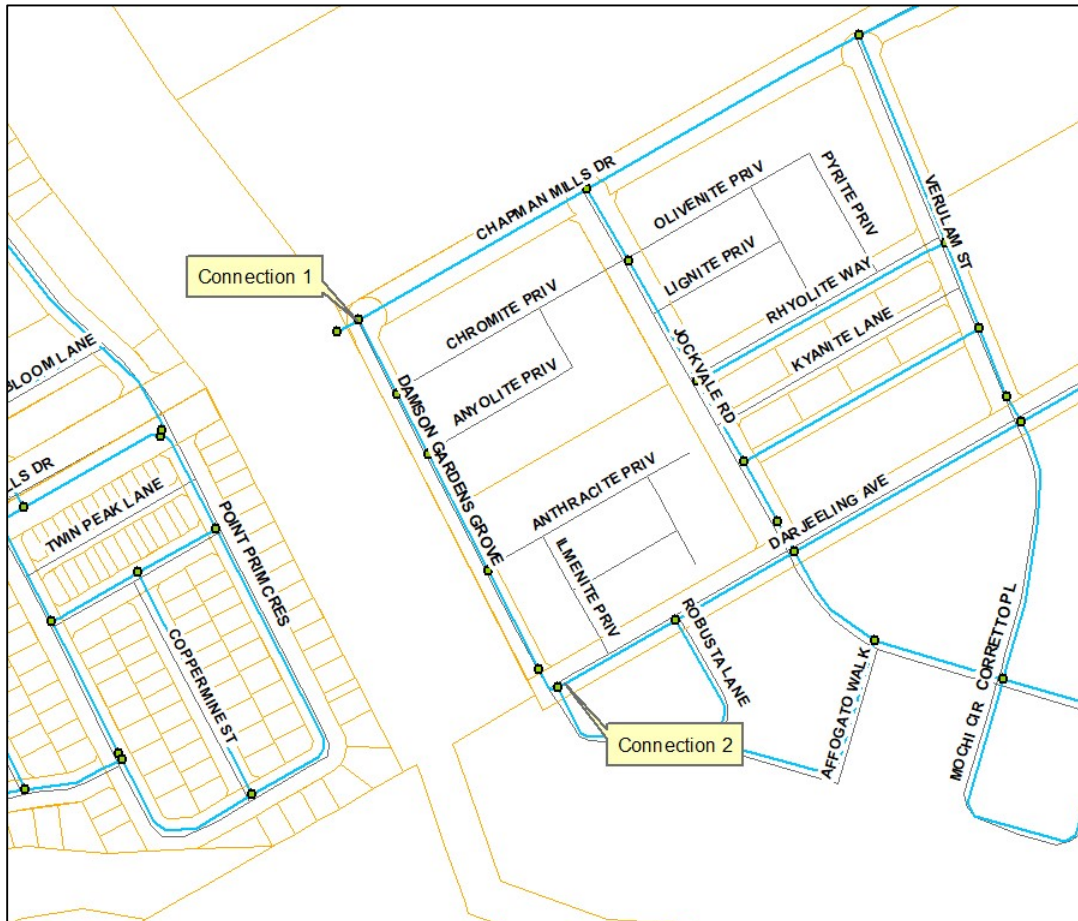
The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Boundary Conditions Barrhaven Conservancy East

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	832	13.87
Maximum Daily Demand	2,080	34.67
Peak Hour	4,576	76.27
Fire Flow Demand #1	13,000	216.67

Location



Scenario 1 Results – Existing Conditions BSDY

Connection 1 – Chapman Mills Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.5	70.7
Basic Day plus Fire 1	131.0	54.2

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	95.1
Peak Hour	142.5	72.1
Basic Day plus Fire 1	127.4	50.6

Ground Elevation = 91.8 m

Scenario 1 Results – Existing Conditions MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	91.2
Peak Hour	137.5	63.5
Max Day plus Fire 1	139.6	66.5

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	92.6
Peak Hour	137.5	64.9
Max Day plus Fire 1	135.8	62.5

Ground Elevation = 91.8 m

Scenario 1 Results – SUC Zone Reconfiguration BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	150.0	81.3
Peak Hour	147.6	77.9
Basic Day plus Fire 1	138.7	65.2

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	150.0	82.8
Peak Hour	147.6	79.2
Basic Day plus Fire 1	135.1	61.5

Ground Elevation = 91.8 m

Scenario 1 Results – SUC Zone Reconfiguration MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	148.6	79.3
Peak Hour	144.2	73.0
Max Day plus Fire 1	137.0	62.8

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	148.6	80.7
Peak Hour	144.0	74.2
Max Day plus Fire 1	133.2	58.8

Ground Elevation = 91.8 m

Scenario 2 Results – Existing Conditions BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.5	70.7
Basic Day plus Fire 1	131.2	54.5

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.5	70.7
Basic Day plus Fire 1	132.4	57.6

Ground Elevation = 91.8 m

Scenario 2 Results – Existing Conditions MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	91.2
Peak Hour	137.5	63.6
Max Day plus Fire 1	139.9	66.9

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	92.6
Peak Hour	137.5	65.0
Max Day plus Fire 1	141.1	70.1

Ground Elevation = 91.8 m

Scenario 2 Results – SUC Zone Reconfiguration BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	80.7
Peak Hour	147.3	77.5
Basic Day plus Fire 1	138.6	65.1

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	80.7
Peak Hour	147.3	77.5
Basic Day plus Fire 1	139.8	68.2

Ground Elevation = 91.8 m

Scenario 2 Results – SUC Zone Reconfiguration MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	78.3
Peak Hour	144.1	72.9
Max Day plus Fire 1	136.8	62.6

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	79.7
Peak Hour	144.1	74.3
Max Day plus Fire 1	138.1	65.8

Ground Elevation = 91.8 m

Notes

1. The watermain on Darjeeling Ave. was upsized to a 300mm diameter pipe between Danson Gardens Grove and Jockvale Road during Scenario 2 for modelling purposes.
2. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
 - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.






BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

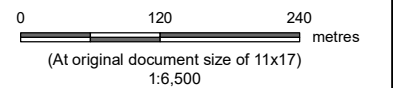
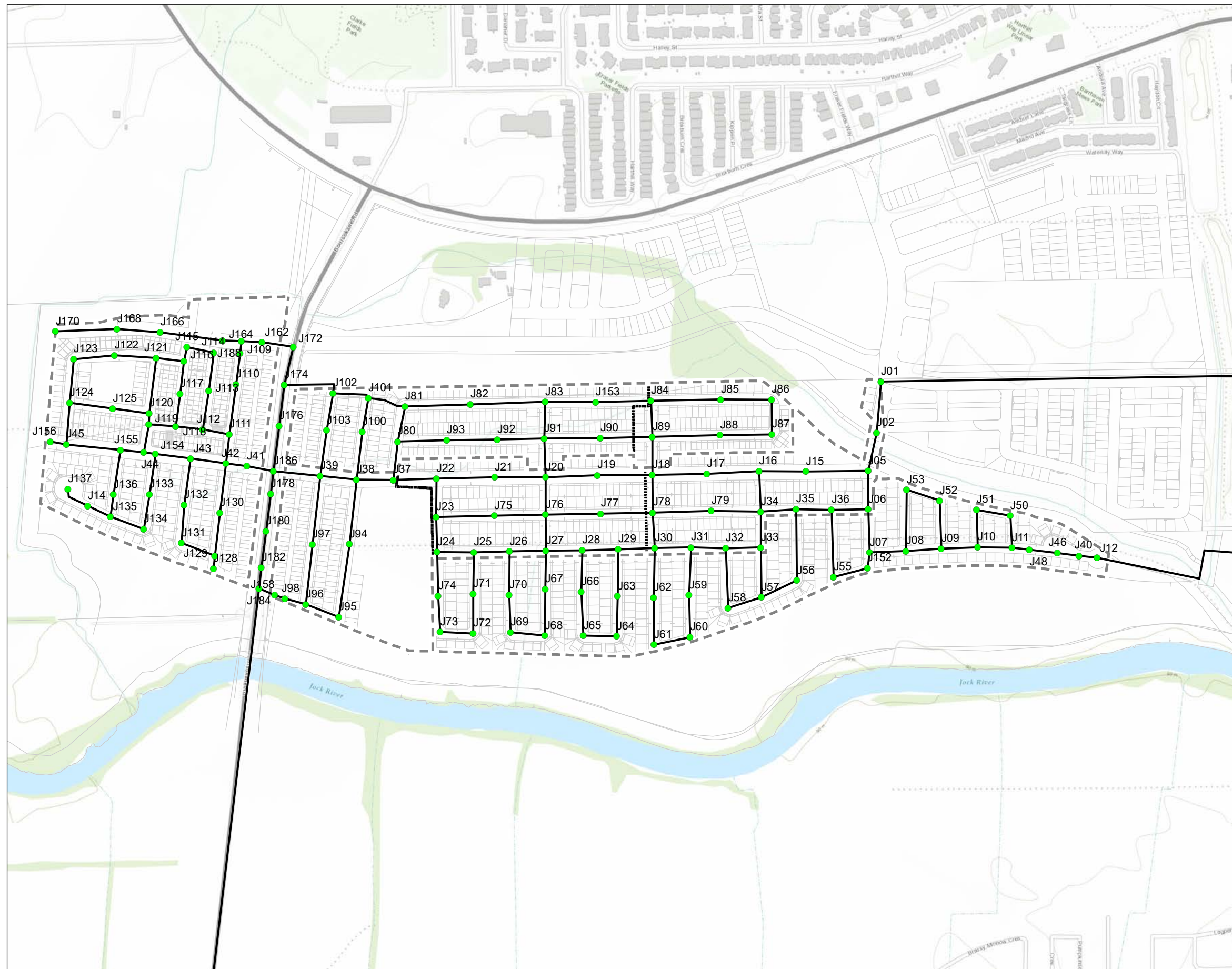
June 2, 2022

Appendix C JUNCTION IDS

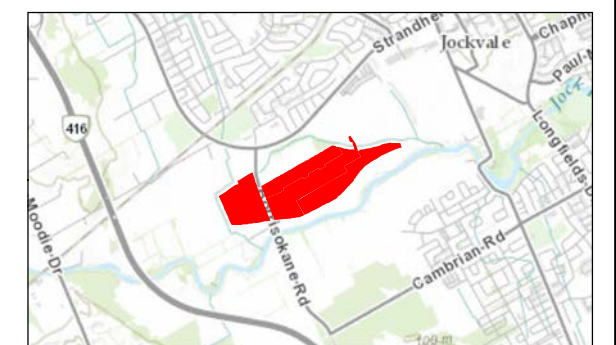


Legend

-  Development Phase Boundary
-  Development Sub-Phase Boundary
-  Property Line
-  Future Watermain
-  Model Junction



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
 Ottawa, ON

Client/Project
 David Schaeffer Engineering Ltd
 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.

C1

Title

Junction IDs

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix D MODEL RESULTS



Option A - 2 Connections - Phase 2 (2A, 2B, 2C)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
<i>Maximum</i>	0.11	150.00	81.74	<i>Maximum</i>	0.11	149.50	81.23	<i>Maximum</i>	0.11	150.00	81.73
<i>Minimum</i>	0.00	150.00	80.32	<i>Minimum</i>	0.00	149.50	79.61	<i>Minimum</i>	0.00	149.99	80.31
J01	0.07	150.00	80.32	J01	0.07	149.50	79.61	J01	0.07	150.00	80.31
J02	0.00	150.00	80.93	J02	0.00	149.50	80.22	J02	0.00	150.00	80.92
J05	0.07	150.00	81.43	J05	0.07	149.50	80.72	J05	0.07	150.00	81.42
J06	0.07	150.00	81.57	J06	0.07	149.50	80.86	J06	0.07	150.00	81.56
J07	0.11	150.00	81.73	J07	0.11	149.50	81.01	J07	0.11	150.00	81.72
J08	0.11	150.00	81.60	J08	0.11	149.50	80.89	J08	0.11	150.00	81.59
J09	0.11	150.00	81.59	J09	0.11	149.50	80.38	J09	0.11	150.00	81.08
J10	0.11	150.00	81.34	J10	0.11	149.50	80.63	J10	0.11	150.00	81.34
J11	0.11	150.00	81.23	J11	0.11	149.50	80.52	J11	0.11	150.00	81.22
J12	0.11	150.00	81.09	J12	0.11	149.50	80.38	J12	0.11	150.00	81.08
J15	0.07	150.00	81.28	J15	0.07	149.50	80.57	J15	0.07	149.99	81.28
J158	0.00	150.00	81.74	J158	0.00	149.50	81.03	J158	0.00	149.99	81.73
J16	0.07	150.00	81.44	J16	0.07	149.50	80.73	J16	0.07	149.99	81.43
J17	0.07	150.00	81.27	J17	0.07	149.50	80.56	J17	0.07	149.99	81.26
J178	0.08	150.00	81.36	J178	0.08	149.50	80.64	J178	0.08	149.99	81.35
J18	0.07	150.00	81.08	J18	0.07	149.50	80.35	J18	0.07	149.99	81.05
J180	0.08	150.00	81.57	J180	0.08	149.50	80.86	J180	0.08	149.99	81.56
J182	0.08	150.00	81.69	J182	0.08	149.50	80.97	J182	0.08	149.99	81.68
J184	0.08	150.00	81.74	J184	0.08	149.50	81.03	J184	0.08	149.99	81.73
J186	0.08	150.00	81.26	J186	0.08	149.50	80.54	J186	0.08	149.99	81.25
J19	0.10	150.00	80.84	J19	0.10	149.50	80.13	J19	0.10	149.99	80.84
J20	0.10	150.00	81.31	J20	0.10	149.50	80.60	J20	0.10	149.99	81.31
J21	0.10	150.00	81.16	J21	0.10	149.50	80.45	J21	0.10	149.99	81.15
J22	0.10	150.00	81.21	J22	0.10	149.50	80.50	J22	0.10	149.99	81.21
J23	0.10	150.00	81.34	J23	0.10	149.50	80.63	J23	0.10	149.99	81.33
J24	0.10	150.00	81.47	J24	0.10	149.50	80.76	J24	0.10	149.99	81.46
J25	0.10	150.00	81.36	J25	0.10	149.50	80.64	J25	0.10	149.99	81.35
J26	0.10	150.00	81.45	J26	0.10	149.50	80.74	J26	0.10	149.99	81.45
J27	0.10	150.00	81.58	J27	0.10	149.50	80.87	J27	0.10	149.99	81.58
J28	0.10	150.00	81.51	J28	0.10	149.50	80.80	J28	0.10	149.99	81.51
J29	0.10	150.00	81.51	J29	0.10	149.50	80.80	J29	0.10	149.99	81.51
J30	0.07	150.00	81.44	J30	0.07	149.50	80.73	J30	0.07	149.99	81.43
J31	0.07	150.00	81.57	J31	0.07	149.50	80.86	J31	0.07	149.99	81.56
J32	0.07	150.00	81.47	J32	0.07	149.50	80.76	J32	0.07	149.99	81.46
J33	0.07	150.00	81.70	J33	0.07	149.50	80.99	J33	0.07	149.99	81.69
J34	0.07	150.00	81.57	J34	0.07	149.50	80.86	J34	0.07	149.99	81.56
J35	0.07	150.00	81.53	J35	0.07	149.50	80.82	J35	0.07	149.99	81.52
J36	0.07	150.00	81.65	J36	0.07	149.50	80.94	J36	0.07	149.99	81.65
J37	0.08	150.00	81.13	J37	0.08	149.50	80.42	J37	0.08	149.99	81.12
J38	0.08	150.00	81.26	J38	0.08	149.50	80.54	J38	0.08	149.99	81.25
J39	0.08	150.00	81.29	J39	0.08	149.50	80.54	J39	0.08	149.99	81.25
J40	0.11	150.00	81.12	J40	0.11	149.50	80.41	J40	0.11	150.00	81.11
J46	0.11	150.00	81.16	J46	0.11	149.50	80.45	J46	0.11	150.00	81.16
J48	0.11	150.00	81.20	J48	0.11	149.50	80.49	J48	0.11	150.00	81.20
J75	0.10	150.00	81.28	J75	0.10	149.50	80.57	J75	0.10	149.99	81.28
J76	0.10	150.00	81.45	J76	0.10	149.50	80.74	J76	0.10	149.99	81.45
J77	0.10	150.00	81.17	J77	0.10	149.50	80.46	J77	0.10	149.99	81.16
J78	0.07	150.00	81.33	J78	0.07	149.50	80.62	J78	0.07	149.99	81.32
J79	0.07	150.00	81.41	J79	0.07	149.50	80.70	J79	0.07	149.99	81.41
J94	0.08	150.00	81.51	J94	0.08	149.50	80.80	J94	0.08	149.99	81.50
J95	0.08	150.00	81.74	J95	0.08	149.50	81.03	J95	0.08	149.99	81.73
J96	0.08	150.00	81.61	J96	0.08	149.50	80.90	J96	0.08	149.99	81.60
J97	0.08	150.00	81.37	J97	0.08	149.50	80.66	J97	0.08	149.99	81.36
J98	0.00	150.00	81.54	J98	0.00	149.50	80.83	J98	0.00	149.99	81.53

Option A - 2 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.15	150.00	81.73	Maximum	0.15	149.50	81.62	Maximum	0.15	149.99	81.71
Minimum	0.00	149.99	80.32	Minimum	0.00	149.49	79.61	Minimum	0.00	149.98	80.30
J01	0.07	150.00	80.32	J01	0.07	149.50	79.61	J01	0.07	149.99	80.30
J02	0.00	150.00	80.93	J02	0.00	149.50	80.22	J02	0.00	149.99	80.91
J05	0.07	150.00	81.42	J05	0.07	149.50	80.71	J05	0.07	149.99	81.41
J06	0.07	150.00	81.57	J06	0.07	149.50	80.86	J06	0.07	149.99	81.55
J07	0.11	150.00	81.72	J07	0.11	149.50	81.01	J07	0.11	149.99	81.71
J08	0.11	150.00	81.60	J08	0.11	149.50	80.88	J08	0.11	149.99	81.58
J09	0.11	150.00	81.68	J09	0.11	149.50	80.97	J09	0.11	149.99	81.07
J10	0.11	150.00	81.34	J10	0.11	149.50	80.63	J10	0.11	149.99	81.33
J100	0.15	149.99	80.80	J100	0.15	149.49	80.08	J100	0.15	149.98	80.78
J101	0.15	149.99	80.85	J101	0.15	149.49	80.24	J101	0.15	149.98	80.93
J102	0.15	149.99	80.84	J102	0.15	149.49	80.13	J102	0.15	149.98	80.82
J103	0.15	149.99	80.68	J103	0.15	149.49	79.97	J103	0.15	149.98	80.66
J11	0.11	150.00	81.23	J11	0.11	149.50	80.52	J11	0.11	149.99	81.22
J12	0.11	150.00	81.09	J12	0.11	149.50	80.37	J12	0.11	149.99	81.08
J15	0.07	150.00	81.28	J15	0.07	149.50	80.57	J15	0.07	149.98	81.26
J153	0.15	149.99	80.77	J153	0.15	149.49	80.06	J153	0.15	149.98	80.75
J158	0.00	149.99	81.73	J158	0.00	149.49	81.02	J158	0.00	149.98	81.71
J16	0.07	150.00	81.44	J16	0.07	149.50	80.73	J16	0.07	149.98	81.42
J17	0.07	150.00	81.27	J17	0.07	149.50	80.56	J17	0.07	149.98	81.25
J174	0.15	149.99	81.02	J174	0.15	149.49	80.31	J174	0.15	149.98	81.00
J176	0.15	149.99	81.14	J176	0.15	149.49	80.43	J176	0.15	149.98	81.12
J178	0.08	149.99	81.35	J178	0.08	149.49	80.64	J178	0.08	149.98	81.33
J18	0.07	150.00	81.05	J18	0.07	149.50	80.34	J18	0.07	149.98	81.03
J180	0.08	149.99	81.56	J180	0.08	149.49	80.85	J180	0.08	149.98	81.54
J182	0.08	149.99	81.68	J182	0.08	149.49	80.97	J182	0.08	149.98	81.66
J184	0.08	149.99	81.73	J184	0.08	149.49	81.02	J184	0.08	149.98	81.71
J186	0.08	149.99	81.25	J186	0.08	149.49	80.54	J186	0.08	149.98	81.23
J19	0.10	150.00	80.84	J19	0.10	149.50	80.13	J19	0.10	149.98	80.82
J20	0.10	150.00	81.31	J20	0.10	149.50	80.60	J20	0.10	149.98	81.29
J21	0.10	150.00	81.15	J21	0.10	149.50	80.44	J21	0.10	149.98	81.13
J22	0.10	150.00	81.21	J22	0.10	149.50	80.50	J22	0.10	149.98	81.19
J23	0.10	150.00	81.34	J23	0.10	149.50	80.63	J23	0.10	149.98	81.32
J24	0.10	150.00	81.46	J24	0.10	149.50	80.75	J24	0.10	149.98	81.44
J25	0.10	150.00	81.35	J25	0.10	149.50	80.64	J25	0.10	149.98	81.33
J26	0.10	150.00	81.45	J26	0.10	149.50	80.74	J26	0.10	149.98	81.43
J27	0.10	150.00	81.58	J27	0.10	149.50	80.87	J27	0.10	149.98	81.56
J28	0.10	150.00	81.51	J28	0.10	149.50	80.80	J28	0.10	149.98	81.49
J29	0.10	150.00	81.51	J29	0.10	149.50	80.80	J29	0.10	149.98	81.49
J30	0.07	150.00	81.44	J30	0.07	149.50	80.73	J30	0.07	149.98	81.42
J31	0.07	150.00	81.56	J31	0.07	149.50	80.85	J31	0.07	149.98	81.55
J32	0.07	150.00	81.47	J32	0.07	149.50	80.75	J32	0.07	149.98	81.45
J33	0.07	150.00	81.69	J33	0.07	149.50	80.98	J33	0.07	149.98	81.68
J34	0.07	150.00	81.57	J34	0.07	149.50	80.85	J34	0.07	149.98	81.55
J35	0.07	150.00	81.52	J35	0.07	149.50	80.81	J35	0.07	149.98	81.51
J36	0.07	150.00	81.65	J36	0.07	149.50	80.94	J36	0.07	149.98	81.64
J37	0.08	149.99	81.12	J37	0.08	149.49	80.41	J37	0.08	149.98	81.10
J38	0.08	149.99	81.25	J38	0.08	149.49	80.54	J38	0.08	149.98	81.23
J39	0.08	149.99	81.25	J39	0.08	149.49	80.54	J39	0.08	149.98	81.23
J40	0.11	150.00	81.12	J40	0.11	149.50	80.40	J40	0.11	149.99	81.11
J46	0.11	150.00	81.16	J46	0.11	149.50	80.45	J46	0.11	149.99	81.15
J48	0.11	150.00	81.20	J48	0.11	149.50	80.49	J48	0.11	149.99	81.19
J75	0.10	150.00	81.28	J75	0.10	149.50	80.57	J75	0.10	149.98	81.26
J76	0.10	150.00	81.45	J76	0.10	149.50	80.74	J76	0.10	149.98	81.43
J77	0.10	150.00	81.17	J77	0.10	149.50	80.46	J77	0.10	149.98	81.15
J78	0.07	150.00	81.32	J78	0.07	149.50	80.61	J78	0.07	149.98	81.30
J79	0.07	150.00	81.41	J79	0.07	149.50	80.70	J79	0.07	149.98	81.39
J80	0.15	149.99	80.98	J80	0.15	149.49	80.27	J80	0.15	149.98	80.96
J81	0.15	149.99	80.80	J81	0.15	149.49	80.08	J81	0.15	149.98	80.78
J82	0.15	149.99	80.84	J82	0.15	149.49	80.13	J82	0.15	149.98	80.82
J83	0.15	149.99	81.05	J83	0.15	149.49	80.34	J83	0.15	149.98	81.03
J84	0.15	150.00	80.92	J84	0.15	149.50	80.21	J84	0.15	149.98	80.90
J85	0.15	149.99	81.02	J85	0.15	149.49	80.31	J85	0.15	149.98	81.00
J86	0.15	149.99	81.24	J86	0.15	149.49	80.53	J86	0.15	149.98	81.22
J87	0.15	149.99	81.19	J87	0.15	149.49	80.48	J87	0.15	149.98	81.17
J88	0.15	149.99	81.05	J88	0.15	149.49	80.34	J88	0.15	149.98	81.03
J89	0.15	150.00	81.05	J89	0.15	149.50	80.34	J89	0.15	149.98	81.03
J90	0.15	150.00	80.88	J90	0.15	149.50	80.17	J90	0.15	149.98	80.86
J91	0.15	149.99	81.18	J91	0.15	149.49	80.47	J91	0.15	149.98	81.16
J92	0.15	149.99	80.99	J92	0.15	149.49	80.28	J92	0.15	149.98	80.98
J93	0.15	149.99	80.88	J93	0.15	149.49	80.17	J93	0.15	149.98	80.86
J94	0.08	149.99	81.51	J94	0.08	149.49	80.80	J94	0.08	149.98	81.49
J95	0.08	149.99	81.73	J95	0.08	149.49	81.02	J95	0.08	149.98	81.71
J96	0.08	149.99	81.61	J96	0.08	149.49	80.90	J96	0.08	149.98	81.59
J97	0.08	149.99	81.36	J97	0.08	149.49	80.65	J97	0.08	149.98	81.34
J98	0.00	149.99	81.53	J98	0.00	149.49	80.82	J98	0.00	149.98	81.52

Option A - 2 Connections - Phase 2 (2A, 2B, 2C)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.58	144.12	73.34	Maximum	0.58	144.09	72.31	Maximum	0.58	144.07	72.15
Minimum	0.00	144.09	71.95	Minimum	0.00	144.06	71.90	Minimum	0.00	144.93	71.81
J01	0.39	144.12	71.96	J01	0.39	144.08	71.90	J01	0.39	144.01	71.81
J02	0.00	144.11	72.56	J02	0.00	144.08	72.51	J02	0.00	143.99	72.38
J05	0.39	144.10	73.05	J05	0.39	144.08	73.01	J05	0.39	143.97	72.85
J06	0.39	144.10	73.18	J06	0.39	144.08	73.15	J06	0.39	143.96	72.99
J07	0.58	144.10	73.34	J07	0.58	144.08	73.31	J07	0.58	143.96	73.15
J08	0.58	144.10	73.21	J08	0.58	144.08	73.18	J08	0.58	143.97	73.02
J09	0.58	144.10	72.70	J09	0.58	144.08	72.67	J09	0.58	143.97	72.51
J10	0.58	144.10	72.95	J10	0.58	144.08	72.93	J10	0.58	143.97	72.77
J11	0.58	144.10	72.84	J11	0.58	144.08	72.82	J11	0.58	143.97	72.66
J12	0.58	144.10	72.70	J12	0.58	144.09	72.68	J12	0.58	143.98	72.52
J15	0.39	144.10	72.90	J15	0.39	144.07	72.86	J15	0.39	143.96	72.70
J158	0.00	144.09	73.33	J158	0.00	144.06	73.30	J158	0.00	143.93	73.11
J16	0.39	144.10	73.05	J16	0.39	144.07	73.01	J16	0.39	143.95	72.85
J17	0.39	144.09	72.86	J17	0.39	144.07	72.84	J17	0.39	143.95	72.67
J178	0.44	144.09	72.85	J178	0.44	144.06	72.91	J178	0.44	143.93	72.72
J18	0.39	144.09	72.66	J18	0.39	144.07	72.62	J18	0.39	143.94	72.45
J180	0.44	144.09	73.16	J180	0.44	144.06	73.13	J180	0.44	143.93	72.94
J182	0.44	144.09	73.26	J182	0.44	144.06	73.24	J182	0.44	143.93	73.05
J184	0.44	144.09	73.33	J184	0.44	144.06	73.30	J184	0.44	143.93	73.11
J186	0.44	144.09	72.85	J186	0.44	144.06	72.81	J186	0.44	143.93	72.62
J19	0.56	144.09	72.45	J19	0.56	144.06	72.41	J19	0.56	143.94	72.23
J20	0.56	144.09	72.91	J20	0.56	144.06	72.88	J20	0.56	143.93	72.69
J21	0.56	144.09	72.75	J21	0.56	144.06	72.72	J21	0.56	143.93	72.53
J22	0.56	144.09	72.81	J22	0.56	144.06	72.77	J22	0.56	143.93	72.59
J23	0.56	144.09	72.94	J23	0.56	144.06	72.90	J23	0.56	143.93	72.72
J24	0.56	144.09	73.07	J24	0.56	144.06	73.03	J24	0.56	143.93	72.84
J25	0.56	144.09	72.95	J25	0.56	144.06	72.92	J25	0.56	143.93	72.73
J26	0.56	144.09	73.05	J26	0.56	144.06	73.01	J26	0.56	143.93	72.83
J27	0.56	144.09	73.18	J27	0.56	144.06	73.14	J27	0.56	143.93	72.96
J28	0.56	144.09	73.11	J28	0.56	144.06	73.07	J28	0.56	143.94	72.89
J29	0.56	144.09	73.11	J29	0.56	144.06	73.07	J29	0.56	143.94	72.89
J30	0.39	144.09	73.04	J30	0.39	144.06	73.00	J30	0.39	143.94	72.83
J31	0.39	144.09	73.17	J31	0.39	144.06	73.13	J31	0.39	143.94	72.96
J32	0.39	144.09	73.07	J32	0.39	144.07	73.03	J32	0.39	143.94	72.86
J33	0.39	144.09	73.30	J33	0.39	144.07	73.26	J33	0.39	143.95	73.09
J34	0.39	144.09	73.17	J34	0.39	144.07	73.14	J34	0.39	143.95	72.97
J35	0.39	144.10	73.13	J35	0.39	144.07	73.10	J35	0.39	143.95	72.93
J36	0.39	144.10	73.27	J36	0.39	144.07	73.23	J36	0.39	143.96	73.07
J37	0.44	144.09	72.72	J37	0.44	144.06	72.69	J37	0.44	143.93	72.50
J38	0.44	144.09	72.85	J38	0.44	144.06	72.81	J38	0.44	143.93	72.62
J39	0.44	144.09	72.85	J39	0.44	144.06	72.81	J39	0.44	143.93	72.62
J40	0.58	144.10	72.73	J40	0.58	144.08	72.71	J40	0.58	143.97	72.55
J46	0.58	144.10	72.77	J46	0.58	144.08	72.75	J46	0.58	143.97	72.59
J48	0.58	144.10	72.81	J48	0.58	144.08	72.79	J48	0.58	143.97	72.63
J75	0.56	144.09	72.86	J75	0.56	144.06	72.84	J75	0.56	143.93	72.66
J76	0.56	144.09	73.05	J76	0.56	144.06	73.02	J76	0.56	143.93	72.83
J77	0.56	144.09	72.77	J77	0.56	144.06	72.73	J77	0.56	143.94	72.55
J78	0.39	144.09	72.93	J78	0.39	144.06	72.89	J78	0.39	143.94	72.71
J79	0.39	144.09	73.01	J79	0.39	144.07	72.98	J79	0.39	143.94	72.81
J94	0.44	144.09	73.11	J94	0.44	144.06	73.07	J94	0.44	143.93	72.88
J95	0.44	144.09	73.33	J95	0.44	144.06	73.30	J95	0.44	143.93	73.11
J96	0.44	144.09	73.21	J96	0.44	144.06	73.17	J96	0.44	143.93	72.98
J97	0.44	144.09	72.96	J97	0.44	144.06	72.93	J97	0.44	143.93	72.74
J98	0.00	144.09	73.14	J98	0.00	144.06	73.10	J98	0.00	143.93	72.91

Option A - 2 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.53	144.04	72.12	Maximum	0.53	144.05	72.25	Maximum	0.53	143.88	72.89
Minimum	0.00	143.94	71.84	Minimum	0.00	143.98	71.85	Minimum	0.00	143.64	71.58
J01	0.39	144.04	71.84	J01	0.39	144.04	71.85	J01	0.39	143.85	71.58
J02	0.00	144.01	72.42	J02	0.00	144.04	72.45	J02	0.00	143.81	72.13
J05	0.39	144.00	72.80	J05	0.39	144.03	72.94	J05	0.39	143.77	72.58
J06	0.39	144.00	73.04	J06	0.39	144.03	73.09	J06	0.39	143.77	72.71
J07	0.58	144.00	73.19	J07	0.58	144.04	73.25	J07	0.58	143.78	72.89
J08	0.58	144.00	73.06	J08	0.58	144.04	73.13	J08	0.58	143.80	72.78
J09	0.58	144.00	72.55	J09	0.58	144.04	72.65	J09	0.58	143.81	72.28
J10	0.58	144.00	72.81	J10	0.58	144.05	72.88	J10	0.58	143.82	72.56
J100	0.83	143.94	72.20	J100	0.83	143.98	72.24	J100	0.83	143.65	71.77
J101	0.83	143.94	72.35	J101	0.83	143.98	72.40	J101	0.83	143.65	71.93
J102	0.83	143.94	72.24	J102	0.83	143.98	72.29	J102	0.83	143.64	71.81
J103	0.83	143.94	72.08	J103	0.83	143.98	72.13	J103	0.83	143.64	71.65
J11	0.58	144.00	72.70	J11	0.58	144.05	72.77	J11	0.58	143.84	72.47
J12	0.58	144.00	72.55	J12	0.58	144.06	72.65	J12	0.58	143.86	72.38
J15	0.39	143.99	72.74	J15	0.39	144.02	72.78	J15	0.39	143.74	72.39
J153	0.83	143.95	72.17	J153	0.83	143.98	72.22	J153	0.83	143.65	71.75
J158	0.00	143.95	73.13	J158	0.00	143.98	73.18	J158	0.00	143.64	72.71
J16	0.39	143.98	72.86	J16	0.39	144.01	72.93	J16	0.39	143.72	72.51
J17	0.39	143.97	72.70	J17	0.39	144.00	72.74	J17	0.39	143.69	72.30
J174	0.83	143.94	72.42	J174	0.83	143.98	72.47	J174	0.83	143.64	72.00
J176	0.83	143.95	72.54	J176	0.83	143.98	72.59	J176	0.83	143.64	72.11
J178	0.44	143.95	72.75	J178	0.44	143.98	72.83	J178	0.44	143.64	72.32
J18	0.39	143.96	72.47	J18	0.39	143.99	72.52	J18	0.39	143.67	72.05
J180	0.44	143.95	72.96	J180	0.44	143.98	73.01	J180	0.44	143.64	72.54
J182	0.44	143.95	73.08	J182	0.44	143.98	73.13	J182	0.44	143.64	72.65
J184	0.44	143.95	72.81	J184	0.44	143.98	72.86	J184	0.44	143.64	72.71
J186	0.44	143.95	72.65	J186	0.44	143.98	72.70	J186	0.44	143.64	72.22
J19	0.56	143.95	72.25	J19	0.56	143.99	72.30	J19	0.56	143.66	71.83
J20	0.56	143.95	72.72	J20	0.56	143.99	72.77	J20	0.56	143.66	72.30
J21	0.56	143.95	72.55	J21	0.56	143.98	72.61	J21	0.56	143.66	72.14
J22	0.56	143.95	72.61	J22	0.56	143.98	72.66	J22	0.56	143.65	72.20
J23	0.56	143.95	72.74	J23	0.56	143.98	72.79	J23	0.56	143.66	72.33
J24	0.56	143.95	72.87	J24	0.56	143.98	72.92	J24	0.56	143.66	72.46
J25	0.56	143.95	72.76	J25	0.56	143.98	72.81	J25	0.56	143.66	72.35
J26	0.56	143.95	72.86	J26	0.56	143.99	72.91	J26	0.56	143.66	72.45
J27	0.56	143.95	72.99	J27	0.56	143.99	73.04	J27	0.56	143.67	72.58
J28	0.56	143.95	72.92	J28	0.56	143.99	72.97	J28	0.56	143.67	72.52
J29	0.56	143.96	72.92	J29	0.56	143.99	72.97	J29	0.56	143.68	72.52
J30	0.39	143.96	72.86	J30	0.39	143.99	72.90	J30	0.39	143.68	72.46
J31	0.39	143.96	72.99	J31	0.39	144.00	73.04	J31	0.39	143.69	72.60
J32	0.39	143.97	72.89	J32	0.39	144.00	72.94	J32	0.39	143.70	72.51
J33	0.39	143.97	73.13	J33	0.39	144.00	73.17	J33	0.39	143.71	72.75
J34	0.39	143.97	73.00	J34	0.39	144.01	73.05	J34	0.39	143.72	72.64
J35	0.39	143.98	72.97	J35	0.39	144.02	73.02	J35	0.39	143.73	72.62
J36	0.39	143.99	73.11	J36	0.39	144.02	73.16	J36	0.39	143.75	72.77
J37	0.44	143.95	72.53	J37	0.44	143.98	72.57	J37	0.44	143.65	72.10
J38	0.44	143.95	72.65	J38	0.44	143.98	72.70	J38	0.44	143.65	72.23
J39	0.44	143.95	72.65	J39	0.44	143.98	72.70	J39	0.44	143.65	72.22
J40	0.58	144.00	72.58	J40	0.58	144.06	72.68	J40	0.58	143.87	72.40
J46	0.58	144.00	72.63	J46	0.58	144.06	72.71	J46	0.58	143.86	72.43
J48	0.58	144.00	72.67	J48	0.58	144.05	72.78	J48	0.58	143.84	72.45
J75	0.56	143.95	72.69	J75	0.56	143.98	72.73	J75	0.56	143.66	72.27
J76	0.56	143.95	72.85	J76	0.56	143.99	72.91	J76	0.56	143.66	72.45
J77	0.56	143.95	72.58	J77	0.56	143.99	72.63	J77	0.56	143.67	72.17
J78	0.39	143.96	72.74	J78	0.39	143.99	72.79	J78	0.39	143.68	72.34
J79	0.39	143.97	72.84	J79	0.39	144.00	72.89	J79	0.39	143.70	72.45
J80	0.83	143.94	72.38	J80	0.83	143.98	72.43	J80	0.83	143.65	71.96
J81	0.83	143.95	72.20	J81	0.83	143.98	72.24	J81	0.83	143.65	71.77
J82	0.83	143.95	72.24	J82	0.83	143.98	72.29	J82	0.83	143.65	71.82
J83	0.83	143.95	72.45	J83	0.83	143.98	72.50	J83	0.83	143.65	72.03
J84	0.80	143.95	72.33	J84	0.80	143.98	72.38	J84	0.80	143.65	71.91
J85	0.80	143.95	72.42	J85	0.80	143.98	72.47	J85	0.80	143.65	72.01
J86	0.80	143.95	72.64	J86	0.80	143.98	72.69	J86	0.80	143.65	72.22
J87	0.80	143.95	72.60	J87	0.80	143.98	72.64	J87	0.80	143.65	72.18
J88	0.80	143.95	72.45	J88	0.80	143.98	72.50	J88	0.80	143.65	72.04
J89	0.80	143.95	72.46	J89	0.80	143.99	72.51	J89	0.80	143.66	72.04
J90	0.83	143.95	72.29	J90	0.83	143.98	72.33	J90	0.83	143.65	71.87
J91	0.83	143.95	72.56	J91	0.83	143.98	72.63	J91	0.83	143.65	72.16
J92	0.83	143.95	72.40	J92	0.83	143.98	72.44	J92	0.83	143.65	71.97
J93	0.83	143.94	72.28	J93	0.83	143.98	72.33	J93	0.83	143.65	71.86
J94	0.44	143.95	72.91	J94	0.44	143.98	72.96	J94	0.44	143.65	72.48
J95	0.44	143.95	73.13	J95	0.44	143.98	73.18	J95	0.44	143.64	72.71
J96	0.44	143.95	73.01	J96	0.44	143.98	73.05	J96	0.44	143.64	72.58
J97	0.44	143.95	72.76	J97	0.44	143.98	72.81	J97	0.44	143.64	72.34
J98	0.00	143.95	72.93	J98	0.00	143.98	72.98	J98	0.00	143.64	72.51

Option A - 2 Connections - Phase 2 (2A, 2B, 2C)				
Junction ID	Base Demand (MXDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)
J01	0.18	13,000	59.93	64,000
J02	0.00	13,000	60.51	64,000
J05	0.18	13,000	61.00	64,000
J06	0.18	13,000	61.17	64,000
J07	0.27	13,000	61.40	64,000
J08	0.27	13,000	61.35	65,000
J09	0.27	13,000	60.94	65,000
J10	0.27	13,000	61.31	67,000
J11	0.27	13,000	61.33	68,000
J12	0.27	13,000	61.61	75,000
J15	0.18	13,000	60.14	55,000
J158	0.00	13,000	53.27	29,000
J16	0.18	13,000	59.88	52,000
J17	0.18	13,000	59.23	48,000
J178	0.20	13,000	54.17	31,000
J18	0.18	13,000	58.59	46,000
J180	0.20	13,000	53.53	30,000
J182	0.20	13,000	53.25	29,000
J184	0.20	13,000	53.24	29,000
J186	0.20	13,000	54.90	33,000
J19	0.25	13,000	57.93	43,000
J20	0.25	13,000	58.04	42,000
J21	0.25	13,000	57.50	40,000
J22	0.25	13,000	57.21	39,000
J23	0.25	13,000	56.74	37,000
J24	0.25	13,000	56.19	35,000
J25	0.25	13,000	55.70	34,000
J26	0.25	13,000	55.73	34,000
J27	0.25	13,000	56.06	35,000
J28	0.25	13,000	55.86	34,000
J29	0.25	13,000	55.98	34,000
J30	0.18	13,000	56.30	35,000
J31	0.18	13,000	56.47	36,000
J32	0.18	13,000	56.69	38,000
J33	0.18	13,000	57.55	39,000
J34	0.18	13,000	58.47	43,000
J35	0.18	13,000	58.79	44,000
J36	0.18	13,000	59.70	49,000
J37	0.20	13,000	56.39	37,000
J38	0.20	13,000	55.90	35,000
J39	0.20	13,000	55.42	34,000
J40	0.27	13,000	61.54	73,000
J46	0.27	13,000	61.47	71,000
J48	0.27	13,000	61.38	69,000
J75	0.25	13,000	47.82	23,000
J76	0.25	13,000	52.46	28,000
J77	0.25	13,000	47.31	23,000
J78	0.18	13,000	52.96	29,000
J79	0.18	13,000	49.39	25,000
J94	0.20	13,000	53.82	30,000
J95	0.20	13,000	53.35	29,000
J96	0.20	13,000	53.38	29,000
J97	0.20	13,000	44.98	21,000
J98	0.00	13,000	53.12	29,000

Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C)				
Junction ID	Base Demand (MXDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)
J01	0.18	13,000	62.43	66,000
J02	0.00	13,000	63.01	66,000
J05	0.18	13,000	63.51	66,000
J06	0.18	13,000	63.67	66,000
J07	0.27	13,000	63.90	66,000
J08	0.27	13,000	63.84	67,000
J09	0.27	13,000	63.43	67,000
J10	0.27	13,000	63.80	69,000
J11	0.27	13,000	63.82	70,000
J12	0.27	13,000	64.10	77,000
J15	0.18	13,000	62.64	57,000
J158	0.00	13,000	55.77	30,000
J16	0.18	13,000	62.38	53,000
J17	0.18	13,000	61.73	50,000
J178	0.20	13,000	56.67	32,000
J18	0.18	13,000	61.09	47,000
J180	0.20	13,000	56.03	30,000
J182	0.20	13,000	55.76	30,000
J184	0.20	13,000	55.74	30,000
J186	0.20	13,000	57.40	34,000
J19	0.25	13,000	60.43	45,000
J20	0.25	13,000	60.54	43,000
J21	0.25	13,000	60.00	41,000
J22	0.25	13,000	59.71	40,000
J23	0.25	13,000	59.24	38,000
J24	0.25	13,000	58.69	36,000
J25	0.25	13,000	58.20	35,000
J26	0.25	13,000	58.23	35,000
J27	0.25	13,000	58.56	36,000
J28	0.25	13,000	58.36	35,000
J29	0.25	13,000	58.48	35,000
J30	0.18	13,000	58.80	36,000
J31	0.18	13,000	58.97	37,000
J32	0.18	13,000	59.19	38,000
J33	0.18	13,000	60.05	40,000
J34	0.18	13,000	60.97	44,000
J35	0.18	13,000	61.29	46,000
J36	0.18	13,000	62.20	50,000
J37	0.20	13,000	58.89	38,000
J38	0.20	13,000	58.40	36,000
J39	0.20	13,000	57.92	35,000
J40	0.27	13,000	64.02	75,000
J46	0.27	13,000	63.96	73,000
J48	0.27	13,000	63.86	71,000
J75	0.25	13,000	50.42	24,000
J76	0.25	13,000	54.96	29,000
J77	0.25	13,000	49.81	24,000
J78	0.18	13,000	55.46	30,000
J79	0.18	13,000	51.89	25,000
J94	0.20	13,000	56.32	31,000
J95	0.20	13,000	56.85	30,000
J96	0.20	13,000	55.88	30,000
J97	0.20	13,000	47.48	22,000
J98	0.00	13,000	55.62	30,000

Option B - 3 Connections - Phase 2 (2A, 2B, 2C)				
Junction ID	Base Demand (MXDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)
J01	0.18	13,000	61.23	28,000
J02	0.00	13,000	51.51	28,000
J05	0.18	13,000	51.87	28,000
J06	0.18	13,000	52.01	28,000
J07	0.27	13,000	52.34	28,000
J08	0.27	13,000	52.46	28,000
J09	0.27	13,000	52.27	28,000
J10	0.27	13,000	52.97	29,000
J11	0.27	13,000	53.36	30,000
J12	0.27	13,000	54.93	32,000
J15	0.18	13,000	49.27	25,000
J158	0.00	13,000	33.14	16,000
J16	0.18	13,000	48.40	24,000
J17	0.18	13,000	46.84	23,000
J178	0.20	13,000	33.01	18,000
J18	0.18	13,000	45.74	22,000
J180	0.20	13,000	32.83	16,000
J182	0.20	13,000	32.86	16,000
J184	0.20	13,000	33.00	16,000
J186	0.20	13,000	33.32	16,000
J19	0.25	13,000	44.33	21,000
J20	0.25	13,000	44.19	21,000
J21	0.25	13,000	43.19	20,000
J22	0.25	13,000	42.93	20,000
J23	0.25	13,000	43.28	20,000
J24	0.25	13,000	43.20	20,000
J25	0.25	13,000	43.14	20,000
J26	0.25	13,000	43.53	21,000
J27	0.25	13,000	44.21	21,000
J28	0.25	13,000	44.36	21,000
J29	0.25	13,000	44.80	21,000
J30	0.18	13,000	45.41	22,000
J31	0.18	13,000	45.83	22,000
J32	0.18	13,000	46.25	22,000
J33	0.18	13,000	47.30	23,000
J34	0.18	13,000	48.53	24,000
J35	0.18	13,000	49.22	25,000
J36	0.18	13,000	50.43	26,000
J37	0.20	13,000	39.38	19,000
J38	0.20	13,000	36.58	17,000
J39	0.20	13,000	34.87	17,000
J40	0.27	13,000	54.54	32,000
J46	0.27	13,000	54.12	31,000
J48	0.27	13,000	53.63	30,000
J75	0.25	13,000	35.45	17,000
J76	0.25	13,000	40.42	19,000
J77	0.25	13,000	35.92	17,000
J78	0.18	13,000	42.00	20,000
J79	0.18	13,000	38.99	18,000
J94	0.20	13,000	34.35	17,000
J95	0.20	13,000	33.61	16,000
J96	0.20	13,000	33.46	16,000
J97	0.20	13,000	24.84	14,000
J98	0.00	13,000	33.06	16,000

Option B - 3 Connections - Break Scenario 7 (Connection 3 Break)				
Junction ID	Base Demand (AVDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)
J01	0.07	13,000	50.75	28,000
J02	0.00	13,000	50.85	28,000
J05	0.07	13,000	51.07	28,000
J06	0.07	13,000	51.13	28,000
J07	0.11	13,000	51.35	28,000
J08	0.11	13,000	51.37	28,000
J09	0.11	13,000	51.02	28,000
J10	0.11	13,000	51.58	28,000
J100	0.15	13,000	35.85	17,000
J101	0.15	13,000	41.53	20,000
J102	0.15	13,000	40.97	20,000
J103	0.15	13,000	34.84	17,000
J109	0.11	13,000	38.31	18,000
J11	0.11	13,000	51.74	29,000
J110	0.11	13,000	38.43	18,000
J111	0.11	13,000	38.95	19,000
J112	0.11	13,000	33.79	17,000
J113	0.11	13,000	25.70	15,000
J114	0.11	13,000	24.48	14,000
J115	0.11	13,000	27.00	15,000
J116	0.11	13,000	29.75	16,000
J117	0.11	13,000	28.09	15,000
J118	0.11	13,000	32.53	16,000
J119	0.11	13,000	33.78	17,000
J12	0.11	13,000	52.90	32,000
J120	0.11	13,000	32.39	16,000
J121	0.11	13,000	29.71	16,000
J122	0.11	13,000	23.29	14,000
J123	0.11	13,000	23.35	14,000
J124	0.11	13,000	29.83	16,000
J125	0.11	13,000	27.27	15,000
J128	0.00	6,000	51.65	13,000
J129	0.11	13,000	22.95	14,000
J130	0.11	13,000	26.54	15,000
J131	0.11	13,000	23.03	14,000
J132	0.11	13,000	26.43	15,000
J133	0.11	13,000	25.95	15,000
J134	0.11	13,000	22.85	14,000
J135	0.11	13,000	23.25	14,000
J136	0.11	13,000	25.15	14,000
J137	0.11	6,000	32.53	7,000
J14	0.11	6,000	50.15	12,000
J15	0.07	13,000	49.53	25,000
J152	0.11	13,000	46.54	23,000
J153	0.15	13,000	42.30	20,000
J154	0.11	13,000	38.63	18,000
J155	0.11	13,000	35.48	17,000
J156	0.00	13,000	31.86	16,000
J158	0.00	13,000	39.85	19,000
J16	0.07	13,000	47.76	24,000
J162	0.00	13,000	38.51	19,000
J164	0.00	13,000	36.88	18,000
J166	0.00	13,000	31.75	16,000
J168	0.00	13,000	28.63	15,000
J17	0.07	13,000	46.35	23,000
J170	0.00	13,000	23.26	14,000
J172	0.00	13,000	39.11	19,000
J174	0.15	13,000	40.56	19,000
J176	0.15	13,000	40.26	19,000
J178	0.08	13,000	40.57	19,000
J18	0.07	13,000	45.44	22,000
J180	0.08	13,000	40.03	19,000
J182	0.08	13,000	39.80	19,000
J184	0.08	13,000	39.81	19,000
J186	0.08	13,000	41.19	20,000
J188	0.00	13,000	38.32	18,000
J19	0.10	13,000	43.99	21,000
J20	0.10	13,000	44.59	22,000
J21	0.10	13,000	43.31	21,000
J22	0.10	13,000	44.32	22,000
J23	0.10	13,000	44.26	21,000
J24	0.10	13,000	44.01	21,000
J25	0.10	13,000	43.82	21,000
J26	0.10	13,000	44.10	21,000
J27	0.10	13,000	44.59	22,000
J28	0.10	13,000	44.71	22,000
J29	0.10	13,000	45.04	22,000
J30	0.07	13,000	45.65	22,000
J31	0.07	13,000	46.08	23,000
J32	0.07	13,000	46.61	23,000
J33	0.07	13,000	47.37	24,000
J34	0.07	13,000	46.16	24,000
J35	0.07	13,000	48.80	25,000
J36	0.07	13,000	50.10	26,000
J37	0.08	13,000	43.11	21,000
J38	0.08	13,000	42.37	20,000
J39	0.08	13,000	41.67	20,000
J40	0.11	13,000	52.60	31,000
J41	0.11	13,000	40.06	19,000
J42	0.11	13,000	39.51	19,000
J43	0.11	13,000	37.87	18,000
J44	0.11	13,000	36.34	18,000
J45	0.11	13,000	33.18	17,000
J46	0.11	13,000	52.29	30,000
J48	0.11	13,000	51.93	30,000
J50	0.11	13,000	44.53	22,000
J51	0.11	13,000	44.20	21,000
J52	0.11	13,000	41.04	20,000
J53	0.11	13,000	40.35	19,000
J55	0.11	13,000	42.46	20,000
J56	0.11	13,000	37.12	18,000
J57	0.11	13,000	39.08	19,000
J58	0.11	13,000	37.63	18,000
J59	0.11	13,000	33.31	16,000
J60	0.11	13,000	29.37	15,000
J61	0.11	13,000	28.96	15,000
J62	0.11	13,000	33.01	16,000
J63	0.11	13,000	32.97	16,000
J64	0.11	13,000	29.49	15,000
J65	0.11	13,000	29.56	15,000
J66	0.11	13,000	33.75	17,000
J67	0.11	13,000	33.92	17,000
J68	0.11	13,000	29.43	15,000
J69	0.11	13,000	29.46	15,000
J70	0.11	13,000	32.93	16,000
J71	0.11	13,000	32.99	16,000
J72	0.11	13,000	29.36	15,000
J73	0.11	13,000	29.53	15,000
J74	0.11	13,000	32.78	16,000
J75	0.10	13,000	35.75	17,000
J76	0.10	13,000	40.43	19,000
J77	0.10	13,000	35.66	17,000
J78	0.07	13,000	41.59	20,000
J79	0.07	13,000	38.58	18,000
J80	0.15	13,000	31.52	16,000
J81	0.15	13,000	41.51	20,000
J82	0.15	13,000	41.49	20,000
J83	0.15	13,000	42.73	21,000
J84	0.15	13,000	43.17	21,000
J85	0.15	13,000	26.43	15,000
J86	0.15	13,000	22.78	14,000
J87	0.15	13,000	22.80	14,000
J88	0.15	13,000	26.90	15,000
J89	0.15	13,000	44.16	21,000
J90	0.15	13,000	35.62	17,000
J91	0.15	13,000	42.23	20,000
J92	0.15	13,000	31.56	16,000
J93	0.15	13,000	28.36	15,000
J94	0.08	13,000	40.36	19,000
J95	0.08	13,000	39.95	19,000
J96	0.08	13,000	39.97	19,000
J97	0.08	13,000	31.50	16,000
J98	0.00	13,000	38.70	18,000

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix E WATER AGE CALCULATIONS



163401660 - Barrhaven Conservancy

Water Age Assessment

Sizing Option A		Pipe Dimensions			Average Day Demand		Water Age		
Phase	Pipe Diameter	Pipe Length	Pipe Volume		(L/s)	(m ³ /s)	(s)	(h)	(d)
	(mm)	(m)	(m ³)						
2	406	2,609	337.81		3.50	0.004	96,464	26.80	1.12
2	305	1,442	105.38		3.50	0.004	30,091	8.36	0.35
2	203	742	24.00		3.50	0.004	6,853	1.90	0.08
2	152	305	5.54		3.50	0.004	1,581	0.44	0.02
2	Total		472.73		3.50	0.004	134,989	37.50	1.56
3	406	2,609	337.81		6.64	0.007	50,906	14.14	0.59
3	305	2,326	169.96		6.64	0.007	25,612	7.11	0.30
3	203	2,001	64.75		6.64	0.007	9,757	2.71	0.11
3	152	427	7.75		6.64	0.007	1,167	0.32	0.01
3	Total		580.27		6.64	0.007	87,442	24.29	1.01
4	406	2,977	385.42		10.30	0.010	37,402	10.39	0.43
4	305	2,982	217.89		10.30	0.010	21,145	5.87	0.24
4	203	3,747	121.28		10.30	0.010	11,769	3.27	0.14
4	152	474	8.60		10.30	0.010	835	0.23	0.01
4	Total		733.20		10.30	0.010	71,151	19.76	0.82
Ultimate	406	2,977	385.42		13.92	0.014	27,691	7.69	0.32
Ultimate	305	2,982	217.89		13.92	0.014	15,654	4.35	0.18
Ultimate	203	6,024	194.98		13.92	0.014	14,008	3.89	0.16
Ultimate	152	555	10.08		13.92	0.014	724	0.20	0.01
Ultimate	Total		808.38		13.92	0.014	58,078	16.13	0.67

Sizing Option B		Pipe Dimensions			Average Day Demand		Water Age		
Phase	Pipe Diameter	Pipe Length	Pipe Volume		(L/s)	(m ³ /s)	(s)	(h)	(d)
	(mm)	(m)	(m ³)						
2	406	0	0.00		3.50	0.004	0	0.00	0.00
2	305	3,933	287.37		3.50	0.004	82,059	22.79	0.95
2	203	742	24.00		3.50	0.004	6,853	1.90	0.08
2	152	305	5.54		3.50	0.004	1,581	0.44	0.02
2	Total		316.90		3.50	0.004	90,493	25.14	1.05
3	406	0	0.00		6.64	0.007	0	0.00	0.00
3	305	4,817	351.95		6.64	0.007	53,036	14.73	0.61
3	203	2,001	64.75		6.64	0.007	9,757	2.71	0.11
3	152	427	7.75		6.64	0.007	1,167	0.32	0.01
3	Total		424.45		6.64	0.007	63,960	17.77	0.74
4	406	0	0.00		10.30	0.010	0	0.00	0.00
4	305	5,841	426.75		10.30	0.010	41,413	11.50	0.48
4	203	3,747	121.28		10.30	0.010	11,769	3.27	0.14
4	152	474	8.60		10.30	0.010	835	0.23	0.01
4	Total		556.64		10.30	0.010	54,017	15.00	0.63
Ultimate	406	0	0.00		13.92	0.014	0	0.00	0.00
Ultimate	305	6,619	483.60		13.92	0.014	34,744	9.65	0.40
Ultimate	203	6,024	194.98		13.92	0.014	14,008	3.89	0.16
Ultimate	152	555	10.08		13.92	0.014	724	0.20	0.01
Ultimate	Total		688.66		13.92	0.014	49,477	13.74	0.57

**Kennedy-Burnett Potable
Water Master Servicing Study**



Prepared for:
City of Ottawa
100 Constellation Crescent
Ottawa, ON K2G 6G8

Prepared by:
Stantec Consulting Ltd.
400-1331 Clyde Avenue
Ottawa, ON K2C 3G4

File No. 1634-01221

April 29, 2014

KENNEDY-BURNETT POTABLE WATER MASTER SERVICING STUDY

Hydraulic Assessment
April 29, 2014

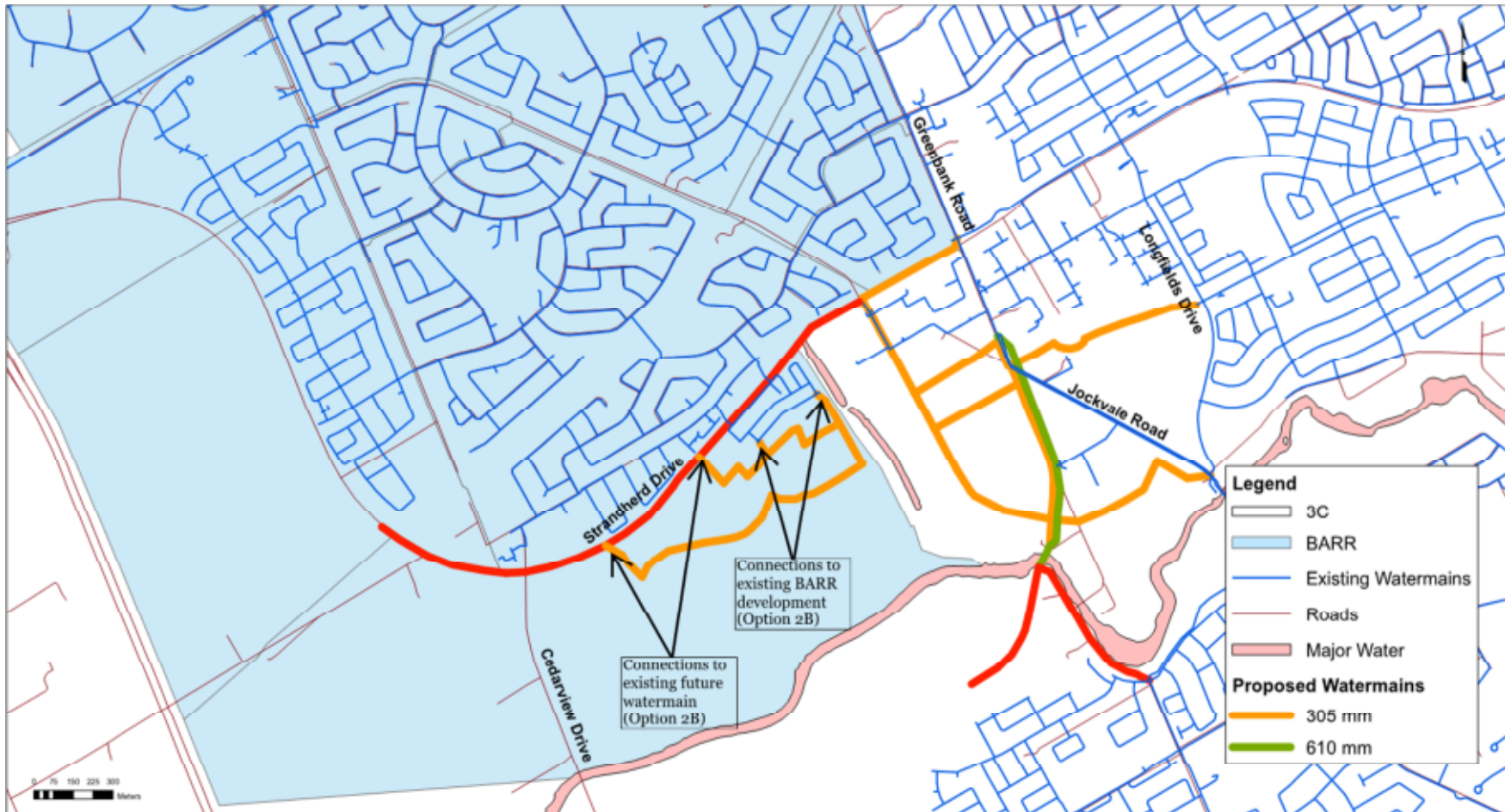


Figure 2-6: Proposed Pipe Layout Post Zone Reconfiguration – Scenario 2B

APPENDIX C

SANITARY

MEMORANDUM

DATE: MAY 30, 2019
TO: JOSÉE VALLEE – CITY OF OTTAWA
FROM: CONRAD STANG – NOVATECH
RE: STRANDHERD DRIVE WIDENING PROJECT
SOUTH NEPEAN COLLECTOR PHASE 3: SANITARY FLOW CALCULATIONS
CC: EDSON DONNELLY – NOVATECH

1.0 PURPOSE

This memorandum provides the sanitary sewer flow calculations and design sheet for Phase 3 of the proposed South Nepean Collector (SNC), as part of the Strandherd Drive Widening Project. Sanitary design flows have been estimated for both current-day operational flows and future development peak design flows. They are based on the latest available planning information for the vacant lands within the SNC sewershed.

2.0 BACKGROUND

In January 1998, the Master Servicing Study for the South Nepean Urban Area provided a conceptual plan for water, wastewater and stormwater infrastructure. The preferred alternative for wastewater servicing was an east/west trunk sewer alignment that was to be completed in several phases. The proposed sanitary trunk sewer was initially called the Jock River Collector, but was renamed the South Nepean Collector during the original functional design study completed in 2003.

Phase 1 of the South Nepean Collector was completed in 2005 and currently terminates at a 2400mm maintenance hole located east of Longfields Drive, north of Bren-Maur Road. Phase 2 was completed in 2016 and currently terminates at a 2400mm maintenance hole located at the intersection of Strandherd Drive and Fraser Fields Way.

Phase 3 will extend the trunk sewer along Strandherd Drive to the intersection of Kennevale Drive. Here it will connect with the existing sanitary trunk sewer that was constructed as part of the 2014 works to improve Strandherd Drive and develop the CitiGate Lands.

The sanitary sewer flows were previously documented in the *South Nepean Collector – Functional Design Report and Update* (Dillon, 2012). Novatech (2016) completed a *Hydraulics Review / Assessment* of the sanitary flows presented in the Dillon Report (attached). This was based on the latest planning information for the vacant lands within the SNC sewershed. The results of the *Hydraulics Review / Assessment* (Novatech, 2016) were similar to the results from the Dillon (2012) analysis.

3.0 DESIGN PARAMETERS AND POPULATION ESTIMATES

3.1 Design Parameters

The sanitary design flow were calculated using the parameters from the City of Ottawa Sewer Design Guidelines (October 2012), revised per Technical Bulletin ISTB-2018-01 (March 2018). These parameters are summarized in **Table 1** and **Table 2**.

Table 1: Peak Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	280 L/cap/day	Harmon Equation, K=0.8 (1.6 min – 3.2 max)	0.33 L/s/ha
Commercial	28,000 L/ha/day	1.0 – 1.5*	
Institutional	28,000 L/ha/day	1.0 – 1.5*	
Other [†]	0 L/ha/day	N/A	

*Peak Factor = 1.5 if contributing area is >20%; Peak Factor = 1.0 if contributing area is <20%

[†]Open Space, Arterial ROW, SWM Blocks, etc. with no sanitary flow contribution (extraneous flow only)

Table 2: Operational Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	200 L/cap/day	Harmon Equation, K=0.6 (1.2 min – 2.4 max)	0.30 L/s/ha
Commercial	17,000 L/ha/day	1.0 (non-coincident peak)	
Institutional	17,000 L/ha/day	1.0 (non-coincident peak)	

*There are no industrial areas identified within the tributary area.

$$\text{Harmon Equation} = 1 + \frac{14}{4 + \left(\frac{P}{1000}\right)^{\frac{1}{2}}} \times K$$

Where:

P = Population

K = Correction Factor:

- Peak Flow = 0.8
- Operational = 0.6

3.2 Land Use Designations & Population Estimates

Population densities and unit counts for future residential development are based on the Novatech (2016) Hydraulics Review / Assessment; refer to **Table 3**. They are based on the concept plans provided by the developers of the future residential areas.

Table 3: Residential Land Use Population Densities

Residential Land Use	Units per ha	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

The land use designations shown in **Table 4** have been applied for the areas within Phases 2 & 3 of the SNC (Node 70 to 130). The Hydraulics Analysis / Review delineated the sewershed areas and land use designations using aerial photos (existing development) and conceptual site plans (future development).

Table 4: Land Use Designations

Land Use Designation	
Secondary Plan	SNC Design
Residential	Residential (Low / Medium / High Density)
Institutional / Office	Institutional
Commercial	Commercial
Recreational	
Business Park	
Prestige Business Park	
Park/Open Space Area	Other*
Ex. Snow Disposal Facility (future commercial)	
Stormwater Management Facility	
Conservation Lands	
Arterial Right-of-Ways	

* No sanitary flow contribution - extraneous flows (inflow/infiltration) only.

The overall residential population estimate and sewershed area for Phases 2 and 3 of the SNC is provided in **Table 5** below. It is assumed that the snow dump facility at the Stranderd Drive and McKenna Casey Drive will ultimately be re-zoned for commercial development.

Table 5: Population Estimates and Areas

Existing / Future	Estimated Population / Area	Novatech (2015)
Existing	Estimated Population	6,944 persons
	Gross Residential Area	60.09 ha
	Gross Commercial / Institutional Area	64.37 ha
	<i>Total Sewershed Area</i>	124.5 ha
Future (full service)	Estimated Population	27,312 persons
	Gross Residential Area	248.48 ha
	Gross Commercial / Institutional Area	228.82 ha
	<i>Total Sewershed Area</i>	477.3 ha

4.0 SANITARY DESIGN FLOWS

The sanitary flow allocations for Phases 2 and 3 of the SNC are provided in **Table 6**. The corresponding sanitary drainage area plan is provided as **Figure 1**. Sanitary sewer flow calculations for Phases 2 and 3 and detailed sanitary sewer design sheets for Phase 3 are attached to this memorandum.

The estimated sanitary design flows from Phase 3 of the SNC (entering Node 90) are as follows:

- Present-Day Operational Design Flows (Theoretical) = 55.1 L/s
- Future Peak Design Flows = 282.5 L/s

The outlet for Phase 3 of the SNC is the existing 900mm outlet pipe at the 2400mm maintenance hole (Node 90) located at the intersection of Strandherd Drive and Fraser Fields Way. Given a minimum design slope of 0.10%, this 900mm sanitary trunk sewer would have a full flow capacity of 597.2 L/s. Therefore, the downstream sanitary trunk sewer would be at 64% capacity, based on the future peak design flow being 282.5 L/s.

ATTACHMENTS:

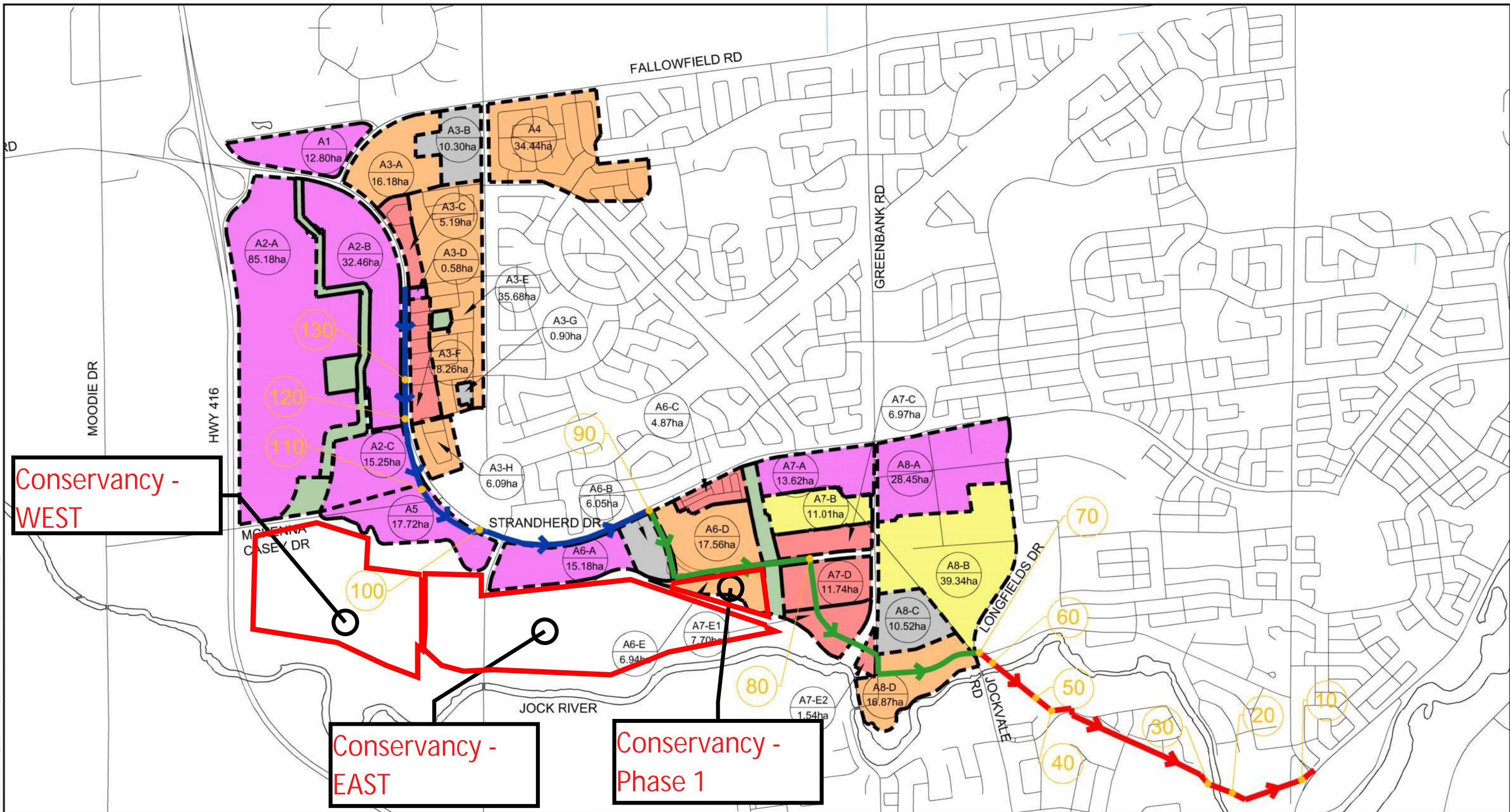
- Figure 1: Sanitary Drainage Areas and Land Use
- Sanitary Sewer Flow Calculations
- Sanitary Sewer Design Sheets (Phase 3)
- South Nepean Collector Phase 2: Hydraulics Review / Assessment (Novatech, 2016)
- Excerpts from Dillion (2012)



Table 6: Updated Allocation of Commercial, Institutional and Residential Demands to Phases 2 & 3 (Nodes 70 – 130) of the SNC by Collection Area

Collection Area	Upstream Node	Existing / Proposed Development	Existing / Proposed Land Use	Area (ha)	Estimated Number of Residential Units	Population Density (persons / ha)	Comment	Reference
A1	130	Proposed	Commercial	12.80	-	-	O'Keefe Court – Conceptual site plan shows proposed commercial.	Conceptual Plans for O'Keefe Court
A2-A	130	Proposed	Commercial	85.18	-	-	CitiGate – Analysis uses same approach as the design for CitiGate.	Detailed Servicing and SWM Report (Phase 1) (Novatech, 2014)
A2-B	130	Proposed	Commercial	32.46	-	-		
A2-C	120	Proposed	Commercial (ex. Snow dump)	15.25	-	-	Existing snow dump facility assumed to be future commercial.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)
A3-A	130	Proposed	Low Density Residential	16.18	461	95.2	Havencrest – Existing single family units.	Havencrest Design Report (IBI, 2013)
A3-B	130	Existing	Institutional	10.30	-	-	Cedarview Middle School and Cedarview Alliance Church.	Aerial Photos / Site Visits
A3-C	130	Existing	Medium Density Residential	5.19	311	162	Existing townhouse units.	
A3-D	130	Existing	Commercial	0.58	-	-	Existing commercial buildings.	
A3-E	130	Existing	Low Density Residential	35.68	999	95.2	Existing single family units.	
A3-F	130	Existing	Medium Density Residential	8.26	496	162.0	Existing townhouse units.	
A3-G	130	Existing	Institutional	0.90	-	-	Ottawa Torah Centre Chibad.	
A3-H	120	Existing	Low Density Residential	6.09	171	95.2	Existing single family units.	
A4	130	Existing	Low Density Residential	34.44	964	95.2	Existing single family units currently serviced by Jockvale pump station; to be redirected to SNC.	
A5	110	Proposed	Commercial	17.72	-	-	Proposed commercial south of McKenna Casey Drive.	Site Visits
A6-A	100	Proposed	Commercial	15.18	-	-	Proposed commercial south of Srandherd Drive; east of Borrisokane Road.	Conceptual Plan for Lands Adjacent the Kennedy-Burnett SWMF provided by Minto (2015)
A6-B	100	Proposed	Institutional	6.05	-	-	Proposed school site on Minto property.	
A6-C	90	Existing	Medium Density Residential	4.87	292	162.0	Existing townhouse units.	Aerial Photos / Site Visits
A6-D	90	Proposed	Low Density Residential	17.56	492	95.2	Proposed single family units on lands owned by Minto / Mion.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A6-E	90	Proposed	Low Density Residential	6.94	203	95.2	Proposed single family units on lands owned by Pavic / Braovac.	
A7-A	80	Existing	Commercial	13.62	-	-	Existing large retail stores (commercial).	Aerial Photos
A7-B	80	Proposed	High Density Residential	11.01	826	135.0	Proposed high density units on lands owned by Richcraft / Trinity.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A7-C	80	Proposed	Medium Density Residential	6.97	418	162.0	Proposed Medium density units on lands owned by Mion.	
A7-D	80	Proposed	Medium Density Residential	11.74	704	162.0	Proposed Medium density units on lands owned by Caivan.	
A7-E1/E2	80	Proposed	Medium Density Residential	9.24	554	162.0	Proposed Medium density units on lands owned by Claridge.	
A8-A	80	Existing	Commercial	28.45	-	-	Existing Barrhaven Market Place (commercial).	Aerial Photos / Site Visits
A8-B	80	Proposed	High Density Residential	39.34	2951	135.0	Future development similar to Ampersands development.	Site Visits
A8-C	80	Existing	Institutional	10.52	-	-	Existing St. Joseph High School.	Aerial Photos / Site Visits
A8-D	80	Proposed	Low Density Residential	16.87	1012	162.0	Proposed 600 low density residential units.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)

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Conservancy - WEST

Conservancy - EAST

Conservancy - Phase 1

LEGEND

- | | | | |
|---|--|--|--|
|  | EXISTING / PROPOSED HIGH DENSITY RESIDENTIAL |  | OTHER LANDS (OPEN SPACE, PARKS, AND SWMFS) |
|  | EXISTING / PROPOSED MEDIUM DENSITY RESIDENTIAL |  | SOUTH NEPEAN COLLECTOR PHASE 1 |
|  | EXISTING / PROPOSED LOW DENSITY RESIDENTIAL |  | SOUTH NEPEAN COLLECTOR PHASE 2 |
|  | EXISTING / PROPOSED COMMERCIAL |  | SOUTH NEPEAN COLLECTOR PHASE 3 |
|  | EXISTING / PROPOSED INSTITUTIONAL |  | SOUTH NEPEAN COLLECTOR NODE ID |



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**SOUTH NEPEAN COLLECTOR SEWER
 SANITARY DRAINAGE AREAS AND LAND USE**

SCALE	1:20 000	
DATE	MAY 2019	FIGURE
JOB	117190	FIG. 1

PROJECT #: 117190
 DESIGNED BY: CMS
 CHECKED BY: RJD
 DATE: December 5, 2018

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Current Operational Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.3 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (200 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-B	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-B	Institutional	130		10.30		10.30					0.0	2.0	3.1	0.0	2.0	3.1	0.0	5.1
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	841	2.40	0.0	0.0	1.6	0.0	2.0	4.6	4.7	11.3
A3-D	Commercial	130	0.58			0.58		841	841	2.40	0.1	0.0	0.2	0.1	2.0	4.8	4.7	11.6
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	4238	2.39	0.0	0.0	10.7	0.1	2.0	15.5	23.4	41.1
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	5576	2.32	0.0	0.0	2.5	0.1	2.0	18.0	29.9	50.1
A3-G	Institutional	130		0.90		0.90			5576	2.32	0.0	0.2	0.3	0.1	2.2	18.3	29.9	50.5
A4	Low Density Residential*	130				0.00			5576	2.32	0.0	0.0	0.0	0.1	2.2	18.3	29.9	50.5
A2-C	Snow Dump Facility	120				0.00			5576	2.32	0.0	0.0	0.0	0.1	2.2	18.3	29.9	50.5
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	6155	2.30	0.0	0.0	1.8	0.1	2.2	20.1	32.7	55.1
A5	Open Space	110				0.00			6155	2.30	0.0	0.0	0.0	0.1	2.2	20.1	32.7	55.1
A6-A	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	2.2	20.1	32.7	55.1
A6-B	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	2.2	20.1	32.7	55.1
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	6944	2.27	0.0	0.0	1.5	0.1	2.2	21.6	36.4	60.3
A6-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	2.2	21.6	36.4	60.3
A6-E	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	2.2	21.6	36.4	60.3
A7-A	Commercial	90	13.62			13.62			6944	2.27	2.7	0.0	4.1	2.8	2.2	25.6	36.4	67.1
A7-B	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A7-C	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A7-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A7-E1/E2	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A8-A	Commercial	80	28.45			28.45			6944	2.27	5.6	0.0	8.5	8.4	2.2	34.2	36.4	81.2
A8-B	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	2.2	34.2	36.4	81.2
A8-C	Institutional	80		10.52		10.52			6944	2.27	0.0	2.1	3.2	8.4	4.3	37.3	36.4	86.4
A8-D	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	4.3	37.3	36.4	86.4
ROW Along SNC Sewer Alignment	-	80				14.34			6944	2.27	0.0	0.0	4.3	8.4	4.3	41.6	36.4	90.7
TOTAL		80	42.65	21.72	60.09	138.80	-	6944	6944	2.27	8.4	4.3	41.6	8.4	4.3	41.6	36.4	90.7

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 0.6
- Institutional / Commercial Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station; currently not directed to SNC

PROJECT #: 117190
 DESIGNED BY: CMS
 CHECKED BY: RJD
 DATE: December 5, 2018

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Future Full Service Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing / Proposed Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (28,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (28,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.33 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (280 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Commercial	130	12.80			12.80					6.2	0.0	4.2	6.2	0.0	4.2	0.0	10.4
A2-A	Commercial	130	85.18			85.18					41.4	0.0	28.1	47.6	0.0	32.3	0.0	80.0
A2-B	Commercial	130	32.46			32.46					15.8	0.0	10.7	63.4	0.0	43.0	0.0	106.5
A3-A	Low Density Residential	130			16.18	16.18	95.2	1540	1540	3.14	0.0	0.0	5.3	63.4	0.0	48.4	15.7	127.5
A3-B	Institutional	130		10.30		10.30		1540	1540	3.14	0.0	3.3	3.4	63.4	3.3	51.8	15.7	134.2
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	2381	3.02	0.0	0.0	1.7	63.4	3.3	53.5	23.3	143.6
A3-D	Commercial	130	0.58			0.58		2381	2381	3.02	0.3	0.0	0.2	63.7	3.3	53.7	23.3	144.0
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	5778	2.75	0.0	0.0	11.8	63.7	3.3	65.5	51.5	184.0
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	7116	2.68	0.0	0.0	2.7	63.7	3.3	68.2	61.8	197.0
A3-G	Institutional	130		0.90		0.90		7116	7116	2.68	0.0	0.3	0.3	63.7	3.6	68.5	61.8	197.6
A4	Low Density Residential	130			34.44	34.44	95.2	3279	10395	2.55	0.0	0.0	11.4	63.7	3.6	79.9	85.9	233.1
A2-C	Commercial (ex. snow dump)	120	15.25			15.25		10395	10395	2.55	7.4	0.0	5.0	71.1	3.6	84.9	85.9	245.5
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	10974	2.53	0.0	0.0	2.0	71.1	3.6	86.9	90.0	251.7
A5	Commercial	110	17.72			17.72		10974	10974	2.53	8.6	0.0	5.8	79.7	3.6	92.7	90.0	266.1
A6-A	Commercial	100	15.18			15.18		10974	10974	2.53	7.4	0.0	5.0	87.1	3.6	97.7	90.0	278.5
A6-B	Institutional	100		6.05		6.05		10974	10974	2.53	0.0	2.0	2.0	87.1	5.6	99.7	90.0	282.5
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	11763	2.51	0.0	0.0	1.6	87.1	5.6	101.4	95.6	289.6
A6-D	Low Density Residential	90			17.56	17.56	95.2	1672	13435	2.46	0.0	0.0	5.8	87.1	5.6	107.1	107.2	307.0
A6-E	Low Density Residential	90			6.94	6.94	95.2	661	14096	2.44	0.0	0.0	2.3	87.1	5.6	109.4	111.7	313.8
A7-A	Commercial	90	13.62			13.62		14096	14096	2.44	6.6	0.0	4.5	93.7	5.6	113.9	111.7	324.9
A7-B	High Density Residential	90			11.01	11.01	135.0	1486	15582	2.41	0.0	0.0	3.6	93.7	5.6	117.6	121.7	338.5
A7-C	Medium Density Residential	90			6.97	6.97	162.0	1129	16711	2.38	0.0	0.0	2.3	93.7	5.6	119.9	129.2	348.3
A7-D	Medium Density Residential	90			11.74	11.74	162.0	1902	18613	2.35	0.0	0.0	3.9	93.7	5.6	123.7	141.6	364.6
A7-E1/E2	Medium Density Residential	90			9.24	9.24	162.0	1497	20110	2.32	0.0	0.0	3.0	93.7	5.6	126.8	151.2	377.3
A8-A	Commercial	80	28.45			28.45		20110	20110	2.32	13.8	0.0	9.4	107.5	5.6	136.2	151.2	400.5
A8-B	High Density Residential	80			39.34	39.34	135.0	5311	25421	2.24	0.0	0.0	13.0	107.5	5.6	149.2	184.4	446.7
A8-C	Institutional	80		10.52		10.52		25421	25421	2.24	0.0	3.4	3.5	107.5	9.0	152.6	184.4	453.6
A8-D	Low Density Residential	80			16.87	16.87	120.9	2040	27461	2.21	0.0	0.0	5.6	107.5	9.0	158.2	196.9	471.6
ROW Along SNC Sewer Alignment	-	80				14.34			27461	2.21	0.0	0.0	4.7	107.5	9.0	162.9	196.9	476.3
TOTAL		80	221.24	77.77	230.38	493.73	-	27461	27461	2.21	107.5	9.0	162.9	107.5	9.0	162.9	196.9	476.3

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 - 28 (28 used)	2.7 - 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 - 60 (60 used)	2.7	162.0
High Density (apartments)	60 - 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 0.8
- Commercial Peaking Factor = 1.5; Institutional Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

See Note (2) in the DSEL "Barrhaven Conservancy - Evaluation of SNC Flows" design sheet

THE PRIOR NOVATECH SNC DESIGN SHEET HAD FLOWS AT **423.6 L/s** AFTER AREA ID "A6-E".
 THIS UPDATED NOVATECH 'PHASE 3' EVALUATION HAS A FLOW OF **313.8 L/s**.
 THE DSEL EVALUATION OF SANITARY FLOWS WITH THE NEW CITY DESIGN PARAMETERS AT THIS SAME NODE (WITH CONSERVANCY WEST AND EAST INCLUDED) IS **-401.58 L/s** (WHICH IS LESS THAN THE PRIOR **423.6 L/s** NOTED ABOVE)

SOUTH NEPEAN COLLECTOR (PHASE 3)
SANITARY SEWER DESIGN SHEET

DECEMBER 5 2018
JOB# 117190



LOCATION			Area				Population		Cumulative Design Flows					PROPOSED SEWER						
From MH	To MH	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (280 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)	Length (m)	Pipe Size (mm)	Type	Slope %	Capacity (L/s)	Full Flow Velocity (m/s)	Ratio (Q/Qfull)
SA 22	SA 21	120	146.27	11.20	105.84	263.31	10974	2.53	71.1	3.6	86.9	90.0	251.7	131.9	750	CONC	0.10	367.3	0.81	69%
SA 21	SA 20	120											251.7	90.6	750	CONC	0.10	367.3	0.81	69%
SA 20	SA 19	120											251.7	90.0	750	CONC	0.10	367.3	0.81	69%
SA 19	SA 18	120											251.7	72.1	750	CONC	0.10	367.3	0.81	69%
SA 18	SA 17	120											251.7	71.9	750	CONC	0.10	367.3	0.81	69%
SA 17	SA 16	120											251.7	71.4	750	CONC	0.10	367.3	0.81	69%
SA 16	SA 15	110	163.99	11.20	105.84	281.03	10974	2.53	79.7	3.6	92.7	90.0	266.1	73.2	750	CONC	0.10	367.3	0.81	72%
SA 15	SA 14	110											266.1	67.5	750	CONC	0.10	367.3	0.81	72%
SA 14	SA 13	110											266.1	56.6	750	CONC	0.10	367.3	0.81	72%
SA 13	SA 12	110											266.1	133.5	750	CONC	0.10	367.3	0.81	72%
SA 12	SA 11	110											266.1	150.0	750	CONC	0.10	367.3	0.81	72%
SA 11	SA 10	100	179.17	17.25	105.84	302.26	10974	2.53	87.1	5.6	99.7	90.0	282.5	97.8	750	CONC	0.10	367.3	0.81	77%
SA 10	SA 9	100											282.5	76.7	750	CONC	0.10	367.3	0.81	77%
SA 9	SA 8	100											282.5	79.7	750	CONC	0.10	367.3	0.81	77%
SA 8	SA 7	100											282.5	75.3	750	CONC	0.10	367.3	0.81	77%
SA 7	SA 6	100											282.5	84.9	750	CONC	0.10	367.3	0.81	77%
SA 6	SA 5	100											282.5	77.1	750	CONC	0.10	367.3	0.81	77%
SA 5	SA 4	100											282.5	78.9	750	CONC	0.10	367.3	0.81	77%
SA 4	SA 3	100											282.5	80.5	750	CONC	0.10	367.3	0.81	77%
SA 3	SA 2	100											282.5	150.0	750	CONC	0.10	367.3	0.81	77%
SA 2	SA 1	100											282.5	114.6	750	CONC	0.10	367.3	0.81	77%
SA 1	EX 80	100											282.5	12.4	750	CONC	0.10	367.3	0.81	77%

Design Parameters:

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles / semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row / townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4+(P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 0.8
- Commercial Peaking Factor = 1.5; Institutional Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units



MEMORANDUM

DATE: MAY 26, 2016
TO: JONATHAN KNOYLE – CITY OF OTTAWA
FROM: CONRAD STANG – NOVATECH
RE: SOUTH NEPEAN COLLECTOR PHASE 2: SANITARY FLOW CALCULATIONS
CC: EDSON DONNELLY – NOVATECH

1.0 PURPOSE

This memorandum provides the sanitary sewer flow calculations and design sheet for Phase 2 of the proposed South Nepean Collector (SNC). Sanitary design flows have been estimated for both current-day operational flows and future development peak design flows, based on the latest available planning information for the vacant lands within the SNC sewershed.

2.0 BACKGROUND

In January 1998, the Master Servicing Study for the South Nepean Urban Area provided a conceptual plan for water, wastewater and stormwater infrastructure. The preferred alternative for wastewater servicing was an east/west trunk sewer alignment that was to be completed in several phases. The proposed sanitary trunk sewer was initially called the Jock River Collector, but was renamed the South Nepean Collector during the original functional design study completed in 2003.

Phase 1 of the South Nepean Collector was completed in 2005 and currently terminates at a 2400mm maintenance hole located east of Longfields Drive, north of Bren-Maur Road. Phase 2 will extend the trunk sewer to Strandherd Drive at the intersection of the proposed transitway along the proposed extension to Chapman Mills Drive. Phase 3 will extend the trunk sewer along Strandherd Drive to the intersection of Maravista Drive.

The sanitary sewer flows were previously documented in the *South Nepean Collector – Functional Design Report and Update* (Dillon, 2012). A review of the sanitary flows provided in the Dillon Report based on the latest planning information for the vacant lands within the SNC sewershed was documented in the technical memorandum titled *South Nepean Collector Phase 2: Hydraulics Review / Assessment* (Novatech, 2015), which is attached to this memorandum. The results of the *Hydraulics Review / Assessment* (Novatech, 2015) were very similar to the results from the Dillon (2012) analysis.

3.0 DESIGN PARAMETERS AND POPULATION ESTIMATES

3.1 Design Parameters

The sanitary design flow were calculated using the parameters from the City of Ottawa Sewer Design Guidelines (October 2012), and are summarized in **Table 1** and **Table 2**.

Table 1: Peak Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	350 L/cap/day	Harmon Equation, K=1 (2.0 min – 4.0 max)	0.28 L/s/ha
Commercial	50,000 L/ha/day	1.5	
Institutional	50,000 L/ha/day	1.5	
Other*	0 L/ha/day	N/A	

*Open Space, Arterial ROW, SWM Blocks, etc. with no sanitary flow contribution (extraneous flow only)

Table 2: Operational Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	300 L/cap/day	Harmon Equation, K=0.6 (1.2 min – 2.4 max)	<u>Dry weather</u> 0.05-0.08 L/s/ha
Commercial	17,000 L/ha/day	1.0 (non-coincident peak)	<u>Wet Weather</u> 0.15 - 0.20 L/s/ha (typical events) 0.28 L/s/ha (large/annual events) 0.30 - 0.50 L/s/ha (extreme events)
Institutional	10,000 L/ha/day	1.0 (non-coincident peak)	

*There are no industrial areas identified within the tributary area.

$$\text{Harmon Equation} = 1 + \frac{14}{4 + \left(\frac{P}{1000}\right)^{\frac{1}{2}}} \times K$$

Where:

P = Population

K = Correction Factor:

- Peak Flow = 1
- Operational = between 0.4 to 0.6 (0.6 used)

3.2 Land Use Designations & Population Estimates

Population densities and unit counts for future residential development are based on the current concept plans for these areas, and are presented in **Table 3**.

Table 3: Residential Land Use Population Densities

Residential Land Use	Units per ha	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

The land use designations shown in **Table 4** have been applied for the areas within Phases 2 and 3 of the SNC (Node 70 to 130). The sewershed areas and land use designations were delineated using aerial photos (existing development) and conceptual site plans (future development).

Table 4: Land Use Designations

Land Use Designation	
Secondary Plan	SNC Design
Residential	Residential (Low / Medium / High Density)
Institutional / Office	Institutional
Commercial	Commercial
Recreational	
Business Park	
Prestige Business Park	
Park/Open Space Area	Other*
Ex. Snow Disposal Facility (future commercial)	
Stormwater Management Facility	
Conservation Lands	
Arterial Right-of-Ways	

* No sanitary flow contribution - extraneous flows (inflow/infiltration) only.

The overall residential population estimate and sewershed area for Phases 2 and 3 of the SNC is provided in **Table 5** below. It is assumed that the snow dump facility at the Stranderd Drive and McKenna Casey Drive will ultimately be re-zoned for commercial development.

Table 5: Population Estimates and Areas

Existing / Future	Estimated Population / Area	Novatech (2015)
Existing	Estimated Population	6,944 persons
	Gross Residential Area	60.09 ha
	Gross Commercial / Institutional Area	64.37 ha
	<i>Total Sewershed Area</i>	124.5 ha
Future (full service)	Estimated Population	27,312 persons
	Gross Residential Area	248.48 ha
	Gross Commercial / Institutional Area	228.82 ha
	<i>Total Sewershed Area</i>	477.3 ha

4.0 SANITARY DESIGN FLOWS

The sanitary flow allocations for Phases 2 and 3 of the SNC are provided in **Table 6**. The corresponding sanitary drainage area plan is provided as **Figure 1**. Sanitary sewer flow calculations for Phases 2 and 3 and detailed sanitary sewer design sheets for Phase 2 are attached to this memorandum.

The estimated sanitary design flows from Phases 2 and 3 of the SNC (entering Node 70) are as follows:

- Present-Day Operational Design Flows (Theoretical) = 72.5 L/s
- Future Peak Design Flows = 634.2 L/s

The outlet for Phase 2 of the SNC is the existing 1050mm outlet pipe at the 2400mm maintenance hole (Node 70) located east of Longfields Drive, north of Bren-Maur Road. Given a minimum design slope of 0.10%, this sanitary trunk sewer would have a full flow capacity of 900.5 L/s. Therefore, the downstream sanitary trunk sewer would be at 70% capacity, based on the future peak design flow being 634.2 L/s.

ATTACHMENTS:

- Figure 1: Sanitary Drainage Areas and Land Use
- Sanitary Sewer Flow Calculations
- Sanitary Sewer Design Sheets (Phase 2)
- South Nepean Collector Phase 2: Hydraulics Review / Assessment (Novatech, 2015)

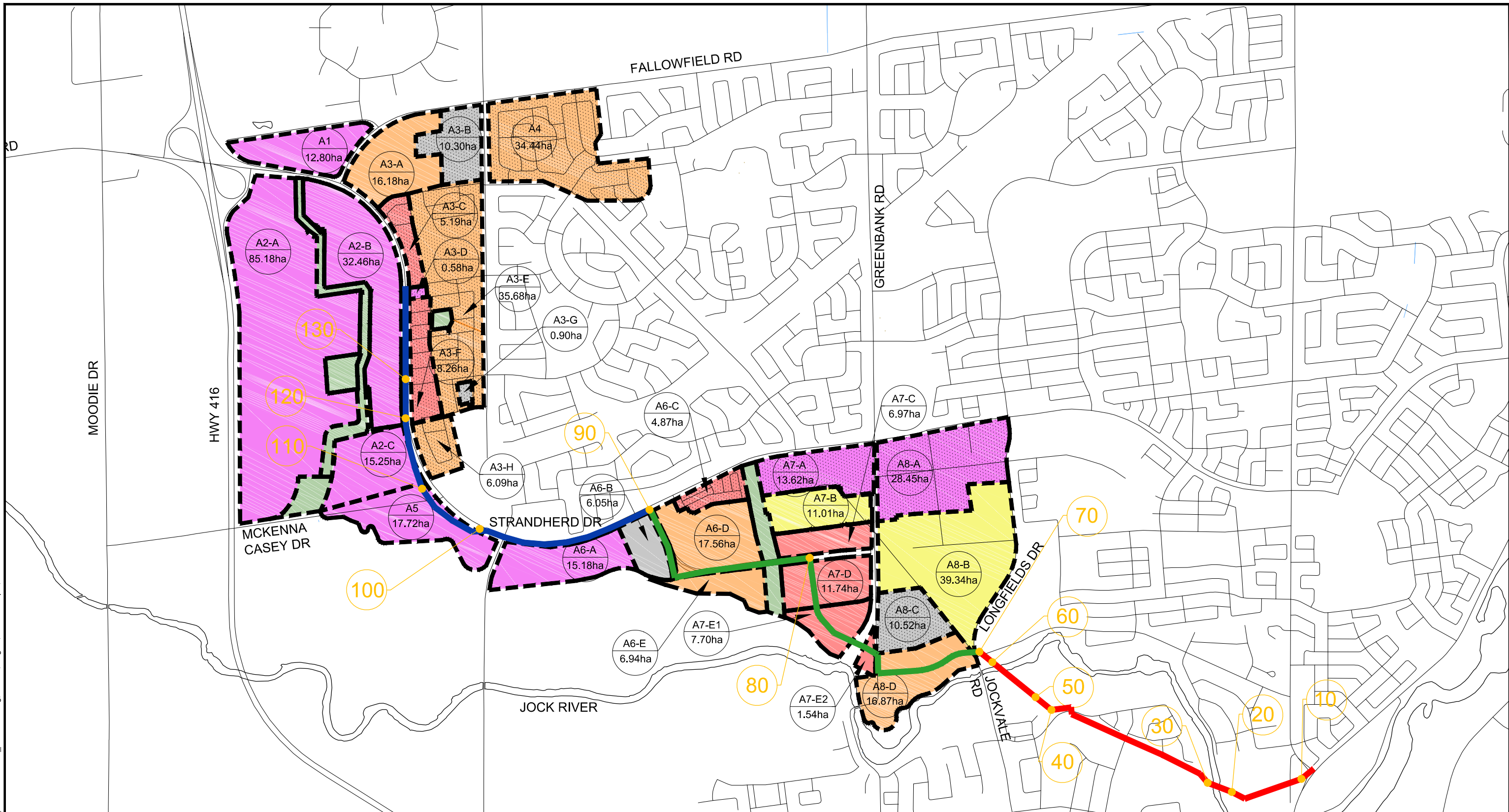


Table 6: Updated Allocation of Commercial, Institutional and Residential Demands to Phases 2 & 3 (Nodes 70 – 130) of the SNC by Collection Area

Collection Area	Upstream Node	Existing / Proposed Development	Existing / Proposed Land Use	Area (ha)	Estimated Number of Residential Units	Population Density (persons / ha)	Comment	Reference
A1	130	Proposed	Commercial	12.80	-	-	O'Keefe Court – Conceptual site plan shows proposed commercial.	Conceptual Plans for O'Keefe Court
A2-A	130	Proposed	Commercial	85.18	-	-	CitiGate – Analysis uses same approach as the design for CitiGate.	Detailed Servicing and SWM Report (Phase 1) (Novatech, 2014)
A2-B	130	Proposed	Commercial	32.46	-	-		
A2-C	120	Proposed	Commercial (ex. Snow dump)	15.25	-	-	Existing snow dump facility assumed to be future commercial.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)
A3-A	130	Proposed	Low Density Residential	16.48	461	95.2	Havencrest – Existing single family units.	Havencrest Design Report (IBI, 2013)
A3-B	130	Existing	Institutional	10.30	-	-	Cedarview Middle School and Cedarview Alliance Church.	Aerial Photos / Site Visits
A3-C	130	Existing	Medium Density Residential	5.19	311	162	Existing townhouse units.	
A3-D	130	Existing	Commercial	0.58	-	-	Existing commercial buildings.	
A3-E	130	Existing	Low Density Residential	35.68	999	95.2	Existing single family units.	
A3-F	130	Existing	Medium Density Residential	8.26	496	162.0	Existing townhouse units.	
A3-G	130	Existing	Institutional	0.90	-	-	Ottawa Torah Centre Chibad.	
A3-H	120	Existing	Low Density Residential	6.09	171	95.2	Existing single family units.	
A4	130	Existing	Low Density Residential	34.44	964	95.2	Existing single family units currently serviced by Jockvale pump station; to be redirected to SNC.	
A5	110	Proposed	Commercial	17.72	-	-	Proposed commercial south of McKenna Casey Drive.	Site Visits
A6-A	100	Proposed	Institutional	20.70	-	-	Proposed school site on Minto property.	Conceptual Plan for Lands Adjacent the Kennedy-Burnett SWMF provided by Minto (2015)
A6-B	90	Existing	Medium Density Residential	4.87	292	162.0	Existing townhouse units.	Aerial Photos / Site Visits
A6-C	90	Proposed	Low Density Residential	10.11	283	95.2	Proposed single family units on lands owned by Minto.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A6-D	90	Proposed	Low Density Residential	5.59	157	95.2	Proposed single family units on lands owned by Mion.	
A6-E	90	Proposed	Low Density Residential	7.24	203	95.2	Proposed single family units on lands owned by Pavic / Braovac.	
A7-A	80	Existing	Commercial	13.62	-	-	Existing large retail stores (commercial).	Aerial Photos
A7-B	80	Proposed	High Density Residential	11.01	826	135.0	Proposed high density units on lands owned by Richcraft / Trinity.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A7-C	80	Proposed	Medium Density Residential	6.97	418	162.0	Proposed Medium density units on lands owned by Mion.	
A7-D	80	Proposed	Medium Density Residential	11.74	704	162.0	Proposed Medium density units on lands owned by Caivan.	
A7-E1/E2	80	Proposed	Medium Density Residential	9.24	554	162.0	Proposed Medium density units on lands owned by Claridge.	
A8-A	80	Existing	Commercial	28.45	-	-	Existing Barrhaven Market Place (commercial).	Aerial Photos / Site Visits
A8-B	80	Proposed	High Density Residential	39.34	2951	135.0	Future development similar to Ampersands development.	Site Visits
A8-C	80	Existing	Institutional	10.52	-	-	Existing St. Joseph High School.	Aerial Photos / Site Visits
A8-D	80	Proposed	Low Density Residential	16.87	1012	162.0	Proposed 600 low density residential units.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)

Attachment 1
Sanitary Drainage Areas and Land Use

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LEGEND

- EXISTING / PROPOSED HIGH DENSITY RESIDENTIAL
- EXISTING / PROPOSED MEDIUM DENSITY RESIDENTIAL
- EXISTING / PROPOSED LOW DENSITY RESIDENTIAL
- EXISTING / PROPOSED COMMERCIAL
- EXISTING / PROPOSED INSTITUTIONAL
- OTHER LANDS (OPEN SPACE, PARKS, AND SWMFS)
- SOUTH NEPEAN COLLECTOR PHASE 1
- SOUTH NEPEAN COLLECTOR PHASE 2
- SOUTH NEPEAN COLLECTOR PHASE 3
- SOUTH NEPEAN COLLECTOR NODE ID



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 Website www.novatech-eng.com

**SOUTH NEPEAN COLLECTOR SEWER
 SANITARY DRAINAGE AREAS AND LAND USE**

SCALE	1:20 000	
DATE	MAY 2016	FIGURE
JOB	115075	FIG. 1

Attachment 2
Sewer Flow Calculations

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Current Operational Peak Wastewater Flow



PROJECT #: 115075
 DESIGNED BY: CMS
 CHECKED BY: MJP
 DATE: August 20, 2015

Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (10,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.05 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (300 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-B	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-B	Institutional	130		10.30		10.30					0.0	1.2	0.5	0.0	1.2	0.5	0.0	1.7
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	841	2.71	0.0	0.0	0.3	0.0	1.2	0.8	7.9	9.9
A3-D	Commercial	130	0.58			0.58		841	841	2.71	0.1	0.0	0.0	0.1	1.2	0.8	7.9	10.0
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	4238	2.39	0.0	0.0	1.8	0.1	1.2	2.6	35.1	39.0
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	5576	2.32	0.0	0.0	0.4	0.1	1.2	3.0	44.9	49.2
A3-G	Institutional	130		0.90		0.90			5576	2.32	0.0	0.1	0.0	0.1	1.3	3.0	44.9	49.4
A4	Low Density Residential*	130				0.00			5576	2.32	0.0	0.0	0.0	0.1	1.3	3.0	44.9	49.4
A2-C	Snow Dump Facility	120				0.00			5576	2.32	0.0	0.0	0.0	0.1	1.3	3.0	44.9	49.4
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	6155	2.30	0.0	0.0	0.3	0.1	1.3	3.4	49.1	53.8
A5	Open Space	110				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-A	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-B	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	6944	2.27	0.0	0.0	0.2	0.1	1.3	3.6	54.6	59.6
A6-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	1.3	3.6	54.6	59.6
A6-E	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	1.3	3.6	54.6	59.6
A7-A	Commercial	90	13.62			13.62			6944	2.27	2.7	0.0	0.7	2.8	1.3	4.3	54.6	63.0
A7-B	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-C	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-E1/E2	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A8-A	Commercial	80	28.45			28.45			6944	2.27	5.6	0.0	1.4	8.4	1.3	5.7	54.6	70.0
A8-B	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	1.3	5.7	54.6	70.0
A8-C	Institutional	80		10.52		10.52			6944	2.27	0.0	1.2	0.5	8.4	2.5	6.2	54.6	71.8
A8-D	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	2.5	6.2	54.6	71.8
ROW Along SNC Sewer Alignment	-	80				14.34			6944	2.27	0.0	0.0	0.7	8.4	2.5	6.9	54.6	72.5
TOTAL		80	42.65	21.72	60.09	138.80	-	6944	6944	2.27	8.4	2.5	6.9	8.4	2.5	6.9	54.6	72.5

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 0.6
- Institutional / Commercial Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station; currently not directed to SNC

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Future Full Service Peak Wastewater Flow



PROJECT #: 115075
 DESIGNED BY: CMS
 CHECKED BY: MJP
 DATE: August 20, 2015

Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing / Proposed Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.28 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (350 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Commercial	130	12.80			12.80					11.1	0.0	3.6	11.1	0.0	3.6	0.0	14.7
A2-A	Commercial	130	85.18			85.18					73.9	0.0	23.9	85.1	0.0	27.4	0.0	112.5
A2-B	Commercial	130	32.46			32.46					28.2	0.0	9.1	113.2	0.0	36.5	0.0	149.8
A3-A	Low Density Residential	130			16.18	16.18	95.2	1540	1540	3.67	0.0	0.0	4.5	113.2	0.0	41.1	22.9	177.2
A3-B	Institutional	130		10.30		10.30		1540	1540	3.67	0.0	8.9	2.9	113.2	8.9	43.9	22.9	189.0
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	2381	3.53	0.0	0.0	1.5	113.2	8.9	45.4	34.0	201.6
A3-D	Commercial	130	0.58			0.58		2381	2381	3.53	0.5	0.0	0.2	113.7	8.9	45.6	34.0	202.2
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	5778	3.19	0.0	0.0	10.0	113.7	8.9	55.5	74.6	252.8
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	7116	3.10	0.0	0.0	2.3	113.7	8.9	57.9	89.4	269.9
A3-G	Institutional	130		0.90		0.90		7116	7116	3.10	0.0	0.8	0.3	113.7	9.7	58.1	89.4	270.9
A4	Low Density Residential	130			34.44	34.44	95.2	3279	10395	2.94	0.0	0.0	9.6	113.7	9.7	67.8	123.7	314.9
A2-C	Commercial (ex. snow dump)	120	15.25			15.25		10395	10395	2.94	13.2	0.0	4.3	127.0	9.7	72.0	123.7	332.4
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	10974	2.91	0.0	0.0	1.7	127.0	9.7	73.7	129.6	340.0
A5	Commercial	110	17.72			17.72		10974	10974	2.91	15.4	0.0	5.0	142.4	9.7	78.7	129.6	360.3
A6-A	Commercial	100	15.18			15.18		10974	10974	2.91	13.2	0.0	4.3	155.5	9.7	82.9	129.6	377.8
A6-B	Institutional	100		6.05		6.05		10974	10974	2.91	0.0	5.3	1.7	155.5	15.0	84.6	129.6	384.7
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	11763	2.88	0.0	0.0	1.4	155.5	15.0	86.0	137.4	393.9
A6-D	Low Density Residential	90			17.56	17.56	95.2	1672	13435	2.83	0.0	0.0	4.9	155.5	15.0	90.9	153.8	415.2
A6-E	Low Density Residential	90			6.94	6.94	95.2	661	14096	2.81	0.0	0.0	1.9	155.5	15.0	92.9	160.2	423.6
A7-A	Commercial	90	13.62			13.62		14096	14096	2.81	11.8	0.0	3.8	167.4	15.0	96.7	160.2	439.2
A7-B	High Density Residential	90			11.01	11.01	135.0	1486	15582	2.76	0.0	0.0	3.1	167.4	15.0	99.8	174.3	456.4
A7-C	Medium Density Residential	90			6.97	6.97	162.0	1129	16711	2.73	0.0	0.0	2.0	167.4	15.0	101.7	184.9	468.9
A7-D	Medium Density Residential	90			11.74	11.74	162.0	1902	18613	2.68	0.0	0.0	3.3	167.4	15.0	105.0	202.4	489.7
A7-E1/E2	Medium Density Residential	90			9.24	9.24	162.0	1497	20110	2.65	0.0	0.0	2.6	167.4	15.0	107.6	215.9	505.8
A8-A	Commercial	80	28.45			28.45		20110	20110	2.65	24.7	0.0	8.0	192.0	15.0	115.5	215.9	538.5
A8-B	High Density Residential	80			39.34	39.34	135.0	5311	25421	2.55	0.0	0.0	11.0	192.0	15.0	126.6	262.4	596.0
A8-C	Institutional	80		10.52		10.52		25421	25421	2.55	0.0	9.1	2.9	192.0	24.1	129.5	262.4	608.1
A8-D	Low Density Residential	80			16.87	16.87	120.9	2040	27461	2.52	0.0	0.0	4.7	192.0	24.1	134.2	279.8	630.2
ROW Along SNC Sewer Alignment	-	80				14.34			27461	2.52	0.0	0.0	4.0	192.0	24.1	138.2	279.8	634.2
TOTAL		80	221.24	27.77	230.38	493.73	-	27461	27461	2.52	192.0	24.1	134.2	192.0	24.1	138.2	279.8	634.2

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 1.0
- Institutional / Commercial Peaking Factor = 1.5

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

THIS PRIOR NOVATECH SNC DESIGN SHEET HAD DESIGN FLOWS AT 423.6 L/S AFTER AREA ID "A6-E".

THE DSEL EVALUATION WITH NEW PARAMTERS AT THIS SAME NODE WITH CONSERVANCY WEST AND EAST INCLUDED IS ~401.58 < 423.6 L/S

Attachment 3
Sanitary Sewer Design Sheets (Phase 2)

**SOUTH NEPEAN COLLECTOR (PHASE 2)
SANITARY SEWER DESIGN SHEET**

MAY 26, 2016
JOB# 115075



LOCATION			Area					Population				Individual Design Flows			Cumulative Design Flows				PROPOSED SEWER							
From MH	To MH	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Right-of-Way (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.28 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (350 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)	Length (m)	Pipe Size (mm)	Type	Slope %	Capacity (L/s)	Full Flow Velocity (m/s)	Ratio (Q/Qfull)
MHSA 1	MHSA 2	90	192.79	17.25	174.17	0.00	384.21	1678	20110	20110	2.65	167.352	14.97	107.58	167.4	15.0	107.6	215.9	505.8	57.3	900	CONC	0.10	597.2	0.91	85%
MHSA 2	MHSA 3	90																	505.8	57.3	900	CONC	0.10	597.2	0.91	85%
MHSA 3	MHSA 4	90																	505.8	73.9	900	CONC	0.10	597.2	0.91	85%
MHSA 4	MHSA 5	90																	505.8	34.6	900	CONC	0.10	597.2	0.91	85%
MHSA 5	MHSA 6	90																	505.8	42.8	900	CONC	0.10	597.2	0.91	85%
MHSA 6	MHSA 7	90																	505.8	84.4	900	CONC	0.10	597.2	0.91	85%
MHSA 7	MHSA 8	90																	505.8	16.5	900	CONC	0.10	597.2	0.91	85%
MHSA 8	MHSA 9	90																	505.8	85.4	900	CONC	0.10	597.2	0.91	85%
MHSA 9	MHSA 10	90																	505.8	70.6	900	CONC	0.10	597.2	0.91	85%
MHSA 10	MHSA 11	90																	505.8	70.6	900	CONC	0.10	597.2	0.91	85%
MHSA 11	MHSA 12	90																	505.8	77.8	900	CONC	0.10	597.2	0.91	85%
MHSA 12	MHSA 13	90																	505.8	77.8	900	CONC	0.10	597.2	0.91	85%
MHSA 13	MHSA 14	90																	505.8	77.8	900	CONC	0.10	597.2	0.91	85%
MHSA 14	MHSA 15	90																	505.8	25.4	900	CONC	0.10	597.2	0.91	85%
MHSA 15	MHSA 16	90																	505.8	34.2	900	CONC	0.10	597.2	0.91	85%
MHSA 16	MHSA 17	90																	505.8	86.7	900	CONC	0.10	597.2	0.91	85%
MHSA 17	MHSA 18	90																	505.8	34.3	900	CONC	0.10	597.2	0.91	85%
MHSA 18	MHSA 19	90																	505.8	68.6	900	CONC	0.10	597.2	0.91	85%
MHSA 19	MHSA 20	90																	505.8	65.5	900	CONC	0.10	597.2	0.91	85%
MHSA 20	MHSA 21	80	221.24	27.77	230.38	14.34	493.73	256	7351	27461	2.52	192.049	24.11	138.24	192.0	24.1	138.2	279.8	634.2	18.2	1050	CONC	0.10	900.9	1.01	70%
MHSA 21	MHSA 22	80																	634.2	81.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 22	MHSA 23	80																	634.2	84.7	1050	CONC	0.10	900.9	1.01	70%
MHSA 23	MHSA 24	80																	634.2	77.4	1050	CONC	0.10	900.9	1.01	70%
MHSA 24	MHSA 25	80																	634.2	45.5	1050	CONC	0.10	900.9	1.01	70%
MHSA 25	MHSA 26	80																	634.2	35.8	1050	CONC	0.10	900.9	1.01	70%
MHSA 26	MHSA 27	80																	634.2	83.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 27	MHSA 28	80																	634.2	74.4	1050	CONC	0.10	900.9	1.01	70%
MHSA 28	MHSA 29	80																	634.2	77.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 29	MHSA 30	80																	634.2	83.8	1050	CONC	0.10	900.9	1.01	70%
MHSA 30	MHSA 31	80																	634.2	42.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 31	MHSA 32	80																	634.2	100.6	1050	CONC	0.10	900.9	1.01	70%
MHSA 32	MHSA 33	80																	634.2	13.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 33	MHSA 34	80																	634.2	99.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 34	MHSA 35	80																	634.2	99.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 35	MHSA 36	80																	634.2	88.7	1050	CONC	0.10	900.9	1.01	70%
MHSA 36	MHSA 37	80																	634.2	88.8	1050	CONC	0.10	900.9	1.01	70%
MHSA 37	MHSA 38	80																	634.2	90.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 38	MHSA 39	80																	634.2	87.5	1050	CONC	0.10	900.9	1.01	70%

Design Parameters:

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles / semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row / townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

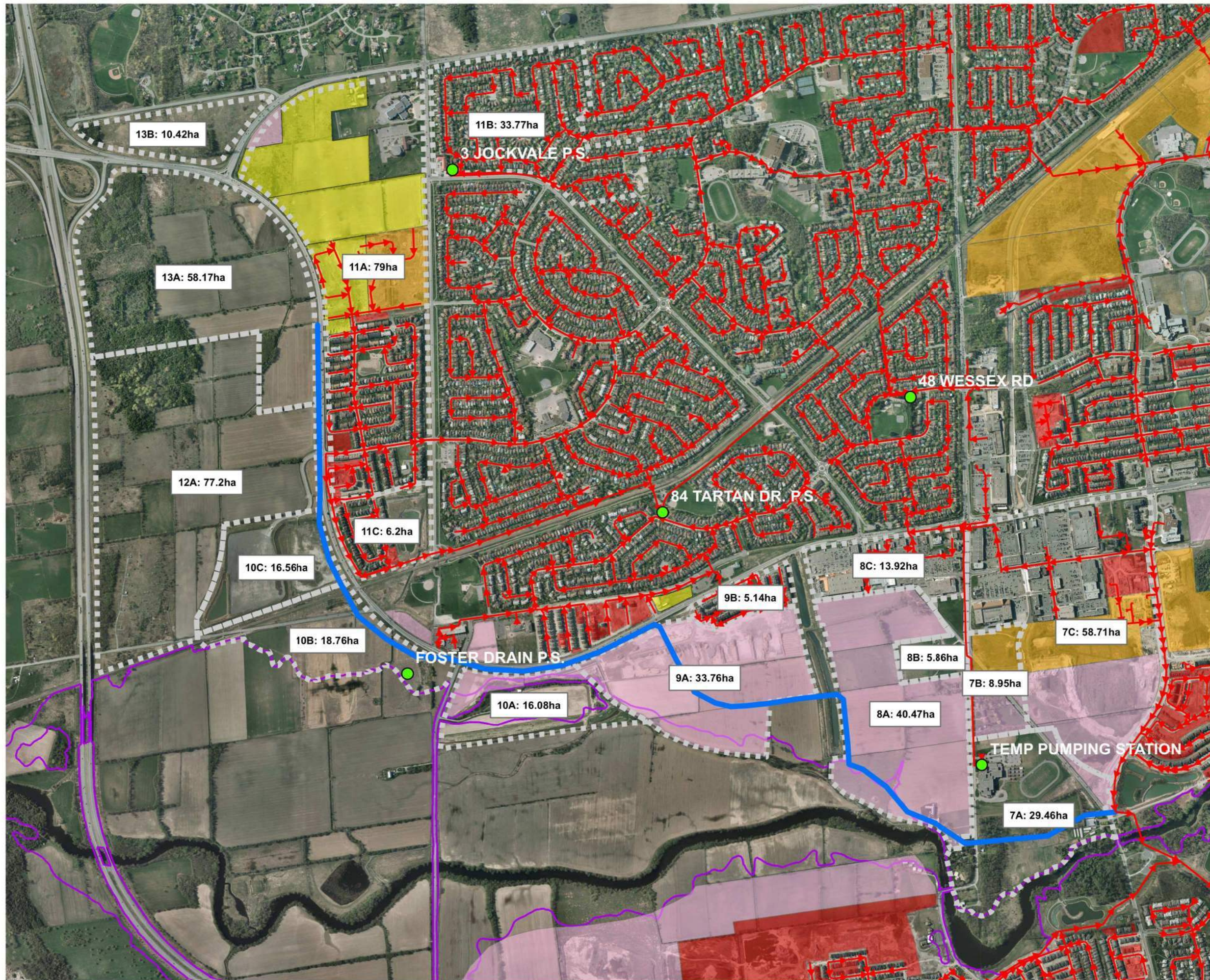
Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 1.0
- Institutional / Commercial Peaking Factor = 1.5

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

Figure 01
Existing Sanitary Network and Collection Areas



- Pump Station
 - Existing Sanitary Main (With Flow Direction)
 - Proposed Alignment for South Nepean Collector
 - Collection Area
- DEVELOPMENT STATUS**
- Registered
 - Draft Approved
 - Pending
 - No Plan
 - Floodplain

NOT TO SCALE



MAP DRAWING INFORMATION:
DATA PROVIDED BY THE CITY OF OTTAWA

MAP CREATED BY: BC
MAP CHECKED BY: MBM
MAP PROJECTION: NO PROJECTION

FILE LOCATION: \\Dillon.ca\dillon_dfs\Ottawa\Ottawa_CA\CAD\2011\115681\Design_GIS\MXD\Figure01c_ExistingSanitaryNetwork.mxd

Table 5.1: Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area

Collection Area	Discharging Node	Estimated from GIS			City of Ottawa VURL Data			Other Space ¹ (ha)	Population (PE)	Residential Density (PE/net ha)	Comments	Additional Source(s)
		Gross Institutional/Commercial (ha)	Gross Residential (ha)	Gross Area (ha)	Net Residential (ha)	Units (#)	Unit Density (#/ha)					
7A	70	13.5	7.4	29.5	4.0	605	0.3	9.1	1637	4.25	Flow calculations include St Joseph H.S. Pump Station firm capacity of 7.0 L/s Additional 600 units (TAC)	3.4ppu (TAC)
7B		0.0	9.24	9.24	6.23	1474	136.7	3.0	3321	638.8	Population from split VURL allocated by area. VURL parcel id 323 - inconsistency between net and gross reported area.	2.7ppu (TAC)
8A		0.0	40.0	40.0	24.1	4462	185.1	15.9	12047.4	499.9		2.7ppu (TAC)
8B		5.9	0.0	5.9	0.0	0	0	0.0			Future Commercial area	
8C		13.9	0.0	13.9	0.0	0	0	0.0			Commercial area includes Home Depot	
9A	80	0.0	33.8	33.8	18.6	635	34.1	15.2	2210	116.2		3.4ppu (TAC)
10A	90	0.0	16.1	16.1	9.7	451	28.0	6.4	1533.4	158.0	Assume net population = 60% gross.	3.4ppu (TAC)
10B	100	18.8	0.0	35.3	0.0	0	0	16.5			Allocated as potential future I/C use as directed by TAC	
10C	110	16.6	0.0	35.3	0.0	0	0	18.7			Area includes current Municipal Snow Dump. Flow allowance is made for potential future I/C use	
11C		0.0	6.2	6.2	Note 2			2.5	306	82.7	This area is south of '11 block' in the existing development	From IBI Apr 2010 Report Figure 1
11A	120	12.5	66.5	79.0				26.6	3923	98.3	Institutional includes 4.38ha church site and 6.89 ha institution at northeast corner, as well and Claridge Commercial (0.56ha) and DCR/Phoenix Commercial (0.64ha)	From IBI Apr2010 Report Figure 1
11B		0.0	37.0	37.0				14.8	1550	69.8	Presently serviced by Jockvale pump station; to be redirected to SNC.	Estimated from 2011 Census Block data
12A		77.2	0.0	77.2				0.0			Allow sanitary peak flow 79.0 L/s	Novatech, Employment Lands Report, Revised Jan 2012
13A	130	58.5	0.0	58.5				0.0			Allow sanitary peak flow 62.8 L/s plus Collection Area 13B, total 82.2 L/s	
13B		12.5	0.0	12.5			0.0			Allow sanitary peak flow 19.4 L/s; gravity discharge to Collection Area 13A	IBI/Novatech	

Notes:

1. Other space includes other residential space accounting for the difference between gross area (measured with GIS) and net area (provided in VURL data), such as sidewalks, roads, greenspace, etc.
2. Collection Area 11A and 11B population and land use as identified under Additional Source(s). Other space reported as 60% of gross residential area, consistent with VURL average.

SOUTH NEPEAN COLLECTOR SEWER
SANITARY SEWER DESIGN SHEET - Operational Service (Average Flow Design Parameters)

Sheet 1 of 1

TRIBUTARY AREA	Design Factors	LOCATION		AREA (ha)						INDIVIDUAL		CUMULATIVE		RESIDENTIAL		COMMERCIAL & INSTITUTION			INFL. INFLOW	PEAK DESIGN	PROPOSED SEWER							OPERATIONAL DESIGN								
		FROM	TO	Gross ICI	Net ICI	Other ICI space (Green, Sidewalks, roads)	Gross RESIDENTIAL Area	Net Residential Area	Other Res (Green, Sidewalks, roads)	TOTAL AREA (Gross ICI plus Gross Residential)	POP	DENSITY (po./ha.)	POP	AREA (ha.)	PEAKING FACTOR	RESIDENT. FLOW (L/s)	PEAKING FACTOR	CUM. AREA	I.C.I. FLOW (l/s)	Q(p)	FLOW Q(d)	LENGTH (m)	GROUND ELEVATION (m)	DEPTH OF COVER (m)	PIPE SIZE (m)	INVERT 1 (m)	INVERT 2 (m)	PIPE TYPE	GRADE	CAPACITY (L/s)	Q(d)/Q(c)	VELOCITY at capacity (m/s)	DEPTH (m)	VELOCITY (m/s)		
13A	1			0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	4.50	0.00	1.00	0.00	0.00	0.00	0.00																
13B	1		Node 130	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	4.50	0.00	1.00	0.00	0.00	0.00	0.00		95.14		5.43												
12A	1		Node 130	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	4.50	0.00	1.00	0.00	0.00	0.00	0.00																
11A	1			12.5	9.4	3.1	66.5	8.0	58.5	1196	148.76	1196	79.0	3.75	15.57	1.00	12.50	2.00	3.95	21.52																
11B	1		Node 120	0.0	0.0	0.0	37.0	22.2	14.8	1550	69.82	2746	116.0	3.47	33.13	1.00	12.50	2.00	5.80	40.93	531.89	93.60	4.42	0.750	88.96	88.43	Conc.	0.10%	353.24	0.13	0.80	0.20	0.58			
11C	1		Node 120	0.0	0.0	0.0	6.2	3.7	2.5	306	82.26	3052	122.2	3.44	36.41	1.00	12.50	2.00	6.11	44.92																
10C	1		Node 110	16.6	12.5	4.2	0.0	0.0	0.0	0		0	16.6	3.44	36.41	1.00	29.10	4.66	6.94	48.01	497.82	93.44	4.76	0.750	88.43	87.93	Conc.	0.10%	353.24	0.14	0.80	0.20	0.58			
10B	1		Node 110	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	3.44	36.41	1.00	29.10	4.66	6.94	48.01	603.17	93.03	4.95	0.750	87.93	87.33	Conc.	0.10%	353.24	0.14	0.80	0.20	0.58			
10A	1		Node 100	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	3.44	36.41	1.00	29.10	4.66	6.94	48.01	430.49	93.75	6.03	0.825	87.33	86.90	Conc.	0.10%	455.17	0.11	0.85	0.21	0.61			
9A	1		Node 90	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	3.44	36.41	1.00	29.10	4.66	6.94	48.01	1268.65	92.37	5.84	0.900	86.90	85.63	Conc.	0.10%	573.71	0.08	0.90	0.18	0.56			
8A	1		Node 80	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	3.44	36.41	1.00	29.10	4.66	6.94	48.01																
8B	1			5.9	4.4	1.5	0.0	0.0	0.0	0		0	5.9	3.44	36.41	1.00	35.00	5.60	7.24	49.25																
8C	1			13.9	10.4	3.5	0.0	0.0	0.0	0		0	13.9	3.44	36.41	1.00	48.90	7.82	7.93	52.17																
7A	1			13.5	10.1	3.4	16.0	1.4	14.6	17	12.14	3069	188.1	3.43	36.59	1.00	62.40	9.98	9.41	55.93																
7B	1		Node 70	0.0	0.0	0.0	0.0	0.0	0.0	0		0	0.0	3.43	36.59	1.00	62.40	9.98	9.41	55.93	1448.98	91.24	6.01	1.050	85.63	84.18	Conc.	0.10%	864.51	0.06	1.00	0.18	0.56			
									188.1																											
									DEFAULTS																											
									q=AVERAGE DAILY FLOW	300	L/CAP.D																									
									l=UNIT OF PEAK EXTR.FLOW	0.050	L/ha.s																									
									Mannings 'n'	0.013																										
									q=AVERAGE COMMERCIAL AND INSTITUTIONAL	0.16	L/ha.s																									
DESIGN		DJG																																		
CHECKED																																				
TODAY:		7/18/2012																																		



Project 11-5681

**SOUTH NEPEAN COLLECTOR SEWER
SANITARY SEWER DESIGN SHEET - Full Service (Peak Flow Design Parameters)**

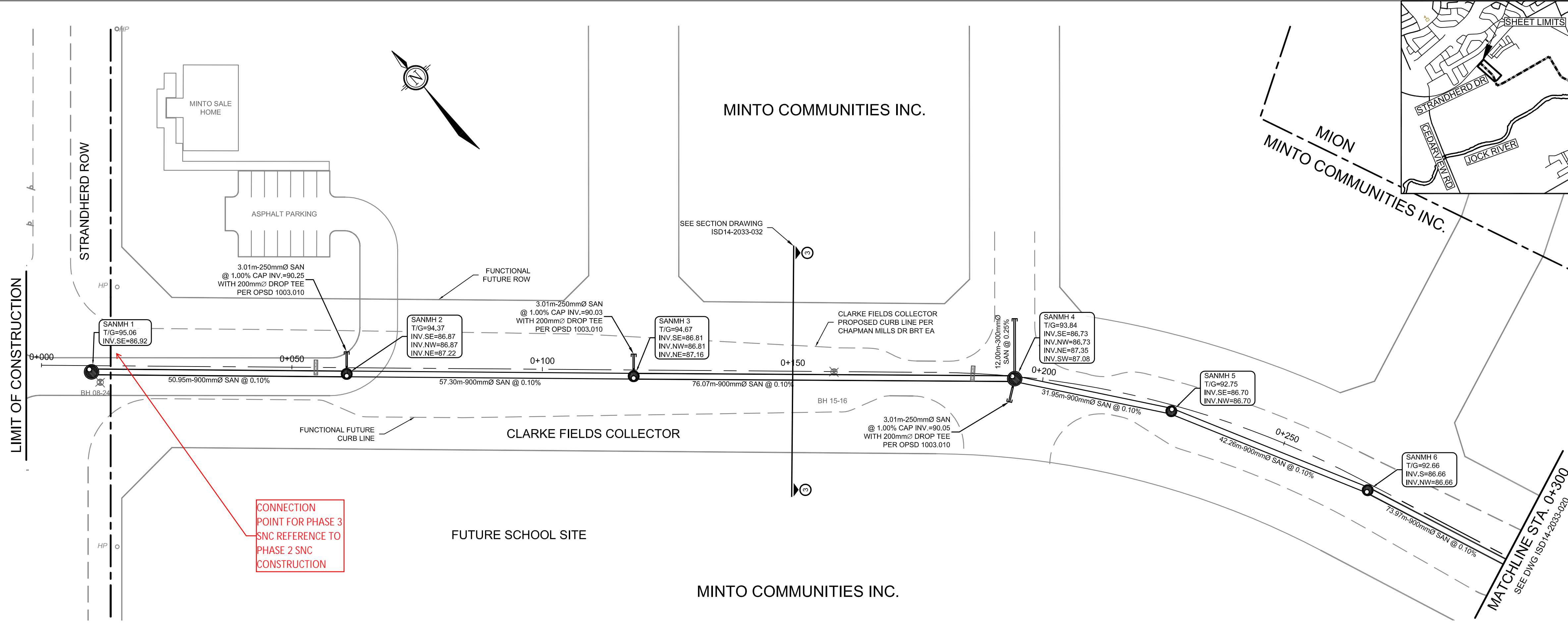
TRIBUTARY AREA	Design Factors	LOCATION		AREA (ha)							INDIVIDUAL		CUMULATIVE		RESIDENTIAL		COMMERCIAL & INSTITUTION			INFL. INFLOW (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER						PEAK DESIGN									
		FROM	TO	Gross ICI	Net ICI	Other ICI space (Green, Sidewalks, roads)	Gross RESIDENTIAL Area	Net Residential Area	Other Res (Sidewalks, roads)	TOTAL AREA (Gross ICI plus Gross Residential)	POP	DENSITY (pers/net ha.)	POP	Total I/C and Res AREA (ha.)	PEAKING FACTOR	RESIDENT. FLOW (L/s)	PEAKING FACTOR	I/C CUM. AREA	I/C FLOW (l/s)			Q(p) (L/s)	LENGTH (m)	GROUND ELEVATION (m)	DEPTH OF COVER (m)	PIPE SIZE (m)	INVERT 1 (m)	INVERT 2 (m)	PIPE TYPE	GRADE	CAPACITY (L/s)	Q(d)/Q(c)	VELOCITY at capacity (m/s)	DEPTH (m)	VELOCITY (m/s)		
13A	1			58.5	43.9	14.6	0.0	0.0	0.0	58.5	0	0	58.5	4.50	0.00	1.50	58.50	50.90	16.38	67.28																	
13B	1		Node 130	12.5	9.4	3.1	0.0	0.0	0.0	12.5	0	0	71.0	4.50	0.00	1.50	71.00	61.77	19.88	81.65																	
12A	1	Node 130		77.2	57.9	19.3	0.0	0.0	0.0	77.2	0	0	148.2	4.50	0.00	1.50	148.20	128.93	41.50	170.43																	
11A	1			12.5	9.4	3.1	66.5	39.9	26.6	79.0	3923	98.32	3923	227.2	3.34	53.09	1.50	160.70	139.81	63.62	256.52																
11B	1		Node 120	0.0	0.0	0.0	37.0	22.2	14.8	37.0	1550	69.82	5473	264.2	3.21	71.13	1.50	160.70	139.81	73.98	284.92	531.89	93.60	4.42	0.750	88.96	88.43	Conc.	0.10%	353.24	0.81	0.80	0.53	0.90			
11C	1		Node 120	0.0	0.0	0.0	6.2	3.7	2.5	6.2	306	82.26	5779	270.4	3.19	74.59	1.50	160.70	139.81	75.71	290.11																
10C	1		Node 110	16.6	12.5	4.2	0.0	0.0	0.0	16.6	0	0	5779	287.0	3.19	74.59	1.50	177.30	154.25	80.36	309.20	497.82	93.44	4.76	0.750	88.43	87.93	Conc.	0.10%	353.24	0.88	0.80	0.55	0.91			
10B	1		Node 110	18.8	14.1	4.7	0.0	0.0	0.0	18.8	0	0	5779	305.8	3.19	74.59	1.50	196.10	170.61	85.62	330.82	603.17	93.03	4.95	0.750	87.93	87.33	Conc.	0.10%	353.24	0.94	0.80	0.58	0.92			
10A	1		Node 100	0.0	0.0	0.0	16.1	9.7	6.4	16.1	1533	158.04	7312	321.9	3.09	91.48	1.50	196.10	170.61	90.13	352.22	430.49	93.75	6.03	0.825	87.33	86.90	Conc.	0.10%	455.17	0.77	0.85	0.55	0.95			
9A	1		Node 90	0.0	0.0	0.0	33.8	18.6	15.2	33.8	2161	116.18	9473	355.7	2.98	114.28	1.50	196.10	170.61	99.60	384.48	1268.65	92.37	5.84	0.900	86.90	85.63	Conc.	0.10%	573.71	0.67	0.90	0.55	0.97			
8A	1		Node 80	0.0	0.0	0.0	40.0	24.1	15.9	40.0	12047	499.88	21520	395.7	2.62	228.45	1.50	196.10	170.61	110.80	509.85																
8R	1			5.9	4.4	1.5	0.0	0.0	0.0	5.9	0	0	21520	401.6	2.62	228.45	1.50	202.00	175.74	112.45	516.64																
8C	1			13.9	10.4	3.5	0.0	0.0	0.0	13.9	0	0	21520	415.5	2.62	228.45	1.50	215.90	187.83	116.34	532.62																
7A	1			13.5	10.1	3.4	16.5	5.2	11.3	30.0	1637	314.81	23157	445.5	2.59	242.84	1.50	229.40	199.58	124.74	567.6																
7B	1		Node 70	0.0	0.0	0.0				9.2	3980	638.84	27137	454.7	2.52	277.05	1.50	229.40	199.58	127.32	603.94	1448.98	91.24	6.01	1.050	85.63	84.18	Conc.	0.10%	864.51	0.70	1.00	0.64	1.07			
							225.3	129.7		454.7		27,137.0										4,781.0															

DESIGN CHECKED TODAY:		DESIGNER	DATE
		DJG	7/18/2012

DEFAULTS		UNIT
q=AVERAGE DAILY FLOW	350	L/CAP.D
I=UNIT OF PEAK EXTR.FLOW	0.280	L/ha.s
Mannings 'n'	0.013	
q=AVERAGE COMMERCIAL AND INSTITUTIONAL	0.58	L/ha.s



Project 11-5681



CITY OF OTTAWA
SOUTH NEPEAN COLLECTOR (SNC)
SEWER PHASE 2 - STRANDHERD DRIVE
TO JOCKVALE ROAD

PLAN AND PROFILE
STA. 0+000 TO 0+300

Contract No. **ISD14-2033** Dwg. No. **019**
Sheet 19 of 51

Asset No. _____ Asset Group **ISD**

Wayne Newell, P. Eng. General Manager
Jonathan Knoyle, P. Eng. Senior Engineer

NOVATECH
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Suite 202, 240 Michael Coopers Drive
Kanata, Ontario, Canada, K2M 1P6
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Facsimile: (613) 254-5867
Email: novatech@novatech-eng.com

PROFESSIONAL ENGINEER
M.A. BISSETT
PROVINCE OF ONTARIO

Des. RJD Chk'd. ERD
Dwn. NCS Chk'd. RJD
Utility Circ. No. _____ Index No. _____
Const. Inspector _____

Scale: HORIZONTAL 1:500
VERTICAL 1:100

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

No.	Description	By	Date (dd/mm/yyyy)
1.	ISSUED FOR PRELIMINARY DESIGN CIRCULATION	ERD	21/12/15
2.	CHANGES TO ORIENTATION ACROSS KB SWM FACILITY	ERD	16/02/16
3.	ISSUED FOR PRELIMINARY DESIGN REPORT	ERD	02/03/16
4.	ISSUED FOR FINAL DESIGN CIRCULATION	ERD	29/04/16
5.	ISSUED FOR MOECC ECA APPLICATION	ERD	26/05/16
6.	ISSUED FOR TENDER	ERD	20/06/16
7.	ISSUED FOR CONSTRUCTION	ERD	30/08/16
8.	SEWER ALIGNMENT SHIFT ON GREENBANK	ERD	16/09/16
9.	REVISED PER MION SERVICING	ERD	08/12/16

LEGEND

EXISTING ITEMS

- WATERMAIN
- WATERMAIN VALVE
- STORM SEWER
- STORM MH
- CATCH BASIN & LEAD
- CULVERT
- STORM STRUCTURE
- SANITARY SEWER
- SANITARY MH

PROPOSED ITEMS

- SANITARY SEWER
- SANITARY MH & LID
- CULVERT
- FUTURE STRUCTURE T/G ADJUSTMENT

- NOTES:**
- CONCRETE PRESSURE PIPE SHALL BE AWWA C301 (L) CL-16. FITTING SHALL BE DESIGNED TO THE SAME CRITERIA AS THE ADJACENT PIPE.
 - CONTRACTOR TO PROVIDE PIPE CLASS CALCULATIONS, AS PER AWWA C304 (DESIGN OF PRESTRESSED CONCRETE CYLINDER PIPE), BY THE PIPE MANUFACTURER, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
 - PIPE EMBEDMENT SHALL BE AS PER CITY OF OTTAWA DETAIL S6. SAND MAY BE USED AS PIPE COVER MATERIAL ABOVE THE SPRINGLINE.
 - A CLOTH DIAPER APPROVED BY THE PIPE MANUFACTURE SHALL BE PLACED AROUND EACH EXTERIOR JOIN RECESS AND FASTENED IN PLACE WITH EITHER WIRE OR STEEL STRAPPING STITCHED INTO ITS EDGES.
 - THE JOINT SHALL BE FILLED WITH MORTAR IN ONCE CONTINUOUS OPERATION AND PATTED OR MANIPULATED TO SETTLE THE MORTAR AND EXPEL AND ENTRAPPED AIR.
 - INTERIOR JOINTS SHALL BE FILLED WITH MORTAR AFTER BACKFILLING AND FINISHED SMOOTH WITH A TROWEL. CEMENT USED SHALL MEET THE REQUIREMENTS OF TYPE HS CEMENT (HIGH-SULPHATE-RESISTANCE), OR APPROVED EQUIVALENT.
 - THE INTERIOR OF THE JOINTS SHALL BE PROTECTED FROM CORROSION WITH EPOXY AND ZINC COATING APPLIED DURING FABRICATION.
 - THE INTERIOR STRUCTURAL CONCRETE CORE SHALL BE MANUFACTURED WITH TYPE HS CEMENT (HIGH-SULPHATE-RESISTANCE), OR APPROVED EQUIVALENT.
 - SEE MANHOLE DETAIL DRAWINGS ISD14-2033-36 TO ISD14-2033-45 FOR ADDITIONAL DETAILS

MAINTENANCE HOLE DATA

MH ID	STATION	OFFSET	STRUCTURE	COVER	T/G ELEV.	LOW. INV.
1	0+010.00	1.25R	OPSD 701.013	S24/S25	95.06	86.92
2	0+060.95	1.25R	OPSD 701.012	S24/S25	94.37	86.87
3	0+118.25	1.25R	OPSD 701.012	S24/S25	94.67	86.81
4	0+194.38	0.46R	OPSD 701.013	S24/S25	93.84	86.73
5	0+226.51	1.25R	OPSD 701.012	S24/S25	92.75	86.70
6	0+269.14	1.25R	OPSD 701.012	S24/S25	92.66	86.66

SANITARY SEWER PIPE DATA

CONNECTED STRUCTURES & INVERTS	DIA (mm)	LENGTH (m)	MATERIAL
SANMH 1 = 86.92 SANMH 2 = 86.87	900	50.95	AWWA C-301 (L)
SANMH 2 = 86.87 SANMH 3 = 86.81	900	57.30	AWWA C-301 (L)
SANMH 3 = 86.81 SANMH 4 = 86.73	900	76.07	AWWA C-301 (L)
SANMH 4 = 86.73 SANMH 5 = 86.70	900	31.95	AWWA C-301 (L)
SANMH 5 = 86.70 SANMH 6 = 86.66	900	42.26	AWWA C-301 (L)
SANMH 6 = 86.66 SANMH 7 = 86.59	900	73.97	AWWA C-301 (L)

STATION	EXISTING ELEVATION	CHAINAGE	DESCRIPTION	INVERT ELEVATION	CHAINAGE	EXISTING ELEVATION
0+000	83.47	0+000			0+000	83.47
0+010	82.76	0+010			0+010	82.76
0+020	82.75	0+020			0+020	82.75
0+030	82.75	0+030			0+030	82.75
0+040	82.75	0+040			0+040	82.75
0+050	82.75	0+050			0+050	82.75
0+060	82.75	0+060			0+060	82.75
0+060.95	82.75	0+060.95	200mm DROP TEE PER OPSD 1003.010	86.87	0+060.95	82.75
0+070	82.75	0+070			0+070	82.75
0+080	82.74	0+080			0+080	82.74
0+090	82.74	0+090			0+090	82.74
0+100	82.74	0+100			0+100	82.74
0+110	82.72	0+110			0+110	82.72
0+118.25	82.67	0+118.25	200mm DRDP TEE PER OPSD 1003.010	86.81	0+118.25	82.67
0+120	82.62	0+120			0+120	82.62
0+130	82.59	0+130			0+130	82.59
0+140	82.59	0+140			0+140	82.59
0+150	82.59	0+150			0+150	82.59
0+160	82.55	0+160			0+160	82.55
0+170	82.51	0+170			0+170	82.51
0+180	82.50	0+180			0+180	82.50
0+190	82.49	0+190			0+190	82.49
0+194.38	82.46	0+194.38	CLAY SEAL PER CITY DETAIL S8	86.73	0+194.38	82.46
0+200	82.46	0+200			0+200	82.46
0+210	82.46	0+210			0+210	82.46
0+220	82.47	0+220			0+220	82.47
0+226.51	82.45	0+226.51	200mm DROP TEE PER OPSD 1003.010	86.70	0+226.51	82.45
0+230	82.45	0+230			0+230	82.45
0+240	82.42	0+240			0+240	82.42
0+250	82.40	0+250			0+250	82.40
0+260	82.38	0+260			0+260	82.38
0+269.14	82.36	0+269.14	CLAY SEAL PER CITY DETAIL S8	86.66	0+269.14	82.36
0+270	82.34	0+270			0+270	82.34
0+280	82.32	0+280			0+280	82.32
0+300	82.29	0+300			0+300	82.29

AS-BUILT

THESE AS-BUILT PLANS HAVE BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE DESIGN PROFESSIONAL HAS NOT VERIFIED THE ACCURACY AND/OR THE COMPLETENESS OF THIS INFORMATION AND SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS WHICH MAY BE INCORPORATED HEREIN AS A RESULT.



CITY OF OTTAWA
SOUTH NEPEAN COLLECTOR (SNC)
SEWER PHASE 2 - STRANDHERD DRIVE
TO JOCKVALE ROAD

PLAN AND PROFILE
STA. 0+300 TO 0+600

Contract No. **ISD14-2033** Dwg. No. **020**
Sheet 20 of 51

Asset No. _____
Asset Group **ISD**

Wayne Newell, P.Eng. General Manager
Jonathan Knoyle, P.Eng. Senior Engineer

NOVATECH
200, 210 Milled Cupfield Drive
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Fax: (613) 254-9817
Email: novatech@novatech.ca

Scale: HORIZONTAL 1:500
VERTICAL 1:100

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

No.	Description	By	Date (dd/mm/yyyy)
1	ISSUED FOR PRELIMINARY DESIGN CIRCULATION	FRN	21/12/15
2	CHANGES TO ORIENTATION ACROSS KB SWM FACILITY	ERD	16/02/16
3	ISSUED FOR PRELIMINARY DESIGN REPORT	ERD	02/03/16
4	ISSUED FOR FINAL DESIGN CIRCULATION	ERD	29/04/16
5	ISSUED FOR MOECC ECA APPLICATION	ERD	26/05/16
6	ISSUED FOR TENDER	ERD	20/06/16
7	ISSUED FOR CONSTRUCTION	ERD	30/08/16
8	SEWER ALIGNMENT SHIFT ON GREENBANK	ERD	16/09/16
9	REVISED PER MION SERVICING	ERD	08/12/16
10	MINTO LANDS MANHOLE UPDATE	ERD	24/04/17
11	ISSUED FOR AS-BUILT	ERD	28/09/17

LEGEND

EXISTING ITEMS
WATERMAIN
WATERMAIN VALVE
STORM SEWER
STORM MH
CATCH BASIN & LEAD
CULVERT
STORM STRUCTURE
SANITARY SEWER
SANITARY MH

PROPOSED ITEMS
SANITARY SEWER
SANITARY MH & LID
CULVERT
FUTURE STRUCTURE T/G
ADJUSTMENT

NOTES:

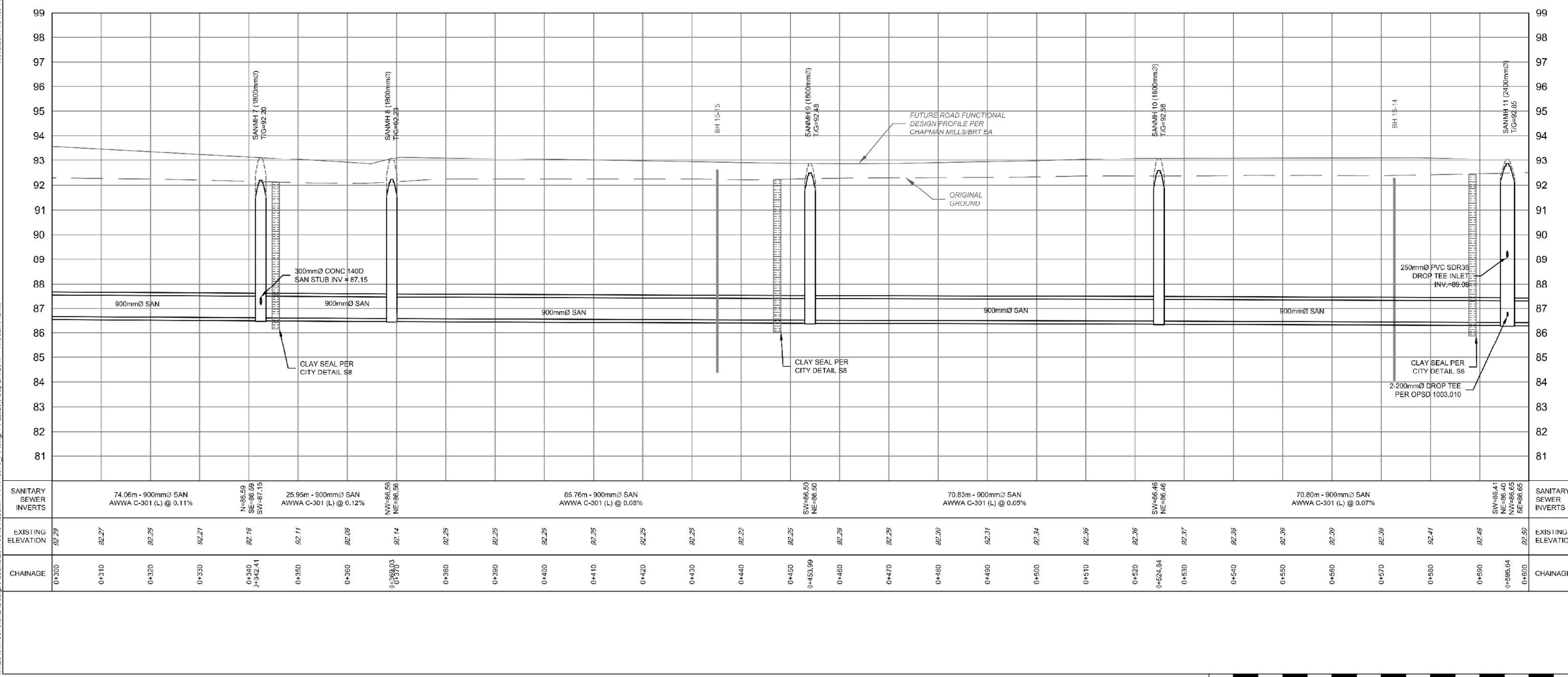
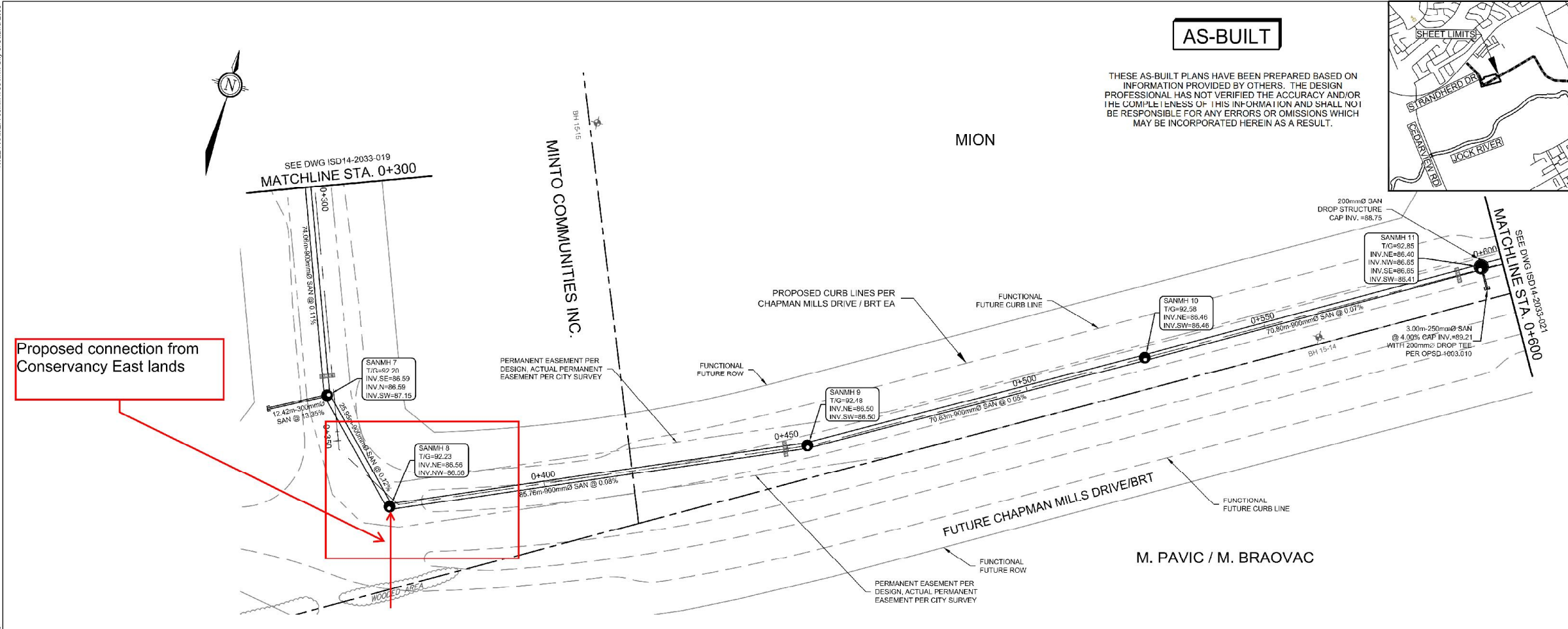
- CONCRETE PRESSURE PIPE SHALL BE AWWA C301 (L) CL-16. FITTING SHALL BE DESIGNED TO THE SAME CRITERIA AS THE ADJACENT PIPE.
- CONTRACTOR TO PROVIDE PIPE CLASS CALCULATIONS, AS PER AWWA C304 (DESIGN OF PRESTRESSED CONCRETE CYLINDER PIPE), BY THE PIPE MANUFACTURER. SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
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- SEE MANHOLE DETAIL DRAWINGS ISD14-2033-36 TO ISD14-2033-45 FOR ADDITIONAL DETAILS

MAINTENANCE HOLE DATA

MH ID	STATION	OFFSET	STRUCTURE	COVER	T/G ELEV	LOW. INV.
7	0+342.41	1.41R	OPSD 701.012	S24/S25	92.20	86.59
8	0+389.03	1.50R	OPSD 701.012	S24/S25	92.23	86.56
9	0+453.99	0.07L	OPSD 701.012	S24/S25	92.48	86.50
10	0+524.84	0.04L	OPSD 701.012	S24/S25	92.58	86.46
11	0+595.64	0.41L	OPSD 701.013	S24/S25	92.85	86.40

SANITARY SEWER PIPE DATA

CONNECTED STRUCTURES & INVERTS	DIA (mm)	LENGTH (m)	MATERIAL
SANMH 6 = 86.68 SANMH 7 = 86.59	900	74.06	AWWA C-301 (L)
SANMH 7 = 86.59 SANMH 8 = 86.56	900	25.95	AWWA C-301 (L)
SANMH 8 = 86.56 SANMH 9 = 86.50	900	85.76	AWWA C-301 (L)
SANMH 10 = 86.46 SANMH 9 = 86.50	900	70.83	AWWA C 301 (L)
SANMH 10 = 86.46 SANMH 11 = 86.41	900	70.80	AWWA C-301 (L)
SANMH 11 = 86.40 SANMH 12 = 86.32	900	78.15	AWWA C-301 (L)



STATION	EXISTING ELEVATION	CHAINAGE	STRUCTURE	COVER	T/G ELEV	LOW. INV.
0+300	82.29	0+300	SANITARY SEWER INVERTS			
0+310	82.27	0+310				
0+320	82.25	0+320				
0+330	82.21	0+330				
0+340	82.16	0+340				
0+350	82.08	0+350				
0+360	82.05	0+360				
0+370	82.05	0+370				
0+380	82.25	0+380				
0+390	82.25	0+390				
0+400	82.25	0+400				
0+410	82.25	0+410				
0+420	82.25	0+420				
0+430	82.25	0+430				
0+440	82.22	0+440				
0+450	82.25	0+450				
0+460	82.28	0+460				
0+470	82.29	0+470				
0+480	82.30	0+480				
0+490	82.37	0+490				
0+500	82.34	0+500				
0+510	82.39	0+510				
0+520	82.36	0+520				
0+530	82.37	0+530				
0+540	82.39	0+540				
0+550	82.39	0+550				
0+560	82.39	0+560				
0+570	82.39	0+570				
0+580	82.46	0+580				
0+590	82.59	0+590				
0+600	82.59	0+600	SANITARY SEWER INVERTS			

TITLE FRAME: 700mm x 634mm, City of Ottawa 2008
Novatech File No. 115075
M:\2015\115075\CADD\Design\Adv\115075_P2.dwg, P2:2020, Sep 27, 2017, 10:08am, nsm

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION						COMM		INSTIT		PARK		C+H		INFILTRATION				PIPE							
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	POP.	CUMULATIVE		PEAK FLOW (l/s)	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL.		
						AREA (ha)	POP.																			(FULL) (m/s)	(ACT.) (m/s)	
Centerline11 - 11																												
			0.21		22	0.21	22				0.00	0.00		0.00		0.21	0.21											
	504A	505A	0.27		28	0.48	50	3.7	0.59		0.00	0.00		0.00	0.00	0.27	0.48	0.14	0.73	85.5	200	0.65	26.44	0.03	0.84	0.36		
	505A	506A	0.13		14	0.61	64	3.6	0.75		0.00	0.00		0.00	0.00	0.13	0.61	0.17	0.93	10.5	200	0.35	19.40	0.05	0.62	0.31		
	506A	508A	0.17		18	0.78	82	3.6	0.96		0.00	0.00		0.00	0.00	0.17	0.78	0.22	1.18	40.0	250	0.25	29.73	0.04	0.61	0.29		
Contribution From Centerline2 - 02, Pipe 507A - 508A						0.41	43				0.00	0.00		0.00		0.41	1.19											
Contribution From Centerline4 - 04, Pipe 503A - 509A						1.67	174				0.00	0.00		0.00		1.67	3.05		2.07	47.5	250	0.25	29.73	0.07	0.61	0.34		
	509A	510A	0.20		21	3.25	340	3.4	3.79		0.00	0.00		0.00	0.00	0.20	3.25	0.93	4.72	48.0	250	0.25	29.73	0.16	0.61	0.44		
	510A	511A	0.03		0	3.28	340	3.4	3.79		0.00	0.00		0.00	0.00	0.03	3.28	0.94	4.73	12.5	250	0.25	29.73	0.16	0.61	0.44		
	511A	519A	0.09		0	3.37	340	3.4	3.79		0.00	0.00		0.00	0.00	0.09	3.37	0.96	4.76	59.5	250	0.25	29.73	0.16	0.61	0.44		
To Centerline1 - 01, Pipe 519A - 521A						3.37	340				0.00	0.00		0.00		3.37												
Centerline10 - 10																												
	512A	513A	0.65		54	0.65	54	3.6	0.64		0.00	0.00		0.00	0.00	0.65	0.65	0.19	0.82	37.5	200	0.65	26.44	0.03	0.84	0.38		
	513A	535A	0.23		34	0.88	88	3.6	1.03		0.00	0.00		0.00	0.00	0.23	0.88	0.25	1.28	48.0	200	0.35	19.40	0.07	0.62	0.34		
To Centerline3 - 03, Pipe 535A - 518A						0.88	88				0.00	0.00		0.00		0.88												
	536A	514A	0.19		34	0.19	34	3.7	0.41		0.00	0.00		0.00	0.00	0.19	0.19	0.05	0.46	47.5	200	0.65	26.44	0.02	0.84	0.32		
	514A	515A	0.15		12	0.34	46	3.7	0.55		0.00	0.00		0.00	0.00	0.15	0.34	0.10	0.64	11.0	200	0.35	19.40	0.03	0.62	0.28		
	515A	519A	0.63		52	0.97	98	3.6	1.14		0.00	0.00		0.00	0.00	0.63	0.97	0.28	1.42	119.5	250	0.25	29.73	0.05	0.61	0.31		
To Centerline1 - 01, Pipe 519A - 521A						0.97	98				0.00	0.00		0.00		0.97												
Centerline3 - 03																												
Contribution From Centerline10 - 10, Pipe 513A - 535A						0.88	88				0.00	0.00		0.00		0.88	0.88											
	535A	518A	0.54		45	1.42	133	3.6	1.54		0.00	0.00		0.00	0.00	0.54	1.42	0.41	1.94	109.0	250	0.25	29.73	0.07	0.61	0.34		
To Centerline1 - 01, Pipe 518A - 519A						1.42	133				0.00	0.00		0.00		1.42												
Centerline1 - 01																												
			0.33		27	0.33	27			13.70	0.00	3.47	3.47			17.50	17.50											
			0.43		45	0.76	72			13.70	0.00	3.47	3.47			0.43	17.93											
	516A	518A	36.45		3771	37.21	3843	2.9	35.86	13.70	0.00	3.47	4.81			36.45	54.38	15.55	56.22	112.5	450	0.15	110.42	0.51	0.69	0.70		
Contribution From Centerline3 - 03, Pipe 535A - 518A						1.42	133				0.00	0.00		0.00		1.42	55.80											
	518A	519A	0.24		25	38.87	4001	2.9	37.17	13.70	0.00	3.47	4.81			0.24	56.04	16.03	58.01	63.5	525	0.11	142.64	0.41	0.66	0.62		
Contribution From Centerline11 - 11, Pipe 511A - 519A						3.37	340				0.00	0.00		0.00		3.37	59.41											
Contribution From Centerline10 - 10, Pipe 515A - 519A						0.97	98				0.00	0.00		0.00		0.97	60.38											
	519A	521A	0.19		20	43.40	4459	2.8	40.93	13.70	0.00	3.47	4.81			0.19	60.57	17.32	63.07	59.0	525	0.10	136.00	0.46	0.63	0.61		
Contribution From Centerline8 - 08, Pipe 520A - 521A						1.02	86				0.00	0.00		0.00		1.02	61.59											
	521A	531A	0.20		21	44.62	4566	2.8	41.80	13.70	0.00	3.47	4.81			0.20	61.79	17.67	64.29	59.5	525	0.10	136.00	0.47	0.63	0.62		
Contribution From Centerline14 - 14, Pipe 528A - 531A						1.82	190				1.47	0.00	0.64			3.93	65.72											
Contribution From Centerline13 - 13, Pipe 534A - 531A						1.08	140				0.00	0.00		0.00		1.08	66.80											
	531A	532A	0.10		0	47.62	4896	2.8	44.47	15.17	0.00	4.11	5.36		0.10	66.90	19.13	68.96	55.0	525	0.10	136.00	0.51	0.63	0.63			

Lower flows than what was previously projected (77.81 L/s) in the Phase 2, 3, & 4 sanitary design sheet downstream.

DESIGN PARAMETERS										Designed: A.S.					PROJECT: Barrhaven Conservancy East Phase 5									
Park Flow = 9300 L/ha/da 0.10764 I/s/ha Average Daily Flow = 280 l/p/day Comm/Inst Flow = 28000 L/ha/da 0.3241 I/s/ha Industrial Flow = 35000 L/ha/da 0.40509 I/s/ha Max Res. Peak Factor = 4.00 Commercial/Inst./Park Peak Factor = 1.00 Institutional = 0.32 I/s/ha										Industrial Peak Factor = as per MOE Graph Extraneous Flow = 0.286 L/s/ha Minimum Velocity = 0.600 m/s Manning's n = (Conc) 0.013 (Pvc) 0.013 Townhouse coeff= 2.7 Single house coeff= 3.4					Checked: W.L./V.W. Dwg. Reference: Sanitary Drainage Plan, Dwg. No.					LOCATION: City of Ottawa File Ref: Date: 01 Dec 2022 Sheet No. 2 of 2				

SANITARY SEWER CALCULATION SHEET

Manning's n=0.013

LOCATION				RESIDENTIAL AREA AND POPULATION							COMM		INSTIT		PARK		C+I-I		INFILTRATION			PIPE									
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	UNITS Singles	UNITS Townhouse	POP.	AREA (ha)	POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL. (FULL) (m/s)	(ACT.) (m/s)		
Elation Heights																															
	112A	113A	0.05	1	1		4	0.05	4	3.76	0.05		0.00	0.00	0.00	0.00	0.00	0.05	0.05	0.02	0.07	15.5	200	0.65	26.44	0.00	0.84	0.17			
To Canoe Street, Pipe 113A - 118A																															
	110A	109A	0.16	1	1		4	0.16	4	3.76	0.05		0.00	0.00	0.00	0.00	0.00	0.16	0.16	0.05	0.10	13.5	200	0.65	26.44	0.00	0.84	0.20			
	109A	117A	0.58	15	15		51	0.74	55	3.64	0.65		0.00	0.00	0.00	0.00	0.00	0.58	0.74	0.24	0.89	103.0	250	0.25	29.73	0.03	0.61	0.27			
To Peninsula Road, Pipe 117A - 118A																															
Jollity Crescent																															
	104A	105A	0.39	10	10		34	0.39	34	3.68	0.41		0.00	0.00	0.00	0.00	0.00	0.39	0.39	0.13	0.53	69.0	200	0.80	29.34	0.02	0.93	0.35			
To Canoe Street, Pipe 105A - 108A																															
	106A	107A	0.15	1	1		4	0.15	4	3.76	0.05		0.00	0.00	0.00	0.00	0.00	0.15	0.15	0.05	0.10	12.0	200	0.70	27.44	0.00	0.87	0.19			
	107A	108A	0.48	12	12		41	0.63	45	3.66	0.53		0.00	0.00	0.00	0.00	0.00	0.48	0.63	0.21	0.74	87.0	250	0.25	29.73	0.02	0.61	0.25			
To Canoe Street, Pipe 108A - 113A																															
Euphoria Crescent																															
	101A	102A	0.36	7	7		24	0.36	24	3.70	0.29		0.00	0.00	0.00	0.00	0.00	0.36	0.36	0.12	0.41	62.5	250	0.65	47.94	0.01	0.98	0.30			
To Canoe Street, Pipe 102A - 105A																															
	98A	99A	0.28	5	5		17	0.28	17	3.71	0.20		0.00	0.00	0.00	0.00	0.00	0.28	0.28	0.09	0.30	41.5	200	1.20	35.93	0.01	1.14	0.33			
To Canoe Street, Pipe 99A - 102A																															
Canoe Street																															
	94A	95A	0.52	3	3		11	0.52	11	3.73	0.13		0.00	0.00	0.00	0.00	0.00	0.52	0.52	0.17	0.30	38.5	200	0.65	26.44	0.01	0.84	0.28			
	95A	96A	0.49	9	9		31	1.01	42	3.66	0.50		0.00	0.00	0.00	0.00	0.00	0.49	1.01	0.33	0.83	83.5	250	0.25	29.73	0.03	0.61	0.26			
	96A	99A	0.10	2	2		7	1.11	49	3.65	0.58		0.00	0.00	0.00	0.00	0.00	0.10	1.11	0.37	0.95	22.0	250	0.25	29.73	0.03	0.61	0.27			
Contribution From Euphoria Crescent, Pipe 98A - 99A																															
	99A	102A	0.18	3	3		11	0.28	17	3.62	0.90		0.00	0.00	0.00	0.00	0.00	0.18	1.57	0.52	1.42	58.5	250	0.25	29.73	0.05	0.61	0.31			
Contribution From Euphoria Crescent, Pipe 101A - 102A																															
	102A	105A	0.22	4	4		14	2.15	115	3.58	1.33		0.00	0.00	0.00	0.00	0.00	0.22	2.15	0.71	2.04	58.5	250	0.25	29.73	0.07	0.61	0.34			
Contribution From Jollity Crescent, Pipe 104A - 105A																															
	105A	108A	0.21	4	4		14	0.39	34	3.54	1.87		0.00	0.00	0.00	0.00	0.00	0.21	2.75	0.91	2.78	58.5	250	0.25	29.73	0.09	0.61	0.38			
Contribution From Jollity Crescent, Pipe 107A - 108A																															
	108A	113A	0.20	4	4		14	0.63	45	3.50	2.52		0.00	0.00	0.00	0.00	0.00	0.20	3.58	1.18	3.70	60.0	250	0.25	29.73	0.12	0.61	0.41			
Contribution From Elation Heights, Pipe 112A - 113A																															
	113A	118A	0.43	10	10		34	0.05	4	0.00			0.00	0.00	0.00	0.00	0.00	0.05	3.63												
Contribution From Peninsula Road, Pipe 117A - 118A																															
	118A	1180A	0.16	3	3		11	2.64	198	0.00			0.00	0.00	0.00	0.00	0.00	0.43	4.06	1.34	4.27	74.0	250	0.25	29.73	0.14	0.61	0.43			
	1180A	119A	0.03	0			0	6.86	469	3.39	5.15		0.00	0.00	0.00	0.00	0.00	0.16	6.86	2.26	7.42	42.5	250	0.25	29.73	0.25	0.61	0.50			
Contribution From Conservancy Drive, Pipe 93A - 119A																															
	119A	120A	0.17				0	6.89	469	3.39	5.15		0.00	0.00	0.00	0.00	0.00	0.03	6.89	2.27	7.43	20.0	250	0.25	29.73	0.25	0.61	0.50			
	120A	121A	0.21	4	4		14	81.01	7660	17.91			17.91	0.00	4.57	106.71	113.60														
	121A	Ex. MH 8						88.07	8129	2.63	69.41		17.91	0.00	4.57	9.44	0.17	113.77	37.54		116.40	75.0	525	0.10	136.00	0.86	0.63	0.71			
								88.28	8143	2.63	69.51		17.91	0.00	4.57	9.44	0.21	113.98	37.61		116.57	87.5	525	0.10	136.00	0.86	0.63	0.71			
								88.28	8143	2.63	69.51		17.91	0.00	4.57	9.44	0.00	113.98	37.61		116.57	10.0	525	0.10	136.00	0.86	0.63	0.71			
Park (Block 773)																															
	210A	18A						0.00					0.00	0.00	3	3.22	0.52	3.22	3.22	1.06	1.58	11.5	200	0.65	26.44	0.06	0.84	0.46			
To Conservancy Drive, Pipe 18A - 23A																															
								0.00	0				0.00	0.00	3.22				3.22												

DESIGN PARAMETERS

Park Flow =	9300	L/ha/da	0.10764
Average Daily Flow =	280	l/p/day	
Comm/Inst Flow =	28000	L/ha/da	0.3241
Industrial Flow =	35000	L/ha/da	0.40509
Max Res. Peak Factor =	4.00		
Commercial/Inst./Park Peak Factor =	1.50		
Institutional =	0.32	l/s/ha	



Industrial Peak Factor =	as per MOE Graph
Extraneous Flow =	0.330 L/s/ha
Minimum Velocity =	0.600 m/s
Manning's n =	(Conc) 0.013 (Pvc) 0.013
Townhouse coeff =	2.7
Single house coeff =	3.4

Designed:

A.K.

Checked:

W.L.

Dwg. Reference:

Sanitary Drainage Plan, Dwgs. No. 110-112

PROJECT:

BARRHAVEN CONCERNANCY EAST PH2, 3, AND JOCK RIVER

LOCATION:

City of Ottawa

File Ref:

20-1180

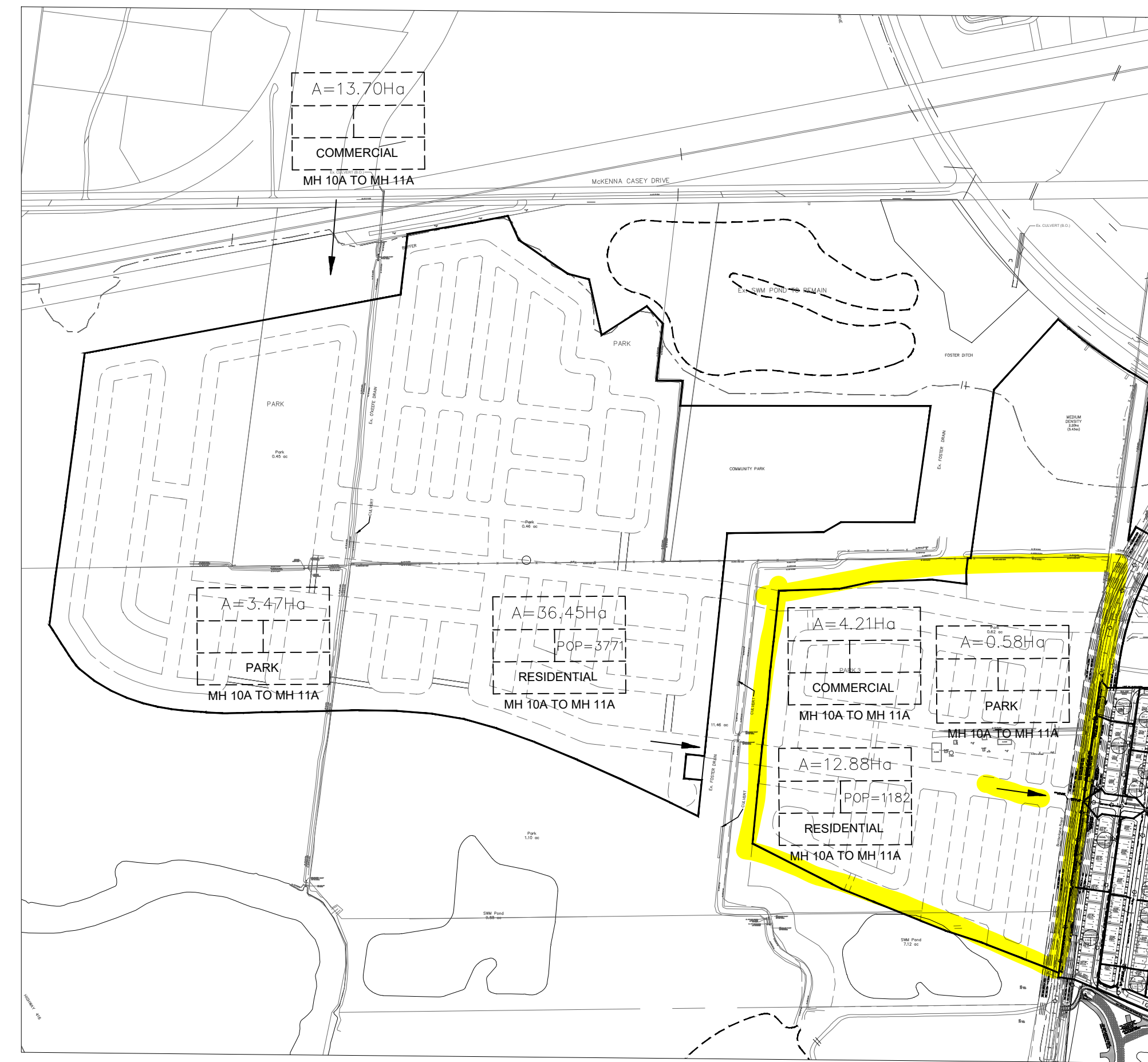
Date:

Aug 2022

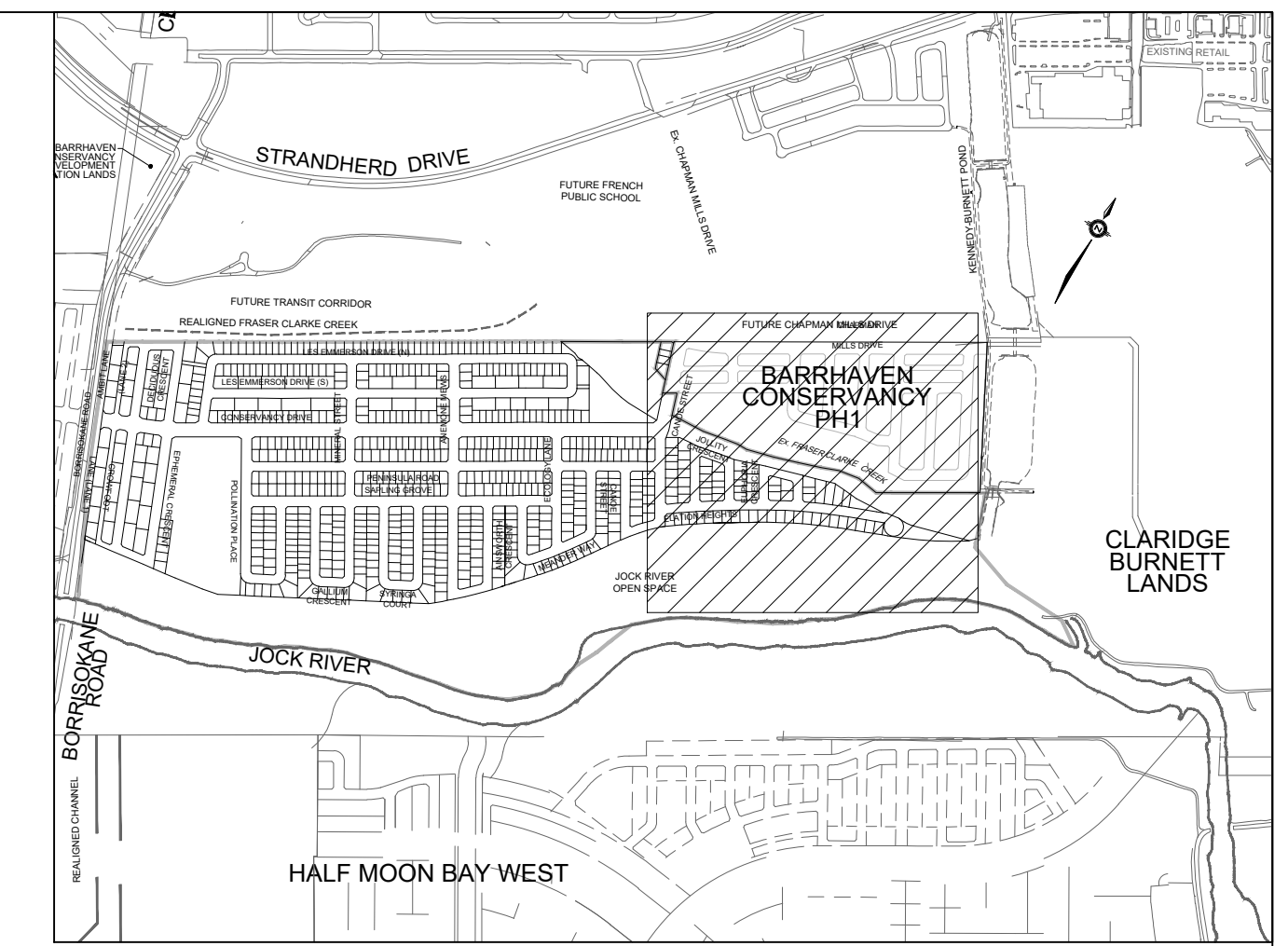
Sheet No.

6

of 6



EXTERNAL SANITARY DRAINAGE PLAN
SCALE: 1:5000



LEGEND

SANITARY DRAINAGE BOUNDARY	
SANITARY SUB-DRAINAGE BOUNDARY	
UPSTREAM MH TO DOWNSTREAM MH	43A - 44A
AREA IN HECTARES	0.78 61
POPULATION	107
UPSTREAM MH TO DOWNSTREAM MH	43A - 44A
AREA IN OTHER PHASES IN HECTARES	53.63
EXTERNAL POPULATION	5739
DENSITY (PERSONS/HECTARE)	RESIDENTIAL
EXTERNAL LAND USE	
MAINTENANCE HOLE	MH202A
CAP	
EXISTING SANITARY MAINTENANCE HOLE	

TOPOGRAPHIC INFORMATION
TOPOGRAPHIC INFORMATION PROVIDED BY J.D. BARNES LIMITED, PROJECT NO. 16-10-127-00, SURVEY DATED APRIL 10, 2018.

LEGAL INFORMATION
M-PLAN PROVIDED BY J.D. BARNES, PROJECT NO. 16-10-127-00, RECEIVED ON AUGUST 2, 2022.

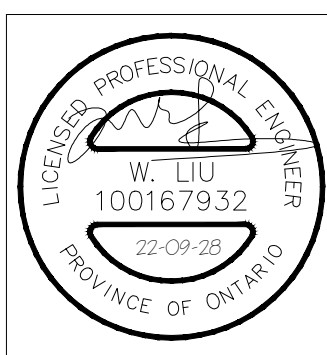
NOT FOR CONSTRUCTION

ELEVATION NOTE
ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM AND ARE REFERRED TO THE PUBLISHED BENCH MARK NO. D011964U3710
ELEVATION = 71.724m

No.	BY	DATE	DESCRIPTION
7	W.L.	22-09-28	RETAINING WALL UPDATES
6	W.L.	22-09-27	WATERMAIN AND CUP UPDATES
5	W.L.	22-08-31	CITY COMMENTS AND TRANSPORTATION UPDATES
4	W.L.	22-08-10	REVISED STREET NAME & LOT 99 SIGHT TRIANGLE
3	W.L.	22-06-28	3rd SUBMISSION
2	W.L.	22-04-22	2nd SUBMISSION
1	W.L.	21-12-22	1st SUBMISSION

CITY OF OTTAWA

PROJECT No. 20-1180



SANITARY DRAINAGE PLAN

BARRHAVEN CONSERVANCY DEVELOPMENT CORPORATION

BARRHAVEN CONSERVANCY EAST PHASE 2, 3 & JOCK RIVER



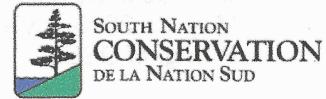
120 Iber Road, Unit 103
Stittville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
www.DSEL.ca

DRAWN BY: A.K./R.A./V.W.	CHECKED BY: W.L.	SHEET NO.
DESIGNED BY: W.L.	CHECKED BY: C.M.	112
SCALE: 1:1000	DATE: DECEMBER 2021	

APPENDIX D

STORMWATER

Conservation Partners Partenaires en conservation



May 31, 2020

City of Ottawa
110 Laurier Avenue,
Ottawa, ON K1P 1J1

Attention: Doug James

Subject: **Barrhaven Conservancy Development Corporation
Status of As-Built Grading
Related: RVCA Permit # RV5-4419 and RV5-1718)
Vacant land on the north side of the Jock River generally bounded by
Highway 416 and the Fraser Clarke Creek, City of Ottawa**

Dear Mr. James:

The RVCA has reviewed information recently submitted by David Schaeffer Engineering Ltd. including as-built grades in support of works approved by the Rideau Valley Conservation Authority under Section 28 of the Conservation Authorities Act (Permit File Number: RV5-4419 and RV5-1718). The RVCA offers the following comments related to future development proposed for the area within the scope of approved the permits.

The subject lands as identified as part of Lots 11, 12, 13, 14, 15 former geographic Township of Nepean, Concessions 3 & 4, now in the City of Ottawa have been addressed through the general placement of fill and the formal construction of a berm around the perimeter of four blocks within the subject lands. The site specific elevations of the berm have been reviewed by the RVCA and are generally accepted as being appropriate as removing these lands from the floodplain in accordance with the aforementioned approved permits.

The detailed grading plans submitted by David Schaeffer Engineering Ltd. titled "As Constructed plan of Berms and Cut Areas – Barrhaven Conservancy", dated May 27, 2020, prepared by Adam Fobert, P.Eng. of DSEL, DSEL File Number 16891 using the following resources:

- Orthoimagery Survey, dated April 20, 2020, acquired and processed by First Base Solutions a division of JD Barnes Ltd and certified by Chris Fox, O.L.S., A.L.S., P. Eng. of JD Barnes Ltd, file reference number 2037OTTA0001; ·
- Topographic Detail of Part of Lot 13, 14, & 15 Concession 3&4, dated May 6, 2020, certified by Chris Fox, O.L.S., A.L.S., P. Eng. of JD Barnes Ltd, file reference number 16-10-127-00; ·



- Contractor as-built collected by the Tomlinson Group of Companies of Phase 1 dated May 15, 2020, reviewed by Jeremy Chouindard, EIT and certified by Stephen Pichette, P.Eng. of DSEL

The above information indicates that land within the berm have generally been raised to exceed the flood elevation cross sections throughout the project area. However, it is noted that as this is considered an active construction site the presence of lower areas to manage construction, on-site erosion and sediment control show lower elevations. These areas will be addressed through the construction process, as sufficient material is presently stockpiled for this purpose to ensure. For the purposes of the floodplain, these areas are considered removed by virtue of the berm.

Conclusion:

The grade modifications, including construction of the berm and filling behind the berm, as documented in the above noted "as constructed" plans, have been completed in accordance with the plans approved by the RVCA under permits RV5-4419 and RV5-1718.

Please feel free to contact our office with any questions or comments you may have.

Respectfully,

Terry Davidson, P.Eng
Director of Engineering and Regulations
Rideau Valley Conservation Authority
613-692-3571 x1107
terry.davidson@rvca.ca

attach: Technical memorandum by Evelyn Liu, M.Asc., P.Eng. Water Resources
 Engineer, RVCA dated May 29, 2020

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)



Local Roads Return Frequency = 2 years
 Collector Roads Return Frequency = 5 years
 Arterial Roads Return Frequency = 10 years

Manning 0.013

LOCATION			AREA (Ha)																FLOW					SEWER DATA										
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO	
Location	From Node	To Node	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full		
Centerline14 - 14																																		
Contribution From Centerline15 - 15, Pipe 526 - 529					2.07					0.00					0.54			0.00		10.95														
Contribution From Centerline15 - 15, Pipe 527 - 529					0.00					0.00					0.61			0.00		10.82														
	529	530	0.52	0.80	1.16	3.22			0.00	0.00			0.00	1.16			0.00	0.00	10.95	73.33	99.42	116.52	170.30	371	825	825	CONC	0.11	26.5	476.0801	0.8906	0.4959	0.780	
	530	534			0.00	3.22			0.00	0.00			0.00	1.16			0.00	0.00	11.45	71.66	97.12	113.81	166.33	363	825	825	CONC	0.11	117.0	476.0801	0.8906	2.1895	0.762	
Contribution From Centerline6 - 06, Pipe 533 - 534					2.67					0.00					0.00			0.00		12.72														
	534	537	0.11	0.80	0.24	6.14			0.00	0.00			0.00	1.16			0.00	0.00	13.64	65.18	88.24	103.36	150.97	520	900	900	CONC	0.15	48.5	701.1305	1.1021	0.7334	0.742	
To Centerline13 - 13, Pipe 537 - 538					6.14					0.00				1.16			0.00		14.37															
Centerline13 - 13																																		
Contribution From Centerline14 - 14, Pipe 534 - 537					6.14					0.00				1.16			0.00		14.37															
Contribution From Centerline1 - 01, Pipe 536 - 537					0.00	6.14	0.06	0.67	0.11	0.98			0.00	1.16			0.00	0.00	12.10															
	537	538	0.28	0.67	0.52	6.66			0.00	0.98			0.00	1.16			0.00	0.00	14.37	63.29	85.65	100.31	146.50	622	975	975	CONC	0.15	83.0	867.9562	1.1625	1.1899	0.716	
	538	543	0.25	0.67	0.47	7.13			0.00	0.98			0.00	1.16			0.00	0.00	15.56	60.48	81.79	95.77	139.84	622	975	975	CONC	0.15	69.0	867.9562	1.1625	0.9892	0.717	
Contribution From Centerline8 - 08, Pipe 542 - 543					1.92					0.00				0.00				0.00		13.56														
	543	528	0.05	0.67	0.09	9.14			0.00	0.98			0.00	1.16			0.00	0.00	16.55	58.34	78.86	92.33	134.80	717	1050	1050	CONC	0.11	29.0	905.6791	1.0459	0.4621	0.792	
Contribution From Centerline12 - 12, Pipe 548 - 528					3.24					0.09				0.00				0.00		17.17														
	528	HW9			0.00	12.38			0.00	1.07			0.00	1.16			0.00	0.00	17.17	57.08	77.15	90.32	131.84	894	1050	1050	CONC	0.20	4.5	1221.2174	1.4103	0.0532	0.732	

Definitions:
 Q = 2.78 AIR, where
 Q = Peak Flow in Litres per second (L/s)
 A = Areas in hectares (ha)
 I = Rainfall Intensity (mm/h)
 R = Runoff Coefficient

Notes:
 1) Ottawa Rainfall-Intensity Curve
 2) Min. Velocity = 0.80 m/s

Designed: A.S.	PROJECT: Barrhaven Conservancy East Phase 5		
Checked: W.L./V.W.	LOCATION: City of Ottawa		
Dwg. Reference:	File Ref:	Date: 01 Dec 2022	Sheet No. SHEET 3 OF 3



**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD
BASED ON A FINE PARTICLE SIZE DISTRIBUTION**



Project Name: 891 Conservancy East	Engineer: DSEL
Location: Ottawa, ON	Contact: K. Murphy
OGS #: 9	Report Date: 20-Oct-22
Area: 7.21 ha	Rainfall Station #: 215
Weighted C: 0.72	Particle Size Distribution: FINE
CDS Model: 4045 (OFFLINE)	CDS Treatment Capacity: 212 l/s

<u>Rainfall Intensity¹</u> <u>(mm/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (l/s)</u>	<u>Treated Flowrate (l/s)</u>	<u>Operating Rate (%)</u>	<u>Removal Efficiency (%)</u>	<u>Incremental Removal (%)</u>
1.0	10.6%	19.8%	14.4	14.4	6.8	96.9	10.3
1.5	9.9%	29.7%	21.6	21.6	10.2	95.9	9.5
2.0	8.4%	38.1%	28.9	28.9	13.6	95.0	8.0
2.5	7.7%	45.8%	36.1	36.1	17.0	94.0	7.2
3.0	5.9%	51.7%	43.3	43.3	20.4	93.0	5.5
3.5	4.4%	56.1%	50.5	50.5	23.8	92.0	4.0
4.0	4.7%	60.7%	57.7	57.7	27.2	91.1	4.2
4.5	3.3%	64.0%	64.9	64.9	30.6	90.1	3.0
5.0	3.0%	67.1%	72.2	72.2	34.0	89.1	2.7
6.0	5.4%	72.4%	86.6	86.6	40.8	87.2	4.7
7.0	4.4%	76.8%	101.0	101.0	47.6	85.2	3.7
8.0	3.5%	80.3%	115.5	115.5	54.4	83.3	2.9
9.0	2.8%	83.2%	129.9	129.9	61.2	81.3	2.3
10.0	2.2%	85.3%	144.3	144.3	67.9	79.4	1.7
15.0	7.0%	92.3%	216.5	212.4	100.0	68.9	4.8
20.0	4.5%	96.9%	288.6	212.4	100.0	51.7	2.3
25.0	1.4%	98.3%	360.8	212.4	100.0	41.3	0.6
30.0	0.7%	99.0%	432.9	212.4	100.0	34.4	0.2
35.0	0.5%	99.5%	505.1	212.4	100.0	29.5	0.1
40.0	0.5%	100.0%	577.3	212.4	100.0	25.8	0.1
							87.1

Removal Efficiency Adjustment ² =	6.5%
Predicted Net Annual Load Removal Efficiency =	80.6%
Predicted Annual Rainfall Treated =	97.1%

- 1 - Based on 42 years of hourly rainfall data from Canadian Station 6105976, Ottawa ON
- 2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.
- 3 - CDS Efficiency based on testing conducted at the University of Central Florida
- 4 - CDS design flowrate and scaling based on standard manufacturer model & product specifications



**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD
BASED ON A FINE PARTICLE SIZE DISTRIBUTION**



Project Name: 891 Conservancy East
Location: Ottawa, ON
OGS #: 10

Engineer: DSEL
Contact: K. Murphy
Report Date: 20-Oct-22

Area 6.61 ha
Weighted C 0.70
CDS Model 4045 (OFFLINE)

Rainfall Station # 215
Particle Size Distribution FINE
CDS Treatment Capacity 212 l/s

<u>Rainfall Intensity¹</u> <u>(mm/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (l/s)</u>	<u>Treated Flowrate (l/s)</u>	<u>Operating Rate (%)</u>	<u>Removal Efficiency (%)</u>	<u>Incremental Removal (%)</u>
1.0	10.6%	19.8%	12.9	12.9	6.1	97.1	10.3
1.5	9.9%	29.7%	19.3	19.3	9.1	96.3	9.5
2.0	8.4%	38.1%	25.7	25.7	12.1	95.4	8.0
2.5	7.7%	45.8%	32.2	32.2	15.1	94.5	7.3
3.0	5.9%	51.7%	38.6	38.6	18.2	93.6	5.6
3.5	4.4%	56.1%	45.0	45.0	21.2	92.8	4.0
4.0	4.7%	60.7%	51.5	51.5	24.2	91.9	4.3
4.5	3.3%	64.0%	57.9	57.9	27.3	91.0	3.0
5.0	3.0%	67.1%	64.3	64.3	30.3	90.2	2.7
6.0	5.4%	72.4%	77.2	77.2	36.3	88.4	4.8
7.0	4.4%	76.8%	90.0	90.0	42.4	86.7	3.8
8.0	3.5%	80.3%	102.9	102.9	48.4	85.0	3.0
9.0	2.8%	83.2%	115.8	115.8	54.5	83.2	2.3
10.0	2.2%	85.3%	128.6	128.6	60.6	81.5	1.8
15.0	7.0%	92.3%	192.9	192.9	90.8	72.8	5.1
20.0	4.5%	96.9%	257.3	212.4	100.0	58.0	2.6
25.0	1.4%	98.3%	321.6	212.4	100.0	46.4	0.7
30.0	0.7%	99.0%	385.9	212.4	100.0	38.6	0.3
35.0	0.5%	99.5%	450.2	212.4	100.0	33.1	0.2
40.0	0.5%	100.0%	514.5	212.4	100.0	29.0	0.2

88.4

Removal Efficiency Adjustment² = 6.5%

Predicted Net Annual Load Removal Efficiency = 81.9%

Predicted Annual Rainfall Treated = 97.8%

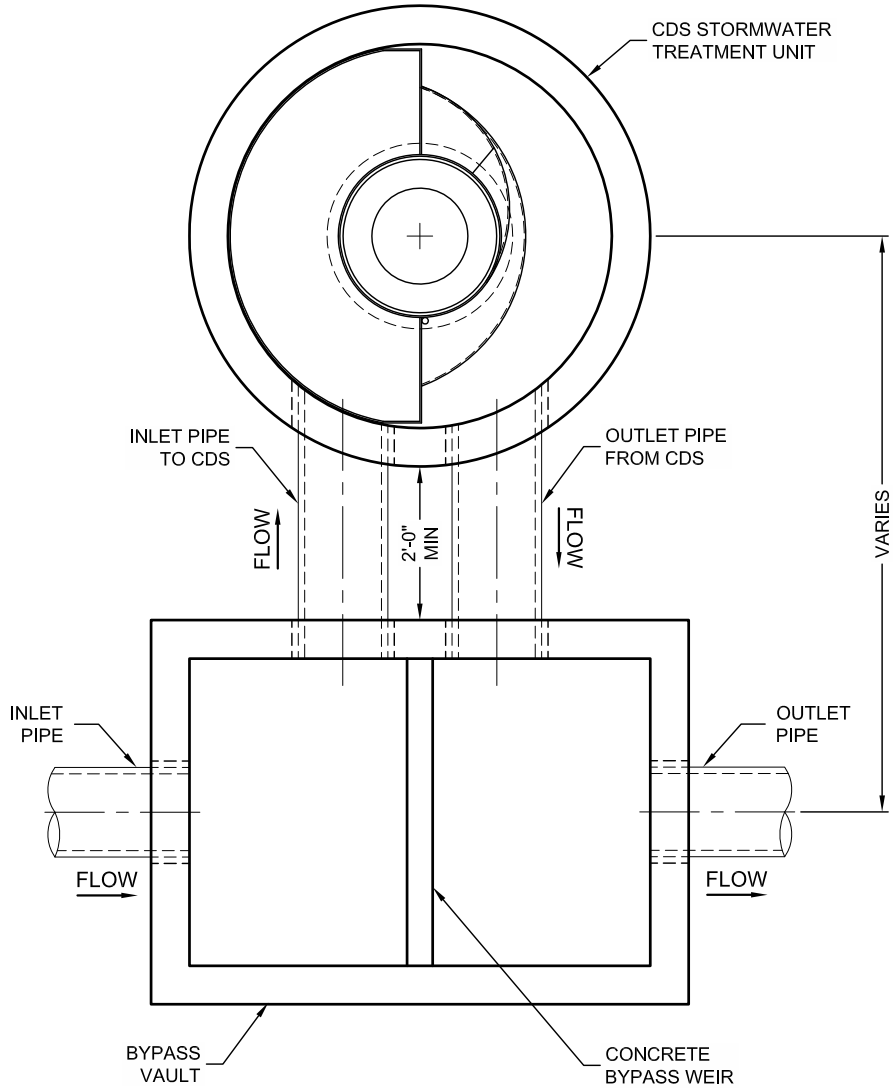
1 - Based on 42 years of hourly rainfall data from Canadian Station 6105976, Ottawa ON

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

3 - CDS Efficiency based on testing conducted at the University of Central Florida

4 - CDS design flowrate and scaling based on standard manufacturer model & product specifications

I:\STORMWATER\COM\WOPS\22 CDS\40 STANDARD DRAWINGS\OFFLINE LAYOUTS DWG\OFFLINE CDS-C LAYOUT BYPASS VAULT STRUCTURE.DWG 3/12/2013 3:35 PM



THIS PRODUCT MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING U.S. PATENTS: 5,788,848; 6,641,720; 6,511,595; 6,581,783; RELATED FOREIGN PATENTS, OR OTHER PATENTS PENDING.

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800-338-1122 513-645-7000 513-645-7993 FAX

CDS STORMWATER TREATMENT SYSTEM
TYPICAL OFFLINE LAYOUT
WITH BYPASS VAULT STRUCTURE

DATE:03/12/13

SCALE: NONE

PROJECT No.: N/A

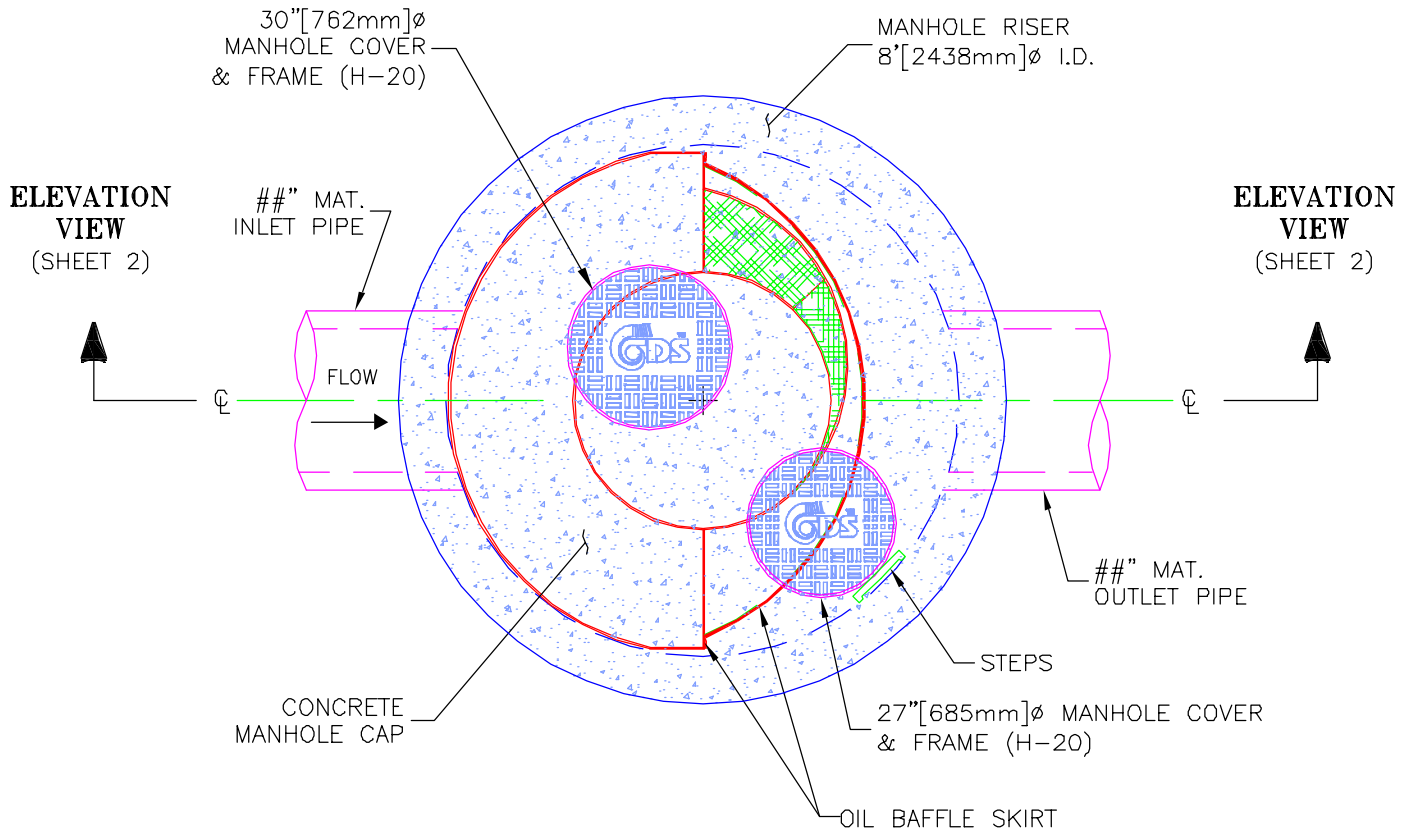
SEQ. No.: N/A

DRAWN: N/A

CHECKED: N/A



PLAN VIEW



CDS MODEL PMSU40_45m, 7.5 CFS TREATMENT CAPACITY STORM WATER TREATMENT UNIT



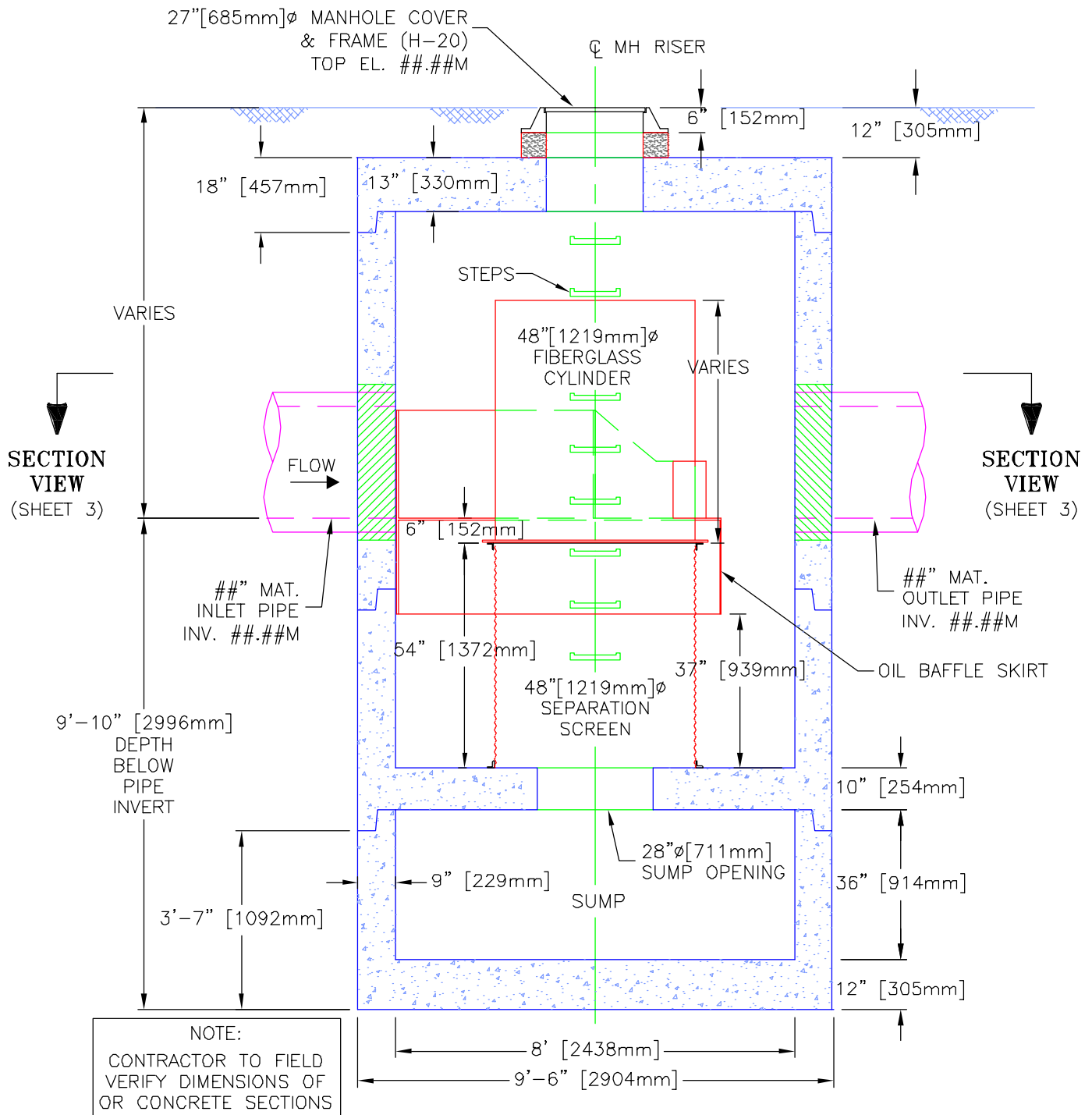
PROJECT NAME
CITY, STATE

JOB# XX-##-###
DATE ##/##/##
DRAWN INITIALS
APPROV.

SCALE
1" = 3'
SHEET
1



ELEVATION VIEW

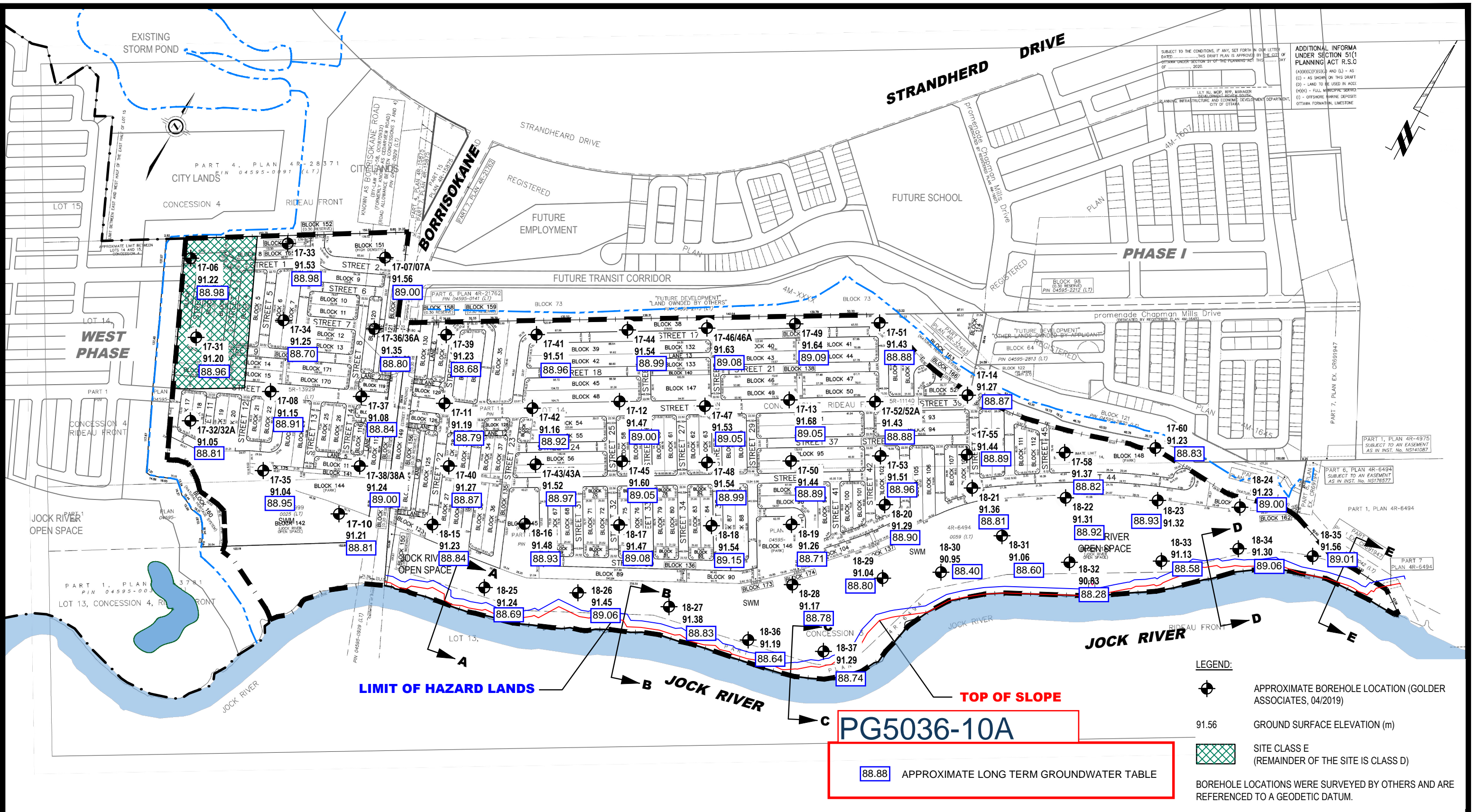


CDS MODEL PMSU40_45m, 7.5 CFS TREATMENT CAPACITY STORM WATER TREATMENT UNIT



PROJECT NAME
CITY, STATE

JOB#	XX-##-###	SCALE	1" = 3'
DATE	##/##/##	SHEET	2
DRAWN	INITIALS		
APPROV.			



ADDITIONAL INFORMATION UNDER SECTION 51(1) PLANNING ACT R.S.O. (A) - AS SHOWN ON THIS DRAFT (B) - AS SHOWN ON THIS DRAFT (C) - LAND TO BE USED IN ACC. (D) - FULL MUNICIPAL SERVICES (E) - OFFSHORE MARINE DEPOSIT; OTTAWA FORMATION, LIMESTONE.

- LEGEND:**
- APPROXIMATE BOREHOLE LOCATION (GOLDER ASSOCIATES, 04/2019)
 - 91.56 GROUND SURFACE ELEVATION (m)
 - SITE CLASS E (REMAINDER OF THE SITE IS CLASS D)
 - BOREHOLE LOCATIONS WERE SURVEYED BY OTHERS AND ARE REFERENCED TO A GEODETIC DATUM.

PG5036-10A

88.88 APPROXIMATE LONG TERM GROUNDWATER TABLE

patersongroup
consulting engineers

154 Colonnade Road South
Ottawa, Ontario K2E 7J5
Tel: (613) 226-7381 Fax: (613) 226-6344

NO.	REVISIONS	DATE	INITIAL
1	CONCEPTUAL PLAN ADDED	09/01/2020	DJG

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROP. RESIDENTIAL DEVELOPMENT - CONSERVANCY LANDS EAST
OTTAWA, ONTARIO

Title: **TEST HOLE LOCATION PLAN**

Scale:	1:6000	Date:	09/2019
Drawn by:	MPG	Report No.:	PG5036-1
Checked by:	SD	Dwg. No.:	PG5036-1
Approved by:	DJG	Revision No.:	1

December 01, 2022

Project Number: 1474(03)

David Schaeffer Engineering Ltd
120 Iber Road, Unit 103
Ottawa, Ontario
K2S 1E9

Attention: Kevin Murphy, P.Eng

Subject: BCDC Phase 5 – Preliminary HGL Analysis

Introduction

Phase 5 of the Barrhaven Conservancy Development is located in Barrhaven, Ontario, north of the Jock River, east of the Foster Creek and West of Borrisokane Road. The proposed development is approximately **13.17 ha** that will primarily comprise of single and townhouse residential lots along with a **0.64 ha** park. The following outlines the preliminary hydraulic grade line (HGL) assessment for the site, to ensure that the proposed minor system within the development is adequately sized to safely convey flows to the Jock River under various conditions. As such the following memo outlines the approach taken in assessing the development's HGL and summarises the findings of this analysis.

Analysis Approach

Preliminary hydraulic grade line calculations for the proposed BCDC Phase 5 development were completed using PCSWMM modelling software. Pipe data, trunk storm sewer layout and Rational Method flows in the storm sewer are as provided by DSEL. The Rational Method flows were calculated based on the 2-, 5- or 10-year level of service requirements, and the 100-year flows in the hydraulic grade line calculations were estimated as 14% greater than the Rational Method flows, to account for the additional flows captured by catchbasin grates, lead pipes and/or inlet control devices under the higher surface water depths of the 100-year storm.

The 14% increase in flows for the 100-year event is based on the assumption that the head on a lead pipe/ICD will increase by 35 cm (maximum allowable major system ponding depth) during the 100-Year event. Taking a typical 250 mm lead pipe and assuming that the head on the pipe is just below the top of the grate (assumed at 1.38 m) results in a peak flow of 209 L/s, then assuming that the head is increased by 35 cm during the 100-Year (head of 1.730m) the flow through the lead pipe would increase to 234.5 L/s, which results in a 12% increase in peak flows. It is important to note that a 12% increase is observed when the same calculations are applied to the various lead pipe and ICD sizes. An additional 2% is added as a safety factor to allow some flexibility in the design, as it is likely that not all lead pipes will have a head of 1.38 m (just below the top of MH) for the level of service specified.

The proposed storm sewer infrastructure data was extracted from DSEL's drawings and incorporated into a PCSWMM model, and flows derived by DSEL's Rational Method calculations were then applied to each Maintenance Hole (MH) in the model as steady flows (using the baseflow option). Exit losses were applied to all storm sewer pipes in the system based on the angle of the downstream connection.

As in line with all other works completed for the BCDC development phases, the preliminary HGL analysis was completed under two conditions:

- 100-year rainfall event on the development and a 5-year spring water level on the Jock River
- Level of service (2/5/10-year) rainfall event on the development and a 100-year spring water level on the Jock River

Note that the water level along the Jock River through the length of this development varies, and as such the nearest corresponding upstream water surface elevation calculated by RVCA's HEC-RAS floodplain mapping model of the Jock River was applied at each of the respective storm sewer outlets. Also, note that assuming a 5-year spring water level on the Jock River for a 100-year rainfall event on the development is an inherently conservative assumption, as the critical storm for the proposed development is a summer (intense rainfall) event while the critical storm for the Jock River is a spring (snowmelt + rainfall) event. A preliminary Single Station Flood Frequency analysis was completed by JFSA using only summer flows (from May 15 to October 31) based on historical flow data recorded at the Moodie Drive Water Survey Canada gauge. This analysis found that the 100-year summer flow on the Jock River is around **99 m³/s**, while the 5-year spring flow is around **123 m³/s**, therefore the downstream boundary condition applied is conservative.

Within the proposed development Oil and Grit Separators (OGS) units in conjunction with LID measures will be implemented to ensure the site meets quality control requirements. Preliminary OGS units and associated by-pass weir elevations have also been included in the model, based on similar drainage areas and imperviousness seen in BCDC Phase 2.

Trunk Sewer 9 will have a drainage area of **7.21 ha** at **75%** imperviousness, Trunk Sewer 10 has a drainage area of **6.56 ha** at **80%** imperviousness. In a preliminary consultation with the OGS manufacturer, it was suggested that the closest match to the detailed OGS sizing for BCDC Phase 2 is **OGS 3**, which used a **PMSU 4045-8** OGS unit with a weir height of **0.65m**, this unit had a drainage area of **6.77 ha** at **64%** imperviousness.

Results

The maximum HGL obtained at each MH has been extracted from the level of service (2/5/10-year) event / 100-year Jock River water level scenario and the 100-year event / 5-year Jock River water level scenario, with the results from this analysis provided in Tables 1 & 2, respectively. As all proposed units within this development will have sump pumps, the simulated HGL was compared against the top of MH elevation to ensure that all storm sewers infrastructure is sufficiently sized and is not surcharging to the major system during the assessed events.

From this analysis, it was found that the critical scenario for HGL within the development was the level of service development event and 100-year water level on the Jock River scenario. Based on this scenario, no MHs will have an HGL elevation above the top of MH (minimum freeboard of **0.64 m** at **MH-514** to **MH-516** and **MH-521**), with an average freeboard of **0.74 m** from the top of MH throughout the proposed development.

For the 100-year event and 5-year water level on the Jock River, no MHs will have HGL elevations above the top of MH (minimum freeboard of **1.22 m** at **MH-514** and **MH-515**), with an average freeboard of **1.38 m** from the top of MH throughout the proposed development. As such it can be concluded that the proposed storm sewer infrastructure is sufficiently sized, to safely convey minor system flows from the development under various extreme conditions.

Conclusion

A preliminary HGL analysis for Phase 5 of the Barrhaven Conservancy Development was completed using PCSWMM based on storm sewer and flow details provided by DSEL. From this analysis, it was found that the proposed storm sewer infrastructure is sufficiently sized to convey all minor system flows to the Jock River and will not result in any MHs surcharging to the street under extreme events such as 100-year rainfall events on the development and a 5-year spring water level on the Jock River and a level of service (2/5/10 Year) rainfall event on the development and a 100-year spring water level on the Jock River, with the former being the more critical scenario for the HGL within the development.

Yours truly,
J.F Sabourin and Associates Inc.



Jonathon Burnett, P.Eng
Water Resources Engineer



cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects

Figures

Figure 1: PCSWMM Model Overview

Tables

Table 1: HGL Result Tables - Level of service (2/5/10-year) BCDC Development & 100-Year Jock River

Table 2: HGL Result Tables - 100-Year BCDC Development & 5-Year Jock River

Attachments

Attachment A: DSEL Rational Method Calculations

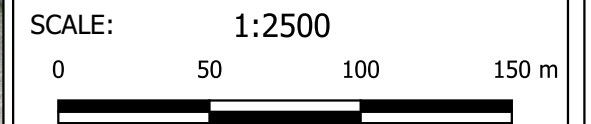
Modelling Files - Provided Electronically

PCSWMM: BCDC-P5_HGL_v01.1-2-5-10-YrDev-100YrJock.inp
BCDC-P5_HGL_v01.1-100-YrDev-5YrJock.inp



Legend

- Junctions
- ▲ Outfalls
- Conduits
- Weirs
- Site Plan



J.F. Sabourin and Associates Inc.
 WATER RESOURCES AND ENVIRONMENTAL CONSULTANTS
 52 Springbrook Drive (613) 836-3884
 Ottawa, ON, K2S 1B9 www.jfsa.com



Barrhaven Conservancy Development
 Phase 5

Figure 1: Preliminary HGL Analysis
 Model Overview

PROJECT	1474(03)-21
DRAWN	JB
DATE	December 2022

Table 1: BCDC Phase 5 - Preliminary HGL Analysis
Level of Service (2/5/10 Year) BCDC Development & 100-Year Jock River

MH-ID	Invert Elevation	Top of MH (m)	Max HGL (m)	Freeboard (m)
MH-507	90.62	93.26	92.54	0.72
MH-513	90.42	93.10	92.43	0.67
MH-514	90.34	93.02	92.38	0.64
MH-515	90.30	93.00	92.36	0.64
MH-516	90.17	92.91	92.27	0.64
MH-521	90.05	92.83	92.19	0.64
MH-524	89.71	92.66	91.99	0.67
MH-525	89.64	92.65	91.93	0.72
MH-525-1	89.63	92.64	91.88	0.76
MH-528	89.63	92.67	91.89	0.78
MH-528-1	89.63	92.67	91.83	0.84
MH-529	90.43	93.01	92.23	0.78
MH-530	90.37	93.01	92.20	0.81
MH-534	90.17	92.99	92.11	0.88
MH-537	90.02	92.91	92.07	0.84
MH-538	89.87	92.91	92.01	0.90
MH-543	89.69	92.65	91.96	0.69
			Min	0.64
			Max	0.90
			Average	0.74

Note: Analysis assumes 100 year spring water level on the Jock River

Model Name:BCDC-P5_HGL_v01.1-2-5-10-YrDev-100YrJock.inp

**Table 2: BCDC Phase 5 - Preliminary HGL Analysis
100-Year BCDC Development & 5-Year Jock River**

MH-ID	Invert Elevation	Top of MH (m)	Max HGL (m)	Freeboard (m)
MH-507	90.62	93.26	92.02	1.24
MH-513	90.42	93.10	91.87	1.23
MH-514	90.34	93.02	91.81	1.22
MH-515	90.30	93.00	91.78	1.22
MH-516	90.17	92.91	91.67	1.24
MH-521	90.05	92.83	91.56	1.27
MH-524	89.71	92.66	91.29	1.37
MH-525	89.64	92.65	91.22	1.43
MH-525-1	89.63	92.64	91.16	1.48
MH-528	89.63	92.67	91.19	1.48
MH-528-1	89.63	92.67	91.12	1.55
MH-529	90.43	93.01	91.63	1.38
MH-530	90.37	93.01	91.59	1.42
MH-534	90.17	92.99	91.47	1.52
MH-537	90.02	92.91	91.42	1.49
MH-538	89.87	92.91	91.34	1.57
MH-543	89.69	92.65	91.27	1.38
			Min	1.22
			Max	1.57
			Average	1.38

Note: Analysis assumes 5 year spring water level on the Jock River

Model Name:BCDC-P5_HGL_v01.1-100-YrDev-5YrJock.inp



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Québec, QC

Attachment A

DSEL Rational Method Calculations

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
 Collector Roads Return Frequency = 5 years
 Arterial Roads Return Frequency = 10 years



Manning 0.013

Location	From Node	To Node	AREA (Ha)																FLOW					SEWER DATA																															
			2 YEAR		5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO																								
			AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full																							
Centerline14 - 14																																																							
Contribution From Centerline15 - 15, Pipe 526 - 529																								2.07										0.00					0.00																
Contribution From Centerline15 - 15, Pipe 527 - 529																								0.00										0.00					0.00																
529	530	0.52	0.80	1.16	3.22					0.00	0.00			0.00	1.16		0.00	10.95	73.33	99.42	116.52	170.30	371	825	825	CONC	0.11	26.5	476.0801	0.8906	0.4959	0.780																							
530	534			0.00	3.22					0.00	0.00			0.00	1.16		0.00	11.45	71.66	97.12	113.81	166.33	363	825	825	CONC	0.11	117.0	476.0801	0.8906	2.1895	0.762																							
Contribution From Centerline6 - 06, Pipe 533 - 534																								2.67										0.00					0.00																
534	537	0.11	0.80	0.24	6.14					0.00	0.00			0.00	1.16		0.00	12.72																																					
To Centerline13 - 13, Pipe 537 - 538																								6.14										0.00			1.16		0.00	13.64	65.18	88.24	103.36	150.97	520	900	900	CONC	0.15	48.5	701.1305	1.1021	0.7334	0.742	
Centerline13 - 13																																																							
Contribution From Centerline14 - 14, Pipe 534 - 537																								6.14										0.00					0.00																
Contribution From Centerline1 - 01, Pipe 536 - 537																								0.00				0.06	0.67	0.11	0.98						0.00	0.98		0.00	12.10														
537	538	0.28	0.67	0.52	6.66					0.00	0.98			0.00	1.16		0.00	14.37	63.29	85.65	100.31	146.50	622	975	975	CONC	0.15	83.0	867.9562	1.1625	1.1899	0.716																							
538	543	0.25	0.67	0.47	7.13					0.00	0.98			0.00	1.16		0.00	15.56	60.48	81.79	95.77	139.84	622	975	975	CONC	0.15	69.0	867.9562	1.1625	0.9892	0.717																							
Contribution From Centerline8 - 08, Pipe 542 - 543																								1.92										0.00					0.00																
543	528	0.05	0.67	0.09	9.14					0.00	0.98			0.00	1.16		0.00	13.56																																					
Contribution From Centerline12 - 12, Pipe 548 - 528																								3.24										0.00					0.00																
528	HW9			0.00	12.38					0.00	1.07			0.00	1.16		0.00	17.17	57.08	77.15	90.32	131.84	894	1050	1050	CONC	0.20	4.5	1221.2174	1.4103	0.0532	0.732																							

Definitions:
 Q = 2.78 AIR, where
 Q = Peak Flow in Litres per second (L/s)
 A = Areas in hectares (ha)
 I = Rainfall Intensity (mm/h)
 R = Runoff Coefficient

Notes:
 1) Ottawa Rainfall-Intensity Curve
 2) Min. Velocity = 0.80 m/s

Designed: A.S.	PROJECT: Barrhaven Conservancy East Phase 5		
Checked: W.L./V.W.	LOCATION: City of Ottawa		
Dwg. Reference:	File Ref:	Date: 01 Dec 2022	Sheet No. SHEET 3 OF 3

March 08, 2021

Project Number: 1474

David Schaeffer Engineering Ltd
120 Iber Road, Unit 103
Ottawa, Ontario
K2S 1E9

Attention: Steve Pichette, P.Eng.

Subject: Review of Quantity Control Requirement for Jock River-Reach One

Introduction

Phase 2 of the Barrhaven Conservancy Development (aka Conservancy East) is located in Barrhaven, Ontario, north of the Jock River, south of the Fraser Clarke Creek and east of the Foster Creek. The proposed development is approximately 59.26 ha that will primarily comprise of single and townhouse residential lots. As a part of the City of Ottawa's review of the proposed development draft plan of Phase 2 of the Barrhaven Conservancy Development, submitted in December 2020, it is proposed that flood quantity control measures will not need to be implemented as a part of this development. This assumption is based on the work completed by Stantec in June 2007 in the "Jock River Reach One Subwatershed Study" which concluded that for future developments within Reach 1 of the Jock River: "No quantity control storage is required for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River" and that "No erosion control storage is required to maintain the predevelopment in-stream erosion condition". Although this study did not consider the future development of the Barrhaven Conservancy Lands, and as such the modelling completed by Stantec has been updated by JFSA to reflect these changes. The following memo outlines data sources, assessed scenarios, assumptions, and conclusions of this independent Jock River Reach One study.

It is noted that RVCA is currently engaging in a formal update/review of the Jock River Reach One Subwatershed Study, with the findings of this study having the potential to affect the above-noted design criteria. While that study is underway J.F. Sabourin and Associates Inc. (JFSA) has completed an independent Jock River - Reach One study to re-assess/confirm that the assumptions presented in the original 2007 study by Stantec are still valid, as any changes to this conclusion could greatly impact the current BCDC Phase 2 development plan.

Background Data

The following outlines all the model and data sources used in this analysis:

- "Jock River Floodplain Mapping Report", (2005 - PSR Group Ltd. & JFSA)
- "Jock River Reach One Subwatershed Study Final Report", (2007 – Stantec)
- "Corrigan Stormwater Management Facility Stormwater Management Report and Design Brief", (2010 - IBI Group)
- "Citi Gate, Highway 416 Employment Lands, Servicing Study and Stormwater Management Report (O'Keefe SWM)", (2012 – Novatech)
- "Foster Stormwater Management Facility, Environmental Study Report", (2013 - CH2MHill)
- "Todd Pond Model Keeper Analysis (Re-Assessment of Existing System Capacity)", (2015 – JFSA)

- “CitiGate 416 Corporate Campus Detailed Servicing and Stormwater Management Report (Phase 1)”, (2015 – Novatech)
- “Kennedy-Burnett Stormwater Management Facility Retrofit, Detailed Design Report”, (2020 – Novatech)
- “Half Moon Bay South / Addendum to April 2015 Todd Pond Model Keeper Analysis, Re-Assessment of Existing System Capacity Report” (2020 - JFSA).

Model Development/Scenarios

The following section outlines the various hydrologic model scenarios developed as a part of this work, with a brief description of the data sources used for each scenario and how they have been incorporated into the existing Jock River subwatershed hydrologic model.

Model 1 - Jock River Floodplain Model – JFSA, 2005

This hydrologic model was developed as a part of the floodplain mapping study of the Jock River completed in 2005. The hydrologic model of the Jock River was developed by JFSA using SWMHYMO, with independent models developed to simulate both summer and spring events. Both models were calibrated to field measured flows, recorded at the Water Survey Canada Flow Gauge at Moodie Drive. These models function as the basis for which all future models (both by JFSA and others) have been built on. Refer to Figure 1 (JFSA, 2005) for an overview of the subcatchments for reach one in this model, with full SWMHYMO input and summary files provided in Attachment A.

Model 2 – Jock River Reach One Model – Stantec, 2007

The hydrologic analysis completed by Stantec in 2007 built upon the JFSA 2005 floodplain mapping modelling. As a part of the Stantec work, the lower reach of the Jock River (3,176 ha) which was represented as a single subcatchment in the 2005 study was subdivided into thirteen (13) subcatchments to better delineate the drainage areas to the various tributaries (O’Keefe, Fraser, Foster, Todd, Corrigan and Clarke) and to also provide a better representation of the existing development areas (Kennedy Burnett, Chapman Mills, Jockvale and Hearts Desire). The remaining natural/undeveloped areas within the Jock River corridor were subdivided into three smaller (3) sub-catchments. Refer to Figure 2 (Stantec, 2007) for an overview of the subcatchments for reach one in this model, with full SWMHYMO input and summary files provided in Attachment B.

As mentioned above from this study, it was concluded that developments located in the lower reaches of the Jock River do not require any quantity control storage for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River and that no erosion control storage is required to maintain the pre-development in-stream erosion conditions.

Model 3 – Jock River Reach One Model Update – JFSA, 2021

As a part of the study outlined in this report, the 2007 Stantec SWMHYMO model of the Jock River was updated to reflect (as best as possible with the available information) proposed, approved and potential future developments, since the 2007 study.

At the time of the 2007 study, it was assumed that the floodplain of reach one of the Jock River (from Highway 416 to Greenbank Road) would not be filled and developed. Furthermore, the assumptions that were made in 2007 for the total imperviousness of future developments are not reflective of the actual constructed conditions observed in 2021; for example, the total impervious area for the Todd drainage area was assumed to be 43% in the 2007 study, while based on latest aerial photography it appears that the imperviousness for this area is closer to 58.5%). Additionally, SWM quantity controls were implemented in some tributaries within Reach One (e.g., O'Keefe, Foster and Kennedy-Burnett) to respect the hydraulic capacity of the local watercourses or other existing hydraulic constraints.

As outlined in the Background Data section of this memo, data from various reports and studies were collected and used to update Stantec's 2007 model, to best reflect existing conditions and known approved and planned development projects. As such, the thirteen (13) subcatchments of the 2007 Stantec model have been further discretized into one hundred ten (110) subcatchments, with numerous additional major system storage, SWM Ponds, and channel routing commands added. This updated existing condition model is reflective of current 2021 conditions, which assumes that the lower Jock River floodplain is undeveloped. It should also be noted that only subcatchments downstream of Highway 416 have been updated as a part of this analysis. Updates to other catchments of the Jock River further upstream, such as the Monahan Drain, Hobbs Creek, King Creek, and development areas in Richmond, where additional new information may be available, have not been included in the model updates at this time. Refer to Figure 3 for an overview of the subcatchments for reach one in this model, with full SWMHYMO input and summary files been provided in Attachment C, detailed schematics of the subcatchments updated as a part of this study have been provided in Attachment F.

Note that Novatech's PCSWMM model of the Kennedy-Burnett area was used to create a detailed SWMHYMO model of the same area. In creating this SWMHYMO model it was found that the 100-year peak outflows from the Kennedy Burnett facility were 1.4 times higher than that reported in the Novatech PCSWMM model. While it is expected that different modelling software will produce slightly different results, this difference is significant and should be investigated further; although it is unlikely that this difference is expected to change the fundamental conclusions of this analysis.

Model 4A & 4B – Jock River Reach One Future Conditions (without and with quantity SWM controls) – JFSA, 2021

Two additional models (4A and 4B) were created (which built on model 3) to evaluate the impacts of developing portions of the lower Jock River floodplain (from Highway 416 to Greenbank Road). These lands make up approximately 156 ha and would include BCDC and other properties on the south side of the Jock River. Model 4A assumes that these lands would be developed without any SWM quantity controls and Model 4B assumes that the lands would be developed with SWM Post to Pre-development quantity controls. Refer to Figure 4 for an overview of the subcatchments for reach one in these models, with full SWMHYMO input and summary files for scenarios 4A and 4B provided in Attachment D & E, respectively.

Results

All hydrologic models were run using a 24 hours SCS storm for the 2-to-100-year events. Note that this analysis focuses on this particular rainfall event as for developed areas the summer rainfall events are more critical than the spring rainfall plus snowmelt conditions. Hence, only the summer peak flows have been summarized and compared for the various scenarios below, as the flow contributions from the developments in the lower Jock River under the spring rainfall + snowmelt event are negligible compared to the flows upstream from the greater Jock River. It is further noted that the same design storms were used in all models.

Peak flows at key locations along Reach One of the Jock River have been extracted from the various hydrologic models and provided in the following section. As a part of this analysis, 5 key locations on the Jock River have been selected to compare the simulated peak flows and are as follows: Highway 416, Borrisokane Road, Greenbank Road, Jockvale Road and the Jock River's confluence with the Rideau River. Note that for the older models (JFSA 2005 & Stantec 2007) results have only been provided at some locations, as these original models were not discretized to this higher level of detail. Additionally, the Stantec 2007 model did not assess flows on the Jock River for the 10- and 50-year events at any locations.

Table 1: Comparison of Summer Peak Flows (m³/s) at Highway 416 (52483.00 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	45.676	66.292	82.076	104.643	122.469	141.415
Model 2: Stantec 2007 Reach One Analysis	45.789	66.413	n/a	104.834	n/a	141.853
Model 3: Updated Model 2 to current/ approved conditions	46.294	67.222	83.235	106.109	124.249	143.580
Model 4A: Model 3 with BCDC & others w/o SWM	46.294	67.222	83.235	106.109	124.249	143.580
Model 4B: Model 3 with BCDC & others with SWM	46.294	67.222	83.235	106.109	124.249	143.580

From Table 1 above it is seen that the peak flows on the Jock River at Highway 416 for Models 3, 4A and 4B are higher than in Models 1 and 2. This is because the computational time step in the updated models was reduced from 5 - 10 minutes to 1 minute. This change was necessary to provide stable results in the various models, especially with the additional ROUTE CHANNEL commands that have been added to the updated models. There were no other changes made to the models upstream of Highway 416.

Table 2: Comparison of Summer Peak Flows (m³/s) at Borrisokane Road (53577.82 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	n/a	n/a	n/a	n/a	n/a	n/a
Model 2: Stantec 2007 Reach One Analysis	46.817	68.124	n/a	107.402	n/a	144.892
Model 3: Updated Model 2 to current/ approved conditions	47.379	69.117	85.613	108.988	127.740	147.849
Model 4A: Model 3 with BCDC & others w/o SWM	47.426	68.998	85.561	109.064	127.650	147.535
Model 4B: Model 3 with BCDC & others with SWM	47.599	69.319	85.870	109.449	128.055	147.939

From Table 2 it is seen that the peak flows on the Jock River at Borrisokane are generally lower in model 4A (developed without SWM controls) than under existing conditions (Model 3). The inclusion of SWM controls for these future developments results in the peak flows increasing from existing conditions for all return periods at this location. Note that the increase in flows at this location between the Stantec model (model 2) and the JFSA existing conditions model (model 3) again is due to the greater discretization of subcatchments at Borrisokane Road in the JFSA model. For example, in the Stantec model, the subcatchment that represents the currently undeveloped lands along the Jock River is represented as a single subcatchment (S-1). Where the JFSA updated model represents these lands as 14 individual subcatchments, all discharging to their respective locations within the Jock River (e.g. O’Keefe Creek, Foster Creek & Borrisokane Road).

Table 3: Comparison of Summer Peak Flows (m³/s) at Greenbank Road (54717.80 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	n/a	n/a	n/a	n/a	n/a	n/a
Model 2: Stantec 2007 Reach One Analysis	49.195	71.220	n/a	111.172	n/a	149.236
Model 3: Updated Model 2 to current/ approved conditions	49.055	70.826	86.895	110.282	128.564	147.488
Model 4A: Model 3 with BCDC & others w/o SWM	48.599	69.773	85.389	103.842	126.050	144.531
Model 4B: Model 3 with BCDC & others with SWM	48.982	70.171	85.928	103.651	126.537	144.894

From Table 3 it is seen that peak flows on the Jock River at Greenbank Road are the lowest without SWM controls in place (Model 4A). With SWM controls in place, the peak flows are lower than the existing conditions, but not as low as when SWM controls are not implemented. Note that the JFSA existing conditions model (model 3) is presenting peak flows lower than the Stantec model (model 2) at this location, again this is due to the greater discretization in the JFSA model as discussed above.

Table 4: Comparison of Summer Peak Flows (m³/s) at Jockvale Road (55476.26 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	n/a	n/a	n/a	n/a	n/a	n/a
Model 2: Stantec 2007 Reach One Analysis	49.870	72.143	n/a	112.074	n/a	150.033
Model 3: Updated Model 2 to current/ approved conditions	49.619	72.224	88.294	111.989	130.865	149.819
Model 4A: Model 3 with BCDC & others w/o SWM	49.482	71.017	86.165	105.082	128.174	146.840
Model 4B: Model 3 with BCDC & others with SWM	49.606	71.408	86.690	104.765	128.229	147.027

From Table 4 is seen that the peak flows at Jockvale Road are generally at their lowest without SWM controls in place, and that either implementing or not implementing SWM controls for future developments results in peak flows at this location being less than existing conditions.

Table 5: Comparison of Summer Peak Flows (m³/s) at Outlet of Jock River (55579.20 ha)

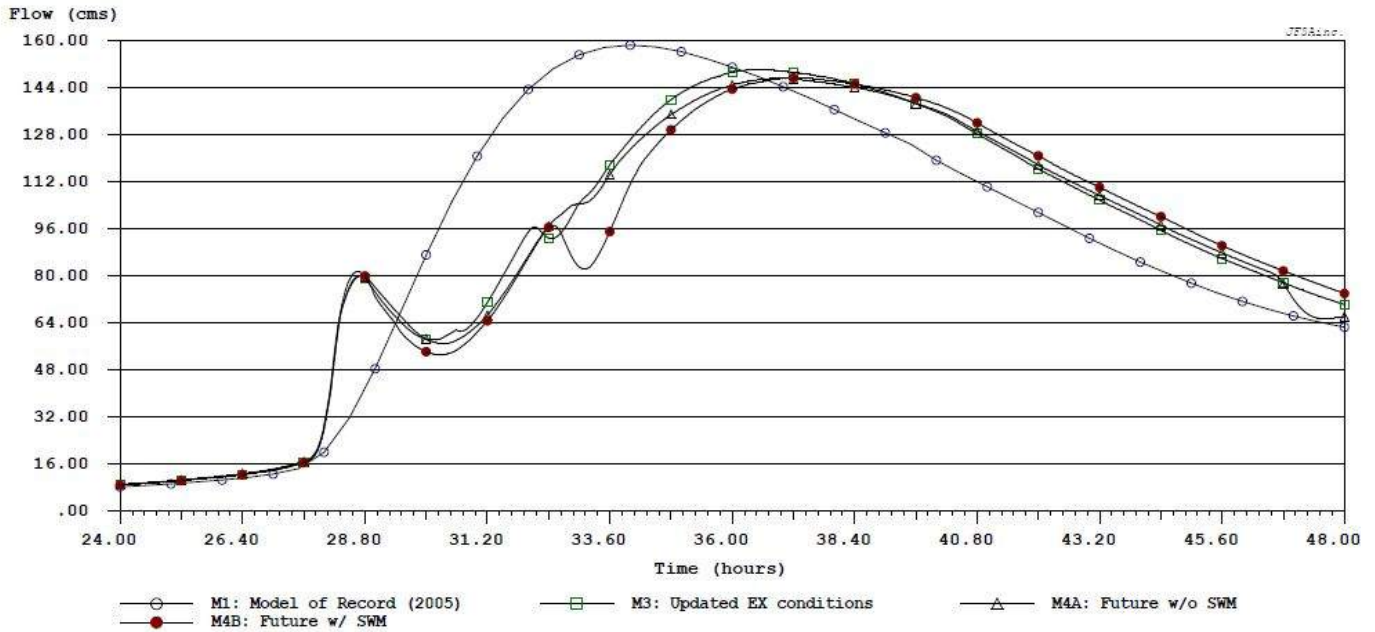
Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	49.16	72.08	89.96	115.84	136.46	158.42
Model 2: Stantec 2007 Reach One Analysis	50.78	73.65	n/a	113.97	n/a	157.69
Model 3: Updated Model 2 to current/ approved conditions	49.72	72.36	88.45	112.2	131.12	150.12
Model 4A: Model 3 with BCDC & others w/o SWM	49.58	71.17	86.35	105.27	128.42	147.10
Model 4B: Model 3 with BCDC & others with SWM	49.70	71.54	86.85	104.96	128.45	147.28

From Table 5 it is seen that the peak flows on the Jock River at the confluence with the Rideau River are generally at their lowest without SWM controls in place, and that either implementing or not implementing SWM controls for future developments results in peak flows at this location to be less than existing conditions.

Discussion

Although not initially obvious, the reason that future urban developments within Reach One of the Jock River are decreasing peak flows on the Jock River is because developing land not only affects the peak of the hydrograph but also the overall shape. Figure A below provides a comparison of the simulated hydrographs at the Jock River's confluence with the Rideau River from the various model scenarios. During any rainfall event, the runoff from the existing and future developments within Reach One will have already peaked and decayed before the peak flows arrive at this location from the upstream drainage area. For the 100-Year SCS storm, the peak from the development in Reach One can be seen in the figure below at around 28 hours, while the peak flow on the Jock River from the upstream drainage areas occurs at around 36-37 hours, this is approximately a 9-hour difference in timing. As such, implementing SWM measures for developments in the lower portions of the Jock River will decrease peak flows from the development, but would also prolong the period of time during which they discharge into the Jock River, thus coinciding with flows from the greater Jock River, ultimately resulting in potential increases in peak flows on the Jock River. This is seen in the figure below with the future condition with SWM controls (Model 4B - Red Circles) having a higher flow in the tail than future conditions without SWM controls (Model 4A – Black Triangles). Note that the difference between Model 1 and all other scenarios is simply due to further discretization of subcatchments within Reach One.

Figure A: Comparison of simulated 100 yr Jock River hydrographs at the confluence with the Rideau River



Hydrograph Statistics:

Legend	Filename & Comment	Time Step (min)	Drainage Area (ha)	Peak Flow (cms)	Time to Peak (hrs)	Runoff Volume (mm)	Runoff Volume (cu.m)	Duration of flow (hrs)	Average flow (cms)
○	N1.0100 : M1: Model of Record (2005)	30.00	55659.00	158.420	34.000	14.52	8.082E+06	24.000	93.538
□	SN_N1.0100 : M3: Updated EX conditions	1.00	55579.20	150.120	36.533	14.24	7.914E+06	24.000	91.603
△	SN_N1.0100 : M4A: Future w/o SWM	1.00	55579.20	147.102	36.917	14.12	7.848E+06	24.000	90.831
●	SN_N1.0100 : M4B: Future w/ SWM	1.00	55579.19	147.276	37.250	14.03	7.798E+06	24.000	90.252

Conclusion

The hydrologic model developed as a part of the Jock River Reach One Subwatershed Study (Stantec 2007) has been updated to provide additional refinements in the lower reaches of the Jock River (downstream of Highway 416) and assumes the development of lands that were previously not considered in the 2007 analysis (e.g. Barrhaven Conservancy). Future development condition models were created with and without SWM controls assumed, and the peak flows extracted from these models at key locations along the lower Jock River. From this analysis, it was found that with these additional developments in the lower Jock River peak flows are generally less than existing conditions without SWM controls in place. Implementing SWM controls for these developments has also been found to generally decrease peak flows on the Jock River, but not at all locations and not to the same degree as without SWM controls. Ultimately these findings are consistent with the fundamental conclusions drawn in Stantec's 2007 Jock River Reach One study, which initially determined that for future developments within Reach One of the Jock River "No quantity control storage is required for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River" and that "No erosion control storage is required to maintain the pre-development in-stream erosion condition". Based on the results of the updated analysis outlined in this memo, it can confirm that the fundamental conclusions drawn in Stantec's 2007 for developments in reach one of the Jock River remain valid.

Yours truly,
J.F Sabourin and Associates Inc.



Jonathon Burnett, P.Eng
Water Resources Engineer

cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects



Figures

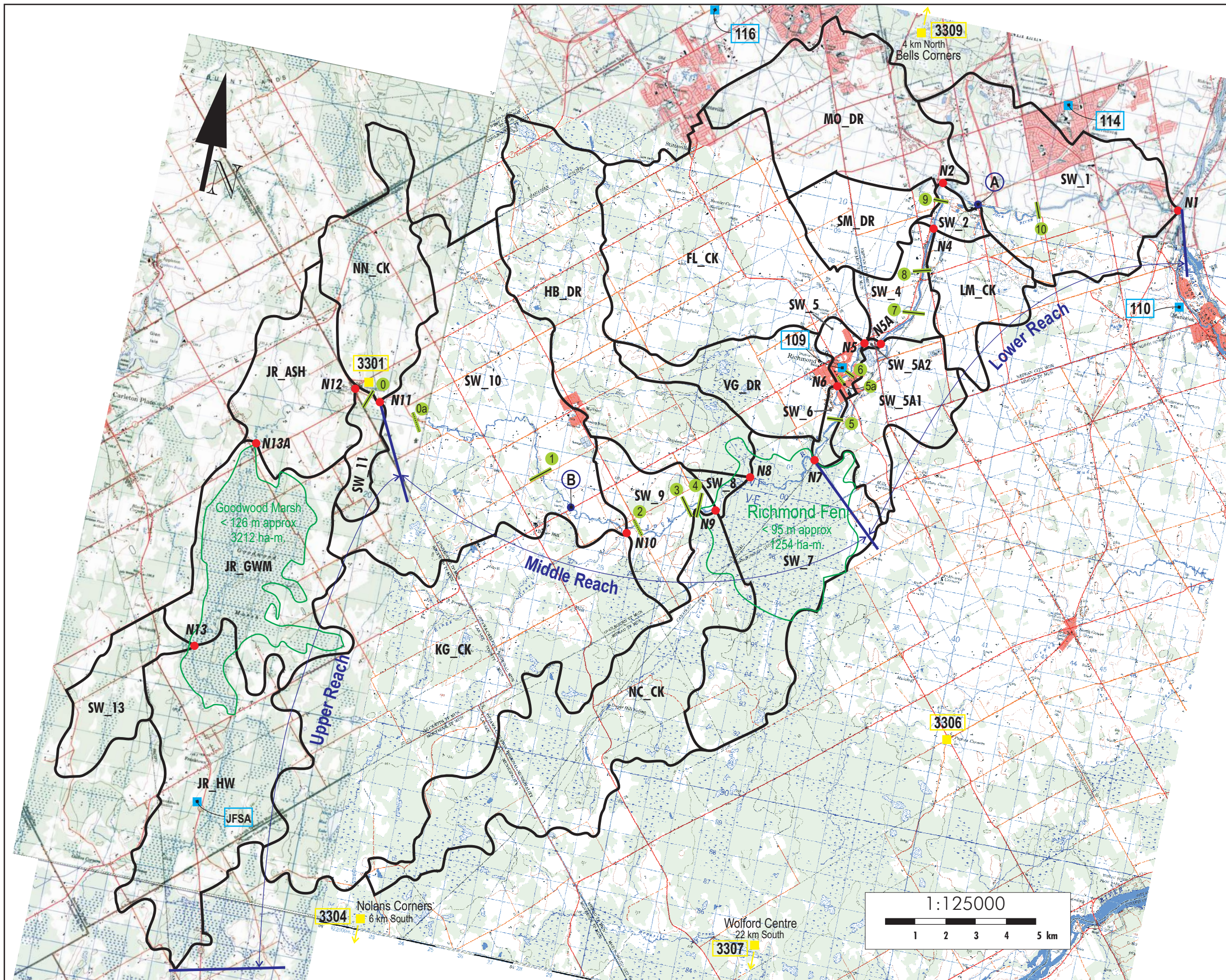
- Figure 1: Model 1 – Jock River Floodplain Model – JFSA, 2005
- Figure 2: Model 2 – Jock River Reach One Model – Stantec, 2007
- Figure 3: Model 3 – Jock River Reach One Model Update - JFSA, 2021
- Figure 4: Model 4A & 4B – Jock River Reach One Future Conditions - JFSA, 2021

Tables

- Table 1: Summer Peak Flows at Highway 416
- Table 2: Summer Peak Flows at Borrisokane Road.
- Table 3: Summer Peak Flows at Greenbank Road
- Table 4: Summer Peak Flows at Jockvale Road
- Table 5: Summer Peak Flows at Outlet of Jock River

Attachments

- Attachment A: Model 1 - SWMHYMO Input & Summary files
- Attachment B: Model 2 - SWMHYMO Input & Summary files
- Attachment C: Model 3 - SWMHYMO Input & Summary files
- Attachment D: Model 4A - SWMHYMO Input & Summary files
- Attachment E: Model 4B - SWMHYMO Input & Summary files
- Attachment F: Updated Subcatchment Schematics & Tables



- Legend:**
- Watershed
 - Bogs (Reservoir)
 - Nodes
 - Flow gauges
 - Rain gauges
 - Snow course stations
 - River Cross-Sections (based on topo. maps)
 - River Cross-Sections adjusted with field data
 - Reaches limits
- Flow gauges ID**
- 02LA007- Jock River near Richmond
 - Jock River at Franktown Rd
- Rain gauges ID**
- Richmond
 - Manotick
 - Barrhaven
 - Maple Grove
 - JFSA Inc, Temporary Rain Gauge 2003
- Snow course stations ID**
- Ashton
 - Nolans Corners
 - Pierces Corners
 - Bells Corners
 - Wolford Centre

Client:

**RIDEAU VALLEY
CONSERVATION AUTHORITY**

Project:
Jock River Flood
Plain Mapping Study

Title:
Watershed Delineation

J.F. Sabourin & Associates Inc.
WATER RESOURCES AND ENVIRONMENTAL CONSULTANTS
OTTAWA (613) 727-5199
GATINEAU (819) 243-6858

Figure 1 Ref. File: Base Map Jock River B.cdr



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 Ottawa ON Canada
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 Tel. 613.722.4420
 Fax. 613.722.2799
 www.stantec.com

Stantec

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Legend

- Existing SWM Facility
- Proposed Stormwater Facility
- Proposed Pond Outlet
- Jock River Tributary (Municipal Drain)
- Jock River Tributary (Non-Municipal Drain)
- Watershed Boundary
- Regulatory Flood Level
- Normal Water Level
- Woodlot Limits
- Sub-drainage Area Limit
- Sub-drainage Area Name
- Sub-drainage Area Size
- % Impervious or Time to Peak (hours)
- SCS Curve Number
- Application Plans Under Review
- 10 Year Flood Line
- 25 Year Flood Line
- 100 Year Flood Line

1	FINAL REPORT	BCB	N.C.	JUNE 2007
Revision		By	Appd.	YY.MM.DD
File Name:		Own.	Chgd.	Dgn.
				YY.MM.DD

Seal

Client/Project

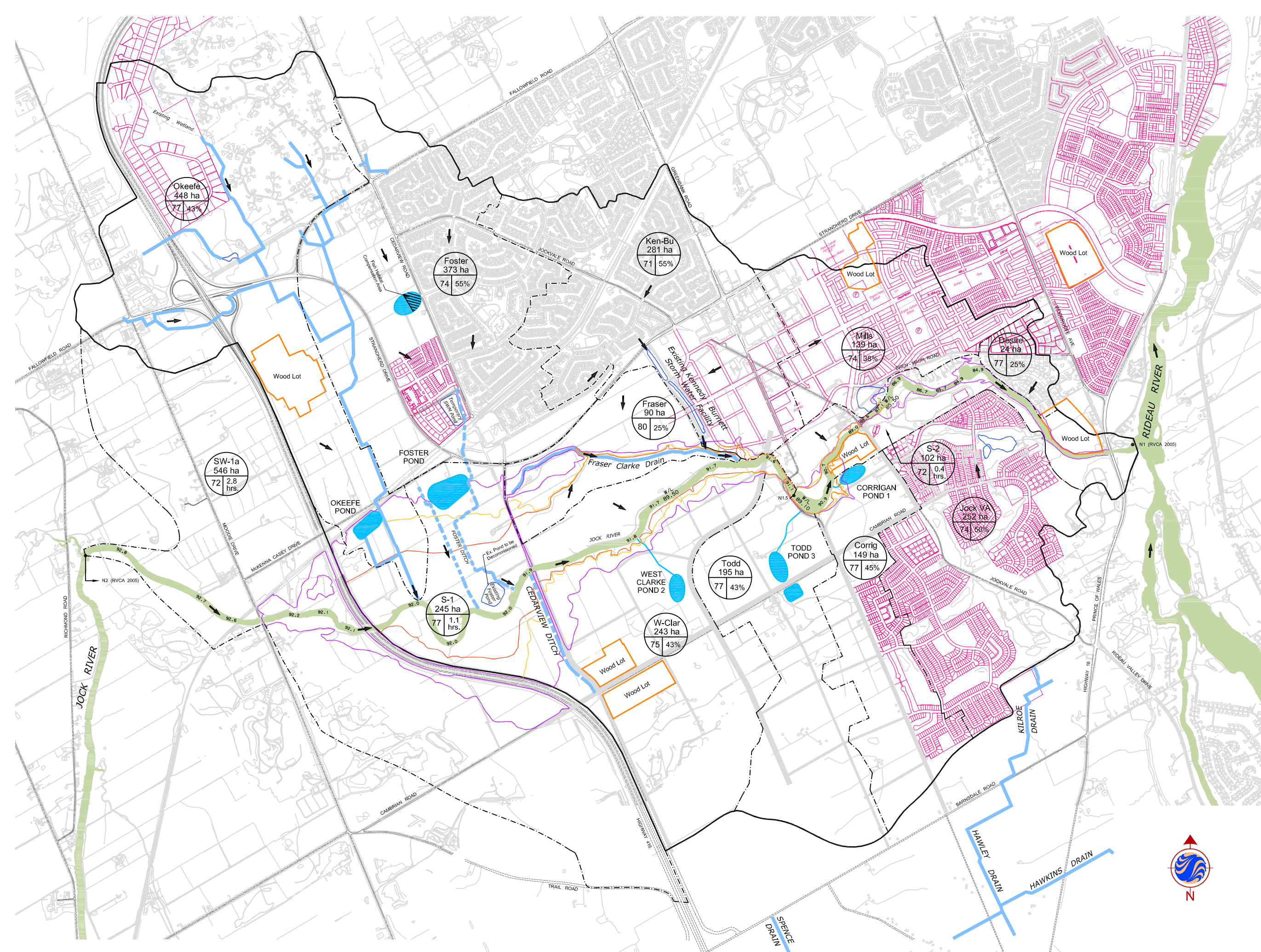
JOCK RIVER REACH ONE
 SUB-WATERSHED STUDY
 Ottawa ON Canada

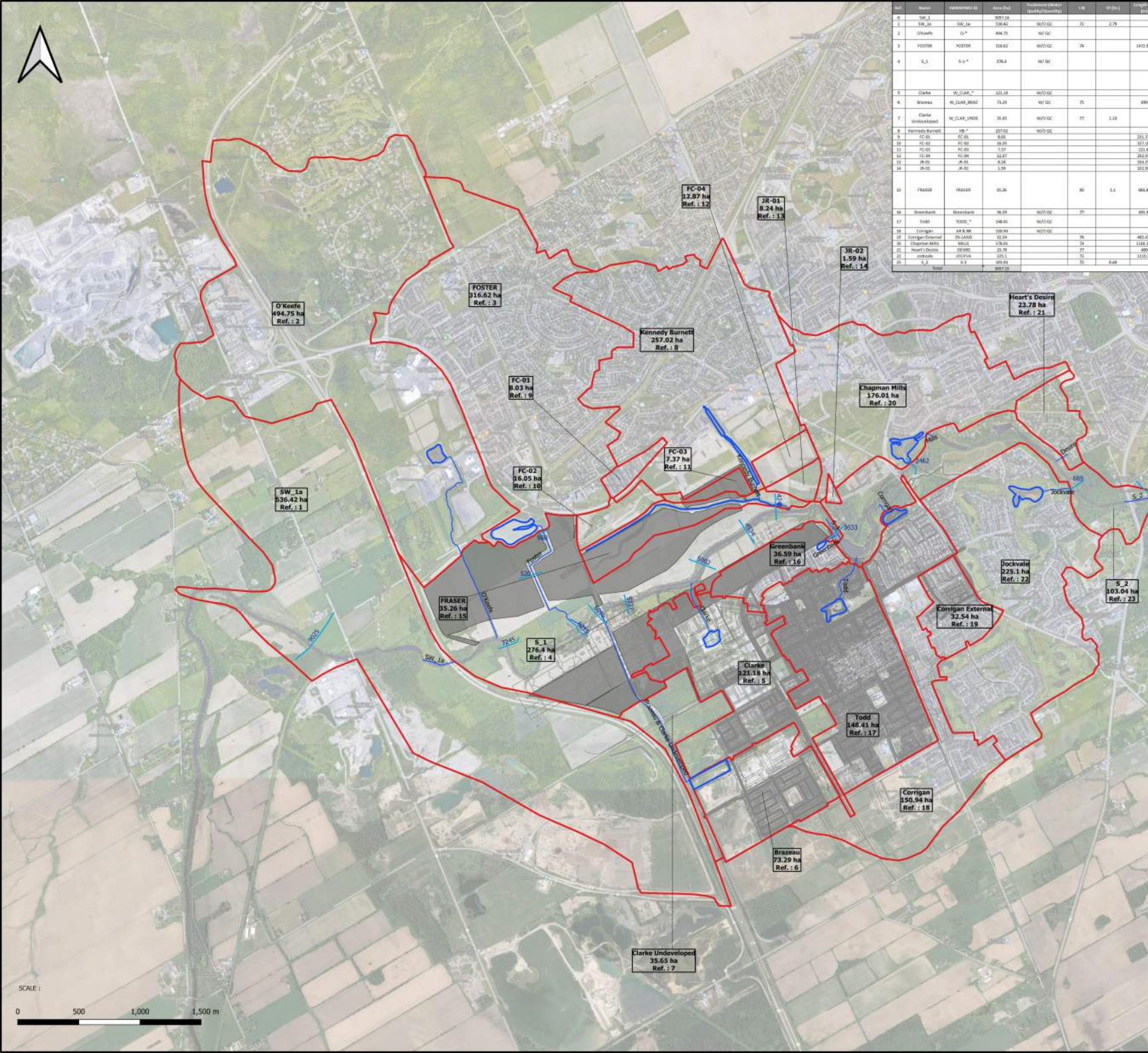
Title
 PROPOSED CONDITIONS
 HYDROLOGIC MODEL
 DRAINAGE BOUNDARIES

Project No. 60400414 Scale 0 100 300 500m
 Drawing No. 1:10,000 Sheet 1 of 1

Figure 2

1 of 1 1





Area ID	Area (ha)	Reference Number	Area Type	Area Description
SW_1a	536.42	Ref. 1	Waterway	Jock River at Station 3300
FC-01	8.03	Ref. 9	Floodplain	Jock River at Station 3300
FC-02	16.05	Ref. 10	Floodplain	Jock River at Station 3300
FC-03	7.37	Ref. 11	Floodplain	Jock River at Station 3300
FC-04	12.87	Ref. 12	Floodplain	Jock River at Station 3300
JR-01	8.24	Ref. 13	Jock River	Jock River at Station 3300
JR-02	1.59	Ref. 14	Jock River	Jock River at Station 3300
O'Keefe	494.75	Ref. 2	Waterway	Jock River at Station 3300
FRASER	35.26	Ref. 15	Floodplain	Jock River at Station 3300
FRASER	35.26	Ref. 15	Floodplain	Jock River at Station 3300
Kennedy Burnett	257.02	Ref. 8	Waterway	Jock River at Station 3300
Kennedy Burnett	257.02	Ref. 8	Waterway	Jock River at Station 3300
Chapman Mills	176.01	Ref. 20	Waterway	Jock River at Station 3300
Greenbank	36.59	Ref. 16	Waterway	Jock River at Station 3300
Greenbank	36.59	Ref. 16	Waterway	Jock River at Station 3300
Jockvale	225.1	Ref. 22	Waterway	Jock River at Station 3300
Jockvale	225.1	Ref. 22	Waterway	Jock River at Station 3300
S_2	103.04	Ref. 23	Waterway	Jock River at Station 3300
S_2	103.04	Ref. 23	Waterway	Jock River at Station 3300
Corrigan External	32.54	Ref. 19	Waterway	Jock River at Station 3300
Clarke	221.18	Ref. 5	Waterway	Jock River at Station 3300
Todd	148.41	Ref. 17	Waterway	Jock River at Station 3300
Corrigan	150.94	Ref. 18	Waterway	Jock River at Station 3300
Brazeau	73.29	Ref. 6	Waterway	Jock River at Station 3300
Clarke Undeveloped	35.65	Ref. 17	Waterway	Jock River at Station 3300

File name: **FIGURE 3 - Overall Jock River Lower Reach one Sub-catchments.pdf**

XS 4534 Cross Section at station 4534

- SW_1a 536.42 ha Ref. 1
- Area ID
- Area (ha)
- Reference Number

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DSEL
david schaeffer engineering ltd

PROJECT : **BCDC - Quantity Control Study**

TITLE : **Figure 3 - Overall Jock River Lower Reach one Sub-catchments
Table 3 - Overall Jock River Lower Reach one Sub-catchments**

PROJECT NO.	1474-16
DRAWN:	MM
DATE:	Mar. 2021





- Legend**
- Channel Cross Sections
 - S-1 Sub-catchments and Fraser Sub-catchments
 - S-1-A
 - S-1-B
 - S-1-D1
 - S-1-D2
 - S-1-D3
 - S-1-D4
 - S-1-D5
 - S-1-D6
 - S-1-D7
 - S-1-D8
 - S-1-FO-D1
 - S-1-FO-D2
 - S-1-FO-F-D
 - S-1-Okeefe
 - FRASER-DRN
 - FRASER-D
- Google Hybrid

File name: 20210304_S-1_Fraser_Schematic-Model4A.pdf

XS 3633 Cross Section at station 3633

S-1-A
90.84 ha
Ref. 1

▼ Area ID
► Area (ha)
► Reference Number

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PROJECT :
BCDC - Quantity Control Study

TITLE :
S-1 Sub-catchment and Fraser Clarke Sub-catchment Schematic

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021

Station	Channel	Area (ha)	Perimeter (m)	Flow (m³/s)	Velocity (m/s)	Depth (m)	Width (m)	Flow Velocity (m/s)	Flow Depth (m)	Flow Width (m)	Flow Area (m²)	Flow Velocity (m/s)	Flow Depth (m)	Flow Width (m)	Flow Area (m²)
3633	FRASER-D	90.84	1200	100	0.5	1.0	100	0.5	1.0	100	100	0.5	1.0	100	100





- Legend**
- Channel Cross Sections
 - S-1 Sub-catchments and Fraser Sub-catchments
 - S-1-A
 - S-1-B
 - S-1-D1
 - S-1-D2
 - S-1-D3
 - S-1-D4
 - S-1-D5
 - S-1-D6
 - S-1-D7
 - S-1-D8
 - S-1-FO-D1
 - S-1-FO-D2
 - S-1-FO-F-D
 - S-1-Okeefe
 - FRASER-DRN
 - FRASER-D
- Google Hybrid

File name: 20210304_S-1_Fraser_Schematic-Model4B.pdf

XS 3633 Cross Section at station 3633

S-1-A
90.84 ha
Ref. 1

▼ Area ID
→ Area (ha)
→ Reference Number

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PROJECT :
 BCDC - Quantity Control Study

TITLE :
 S-1 Sub-catchment and Fraser Clarke
 Sub-catchment Schematic

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021



Station	Area (ha)	Reference	Notes
S-1-A	90.84	Ref. 1	
S-1-B	55.36	Ref. 2	
S-1-D1	21.67	Ref. 3	
S-1-D2	18.67	Ref. 4	
S-1-D3	6.79	Ref. 5	
S-1-D4	3.28	Ref. 6	
S-1-D5	12.84	Ref. 7	
S-1-D6	1.75	Ref. 8	
S-1-D7	2.03	Ref. 9	
S-1-D8	5.27	Ref. 10	
S-1-FO-D1	5.11	Ref. 11	
S-1-FO-D2	4.94	Ref. 12	
S-1-FO-F-D	14.96	Ref. 13	
S-1-Okeefe	44.93	Ref. 14	
FRASER-D	21.61	Ref. 16	
FRASER-DRN	13.65	Ref. 15	



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Gatineau, QC
Montréal, QC
Québec, QC

Attachment A

Model 1 – Jock River Floodplain Model

JFSA, 2005

SWMHYMO Input & Summary files

```

1  20    Metric units / ID numbers OFF
2  *#*****
3  *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *#*****
5  *# Project Name: [Jock River]    Project Number: [411-02]
6  *# Date       : 06-06-2003
7  *# Modeller   : [JoF]
8  *# Company    : JFSAinc.
9  *# License #  : 2549237
10 *#*****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *
16 * Calibrated parameters for Summer 2003 data:  APII=50, APIK=0.85, CN=varies,
17 *                                                SK=0.01, InterEventTime=12,
18 *                                                GWResk=0.96, VHydCond=0.055
19 *
20 *# -----
21 *
22 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
23 *              ["XAVG0315.STM"] average storm data a 15 minute time step
24 *              The above rainf file is an average of the JFSA gauge data
25 *              with the City of Ottawa rainfall data collected during
26 *              the same period.
27 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
28 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
29 *%              ["C24SC002.stm"] <--storm filename, one per line for NSTORM time
30 *%-----|-----|
31 *%-----|-----|
32 READ STORM     STORM_FILENAME=["storm.001"]
33 *%-----|-----|
34 MODIFY STORM   ICASEms=[1], NSHIFT=[96],
35 *              RedFACT=[1],
36 *%-----|-----|
37 COMPUTE API   APII=[50], APIK=[.85]/day
38 *%-----|-----|
39 *%-----|-----|
40 *#
41 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
42 *# of 1.32
43 *%-----|-----|
44 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[30]min, AREA=[3680](ha),
45 *              DWF=[0](cms), CN/C=[64], IA=[2.5](mm),
46 *              N=[3.0], TP=[7.13]hrs,
47 *              Continuous simulation parameters:
48 *              IaREcper=[4](hrs),
49 *              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
50 *              InterEventTime=[12](hrs)
51 *              Baseflow simulation parameters:
52 *              BaseFlowOption=[1] ,
53 *              InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
54 *              VHydCond=[0.055](mm/hr), END=-1
55 *%-----|-----|
56 *#
57 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
58 *# of 1.32
59 *%-----|-----|
60 CONTINUOUS NASHYD NHYD=["SW_13"], DT=[30]min, AREA=[971](ha),
61 *              DWF=[0](cms), CN/C=[61], IA=[2.5](mm),
62 *              N=[3.0], TP=[3.76]hrs,
63 *              Continuous simulation parameters:
64 *              IaREcper=[4](hrs),
65 *              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
66 *              InterEventTime=[12](hrs)

```

```

67         Baseflow simulation parameters:
68         BaseFlowOption=[1] ,
69         InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
70         VHydCond=[0.055](mm/hr) ,    END=-1
71     *%-----|-----
72     *#
73     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
74     *# of 1.80
75     *%-----|-----
76     CONTINUOUS NASHYD  NHYD=["JR_GWM"] , DT=[30]min , AREA=[3074](ha) ,
77                       DWF=[0](cms) , CN/C=[55] , IA=[2.5](mm) ,
78                       N=[3] , TP=[11.33]hrs ,
79                       Continuous simulation parameters:
80                       IaRECPper=[4](hrs) ,
81                       SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
82                       InterEventTime=[12](hrs)
83                       Baseflow simulation parameters:
84                       BaseFlowOption=[1] ,
85                       InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
86                       VHydCond=[0.055](mm/hr) ,    END=-1
87     *%-----|-----
88     CONTINUOUS NASHYD  NHYD=["JR_ASH"] , DT=[30]min , AREA=[1781](ha) ,
89                       DWF=[0](cms) , CN/C=[72] , IA=[2.5](mm) ,
90                       N=[3.0] , TP=[3.91]hrs ,
91                       Continuous simulation parameters:
92                       IaRECPper=[4](hrs) ,
93                       SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
94                       InterEventTime=[12](hrs)
95                       Baseflow simulation parameters:
96                       BaseFlowOption=[1] ,
97                       InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
98                       VHydCond=[0.055](mm/hr) ,    END=-1
99     *%-----|-----
100    CONTINUOUS NASHYD  NHYD=["SW_11"] , DT=[30]min , AREA=[500](ha) ,
101                      DWF=[0](cms) , CN/C=[66] , IA=[2.5](mm) ,
102                      N=[3.0] , TP=[1.24]hrs ,
103                      Continuous simulation parameters:
104                      IaRECPper=[4](hrs) ,
105                      SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
106                      InterEventTime=[12](hrs)
107                      Baseflow simulation parameters:
108                      BaseFlowOption=[1] ,
109                      InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
110                      VHydCond=[0.055](mm/hr) ,    END=-1
111    *%-----|-----
112    *#
113    *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
114    *# of 1.80
115    *%-----|-----
116    CONTINUOUS NASHYD  NHYD=["NN_CK"] , DT=[30]min , AREA=[1917](ha) ,
117                      DWF=[0](cms) , CN/C=[66] , IA=[2.5](mm) ,
118                      N=[3.0] , TP=[5.29]hrs ,
119                      Continuous simulation parameters:
120                      IaRECPper=[4](hrs) ,
121                      SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
122                      InterEventTime=[12](hrs)
123                      Baseflow simulation parameters:
124                      BaseFlowOption=[1] ,
125                      InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
126                      VHydCond=[0.055](mm/hr) ,    END=-1
127    *%-----|-----
128    *#
129    *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
130    *# of 1.52
131    *%-----|-----
132    CONTINUOUS NASHYD  NHYD=["SW_10"] , DT=[30]min , AREA=[5666](ha) ,

```

```

133 DWF=[0](cms), CN/C=[72], IA=[2.5](mm),
134 N=[3.0], TP=[8.00]hrs,
135 Continuous simulation parameters:
136 IaREcper=[4](hrs),
137 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
138 InterEventTime=[12](hrs)
139 Baseflow simulation parameters:
140 BaseFlowOption=[1] ,
141 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
142 VHydCond=[0.055](mm/hr), END=-1
143 *%-----|-----
144 *#
145 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
146 *# of 1.75
147 *%-----|-----
148 CONTINUOUS NASHYD NHYD=["KG_CK"], DT=[30]min, AREA=[8376](ha),
149 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
150 N=[3.0], TP=[11.66]hrs,
151 Continuous simulation parameters:
152 IaREcper=[4](hrs),
153 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
154 InterEventTime=[12](hrs)
155 Baseflow simulation parameters:
156 BaseFlowOption=[1] ,
157 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
158 VHydCond=[0.055](mm/hr), END=-1
159 *%-----|-----
160 *#
161 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
162 *# of 1.68
163 *%-----|-----
164 CONTINUOUS NASHYD NHYD=["SW_9"], DT=[30]min, AREA=[1132](ha),
165 DWF=[0](cms), CN/C=[70], IA=[2.5](mm),
166 N=[3.0], TP=[2.51]hrs,
167 Continuous simulation parameters:
168 IaREcper=[4](hrs),
169 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
170 InterEventTime=[12](hrs)
171 Baseflow simulation parameters:
172 BaseFlowOption=[1] ,
173 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
174 VHydCond=[0.055](mm/hr), END=-1
175 *%-----|-----
176 *#
177 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
178 *# of 1.82
179 *%-----|-----
180 CONTINUOUS NASHYD NHYD=["NC_CK"], DT=[30]min, AREA=[4464](ha),
181 DWF=[0](cms), CN/C=[62], IA=[2.5](mm),
182 N=[3.0], TP=[11.32]hrs,
183 Continuous simulation parameters:
184 IaREcper=[4](hrs),
185 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
186 InterEventTime=[12](hrs)
187 Baseflow simulation parameters:
188 BaseFlowOption=[1] ,
189 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
190 VHydCond=[0.055](mm/hr), END=-1
191 *%-----|-----
192 *#
193 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
194 *# of 1.80
195 *%-----|-----
196 CONTINUOUS NASHYD NHYD=["SW_8"], DT=[30]min, AREA=[131](ha),
197 DWF=[0](cms), CN/C=[63], IA=[2.5](mm),
198 N=[3.0], TP=[0.90]hrs,

```

```

199         Continuous simulation parameters:
200         IaREcper=[4](hrs),
201         SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
202         InterEventTime=[12](hrs)
203         Baseflow simulation parameters:
204         BaseFlowOption=[1] ,
205         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
206         VHydCond=[0.055](mm/hr),  END=-1
207     *%-----|-----
208     *#
209     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
210     *# of 1.65
211     *%-----|-----
212     CONTINUOUS NASHYD  NHYD=["HB_DR"], DT=[30]min, AREA=[3854](ha),
213                       DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),
214                       N=[3.0], TP=[8.42]hrs,
215                       Continuous simulation parameters:
216                       IaREcper=[4](hrs),
217                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
218                       InterEventTime=[12](hrs)
219                       Baseflow simulation parameters:
220                       BaseFlowOption=[1] ,
221                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
222                       VHydCond=[0.055](mm/hr),  END=-1
223     *%-----|-----
224     *#
225     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
226     *# of 1.82
227     *%-----|-----
228     CONTINUOUS NASHYD  NHYD=["SW_7"], DT=[30]min, AREA=[3197](ha),
229                       DWF=[0](cms),  CN/C=[57], IA=[2.5](mm),
230                       N=[3.0], TP=[6.65]hrs,
231                       Continuous simulation parameters:
232                       IaREcper=[4](hrs),
233                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
234                       InterEventTime=[12](hrs)
235                       Baseflow simulation parameters:
236                       BaseFlowOption=[1] ,
237                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
238                       VHydCond=[0.055](mm/hr),  END=-1
239     *%-----|-----
240     *#
241     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
242     *# of 1.75
243     *%-----|-----
244     CONTINUOUS NASHYD  NHYD=["SW_6"], DT=[30]min, AREA=[165](ha),
245                       DWF=[0](cms),  CN/C=[67], IA=[2.5](mm),
246                       N=[3.0], TP=[4.18]hrs,
247                       Continuous simulation parameters:
248                       IaREcper=[4](hrs),
249                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
250                       InterEventTime=[12](hrs)
251                       Baseflow simulation parameters:
252                       BaseFlowOption=[1] ,
253                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
254                       VHydCond=[0.055](mm/hr),  END=-1
255     *%-----|-----
256     *#
257     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
258     *# of 1.67
259     *%-----|-----
260     CONTINUOUS NASHYD  NHYD=["VG_DR"], DT=[30]min, AREA=[1332](ha),
261                       DWF=[0](cms),  CN/C=[72], IA=[2.5](mm),
262                       N=[3.0], TP=[5.95]hrs,
263                       Continuous simulation parameters:
264                       IaREcper=[4](hrs),

```

```

265 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
266 InterEventTime=[12](hrs)
267 Baseflow simulation parameters:
268 BaseFlowOption=[1] ,
269 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
270 VHydCond=[0.055](mm/hr), END=-1
271 *%-----|
272 CONTINUOUS NASHYD NHYD=["SW_5"], DT=[30]min, AREA=[224](ha),
273 DWF=[0](cms), CN/C=[77], IA=[2.5](mm),
274 N=[3.0], TP=[0.75]hrs,
275 Continuous simulation parameters:
276 IaREcper=[4](hrs),
277 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
278 InterEventTime=[12](hrs)
279 Baseflow simulation parameters:
280 BaseFlowOption=[1] ,
281 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
282 VHydCond=[0.055](mm/hr), END=-1
283 *%-----|
284 *#
285 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
286 *# of 1.20
287 *%-----|
288 CONTINUOUS NASHYD NHYD=["FL_CK"], DT=[30]min, AREA=[4945](ha),
289 DWF=[0](cms), CN/C=[74], IA=[2.5](mm),
290 N=[3.0], TP=[4.45]hrs,
291 Continuous simulation parameters:
292 IaREcper=[4](hrs),
293 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
294 InterEventTime=[12](hrs)
295 Baseflow simulation parameters:
296 BaseFlowOption=[1] ,
297 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
298 VHydCond=[0.055](mm/hr), END=-1
299 *%-----|
300 CONTINUOUS NASHYD NHYD=["SW_5A2"], DT=[30]min, AREA=[20](ha),
301 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
302 N=[3.0], TP=[0.62]hrs,
303 Continuous simulation parameters:
304 IaREcper=[4](hrs),
305 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
306 InterEventTime=[12](hrs)
307 Baseflow simulation parameters:
308 BaseFlowOption=[1] ,
309 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
310 VHydCond=[0.055](mm/hr), END=-1
311 *%-----|
312 *#
313 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
314 *# of 1.61
315 *%-----|
316 CONTINUOUS NASHYD NHYD=["SW_5A1"], DT=[30]min, AREA=[1412](ha),
317 DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
318 N=[3.0], TP=[8.00]hrs,
319 Continuous simulation parameters:
320 IaREcper=[4](hrs),
321 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
322 InterEventTime=[12](hrs)
323 Baseflow simulation parameters:
324 BaseFlowOption=[1] ,
325 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
326 VHydCond=[0.055](mm/hr), END=-1
327 *%-----|
328 CONTINUOUS NASHYD NHYD=["SW_4"], DT=[30]min, AREA=[585](ha),
329 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
330 N=[3.0], TP=[1.75]hrs,

```

```

331 Continuous simulation parameters:
332 IaREcper=[4](hrs),
333 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
334 InterEventTime=[12](hrs)
335 Baseflow simulation parameters:
336 BaseFlowOption=[1] ,
337 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
338 VHydCond=[0.055](mm/hr), END=-1
339 *%-----|
340 CONTINUOUS NASHYD NHYD=["LM_CK"], DT=[30]min, AREA=[1021](ha),
341 DWF=[0](cms), CN/C=[80], IA=[2.5](mm),
342 N=[3.0], TP=[2.46]hrs,
343 Continuous simulation parameters:
344 IaREcper=[4](hrs),
345 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
346 InterEventTime=[12](hrs)
347 Baseflow simulation parameters:
348 BaseFlowOption=[1] ,
349 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
350 VHydCond=[0.055](mm/hr), END=-1
351 *%-----|
352 CONTINUOUS NASHYD NHYD=["SW_2"], DT=[30]min, AREA=[177](ha),
353 DWF=[0](cms), CN/C=[77], IA=[2.5](mm),
354 N=[3.0], TP=[0.75]hrs,
355 Continuous simulation parameters:
356 IaREcper=[4](hrs),
357 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
358 InterEventTime=[12](hrs)
359 Baseflow simulation parameters:
360 BaseFlowOption=[1] ,
361 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
362 VHydCond=[0.055](mm/hr), END=-1
363 *%-----|
364 CONTINUOUS NASHYD NHYD=["SM_DR"], DT=[30]min, AREA=[1122](ha),
365 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
366 N=[3.0], TP=[3.25]hrs,
367 Continuous simulation parameters:
368 IaREcper=[4](hrs),
369 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
370 InterEventTime=[12](hrs)
371 Baseflow simulation parameters:
372 BaseFlowOption=[1] ,
373 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
374 VHydCond=[0.055](mm/hr), END=-1
375 *%-----|
376 CONTINUOUS NASHYD NHYD=["MO_DR"], DT=[30]min, AREA=[2737](ha),
377 DWF=[0](cms), CN/C=[76], IA=[2.5](mm),
378 N=[3.0], TP=[3.03]hrs,
379 Continuous simulation parameters:
380 IaREcper=[4](hrs),
381 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
382 InterEventTime=[12](hrs)
383 Baseflow simulation parameters:
384 BaseFlowOption=[1] ,
385 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
386 VHydCond=[0.055](mm/hr), END=-1
387 *%-----|
388 CONTINUOUS NASHYD NHYD=["SW_1"], DT=[30]min, AREA=[3176](ha),
389 DWF=[0](cms), CN/C=[78], IA=[2.5](mm),
390 N=[3.0], TP=[3.56]hrs,
391 Continuous simulation parameters:
392 IaREcper=[4](hrs),
393 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
394 InterEventTime=[12](hrs)
395 Baseflow simulation parameters:
396 BaseFlowOption=[1] ,

```



```

397          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
398          VHydCond=[0.055](mm/hr),   END=-1
399  *%-----|-----
400  *#
401  *# Routing hydrographs
402  *#
403  *# Starting with the addition of Jock River Headwater and Subwatershed 13
404  *#
405  ADD HYD          NHYDsum=["S_N13"], NHYDs to add=["JR_HW"+"SW_13"]
406  *%-----|-----
407  *#
408  *# Sum of hydrographs from Node 13 routed to Node 13A
409  *# (Approximated cross-section - see cross-section 258)
410  *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
411  *#
412  ROUTE CHANNEL    NHYDout=["N13A"], NHYDin=["S_N13"],
413                  RDT=[30](min),
414                  CHLGTH=[9074](m),  CHSLOPE=[0.0220](%),
415                  FPSLOPE=[0.0220](%),
416                  SECNUM=[1.0],      NSEG=[1]
417                  ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
418                  ( DISTANCE (m), ELEVATION (m))=
419                      [-40, 132.5]
420                      [-30, 132]
421                      [-25, 131.5]
422                      [-13, 130]
423                      [-8, 127.00]
424                      [-7, 126.50]
425                      [-6, 126]
426                      [-5.5, 125.50]
427                      [0, 123.75]
428                      [4.5, 125.50]
429                      [6, 126]
430                      [7.5, 126.5]
431                      [9, 127]
432                      [10, 127.5]
433                      [11.5, 128.0]
434                      [15.5, 129.5]
435  *%-----|-----
436  *#
437  *# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
438  *#
439  ADD HYD          NHYDsum=["SN13A"], NHYDs to add=["N13A"+"JR_GWM"]
440  *%-----|-----
441  *#
442  *# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
443  *#
444  ROUTE RESERVOIR NHYDout=["RES_GM"], NHYDin=["SN13A"],
445                  RDT=[30](min),
446                  TABLE of ( OUTFLOW-STORAGE ) values
447                      (cms) - (ha-m)
448                      [ 0.0 , 0.0 ]
449                      [1.991, 2.144 ]
450                      [2.693, 39.826 ]
451                      [3.509, 81.697 ]
452                      [4.578, 318.774 ]
453                      [5.647, 594.947 ]
454                      [7.109, 910.219 ]
455                      [8.616, 1264.589 ]
456                      [10.371, 1658.057 ]
457                      [12.402, 2090.622 ]
458                      [22.056, 3462.487 ]
459                      [ -1 , -1 ] (max twenty pts)
460                  NHYDovf=[" " ],
461  *%-----|-----
462  *#

```

```

463 SAVE HYD           NHYD=["RES_GM"], # OF PCYCLES=[-1], ICASEsh=[-1]
464                   HYD_FILENAME=["H_RESGM"]
465                   HYD_COMMENT=["Outflow from Res GM"]
466 *%-----|-----
467 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
468 *# (Approximated cross-section - see cross-section 258)
469 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
470 ROUTE CHANNEL     NHYDout=["N12"] ,NHYDin=["RES_GM"] ,
471                   RDT=[30](min),
472                   CHLGTH=[5926](m),   CHSLOPE=[0.0759](%),
473                                           FPSLOPE=[0.0759](%),
474                   SECNUM=[1.0],       NSEG=[1]
475                   ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
476                   ( DISTANCE (m), ELEVATION (m))=
477                       [-40, 132.5]
478                       [-30, 132]
479                       [-25, 131.5]
480                       [-13, 130]
481                       [-8, 127.00]
482                       [-7, 126.50]
483                       [-6, 126]
484                       [-5.5, 125.50]
485                       [0, 123.75]
486                       [4.5, 125.50]
487                       [6, 126]
488                       [7.5, 126.5]
489                       [9, 127]
490                       [10, 127.5]
491                       [11.5, 128.00]
492                       [15.5, 129.5]
493 *%-----|-----
494 *#
495 *# Addition of Subwatershed Jock River at Ashton to Node 12
496 *#
497 ADD HYD           NHYDsum=["S_N12"], NHYDs to add=["N12"+"JR_ASH"]
498 SAVE HYD         NHYD=["S_N12"], # OF PCYCLES=[-1], ICASEsh=[-1]
499                   HYD_FILENAME=["H_SN12"]
500                   HYD_COMMENT=["flow at S_N12 near Ashton"]
501 *%-----|-----
502 *#
503 *# Sum of hydrographs from Node 12 routed to Node 11
504 *# (Approximated cross-section - see cross-section 258)
505 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
506 ROUTE CHANNEL     NHYDout=["N11"] ,NHYDin=["S_N12"] ,
507                   RDT=[30](min),
508                   CHLGTH=[972](m),   CHSLOPE=[0.0514](%),
509                                           FPSLOPE=[0.0514](%),
510                   SECNUM=[1.0],       NSEG=[1]
511                   ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
512                   ( DISTANCE (m), ELEVATION (m))=
513                       [-40, 132.5]
514                       [-30, 132]
515                       [-25, 131.5]
516                       [-13, 130]
517                       [-8, 127.00]
518                       [-7, 126.50]
519                       [-6, 126]
520                       [-5.5, 125.50]
521                       [0, 123.75]
522                       [4.5, 125.50]
523                       [6, 126]
524                       [7.5, 126.5]
525                       [9, 127]
526                       [10, 127.5]
527                       [11.5, 128.00]
528                       [15.5, 129.5]

```



```

595  *#
596  *# Addition of Subwatershed 10 to Node 10
597  *#
598  ADD HYD          NHYDsum=["S_N10"], NHYDs to add=["N10"+"SW_10"]
599  *%-----|-----|
600  SAVE HYD        NHYD=["S_N10"], # OF PCYCLES=[-1], ICASEsh=[-1]
601                  HYD_FILENAME=["H_SN10"]
602                  HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
603  *%-----|-----|
604  *# Addition of Kings Creek to S_N10
605  *#
606  ADD HYD          NHYDsum=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
607  *%-----|-----|
608  *#
609  *# Sum of hydrographs from Node 10 routed to Node 9
610  *# Section 2
611  *#
612  ROUTE CHANNEL   NHYDout=["N9"] ,NHYDin=["S_N10A"] ,
613                  RDT=[30](min),
614                  CHLGTH=[3982](m),  CHSLOPE=[0.0753](%),
615                                      FPSLOPE=[0.0753](%),
616                  SECNUM=[1.0],      NSEG=[4]
617                  ( SEGROUGH, SEGDIST (m))=
618                    [0.04,-30.27
619                     0.05,-18.42
620                    -0.05,18.42
621                    0.04,131.58] NSEG times
622                  ( DISTANCE (m), ELEVATION (m))=
623                    [-446.74, 106.00]
624                    [-415.68, 105.50]
625                    [-285.40, 105.00]
626                    [-173.77, 104.50]
627                    [-144.95, 104.00]
628                    [-111.18, 103.50]
629                    [-94.06, 103.00]
630                    [-71.02, 102.50]
631                    [-30.27, 102.00]
632                    [-19.33, 100.00]
633                    [-18.42, 99.50]
634                    [18.42, 99.50]
635                    [20.77, 100.00]
636                    [27.93, 101.00]
637                    [52.29, 101.00]
638                    [68.80, 101.50]
639                    [79.66, 103.00]
640                    [91.50, 103.50]
641                    [131.58, 104.00]
642  *%-----|-----|
643  *#
644  *# Addition of Subwatershed 9 and Nichols Creek to Node 9
645  *#
646  ADD HYD          NHYDsum=["S_N9"], NHYDs to add=["N9"+"SW_9"+"NC_CK"]
647  *%-----|-----|
648  *#
649  *# Sum of hydrographs from Node 9 routed to Node 8
650  *# Section 3
651  *#
652  ROUTE CHANNEL   NHYDout=["N8"] ,NHYDin=["S_N9"] ,
653                  RDT=[30](min),
654                  CHLGTH=[2269](m),  CHSLOPE=[0.0882](%),
655                                      FPSLOPE=[0.0882](%),
656                  SECNUM=[1.0],      NSEG=[3]
657                  ( SEGROUGH, SEGDIST (m))=
658                    [0.1,-17.99
659                     -0.045,17.31
660                    0.1,456.58] NSEG times

```

```

661      ( DISTANCE (m), ELEVATION (m))=
662          [-201.19,100.50]
663          [-135.21, 100.00]
664          [-94.83, 99.50]
665          [-67.05, 99.00]
666          [-17.99, 98.50]
667          [-16.02, 98.00]
668          [-13.95, 97.50]
669          [13.95, 97.50]
670          [15.64, 98.00]
671          [17.31, 98.50]
672          [162.02, 98.50]
673          [172.89 ,99.00]
674          [314.38, 99.00]
675          [343.78, 99.50]
676          [365.67, 100.00]
677          [376.68, 100.00 ]
678          [393.11, 99.50]
679          [404.97, 99.50]
680          [431.70, 100.00]
681          [456.58, 100.50 ]
682  *%-----|-----|
683  *#
684  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
685  *#
686  ADD HYD          NHYDsum=["S_N8"], NHYDs to add=["N8"+"SW_8"+"HB_DR"]
687  *%-----|-----|
688  *#
689  *# Sum of hydrographs from Node 8 routed to Node 7
690  *# Section 4
691  *#
692  ROUTE CHANNEL    NHYDout=["N7"] ,NHYDin=["S_N8"],
693                  RDT=[30](min),
694                  CHLGTH=[3750](m),  CHSLOPE=[0.0533](%),
695                                          FPSLOPE=[0.0533](%),
696                  SECNUM=[1.0],        NSEG=[3]
697                  ( SEGROUGH, SEGDIST (m))=
698                      [0.12,-18.11
699                      -0.07,17.22
700                      0.12,590.05] NSEG times
701      ( DISTANCE (m), ELEVATION (m))=
702          [-433.21, 102.00]
703          [-425.34, 101.50]
704          [-377.56, 101.50]
705          [-366.23, 101.00]
706          [-202.60, 100.50]
707          [-96.25, 99.50]
708          [-68.36 99.00]
709          [-18.11, 98.50]
710          [-13.81, 97.50]
711          [13.81, 97.50]
712          [17.22, 98.50]
713          [161.95, 98.50]
714          [173.11, 99.00]
715          [314.05, 99.00]
716          [365.52, 100.00]
717          [404.70, 99.50]
718          [476.74, 100.50]
719          [502.31, 101.00]
720          [584.69, 101.00]
721          [585.79, 101.00]
722          [590.05, 102.00]
723  *%-----|-----|
724  *#
725  *# Addition of Subwatershed 7 to Node 7
726  *#

```

```

727 ADD HYD          NHYDsum=["S_N7"], NHYDs to add=["N7"+"SW_7"]
728 *%-----|-----|
729 SAVE HYD        NHYD=["S_N7"], # OF PCYCLES=[-1], ICASEsh=[-1]
730                HYD_FILENAME=["H_SN7"]
731                HYD_COMMENT=["flow at S_N7: N7 + SW_7"]
732 *%-----|-----|
733 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
734 *# Storage area and volumes were estimated from available topo maps.
735 *# Release rate from fen was assumed to be controlled by the downstream
736 *# river cross-section for summer conditions. It is was assumed that for up to
737 *# 0.75 m of water, the main channel of the river provided the storage. Above
738 *# this depth, the wetland starts to signigicantly store water.
739 *#
740 ROUTE RESERVOIR  NHYDout=["RES_RF"] ,NHYDin=["S_N7"] ,
741                RDT=[30](min),
742                TABLE of ( OUTFLOW-STORAGE ) values
743                (cms) - (ha-m)
744                TABLE of ( OUTFLOW-STORAGE ) values
745                (cms) - (ha-m)
746                [ 0.0 , 0.0 ]
747                [0.9051, 2.40]
748                [2.907, 4.13]
749                [9.744, 9.18]
750                [20.304, 14.96]
751                [34.167, 310.21]
752                [74.993, 605.46]
753                [104.876, 900.71]
754                [140.56, 2892.00]
755                [225.00, 3615.63]
756                [ -1 , -1 ] (max twenty pts)
757                NHYDovf=[" " ] ,
758 *%-----|-----|
759 SAVE HYD        NHYD=["RES_RF"], # OF PCYCLES=[-1], ICASEsh=[-1]
760                HYD_FILENAME=["H_ResRF"]
761                HYD_COMMENT=["outflow of Richmond Fen"]
762 *%-----|-----|
763 *#
764 *# Sum of hydrographs from Node 7 routed to Node 6
765 *# Section 5
766 *#
767 ROUTE CHANNEL    NHYDout=["N6"] ,NHYDin=["RES_RF"] ,
768                RDT=[30](min),
769                CHLGTH=[3056](m), CHSLOPE=[0.0818](%),
770                FPSLOPE=[0.0818](%),
771                SECNUM=[1.0], NSEG=[5]
772                ( SEGROUGH,SEGDIST (m))=
773                [0.025,-70.8
774                0.1,-23.9
775                -0.05,23.9
776                0.06,39.8
777                0.05,96.3] NSEG times
778                ( DISTANCE (m), ELEVATION (m))=
779                [-100.8, 97.00]
780                [-70.8, 96.50]
781                [-52.0, 96.00]
782                [-35.1, 95.50]
783                [-30.6, 95.00]
784                [-23.9, 94.54]
785                [23.9, 94.54]
786                [39.8, 95.00]
787                [50.4, 95.50]
788                [93.5, 96.00]
789                [94.9, 96.50]
790                [96.3, 97.00]
791 *%-----|-----|
792 *#

```

```

793  *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
794  *#
795  ADD HYD          NHYDsum=["S_N6"], NHYDs to add=["N6"+"SW_6"+"VG_DR"]
796  *%-----|-----
797  *#
798  *# Sum of hydrographs from Node 6 routed to Node 5
799  *# Section 6
800  *#
801  ROUTE CHANNEL    NHYDout=["N5"] ,NHYDin=["S_N6"] ,
802                  RDT=[30](min),
803                  CHLGTH=[1852](m),  CHSLOPE=[0.0540](%),
804                  FPSLOPE=[0.0540](%),
805                  SECNUM=[1.0],      NSEG=[3]
806                  ( SEGROUGH, SEGDIST (m))=
807                    [0.035,-131.59
808                    -0.045,48.96
809                    0.1,239.04] NSEG times
810                  ( DISTANCE (m), ELEVATION (m))=
811                    [-686.30, 94.50]
812                    [-675.70, 94.00]
813                    [-492.52, 93.00]
814                    [-467.28, 94.00]
815                    [-131.59, 94.00]
816                    [-92.79, 92.50]
817                    [-18.06, 91.00]
818                    [18.06, 91.00]
819                    [43.47, 92.50]
820                    [48.96, 94.00]
821                    [177.43, 94.00]
822                    [239.04,94.50]
823  *%-----|-----
824  *#
825  *# Addition of Subwatershed 5 and Flowing Creek to Node 5
826  *#
827  ADD HYD          NHYDsum=["S_N5"], NHYDs to add=["N5"+"SW_5"+"FL CK"]
828  *%-----|-----
829  *#
830  *# Sum of hydrographs from Node 5 routed to Node 5A
831  *# Section 7
832  *#
833  ROUTE CHANNEL    NHYDout=["N5A"] ,NHYDin=["S_N5"] ,
834                  RDT=[30](min),
835                  CHLGTH=[556](m),  CHSLOPE=[0.0900](%),
836                  FPSLOPE=[0.0900](%),
837                  SECNUM=[1.0],      NSEG=[4]
838                  ( SEGROUGH, SEGDIST (m))=
839                    [0.04,-41.5
840                    0.1,-14.0
841                    -0.045,14.0
842                    0.1,41.1] NSEG times
843                  ( DISTANCE (m), ELEVATION (m))=
844                    [-275.8, 93.00]
845                    [-248.6, 92.50]
846                    [-237.0, 92.00]
847                    [-219.3, 91.50]
848                    [-202.1, 91.50]
849                    [-186.0, 92.00]
850                    [-129.2, 92.00]
851                    [-117.6, 91.50]
852                    [-100.6, 91.00]
853                    [-41.5, 91.00]
854                    [-20.0, 91.00]
855                    [-14.0, 90.54]
856                    [14.0, 90.54]
857                    [15.3, 91.00]
858                    [17.3, 91.50]

```

```

859                                     [38.4, 92.00]
860                                     [39.8, 92.50]
861                                     [41.1, 93.00]
862 *%-----|-----
863 *#
864 *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
865 *#
866 ADD HYD                NHYDsum=["S_N5A"], NHYDs to add=["N5A"+"SW_5A2"+"SW_5A1"]
867 *%-----|-----
868 *#
869 *# Sum of hydrographs from Node 5A routed to Node 4
870 *# Section 8
871 *#
872 ROUTE CHANNEL        NHYDout=["N4"] ,NHYDin=["S_N5A"] ,
873 RDT=[30](min),
874 CHLGTH=[4630](m),  CHSLOPE=[0.0432](%),
875                                FPSLOPE=[0.0432](%),
876 SECNUM=[1.0],      NSEG=[3]
877 ( SEGROUGH, SEGDIST (m))=
878 [0.05,-28.2
879 -0.035,28.2
880 0.05,173.1] NSEG times
881 ( DISTANCE (m), ELEVATION (m))=
882 [-38.9, 92.00]
883 [-35.8, 91.50]
884 [-33.3, 91.00]
885 [-28.2, 90.50]
886 [-15.0, 87.48]
887 [-5.0, 88.34]
888 [5.0, 86.20]
889 [15.0, 88.55]
890 [28.2, 90.50]
891 [29.7, 91.00]
892 [46.5, 91.00]
893 [127.8, 91.00]
894 [148.7, 91.50]
895 [173.1, 92.00]
896 *%-----|-----
897 *#
898 *# Addition of Subwatershed 4 and Leamy Creek to Node 4
899 *#
900 ADD HYD                NHYDsum=["S_N4"], NHYDs to add=["N4"+"SW_4"+"LM_CK"]
901 SAVE HYD              NHYD=["S_N4"], # OF PCYCLES=[-1], ICASEsh=[1]
902                                HYD_COMMENT=["flow at S_N4"]
903 *%-----|-----
904 *#
905 *# Sum of hydrographs from Node 4 routed to Node 2
906 *# Section 9
907 *#
908 ROUTE CHANNEL        NHYDout=["N2"] ,NHYDin=["S_N4"] ,
909 RDT=[30](min),
910 CHLGTH=[1667](m),  CHSLOPE=[0.0600](%),
911                                FPSLOPE=[0.0600](%),
912 SECNUM=[1.0],      NSEG=[4]
913 ( SEGROUGH, SEGDIST (m))=
914 [0.1,-28.0
915 -0.04,28.4
916 0.06,31.7
917 0.04,80.2] NSEG times
918 ( DISTANCE (m), ELEVATION (m))=
919 [-36.3, 92.00]
920 [-32.6, 91.50]
921 [-30.2, 91.00]
922 [-28.0, 90.45]
923 [-15.0, 87.48]
924 [-5.0, 88.34]

```



```

925             [5.0, 86.20]
926             [15.0, 88.55]
927             [28.0, 90.45]
928             [28.4, 90.50]
929             [30.4, 91.00]
930             [31.7, 91.50]
931             [80.2, 92.00]
932 *%-----|-----
933 *#
934 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
935 *#
936 ADD HYD           NHYDsum=["S_N2"], NHYDs to add=["N2"+"SW_2"+"SM_DR"+"MO_DR"]
937 *%-----|-----
938 SAVE HYD         NHYD=["S_N2"], # OF PCYCLES=[-1], ICASEsh=[-1]
939                   HYD_FILENAME=["H_SN2"]
940                   HYD_COMMENT=["flow at S_N2 Jock River Gauge at Moodie Dr."]
941 *%-----|-----
942 *#
943 *# Sum of hydrographs from Node 2 routed to Node 1
944 *# Section 10
945 *#
946 ROUTE CHANNEL   NHYDout=["N1"] ,NHYDin=["S_N2"] ,
947                   RDT=[30](min),
948                   CHLGTH=[10046](m),  CHSLOPE=[0.0498](%),
949                                     FPSLOPE=[0.0498](%),
950                   SECNUM=[1.0],      NSEG=[5]
951                   ( SEGROUGH, SEGDIST (m))=
952                     [0.04,-27.6
953                     0.06,-15.0
954                     -0.045,15.0
955                     0.06,25.4
956                     0.04,122.6] NSEG times
957                   ( DISTANCE (m), ELEVATION (m))=
958                     [-87.0, 91.50]
959                     [-32.4, 91.00]
960                     [-27.6, 90.50]
961                     [-25.0, 90.00]
962                     [-22.9, 89.57]
963                     [-15.0, 86.20]
964                     [-5.0, 84.83]
965                     [5.0, 84.83]
966                     [15.0, 88.11]
967                     [22.9, 89.57]
968                     [25.4, 90.00]
969                     [27.9, 90.50]
970                     [38.0, 91.00]
971                     [112.5, 91.00]
972                     [114.3, 90.50]
973                     [115.1, 90.26]
974                     [116.3, 90.50]
975                     [119.0, 91.00]
976                     [121.0, 91.50]
977                     [122.6, 92.00]
978 *%-----|-----
979 *#
980 *# Addition of Subwatershed 1 to Node 1
981 *#
982 ADD HYD           NHYDsum=["N1"], NHYDs to add=["N1"+"SW_1"]
983 SAVE HYD         NHYD=["N1"], # OF PCYCLES=[-1], ICASEsh=[1]
984                   HYD_COMMENT=["total outflow of Jock River"]
985 *%-----|-----
986 *#####
987 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
988 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
989 *%               ["C24SC005.stm"] <--storm filename, one per line for NSTORM time
990 *%-----|-----

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```
991  *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
992  START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
993  *%              ["C24SC010.stm"] <--storm filename, one per line for NSTORM time
994  *%-----|-----|
995  *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
996  START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
997  *%              ["C24SC025.stm"] <--storm filename, one per line for NSTORM time
998  *%-----|-----|
999  *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
1000 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
1001 *%              ["C24SC050.stm"] <--storm filename, one per line for NSTORM time
1002 *%-----|-----|
1003 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
1004 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
1005 *%              ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
1006 FINISH
1007
```

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00001 *****
00002 *****
00003 SSSSS W M M M H H Y Y M M O O 222 000 11 5555 *****
00004 S W W M M T T M M M O O 2 0 0 11 5 *****
00005 SSSSS W W M M M H H H H Y Y M M M O O 2 0 0 11 5 Ver 5.800
00006 S W W M M H H Y Y M M O O 222 0 0 11 555 PRE 20.0
00007 SSSSS W W M M M Y M M O O 2 0 0 11 5 *****
00008 *****
00009 Stormwater Management Hydrologic Model
00010 *****
00011 *****
00012 ***** SWHYMO Ver 5.800 *****
00013 ***** A single event and continuous hydrologic simulation model *****
00014 ***** based on the principles of HMO and its successors *****
00015 ***** OTHYMO-83 and OTHYMO-89 *****
00016 *****
00017 ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018 ***** Ottawa, Ontario: (613) 886-8884 *****
00019 ***** Gatineau, Quebec: (819) 243-6888 *****
00020 ***** E-Mail: swhy@jfsa.com *****
00021 *****
00022 *****
00023 *****
00024 ***** Licensed user: JFSaInc. *****
00025 ***** SERIAL#:2549237 *****
00026 *****
00027 *****
00028 *****
00029 ***** ***** PROGRAM ARRAY DIMENSIONS *****
00030 ***** ***** Maximum value for ID numbers : 11 *****
00031 ***** ***** Max. number of rainfall points: 105408 *****
00032 ***** ***** Max. number of flow points : 105408 *****
00033 *****
00034 *****
00035 ***** ***** S U M M A R Y O U T P U T *****
00036 ***** *****
00037 ***** ***** RUN DATE: 2021-02-22 TIME: 15:43:08 RUN COUNTER: 019194 *****
00038 ***** ***** Input file: T:\PROJ\1474-16\Design\20210226-QuantityControlAnalysis\SWHYMO\SWHYMO-Model\summe *****
00039 ***** ***** *****
00040 ***** ***** DAT *****
00041 ***** *****
00042 ***** ***** Output file: T:\PROJ\1474-16\Design\20210226-QuantityControlAnalysis\SWHYMO\SWHYMO-Model\summe *****
00043 ***** ***** *****
00044 ***** ***** Summary file: T:\PROJ\1474-16\Design\20210226-QuantityControlAnalysis\SWHYMO\SWHYMO-Model\summe *****
00045 ***** ***** *****
00046 ***** ***** User comment: *****
00047 ***** *****
00048 ***** *****
00049 ***** *****
00050 ***** *****
00051 ***** *****
00052 ***** *****
00053 ***** *****
00054 ***** ***** SWHYMO Ver 5.800 SWHYMO INPUT DATA FILE *****
00055 ***** *****
00056 ***** ***** Project Name: [Jock River] Project Number: (411-02) *****
00057 ***** ***** Date : 06-06-2003 *****
00058 ***** ***** Modeller : [JFP] *****
00059 ***** ***** Company : JFSaInc. *****
00060 ***** ***** License # : 2549237 *****
00061 ***** *****
00062 ***** ***** CALIBRATION OF SUMMER MODEL PARAMETERS *****
00063 ***** *****
00064 ***** ***** USING CONTINUOUS SIMULATIONS *****
00065 ***** ***** Rainfall data from OFPA rain gauge installed at site + other gauges by the City *****
00066 ***** ***** Use data collected from May list to July 14, 2003 *****
00067 ***** *****
00068 ***** ***** ** END OF RUN : 1 *****
00069 ***** *****
00070 ***** *****
00071 ***** *****
00072 ***** *****
00073 ***** *****
00074 ***** *****
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00187 ***** *****

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00188 ***** [CM: 63.0; N= 3.00; Tp: .90] *****
00189 ***** [IAREC: 4.00; SMIN: 53.62; SMAX:396.11; SK: .010] *****
00190 ***** [InterEventTime= 12.00] *****
00191 ***** *****
00192 ***** # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00193 ***** # of 1.65 *****
00194 ***** [CM: 63.0; N= 3.00; Tp: 1.5] *****
00195 ***** [IAREC: 4.00; SMIN: 53.62; SMAX:396.11; SK: .010] *****
00196 ***** [InterEventTime= 12.00] *****
00197 ***** [IAREC: 4.00; SMIN: 52.62; SMAX:350.79; SK: .010] *****
00198 ***** [InterEventTime= 12.00] *****
00199 ***** *****
00200 ***** # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00201 ***** # of 1.52 *****
00202 ***** [CM: 67.0; N= 3.00; Tp: 1.8] *****
00203 ***** [IAREC: 4.00; SMIN: 50.35; SMAX:336.97; SK: .010] *****
00204 ***** [InterEventTime= 12.00] *****
00205 ***** [IAREC: 4.00; SMIN: 76.32; SMAX:508.81; SK: .010] *****
00206 ***** [InterEventTime= 12.00] *****
00207 ***** *****
00208 ***** # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00209 ***** # of 1.75 *****
00210 ***** [CM: 67.0; N= 3.00; Tp: 2.1] *****
00211 ***** [IAREC: 4.00; SMIN: 50.35; SMAX:336.97; SK: .010] *****
00212 ***** [InterEventTime= 12.00] *****
00213 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00214 ***** [InterEventTime= 12.00] *****
00215 ***** *****
00216 ***** # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00217 ***** # of 1.6 *****
00218 ***** [CM: 67.0; N= 3.00; Tp: 1.8] *****
00219 ***** [IAREC: 4.00; SMIN: 50.35; SMAX:336.97; SK: .010] *****
00220 ***** [InterEventTime= 12.00] *****
00221 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00222 ***** [InterEventTime= 12.00] *****
00223 ***** [CM: 72.0; N= 3.00; Tp: 2.1] *****
00224 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00225 ***** [InterEventTime= 12.00] *****
00226 ***** [IAREC: 4.00; SMIN: 31.15; SMAX:207.66; SK: .010] *****
00227 ***** [InterEventTime= 12.00] *****
00228 ***** *****
00229 ***** # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00230 ***** # of 1.20 *****
00231 ***** [CM: 72.0; N= 3.00; Tp: 1.5] *****
00232 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00233 ***** [InterEventTime= 12.00] *****
00234 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00235 ***** [InterEventTime= 12.00] *****
00236 ***** [CM: 81.0; N= 3.00; Tp: .62] *****
00237 ***** [IAREC: 4.00; SMIN: 32.46; SMAX:168.09; SK: .010] *****
00238 ***** [InterEventTime= 12.00] *****
00239 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00240 ***** [InterEventTime= 12.00] *****
00241 ***** *****
00242 ***** # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00243 ***** # of 1.61 *****
00244 ***** [CM: 74.0; N= 3.00; Tp: 4.45] *****
00245 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00246 ***** [InterEventTime= 12.00] *****
00247 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00248 ***** [InterEventTime= 12.00] *****
00249 ***** [CM: 81.0; N= 3.00; Tp: .62] *****
00250 ***** [IAREC: 4.00; SMIN: 32.46; SMAX:168.09; SK: .010] *****
00251 ***** [InterEventTime= 12.00] *****
00252 ***** [IAREC: 4.00; SMIN: 25.21; SMAX:168.09; SK: .010] *****
00253 ***** [InterEventTime= 12.00] *****
00254 ***** [CM: 80.0; N= 3.00; Tp: 2.46] *****
00255 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00256 ***** [InterEventTime= 12.00] *****
00257 ***** [CM: 80.0; N= 3.00; Tp: 2.46] *****
00258 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00259 ***** [InterEventTime= 12.00] *****
00260 ***** [CM: 81.0; N= 3.00; Tp: .62] *****
00261 ***** [IAREC: 4.00; SMIN: 32.46; SMAX:168.09; SK: .010] *****
00262 ***** [InterEventTime= 12.00] *****
00263 ***** [IAREC: 4.00; SMIN: 31.15; SMAX:207.66; SK: .010] *****
00264 ***** [InterEventTime= 12.00] *****
00265 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00266 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00267 ***** [InterEventTime= 12.00] *****
00268 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00269 ***** [CM: 81.0; N= 3.00; Tp: 3.25] *****
00270 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00271 ***** [InterEventTime= 12.00] *****
00272 ***** [CM: 76.0; N= 3.00; Tp: 3.03] *****
00273 ***** [IAREC: 4.00; SMIN: 32.46; SMAX:168.09; SK: .010] *****
00274 ***** [InterEventTime= 12.00] *****
00275 ***** [CM: 78.0; N= 3.00; Tp: 3.56] *****
00276 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00277 ***** [InterEventTime= 12.00] *****
00278 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00279 ***** [InterEventTime= 12.00] *****
00280 ***** Routing hydrographs *****
00281 ***** *****
00282 ***** # Starting with the addition of Jock River Headwater and Subwatershed 13 *****
00283 ***** *****
00284 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00285 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00286 ***** [InterEventTime= 12.00] *****
00287 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00288 ***** [InterEventTime= 12.00] *****
00289 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00290 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00291 ***** [InterEventTime= 12.00] *****
00292 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00293 ***** [InterEventTime= 12.00] *****
00294 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00295 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00296 ***** [InterEventTime= 12.00] *****
00297 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00298 ***** [InterEventTime= 12.00] *****
00299 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00300 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00301 ***** [InterEventTime= 12.00] *****
00302 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00303 ***** [InterEventTime= 12.00] *****
00304 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00305 ***** [InterEventTime= 12.00] *****
00306 ***** # Insertion of a reservoir to simulate the effects of the Goodwood Marsh *****
00307 ***** *****
00308 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00309 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00310 ***** [InterEventTime= 12.00] *****
00311 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00312 ***** [InterEventTime= 12.00] *****
00313 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00314 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00315 ***** [InterEventTime= 12.00] *****
00316 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00317 ***** [InterEventTime= 12.00] *****
00318 ***** [CM: 81.0; N= 3.00; Tp: 1.5] *****
00319 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00320 ***** [InterEventTime= 12.00] *****
00321 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00322 ***** [InterEventTime= 12.00] *****
00323 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00324 ***** [InterEventTime= 12.00] *****
00325 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00326 ***** [InterEventTime= 12.00] *****
00327 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00328 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00329 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00330 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00331 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00332 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00333 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00334 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00335 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00336 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00337 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00338 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00339 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00340 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00341 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00342 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00343 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00344 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00345 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00346 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
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00352 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00353 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00354 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00355 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00356 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00357 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00358 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00359 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00360 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00361 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00362 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00363 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00364 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00365 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00366 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00367 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00368 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00369 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00370 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00371 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00372 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00373 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****
00374 ***** [IAREC: 4.00; SMIN: 39.75; SMAX:264.99; SK: .010] *****

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00749# CONTINUOUS NASHVDDRAIN-ID:HYD.....AREAha-OPEAKcm-TpeakDate_hh:mm-.....Rvm-R.C-.....DWfms
00750# [Cm 76.0i Na 3.00i Tm 1.75]
00751# [IAREK 4.00i SMIN: 33.81i SMAx:225.43i EK: -010]
00752# [InterVTime= 12.00]
00753# ROUTE CHANNEL -> 30.0 01:R_M3 1412.00 4.515_NaDate 37:30 21.96 384 .000
00754# ADD HYD + 30.0 01:R_H1 585.00 6.551_NaDate 29:30 25.59 448 .000
00755# SUM= 30.0 01:R_M3 1412.00 4.515_NaDate 37:30 21.96 384 .000

00936#
00937# RO005:CO0050-.....DRAIN-ID:HYD.....AREAha-OPEAKcm-TpeakDate_hh:mm-.....Rvm-R.C-.....DWfms
00938# ADD HYD + 30.0 02:R_M7 3544.00 46.889_NaDate 45:00 17.17 n/a .000
00939# [I/S/n= 3795./ .051/.070]
00940# [Vmax= 484iDmax= 1.839]
00941# ROUTE CHANNEL -> 30.0 02:R_M7 3544.00 46.889_NaDate 45:00 17.17 n/a .000
00942# [I/S/n= 3795./ .051/.070]
00943# [Vmax= 484iDmax= 1.839]


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01871# + 30.0 0218M_CK 1917.00 10.139 NoDate 34:00 26.99 n/a .000
01872# SUM# 11.0121_M10 11923.00 27.440 NoDate 33:00 25.40 n/a .000
01873# #
01874# # Sum of hydrographs from Node 11 routed to Node 10
01875# # Section 1
01876# #
01877# ROUTE CHANNEL -> 30.0 0218M_11 11923.00 27.440 NoDate 33:00 25.40 n/a .000
01878# [R/D=30.00] out- 30.0 0118M10 11923.00 27.756 NoDate 40:00 25.40 n/a .000
01879# [L/S/=14028./, .157/,040]
01880# [Vmax=.463/Dmax=1.320]
01881# #
01882# #
01883# # Addition of Subwatershed 10 to Node 10
01884# #
01885# ROUTE CHANNEL -> 30.0 0218M10 17859.00 44.045 NoDate 38:30 27.35 n/a .000
01886# ADD HYD + 30.0 0218M_10 5666.00 26.665 NoDate 37:30 31.47 n/a .000
01887# SUM# 30.0 0118M10 17859.00 44.045 NoDate 38:30 27.35 n/a .000
01888# ROUTE CHANNEL -> 30.0 0218M10 17859.00 44.045 NoDate 38:30 27.35 n/a .000
01889# [R/D=30.00] out- 30.0 0118M10 17859.00 44.045 NoDate 38:30 27.35 n/a .000
01890# [L/S/=14028./, .050/,040]
01891# [Vmax=.998/Dmax=3.955]
01892# #
01893# # Addition of Subwatershed 11 to Node 1
01894# #
02074# ROUTE CHANNEL -> 30.0 0218M_32 52483.00 104.643 NoDate 33:00 28.62 n/a .000
02075# ADD HYD + 30.0 0218M_1 52483.00 92.450 NoDate 36:00 28.62 n/a .000
02076# SUM# 30.0 0118M_1 52483.00 92.450 NoDate 36:00 28.62 n/a .000
02077# ROUTE CHANNEL -> 30.0 0218M_32 52483.00 104.643 NoDate 33:00 28.62 n/a .000
02078# [R/D=30.00] out- 30.0 0118M1 52483.00 92.450 NoDate 36:00 28.62 n/a .000
02079# [L/S/=14028./, .050/,040]
02080# [Vmax=.998/Dmax=3.955]
02081# #
02082# # Addition of Subwatershed 1 to Node 1
02083# #
02074# ROUTE CHANNEL -> 30.0 0218M_32 52483.00 104.643 NoDate 33:00 28.62 n/a .000
02075# ADD HYD + 30.0 0218M_1 52483.00 92.450 NoDate 36:00 28.62 n/a .000
02076# SUM# 30.0 0118M_1 52483.00 92.450 NoDate 36:00 28.62 n/a .000
02077# ROUTE CHANNEL -> 30.0 0218M_32 52483.00 104.643 NoDate 33:00 28.62 n/a .000
02078# [R/D=30.00] out- 30.0 0118M1 52483.00 92.450 NoDate 36:00 28.62 n/a .000
02079# [L/S/=14028./, .050/,040]
02080# [Vmax=.998/Dmax=3.955]
02081# #
02082# # Addition of Kings Creek to S_M10
02083# #
02084# #
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02245 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02246 # of 1.28
02247 R0550=C00021-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
CONTINUOUS HASYD 30.01:1M_1K 4945.00 43.824 No.Date 33:00 38.35 /470 .000
02248 [Cm 74.0I No 3.00I Tpk 4.45]
02249 [IAREC 4.00I SMIN: 36.67I SMAK:244.49I EK: -010]
02250 #
02251 CONTINUOUS HASYD 30.01:1M_2K 20.00 .873 No.Date 28:30 45.57 /559 .000
02252 [Cm 81.0I No 3.00I Tpk .62]
02253 [IAREC 4.00I SMIN: 31.81I SMAK:168.09I EK: -010]
02254 #
02255 CONTINUOUS HASYD 30.01:1M_4 585.00 12.656 No.Date 29:30 45.57 /559 .000
02256 [Cm 81.0I No 3.00I Tpk 1.75]
02257 #
02258 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02259 # of 1.61
02260 CONTINUOUS HASYD 30.01:1M_5A1 1412.00 8.537 No.Date 37:30 39.50 /490 .000
02261 [Cm 75.0I No 3.00I Tpk 8.00]
02262 [IAREC 4.00I SMIN: 31.81I SMAK:225.43I EK: -010]
02263 #
02264 CONTINUOUS HASYD 30.01:1M_4 585.00 12.656 No.Date 29:30 45.57 /559 .000
02265 R0550=C00024-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02266 [IAREC 4.00I SMIN: 25.21I SMAK:168.09I EK: -010]
02267 #
02268 CONTINUOUS HASYD 30.01:1M_1K 1021.00 16.828 No.Date 30:30 44.74 /549 .000
02269 [Cm 80.0I No 3.00I Tpk 2.46]
02270 #
02271 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02272 [IAREC 4.00I SMIN: 26.32I SMAK:175.50I EK: -010]
02273 #
02274 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02275 [Cm 77.0I No 3.00I Tpk 7.81]
02276 #
02277 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02278 [IAREC 4.00I SMIN: 31.15I SMAK:207.66I EK: -010]
02279 #
02280 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02281 [Cm 81.0I No 3.00I Tpk 1.25]
02282 [IAREC 4.00I SMIN: 25.21I SMAK:168.09I EK: -010]
02283 #
02284 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02285 [Cm 76.0I No 3.00I Tpk 2.81]
02286 #
02287 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02288 [IAREC 4.00I SMIN: 32.46I SMAK:216.39I EK: -010]
02289 #
02290 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02291 [Cm 76.0I No 3.00I Tpk 3.56]
02292 #
02293 CONTINUOUS HASYD 30.01:1M_2 177.00 6.279 No.Date 28:30 41.48 /509 .000
02294 [IAREC 4.00I SMIN: 29.88I SMAK:199.22I EK: -010]
02295 #
02296 # Routing hydrographs
02297 #
02298 # Starting with the addition of Jock River Headwater and Subwatershed 13
02299 #
02300 R0550=C00030-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ADD HYD 3680.00 17.963 No.Date 36:30 30.30 /n/a .000
02301 #
02302 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02303 #
02304 # Sum of hydrographs from Node 13 routed to Node 13A
02305 #
02306 # (Approximated cross-section - see cross-section 28)
02307 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
02308 #
02309 R0550=C00031-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ROUTE CHANNEL 30.01:1M_13 4651.00 18.758 No.Date 38:30 29.87 /n/a .000
02310 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02311 [RFD=30.00I out<- 30.01:1M_13 4651.00 18.758 No.Date 38:30 29.87 /n/a .000]
02312 [L/S=N: 9074. / .027 / .040]
02313 [Vmax: 571I Dmax: 3.1]
02314 #
02315 # Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
02316 #
02317 R0550=C00032-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ADD HYD 4651.00 18.758 No.Date 38:30 29.87 /n/a .000
02318 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02319 #
02320 # Sum of hydrographs from Node 13A routed to Node 13
02321 #
02322 # Insertion of a reservoir to simulate the effects of the Goodwood Marsh
02323 #
02324 R0550=C00033-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ROUTE RESERVOIR 30.01:1M_13A 7725.00 37.992 No.Date 62:30 27.65 /n/a .000
02325 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02326 [RFD=30.00I out<- 30.01:1M_13A 7725.00 37.992 No.Date 62:30 27.65 /n/a .000]
02327 [L/S=N: 14558. / .14558 / .040]
02328 #
02329 R0550=C00034-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
SAVE HYD 7725.00 37.992 No.Date 62:30 27.65 /n/a .000
02330 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02331 #
02332 # Sum of hydrographs from Node 13 routed to Node 13A
02333 #
02334 # (Approximated cross-section)
02335 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
02336 #
02337 R0550=C00035-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ROUTE CHANNEL 30.01:1M_13 4651.00 18.758 No.Date 38:30 29.87 /n/a .000
02338 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02339 [RFD=30.00I out<- 30.01:1M_13 4651.00 18.758 No.Date 38:30 29.87 /n/a .000]
02340 [L/S=N: 9074. / .027 / .040]
02341 [Vmax: 556I Dmax: 1.93]
02342 #
02343 # Addition of Subwatershed Jock River at Ashton to Node 12
02344 #
02345 R0550=C00036-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ADD HYD 1781.00 16.847 No.Date 32:30 36.82 /n/a .000
02346 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02347 #
02348 # Sum of hydrographs from Node 13A routed to Node 13
02349 #
02350 #
02351 #
02352 # Sum of hydrographs from Node 12 routed to Node 11
02353 #
02354 # (Approximated cross-section - see cross-section 28)
02355 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
02356 #
02357 R0550=C00037-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ROUTE CHANNEL 30.01:1M_12 9506.00 18.583 No.Date 32:30 29.37 /n/a .000
02358 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02359 [RFD=30.00I out<- 30.01:1M_12 9506.00 18.583 No.Date 32:30 29.37 /n/a .000]
02360 [L/S=N: 972. / .051 / .040]
02361 [Vmax: 734I Dmax: 3.040]
02362 #
02363 # Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
02364 #
02365 #
02366 #
02367 #
02368 #
02369 #
02370 # Addition of Subwatershed 11 and No Name Creek to Node 11
02371 #
02372 R0550=C00040-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ADD HYD 9506.00 18.583 No.Date 32:30 29.37 /n/a .000
02373 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02374 #
02375 # Sum of hydrographs from Node 11 routed to Node 10
02376 #
02377 #
02378 #
02379 #
02380 #
02381 #
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02383 #
02384 #
02385 #
02386 #
02387 # Addition of Subwatershed 10 to Node 10
02388 #
02389 R0550=C00042-----DtmIn-ID:HYD-----AREHA-A-QPEARCS-TpeakDate_hh:mm-----Rvm-R.C-----DWFCMS
ADD HYD 11923.00 20.165 No.Date 40:00 29.84 /n/a .000
02390 [IAREC 4.00I SMIN: 30.00I SMAK:13.97I EK: -010]
02391 #
02392 # Sum of hydrographs from Node 10 routed to Node 9
02393 #
02394 #
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02404 # Sum of hydrographs from Node 10 routed to Node 9
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02421 # Sum of hydrographs from Node 9 routed to Node 8
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02993 #
02994 # Sum of hydrographs from Node 6 routed to Node 5
02995 # Section 6
02996 #
02997 R0100:C00056-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
02998 ROUTE CHANNEL > 30.0 02:R_6 45409.01 60.497 No_date 59:30 36.31 n/a .000
02999 [RFS=30.00] out< 30.0 01:R_6 40240.01 60.383 No_date 60:30 36.31 n/a .000
03000 [L/S/= 1852./ .054/.035]
03001 [Vmax= .490/Dmax= 1.451]
03002 #
03003 # Addition of Subwatershed 5 and Flowing Creek to Node 5
03004 #
03005 R0100:C00057-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03006 ADD HYD + 30.0 02:R_5 45409.01 60.383 No_date 60:30 36.31 n/a .000
03007 + 30.0 02:SM_5 224.00 9.294 No_date 28:30 47.59 n/a .000
03008 + 30.0 02:FL_CK 4848.00 51.121 No_date 33:00 44.15 n/a .000
03009 SUM= 30.0 01:R_5 45409.01 79.891 No_date 34:00 37.22 n/a .000
03010 #
03011 # Sum of hydrographs from Node 5 routed to Node 5A
03012 # Section 7
03013 #
03014 R0100:C00058-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03015 ROUTE CHANNEL > 30.0 02:R_5A 45409.01 79.891 No_date 34:00 37.22 n/a .000
03016 [RFS=30.00] out< 30.0 01:R_5A 45409.01 79.891 No_date 34:00 37.22 n/a .000
03017 [L/S/= 556./ .090/.040]
03018 [Vmax= .544/Dmax= 1.346]
03019 #
03020 # Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
03021 #
03022 R0100:C00059-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03023 ADD HYD + 30.0 02:R_5A1 45409.01 79.891 No_date 34:00 37.22 n/a .000
03024 + 30.0 02:SM_5A2 20.00 1.014 No_date 28:30 52.03 n/a .000
03025 + 30.0 02:SM_5A1 1432.00 9.486 No_date 37:30 45.85 n/a .000
03026 SUM= 30.0 01:R_5A 46841.01 88.619 No_date 34:30 37.48 n/a .000
03027 #
03028 # Sum of hydrographs from Node 5A routed to Node 4
03029 # Section 8
03030 #
03031 R0100:C00060-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03032 ROUTE CHANNEL > 30.0 02:R_4 46841.01 88.619 No_date 34:30 37.48 n/a .000
03033 [RFS=30.00] out< 30.0 01:R_4 46841.01 84.955 No_date 36:00 37.48 n/a .000
03034 [L/S/= 4631./ .041/.051]
03035 [Vmax= .501/Dmax= 3.849]
03036 #
03037 # Addition of Subwatershed 4 and Leamy Creek to Node 4
03038 #
03039 R0100:C00061-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03040 ADD HYD + 30.0 02:R_4 46841.01 84.955 No_date 36:00 37.48 n/a .000
03041 + 30.0 02:SM_4 585.00 14.644 No_date 29:30 52.03 n/a .000
03042 + 30.0 02:SM_CK 1021.00 19.815 No_date 30:30 51.13 n/a .000
03043 SUM= 30.0 01:R_4 48447.00 95.694 No_date 34:30 37.95 n/a .000
03044 R0100:C00062-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03045 SAVE HYD 30.0 01:R_4 48447.00 95.694 No_date 34:30 37.95 n/a .000
03046 fname 'S_H4.010
03047 remark:flow at S_H4
03048 #
03049 # Sum of hydrographs from Node 4 routed to Node 2
03050 # Section 9
03051 #
03052 R0100:C00063-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03053 ROUTE CHANNEL > 30.0 02:R_2 48447.00 95.694 No_date 34:30 37.95 n/a .000
03054 [RFS=30.00] out< 30.0 01:R_2 48447.00 95.694 No_date 35:00 37.95 n/a .000
03055 [L/S/= 1667./ .060/.040]
03056 [Vmax= .942/Dmax= 3.015]
03057 #
03058 # Addition of Subwatershed 2 with Mosohan Drain and Smith Drain to Node 2
03059 #
03060 R0100:C00064-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03061 ADD HYD + 30.0 02:R_2 48447.00 95.694 No_date 35:00 37.95 n/a .000
03062 + 30.0 02:SM_2 177.00 7.344 No_date 28:30 47.59 n/a .000
03063 + 30.0 02:SM_DR 1122.00 17.710 No_date 31:30 52.03 n/a .000
03064 + 30.0 02:SM_DR 2737.00 40.026 No_date 31:00 46.72 n/a .000
03065 SUM= 30.0 01:R_2 52483.00 141.415 No_date 32:30 38.74 n/a .000
03066 R0100:C00065-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03067 SAVE HYD 30.0 01:R_2 52483.00 141.415 No_date 32:30 38.74 n/a .000
03068 fname 'S_H2.010
03069 remark:flow at S_H2 Jock River Gauge at Moodie Dr.
03070 #
03071 # Sum of hydrographs from Node 2 routed to Node 1
03072 # Section 10
03073 #
03074 R0100:C00066-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03075 ROUTE CHANNEL > 30.0 02:R_1 52483.00 141.415 No_date 32:30 38.74 n/a .000
03076 [RFS=30.00] out< 30.0 01:R_1 52483.00 124.304 No_date 35:00 38.74 n/a .000
03077 [L/S/=1046./ .050/.040]
03078 [Vmax= 1.091/Dmax= 4.553]
03079 #
03080 # Addition of Subwatershed 1 to Node 1
03081 #
03082 R0100:C00067-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03083 ADD HYD + 30.0 02:R_1 52483.00 124.304 No_date 35:00 38.74 n/a .000
03084 + 30.0 02:SM_1 3176.00 43.079 No_date 32:00 48.46 n/a .000
03085 SUM= 30.0 01:R_1 55659.00 158.420 No_date 34:00 39.29 n/a .000
03086 R0100:C00068-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03087 SAVE HYD 30.0 01:R_1 55659.00 158.420 No_date 34:00 39.29 n/a .000
03088 fname 'N1.010
03089 remark:outflow of Jock River
03090 #####
03091 R0100:C00002-----Dtain-ID:HYD-----AREAb-QPEAKGms-TpeakDate_hh:mm-----RVm-R.C-----DWFCms
03092 FINISH
03093 #
03094 #
03095 # WARNINGS / ERRORS / NOTES
03096 #
03097 R0202:C00015 CONTINUOUS HABYD
03098 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03099 R0202:C00020 CONTINUOUS HABYD
03100 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03101 R0202:C00022 CONTINUOUS HABYD
03102 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03103 R0202:C00024 CONTINUOUS HABYD
03104 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03105 R0205:C00015 CONTINUOUS HABYD
03106 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03107 R0205:C00020 CONTINUOUS HABYD
03108 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03109 R0205:C00022 CONTINUOUS HABYD
03110 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03111 R0205:C00026 CONTINUOUS HABYD
03112 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03113 R0101:C00015 CONTINUOUS HABYD
03114 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03115 R0101:C00020 CONTINUOUS HABYD
03116 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03117 R0101:C00022 CONTINUOUS HABYD
03118 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03119 R0101:C00026 CONTINUOUS HABYD
03120 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03121 R0225:C00015 CONTINUOUS HABYD
03122 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03123 R0225:C00020 CONTINUOUS HABYD
03124 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03125 R0225:C00022 CONTINUOUS HABYD
03126 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03127 R0225:C00026 CONTINUOUS HABYD
03128 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03129 R0250:C00015 CONTINUOUS HABYD
03130 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03131 R0250:C00020 CONTINUOUS HABYD
03132 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03133 R0250:C00022 CONTINUOUS HABYD
03134 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03135 R0250:C00026 CONTINUOUS HABYD
03136 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03137 R0101:C00015 CONTINUOUS HABYD
03138 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03139 R0101:C00020 CONTINUOUS HABYD
03140 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03141 R0101:C00022 CONTINUOUS HABYD
03142 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03143 R0101:C00026 CONTINUOUS HABYD
03144 *** WARNING: Time step is too large for value of TP. RV may be ok. Peak flow could be off.
03145 * Simulation ended on 2021-02-22 at 15:43:22
03146 #
03147 #
03148 #

```

Attachment B

Model 2 – Jock River Reach One Model

Stantec, 2007

SWMHYMO Input & Summary files

```

1 2 Metric units
2 *#*****
3 *# Project Name: [Jock River Reach 1 SubWatershed Study]Project #: [160400414]
4 *# Date : October 2006
5 *# Modeller : [Navin Gautam/ Original by Ana M Paerez]
6 *# Company : Stantec.
7 *# License # : 3824306
8 *#*****
9 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[002]
10 *% ["C24SC002.stm"] <--storm filename, one per line for NSTORM time
11 *#-----|-----|
12 READ STORM STORM_FILENAME=["storm.001"]
13 *#-----|-----|
14 MODIFY STORM ICASEms=[1], NSHIFT=[96],
15 RedFACT=[1],
16 *#-----|-----|
17 DEFAULT VALUES ICASEdv=[1], read and print values
18 DEFVAL_FILENAME=["MODIFIED.VAL"]
19 COMPUTE API APII=[50], APIK=[.85]/day
20 *#*****
21 *#
22 *# JOCK RIVER REACH 1 SUBWATERSHED STUDY DISCRETIZED MODEL
23 *# PROPOSED CONDITIONS DESIGN STORM MODEL (SUMMER)
24 *#
25 *# Version: Draft Final Report, October 2006
26 *# Revision History
27 *# -Draft Interim Condition Report, Nov. 2005
28 *#*****
29 *# Assumptions
30 *# - All catchments are assumed to be developed except S-1, S-2, and SW-1a
31 *# - SWM facilities are modeled
32 *# - Rating curves were estimated based on existing reports and modeling for the
33 *# proposed SWM facilities
34 *# - The rating curve for the existing Kennedy Burnett SWM Facility was obtained from
35 *# the Urban Runoff Treatment in the Kennedy Burnett Settling Pond (URTKBP)- Regional
36 *# Municipality of Ottawa Carleton, March 1983
37 *# - River routing modeled
38 *# - River cross sections obtained from RVCA's HEC-RAS hydraulic model
39 *#-----|-----|
40 *# Parameters
41 *# - Design Storms: 2,5,10,25,50 & 1 00yr events: 24hr SCS (DT=10min)-model comparison
42 *# - Impervious area weighted based on: rural subdivision @20%, urban @55%
43 *# - NRCS(SCS) CN based on landuse (airphoto) and soil type (base mapping)
44 *# - Time to peak using Uplands Method
45 *#-----|-----|
46 *#*****
47 *#Read hydrograph upstream of N2 from RVCA Jock R. floodrisk watershed modeling
48 *#*****
49 READ HYD ID=[ 1 ], NHYD=["S_N2"],
50 HYD_FILENAME=["H-S_N2"]
51 *#-----|-----|
52 *#
53 *# Hydrograph from Node 2 routed to Node 416
54 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
55 *#
56 ROUTE CHANNEL IDout=[4], NHYD=["N_416"] ,IDin=[1] ,
57 RDT=[10](min),
58 CHLGTH=[2327](m), CHSLOPE=[0.0498](%),
59 FPSLOPE=[0.0498](%),
60 SECNUM=[1.0], NSEG=[3]
61 ( SEGROUGH, SEGDIST (m))=
62 [0.075,-23.96
63 -0.055,23.96
64 0.075,157.38] NSEG times
65 ( DISTANCE (m), ELEVATION (m))=
66 [-336.97,93.5]

```

```

67      [-318.85,93]
68      [-259,92.5]
69      [-133.18,92]
70      [-33.17,92]
71      [-27.21,92]
72      [-26.14,91.5]
73      [-24.99,91]
74      [-23.96,90.5]
75      [-14.33,88.26]
76      [-0.68,88.12]
77      [14.33,88.26]
78      [23.96,90.5]
79      [32.12,91]
80      [43.74,91.5]
81      [57.09,92]
82      [73.53,92.5]
83      [108.27,93]
84      [125.88,93.5]
85      [144.81,94]
86      [157.38,94.5]
87  *%-----|-----|
88  *#*****|*****|
89  *#   Catchment SW-1a
90  *#   - Portion of RVCA catchment SW_1 outside of Reach 1 subwatershed
91  *#   - Undeveloped agricultural land
92  *#*****|*****|
93  CONTINUOUS NASHYD   ID=[2], NHYD=["SW_1a"], DT=[5]min, AREA=[546](ha),
94                    DWF=[0](cms), CN/C=[72],   IA=[4.67](mm),
95                    N=[3], TP=[2.79]hrs,
96                    Continuous simulation parameters:
97                    IaRECper=[4](hrs),
98                    SMIN=[-1](mm),   SMAX=[-1](mm), SK=[0.010]/(mm),
99                    InterEventTime=[12](hrs)
100                   Baseflow simulation parameters:
101                   BaseFlowOption=[1] ,
102                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
103                   VHydCond=[0.055](mm/hr),   END=-1
104  *%-----|-----|
105  ADD HYD             IDsum=[ 3 ], NHYD=["SN_416"], IDs to add=[4,2]
106  *%-----|-----|
107  SAVE HYD           ID=[ 3 ],   # OF PCYCLES=[-1], ICASEsh=[1]
108                   HYD_COMMENT=["Total Flows at Highway 416"]
109  *%-----|-----|
110  *#
111  *# Hydrograph from Node 416 routed to Node at Okeefe drain
112  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245
113  *#
114  ROUTE CHANNEL      IDout=[1], NHYD=["N_OK"] ,IDin=[3] ,
115                   RDT=[5](min),
116                   CHLGTH=[497](m),   CHSLOPE=[0.3018](%),
117                                       FPSLOPE=[0.3018](%),
118                   SECNUM=[1.0],      NSEG=[3]
119                   ( SEGROUGH, SEGDIST (m))=
120                   [0.075,-19.40
121                   -0.055,19.40
122                   0.075,377.02] NSEG times
123                   ( DISTANCE (m), ELEVATION (m))=
124                   [-1062.81, 93.00]
125                   [-1061.41, 92.50]
126                   [-945.91, 92.00]
127                   [-783.64, 91.50]
128                   [-136.74, 91.00]
129                   [-134.06, 91.00]
130                   [-128.97, 91.00]
131                   [-86.04, 91.00]
132                   [-20.86, 91.00]

```

```

133         [-20.18, 90.50]
134         [-19.40, 90.00]
135         [-11.68, 86.89]
136         [0.00, 86.10]
137         [12.09, 86.81]
138         [19.40, 90.00]
139         [34.68, 90.50]
140         [60.56, 91.00]
141         [170.14, 91.00]
142         [175.05, 90.50]
143         [180.29, 90.00]
144         [193.41, 90.00]
145         [195.98, 90.50]
146         [377.02, 92.50]
147 *%-----|-----|
148 *#*****|
149 *#      Catchment OKEEFE
150 *#      - To O'Keefe drain (north of the Jock)
151 *#      - Developed with assumed 43% imp.
152 *#*****|
153 CONTINUOUS STANDHYD ID=[2], NHYD=["OKEEFE"], DT=[5](min), AREA=[448](ha),
154 XIMP=[0.43], TIMP=[0.43], DWF=[0](cms), LOSS=[2],
155 SCS curve number CN=[77],
156 Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
157 LGP=[40](m), MNP=[0.25], SCP=[0](min),
158 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
159 LGI=[1728](m), MNI=[0.013], SCI=[0](min),
160 Continuous simulation parameters:
161 IaRECper=[4](hrs), IaRECimp=[4](hrs),
162 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
163 InterEventTime=[18](hrs), END=-1
164
165 *#*****|
166 *#      Okeefe Pond
167 *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
168 *#      and a ratio of the catchment area to the West Clarke pond rating curve
169 *#      from the MSS for the next coordinates
170 *#*****|
171 ROUTE RESERVOIR IDout=[4], NHYD=["P_OKE"], IDin=[2],
172 RDT=[5](min),
173 TABLE of ( OUTFLOW-STORAGE ) values
174 (cms) - (ha-m)
175 [ 0.0 , 0.0 ]
176 [ 0.20 , 1.72]
177 [ -1 , -1 ] (max twenty pts)
178 IDovf=[9], NHYDovf=["ok-OVF"]
179
180 *%-----|-----|
181 ADD HYD IDsum=[ 3 ], NHYD=["SN_OK"], IDs to add=[1,4,9]
182 *%-----|-----|
183 SAVE HYD ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
184 HYD_COMMENT=["Total Flows at Okeefe Drain"]
185 *%-----|-----|
186 *#
187 *# Hydrograph from Node Okeefe routed to Node at Foster Drain
188 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
189 *#
190 ROUTE CHANNEL IDout=[1], NHYD=["N_FO"], IDin=[3],
191 RDT=[5](min),
192 CHLGTH=[1183](m), CHSLOPE=[0.0761](%),
193 FPSLOPE=[0.0761](%),
194 SECNUM=[1.0], NSEG=[3]
195 ( SEGROUGH, SEGDIST (m))=
196 [0.050,-33.89
197 -0.035,31.59
198 0.050,854.54] NSEG times

```

```

199      ( DISTANCE (m), ELEVATION (m))=
200      [-1075.50, 93.00]
201      [-1070.59, 92.50]
202      [-1003.21, 92.00]
203      [-1001.67, 92.00]
204      [-986.64, 92.00]
205      [-816.61, 91.50]
206      [-797.29, 91.00]
207      [-794.18, 91.00]
208      [-775.41, 91.50]
209      [-702.63, 91.50]
210      [-546.19, 91.50]
211      [-529.54, 91.50]
212      [-323.44, 91.00]
213      [-320.71, 91.00]
214      [-183.59, 91.00]
215      [-182.54, 90.50]
216      [-181.36, 90.00]
217      [-177.37, 90.00]
218      [-87.70, 90.00]
219      [-33.89, 90.00]
220      [-18.52, 86.88]
221      [0.00,85.20]
222      [16.20, 86.83]
223      [31.59, 90.00]
224      [33.03, 90.50]
225      [34.41, 91.00]
226      [34.99, 91.00]
227      [72.19, 91.00]
228      [208.76, 91.50]
229      [846.25, 92.00]
230      [854.54, 94.00]
231  *%-----|-----|
232  *#*****|
233  *#      Catchment FOSTER
234  *#      - To Foster ditch (north of the Jock)
235  *#      - Partially developed (medium density); remaining agricultural
236  *#*****|
237  CONTINUOUS STANDHYD ID=[2], NHYD=["FOSTER"], DT=[5]min, AREA=[373](ha),
238  XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
239  SCS curve number CN=[74],
240  Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
241  LGP=[40](m), MNP=[0.25], SCP=[0](min),
242  Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
243  LGI=[1577](m), MNI=[0.013], SCI=[0](min),
244  Continuous simulation parameters:
245  IaREcper=[4](hrs), IaREcimp=[4](hrs),
246  SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
247  InterEventTime=[18](hrs), END=-1
248
249  *#*****|
250  *#      Foster Pond
251  *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
252  *#      and a ratio of the catchment area to the West Clarke pond rating curve
253  *#      from the MSS for the next coordinates
254  *#*****|
255  ROUTE RESERVOIR IDout=[4], NHYD=["P_FOS"], IDin=[2],
256  RDT=[5](min),
257  TABLE of ( OUTFLOW-STORAGE ) values
258  (cms) - (ha-m)
259  [ 0.0 , 0.0 ]
260  [ 0.20 , 1.72]
261  [ -1 , -1 ] (max twenty pts)
262  IDovf=[9], NHYDovf=["FO-OVF"]
263  *%-----|-----|
264  ADD HYD IDsum=[ 3 ], NHYD=["SN_FO"], IDs to add=[1,4,9]

```



```

265 *%-----|-----|
266 SAVE HYD ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
267 HYD_COMMENT=["Total Flows at Foster Drain"]
268 *%-----|-----|
269 *#
270 *# Hydrograph from Node Foster routed to Node at Cedarview Road
271 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
272 *#
273 ROUTE CHANNEL IDout=[1], NHYD=["N_CE"] ,IDin=[3] ,
274 RDT=[5](min),
275 CHLGTH=[159](m), CHSLOPE=[0.0818](%),
276 FPSLOPE=[0.0818](%),
277 SECNUM=[1.0], NSEG=[3]
278 ( SEGROUGH, SEGDIST (m))=
279 [0.050,-15.46
280 -0.035,26.55
281 0.050,1299.52] NSEG times
282 ( DISTANCE (m), ELEVATION (m))=
283 [-891.38, 93.00]
284 [-882.49, 93.00]
285 [-880.92, 92.50]
286 [-879.37, 92.00]
287 [-877.72, 91.50]
288 [-876.10, 91.00]
289 [-873.23, 91.00]
290 [-871.82, 91.50]
291 [-870.40, 92.00]
292 [-803.44, 92.00]
293 [-645.23, 91.50]
294 [-391.20, 91.50]
295 [-91.00, 91.50]
296 [-85.52, 91.50]
297 [-15.46, 89.40]
298 [-9.79, 89.31]
299 [-3.22, 86.24]
300 [3.22, 85.07]
301 [10.96, 85.79]
302 [16.44, 86.49]
303 [26.55, 89.45]
304 [29.03, 90.27]
305 [35.76, 90.67]
306 [36.67, 91.00]
307 [108.08, 91.00]
308 [109.82, 90.50]
309 [112.04, 90.50]
310 [114.62, 91.00]
311 [116.76, 91.50]
312 [118.42, 92.00]
313 [449.53, 92.50]
314 [571.98, 92.50]
315 [1093.81, 93.50]
316 [1150.48, 94.00]
317 [1299.52, 95.00]
318 *%-----|-----|
319 *#*****
320 *# Catchment S-1
321 *# - To Jock River (north and south of Jock)
322 *# - Primarily agricultural fields; portion of sand quarry
323 *#*****
324 CONTINUOUS NASHYD ID=[2], NHYD=["S-1"], DT=[5]min, AREA=[245](ha),
325 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
326 N=[3], TP=[1.10]hrs,
327 Continuous simulation parameters:
328 IaRECper=[4](hrs),
329 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
330 InterEventTime=[12](hrs)

```

```

331 Baseflow simulation parameters:
332 BaseFlowOption=[1] ,
333 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
334 VHydCond=[0.055](mm/hr), END=-1
335
336 *%-----|-----|
337 ADD HYD IDsum=[ 3 ], NHYD=["SN_CE"], IDs to add=[1,2]
338 *%-----|-----|
339 SAVE HYD ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
340 HYD_COMMENT=["Total Flows at Cedarview Road"]
341 *%-----|-----|
342 *#
343 *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
344 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002
345 *#
346 ROUTE CHANNEL IDout=[1], NHYD=["N_WC"] ,IDin=[3] ,
347 RDT=[5](min),
348 CHLGTH=[825](m), CHSLOPE=[0.01](%),
349 FPSLOPE=[0.01](%),
350 SECNUM=[1.0], NSEG=[3]
351 ( SEGRROUGH, SEGDIST (m))=
352 [0.050,-37.5
353 -0.035,37.50
354 0.050,1367.08] NSEG times
355 ( DISTANCE (m), ELEVATION (m))=
356 [-1095.18, 94.00]
357 [-1091.79, 93.50]
358 [-1088.95, 93.00]
359 [-1086.77, 93.00]
360 [-1069.38, 93.00]
361 [-1063.14, 93.00]
362 [-1017.52, 93.00]
363 [-899.70, 93.00]
364 [-877.78, 93.00]
365 [-859.62, 92.50]
366 [-803.18, 93.00]
367 [-789.92, 92.00]
368 [-37.50, 90.00]
369 [-19.61, 87.04]
370 [0.00, 85.70]
371 [14.87, 86.93]
372 [37.50, 90.00]
373 [38.54, 90.50]
374 [42.23, 91]
375 [157.05,91.50]
376 [161.44, 91.50]
377 [236.48, 93.00]
378 [385.47, 92.50]
379 [390.78, 92.50]
380 [863.80, 93.00]
381 [866.13, 93.00]
382 [990.85, 92.50]
383 [991.82, 92.50]
384 [993.04, 93.00]
385 [994.81, 93.50]
386 [1005.36, 93.00]
387 [1190.52, 93.00]
388 [1267.97, 93.50]
389 [1318.99, 94.00]
390 [1367.08, 94.50]
391 *%-----|-----|
392 *#*****|
393 *# Catchment W_CLAR
394 *# - To West Clarke Drain (south of the Jock)
395 *# - Subdivision with 43% imp. as per Barrhaven South MSS
396 *#*****|

```

```

397 CONTINUOUS STANDHYD ID=[2], NHYD=["W_CLAR"], DT=[5]min, AREA=[243](ha),
398 XIMP=[0.43], TIMP=[0.43], DWF=[0](cms), LOSS=[2],
399 SCS curve number CN=[75],
400 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
401 LGP=[40](m), MNP=[0.25], SCP=[0](min),
402 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
403 LGI=[1273](m), MNI=[0.013], SCI=[0](min),
404 Continuous simulation parameters:
405 IaRECper=[4](hrs), IaRECimp=[4](hrs),
406 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
407 InterEventTime=[18](hrs), END=-1
408
409 *%-----|-----|
410 *#*****|
411 *# West Clarke Pond 2
412 *# - Rating curve obtained from Barrhaven South MSS modeling
413 *# - Tributary Drainage Area to MSS Pond 2 = 241 ha
414 *#*****|
415 ROUTE RESERVOIR IDout=[8], NHYD=["MS_P2"], IDin=[2],
416 RDT=[5](min),
417 TABLE of ( OUTFLOW-STORAGE ) values
418 (cms) - (ha-m)
419 [ 0.0 , 0.0 ]
420 [ 0.11 , 0.96]
421 [ -1 , -1 ] (max twenty pts)
422 IDovf=[9], NHYDovf=["P2-OVF"]
423 *%-----|-----|
424 ADD HYD IDsum=[ 4 ], NHYD=["SN_WC"], IDs to add=[8,9,1]
425 *%-----|-----|
426 SAVE HYD ID=[4], # OF PCYCLES=[-1], ICASEsh=[1]
427 HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
428 *%-----|-----|
429 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
430 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
431 *#
432 ROUTE CHANNEL IDout=[1], NHYD=["N_KB"], IDin=[4],
433 RDT=[5](min),
434 CHLGTH=[1020](m), CHSLOPE=[0.0498](%),
435 FPSLOPE=[0.0498](%),
436 SECNUM=[1.0], NSEG=[3]
437 ( SEGROUGH, SEGDIST (m))=
438 [0.050,-23.63
439 -0.035,23.63
440 0.050,728.3] NSEG times
441 ( DISTANCE (m), ELEVATION (m))=
442 [-1082.01,94]
443 [-1028.17,92.5]
444 [-992.3,93.5]
445 [-279.34,90]
446 [-23.63,90]
447 [-13.45,87.13]
448 [-0.07,86.24]
449 [10.54,87.15]
450 [23.63,90]
451 [24.86,90.5]
452 [26.72,91]
453 [45.07,91.5]
454 [128.17,91.5]
455 [270.7,92.5]
456 [728.3,95]
457
458 *%-----|-----|
459 *#*****|
460 *# Catchment KEN_BU
461 *# - To Kennedy-Burnett SWM Facility
462 *# - Outlets to Fraser-Clarke drain (north of the Jock)

```

```

463 *# - Medium density residential subdivision
464 *#*****
465 CONTINUOUS STANDHYD ID=[2], NHYD=["KEN_BU"], DT=[5]min, AREA=[281](ha),
466 XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
467 SCS curve number CN=[71],
468 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
469 LGP=[40](m), MNP=[0.25], SCP=[0](min),
470 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
471 LGI=[1369](m), MNI=[0.013], SCI=[0](min),
472 Continuous simulation parameters:
473 IaREcper=[4](hrs), IaREcimp=[4](hrs),
474 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
475 InterEventTime=[18](hrs), END=-1
476 *%-----|-----
477 *#*****
478 *# Existing Kennedy-Burnett SWM Facility
479 *# - Rating curve obtained from URTKBP
480 *# - Tributary Drainage Area to Pond = 160 ha
481 *#*****
482 ROUTE RESERVOIR IDout=[5], NHYD=["KEN_P"], IDin=[2],
483 RDT=[5](min),
484 TABLE of ( OUTFLOW-STORAGE ) values
485 (cms) - (ha-m)
486 [ 0.0 , 0.0 ]
487 [ 0.13 , 0.26 ]
488 [ 0.43 , 0.56 ]
489 [ 0.67 , 0.90 ]
490 [ 0.86 , 1.32 ]
491 [ 1.01 , 1.79 ]
492 [ 1.15 , 2.33 ]
493 [ -1 , -1 ] (max twenty pts)
494 IDovf=[6], NHYDovf=["KEN-OV"]
495 *%-----|-----
496 *#*****
497 *# Catchment FRASER
498 *# - To Fraser-Clarke drain (north of the Jock)
499 *# - Developed land with assumed 43% imp.
500 *#*****
501 CONTINUOUS STANDHYD ID=[7], NHYD=["FRASER"], DT=[5]min, AREA=[90](ha),
502 XIMP=[0.25], TIMP=[0.25], DWF=[0](cms), LOSS=[2],
503 SCS curve number CN=[80],
504 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
505 LGP=[40](m), MNP=[0.25], SCP=[0](min),
506 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
507 LGI=[775](m), MNI=[0.013], SCI=[0](min),
508 Continuous simulation parameters:
509 IaREcper=[4](hrs), IaREcimp=[4](hrs),
510 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
511 InterEventTime=[18](hrs), END=-1
512
513 *%-----|-----
514 ROUTE RESERVOIR IDout=[8], NHYD=["MS_P2"], IDin=[7],
515 RDT=[5](min),
516 TABLE of ( OUTFLOW-STORAGE ) values
517 (cms) - (ha-m)
518 [ 0.0 , 0.0 ]
519 [ 0.04 , 0.36 ]
520 [ -1 , -1 ] (max twenty pts)
521 IDovf=[9], NHYDovf=["P2-OVF"]
522 *%-----|-----
523 ADD HYD IDsum=[ 4 ], NHYD=["SN_KB"], IDs to add=[5,6,8,9,1]
524 *%-----|-----
525 SAVE HYD ID=[4], # OF PCYCLES=[-1], ICASEsh=[1]
526 HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
527 *%-----|-----
528 *# Hydrograph from Node Kennedy - Burnett Drain to Node Todd Drain

```

```

529  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
530  *#
531  ROUTE CHANNEL      IDout=[1], NHYD=["N_TO"] ,IDin=[4] ,
532                    RDT=[5](min),
533                    CHLGTH=[650](m),  CHSLOPE=[0.0498](%),
534                    FPSLOPE=[0.0498](%),
535                    SECNUM=[1.0],      NSEG=[3]
536                    ( SEGRROUGH, SEGDIST (m))=
537                      [0.050,-23.74
538                      -0.035,23.74
539                      0.050,74.7] NSEG times
540                    ( DISTANCE (m), ELEVATION (m))=
541                      [-74.18, 92.5]
542                      [-65.96, 92]
543                      [-54.17, 91.5]
544                      [-29.24, 91]
545                      [-27.41, 90.5]
546                      [-25.64, 90]
547                      [-23.74, 89.5]
548                      [-22,89. 26]
549                      [-20, 88.51]
550                      [-19, 88.32]
551                      [-15, 88.1]
552                      [-10, 88.11]
553                      [-5, 88.17]
554                      [0, 88.27]
555                      [5, 88.19]
556                      [10, 88.06]
557                      [15, 88.48]
558                      [16, 88.7]
559                      [23.74, 89.5]
560                      [24.68, 90]
561                      [25.57, 90.5]
562                      [26.5, 91]
563                      [47.55, 91]
564                      [74.7, 92.5]
565  *%-----|-----|
566
567  *#*****
568  *#      Catchment TODD
569  *#      - To Todd Drain (south of the Jock)
570  *#      - Subdivision with 43% imp. as per Barrhaven South MSS
571  *#*****
572  CONTINUOUS STANDHYD ID=[3], NHYD=["TODD"], DT=[5]min, AREA=[195](ha),
573                    XIMP=[0.43], TIMP=[0.43], DWF=[0](cms), LOSS=[2],
574                    SCS curve number CN=[77],
575                    Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
576                    LGP=[40](m), MNP=[0.25], SCP=[0](min),
577                    Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
578                    LGI=[1140](m), MNI=[0.013], SCI=[0](min),
579                    Continuous simulation parameters:
580                    IaRECper=[4](hrs), IaRECimp=[4](hrs),
581                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
582                    InterEventTime=[18](hrs), END=-1
583
584  *#*****
585  *#      Todd Pond 3
586  *#      - Rating curve obtained from Barrhaven South MSS modeling
587  *#      - Tributary Drainage Area to MSS Pond 3 = 193 ha
588  *#*****
589  ROUTE RESERVOIR   IDout=[2],  NHYD=["MS_P3"],  IDin=[3],
590                    RDT=[5](min),
591                    TABLE of ( OUTFLOW-STORAGE ) values
592                    (cms) - (ha-m)
593                    [ 0.0 , 0.0 ]
594                    [ 0.08 , 0.78]

```

```

595                                     [ -1 , -1 ] (max twenty pts)
596                                     IDovf=[9], NHYDovf=["P3-OVF"]
597 *%-----|-----|
598 ADD HYD                               IDsum=[10], NHYD=["SN_TO"], IDs to add=[1,2,9]
599 *%-----|-----|
600 SAVE HYD                               ID=[ 10 ], # OF PCYCLES=[-1], ICASEsh=[1]
601                                     HYD_COMMENT=["Total Flows at Todd Drain"]
602 *%-----|-----|
603 *#
604 *# Hydrograph from Todd Drain routed to Corrigan Drain
605 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
606 *#
607 ROUTE CHANNEL                          IDout=[1], NHYD=["N_TO"] ,IDin=[10] ,
608                                     RDT=[5](min),
609                                     CHLGTH=[280](m), CHSLOPE=[0.033](%),
610                                     FPSLOPE=[0.033](%),
611                                     SECNUM=[1.0], NSEG=[3]
612                                     ( SEGROUGH, SEGDIST (m))=
613                                     [0.075,-17.72
614                                     -0.045,17.72
615                                     0.075,80.62] NSEG times
616                                     ( DISTANCE (m), ELEVATION (m))=
617                                     [-83.32, 90.00]
618                                     [-81.36, 89.50]
619                                     [-79.12, 89.00]
620                                     [-76.13, 88.50]
621                                     [-20.46, 88.00]
622                                     [-19.36, 87.50]
623                                     [-18.51, 87.00]
624                                     [-17.72, 86.50]
625                                     [-11.95, 85.24]
626                                     [-0.11, 85.12]
627                                     [11.49, 85.20]
628                                     [17.72, 86.50]
629                                     [19.74, 87.00]
630                                     [21.22, 87.50]
631                                     [22.68, 88.00]
632                                     [24.28, 88.50]
633                                     [26.79, 89.00]
634                                     [71.98, 90.00]
635                                     [80.62, 90.50]
636 *%-----|-----|
637 *#*****|*****|
638 *# Catchment CORRIG
639 *# - To Corrigan Drain (south of the Jock)
640 *# - Primarily Developed (medium density)
641 *#*****|*****|
642 CONTINUOUS STANDHYD ID=[2], NHYD=["CORRIG"], DT=[5]min, AREA=[149](ha),
643                                     XIMP=[0.45], TIMP=[0.45], DWF=[0](cms), LOSS=[2],
644                                     SCS curve number CN=[77],
645                                     Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
646                                     LGP=[40](m), MNP=[0.25], SCP=[0](min),
647                                     Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
648                                     LGI=[997](m), MNI=[0.013], SCI=[0](min),
649                                     Continuous simulation parameters:
650                                     IaREcper=[4](hrs), IaREcimp=[4](hrs),
651                                     SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
652                                     InterEventTime=[18](hrs), END=-1
653
654 *%-----|-----|
655 *#*****|*****|
656 *# Corrigan Pond 1
657 *# - Rating curve obtained from Barrhaven South MSS modeling
658 *# - Tributary Drainage Area to MSS Pond 1 = 145 ha
659 *#*****|*****|
660 ROUTE RESERVOIR IDout=[5], NHYD=["MS_P1"], IDin=[2],

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```

661 RDT=[5](min),
662 TABLE of ( OUTFLOW-STORAGE ) values
663 (cms) - (ha-m)
664 [ 0.0 , 0.0 ]
665 [ 0.06 , 0.58]
666 [ -1 , -1 ] (max twenty pts)
667 IDovf=[4], NHYDovf=["P1-OVF"]
668 *%-----|-----|
669 ADD HYD IDsum=[ 3 ], NHYD=["SN_CO"], IDs to add=[1,4,5]
670 *%-----|-----|
671 SAVE HYD ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
672 HYD_COMMENT=["Total Flows at Corrigan Drain"]
673 *%-----|-----|
674 *#
675 *# Hydrograph from Corrigan Drain routed to Jockvale Road
676 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
677 *#
678 ROUTE CHANNEL IDout=[1], NHYD=["N_MI"] ,IDin=[3] ,
679 RDT=[5](min),
680 CHLGTH=[580](m), CHSLOPE=[0.4448](%),
681 FPSLOPE=[0.4448](%),
682 SECNUM=[1.0], NSEG=[3]
683 ( SEGROUGH, SEGDIST (m))=
684 [0.075,-17.72
685 -0.045,17.72
686 0.075,80.62] NSEG times
687 ( DISTANCE (m), ELEVATION (m))=
688 [-83.32, 90.00]
689 [-81.36, 89.50]
690 [-79.12, 89.00]
691 [-76.13, 88.50]
692 [-20.46, 88.00]
693 [-19.36, 87.50]
694 [-18.51, 87.00]
695 [-17.72, 86.50]
696 [-11.95, 85.24]
697 [-0.11, 85.12]
698 [11.49, 85.20]
699 [17.72, 86.50]
700 [19.74, 87.00]
701 [21.22, 87.50]
702 [22.68, 88.00]
703 [24.28, 88.50]
704 [26.79, 89.00]
705 [71.98, 90.00]
706 [80.62, 90.50]
707 *%-----|-----|
708 *#*****
709 *# Catchment MILLS
710 *# - To SWM Facility north of the Jock
711 *# - Primarily residential development
712 *#*****
713 CONTINUOUS STANDHYD ID=[2], NHYD=["MILLS"], DT=[5]min, AREA=[139](ha),
714 XIMP=[0.38], TIMP=[0.38], DWF=[0](cms), LOSS=[2],
715 SCS curve number CN=[74],
716 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
717 LGP=[40](m), MNP=[0.25], SCP=[0](min),
718 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
719 LGI=[963](m), MNI=[0.013], SCI=[0](min),
720 Continuous simulation parameters:
721 IaRECper=[4](hrs), IaRECimp=[4](hrs),
722 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
723 InterEventTime=[18](hrs), END=-1
724
725 *%-----|-----|
726 *#*****

```

```

727 *# Chapman Mills SWM Pond
728 *# - Rating curve obtained from CCL hydraulic modeling
729 *#*****
730 ROUTE RESERVOIR IDout=[5], NHYD=["MILL_P"], IDin=[2],
731 RDT=[5](min),
732 TABLE of ( OUTFLOW-STORAGE ) values
733 (cms) - (ha-m)
734 [ 0.0 , 0.0 ]
735 [ 0.01 , 0.01]
736 [ 0.05 , 0.06]
737 [ 0.09 , 0.11]
738 [ 0.13 , 0.15]
739 [ 0.18 , 0.19]
740 [ 0.28 , 0.28]
741 [ 0.37 , 0.34]
742 [ 0.45 , 0.40]
743 [ 0.51 , 0.44]
744 [ 0.56 , 0.47]
745 [ 0.64 , 0.52]
746 [ 0.76 , 0.59]
747 [ 0.86 , 0.65]
748 [ 1.09 , 0.78]
749 [ 1.44 , 0.96]
750 [ 3.18 , 1.84]
751 [ 4.05 , 2.31]
752 [ -1 , -1 ] (max twenty pts)
753 IDovf=[4], NHYDovf=["MIL-OV"]
754 *%-----|-----|
755 ADD HYD IDsum=[ 3 ], NHYD=["SN_MI"], IDs to add=[1,4,5]
756 *%-----|-----|
757 SAVE HYD ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
758 HYD_COMMENT=["Total Flows at Jockvale Road"]
759 *%-----|-----|
760 *#
761 *# Hydrograph from Jockvale Road routed to Heart's Desire
762 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689
763 *#
764 ROUTE CHANNEL IDout=[1], NHYD=["N_DE"] ,IDin=[3] ,
765 RDT=[5](min),
766 CHLGTH=[1962](m), CHSLOPE=[0.2227](%),
767 FPSLOPE=[0.2227](%),
768 SECNUM=[1.0], NSEG=[3]
769 ( SEGROUGH, SEGDIST (m))=
770 [0.075,-17.56
771 -0.045,18.27
772 0.075,67.59] NSEG times
773 ( DISTANCE (m), ELEVATION (m))=
774 [-111.59, 88.00]
775 [-102.58, 87.50]
776 [-96.20, 87.00]
777 [-90.04, 86.50]
778 [-84.02, 86.00]
779 [-77.54, 85.50]
780 [-54.07, 85.00]
781 [-39.43, 84.50]
782 [-28.30, 84.00]
783 [-24.12, 83.50]
784 [-22.30, 83.00]
785 [-20.55, 82.50]
786 [-17.56, 82.00]
787 [-12.63, 81.22]
788 [-0.11, 80.75]
789 [11.55, 81.22]
790 [18.27, 82.00]
791 [19.82, 82.50]
792 [22.48, 83.00]

```


793 [27.90, 83.50]
794 [29.31, 84.00]
795 [30.81, 84.50]
796 [32.51, 85.00]
797 [34.24, 85.50]
798 [36.34, 86.00]
799 [41.65, 86.50]
800 [62.64, 87.00]
801 [65.14, 87.50]
802 [67.59, 88.00]

*%-----|-----|
804 *#*****
805 *# Catchment DESIRE
806 *# - To Jock River (north of the Jock)
807 *# - Rural-estate subdivision (Heart's Desire Community)
808 *#*****
809 **CONTINUOUS STANDHYD** ID=[2], NHYD=["DESIRE"], DT=[5]min, AREA=[24](ha),
810 XIMP=[0.25], TIMP=[0.25], DWF=[0](cms), LOSS=[2],
811 SCS curve number CN=[77],
812 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
813 LGP=[40](m), MNP=[0.25], SCP=[0](min),
814 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
815 LGI=[400](m), MNI=[0.013], SCI=[0](min),
816 Continuous simulation parameters:
817 IaREcper=[4](hrs), IaREcimp=[4](hrs),
818 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
819 InterEventTime=[18](hrs), END=-1

*%-----|-----|
822 *#*****
823 *# Catchment JOCKVA
824 *# - To Jockvale SWM Facility
825 *# - Residential development & golf course
826 *#*****
827 **CONTINUOUS STANDHYD** ID=[3], NHYD=["JOCKVA"], DT=[5]min, AREA=[252](ha),
828 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
829 SCS curve number CN=[74],
830 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
831 LGP=[40](m), MNP=[0.25], SCP=[0](min),
832 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
833 LGI=[1296](m), MNI=[0.013], SCI=[0](min),
834 Continuous simulation parameters:
835 IaREcper=[4](hrs), IaREcimp=[4](hrs),
836 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
837 InterEventTime=[18](hrs), END=-1

*%-----|-----|
840 *#*****
841 *# Jockvale SWM Facility
842 *# - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
843 *#*****

844 **ROUTE RESERVOIR** IDout=[5], NHYD=["JOCK_P"], IDin=[3],
845 RDT=[5](min),
846 TABLE of (OUTFLOW-STORAGE) values

847 (cms) - (ha-m)
848 [0.0 , 0.0]
849 [0.27 , 0.03]
850 [0.28 , 0.55]
851 [0.29 , 1.14]
852 [0.30 , 1.80]
853 [0.31 , 2.32]
854 [1.12 , 2.87]
855 [2.92 , 3.45]
856 [4.64 , 4.07]
857 [6.69 , 4.72]
858 [9.02 , 5.39]

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859             [ 11.62 , 6.10]
860             [ 14.42 , 6.85]
861             [ 17.45 , 7.62]
862             [ 20.69 , 8.44]
863             [ 24.08 , 9.28]
864             [ 27.68 , 10.17]
865             [ -1 , -1 ] (max twenty pts)
866             IDovf=[4], NHYDovf=["JO-OVF"]
867 *%-----|-----|
868 ADD HYD      IDsum=[ 3 ], NHYD=["SN_DE"], IDs to add=[1,2,4,5]
869 *%-----|-----|
870 SAVE HYD     ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
871             HYD_COMMENT=["Total Flows at Heart's Desire"]
872 *%-----|-----|
873 *#
874 *# Hydrograph from Heart's Desire routed to Rideau River
875 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
876 *#
877 ROUTE CHANNEL IDout=[1], NHYD=["N1"] ,IDin=[3] ,
878             RDT=[5](min),
879             CHLGTH=[563](m), CHSLOPE=[0.9668](%),
880             FPSLOPE=[0.9668](%),
881             SECNUM=[1.0], NSEG=[3]
882             ( SEGROUGH, SEGDIST (m))=
883             [0.075,-30.20
884             -0.045,30.20
885             0.075,168.81] NSEG times
886             ( DISTANCE (m), ELEVATION (m))=
887             [-170.17, 86.00]
888             [-164.75, 85.50]
889             [-158.08, 85.00]
890             [-113.12, 82.00]
891             [-98.46, 81.50]
892             [-92.24, 81.00]
893             [-86.88, 80.50]
894             [-81.54, 80.00]
895             [-74.36, 79.50]
896             [-63.54, 79.00]
897             [-39.23, 78.50]
898             [-34.51, 78.00]
899             [-33.01, 77.50]
900             [-30.20, 77.00]
901             [-13.42, 76.18]
902             [-1.14, 76.09]
903             [17.06, 76.18]
904             [30.20, 77.00]
905             [32.95, 77.50]
906             [34.06, 78.00]
907             [35.11, 78.50]
908             [36.32, 79.00]
909             [37.74, 79.50]
910             [48.48, 81.50]
911             [49.25, 82.00]
912             [55.61, 84.50]
913             [57.09, 85.00]
914             [59.51, 85.50]
915             [64.34, 86.00]
916             [66.30, 86.00]
917             [76.71, 86.50]
918             [101.83, 86.50]
919             [119.73, 87.00]
920             [142.04, 87.50]
921             [168.81, 88.00]
922 *%-----|-----|
923 *#*****
924 *# Catchment S-2

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925 *# - To Jock River (north and south)
926 *# - Undeveloped floodplain and river
927 *#*****
928 CONTINUOUS NASHYD ID=[2], NHYD=["S-2"], DT=[5]min, AREA=[102](ha),
929 DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
930 N=[3], TP=[0.40]hrs,
931 Continuous simulation parameters:
932 IaRECper=[4](hrs),
933 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
934 InterEventTime=[12](hrs)
935 Baseflow simulation parameters:
936 BaseFlowOption=[1] ,
937 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
938 VHydCond=[0.055](mm/hr), END=-1
939
940 *%-----|-----|
941 ADD HYD IDsum=[ 3 ], NHYD=["SN_N1"], IDs to add=[1,2]
942 *%-----|-----|
943 SAVE HYD ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
944 HYD_COMMENT=["Total Flows at Rideau River"]
945 *%-----|-----|
946 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
947 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[005]
948 *% ["C24SC005.stm"] <--storm filename, one per line for NSTORM time
949 *%-----|-----|
950 *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
951 *%START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[010]
952 *% ["C24SC010.stm"] <--storm filename, one per line for NSTORM time
953 *%-----|-----|
954 *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
955 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[025]
956 *% ["C24SC025.stm"] <--storm filename, one per line for NSTORM time
957 *%-----|-----|
958 *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
959 *%START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[050]
960 *% ["C24SC050.stm"] <--storm filename, one per line for NSTORM time
961 *%-----|-----|
962 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
963 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
964 *% ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
965 *%-----|-----|
966
967 *#####
968 FINISH
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00003 SSSSS W W M H H Y Y M M O O O 999 999 .....
00004 S W M M M M M H H H H Y Y M M M O O # # 9 9 9 9 Ver5 Beta
00005 SSSSS W W M M M H H H H Y Y M M M O O # # 9 9 9 9 Ver5 Beta
00006 S W M M M H H Y Y M M O O 9999 9999 Sept 2000
00007 SSSSS W W M M M H H Y Y M M O O 9 9 9 9 .....
00008 .....
00009 Stormwater Management Hydrologic Model
00010 .....
00011 Stormwater Management Hydrologic Model
00012 .....
00013 ***** SMWHD00 Ver/5 Beta *****
00014 ***** A single event and continuous hydrologic simulation model *****
00015 ***** based on the principles of HWSO and its successor *****
00016 ***** OTTHYD03 and OTTHYD09 *****
00017 ***** Distributed by: J.P. Sabourin and Associates Inc. *****
00018 ***** Ottawa, Ontario: (613) 727-5199 *****
00019 ***** Ottawa, Quebec: (819) 243-6188 *****
00020 ***** E-Mail: smwbyso@jfsa.com *****
00021 *****
00022 *****
00023 *****
00024 ***** Licensed user: Ottawa - Ottawa - 604 *****
00025 ***** Serial#:3783815 *****
00026 *****
00027 *****
00028 *****
00029 ***** PROGRAM ARRAY DIMENSIONS *****
00030 ***** Maximum value for ID numbers: 10 *****
00031 ***** Max. number of rainfall points: 52750 *****
00032 ***** Max. number of flow points: 52750 *****
00033 *****
00034 ***** DESCRIPTION SUMMARY TABLE HEADERS (units depend on MOUTPUT on START) *****
00035 ***** ID: Hydrograph identification numbers. (1-10) *****
00036 ***** HNRID: Hydrograph reference numbers. (6 digits or characters) *****
00037 ***** AREA: Drainage area associated with hydrograph, (in) or (ha). *****
00038 ***** QPEAK: Peak flow of simulated hydrograph. (l/s) or (m3/s). *****
00039 ***** TpeakDate_hh:mm is the date and time of the peak flow. *****
00040 ***** R.V.: Runoff volume of simulated hydrograph, (in) or (mm). *****
00041 ***** R.C.: Runoff Coefficient of simulated hydrograph. (ratio). *****
00042 ***** : see WARNING or NOTE message printed at end of run. *****
00043 ***** : *****
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00053 ***** S U M M A R Y O U T P U T *****
00054 ***** *****
00055 ***** DATE: 2006-11-15 TIME: 14:33:15 RUN COUNTER: 000132 *****
00056 ***** *****
00057 ***** Input filename: C:\Navin\OCT08-1\CORNTIN-1\SM_POST\SM_POST.dat *****
00058 ***** Output filename: C:\Navin\OCT08-1\CORNTIN-1\SM_POST\SM_POST.out *****
00059 ***** Summary filename: C:\Navin\OCT08-1\CORNTIN-1\SM_POST\SM_POST.sum *****
00060 ***** User comments: *****
00061 ***** 1 *****
00062 ***** 2 *****
00063 ***** 3 *****
00064 ***** *****
00065 *****
00066 *****
00067 ***** Project Name [Jock River Reach 1 Subwatershed Study/Project #: [160400044] *****
00068 ***** Date [October 2006] *****
00069 ***** Modeller [Nevin Gautam/ Original by Ana M Paeres] *****
00070 ***** Company [Stantec] *****
00071 ***** License # [3824306] *****
00072 ***** *****
00073 ***** *****
00074 ***** END OF RUN : 1 *****
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007469 # [DT= 5.00] SUM= 09:04:0V 359.57 16.663 No_date 28:20 34.02 n/a
007470 # [Subdivision with 43# imp. as per Barhaven South MSS
007471 # [L/S/n= 181.7 / 0.57 / 0.35]
007472 # [Vmax= 1.826 / Dmax= 3.15]
007473 # [Smin= 36.67 / Smax=244.49 / Sx= 0.01]
007474 # [InterVTime= 12.00]
007475 # [Impervious area: IArea= 4.675SLP=1.00/LD= 40.0MP=250(SCP= 0]
007476 # [IARcImp= 4.00 / IARcPer= 4.00]
007477 # [Smin= 31.15 / Smax=207.66 / Sx= 0.01]
007478 # [Rating curve obtained assuming 40#3/ha in 24 hours for quality control
007479 # and a ratio of the catchment area to West Clarke pond rating curve
007480 # for the MSB for the next coordinates
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Attachment C

Model 3 – Jock River Reach One Update

JFSA, 2021

SWMHYMO Input & Summary files

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1  20    Metric units / ID numbers OFF
2  *#*****
3  *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *#*****
5  *# Project Name: [Jock River]    Project Number: [1474-16]
6  *# Date       : 04-03-2021
7  *# Modeller   : [M.M.]
8  *# Company    : JFSAinc.
9  *# License #  : 2549237
10 *#*****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *# 2020-11-30 change TMJSTO in COMPUTE DUALHYD (TMJSTO = 0.1 instead of 0.0001)
16 *# 2020-12-01 correct pond curve values
17 *# 2020-12-01 change W_CLAR_BRAZ XIMP to 0.55, SLPI=[0.5](%) (impervious slope), and
LGI up to 700m
18 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NHYDout=["N_TO"]
,NHYDin=["SN_TO"]) from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model
will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELs
aren't well suited to really flat slopes.
19 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NHYDout=["N_WC"]
,NHYDin=["SN_CE"]) from 0.01 % (as per Stantec Report 2007) to 0.0255 % so the model
will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELs
aren't well suited to really flat slopes.
20 *
21 * Calibrated parameters for Summer 2003 data:  APII=50, APIK=0.85, CN=varies,
22 *                                               SK=0.01, InterEventTime=12,
23 *                                               GWResk=0.96, VHydCond=0.055
24 *
25 *# -----
26 *
27 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
28 *              ["XAVG0315.STM"] average storm data a 15 minute time step
29 *              The above rainf file is an average of the JFSA gauge data
30 *              with the City of Ottawa rainfall data collected during
31 *              the same period.
32 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
33 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
34                ["C24SC002.stm"] <--storm filename, one per line for NSTORM time
35 *%-----|-----|
36 *%-----|-----|
37 READ STORM    STORM_FILENAME=["storm.001"]
38 *%-----|-----|
39 MODIFY STORM  ICASEms=[1], NSHIFT=[96],
40                RedFACT=[1],
41 *%-----|-----|
42 DEFAULT VALUES ICASEdef=[1], read and print values
43                DEFVAL_FILENAME=["CitiGate.DEF"]
44 *%-----|-----|
45 COMPUTE API   APII=[50], APIK=[.85]/day
46 *%-----|-----|
47 *%-----|-----|
48 *#
49 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
50 *# of 1.32
51 *%-----|-----|
52 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[1]min, AREA=[3680](ha),
53                DWF=[0](cms), CN/C=[64], IA=[2.5](mm),
54                N=[3.0], TP=[7.13]hrs,
55                Continuous simulation parameters:
56                IaRECper=[4](hrs),
57                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
58                InterEventTime=[12](hrs)
59                Baseflow simulation parameters:

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60         BaseFlowOption=[1] ,
61         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
62         VHydCond=[0.055](mm/hr),     END=-1
63     *%-----|-----
64     *#
65     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
66     *# of 1.32
67     *%-----|-----
68     CONTINUOUS NASHYD  NHYD=["SW_13"], DT=[1]min, AREA=[971](ha),
69                       DWF=[0](cms),  CN/C=[61], IA=[2.5](mm),
70                       N=[3.0], TP=[3.76]hrs,
71                       Continuous simulation parameters:
72                       IaRECper=[4](hrs),
73                       SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
74                       InterEventTime=[12](hrs)
75                       Baseflow simulation parameters:
76                       BaseFlowOption=[1] ,
77                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
78                       VHydCond=[0.055](mm/hr),     END=-1
79     *%-----|-----
80     *#
81     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
82     *# of 1.80
83     *%-----|-----
84     CONTINUOUS NASHYD  NHYD=["JR_GWM"], DT=[1]min, AREA=[3074](ha),
85                       DWF=[0](cms),  CN/C=[55], IA=[2.5](mm),
86                       N=[3], TP=[11.33]hrs,
87                       Continuous simulation parameters:
88                       IaRECper=[4](hrs),
89                       SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
90                       InterEventTime=[12](hrs)
91                       Baseflow simulation parameters:
92                       BaseFlowOption=[1] ,
93                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
94                       VHydCond=[0.055](mm/hr),     END=-1
95     *%-----|-----
96     CONTINUOUS NASHYD  NHYD=["JR_ASH"], DT=[1]min, AREA=[1781](ha),
97                       DWF=[0](cms),  CN/C=[72], IA=[2.5](mm),
98                       N=[3.0], TP=[3.91]hrs,
99                       Continuous simulation parameters:
100                      IaRECper=[4](hrs),
101                      SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
102                      InterEventTime=[12](hrs)
103                      Baseflow simulation parameters:
104                      BaseFlowOption=[1] ,
105                      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
106                      VHydCond=[0.055](mm/hr),     END=-1
107     *%-----|-----
108     CONTINUOUS NASHYD  NHYD=["SW_11"], DT=[1]min, AREA=[500](ha),
109                       DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),
110                       N=[3.0], TP=[1.24]hrs,
111                       Continuous simulation parameters:
112                       IaRECper=[4](hrs),
113                       SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
114                       InterEventTime=[12](hrs)
115                       Baseflow simulation parameters:
116                       BaseFlowOption=[1] ,
117                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
118                       VHydCond=[0.055](mm/hr),     END=-1
119     *%-----|-----
120     *#
121     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
122     *# of 1.80
123     *%-----|-----
124     CONTINUOUS NASHYD  NHYD=["NN_CK"], DT=[1]min, AREA=[1917](ha),
125                       DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),

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126 N=[3.0], TP=[5.29]hrs,
127 Continuous simulation parameters:
128 IaRECper=[4](hrs),
129 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
130 InterEventTime=[12](hrs)
131 Baseflow simulation parameters:
132 BaseFlowOption=[1] ,
133 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
134 VHydCond=[0.055](mm/hr), END=-1
135 *%-----|-----
136 *#
137 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
138 *# of 1.52
139 *%-----|-----
140 CONTINUOUS NASHYD NHYD=["SW_10"], DT=[1]min, AREA=[5666](ha),
141 DWF=[0](cms), CN/C=[72], IA=[2.5](mm),
142 N=[3.0], TP=[8.00]hrs,
143 Continuous simulation parameters:
144 IaRECper=[4](hrs),
145 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
146 InterEventTime=[12](hrs)
147 Baseflow simulation parameters:
148 BaseFlowOption=[1] ,
149 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
150 VHydCond=[0.055](mm/hr), END=-1
151 *%-----|-----
152 *#
153 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
154 *# of 1.75
155 *%-----|-----
156 CONTINUOUS NASHYD NHYD=["KG CK"], DT=[1]min, AREA=[8376](ha),
157 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
158 N=[3.0], TP=[11.66]hrs,
159 Continuous simulation parameters:
160 IaRECper=[4](hrs),
161 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
162 InterEventTime=[12](hrs)
163 Baseflow simulation parameters:
164 BaseFlowOption=[1] ,
165 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
166 VHydCond=[0.055](mm/hr), END=-1
167 *%-----|-----
168 *#
169 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
170 *# of 1.68
171 *%-----|-----
172 CONTINUOUS NASHYD NHYD=["SW_9"], DT=[1]min, AREA=[1132](ha),
173 DWF=[0](cms), CN/C=[70], IA=[2.5](mm),
174 N=[3.0], TP=[2.51]hrs,
175 Continuous simulation parameters:
176 IaRECper=[4](hrs),
177 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
178 InterEventTime=[12](hrs)
179 Baseflow simulation parameters:
180 BaseFlowOption=[1] ,
181 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
182 VHydCond=[0.055](mm/hr), END=-1
183 *%-----|-----
184 *#
185 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
186 *# of 1.82
187 *%-----|-----
188 CONTINUOUS NASHYD NHYD=["NC CK"], DT=[1]min, AREA=[4464](ha),
189 DWF=[0](cms), CN/C=[62], IA=[2.5](mm),
190 N=[3.0], TP=[11.32]hrs,
191 Continuous simulation parameters:

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192 IaREcper=[4](hrs),
193 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
194 InterEventTime=[12](hrs)
195 Baseflow simulation parameters:
196 BaseFlowOption=[1] ,
197 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
198 VHydCond=[0.055](mm/hr), END=-1
199 *%-----|-----
200 *#
201 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
202 *# of 1.80
203 *%-----|-----
204 CONTINUOUS NASHYD NHYD=["SW_8"], DT=[1]min, AREA=[131](ha),
205 DWF=[0](cms), CN/C=[63], IA=[2.5](mm),
206 N=[3.0], TP=[0.90]hrs,
207 Continuous simulation parameters:
208 IaREcper=[4](hrs),
209 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
210 InterEventTime=[12](hrs)
211 Baseflow simulation parameters:
212 BaseFlowOption=[1] ,
213 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
214 VHydCond=[0.055](mm/hr), END=-1
215 *%-----|-----
216 *#
217 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
218 *# of 1.65
219 *%-----|-----
220 CONTINUOUS NASHYD NHYD=["HB_DR"], DT=[1]min, AREA=[3854](ha),
221 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
222 N=[3.0], TP=[8.42]hrs,
223 Continuous simulation parameters:
224 IaREcper=[4](hrs),
225 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
226 InterEventTime=[12](hrs)
227 Baseflow simulation parameters:
228 BaseFlowOption=[1] ,
229 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
230 VHydCond=[0.055](mm/hr), END=-1
231 *%-----|-----
232 *#
233 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
234 *# of 1.82
235 *%-----|-----
236 CONTINUOUS NASHYD NHYD=["SW_7"], DT=[1]min, AREA=[3197](ha),
237 DWF=[0](cms), CN/C=[57], IA=[2.5](mm),
238 N=[3.0], TP=[6.65]hrs,
239 Continuous simulation parameters:
240 IaREcper=[4](hrs),
241 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
242 InterEventTime=[12](hrs)
243 Baseflow simulation parameters:
244 BaseFlowOption=[1] ,
245 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
246 VHydCond=[0.055](mm/hr), END=-1
247 *%-----|-----
248 *#
249 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
250 *# of 1.75
251 *%-----|-----
252 CONTINUOUS NASHYD NHYD=["SW_6"], DT=[1]min, AREA=[165](ha),
253 DWF=[0](cms), CN/C=[67], IA=[2.5](mm),
254 N=[3.0], TP=[4.18]hrs,
255 Continuous simulation parameters:
256 IaREcper=[4](hrs),
257 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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258 InterEventTime=[12](hrs)
259 Baseflow simulation parameters:
260 BaseFlowOption=[1] ,
261 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
262 VHydCond=[0.055](mm/hr) , END=-1
263 *%-----|-----
264 *#
265 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
266 *# of 1.67
267 *%-----|-----
268 CONTINUOUS NASHYD NHYD=["VG_DR"] , DT=[1]min , AREA=[1332](ha) ,
269 DWF=[0](cms) , CN/C=[72] , IA=[2.5](mm) ,
270 N=[3.0] , TP=[5.95]hrs ,
271 Continuous simulation parameters:
272 IaREcper=[4](hrs) ,
273 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
274 InterEventTime=[12](hrs)
275 Baseflow simulation parameters:
276 BaseFlowOption=[1] ,
277 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
278 VHydCond=[0.055](mm/hr) , END=-1
279 *%-----|-----
280 CONTINUOUS NASHYD NHYD=["SW_5"] , DT=[1]min , AREA=[224](ha) ,
281 DWF=[0](cms) , CN/C=[77] , IA=[2.5](mm) ,
282 N=[3.0] , TP=[0.75]hrs ,
283 Continuous simulation parameters:
284 IaREcper=[4](hrs) ,
285 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
286 InterEventTime=[12](hrs)
287 Baseflow simulation parameters:
288 BaseFlowOption=[1] ,
289 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
290 VHydCond=[0.055](mm/hr) , END=-1
291 *%-----|-----
292 *#
293 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
294 *# of 1.20
295 *%-----|-----
296 CONTINUOUS NASHYD NHYD=["FL_CK"] , DT=[1]min , AREA=[4945](ha) ,
297 DWF=[0](cms) , CN/C=[74] , IA=[2.5](mm) ,
298 N=[3.0] , TP=[4.45]hrs ,
299 Continuous simulation parameters:
300 IaREcper=[4](hrs) ,
301 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
302 InterEventTime=[12](hrs)
303 Baseflow simulation parameters:
304 BaseFlowOption=[1] ,
305 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
306 VHydCond=[0.055](mm/hr) , END=-1
307 *%-----|-----
308 CONTINUOUS NASHYD NHYD=["SW_5A2"] , DT=[1]min , AREA=[20](ha) ,
309 DWF=[0](cms) , CN/C=[81] , IA=[2.5](mm) ,
310 N=[3.0] , TP=[0.62]hrs ,
311 Continuous simulation parameters:
312 IaREcper=[4](hrs) ,
313 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
314 InterEventTime=[12](hrs)
315 Baseflow simulation parameters:
316 BaseFlowOption=[1] ,
317 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
318 VHydCond=[0.055](mm/hr) , END=-1
319 *%-----|-----
320 *#
321 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
322 *# of 1.61
323 *%-----|-----

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324 CONTINUOUS NASHYD  NHYD=["SW_5A1"], DT=[1]min, AREA=[1412](ha),
325 DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
326 N=[3.0], TP=[8.00]hrs,
327 Continuous simulation parameters:
328 IaREcper=[4](hrs),
329 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
330 InterEventTime=[12](hrs)
331 Baseflow simulation parameters:
332 BaseFlowOption=[1] ,
333 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
334 VHydCond=[0.055](mm/hr), END=-1
335 *%-----|
336 CONTINUOUS NASHYD  NHYD=["SW_4"], DT=[1]min, AREA=[585](ha),
337 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
338 N=[3.0], TP=[1.75]hrs,
339 Continuous simulation parameters:
340 IaREcper=[4](hrs),
341 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
342 InterEventTime=[12](hrs)
343 Baseflow simulation parameters:
344 BaseFlowOption=[1] ,
345 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
346 VHydCond=[0.055](mm/hr), END=-1
347 *%-----|
348 CONTINUOUS NASHYD  NHYD=["LM_CK"], DT=[1]min, AREA=[1021](ha),
349 DWF=[0](cms), CN/C=[80], IA=[2.5](mm),
350 N=[3.0], TP=[2.46]hrs,
351 Continuous simulation parameters:
352 IaREcper=[4](hrs),
353 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
354 InterEventTime=[12](hrs)
355 Baseflow simulation parameters:
356 BaseFlowOption=[1] ,
357 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
358 VHydCond=[0.055](mm/hr), END=-1
359 *%-----|
360 CONTINUOUS NASHYD  NHYD=["SW_2"], DT=[1]min, AREA=[177](ha),
361 DWF=[0](cms), CN/C=[77], IA=[2.5](mm),
362 N=[3.0], TP=[0.75]hrs,
363 Continuous simulation parameters:
364 IaREcper=[4](hrs),
365 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
366 InterEventTime=[12](hrs)
367 Baseflow simulation parameters:
368 BaseFlowOption=[1] ,
369 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
370 VHydCond=[0.055](mm/hr), END=-1
371 *%-----|
372 CONTINUOUS NASHYD  NHYD=["SM_DR"], DT=[1]min, AREA=[1122](ha),
373 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
374 N=[3.0], TP=[3.25]hrs,
375 Continuous simulation parameters:
376 IaREcper=[4](hrs),
377 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
378 InterEventTime=[12](hrs)
379 Baseflow simulation parameters:
380 BaseFlowOption=[1] ,
381 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
382 VHydCond=[0.055](mm/hr), END=-1
383 *%-----|
384 CONTINUOUS NASHYD  NHYD=["MO_DR"], DT=[1]min, AREA=[2737](ha),
385 DWF=[0](cms), CN/C=[76], IA=[2.5](mm),
386 N=[3.0], TP=[3.03]hrs,
387 Continuous simulation parameters:
388 IaREcper=[4](hrs),
389 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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456 (cms) - (ha-m)
457 [ 0.0 , 0.0 ]
458 [1.991, 2.144 ]
459 [2.693, 39.826 ]
460 [3.509, 81.697 ]
461 [4.578, 318.774 ]
462 [5.647, 594.947 ]
463 [7.109, 910.219 ]
464 [8.616, 1264.589 ]
465 [10.371, 1658.057 ]
466 [12.402, 2090.622 ]
467 [22.056, 3462.487 ]
468 [ -1 , -1 ] (max twenty pts)
469
470 NHYDovf=[ " " ] ,
471
472 *%-----|-----
473 *#
474 SAVE HYD NHYD=["RES_GM"], # OF PCYCLES=[-1], ICASEsh=[-1]
475 HYD_FILENAME=["H_RESGM"]
476 HYD_COMMENT=["Outflow from Res GM"]
477
478 *%-----|-----
479 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
480 *# (Approximated cross-section - see cross-section 258)
481 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
482 ROUTE CHANNEL NHYDout=["N12"] ,NHYDin=["RES_GM"] ,
483 RDT=[1](min),
484 CHLGTH=[5926](m), CHSLOPE=[0.0759](%),
485 FPSLOPE=[0.0759](%),
486 SECNUM=[1.0], NSEG=[1]
487 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
488 ( DISTANCE (m), ELEVATION (m))=
489 [-40, 132.5]
490 [-30, 132]
491 [-25, 131.5]
492 [-13, 130]
493 [-8, 127.00]
494 [-7, 126.50]
495 [-6, 126]
496 [-5.5, 125.50]
497 [0, 123.75]
498 [4.5, 125.50]
499 [6, 126]
500 [7.5, 126.5]
501 [9, 127]
502 [10, 127.5]
503 [11.5, 128.00]
504 [15.5, 129.5]
505
506 *%-----|-----
507 *#
508 *# Addition of Subwatershed Jock River at Ashton to Node 12
509 *#
510 ADD HYD NHYDsum=["S_N12"], NHYDs to add=["N12"+"JR_ASH"]
511 SAVE HYD NHYD=["S_N12"], # OF PCYCLES=[-1], ICASEsh=[-1]
512 HYD_FILENAME=["H_SN12"]
513 HYD_COMMENT=["flow at S_N12 near Ashton"]
514
515 *%-----|-----
516 *#
517 *# Sum of hydrographs from Node 12 routed to Node 11
518 *# (Approximated cross-section - see cross-section 258)
519 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
520 *ROUTE CHANNEL NHYDout=["N11"] ,NHYDin=["S_N12"] ,
521 * RDT=[1](min),
522 * CHLGTH=[972](m), CHSLOPE=[0.0514](%),
523 * FPSLOPE=[0.0514](%),
524 * SECNUM=[1.0], NSEG=[1]
525 * ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
526 * ( DISTANCE (m), ELEVATION (m))=

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```

522 * [-40, 132.5]
523 * [-30, 132]
524 * [-25, 131.5]
525 * [-13, 130]
526 * [-8, 127.00]
527 * [-7, 126.50]
528 * [-6, 126]
529 * [-5.5, 125.50]
530 * [0, 123.75]
531 * [4.5, 125.50]
532 * [6, 126]
533 * [7.5, 126.5]
534 * [9, 127]
535 * [10, 127.5]
536 * [11.5, 128.00]
537 * [15.5, 129.5]
538 *%-----|-----
539 *#
540 *# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
541 *#
542 ROUTE CHANNEL NHYDout=["Dum11"] ,NHYDin=["S_N12"] ,
543 RDT=[1](min),
544 CHLGTH=[972](m), CHSLOPE=[0.054](%),
545 FPSLOPE=[0.054](%),
546 SECNUM=[1.0], NSEG=[1]
547 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
548 ( DISTANCE (m), ELEVATION (m))=
549 [-40, 132.5]
550 [-30, 132]
551 [-25, 131.5]
552 [-13, 130]
553 [-8, 127.00]
554 [-7, 126.50]
555 [-6, 126]
556 [-5.5, 125.50]
557 [0, 123.75]
558 [4.5, 125.50]
559 [6, 126]
560 [7.5, 126.5]
561 [9, 127]
562 [10, 127.5]
563 [11.5, 128.00]
564 [15.5, 129.5]
565 *%-----|-----
566 *#
567 *# Addition of Subwatershed 11 and No Name Creek to Node 11
568 *#
569 ADD HYD NHYDsum=["S_N11"], NHYDs to add=["Dum11"+"SW_11"+"NN_CK"]
570 *%-----|-----
571 *#
572 *# Sum of hydrographs from Node 11 routed to Node 10
573 *# Section 1
574 *#
575 ROUTE CHANNEL NHYDout=["N10"] ,NHYDin=["S_N11"] ,
576 RDT=[1](min),
577 CHLGTH=[14028](m), CHSLOPE=[0.1568](%),
578 FPSLOPE=[0.1568](%),
579 SECNUM=[1.0], NSEG=[5]
580 ( SEGROUGH, SEGDIST (m))=
581 [0.04,-52.82
582 0.1,-6.47
583 -0.05,6.47
584 0.1,45.36
585 0.04,423.88] NSEG times
586 ( DISTANCE (m), ELEVATION (m))=
587 [-226.24 ,112.50]

```

```

588 [-167.50 ,111.50]
589 [-106.81 ,111.00]
590 [-92.37 ,110.00]
591 [-52.82 ,109.00]
592 [-24.90 , 109.00]
593 [-17.02, 108.50]
594 [-6.47, 108.00]
595 [6.47, 108.00]
596 [15.67, 108.50]
597 [18.95, 109.00]
598 [45.36, 109.50]
599 [120.79, 110.00]
600 [145.72, 111.00]
601 [181.56, 111.50]
602 [423.88, 112.50]
603 *%-----|-----|
604 *#
605 *# Addition of Subwatershed 10 to Node 10
606 *#
607 ADD HYD          NHYDsum=["S_N10"], NHYDs to add=["N10"+"SW_10"]
608 *%-----|-----|
609 SAVE HYD       NHYD=["S_N10"], # OF PCYCLES=[-1], ICASEsh=[-1]
610                   HYD_FILENAME=["H_SN10"]
611                   HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
612 *%-----|-----|
613 *# Addition of Kings Creek to S_N10
614 *#
615 ADD HYD          NHYDsum=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
616 *%-----|-----|
617 *#
618 *# Sum of hydrographs from Node 10 routed to Node 9
619 *# Section 2
620 *#
621 ROUTE CHANNEL   NHYDout=["N9"] ,NHYDin=["S_N10A"] ,
622                   RDT=[1](min),
623                   CHLGTH=[3982](m),  CHSLOPE=[0.0753](%),
624                                           FPSLOPE=[0.0753](%),
625                   SECNUM=[1.0],      NSEG=[4]
626                   ( SEGROUGH, SEGDIST (m))=
627                     [0.04,-30.27
628                      0.05,-18.42
629                      -0.05,18.42
630                      0.04,131.58] NSEG times
631                   ( DISTANCE (m), ELEVATION (m))=
632                     [-446.74, 106.00]
633                     [-415.68, 105.50]
634                     [-285.40, 105.00]
635                     [-173.77, 104.50]
636                     [-144.95, 104.00]
637                     [-111.18, 103.50]
638                     [-94.06, 103.00]
639                     [-71.02, 102.50]
640                     [-30.27, 102.00]
641                     [-19.33, 100.00]
642                     [-18.42, 99.50]
643                     [18.42, 99.50]
644                     [20.77, 100.00]
645                     [27.93, 101.00]
646                     [52.29, 101.00]
647                     [68.80, 101.50]
648                     [79.66, 103.00]
649                     [91.50, 103.50]
650                     [131.58, 104.00]
651 *%-----|-----|
652 *#
653 *# Addition of Subwatershed 9 and Nichols Creek to Node 9

```

```

654  *#
655  ADD HYD          NHYDsum=["S_N9"], NHYDs to add=["N9"+"SW_9"+"NC_CK"]
656  *%-----|-----
657  *#
658  *# Sum of hydrographs from Node 9 routed to Node 8
659  *# Section 3
660  *#
661  ROUTE CHANNEL    NHYDout=["N8"] ,NHYDin=["S_N9"] ,
662                  RDT=[1](min),
663                  CHLGTH=[2269](m),  CHSLOPE=[0.0882](%),
664                                          FPSLOPE=[0.0882](%),
665                  SECNUM=[1.0],      NSEG=[3]
666                  ( SEGROUGH, SEGDIST (m))=
667                    [0.1,-17.99
668                    -0.045,17.31
669                    0.1,456.58] NSEG times
670                  ( DISTANCE (m), ELEVATION (m))=
671                    [-201.19,100.50]
672                    [-135.21, 100.00]
673                    [-94.83, 99.50]
674                    [-67.05, 99.00]
675                    [-17.99, 98.50]
676                    [-16.02, 98.00]
677                    [-13.95, 97.50]
678                    [13.95, 97.50]
679                    [15.64, 98.00]
680                    [17.31, 98.50]
681                    [162.02, 98.50]
682                    [172.89 ,99.00]
683                    [314.38, 99.00]
684                    [343.78, 99.50]
685                    [365.67, 100.00]
686                    [376.68, 100.00 ]
687                    [393.11, 99.50]
688                    [404.97, 99.50]
689                    [431.70, 100.00]
690                    [456.58, 100.50 ]
691  *%-----|-----
692  *#
693  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
694  *#
695  ADD HYD          NHYDsum=["S_N8"], NHYDs to add=["N8"+"SW_8"+"HB_DR"]
696  *%-----|-----
697  *#
698  *# Sum of hydrographs from Node 8 routed to Node 7
699  *# Section 4
700  *#
701  ROUTE CHANNEL    NHYDout=["N7"] ,NHYDin=["S_N8"],
702                  RDT=[1](min),
703                  CHLGTH=[3750](m),  CHSLOPE=[0.0533](%),
704                                          FPSLOPE=[0.0533](%),
705                  SECNUM=[1.0],      NSEG=[3]
706                  ( SEGROUGH, SEGDIST (m))=
707                    [0.12,-18.11
708                    -0.07,17.22
709                    0.12,590.05] NSEG times
710                  ( DISTANCE (m), ELEVATION (m))=
711                    [-433.21, 102.00]
712                    [-425.34, 101.50]
713                    [-377.56, 101.50]
714                    [-366.23, 101.00]
715                    [-202.60, 100.50]
716                    [-96.25, 99.50]
717                    [-68.36 99.00]
718                    [-18.11, 98.50]
719                    [-13.81, 97.50]

```

```

720             [13.81, 97.50]
721             [17.22, 98.50]
722             [161.95, 98.50]
723             [173.11, 99.00]
724             [314.05, 99.00]
725             [365.52, 100.00]
726             [404.70, 99.50]
727             [476.74, 100.50]
728             [502.31, 101.00]
729             [584.69, 101.00]
730             [585.79, 101.00]
731             [590.05, 102.00]
732 *%-----|-----
733 *#
734 *# Addition of Subwatershed 7 to Node 7
735 *#
736 ADD HYD           NHYDsum=["S_N7"], NHYDs to add=["N7"+"SW_7"]
737 *%-----|-----
738 SAVE HYD         NHYD=["S_N7"], # OF PCYCLES=[-1], ICASEsh=[-1]
739                   HYD_FILENAME=["H_SN7"]
740                   HYD_COMMENT=["flow at S_N7: N7 + SW_7"]
741 *%-----|-----
742 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
743 *# Storage area and volumes were estimated from available topo maps.
744 *# Release rate from fen was assumed to be controlled by the downstream
745 *# river cross-section for summer conditions. It is was assumed that for up to
746 *# 0.75 m of water, the main channel of the river provided the storage. Above
747 *# this depth, the wetland starts to signigicantly store water.
748 *#
749 ROUTE RESERVOIR  NHYDout=["RES_RF"] ,NHYDin=["S_N7"] ,
750                   RDT=[1](min),
751                   TABLE of ( OUTFLOW-STORAGE ) values
752                             (cms) - (ha-m)
753                   TABLE of ( OUTFLOW-STORAGE ) values
754                             (cms) - (ha-m)
755                             [ 0.0 , 0.0 ]
756                             [0.9051, 2.40]
757                             [2.907, 4.13]
758                             [9.744, 9.18]
759                             [20.304, 14.96]
760                             [34.167, 310.21]
761                             [74.993, 605.46]
762                             [104.876, 900.71]
763                             [140.56, 2892.00]
764                             [225.00, 3615.63]
765                             [ -1 , -1 ] (max twenty pts)
766                   NHYDovf=[" " ] ,
767 *%-----|-----
768 SAVE HYD         NHYD=["RES_RF"], # OF PCYCLES=[-1], ICASEsh=[-1]
769                   HYD_FILENAME=["H_ResRF"]
770                   HYD_COMMENT=["outflow of Richmond Fen"]
771 *%-----|-----
772 *#
773 *# Sum of hydrographs from Node 7 routed to Node 6
774 *# Section 5
775 *#
776 ROUTE CHANNEL   NHYDout=["N6"] ,NHYDin=["RES_RF"] ,
777                   RDT=[1](min),
778                   CHLGTH=[3056](m), CHSLOPE=[0.0818](%),
779                   FPSLOPE=[0.0818](%),
780                   SECNUM=[1.0], NSEG=[5]
781                   ( SEGROUGH, SEGDIST (m))=
782                   [0.025,-70.8
783                   0.1,-23.9
784                   -0.05,23.9
785                   0.06,39.8

```

```

786             0.05,96.3] NSEG times
787             ( DISTANCE (m), ELEVATION (m))=
788                 [-100.8, 97.00]
789                 [-70.8, 96.50]
790                 [-52.0, 96.00]
791                 [-35.1, 95.50]
792                 [-30.6, 95.00]
793                 [-23.9, 94.54]
794                 [23.9, 94.54]
795                 [39.8, 95.00]
796                 [50.4, 95.50]
797                 [93.5, 96.00]
798                 [94.9, 96.50]
799                 [96.3, 97.00]
800 *%-----|-----
801 *#
802 *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
803 *#
804 ADD HYD             NHYDsum=["S_N6"], NHYDs to add=["N6"+"SW_6"+"VG_DR"]
805 *%-----|-----
806 *#
807 *# Sum of hydrographs from Node 6 routed to Node 5
808 *# Section 6
809 *#
810 ROUTE CHANNEL     NHYDout=["N5"] ,NHYDin=["S_N6"] ,
811                     RDT=[1](min),
812                     CHLGTH=[1852](m),   CHSLOPE=[0.0540](%),
813                                     FPSLOPE=[0.0540](%),
814                     SECNUM=[1.0],       NSEG=[3]
815                     ( SEGROUGH, SEGDIST (m))=
816                         [0.035,-131.59
817                         -0.045,48.96
818                         0.1,239.04] NSEG times
819                     ( DISTANCE (m), ELEVATION (m))=
820                         [-686.30, 94.50]
821                         [-675.70, 94.00]
822                         [-492.52, 93.00]
823                         [-467.28, 94.00]
824                         [-131.59, 94.00]
825                         [-92.79, 92.50]
826                         [-18.06, 91.00]
827                         [18.06, 91.00]
828                         [43.47, 92.50]
829                         [48.96, 94.00]
830                         [177.43, 94.00]
831                         [239.04,94.50]
832 *%-----|-----
833 *#
834 *# Addition of Subwatershed 5 and Flowing Creek to Node 5
835 *#
836 ADD HYD             NHYDsum=["S_N5"], NHYDs to add=["N5"+"SW_5"+"FL_CK"]
837 *%-----|-----
838 *#
839 *# Sum of hydrographs from Node 5 routed to Node 5A
840 *# Section 7
841 *#
842 ROUTE CHANNEL     NHYDout=["N5A"] ,NHYDin=["S_N5"] ,
843                     RDT=[1](min),
844                     CHLGTH=[556](m),   CHSLOPE=[0.0900](%),
845                                     FPSLOPE=[0.0900](%),
846                     SECNUM=[1.0],       NSEG=[4]
847                     ( SEGROUGH, SEGDIST (m))=
848                         [0.04,-41.5
849                         0.1,-14.0
850                         -0.045,14.0
851                         0.1,41.1] NSEG times

```

```

852          ( DISTANCE (m), ELEVATION (m))=
853              [-275.8, 93.00]
854              [-248.6, 92.50]
855              [-237.0, 92.00]
856              [-219.3, 91.50]
857              [-202.1, 91.50]
858              [-186.0, 92.00]
859              [-129.2, 92.00]
860              [-117.6, 91.50]
861              [-100.6, 91.00]
862              [-41.5, 91.00]
863              [-20.0, 91.00]
864              [-14.0, 90.54]
865              [14.0, 90.54]
866              [15.3, 91.00]
867              [17.3, 91.50]
868              [38.4, 92.00]
869              [39.8, 92.50]
870              [41.1, 93.00]
871  *%-----|-----
872  *#
873  *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
874  *#
875  ADD HYD          NHYDsum=["S_N5A"], NHYDs to add=["N5A"+"SW_5A2"+"SW_5A1"]
876  *%-----|-----
877  *#
878  *# Sum of hydrographs from Node 5A routed to Node 4
879  *# Section 8
880  *#
881  ROUTE CHANNEL    NHYDout=["N4"] ,NHYDin=["S_N5A"] ,
882                  RDT=[1](min),
883                  CHLGTH=[4630](m),  CHSLOPE=[0.0432](%),
884                                          FPSLOPE=[0.0432](%),
885                  SECNUM=[1.0],      NSEG=[3]
886                  ( SEGROUGH, SEGDIST (m))=
887                      [0.05,-28.2
888                      -0.035,28.2
889                      0.05,173.1] NSEG times
890                  ( DISTANCE (m), ELEVATION (m))=
891                      [-38.9, 92.00]
892                      [-35.8, 91.50]
893                      [-33.3, 91.00]
894                      [-28.2, 90.50]
895                      [-15.0, 87.48]
896                      [-5.0, 88.34]
897                      [5.0, 86.20]
898                      [15.0, 88.55]
899                      [28.2, 90.50]
900                      [29.7, 91.00]
901                      [46.5, 91.00]
902                      [127.8, 91.00]
903                      [148.7, 91.50]
904                      [173.1, 92.00]
905  *%-----|-----
906  *#
907  *# Addition of Subwatershed 4 and Leamy Creek to Node 4
908  *#
909  ADD HYD          NHYDsum=["S_N4"], NHYDs to add=["N4"+"SW_4"+"LM_CK"]
910  SAVE HYD        NHYD=["S_N4"], # OF PCYCLES=[-1], ICASEsh=[1]
911                  HYD_COMMENT=["flow at S_N4"]
912  *%-----|-----
913  *#
914  *# Sum of hydrographs from Node 4 routed to Node 2
915  *# Section 9
916  *#
917  ROUTE CHANNEL    NHYDout=["N2"] ,NHYDin=["S_N4"] ,

```



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918 RDT=[1](min),
919 CHLGTH=[1667](m), CHSLOPE=[0.0600](%),
920 FPSLOPE=[0.0600](%),
921 SECNUM=[1.0], NSEG=[4]
922 ( SEGROUGH, SEGDIST (m))=
923 [0.1,-28.0
924 -0.04,28.4
925 0.06,31.7
926 0.04,80.2] NSEG times
927 ( DISTANCE (m), ELEVATION (m))=
928 [-36.3, 92.00]
929 [-32.6, 91.50]
930 [-30.2, 91.00]
931 [-28.0, 90.45]
932 [-15.0, 87.48]
933 [-5.0, 88.34]
934 [5.0, 86.20]
935 [15.0, 88.55]
936 [28.0, 90.45]
937 [28.4, 90.50]
938 [30.4, 91.00]
939 [31.7, 91.50]
940 [80.2, 92.00]
941 *%-----|-----
942 *#
943 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
944 *#
945 ADD HYD NHYDsum=["S_N2"], NHYDs to add=["N2"+"SW_2"+"SM_DR"+"MO_DR"]
946 *%-----|-----
947 SAVE HYD NHYD=["S_N2"], # OF PCYCLES=[-1], ICASEsh=[-1]
948 HYD_FILENAME=["H_SN2"]
949 HYD_COMMENT=["flow at S_N2 Jock River Gauge at Moodie Dr."]
950 *%-----|-----
951 *#
952 *# Sum of hydrographs from Node 2 routed to Node 1
953 *# Section 10
954 *#
955 *#*****
956 *%READ HYD NHYD=["S_N2"],
957 *% HYD_FILENAME=["H-S_N2"]
958 *%-----|-----
959 *#
960 *# Hydrograph from Node 2 routed to Node 416
961 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
962 *#
963 ROUTE CHANNEL NHYDout=["N_416"] ,NHYDin=["S_N2"] ,
964 RDT=[1](min),
965 CHLGTH=[2327](m), CHSLOPE=[0.0498](%),
966 FPSLOPE=[0.0498](%),
967 SECNUM=[1.0], NSEG=[3]
968 ( SEGROUGH, SEGDIST (m))=
969 [0.075,-23.96
970 -0.055,23.96
971 0.075,157.38] NSEG times
972 ( DISTANCE (m), ELEVATION (m))=
973 [-336.97,93.5]
974 [-318.85,93]
975 [-259,92.5]
976 [-133.18,92]
977 [-33.17,92]
978 [-27.21,92]
979 [-26.14,91.5]
980 [-24.99,91]
981 [-23.96,90.5]
982 [-14.33,88.26]
983 [-0.68,88.12]

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```

984 [14.33,88.26]
985 [23.96,90.5]
986 [32.12,91]
987 [43.74,91.5]
988 [57.09,92]
989 [73.53,92.5]
990 [108.27,93]
991 [125.88,93.5]
992 [144.81,94]
993 [157.38,94.5]
994 *%-----|-----|
995 *#*****|
996 *# Catchment SW-1a
997 *# - Portion of RVCA catchment SW_1 outside of Reach 1 subwatershed
998 *# - Undeveloped agricultural land
999 *#*****|
1000 CONTINUOUS NASHYD NHYD=["SW_1a"], DT=[1]min, AREA=[536.42](ha),
1001 DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
1002 N=[3], TP=[2.79]hrs,
1003 Continuous simulation parameters:
1004 IaREcper=[4](hrs),
1005 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1006 InterEventTime=[12](hrs)
1007 Baseflow simulation parameters:
1008 BaseFlowOption=[1],
1009 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1010 VHydCond=[0.055](mm/hr), END=-1
1011 *%-----|-----|
1012 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
1013 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[1](min), AREA=[44.93](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
1014 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
1015 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
1016 * LGI=[547.296](m), MNI=[0.013], SCI=[0](min),
1017 * Continuous simulation parameters:
1018 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
1019 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1020 * InterEventTime=[12](hrs), END=-1
1021 *%-----|-----|
1022 CONTINUOUS NASHYD NHYD=["S-1-Okeefe"], DT=[1]min, AREA=[44.93](ha),
1023 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1024 N=[3], TP=[1.049]hrs,
1025 Continuous simulation parameters:
1026 IaREcper=[4](hrs),
1027 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1028 InterEventTime=[12](hrs)
1029 Baseflow simulation parameters:
1030 BaseFlowOption=[1],
1031 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1032 VHydCond=[0.055](mm/hr), END=-1
1033 *%-----|-----|
1034 *COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CINLET=[4.796](cms), NINLET=[1],
1035 * MajNHYD=["S-1-OkMJ"]
1036 * MinNHYD=["S-1-OkMN"]
1037 * TMJSTO=[9999999](cu-m)
1038 *%-----|-----|
1039 *ADD HYD NHYDsum=["S-1-OkS"], NHYDs to add=["S-1-OkMJ"+"S-1-OkMN"]
1040 *%-----|-----|
1041 *ROUTE RESERVOIR NHYDout=["S-1-OkSR"],NHYDin=["S-1-OkS"],
1042 * RDT=[1](min),
1043 * TABLE of ( OUTFLOW-STORAGE ) values
1044 * (cms) - (ha-m)
1045 * [ 0.0 , 0.0 ]

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1046 * [ 0.5370, 1.7917 ]
1047 * [ -1 , -1 ] (max twenty pts)
1048 * NHYDovf=["S-1-OkSovf"]
1049 *%-----|-----|
1050 ADD HYD NHYDsum=["SN_416"], NHYDs to add=["N_416"+"SW_1a"+"S-1-Okeefe"]
1051 *%-----|-----|
1052 SAVE HYD NHYD=["SN_416"], # OF PCYCLES=[-1], ICASEsh=[1]
1053 HYD_COMMENT=["Total Flows at Highway 416"]
1054 *%-----|-----|
1055 *#
1056 *# Hydrograph from Node 416 routed to Node at Okeefe drain
1057 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245
1058 *#
1059 ROUTE CHANNEL NHYDout=["N_OK"] ,NHYDin=["SN_416"] ,
1060 RDT=[1](min),
1061 CHLGTH=[497](m), CHSLOPE=[0.3018](%),
1062 FPSLOPE=[0.3018](%),
1063 SECNUM=[1.0], NSEG=[3]
1064 ( SEGROUGH, SEGDIST (m))=
1065 [0.075,-19.40
1066 -0.055,19.40
1067 0.075,377.02] NSEG times
1068 ( DISTANCE (m), ELEVATION (m))=
1069 [-1061.41, 92.50]
1070 [-945.91, 92.00]
1071 [-783.64, 91.50]
1072 [-136.74, 91.00]
1073 [-86.04, 91.00]
1074 [-20.86, 91.00]
1075 [-20.18, 90.50]
1076 [-19.40, 90.00]
1077 [-11.68, 86.89]
1078 [0.00, 86.10]
1079 [12.09, 86.81]
1080 [19.40, 90.00]
1081 [34.68, 90.50]
1082 [60.56, 91.00]
1083 [170.14, 91.00]
1084 [175.05, 90.50]
1085 [180.29, 90.00]
1086 [193.41, 90.00]
1087 [195.98, 90.50]
1088 [377.02, 92.50]
1089 *%-----|-----|
1090 *#*****
1091 *# Catchment OKEEFE
1092 *# - To O'Keefe drain (north of the Jock)
1093 *# - Developed with assumed 43% imp.
1094 *# - 2020-12-01 add Okeefe model (Area 513.02 HA) instead of current Okeefe (Area
1095 513.02 HA)
1096 *# - 2020-11-20 Okeefe detailed model was added as per the NOVATECH SWMHYMO model
1097 (Citi-Gate 2014).
1098 *%-----|-----|
1099 *#*****
1100 CONTINUOUS NASHYD NHYD=["O-1"], DT=[1]min, AREA=[63.72](ha),
1101 DWF=[0](cms), CN/C=[61], IA=[6.2](mm), N=[3], TP=[.9]hrs,
1102 Continuous simulation parameters:
1103 IaRECper=[4](hrs),
1104 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1105 InterEventTime=[12](hrs)
1106 Baseflow simulation parameters:
1107 BaseFlowOption=[1] ,
1108 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1109 VHydCond=[0.055](mm/hr), END=-1

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1110 *%-----|-----
1111 *ROUTE FLOW THROUGH AREA 0-2
1112 ROUTE CHANNEL NHYDout=["O-1R"], NHYDin=["O-1"], RDT=[1](min),
1113 CHLGTH=[960](m), CHSLOPE=[0.63](%), FPSLOPE=[0.63](%),
1114 SECNUM=[1], NSEG=[3]
1115 ( SEGROUGH, SEGDIST (m))=[0.06,4 -.043,6 0.06,10] NSEG times
1116 ( DISTANCE (m), ELEVATION (m))=[0.00, 2.0]
1117 [0.0, 2.0]
1118 [4.0, 0.0]
1119 [6.0, 0.0]
1120 [10.0, 2.0]
1121 *%-----|-----
1122 CONTINUOUS NASHYD NHYD=["O-2"], DT=[1]min, AREA=[28.61](ha),
1123 DWF=[0](cms), CN/C=[57], IA=[5.2](mm), N=[3], TP=[1.1]hrs,
1124 Continuous simulation parameters:
1125 IaRECper=[4](hrs),
1126 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1127 InterEventTime=[12](hrs)
1128 Baseflow simulation parameters:
1129 BaseFlowOption=[1] ,
1130 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1131 VHydCond=[0.055](mm/hr), END=-1
1132 *%-----|-----
1133 CONTINUOUS NASHYD NHYD=["O-4"], DT=[1]min, AREA=[46.94](ha),
1134 DWF=[0](cms), CN/C=[49], IA=[9.2](mm), N=[3], TP=[0.9]hrs,
1135 Continuous simulation parameters:
1136 IaRECper=[4](hrs),
1137 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1138 InterEventTime=[12](hrs)
1139 Baseflow simulation parameters:
1140 BaseFlowOption=[1] ,
1141 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1142 VHydCond=[0.055](mm/hr), END=-1
1143 *%-----|-----
1144 *TOTAL EXTERNAL FLOW NORTH OF O'KEEFE CT. CROSSING
1145 ADD HYD NHYDsum=["OKF-N"], NHYDs to add=["O-1R"+"O-2"+"O-4"]
1146 *%-----|-----
1147 *ROUTE FLOW THROUGH AREA 0-6
1148 ROUTE CHANNEL ROUTE CHANNEL NHYDout=["OKF-NR"], NHYDin=["OKF-N"], RDT=[1](min),
1149 CHLGTH=[210](m), CHSLOPE=[.81](%), FPSLOPE=[.81](%),
1150 SECNUM=[1], NSEG=[3]
1151 ( SEGROUGH, SEGDIST (m))=[0.043,22.43 -0.043,25.07
1152 0.043,45.54] NSEG times
1153 ( DISTANCE (m), ELEVATION (m))=[0.00, 3.73]
1154 (14.62, 1.56)
1155 (18.41, 1.44)
1156 (22.43, 0.00)
1157 (25.07, 0.70)
1158 (29.10, 1.79)
1159 (33.73, 2.71)
1160 (45.54, 3.58)
1161 *%-----|-----
1162 CONTINUOUS NASHYD NHYD=["O-6"], DT=[1]min, AREA=[16.46](ha),
1163 DWF=[0](cms), CN/C=[43], IA=[9.2](mm), N=[3], TP=[0.7]hrs,
1164 Continuous simulation parameters:
1165 IaRECper=[4](hrs),
1166 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1167 InterEventTime=[12](hrs)
1168 Baseflow simulation parameters:
1169 BaseFlowOption=[1] ,
1170 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1171 VHydCond=[0.055](mm/hr), END=-1
1172 *%-----|-----
1173 CONTINUOUS STANDHYD NHYD=["O-3"], DT=[1](min), AREA=[39.67](ha), XIMP=[0.15],
TIMP=[0.30], DWF=[0](cms),

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1174      LOSS=[2], SCS curve number CN=[50], Pervious surfaces:
1175      IAper=[4.67](mm), SLPP=[0.32](%),
1176      LGP=[440](m), MNP=[0.035], SCP=[0](min), Impervious surfaces:
1177      IAimp=[1.57](mm), SLPI=[0.32](%),
1178      LGI=[1880](m), MNI=[0.013], SCI=[0](min),
1179      Continuous simulation parameters:
1180      IaRECPper=[4](hrs), IaRECImp=[4](hrs),
1181      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1182      InterEventTime=[12](hrs), END=-1
1183 *%-----|-----|
1184 CONTINUOUS STANDHYD NHYD=["O-5"], DT=[1](min), AREA=[60.63](ha), XIMP=[0.13],
1185 TIMP=[0.26], DWF=[0](cms),
1186      LOSS=[2], SCS curve number CN=[61],
1187      Pervious surfaces: IAper=[4.67](mm), SLPP=[1.38](%),
1188      LGP=[550](m), MNP=[0.035], SCP=[0](min), Impervious surfaces:
1189      IAimp=[1.57](mm), SLPI=[1.38](%),
1190      LGI=[1450](m), MNI=[0.013], SCI=[0](min),
1191      Continuous simulation parameters:
1192      IaRECPper=[4](hrs), IaRECImp=[4](hrs),
1193      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1194      InterEventTime=[12](hrs), END=-1
1195 *%-----|-----|
1196 *TOTAL EXTERNAL FLOWS WEST OF THE SITE AND NORTH OF O'KEEFE CRT
1197 *%-----|-----|
1198 ADD HYD NHYDsum=["PT1"], NHYDs to add=["OKF-NR"+"O-3"+"O-5"+"O-6"]
1199 *%-----|-----|
1200 CONTINUOUS NASHYD NHYD=["O-7"], DT=[1]min, AREA=[5.28](ha),
1201 DWF=[0](cms), CN/C=[54], IA=[7.5](mm), N=[3], TP=[0.6]hrs,
1202 Continuous simulation parameters:
1203 IaRECPper=[4](hrs),
1204 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1205 InterEventTime=[12](hrs)
1206 Baseflow simulation parameters:
1207 BaseFlowOption=[1] ,
1208 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1209 VHydCond=[0.055](mm/hr), END=-1
1210 *%-----|-----|
1211 *ANALYSIS POINT 1 - TOTAL FLOW NORTH OF FALLOWFIELD DR. AND O'KEEFE CRT.
1212 ADD HYD NHYDsum=["FF"], NHYDs to add=["PT1"+"O-7"]
1213 *%-----|-----|
1214 *ROUTE FLOW through O'Keefe Drain 1
1215 ROUTE CHANNEL NHYDout=["DRAIN1"], NHYDin=["FF"], RDT=[1](min),
1216 CHLGTH=[302]{m}, CHSLOPE=[1.00](%), FPSLOPE=[1.00](%),
1217 SECNUM=[1], NSEG=[3]
1218 ( SEGROUGH, SEGDIST (m))=[0.07,13.45 -0.043,16.55 0.07,30.00] NSEG
1219 times
1220 ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1221 (3.45, 0.60)
1222 (13.45, 0.50)
1223 (14.45, 0.00)
1224 (15.55, 0.00)
1225 (16.55, 0.50)
1226 (26.55, 0.60)
1227 (30.00, 1.70)
1228 *%-----|-----|
1229 CONTINUOUS NASHYD NHYD=["D1"], DT=[1]min, AREA=[1.17](ha),
1230 DWF=[0](cms), CN/C=[84], IA=[9.0](mm), N=[3], TP=[0.28]hrs,
1231 Continuous simulation parameters:
1232 IaRECPper=[4](hrs),
1233 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1234 InterEventTime=[12](hrs)
1235 Baseflow simulation parameters:
1236 BaseFlowOption=[1] ,
1237 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1238 VHydCond=[0.055](mm/hr), END=-1
1239 *%-----|-----|

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1235 CONTINUOUS STANDHYD NHYD=["A1"], DT=[1]min, AREA=[2.50](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1236 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1237 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1238 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[223.607](m), MNI=[0.013], SCI=[0](min),
1239 Continuous simulation parameters:
1240 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1241 *%-----|-----|
1242 ROUTE RESERVOIR NHYDout=["A1-STR"], NHYDin=["A1"], RDT=[1](min),
1243 TABLE of ( OUTFLOW-STORAGE ) values
1244 (cms) - (ha-m)
1245 [ 0.000 , 0.000 ]
1246 [ 0.035 , 0.038 ]
1247 [ 0.072 , 0.051 ]
1248 [ 0.100 , 0.059 ]
1249 [ 0.125 , 0.070 ]
1250 [ 0.160 , 0.074 ]
1251 [ 0.185 , 0.081 ]
1252 [ -1 , -1 ] (max twenty pts)
1253 NHYDovf=["A1-OVF"]
1254 *%-----|-----|
1255 CONTINUOUS STANDHYD NHYD=["ST-2"], DT=[1]min, AREA=[0.59](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1256 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1257 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1258 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[108.628](m), MNI=[0.013], SCI=[0](min),
1259 Continuous simulation parameters:
1260 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1261 *%-----|-----|
1262 ROUTE RESERVOIR NHYDout=["ST2STR"], NHYDin=["ST-2"], RDT=[1](min),
1263 TABLE of ( OUTFLOW-STORAGE ) values
1264 (cms) - (ha-m)
1265 [ 0.000 , 0.0000 ]
1266 [ 0.052 , 0.0010 ]
1267 [ 0.053 , 0.0080 ]
1268 [ -1 , -1 ] (max twenty pts)
1269 NHYDovf=["ST2OVF"]
1270 *%-----|-----|
1271 *%-----|-----|
1272 *TOTAL FLOW NORTH OF STRANDHERD DR. (EAST BRANCH) CROSSING
1273 *%-----|-----|
1274 CONTINUOUS NASHYD NHYD=["O-8"], DT=[1]min, AREA=[60.55](ha),
1275 DWF=[0](cms), CN/C=[69], IA=[4.0](mm), N=[3], TP=[1.0]hrs,
1276 Continuous simulation parameters:
1277 IaREcper=[4](hrs),
1278 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1279 InterEventTime=[12](hrs)
1280 Baseflow simulation parameters:
1281 BaseFlowOption=[1] ,
1282 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1283 VHydCond=[0.055](mm/hr), END=-1
1284 *%-----|-----|
1285 ROUTE PIPE PTYPE=[2]rect, NHYDout=["O8PIPE"], RNUMBER=[1], PWIDTH=[1800](mm),
PHEIGHT=[1200](mm), PLNGTH=[335.1](m),
1286 PROUGH=[0.013], PSLOPE=[0.001](m/m), NHYDin=["O-8"], RDT=[1](min)
1287 *%-----|-----|
1288 *%-----|-----|
1289 ADD HYD NHYDsum=["ST2-IN"], NHYDs to

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add=["DRAIN1"+"D1"+"A1-STR"+"A1-OVF"+"ST2STR"+"ST2OVF"+"O8PIPE"]
1290 *%-----|-----|
1291 CONTINUOUS STANDHYD NHYD=["A7"], DT=[1]min, AREA=[3.51](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1292 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1293 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1294 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[264.953](m), MNI=[0.013], SCI=[0](min),
1295 Continuous simulation parameters:
1296 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1297 *%-----|-----|
1298 ROUTE RESERVOIR NHYDout=["A7-STR"], NHYDin=["A7"], RDT=[1](min),
1299 TABLE of ( OUTFLOW-STORAGE ) values
1300 (cms) - (ha-m)
1301 [ 0.000 , 0.000 ]
1302 [ 0.049 , 0.054 ]
1303 [ 0.102 , 0.072 ]
1304 [ 0.140 , 0.082 ]
1305 [ 0.175 , 0.099 ]
1306 [ 0.225 , 0.105 ]
1307 [ 0.260 , 0.114 ]
1308 [ -1 , -1 ] (max twenty pts)
1309 NHYDovf=["A7-OVF"]
1310 *%-----|-----|
1311 CONTINUOUS STANDHYD NHYD=["ST-3"], DT=[1]min, AREA=[0.71](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1312 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1313 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1314 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[119.164](m), MNI=[0.013], SCI=[0](min),
1315 Continuous simulation parameters:
1316 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1317 *%-----|-----|
1318 ROUTE RESERVOIR NHYDout=["ST3STR"], NHYDin=["ST-3"], RDT=[1](min),
1319 TABLE of ( OUTFLOW-STORAGE ) values
1320 (cms) - (ha-m)
1321 [ 0.000 , 0.0000 ]
1322 [ 0.063 , 0.0010 ]
1323 [ 0.064 , 0.0094 ]
1324 [ -1 , -1 ] (max twenty pts)
1325 NHYDovf=["ST3OVF"]
1326 *%-----|-----|
1327 *ANALYSIS POINT 2 - TOTAL FLOW AT OUTLET OF STREET 2/3 INTERSECTION
1328 *%-----|-----|
1329 ADD HYD NHYDsum=["PT2ST3"], NHYDs to
add=["ST2-IN"+"A7-STR"+"A7-OVF"+"ST3STR"+"ST3OVF"]
1330 *%-----|-----|
1331 *ROUTE FLOW through O'Keefe Drain 2
1332 ROUTE CHANNEL NHYDout=["DRAIN2"], NHYDin=["PT2ST3"], RDT=[1](min),
1333 CHLGTH=[592](m), CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1334 SECNUM=[1], NSEG=[3]
1335 ( SEGROUGH, SEGDIST (m))=[0.07,12.60 -0.043,17.40 0.07,30.00] NSEG
times
1336 ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1337 (2.60, 0.95)
1338 (12.60, 0.75)
1339 (14.10, 0.00)
1340 (15.90, 0.00)
1341 (17.40, 0.75)
1342 (27.40, 0.95)

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1343         (30.00, 1.70)
1344 *%-----|-----|
1345 CONTINUOUS NASHYD NHYD=["D2"], DT=[1]min, AREA=[2.28](ha), DWF=[0](cms), CN/C=[84],
IA=[9.0](mm),
1346 N=[3], TP=[0.99]hrs,
1347 Continuous simulation parameters:
1348 IaRECper=[4](hrs),
1349 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1350 InterEventTime=[12](hrs)
1351 Baseflow simulation parameters:
1352 BaseFlowOption=[1],
1353 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1354 VHydCond=[0.055](mm/hr), END=-1
1355 *%-----|-----|
1356 CONTINUOUS STANDHYD NHYD=["A17"], DT=[1]min, AREA=[12.04](ha), XIMP=[0.68],
TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
1357 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1358 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1359 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[490.714](m), MNI=[0.013], SCI=[0](min),
1360 Continuous simulation parameters:
1361 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1362 *%-----|-----|
1363 ROUTE RESERVOIR NHYDout=["A17STR"], NHYDin=["A17"], RDT=[1](min),
1364 TABLE of ( OUTFLOW-STORAGE ) values
1365 (cms) - (ha-m)
1366 [ 0.000 , 0.000 ]
1367 [ 0.169 , 0.185 ]
1368 [ 0.349 , 0.248 ]
1369 [ 0.482 , 0.283 ]
1370 [ 0.602 , 0.338 ]
1371 [ 0.771 , 0.359 ]
1372 [ 0.891 , 0.391 ]
1373 [ -1 , -1 ] (max twenty pts)
1374 NHYDovf=["A17OVF"]
1375 *%-----|-----|
1376 CONTINUOUS STANDHYD NHYD=["ST-4"], DT=[1]min, AREA=[0.35](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1377 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1378 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1379 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[83.666](m),
MNI=[0.013], SCI=[0](min),
1380 Continuous simulation parameters:
1381 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1382 *%-----|-----|
1383 ROUTE RESERVOIR NHYDout=["ST4STR"], NHYDin=["ST-4"], RDT=[1](min),
1384 TABLE of ( OUTFLOW-STORAGE ) values
1385 (cms) - (ha-m)
1386 [ 0.000 , 0.0000 ]
1387 [ 0.031 , 0.0010 ]
1388 [ 0.032 , 0.0050 ]
1389 [ -1 , -1 ] (max twenty pts)
1390 NHYDovf=["ST4OVF"]
1391 *%-----|-----|
1392 CONTINUOUS STANDHYD NHYD=["A18"], DT=[1]min, AREA=[5.30](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1393 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1394 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),

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1395      Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1396      LGI=[325.576](m), MNI=[0.013], SCI=[0](min),
1397      Continuous simulation parameters:
1397      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1397      END=-1
1398  *%-----|-----|
1399  ROUTE RESERVOIR  NHYDout=["A18STR"], NHYDin=["A18"], RDT=[1](min),
1400                   TABLE of ( OUTFLOW-STORAGE ) values
1401                   (cms) - (ha-m)
1402                   [ 0.000 , 0.000 ]
1403                   [ 0.074 , 0.082 ]
1404                   [ 0.154 , 0.109 ]
1405                   [ 0.212 , 0.125 ]
1406                   [ 0.265 , 0.149 ]
1407                   [ 0.339 , 0.158 ]
1408                   [ 0.392 , 0.172 ]
1409                   [ -1 , -1 ] (max twenty pts)
1410                   NHYDovf=["A18OVF"]
1411  *%-----|-----|
1412  *ANALYSIS POINT 3 - TOTAL FLOW AT OUTLET OF STREET 4
1413  *%-----|-----|
1414  ADD HYD          NHYDsum=["PT3ST4"], NHYDs to
1414  add=["DRAIN2"+"D2"+"A17STR"+"A17OVF"+"ST4STR"+"ST4OVF"+"A18STR"+"A18OVF"]
1415  *%-----|-----|
1416  *ROUTE FLOW through O'Keefe Drain 3
1417  ROUTE CHANNEL   NHYDout=["DRAIN3"], NHYDin=["PT3ST4"], RDT=[1](min),
1418                   CHLGTH=[525]{m}, CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1419                   SECNUM=[1], NSEG=[3]
1420                   ( SEGRROUGH, SEGDIST (m))=[0.07,12.50 -0.043,17.50 0.07,30.00] NSEG
1421                   times
1422                   ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1423                   (2.50, 1.00)
1424                   (12.50, 0.80)
1425                   (14.10, 0.00)
1426                   (15.90, 0.00)
1427                   (17.50, 0.80)
1428                   (27.50, 1.00)
1429                   (30.00, 1.70)
1429  *%-----|-----|
1430  CONTINUOUS NASHYD  NHYD=["D3"], DT=[1]min, AREA=[2.51](ha),
1431                   DWF=[0](cms), CN/C=[86], IA=[8.7](mm), N=[3], TP=[0.73]hrs,
1432                   Continuous simulation parameters:
1433                   IaREcper=[4](hrs),
1434                   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1435                   InterEventTime=[12](hrs)
1436                   Baseflow simulation parameters:
1437                   BaseFlowOption=[1] ,
1438                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1439                   VHydCond=[0.055](mm/hr), END=-1
1440  *%-----|-----|
1441  CONTINUOUS STANDHYD  NHYD=["C1"], DT=[1]min, AREA=[3.41](ha), XIMP=[0.68], TIMP=[0.85],
1442  DWF=[0](cms), LOSS=[1]:
1443                   Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1444                   F=[0.00](mm),
1445                   Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1446                   MNP=[0.250], SCP=[0](min),
1447                   Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1448                   LGI=[261.151](m), MNI=[0.013], SCI=[0](min),
1449                   Continuous simulation parameters:
1450                   IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1451                   END=-1
1451  *%-----|-----|
1452  ROUTE RESERVOIR  NHYDout=["C1-STR"], NHYDin=["C1"], RDT=[1](min),
1453                   TABLE of ( OUTFLOW-STORAGE ) values
1454                   (cms) - (ha-m)
1455                   [ 0.000 , 0.000 ]

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```

1452         [ 0.048 , 0.052 ]
1453         [ 0.099 , 0.070 ]
1454         [ 0.136 , 0.080 ]
1455         [ 0.170 , 0.096 ]
1456         [ 0.218 , 0.102 ]
1457         [ 0.252 , 0.111 ]
1458         [ -1 , -1 ] (max twenty pts)
1459         NHYDovf=["C1-OVF"]
1460 *%-----|-----|
1461 CONTINUOUS STANDHYD NHYD=["ST-5"], DT=[1]min, AREA=[0.45](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1462         Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1463         Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1464         Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[94.868](m),
MNI=[0.013], SCI=[0](min),
1465         Continuous simulation parameters:
1466         IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1467 *%-----|-----|
1468 ROUTE RESERVOIR NHYDout=["ST5STR"], NHYDin=["ST-5"], RDT=[1](min),
1469         TABLE of ( OUTFLOW-STORAGE ) values
1470         (cms) - (ha-m)
1471         [ 0.000 , 0.0000 ]
1472         [ 0.040 , 0.0010 ]
1473         [ 0.041 , 0.0062 ]
1474         [ -1 , -1 ] (max twenty pts)
1475         NHYDovf=["ST5OVF"]
1476 *%-----|-----|
1477 ADD HYD NHYDsum=["ST5-E"], NHYDs to
add=["DRAIN3"+"D3"+"C1-STR"+"C1-OVF"+"ST5STR"+"ST5OVF"]
1478 *%-----|-----|
1479 CONTINUOUS STANDHYD NHYD=["STRAND"], DT=[1](min), AREA=[7.59](ha),
1480 XIMP=[0.64], TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
1481         Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1482         Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
1483         Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[1230](m),
MNI=[0.013], SCI=[0](min),
1484         Continuous simulation parameters:
1485         IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1486 *%-----|-----|
1487 ROUTE RESERVOIR NHYDout=["S-POND"], NHYDin=["STRAND"], RDT=[1](min),
1488         TABLE of ( OUTFLOW-STORAGE ) values
1489         (cms) - (ha-m)
1490         [ 0.000 , 0.000 ]
1491         [ 0.033 , 0.188 ]
1492         [ 0.057 , 0.253 ]
1493         [ 0.104 , 0.287 ]
1494         [ 0.160 , 0.336 ]
1495         [ 0.340 , 0.346 ]
1496         [ 0.471 , 0.360 ]
1497         [ 0.824 , 0.390 ]
1498         [ -1 , -1 ] (max twenty pts)
1499         NHYDovf=["S-OVF"]
1500 *%-----|-----|
1501 ADD HYD NHYDsum=["SSAOUT"], NHYDs to add=["ST5-E"+"S-POND"+"S-OVF"]
1502 *%-----|-----|
1503 SAVE HYD NHYD=["SSAOUT"], # OF PCYCLES=[5], ICASEsh=[1]
1504         HYD_COMMENT=["SSAOUT"]
1505 *%-----|-----|
1506 CONTINUOUS STANDHYD NHYD=["Area-A"], DT=[1]min, AREA=[66.75](ha), XIMP=[0.64],
TIMP=[0.80], DWF=[0](cms), LOSS=[1]:

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1507 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1508 F=[0.00](mm),
1509 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1510 MNP=[0.250], SCP=[0](min),
1511 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1512 LGI=[1155.422](m), MNI=[0.013], SCI=[0](min),
1513 Continuous simulation parameters:
1514 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1515 END=-1
1516 *%-----|-----|
1517 SAVE HYD NHYD=["Area-A"], # OF PCYCLES=[1], ICASEsh=[1]
1518 HYD_COMMENT=["SMWF-A Inflow"]
1519 *%-----|-----|
1520 ROUTE RESERVOIR NHYDout=["SWMF-A"], NHYDin=["Area-A"], RDT=[1](min),
1521 TABLE of ( OUTFLOW-STORAGE ) values
1522 (cms) - (ha-m)
1523 [ 0.000 , 0.000 ]
1524 [ 0.103 , 1.077 ]
1525 [ 0.128 , 1.749 ]
1526 [ 0.382 , 2.282 ]
1527 [ 0.703 , 2.582 ]
1528 [ 1.256 , 2.978 ]
1529 [ 1.567 , 3.202 ]
1530 [ 1.955 , 3.493 ]
1531 [ 2.100 , 3.600 ]
1532 [ -1 , -1 ] (max twenty pts)
1533 NHYDovf=["SWMAOV"]
1534 *%-----|-----|
1535 SAVE HYD NHYD=["SWMF-A"], # OF PCYCLES=[1], ICASEsh=[1]
1536 HYD_COMMENT=["SMWF-A Outflow"]
1537 *%-----|-----|
1538 *ANALYSIS POINT 4 - TOTAL FLOW AT OUTLET OF STREET 5
1539 *%-----|-----|
1540 ADD HYD NHYDsum=["PT4ST5"], NHYDs to add=["SSAOUT"+"SWMF-A"+"SWMAOV"]
1541 *%-----|-----|
1542 CONTINUOUS STANDHYD NHYD=["C6"], DT=[1]min, AREA=[1.87](ha), XIMP=[0.68], TIMP=[0.85],
1543 DWF=[0](cms), LOSS=[1]:
1544 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1545 F=[0.00](mm),
1546 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1547 MNP=[0.250], SCP=[0](min),
1548 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1549 LGI=[193.391](m), MNI=[0.013], SCI=[0](min),
1550 Continuous simulation parameters:
1551 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1552 END=-1
1553 *%-----|-----|
1554 ROUTE RESERVOIR NHYDout=["C6-STR"], NHYDin=["C6"], RDT=[1](min),
1555 TABLE of ( OUTFLOW-STORAGE ) values
1556 (cms) - (ha-m)
1557 [ 0.000 , 0.000 ]
1558 [ 0.026 , 0.029 ]
1559 [ 0.054 , 0.038 ]
1560 [ 0.075 , 0.044 ]
1561 [ 0.093 , 0.052 ]
1562 [ 0.120 , 0.056 ]
1563 [ 0.138 , 0.061 ]
1564 [ -1 , -1 ] (max twenty pts)
1565 NHYDovf=["C6-OVF"]
1566 *%-----|-----|
1567 CONTINUOUS STANDHYD NHYD=["C7"], DT=[1]min, AREA=[1.62](ha), XIMP=[0.68], TIMP=[0.85],
1568 DWF=[0](cms), LOSS=[1]:
1569 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1570 F=[0.00](mm),
1571 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1572 MNP=[0.250], SCP=[0](min),

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1561      Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1562      LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
1563      Continuous simulation parameters:
1564      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1565      END=-1
1566
1567 *%-----|-----|
1568 ROUTE RESERVOIR NHYDout=["C7-STR"], NHYDin=["C7"], RDT=[1](min),
1569      TABLE of ( OUTFLOW-STORAGE ) values
1570      (cms) - (ha-m)
1571      [ 0.000 , 0.000 ]
1572      [ 0.023 , 0.025 ]
1573      [ 0.047 , 0.033 ]
1574      [ 0.065 , 0.038 ]
1575      [ 0.081 , 0.045 ]
1576      [ 0.104 , 0.048 ]
1577      [ 0.120 , 0.053 ]
1578      [ -1 , -1 ] (max twenty pts)
1579      NHYDovf=["C7-OVF"]
1580
1581 *%-----|-----|
1582 CONTINUOUS STANDHYD NHYD=["ST-6"], DT=[1]min, AREA=[0.41](ha), XIMP=[0.46], TIMP=[0.57],
1583 DWF=[0](cms), LOSS=[1]:
1584      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1585      F=[0.00](mm),
1586      Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1587      MNP=[0.250], SCP=[0](min),
1588      Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[90.554](m),
1589      MNI=[0.013], SCI=[0](min),
1590      Continuous simulation parameters:
1591      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1592      END=-1
1593
1594 *%-----|-----|
1595 ROUTE RESERVOIR NHYDout=["ST6STR"], NHYDin=["ST-6"], RDT=[1](min),
1596      TABLE of ( OUTFLOW-STORAGE ) values
1597      (cms) - (ha-m)
1598      [ 0.000 , 0.0000 ]
1599      [ 0.036 , 0.0010 ]
1600      [ 0.037 , 0.0058 ]
1601      [ -1 , -1 ] (max twenty pts)
1602      NHYDovf=["ST6OVF"]
1603
1604 *%-----|-----|
1605 *ANALYSIS POINT 5 - TOTAL FLOW AT OUTLET OF STREET 6
1606 *%-----|-----|
1607 ADD HYD NHYDsum=["PT5ST6"], NHYDs to
1608 add=["PT4ST5"+"C6-STR"+"C6-OVF"+"C7-STR"+"C7-OVF"+"ST6STR"+"ST6OVF"]
1609 *%-----|-----|
1610 *ROUTE FLOW through O'Keefe Drain 4
1611 ROUTE CHANNEL NHYDout=["DRAIN4"], NHYDin=["PT5ST6"], RDT=[1](min),
1612      CHLGTH=[324]{m}, CHSLOPE=[.10](%), FPSLOPE=[.10](%),
1613      SECNUM=[1], NSEG=[3]
1614      ( SEGROUGH, SEGDIST (m))=[0.07,12.00 -0.043,18.00 0.07,30.00] NSEG
1615      times
1616      ( DISTANCE (m), ELEVATION (m))=[0.00, 2.00]
1617      (2.00, 1.20)
1618      (12.00, 1.00)
1619      (14.00, 0.00)
1620      (16.00, 0.00)
1621      (18.00, 1.00)
1622      (28.00, 1.20)
1623      (30.00, 2.00)
1624
1625 *%-----|-----|
1626 CONTINUOUS NASHYD NHYD=["D4"], DT=[1]min, AREA=[1.73](ha), DWF=[0](cms), CN/C=[88],
1627 IA=[8.4](mm),
1628      N=[3], TP=[0.60]hrs,
1629      Continuous simulation parameters:
1630      IaREcper=[4](hrs),
1631      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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1617 InterEventTime=[12](hrs)
1618 Baseflow simulation parameters:
1619 BaseFlowOption=[1] ,
1620 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
1621 VHydCond=[0.055](mm/hr) , END=-1
1622 *%-----|-----|
1623 CONTINUOUS STANDHYD NHYD=["Area-B"] , DT=[1]min , AREA=[24.04](ha) , XIMP=[0.62] ,
TIMP=[0.77] , DWF=[0](cms) , LOSS=[1]:
1624 Horton: Fo=[76.20](mm/hr) , Fc=[13.20](mm/hr) , DCAY=[4.14](/hr) ,
F=[0.00](mm) ,
1625 Pervious areas: IAper=[4.67](mm) , SLPP=[1.4](%) , LGP=[50](m) ,
MNP=[0.250] , SCP=[0](min) ,
1626 Impervious areas: IAimp=[1.57](mm) , SLPI=[1.4](%) ,
LGI=[693.397](m) , MNI=[0.013] , SCI=[0](min) ,
1627 Continuous simulation parameters:
1628 IaREcper=[4](hrs) , IaREcimp=[4](hrs) , InterEventTime=[12](hrs) ,
END=-1
1629 *%-----|-----|
1630 ROUTE RESERVOIR NHYDout=["SWMF-B"] , NHYDin=["Area-B"] , RDT=[1](min) ,
1631 TABLE of ( OUTFLOW-STORAGE ) values
1632 (cms) - (ha-m)
1633 [ 0.000 , 0.000 ]
1634 [ 0.025 , 0.090 ]
1635 [ 0.175 , 0.510 ]
1636 [ 0.350 , 0.710 ]
1637 [ 0.495 , 0.820 ]
1638 [ 0.648 , 0.980 ]
1639 [ 0.965 , 1.045 ]
1640 [ 1.072 , 1.140 ]
1641 [ -1 , -1 ] (max twenty pts)
1642 NHYDovf=["SWMBOVF"]
1643 *%-----|-----|
1644 ADD HYD NHYDsum=["D4-EX"] , NHYDs to add=["DRAIN4"+"D4"+"SWMF-B"+"SWMBOVF"]
1645 *%-----|-----|
1646 *ROUTE FLOW THROUGH O'Keefe Drain 5
1647 * JFSA: Nov. 2020, added en points to close X-Section
1648 ROUTE CHANNEL NHYDout=["DRAIN5"] , NHYDin=["D4-EX"] , RDT=[1](min) ,
1649 CHLGTH=[413.0](m) , CHSLOPE=[0.16](%) , FPSLOPE=[0.16](%) ,
1650 SECNUM=[1] , NSEG=[3]
1651 ( SEGROUGH , SEGDIST (m))=[0.043,12.29 -0.033,17.97
1652 0.043,32.84] NSEG times
1653 ( DISTANCE (m) , ELEVATION (m))=(-0.01 , 2.50)
1654 [0.00 , 1.41]
1655 [6.13 , 0.97]
1656 [12.29 , 0.89]
1657 [15.71 , 0.00]
1658 [17.97 , 0.39]
1659 [23.04 , 0.35]
1660 [32.83 , 0.96]
1661 [32.84 , 2.50]
1662 *%-----|-----|
1663 CONTINUOUS NASHYD NHYD=["D5"] , DT=[1]min , AREA=[1.90](ha) ,
1664 DWF=[0](cms) , CN/C=[86] , IA=[8.7](mm) , N=[3] , TP=[0.69]hrs ,
1665 Continuous simulation parameters:
1666 IaREcper=[4](hrs) ,
1667 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
1668 InterEventTime=[12](hrs)
1669 Baseflow simulation parameters:
1670 BaseFlowOption=[1] ,
1671 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
1672 VHydCond=[0.055](mm/hr) , END=-1
1673 *%-----|-----|
1674 *EXTERNAL FLOWS SOUTHEAST OF THE SITE NORTH OF McKENNA CASEY DR.
1675 CONTINUOUS NASHYD NHYD=["O-13SDF"] , DT=[1]min , AREA=[9.74](ha) ,
1676 DWF=[0](cms) , CN/C=[81] , IA=[4.0](mm) , N=[3] , TP=[.43]hrs ,
1677 Continuous simulation parameters:

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1678 IaREcper=[4](hrs),
1679 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1680 InterEventTime=[12](hrs)
1681 Baseflow simulation parameters:
1682 BaseFlowOption=[1] ,
1683 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1684 VHydCond=[0.055](mm/hr), END=-1
1685 *%-----|-----
1686 *SNOW DISPOSAL FACILITY
1687 *PARAMETERS BASED ON ROBINSON 2006 MODEL
1688 ROUTE RESERVOIR NHYDout=["SDF"], NHYDin=["O-13SDF"], RDT=[1](min),
1689 TABLE of ( OUTFLOW-STORAGE ) values
1690 (cms) - (ha-m)
1691 [0.000,0.000]
1692 [0.150,0.600]
1693 (0.200,1.500)
1694 [ -1 , -1 ] (max twenty pts)
1695 NHYDovf=["OVFSDF"]
1696 *%-----|-----
1697 *ANALYSIS POINT 6 - McKenna Casey Dr.
1698 *%-----|-----
1699 ADD HYD NHYDsum=["PT6MC"], NHYDs to add=["DRAIN5"+"D5"+"SDF"]
1700 *%-----|-----
1701 CONTINUOUS NASHYD NHYD=["O-15"], DT=[1]min, AREA=[10.67](ha),
1702 DWF=[0](cms), CN/C=[82], IA=[7.5](mm), N=[3], TP=[0.30]hrs,
1703 Continuous simulation parameters:
1704 IaREcper=[4](hrs),
1705 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1706 InterEventTime=[12](hrs)
1707 Baseflow simulation parameters:
1708 BaseFlowOption=[1] ,
1709 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1710 VHydCond=[0.055](mm/hr), END=-1
1711 *%-----|-----
1712 *TOTAL FLOW NORTH OF McKENNA CASEY DR.
1713 ADD HYD NHYDsum=["M-C"], NHYDs to add=["PT6MC"+"O-15"]
1714 *%-----|-----
1715 *ROUTE FLOW THROUGH AREA O-14
1716 * JFSA: Nov. 2020, added end points to close X-section
1717 ROUTE CHANNEL NHYDout=["O-14Ch"], NHYDin=["M-C"], RDT=[1](min),
1718 CHLGTH=[845.3](m), CHSLOPE=[0.10](%), FPSLOPE=[0.10](%),
1719 SECNUM=[1], NSEG=[3]
1720 ( SEGROUGH, SEGDIST (m))=[0.06,15.00 -0.033,18.04 0.06,31.85] NSEG
times
1721 ( DISTANCE (m), ELEVATION (m))=[-0.01, 2.5
1722 (0.00, 1.53]
1723 (5.56, 1.47)
1724 (9.21, 1.45)
1725 (12.45, 1.53)
1726 (13.70, 1.50)
1727 (15.00, 0.69)
1728 (15.34, 0.00)
1729 (16.51, 0.05)
1730 (17.30, 0.17)
1731 (18.04, 0.74)
1732 (19.29, 1.32)
1733 (22.73, 1.47)
1734 (31.84, 1.41)
1735 (31.85, 2.50)
1736 *%-----|-----
1737 *% -Change O-14 from NASHYD to STANDHYD, name it "S-1-Okeefe" and add it to S-1
subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
1738 *% -JFSA 2021-02-16, add detailed subcatchment drainage area for each subcatchment
in Corrigan sub-catchment. After adding part of O-14 to S_1 sub-catchment so O-14
becomes 5 ha instead of 30.02 ha and TP becomes 0.133 (5*0.8/30.02) instead of 0.8
1739 CONTINUOUS NASHYD NHYD=["O-14"], DT=[1]min, AREA=[5](ha),

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1740 DWF=[0](cms), CN/C=[82], IA=[7.5](mm), N=[3], TP=[0.133]hrs,
1741 Continuous simulation parameters:
1742 IaREcper=[4](hrs),
1743 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1744 InterEventTime=[12](hrs)
1745 Baseflow simulation parameters:
1746 BaseFlowOption=[1] ,
1747 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1748 VHydCond=[0.055](mm/hr), END=-1
1749 *
1750 *%-----|-----|
1751 *ANALYSIS POINT 7 - JOCK RIVER
1752 * 2020-12-01 To Foster Drain
1753 * 2020-12-01 replace ("PT7JR") by ("OKEEFE")
1754 *%-----|-----|
1755 ADD HYD NHYDsum=["OKEEFE"], NHYDs to add=["O-14Ch"+"O-14"]
1756 *%-----|-----|
1757 *CONTINUOUS STANDHYD NHYD=["OKEEFE"], DT=[1](min), AREA=[448](ha),
1758 * XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
1759 * SCS curve number CN=[77],
1760 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1761 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
1762 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1763 * LGI=[1728](m), MNI=[0.013], SCI=[0](min),
1764 * Continuous simulation parameters:
1765 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
1766 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1767 * InterEventTime=[18](hrs), END=-1
1768 *#*****
1769 *# Okeefe Pond
1770 *# - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1771 *# and a ratio of the catchment area to the West Clarke pond rating curve
1772 *# from the MSS for the next coordinates
1773 *#*****
1774 *ROUTE RESERVOIR NHYDout=["P_OKE"], NHYDin=["OKEEFE"],
1775 * RDT=[1](min),
1776 * TABLE of ( OUTFLOW-STORAGE ) values
1777 * (cms) - (ha-m)
1778 * [ 0.0 , 0.0]
1779 * [ 14.13 , 13.0]
1780 * [ -1 , -1 ] (maximum one hundred pairs of points)
1781 * NHYDovf=["ok-OVF"],
1782 *%-----|-----|
1783 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
1784 * moved to drain before station 6215 on Jock River
1785 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XIMP=[0.65],
1786 * TIMP=[0.65], DWF=[0](cms),
1787 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1788 * IAper=[4.67](mm), SLPP=[2.0](%),
1789 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1790 * IAimp=[1.57](mm), SLPI=[0.75](%),
1791 * LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
1792 *
1793 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
1794 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1795 * InterEventTime=[12](hrs), END=-1
1796 *%-----|-----|
1797 CONTINUOUS NASHYD NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
1798 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1799 * N=[3], TP=[1.120]hrs,
1800 * Continuous simulation parameters:
1801 * IaREcper=[4](hrs),
1802 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1803 * InterEventTime=[12](hrs)
1804 * Baseflow simulation parameters:

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1801 BaseFlowOption=[1] ,
1802 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1803 VHydCond=[0.055](mm/hr), END=-1
1804 *%-----|-----|
1805 *COMPUTE DUALHYD NHYDin=["S-1-D2"], CINLET=[2.062](cms), NINLET=[1],
1806 * MajNHYD=["S-1-D2J"]
1807 * MinNHYD=["S-1-D2N"]
1808 * TMJSTO=[9999999](cu-m)
1809 *%-----|-----|
1810 *ADD HYD NHYDsum=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
1811 *%-----|-----|
1812 *ROUTE RESERVOIR NHYDout=["S-1-D2R"],NHYDin=["S-1-D2S"] ,
1813 * RDT=[1](min),
1814 * TABLE of ( OUTFLOW-STORAGE ) values
1815 * (cms) - (ha-m)
1816 * [ 0.0 , 0.0 ]
1817 * [ 0.2231, 0.7445 ]
1818 * [ -1 , -1 ] (max twenty pts)
1819 * NHYDovf=["S-1-D2Rovf"]
1820 *%-----|-----|
1821 *CONTINUOUS STANDHYD NHYD=["S-1-D3"], DT=[1](min), AREA=[6.79](ha), XIMP=[0.65],
1822 * TIMP=[0.65], DWF=[0](cms),
1823 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1824 * IAper=[4.67](mm), SLPP=[2.0](%),
1825 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1826 * IAimp=[1.57](mm), SLPI=[0.75](%),
1827 * LGI=[212.760](m), MNI=[0.013], SCI=[0](min),
1828 * Continuous simulation parameters:
1829 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
1830 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1831 * InterEventTime=[12](hrs), END=-1
1832 *%-----|-----|
1833 CONTINUOUS NASHYD NHYD=["S-1-D3"], DT=[1]min, AREA=[6.79](ha),
1834 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1835 * N=[3], TP=[1.281]hrs,
1836 * Continuous simulation parameters:
1837 * IaREcper=[4](hrs),
1838 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1839 * InterEventTime=[12](hrs)
1840 * Baseflow simulation parameters:
1841 * BaseFlowOption=[1] ,
1842 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1843 * VHydCond=[0.055](mm/hr), END=-1
1844 *%-----|-----|
1845 *COMPUTE DUALHYD NHYDin=["S-1-D3"], CINLET=[0.719](cms), NINLET=[1],
1846 * MajNHYD=["S-1-D3J"]
1847 * MinNHYD=["S-1-D3N"]
1848 * TMJSTO=[9999999](cu-m)
1849 *%-----|-----|
1850 *ADD HYD NHYDsum=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
1851 *%-----|-----|
1852 *ROUTE RESERVOIR NHYDout=["S-1-D3R"],NHYDin=["S-1-D3S"] ,
1853 * RDT=[1](min),
1854 * TABLE of ( OUTFLOW-STORAGE ) values
1855 * (cms) - (ha-m)
1856 * [ 0.0 , 0.0 ]
1857 * [ 0.0811, 0.2708 ]
1858 * [ -1 , -1 ] (max twenty pts)
1859 * NHYDovf=["S-1-D3Rovf"]
1860 *%-----|-----|
1861 ADD HYD NHYDsum=["SN_OK"], NHYDs to add=["N_OK"+"OKEEFE"+"S-1-D2"+"S-1-D3"]
1862 *%-----|-----|
1863 SAVE HYD NHYD=["SN_OK"], # OF PCYCLES=[-1], ICASEsh=[1]
1864 * HYD_COMMENT=["Total Flows at Okeefe Drain"]
1865 *%-----|-----|
1866 *#

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1864  *# Hydrograph from Node Okeefe routed to Node at Foster Drain
1865  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
1866  *#
1867  ROUTE CHANNEL      NHYDout=["N_FO"] ,NHYDin=["SN_OK"] ,
1868                    RDT=[1](min),
1869                    CHLGTH=[1183](m),  CHSLOPE=[0.0761](%),
1870                    FPSLOPE=[0.0761](%),
1871                    SECNUM=[1.0],      NSEG=[3]
1872                    ( SEGRROUGH, SEGDIST (m))=
1873                      [0.050,-33.89
1874                      -0.035,31.59
1875                      0.050,34.41] NSEG times
1876                    ( DISTANCE (m), ELEVATION (m))=
1877                    [-794.18, 91.00]
1878                    [-775.41, 91.50]
1879                    [-702.63, 91.50]
1880                    [-546.19, 91.50]
1881                    [-529.54, 91.50]
1882                    [-323.44, 91.00]
1883                    [-320.71, 91.00]
1884                    [-183.59, 91.00]
1885                    [-182.54, 90.50]
1886                    [-181.36, 90.00]
1887                    [-177.37, 90.00]
1888                    [-87.70, 90.00]
1889                    [-33.89, 90.00]
1890                    [-18.52, 86.88]
1891                    [0.00,85.20]
1892                    [16.20, 86.83]
1893                    [31.59, 90.00]
1894                    [33.03, 90.50]
1895                    [34.41, 91.00]
1896  *%-----|-----|
1897  *#*****
1898  *#   Catchment FOSTER
1899  *#   - To Foster ditch (north of the Jock)
1900  *#   - Partially developed (medium density); remaining agricultural
1901  *#   - 2020-12-01 JFSA Foster area is 332 as per Foster SWMF Environmental Study
1902  *#   - 2020-12-01 decrease Foster drainage area from (373 HA) to (307.98 HA) after
1903  *#   - 2021-02-12 update Foster area to 325.44 ha as measured from QGIS
1904  *#*****
1905  CONTINUOUS STANDHYD NHYD=["FOSTER"], DT=[1]min, AREA=[325.44](ha),
1906                    XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
1907                    SCS curve number CN=[74],
1908                    Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1909                    LGP=[40](m), MNP=[0.25], SCP=[0](min),
1910                    Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1911                    LGI=[1472.956](m), MNI=[0.013], SCI=[0](min),
1912                    Continuous simulation parameters:
1913                    IaRECper=[4](hrs), IaRECimp=[4](hrs),
1914                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1915                    InterEventTime=[18](hrs), END=-1
1916  *#*****
1917  *#   Foster Pond
1918  *#   - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1919  *#   and a ratio of the catchment area to the West Clarke pond rating curve
1920  *#   from the MSS for the next coordinates
1921  *#*****
1922  ROUTE RESERVOIR    NHYDout=["P_FOS"], NHYDin=["FOSTER"],
1923                    RDT=[1](min),
1924                    TABLE of ( OUTFLOW-STORAGE ) values
1925                    (cms) - (ha-m)
1926                    [ 0.0 , 0.0 ]

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1927         [ 10.34 , 10]
1928         [ -1 , -1 ] (max twenty pts)
1929         NHYDovf=["FO-OVF"]
1930 *%-----|-----|
1931 ADD HYD          NHYDsum=["FOSTER-OUT"], NHYDs to add=["P_FOS"+"FO-OVF"]
1932 *%-----|-----|
1933 *#*****
1934 *   -Brazeau area from P 1800-19 =[71.751], change to 63.59 ha based on GIS measurements
1935 *   -JFSA, 2021-01-19 update "W_CLAR_BRAZ" to 73.29 ha based on GIS measurements
1936 *   -JFSA, 2021-01-22 Brazeau ("MS_P10"+"P10-OVF")brazeau pond discharges directly
to the jock river through a road side ditch on the west side of Borrisokane road
(station 6016)
1937 CONTINUOUS STANDHYD NHYD=["W_CLAR_BRAZ"], DT=[1]min, AREA=[73.29](ha),
1938 XIMP=[0.6], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
1939 SCS curve number CN=[77],
1940 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
1941 LGP=[40](m), MNP=[0.25], SCP=[0](min),
1942 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1943 LGI=[699.00](m), MNI=[0.013], SCI=[0](min),
1944 Continuous simulation parameters:
1945 IaRECper=[4](hrs), IaRECimp=[4](hrs),
1946 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1947 InterEventTime=[18](hrs), END=-1
1948 *%-----|-----|
1949 * 2020-12-01 correct pond curve values
1950 ROUTE RESERVOIR   NHYDout=["MS_P10"], NHYDin=["W_CLAR_BRAZ"],
1951 RDT=[1](min),
1952         TABLE of ( OUTFLOW-STORAGE ) values
1953         (cms) - (ha-m)
1954         [ 0.0 , 0.0 ]
1955         [ 0.068 , 0.001 ]
1956         [ 0.271 , 0.022 ]
1957         [ 0.379 , 0.051 ]
1958         [ 0.48 , 0.091 ]
1959         [ 0.853 , 0.341 ]
1960         [ 1.005 , 0.61 ]
1961         [ 1.128 , 1.231 ]
1962         [ 1.155 , 1.592 ]
1963         [ 1.194 , 1.876 ]
1964         [ 1.2 , 1.921 ]
1965         [ 1.259 , 2.369 ]
1966         [ 1.3 , 2.665 ]
1967         [ 1.349 , 2.813 ]
1968         [ -1 , -1 ] (max twenty pts)
1969         NHYDovf=["P10-OVF"]
1970 *%-----|-----|
1971 *   -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
1972 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
1973 * XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
1974 * SCS curve number CN=[74],
1975 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1976 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
1977 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1978 * LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
1979 * Continuous simulation parameters:
1980 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
1981 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1982 * InterEventTime=[18](hrs), END=-1
1983 *%-----|-----|
1984 CONTINUOUS NASHYD  NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
1985 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1986 N=[3], TP=[1.10]hrs,
1987 Continuous simulation parameters:
1988 IaRECper=[4](hrs),
1989 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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1990 InterEventTime=[12](hrs)
1991 Baseflow simulation parameters:
1992 BaseFlowOption=[1] ,
1993 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
1994 VHydCond=[0.055](mm/hr) , END=-1
1995 *%-----|-----|
1996 *COMPUTE DUALHYD NHYDin=["S-1-FO-D2"] , CINLET=[0.508](cms) , NINLET=[1] ,
1997 * MajNHYD=["S-1-FO-D2J"]
1998 * MinNHYD=["S-1-FO-D2N"]
1999 * TMJSTO=[9999999](cu-m)
2000 *%-----|-----|
2001 *ADD HYD NHYDsum=["S-1-FO-D2S"] , NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]
2002 *%-----|-----|
2003 *ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"] ,NHYDin=["S-1-FO-D2S"] ,
2004 * RDT=[1](min) ,
2005 * TABLE of ( OUTFLOW-STORAGE ) values
2007 * [ 0.0 , 0.0 ]
2008 * [ 0.0590 , 0.1970 ]
2009 * [ -1 , -1 ] (max twenty pts)
2010 * NHYDovf=["S-1FOD2ovf"]
2011 *%-----|-----|
2012 ADD HYD NHYDsum=["980"] , NHYDs to add=["FOSTER-OUT"+"S-1-FO-D2"]
2013 *%-----|-----|
2014 SAVE HYD NHYD=["980"] , # OF PCYCLES=[-1] , ICASEsh=[1]
2015 HYD_COMMENT=["Total Flows at Station 980 on Foster Drain"]
2016 *%-----|-----|
2017 *#
2018 *# Hydrograph from Node Foster SWM (Station 980)to Node at station 520
2019 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 980
2020 *#
2021 ROUTE CHANNEL NHYDout=["980-out"] ,NHYDin=["980"] ,
2022 RDT=[1](min) ,
2023 CHLGTH=[460](m) , CHSLOPE=[0.04348](%) ,
2024 FPSLOPE=[0.04348](%) ,
2025 SECNUM=[1.0] , NSEG=[3]
2026 ( SEGROUGH , SEGDIST (m))=
2027 [0.050,45.90
2028 -0.035,53.30
2029 0.050,100] NSEG times
2030 ( DISTANCE (m) , ELEVATION (m))=
2031 [0 , 91.75 ]
2032 [42.4 , 92.18 ]
2033 [43.5 , 92.16 ]
2034 [44.1 , 92.1 ]
2035 [44.6 , 92 ]
2036 [44.8 , 91.86 ]
2037 [45.9 , 91.04 ]
2038 [46.4 , 90.65 ]
2039 [46.8 , 90.36 ]
2040 [47.9 , 90.32 ]
2041 [48.7 , 90.35 ]
2042 [50.7 , 90.33 ]
2043 [52.2 , 90.38 ]
2044 [52.5 , 90.59 ]
2045 [53.3 , 91.28 ]
2046 [54 , 91.83 ]
2047 [54.3 , 92 ]
2048 [54.8 , 92.08 ]
2049 [55.4 , 92.12 ]
2050 [100 , 91.84 ]
2051 *%-----|-----|
2052 * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2053 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D1"] , DT=[1]min , AREA=[5.11](ha) ,
2054 * XIMP=[0.65] , TIMP=[0.65] , DWF=[0](cms) , LOSS=[2] ,

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2055 *           SCS curve number CN=[74],
2056 *           Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2057 *           LGP=[40](m), MNP=[0.25], SCP=[0](min),
2058 *           Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2059 *           LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2060 *           Continuous simulation parameters:
2061 *           IaREcper=[4](hrs), IaREcimp=[4](hrs),
2062 *           SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2063 *           InterEventTime=[18](hrs), END=-1
2064 *%-----|-----|
2065 CONTINUOUS NASHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2066 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2067 N=[3], TP=[1.10]hrs,
2068 Continuous simulation parameters:
2069 IaREcper=[4](hrs),
2070 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2071 InterEventTime=[12](hrs)
2072 Baseflow simulation parameters:
2073 BaseFlowOption=[1] ,
2074 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2075 VHydCond=[0.055](mm/hr), END=-1
2076 *%-----|-----|
2077 *COMPUTE DUALHYD NHYDin=["S-1-FO-D1"], CINLET=[0.605](cms), NINLET=[1],
2078 *           MajNHYD=["S-1-FO-D1J"]
2079 *           MinNHYD=["S-1-FO-D1N"]
2080 *           TMJSTO=[9999999](cu-m)
2081 *%-----|-----|
2082 *ADD HYD NHYDsum=["S-1-FO-D1S"], NHYDs to add=["S-1-FO-D1N"+"S-1-FO-D1J"]
2083 *%-----|-----|
2084 *ROUTE RESERVOIR NHYDout=["S-1-FO-D1R"] ,NHYDin=["S-1-FO-D1S"] ,
2085 *           RDT=[1](min),
2086 *           TABLE of ( OUTFLOW-STORAGE ) values
2087 *           (cms) - (ha-m)
2088 *           [ 0.0      , 0.0 ]
2089 *           [ 0.0611, 0.2038 ]
2090 *           [ -1     , -1     ] (max twenty pts)
2091 *           NHYDovf=["S-1FODlovf"]
2092 *%-----|-----|
2093 ADD HYD NHYDsum=["520"], NHYDs to add=["980-out"+"S-1-FO-D1"]
2094 *%-----|-----|
2095 SAVE HYD NHYD=["520"], # OF PCYCLES=[-1], ICASEsh=[1]
2096 HYD_COMMENT=["Total Flows at Sation 520 on Foster Drain"]
2097 *%-----|-----|
2098 *# Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock
River)
2099 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 520
2100 *#
2101 ROUTE CHANNEL NHYDout=["520-out"] ,NHYDin=["520"] ,
2102 RDT=[1](min),
2103 CHLGTH=[860](m), CHSLOPE=[0.5872](%),
2104 FPSLOPE=[0.5872](%),
2105 SECNUM=[1.0], NSEG=[3]
2106 ( SEGROUGH, SEGDIST (m))=
2107 [0.050,45.90
2108 -0.035,54.3
2109 0.050,100.1097] NSEG times
2110 ( DISTANCE (m), ELEVATION (m))=
2111 [0, 91.26 ]
2112 [44.9, 91.46 ]
2113 [45.1, 91.37 ]
2114 [45.9, 90.84 ]
2115 [47, 90.32 ]
2116 [47.5, 90.22 ]
2117 [48, 90.17 ]
2118 [50.7, 90.19 ]
2119 [51.5, 90.17 ]

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2120 [52.2, 90.13 ]
2121 [52.7, 90.12 ]
2122 [53.3, 90.14 ]
2123 [53.5, 90.31 ]
2124 [53.9, 90.59 ]
2125 [54.3, 90.87 ]
2126 [54.7, 91.04 ]
2127 [55.3, 91.24 ]
2128 [55.5, 91.26 ]
2129 [63.7, 91.37 ]
2130 [100.1097, 91.43 ]
2131 *%-----|-----|
2132 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2133 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2134 * XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2135 * SCS curve number CN=[74],
2136 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2137 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
2138 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2139 * LGI=[315.806](m), MNI=[0.013], SCI=[0](min),
2140 * Continuous simulation parameters:
2141 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
2142 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2143 * InterEventTime=[18](hrs), END=-1
2144 *%-----|-----|
2145 CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2146 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2147 N=[3], TP=[1.007]hrs,
2148 Continuous simulation parameters:
2149 IaREcper=[4](hrs),
2150 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2151 InterEventTime=[12](hrs)
2152 Baseflow simulation parameters:
2153 BaseFlowOption=[1],
2154 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2155 VHydCond=[0.055](mm/hr), END=-1
2156 *%-----|-----|
2157 *COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CINLET=[1.749](cms), NINLET=[1],
2158 * MajNHYD=["S-1FO-F-DJ"]
2159 * MinNHYD=["S-1FO-F-DN"]
2160 * TMJSTO=[9999999](cu-m)
2161 *%-----|-----|
2162 *ADD HYD NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2163 *%-----|-----|
2164 *ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"],NHYDin=["S-1FO-F-DS"],
2165 * RDT=[1](min),
2166 * TABLE of ( OUTFLOW-STORAGE ) values
2167 * (cms) - (ha-m)
2168 * [ 0.0 , 0.0 ]
2169 * [ 0.1788, 0.5966 ]
2170 * [ -1 , -1 ] (max twenty pts)
2171 * NHYDovf=["S-1FoFDovf"]
2172 *%-----|-----|
2173 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2174 * -JFSA 2021-03-02 "S-1-D8" is Borrisokane Rd. so it will remain STANDHYD in all
scenarios
2175 CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1](min), AREA=[5.27](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2176 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAPER=[4.67](mm), SLPP=[2.0](%),
2177 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2178 LGI=[187.439](m), MNI=[0.013], SCI=[0](min),
2179 Continuous simulation parameters:

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2180 IaREcper=[4](hrs), IaREcimp=[4](hrs),
2181 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2182 InterEventTime=[12](hrs), END=-1
2183 *%-----|-----
2184 * This is a road so it is always STANDHYD
2185 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha),
2186 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2187 * N=[3], TP=[1.10]hrs,
2188 * Continuous simulation parameters:
2189 * IaREcper=[4](hrs),
2190 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2191 * InterEventTime=[12](hrs)
2192 * Baseflow simulation parameters:
2193 * BaseFlowOption=[1] ,
2194 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2195 * VHydCond=[0.055](mm/hr), END=-1
2196 *%-----|-----
2197 *COMPUTE DUALHYD NHYDin=["S-1-D8"], CINLET=[2.279](cms), NINLET=[1],
2198 * MajNHYD=["S-1-D8J"]

2200 * TMJSTO=[9999999](cu-m)
2201 *%-----|-----
2202 *ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2203 *%-----|-----
2204 *ADD HYD NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Fost"]
2205 *%-----|-----
2206 *COMPUTE DUALHYD NHYDin=["S-1-D"], CINLET=[11.616](cms), NINLET=[1],
2207 * MajNHYD=["S-1-D-MJ"]
2208 * MinNHYD=["S-1-D-MN"]
2209 * TMJSTO=[5974](cu-m)
2210 *%-----|-----
2211 *ADD HYD NHYDsum=["S-1-DEV"], NHYDs to add=["S-1-D-MJ"+"S-1-D-MN"]
2212 *%-----|-----
2213 *ROUTE RESERVOIR NHYDout=["S-1-D8R"],NHYDin=["S-1-D8S"],
2214 * RDT=[1](min),
2215 * TABLE of ( OUTFLOW-STORAGE ) values
2216 * (cms) - (ha-m)
2217 * [ 0.0 , 0.0 ]
2218 * [ 0.0630, 0.2102 ]
2219 * [ -1 , -1 ] (max twenty pts)
2220 * NHYDovf=["S-1-D8Rovf"]
2221 *%-----|-----
2222 * -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2223 CONTINUOUS NASHYD NHYD=["S-1-A"], DT=[1]min, AREA=[75.88](ha),
2224 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2225 N=[3], TP=[0.619]hrs,
2226 Continuous simulation parameters:
2227 IaREcper=[4](hrs),
2228 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2229 InterEventTime=[12](hrs)
2230 Baseflow simulation parameters:
2231 BaseFlowOption=[1] ,
2232 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2233 VHydCond=[0.055](mm/hr), END=-1
2234 *%-----|-----
2235 * -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2236 CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2237 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2238 N=[3], TP=[1.10]hrs,
2239 Continuous simulation parameters:
2240 IaREcper=[4](hrs),
2241 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2242 InterEventTime=[12](hrs)

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2243 Baseflow simulation parameters:
2244 BaseFlowOption=[1] ,
2245 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2246 VHydCond=[0.055](mm/hr), END=-1
2247 *%-----|-----|
2248 ADD HYD NHYDsum=["SN_FO"], NHYDs to
add=["N_FO"+"520-out"+"MS_P10"+"P10-OVF"+"W_CLAR_UNDE"+"S-1-FO-F-D"+"S-1-D8"+"S-1-A"]
2249 *%-----|-----|
2250 SAVE HYD NHYD=["SN_FO"], # OF PCYCLES=[-1], ICASEsh=[1]
2251 HYD_COMMENT=["Total Flows at Foster Drain"]
2252 *%-----|-----|
2253 *# Hydrograph from Node Foster routed to Node at Cedarview Road
2254 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
2255 *#
2256 ROUTE CHANNEL NHYDout=["N_CE"] ,NHYDin=["SN_FO"] ,
2257 RDT=[1](min),
2258 CHLGTH=[159](m), CHSLOPE=[0.0818](%),
2259 FPSLOPE=[0.0818](%),
2260 SECNUM=[1.0], NSEG=[3]
2261 ( SEGRROUGH, SEGDIST (m))=
2262 [0.050,-15.46
2263 -0.035,26.55
2264 0.050,116.76] NSEG times
2265 ( DISTANCE (m), ELEVATION (m))=
2266 [-645.23, 91.50]
2267 [-391.20, 91.50]
2268 [-91.00, 91.50]
2269 [-85.52, 91.50]
2270 [-15.46, 89.40]
2271 [-9.79, 89.31]
2272 [-3.22, 86.24]
2273 [3.22, 85.07]
2274 [10.96, 85.79]
2275 [16.44, 86.49]
2276 [26.55, 89.45]
2277 [29.03, 90.27]
2278 [35.76, 90.67]
2279 [36.67, 91.00]
2280 [108.08, 91.00]
2281 [109.82, 90.50]
2282 [112.04, 90.50]
2283 [114.62, 91.00]
2284 [116.76, 91.50]
2285 *%-----|-----|
2286 *#*****
2287 *# Catchment S-1
2288 *# - To Jock River (north and south of Jock)
2289 *# - Primarily agricultural fields; portion of sand quarry
2290 *%-----|-----|
2291 *# -2020-12-17 "S-1-Undev" and "S-1-Fost" was a part of Foster drain, they are below
the foster pond. Now they are added to S-1 subcatchment based on Project 1474-BCDC,
JFSA, Nov. 2020
2292 *# -2020-12-17 Change O-14 (it was part of Okeefe drain) to "S-1-Okeefe" and add it
to S-1 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
2293 *# -2020-12-17 Add "S-1-BCDC" as NASHYD
2294 *# -2020-12-17 all other S-1 subcatchment as STANDHYD with DUALHYD and ROUTE RESERVOIR
2295 *%-----|-----|
2296 *#*****
2297 *# -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2298 *CONTINUOUS NASHYD NHYD=["S-1-A"], DT=[1]min, AREA=[75.88](ha),
2299 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2300 * N=[3], TP=[0.619]hrs,
2301 * Continuous simulation parameters:
2302 * IaRECper=[4](hrs),
2303 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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2304 *           InterEventTime=[12](hrs)
2305 *           Baseflow simulation parameters:
2306 *           BaseFlowOption=[1] ,
2307 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2308 *           VHydCond=[0.055](mm/hr),   END=-1
2309 *%-----|-----|
2310 CONTINUOUS NASHYD NHYD=["S-1-B"], DT=[1]min, AREA=[55.36](ha),
2311 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2312 N=[3], TP=[0.451]hrs,
2313 Continuous simulation parameters:
2314 IaRECper=[4](hrs),
2315 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2316 InterEventTime=[12](hrs)
2317 Baseflow simulation parameters:
2318 BaseFlowOption=[1] ,
2319 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2320 VHydCond=[0.055](mm/hr),   END=-1
2321 *%-----|-----|
2322 *# - JFSA 2021-02-24 change the name from S-1-BCDC to S-1-A and S-1-B. Change their
TP values based on the new areas compared to the old ones.
2323 *CONTINUOUS NASHYD NHYD=["S-1-BCDC"], DT=[1]min, AREA=[134.9](ha),
2324 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2325 * N=[3], TP=[1.10]hrs,
2326 * Continuous simulation parameters:
2327 * IaRECper=[4](hrs),
2328 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2329 * InterEventTime=[12](hrs)
2330 * Baseflow simulation parameters:
2331 * BaseFlowOption=[1] ,
2332 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2333 * VHydCond=[0.055](mm/hr),   END=-1
2334 *%-----|-----|
2335 *# - JFSA 2021-02-24 "S-1-BCDC-1" and "S-1-BCDC-2" are not existing anymore.
"S-1-BCDC-1" is part of "S-1-FO-D2" and "S-1-BCDC-2" is part of "S-1-D2" and "S-1-D3"
2336 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-1"], DT=[1]min, AREA=[0.3](ha),
2337 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2338 * N=[3], TP=[1.10]hrs,
2339 * Continuous simulation parameters:
2340 * IaRECper=[4](hrs),
2341 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2342 * InterEventTime=[12](hrs)
2343 * Baseflow simulation parameters:
2344 * BaseFlowOption=[1] ,
2345 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2346 * VHydCond=[0.055](mm/hr),   END=-1
2347 *%-----|-----|
2348 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-2"], DT=[1]min, AREA=[1.3](ha),
2349 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2350 * N=[3], TP=[1.10]hrs,
2351 * Continuous simulation parameters:
2352 * IaRECper=[4](hrs),
2353 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2354 * InterEventTime=[12](hrs)
2355 * Baseflow simulation parameters:
2356 * BaseFlowOption=[1] ,
2357 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2358 * VHydCond=[0.055](mm/hr),   END=-1
2359 *%-----|-----|
2360 *# - JFSA 2021-01-19, after adding Greenbank pond, "S-1-BCDC-3" is not existing
anymore
2361 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-3"], DT=[1]min, AREA=[3.9](ha),
2362 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2363 * N=[3], TP=[1.10]hrs,
2364 * Continuous simulation parameters:
2365 * IaRECper=[4](hrs),
2366 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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2367 * InterEventTime=[12](hrs)
2368 * Baseflow simulation parameters:
2369 * BaseFlowOption=[1] ,
2370 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2371 * VHydCond=[0.055](mm/hr), END=-1
2372 *%-----|-----|
2373 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
2374 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[1](min), AREA=[44.93](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2375 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2376 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2377 * LGI=[547.296](m), MNI=[0.013], SCI=[0](min),
2378 * Continuous simulation parameters:
2379 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2380 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2381 * InterEventTime=[12](hrs), END=-1
2382 *%-----|-----|
2383 *COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CINLET=[4.796](cms), NINLET=[1],
2384 * MajNHYD=["S-1-OkMJ"]
2385 * MinNHYD=["S-1-OkMN"]
2386 * TMJSTO=[9999999](cu-m)
2387 *%-----|-----|
2388 *ADD HYD NHYDsum=["S-1-OkS"], NHYDs to add=["S-1-OkMJ"+"S-1-OkMN"]
2389 *%-----|-----|
2390 *ROUTE RESERVOIR NHYDout=["S-1-OkSR"], NHYDin=["S-1-OkS"],
2391 * RDT=[1](min),
2392 * TABLE of ( OUTFLOW-STORAGE ) values
2393 * (cms) - (ha-m)
2394 * [ 0.0 , 0.0 ]
2395 * [ 0.5370, 1.7917 ]
2396 * [ -1 , -1 ] (max twenty pts)
2397 * NHYDovf=["S-1-OkSovf"]
2398 *%-----|-----|
2399 *CONTINUOUS NASHYD NHYD=["S-1-Okeefe"], DT=[1]min, AREA=[44.93](ha),
2400 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2401 * N=[3], TP=[1.049]hrs,
2402 * Continuous simulation parameters:
2403 * IaRECper=[4](hrs),
2404 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2405 * InterEventTime=[12](hrs)
2406 * Baseflow simulation parameters:
2407 * BaseFlowOption=[1] ,
2408 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2409 * VHydCond=[0.055](mm/hr), END=-1
2410 *%-----|-----|
2411 * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2412 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2413 * XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2414 * SCS curve number CN=[74],
2415 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2416 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
2417 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2418 * LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2419 * Continuous simulation parameters:
2420 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2421 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2422 * InterEventTime=[18](hrs), END=-1
2423 *%-----|-----|
2424 *COMPUTE DUALHYD NHYDin=["S-1-FO-D1"], CINLET=[0.605](cms), NINLET=[1],
2425 * MajNHYD=["S-1-FO-D1J"]
2426 * MinNHYD=["S-1-FO-D1N"]
2427 * TMJSTO=[9999999](cu-m)

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2428 *%-----|-----|
2429 *ADD HYD          NHYDsum=["S-1-FO-D1S"], NHYDs to add=["S-1-FO-D1N"+"S-1-FO-D1J"]
2430 *%-----|-----|
2431 *ROUTE RESERVOIR NHYDout=["S-1-FO-D1R"], NHYDin=["S-1-FO-D1S"],
2432 *                RDT=[1](min),
2433 *                TABLE of ( OUTFLOW-STORAGE ) values
2434 *                (cms) - (ha-m)
2435 *                [ 0.0      , 0.0 ]
2436 *                [ 0.0611, 0.2038 ]
2437 *                [   -1   ,  -1   ] (max twenty pts)
2438 *                NHYDovf=["S-1FOD1ovf"]
2439 *%-----|-----|
2440 *CONTINUOUS NASHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2441 *                DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2442 *                N=[3], TP=[1.10]hrs,
2443 *                Continuous simulation parameters:
2444 *                IaRECper=[4](hrs),
2445 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2446 *                InterEventTime=[12](hrs)
2447 *                Baseflow simulation parameters:
2448 *                BaseFlowOption=[1],
2449 *                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2450 *                VHydCond=[0.055](mm/hr), END=-1
2451 *%-----|-----|
2452 * -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
2453 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
2454 *                XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
2455 *                SCS curve number CN=[74],
2456 *                Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2457 *                LGP=[40](m), MNP=[0.25], SCP=[0](min),
2458 *                Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2459 *                LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
2460 *                Continuous simulation parameters:
2461 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
2462 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2463 *                InterEventTime=[18](hrs), END=-1
2464 *%-----|-----|
2465 *CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
2466 *                DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2467 *                N=[3], TP=[1.10]hrs,
2468 *                Continuous simulation parameters:
2469 *                IaRECper=[4](hrs),
2470 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2471 *                InterEventTime=[12](hrs)
2472 *                Baseflow simulation parameters:
2473 *                BaseFlowOption=[1],
2474 *                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2475 *                VHydCond=[0.055](mm/hr), END=-1
2476 *%-----|-----|
2477 *COMPUTE DUALHYD  NHYDin=["S-1-FO-D2"], CINLET=[0.508](cms), NINLET=[1],
2478 *                MajNHYD=["S-1-FO-D2J"]
2479 *                MinNHYD=["S-1-FO-D2N"]
2480 *                TMJSTO=[9999999](cu-m)
2481 *%-----|-----|
2482 *ADD HYD          NHYDsum=["S-1-FO-D2S"], NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]
2483 *%-----|-----|
2484 *ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"], NHYDin=["S-1-FO-D2S"],
2485 *                RDT=[1](min),
2486 *                TABLE of ( OUTFLOW-STORAGE ) values
2487 *                (cms) - (ha-m)
2488 *                [ 0.0      , 0.0 ]
2489 *                [ 0.0590, 0.1970 ]
2490 *                [   -1   ,  -1   ] (max twenty pts)
2491 *                NHYDovf=["S-1FOD2ovf"]
2492 *%-----|-----|

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2493 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2494 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2495 * XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2496 * SCS curve number CN=[74],
2497 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2498 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
2499 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2500 * LGI=[315.806](m), MNI=[0.013], SCI=[0](min),
2501 * Continuous simulation parameters:
2502 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2503 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2504 * InterEventTime=[18](hrs), END=-1
2505 *%-----|-----|
2506 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2507 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2508 * N=[3], TP=[1.007]hrs,
2509 * Continuous simulation parameters:
2510 * IaRECper=[4](hrs),
2511 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2512 * InterEventTime=[12](hrs)
2513 * Baseflow simulation parameters:
2514 * BaseFlowOption=[1] ,
2515 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2516 * VHydCond=[0.055](mm/hr), END=-1
2517 *%-----|-----|
2518 *COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CINLET=[1.749](cms), NINLET=[1],
2519 * MajNHYD=["S-1FO-F-DJ"]
2520 * MinNHYD=["S-1FO-F-DN"]
2521 * TMJSTO=[9999999](cu-m)
2522 *%-----|-----|
2523 *ADD HYD NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2524 *%-----|-----|
2525 *ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"],NHYDin=["S-1FO-F-DS"] ,
2526 * RDT=[1](min),
2527 * TABLE of ( OUTFLOW-STORAGE ) values
2528 * (cms) - (ha-m)
2529 * [ 0.0 , 0.0 ]
2530 * [ 0.1788, 0.5966 ]
2531 * [ -1 , -1 ] (max twenty pts)
2532 * NHYDovf=["S-1FoFDovf"]
2533 *%-----|-----|
2534 *CONTINUOUS STANDHYD NHYD=["S-1-D1"], DT=[1](min), AREA=[21.67](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2535 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2536 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2537 * LGI=[380.088](m), MNI=[0.013], SCI=[0](min),
2538 * Continuous simulation parameters:
2539 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2540 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2541 * InterEventTime=[12](hrs), END=-1
2542 *%-----|-----|
2543 CONTINUOUS NASHYD NHYD=["S-1-D1"], DT=[1]min, AREA=[21.67](ha),
2544 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2545 N=[3], TP=[1.066]hrs,
2546 Continuous simulation parameters:
2547 IaRECper=[4](hrs),
2548 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2549 InterEventTime=[12](hrs)
2550 Baseflow simulation parameters:
2551 BaseFlowOption=[1] ,
2552 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2553 VHydCond=[0.055](mm/hr), END=-1
2554 *%-----|-----|

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2614 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2615 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2616 * InterEventTime=[12](hrs), END=-1
2617 *%-----|-----
2618 *CONTINUOUS NASHYD NHYD=["S-1-D3"], DT=[1]min, AREA=[6.79](ha),
2619 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2620 * N=[3], TP=[1.281]hrs,
2621 * Continuous simulation parameters:
2622 * IaRECper=[4](hrs),
2623 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2624 * InterEventTime=[12](hrs)
2625 * Baseflow simulation parameters:
2626 * BaseFlowOption=[1] ,
2627 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2628 * VHydCond=[0.055](mm/hr), END=-1
2629 *%-----|-----
2630 *COMPUTE DUALHYD NHYDin=["S-1-D3"], CINLET=[0.719](cms), NINLET=[1],
2631 * MajNHYD=["S-1-D3J"]
2632 * MinNHYD=["S-1-D3N"]
2633 * TMJSTO=[9999999](cu-m)
2634 *%-----|-----
2635 *ADD HYD NHYDsum=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
2636 *%-----|-----
2637 *ROUTE RESERVOIR NHYDout=["S-1-D3R"],NHYDin=["S-1-D3S"] ,
2638 * RDT=[1](min),
2639 * TABLE of ( OUTFLOW-STORAGE ) values
2640 * (cms) - (ha-m)
2641 * [ 0.0 , 0.0 ]
2642 * [ 0.0811, 0.2708 ]
2643 * [ -1 , -1 ] (max twenty pts)
2644 * NHYDovf=["S-1-D3Rovf"]
2645 *%-----|-----
2646 *CONTINUOUS STANDHYD NHYD=["S-1-D4"], DT=[1](min), AREA=[3.28](ha), XIMP=[0.65],
2647 * TIMP=[0.65], DWF=[0](cms),
2648 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2649 * IAper=[4.67](mm), SLPP=[2.0](%),
2650 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2651 * IAimp=[1.57](mm), SLPI=[0.75](%),
2652 * LGI=[147.874](m), MNI=[0.013], SCI=[0](min),
2653 * Continuous simulation parameters:
2654 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2655 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2656 * InterEventTime=[12](hrs), END=-1
2657 *%-----|-----
2658 CONTINUOUS NASHYD NHYD=["S-1-D4"], DT=[1]min, AREA=[3.28](ha),
2659 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2660 * N=[3], TP=[1.10]hrs,
2661 * Continuous simulation parameters:
2662 * IaRECper=[4](hrs),
2663 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2664 * InterEventTime=[12](hrs)
2665 * Baseflow simulation parameters:
2666 * BaseFlowOption=[1] ,
2667 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2668 * VHydCond=[0.055](mm/hr), END=-1
2669 *%-----|-----
2670 *COMPUTE DUALHYD NHYDin=["S-1-D4"], CINLET=[0.373](cms), NINLET=[1],
2671 * MajNHYD=["S-1-D4J"]
2672 * MinNHYD=["S-1-D4N"]
2673 * TMJSTO=[9999999](cu-m)
2674 *%-----|-----
2675 *ADD HYD NHYDsum=["S-1-D4S"], NHYDs to add=["S-1-D4J"+"S-1-D4N"]
2676 *%-----|-----
2677 *ROUTE RESERVOIR NHYDout=["S-1-D4R"],NHYDin=["S-1-D4S"] ,
2678 * RDT=[1](min),
2679 * TABLE of ( OUTFLOW-STORAGE ) values

```

```

2677 *                (cms) - (ha-m)
2678 *                [ 0.0      , 0.0 ]
2679 *                [ 0.0392, 0.1308 ]
2680 *                [   -1   ,   -1   ] (max twenty pts)
2681 *                NHYDovf=["S-1-D4Rovf"]
2682 *%-----|-----|
2683 *CONTINUOUS STANDHYD NHYD=["S-1-D5"], DT=[1](min), AREA=[12.84](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2685 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2686 *                LGI=[292.57](m), MNI=[0.013], SCI=[0](min),
2687 *                Continuous simulation parameters:
2688 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
2689 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2690 *                InterEventTime=[12](hrs), END=-1
2691 *%-----|-----|
2692 CONTINUOUS NASHYD NHYD=["S-1-D5"], DT=[1]min, AREA=[12.84](ha),
2693 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2694 N=[3], TP=[1.10]hrs,
2695 Continuous simulation parameters:
2696 IaRECper=[4](hrs),
2697 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2698 InterEventTime=[12](hrs)
2699 Baseflow simulation parameters:
2700 BaseFlowOption=[1] ,
2701 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2702 VHydCond=[0.055](mm/hr), END=-1
2703 *%-----|-----|
2704 *COMPUTE DUALHYD NHYDin=["S-1-D5"], CINLET=[1.395](cms), NINLET=[1],
2705 *                MajNHYD=["S-1-D5J"]
2706 *                MinNHYD=["S-1-D5N"]
2707 *                TMJSTO=[9999999](cu-m)
2708 *%-----|-----|
2709 *ADD HYD NHYDsum=["S-1-D5S"], NHYDs to add=["S-1-D5J"+"S-1-D5N"]
2710 *%-----|-----|
2711 *ROUTE RESERVOIR NHYDout=["S-1-D5R"] ,NHYDin=["S-1-D5S"] ,
2712 *                RDT=[1](min),
2713 *                TABLE of ( OUTFLOW-STORAGE ) values
2714 *                (cms) - (ha-m)
2715 *                [ 0.0      , 0.0 ]
2717 *                [   -1   ,   -1   ] (max twenty pts)
2718 *                NHYDovf=["S-1-D5Rovf"]
2719 *%-----|-----|
2720 *CONTINUOUS STANDHYD NHYD=["S-1-D6"], DT=[1](min), AREA=[1.75](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2721 *                LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2722 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2723 *                LGI=[108.01](m), MNI=[0.013], SCI=[0](min),
2724 *                Continuous simulation parameters:
2725 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
2726 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2727 *                InterEventTime=[12](hrs), END=-1
2728 *%-----|-----|
2729 CONTINUOUS NASHYD NHYD=["S-1-D6"], DT=[1]min, AREA=[1.75](ha),
2730 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2731 N=[3], TP=[1.10]hrs,
2732 Continuous simulation parameters:
2733 IaRECper=[4](hrs),
2734 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2735 InterEventTime=[12](hrs)
2736 Baseflow simulation parameters:

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2737 BaseFlowOption=[1] ,
2738 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2739 VHydCond=[0.055](mm/hr), END=-1
2740 *%-----|-----|
2741 *COMPUTE DUALHYD NHYDin=["S-1-D6"], CINLET=[0.218](cms), NINLET=[1],
2742 * MajNHYD=["S-1-D6J"]
2743 * MinNHYD=["S-1-D6N"]
2744 * TMJSTO=[9999999](cu-m)
2745 *%-----|-----|
2746 *ADD HYD NHYDsum=["S-1-D6S"], NHYDs to add=["S-1-D6J"+"S-1-D6N"]
2747 *%-----|-----|
2748 *ROUTE RESERVOIR NHYDout=["S-1-D6R"],NHYDin=["S-1-D6S"] ,
2749 * RDT=[1](min),
2750 * TABLE of ( OUTFLOW-STORAGE ) values
2751 * (cms) - (ha-m)
2752 * [ 0.0 , 0.0 ]
2753 * [ 0.0209, 0.0698 ]
2754 * [ -1 , -1 ] (max twenty pts)
2755 * NHYDovf=["S-1-D6Rovf"]
2756 *%-----|-----|
2757 *CONTINUOUS STANDHYD NHYD=["S-1-D7"], DT=[1](min), AREA=[2.03](ha), XIMP=[0.65],
2758 * TIMP=[0.65], DWF=[0](cms),
2759 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2760 * IAper=[4.67](mm), SLPP=[2.0](%),
2761 * LGI=[116.33](m), MNI=[0.013], SCI=[0](min),
2762 * Continuous simulation parameters:
2763 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
2764 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2765 * InterEventTime=[12](hrs), END=-1
2766 *%-----|-----|
2767 CONTINUOUS NASHYD NHYD=["S-1-D7"], DT=[1]min, AREA=[2.03](ha),
2768 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2769 * N=[3], TP=[1.10]hrs,
2770 * Continuous simulation parameters:
2771 * IaREcper=[4](hrs),
2772 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2773 * InterEventTime=[12](hrs)
2774 * Baseflow simulation parameters:
2775 * BaseFlowOption=[1] ,
2776 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2777 * VHydCond=[0.055](mm/hr), END=-1
2778 *%-----|-----|
2779 *COMPUTE DUALHYD NHYDin=["S-1-D7"], CINLET=[2.279](cms), NINLET=[1],
2780 * MajNHYD=["S-1-D7J"]
2781 * MinNHYD=["S-1-D7N"]
2782 * TMJSTO=[9999999](cu-m)
2783 *%-----|-----|
2784 *ADD HYD NHYDsum=["S-1-D7S"], NHYDs to add=["S-1-D7J"+"S-1-D7N"]
2785 *%-----|-----|
2786 *ROUTE RESERVOIR NHYDout=["S-1-D7R"],NHYDin=["S-1-D7S"] ,
2787 * RDT=[1](min),
2788 * TABLE of ( OUTFLOW-STORAGE ) values
2789 * (cms) - (ha-m)
2790 * [ 0.0 , 0.0 ]
2791 * [ 0.0243, 0.0810 ]
2792 * [ -1 , -1 ] (max twenty pts)
2793 * NHYDovf=["S-1-D8Rovf"]
2794 *%-----|-----|
2795 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
2796 * before station 6016 on Jock River
2797 *CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1](min), AREA=[5.27](ha), XIMP=[0.65],
2798 * TIMP=[0.65], DWF=[0](cms),
2799 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2800 * IAper=[4.67](mm), SLPP=[2.0](%),

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2797 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2798 *                LGI=[187.439](m), MNI=[0.013], SCI=[0](min),
2799 *                Continuous simulation parameters:
2800 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
2801 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2802 *                InterEventTime=[12](hrs), END=-1
2803 *%-----|-----
2804 *CONTINUOUS NASHYD  NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha),
2805 *                DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2806 *                N=[3], TP=[1.10]hrs,
2807 *                Continuous simulation parameters:
2808 *                IaRECper=[4](hrs),
2809 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2810 *                InterEventTime=[12](hrs)
2811 *                Baseflow simulation parameters:
2812 *                BaseFlowOption=[1],
2813 *                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2814 *                VHydCond=[0.055](mm/hr), END=-1
2815 *%-----|-----
2816 *COMPUTE DUALHYD  NHYDin=["S-1-D8"], CINLET=[2.279](cms), NINLET=[1],
2817 *                MajNHYD=["S-1-D8J"]
2818 *                MinNHYD=["S-1-D8N"]
2819 *                TMJSTO=[9999999](cu-m)
2820 *%-----|-----
2821 *ADD HYD           NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2822 *%-----|-----
2823 *ADD HYD           NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Fost"]
2824 *%-----|-----
2825 *COMPUTE DUALHYD  NHYDin=["S-1-D"], CINLET=[11.616](cms), NINLET=[1],
2826 *                MajNHYD=["S-1-D-MJ"]
2827 *                MinNHYD=["S-1-D-MN"]
2828 *                TMJSTO=[5974](cu-m)
2829 *%-----|-----
2830 *ADD HYD           NHYDsum=["S-1-DEV"], NHYDs to add=["S-1-D-MJ"+"S-1-D-MN"]
2831 *%-----|-----
2832 *ROUTE RESERVOIR  NHYDout=["S-1-D8R"], NHYDin=["S-1-D8S"],
2833 *                RDT=[1](min),
2834 *                TABLE of ( OUTFLOW-STORAGE ) values
2835 *                (cms) - (ha-m)
2836 *                [ 0.0      , 0.0 ]
2837 *                [ 0.0630, 0.2102 ]
2838 *                [   -1   ,  -1   ] (max twenty pts)
2839 *                NHYDovf=["S-1-D8Rovf"]
2840 *%-----|-----
2841 *%-----|-----
2842 *                -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
2843 *#      Catchment W_CLAR
2844 *#      - To West Clarke Drain (south of the Jock)
2845 *#      - Subdivision with 43% imp. as per Barrhaven South MSS
2846 *#      - 2020-11-30 update CLARKE Tributary Drainage Area to = 121 ha based on
P598(04)-11
2847 *#      - 2020-11-30 split CLARKE Drainage Area to MAJOR and ALL
2848 *#*****
2849 CONTINUOUS STANDHYD NHYD=["W_CLAR_MJ"], DT=[1]min, AREA=[1.772](ha),
2850 *                XIMP=[0.46], TIMP=[0.59], DWF=[0](cms), LOSS=[2],
2851 *                SCS curve number CN=[77],
2852 *                Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
2853 *                LGP=[40](m), MNP=[0.25], SCP=[0](min),
2854 *                Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
2855 *                LGI=[109](m), MNI=[0.013], SCI=[0](min),
2856 *                Continuous simulation parameters:
2857 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
2858 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2859 *                InterEventTime=[18](hrs), END=-1

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2860 *%-----|-----|
2861 *COMPUTE DUALHYD      NHYDin=["W_CLAR_MJ"], CINLET=[0.213](cms), NINLET=[1],
2862 *                      MajNHYD=["W_CLAR_MJj"]
2863 *                      MinNHYD=["W_CLAR_MJn"]
2864 *                      TMJSTO=[0.1](cu-m)
2865 *%-----|-----|
2866 *# 5-Year + 12% Capture
2867 ROUTE RESERVOIR      NHYDout=["W_CLAR_MJn"], NHYDin=["W_CLAR_MJ"],
2868                      RDT=[1](min),
2869                      TABLE of ( OUTFLOW-STORAGE ) values
2870                      (cms) - (ha-m)
2871                      [ 0.0      , 0.0 ]
2872                      [ 0.213  , 0.0001 ]
2873                      [      -1  ,  -1   ] (max twenty pts)
2874                      NHYDovf=["W_CLAR_MJj"],
2875 *%-----|-----|
2876 *      -Clarke_All area from P 598(04)-11 = 120.207 ha, change to 127.298 ha based on
GIS measurements,
2877 *      -JFSA, 2021-01-19 update W_CLAR_ALL to (121.17-1.772=119.398) ha based on GIS
measurements W_CLAR is 121.17 ha and W_CLAR_MJ is 1.772 ha
2878 CONTINUOUS STANDHYD NHYD=["W_CLAR_ALL"], DT=[1]min, AREA=[119.398](ha),
2879                      XIMP=[0.60], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2880                      SCS curve number CN=[77],
2881                      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
2882                      LGP=[40](m), MNP=[0.25], SCP=[0](min),
2883                      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
2884                      LGI=[892.18](m), MNI=[0.013], SCI=[0](min),
2885                      Continuous simulation parameters:
2886                      IaRECPper=[4](hrs), IaRECImp=[4](hrs),
2887                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2888                      InterEventTime=[18](hrs), END=-1
2889 *%-----|-----|
2890 ADD HYD              NHYDsum=["W_CLAR"], NHYDs to add=["W_CLAR_ALL"+"W_CLAR_MJj"]
2891 *%-----|-----|
2892 SAVE HYD            NHYD=["W_CLAR"], # OF PCYCLES=[-1], ICASEsh=[1]
2893                      HYD_COMMENT=["Total Flows to West Clarke"]
2894 *#*****
2895 *#      West Clarke Pond 2
2896 *#      - Rating curve obtained from Barrhaven South MSS modeling
2897 *#      - Tributary Drainage Area to MSS Pond 2 = 241 ha
2898 *#*****
2899 ROUTE RESERVOIR      NHYDout=["MS_P2"], NHYDin=["W_CLAR"],
2900                      RDT=[1](min),
2901                      TABLE of ( OUTFLOW-STORAGE ) values
2902                      (cms) - (ha-m)
2903                      [ 0.0      , 0.0 ]
2904                      [ 0.128  , 0.161 ]
2905                      [ 0.138  , 0.409 ]
2906                      [ 0.148  , 0.68 ]
2907                      [ 0.227  , 0.931 ]
2908                      [ 0.354  , 1.223 ]
2909                      [ 0.505  , 1.52 ]
2910                      [ 0.666  , 1.821 ]
2911                      [ 0.831  , 2.123 ]
2912                      [ 0.995  , 2.434 ]
2913                      [ 1.069  , 2.583 ]
2914                      [ 1.51   , 2.647 ]
2915                      [ 4.904  , 2.861 ]
2916                      [ 13.048 , 3.188 ]
2917                      [ 23.745 , 3.523 ]
2918                      [ 36.474 , 3.871 ]
2919                      [ 45.938 , 4.127 ]
2920                      [ 61.652 , 4.539 ]
2921                      [      -1  ,  -1   ] (max twenty pts)
2922                      NHYDovf=["P2-OVF"]
2923 *%-----|-----|

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2924 *#*****
2925 * -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2926 *CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2927 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2928 * N=[3], TP=[1.10]hrs,
2929 * Continuous simulation parameters:
2930 * IaRECper=[4](hrs),
2931 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2932 * InterEventTime=[12](hrs)
2933 * Baseflow simulation parameters:
2934 * BaseFlowOption=[1] ,
2935 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2936 * VHydCond=[0.055](mm/hr), END=-1
2937 *%-----|-----|
2938 ADD HYD NHYDsum=["SN_CE"], NHYDs to add=["N_CE"+
2939 + "S-1-D4"+"S-1-D5"+"MS_P2"+"P2-OVF" ]
2940 *%-----|-----|
2941 SAVE HYD NHYD=["SN_CE"], # OF PCYCLES=[-1], ICASEsh=[1]
2942 HYD_COMMENT=["Total Flows before Station 5737 on Jock River"]
2943 *%-----|-----|
2944 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5737
2945 *# JFSA 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted
from the HEC-RAS model
T:\PROJ\1474-16\Design\20201026-QuantityControlAnalysis\HEC-RAS\JockLidar2005
2946 *# JFSA 2021-03-02 change the slope to 0.1% instead of 0.0175 to stabilize the model
2947 ROUTE CHANNEL NHYDout=["5737"] ,NHYDin=["SN_CE"] ,
2948 RDT=[1](min),
2949 CHLGTH=[270](m), CHSLOPE=[0.0175](%),
2950 FPSLOPE=[0.0175](%),
2951 SECNUM=[1.0], NSEG=[3]
2952 ( SEGROUGH, SEGDIST (m))=
2953 [0.050,-24.04
2954 -0.035,23.92
2955 0.050,1130.8] NSEG times
2956 ( DISTANCE (m), ELEVATION (m))=
2957 [-1060.52, 94 ]
2958 [-268.6, 91.5 ]
2959 [-259.43, 91.5 ]
2960 [-179.48, 91.5 ]
2961 [-67.9, 91.5 ]
2962 [-59.21, 91.5 ]
2963 [-33.19, 91 ]
2964 [-26.08, 90.5 ]
2965 [-24.04, 90 ]
2966 [-13.14, 86.77 ]
2967 [0, 85 ]
2968 [14.68, 86.74 ]
2969 [23.92, 90 ]
2970 [25.78, 90.5 ]
2971 [31.91, 91 ]
2972 [91.95, 91.5 ]
2973 [772.15, 92 ]
2974 [961.49, 92.5 ]
2975 [1044.69, 93 ]
2976 [1130.8, 95 ]
2977 *%-----|-----|
2978 ADD HYD NHYDsum=["5002"], NHYDs to add=["5737"+
2979 + "S-1-D1"+"S-1-D6"+"S-1-D7" ]
2980 *%-----|-----|
2981 SAVE HYD NHYD=["5002"], # OF PCYCLES=[-1], ICASEsh=[1]
2982 HYD_COMMENT=["Total Flows before Station 5002 on Jock River"]
2983 *%-----|-----|
2984 *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
2985 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002

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2986  *# JFSA 2021-02-19 Change the slope from 0.01 % (as per Stantec Report 2007) to 0.0255
      % so the model will be more stable and give reasonable results. It is justifiable as
      ROUTE CHANNELs aren't well suited to really flat slopes.
2987  *# JFSA 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one
      with 825 m length so the model will be more stable
2988  *# JFSA 2021-02-26 change the length of 5002 route channel from 825 m to 736 m. That is
      because of adding station 5737 between station 6016 and station 5002. Then the length
      from station 5737 to station 5002 is 736 m. Change the slope from 0.0255 % to 0.09511 %
2989  *
2990  ROUTE CHANNEL      NHYDout=["N_WCa" ] ,NHYDin=["5002" ] ,
2991                    RDT=[1](min) ,
2992                    CHLGTH=[245.33333](m) ,   CHSLOPE=[0.09511](%) ,
2993                    FPSLOPE=[0.09511](%) ,
2994                    SECNUM=[1.0] ,           NSEG=[3]
2995                    ( SEGROUGH, SEGDIST (m))=
2996                      [0.050,-37.5
2997                      -0.035,37.50
2998                      0.050,157.05] NSEG times
2999                    ( DISTANCE (m), ELEVATION (m))=
3000                    [-601.81, 91.5]
3001                    [-37.50, 90.00]
3002                    [-19.61, 87.04]
3003                    [0.00, 85.70]
3004                    [14.87, 86.93]
3005                    [37.50, 90.00]
3006                    [38.54, 90.50]
3007                    [42.23, 91]
3008                    [157.05,91.50]
3009  *                  [161.44, 91.50]
3010  *                  [236.48, 93.00]
3011  *                  [385.47, 92.50]
3012  *                  [390.78, 92.50]
3013  *%-----|
3014  ROUTE CHANNEL      NHYDout=["N_WCb" ] ,NHYDin=["N_WCa" ] ,
3015                    RDT=[1](min) ,
3016                    CHLGTH=[245.33333](m) ,   CHSLOPE=[0.09511](%) ,
3017                    FPSLOPE=[0.09511](%) ,
3018                    SECNUM=[1.0] ,           NSEG=[3]
3019                    ( SEGROUGH, SEGDIST (m))=
3020                      [0.050,-37.5
3021                      -0.035,37.50
3022                      0.050,157.05] NSEG times
3023                    ( DISTANCE (m), ELEVATION (m))=
3024                    [-601.81, 91.5]
3025                    [-37.50, 90.00]
3026                    [-19.61, 87.04]
3027                    [0.00, 85.70]
3028                    [14.87, 86.93]
3029                    [37.50, 90.00]
3030                    [38.54, 90.50]
3031                    [42.23, 91]
3032                    [157.05,91.50]
3033  *%-----|
3034  ROUTE CHANNEL      NHYDout=["N_WC" ] ,NHYDin=["N_WCb" ] ,
3035                    RDT=[1](min) ,
3036                    CHLGTH=[245.33333](m) ,   CHSLOPE=[0.09511](%) ,
3037                    FPSLOPE=[0.09511](%) ,
3038                    SECNUM=[1.0] ,           NSEG=[3]
3039                    ( SEGROUGH, SEGDIST (m))=
3040                      [0.050,-37.5
3041                      -0.035,37.50
3042                      0.050,157.05] NSEG times
3043                    ( DISTANCE (m), ELEVATION (m))=
3044                    [-601.81, 91.5]
3045                    [-37.50, 90.00]
3046                    [-19.61, 87.04]

```

```

3047         [0.00, 85.70]
3048         [14.87, 86.93]
3049         [37.50, 90.00]
3050         [38.54, 90.50]
3051         [42.23, 91]
3052         [157.05,91.50]
3053 *#*****
3054 *       -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
3055 *ADD HYD          NHYDsum=["SN_WC"], NHYDs to
add=["MS_P2"+"P2-OVF"+"N_WC"+"W_CLAR_UNDE"]
3056 *%-----|-----|
3057 *SAVE HYD          NHYD=["SN_WC"], # OF PCYCLES=[-1], ICASEsh=[1]
3058 *                HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
3059 *%-----|-----|
3060 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
3061 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
3062 *#
3063 ROUTE CHANNEL    NHYDout=["N_KB"] ,NHYDin=["N_WC"] ,
3064                    RDT=[1](min),
3065                    CHLGTH=[1020](m),  CHSLOPE=[0.0498](%),
3066                    FPSLOPE=[0.0498](%),
3067                    SECNUM=[1.0],      NSEG=[3]
3068                    ( SEGROUGH, SEGDIST (m))=
3069                    [0.050,-23.63
3070                    -0.035,23.63
3071                    0.050,728.3] NSEG times
3072                    ( DISTANCE (m), ELEVATION (m))=
3073                    [-1082.01,94]
3074                    [-1028.17,92.5]
3075                    [-992.3,93.5]
3076                    [-279.34,90]
3077                    [-23.63,90]
3078                    [-13.45,87.13]
3079                    [-0.07,86.24]
3080                    [10.54,87.15]
3081                    [23.63,90]
3082                    [24.86,90.5]
3083                    [26.72,91]
3084                    [45.07,91.5]
3085                    [128.17,91.5]
3086                    [270.7,92.5]
3087                    [728.3,95]
3088 *%-----|-----|
3089 *#*****
3090 *#       Catchment KEN_BU
3091 *#       - To Kennedy-Burnett SWM Facility
3092 *#       - Outlets to Fraser-Clarke drain (north of the Jock)
3093 *#       - Medium density residential subdivision
3094 *       - Add Kennedy Burnett model (Convert PCSWMM from NOVATECH June, 2020 to SWMHYMO)
3095 *#*****
3096 *CONTINUOUS STANDHYD NHYD=["KEN_BU"], DT=[1]min, AREA=[281](ha),
3097 *                XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
3098 *                SCS curve number CN=[71],
3099 *                Pervious  surfaces: IAper=[4.67](mm), SLPP=[1](%),
3100 *                LGP=[40](m), MNP=[0.25], SCP=[0](min),
3101 *                Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3102 *                LGI=[1369](m), MNI=[0.013], SCI=[0](min),
3103 *                Continuous simulation parameters:
3104 *                IaRECper=[4](hrs),  IaRECimp=[4](hrs),
3105 *                SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
3106 *                InterEventTime=[18](hrs),  END=-1
3107 *%-----|-----|
3108 *#*****
3109 *#       Existing Kennedy-Burnett SWM Facility
3110 *#       - Rating curve obtained from URTKBP

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```

3111 *# - Tributary Drainage Area to Pond = 160 ha
3112 *#*****
3113 *ROUTE RESERVOIR NHYDout=["KEN_P"], NHYDin=["KEN_BU"],
3114 * RDT=[1](min),
3115 *
3116 * TABLE of ( OUTFLOW-STORAGE ) values
3117 * (cms) - (ha-m)
3118 * [ 0.0 , 0.0 ]
3119 * [ 0.13 , 0.26 ]
3120 * [ 0.43 , 0.56 ]
3121 * [ 0.67 , 0.90 ]
3122 * [ 0.86 , 1.32 ]
3123 * [ 1.01 , 1.79 ]
3124 * [ 1.15 , 2.33 ]
3125 * [ -1 , -1 ] (max twenty pts)
3126 * NHYDovf=["KEN-OV"]
3127 *%-----|-----
3128 * -JFSA, 2021-01-19 update all KEN_BU areas based on GIS measurements
3129 CONTINUOUS STANDHYD NHYD=["KB-01A"], DT=[1]min, AREA=[40.82](ha), XIMP=[0.097],
3130 TIMP=[0.4], DWF=[0](cms), LOSS=[1]:
3131 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3132 F=[0.00](mm),
3133 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
3134 MNP=[0.250], SCP=[0](min),
3135 Impervious areas: IAimp=[0.785](mm), SLPI=[0.5](%),
3136 LGI=[521.664](m), MNI=[0.013], SCI=[0](min),
3137 Continuous simulation parameters:
3138 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
3139 END=-1
3140 *%-----|-----
3141 COMPUTE DUALHYD NHYDin=["KB-01A"], CINLET=[3.6](cms), NINLET=[1],
3142 MajNHYD=["KB-01A-MJ"]
3143 MinNHYD=["KB-01A-MN"]
3144 TMJSTO=[4995](cu-m)
3145 *%-----|-----
3146 ADD HYD NHYDsum=["KB-01A-S"], NHYDs to add=["KB-01A-MJ"+"KB-01A-MN"]
3147 *%-----|-----
3148 CONTINUOUS STANDHYD NHYD=["KB-01B"], DT=[1]min, AREA=[31.1](ha), XIMP=[0.1875],
3149 TIMP=[0.375], DWF=[0](cms), LOSS=[1]:
3150 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3151 F=[0.00](mm),
3152 Pervious areas: IAper=[4.67](mm), SLPP=[0.42](%), LGP=[40](m),
3153 MNP=[0.250], SCP=[0](min),
3154 Impervious areas: IAimp=[0.785](mm), SLPI=[0.42](%),
3155 LGI=[455.339](m), MNI=[0.013], SCI=[0](min),
3156 Continuous simulation parameters:
3157 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
3158 END=-1
3159 *%-----|-----
3160 COMPUTE DUALHYD NHYDin=["KB-01B"], CINLET=[1.585](cms), NINLET=[1],
3161 MajNHYD=["KB-01B-MJ"]
3162 MinNHYD=["KB-01B-MN"]
3163 TMJSTO=[6075](cu-m)
3164 *%-----|-----
3165 ADD HYD NHYDsum=["KB-01B-S"], NHYDs to add=["KB-01B-MJ"+"KB-01B-MN"]
3166 *%-----|-----
3167 CONTINUOUS STANDHYD NHYD=["KB-01C"], DT=[1]min, AREA=[13.78](ha), XIMP=[0.2045],
3168 TIMP=[0.409], DWF=[0](cms), LOSS=[1]:
3169 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3170 F=[0.00](mm),
3171 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3172 MNP=[0.250], SCP=[0](min),
3173 Impervious areas: IAimp=[0.785](mm), SLPI=[0.5](%),
3174 LGI=[303.095](m), MNI=[0.013], SCI=[0](min),
3175 Continuous simulation parameters:
3176 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
3177 END=-1

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3162 *%-----|-----|
3163 COMPUTE DUALHYD NHYDin=["KB-01C"], CINLET=[1.35](cms), NINLET=[1],
3164 MajNHYD=["KB-01C-MJ"]
3165 MinNHYD=["KB-01C-MN"]
3166 TMJSTO=[1880](cu-m)
3167 *%-----|-----|
3168 ADD HYD NHYDsum=["KB-01C-S"], NHYDs to add=["KB-01C-MJ"+"KB-01C-MN"]
3169 *%-----|-----|
3170 CONTINUOUS STANDHYD NHYD=["KB-03"], DT=[1]min, AREA=[84.78](ha), XIMP=[0.197],
TIMP=[0.394], DWF=[0](cms), LOSS=[1]:
3171 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3172 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3173 Impervious areas: IAimp=[0.785](mm), SLPI=[0.63](%),
LGI=[751.798](m), MNI=[0.013], SCI=[0](min),
3174 Continuous simulation parameters:
3175 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3176 *%-----|-----|
3177 COMPUTE DUALHYD NHYDin=["KB-03"], CINLET=[5.27](cms), NINLET=[1],
3178 MajNHYD=["KB-03-MJ"]
3179 MinNHYD=["KB-03-MN"]
3180 TMJSTO=[15500](cu-m)
3181 *%-----|-----|
3182 ADD HYD NHYDsum=["KB-03-S"], NHYDs to add=["KB-03-MJ"+"KB-03-MN"]
3183 *%-----|-----|
3184 CONTINUOUS STANDHYD NHYD=["KB-04"], DT=[1]min, AREA=[6.95](ha), XIMP=[0.85],
TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
3185 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3186 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3187 Impervious areas: IAimp=[0.942](mm), SLPI=[0.5](%),
LGI=[215.252](m), MNI=[0.013], SCI=[0](min),
3188 Continuous simulation parameters:
3189 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3190 *%-----|-----|
3191 COMPUTE DUALHYD NHYDin=["KB-04"], CINLET=[0.503](cms), NINLET=[1],
3192 MajNHYD=["KB-04-MJ"]
3193 MinNHYD=["KB-04-MN"]
3194 TMJSTO=[1972](cu-m)
3195 *%-----|-----|
3196 ADD HYD NHYDsum=["KB-04-S"], NHYDs to add=["KB-04-MJ"+"KB-04-MN"]
3197 *%-----|-----|
3198 CONTINUOUS STANDHYD NHYD=["KB-05"], DT=[1]min, AREA=[5.19](ha), XIMP=[0.93],
TIMP=[0.93], DWF=[0](cms), LOSS=[1]:
3199 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3200 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3201 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[186.011](m), MNI=[0.013], SCI=[0](min),
3202 Continuous simulation parameters:
3203 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3204 *%-----|-----|
3205 *%-----|-----|
3206 CONTINUOUS STANDHYD NHYD=["KB-06"], DT=[1]min, AREA=[12.93](ha), XIMP=[0.873],
TIMP=[0.873], DWF=[0](cms), LOSS=[1]:
3207 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3208 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3209 Impervious areas: IAimp=[0.942](mm), SLPI=[4.75](%),

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3210 LGI=[293.598](m), MNI=[0.013], SCI=[0](min),
3211 Continuous simulation parameters:
3212 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3213 END=-1
3214 *%-----|-----|
3215 COMPUTE DUALHYD NHYDin=["KB-06"], CINLET=[2.262](cms), NINLET=[1],
3216 MajNHYD=["KB-06-MJ"]
3217 MinNHYD=["KB-06-MN"]
3218 TMJSTO=[1950](cu-m)
3219 *%-----|-----|
3220 ADD HYD NHYDsum=["KB-06-S"], NHYDs to add=["KB-06-MJ"+"KB-06-MN"]
3221 *%-----|-----|
3222 CONTINUOUS STANDHYD NHYD=["KB-11"], DT=[1]min, AREA=[4.03](ha), XIMP=[0.675],
3223 TIMP=[0.675], DWF=[0](cms), LOSS=[1]:
3224 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3225 F=[0.00](mm),
3226 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3227 MNP=[0.250], SCP=[0](min),
3228 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
3229 LGI=[163.911](m), MNI=[0.013], SCI=[0](min),
3230 Continuous simulation parameters:
3231 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3232 END=-1
3233 *%-----|-----|
3234 COMPUTE DUALHYD NHYDin=["KB-11"], CINLET=[0.5773](cms), NINLET=[1],
3235 MajNHYD=["KB-11-MJ"]
3236 MinNHYD=["KB-11-MN"]
3237 TMJSTO=[597](cu-m)
3238 *%-----|-----|
3239 ADD HYD NHYDsum=["KB-11-S"], NHYDs to add=["KB-11-MJ"+"KB-11-MN"]
3240 *%-----|-----|
3241 CONTINUOUS STANDHYD NHYD=["S1"], DT=[1]min, AREA=[4.99](ha), XIMP=[0.93], TIMP=[0.93],
3242 DWF=[0](cms), LOSS=[1]:
3243 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3244 F=[0.00](mm),
3245 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3246 MNP=[0.250], SCP=[0](min),
3247 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
3248 LGI=[182.392](m), MNI=[0.013], SCI=[0](min),
3249 Continuous simulation parameters:
3250 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3251 END=-1
3252 *%-----|-----|
3253 CONTINUOUS STANDHYD NHYD=["KB-15"], DT=[1]min, AREA=[2.15](ha), XIMP=[0.79],
3254 TIMP=[0.79], DWF=[0](cms), LOSS=[1]:
3255 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3256 F=[0.00](mm),
3257 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3258 MNP=[0.250], SCP=[0](min),
3259 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
3260 LGI=[119.722](m), MNI=[0.013], SCI=[0](min),
3261 Continuous simulation parameters:
3262 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3263 END=-1
3264 *%-----|-----|
3265 ADD HYD NHYDsum=["KB-P1"], NHYDs to
3266 add=["KB-01A-S"+"KB-01B-S"+"KB-01C-S"+"KB-03-S"+"KB-04-S"+"KB-05"+"KB-06-S"+"KB-11-S"+"KB
3267 -15"+"S1"]
3268 *%-----|-----|
3269 ROUTE RESERVOIR NHYDout=["KB-P1R"], NHYDin=["KB-P1"],
3270 RDT=[1](min),
3271 TABLE of ( OUTFLOW-STORAGE ) values
3272 (cms) - (ha-m)
3273 [ 0.0 , 0.0 ]
3274 [0.076,0.003]

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```

3257 [0.088,0.006]
3258 [0.136,0.011]
3259 [0.301,0.017]
3260 [0.454,0.027]
3261 [0.631,0.041]
3262 [1.173,0.068]
3263 [1.91,0.111]
3264 [4.847,0.231]
3265 [9.813,0.436]
3266 [12.134,0.617]
3267 [12.438,0.732]
3268 [12.424,0.811]
3269 [12.425,0.894]
3270 [ -1 , -1 ] (max twenty pts)
3271 NHYDovf=["KB-Plovf"]
3272 *%-----|-----|
3273 ADD HYD NHYDsum=["KB-Pond1"], NHYDs to add=["KB-P1R"+"KB-Plovf"]
3274 *%-----|-----|
3275 SAVE HYD NHYD=["KB-Pond1"], # OF PCYCLES=[-1], ICASEsh=[1]
3276 HYD_COMMENT=["Total Flows at KB first pond"]
3277 *%-----|-----|
3278 CONTINUOUS STANDHYD NHYD=["KB-07"], DT=[1]min, AREA=[10.86](ha), XIMP=[0.86],
3279 TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3280 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3281 F=[0.00](mm),
3282 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3283 MNP=[0.250], SCP=[0](min),
3284 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
3285 LGI=[269.072](m), MNI=[0.013], SCI=[0](min),
3286 Continuous simulation parameters:
3287 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3288 END=-1
3289 *%-----|-----|
3290 COMPUTE DUALHYD NHYDin=["KB-07"], CINLET=[2.094](cms), NINLET=[1],
3291 MaJNHYD=["KB-07-MJ"]
3292 MinNHYD=["KB-07-MN"]
3293 TMJSTO=[1378](cu-m)
3294 *%-----|-----|
3295 ADD HYD NHYDsum=["KB-07-S"], NHYDs to add=["KB-07-MJ"+"KB-07-MN"]
3296 *%-----|-----|
3297 CONTINUOUS STANDHYD NHYD=["KB-08"], DT=[1]min, AREA=[6.61](ha), XIMP=[0.64],
3298 TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3299 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3300 F=[0.00](mm),
3301 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3302 MNP=[0.250], SCP=[0](min),
3303 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
3304 LGI=[209.921](m), MNI=[0.013], SCI=[0](min),
3305 Continuous simulation parameters:
3306 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3307 END=-1
3308 *%-----|-----|
3309 COMPUTE DUALHYD NHYDin=["KB-08"], CINLET=[1.058](cms), NINLET=[1],
3310 MaJNHYD=["KB-08-MJ"]
3311 MinNHYD=["KB-08-MN"]
3312 TMJSTO=[787](cu-m)
3313 *%-----|-----|
3314 ADD HYD NHYDsum=["KB-08-S"], NHYDs to add=["KB-08-MJ"+"KB-08-MN"]
3315 *%-----|-----|
3316 CONTINUOUS STANDHYD NHYD=["KB-09"], DT=[1]min, AREA=[2.6](ha), XIMP=[0.86],
3317 TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3318 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3319 F=[0.00](mm),
3320 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3321 MNP=[0.250], SCP=[0](min),
3322 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),

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3310          LGI=[131.656](m), MNI=[0.013], SCI=[0](min),
3311          Continuous simulation parameters:
3312          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3313          END=-1
3314          *%-----|-----|
3315          *%-----|-----|
3316          CONTINUOUS STANDHYD NHYD=["KB-10_1"], DT=[1]min, AREA=[2.37](ha), XIMP=[0.86],
3317          TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3318          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3319          F=[0.00](mm),
3320          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3321          MNP=[0.250], SCP=[0](min),
3322          Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
3323          LGI=[125.698](m), MNI=[0.013], SCI=[0](min),
3324          Continuous simulation parameters:
3325          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3326          END=-1
3327          *%-----|-----|
3328          *%-----|-----|
3329          CONTINUOUS STANDHYD NHYD=["KB-10_2"], DT=[1]min, AREA=[1.14](ha), XIMP=[0.86],
3330          TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3331          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3332          F=[0.00](mm),
3333          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3334          MNP=[0.250], SCP=[0](min),
3335          Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%), LGI=[87.178](m),
3336          MNI=[0.013], SCI=[0](min),
3337          Continuous simulation parameters:
3338          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3339          END=-1
3340          *%-----|-----|
3341          *%-----|-----|
3342          CONTINUOUS STANDHYD NHYD=["KB-12"], DT=[1]min, AREA=[4.86](ha), XIMP=[0.79],
3343          TIMP=[0.79], DWF=[0](cms), LOSS=[1]:
3344          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3345          F=[0.00](mm),
3346          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3347          MNP=[0.250], SCP=[0](min),
3348          Impervious areas: IAimp=[1.099](mm), SLPI=[2.0](%),
3349          LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
3350          Continuous simulation parameters:
3351          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3352          END=-1
3353          *%-----|-----|
3354          *%-----|-----|
3355          COMPUTE DUALHYD NHYDin=["KB-12"], CINLET=[0.8665](cms), NINLET=[1],
3356          MajNHYD=["KB-12-MJ"]
3357          MinNHYD=["KB-12-MN"]
3358          TMJSTO=[632](cu-m)
3359          *%-----|-----|
3360          *%-----|-----|
3361          ADD HYD NHYDsum=["KB-12-S"], NHYDs to add=["KB-12-MJ"+"KB-12-MN"]
3362          *%-----|-----|
3363          *%-----|-----|
3364          CONTINUOUS STANDHYD NHYD=["KB-13"], DT=[1]min, AREA=[10.19](ha), XIMP=[0.64],
3365          TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3366          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3367          F=[0.00](mm),
3368          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3369          MNP=[0.250], SCP=[0](min),
3370          Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
3371          LGI=[260.640](m), MNI=[0.013], SCI=[0](min),
3372          Continuous simulation parameters:
3373          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3374          END=-1
3375          *%-----|-----|
3376          *%-----|-----|
3377          COMPUTE DUALHYD NHYDin=["KB-13"], CINLET=[1.722](cms), NINLET=[1],
3378          MajNHYD=["KB-13-MJ"]
3379          MinNHYD=["KB-13-MN"]
3380          TMJSTO=[1077](cu-m)

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3354 *%-----|-----|
3355 ADD HYD      NHYDsum=["KB-13-S"], NHYDs to add=["KB-13-MJ"+"KB-13-MN"]
3356 *%-----|-----|
3357 CONTINUOUS STANDHYD NHYD=["KB-14"], DT=[1]min, AREA=[5.47](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3358 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3359 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3360 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[190.962](m), MNI=[0.013], SCI=[0](min),
3361 Continuous simulation parameters:
3362 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3363 *%-----|-----|
3364 COMPUTE DUALHYD NHYDin=["KB-14"], CINLET=[0.8734](cms), NINLET=[1],
3365 MajNHYD=["KB-14-MJ"]
3366 MinNHYD=["KB-14-MN"]
3367 TMJSTO=[631](cu-m)
3368 *%-----|-----|
3369 ADD HYD      NHYDsum=["KB-14-S"], NHYDs to add=["KB-14-MJ"+"KB-14-MN"]
3370 *%-----|-----|
3371 *%-----|-----|
3372 CONTINUOUS STANDHYD NHYD=["KB-16_2"], DT=[1]min, AREA=[3.42](ha), XIMP=[0.71],
TIMP=[0.71], DWF=[0](cms), LOSS=[1]:
3373 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3374 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3375 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[150.997](m), MNI=[0.013], SCI=[0](min),
3376 Continuous simulation parameters:
3377 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3378 *%-----|-----|
3379 ADD HYD      NHYDsum=["KB-P2"], NHYDs to
add=["KB-Pond1"+"KB-07-S"+"KB-08-S"+"KB-09"+"KB-10_1"+"KB-10_2"+"KB-12-S"+"KB-13-S"+"KB-1
4-S"+"KB-16_2"]
3380 *%-----|-----|
3381 ROUTE RESERVOIR NHYDout=["KB-P2R"], NHYDin=["KB-P2"],
3382 RDT=[1](min),
3383 TABLE of ( OUTFLOW-STORAGE ) values
3384 (cms) - (ha-m)
3385 [ 0.0 , 0.0 ]
3386 [0.053,0.005]
3387 [0.132,0.009]
3388 [0.269,0.014]
3389 [0.455,0.023]
3390 [0.699,0.037]
3391 [0.947,0.056]
3392 [1.853,0.09]
3393 [2.712,0.146]
3394 [6.626,0.287]
3395 [11.228,0.515]
3396 [14.885,0.738]
3397 [16.473,0.893]
3398 [17.311,0.998]
3399 [17.633,1.063]
3400 [17.634,1.112]
3401 [ -1 , -1 ] (max twenty pts)
3402 NHYDovf=["KB-P2ovf"]
3403 *%-----|-----|
3404 ADD HYD      NHYDsum=["KB-Pond2"], NHYDs to add=["KB-P2R"+"KB-P2ovf"]
3405 *%-----|-----|
3406 SAVE HYD     NHYD=["KB-Pond2"], # OF PCYCLES=[-1], ICASEsh=[1]
3407 HYD_COMMENT=["Total Flows at KB second pond"]

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3408 *%-----|-----|
3409 CONTINUOUS STANDHYD NHYD=["KB-16_1"], DT=[1]min, AREA=[2.8](ha), XIMP=[0.75],
TIMP=[0.75], DWF=[0](cms), LOSS=[1]:
3410 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3411 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3412 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[136.626](m), MNI=[0.013], SCI=[0](min),
3413 Continuous simulation parameters:
3414 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3415 *%-----|-----|
3416 ADD HYD NHYDsum=["KB-P3"], NHYDs to add=["KB-Pond2"+"KB-16_1"]
3417 *%-----|-----|
3418 *%-----|-----|
3419 * One inflow node from pond 3 is added to the model (ROUTE RESERVOIR)
3420 * Another inflow node from right side of pond 3 is not added to the model
3421 ROUTE RESERVOIR NHYDout=["KB-P3R"], NHYDin=["KB-P3"],
3422 RDT=[1](min),
3423 TABLE of ( OUTFLOW-STORAGE ) values
3424 (cms) - (ha-m)
3425 [ 0.0 , 0.0 ]
3426 [0.051,0.002]
3427 [0.048,0.003]
3428 [0.057,0.029]
3429 [0.089,0.045]
3430 [0.133,0.069]
3431 [0.199,0.106]
3432 [0.321,0.172]
3433 [1.029,0.306]
3434 [4.036,0.527]
3435 [8.332,0.761]
3436 [11.727,0.941]
3437 [14.125,1.067]
3438 [15.675,1.149]
3439 [16.555,1.196]
3440 [16.911,1.214]
3441 [ -1 , -1 ] (max twenty pts)
3442 NHYDovf=["KB-P3ovf"]
3443 *%-----|-----|
3444 ADD HYD NHYDsum=["KB-Pond3"], NHYDs to add=["KB-P3R"+"KB-P3ovf"]
3445 *%-----|-----|
3446 SAVE HYD NHYD=["KB-Pond3"], # OF PCYCLES=[-1], ICASEsh=[1]
3447 HYD_COMMENT=["Total Flows at KB third pond"]
3448 *%-----|-----|
3449 *#*****
3450 *# EXISTING / PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM
Modeling Approach, NOVATECH Report June, 2020)
3451 *# - TO FRASER-CLARKE DRAIN
3452 *#*****
3453 CONTINUOUS STANDHYD NHYD=["FC-01"], DT=[1]min, AREA=[8.03](ha), XIMP=[0.47],
TIMP=[0.47], DWF=[0](cms), LOSS=[1]:
3454 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3455 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3456 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[231.373](m), MNI=[0.013], SCI=[0](min),
3457 Continuous simulation parameters:
3458 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3459 *%-----|-----|
3460 COMPUTE DUALHYD NHYDin=["FC-01"], CINLET=[0.756](cms), NINLET=[1],
3461 MajNHYD=["FC-01-MJ"]
3462 MinNHYD=["FC-01-MN"]

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3463          TMJSTO=[714](cu-m)
3464  *%-----|-----|
3465  ADD HYD      NHYDsum=["FC-01-S"], NHYDs to add=["FC-01-MJ"+"FC-01-MN"]
3466  *%-----|-----|
3467  CONTINUOUS STANDHYD NHYD=["FC-02"], DT=[1]min, AREA=[16.05](ha), XIMP=[0.93],
TIMP=[0.93], DWF=[0](cms), LOSS=[1]:
3468          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3469          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3470          Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[327.109](m), MNI=[0.013], SCI=[0](min),
3471          Continuous simulation parameters:
3472          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3473  *%-----|-----|
3474  COMPUTE DUALHYD NHYDin=["FC-02"], CINLET=[1.159](cms), NINLET=[1],
3475          MajNHYD=["FC-02-MJ"]
3476          MinNHYD=["FC-02-MN"]
3477          TMJSTO=[2385](cu-m)
3478  *%-----|-----|
3479  ADD HYD      NHYDsum=["FC-02-S"], NHYDs to add=["FC-02-MJ"+"FC-02-MN"]
3480  *%-----|-----|
3481  CONTINUOUS STANDHYD NHYD=["FC-03"], DT=[1]min, AREA=[7.37](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3482          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3483          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3484          Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[221.660](m), MNI=[0.013], SCI=[0](min),
3485          Continuous simulation parameters:
3486          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3487  *%-----|-----|
3488  COMPUTE DUALHYD NHYDin=["FC-03"], CINLET=[0.358](cms), NINLET=[1],
3489          MajNHYD=["FC-03-MJ"]
3490          MinNHYD=["FC-03-MN"]
3491          TMJSTO=[1131](cu-m)
3492  *%-----|-----|
3493  ADD HYD      NHYDsum=["FC-03-S"], NHYDs to add=["FC-03-MJ"+"FC-03-MN"]
3494  *%-----|-----|
3495  CONTINUOUS STANDHYD NHYD=["FC-04"], DT=[1]min, AREA=[12.87](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3496          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3497          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3498          Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[292.916](m), MNI=[0.013], SCI=[0](min),
3499          Continuous simulation parameters:
3500          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3501  *%-----|-----|
3502  COMPUTE DUALHYD NHYDin=["FC-04"], CINLET=[0.741](cms), NINLET=[1],
3503          MajNHYD=["FC-04-MJ"]
3504          MinNHYD=["FC-04-MN"]
3505          TMJSTO=[1794](cu-m)
3506  *%-----|-----|
3507  ADD HYD      NHYDsum=["FC-04-S"], NHYDs to add=["FC-04-MJ"+"FC-04-MN"]
3508  *%-----|-----|
3509  *#*****
3510  *#   PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM Modeling
Approach, NOVATECH Report June, 2020)
3511  *#   - TO JOCK RIVER
3512  *#*****

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3513 CONTINUOUS STANDHYD NHYD=["JR-01"], DT=[1]min, AREA=[8.24](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3514 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3515 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3516 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[234.379](m), MNI=[0.013], SCI=[0](min),
3517 Continuous simulation parameters:
3518 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3519 *%-----|-----|
3520 COMPUTE DUALHYD NHYDin=["JR-01"], CINLET=[0.563](cms), NINLET=[1],
3521 MajNHYD=["JR-01-MJ"]
3522 MinNHYD=["JR-01-MN"]
3523 TMJSTO=[1040](cu-m)
3524 *%-----|-----|
3525 ADD HYD NHYDsum=["JR-01-S"], NHYDs to add=["JR-01-MJ"+"JR-01-MN"]
3526 *%-----|-----|
3527 CONTINUOUS STANDHYD NHYD=["JR-02"], DT=[1]min, AREA=[1.59](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3528 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3529 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3530 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[102.956](m), MNI=[0.013], SCI=[0](min),
3531 Continuous simulation parameters:
3532 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3533 *%-----|-----|
3534 COMPUTE DUALHYD NHYDin=["JR-02"], CINLET=[0.153](cms), NINLET=[1],
3535 MajNHYD=["JR-02-MJ"]
3536 MinNHYD=["JR-02-MN"]
3537 TMJSTO=[153](cu-m)
3538 *%-----|-----|
3539 ADD HYD NHYDsum=["JR-02-S"], NHYDs to add=["JR-02-MJ"+"JR-02-MN"]
3540 *%-----|-----|
3541 *#*****
3542 *# Catchment FRASER
3543 *# - To Fraser-Clarke drain (north of the Jock)
3544 *# - Developed land with assumed 43% imp.
3545 *# - 2020-12-17 Change Fraser area to be 35.1 as measured from QGIS
3546 *# - 2020-12-17 All Fraser is undeveloped (Nashyd)
3547 *#*****
3548 CONTINUOUS NASHYD NHYD=["FRASER-DRN"], DT=[1]min, AREA=[13.65](ha),
3549 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3550 N=[3], TP=[0.4258]hrs,
3551 Continuous simulation parameters:
3552 IaRECper=[4](hrs),
3553 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3554 InterEventTime=[12](hrs)
3555 Baseflow simulation parameters:
3556 BaseFlowOption=[1],
3557 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3558 VHydCond=[0.055](mm/hr), END=-1
3559 *%-----|-----|
3560 *CONTINUOUS STANDHYD NHYD=["FRASER-D"], DT=[1]min, AREA=[21.61](ha),
3561 * XIMP=[0.585], TIMP=[0.585], DWF=[0](cms), LOSS=[2],
3562 * SCS curve number CN=[80],
3563 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3564 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3565 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3566 * LGI=[379.561](m), MNI=[0.013], SCI=[0](min),
3567 * Continuous simulation parameters:
3568 * IaRECper=[4](hrs), IaRECimp=[4](hrs),

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3569 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3570 * InterEventTime=[18](hrs), END=-1
3571 *%-----|-----|
3572 CONTINUOUS NASHYD NHYD=["FRASER-D"], DT=[1]min, AREA=[21.61](ha),
3573 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3574 N=[3], TP=[0.674]hrs,
3575 Continuous simulation parameters:
3576 IaRECper=[4](hrs),
3577 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3578 InterEventTime=[12](hrs)
3579 Baseflow simulation parameters:
3580 BaseFlowOption=[1],
3581 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3582 VHydCond=[0.055](mm/hr), END=-1
3583 *%-----|-----|
3584 *COMPUTE DUALHYD NHYDin=["FRASER-D"], CINLET=[3.545](cms), NINLET=[1],
3585 * MajNHYD=["FRASER-J"]
3586 * MinNHYD=["FRASER-N"]
3587 * TMJSTO=[9999999](cu-m)
3588 *%-----|-----|
3589 *ADD HYD NHYDsum=["FRASER-S"], NHYDs to add=["FRASER-J"+"FRASER-N"]
3590 *%-----|-----|
3591 *ROUTE RESERVOIR NHYDout=["MS_P20"], NHYDin=["FRASER"],
3592 * RDT=[1](min),
3593 * TABLE of ( OUTFLOW-STORAGE ) values
3594 * (cms) - (ha-m)
3595 * [ 0.0 , 0.0 ]
3596 * [ 0.04 , 0.36 ]
3597 * [ -1 , -1 ] (max twenty pts)
3598 * NHYDovf=["P20-OVF"]
3599 *%-----|-----|
3600 ADD HYD NHYDsum=["4241"], NHYDs to
add=["KB-Pond3"+"S-1-B"+"FRASER-DRN"+"FRASER-D"+"N_KB"+"FC-01-S"+"FC-02-S"+"FC-03-S"]
3601 *%-----|-----|
3602 SAVE HYD NHYD=["4241"], # OF PCYCLES=[-1], ICASEsh=[1]
3603 HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
3604 *%-----|-----|
3605 *# Hydrograph from Node Ken-Burnett to station 3633
3606 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4241
3607 *#
3608 ROUTE CHANNEL NHYDout=["4241-out"], NHYDin=["4241"], RDT=[1](min),
3609 CHLGTH=[294](m), CHSLOPE=[0.1088](%), FPSLOPE=[0.1088](%),
3610 SECNUM=[1.0], NSEG=[3]
3611 ( SEGROUGH, SEGDIST (m))=[0.05, -20.12
3612 -0.035, 45.26
3613 0.05, 403.84] NSEG times
3614 ( DISTANCE (m), ELEVATION (m))=[]
3615 [-909.72, 95 ]
3616 [-907.09, 94.5 ]
3617 [-904.65, 94 ]
3618 [-902.26, 93.5 ]
3619 [-44.51, 91.5 ]
3620 [-25.1, 91.5 ]
3621 [-20.98, 91 ]
3622 [-20.61, 90.5 ]
3623 [-20.12, 90 ]
3624 [-6.13, 87.26 ]
3625 [17.51, 86.56 ]
3626 [31.37, 87.2 ]
3627 [45.26, 90 ]
3628 [50.41, 90.5 ]
3629 [63.06, 91 ]
3630 [134.5, 91.5 ]
3631 [190.63, 92 ]
3632 [251.98, 92.5 ]
3633 [321.32, 93.5 ]

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3634         [403.84, 95 ]
3635 *%-----|-----|
3636 ADD HYD      NHYDsum=["SN_KB"], NHYDs to
add=["4241-out"+"FC-04-S"+"JR-01-S"+"JR-02-S"]
3637 *%-----|-----|
3638 SAVE HYD    NHYD=["SN_KB"], # OF PCYCLES=[-1], ICASEsh=[1]
3639           HYD_COMMENT=["Total Flows before Station 3633"]
3640 *%-----|-----|
3641 *# Hydrograph from Station 3633 to Node Todd
3642 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
3643 *# JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and
change the slope from 0.0498% to 0.24671%. That is because of adding station 4241
between station 4534 and station 3633
3644 *#
3645 ROUTE CHANNEL  NHYDout=["N_TO"], NHYDin=["SN_KB"], RDT=[1](min),
3646               CHLGTH=[608](m), CHSLOPE=[0.24671](%), FPSLOPE=[0.24671](%),
3647               SECNUM=[1.0], NSEG=[3]
3648               ( SEGRROUGH, SEGDIST (m))=[0.05, -23.74
3649               -0.035, 23.74
3650               0.05, 26.50] NSEG times
3651               ( DISTANCE (m), ELEVATION (m))=[]
3652               -29.24, 91.0
3653               -27.41, 90.5
3654               -25.64, 90
3655               -23.74, 89.5
3656               -22, 89.26
3657               -20, 88.51
3658               -19, 88.32
3659               -15, 88.1
3660               -10, 88.11
3661               -5, 88.17
3662               0, 88.27
3663               5, 88.19
3664               10, 88.06
3665               15, 88.48
3666               16, 88.7
3667               23.74, 89.5
3668               24.68, 90
3669               25.57, 90.5
3670               26.50, 91.0
3671 *           [-29.24, 91]
3672 *           [-27.41, 90.5]
3673 *           [-25.64, 90]
3674 *           [-23.74, 89.5]
3675 *           [-22, 89.26]
3676 *           [-20, 88.51]
3677 *           [-19, 88.32]
3678 *           [-15, 88.1]
3679 *           [-10, 88.11]
3680 *           [-5, 88.17]
3681 *           [0, 88.27]
3682 *           [5, 88.19]
3683 *           [10, 88.06]
3684 *           [15, 88.48]
3685 *           [16, 88.7]
3686 *           [23.74, 89.5]
3687 *           [24.68, 90]
3688 *           [25.57, 90.5]
3689 *%-----|-----|
3690 *#*****
3691 *# Catchment Greenbank
3692 *# - To Greenbank Drain (south of the Jock)
3693 *# - JFSA 2021-01-18 add Greenbank pond as per JFSA, P598(06)-15, June 2016
3694 *# - JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
3695 *#*****
3696 CONTINUOUS STANDHYD NHYD=["Greenbank"], DT=[1]min, AREA=[36.6](ha),

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3697 XIMP=[0.639], TIMP=[0.682], DWF=[0](cms), LOSS=[2],
3698 SCS curve number CN=[77],
3699 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3700 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3701 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3702 LGI=[493.96](m), MNI=[0.013], SCI=[0](min),
3703 Continuous simulation parameters:
3704 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3705 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3706 InterEventTime=[18](hrs), END=-1
3707 *%-----|-----|
3708 ROUTE RESERVOIR NHYDout=["GreenB_MN"], NHYDin=["Greenbank"],
3709 RDT=[1](min),
3710 TABLE of ( OUTFLOW-STORAGE ) values
3711 (cms) - (ha-m)
3712 [ 0.0 , 0.0 ]
3713 [ 0.033 , 0.084 ]
3714 [ 0.039 , 0.201 ]
3715 [ 0.113 , 0.292 ]
3716 [ 0.237 , 0.386 ]
3717 [ 0.382 , 0.484 ]
3718 [ 0.539 , 0.585 ]
3719 [ 0.7 , 0.692 ]
3720 [ 0.86 , 0.804 ]
3721 [ 4.684 , 0.922 ]
3722 [ 11.539 , 1.052 ]
3723 [ 20.867 , 1.168 ]
3724 [ 103.616 , 1.974 ]
3725 [ -1 , -1 ] (max twenty pts)
3726 NHYDovf=["GreenB_MJ"],
3727 *%-----|-----|
3728 *%-----|-----|
3729 ADD HYD NHYDsum=["GreenB"], NHYDs to add=["N_TO"+"GreenB_MJ"+"GreenB_MN"]
3730 *%-----|-----|
3731 SAVE HYD NHYD=["GreenB"], # OF PCYCLES=[-1], ICASEsh=[1]
3732 HYD_COMMENT=["Total Flows at Greenbank Drain"]
3733 *%-----|-----|
3734 *#*****|*****|
3735 *# Catchment TODD
3736 *# - To Todd Drain (south of the Jock)
3737 *# - Subdivision with 43% imp. as per Barrhaven South MSS
3738 *# - 2020-11-30 increase imp. based on P598(04)-11
3739 *# - 2020-11-30 update TODD Tributary Drainage Area to = 146.015 ha based on
P598(04)-11
3740 *# - 2020-11-30 split TODD Drainage Area to MAJOR, MINOR, POND and ALL
3741 *#*****|*****|
3742 *# - JFSA 2021-01-19 add "TODD_MN1" as part of Clarke("W_CLAR_MJ") and remove it
from Todd
3743 *CONTINUOUS STANDHYD NHYD=["TODD_MN1"], DT=[1]min, AREA=[1.772](ha),
3744 * XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3745 * SCS curve number CN=[77],
3746 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3747 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3748 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3749 * LGI=[108.689](m), MNI=[0.013], SCI=[0](min),
3750 * Continuous simulation parameters:
3751 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
3752 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3753 * InterEventTime=[18](hrs), END=-1
3754 *%-----|-----|
3755 CONTINUOUS STANDHYD NHYD=["TODD_MN2"], DT=[1]min, AREA=[2.1](ha),
3756 XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3757 SCS curve number CN=[77],
3758 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3759 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3760 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),

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3761             LGI=[118.322](m), MNI=[0.013], SCI=[0](min),
3762 Continuous simulation parameters:
3763 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3764 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3765 InterEventTime=[18](hrs), END=-1
3766 *%-----|-----|
3767 CONTINUOUS STANDHYD NHYD=["TODD_MN3"], DT=[1]min, AREA=[0.117](ha),
3768 XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3769 SCS curve number CN=[77],
3770 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3771 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3772 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3773 LGI=[27.928](m), MNI=[0.013], SCI=[0](min),
3774 Continuous simulation parameters:
3775 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3776 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3777 InterEventTime=[18](hrs), END=-1
3778 *%-----|-----|
3779 CONTINUOUS STANDHYD NHYD=["TODD_MJ"], DT=[1]min, AREA=[30.230](ha),
3780 XIMP=[0.52], TIMP=[0.64], DWF=[0](cms), LOSS=[2],
3781 SCS curve number CN=[77],
3782 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3783 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3784 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3785 LGI=[448.925](m), MNI=[0.013], SCI=[0](min),
3786 Continuous simulation parameters:
3787 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3788 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3789 InterEventTime=[18](hrs), END=-1
3790 *%-----|-----|
3791 * -JFSA, 2021-01-19 update "TODD_ALL" area from 108.741 ha to 112.908 ha based on
GIS measurements (148.41-30.23-0.117-2.1-3.055=112.908 ha)
3792 CONTINUOUS STANDHYD NHYD=["TODD_ALL"], DT=[1]min, AREA=[112.908](ha),
3793 XIMP=[0.52], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3794 SCS curve number CN=[77],
3795 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3796 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3797 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3798 LGI=[867.594](m), MNI=[0.013], SCI=[0](min),
3799 Continuous simulation parameters:
3800 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3801 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3802 InterEventTime=[18](hrs), END=-1
3803 *%-----|-----|
3804 CONTINUOUS STANDHYD NHYD=["TODD_P"], DT=[1]min, AREA=[3.055](ha),
3805 XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3806 SCS curve number CN=[77],
3807 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3808 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3809 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3810 LGI=[142.712](m), MNI=[0.013], SCI=[0](min),
3811 Continuous simulation parameters:
3812 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3813 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3814 InterEventTime=[18](hrs), END=-1
3815 *%-----|-----|
3816 *%-----|-----|
3817 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3818 *CONTINUOUS STANDHYD NHYD=["TODD_DEVL"], DT=[1]min, AREA=[15.87](ha),
3819 * XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3820 * SCS curve number CN=[77],
3821 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3822 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3823 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),

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3824 *           LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
3825 *           Continuous simulation parameters:
3826 *           IaRECper=[4](hrs), IaRECimp=[4](hrs),
3827 *           SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3828 *           InterEventTime=[18](hrs), END=-1
3829 *%-----|-----
3830 *           -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3831 *CONTINUOUS NASHYD  NHYD=["TODD_UnD"], DT=[1]min, AREA=[12.47](ha),
3832 *           DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3833 *           N=[3], TP=[1.10]hrs,
3834 *           Continuous simulation parameters:
3835 *           IaRECper=[4](hrs),
3836 *           SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3837 *           InterEventTime=[12](hrs)
3838 *           Baseflow simulation parameters:
3839 *           BaseFlowOption=[1],
3840 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3841 *           VHydCond=[0.055](mm/hr), END=-1
3842 *%-----|-----
3843 *# 5-Year + 12% Capture
3844 *COMPUTE DUALHYD  NHYDin=["TODD_MJ"], CINLET=[3.314](cms), NINLET=[1],
3845 *           MajNHYD=["TODD_MJj"]
3846 *           MinNHYD=["TODD_MJn"]
3847 *           TMJSTO=[0.1](cu-m)
3848 ROUTE RESERVOIR  NHYDout=["TODD_MJn"],NHYDin=["TODD_MJ"],
3849 RDT=[1](min),
3850           TABLE of ( OUTFLOW-STORAGE ) values
3851           (cms) - (ha-m)
3852           [ 0.0 , 0.0 ]
3853           [ 3.314 , 0.0001 ]
3854           [ -1 , -1 ] (max twenty pts)
3855           NHYDovf=["TODD_MJj"],
3856 *%-----|-----
3857 *# 5-Year + 12% Capture
3858 *COMPUTE DUALHYD  NHYDin=["TODD_MN1"], CINLET=[0.227](cms), NINLET=[1],
3859 *           MajNHYD=["TODD_MN1j"]
3860 *           MinNHYD=["TODD_MN1n"]
3861 *           TMJSTO=[0.1](cu-m)
3862 *ROUTE RESERVOIR  NHYDout=["TODD_MN1n"],NHYDin=["TODD_MN1"],
3863 RDT=[1](min),
3864           TABLE of ( OUTFLOW-STORAGE ) values
3865           (cms) - (ha-m)
3866           [ 0.0 , 0.0 ]
3867           [ 0.227 , 0.0001 ]
3868           [ -1 , -1 ] (max twenty pts)
3869           NHYDovf=["TODD_MN1j"],
3870 *%-----|-----
3871 *COMPUTE DUALHYD  NHYDin=["TODD_MN2"], CINLET=[0.268](cms), NINLET=[1],
3872 *           MajNHYD=["TODD_MN2j"]
3873 *           MinNHYD=["TODD_MN2n"]
3874 *           TMJSTO=[0.1](cu-m)
3875 ROUTE RESERVOIR  NHYDout=["TODD_MN2n"],NHYDin=["TODD_MN2"],
3876 RDT=[1](min),
3877           TABLE of ( OUTFLOW-STORAGE ) values
3878           (cms) - (ha-m)
3879           [ 0.0 , 0.0 ]
3880           [ 0.268 , 0.0001 ]
3881           [ -1 , -1 ] (max twenty pts)
3882           NHYDovf=["TODD_MN2j"],
3883 *%-----|-----
3884 *COMPUTE DUALHYD  NHYDin=["TODD_MN3"], CINLET=[0.016](cms), NINLET=[1],
3885 *           MajNHYD=["TODD_MN3j"]
3886 *           MinNHYD=["TODD_MN3n"]
3887 *           TMJSTO=[0.1](cu-m)
3888 ROUTE RESERVOIR  NHYDout=["TODD_MN3n"],NHYDin=["TODD_MN3"],

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3889         RDT=[1](min),
3890         TABLE of ( OUTFLOW-STORAGE ) values
3891             (cms) - (ha-m)
3892             [ 0.0 , 0.0 ]
3893             [ 0.016 , 0.0001 ]
3894             [ -1 , -1 ] (max twenty pts)
3895         NHYDovf=["TODD_MN3j"] ,
3896 *%-----|-----|
3897 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
3898 CONTINUOUS STANDHYD NHYD=["A2"], DT=[1]min, AREA=[25.5](ha),
3899 XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
3900 SCS curve number CN=[75],
3901 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3902 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3903 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3904 LGI=[566](m), MNI=[0.013], SCI=[0](min),
3905 Continuous simulation parameters:
3906 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3907 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3908 InterEventTime=[18](hrs), END=-1
3909 *%-----|-----|
3910 COMPUTE DUALHYD NHYDin=["A2"], CINLET=[1.818](cms), NINLET=[1],
3911 MajNHYD=["A2-MJ"]
3912 MinNHYD=["A2-MN"]
3913 TMJSTO=[924](cu-m)
3914 *%-----|-----|
3915 ADD HYD NHYDsum=["TODD"], NHYDs to
add=["TODD_MN2n"+"TODD_MN3n"+"TODD_MJj"+"TODD_P"+"TODD_ALL"+"W_CLAR_MJn"]
3916 *%-----|-----|
3917 SAVE HYD NHYD=["TODD"], # OF PCYCLES=[-1], ICASEsh=[1]
3918 HYD_COMMENT=["Total Flows at Todd Drain"]
3919 *%-----|-----|
3920 *#*****
3921 *# Todd Pond 3
3922 *# - Rating curve obtained from Barrhaven South MSS modeling
3923 *# - stantec 2007, Tributary Drainage Area to MSS Pond 3 = 193 ha
3924 *#*****
3925 ROUTE RESERVOIR NHYDout=["MS_P3"], NHYDin=["TODD"],
3926 RDT=[1](min),
3927 TABLE of ( OUTFLOW-STORAGE ) values
3928     (cms) - (ha-m)
3929     [ 0.0 , 0.0 ]
3930     [ 0.014 , 0.155 ]
3931     [ 0.048 , 0.394 ]
3932     [ 0.061 , 0.56 ]
3933     [ 0.08 , 0.909 ]
3934     [ 0.088 , 1.089 ]
3935     [ 0.109 , 1.652 ]
3936     [ 0.118 , 1.952 ]
3937     [ 0.122 , 2.099 ]
3938     [ 1.972 , 2.269 ]
3939     [ 9.135 , 2.598 ]
3940     [ 15.608 , 2.826 ]
3941     [ 19.256 , 2.942 ]
3942     [ 27.282 , 3.181 ]
3943     [ 40.957 , 3.55 ]
3944     [ 56.372 , 3.929 ]
3945     [ 73.349 , 4.317 ]
3946     [ 85.469 , 4.579 ]
3947     [ 104.771 , 4.977 ]
3948     [ -1 , -1 ] (max twenty pts)
3949     NHYDovf=["P3-OVF"]
3950 *%-----|-----|
3951 ADD HYD NHYDsum=["SN_TO"], NHYDs to
add=["GreenB"+"MS_P3"+"P3-OVF"+"TODD_MN2j"+"A2-MJ"]

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3952 *%-----|-----|
3953 SAVE HYD      NHYD=["SN_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3954              HYD_COMMENT=["Total Flows at Todd Drain"]
3955 *%-----|-----|
3956 *#
3957 *# Hydrograph from Todd Drain routed to Corrigan Drain
3958 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
3959 *# 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so
the model will be more stable and give reasonable results. It is justifiable as ROUTE
CHANNELs aren't well suited to really flat slopes.
3960 *
3961 ROUTE CHANNEL  NHYDout=["N_TO"] ,NHYDin=["SN_TO"] ,
3962              RDT=[1](min),
3963              CHLGTH=[280](m),  CHSLOPE=[0.05](%),
3964              FPSLOPE=[0.05](%),
3965              SECNUM=[1.0],      NSEG=[3]
3966              ( SEGROUGH, SEGDIST (m))=
3967                [0.075,-17.72
3968                 -0.045,17.72
3969                 0.075,80.62] NSEG times
3970              ( DISTANCE (m), ELEVATION (m))=
3971                [-83.32, 90.00]
3972                [-81.36, 89.50]
3973                [-79.12, 89.00]
3974                [-76.13, 88.50]
3975                [-20.46, 88.00]
3976                [-19.36, 87.50]
3977                [-18.51, 87.00]
3978                [-17.72, 86.50]
3979                [-11.95, 85.24]
3980                [-0.11, 85.12]
3981                [11.49, 85.20]
3982                [17.72, 86.50]
3983                [19.74, 87.00]
3984                [21.22, 87.50]
3985                [22.68, 88.00]
3986                [24.28, 88.50]
3987                [26.79, 89.00]
3988                [71.98, 90.00]
3989                [80.62, 90.50]
3990 *%-----|-----|
3991 SAVE HYD      NHYD=["N_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3992              HYD_COMMENT=["Total inflows at Station 2462"]
3993 *%-----|-----|
3994 *#*****
3995 *# Catchment CORRIG
3996 *# - To Corrigan Drain (south of the Jock)
3997 *# - Primarily Developed (medium density)
3998 *# - JFSA JAN 2021, add Corrigan subcatchments as per IBI, July 2008
3999 *#*****
4000 *ROUTE RESERVOIR  NHYDout=["MS_P1"], NHYDin=["CORRIG"],
4001 *              RDT=[1](min),
4002 *              TABLE of ( OUTFLOW-STORAGE ) values
4003 *                  (cms) - (ha-m)
4004 *                  [ 0.0 , 0.0 ]
4005 *                  [ 0.06 , 0.58]
4006 *                  [ -1 , -1 ] (max twenty pts)
4007 *              NHYDovf=["P1-OVF"]
4008 *%-----|-----|
4009 *ADD HYD      NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
4010 *%-----|-----|
4011 *SAVE HYD      NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
4012 *              HYD_COMMENT=["Total Flows at Corrigan Drain"]
4013 *%-----|-----|
4014 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"

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and its parameters remain the same.
4015 CONTINUOUS STANDHYD NHYD=["corr1"], DT=[1]min, AREA=[15.87](ha),
4016 XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
4017 SCS curve number CN=[77],
4018 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4019 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4020 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4021 LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
4022 Continuous simulation parameters:
4023 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4024 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4025 InterEventTime=[18](hrs), END=-1
4026 *%-----|-----|
4027 * -JFSA 2021-02-23 add DUALHYD for "corr1". "corr1" DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to "corr1".
4028 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
4029 COMPUTE DUALHYD NHYDin=["corr1"], CINLET=[1.818](cms), NINLET=[1],
4030 MajNHYD=["corr1-MJ"]
4031 MinNHYD=["corr1-MN"]
4032 TMJSTO=[924](cu-m)
4033 *%-----|-----|
4034 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
4035 CONTINUOUS NASHYD NHYD=["corr2"], DT=[1]min, AREA=[12.47](ha),
4036 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
4037 N=[3], TP=[1.10]hrs,
4038 Continuous simulation parameters:
4039 IaREcper=[4](hrs),
4040 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4041 InterEventTime=[12](hrs)
4042 Baseflow simulation parameters:
4043 BaseFlowOption=[1] ,
4044 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4045 VHydCond=[0.055](mm/hr), END=-1
4046 *%-----|-----|
4047 * -JFSA 2021-01-19 change Al-Corrig to be developed as per geottawa website and
apply the parameters of A2, the nearest sub-catchment to Al-Corrig, LGI is calculated
based on Al-Corrig area
4048 * -JFSA 2021-01-19 update all Corrigan areas based on GIS measurements, and keep
LGI as it is from Corrigan Report, IBI Group, 2008 because LGI calculated is less than
LGI from the Corrigan Report
4049 CONTINUOUS STANDHYD NHYD=["Al-Corrig"], DT=[1]min, AREA=[15.75](ha),
4050 XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
4051 SCS curve number CN=[75],
4052 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4053 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4054 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4055 LGI=[324.037](m), MNI=[0.013], SCI=[0](min),
4056 Continuous simulation parameters:
4057 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4058 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4059 InterEventTime=[18](hrs), END=-1
4060 *
4061 * -JFSA 2021-01-25 add DUALHYD for Al-Corrig. Al-Corrig DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to Al-Corrig.
4062 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
4063 COMPUTE DUALHYD NHYDin=["Al-Corrig"], CINLET=[1.818](cms), NINLET=[1],
4064 MajNHYD=["Al-MJ"]
4065 MinNHYD=["Al-MN"]
4066 TMJSTO=[924](cu-m)
4067 *%-----|-----|
4068 *CONTINUOUS NASHYD NHYD=["Al-Corrig"], DT=[1]min, AREA=[15.75](ha),
4069 * DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
4070 * N=[3.0], TP=[0.36]hrs,

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4071 *           Continuous simulation parameters:
4072 *           IaREcper=[4](hrs),
4073 *           SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
4074 *           InterEventTime=[12](hrs)
4075 *           Baseflow simulation parameters:
4076 *           BaseFlowOption=[1] ,
4077 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4078 *           VHydCond=[0.055](mm/hr),  END=-1
4079 *%-----|-----
4080 CONTINUOUS NASHYD NHYD=["B1"], DT=[1]min, AREA=[2.77](ha),
4081 DWF=[0](cms),  CN/C=[56], IA=[2.5](mm),
4082 N=[3.0], TP=[0.23]hrs,
4083 Continuous simulation parameters:
4084 IaREcper=[4](hrs),
4085 SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
4086 InterEventTime=[12](hrs)
4087 Baseflow simulation parameters:
4088 BaseFlowOption=[1] ,
4089 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4090 VHydCond=[0.055](mm/hr),  END=-1
4091 *%-----|-----
4092 CONTINUOUS STANDHYD NHYD=["A4"], DT=[1]min, AREA=[1.27](ha),
4093 XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
4094 SCS curve number CN=[75],
4095 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4096 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4097 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4098 LGI=[253](m), MNI=[0.013], SCI=[0](min),
4099 Continuous simulation parameters:
4100 IaREcper=[4](hrs),  IaREcimp=[4](hrs),
4101 SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
4102 InterEventTime=[18](hrs),  END=-1
4103 *%-----|-----
4104 COMPUTE DUALHYD NHYDin=["A4"], CINLET=[0.405](cms), NINLET=[1],
4105 MajNHYD=["A4-MJ"]
4106 MinNHYD=["A4-MN"]
4107 TMJSTO=[68](cu-m)
4108 *%-----|-----
4109 ADD HYD NHYDsum=["MH101"], NHYDs to
add=["A1-MJ"+"A1-MN"+"corr1-MJ"+"corr1-MN"+"corr2"+"B1"+"A4-MN"]
4110 *%-----|-----
4111 SAVE HYD NHYD=["MH101"],  # OF PCYCLES=[-1], ICASEsh=[1]
4112 HYD_COMMENT=["Total Flows at MH101"]
4113 *%-----|-----
4114 ROUTE PIPE PTYPE=[1]circ, NHYDout=["101-102"], RNUMBER=[1.0], PDIAM=[1050](mm),
4115 PLNGTH=[368](m), PROUGH=[0.013], PSLOPE=[0.0054](m/m),
NHYDin=["MH101"], RDT=[1]
4116 *%-----|-----
4117 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
4118 *CONTINUOUS STANDHYD NHYD=["A2"], DT=[1]min, AREA=[25.5](ha),
4119 * XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
4120 * SCS curve number CN=[75],
4121 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4122 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
4123 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4124 * LGI=[566](m), MNI=[0.013], SCI=[0](min),
4125 * Continuous simulation parameters:
4126 * IaREcper=[4](hrs),  IaREcimp=[4](hrs),
4127 * SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
4128 * InterEventTime=[18](hrs),  END=-1
4129 *%-----|-----
4130 *COMPUTE DUALHYD NHYDin=["A2"], CINLET=[1.818](cms), NINLET=[1],
4131 * MajNHYD=["A2-MJ"]
4132 * MinNHYD=["A2-MN"]
4133 * TMJSTO=[924](cu-m)

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4134 *%-----|-----|
4135 ADD HYD      NHYDsum=["MH102"], NHYDs to add=["A2-MN"+"101-102"]
4136 *%-----|-----|
4137 SAVE HYD     NHYD=["MH102"], # OF PCYCLES=[-1], ICASEsh=[1]
4138             HYD_COMMENT=["Total Flows at MH102"]
4139 *%-----|-----|
4140 CONTINUOUS STANDHYD NHYD=["A5"], DT=[1]min, AREA=[1.6](ha),
4141             XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4142             SCS curve number CN=[75],
4143             Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4144             LGP=[40](m), MNP=[0.25], SCP=[0](min),
4145             Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4146             LGI=[300](m), MNI=[0.013], SCI=[0](min),
4147             Continuous simulation parameters:
4148             IaRECper=[4](hrs), IaRECimp=[4](hrs),
4149             SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4150             InterEventTime=[18](hrs), END=-1
4151 *%-----|-----|
4152 ADD HYD      NHYDsum=["A5T"], NHYDs to add=["A4-MJ"+"A5"]
4153 *%-----|-----|
4154 COMPUTE DUALHYD NHYDin=["A5T"], CINLET=[0.357](cms), NINLET=[1],
4155             MajNHYD=["A5-MJ"]
4156             MinNHYD=["A5-MN"]
4157             TMJSTO=[60](cu-m)
4158 *%-----|-----|
4159 * -JFSA Jan. 2021, A3 is a part of Todd so it is removed
4160 * -JFSA Jan. 2021, "A2-MJ" added to "Todd"
4161 *CONTINUOUS STANDHYD NHYD=["A3"], DT=[1]min, AREA=[18.4](ha),
4162 *             XIMP=[0.58], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
4163 *             SCS curve number CN=[75],
4164 *             Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4165 *             LGP=[40](m), MNP=[0.25], SCP=[0](min),
4166 *             Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4167 *             LGI=[450](m), MNI=[0.013], SCI=[0](min),
4168 *             Continuous simulation parameters:
4169 *             IaRECper=[4](hrs), IaRECimp=[4](hrs),
4170 *             SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4171 *             InterEventTime=[18](hrs), END=-1
4172 *%-----|-----|
4173 *ADD HYD      NHYDsum=["A3-A2MJ"], NHYDs to add=["A2-MJ"+"A3"]
4174 *%-----|-----|
4175 *COMPUTE DUALHYD NHYDin=["A3-A2MJ"], CINLET=[2.208](cms), NINLET=[1],
4176 *             MajNHYD=["A3R-MJ"]
4177 *             MinNHYD=["A3R-MN"]
4178 *             TMJSTO=[908](cu-m)
4179 *%-----|-----|
4180 ROUTE PIPE    PTYPE=[1]circ, NHYDout=["102-103"], RNUMBER=[1.0], PDIAM=[1500](mm),
4181             PLNGTH=[504](m), PROUGH=[0.013], PSLOPE=[0.0028](m/m),
4182             NHYDin=["MH102"], RDT=[1]
4183 *%-----|-----|
4184 ADD HYD      NHYDsum=["MH103"], NHYDs to add=["102-103"+"A5-MN"]
4185 *%-----|-----|
4186 SAVE HYD     NHYD=["MH103"], # OF PCYCLES=[-1], ICASEsh=[1]
4187             HYD_COMMENT=["Total Flows at MH103"]
4188 *%-----|-----|
4189 ROUTE PIPE    PTYPE=[1]circ, NHYDout=["103-104"], RNUMBER=[1.0], PDIAM=[1650](mm),
4190             PLNGTH=[438](m), PROUGH=[0.013], PSLOPE=[0.0046](m/m),
4191             NHYDin=["MH103"], RDT=[1]
4192 *%-----|-----|
4193 CONTINUOUS STANDHYD NHYD=["A6"], DT=[1]min, AREA=[1.56](ha),
4194             XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4195             SCS curve number CN=[75],
4196             Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4197             LGP=[40](m), MNP=[0.25], SCP=[0](min),
4198             Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4199             LGI=[280](m), MNI=[0.013], SCI=[0](min),

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4198 Continuous simulation parameters:
4199 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4200 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4201 InterEventTime=[18](hrs), END=-1
4202 *%-----|-----|
4203 ADD HYD NHYDsum=["A6T"], NHYDs to add=["A5-MJ"+"A6"]
4204 *%-----|-----|
4205 COMPUTE DUALHYD NHYDin=["A6T"], CINLET=[0.357](cms), NINLET=[1],
4206 MajNHYD=["A6-MJ"]
4207 MinNHYD=["A6-MN"]
4208 TMJSTO=[60](cu-m)
4209 *%-----|-----|
4210 * -JFSA Jan. 2021, A7-corrig is a part of Todd so it is removed
4211 *CONTINUOUS STANDHYD NHYD=["A7-corrig"], DT=[1]min, AREA=[11.8](ha),
4212 * XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4213 * SCS curve number CN=[75],
4214 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4215 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
4216 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4217 * LGI=[438](m), MNI=[0.013], SCI=[0](min),
4218 * Continuous simulation parameters:
4219 * IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4220 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4221 * InterEventTime=[18](hrs), END=-1
4222 *%-----|-----|
4223 *ADD HYD NHYDsum=["A7-A3RMJ"], NHYDs to add=["A3R-MJ"+"A7-corrig"]
4224 *%-----|-----|
4225 *COMPUTE DUALHYD NHYDin=["A7-A3RMJ"], CINLET=[1.003](cms), NINLET=[1],
4226 * MajNHYD=["A7R-MJ"]
4227 * MinNHYD=["A7R-MN"]
4228 * TMJSTO=[496](cu-m)
4229 *%-----|-----|
4230 ADD HYD NHYDsum=["MH104"], NHYDs to add=["A6-MN"+"103-104"+"TODD_MJn"]
4231 *%-----|-----|
4232 SAVE HYD NHYD=["MH104"], # OF PCYCLES=[-1], ICASEsh=[1]
4233 HYD_COMMENT=["Total Flows at MH104"]
4234 *%-----|-----|
4235 CONTINUOUS STANDHYD NHYD=["B2"], DT=[1]min, AREA=[12.31](ha),
4236 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4237 SCS curve number CN=[75],
4238 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4239 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4240 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4241 LGI=[417](m), MNI=[0.013], SCI=[0](min),
4242 Continuous simulation parameters:
4243 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4244 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4245 InterEventTime=[18](hrs), END=-1
4246 *%-----|-----|
4247 COMPUTE DUALHYD NHYDin=["B2"], CINLET=[1.029](cms), NINLET=[1],
4248 MajNHYD=["B2-MJ"]
4249 MinNHYD=["B2-MN"]
4250 TMJSTO=[508](cu-m)
4251 *%-----|-----|
4252 ROUTE PIPE PTYPE=[1]circ, NHYDout=["315-333"], RNUMBER=[1.0], PDIAM=[1200](mm),
4253 PLNGTH=[254](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["B2-MN"], RDT=[1]
4254 *%-----|-----|
4255 CONTINUOUS STANDHYD NHYD=["B3"], DT=[1]min, AREA=[5.59](ha),
4256 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4257 SCS curve number CN=[75],
4258 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4259 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4260 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4261 LGI=[345](m), MNI=[0.013], SCI=[0](min),
4262 Continuous simulation parameters:

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4263 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4264 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4265 InterEventTime=[18](hrs), END=-1
4266 *%-----|-----|
4267 COMPUTE DUALHYD NHYDin=["B3"], CINLET=[0.459](cms), NINLET=[1],
4268 MajNHYD=["B3-MJ"]
4269 MinNHYD=["B3-MN"]
4270 TMJSTO=[227](cu-m)
4271 *%-----|-----|
4272 ADD HYD NHYDsum=["MH333"], NHYDs to add=["B3-MN"+"315-333"]
4273 *%-----|-----|
4274 SAVE HYD NHYD=["MH333"], # OF PCYCLES=[-1], ICASEsh=[1]
4275 HYD_COMMENT=["Total Flows at MH333"]
4276 *%-----|-----|
4277 ROUTE PIPE PTYPE=[1]circ, NHYDout=["333-335"], RNUMBER=[1.0], PDIAM=[1200](mm),
4278 PLNGTH=[251](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH333"], RDT=[1]
4279 *%-----|-----|
4280 ROUTE PIPE PTYPE=[1]circ, NHYDout=["335-338"], RNUMBER=[1.0], PDIAM=[1200](mm),
4281 PLNGTH=[185](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["333-335"], RDT=[1]
4282 *%-----|-----|
4283 ROUTE PIPE PTYPE=[1]circ, NHYDout=["338-340"], RNUMBER=[1.0], PDIAM=[1350](mm),
4284 PLNGTH=[233](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["335-338"], RDT=[1]
4285 *%-----|-----|
4286 CONTINUOUS STANDHYD NHYD=["B4"], DT=[1]min, AREA=[7.6](ha),
4287 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4288 SCS curve number CN=[75],
4289 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4290 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4291 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4292 LGI=[388](m), MNI=[0.013], SCI=[0](min),
4293 Continuous simulation parameters:
4294 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4295 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4296 InterEventTime=[18](hrs), END=-1
4297 *%-----|-----|
4298 COMPUTE DUALHYD NHYDin=["B4"], CINLET=[0.655](cms), NINLET=[1],
4299 MajNHYD=["B4-MJ"]
4300 MinNHYD=["B4-MN"]
4301 TMJSTO=[323](cu-m)
4302 *%-----|-----|
4303 ADD HYD NHYDsum=["MH340"], NHYDs to add=["338-340"+"B4-MN"]
4304 *%-----|-----|
4305 SAVE HYD NHYD=["MH340"], # OF PCYCLES=[-1], ICASEsh=[1]
4306 HYD_COMMENT=["Total Flows at MH340"]
4307 *%-----|-----|
4308 ROUTE PIPE PTYPE=[1]circ, NHYDout=["340-104"], RNUMBER=[1.0], PDIAM=[1650](mm),
4309 PLNGTH=[240](m), PROUGH=[0.013], PSLOPE=[0.0015](m/m),
NHYDin=["MH340"], RDT=[1]
4310 *%-----|-----|
4311 ADD HYD NHYDsum=["MH104T"], NHYDs to add=["340-104"+"MH104"]
4312 *%-----|-----|
4313 ROUTE PIPE PTYPE=[2]rect, NHYDout=["104-105"], RNUMBER=[1.0],
PWIDTh=[2400](mm) by PHEIGHT=[2100](mm),
4314 PLNGTH=[380](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH104T"], RDT=[1]
4315 *%-----|-----|
4316 CONTINUOUS STANDHYD NHYD=["B5"], DT=[1]min, AREA=[2.2](ha),
4317 XIMP=[0.57], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
4318 SCS curve number CN=[75],
4319 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4320 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4321 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),

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4322             LGI=[187](m), MNI=[0.013], SCI=[0](min),
4323 Continuous simulation parameters:
4324 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4325 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4326 InterEventTime=[18](hrs), END=-1
4327 *%-----|-----|
4328 COMPUTE DUALHYD NHYDin=["B5"], CINLET=[0.260](cms), NINLET=[1],
4329 MajNHYD=["B5-MJ"]
4330 MinNHYD=["B5-MN"]
4331 TMJSTO=[250](cu-m)
4332 *%-----|-----|
4333 CONTINUOUS STANDHYD NHYD=["A8"], DT=[1]min, AREA=[0.96](ha),
4334 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4335 SCS curve number CN=[75],
4336 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4337 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4338 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4339 LGI=[186](m), MNI=[0.013], SCI=[0](min),
4340 Continuous simulation parameters:
4341 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4342 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4343 InterEventTime=[18](hrs), END=-1
4344 *%-----|-----|
4345 ADD HYD NHYDsum=["A8T"], NHYDs to add=["A6-MJ"+"A8"]
4346 *%-----|-----|
4347 COMPUTE DUALHYD NHYDin=["A8T"], CINLET=[0.238](cms), NINLET=[1],
4348 MajNHYD=["A8-MJ"]
4349 MinNHYD=["A8-MN"]
4350 TMJSTO=[40](cu-m)
4351 *%-----|-----|
4352 ADD HYD NHYDsum=["MH105"], NHYDs to
add=["104-105"+"B5-MN"+"A8-MN"+"TODD_MN3j"]
4353 *%-----|-----|
4354 SAVE HYD NHYD=["MH105"], # OF PCYCLES=[-1], ICASEsh=[1]
4355 HYD_COMMENT=["Total Flows at MH105"]
4356 *%-----|-----|
4357 DIVERT HYD NHYDin=["A8-MJ"] NIDout=[2]max five,
4358 outflow hydrographs (NHYDs)=["A8-MJ-JR" "A8-MJ-B6"]
4359 flow distribution table: (modify as necessary)
4360 Note: all flows are in (cms)
4361 QIDi + QIDii = QTOTAL
4362 [ 0 + 0 = 0 ]
4363 [ 50 + 50 = 100 ] end
4364 *%-----|-----|
4365 DIVERT HYD NHYDin=["MH105"] NIDout=[2]max five,
4366 outflow hydrographs (NHYDs)=["MH105-JR" "MH105-B6"]
4367 flow distribution table: (modify as necessary)
4368 Note: all flows are in (cms)
4369 QIDi + QIDii = QTOTAL
4370 [ 0 + 0 = 0 ]
4371 [ 0 + 3.0 = 3.0 ]
4372 [ 96.9+ 3.1 = 100 ] end
4373 *%-----|-----|
4374 CONTINUOUS STANDHYD NHYD=["B7"], DT=[1]min, AREA=[7.19](ha),
4375 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4376 SCS curve number CN=[75],
4377 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4378 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4379 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4380 LGI=[211](m), MNI=[0.013], SCI=[0](min),
4381 Continuous simulation parameters:
4382 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4383 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4384 InterEventTime=[18](hrs), END=-1

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4385 *%-----|-----|
4386 ADD HYD NHYDsum=["B7-B4MJ"], NHYDs to add=["B4-MJ"+"B7"]
4387 *%-----|-----|
4388 COMPUTE DUALHYD NHYDin=["B7-B4MJ"], CINLET=[0.629](cms), NINLET=[1],
4389 MajNHYD=["B7R-MJ"]
4390 MinNHYD=["B7R-MN"]
4391 TMJSTO=[311](cu-m)
4392 *%-----|-----|
4393 ROUTE PIPE PTYPE=[1]circ, NHYDout=["360-106A"], RNUMBER=[1.0], PDIAM=[1050](mm),
4394 PLNGTH=[167](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["B7R-MN"], RDT=[1]
4395 *%-----|-----|
4396 * -JFSA 2021-01-19 change B6 to be developed as per geottawa website and apply the
parameters of A7, the nearest sub-catchment to B6, LGI is calculated based on B6 area
4397 CONTINUOUS STANDHYD NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4398 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4399 SCS curve number CN=[75],
4400 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4401 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4402 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4403 LGI=[148.099](m), MNI=[0.013], SCI=[0](min),
4404 Continuous simulation parameters:
4405 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4406 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4407 InterEventTime=[18](hrs), END=-1
4408 *%-----|-----|
4409 * -JFSA 2021-01-25 add B1 DUALHYD as per Corrigan Report, IBI Group, 2008
4410 COMPUTE DUALHYD NHYDin=["B6"], CINLET=[0.064](cms), NINLET=[1],
4411 MajNHYD=["B6-MJ"]
4412 MinNHYD=["B6-MN"]
4413 TMJSTO=[5484](cu-m)
4414 *%-----|-----|
4415 *CONTINUOUS NASHYD NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4416 * DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
4417 * N=[3.0], TP=[0.36]hrs,
4418 * Continuous simulation parameters:
4419 * IaRECper=[4](hrs),
4420 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4421 * InterEventTime=[12](hrs)
4422 * Baseflow simulation parameters:
4423 * BaseFlowOption=[1],
4424 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4425 * VHydCond=[0.055](mm/hr), END=-1
4426 *%-----|-----|
4427 *% -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
4428 CONTINUOUS STANDHYD NHYD=["EX-LAND"], DT=[1]min, AREA=[32.5](ha),
4429 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
4430 SCS curve number CN=[74],
4431 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4432 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4433 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4434 LGI=[465.475](m), MNI=[0.013], SCI=[0](min),
4435 Continuous simulation parameters:
4436 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4437 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4438 InterEventTime=[18](hrs), END=-1
4439 *%-----|-----|
4440 COMPUTE DUALHYD NHYDin=["EX-LAND"], CINLET=[2.275](cms), NINLET=[1],
4441 MajNHYD=["EX-LAND-MJ"]
4442 MinNHYD=["EX-LAND-MN"]
4443 TMJSTO=[1365](cu-m)
4444 *%-----|-----|
4445 ADD HYD NHYDsum=["B6-B7ExMJ"], NHYDs to
add=["B7R-MJ"+"EX-LAND-MJ"+"B5-MJ"+"B6-MJ"+"B6-MN"+"A8-MJ-B6"]
4446 *%-----|-----|

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4447 COMPUTE DUALHYD NHYDin=["B6-B7ExMJ"], CINLET=[0.064](cms), NINLET=[1],
4448 MajNHYD=["B6R-MJ"]
4449 MinNHYD=["B6R-MN"]
4450 TMJSTO=[5484](cu-m)
4451 *%-----|-----|
4452 ROUTE PIPE PTYPE=[1]circ, NHYDout=["105-106A"], RNUMBER=[1.0], PDIAM=[1800](mm),
4453 PLNGTH=[208](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH105-B6"], RDT=[1]
4454 *%-----|-----|
4455 ADD HYD NHYDsum=["MH106A"], NHYDs to
add=["360-106A"+"105-106A"+"B6R-MN"+"B6R-MJ"]
4456 *%-----|-----|
4457 SAVE HYD NHYD=["MH106A"], # OF PCYCLES=[-1], ICASEsh=[1]
4458 HYD_COMMENT=["Total Flows at MH106A"]
4459 *%-----|-----|
4460 *% -JFSA 2021-01-12 THE MANHOLE MH106 is called MH117/106 in Corrigan Report, IBI
Group, July 2008
4461 *%
4462 ROUTE PIPE PTYPE=[1]circ, NHYDout=["106A-106"], RNUMBER=[1.0], PDIAM=[1800](mm),
4463 PLNGTH=[190](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH106A"], RDT=[1]
4464 *%-----|-----|
4465 CONTINUOUS STANDHYD NHYD=["A9"], DT=[1]min, AREA=[2.44](ha),
4466 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4467 SCS curve number CN=[75],
4468 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4469 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4470 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4471 LGI=[262](m), MNI=[0.013], SCI=[0](min),
4472 Continuous simulation parameters:
4473 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4474 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4475 InterEventTime=[18](hrs), END=-1
4476 *%-----|-----|
4477 COMPUTE DUALHYD NHYDin=["A9"], CINLET=[0.547](cms), NINLET=[1],
4478 MajNHYD=["A9-MJ"]
4479 MinNHYD=["A9-MN"]
4480 TMJSTO=[0](cu-m)
4481 *%-----|-----|
4482 ADD HYD NHYDsum=["MH106"], NHYDs to add=["106A-106"+"A9-MN"]
4483 *%-----|-----|
4484 SAVE HYD NHYD=["MH106"], # OF PCYCLES=[-1], ICASEsh=[1]
4485 HYD_COMMENT=["Total Flows at MH106"]
4486 *%-----|-----|
4487 *% -JFSA 2021-01-12 THE MANHOLE MH107 is called MH118/107 in Corrigan Report, IBI
Group, July 2008
4488 *%
4489 ROUTE PIPE PTYPE=[1]circ, NHYDout=["106-107"], RNUMBER=[1.0], PDIAM=[1800](mm),
4490 PLNGTH=[122.5](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH106"], RDT=[1]
4491 *%-----|-----|
4492 CONTINUOUS STANDHYD NHYD=["A10"], DT=[1]min, AREA=[4.14](ha),
4493 XIMP=[0.35], TIMP=[0.47], DWF=[0](cms), LOSS=[2],
4494 SCS curve number CN=[75],
4495 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4496 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4497 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4498 LGI=[183](m), MNI=[0.013], SCI=[0](min),
4499 Continuous simulation parameters:
4500 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4501 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4502 InterEventTime=[18](hrs), END=-1
4503 *%-----|-----|
4504 COMPUTE DUALHYD NHYDin=["A10"], CINLET=[0.310](cms), NINLET=[1],
4505 MajNHYD=["A10-MJ"]
4506 MinNHYD=["A10-MN"]

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4507          TMJSTO=[228](cu-m)
4508  *%-----|-----
4509  CONTINUOUS STANDHYD NHYD=["A11"], DT=[1]min, AREA=[10.61](ha),
4510          XIMP=[0.53], TIMP=[0.62], DWF=[0](cms), LOSS=[2],
4511          SCS curve number CN=[75],
4512          Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4513          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4514          Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4515          LGI=[379](m), MNI=[0.013], SCI=[0](min),
4516          Continuous simulation parameters:
4517          IaREcper=[4](hrs), IaREcimp=[4](hrs),
4518          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4519          InterEventTime=[18](hrs), END=-1
4520  *%-----|-----
4521  COMPUTE DUALHYD     NHYDin=["A11"], CINLET=[0.993](cms), NINLET=[1],
4522          MajNHYD=["A11-MJ"]
4523          MinNHYD=["A11-MN"]
4524          TMJSTO=[556](cu-m)
4525  *%-----|-----
4526  ADD HYD             NHYDsum=["MH107"], NHYDs to add=["106-107"+"A10-MN"+"A11-MN"]
4527  *%-----|-----
4528  SAVE HYD           NHYD=["MH107"], # OF PCYCLES=[-1], ICASEsh=[1]
4529          HYD_COMMENT=["Total Flows at MH107"]
4530  *%-----|-----
4531  ROUTE PIPE         PTYPE=[1]circ, NHYDout=["107-119"], RNUMBER=[1.0], PDIAM=[1800](mm),
4532          PLNGTH=[114](m), PROUGH=[0.013], PSLOPE=[0.0012](m/m),
          NHYDin=["MH107"], RDT=[1]
4533  *%-----|-----
4534  *% -JFSA 2021-01-12 THE MANHOLE MH108 is called MH120/108 in Corrigan Report, IBI
Group, July 2008
4535  *%
4536  ROUTE PIPE         PTYPE=[1]circ, NHYDout=["119-108"], RNUMBER=[1.0], PDIAM=[1800](mm),
4537          PLNGTH=[65.8](m), PROUGH=[0.013], PSLOPE=[0.0012](m/m),
          NHYDin=["107-119"], RDT=[1]
4538  *%-----|-----
4539  CONTINUOUS STANDHYD NHYD=["A12"], DT=[1]min, AREA=[12.29](ha),
4540          XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4541          SCS curve number CN=[75],
4542          Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4543          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4544          Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4545          LGI=[183](m), MNI=[0.013], SCI=[0](min),
4546          Continuous simulation parameters:
4547          IaREcper=[4](hrs), IaREcimp=[4](hrs),
4548          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4549          InterEventTime=[18](hrs), END=-1
4550  *%-----|-----
4551  COMPUTE DUALHYD     NHYDin=["A12"], CINLET=[1.029](cms), NINLET=[1],
4552          MajNHYD=["A12-MJ"]
4553          MinNHYD=["A12-MN"]
4554          TMJSTO=[672](cu-m)
4555  *%-----|-----
4556  CONTINUOUS STANDHYD NHYD=["A13"], DT=[1]min, AREA=[2.59](ha),
4557          XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4558          SCS curve number CN=[75],
4559          Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4560          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4561          Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4562          LGI=[379](m), MNI=[0.013], SCI=[0](min),
4563          Continuous simulation parameters:
4564          IaREcper=[4](hrs), IaREcimp=[4](hrs),
4565          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4566          InterEventTime=[18](hrs), END=-1
4567  *%-----|-----
4568  COMPUTE DUALHYD     NHYDin=["A13"], CINLET=[0.571](cms), NINLET=[1],
4569          MajNHYD=["A13-MJ"]

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4570 MinNHYD=["A13-MN"]
4571 TMJSTO=[0](cu-m)
4572 *%-----|-----
4573 * -JFSA 2021-01-22 add the Corrigan pond area ("Pond-Block")
4574 CONTINUOUS STANDHYD NHYD=["Pond-Block"], DT=[1]min, AREA=[2.94](ha),
4575 XIMP=[0.415], TIMP=[0.415], DWF=[0](cms), LOSS=[2],
4576 SCS curve number CN=[75],
4577 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4578 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4579 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4580 LGI=[183](m), MNI=[0.013], SCI=[0](min),
4581 Continuous simulation parameters:
4582 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4583 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4584 InterEventTime=[18](hrs), END=-1
4585 *%-----|-----
4586 ADD HYD NHYDsum=["MH108"], NHYDs to add=["119-108"+"A13-MN"+"A12-MN"]
4587 *%-----|-----
4588 SAVE HYD NHYD=["MH108"], # OF PCYCLES=[-1], ICASEsh=[1]
4589 HYD_COMMENT=["Total Flows at MH108"]
4590 *%-----|-----
4591 ROUTE PIPE PTYPE=[1]circ, NHYDout=["108-116"], RNUMBER=[1.0], PDIAM=[1800](mm),
4592 PLNGTH=[76.6](m), PROUGH=[0.013], PSLOPE=[0.0013](m/m),
4593 NHYDin=["MH108"], RDT=[1]
4594 *%-----|-----
4595 ROUTE PIPE PTYPE=[1]circ, NHYDout=["116-corrig"], RNUMBER=[1.0],
4596 PDIAM=[1800](mm),
4597 PLNGTH=[79.5](m), PROUGH=[0.013], PSLOPE=[0.0013](m/m),
4598 NHYDin=["108-116"], RDT=[1]
4599 *%-----|-----
4600 ADD HYD NHYDsum=["Corrigan"], NHYDs to add=["116-corrig"+"Pond-Block"]
4601 *%-----|-----
4602 SAVE HYD NHYD=["Corrigan"], # OF PCYCLES=[-1], ICASEsh=[1]
4603 HYD_COMMENT=["Total Flows at Corrigan Pond"]
4604 *%-----|-----
4605 ROUTE RESERVOIR NHYDout=["Co-P"], NHYDin=["Corrigan"],
4606 RDT=[1](min),
4607 TABLE of ( OUTFLOW-STORAGE ) values
4608 (cms) - (ha-m)
4609 [ 0.0 , 0.0 ]
4610 [ 0.015 , 0.04118 ]
4611 [ 0.030 , 0.08297 ]
4612 [ 0.045 , 0.12537 ]
4613 [ 0.060 , 0.16837 ]
4614 [ 0.075 , 0.21199 ]
4615 [ 0.090 , 0.27545 ]
4616 [ 0.105 , 0.34650 ]
4617 [ 0.120 , 0.42049 ]
4618 [ 0.135 , 0.50188 ]
4619 [ 0.186 , 0.60307 ]
4620 [ 2.110 , 0.79083 ]
4621 [ 5.874 , 1.00271 ]
4622 [ 11.395 , 1.29643 ]
4623 [ 18.770 , 1.62054 ]
4624 [ 28.143 , 1.97516 ]
4625 [ -1 , -1 ] (max twenty pts)
4626 NHYDovf=["Co-P-OVF"]
4627 *%-----|-----
4628 ADD HYD NHYDsum=["corrig"], NHYDs to
4629 add=["Co-P-OVF"+"Co-P"+"N_TO"+"MH105-JR"+"A8-MJ-JR"+"A9-MJ"+"A10-MJ"+"A11-MJ"+"A12-MJ"+"A
4630 13-MJ"]
4631 *%-----|-----
4632 SAVE HYD NHYD=["corrig"], # OF PCYCLES=[-1], ICASEsh=[1]
4633 HYD_COMMENT=["Total Flows at Corrigan Pond"]
4634 *%-----|-----
4635 *#*****

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4631 *#   Corrigan Pond 1
4632 *#   - Rating curve obtained from Barrhaven South MSS modeling
4633 *#   - Tributary Drainage Area to MSS Pond 1 = 145 ha
4634 *#*****
4635 *ROUTE RESERVOIR      NHYDout=["MS_P1"],  NHYDin=["CORRIG"],
4636 *                      RDT=[1](min),
4637 *                      TABLE of ( OUTFLOW-STORAGE ) values
4638 *                      (cms) - (ha-m)
4639 *                      [ 0.0 , 0.0 ]
4640 *                      [ 0.06 , 0.58]
4641 *                      [ -1 , -1 ] (max twenty pts)
4642 *                      NHYDovf=["P1-OVF"]
4643 *%-----|-----
4644 *ADD HYD              NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
4645 *%-----|-----
4646 *SAVE HYD            NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
4647 *                      HYD_COMMENT=["Total Flows at Corrigan Drain"]
4648 *%-----|-----
4649 *#
4650 *# Hydrograph from Corrigan Drain routed to Jockvale Road
4651 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
4652 *#
4653 ROUTE CHANNEL      NHYDout=["N_MI"] ,NHYDin=["corrig"] ,
4654 *                      RDT=[1](min),
4655 *                      CHLGTH=[580](m),  CHSLOPE=[0.4448](%),
4656 *                      FPSLOPE=[0.4448](%),
4657 *                      SECNUM=[1.0],      NSEG=[3]
4658 *                      ( SEGRROUGH, SEGDIST (m))=
4659 *                      [0.075,-17.72
4660 *                      -0.045,17.72
4661 *                      0.075,80.62] NSEG times
4662 *                      ( DISTANCE (m), ELEVATION (m))=
4663 *                      [-83.32, 90.00]
4664 *                      [-81.36, 89.50]
4665 *                      [-79.12, 89.00]
4666 *                      [-76.13, 88.50]
4667 *                      [-20.46, 88.00]
4668 *                      [-19.36, 87.50]
4669 *                      [-18.51, 87.00]
4670 *                      [-17.72, 86.50]
4671 *                      [-11.95, 85.24]
4672 *                      [-0.11, 85.12]
4673 *                      [11.49, 85.20]
4674 *                      [17.72, 86.50]
4675 *                      [19.74, 87.00]
4676 *                      [21.22, 87.50]
4677 *                      [22.68, 88.00]
4678 *                      [24.28, 88.50]
4679 *                      [26.79, 89.00]
4680 *                      [71.98, 90.00]
4681 *                      [80.62, 90.50]
4682 *%-----|-----
4683 *#*****
4684 *#   Catchment MILLS
4685 *#   - To SWM Facility north of the Jock
4686 *#   - Primarily residential development
4687 *#*****
4688 CONTINUOUS STANDHYD NHYD=["MILLS"], DT=[1]min, AREA=[175.99](ha),
4689 *                      XIMP=[0.38], TIMP=[0.38], DWF=[0](cms), LOSS=[2],
4690 *                      SCS curve number CN=[74],
4691 *                      Pervious surfaces: IAPER=[4.67](mm), SLPP=[1](%),
4692 *                      LGP=[40](m), MNP=[0.25], SCP=[0](min),
4693 *                      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4694 *                      LGI=[1118.123](m), MNI=[0.013], SCI=[0](min),
4695 *                      Continuous simulation parameters:
4696 *                      IaREcper=[4](hrs), IaREcimp=[4](hrs),

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4697          SMIN=[-1](mm),   SMAX=[-1](mm), SK=[0.010]/(mm),
4698          InterEventTime=[18](hrs),   END=-1
4699  *%-----|-----|
4700  *#*****|
4701  *#   Chapman Mills SWM Pond
4702  *#   - Rating curve obtained from CCL hydraulic modeling
4703  *#*****|
4704  ROUTE RESERVOIR   NHYDout=["MILL_P"],   NHYDin=["MILLS"],
4705                  RDT=[1](min),
4706                  TABLE of ( OUTFLOW-STORAGE ) values
4707                  (cms) - (ha-m)
4708                  [ 0.0 , 0.0 ]
4709                  [ 0.01 , 0.01]
4710                  [ 0.05 , 0.06]
4711                  [ 0.09 , 0.11]
4712                  [ 0.13 , 0.15]
4713                  [ 0.18 , 0.19]
4714                  [ 0.28 , 0.28]
4715                  [ 0.37 , 0.34]
4716                  [ 0.45 , 0.40]
4717                  [ 0.51 , 0.44]
4718                  [ 0.56 , 0.47]
4719                  [ 0.64 , 0.52]
4720                  [ 0.76 , 0.59]
4721                  [ 0.86 , 0.65]
4722                  [ 1.09 , 0.78]
4723                  [ 1.44 , 0.96]
4724                  [ 3.18 , 1.84]
4725                  [ 4.05 , 2.31]
4726                  [ -1 , -1 ] (max twenty pts)
4727                  NHYDovf=["MIL-OV"]
4728  *%-----|-----|
4729  ADD HYD           NHYDsum=["SN_MI"], NHYDs to add=["N_MI"+"MIL-OV"+"MILL_P"]
4730  *%-----|-----|
4731  SAVE HYD         NHYD=["SN_MI"],   # OF PCYCLES=[-1],   ICASEsh=[1]
4732                  HYD_COMMENT=["Total Flows at Jockvale Road"]
4733  *%-----|-----|
4734  *#
4735  *# Hydrograph from Jockvale Road routed to Heart's Desire
4736  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689
4737  *#
4738  ROUTE CHANNEL    NHYDout=["N_DE"] ,NHYDin=["SN_MI"] ,
4739                  RDT=[1](min),
4740                  CHLGTH=[1962](m),   CHSLOPE=[0.2227](%),
4741                  FPSLOPE=[0.2227](%),
4742                  SECNUM=[1.0],       NSEG=[3]
4743                  ( SEGROUGH, SEGDIST (m))=
4744                  [0.075,-17.56
4745                  -0.045,18.27
4746                  0.075,32.51] NSEG times
4747                  ( DISTANCE (m), ELEVATION (m))=
4748                  [-54.07, 85.00]
4749                  [-39.43, 84.50]
4750                  [-28.30, 84.00]
4751                  [-24.12, 83.50]
4752                  [-22.30, 83.00]
4753                  [-20.55, 82.50]
4754                  [-17.56, 82.00]
4755                  [-12.63, 81.22]
4756                  [-0.11, 80.75]
4757                  [11.55, 81.22]
4758                  [18.27, 82.00]
4759                  [19.82, 82.50]
4760                  [22.48, 83.00]
4761                  [27.90, 83.50]
4762                  [29.31, 84.00]

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4763             [30.81, 84.50]
4764             [32.51, 85.00]
4765 *%-----|-----|
4766 *#*****|
4767 *#   Catchment DESIRE
4768 *#   - To Jock River (north of the Jock)
4769 *#   - Rural-estate subdivision (Heart's Desire Community)
4770 *#*****|
4771 CONTINUOUS STANDHYD NHYD=["DESIRE"], DT=[1]min, AREA=[23.78](ha),
4772 XIMP=[0.25], TIMP=[0.25], DWF=[0](cms), LOSS=[2],
4773 SCS curve number CN=[77],
4774 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4775 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4776 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4777 LGI=[400](m), MNI=[0.013], SCI=[0](min),
4778 Continuous simulation parameters:
4779 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4780 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4781 InterEventTime=[18](hrs), END=-1
4782 *%-----|-----|
4783 *#*****|
4784 *#   Catchment JOCKVA
4785 *#   - To Jockvale SWM Facility
4786 *#   - Residential development & golf course
4787 *#   - JFSA 2021-01-11 update JOCKVA after updating CORRIG as per IBI GROUP, July 2008.
4788 *#   JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two
4789 *#   areas JOCKVA and EX-LAND 32.5 ha as per IBI GROUP, July 2008.
4790 *#*****|
4791 CONTINUOUS STANDHYD NHYD=["JOCKVA"], DT=[1]min, AREA=[225.13](ha),
4792 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
4793 SCS curve number CN=[74],
4794 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4795 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4796 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4797 LGI=[1310.55](m), MNI=[0.013], SCI=[0](min),
4798 Continuous simulation parameters:
4799 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4800 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4801 InterEventTime=[18](hrs), END=-1
4802 *%-----|-----|
4803 ADD HYD NHYDsum=["JOCKVA-TO"], NHYDs to
4804 add=["EX-LAND-MN"+"JOCKVA"+"B2-MJ"+"B3-MJ"]
4805 *%-----|-----|
4806 SAVE HYD NHYD=["JOCKVA-TO"], # OF PCYCLES=[-1], ICASEsh=[1]
4807 HYD_COMMENT=["Total Flows at KB first pond"]
4808 *%-----|-----|
4809 *#*****|
4810 *#   Jockvale SWM Facility
4811 *#   - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
4812 *#*****|
4813 ROUTE RESERVOIR NHYDout=["JOCK_P"], NHYDin=["JOCKVA-TO"],
4814 RDT=[1](min),
4815 TABLE of ( OUTFLOW-STORAGE ) values
4816 (cms) - (ha-m)
4817 [ 0.0 , 0.0 ]
4818 [ 0.27 , 0.03 ]
4819 [ 0.28 , 0.55 ]
4820 [ 0.29 , 1.14 ]
4821 [ 0.30 , 1.80 ]
4822 [ 0.31 , 2.32 ]
4823 [ 1.12 , 2.87 ]
4824 [ 2.92 , 3.45 ]
4825 [ 4.64 , 4.07 ]
4826 [ 6.69 , 4.72 ]
4827 [ 9.02 , 5.39 ]
4828 [ 11.62 , 6.10 ]

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4827             [ 14.42 , 6.85]
4828             [ 17.45 , 7.62]
4829             [ 20.69 , 8.44]
4830             [ 24.08 , 9.28]
4831             [ 27.68 , 10.17]
4832             [ -1 , -1 ] (max twenty pts)
4833             NHYDovf=["JO-OVF"]
4834 *%-----|-----|
4835 ADD HYD      NHYDsum=["SN_DE"], NHYDs to add=["N_DE"+"DESIRE"+"JO-OVF"+"JOCK_P"]
4836 *%-----|-----|
4837 SAVE HYD     NHYD=["SN_DE"], # OF PCYCLES=[-1], ICASEsh=[1]
4838             HYD_COMMENT=["Total Flows at Heart's Desire"]
4839 *%-----|-----|
4840 *#
4841 *# Hydrograph from Heart's Desire routed to Rideau River
4842 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
4843 *#
4844 ROUTE CHANNEL NHYDout=["N1"] ,NHYDin=["SN_DE"] ,
4845             RDT=[1](min),
4846             CHLGTH=[563](m), CHSLOPE=[0.9668](%),
4847             FPSLOPE=[0.9668](%),
4848             SECNUM=[1.0], NSEG=[3]
4849             ( SEGROUGH, SEGDIST (m))=
4850             [0.075,-30.20
4851             -0.045,30.20
4852             0.075,48.48] NSEG times
4853             ( DISTANCE (m), ELEVATION (m))=
4854             [-98.46, 81.50]
4855             [-92.24, 81.00]
4856             [-86.88, 80.50]
4857             [-81.54, 80.00]
4858             [-74.36, 79.50]
4859             [-63.54, 79.00]
4860             [-39.23, 78.50]
4861             [-34.51, 78.00]
4862             [-33.01, 77.50]
4863             [-30.20, 77.00]
4864             [-13.42, 76.18]
4865             [-1.14, 76.09]
4866             [17.06, 76.18]
4867             [30.20, 77.00]
4868             [32.95, 77.50]
4869             [34.06, 78.00]
4870             [35.11, 78.50]
4871             [36.32, 79.00]
4872             [37.74, 79.50]
4873             [48.48, 81.50]
4874 *%-----|-----|
4875 *#*****|*****|
4876 *# Catchment S-2
4877 *# - To Jock River (north and south)
4878 *# - Undeveloped floodplain and river
4879 *#*****|*****|
4880 CONTINUOUS NASHYD NHYD=["S-2"], DT=[1]min, AREA=[102.94](ha),
4881             DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
4882             N=[3], TP=[0.40]hrs,
4883             Continuous simulation parameters:
4884             IaREcper=[4](hrs),
4885             SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4886             InterEventTime=[12](hrs)
4887             Baseflow simulation parameters:
4888             BaseFlowOption=[1] ,
4889             InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4890             VHydCond=[0.055](mm/hr), END=-1
4891 *%-----|-----|
4892 ADD HYD      NHYDsum=["SN_N1"], NHYDs to add=["N1"+"S-2"]

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4893  *%-----|-----|
4894  SAVE HYD      NHYD=["SN_N1"], # OF PCYCLES=[-1], ICASEsh=[1]
4895              HYD_COMMENT=["Total Flows at Rideau River"]
4896  *%-----|-----|
4897  *#####
4898  *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4899  START        TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
4900  *%          ["C24SC005.stm"] <--storm filename, one per line for NSTORM time
4901  *%-----|-----|
4902  *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4903  START        TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
4904  *%          ["C24SC010.stm"] <--storm filename, one per line for NSTORM time
4905  *%-----|-----|
4906  *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4907  START        TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
4908  *%          ["C24SC025.stm"] <--storm filename, one per line for NSTORM time
4909  *%-----|-----|
4910  *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4911  START        TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
4912  *%          ["C24SC050.stm"] <--storm filename, one per line for NSTORM time
4913  *%-----|-----|
4914  *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4915  *START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4916  *%          ["100YC3H.STM"] <--storm filename, one per line for NSTORM time
4917  *%-----|-----|
4918  *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4919  START        TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4920  *%          ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
4921  *%-----|-----|
4922  *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4923  *START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4924  *%          ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
4925  *START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[101]
4926  *%          ["A24SC100.stm"] <--storm filename, one per line for NSTORM time
4927  *START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
4928  *%          ["A24SC100_60.stm"] <--storm filename, one per line for NSTORM time
4929  FINISH
4930

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00005 SSSS W W M M M H H H H H Y Y M M M O O 2 0 0 11 5 Ver 5.800
00006 S W W M M H H H Y Y M M O O 222 0 0 11 555 PRE 2015
00007 SSSS W W M H H Y Y M M O O 2 2 0 0 11 5 .....
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00009 Stormwater Management Hydrologic Model 222 0 0 11 5 .....
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00375# #
00376# R002/C00039-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00377# ADD HYD + 1.0 02:01M1 9506.00 7.379_MDate 33:12 11.20 n/a .000
00378# ROUTE CHANNEL -> 1.0 02:01M1 500.00 2.720_MDate 29:22 11.98 n/a .000
00379# + 1.0 02:01M2 1917.00 4.042_MDate 34:34 11.98 n/a .000
00380# SUM= 1.0 02:01M1 11923.00 12.077_MDate 33:14 11.36 n/a .000
00381#
00382# Sum of hydrographs from Node 11 routed to Node 10
00383# Section 1
00384#
00385# R002/C00040-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00386# ROUTE CHANNEL -> 1.0 02:01M1 11923.00 12.077_MDate 33:14 11.36 n/a .000
00387# [RPT= 1.00] out<- 1.0 01:01M0 11923.00 8.276_MDate 39:46 11.36 n/a .000
00388# [L/S= 3982 / .057 / .040]
00389# [Vmax=.462;Dmax=.886]
00390#
00391# Addition of Subwatershed 10 to Node 10
00392#
00393# R002/C00041-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00394# ADD HYD + 1.0 02:01M0 17859.00 19.451_MDate 38:31 12.19 n/a .000
00395# + 1.0 02:01M2 6566.00 11.228_MDate 38:07 13.94 n/a .000
00396# SUM= 1.0 01:5_M0 17859.00 19.451_MDate 38:31 12.19 n/a .000
00397# R002/C00042-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00398# SAVE HYD + 1.0 01:5_M0 17859.00 19.451_MDate 38:31 12.19 n/a .000
00399# fname_H_SMI0
00400# remark:flow at S_M10: M10 + SW_10
00401# Addition of Kings Creek to S_M10
00402#
00403# R002/C00043-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00404# ADD HYD + 1.0 02:01M0 17859.00 19.451_MDate 38:31 12.19 n/a .000
00405# + 1.0 02:01M2 8376.00 11.072_MDate 39:59 11.98 n/a .000
00406# SUM= 1.0 01:5_M10A 25965.00 30.328_MDate 39:58 12.12 n/a .000
00407#
00408# Sum of hydrographs from Node 10 routed to Node 9
00409# Section 2
00410#
00411# R002/C00044-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00412# ROUTE CHANNEL -> 1.0 02:01M0 25965.00 30.328_MDate 39:58 12.12 n/a .000
00413# [RPT= 1.00] out<- 1.0 01:01M9 25965.00 29.579_MDate 39:59 12.12 n/a .000
00414# [L/S= 3982 / .075 / .040]
00415# [Vmax=.595;Dmax=1.208]
00416#
00417# Addition of Subwatershed 9 and Nichols Creek to Node 9
00418#
00419# R002/C00045-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00420# ADD HYD + 1.0 02:01M9 25965.00 29.579_MDate 39:59 12.12 n/a .000
00421# + 1.0 02:01M9 1132.00 4.404_MDate 39:59 12.12 n/a .000
00422# + 1.0 02:01M2 4464.00 5.504_MDate 39:59 10.98 n/a .000
00423# SUM= 1.0 01:5_M9 31561.00 36.313_MDate 39:59 12.00 n/a .000
00424#
00425# Sum of hydrographs from Node 9 routed to Node 8
00426# Section 3
00427#
00428# R002/C00046-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00429# ROUTE CHANNEL -> 1.0 02:01M9 31561.00 36.313_MDate 39:59 12.00 n/a .000
00430# [RPT= 1.00] out<- 1.0 01:01M8 31561.00 34.713_MDate 39:59 12.00 n/a .000
00431# [L/S= 3982 / .087 / .040]
00432# [Vmax=.418;Dmax=1.281]
00433#
00434# Addition of Subwatershed 8 and Bobb's Drain to Node 8
00435#
00436# R002/C00047-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00437# ADD HYD + 1.0 02:01M8 31561.00 34.713_MDate 39:59 12.00 n/a .000
00438# + 1.0 02:01M9 1312.00 4.404_MDate 39:59 11.92 n/a .000
00439# + 1.0 02:01M2 3854.00 6.242_MDate 38:46 11.98 n/a .000
00440# SUM= 1.0 01:5_M8 35546.00 40.474_MDate 39:59 12.00 n/a .000
00441#
00442# Sum of hydrographs from Node 8 routed to Node 7
00443# Section 4
00444#
00445# R002/C00048-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00446# ROUTE CHANNEL -> 1.0 02:01M8 35546.00 40.474_MDate 39:59 12.00 n/a .000
00447# [RPT= 1.00] out<- 1.0 01:01M7 35546.00 32.892_MDate 44:30 12.00 n/a .000
00448# [L/S= 3750 / .057 / .030]
00449# [Vmax=.208;Dmax=1.651]
00450#
00451# Addition of Subwatershed 7 to Node 7
00452#
00453# R002/C00049-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00454# ADD HYD + 1.0 02:01M7 35546.00 32.892_MDate 44:30 12.00 n/a .000
00455# + 1.0 02:01M7 3197.00 4.453_MDate 36:31 8.45 n/a .000
00456# SUM= 1.0 01:5_M7 38743.00 35.071_MDate 43:33 11.82 n/a .000
00457# R002/C00050-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00458# SAVE HYD + 1.0 01:5_M7 38743.00 35.071_MDate 43:33 11.82 n/a .000
00459# fname_H_SMT
00460# remark:flow at S_M7: M7 + SW_7
00461# Insertion of a Reservoir to simulate the effects of the Richmond Fen.
00462# Storage area and volumes were estimated from available topo maps.
00463# Release rate from Fen was assumed to be controlled by the downstream
00464# river cross-section for which it is assumed that for up to
00465# 0.75 m of water, the main channel of the river provided the storage. Above
00466# this depth, the wetland starts to significantly store water.
00467#
00468# R002/C00051-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00469# ROUTE CHANNEL -> 1.0 02:01M7 38743.00 35.071_MDate 43:33 11.82 n/a .000
00470# out<- 1.0 01:RES_RP 38743.00 23.265_MDate 55:09 11.82 n/a .000
00471# [Med:0.000;Vol:0.000]
00472# R002/C00052-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00473# SAVE HYD + 1.0 01:RES_RP 38743.00 23.265_MDate 55:09 11.82 n/a .000
00474# fname_H_RESFP
00475# remark:outflow of Richmond Fen
00476#
00477# Sum of hydrographs from Node 7 routed to Node 6
00478# Section 5
00479#
00480# R002/C00053-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00481# ROUTE CHANNEL -> 1.0 02:01M7 38743.00 23.265_MDate 55:09 11.82 n/a .000
00482# [RPT= 1.00] out<- 1.0 01:01M6 38743.00 23.228_MDate 56:38 11.82 n/a .000
00483# [L/S= 3066 / .087 / .040]
00484# [Vmax=.432;Dmax=.808]
00485#
00486# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
00487#
00488# R002/C00054-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00489# ADD HYD + 1.0 02:01M6 38743.00 23.228_MDate 56:38 11.82 n/a .000
00490# + 1.0 02:01M6 1455.00 4.423_MDate 56:38 11.82 n/a .000
00491# + 1.0 02:01M2 1332.00 3.148_MDate 35:23 13.94 n/a .000
00492# SUM= 1.0 01:5_M6 40240.00 23.318_MDate 39:59 11.89 n/a .000
00493#
00494# Sum of hydrographs from Node 6 routed to Node 5
00495# Section 6
00496#
00497# R002/C00055-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00498# ROUTE CHANNEL -> 1.0 02:01M6 40240.00 23.318_MDate 39:59 11.89 n/a .000
00499# [RPT= 1.00] out<- 1.0 01:01M5 40240.00 23.285_MDate 56:09 11.89 n/a .000
00500# [L/S= 3066 / .057 / .040]
00501# [Vmax=.378;Dmax=.917]
00502#
00503# Addition of Subwatershed 5 and Flowing Creek to Node 5
00504#
00505# R002/C00056-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00506# ADD HYD + 1.0 02:01M5 40240.00 23.285_MDate 56:09 11.89 n/a .000
00507# + 1.0 02:01M5 224.94 2.587_MDate 45:45 15.21 n/a .000
00508# + 1.0 02:01M2 4949.00 14.839_MDate 33:25 14.57 n/a .000
00509# SUM= 1.0 01:5_M5 45408.00 31.366_MDate 37:08 12.20 n/a .000
00510#
00511# Sum of hydrographs from Node 5 routed to Node 5A
00512# Section 7
00513#
00514# R002/C00057-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00515# ROUTE CHANNEL -> 1.0 02:01M5 45408.00 31.366_MDate 37:08 12.20 n/a .000
00516# [RPT= 1.00] out<- 1.0 01:01M5A 45408.00 31.366_MDate 37:08 12.20 n/a .000
00517# [L/S= 556 / .090 / .040]
00518# [Vmax=.443;Dmax=.937]
00519#
00520# Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
00521#
00522# R002/C00058-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00523# ADD HYD + 1.0 02:01M5A 45408.00 31.366_MDate 37:08 12.20 n/a .000
00524# + 1.0 02:01M5A2 20.00 .509_MDate 28:36 17.79 n/a .000
00525# + 1.0 02:01M5A1 1622.00 3.090_MDate 38:04 15.22 n/a .000
00526# SUM= 1.0 01:5_M5A 46841.00 36.266_MDate 37:28 12.30 n/a .000
00527#
00528# Sum of hydrographs from Node 5A routed to Node 4
00529# Section 8
00530#
00531# R002/C00059-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00532# ROUTE CHANNEL -> 1.0 02:01M5A 46841.00 36.266_MDate 37:28 12.30 n/a .000
00533# [RPT= 1.00] out<- 1.0 01:01M4 46841.00 35.288_MDate 39:22 12.30 n/a .000
00534# [L/S= 4630 / .047 / .035]
00535# [Vmax=.695;Dmax=2.444]
00536#
00537# Addition of Subwatershed 4 and Leamy Creek to Node 4
00538#
00539# R002/C00060-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00540# ADD HYD + 1.0 02:01M4 46841.00 35.288_MDate 39:22 12.30 n/a .000
00541# + 1.0 02:01M4 585.00 4.325_MDate 29:58 17.79 n/a .000
00542# + 1.0 02:01M2 1022.00 5.747_MDate 30:50 17.39 n/a .000
00543# SUM= 1.0 01:5_M4 48447.00 37.581_MDate 38:13 12.47 n/a .000
00544# R002/C00061-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00545# SAVE HYD + 1.0 01:5_M4 48447.00 37.581_MDate 38:13 12.47 n/a .000
00546# fname_H_4.0002
00547# remark:flow at S_M4
00548#
00549# Sum of hydrographs from Node 4 routed to Node 2
00550# Section 9
00551#
00552# R002/C00062-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00553# ROUTE CHANNEL -> 1.0 02:01M4 48447.00 37.581_MDate 38:13 12.47 n/a .000
00554# [RPT= 1.00] out<- 1.0 01:01M2 48447.00 37.455_MDate 38:13 12.47 n/a .000
00555# [L/S= 1467 / .060 / .040]
00556# [Vmax=.715;Dmax=2.445]
00557#
00558# Addition of Subwatershed 2 with Monahan Drain and Smith Drain to Node 2
00559#
00560# R002/C00063-----DtnIn-ID:HYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCMS
00561# ADD HYD + 1.0 02:01M2 48447.00 37.455_MDate 38:13 12.47 n/a .000

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10123 [IAREC 4.00] SMIN 31.15: SMAK-207.66: EK- 010]
10124 *****
10125 R002C00184 *****
10126 ADD HYD + 1.0 02:05-D1 4.94 .040 NoDate 29:12 14.83 n/a .000
10127 *****
10128 *****
10129 *****
10130 *****
10131 *****
10132 *****
10133 *****
10134 # Hydrograph from Node Foster SSM (Station 980) to Node at station 520
10135 # Channel X-Section obtained from RWCA Hydraulic Model - Station 980
10136 *****
10137 R002C00197 *****
10138 *****
10139 *****
10140 *****
10141 *****
10142 *****
10143 *****
10144 *****
10145 *****
10146 *****
10147 *****
10148 *****
10149 *****
10150 *****
10151 *****
10152 *****
10153 *****
10154 *****
10155 # Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock River)
10156 # Channel X-Section obtained from RWCA Hydraulic Model - Station 520
10157 *****
10158 R002C00161 *****
10159 *****
10160 *****
10161 *****
10162 *****
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10198 *****
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10200 *****
10201 # Channel X-Section obtained from RWCA Hydraulic Model - Station 6016
10202 *****
10203 *****
10204 *****
10205 *****
10206 *****
10207 *****
10208 *****
10209 # Catchment S-1
10210 # To Jock River (north and south of Jock)
10211 # Primary agricultural fields; portion of sand quarry
10212 *****
10213 R002C00169 *****
10214 *****
10215 *****
10216 *****
10217 *****
10218 *****
10219 *****
10220 *****
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10244 *****
10245 *****
10246 # Catchment M_CLEAR
10247 # To West Clarke Pond (south of the Jock)
10248 # Subdivided with 138 cells per Barharrowen South MSS
10249 # 2020-11-10 update CLARKE Tributary Drainage Area to = 121 ha based on P598(04)-11
10250 # 2020-11-10 update CLARKE Tributary Drainage Area to MATOR and MATOR
10251 *****
10252 *****
10253 *****
10254 *****
10255 *****
10256 *****
10257 *****
10258 *****
10259 *****
10260 # Year = 18 Capture
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10500 *****

Table with multiple columns containing alphanumeric codes, numerical values, and descriptive text. Includes entries like 104977, 104984, 105000, 105001, etc., with various parameters and units.

Table with multiple columns containing alphanumeric codes, numerical values, and descriptive text. Includes entries like 106844, 106845, 106846, 106847, etc., with various parameters and units.


```

01871# # - 2020-11-30 Increase Imp. based on P598(04)-11
01872# # - 2020-11-30 update RUDS Runary Drainage Area to = 146.015 ha based on P598(04)-11
01873# # - 2020-11-30 split TOOD Drainage Area to MAJOR, MINOR, POND and ALL
01874# # *****
01875# # - JFSA 2021-01-19 add TOOD_MN1 as part of Clarke's M_Clarke_M1 and remove it from Todd
01876# # R0002-C00276-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01877# # CONTINUOUS STANDBYD 1.0 01:TOOD_MN2 2.10 1.79 No_date 28:00 28.91 635 000
01878# # [XMP: 52:TMP:57]
01879# # [LOSS: 2 CN: 77.0]
01880# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
01881# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 118: MN1: 013:SCI: 0]
01882# # [IARECimp: 4.00: IARECPE: 4.00]
01883# # [SMN: 31.15: SMAK:207.66: SK: 010]
01884# # R0002-C00277-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01885# # CONTINUOUS STANDBYD 1.0 01:TOOD_MN2 1.2 0.11 No_date 28:00 28.91 635 000
01886# # [XMP: 53:TMP:57]
01887# # [LOSS: 2 CN: 77.0]
01888# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
01889# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 28: MN1: 013:SCI: 0]
01890# # [IARECimp: 4.00: IARECPE: 4.00]
01891# # [SMN: 31.15: SMAK:207.66: SK: 010]
01892# # R0002-C00278-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01893# # CONTINUOUS STANDBYD 1.0 01:TOOD_MN2 30.23 2.154 No_date 28:03 29.64 651 000
01894# # [XMP: 52:TMP:64]
01895# # [LOSS: 2 CN: 77.0]
01896# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
01897# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 449: MN1: 013:SCI: 0]
01898# # [IARECimp: 4.00: IARECPE: 4.00]
01899# # [SMN: 31.15: SMAK:207.66: SK: 010]
01900# # R0002-C00279-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01901# # CONTINUOUS STANDBYD 1.0 01:TOOD_MN2 112.91 6.533 No_date 28:06 28.70 631 000
01902# # [XMP: 52:TMP:57]
01903# # [LOSS: 2 CN: 77.0]
01904# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
01905# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 868: MN1: 013:SCI: 0]
01906# # [IARECimp: 4.00: IARECPE: 4.00]
01907# # [SMN: 31.15: SMAK:207.66: SK: 010]
01908# # R0002-C00280-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01909# # CONTINUOUS STANDBYD 1.0 01:TOOD_P 3.06 .295 No_date 28:00 31.76 698 000
01910# # [XMP: 63:TMP:63]
01911# # [LOSS: 2 CN: 77.0]
01912# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
01913# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 143: MN1: 013:SCI: 0]
01914# # [IARECimp: 4.00: IARECPE: 4.00]
01915# # [SMN: 31.15: SMAK:221.43: SK: 010]
01916# # 5-Year + 124 Capture
01917# # R0002-C00281-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01918# # ROUTE RESERVOIR -> 1.0 02:TOOD_MN 30.23 2.154 No_date 28:03 29.64 n/a 000
01919# # out -> 1.0 01:TOOD_MN 30.23 2.154 No_date 28:03 29.64 n/a 000
01920# # overflow -> 0.0 03:TOOD_MN 0.00 0.00 No_date 0:00 0.00 n/a 000
01921# # (MxTotVols: 68516-04 n3, TotVolVols: 00000-00 n3, N-Ovrf: 0, TotDurVov: 0 hrs)
01922# # 5-Year + 124 Capture
01923# # R0002-C00282-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01924# # ROUTE RESERVOIR -> 1.0 02:TOOD_MN 2.10 1.79 No_date 28:00 28.91 n/a 000
01925# # out -> 1.0 01:TOOD_MN 2.10 1.79 No_date 28:00 28.91 n/a 000
01926# # overflow -> 1.0 03:TOOD_MN 0.00 0.00 No_date 0:00 0.00 n/a 000
01927# # (MxTotVols: 67616-04 n3, TotVolVols: 00000-00 n3, N-Ovrf: 0, TotDurVov: 0 hrs)
01928# # R0002-C00283-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01929# # ROUTE RESERVOIR -> 1.0 02:TOOD_MN 12 0.10 No_date 28:00 28.91 n/a 000
01930# # out -> 1.0 01:TOOD_MN 12 0.10 No_date 28:00 28.91 n/a 000
01931# # overflow -> 0.0 03:TOOD_MN 0.00 0.00 No_date 0:00 0.00 n/a 000
01932# # (MxTotVols: 66666-04 n3, TotVolVols: 00000-00 n3, N-Ovrf: 0, TotDurVov: 0 hrs)
01933# # R0002-C00284-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01934# # CONTINUOUS STANDBYD 1.0 01:TOOD_P 25.50 1.427 No_date 28:04 25.60 563 000
01935# # [XMP: 42:TMP:52]
01936# # [LOSS: 2 CN: 75.0]
01937# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
01938# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 566: MN1: 013:SCI: 0]
01939# # [IARECimp: 4.00: IARECPE: 4.00]
01940# # [SMN: 31.15: SMAK:221.43: SK: 010]
01941# # R0002-C00285-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01942# # COMPUTE DIALYD 1.0 01:IA2 25.50 1.427 No_date 28:04 25.60 n/a 000
01943# # Major System / 1.0 02:IA2-MJ 25.50 1.427 No_date 28:04 25.60 n/a 000
01944# # Minor System / 1.0 03:IA2-MJ 25.50 1.427 No_date 28:04 25.60 n/a 000
01945# # [MjSysVol: 00000-00, TotVolVols: 00000-00, N-Ovrf: 0, TotDurVov: 0 hrs]
01946# # R0002-C00286-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01947# # ADD HYD + 1.0 02:TOOD_MN 2.10 1.79 No_date 28:00 28.91 n/a 000
01948# # out -> 1.0 02:TOOD_MN 2.10 1.79 No_date 28:00 28.91 n/a 000
01949# # out -> 1.0 02:TOOD_MJ 0.00 0.00 No_date 0:00 0.00 n/a 000
01950# # out -> 1.0 02:TOOD_P 3.06 .295 No_date 28:00 31.76 n/a 000
01951# # out -> 1.0 01:TOOD_MN 112.91 6.533 No_date 28:06 28.70 n/a 000
01952# # out -> 1.0 02:Clar_MN 1.77 .141 No_date 28:00 27.78 n/a 000
01953# # out -> 1.0 02:Clar_MN 1.77 .141 No_date 28:00 27.78 n/a 000
01954# # R0002-C00287-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01955# # SAVE HYD 1.0 01:TOOD 119.95 6.892 No_date 28:05 28.77 n/a 000
01956# # remark: TODD.0002
01957# # remark: Total Flows at Todd Drain
01958# # *****
01959# # Todd Pond
01960# # Rating Curve obtained from Barhaven South MSB modeling
01961# # - stantec 2007, Tributary Drainage Area to MSB Pond = 193 ha
01962# # *****
01963# # R0002-C00288-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01964# # ROUTE RESERVOIR -> 1.0 02:TOOD_P 119.95 6.892 No_date 28:05 28.77 n/a 000
01965# # out -> 1.0 02:TOOD_P 119.95 6.892 No_date 28:05 28.77 n/a 000
01966# # overflow -> 1.0 03:TOOD_P 0.00 0.00 No_date 0:00 0.00 n/a 000
01967# # (MxTotVols: 103316-04 n3, TotVolVols: 00000-00 n3, N-Ovrf: 0, TotDurVov: 0 hrs)
01968# # R0002-C00289-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01969# # ADD HYD + 1.0 02:TOOD_P 5437.73 49.202 No_date 37:09 13.08 n/a 000
01970# # out -> 1.0 02:TOOD_P 5437.73 49.202 No_date 37:09 13.08 n/a 000
01971# # out -> 1.0 02:TOOD_MN 0.00 0.00 No_date 0:00 0.00 n/a 000
01972# # out -> 1.0 02:TOOD_P 0.00 0.00 No_date 0:00 0.00 n/a 000
01973# # out -> 1.0 02:IA2-MJ 0.00 0.00 No_date 0:00 0.00 n/a 000
01974# # out -> 1.0 02:IA2-MJ 5437.69 49.202 No_date 37:09 13.08 n/a 000
01975# # R0002-C00290-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01976# # SAVE HYD 1.0 01:TOOD 5437.69 49.202 No_date 37:09 13.08 n/a 000
01977# # remark: IEN_TOOD.0002
01978# # remark: Total Flows at Todd Drain
01979# # *****
01980# # Hydrograph from Todd Drain routed to Corrigan Drain
01981# # Channel X-Section obtained from RUCS Hydraulic Model at Station 2462
01982# # 021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model will be more stable and g
01983# # R0002-C00291-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01984# # ROUTE RESERVOIR -> 1.0 02:TOOD_P 5437.69 49.202 No_date 37:09 13.08 n/a 000
01985# # out -> [SDP: 1.00] out -> 1.0 01:TOOD 5437.69 49.202 No_date 38:25 13.08 n/a 000
01986# # [L/S/n: 254./ 100./ 013]
01987# # [Wmax: 1.20:Dused: 1.20]
01988# # R0002-C00292-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01989# # (Vmax: .730:Dmax: 2.134)
01990# # SAVE HYD 1.0 01:TOOD 5437.69 49.202 No_date 38:25 13.08 n/a 000
01991# # remark: IEN_TOOD.0002
01992# # remark: Total Flows at Station 2462
01993# # *****
01994# # - To Corrigan Drain (south of the Jock)
01995# # - Primarily Developed (medium density)
01996# # - JFSA JAN 2021, add Corrigan subcatchments as per IRI, July 2008
01997# # *****
01998# # R0002-C00293-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
01999# # CONTINUOUS STANDBYD 1.0 01:corr1 15.87 1.352 No_date 28:02 31.77 698 000
02000# # [XMP: 63:TMP:63]
02001# # [LOSS: 2 CN: 77.0]
02002# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
02003# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 325: MN1: 013:SCI: 0]
02004# # [IARECimp: 4.00: IARECPE: 4.00]
02005# # [SMN: 31.15: SMAK:207.66: SK: 010]
02006# # R0002-C00294-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02007# # COMPUTE DIALYD 1.0 01:corr1 15.87 1.352 No_date 28:02 31.77 n/a 000
02008# # Major System / 1.0 02:corr1-MJ 15.87 1.352 No_date 28:02 31.77 n/a 000
02009# # Minor System / 1.0 03:corr1-MJ 15.87 1.352 No_date 28:02 31.77 n/a 000
02010# # [MjSysVol: 00000-00, TotVolVols: 00000-00, N-Ovrf: 0, TotDurVov: 0 hrs]
02011# # R0002-C00295-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02012# # CONTINUOUS STANDBYD 1.0 01:corr2 12.47 .100 No_date 29:12 14.83 136 000
02013# # [CN: 77.0] [N: 3.00] [Tpe: 1.10]
02014# # [IAREC: 4.00] [SMN: 31.15: SMAK:207.66: SK: 010]
02015# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
02016# # R0002-C00296-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02017# # CONTINUOUS STANDBYD 1.0 01:Al-Corriv 15.75 .992 No_date 28:02 25.60 563 000
02018# # [XMP: 42:TMP:52]
02019# # [LOSS: 2 CN: 75.0]
02020# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
02021# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 324: MN1: 013:SCI: 0]
02022# # [IARECimp: 4.00: IARECPE: 4.00]
02023# # [SMN: 31.15: SMAK:221.43: SK: 010]
02024# # R0002-C00297-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02025# # COMPUTE DIALYD 1.0 01:Al-Corriv 15.75 .992 No_date 28:02 25.60 n/a 000
02026# # Major System / 1.0 02:Al-MJ 15.75 .992 No_date 28:02 25.60 n/a 000
02027# # Minor System / 1.0 03:Al-MJ 15.75 .992 No_date 28:02 25.60 n/a 000
02028# # [MjSysVol: 00000-00, TotVolVols: 00000-00, N-Ovrf: 0, TotDurVov: 0 hrs]
02029# # R0002-C00298-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02030# # CONTINUOUS STANDBYD 1.0 01:BL 2.77 .033 No_date 28:09 9.64 212 000
02031# # [CN: 56.0] [I: 3.00] [Tpe: 1.21]
02032# # [IAREC: 4.00] [SMN: 75.69: SMAK:531.24: SK: 010]
02033# # [InterValTime: 12.00]
02034# # R0002-C00299-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02035# # CONTINUOUS STANDBYD 1.0 01:IA4 1.27 .116 No_date 28:01 32.20 707 000
02036# # [XMP: 63:TMP:63]
02037# # [LOSS: 2 CN: 75.0]
02038# # [Previous area: IApex: 4.67:SLP1:1.00:LDP: 40: MNP: 250:SCP: 0]
02039# # [Impervious area: IAImp: 1.57:SLP1:1.00:LGI: 251: MN1: 013:SCI: 0]
02040# # [IARECimp: 4.00: IARECPE: 4.00]
02041# # [SMN: 31.15: SMAK:207.66: SK: 010]
02042# # R0002-C00300-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02043# # COMPUTE DIALYD 1.0 01:IA4 1.27 .116 No_date 28:01 32.20 n/a 000
02044# # Major System / 1.0 02:IA4-MJ 1.27 .116 No_date 28:01 32.20 n/a 000
02045# # Minor System / 1.0 03:IA4-MJ 1.27 .116 No_date 28:01 32.20 n/a 000
02046# # [MjSysVol: 00000-00, TotVolVols: 00000-00, N-Ovrf: 0, TotDurVov: 0 hrs]
02047# # R0002-C00301-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02048# # ADD HYD + 1.0 02:IA4-MJ 15.75 .992 No_date 28:02 25.60 n/a 000
02049# # out -> 1.0 02:IA4-MJ 15.75 .992 No_date 28:02 25.60 n/a 000
02050# # out -> 1.0 02:corr1-MJ 15.87 1.352 No_date 28:02 31.77 n/a 000
02051# # out -> 1.0 02:corr1-MJ 15.87 1.352 No_date 28:02 31.77 n/a 000
02052# # out -> 1.0 02:corr2 12.47 .100 No_date 29:12 14.83 n/a 000
02053# # out -> 1.0 01:BL 2.77 .033 No_date 28:09 9.64 n/a 000
02054# # out -> 1.0 02:IA4-MJ 1.27 .116 No_date 28:01 32.20 n/a 000
02055# # out -> 1.0 01:BL 2.77 .033 No_date 28:09 9.64 n/a 000
02056# # R0002-C00302-----DRAIN-ID:INVDY-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWFFCS
02057# # SAVE HYD 1.0 01:MH101 48.13 2.497 No_date 28:02 24.10 n/a 000

```

Table with columns: ID, Description, Parameters, Values, Units, and Status. Contains a large amount of technical data including flow rates, elevations, and system identifiers.

```

02619 overflow <= 1.0 03:20-0VDF 0.00 0.00 No_date 0:00 0.00 n/a 0.00
02620 (MaxOvflw=3346.0) TotalOvflw=0.000E+00 83 0:00:00 0.0000000000
02621 R0025-CO0398-----DtnIn-ID:HYDF-----AREHA-QPEARComs-TpeakDate_hh:mm-----RvM-R.C-----DWFFms
02622 ADD HYD + 1.0 02:01:SM 55476.26 49.619 No_date 38:53 13.23 n/a 0.00
02623 + 1.0 02:01:SDRE 23.78 .936 No_date 28:03 19.26 n/a 0.00
02624 + 1.0 02:30-0VDF 0.00 0.00 No_date 0:00 0.00 n/a 0.00
02625 SUM = 1.0 02:30:0P 257.63 2.560 No_date 29:05 26.85 n/a 0.00
02626 R0025-CO0397-----DtnIn-ID:HYDF-----AREHA-QPEARComs-TpeakDate_hh:mm-----RvM-R.C-----DWFFms
02627 SAVE HYD 1.0 01:SM_DE 55476.26 49.619 No_date 38:49 13.23 n/a 0.00
02628 name 'SML_DE_0022
02629 remark:Total Flows at Heart's Desire
02630
02631 #
02632 # Hydrograph from Heart's Desire routed to Rideau River
02633 # Channel X-Section obtained from RWCA Hydraulic Model - Station 0
02634 #
02635 R0025-CO0398-----DtnIn-ID:HYDF-----AREHA-QPEARComs-TpeakDate_hh:mm-----RvM-R.C-----DWFFms
02636 ROUTE CHANNEL -> 1.0 02:SM_DE 55476.26 49.619 No_date 38:49 13.23 n/a 0.00
02637 [R/S]= 563. / 367. / 045]
02638 (Vmax= 1.491;Dmax= 0.01)
02639
02640 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02641 # of 1.75
02642 # [AREC= 4.00; EMIN= 59.42; SMAX=396.11; SK= .010]
02643 [InterEventTime= 12.00]
02644
02645 R0025-CO0399-----DtnIn-ID:HYDF-----AREHA-QPEARComs-TpeakDate_hh:mm-----RvM-R.C-----DWFFms
02646 CONTINUOUS NASHVD 1.0 01:SM_2 102.94 1.373 No_date 28:20 13.01 286 .000
02647 [Cm= 72.0; N= 3.00; Tp= .40]
02648 [InterEventTime= 12.00]
02649
02650 R0025-CO0400-----DtnIn-ID:HYDF-----AREHA-QPEARComs-TpeakDate_hh:mm-----RvM-R.C-----DWFFms
02651 ADD HYD + 1.0 02:SM 55476.26 49.619 No_date 38:54 13.23 n/a 0.00
02652 + 1.0 02:30-2 102.94 1.373 No_date 28:20 13.01 n/a 0.00
02653 SUM = 1.0 02:30:0P 55479.20 49.715 No_date 38:54 13.23 n/a 0.00
02654 R0025-CO0401-----DtnIn-ID:HYDF-----AREHA-QPEARComs-TpeakDate_hh:mm-----RvM-R.C-----DWFFms
02655 SAVE HYD 1.0 01:SM_M1 55479.20 49.715 No_date 38:54 13.23 n/a 0.00
02656 name 'SML_N1_0002
02657 remark:Total Flows at Rideau River
02658 #
02659 # ***** END OF RUN *****
02660 # ** END OF RUN **
02661
02662
02663
02664
02665
02666
02667 R005-COMMANDS
02668 START
02669 [MTOFF= 0 hrs on 0]
02670 [MTOFF= 1]
02671 [MTOFF= 2 (1=Imperial, 2=metric output)]
02672 [MTOFF= 3]
02673 [MTOFF= 4]
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02993# SUM= 1.0 01:15:00 25965.00 44.722 No_date 39:35 17.37 n/a .000
02994# #
02995# Sum of hydrographs from Node 10 routed to Node 9
02996# Section 2
02997#
02998# ROUTE CHANNEL -> 1.0 02:15:00 25965.00 44.722 No_date 39:35 17.37 n/a .000
02999# [R/S= 3982./ .071/.035]
03000# [Vmax= .664;Dmax= 1.502]
03001#
03002# Addition of Subwatershed 9 and Nichols Creek to Node 9
03003#
03004# ROUTES-CO0044-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03005# ADD HYD + 1.0 02:15R 25965.00 43.534 No_date 39:59 17.37 n/a .000
03006# [R/S= 3982./ .071/.035]
03007# [Vmax= .664;Dmax= 1.502]
03008#
03009# SUM= 1.0 02:15R 41661.00 53.366 No_date 39:59 17.20 n/a .000
03010#
03011# Sum of hydrographs from Node 9 routed to Node 8
03012# Section 3
03013#
03014# ROUTE CHANNEL -> 1.0 02:15R 31561.00 53.366 No_date 39:59 17.20 n/a .000
03015# [R/S= 2269./ .089/.040]
03016# [Vmax= .370;Dmax= 1.520]
03017#
03018# Addition of Subwatershed 8 and Hobbs's Drain to Node 8
03019#
03020# ROUTES-CO0044-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03021# ADD HYD + 1.0 02:15R 31561.00 49.404 No_date 39:59 17.20 n/a .000
03022# [R/S= 2269./ .089/.040]
03023# [Vmax= .370;Dmax= 1.520]
03024#
03025# SUM= 1.0 02:15R 3854.00 9.385 No_date 38:41 17.18 n/a .000
03026#
03027# Sum of hydrographs from Node 8 routed to Node 7
03028# Section
03029#
03030# ROUTE CHANNEL -> 1.0 02:15R 35546.00 58.845 No_date 39:59 17.19 n/a .000
03031# [R/S= 1.001 out.- 1.0 01:18R 35546.00 48.127 No_date 45:08 17.19 n/a .000]
03032# [L/S= 2467./ .057/.030]
03033# [Vmax= .208;Dmax= 1.855]
03034#
03035# Addition of Subwatershed 7 to Node 7
03036#
03037# ROUTES-CO0044-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03038# ADD HYD + 1.0 02:15R 35546.00 48.127 No_date 45:08 17.19 n/a .000
03039# [R/S= 2467./ .057/.030]
03040# [Vmax= .208;Dmax= 1.855]
03041#
03042# SUM= 1.0 02:15R 38743.00 51.395 No_date 44:14 16.92 n/a .000
03043#
03044# SAVE HYD + 1.0 01:15R 38743.00 51.395 No_date 44:14 16.92 n/a .000
03045# name_H_LNBT
03046# remark:flow at 8_N7 = SW 7
03047#
03048# Insertion of a reservoir to simulate the effects of the Richmond Fen.
03049# Storage area and volumes are estimated from available topo maps.
03050# Release rate from fen was assumed to be controlled by the downstream
03051# river cross-section for subwatershed 7. It was assumed that for up to
03052# 0.75 m of water, the main channel of the river provided the storage. Above
03053# this depth, the wetland starts to significantly store water.
03054#
03055# ROUTES-CO0054-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03056# ROUTE RESERVOIR + 1.0 02:15R 38743.00 51.395 No_date 44:14 16.92 n/a .000
03057# [R/S= 1.001 out.- 1.0 01:18R 38743.00 27.976 No_date 59:12 16.92 n/a .000]
03058# [Vmax= .174;Dmax= 1.744]
03059#
03060# SUM= 1.0 02:15R 38743.00 27.976 No_date 59:12 16.92 n/a .000
03061#
03062# name_H_RESRF
03063# remark:outflow of Richmond Fen
03064#
03065# Sum of hydrographs from Node 7 routed to Node 6
03066# Section 5
03067#
03068# ROUTE CHANNEL -> 1.0 02:15R 38743.00 27.976 No_date 59:12 16.92 n/a .000
03069# [R/S= 1.001 out.- 1.0 01:18R 38743.00 27.930 No_date 60:29 16.92 n/a .000]
03070# [L/S= 3066./ .027/.025]
03071# [Vmax= .460;Dmax= .895]
03072#
03073# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
03074#
03075# ROUTES-CO0054-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03076# ADD HYD + 1.0 02:15R 38743.00 27.930 No_date 60:29 16.92 n/a .000
03077# [R/S= 3066./ .027/.025]
03078# [Vmax= .460;Dmax= .895]
03079#
03080# SUM= 1.0 02:15R 1032.00 4.803 No_date 35:19 20.12 n/a .000
03081#
03082# Sum of hydrographs from Node 6 routed to Node 5
03083# Section 6
03084#
03085# ROUTES-CO0055-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03086# ROUTE CHANNEL -> 1.0 02:15R 40240.01 27.944 No_date 60:06 17.03 n/a .000
03087# [R/S= 1.001 out.- 1.0 01:18R 40240.01 27.930 No_date 60:29 16.92 n/a .000]
03088# [L/S= 566./ .097/.040]
03089# [Vmax= .397;Dmax= 1.002]
03090#
03091# Addition of Subwatershed 5 and Flowing Creek to Node 5
03092#
03093# ROUTES-CO0055-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03094# ADD HYD + 1.0 02:15R 40240.01 27.930 No_date 60:29 17.03 n/a .000
03095# [R/S= 566./ .097/.040]
03096# [Vmax= .397;Dmax= 1.002]
03097#
03098# SUM= 1.0 02:15R 4945.00 22.837 No_date 33:22 21.04 n/a .000
03099#
03100# Sum of hydrographs from Node 5 routed to Node 5A
03101# Section
03102#
03103# ROUTE CHANNEL -> 1.0 02:15R 45409.01 43.566 No_date 35:28 17.49 n/a .000
03104# [R/S= 1.001 out.- 1.0 01:18R 45409.01 43.490 No_date 35:47 17.49 n/a .000]
03105# [L/S= 1467./ .067/.040]
03106# [Vmax= .465;Dmax= 1.060]
03107#
03108# Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
03109#
03110# ROUTES-CO0058-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03111# ADD HYD + 1.0 02:15R 45409.01 43.490 No_date 35:47 17.49 n/a .000
03112# [R/S= 1467./ .067/.040]
03113# [Vmax= .465;Dmax= 1.060]
03114#
03115# SUM= 1.0 02:15R 1642.00 4.466 No_date 37:58 21.98 n/a .000
03116#
03117# Sum of hydrographs from Node 5A routed to Node 4
03118# Section 8
03119#
03120# ROUTES-CO0059-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03121# ROUTE CHANNEL -> 1.0 02:15R 46841.01 47.976 No_date 35:58 17.63 n/a .000
03122# [R/S= 4430./ .041/.035]
03123# [Vmax= .756;Dmax= 3.116]
03124#
03125# Addition of Subwatershed 4 and Leamy Creek to Node 4
03126#
03127# ROUTES-CO0060-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03128# ADD HYD + 1.0 02:15R 46841.01 46.217 No_date 37:26 17.63 n/a .000
03129# [R/S= 4430./ .041/.035]
03130# [Vmax= .756;Dmax= 3.116]
03131#
03132# SUM= 1.0 02:15R 1021.00 8.861 No_date 30:48 25.07 n/a .000
03133#
03134# SAVE HYD + 1.0 01:15R 46841.00 50.308 No_date 36:47 17.69 n/a .000
03135# name_R_H4.0005
03136# remark:flow at 8_N4
03137#
03138# Sum of hydrographs from Node 4 routed to Node 2
03139# Section 9
03140#
03141# ROUTES-CO0062-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03142# ROUTE CHANNEL -> 1.0 02:15R 48447.00 50.308 No_date 36:47 17.89 n/a .000
03143# [R/S= 1467./ .067/.040]
03144# [Vmax= .781;Dmax= 3.131]
03145#
03146# Addition of Subwatershed 2 with Monahan Drain and Smith Drain to Node 2
03147#
03148# ROUTES-CO0063-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03149# ADD HYD + 1.0 02:15R 48447.00 50.308 No_date 37:08 17.89 n/a .000
03150# [R/S= 1467./ .067/.040]
03151# [Vmax= .781;Dmax= 3.131]
03152#
03153# SUM= 1.0 02:15R 1122.00 8.165 No_date 31:48 25.62 n/a .000
03154#
03155# ROUTES-CO0064-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03156# SAVE HYD + 1.0 01:15R 52483.00 67.222 No_date 33:17 18.31 n/a .000
03157# name_H_LN2
03158# remark:flow at 8_N2 Jock River Gauge at Noddie Dr.
03159#
03160# Sum of hydrographs from Node 2 routed to Node 1
03161# Section 10
03162#
03163# Hydrograph from Node 2 routed to Node 416
03164# Channel X-Section obtained from RWCA Hydraulic Model - Station 9025
03165#
03166# ROUTES-CO0065-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03167# ROUTE CHANNEL -> 1.0 02:15R 52483.00 67.222 No_date 33:17 18.31 n/a .000
03168# [R/S= 1.001 out.- 1.0 01:15R 52483.00 65.464 No_date 33:17 18.31 n/a .000]
03169# [L/S= 2327./ .050/.055]
03170# [Vmax= 2.867;Dmax= 2.867]
03171#
03172# Catchment SW 1
03173# Portion of RWCA catchment SW 1 outside of Reach 1 subwatershed
03174# Undeveloped agricultural land
03175#
03176# ROUTES-CO0066-----Dtain-ID:HYND-----AREHA-GPEARCS-TpeaDate_hh:mm-----Rvm-R-C-----DWfms
03177# CONTINUOUS STANBYD + 1.0 01:15R 536.42 3.012 No_date 31:18 19.00 .333 .000
03178# [Cm= 72.01 Nv 3.001 Tpe= 2.79]
03179# [IARCC= 4.00; SMIN= 39.75; SMAX= 264.99; SK= .010]

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039677 ROD5-CO0109-2-DtmIn-DI-HSDY-AREHA-QPEA-CMS-TPeakDate_hh-mm-RvM-R-C-DWFCMS...
039678 CONTINUOUS STANDYD 2.0 01:01:28A-A 66.75 5.433 No.Date 28:11 42.06 7736 .000

04115 COMPUTE DIALHVD 1.0 01:3B-07 10.86 1.770 M.Date 28:00 50.11 n/a .000
04116 Major System / 1.0 01:3B-12-MJ 0.00 0.00 M.Date 28:00 00.00 n/a .000
04117 Minor System \ 1.0 01:3B-12-MN 10.86 1.770 M.Date 28:00 50.11 n/a .000
04118 [MjSystem:0000E+00, TotVolVol:0.000E+00, N-Ovr: 0, TotDurVrf: 0 hrs]

04302 [XMP:64-TIMP:64]
04303 [Horton parameters: Fw = 76.20Pc = 13.20DCAV+4.14 Pa = 0]
04304 [Previous area: IArea = 4.67:SLP2+1.00:LDP = 40.1MP = 250:SCP = 0]
04305 [Impervious area: IAlmp = 1.57:SLP1+1.00:LGI = 232.36MI = 013:SCI = 0]
04306 [IaBrcp = 4.00: IaBrcp = 4.00]

Table with columns: ID, Description, Values, Units, and Date. The table contains project data including pipe specifications, flow rates, and material properties.


```

071079 R0010-C00284-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
071080 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07109 [XIMP: 41-TIMP: 52]
07110 [LOSS: 2 CM: 75.0]
07111 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07112 [IntersectTime: 12.00]
07113 [IARCimp: 4.00: IARCEPex: 4.00]
07114 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07115 R0010-C00285-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07116 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07117 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07118 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07119 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07120 R0010-C00286-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07121 ADD HYD 1.0 02:10:02-MH2 2.10 2.688 No.Date 27:58 44.92 n/a .000
07122 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07123 [IntersectTime: 12.00]
07124 [IARCimp: 4.00: IARCEPex: 4.00]
07125 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07126 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07127 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07128 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07129 R0010-C00287-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07130 SAVE HYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07131 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07132 [IntersectTime: 12.00]
07133 [IARCimp: 4.00: IARCEPex: 4.00]
07134 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07135 remark: Total Flows at Todd Drain
07136 [Loss: 2 CM: 75.0]
07137 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07138 [IntersectTime: 12.00]
07139 [IARCimp: 4.00: IARCEPex: 4.00]
07140 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07141 # Rating curve obtained from Barbours SWP modeling
07142 # stantec 2007, Tributary Drainage Area to MSH Pond 3 = 193 ha
07143 R0010-C00288-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07144 ROUTE REVERSEVD -> 1.0 02:10:02-MH2 120.03 11.699 No.Date 28:03 44.79 n/a .000
07145 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07146 [IntersectTime: 12.00]
07147 [IARCimp: 4.00: IARCEPex: 4.00]
07148 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07149 overflow <= 1.0 03:19:00-OP 0.00 .000 No.Date 0:00 .00 n/a .000
07150 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07151 [IntersectTime: 12.00]
07152 [IARCimp: 4.00: IARCEPex: 4.00]
07153 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07154 R0010-C00289-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07155 ADD HYD 1.0 02:02:00-MH3 54937.78 87.200 No.Date 28:16 23.00 n/a .000
07156 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07157 [IntersectTime: 12.00]
07158 [IARCimp: 4.00: IARCEPex: 4.00]
07159 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07160 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07161 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07162 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07163 R0010-C00290-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07164 SAVE HYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07165 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07166 [IntersectTime: 12.00]
07167 [IARCimp: 4.00: IARCEPex: 4.00]
07168 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07169 remark: Total Flows at Todd Drain
07170 [Loss: 2 CM: 75.0]
07171 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07172 [IntersectTime: 12.00]
07173 [IARCimp: 4.00: IARCEPex: 4.00]
07174 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07175 # Hydrograph from Todd Drain routed to Corrigan Drain
07176 # Channel X-section obtained from RIVA Hydraulic Model - Station 2462
07177 # 2021-12-18 Change the bed slope from 0.13 (as per stantec 2007) to 0.05 so the model will be more stable and g
07178 R0010-C00291-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07179 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07180 [XIMP: 41-TIMP: 52]
07181 [LOSS: 2 CM: 75.0]
07182 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07183 [IntersectTime: 12.00]
07184 [IARCimp: 4.00: IARCEPex: 4.00]
07185 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07186 R0010-C00292-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07187 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07188 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07189 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07190 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07191 R0010-C00293-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07192 ADD HYD 1.0 02:02:00-MH3 54937.78 87.200 No.Date 28:16 23.00 n/a .000
07193 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07194 [IntersectTime: 12.00]
07195 [IARCimp: 4.00: IARCEPex: 4.00]
07196 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07197 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07198 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07199 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07200 R0010-C00294-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07201 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07202 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07203 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07204 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07205 R0010-C00295-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07206 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07207 [XIMP: 41-TIMP: 52]
07208 [LOSS: 2 CM: 75.0]
07209 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07210 [IntersectTime: 12.00]
07211 [IARCimp: 4.00: IARCEPex: 4.00]
07212 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07213 R0010-C00296-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07214 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07215 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07216 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07217 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07218 R0010-C00297-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07219 ADD HYD 1.0 02:02:00-MH3 54937.78 87.200 No.Date 28:16 23.00 n/a .000
07220 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07221 [IntersectTime: 12.00]
07222 [IARCimp: 4.00: IARCEPex: 4.00]
07223 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07224 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07225 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07226 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07227 R0010-C00298-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07228 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07229 [XIMP: 41-TIMP: 52]
07230 [LOSS: 2 CM: 75.0]
07231 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07232 [IntersectTime: 12.00]
07233 [IARCimp: 4.00: IARCEPex: 4.00]
07234 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07235 R0010-C00299-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07236 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07237 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07238 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07239 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07240 R0010-C00300-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07241 ADD HYD 1.0 02:02:00-MH3 54937.78 87.200 No.Date 28:16 23.00 n/a .000
07242 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07243 [IntersectTime: 12.00]
07244 [IARCimp: 4.00: IARCEPex: 4.00]
07245 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07246 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07247 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07248 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07249 R0010-C00301-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07250 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07251 [XIMP: 41-TIMP: 52]
07252 [LOSS: 2 CM: 75.0]
07253 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07254 [IntersectTime: 12.00]
07255 [IARCimp: 4.00: IARCEPex: 4.00]
07256 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07257 R0010-C00302-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07258 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07259 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07260 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07261 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07262 R0010-C00303-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07263 ADD HYD 1.0 02:02:00-MH3 54937.78 87.200 No.Date 28:16 23.00 n/a .000
07264 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07265 [IntersectTime: 12.00]
07266 [IARCimp: 4.00: IARCEPex: 4.00]
07267 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07268 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07269 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07270 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07271 R0010-C00304-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07272 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07273 [XIMP: 41-TIMP: 52]
07274 [LOSS: 2 CM: 75.0]
07275 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07276 [IntersectTime: 12.00]
07277 [IARCimp: 4.00: IARCEPex: 4.00]
07278 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07279 R0010-C00305-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07280 COMPUTE DUALHYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 n/a .000
07281 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07282 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07283 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07284 R0010-C00306-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07285 ADD HYD 1.0 02:02:00-MH3 54937.78 87.200 No.Date 28:16 23.00 n/a .000
07286 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07287 [IntersectTime: 12.00]
07288 [IARCimp: 4.00: IARCEPex: 4.00]
07289 [SMIN: 33.81: SMAK: 225.43: SK: .010]
07290 Major System / 1.0 02:01:24-MJ 0.00 .000 No.Date 28:00 44.90 n/a .000
07291 Minor System / 1.0 03:01:24-MN 25.50 1.818 No.Date 28:00 40.97 n/a .000
07292 [MjSysSto: .000000+0.0, TotDvFVol: .000000+0.0, N-DvF: 0, TotDvFovF: 0.0hrs]
07293 R0010-C00307-----DRAIN-ID:HYD-----AREAA-GPEAFCS-TPeakDate_hh:mm-----RVM-R.C-----DWPFMS
07294 CONTINUOUS STANDBYD 1.0 01:01:24 25.50 2.388 No.Date 28:03 40.97 633 .000
07295 [XIMP: 41-TIMP: 52]
07296 [LOSS: 2 CM: 75.0]
07297 [Previous area: IApex: 4.67:SLP:1.00:LD: 40.IMP: 250:SCP: .0]
07298 [IntersectTime: 12.00]
07299 [IARCimp: 4.00: IARCEPex: 4.00]
07300 [SMIN: 33.81: SMAK: 225.43: SK: .010]

```

Table with multiple columns containing technical data, including file names, dates, and numerical values. The data is organized into rows with various headers and sub-headers, such as 'Permissible area', 'Major System', and 'Minor System'. The table is truncated on the right side.


```

09351 # CONTINUOUS STANDBYD 1.0 01:18-13 10.19 1.930 No.Date 28:00 53.97 726 .000
09352 [XIMP: 64:TIMD:64]
09353 [Horton parameters] Fw = 76.20:Fc = 13.20:DOCV4:1.41 Fw .00]
09354 [Previous area] IAlp = 4.67:SLP2:0.10:LDL = 40:HMW:250:SPCV:0]
09355 [Impervious area] IAlp = 79:SLP1:0.10:LDL = 193:HMW:013:SPCV:0]
09356 [IaBcLp = 4.00: IaBcPcr = 4.00]
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10473# Parameters for IMPERVIOUS surfaces in STANDHYD:
10474# [kings 1.57 mm] [cru 1.50] [mtr .015]
10475# Parameters used in BASSETD:
10476# [a 1.0] [b 1.0] [c 1.0]
10477# Average monthly Pan Evaporation data in (mm)
10478# JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
10479# .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
10480# Average monthly Potential Evapotranspiration in (mm)
10481# JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
10482# .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
10483#-----
10484# R0550 C00010
10485# COMPUTE API
10486# [APIIn=50.00; APIQty= .8500; APIDir=.9988]
10487# [APITime=11.33; APIType= 44.87]
10488#
10489# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10490# # of 1.32
10491# R0550 C00010-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10492# CONTINUOUS NASHVD 1.0 01:28_RM 3840.00 18.440 No.Date 36:55 30.33 372 .000
10493# [Cm 64.0i No 3.00i Tp 1.13]
10494# [IAREK 4.00i SMIN= 35.67i SMAK=350.32i EK= .010]
10495# [InterVntTime= 12.00i]
10496#
10497# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10498# # of 1.32
10499# CONTINUOUS NASHVD 1.0 01:28_RM 3840.00 18.440 No.Date 36:55 30.33 372 .000
10500# [Cm 61.0i No 3.00i Tp 1.76]
10501# [IAREK 4.00i SMIN= 64.55i SMAK=430.01i EK= .010]
10502# [InterVntTime= 12.00i]
10503#
10504# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10505# # of 1.80
10506# R0550 C00010-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10507# CONTINUOUS NASHVD 1.0 01:28_RM 3074.00 8.912 No.Date 39:59 24.31 298 .000
10508# [Cm 55.0i No 3.00i Tp=1.33]
10509# [IAREK 4.00i SMIN= 83.24i SMAK=554.96i EK= .010]
10510# [InterVntTime= 12.00i]
10511# R0550 C00010-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10512# CONTINUOUS NASHVD 1.0 01:28_RM 3640.00 16.434 No.Date 32:39 36.65 452 .000
10513# [Cm 72.0i No 3.00i Tp= 3.91]
10514# [IAREK 4.00i SMIN= 35.67i SMAK=264.99i EK= .010]
10515# [InterVntTime= 12.00i]
10516# R0550 C00010-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10517# CONTINUOUS NASHVD 1.0 01:28_RM 4007.00 11.024 No.Date 39:59 31.73 389 .000
10518# [Cm 66.0i No 3.00i Tp 1.24]
10519# [IAREK 4.00i SMIN= 52.62i SMAK=350.79i EK= .010]
10520# [InterVntTime= 12.00i]
10521#
10522# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10523# # of 1.80
10524# R0550 C00011-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10525# CONTINUOUS NASHVD 1.0 01:28_RM 1917.00 12.342 No.Date 34:26 31.73 389 .000
10526# [Cm 66.0i No 3.00i Tp 1.24]
10527# [IAREK 4.00i SMIN= 52.62i SMAK=350.79i EK= .010]
10528# [InterVntTime= 12.00i]
10529#
10530# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10531# # of 1.52
10532# R0550 C00012-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10533# CONTINUOUS NASHVD 1.0 01:28_RM 5666.00 32.402 No.Date 37:52 36.85 452 .000
10534# [Cm 72.0i No 3.00i Tp= 4.00]
10535# [IAREK 4.00i SMIN= 39.75i SMAK=264.99i EK= .010]
10536# [InterVntTime= 12.00i]
10537#
10538# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10539# # of 1.75
10540# R0550 C00013-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10541# CONTINUOUS NASHVD 1.0 01:28_RM 8976.00 11.024 No.Date 39:59 31.73 389 .000
10542# [Cm 66.0i No 3.00i Tp=1.66]
10543# [IAREK 4.00i SMIN= 52.62i SMAK=350.79i EK= .010]
10544# [InterVntTime= 12.00i]
10545#
10546# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10547# # of 1.68
10548# R0550 C00014-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10549# CONTINUOUS NASHVD 1.0 01:28_RM 1132.00 14.039 No.Date 30:53 35.35 434 .000
10550# [Cm 73.0i No 3.00i Tp 1.42]
10551# [IAREK 4.00i SMIN= 43.07i SMAK=287.10i EK= .010]
10552# [InterVntTime= 12.00i]
10553#
10554# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10555# # of 1.82
10556# R0550 C00015-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10557# CONTINUOUS NASHVD 1.0 01:28_RM 4464.00 15.472 No.Date 39:59 28.95 355 .000
10558# [Cm 62.0i No 3.00i Tp=1.32]
10559# [IAREK 4.00i SMIN= 61.90i SMAK=412.66i EK= .010]
10560# [InterVntTime= 12.00i]
10561#
10562# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10563# # of 1.80
10564# R0550 C00016-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10565# CONTINUOUS NASHVD 1.0 01:28_RM 131.00 2.740 No.Date 28:57 29.64 364 .000
10566# [Cm 63.0i No 3.00i Tp= .90]
10567# [IAREK 4.00i SMIN= 59.42i SMAK=396.11i EK= .010]
10568# [InterVntTime= 12.00i]
10569#
10570# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10571# # of 1.65
10572# R0550 C00017-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10573# CONTINUOUS NASHVD 1.0 01:28_RM 3854.00 18.180 No.Date 38:32 31.73 389 .000
10574# [Cm 64.0i No 3.00i Tp 1.29]
10575# [IAREK 4.00i SMIN= 52.62i SMAK=350.79i EK= .010]
10576# [InterVntTime= 12.00i]
10577#
10578# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10579# # of 1.82
10580# R0550 C00018-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10581# CONTINUOUS NASHVD 1.0 01:28_RM 3197.00 13.937 No.Date 36:23 25.61 314 .000
10582# [Cm 57.0i No 3.00i Tp= 6.65]
10583# [IAREK 4.00i SMIN= 35.67i SMAK=508.81i EK= .010]
10584# [InterVntTime= 12.00i]
10585#
10586# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10587# # of 1.75
10588# R0550 C00019-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10589# CONTINUOUS NASHVD 1.0 01:28_RM 165.00 1.285 No.Date 33:02 32.44 398 .000
10590# [Cm 67.0i No 3.00i Tp= 4.18]
10591# [IAREK 4.00i SMIN= 35.67i SMAK=396.11i EK= .010]
10592# [InterVntTime= 12.00i]
10593#
10594# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10595# # of 1.41
10596# R0550 C00020-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10597# CONTINUOUS NASHVD 1.0 01:28_RM 1332.00 9.332 No.Date 35:12 36.85 452 .000
10598# [Cm 75.0i No 3.00i Tp= 1.98]
10599# [IAREK 4.00i SMIN= 39.75i SMAK=264.99i EK= .010]
10600# [InterVntTime= 12.00i]
10601# R0550 C00021-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10602# CONTINUOUS NASHVD 1.0 01:28_RM 224.00 8.187 No.Date 28:45 41.51 509 .000
10603# [Cm 77.0i No 3.00i Tp= .75]
10604# [IAREK 4.00i SMIN= 31.15i SMAK=207.66i EK= .010]
10605# [InterVntTime= 12.00i]
10606#
10607# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10608# # of 1.20
10609# R0550 C00022-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10610# CONTINUOUS NASHVD 1.0 01:28_RM 4945.00 14.623 No.Date 31:18 38.77 471 .000
10611# [Cm 74.0i No 3.00i Tp= 4.45]
10612# [IAREK 4.00i SMIN= 35.67i SMAK=244.49i EK= .010]
10613# [InterVntTime= 12.00i]
10614# R0550 C00023-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10615# CONTINUOUS NASHVD 1.0 01:28_RM 20.00 .543 No.Date 28:35 45.60 560 .000
10616# [Cm 61.0i No 3.00i Tp= .62]
10617# [IAREK 4.00i SMIN= 25.21i SMAK=168.09i EK= .010]
10618# [InterVntTime= 12.00i]
10619#
10620# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
10621# # of 1.61
10622# R0550 C00024-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10623# CONTINUOUS NASHVD 1.0 01:28_RM 1412.00 8.794 No.Date 37:48 39.93 490 .000
10624# [Cm 75.0i No 3.00i Tp= 1.78]
10625# [IAREK 4.00i SMIN= 33.81i SMAK=225.43i EK= .010]
10626# [InterVntTime= 12.00i]
10627# R0550 C00025-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10628# CONTINUOUS NASHVD 1.0 01:28_RM 585.00 12.896 No.Date 29:55 45.60 560 .000
10629# [Cm 81.0i No 3.00i Tp= 1.78]
10630# [IAREK 4.00i SMIN= 25.21i SMAK=168.09i EK= .010]
10631# [InterVntTime= 12.00i]
10632# R0550 C00026-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10633# CONTINUOUS NASHVD 1.0 01:28_RM 1021.00 17.059 No.Date 30:46 44.77 549 .000
10634# [Cm 80.0i No 3.00i Tp= 2.40]
10635# [IAREK 4.00i SMIN= 26.32i SMAK=175.50i EK= .010]
10636# [InterVntTime= 12.00i]
10637# R0550 C00027-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10638# CONTINUOUS NASHVD 1.0 01:28_RM 177.00 6.469 No.Date 28:45 41.51 509 .000
10639# [Cm 77.0i No 3.00i Tp= .75]
10640# [IAREK 4.00i SMIN= 31.15i SMAK=207.66i EK= .010]
10641# [InterVntTime= 12.00i]
10642# R0550 C00028-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10643# CONTINUOUS NASHVD 1.0 01:28_RM 1132.00 15.444 No.Date 31:43 45.60 560 .000
10644# [Cm 81.0i No 3.00i Tp= 3.25]
10645# [IAREK 4.00i SMIN= 31.15i SMAK=168.09i EK= .010]
10646# [InterVntTime= 12.00i]
10647# R0550 C00029-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10648# CONTINUOUS NASHVD 1.0 01:28_RM 2737.00 14.946 No.Date 31:29 40.72 500 .000
10649# [Cm 76.0i No 3.00i Tp= 3.03]
10650# [IAREK 4.00i SMIN= 31.15i SMAK=216.39i EK= .010]
10651# [InterVntTime= 12.00i]
10652#
10653# Routing Hydrographs
10654#
10655# Starting with the addition of Jock River Headwater and Subwatershed 13
10656#
10657# R0550 C00030-----DtnID-IDHYD-----AREHA-QPEARms-TpeakDate_hh:mm-----RvM-R-C-----DWPFms
10658# ADD HYD + 1.0 02:18_RM 3680.00 18.440 No.Date 36:55 30.33 n/a .000
10659# [SDR= 1.00i] out= 1.0 02:18_RM 971.00 6.937 No.Date 32:34 28.27 n/a .000

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Table with columns for ID, description, and numerical values. Includes sections for 'Section 5', 'Section 7', 'Section 8', 'Section 9', and 'Section 10'. Each entry typically contains a node ID, a description of the hydrograph or subwatershed, and several numerical data points.

134655 # [DRAIN]-----DRAIN-----ARESHA-GPEARCS-TrackDate,hh:mm-----Rvm-R-C-----DWPMCS
134656 ROUTE CHANNEL --> 1.0 02:18_N4 4684.01 89.756_Hdate 34:38 37.51 n/a .000
134657 [RDP: 1.00] out< 1.0 01:82 4844.00 96.418_Hdate 35:10 37.51 n/a .000
134658 [L/S/m: 4630./ .043/.055]
134659 [Vmax: .904/Dmax: 3.866]
13470 #
13471 #
13472 # Addition of Subwatershed 4 and Leamy Creek to Node 4
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15709 *** WARNING: New pipe size used for routing.
15710 RO100:C00378 ROUTE PIPE ->
15711 *** WARNING: New pipe size used for routing.
15712 RO100:C00378 ROUTE PIPE ->
15713 *** WARNING: New pipe size used for routing.
15714 RO100:C00379 ROUTE PIPE ->
15715 *** WARNING: New pipe size used for routing.
15716 RO100:C00303 ROUTE PIPE ->
15717 *** WARNING: New pipe size used for routing.
15718 RO100:C00309 ROUTE PIPE ->
15719 *** WARNING: New pipe size used for routing.
15720 RO100:C00325 ROUTE PIPE ->
15721 *** WARNING: New pipe size used for routing.
15722 RO100:C00326 ROUTE PIPE ->
15723 *** WARNING: New pipe size used for routing.
15724 RO100:C00334 ROUTE PIPE ->
15725 *** WARNING: New pipe size used for routing.
15726 RO100:C00342 DIVERT HYD ->
15727 *** NOTE: Inflow hyd. is dry and cannot be diverted.
15728 RO100:C00357 ROUTE PIPE ->
15729 *** WARNING: New pipe size used for routing.
15730 RO100:C00362 ROUTE PIPE ->
15731 *** WARNING: New pipe size used for routing.
15732 RO100:C00369 ROUTE PIPE ->
15733 *** WARNING: New pipe size used for routing.
15734 RO100:C00370 ROUTE PIPE ->
15735 *** WARNING: New pipe size used for routing.
15736 RO100:C00378 ROUTE PIPE ->
15737 *** WARNING: New pipe size used for routing.
15738 RO100:C00379 ROUTE PIPE ->
15739 *** WARNING: New pipe size used for routing.
15740 Simulation ended on 2021-03-04 at 11:53:36
15741 .....
15742
15743
```

Attachment D

Model 4A – Jock River Reach One Future Conditions – Without SWM controls

JFSA, 2021

SWMHYMO Input & Summary files

```

1  20  Metric units / ID numbers OFF
2  *#*****
3  *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *#*****
5  *# Project Name: [Jock River] Project Number: [1474-16]
6  *# Date : 04-03-2021
7  *# Modeller : [M.M.]
8  *# Company : JFSAinc.
9  *# License # : 2549237
10 *#*****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *# 2020-11-30 change TMJSTO in COMPUTE DUALHYD (TMJSTO = 0.1 instead of 0.0001)
16 *# 2020-12-01 correct pond curve values
17 *# 2020-12-01 change W_CLAR_BRAZ XIMP to 0.55, SLPI=[0.5](%) (impervious slope), and
LGI up to 700m
18 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NHYDout=["N_TO"]
,NHYDin=["SN_TO"]) from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model
will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELs
aren't well suited to really flat slopes.
19 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NHYDout=["N_WC"]
,NHYDin=["SN_CE"]) from 0.01 % (as per Stantec Report 2007) to 0.0255 % so the model
will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELs
aren't well suited to really flat slopes.
20 *
21 * Calibrated parameters for Summer 2003 data: APII=50, APIK=0.85, CN=varies,
22 * SK=0.01, InterEventTime=12,
23 * GWResk=0.96, VHydCond=0.055
24 *
25 *# -----
26 *
27 *START TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
28 * ["XAVG0315.STM"] average storm data a 15 minute time step
29 * The above rainf file is an average of the JFSA gauge data
30 * with the City of Ottawa rainfall data collected during
31 * the same period.
32 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
33 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
34 ["C24SC002.stm"] <--storm filename, one per line for NSTORM time
35 *%-----|-----|
36 *%-----|-----|
37 READ STORM STORM_FILENAME=["storm.001"]
38 *%-----|-----|
39 MODIFY STORM ICASEms=[1], NSHIFT=[96],
40 RedFACT=[1],
41 *%-----|-----|
42 DEFAULT VALUES ICASEdef=[1], read and print values
43 DEFVAL_FILENAME=["CitiGate.DEF"]
44 *%-----|-----|
45 COMPUTE API APII=[50], APIK=[.85]/day
46 *%-----|-----|
47 *%-----|-----|
48 *#
49 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
50 *# of 1.32
51 *%-----|-----|
52 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[1]min, AREA=[3680](ha),
53 DWF=[0](cms), CN/C=[64], IA=[2.5](mm),
54 N=[3.0], TP=[7.13]hrs,
55 Continuous simulation parameters:
56 IaRECper=[4](hrs),
57 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
58 InterEventTime=[12](hrs)
59 Baseflow simulation parameters:

```



```

60         BaseFlowOption=[1] ,
61         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
62         VHydCond=[0.055](mm/hr),   END=-1
63     *%-----|-----
64     *#
65     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
66     *# of 1.32
67     *%-----|-----
68     CONTINUOUS NASHYD  NHYD=["SW_13"], DT=[1]min, AREA=[971](ha),
69                        DWF=[0](cms),  CN/C=[61], IA=[2.5](mm),
70                        N=[3.0], TP=[3.76]hrs,
71                        Continuous simulation parameters:
72                        IaRECper=[4](hrs),
73                        SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
74                        InterEventTime=[12](hrs)
75                        Baseflow simulation parameters:
76                        BaseFlowOption=[1] ,
77                        InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
78                        VHydCond=[0.055](mm/hr),   END=-1
79     *%-----|-----
80     *#
81     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
82     *# of 1.80
83     *%-----|-----
84     CONTINUOUS NASHYD  NHYD=["JR_GWM"], DT=[1]min, AREA=[3074](ha),
85                        DWF=[0](cms),  CN/C=[55], IA=[2.5](mm),
86                        N=[3], TP=[11.33]hrs,
87                        Continuous simulation parameters:
88                        IaRECper=[4](hrs),
89                        SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
90                        InterEventTime=[12](hrs)
91                        Baseflow simulation parameters:
92                        BaseFlowOption=[1] ,
93                        InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
94                        VHydCond=[0.055](mm/hr),   END=-1
95     *%-----|-----
96     CONTINUOUS NASHYD  NHYD=["JR_ASH"], DT=[1]min, AREA=[1781](ha),
97                        DWF=[0](cms),  CN/C=[72], IA=[2.5](mm),
98                        N=[3.0], TP=[3.91]hrs,
99                        Continuous simulation parameters:
100                       IaRECper=[4](hrs),
101                       SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
102                       InterEventTime=[12](hrs)
103                       Baseflow simulation parameters:
104                       BaseFlowOption=[1] ,
105                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
106                       VHydCond=[0.055](mm/hr),   END=-1
107     *%-----|-----
108     CONTINUOUS NASHYD  NHYD=["SW_11"], DT=[1]min, AREA=[500](ha),
109                        DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),
110                        N=[3.0], TP=[1.24]hrs,
111                        Continuous simulation parameters:
112                        IaRECper=[4](hrs),
113                        SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
114                        InterEventTime=[12](hrs)
115                        Baseflow simulation parameters:
116                        BaseFlowOption=[1] ,
117                        InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
118                        VHydCond=[0.055](mm/hr),   END=-1
119     *%-----|-----
120     *#
121     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
122     *# of 1.80
123     *%-----|-----
124     CONTINUOUS NASHYD  NHYD=["NN_CK"], DT=[1]min, AREA=[1917](ha),
125                        DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),

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126 N=[3.0], TP=[5.29]hrs,
127 Continuous simulation parameters:
128 IaREcper=[4](hrs),
129 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
130 InterEventTime=[12](hrs)
131 Baseflow simulation parameters:
132 BaseFlowOption=[1] ,
133 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
134 VHydCond=[0.055](mm/hr), END=-1
135 *%-----|-----
136 *#
137 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
138 *# of 1.52
139 *%-----|-----
140 CONTINUOUS NASHYD NHYD=["SW_10"], DT=[1]min, AREA=[5666](ha),
141 DWF=[0](cms), CN/C=[72], IA=[2.5](mm),
142 N=[3.0], TP=[8.00]hrs,
143 Continuous simulation parameters:
144 IaREcper=[4](hrs),
145 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
146 InterEventTime=[12](hrs)
147 Baseflow simulation parameters:
148 BaseFlowOption=[1] ,
149 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
150 VHydCond=[0.055](mm/hr), END=-1
151 *%-----|-----
152 *#
153 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
154 *# of 1.75
155 *%-----|-----
156 CONTINUOUS NASHYD NHYD=["KG_CK"], DT=[1]min, AREA=[8376](ha),
157 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
158 N=[3.0], TP=[11.66]hrs,
159 Continuous simulation parameters:
160 IaREcper=[4](hrs),
161 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
162 InterEventTime=[12](hrs)
163 Baseflow simulation parameters:
164 BaseFlowOption=[1] ,
165 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
166 VHydCond=[0.055](mm/hr), END=-1
167 *%-----|-----
168 *#
169 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
170 *# of 1.68
171 *%-----|-----
172 CONTINUOUS NASHYD NHYD=["SW_9"], DT=[1]min, AREA=[1132](ha),
173 DWF=[0](cms), CN/C=[70], IA=[2.5](mm),
174 N=[3.0], TP=[2.51]hrs,
175 Continuous simulation parameters:
176 IaREcper=[4](hrs),
177 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
178 InterEventTime=[12](hrs)
179 Baseflow simulation parameters:
180 BaseFlowOption=[1] ,
181 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
182 VHydCond=[0.055](mm/hr), END=-1
183 *%-----|-----
184 *#
185 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
186 *# of 1.82
187 *%-----|-----
188 CONTINUOUS NASHYD NHYD=["NC_CK"], DT=[1]min, AREA=[4464](ha),
189 DWF=[0](cms), CN/C=[62], IA=[2.5](mm),
190 N=[3.0], TP=[11.32]hrs,
191 Continuous simulation parameters:

```

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192         IaREcper=[4](hrs),
193         SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
194         InterEventTime=[12](hrs)
195         Baseflow simulation parameters:
196         BaseFlowOption=[1] ,
197         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
198         VHydCond=[0.055](mm/hr),  END=-1
199     *%-----|-----
200     *#
201     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
202     *# of 1.80
203     *%-----|-----
204     CONTINUOUS NASHYD  NHYD=["SW_8"], DT=[1]min, AREA=[131](ha),
205                       DWF=[0](cms),  CN/C=[63], IA=[2.5](mm),
206                       N=[3.0], TP=[0.90]hrs,
207                       Continuous simulation parameters:
208                       IaREcper=[4](hrs),
209                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
210                       InterEventTime=[12](hrs)
211                       Baseflow simulation parameters:
212                       BaseFlowOption=[1] ,
213                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
214                       VHydCond=[0.055](mm/hr),  END=-1
215     *%-----|-----
216     *#
217     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
218     *# of 1.65
219     *%-----|-----
220     CONTINUOUS NASHYD  NHYD=["HB_DR"], DT=[1]min, AREA=[3854](ha),
221                       DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),
222                       N=[3.0], TP=[8.42]hrs,
223                       Continuous simulation parameters:
224                       IaREcper=[4](hrs),
225                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
226                       InterEventTime=[12](hrs)
227                       Baseflow simulation parameters:
228                       BaseFlowOption=[1] ,
229                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
230                       VHydCond=[0.055](mm/hr),  END=-1
231     *%-----|-----
232     *#
233     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
234     *# of 1.82
235     *%-----|-----
236     CONTINUOUS NASHYD  NHYD=["SW_7"], DT=[1]min, AREA=[3197](ha),
237                       DWF=[0](cms),  CN/C=[57], IA=[2.5](mm),
238                       N=[3.0], TP=[6.65]hrs,
239                       Continuous simulation parameters:
240                       IaREcper=[4](hrs),
241                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),
242                       InterEventTime=[12](hrs)
243                       Baseflow simulation parameters:
244                       BaseFlowOption=[1] ,
245                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
246                       VHydCond=[0.055](mm/hr),  END=-1
247     *%-----|-----
248     *#
249     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
250     *# of 1.75
251     *%-----|-----
252     CONTINUOUS NASHYD  NHYD=["SW_6"], DT=[1]min, AREA=[165](ha),
253                       DWF=[0](cms),  CN/C=[67], IA=[2.5](mm),
254                       N=[3.0], TP=[4.18]hrs,
255                       Continuous simulation parameters:
256                       IaREcper=[4](hrs),
257                       SMIN=[-1](mm),  SMAx=[-1](mm), SK=[0.010]/(mm),

```

```

258 InterEventTime=[12](hrs)
259 Baseflow simulation parameters:
260 BaseFlowOption=[1] ,
261 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
262 VHydCond=[0.055](mm/hr) , END=-1
263 *%-----|-----
264 *#
265 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
266 *# of 1.67
267 *%-----|-----
268 CONTINUOUS NASHYD NHYD=["VG_DR"] , DT=[1]min , AREA=[1332](ha) ,
269 DWF=[0](cms) , CN/C=[72] , IA=[2.5](mm) ,
270 N=[3.0] , TP=[5.95]hrs ,
271 Continuous simulation parameters:
272 IaREcper=[4](hrs) ,
273 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
274 InterEventTime=[12](hrs)
275 Baseflow simulation parameters:
276 BaseFlowOption=[1] ,
277 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
278 VHydCond=[0.055](mm/hr) , END=-1
279 *%-----|-----
280 CONTINUOUS NASHYD NHYD=["SW_5"] , DT=[1]min , AREA=[224](ha) ,
281 DWF=[0](cms) , CN/C=[77] , IA=[2.5](mm) ,
282 N=[3.0] , TP=[0.75]hrs ,
283 Continuous simulation parameters:
284 IaREcper=[4](hrs) ,
285 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
286 InterEventTime=[12](hrs)
287 Baseflow simulation parameters:
288 BaseFlowOption=[1] ,
289 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
290 VHydCond=[0.055](mm/hr) , END=-1
291 *%-----|-----
292 *#
293 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
294 *# of 1.20
295 *%-----|-----
296 CONTINUOUS NASHYD NHYD=["FL_CK"] , DT=[1]min , AREA=[4945](ha) ,
297 DWF=[0](cms) , CN/C=[74] , IA=[2.5](mm) ,
298 N=[3.0] , TP=[4.45]hrs ,
299 Continuous simulation parameters:
300 IaREcper=[4](hrs) ,
301 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
302 InterEventTime=[12](hrs)
303 Baseflow simulation parameters:
304 BaseFlowOption=[1] ,
305 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
306 VHydCond=[0.055](mm/hr) , END=-1
307 *%-----|-----
308 CONTINUOUS NASHYD NHYD=["SW_5A2"] , DT=[1]min , AREA=[20](ha) ,
309 DWF=[0](cms) , CN/C=[81] , IA=[2.5](mm) ,
310 N=[3.0] , TP=[0.62]hrs ,
311 Continuous simulation parameters:
312 IaREcper=[4](hrs) ,
313 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
314 InterEventTime=[12](hrs)
315 Baseflow simulation parameters:
316 BaseFlowOption=[1] ,
317 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
318 VHydCond=[0.055](mm/hr) , END=-1
319 *%-----|-----
320 *#
321 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
322 *# of 1.61
323 *%-----|-----

```

```

324 CONTINUOUS NASHYD NHYD=["SW_5A1"], DT=[1]min, AREA=[1412](ha),
325 DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
326 N=[3.0], TP=[8.00]hrs,
327 Continuous simulation parameters:
328 IaREcper=[4](hrs),
329 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
330 InterEventTime=[12](hrs)
331 Baseflow simulation parameters:
332 BaseFlowOption=[1] ,
333 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
334 VHydCond=[0.055](mm/hr), END=-1
335 *%-----|
336 CONTINUOUS NASHYD NHYD=["SW_4"], DT=[1]min, AREA=[585](ha),
337 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
338 N=[3.0], TP=[1.75]hrs,
339 Continuous simulation parameters:
340 IaREcper=[4](hrs),
341 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
342 InterEventTime=[12](hrs)
343 Baseflow simulation parameters:
344 BaseFlowOption=[1] ,
345 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
346 VHydCond=[0.055](mm/hr), END=-1
347 *%-----|
348 CONTINUOUS NASHYD NHYD=["LM_CK"], DT=[1]min, AREA=[1021](ha),
349 DWF=[0](cms), CN/C=[80], IA=[2.5](mm),
350 N=[3.0], TP=[2.46]hrs,
351 Continuous simulation parameters:
352 IaREcper=[4](hrs),
353 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
354 InterEventTime=[12](hrs)
355 Baseflow simulation parameters:
356 BaseFlowOption=[1] ,
357 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
358 VHydCond=[0.055](mm/hr), END=-1
359 *%-----|
360 CONTINUOUS NASHYD NHYD=["SW_2"], DT=[1]min, AREA=[177](ha),
361 DWF=[0](cms), CN/C=[77], IA=[2.5](mm),
362 N=[3.0], TP=[0.75]hrs,
363 Continuous simulation parameters:
364 IaREcper=[4](hrs),
365 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
366 InterEventTime=[12](hrs)
367 Baseflow simulation parameters:
368 BaseFlowOption=[1] ,
369 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
370 VHydCond=[0.055](mm/hr), END=-1
371 *%-----|
372 CONTINUOUS NASHYD NHYD=["SM_DR"], DT=[1]min, AREA=[1122](ha),
373 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
374 N=[3.0], TP=[3.25]hrs,
375 Continuous simulation parameters:
376 IaREcper=[4](hrs),
377 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
378 InterEventTime=[12](hrs)
379 Baseflow simulation parameters:
380 BaseFlowOption=[1] ,
381 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
382 VHydCond=[0.055](mm/hr), END=-1
383 *%-----|
384 CONTINUOUS NASHYD NHYD=["MO_DR"], DT=[1]min, AREA=[2737](ha),
385 DWF=[0](cms), CN/C=[76], IA=[2.5](mm),
386 N=[3.0], TP=[3.03]hrs,
387 Continuous simulation parameters:
388 IaREcper=[4](hrs),
389 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

```



```

456 (cms) - (ha-m)
457 [ 0.0 , 0.0 ]
458 [1.991, 2.144 ]
459 [2.693, 39.826 ]
460 [3.509, 81.697 ]
461 [4.578, 318.774 ]
462 [5.647, 594.947 ]
463 [7.109, 910.219 ]
464 [8.616, 1264.589 ]
465 [10.371, 1658.057 ]
466 [12.402, 2090.622 ]
467 [22.056, 3462.487 ]
468 [ -1 , -1 ] (max twenty pts)
469
470 NHYDovf=[" " ] ,
471
472 *%-----|-----
473 *#
474 SAVE HYD NHYD=["RES_GM"], # OF PCYCLES=[-1], ICASEsh=[-1]
475 HYD_FILENAME=["H_RESGM"]
476 HYD_COMMENT=["Outflow from Res GM"]
477
478 *%-----|-----
479 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
480 *# (Approximated cross-section - see cross-section 258)
481 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
482 ROUTE CHANNEL NHYDout=["N12"] ,NHYDin=["RES_GM"] ,
483 RDT=[1](min),
484 CHLGTH=[5926](m), CHSLOPE=[0.0759](%),
485 FPSLOPE=[0.0759](%),
486 SECNUM=[1.0], NSEG=[1]
487 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
488 ( DISTANCE (m), ELEVATION (m))=
489 [-40, 132.5]
490 [-30, 132]
491 [-25, 131.5]
492 [-13, 130]
493 [-8, 127.00]
494 [-7, 126.50]
495 [-6, 126]
496 [-5.5, 125.50]
497 [0, 123.75]
498 [4.5, 125.50]
499 [6, 126]
500 [7.5, 126.5]
501 [9, 127]
502 [10, 127.5]
503 [11.5, 128.00]
504 [15.5, 129.5]
505
506 *%-----|-----
507 *#
508 *# Addition of Subwatershed Jock River at Ashton to Node 12
509 *#
510 ADD HYD NHYDsum=["S_N12"], NHYDs to add=["N12"+"JR_ASH"]
511 SAVE HYD NHYD=["S_N12"], # OF PCYCLES=[-1], ICASEsh=[-1]
512 HYD_FILENAME=["H_SN12"]
513 HYD_COMMENT=["flow at S_N12 near Ashton"]
514
515 *%-----|-----
516 *#
517 *# Sum of hydrographs from Node 12 routed to Node 11
518 *# (Approximated cross-section - see cross-section 258)
519 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
520 *# ROUTE CHANNEL NHYDout=["N11"] ,NHYDin=["S_N12"] ,
521 * RDT=[1](min),
522 * CHLGTH=[972](m), CHSLOPE=[0.0514](%),
523 * FPSLOPE=[0.0514](%),
524 * SECNUM=[1.0], NSEG=[1]
525 * ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
526 * ( DISTANCE (m), ELEVATION (m))=

```

```

522 * [-40, 132.5]
523 * [-30, 132]
524 * [-25, 131.5]
525 * [-13, 130]
526 * [-8, 127.00]
527 * [-7, 126.50]
528 * [-6, 126]
529 * [-5.5, 125.50]
530 * [0, 123.75]
531 * [4.5, 125.50]
532 * [6, 126]
533 * [7.5, 126.5]
534 * [9, 127]
535 * [10, 127.5]
536 * [11.5, 128.00]
537 * [15.5, 129.5]
538 *%-----|-----
539 *#
540 *# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
541 *#
542 ROUTE CHANNEL NHYDout=["Dum11"] ,NHYDin=["S_N12"] ,
543 RDT=[1](min),
544 CHLGTH=[972](m), CHSLOPE=[0.054](%),
545 FPSLOPE=[0.054](%),
546 SECNUM=[1.0], NSEG=[1]
547 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
548 ( DISTANCE (m), ELEVATION (m))=
549 [-40, 132.5]
550 [-30, 132]
551 [-25, 131.5]
552 [-13, 130]
553 [-8, 127.00]
554 [-7, 126.50]
555 [-6, 126]
556 [-5.5, 125.50]
557 [0, 123.75]
558 [4.5, 125.50]
559 [6, 126]
560 [7.5, 126.5]
561 [9, 127]
562 [10, 127.5]
563 [11.5, 128.00]
564 [15.5, 129.5]
565 *%-----|-----
566 *#
567 *# Addition of Subwatershed 11 and No Name Creek to Node 11
568 *#
569 ADD HYD NHYDsum=["S_N11"], NHYDs to add=["Dum11"+"SW_11"+"NN_CK"]
570 *%-----|-----
571 *#
572 *# Sum of hydrographs from Node 11 routed to Node 10
573 *# Section 1
574 *#
575 ROUTE CHANNEL NHYDout=["N10"] ,NHYDin=["S_N11"] ,
576 RDT=[1](min),
577 CHLGTH=[14028](m), CHSLOPE=[0.1568](%),
578 FPSLOPE=[0.1568](%),
579 SECNUM=[1.0], NSEG=[5]
580 ( SEGROUGH, SEGDIST (m))=
581 [0.04,-52.82
582 0.1,-6.47
583 -0.05,6.47
584 0.1,45.36
585 0.04,423.88] NSEG times
586 ( DISTANCE (m), ELEVATION (m))=
587 [-226.24 ,112.50]

```



```

588 [-167.50 ,111.50]
589 [-106.81 ,111.00]
590 [-92.37 ,110.00]
591 [-52.82 ,109.00]
592 [-24.90, 109.00]
593 [-17.02, 108.50]
594 [-6.47, 108.00]
595 [6.47, 108.00]
596 [15.67, 108.50]
597 [18.95, 109.00]
598 [45.36, 109.50]
599 [120.79, 110.00]
600 [145.72, 111.00]
601 [181.56, 111.50]
602 [423.88, 112.50]
603 *%-----|-----|
604 *#
605 *# Addition of Subwatershed 10 to Node 10
606 *#
607 ADD HYD          NHYDsum=["S_N10"], NHYDs to add=["N10"+"SW_10"]
608 *%-----|-----|
609 SAVE HYD       NHYD=["S_N10"], # OF PCYCLES=[-1], ICASEsh=[-1]
610                   HYD_FILENAME=["H_SN10"]
611                   HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
612 *%-----|-----|
613 *# Addition of Kings Creek to S_N10
614 *#
615 ADD HYD          NHYDsum=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
616 *%-----|-----|
617 *#
618 *# Sum of hydrographs from Node 10 routed to Node 9
619 *# Section 2
620 *#
621 ROUTE CHANNEL   NHYDout=["N9"] ,NHYDin=["S_N10A"] ,
622                   RDT=[1](min),
623                   CHLGTH=[3982](m),  CHSLOPE=[0.0753](%),
624                                           FPSLOPE=[0.0753](%),
625                   SECNUM=[1.0],      NSEG=[4]
626                   ( SEGROUGH, SEGDIST (m))=
627                     [0.04,-30.27
628                     0.05,-18.42
629                     -0.05,18.42
630                     0.04,131.58] NSEG times
631                   ( DISTANCE (m), ELEVATION (m))=
632                     [-446.74, 106.00]
633                     [-415.68, 105.50]
634                     [-285.40, 105.00]
635                     [-173.77, 104.50]
636                     [-144.95, 104.00]
637                     [-111.18, 103.50]
638                     [-94.06, 103.00]
639                     [-71.02, 102.50]
640                     [-30.27, 102.00]
641                     [-19.33, 100.00]
642                     [-18.42, 99.50]
643                     [18.42, 99.50]
644                     [20.77, 100.00]
645                     [27.93, 101.00]
646                     [52.29, 101.00]
647                     [68.80, 101.50]
648                     [79.66, 103.00]
649                     [91.50, 103.50]
650                     [131.58, 104.00]
651 *%-----|-----|
652 *#
653 *# Addition of Subwatershed 9 and Nichols Creek to Node 9

```

```

654  *#
655  ADD HYD          NHYDsum=["S_N9"], NHYDs to add=["N9"+"SW_9"+"NC_CK"]
656  *%-----|-----
657  *#
658  *# Sum of hydrographs from Node 9 routed to Node 8
659  *# Section 3
660  *#
661  ROUTE CHANNEL    NHYDout=["N8"] ,NHYDin=["S_N9"] ,
662                  RDT=[1](min),
663                  CHLGTH=[2269](m),  CHSLOPE=[0.0882](%),
664                                          FPSLOPE=[0.0882](%),
665                  SECNUM=[1.0],      NSEG=[3]
666                  ( SEGROUGH, SEGDIST (m))=
667                    [0.1,-17.99
668                    -0.045,17.31
669                    0.1,456.58] NSEG times
670                  ( DISTANCE (m), ELEVATION (m))=
671                    [-201.19,100.50]
672                    [-135.21, 100.00]
673                    [-94.83, 99.50]
674                    [-67.05, 99.00]
675                    [-17.99, 98.50]
676                    [-16.02, 98.00]
677                    [-13.95, 97.50]
678                    [13.95, 97.50]
679                    [15.64, 98.00]
680                    [17.31, 98.50]
681                    [162.02, 98.50]
682                    [172.89 ,99.00]
683                    [314.38, 99.00]
684                    [343.78, 99.50]
685                    [365.67, 100.00]
686                    [376.68, 100.00 ]
687                    [393.11, 99.50]
688                    [404.97, 99.50]
689                    [431.70, 100.00]
690                    [456.58, 100.50 ]
691  *%-----|-----
692  *#
693  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
694  *#
695  ADD HYD          NHYDsum=["S_N8"], NHYDs to add=["N8"+"SW_8"+"HB_DR"]
696  *%-----|-----
697  *#
698  *# Sum of hydrographs from Node 8 routed to Node 7
699  *# Section 4
700  *#
701  ROUTE CHANNEL    NHYDout=["N7"] ,NHYDin=["S_N8"],
702                  RDT=[1](min),
703                  CHLGTH=[3750](m),  CHSLOPE=[0.0533](%),
704                                          FPSLOPE=[0.0533](%),
705                  SECNUM=[1.0],      NSEG=[3]
706                  ( SEGROUGH, SEGDIST (m))=
707                    [0.12,-18.11
708                    -0.07,17.22
709                    0.12,590.05] NSEG times
710                  ( DISTANCE (m), ELEVATION (m))=
711                    [-433.21, 102.00]
712                    [-425.34, 101.50]
713                    [-377.56, 101.50]
714                    [-366.23, 101.00]
715                    [-202.60, 100.50]
716                    [-96.25, 99.50]
717                    [-68.36 99.00]
718                    [-18.11, 98.50]
719                    [-13.81, 97.50]

```

```

720 [13.81, 97.50]
721 [17.22, 98.50]
722 [161.95, 98.50]
723 [173.11, 99.00]
724 [314.05, 99.00]
725 [365.52, 100.00]
726 [404.70, 99.50]
727 [476.74, 100.50]
728 [502.31, 101.00]
729 [584.69, 101.00]
730 [585.79, 101.00]
731 [590.05, 102.00]
732 *%-----|-----
733 *#
734 *# Addition of Subwatershed 7 to Node 7
735 *#
736 ADD HYD NHYDsum=["S_N7"], NHYDs to add=["N7"+"SW_7"]
737 *%-----|-----
738 SAVE HYD NHYD=["S_N7"], # OF PCYCLES=[-1], ICASEsh=[-1]
739 HYD_FILENAME=["H_SN7"]
740 HYD_COMMENT=["flow at S_N7: N7 + SW_7"]
741 *%-----|-----
742 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
743 *# Storage area and volumes were estimated from available topo maps.
744 *# Release rate from fen was assumed to be controlled by the downstream
745 *# river cross-section for summer conditions. It is was assumed that for up to
746 *# 0.75 m of water, the main channel of the river provided the storage. Above
747 *# this depth, the wetland starts to signigicantly store water.
748 *#
749 ROUTE RESERVOIR NHYDout=["RES_RF"] ,NHYDin=["S_N7"] ,
750 RDT=[1](min),
751 TABLE of ( OUTFLOW-STORAGE ) values
752 (cms) - (ha-m)
753 TABLE of ( OUTFLOW-STORAGE ) values
754 (cms) - (ha-m)
755 [ 0.0 , 0.0 ]
756 [0.9051, 2.40]
757 [2.907, 4.13]
758 [9.744, 9.18]
759 [20.304, 14.96]
760 [34.167, 310.21]
761 [74.993, 605.46]
762 [104.876, 900.71]
763 [140.56, 2892.00]
764 [225.00, 3615.63]
765 [ -1 , -1 ] (max twenty pts)
766 NHYDovf=[" " ] ,
767 *%-----|-----
768 SAVE HYD NHYD=["RES_RF"], # OF PCYCLES=[-1], ICASEsh=[-1]
769 HYD_FILENAME=["H_ResRF"]
770 HYD_COMMENT=["outflow of Richmond Fen"]
771 *%-----|-----
772 *#
773 *# Sum of hydrographs from Node 7 routed to Node 6
774 *# Section 5
775 *#
776 ROUTE CHANNEL NHYDout=["N6"] ,NHYDin=["RES_RF"] ,
777 RDT=[1](min),
778 CHLGTH=[3056](m), CHSLOPE=[0.0818](%),
779 FPSLOPE=[0.0818](%),
780 SECNUM=[1.0], NSEG=[5]
781 ( SEGROUGH, SEGDIST (m))=
782 [0.025,-70.8
783 0.1,-23.9
784 -0.05,23.9
785 0.06,39.8

```

```

786             0.05,96.3] NSEG times
787             ( DISTANCE (m), ELEVATION (m))=
788                 [-100.8, 97.00]
789                 [-70.8, 96.50]
790                 [-52.0, 96.00]
791                 [-35.1, 95.50]
792                 [-30.6, 95.00]
793                 [-23.9, 94.54]
794                 [23.9, 94.54]
795                 [39.8, 95.00]
796                 [50.4, 95.50]
797                 [93.5, 96.00]
798                 [94.9, 96.50]
799                 [96.3, 97.00]
800 *%-----|-----
801 *#
802 *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
803 *#
804 ADD HYD             NHYDsum=["S_N6"], NHYDs to add=["N6"+"SW_6"+"VG_DR"]
805 *%-----|-----
806 *#
807 *# Sum of hydrographs from Node 6 routed to Node 5
808 *# Section 6
809 *#
810 ROUTE CHANNEL     NHYDout=["N5"] ,NHYDin=["S_N6"] ,
811                     RDT=[1](min),
812                     CHLGTH=[1852](m),   CHSLOPE=[0.0540](%),
813                                     FPSLOPE=[0.0540](%),
814                     SECNUM=[1.0],       NSEG=[3]
815                     ( SEGROUGH, SEGDIST (m))=
816                         [0.035,-131.59
817                         -0.045,48.96
818                         0.1,239.04] NSEG times
819                     ( DISTANCE (m), ELEVATION (m))=
820                         [-686.30, 94.50]
821                         [-675.70, 94.00]
822                         [-492.52, 93.00]
823                         [-467.28, 94.00]
824                         [-131.59, 94.00]
825                         [-92.79, 92.50]
826                         [-18.06, 91.00]
827                         [18.06, 91.00]
828                         [43.47, 92.50]
829                         [48.96, 94.00]
830                         [177.43, 94.00]
831                         [239.04,94.50]
832 *%-----|-----
833 *#
834 *# Addition of Subwatershed 5 and Flowing Creek to Node 5
835 *#
836 ADD HYD             NHYDsum=["S_N5"], NHYDs to add=["N5"+"SW_5"+"FL_CK"]
837 *%-----|-----
838 *#
839 *# Sum of hydrographs from Node 5 routed to Node 5A
840 *# Section 7
841 *#
842 ROUTE CHANNEL     NHYDout=["N5A"] ,NHYDin=["S_N5"] ,
843                     RDT=[1](min),
844                     CHLGTH=[556](m),   CHSLOPE=[0.0900](%),
845                                     FPSLOPE=[0.0900](%),
846                     SECNUM=[1.0],       NSEG=[4]
847                     ( SEGROUGH, SEGDIST (m))=
848                         [0.04,-41.5
849                         0.1,-14.0
850                         -0.045,14.0
851                         0.1,41.1] NSEG times

```

```

852          ( DISTANCE (m), ELEVATION (m))=
853              [-275.8, 93.00]
854              [-248.6, 92.50]
855              [-237.0, 92.00]
856              [-219.3, 91.50]
857              [-202.1, 91.50]
858              [-186.0, 92.00]
859              [-129.2, 92.00]
860              [-117.6, 91.50]
861              [-100.6, 91.00]
862              [-41.5, 91.00]
863              [-20.0, 91.00]
864              [-14.0, 90.54]
865              [14.0, 90.54]
866              [15.3, 91.00]
867              [17.3, 91.50]
868              [38.4, 92.00]
869              [39.8, 92.50]
870              [41.1, 93.00]
871  *%-----|-----
872  *#
873  *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
874  *#
875  ADD HYD          NHYDsum=["S_N5A"], NHYDs to add=["N5A"+"SW_5A2"+"SW_5A1"]
876  *%-----|-----
877  *#
878  *# Sum of hydrographs from Node 5A routed to Node 4
879  *# Section 8
880  *#
881  ROUTE CHANNEL    NHYDout=["N4"] ,NHYDin=["S_N5A"] ,
882                  RDT=[1](min),
883                  CHLGTH=[4630](m),  CHSLOPE=[0.0432](%),
884                                          FPSLOPE=[0.0432](%),
885                  SECNUM=[1.0],      NSEG=[3]
886                  ( SEGROUGH, SEGDIST (m))=
887                      [0.05,-28.2
888                      -0.035,28.2
889                      0.05,173.1] NSEG times
890                  ( DISTANCE (m), ELEVATION (m))=
891                      [-38.9, 92.00]
892                      [-35.8, 91.50]
893                      [-33.3, 91.00]
894                      [-28.2, 90.50]
895                      [-15.0, 87.48]
896                      [-5.0, 88.34]
897                      [5.0, 86.20]
898                      [15.0, 88.55]
899                      [28.2, 90.50]
900                      [29.7, 91.00]
901                      [46.5, 91.00]
902                      [127.8, 91.00]
903                      [148.7, 91.50]
904                      [173.1, 92.00]
905  *%-----|-----
906  *#
907  *# Addition of Subwatershed 4 and Leamy Creek to Node 4
908  *#
909  ADD HYD          NHYDsum=["S_N4"], NHYDs to add=["N4"+"SW_4"+"LM_CK"]
910  SAVE HYD        NHYD=["S_N4"], # OF PCYCLES=[-1], ICASEsh=[1]
911                  HYD_COMMENT=["flow at S_N4"]
912  *%-----|-----
913  *#
914  *# Sum of hydrographs from Node 4 routed to Node 2
915  *# Section 9
916  *#
917  ROUTE CHANNEL    NHYDout=["N2"] ,NHYDin=["S_N4"] ,

```

```

918 RDT=[1](min),
919 CHLGTH=[1667](m), CHSLOPE=[0.0600](%),
920 FPSLOPE=[0.0600](%),
921 SECNUM=[1.0], NSEG=[4]
922 ( SEGROUGH, SEGDIST (m))=
923 [0.1,-28.0
924 -0.04,28.4
925 0.06,31.7
926 0.04,80.2] NSEG times
927 ( DISTANCE (m), ELEVATION (m))=
928 [-36.3, 92.00]
929 [-32.6, 91.50]
930 [-30.2, 91.00]
931 [-28.0, 90.45]
932 [-15.0, 87.48]
933 [-5.0, 88.34]
934 [5.0, 86.20]
935 [15.0, 88.55]
936 [28.0, 90.45]
937 [28.4, 90.50]
938 [30.4, 91.00]
939 [31.7, 91.50]
940 [80.2, 92.00]
941 *%-----|-----
942 *#
943 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
944 *#
945 ADD HYD NHYDsum=["S_N2"], NHYDs to add=["N2"+"SW_2"+"SM_DR"+"MO_DR"]
946 *%-----|-----
947 SAVE HYD NHYD=["S_N2"], # OF PCYCLES=[-1], ICASEsh=[-1]
948 HYD_FILENAME=["H_SN2"]
949 HYD_COMMENT=["flow at S_N2 Jock River Gauge at Moodie Dr."]
950 *%-----|-----
951 *#
952 *# Sum of hydrographs from Node 2 routed to Node 1
953 *# Section 10
954 *#
955 *#*****
956 *%READ HYD NHYD=["S_N2"],
957 *% HYD_FILENAME=["H-S_N2"]
958 *%-----|-----
959 *#
960 *# Hydrograph from Node 2 routed to Node 416
961 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
962 *#
963 ROUTE CHANNEL NHYDout=["N_416"] ,NHYDin=["S_N2"] ,
964 RDT=[1](min),
965 CHLGTH=[2327](m), CHSLOPE=[0.0498](%),
966 FPSLOPE=[0.0498](%),
967 SECNUM=[1.0], NSEG=[3]
968 ( SEGROUGH, SEGDIST (m))=
969 [0.075,-23.96
970 -0.055,23.96
971 0.075,157.38] NSEG times
972 ( DISTANCE (m), ELEVATION (m))=
973 [-336.97,93.5]
974 [-318.85,93]
975 [-259,92.5]
976 [-133.18,92]
977 [-33.17,92]
978 [-27.21,92]
979 [-26.14,91.5]
980 [-24.99,91]
981 [-23.96,90.5]
982 [-14.33,88.26]
983 [-0.68,88.12]

```

```

984 [14.33,88.26]
985 [23.96,90.5]
986 [32.12,91]
987 [43.74,91.5]
988 [57.09,92]
989 [73.53,92.5]
990 [108.27,93]
991 [125.88,93.5]
992 [144.81,94]
993 [157.38,94.5]
994 *%-----|-----|
995 *#*****|
996 *# Catchment SW-1a
997 *# - Portion of RVCA catchment SW_1 outside of Reach 1 subwatershed
998 *# - Undeveloped agricultural land
999 *#*****|
1000 CONTINUOUS NASHYD NHYD=["SW_1a"], DT=[1]min, AREA=[536.42](ha),
1001 DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
1002 N=[3], TP=[2.79]hrs,
1003 Continuous simulation parameters:
1004 IaREcper=[4](hrs),
1005 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1006 InterEventTime=[12](hrs)
1007 Baseflow simulation parameters:
1008 BaseFlowOption=[1],
1009 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1010 VHydCond=[0.055](mm/hr), END=-1
1011 *%-----|-----|
1012 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
1013 CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[1](min), AREA=[44.93](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
1014 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAPER=[4.67](mm), SLPP=[2.0](%),
1015 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
1016 LGI=[547.296](m), MNI=[0.013], SCI=[0](min),
1017 Continuous simulation parameters:
1018 IaREcper=[4](hrs), IaREcimp=[4](hrs),
1019 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1020 InterEventTime=[12](hrs), END=-1
1021 *%-----|-----|
1022 *COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CINLET=[4.796](cms), NINLET=[1],
1023 * MajNHYD=["S-1-OkMJ"]
1024 * MinNHYD=["S-1-OkMN"]
1025 * TMJSTO=[9999999](cu-m)
1026 *%-----|-----|
1027 *ADD HYD NHYDsum=["S-1-OkS"], NHYDs to add=["S-1-OkMJ"+"S-1-OkMN"]
1028 *%-----|-----|
1029 *ROUTE RESERVOIR NHYDout=["S-1-OkSR"],NHYDin=["S-1-OkS"],
1030 * RDT=[1](min),
1031 * TABLE of ( OUTFLOW-STORAGE ) values
1032 * (cms) - (ha-m)
1033 * [ 0.0 , 0.0 ]
1034 * [ 0.5370, 1.7917 ]
1035 * [ -1 , -1 ] (max twenty pts)
1036 * NHYDovf=["S-1-OkSovf"]
1037 *%-----|-----|
1038 ADD HYD NHYDsum=["SN_416"], NHYDs to add=["N_416"+"SW_1a"+"S-1-Okeefe"]
1039 *%-----|-----|
1040 SAVE HYD NHYD=["SN_416"], # OF PCYCLES=[-1], ICASEsh=[1]
1041 HYD_COMMENT=["Total Flows at Highway 416"]
1042 *%-----|-----|
1043 *#
1044 *# Hydrograph from Node 416 routed to Node at Okeefe drain
1045 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245

```

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1046 *#
1047 ROUTE CHANNEL      NHYDout=["N_OK"] ,NHYDin=["SN_416"] ,
1048                   RDT=[1](min),
1049                   CHLGTH=[497](m),   CHSLOPE=[0.3018](%),
1050                                     FPSLOPE=[0.3018](%),
1051                   SECNUM=[1.0],      NSEG=[3]
1052                   ( SEGROUGH, SEGDIST (m))=
1053                   [0.075,-19.40
1054                   -0.055,19.40
1055                   0.075,377.02] NSEG times
1056                   ( DISTANCE (m), ELEVATION (m))=
1057                   [-1061.41, 92.50]
1058                   [-945.91, 92.00]
1059                   [-783.64, 91.50]
1060                   [-136.74, 91.00]
1061                   [-86.04, 91.00]
1062                   [-20.86, 91.00]
1063                   [-20.18, 90.50]
1064                   [-19.40, 90.00]
1065                   [-11.68, 86.89]
1066                   [0.00, 86.10]
1067                   [12.09, 86.81]
1068                   [19.40, 90.00]
1069                   [34.68, 90.50]
1070                   [60.56, 91.00]
1071                   [170.14, 91.00]
1072                   [175.05, 90.50]
1073                   [180.29, 90.00]
1074                   [193.41, 90.00]
1075                   [195.98, 90.50]
1076                   [377.02, 92.50]
1077 *%-----|-----|
1078 *#*****|*****|
1079 *#   Catchment OKEEFE
1080 *#   - To O'Keefe drain (north of the Jock)
1081 *#   - Developed with assumed 43% imp.
1082 *#   - 2020-12-01 add Okeefe model (Area 513.02 HA) instead of current Okeefe (Area
1083 *#   - 2020-11-20 Okeefe detailed model was added as per the NOVATECH SWMHYMO model
1084 *#   (Citi-Gate 2014).
1085 *%-----|-----|
1086 *#*****|*****|
1087 *#*****|*****|
1088 CONTINUOUS NASHYD  NHYD=["O-1"], DT=[1]min, AREA=[63.72](ha),
1089                   DWF=[0](cms), CN/C=[61], IA=[6.2](mm), N=[3], TP=[.9]hrs,
1090                   Continuous simulation parameters:
1091                   IaRECPper=[4](hrs),
1092                   SMIN=[-1](mm),   SMAX=[-1](mm), SK=[0.010]/(mm),
1093                   InterEventTime=[12](hrs)
1094                   Baseflow simulation parameters:
1095                   BaseFlowOption=[1] ,
1096                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1097                   VHydCond=[0.055](mm/hr),   END=-1
1098 *%-----|-----|
1099 *%-----|-----|
1100 *ROUTE FLOW THROUGH AREA 0-2
1101 ROUTE CHANNEL      NHYDout=["O-1R"], NHYDin=["O-1"], RDT=[1](min),
1102                   CHLGTH=[960](m), CHSLOPE=[0.63](%), FPSLOPE=[0.63](%),
1103                   SECNUM=[1], NSEG=[3]
1104                   ( SEGROUGH, SEGDIST (m))=[0.06,4 -.043,6 0.06,10] NSEG times
1105                   ( DISTANCE (m), ELEVATION (m))=[0.00, 2.0]
1106                   [0.0, 2.0]
1107                   [4.0, 0.0]
1108                   [6.0, 0.0]
1109                   [10.0, 2.0]

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1109  *%-----|-----
1110  CONTINUOUS NASHYD  NHYD=["O-2"], DT=[1]min, AREA=[28.61](ha),
1111                    DWF=[0](cms), CN/C=[57], IA=[5.2](mm), N=[3], TP=[1.1]hrs,
1112                    Continuous simulation parameters:
1113                    IaREcper=[4](hrs),
1114                    SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1115                    InterEventTime=[12](hrs)
1116                    Baseflow simulation parameters:
1117                    BaseFlowOption=[1] ,
1118                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1119                    VHydCond=[0.055](mm/hr),  END=-1
1120  *%-----|-----
1121  CONTINUOUS NASHYD  NHYD=["O-4"], DT=[1]min, AREA=[46.94](ha),
1122                    DWF=[0](cms), CN/C=[49], IA=[9.2](mm), N=[3], TP=[0.9]hrs,
1123                    Continuous simulation parameters:
1124                    IaREcper=[4](hrs),
1125                    SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1126                    InterEventTime=[12](hrs)
1127                    Baseflow simulation parameters:
1128                    BaseFlowOption=[1] ,
1129                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1130                    VHydCond=[0.055](mm/hr),  END=-1
1131  *%-----|-----
1132  *TOTAL EXTERNAL FLOW NORTH OF O'KEEFE CT. CROSSING
1133  ADD HYD           NHYDsum=["OKF-N"], NHYDs to add=["O-1R"+"O-2"+"O-4"]
1134  *%-----|-----
1135  *ROUTE FLOW THROUGH AREA O-6
1136  ROUTE CHANNEL    ROUTE CHANNEL NHYDout=["OKF-NR"], NHYDin=["OKF-N"], RDT=[1](min),
1137                    CHLGTH=[210](m), CHSLOPE=[.81](%), FPSLOPE=[.81](%),
1138                    SECNUM=[1], NSEG=[3]
1139                    ( SEGROUGH, SEGDIST (m))=[0.043,22.43 -0.043,25.07
1140                    0.043,45.54] NSEG times
1141                    ( DISTANCE (m), ELEVATION (m))=[0.00, 3.73]
1142                    (14.62, 1.56)
1143                    (18.41, 1.44)
1144                    (22.43, 0.00)
1145                    (25.07, 0.70)
1146                    (29.10, 1.79)
1147                    (33.73, 2.71)
1148                    (45.54, 3.58)
1149  *%-----|-----
1150  CONTINUOUS NASHYD  NHYD=["O-6"], DT=[1]min, AREA=[16.46](ha),
1151                    DWF=[0](cms), CN/C=[43], IA=[9.2](mm), N=[3], TP=[0.7]hrs,
1152                    Continuous simulation parameters:
1153                    IaREcper=[4](hrs),
1154                    SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1155                    InterEventTime=[12](hrs)
1156                    Baseflow simulation parameters:
1157                    BaseFlowOption=[1] ,
1158                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1159                    VHydCond=[0.055](mm/hr),  END=-1
1160  *%-----|-----
1161  CONTINUOUS STANDHYD NHYD=["O-3"], DT=[1](min), AREA=[39.67](ha), XIMP=[0.15],
1162  TIMP=[0.30], DWF=[0](cms),
1163                    LOSS=[2], SCS curve number CN=[50], Pervious surfaces:
1164                    IAper=[4.67](mm), SLPP=[0.32](%),
1165                    LGP=[440](m), MNP=[0.035], SCP=[0](min), Impervious surfaces:
1166                    IAimp=[1.57](mm), SLPI=[0.32](%),
1167                    LGI=[1880](m), MNI=[0.013], SCI=[0](min),
1168                    Continuous simulation parameters:
1169                    IaREcper=[4](hrs), IaREcimp=[4](hrs),
1170                    SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1171                    InterEventTime=[12](hrs),  END=-1
1172  *%-----|-----
1173  CONTINUOUS STANDHYD NHYD=["O-5"], DT=[1](min), AREA=[60.63](ha), XIMP=[0.13],
1174  TIMP=[0.26], DWF=[0](cms),

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1171      LOSS=[2], SCS curve number CN=[61],
1172      Pervious surfaces: IAPER=[4.67](mm), SLPP=[1.38](%),
1173      LGP=[550](m), MNP=[0.035], SCP=[0](min), Impervious surfaces:
1174      IAimp=[1.57](mm), SLPI=[1.38](%),
1175      LGI=[1450](m), MNI=[0.013], SCI=[0](min),
1176      Continuous simulation parameters:
1177      IaREcper=[4](hrs), IaREcimp=[4](hrs),
1178      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1179      InterEventTime=[12](hrs), END=-1
1180 *%-----|-----|
1181 *TOTAL EXTERNAL FLOWS WEST OF THE SITE AND NORTH OF O'KEEFE CRT
1182 *%-----|-----|
1183 ADD HYD      NHYDsum=["PT1"], NHYDs to add=["OKF-NR"+"O-3"+"O-5"+"O-6"]
1184 *%-----|-----|
1185 CONTINUOUS NASHYD      NHYD=["O-7"], DT=[1]min, AREA=[5.28](ha),
1186      DWF=[0](cms), CN/C=[54], IA=[7.5](mm), N=[3], TP=[0.6]hrs,
1187      Continuous simulation parameters:
1188      IaREcper=[4](hrs),
1189      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1190      InterEventTime=[12](hrs)
1191      Baseflow simulation parameters:
1192      BaseFlowOption=[1] ,
1193      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1194      VHydCond=[0.055](mm/hr), END=-1
1195 *%-----|-----|
1196 *ANALYSIS POINT 1 - TOTAL FLOW NORTH OF FALLOWFIELD DR. AND O'KEEFE CRT.
1197 ADD HYD      NHYDsum=["FF"], NHYDs to add=["PT1"+"O-7"]
1198 *%-----|-----|
1199 *ROUTE FLOW through O'Keefe Drain 1
1200 ROUTE CHANNEL      NHYDout=["DRAIN1"], NHYDin=["FF"], RDT=[1](min),
1201      CHLGTH=[302]{m}, CHSLOPE=[1.00](%), FPSLOPE=[1.00](%),
1202      SECNUM=[1], NSEG=[3]
1203      ( SEGROUGH, SEGDIST (m))=[0.07,13.45 -0.043,16.55 0.07,30.00] NSEG
1204      times
1205      ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1206      (3.45, 0.60)
1207      (13.45, 0.50)
1208      (14.45, 0.00)
1209      (15.55, 0.00)
1210      (16.55, 0.50)
1211      (26.55, 0.60)
1212      (30.00, 1.70)
1213 *%-----|-----|
1214 CONTINUOUS NASHYD      NHYD=["D1"], DT=[1]min, AREA=[1.17](ha),
1215      DWF=[0](cms), CN/C=[84], IA=[9.0](mm), N=[3], TP=[0.28]hrs,
1216      Continuous simulation parameters:
1217      IaREcper=[4](hrs),
1218      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1219      InterEventTime=[12](hrs)
1220      Baseflow simulation parameters:
1221      BaseFlowOption=[1] ,
1222      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1223      VHydCond=[0.055](mm/hr), END=-1
1224 *%-----|-----|
1225 CONTINUOUS STANDHYD      NHYD=["A1"], DT=[1]min, AREA=[2.50](ha), XIMP=[0.68], TIMP=[0.85],
1226      DWF=[0](cms), LOSS=[1]:
1227      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1228      F=[0.00](mm),
1229      Pervious areas: IAPER=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1230      MNP=[0.250], SCP=[0](min),
1231      Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1232      LGI=[223.607](m), MNI=[0.013], SCI=[0](min),
1233      Continuous simulation parameters:
1234      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1235      END=-1
1236 *%-----|-----|

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1230 ROUTE RESERVOIR      NHYDout=["A1-STR"], NHYDin=["A1"], RDT=[1](min),
1231                      TABLE of ( OUTFLOW-STORAGE ) values
1232                      (cms) - (ha-m)
1233                      [ 0.000 , 0.000 ]
1234                      [ 0.035 , 0.038 ]
1235                      [ 0.072 , 0.051 ]
1236                      [ 0.100 , 0.059 ]
1237                      [ 0.125 , 0.070 ]
1238                      [ 0.160 , 0.074 ]
1239                      [ 0.185 , 0.081 ]
1240                      [ -1 , -1 ] (max twenty pts)
1241                      NHYDovf=["A1-OVF"]
1242 *%-----|-----|
1243 CONTINUOUS STANDHYD NHYD=["ST-2"], DT=[1]min, AREA=[0.59](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1244 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1245 Pervious areas: IAPER=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1246 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[108.628](m), MNI=[0.013], SCI=[0](min),
1247 Continuous simulation parameters:
1248 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1249 *%-----|-----|
1250 ROUTE RESERVOIR      NHYDout=["ST2STR"], NHYDin=["ST-2"], RDT=[1](min),
1251                      TABLE of ( OUTFLOW-STORAGE ) values
1252                      (cms) - (ha-m)
1253                      [ 0.000 , 0.0000 ]
1254                      [ 0.052 , 0.0010 ]
1255                      [ 0.053 , 0.0080 ]
1256                      [ -1 , -1 ] (max twenty pts)
1257                      NHYDovf=["ST2OVF"]
1258 *%-----|-----|
1259 *%-----|-----|
1260 *TOTAL FLOW NORTH OF STRANDHERD DR. (EAST BRANCH) CROSSING
1261 *%-----|-----|
1262 CONTINUOUS NASHYD    NHYD=["O-8"], DT=[1]min, AREA=[60.55](ha),
1263 DWF=[0](cms), CN/C=[69], IA=[4.0](mm), N=[3], TP=[1.0]hrs,
1264 Continuous simulation parameters:
1265 IaRECper=[4](hrs),
1266 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1267 InterEventTime=[12](hrs)
1268 Baseflow simulation parameters:
1269 BaseFlowOption=[1] ,
1270 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1271 VHydCond=[0.055](mm/hr), END=-1
1272 *%-----|-----|
1273 ROUTE PIPE           PTYPE=[2]rect, NHYDout=["O8PIPE"], RNUMBER=[1], PWIDTH=[1800](mm),
PHEIGHT=[1200](mm), PLNGTH=[335.1](m),
1274 PROUGH=[0.013], PSLOPE=[0.001](m/m), NHYDin=["O-8"], RDT=[1](min)
1275 *%-----|-----|
1276 *%-----|-----|
1277 ADD HYD              NHYDsum=["ST2-IN"], NHYDs to
add=["DRAIN1"+"D1"+"A1-STR"+"A1-OVF"+"ST2STR"+"ST2OVF"+"O8PIPE"]
1278 *%-----|-----|
1279 CONTINUOUS STANDHYD NHYD=["A7"], DT=[1]min, AREA=[3.51](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1280 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1281 Pervious areas: IAPER=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1282 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[264.953](m), MNI=[0.013], SCI=[0](min),
1283 Continuous simulation parameters:
1284 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),

```

END=-1

```

1285 *%-----|-----|
1286 ROUTE RESERVOIR NHYDout=["A7-STR"], NHYDin=["A7"], RDT=[1](min),
1287             TABLE of ( OUTFLOW-STORAGE ) values
1288             (cms) - (ha-m)
1289             [ 0.000 , 0.000 ]
1290             [ 0.049 , 0.054 ]
1291             [ 0.102 , 0.072 ]
1292             [ 0.140 , 0.082 ]
1293             [ 0.175 , 0.099 ]
1294             [ 0.225 , 0.105 ]
1295             [ 0.260 , 0.114 ]
1296             [ -1 , -1 ] (max twenty pts)
1297             NHYDovf=["A7-OVF"]

```

```

1298 *%-----|-----|
1299 CONTINUOUS STANDHYD NHYD=["ST-3"], DT=[1]min, AREA=[0.71](ha), XIMP=[0.46],
1300 TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1301 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1302 F=[0.00](mm),
1303 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1304 MNP=[0.250], SCP=[0](min),
1305 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1306 LGI=[119.164](m), MNI=[0.013], SCI=[0](min),
1307 Continuous simulation parameters:
1308 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1309 END=-1

```

```

1305 *%-----|-----|
1306 ROUTE RESERVOIR NHYDout=["ST3STR"], NHYDin=["ST-3"], RDT=[1](min),
1307             TABLE of ( OUTFLOW-STORAGE ) values
1308             (cms) - (ha-m)
1309             [ 0.000 , 0.0000 ]
1310             [ 0.063 , 0.0010 ]
1311             [ 0.064 , 0.0094 ]
1312             [ -1 , -1 ] (max twenty pts)
1313             NHYDovf=["ST3OVF"]

```

```

1314 *%-----|-----|
1315 *ANALYSIS POINT 2 - TOTAL FLOW AT OUTLET OF STREET 2/3 INTERSECTION
1316 *%-----|-----|

```

```

1317 ADD HYD NHYDsum=["PT2ST3"], NHYDs to
1318 add=["ST2-IN"+"A7-STR"+"A7-OVF"+"ST3STR"+"ST3OVF"]

```

```

1319 *ROUTE FLOW through O'Keefe Drain 2

```

```

1320 ROUTE CHANNEL NHYDout=["DRAIN2"], NHYDin=["PT2ST3"], RDT=[1](min),
1321             CHLGTH=[592]{m}, CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1322             SECNUM=[1], NSEG=[3]
1323             ( SEGROUGH, SEGDIST (m))=[0.07,12.60 -0.043,17.40 0.07,30.00] NSEG
1324             times
1325             ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1326             (2.60, 0.95)
1327             (12.60, 0.75)
1328             (14.10, 0.00)
1329             (15.90, 0.00)
1330             (17.40, 0.75)
1331             (27.40, 0.95)
1332             (30.00, 1.70)

```

```

1333 *%-----|-----|
1334 CONTINUOUS NASHYD NHYD=["D2"], DT=[1]min, AREA=[2.28](ha), DWF=[0](cms), CN/C=[84],
1335 IA=[9.0](mm),
1336 N=[3], TP=[0.99]hrs,
1337 Continuous simulation parameters:
1338 IaREcper=[4](hrs),
1339 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1340 InterEventTime=[12](hrs)
1341 Baseflow simulation parameters:
1342 BaseFlowOption=[1] ,
1343 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)

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1342          VHydCond=[0.055](mm/hr),   END=-1
1343  *%-----|-----|
1344  CONTINUOUS STANDHYD NHYD=["A17"], DT=[1]min, AREA=[12.04](ha), XIMP=[0.68],
1345  TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
1346  Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1347          F=[0.00](mm),
1348  Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1349  MNP=[0.250], SCP=[0](min),
1350  Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1351  LGI=[490.714](m), MNI=[0.013], SCI=[0](min),
1352  Continuous simulation parameters:
1353  IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1354  END=-1
1355  *%-----|-----|
1356  ROUTE RESERVOIR NHYDout=["A17STR"], NHYDin=["A17"], RDT=[1](min),
1357          TABLE of ( OUTFLOW-STORAGE ) values
1358          (cms) - (ha-m)
1359          [ 0.000 , 0.000 ]
1360          [ 0.169 , 0.185 ]
1361          [ 0.349 , 0.248 ]
1362          [ 0.482 , 0.283 ]
1363          [ 0.602 , 0.338 ]
1364          [ 0.771 , 0.359 ]
1365          [ 0.891 , 0.391 ]
1366          [ -1 , -1 ] (max twenty pts)
1367  NHYDovf=["A17OVF"]
1368  *%-----|-----|
1369  CONTINUOUS STANDHYD NHYD=["ST-4"], DT=[1]min, AREA=[0.35](ha), XIMP=[0.46],
1370  TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1371  Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1372          F=[0.00](mm),
1373  Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1374  MNP=[0.250], SCP=[0](min),
1375  Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[83.666](m),
1376  MNI=[0.013], SCI=[0](min),
1377  Continuous simulation parameters:
1378  IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1379  END=-1
1380  *%-----|-----|
1381  ROUTE RESERVOIR NHYDout=["ST4STR"], NHYDin=["ST-4"], RDT=[1](min),
1382          TABLE of ( OUTFLOW-STORAGE ) values
1383          (cms) - (ha-m)
1384          [ 0.000 , 0.0000 ]
1385          [ 0.031 , 0.0010 ]
1386          [ 0.032 , 0.0050 ]
1387          [ -1 , -1 ] (max twenty pts)
1388  NHYDovf=["ST4OVF"]
1389  *%-----|-----|
1390  CONTINUOUS STANDHYD NHYD=["A18"], DT=[1]min, AREA=[5.30](ha), XIMP=[0.68], TIMP=[0.85],
1391  DWF=[0](cms), LOSS=[1]:
1392  Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1393          F=[0.00](mm),
1394  Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1395  MNP=[0.250], SCP=[0](min),
1396  Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1397  LGI=[325.576](m), MNI=[0.013], SCI=[0](min),
1398  Continuous simulation parameters:
1399  IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1400  END=-1
1401  *%-----|-----|
1402  ROUTE RESERVOIR NHYDout=["A18STR"], NHYDin=["A18"], RDT=[1](min),
1403          TABLE of ( OUTFLOW-STORAGE ) values
1404          (cms) - (ha-m)
1405          [ 0.000 , 0.000 ]
1406          [ 0.074 , 0.082 ]
1407          [ 0.154 , 0.109 ]

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1393         [ 0.212 , 0.125 ]
1394         [ 0.265 , 0.149 ]
1395         [ 0.339 , 0.158 ]
1396         [ 0.392 , 0.172 ]
1397         [ -1 , -1 ] (max twenty pts)
1398         NHYDovf=["A18OVF"]
1399 *%-----|-----|
1400 *ANALYSIS POINT 3 - TOTAL FLOW AT OUTLET OF STREET 4
1401 *%-----|-----|
1402 ADD HYD          NHYDsum=["PT3ST4"], NHYDs to
add=["DRAIN2"+"D2"+"A17STR"+"A17OVF"+"ST4STR"+"ST4OVF"+"A18STR"+"A18OVF"]
1403 *%-----|-----|
1404 *ROUTE FLOW through O'Keefe Drain 3
1405 ROUTE CHANNEL   NHYDout=["DRAIN3"], NHYDin=["PT3ST4"], RDT=[1](min),
1406                 CHLGTH=[525]{m}, CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1407                 SECNUM=[1], NSEG=[3]
1408                 ( SEGROUGH, SEGDIST (m))=[0.07,12.50 -0.043,17.50 0.07,30.00] NSEG
times
1409                 ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1410                 (2.50, 1.00)
1411                 (12.50, 0.80)
1412                 (14.10, 0.00)
1413                 (15.90, 0.00)
1414                 (17.50, 0.80)
1415                 (27.50, 1.00)
1416                 (30.00, 1.70)
1417 *%-----|-----|
1418 CONTINUOUS NASHYD NHYD=["D3"], DT=[1]min, AREA=[2.51](ha),
1419                 DWF=[0](cms), CN/C=[86], IA=[8.7](mm), N=[3], TP=[0.73]hrs,
1420                 Continuous simulation parameters:
1421                 IaRECper=[4](hrs),
1422                 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1423                 InterEventTime=[12](hrs)
1424                 Baseflow simulation parameters:
1425                 BaseFlowOption=[1] ,
1426                 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1427                 VHydCond=[0.055](mm/hr), END=-1
1428 *%-----|-----|
1429 CONTINUOUS STANDHYD NHYD=["C1"], DT=[1]min, AREA=[3.41](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1430                 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1431                 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1432                 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[261.151](m), MNI=[0.013], SCI=[0](min),
1433                 Continuous simulation parameters:
1434                 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1435 *%-----|-----|
1436 ROUTE RESERVOIR NHYDout=["C1-STR"], NHYDin=["C1"], RDT=[1](min),
1437                 TABLE of ( OUTFLOW-STORAGE ) values
1438                 (cms) - (ha-m)
1439                 [ 0.000 , 0.000 ]
1440                 [ 0.048 , 0.052 ]
1441                 [ 0.099 , 0.070 ]
1442                 [ 0.136 , 0.080 ]
1443                 [ 0.170 , 0.096 ]
1444                 [ 0.218 , 0.102 ]
1445                 [ 0.252 , 0.111 ]
1446                 [ -1 , -1 ] (max twenty pts)
1447                 NHYDovf=["C1-OVF"]
1448 *%-----|-----|
1449 CONTINUOUS STANDHYD NHYD=["ST-5"], DT=[1]min, AREA=[0.45](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1450                 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),

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1451           F=[0.00](mm),
           Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1452           MNP=[0.250], SCP=[0](min),
           Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[94.868](m),
1453           MNI=[0.013], SCI=[0](min),
           Continuous simulation parameters:
1454           IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
           END=-1
1455 *%-----|-----|
1456 ROUTE RESERVOIR NHYDout=["ST5STR"], NHYDin=["ST-5"], RDT=[1](min),
1457           TABLE of ( OUTFLOW-STORAGE ) values
1458           (cms) - (ha-m)
1459           [ 0.000 , 0.0000 ]
1460           [ 0.040 , 0.0010 ]
1461           [ 0.041 , 0.0062 ]
1462           [ -1 , -1 ] (max twenty pts)
1463           NHYDovf=["ST5OVF"]
1464 *%-----|-----|
1465 ADD HYD NHYDsum=["ST5-E"], NHYDs to
add=["DRAIN3"+"D3"+"C1-STR"+"C1-OVF"+"ST5STR"+"ST5OVF"]
1466 *%-----|-----|
1467 CONTINUOUS STANDHYD NHYD=["STRAND"], DT=[1](min), AREA=[7.59](ha),
1468           XIMP=[0.64], TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
1469           Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
           F=[0.00](mm),
1470           Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
           MNP=[0.250], SCP=[0](min),
1471           Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[1230](m),
           MNI=[0.013], SCI=[0](min),
1472           Continuous simulation parameters:
1473           IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
           END=-1
1474 *%-----|-----|
1475 ROUTE RESERVOIR NHYDout=["S-POND"], NHYDin=["STRAND"], RDT=[1](min),
1476           TABLE of ( OUTFLOW-STORAGE ) values
1477           (cms) - (ha-m)
1478           [ 0.000 , 0.000 ]
1479           [ 0.033 , 0.188 ]
1480           [ 0.057 , 0.253 ]
1481           [ 0.104 , 0.287 ]
1482           [ 0.160 , 0.336 ]
1483           [ 0.340 , 0.346 ]
1484           [ 0.471 , 0.360 ]
1485           [ 0.824 , 0.390 ]
1486           [ -1 , -1 ] (max twenty pts)
1487           NHYDovf=["S-OVF"]
1488 *%-----|-----|
1489 ADD HYD NHYDsum=["SSAOUT"], NHYDs to add=["ST5-E"+"S-POND"+"S-OVF"]
1490 *%-----|-----|
1491 SAVE HYD NHYD=["SSAOUT"], # OF PCYCLES=[5], ICASEsh=[1]
1492           HYD_COMMENT=["SSAOUT"]
1493 *%-----|-----|
1494 CONTINUOUS STANDHYD NHYD=["Area-A"], DT=[1]min, AREA=[66.75](ha), XIMP=[0.64],
1495           TIMP=[0.80], DWF=[0](cms), LOSS=[1]:
           Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
           F=[0.00](mm),
1496           Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
           MNP=[0.250], SCP=[0](min),
1497           Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
           LGI=[1155.422](m), MNI=[0.013], SCI=[0](min),
1498           Continuous simulation parameters:
1499           IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
           END=-1
1500 *%-----|-----|
1501 SAVE HYD NHYD=["Area-A"], # OF PCYCLES=[1], ICASEsh=[1]
1502           HYD_COMMENT=["SMWF-A Inflow"]

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1503 *%-----|-----|
1504 ROUTE RESERVOIR NHYDout=["SWMF-A"], NHYDin=["Area-A"], RDT=[1](min),
1505 TABLE of ( OUTFLOW-STORAGE ) values
1506 (cms) - (ha-m)
1507 [ 0.000 , 0.000 ]
1508 [ 0.103 , 1.077 ]
1509 [ 0.128 , 1.749 ]
1510 [ 0.382 , 2.282 ]
1511 [ 0.703 , 2.582 ]
1512 [ 1.256 , 2.978 ]
1513 [ 1.567 , 3.202 ]
1514 [ 1.955 , 3.493 ]
1515 [ 2.100 , 3.600 ]
1516 [ -1 , -1 ] (max twenty pts)
1517 NHYDovf=["SWMAOV"]
1518 *%-----|-----|
1519 SAVE HYD NHYD=["SWMF-A"], # OF PCYCLES=[1], ICASEsh=[1]
1520 HYD_COMMENT=["SMWF-A Outflow"]
1521 *%-----|-----|
1522 *ANALYSIS POINT 4 - TOTAL FLOW AT OUTLET OF STREET 5
1523 *%-----|-----|
1524 ADD HYD NHYDsum=["PT4ST5"], NHYDs to add=["SSAOUT"+"SWMF-A"+"SWMAOV"]
1525 *%-----|-----|
1526 CONTINUOUS STANDHYD NHYD=["C6"], DT=[1]min, AREA=[1.87](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1527 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1528 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1529 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[193.391](m), MNI=[0.013], SCI=[0](min),
1530 Continuous simulation parameters:
1531 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1532 *%-----|-----|
1533 ROUTE RESERVOIR NHYDout=["C6-STR"], NHYDin=["C6"], RDT=[1](min),
1534 TABLE of ( OUTFLOW-STORAGE ) values
1535 (cms) - (ha-m)
1536 [ 0.000 , 0.000 ]
1537 [ 0.026 , 0.029 ]
1538 [ 0.054 , 0.038 ]
1539 [ 0.075 , 0.044 ]
1540 [ 0.093 , 0.052 ]
1541 [ 0.120 , 0.056 ]
1542 [ 0.138 , 0.061 ]
1543 [ -1 , -1 ] (max twenty pts)
1544 NHYDovf=["C6-OVF"]
1545 *%-----|-----|
1546 CONTINUOUS STANDHYD NHYD=["C7"], DT=[1]min, AREA=[1.62](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1547 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1548 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1549 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
1550 Continuous simulation parameters:
1551 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1552 *%-----|-----|
1553 ROUTE RESERVOIR NHYDout=["C7-STR"], NHYDin=["C7"], RDT=[1](min),
1554 TABLE of ( OUTFLOW-STORAGE ) values
1555 (cms) - (ha-m)
1556 [ 0.000 , 0.000 ]
1557 [ 0.023 , 0.025 ]
1558 [ 0.047 , 0.033 ]

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1559         [ 0.065 , 0.038 ]
1560         [ 0.081 , 0.045 ]
1561         [ 0.104 , 0.048 ]
1562         [ 0.120 , 0.053 ]
1563         [ -1 , -1 ] (max twenty pts)
1564         NHYDovf=["C7-OVF"]
1565 *%-----|-----|
1566 CONTINUOUS STANDHYD NHYD=["ST-6"], DT=[1]min, AREA=[0.41](ha),XIMP=[0.46], TIMP=[0.57],
DWF=[0](cms), LOSS=[1]:
1567         Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1568         Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1569         Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[90.554](m),
MNI=[0.013], SCI=[0](min),
1570         Continuous simulation parameters:
1571         IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1572 *%-----|-----|
1573 ROUTE RESERVOIR NHYDout=["ST6STR"], NHYDin=["ST-6"], RDT=[1](min),
1574         TABLE of ( OUTFLOW-STORAGE ) values
1575         (cms) - (ha-m)
1576         [ 0.000 , 0.0000 ]
1577         [ 0.036 , 0.0010 ]
1578         [ 0.037 , 0.0058 ]
1579         [ -1 , -1 ] (max twenty pts)
1580         NHYDovf=["ST6OVF"]
1581 *%-----|-----|
1582 *ANALYSIS POINT 5 - TOTAL FLOW AT OUTLET OF STREET 6
1583 *%-----|-----|
1584 ADD HYD NHYDsum=["PT5ST6"], NHYDs to
add=["PT4ST5"+"C6-STR"+"C6-OVF"+"C7-STR"+"C7-OVF"+"ST6STR"+"ST6OVF"]
1585 *%-----|-----|
1586 *ROUTE FLOW through O'Keefe Drain 4
1587 ROUTE CHANNEL NHYDout=["DRAIN4"], NHYDin=["PT5ST6"], RDT=[1](min),
1588         CHLGTH=[324]{m}, CHSLOPE=[.10](%), FPSLOPE=[.10](%),
1589         SECNUM=[1], NSEG=[3]
1590         ( SEGROUGH, SEGDIST (m))=[0.07,12.00 -0.043,18.00 0.07,30.00] NSEG
times
1591         ( DISTANCE (m), ELEVATION (m))=[0.00, 2.00]
1592         (2.00, 1.20)
1593         (12.00, 1.00)
1594         (14.00, 0.00)
1595         (16.00, 0.00)
1596         (18.00, 1.00)
1597         (28.00, 1.20)
1598         (30.00, 2.00)
1599 *%-----|-----|
1600 CONTINUOUS NASHYD NHYD=["D4"], DT=[1]min, AREA=[1.73](ha), DWF=[0](cms), CN/C=[88],
IA=[8.4](mm),
1601         N=[3], TP=[0.60]hrs,
1602         Continuous simulation parameters:
1603         IaRECper=[4](hrs),
1604         SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1605         InterEventTime=[12](hrs)
1606         Baseflow simulation parameters:
1607         BaseFlowOption=[1] ,
1608         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1609         VHydCond=[0.055](mm/hr), END=-1
1610 *%-----|-----|
1611 CONTINUOUS STANDHYD NHYD=["Area-B"], DT=[1]min, AREA=[24.04](ha), XIMP=[0.62],
TIMP=[0.77], DWF=[0](cms), LOSS=[1]:
1612         Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1613         Pervious areas: IAper=[4.67](mm), SLPP=[1.4](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),

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1614      Impervious areas: IAimp=[1.57](mm), SLPI=[1.4](%),
1615      LGI=[693.397](m), MNI=[0.013], SCI=[0](min),
1616      Continuous simulation parameters:
1617      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1618      END=-1
1619  *%-----|-----|
1620  ROUTE RESERVOIR      NHYDout=["SWMF-B"], NHYDin=["Area-B"], RDT=[1](min),
1621      TABLE of ( OUTFLOW-STORAGE ) values
1622      (cms) - (ha-m)
1623      [ 0.000 , 0.000 ]
1624      [ 0.025 , 0.090 ]
1625      [ 0.175 , 0.510 ]
1626      [ 0.350 , 0.710 ]
1627      [ 0.495 , 0.820 ]
1628      [ 0.648 , 0.980 ]
1629      [ 0.965 , 1.045 ]
1630      [ 1.072 , 1.140 ]
1631      [ -1 , -1 ] (max twenty pts)
1632      NHYDovf=["SWMBOVF"]
1633  *%-----|-----|
1634  ADD HYD              NHYDsum=["D4-EX"], NHYDs to add=["DRAIN4"+"D4"+"SWMF-B"+"SWMBOVF"]
1635  *%-----|-----|
1636  *ROUTE FLOW THROUGH O'Keefe Drain 5
1637  * JFSA: Nov. 2020, added en points to close X-Section
1638  ROUTE CHANNEL      NHYDout=["DRAIN5"], NHYDin=["D4-EX"], RDT=[1](min),
1639      CHLGTH=[413.0](m), CHSLOPE=[0.16](%), FPSLOPE=[0.16](%),
1640      SECNUM=[1], NSEG=[3]
1641      ( SEGROUGH, SEGDIST (m))=[0.043,12.29 -0.033,17.97
1642      0.043,32.84] NSEG times
1643      ( DISTANCE (m), ELEVATION (m))=(-0.01, 2.50)
1644      [0.00, 1.41]
1645      [6.13, 0.97]
1646      [12.29, 0.89]
1647      [15.71, 0.00]
1648      [17.97, 0.39]
1649      [23.04, 0.35]
1650      [32.83, 0.96]
1651      (32.84, 2.50)
1652  *%-----|-----|
1653  CONTINUOUS NASHYD   NHYD=["D5"], DT=[1]min, AREA=[1.90](ha),
1654      DWF=[0](cms), CN/C=[86], IA=[8.7](mm), N=[3], TP=[0.69]hrs,
1655      Continuous simulation parameters:
1656      IaREcper=[4](hrs),
1657      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1658      InterEventTime=[12](hrs)
1659      Baseflow simulation parameters:
1660      BaseFlowOption=[1] ,
1661      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1662      VHydCond=[0.055](mm/hr), END=-1
1663  *%-----|-----|
1664  *EXTERNAL FLOWS SOUTHEAST OF THE SITE NORTH OF McKENNA CASEY DR.
1665  CONTINUOUS NASHYD   NHYD=["O-13SDF"], DT=[1]min, AREA=[9.74](ha),
1666      DWF=[0](cms), CN/C=[81], IA=[4.0](mm), N=[3], TP=[.43]hrs,
1667      Continuous simulation parameters:
1668      IaREcper=[4](hrs),
1669      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1670      InterEventTime=[12](hrs)
1671      Baseflow simulation parameters:
1672      BaseFlowOption=[1] ,
1673      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1674      VHydCond=[0.055](mm/hr), END=-1
1675  *%-----|-----|
1676  *SNOW DISPOSAL FACILITY
1677  *PARAMETERS BASED ON ROBINSON 2006 MODEL
1678  ROUTE RESERVOIR      NHYDout=["SDF"], NHYDin=["O-13SDF"], RDT=[1](min),
1679      TABLE of ( OUTFLOW-STORAGE ) values

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1678             (cms) - (ha-m)
1679             [0.000,0.000]
1680             [0.150,0.600]
1681             (0.200,1.500)
1682             [ -1 , -1 ] (max twenty pts)
1683             NHYDovf=["OVFSDF"]
1684 *%-----|-----|
1685 *ANALYSIS POINT 6 - McKenna Casey Dr.
1686 *%-----|-----|
1687 ADD HYD      NHYDsum=["PT6MC"], NHYDs to add=["DRAIN5"+"D5"+"SDF"]
1688 *%-----|-----|
1689 CONTINUOUS NASHYD  NHYD=["O-15"], DT=[1]min, AREA=[10.67](ha),
1690                   DWF=[0](cms), CN/C=[82], IA=[7.5](mm), N=[3], TP=[0.30]hrs,
1691                   Continuous simulation parameters:
1692                   IaRECper=[4](hrs),
1693                   SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1694                   InterEventTime=[12](hrs)
1695                   Baseflow simulation parameters:
1696                   BaseFlowOption=[1] ,
1697                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1698                   VHydCond=[0.055](mm/hr),  END=-1
1699 *%-----|-----|
1700 *TOTAL FLOW NORTH OF McKENNA CASEY DR.
1701 ADD HYD      NHYDsum=["M-C"], NHYDs to add=["PT6MC"+"O-15"]
1702 *%-----|-----|
1703 *ROUTE FLOW THROUGH AREA O-14
1704 * JFSA: Nov. 2020, added end points to close X-section
1705 ROUTE CHANNEL  NHYDout=["O-14Ch"], NHYDin=["M-C"], RDT=[1](min),
1706                   CHLGTH=[845.3](m), CHSLOPE=[0.10](%), FPSLOPE=[0.10](%),
1707                   SECNUM=[1], NSEG=[3]
1708                   ( SEGROUGH, SEGDIST (m))=[0.06,15.00 -0.033,18.04 0.06,31.85] NSEG
1709                   times
1710                   ( DISTANCE (m), ELEVATION (m))=[-0.01, 2.5
1711                   (0.00, 1.53]
1712                   (5.56, 1.47)
1713                   (9.21, 1.45)
1714                   (12.45, 1.53)
1715                   (13.70, 1.50)
1716                   (15.00, 0.69)
1717                   (15.34, 0.00)
1718                   (16.51, 0.05)
1719                   (17.30, 0.17)
1720                   (18.04, 0.74)
1721                   (19.29, 1.32)
1722                   (22.73, 1.47)
1723                   (31.84, 1.41)
1724                   (31.85, 2.50)
1725 *%-----|-----|
1726 *% -Change O-14 from NASHYD to STANDHYD, name it "S-1-Okeefe" and add it to S-1
1727 *% -JFSA 2021-02-16, add detailed subcatchment drainage area for each subcatchment
1728 *% -in Corrigan sub-catchment. After adding part of O-14 to S_1 sub-catchment so O-14
1729 *% -becomes 5 ha instead of 30.02 ha and TP becomes 0.133 (5*0.8/30.02) instead of 0.8
1730 CONTINUOUS NASHYD  NHYD=["O-14"], DT=[1]min, AREA=[5](ha),
1731                   DWF=[0](cms), CN/C=[82], IA=[7.5](mm), N=[3], TP=[0.133]hrs,
1732                   Continuous simulation parameters:
1733                   IaRECper=[4](hrs),
1734                   SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1735                   InterEventTime=[12](hrs)
1736                   Baseflow simulation parameters:
1737                   BaseFlowOption=[1] ,
1738                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1739                   VHydCond=[0.055](mm/hr),  END=-1
1740 *
1741 *%-----|-----|
1742 *ANALYSIS POINT 7 - JOCK RIVER

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1740 * 2020-12-01 To Foster Drain
1741 * 2020-12-01 replace ("PT7JR") by ("OKEEFE")
1742 *%-----|-----
1743 ADD HYD          NHYDsum=["OKEEFE"], NHYDs to add=["O-14Ch"+"O-14"]
1744 *%-----|-----
1745 *CONTINUOUS STANDHYD NHYD=["OKEEFE"], DT=[1](min), AREA=[448](ha),
1746 *                XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
1747 *                SCS curve number CN=[77],
1748 *                Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1749 *                LGP=[40](m), MNP=[0.25], SCP=[0](min),
1750 *                Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1751 *                LGI=[1728](m), MNI=[0.013], SCI=[0](min),
1752 *                Continuous simulation parameters:
1753 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
1754 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1755 *                InterEventTime=[18](hrs), END=-1
1756 *#*****
1757 *#      Okeefe Pond
1758 *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1759 *#      and a ratio of the catchment area to the West Clarke pond rating curve
1760 *#      from the MSS for the next coordinates
1761 *#*****
1762 *ROUTE RESERVOIR  NHYDout=["P_OKE"], NHYDin=["OKEEFE"],
1763 *                RDT=[1](min),
1764 *                TABLE of ( OUTFLOW-STORAGE ) values
1765 *                (cms) - (ha-m)
1766 *                [  0.0 ,  0.0]
1767 *                [ 14.13 , 13.0]
1768 *                [  -1 ,  -1 ] (maximum one hundred pairs of points)
1769 *                NHYDovf=["ok-OVF"],
1770 *%-----|-----
1771 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
1772 * moved to drain before station 6215 on Jock River
1773 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XIMP=[0.65],
1774 *                TIMP=[0.65], DWF=[0](cms),
1775 *                LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1776 *                IAper=[4.67](mm), SLPP=[2.0](%),
1777 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1778 *                IAimp=[1.57](mm), SLPI=[0.75](%),
1779 *                LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
1780 *                Continuous simulation parameters:
1781 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
1782 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1783 *                InterEventTime=[12](hrs), END=-1
1784 *%-----|-----
1785 *CONTINUOUS NASHYD  NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
1786 *                DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1787 *                N=[3], TP=[1.120]hrs,
1788 *                Continuous simulation parameters:
1789 *                IaRECper=[4](hrs),
1790 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1791 *                InterEventTime=[12](hrs)
1792 *                Baseflow simulation parameters:
1793 *                BaseFlowOption=[1] ,
1794 *                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1795 *                VHydCond=[0.055](mm/hr), END=-1
1796 *%-----|-----
1797 *COMPUTE DUALHYD   NHYDin=["S-1-D2"], CINLET=[2.062](cms), NINLET=[1],
1798 *                MajNHYD=["S-1-D2J"]
1799 *                MinNHYD=["S-1-D2N"]
1800 *                TMJSTO=[9999999](cu-m)
1801 *%-----|-----
1802 *ADD HYD           NHYDsum=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
1803 *%-----|-----
1804 *ROUTE RESERVOIR  NHYDout=["S-1-D2R"] ,NHYDin=["S-1-D2S"] ,

```

```

1801 *          RDT=[1](min),
1802 *          TABLE of ( OUTFLOW-STORAGE ) values
1803 *          (cms) - (ha-m)
1804 *          [ 0.0      , 0.0 ]
1805 *          [ 0.2231, 0.7445 ]
1806 *          [   -1   ,  -1   ] (max twenty pts)
1807 *          NHYDovf=["S-1-D2Rovf"]
1808 *%-----|-----
1809 CONTINUOUS STANDHYD NHYD=["S-1-D3"], DT=[1](min), AREA=[6.79](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
1810 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
1811 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
1812 LGI=[212.760](m), MNI=[0.013], SCI=[0](min),
1813 Continuous simulation parameters:
1814 IaREcper=[4](hrs), IaREcimp=[4](hrs),
1815 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1816 InterEventTime=[12](hrs), END=-1
1817 *%-----|-----
1818 *CONTINUOUS NASHYD NHYD=["S-1-D3"], DT=[1]min, AREA=[6.79](ha),
1819 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1820 * N=[3], TP=[1.281]hrs,
1821 * Continuous simulation parameters:
1822 * IaREcper=[4](hrs),
1823 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1824 * InterEventTime=[12](hrs)
1825 * Baseflow simulation parameters:
1826 * BaseFlowOption=[1] ,
1827 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1828 * VHydCond=[0.055](mm/hr), END=-1
1829 *%-----|-----
1830 *COMPUTE DUALHYD NHYDin=["S-1-D3"], CINLET=[0.719](cms), NINLET=[1],
1831 * MajNHYD=["S-1-D3J"]
1832 * MinNHYD=["S-1-D3N"]
1833 * TMJSTO=[9999999](cu-m)
1834 *%-----|-----
1835 *ADD HYD NHYDsum=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
1836 *%-----|-----
1837 *ROUTE RESERVOIR NHYDout=["S-1-D3R"] ,NHYDin=["S-1-D3S"] ,
1838 * RDT=[1](min),
1839 *          TABLE of ( OUTFLOW-STORAGE ) values
1840 *          (cms) - (ha-m)
1841 *          [ 0.0      , 0.0 ]
1842 *          [ 0.0811, 0.2708 ]
1843 *          [   -1   ,  -1   ] (max twenty pts)
1844 *          NHYDovf=["S-1-D3Rovf"]
1845 *%-----|-----
1846 ADD HYD NHYDsum=["SN_OK"], NHYDs to add=["N_OK"+"OKEEFE"+"S-1-D2"+"S-1-D3"]
1847 *%-----|-----
1848 SAVE HYD NHYD=["SN_OK"], # OF PCYCLES=[-1], ICASEsh=[1]
1849 HYD_COMMENT=["Total Flows at Okeefe Drain"]
1850 *%-----|-----
1851 *#
1852 *# Hydrograph from Node Okeefe routed to Node at Foster Drain
1853 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
1854 *#
1855 ROUTE CHANNEL NHYDout=["N_FO"] ,NHYDin=["SN_OK"] ,
1856 RDT=[1](min),
1857 CHLGTH=[1183](m), CHSLOPE=[0.0761](%),
1858 FPSLOPE=[0.0761](%),
1859 SECNUM=[1.0], NSEG=[3]
1860 ( SEGROUGH, SEGDIST (m))=
1861 [0.050,-33.89
1862 -0.035,31.59
1863 0.050,34.41] NSEG times

```

```

1864      ( DISTANCE (m), ELEVATION (m))=
1865      [-794.18, 91.00]
1866      [-775.41, 91.50]
1867      [-702.63, 91.50]
1868      [-546.19, 91.50]
1869      [-529.54, 91.50]
1870      [-323.44, 91.00]
1871      [-320.71, 91.00]
1872      [-183.59, 91.00]
1873      [-182.54, 90.50]
1874      [-181.36, 90.00]
1875      [-177.37, 90.00]
1876      [-87.70, 90.00]
1877      [-33.89, 90.00]
1878      [-18.52, 86.88]
1879      [0.00,85.20]
1880      [16.20, 86.83]
1881      [31.59, 90.00]
1882      [33.03, 90.50]
1883      [34.41, 91.00]
1884      *%-----|-----|
1885      *#*****|
1886      *#      Catchment FOSTER
1887      *#      - To Foster ditch (north of the Jock)
1888      *#      - Partially developed (medium density); remaining agricultural
1889      *#      - 2020-12-01 JFSA Foster area is 332 as per Foster SWMF Environmental Study
1890      *#      - 2020-12-01 decrease Foster drainage area from (373 HA) to (307.98 HA) after
1891      *#      - 2021-02-12 update Foster area to 325.44 ha as measured from QGIS
1892      *#*****|
1893      CONTINUOUS STANDHYD NHYD=["FOSTER"], DT=[1]min, AREA=[325.44](ha),
1894      XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
1895      SCS curve number CN=[74],
1896      Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1897      LGP=[40](m), MNP=[0.25], SCP=[0](min),
1898      Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1899      LGI=[1472.956](m), MNI=[0.013], SCI=[0](min),
1900      Continuous simulation parameters:
1901      IaRECper=[4](hrs), IaRECimp=[4](hrs),
1902      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1903      InterEventTime=[18](hrs), END=-1
1904      *#*****|
1905      *#      Foster Pond
1906      *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1907      *#      and a ratio of the catchment area to the West Clarke pond rating curve
1908      *#      from the MSS for the next coordinates
1909      *#*****|
1910      ROUTE RESERVOIR      NHYDout=["P_FOS"], NHYDin=["FOSTER"],
1911      RDT=[1](min),
1912      TABLE of ( OUTFLOW-STORAGE ) values
1913      (cms) - (ha-m)
1914      [ 0.0 , 0.0 ]
1915      [ 10.34 , 10]
1916      [ -1 , -1 ] (max twenty pts)
1917      NHYDovf=["FO-OVF"]
1918      *%-----|-----|
1919      ADD HYD      NHYDsum=["FOSTER-OUT"], NHYDs to add=["P_FOS"+"FO-OVF"]
1920      *%-----|-----|
1921      *#*****|
1922      *      -Brazeau area from P 1800-19 =[71.751], change to 63.59 ha based on GIS measurements
1923      *      -JFSA, 2021-01-19 update "W_CLAR_BRAZ" to 73.29 ha based on GIS measurements
1924      *      -JFSA, 2021-01-22 Brazeau ("MS_P10"+"P10-OVF")brazeau pond discharges directly
1925      to the jock river through a road side ditch on the west side of Borrisokane road
1926      (station 6016)

```

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1925 CONTINUOUS STANDHYD NHYD=["W_CLAR_BRAZ"], DT=[1]min, AREA=[73.29](ha),
1926 XIMP=[0.6], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
1927 SCS curve number CN=[77],
1928 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
1929 LGP=[40](m), MNP=[0.25], SCP=[0](min),
1930 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1931 LGI=[699.00](m), MNI=[0.013], SCI=[0](min),
1932 Continuous simulation parameters:
1933 IaRECper=[4](hrs), IaRECimp=[4](hrs),
1934 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1935 InterEventTime=[18](hrs), END=-1
1936 *%-----|-----
1937 * 2020-12-01 correct pond curve values
1938 ROUTE RESERVOIR NHYDout=["MS_P10"], NHYDin=["W_CLAR_BRAZ"],
1939 RDT=[1](min),
1940 TABLE of ( OUTFLOW-STORAGE ) values
1941 ( cms ) - ( ha-m )
1942 [ 0.0 , 0.0 ]
1943 [ 0.068 , 0.001 ]
1944 [ 0.271 , 0.022 ]
1945 [ 0.379 , 0.051 ]
1946 [ 0.48 , 0.091 ]
1947 [ 0.853 , 0.341 ]
1948 [ 1.005 , 0.61 ]
1949 [ 1.128 , 1.231 ]
1950 [ 1.155 , 1.592 ]
1951 [ 1.194 , 1.876 ]
1952 [ 1.2 , 1.921 ]
1953 [ 1.259 , 2.369 ]
1954 [ 1.3 , 2.665 ]
1955 [ 1.349 , 2.813 ]
1956 [ -1 , -1 ] (max twenty pts)
1957 NHYDovf=["P10-OVF"]
1958 *%-----|-----
1959 * -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
1960 CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
1961 XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
1962 SCS curve number CN=[74],
1963 Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1964 LGP=[40](m), MNP=[0.25], SCP=[0](min),
1965 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1966 LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
1967 Continuous simulation parameters:
1968 IaRECper=[4](hrs), IaRECimp=[4](hrs),
1969 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1970 InterEventTime=[18](hrs), END=-1
1971 *%-----|-----
1972 *CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
1973 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1974 * N=[3], TP=[1.10]hrs,
1975 * Continuous simulation parameters:
1976 * IaRECper=[4](hrs),
1977 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1978 * InterEventTime=[12](hrs)
1979 * Baseflow simulation parameters:
1980 * BaseFlowOption=[1] ,
1981 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1982 * VHydCond=[0.055](mm/hr), END=-1
1983 *%-----|-----
1984 *COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CINLET=[0.508](cms), NINLET=[1],
1985 * MajNHYD=["S-1-FO-D2J"]
1986 * MinNHYD=["S-1-FO-D2N"]
1987 * TMJSTO=[9999999](cu-m)
1988 *%-----|-----
1989 *ADD HYD NHYDsum=["S-1-FO-D2S"], NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]

```

```

1990  *%-----|-----|
1991  *ROUTE RESERVOIR      NHYDout=["S-1-FO-D2R"] ,NHYDin=["S-1-FO-D2S"] ,
1992  *                      RDT=[1](min),
1993  *                      TABLE of ( OUTFLOW-STORAGE ) values
1994  *                      (cms) - (ha-m)
1995  *                      [ 0.0      , 0.0 ]
1996  *                      [ 0.0590, 0.1970 ]
1997  *                      [   -1   ,  -1   ] (max twenty pts)
1998  *                      NHYDovf=["S-1FOD2ovf"]
1999  *%-----|-----|
2000  ADD HYD                NHYDsum=["980"], NHYDs to add=["FOSTER-OUT"+"S-1-FO-D2"]
2001  *%-----|-----|
2002  SAVE HYD               NHYD=["980"], # OF PCYCLES=[-1], ICASEsh=[1]
2003  HYD_COMMENT=["Total Flows at Station 980 on Foster Drain"]
2004  *%-----|-----|
2005  *#
2006  *# Hydrograph from Node Foster SWM (Station 980)to Node at station 520
2007  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 980
2008  *#
2009  ROUTE CHANNEL          NHYDout=["980-out"] ,NHYDin=["980"] ,
2010  RDT=[1](min),
2011  CHLGTH=[460](m),    CHSLOPE=[0.04348](%),
2012  FPSLOPE=[0.04348](%),
2013  SECNUM=[1.0],      NSEG=[3]
2014  ( SEGROUGH, SEGDIST (m))=
2015  [0.050,45.90
2016  -0.035,53.30
2017  0.050,100] NSEG times
2018  ( DISTANCE (m), ELEVATION (m))=
2019  [0, 91.75 ]
2020  [42.4, 92.18 ]
2021  [43.5, 92.16 ]
2022  [44.1, 92.1 ]
2023  [44.6, 92 ]
2024  [44.8, 91.86 ]
2025  [45.9, 91.04 ]
2026  [46.4, 90.65 ]
2027  [46.8, 90.36 ]
2028  [47.9, 90.32 ]
2029  [48.7, 90.35 ]
2030  [50.7, 90.33 ]
2031  [52.2, 90.38 ]
2032  [52.5, 90.59 ]
2033  [53.3, 91.28 ]
2034  [54, 91.83 ]
2035  [54.3, 92 ]
2036  [54.8, 92.08 ]
2037  [55.4, 92.12 ]
2038  [100, 91.84 ]
2039  *%-----|-----|
2040  * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2041  CONTINUOUS STANDHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2042  XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2043  SCS curve number CN=[74],
2044  Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2045  LGP=[40](m), MNP=[0.25], SCP=[0](min),
2046  Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2047  LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2048  Continuous simulation parameters:
2049  IaRECper=[4](hrs), IaRECimp=[4](hrs),
2050  SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2051  InterEventTime=[18](hrs), END=-1
2052  *%-----|-----|
2053  *COMPUTE DUALHYD      NHYDin=["S-1-FO-D1"], CINLET=[0.605](cms), NINLET=[1],
2054  *                      MajNHYD=["S-1-FO-D1J"]

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```

2055 *           MinNHYD=["S-1-FO-D1N"]
2056 *           TMJSTO=[9999999](cu-m)
2057 *%-----|-----|
2058 *ADD HYD           NHYDsum=["S-1-FO-D1S"], NHYDs to add=["S-1-FO-D1N"+"S-1-FO-D1J"]
2059 *%-----|-----|
2060 *ROUTE RESERVOIR  NHYDout=["S-1-FO-D1R"] ,NHYDin=["S-1-FO-D1S"] ,
2061 *           RDT=[1](min),
2062 *           TABLE of ( OUTFLOW-STORAGE ) values
2063 *           (cms) - (ha-m)
2064 *           [ 0.0      , 0.0 ]
2065 *           [ 0.0611, 0.2038 ]
2066 *           [   -1   ,  -1   ] (max twenty pts)
2067 *           NHYDovf=["S-1FOD1ovf"]
2068 *%-----|-----|
2069 ADD HYD           NHYDsum=["520"], NHYDs to add=["980-out"+"S-1-FO-D1"]
2070 *%-----|-----|
2071 SAVE HYD        NHYD=["520"], # OF PCYCLES=[-1], ICASEsh=[1]
2072           HYD_COMMENT=["Total Flows at Sation 520 on Foster Drain"]
2073 *%-----|-----|
2074 *# Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock
River)
2075 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 520
2076 *#
2077 ROUTE CHANNEL   NHYDout=["520-out"] ,NHYDin=["520"] ,
2078           RDT=[1](min),
2079           CHLGTH=[860](m),  CHSLOPE=[0.5872](%),
2080           FPSLOPE=[0.5872](%),
2081           SECNUM=[1.0],      NSEG=[3]
2082           ( SEGROUGH, SEGDIST (m))=
2083           [0.050,45.90
2084           -0.035,54.3
2085           0.050,100.1097] NSEG times
2086           ( DISTANCE (m), ELEVATION (m))=
2087           [0, 91.26 ]
2088           [44.9, 91.46 ]
2089           [45.1, 91.37 ]
2090           [45.9, 90.84 ]
2091           [47, 90.32 ]
2092           [47.5, 90.22 ]
2093           [48, 90.17 ]
2094           [50.7, 90.19 ]
2095           [51.5, 90.17 ]
2096           [52.2, 90.13 ]
2097           [52.7, 90.12 ]
2098           [53.3, 90.14 ]
2099           [53.5, 90.31 ]
2100           [53.9, 90.59 ]
2101           [54.3, 90.87 ]
2102           [54.7, 91.04 ]
2103           [55.3, 91.24 ]
2104           [55.5, 91.26 ]
2105           [63.7, 91.37 ]
2106           [100.1097, 91.43 ]
2107 *%-----|-----|
2108 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2109 CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2110           XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2111           SCS curve number CN=[74],
2112           Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2113           LGP=[40](m), MNP=[0.25], SCP=[0](min),
2114           Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2115           LGI=[315.806](m), MNI=[0.013], SCI=[0](min),
2116           Continuous simulation parameters:
2117           IaREcper=[4](hrs), IaREcimp=[4](hrs),
2118           SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

```

```

2119 InterEventTime=[18](hrs), END=-1
2120 *%-----|-----|
2121 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2122 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2123 * N=[3], TP=[1.007]hrs,
2124 * Continuous simulation parameters:
2125 * IaRECper=[4](hrs),
2126 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2127 * InterEventTime=[12](hrs)
2128 * Baseflow simulation parameters:
2129 * BaseFlowOption=[1] ,
2130 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2131 * VHydCond=[0.055](mm/hr), END=-1
2132 *%-----|-----|
2133 *COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CINLET=[1.749](cms), NINLET=[1],
2134 * MajNHYD=["S-1FO-F-DJ"]
2135 * MinNHYD=["S-1FO-F-DN"]
2136 * TMJSTO=[9999999](cu-m)
2137 *%-----|-----|
2138 *ADD HYD NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2139 *%-----|-----|
2140 *ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"],NHYDin=["S-1FO-F-DS"] ,
2141 * RDT=[1](min),
2142 * TABLE of ( OUTFLOW-STORAGE ) values
2143 * (cms) - (ha-m)
2144 * [ 0.0 , 0.0 ]
2145 * [ 0.1788, 0.5966 ]
2146 * [ -1 , -1 ] (max twenty pts)
2147 * NHYDovf=["S-1FoFDovf"]
2148 *%-----|-----|
2149 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2150 CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha), XIMP=[0.325],
TIMP=[0.65], DWF=[0](cms), LOSS=[1]:
2151 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
2152 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
2153 Impervious areas: IAimp=[0.785](mm), SLPI=[0.75](%),
LGI=[187.439](m), MNI=[0.013], SCI=[0](min),
2154 Continuous simulation parameters:
2155 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
2156 *%-----|-----|
2157 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha),
2158 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2159 * N=[3], TP=[1.10]hrs,
2160 * Continuous simulation parameters:
2161 * IaRECper=[4](hrs),
2162 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2163 * InterEventTime=[12](hrs)
2164 * Baseflow simulation parameters:
2165 * BaseFlowOption=[1] ,
2166 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2167 * VHydCond=[0.055](mm/hr), END=-1
2168 *%-----|-----|
2169 *COMPUTE DUALHYD NHYDin=["S-1-D8"], CINLET=[2.279](cms), NINLET=[1],
2170 * MajNHYD=["S-1-D8J"]
2171 * MinNHYD=["S-1-D8N"]
2172 * TMJSTO=[9999999](cu-m)
2173 *%-----|-----|
2174 *ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2175 *%-----|-----|
2176 *ADD HYD NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Post"]
2177 *%-----|-----|
2178 *COMPUTE DUALHYD NHYDin=["S-1-D"], CINLET=[11.616](cms), NINLET=[1],

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2179 *           MajNHYD=["S-1-D-MJ"]
2180 *           MinNHYD=["S-1-D-MN"]
2181 *           TMJSTO=[5974](cu-m)
2182 *%-----|-----|
2183 *ADD HYD           NHYDsum=["S-1-DEV"], NHYDs to add=["S-1-D-MJ"+"S-1-D-MN"]
2184 *%-----|-----|
2185 *ROUTE RESERVOIR  NHYDout=["S-1-D8R"], NHYDin=["S-1-D8S"],
2186 *           RDT=[1](min),
2187 *           TABLE of ( OUTFLOW-STORAGE ) values
2188 *                   (cms) - (ha-m)
2189 *                   [ 0.0      , 0.0 ]
2190 *                   [ 0.0630, 0.2102 ]
2191 *                   [   -1    ,  -1    ] (max twenty pts)
2192 *           NHYDovf=["S-1-D8Rovf"]
2193 *%-----|-----|
2194 *   -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2195 CONTINUOUS NASHYD NHYD=["S-1-A"], DT=[1]min, AREA=[75.88](ha),
2196 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2197 N=[3], TP=[0.619]hrs,
2198 Continuous simulation parameters:
2199 IaRECPper=[4](hrs),
2200 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2201 InterEventTime=[12](hrs)
2202 Baseflow simulation parameters:
2203 BaseFlowOption=[1],
2204 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2205 VHydCond=[0.055](mm/hr), END=-1
2206 *%-----|-----|
2207 *   -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2208 CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2209 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2210 N=[3], TP=[1.10]hrs,
2211 Continuous simulation parameters:
2212 IaRECPper=[4](hrs),
2213 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2214 InterEventTime=[12](hrs)
2215 Baseflow simulation parameters:
2216 BaseFlowOption=[1],
2217 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2218 VHydCond=[0.055](mm/hr), END=-1
2219 *%-----|-----|
2220 ADD HYD           NHYDsum=["SN_FO"], NHYDs to
add=["N_FO"+"520-out"+"MS_P10"+"P10-OVF"+"W_CLAR_UNDE"+"S-1-FO-F-D"+"S-1-D8"+"S-1-A"]
2221 *%-----|-----|
2222 SAVE HYD         NHYD=["SN_FO"], # OF PCYCLES=[-1], ICASEsh=[1]
2223 HYD_COMMENT=["Total Flows at Foster Drain"]
2224 *%-----|-----|
2225 *# Hydrograph from Node Foster routed to Node at Cedarview Road
2226 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
2227 *#
2228 ROUTE CHANNEL   NHYDout=["N_CE"], NHYDin=["SN_FO"],
2229 RDT=[1](min),
2230 CHLGTH=[159](m), CHSLOPE=[0.0818](%),
2231 FPSLOPE=[0.0818](%),
2232 SECNUM=[1.0], NSEG=[3]
2233 ( SEGROUGH, SEGDIST (m))=
2234 [0.050,-15.46
2235 -0.035,26.55
2236 0.050,116.76] NSEG times
2237 ( DISTANCE (m), ELEVATION (m))=
2238 [-645.23, 91.50]
2239 [-391.20, 91.50]
2240 [-91.00, 91.50]

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2241          [-85.52, 91.50]
2242          [-15.46, 89.40]
2243          [-9.79, 89.31]
2244          [-3.22, 86.24]
2245          [3.22, 85.07]
2246          [10.96, 85.79]
2247          [16.44, 86.49]
2248          [26.55, 89.45]
2249          [29.03, 90.27]
2250          [35.76, 90.67]
2251          [36.67, 91.00]
2252          [108.08, 91.00]
2253          [109.82, 90.50]
2254          [112.04, 90.50]
2255          [114.62, 91.00]
2256          [116.76, 91.50]
2257  *%-----|-----|
2258  *#*****|*****|
2259  *#      Catchment S-1
2260  *#      - To Jock River (north and south of Jock)
2261  *#      - Primarily agricultural fields; portion of sand quarry
2262  *%-----|-----|
2263  *%      -2020-12-17 "S-1-Undev" and "S-1-Fost" was a part of Foster drain, they are below
the foster pond. Now they are added to S-1 subcatchment based on Project 1474-BCDC,
JFSA, Nov. 2020
2264  *%      -2020-12-17 Change O-14 (it was part of Okeefe drain) to "S-1-Okeefe" and add it
to S-1 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
2265  *%      -2020-12-17 Add "S-1-BCDC" as NASHYD
2266  *%      -2020-12-17 all other S-1 subcatchment as STANDHYD with DUALHYD and ROUTE RESERVOIR
2267  *%-----|-----|
2268  *#*****|*****|
2269  *      -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2270  *CONTINUOUS NASHYD  NHYD=["S-1-A"], DT=[1]min, AREA=[75.88](ha),
2271  *                    DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2272  *                    N=[3], TP=[0.619]hrs,
2273  *                    Continuous simulation parameters:
2274  *                    IaRECper=[4](hrs),
2275  *                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2276  *                    InterEventTime=[12](hrs)
2277  *                    Baseflow simulation parameters:
2278  *                    BaseFlowOption=[1] ,
2279  *                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2280  *                    VHydCond=[0.055](mm/hr), END=-1
2281  *%-----|-----|
2282  CONTINUOUS NASHYD  NHYD=["S-1-B"], DT=[1]min, AREA=[55.36](ha),
2283  *                    DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2284  *                    N=[3], TP=[0.451]hrs,
2285  *                    Continuous simulation parameters:
2286  *                    IaRECper=[4](hrs),
2287  *                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2288  *                    InterEventTime=[12](hrs)
2289  *                    Baseflow simulation parameters:
2290  *                    BaseFlowOption=[1] ,
2291  *                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2292  *                    VHydCond=[0.055](mm/hr), END=-1
2293  *%-----|-----|
2294  *#      - JFSA 2021-02-24 change the name from S-1-BCDC to S-1-A and S-1-B. Change their
TP values based on the new areas compared to the old ones.
2295  *CONTINUOUS NASHYD  NHYD=["S-1-BCDC"], DT=[1]min, AREA=[134.9](ha),
2296  *                    DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2297  *                    N=[3], TP=[1.10]hrs,
2298  *                    Continuous simulation parameters:
2299  *                    IaRECper=[4](hrs),
2300  *                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2301  *                    InterEventTime=[12](hrs)

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2302 *           Baseflow simulation parameters:
2303 *           BaseFlowOption=[1] ,
2304 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2305 *           VHydCond=[0.055](mm/hr),   END=-1
2306 *%-----|-----
2307 *#    - JFSA 2021-02-24 "S-1-BCDC-1" and "S-1-BCDC-2" are not existing anymore.
    "S-1-BCDC-1" is part of "S-1-FO-D2" and "S-1-BCDC-2" is part of "S-1-D2" and "S-1-D3"
2308 *CONTINUOUS NASHYD  NHYD=["S-1-BCDC-1"], DT=[1]min, AREA=[0.3](ha),
2309 *           DWF=[0](cms), CN/C=[77],   IA=[4.67](mm),
2310 *           N=[3], TP=[1.10]hrs,
2311 *           Continuous simulation parameters:
2312 *           IaRECper=[4](hrs),
2313 *           SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
2314 *           InterEventTime=[12](hrs)
2315 *           Baseflow simulation parameters:
2316 *           BaseFlowOption=[1] ,
2317 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2318 *           VHydCond=[0.055](mm/hr),   END=-1
2319 *%-----|-----
2320 *CONTINUOUS NASHYD  NHYD=["S-1-BCDC-2"], DT=[1]min, AREA=[1.3](ha),
2321 *           DWF=[0](cms), CN/C=[77],   IA=[4.67](mm),
2322 *           N=[3], TP=[1.10]hrs,
2323 *           Continuous simulation parameters:
2324 *           IaRECper=[4](hrs),
2325 *           SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
2326 *           InterEventTime=[12](hrs)
2327 *           Baseflow simulation parameters:
2328 *           BaseFlowOption=[1] ,
2329 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2330 *           VHydCond=[0.055](mm/hr),   END=-1
2331 *%-----|-----
2332 *#    - JFSA 2021-01-19, after adding Greenbank pond, "S-1-BCDC-3" is not existing
    anymore
2333 *CONTINUOUS NASHYD  NHYD=["S-1-BCDC-3"], DT=[1]min, AREA=[3.9](ha),
2334 *           DWF=[0](cms), CN/C=[77],   IA=[4.67](mm),
2335 *           N=[3], TP=[1.10]hrs,
2336 *           Continuous simulation parameters:
2337 *           IaRECper=[4](hrs),
2338 *           SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
2339 *           InterEventTime=[12](hrs)
2340 *           Baseflow simulation parameters:
2341 *           BaseFlowOption=[1] ,
2342 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2343 *           VHydCond=[0.055](mm/hr),   END=-1
2344 *%-----|-----
2345 *    -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
    before station 7245 on Jock River
2346 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[1](min), AREA=[44.93](ha), XIMP=[0.65],
    TIMP=[0.65], DWF=[0](cms),
2347 *           LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
    IAper=[4.67](mm), SLPP=[2.0](%),
2348 *           LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
    IAimp=[1.57](mm), SLPI=[0.75](%),
2349 *           LGI=[547.296](m), MNI=[0.013], SCI=[0](min),
2350 *           Continuous simulation parameters:
2351 *           IaRECper=[4](hrs), IaRECimp=[4](hrs),
2352 *           SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
2353 *           InterEventTime=[12](hrs),   END=-1
2354 *%-----|-----
2355 *COMPUTE DUALHYD  NHYDin=["S-1-Okeefe"], CINLET=[4.796](cms), NINLET=[1],
2356 *           MajNHYD=["S-1-OkMJ"]
2357 *           MinNHYD=["S-1-OkMN"]
2358 *           TMJSTO=[9999999](cu-m)
2359 *%-----|-----
2360 *ADD HYD           NHYDsum=["S-1-OkS"], NHYDs to add=["S-1-OkMJ"+"S-1-OkMN"]
2361 *%-----|-----

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2362 *ROUTE RESERVOIR      NHYDout=["S-1-OksR" ] ,NHYDin=["S-1-Oks" ] ,
2363 *                      RDT=[1](min),
2364 *                      TABLE of ( OUTFLOW-STORAGE ) values
2365 *                      (cms) - (ha-m)
2366 *                      [ 0.0      , 0.0 ]
2367 *                      [ 0.5370, 1.7917 ]
2368 *                      [   -1   ,  -1   ] (max twenty pts)
2369 *                      NHYDovf=["S-1-OkSovf" ]
2370 *%-----|-----|
2371 *CONTINUOUS NASHYD     NHYD=["S-1-Okeefe"], DT=[1]min, AREA=[44.93](ha),
2372 *                      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2373 *                      N=[3], TP=[1.049]hrs,
2374 *                      Continuous simulation parameters:
2375 *                      IaRECper=[4](hrs),
2376 *                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2377 *                      InterEventTime=[12](hrs)
2378 *                      Baseflow simulation parameters:
2379 *                      BaseFlowOption=[1] ,
2380 *                      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2381 *                      VHydCond=[0.055](mm/hr), END=-1
2382 *%-----|-----|
2383 * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2384 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2385 *                      XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2386 *                      SCS curve number CN=[74],
2387 *                      Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2388 *                      LGP=[40](m), MNP=[0.25], SCP=[0](min),
2389 *                      Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2390 *                      LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2391 *                      Continuous simulation parameters:
2392 *                      IaRECper=[4](hrs), IaRECimp=[4](hrs),
2393 *                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2394 *                      InterEventTime=[18](hrs), END=-1
2395 *%-----|-----|
2396 *COMPUTE DUALHYD      NHYDin=["S-1-FO-D1"], CINLET=[0.605](cms), NINLET=[1],
2397 *                      MajNHYD=["S-1-FO-D1J"]
2398 *                      MinNHYD=["S-1-FO-D1N"]
2399 *                      TMJSTO=[9999999](cu-m)
2400 *%-----|-----|
2401 *ADD HYD               NHYDsum=["S-1-FO-D1S"], NHYDs to add=["S-1-FO-D1N"+"S-1-FO-D1J"]
2402 *%-----|-----|
2403 *ROUTE RESERVOIR      NHYDout=["S-1-FO-D1R" ] ,NHYDin=["S-1-FO-D1S" ] ,
2404 *                      RDT=[1](min),
2405 *                      TABLE of ( OUTFLOW-STORAGE ) values
2406 *                      (cms) - (ha-m)
2407 *                      [ 0.0      , 0.0 ]
2408 *                      [ 0.0611, 0.2038 ]
2409 *                      [   -1   ,  -1   ] (max twenty pts)
2410 *                      NHYDovf=["S-1FOD1ovf" ]
2411 *%-----|-----|
2412 *CONTINUOUS NASHYD     NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2413 *                      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2414 *                      N=[3], TP=[1.10]hrs,
2415 *                      Continuous simulation parameters:
2416 *                      IaRECper=[4](hrs),
2417 *                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2418 *                      InterEventTime=[12](hrs)
2419 *                      Baseflow simulation parameters:
2420 *                      BaseFlowOption=[1] ,
2421 *                      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2422 *                      VHydCond=[0.055](mm/hr), END=-1
2423 *%-----|-----|
2424 * -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
2425 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),

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2426 * XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
2427 * SCS curve number CN=[74],
2428 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2429 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
2430 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2431 * LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
2432 * Continuous simulation parameters:
2433 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2434 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2435 * InterEventTime=[18](hrs), END=-1
2436 *%-----|-----
2437 *CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
2438 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2439 * N=[3], TP=[1.10]hrs,
2440 * Continuous simulation parameters:
2441 * IaRECper=[4](hrs),
2442 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2443 * InterEventTime=[12](hrs)
2444 * Baseflow simulation parameters:
2445 * BaseFlowOption=[1] ,
2446 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2447 * VHydCond=[0.055](mm/hr), END=-1
2448 *%-----|-----
2449 *COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CINLET=[0.508](cms), NINLET=[1],
2450 * MajNHYD=["S-1-FO-D2J"]
2451 * MinNHYD=["S-1-FO-D2N"]
2452 * TMJSTO=[9999999](cu-m)
2453 *%-----|-----
2454 *ADD HYD NHYDsum=["S-1-FO-D2S"], NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]
2455 *%-----|-----
2456 *ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"],NHYDin=["S-1-FO-D2S"] ,
2457 * RDT=[1](min),
2458 * TABLE of ( OUTFLOW-STORAGE ) values
2459 * (cms) - (ha-m)
2460 * [ 0.0 , 0.0 ]
2461 * [ 0.0590, 0.1970 ]
2462 * [ -1 , -1 ] (max twenty pts)
2463 * NHYDovf=["S-1FOD2ovf"]
2464 *%-----|-----
2465 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2466 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2467 * XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2468 * SCS curve number CN=[74],
2469 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2470 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
2471 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2472 * LGI=[315.806](m), MNI=[0.013], SCI=[0](min),
2473 * Continuous simulation parameters:
2474 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2475 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2476 * InterEventTime=[18](hrs), END=-1
2477 *%-----|-----
2478 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2479 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2480 * N=[3], TP=[1.007]hrs,
2481 * Continuous simulation parameters:
2482 * IaRECper=[4](hrs),
2483 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2484 * InterEventTime=[12](hrs)
2485 * Baseflow simulation parameters:
2486 * BaseFlowOption=[1] ,
2487 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2488 * VHydCond=[0.055](mm/hr), END=-1
2489 *%-----|-----
2490 *COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CINLET=[1.749](cms), NINLET=[1],

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2491 *           MajNHYD=["S-1FO-F-DJ"]
2492 *           MinNHYD=["S-1FO-F-DN"]
2493 *           TMJSTO=[9999999](cu-m)
2494 *%-----|-----|
2495 *ADD HYD           NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2496 *%-----|-----|
2497 *ROUTE RESERVOIR  NHYDout=["S-1FO-F-DR"],NHYDin=["S-1FO-F-DS"],
2498 *                 RDT=[1](min),
2499 *                 TABLE of ( OUTFLOW-STORAGE ) values
2500 *                 (cms) - (ha-m)
2501 *                 [ 0.0      , 0.0 ]
2502 *                 [ 0.1788, 0.5966 ]
2503 *                 [   -1   ,  -1   ] (max twenty pts)
2504 *                 NHYDovf=["S-1FoFDovf"]
2505 *%-----|-----|
2506 CONTINUOUS STANDHYD NHYD=["S-1-D1"], DT=[1](min), AREA=[21.67](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2507 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2508 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2509 LGI=[380.088](m), MNI=[0.013], SCI=[0](min),
2510 Continuous simulation parameters:
2511 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2512 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2513 InterEventTime=[12](hrs), END=-1
2514 *%-----|-----|
2515 *CONTINUOUS NASHYD NHYD=["S-1-D1"], DT=[1]min, AREA=[21.67](ha),
2516 *                 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2517 *                 N=[3], TP=[1.066]hrs,
2518 *                 Continuous simulation parameters:
2519 *                 IaRECper=[4](hrs),
2520 *                 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2521 *                 InterEventTime=[12](hrs)
2522 *                 Baseflow simulation parameters:
2523 *                 BaseFlowOption=[1],
2524 *                 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2525 *                 VHydCond=[0.055](mm/hr), END=-1
2526 *%-----|-----|
2527 *COMPUTE DUALHYD  NHYDin=["S-1-D1"], CINLET=[2.482](cms), NINLET=[1],
2528 *                 MajNHYD=["S-1-D1J"]
2529 *                 MinNHYD=["S-1-D1N"]
2530 *                 TMJSTO=[9999999](cu-m)
2531 *%-----|-----|
2532 *ADD HYD           NHYDsum=["S-1-D1S"], NHYDs to add=["S-1-D1J"+"S-1-D1N"]
2533 *%-----|-----|
2534 *ROUTE RESERVOIR  NHYDout=["S-1-D1R"],NHYDin=["S-1-D1S"],
2535 *                 RDT=[1](min),
2536 *                 TABLE of ( OUTFLOW-STORAGE ) values
2537 *                 (cms) - (ha-m)
2538 *                 [ 0.0      , 0.0 ]
2539 *                 [ 0.2590, 0.8642 ]
2540 *                 [   -1   ,  -1   ] (max twenty pts)
2541 *                 NHYDovf=["S-1-D1Rovf"]
2542 *%-----|-----|
2543 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
moved to drain before station 6215 on Jock River
2544 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2545 *                 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2546 *                 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2547 *                 LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
2548 *                 Continuous simulation parameters:
2549 *                 IaRECper=[4](hrs), IaRECimp=[4](hrs),

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2550 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2551 * InterEventTime=[12](hrs), END=-1
2552 *%-----|-----
2553 *CONTINUOUS NASHYD NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
2554 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2555 * N=[3], TP=[1.120]hrs,
2556 * Continuous simulation parameters:
2557 * IaRECper=[4](hrs),
2558 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2559 * InterEventTime=[12](hrs)
2560 * Baseflow simulation parameters:
2561 * BaseFlowOption=[1] ,
2562 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2563 * VHydCond=[0.055](mm/hr), END=-1
2564 *%-----|-----
2565 *COMPUTE DUALHYD NHYDin=["S-1-D2"], CINLET=[2.062](cms), NINLET=[1],
2566 * MajNHYD=["S-1-D2J"]
2567 * MinNHYD=["S-1-D2N"]
2568 * TMJSTO=[9999999](cu-m)
2569 *%-----|-----
2570 *ADD HYD NHYDsum=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
2571 *%-----|-----
2572 *ROUTE RESERVOIR NHYDout=["S-1-D2R"],NHYDin=["S-1-D2S"] ,
2573 * RDT=[1](min),
2574 * TABLE of ( OUTFLOW-STORAGE ) values
2575 * (cms) - (ha-m)
2576 * [ 0.0 , 0.0 ]
2577 * [ 0.2231, 0.7445 ]
2578 * [ -1 , -1 ] (max twenty pts)
2579 * NHYDovf=["S-1-D2Rovf"]
2580 *%-----|-----
2581 *CONTINUOUS STANDHYD NHYD=["S-1-D3"], DT=[1](min), AREA=[6.79](ha), XIMP=[0.65],
2582 * TIMP=[0.65], DWF=[0](cms),
2583 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2584 * IAper=[4.67](mm), SLPP=[2.0](%),
2585 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2586 * IAimp=[1.57](mm), SLPI=[0.75](%),
2587 * LGI=[212.760](m), MNI=[0.013], SCI=[0](min),
2588 * Continuous simulation parameters:
2589 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2590 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2591 * InterEventTime=[12](hrs), END=-1
2592 *%-----|-----
2593 *CONTINUOUS NASHYD NHYD=["S-1-D3"], DT=[1]min, AREA=[6.79](ha),
2594 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2595 * N=[3], TP=[1.281]hrs,
2596 * Continuous simulation parameters:
2597 * IaRECper=[4](hrs),
2598 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2599 * InterEventTime=[12](hrs)
2600 * Baseflow simulation parameters:
2601 * BaseFlowOption=[1] ,
2602 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2603 * VHydCond=[0.055](mm/hr), END=-1
2604 *%-----|-----
2605 *COMPUTE DUALHYD NHYDin=["S-1-D3"], CINLET=[0.719](cms), NINLET=[1],
2606 * MajNHYD=["S-1-D3J"]
2607 * MinNHYD=["S-1-D3N"]
2608 * TMJSTO=[9999999](cu-m)
2609 *%-----|-----
2610 *ADD HYD NHYDsum=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
2611 *%-----|-----
2612 *ROUTE RESERVOIR NHYDout=["S-1-D3R"],NHYDin=["S-1-D3S"] ,
2613 * RDT=[1](min),
2614 * TABLE of ( OUTFLOW-STORAGE ) values
2615 * (cms) - (ha-m)

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2613 * [ 0.0 , 0.0 ]
2614 * [ 0.0811, 0.2708 ]
2615 * [ -1 , -1 ] (max twenty pts)
2616 * NHYDovf=["S-1-D3Rovf"]
2617 *%-----|-----|
2618 CONTINUOUS STANDHYD NHYD=["S-1-D4"], DT=[1](min), AREA=[3.28](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2619 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2620 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2621 LGI=[147.874](m), MNI=[0.013], SCI=[0](min),
2622 Continuous simulation parameters:
IaRECper=[4](hrs), IaRECimp=[4](hrs),
2623 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2624 InterEventTime=[12](hrs), END=-1
2625
2626 *%-----|-----|
2627 *CONTINUOUS NASHYD NHYD=["S-1-D4"], DT=[1]min, AREA=[3.28](ha),
2628 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2629 * N=[3], TP=[1.10]hrs,
2630 * Continuous simulation parameters:
2631 * IaRECper=[4](hrs),
2632 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2633 * InterEventTime=[12](hrs)
2634 * Baseflow simulation parameters:
2635 * BaseFlowOption=[1] ,
2636 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2637 * VHydCond=[0.055](mm/hr), END=-1
2638 *%-----|-----|
2639 *COMPUTE DUALHYD NHYDin=["S-1-D4"], CINLET=[0.373](cms), NINLET=[1],
2640 * MajNHYD=["S-1-D4J"]
2641 * MinNHYD=["S-1-D4N"]
2642 * TMJSTO=[9999999](cu-m)
2643 *%-----|-----|
2644 *ADD HYD NHYDsum=["S-1-D4S"], NHYDs to add=["S-1-D4J"+"S-1-D4N"]
2645 *%-----|-----|
2646 *ROUTE RESERVOIR NHYDout=["S-1-D4R"],NHYDin=["S-1-D4S"],
2647 * RDT=[1](min),
2648 * TABLE of ( OUTFLOW-STORAGE ) values
2649 * (cms) - (ha-m)
2650 * [ 0.0 , 0.0 ]
2651 * [ 0.0392, 0.1308 ]
2652 * [ -1 , -1 ] (max twenty pts)
2653 * NHYDovf=["S-1-D4Rovf"]
2654 *%-----|-----|
2655 CONTINUOUS STANDHYD NHYD=["S-1-D5"], DT=[1](min), AREA=[12.84](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2656 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2657 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2658 LGI=[292.57](m), MNI=[0.013], SCI=[0](min),
2659 Continuous simulation parameters:
IaRECper=[4](hrs), IaRECimp=[4](hrs),
2660 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2661 InterEventTime=[12](hrs), END=-1
2662
2663 *%-----|-----|
2664 *CONTINUOUS NASHYD NHYD=["S-1-D5"], DT=[1]min, AREA=[12.84](ha),
2665 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2666 * N=[3], TP=[1.10]hrs,
2667 * Continuous simulation parameters:
2668 * IaRECper=[4](hrs),
2669 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2670 * InterEventTime=[12](hrs)
2671 * Baseflow simulation parameters:
2672 * BaseFlowOption=[1] ,

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2673 *          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2674 *          VHydCond=[0.055](mm/hr),    END=-1
2675 *%-----|-----
2676 *COMPUTE DUALHYD  NHYDin=["S-1-D5"], CINLET=[1.395](cms), NINLET=[1],
2677 *                  MajNHYD=["S-1-D5J"]
2678 *                  MinNHYD=["S-1-D5N"]
2679 *                  TMJSTO=[9999999](cu-m)
2680 *%-----|-----
2681 *ADD HYD          NHYDsum=["S-1-D5S"], NHYDs to add=["S-1-D5J"+"S-1-D5N"]
2682 *%-----|-----
2683 *ROUTE RESERVOIR NHYDout=["S-1-D5R"],NHYDin=["S-1-D5S"],
2684 *                  RDT=[1](min),
2685 *                  TABLE of ( OUTFLOW-STORAGE ) values
2686 *                  (cms) - (ha-m)
2687 *                  [ 0.0      , 0.0 ]
2688 *                  [ 0.1535, 0.5120 ]
2689 *                  [   -1   ,  -1   ] (max twenty pts)
2690 *                  NHYDovf=["S-1-D5Rovf"]
2691 *%-----|-----
2692 CONTINUOUS STANDHYD NHYD=["S-1-D6"], DT=[1](min), AREA=[1.75](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2693 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2694 IAper=[4.67](mm), SLPP=[2.0](%),
LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2695 IAimp=[1.57](mm), SLPI=[0.75](%),
2696 LGI=[108.01](m), MNI=[0.013], SCI=[0](min),
2697 Continuous simulation parameters:
2698 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2699 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
InterEventTime=[12](hrs),    END=-1
2700 *%-----|-----
2701 *CONTINUOUS NASHYD  NHYD=["S-1-D6"], DT=[1]min, AREA=[1.75](ha),
2702 *                  DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2703 *                  N=[3], TP=[1.10]hrs,
2704 *                  Continuous simulation parameters:
2705 *                  IaRECper=[4](hrs),
2706 *                  SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2707 *                  InterEventTime=[12](hrs)
2708 *                  Baseflow simulation parameters:
2709 *                  BaseFlowOption=[1],
2710 *                  InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2711 *                  VHydCond=[0.055](mm/hr),    END=-1
2712 *%-----|-----
2713 *COMPUTE DUALHYD  NHYDin=["S-1-D6"], CINLET=[0.218](cms), NINLET=[1],
2714 *                  MajNHYD=["S-1-D6J"]
2715 *                  MinNHYD=["S-1-D6N"]
2716 *                  TMJSTO=[9999999](cu-m)
2717 *%-----|-----
2718 *ADD HYD          NHYDsum=["S-1-D6S"], NHYDs to add=["S-1-D6J"+"S-1-D6N"]
2719 *%-----|-----
2720 *ROUTE RESERVOIR NHYDout=["S-1-D6R"],NHYDin=["S-1-D6S"],
2721 *                  RDT=[1](min),
2722 *                  TABLE of ( OUTFLOW-STORAGE ) values
2723 *                  (cms) - (ha-m)
2724 *                  [ 0.0      , 0.0 ]
2725 *                  [ 0.0209, 0.0698 ]
2726 *                  [   -1   ,  -1   ] (max twenty pts)
2727 *                  NHYDovf=["S-1-D6Rovf"]
2728 *%-----|-----
2729 CONTINUOUS STANDHYD NHYD=["S-1-D7"], DT=[1](min), AREA=[2.03](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2730 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2731 IAper=[4.67](mm), SLPP=[2.0](%),
LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2732 IAimp=[1.57](mm), SLPI=[0.75](%),
LGI=[116.33](m), MNI=[0.013], SCI=[0](min),

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2733 Continuous simulation parameters:
2734 IaREcper=[4](hrs), IaREcimp=[4](hrs),
2735 SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2736 InterEventTime=[12](hrs), END=-1
2737 *%-----|-----|
2738 *CONTINUOUS NASHYD NHYD=["S-1-D7"], DT=[1]min, AREA=[2.03](ha),
2739 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2740 * N=[3], TP=[1.10]hrs,
2741 * Continuous simulation parameters:
2742 * IaREcper=[4](hrs),
2743 * SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2744 * InterEventTime=[12](hrs)
2745 * Baseflow simulation parameters:
2746 * BaseFlowOption=[1] ,
2747 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2748 * VHydCond=[0.055](mm/hr), END=-1
2749 *%-----|-----|
2750 *COMPUTE DUALHYD NHYDin=["S-1-D7"], CINLET=[2.279](cms), NINLET=[1],
2751 * MajNHYD=["S-1-D7J"]
2752 * MinNHYD=["S-1-D7N"]
2753 * TMJSTO=[9999999](cu-m)
2754 *%-----|-----|
2755 *ADD HYD NHYDsum=["S-1-D7S"], NHYDs to add=["S-1-D7J"+"S-1-D7N"]
2756 *%-----|-----|
2757 *ROUTE RESERVOIR NHYDout=["S-1-D7R"],NHYDin=["S-1-D7S"] ,
2758 * RDT=[1](min),
2759 * TABLE of ( OUTFLOW-STORAGE ) values
2760 * (cms) - (ha-m)
2761 * [ 0.0 , 0.0 ]
2762 * [ 0.0243, 0.0810 ]
2763 * [ -1 , -1 ] (max twenty pts)
2764 * NHYDovf=["S-1-D8Rovf"]
2765 *%-----|-----|
2766 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2767 *CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1](min), AREA=[5.27](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2768 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2769 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2770 * LGI=[187.439](m), MNI=[0.013], SCI=[0](min),
2771 * Continuous simulation parameters:
2772 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
2773 * SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2774 * InterEventTime=[12](hrs), END=-1
2775 *%-----|-----|
2776 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha),
2777 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2778 * N=[3], TP=[1.10]hrs,
2779 * Continuous simulation parameters:
2780 * IaREcper=[4](hrs),
2781 * SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2782 * InterEventTime=[12](hrs)
2783 * Baseflow simulation parameters:
2784 * BaseFlowOption=[1] ,
2785 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2786 * VHydCond=[0.055](mm/hr), END=-1
2787 *%-----|-----|
2788 *COMPUTE DUALHYD NHYDin=["S-1-D8"], CINLET=[2.279](cms), NINLET=[1],
2789 * MajNHYD=["S-1-D8J"]
2790 * MinNHYD=["S-1-D8N"]
2791 * TMJSTO=[9999999](cu-m)
2792 *%-----|-----|
2793 *ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2794 *%-----|-----|

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2795 *ADD HYD NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Post"]
2796 *%-----|-----|
2797 *COMPUTE DUALHYD NHYDin=["S-1-D"], CINLET=[11.616](cms), NINLET=[1],
2798 * MajNHYD=["S-1-D-MJ"]
2799 * MinNHYD=["S-1-D-MN"]
2800 * TMJSTO=[5974](cu-m)
2801 *%-----|-----|
2802 *ADD HYD NHYDsum=["S-1-DEV"], NHYDs to add=["S-1-D-MJ"+"S-1-D-MN"]
2803 *%-----|-----|
2804 *ROUTE RESERVOIR NHYDout=["S-1-D8R"] ,NHYDin=["S-1-D8S"] ,
2805 * RDT=[1](min),
2806 * TABLE of ( OUTFLOW-STORAGE ) values
2807 * (cms) - (ha-m)
2808 * [ 0.0 , 0.0 ]
2809 * [ 0.0630, 0.2102 ]
2810 * [ -1 , -1 ] (max twenty pts)
2811 * NHYDovf=["S-1-D8Rovf"]
2812 *%-----|-----|
2813 *%-----|-----|
2814 * -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
2815 *# Catchment W_CLAR
2816 *# - To West Clarke Drain (south of the Jock)
2817 *# - Subdivision with 43% imp. as per Barrhaven South MSS
2818 *# - 2020-11-30 update CLARKE Tributary Drainage Area to = 121 ha based on
P598(04)-11
2819 *# - 2020-11-30 split CLARKE Drainage Area to MAJOR and ALL
2820 *#*****
2821 CONTINUOUS STANDHYD NHYD=["W_CLAR_MJ"], DT=[1]min, AREA=[1.772](ha),
2822 XIMP=[0.46], TIMP=[0.59], DWF=[0](cms), LOSS=[2],
2823 SCS curve number CN=[77],
2824 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
2825 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2826 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
2827 LGI=[109](m), MNI=[0.013], SCI=[0](min),
2828 Continuous simulation parameters:
2829 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
2830 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2831 InterEventTime=[18](hrs), END=-1
2832 *%-----|-----|
2833 *COMPUTE DUALHYD NHYDin=["W_CLAR_MJ"], CINLET=[0.213](cms), NINLET=[1],
2834 * MajNHYD=["W_CLAR_MJj"]
2835 * MinNHYD=["W_CLAR_MJn"]
2836 * TMJSTO=[0.1](cu-m)
2837 *%-----|-----|
2838 *# 5-Year + 12% Capture
2839 ROUTE RESERVOIR NHYDout=["W_CLAR_MJn"] ,NHYDin=["W_CLAR_MJ"] ,
2840 RDT=[1](min),
2841 TABLE of ( OUTFLOW-STORAGE ) values
2842 (cms) - (ha-m)
2843 [ 0.0 , 0.0 ]
2844 [ 0.213 , 0.0001 ]
2845 [ -1 , -1 ] (max twenty pts)
2846 NHYDovf=["W_CLAR_MJj"] ,
2847 *%-----|-----|
2848 * -Clarke_All area from P 598(04)-11 = 120.207 ha, change to 127.298 ha based on
GIS measurements,
2849 * -JFSA, 2021-01-19 update W_CLAR_ALL to (121.17-1.772=119.398) ha based on GIS
measurements W_CLAR is 121.17 ha and W_CLAR_MJ is 1.772 ha
2850 CONTINUOUS STANDHYD NHYD=["W_CLAR_ALL"], DT=[1]min, AREA=[119.398](ha),
2851 XIMP=[0.60], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2852 SCS curve number CN=[77],
2853 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
2854 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2855 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
2856 LGI=[892.18](m), MNI=[0.013], SCI=[0](min),

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2857 Continuous simulation parameters:
2858 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2859 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2860 InterEventTime=[18](hrs), END=-1
2861 *%-----|-----|
2862 ADD HYD NHYDsum=["W_CLAR"], NHYDs to add=["W_CLAR_ALL"+"W_CLAR_MJj"]
2863 *%-----|-----|
2864 SAVE HYD NHYD=["W_CLAR"], # OF PCYCLES=[-1], ICASEsh=[1]
2865 HYD_COMMENT=["Total Flows to West Clarke"]
2866 *#*****
2867 *# West Clarke Pond 2
2868 *# - Rating curve obtained from Barrhaven South MSS modeling
2869 *# - Tributary Drainage Area to MSS Pond 2 = 241 ha
2870 *#*****
2871 ROUTE RESERVOIR NHYDout=["MS_P2"], NHYDin=["W_CLAR"],
2872 RDT=[1](min),
2873 TABLE of ( OUTFLOW-STORAGE ) values
2874 (cms) - (ha-m)
2875 [ 0.0 , 0.0 ]
2876 [ 0.128 , 0.161 ]
2877 [ 0.138 , 0.409 ]
2878 [ 0.148 , 0.68 ]
2879 [ 0.227 , 0.931 ]
2880 [ 0.354 , 1.223 ]
2881 [ 0.505 , 1.52 ]
2882 [ 0.666 , 1.821 ]
2883 [ 0.831 , 2.123 ]
2884 [ 0.995 , 2.434 ]
2885 [ 1.069 , 2.583 ]
2886 [ 1.51 , 2.647 ]
2887 [ 4.904 , 2.861 ]
2888 [ 13.048 , 3.188 ]
2889 [ 23.745 , 3.523 ]
2890 [ 36.474 , 3.871 ]
2891 [ 45.938 , 4.127 ]
2892 [ 61.652 , 4.539 ]
2893 [ -1 , -1 ] (max twenty pts)
2894 NHYDovf=["P2-OVF"]
2895 *%-----|-----|
2896 *#*****
2897 * -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2898 *CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2899 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2900 * N=[3], TP=[1.10]hrs,
2901 * Continuous simulation parameters:
2902 * IaRECper=[4](hrs),
2903 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2904 * InterEventTime=[12](hrs)
2905 * Baseflow simulation parameters:
2906 * BaseFlowOption=[1] ,
2907 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2908 * VHydCond=[0.055](mm/hr), END=-1
2909 *%-----|-----|
2910 ADD HYD NHYDsum=["SN_CE"], NHYDs to
add=["N_CE"+"S-1-D4"+"S-1-D5"+"MS_P2"+"P2-OVF"]
2911 *%-----|-----|
2912 SAVE HYD NHYD=["SN_CE"], # OF PCYCLES=[-1], ICASEsh=[1]
2913 HYD_COMMENT=["Total Flows before Station 5737 on Jock River"]
2914 *%-----|-----|
2915 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5737
2916 *# 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted from the
HEC-RAS model
T:\PROJ\1474-16\Design\20201026-QuantityControlAnalysis\HEC-RAS\JockLidar2005
2917 *# JFSA 2021-03-02 change the slope to 0.0175% instead of 0.02593 to stabilize the model

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2918 ROUTE CHANNEL      NHYDout=["5737"] ,NHYDin=["SN_CE" ] ,
2919                      RDT=[1](min),
2920                      CHLGTH=[270](m),   CHSLOPE=[0.0175](%),
2921                      FPSLOPE=[0.0175](%),
2922                      SECNUM=[1.0],      NSEG=[3]
2923                      ( SEGROUGH, SEGDIST (m))=
2924                      [0.050,-24.04
2925                      -0.035,23.92
2926                      0.050,1130.8] NSEG times
2927                      ( DISTANCE (m), ELEVATION (m))=
2928                      [-1060.52, 94 ]
2929                      [-268.6, 91.5 ]
2930                      [-259.43, 91.5 ]
2931                      [-179.48, 91.5 ]
2932                      [-67.9, 91.5 ]
2933                      [-59.21, 91.5 ]
2934                      [-33.19, 91 ]
2935                      [-26.08, 90.5 ]
2936                      [-24.04, 90 ]
2937                      [-13.14, 86.77 ]
2938                      [0, 85 ]
2939                      [14.68, 86.74 ]
2940                      [23.92, 90 ]
2941                      [25.78, 90.5 ]
2942                      [31.91, 91 ]
2943                      [91.95, 91.5 ]
2944                      [772.15, 92 ]
2945                      [961.49, 92.5 ]
2946                      [1044.69, 93 ]
2947                      [1130.8, 95 ]
2948 *%-----|-----|
2949 ADD HYD      NHYDsum=["5002"], NHYDs to add=["5737"+"S-1-D1"+"S-1-D6"+"S-1-D7"]
2950 *%-----|-----|
2951 SAVE HYD     NHYD=["5002"], # OF PCYCLES=[-1], ICASEsh=[1]
2952             HYD_COMMENT=["Total Flows before Station 5002 on Jock River"]
2953 *%-----|-----|
2954 *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
2955 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002
2956 *# JFSA 2021-02-19 Change the slope from 0.01 % (as per Stantec Report 2007) to 0.0255
% so the model will be more stable and give reasonable results. It is justifiable as
ROUTE CHANNELs aren't well suited to really flat slopes.
2957 *# JFSA 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one
with 825 m length so the model will be more stable
2958 *# JFSA 2021-02-26 change the length of 5002 route channel from 825 m to 736 m. That is
because of adding station 5737 between station 6016 and station 5002. Then the length
from station 5737 to station 5002 is 736 m. Change the slope from 0.0255 % to 0.09511 %
2959 *
2960 ROUTE CHANNEL      NHYDout=["N_WCa"] ,NHYDin=["5002"] ,
2961                      RDT=[1](min),
2962                      CHLGTH=[245.33333](m),   CHSLOPE=[0.09511](%),
2963                      FPSLOPE=[0.09511](%),
2964                      SECNUM=[1.0],      NSEG=[3]
2965                      ( SEGROUGH, SEGDIST (m))=
2966                      [0.050,-37.5
2967                      -0.035,37.50
2968                      0.050,157.05] NSEG times
2969                      ( DISTANCE (m), ELEVATION (m))=
2970                      [-601.81, 91.5]
2971                      [-37.50, 90.00]
2972                      [-19.61, 87.04]
2973                      [0.00, 85.70]
2974                      [14.87, 86.93]
2975                      [37.50, 90.00]
2976                      [38.54, 90.50]
2977                      [42.23, 91]
2978                      [157.05,91.50]

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2979 * [161.44, 91.50]
2980 * [236.48, 93.00]
2981 * [385.47, 92.50]
2982 * [390.78, 92.50]
2983 *%-----|-----
2984 ROUTE CHANNEL NHYDout=["N_WCb"] ,NHYDin=["N_WCa"] ,
2985 RDT=[1](min),
2986 CHLGTH=[245.33333](m), CHSLOPE=[0.09511](%),
2987 FPSLOPE=[0.09511](%),
2988 SECNUM=[1.0], NSEG=[3]
2989 ( SEGROUGH, SEGDIST (m))=
2990 [0.050,-37.5
2991 -0.035,37.50
2992 0.050,157.05] NSEG times
2993 ( DISTANCE (m), ELEVATION (m))=
2994 [-601.81, 91.5]
2995 [-37.50, 90.00]
2996 [-19.61, 87.04]
2997 [0.00, 85.70]
2998 [14.87, 86.93]
2999 [37.50, 90.00]
3000 [38.54, 90.50]
3001 [42.23, 91]
3002 [157.05,91.50]
3003 *%-----|-----
3004 ROUTE CHANNEL NHYDout=["N_WC"] ,NHYDin=["N_WCb"] ,
3005 RDT=[1](min),
3006 CHLGTH=[245.33333](m), CHSLOPE=[0.09511](%),
3007 FPSLOPE=[0.09511](%),
3008 SECNUM=[1.0], NSEG=[3]
3009 ( SEGROUGH, SEGDIST (m))=
3010 [0.050,-37.5
3011 -0.035,37.50
3012 0.050,157.05] NSEG times
3013 ( DISTANCE (m), ELEVATION (m))=
3014 [-601.81, 91.5]
3015 [-37.50, 90.00]
3016 [-19.61, 87.04]
3017 [0.00, 85.70]
3018 [14.87, 86.93]
3019 [37.50, 90.00]
3020 [38.54, 90.50]
3021 [42.23, 91]
3022 [157.05,91.50]
3023 *#*****
3024 * -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
3025 *ADD HYD NHYDsum=["SN_WC"], NHYDs to
add=["MS_P2"+"P2-OVF"+"N_WC"+"W_CLAR_UNDE"]
3026 *%-----|-----
3027 *SAVE HYD NHYD=["SN_WC"], # OF PCYCLES=[-1], ICASEsh=[1]
3028 * HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
3029 *%-----|-----
3030 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
3031 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
3032 *#
3033 ROUTE CHANNEL NHYDout=["N_KB"] ,NHYDin=["N_WC"] ,
3034 RDT=[1](min),
3035 CHLGTH=[1020](m), CHSLOPE=[0.0498](%),
3036 FPSLOPE=[0.0498](%),
3037 SECNUM=[1.0], NSEG=[3]
3038 ( SEGROUGH, SEGDIST (m))=
3039 [0.050,-23.63
3040 -0.035,23.63
3041 0.050,728.3] NSEG times
3042 ( DISTANCE (m), ELEVATION (m))=

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3043 [-1082.01,94]
3044 [-1028.17,92.5]
3045 [-992.3,93.5]
3046 [-279.34,90]
3047 [-23.63,90]
3048 [-13.45,87.13]
3049 [-0.07,86.24]
3050 [10.54,87.15]
3051 [23.63,90]
3052 [24.86,90.5]
3053 [26.72,91]
3054 [45.07,91.5]
3055 [128.17,91.5]
3056 [270.7,92.5]
3057 [728.3,95]
3058 *%-----|-----|
3059 *#*****|
3060 *# Catchment KEN_BU
3061 *# - To Kennedy-Burnett SWM Facility
3062 *# - Outlets to Fraser-Clarke drain (north of the Jock)
3063 *# - Medium density residential subdivision
3064 * - Add Kennedy Burnett model (Convert PCSWMM from NOVATECH June, 2020 to SWMHYMO)
3065 *#*****|
3066 *CONTINUOUS STANDHYD NHYD=["KEN_BU"], DT=[1]min, AREA=[281](ha),
3067 * XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
3068 * SCS curve number CN=[71],
3069 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3070 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3071 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3072 * LGI=[1369](m), MNI=[0.013], SCI=[0](min),
3073 * Continuous simulation parameters:
3074 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
3075 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3076 * InterEventTime=[18](hrs), END=-1
3077 *%-----|-----|
3078 *#*****|
3079 *# Existing Kennedy-Burnett SWM Facility
3080 *# - Rating curve obtained from URTKBP
3081 *# - Tributary Drainage Area to Pond = 160 ha
3082 *#*****|
3083 *ROUTE RESERVOIR NHYDout=["KEN_P"], NHYDin=["KEN_BU"],
3084 * RDT=[1](min),
3085 * TABLE of ( OUTFLOW-STORAGE ) values
3086 * (cms) - (ha-m)
3087 * [ 0.0 , 0.0 ]
3088 * [ 0.13 , 0.26 ]
3089 * [ 0.43 , 0.56 ]
3090 * [ 0.67 , 0.90 ]
3091 * [ 0.86 , 1.32 ]
3092 * [ 1.01 , 1.79 ]
3093 * [ 1.15 , 2.33 ]
3094 * [ -1 , -1 ] (max twenty pts)
3095 * NHYDovf=["KEN-OV"]
3096 *%-----|-----|
3097 * -JFSA, 2021-01-19 update all KEN_BU areas based on GIS measurements
3098 CONTINUOUS STANDHYD NHYD=["KB-01A"], DT=[1]min, AREA=[40.82](ha), XIMP=[0.097],
3099 TIMP=[0.4], DWF=[0](cms), LOSS=[1]:
3099 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3100 F=[0.00](mm),
3100 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
3101 MNP=[0.250], SCP=[0](min),
3101 Impervious areas: IAimp=[0.785](mm), SLPI=[0.5](%),
3102 LGI=[521.664](m), MNI=[0.013], SCI=[0](min),
3102 Continuous simulation parameters:
3103 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
3103 END=-1

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3104 *%-----|-----|
3105 COMPUTE DUALHYD NHYDin=["KB-01A"], CINLET=[3.6](cms), NINLET=[1],
3106 MajNHYD=["KB-01A-MJ"]
3107 MinNHYD=["KB-01A-MN"]
3108 TMJSTO=[4995](cu-m)
3109 *%-----|-----|
3110 ADD HYD NHYDsum=["KB-01A-S"], NHYDs to add=["KB-01A-MJ"+"KB-01A-MN"]
3111 *%-----|-----|
3112 CONTINUOUS STANDHYD NHYD=["KB-01B"], DT=[1]min, AREA=[31.1](ha), XIMP=[0.1875],
TIMP=[0.375], DWF=[0](cms), LOSS=[1]:
3113 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3114 Pervious areas: IAper=[4.67](mm), SLPP=[0.42](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3115 Impervious areas: IAimp=[0.785](mm), SLPI=[0.42](%),
LGI=[455.339](m), MNI=[0.013], SCI=[0](min),
3116 Continuous simulation parameters:
3117 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3118 *%-----|-----|
3119 COMPUTE DUALHYD NHYDin=["KB-01B"], CINLET=[1.585](cms), NINLET=[1],
3120 MajNHYD=["KB-01B-MJ"]
3121 MinNHYD=["KB-01B-MN"]
3122 TMJSTO=[6075](cu-m)
3123 *%-----|-----|
3124 ADD HYD NHYDsum=["KB-01B-S"], NHYDs to add=["KB-01B-MJ"+"KB-01B-MN"]
3125 *%-----|-----|
3126 CONTINUOUS STANDHYD NHYD=["KB-01C"], DT=[1]min, AREA=[13.78](ha), XIMP=[0.2045],
TIMP=[0.409], DWF=[0](cms), LOSS=[1]:
3127 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3128 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3129 Impervious areas: IAimp=[0.785](mm), SLPI=[0.5](%),
LGI=[303.095](m), MNI=[0.013], SCI=[0](min),
3130 Continuous simulation parameters:
3131 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3132 *%-----|-----|
3133 COMPUTE DUALHYD NHYDin=["KB-01C"], CINLET=[1.35](cms), NINLET=[1],
3134 MajNHYD=["KB-01C-MJ"]
3135 MinNHYD=["KB-01C-MN"]
3136 TMJSTO=[1880](cu-m)
3137 *%-----|-----|
3138 ADD HYD NHYDsum=["KB-01C-S"], NHYDs to add=["KB-01C-MJ"+"KB-01C-MN"]
3139 *%-----|-----|
3140 CONTINUOUS STANDHYD NHYD=["KB-03"], DT=[1]min, AREA=[84.78](ha), XIMP=[0.197],
TIMP=[0.394], DWF=[0](cms), LOSS=[1]:
3141 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3142 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3143 Impervious areas: IAimp=[0.785](mm), SLPI=[0.63](%),
LGI=[751.798](m), MNI=[0.013], SCI=[0](min),
3144 Continuous simulation parameters:
3145 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3146 *%-----|-----|
3147 COMPUTE DUALHYD NHYDin=["KB-03"], CINLET=[5.27](cms), NINLET=[1],
3148 MajNHYD=["KB-03-MJ"]
3149 MinNHYD=["KB-03-MN"]
3150 TMJSTO=[15500](cu-m)
3151 *%-----|-----|
3152 ADD HYD NHYDsum=["KB-03-S"], NHYDs to add=["KB-03-MJ"+"KB-03-MN"]
3153 *%-----|-----|
3154 CONTINUOUS STANDHYD NHYD=["KB-04"], DT=[1]min, AREA=[6.95](ha), XIMP=[0.85],

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TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
3155 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3156 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3157 Impervious areas: IAimp=[0.942](mm), SLPI=[0.5](%),
LGI=[215.252](m), MNI=[0.013], SCI=[0](min),
3158 Continuous simulation parameters:
3159 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3160 *%-----|-----|
3161 COMPUTE DUALHYD NHYDin=["KB-04"], CINLET=[0.503](cms), NINLET=[1],
3162 MaJNHYD=["KB-04-MJ"]
3163 MinNHYD=["KB-04-MN"]
3164 TMJSTO=[1972](cu-m)
3165 *%-----|-----|
3166 ADD HYD NHYDsum=["KB-04-S"], NHYDs to add=["KB-04-MJ"+"KB-04-MN"]
3167 *%-----|-----|
3168 CONTINUOUS STANDHYD NHYD=["KB-05"], DT=[1]min, AREA=[5.19](ha), XIMP=[0.93],
TIMP=[0.93], DWF=[0](cms), LOSS=[1]:
3169 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3170 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3171 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[186.011](m), MNI=[0.013], SCI=[0](min),
3172 Continuous simulation parameters:
3173 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3174 *%-----|-----|
3175 *%-----|-----|
3176 CONTINUOUS STANDHYD NHYD=["KB-06"], DT=[1]min, AREA=[12.93](ha), XIMP=[0.873],
TIMP=[0.873], DWF=[0](cms), LOSS=[1]:
3177 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3178 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3179 Impervious areas: IAimp=[0.942](mm), SLPI=[4.75](%),
LGI=[293.598](m), MNI=[0.013], SCI=[0](min),
3180 Continuous simulation parameters:
3181 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3182 *%-----|-----|
3183 COMPUTE DUALHYD NHYDin=["KB-06"], CINLET=[2.262](cms), NINLET=[1],
3184 MaJNHYD=["KB-06-MJ"]
3185 MinNHYD=["KB-06-MN"]
3186 TMJSTO=[1950](cu-m)
3187 *%-----|-----|
3188 ADD HYD NHYDsum=["KB-06-S"], NHYDs to add=["KB-06-MJ"+"KB-06-MN"]
3189 *%-----|-----|
3190 CONTINUOUS STANDHYD NHYD=["KB-11"], DT=[1]min, AREA=[4.03](ha), XIMP=[0.675],
TIMP=[0.675], DWF=[0](cms), LOSS=[1]:
3191 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3192 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3193 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[163.911](m), MNI=[0.013], SCI=[0](min),
3194 Continuous simulation parameters:
3195 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3196 *%-----|-----|
3197 COMPUTE DUALHYD NHYDin=["KB-11"], CINLET=[0.5773](cms), NINLET=[1],
3198 MaJNHYD=["KB-11-MJ"]
3199 MinNHYD=["KB-11-MN"]
3200 TMJSTO=[597](cu-m)

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3201  *%-----|-----|
3202  ADD HYD      NHYDsum=["KB-11-S"], NHYDs to add=["KB-11-MJ"+"KB-11-MN"]
3203  *%-----|-----|
3204  CONTINUOUS STANDHYD NHYD=["S1"], DT=[1]min, AREA=[4.99](ha), XIMP=[0.93], TIMP=[0.93],
DWF=[0](cms), LOSS=[1]:
3205      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3206      Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3207      Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
LGI=[182.392](m), MNI=[0.013], SCI=[0](min),
3208      Continuous simulation parameters:
3209      IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3210  *%-----|-----|
3211  CONTINUOUS STANDHYD NHYD=["KB-15"], DT=[1]min, AREA=[2.15](ha), XIMP=[0.79],
TIMP=[0.79], DWF=[0](cms), LOSS=[1]:
3212      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3213      Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3214      Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[119.722](m), MNI=[0.013], SCI=[0](min),
3215      Continuous simulation parameters:
3216      IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3217  *%-----|-----|
3218  *%-----|-----|
3219  ADD HYD      NHYDsum=["KB-P1"], NHYDs to
add=["KB-01A-S"+"KB-01B-S"+"KB-01C-S"+"KB-03-S"+"KB-04-S"+"KB-05"+"KB-06-S"+"KB-11-S"+"KB
-15"+"S1"]
3220  *%-----|-----|
3221  ROUTE RESERVOIR NHYDout=["KB-P1R"], NHYDin=["KB-P1"],
3222      RDT=[1](min),
3223      TABLE of ( OUTFLOW-STORAGE ) values
3224      (cms) - (ha-m)
3225      [ 0.0 , 0.0 ]
3226      [0.076,0.003]
3227      [0.088,0.006]
3228      [0.136,0.011]
3229      [0.301,0.017]
3230      [0.454,0.027]
3231      [0.631,0.041]
3232      [1.173,0.068]
3233      [1.91,0.111]
3234      [4.847,0.231]
3235      [9.813,0.436]
3236      [12.134,0.617]
3237      [12.438,0.732]
3238      [12.424,0.811]
3239      [12.425,0.894]
3240      [ -1 , -1 ] (max twenty pts)
3241      NHYDovf=["KB-P1ovf"]
3242  *%-----|-----|
3243  ADD HYD      NHYDsum=["KB-Pond1"], NHYDs to add=["KB-P1R"+"KB-P1ovf"]
3244  *%-----|-----|
3245  SAVE HYD     NHYD=["KB-Pond1"], # OF PCYCLES=[-1], ICASEsh=[1]
3246      HYD_COMMENT=["Total Flows at KB first pond"]
3247  *%-----|-----|
3248  CONTINUOUS STANDHYD NHYD=["KB-07"], DT=[1]min, AREA=[10.86](ha), XIMP=[0.86],
TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3249      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3250      Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3251      Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),

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3252 LGI=[269.072](m), MNI=[0.013], SCI=[0](min),
3253 Continuous simulation parameters:
3254 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3255 END=-1
3254 *%-----|-----|
3255 COMPUTE DUALHYD NHYDin=["KB-07"], CINLET=[2.094](cms), NINLET=[1],
3256 MajNHYD=["KB-07-MJ"]
3257 MinNHYD=["KB-07-MN"]
3258 TMJSTO=[1378](cu-m)
3259 *%-----|-----|
3260 ADD HYD NHYDsum=["KB-07-S"], NHYDs to add=["KB-07-MJ"+"KB-07-MN"]
3261 *%-----|-----|
3262 CONTINUOUS STANDHYD NHYD=["KB-08"], DT=[1]min, AREA=[6.61](ha), XIMP=[0.64],
3263 TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3264 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3265 F=[0.00](mm),
3266 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3267 MNP=[0.250], SCP=[0](min),
3268 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
3269 LGI=[209.921](m), MNI=[0.013], SCI=[0](min),
3270 Continuous simulation parameters:
3271 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3272 END=-1
3273 *%-----|-----|
3274 COMPUTE DUALHYD NHYDin=["KB-08"], CINLET=[1.058](cms), NINLET=[1],
3275 MajNHYD=["KB-08-MJ"]
3276 MinNHYD=["KB-08-MN"]
3277 TMJSTO=[787](cu-m)
3278 *%-----|-----|
3279 ADD HYD NHYDsum=["KB-08-S"], NHYDs to add=["KB-08-MJ"+"KB-08-MN"]
3280 *%-----|-----|
3281 CONTINUOUS STANDHYD NHYD=["KB-09"], DT=[1]min, AREA=[2.6](ha), XIMP=[0.86],
3282 TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3283 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3284 F=[0.00](mm),
3285 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3286 MNP=[0.250], SCP=[0](min),
3287 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
3288 LGI=[131.656](m), MNI=[0.013], SCI=[0](min),
3289 Continuous simulation parameters:
3290 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3291 END=-1
3292 *%-----|-----|
3293 CONTINUOUS STANDHYD NHYD=["KB-10_1"], DT=[1]min, AREA=[2.37](ha), XIMP=[0.86],
3294 TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3295 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3296 F=[0.00](mm),
3297 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3298 MNP=[0.250], SCP=[0](min),
3299 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
3300 LGI=[125.698](m), MNI=[0.013], SCI=[0](min),
3301 Continuous simulation parameters:
3302 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3303 END=-1
3304 *%-----|-----|
3305 CONTINUOUS STANDHYD NHYD=["KB-10_2"], DT=[1]min, AREA=[1.14](ha), XIMP=[0.86],
3306 TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3307 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3308 F=[0.00](mm),
3309 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3310 MNP=[0.250], SCP=[0](min),
3311 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%), LGI=[87.178](m),
3312 MNI=[0.013], SCI=[0](min),
3313 Continuous simulation parameters:
3314 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),

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END=-1
3297 *%-----|-----|
3298 *%-----|-----|
3299 CONTINUOUS STANDHYD NHYD=["KB-12"], DT=[1]min, AREA=[4.86](ha), XIMP=[0.79],
TIMP=[0.79], DWF=[0](cms), LOSS=[1]:
3300 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3301 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3302 Impervious areas: IAimp=[1.099](mm), SLPI=[2.0](%),
LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
3303 Continuous simulation parameters:
3304 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3305 *%-----|-----|
3306 COMPUTE DUALHYD NHYDin=["KB-12"], CINLET=[0.8665](cms), NINLET=[1],
3307 MajNHYD=["KB-12-MJ"]
3308 MinNHYD=["KB-12-MN"]
3309 TMJSTO=[632](cu-m)
3310 *%-----|-----|
3311 ADD HYD NHYDsum=["KB-12-S"], NHYDs to add=["KB-12-MJ"+"KB-12-MN"]
3312 *%-----|-----|
3313 CONTINUOUS STANDHYD NHYD=["KB-13"], DT=[1]min, AREA=[10.19](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3314 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3315 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3316 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[260.640](m), MNI=[0.013], SCI=[0](min),
3317 Continuous simulation parameters:
3318 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3319 *%-----|-----|
3320 COMPUTE DUALHYD NHYDin=["KB-13"], CINLET=[1.722](cms), NINLET=[1],
3321 MajNHYD=["KB-13-MJ"]
3322 MinNHYD=["KB-13-MN"]
3323 TMJSTO=[1077](cu-m)
3324 *%-----|-----|
3325 ADD HYD NHYDsum=["KB-13-S"], NHYDs to add=["KB-13-MJ"+"KB-13-MN"]
3326 *%-----|-----|
3327 CONTINUOUS STANDHYD NHYD=["KB-14"], DT=[1]min, AREA=[5.47](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3328 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3329 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3330 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[190.962](m), MNI=[0.013], SCI=[0](min),
3331 Continuous simulation parameters:
3332 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3333 *%-----|-----|
3334 COMPUTE DUALHYD NHYDin=["KB-14"], CINLET=[0.8734](cms), NINLET=[1],
3335 MajNHYD=["KB-14-MJ"]
3336 MinNHYD=["KB-14-MN"]
3337 TMJSTO=[631](cu-m)
3338 *%-----|-----|
3339 ADD HYD NHYDsum=["KB-14-S"], NHYDs to add=["KB-14-MJ"+"KB-14-MN"]
3340 *%-----|-----|
3341 *%-----|-----|
3342 CONTINUOUS STANDHYD NHYD=["KB-16_2"], DT=[1]min, AREA=[3.42](ha), XIMP=[0.71],
TIMP=[0.71], DWF=[0](cms), LOSS=[1]:
3343 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3344 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),

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3345 MNP=[0.250], SCP=[0](min),
Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
3346 LGI=[150.997](m), MNI=[0.013], SCI=[0](min),
3347 Continuous simulation parameters:
IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3348 *%-----|-----|
3349 ADD HYD NHYDsum=["KB-P2"], NHYDs to
add=["KB-Pond1"+"KB-07-S"+"KB-08-S"+"KB-09"+"KB-10_1"+"KB-10_2"+"KB-12-S"+"KB-13-S"+"KB-1
4-S"+"KB-16_2"]
3350 *%-----|-----|
3351 ROUTE RESERVOIR NHYDout=["KB-P2R"], NHYDin=["KB-P2"],
3352 RDT=[1](min),
3353 TABLE of ( OUTFLOW-STORAGE ) values
3354 (cms) - (ha-m)
3355 [ 0.0 , 0.0 ]
3356 [0.053,0.005]
3357 [0.132,0.009]
3358 [0.269,0.014]
3359 [0.455,0.023]
3360 [0.699,0.037]
3361 [0.947,0.056]
3362 [1.853,0.09]
3363 [2.712,0.146]
3364 [6.626,0.287]
3365 [11.228,0.515]
3366 [14.885,0.738]
3367 [16.473,0.893]
3368 [17.311,0.998]
3369 [17.633,1.063]
3370 [17.634,1.112]
3371 [ -1 , -1 ] (max twenty pts)
3372 NHYDovf=["KB-P2ovf"]
3373 *%-----|-----|
3374 ADD HYD NHYDsum=["KB-Pond2"], NHYDs to add=["KB-P2R"+"KB-P2ovf"]
3375 *%-----|-----|
3376 SAVE HYD NHYD=["KB-Pond2"], # OF PCYCLES=[-1], ICASEsh=[1]
3377 HYD_COMMENT=["Total Flows at KB second pond"]
3378 *%-----|-----|
3379 CONTINUOUS STANDHYD NHYD=["KB-16_1"], DT=[1]min, AREA=[2.8](ha), XIMP=[0.75],
TIMP=[0.75], DWF=[0](cms), LOSS=[1]:
3380 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3381 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3382 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[136.626](m), MNI=[0.013], SCI=[0](min),
3383 Continuous simulation parameters:
3384 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3385 *%-----|-----|
3386 ADD HYD NHYDsum=["KB-P3"], NHYDs to add=["KB-Pond2"+"KB-16_1"]
3387 *%-----|-----|
3388 *%-----|-----|
3389 * One inflow node from pond 3 is added to the model (ROUTE RESERVOIR)
3390 * Another inflow node from right side of pond 3 is not added to the model
3391 ROUTE RESERVOIR NHYDout=["KB-P3R"], NHYDin=["KB-P3"],
3392 RDT=[1](min),
3393 TABLE of ( OUTFLOW-STORAGE ) values
3394 (cms) - (ha-m)
3395 [ 0.0 , 0.0 ]
3396 [0.051,0.002]
3397 [0.048,0.003]
3398 [0.057,0.029]
3399 [0.089,0.045]
3400 [0.133,0.069]

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3401 [0.199,0.106]
3402 [0.321,0.172]
3403 [1.029,0.306]
3404 [4.036,0.527]
3405 [8.332,0.761]
3406 [11.727,0.941]
3407 [14.125,1.067]
3408 [15.675,1.149]
3409 [16.555,1.196]
3410 [16.911,1.214]
3411 [ -1 , -1 ] (max twenty pts)
3412 NHYDovf=["KB-P3ovf"]
3413 *%-----|-----|
3414 ADD HYD NHYDsum=["KB-Pond3"], NHYDs to add=["KB-P3R"+"KB-P3ovf"]
3415 *%-----|-----|
3416 SAVE HYD NHYD=["KB-Pond3"], # OF PCYCLES=[-1], ICASEsh=[1]
3417 HYD_COMMENT=["Total Flows at KB third pond"]
3418 *%-----|-----|
3419 *#*****|
3420 *# EXISTING / PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM
3421 Modeling Approach, NOVATECH Report June, 2020)
3422 *# - TO FRASER-CLARKE DRAIN
3423 *#*****|
3424 CONTINUOUS STANDHYD NHYD=["FC-01"], DT=[1]min, AREA=[8.03](ha), XIMP=[0.47],
3425 TIMP=[0.47], DWF=[0](cms), LOSS=[1]:
3426 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3427 F=[0.00](mm),
3428 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3429 MNP=[0.250], SCP=[0](min),
3430 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
3431 LGI=[231.373](m), MNI=[0.013], SCI=[0](min),
3432 Continuous simulation parameters:
3433 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
3434 END=-1
3435 *%-----|-----|
3436 COMPUTE DUALHYD NHYDin=["FC-01"], CINLET=[0.756](cms), NINLET=[1],
3437 MaJNHYD=["FC-01-MJ"]
3438 MinNHYD=["FC-01-MN"]
3439 TMJSTO=[714](cu-m)
3440 *%-----|-----|
3441 ADD HYD NHYDsum=["FC-01-S"], NHYDs to add=["FC-01-MJ"+"FC-01-MN"]
3442 *%-----|-----|
3443 CONTINUOUS STANDHYD NHYD=["FC-02"], DT=[1]min, AREA=[16.05](ha), XIMP=[0.93],
3444 TIMP=[0.93], DWF=[0](cms), LOSS=[1]:
3445 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3446 F=[0.00](mm),
3447 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3448 MNP=[0.250], SCP=[0](min),
3449 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
3450 LGI=[327.109](m), MNI=[0.013], SCI=[0](min),
3451 Continuous simulation parameters:
3452 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
3453 END=-1
3454 *%-----|-----|
3455 COMPUTE DUALHYD NHYDin=["FC-02"], CINLET=[1.159](cms), NINLET=[1],
3456 MaJNHYD=["FC-02-MJ"]
3457 MinNHYD=["FC-02-MN"]
3458 TMJSTO=[2385](cu-m)
3459 *%-----|-----|
3460 ADD HYD NHYDsum=["FC-02-S"], NHYDs to add=["FC-02-MJ"+"FC-02-MN"]
3461 *%-----|-----|
3462 CONTINUOUS STANDHYD NHYD=["FC-03"], DT=[1]min, AREA=[7.37](ha), XIMP=[0.64],
3463 TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3464 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3465 F=[0.00](mm),
3466 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),

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3454 MNP=[0.250], SCP=[0](min),
Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[221.660](m), MNI=[0.013], SCI=[0](min),
3455 Continuous simulation parameters:
3456 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3457 *%-----|-----|
3458 COMPUTE DUALHYD NHYDin=["FC-03"], CINLET=[0.358](cms), NINLET=[1],
3459 MajNHYD=["FC-03-MJ"]
3460 MinNHYD=["FC-03-MN"]
3461 TMJSTO=[1131](cu-m)
3462 *%-----|-----|
3463 ADD HYD NHYDsum=["FC-03-S"], NHYDs to add=["FC-03-MJ"+"FC-03-MN"]
3464 *%-----|-----|
3465 CONTINUOUS STANDHYD NHYD=["FC-04"], DT=[1]min, AREA=[12.87](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3466 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3467 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3468 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[292.916](m), MNI=[0.013], SCI=[0](min),
3469 Continuous simulation parameters:
3470 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3471 *%-----|-----|
3472 COMPUTE DUALHYD NHYDin=["FC-04"], CINLET=[0.741](cms), NINLET=[1],
3473 MajNHYD=["FC-04-MJ"]
3474 MinNHYD=["FC-04-MN"]
3475 TMJSTO=[1794](cu-m)
3476 *%-----|-----|
3477 ADD HYD NHYDsum=["FC-04-S"], NHYDs to add=["FC-04-MJ"+"FC-04-MN"]
3478 *%-----|-----|
3479 *#*****
3480 *# PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM Modeling
Approach, NOVATECH Report June, 2020)
3481 *# - TO JOCK RIVER
3482 *#*****
3483 CONTINUOUS STANDHYD NHYD=["JR-01"], DT=[1]min, AREA=[8.24](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3484 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3485 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3486 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[234.379](m), MNI=[0.013], SCI=[0](min),
3487 Continuous simulation parameters:
3488 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3489 *%-----|-----|
3490 COMPUTE DUALHYD NHYDin=["JR-01"], CINLET=[0.563](cms), NINLET=[1],
3491 MajNHYD=["JR-01-MJ"]
3492 MinNHYD=["JR-01-MN"]
3493 TMJSTO=[1040](cu-m)
3494 *%-----|-----|
3495 ADD HYD NHYDsum=["JR-01-S"], NHYDs to add=["JR-01-MJ"+"JR-01-MN"]
3496 *%-----|-----|
3497 CONTINUOUS STANDHYD NHYD=["JR-02"], DT=[1]min, AREA=[1.59](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3498 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3499 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3500 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[102.956](m), MNI=[0.013], SCI=[0](min),
3501 Continuous simulation parameters:

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3502 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
      END=-1
3503 *%-----|-----|
3504 COMPUTE DUALHYD NHYDin=["JR-02"], CINLET=[0.153](cms), NINLET=[1],
3505 MajNHYD=["JR-02-MJ"]
3506 MinNHYD=["JR-02-MN"]
3507 TMJSTO=[153](cu-m)
3508 *%-----|-----|
3509 ADD HYD NHYDsum=["JR-02-S"], NHYDs to add=["JR-02-MJ"+"JR-02-MN"]
3510 *%-----|-----|
3511 *#*****|*****|
3512 *# Catchment FRASER
3513 *# - To Fraser-Clarke drain (north of the Jock)
3514 *# - Developed land with assumed 43% imp.
3515 *# - 2020-12-17 Change Fraser area to be 35.1 as measured from QGIS
3516 *# - 2020-12-17 All Fraser is undeveloped (Nashyd)
3517 *#*****|*****|
3518 CONTINUOUS NASHYD NHYD=["FRASER-DRN"], DT=[1]min, AREA=[13.65](ha),
3519 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3520 N=[3], TP=[0.4258]hrs,
3521 Continuous simulation parameters:
3522 IaREcper=[4](hrs),
3523 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3524 InterEventTime=[12](hrs)
3525 Baseflow simulation parameters:
3526 BaseFlowOption=[1],
3527 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3528 VHydCond=[0.055](mm/hr), END=-1
3529 *
3530 CONTINUOUS STANDHYD NHYD=["FRASER-D"], DT=[1]min, AREA=[21.61](ha),
3531 XIMP=[0.585], TIMP=[0.585], DWF=[0](cms), LOSS=[2],
3532 SCS curve number CN=[80],
3533 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3534 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3535 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3536 LGI=[379.561](m), MNI=[0.013], SCI=[0](min),
3537 Continuous simulation parameters:
3538 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3539 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3540 InterEventTime=[18](hrs), END=-1
3541 *%-----|-----|
3542 COMPUTE DUALHYD NHYDin=["FRASER-D"], CINLET=[2.281](cms), NINLET=[1],
3543 MajNHYD=["FRASER-J"]
3544 MinNHYD=["FRASER-N"]
3545 TMJSTO=[9999999](cu-m)
3546 *%-----|-----|
3547 ADD HYD NHYDsum=["FRASER-S"], NHYDs to add=["FRASER-J"+"FRASER-N"]
3548 *%-----|-----|
3549 *ROUTE RESERVOIR NHYDout=["MS_P20"], NHYDin=["FRASER"],
3550 * RDT=[1](min),
3551 * TABLE of ( OUTFLOW-STORAGE ) values
3552 * (cms) - (ha-m)
3553 * [ 0.0 , 0.0 ]
3554 * [ 0.04 , 0.36 ]
3555 * [ -1 , -1 ] (max twenty pts)
3556 * NHYDovf=["P20-OVF"]
3557 *%-----|-----|
3558 ADD HYD NHYDsum=["4241"], NHYDs to
add=["KB-Pond3"+"S-1-B"+"FRASER-DRN"+"FRASER-S"+"N_KB"+"FC-01-S"+"FC-02-S"+"FC-03-S"]
3559 *%-----|-----|
3560 SAVE HYD NHYD=["4241"], # OF PCYCLES=[-1], ICASEsh=[1]
3561 HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
3562 *%-----|-----|
3563 *# Hydrograph from Node Ken-Burnett to station 3633
3564 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4241
3565 *#

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```

3566 ROUTE CHANNEL      NHYDout=["4241-out"],NHYDin=["4241"], RDT=[1](min),
3567 CHLGTH=[294](m),   CHSLOPE=[0.1088](%), FPSLOPE=[0.1088](%),
3568 SECNUM=[1.0],      NSEG=[3]
3569 ( SEGROUGH, SEGDIST (m))=[0.05, -20.12
3570                               -0.035, 45.26
3571                               0.05, 403.84] NSEG times
3572 ( DISTANCE (m), ELEVATION (m))=[]
3573 [-909.72, 95 ]
3574 [-907.09, 94.5 ]
3575 [-904.65, 94 ]
3576 [-902.26, 93.5 ]
3577 [-44.51, 91.5 ]
3578 [-25.1, 91.5 ]
3579 [-20.98, 91 ]
3580 [-20.61, 90.5 ]
3581 [-20.12, 90 ]
3582 [-6.13, 87.26 ]
3583 [17.51, 86.56 ]
3584 [31.37, 87.2 ]
3585 [45.26, 90 ]
3586 [50.41, 90.5 ]
3587 [63.06, 91 ]
3588 [134.5, 91.5 ]
3589 [190.63, 92 ]
3590 [251.98, 92.5 ]
3591 [321.32, 93.5 ]
3592 [403.84, 95 ]
3593 *%-----|-----|
3594 ADD HYD          NHYDsum=["SN_KB"], NHYDs to
add=["4241-out"+"FC-04-S"+"JR-01-S"+"JR-02-S"]
3595 *%-----|-----|
3596 SAVE HYD        NHYD=["SN_KB"], # OF PCYCLES=[-1], ICASEsh=[1]
3597 HYD_COMMENT=["Total Flows before Station 3633"]
3598 *%-----|-----|
3599 *# Hydrograph from Station 3633 to Node Todd
3600 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
3601 *# JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and
change the slope from 0.0498% to 0.24671%. That is because of adding station 4241
between station 4534 and station 3633
3602 *#
3603 ROUTE CHANNEL   NHYDout=["N_TO"], NHYDin=["SN_KB"], RDT=[1](min),
3604 CHLGTH=[608](m), CHSLOPE=[0.24671](%), FPSLOPE=[0.24671](%),
3605 SECNUM=[1.0],   NSEG=[3]
3606 ( SEGROUGH, SEGDIST (m))=[0.05, -23.74
3607                               -0.035, 23.74
3608                               0.05, 26.50] NSEG times
3609 ( DISTANCE (m), ELEVATION (m))=[]
3610 -29.24, 91.0
3611 -27.41, 90.5
3612 -25.64, 90
3613 -23.74, 89.5
3614 -22, 89.26
3615 -20, 88.51
3616 -19, 88.32
3617 -15, 88.1
3618 -10, 88.11
3619 -5, 88.17
3620 0, 88.27
3621 5, 88.19
3622 10, 88.06
3623 15, 88.48
3624 16, 88.7
3625 23.74, 89.5
3626 24.68, 90
3627 25.57, 90.5
3628 26.50, 91.0

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3629 * [-29.24, 91]
3630 * [-27.41, 90.5]
3631 * [-25.64, 90]
3632 * [-23.74, 89.5]
3633 * [-22, 89.26]
3634 * [-20, 88.51]
3635 * [-19, 88.32]
3636 * [-15, 88.1]
3637 * [-10, 88.11]
3638 * [-5, 88.17]
3639 * [0, 88.27]
3640 * [5, 88.19]
3641 * [10, 88.06]
3642 * [15, 88.48]
3643 * [16, 88.7]
3644 * [23.74, 89.5]
3645 * [24.68, 90]
3646 * [25.57, 90.5]
3647 *%-----|-----|
3648 *#*****|*****|
3649 *# Catchment Greenbank
3650 *# - To Greenbank Drain (south of the Jock)
3651 *# - JFSA 2021-01-18 add Greenbank pond as per JFSA, P598(06)-15, June 2016
3652 *# - JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
3653 *#*****|*****|
3654 CONTINUOUS STANDHYD NHYD=["Greenbank"], DT=[1]min, AREA=[36.6](ha),
3655 XIMP=[0.639], TIMP=[0.682], DWF=[0](cms), LOSS=[2],
3656 SCS curve number CN=[77],
3657 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3658 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3659 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3660 LGI=[493.96](m), MNI=[0.013], SCI=[0](min),
3661 Continuous simulation parameters:
3662 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3663 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3664 InterEventTime=[18](hrs), END=-1
3665 *%-----|-----|
3666 ROUTE RESERVOIR NHYDout=["GreenB_MN"], NHYDin=["Greenbank"],
3667 RDT=[1](min),
3668 TABLE of ( OUTFLOW-STORAGE ) values
3669 (cms) - (ha-m)
3670 [ 0.0 , 0.0 ]
3671 [ 0.033 , 0.084 ]
3672 [ 0.039 , 0.201 ]
3673 [ 0.113 , 0.292 ]
3674 [ 0.237 , 0.386 ]
3675 [ 0.382 , 0.484 ]
3676 [ 0.539 , 0.585 ]
3677 [ 0.7 , 0.692 ]
3678 [ 0.86 , 0.804 ]
3679 [ 4.684 , 0.922 ]
3680 [ 11.539 , 1.052 ]
3681 [ 20.867 , 1.168 ]
3682 [ 103.616 , 1.974 ]
3683 [ -1 , -1 ] (max twenty pts)
3684 NHYDovf=["GreenB_MJ"],
3685 *%-----|-----|
3686 *%-----|-----|
3687 ADD HYD NHYDsum=["GreenB"], NHYDs to add=["N_TO"+"GreenB_MJ"+"GreenB_MN"]
3688 *%-----|-----|
3689 SAVE HYD NHYD=["GreenB"], # OF PCYCLES=[-1], ICASEsh=[1]
3690 HYD_COMMENT=["Total Flows at Greenbank Drain"]
3691 *%-----|-----|
3692 *#*****|*****|
3693 *# Catchment TODD
3694 *# - To Todd Drain (south of the Jock)

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3695 *# - Subdivision with 43% imp. as per Barrhaven South MSS
3696 *# - 2020-11-30 increase imp. based on P598(04)-11
3697 *# - 2020-11-30 update TODD Tributary Drainage Area to = 146.015 ha based on
P598(04)-11
3698 *# - 2020-11-30 split TODD Drainage Area to MAJOR, MINOR, POND and ALL
3699 *#*****
3700 *# - JFSA 2021-01-19 add "TODD_MN1" as part of Clarke("W_CLAR_MJ") and remove it
from Todd
3701 *CONTINUOUS STANDHYD NHYD=["TODD_MN1"], DT=[1]min, AREA=[1.772](ha),
3702 * XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3703 * SCS curve number CN=[77],
3704 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3705 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3706 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3707 * LGI=[108.689](m), MNI=[0.013], SCI=[0](min),
3708 * Continuous simulation parameters:
3709 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
3710 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3711 * InterEventTime=[18](hrs), END=-1
3712 *%-----|-----
3713 CONTINUOUS STANDHYD NHYD=["TODD_MN2"], DT=[1]min, AREA=[2.1](ha),
3714 XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3715 SCS curve number CN=[77],
3716 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3717 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3718 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3719 LGI=[118.322](m), MNI=[0.013], SCI=[0](min),
3720 Continuous simulation parameters:
3721 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3722 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3723 InterEventTime=[18](hrs), END=-1
3724 *%-----|-----
3725 CONTINUOUS STANDHYD NHYD=["TODD_MN3"], DT=[1]min, AREA=[0.117](ha),
3726 XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3727 SCS curve number CN=[77],
3728 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3729 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3730 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3731 LGI=[27.928](m), MNI=[0.013], SCI=[0](min),
3732 Continuous simulation parameters:
3733 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3734 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3735 InterEventTime=[18](hrs), END=-1
3736 *%-----|-----
3737 CONTINUOUS STANDHYD NHYD=["TODD_MJ"], DT=[1]min, AREA=[30.230](ha),
3738 XIMP=[0.52], TIMP=[0.64], DWF=[0](cms), LOSS=[2],
3739 SCS curve number CN=[77],
3740 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3741 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3742 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3743 LGI=[448.925](m), MNI=[0.013], SCI=[0](min),
3744 Continuous simulation parameters:
3745 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3746 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3747 InterEventTime=[18](hrs), END=-1
3748 *%-----|-----
3749 * -JFSA, 2021-01-19 update "TODD_ALL" area from 108.741 ha to 112.908 ha based on
GIS measurements (148.41-30.23-0.117-2.1-3.055=112.908 ha)
3750 CONTINUOUS STANDHYD NHYD=["TODD_ALL"], DT=[1]min, AREA=[112.908](ha),
3751 XIMP=[0.52], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3752 SCS curve number CN=[77],
3753 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3754 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3755 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3756 LGI=[867.594](m), MNI=[0.013], SCI=[0](min),
3757 Continuous simulation parameters:

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3758 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3759 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3760 InterEventTime=[18](hrs), END=-1
3761 *%-----|-----|
3762 CONTINUOUS STANDHYD NHYD=["TODD_P"], DT=[1]min, AREA=[3.055](ha),
3763 XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3764 SCS curve number CN=[77],
3765 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3766 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3767 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3768 LGI=[142.712](m), MNI=[0.013], SCI=[0](min),
3769 Continuous simulation parameters:
3770 IaREcper=[4](hrs), IaREcimp=[4](hrs),
3771 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3772 InterEventTime=[18](hrs), END=-1
3773 *%-----|-----|
3774 *%-----|-----|
3775 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3776 *CONTINUOUS STANDHYD NHYD=["TODD_DEVL"], DT=[1]min, AREA=[15.87](ha),
3777 * XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3778 * SCS curve number CN=[77],
3779 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3780 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3781 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3782 * LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
3783 * Continuous simulation parameters:
3784 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
3785 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3786 * InterEventTime=[18](hrs), END=-1
3787 *%-----|-----|
3788 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3789 *CONTINUOUS NASHYD NHYD=["TODD_UnD"], DT=[1]min, AREA=[12.47](ha),
3790 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3791 * N=[3], TP=[1.10]hrs,
3792 * Continuous simulation parameters:
3793 * IaREcper=[4](hrs),
3794 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3795 * InterEventTime=[12](hrs)
3796 * Baseflow simulation parameters:
3797 * BaseFlowOption=[1] ,
3798 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3799 * VHydCond=[0.055](mm/hr), END=-1
3800 *%-----|-----|
3801 *# 5-Year + 12% Capture
3802 *COMPUTE DUALHYD NHYDin=["TODD_MJ"], CINLET=[3.314](cms), NINLET=[1],
3803 * MajNHYD=["TODD_MJj"]
3804 * MinNHYD=["TODD_MJn"]
3805 * TMJSTO=[0.1](cu-m)
3806 ROUTE RESERVOIR NHYDout=["TODD_MJn"],NHYDin=["TODD_MJ"] ,
3807 RDT=[1](min),
3808 TABLE of ( OUTFLOW-STORAGE ) values
3809 (cms) - (ha-m)
3810 [ 0.0 , 0.0 ]
3811 [ 3.314 , 0.0001 ]
3812 [ -1 , -1 ] (max twenty pts)
3813 NHYDovf=["TODD_MJj"] ,
3814 *%-----|-----|
3815 *# 5-Year + 12% Capture
3816 *COMPUTE DUALHYD NHYDin=["TODD_MN1"], CINLET=[0.227](cms), NINLET=[1],
3817 * MajNHYD=["TODD_MN1j"]
3818 * MinNHYD=["TODD_MN1n"]
3819 * TMJSTO=[0.1](cu-m)
3820 *ROUTE RESERVOIR NHYDout=["TODD_MN1n"],NHYDin=["TODD_MN1"] ,

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3821 *          RDT=[1](min),
3822 *          TABLE of ( OUTFLOW-STORAGE ) values
3823 *              (cms) - (ha-m)
3824 *              [ 0.0 , 0.0 ]
3825 *              [ 0.227 , 0.0001 ]
3826 *              [ -1 , -1 ] (max twenty pts)
3827 *          NHYDovf=["TODD_MN1j"] ,
3828 *%-----|-----|
3829 *COMPUTE DUALHYD  NHYDin=["TODD_MN2"], CINLET=[0.268](cms), NINLET=[1],
3830 *              MajNHYD=["TODD_MN2j"]
3831 *              MinNHYD=["TODD_MN2n"]
3832 *              TMJSTO=[0.1](cu-m)
3833 ROUTE RESERVOIR  NHYDout=["TODD_MN2n"] ,NHYDin=["TODD_MN2"] ,
3834 RDT=[1](min),
3835          TABLE of ( OUTFLOW-STORAGE ) values
3836              (cms) - (ha-m)
3837              [ 0.0 , 0.0 ]
3838              [ 0.268 , 0.0001 ]
3839              [ -1 , -1 ] (max twenty pts)
3840          NHYDovf=["TODD_MN2j"] ,
3841 *%-----|-----|
3842 *COMPUTE DUALHYD  NHYDin=["TODD_MN3"], CINLET=[0.016](cms), NINLET=[1],
3843 *              MajNHYD=["TODD_MN3j"]
3844 *              MinNHYD=["TODD_MN3n"]
3845 *              TMJSTO=[0.1](cu-m)
3846 ROUTE RESERVOIR  NHYDout=["TODD_MN3n"] ,NHYDin=["TODD_MN3"] ,
3847 RDT=[1](min),
3848          TABLE of ( OUTFLOW-STORAGE ) values
3849              (cms) - (ha-m)
3850              [ 0.0 , 0.0 ]
3851              [ 0.016 , 0.0001 ]
3852              [ -1 , -1 ] (max twenty pts)
3853          NHYDovf=["TODD_MN3j"] ,
3854 *%-----|-----|
3855 *          -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
3856 CONTINUOUS STANDHYD NHYD=["A2"], DT=[1]min, AREA=[25.5](ha),
3857 XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
3858 SCS curve number CN=[75],
3859 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3860 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3861 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3862 LGI=[566](m), MNI=[0.013], SCI=[0](min),
3863 Continuous simulation parameters:
3864 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3865 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3866 InterEventTime=[18](hrs), END=-1
3867 *%-----|-----|
3868 COMPUTE DUALHYD  NHYDin=["A2"], CINLET=[1.818](cms), NINLET=[1],
3869 MajNHYD=["A2-MJ"]
3870 MinNHYD=["A2-MN"]
3871 TMJSTO=[924](cu-m)
3872 *%-----|-----|
3873 ADD HYD          NHYDsum=["TODD"], NHYDs to
add=["TODD_MN2n"+"TODD_MN3n"+"TODD_MJj"+"TODD_P"+"TODD_ALL"+"W_CLAR_MJn"]
3874 *%-----|-----|
3875 SAVE HYD        NHYD=["TODD"], # OF PCYCLES=[-1], ICASEsh=[1]
3876 HYD_COMMENT=["Total Flows at Todd Drain"]
3877 *%-----|-----|
3878 *#*****
3879 *# Todd Pond 3
3880 *# - Rating curve obtained from Barrhaven South MSS modeling
3881 *# - stantec 2007, Tributary Drainage Area to MSS Pond 3 = 193 ha
3882 *#*****
3883 ROUTE RESERVOIR  NHYDout=["MS_P3"], NHYDin=["TODD"],
3884 RDT=[1](min),

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```

3885             TABLE of ( OUTFLOW-STORAGE ) values
3886                 (cms) - (ha-m)
3887                 [ 0.0 , 0.0 ]
3888                 [ 0.014 , 0.155 ]
3889                 [ 0.048 , 0.394 ]
3890                 [ 0.061 , 0.56 ]
3891                 [ 0.08 , 0.909 ]
3892                 [ 0.088 , 1.089 ]
3893                 [ 0.109 , 1.652 ]
3894                 [ 0.118 , 1.952 ]
3895                 [ 0.122 , 2.099 ]
3896                 [ 1.972 , 2.269 ]
3897                 [ 9.135 , 2.598 ]
3898                 [ 15.608 , 2.826 ]
3899                 [ 19.256 , 2.942 ]
3900                 [ 27.282 , 3.181 ]
3901                 [ 40.957 , 3.55 ]
3902                 [ 56.372 , 3.929 ]
3903                 [ 73.349 , 4.317 ]
3904                 [ 85.469 , 4.579 ]
3905                 [ 104.771 , 4.977 ]
3906                 [ -1 , -1 ] (max twenty pts)
3907             NHYDovf=["P3-OVF"]
3908 *%-----|-----|
3909 ADD HYD           NHYDsum=["SN_TO"], NHYDs to
add=["GreenB"+"MS_P3"+"P3-OVF"+"TODD_MN2j"+"A2-MJ"]
3910 *%-----|-----|
3911 SAVE HYD        NHYD=["SN_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3912             HYD_COMMENT=["Total Flows at Todd Drain"]
3913 *%-----|-----|
3914 *#
3915 *# Hydrograph from Todd Drain routed to Corrigan Drain
3916 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
3917 *# 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so
the model will be more stable and give reasonable results. It is justifiable as ROUTE
CHANNELs aren't well suited to really flat slopes.
3918 *
3919 ROUTE CHANNEL   NHYDout=["N_TO"] ,NHYDin=["SN_TO"] ,
3920             RDT=[1](min),
3921             CHLGTH=[280](m),  CHSLOPE=[0.05](%),
3922             FPSLOPE=[0.05](%),
3923             SECNUM=[1.0],      NSEG=[3]
3924             ( SEGROUGH, SEGDIST (m))=
3925             [0.075,-17.72
3926             -0.045,17.72
3927             0.075,80.62] NSEG times
3928             ( DISTANCE (m), ELEVATION (m))=
3929             [-83.32, 90.00]
3930             [-81.36, 89.50]
3931             [-79.12, 89.00]
3932             [-76.13, 88.50]
3933             [-20.46, 88.00]
3934             [-19.36, 87.50]
3935             [-18.51, 87.00]
3936             [-17.72, 86.50]
3937             [-11.95, 85.24]
3938             [-0.11, 85.12]
3939             [11.49, 85.20]
3940             [17.72, 86.50]
3941             [19.74, 87.00]
3942             [21.22, 87.50]
3943             [22.68, 88.00]
3944             [24.28, 88.50]
3945             [26.79, 89.00]
3946             [71.98, 90.00]
3947             [80.62, 90.50]

```



```

3948 *%-----|-----|
3949 SAVE HYD NHYD=["N_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3950 HYD_COMMENT=["Total inflows at Station 2462"]
3951 *%-----|-----|
3952 *#*****|-----|
3953 *# Catchment CORRIG
3954 *# - To Corrigan Drain (south of the Jock)
3955 *# - Primarily Developed (medium density)
3956 *# - JFSA JAN 2021, add Corrigan subcatchments as per IBI, July 2008
3957 *#*****|-----|
3958 *ROUTE RESERVOIR NHYDout=["MS_P1"], NHYDin=["CORRIG"],
3959 * RDT=[1](min),
3960 * TABLE of ( OUTFLOW-STORAGE ) values
3961 * (cms) - (ha-m)
3962 * [ 0.0 , 0.0 ]
3963 * [ 0.06 , 0.58]
3964 * [ -1 , -1 ] (max twenty pts)
3965 * NHYDovf=["P1-OVF"]
3966 *%-----|-----|
3967 *ADD HYD NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
3968 *%-----|-----|
3969 *SAVE HYD NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
3970 * HYD_COMMENT=["Total Flows at Corrigan Drain"]
3971 *%-----|-----|
3972 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3973 CONTINUOUS STANDHYD NHYD=["corr1"], DT=[1]min, AREA=[15.87](ha),
3974 XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3975 SCS curve number CN=[77],
3976 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3977 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3978 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3979 LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
3980 Continuous simulation parameters:
3981 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3982 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3983 InterEventTime=[18](hrs), END=-1
3984 *%-----|-----|
3985 * -JFSA 2021-02-23 add DUALHYD for "corr1". "corr1" DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to "corr1".
3986 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
3987 COMPUTE DUALHYD NHYDin=["corr1"], CINLET=[1.818](cms), NINLET=[1],
3988 MajNHYD=["corr1-MJ"]
3989 MinNHYD=["corr1-MN"]
3990 TMJSTO=[924](cu-m)
3991 *%-----|-----|
3992 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3993 CONTINUOUS NASHYD NHYD=["corr2"], DT=[1]min, AREA=[12.47](ha),
3994 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3995 N=[3], TP=[1.10]hrs,
3996 Continuous simulation parameters:
3997 IaRECper=[4](hrs),
3998 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3999 InterEventTime=[12](hrs)
4000 Baseflow simulation parameters:
4001 BaseFlowOption=[1] ,
4002 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4003 VHydCond=[0.055](mm/hr), END=-1
4004 *%-----|-----|
4005 * -JFSA 2021-01-19 change Al-Corrig to be developed as per geottawa website and
apply the parameters of A2, the nearest sub-catchment to Al-Corrig, LGI is calculated
based on Al-Corrig area
4006 * -JFSA 2021-01-19 update all Corrigan areas based on GIS measurements, and keep

```

LGI as it is from Corrigan Report, IBI Group, 2008 because LGI calculated is less than LGI from the Corrigan Report

```
4007 CONTINUOUS STANDHYD NHYD=["A1-Corrig"], DT=[1]min, AREA=[15.75](ha),
4008 XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
4009 SCS curve number CN=[75],
4010 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4011 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4012 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4013 LGI=[324.037](m), MNI=[0.013], SCI=[0](min),
4014 Continuous simulation parameters:
4015 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4016 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4017 InterEventTime=[18](hrs), END=-1
4018 *
4019 * -JFSA 2021-01-25 add DUALHYD for A1-Corrig. A1-Corrig DUALHYD Parameters are the
4020 * same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to A1-Corrig.
4021 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
4022 A1-Corrig
4023 COMPUTE DUALHYD NHYDin=["A1-Corrig"], CINLET=[1.818](cms), NINLET=[1],
4024 MajNHYD=["A1-MJ"]
4025 MinNHYD=["A1-MN"]
4026 TMJSTO=[924](cu-m)
4027 *%-----|-----|
4028 *CONTINUOUS NASHYD NHYD=["A1-Corrig"], DT=[1]min, AREA=[15.75](ha),
4029 * DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
4030 * N=[3.0], TP=[0.36]hrs,
4031 * Continuous simulation parameters:
4032 * IaREcper=[4](hrs),
4033 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4034 * InterEventTime=[12](hrs)
4035 * Baseflow simulation parameters:
4036 * BaseFlowOption=[1] ,
4037 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4038 * VHydCond=[0.055](mm/hr), END=-1
4039 *%-----|-----|
4040 CONTINUOUS NASHYD NHYD=["B1"], DT=[1]min, AREA=[2.77](ha),
4041 DWF=[0](cms), CN/C=[56], IA=[2.5](mm),
4042 N=[3.0], TP=[0.23]hrs,
4043 Continuous simulation parameters:
4044 IaREcper=[4](hrs),
4045 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4046 InterEventTime=[12](hrs)
4047 Baseflow simulation parameters:
4048 BaseFlowOption=[1] ,
4049 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4050 VHydCond=[0.055](mm/hr), END=-1
4051 *%-----|-----|
4052 CONTINUOUS STANDHYD NHYD=["A4"], DT=[1]min, AREA=[1.27](ha),
4053 XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
4054 SCS curve number CN=[75],
4055 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4056 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4057 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4058 LGI=[253](m), MNI=[0.013], SCI=[0](min),
4059 Continuous simulation parameters:
4060 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4061 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4062 InterEventTime=[18](hrs), END=-1
4063 *%-----|-----|
4064 COMPUTE DUALHYD NHYDin=["A4"], CINLET=[0.405](cms), NINLET=[1],
4065 MajNHYD=["A4-MJ"]
4066 MinNHYD=["A4-MN"]
4067 TMJSTO=[68](cu-m)
4068 *%-----|-----|
4069 ADD HYD NHYDsum=["MH101"], NHYDs to
4070 add=["A1-MJ"+"A1-MN"+"corr1-MJ"+"corr1-MN"+"corr2"+"B1"+"A4-MN"]
```

```

4068 *%-----|-----|
4069 SAVE HYD      NHYD=["MH101"], # OF PCYCLES=[-1], ICASEsh=[1]
4070              HYD_COMMENT=["Total Flows at MH101"]
4071 *%-----|-----|
4072 ROUTE PIPE     PTYPE=[1]circ, NHYDout=["101-102"], RNUMBER=[1.0], PDIAM=[1050](mm),
4073              PLNGTH=[368](m), PROUGH=[0.013], PSLOPE=[0.0054](m/m),
4074              NHYDin=["MH101"], RDT=[1]
4075 *%-----|-----|
4076 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
4077 major system from A2 can be added to Todd
4078 *CONTINUOUS STANDHYD NHYD=["A2"], DT=[1]min, AREA=[25.5](ha),
4079 *              XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
4080 *              SCS curve number CN=[75],
4081 *              Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4082 *              LGP=[40](m), MNP=[0.25], SCP=[0](min),
4083 *              Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4084 *              LGI=[566](m), MNI=[0.013], SCI=[0](min),
4085 *              Continuous simulation parameters:
4086 *              IaRECper=[4](hrs), IaRECimp=[4](hrs),
4087 *              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4088 *              InterEventTime=[18](hrs), END=-1
4089 *%-----|-----|
4090 *CONTINUE DUALHYD NHYDin=["A2"], CINLET=[1.818](cms), NINLET=[1],
4091 *              MaJNHYD=["A2-MJ"]
4092 *              MinNHYD=["A2-MN"]
4093 *              TMJSTO=[924](cu-m)
4094 *%-----|-----|
4095 ADD HYD        NHYDsum=["MH102"], NHYDs to add=["A2-MN"+"101-102"]
4096 *%-----|-----|
4097 SAVE HYD      NHYD=["MH102"], # OF PCYCLES=[-1], ICASEsh=[1]
4098              HYD_COMMENT=["Total Flows at MH102"]
4099 *%-----|-----|
4100 CONTINUOUS STANDHYD NHYD=["A5"], DT=[1]min, AREA=[1.6](ha),
4101 *              XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4102 *              SCS curve number CN=[75],
4103 *              Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4104 *              LGP=[40](m), MNP=[0.25], SCP=[0](min),
4105 *              Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4106 *              LGI=[300](m), MNI=[0.013], SCI=[0](min),
4107 *              Continuous simulation parameters:
4108 *              IaRECper=[4](hrs), IaRECimp=[4](hrs),
4109 *              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4110 *              InterEventTime=[18](hrs), END=-1
4111 *%-----|-----|
4112 ADD HYD        NHYDsum=["A5T"], NHYDs to add=["A4-MJ"+"A5"]
4113 *%-----|-----|
4114 COMPUTE DUALHYD NHYDin=["A5T"], CINLET=[0.357](cms), NINLET=[1],
4115 *              MaJNHYD=["A5-MJ"]
4116 *              MinNHYD=["A5-MN"]
4117 *              TMJSTO=[60](cu-m)
4118 *%-----|-----|
4119 * -JFSA Jan. 2021, A3 is a part of Todd so it is removed
4120 * -JFSA Jan. 2021, "A2-MJ" added to "Todd"
4121 *CONTINUOUS STANDHYD NHYD=["A3"], DT=[1]min, AREA=[18.4](ha),
4122 *              XIMP=[0.58], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
4123 *              SCS curve number CN=[75],
4124 *              Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4125 *              LGP=[40](m), MNP=[0.25], SCP=[0](min),
4126 *              Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4127 *              LGI=[450](m), MNI=[0.013], SCI=[0](min),
4128 *              Continuous simulation parameters:
4129 *              IaRECper=[4](hrs), IaRECimp=[4](hrs),
4130 *              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4131 *              InterEventTime=[18](hrs), END=-1
4132 *%-----|-----|
4133 *ADD HYD        NHYDsum=["A3-A2MJ"], NHYDs to add=["A2-MJ"+"A3"]

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4132 *%-----|-----|
4133 *COMPUTE DUALHYD   NHYDin=["A3-A2MJ"], CINLET=[2.208](cms), NINLET=[1],
4134 *                   MajNHYD=["A3R-MJ"]
4135 *                   MinNHYD=["A3R-MN"]
4136 *                   TMJSTO=[908](cu-m)
4137 *%-----|-----|
4138 ROUTE PIPE         PTYPE=[1]circ, NHYDout=["102-103"], RNUMBER=[1.0], PDIAM=[1500](mm),
4139                   PLNGTH=[504](m), PROUGH=[0.013], PSLOPE=[0.0028](m/m),
                   NHYDin=["MH102"], RDT=[1]
4140 *%-----|-----|
4141 ADD HYD            NHYDsum=["MH103"], NHYDs to add=["102-103"+"A5-MN"]
4142 *%-----|-----|
4143 SAVE HYD           NHYD=["MH103"], # OF PCYCLES=[-1], ICASEsh=[1]
4144                   HYD_COMMENT=["Total Flows at MH103"]
4145 *%-----|-----|
4146 ROUTE PIPE         PTYPE=[1]circ, NHYDout=["103-104"], RNUMBER=[1.0], PDIAM=[1650](mm),
4147                   PLNGTH=[438](m), PROUGH=[0.013], PSLOPE=[0.0046](m/m),
                   NHYDin=["MH103"], RDT=[1]
4148 *%-----|-----|
4149 CONTINUOUS STANDHYD NHYD=["A6"], DT=[1]min, AREA=[1.56](ha),
4150                   XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4151                   SCS curve number CN=[75],
4152                   Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4153                   LGP=[40](m), MNP=[0.25], SCP=[0](min),
4154                   Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4155                   LGI=[280](m), MNI=[0.013], SCI=[0](min),
4156                   Continuous simulation parameters:
4157                   IaRECper=[4](hrs), IaRECimp=[4](hrs),
4158                   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4159                   InterEventTime=[18](hrs), END=-1
4160 *%-----|-----|
4161 ADD HYD            NHYDsum=["A6T"], NHYDs to add=["A5-MJ"+"A6"]
4162 *%-----|-----|
4163 COMPUTE DUALHYD   NHYDin=["A6T"], CINLET=[0.357](cms), NINLET=[1],
4164 *                   MajNHYD=["A6-MJ"]
4165 *                   MinNHYD=["A6-MN"]
4166 *                   TMJSTO=[60](cu-m)
4167 *%-----|-----|
4168 *   -JFSA Jan. 2021, A7-corrig is a part of Todd so it is removed
4169 *CONTINUOUS STANDHYD NHYD=["A7-corrig"], DT=[1]min, AREA=[11.8](ha),
4170 *                   XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4171 *                   SCS curve number CN=[75],
4172 *                   Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4173 *                   LGP=[40](m), MNP=[0.25], SCP=[0](min),
4174 *                   Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4175 *                   LGI=[438](m), MNI=[0.013], SCI=[0](min),
4176 *                   Continuous simulation parameters:
4177 *                   IaRECper=[4](hrs), IaRECimp=[4](hrs),
4178 *                   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4179 *                   InterEventTime=[18](hrs), END=-1
4180 *%-----|-----|
4181 *ADD HYD           NHYDsum=["A7-A3RMJ"], NHYDs to add=["A3R-MJ"+"A7-corrig"]
4182 *%-----|-----|
4183 *COMPUTE DUALHYD   NHYDin=["A7-A3RMJ"], CINLET=[1.003](cms), NINLET=[1],
4184 *                   MajNHYD=["A7R-MJ"]
4185 *                   MinNHYD=["A7R-MN"]
4186 *                   TMJSTO=[496](cu-m)
4187 *%-----|-----|
4188 ADD HYD            NHYDsum=["MH104"], NHYDs to add=["A6-MN"+"103-104"+"TODD_MJn"]
4189 *%-----|-----|
4190 SAVE HYD           NHYD=["MH104"], # OF PCYCLES=[-1], ICASEsh=[1]
4191                   HYD_COMMENT=["Total Flows at MH104"]
4192 *%-----|-----|
4193 CONTINUOUS STANDHYD NHYD=["B2"], DT=[1]min, AREA=[12.31](ha),
4194                   XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4195                   SCS curve number CN=[75],

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4196 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4197 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4198 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4199 LGI=[417](m), MNI=[0.013], SCI=[0](min),
4200 Continuous simulation parameters:
4201 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4202 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4203 InterEventTime=[18](hrs), END=-1
4204 *%-----|
4205 COMPUTE DUALHYD NHYDin=["B2"], CINLET=[1.029](cms), NINLET=[1],
4206 MajNHYD=["B2-MJ"]
4207 MinNHYD=["B2-MN"]
4208 TMJSTO=[508](cu-m)
4209 *%-----|
4210 ROUTE PIPE PTYPE=[1]circ, NHYDout=["315-333"], RNUMBER=[1.0], PDIAM=[1200](mm),
4211 PLNGTH=[254](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["B2-MN"], RDT=[1]
4212 *%-----|
4213 CONTINUOUS STANDHYD NHYD=["B3"], DT=[1]min, AREA=[5.59](ha),
4214 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4215 SCS curve number CN=[75],
4216 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4217 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4218 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4219 LGI=[345](m), MNI=[0.013], SCI=[0](min),
4220 Continuous simulation parameters:
4221 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4222 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4223 InterEventTime=[18](hrs), END=-1
4224 *%-----|
4225 COMPUTE DUALHYD NHYDin=["B3"], CINLET=[0.459](cms), NINLET=[1],
4226 MajNHYD=["B3-MJ"]
4227 MinNHYD=["B3-MN"]
4228 TMJSTO=[227](cu-m)
4229 *%-----|
4230 ADD HYD NHYDsum=["MH333"], NHYDs to add=["B3-MN"+"315-333"]
4231 *%-----|
4232 SAVE HYD NHYD=["MH333"], # OF PCYCLES=[-1], ICASEsh=[1]
4233 HYD_COMMENT=["Total Flows at MH333"]
4234 *%-----|
4235 ROUTE PIPE PTYPE=[1]circ, NHYDout=["333-335"], RNUMBER=[1.0], PDIAM=[1200](mm),
4236 PLNGTH=[251](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH333"], RDT=[1]
4237 *%-----|
4238 ROUTE PIPE PTYPE=[1]circ, NHYDout=["335-338"], RNUMBER=[1.0], PDIAM=[1200](mm),
4239 PLNGTH=[185](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["333-335"], RDT=[1]
4240 *%-----|
4241 ROUTE PIPE PTYPE=[1]circ, NHYDout=["338-340"], RNUMBER=[1.0], PDIAM=[1350](mm),
4242 PLNGTH=[233](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["335-338"], RDT=[1]
4243 *%-----|
4244 CONTINUOUS STANDHYD NHYD=["B4"], DT=[1]min, AREA=[7.6](ha),
4245 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4246 SCS curve number CN=[75],
4247 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4248 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4249 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4250 LGI=[388](m), MNI=[0.013], SCI=[0](min),
4251 Continuous simulation parameters:
4252 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4253 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4254 InterEventTime=[18](hrs), END=-1
4255 *%-----|
4256 COMPUTE DUALHYD NHYDin=["B4"], CINLET=[0.655](cms), NINLET=[1],
4257 MajNHYD=["B4-MJ"]

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4258 MinNHYD=["B4-MN"]
4259 TMJSTO=[323](cu-m)
4260 *%-----|-----|
4261 ADD HYD NHYDsum=["MH340"], NHYDs to add=["338-340"+"B4-MN"]
4262 *%-----|-----|
4263 SAVE HYD NHYD=["MH340"], # OF PCYCLES=[-1], ICASEsh=[1]
4264 HYD_COMMENT=["Total Flows at MH340"]
4265 *%-----|-----|
4266 ROUTE PIPE PTYPE=[1]circ, NHYDout=["340-104"], RNUMBER=[1.0], PDIAM=[1650](mm),
4267 PLNGTH=[240](m), PROUGH=[0.013], PSLOPE=[0.0015](m/m),
NHYDin=["MH340"], RDT=[1]
4268 *%-----|-----|
4269 ADD HYD NHYDsum=["MH104T"], NHYDs to add=["340-104"+"MH104"]
4270 *%-----|-----|
4271 ROUTE PIPE PTYPE=[2]rect, NHYDout=["104-105"], RNUMBER=[1.0],
4272 PWIDTH=[2400](mm) by PHEIGHT=[2100](mm),
PLNGTH=[380](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH104T"], RDT=[1]
4273 *%-----|-----|
4274 CONTINUOUS STANDHYD NHYD=["B5"], DT=[1]min, AREA=[2.2](ha),
4275 XIMP=[0.57], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
4276 SCS curve number CN=[75],
4277 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4278 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4279 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4280 LGI=[187](m), MNI=[0.013], SCI=[0](min),
4281 Continuous simulation parameters:
4282 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4283 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4284 InterEventTime=[18](hrs), END=-1
4285 *%-----|-----|
4286 COMPUTE DUALHYD NHYDin=["B5"], CINLET=[0.260](cms), NINLET=[1],
4287 MajNHYD=["B5-MJ"]
4288 MinNHYD=["B5-MN"]
4289 TMJSTO=[250](cu-m)
4290 *%-----|-----|
4291 CONTINUOUS STANDHYD NHYD=["A8"], DT=[1]min, AREA=[0.96](ha),
4292 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4293 SCS curve number CN=[75],
4294 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4295 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4296 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4297 LGI=[186](m), MNI=[0.013], SCI=[0](min),
4298 Continuous simulation parameters:
4299 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4300 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4301 InterEventTime=[18](hrs), END=-1
4302 *%-----|-----|
4303 ADD HYD NHYDsum=["A8T"], NHYDs to add=["A6-MJ"+"A8"]
4304 *%-----|-----|
4305 COMPUTE DUALHYD NHYDin=["A8T"], CINLET=[0.238](cms), NINLET=[1],
4306 MajNHYD=["A8-MJ"]
4307 MinNHYD=["A8-MN"]
4308 TMJSTO=[40](cu-m)
4309 *%-----|-----|
4310 ADD HYD NHYDsum=["MH105"], NHYDs to
add=["104-105"+"B5-MN"+"A8-MN"+"TODD_MN3j"]
4311 *%-----|-----|
4312 SAVE HYD NHYD=["MH105"], # OF PCYCLES=[-1], ICASEsh=[1]
4313 HYD_COMMENT=["Total Flows at MH105"]
4314 *%-----|-----|
4315 DIVERT HYD NHYDin=["A8-MJ"] NIDout=[2]max five,
4316 outflow hydrographs (NHYDs)=["A8-MJ-JR" "A8-MJ-B6"]
4317 flow distribution table: (modify as necessary)
4318 Note: all flows are in (cms)

```

```

4319          QIDi + QIDii = QTOTAL
4320          [ 0 + 0 = 0 ]
4321          [ 50 + 50 = 100 ] end
4322  *%-----|-----
|
4323  DIVERT HYD      NHYDin=["MH105"] NIDout=[2]max five,
4324                  outflow hydrographs (NHYDs)=["MH105-JR" "MH105-B6"]
4325                  flow distribution table: (modify as necessary)
4326                  Note: all flows are in (cms)
4327                  QIDi + QIDii = QTOTAL
4328                  [ 0 + 0 = 0 ]
4329                  [ 0 + 3.0 = 3.0 ]
4330                  [ 96.9+ 3.1 = 100 ] end
4331  *%-----|-----
|
4332  CONTINUOUS STANDHYD  NHYD=["B7"], DT=[1]min, AREA=[7.19](ha),
4333                  XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4334                  SCS curve number CN=[75],
4335                  Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4336                  LGP=[40](m), MNP=[0.25], SCP=[0](min),
4337                  Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4338                  LGI=[211](m), MNI=[0.013], SCI=[0](min),
4339                  Continuous simulation parameters:
4340                  IaRECper=[4](hrs), IaRECimp=[4](hrs),
4341                  SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4342                  InterEventTime=[18](hrs), END=-1
4343  *%-----|-----
4344  ADD HYD          NHYDsum=["B7-B4MJ"], NHYDs to add=["B4-MJ"+"B7"]
4345  *%-----|-----
4346  COMPUTE DUALHYD  NHYDin=["B7-B4MJ"], CINLET=[0.629](cms), NINLET=[1],
4347                  MajNHYD=["B7R-MJ"]
4348                  MinNHYD=["B7R-MN"]
4349                  TMJSTO=[311](cu-m)
4350  *%-----|-----
4351  ROUTE PIPE       PTYPE=[1]circ, NHYDout=["360-106A"], RNUMBER=[1.0], PDIAM=[1050](mm),
4352                  PLNGTH=[167](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
4353                  NHYDin=["B7R-MN"], RDT=[1]
4354  *%-----|-----
4355  * -JFSA 2021-01-19 change B6 to be developed as per geottawa website and apply the
4356  parameters of A7, the nearest sub-catchment to B6, LGI is calculated based on B6 area
4357  CONTINUOUS STANDHYD  NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4358                  XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4359                  SCS curve number CN=[75],
4360                  Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4361                  LGP=[40](m), MNP=[0.25], SCP=[0](min),
4362                  Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4363                  LGI=[148.099](m), MNI=[0.013], SCI=[0](min),
4364                  Continuous simulation parameters:
4365                  IaRECper=[4](hrs), IaRECimp=[4](hrs),
4366                  SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4367                  InterEventTime=[18](hrs), END=-1
4368  *%-----|-----
4369  * -JFSA 2021-01-25 add B1 DUALHYD as per Corrigan Report, IBI Group, 2008
4370  COMPUTE DUALHYD  NHYDin=["B6"], CINLET=[0.064](cms), NINLET=[1],
4371                  MajNHYD=["B6-MJ"]
4372                  MinNHYD=["B6-MN"]
4373                  TMJSTO=[5484](cu-m)
4374  *%-----|-----
4375  *CONTINUOUS NASHHYD  NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4376                  * DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
4377                  * N=[3.0], TP=[0.36]hrs,
4378                  * Continuous simulation parameters:
4379                  * IaRECper=[4](hrs),
4380                  * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4381                  * InterEventTime=[12](hrs)
4382                  * Baseflow simulation parameters:

```

```

4381 *           BaseFlowOption=[1] ,
4382 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4383 *           VHydCond=[0.055](mm/hr),   END=-1
4384 *%-----|-----
4385 *%   -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
4386 CONTINUOUS STANDHYD NHYD=["EX-LAND"], DT=[1]min, AREA=[32.5](ha),
4387 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
4388 SCS curve number CN=[74],
4389 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4390 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4391 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4392 LGI=[465.475](m), MNI=[0.013], SCI=[0](min),
4393 Continuous simulation parameters:
4394 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4395 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4396 InterEventTime=[18](hrs),   END=-1
4397 *%-----|-----
4398 COMPUTE DUALHYD NHYDin=["EX-LAND"], CINLET=[2.275](cms), NINLET=[1],
4399 MajNHYD=["EX-LAND-MJ"]
4400 MinNHYD=["EX-LAND-MN"]
4401 TMJSTO=[1365](cu-m)
4402 *%-----|-----
4403 ADD HYD NHYDsum=["B6-B7ExMJ"], NHYDs to
add=["B7R-MJ"+"EX-LAND-MJ"+"B5-MJ"+"B6-MJ"+"B6-MN"+"A8-MJ-B6"]
4404 *%-----|-----
4405 COMPUTE DUALHYD NHYDin=["B6-B7ExMJ"], CINLET=[0.064](cms), NINLET=[1],
4406 MajNHYD=["B6R-MJ"]
4407 MinNHYD=["B6R-MN"]
4408 TMJSTO=[5484](cu-m)
4409 *%-----|-----
4410 ROUTE PIPE PTYPE=[1]circ, NHYDout=["105-106A"], RNUMBER=[1.0], PDIAM=[1800](mm),
4411 PLNGTH=[208](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH105-B6"], RDT=[1]
4412 *%-----|-----
4413 ADD HYD NHYDsum=["MH106A"], NHYDs to
add=["360-106A"+"105-106A"+"B6R-MN"+"B6R-MJ"]
4414 *%-----|-----
4415 SAVE HYD NHYD=["MH106A"], # OF PCYCLES=[-1], ICASEsh=[1]
4416 HYD_COMMENT=["Total Flows at MH106A"]
4417 *%-----|-----
4418 *%   -JFSA 2021-01-12 THE MANHOLE MH106 is called MH117/106 in Corrigan Report, IBI
Group, July 2008
4419 *%
4420 ROUTE PIPE PTYPE=[1]circ, NHYDout=["106A-106"], RNUMBER=[1.0], PDIAM=[1800](mm),
4421 PLNGTH=[190](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH106A"], RDT=[1]
4422 *%-----|-----
4423 CONTINUOUS STANDHYD NHYD=["A9"], DT=[1]min, AREA=[2.44](ha),
4424 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4425 SCS curve number CN=[75],
4426 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4427 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4428 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4429 LGI=[262](m), MNI=[0.013], SCI=[0](min),
4430 Continuous simulation parameters:
4431 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4432 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4433 InterEventTime=[18](hrs),   END=-1
4434 *%-----|-----
4435 COMPUTE DUALHYD NHYDin=["A9"], CINLET=[0.547](cms), NINLET=[1],
4436 MajNHYD=["A9-MJ"]
4437 MinNHYD=["A9-MN"]
4438 TMJSTO=[0](cu-m)
4439 *%-----|-----
4440 ADD HYD NHYDsum=["MH106"], NHYDs to add=["106A-106"+"A9-MN"]

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4441  *%-----|-----|
4442  SAVE HYD      NHYD=["MH106"], # OF PCYCLES=[-1], ICASEsh=[1]
4443              HYD_COMMENT=["Total Flows at MH106"]
4444  *%-----|-----|
4445  *%    -JFSA 2021-01-12 THE MANHOLE MH107 is called MH118/107 in Corrigan Report, IBI
Group, July 2008
4446  *%
4447  ROUTE PIPE    PTYPE=[1]circ, NHYDout=["106-107"], RNUMBER=[1.0], PDIAM=[1800](mm),
4448              PLNGTH=[122.5](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
              NHYDin=["MH106"], RDT=[1]
4449  *%-----|-----|
4450  CONTINUOUS STANDHYD NHYD=["A10"], DT=[1]min, AREA=[4.14](ha),
4451              XIMP=[0.35], TIMP=[0.47], DWF=[0](cms), LOSS=[2],
4452              SCS curve number CN=[75],
4453              Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4454              LGP=[40](m), MNP=[0.25], SCP=[0](min),
4455              Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4456              LGI=[183](m), MNI=[0.013], SCI=[0](min),
4457              Continuous simulation parameters:
4458              IaRECper=[4](hrs), IaRECimp=[4](hrs),
4459              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4460              InterEventTime=[18](hrs), END=-1
4461  *%-----|-----|
4462  COMPUTE DUALHYD NHYDin=["A10"], CINLET=[0.310](cms), NINLET=[1],
4463              MajNHYD=["A10-MJ"]
4464              MinNHYD=["A10-MN"]
4465              TMJSTO=[228](cu-m)
4466  *%-----|-----|
4467  CONTINUOUS STANDHYD NHYD=["A11"], DT=[1]min, AREA=[10.61](ha),
4468              XIMP=[0.53], TIMP=[0.62], DWF=[0](cms), LOSS=[2],
4469              SCS curve number CN=[75],
4470              Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4471              LGP=[40](m), MNP=[0.25], SCP=[0](min),
4472              Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4473              LGI=[379](m), MNI=[0.013], SCI=[0](min),
4474              Continuous simulation parameters:
4475              IaRECper=[4](hrs), IaRECimp=[4](hrs),
4476              SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4477              InterEventTime=[18](hrs), END=-1
4478  *%-----|-----|
4479  COMPUTE DUALHYD NHYDin=["A11"], CINLET=[0.993](cms), NINLET=[1],
4480              MajNHYD=["A11-MJ"]
4481              MinNHYD=["A11-MN"]
4482              TMJSTO=[556](cu-m)
4483  *%-----|-----|
4484  ADD HYD      NHYDsum=["MH107"], NHYDs to add=["106-107"+"A10-MN"+"A11-MN"]
4485  *%-----|-----|
4486  SAVE HYD      NHYD=["MH107"], # OF PCYCLES=[-1], ICASEsh=[1]
4487              HYD_COMMENT=["Total Flows at MH107"]
4488  *%-----|-----|
4489  ROUTE PIPE    PTYPE=[1]circ, NHYDout=["107-119"], RNUMBER=[1.0], PDIAM=[1800](mm),
4490              PLNGTH=[114](m), PROUGH=[0.013], PSLOPE=[0.0012](m/m),
              NHYDin=["MH107"], RDT=[1]
4491  *%-----|-----|
4492  *%    -JFSA 2021-01-12 THE MANHOLE MH108 is called MH120/108 in Corrigan Report, IBI
Group, July 2008
4493  *%
4494  ROUTE PIPE    PTYPE=[1]circ, NHYDout=["119-108"], RNUMBER=[1.0], PDIAM=[1800](mm),
4495              PLNGTH=[65.8](m), PROUGH=[0.013], PSLOPE=[0.0012](m/m),
              NHYDin=["107-119"], RDT=[1]
4496  *%-----|-----|
4497  CONTINUOUS STANDHYD NHYD=["A12"], DT=[1]min, AREA=[12.29](ha),
4498              XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4499              SCS curve number CN=[75],
4500              Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4501              LGP=[40](m), MNP=[0.25], SCP=[0](min),

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4502      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4503          LGI=[183](m), MNI=[0.013], SCI=[0](min),
4504      Continuous simulation parameters:
4505      IaREcper=[4](hrs), IaREcimp=[4](hrs),
4506      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4507      InterEventTime=[18](hrs), END=-1
4508  *%-----|-----|
4509  COMPUTE DUALHYD  NHYDin=["A12"], CINLET=[1.029](cms), NINLET=[1],
4510      MajNHYD=["A12-MJ"]
4511      MinNHYD=["A12-MN"]
4512      TMJSTO=[672](cu-m)
4513  *%-----|-----|
4514  CONTINUOUS STANDHYD  NHYD=["A13"], DT=[1]min, AREA=[2.59](ha),
4515      XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4516      SCS curve number CN=[75],
4517      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4518          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4519      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4520          LGI=[379](m), MNI=[0.013], SCI=[0](min),
4521      Continuous simulation parameters:
4522      IaREcper=[4](hrs), IaREcimp=[4](hrs),
4523      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4524      InterEventTime=[18](hrs), END=-1
4525  *%-----|-----|
4526  COMPUTE DUALHYD  NHYDin=["A13"], CINLET=[0.571](cms), NINLET=[1],
4527      MajNHYD=["A13-MJ"]
4528      MinNHYD=["A13-MN"]
4529      TMJSTO=[0](cu-m)
4530  *%-----|-----|
4531  * -JFSA 2021-01-22 add the Corrigan pond area ("Pond-Block")
4532  CONTINUOUS STANDHYD  NHYD=["Pond-Block"], DT=[1]min, AREA=[2.94](ha),
4533      XIMP=[0.415], TIMP=[0.415], DWF=[0](cms), LOSS=[2],
4534      SCS curve number CN=[75],
4535      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4536          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4537      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4538          LGI=[183](m), MNI=[0.013], SCI=[0](min),
4539      Continuous simulation parameters:
4540      IaREcper=[4](hrs), IaREcimp=[4](hrs),
4541      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4542      InterEventTime=[18](hrs), END=-1
4543  *%-----|-----|
4544  ADD HYD  NHYDsum=["MH108"], NHYDs to add=["119-108"+"A13-MN"+"A12-MN"]
4545  *%-----|-----|
4546  SAVE HYD  NHYD=["MH108"], # OF PCYCLES=[-1], ICASEsh=[1]
4547      HYD_COMMENT=["Total Flows at MH108"]
4548  *%-----|-----|
4549  ROUTE PIPE  PTYPE=[1]circ, NHYDout=["108-116"], RNUMBER=[1.0], PDIAM=[1800](mm),
4550      PLNGTH=[76.6](m), PROUGH=[0.013], PSLOPE=[0.0013](m/m),
      NHYDin=["MH108"], RDT=[1]
4551  *%-----|-----|
4552  ROUTE PIPE  PTYPE=[1]circ, NHYDout=["116-corrig"], RNUMBER=[1.0],
      PDIAM=[1800](mm),
4553      PLNGTH=[79.5](m), PROUGH=[0.013], PSLOPE=[0.0013](m/m),
      NHYDin=["108-116"], RDT=[1]
4554  *%-----|-----|
4555  ADD HYD  NHYDsum=["Corrigan"], NHYDs to add=["116-corrig"+"Pond-Block"]
4556  *%-----|-----|
4557  SAVE HYD  NHYD=["Corrigan"], # OF PCYCLES=[-1], ICASEsh=[1]
4558      HYD_COMMENT=["Total Flows at Corrigan Pond"]
4559  *%-----|-----|
4560  ROUTE RESERVOIR  NHYDout=["Co-P"], NHYDin=["Corrigan"],
4561      RDT=[1](min),
4562      TABLE of ( OUTFLOW-STORAGE ) values
4563          (cms) - (ha-m)
4564          [ 0.0 , 0.0 ]

```

```

4565 [ 0.015 , 0.04118]
4566 [ 0.030 , 0.08297]
4567 [ 0.045 , 0.12537]
4568 [ 0.060 , 0.16837]
4569 [ 0.075 , 0.21199]
4570 [ 0.090 , 0.27545]
4571 [ 0.105 , 0.34650]
4572 [ 0.120 , 0.42049]
4573 [ 0.135 , 0.50188]
4574 [ 0.186 , 0.60307]
4575 [ 2.110 , 0.79083]
4576 [ 5.874 , 1.00271]
4577 [ 11.395 , 1.29643]
4578 [ 18.770 , 1.62054]
4579 [ 28.143 , 1.97516]
4580 [ -1 , -1 ] (max twenty pts)
4581 NHYDovf=[ "Co-P-OVF" ]
4582 *%-----|-----|
4583 ADD HYD NHYDsum=["corrig"], NHYDs to
add=["Co-P-OVF"+"Co-P"+"N_TO"+"MH105-JR"+"A8-MJ-JR"+"A9-MJ"+"A10-MJ"+"A11-MJ"+"A12-MJ"+"A
13-MJ"]
4584 *%-----|-----|
4585 SAVE HYD NHYD=["corrig"], # OF PCYCLES=[-1], ICASEsh=[1]
4586 HYD_COMMENT=["Total Flows at Corrigan Pond"]
4587 *%-----|-----|
4588 *#*****|
4589 *# Corrigan Pond 1
4590 *# - Rating curve obtained from Barrhaven South MSS modeling
4591 *# - Tributary Drainage Area to MSS Pond 1 = 145 ha
4592 *#*****|
4593 *ROUTE RESERVOIR NHYDout=["MS_P1"], NHYDin=["CORRIG"],
4594 * RDT=[1](min),
4595 * TABLE of ( OUTFLOW-STORAGE ) values
4596 * (cms) - (ha-m)
4597 * [ 0.0 , 0.0 ]
4598 * [ 0.06 , 0.58]
4599 * [ -1 , -1 ] (max twenty pts)
4600 * NHYDovf=["P1-OVF"]
4601 *%-----|-----|
4602 *ADD HYD NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
4603 *%-----|-----|
4604 *SAVE HYD NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
4605 * HYD_COMMENT=["Total Flows at Corrigan Drain"]
4606 *%-----|-----|
4607 *#
4608 *# Hydrograph from Corrigan Drain routed to Jockvale Road
4609 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
4610 *#
4611 ROUTE CHANNEL NHYDout=["N_MI"] ,NHYDin=["corrig"] ,
4612 RDT=[1](min),
4613 CHLGTH=[580](m), CHSLOPE=[0.4448](%),
4614 FPSLOPE=[0.4448](%),
4615 SECNUM=[1.0], NSEG=[3]
4616 ( SEGROUGH, SEGDIST (m))=
4617 [0.075,-17.72
4618 -0.045,17.72
4619 0.075,80.62] NSEG times
4620 ( DISTANCE (m), ELEVATION (m))=
4621 [-83.32, 90.00]
4622 [-81.36, 89.50]
4623 [-79.12, 89.00]
4624 [-76.13, 88.50]
4625 [-20.46, 88.00]
4626 [-19.36, 87.50]
4627 [-18.51, 87.00]
4628 [-17.72, 86.50]

```

4629 [-11.95, 85.24]
4630 [-0.11, 85.12]
4631 [11.49, 85.20]
4632 [17.72, 86.50]
4633 [19.74, 87.00]
4634 [21.22, 87.50]
4635 [22.68, 88.00]
4636 [24.28, 88.50]
4637 [26.79, 89.00]
4638 [71.98, 90.00]
4639 [80.62, 90.50]

4640 *%-----|-----|
4641 *#*****|
4642 *# Catchment MILLS
4643 *# - To SWM Facility north of the Jock
4644 *# - Primarily residential development
4645 *#*****|

4646 **CONTINUOUS STANDHYD** NHYD=["MILLS"], DT=[1]min, AREA=[175.99](ha),
4647 XIMP=[0.38], TIMP=[0.38], DWF=[0](cms), LOSS=[2],
4648 SCS curve number CN=[74],
4649 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4650 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4651 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4652 LGI=[1118.123](m), MNI=[0.013], SCI=[0](min),
4653 Continuous simulation parameters:
4654 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4655 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4656 InterEventTime=[18](hrs), END=-1

4657 *%-----|-----|
4658 *#*****|
4659 *# Chapman Mills SWM Pond
4660 *# - Rating curve obtained from CCL hydraulic modeling
4661 *#*****|

4662 **ROUTE RESERVOIR** NHYDout=["MILL_P"], NHYDin=["MILLS"],
4663 RDT=[1](min),
4664 TABLE of (OUTFLOW-STORAGE) values
4665 (cms) - (ha-m)
4666 [0.0 , 0.0]
4667 [0.01 , 0.01]
4668 [0.05 , 0.06]
4669 [0.09 , 0.11]
4670 [0.13 , 0.15]
4671 [0.18 , 0.19]
4672 [0.28 , 0.28]
4673 [0.37 , 0.34]
4674 [0.45 , 0.40]
4675 [0.51 , 0.44]
4676 [0.56 , 0.47]
4677 [0.64 , 0.52]
4678 [0.76 , 0.59]
4679 [0.86 , 0.65]
4680 [1.09 , 0.78]
4681 [1.44 , 0.96]
4682 [3.18 , 1.84]
4683 [4.05 , 2.31]
4684 [-1 , -1] (max twenty pts)

4685 NHYDovf=["MIL-OV"]
4686 *%-----|-----|
4687 **ADD HYD** NHYDsum=["SN_MI"], NHYDs to add=["N_MI"+"MIL-OV"+"MILL_P"]
4688 *%-----|-----|
4689 **SAVE HYD** NHYD=["SN_MI"], # OF PCYCLES=[-1], ICASEsh=[1]
4690 HYD_COMMENT=["Total Flows at Jockvale Road"]

4691 *%-----|-----|
4692 *#
4693 *# Hydrograph from Jockvale Road routed to Heart's Desire
4694 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689

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4695 *#
4696 ROUTE CHANNEL NHYDout=["N_DE"] ,NHYDin=["SN_MI"] ,
4697 RDT=[1](min),
4698 CHLGTH=[1962](m), CHSLOPE=[0.2227](%),
4699 FPSLOPE=[0.2227](%),
4700 SECNUM=[1.0], NSEG=[3]
4701 ( SEGROUGH, SEGDIST (m))=
4702 [0.075,-17.56
4703 -0.045,18.27
4704 0.075,32.51] NSEG times
4705 ( DISTANCE (m), ELEVATION (m))=
4706 [-54.07, 85.00]
4707 [-39.43, 84.50]
4708 [-28.30, 84.00]
4709 [-24.12, 83.50]
4710 [-22.30, 83.00]
4711 [-20.55, 82.50]
4712 [-17.56, 82.00]
4713 [-12.63, 81.22]
4714 [-0.11, 80.75]
4715 [11.55, 81.22]
4716 [18.27, 82.00]
4717 [19.82, 82.50]
4718 [22.48, 83.00]
4719 [27.90, 83.50]
4720 [29.31, 84.00]
4721 [30.81, 84.50]
4722 [32.51, 85.00]
4723 *%-----|-----|
4724 *#*****|
4725 *# Catchment DESIRE
4726 *# - To Jock River (north of the Jock)
4727 *# - Rural-estate subdivision (Heart's Desire Community)
4728 *#*****|
4729 CONTINUOUS STANDHYD NHYD=["DESIRE"], DT=[1]min, AREA=[23.78](ha),
4730 XIMP=[0.25], TIMP=[0.25], DWF=[0](cms), LOSS=[2],
4731 SCS curve number CN=[77],
4732 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4733 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4734 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4735 LGI=[400](m), MNI=[0.013], SCI=[0](min),
4736 Continuous simulation parameters:
4737 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4738 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4739 InterEventTime=[18](hrs), END=-1
4740 *%-----|-----|
4741 *#*****|
4742 *# Catchment JOCKVA
4743 *# - To Jockvale SWM Facility
4744 *# - Residential development & golf course
4745 *# - JFSA 2021-01-11 update JOCKVA after updating CORRIG as per IBI GROUP, July 2008.
4746 *# JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two
areas JOCKVA and EX-LAND 32.5 ha as per IBI GROUP, July 2008.
4747 *#*****|
4748 CONTINUOUS STANDHYD NHYD=["JOCKVA"], DT=[1]min, AREA=[225.13](ha),
4749 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
4750 SCS curve number CN=[74],
4751 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4752 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4753 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4754 LGI=[1310.55](m), MNI=[0.013], SCI=[0](min),
4755 Continuous simulation parameters:
4756 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4757 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4758 InterEventTime=[18](hrs), END=-1
4759 *%-----|-----|

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4760 ADD HYD          NHYDsum=["JOCKVA-TO"], NHYDs to
add=["EX-LAND-MN"+"JOCKVA"+"B2-MJ"+"B3-MJ"]
4761 *%-----|-----|
4762 SAVE HYD          NHYD=["JOCKVA-TO"], # OF PCYCLES=[-1], ICASEsh=[1]
4763 HYD_COMMENT=["Total Flows at KB first pond"]
4764 *%-----|-----|
4765 *#*****|
4766 *#   Jockvale SWM Facility
4767 *#   - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
4768 *#*****|
4769 ROUTE RESERVOIR  NHYDout=["JOCK_P"], NHYDin=["JOCKVA-TO"],
4770 RDT=[1](min),
4771             TABLE of ( OUTFLOW-STORAGE ) values
4772             (cms) - (ha-m)
4773             [ 0.0 , 0.0 ]
4774             [ 0.27 , 0.03]
4775             [ 0.28 , 0.55]
4776             [ 0.29 , 1.14]
4777             [ 0.30 , 1.80]
4778             [ 0.31 , 2.32]
4779             [ 1.12 , 2.87]
4780             [ 2.92 , 3.45]
4781             [ 4.64 , 4.07]
4782             [ 6.69 , 4.72]
4783             [ 9.02 , 5.39]
4784             [ 11.62 , 6.10]
4785             [ 14.42 , 6.85]
4786             [ 17.45 , 7.62]
4787             [ 20.69 , 8.44]
4788             [ 24.08 , 9.28]
4789             [ 27.68 , 10.17]
4790             [ -1 , -1 ] (max twenty pts)
4791             NHYDovf=["JO-OVF"]
4792 *%-----|-----|
4793 ADD HYD          NHYDsum=["SN_DE"], NHYDs to add=["N_DE"+"DESIRE"+"JO-OVF"+"JOCK_P"]
4794 *%-----|-----|
4795 SAVE HYD          NHYD=["SN_DE"], # OF PCYCLES=[-1], ICASEsh=[1]
4796 HYD_COMMENT=["Total Flows at Heart's Desire"]
4797 *%-----|-----|
4798 *#
4799 *# Hydrograph from Heart's Desire routed to Rideau River
4800 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
4801 *#
4802 ROUTE CHANNEL    NHYDout=["N1"] ,NHYDin=["SN_DE"] ,
4803 RDT=[1](min),
4804 CHLGTH=[563](m), CHSLOPE=[0.9668](%),
4805             FPSLOPE=[0.9668](%),
4806 SECNUM=[1.0], NSEG=[3]
4807 ( SEGROUGH, SEGDIST (m))=
4808 [0.075,-30.20
4809 -0.045,30.20
4810 0.075,48.48] NSEG times
4811 ( DISTANCE (m), ELEVATION (m))=
4812 [-98.46, 81.50]
4813 [-92.24, 81.00]
4814 [-86.88, 80.50]
4815 [-81.54, 80.00]
4816 [-74.36, 79.50]
4817 [-63.54, 79.00]
4818 [-39.23, 78.50]
4819 [-34.51, 78.00]
4820 [-33.01, 77.50]
4821 [-30.20, 77.00]
4822 [-13.42, 76.18]
4823 [-1.14, 76.09]
4824 [17.06, 76.18]

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4825         [30.20, 77.00]
4826         [32.95, 77.50]
4827         [34.06, 78.00]
4828         [35.11, 78.50]
4829         [36.32, 79.00]
4830         [37.74, 79.50]
4831         [48.48, 81.50]
4832 *%-----|-----|
4833 *#*****|*****|
4834 *#   Catchment S-2
4835 *#   - To Jock River (north and south)
4836 *#   - Undeveloped floodplain and river
4837 *#*****|*****|
4838 CONTINUOUS NASHYD  NHYD=["S-2"], DT=[1]min, AREA=[102.94](ha),
4839                   DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
4840                   N=[3], TP=[0.40]hrs,
4841                   Continuous simulation parameters:
4842                   IaREcper=[4](hrs),
4843                   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4844                   InterEventTime=[12](hrs)
4845                   Baseflow simulation parameters:
4846                   BaseFlowOption=[1] ,
4847                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4848                   VHydCond=[0.055](mm/hr),   END=-1
4849 *%-----|-----|
4850 ADD HYD           NHYDsum=["SN_N1"], NHYDs to add=["N1"+"S-2"]
4851 *%-----|-----|
4852 SAVE HYD        NHYD=["SN_N1"], # OF PCYCLES=[-1], ICASEsh=[1]
4853                   HYD_COMMENT=["Total Flows at Rideau River"]
4854 *%-----|-----|
4855 *#####|#####|
4856 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4857 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
4858 *%           ["C24SC005.stm"] <--storm filename, one per line for NSTORM time
4859 *%-----|-----|
4860 *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4861 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
4862 *%           ["C24SC010.stm"] <--storm filename, one per line for NSTORM time
4863 *%-----|-----|
4864 *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4865 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
4866 *%           ["C24SC025.stm"] <--storm filename, one per line for NSTORM time
4867 *%-----|-----|
4868 *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4869 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
4870 *%           ["C24SC050.stm"] <--storm filename, one per line for NSTORM time
4871 *%-----|-----|
4872 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4873 *START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4874 *%           ["100YC3H.STM"] <--storm filename, one per line for NSTORM time
4875 *%-----|-----|
4876 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4877 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4878 *%           ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
4879 *%-----|-----|
4880 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4881 *START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4882 *%           ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
4883 *START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[101]
4884 *%           ["A24SC100.stm"] <--storm filename, one per line for NSTORM time
4885 *START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
4886 *%           ["A24SC100_60.stm"] <--storm filename, one per line for NSTORM time
4887 FINISH
4888

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00375# #
00376# R002/C00039 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00377# ADD HYD + 1.0 02:01M1 9506.00 7.472_MDate 33:12 11.20 n/a .000
00378# ROUTE CHANNEL -> 1.0 02:01M1 500.00 2.720_MDate 29:22 11.98 n/a .000
00379# + 1.0 02:01M2 1917.00 4.042_MDate 34:34 11.98 n/a .000
00380# SUM= 1.0 01:01M1 11923.00 12.077_MDate 33:14 11.36 n/a .000
00381#
00382# Sum of hydrographs from Node 11 routed to Node 10
00383# Section 1
00384#
00385# R002/C00040 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00386# ROUTE CHANNEL -> 1.0 02:01M1 11923.00 12.077_MDate 33:14 11.36 n/a .000
00387# [RDP= 1.00] out<= 1.0 01:01M0 11923.00 8.276_MDate 39:46 11.36 n/a .000
00388# [L/S= 2429 / .057/.040]
00389# [Vmax=.462;Dmax=.886]
00390#
00391# Addition of Subwatershed 10 to Node 10
00392#
00393# R002/C00041 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00394# ADD HYD + 1.0 02:01M0 11923.00 8.276_MDate 39:46 11.36 n/a .000
00395# + 1.0 02:01M1 5646.00 11.228_MDate 38:07 13.94 n/a .000
00396# SUM= 1.0 01:01M0 17589.00 19.451_MDate 38:31 12.19 n/a .000
00397# R002/C00042 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00398# SAVE HYD + 1.0 01:01M0 17589.00 19.451_MDate 38:31 12.19 n/a .000
00399# frame :H_SMI0
00400# remark:flow at_R_N10: M10 + SW_10
00401# Addition of Kings Creek to S_M10
00402#
00403# R002/C00043 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00404# ADD HYD + 1.0 02:01M0 17589.00 19.451_MDate 38:31 12.19 n/a .000
00405# + 1.0 02:01M1 8376.00 11.072_MDate 39:59 11.98 n/a .000
00406# SUM= 1.0 01:01M0 25965.00 30.328_MDate 39:58 12.12 n/a .000
00407#
00408# Sum of hydrographs from Node 10 routed to Node 9
00409# Section 2
00410#
00411# R002/C00044 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00412# ROUTE CHANNEL -> 1.0 02:01M0 25965.00 30.328_MDate 39:58 12.12 n/a .000
00413# [RDP= 1.00] out<= 1.0 01:01M9 25965.00 29.579_MDate 39:59 12.12 n/a .000
00414# [L/S= 3922 / .075/.040]
00415# [Vmax=.595;Dmax=1.208]
00416#
00417# Addition of Subwatershed 9 and Nichols Creek to Node 9
00418#
00419# R002/C00045 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00420# ADD HYD + 1.0 02:01M9 25965.00 29.579_MDate 39:59 12.12 n/a .000
00421# + 1.0 02:01M9 1132.00 4.484_MDate 39:59 12.12 n/a .000
00422# + 1.0 02:01M2 4464.00 5.504_MDate 39:59 10.98 n/a .000
00423# SUM= 1.0 01:01M9 31561.00 36.313_MDate 39:59 12.00 n/a .000
00424#
00425# Sum of hydrographs from Node 9 routed to Node 8
00426# Section 3
00427#
00428# R002/C00046 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00429# ROUTE CHANNEL -> 1.0 02:01M9 31561.00 36.313_MDate 39:59 12.00 n/a .000
00430# [RDP= 1.00] out<= 1.0 01:01M8 31561.00 34.173_MDate 39:59 12.00 n/a .000
00431# [L/S= 4028 / .087/.040]
00432# [Vmax=.418;Dmax=1.281]
00433#
00434# Addition of Subwatershed 8 and Bobb's Drain to Node 8
00435#
00436# R002/C00047 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00437# ADD HYD + 1.0 02:01M8 31561.00 34.173_MDate 39:59 12.00 n/a .000
00438# + 1.0 02:01M8 31561.00 34.173_MDate 39:59 12.00 n/a .000
00439# + 1.0 02:01M2 3854.00 6.242_MDate 38:46 11.98 n/a .000
00440# SUM= 1.0 01:01M8 35466.00 40.474_MDate 39:59 12.00 n/a .000
00441#
00442# Sum of hydrographs from Node 8 routed to Node 7
00443# Section 4
00444#
00445# R002/C00048 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00446# ROUTE CHANNEL -> 1.0 02:01M8 35466.00 40.474_MDate 39:59 12.00 n/a .000
00447# [RDP= 1.00] out<= 1.0 01:01M7 35466.00 32.892_MDate 44:30 12.00 n/a .000
00448# [L/S= 3750 / .053/.070]
00449# [Vmax=.208;Dmax=1.651]
00450#
00451# Addition of Subwatershed 7 to Node 7
00452#
00453# R002/C00049 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00454# ADD HYD + 1.0 02:01M7 35466.00 32.892_MDate 44:30 12.00 n/a .000
00455# + 1.0 02:01M7 3197.00 4.641_MDate 36:31 9.45 n/a .000
00456# SUM= 1.0 01:01M7 38743.00 35.071_MDate 43:33 11.82 n/a .000
00457# R002/C00050 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00458# SAVE HYD + 1.0 01:01M7 38743.00 35.071_MDate 43:33 11.82 n/a .000
00459# frame :H_LBPT
00460# remark:flow at_R_N7: M7 + SW_7
00461# Insertion of a Reservoir to simulate the effects of the Richmond Fen.
00462# Storage area and volumes were estimated from available topo maps.
00463# Release rate from Fen was assumed to be controlled by the downstream
00464# a river cross-section for which it is was assumed that for up to
00465# 0.75 m of water, the main channel of the river provided the storage. Above
00466# this depth, the wetland starts to significantly store water.
00467#
00468# R002/C00051 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00469# ROUTE CHANNEL -> 1.0 02:01M7 38743.00 35.071_MDate 43:33 11.82 n/a .000
00470# out<= 1.0 01:RES_RP 38743.00 23.265_MDate 55:09 11.82 n/a .000
00471# [Med:0.000;7001.00]
00472# R002/C00052 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00473# SAVE HYD + 1.0 01:RES_RP 38743.00 23.265_MDate 55:09 11.82 n/a .000
00474# frame :H_ResFp
00475# remark:outflow of Richmond Fen
00476#
00477# Sum of hydrographs from Node 7 routed to Node 6
00478# Section 5
00479#
00480# R002/C00053 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00481# ROUTE CHANNEL -> 1.0 02:01M7 38743.00 23.265_MDate 55:09 11.82 n/a .000
00482# [RDP= 1.00] out<= 1.0 01:01M6 38743.00 23.228_MDate 56:38 11.82 n/a .000
00483# [L/S= 3036 / .081/.040]
00484# [Vmax=.432;Dmax=.808]
00485#
00486# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
00487#
00488# R002/C00054 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00489# ADD HYD + 1.0 02:01M6 38743.00 23.228_MDate 56:38 11.82 n/a .000
00490# + 1.0 02:01M6 1455.00 4.812_MDate 28:12 12.20 n/a .000
00491# + 1.0 02:01M2 1332.00 3.148_MDate 35:23 13.94 n/a .000
00492# SUM= 1.0 01:01M6 40949.00 31.186_MDate 39:59 11.89 n/a .000
00493#
00494# Sum of hydrographs from Node 6 routed to Node 5
00495# Section 6
00496#
00497# R002/C00055 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00498# ROUTE CHANNEL -> 1.0 02:01M6 40949.00 31.186_MDate 39:59 11.89 n/a .000
00499# [RDP= 1.00] out<= 1.0 01:01M5 40949.00 31.285_MDate 56:09 11.89 n/a .000
00500# [L/S= 3036 / .057/.040]
00501# [Vmax=.378;Dmax=.917]
00502#
00503# Addition of Subwatershed 5 and Flowing Creek to Node 5
00504#
00505# R002/C00056 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00506# ADD HYD + 1.0 02:01M5 40949.00 31.285_MDate 56:09 11.89 n/a .000
00507# + 1.0 02:01M5 224.94 2.587_MDate 28:12 12.20 n/a .000
00508# + 1.0 02:01M2 4949.00 14.839_MDate 33:25 14.57 n/a .000
00509# SUM= 1.0 01:01M5 45408.00 31.166_MDate 37:08 12.20 n/a .000
00510#
00511# Sum of hydrographs from Node 5 routed to Node 5A
00512# Section 7
00513#
00514# R002/C00057 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00515# ROUTE CHANNEL -> 1.0 02:01M5 45409.00 31.166_MDate 37:08 12.20 n/a .000
00516# [RDP= 1.00] out<= 1.0 01:01M5A 45409.00 31.335_MDate 37:10 12.20 n/a .000
00517# [L/S= 556 / .090/.040]
00518# [Vmax=.443;Dmax=.937]
00519#
00520# Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
00521#
00522# R002/C00058 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00523# ADD HYD + 1.0 02:01M5A 45409.00 31.335_MDate 37:10 12.20 n/a .000
00524# + 1.0 02:01M5A 20.00 .000_MDate 28:16 12.20 n/a .000
00525# + 1.0 02:01M5A 1622.00 3.090_MDate 38:04 15.22 n/a .000
00526# SUM= 1.0 01:01M5A 46841.00 36.236_MDate 37:28 12.30 n/a .000
00527#
00528# Sum of hydrographs from Node 5A routed to Node 4
00529# Section 8
00530#
00531# R002/C00059 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00532# ROUTE CHANNEL -> 1.0 02:01M5A 46841.00 36.236_MDate 37:28 12.30 n/a .000
00533# [RDP= 1.00] out<= 1.0 01:01M4 46841.00 35.288_MDate 37:12 12.30 n/a .000
00534# [L/S= 4630 / .043/.035]
00535# [Vmax=.695;Dmax=2.444]
00536#
00537# Addition of Subwatershed 4 and Leamy Creek to Node 4
00538#
00539# R002/C00060 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00540# ADD HYD + 1.0 02:01M4 46841.00 35.288_MDate 37:12 12.30 n/a .000
00541# + 1.0 02:01M4 585.00 4.325_MDate 29:58 17.79 n/a .000
00542# + 1.0 02:01M2 1021.00 5.747_MDate 30:50 17.39 n/a .000
00543# SUM= 1.0 01:01M4 48447.00 37.581_MDate 38:13 12.47 n/a .000
00544# R002/C00061 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00545# SAVE HYD + 1.0 01:01M4 48447.00 37.581_MDate 38:13 12.47 n/a .000
00546# frame :R_4.0.0002
00547# remark:flow at_R_N4
00548#
00549# Sum of hydrographs from Node 4 routed to Node 2
00550# Section 9
00551#
00552# R002/C00062 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00553# ROUTE CHANNEL -> 1.0 02:01M4 48447.00 37.581_MDate 38:13 12.47 n/a .000
00554# [RDP= 1.00] out<= 1.0 01:01M2 48447.00 37.455_MDate 38:49 12.47 n/a .000
00555# [L/S= 1467 / .060/.040]
00556# [Vmax=.715;Dmax=2.485]
00557#
00558# Addition of Subwatershed 2 with Monahan Drain and Smith Drain to Node 2
00559#
00560# R002/C00063 -----DtnIn-ID:HYD-----AREAh-QPEARcMs-TPeakDate,h:mm-----RvM-R,C-----DWPFms
00561# ADD HYD + 1.0 02:01M2 48447.00 37.455_MDate 38:49 12.47 n/a .000

Table with columns for station ID (e.g., 00749, 00750), parameters (e.g., [XIMP:68.TIMP:85]), flow characteristics (e.g., [Horton parameters: Fw=76.20Pfc=13.20]), and flow rates (e.g., 1.00127, 3.51, 148.00). The table contains multiple rows of data for various stations, including flow rates, velocities, and other hydraulic parameters.

Table with columns for ID, Description, and Date. Rows contain detailed engineering data including flow rates, dates, and technical specifications for various systems like 'CONTINUOUS STANDBY', 'ROUTING CHANNEL', and 'COMPUTE DRAWDY'.

Table of data entries with columns for ID (e.g., 022425), description (e.g., [IARC] 4.00: IARCPer = 4.00), and various numerical values. The table contains multiple sections of data, including model parameters, routing pipes, and flow calculations. The text is partially cut off on the right side.

026139 # Catchment DESIRE
026140 # To Jock River (north of the Jock)
026141 # Rural-estate subdivision (Heart's Desire Community)
02622 # *****
02623 # R0002-CO0393-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02624 # CONTINUOUS NASHVD 1.0 01:SR 23.78 .936 No_date 28:03 19:26 423 .000
02625 # [XMP=50;TMD=25]
02626 # [LQSE=2;CN=77.0]
02627 # [IperV=4.67;SLIP=1.00;LIP= 40.0;MP=250;SCP= 0]
02628 # [IperV=4.67;SLIP=1.57;SLIP=1.00;LIP= 400.0;MI=013;SCI= 0]
02629 # [IAREC= 4.00;SMIN= 39.75;SMAX=264.99;SK= 0.010]
02630 # [SMIN= 31.15;SMAX= 8.00;T= 0.010]
02631 # [InterVTime= 12.00]
02632 # *****
02633 # Catchment JCKVA
02634 # To Jockvale SWM Facility
02635 # Residential development & golf course
02636 # JPSA 2021-01 update JOCKVA after updating CORRIE as per IRI CORRIE, July 2008.
02637 # JockVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX-LAND 32.5 ha as
02638 # *****
02639 # R0002-CO0394-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02640 # CONTINUOUS NASHVD 1.0 01:SR 225.13 10.636 No_date 28:10 26:45 490 .000
02641 # [XMP=50;TMD=50]
02642 # [LQSE=2;CN=74.0]
02643 # [IperV=4.67;SLIP=1.00;LIP= 40.0;MP=250;SCP= 0]
02644 # [IperV=4.67;SLIP=1.57;SLIP=1.00;LIP=1311.0;MI=013;SCI= 0]
02645 # [IAREC= 4.00;SMIN= 39.75;SMAX=264.99;SK= 0.010]
02646 # [SMIN= 36.67;SMAX=244.49;SK= 0.010]
02647 # ADD HYD + 1.0 02:IR-LAND-MN 32.50 2.093 No_date 28:02 26:85 n/a .000
02648 # + 1.0 02:JCKVA 225.13 10.636 No_date 28:10 26:85 n/a .000
02649 # [I/S= 1.0 02:IR-MJ 0.00 0.000 No_date 0:00 .00 n/a .000
02650 # + 1.0 02:IR-MJ 0.00 0.000 No_date 0:00 .00 n/a .000
02651 # SUM= 1.0 01:SR_ML 257.63 12.234 No_date 28:07 26:85 n/a .000
02652 # R0002-CO0396-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02653 # SAVE HYD 1.0 01:JCKVA-TO.0002 257.63 12.234 No_date 28:07 26:85 n/a .000
02654 # fname JCKVA-TO.0002
02655 # remark:Total Flows at KB first pond
02656 # *****
02657 # Jockvale SWM Facility
02658 # Rating curve obtained from Jockvale Servicing Study (CCL 1999)
02659 # *****
02660 # R0002-CO0399-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02661 # ROUTE RESERVOIR -> 1.0 02:JCKVA-TO 257.63 12.234 No_date 28:07 26:85 n/a .000
02662 # out <= 1.0 01:JCKV_P 257.63 2.660 No_date 29:05 26:85 n/a .000
02663 # overFlow= 1.0 02:OWF 102.94 1.373 No_date 28:20 13:01 n/a .000
02664 # [MstUsed=.333AE=0;L,ToVdV=0.000;N3,NOv=V 0,ToVdV=V.0;h=] *****
02665 # R0002-CO0398-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02666 # ADD HYD + 1.0 02:IR_DE 55194.85 49.134 No_date 38:33 13:21 n/a .000
02667 # + 1.0 02:DESIRE 23.78 .936 No_date 28:03 19:26 n/a .000
02668 # + 1.0 02:OWF 102.94 1.373 No_date 28:20 13:01 n/a .000
02669 # + 1.0 02:JCKV_P 257.63 2.660 No_date 29:05 26:85 n/a .000
02670 # SUM= 1.0 01:SR_ML 55476.26 49.482 No_date 38:27 13:27 n/a .000
02671 # R0002-CO0399-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02672 # SAVE HYD 1.0 01:SR_DE 55476.26 49.482 No_date 38:27 13:27 n/a .000
02673 # fname SR_DE.0002
02674 # remark:Total Flows at Heart's Desire
02675 # *****
02676 # Hydrograph from Heart's Desire routed to Rideau River
02677 # Channel X-section obtained from RWQ hydrologic Model - Station 0
02678 # *****
02679 # R0002-CO0400-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02680 # ROUTE CHANNEL -> 1.0 02:IR_DE 55476.26 49.482 No_date 38:27 13:27 n/a .000
02681 # [RFD= 1.00] out <= 1.0 01:MI 55476.26 49.482 No_date 38:33 13.27 n/a .000
02682 # [I/S= 563 / 967;D=63]
02683 # [Vmax= 1.489;Dmax= .800]
02684 # [InterVTime= 12.00]
02685 # Catchment S-2
02686 # To Jock River (north and south)
02687 # Undeveloped floodplain
02688 # *****
02689 # R0002-CO0401-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02690 # CONTINUOUS NASHVD 1.0 01:SR 102.94 1.373 No_date 28:20 13:01 286 .000
02691 # [CN= 72.0; N= 3.00; T= 0.010]
02692 # [IAREC= 4.00; SMIN= 39.75; SMAX=264.99; SK= 0.010]
02693 # [InterVTime= 12.00]
02694 # R0002-CO0402-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02695 # ADD HYD + 1.0 02:IR 55476.26 49.480 No_date 38:33 13.27 n/a .000
02696 # + 1.0 02:SR 102.94 1.373 No_date 28:20 13.01 n/a .000
02697 # + 1.0 01:SR_ML 55579.20 49.578 No_date 38:33 13.27 n/a .000
02698 # SUM= 1.0 01:SR_ML 55579.20 49.578 No_date 38:33 13.27 n/a .000
02699 # R0002-CO0403-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02700 # SAVE HYD 1.0 01:SR_ML 55579.20 49.578 No_date 38:33 13.27 n/a .000
02701 # fname SR_ML.0002
02702 # remark:Total Flows at Rideau River
02703 # *****
02704 # ** END OF RUN : 4
02705 # *****
02706 # *****
02707 # *****
02708 # *****
02709 # *****
02710 # *****
02711 # RNN=COMMANDS
02712 # R0002-CO0001-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02713 # START
02714 # [TZERO = .00 hrs on 0]
02715 # [INSTOR = 1 (1=imperial, 2=metric output)]
02716 # [INSTORM = 1]
02717 # [RNN = 0005]
02718 # *****
02719 # SWMNOY Ver:02/Jan 2001 -SETA / INPUT DATA FILE
02720 # *****
02721 # Project Name [Jock River] Project Number: [1474-16]
02722 # Date [01/01/2021]
02723 # Modeller [M.M.]
02724 # Company JFSaInc.
02725 # License # 254923
02726 # *****
02727 # CALIBRATION & CONVERSION SECTION
02728 # USING CONTINUOUS SIMULATIONS
02729 # Rainfall data from FFS rain gauge installed at site + other gauges by City
02730 # Use data collected from May 1st to July 14, 2003
02731 # 2020-11-30 change TMDST0 IN COMPUTE DUALYD (TMDST= 0.1 instead of 0.0001)
02732 # 2020-11-30 correct soil curve
02733 # 2020-11-01 change W_CLEAR_SBR_XIMP to 0.55, SLP([0.5]) (impervious slope), and LQI up to 70m
02734 # 2021-02-19 change the slope for ROUTE CHANNEL (MSTOR= "R.01") (impervious slope), and LQI up to 70m
02735 # 2021-02-19 Change the slope for ROUTE CHANNEL SECTION 5002 (INHVD="M.UM"), (MHYD="SM.01") from 0.03 & (as per ST
02736 # *****
02737 # R0002-CO0002-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02738 # READ STORM
02739 # filename storm.001
02740 # Comment # pluie SCS de 24 hrs 1:5 as pour Ottawa CDA
02741 # [I/S= 1.0;R= 24.00;T= 96.00; D= 17.12]
02742 # R0002-CO0003-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02743 # MODIFY STORM
02744 # IAREC= 1.0 02:IRHTF= 96.00 min]
02745 # [RFD=1.00] out <= 1.0 01:PROD= 57.12]
02746 # R0002-CO0004-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02747 # DEFAULT VALUES
02748 # filename T:\WORK\1474-16\Design\20201026-QuantityControlAnalysis\SWMNOY\SRM-Model\updated\Citigade.DEP
02749 # [C=0.05] V= 1 (read and print data)
02750 # filename File comment: [Based on various calibration exercises in Ont
02751 # [R= 1.0] USE THE PARAMETER ARE USED BY THE REGION STANDARD COM
02752 # Horton's infiltration equation parameters:
02753 # [F= 76.20 mm/hr] [F= 10.20 mm/hr] [CN2= 4.14 /h] [P= .00 mm]
02754 # Parameters for DEVI008 surfaces in STANHYD:
02755 # [IperV= 4.67 mm] [LQ=50.00 ml] [MP= 230]
02756 # Parameters for IMPREV008 surfaces in STANHYD:
02757 # [Ialmp= 1.57 mm] [CL=1.50] [MNI= .013]
02758 # Parameters used in NASHVD:
02759 # [Ia= 4.67 mm] [N= 3.00]
02760 # Average monthly Pan Evaporation data in (mm)
02761 # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
02762 # 00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
02763 # Average monthly Potential Evapotranspiration in (mm)
02764 # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
02765 # 00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
02766 # R0002-CO0005-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02767 # COMPUTE AP
02768 # [Iare= 50.00; APIDky= .8500; APIDkz= .9989]
02769 # [APIMax= 90.83; APITavg= 60.09; APIDmin= 44.87]
02770 # *****
02771 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02772 # of 1.32
02773 # R0002-CO0006-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02774 # CONTINUOUS NASHVD 1.0 01:SR_RW 3680.00 9.398 No_date 37:02 16.41 287 .000
02775 # [CN= 60.0; N= 3.00; T= 1.31]
02776 # [IAREC= 4.00; SMIN= 39.75; SMAX=380.32; SK= 0.010]
02777 # [InterVTime= 12.00]
02778 # *****
02779 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02780 # of 1.32
02781 # R0002-CO0007-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02782 # CONTINUOUS NASHVD 1.0 01:SR_RW 3771.00 9.405 No_date 32:36 15.29 268 .000
02783 # [CN= 61.0; N= 3.00; T= 1.76]
02784 # [IAREC= 4.00; SMIN= 39.75; SMAX=430.01; SK= 0.010]
02785 # [InterVTime= 12.00]
02786 # *****
02787 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02788 # of 1.80
02789 # R0002-CO0008-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02790 # CONTINUOUS NASHVD 1.0 01:SR_RM 3074.00 4.682 No_date 39:59 13.23 232 .000
02791 # [CN= 57.0; N= 3.00; T= 1.24]
02792 # [IAREC= 4.00; SMIN= 83.24; SMAX=554.98; SK= 0.010]
02793 # [InterVTime= 12.00]
02794 # R0002-CO0009-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02795 # CONTINUOUS NASHVD 1.0 01:SR_ASH 1781.00 8.521 No_date 32:43 20.12 352 .000
02796 # [CN= 72.0; N= 3.00; T= 1.31]
02797 # [IAREC= 4.00; SMIN= 39.75; SMAX=264.99; SK= 0.010]
02798 # [InterVTime= 12.00]
02799 # *****
02800 # R0002-CO0010-----Dtain-ID:HYD-----AREHA-GPEAKcm-TeakDate_hh:mm-----RvM-R.C-----DWfcm
02801 # CONTINUOUS NASHVD 1.0 01:SR_M1 500.00 4.354 No_date 29:22 17.18 301 .000
02802 # [CN= 60.0; N= 3.00; T= 1.24]
02803 # [IAREC= 4.00; SMIN= 52.62; SMAX=350.79; SK= 0.010]
02804 # [InterVTime= 12.00]
02805 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

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02993 # Sum of hydrographs from Node 12 routed to Node 11
02994 # (Approximated cross-section to same cross-section 251)
02995 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
02996
02997 # Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
02998
02999 R005/C00038 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03000 ROUTE CHANNEL -> 1.0 02:R_M12 9506.00 10.498_NDate 32:46 15.96 n/a .000
03001 [RDT 1.0] out-> 1.0 01:R_M11 9506.00 10.383_NDate 33:07 15.96 n/a .000
03002 [L/S/n= 972./ .054/.040]
03003 [Vmax=.648;Dmax=2.466]
03004
03005 # Addition of Subwatershed 11 and No Name Creek to Node 11
03006
03007 R005/C00039 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03008 ADD HYD + 1.0 02:R_M10 11923.00 12.006_NDate 38:17 16.21 n/a .000
03009 SUM + 1.0 02:R_M11 9506.00 10.383_NDate 33:07 15.96 n/a .000
03010 + 1.0 02:R_M10 11923.00 12.006_NDate 38:17 16.21 n/a .000
03011 + 1.0 02:R_M11 9506.00 10.383_NDate 33:07 15.96 n/a .000
03012 SUM 1.0 01:R_M11 11923.00 12.006_NDate 38:17 16.21 n/a .000
03013 # Sum of hydrographs from Node 11 routed to Node 10
03014 # Section 1
03015
03016 R005/C00040 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03017 ROUTE CHANNEL -> 1.0 02:R_M11 11923.00 12.006_NDate 38:17 16.21 n/a .000
03018 [RDT 1.0] out-> 1.0 01:R_M10 11923.00 12.006_NDate 38:17 16.21 n/a .000
03019 [L/S/n=14028./ .157/.040]
03020 [Vmax=.461;Dmax=1.087]
03021 #
03022 # Addition of Subwatershed 10 to Node 10
03023
03024 R005/C00041 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03025 ADD HYD + 1.0 02:R_M10 11923.00 12.006_NDate 38:17 16.21 n/a .000
03026 + 1.0 02:R_M10 5666.00 16.924_NDate 38:02 20.12 n/a .000
03027 SUM 1.0 02:R_M10 11923.00 12.006_NDate 38:17 16.21 n/a .000
03028 R005/C00042 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03029 SAVE HYD 1.0 01:R_M10 17589.00 28.927_NDate 38:09 17.47 n/a .000
03030 fname_H_S10
03031 remark:flow at S_M10: M10 + SW_10
03032 # Addition of Kings Creek to S_M10
03033
03034 R005/C00043 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03035 ADD HYD + 1.0 01:R_M10 17589.00 28.927_NDate 38:09 17.47 n/a .000
03036 + 1.0 02:R_M10 8376.00 16.342_NDate 39:59 17.38 n/a .000
03037 SUM 1.0 01:R_M10 25965.00 44.722_NDate 39:59 17.38 n/a .000
03038 #
03039 # Sum of hydrographs from Node 10 routed to Node 9
03040 # Section 2
03041
03042 R005/C00044 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03043 ROUTE CHANNEL -> 1.0 02:R_M10A 25965.00 44.722_NDate 39:59 17.37 n/a .000
03044 [RDT 1.0] out-> 1.0 02:R_M10 25965.00 44.722_NDate 39:59 17.37 n/a .000
03045 [L/S/n= 3982./ .075/.040]
03046 [Vmax=.664;Dmax=1.922]
03047 #
03048 # Addition of Subwatershed 9 and Nichols Creek to Node 9
03049
03050 R005/C00045 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03051 ADD HYD + 1.0 02:R_M9 31561.00 53.366_NDate 39:59 17.20 n/a .000
03052 + 1.0 02:R_M9 1132.00 6.963_NDate 30:55 19.24 n/a .000
03053 + 1.0 02:R_M9 4840.00 8.109_NDate 29:59 15.66 n/a .000
03054 SUM 1.0 01:R_M9 31663.00 53.366_NDate 39:59 17.20 n/a .000
03055 #
03056 # Sum of hydrographs from Node 9 routed to Node 8
03057 # Section 3
03058
03059 R005/C00046 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03060 ROUTE CHANNEL -> 1.0 02:R_M9 31561.00 53.366_NDate 39:59 17.20 n/a .000
03061 [RDT 1.0] out-> 1.0 02:R_M9 31561.00 53.366_NDate 39:59 17.20 n/a .000
03062 [L/S/n= 2269./ .089/.045]
03063 [Vmax=.570;Dmax=2.051]
03064 #
03065 # Addition of Subwatershed 8 and Hobbs' Drain to Node 8
03066
03067 R005/C00047 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03068 ADD HYD + 1.0 02:R_M8 31561.00 53.366_NDate 39:59 17.20 n/a .000
03069 + 1.0 02:R_M8 1351.00 1.298_NDate 28:57 16.03 n/a .000
03070 + 1.0 02:R_M8 4840.00 8.109_NDate 28:41 17.18 n/a .000
03071 SUM 1.0 01:R_M8 35546.00 58.845_NDate 39:59 17.19 n/a .000
03072 #
03073 # Sum of hydrographs from Node 8 routed to Node 7
03074 # Section 4
03075
03076 R005/C00048 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03077 ROUTE CHANNEL -> 1.0 02:R_M8 35546.00 58.845_NDate 39:59 17.19 n/a .000
03078 [RDT 1.0] out-> 1.0 01:R7 35546.00 48.127_NDate 45:08 17.19 n/a .000
03079 [L/S/n= 3780./ .051/.070]
03080 [Vmax=.208;Dmax=1.000]
03081 #
03082 # Addition of Subwatershed 7 to Node 7
03083
03084 R005/C00049 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03085 ADD HYD + 1.0 02:R_M7 35546.00 48.127_NDate 45:08 17.19 n/a .000
03086 + 1.0 02:R_M7 3137.00 7.027_NDate 36:28 13.89 n/a .000
03087 SUM 1.0 01:R_M7 38743.00 51.395_NDate 44:14 16.92 n/a .000
03088 R005/C00050 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03089 SAVE HYD 1.0 01:R_M7 38743.00 51.395_NDate 44:14 16.92 n/a .000
03090 fname_H_S10
03091 remark:flow at S_M7: M7 + SW_7
03092 # Insertion of a reservoir to simulate the effects of the Richmond Fen.
03093 # Storage area and volumes were estimated from available top maps.
03094 # Release rate from Fen was assumed to be controlled by the downstream
03095 # river cross-section for summer conditions. It is assumed that for up to
03096 # 0.75 m of water, the maximum of the river provided the storage. Above
03097 # this depth, the wetland starts to significantly store water.
03098
03099 R005/C00051 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03100 ROUTE RESERVOIR -> 1.0 02:R_M7 38743.00 51.395_NDate 44:14 16.92 n/a .000
03101 [RDT 1.0] out-> 1.0 01:R_RES_FP 38743.00 27.976_NDate 59:12 16.92 n/a .000
03102 [Method=1.784E+03]
03103 R005/C00052 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03104 SAVE HYD 1.0 01:R_RES_FP 38743.00 27.976_NDate 59:12 16.92 n/a .000
03105 fname_H_RESFP
03106 remark:flow of Richmond Fen
03107 #
03108 # Sum of hydrographs from Node 7 routed to Node 6
03109 # Section 5
03110
03111 R005/C00053 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03112 ROUTE CHANNEL -> 1.0 02:R_RES_FP 38743.00 27.976_NDate 59:12 16.92 n/a .000
03113 [RDT 1.0] out-> 1.0 01:R_RES_FP 38743.00 27.976_NDate 60:29 16.92 n/a .000
03114 [L/S/n= 3056./ .082/.025]
03115 [Vmax=.460;Dmax=.955]
03116 #
03117 # Addition of Subwatershed 6 and Van Gaa! Drain to Node 6
03118
03119 R005/C00054 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03120 ADD HYD + 1.0 02:R_M6 40240.01 27.922_NDate 60:29 17.03 n/a .000
03121 + 1.0 02:R_M6 185.00 .641_NDate 33:06 17.58 n/a .000
03122 + 1.0 01:R_M6 4840.00 8.109_NDate 28:41 17.18 n/a .000
03123 SUM 1.0 01:R_M6 40240.01 27.944_NDate 60:06 17.03 n/a .000
03124 #
03125 # Sum of hydrographs from Node 6 routed to Node 5
03126 # Section 6
03127
03128 R005/C00055 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03129 ROUTE CHANNEL -> 1.0 02:R_M6 40240.01 27.944_NDate 60:06 17.03 n/a .000
03130 [RDT 1.0] out-> 1.0 01:R_M5 40240.01 27.922_NDate 60:57 17.03 n/a .000
03131 [L/S/n= 1892./ .054/.035]
03132 [Vmax=.397;Dmax=1.002]
03133 #
03134 # Addition of Subwatershed 5 and Flowing Creek to Node 5
03135
03136 R005/C00056 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03137 ADD HYD + 1.0 02:R_M5 40240.01 27.922_NDate 60:57 17.03 n/a .000
03138 + 1.0 02:R_M5 224.00 4.102_NDate 28:45 22.97 n/a .000
03139 + 1.0 02:R_M5 4840.00 22.837_NDate 33:06 17.04 n/a .000
03140 SUM 1.0 01:R_M5 45409.01 43.866_NDate 35:28 17.49 n/a .000
03141 #
03142 # Sum of hydrographs from Node 5 routed to Node 5A
03143 # Section 7
03144
03145 R005/C00057 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03146 ROUTE CHANNEL -> 1.0 02:R_M5 45409.01 43.866_NDate 35:28 17.49 n/a .000
03147 [RDT 1.0] out-> 1.0 01:R5A 45409.01 43.850_NDate 35:47 17.49 n/a .000
03148 [L/S/n= 586./ .099/.040]
03149 [Vmax=.465;Dmax=1.060]
03150 #
03151 # Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
03152
03153 R005/C00058 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03154 ADD HYD + 1.0 01:R5A 45409.01 43.850_NDate 35:47 17.49 n/a .000
03155 + 1.0 02:R_M5A2 20.00 4.893_NDate 28:36 25.62 n/a .000
03156 + 1.0 02:R_M5A1 1832.00 8.862_NDate 37:18 21.98 n/a .000
03157 SUM 1.0 01:R_M5A 46841.01 47.976_NDate 35:58 17.63 n/a .000
03158 #
03159 # Sum of hydrographs from Node 5A routed to Node 4
03160 # Section 8
03161
03162 R005/C00059 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03163 ROUTE CHANNEL -> 1.0 02:R_M5A 46841.01 47.976_NDate 37:26 17.63 n/a .000
03164 [RDT 1.0] out-> 1.0 01:R4 46841.01 46.217_NDate 37:26 17.63 n/a .000
03165 [L/S/n= 1097./ .040/.035]
03166 [Vmax=.756;Dmax=3.116]
03167 #
03168 # Addition of Subwatershed 4 and Leamy Creek to Node 4
03169
03170 R005/C00060 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03171 ADD HYD + 1.0 02:R4 46841.01 46.217_NDate 37:26 17.63 n/a .000
03172 + 1.0 02:R_M4 8.688_NDate 29:59 15.66 n/a .000
03173 + 1.0 02:R_M4 1021.00 6.466_NDate 30:48 15.07 n/a .000
03174 SUM 1.0 01:R_M4 48447.00 50.308_NDate 36:47 17.89 n/a .000
03175 R005/C00061 -----Dtain-ID:HYD-----AREHA-OPEARCS=PeakDate,hm-----RvM-R,C-----DWFCM
03176 SAVE HYD 1.0 01:R_M4 48447.00 50.308_NDate 36:47 17.89 n/a .000
03177 fname_LM4_0005
03178 remark:flow at S_M4
03179 #

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033677 (Vmax = 991;Dmax = 368)
033678 [XIMP = 68;TIMP = 85]
033679 [Norton parameters: Fw = 76.20;Pc = 13.20;DCAV = 4.14; Pw = .00]
033680 [Previous area: IArea = 4.67;SLDIP = 50;LIDP = 50;HNP = 250;SCP = .0]
033681 [Impervious area: IArea = 1.57;SLDIP = 50;LIDP = 193;HNP = 0.13;SCP = .0]
033682 [IArea = 4.00; IAreaCp = 4.00]
033683 [Previous area: IArea = 4.67;SLDIP = 50;LIDP = 50;HNP = 250;SCP = .0]
033684 [Impervious area: IArea = 1.57;SLDIP = 50;LIDP = 193;HNP = 0.13;SCP = .0]
033685 [IArea = 4.00; IAreaCp = 4.00]
033686 ROUTE RESERVOIR >> 1.0 02:01-C 3.41 .460 No.Date 28:01 44.03 n/a .000
033687 out << 1.0 01:AL1-STR 2.50 .076 No.Date 28:02 44.03 n/a .000
033688 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033689 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033690 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033691 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033692 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033693 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033694 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033695 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033696 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033697 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033698 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033699 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033700 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
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033705 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033706 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
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033718 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033719 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033720 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033721 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033722 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033723 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033724 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033725 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
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033730 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033731 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033732 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033733 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033734 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033735 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033736 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033737 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033738 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033739 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000
033740 + 1.0 02:AL7-STR 12.08 .853 No.Date 28:12 43.99 n/a .000

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04689 #
04690 ROUTE CHANNEL -> 1.0 02:02:41 5458.44 69.643_Hdate 36:50 18.70 n/a .000
04691 [SDP: 1.0] out< 1.0 01:01:24 5458.44 69.650_Hdate 36:38 18.70 n/a .000
04692 (L/S/m= 294 / .109/0.03)
04693 (Vmax: 1.095/Dmax: 1.926)
04694 #
04695 #
04696 ADD HYD + 1.0 02:02:41 out 5458.44 69.650_Hdate 36:38 18.70 n/a .000
04697 + 1.0 02:02:41 5458.44 69.650_Hdate 36:38 18.70 n/a .000
04698 + 1.0 02:02:38-01-8 8.24 .163_Hdate 27:50 40.05 n/a .000
04699 + 1.0 02:02:38-2 1.39 .163_Hdate 27:50 40.05 n/a .000
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04862 #

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Table with columns for ID, Description, Date, and various status indicators. Rows include system components like ROUTE PIPE, MINOR SYSTEM, and CONTINUOUS STANDHYD, along with specific equipment like pumps and valves. Each row is preceded by a unique ID number.

06359 # [Impervious area IAlp=1.57SLP1-50.1MI=1473.NMI-.013:SCI-.0]
06360 # [ISRC= 4.00 IAREP= 4.00]
06361 # [SMIN=36.67I SMAK=244.49I SK=.010]
06362 #
06363 # Poster Pond
06364 # - Rating curve obtained assuming 4003/ha in 24 hours for quality control
06365 # and a ratio of the catchment area to the West Clarke pond rating curve
06366 # from the MSF for the next coordinates
06367 #
06368 #
06369 #
06370 #
06371 #
06372 # [MsfTotAed=1341E+01 n3_TotVolVol=0.000E+00 n3_HvOf= 0_TotDurDv= 0_hrs]
06373 #
06374 # ADD HYD
06375 #
06376 #
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Table with multiple columns including file names, flow types, dates, and various parameters. The table is a detailed report of water flow data, organized by file name and containing numerous rows of numerical and textual information.

```

07855 # Rating curve obtained from CCL hydraulic modeling
07856 *****
07857 ROUTE CHANNL -> 1.0 01:01:MI 175.99 85.776 Mo.Date 28:07 36.76 n/a .000
07858 ROUTE CHANNL -> 1.0 01:01:MI 175.99 85.776 Mo.Date 28:07 36.76 n/a .000
07859 overlow <= 1.0 01:01:MI-0V 0.00 0.00 Mo.Date 0:00 .00 n/a .000
07860 [MsdtoLead=.4673E+01 n3, TotDvVol=.0000E+00 n3, N-ov=0, TotDvOvr=.0 hrs]
07861 ADD HYD + 1.0 02:02:DESIRE 55194.85 85.776 Mo.Date 37:18 23.12 n/a .000
07862 ROUTE CHANNL -> 1.0 01:01:MI 175.99 85.776 Mo.Date 28:07 36.76 n/a .000
07863 ADD HYD + 1.0 02:02:DESIRE 55194.85 85.776 Mo.Date 37:18 23.12 n/a .000
07864 + 1.0 02:02:MI-0V 0.00 0.00 Mo.Date 0:00 .00 n/a .000
07865 [MsdtoLead=.4673E+01 n3, TotDvVol=.0000E+00 n3, N-ov=0, TotDvOvr=.0 hrs]
07866 SIMM + 1.0 01:01:MI 175.99 85.776 Mo.Date 28:07 36.76 n/a .000
07867 ROUTE CHANNL -> 1.0 01:01:MI 175.99 85.776 Mo.Date 28:07 36.76 n/a .000
07868 SAVE HYD *****
07869 *****
07870 *****
07871 *****
07872 *****
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07874 *****
07875 *****
07876 *****
07877 *****
07878 *****
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082229 # (MxStoUsed=.1192E+03)
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Table with columns for ID, description, date, and numerical values. Includes various engineering notes and data points.

Table with columns for line numbers, descriptions, and numerical values. Includes entries for 'JFSa inc.', 'JFSa inc.', and 'Page 27'. The table contains detailed technical data and notes for various systems and components.


```

10847# #
10848# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13a
10849#
10850# RO505-C00032 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10851# ADD HYD + 1.0 021813A 4651.00 19.136_MoDate 39:06 29.90 n/a .000
10852# [L/S/n= 1.02] out<- 1.0 021813M 3074.00 8.912_MoDate 39:59 24.31 n/a .000
10853# SUM= 1.0 011813A 7725.00 27.939_MoDate 39:54 27.68 n/a .000
10854#
10855# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
10856#
10857# RO505-C00033 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10858# ROUTE RESERVOIR -> 1.0 021813A 7725.00 27.939_MoDate 39:54 27.68 n/a .000
10859# out<- 1.0 01RES_RM 7725.00 3.808_MoDate 61:35 27.67 n/a .000
10860# [L/S/n= 5926./0767.080]
10861# (Vmax=.14818)
10862# RO505-C00034 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10863# SAVE HYD 1.0 01RES_RM 7725.00 3.808_MoDate 61:35 27.67 n/a .000
10864# name_H_RESRM
10865# remark:outflow from Res OM
10866# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
10867# (Approximated cross-section - see cross-section 258)
10868# Use n=0.04 for summer conditions and n=0.025 for spring conditions
10869# RO505-C00035 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10870# ROUTE CHANNEL -> 1.0 0218M2 7725.00 3.808_MoDate 61:35 27.67 n/a .000
10871# [RD7= 1.00] out<- 1.0 0118M2 7725.00 3.804_MoDate 64:19 27.68 n/a .000
10872# [L/S/n= 8226./0767.080]
10873# (Vmax=.556/Dmax=1.541)
10874#
10875# Addition of Subwatershed Jock River at Ashton to Node 12
10876#
10877# RO505-C00036 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10878# ADD HYD + 1.0 021812 7725.00 3.804_MoDate 64:19 27.68 n/a .000
10879# [L/S/n= 16028./ASH]
10880# SUM= 1.0 0118M2 9506.00 18.867_MoDate 32:42 29.39 n/a .000
10881# RO505-C00037 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10882# SAVE HYD 1.0 0118M2 9506.00 18.867_MoDate 32:42 29.39 n/a .000
10883# name_H_R12
10884# remark:flow at S_M12 near Ashton
10885#
10886# Sum of hydrographs from Node 12 routed to Node 11
10887# (Approximated cross-section - see cross-section 258)
10888# Use n=0.04 for summer conditions and n=0.025 for spring conditions
10889#
10890# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
10891#
10892# RO505-C00038 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10893# ROUTE CHANNEL -> 1.0 0218M2 9506.00 18.867_MoDate 32:42 29.39 n/a .000
10894# [RD7= 1.00] out<- 1.0 01Dum11 9506.00 18.867_MoDate 32:59 29.39 n/a .000
10895# [L/S/n= 972./0547.040]
10896# (Vmax=.781/Dmax=3.028)
10897#
10898# Addition of Subwatershed 11 and No Name Creek to Node 11
10899#
10900# RO505-C00039 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10901# ADD HYD + 1.0 021Dum11 9506.00 18.867_MoDate 32:59 29.39 n/a .000
10902# [L/S/n= 500.]
10903# ROUTE CHANNEL -> 1.0 0218M2 1937.00 9.061_MoDate 29:21 31.73 n/a .000
10904# SUM= 1.0 0118M11 11923.00 32.851_MoDate 33:00 29.87 n/a .000
10905#
10906# Sum of hydrographs from Node 11 routed to Node 10
10907# Section 1
10908#
10909# RO505-C00040 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10910# ROUTE CHANNEL -> 1.0 0218M11 11923.00 20.490_MoDate 40:02 29.87 n/a .000
10911# [RD7= 1.00] out<- 1.0 011810 11923.00 20.490_MoDate 40:02 29.87 n/a .000
10912# [L/S/n= 474./Dmax=1.423]
10913# (Vmax=.474/Dmax=1.423)
10914#
10915# Addition of Subwatershed 10 to Node 10
10916#
10917# RO505-C00041 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10918# ADD HYD + 1.0 021810 11923.00 20.490_MoDate 40:02 29.87 n/a .000
10919# [L/S/n= 5646.]
10920# SUM= 1.0 0118M10 5646.00 52.600_MoDate 38:19 32.12 n/a .000
10921# RO505-C00042 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10922# SAVE HYD 1.0 0118M10 5646.00 52.600_MoDate 38:19 32.12 n/a .000
10923# name_H_S10
10924# remark:flow at S_M10 SW_10
10925# Addition of Kings Creek to S_M10
10926#
10927# RO505-C00043 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10928# ADD HYD + 1.0 0218M10 17589.00 52.600_MoDate 38:19 32.12 n/a .000
10929# [L/S/n= 8776.]
10930# SUM= 1.0 0118M10A 25965.00 82.746_MoDate 39:45 31.99 n/a .000
10931#
10932# Sum of hydrographs from Node 10 routed to Node 9
10933# Section 2
10934#
10935# RO505-C00044 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10936# ROUTE CHANNEL -> 1.0 0218M10A 25965.00 82.746_MoDate 39:59 31.99 n/a .000
10937# [RD7= 1.00] out<- 1.0 0118M 25965.00 80.980_MoDate 39:59 31.99 n/a .000
10938# [L/S/n= 3982./0767.080]
10939# (Vmax=.744/Dmax=2.015)
10940#
10941# Addition of Subwatershed 9 and Nichols Creek to Node 9
10942#
10943# RO505-C00045 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10944# ADD HYD + 1.0 0218M 25965.00 80.980_MoDate 39:59 31.99 n/a .000
10945# [L/S/n= 1132.]
10946# SUM= 1.0 0218M_CK 4464.00 15.472_MoDate 39:59 28.95 n/a .000
10947# 1.0 0118M_N3 31561.00 99.424_MoDate 39:59 31.68 n/a .000
10948#
10949# Sum of hydrographs from Node 9 routed to Node 8
10950# Section 3
10951#
10952# RO505-C00046 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10953# ROUTE CHANNEL -> 1.0 0218M 31561.00 99.424_MoDate 39:59 31.68 n/a .000
10954# [RD7= 1.00] out<- 1.0 0118M 31561.00 99.665_MoDate 39:59 31.68 n/a .000
10955# [L/S/n= 1087.]
10956# (Vmax=.367/Dmax=1.834)
10957#
10958# Addition of Subwatershed 8 and Bobb's Drain to Node 8
10959#
10960# RO505-C00047 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10961# ADD HYD + 1.0 0218M 31561.00 99.665_MoDate 39:59 31.68 n/a .000
10962# [L/S/n= 121.]
10963# SUM= 1.0 0218M_DR 3854.00 18.180_MoDate 38:32 31.73 n/a .000
10964# 1.0 0118M_N3 35466.00 111.843_MoDate 39:59 31.68 n/a .000
10965#
10966# Sum of hydrographs from Node 8 routed to Node 7
10967# Section 4
10968#
10969# RO505-C00048 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10970# ROUTE CHANNEL -> 1.0 0218M 35466.00 111.843_MoDate 39:59 31.68 n/a .000
10971# [RD7= 1.00] out<- 1.0 0118M 35466.00 95.475_MoDate 44:55 31.68 n/a .000
10972# [L/S/n= 1057./231.290]
10973# (Vmax=.231/Dmax=2.290)
10974#
10975# Addition of Subwatershed 7 to Node 7
10976#
10977# RO505-C00049 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10978# ADD HYD + 1.0 0218M 35466.00 95.475_MoDate 44:55 31.68 n/a .000
10979# [L/S/n= 3197.]
10980# SUM= 1.0 0218M_N7 38743.00 102.892_MoDate 43:46 31.18 n/a .000
10981# 1.0 0218M_DR 1132.00 9.812_MoDate 35:12 36.85 n/a .000
10982# SAVE HYD 1.0 0118M7 38743.00 102.892_MoDate 43:46 31.18 n/a .000
10983# name_H_SM7
10984# remark:flow at S_M7 N7 SW_7
10985# Insertion of a reservoir to simulate the effects of the Richmond Fen.
10986# Storage area and volume were estimated from available topo maps.
10987# Release rate from fen was assumed to be controlled by the downstream
10988# river cross-section for summer conditions. It is assumed that for up to
10989# 0.75 m of water, the main channel of the river provided the storage. Above
10990# this depth, the wetland starts to significantly store water.
10991#
10992# RO505-C00051 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10993# ROUTE RESERVOIR -> 1.0 0218M7 38743.00 102.892_MoDate 43:46 31.18 n/a .000
10994# out<- 1.0 01RES_RF 38743.00 52.029_MoDate 59:07 31.18 n/a .000
10995# (Hydrologic.4394E+03)
10996# RO505-C00052 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
10997# SAVE HYD 1.0 01RES_RF 38743.00 52.029_MoDate 59:07 31.18 n/a .000
10998# name_H_RESRF
10999# remark:outflow of Richmond Fen
11000#
11001# Sum of hydrographs from Node 7 routed to Node 6
11002# Section 5
11003#
11004# RO505-C00053 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
11005# ROUTE CHANNEL -> 1.0 0218M7 38743.00 52.029_MoDate 59:07 31.18 n/a .000
11006# [RD7= 1.00] out<- 1.0 0118M 38743.00 51.784_MoDate 60:27 31.18 n/a .000
11007# [L/S/n= 3086./087./0547.040]
11008# (Vmax=.581/Dmax=1.253)
11009#
11010# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
11011#
11012# RO505-C00054 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
11013# ADD HYD + 1.0 0218M 38743.00 51.784_MoDate 60:27 31.18 n/a .000
11014# [L/S/n= 185.]
11015# SUM= 1.0 0218M_DR 1132.00 9.812_MoDate 35:12 36.85 n/a .000
11016# 1.0 0118M_N6 40240.01 51.810_MoDate 60:20 32.44 n/a .000
11017#
11018# Sum of hydrographs from Node 6 routed to Node 5
11019# Section 6
11020#
11021# RO505-C00055 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
11022# ROUTE CHANNEL -> 1.0 0218M 40240.01 51.810_MoDate 60:20 32.44 n/a .000
11023# [RD7= 1.00] out<- 1.0 0118M 40240.01 51.693_MoDate 61:06 31.37 n/a .000
11024# [L/S/n= 1927./0547.040]
11025# (Vmax=.469/Dmax=1.351)
11026#
11027# Addition of Subwatershed 5 and Flowing Creek to Node 5
11028#
11029# RO505-C00056 ---Dtn-ID-NHYD---AREAhA-OPEARComs-TPeakDate_hh:mm---RvM-R-C---DWFCms
11030# ADD HYD + 1.0 0218M 40240.01 51.693_MoDate 61:06 31.37 n/a .000
11031# [L/S/n= 124.]
11032# SUM= 1.0 0218M_CK 4945.00 44.623_MoDate 33:18 38.37 n/a .000
11033# 1.0 0118M_N5 45409.01 71.514_MoDate 34:20 32.38 n/a .000

```


Table with multiple columns: ID (e.g., 11598), Model Name (e.g., R0505CO0147), Location (e.g., Hydrograph from Node Foster Drain), Parameters (e.g., [LS/N= 1.239/Dmax= 3.774]), and various numerical values (e.g., 5377.82, 128.036, No.Date, 33.19, 33.65, n/a, .000).

Table with columns for ID, description, parameters, and values. Rows include various system parameters like 'Major System', 'Minor System', 'ADD HYD', 'CONTINUOUS STANDBY', etc., with associated numerical values and units.

Table with columns for system identifiers (e.g., 127170, 127171), component names (e.g., [M]SysSto, ROUTE PIPE), and various numerical parameters. The table contains multiple rows of data organized into sections.

```

13091 # Tributary Drainage Area to M55 Pond = 145 ha
13092 *****
13093 #
13094 # Hydrograph from Curragh Drain routed to Jockvale Road
13095 # Channel X-Section obtained from RWCA Hydraulic Model - Station 2462
13096 #
13097 R050=C00387-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13098 ROUTE CHANNEL -> 1.0 0.02=COV 55019.59 126.926 NoDate 36:27 34.22 n/a .000
13099 [RDP=1.00] out<= 1.0 01=MI_ML 55019.59 126.908 NoDate 36:31 34.22 n/a .000
13100 [L/S= 580./ 445./495]
13101 [Vmax=0.065/0.061]
13102 *****
13103 # Catchment MILLS
13104 # To SWM Facility north of the Jock
13105 # - Primarily residential development
13106 *****
13107 R050=C00388-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13108 CONTINUOUS STANHDV 1.0 01=MI_ML 175.99 17.756 NoDate 28:06 50.66 422 .000
13109 [XMG= 2 /CN= 74.0]
13110 [Perovous area IArea= 4.675/SLP=1.00/LPI= 400./NMI= 250/SCP= .0]
13111 [Impervious area IArea= 1.57/SLP=1.00/LPI= 1118./NMI= 013/SCI= .0]
13112 [ImpScp= 4.00./IAR=SC= 4.00]
13113 [SMIN= 36.67/ SMAX=244.49/ SK= .010]
13114 *****
13115 # Chapman Mills SWM Pond
13116 #
13117 # Rating curve obtained from CCL hydraulic modeling
13118 *****
13119 R050=C00389-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13120 ROUTE RESERVOIR 1.0 02=MILLS 179.99 17.756 NoDate 28:06 50.66 n/a .000
13121 [RDP= 1.00] out<= 1.0 01=MI_DE 153.87 4.000 NoDate 28:11 50.66 n/a .000
13122 overflow<= 1.0 03=MI-OV 22.12 12.440 NoDate 28:11 50.66 n/a .000
13123 [MaxTotMed= 21120.00 n3, TotDuvVol= 11210.00 n3, M=0.0v 2, TotDuvOv= 1.3ha]
13124 R050=C00390-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13125 ADD HYD + 1.0 02=MI_ML 55019.59 126.926 NoDate 36:31 34.22 n/a .000
13126 + 1.0 02=MI-OV 22.12 12.440 NoDate 28:11 50.66 n/a .000
13127 + 1.0 02=MILLS 153.87 4.000 NoDate 28:11 50.66 n/a .000
13128 + 1.0 01=MI_ML 55019.59 127.513 NoDate 36:30 34.27 n/a .000
13129 R050=C00391-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13130 SAVE HYD 1.0 01=MI_DE 55019.59 127.513 NoDate 36:30 34.27 n/a .000
13131 *****
13132 # remark/Total Flows at Jockvale Road
13133 #
13134 # Hydrograph from Jockvale Road routed to Heart's Desire
13135 # Channel X-Section obtained from RWCA Hydraulic Model - Station 689
13136 #
13137 R050=C00392-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13138 ROUTE CHANNEL -> 1.0 02=MI_ML 55195.58 127.513 NoDate 36:30 34.27 n/a .000
13139 [RDP=1.00] out<= 1.0 01=MI_DE 55195.58 127.513 NoDate 36:34 34.27 n/a .000
13140 [L/S= 592./ 227./415]
13141 [Vmax=1.577/1.260/2.490]
13142 *****
13143 # Catchment DESIRE
13144 # To Jock River (north of the Jock)
13145 # - Rural-estate subdivision (Heart's Desire Community)
13146 *****
13147 R050=C00393-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13148 CONTINUOUS STANHDV 1.0 01=DESIRE 23.78 2.563 NoDate 28:03 46.85 575 .000
13149 [XMG= 2 /CN= 77.0]
13150 [Perovous area IArea= 4.675/SLP=1.00/LPI= 400./NMI= 250/SCP= .0]
13151 [Impervious area IArea= 1.57/SLP=1.00/LPI= 1118./NMI= 013/SCI= .0]
13152 [ImpScp= 4.00./IAR=SC= 4.00]
13153 [SMIN= 36.67/ SMAX=244.49/ SK= .010]
13154 *****
13155 # Jockvale SWM Facility
13156 # - Residential development & golf course
13157 # - JFSA 2020-01-11 update: After updating COBRIG as per IRI GROUP, July 2008.
13158 # - Jockvale area became 235.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX-LAND 32.5 ha as
13159 *****
13160 R050=C00394-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13161 CONTINUOUS STANHDV 1.0 01=JOCKVA 225.13 25.293 NoDate 28:07 56.33 691 .000
13162 [XMG= 50./CN= 50]
13163 [RDP= 1.00] out<= 1.0 01=JOCKVA 225.13 25.293 NoDate 28:07 56.33 691 .000
13164 [L/S= 74.0]
13165 [Perovous area IArea= 4.675/SLP=1.00/LPI= 400./NMI= 250/SCP= .0]
13166 [Impervious area IArea= 1.57/SLP=1.00/LPI= 1118./NMI= 013/SCI= .0]
13167 [ImpScp= 4.00./IAR=SC= 4.00]
13168 [SMIN= 36.67/ SMAX=244.49/ SK= .010]
13169 *****
13170 R050=C00395-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13171 ADD HYD + 1.0 02=JOCKVA 225.13 25.293 NoDate 28:07 56.33 n/a .000
13172 + 1.0 02=MI_ML 225.13 25.293 NoDate 28:07 56.33 n/a .000
13173 + 1.0 02=MI-OV .02 .083 NoDate 28:09 56.26 n/a .000
13174 + 1.0 02=MI-M 0.02 .083 NoDate 28:09 56.26 n/a .000
13175 + 1.0 01=MI_DE 225.13 25.293 NoDate 28:07 56.36 n/a .000
13176 R050=C00396-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13177 SAVE HYD 1.0 01=MI_DE 225.13 25.293 NoDate 28:07 56.36 n/a .000
13178 *****
13179 # remark/Total Flows at KB first pond
13180 #
13181 # Jockvale SWM Facility
13182 # Rating curve obtained from Jockvale Servicing Study (CCL 1998)
13183 *****
13184 R050=C00397-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13185 ROUTE RESERVOIR 1.0 01=JOCK_P 225.13 25.293 NoDate 28:07 56.36 n/a .000
13186 out<= 1.0 01=JOCK_P 225.13 25.293 NoDate 28:07 56.36 n/a .000
13187 out<= 1.0 01=JOCK_P 225.13 25.293 NoDate 28:07 56.36 n/a .000
13188 [MaxTotMed= 59288.00 n3, TotDuvVol= 0.000000 n3, M=0.0v 0, TotDuvOv= 0.3ha]
13189 R050=C00398-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13190 ADD HYD + 1.0 02=DESIRE 23.78 2.563 NoDate 28:03 46.85 n/a .000
13191 + 1.0 01=JOCK_P 225.13 25.293 NoDate 28:07 56.36 n/a .000
13192 + 1.0 02=MI-M 0.02 .083 NoDate 28:09 56.26 n/a .000
13193 + 1.0 02=MI-M 0.02 .083 NoDate 28:09 56.26 n/a .000
13194 + 1.0 01=MI_DE 225.13 25.293 NoDate 28:07 56.36 n/a .000
13195 R050=C00399-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13196 SAVE HYD 1.0 01=MI_DE 55476.26 128.174 NoDate 36:42 34.38 n/a .000
13197 *****
13198 # remark/Total Flows at Heart's Desire
13199 #
13200 # Hydrograph from Heart's Desire routed to Rideau River
13201 # Channel X-Section obtained from RWCA Hydraulic Model - Station 0
13202 *****
13203 R050=C00400-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13204 ROUTE CHANNEL -> 1.0 01=MI_ML 55476.26 128.174 NoDate 36:42 34.38 n/a .000
13205 [RDP=1.00] out<= 1.0 01=MI 55476.26 128.160 NoDate 36:45 34.38 n/a .000
13206 [L/S= 621./ 367./445]
13207 [Vmax= 2.123/1.248]
13208 *****
13209 # Catchment S-2
13210 # To Jock River (north and south)
13211 # - Undeveloped floodplain
13212 *****
13213 R050=C00401-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13214 CONTINUOUS NASHVD 1.0 01=S-2 102.94 4.795 NoDate 28:20 35.39 434 .000
13215 [CN= 72.0/ N= 3.00/ Tp= .40]
13216 [IAR= 4.00/ SMIN= 39.75/ SMAX=264.99/ SK= .010]
13217 [InterEventTime= 12.00]
13218 R050=C00402-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13219 ADD HYD + 1.0 02=MI 55476.26 128.160 NoDate 36:45 34.38 n/a .000
13220 + 1.0 02=MI 102.94 4.795 NoDate 28:20 35.39 n/a .000
13221 + 1.0 01=MI_ML 55579.20 128.417 NoDate 36:44 34.38 n/a .000
13222 R050=C00403-----Dtm=ID-NHVD-----AREAh-QPEArms-TPeakDate_hh:mm-----Rvm-R-C-----DWFms
13223 SAVE HYD 1.0 01=MI_ML 55579.20 128.417 NoDate 36:44 34.38 n/a .000
13224 *****
13225 # remark/Total Flows at Rideau River
13226 *****
13227 ** END OF RUN : 99
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Table with multiple columns containing alphanumeric codes (e.g., R0100C0001), dates, times, and various status indicators (e.g., n/a, 0.00, 1.00). Includes a footer section with a 'Rating curve obtained assuming 4003/h in 24 hours...' and a 'FROM THE MES FOR THE next occurrences' note.


```

15709 # + 1.0 02:09-MJ .00 .000 No_date 0:00 .00 n/a .000
15710 # + 1.0 02:09-MJ .08 218 No_date 28:05 59.73 n/a .000
15711 # + 1.0 02:09-MJ 16 564 No_date 28:06 67.38 n/a .000
15712 # + 1.0 02:09-MJ 23 159 No_date 28:05 62.88 n/a .000
15713 # + 1.0 02:09-MJ .00 .000 No_date 0:00 .00 n/a .000
15714 # + 1.0 01:CORRIG 5520.08 145.505 No_date 36:37 39.46 n/a .000
15715 # ROUTE CHANNEL 1.0 01:CORRIG 5520.08 145.505 No_date 36:37 39.46 n/a .000
15716 # [L/S/N= 583 / 445 / 045]
15717 # [Vmax= 2.170 / Dmax= 2.124]
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Attachment E

Model 4B – Jock River Reach One Future Conditions – With SWM controls

JFSA, 2021

SWMHYMO Input & Summary files

```

1  20    Metric units / ID numbers OFF
2  *#*****
3  *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *#*****
5  *# Project Name: [Jock River]    Project Number: [1474-16]
6  *# Date       : 04-03-2021
7  *# Modeller   : [M.M.]
8  *# Company    : JFSAinc.
9  *# License #  : 2549237
10 *#*****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *# 2020-11-30 change TMJSTO in COMPUTE DUALHYD (TMJSTO = 0.1 instead of 0.0001)
16 *# 2020-12-01 correct pond curve values
17 *# 2020-12-01 change W_CLAR_BRAZ XIMP to 0.55, SLPI=[0.5](%) (impervious slope), and
LGI up to 700m
18 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NHYDout=["N_TO"]
,NHYDin=["SN_TO"]) from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model
will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELs
aren't well suited to really flat slopes.
19 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NHYDout=["N_WC"]
,NHYDin=["SN_CE"]) from 0.01 % (as per Stantec Report 2007) to 0.0255 % so the model
will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELs
aren't well suited to really flat slopes.
20 *
21 * Calibrated parameters for Summer 2003 data:  APII=50, APIK=0.85, CN=varies,
22 *                                               SK=0.01, InterEventTime=12,
23 *                                               GWResk=0.96, VHydCond=0.055
24 *
25 *# -----
26 *
27 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
28 *              ["XAVG0315.STM"] average storm data a 15 minute time step
29 *              The above rainf file is an average of the JFSA gauge data
30 *              with the City of Ottawa rainfall data collected during
31 *              the same period.
32 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
33 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
34                ["C24SC002.stm"] <--storm filename, one per line for NSTORM time
35 *%-----|-----|
36 *%-----|-----|
37 READ STORM    STORM_FILENAME=["storm.001"]
38 *%-----|-----|
39 MODIFY STORM  ICASEms=[1], NSHIFT=[96],
40                RedFACT=[1],
41 *%-----|-----|
42 DEFAULT VALUES ICASEdef=[1], read and print values
43                DEFVAL_FILENAME=["CitiGate.DEF"]
44 *%-----|-----|
45 COMPUTE API   APII=[50], APIK=[.85]/day
46 *%-----|-----|
47 *%-----|-----|
48 *#
49 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
50 *# of 1.32
51 *%-----|-----|
52 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[1]min, AREA=[3680](ha),
53                DWF=[0](cms), CN/C=[64], IA=[2.5](mm),
54                N=[3.0], TP=[7.13]hrs,
55                Continuous simulation parameters:
56                IaRECper=[4](hrs),
57                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
58                InterEventTime=[12](hrs)
59                Baseflow simulation parameters:

```

```

60         BaseFlowOption=[1] ,
61         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
62         VHydCond=[0.055](mm/hr),   END=-1
63     *%-----|-----
64     *#
65     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
66     *# of 1.32
67     *%-----|-----
68     CONTINUOUS NASHYD  NHYD=["SW_13"], DT=[1]min, AREA=[971](ha),
69                        DWF=[0](cms),  CN/C=[61], IA=[2.5](mm),
70                        N=[3.0], TP=[3.76]hrs,
71                        Continuous simulation parameters:
72                        IaRECper=[4](hrs),
73                        SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
74                        InterEventTime=[12](hrs)
75                        Baseflow simulation parameters:
76                        BaseFlowOption=[1] ,
77                        InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
78                        VHydCond=[0.055](mm/hr),   END=-1
79     *%-----|-----
80     *#
81     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
82     *# of 1.80
83     *%-----|-----
84     CONTINUOUS NASHYD  NHYD=["JR_GWM"], DT=[1]min, AREA=[3074](ha),
85                        DWF=[0](cms),  CN/C=[55], IA=[2.5](mm),
86                        N=[3], TP=[11.33]hrs,
87                        Continuous simulation parameters:
88                        IaRECper=[4](hrs),
89                        SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
90                        InterEventTime=[12](hrs)
91                        Baseflow simulation parameters:
92                        BaseFlowOption=[1] ,
93                        InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
94                        VHydCond=[0.055](mm/hr),   END=-1
95     *%-----|-----
96     CONTINUOUS NASHYD  NHYD=["JR_ASH"], DT=[1]min, AREA=[1781](ha),
97                        DWF=[0](cms),  CN/C=[72], IA=[2.5](mm),
98                        N=[3.0], TP=[3.91]hrs,
99                        Continuous simulation parameters:
100                       IaRECper=[4](hrs),
101                       SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
102                       InterEventTime=[12](hrs)
103                       Baseflow simulation parameters:
104                       BaseFlowOption=[1] ,
105                       InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
106                       VHydCond=[0.055](mm/hr),   END=-1
107     *%-----|-----
108     CONTINUOUS NASHYD  NHYD=["SW_11"], DT=[1]min, AREA=[500](ha),
109                        DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),
110                        N=[3.0], TP=[1.24]hrs,
111                        Continuous simulation parameters:
112                        IaRECper=[4](hrs),
113                        SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
114                        InterEventTime=[12](hrs)
115                        Baseflow simulation parameters:
116                        BaseFlowOption=[1] ,
117                        InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
118                        VHydCond=[0.055](mm/hr),   END=-1
119     *%-----|-----
120     *#
121     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
122     *# of 1.80
123     *%-----|-----
124     CONTINUOUS NASHYD  NHYD=["NN_CK"], DT=[1]min, AREA=[1917](ha),
125                        DWF=[0](cms),  CN/C=[66], IA=[2.5](mm),

```

```

126 N=[3.0], TP=[5.29]hrs,
127 Continuous simulation parameters:
128 IaRECper=[4](hrs),
129 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
130 InterEventTime=[12](hrs)
131 Baseflow simulation parameters:
132 BaseFlowOption=[1] ,
133 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
134 VHydCond=[0.055](mm/hr), END=-1
135 *%-----|-----
136 *#
137 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
138 *# of 1.52
139 *%-----|-----
140 CONTINUOUS NASHYD NHYD=["SW_10"], DT=[1]min, AREA=[5666](ha),
141 DWF=[0](cms), CN/C=[72], IA=[2.5](mm),
142 N=[3.0], TP=[8.00]hrs,
143 Continuous simulation parameters:
144 IaRECper=[4](hrs),
145 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
146 InterEventTime=[12](hrs)
147 Baseflow simulation parameters:
148 BaseFlowOption=[1] ,
149 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
150 VHydCond=[0.055](mm/hr), END=-1
151 *%-----|-----
152 *#
153 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
154 *# of 1.75
155 *%-----|-----
156 CONTINUOUS NASHYD NHYD=["KG CK"], DT=[1]min, AREA=[8376](ha),
157 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
158 N=[3.0], TP=[11.66]hrs,
159 Continuous simulation parameters:
160 IaRECper=[4](hrs),
161 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
162 InterEventTime=[12](hrs)
163 Baseflow simulation parameters:
164 BaseFlowOption=[1] ,
165 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
166 VHydCond=[0.055](mm/hr), END=-1
167 *%-----|-----
168 *#
169 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
170 *# of 1.68
171 *%-----|-----
172 CONTINUOUS NASHYD NHYD=["SW_9"], DT=[1]min, AREA=[1132](ha),
173 DWF=[0](cms), CN/C=[70], IA=[2.5](mm),
174 N=[3.0], TP=[2.51]hrs,
175 Continuous simulation parameters:
176 IaRECper=[4](hrs),
177 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
178 InterEventTime=[12](hrs)
179 Baseflow simulation parameters:
180 BaseFlowOption=[1] ,
181 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
182 VHydCond=[0.055](mm/hr), END=-1
183 *%-----|-----
184 *#
185 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
186 *# of 1.82
187 *%-----|-----
188 CONTINUOUS NASHYD NHYD=["NC CK"], DT=[1]min, AREA=[4464](ha),
189 DWF=[0](cms), CN/C=[62], IA=[2.5](mm),
190 N=[3.0], TP=[11.32]hrs,
191 Continuous simulation parameters:

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192 IaREcper=[4](hrs),
193 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
194 InterEventTime=[12](hrs)
195 Baseflow simulation parameters:
196 BaseFlowOption=[1] ,
197 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
198 VHydCond=[0.055](mm/hr), END=-1
199 *%-----|-----
200 *#
201 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
202 *# of 1.80
203 *%-----|-----
204 CONTINUOUS NASHYD NHYD=["SW_8"], DT=[1]min, AREA=[131](ha),
205 DWF=[0](cms), CN/C=[63], IA=[2.5](mm),
206 N=[3.0], TP=[0.90]hrs,
207 Continuous simulation parameters:
208 IaREcper=[4](hrs),
209 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
210 InterEventTime=[12](hrs)
211 Baseflow simulation parameters:
212 BaseFlowOption=[1] ,
213 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
214 VHydCond=[0.055](mm/hr), END=-1
215 *%-----|-----
216 *#
217 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
218 *# of 1.65
219 *%-----|-----
220 CONTINUOUS NASHYD NHYD=["HB_DR"], DT=[1]min, AREA=[3854](ha),
221 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
222 N=[3.0], TP=[8.42]hrs,
223 Continuous simulation parameters:
224 IaREcper=[4](hrs),
225 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
226 InterEventTime=[12](hrs)
227 Baseflow simulation parameters:
228 BaseFlowOption=[1] ,
229 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
230 VHydCond=[0.055](mm/hr), END=-1
231 *%-----|-----
232 *#
233 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
234 *# of 1.82
235 *%-----|-----
236 CONTINUOUS NASHYD NHYD=["SW_7"], DT=[1]min, AREA=[3197](ha),
237 DWF=[0](cms), CN/C=[57], IA=[2.5](mm),
238 N=[3.0], TP=[6.65]hrs,
239 Continuous simulation parameters:
240 IaREcper=[4](hrs),
241 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
242 InterEventTime=[12](hrs)
243 Baseflow simulation parameters:
244 BaseFlowOption=[1] ,
245 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
246 VHydCond=[0.055](mm/hr), END=-1
247 *%-----|-----
248 *#
249 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
250 *# of 1.75
251 *%-----|-----
252 CONTINUOUS NASHYD NHYD=["SW_6"], DT=[1]min, AREA=[165](ha),
253 DWF=[0](cms), CN/C=[67], IA=[2.5](mm),
254 N=[3.0], TP=[4.18]hrs,
255 Continuous simulation parameters:
256 IaREcper=[4](hrs),
257 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

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258 InterEventTime=[12](hrs)
259 Baseflow simulation parameters:
260 BaseFlowOption=[1] ,
261 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
262 VHydCond=[0.055](mm/hr) , END=-1
263 *%-----|-----
264 *#
265 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
266 *# of 1.67
267 *%-----|-----
268 CONTINUOUS NASHYD NHYD=["VG_DR"] , DT=[1]min , AREA=[1332](ha) ,
269 DWF=[0](cms) , CN/C=[72] , IA=[2.5](mm) ,
270 N=[3.0] , TP=[5.95]hrs ,
271 Continuous simulation parameters:
272 IaREcper=[4](hrs) ,
273 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
274 InterEventTime=[12](hrs)
275 Baseflow simulation parameters:
276 BaseFlowOption=[1] ,
277 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
278 VHydCond=[0.055](mm/hr) , END=-1
279 *%-----|-----
280 CONTINUOUS NASHYD NHYD=["SW_5"] , DT=[1]min , AREA=[224](ha) ,
281 DWF=[0](cms) , CN/C=[77] , IA=[2.5](mm) ,
282 N=[3.0] , TP=[0.75]hrs ,
283 Continuous simulation parameters:
284 IaREcper=[4](hrs) ,
285 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
286 InterEventTime=[12](hrs)
287 Baseflow simulation parameters:
288 BaseFlowOption=[1] ,
289 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
290 VHydCond=[0.055](mm/hr) , END=-1
291 *%-----|-----
292 *#
293 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
294 *# of 1.20
295 *%-----|-----
296 CONTINUOUS NASHYD NHYD=["FL_CK"] , DT=[1]min , AREA=[4945](ha) ,
297 DWF=[0](cms) , CN/C=[74] , IA=[2.5](mm) ,
298 N=[3.0] , TP=[4.45]hrs ,
299 Continuous simulation parameters:
300 IaREcper=[4](hrs) ,
301 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
302 InterEventTime=[12](hrs)
303 Baseflow simulation parameters:
304 BaseFlowOption=[1] ,
305 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
306 VHydCond=[0.055](mm/hr) , END=-1
307 *%-----|-----
308 CONTINUOUS NASHYD NHYD=["SW_5A2"] , DT=[1]min , AREA=[20](ha) ,
309 DWF=[0](cms) , CN/C=[81] , IA=[2.5](mm) ,
310 N=[3.0] , TP=[0.62]hrs ,
311 Continuous simulation parameters:
312 IaREcper=[4](hrs) ,
313 SMIN=[-1](mm) , SMAX=[-1](mm) , SK=[0.010]/(mm) ,
314 InterEventTime=[12](hrs)
315 Baseflow simulation parameters:
316 BaseFlowOption=[1] ,
317 InitGWResVol=[50](mm) , GWResK=[0.96](mm/day/mm)
318 VHydCond=[0.055](mm/hr) , END=-1
319 *%-----|-----
320 *#
321 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
322 *# of 1.61
323 *%-----|-----

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324 CONTINUOUS NASHYD NHYD=["SW_5A1"], DT=[1]min, AREA=[1412](ha),
325 DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
326 N=[3.0], TP=[8.00]hrs,
327 Continuous simulation parameters:
328 IaREcper=[4](hrs),
329 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
330 InterEventTime=[12](hrs)
331 Baseflow simulation parameters:
332 BaseFlowOption=[1] ,
333 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
334 VHydCond=[0.055](mm/hr), END=-1
335 *%-----|
336 CONTINUOUS NASHYD NHYD=["SW_4"], DT=[1]min, AREA=[585](ha),
337 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
338 N=[3.0], TP=[1.75]hrs,
339 Continuous simulation parameters:
340 IaREcper=[4](hrs),
341 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
342 InterEventTime=[12](hrs)
343 Baseflow simulation parameters:
344 BaseFlowOption=[1] ,
345 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
346 VHydCond=[0.055](mm/hr), END=-1
347 *%-----|
348 CONTINUOUS NASHYD NHYD=["LM_CK"], DT=[1]min, AREA=[1021](ha),
349 DWF=[0](cms), CN/C=[80], IA=[2.5](mm),
350 N=[3.0], TP=[2.46]hrs,
351 Continuous simulation parameters:
352 IaREcper=[4](hrs),
353 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
354 InterEventTime=[12](hrs)
355 Baseflow simulation parameters:
356 BaseFlowOption=[1] ,
357 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
358 VHydCond=[0.055](mm/hr), END=-1
359 *%-----|
360 CONTINUOUS NASHYD NHYD=["SW_2"], DT=[1]min, AREA=[177](ha),
361 DWF=[0](cms), CN/C=[77], IA=[2.5](mm),
362 N=[3.0], TP=[0.75]hrs,
363 Continuous simulation parameters:
364 IaREcper=[4](hrs),
365 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
366 InterEventTime=[12](hrs)
367 Baseflow simulation parameters:
368 BaseFlowOption=[1] ,
369 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
370 VHydCond=[0.055](mm/hr), END=-1
371 *%-----|
372 CONTINUOUS NASHYD NHYD=["SM_DR"], DT=[1]min, AREA=[1122](ha),
373 DWF=[0](cms), CN/C=[81], IA=[2.5](mm),
374 N=[3.0], TP=[3.25]hrs,
375 Continuous simulation parameters:
376 IaREcper=[4](hrs),
377 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
378 InterEventTime=[12](hrs)
379 Baseflow simulation parameters:
380 BaseFlowOption=[1] ,
381 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
382 VHydCond=[0.055](mm/hr), END=-1
383 *%-----|
384 CONTINUOUS NASHYD NHYD=["MO_DR"], DT=[1]min, AREA=[2737](ha),
385 DWF=[0](cms), CN/C=[76], IA=[2.5](mm),
386 N=[3.0], TP=[3.03]hrs,
387 Continuous simulation parameters:
388 IaREcper=[4](hrs),
389 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),

```

```

390 InterEventTime=[12](hrs)
391 Baseflow simulation parameters:
392 BaseFlowOption=[1] ,
393 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
394 VHydCond=[0.055](mm/hr), END=-1
395 *%-----|-----
396 * -JFSA 2020 replaced SW_1 with a detailed model from Stantec Report 2007
397 *CONTINUOUS NASHYD NHYD=["SW_1"], DT=[1]min, AREA=[3176](ha),
398 * DWF=[0](cms), CN/C=[78], IA=[2.5](mm),
399 * N=[3.0], TP=[3.56]hrs,
400 * Continuous simulation parameters:
401 * IaRECper=[4](hrs),
402 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
403 * InterEventTime=[12](hrs)
404 * Baseflow simulation parameters:
405 * BaseFlowOption=[1] ,
406 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
407 * VHydCond=[0.055](mm/hr), END=-1
408 *%-----|-----
409 *#
410 *# Routing hydrographs
411 *#
412 *# Starting with the addition of Jock River Headwater and Subwatershed 13
413 *#
414 ADD HYD NHYDsum=["S_N13"], NHYDs to add=["JR_HW"+"SW_13"]
415 *%-----|-----
416 *#
417 *# Sum of hydrographs from Node 13 routed to Node 13A
418 *# (Approximated cross-section - see cross-section 258)
419 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
420 *#
421 ROUTE CHANNEL NHYDout=["N13A"] ,NHYDin=["S_N13"],
422 RDT=[1](min),
423 CHLGTH=[9074](m), CHSLOPE=[0.0220](%),
424 FPSLOPE=[0.0220](%),
425 SECNUM=[1.0], NSEG=[1]
426 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
427 ( DISTANCE (m), ELEVATION (m))=
428 [-40, 132.5]
429 [-30, 132]
430 [-25, 131.5]
431 [-13, 130]
432 [-8, 127.00]
433 [-7, 126.50]
434 [-6, 126]
435 [-5.5, 125.50]
436 [0, 123.75]
437 [4.5, 125.50]
438 [6, 126]
439 [7.5, 126.5]
440 [9, 127]
441 [10, 127.5]
442 [11.5, 128.0]
443 [15.5, 129.5]
444 *%-----|-----
445 *#
446 *# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
447 *#
448 ADD HYD NHYDsum=["SN13A"], NHYDs to add=["N13A"+"JR_GWM"]
449 *%-----|-----
450 *#
451 *# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
452 *#
453 ROUTE RESERVOIR NHYDout=["RES_GM"] ,NHYDin=["SN13A"],
454 RDT=[1](min),
455 TABLE of ( OUTFLOW-STORAGE ) values

```

```

456 (cms) - (ha-m)
457 [ 0.0 , 0.0 ]
458 [1.991, 2.144 ]
459 [2.693, 39.826 ]
460 [3.509, 81.697 ]
461 [4.578, 318.774 ]
462 [5.647, 594.947 ]
463 [7.109, 910.219 ]
464 [8.616, 1264.589 ]
465 [10.371, 1658.057 ]
466 [12.402, 2090.622 ]
467 [22.056, 3462.487 ]
468 [ -1 , -1 ] (max twenty pts)
469
470 NHYDovf=[ " " ] ,
471
472 *%-----|-----
473 *#
474 SAVE HYD NHYD=["RES_GM"], # OF PCYCLES=[-1], ICASEsh=[-1]
475 HYD_FILENAME=["H_RESGM"]
476 HYD_COMMENT=["Outflow from Res GM"]
477
478 *%-----|-----
479 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
480 *# (Approximated cross-section - see cross-section 258)
481 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
482 ROUTE CHANNEL NHYDout=["N12"] ,NHYDin=["RES_GM"] ,
483 RDT=[1](min),
484 CHLGTH=[5926](m), CHSLOPE=[0.0759](%),
485 FPSLOPE=[0.0759](%),
486 SECNUM=[1.0], NSEG=[1]
487 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
488 ( DISTANCE (m), ELEVATION (m))=
489 [-40, 132.5]
490 [-30, 132]
491 [-25, 131.5]
492 [-13, 130]
493 [-8, 127.00]
494 [-7, 126.50]
495 [-6, 126]
496 [-5.5, 125.50]
497 [0, 123.75]
498 [4.5, 125.50]
499 [6, 126]
500 [7.5, 126.5]
501 [9, 127]
502 [10, 127.5]
503 [11.5, 128.00]
504 [15.5, 129.5]
505
506 *%-----|-----
507 *#
508 *# Addition of Subwatershed Jock River at Ashton to Node 12
509 *#
510 ADD HYD NHYDsum=["S_N12"], NHYDs to add=["N12"+"JR_ASH"]
511 SAVE HYD NHYD=["S_N12"], # OF PCYCLES=[-1], ICASEsh=[-1]
512 HYD_FILENAME=["H_SN12"]
513 HYD_COMMENT=["flow at S_N12 near Ashton"]
514
515 *%-----|-----
516 *#
517 *# Sum of hydrographs from Node 12 routed to Node 11
518 *# (Approximated cross-section - see cross-section 258)
519 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
520 *# ROUTE CHANNEL NHYDout=["N11"] ,NHYDin=["S_N12"] ,
521 * RDT=[1](min),
522 * CHLGTH=[972](m), CHSLOPE=[0.0514](%),
523 * FPSLOPE=[0.0514](%),
524 * SECNUM=[1.0], NSEG=[1]
525 * ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
526 * ( DISTANCE (m), ELEVATION (m))=

```

```

522 * [-40, 132.5]
523 * [-30, 132]
524 * [-25, 131.5]
525 * [-13, 130]
526 * [-8, 127.00]
527 * [-7, 126.50]
528 * [-6, 126]
529 * [-5.5, 125.50]
530 * [0, 123.75]
531 * [4.5, 125.50]
532 * [6, 126]
533 * [7.5, 126.5]
534 * [9, 127]
535 * [10, 127.5]
536 * [11.5, 128.00]
537 * [15.5, 129.5]
538 *%-----|-----
539 *#
540 *# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
541 *#
542 ROUTE CHANNEL NHYDout=["Dum11"] ,NHYDin=["S_N12"] ,
543 RDT=[1](min),
544 CHLGTH=[972](m), CHSLOPE=[0.054](%),
545 FPSLOPE=[0.054](%),
546 SECNUM=[1.0], NSEG=[1]
547 ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
548 ( DISTANCE (m), ELEVATION (m))=
549 [-40, 132.5]
550 [-30, 132]
551 [-25, 131.5]
552 [-13, 130]
553 [-8, 127.00]
554 [-7, 126.50]
555 [-6, 126]
556 [-5.5, 125.50]
557 [0, 123.75]
558 [4.5, 125.50]
559 [6, 126]
560 [7.5, 126.5]
561 [9, 127]
562 [10, 127.5]
563 [11.5, 128.00]
564 [15.5, 129.5]
565 *%-----|-----
566 *#
567 *# Addition of Subwatershed 11 and No Name Creek to Node 11
568 *#
569 ADD HYD NHYDsum=["S_N11"], NHYDs to add=["Dum11"+"SW_11"+"NN_CK"]
570 *%-----|-----
571 *#
572 *# Sum of hydrographs from Node 11 routed to Node 10
573 *# Section 1
574 *#
575 ROUTE CHANNEL NHYDout=["N10"] ,NHYDin=["S_N11"] ,
576 RDT=[1](min),
577 CHLGTH=[14028](m), CHSLOPE=[0.1568](%),
578 FPSLOPE=[0.1568](%),
579 SECNUM=[1.0], NSEG=[5]
580 ( SEGROUGH, SEGDIST (m))=
581 [0.04,-52.82
582 0.1,-6.47
583 -0.05,6.47
584 0.1,45.36
585 0.04,423.88] NSEG times
586 ( DISTANCE (m), ELEVATION (m))=
587 [-226.24 ,112.50]

```

```

588             [-167.50 ,111.50]
589             [-106.81 ,111.00]
590             [-92.37 ,110.00]
591             [-52.82 ,109.00]
592             [-24.90, 109.00]
593             [-17.02, 108.50]
594             [-6.47, 108.00]
595             [6.47, 108.00]
596             [15.67, 108.50]
597             [18.95, 109.00]
598             [45.36, 109.50]
599             [120.79, 110.00]
600             [145.72, 111.00]
601             [181.56, 111.50]
602             [423.88, 112.50]
603 *%-----|-----|
604 *#
605 *# Addition of Subwatershed 10 to Node 10
606 *#
607 ADD HYD           NHYDsum=["S_N10"], NHYDs to add=["N10"+"SW_10"]
608 *%-----|-----|
609 SAVE HYD        NHYD=["S_N10"], # OF PCYCLES=[-1], ICASEsh=[-1]
610                   HYD_FILENAME=["H_SN10"]
611                   HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
612 *%-----|-----|
613 *# Addition of Kings Creek to S_N10
614 *#
615 ADD HYD           NHYDsum=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
616 *%-----|-----|
617 *#
618 *# Sum of hydrographs from Node 10 routed to Node 9
619 *# Section 2
620 *#
621 ROUTE CHANNEL   NHYDout=["N9"] ,NHYDin=["S_N10A"] ,
622                   RDT=[1](min),
623                   CHLGTH=[3982](m),  CHSLOPE=[0.0753](%),
624                                     FPSLOPE=[0.0753](%),
625                   SECNUM=[1.0],      NSEG=[4]
626                   ( SEGROUGH, SEGDIST (m))=
627                     [0.04,-30.27
628                      0.05,-18.42
629                      -0.05,18.42
630                      0.04,131.58] NSEG times
631                   ( DISTANCE (m), ELEVATION (m))=
632                     [-446.74, 106.00]
633                     [-415.68, 105.50]
634                     [-285.40, 105.00]
635                     [-173.77, 104.50]
636                     [-144.95, 104.00]
637                     [-111.18, 103.50]
638                     [-94.06, 103.00]
639                     [-71.02, 102.50]
640                     [-30.27, 102.00]
641                     [-19.33, 100.00]
642                     [-18.42, 99.50]
643                     [18.42, 99.50]
644                     [20.77, 100.00]
645                     [27.93, 101.00]
646                     [52.29, 101.00]
647                     [68.80, 101.50]
648                     [79.66, 103.00]
649                     [91.50, 103.50]
650                     [131.58, 104.00]
651 *%-----|-----|
652 *#
653 *# Addition of Subwatershed 9 and Nichols Creek to Node 9

```

```

654  *#
655  ADD HYD          NHYDsum=["S_N9"], NHYDs to add=["N9"+"SW_9"+"NC_CK"]
656  *%-----|-----
657  *#
658  *# Sum of hydrographs from Node 9 routed to Node 8
659  *# Section 3
660  *#
661  ROUTE CHANNEL    NHYDout=["N8"] ,NHYDin=["S_N9"] ,
662                  RDT=[1](min),
663                  CHLGTH=[2269](m),  CHSLOPE=[0.0882](%),
664                                          FPSLOPE=[0.0882](%),
665                  SECNUM=[1.0],      NSEG=[3]
666                  ( SEGROUGH, SEGDIST (m))=
667                    [0.1,-17.99
668                    -0.045,17.31
669                    0.1,456.58] NSEG times
670                  ( DISTANCE (m), ELEVATION (m))=
671                    [-201.19,100.50]
672                    [-135.21, 100.00]
673                    [-94.83, 99.50]
674                    [-67.05, 99.00]
675                    [-17.99, 98.50]
676                    [-16.02, 98.00]
677                    [-13.95, 97.50]
678                    [13.95, 97.50]
679                    [15.64, 98.00]
680                    [17.31, 98.50]
681                    [162.02, 98.50]
682                    [172.89 ,99.00]
683                    [314.38, 99.00]
684                    [343.78, 99.50]
685                    [365.67, 100.00]
686                    [376.68, 100.00 ]
687                    [393.11, 99.50]
688                    [404.97, 99.50]
689                    [431.70, 100.00]
690                    [456.58, 100.50 ]
691  *%-----|-----
692  *#
693  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
694  *#
695  ADD HYD          NHYDsum=["S_N8"], NHYDs to add=["N8"+"SW_8"+"HB_DR"]
696  *%-----|-----
697  *#
698  *# Sum of hydrographs from Node 8 routed to Node 7
699  *# Section 4
700  *#
701  ROUTE CHANNEL    NHYDout=["N7"] ,NHYDin=["S_N8"],
702                  RDT=[1](min),
703                  CHLGTH=[3750](m),  CHSLOPE=[0.0533](%),
704                                          FPSLOPE=[0.0533](%),
705                  SECNUM=[1.0],      NSEG=[3]
706                  ( SEGROUGH, SEGDIST (m))=
707                    [0.12,-18.11
708                    -0.07,17.22
709                    0.12,590.05] NSEG times
710                  ( DISTANCE (m), ELEVATION (m))=
711                    [-433.21, 102.00]
712                    [-425.34, 101.50]
713                    [-377.56, 101.50]
714                    [-366.23, 101.00]
715                    [-202.60, 100.50]
716                    [-96.25, 99.50]
717                    [-68.36 99.00]
718                    [-18.11, 98.50]
719                    [-13.81, 97.50]

```

```

720             [13.81, 97.50]
721             [17.22, 98.50]
722             [161.95, 98.50]
723             [173.11, 99.00]
724             [314.05, 99.00]
725             [365.52, 100.00]
726             [404.70, 99.50]
727             [476.74, 100.50]
728             [502.31, 101.00]
729             [584.69, 101.00]
730             [585.79, 101.00]
731             [590.05, 102.00]
732 *%-----|-----
733 *#
734 *# Addition of Subwatershed 7 to Node 7
735 *#
736 ADD HYD           NHYDsum=["S_N7"], NHYDs to add=["N7"+"SW_7"]
737 *%-----|-----
738 SAVE HYD        NHYD=["S_N7"], # OF PCYCLES=[-1], ICASEsh=[-1]
739                   HYD_FILENAME=["H_SN7"]
740                   HYD_COMMENT=["flow at S_N7: N7 + SW_7"]
741 *%-----|-----
742 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
743 *# Storage area and volumes were estimated from available topo maps.
744 *# Release rate from fen was assumed to be controlled by the downstream
745 *# river cross-section for summer conditions. It is was assumed that for up to
746 *# 0.75 m of water, the main channel of the river provided the storage. Above
747 *# this depth, the wetland starts to signigicantly store water.
748 *#
749 ROUTE RESERVOIR  NHYDout=["RES_RF"] ,NHYDin=["S_N7"] ,
750                   RDT=[1](min),
751                   TABLE of ( OUTFLOW-STORAGE ) values
752                             (cms) - (ha-m)
753                   TABLE of ( OUTFLOW-STORAGE ) values
754                             (cms) - (ha-m)
755                             [ 0.0 ,    0.0 ]
756                             [0.9051,   2.40]
757                             [2.907,    4.13]
758                             [9.744,    9.18]
759                             [20.304,   14.96]
760                             [34.167,   310.21]
761                             [74.993,   605.46]
762                             [104.876,  900.71]
763                             [140.56,  2892.00]
764                             [225.00,  3615.63]
765                             [ -1 ,    -1 ] (max twenty pts)
766                   NHYDovf=["      " ] ,
767 *%-----|-----
768 SAVE HYD        NHYD=["RES_RF"], # OF PCYCLES=[-1], ICASEsh=[-1]
769                   HYD_FILENAME=["H_ResRF"]
770                   HYD_COMMENT=["outflow of Richmond Fen"]
771 *%-----|-----
772 *#
773 *# Sum of hydrographs from Node 7 routed to Node 6
774 *# Section 5
775 *#
776 ROUTE CHANNEL  NHYDout=["N6"] ,NHYDin=["RES_RF"] ,
777                   RDT=[1](min),
778                   CHLGTH=[3056](m),  CHSLOPE=[0.0818](%),
779                                       FPSLOPE=[0.0818](%),
780                   SECNUM=[1.0],      NSEG=[5]
781                   ( SEGROUGH, SEGDIST (m))=
782                   [0.025,-70.8
783                   0.1,-23.9
784                   -0.05,23.9
785                   0.06,39.8

```

```

786             0.05,96.3] NSEG times
787             ( DISTANCE (m), ELEVATION (m))=
788                 [-100.8, 97.00]
789                 [-70.8, 96.50]
790                 [-52.0, 96.00]
791                 [-35.1, 95.50]
792                 [-30.6, 95.00]
793                 [-23.9, 94.54]
794                 [23.9, 94.54]
795                 [39.8, 95.00]
796                 [50.4, 95.50]
797                 [93.5, 96.00]
798                 [94.9, 96.50]
799                 [96.3, 97.00]
800 *%-----|-----
801 *#
802 *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
803 *#
804 ADD HYD             NHYDsum=["S_N6"], NHYDs to add=["N6"+"SW_6"+"VG_DR"]
805 *%-----|-----
806 *#
807 *# Sum of hydrographs from Node 6 routed to Node 5
808 *# Section 6
809 *#
810 ROUTE CHANNEL     NHYDout=["N5"] ,NHYDin=["S_N6"] ,
811                       RDT=[1](min),
812                       CHLGTH=[1852](m),   CHSLOPE=[0.0540](%),
813                                           FPSLOPE=[0.0540](%),
814                       SECNUM=[1.0],       NSEG=[3]
815                       ( SEGROUGH, SEGDIST (m))=
816                           [0.035,-131.59
817                           -0.045,48.96
818                           0.1,239.04] NSEG times
819                       ( DISTANCE (m), ELEVATION (m))=
820                           [-686.30, 94.50]
821                           [-675.70, 94.00]
822                           [-492.52, 93.00]
823                           [-467.28, 94.00]
824                           [-131.59, 94.00]
825                           [-92.79, 92.50]
826                           [-18.06, 91.00]
827                           [18.06, 91.00]
828                           [43.47, 92.50]
829                           [48.96, 94.00]
830                           [177.43, 94.00]
831                           [239.04,94.50]
832 *%-----|-----
833 *#
834 *# Addition of Subwatershed 5 and Flowing Creek to Node 5
835 *#
836 ADD HYD             NHYDsum=["S_N5"], NHYDs to add=["N5"+"SW_5"+"FL_CK"]
837 *%-----|-----
838 *#
839 *# Sum of hydrographs from Node 5 routed to Node 5A
840 *# Section 7
841 *#
842 ROUTE CHANNEL     NHYDout=["N5A"] ,NHYDin=["S_N5"] ,
843                       RDT=[1](min),
844                       CHLGTH=[556](m),   CHSLOPE=[0.0900](%),
845                                           FPSLOPE=[0.0900](%),
846                       SECNUM=[1.0],       NSEG=[4]
847                       ( SEGROUGH, SEGDIST (m))=
848                           [0.04,-41.5
849                           0.1,-14.0
850                           -0.045,14.0
851                           0.1,41.1] NSEG times

```



```

852          ( DISTANCE (m), ELEVATION (m))=
853              [-275.8, 93.00]
854              [-248.6, 92.50]
855              [-237.0, 92.00]
856              [-219.3, 91.50]
857              [-202.1, 91.50]
858              [-186.0, 92.00]
859              [-129.2, 92.00]
860              [-117.6, 91.50]
861              [-100.6, 91.00]
862              [-41.5, 91.00]
863              [-20.0, 91.00]
864              [-14.0, 90.54]
865              [14.0, 90.54]
866              [15.3, 91.00]
867              [17.3, 91.50]
868              [38.4, 92.00]
869              [39.8, 92.50]
870              [41.1, 93.00]
871  *%-----|-----
872  *#
873  *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
874  *#
875  ADD HYD          NHYDsum=["S_N5A"], NHYDs to add=["N5A"+"SW_5A2"+"SW_5A1"]
876  *%-----|-----
877  *#
878  *# Sum of hydrographs from Node 5A routed to Node 4
879  *# Section 8
880  *#
881  ROUTE CHANNEL    NHYDout=["N4"] ,NHYDin=["S_N5A"] ,
882                  RDT=[1](min),
883                  CHLGTH=[4630](m),  CHSLOPE=[0.0432](%),
884                                          FPSLOPE=[0.0432](%),
885                  SECNUM=[1.0],      NSEG=[3]
886                  ( SEGROUGH, SEGDIST (m))=
887                      [0.05,-28.2
888                      -0.035,28.2
889                      0.05,173.1] NSEG times
890                  ( DISTANCE (m), ELEVATION (m))=
891                      [-38.9, 92.00]
892                      [-35.8, 91.50]
893                      [-33.3, 91.00]
894                      [-28.2, 90.50]
895                      [-15.0, 87.48]
896                      [-5.0, 88.34]
897                      [5.0, 86.20]
898                      [15.0, 88.55]
899                      [28.2, 90.50]
900                      [29.7, 91.00]
901                      [46.5, 91.00]
902                      [127.8, 91.00]
903                      [148.7, 91.50]
904                      [173.1, 92.00]
905  *%-----|-----
906  *#
907  *# Addition of Subwatershed 4 and Leamy Creek to Node 4
908  *#
909  ADD HYD          NHYDsum=["S_N4"], NHYDs to add=["N4"+"SW_4"+"LM_CK"]
910  SAVE HYD        NHYD=["S_N4"], # OF PCYCLES=[-1], ICASEsh=[1]
911                  HYD_COMMENT=["flow at S_N4"]
912  *%-----|-----
913  *#
914  *# Sum of hydrographs from Node 4 routed to Node 2
915  *# Section 9
916  *#
917  ROUTE CHANNEL    NHYDout=["N2"] ,NHYDin=["S_N4"] ,

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918 RDT=[1](min),
919 CHLGTH=[1667](m), CHSLOPE=[0.0600](%),
920 FPSLOPE=[0.0600](%),
921 SECNUM=[1.0], NSEG=[4]
922 ( SEGROUGH, SEGDIST (m))=
923 [0.1,-28.0
924 -0.04,28.4
925 0.06,31.7
926 0.04,80.2] NSEG times
927 ( DISTANCE (m), ELEVATION (m))=
928 [-36.3, 92.00]
929 [-32.6, 91.50]
930 [-30.2, 91.00]
931 [-28.0, 90.45]
932 [-15.0, 87.48]
933 [-5.0, 88.34]
934 [5.0, 86.20]
935 [15.0, 88.55]
936 [28.0, 90.45]
937 [28.4, 90.50]
938 [30.4, 91.00]
939 [31.7, 91.50]
940 [80.2, 92.00]
941 *%-----|-----
942 *#
943 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
944 *#
945 ADD HYD NHYDsum=["S_N2"], NHYDs to add=["N2"+"SW_2"+"SM_DR"+"MO_DR"]
946 *%-----|-----
947 SAVE HYD NHYD=["S_N2"], # OF PCYCLES=[-1], ICASEsh=[-1]
948 HYD_FILENAME=["H_SN2"]
949 HYD_COMMENT=["flow at S_N2 Jock River Gauge at Moodie Dr."]
950 *%-----|-----
951 *#
952 *# Sum of hydrographs from Node 2 routed to Node 1
953 *# Section 10
954 *#
955 *#*****
956 *%READ HYD NHYD=["S_N2"],
957 *% HYD_FILENAME=["H-S_N2"]
958 *%-----|-----
959 *#
960 *# Hydrograph from Node 2 routed to Node 416
961 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
962 *#
963 ROUTE CHANNEL NHYDout=["N_416"] ,NHYDin=["S_N2"] ,
964 RDT=[1](min),
965 CHLGTH=[2327](m), CHSLOPE=[0.0498](%),
966 FPSLOPE=[0.0498](%),
967 SECNUM=[1.0], NSEG=[3]
968 ( SEGROUGH, SEGDIST (m))=
969 [0.075,-23.96
970 -0.055,23.96
971 0.075,157.38] NSEG times
972 ( DISTANCE (m), ELEVATION (m))=
973 [-336.97,93.5]
974 [-318.85,93]
975 [-259,92.5]
976 [-133.18,92]
977 [-33.17,92]
978 [-27.21,92]
979 [-26.14,91.5]
980 [-24.99,91]
981 [-23.96,90.5]
982 [-14.33,88.26]
983 [-0.68,88.12]

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```

984 [14.33,88.26]
985 [23.96,90.5]
986 [32.12,91]
987 [43.74,91.5]
988 [57.09,92]
989 [73.53,92.5]
990 [108.27,93]
991 [125.88,93.5]
992 [144.81,94]
993 [157.38,94.5]
994 *%-----|-----|
995 *#*****|
996 *# Catchment SW-1a
997 *# - Portion of RVCA catchment SW_1 outside of Reach 1 subwatershed
998 *# - Undeveloped agricultural land
999 *#*****|
1000 CONTINUOUS NASHYD NHYD=["SW_1a"], DT=[1]min, AREA=[536.42](ha),
1001 DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
1002 N=[3], TP=[2.79]hrs,
1003 Continuous simulation parameters:
1004 IaREcper=[4](hrs),
1005 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1006 InterEventTime=[12](hrs)
1007 Baseflow simulation parameters:
1008 BaseFlowOption=[1],
1009 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1010 VHydCond=[0.055](mm/hr), END=-1
1011 *%-----|-----|
1012 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
1013 CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[1](min), AREA=[44.93](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
1014 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAPER=[4.67](mm), SLPP=[2.0](%),
1015 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
1016 LGI=[547.296](m), MNI=[0.013], SCI=[0](min),
1017 Continuous simulation parameters:
1018 IaREcper=[4](hrs), IaREcimp=[4](hrs),
1019 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1020 InterEventTime=[12](hrs), END=-1
1021 *%-----|-----|
1022 COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CINLET=[4.591](cms), NINLET=[1],
1023 MajNHYD=["S-1-OkMJ"]
1024 MinNHYD=["S-1-OkMN"]
1025 TMJSTO=[9999999](cu-m)
1026 *%-----|-----|
1027 ADD HYD NHYDsum=["S-1-OkS"], NHYDs to add=["S-1-OkMJ"+"S-1-OkMN"]
1028 *%-----|-----|
1029 ROUTE RESERVOIR NHYDout=["S-1-OkSR"],NHYDin=["S-1-OkS"],
1030 RDT=[1](min),
1031 TABLE of ( OUTFLOW-STORAGE ) values
1032 (cms) - (ha-m)
1033 [ 0.0 , 0.0 ]
1034 [ 0.5370, 1.7917 ]
1035 [ -1 , -1 ] (max twenty pts)
1036 NHYDovf=["S-1-OkSovf"]
1037 *%-----|-----|
1038 ADD HYD NHYDsum=["SN_416"], NHYDs to
add=["N_416"+"SW_1a"+"S-1-OkSR"+"S-1-OkSovf"]
1039 *%-----|-----|
1040 SAVE HYD NHYD=["SN_416"], # OF PCYCLES=[-1], ICASEsh=[1]
1041 HYD_COMMENT=["Total Flows at Highway 416 before Station 7245"]
1042 *%-----|-----|
1043 *#
1044 *# Hydrograph from Node 416 routed to Node at Okeefe drain

```

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1045  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245
1046  *#
1047  ROUTE CHANNEL      NHYDout=["N_OK"] ,NHYDin=["SN_416"] ,
1048                    RDT=[1](min),
1049                    CHLGTH=[497](m),  CHSLOPE=[0.3018](%),
1050                    FPSLOPE=[0.3018](%),
1051                    SECNUM=[1.0],      NSEG=[3]
1052                    ( SEGROUGH, SEGDIST (m))=
1053                    [0.075,-19.40
1054                    -0.055,19.40
1055                    0.075,377.02] NSEG times
1056                    ( DISTANCE (m), ELEVATION (m))=
1057                    [-1061.41, 92.50]
1058                    [-945.91, 92.00]
1059                    [-783.64, 91.50]
1060                    [-136.74, 91.00]
1061                    [-86.04, 91.00]
1062                    [-20.86, 91.00]
1063                    [-20.18, 90.50]
1064                    [-19.40, 90.00]
1065                    [-11.68, 86.89]
1066                    [0.00, 86.10]
1067                    [12.09, 86.81]
1068                    [19.40, 90.00]
1069                    [34.68, 90.50]
1070                    [60.56, 91.00]
1071                    [170.14, 91.00]
1072                    [175.05, 90.50]
1073                    [180.29, 90.00]
1074                    [193.41, 90.00]
1075                    [195.98, 90.50]
1076                    [377.02, 92.50]
1077  *%-----|-----|
1078  *#*****|
1079  *# Catchment OKEEFE
1080  *# - To O'Keefe drain (north of the Jock)
1081  *# - Developed with assumed 43% imp.
1082  *# - 2020-12-01 add Okeefe model (Area 513.02 HA) instead of current Okeefe (Area
1083  *# - 2020-11-20 Okeefe detailed model was added as per the NOVATECH SWMHYMO model
1084  *# - Citi-Gate 2014).
1085  *%-----|-----|
1086  *#*****|
1087  *#*****|
1088  CONTINUOUS NASHYD  NHYD=["O-1"], DT=[1]min, AREA=[63.72](ha),
1089                    DWF=[0](cms), CN/C=[61], IA=[6.2](mm), N=[3], TP=[.9]hrs,
1090                    Continuous simulation parameters:
1091                    IaREcper=[4](hrs),
1092                    SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1093                    InterEventTime=[12](hrs)
1094                    Baseflow simulation parameters:
1095                    BaseFlowOption=[1] ,
1096                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1097                    VHydCond=[0.055](mm/hr),  END=-1
1098  *%-----|-----|
1099  *#*****|
1100  *#*****|
1101  ROUTE CHANNEL      NHYDout=["O-1R"], NHYDin=["O-1"], RDT=[1](min),
1102                    CHLGTH=[960](m), CHSLOPE=[0.63](%), FPSLOPE=[0.63](%),
1103                    SECNUM=[1], NSEG=[3]
1104                    ( SEGROUGH, SEGDIST (m))=[0.06,4 -.043,6 0.06,10] NSEG times
1105                    ( DISTANCE (m), ELEVATION (m))=[0.00, 2.0]
1106                    [0.0, 2.0]
1107                    [4.0, 0.0]
1108                    [6.0, 0.0]

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1108         [10.0, 2.0]
1109 *%-----|-----|
1110 CONTINUOUS NASHYD NHYD=["O-2"], DT=[1]min, AREA=[28.61](ha),
1111 DWF=[0](cms), CN/C=[57], IA=[5.2](mm), N=[3], TP=[1.1]hrs,
1112 Continuous simulation parameters:
1113 IaREcper=[4](hrs),
1114 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1115 InterEventTime=[12](hrs)
1116 Baseflow simulation parameters:
1117 BaseFlowOption=[1] ,
1118 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1119 VHydCond=[0.055](mm/hr), END=-1
1120 *%-----|-----|
1121 CONTINUOUS NASHYD NHYD=["O-4"], DT=[1]min, AREA=[46.94](ha),
1122 DWF=[0](cms), CN/C=[49], IA=[9.2](mm), N=[3], TP=[0.9]hrs,
1123 Continuous simulation parameters:
1124 IaREcper=[4](hrs),
1125 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1126 InterEventTime=[12](hrs)
1127 Baseflow simulation parameters:
1128 BaseFlowOption=[1] ,
1129 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1130 VHydCond=[0.055](mm/hr), END=-1
1131 *%-----|-----|
1132 *TOTAL EXTERNAL FLOW NORTH OF O'KEEFE CT. CROSSING
1133 ADD HYD NHYDsum=["OKF-N"], NHYDs to add=["O-1R"+"O-2"+"O-4"]
1134 *%-----|-----|
1135 *ROUTE FLOW THROUGH AREA O-6
1136 ROUTE CHANNEL ROUTE CHANNEL NHYDout=["OKF-NR"], NHYDin=["OKF-N"], RDT=[1](min),
1137 CHLGTH=[210](m), CHSLOPE=[.81](%), FPSLOPE=[.81](%),
1138 SECNUM=[1], NSEG=[3]
1139 ( SEGRROUGH, SEGDIST (m))=[0.043,22.43 -0.043,25.07
1140 0.043,45.54] NSEG times
1141 ( DISTANCE (m), ELEVATION (m))=[0.00, 3.73]
1142 (14.62, 1.56)
1143 (18.41, 1.44)
1144 (22.43, 0.00)
1145 (25.07, 0.70)
1146 (29.10, 1.79)
1147 (33.73, 2.71)
1148 (45.54, 3.58)
1149 *%-----|-----|
1150 CONTINUOUS NASHYD NHYD=["O-6"], DT=[1]min, AREA=[16.46](ha),
1151 DWF=[0](cms), CN/C=[43], IA=[9.2](mm), N=[3], TP=[0.7]hrs,
1152 Continuous simulation parameters:
1153 IaREcper=[4](hrs),
1154 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1155 InterEventTime=[12](hrs)
1156 Baseflow simulation parameters:
1157 BaseFlowOption=[1] ,
1158 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1159 VHydCond=[0.055](mm/hr), END=-1
1160 *%-----|-----|
1161 CONTINUOUS STANDHYD NHYD=["O-3"], DT=[1](min), AREA=[39.67](ha), XIMP=[0.15],
1162 TIMP=[0.30], DWF=[0](cms),
1163 LOSS=[2], SCS curve number CN=[50], Pervious surfaces:
1164 IAper=[4.67](mm), SLPP=[0.32](%),
1165 LGP=[440](m), MNP=[0.035], SCP=[0](min), Impervious surfaces:
1166 IAimp=[1.57](mm), SLPI=[0.32](%),
1167 LGI=[1880](m), MNI=[0.013], SCI=[0](min),
1168 Continuous simulation parameters:
1169 IaREcper=[4](hrs), IaREcimp=[4](hrs),
1170 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1171 InterEventTime=[12](hrs), END=-1
1172 *%-----|-----|
1173 CONTINUOUS STANDHYD NHYD=["O-5"], DT=[1](min), AREA=[60.63](ha), XIMP=[0.13],

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TIMP=[0.26], DWF=[0](cms),
1171      LOSS=[2], SCS curve number CN=[61],
1172      Pervious surfaces: IAper=[4.67](mm), SLPP=[1.38](%),
1173      LGP=[550](m), MNP=[0.035], SCP=[0](min), Impervious surfaces:
      IAimp=[1.57](mm), SLPI=[1.38](%),
1174      LGI=[1450](m), MNI=[0.013], SCI=[0](min),
1175      Continuous simulation parameters:
1176      IaRECper=[4](hrs), IaRECimp=[4](hrs),
1177      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1178      InterEventTime=[12](hrs), END=-1
1179  *%-----|-----|
1180  *TOTAL EXTERNAL FLOWS WEST OF THE SITE AND NORTH OF O'KEEFE CRT
1181  *%-----|-----|
1182  ADD HYD      NHYDsum=["PT1"], NHYDs to add=["OKF-NR"+"O-3"+"O-5"+"O-6"]
1183  *%-----|-----|
1184  CONTINUOUS NASHYD  NHYD=["O-7"], DT=[1]min, AREA=[5.28](ha),
1185      DWF=[0](cms), CN/C=[54], IA=[7.5](mm), N=[3], TP=[0.6]hrs,
1186      Continuous simulation parameters:
1187      IaRECper=[4](hrs),
1188      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1189      InterEventTime=[12](hrs)
1190      Baseflow simulation parameters:
1191      BaseFlowOption=[1] ,
1192      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1193      VHydCond=[0.055](mm/hr), END=-1
1194  *%-----|-----|
1195  *ANALYSIS POINT 1 - TOTAL FLOW NORTH OF FALLOWFIELD DR. AND O'KEEFE CRT.
1196  ADD HYD      NHYDsum=["FF"], NHYDs to add=["PT1"+"O-7"]
1197  *%-----|-----|
1198  *ROUTE FLOW through O'Keefe Drain 1
1199  ROUTE CHANNEL  NHYDout=["DRAIN1"], NHYDin=["FF"], RDT=[1](min),
1200      CHLGTH=[302]{m}, CHSLOPE=[1.00](%), FPSLOPE=[1.00](%),
1201      SECNUM=[1], NSEG=[3]
1202      ( SEGROUGH, SEGDIST (m))=[0.07,13.45 -0.043,16.55 0.07,30.00] NSEG
      times
1203      ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1204      (3.45, 0.60)
1205      (13.45, 0.50)
1206      (14.45, 0.00)
1207      (15.55, 0.00)
1208      (16.55, 0.50)
1209      (26.55, 0.60)
1210      (30.00, 1.70)
1211  *%-----|-----|
1212  CONTINUOUS NASHYD  NHYD=["D1"], DT=[1]min, AREA=[1.17](ha),
1213      DWF=[0](cms), CN/C=[84], IA=[9.0](mm), N=[3], TP=[0.28]hrs,
1214      Continuous simulation parameters:
1215      IaRECper=[4](hrs),
1216      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1217      InterEventTime=[12](hrs)
1218      Baseflow simulation parameters:
1219      BaseFlowOption=[1] ,
1220      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1221      VHydCond=[0.055](mm/hr), END=-1
1222  *%-----|-----|
1223  CONTINUOUS STANDHYD  NHYD=["A1"], DT=[1]min, AREA=[2.50](ha), XIMP=[0.68], TIMP=[0.85],
      DWF=[0](cms), LOSS=[1]:
1224      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
      F=[0.00](mm),
1225      Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
      MNP=[0.250], SCP=[0](min),
1226      Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
      LGI=[223.607](m), MNI=[0.013], SCI=[0](min),
1227      Continuous simulation parameters:
1228      IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
      END=-1

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1229 *%-----|-----|
1230 ROUTE RESERVOIR NHYDout=["A1-STR"], NHYDin=["A1"], RDT=[1](min),
1231 TABLE of ( OUTFLOW-STORAGE ) values
1232 (cms) - (ha-m)
1233 [ 0.000 , 0.000 ]
1234 [ 0.035 , 0.038 ]
1235 [ 0.072 , 0.051 ]
1236 [ 0.100 , 0.059 ]
1237 [ 0.125 , 0.070 ]
1238 [ 0.160 , 0.074 ]
1239 [ 0.185 , 0.081 ]
1240 [ -1 , -1 ] (max twenty pts)
1241 NHYDovf=["A1-OVF"]
1242 *%-----|-----|
1243 CONTINUOUS STANDHYD NHYD=["ST-2"], DT=[1]min, AREA=[0.59](ha), XIMP=[0.46],
TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1244 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1245 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1246 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[108.628](m), MNI=[0.013], SCI=[0](min),
1247 Continuous simulation parameters:
1248 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1249 *%-----|-----|
1250 ROUTE RESERVOIR NHYDout=["ST2STR"], NHYDin=["ST-2"], RDT=[1](min),
1251 TABLE of ( OUTFLOW-STORAGE ) values
1252 (cms) - (ha-m)
1253 [ 0.000 , 0.0000 ]
1254 [ 0.052 , 0.0010 ]
1255 [ 0.053 , 0.0080 ]
1256 [ -1 , -1 ] (max twenty pts)
1257 NHYDovf=["ST2OVF"]
1258 *%-----|-----|
1259 *%-----|-----|
1260 *TOTAL FLOW NORTH OF STRANDHERD DR. (EAST BRANCH) CROSSING
1261 *%-----|-----|
1262 CONTINUOUS NASHYD NHYD=["O-8"], DT=[1]min, AREA=[60.55](ha),
1263 DWF=[0](cms), CN/C=[69], IA=[4.0](mm), N=[3], TP=[1.0]hrs,
1264 Continuous simulation parameters:
1265 IaREcper=[4](hrs),
1266 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1267 InterEventTime=[12](hrs)
1268 Baseflow simulation parameters:
1269 BaseFlowOption=[1] ,
1270 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1271 VHydCond=[0.055](mm/hr), END=-1
1272 *%-----|-----|
1273 ROUTE PIPE PTYPE=[2]rect, NHYDout=["O8PIPE"], RNUMBER=[1], PWIDTH=[1800](mm),
PHEIGHT=[1200](mm), PLNGTH=[335.1](m),
1274 PROUGH=[0.013], PSLOPE=[0.001](m/m), NHYDin=["O-8"], RDT=[1](min)
1275 *%-----|-----|
1276 *%-----|-----|
1277 ADD HYD NHYDsum=["ST2-IN"], NHYDs to
add=["DRAIN1"+"D1"+"A1-STR"+"A1-OVF"+"ST2STR"+"ST2OVF"+"O8PIPE"]
1278 *%-----|-----|
1279 CONTINUOUS STANDHYD NHYD=["A7"], DT=[1]min, AREA=[3.51](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1280 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1281 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1282 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[264.953](m), MNI=[0.013], SCI=[0](min),
1283 Continuous simulation parameters:

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1284 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
      END=-1
1285 *%-----|-----|
1286 ROUTE RESERVOIR NHYDout=["A7-STR"], NHYDin=["A7"], RDT=[1](min),
1287 TABLE of ( OUTFLOW-STORAGE ) values
1288 (cms) - (ha-m)
1289 [ 0.000 , 0.000 ]
1290 [ 0.049 , 0.054 ]
1291 [ 0.102 , 0.072 ]
1292 [ 0.140 , 0.082 ]
1293 [ 0.175 , 0.099 ]
1294 [ 0.225 , 0.105 ]
1295 [ 0.260 , 0.114 ]
1296 [ -1 , -1 ] (max twenty pts)
1297 NHYDovf=["A7-OVF"]
1298 *%-----|-----|
1299 CONTINUOUS STANDHYD NHYD=["ST-3"], DT=[1]min, AREA=[0.71](ha), XIMP=[0.46],
      TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1300 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
      F=[0.00](mm),
1301 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
      MNP=[0.250], SCP=[0](min),
1302 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
      LGI=[119.164](m), MNI=[0.013], SCI=[0](min),
1303 Continuous simulation parameters:
1304 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
      END=-1
1305 *%-----|-----|
1306 ROUTE RESERVOIR NHYDout=["ST3STR"], NHYDin=["ST-3"], RDT=[1](min),
1307 TABLE of ( OUTFLOW-STORAGE ) values
1308 (cms) - (ha-m)
1309 [ 0.000 , 0.0000 ]
1310 [ 0.063 , 0.0010 ]
1311 [ 0.064 , 0.0094 ]
1312 [ -1 , -1 ] (max twenty pts)
1313 NHYDovf=["ST3OVF"]
1314 *%-----|-----|
1315 *ANALYSIS POINT 2 - TOTAL FLOW AT OUTLET OF STREET 2/3 INTERSECTION
1316 *%-----|-----|
1317 ADD HYD NHYDsum=["PT2ST3"], NHYDs to
      add=["ST2-IN"+"A7-STR"+"A7-OVF"+"ST3STR"+"ST3OVF"]
1318 *%-----|-----|
1319 *ROUTE FLOW through O'Keefe Drain 2
1320 ROUTE CHANNEL NHYDout=["DRAIN2"], NHYDin=["PT2ST3"], RDT=[1](min),
1321 CHLGTH=[592]{m}, CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1322 SECNUM=[1], NSEG=[3]
1323 ( SEGROUGH, SEGDIST (m))=[0.07,12.60 -0.043,17.40 0.07,30.00] NSEG
      times
1324 ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1325 (2.60, 0.95)
1326 (12.60, 0.75)
1327 (14.10, 0.00)
1328 (15.90, 0.00)
1329 (17.40, 0.75)
1330 (27.40, 0.95)
1331 (30.00, 1.70)
1332 *%-----|-----|
1333 CONTINUOUS NASHYD NHYD=["D2"], DT=[1]min, AREA=[2.28](ha), DWF=[0](cms), CN/C=[84],
      IA=[9.0](mm),
1334 N=[3], TP=[0.99]hrs,
1335 Continuous simulation parameters:
1336 IaREcper=[4](hrs),
1337 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1338 InterEventTime=[12](hrs)
1339 Baseflow simulation parameters:
1340 BaseFlowOption=[1] ,

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1341          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1342          VHydCond=[0.055](mm/hr),   END=-1
1343  *%-----|-----|
1344  CONTINUOUS STANDHYD NHYD=["A17"], DT=[1]min, AREA=[12.04](ha), XIMP=[0.68],
1345  TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
1346          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1347          F=[0.00](mm),
1348          Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1349          MNP=[0.250], SCP=[0](min),
1350          Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1351          LGI=[490.714](m), MNI=[0.013], SCI=[0](min),
1352          Continuous simulation parameters:
1353          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1354          END=-1
1355  *%-----|-----|
1356  ROUTE RESERVOIR NHYDout=["A17STR"], NHYDin=["A17"], RDT=[1](min),
1357          TABLE of ( OUTFLOW-STORAGE ) values
1358          (cms) - (ha-m)
1359          [ 0.000 , 0.000 ]
1360          [ 0.169 , 0.185 ]
1361          [ 0.349 , 0.248 ]
1362          [ 0.482 , 0.283 ]
1363          [ 0.602 , 0.338 ]
1364          [ 0.771 , 0.359 ]
1365          [ 0.891 , 0.391 ]
1366          [ -1 , -1 ] (max twenty pts)
1367          NHYDovf=["A17OVF"]
1368  *%-----|-----|
1369  CONTINUOUS STANDHYD NHYD=["ST-4"], DT=[1]min, AREA=[0.35](ha), XIMP=[0.46],
1370  TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1371          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1372          F=[0.00](mm),
1373          Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1374          MNP=[0.250], SCP=[0](min),
1375          Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[83.666](m),
1376          MNI=[0.013], SCI=[0](min),
1377          Continuous simulation parameters:
1378          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1379          END=-1
1380  *%-----|-----|
1381  ROUTE RESERVOIR NHYDout=["ST4STR"], NHYDin=["ST-4"], RDT=[1](min),
1382          TABLE of ( OUTFLOW-STORAGE ) values
1383          (cms) - (ha-m)
1384          [ 0.000 , 0.0000 ]
1385          [ 0.031 , 0.0010 ]
1386          [ 0.032 , 0.0050 ]
1387          [ -1 , -1 ] (max twenty pts)
1388          NHYDovf=["ST4OVF"]
1389  *%-----|-----|
1390  CONTINUOUS STANDHYD NHYD=["A18"], DT=[1]min, AREA=[5.30](ha), XIMP=[0.68], TIMP=[0.85],
1391  DWF=[0](cms), LOSS=[1]:
1392          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1393          F=[0.00](mm),
1394          Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1395          MNP=[0.250], SCP=[0](min),
1396          Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1397          LGI=[325.576](m), MNI=[0.013], SCI=[0](min),
1398          Continuous simulation parameters:
1399          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1400          END=-1
1401  *%-----|-----|
1402  ROUTE RESERVOIR NHYDout=["A18STR"], NHYDin=["A18"], RDT=[1](min),
1403          TABLE of ( OUTFLOW-STORAGE ) values
1404          (cms) - (ha-m)
1405          [ 0.000 , 0.000 ]
1406          [ 0.074 , 0.082 ]

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1392         [ 0.154 , 0.109 ]
1393         [ 0.212 , 0.125 ]
1394         [ 0.265 , 0.149 ]
1395         [ 0.339 , 0.158 ]
1396         [ 0.392 , 0.172 ]
1397         [ -1 , -1 ] (max twenty pts)
1398         NHYDovf=["A18OVF"]
1399 *%-----|-----|
1400 *ANALYSIS POINT 3 - TOTAL FLOW AT OUTLET OF STREET 4
1401 *%-----|-----|
1402 ADD HYD          NHYDsum=["PT3ST4"], NHYDs to
add=["DRAIN2"+"D2"+"A17STR"+"A17OVF"+"ST4STR"+"ST4OVF"+"A18STR"+"A18OVF"]
1403 *%-----|-----|
1404 *ROUTE FLOW through O'Keefe Drain 3
1405 ROUTE CHANNEL   NHYDout=["DRAIN3"], NHYDin=["PT3ST4"], RDT=[1](min),
1406                CHLGTH=[525](m), CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1407                SECNUM=[1], NSEG=[3]
1408                ( SEGROUGH, SEGDIST (m))=[0.07,12.50 -0.043,17.50 0.07,30.00] NSEG
1409                times
1410                ( DISTANCE (m), ELEVATION (m))=[0.00, 1.70]
1411                (2.50, 1.00)
1412                (12.50, 0.80)
1413                (14.10, 0.00)
1414                (15.90, 0.00)
1415                (17.50, 0.80)
1416                (27.50, 1.00)
1417                (30.00, 1.70)
1418 *%-----|-----|
1419 CONTINUOUS NASHYD NHYD=["D3"], DT=[1]min, AREA=[2.51](ha),
1420                DWF=[0](cms), CN/C=[86], IA=[8.7](mm), N=[3], TP=[0.73]hrs,
1421                Continuous simulation parameters:
1422                IaREcper=[4](hrs),
1423                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1424                InterEventTime=[12](hrs)
1425                Baseflow simulation parameters:
1426                BaseFlowOption=[1] ,
1427                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1428                VHydCond=[0.055](mm/hr), END=-1
1429 *%-----|-----|
1430 CONTINUOUS STANDHYD NHYD=["C1"], DT=[1]min, AREA=[3.41](ha), XIMP=[0.68], TIMP=[0.85],
1431                DWF=[0](cms), LOSS=[1]:
1432                Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1433                F=[0.00](mm),
1434                Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1435                MNP=[0.250], SCP=[0](min),
1436                Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
1437                LGI=[261.151](m), MNI=[0.013], SCI=[0](min),
1438                Continuous simulation parameters:
1439                IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
1440                END=-1
1441 *%-----|-----|
1442 ROUTE RESERVOIR NHYDout=["C1-STR"], NHYDin=["C1"], RDT=[1](min),
1443                TABLE of ( OUTFLOW-STORAGE ) values
1444                (cms) - (ha-m)
1445                [ 0.000 , 0.000 ]
1446                [ 0.048 , 0.052 ]
1447                [ 0.099 , 0.070 ]
1448                [ 0.136 , 0.080 ]
1449                [ 0.170 , 0.096 ]
1450                [ 0.218 , 0.102 ]
1451                [ 0.252 , 0.111 ]
1452                [ -1 , -1 ] (max twenty pts)
1453                NHYDovf=["C1-OVF"]
1454 *%-----|-----|
1455 CONTINUOUS STANDHYD NHYD=["ST-5"], DT=[1]min, AREA=[0.45](ha), XIMP=[0.46],
1456                TIMP=[0.57], DWF=[0](cms), LOSS=[1]:

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1450 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
      F=[0.00](mm),
1451 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
      MNP=[0.250], SCP=[0](min),
1452 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[94.868](m),
      MNI=[0.013], SCI=[0](min),
1453 Continuous simulation parameters:
1454 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
      END=-1
1455 *%-----|-----|
1456 ROUTE RESERVOIR NHYDout=["ST5STR"], NHYDin=["ST-5"], RDT=[1](min),
1457 TABLE of ( OUTFLOW-STORAGE ) values
1458 (cms) - (ha-m)
1459 [ 0.000 , 0.0000 ]
1460 [ 0.040 , 0.0010 ]
1461 [ 0.041 , 0.0062 ]
1462 [ -1 , -1 ] (max twenty pts)
1463 NHYDovf=["ST5OVF"]
1464 *%-----|-----|
1465 ADD HYD NHYDsum=["ST5-E"], NHYDs to
add=["DRAIN3"+"D3"+"C1-STR"+"C1-OVF"+"ST5STR"+"ST5OVF"]
1466 *%-----|-----|
1467 CONTINUOUS STANDHYD NHYD=["STRAND"], DT=[1](min), AREA=[7.59](ha),
1468 XIMP=[0.64], TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
1469 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
      F=[0.00](mm),
1470 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
      MNP=[0.250], SCP=[0](min),
1471 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[1230](m),
      MNI=[0.013], SCI=[0](min),
1472 Continuous simulation parameters:
1473 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
      END=-1
1474 *%-----|-----|
1475 ROUTE RESERVOIR NHYDout=["S-POND"], NHYDin=["STRAND"], RDT=[1](min),
1476 TABLE of ( OUTFLOW-STORAGE ) values
1477 (cms) - (ha-m)
1478 [ 0.000 , 0.000 ]
1479 [ 0.033 , 0.188 ]
1480 [ 0.057 , 0.253 ]
1481 [ 0.104 , 0.287 ]
1482 [ 0.160 , 0.336 ]
1483 [ 0.340 , 0.346 ]
1484 [ 0.471 , 0.360 ]
1485 [ 0.824 , 0.390 ]
1486 [ -1 , -1 ] (max twenty pts)
1487 NHYDovf=["S-OVF"]
1488 *%-----|-----|
1489 ADD HYD NHYDsum=["SSAOUT"], NHYDs to add=["ST5-E"+"S-POND"+"S-OVF"]
1490 *%-----|-----|
1491 SAVE HYD NHYD=["SSAOUT"], # OF PCYCLES=[5], ICASEsh=[1]
1492 HYD_COMMENT=["SSAOUT"]
1493 *%-----|-----|
1494 CONTINUOUS STANDHYD NHYD=["Area-A"], DT=[1]min, AREA=[66.75](ha), XIMP=[0.64],
1495 TIMP=[0.80], DWF=[0](cms), LOSS=[1]:
1496 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
      F=[0.00](mm),
1497 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
      MNP=[0.250], SCP=[0](min),
1498 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
      LGI=[1155.422](m), MNI=[0.013], SCI=[0](min),
1499 Continuous simulation parameters:
IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1500 *%-----|-----|
1501 SAVE HYD NHYD=["Area-A"], # OF PCYCLES=[1], ICASEsh=[1]

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1502 HYD_COMMENT=["SMWF-A Inflow"]
1503 *%-----|-----|
1504 ROUTE RESERVOIR NHYDout=["SMWF-A"], NHYDin=["Area-A"], RDT=[1](min),
1505 TABLE of ( OUTFLOW-STORAGE ) values
1506 (cms) - (ha-m)
1507 [ 0.000 , 0.000 ]
1508 [ 0.103 , 1.077 ]
1509 [ 0.128 , 1.749 ]
1510 [ 0.382 , 2.282 ]
1511 [ 0.703 , 2.582 ]
1512 [ 1.256 , 2.978 ]
1513 [ 1.567 , 3.202 ]
1514 [ 1.955 , 3.493 ]
1515 [ 2.100 , 3.600 ]
1516 [ -1 , -1 ] (max twenty pts)
1517 NHYDovf=["SWMAOV"]
1518 *%-----|-----|
1519 SAVE HYD NHYD=["SMWF-A"], # OF PCYCLES=[1], ICASEsh=[1]
1520 HYD_COMMENT=["SMWF-A Outflow"]
1521 *%-----|-----|
1522 *ANALYSIS POINT 4 - TOTAL FLOW AT OUTLET OF STREET 5
1523 *%-----|-----|
1524 ADD HYD NHYDsum=["PT4ST5"], NHYDs to add=["SSAOUT"+"SMWF-A"+"SWMAOV"]
1525 *%-----|-----|
1526 CONTINUOUS STANDHYD NHYD=["C6"], DT=[1]min, AREA=[1.87](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1527 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1528 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1529 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[193.391](m), MNI=[0.013], SCI=[0](min),
1530 Continuous simulation parameters:
1531 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1532 *%-----|-----|
1533 ROUTE RESERVOIR NHYDout=["C6-STR"], NHYDin=["C6"], RDT=[1](min),
1534 TABLE of ( OUTFLOW-STORAGE ) values
1535 (cms) - (ha-m)
1536 [ 0.000 , 0.000 ]
1537 [ 0.026 , 0.029 ]
1538 [ 0.054 , 0.038 ]
1539 [ 0.075 , 0.044 ]
1540 [ 0.093 , 0.052 ]
1541 [ 0.120 , 0.056 ]
1542 [ 0.138 , 0.061 ]
1543 [ -1 , -1 ] (max twenty pts)
1544 NHYDovf=["C6-OVF"]
1545 *%-----|-----|
1546 CONTINUOUS STANDHYD NHYD=["C7"], DT=[1]min, AREA=[1.62](ha), XIMP=[0.68], TIMP=[0.85],
DWF=[0](cms), LOSS=[1]:
1547 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1548 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1549 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
1550 Continuous simulation parameters:
1551 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1552 *%-----|-----|
1553 ROUTE RESERVOIR NHYDout=["C7-STR"], NHYDin=["C7"], RDT=[1](min),
1554 TABLE of ( OUTFLOW-STORAGE ) values
1555 (cms) - (ha-m)
1556 [ 0.000 , 0.000 ]
1557 [ 0.023 , 0.025 ]

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1558         [ 0.047 , 0.033 ]
1559         [ 0.065 , 0.038 ]
1560         [ 0.081 , 0.045 ]
1561         [ 0.104 , 0.048 ]
1562         [ 0.120 , 0.053 ]
1563         [ -1 , -1 ] (max twenty pts)
1564         NHYDovf=["C7-OVF"]
1565 *%-----|-----|
1566 CONTINUOUS STANDHYD NHYD=["ST-6"], DT=[1]min, AREA=[0.41](ha),XIMP=[0.46], TIMP=[0.57],
DWF=[0](cms), LOSS=[1]:
1567         Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1568         Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1569         Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[90.554](m),
MNI=[0.013], SCI=[0](min),
1570         Continuous simulation parameters:
1571         IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1572 *%-----|-----|
1573 ROUTE RESERVOIR NHYDout=["ST6STR"], NHYDin=["ST-6"], RDT=[1](min),
1574         TABLE of ( OUTFLOW-STORAGE ) values
1575         (cms) - (ha-m)
1576         [ 0.000 , 0.0000 ]
1577         [ 0.036 , 0.0010 ]
1578         [ 0.037 , 0.0058 ]
1579         [ -1 , -1 ] (max twenty pts)
1580         NHYDovf=["ST6OVF"]
1581 *%-----|-----|
1582 *ANALYSIS POINT 5 - TOTAL FLOW AT OUTLET OF STREET 6
1583 *%-----|-----|
1584 ADD HYD NHYDsum=["PT5ST6"], NHYDs to
add=["PT4ST5"+"C6-STR"+"C6-OVF"+"C7-STR"+"C7-OVF"+"ST6STR"+"ST6OVF"]
1585 *%-----|-----|
1586 *ROUTE FLOW through O'Keefe Drain 4
1587 ROUTE CHANNEL NHYDout=["DRAIN4"], NHYDin=["PT5ST6"], RDT=[1](min),
1588         CHLGTH=[324]{m}, CHSLOPE=[.10](%), FPSLOPE=[.10](%),
1589         SECNUM=[1], NSEG=[3]
1590         ( SEGROUGH, SEGDIST (m))=[0.07,12.00 -0.043,18.00 0.07,30.00] NSEG
times
1591         ( DISTANCE (m), ELEVATION (m))=[0.00, 2.00]
1592         (2.00, 1.20)
1593         (12.00, 1.00)
1594         (14.00, 0.00)
1595         (16.00, 0.00)
1596         (18.00, 1.00)
1597         (28.00, 1.20)
1598         (30.00, 2.00)
1599 *%-----|-----|
1600 CONTINUOUS NASHYD NHYD=["D4"], DT=[1]min, AREA=[1.73](ha), DWF=[0](cms), CN/C=[88],
IA=[8.4](mm),
1601         N=[3], TP=[0.60]hrs,
1602         Continuous simulation parameters:
1603         IaRECper=[4](hrs),
1604         SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1605         InterEventTime=[12](hrs)
1606         Baseflow simulation parameters:
1607         BaseFlowOption=[1] ,
1608         InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1609         VHydCond=[0.055](mm/hr), END=-1
1610 *%-----|-----|
1611 CONTINUOUS STANDHYD NHYD=["Area-B"], DT=[1]min, AREA=[24.04](ha), XIMP=[0.62],
TIMP=[0.77], DWF=[0](cms), LOSS=[1]:
1612         Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1613         Pervious areas: IAper=[4.67](mm), SLPP=[1.4](%), LGP=[50](m),

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MNP=[0.250], SCP=[0](min),
1614 Impervious areas: IAimp=[1.57](mm), SLPI=[1.4](%),
LGI=[693.397](m), MNI=[0.013], SCI=[0](min),
1615 Continuous simulation parameters:
1616 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1

1617 *%-----|-----|
1618 ROUTE RESERVOIR NHYDout=["SWMF-B"], NHYDin=["Area-B"], RDT=[1](min),
1619 TABLE of ( OUTFLOW-STORAGE ) values
1620 (cms) - (ha-m)
1621 [ 0.000 , 0.000 ]
1622 [ 0.025 , 0.090 ]
1623 [ 0.175 , 0.510 ]
1624 [ 0.350 , 0.710 ]
1625 [ 0.495 , 0.820 ]
1626 [ 0.648 , 0.980 ]
1627 [ 0.965 , 1.045 ]
1628 [ 1.072 , 1.140 ]
1629 [ -1 , -1 ] (max twenty pts)
1630 NHYDovf=["SWMBOVF"]

1631 *%-----|-----|
1632 ADD HYD NHYDsum=["D4-EX"], NHYDs to add=["DRAIN4"+"D4"+"SWMF-B"+"SWMBOVF"]
1633 *%-----|-----|
1634 *ROUTE FLOW THROUGH O'Keefe Drain 5
1635 * JFSA: Nov. 2020, added en points to close X-Section
1636 ROUTE CHANNEL NHYDout=["DRAIN5"], NHYDin=["D4-EX"], RDT=[1](min),
1637 CHLGTH=[413.0](m), CHSLOPE=[0.16](%), FPSLOPE=[0.16](%),
1638 SECNUM=[1], NSEG=[3]
1639 ( SEGROUGH, SEGDIST (m))=[0.043,12.29 -0.033,17.97
1640 0.043,32.84] NSEG times
1641 ( DISTANCE (m), ELEVATION (m))=(-0.01, 2.50)
1642 [0.00, 1.41]
1643 [6.13, 0.97]
1644 [12.29, 0.89]
1645 [15.71, 0.00]
1646 [17.97, 0.39]
1647 [23.04, 0.35]
1648 [32.83, 0.96]
1649 (32.84, 2.50)

1650 *%-----|-----|
1651 CONTINUOUS NASHYD NHYD=["D5"], DT=[1]min, AREA=[1.90](ha),
1652 DWF=[0](cms), CN/C=[86], IA=[8.7](mm), N=[3], TP=[0.69]hrs,
1653 Continuous simulation parameters:
1654 IaREcper=[4](hrs),
1655 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1656 InterEventTime=[12](hrs)
1657 Baseflow simulation parameters:
1658 BaseFlowOption=[1] ,
1659 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1660 VHydCond=[0.055](mm/hr), END=-1

1661 *%-----|-----|
1662 *EXTERNAL FLOWS SOUTHEAST OF THE SITE NORTH OF McKENNA CASEY DR.
1663 CONTINUOUS NASHYD NHYD=["O-13SDF"], DT=[1]min, AREA=[9.74](ha),
1664 DWF=[0](cms), CN/C=[81], IA=[4.0](mm), N=[3], TP=[.43]hrs,
1665 Continuous simulation parameters:
1666 IaREcper=[4](hrs),
1667 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1668 InterEventTime=[12](hrs)
1669 Baseflow simulation parameters:
1670 BaseFlowOption=[1] ,
1671 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1672 VHydCond=[0.055](mm/hr), END=-1

1673 *%-----|-----|
1674 *SNOW DISPOSAL FACILITY
1675 *PARAMETERS BASED ON ROBINSON 2006 MODEL
1676 ROUTE RESERVOIR NHYDout=["SDF"], NHYDin=["O-13SDF"], RDT=[1](min),

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1677             TABLE of ( OUTFLOW-STORAGE ) values
1678                 (cms) - (ha-m)
1679                 [0.000,0.000]
1680                 [0.150,0.600]
1681                 (0.200,1.500)
1682                 [ -1 , -1 ] (max twenty pts)
1683                 NHYDovf=["OVFSDF"]
1684 *%-----|-----|
1685 *ANALYSIS POINT 6 - McKenna Casey Dr.
1686 *%-----|-----|
1687 ADD HYD      NHYDsum=["PT6MC"], NHYDs to add=["DRAIN5"+"D5"+"SDF"]
1688 *%-----|-----|
1689 CONTINUOUS NASHYD  NHYD=["O-15"], DT=[1]min, AREA=[10.67](ha),
1690                   DWF=[0](cms), CN/C=[82], IA=[7.5](mm), N=[3], TP=[0.30]hrs,
1691                   Continuous simulation parameters:
1692                   IaREcper=[4](hrs),
1693                   SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1694                   InterEventTime=[12](hrs)
1695                   Baseflow simulation parameters:
1696                   BaseFlowOption=[1] ,
1697                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1698                   VHydCond=[0.055](mm/hr),  END=-1
1699 *%-----|-----|
1700 *TOTAL FLOW NORTH OF McKENNA CASEY DR.
1701 ADD HYD      NHYDsum=["M-C"], NHYDs to add=["PT6MC"+"O-15"]
1702 *%-----|-----|
1703 *ROUTE FLOW THROUGH AREA O-14
1704 * JFSA: Nov. 2020, added end points to close X-section
1705 ROUTE CHANNEL  NHYDout=["O-14Ch"], NHYDin=["M-C"], RDT=[1](min),
1706                CHLGTH=[845.3](m), CHSLOPE=[0.10](%), FPSLOPE=[0.10](%),
1707                SECNUM=[1], NSEG=[3]
1708                ( SEGROUGH, SEGDIST (m))=[0.06,15.00 -0.033,18.04 0.06,31.85] NSEG
1709                times
1710                ( DISTANCE (m), ELEVATION (m))=[-0.01, 2.5
1711                (0.00, 1.53]
1712                (5.56, 1.47)
1713                (9.21, 1.45)
1714                (12.45, 1.53)
1715                (13.70, 1.50)
1716                (15.00, 0.69)
1717                (15.34, 0.00)
1718                (16.51, 0.05)
1719                (17.30, 0.17)
1720                (18.04, 0.74)
1721                (19.29, 1.32)
1722                (22.73, 1.47)
1723                (31.84, 1.41)
1724                (31.85, 2.50)
1725 *%-----|-----|
1726 *% -Change O-14 from NASHYD to STANDHYD, name it "S-1-Okeefe" and add it to S-1
1727 *% -JFSA 2021-02-16, add detailed subcatchment drainage area for each subcatchment
1728 *% in Corrigan sub-catchment. After adding part of O-14 to S_1 sub-catchment so O-14
1729 *% becomes 5 ha instead of 30.02 ha and TP becomes 0.133 (5*0.8/30.02) instead of 0.8
1730 CONTINUOUS NASHYD  NHYD=["O-14"], DT=[1]min, AREA=[5](ha),
1731                   DWF=[0](cms), CN/C=[82], IA=[7.5](mm), N=[3], TP=[0.133]hrs,
1732                   Continuous simulation parameters:
1733                   IaREcper=[4](hrs),
1734                   SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
1735                   InterEventTime=[12](hrs)
1736                   Baseflow simulation parameters:
1737                   BaseFlowOption=[1] ,
1738                   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1739                   VHydCond=[0.055](mm/hr),  END=-1
1740 *
1741 *%-----|-----|

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1739 *ANALYSIS POINT 7 - JOCK RIVER
1740 * 2020-12-01 To Foster Drain
1741 * 2020-12-01 replace ("PT7JR") by ("OKEEFE")
1742 *%-----|-----
1743 ADD HYD          NHYDsum=["OKEEFE"], NHYDs to add=["O-14Ch"+"O-14"]
1744 *%-----|-----
1745 *CONTINUOUS STANDHYD NHYD=["OKEEFE"], DT=[1](min), AREA=[448](ha),
1746 *                XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
1747 *                SCS curve number CN=[77],
1748 *                Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1749 *                LGP=[40](m), MNP=[0.25], SCP=[0](min),
1750 *                Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1751 *                LGI=[1728](m), MNI=[0.013], SCI=[0](min),
1752 *                Continuous simulation parameters:
1753 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
1754 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1755 *                InterEventTime=[18](hrs), END=-1
1756 *#*****
1757 *#      Okeefe Pond
1758 *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1759 *#      and a ratio of the catchment area to the West Clarke pond rating curve
1760 *#      from the MSS for the next coordinates
1761 *#*****
1762 *ROUTE RESERVOIR  NHYDout=["P_OKE"], NHYDin=["OKEEFE"],
1763 *                RDT=[1](min),
1764 *                TABLE of ( OUTFLOW-STORAGE ) values
1765 *                (cms) - (ha-m)
1766 *                [  0.0 ,  0.0]
1767 *                [ 14.13 , 13.0]
1768 *                [  -1 ,  -1 ] (maximum one hundred pairs of points)
1769 *                NHYDovf=["ok-OVF"],
1770 *%-----|-----
1771 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
1772 * moved to drain before station 6215 on Jock River
1773 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XIMP=[0.65],
1774 *                TIMP=[0.65], DWF=[0](cms),
1775 *                LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1776 *                IAper=[4.67](mm), SLPP=[2.0](%),
1777 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1778 *                IAimp=[1.57](mm), SLPI=[0.75](%),
1779 *                LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
1780 *                Continuous simulation parameters:
1781 *                IaRECper=[4](hrs), IaRECimp=[4](hrs),
1782 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1783 *                InterEventTime=[12](hrs), END=-1
1784 *%-----|-----
1785 *CONTINUOUS NASHYD  NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
1786 *                DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1787 *                N=[3], TP=[1.120]hrs,
1788 *                Continuous simulation parameters:
1789 *                IaRECper=[4](hrs),
1790 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1791 *                InterEventTime=[12](hrs)
1792 *                Baseflow simulation parameters:
1793 *                BaseFlowOption=[1] ,
1794 *                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1795 *                VHydCond=[0.055](mm/hr), END=-1
1796 *%-----|-----
1797 *COMPUTE DUALHYD  NHYDin=["S-1-D2"], CINLET=[2.097](cms), NINLET=[1],
1798 *                MajNHYD=["S-1-D2J"]
1799 *                MinNHYD=["S-1-D2N"]
1800 *                TMJSTO=[9999999](cu-m)
1801 *%-----|-----
1802 *ADD HYD          NHYDsum=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
1803 *%-----|-----

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1800 ROUTE RESERVOIR      NHYDout=["S-1-D2R"] ,NHYDin=["S-1-D2S"] ,
1801                      RDT=[1](min),
1802                      TABLE of ( OUTFLOW-STORAGE ) values
1803                          (cms) - (ha-m)
1804                          [ 0.0      , 0.0 ]
1805                          [ 0.2231, 0.7445 ]
1806                          [   -1   ,  -1   ] (max twenty pts)
1807                      NHYDovf=["S-1-D2Rovf"]
1808 *%-----|-----
1809 CONTINUOUS STANDHYD  NHYD=["S-1-D3"], DT=[1](min), AREA=[6.79](ha), XIMP=[0.65],
1810                      TIMP=[0.65], DWF=[0](cms),
1811                      LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1812                      IAper=[4.67](mm), SLPP=[2.0](%),
1813                      LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1814                      IAimp=[1.57](mm), SLPI=[0.75](%),
1815                      LGI=[212.760](m), MNI=[0.013], SCI=[0](min),
1816                      Continuous simulation parameters:
1817                      IaRECper=[4](hrs), IaRECimp=[4](hrs),
1818                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1819                      InterEventTime=[12](hrs), END=-1
1820 *%-----|-----
1821 *CONTINUOUS NASHYD   NHYD=["S-1-D3"], DT=[1]min, AREA=[6.79](ha),
1822 *                      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
1823 *                      N=[3], TP=[1.281]hrs,
1824 *                      Continuous simulation parameters:
1825 *                      IaRECper=[4](hrs),
1826 *                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1827 *                      InterEventTime=[12](hrs)
1828 *                      Baseflow simulation parameters:
1829 *                      BaseFlowOption=[1] ,
1830 *                      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1831 *                      VHydCond=[0.055](mm/hr), END=-1
1832 *%-----|-----
1833 COMPUTE DUALHYD      NHYDin=["S-1-D3"], CINLET=[0.831](cms), NINLET=[1],
1834                      MajNHYD=["S-1-D3J"]
1835                      MinNHYD=["S-1-D3N"]
1836                      TMJSTO=[9999999](cu-m)
1837 *%-----|-----
1838 ADD HYD              NHYDsum=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
1839 *%-----|-----
1840 ROUTE RESERVOIR      NHYDout=["S-1-D3R"] ,NHYDin=["S-1-D3S"] ,
1841                      RDT=[1](min),
1842                      TABLE of ( OUTFLOW-STORAGE ) values
1843                          (cms) - (ha-m)
1844                          [ 0.0      , 0.0 ]
1845                          [ 0.0811, 0.2708 ]
1846                          [   -1   ,  -1   ] (max twenty pts)
1847                      NHYDovf=["S-1-D3Rovf"]
1848 *%-----|-----
1849 ADD HYD              NHYDsum=["SN_OK"], NHYDs to
1850                      add=["N_OK"+"OKEEFE"+"S-1-D2R"+"S-1-D3R"+"S-1-D2Rovf"+"S-1-D3Rovf"]
1851 *%-----|-----
1852 SAVE HYD            NHYD=["SN_OK"], # OF PCYCLES=[-1], ICASEsh=[1]
1853                      HYD_COMMENT=["Total Flows at Okeefe Drain"]
1854 *%-----|-----
1855 ROUTE CHANNEL        NHYDout=["N_FO"] ,NHYDin=["SN_OK"] ,
1856                      RDT=[1](min),
1857                      CHLGTH=[1183](m), CHSLOPE=[0.0761](%),
1858                      FPSLOPE=[0.0761](%),
1859                      SECNUM=[1.0], NSEG=[3]
1860                      ( SEGROUGH, SEGDIST (m))=
1861                      [0.050,-33.89

```

```

1862         -0.035,31.59
1863         0.050,34.41] NSEG times
1864         ( DISTANCE (m), ELEVATION (m))=
1865         [-794.18, 91.00]
1866         [-775.41, 91.50]
1867         [-702.63, 91.50]
1868         [-546.19, 91.50]
1869         [-529.54, 91.50]
1870         [-323.44, 91.00]
1871         [-320.71, 91.00]
1872         [-183.59, 91.00]
1873         [-182.54, 90.50]
1874         [-181.36, 90.00]
1875         [-177.37, 90.00]
1876         [-87.70, 90.00]
1877         [-33.89, 90.00]
1878         [-18.52, 86.88]
1879         [0.00,85.20]
1880         [16.20, 86.83]
1881         [31.59, 90.00]
1882         [33.03, 90.50]
1883         [34.41, 91.00]
1884 *%-----|-----|
1885 *#*****
1886 *#   Catchment FOSTER
1887 *#   - To Foster ditch (north of the Jock)
1888 *#   - Partially developed (medium density); remaining agricultural
1889 *#   - 2020-12-01 JFSA Foster area is 332 as per Foster SWMF Environmental Study
1890 *#   - 2020-12-01 decrease Foster drainage area from (373 HA) to (307.98 HA) after
1891 *#   - 2021-02-12 update Foster area to 325.44 ha as measured from QGIS
1892 *#*****
1893 CONTINUOUS STANDHYD NHYD=["FOSTER"], DT=[1]min, AREA=[325.44](ha),
1894 XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
1895 SCS curve number CN=[74],
1896 Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1897 LGP=[40](m), MNP=[0.25], SCP=[0](min),
1898 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1899 LGI=[1472.956](m), MNI=[0.013], SCI=[0](min),
1900 Continuous simulation parameters:
1901 IaRECper=[4](hrs), IaRECimp=[4](hrs),
1902 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1903 InterEventTime=[18](hrs), END=-1
1904 *#*****
1905 *#   Foster Pond
1906 *#   - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1907 *#   and a ratio of the catchment area to the West Clarke pond rating curve
1908 *#   from the MSS for the next coordinates
1909 *#*****
1910 ROUTE RESERVOIR NHYDout=["P_FOS"], NHYDin=["FOSTER"],
1911 RDT=[1](min),
1912 TABLE of ( OUTFLOW-STORAGE ) values
1913 (cms) - (ha-m)
1914 [ 0.0 , 0.0 ]
1915 [ 10.34 , 10]
1916 [ -1 , -1 ] (max twenty pts)
1917 NHYDovf=["FO-OVF"]
1918 *%-----|-----|
1919 ADD HYD NHYDsum=["FOSTER-OUT"], NHYDs to add=["P_FOS"+"FO-OVF"]
1920 *%-----|-----|
1921 *#*****
1922 * -Brazeau area from P 1800-19 =[71.751], change to 63.59 ha based on GIS measurements
1923 * -JFSA, 2021-01-19 update "W_CLAR_BRAZ" to 73.29 ha based on GIS measurements
1924 * -JFSA, 2021-01-22 Brazeau ("MS_P10"+"P10-OVF")brazeau pond discharges directly

```

to the jock river through a road side ditch on the west side of Borrisokane road (station 6016)

```
1925 CONTINUOUS STANDHYD NHYD=["W_CLAR_BRAZ"], DT=[1]min, AREA=[73.29](ha),
1926 XIMP=[0.6], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
1927 SCS curve number CN=[77],
1928 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
1929 LGP=[40](m), MNP=[0.25], SCP=[0](min),
1930 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1931 LGI=[699.00](m), MNI=[0.013], SCI=[0](min),
1932 Continuous simulation parameters:
1933 IaREcper=[4](hrs), IaREcimp=[4](hrs),
1934 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1935 InterEventTime=[18](hrs), END=-1
```

```
*%-----|-----
* 2020-12-01 correct pond curve values
```

```
1938 ROUTE RESERVOIR NHYDout=["MS_P10"], NHYDin=["W_CLAR_BRAZ"],
1939 RDT=[1](min),
```

```
1940 TABLE of ( OUTFLOW-STORAGE ) values
1941 (cms) - (ha-m)
1942 [ 0.0 , 0.0 ]
1943 [ 0.068 , 0.001 ]
1944 [ 0.271 , 0.022 ]
1945 [ 0.379 , 0.051 ]
1946 [ 0.48 , 0.091 ]
1947 [ 0.853 , 0.341 ]
1948 [ 1.005 , 0.61 ]
1949 [ 1.128 , 1.231 ]
1950 [ 1.155 , 1.592 ]
1951 [ 1.194 , 1.876 ]
1952 [ 1.2 , 1.921 ]
1953 [ 1.259 , 2.369 ]
1954 [ 1.3 , 2.665 ]
1955 [ 1.349 , 2.813 ]
1956 [ -1 , -1 ] (max twenty pts)
1957 NHYDovf=["P10-OVF"]
```

```
*%-----|-----
* -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
```

```
1960 CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
1961 XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
1962 SCS curve number CN=[74],
1963 Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
1964 LGP=[40](m), MNP=[0.25], SCP=[0](min),
1965 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
1966 LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
1967 Continuous simulation parameters:
1968 IaREcper=[4](hrs), IaREcimp=[4](hrs),
1969 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1970 InterEventTime=[18](hrs), END=-1
```

```
*%-----|-----
*CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
* DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
* N=[3], TP=[1.10]hrs,
* Continuous simulation parameters:
* IaREcper=[4](hrs),
* SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
* InterEventTime=[12](hrs)
* Baseflow simulation parameters:
* BaseFlowOption=[1] ,
* InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
* VHydCond=[0.055](mm/hr), END=-1
```

```
1984 COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CINLET=[0.508](cms), NINLET=[1],
1985 MajNHYD=["S-1-FO-D2J"]
1986 MinNHYD=["S-1-FO-D2N"]
1987 TMJSTO=[9999999](cu-m)
```

```

1988  *%-----|-----|
1989  ADD HYD      NHYDsum=["S-1-FO-D2S"], NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]
1990  *%-----|-----|
1991  ROUTE RESERVOIR  NHYDout=["S-1-FO-D2R"] ,NHYDin=["S-1-FO-D2S"] ,
1992  RDT=[1](min),
1993  TABLE of ( OUTFLOW-STORAGE ) values
1994  (cms) - (ha-m)
1995  [ 0.0      , 0.0 ]
1996  [ 0.0590, 0.1970 ]
1997  [      -1 , -1      ] (max twenty pts)
1998  NHYDovf=["S-1FOD2ovf"]
1999  *%-----|-----|
2000  ADD HYD      NHYDsum=["980"], NHYDs to
2001  add=["FOSTER-OUT"+"S-1-FO-D2R"+"S-1FOD2ovf"]
2002  *%-----|-----|
2003  SAVE HYD     NHYD=["980"], # OF PCYCLES=[-1], ICASEsh=[1]
2004  HYD_COMMENT=["Total Flows at Station 980 on Foster Drain"]
2005  *%-----|-----|
2006  *#
2007  *# Hydrograph from Node Foster SWM (Station 980)to Node at station 520
2008  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 980
2009  *#
2010  ROUTE CHANNEL  NHYDout=["980-out"] ,NHYDin=["980"] ,
2011  RDT=[1](min),
2012  CHLGTH=[460](m),  CHSLOPE=[0.04348](%),
2013  FPSLOPE=[0.04348](%),
2014  SECNUM=[1.0],      NSEG=[3]
2015  ( SEGROUGH, SEGDIST (m))=
2016  [0.050,45.90
2017  -0.035,53.30
2018  0.050,100] NSEG times
2019  ( DISTANCE (m), ELEVATION (m))=
2020  [0, 91.75 ]
2021  [42.4, 92.18 ]
2022  [43.5, 92.16 ]
2023  [44.1, 92.1 ]
2024  [44.6, 92 ]
2025  [44.8, 91.86 ]
2026  [45.9, 91.04 ]
2027  [46.4, 90.65 ]
2028  [46.8, 90.36 ]
2029  [47.9, 90.32 ]
2030  [48.7, 90.35 ]
2031  [50.7, 90.33 ]
2032  [52.2, 90.38 ]
2033  [52.5, 90.59 ]
2034  [53.3, 91.28 ]
2035  [54, 91.83 ]
2036  [54.3, 92 ]
2037  [54.8, 92.08 ]
2038  [55.4, 92.12 ]
2039  [100, 91.84 ]
2040  *%-----|-----|
2041  * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
2042  before station 520 on Foster Drain
2043  CONTINUOUS STANDHYD  NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2044  XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2045  SCS curve number CN=[74],
2046  Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2047  LGP=[40](m), MNP=[0.25], SCP=[0](min),
2048  Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2049  LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2050  Continuous simulation parameters:
2051  IaRECper=[4](hrs), IaRECimp=[4](hrs),
2052  SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2053  InterEventTime=[18](hrs), END=-1

```

```

2052 *%-----|-----|
2053 COMPUTE DUALHYD NHYDin=["S-1-FO-D1"], CINLET=[0.605](cms), NINLET=[1],
2054 MajNHYD=["S-1-FO-D1J"]
2055 MinNHYD=["S-1-FO-D1N"]
2056 TMJSTO=[9999999](cu-m)
2057 *%-----|-----|
2058 ADD HYD NHYDsum=["S-1-FO-D1S"], NHYDs to add=["S-1-FO-D1N"+"S-1-FO-D1J"]
2059 *%-----|-----|
2060 ROUTE RESERVOIR NHYDout=["S-1-FO-D1R"], NHYDin=["S-1-FO-D1S"],
2061 RDT=[1](min),
2062 TABLE of ( OUTFLOW-STORAGE ) values
2063 (cms) - (ha-m)
2064 [ 0.0 , 0.0 ]
2065 [ 0.0611, 0.2038 ]
2066 [ -1 , -1 ] (max twenty pts)
2067 NHYDovf=["S-1FODlovf"]
2068 *%-----|-----|
2069 ADD HYD NHYDsum=["520"], NHYDs to add=["980-out"+"S-1-FO-D1R"+"S-1FODlovf"]
2070 *%-----|-----|
2071 SAVE HYD NHYD=["520"], # OF PCYCLES=[-1], ICASEsh=[1]
2072 HYD_COMMENT=["Total Flows at Sation 520 on Foster Drain"]
2073 *%-----|-----|
2074 *# Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock
River)
2075 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 520
2076 *#
2077 ROUTE CHANNEL NHYDout=["520-out"], NHYDin=["520"],
2078 RDT=[1](min),
2079 CHLGTH=[860](m), CHSLOPE=[0.5872](%),
2080 FPSLOPE=[0.5872](%),
2081 SECNUM=[1.0], NSEG=[3]
2082 ( SEGROUGH, SEGDIST (m))=
2083 [0.050,45.90
2084 -0.035,54.3
2085 0.050,100.1097] NSEG times
2086 ( DISTANCE (m), ELEVATION (m))=
2087 [0, 91.26 ]
2088 [44.9, 91.46 ]
2089 [45.1, 91.37 ]
2090 [45.9, 90.84 ]
2091 [47, 90.32 ]
2092 [47.5, 90.22 ]
2093 [48, 90.17 ]
2094 [50.7, 90.19 ]
2095 [51.5, 90.17 ]
2096 [52.2, 90.13 ]
2097 [52.7, 90.12 ]
2098 [53.3, 90.14 ]
2099 [53.5, 90.31 ]
2100 [53.9, 90.59 ]
2101 [54.3, 90.87 ]
2102 [54.7, 91.04 ]
2103 [55.3, 91.24 ]
2104 [55.5, 91.26 ]
2105 [63.7, 91.37 ]
2106 [100.1097, 91.43 ]
2107 *%-----|-----|
2108 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2109 CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2110 XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2111 SCS curve number CN=[74],
2112 Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2113 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2114 Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2115 LGI=[315.806](m), MNI=[0.013], SCI=[0](min),

```

```

2116 Continuous simulation parameters:
2117 IaREcper=[4](hrs), IaREcimp=[4](hrs),
2118 SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2119 InterEventTime=[18](hrs), END=-1
2120 *%-----|-----|
2121 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2122 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2123 * N=[3], TP=[1.007]hrs,
2124 * Continuous simulation parameters:
2125 * IaREcper=[4](hrs),
2126 * SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2127 * InterEventTime=[12](hrs)
2128 * Baseflow simulation parameters:
2129 * BaseFlowOption=[1] ,
2130 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2131 * VHydCond=[0.055](mm/hr), END=-1
2132 *%-----|-----|
2133 COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CINLET=[1.615](cms), NINLET=[1],
2134 MajNHYD=["S-1FO-F-DJ"]
2135 MinNHYD=["S-1FO-F-DN"]
2136 TMJSTO=[9999999](cu-m)
2137 *%-----|-----|
2138 ADD HYD NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2139 *%-----|-----|
2140 ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"],NHYDin=["S-1FO-F-DS"] ,
2141 RDT=[1](min),
2142 TABLE of ( OUTFLOW-STORAGE ) values
2143 (cms) - (ha-m)
2144 [ 0.0 , 0.0 ]
2145 [ 0.1788, 0.5966 ]
2146 [ -1 , -1 ] (max twenty pts)
2147 NHYDovf=["S-1FoFDovf"]
2148 *%-----|-----|
2149 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2150 CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha), XIMP=[0.325],
TIMP=[0.65], DWF=[0](cms), LOSS=[1]:
2151 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
2152 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
2153 Impervious areas: IAimp=[0.785](mm), SLPI=[0.75](%),
LGI=[187.439](m), MNI=[0.013], SCI=[0](min),
2154 Continuous simulation parameters:
2155 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
2156 *%-----|-----|
2157 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha),
2158 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2159 * N=[3], TP=[1.10]hrs,
2160 * Continuous simulation parameters:
2161 * IaREcper=[4](hrs),
2162 * SMIN=[-1](mm), SMAx=[-1](mm), SK=[0.010]/(mm),
2163 * InterEventTime=[12](hrs)
2164 * Baseflow simulation parameters:
2165 * BaseFlowOption=[1] ,
2166 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2167 * VHydCond=[0.055](mm/hr), END=-1
2168 *%-----|-----|
2169 COMPUTE DUALHYD NHYDin=["S-1-D8"], CINLET=[0.672](cms), NINLET=[1],
2170 MajNHYD=["S-1-D8J"]
2171 MinNHYD=["S-1-D8N"]
2172 TMJSTO=[9999999](cu-m)
2173 *%-----|-----|
2174 ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2175 *%-----|-----|

```

```

2176 *ADD HYD          NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Post"]
2177 *%-----|-----|
2178 *COMPUTE DUALHYD  NHYDin=["S-1-D"], CINLET=[11.616](cms), NINLET=[1],
2179 *                MajNHYD=["S-1-D-MJ"]
2180 *                MinNHYD=["S-1-D-MN"]
2181 *                TMJSTO=[5974](cu-m)
2182 *%-----|-----|
2183 *ADD HYD          NHYDsum=["S-1-DEV"], NHYDs to add=["S-1-D-MJ"+"S-1-D-MN"]
2184 *%-----|-----|
2185 ROUTE RESERVOIR NHYDout=["S-1-D8R"] ,NHYDin=["S-1-D8S"] ,
2186 RDT=[1](min),
2187             TABLE of ( OUTFLOW-STORAGE ) values
2188             (cms) - (ha-m)
2189             [ 0.0      , 0.0 ]
2190             [ 0.0630, 0.2102 ]
2191             [      -1 , -1      ] (max twenty pts)
2192             NHYDovf=["S-1-D8Rovf"]
2193 *%-----|-----|
2194 *   -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2195 CONTINUOUS NASHYD NHYD=["S-1-A"], DT=[1]min, AREA=[75.88](ha),
2196 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2197 N=[3], TP=[0.619]hrs,
2198 Continuous simulation parameters:
2199 IaREcper=[4](hrs),
2200 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2201 InterEventTime=[12](hrs)
2202 Baseflow simulation parameters:
2203 BaseFlowOption=[1] ,
2204 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2205 VHydCond=[0.055](mm/hr), END=-1
2206 *%-----|-----|
2207 *   -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2208 CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2209 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2210 N=[3], TP=[1.10]hrs,
2211 Continuous simulation parameters:
2212 IaREcper=[4](hrs),
2213 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2214 InterEventTime=[12](hrs)
2215 Baseflow simulation parameters:
2216 BaseFlowOption=[1] ,
2217 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2218 VHydCond=[0.055](mm/hr), END=-1
2219 *%-----|-----|
2220 ADD HYD          NHYDsum=["SN_FO"], NHYDs to
add=["N_FO"+"520-out"+"MS_P10"+"P10-OVF"+"W_CLAR_UNDE"+"S-1FoFDovf"+"S-1FO-F-DR"+"S-1-D8R
ovf"+"S-1-D8R"+"S-1-A"]
2221 *%-----|-----|
2222 SAVE HYD        NHYD=["SN_FO"], # OF PCYCLES=[-1], ICASEsh=[1]
2223 HYD_COMMENT=["Total Flows at Foster Drain"]
2224 *%-----|-----|
2225 *# Hydrograph from Node Foster routed to Node at Cedarview Road
2226 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
2227 *#
2228 ROUTE CHANNEL  NHYDout=["N_CE"] ,NHYDin=["SN_FO"] ,
2229 RDT=[1](min),
2230 CHLGTH=[159](m), CHSLOPE=[0.0818](%),
2231             FPSLOPE=[0.0818](%),
2232 SECNUM=[1.0], NSEG=[3]
2233 ( SEGROUGH, SEGDIST (m))=
2234 [0.050,-15.46
2235 -0.035,26.55
2236 0.050,116.76] NSEG times

```

```

2237      ( DISTANCE (m), ELEVATION (m))=
2238      [-645.23, 91.50]
2239      [-391.20, 91.50]
2240      [-91.00, 91.50]
2241      [-85.52, 91.50]
2242      [-15.46, 89.40]
2243      [-9.79, 89.31]
2244      [-3.22, 86.24]
2245      [3.22, 85.07]
2246      [10.96, 85.79]
2247      [16.44, 86.49]
2248      [26.55, 89.45]
2249      [29.03, 90.27]
2250      [35.76, 90.67]
2251      [36.67, 91.00]
2252      [108.08, 91.00]
2253      [109.82, 90.50]
2254      [112.04, 90.50]
2255      [114.62, 91.00]
2256      [116.76, 91.50]
2257      *%-----|-----|
2258      *#*****|
2259      *#      Catchment S-1
2260      *#      - To Jock River (north and south of Jock)
2261      *#      - Primarily agricultural fields; portion of sand quarry
2262      *%-----|-----|
2263      *%      -2020-12-17 "S-1-Undev" and "S-1-Fost" was a part of Foster drain, they are below
the foster pond. Now they are added to S-1 subcatchment based on Project 1474-BCDC,
JFSA, Nov. 2020
2264      *%      -2020-12-17 Change O-14 (it was part of Okeefe drain) to "S-1-Okeefe" and add it
to S-1 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
2265      *%      -2020-12-17 Add "S-1-BCDC" as NASHYD
2266      *%      -2020-12-17 all other S-1 subcatchment as STANDHYD with DUALHYD and ROUTE RESERVOIR
2267      *%-----|-----|
2268      *#*****|
2269      *      -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2270      *CONTINUOUS NASHYD      NHYD=["S-1-A"], DT=[1]min, AREA=[75.88](ha),
2271      *      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2272      *      N=[3], TP=[0.619]hrs,
2273      *      Continuous simulation parameters:
2274      *      IaRECper=[4](hrs),
2275      *      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2276      *      InterEventTime=[12](hrs)
2277      *      Baseflow simulation parameters:
2278      *      BaseFlowOption=[1] ,
2279      *      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2280      *      VHydCond=[0.055](mm/hr), END=-1
2281      *%-----|-----|
2282      CONTINUOUS NASHYD      NHYD=["S-1-B"], DT=[1]min, AREA=[55.36](ha),
2283      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2284      N=[3], TP=[0.451]hrs,
2285      Continuous simulation parameters:
2286      IaRECper=[4](hrs),
2287      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2288      InterEventTime=[12](hrs)
2289      Baseflow simulation parameters:
2290      BaseFlowOption=[1] ,
2291      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2292      VHydCond=[0.055](mm/hr), END=-1
2293      *%-----|-----|
2294      *#      - JFSA 2021-02-24 change the name from S-1-BCDC to S-1-A and S-1-B. Change their
TP values based on the new areas compared to the old ones.
2295      *CONTINUOUS NASHYD      NHYD=["S-1-BCDC"], DT=[1]min, AREA=[134.9](ha),
2296      *      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2297      *      N=[3], TP=[1.10]hrs,

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2298 * Continuous simulation parameters:
2299 * IaRECper=[4](hrs),
2300 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2301 * InterEventTime=[12](hrs)
2302 * Baseflow simulation parameters:
2303 * BaseFlowOption=[1] ,
2304 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2305 * VHydCond=[0.055](mm/hr), END=-1
2306 *%-----|-----
2307 *# - JFSA 2021-02-24 "S-1-BCDC-1" and "S-1-BCDC-2" are not existing anymore.
    "S-1-BCDC-1" is part of "S-1-FO-D2" and "S-1-BCDC-2" is part of "S-1-D2" and "S-1-D3"
2308 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-1"], DT=[1]min, AREA=[0.3](ha),
2309 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2310 * N=[3], TP=[1.10]hrs,
2311 * Continuous simulation parameters:
2312 * IaRECper=[4](hrs),
2313 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2314 * InterEventTime=[12](hrs)
2315 * Baseflow simulation parameters:
2316 * BaseFlowOption=[1] ,
2317 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2318 * VHydCond=[0.055](mm/hr), END=-1
2319 *%-----|-----
2320 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-2"], DT=[1]min, AREA=[1.3](ha),
2321 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2322 * N=[3], TP=[1.10]hrs,
2323 * Continuous simulation parameters:
2324 * IaRECper=[4](hrs),
2325 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2326 * InterEventTime=[12](hrs)
2327 * Baseflow simulation parameters:
2328 * BaseFlowOption=[1] ,
2329 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2330 * VHydCond=[0.055](mm/hr), END=-1
2331 *%-----|-----
2332 *# - JFSA 2021-01-19, after adding Greenbank pond, "S-1-BCDC-3" is not existing
    anymore
2333 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-3"], DT=[1]min, AREA=[3.9](ha),
2334 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2335 * N=[3], TP=[1.10]hrs,
2336 * Continuous simulation parameters:
2337 * IaRECper=[4](hrs),
2338 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2339 * InterEventTime=[12](hrs)
2340 * Baseflow simulation parameters:
2341 * BaseFlowOption=[1] ,
2342 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2343 * VHydCond=[0.055](mm/hr), END=-1
2344 *%-----|-----
2345 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
    before station 7245 on Jock River
2346 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[1](min), AREA=[44.93](ha), XIMP=[0.65],
    TIMP=[0.65], DWF=[0](cms),
2347 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
    IAper=[4.67](mm), SLPP=[2.0](%),
2348 * LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
    IAimp=[1.57](mm), SLPI=[0.75](%),
2349 * LGI=[547.296](m), MNI=[0.013], SCI=[0](min),
2350 * Continuous simulation parameters:
2351 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2352 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2353 * InterEventTime=[12](hrs), END=-1
2354 *%-----|-----
2355 *COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CINLET=[4.796](cms), NINLET=[1],
2356 * MajNHYD=["S-1-OkMJ"]
2357 * MinNHYD=["S-1-OkMN"]

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2358 * TMJSTO=[9999999](cu-m)
2359 *%-----|-----|
2360 *ADD HYD NHYDsum=["S-1-OkS"], NHYDs to add=["S-1-OkMJ"+"S-1-OkMN"]
2361 *%-----|-----|
2362 *ROUTE RESERVOIR NHYDout=["S-1-OkSR"] ,NHYDin=["S-1-OkS"] ,
2363 * RDT=[1](min),
2364 * TABLE of ( OUTFLOW-STORAGE ) values
2365 * (cms) - (ha-m)
2366 * [ 0.0 , 0.0 ]
2367 * [ 0.5370, 1.7917 ]
2368 * [ -1 , -1 ] (max twenty pts)
2369 * NHYDovf=["S-1-OkSovf"]
2370 *%-----|-----|
2371 *CONTINUOUS NASHYD NHYD=["S-1-Okeefe"], DT=[1]min, AREA=[44.93](ha),
2372 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2373 * N=[3], TP=[1.049]hrs,
2374 * Continuous simulation parameters:
2375 * IaRECper=[4](hrs),
2376 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2377 * InterEventTime=[12](hrs)
2378 * Baseflow simulation parameters:
2379 * BaseFlowOption=[1] ,
2380 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2381 * VHydCond=[0.055](mm/hr), END=-1
2382 *%-----|-----|
2383 * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2384 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2385 * XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2386 * SCS curve number CN=[74],
2387 * Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2388 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
2389 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2390 * LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2391 * Continuous simulation parameters:
2392 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
2393 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2394 * InterEventTime=[18](hrs), END=-1
2395 *%-----|-----|
2396 *COMPUTE DUALHYD NHYDin=["S-1-FO-D1"], CINLET=[0.605](cms), NINLET=[1],
2397 * MajNHYD=["S-1-FO-D1J"]
2398 * MinNHYD=["S-1-FO-D1N"]
2399 * TMJSTO=[9999999](cu-m)
2400 *%-----|-----|
2401 *ADD HYD NHYDsum=["S-1-FO-D1S"], NHYDs to add=["S-1-FO-D1N"+"S-1-FO-D1J"]
2402 *%-----|-----|
2403 *ROUTE RESERVOIR NHYDout=["S-1-FO-D1R"] ,NHYDin=["S-1-FO-D1S"] ,
2404 * RDT=[1](min),
2405 * TABLE of ( OUTFLOW-STORAGE ) values
2406 * (cms) - (ha-m)
2407 * [ 0.0 , 0.0 ]
2408 * [ 0.0611, 0.2038 ]
2409 * [ -1 , -1 ] (max twenty pts)
2410 * NHYDovf=["S-1FOD1ovf"]
2411 *%-----|-----|
2412 *CONTINUOUS NASHYD NHYD=["S-1-FO-D1"], DT=[1]min, AREA=[5.11](ha),
2413 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2414 * N=[3], TP=[1.10]hrs,
2415 * Continuous simulation parameters:
2416 * IaRECper=[4](hrs),
2417 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2418 * InterEventTime=[12](hrs)
2419 * Baseflow simulation parameters:
2420 * BaseFlowOption=[1] ,
2421 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2422 * VHydCond=[0.055](mm/hr), END=-1

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2423 *%-----|-----|
2424 *   -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
2425 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
2426 *   XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
2427 *   SCS curve number CN=[74],
2428 *   Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2429 *   LGP=[40](m), MNP=[0.25], SCP=[0](min),
2430 *   Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2431 *   LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
2432 *   Continuous simulation parameters:
2433 *   IaRECper=[4](hrs), IaRECimp=[4](hrs),
2434 *   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2435 *   InterEventTime=[18](hrs), END=-1
2436 *%-----|-----|
2437 *CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[1]min, AREA=[4.94](ha),
2438 *   DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2439 *   N=[3], TP=[1.10]hrs,
2440 *   Continuous simulation parameters:
2441 *   IaRECper=[4](hrs),
2442 *   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2443 *   InterEventTime=[12](hrs)
2444 *   Baseflow simulation parameters:
2445 *   BaseFlowOption=[1] ,
2446 *   InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2447 *   VHydCond=[0.055](mm/hr), END=-1
2448 *%-----|-----|
2449 *COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CINLET=[0.508](cms), NINLET=[1],
2450 *   MajNHYD=["S-1-FO-D2J"]
2451 *   MinNHYD=["S-1-FO-D2N"]
2452 *   TMJSTO=[9999999](cu-m)
2453 *%-----|-----|
2454 *ADD HYD NHYDsum=["S-1-FO-D2S"], NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]
2455 *%-----|-----|
2456 *ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"], NHYDin=["S-1-FO-D2S"] ,
2457 *   RDT=[1](min),
2458 *   TABLE of ( OUTFLOW-STORAGE ) values
2459 *   (cms) - (ha-m)
2460 *   [ 0.0 , 0.0 ]
2461 *   [ 0.0590, 0.1970 ]
2462 *   [ -1 , -1 ] (max twenty pts)
2463 *   NHYDovf=["S-1FOD2ovf"]
2464 *%-----|-----|
2465 *   -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2466 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2467 *   XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2468 *   SCS curve number CN=[74],
2469 *   Pervious surfaces: IAper=[4.67](mm), SLPP=[0.5](%),
2470 *   LGP=[40](m), MNP=[0.25], SCP=[0](min),
2471 *   Impervious surfaces: IAimp=[1.57](mm), SLPI=[0.5](%),
2472 *   LGI=[315.806](m), MNI=[0.013], SCI=[0](min),
2473 *   Continuous simulation parameters:
2474 *   IaRECper=[4](hrs), IaRECimp=[4](hrs),
2475 *   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2476 *   InterEventTime=[18](hrs), END=-1
2477 *%-----|-----|
2478 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2479 *   DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2480 *   N=[3], TP=[1.007]hrs,
2481 *   Continuous simulation parameters:
2482 *   IaRECper=[4](hrs),
2483 *   SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2484 *   InterEventTime=[12](hrs)
2485 *   Baseflow simulation parameters:
2486 *   BaseFlowOption=[1] ,

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2487 *          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2488 *          VHydCond=[0.055](mm/hr),   END=-1
2489 *%-----|-----|
2490 *COMPUTE DUALHYD  NHYDin=["S-1-FO-F-D"], CINLET=[1.749](cms), NINLET=[1],
2491 *                MajNHYD=["S-1FO-F-DJ"]
2492 *                MinNHYD=["S-1FO-F-DN"]
2493 *                TMJSTO=[9999999](cu-m)
2494 *%-----|-----|
2495 *ADD HYD          NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2496 *%-----|-----|
2497 *ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"],NHYDin=["S-1FO-F-DS"],
2498 *                RDT=[1](min),
2499 *                TABLE of ( OUTFLOW-STORAGE ) values
2500 *                    (cms) - (ha-m)
2501 *                    [ 0.0      , 0.0 ]
2502 *                    [ 0.1788, 0.5966 ]
2503 *                    [   -1   , -1   ] (max twenty pts)
2504 *                NHYDovf=["S-1FoFDovf"]
2505 *%-----|-----|
2506 CONTINUOUS STANDHYD NHYD=["S-1-D1"], DT=[1](min), AREA=[21.67](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2507 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2508 IAper=[4.67](mm), SLPP=[2.0](%),
LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2509 IAimp=[1.57](mm), SLPI=[0.75](%),
2510 LGI=[380.088](m), MNI=[0.013], SCI=[0](min),
2511 Continuous simulation parameters:
2512 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2513 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2514 InterEventTime=[12](hrs),   END=-1
2515 *%-----|-----|
2516 *CONTINUOUS NASHYD NHYD=["S-1-D1"], DT=[1]min, AREA=[21.67](ha),
2517 *                DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2518 *                N=[3], TP=[1.066]hrs,
2519 *                Continuous simulation parameters:
2520 *                IaRECper=[4](hrs),
2521 *                SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2522 *                InterEventTime=[12](hrs)
2523 *                Baseflow simulation parameters:
2524 *                BaseFlowOption=[1],
2525 *                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2526 *                VHydCond=[0.055](mm/hr),   END=-1
2527 *%-----|-----|
2528 COMPUTE DUALHYD  NHYDin=["S-1-D1"], CINLET=[2.409](cms), NINLET=[1],
2529 *                MajNHYD=["S-1-D1J"]
2530 *                MinNHYD=["S-1-D1N"]
2531 *                TMJSTO=[9999999](cu-m)
2532 *%-----|-----|
2533 ADD HYD          NHYDsum=["S-1-D1S"], NHYDs to add=["S-1-D1J"+"S-1-D1N"]
2534 *%-----|-----|
2535 ROUTE RESERVOIR NHYDout=["S-1-D1R"],NHYDin=["S-1-D1S"],
2536 *                RDT=[1](min),
2537 *                TABLE of ( OUTFLOW-STORAGE ) values
2538 *                    (cms) - (ha-m)
2539 *                    [ 0.0      , 0.0 ]
2540 *                    [ 0.2590, 0.8642 ]
2541 *                    [   -1   , -1   ] (max twenty pts)
2542 *                NHYDovf=["S-1-D1Rovf"]
2543 *%-----|-----|
2544 *          -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
moved to drain before station 6215 on Jock River
2545 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2546 *                LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2547 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:

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IAimp=[1.57](mm), SLPI=[0.75](%),
2547 *          LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
2548 *          Continuous simulation parameters:
2549 *          IaRECper=[4](hrs), IaRECimp=[4](hrs),
2550 *          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2551 *          InterEventTime=[12](hrs), END=-1
2552 *%-----|-----
2553 *CONTINUOUS NASHYD NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
2554 *          DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2555 *          N=[3], TP=[1.120]hrs,
2556 *          Continuous simulation parameters:
2557 *          IaRECper=[4](hrs),
2558 *          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2559 *          InterEventTime=[12](hrs)
2560 *          Baseflow simulation parameters:
2561 *          BaseFlowOption=[1] ,
2562 *          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2563 *          VHydCond=[0.055](mm/hr), END=-1
2564 *%-----|-----
2565 *COMPUTE DUALHYD NHYDin=["S-1-D2"], CINLET=[2.062](cms), NINLET=[1],
2566 *          MajNHYD=["S-1-D2J"]
2567 *          MinNHYD=["S-1-D2N"]
2568 *          TMJSTO=[9999999](cu-m)
2569 *%-----|-----
2570 *ADD HYD NHYDsum=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
2571 *%-----|-----
2572 *ROUTE RESERVOIR NHYDout=["S-1-D2R"], NHYDin=["S-1-D2S"],
2573 *          RDT=[1](min),
2574 *          TABLE of ( OUTFLOW-STORAGE ) values
2575 *          (cms) - (ha-m)
2576 *          [ 0.0      , 0.0 ]
2577 *          [ 0.2231, 0.7445 ]
2578 *          [   -1   ,  -1   ] (max twenty pts)
2579 *          NHYDovf=["S-1-D2Rovf"]
2580 *%-----|-----
2581 *CONTINUOUS STANDHYD NHYD=["S-1-D3"], DT=[1](min), AREA=[6.79](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2582 *          LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2583 *          LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2584 *          LGI=[212.760](m), MNI=[0.013], SCI=[0](min),
2585 *          Continuous simulation parameters:
2586 *          IaRECper=[4](hrs), IaRECimp=[4](hrs),
2587 *          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2588 *          InterEventTime=[12](hrs), END=-1
2589 *%-----|-----
2590 *CONTINUOUS NASHYD NHYD=["S-1-D3"], DT=[1]min, AREA=[6.79](ha),
2591 *          DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2592 *          N=[3], TP=[1.281]hrs,
2593 *          Continuous simulation parameters:
2594 *          IaRECper=[4](hrs),
2595 *          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2596 *          InterEventTime=[12](hrs)
2597 *          Baseflow simulation parameters:
2598 *          BaseFlowOption=[1] ,
2599 *          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2600 *          VHydCond=[0.055](mm/hr), END=-1
2601 *%-----|-----
2602 *COMPUTE DUALHYD NHYDin=["S-1-D3"], CINLET=[0.719](cms), NINLET=[1],
2603 *          MajNHYD=["S-1-D3J"]
2604 *          MinNHYD=["S-1-D3N"]
2605 *          TMJSTO=[9999999](cu-m)
2606 *%-----|-----
2607 *ADD HYD NHYDsum=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
2608 *%-----|-----

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2609 *ROUTE RESERVOIR      NHYDout=["S-1-D3R"] ,NHYDin=["S-1-D3S"] ,
2610 *                      RDT=[1](min),
2611 *                      TABLE of ( OUTFLOW-STORAGE ) values
2612 *                      (cms) - (ha-m)
2613 *                      [ 0.0      , 0.0 ]
2614 *                      [ 0.0811, 0.2708 ]
2615 *                      [   -1   ,  -1   ] (max twenty pts)
2616 *                      NHYDovf=["S-1-D3Rovf"]
2617 *%-----|-----
2618 CONTINUOUS STANDHYD NHYD=["S-1-D4"], DT=[1](min), AREA=[3.28](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2619 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2620 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2621 LGI=[147.874](m), MNI=[0.013], SCI=[0](min),
2622 Continuous simulation parameters:
2623 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2624 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2625 InterEventTime=[12](hrs), END=-1
2626 *%-----|-----
2627 *CONTINUOUS NASHYD    NHYD=["S-1-D4"], DT=[1]min, AREA=[3.28](ha),
2628 *                      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2629 *                      N=[3], TP=[1.10]hrs,
2630 *                      Continuous simulation parameters:
2631 *                      IaRECper=[4](hrs),
2632 *                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2633 *                      InterEventTime=[12](hrs)
2634 *                      Baseflow simulation parameters:
2635 *                      BaseFlowOption=[1] ,
2636 *                      InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2637 *                      VHydCond=[0.055](mm/hr), END=-1
2638 *%-----|-----
2639 COMPUTE DUALHYD    NHYDin=["S-1-D4"], CINLET=[0.421](cms), NINLET=[1],
2640 *                      MajNHYD=["S-1-D4J"]
2641 *                      MinNHYD=["S-1-D4N"]
2642 *                      TMJSTO=[9999999](cu-m)
2643 *%-----|-----
2644 ADD HYD           NHYDsum=["S-1-D4S"], NHYDs to add=["S-1-D4J"+"S-1-D4N"]
2645 *%-----|-----
2646 ROUTE RESERVOIR  NHYDout=["S-1-D4R"] ,NHYDin=["S-1-D4S"] ,
2647 *                      RDT=[1](min),
2648 *                      TABLE of ( OUTFLOW-STORAGE ) values
2649 *                      (cms) - (ha-m)
2650 *                      [ 0.0      , 0.0 ]
2651 *                      [ 0.0392, 0.1308 ]
2652 *                      [   -1   ,  -1   ] (max twenty pts)
2653 *                      NHYDovf=["S-1-D4Rovf"]
2654 *%-----|-----
2655 CONTINUOUS STANDHYD NHYD=["S-1-D5"], DT=[1](min), AREA=[12.84](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2656 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
IAper=[4.67](mm), SLPP=[2.0](%),
2657 LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
IAimp=[1.57](mm), SLPI=[0.75](%),
2658 LGI=[292.57](m), MNI=[0.013], SCI=[0](min),
2659 Continuous simulation parameters:
2660 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2661 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2662 InterEventTime=[12](hrs), END=-1
2663 *%-----|-----
2664 *CONTINUOUS NASHYD    NHYD=["S-1-D5"], DT=[1]min, AREA=[12.84](ha),
2665 *                      DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2666 *                      N=[3], TP=[1.10]hrs,
2667 *                      Continuous simulation parameters:
2668 *                      IaRECper=[4](hrs),

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2669 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2670 * InterEventTime=[12](hrs)
2671 * Baseflow simulation parameters:
2672 * BaseFlowOption=[1] ,
2673 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2674 * VHydCond=[0.055](mm/hr), END=-1
2675 *%-----|-----
2676 COMPUTE DUALHYD NHYDin=["S-1-D5"], CINLET=[1.5](cms), NINLET=[1],
2677 MajNHYD=["S-1-D5J"]
2678 MinNHYD=["S-1-D5N"]
2679 TMJSTO=[9999999](cu-m)
2680 *%-----|-----
2681 ADD HYD NHYDsum=["S-1-D5S"], NHYDs to add=["S-1-D5J"+"S-1-D5N"]
2682 *%-----|-----
2683 ROUTE RESERVOIR NHYDout=["S-1-D5R"],NHYDin=["S-1-D5S"] ,
2684 RDT=[1](min),
2685 TABLE of ( OUTFLOW-STORAGE ) values
2686 (cms) - (ha-m)
2687 [ 0.0 , 0.0 ]
2688 [ 0.1535, 0.5120 ]
2689 [ -1 , -1 ] (max twenty pts)
2690 NHYDovf=["S-1-D5Rovf"]
2691 *%-----|-----
2692 CONTINUOUS STANDHYD NHYD=["S-1-D6"], DT=[1](min), AREA=[1.75](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2693 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2694 IAper=[4.67](mm), SLPP=[2.0](%),
LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2695 IAimp=[1.57](mm), SLPI=[0.75](%),
2696 LGI=[108.01](m), MNI=[0.013], SCI=[0](min),
2697 Continuous simulation parameters:
2698 IaREcper=[4](hrs), IaREcimp=[4](hrs),
2699 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
InterEventTime=[12](hrs), END=-1
2700 *%-----|-----
2701 *CONTINUOUS NASHYD NHYD=["S-1-D6"], DT=[1]min, AREA=[1.75](ha),
2702 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2703 N=[3], TP=[1.10]hrs,
2704 Continuous simulation parameters:
2705 IaREcper=[4](hrs),
2706 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2707 InterEventTime=[12](hrs)
2708 Baseflow simulation parameters:
2709 BaseFlowOption=[1] ,
2710 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2711 VHydCond=[0.055](mm/hr), END=-1
2712 *%-----|-----
2713 COMPUTE DUALHYD NHYDin=["S-1-D6"], CINLET=[0.232](cms), NINLET=[1],
2714 MajNHYD=["S-1-D6J"]
2715 MinNHYD=["S-1-D6N"]
2716 TMJSTO=[9999999](cu-m)
2717 *%-----|-----
2718 ADD HYD NHYDsum=["S-1-D6S"], NHYDs to add=["S-1-D6J"+"S-1-D6N"]
2719 *%-----|-----
2720 ROUTE RESERVOIR NHYDout=["S-1-D6R"],NHYDin=["S-1-D6S"] ,
2721 RDT=[1](min),
2722 TABLE of ( OUTFLOW-STORAGE ) values
2723 (cms) - (ha-m)
2724 [ 0.0 , 0.0 ]
2725 [ 0.0209, 0.0698 ]
2726 [ -1 , -1 ] (max twenty pts)
2727 NHYDovf=["S-1-D6Rovf"]
2728 *%-----|-----
2729 CONTINUOUS STANDHYD NHYD=["S-1-D7"], DT=[1](min), AREA=[2.03](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2730 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:

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2731 IAper=[4.67](mm), SLPP=[2.0](%),
LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
2732 IAimp=[1.57](mm), SLPI=[0.75](%),
2733 LGI=[116.33](m), MNI=[0.013], SCI=[0](min),
2734 Continuous simulation parameters:
2735 IaREcper=[4](hrs), IaREcimp=[4](hrs),
2736 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2737 InterEventTime=[12](hrs), END=-1
2738 *%-----|-----
2738 *CONTINUOUS NASHYD NHYD=["S-1-D7"], DT=[1]min, AREA=[2.03](ha),
2739 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2740 * N=[3], TP=[1.10]hrs,
2741 * Continuous simulation parameters:
2742 * IaREcper=[4](hrs),
2743 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2744 * InterEventTime=[12](hrs)
2745 * Baseflow simulation parameters:
2746 * BaseFlowOption=[1] ,
2747 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2748 * VHydCond=[0.055](mm/hr), END=-1
2749 *%-----|-----
2750 COMPUTE DUALHYD NHYDin=["S-1-D7"], CINLET=[0.265](cms), NINLET=[1],
2751 MajNHYD=["S-1-D7J"]
2752 MinNHYD=["S-1-D7N"]
2753 TMJSTO=[9999999](cu-m)
2754 *%-----|-----
2755 ADD HYD NHYDsum=["S-1-D7S"], NHYDs to add=["S-1-D7J"+"S-1-D7N"]
2756 *%-----|-----
2757 ROUTE RESERVOIR NHYDout=["S-1-D7R"],NHYDin=["S-1-D7S"] ,
2758 RDT=[1](min),
2759 TABLE of ( OUTFLOW-STORAGE ) values
2760 (cms) - (ha-m)
2761 [ 0.0 , 0.0 ]
2762 [ 0.0243, 0.0810 ]
2763 [ -1 , -1 ] (max twenty pts)
2764 NHYDovf=["S-1-D7Rovf"]
2765 *%-----|-----
2766 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2767 *CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1](min), AREA=[5.27](ha), XIMP=[0.65],
TIMP=[0.65], DWF=[0](cms),
2768 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
2769 IAper=[4.67](mm), SLPP=[2.0](%),
2770 * LGI=[187.439](m), MNI=[0.013], SCI=[0](min),
2771 * Continuous simulation parameters:
2772 * IaREcper=[4](hrs), IaREcimp=[4](hrs),
2773 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2774 * InterEventTime=[12](hrs), END=-1
2775 *%-----|-----
2776 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[1]min, AREA=[5.27](ha),
2777 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2778 * N=[3], TP=[1.10]hrs,
2779 * Continuous simulation parameters:
2780 * IaREcper=[4](hrs),
2781 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2782 * InterEventTime=[12](hrs)
2783 * Baseflow simulation parameters:
2784 * BaseFlowOption=[1] ,
2785 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2786 * VHydCond=[0.055](mm/hr), END=-1
2787 *%-----|-----
2788 *COMPUTE DUALHYD NHYDin=["S-1-D8"], CINLET=[2.279](cms), NINLET=[1],
2789 * MajNHYD=["S-1-D8J"]
2790 * MinNHYD=["S-1-D8N"]

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2791 * TMJSTO=[9999999](cu-m)
2792 *%-----|-----|
2793 *ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2794 *%-----|-----|
2795 *ADD HYD NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Fost"]
2796 *%-----|-----|
2797 *COMPUTE DUALHYD NHYDin=["S-1-D"], CINLET=[11.616](cms), NINLET=[1],
2798 * MajNHYD=["S-1-D-MJ"]
2799 * MinNHYD=["S-1-D-MN"]
2800 * TMJSTO=[5974](cu-m)
2801 *%-----|-----|
2802 *ADD HYD NHYDsum=["S-1-DEV"], NHYDs to add=["S-1-D-MJ"+"S-1-D-MN"]
2803 *%-----|-----|
2804 *ROUTE RESERVOIR NHYDout=["S-1-D8R"], NHYDin=["S-1-D8S"],
2805 * RDT=[1](min),
2806 * TABLE of ( OUTFLOW-STORAGE ) values
2807 * (cms) - (ha-m)
2808 * [ 0.0 , 0.0 ]
2809 * [ 0.0630, 0.2102 ]
2810 * [ -1 , -1 ] (max twenty pts)
2811 * NHYDovf=["S-1-D8Rovf"]
2812 *%-----|-----|
2813 *%-----|-----|
2814 * -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
2815 *# Catchment W_CLAR
2816 *# - To West Clarke Drain (south of the Jock)
2817 *# - Subdivision with 43% imp. as per Barrhaven South MSS
2818 *# - 2020-11-30 update CLARKE Tributary Drainage Area to = 121 ha based on
P598(04)-11
2819 *# - 2020-11-30 split CLARKE Drainage Area to MAJOR and ALL
2820 *#*****
2821 CONTINUOUS STANDHYD NHYD=["W_CLAR_MJ"], DT=[1]min, AREA=[1.772](ha),
2822 XIMP=[0.46], TIMP=[0.59], DWF=[0](cms), LOSS=[2],
2823 SCS curve number CN=[77],
2824 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
2825 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2826 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
2827 LGI=[109](m), MNI=[0.013], SCI=[0](min),
2828 Continuous simulation parameters:
2829 IaRECper=[4](hrs), IaRECimp=[4](hrs),
2830 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2831 InterEventTime=[18](hrs), END=-1
2832 *%-----|-----|
2833 *COMPUTE DUALHYD NHYDin=["W_CLAR_MJ"], CINLET=[0.213](cms), NINLET=[1],
2834 * MajNHYD=["W_CLAR_MJj"]
2835 * MinNHYD=["W_CLAR_MJn"]
2836 * TMJSTO=[0.1](cu-m)
2837 *%-----|-----|
2838 *# 5-Year + 12% Capture
2839 ROUTE RESERVOIR NHYDout=["W_CLAR_MJn"], NHYDin=["W_CLAR_MJ"],
2840 RDT=[1](min),
2841 TABLE of ( OUTFLOW-STORAGE ) values
2842 (cms) - (ha-m)
2843 [ 0.0 , 0.0 ]
2844 [ 0.213 , 0.0001 ]
2845 [ -1 , -1 ] (max twenty pts)
2846 NHYDovf=["W_CLAR_MJj"],
2847 *%-----|-----|
2848 * -Clarke_All area from P 598(04)-11 = 120.207 ha, change to 127.298 ha based on
GIS measurements,
2849 * -JFSA, 2021-01-19 update W_CLAR_ALL to (121.17-1.772=119.398) ha based on GIS
measurements W_CLAR is 121.17 ha and W_CLAR_MJ is 1.772 ha
2850 CONTINUOUS STANDHYD NHYD=["W_CLAR_ALL"], DT=[1]min, AREA=[119.398](ha),
2851 XIMP=[0.60], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
2852 SCS curve number CN=[77],

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2853 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
2854 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2855 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
2856 LGI=[892.18](m), MNI=[0.013], SCI=[0](min),
2857 Continuous simulation parameters:
2858 IaREcper=[4](hrs), IaREcimp=[4](hrs),
2859 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2860 InterEventTime=[18](hrs), END=-1
2861 *%-----|-----|
2862 ADD HYD NHYDsum=["W_CLAR"], NHYDs to add=["W_CLAR_ALL"+"W_CLAR_MJj"]
2863 *%-----|-----|
2864 SAVE HYD NHYD=["W_CLAR"], # OF PCYCLES=[-1], ICASEsh=[1]
2865 HYD_COMMENT=["Total Flows to West Clarke"]
2866 *#*****
2867 *# West Clarke Pond 2
2868 *# - Rating curve obtained from Barrhaven South MSS modeling
2869 *# - Tributary Drainage Area to MSS Pond 2 = 241 ha
2870 *#*****
2871 ROUTE RESERVOIR NHYDout=["MS_P2"], NHYDin=["W_CLAR"],
2872 RDT=[1](min),
2873 TABLE of ( OUTFLOW-STORAGE ) values
2874 (cms) - (ha-m)
2875 [ 0.0 , 0.0 ]
2876 [ 0.128 , 0.161 ]
2877 [ 0.138 , 0.409 ]
2878 [ 0.148 , 0.68 ]
2879 [ 0.227 , 0.931 ]
2880 [ 0.354 , 1.223 ]
2881 [ 0.505 , 1.52 ]
2882 [ 0.666 , 1.821 ]
2883 [ 0.831 , 2.123 ]
2884 [ 0.995 , 2.434 ]
2885 [ 1.069 , 2.583 ]
2886 [ 1.51 , 2.647 ]
2887 [ 4.904 , 2.861 ]
2888 [ 13.048 , 3.188 ]
2889 [ 23.745 , 3.523 ]
2890 [ 36.474 , 3.871 ]
2891 [ 45.938 , 4.127 ]
2892 [ 61.652 , 4.539 ]
2893 [ -1 , -1 ] (max twenty pts)
2894 NHYDovf=["P2-OVF"]
2895 *%-----|-----|
2896 *#*****
2897 * -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2898 *CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2899 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
2900 * N=[3], TP=[1.10]hrs,
2901 * Continuous simulation parameters:
2902 * IaREcper=[4](hrs),
2903 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2904 * InterEventTime=[12](hrs)
2905 * Baseflow simulation parameters:
2906 * BaseFlowOption=[1] ,
2907 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2908 * VHydCond=[0.055](mm/hr), END=-1
2909 *%-----|-----|
2910 ADD HYD NHYDsum=["SN_CE"], NHYDs to
add=["N_CE"+"S-1-D4R"+"S-1-D5R"+"S-1-D4Rovf"+"S-1-D5Rovf"+"MS_P2"+"P2-OVF"]
2911 *%-----|-----|
2912 SAVE HYD NHYD=["SN_CE"], # OF PCYCLES=[-1], ICASEsh=[1]
2913 HYD_COMMENT=["Total Flows before Station 5737 on Jock River"]
2914 *%-----|-----|
2915 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5737

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2916  *# 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted from the
      HEC-RAS model
      T:\PROJ\1474-16\Design\20201026-QuantityControlAnalysis\HEC-RAS\JockLidar2005
2917  *# JFSA 2021-03-02 change the slope to 0.0175% instead of 0.02593 to stabilize the model
2918  ROUTE CHANNEL      NHYDout=["5737"] ,NHYDin=["SN_CE"] ,
2919                      RDT=[1](min),
2920                      CHLGTH=[270](m),   CHSLOPE=[0.0175](%),
2921                      FPSLOPE=[0.0175](%),
2922                      SECNUM=[1.0],      NSEG=[3]
2923                      ( SEGROUGH, SEGDIST (m))=
2924                      [0.050,-24.04
2925                      -0.035,23.92
2926                      0.050,1130.8] NSEG times
2927                      ( DISTANCE (m), ELEVATION (m))=
2928                      [-1060.52, 94 ]
2929                      [-268.6, 91.5 ]
2930                      [-259.43, 91.5 ]
2931                      [-179.48, 91.5 ]
2932                      [-67.9, 91.5 ]
2933                      [-59.21, 91.5 ]
2934                      [-33.19, 91 ]
2935                      [-26.08, 90.5 ]
2936                      [-24.04, 90 ]
2937                      [-13.14, 86.77 ]
2938                      [0, 85 ]
2939                      [14.68, 86.74 ]
2940                      [23.92, 90 ]
2941                      [25.78, 90.5 ]
2942                      [31.91, 91 ]
2943                      [91.95, 91.5 ]
2944                      [772.15, 92 ]
2945                      [961.49, 92.5 ]
2946                      [1044.69, 93 ]
2947                      [1130.8, 95 ]
2948  *%-----|-----|
2949  ADD HYD      NHYDsum=["5002"], NHYDs to
      add=["5737"+"S-1-D1R"+"S-1-D6R"+"S-1-D7R"+"S-1-D1Rovf"+"S-1-D6Rovf"+"S-1-D7Rovf"]
2950  *%-----|-----|
2951  SAVE HYD      NHYD=["5002"], # OF PCYCLES=[-1], ICASEsh=[1]
2952                      HYD_COMMENT=["Total Flows before Station 5002 on Jock River"]
2953  *%-----|-----|
2954  *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
2955  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002
2956  *# JFSA 2021-02-19 Change the slope from 0.01 % (as per Stantec Report 2007) to 0.0255
      % so the model will be more stable and give reasonable results. It is justifiable as
      ROUTE CHANNELs aren't well suited to really flat slopes.
2957  *# JFSA 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one
      with 825 m length so the model will be more stable
2958  *# JFSA 2021-02-26 change the length of 5002 route channel from 825 m to 736 m. That is
      because of adding station 5737 between station 6016 and station 5002. Then the length
      from station 5737 to station 5002 is 736 m. Change the slope from 0.0255 % to 0.09511 %
2959  *
2960  ROUTE CHANNEL      NHYDout=["N_WCa"] ,NHYDin=["5002"] ,
2961                      RDT=[1](min),
2962                      CHLGTH=[245.33333](m),   CHSLOPE=[0.09511](%),
2963                      FPSLOPE=[0.09511](%),
2964                      SECNUM=[1.0],      NSEG=[3]
2965                      ( SEGROUGH, SEGDIST (m))=
2966                      [0.050,-37.5
2967                      -0.035,37.50
2968                      0.050,157.05] NSEG times
2969                      ( DISTANCE (m), ELEVATION (m))=
2970                      [-601.81, 91.5]
2971                      [-37.50, 90.00]
2972                      [-19.61, 87.04]
2973                      [0.00, 85.70]

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```

2974      [14.87, 86.93]
2975      [37.50, 90.00]
2976      [38.54, 90.50]
2977      [42.23, 91]
2978      [157.05,91.50]
2979      *      [161.44, 91.50]
2980      *      [236.48, 93.00]
2981      *      [385.47, 92.50]
2982      *      [390.78, 92.50]
2983      *%-----|-----
2984      ROUTE CHANNEL      NHYDout=["N_WCb"] ,NHYDin=["N_WCa"] ,
2985      RDT=[1](min),
2986      CHLGTH=[245.33333](m),  CHSLOPE=[0.09511](%),
2987      FPSLOPE=[0.09511](%),
2988      SECNUM=[1.0],      NSEG=[3]
2989      ( SEGROUGH, SEGDIST (m))=
2990      [0.050,-37.5
2991      -0.035,37.50
2992      0.050,157.05] NSEG times
2993      ( DISTANCE (m), ELEVATION (m))=
2994      [-601.81, 91.5]
2995      [-37.50, 90.00]
2996      [-19.61, 87.04]
2997      [0.00, 85.70]
2998      [14.87, 86.93]
2999      [37.50, 90.00]
3000      [38.54, 90.50]
3001      [42.23, 91]
3002      [157.05,91.50]
3003      *%-----|-----
3004      ROUTE CHANNEL      NHYDout=["N_WC"] ,NHYDin=["N_WCb"] ,
3005      RDT=[1](min),
3006      CHLGTH=[245.33333](m),  CHSLOPE=[0.09511](%),
3007      FPSLOPE=[0.09511](%),
3008      SECNUM=[1.0],      NSEG=[3]
3009      ( SEGROUGH, SEGDIST (m))=
3010      [0.050,-37.5
3011      -0.035,37.50
3012      0.050,157.05] NSEG times
3013      ( DISTANCE (m), ELEVATION (m))=
3014      [-601.81, 91.5]
3015      [-37.50, 90.00]
3016      [-19.61, 87.04]
3017      [0.00, 85.70]
3018      [14.87, 86.93]
3019      [37.50, 90.00]
3020      [38.54, 90.50]
3021      [42.23, 91]
3022      [157.05,91.50]
3023      *#*****
3024      *      -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
3025      *ADD HYD      NHYDsum=["SN_WC"], NHYDs to
add=["MS_P2"+"P2-OVF"+"N_WC"+"W_CLAR_UNDE"]
3026      *%-----|-----
3027      *SAVE HYD      NHYD=["SN_WC"], # OF PCYCLES=[-1], ICASEsh=[1]
3028      *      HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
3029      *%-----|-----
3030      *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
3031      *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
3032      *#
3033      ROUTE CHANNEL      NHYDout=["N_KB"] ,NHYDin=["N_WC"] ,
3034      RDT=[1](min),
3035      CHLGTH=[1020](m),  CHSLOPE=[0.0498](%),
3036      FPSLOPE=[0.0498](%),
3037      SECNUM=[1.0],      NSEG=[3]

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3038      ( SEGROUGH, SEGDIST (m))=
3039      [0.050,-23.63
3040      -0.035,23.63
3041      0.050,728.3] NSEG times
3042      ( DISTANCE (m), ELEVATION (m))=
3043      [-1082.01,94]
3044      [-1028.17,92.5]
3045      [-992.3,93.5]
3046      [-279.34,90]
3047      [-23.63,90]
3048      [-13.45,87.13]
3049      [-0.07,86.24]
3050      [10.54,87.15]
3051      [23.63,90]
3052      [24.86,90.5]
3053      [26.72,91]
3054      [45.07,91.5]
3055      [128.17,91.5]
3056      [270.7,92.5]
3057      [728.3,95]
3058      *%-----|-----|
3059      *#*****|
3060      *#      Catchment KEN_BU
3061      *#      - To Kennedy-Burnett SWM Facility
3062      *#      - Outlets to Fraser-Clarke drain (north of the Jock)
3063      *#      - Medium density residential subdivision
3064      *      - Add Kennedy Burnett model (Convert PCSWMM from NOVATECH June, 2020 to SWMHYMO)
3065      *#*****|
3066      *CONTINUOUS STANDHYD NHYD=["KEN_BU"], DT=[1]min, AREA=[281](ha),
3067      *          XIMP=[0.55], TIMP=[0.55], DWF=[0](cms), LOSS=[2],
3068      *          SCS curve number CN=[71],
3069      *          Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3070      *          LGP=[40](m), MNP=[0.25], SCP=[0](min),
3071      *          Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3072      *          LGI=[1369](m), MNI=[0.013], SCI=[0](min),
3073      *          Continuous simulation parameters:
3074      *          IaRECper=[4](hrs), IaRECimp=[4](hrs),
3075      *          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3076      *          InterEventTime=[18](hrs), END=-1
3077      *%-----|-----|
3078      *#*****|
3079      *#      Existing Kennedy-Burnett SWM Facility
3080      *#      - Rating curve obtained from URTKBP
3081      *#      - Tributary Drainage Area to Pond = 160 ha
3082      *#*****|
3083      *ROUTE RESERVOIR      NHYDout=["KEN_P"], NHYDin=["KEN_BU"],
3084      *          RDT=[1](min),
3085      *          TABLE of ( OUTFLOW-STORAGE ) values
3086      *          (cms) - (ha-m)
3087      *          [ 0.0 , 0.0 ]
3088      *          [ 0.13 , 0.26]
3089      *          [ 0.43 , 0.56]
3090      *          [ 0.67 , 0.90]
3091      *          [ 0.86 , 1.32]
3092      *          [ 1.01 , 1.79]
3093      *          [ 1.15 , 2.33]
3094      *          [ -1 , -1 ] (max twenty pts)
3095      *          NHYDovf=["KEN-OV"]
3096      *%-----|-----|
3097      *      -JFSA, 2021-01-19 update all KEN_BU areas based on GIS measurements
3098      CONTINUOUS STANDHYD NHYD=["KB-01A"], DT=[1]min, AREA=[40.82](ha), XIMP=[0.097],
3099      TIMP=[0.4], DWF=[0](cms), LOSS=[1]:
3099      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3100      F=[0.00](mm),
3100      Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
3100      MNP=[0.250], SCP=[0](min),

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3101      Impervious areas: IAimp=[0.785](mm), SLPI=[0.5](%),
3102      LGI=[521.664](m), MNI=[0.013], SCI=[0](min),
3103      Continuous simulation parameters:
3104      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3105      END=-1
3106
3107      *%-----|-----|
3108      COMPUTE DUALHYD NHYDin=["KB-01A"], CINLET=[3.6](cms), NINLET=[1],
3109      MajNHYD=["KB-01A-MJ"]
3110      MinNHYD=["KB-01A-MN"]
3111      TMJSTO=[4995](cu-m)
3112
3113      *%-----|-----|
3114      ADD HYD NHYDsum=["KB-01A-S"], NHYDs to add=["KB-01A-MJ"+"KB-01A-MN"]
3115      *%-----|-----|
3116      CONTINUOUS STANDHYD NHYD=["KB-01B"], DT=[1]min, AREA=[31.1](ha), XIMP=[0.1875],
3117      TIMP=[0.375], DWF=[0](cms), LOSS=[1]:
3118      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3119      F=[0.00](mm),
3120      Pervious areas: IAper=[4.67](mm), SLPP=[0.42](%), LGP=[40](m),
3121      MNP=[0.250], SCP=[0](min),
3122      Impervious areas: IAimp=[0.785](mm), SLPI=[0.42](%),
3123      LGI=[455.339](m), MNI=[0.013], SCI=[0](min),
3124      Continuous simulation parameters:
3125      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3126      END=-1
3127
3128      *%-----|-----|
3129      COMPUTE DUALHYD NHYDin=["KB-01B"], CINLET=[1.585](cms), NINLET=[1],
3130      MajNHYD=["KB-01B-MJ"]
3131      MinNHYD=["KB-01B-MN"]
3132      TMJSTO=[6075](cu-m)
3133
3134      *%-----|-----|
3135      ADD HYD NHYDsum=["KB-01B-S"], NHYDs to add=["KB-01B-MJ"+"KB-01B-MN"]
3136      *%-----|-----|
3137      CONTINUOUS STANDHYD NHYD=["KB-01C"], DT=[1]min, AREA=[13.78](ha), XIMP=[0.2045],
3138      TIMP=[0.409], DWF=[0](cms), LOSS=[1]:
3139      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3140      F=[0.00](mm),
3141      Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3142      MNP=[0.250], SCP=[0](min),
3143      Impervious areas: IAimp=[0.785](mm), SLPI=[0.5](%),
3144      LGI=[303.095](m), MNI=[0.013], SCI=[0](min),
3145      Continuous simulation parameters:
3146      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3147      END=-1
3148
3149      *%-----|-----|
3150      COMPUTE DUALHYD NHYDin=["KB-01C"], CINLET=[1.35](cms), NINLET=[1],
3151      MajNHYD=["KB-01C-MJ"]
3152      MinNHYD=["KB-01C-MN"]
3153      TMJSTO=[1880](cu-m)
3154
3155      *%-----|-----|
3156      ADD HYD NHYDsum=["KB-01C-S"], NHYDs to add=["KB-01C-MJ"+"KB-01C-MN"]
3157      *%-----|-----|
3158      CONTINUOUS STANDHYD NHYD=["KB-03"], DT=[1]min, AREA=[84.78](ha), XIMP=[0.197],
3159      TIMP=[0.394], DWF=[0](cms), LOSS=[1]:
3160      Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3161      F=[0.00](mm),
3162      Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3163      MNP=[0.250], SCP=[0](min),
3164      Impervious areas: IAimp=[0.785](mm), SLPI=[0.63](%),
3165      LGI=[751.798](m), MNI=[0.013], SCI=[0](min),
3166      Continuous simulation parameters:
3167      IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3168      END=-1
3169
3170      *%-----|-----|
3171      COMPUTE DUALHYD NHYDin=["KB-03"], CINLET=[5.27](cms), NINLET=[1],
3172      MajNHYD=["KB-03-MJ"]
3173      MinNHYD=["KB-03-MN"]

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3150          TMJSTO=[15500](cu-m)
3151  *%-----|-----|
3152  ADD HYD      NHYDsum=["KB-03-S"], NHYDs to add=["KB-03-MJ"+"KB-03-MN"]
3153  *%-----|-----|
3154  CONTINUOUS STANDHYD NHYD=["KB-04"], DT=[1]min, AREA=[6.95](ha), XIMP=[0.85],
TIMP=[0.85], DWF=[0](cms), LOSS=[1]:
3155          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3156          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3157          Impervious areas: IAimp=[0.942](mm), SLPI=[0.5](%),
LGI=[215.252](m), MNI=[0.013], SCI=[0](min),
3158          Continuous simulation parameters:
3159          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3160  *%-----|-----|
3161  COMPUTE DUALHYD NHYDin=["KB-04"], CINLET=[0.503](cms), NINLET=[1],
3162          MajNHYD=["KB-04-MJ"]
3163          MinNHYD=["KB-04-MN"]
3164          TMJSTO=[1972](cu-m)
3165  *%-----|-----|
3166  ADD HYD      NHYDsum=["KB-04-S"], NHYDs to add=["KB-04-MJ"+"KB-04-MN"]
3167  *%-----|-----|
3168  CONTINUOUS STANDHYD NHYD=["KB-05"], DT=[1]min, AREA=[5.19](ha), XIMP=[0.93],
TIMP=[0.93], DWF=[0](cms), LOSS=[1]:
3169          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3170          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3171          Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
LGI=[186.011](m), MNI=[0.013], SCI=[0](min),
3172          Continuous simulation parameters:
3173          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3174  *%-----|-----|
3175  *%-----|-----|
3176  CONTINUOUS STANDHYD NHYD=["KB-06"], DT=[1]min, AREA=[12.93](ha), XIMP=[0.873],
TIMP=[0.873], DWF=[0](cms), LOSS=[1]:
3177          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3178          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3179          Impervious areas: IAimp=[0.942](mm), SLPI=[4.75](%),
LGI=[293.598](m), MNI=[0.013], SCI=[0](min),
3180          Continuous simulation parameters:
3181          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3182  *%-----|-----|
3183  COMPUTE DUALHYD NHYDin=["KB-06"], CINLET=[2.262](cms), NINLET=[1],
3184          MajNHYD=["KB-06-MJ"]
3185          MinNHYD=["KB-06-MN"]
3186          TMJSTO=[1950](cu-m)
3187  *%-----|-----|
3188  ADD HYD      NHYDsum=["KB-06-S"], NHYDs to add=["KB-06-MJ"+"KB-06-MN"]
3189  *%-----|-----|
3190  CONTINUOUS STANDHYD NHYD=["KB-11"], DT=[1]min, AREA=[4.03](ha), XIMP=[0.675],
TIMP=[0.675], DWF=[0](cms), LOSS=[1]:
3191          Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3192          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3193          Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[163.911](m), MNI=[0.013], SCI=[0](min),
3194          Continuous simulation parameters:
3195          IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1

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3196 *%-----|-----|
3197 COMPUTE DUALHYD NHYDin=["KB-11"], CINLET=[0.5773](cms), NINLET=[1],
3198 MajNHYD=["KB-11-MJ"]
3199 MinNHYD=["KB-11-MN"]
3200 TMJSTO=[597](cu-m)
3201 *%-----|-----|
3202 ADD HYD NHYDsum=["KB-11-S"], NHYDs to add=["KB-11-MJ"+"KB-11-MN"]
3203 *%-----|-----|
3204 CONTINUOUS STANDHYD NHYD=["S1"], DT=[1]min, AREA=[4.99](ha), XIMP=[0.93], TIMP=[0.93],
DWF=[0](cms), LOSS=[1]:
3205 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3206 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3207 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
LGI=[182.392](m), MNI=[0.013], SCI=[0](min),
3208 Continuous simulation parameters:
3209 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3210 *%-----|-----|
3211 CONTINUOUS STANDHYD NHYD=["KB-15"], DT=[1]min, AREA=[2.15](ha), XIMP=[0.79],
TIMP=[0.79], DWF=[0](cms), LOSS=[1]:
3212 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3213 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3214 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[119.722](m), MNI=[0.013], SCI=[0](min),
3215 Continuous simulation parameters:
3216 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3217 *%-----|-----|
3218 *%-----|-----|
3219 ADD HYD NHYDsum=["KB-P1"], NHYDs to
add=["KB-01A-S"+"KB-01B-S"+"KB-01C-S"+"KB-03-S"+"KB-04-S"+"KB-05"+"KB-06-S"+"KB-11-S"+"KB
-15"+"S1"]
3220 *%-----|-----|
3221 ROUTE RESERVOIR NHYDout=["KB-P1R"], NHYDin=["KB-P1"],
3222 RDT=[1](min),
3223 TABLE of ( OUTFLOW-STORAGE ) values
3224 (cms) - (ha-m)
3225 [ 0.0 , 0.0 ]
3226 [0.076,0.003]
3227 [0.088,0.006]
3228 [0.136,0.011]
3229 [0.301,0.017]
3230 [0.454,0.027]
3231 [0.631,0.041]
3232 [1.173,0.068]
3233 [1.91,0.111]
3234 [4.847,0.231]
3235 [9.813,0.436]
3236 [12.134,0.617]
3237 [12.438,0.732]
3238 [12.424,0.811]
3239 [12.425,0.894]
3240 [ -1 , -1 ] (max twenty pts)
3241 NHYDovf=["KB-P1ovf"]
3242 *%-----|-----|
3243 ADD HYD NHYDsum=["KB-Pond1"], NHYDs to add=["KB-P1R"+"KB-P1ovf"]
3244 *%-----|-----|
3245 SAVE HYD NHYD=["KB-Pond1"], # OF PCYCLES=[-1], ICASEsh=[1]
3246 HYD_COMMENT=["Total Flows at KB first pond"]
3247 *%-----|-----|
3248 CONTINUOUS STANDHYD NHYD=["KB-07"], DT=[1]min, AREA=[10.86](ha), XIMP=[0.86],
TIMP=[0.86], DWF=[0](cms), LOSS=[1]:

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3249 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3250 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3251 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[269.072](m), MNI=[0.013], SCI=[0](min),
3252 Continuous simulation parameters:
3253 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3254 *%-----|-----|
3255 COMPUTE DUALHYD NHYDin=["KB-07"], CINLET=[2.094](cms), NINLET=[1],
3256 MajNHYD=["KB-07-MJ"]
3257 MinNHYD=["KB-07-MN"]
3258 TMJSTO=[1378](cu-m)
3259 *%-----|-----|
3260 ADD HYD NHYDsum=["KB-07-S"], NHYDs to add=["KB-07-MJ"+"KB-07-MN"]
3261 *%-----|-----|
3262 CONTINUOUS STANDHYD NHYD=["KB-08"], DT=[1]min, AREA=[6.61](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3263 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3264 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3265 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[209.921](m), MNI=[0.013], SCI=[0](min),
3266 Continuous simulation parameters:
3267 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3268 *%-----|-----|
3269 COMPUTE DUALHYD NHYDin=["KB-08"], CINLET=[1.058](cms), NINLET=[1],
3270 MajNHYD=["KB-08-MJ"]
3271 MinNHYD=["KB-08-MN"]
3272 TMJSTO=[787](cu-m)
3273 *%-----|-----|
3274 ADD HYD NHYDsum=["KB-08-S"], NHYDs to add=["KB-08-MJ"+"KB-08-MN"]
3275 *%-----|-----|
3276 CONTINUOUS STANDHYD NHYD=["KB-09"], DT=[1]min, AREA=[2.6](ha), XIMP=[0.86],
TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3277 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3278 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3279 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
LGI=[131.656](m), MNI=[0.013], SCI=[0](min),
3280 Continuous simulation parameters:
3281 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3282 *%-----|-----|
3283 *%-----|-----|
3284 CONTINUOUS STANDHYD NHYD=["KB-10_1"], DT=[1]min, AREA=[2.37](ha), XIMP=[0.86],
TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3285 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3286 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3287 Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%),
LGI=[125.698](m), MNI=[0.013], SCI=[0](min),
3288 Continuous simulation parameters:
3289 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3290 *%-----|-----|
3291 CONTINUOUS STANDHYD NHYD=["KB-10_2"], DT=[1]min, AREA=[1.14](ha), XIMP=[0.86],
TIMP=[0.86], DWF=[0](cms), LOSS=[1]:
3292 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3293 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),

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3294 MNP=[0.250], SCP=[0](min),
Impervious areas: IAimp=[1.57](mm), SLPI=[2.0](%), LGI=[87.178](m),
3295 MNI=[0.013], SCI=[0](min),
3296 Continuous simulation parameters:
IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3297 *%-----|-----|
3298 *%-----|-----|
3299 CONTINUOUS STANDHYD NHYD=["KB-12"], DT=[1]min, AREA=[4.86](ha), XIMP=[0.79],
TIMP=[0.79], DWF=[0](cms), LOSS=[1]:
3300 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3301 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3302 Impervious areas: IAimp=[1.099](mm), SLPI=[2.0](%),
LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
3303 Continuous simulation parameters:
3304 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3305 *%-----|-----|
3306 COMPUTE DUALHYD NHYDin=["KB-12"], CINLET=[0.8665](cms), NINLET=[1],
3307 MajNHYD=["KB-12-MJ"]
3308 MinNHYD=["KB-12-MN"]
3309 TMJSTO=[632](cu-m)
3310 *%-----|-----|
3311 ADD HYD NHYDsum=["KB-12-S"], NHYDs to add=["KB-12-MJ"+"KB-12-MN"]
3312 *%-----|-----|
3313 CONTINUOUS STANDHYD NHYD=["KB-13"], DT=[1]min, AREA=[10.19](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3314 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3315 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3316 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[260.640](m), MNI=[0.013], SCI=[0](min),
3317 Continuous simulation parameters:
3318 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3319 *%-----|-----|
3320 COMPUTE DUALHYD NHYDin=["KB-13"], CINLET=[1.722](cms), NINLET=[1],
3321 MajNHYD=["KB-13-MJ"]
3322 MinNHYD=["KB-13-MN"]
3323 TMJSTO=[1077](cu-m)
3324 *%-----|-----|
3325 ADD HYD NHYDsum=["KB-13-S"], NHYDs to add=["KB-13-MJ"+"KB-13-MN"]
3326 *%-----|-----|
3327 CONTINUOUS STANDHYD NHYD=["KB-14"], DT=[1]min, AREA=[5.47](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3328 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3329 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3330 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
LGI=[190.962](m), MNI=[0.013], SCI=[0](min),
3331 Continuous simulation parameters:
3332 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3333 *%-----|-----|
3334 COMPUTE DUALHYD NHYDin=["KB-14"], CINLET=[0.8734](cms), NINLET=[1],
3335 MajNHYD=["KB-14-MJ"]
3336 MinNHYD=["KB-14-MN"]
3337 TMJSTO=[631](cu-m)
3338 *%-----|-----|
3339 ADD HYD NHYDsum=["KB-14-S"], NHYDs to add=["KB-14-MJ"+"KB-14-MN"]
3340 *%-----|-----|
3341 *%-----|-----|

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3342 CONTINUOUS STANDHYD NHYD=["KB-16_2"], DT=[1]min, AREA=[3.42](ha), XIMP=[0.71],
TIMP=[0.71], DWF=[0](cms), LOSS=[1]:
3343 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3344 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3345 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[150.997](m), MNI=[0.013], SCI=[0](min),
3346 Continuous simulation parameters:
3347 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3348 *%-----|-----|
3349 ADD HYD NHYDsum=["KB-P2"], NHYDs to
add=["KB-Pond1"+"KB-07-S"+"KB-08-S"+"KB-09"+"KB-10_1"+"KB-10_2"+"KB-12-S"+"KB-13-S"+"KB-14-S"+"KB-16_2"]
3350 *%-----|-----|
3351 ROUTE RESERVOIR NHYDout=["KB-P2R"], NHYDin=["KB-P2"],
3352 RDT=[1](min),
3353 TABLE of ( OUTFLOW-STORAGE ) values
3354 (cms) - (ha-m)
3355 [ 0.0 , 0.0 ]
3356 [0.053,0.005]
3357 [0.132,0.009]
3358 [0.269,0.014]
3359 [0.455,0.023]
3360 [0.699,0.037]
3361 [0.947,0.056]
3362 [1.853,0.09]
3363 [2.712,0.146]
3364 [6.626,0.287]
3365 [11.228,0.515]
3366 [14.885,0.738]
3367 [16.473,0.893]
3368 [17.311,0.998]
3369 [17.633,1.063]
3370 [17.634,1.112]
3371 [ -1 , -1 ] (max twenty pts)
3372 NHYDovf=["KB-P2ovf"]
3373 *%-----|-----|
3374 ADD HYD NHYDsum=["KB-Pond2"], NHYDs to add=["KB-P2R"+"KB-P2ovf"]
3375 *%-----|-----|
3376 SAVE HYD NHYD=["KB-Pond2"], # OF PCYCLES=[-1], ICASEsh=[1]
3377 HYD_COMMENT=["Total Flows at KB second pond"]
3378 *%-----|-----|
3379 CONTINUOUS STANDHYD NHYD=["KB-16_1"], DT=[1]min, AREA=[2.8](ha), XIMP=[0.75],
TIMP=[0.75], DWF=[0](cms), LOSS=[1]:
3380 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3381 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3382 Impervious areas: IAimp=[0.157](mm), SLPI=[0.3](%),
LGI=[136.626](m), MNI=[0.013], SCI=[0](min),
3383 Continuous simulation parameters:
3384 IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3385 *%-----|-----|
3386 ADD HYD NHYDsum=["KB-P3"], NHYDs to add=["KB-Pond2"+"KB-16_1"]
3387 *%-----|-----|
3388 *%-----|-----|
3389 * One inflow node from pond 3 is added to the model (ROUTE RESERVOIR)
3390 * Another inflow node from right side of pond 3 is not added to the model
3391 ROUTE RESERVOIR NHYDout=["KB-P3R"], NHYDin=["KB-P3"],
3392 RDT=[1](min),
3393 TABLE of ( OUTFLOW-STORAGE ) values
3394 (cms) - (ha-m)
3395 [ 0.0 , 0.0 ]

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3396 [0.051,0.002]
3397 [0.048,0.003]
3398 [0.057,0.029]
3399 [0.089,0.045]
3400 [0.133,0.069]
3401 [0.199,0.106]
3402 [0.321,0.172]
3403 [1.029,0.306]
3404 [4.036,0.527]
3405 [8.332,0.761]
3406 [11.727,0.941]
3407 [14.125,1.067]
3408 [15.675,1.149]
3409 [16.555,1.196]
3410 [16.911,1.214]
3411 [ -1 , -1 ] (max twenty pts)
3412 NHYDovf=["KB-P3ovf"]
3413 *%-----|-----|
3414 ADD HYD NHYDsum=["KB-Pond3"], NHYDs to add=["KB-P3R"+"KB-P3ovf"]
3415 *%-----|-----|
3416 SAVE HYD NHYD=["KB-Pond3"], # OF PCYCLES=[-1], ICASEsh=[1]
3417 HYD_COMMENT=["Total Flows at KB third pond"]
3418 *%-----|-----|
3419 *#*****|
3420 *# EXISTING / PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM
Modeling Approach, NOVATECH Report June, 2020)
3421 *# - TO FRASER-CLARKE DRAIN
3422 *#*****|
3423 CONTINUOUS STANDHYD NHYD=["FC-01"], DT=[1]min, AREA=[8.03](ha), XIMP=[0.47],
TIMP=[0.47], DWF=[0](cms), LOSS=[1]:
3424 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3425 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3426 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[231.373](m), MNI=[0.013], SCI=[0](min),
3427 Continuous simulation parameters:
3428 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3429 *%-----|-----|
3430 COMPUTE DUALHYD NHYDin=["FC-01"], CINLET=[0.756](cms), NINLET=[1],
3431 MajNHYD=["FC-01-MJ"]
3432 MinNHYD=["FC-01-MN"]
3433 TMJSTO=[714](cu-m)
3434 *%-----|-----|
3435 ADD HYD NHYDsum=["FC-01-S"], NHYDs to add=["FC-01-MJ"+"FC-01-MN"]
3436 *%-----|-----|
3437 CONTINUOUS STANDHYD NHYD=["FC-02"], DT=[1]min, AREA=[16.05](ha), XIMP=[0.93],
TIMP=[0.93], DWF=[0](cms), LOSS=[1]:
3438 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3439 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3440 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[327.109](m), MNI=[0.013], SCI=[0](min),
3441 Continuous simulation parameters:
3442 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3443 *%-----|-----|
3444 COMPUTE DUALHYD NHYDin=["FC-02"], CINLET=[1.159](cms), NINLET=[1],
3445 MajNHYD=["FC-02-MJ"]
3446 MinNHYD=["FC-02-MN"]
3447 TMJSTO=[2385](cu-m)
3448 *%-----|-----|
3449 ADD HYD NHYDsum=["FC-02-S"], NHYDs to add=["FC-02-MJ"+"FC-02-MN"]
3450 *%-----|-----|

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3451 CONTINUOUS STANDHYD NHYD=["FC-03"], DT=[1]min, AREA=[7.37](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3452 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3453 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3454 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[221.660](m), MNI=[0.013], SCI=[0](min),
3455 Continuous simulation parameters:
3456 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3457 *%-----|-----|
3458 COMPUTE DUALHYD NHYDin=["FC-03"], CINLET=[0.358](cms), NINLET=[1],
3459 MajNHYD=["FC-03-MJ"]
3460 MinNHYD=["FC-03-MN"]
3461 TMJSTO=[1131](cu-m)
3462 *%-----|-----|
3463 ADD HYD NHYDsum=["FC-03-S"], NHYDs to add=["FC-03-MJ"+"FC-03-MN"]
3464 *%-----|-----|
3465 CONTINUOUS STANDHYD NHYD=["FC-04"], DT=[1]min, AREA=[12.87](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3466 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3467 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3468 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[292.916](m), MNI=[0.013], SCI=[0](min),
3469 Continuous simulation parameters:
3470 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3471 *%-----|-----|
3472 COMPUTE DUALHYD NHYDin=["FC-04"], CINLET=[0.741](cms), NINLET=[1],
3473 MajNHYD=["FC-04-MJ"]
3474 MinNHYD=["FC-04-MN"]
3475 TMJSTO=[1794](cu-m)
3476 *%-----|-----|
3477 ADD HYD NHYDsum=["FC-04-S"], NHYDs to add=["FC-04-MJ"+"FC-04-MN"]
3478 *%-----|-----|
3479 *#*****
3480 *# PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM Modeling
Approach, NOVATECH Report June, 2020)
3481 *# - TO JOCK RIVER
3482 *#*****
3483 CONTINUOUS STANDHYD NHYD=["JR-01"], DT=[1]min, AREA=[8.24](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3484 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3485 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3486 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[234.379](m), MNI=[0.013], SCI=[0](min),
3487 Continuous simulation parameters:
3488 IaRECper=[4](hrs), IaRECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3489 *%-----|-----|
3490 COMPUTE DUALHYD NHYDin=["JR-01"], CINLET=[0.563](cms), NINLET=[1],
3491 MajNHYD=["JR-01-MJ"]
3492 MinNHYD=["JR-01-MN"]
3493 TMJSTO=[1040](cu-m)
3494 *%-----|-----|
3495 ADD HYD NHYDsum=["JR-01-S"], NHYDs to add=["JR-01-MJ"+"JR-01-MN"]
3496 *%-----|-----|
3497 CONTINUOUS STANDHYD NHYD=["JR-02"], DT=[1]min, AREA=[1.59](ha), XIMP=[0.64],
TIMP=[0.64], DWF=[0](cms), LOSS=[1]:
3498 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),

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3499          Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3500          MNP=[0.250], SCP=[0](min),
3501          Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
3502          LGI=[102.956](m), MNI=[0.013], SCI=[0](min),
3503          Continuous simulation parameters:
3504          IaREcper=[4](hrs), IaREcimp=[4](hrs), InterEventTime=[12](hrs),
3505          END=-1
3506
3507 *%-----|-----|
3508 COMPUTE DUALHYD NHYDin=["JR-02"], CINLET=[0.153](cms), NINLET=[1],
3509          MajNHYD=["JR-02-MJ"]
3510          MinNHYD=["JR-02-MN"]
3511          TMJSTO=[153](cu-m)
3512
3513 *%-----|-----|
3514 ADD HYD NHYDsum=["JR-02-S"], NHYDs to add=["JR-02-MJ"+"JR-02-MN"]
3515
3516 *%-----|-----|
3517 *#*****|*****|
3518 *# Catchment FRASER
3519 *# - To Fraser-Clarke drain (north of the Jock)
3520 *# - Developed land with assumed 43% imp.
3521 *# - 2020-12-17 Change Fraser area to be 35.1 as measured from QGIS
3522 *# - 2020-12-17 All Fraser is undeveloped (Nashyd)
3523 *#*****|*****|
3524 CONTINUOUS NASHYD NHYD=["FRASER-DRN"], DT=[1]min, AREA=[13.65](ha),
3525          DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3526          N=[3], TP=[0.4258]hrs,
3527          Continuous simulation parameters:
3528          IaREcper=[4](hrs),
3529          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3530          InterEventTime=[12](hrs)
3531          Baseflow simulation parameters:
3532          BaseFlowOption=[1],
3533          InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3534          VHydCond=[0.055](mm/hr), END=-1
3535
3536 *
3537 CONTINUOUS STANDHYD NHYD=["FRASER-D"], DT=[1]min, AREA=[21.61](ha),
3538          XIMP=[0.585], TIMP=[0.585], DWF=[0](cms), LOSS=[2],
3539          SCS curve number CN=[80],
3540          Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3541          LGP=[40](m), MNP=[0.25], SCP=[0](min),
3542          Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3543          LGI=[379.561](m), MNI=[0.013], SCI=[0](min),
3544          Continuous simulation parameters:
3545          IaREcper=[4](hrs), IaREcimp=[4](hrs),
3546          SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3547          InterEventTime=[18](hrs), END=-1
3548
3549 *%-----|-----|
3550 COMPUTE DUALHYD NHYDin=["FRASER-D"], CINLET=[2.281](cms), NINLET=[1],
3551          MajNHYD=["FRASER-J"]
3552          MinNHYD=["FRASER-N"]
3553          TMJSTO=[9999999](cu-m)
3554
3555 *%-----|-----|
3556 ADD HYD NHYDsum=["FRASER-S"], NHYDs to add=["FRASER-J"+"FRASER-N"]
3557
3558 *%-----|-----|
3559 *ROUTE RESERVOIR NHYDout=["MS_P20"], NHYDin=["FRASER"],
3560          *
3561          RDT=[1](min),
3562          *
3563          TABLE of ( OUTFLOW-STORAGE ) values
3564          *
3565          (cms) - (ha-m)
3566          *
3567          [ 0.0 , 0.0 ]
3568          *
3569          [ 0.04 , 0.36 ]
3570          *
3571          [ -1 , -1 ] (max twenty pts)
3572          *
3573          NHYDovf=["P20-OVF"]
3574
3575 *%-----|-----|
3576 ADD HYD NHYDsum=["4241"], NHYDs to
3577          add=["KB-Pond3"+"S-1-B"+"FRASER-DRN"+"FRASER-S"+"N_KB"+"FC-01-S"+"FC-02-S"+"FC-03-S"]
3578
3579 *%-----|-----|
3580 SAVE HYD NHYD=["4241"], # OF PCYCLES=[-1], ICASEsh=[1]

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3561 HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
3562 *%-----|-----|
3563 *# Hydrograph from Node Ken-Burnett to station 3633
3564 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4241
3565 *#
3566 ROUTE CHANNEL NHYDout=["4241-out"], NHYDin=["4241"], RDT=[1](min),
3567 CHLGTH=[294](m), CHSLOPE=[0.1088](%), FPSLOPE=[0.1088](%),
3568 SECNUM=[1.0], NSEG=[3]
3569 ( SEGROUGH, SEGDIST (m))=[0.05, -20.12
3570 -0.035, 45.26
3571 0.05, 403.84] NSEG times
3572 ( DISTANCE (m), ELEVATION (m))=[]
3573 [-909.72, 95 ]
3574 [-907.09, 94.5 ]
3575 [-904.65, 94 ]
3576 [-902.26, 93.5 ]
3577 [-44.51, 91.5 ]
3578 [-25.1, 91.5 ]
3579 [-20.98, 91 ]
3580 [-20.61, 90.5 ]
3581 [-20.12, 90 ]
3582 [-6.13, 87.26 ]
3583 [17.51, 86.56 ]
3584 [31.37, 87.2 ]
3585 [45.26, 90 ]
3586 [50.41, 90.5 ]
3587 [63.06, 91 ]
3588 [134.5, 91.5 ]
3589 [190.63, 92 ]
3590 [251.98, 92.5 ]
3591 [321.32, 93.5 ]
3592 [403.84, 95 ]
3593 *%-----|-----|
3594 ADD HYD NHYDsum=["SN_KB"], NHYDs to
add=["4241-out"+"FC-04-S"+"JR-01-S"+"JR-02-S"]
3595 *%-----|-----|
3596 SAVE HYD NHYD=["SN_KB"], # OF PCYCLES=[-1], ICASEsh=[1]
3597 HYD_COMMENT=["Total Flows before Station 3633"]
3598 *%-----|-----|
3599 *# Hydrograph from Station 3633 to Node Todd
3600 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
3601 *# JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and
change the slope from 0.0498% to 0.24671%. That is because of adding station 4241
between station 4534 and station 3633
3602 *#
3603 ROUTE CHANNEL NHYDout=["N_TO"], NHYDin=["SN_KB"], RDT=[1](min),
3604 CHLGTH=[608](m), CHSLOPE=[0.24671](%), FPSLOPE=[0.24671](%),
3605 SECNUM=[1.0], NSEG=[3]
3606 ( SEGROUGH, SEGDIST (m))=[0.05, -23.74
3607 -0.035, 23.74
3608 0.05, 26.50] NSEG times
3609 ( DISTANCE (m), ELEVATION (m))=[]
3610 -29.24, 91.0
3611 -27.41, 90.5
3612 -25.64, 90
3613 -23.74, 89.5
3614 -22, 89.26
3615 -20, 88.51
3616 -19, 88.32
3617 -15, 88.1
3618 -10, 88.11
3619 -5, 88.17
3620 0, 88.27
3621 5, 88.19
3622 10, 88.06
3623 15, 88.48

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3624      16, 88.7
3625      23.74, 89.5
3626      24.68, 90
3627      25.57, 90.5
3628      26.50, 91.0
3629      *                [-29.24, 91]
3630      *                [-27.41, 90.5]
3631      *                [-25.64, 90]
3632      *                [-23.74, 89.5]
3633      *                [-22, 89.26]
3634      *                [-20, 88.51]
3635      *                [-19, 88.32]
3636      *                [-15, 88.1]
3637      *                [-10, 88.11]
3638      *                [-5, 88.17]
3639      *                [0, 88.27]
3640      *                [5, 88.19]
3641      *                [10, 88.06]
3642      *                [15, 88.48]
3643      *                [16, 88.7]
3644      *                [23.74, 89.5]
3645      *                [24.68, 90]
3646      *                [25.57, 90.5]
3647      *%-----|-----|
3648      *#*****|
3649      *#   Catchment Greenbank
3650      *#   - To Greenbank Drain (south of the Jock)
3651      *#   - JFSA 2021-01-18 add Greenbank pond as per JFSA, P598(06)-15, June 2016
3652      *#   - JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
3653      *#*****|
3654      CONTINUOUS STANDHYD NHYD=["Greenbank"], DT=[1]min, AREA=[36.6](ha),
3655      XIMP=[0.639], TIMP=[0.682], DWF=[0](cms), LOSS=[2],
3656      SCS curve number CN=[77],
3657      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3658      LGP=[40](m), MNP=[0.25], SCP=[0](min),
3659      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3660      LGI=[493.96](m), MNI=[0.013], SCI=[0](min),
3661      Continuous simulation parameters:
3662      IaREcper=[4](hrs), IaREcimp=[4](hrs),
3663      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3664      InterEventTime=[18](hrs), END=-1
3665      *%-----|-----|
3666      ROUTE RESERVOIR NHYDout=["GreenB_MN"], NHYDin=["Greenbank"],
3667      RDT=[1](min),
3668      TABLE of ( OUTFLOW-STORAGE ) values
3669      (cms) - (ha-m)
3670      [ 0.0 , 0.0 ]
3671      [ 0.033 , 0.084 ]
3672      [ 0.039 , 0.201 ]
3673      [ 0.113 , 0.292 ]
3674      [ 0.237 , 0.386 ]
3675      [ 0.382 , 0.484 ]
3676      [ 0.539 , 0.585 ]
3677      [ 0.7 , 0.692 ]
3678      [ 0.86 , 0.804 ]
3679      [ 4.684 , 0.922 ]
3680      [ 11.539 , 1.052 ]
3681      [ 20.867 , 1.168 ]
3682      [ 103.616 , 1.974 ]
3683      [ -1 , -1 ] (max twenty pts)
3684      NHYDovf=["GreenB_MJ"],
3685      *%-----|-----|
3686      *%-----|-----|
3687      ADD HYD NHYDsum=["GreenB"], NHYDs to add=["N_TO"+"GreenB_MJ"+"GreenB_MN"]
3688      *%-----|-----|
3689      SAVE HYD NHYD=["GreenB"], # OF PCYCLES=[-1], ICASEsh=[1]

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3690 HYD_COMMENT=["Total Flows at Greenbank Drain"]
3691 *%-----|-----|
3692 *#*****|
3693 *# Catchment TODD
3694 *# - To Todd Drain (south of the Jock)
3695 *# - Subdivision with 43% imp. as per Barrhaven South MSS
3696 *# - 2020-11-30 increase imp. based on P598(04)-11
3697 *# - 2020-11-30 update TODD Tributary Drainage Area to = 146.015 ha based on
P598(04)-11
3698 *# - 2020-11-30 split TODD Drainage Area to MAJOR, MINOR, POND and ALL
3699 *#*****|
3700 *# - JFSA 2021-01-19 add "TODD_MN1" as part of Clarke("W_CLAR_MJ") and remove it
from Todd
3701 *CONTINUOUS STANDHYD NHYD=["TODD_MN1"], DT=[1]min, AREA=[1.772](ha),
3702 * XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3703 * SCS curve number CN=[77],
3704 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3705 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3706 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3707 * LGI=[108.689](m), MNI=[0.013], SCI=[0](min),
3708 * Continuous simulation parameters:
3709 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
3710 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3711 * InterEventTime=[18](hrs), END=-1
3712 *%-----|-----|
3713 CONTINUOUS STANDHYD NHYD=["TODD_MN2"], DT=[1]min, AREA=[2.1](ha),
3714 XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3715 SCS curve number CN=[77],
3716 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3717 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3718 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3719 LGI=[118.322](m), MNI=[0.013], SCI=[0](min),
3720 Continuous simulation parameters:
3721 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3722 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3723 InterEventTime=[18](hrs), END=-1
3724 *%-----|-----|
3725 CONTINUOUS STANDHYD NHYD=["TODD_MN3"], DT=[1]min, AREA=[0.117](ha),
3726 XIMP=[0.53], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3727 SCS curve number CN=[77],
3728 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3729 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3730 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3731 LGI=[27.928](m), MNI=[0.013], SCI=[0](min),
3732 Continuous simulation parameters:
3733 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3734 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3735 InterEventTime=[18](hrs), END=-1
3736 *%-----|-----|
3737 CONTINUOUS STANDHYD NHYD=["TODD_MJ"], DT=[1]min, AREA=[30.230](ha),
3738 XIMP=[0.52], TIMP=[0.64], DWF=[0](cms), LOSS=[2],
3739 SCS curve number CN=[77],
3740 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3741 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3742 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3743 LGI=[448.925](m), MNI=[0.013], SCI=[0](min),
3744 Continuous simulation parameters:
3745 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3746 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3747 InterEventTime=[18](hrs), END=-1
3748 *%-----|-----|
3749 * -JFSA, 2021-01-19 update "TODD_ALL" area from 108.741 ha to 112.908 ha based on
GIS measurements (148.41-30.23-0.117-2.1-3.055=112.908 ha)
3750 CONTINUOUS STANDHYD NHYD=["TODD_ALL"], DT=[1]min, AREA=[112.908](ha),
3751 XIMP=[0.52], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
3752 SCS curve number CN=[77],

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3753 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3754 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3755 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3756 LGI=[867.594](m), MNI=[0.013], SCI=[0](min),
3757 Continuous simulation parameters:
3758 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3759 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3760 InterEventTime=[18](hrs), END=-1
3761 *%-----|-----
3762 CONTINUOUS STANDHYD NHYD=["TODD_P"], DT=[1]min, AREA=[3.055](ha),
3763 XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3764 SCS curve number CN=[77],
3765 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3766 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3767 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3768 LGI=[142.712](m), MNI=[0.013], SCI=[0](min),
3769 Continuous simulation parameters:
3770 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3771 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3772 InterEventTime=[18](hrs), END=-1
3773 *%-----|-----
3774 *%-----|-----
3775 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3776 *CONTINUOUS STANDHYD NHYD=["TODD_DEVL"], DT=[1]min, AREA=[15.87](ha),
3777 * XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3778 * SCS curve number CN=[77],
3779 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3780 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3781 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3782 * LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
3783 * Continuous simulation parameters:
3784 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
3785 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3786 * InterEventTime=[18](hrs), END=-1
3787 *%-----|-----
3788 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3789 *CONTINUOUS NASHYD NHYD=["TODD_UnD"], DT=[1]min, AREA=[12.47](ha),
3790 * DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3791 * N=[3], TP=[1.10]hrs,
3792 * Continuous simulation parameters:
3793 * IaRECper=[4](hrs),
3794 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3795 * InterEventTime=[12](hrs)
3796 * Baseflow simulation parameters:
3797 * BaseFlowOption=[1] ,
3798 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3799 * VHydCond=[0.055](mm/hr), END=-1
3800 *%-----|-----
3801 *# 5-Year + 12% Capture
3802 *COMPUTE DUALHYD NHYDin=["TODD_MJ"], CINLET=[3.314](cms), NINLET=[1],
3803 * MajNHYD=["TODD_MJj"]
3804 * MinNHYD=["TODD_MJn"]
3805 * TMJSTO=[0.1](cu-m)
3806 ROUTE RESERVOIR NHYDout=["TODD_MJn"],NHYDin=["TODD_MJ"],
3807 RDT=[1](min),
3808 TABLE of ( OUTFLOW-STORAGE ) values
3809 (cms) - (ha-m)
3810 [ 0.0 , 0.0 ]
3811 [ 3.314 , 0.0001 ]
3812 [ -1 , -1 ] (max twenty pts)
3813 NHYDovf=["TODD_MJj"],
3814 *%-----|-----
3815 *# 5-Year + 12% Capture

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3816 *COMPUTE DUALHYD      NHYDin=["TODD_MN1"], CINLET=[0.227](cms), NINLET=[1],
3817 *                      MajNHYD=["TODD_MN1j"]
3818 *                      MinNHYD=["TODD_MN1n"]
3819 *                      TMJSTO=[0.1](cu-m)
3820 *ROUTE RESERVOIR      NHYDout=["TODD_MN1n"], NHYDin=["TODD_MN1"],
3821 *                      RDT=[1](min),
3822 *                      TABLE of ( OUTFLOW-STORAGE ) values
3823 *                      (cms) - (ha-m)
3824 *                      [ 0.0 , 0.0 ]
3825 *                      [ 0.227 , 0.0001 ]
3826 *                      [ -1 , -1 ] (max twenty pts)
3827 *                      NHYDovf=["TODD_MN1j"],
3828 *%-----|-----|
3829 *COMPUTE DUALHYD      NHYDin=["TODD_MN2"], CINLET=[0.268](cms), NINLET=[1],
3830 *                      MajNHYD=["TODD_MN2j"]
3831 *                      MinNHYD=["TODD_MN2n"]
3832 *                      TMJSTO=[0.1](cu-m)
3833 ROUTE RESERVOIR      NHYDout=["TODD_MN2n"], NHYDin=["TODD_MN2"],
3834 *                      RDT=[1](min),
3835 *                      TABLE of ( OUTFLOW-STORAGE ) values
3836 *                      (cms) - (ha-m)
3837 *                      [ 0.0 , 0.0 ]
3838 *                      [ 0.268 , 0.0001 ]
3839 *                      [ -1 , -1 ] (max twenty pts)
3840 *                      NHYDovf=["TODD_MN2j"],
3841 *%-----|-----|
3842 *COMPUTE DUALHYD      NHYDin=["TODD_MN3"], CINLET=[0.016](cms), NINLET=[1],
3843 *                      MajNHYD=["TODD_MN3j"]
3844 *                      MinNHYD=["TODD_MN3n"]
3845 *                      TMJSTO=[0.1](cu-m)
3846 ROUTE RESERVOIR      NHYDout=["TODD_MN3n"], NHYDin=["TODD_MN3"],
3847 *                      RDT=[1](min),
3848 *                      TABLE of ( OUTFLOW-STORAGE ) values
3849 *                      (cms) - (ha-m)
3850 *                      [ 0.0 , 0.0 ]
3851 *                      [ 0.016 , 0.0001 ]
3852 *                      [ -1 , -1 ] (max twenty pts)
3853 *                      NHYDovf=["TODD_MN3j"],
3854 *%-----|-----|
3855 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
3856 CONTINUOUS STANDHYD  NHYD=["A2"], DT=[1]min, AREA=[25.5](ha),
3857 *                      XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
3858 *                      SCS curve number CN=[75],
3859 *                      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3860 *                      LGP=[40](m), MNP=[0.25], SCP=[0](min),
3861 *                      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3862 *                      LGI=[566](m), MNI=[0.013], SCI=[0](min),
3863 *                      Continuous simulation parameters:
3864 *                      IaREcper=[4](hrs), IaREcimp=[4](hrs),
3865 *                      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3866 *                      InterEventTime=[18](hrs), END=-1
3867 *%-----|-----|
3868 COMPUTE DUALHYD      NHYDin=["A2"], CINLET=[1.818](cms), NINLET=[1],
3869 *                      MajNHYD=["A2-MJ"]
3870 *                      MinNHYD=["A2-MN"]
3871 *                      TMJSTO=[924](cu-m)
3872 *%-----|-----|
3873 ADD HYD              NHYDsum=["TODD"], NHYDs to
add=["TODD_MN2n"+"TODD_MN3n"+"TODD_MJj"+"TODD_P"+"TODD_ALL"+"W_CLAR_MJn"]
3874 *%-----|-----|
3875 SAVE HYD            NHYD=["TODD"], # OF PCYCLES=[-1], ICASEsh=[1]
3876 *                      HYD_COMMENT=["Total Flows at Todd Drain"]
3877 *%-----|-----|
3878 *#*****
3879 *# Todd Pond 3

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3880 *# - Rating curve obtained from Barrhaven South MSS modeling
3881 *# - stantec 2007, Tributary Drainage Area to MSS Pond 3 = 193 ha
3882 *#*****
3883 ROUTE RESERVOIR      NHYDout=["MS_P3"],  NHYDin=["TODD"],
3884                      RDT=[1](min),
3885                      TABLE of ( OUTFLOW-STORAGE ) values
3886                      (cms) - (ha-m)
3887                      [ 0.0 , 0.0 ]
3888                      [ 0.014 , 0.155 ]
3889                      [ 0.048 , 0.394 ]
3890                      [ 0.061 , 0.56 ]
3891                      [ 0.08 , 0.909 ]
3892                      [ 0.088 , 1.089 ]
3893                      [ 0.109 , 1.652 ]
3894                      [ 0.118 , 1.952 ]
3895                      [ 0.122 , 2.099 ]
3896                      [ 1.972 , 2.269 ]
3897                      [ 9.135 , 2.598 ]
3898                      [ 15.608 , 2.826 ]
3899                      [ 19.256 , 2.942 ]
3900                      [ 27.282 , 3.181 ]
3901                      [ 40.957 , 3.55 ]
3902                      [ 56.372 , 3.929 ]
3903                      [ 73.349 , 4.317 ]
3904                      [ 85.469 , 4.579 ]
3905                      [ 104.771 , 4.977 ]
3906                      [ -1 , -1 ] (max twenty pts)
3907                      NHYDovf=["P3-OVF"]
3908 *%-----|-----
3909 ADD HYD          NHYDsum=["SN_TO"], NHYDs to
add=["GreenB"+"MS_P3"+"P3-OVF"+"TODD_MN2j"+"A2-MJ"]
3910 *%-----|-----
3911 SAVE HYD        NHYD=["SN_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3912                HYD_COMMENT=["Total Flows at Todd Drain"]
3913 *%-----|-----
3914 *#
3915 *# Hydrograph from Todd Drain routed to Corrigan Drain
3916 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
3917 *# 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so
the model will be more stable and give reasonable results. It is justifiable as ROUTE
CHANNELs aren't well suited to really flat slopes.
3918 *
3919 ROUTE CHANNEL    NHYDout=["N_TO"] ,NHYDin=["SN_TO"] ,
3920                RDT=[1](min),
3921                CHLGTH=[280](m),  CHSLOPE=[0.05](%),
3922                FPSLOPE=[0.05](%),
3923                SECNUM=[1.0],      NSEG=[3]
3924                ( SEGROUGH, SEGDIST (m))=
3925                [0.075,-17.72
3926                -0.045,17.72
3927                0.075,80.62] NSEG times
3928                ( DISTANCE (m), ELEVATION (m))=
3929                [-83.32, 90.00]
3930                [-81.36, 89.50]
3931                [-79.12, 89.00]
3932                [-76.13, 88.50]
3933                [-20.46, 88.00]
3934                [-19.36, 87.50]
3935                [-18.51, 87.00]
3936                [-17.72, 86.50]
3937                [-11.95, 85.24]
3938                [-0.11, 85.12]
3939                [11.49, 85.20]
3940                [17.72, 86.50]
3941                [19.74, 87.00]
3942                [21.22, 87.50]

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3943 [22.68, 88.00]
3944 [24.28, 88.50]
3945 [26.79, 89.00]
3946 [71.98, 90.00]
3947 [80.62, 90.50]
3948 *%-----|-----|
3949 SAVE HYD NHYD=["N_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3950 HYD_COMMENT=["Total inflows at Station 2462"]
3951 *%-----|-----|
3952 *#*****|*****|
3953 *# Catchment CORRIG
3954 *# - To Corrigan Drain (south of the Jock)
3955 *# - Primarily Developed (medium density)
3956 *# - JFSA JAN 2021, add Corrigan subcatchments as per IBI, July 2008
3957 *#*****|*****|
3958 *ROUTE RESERVOIR NHYDout=["MS_P1"], NHYDin=["CORRIG"],
3959 * RDT=[1](min),
3960 * TABLE of ( OUTFLOW-STORAGE ) values
3961 * (cms) - (ha-m)
3962 * [ 0.0 , 0.0 ]
3963 * [ 0.06 , 0.58]
3964 * [ -1 , -1 ] (max twenty pts)
3965 * NHYDovf=["P1-OVF"]
3966 *%-----|-----|
3967 *ADD HYD NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
3968 *%-----|-----|
3969 *SAVE HYD NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
3970 * HYD_COMMENT=["Total Flows at Corrigan Drain"]
3971 *%-----|-----|
3972 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3973 CONTINUOUS STANDHYD NHYD=["corr1"], DT=[1]min, AREA=[15.87](ha),
3974 XIMP=[0.63], TIMP=[0.63], DWF=[0](cms), LOSS=[2],
3975 SCS curve number CN=[77],
3976 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3977 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3978 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3979 LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
3980 Continuous simulation parameters:
3981 IaRECper=[4](hrs), IaRECimp=[4](hrs),
3982 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3983 InterEventTime=[18](hrs), END=-1
3984 *%-----|-----|
3985 * -JFSA 2021-02-23 add DUALHYD for "corr1". "corr1" DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to "corr1".
3986 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
3987 COMPUTE DUALHYD NHYDin=["corr1"], CINLET=[1.818](cms), NINLET=[1],
3988 MajNHYD=["corr1-MJ"]
3989 MinNHYD=["corr1-MN"]
3990 TMJSTO=[924](cu-m)
3991 *%-----|-----|
3992 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3993 CONTINUOUS NASHHYD NHYD=["corr2"], DT=[1]min, AREA=[12.47](ha),
3994 DWF=[0](cms), CN/C=[77], IA=[4.67](mm),
3995 N=[3], TP=[1.10]hrs,
3996 Continuous simulation parameters:
3997 IaRECper=[4](hrs),
3998 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3999 InterEventTime=[12](hrs)
4000 Baseflow simulation parameters:
4001 BaseFlowOption=[1] ,
4002 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4003 VHydCond=[0.055](mm/hr), END=-1

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4004 *%-----|-----|
4005 * -JFSA 2021-01-19 change A1-Corrig to be developed as per geottawa website and
apply the parameters of A2, the nearest sub-catchment to A1-Corrig, LGI is calculated
based on A1-Corrig area
4006 * -JFSA 2021-01-19 update all Corrigan areas based on GIS measurements, and keep
LGI as it is from Corrigan Report, IBI Group, 2008 because LGI calculated is less than
LGI from the Corrigan Report
4007 CONTINUOUS STANDHYD NHYD=["A1-Corrig"], DT=[1]min, AREA=[15.75](ha),
4008 XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
4009 SCS curve number CN=[75],
4010 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4011 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4012 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4013 LGI=[324.037](m), MNI=[0.013], SCI=[0](min),
4014 Continuous simulation parameters:
4015 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4016 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4017 InterEventTime=[18](hrs), END=-1
4018 *
4019 * -JFSA 2021-01-25 add DUALHYD for A1-Corrig. A1-Corrig DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to A1-Corrig.
4020 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
A1-Corrig
4021 COMPUTE DUALHYD NHYDin=["A1-Corrig"], CINLET=[1.818](cms), NINLET=[1],
4022 MajNHYD=["A1-MJ"]
4023 MinNHYD=["A1-MN"]
4024 TMJSTO=[924](cu-m)
4025 *%-----|-----|
4026 *CONTINUOUS NASHYD NHYD=["A1-Corrig"], DT=[1]min, AREA=[15.75](ha),
4027 DWF=[0](cms), CN/C=[66], IA=[2.5](mm),
4028 N=[3.0], TP=[0.36]hrs,
4029 * Continuous simulation parameters:
4030 IaRECper=[4](hrs),
4031 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4032 InterEventTime=[12](hrs)
4033 * Baseflow simulation parameters:
4034 BaseFlowOption=[1] ,
4035 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4036 VHydCond=[0.055](mm/hr), END=-1
4037 *%-----|-----|
4038 CONTINUOUS NASHYD NHYD=["B1"], DT=[1]min, AREA=[2.77](ha),
4039 DWF=[0](cms), CN/C=[56], IA=[2.5](mm),
4040 N=[3.0], TP=[0.23]hrs,
4041 Continuous simulation parameters:
4042 IaRECper=[4](hrs),
4043 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4044 InterEventTime=[12](hrs)
4045 Baseflow simulation parameters:
4046 BaseFlowOption=[1] ,
4047 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4048 VHydCond=[0.055](mm/hr), END=-1
4049 *%-----|-----|
4050 CONTINUOUS STANDHYD NHYD=["A4"], DT=[1]min, AREA=[1.27](ha),
4051 XIMP=[0.65], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
4052 SCS curve number CN=[75],
4053 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4054 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4055 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4056 LGI=[253](m), MNI=[0.013], SCI=[0](min),
4057 Continuous simulation parameters:
4058 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4059 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4060 InterEventTime=[18](hrs), END=-1
4061 *%-----|-----|
4062 COMPUTE DUALHYD NHYDin=["A4"], CINLET=[0.405](cms), NINLET=[1],
4063 MajNHYD=["A4-MJ"]

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4064 MinNHYD=["A4-MN"]
4065 TMJSTO=[68](cu-m)
4066 *%-----|-----|
4067 ADD HYD NHYDsum=["MH101"], NHYDs to
add=["A1-MJ"+"A1-MN"+"corr1-MJ"+"corr1-MN"+"corr2"+"B1"+"A4-MN"]
4068 *%-----|-----|
4069 SAVE HYD NHYD=["MH101"], # OF PCYCLES=[-1], ICASEsh=[1]
4070 HYD_COMMENT=["Total Flows at MH101"]
4071 *%-----|-----|
4072 ROUTE PIPE PTYPE=[1]circ, NHYDout=["101-102"], RNUMBER=[1.0], PDIAM=[1050](mm),
4073 PLNGTH=[368](m), PROUGH=[0.013], PSLOPE=[0.0054](m/m),
NHYDin=["MH101"], RDT=[1]
4074 *%-----|-----|
4075 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
4076 *CONTINUOUS STANDHYD NHYD=["A2"], DT=[1]min, AREA=[25.5](ha),
4077 * XIMP=[0.42], TIMP=[0.52], DWF=[0](cms), LOSS=[2],
4078 * SCS curve number CN=[75],
4079 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4080 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
4081 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4082 * LGI=[566](m), MNI=[0.013], SCI=[0](min),
4083 * Continuous simulation parameters:
4084 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
4085 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4086 * InterEventTime=[18](hrs), END=-1
4087 *%-----|-----|
4088 *COMPUTE DUALHYD NHYDin=["A2"], CINLET=[1.818](cms), NINLET=[1],
4089 * MajNHYD=["A2-MJ"]
4090 * MinNHYD=["A2-MN"]
4091 * TMJSTO=[924](cu-m)
4092 *%-----|-----|
4093 ADD HYD NHYDsum=["MH102"], NHYDs to add=["A2-MN"+"101-102"]
4094 *%-----|-----|
4095 SAVE HYD NHYD=["MH102"], # OF PCYCLES=[-1], ICASEsh=[1]
4096 HYD_COMMENT=["Total Flows at MH102"]
4097 *%-----|-----|
4098 CONTINUOUS STANDHYD NHYD=["A5"], DT=[1]min, AREA=[1.6](ha),
4099 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4100 SCS curve number CN=[75],
4101 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4102 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4103 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4104 LGI=[300](m), MNI=[0.013], SCI=[0](min),
4105 Continuous simulation parameters:
4106 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4107 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4108 InterEventTime=[18](hrs), END=-1
4109 *%-----|-----|
4110 ADD HYD NHYDsum=["A5T"], NHYDs to add=["A4-MJ"+"A5"]
4111 *%-----|-----|
4112 COMPUTE DUALHYD NHYDin=["A5T"], CINLET=[0.357](cms), NINLET=[1],
4113 MajNHYD=["A5-MJ"]
4114 MinNHYD=["A5-MN"]
4115 TMJSTO=[60](cu-m)
4116 *%-----|-----|
4117 * -JFSA Jan. 2021, A3 is a part of Todd so it is removed
4118 * -JFSA Jan. 2021, "A2-MJ" added to "Todd"
4119 *CONTINUOUS STANDHYD NHYD=["A3"], DT=[1]min, AREA=[18.4](ha),
4120 * XIMP=[0.58], TIMP=[0.65], DWF=[0](cms), LOSS=[2],
4121 * SCS curve number CN=[75],
4122 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4123 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
4124 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4125 * LGI=[450](m), MNI=[0.013], SCI=[0](min),
4126 * Continuous simulation parameters:

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4127 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
4128 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4129 * InterEventTime=[18](hrs), END=-1
4130 *%-----|-----|
4131 *ADD HYD NHYDsum=["A3-A2MJ"], NHYDs to add=["A2-MJ"+"A3"]
4132 *%-----|-----|
4133 *COMPUTE DUALHYD NHYDin=["A3-A2MJ"], CINLET=[2.208](cms), NINLET=[1],
4134 * MajNHYD=["A3R-MJ"]
4135 * MinNHYD=["A3R-MN"]
4136 * TMJSTO=[908](cu-m)
4137 *%-----|-----|
4138 ROUTE PIPE PTYPE=[1]circ, NHYDout=["102-103"], RNUMBER=[1.0], PDIAM=[1500](mm),
4139 PLNGTH=[504](m), PROUGH=[0.013], PSLOPE=[0.0028](m/m),
NHYDin=["MH102"], RDT=[1]
4140 *%-----|-----|
4141 ADD HYD NHYDsum=["MH103"], NHYDs to add=["102-103"+"A5-MN"]
4142 *%-----|-----|
4143 SAVE HYD NHYD=["MH103"], # OF PCYCLES=[-1], ICASEsh=[1]
4144 HYD_COMMENT=["Total Flows at MH103"]
4145 *%-----|-----|
4146 ROUTE PIPE PTYPE=[1]circ, NHYDout=["103-104"], RNUMBER=[1.0], PDIAM=[1650](mm),
4147 PLNGTH=[438](m), PROUGH=[0.013], PSLOPE=[0.0046](m/m),
NHYDin=["MH103"], RDT=[1]
4148 *%-----|-----|
4149 CONTINUOUS STANDHYD NHYD=["A6"], DT=[1]min, AREA=[1.56](ha),
4150 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4151 SCS curve number CN=[75],
4152 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4153 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4154 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4155 LGI=[280](m), MNI=[0.013], SCI=[0](min),
4156 Continuous simulation parameters:
4157 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4158 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4159 InterEventTime=[18](hrs), END=-1
4160 *%-----|-----|
4161 ADD HYD NHYDsum=["A6T"], NHYDs to add=["A5-MJ"+"A6"]
4162 *%-----|-----|
4163 COMPUTE DUALHYD NHYDin=["A6T"], CINLET=[0.357](cms), NINLET=[1],
4164 MajNHYD=["A6-MJ"]
4165 MinNHYD=["A6-MN"]
4166 TMJSTO=[60](cu-m)
4167 *%-----|-----|
4168 * -JFSA Jan. 2021, A7-corrig is a part of Todd so it is removed
4169 *CONTINUOUS STANDHYD NHYD=["A7-corrig"], DT=[1]min, AREA=[11.8](ha),
4170 * XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4171 * SCS curve number CN=[75],
4172 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4173 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
4174 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4175 * LGI=[438](m), MNI=[0.013], SCI=[0](min),
4176 * Continuous simulation parameters:
4177 * IaRECper=[4](hrs), IaRECimp=[4](hrs),
4178 * SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4179 * InterEventTime=[18](hrs), END=-1
4180 *%-----|-----|
4181 *ADD HYD NHYDsum=["A7-A3RMJ"], NHYDs to add=["A3R-MJ"+"A7-corrig"]
4182 *%-----|-----|
4183 *COMPUTE DUALHYD NHYDin=["A7-A3RMJ"], CINLET=[1.003](cms), NINLET=[1],
4184 * MajNHYD=["A7R-MJ"]
4185 * MinNHYD=["A7R-MN"]
4186 * TMJSTO=[496](cu-m)
4187 *%-----|-----|
4188 ADD HYD NHYDsum=["MH104"], NHYDs to add=["A6-MN"+"103-104"+"TODD_MJn"]
4189 *%-----|-----|
4190 SAVE HYD NHYD=["MH104"], # OF PCYCLES=[-1], ICASEsh=[1]

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4191 HYD_COMMENT=["Total Flows at MH104"]
4192 *%-----|-----|
4193 CONTINUOUS STANDHYD NHYD=["B2"], DT=[1]min, AREA=[12.31](ha),
4194 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4195 SCS curve number CN=[75],
4196 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4197 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4198 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4199 LGI=[417](m), MNI=[0.013], SCI=[0](min),
4200 Continuous simulation parameters:
4201 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4202 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4203 InterEventTime=[18](hrs), END=-1
4204 *%-----|-----|
4205 COMPUTE DUALHYD NHYDin=["B2"], CINLET=[1.029](cms), NINLET=[1],
4206 MajNHYD=["B2-MJ"]
4207 MinNHYD=["B2-MN"]
4208 TMJSTO=[508](cu-m)
4209 *%-----|-----|
4210 ROUTE PIPE PTYPE=[1]circ, NHYDout=["315-333"], RNUMBER=[1.0], PDIAM=[1200](mm),
4211 PLNGTH=[254](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["B2-MN"], RDT=[1]
4212 *%-----|-----|
4213 CONTINUOUS STANDHYD NHYD=["B3"], DT=[1]min, AREA=[5.59](ha),
4214 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4215 SCS curve number CN=[75],
4216 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4217 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4218 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4219 LGI=[345](m), MNI=[0.013], SCI=[0](min),
4220 Continuous simulation parameters:
4221 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4222 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4223 InterEventTime=[18](hrs), END=-1
4224 *%-----|-----|
4225 COMPUTE DUALHYD NHYDin=["B3"], CINLET=[0.459](cms), NINLET=[1],
4226 MajNHYD=["B3-MJ"]
4227 MinNHYD=["B3-MN"]
4228 TMJSTO=[227](cu-m)
4229 *%-----|-----|
4230 ADD HYD NHYDsum=["MH333"], NHYDs to add=["B3-MN"+"315-333"]
4231 *%-----|-----|
4232 SAVE HYD NHYD=["MH333"], # OF PCYCLES=[-1], ICASEsh=[1]
4233 HYD_COMMENT=["Total Flows at MH333"]
4234 *%-----|-----|
4235 ROUTE PIPE PTYPE=[1]circ, NHYDout=["333-335"], RNUMBER=[1.0], PDIAM=[1200](mm),
4236 PLNGTH=[251](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH333"], RDT=[1]
4237 *%-----|-----|
4238 ROUTE PIPE PTYPE=[1]circ, NHYDout=["335-338"], RNUMBER=[1.0], PDIAM=[1200](mm),
4239 PLNGTH=[185](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["333-335"], RDT=[1]
4240 *%-----|-----|
4241 ROUTE PIPE PTYPE=[1]circ, NHYDout=["338-340"], RNUMBER=[1.0], PDIAM=[1350](mm),
4242 PLNGTH=[233](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["335-338"], RDT=[1]
4243 *%-----|-----|
4244 CONTINUOUS STANDHYD NHYD=["B4"], DT=[1]min, AREA=[7.6](ha),
4245 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4246 SCS curve number CN=[75],
4247 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4248 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4249 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4250 LGI=[388](m), MNI=[0.013], SCI=[0](min),
4251 Continuous simulation parameters:
4252 IaREcper=[4](hrs), IaREcimp=[4](hrs),

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4253 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4254 InterEventTime=[18](hrs), END=-1
4255 *%-----|-----|
4256 COMPUTE DUALHYD NHYDin=["B4"], CINLET=[0.655](cms), NINLET=[1],
4257 MajNHYD=["B4-MJ"]
4258 MinNHYD=["B4-MN"]
4259 TMJSTO=[323](cu-m)
4260 *%-----|-----|
4261 ADD HYD NHYDsum=["MH340"], NHYDs to add=["338-340"+"B4-MN"]
4262 *%-----|-----|
4263 SAVE HYD NHYD=["MH340"], # OF PCYCLES=[-1], ICASEsh=[1]
4264 HYD_COMMENT=["Total Flows at MH340"]
4265 *%-----|-----|
4266 ROUTE PIPE PTYPE=[1]circ, NHYDout=["340-104"], RNUMBER=[1.0], PDIAM=[1650](mm),
4267 PLNGTH=[240](m), PROUGH=[0.013], PSLOPE=[0.0015](m/m),
NHYDin=["MH340"], RDT=[1]
4268 *%-----|-----|
4269 ADD HYD NHYDsum=["MH104T"], NHYDs to add=["340-104"+"MH104"]
4270 *%-----|-----|
4271 ROUTE PIPE PTYPE=[2]rect, NHYDout=["104-105"], RNUMBER=[1.0],
PWIDTTH=[2400](mm) by PHEIGHT=[2100](mm),
4272 PLNGTH=[380](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH104T"], RDT=[1]
4273 *%-----|-----|
4274 CONTINUOUS STANDHYD NHYD=["B5"], DT=[1]min, AREA=[2.2](ha),
4275 XIMP=[0.57], TIMP=[0.57], DWF=[0](cms), LOSS=[2],
4276 SCS curve number CN=[75],
4277 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4278 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4279 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4280 LGI=[187](m), MNI=[0.013], SCI=[0](min),
4281 Continuous simulation parameters:
4282 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4283 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4284 InterEventTime=[18](hrs), END=-1
4285 *%-----|-----|
4286 COMPUTE DUALHYD NHYDin=["B5"], CINLET=[0.260](cms), NINLET=[1],
4287 MajNHYD=["B5-MJ"]
4288 MinNHYD=["B5-MN"]
4289 TMJSTO=[250](cu-m)
4290 *%-----|-----|
4291 CONTINUOUS STANDHYD NHYD=["A8"], DT=[1]min, AREA=[0.96](ha),
4292 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4293 SCS curve number CN=[75],
4294 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4295 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4296 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4297 LGI=[186](m), MNI=[0.013], SCI=[0](min),
4298 Continuous simulation parameters:
4299 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4300 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4301 InterEventTime=[18](hrs), END=-1
4302 *%-----|-----|
4303 ADD HYD NHYDsum=["A8T"], NHYDs to add=["A6-MJ"+"A8"]
4304 *%-----|-----|
4305 COMPUTE DUALHYD NHYDin=["A8T"], CINLET=[0.238](cms), NINLET=[1],
4306 MajNHYD=["A8-MJ"]
4307 MinNHYD=["A8-MN"]
4308 TMJSTO=[40](cu-m)
4309 *%-----|-----|
4310 ADD HYD NHYDsum=["MH105"], NHYDs to
add=["104-105"+"B5-MN"+"A8-MN"+"TODD_MN3j"]
4311 *%-----|-----|
4312 SAVE HYD NHYD=["MH105"], # OF PCYCLES=[-1], ICASEsh=[1]
4313 HYD_COMMENT=["Total Flows at MH105"]

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4314 *%-----|-----|
4315 DIVERT HYD NHYDin=["A8-MJ"] NIDout=[2]max five,
4316 outflow hydrographs (NHYDs)=["A8-MJ-JR" "A8-MJ-B6"]
4317 flow distribution table: (modify as necessary)
4318 Note: all flows are in (cms)
4319 QIDi + QIDii = QTOTAL
4320 [ 0 + 0 = 0 ]
4321 [ 50 + 50 = 100 ] end
4322 *%-----|-----|
-----|-----|
4323 DIVERT HYD NHYDin=["MH105"] NIDout=[2]max five,
4324 outflow hydrographs (NHYDs)=["MH105-JR" "MH105-B6"]
4325 flow distribution table: (modify as necessary)
4326 Note: all flows are in (cms)
4327 QIDi + QIDii = QTOTAL
4328 [ 0 + 0 = 0 ]
4329 [ 0 + 3.0 = 3.0 ]
4330 [ 96.9+ 3.1 = 100 ] end
4331 *%-----|-----|
-----|-----|
4332 CONTINUOUS STANDHYD NHYD=["B7"], DT=[1]min, AREA=[7.19](ha),
4333 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4334 SCS curve number CN=[75],
4335 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4336 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4337 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4338 LGI=[211](m), MNI=[0.013], SCI=[0](min),
4339 Continuous simulation parameters:
4340 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4341 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4342 InterEventTime=[18](hrs), END=-1
4343 *%-----|-----|
4344 ADD HYD NHYDsum=["B7-B4MJ"], NHYDs to add=["B4-MJ"+"B7"]
4345 *%-----|-----|
4346 COMPUTE DUALHYD NHYDin=["B7-B4MJ"], CINLET=[0.629](cms), NINLET=[1],
4347 MajNHYD=["B7R-MJ"]
4348 MinNHYD=["B7R-MN"]
4349 TMJSTO=[311](cu-m)
4350 *%-----|-----|
4351 ROUTE PIPE PTYPE=[1]circ, NHYDout=["360-106A"], RNUMBER=[1.0], PDIAM=[1050](mm),
4352 PLNGTH=[167](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["B7R-MN"], RDT=[1]
4353 *%-----|-----|
4354 * -JFSA 2021-01-19 change B6 to be developed as per geottawa website and apply the
parameters of A7, the nearest sub-catchment to B6, LGI is calculated based on B6 area
4355 CONTINUOUS STANDHYD NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4356 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4357 SCS curve number CN=[75],
4358 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4359 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4360 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4361 LGI=[148.099](m), MNI=[0.013], SCI=[0](min),
4362 Continuous simulation parameters:
4363 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4364 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4365 InterEventTime=[18](hrs), END=-1
4366 *%-----|-----|
4367 * -JFSA 2021-01-25 add B1 DUALHYD as per Corrigan Report, IBI Group, 2008
4368 COMPUTE DUALHYD NHYDin=["B6"], CINLET=[0.064](cms), NINLET=[1],
4369 MajNHYD=["B6-MJ"]
4370 MinNHYD=["B6-MN"]
4371 TMJSTO=[5484](cu-m)
4372 *%-----|-----|
4373 *CONTINUOUS NASHYD NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4374 * DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
4375 * N=[3.0], TP=[0.36]hrs,

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4376 *           Continuous simulation parameters:
4377 *           IaRECper=[4](hrs),
4378 *           SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
4379 *           InterEventTime=[12](hrs)
4380 *           Baseflow simulation parameters:
4381 *           BaseFlowOption=[1] ,
4382 *           InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4383 *           VHydCond=[0.055](mm/hr),  END=-1
4384 *%-----|-----|
4385 *%   -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
4386 CONTINUOUS STANDHYD NHYD=["EX-LAND"], DT=[1]min, AREA=[32.5](ha),
4387 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
4388 SCS curve number CN=[74],
4389 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4390 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4391 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4392 LGI=[465.475](m), MNI=[0.013], SCI=[0](min),
4393 Continuous simulation parameters:
4394 IaRECper=[4](hrs),  IaRECimp=[4](hrs),
4395 SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
4396 InterEventTime=[18](hrs),  END=-1
4397 *%-----|-----|
4398 COMPUTE DUALHYD NHYDin=["EX-LAND"], CINLET=[2.275](cms), NINLET=[1],
4399 MajNHYD=["EX-LAND-MJ"]
4400 MinNHYD=["EX-LAND-MN"]
4401 TMJSTO=[1365](cu-m)
4402 *%-----|-----|
4403 ADD HYD NHYDsum=["B6-B7ExMJ"], NHYDs to
add=["B7R-MJ"+"EX-LAND-MJ"+"B5-MJ"+"B6-MJ"+"B6-MN"+"A8-MJ-B6"]
4404 *%-----|-----|
4405 COMPUTE DUALHYD NHYDin=["B6-B7ExMJ"], CINLET=[0.064](cms), NINLET=[1],
4406 MajNHYD=["B6R-MJ"]
4407 MinNHYD=["B6R-MN"]
4408 TMJSTO=[5484](cu-m)
4409 *%-----|-----|
4410 ROUTE PIPE PTYPE=[1]circ, NHYDout=["105-106A"], RNUMBER=[1.0], PDIAM=[1800](mm),
4411 PLNGTH=[208](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH105-B6"], RDT=[1]
4412 *%-----|-----|
4413 ADD HYD NHYDsum=["MH106A"], NHYDs to
add=["360-106A"+"105-106A"+"B6R-MN"+"B6R-MJ"]
4414 *%-----|-----|
4415 SAVE HYD NHYD=["MH106A"], # OF PCYCLES=[-1], ICASEsh=[1]
4416 HYD_COMMENT=["Total Flows at MH106A"]
4417 *%-----|-----|
4418 *%   -JFSA 2021-01-12 THE MANHOLE MH106 is called MH117/106 in Corrigan Report, IBI
Group, July 2008
4419 *%
4420 ROUTE PIPE PTYPE=[1]circ, NHYDout=["106A-106"], RNUMBER=[1.0], PDIAM=[1800](mm),
4421 PLNGTH=[190](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH106A"], RDT=[1]
4422 *%-----|-----|
4423 CONTINUOUS STANDHYD NHYD=["A9"], DT=[1]min, AREA=[2.44](ha),
4424 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4425 SCS curve number CN=[75],
4426 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4427 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4428 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4429 LGI=[262](m), MNI=[0.013], SCI=[0](min),
4430 Continuous simulation parameters:
4431 IaRECper=[4](hrs),  IaRECimp=[4](hrs),
4432 SMIN=[-1](mm),  SMAX=[-1](mm), SK=[0.010]/(mm),
4433 InterEventTime=[18](hrs),  END=-1
4434 *%-----|-----|
4435 COMPUTE DUALHYD NHYDin=["A9"], CINLET=[0.547](cms), NINLET=[1],

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4436      MajNHYD=["A9-MJ"]
4437      MinNHYD=["A9-MN"]
4438      TMJSTO=[0](cu-m)
4439  *%-----|-----|
4440  ADD HYD      NHYDsum=["MH106"], NHYDs to add=["106A-106"+"A9-MN"]
4441  *%-----|-----|
4442  SAVE HYD     NHYD=["MH106"], # OF PCYCLES=[-1], ICASEsh=[1]
4443      HYD_COMMENT=["Total Flows at MH106"]
4444  *%-----|-----|
4445  *%      -JFSA 2021-01-12 THE MANHOLE MH107 is called MH118/107 in Corrigan Report, IBI
Group, July 2008
4446  *%-----|-----|
4447  ROUTE PIPE   PTYPE=[1]circ, NHYDout=["106-107"], RNUMBER=[1.0], PDIAM=[1800](mm),
4448      PLNGTH=[122.5](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
NHYDin=["MH106"], RDT=[1]
4449  *%-----|-----|
4450  CONTINUOUS STANDHYD NHYD=["A10"], DT=[1]min, AREA=[4.14](ha),
4451      XIMP=[0.35], TIMP=[0.47], DWF=[0](cms), LOSS=[2],
4452      SCS curve number CN=[75],
4453      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4454          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4455      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4456          LGI=[183](m), MNI=[0.013], SCI=[0](min),
4457      Continuous simulation parameters:
4458      IaREcper=[4](hrs), IaREcimp=[4](hrs),
4459      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4460      InterEventTime=[18](hrs), END=-1
4461  *%-----|-----|
4462  COMPUTE DUALHYD NHYDin=["A10"], CINLET=[0.310](cms), NINLET=[1],
4463      MajNHYD=["A10-MJ"]
4464      MinNHYD=["A10-MN"]
4465      TMJSTO=[228](cu-m)
4466  *%-----|-----|
4467  CONTINUOUS STANDHYD NHYD=["A11"], DT=[1]min, AREA=[10.61](ha),
4468      XIMP=[0.53], TIMP=[0.62], DWF=[0](cms), LOSS=[2],
4469      SCS curve number CN=[75],
4470      Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4471          LGP=[40](m), MNP=[0.25], SCP=[0](min),
4472      Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4473          LGI=[379](m), MNI=[0.013], SCI=[0](min),
4474      Continuous simulation parameters:
4475      IaREcper=[4](hrs), IaREcimp=[4](hrs),
4476      SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4477      InterEventTime=[18](hrs), END=-1
4478  *%-----|-----|
4479  COMPUTE DUALHYD NHYDin=["A11"], CINLET=[0.993](cms), NINLET=[1],
4480      MajNHYD=["A11-MJ"]
4481      MinNHYD=["A11-MN"]
4482      TMJSTO=[556](cu-m)
4483  *%-----|-----|
4484  ADD HYD      NHYDsum=["MH107"], NHYDs to add=["106-107"+"A10-MN"+"A11-MN"]
4485  *%-----|-----|
4486  SAVE HYD     NHYD=["MH107"], # OF PCYCLES=[-1], ICASEsh=[1]
4487      HYD_COMMENT=["Total Flows at MH107"]
4488  *%-----|-----|
4489  ROUTE PIPE   PTYPE=[1]circ, NHYDout=["107-119"], RNUMBER=[1.0], PDIAM=[1800](mm),
4490      PLNGTH=[114](m), PROUGH=[0.013], PSLOPE=[0.0012](m/m),
NHYDin=["MH107"], RDT=[1]
4491  *%-----|-----|
4492  *%      -JFSA 2021-01-12 THE MANHOLE MH108 is called MH120/108 in Corrigan Report, IBI
Group, July 2008
4493  *%-----|-----|
4494  ROUTE PIPE   PTYPE=[1]circ, NHYDout=["119-108"], RNUMBER=[1.0], PDIAM=[1800](mm),
4495      PLNGTH=[65.8](m), PROUGH=[0.013], PSLOPE=[0.0012](m/m),
NHYDin=["107-119"], RDT=[1]
4496  *%-----|-----|

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4497 CONTINUOUS STANDHYD NHYD=["A12"], DT=[1]min, AREA=[12.29](ha),
4498 XIMP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4499 SCS curve number CN=[75],
4500 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4501 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4502 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4503 LGI=[183](m), MNI=[0.013], SCI=[0](min),
4504 Continuous simulation parameters:
4505 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4506 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4507 InterEventTime=[18](hrs), END=-1
4508 *%-----|
4509 COMPUTE DUALHYD NHYDin=["A12"], CINLET=[1.029](cms), NINLET=[1],
4510 MajNHYD=["A12-MJ"]
4511 MinNHYD=["A12-MN"]
4512 TMJSTO=[672](cu-m)
4513 *%-----|
4514 CONTINUOUS STANDHYD NHYD=["A13"], DT=[1]min, AREA=[2.59](ha),
4515 XIMP=[0.71], TIMP=[0.71], DWF=[0](cms), LOSS=[2],
4516 SCS curve number CN=[75],
4517 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4518 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4519 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4520 LGI=[379](m), MNI=[0.013], SCI=[0](min),
4521 Continuous simulation parameters:
4522 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4523 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4524 InterEventTime=[18](hrs), END=-1
4525 *%-----|
4526 COMPUTE DUALHYD NHYDin=["A13"], CINLET=[0.571](cms), NINLET=[1],
4527 MajNHYD=["A13-MJ"]
4528 MinNHYD=["A13-MN"]
4529 TMJSTO=[0](cu-m)
4530 *%-----|
4531 * -JFSA 2021-01-22 add the Corrigan pond area ("Pond-Block")
4532 CONTINUOUS STANDHYD NHYD=["Pond-Block"], DT=[1]min, AREA=[2.94](ha),
4533 XIMP=[0.415], TIMP=[0.415], DWF=[0](cms), LOSS=[2],
4534 SCS curve number CN=[75],
4535 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4536 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4537 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4538 LGI=[183](m), MNI=[0.013], SCI=[0](min),
4539 Continuous simulation parameters:
4540 IaRECPper=[4](hrs), IaRECImp=[4](hrs),
4541 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4542 InterEventTime=[18](hrs), END=-1
4543 *%-----|
4544 ADD HYD NHYDsum=["MH108"], NHYDs to add=["119-108"+"A13-MN"+"A12-MN"]
4545 *%-----|
4546 SAVE HYD NHYD=["MH108"], # OF PCYCLES=[-1], ICASEsh=[1]
4547 HYD_COMMENT=["Total Flows at MH108"]
4548 *%-----|
4549 ROUTE PIPE PTYPE=[1]circ, NHYDout=["108-116"], RNUMBER=[1.0], PDIAM=[1800](mm),
4550 PLNGTH=[76.6](m), PROUGH=[0.013], PSLOPE=[0.0013](m/m),
NHYDin=["MH108"], RDT=[1]
4551 *%-----|
4552 ROUTE PIPE PTYPE=[1]circ, NHYDout=["116-corrig"], RNUMBER=[1.0],
4553 PDIAM=[1800](mm),
PLNGTH=[79.5](m), PROUGH=[0.013], PSLOPE=[0.0013](m/m),
NHYDin=["108-116"], RDT=[1]
4554 *%-----|
4555 ADD HYD NHYDsum=["Corrigan"], NHYDs to add=["116-corrig"+"Pond-Block"]
4556 *%-----|
4557 SAVE HYD NHYD=["Corrigan"], # OF PCYCLES=[-1], ICASEsh=[1]
4558 HYD_COMMENT=["Total Flows at Corrigan Pond"]
4559 *%-----|

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4560 ROUTE RESERVOIR      NHYDout=["Co-P"],  NHYDin=["Corrigan"],
4561 RDT=[1](min),
4562          TABLE of ( OUTFLOW-STORAGE ) values
4563                    (cms) - (ha-m)
4564                    [ 0.0 , 0.0 ]
4565                    [ 0.015 , 0.04118]
4566                    [ 0.030 , 0.08297]
4567                    [ 0.045 , 0.12537]
4568                    [ 0.060 , 0.16837]
4569                    [ 0.075 , 0.21199]
4570                    [ 0.090 , 0.27545]
4571                    [ 0.105 , 0.34650]
4572                    [ 0.120 , 0.42049]
4573                    [ 0.135 , 0.50188]
4574                    [ 0.186 , 0.60307]
4575                    [ 2.110 , 0.79083]
4576                    [ 5.874 , 1.00271]
4577                    [ 11.395 , 1.29643]
4578                    [ 18.770 , 1.62054]
4579                    [ 28.143 , 1.97516]
4580                    [ -1 , -1 ] (max twenty pts)
4581          NHYDovf=["Co-P-OVF"]
4582 *%-----|-----|
4583 ADD HYD      NHYDsum=["corrig"], NHYDs to
add=["Co-P-OVF"+"Co-P"+"N_TO"+"MH105-JR"+"A8-MJ-JR"+"A9-MJ"+"A10-MJ"+"A11-MJ"+"A12-MJ"+"A
13-MJ"]
4584 *%-----|-----|
4585 SAVE HYD     NHYD=["corrig"], # OF PCYCLES=[-1], ICASEsh=[1]
4586             HYD_COMMENT=["Total Flows at Corrigan Pond"]
4587 *%-----|-----|
4588 *#*****|*****|
4589 *#   Corrigan Pond 1
4590 *#   - Rating curve obtained from Barrhaven South MSS modeling
4591 *#   - Tributary Drainage Area to MSS Pond 1 = 145 ha
4592 *#*****|*****|
4593 *ROUTE RESERVOIR      NHYDout=["MS_P1"],  NHYDin=["CORRIG"],
4594 * RDT=[1](min),
4595 *          TABLE of ( OUTFLOW-STORAGE ) values
4596 *                    (cms) - (ha-m)
4597 *                    [ 0.0 , 0.0 ]
4598 *                    [ 0.06 , 0.58]
4599 *                    [ -1 , -1 ] (max twenty pts)
4600 *          NHYDovf=["P1-OVF"]
4601 *%-----|-----|
4602 *ADD HYD      NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
4603 *%-----|-----|
4604 *SAVE HYD     NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
4605 *             HYD_COMMENT=["Total Flows at Corrigan Drain"]
4606 *%-----|-----|
4607 *#
4608 *# Hydrograph from Corrigan Drain routed to Jockvale Road
4609 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
4610 *#
4611 ROUTE CHANNEL      NHYDout=["N_MI"] ,NHYDin=["corrig"] ,
4612 RDT=[1](min),
4613 CHLGTH=[580](m),  CHSLOPE=[0.4448](%),
4614                    FPSLOPE=[0.4448](%),
4615 SECNUM=[1.0],      NSEG=[3]
4616 ( SEGROUGH, SEGDIST (m))=
4617 [0.075,-17.72
4618 -0.045,17.72
4619 0.075,80.62] NSEG times
4620 ( DISTANCE (m), ELEVATION (m))=
4621 [-83.32, 90.00]
4622 [-81.36, 89.50]
4623 [-79.12, 89.00]

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4624 [-76.13, 88.50]
4625 [-20.46, 88.00]
4626 [-19.36, 87.50]
4627 [-18.51, 87.00]
4628 [-17.72, 86.50]
4629 [-11.95, 85.24]
4630 [-0.11, 85.12]
4631 [11.49, 85.20]
4632 [17.72, 86.50]
4633 [19.74, 87.00]
4634 [21.22, 87.50]
4635 [22.68, 88.00]
4636 [24.28, 88.50]
4637 [26.79, 89.00]
4638 [71.98, 90.00]
4639 [80.62, 90.50]

4640 *%-----|-----|
4641 *#*****|

4642 *# Catchment MILLS
4643 *# - To SWM Facility north of the Jock
4644 *# - Primarily residential development
4645 *#*****|

4646 **CONTINUOUS STANDHYD** NHYD=["MILLS"], DT=[1]min, AREA=[175.99](ha),
4647 XIMP=[0.38], TIMP=[0.38], DWF=[0](cms), LOSS=[2],
4648 SCS curve number CN=[74],
4649 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4650 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4651 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4652 LGI=[1118.123](m), MNI=[0.013], SCI=[0](min),
4653 Continuous simulation parameters:
4654 IaRECper=[4](hrs), IaRECimp=[4](hrs),
4655 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4656 InterEventTime=[18](hrs), END=-1

4657 *%-----|-----|
4658 *#*****|

4659 *# Chapman Mills SWM Pond
4660 *# - Rating curve obtained from CCL hydraulic modeling
4661 *#*****|

4662 **ROUTE RESERVOIR** NHYDout=["MILL_P"], NHYDin=["MILLS"],
4663 RDT=[1](min),

4664 TABLE of (OUTFLOW-STORAGE) values
4665 (cms) - (ha-m)

4666	[0.0 , 0.0]
4667	[0.01 , 0.01]
4668	[0.05 , 0.06]
4669	[0.09 , 0.11]
4670	[0.13 , 0.15]
4671	[0.18 , 0.19]
4672	[0.28 , 0.28]
4673	[0.37 , 0.34]
4674	[0.45 , 0.40]
4675	[0.51 , 0.44]
4676	[0.56 , 0.47]
4677	[0.64 , 0.52]
4678	[0.76 , 0.59]
4679	[0.86 , 0.65]
4680	[1.09 , 0.78]
4681	[1.44 , 0.96]
4682	[3.18 , 1.84]
4683	[4.05 , 2.31]
4684	[-1 , -1] (max twenty pts)

4685 NHYDovf=["MIL-OV"]

4686 *%-----|-----|
4687 **ADD HYD** NHYDsum=["SN_MI"], NHYDs to add=["N_MI"+"MIL-OV"+"MILL_P"]

4688 *%-----|-----|

4689 **SAVE HYD** NHYD=["SN_MI"], # OF PCYCLES=[-1], ICASEsh=[1]


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4690 HYD_COMMENT=["Total Flows at Jockvale Road"]
4691 *%-----|-----|
4692 *#
4693 *# Hydrograph from Jockvale Road routed to Heart's Desire
4694 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689
4695 *#
4696 ROUTE CHANNEL NHYDout=["N_DE"] ,NHYDin=["SN_MI"] ,
4697 RDT=[1](min),
4698 CHLGTH=[1962](m), CHSLOPE=[0.2227](%),
4699 FPSLOPE=[0.2227](%),
4700 SECNUM=[1.0], NSEG=[3]
4701 ( SEGROUGH, SEGDIST (m))=
4702 [0.075,-17.56
4703 -0.045,18.27
4704 0.075,32.51] NSEG times
4705 ( DISTANCE (m), ELEVATION (m))=
4706 [-54.07, 85.00]
4707 [-39.43, 84.50]
4708 [-28.30, 84.00]
4709 [-24.12, 83.50]
4710 [-22.30, 83.00]
4711 [-20.55, 82.50]
4712 [-17.56, 82.00]
4713 [-12.63, 81.22]
4714 [-0.11, 80.75]
4715 [11.55, 81.22]
4716 [18.27, 82.00]
4717 [19.82, 82.50]
4718 [22.48, 83.00]
4719 [27.90, 83.50]
4720 [29.31, 84.00]
4721 [30.81, 84.50]
4722 [32.51, 85.00]
4723 *%-----|-----|
4724 *#*****
4725 *# Catchment DESIRE
4726 *# - To Jock River (north of the Jock)
4727 *# - Rural-estate subdivision (Heart's Desire Community)
4728 *#*****
4729 CONTINUOUS STANDHYD NHYD=["DESIRE"], DT=[1]min, AREA=[23.78](ha),
4730 XIMP=[0.25], TIMP=[0.25], DWF=[0](cms), LOSS=[2],
4731 SCS curve number CN=[77],
4732 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4733 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4734 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4735 LGI=[400](m), MNI=[0.013], SCI=[0](min),
4736 Continuous simulation parameters:
4737 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4738 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4739 InterEventTime=[18](hrs), END=-1
4740 *%-----|-----|
4741 *#*****
4742 *# Catchment JOCKVA
4743 *# - To Jockvale SWM Facility
4744 *# - Residential development & golf course
4745 *# - JFSA 2021-01-11 update JOCKVA after updating CORRIG as per IBI GROUP, July 2008.
4746 *# JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two
4747 *# areas JOCKVA and EX-LAND 32.5 ha as per IBI GROUP, July 2008.
4748 *#*****
4749 CONTINUOUS STANDHYD NHYD=["JOCKVA"], DT=[1]min, AREA=[225.13](ha),
4750 XIMP=[0.50], TIMP=[0.50], DWF=[0](cms), LOSS=[2],
4751 SCS curve number CN=[74],
4752 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4753 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4754 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4755 LGI=[1310.55](m), MNI=[0.013], SCI=[0](min),

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4755 Continuous simulation parameters:
4756 IaREcper=[4](hrs), IaREcimp=[4](hrs),
4757 SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4758 InterEventTime=[18](hrs), END=-1
4759 *%-----|-----|
4760 ADD HYD NHYDsum=["JOCKVA-TO"], NHYDs to
add=["EX-LAND-MN"+"JOCKVA"+"B2-MJ"+"B3-MJ"]
4761 *%-----|-----|
4762 SAVE HYD NHYD=["JOCKVA-TO"], # OF PCYCLES=[-1], ICASEsh=[1]
4763 HYD_COMMENT=["Total Flows at KB first pond"]
4764 *%-----|-----|
4765 *#*****|*****|
4766 *# Jockvale SWM Facility
4767 *# - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
4768 *#*****|*****|
4769 ROUTE RESERVOIR NHYDout=["JOCK_P"], NHYDin=["JOCKVA-TO"],
4770 RDT=[1](min),
4771 TABLE of ( OUTFLOW-STORAGE ) values
4772 (cms) - (ha-m)
4773 [ 0.0 , 0.0 ]
4774 [ 0.27 , 0.03]
4775 [ 0.28 , 0.55]
4776 [ 0.29 , 1.14]
4777 [ 0.30 , 1.80]
4778 [ 0.31 , 2.32]
4779 [ 1.12 , 2.87]
4780 [ 2.92 , 3.45]
4781 [ 4.64 , 4.07]
4782 [ 6.69 , 4.72]
4783 [ 9.02 , 5.39]
4784 [ 11.62 , 6.10]
4785 [ 14.42 , 6.85]
4786 [ 17.45 , 7.62]
4787 [ 20.69 , 8.44]
4788 [ 24.08 , 9.28]
4789 [ 27.68 , 10.17]
4790 [ -1 , -1 ] (max twenty pts)
4791 NHYDovf=["JO-OVF"]
4792 *%-----|-----|
4793 ADD HYD NHYDsum=["SN_DE"], NHYDs to add=["N_DE"+"DESIRE"+"JO-OVF"+"JOCK_P"]
4794 *%-----|-----|
4795 SAVE HYD NHYD=["SN_DE"], # OF PCYCLES=[-1], ICASEsh=[1]
4796 HYD_COMMENT=["Total Flows at Heart's Desire"]
4797 *%-----|-----|
4798 *#
4799 *# Hydrograph from Heart's Desire routed to Rideau River
4800 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
4801 *#
4802 ROUTE CHANNEL NHYDout=["N1"] ,NHYDin=["SN_DE"] ,
4803 RDT=[1](min),
4804 CHLGTH=[563](m), CHSLOPE=[0.9668](%),
4805 FPSLOPE=[0.9668](%),
4806 SECNUM=[1.0], NSEG=[3]
4807 ( SEGROUGH, SEGDIST (m))=
4808 [0.075,-30.20
4809 -0.045,30.20
4810 0.075,48.48] NSEG times
4811 ( DISTANCE (m), ELEVATION (m))=
4812 [-98.46, 81.50]
4813 [-92.24, 81.00]
4814 [-86.88, 80.50]
4815 [-81.54, 80.00]
4816 [-74.36, 79.50]
4817 [-63.54, 79.00]
4818 [-39.23, 78.50]
4819 [-34.51, 78.00]

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4820          [-33.01, 77.50]
4821          [-30.20, 77.00]
4822          [-13.42, 76.18]
4823          [-1.14, 76.09]
4824          [17.06, 76.18]
4825          [30.20, 77.00]
4826          [32.95, 77.50]
4827          [34.06, 78.00]
4828          [35.11, 78.50]
4829          [36.32, 79.00]
4830          [37.74, 79.50]
4831          [48.48, 81.50]
4832  *%-----|-----|
4833  *#*****|*****|
4834  *#      Catchment S-2
4835  *#      - To Jock River (north and south)
4836  *#      - Undeveloped floodplain and river
4837  *#*****|*****|
4838  CONTINUOUS NASHYD  NHYD=["S-2"], DT=[1]min, AREA=[102.94](ha),
4839                    DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
4840                    N=[3], TP=[0.40]hrs,
4841                    Continuous simulation parameters:
4842                    IaREcper=[4](hrs),
4843                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4844                    InterEventTime=[12](hrs)
4845                    Baseflow simulation parameters:
4846                    BaseFlowOption=[1] ,
4847                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4848                    VHydCond=[0.055](mm/hr), END=-1
4849  *%-----|-----|
4850  ADD HYD            NHYDsum=["SN_N1"], NHYDs to add=["N1"+"S-2"]
4851  *%-----|-----|
4852  SAVE HYD          NHYD=["SN_N1"], # OF PCYCLES=[-1], ICASEsh=[1]
4853                    HYD_COMMENT=["Total Flows at Rideau River"]
4854  *%-----|-----|
4855  *#####|#####|
4856  *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4857  START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
4858  *%              ["C24SC005.stm"] <--storm filename, one per line for NSTORM time
4859  *%-----|-----|
4860  *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4861  START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
4862  *%              ["C24SC010.stm"] <--storm filename, one per line for NSTORM time
4863  *%-----|-----|
4864  *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4865  START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
4866  *%              ["C24SC025.stm"] <--storm filename, one per line for NSTORM time
4867  *%-----|-----|
4868  *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4869  START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
4870  *%              ["C24SC050.stm"] <--storm filename, one per line for NSTORM time
4871  *%-----|-----|
4872  *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4873  *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4874  *%              ["100YC3H.STM"] <--storm filename, one per line for NSTORM time
4875  *%-----|-----|
4876  *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4877  START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4878  *%              ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
4879  *%-----|-----|
4880  *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4881  *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4882  *%              ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
4883  *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[101]
4884  *%              ["A24SC100.stm"] <--storm filename, one per line for NSTORM time
4885  *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]

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4886  *% ["A24SC100_60.stm"] <--storm filename, one per line for NSTORM time
4887  FINISH
4888
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00001 .....
00002 .....
00003 SSSSS W W M M H H Y Y V M M OOO .....
00004 S W W M M H H T T M M M O O .....
00005 SSSSS W W M M M H H H H H Y Y M M M O O .....
00006 S W W M M H H Y Y M M O O .....
00007 SSSSS W W M M H H Y Y M M O O .....
00008 .....
00009 Stormwater Management Hydrologic Model .....
00010 .....
00011 .....
00012 ***** SMOHYMO Ver 5.000 *****
00013 .....
00014 ***** A single event and continuous hydrologic simulation model *****
00015 ***** based on the principles of HMO and its successors *****
00016 ***** OTHYMO=83 and OTHYMO=89 *****
00017 .....
00018 ***** Distributed by: J.P. Sabourin and Associates Inc. *****
00019 ***** Ottawa, Ontario (613) 886-8884 *****
00020 ***** Gatineau, Quebec (819) 243-8888 *****
00021 ***** E-Mail: smohy@jfsa.com *****
00022 .....
00023 .....
00024 ***** Licensed user: JFSaInc *****
00025 ***** Ottawa ***** SERIAL#:2549237 *****
00026 ***** ***** *****
00027 .....
00028 ***** ***** PROGRAM ARRAY DIMENSIONS *****
00029 ***** Maximum value for ID numbers : 11 *****
00030 ***** Max. number of rainfall points: 105408 *****
00031 ***** ***** *****
00032 ***** Max. number of flow points : 105408 *****
00033 ***** ***** *****
00034 .....
00035 ***** SUMMARY OUTPUT *****
00036 ***** ***** *****
00037 ***** RUN DATE: 2021-03-04 TIME: 11:57:49 RUN COUNTER: 020284 *****
00038 ***** ***** *****
00039 ***** Input file: T:\PROJ\1474-16\Design\20210206-QuantityControlAnalysis\SMOHYMO\SMH-Model\Updated *****
00040 ***** 3\SMR_S-1_SDR-PP_SDr.dat *****
00041 ***** ***** *****
00042 ***** Output file: T:\PROJ\1474-16\Design\20210206-QuantityControlAnalysis\SMOHYMO\SMH-Model\Updated *****
00043 ***** 3\SMR_S-1_SDR-PP_SDr.dat *****
00044 ***** Summary file: T:\PROJ\1474-16\Design\20210206-QuantityControlAnalysis\SMOHYMO\SMH-Model\Updated *****
00045 ***** 3\SMR_S-1_SDR-PP_SDr.dat *****
00046 ***** User comment: *****
00047 ***** 1: *****
00048 ***** 2: *****
00049 ***** 3: *****
00050 ***** ***** *****
00051 ***** ***** *****
00052 ***** ***** *****
00053 ***** ***** *****
00054 ***** SMOHYMO Ver 5.000 ***** INPUT DATA FILE *****
00055 ***** ***** *****
00056 ***** Project Name: [Jock River] Project Number: [1474-16] *****
00057 ***** Date: [04-03-2021] *****
00058 ***** Modeller: [J.F.M.] *****
00059 ***** Company: [JFSaInc.] *****
00060 ***** License #: [2549237] *****
00061 ***** ***** *****
00062 ***** CALIBRATION OF SUMMER MODEL PARAMETERS *****
00063 ***** USING CONTINUOUS SIMULATIONS *****
00064 ***** Rainfall data from JFSA rain gauge installed at site + other gauges by the City *****
00065 ***** Use data collected from May 1st to July 14, 2003 *****
00066 ***** 2020-11-30 Change WMPROD in COMPUTE CHANNEL TWSTD = 0.1 instead of 0.0001 *****
00067 ***** 2020-12-01 correct pond curve values *****
00068 ***** 2020-12-01 change W_CLAMP_SDRP to 0.55, SDRP=[0.5/1] (impervious slope), and LGT up to 70m *****
00069 ***** 2021-02-19 Change W_CLAMP_SDRP in COMPUTE CHANNEL TWSTD = 0.1 instead of 0.0001 *****
00070 ***** 2021-02-19 Change the slope for ROUTE CHANNEL STATION 5002 (HNDOut="R_NC") ,HNDIn("SM_NC") from 0.01 to (as per S *****
00071 ***** ***** *****
00072 ***** ** END OF RUN : 1 *****
00073 ***** ***** *****
00074 ***** ***** *****
00075 ***** ***** *****
00076 ***** ***** *****
00077 ***** ***** *****
00078 ***** ***** *****
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00184 ***** ***** *****
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00186 ***** ***** *****
00187 ***** ***** *****

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00188 ***** [InterEventTime= 12.00] *****
00189 ***** ***** *****
00190 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00191 ***** # of 1.75 *****
00192 ***** ***** *****
00193 ***** CONTINUOUS NASHVD 1.0 01:SR_13 8376.00 4.434 No.date 39:59 11.98 263 .....
00194 ***** [Cm= 61.0 N= 3.00; Tps= 1.13] *****
00195 ***** [IaREC= 4.00; SMIN= 52.62; SMAX=350.79; SK= .010] *****
00196 ***** [InterEventTime= 12.00] *****
00197 ***** ***** *****
00198 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00199 ***** of 1.68 *****
00200 ***** ***** *****
00201 ***** CONTINUOUS NASHVD 1.0 01:SR_9 1132.00 4.434 No.date 39:59 11.98 263 .....
00202 ***** [Cm= 70.0 N= 3.00; Tps= 2.51] *****
00203 ***** [IaREC= 4.00; SMIN= 43.07; SMAX=287.10; SK= .010] *****
00204 ***** [InterEventTime= 12.00] *****
00205 ***** ***** *****
00206 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00207 ***** of 1.62 *****
00208 ***** ***** *****
00209 ***** CONTINUOUS NASHVD 1.0 01:SR_10 4664.00 5.504 No.date 39:59 10.98 241 .....
00210 ***** [Cm= 62.0 N= 3.00; Tps= 1.21] *****
00211 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00212 ***** [InterEventTime= 12.00] *****
00213 ***** ***** *****
00214 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00215 ***** of 1.60 *****
00216 ***** ***** *****
00217 ***** CONTINUOUS NASHVD 1.0 01:SR_8 131.00 .805 No.date 28:57 11.22 247 .....
00218 ***** [Cm= 63.0 N= 3.00; Tps= .93] *****
00219 ***** [IaREC= 4.00; SMIN= 59.42; SMAX=396.11; SK= .010] *****
00220 ***** [InterEventTime= 12.00] *****
00221 ***** ***** *****
00222 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00223 ***** of 1.65 *****
00224 ***** ***** *****
00225 ***** CONTINUOUS NASHVD 1.0 01:SR_8 388.00 4.432 No.date 38:46 11.98 263 .....
00226 ***** [Cm= 66.0 N= 3.00; Tps= 8.42] *****
00227 ***** [IaREC= 4.00; SMIN= 51.31; SMAX=350.79; SK= .010] *****
00228 ***** [InterEventTime= 12.00] *****
00229 ***** ***** *****
00230 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00231 ***** of 1.62 *****
00232 ***** ***** *****
00233 ***** CONTINUOUS NASHVD 1.0 01:SR_7 3397.00 4.651 No.date 36:31 9.85 217 .....
00234 ***** [Cm= 63.0 N= 3.00; Tps= .93] *****
00235 ***** [IaREC= 4.00; SMIN= 76.32; SMAX=508.81; SK= .010] *****
00236 ***** [InterEventTime= 12.00] *****
00237 ***** ***** *****
00238 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00239 ***** of 1.75 *****
00240 ***** ***** *****
00241 ***** CONTINUOUS NASHVD 1.0 01:SR_16 165.00 .413 No.date 33:07 12.24 269 .....
00242 ***** [Cm= 67.0 N= 3.00; Tps= 4.18] *****
00243 ***** [IaREC= 4.00; SMIN= 60.55; SMAX=336.97; SK= .010] *****
00244 ***** [InterEventTime= 12.00] *****
00245 ***** ***** *****
00246 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00247 ***** of 1.67 *****
00248 ***** ***** *****
00249 ***** CONTINUOUS NASHVD 1.0 01:VDR 1332.00 3.148 No.date 35:23 13.94 306 .....
00250 ***** [Cm= 72.0 N= 3.00; Tps= 5.93] *****
00251 ***** [IaREC= 4.00; SMIN= 39.78; SMAX=264.99; SK= .010] *****
00252 ***** [InterEventTime= 12.00] *****
00253 ***** ***** *****
00254 ***** CONTINUOUS NASHVD 1.0 01:SR_5 224.00 2.957 No.date 28:45 15.91 350 .....
00255 ***** [Cm= 77.0 N= 3.00; Tps= .78] *****
00256 ***** [IaREC= 4.00; SMIN= 31.31; SMAX=207.66; SK= .010] *****
00257 ***** [InterEventTime= 12.00] *****
00258 ***** ***** *****
00259 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00260 ***** of 1.23 *****
00261 ***** ***** *****
00262 ***** CONTINUOUS NASHVD 1.0 01:F1_CK 4945.00 14.839 No.date 33:25 14.57 320 .....
00263 ***** [Cm= 74.0 N= 3.00; Tps= 4.63] *****
00264 ***** [IaREC= 4.00; SMIN= 36.67; SMAX=244.49; SK= .010] *****
00265 ***** [InterEventTime= 12.00] *****
00266 ***** ***** *****
00267 ***** CONTINUOUS NASHVD 1.0 01:SR_5A2 20.00 .309 No.date 28:36 17.79 391 .....
00268 ***** [Cm= 81.0 N= 3.00; Tps= 4.1] *****
00269 ***** [IaREC= 4.00; SMIN= 25.21; SMAX=168.09; SK= .010] *****
00270 ***** [InterEventTime= 12.00] *****
00271 ***** ***** *****
00272 ***** The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4) *****
00273 ***** of 1.61 *****
00274 ***** ***** *****
00275 ***** CONTINUOUS NASHVD 1.0 01:SR_14 1413.00 3.090 No.date 38:04 11.22 334 .....
00276 ***** [Cm= 75.0 N= 3.00; Tps= 8.00] *****
00277 ***** [IaREC= 4.00; SMIN= 32.81; SMAX=225.43; SK= .010] *****
00278 ***** [InterEventTime= 12.00] *****
00279 ***** ***** *****
00280 ***** CONTINUOUS NASHVD 1.0 01:SR_4 585.00 4.325 No.date 29:58 17.79 391 .....
00281 ***** [Cm= 81.0 N= 3.00; Tps= 1.78] *****
00282 ***** [IaREC= 4.00; SMIN= 45.21; SMAX=168.09; SK= .010] *****
00283 ***** [InterEventTime= 12.00] *****
00284 ***** ***** *****
00285 ***** CONTINUOUS NASHVD 1.0 01:LM_CK 1021.00 5.747 No.date 30:50 17.39 382 .....
00286 ***** [Cm= 80.0 N= 3.00; Tps= .88] *****
00287 ***** [IaREC= 4.00; SMIN= 39.32; SMAX=175.50; SK= .010] *****
00288 ***** [InterEventTime= 12.00] *****
00289 ***** ***** *****
00290 ***** CONTINUOUS NASHVD 1.0 01:SR_2 177.00 2.052 No.date 28:45 15.91 350 .....
00291 ***** [Cm= 77.0 N= 3.00; Tps= .78] *****
00292 ***** [IaREC= 4.00; SMIN= 31.35; SMAX=207.66; SK= .010] *****
00293 ***** [InterEventTime= 12.00] *****
00294 ***** ***** *****
00295 ***** CONTINUOUS NASHVD 1.0 01:SR_10 1322.00 5.337 No.date 31:50 17.79 391 .....
00296 ***** [Cm= 81.0 N= 3.00; Tps= 4.63] *****
00297 ***** [IaREC= 4.00; SMIN= 25.21; SMAX=168.09; SK= .010] *****
00298 ***** [InterEventTime= 12.00] *****
00299 ***** ***** *****
00300 ***** CONTINUOUS NASHVD 1.0 01:SR_10 2737.00 11.528 No.date 31:35 15.56 342 .....
00301 ***** [Cm= 76.0 N= 3.00; Tps= 1.21] *****
00302 ***** [IaREC= 4.00; SMIN= 32.46; SMAX=216.39; SK= .010] *****
00303 ***** [InterEventTime= 12.00] *****
00304 ***** ***** *****
00305 ***** Routing hydrographs *****
00306 ***** ***** *****
00307 ***** Starting with the addition of Jock River Headwater and Subwatershed 13 *****
00308 ***** ***** *****
00309 ***** CONTINUOUS NASHVD 1.0 01:SR_10 3680.00 6.204 No.date 37:06 11.47 252 .....
00310 ***** ADD HYD *****
00311 ***** [IaREC= 4.00; SMIN= 43.07; SMAX=350.79; SK= .010] *****
00312 ***** [IaREC= 4.00; SMIN= 32.46; SMAX=216.39; SK= .010] *****
00313 ***** [InterEventTime= 12.00] *****
00314 ***** ***** *****
00315 ***** # of 1.64 *****
00316 ***** ***** *****
00317 ***** CONTINUOUS NASHVD 1.0 01:SR_10 3680.00 6.204 No.date 37:06 11.47 252 .....
00318 ***** [IaREC= 4.00; SMIN= 43.07; SMAX=350.79; SK= .010] *****
00319 ***** [IaREC= 4.00; SMIN= 32.46; SMAX=216.39; SK= .010] *****
00320 ***** [InterEventTime= 12.00] *****
00321 ***** ***** *****
00322 ***** [IaREC= 4.00; SMIN= 32.46; SMAX=216.39; SK= .010] *****
00323 ***** [InterEventTime= 12.00] *****
00324 ***** ***** *****
00325 ***** ***** *****
00326 ***** CONTINUOUS NASHVD 1.0 01:SR_10 4664.00 5.504 No.date 39:59 10.98 241 .....
00327 ***** ADD HYD *****
00328 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00329 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00330 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00331 ***** [InterEventTime= 12.00] *****
00332 ***** ***** *****
00333 ***** R0202:CO0013 *****
00334 ***** ***** *****
00335 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00336 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00337 ***** [IaREC= 4.00; SMIN= 61.90; SMAX=412.66; SK= .010] *****
00338 ***** ***** *****
00339 ***** R0202:CO0034 *****
00340 ***** ***** *****
00341 ***** ***** *****
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00375 #
00376 R002/C00039 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00377 ADD HYD + 1.0 02:DM11 9506.00 7.379_MoDate 33:12 11.20 n/a .000
00378 ROUTE CHANNEL -> 1.0 02:SM_11 500.00 2.720_MoDate 29:22 11.98 n/a .000
00379 + 1.0 02:SM_CK 1917.00 4.042_MoDate 34:34 11.98 n/a .000
00380 SUM= 1.0 01:SM_N10 11923.00 12.077_MoDate 33:14 11.36 n/a .000
00381 #
00382 # Sum of hydrographs from Node 11 routed to Node 10
00383 # Section 1
00384 #
00385 R002/C00040 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00386 ROUTE CHANNEL -> 1.0 02:SM_11 11923.00 12.077_MoDate 33:14 11.36 n/a .000
00387 [RPT= 1.00] out<- 1.0 01:SM10 11923.00 8.276_MoDate 39:46 11.36 n/a .000
00388 [L/S/N= 392.7 / .057 / .040]
00389 [Vmax=.462;Dmax=.886]
00390 #
00391 # Addition of Subwatershed 10 to Node 10
00392 #
00393 R002/C00041 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00394 ADD HYD + 1.0 02:SM10 17859.00 19.451_MoDate 38:31 12.19 n/a .000
00395 ROUTE CHANNEL -> 1.0 02:SM_N10 6566.00 11.228_MoDate 38:07 13.94 n/a .000
00396 + 1.0 01:SM_N10 17859.00 19.451_MoDate 38:31 12.19 n/a .000
00397 SUM= 1.0 01:SM_N10 17859.00 19.451_MoDate 38:31 12.19 n/a .000
00398 SAVE HYD + 1.0 01:SM_N10 17859.00 19.451_MoDate 38:31 12.19 n/a .000
00399 fname :R_H2M10
00400 remark:flow at R_N10: M10 + SK_10
00401 # Addition of Kings Creek to S_N10
00402 #
00403 R002/C00043 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00404 ADD HYD + 1.0 02:SM_N10 17859.00 19.451_MoDate 38:31 12.19 n/a .000
00405 ROUTE CHANNEL -> 1.0 02:SM_N10 8376.00 11.072_MoDate 38:59 11.98 n/a .000
00406 + 1.0 02:SM_CK 25965.00 30.328_MoDate 39:58 12.12 n/a .000
00407 SUM= 1.0 01:SM_N10A 25965.00 30.328_MoDate 39:58 12.12 n/a .000
00408 # Sum of hydrographs from Node 10 routed to Node 9
00409 # Section 2
00410 #
00411 R002/C00044 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00412 ROUTE CHANNEL -> 1.0 02:SM_N10A 25965.00 29.579_MoDate 39:59 12.12 n/a .000
00413 [RPT= 1.00] out<- 1.0 01:SM 25965.00 29.579_MoDate 39:59 12.12 n/a .000
00414 [L/S/N= 392.7 / .077 / .040]
00415 [Vmax=.595;Dmax=1.208]
00416 #
00417 # Addition of Subwatershed 9 and Nichols Creek to Node 9
00418 #
00419 R002/C00045 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00420 ADD HYD + 1.0 02:SM 25965.00 29.579_MoDate 39:59 12.12 n/a .000
00421 ROUTE CHANNEL -> 1.0 02:SM_N 4464.00 6.504_MoDate 39:59 10.98 n/a .000
00422 + 1.0 02:SM_CK 3166.00 36.313_MoDate 39:59 12.00 n/a .000
00423 SUM= 1.0 01:SM_N 3166.00 36.313_MoDate 39:59 12.00 n/a .000
00424 #
00425 # Sum of hydrographs from Node 9 routed to Node 8
00426 # Section 3
00427 #
00428 R002/C00046 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00429 ROUTE CHANNEL -> 1.0 02:SM_N 3166.00 36.313_MoDate 39:59 12.00 n/a .000
00430 [RPT= 1.00] out<- 1.0 01:SM 3166.00 34.713_MoDate 39:59 12.00 n/a .000
00431 [L/S/N= 392.7 / .087 / .040]
00432 [Vmax=.418;Dmax=1.281]
00433 #
00434 # Addition of Subwatershed 8 and Bobb's Drain to Node 8
00435 #
00436 R002/C00047 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00437 ADD HYD + 1.0 02:SM 3166.00 34.713_MoDate 39:59 12.00 n/a .000
00438 ROUTE CHANNEL -> 1.0 02:SM_N 8376.00 11.072_MoDate 38:59 11.98 n/a .000
00439 + 1.0 02:SM_CK 3854.00 6.242_MoDate 38:46 11.98 n/a .000
00440 SUM= 1.0 01:SM_N 3546.00 40.474_MoDate 39:59 12.00 n/a .000
00441 #
00442 # Sum of hydrographs from Node 8 routed to Node 7
00443 # Section 4
00444 #
00445 R002/C00048 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00446 ROUTE CHANNEL -> 1.0 02:SM_N 3546.00 40.474_MoDate 39:59 12.00 n/a .000
00447 [RPT= 1.00] out<- 1.0 01:SM 3546.00 32.892_MoDate 44:30 12.00 n/a .000
00448 [L/S/N= 375.0 / .057 / .070]
00449 [Vmax=.208;Dmax=1.651]
00450 #
00451 # Addition of Subwatershed 7 to Node 7
00452 #
00453 R002/C00049 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00454 ADD HYD + 1.0 02:SM 3546.00 32.892_MoDate 44:30 12.00 n/a .000
00455 ROUTE CHANNEL -> 1.0 02:SM_N 3197.00 6.317_MoDate 46:31 9.45 n/a .000
00456 + 1.0 01:SM_N 38743.00 35.071_MoDate 43:33 11.82 n/a .000
00457 SUM= 1.0 01:SM_N 38743.00 35.071_MoDate 43:33 11.82 n/a .000
00458 SAVE HYD + 1.0 01:SM_N 38743.00 35.071_MoDate 43:33 11.82 n/a .000
00459 fname :R_H2M7
00460 remark:flow at R_N7: M7 + SK_7
00461 # Insertion of a Reservoir to simulate the effects of the Richmond Fen.
00462 # Storage area and volumes were estimated from available topo maps.
00463 # Release rate from Fen was assumed to be controlled by the downstream
00464 # river cross-section for subwatershed. If it was assumed that for up to
00465 # 0.75 m of water, the main channel of the river provided the storage. Above
00466 # this depth, the wetland stored a significantly store water.
00467 #
00468 R002/C00051 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00469 ROUTE CHANNEL -> 1.0 02:SM_N 38743.00 35.071_MoDate 43:33 11.82 n/a .000
00470 out<- 1.0 01:RES_RF 38743.00 23.265_MoDate 55:09 11.82 n/a .000
00471 [Med:0.05;Tol:0.0] out<- 1.0 01:RES_RF 38743.00 23.265_MoDate 55:09 11.82 n/a .000
00472 R002/C00052 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00473 SAVE HYD + 1.0 01:RES_RF 38743.00 23.265_MoDate 55:09 11.82 n/a .000
00474 fname :R_H2RESF
00475 remark:outflow of Richmond Fen
00476 #
00477 # Sum of hydrographs from Node 7 routed to Node 6
00478 # Section 5
00479 #
00480 R002/C00053 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00481 ROUTE CHANNEL -> 1.0 02:RES_RF 38743.00 23.265_MoDate 55:09 11.82 n/a .000
00482 [RPT= 1.00] out<- 1.0 01:SM 38743.00 23.228_MoDate 56:38 11.82 n/a .000
00483 [L/S/N= 306.6 / .087 / .040]
00484 [Vmax=.432;Dmax=.808]
00485 #
00486 # Addition of Subwatershed 6 and Van Gaal Drain to Node 6
00487 #
00488 R002/C00054 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00489 ADD HYD + 1.0 02:SM 38743.00 23.228_MoDate 56:38 11.82 n/a .000
00490 ROUTE CHANNEL -> 1.0 02:SM_N 1455.00 2.509_MoDate 28:36 13.24 n/a .000
00491 + 1.0 02:SM_CK 1332.00 3.148_MoDate 35:23 13.94 n/a .000
00492 SUM= 1.0 01:SM_N 40240.01 23.318_MoDate 39:59 11.89 n/a .000
00493 #
00494 # Sum of hydrographs from Node 6 routed to Node 5
00495 # Section 6
00496 #
00497 R002/C00055 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00498 ROUTE CHANNEL -> 1.0 02:SM_N 40240.01 23.318_MoDate 39:59 11.89 n/a .000
00499 [RPT= 1.00] out<- 1.0 01:SM 40240.01 23.288_MoDate 56:09 11.89 n/a .000
00500 [L/S/N= 306.6 / .057 / .040]
00501 [Vmax=.378;Dmax=.917]
00502 #
00503 # Addition of Subwatershed 5 and Flowing Creek to Node 5
00504 #
00505 R002/C00056 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00506 ADD HYD + 1.0 02:SM 40240.01 23.285_MoDate 56:09 11.89 n/a .000
00507 ROUTE CHANNEL -> 1.0 02:SM_N 224.94 0.257_MoDate 28:36 13.24 n/a .000
00508 + 1.0 02:SM_CK 4945.00 14.839_MoDate 33:25 14.57 n/a .000
00509 SUM= 1.0 01:SM_N 45409.01 33.166_MoDate 37:08 12.20 n/a .000
00510 #
00511 # Sum of hydrographs from Node 5 routed to Node 5A
00512 # Section 7
00513 #
00514 R002/C00057 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00515 ROUTE CHANNEL -> 1.0 02:SM_N 45409.01 33.166_MoDate 37:08 12.20 n/a .000
00516 [RPT= 1.00] out<- 1.0 01:SM 45409.01 33.125_MoDate 37:20 12.20 n/a .000
00517 [L/S/N= 556.7 / .097 / .040]
00518 [Vmax=.443;Dmax=.937]
00519 #
00520 # Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
00521 #
00522 R002/C00058 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00523 ADD HYD + 1.0 02:SM 45409.01 33.116_MoDate 37:20 12.20 n/a .000
00524 ROUTE CHANNEL -> 1.0 02:SM_N 162.00 0.309_MoDate 38:04 15.22 n/a .000
00525 + 1.0 02:SM_CK 1622.00 3.090_MoDate 38:04 15.22 n/a .000
00526 SUM= 1.0 01:SM_N 46841.01 36.216_MoDate 37:28 12.30 n/a .000
00527 #
00528 # Sum of hydrographs from Node 5A routed to Node 4
00529 # Section 8
00530 #
00531 R002/C00059 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00532 ROUTE CHANNEL -> 1.0 02:SM_N 46841.01 36.216_MoDate 37:28 12.30 n/a .000
00533 [RPT= 1.00] out<- 1.0 01:SM 46841.01 35.288_MoDate 39:22 12.30 n/a .000
00534 [L/S/N= 463.0 / .047 / .035]
00535 [Vmax=.695;Dmax=2.444]
00536 #
00537 # Addition of Subwatershed 4 and Leamy Creek to Node 4
00538 #
00539 R002/C00060 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00540 ADD HYD + 1.0 02:SM 46841.01 35.288_MoDate 39:22 12.30 n/a .000
00541 ROUTE CHANNEL -> 1.0 02:SM_N 585.00 4.325_MoDate 29:58 17.79 n/a .000
00542 + 1.0 02:SM_CK 1021.00 5.747_MoDate 30:50 17.79 n/a .000
00543 SUM= 1.0 01:SM_N 48447.00 37.581_MoDate 38:13 12.47 n/a .000
00544 R002/C00061 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00545 SAVE HYD + 1.0 01:SM_N 48447.00 37.581_MoDate 38:13 12.47 n/a .000
00546 fname :R_H4.0002
00547 remark:flow at R_N4
00548 #
00549 # Sum of hydrographs from Node 4 routed to Node 2
00550 # Section 9
00551 #
00552 R002/C00062 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00553 ROUTE CHANNEL -> 1.0 02:SM_N 48447.00 37.581_MoDate 38:13 12.47 n/a .000
00554 [RPT= 1.00] out<- 1.0 01:SM 48447.00 37.455_MoDate 38:19 12.47 n/a .000
00555 [L/S/N= 1467.7 / .067 / .040]
00556 [Vmax=.715;Dmax=2.845]
00557 #
00558 # Addition of Subwatershed 2 with Monahan Drain and Smith Drain to Node 2
00559 #
00560 R002/C00063 -----DtnIn-ID:HYD-----AREHA-QPEAFCms-TpeaDate,hh:mm-----RvM-R,C-----DWFCms
00561 ADD HYD + 1.0 02:SM 48447.00 37.455_MoDate 38:19 12.47 n/a .000


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01123# #
01124# R0002/C00157-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01125# ROUTE CHANNEL --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01126# [RSP: 1.0] out << 1.0 01:51M_PG 5377.82 47.599_MDate 35:11 12.82 n/a .000
01127# (L/S: 1183 / .076 / 035)
01128# [Vmax :.919;Dmax: 2.66]
01129# *****
01130# Catchment FORSTER
01131# To Foster ditch (north of the Dock)
01132# - Partially developed (medium density) remaining agricultural
01133# - 2020-12-01 OFRA Poster area is 332 ac per Foster MSW Environmental Study Report, CH2M Hill, Aug 2013.
01134# - 2020-12-01 decrease area to 173.88 ac (201_38 HA) after increasing drainage area to 5
01135# - 2021-02-12 update Foster area to 326.44 ac as measured from COIS
01136# *****
01137# R0002/C00158-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01138# CONTINUOUS STANDBY 1.0 01:51M_PG 325.44 13.697_MDate 28:15 28.56 .628 .000
01139# [XMP: 55;TMP: 65]
01140# [LGS: 2 CM: 74.0]
01141# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01142# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01143# [IARECLIP: 4.00; IARECP: 4.00]
01144# [SMN: 36.67; SMAK: 244.49; SK: .010]
01145# *****
01146# Foster Pond
01147# - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
01148# - and a ratio of the catchment area to the West Clarke pond rating curve
01149# - from the MSB for the next coordinates
01150# *****
01151# R0002/C00159-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01152# ROUTE CHANNEL --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01153# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01154# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01155# R0002/C00160-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01156# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01157# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01158# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01159# R0002/C00161-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01160# CONTINUOUS STANDBY 1.0 01:51M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01161# [XMP: 60;TMP: 65]
01162# [LGS: 2 CM: 74.0]
01163# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01164# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01165# [IARECLIP: 4.00; IARECP: 4.00]
01166# [SMN: 31.15; SMAK: 207.66; SK: .010]
01167# *****
01168# R0002/C00162-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01169# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01170# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01171# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01172# R0002/C00163-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01173# CONTINUOUS STANDBY 1.0 01:51M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01174# [XMP: 55;TMP: 65]
01175# [LGS: 2 CM: 74.0]
01176# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01177# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01178# [IARECLIP: 4.00; IARECP: 4.00]
01179# [SMN: 36.67; SMAK: 244.49; SK: .010]
01180# *****
01181# R0002/C00164-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01182# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01183# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01184# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01185# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01186# R0002/C00165-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01187# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01188# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01189# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01190# R0002/C00166-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01191# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01192# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01193# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01194# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01195# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01196# R0002/C00167-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01197# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01198# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01199# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01200# R0002/C00168-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01201# SAVE HYD --> 1.0 01:51M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01202# frame_980.0002
01203# remark:Total Flows at Station 980 to Node Drain
01204# *****
01205# Hydrograph from Node Foster SSM (Station 980) to Node at station 520
01206# Channel X-Section obtained from RWCA Hydraulic Model - Station 980
01207# *****
01208# R0002/C00169-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01209# ROUTE CHANNEL --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01210# [RSP: 1.0] out << 1.0 01:51M_PG 330.38 3.791_MDate 29:25 28.56 n/a .000
01211# (L/S: 1183 / .041 / 035)
01212# [Vmax :.544;Dmax: 1.992]
01213# *****
01214# R0002/C00170-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01215# CONTINUOUS STANDBY 1.0 01:51M_PG 330.38 3.791_MDate 29:25 28.56 n/a .000
01216# [XMP: 65;TMP: 65]
01217# [LGS: 2 CM: 74.0]
01218# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01219# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01220# [IARECLIP: 4.00; IARECP: 4.00]
01221# [SMN: 36.67; SMAK: 244.49; SK: .010]
01222# *****
01223# R0002/C00171-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01224# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01225# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01226# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01227# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01228# R0002/C00172-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01229# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01230# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01231# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01232# R0002/C00173-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01233# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01234# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01235# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01236# R0002/C00174-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01237# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01238# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01239# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01240# R0002/C00175-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01241# SAVE HYD --> 1.0 01:51M_PG 330.38 3.791_MDate 29:25 28.56 n/a .000
01242# frame_1520.0002
01243# remark:Total Flows at Station 520 to Node Drain
01244# Channel X-Section obtained from RWCA Hydraulic Model - Station 520
01245# *****
01246# R0002/C00176-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01247# ROUTE CHANNEL --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01248# [RSP: 1.0] out << 1.0 01:51M_PG 335.49 3.870_MDate 29:25 28.61 n/a .000
01249# (L/S: 1183 / .041 / 035)
01250# [Vmax :.544;Dmax: 1.992]
01251# *****
01252# R0002/C00177-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01253# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:25 28.61 n/a .000
01254# [XMP: 65;TMP: 65]
01255# [LGS: 2 CM: 74.0]
01256# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01257# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01258# [IARECLIP: 4.00; IARECP: 4.00]
01259# [SMN: 36.67; SMAK: 244.49; SK: .010]
01260# *****
01261# R0002/C00178-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01262# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01263# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01264# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01265# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01266# R0002/C00179-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01267# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01268# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01269# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01270# R0002/C00180-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01271# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01272# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01273# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01274# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01275# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01276# R0002/C00181-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01277# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:25 28.61 n/a .000
01278# [XMP: 52;TMP: 65]
01279# [LGS: 2 CM: 74.0]
01280# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01281# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01282# [IARECLIP: 4.00; IARECP: 4.00]
01283# [SMN: 36.67; SMAK: 244.49; SK: .010]
01284# *****
01285# R0002/C00182-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01286# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01287# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01288# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01289# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01290# R0002/C00183-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01291# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01292# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01293# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01294# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01295# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01296# R0002/C00184-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01297# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:25 28.61 n/a .000
01298# [XMP: 52;TMP: 65]
01299# [LGS: 2 CM: 74.0]
01300# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01301# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01302# [IARECLIP: 4.00; IARECP: 4.00]
01303# [SMN: 36.67; SMAK: 244.49; SK: .010]
01304# *****
01305# R0002/C00185-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01306# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:25 28.61 n/a .000
01307# [XMP: 77.0; TMP: 1.00]
01308# [LGS: 2 CM: 74.0]
01309# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01310# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01311# [IARECLIP: 4.00; IARECP: 4.00]
01312# [SMN: 36.67; SMAK: 244.49; SK: .010]
01313# *****
01314# R0002/C00187-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01315# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01316# overFlow << 1.0 02:51M_PG 335.49 3.870_MDate 29:32 28.61 n/a .000
01317# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01318# R0002/C00188-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01319# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:32 28.61 n/a .000
01320# [XMP: 65;TMP: 65]
01321# [LGS: 2 CM: 74.0]
01322# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01323# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01324# [IARECLIP: 4.00; IARECP: 4.00]
01325# [SMN: 36.67; SMAK: 244.49; SK: .010]
01326# *****
01327# R0002/C00189-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01328# ROUTE CHANNEL --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01329# overFlow << 1.0 02:51M_PG 335.49 3.870_MDate 29:32 28.61 n/a .000
01330# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01331# R0002/C00190-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01332# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:32 28.61 n/a .000
01333# [XMP: 65;TMP: 65]
01334# [LGS: 2 CM: 74.0]
01335# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01336# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01337# [IARECLIP: 4.00; IARECP: 4.00]
01338# [SMN: 36.67; SMAK: 244.49; SK: .010]
01339# *****
01340# R0002/C00191-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01341# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01342# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01343# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01344# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01345# R0002/C00192-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01346# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01347# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01348# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01349# R0002/C00193-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01350# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01351# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01352# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01353# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01354# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01355# R0002/C00194-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01356# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:32 28.61 n/a .000
01357# [XMP: 65;TMP: 65]
01358# [LGS: 2 CM: 74.0]
01359# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01360# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01361# [IARECLIP: 4.00; IARECP: 4.00]
01362# [SMN: 36.67; SMAK: 244.49; SK: .010]
01363# *****
01364# R0002/C00195-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01365# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01366# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01367# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01368# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01369# R0002/C00196-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01370# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01371# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01372# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01373# R0002/C00197-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01374# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01375# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01376# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01377# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01378# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01379# R0002/C00198-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01380# CONTINUOUS STANDBY 1.0 01:51M_PG 335.49 3.870_MDate 29:32 28.61 n/a .000
01381# [XMP: 65;TMP: 65]
01382# [LGS: 2 CM: 74.0]
01383# [Previous area: IArea: 4.67;SLP: 50.10;D: 40. MHP: 250;SCP: .0]
01384# [Impervious area: IAlp: 1.57;SLP: 50.10;MI: 181;MI: 013;SCT: .0]
01385# [IARECLIP: 4.00; IARECP: 4.00]
01386# [SMN: 36.67; SMAK: 244.49; SK: .010]
01387# *****
01388# R0002/C00199-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01389# COMPUTE DUALVD 1.0 01:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01390# Major System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01391# Minor System --> 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01392# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01393# R0002/C00200-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01394# ADD HYD --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01395# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01396# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01397# R0002/C00201-----DRAIN-ID:BYHDY-----AREHA-GPEARMS-TpeakDate,h_hmm-----Rvm-R,C-----DWPFMS
01398# ROUTE RESERVOIR --> 1.0 02:51M_PG 5377.82 47.629_MDate 34:57 12.82 n/a .000
01399# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01400# [MStoVol: 4.00; MStoVol: 0.000E+00; N-Ovf: 0; TotDurOvf: 0 hrs]
01401# overFlow << 1.0 03:15M_PG 325.44 13.697_MDate 29:12 28.56 n/a .000
01402# [MStoVol: 4.00; MStoVol: 0.000E+00;
```


014977 overflow << 1.0 03:02:00WVF 0.000 NoDate 0:00 0.00 n/a 0.000

014978 (MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

014979 014980

015000 R0002-C00211-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015001 ADD HYD 1.0 02:0E-CE 54118.36 49.269 NoDate 34:41 12.96 n/a 0.00

015002 + 1.0 02:0E-1-D4R 3.28 0.019 NoDate 29:16 32.20 n/a 0.00

015003 + 1.0 02:0E-1-D4Rovf 12.84 0.076 NoDate 29:23 32.20 n/a 0.00

015004 + 1.0 02:0E-1-D4Rovf 0.00 0.000 NoDate 0:00 0.00 n/a 0.00

015005 + 1.0 02:0E-1-D4Rovf 0.00 0.000 NoDate 0:00 0.00 n/a 0.00

015006 + 1.0 02:0E-2 139.40 49.816 NoDate 29:25 31.34 n/a 0.00

015007 + 1.0 02:0E-2 0.00 0.000 NoDate 0:00 0.00 n/a 0.00

015008 SUM 54253.88 49.833 NoDate 34:41 13.00 n/a 0.00

015009 R0002-C00212-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015010 SAVE HYD 1.0 01:0E-CE 54253.88 49.833 NoDate 34:41 13.00 n/a 0.00

015011 frame 18m_CE.0002

015012 remark:Total Flows before Station 5737 on Jack River

015013 # Channel X-Section obtained from RWCA Hydraulic Model - Station 5737

015014 # 2021-02-25 add section 5737 before station 5002. Station 5737 was extracted from the HEC-RAS model V7 (V9M1)474-16/0es

015015 # FFS# 2021-02-19 Change the slope at 474.01178 instead of 0.02583 to stabilize the model.

015016 R0002-C00219-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015017 ROUTE CHANNEL -> 1.0 02:0E-WC 54259.33 48.483 NoDate 37:44 13.01 n/a 0.00

015018 [L/S= 276 / 0.97 / 0.95]

015019 [R/S= 1.00] out<- 1.0 01:0E:P37 54253.88 48.835 NoDate 37:45 13.00 n/a 0.00

015020 (Vmax= 611.0max= 3.498)

015021 R0002-C00224-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015022 ADD HYD 1.0 02:0E-CE 54259.33 48.835 NoDate 37:45 13.00 n/a 0.00

015023 + 1.0 02:0E-1-D1R 21.67 0.126 NoDate 29:27 32.20 n/a 0.00

015024 + 1.0 02:0E-1-D4R 1.75 0.010 NoDate 29:24 32.20 n/a 0.00

015025 + 1.0 02:0E-1-D7R 2.03 0.012 NoDate 29:15 32.20 n/a 0.00

015026 + 1.0 02:0E-1-D7Rovf 0.00 0.000 NoDate 0:00 0.00 n/a 0.00

015027 (R/S= 1.00] out<- 1.0 01:0E:P37 54279.33 48.483 NoDate 37:44 13.01 n/a 0.00

015028 + 1.0 02:0E-1-D1Rovf 0.00 0.000 NoDate 0:00 0.00 n/a 0.00

015029 SUM 54279.33 48.483 NoDate 37:44 13.01 n/a 0.00

015030 R0002-C00221-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015031 SAVE HYD 1.0 01:0E:CE 54279.33 48.483 NoDate 37:44 13.01 n/a 0.00

015032 frame 15002.0002

015033 remark:Total Flows before Station 5002 on Jack River

015034 # Hydrograph from Node output to Node at West Clarke Drain

015035 # Channel X-Section obtained from RWCA Hydraulic Model - Station 5002

015036 # FFS# 2021-02-19 Change the slope at 502.0 to 0.02585 so the model will be more stable

015037 # FFS# 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one with 825 m length so the model will

015038 # FFS# 2021-02-26 change the length of ROUTE channel from 825 m to 786 m. That is because of adding station 5737 be

015039 ROUTE CHANNEL -> 1.0 02:0E:W 54279.33 48.483 NoDate 37:44 13.01 n/a 0.00

015040 (L/S= 245 / 0.95 / 0.95)

015041 [R/S= 1.00] out<- 1.0 01:0E:W 54279.33 48.484 NoDate 37:20 13.01 n/a 0.00

015042 [L/S= 245 / 0.95 / 0.95]

015043 frame 9612.0000

015044 R0002-C00223-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015045 ROUTE CHANNEL -> 1.0 02:0E:Wca 54279.33 48.484 NoDate 37:20 13.01 n/a 0.00

015046 (L/S= 245 / 0.95 / 0.95)

015047 [R/S= 245 / 0.95 / 0.95]

015048 frame 9612.0000

015049 R0002-C00224-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015050 ROUTE CHANNEL -> 1.0 02:0E:W 54279.33 48.484 NoDate 37:16 13.01 n/a 0.00

015051 (L/S= 245 / 0.95 / 0.95)

015052 [R/S= 245 / 0.95 / 0.95]

015053 frame 9612.0000

015054 # Hydrograph from Node output to Node at West Drain

015055 # Channel X-Section obtained from RWCA Hydraulic Model - Station 4534

015056 # FFS# 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one with 825 m length so the model will

015057 ROUTE CHANNEL -> 1.0 02:0E:W 54279.33 48.460 NoDate 36:26 13.01 n/a 0.00

015058 (L/S= 1020 / 0.95 / 0.95)

015059 [R/S= 1020 / 0.95 / 0.95]

015060 frame 1020.0000

015061 [L/S= 1020 / 0.95 / 0.95]

015062 [R/S= 1020 / 0.95 / 0.95]

015063 # Catchment KEN_RJ

015064 # To Kennedy-Burnett SSM Facility

015065 # - Outlet to Fraser-Clarke drain (north of the dock)

015066 # Minimum velocity reduced to 0.1 (as per Station Report 2002) to 0.02585 so the model will be more stable

015067 # Existing Kennedy-Burnett SSM Facility

015068 # Rating curve obtained from OTRP&B

015069 # Tributary drainage length 160 ha

015070 # Existing Kennedy-Burnett SSM Facility

015071 # - Rating curve obtained from OTRP&B

015072 # Tributary drainage length 160 ha

015073 # Existing Kennedy-Burnett SSM Facility

015074 R0002-C00228-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015075 CONTINUOUS STANDVHD 1.0 01:0E-01A 40.82 1.627 NoDate 28:18 14.35 315 0.00

015076 [XIMP= 10-TIMP=84]

015077 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015078 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015079 (Impervious area: IAimp= 79.5;L1= 40.0;M= 250;I=0.0)

015080 [IaRcp= 4.00; IaRcp= 4.00]

015081 R0002-C00229-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015082 COMPUTE DUALHD 1.0 01:0E-01A 40.82 1.627 NoDate 28:18 14.35 n/a 0.00

015083 Major System / 1.0 02:0E-D1A-MJ 0.00 0.000 NoDate 0:00 0.00 n/a 0.00

015084 Minor System / 1.0 02:0E-D1A-MN 40.778 1.627 NoDate 28:18 14.35 n/a 0.00

015085 [MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

015086 R0002-C00228-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015087 ADD HYD 1.0 02:0E-D1A-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015088 + 1.0 02:0E-D1A-MN 40.778 1.627 NoDate 28:18 14.35 n/a 0.00

015089 + 1.0 02:0E-D1A-M 40.82 1.627 NoDate 28:18 14.35 n/a 0.00

015090 SUM 54279.33 48.460 NoDate 36:26 13.01 n/a 0.00

015091 CONTINUOUS STANDVHD 1.0 01:0E-01A 40.82 1.627 NoDate 28:08 16.28 388 0.00

015092 [XIMP= 19-TIMP=38]

015093 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015094 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015095 (Impervious area: IAimp= 79.5;L1= 42.0;M= 455;M= 0.13;I=0.0)

015096 [IaRcp= 4.00; IaRcp= 4.00]

015097 R0002-C00230-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015098 COMPUTE DUALHD 1.0 01:0E-01A 40.82 1.627 NoDate 28:08 16.28 n/a 0.00

015099 Major System / 1.0 02:0E-D1A-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015100 Minor System / 1.0 02:0E-D1A-MN 40.778 1.627 NoDate 28:08 16.28 n/a 0.00

015101 [MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

015102 R0002-C00231-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015103 CONTINUOUS STANDVHD 1.0 01:0E-01C 31.10 1.062 NoDate 28:08 16.28 n/a 0.00

015104 [XIMP= 20-TIMP=84]

015105 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015106 (Previous area: IAmp= 4.67;SLP=2.0;L1= 30.0;M= 210;I=0.0)

015107 (Impervious area: IAimp= 79.5;L1= 40.0;M= 250;I=0.0)

015108 [IaRcp= 4.00; IaRcp= 4.00]

015109 R0002-C00233-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015110 COMPUTE DUALHD 1.0 01:0E-01C 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015111 Major System / 1.0 02:0E-D1C-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015112 Minor System / 1.0 02:0E-D1C-MN 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015113 [MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

015114 R0002-C00232-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015115 CONTINUOUS STANDVHD 1.0 01:0E-01C 13.78 0.776 NoDate 28:06 17.09 375 0.00

015116 [XIMP= 20-TIMP=84]

015117 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015118 (Previous area: IAmp= 4.67;SLP=2.0;L1= 30.0;M= 210;I=0.0)

015119 (Impervious area: IAimp= 79.5;L1= 40.0;M= 250;I=0.0)

015120 [IaRcp= 4.00; IaRcp= 4.00]

015121 R0002-C00233-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015122 COMPUTE DUALHD 1.0 01:0E-01C 13.78 0.776 NoDate 28:06 17.09 n/a 0.00

015123 Major System / 1.0 02:0E-D1C-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015124 Minor System / 1.0 02:0E-D1C-MN 13.78 0.776 NoDate 28:06 17.09 n/a 0.00

015125 [MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

015126 R0002-C00234-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015127 ADD HYD 1.0 02:0E-D1C-M 13.78 0.776 NoDate 28:06 17.09 n/a 0.00

015128 + 1.0 02:0E-D1C-MN 13.78 0.776 NoDate 28:06 17.09 n/a 0.00

015129 SUM 54279.33 48.460 NoDate 36:26 13.01 n/a 0.00

015130 CONTINUOUS STANDVHD 1.0 01:0E-01C 84.78 3.788 NoDate 28:12 16.73 368 0.00

015131 [XIMP= 19-TIMP=38]

015132 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015133 (Previous area: IAmp= 4.67;SLP=2.0;L1= 30.0;M= 210;I=0.0)

015134 (Impervious area: IAimp= 79.5;L1= 40.0;M= 250;I=0.0)

015135 [IaRcp= 4.00; IaRcp= 4.00]

015136 R0002-C00237-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015137 ADD HYD 1.0 02:0E-D1M-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015138 + 1.0 02:0E-D1M-MN 84.78 3.788 NoDate 28:12 16.73 n/a 0.00

015139 + 1.0 02:0E-D1M 84.78 3.788 NoDate 28:12 16.73 n/a 0.00

015140 SUM 54279.33 48.460 NoDate 36:26 13.01 n/a 0.00

015141 CONTINUOUS STANDVHD 1.0 01:0E-03 84.78 3.788 NoDate 28:12 16.73 368 0.00

015142 [XIMP= 19-TIMP=38]

015143 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015144 (Previous area: IAmp= 4.67;SLP=2.0;L1= 30.0;M= 210;I=0.0)

015145 (Impervious area: IAimp= 79.5;L1= 40.0;M= 250;I=0.0)

015146 [IaRcp= 4.00; IaRcp= 4.00]

015147 R0002-C00239-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015148 COMPUTE DUALHD 1.0 01:0E-03 84.78 3.788 NoDate 28:12 16.73 n/a 0.00

015149 Major System / 1.0 02:0E-D3-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015150 Minor System / 1.0 02:0E-D3-MN 84.78 3.788 NoDate 28:12 16.73 n/a 0.00

015151 [MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

015152 R0002-C00237-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015153 ADD HYD 1.0 02:0E-D3-M 84.78 3.788 NoDate 28:12 16.73 n/a 0.00

015154 + 1.0 02:0E-D3-MN 84.78 3.788 NoDate 28:12 16.73 n/a 0.00

015155 SUM 54279.33 48.460 NoDate 36:26 13.01 n/a 0.00

015156 CONTINUOUS STANDVHD 1.0 01:0E-03 84.78 3.788 NoDate 28:12 16.73 368 0.00

015157 [XIMP= 19-TIMP=38]

015158 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015159 (Previous area: IAmp= 4.67;SLP=2.0;L1= 30.0;M= 210;I=0.0)

015160 (Impervious area: IAimp= 79.5;L1= 40.0;M= 250;I=0.0)

015161 [IaRcp= 4.00; IaRcp= 4.00]

015162 R0002-C00242-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015163 CONTINUOUS STANDVHD 1.0 01:0E-06 12.93 1.683 NoDate 28:00 39.68 872 0.00

015164 [XIMP= 87-TIMP=87]

015165 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015166 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015167 (Impervious area: IAimp= 94.5;L1= 4.75;L1= 294.0;M= 0.13;I=0.0)

015168 [IaRcp= 4.00; IaRcp= 4.00]

015169 R0002-C00243-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015170 COMPUTE DUALHD 1.0 01:0E-06 12.93 1.683 NoDate 28:00 39.68 n/a 0.00

015171 Major System / 1.0 03:0E-06-MJ 0.00 0.000 NoDate 0:00 n/a 0.00

015172 Minor System / 1.0 03:0E-06-MN 12.93 1.683 NoDate 28:00 39.68 n/a 0.00

015173 [MjSysto=0.000E+00, TotVolVol=0.000E+00, N=0v=0, TotDurDvf=0 hrs)

015174 R0002-C00244-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015175 ADD HYD 1.0 03:0E-06-M 12.93 1.683 NoDate 28:00 39.68 n/a 0.00

015176 + 1.0 03:0E-06-MN 12.93 1.683 NoDate 28:00 39.68 n/a 0.00

015177 SUM 54279.33 48.460 NoDate 36:26 13.01 n/a 0.00

015178 CONTINUOUS STANDVHD 1.0 01:0E-11 4.03 0.425 NoDate 28:00 32.17 707 0.00

015179 [XIMP= 70-TIMP=84]

015180 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015181 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015182 (Impervious area: IAimp= 79.5;L1= 2.0;L1= 164.0;M= 0.13;I=0.0)

015183 [IaRcp= 4.00; IaRcp= 4.00]

015184 R0002-C00245-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015185 CONTINUOUS STANDVHD 1.0 01:0E-11 4.03 0.425 NoDate 28:00 32.17 707 0.00

015186 [XIMP= 70-TIMP=84]

015187 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015188 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015189 (Impervious area: IAimp= 79.5;L1= 2.0;L1= 164.0;M= 0.13;I=0.0)

015190 [IaRcp= 4.00; IaRcp= 4.00]

015191 R0002-C00246-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015192 CONTINUOUS STANDVHD 1.0 01:0E-15 2.15 0.241 NoDate 28:00 37.11 815 0.00

015193 [XIMP= 70-TIMP=84]

015194 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015195 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015196 (Impervious area: IAimp= 16.5;L1= 120.0;L1= 120.0;M= 0.13;I=0.0)

015197 [IaRcp= 4.00; IaRcp= 4.00]

015198 R0002-C00249-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015199 CONTINUOUS STANDVHD 1.0 01:0E-15 2.15 0.241 NoDate 28:00 37.11 815 0.00

015200 [XIMP= 70-TIMP=84]

015201 (Horton parameters: Pow= 76.20;Pc= 13.20;DCAV=4.14; P= 0.0)

015202 (Previous area: IAmp= 4.67;SLP=2.0;L1= 40.0;M= 250;I=0.0)

015203 (Impervious area: IAimp= 16.5;L1= 120.0;L1= 120.0;M= 0.13;I=0.0)

015204 [IaRcp= 4.00; IaRcp= 4.00]

015205 R0002-C00250-----DtnIn-ID:HVND-----AREBA-GPEARCS-TpeakDate_hh:mm-----RvM-R-C-----DWPMCS

015206 ADD HYD 1.0 02:0E-D1A-0 40.82 1.627 NoDate 28:18 14.35 n/a 0.00

015207 + 1.0 02:0E-D1A-0M 31.10 1.062 NoDate 28:08 16.28 n/a 0.00

015208 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015209 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015210 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015211 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015212 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015213 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015214 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015215 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015216 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015217 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015218 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015219 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015220 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015221 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015222 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015223 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015224 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015225 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015226 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015227 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015228 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015229 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015230 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015231 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015232 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015233 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015234 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015235 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015236 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015237 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015238 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015239 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015240 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015241 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015242 + 1.0 02:0E-D1A-0M 31.78 0.776 NoDate 28:06 17.09 n/a 0.00

015243 + 1.0 02:0E-D1A

Table with columns: ID, Description, Parameters, Values, Units, and Status. The table contains detailed system configuration data for various components like pumps, valves, and flow meters, organized into sections such as R0002-C00368 and R0002-C00369.


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02993 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
02994 # of 1.52
02995 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
02996 [RPT= 1.00] out<- 1.0 02:18M11 9506.00 10.383 No.Date 33:07 15.96 n/a .000
02997 [L/S/n= 972./ .044/.040]
02998 [Vmax= 648/Imax= 2.406]
02999
03000 #
03001 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03002 # of 3.75
03003 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.383 No.Date 33:07 15.96 n/a .000
03004 CONTINUOUS NASHYD 1.0 01:18M_CK 8676.00 16.342 No.Date 39:59 17.18 .000
03005 [Cm 72.0/ N= 3.00/ Tp= 1.32]
03006 [AREC 4.0/0/ SMIN= 39.75/ SMAX=264.99/ EK= .010]
03007 [InterVTime= 12.00]
03008 #
03009 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03010 # of 1.68
03011 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03012 CONTINUOUS NASHYD 1.0 01:18M_9 1132.00 6.963 No.Date 30:55 19.24 .337 .000
03013 [Cm 70.0/ N= 3.00/ Tp= 2.51]
03014 [AREC 4.0/0/ SMIN= 41.07/ SMAX=350.79/ EK= .010]
03015 [InterVTime= 12.00]
03016 #
03017 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03018 # of 1.82
03019 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03020 CONTINUOUS NASHYD 1.0 01:18M_CK 4464.00 8.109 No.Date 39:59 15.66 .274 .000
03021 [Cm 62.0/ N= 3.00/ Tp= 1.32]
03022 [AREC 4.0/0/ SMIN= 61.90/ SMAX=412.66/ EK= .010]
03023 [InterVTime= 12.00]
03024 #
03025 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03026 # of 1.80
03027 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03028 CONTINUOUS NASHYD 1.0 01:18M_8 131.00 1.298 No.Date 28:57 16.03 .281 .000
03029 [Cm 61.0/ N= 3.00/ Tp= .90]
03030 [AREC 4.0/0/ SMIN= 55.42/ SMAX=396.11/ EK= .010]
03031 [InterVTime= 12.00]
03032 #
03033 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03034 # of 1.85
03035 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03036 CONTINUOUS NASHYD 1.0 01:18M_DR 3854.00 9.385 No.Date 38:41 17.18 .301 .000
03037 [Cm 70.0/ N= 3.00/ Tp= 1.88]
03038 [AREC 4.0/0/ SMIN= 52.62/ SMAX=350.79/ EK= .010]
03039 [InterVTime= 12.00]
03040 #
03041 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03042 # of 1.82
03043 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03044 CONTINUOUS NASHYD 1.0 01:18M_CK 3197.00 7.027 No.Date 36:28 13.49 .243 .000
03045 [Cm 57.0/ N= 3.00/ Tp= 6.65]
03046 [AREC 4.0/0/ SMIN= 50.81/ SMAX=508.81/ EK= .010]
03047 [InterVTime= 12.00]
03048 #
03049 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03050 # of 3.75
03051 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03052 CONTINUOUS NASHYD 1.0 01:18M_6 165.00 .641 No.Date 33:06 17.58 .308 .000
03053 [Cm 47.0/ N= 3.00/ Tp= 1.88]
03054 [AREC 4.0/0/ SMIN= 50.55/ SMAX=336.97/ EK= .010]
03055 [InterVTime= 12.00]
03056 #
03057 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03058 # of 1.87
03059 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03060 CONTINUOUS NASHYD 1.0 01:18M_DR 1132.00 4.803 No.Date 35:19 20.12 .352 .000
03061 [Cm 72.0/ N= 3.00/ Tp= 8.90]
03062 [AREC 4.0/0/ SMIN= 39.75/ SMAX=264.99/ EK= .010]
03063 [InterVTime= 12.00]
03064 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03065 CONTINUOUS NASHYD 1.0 01:18M_5 224.00 4.100 No.Date 28:45 22.97 .402 .000
03066 [Cm 77.0/ N= 3.00/ Tp= .75]
03067 [AREC 4.0/0/ SMIN= 31.15/ SMAX=207.66/ EK= .010]
03068 [InterVTime= 12.00]
03069 #
03070 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03071 # of 1.50
03072 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03073 CONTINUOUS NASHYD 1.0 01:18M_CK 4945.00 22.837 No.Date 33:22 21.04 .368 .000
03074 [Cm 74.0/ N= 3.00/ Tp= 4.45]
03075 [AREC 4.0/0/ SMIN= 42.47/ SMAX=244.49/ EK= .010]
03076 [InterVTime= 12.00]
03077 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03078 CONTINUOUS NASHYD 1.0 01:18M_2 20.00 .483 No.Date 28:36 25.62 .448 .000
03079 [Cm 81.0/ N= 3.00/ Tp= .62]
03080 [AREC 4.0/0/ SMIN= 25.21/ SMAX=168.09/ EK= .010]
03081 [InterVTime= 12.00]
03082 #
03083 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
03084 # of 1.61
03085 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03086 CONTINUOUS NASHYD 1.0 01:18M_SAI 1412.00 4.646 No.Date 37:58 21.98 .385 .000
03087 [Cm 70.0/ N= 3.00/ Tp= 8.90]
03088 [AREC 4.0/0/ SMIN= 31.81/ SMAX=225.43/ EK= .010]
03089 [InterVTime= 12.00]
03090 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03091 CONTINUOUS NASHYD 1.0 01:18M_4 585.00 6.888 No.Date 29:57 25.62 .448 .000
03092 [Cm 81.0/ N= 3.00/ Tp= 2.66]
03093 [AREC 4.0/0/ SMIN= 25.21/ SMAX=168.09/ EK= .010]
03094 [InterVTime= 12.00]
03095 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03096 CONTINUOUS NASHYD 1.0 01:18M_CK 1021.00 8.861 No.Date 30:48 25.07 .439 .000
03097 [Cm 80.0/ N= 3.00/ Tp= 2.46]
03098 [AREC 4.0/0/ SMIN= 26.32/ SMAX=175.50/ EK= .010]
03099 [InterVTime= 12.00]
03100 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03101 CONTINUOUS NASHYD 1.0 01:18M_3 177.00 3.240 No.Date 28:45 22.97 .402 .000
03102 [Cm 77.0/ N= 3.00/ Tp= .75]
03103 [AREC 4.0/0/ SMIN= 31.15/ SMAX=207.66/ EK= .010]
03104 [InterVTime= 12.00]
03105 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03106 CONTINUOUS NASHYD 1.0 01:18M_2 1132.00 8.265 No.Date 31:48 25.62 .448 .000
03107 [Cm 81.0/ N= 3.00/ Tp= 3.25]
03108 [AREC 4.0/0/ SMIN= 31.15/ SMAX=168.09/ EK= .010]
03109 [InterVTime= 12.00]
03110 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03111 CONTINUOUS NASHYD 1.0 01:18M_CK 2737.00 17.459 No.Date 31:33 22.47 .393 .000
03112 [Cm 76.0/ N= 3.00/ Tp= 3.03]
03113 [AREC 4.0/0/ SMIN= 31.15/ SMAX=216.39/ EK= .010]
03114 [InterVTime= 12.00]
03115 #
03116 Routing hydrographs
03117 #
03118 Starting with the addition of Jock River Headwater and Subwatershed 13
03119 #
03120 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03121 ADD HYD + 1.0 02:18M_3M 3680.00 9.398 No.Date 37:02 16.41 n/a .000
03122 SUM + 1.0 01:18M_CK 3874.00 13.405 No.Date 32:16 15.29 n/a .000
03123 SUM + 1.0 01:18M_13 4651.00 11.949 No.Date 35:33 16.37 n/a .000
03124 #
03125 Sum of hydrographs from Node 13 routed to Node 13A
03126 [Approximated cross-section - see cross-section 258]
03127 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
03128 #
03129 ROUTE CHANNEL -> 1.0 01:18M_13A 4651.00 9.514 No.Date 39:57 16.17 n/a .000
03130 [RPT= 1.00] out<- 1.0 01:18M_13A 4651.00 9.514 No.Date 39:57 16.17 n/a .000
03131 [L/S/n= 5074./ .027/.040]
03132 [Vmax= 478/Imax= 3.020]
03133 #
03134 Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
03135 #
03136 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03137 ADD HYD + 1.0 02:18M_13A 4651.00 9.514 No.Date 39:57 16.17 n/a .000
03138 SUM + 1.0 01:18M_13A 7725.00 14.196 No.Date 39:59 15.00 n/a .000
03139 [AREC 4.0/0/ SMIN= 3974.00 4.682 No.Date 39:59 13.23 n/a .000]
03140 #
03141 Insertion of a reservoir to simulate the effects of the Goodwood Marsh
03142 #
03143 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03144 ROUTE RESERVOIR -> 1.0 02:18M_13A 7725.00 14.196 No.Date 39:59 15.00 n/a .000
03145 [Mkt=0.05d=.631E+02 n3]
03146 #
03147 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03148 SAVE HYD + 1.0 01:18M_13A 7725.00 3.149 No.Date 57:25 15.00 n/a .000
03149 [Mkt=0.05d=.631E+02 n3]
03150 #
03151 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03152 frame_H_RSEM 1.0 01:18M_13A 7725.00 3.149 No.Date 57:25 15.00 n/a .000
03153 remark:flow at Res Out
03154 #
03155 Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
03156 [Approximated cross-section - see cross-section 258]
03157 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
03158 ROUTE CHANNEL -> 1.0 02:18M12 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03159 [RPT= 1.00] out<- 1.0 02:18M12 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03160 [L/S/n= 5926./ .076/.040]
03161 [Vmax= 527/Imax= 1.429]
03162 #
03163 Addition of Subwatershed Jock River at Ashton to Node 12
03164 #
03165 ROUTE CHANNEL -> 1.0 02:18M12 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03166 ADD HYD + 1.0 02:18M_ASH 1781.00 8.521 No.Date 32:43 20.12 n/a .000
03167 SUM + 1.0 02:18M12 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03168 [AREC 4.0/0/ SMIN= 3974.00 4.682 No.Date 39:59 13.23 n/a .000]
03169 ROUTE CHANNEL -> 1.0 02:18M12 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03170 frame_H_RN12 1.0 01:18M12 9506.00 10.498 No.Date 32:46 15.96 n/a .000
03171 remark:flow at R_N12 near Ashton
03172 #
03173 Sum of hydrographs from Node 12 routed to Node 11
03174 [Approximated cross-section - see cross-section 258]
03175 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
03176 #
03177 Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
03178 #
03179 ROUTE CHANNEL -> 1.0 02:18M11 9506.00 10.498 No.Date 32:46 15.96 n/a .000

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Table with columns for ID, description, parameters, and values. The table lists various engineering specifications and calculations, including flow rates, pressures, and system configurations. It includes sub-sections like 'CONTINUOUS STANDBY', 'COMPUTE DUALHDY', and 'PROPOSED Subcatchments'.


```

05611 # - To Jock River (north of the Jock)
05612 # Rural/area subdivided (Desire Community)
05613 #*****
05614 #*****
05615 CONTINUOUS STANBYD 1.0 01:DESIRE 23.78 1.359 No.Date 28:03 27.22 477 000
05616 [XMP= 25.7IMP= 25]
05617 [LQSS= 2 CN= 77.0]
05618 [Previous area Iaper= 4.67;SLP1=0.0;LID= 4.0;MND= 250;RCP= 0]
05619 [Impervious area Iaper= 1.57;SLP1=0.0;LID= 4.0;MND= 013;RCP= 0]
05620 [IareClmp= 4.00;IareKper= 4.00]
05621 [SMD= 15.0]
05622 [SMD= 15.0]
05623 # Catchment JOCKVA
05624 # To Jockvale SWM Facility
05625 # - Residential development & golf course
05626 # - JFSA 2021-11 updated JOCKVA area updating CONSIG as per IRI GROUP, July 2008.
05627 # JOCKVA area became 235.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX-LAND 32.5 ha as
05628 #*****
05629 #*****
05630 CONTINUOUS STANBYD 1.0 01:JOCKVA 225.13 14.675 No.Date 28:09 35.73 626 000
05631 [XMP= 50.7IMP= 50]
05632 [LQSS= 2 CN= 74.0]
05633 [Previous area Iaper= 4.67;SLP1=0.0;LID= 4.0;MND= 250;RCP= 0]
05634 [Impervious area Iaper= 1.57;SLP1=0.0;LID= 4.0;MND= 013;RCP= 0]
05635 [IareClmp= 4.00;IareKper= 4.00]
05636 [SMD= 35.0;SMD= 44.9;SK= 010]
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05989# [Approximated cross-section - see cross-section 258]
05990# Use tw/4 for summer conditions and tw/2 for spring conditions
05991#
05992# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
05993#
05994# ROUTE CHANNEL -> 1.0 02:18_M12 9506.00 12.434 No.Date 31:45 19.65 n/a .000
05995# [RDT= 1.00] out<- 1.0 01:01:M11 9506.00 12.710 No.Date 33:02 19.65 n/a .000
05996# [L/S= 10428.7 / .157/180]
05997# [Vmax= .680;Dmax= 2.598]
05998#
05999# Addition of Subwatershed 11 and No Name Creek to Node 11
06000#
06001# ROUTE CHANNEL -> 1.0 02:18_M11 11923.00 21.813 No.Date 33:05 19.96 n/a .000
06002# [RDT= 1.00] out<- 1.0 01:01:M10 11923.00 14.761 No.Date 39:58 19.96 n/a .000
06003# [L/S= 10428.7 / .157/180]
06004# [Vmax= .452;Dmax= 1.212]
06005#
06006# Addition of Subwatershed 10 to Node 10
06007#
06008# ROUTE CHANNEL -> 1.0 02:18_M11 11923.00 21.813 No.Date 33:05 19.96 n/a .000
06009# [RDT= 1.00] out<- 1.0 01:01:M10 11923.00 14.761 No.Date 39:58 19.96 n/a .000
06010# [L/S= 10428.7 / .157/180]
06011# [Vmax= .452;Dmax= 1.212]
06012#
06013# Addition of Subwatershed 10 to Node 10
06014#
06015# ROUTE CHANNEL -> 1.0 02:18_M11 11923.00 21.813 No.Date 33:05 19.96 n/a .000
06016# ADD HYD + 1.0 02:18_M10 11923.00 14.761 No.Date 39:58 19.96 n/a .000
06017# SUM= 1.0 02:18_M11 11923.00 21.813 No.Date 33:05 19.96 n/a .000
06018#
06019# ROUTE CHANNEL -> 1.0 02:18_M10 17899.00 35.808 No.Date 38:35 21.52 n/a .000
06020# [RDT= 1.00] out<- 1.0 01:01:M10 17899.00 35.808 No.Date 38:35 21.52 n/a .000
06021# [L/S= 10428.7 / .157/180]
06022# [Vmax= .680;Dmax= 2.598]
06023#
06024# frame_H_RUNID
06025# remark:flow at RN10: M10 + SW_10
06026#
06027# Addition of Kings Creek to S_M10
06028#
06029# ROUTE CHANNEL -> 1.0 02:18_M10 17899.00 35.808 No.Date 38:35 21.52 n/a .000
06030# ADD HYD + 1.0 02:18_M10 8376.00 35.808 No.Date 38:35 21.52 n/a .000
06031# SUM= 1.0 02:18_M10 25665.00 55.807 No.Date 39:58 21.41 n/a .000
06032#
06033# Sum of hydrographs from Node 10 routed to Node 9
06034#
06035# ROUTE CHANNEL -> 1.0 02:18_M10A 25665.00 54.076 No.Date 39:59 21.41 n/a .000
06036# [RDT= 1.00] out<- 1.0 01:01:M10 25665.00 54.076 No.Date 39:59 21.41 n/a .000
06037# [L/S= 10428.7 / .157/180]
06038# [Vmax= .682;Dmax= 1.695]
06039#
06040# Addition of Subwatershed 9 and Nichols Creek to Node 9
06041#
06042# ROUTE CHANNEL -> 1.0 02:18_M10 25665.00 54.076 No.Date 39:59 21.41 n/a .000
06043# ADD HYD + 1.0 02:18_M9 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06044# SUM= 1.0 02:18_M10 57326.00 120.360 No.Date 39:59 21.20 n/a .000
06045# [L/S= 10428.7 / .157/180]
06046# [Vmax= .363;Dmax= 1.619]
06047#
06048# Sum of hydrographs from Node 9 routed to Node 8
06049#
06050# ROUTE CHANNEL -> 1.0 02:18_M9 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06051# [RDT= 1.00] out<- 1.0 01:01:M10 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06052# [L/S= 10428.7 / .157/180]
06053# [Vmax= .363;Dmax= 1.619]
06054#
06055# Addition of Subwatershed 8 and Robb's Drain to Node 8
06056#
06057# ROUTE CHANNEL -> 1.0 02:18_M9 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06058# ADD HYD + 1.0 02:18_M8 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06059# SUM= 1.0 02:18_M9 63322.00 127.767 No.Date 39:59 21.20 n/a .000
06060# [L/S= 10428.7 / .157/180]
06061# [Vmax= .363;Dmax= 1.619]
06062#
06063# Sum of hydrographs from Node 8 routed to Node 7
06064#
06065# ROUTE CHANNEL -> 1.0 02:18_M8 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06066# [RDT= 1.00] out<- 1.0 01:01:M10 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06067# [L/S= 10428.7 / .157/180]
06068# [Vmax= .363;Dmax= 1.619]
06069#
06070# Addition of Subwatershed 7 to Node 7
06071#
06072# ROUTE CHANNEL -> 1.0 02:18_M8 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06073# ADD HYD + 1.0 02:18_M7 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06074# SUM= 1.0 02:18_M8 63322.00 127.767 No.Date 39:59 21.20 n/a .000
06075# [L/S= 10428.7 / .157/180]
06076# [Vmax= .363;Dmax= 1.619]
06077#
06078# Sum of hydrographs from Node 7 routed to Node 6
06079#
06080# ROUTE CHANNEL -> 1.0 02:18_M7 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06081# [RDT= 1.00] out<- 1.0 01:01:M10 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06082# [L/S= 10428.7 / .157/180]
06083# [Vmax= .363;Dmax= 1.619]
06084#
06085# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
06086#
06087# ROUTE CHANNEL -> 1.0 02:18_M7 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06088# ADD HYD + 1.0 02:18_M6 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06089# SUM= 1.0 02:18_M7 63322.00 127.767 No.Date 39:59 21.20 n/a .000
06090# [L/S= 10428.7 / .157/180]
06091# [Vmax= .363;Dmax= 1.619]
06092#
06093# Sum of hydrographs from Node 6 routed to Node 5
06094#
06095# ROUTE CHANNEL -> 1.0 02:18_M6 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06096# [RDT= 1.00] out<- 1.0 01:01:M10 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06097# [L/S= 10428.7 / .157/180]
06098# [Vmax= .363;Dmax= 1.619]
06099#
06100# Addition of Subwatershed 5 and Flooding Creek to Node 5
06101#
06102# ROUTE CHANNEL -> 1.0 02:18_M6 31661.00 66.284 No.Date 39:59 21.20 n/a .000
06103# [RDT= 1.00] out<- 1.0 01:01:M10 31661.00 61.483 No.Date 39:57 21.20 n/a .000
06104# [L/S= 10428.7 / .157/180]
06105# [Vmax= .363;Dmax= 1.619]
06106#
06107# Sum of hydrographs from Node 5 routed to Node 4
06108#
06109# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06110# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06111# [L/S= 556.7 / .090/400]
06112# [Vmax= .485;Dmax= 1.131]
06113#
06114# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
06115#
06116# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06117# ADD HYD + 1.0 02:18_M4 585.00 8.458 No.Date 29:57 31.37 n/a .000
06118# SUM= 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06119# [L/S= 556.7 / .090/400]
06120# [Vmax= .485;Dmax= 1.131]
06121#
06122# Sum of hydrographs from Node 5A routed to Node 4
06123#
06124# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06125# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06126# [L/S= 556.7 / .090/400]
06127# [Vmax= .485;Dmax= 1.131]
06128#
06129# Addition of Subwatershed 4 and Leamy Creek to Node 4
06130#
06131# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06132# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06133# [L/S= 556.7 / .090/400]
06134# [Vmax= .485;Dmax= 1.131]
06135#
06136# Sum of hydrographs from Node 4 routed to Node 3
06137#
06138# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06139# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06140# [L/S= 556.7 / .090/400]
06141# [Vmax= .485;Dmax= 1.131]
06142#
06143# Addition of Subwatershed 3 and Leamy Creek to Node 3
06144#
06145# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06146# ADD HYD + 1.0 02:18_M4 585.00 8.458 No.Date 29:57 31.37 n/a .000
06147# SUM= 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06148# [L/S= 556.7 / .090/400]
06149# [Vmax= .485;Dmax= 1.131]
06150#
06151# Sum of hydrographs from Node 3 routed to Node 2
06152#
06153# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06154# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06155# [L/S= 556.7 / .090/400]
06156# [Vmax= .485;Dmax= 1.131]
06157#
06158# Addition of Subwatershed 2 with Monahan Drain and Smith Drain to Node 2
06159#
06160# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06161# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06162# [L/S= 556.7 / .090/400]
06163# [Vmax= .485;Dmax= 1.131]
06164#
06165# Sum of hydrographs from Node 2 routed to Node 1
06166#
06167# ROUTE CHANNEL -> 1.0 02:18_M5 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06168# [RDT= 1.00] out<- 1.0 01:01:M10 45409.01 51.448 No.Date 34:54 21.56 n/a .000
06169# [L/S= 556.7 / .090/400]
06170# [Vmax= .485;Dmax= 1.131]
06171#
06172# Sum of hydrographs from Node 4 routed to Node 2
06173#

Table with columns for ID, description, and numerical data. The table contains multiple rows of data, including headers like 'CONTINUOUS STANDBY' and various numerical values such as '1.0 01:ST-2', '59', and '0.91'. It also includes descriptive text like 'ROUTING AREA' and 'PREVIOUS AREA'.

10847*	ROUTE PIPE	>>	1.0	02:15:338	17.90	1.490	Ndate	28:26	49.83	n/a	0.00	10134*	ADD HYD		1.0	02:10:6A-106	111.46	3.699	Ndate	28:16	50.20	n/a	0.00	
10848*	[R/W= 233 / 100 / 013]											10135*	ROUTE PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10849*	[Vmax = 1.38 / Dmax = 1.5]											10136*	SUM	*	1.0	01:MHI06	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10850*	[Dir= 1.80 / Dused= 1.8]											10137*	remark:GPEARS-TpeakDate.hhm	AREA=										
10851*	[Dir= 1.80 / Dused= 1.8]											10138*	SAVE HYD		1.0	01:MHI06	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10852*	CONTINUOUS STANDBY											10139*	frame:MH106.0025											
10853*	[LGS= 2 CN= 75.0]											10140*	remark:Total Flows at MH106											
10854*	[LGS= 2 CN= 75.0]											10141*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10855*	[LGS= 2 CN= 75.0]											10142*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10856*	[LGS= 2 CN= 75.0]											10143*	[SDR= 1.00] out<	>>	1.0	01:10:16-107	113.90	4.067	Ndate	28:03	50.41	n/a	0.00	
10857*	[LGS= 2 CN= 75.0]											10144*	[L/S= 127.7 / 130 / 013]											
10858*	[Vmax = 1.68 / Dmax = 1.92]											10145*	[Vmax = 1.68 / Dmax = 1.92]											
10859*	[Dir= 1.80 / Dused= 1.8]											10146*	[Dir= 1.80 / Dused= 1.8]											
10860*	CONTINUOUS STANDBY											10147*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10861*	[LGS= 2 CN= 75.0]											10148*	CONTINUOUS STANDBY											
10862*	[LGS= 2 CN= 75.0]											10149*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10863*	[LGS= 2 CN= 75.0]											10150*	[LGS= 2 CN= 75.0]											
10864*	[LGS= 2 CN= 75.0]											10151*	[Previous area: IApex = 4.67:SLPP=1.00:LGP= 40:MPM= 250:SCP= 0]											
10865*	[LGS= 2 CN= 75.0]											10152*	[Impervious area: IAImp = 1.57:SLPP=1.00:LGI= 379:MHI= 013:IC= 0]											
10866*	[LGS= 2 CN= 75.0]											10153*	[IArcImp = 4.00: IArcP = 4.00]											
10867*	[LGS= 2 CN= 75.0]											10154*	[SMIN = 31.81: SMAX = 235.43: SK = 010]											
10868*	CONTINUOUS STANDBY											10155*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10869*	[LGS= 2 CN= 75.0]											10156*	CONTINUOUS STANDBY											
10870*	[LGS= 2 CN= 75.0]											10157*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10871*	[LGS= 2 CN= 75.0]											10158*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10872*	[LGS= 2 CN= 75.0]											10159*	[Previous area: IApex = 4.67:SLPP=1.00:LGP= 40:MPM= 250:SCP= 0]											
10873*	[LGS= 2 CN= 75.0]											10160*	[Impervious area: IAImp = 1.57:SLPP=1.00:LGI= 379:MHI= 013:IC= 0]											
10874*	[LGS= 2 CN= 75.0]											10161*	[IArcImp = 4.00: IArcP = 4.00]											
10875*	[LGS= 2 CN= 75.0]											10162*	[SMIN = 31.81: SMAX = 235.43: SK = 010]											
10876*	[LGS= 2 CN= 75.0]											10163*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10877*	[LGS= 2 CN= 75.0]											10164*	CONTINUOUS STANDBY											
10878*	[LGS= 2 CN= 75.0]											10165*	[Previous area: IApex = 4.67:SLPP=1.00:LGP= 40:MPM= 250:SCP= 0]											
10879*	[LGS= 2 CN= 75.0]											10166*	[Impervious area: IAImp = 1.57:SLPP=1.00:LGI= 379:MHI= 013:IC= 0]											
10880*	[LGS= 2 CN= 75.0]											10167*	[IArcImp = 4.00: IArcP = 4.00]											
10881*	[LGS= 2 CN= 75.0]											10168*	[SMIN = 31.81: SMAX = 235.43: SK = 010]											
10882*	CONTINUOUS STANDBY											10169*	ROUTE PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10883*	[LGS= 2 CN= 75.0]											10170*	CONTINUOUS STANDBY											
10884*	[LGS= 2 CN= 75.0]											10171*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10885*	[LGS= 2 CN= 75.0]											10172*	CONTINUOUS STANDBY											
10886*	[LGS= 2 CN= 75.0]											10173*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10887*	[LGS= 2 CN= 75.0]											10174*	ADD HYD		1.0	02:10:16-107	113.90	4.067	Ndate	28:03	50.41	n/a	0.00	
10888*	[LGS= 2 CN= 75.0]											10175*	ROUTE PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10889*	[LGS= 2 CN= 75.0]											10176*	ADD HYD		1.0	02:10:16-107	113.90	4.067	Ndate	28:03	50.41	n/a	0.00	
10890*	[LGS= 2 CN= 75.0]											10177*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10891*	[LGS= 2 CN= 75.0]											10178*	CONTINUOUS STANDBY											
10892*	[LGS= 2 CN= 75.0]											10179*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10893*	[LGS= 2 CN= 75.0]											10180*	CONTINUOUS STANDBY											
10894*	[LGS= 2 CN= 75.0]											10181*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10895*	[LGS= 2 CN= 75.0]											10182*	CONTINUOUS STANDBY											
10896*	[LGS= 2 CN= 75.0]											10183*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10897*	[LGS= 2 CN= 75.0]											10184*	CONTINUOUS STANDBY											
10898*	[LGS= 2 CN= 75.0]											10185*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10899*	[LGS= 2 CN= 75.0]											10186*	CONTINUOUS STANDBY											
10900*	[LGS= 2 CN= 75.0]											10187*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10901*	[LGS= 2 CN= 75.0]											10188*	CONTINUOUS STANDBY											
10902*	[LGS= 2 CN= 75.0]											10189*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10903*	[LGS= 2 CN= 75.0]											10190*	CONTINUOUS STANDBY											
10904*	[LGS= 2 CN= 75.0]											10191*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10905*	[LGS= 2 CN= 75.0]											10192*	CONTINUOUS STANDBY											
10906*	[LGS= 2 CN= 75.0]											10193*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10907*	[LGS= 2 CN= 75.0]											10194*	CONTINUOUS STANDBY											
10908*	[LGS= 2 CN= 75.0]											10195*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10909*	[LGS= 2 CN= 75.0]											10196*	CONTINUOUS STANDBY											
10910*	[LGS= 2 CN= 75.0]											10197*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a	0.00	
10911*	[LGS= 2 CN= 75.0]											10198*	CONTINUOUS STANDBY											
10912*	[LGS= 2 CN= 75.0]											10199*	ROUTER PIPE	>>	1.0	02:18:M16	113.90	4.090	Ndate	28:01	50.41	n/a		

11221- remark/Total Flows at Jockvale Road

11222- #

11223- # Hydrograph from Jockvale Road routed to Heart's Desire

11224- # Channel X-Section obtained from RWCA Hydraulic Model - Station 669

11225- #

11226- RO525-C00428-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11227- ROUTE CHANNEL -> 1.0 O1=SM_DE 55194.86 104.140 No.Date 39:48 29.33 n/a .000

11228- [RFD=1.00] out<- 1.0 O1=SM_DE 55194.86 104.140 No.Date 39:45 29.33 n/a .000

11229- [L/S= 221.0] out<- 1.0 O1=SM_DE 55194.86 104.140 No.Date 39:45 29.33 n/a .000

11230- [Vmax=1.483;Dmax=2.264]

11231- # Catchment DESIRE

11232- # To Jock River (north of the Jock)

11233- # Rural-estate subdivision Heart's Desire Community

11234- #

11235- #

11236- RO525-C00428-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11237- CONTINUOUS STANHYD 1.0 O1=DESIRE 23.78 2.161 No.Date 28:03 40.77 1548 .000

11238- [XMP=25.5;TMD=25]

11239- [LOGS=2 C/N= 74.0]

11240- [Pervious area IApex=4.67;SLD=1.00;LID= 4.0;IMP=250;SICI= 0]

11241- [Impervious area IAlmp=1.57;SLD=1.00;LID=1.0;IMP= 400;IM1= 0;ISICI= 0]

11242- [SBRc= 4.00; IAPRc= 4.00]

11243- [SWR= 21.15; SWM=207.66; SE= 200]

11244- # Catchment JOCKVA

11245- # To Jockvale SSM Facility

11246- # Residential development & golf course

11247- # JWSA 2021-01-11 update JOCKVA after updating CORRIG as per INV SURVEY July 2008.

11248- # JOCKVA area became 235.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX-LAND 12.5 ha as

11249- #

11250- #

11251- RO525-C00430-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11252- CONTINUOUS STANHYD 1.0 O1=JOCKVA 225.13 21.797 No.Date 28:07 50.08 673 .000

11253- [XMP=25.5;TMD=25]

11254- [LOGS=2 C/N= 74.0]

11255- [Pervious area IApex=4.67;SLD=1.00;LID= 4.0;IMP=250;SICI= 0]

11256- [Impervious area IAlmp=1.57;SLD=1.00;LID=1.0;IMP= 400;IM1= 0;ISICI= 0]

11257- [SBRc= 4.00; IAPRc= 4.00]

11258- [SWR= 36.47; SWM=244.99; SE= 200]

11259- RO525-C00431-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11260- AID RVD + 1.0 O1=JOCK_P 225.13 21.797 No.Date 28:07 50.08 n/a .000

11261- + 1.0 O2=JOCKVA 225.13 21.797 No.Date 28:07 50.08 n/a .000

11262- + 1.0 O2=SM_DE 257.63 24.072 No.Date 28:07 50.10 n/a .000

11263- + 1.0 O2=RS-NM 0.00 0.00 No.Date 0:00 0.0 n/a .000

11264- SUM= 1.0 O1=JOCKVA 225.13 21.797 No.Date 28:07 50.10 n/a .000

11265- RO525-C00432-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11266- SAVE RVD 1.0 O1=JOCKVA 257.63 24.072 No.Date 28:07 50.10 n/a .000

11267- frame -SN_NL_0025

11268- remark/Total Flows at KB first pond

11269- #

11270- # Jockvale SSM Facility

11271- # Rating curve obtained from Jockvale Servicing Study (CC 1999)

11272- #

11273- RO525-C00433-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11274- ROUTE RESERVOIR -> 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:44 29.43 n/a .000

11275- out<- 1.0 O1=JOCK_P 257.63 9.145 No.Date 28:37 50.10 n/a .000

11276- overflow <- 1.0 O3=20-WVP 40.00 0.00 No.Date 0:00 0.0 n/a .000

11277- [MaxOutflow=240000.000000-00 43.0 0.00; 0.0; 0.0; 0.0; 0.0; 0.0]

11278- RO525-C00434-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11279- AID RVD + 1.0 O1=SM_DE 55194.86 104.140 No.Date 39:45 29.33 n/a .000

11280- + 1.0 O2=DESIRE 23.78 2.161 No.Date 28:03 40.77 n/a .000

11281- + 1.0 O2=SM_DE 257.63 9.145 No.Date 28:37 50.10 n/a .000

11282- + 1.0 O2=JOCK_P 257.63 9.145 No.Date 28:37 50.10 n/a .000

11283- SUM= 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:44 29.43 n/a .000

11284- RO525-C00435-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11285- SAVE RVD 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:44 29.43 n/a .000

11286- frame -SN_NL_0025

11287- remark/Total Flows at Heart's Desire

11288- #

11289- # Hydrograph from Heart's Desire routed to Rideau River

11290- # Channel X-Section obtained from RWCA Hydraulic Model - Station 0

11291- #

11292- RO525-C00436-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11293- ROUTE CHANNEL -> 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:44 29.43 n/a .000

11294- [RFD=1.00] out<- 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:48 29.43 n/a .000

11295- [L/S= 583.7 / 3677.046]

11296- [Vmax= 1.942;Dmax= 1.184]

11297- # Catchment S2

11298- # To Jock River (north and south)

11299- # Undeveloped floodplain and river

11300- #

11301- #

11302- RO525-C00437-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11303- CONTINUOUS STANHYD 1.0 O1=SM_DE 100.94 3.971 No.Date 28:20 30.13 465 .000

11304- [C/W= 72.0; R= 3.00; Tp= .40]

11305- [IAPRc=4.00; SWM= 19.70; SWM=264.99; SE= 200]

11306- [InterEventTime= 12.00]

11307- RO525-C00438-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11308- AID RVD + 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:48 29.43 n/a .000

11309- + 1.0 O2=JOCK_P 257.63 9.145 No.Date 28:20 30.13 n/a .000

11310- + 1.0 O2=SM_DE 55476.27 104.765 No.Date 39:48 29.43 n/a .000

11311- RO525-C00439-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11312- SAVE RVD 1.0 O1=SM_DE 55476.27 104.765 No.Date 39:48 29.43 n/a .000

11313- frame -SN_NL_0025

11314- remark/Total Flows at Rideau River

11315- #####

11316- ** END OF RUN : 49

11317- #

11318- #

11319- #

11320- #

11321- #

11322- #

11323- #

11324- RUN COMMANDS

11325- RO595-C00001

11326- START

11327- [TSEED = 0 hrs on 0]

11328- [METOPT= 2 (1=imperial, 2=metric output)]

11329- [NSURF= 1]

11330- [NSUN = 0050]

11331- #

11332- # SHMANY Ver:5.02;/Jan 2001 <SET> / INPUT DATA FILE

11333- #

11334- # Project Name (local) Project Number: [474-16]

11335- # Date 04-03-2021

11336- # Modeller

11337- # Company JFSaInc.

11338- # License # 254923

11339- #

11340- # CALIBRATION OF SUMMER MODEL PARAMETERS

11341- # SUMMER MODEL SIMULATION PARAMETERS

11342- # Mainfall data from UFSA rangebase installed at site - other gauges by the City

11343- # Use data collected from July 14, 2001 to 01:00:00 No.Date

11344- # 2020-11-30 change TMDSTO to COMPUTE DUALDYO (TMDSTO = 0.1 instead of 0.0001)

11345- # 2020-12-01 correct pond curve values

11346- # 2020-12-01 change M_ADI to 0.55, SLPD(0.5/1%) (impervious slope), and LCI up to 70m

11347- # 2021-07-19 change the slope for ROUTE CHANNEL station 2462 (NHVDou=N_TO; NHVIN=N_TO) from 0.033 % (as per st)

11348- # 2021-02-18 change the slope for ROUTE CHANNEL (NHVDou=N_TO; NHVIN=N_TO) from 0.033 % (as per st)

11349- #

11350- RO595-C00002

11351- READ STORM

11352- File Name storm_001

11353- Comment # Plus Size of 24 hrs 150 and pour Octava CDA

11354- [SFD=10.00;SDUR= 24.00;PDT0= 81.51]

11355- RO595-C00003

11356- MODIFY STORM

11357- [SFACT=1.00;TBSFT= 96.00 min]

11358- [SFD=10.00;SDUR= 40.00;PDT0= 81.51]

11359- RO595-C00004

11360- DISPLAY VALUES

11361- File Name T:\PROJ\1474-16\Design\2020\1026-QualityControlAnalysis\SHMNT\SHM-Model\Updated3\Citigate.DEP

11362- [CSEASV = 1 (sub and print data)]

11363- FileTitle File comment Based on various calibration exercises in Onta

11364- The FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANHYD COM

11365- Horton's infiltration equation parameters

11366- [Fov 76.20 mm/hr] [Fov1=10.20 mm/hr] [DOCV= 4.14]/r [P_ .00 mm]

11367- Parameters for SEVICUS surfaces in STANHYD

11368- [Iapex= 4.67 mm] [LID=50.00 mm] [MWD= .280]

11369- Parameters for DRNBVCUS surfaces in STANHYD

11370- [Iapex= 1.57 mm] [LID= 1.00] [MWD= 0.13]

11371- Parameters used in NASHVD

11372- [Iap= 4.67 mm] [N= 1.00]

11373- Average monthly Pan Evaporation data in (mm)

11374- JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC

11375- .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

11376- Average monthly Potential Evapotranspiration in (mm)

11377- JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC

11378- .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

11379- RO595-C00005

11380- COMPUTE API

11381- [Adfn=10.00; ADIKdy= 8500; ADIKhr= 9988]

11382- [ADfn=11.33; ADIavg= 87.14; ADfn= 44.87]

11383- #

11384- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11385- # of 1.2

11386- RO595-C00006-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11387- CONTINUOUS NASHVD 1.0 O1=SM_DE 3680.00 18.440 No.Date 36:55 30.33 172 .000

11388- [IAPRc=4.00; SWM= 3.00; Tp= 1.11]

11389- [IAPRc=4.00; SWM= 64.50; SWM=430.01; SE= 200]

11390- [InterEventTime= 12.00]

11391- #

11392- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11393- # of 1.3

11394- RO595-C00007-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11395- CONTINUOUS NASHVD 1.0 O1=SM_DE 971.00 6.937 No.Date 32:34 28.27 347 .000

11396- [IAPRc=4.00; SWM= 3.00; Tp= 1.11]

11397- [IAPRc=4.00; SWM= 64.50; SWM=430.01; SE= 200]

11398- [InterEventTime= 12.00]

11399- #

11400- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11401- # of 1.8

11402- RO595-C00008-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11403- CONTINUOUS NASHVD 1.0 O1=SM_DE 3074.00 6.812 No.Date 39:59 24.31 298 .000

11404- [IAPRc=4.00; SWM= 3.00; Tp= 1.11]

11405- [IAPRc=4.00; SWM= 120.81; SWM=554.96; SE= 200]

11406- [InterEventTime= 12.00]

11407- RO595-C00009-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11408- CONTINUOUS NASHVD 1.0 O1=RASH 1781.00 16.834 No.Date 32:39 36.85 452 .000

11409- [C/W= 81.0; R= 3.00; Tp= 1.11]

11410- [IAPRc= 4.00; SWM= 39.75; SWM=264.99; SE= 200]

11411- [InterEventTime= 12.00]

11412- RO595-C00010-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11413- CONTINUOUS NASHVD 1.0 O1=SM_11 500.00 9.061 No.Date 29:21 31.73 389 .000

11414- [IAPRc=4.00; SWM= 3.00; Tp= 1.11]

11415- [IAPRc=4.00; SWM= 52.62; SWM=350.79; SE= 200]

11416- [InterEventTime= 12.00]

11417- #

11418- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11419- # of 1.8

11420- RO595-C00011-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11421- CONTINUOUS NASHVD 1.0 O1=SM_2 1317.00 14.039 No.Date 34:26 31.73 389 .000

11422- [C/W= 66.0; R= 3.00; Tp= 5.29]

11423- [IAPRc=4.00; SWM= 33.62; SWM=350.79; SE= 200]

11424- [InterEventTime= 12.00]

11425- #

11426- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11427- # of 1.52

11428- RO595-C00012-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11429- CONTINUOUS NASHVD 1.0 O1=SM_10 5666.00 32.402 No.Date 37:52 36.85 452 .000

11430- [C/W= 72.0; R= 3.00; Tp= 8.03]

11431- [IAPRc= 4.00; SWM= 39.75; SWM=264.99; SE= 200]

11432- [InterEventTime= 12.00]

11433- #

11434- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11435- # of 1.75

11436- RO595-C00013-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11437- CONTINUOUS NASHVD 1.0 O1=SM_2K 8376.00 31.024 No.Date 39:59 31.73 389 .000

11438- [C/W= 66.0; R= 3.00; Tp= 11.68]

11439- [IAPRc=4.00; SWM= 33.62; SWM=350.79; SE= 200]

11440- [InterEventTime= 12.00]

11441- #

11442- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11443- # of 1.68

11444- RO595-C00014-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11445- CONTINUOUS NASHVD 1.0 O1=SM_9 3745.00 30.53 38.95 424 .000

11446- [C/W= 70.0; R= 3.00; Tp= 2.51]

11447- [IAPRc=4.00; SWM= 30.77; SWM=287.10; SE= 200]

11448- [InterEventTime= 12.00]

11449- #

11450- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11451- # of 1.82

11452- RO595-C00015-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11453- CONTINUOUS NASHVD 1.0 O1=HCK 4464.00 15.472 No.Date 39:59 28.95 355 .000

11454- [IAPRc= 4.00; SWM= 61.90; SWM=412.66; SE= 200]

11455- [InterEventTime= 12.00]

11456- #

11457- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11458- # of 1.6

11459- RO595-C00016-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11460- CONTINUOUS NASHVD 1.0 O1=SM_2 131.00 2.740 No.Date 28:57 29.64 364 .000

11461- [C/W= 63.0; R= 3.00; Tp= .50]

11462- [IAPRc=4.00; SWM= 59.42; SWM=396.11; SE= 200]

11463- [InterEventTime= 12.00]

11464- #

11465- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11466- # of 1.65

11467- RO595-C00017-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11468- CONTINUOUS NASHVD 1.0 O1=HBR_UR 3854.00 18.180 No.Date 38:32 31.73 389 .000

11469- [C/W= 66.0; R= 3.00; Tp= 8.42]

11470- [IAPRc=4.00; SWM= 52.62; SWM=350.79; SE= 200]

11471- [InterEventTime= 12.00]

11472- #

11473- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11474- # of 1.82

11475- RO595-C00018-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11476- CONTINUOUS NASHVD 1.0 O1=HCK 1397.00 13.937 No.Date 36:23 25.61 314 .000

11477- [IAPRc= 4.00; SWM= 76.32; SWM=508.81; SE= 200]

11478- [InterEventTime= 12.00]

11479- #

11480- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11481- # of 1.67

11482- RO595-C00019-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11483- CONTINUOUS NASHVD 1.0 O1=VLRD 1332.00 9.332 No.Date 35:12 36.85 452 .000

11484- [IAPRc=4.00; SWM= 39.75; SWM=264.99; SE= 200]

11485- [InterEventTime= 12.00]

11486- #

11487- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11488- # of 4.18

11489- RO595-C00020-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11490- CONTINUOUS NASHVD 1.0 O1=SM_5 224.00 8.187 No.Date 28:45 41.51 509 .000

11491- [IAPRc=4.00; SWM= 31.15; SWM=207.66; SE= 200]

11492- [InterEventTime= 12.00]

11493- #

11494- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11495- # of 1.20

11496- RO595-C00021-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11497- CONTINUOUS NASHVD 1.0 O1=PLCK 4845.00 44.623 No.Date 33:18 38.37 471 .000

11498- [IAPRc= 4.00; SWM= 36.67; SWM=244.49; SE= 200]

11499- [InterEventTime= 12.00]

11500- #

11501- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11502- # of 1.60

11503- RO595-C00022-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11504- CONTINUOUS NASHVD 1.0 O1=PLCK 4945.00 44.623 No.Date 33:18 38.37 471 .000

11505- [IAPRc=4.00; SWM= 36.67; SWM=244.49; SE= 200]

11506- [InterEventTime= 12.00]

11507- #

11508- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11509- # of 1.60

11510- RO595-C00023-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11511- CONTINUOUS NASHVD 1.0 O1=SM_5A2 20.00 .943 No.Date 28:35 45.60 560 .000

11512- [IAPRc= 4.00; SWM= 25.21; SWM=168.09; SE= 200]

11513- [InterEventTime= 12.00]

11514- #

11515- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11516- # of 1.62

11517- RO595-C00024-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11518- CONTINUOUS NASHVD 1.0 O1=SM_SAI 1412.00 8.794 No.Date 37:48 39.93 490 .000

11519- [IAPRc=4.00; SWM= 33.11; SWM=225.43; SE= 200]

11520- [InterEventTime= 12.00]

11521- #

11522- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11523- # of 1.52

11524- RO595-C00025-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11525- CONTINUOUS NASHVD 1.0 O1=SM_4 888.00 12.896 No.Date 29:55 45.60 560 .000

11526- [IAPRc=4.00; SWM= 25.21; SWM=168.09; SE= 200]

11527- [InterEventTime= 12.00]

11528- RO595-C00026-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11529- CONTINUOUS NASHVD 1.0 O1=LKCK 1021.00 10.759 No.Date 30:46 44.77 549 .000

11530- [C/W= 80.0; R= 3.00; Tp= 2.46]

11531- [IAPRc=4.00; SWM= 26.32; SWM=175.50; SE= 200]

11532- [InterEventTime= 12.00]

11533- #

11534- RO595-C00027-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11535- CONTINUOUS NASHVD 1.0 O1=SM_2 177.00 6.469 No.Date 28:45 41.51 509 .000

11536- [IAPRc=4.00; SWM= 31.15; SWM=207.66; SE= 200]

11537- [InterEventTime= 12.00]

11538- #

11539- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11540- # of 1.20

11541- RO595-C00028-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11542- CONTINUOUS NASHVD 1.0 O1=SM_2 1122.00 15.544 No.Date 31:43 45.60 560 .000

11543- [IAPRc= 4.00; SWM= 25.21; SWM=168.09; SE= 200]

11544- [InterEventTime= 12.00]

11545- #

11546- # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)

11547- # of 1.20

11548- RO595-C00029-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11549- CONTINUOUS NASHVD 1.0 O1=SM_2 2737.00 34.946 No.Date 31:29 40.72 500 .000

11550- [IAPRc=4.00; SWM= 32.46; SWM=216.39; SE= 200]

11551- [InterEventTime= 12.00]

11552- #

11553- # Routing hydrographs

11554- #

11555- # Starting with the addition of Jock River Headwater and Subwatershed 13

11556- #

11557- RO595-C00030-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11558- AID RVD + 1.0 O2=RS_N 3680.00 18.440 No.Date 36:55 30.33 n/a .000

11559- + 1.0 O2=SM_13 971.00 6.937 No.Date 32:34 28.27 n/a .000

11560- + 1.0 O1=SM_13 4651.00 23.559 No.Date 35:24 29.90 n/a .000

11561- SUM= 1.0 O1=SM_13 4651.00 23.559 No.Date 35:24 29.90 n/a .000

11562- RO595-C00031-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11563- ROUTE CHANNEL -> 1.0 O2=SM_13 4651.00 23.559 No.Date 35:24 29.90 n/a .000

11564- [RFD=1.00] out<- 1.0 O1=SM_13A 4651.00 19.136 No.Date 39:54 27.68 n/a .000

11565- [L/S= 9074.7 / 622/040]

11566- [Vmax= 874.0; Dmax= 3.200]

11567- #

11568- # Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A

11569- #

11570- RO595-C00032-----Dtn=ID=NHV-----AREA=OPEARc=PeakDate_hh:mm-----RvM-R-C-----DWfms

11571- AID RVD + 1.0 O2=SM_13A 4651.00 19.136 No.Date 39:54 27.68 n/a .00

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11595 # Addition of Subwatershed Jock River at Ashton to Node 12
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Table with multiple columns containing technical data, codes (e.g., 13839, 13840), and various alphanumeric strings. It appears to be a detailed engineering or data report with various headers and sub-headers throughout the rows.

14213 R0100C0008 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14214 CONTINUOUS NASHVD 1.0 01:01R_ASH 3074.00 10.428 No.Date 39:59 28.29 .000
14215 [Cm 72.0i Nn 3.00i Tp 11.33i]
14216 [IAREC 4.00i SMIN= 55.62i SMAK=554.96i GK= .010i]
14217 [InterventTime= 12.00i]
14218 R0100C0009 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14219 CONTINUOUS NASHVD 1.0 01:01R_ASH 1781.00 19.695 No.Date 32:38 42.49 .000
14220 [Cm 72.0i Nn 3.00i Tp 8.91i]
14221 [IAREC 4.00i SMIN= 55.62i SMAK=264.99i GK= .010i]
14222 [InterventTime= 12.00i]
14223 R0100C0010 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14224 CONTINUOUS NASHVD 1.0 01:01R_M11 800.00 10.735 No.Date 29:21 36.76 .415 .000
14225 [Cm 66.0i Nn 3.00i Tp 1.24i]
14226 [IAREC 4.00i SMIN= 52.62i SMAK=350.79i GK= .010i]
14227 [InterventTime= 12.00i]
14228 #
14229 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14230 # of 1.80
14231 R0100C0011 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14232 CONTINUOUS NASHVD 1.0 01:01R_MX 1937.00 14.496 No.Date 34:24 36.76 .415 .000
14233 [Cm 66.0i Nn 3.00i Tp 1.24i]
14234 [IAREC 4.00i SMIN= 52.62i SMAK=350.79i GK= .010i]
14235 [InterventTime= 12.00i]
14236 #
14237 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14238 # of 1.82
14239 R0100C0012 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14240 CONTINUOUS NASHVD 1.0 01:01R_MX 5666.00 18.060 No.Date 37:48 42.49 .480 .000
14241 [Cm 72.0i Nn 3.00i Tp 8.00i]
14242 [IAREC 4.00i SMIN= 39.75i SMAK=264.99i GK= .010i]
14243 [InterventTime= 12.00i]
14244 #
14245 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14246 # of 1.75
14247 R0100C0013 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14248 CONTINUOUS NASHVD 1.0 01:01R_CK 8376.00 36.118 No.Date 39:59 36.76 .415 .000
14249 [Cm 66.0i Nn 3.00i Tp 11.66i]
14250 [IAREC 4.00i SMIN= 52.62i SMAK=350.79i GK= .010i]
14251 [InterventTime= 12.00i]
14252 #
14253 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14254 # of 1.80
14255 R0100C0014 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14256 CONTINUOUS NASHVD 1.0 01:01R_S 1132.00 16.501 No.Date 30:52 40.82 .461 .000
14257 [Cm 72.0i Nn 3.00i Tp 8.00i]
14258 [IAREC 4.00i SMIN= 41.07i SMAK=287.10i GK= .010i]
14259 [InterventTime= 12.00i]
14260 #
14261 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14262 # of 1.82
14263 R0100C0015 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14264 CONTINUOUS NASHVD 1.0 01:01R_CK 4864.00 18.060 No.Date 39:59 31.61 .380 .000
14265 [Cm 62.0i Nn 3.00i Tp 11.32i]
14266 [IAREC 4.00i SMIN= 52.62i SMAK=412.66i GK= .010i]
14267 [InterventTime= 12.00i]
14268 #
14269 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14270 # of 1.80
14271 R0100C0016 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14272 CONTINUOUS NASHVD 1.0 01:01R_S 131.00 3.259 No.Date 28:57 34.39 .388 .000
14273 [Cm 43.0i Nn 3.00i Tp 1.90i]
14274 [IAREC 4.00i SMIN= 55.42i SMAK=396.11i GK= .010i]
14275 [InterventTime= 12.00i]
14276 #
14277 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14278 # of 1.88
14279 R0100C0017 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14280 CONTINUOUS NASHVD 1.0 01:01R_R 3854.00 21.238 No.Date 38:28 36.76 .415 .000
14281 [Cm 66.0i Nn 3.00i Tp 11.81i]
14282 [IAREC 4.00i SMIN= 52.62i SMAK=350.79i GK= .010i]
14283 [InterventTime= 12.00i]
14284 #
14285 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14286 # of 1.82
14287 R0100C0018 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14288 CONTINUOUS NASHVD 1.0 01:01R_CK 3197.00 16.401 No.Date 36:21 39.79 .336 .000
14289 [Cm 57.0i Nn 3.00i Tp 6.65i]
14290 [IAREC 4.00i SMIN= 55.58i SMAK=508.81i GK= .010i]
14291 [InterventTime= 12.00i]
14292 #
14293 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14294 # of 1.75
14295 R0100C0019 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14296 CONTINUOUS NASHVD 1.0 01:01R_S 165.00 1.511 No.Date 33:01 37.57 .424 .000
14297 [Cm 47.0i Nn 3.00i Tp 11.62i]
14298 [IAREC 4.00i SMIN= 55.58i SMAK=336.97i GK= .010i]
14299 [InterventTime= 12.00i]
14300 #
14301 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14302 # of 1.87
14303 R0100C0020 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14304 CONTINUOUS NASHVD 1.0 01:01R_R 1332.00 10.892 No.Date 35:10 42.49 .480 .000
14305 [Cm 72.0i Nn 3.00i Tp 7.93i]
14306 [IAREC 4.00i SMIN= 39.75i SMAK=264.99i GK= .010i]
14307 [InterventTime= 12.00i]
14308 R0100C0021 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14309 CONTINUOUS NASHVD 1.0 01:01R_CK 224.00 3.976 No.Date 28:44 47.62 .538 .000
14310 [Cm 77.0i Nn 3.00i Tp 7.5i]
14311 [IAREC 4.00i SMIN= 31.55i SMAK=207.66i GK= .010i]
14312 [InterventTime= 12.00i]
14313 #
14314 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14315 # of 1.20
14316 R0100C0022 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14317 CONTINUOUS NASHVD 1.0 01:01R_C 4945.00 52.056 No.Date 33:16 44.17 .499 .000
14318 [Cm 74.0i Nn 3.00i Tp 4.45i]
14319 [IAREC 4.00i SMIN= 36.45i SMAK=244.49i GK= .010i]
14320 [InterventTime= 12.00i]
14321 R0100C0023 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14322 CONTINUOUS NASHVD 1.0 01:01R_S 20.00 1.097 No.Date 28:35 52.06 .588 .000
14323 [Cm 81.0i Nn 3.00i Tp 1.62i]
14324 [IAREC 4.00i SMIN= 25.21i SMAK=168.09i GK= .010i]
14325 [InterventTime= 12.00i]
14326 #
14327 # The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
14328 # of 1.81
14329 R0100C0024 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14330 CONTINUOUS NASHVD 1.0 01:01R_S 1412.00 10.184 No.Date 37:44 45.88 .518 .000
14331 [Cm 75.0i Nn 3.00i Tp 11.40i]
14332 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14333 [InterventTime= 12.00i]
14334 R0100C0025 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14335 CONTINUOUS NASHVD 1.0 01:01R_C 585.00 14.953 No.Date 29:55 52.06 .588 .000
14336 [Cm 81.0i Nn 3.00i Tp 1.75i]
14337 [IAREC 4.00i SMIN= 25.21i SMAK=168.09i GK= .010i]
14338 [InterventTime= 12.00i]
14339 R0100C0026 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14340 CONTINUOUS NASHVD 1.0 01:01R_C 1021.00 19.792 No.Date 30:45 51.16 .578 .000
14341 [Cm 80.0i Nn 3.00i Tp 2.46i]
14342 [IAREC 4.00i SMIN= 31.81i SMAK=175.50i GK= .010i]
14343 [InterventTime= 12.00i]
14344 R0100C0027 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14345 CONTINUOUS NASHVD 1.0 01:01R_C 177.00 7.767 No.Date 28:44 47.62 .538 .000
14346 [Cm 77.0i Nn 3.00i Tp 7.5i]
14347 [IAREC 4.00i SMIN= 21.15i SMAK=207.66i GK= .010i]
14348 [InterventTime= 12.00i]
14349 R0100C0028 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14350 CONTINUOUS NASHVD 1.0 01:01R_C 1122.00 17.991 No.Date 31:42 52.06 .588 .000
14351 [Cm 81.0i Nn 3.00i Tp 1.25i]
14352 [IAREC 4.00i SMIN= 25.21i SMAK=168.09i GK= .010i]
14353 [InterventTime= 12.00i]
14354 R0100C0029 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14355 CONTINUOUS NASHVD 1.0 01:01R_LR 2737.00 40.730 No.Date 31:28 46.75 .528 .000
14356 [Cm 76.0i Nn 3.00i Tp 1.03i]
14357 [IAREC 4.00i SMIN= 32.46i SMAK=216.39i GK= .010i]
14358 [InterventTime= 12.00i]
14359 #
14360 # Routing hydrographs
14361 #
14362 # Starting with the addition of Jock River Headwater and Subwatershed 13
14363 #
14364 R0100C0030 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14365 ADD HYD 1.0 02:1R_S 3680.00 21.616 No.Date 36:52 35.18 n/a .000
14366 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14367 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14368 #
14369 # Sum of hydrographs from Node 13 routed to Node 13A
14370 # (Approximated cross-section - see cross-section 258)
14371 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
14372 #
14373 R0100C0031 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14374 ROUTE CHANNEL -> 1.0 02:1R_M3 4651.00 27.660 No.Date 35:21 34.69 n/a .000
14375 [RD= 1.00i out= 1.0 01:1R_M3 4651.00 22.598 No.Date 38:56 34.69 n/a .000
14376 [I/S= 0.04i / 0.02i / 0.04i / 0.04i]
14377 [Vmax= .598i/Dmax= 4.178i]
14378 #
14379 # Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
14380 #
14381 R0100C0032 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14382 ADD HYD 1.0 02:1R_M3 4651.00 22.598 No.Date 38:56 34.69 n/a .000
14383 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14384 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14385 #
14386 # Insertion of a reservoir to simulate the effects of the Goodwood Marsh
14387 #
14388 R0100C0033 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14389 ROUTE RESERVOIR -> 1.0 02:1R_M3 7725.00 32.845 No.Date 39:44 32.14 n/a .000
14390 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14391 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14392 #
14393 R0100C0034 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14394 SAVE HYD 1.0 01:01R_C 7725.00 3.950 No.Date 62:26 32.14 n/a .000
14395 frame_H_RESUM
14396 remark:Outflow from Res CM
14397 # Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
14398 # (Approximated cross-section - see cross-section 258)
14399 # Use n=0.04 for summer conditions and n=0.025 for spring conditions

14400 R0100C0035 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14401 ROUTE CHANNEL -> 1.0 02:1R_C 7725.00 3.950 No.Date 62:26 32.14 n/a .000
14402 [RD= 1.00i out= 1.0 01:M12 7725.00 3.947 No.Date 64:43 32.14 n/a .000
14403 [I/S= 0.04i / 0.02i / 0.04i / 0.04i]
14404 [Vmax= .560i/Dmax= 1.560i]
14405 #
14406 # Addition of Subwatershed Jock River at Ashton to Node 12
14407 #
14408 R0100C0036 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14409 ADD HYD 1.0 02:1M12 7725.00 3.947 No.Date 64:43 32.14 n/a .000
14410 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14411 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14412 R0100C0037 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14413 SAVE HYD 1.0 01:01R_M12 9506.00 21.745 No.Date 32:41 34.08 n/a .000
14414 frame_H_M12
14415 remark:flow at S_M12 near Ashton
14416 #
14417 # Sum of hydrographs from Node 12 routed to Node 11
14418 # (Approximated cross-section - see cross-section 258)
14419 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
14420 #
14421 # Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
14422 #
14423 R0100C0038 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14424 ROUTE CHANNEL -> 1.0 02:S_M12 9506.00 21.745 No.Date 32:41 34.08 n/a .000
14425 [RD= 1.00i out= 1.0 01:Dum11 9506.00 21.522 No.Date 32:57 34.08 n/a .000
14426 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14427 [Vmax= .771i/Dmax= 3.194i]
14428 #
14429 # Addition of Subwatershed 11 and No Name Creek to Node 11
14430 #
14431 R0100C0039 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14432 ADD HYD 1.0 02:Dum11 9506.00 21.522 No.Date 32:57 34.08 n/a .000
14433 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14434 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14435 #
14436 # Sum of hydrographs from Node 11 routed to Node 10
14437 # Section 1
14438 #
14439 R0100C0040 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14440 ROUTE CHANNEL -> 1.0 02:1R_M11 11923.00 23.609 No.Date 39:19 34.62 n/a .000
14441 [RD= 1.00i out= 1.0 01:N10 11923.00 23.609 No.Date 39:19 34.62 n/a .000
14442 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14443 [Vmax= 14028i / 157 / 040i]
14444 #
14445 # Addition of Subwatershed 10 to Node 10
14446 #
14447 R0100C0041 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14448 ADD HYD 1.0 02:1R_M11 11923.00 23.609 No.Date 39:19 34.62 n/a .000
14449 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14450 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14451 R0100C0042 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14452 SAVE HYD 1.0 01:01R_M10 17889.00 61.058 No.Date 38:16 37.16 n/a .000
14453 frame_H_S10
14454 remark:flow at S_M10 NW - SW 10
14455 #
14456 # Addition of Kings Creek to S_M10
14457 #
14458 R0100C0043 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14459 ADD HYD 1.0 02:1R_M10 17889.00 61.058 No.Date 38:16 37.16 n/a .000
14460 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14461 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14462 #
14463 # Sum of hydrographs from Node 10 routed to Node 9
14464 # Section 2
14465 #
14466 R0100C0044 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14467 ROUTE CHANNEL -> 1.0 02:1R_M10 29565.00 96.504 No.Date 39:40 37.03 n/a .000
14468 [RD= 1.00i out= 1.0 01:R9 29565.00 96.504 No.Date 39:57 37.03 n/a .000
14469 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14470 [Vmax= .771i/Dmax= 2.11i]
14471 #
14472 # Addition of Subwatershed 9 and Nichols Creek to Node 9
14473 #
14474 R0100C0045 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14475 ADD HYD 1.0 02:1R_S 13320.00 16.501 No.Date 30:52 40.83 n/a .000
14476 [IAREC 4.00i SMIN= 55.58i SMAK=508.81i GK= .010i]
14477 [IAREC 4.00i SMIN= 55.58i SMAK=508.81i GK= .010i]
14478 #
14479 # Sum of hydrographs from Node 9 routed to Node 8
14480 # Section 3
14481 #
14482 R0100C0046 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14483 ROUTE CHANNEL -> 1.0 02:1R_S 31561.00 115.681 No.Date 39:59 36.68 n/a .000
14484 [RD= 1.00i out= 1.0 01:N8 31561.00 109.395 No.Date 39:59 36.68 n/a .000
14485 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14486 [Vmax= .269i / .088 / .045i]
14487 [Vmax= .374i/Dmax= 1.924i]
14488 #
14489 # Addition of Subwatershed 8 and Hobb's Drain to Node 8
14490 #
14491 R0100C0047 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14492 ROUTE CHANNEL -> 1.0 02:1R_S 31561.00 109.395 No.Date 39:59 36.68 n/a .000
14493 [RD= 1.00i out= 1.0 02:1R_S 31561.00 109.395 No.Date 39:59 36.68 n/a .000
14494 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14495 [Vmax= 3854i / 21.238 No.Date 38:28 36.76 n/a .000
14496 [Vmax= 10.939 No.Date 39:59 36.68 n/a .000
14497 #
14498 # Sum of hydrographs from Node 8 routed to Node 7
14499 # Section 4
14500 #
14501 R0100C0048 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14502 ROUTE CHANNEL -> 1.0 02:1R_S 35846.00 110.933 No.Date 39:59 36.68 n/a .000
14503 [RD= 1.00i out= 1.0 01:N7 35846.00 111.942 No.Date 44:52 36.68 n/a .000
14504 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14505 [Vmax= .237i/Dmax= 2.412i]
14506 #
14507 # Addition of Subwatershed 7 to Node 7
14508 #
14509 R0100C0049 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14510 ADD HYD 1.0 02:1R_S 35846.00 111.942 No.Date 44:52 36.68 n/a .000
14511 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14512 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14513 R0100C0050 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14514 SAVE HYD 1.0 01:01R_S 38743.00 62.351 No.Date 58:25 36.11 n/a .000
14515 frame_H_S7
14516 remark:flow at S_M7 NW - SW 7
14517 #
14518 # Insertion of a reservoir to simulate the effects of the Richmond Fen.
14519 # Storage area and volume were estimated from available top maps.
14520 # Release rate from fen was assumed to be controlled by the downstream
14521 # river cross-section for summer conditions. It was assumed that for up to
14522 # 0.75 m of water above the main channel of the river provided the storage. Above
14523 # this depth, the wetland starts to significantly store water.
14524 #
14525 R0100C0051 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14526 ROUTE RESERVOIR -> 1.0 02:1R_S 38743.00 120.740 No.Date 43:36 36.11 n/a .000
14527 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14528 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14529 #
14530 # Sum of hydrographs from Node 7 routed to Node 6
14531 # Section 5
14532 #
14533 R0100C0052 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14534 ROUTE CHANNEL -> 1.0 02:1R_S 38743.00 62.351 No.Date 58:25 36.11 n/a .000
14535 [RD= 1.00i out= 1.0 02:1R_S 38743.00 62.351 No.Date 58:25 36.11 n/a .000
14536 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14537 [Vmax= 558i/Dmax= 1.373i]
14538 #
14539 # Addition of Subwatershed 6 and Van Gal Drain to Node 6
14540 #
14541 R0100C0054 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14542 ADD HYD 1.0 02:1R_S 38743.00 62.067 No.Date 59:46 36.11 n/a .000
14543 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14544 [IAREC 4.00i SMIN= 31.81i SMAK=226.43i GK= .010i]
14545 #
14546 # Sum of hydrographs from Node 6 routed to Node 5
14547 # Section 6
14548 #
14549 R0100C0055 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14550 ROUTE CHANNEL -> 1.0 02:1R_S 42042.01 62.114 No.Date 59:29 36.29 n/a .000
14551 [RD= 1.00i out= 1.0 02:1R_S 42042.01 61.993 No.Date 60:21 36.29 n/a .000
14552 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14553 [Vmax= 494i/Dmax= 1.468i]
14554 #
14555 # Addition of Subwatershed 5 and Ploving Creek to Node 5
14556 #
14557 R0100C0056 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
14558 ROUTE CHANNEL -> 1.0 02:1R_S 42042.01 62.114 No.Date 59:29 36.29 n/a .000
14559 [RD= 1.00i out= 1.0 02:1R_S 42042.01 62.114 No.Date 59:29 36.29 n/a .000
14560 [I/S= 0.04i / 0.04i / 0.04i / 0.04i]
14561 [Vmax= 558i/Dmax= 1.373i]
14562 #
14563 # Sum of hydrographs from Node 5 routed to Node 4
14564 # Section 7
14565 #
14566 R0100C0057 -----Dtm=ID-NHVD-----AREA=OPEACms-TpeakDate_hh:mm-----Rvm=R.C.--DWPCms
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145878	ROUTE CHANNEL	1.0	02/28/MSA	4684.01	89.756	MoDate	34/38	37.51	n/a	.000
145879	[RFD=1.00] out	1.0	01/28/OK	4684.01	85.943	MoDate	36/10	37.51	n/a	.000
145880	[L/S= 4630 / .043 / 035]									
145881	[Vmax=.504;Dmax=3.865]									
145892	# Addition of Subwatershed 4 and Leamy Creek to Node 4									
145930	# Section 9									
145931	ROUTE CHANNEL	1.0	02/28/MSA	4684.01	85.943	MoDate	36/10	37.51	n/a	.000
145932	[RFD=1.00] out	1.0	02/28/MSA	4684.01	85.943	MoDate	36/10	37.51	n/a	.000
145933	[L/S= 4630 / .043 / 035]									
145934	[Vmax=.504;Dmax=3.865]									
145940	ADD HYD	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145941	[RFD=1.00] out	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145942	[L/S= 4630 / .043 / 035]									
145943	[Vmax=.504;Dmax=3.865]									
145944	SUM	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145945	CONTINUED STANDYD	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145946	[RFD=1.00] out	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145947	[L/S= 4630 / .043 / 035]									
145948	[Vmax=.504;Dmax=3.865]									
145950	ADD HYD	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145951	[RFD=1.00] out	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145952	[L/S= 4630 / .043 / 035]									
145953	[Vmax=.504;Dmax=3.865]									
145960	SUM	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145961	CONTINUED STANDYD	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145962	[RFD=1.00] out	1.0	02/28/MSA	4885.00	14.933	MoDate	29/55	52.06	n/a	.000
145963	[L/S= 4630 / .043 / 035]									
145964	[Vmax=.504;Dmax=3.865]									
146000	SAVE HYD	1.0	02/28/MSA	4884.07	86.418	MoDate	35/12	37.97	n/a	.000
146001	fname R_34.010									
146002	remark/flow at R_34									
146003	remark/flow at R_34									
146004	Sum of hydrographs from Node 4 routed to Node 2									
146005	# Section 9									
146006	ROUTE CHANNEL	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146007	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146008	[L/S= 1667 / .067 / 040]									
146009	[Vmax=.844;Dmax=3.929]									
146010	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146011	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146012	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146013	[L/S= 1667 / .067 / 040]									
146014	[Vmax=.844;Dmax=3.929]									
146020	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146021	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146022	[L/S= 1667 / .067 / 040]									
146023	[Vmax=.844;Dmax=3.929]									
146024	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146025	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146026	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146027	[L/S= 1667 / .067 / 040]									
146028	[Vmax=.844;Dmax=3.929]									
146030	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146031	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146032	[L/S= 1667 / .067 / 040]									
146033	[Vmax=.844;Dmax=3.929]									
146034	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146035	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146036	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146037	[L/S= 1667 / .067 / 040]									
146038	[Vmax=.844;Dmax=3.929]									
146040	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146041	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146042	[L/S= 1667 / .067 / 040]									
146043	[Vmax=.844;Dmax=3.929]									
146044	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146045	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146046	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146047	[L/S= 1667 / .067 / 040]									
146048	[Vmax=.844;Dmax=3.929]									
146050	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146051	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146052	[L/S= 1667 / .067 / 040]									
146053	[Vmax=.844;Dmax=3.929]									
146054	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146055	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146056	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146057	[L/S= 1667 / .067 / 040]									
146058	[Vmax=.844;Dmax=3.929]									
146060	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146061	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146062	[L/S= 1667 / .067 / 040]									
146063	[Vmax=.844;Dmax=3.929]									
146064	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146065	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146066	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146067	[L/S= 1667 / .067 / 040]									
146068	[Vmax=.844;Dmax=3.929]									
146070	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146071	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146072	[L/S= 1667 / .067 / 040]									
146073	[Vmax=.844;Dmax=3.929]									
146074	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146075	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146076	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146077	[L/S= 1667 / .067 / 040]									
146078	[Vmax=.844;Dmax=3.929]									
146080	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146081	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146082	[L/S= 1667 / .067 / 040]									
146083	[Vmax=.844;Dmax=3.929]									
146084	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146085	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146086	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146087	[L/S= 1667 / .067 / 040]									
146088	[Vmax=.844;Dmax=3.929]									
146090	ADD HYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146091	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146092	[L/S= 1667 / .067 / 040]									
146093	[Vmax=.844;Dmax=3.929]									
146094	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146095	CONTINUED STANDYD	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146096	[RFD=1.00] out	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146097	[L/S= 1667 / .067 / 040]									
146098	[Vmax=.844;Dmax=3.929]									
146099	SUM	1.0	02/28/MSA	4844.07	86.418	MoDate	35/12	37.97	n/a	.000
146100										

16083# + 1.0 02:PC-01-S 8.03 .756 No_Date 2749 54.46 n/a .000
16084# 16084# 16084# 16084# 16084#
16085# + 1.0 02:PC-03-S 7.37 1.019 No_Date 2806 64.90 n/a .000
16086# SUM# 54658.51 144.704 No_Date 3648 39.28 n/a .000
16087# ROUTE ID#003005 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16088# SAVE HYD 1.0 01:24:41 54658.51 144.704 No_Date 3648 39.28 n/a .000
16089# Frame 14241.0100
16090# remark:Total Flows at Ken-Burrett Outlet
16091# # Hydrograph from Node to Station 3633
16092# # Channel X-Section obtained from RYCA Hydraulic Model - Station 3633
16093# # JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and change the slope from 0.0489 to 0.2
16094# ROUTE ID#003009 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16095# ROUTE CHANNEL --> 1.0 02:SM_KB 54658.51 144.704 No_Date 3648 39.28 n/a .000
16096# (RDV: 1.00) out< 1.0 01:24:41 54658.51 144.704 No_Date 3648 39.28 n/a .000
16097# (L/S= 294./ 109./035)
16098# (Max 1.394/Imax 3.198)
16099# ROUTE ID#003007 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16100# ADD HYD + 1.0 02:42:04 54658.51 144.704 No_Date 3644 39.28 n/a .000
16101# (RDV: 1.0) out< 1.0 02:42:04 54658.51 144.704 No_Date 3648 39.28 n/a .000
16102# (L/S= 294./ 109./035)
16103# (Max 1.394/Imax 3.198)
16104# 16104# 16104# 16104# 16104#
16105# SUM# 1.0 02:SM_KB 54658.20 144.787 No_Date 3644 39.29 n/a .000
16106# 16106# 16106# 16106# 16106#
16107# SAVE HYD 1.0 01:SM_KB 54658.20 144.787 No_Date 3644 39.29 n/a .000
16108# fname 1SM_KB.0100
16109# remark:Total Flows before Station 3633
16110# # Hydrograph from Station 3633 to Node Todd
16111# # Channel X-Section obtained from RYCA Hydraulic Model - Station 3633
16112# # JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and change the slope from 0.0489 to 0.2
16113# ROUTE ID#003009 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16114# ROUTE CHANNEL --> 1.0 02:SM_KB 54658.20 144.787 No_Date 3644 39.29 n/a .000
16115# (RDV: 1.0) out< 1.0 01:SM_KB 54658.20 144.787 No_Date 3648 39.29 n/a .000
16116# (L/S= 608./ 247./035)
16117# (Max 1.906/Imax 3.428)
16118# *****
16119# # Catchment Drenbank
16120# # To GreensBank Drain (south of the Jack)
16121# # - JFSA 2021-01-19 add Todd's bank pond as per JFSA_P598(04)-11, June 2016
16122# # JFSA 2021-01-19 update area to be 146.01 ha based on GDS measurements
16123# *****
16124# ROUTE ID#003011 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16125# CONTINUOUS STANDBYD 36.60 7.069 No_Date 2801 71.80 81.11 .000
16126# (XIMP: 64.TIMP: 68)
16127# (L/S= 2 CM: 77.0)
16128# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16129# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 494.0:MI: 013:ICI: 0)
16130# (ISaerClmp: 4.00: ISaerPct: 4.00)
16131# (SMIN: 31.15: SMAK:207.66: SK: 0.01)
16132# ROUTE ID#003011 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16133# ROUTE RESERVOIR --> 1.0 02:DRENBM 36.60 7.069 No_Date 2801 71.80 n/a .000
16134# (RDV: 1.0) out< 1.0 01:DRENBM 36.60 7.069 No_Date 2807 65.29 n/a .000
16135# overflow <= 1.0 03:DRENBM 0.00 0.00 No_Date 0.00 0.00 n/a .000
16136# (MfctCode= 9808E-04 n3, TotVolVol: 1094E-03 n3, N-Ofv: 1, TotDurDvt: 0.hrs)
16137# ROUTE ID#003011 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16138# ADD HYD + 1.0 02:TO 54658.20 144.732 No_Date 3650 39.29 n/a .000
16139# (RDV: 1.0) out< 1.0 02:DRENBM 36.60 7.069 No_Date 2807 71.80 n/a .000
16140# (L/S= 608./ 247./035)
16141# (Max 1.906/Imax 3.428)
16142# ROUTE ID#003011 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16143# SAVE HYD 1.0 01:DRENBM 54717.80 144.894 No_Date 3650 39.32 n/a .000
16144# fname 1DRENBM.0100
16145# remark:Total Flows at GreensBank Drain
16146# # Catchment TODD
16147# # To Todd's Drain (south of the Jack)
16148# # - Subdivision with 438 imp. as per Barhaven South MSS
16149# # - 2020-11-30 increase imp. based on P598(04)-11
16150# # - 2020-11-30 update Tributary Drainage area to = 146.015 ha based on P598(04)-11
16151# # 2020-11-30 split TODD Drainage Area to WAHOR, MINOR, POND and ALL
16152# *****
16153# # JFSA 2021-01-19 add TODD's area as part of Clarke's W.CLAR.M1 and remove it from Todd
16154# # JFSA 2021-01-19 update area to be 146.015 ha based on GDS measurements
16155# *****
16156# ROUTE ID#003011 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16157# CONTINUOUS STANDBYD 1.0 01:TODD_M2 21.0 42.8 No_Date 2800 66.78 75.4 .000
16158# (XIMP: 53.TIMP: 57)
16159# (L/S= 2 CM: 77.0)
16160# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16161# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 118.0:MI: 013:ICI: 0)
16162# (ISaerClmp: 4.00: ISaerPct: 4.00)
16163# (SMIN: 31.15: SMAK:207.66: SK: 0.01)
16164# ROUTE ID#003015 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16165# CONTINUOUS STANDBYD 1.0 01:TODD_M3 12. 025 No_Date 2800 66.78 75.4 .000
16166# (XIMP: 53.TIMP: 57)
16167# (L/S= 2 CM: 77.0)
16168# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16169# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 28.0:MI: 013:ICI: 0)
16170# (ISaerClmp: 4.00: ISaerPct: 4.00)
16171# (SMIN: 31.15: SMAK:207.66: SK: 0.01)
16172# ROUTE ID#003016 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16173# CONTINUOUS STANDBYD 1.0 01:TODD_M4 30.23 5.210 No_Date 2802 68.65 77.9 .000
16174# (XIMP: 52.TIMP: 64)
16175# (L/S= 2 CM: 77.0)
16176# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16177# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 449.0:MI: 013:ICI: 0)
16178# (ISaerClmp: 4.00: ISaerPct: 4.00)
16179# (SMIN: 31.15: SMAK:207.66: SK: 0.01)
16180# ROUTE ID#003011 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16181# CONTINUOUS STANDBYD 1.0 01:TODD_ALL 132.91 37.286 No_Date 2804 66.60 75.2 .000
16182# (XIMP: 52.TIMP: 57)
16183# (L/S= 2 CM: 77.0)
16184# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16185# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 668.0:MI: 013:ICI: 0)
16186# (ISaerClmp: 4.00: ISaerPct: 4.00)
16187# (SMIN: 31.15: SMAK:207.66: SK: 0.01)
16188# ROUTE ID#003018 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16189# CONTINUOUS STANDBYD 1.0 01:TODD_P 3.06 .669 No_Date 2800 70.28 79.4 .000
16190# (L/S= 2 CM: 77.0)
16191# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16192# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 143.0:MI: 013:ICI: 0)
16193# (ISaerClmp: 4.00: ISaerPct: 4.00)
16194# (SMIN: 31.15: SMAK:207.66: SK: 0.01)
16195# # Year = 124 Capture
16196# ROUTE ID#003018 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16197# ROUTE RESERVOIR --> 1.0 02:TODD_M2 30.23 5.210 No_Date 2802 68.65 n/a .000
16198# (RDV: 1.0) out< 1.0 01:TODD_P 3.06 .669 No_Date 2804 66.78 n/a .000
16199# overflow <= 1.0 03:TODD_M2 1.99 1.889 No_Date 2802 68.65 n/a .000
16200# (MfctCode= 112E-03 n3, TotVolVol: 1094E-03 n3, N-Ofv: 1, TotDurDvt: 0.hrs)
16201# # Year = 124 Capture
16202# ROUTE ID#003020 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16203# ROUTE RESERVOIR --> 1.0 02:TODD_P 3.06 .669 No_Date 2800 66.78 n/a .000
16204# (RDV: 1.0) out< 1.0 01:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16205# (L/S= 2 CM: 77.0)
16206# (MfctCode= 1204E-03 n3, TotVolVol: 728E-02 n3, N-Ofv: 1, TotDurDvt: 0.hrs)
16207# *****
16208# ROUTE ID#003021 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16209# ROUTE RESERVOIR --> 1.0 02:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16210# (RDV: 1.0) out< 1.0 01:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16211# (L/S= 2 CM: 77.0)
16212# (MfctCode= 9808E-04 n3, TotVolVol: 393E-03 n3, N-Ofv: 1, TotDurDvt: 0.hrs)
16213# *****
16214# ROUTE ID#003022 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16215# CONTINUOUS STANDBYD 1.0 01:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16216# (XIMP: 42.TIMP: 52)
16217# (L/S= 2 CM: 77.0)
16218# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16219# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 566.0:MI: 013:ICI: 0)
16220# (ISaerClmp: 4.00: ISaerPct: 4.00)
16221# (SMIN: 31.15: SMAK:225.43: SK: 0.01)
16222# ROUTE ID#003023 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16223# COMPUTE DUALHYD 1.0 01:A2 25.50 3.886 No_Date 2803 62.46 n/a .000
16224# Major System / 1.0 02:A2-M2 1.43 2.026 No_Date 2804 62.46 n/a .000
16225# Minor System \ 1.0 03:A2-M1 24.07 1.818 No_Date 2750 62.71 n/a .000
16226# (MfctCode= 940E-03, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16227# *****
16228# ROUTE ID#003024 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16229# ADD HYD + 1.0 02:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16230# (RDV: 1.0) out< 1.0 02:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16231# (L/S= 2 CM: 77.0)
16232# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16233# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 566.0:MI: 013:ICI: 0)
16234# (ISaerClmp: 4.00: ISaerPct: 4.00)
16235# (SMIN: 31.15: SMAK:225.43: SK: 0.01)
16236# ROUTE ID#003025 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16237# COMPUTE DUALHYD 1.0 01:A2 25.50 3.886 No_Date 2803 62.46 n/a .000
16238# Major System / 1.0 02:A2-M2 1.43 2.026 No_Date 2804 62.46 n/a .000
16239# Minor System \ 1.0 03:A2-M1 24.07 1.818 No_Date 2750 62.71 n/a .000
16240# (MfctCode= 940E-03, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16241# *****
16242# ROUTE ID#003026 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16243# ADD HYD + 1.0 02:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16244# (RDV: 1.0) out< 1.0 02:TODD_Min3 1.99 .268 No_Date 2752 66.78 n/a .000
16245# (L/S= 2 CM: 77.0)
16246# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16247# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 566.0:MI: 013:ICI: 0)
16248# (ISaerClmp: 4.00: ISaerPct: 4.00)
16249# (SMIN: 31.15: SMAK:225.43: SK: 0.01)
16250# ROUTE ID#003027 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16251# COMPUTE DUALHYD 1.0 01:A2 25.50 3.886 No_Date 2803 62.46 n/a .000
16252# Major System / 1.0 02:A2-M2 1.43 2.026 No_Date 2804 62.46 n/a .000
16253# Minor System \ 1.0 03:A2-M1 24.07 1.818 No_Date 2750 62.71 n/a .000
16254# (MfctCode= 940E-03, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16255# *****
16256# ROUTE ID#003028 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16257# SAVE HYD 1.0 01:SM_TO 54840.69 145.243 No_Date 3649 39.38 n/a .000
16258# fname 1SM_TO.0100
16259# remark:Total Flows at Todd Drain
16260# # Hydrograph from Todd Drain routed to Corrigan Drain
16261# # Channel X-Section obtained from RYCA Hydraulic Model - Station 2462
16262# # 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model will be more stable and g
16263# ROUTE ID#003029 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16264# ROUTE RESERVOIR --> 1.0 02:TRN_P3 121.35 15.844 No_Date 2809 66.71 n/a .000
16265# (RDV: 1.0) out< 1.0 01:TRN_P3 121.35 15.844 No_Date 2809 66.71 n/a .000
16266# overflow <= 1.0 03:TRN_P3 0.00 0.00 No_Date 0.00 0.00 n/a .000
16267# (MfctCode= 9808E-04 n3, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16268# *****
16269# ROUTE ID#003027 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16270# ADD HYD + 1.0 02:SM_P3 121.35 15.844 No_Date 2809 66.71 n/a .000
16271# (RDV: 1.0) out< 1.0 02:TRN_P3 121.35 15.844 No_Date 2809 66.71 n/a .000
16272# (L/S= 2 CM: 77.0)
16273# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16274# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 566.0:MI: 013:ICI: 0)
16275# (ISaerClmp: 4.00: ISaerPct: 4.00)
16276# (SMIN: 31.15: SMAK:225.43: SK: 0.01)
16277# ROUTE ID#003028 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16278# COMPUTE DUALHYD 1.0 01:A2 25.50 3.886 No_Date 2803 62.46 n/a .000
16279# Major System / 1.0 02:A2-M2 1.43 2.026 No_Date 2804 62.46 n/a .000
16280# Minor System \ 1.0 03:A2-M1 24.07 1.818 No_Date 2750 62.71 n/a .000
16281# (MfctCode= 940E-03, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16282# *****
16283# ROUTE ID#003029 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16284# ADD HYD + 1.0 02:SM_P3 121.35 15.844 No_Date 2809 66.71 n/a .000
16285# (RDV: 1.0) out< 1.0 03:A2-M2 1.43 2.026 No_Date 2804 62.46 n/a .000
16286# (L/S= 2 CM: 77.0)
16287# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16288# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 566.0:MI: 013:ICI: 0)
16289# (ISaerClmp: 4.00: ISaerPct: 4.00)
16290# (SMIN: 31.15: SMAK:225.43: SK: 0.01)
16291# ROUTE ID#003030 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16292# CONTINUOUS STANDBYD 1.0 01:MK33 17.35 1.488 No_Date 2835 62.96 n/a .000
16293# (XIMP: 44.TIMP: 47)
16294# (L/S= 2 CM: 75.0)
16295# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16296# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 345.0:MI: 013:ICI: 0)
16297# (ISaerClmp: 4.00: ISaerPct: 4.00)
16298# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16299# ROUTE ID#003031 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16300# CONTINUOUS STANDBYD 1.0 01:MK33 17.35 1.488 No_Date 2835 62.96 n/a .000
16301# (XIMP: 44.TIMP: 47)
16302# (L/S= 2 CM: 75.0)
16303# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16304# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 345.0:MI: 013:ICI: 0)
16305# (ISaerClmp: 4.00: ISaerPct: 4.00)
16306# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16307# ROUTE ID#003032 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16308# COMPUTE DUALHYD 1.0 01:A4 1.27 .266 No_Date 2800 70.58 n/a .000
16309# Major System / 1.0 02:A4-MJ 1.60 .346 No_Date 2801 73.40 n/a .000
16310# Minor System \ 1.0 03:A4-MN 1.56 .339 No_Date 2800 73.40 n/a .000
16311# (MfctCode= 0.000E+00, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16312# *****
16313# ROUTE ID#003033 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16314# ADD HYD + 1.0 02:A4-MJ 1.60 .346 No_Date 2801 73.40 n/a .000
16315# (RDV: 1.0) out< 1.0 02:A4-MN 1.56 .339 No_Date 2800 73.40 n/a .000
16316# (L/S= 2 CM: 75.0)
16317# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16318# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 293.0:MI: 013:ICI: 0)
16319# (ISaerClmp: 4.00: ISaerPct: 4.00)
16320# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16321# ROUTE ID#003033 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16322# COMPUTE DUALHYD 1.0 01:A4 1.27 .266 No_Date 2800 70.58 n/a .000
16323# Major System / 1.0 02:A4-MJ 1.60 .346 No_Date 2801 73.40 n/a .000
16324# Minor System \ 1.0 03:A4-MN 1.56 .339 No_Date 2800 70.58 n/a .000
16325# (MfctCode= 0.000E+00, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16326# *****
16327# ROUTE ID#003034 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16328# ADD HYD + 1.0 02:A4-MJ 1.60 .346 No_Date 2801 73.40 n/a .000
16329# (RDV: 1.0) out< 1.0 02:A4-MN 1.56 .339 No_Date 2800 73.40 n/a .000
16330# (L/S= 2 CM: 75.0)
16331# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16332# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 293.0:MI: 013:ICI: 0)
16333# (ISaerClmp: 4.00: ISaerPct: 4.00)
16334# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16335# ROUTE ID#003034 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16336# SUM# 1.0 01:MK101 48.13 4.085 No_Date 2802 59.20 n/a .000
16337# (L/S= 2 CM: 75.0)
16338# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16339# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 293.0:MI: 013:ICI: 0)
16340# (ISaerClmp: 4.00: ISaerPct: 4.00)
16341# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16342# ROUTE ID#003040 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16343# SAVE HYD 1.0 01:MK101 48.13 4.085 No_Date 2802 59.20 n/a .000
16344# fname MK101.0100
16345# remark:Total Flows at MK101
16346# ROUTE ID#003041 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16347# CONTINUOUS STANDBYD 1.0 01:MK101 48.13 4.085 No_Date 2802 59.20 n/a .000
16348# (RDV: 1.0) out< 1.0 01:MK101 48.13 4.085 No_Date 2803 59.20 n/a .000
16349# (L/S= 2 CM: 75.0)
16350# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16351# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 300.0:MI: 013:ICI: 0)
16352# (ISaerClmp: 4.00: ISaerPct: 4.00)
16353# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16354# ROUTE ID#003041 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16355# CONTINUOUS STANDBYD 1.0 01:MK101 48.13 4.085 No_Date 2802 59.20 n/a .000
16356# (XIMP: 71.TIMP: 71)
16357# (L/S= 2 CM: 75.0)
16358# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16359# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 300.0:MI: 013:ICI: 0)
16360# (ISaerClmp: 4.00: ISaerPct: 4.00)
16361# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16362# ROUTE ID#003045 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16363# ADD HYD + 1.0 02:A4-MJ 1.60 .346 No_Date 2801 73.40 n/a .000
16364# (RDV: 1.0) out< 1.0 02:A4-MN 1.56 .339 No_Date 2801 73.40 n/a .000
16365# (L/S= 2 CM: 75.0)
16366# (Previous area: Ipaer: 4.67:SLPp:1.00:LDP: 40.0:IMP: 250:ICP: 0)
16367# (Impervious area: IAlmp: 1.57:SLP:1.00:LDI: 300.0:MI: 013:ICI: 0)
16368# (ISaerClmp: 4.00: ISaerPct: 4.00)
16369# (SMIN: 31.81: SMAK:215.43: SK: 0.01)
16370# ROUTE ID#003047 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16371# COMPUTE DUALHYD 1.0 01:A4 1.60 .346 No_Date 2801 73.40 n/a .000
16372# Major System / 1.0 02:A4-MJ 1.60 .346 No_Date 2801 73.40 n/a .000
16373# Minor System \ 1.0 03:A4-MN 1.56 .339 No_Date 2801 73.40 n/a .000
16374# (MfctCode= 0.000E+00, TotVolVol: 0.000E+00, N-Ofv: 0, TotDurDvt: 0.hrs)
16375# *****
16376# ROUTE ID#003047 -----DtmIn:ID#HYD-----ARESHA-GPEARCSms-TpeaDate_hm.....Rvm-R-C.....DWFCSms
16377# ROUTE RESERVOIR --> 1.0 02:MK101 48.13 4.085 No_Date 2802 59.20 n/a .000
16378# (RDV: 1.0) out< 1.0 01:MK101 48.13 4.085 No_Date 2812 60.37 n/a .000
16379# (L/S=


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16831 ROUTE RESERVOIR -> 1.0 01:01:MI_P 175.99 20.390 No_date 28:06 56.87 n/a .000
16832 out <= 1.0 01:01:MI_P 146.22 4.050 No_date 28:08 56.87 n/a .000
16833 overflow <= 1.0 01:01:MI_P 29.77 16.228 No_date 28:08 56.87 n/a .000
16834 (MxOv=2.110) out <= 1.0 01:01:MI_P 29.77 16.228 No_date 28:08 56.87 n/a .000
16835 R0100:CO0426-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16836 ADD HYD + 1.0 02:MI 5520.07 145.768 No_date 36:59 39.46 n/a .000
16837 + 1.0 02:MI_P 146.22 4.050 No_date 28:08 56.87 n/a .000
16838 SUM+ 1.0 01:SN_DE 55196.05 146.199 No_date 36:58 39.51 n/a .000
16839 R0100:CO0427-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16840 SAVE HYD 1.0 01:SN_DE 55196.05 146.199 No_date 36:58 39.51 n/a .000
16841 frame :SN_MI_0100
16842 remark:Total Flows at Jockvale Road
16843 #
16844 #
16845 # Hydrograph from Jockvale Road routed to Heart's Desire
16846 # Channel X-Section obtained from RWCA Hydraulic Model - Station 689
16847 #
16848 R0100:CO0428-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16849 ROUTE CHANNEL + 1.0 02:MI 55196.05 146.199 No_date 36:58 39.51 n/a .000
16850 [RD+ 1.00] out <= 1.0 01:MI_DE 55196.05 146.071 No_date 37:13 39.51 n/a .000
16851 [L/S/m= 192.7 / 227.048]
16852 [Vmax= 1.642:Dmax= 2.661]
16853 #
16854 # Catchment DESIRE
16855 # - To Jock River (north of the Jock)
16856 # Rural-subur subdivision (Heart's Desire Community)
16857 #
16858 R0100:CO0429-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16859 CONTINUOUS STANDHYD 1.0 01:DESIRE 23.78 3.004 No_date 28:03 53.11 .600 .000
16860 [XIMP= 25:TIMP= 25]
16861 [LOSS= 2 :CM= 77.0]
16862 [Previous area IArea= 4.67:SLIP=1.00:IDW= 40.0:MPD= 250:SCP= 0]
16863 [Impervious area IArea= 1.57:SLIP=1.00:IDW= 400.0:MI= 013:ICI= 0]
16864 [IAREClimp= 4.00: IAREKPer= 4.00]
16865 [SIN= 31.35: SMAX= 44.49: SK= 010]
16866 #
16867 # Catchment JOCKVA
16868 # - To Jockvale SSM Facility
16869 # - Residential development & golf course
16870 # JESA 2021-01-11 update JOCKVA only after updating COBJG as per IRI (GROU, July 2008.
16871 # JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX-LAND 32.5 ha as
16872 #
16873 R0100:CO0430-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16874 CONTINUOUS STANDHYD 1.0 01:JOCKVA 225.13 28.623 No_date 28:07 62.70 .708 .000
16875 [XIMP= 25:TIMP= 50]
16876 [LOSS= 2 :CM= 74.0]
16877 [Previous area IArea= 4.67:SLIP=1.00:IDW= 40.0:MPD= 250:SCP= 0]
16878 [Impervious area IArea= 1.57:SLIP=1.00:IDW= 400.0:MI= 013:ICI= 0]
16879 [IAREClimp= 4.00: IAREKPer= 4.00]
16880 [SIN= 31.35: SMAX= 44.49: SK= 010]
16881 R0100:CO0431-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16882 ADD HYD + 1.0 02:MI 55196.05 146.071 No_date 37:13 39.51 n/a .000
16883 + 1.0 02:JOCKVA 225.13 28.623 No_date 28:07 62.70 n/a .000
16884 + 1.0 02:MI_P 36.420 No_date 28:05 62.88 n/a .000
16885 + 1.0 02:MI_P 19.402 No_date 28:04 62.88 n/a .000
16886 SUM+ 1.0 01:JOCKVA 256.41 31.850 No_date 28:06 62.71 n/a .000
16887 R0100:CO0432-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16888 SAVE HYD 1.0 01:JOCKVA 256.41 31.850 No_date 28:06 62.71 n/a .000
16889 frame :JOCKVA-to-0100
16890 remark:Total Flows at KB first pond
16891 #
16892 # Jockvale SSM Facility
16893 # Rating curve obtained from Jockvale Servicing Study (CC, 1999)
16894 #
16895 R0100:CO0433-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16896 ROUTE RESERVOIR + 1.0 01:JOCKVA 256.41 31.850 No_date 28:06 62.71 n/a .000
16897 out <= 1.0 01:JOCKVA 256.41 12.850 No_date 28:35 62.71 n/a .000
16898 overflow <= 1.0 01:JOCKVA 0.00 0.00 No_date 0:00 .00 n/a .000
16899 (MxOv=2.430) out <= 1.0 01:JOCKVA 0.00 0.00 No_date 0:00 .00 n/a .000
16900 R0100:CO0434-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16901 ADD HYD + 1.0 02:MI_DE 55196.05 146.071 No_date 37:13 39.51 n/a .000
16902 + 1.0 01:DESIRE 23.78 3.004 No_date 28:03 53.11 n/a .000
16903 + 1.0 02:MI_P 36.420 No_date 28:05 62.88 n/a .000
16904 + 1.0 02:MI_P 19.402 No_date 28:04 62.88 n/a .000
16905 SUM+ 1.0 01:SN_DE 55476.25 147.027 No_date 37:12 39.63 n/a .000
16906 R0100:CO0435-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16907 SAVE HYD 1.0 01:SN_DE 55476.25 147.027 No_date 37:12 39.63 n/a .000
16908 frame :SN_DE_0100
16909 remark:Total Flows at Heart's Desire
16910 #
16911 # Hydrograph from Heart's Desire routed to Rideau River
16912 # Channel X-Section obtained from RWCA Hydraulic Model - Station 0
16913 #
16914 R0100:CO0436-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16915 ROUTE CHANNEL + 1.0 02:MI_DE 55476.25 147.027 No_date 37:12 39.63 n/a .000
16916 [RD+ 1.00] out <= 1.0 01:MI 55476.25 147.014 No_date 37:15 39.63 n/a .000
16917 [L/S/m= 563.7 / 367.048]
16918 [Vmax= 2.219:Dmax= 1.78]
16919 #
16920 # Catchment 0-0
16921 # - To Jock River (north and south)
16922 # Undeveloped floodplain and river
16923 #
16924 R0100:CO0437-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16925 CONTINUOUS STANDHYD 1.0 01:0-2 102.94 5.685 No_date 28:20 40.95 .462 .000
16926 [CM= 72.0: N= 3.00: Tpe= 40]
16927 [IAREC= 4.00: SMAX= 75.75: SMAX= 264.99: SK= 010]
16928 [InterEventTime= 12.00]
16929 R0100:CO0438-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16930 ADD HYD + 1.0 02:MI 55476.25 147.014 No_date 37:15 39.63 n/a .000
16931 + 1.0 02:0-2 102.94 5.685 No_date 28:20 40.95 n/a .000
16932 SUM+ 1.0 01:SN_MI 55579.19 147.276 No_date 37:15 39.63 n/a .000
16933 R0100:CO0439-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16934 SAVE HYD 1.0 01:SN_MI 55579.19 147.276 No_date 37:15 39.63 n/a .000
16935 frame :SN_MI_0100
16936 remark:Total Flows at Rideau River
16937 #
16938 R0100:CO0502-----Dhain-ID:BNYD-----AREha-QPEARcm-TpeakDate,hh:mm-----RvM-R-C-----DWFCm
16939 FINISH
16940 #
16941 #
16942 # WARNINGS / ERRORS / NOTES
16943 #
16944 R0202:CO0319 ROUTE RESERVOIR ->
16945 *** WARNING: Inflow peak was not reduced! Check OUTFLOW/STORAGE table or reduce DT.
16946 R0202:CO0341 ROUTE PIPE ->
16947 *** WARNING: New pipe size used for routing.
16948 R0202:CO0347 ROUTE PIPE ->
16949 *** WARNING: New pipe size used for routing.
16950 R0202:CO0386 DIVERT HYD ->
16951 *** NOTE: Inflow hyd. is dry and cannot be diverted.
16952 R0202:CO0400 ROUTE PIPE ->
16953 *** WARNING: New pipe size used for routing.
16954 R0202:CO0407 ROUTE PIPE ->
16955 *** WARNING: New pipe size used for routing.
16956 R0202:CO0408 ROUTE PIPE ->
16957 *** WARNING: New pipe size used for routing.
16958 R0202:CO0416 ROUTE PIPE ->
16959 *** WARNING: New pipe size used for routing.
16960 R0202:CO0417 ROUTE PIPE ->
16961 *** WARNING: New pipe size used for routing.
16962 R0202:CO0319 ROUTE RESERVOIR ->
16963 *** WARNING: Inflow peak was not reduced! Check OUTFLOW/STORAGE table or reduce DT.
16964 R0202:CO0341 ROUTE PIPE ->
16965 *** WARNING: New pipe size used for routing.
16966 R0202:CO0347 ROUTE PIPE ->
16967 *** WARNING: New pipe size used for routing.
16968 R0202:CO0386 DIVERT HYD ->
16969 *** WARNING: New pipe size used for routing.
16970 R0202:CO0400 ROUTE PIPE ->
16971 *** WARNING: New pipe size used for routing.
16972 R0202:CO0407 ROUTE PIPE ->
16973 *** WARNING: New pipe size used for routing.
16974 R0202:CO0408 DIVERT HYD ->
16975 *** NOTE: Inflow hyd. is dry and cannot be diverted.
16976 R0202:CO0395 ROUTE PIPE ->
16977 *** WARNING: New pipe size used for routing.
16978 R0202:CO0400 ROUTE PIPE ->
16979 *** WARNING: New pipe size used for routing.
16980 R0202:CO0407 ROUTE PIPE ->
16981 *** WARNING: New pipe size used for routing.
16982 R0202:CO0408 ROUTE PIPE ->
16983 *** WARNING: New pipe size used for routing.
16984 R0202:CO0416 ROUTE PIPE ->
16985 *** WARNING: New pipe size used for routing.
16986 R0202:CO0417 ROUTE PIPE ->
16987 *** WARNING: New pipe size used for routing.
16988 R0202:CO0341 ROUTE PIPE ->
16989 *** WARNING: New pipe size used for routing.
16990 R0202:CO0347 ROUTE PIPE ->
16991 *** WARNING: New pipe size used for routing.
16992 R0202:CO0386 DIVERT HYD ->
16993 *** WARNING: New pipe size used for routing.
16994 R0202:CO0400 ROUTE PIPE ->
16995 *** WARNING: New pipe size used for routing.
16996 R0202:CO0407 ROUTE PIPE ->
16997 *** WARNING: New pipe size used for routing.
16998 R0202:CO0408 DIVERT HYD ->
16999 *** NOTE: Inflow hyd. is dry and cannot be diverted.
17000 R0202:CO0395 ROUTE PIPE ->
17001 *** WARNING: New pipe size used for routing.
17002 R0202:CO0400 ROUTE PIPE ->
17003 *** WARNING: New pipe size used for routing.
17004 R0202:CO0407 ROUTE PIPE ->
17005 *** WARNING: New pipe size used for routing.
17006 R0202:CO0408 ROUTE PIPE ->
17007 *** WARNING: New pipe size used for routing.
17008 R0202:CO0416 ROUTE PIPE ->
17009 *** WARNING: New pipe size used for routing.
17010 R0202:CO0417 ROUTE PIPE ->
17011 *** WARNING: New pipe size used for routing.
17012 R0202:CO0341 ROUTE PIPE ->
17013 *** WARNING: New pipe size used for routing.
17014 R0202:CO0347 ROUTE PIPE ->
17015 *** WARNING: New pipe size used for routing.
17016 R0202:CO0386 DIVERT HYD ->
17017 *** WARNING: New pipe size used for routing.
```

Attachment F

Updated Subcatchment Schematics & Tables



Legend
 — Channel Cross Sections
 S-1 Sub-catchments and Fraser Sub-catchments
 S-1 Sub-catchments
 FRASER-DRN
 FRASER-D
 Google Hybrid

File name:
 Figure 4A - S-1 & Fraser Clarke Sub-catchments.pdf

XS 3633 Cross Section at station 3633

S-1-A	▼ Area ID
90.84 ha	► Area (ha)
Ref. 1	► Reference Number

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PROJECT :
 BCDC - Quantity Control Study

TITLE :
 Figure 4A - S-1 & Fraser Clarke Sub-catchments
 Table 4A - S-1 & Fraser Clarke Sub-catchments

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021

Station	Area (ha)	Area ID	Reference
3633	90.84	S-1-A	Ref. 1
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Ref.	Name ID (S-1 is developed and Fraser is developed with DUALHYD)	Description	Area (ha)	Major System To	Minor System To	TP (hr) [TIMP]	Slope (%)		LGI (m)	CN	DUALHYD parameters		ROUTE CHANNEL (Station 5002)							
							SLPP (Pervious)	SLPI (Impervious)			Max Flow Rate(cms)-5-year inlet capture rate (24-hour SCS)	Major System storage (cu-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)		
1	S-1-A	will remain undeveloped	75.88	SN_FO (Total Flows at Foster Drain)		0.619												-1060.52	94	
2	S-1-B	will remain undeveloped	55.36	4241 (Total Flows before Station 4241)		0.451												-268.6	91.5	
3	S-1-D1	will be Developed	21.67	5002 (Total Flows before Station 5002)		[0.65]	2	0.75	380.088	75								-259.43	91.5	
4	S-1-D2	will be Developed	18.67	SN_OK (Total Flows at Okeefe Drain)		[0.65]	2	0.75	352.798	75								-179.48	91.5	
5	S-1-D3	will be Developed	6.79	SN_OK (Total Flows at Okeefe Drain)		[0.65]	2	0.75	212.760	75								-67.9	91.5	
6	S-1-D4	will be Developed	3.28	SN_CE (Total Flows before Station 5737)		[0.65]	2	0.75	147.874	75								-59.21	91.5	
7	S-1-D5	will be Developed	12.84	SN_CE (Total Flows before Station 5737)		[0.65]	2	0.75	292.575	75								-33.19	91	
8	S-1-D6	will be Developed	1.75	5002 (Total Flows before Station 5002)		[0.65]	2	0.75	108.012	75								-26.08	90.5	
9	S-1-D7	will be Developed	2.03	5002 (Total Flows before Station 5002)		[0.65]	2	0.75	116.333	75								-24.04	90	
10	S-1-D8	will be Developed	5.27	SN_FO (Total Flows at Foster Drain)		[0.65]	2	0.75	187.439									-13.14	86.77	
11	S-1-FO-D1	was part of Foster (Developed)	5.11	520 (Total Flows at Station 520 on Foster Drain)		[0.65]	0.5	0.5	184.572	74								0	85	
12	S-1-FO-D2	was part of Foster (Developed)	4.94	980 (Total Flows at Station 980 on Foster Drain)		[0.55]	0.5	0.5	181.475	74								14.68	86.74	
13	S-1-FO-F-D	was part of Foster (will be Developed in the Future)	14.96	SN_FO (Total Flows at Foster Drain)		[0.65]	0.5	0.5	315.806	74								23.92	90	
14	S-1-Okeefe	was part of O'Keefe	44.93	SN_416 (Total Flows after Highway 416 and before XS Station 7245)		[0.65]	2	0.75	547.296	75								25.78	90.5	
																		31.91	91	
																		31.91	91.5	
																		772.15	92	
																			961.49	92.5
																			1044.69	93
																			1130.8	95
																			-601.81	91.5
																			-37.5	90
																			-19.61	87.04
																			0	85.7
																			14.87	86.93
																			37.5	90
																			38.54	90.5
																			42.23	91
																			157.05	91.5
																			-1082.01	94
																			-1028.17	92.5
																			-992.3	93.5
																			-279.34	90
																			-23.63	90
																			-13.45	87.13
																			-0.07	86.24
																			10.54	87.15
																			23.63	90
																			24.86	90.5
																			26.72	91
																			45.07	91.5
																			128.17	91.5
																			270.7	92.5
																			728.3	95
15	FRASER-D	Fraser Clarke Drain (will remain undeveloped)	21.61	4241 (Total Flows at Ken-Burnett Outlet and before XS Station 4241)		[0.585]	1	1	379.561	80		9999999 (an imaginary number so all water is kept inside)							-909.72	95
16	FRASER-DRN	Fraser Clarke Developed Area	13.65	4241 (Total Flows at Ken-Burnett Outlet and before XS Station 4241)		0.4258				77									-907.09	94.5
																			-904.65	94
																			-902.26	93.5
																			-44.51	91.5
																			-25.1	91.5
																			-20.98	91
																			-20.61	90.5
																			-20.12	90
																			-6.13	87.26
																			17.51	86.56
																			31.37	87.2
																			45.26	90
																			50.41	90.5
																			63.06	91
																			134.5	91.5
																			190.63	92
251.98	92.5																			
321.32	93.5																			
403.84	95																			
																			-29.24	91
																			-27.41	90.5
																			-25.64	90
																			-23.74	89.5
																			-22	89.26
																			-20	88.51
																			-19	88.32
																			-15	88.1
																			-10	88.11
																			-5	88.17
																			0	88.27
																			5	88.19
																			10	88.06
																			15	88.48
																			16	88.7
																			23.74	89.5
																			24.68	90
																			25.57	90.5
																			26.5	91

(1) N_CE is NHYDout from ROUTE CHANNEL at Channel X-Section obtained from RVCA Hydraulic Model - Station 6016 (Hydrograph from Node Foster routed to Node at Cedarview Road)

(2) "MS_P2" and "P2-OVF" are the outflow and the overflow from ROUTE RESERVOIR of the Clarke sub-catchment

(3) "KB-Pond3" = Total Flows at KB third pond

(4 & 5 & 6 & 7) "FC-01-S" & "FC-02-S" & "FC-03-S" & "FC-04-S" = Existing / Proposed Subcatchments To Fraser Clarke Drain as per NOVATECH Report June, 2020

(8 & 9) "JR-01-S" & "JR-02-S" = Proposed Subcatchments To Jock River as per NOVATECH Report June, 2020



Legend

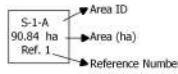
- Channel Cross Sections
- S-1 Sub-catchments and Fraser Sub-catchments
- S-1 Sub-catchments
- FRASER-DRN
- FRASER-D
- Google Hybrid

File name:

Figure 4B - S-1 & Fraser Clarke Sub-catchments.pdf

XS 3633

Cross Section at station 3633



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PROJECT :

BCDC - Quantity Control Study

TITLE :

Figure 4B - S-1 & Fraser Clarke Sub-catchments
 Table 4B - S-1 & Fraser Clarke Sub-catchments

PROJECT NO.

1474-16

DRAWN:

MM

DATE:

Mar. 2021

Station	Area (ha)	Reference	Channel Cross Section	Channel Width (m)	Channel Depth (m)	Channel Slope (m/m)	Channel Velocity (m/s)	Channel Discharge (m³/s)	Channel Capacity (m³/s)
1	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
2	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
3	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
4	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
5	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
6	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
7	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
8	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
9	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5
10	90.84	Ref. 1	Channel Cross Section	10.0	1.0	0.001	0.5	0.5	0.5



Ref.	Name ID (S-1 is developed with DUALHYD & ROUTE RESERVOIR and Fraser is developed with DUALHYD)	Description	Area (ha)	Major System To	Minor System To	TP (hr) [TIMP]	Slope (%)		LGI (m)	CN	DUALHYD parameters		ROUTE RESERVOIR					ROUTE CHANNEL (Station 5002)							
							SLPP (Pervious)	SLPI (Impervious)			Max Flow Rate (cms) - 5-year inlet capture rate (24-hour SCS)	Major System storage (cu-m)	NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)		
1	S-1-A	will remain undeveloped	75.88	SN_FO (Total Flows at Foster Drain)		0.619				77													-1060.52	94	
2	S-1-B	will remain undeveloped	55.36	4241 (Total Flows before Station 4241)		0.451				77													-268.6	91.5	
3	S-1-D1	will be Developed	21.67	5002 (Total Flows before Station 5002)		[0.65]	2	0.75	380.088	75	2.409	9999999	S-1-D1S	S-1-D1R	S-1-D1Rovf	0.3033	1.0125							-259.43	91.5
4	S-1-D2	will be Developed	18.67	SN_OK (Total Flows at Okeefe Drain)		[0.65]	2	0.75	352.798	75	2.097	9999999	S-1-D2S	S-1-D2R	S-1-D2Rovf	0.2485	0.8297							-179.48	91.5
5	S-1-D3	will be Developed	6.79	SN_OK (Total Flows at Okeefe Drain)		[0.65]	2	0.75	212.760	75	0.831	9999999	S-1-D3S	S-1-D3R	S-1-D3Rovf	0.079	0.2639							-67.9	91.5
6	S-1-D4	will be Developed	3.28	SN_CE (Total Flows before Station 5737)		[0.65]	2	0.75	147.874	75	0.421	9999999	S-1-D4S	S-1-D4R	S-1-D4Rovf	0.0392	0.1308							-59.21	91.5
7	S-1-D5	will be Developed	12.84	SN_CE (Total Flows before Station 5737)		[0.65]	2	0.75	292.575	75	1.5	9999999	S-1-D5S	S-1-D5R	S-1-D5Rovf	0.1611	0.5377							-33.19	91
8	S-1-D6	will be Developed	1.75	5002 (Total Flows before Station 5002)		[0.65]	2	0.75	108.012	75	0.232	9999999	S-1-D6S	S-1-D6R	S-1-D6Rovf	0.0222	0.0742							-26.08	90.5
9	S-1-D7	will be Developed	2.03	5002 (Total Flows before Station 5002)		[0.65]	2	0.75	116.333	75	0.265	9999999	S-1-D7S	S-1-D7R	S-1-D7Rovf	0.2767	0.9238							-24.04	90
10	S-1-D8	will be Developed	5.27	SN_FO (Total Flows at Foster Drain)		[0.65]	2	0.75	187.439		0.672	9999999	S-1-D8S	S-1-D8R	S-1-D8Rovf	0.0630	0.2102							-13.14	86.77
11	S-1-FO-D1	was part of Foster (Developed)	5.11	520 (Total Flows at Station 520 on Foster Drain)		[0.65]	0.5	0.5	184.572	74	0.605	9999999	S-1-FO-D1S	S-1-FO-D1R	S-1FOD1ovf	0.0693	0.2313							0	85
12	S-1-FO-D2	was part of Foster (Developed)	4.94	980 (Total Flows at Station 980 on Foster Drain)		[0.55]	0.5	0.5	181.475	74	0.508	9999999	S-1-FO-D2S	S-1-FO-D2R	S-1FOD2ovf	0.067	0.2236							14.68	86.74
13	S-1-FO-F-D	was part of Foster (will be Developed in the Future)	14.96	SN_FO (Total Flows at Foster Drain)		[0.65]	0.5	0.5	315.806	74	1.615	9999999	S-1-FO-F-Ds	S-1-FO-F-DR	S-1-FO-F-DR	0.2215	0.7396							23.92	90
14	S-1-Okeefe	was part of O'Keefe	44.93	SN_416 (Total Flows after Highway 416 and before XS Station 7245)		[0.65]	2	0.75	547.296	75	4.591	9999999	S-1-Oks	S-1-OksR	S-1-OksRovf	0.639	2.1333							25.78	90.5
15	FRASER-D	Fraser Clarke Drain (will remain undeveloped)	21.61	4241 (Total Flows at Ken-Burnett Outlet and before XS Station 4241)		[0.585]	1	1	379.561	80	2.281	9999999 (an imaginary number so all water is kept inside)												904.65	94
16	FRASER-DRN	Fraser Clarke Developed Area	13.65	4241 (Total Flows at Ken-Burnett Outlet and before XS Station 4241)		0.4258				77														-29.24	91
S-1 Total Area (ha)			273.48																						
FRASER Total Area (ha)			35.26																						

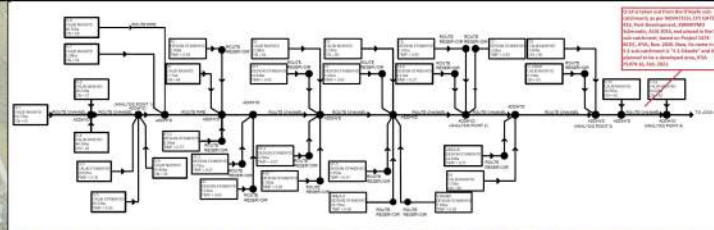
(1) N_CE is NHYDout from ROUTE CHANNEL at Channel X-Section obtained from RVCA Hydraulic Model - Station 6016 (Hydrograph from Node Foster routed to Node at Cedarview Road)

(2) "MS_P2" and "P2-OVF" are the outflow and the overflow from ROUTE RESERVOIR of the Clarke sub-catchment

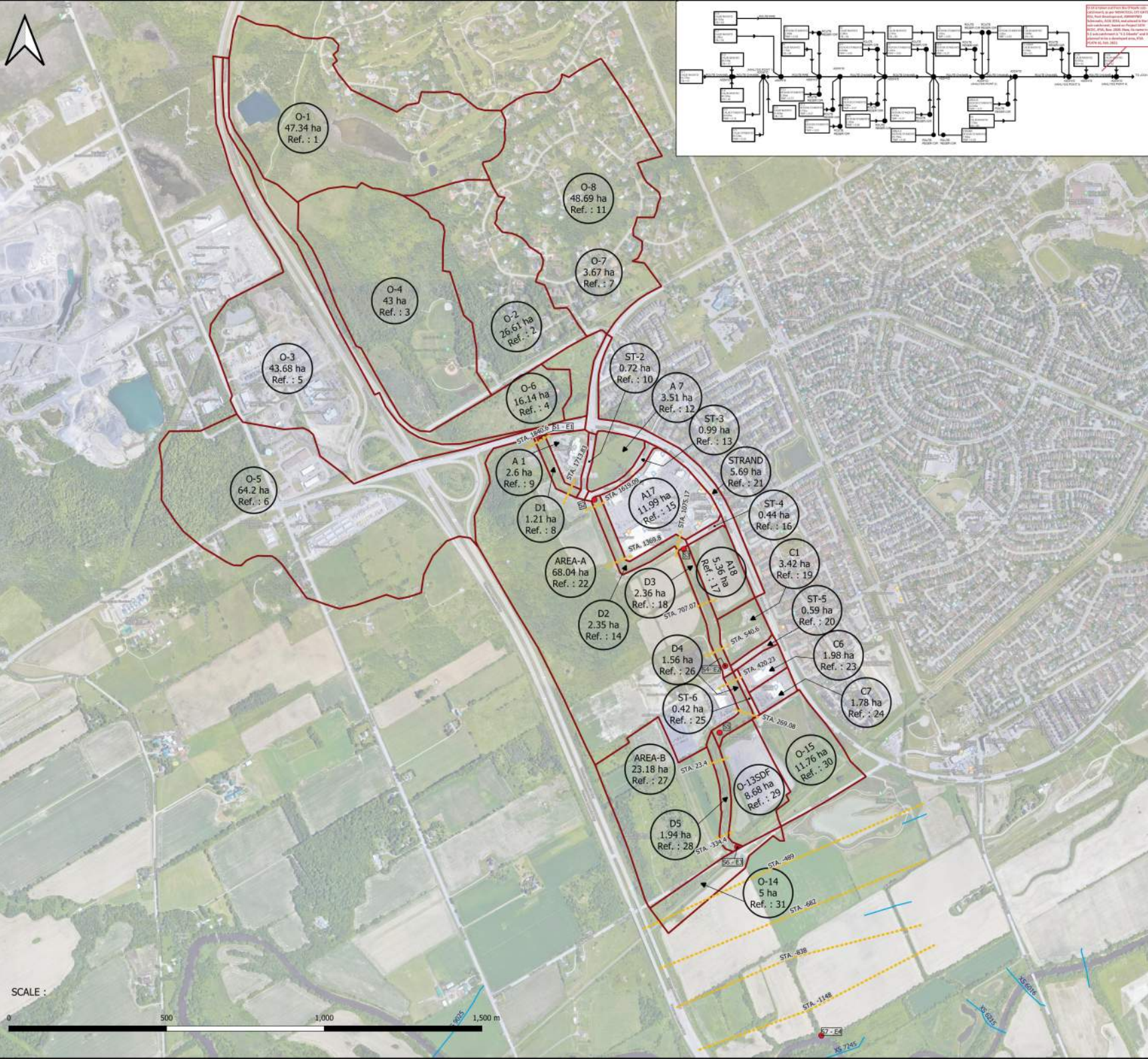
(3) "KB-Pond3" = Total Flows at KB third pond

(4, 5, 6 & 7) "FC-01-S" & "FC-02-S" & "FC-03-S" & "FC-04-S" = Existing / Proposed Subcatchments To Fraser Clarke Drain as per NOVATECH Report June, 2020

(8 & 9) "JR-01-S" & "JR-02-S" = Proposed Subcatchments To Jock River as per NOVATECH Report June, 2020



Area ID	Area (ha)	Reference Number	Area Type
O-1	47.34	1	O'Keefe Sub-catchment
O-2	26.51	2	O'Keefe Sub-catchment
O-3	43.68	5	O'Keefe Sub-catchment
O-4	43	3	O'Keefe Sub-catchment
O-5	64.2	6	O'Keefe Sub-catchment
O-6	16.14	4	O'Keefe Sub-catchment
O-7	3.67	7	O'Keefe Sub-catchment
O-8	48.69	11	O'Keefe Sub-catchment
A-1	2.6	9	Area
A-7	3.51	12	Area
A-17	11.99	15	Area
D-1	1.21	8	Area
D-2	2.35	14	Area
D-3	2.36	18	Area
D-4	1.56	26	Area
D-5	1.94	28	Area
AREA-A	68.04	22	Area
AREA-B	23.18	27	Area
STRAND	5.69	21	Area
ST-2	0.72	10	Structure
ST-3	0.99	13	Structure
ST-4	0.44	16	Structure
ST-5	0.59	20	Structure
ST-6	0.42	25	Structure
C-1	3.42	19	Area
C-6	1.98	23	Area
C-7	1.78	24	Area
O-13SDP	8.68	29	O'Keefe Sub-catchment
O-14	5	31	O'Keefe Sub-catchment
O-15	11.76	30	O'Keefe Sub-catchment



File name: Figure F1 - O'Keefe Sub-catchments.pdf

Legend
 20210129-O'Keefe Sub-catchment Boundaries XS 7245 Cross Section at station 7245
 O'Keefe Sub-catchment Boundaries
 Google Hybrid

S1 - E1 HYDROLOGIC MODELING (SWHYMO) ANALYSIS POINT ENVIRONMENTAL MANAGEMENT (EMP) ANALYSIS POINT

O-1 Area ID
 47.34 Area (ha)
 Ref. : 1 Reference Number

STA. 1840.6 CROSS SECTION AND STATION

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PROJECT : BCDC - Quantity Control Study

TITLE : Figure F1 - O'Keefe Sub-catchments
 Table F1 - O'Keefe Sub-catchments
 Schematic F1 - O'Keefe Sub-catchments

PROJECT NO.	1474
DRAWN:	MM
DATE:	Mar. 2021



Ref.	Name ID	Area (ha)	Major System In	Minor System To	I. Ingress	IRMP	CN	Slope (‰)		ROUTE RESOURCES											
								SCIP (Permeable)	SCIP (Impermeable)	NWDoc	NWDoc	NWDoc (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NWDoc	NWDoc	LENGTH (m)	SLOPE (‰)	DISTANCE (m)	ELEVATION (m)	
1	FOSTER	325.44	FOSTER OUT - ("FO" + "FO-DWP")		0.55	0.55	78	1472.954011	0.5	0.5	FOSTER	F_FOS	FO-DWP	18.34	30	SN_FO (Total Flow at Foster Drain)	N_CE	359	0.0018	-445.23	91.5
																			392.2	91.5	
																			-91	91.5	
																			-85.52	91.5	
																			22.46	89.4	
																			-9.79	89.31	
																			-3.22	88.24	
																			3.22	86.07	
																			10.96	85.79	
																			16.44	86.49	
																			36.55	88.45	
																			29.63	92.27	
																			95.76	90.87	
																			36.67	91	
																			308.08	91	
																			189.82	90.5	
																			112.04	90.3	
																			114.62	91	
																			216.76	93.5	



File name:
Figure F2 - Foster Sub-catchment.pdf

Legend

- Channel Cross Sections
- SWMF Drains
- Foster Drain
- SWMF ponds
- Foster Pond
- Foster Sub-catchment
- FOSTER
- Google Hybrid

XS 6016 Cross Section at station 6016

FOSTER Area ID
325.44 ha Area (ha)
Ref. 1 Reference Number

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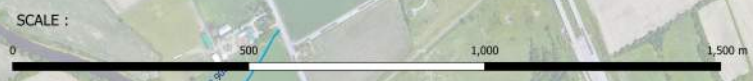
PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure F2 - Foster Sub-catchment
Table F2 - Foster Sub-catchment

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021



Ref.	Name ID	Area (ha)	Major System To	Minor System To	T. Imperv.	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)		ROUTE RESERVOIR					ROUTE CHANNEL (Station 6016)					
									SLPP (Pervious)	SLPI (Impervious)	NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)
1	FOSTER	325.44	FOSTER-OUT = ["P_FOS"+"FO-OVF"]		0.55	0.55	74	1472.956211	0.5	0.5	FOSTER	P_FOS	FO-OVF	10.34	10	SN_FO (Total Flows at Foster Drain)	N_CE	159	0.0818	-645.23	91.5
																				-391.2	91.5
																				-91	91.5
																				-85.52	91.5
																				-15.46	89.4
																				-9.79	89.31
																				-3.22	86.24
																				3.22	85.07
																				10.96	85.79
																				16.44	86.49
																				26.55	89.45
																				29.03	90.27
																				35.76	90.67
																				36.67	91
																				108.08	91
																				109.82	90.5
																				112.04	90.5
																				114.62	91
																				116.76	91.5



Ref.	Name ID (Clarke Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. (FIP (m))	XBRP	CR	LGR Length (m)	SLOPE (%)	ROUTE RESERVOIR			ROUTE CHANNEL (Station 4534)				
										WYDIn	WYDOut	WYDFlow (OVERFLOW)	WYDIn	WYDOut	WYDFlow (m)		
1	W_CLAR_MJ	1,772	Clarke	Todd	0.50	0.80	77	100	3	W_CLAR_MJ	W_CLAR_Min	W_CLAR_MJ	0.233	0.0001*			
2	W_CLAR_ALL	119,398	Clarke		0.05	0.4	77	852.18	3	W_CLAR = W_CLAR_ALL + W_CLAR_MJ	MS_P2	P2-OVF	0.138	0.161	-1082.01	94	
													0.138	0.409	-1028.17	92.5	
													0.227	0.931	-995.1	95.5	
													0.334	1.223	-279.98	90	
													0.305	1.32	-23.63	90	
													0.666	1.823	-13.45	87.13	
													0.833	2.223	-0.07	86.26	
													0.995	2.434	10.54	87.13	
													1.069	2.583	23.63	90	
													1.31	2.647	24.86	90.5	
4.884	2.863	28.72	91														
13.048	3.188	45.07	91.5														
23.745	3.523	128.17	91.5														
36.474	3.871	270.7	92.5														
45.910	4.227	228.5	95														
3	W_CLAR_UNDE	35.65	Jock River (Station 4534)		1.30		77										

* Small storage was assumed to allow overflow and direct the flow towards minor and major systems.
 ** N_WC is WYDOut from ROUTE CHANNEL at Channel X-Section obtained from RICA Hydraulic Model - Station 5002 (Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain)

Legend

- Channel Cross Sections
- SWMF Drains
 - Brazeau & Clarke Undeveloped
 - Clarke
- SWMF ponds
 - West Clarke
 - Brazeau
- Clarke Sub-catchment & Brazeau Sub-catchment
 - W_CLAR_MJ
 - W_CLAR_MJ (Major Only to Clarke SWM Pond)
 - W_CLAR_ALL
 - W_CLAR_BRAZ
 - W_CLAR_UNDE
 - Clarke-Brazeau-CAD
- Google Hybrid

File name: Figure F3 - Clarke & Brazeau Sub-catchments.pdf

XS 4534 Cross Section at station 4534 Minor System

W_CLAR_MJ (Major Only to Clarke SWM Pond) 1,772 ha & Ref. 1

- Area ID
- Reference Number
- Area (ha)

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PROJECT :
 BCDC - Quantity Control Study

TITLE : Figure F3 - Clarke & Brazeau Sub-catchments
 Table F3-1 - Clarke Sub-catchments
 Figure F3-2 - Brazeau Sub-catchment

PROJECT NO.	1474-16
DRAWN:	MM
DATE:	Mar. 2021

Ref.	Name ID (Brazeau Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. (FIP (m))	XBRP	CR	LGR Length (m)	SLOPE (%)	ROUTE RESERVOIR			ROUTE CHANNEL (Station 4534)			
										WYDIn	WYDOut	WYDFlow (OVERFLOW)	WYDIn	WYDOut	WYDFlow (m)	
4	W_CLAR_BRAZ	73.29			0.05	0.4	77	869	3	W_CLAR_BRAZ	MS_P2	P2-OVF	0.138	0.161	-1082.01	94
													0.138	0.409	-1028.17	92.5
													0.227	0.931	-995.1	95.5
													0.334	1.223	-279.98	90
													0.305	1.32	-23.63	90
													0.666	1.823	-13.45	87.13
													0.833	2.223	-0.07	86.26
													0.995	2.434	10.54	87.13
													1.069	2.583	23.63	90
													1.31	2.647	24.86	90.5
													4.884	2.863	28.72	91
													13.048	3.188	45.07	91.5
													23.745	3.523	128.17	91.5
													36.474	3.871	270.7	92.5
													45.910	4.227	228.5	95

* Show pond discharge directly to the pond over through a road side ditch on the west side of Bonaventure Road (Station 4534)

Ref.	Name ID (Clarke Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. [TP (hr)]	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)	ROUTE RESERVOIR					ROUTE CHANNEL (Station 4534)						
										NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)	
1	W_CLAR_MJ	1.772	Clarke	Todd	0.59	0.46	77	109	1	W_CLAR_MJ	W_CLAR_MJn	W_CLAR_MJj	0.213	0.0001*							
2	W_CLAR_ALL	119.398	Clarke		0.65	0.6	77	892.18	1	W_CLAR = W_CLAR_ALL + W_CLAR_MJj	MS_P2	P2-OVF	0.128	0.161	N_WC**	N_KB	1020	0.0498			
													0.138	0.409					-1082.01	94	
													0.148	0.68					-1028.17	92.5	
													0.227	0.931					-992.3	93.5	
													0.354	1.223					-279.34	90	
													0.505	1.52					-23.63	90	
													0.666	1.821					-13.45	87.13	
													0.831	2.123					-0.07	86.24	
													0.995	2.434					10.54	87.15	
													1.069	2.583					23.63	90	
													1.51	2.647					24.86	90.5	
													4.904	2.861					26.72	91	
													13.048	3.188					45.07	91.5	
													23.745	3.523					128.17	91.5	
36.474	3.871	270.7	92.5																		
45.938	4.127	728.3	95																		
61.652	4.539																				
3	W_CLAR_UNDE	35.65	Jock River (Station		[1.10]		77														
Total		156.82																			

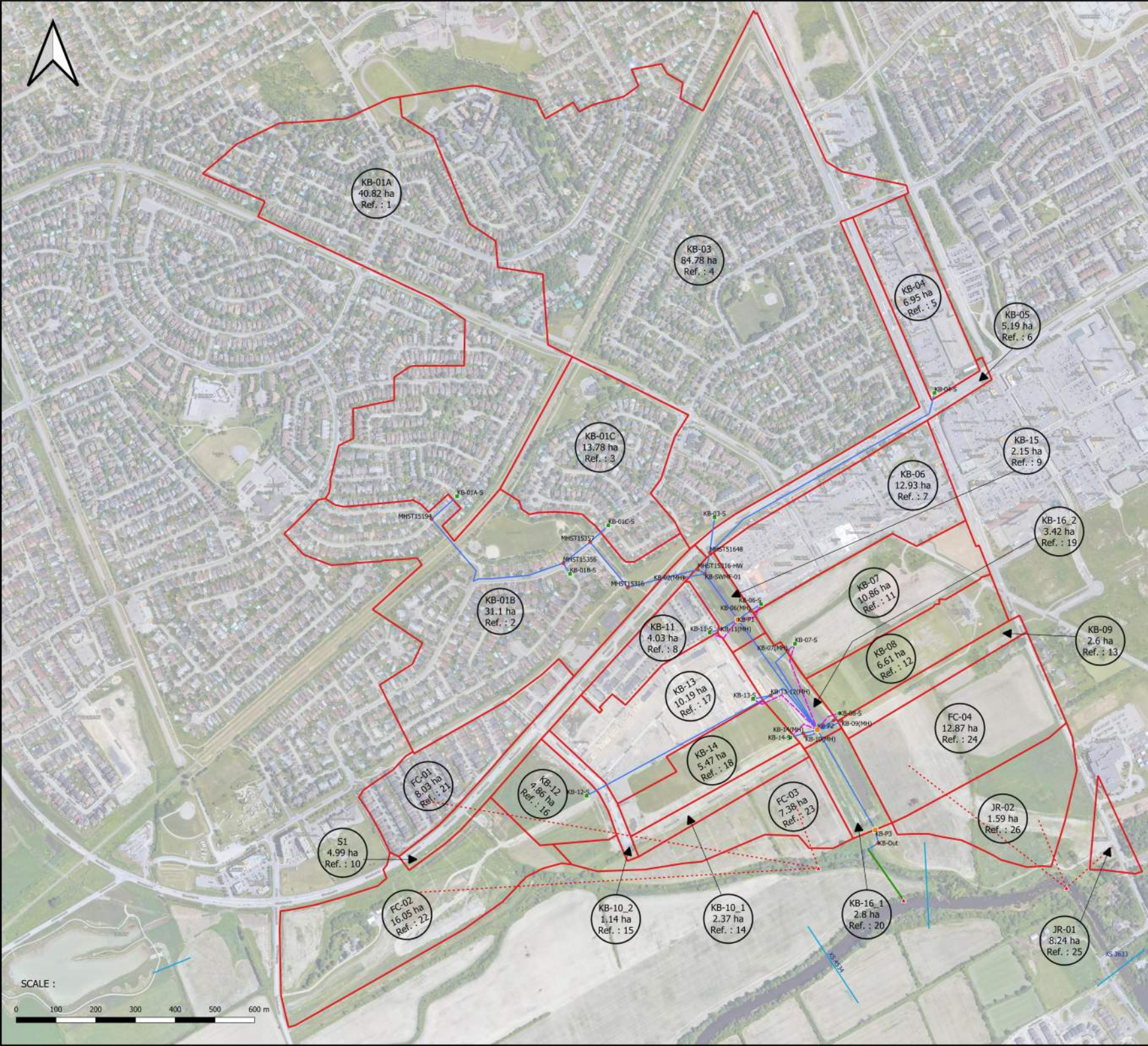
* Small storage was assumed to allow overflow and direct the flow towards minor and major systems

** N_WC is NHYDout from ROUTE CHANNEL at Channel X-Section obtained from RVCA Hydraulic Model - Station 5002 (Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain)

Ref.	ID (Brazeau Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. [TP (hr)]	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)		ROUTE RESERVOIR					ROUTE CHANNEL (Station 6016)					
									SLPP (Pervious)	SLPI (Impervious)	NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)
4	W_CLAR_BRAZ	73.29	Pond in Brazeau which drains to Jock River (station 6016)*		0.65	0.6	77	699	1	0.5	W_CLAR_BRAZ	MS_P10	P10-OVF	0.068	0.001	SN_FO (Total Flows at Foster Drain)	N_CE**	159	0.0818	-645.23	91.5
														0.271	0.022					-391.2	91.5
														0.379	0.051					-91	91.5
														0.48	0.091					-85.52	91.5
														0.853	0.341					-15.46	89.4
														1.005	0.61					-9.79	89.31
														1.128	1.231					-3.22	86.24
														1.155	1.592					3.22	85.07
														1.194	1.876					10.96	85.79
														1.2	1.921					16.44	86.49
														1.259	2.369					26.55	89.45
														1.3	2.665					29.03	90.27
														1.349	2.813					35.76	90.67
																				36.67	91
																				108.08	91
		109.82	90.5																		
		112.04	90.5																		
		114.62	91																		
		116.76	91.5																		
Total		73.29																			

* Brazeau pond discharges directly to the jock river through a road side ditch on the west side of Borrissokane road (station 6016)

** N_CE is NHYDout from ROUTE CHANNEL at Channel X-Section obtained from RVCA Hydraulic Model - Station 6016 (Hydrograph from Node Foster routed to Node at Cedarview Road)



Sub-catchment	Area (ha)	Reference
KB-01A	40.82	1
KB-03	84.78	4
KB-04	6.95	5
KB-05	5.19	6
KB-06	12.93	7
KB-07	10.86	11
KB-08	6.61	12
KB-09	2.6	13
KB-10_1	2.37	14
KB-10_2	1.14	15
KB-11	4.03	8
KB-12	4.86	16
KB-13	10.19	17
KB-14	5.47	18
KB-15	2.15	9
KB-16_1	2.8	20
KB-16_2	3.42	19
KB-17	10.19	17
KB-18	5.47	18
KB-19	3.42	19
KB-20	2.8	20
KB-21	8.03	21
KB-22	16.05	22
FC-01	8.03	21
FC-02	16.05	22
FC-03	7.38	23
FC-04	12.87	24
JR-01	8.24	25
JR-02	1.59	26

- Legend**
- Ken-BU storage
 - Ken-BU MH
 - ▲ Ken-BU Pond
 - ▲ Ken-BU Outfalls
 - Ken-BU Pipe
 - Ken-BU Major System
 - Ken-BU Channel
 - Ken-BU to FC (Fraser Clarke) and JR(Jock River)
 - Kennedy Burnett Sub-catchment
 - Kennedy Burnett (Ken-BU)
 - Google Hybrid

File name: Figure F4 - Kennedy Burnett Sub-catchments.pdf



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PROJECT : BCDC - Quantity Control Study

TITLE : Figure F4 - Kennedy Burnett Sub-catchments
 Table F4 - Kennedy Burnett Sub-catchments

PROJECT NO.	1474
DRAWN:	MM
DATE:	Mar. 2021



Ref.	ID KENNEDY-BURNETT	Area (ha)	Runoff Coef. (1)	Zero Imperv. (%)	Slope (%)	Equivalent Width (m)	Flow Length (m)	Percent Routed (%)	TIMP (2)	XIMP (2)	LGI: Length (m) =SQRT (Area*10000/1.5)	SLPP (%)	SLP1 (2)	Minor System Criteria (1)	Major System Criteria (1)	Major and Minor System To	DUALHYD parameters				ROUTE RESERVOIR				ROUTE CHANNEL (Station 3633)																													
																	Major System	Minor System	Max. Release Rate (2) (cms)	Storage (2)	NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)																							
Existing Subcatchments to KB-SWMF																																																						
1	KB-01A	40.82	0.49	50%	0.97%	1879	220	76%	0.408	0.09792	521.664	0.5	0.5	10 L/s/inlet & 15 L/s/inlet ICDs	No MS flows to KB-SWMF	KB-01A-S	KB-01A-MJ	KB-01A-MN	3.6	4995				0.076	0.003																													
2	KB-01B	31.1	0.46	50%	0.42%	312	1000	50%	0.375	0.1875	455.339	0.42	0.42	10 L/s/inlet & 15 L/s/inlet ICDs	No MS flows to KB-SWMF	KB-01B-S	KB-01B-MJ	KB-01B-MN	1.585	6075				0.088	0.006																													
3	KB-01C	13.78	0.49	50%	0.50%	403	335	50%	0.409	0.2045	303.095	2.0	0.5	10 L/s/inlet & 15 L/s/inlet ICDs	No MS flows to KB-SWMF	KB-01C-S	KB-01C-MJ	KB-01C-MN	1.35	1880				0.136	0.011																													
4	KB-03	84.78	0.48	50%	0.63%	647	1300	50%	0.394	0.197	751.798	2.0	0.63	10 L/s/inlet & 15 L/s/inlet ICDs	No MS flows to KB-SWMF	KB-03-S	KB-03-MJ	KB-03-MN	5.27	15500				0.301	0.017																													
5	KB-04	6.95	0.80	40%	0.50%	55	1265	100%	0.85	0.85	215.252	2.0	0.5	Controlled to 503 L/s (approx. 5-year inlet capture rate)	MS flows stored on-site	KB-04-S	KB-04-MJ	KB-04-MN	0.503	1972				0.454	0.027																													
6	KB-05	5.19	0.65	0%	2.00%	51	1000	100%	0.93	0.93	186.011	2.0	0.50	10-year inlet capture rate for storm sewers	MS flows to KB-SWMF	KB-P1								0.631	0.041																													
7	KB-06	12.93	0.81	40%	4.75%	217	600	100%	0.873	0.873	293.598	2.0	4.75	Controlled to Qcap (1350mm) 1,760 L/s (approx. 5-year inlet capture rate)	MS flows stored on-site	KB-06-S	KB-06-MJ	KB-06-MN	2.262	1950				1.173	0.068																													
8	KB-11	4.03	0.67	50%	2.00%	116	350	100%	0.675	0.675	163.911	2.0	2.0	Controlled to 577 L/s (approx. 5-year inlet capture rate)	MS flows to KB-SWMF	KB-11-S	KB-11-MJ	KB-11-MN	0.577	597				1.91	0.111																													
9	KB-15	2.15	0.90	90%	0.30%	476	45	100%	0.79	0.79	119.722	2.0	0.3	Uncontrolled		KB-P1								9.813	0.436																													
10	S1	4.99	0.65	0	0.02	50	1000	1	0.93	0.93	182.3915203	2	2	10-year inlet capture rate for storm sewers	MS flows to KB-SWMF	KB-P1							12.134	0.617																														
																								12.438	0.732																													
																								12.424	0.811																													
																								12.425	0.894																													
																									0.051	0.002									-29.24	91																		
																								0.048	0.003											-27.41	90.5																	
																								0.057	0.029												-25.64	90																
																								0.089	0.045													-23.74	89.5															
																								0.133	0.069														-22	89.26														
																								0.199	0.106														-20	88.51														
																								0.321	0.172														-19	88.32														
20	KB-16_1	2.800	0.900	0.900	0.003	550.000	50.000	1.000	0.750	0.750	136.626	2.000	0.300	Uncontrolled		KB-P3								1.029	0.306												-15	88.1																
																								4.036	0.527														-10	88.11														
																								8.332	0.761														-5	88.17														
																								11.727	0.941														0	88.27														
																								14.125	1.067														5	88.19														
																								15.675	1.149														10	88.06														
																								16.555	1.196														15	88.48														
																								16.911	1.214														16	88.7														
																									0.053	0.005														23.74	89.5													
																								0.132	0.009															24.68	90													
																								0.269	0.014																25.57	90.5												
																								0.455	0.023																26.5	91												
																								0.699	0.037																													
																								0.947	0.056																													
																								1.853	0.09																													
Proposed Subcatchments to KB-SWMF																																																						
11	KB-07	10.86	0.8	0.5	0.02	209	525	1	0.86	0.86	269.072	2.0	2.0	5-year inlet capture rate (3-hour Chicago)	MS flows to KB-SWMF	KB-07-S	KB-07-MJ	KB-07-MN	2.094	1378					2.712	0.146																												
12	KB-08	6.61	0.65	0.5	0.02	133	500	1	0.64	0.64	209.921	2.0	2.0	5-year inlet capture rate (3-hour Chicago)	MS flows to KB-SWMF	KB-08-S	KB-08-MJ	KB-08-MN	1.058	787					6.626	0.287																												
13	KB-09	2.6	0.8	0	0.02	70	500	1	0.86	0.86	131.656	2.0	2.0	10-year inlet capture rate for storm sewers	MS flows to KB-SWMF	KB-P2								11.228	0.515																													
14	KB-10_1	2.37	0.8	0	0.02	64	500	1	0.86	0.86	125.698	2.0	2.0	10-year inlet capture rate for storm sewers	MS flows to KB-SWMF	KB-P3									14.885	0.738																												
15	KB-10_2	1.14	0.8	0	0.02	57	200	1	0.86	0.86	87.178	2.0	2.0	10-year inlet capture rate for storm sewers	MS flows to KB-SWMF	KB-P4									16.473	0.893																												
16	KB-12	4.86	0.75	0.3	0.02	227	215	1	0.79	0.79	180.000	2.0	2.0	5-year inlet capture rate (3-hour Chicago)	MS flows to FC-Drain	KB-12-S	KB-12-MJ	KB-12-MN	0.8665	632					17.311	0.998																												
17	KB-13	10.19	0.65	50%	2.00%	227	450	100%	0.64	0.64	260.640	2.0	2.0	5-year inlet capture rate (3-hour Chicago)	MS flows to KB-SWMF	KB-13-S	KB-13-MJ	KB-13-MN	1.722	1077					17.633	1.063																												
18	KB-14	5.47	0.65	50%	2.00%	121	450	100%	0.64	0.64	190.962	2.0	2.0	5-year inlet capture rate (3-hour Chicago)	MS flows to KB-SWMF	KB-14-S	KB-14-MJ	KB-14-MN	0.8734	631					17.634	1.112																												
Existing and Proposed Subcatchments Total Area to KB-SWMF (ha)										257.04																																												
EXISTING / PROPOSED SUBCATCHMENTS TO FRASER-CLARKE DRAIN																																																						
21	FC-01	8.03	0.53						0.47	0.47	231.373	2.0	1.0	Controlled to 756 L/s	No MS flows to KB-SWMF	FC-01-S	FC-01-MJ	FC-01-MN	0.756	714																																		
22	FC-02	16.05	0.85						0.93	0.93	327.109	2.0	1.0	5-year inlet capture rate (3-hour Chicago)	MS flows to FC-Drain	FC-02-S	FC-02-MJ	FC-02-MN	1.159	2385																																		
23	FC-03	7.37	0.65						0.64	0.64	221.660	2.0	1.0	5-year inlet capture rate (3-hour Chicago)	MS flows to FC-Drain	FC-03-S	FC-03-MJ	FC-03-MN	0.358	1131																																		
24	FC-04	12.87	0.65						0.64	0.64	292.916	2.0	1.0	5-year inlet capture rate (3-hour Chicago)	MS flows to FC-Drain	FC-04-S	FC-04-MJ	FC-04-MN	0.741	1794																																		
Existing / Proposed Subcatchments Total Area to Fraser-Clarke Drain (ha)										44.32																																												
PROPOSED SUBCATCHMENTS TO JOCK RIVER																																																						
25	JR-01	8.24	0.65						0.64	0.64	234.379	2.0	1.0	5-year inlet																																								

Ref.	Name ID (Todd)	Area (ha)	Major System To	Minor System To	T. Imperv.	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)	ROUTE RESERVOIR					ROUTE CHANNEL (Station 2462)					
										NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)
1	TODD_MN2	2.1	Jock River (Station 2462)	Todd	0.57	0.53	77	118.322	1	TODD_MN2	TODD_MN2n	TODD_MN2j	0.268	0.0001*						
2	TODD_MN3	0.117	Corrigan	Todd	0.57	0.53	77	27.928	1	TODD_MN3	TODD_MN3n	TODD_MN3j	0.016	0.0001*						
3	TODD_MJ	30.23	Todd	Corrigan	0.64	0.52	77	448.925	1	TODD_MJ	TODD_MJn	TODD_MJj	3.314	0.0001*						
4	TODD_ALL	112.908	Todd	Todd	0.57	0.52	77	867.594	1											
5	TODD_P	3.055	Todd	Todd	0.63	0.63	77	142.712	1	"TODD" = ["TODD_MN2n"+ "TODD_MN3n"+ "TODD_MJj"+ "TODD_P"+ "TODD_ALL"+ "W_CLAR_MJn"***]	MS_P3	P3-OVF	0.014 0.048 0.061 0.08 0.088 0.109 0.118 0.122 1.972 9.135 15.608 19.256 27.282 40.957 56.372 73.349 85.469 104.771	0.155 0.394 0.56 0.909 1.089 1.652 1.952 2.099 2.269 2.598 2.826 2.942 3.181 3.55 3.929 4.317 4.579 4.977	SN_TO = ["GreenB" (Greenbank Pond)+ "MS_P3"+"P3-OVF"+ "TODD_MN2j"+ "A2-MJ"***]	N_TO	280	0.033	-83.32 -81.36 -79.12 -76.13 -20.46 -19.36 -18.51 -17.72 -11.95 -0.11 11.49 17.72 19.74 21.22 22.68 24.28 26.79 71.98 80.62	90 89.5 89 88.5 88 87.5 87 86.5 85.24 85.12 85.2 86.5 87 87.5 88 88.5 89 90 90.5
Total		148.41																		

* Small storage was assumed to allow overflow and direct the flow towards minor and major systems

** "W_CLAR_MJn" is the minor system from the major system area(Area = 1.772 ha & TIMP = 0.59 & XIMP = 0.46 & CN = 75 & Slope = 1% & Outflow = 0.213 cms)in Clarke sub-catchment (Schematic 3) to Jock River (Station 2462)

*** "A2-MJ" is the major system from A2 area (Area = 25.5 ha & TIMP = 0.52 & XIMP = 0.42 & CN = 75 & Slope = 1% & Storage = 924 cu-m & Flow rate = 1.818 cms) in Corrigan sub-catchment (Schematic 1) to Todd sub-catchment



Area ID	Area (ha)	Reference Number	Manhole	Notes
A1	15.75	1	MH101	
A2	25.5	6	MH102	
A3	1.27	5	MH103	
A4	1.6	7	MH104	
A5	1.56	8	MH105	
A6	0.96	13	MH106	
A7	2.2	12	MH107	
A8	4.14	18	MH108	
A9	6.27	19	MH109	
A10	7.6	11	MH116	
A11	12.29	20	MH119	
A12	15.87	2	MH333	
A13	15.75	1	MH335	
B1	2.77	4	MH340	
B2	5.59	10	MH358	
B3	7.6	11	MH360	
B4	7.19	14		
B5	3.29	15		
B6	3.29	15		
B7	7.19	14		
B8	3.29	15		
B9	3.29	15		
B10	3.29	15		
B11	3.29	15		
B12	3.29	15		
B13	3.29	15		
B14	3.29	15		
B15	3.29	15		
B16	3.29	15		
B17	3.29	15		
B18	3.29	15		
B19	3.29	15		
B20	3.29	15		
B21	3.29	15		
B22	3.29	15		
B23	3.29	15		
B24	3.29	15		
B25	3.29	15		
B26	3.29	15		
B27	3.29	15		
B28	3.29	15		
B29	3.29	15		
B30	3.29	15		
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B38	3.29	15		
B39	3.29	15		
B40	3.29	15		
B41	3.29	15		
B42	3.29	15		
B43	3.29	15		
B44	3.29	15		
B45	3.29	15		
B46	3.29	15		
B47	3.29	15		
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B63	3.29	15		
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B99	3.29	15		
B100	3.29	15		

File name:
Figure F6 - Corrigan Sub-catchments.pdf

Major System
 Overflow

Legend
 Channel Cross Section
 Corrigan-Pipe Line
 Corrigan-MH
 Corrigan Drainage Boundaries
 XS 0 Cross Section at station 0

Area ID
 Area (ha)
 Reference Number

J.F. Sabourin and Associates Inc.
 WATER RESOURCES AND ENVIRONMENTAL CONSULTANTS
 52 Springbrook Drive
 Ottawa, ON, K2S 1B9
 (613) 836-3884
 www.jfsa.com

DSEL
 david schaeffer engineering ltd

PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure F6 - Corrigan Sub-catchments
Table F6 - Corrigan Sub-catchments

PROJECT NO. : 1474

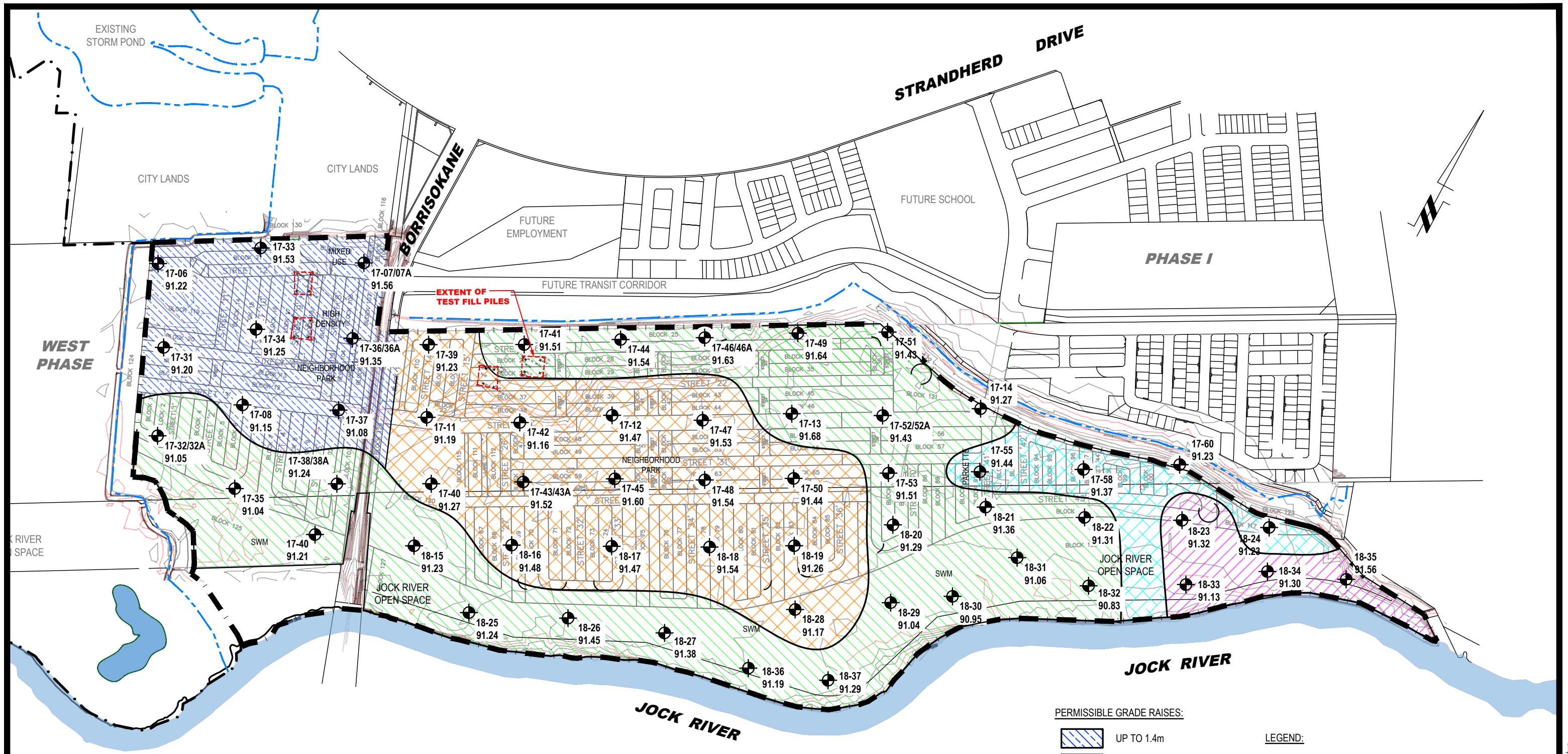
DRAWN: MM

DATE: Mar. 2021



APPENDIX E


GEO TECHNICAL



PERMISSIBLE GRADE RAISES:

-  UP TO 1.4m
-  UP TO 1.6m
-  UP TO 1.8m
-  UP TO 2.0m
-  UP TO 2.2m

LEGEND:

-  APPROXIMATE BOREHOLE LOCATION (GOLDER ASSOCIATES, 04/2019)
- 91.56 GROUND SURFACE ELEVATION (m)
- BOREHOLE LOCATIONS WERE SURVEYED BY OTHERS AND ARE REFERENCED TO A GEODETIC DATUM.

patersongroup
consulting engineers

154 Colonnade Road South
Ottawa, Ontario K2E 7J5
Tel: (613) 226-7381 Fax: (613) 226-6344

NO.	REVISIONS	DATE	INITIAL
2	REVISED CONCEPTUAL PLAN, UPDATED GRADE RAISE RESTRICTIONS	04/02/2021	OC
1	REVISED CONCEPTUAL PLAN, ADDED TEST FILL PILE INFO AND UPDATED GRADE RAISE RESTRICTIONS	14/05/2020	DJG

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROP. RESIDENTIAL DEVELOPMENT - CONSERVANCY LANDS EAST
OTTAWA, ONTARIO

Title: **PERMISSIBLE GRADE RAISE PLAN**

Scale:	1:6000	Date:	09/2019
Drawn by:	MPG	Report No.:	PG5036-1
Checked by:	OC	PG5036-2	Revision No.: 2
Approved by:	DJG		

re: **Road Grade Exceedance Review**
Proposed Residential Development - Conservancy Lands
Borrisokane Road - Ottawa

to: Caivan Communities - **Mr. Hugo Lalonde** - hugo.lalonde@caivan.com
David Schaeffer Engineering Ltd. - **Mr. Kevin Murphy** - KMurphy@dsel.ca

date: March 8, 2021

file: PG5036-MEMO.10

Paterson Group (Paterson) prepared the following memo to provide a geotechnical review of permissible grade raise exceedances of the proposed roadway grading at various locations throughout the aforementioned development. This memorandum should be read in conjunction with Paterson Group Report PG5036-1 Revision 1 dated February 3, 2021.

The following drawings prepared by David Schaeffer Engineering Ltd. were reviewed from a geotechnical perspective:

- Barrhaven Conservancy - Maximum Grade Raise Exceedance - Project No. 16-891 - Drawing No. 6 and 7 - dated March 2021
- Latest road grading AutoCAD file - 891_Grad_PS_Lowered_Mar5-21

Based on the available drawings, it is understood that minor permissible grade raise exceedances occur within the proposed roadway grades at various locations throughout the subject development.

Geotechnical Review

Based on our detailed review, the grading exceedances are considered acceptable from a geotechnical perspective and lightweight fill will not be required within the City of Ottawa right-of-way(s).

We trust that this information satisfies your immediate requirements.

Paterson Group Inc.



Owen Canton, E.I.T.



David J. Gilbert, P.Eng.

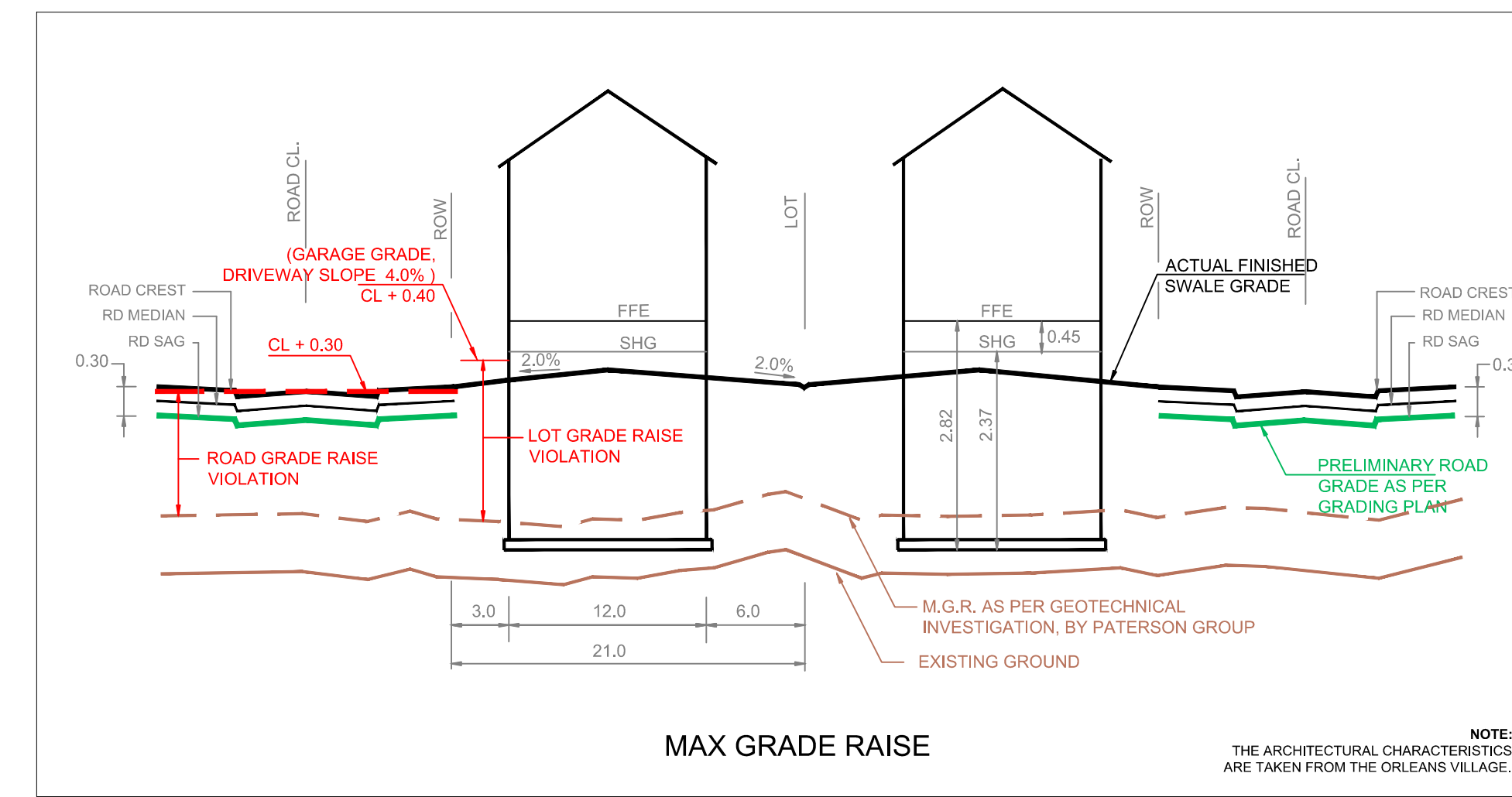
Paterson Group Inc.

Head Office and Laboratory
154 Colonnade Road South
Ottawa - Ontario - K2E 7J5
Tel: (613) 226-7381 Fax: (613) 226-6344

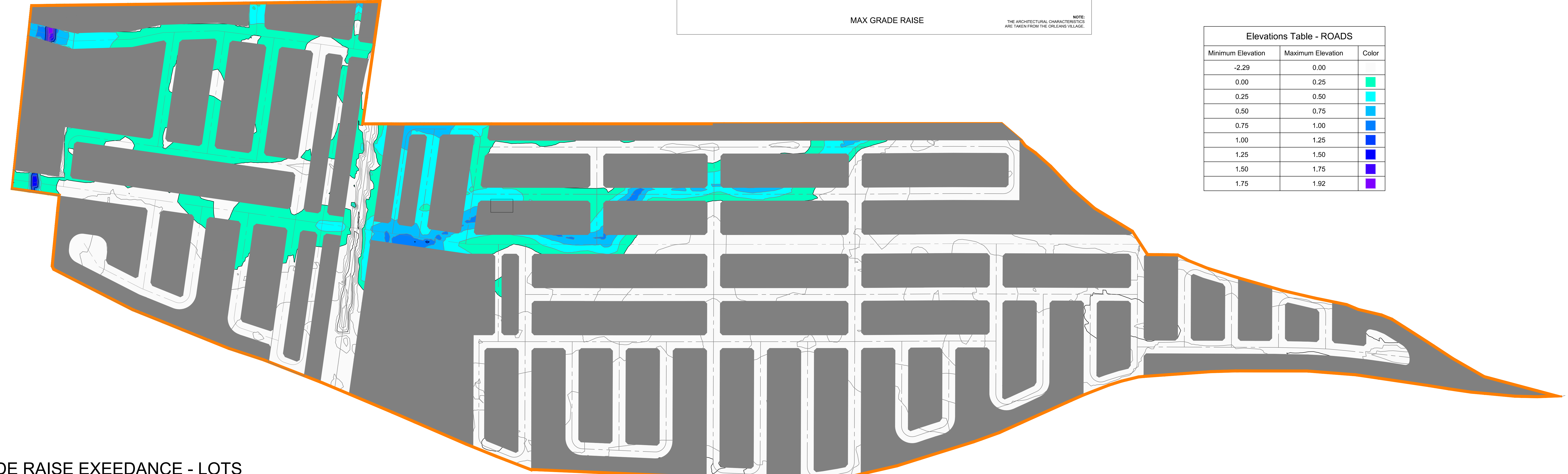
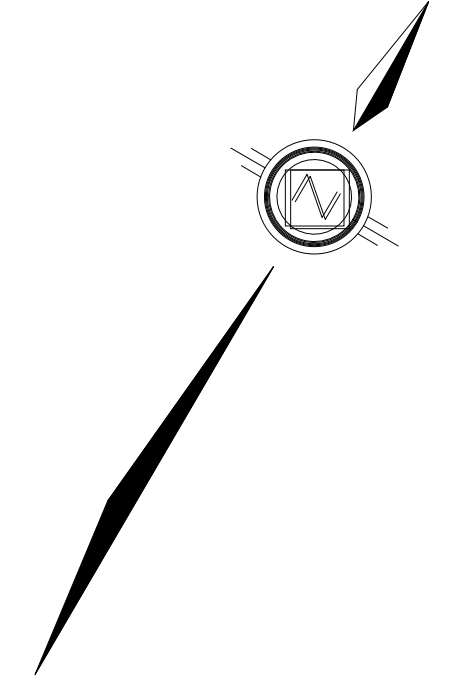
Northern Office and Laboratory
63 Gibson Street
North Bay - Ontario - P1B 8Z4
Tel: (705) 472-5331 Fax: (705) 472-2334

St. Lawrence Office
993 Princess Street
Kingston - Ontario - K7L 1H3
Tel: (613) 542-7381

MAXIMUM GRADE RAISE EXCEEDANCE - ROADS

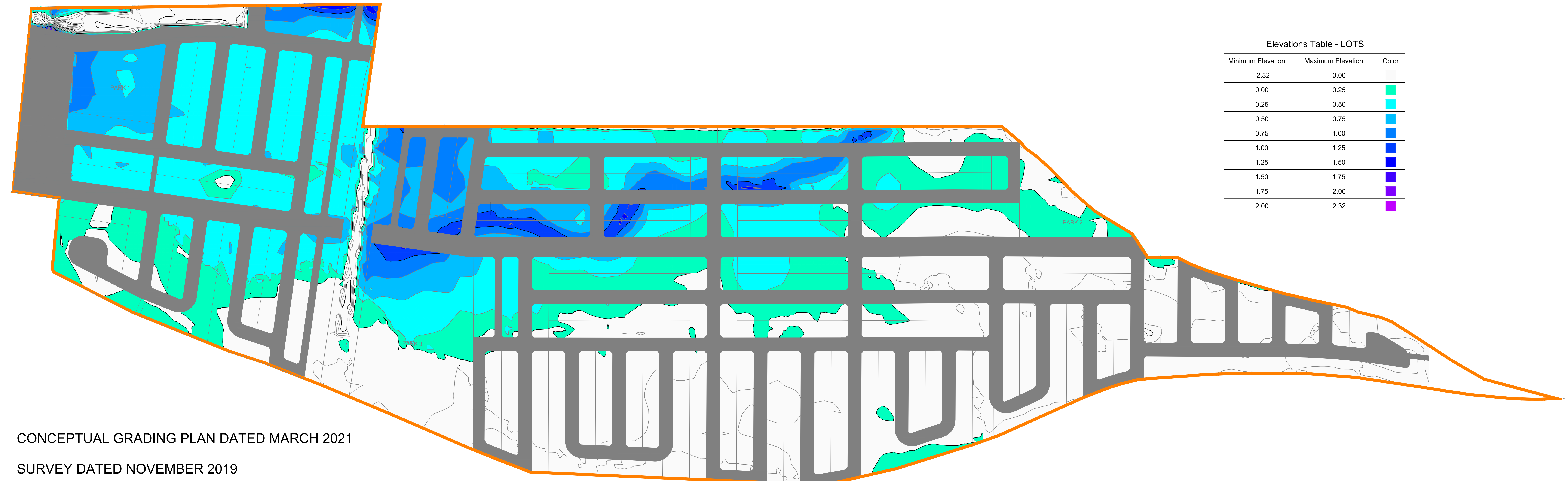


NOTES:
 1. LOW FLOW CHANNEL ELEVATION TO BE VERIFIED IN THE FIELD AT ALL ROAD AND SEWER CROSSING LOCATIONS.
 2. DETAILED DESIGN ROAD GRADES REPRESENT LOW POINTS. ACTUAL DESIGN GRADES WILL RISE AND FALL BY APPROXIMATELY 0.3m ABOVE THE TROUGH/SAG GRADES PRESENTED ON THIS PLAN.
 3. EARTHWORKS VOLUMES ARE BASED ON THE MEDIAN ROAD GRADES, WHICH ARE ESTIMATED TO BE 0.09m HIGHER THAN THE TROUGH/SAG GRADES PRESENTED ON THIS PLAN.
 EARTHWORKS DEPTHS AND VOLUMES ARE APPROXIMATE BASED ON USING ASSUMED MEDIAN VALUES FOR THE MANY VARIABLES THAT AFFECT EARTHWORKS. WHILE THE MEDIAN DEPTH AND VOLUME ARE GENERALLY REFLECTIVE OF THE SITE CHARACTERISTICS, THE ACTUAL DEPTH AND VOLUME AT ANY PRECISE LOCATION WILL VARY (UP OR DOWN) BASED ON THE LOCATION OF HIGH POINTS AND LOW POINTS IN THE ROAD AND ON THE LOTS.
 THE SITE GRADING PLAN HAS BEEN PREPARED IN GENERAL CONFORMANCE WITH CITY OF OTTAWA DESIGN CRITERIA AND HISTORIC PRACTICES. THE GRADING PLAN HAS NOT BEEN REVIEWED BY CITY OF OTTAWA AND IS SUBJECT TO CHANGE FOLLOWING CITY REVIEW. ANY CHANGE IN THE GRADING PLAN WILL HAVE A CORRESPONDING CHANGE IN EARTHWORKS.



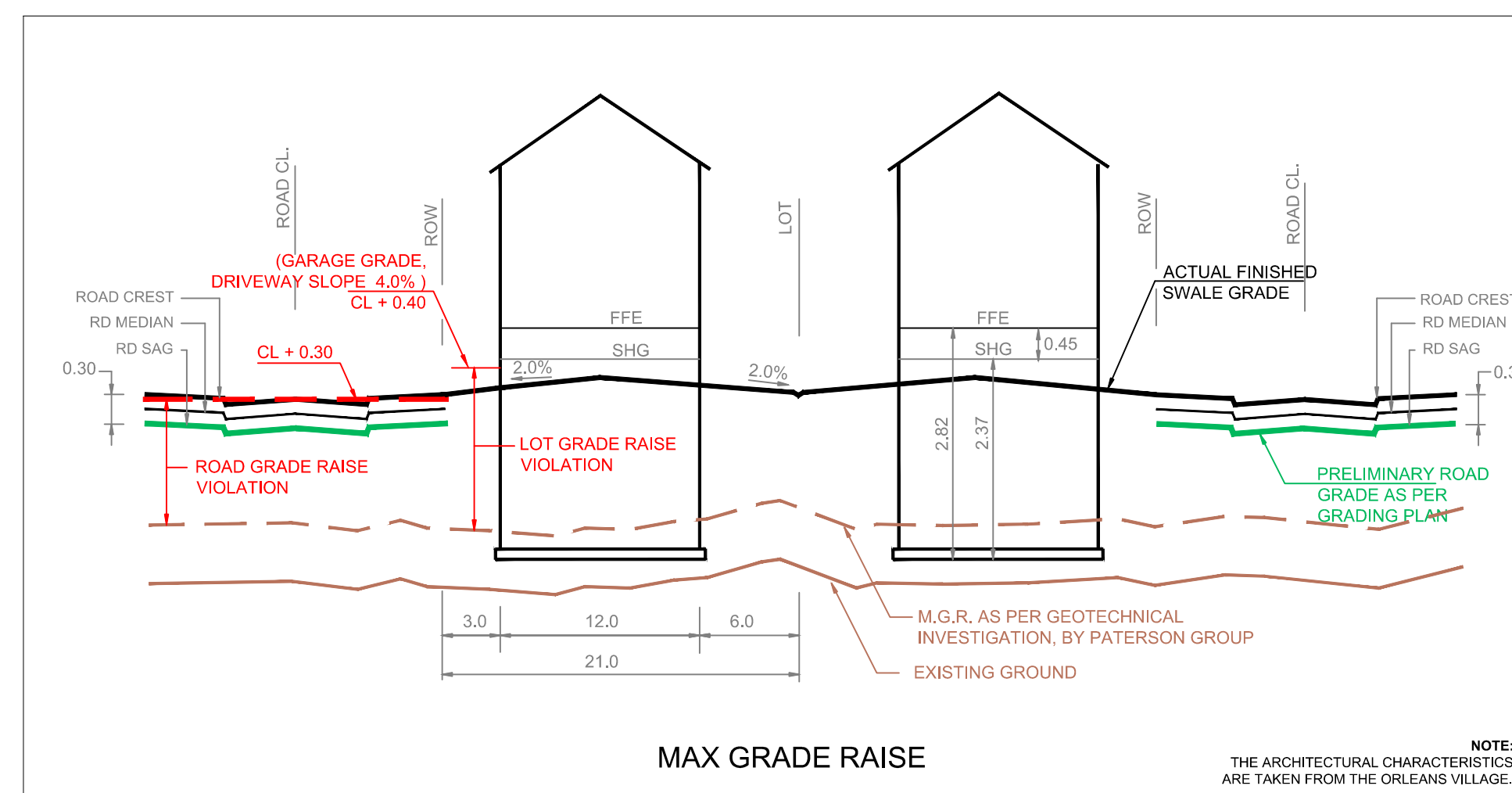
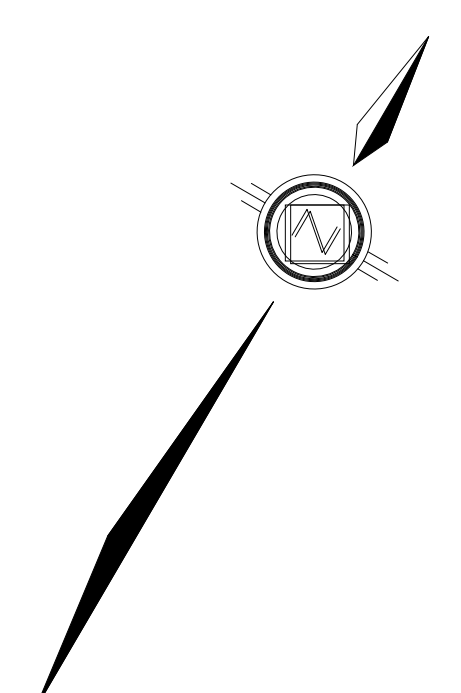
Minimum Elevation	Maximum Elevation	Color
-2.29	0.00	
0.00	0.25	Light Green
0.25	0.50	Light Blue
0.50	0.75	Medium Blue
0.75	1.00	Dark Blue
1.00	1.25	Blue
1.25	1.50	Dark Blue
1.50	1.75	Purple
1.75	1.92	Dark Purple

MAXIMUM GRADE RAISE EXCEEDANCE - LOTS

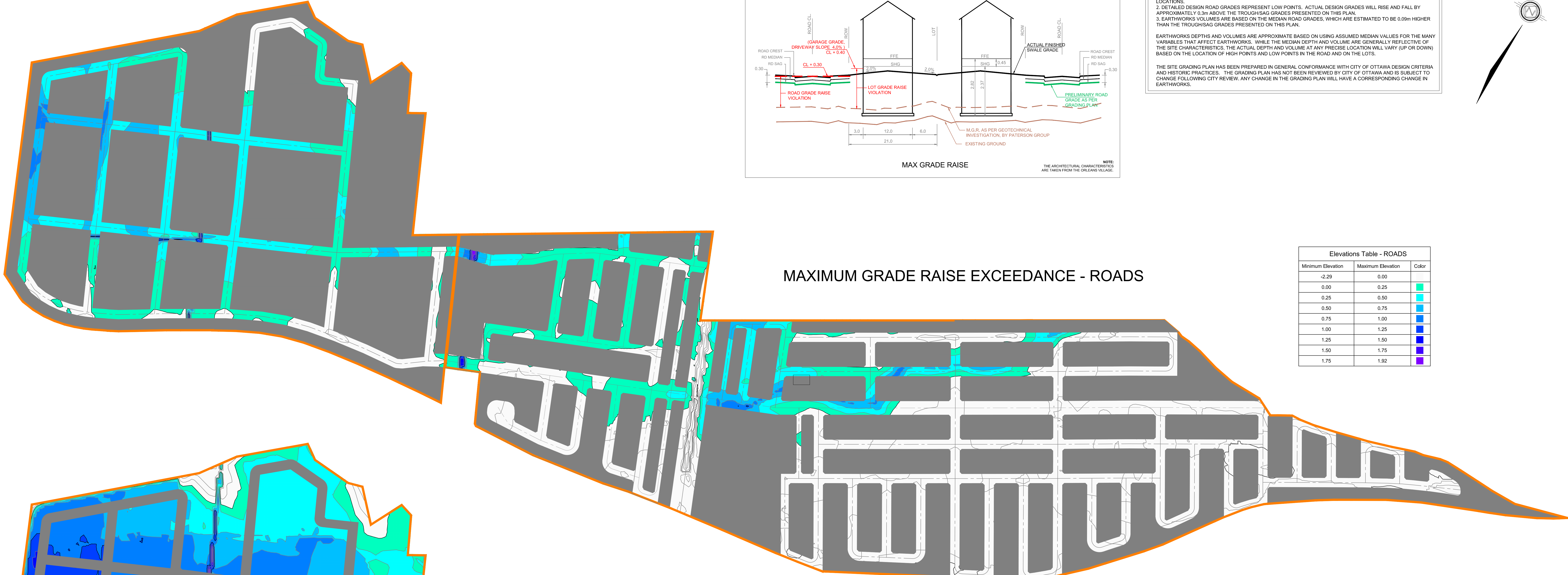


Minimum Elevation	Maximum Elevation	Color
-2.32	0.00	
0.00	0.25	Light Green
0.25	0.50	Light Blue
0.50	0.75	Medium Blue
0.75	1.00	Dark Blue
1.00	1.25	Blue
1.25	1.50	Dark Blue
1.50	1.75	Purple
1.75	2.00	Dark Purple
2.00	2.32	Very Dark Purple

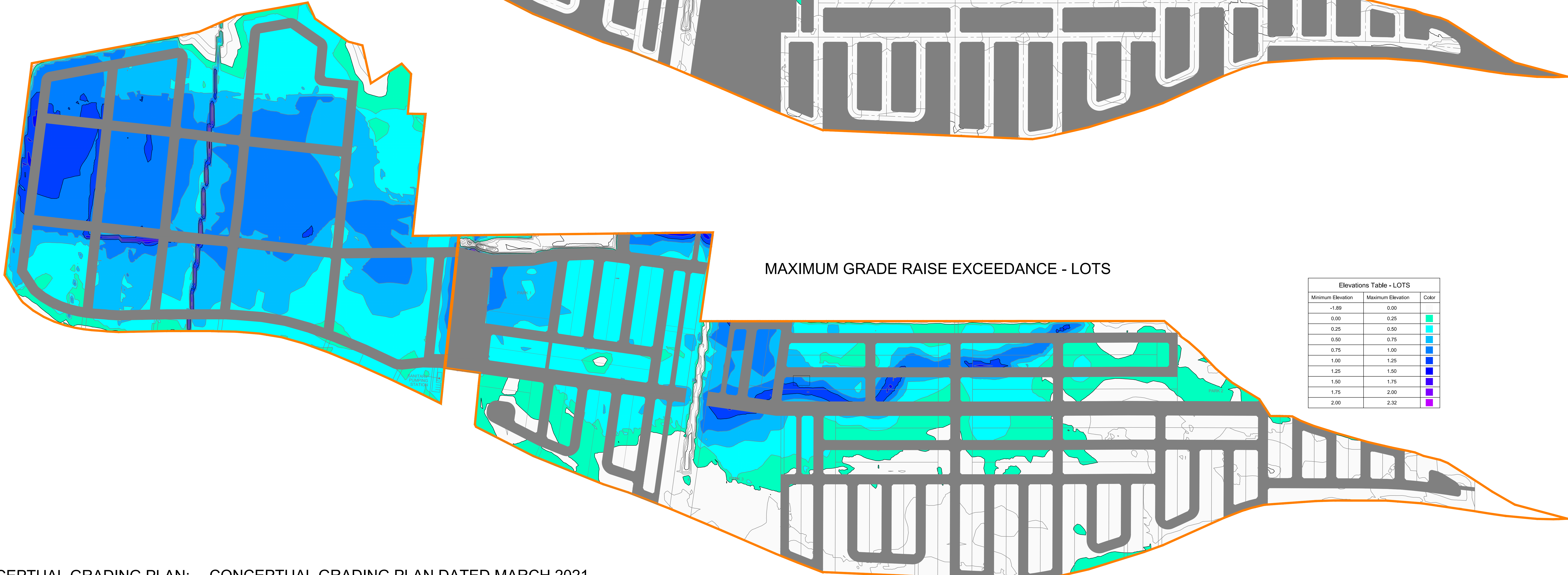
CONCEPTUAL GRADING PLAN: CONCEPTUAL GRADING PLAN DATED MARCH 2021
 ORIGINAL GROUND: SURVEY DATED NOVEMBER 2019
 MAXIMUM GRADE RAISE AS PER : GEOTECHNICAL INVESTIGATION, BY PATERSON GROUP DATED SEPTEMBER 27, 2019 - REVISED MAY 15, 2020



NOTES:
 1. LOW FLOW CHANNEL ELEVATION TO BE VERIFIED IN THE FIELD AT ALL ROAD AND SEWER CROSSING LOCATIONS.
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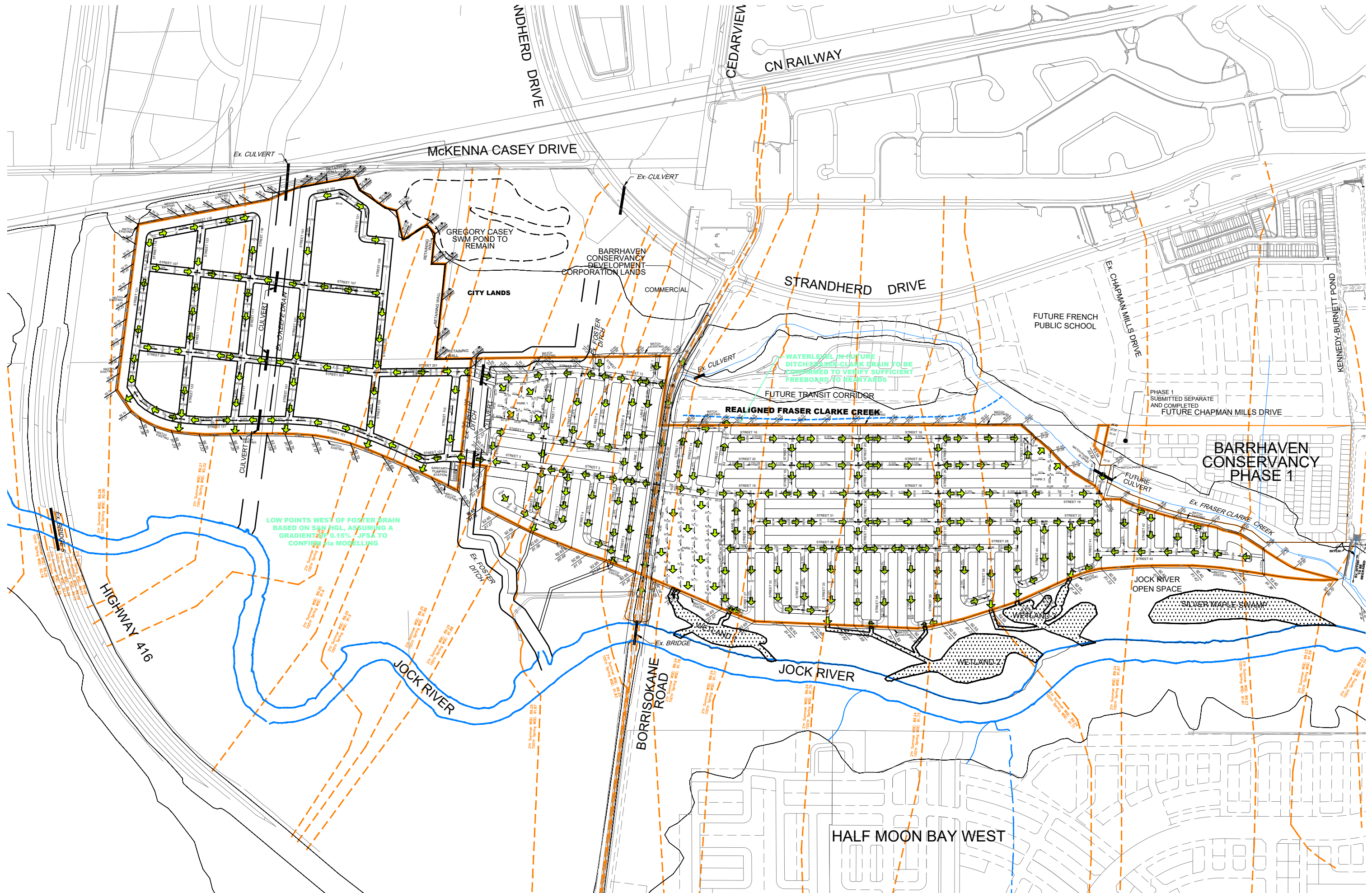


Minimum Elevation	Maximum Elevation	Color
-2.29	0.00	Light Blue
0.00	0.25	Light Cyan
0.25	0.50	Light Green
0.50	0.75	Light Blue
0.75	1.00	Light Cyan
1.00	1.25	Light Green
1.25	1.50	Light Blue
1.50	1.75	Light Cyan
1.75	1.92	Light Green



Minimum Elevation	Maximum Elevation	Color
-1.89	0.00	Light Blue
0.00	0.25	Light Cyan
0.25	0.50	Light Green
0.50	0.75	Light Blue
0.75	1.00	Light Cyan
1.00	1.25	Light Green
1.25	1.50	Light Blue
1.50	1.75	Light Cyan
1.75	2.00	Light Green
2.00	2.32	Light Blue

CONCEPTUAL GRADING PLAN: CONCEPTUAL GRADING PLAN DATED MARCH 2021
 ORIGINAL GROUND: SURVEY DATED NOVEMBER 2019
 MAXIMUM GRADE RAISE AS PER : GEOTECHNICAL INVESTIGATION, BY PATERSON GROUP
 DATED SEPTEMBER 27, 2019 - REVISED MAY 15, 2020



McKENNA CASEY DRIVE

STRANDHERD DRIVE

CEDARVIEW

CN RAILWAY

GREGORY CASEY SWM POND TO REMAIN

BARRHAVEN CONSERVANCY DEVELOPMENT CORPORATION LANDS

CITY LANDS

COMMERCIAL

FUTURE FRENCH PUBLIC SCHOOL

REALIGNED FRASER CLARKE CREEK

FUTURE TRANSIT CORRIDOR

PHASE 1 SUBMITTED SEPARATE AND COMPLETED FUTURE CHAPMAN MILLS DRIVE

BARRHAVEN CONSERVANCY PHASE 1

LOW POINTS WEST OF FOSTER DRAIN BASED ON SAN HGL, ASSUMING A GRADIENT OF 0.15% - JFSA TO CONFIRM via MODELLING

JOCK RIVER

BORRISOKANE ROAD

HIGHWAY 416

JOCK RIVER OPEN SPACE

SILVER MAPLE SWAMP

HALF MOON BAY WEST