# THREE-STOREY APARTMENT BUILDING ADDITION LOT RE-DEVELOPEMENT SITE

PART OF LOT "D"

R-PLAN 82717

314 BELL STREET SOUTH

**CITY OF OTTAWA** 

SERVICEABILITY REPORT
REPORT R-821-102A

T.L. MAK ENGINEERING CONSULTANTS LTD.

FEBRUARY 2022

REFERENCE FILE NUMBER 821-102

#### Introduction

The developer of this property is proposed to redevelop the existing residential lot described as Part of Lot D R-Plan 82717 by constructing a new three-storey residential apartment building addition consisting of eight (2)-bedroom apartments to the existing 3-storey apartment building which currently occupies the south half of the lot.

The municipal address of this property is referenced as 314 Bell Street South and it is located in the City Ward (17 – Capital). The site is situated on the east side of Bell Street South, the lot is south of Champlain Street, and north of Henry Street, see site plan and legal survey plan in Appendix A for details.

The total area of this property is ±0.0497 hectares. In addition to the building addition the other development features will comprise of an interlock paver access to the front and south side entrance of the building, an amenity area in the rear yard, bicycle parking, landscaped areas throughout the site, etc., to meet the City of Ottawa's site plan requirements.

A site geotechnical report was prepared by the owner's soils engineer Paterson Group entitled Geotechnical Investigation Report (Report No. PG5905-1) dated August 23, 2021.

The serviceability report will provide the City of Ottawa with our serviceability brief to address the proposed servicing scheme for this site.

#### **Existing Site Conditions and Servicing**

This property is currently occupied by a three-storey brick and vinyl side 3-storey apartment residential building, which houses four '(2)-bedroom units and with the remaining surfaces of the lot being asphalted for vehicle parking and vehicle access along the south side of the existing building. For additional details of the site's pre-development condition, refer to the Google Image (2020) and aerial photography from (GeoOttawa–2019) in Appendix B.

The site is mainly hard impermeable surfaces that are covered with roof areas, asphalt laneway walkway, and rear yard parking.

The topography of the land is found to be entirely sloping from east to west or front to back.

There are existing water and sanitary service laterals currently servicing the existing apartment on Bell Street South which will be removed and interconnected to the proposed new services. The existing water service shall be blanked at the main and the existing building laterals shall be capped at the front property line for re-development of this lot.

As for the availability of underground municipal services, there are existing municipal services along Bell Street South in front of this property consisting of a 600mm diameter combined sewer and a 200mm diameter watermain for development of this property. Refer to the City of Ottawa Bell Street plan and profile drawings included in Appendix C for details.

Because the site is located within a combined sewershed, therefore, the approval exemption under Ontario Regulations 525/98 would not apply since stormwater discharges from this site will outlet flow into a combined sewer and not a storm sewer. Thus, an Environmental Compliance Approval (ECA) application will be required to be submitted to the Ministry.

#### **Proposed Residential Apartment Building Site**

There are no requirements for vehicle access or parking for this site. Interlock pavers are proposed at the front and south side of the new building for pedestrian access. Bicycle parking and spaces and covered garage enclosure are proposed at the rear yard of the new building addition area.

#### A. Water Supply

The proposed building addition located within Pressure Zone 1W at 314 Bell Street South will be a 3-storey residential apartment building with a basement. The building contains four (2)-bedroom units. Each floor covers an area of approximately 1,375 ft $^2$  (127.7 m $^2$ ) for a gross floor area of 5,500 ft $^2$  (511 m $^2$ ). The building is to be serviced by the 200 mm diameter watermain along Bell Street South.

The ground elevation on the property in question is approximately 73.98 m, as obtained from the attached **Topographic Plan** in Appendix D.

#### **Demand Projections**

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines and Technical Bulletin ISTB-2021-03, where the residential consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 2.5. Peak hour (PKHR) demands were calculated by multiplying MXDY demands by a factor of 2.2. Persons per unit (PPU) for each unit were estimated based on the City of Ottawa's Water Design Guidelines. **Table 1** shows the estimated domestic demands of the proposed building.

**Table 1: Estimated Domestic Demand** 

Unit Type	Unit	PPU	Consumption	AV	DY	MX	DY	PKI	HR
ome type	Count	110	Consumption	L/d	L/s	L/d	L/s	L/d	L/s
Apartment, 2-Bedroom	4	2.1	280	2,352	0.03	5,880	0.07	12,936	0.15
Total	4			2,352	0.03	5,880	0.07	12,936	0.15

The fire flow required was determined following the Fire Underwriter Survey (FUS) method and is provided in the attached worksheet. For this analysis, the building was classified as wood frame construction with building contents that are limited in combustibility. It is understood that it will not have a sprinkler system. The basement is 45% (less than 50%) below ground level. The resulting total required fire flow is 10,000 L/min (167 L/s) for a duration of 2.00 hours. Should the basement be built more than 50% below ground level, the resulting total required fire flow is 8,000 L/min (133 L/s) for a duration of 2.00 hours.

Details are provided in the attached **FUS Fire Flow Calculations** in Appendix D. **Figure 1** provides separation distances from adjacent buildings. The proposed **Site Plan and Architectural details** attached in Appendix D were used to determine distances from the proposed building to the property lines.

In summary, the estimated water demands for the proposed building addition are as follows:

- AVDY = 2,352 L/d (0.03 L/s)
- MXDY = 5,880 L/d (0.07 L/s);
- PKHR = 12,936 L/d (0.15 L/s);
- Fire Flow for basement less than 50% below ground level = 10,000 L/min (167 L/s), and
- Fire Flow for <u>basement more than 50% below ground level</u> = 8,000 L/min (133L/s).

#### **Boundary Conditions**

The hydraulic gradeline (HGL) boundary conditions for 314 Bell Street South, as presented in **Table 2**, were provided by the City on October 28, 2021 (see attached **Water Boundary Conditions Email** in Appendix D).

**Table 2: Boundary Conditions** 

Demand Scenario	Head (m)
Minimum HGL (Peak Hour)	106.7
Maximum HGL (Average Day)	115.1
Maximum Day + Fire Flow (133 L/s)	104.6
Maximum Day + Fire Flow (167 L/s)	102.0

#### **Hydraulic Analysis**

#### Peak Hour & Average Day

During peak hour demands, the resulting minimum hydraulic gradeline of 106.7 m corresponds to a peak hour pressure of 321 kPa (47 psi). This value is above the minimum pressure objective of 276 kPa (40 psi) for residential buildings up to two storeys. The peak hour pressure exceeds this objective and is therefore considered acceptable. Given that this building addition consists of a total of 3 storeys, further consideration will be needed to service the higher floors. Adding 5 psi per floor above two stories, a minimum pressure of 310 kPa (45 psi) would be required for the third floor. The peak hour pressure exceeds this objective and is therefore considered acceptable.

During average day demands, the resulting maximum hydraulic gradeline of 115.1 m corresponds to a maximum pressure of 403 kPa (58 psi). This value is below the maximum pressure objective of 552 kPa (80 psi) and is therefore considered acceptable.

Supporting hydraulic calculations are attached in Appendix D.

#### Maximum Day + Fire Flow

A maximum day plus fire flow hydraulic gradeline of 102.0 m corresponds to a residual pressure of 275 kPa (40 psi) at this location and is well above the minimum residual pressure requirements of 140 kPa (20 psi).

The combined hydrant flow coverage for the building was estimated based on Table 1 of Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02 and a desktop review (i.e., Google Street View) to identify hydrant class. Two Class AA hydrants are located within 76 m from the site, with a capacity of up to 5,678 L/min each. A third Class AA hydrant is located within 76 m to 122 m from the site, with a capacity of up to 3,785 L/min. The combined hydrant flow coverage for 314 Bell St S is therefore 15,141 L/min, which is above the FUS required fire flow (RFF) of 10,000 L/min (if the basement is less than 50% below ground level).

Hydrant coverage and classes are illustrated in Figure 2 attached in Appendix D. A breakdown of available hydrant flow is summarized in Table 3.

Table 3: Fire Hydrant Coverage

	Calculated FUS			Fire Hydrai	nts		Combined	
Building	Fire Flow	Hydrant	Withi	n 76 m	Between 76 m and 122 m		Hydrant	
	Demand (L/min)	Class	Quantity	Contrib. to RFF	Quantity	Contrib. to RFF	Flow Coverage (L/min)	
		AA	2	5,678	1	3,785		
314 Bell St	8 000(1) 10 000(2)	8,000(1)-10,000(2)	000 <sup>(1)</sup> -10,000 <sup>(2)</sup> A					15,141
S	8,000(1/-10,000(2)							
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In conclusion, based on the boundary conditions provided, the watermain along Bell Street South provides adequate fire flow capacity as per the Fire Underwriters Survey.

Anticipated pressures at the property line during basic day and peak hour demand conditions are within the pressure objectives as per the City of Ottawa's Drinking Water Design Guidelines.

#### B. Sanitary Flow

The peak sanitary flow for the 4 unit apartment building addition, which compromise of four (2)-bedroom apartment units, is estimated at Q = 0.13 L/s with an infiltration rate of 0.02 L/s. Refer to Appendix E regarding details of sanitary flow calculations. By combining peak sanitary flow from the existing (4) unit apartment building, the total peak sanitary flow for this site is 0.24 L/s. This flow will enter the existing 600mm dia. combined sewer on Bell Street South via the proposed 150 mm dia. PVC sanitary lateral from the combined two (2) residential apartment buildings.

Presently the existing peak sanitary flow of the site from the existing apartment building is Q = 0.13 L/s with an infiltration rate of 0.2 L/s. The net increase in flow from this proposed development is 0.11 L/s.

The peak sanitary flow of 0.24 L/s will be subtracted from the net allowable controlled release rate regarding stormwater management controlled flow for this site into the existing combined sewer.

Waste water from the Bell Street South 600mm dia. combined sewer then in turn outlets north then westward into the existing 1650mm dia. concrete combined sewer located along Preston Street.

#### C. Storm Flow

The storm-water outlet for the proposed development property will be the existing 600mm dia. combined sewer located on Bell Street South. Stormwater attenuation on-site for the new building addition will be accomplished by means of rooftop storage with controlled roof drains that regulate flow off-site.

The building foundation weeping-tile drainage system shall have its own separate pipe for gravity flow where weeping-tile water is outletted via a 150mm diameter storm pipe to the existing 600mm diameter combined sewer. The stormwater outlet for the rooftop water from roof drains will be a separately designated proposed 150mm diameter PVC pipe that will also be outletted directly into the existing 600mm diameter combined sewer.

For on-site Storm Water Management attenuation measures, three (3) roof drains are proposed for the new 3-storey apartment building addition with flat roof to restrict flow at a rate of 0.32 L/s per drain or 3 x 0.32 L/s = 0.96 L/s into the Bell Street South combined sewer. The calculated net allowable controlled release rate from this site is 5.07 L/s (5.31 L/s - 0 L/s - 0.24 L/s). Therefore, the designed restricted flow of 0.96 L/s proposed for this site is less than the net allowable controlled release rate calculated at 5.07 L/s. See Storm Drainage Report (R-821-102) for details.

Based on the residential site plan from the owner's architect, the average post-development runoff coefficient for this lot is estimated at C = 0.66 and A = 0.0497 hectares.

An estimation of the pre-development flow condition was carried out using the criteria accepted by the City of Ottawa. If post-development C valve exceeds the lesser of the  $C_{pre} = 0.89$  or  $C_{allow} = 0.5$  (max) then SWM is required. So from our calculations, the  $C_{allow} = 0.5$  value will be used at  $t_c = 10$  minutes for pre-development allowable flow calculation off-site.

The pre-development flow rate calculation into the 600mm dia. combined sewer for this residential area is the lesser of the two (2)-year storm event where  $C_{allow} = 0.5$  (max.) runoff value and  $t_c = 10$  minutes or the average  $C_{pre}$  value which is 0.89 using  $t_c = 10$  minutes. Because this site  $C_{post} = 0.66$  and  $C_{pre} = 0.5$  then SWM measures are required.

Therefore, based on our calculation, on-site retention is required for the proposed new building, because the site post-development C value of 0.66 is greater than the  $C_{pre} = 0.5$ .

In assessing the 2-Year up to the 100-Year storm events under pre-development conditions to the same storm events under post-development conditions with implementation of the proposed on-site SWM measures (flat rooftop storage with (3) specified controlled drains) it was determined that post development release rates has been improved for the site compared with the current existing flow rates.

The pre-development flow at the 2-Year storm event is estimated at 9.44 L/s and 24.68 L/s for the 100-Year event. By incorporating the proposed SWM attenuation measures, the post development 2-Year flow off-site is estimated at 5.54 L/s (4.34 L/s + 0.96 L/s + 0.24 L/s) and the 100-Year flow is estimated at 12.87 L/s (11.67 L/s + 0.96 L/s + 0.24 L/s).

Therefore for this proposed development site, the two (2)-Year post development release rate of 5.54 L/s is less than the 2-Year pre-development flow rate of 9.44 L/s. For storm events up to and including the 100-Year event, the total 100-Year post-development release rate of 12.87 L/s is less than the 100-Year pre-development flow of 24.68 L/s.

The storage volume for the two (2)-year, five (5)-year and up to the 100-year storm event will be stored by means of flat rooftop at the third floor of the new 3-storey apartment building addition. Also refer to the site storm drainage report (Report No. R-821-102) for further details.

To control the two (2)-year storm water release rate to a flow rate of 0.96 L/s, a site storage volume of approximately 1.17 m³ minimum is required during the two (2)-year event. For this site, three (3) flat rooftop storage areas will be used for stormwater management attenuation.

During the two (2)-year storm event for the flat rooftop storage, the ponding depth of rooftop area 1, 2, and 3 is estimated at 100mm at the drain and 0mm at the roof perimeter, assuming a 2.0% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 0.63  $m^3$ , roof storage available at Roof Area 2 is 0.71  $m^3$  and the rooftop storage available at Roof Area 3 is 0.60  $m^3$ , for a total of 1.94  $m^3$ , which is greater than the required volume of 1.17  $m^3$ .

To control the five (5)-year stormwater release rate off-site to a net allowable rate of 0.96 L/s, a site storage volume of approximately 1.97 m<sup>3</sup> minimum is required during the five (5)-year event.

During the five (5)-year storm event for the flat rooftop storage, the ponding depth of Roof Area 1, 2 and 3 is estimated at 120 mm at the drain and 0mm at the roof perimeter, assuming a 2.0% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is  $1.01 \, \text{m}^3$ , Roof Area 2 is  $1.23 \, \text{m}^3$  and the rooftop storage available at Roof Area 3 is  $0.95 \, \text{m}^3$ , for a total of  $3.19 \, \text{m}^3$ , which is greater than the required volume of  $1.97 \, \text{m}^3$ .

To control the 100-year stormwater release rate off-site to a net allowable rate of 0.96 L/s, a site storage volume of approximately 4.90 m<sup>3</sup> minimum is required during the 100-year event.

During the 100-year storm event for the flat rooftop storage, the ponding depth of Roof Area 1, 2 and 3 is estimated at 150 mm at the drain and 0mm at the roof perimeter, assuming a 2.0% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 2.09  $m^3$ , Roof Area 2 is 2.37  $m^3$  and the rooftop storage available at Roof Area 3 is 6.36  $m^3$ , which is greater than the required volume of 4.90  $m^3$ .

Therefore, by means of flat building rooftop storage and grading the site to the proposed grades as shown on the Proposed Grading and Servicing Plan and Proposed Rooftop Stormwater Management Plan Dwg. 821-102 G-1 and 821-102 SWM-1 respectively, the desirable two (2)-year, five (5)-year storm and 100-year storm event detention volume of 1.94 m³, 3.19 m³ and 6.36 m³ respectively will be available on site.

The building weeping tile drainage will outlet via its separate 150mm diameter PVC storm lateral. The roof drains will be outletted also via a separate 150mm PVC storm lateral, where upon both laterals are connected directly to the existing Bell Street South 600mm diameter combined sewer. Refer to the proposed Grading and Servicing Plan Dwg. 821-102 G-1 for details.

#### **Erosion and Sediment Control**

The contractor shall implement Best Management Practices to provide for protection of the receiving storm sewer during construction activities. These practices are required to ensure no sediment and/or associated pollutants are released to the receiving watercourse. These practices include installation of a "siltsack" catch basin sediment control device or equal in catch basins as recommended by manufacturer on-site and off-site within the Bell Street South road right of way adjacent to this property. Siltsack shall be inspected every 2 to 3 weeks and after major storm. The deposits will be disposed of as per the requirements of the contract. See Dwg. #821-102 ESC-1 for details.

Refer to Appendix F for the summary of the Development Servicing Study Checklist that is applicable to this development.

PREPARED BY T.L. MAK ENGINEERING CONSULTANTS LTD.

TONY L. MAK, P.ENG

### THREE-STOREY APARTMENT BUILDING ADDITION

**LOT RE-DEVELOPEMENT SITE** 

PART OF LOT "D"

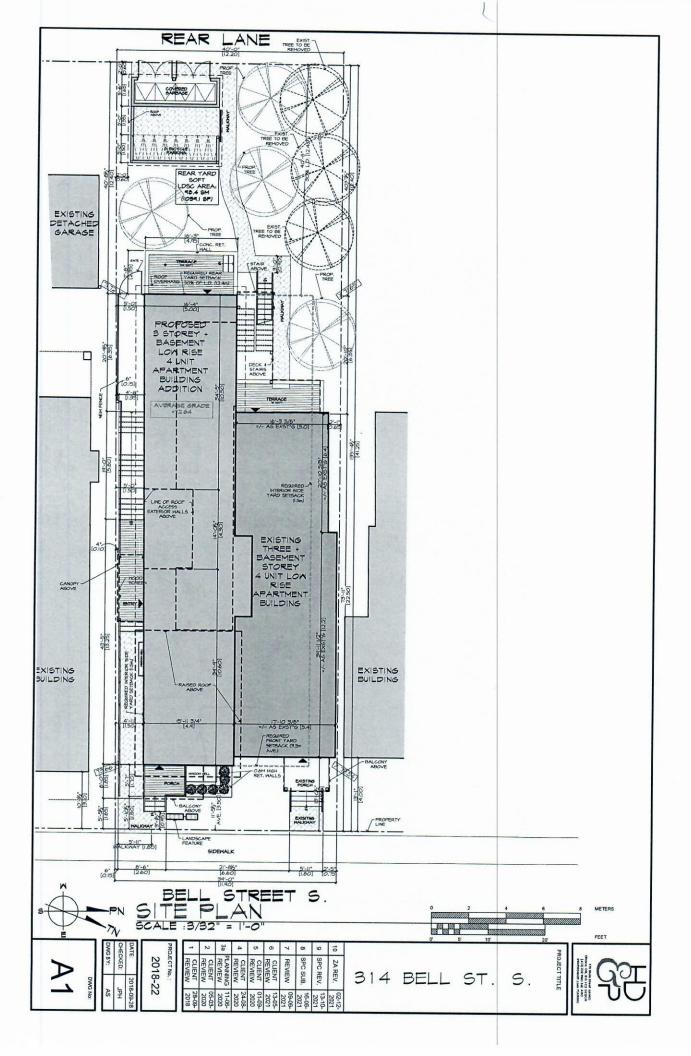
R-PLAN 82717

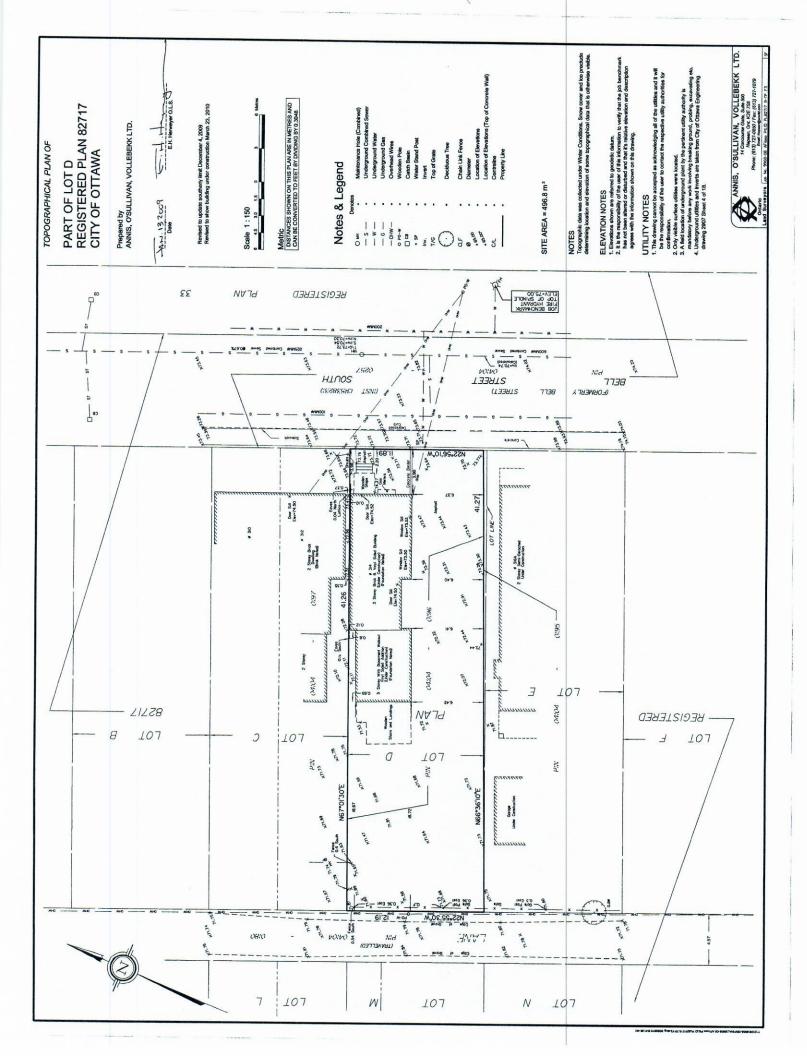
314 BELL STREET SOUTH

**CITY OF OTTAWA** 

APPENDIX A

SITE PLAN AND LEGAL SURVEY PLAN





# THREE-STOREY APARTMENT BUILDING ADDITION

**LOT RE-DEVELOPEMENT SITE** 

PART OF LOT "D"

**R-PLAN 82717** 

**314 BELL STREET SOUTH** 

**CITY OF OTTAWA** 

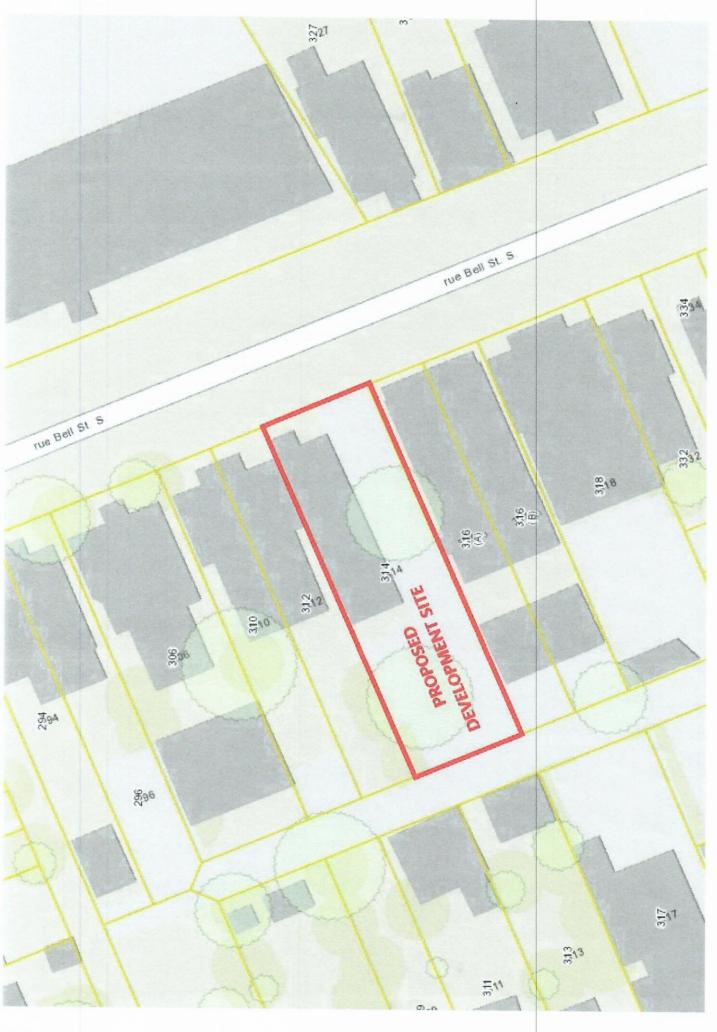
#### **APPENDIX B**

SITE PRE-DEVELOPMENT CONDITION

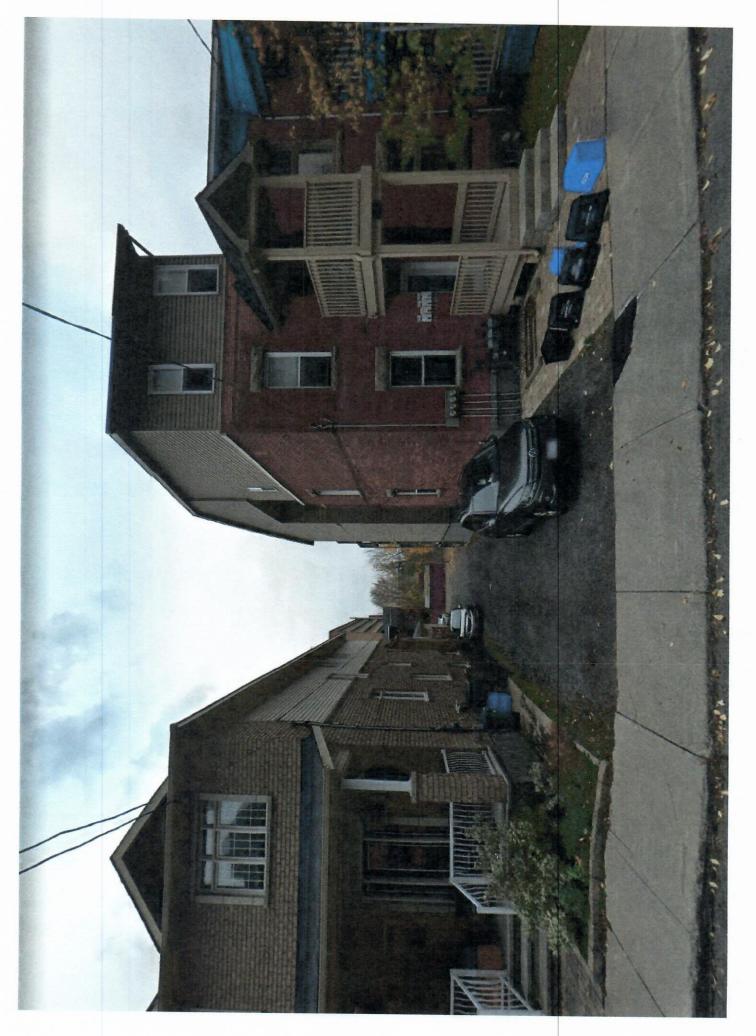
**GOOGLE IMAGE (2020)** 

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**AERIAL PHOTOGRAPHY 2019 (GEOOTTAWA)** 







## THREE-STOREY APARTMENT BUILDING ADDITION

**LOT RE-DEVELOPEMENT SITE** 

PART OF LOT "D"

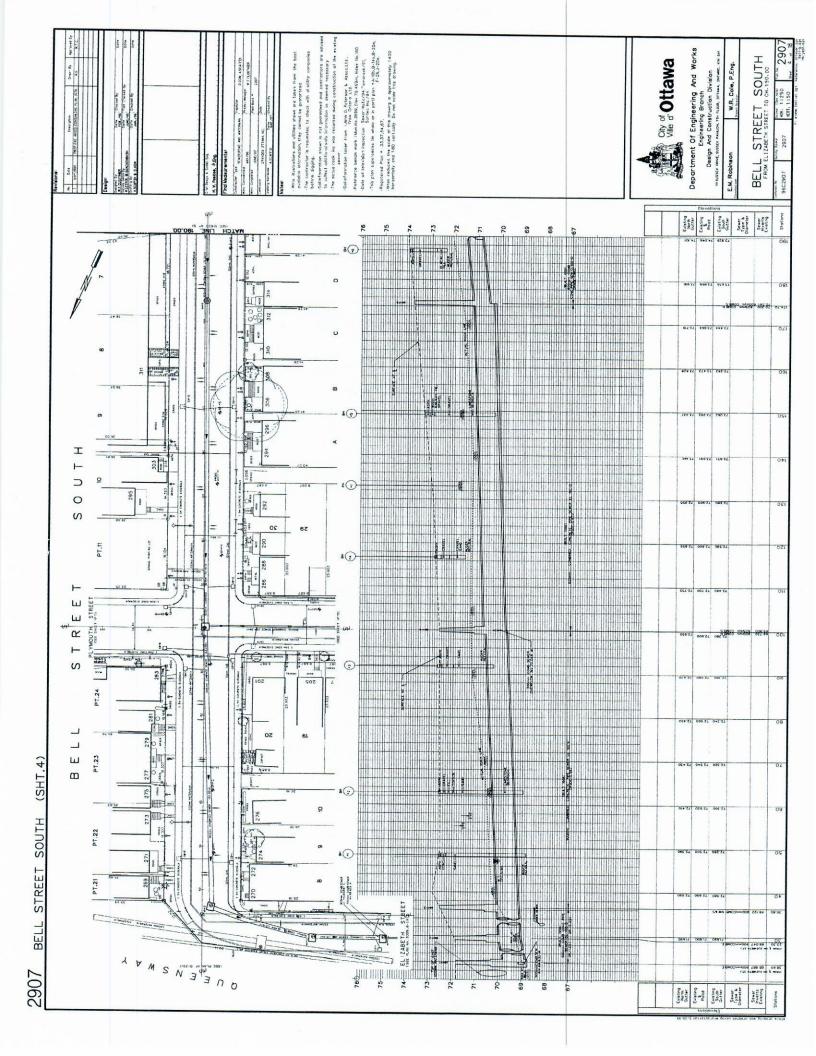
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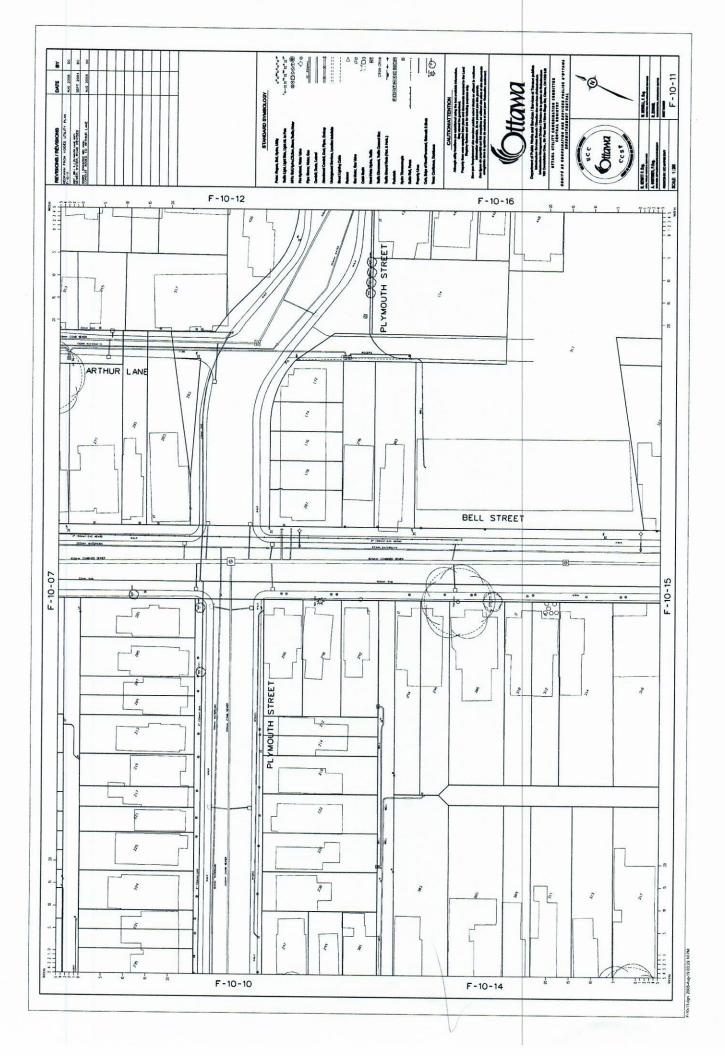
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**CITY OF OTTAWA** 

APPENDIX C
BELL STREET SOUTH
CITY OF OTTAWA
PLAN AND PROFILE
AND

**UCC DRAWING** 





### THREE-STOREY APARTMENT BUILDING ADDITION

#### LOT RE-DEVELOPEMENT SITE

PART OF LOT "D"

R-PLAN 82717

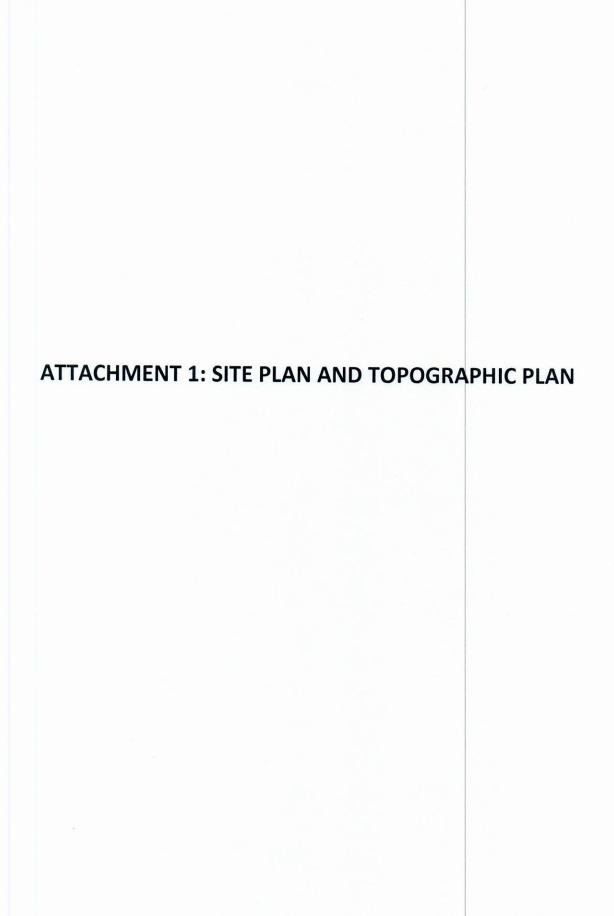
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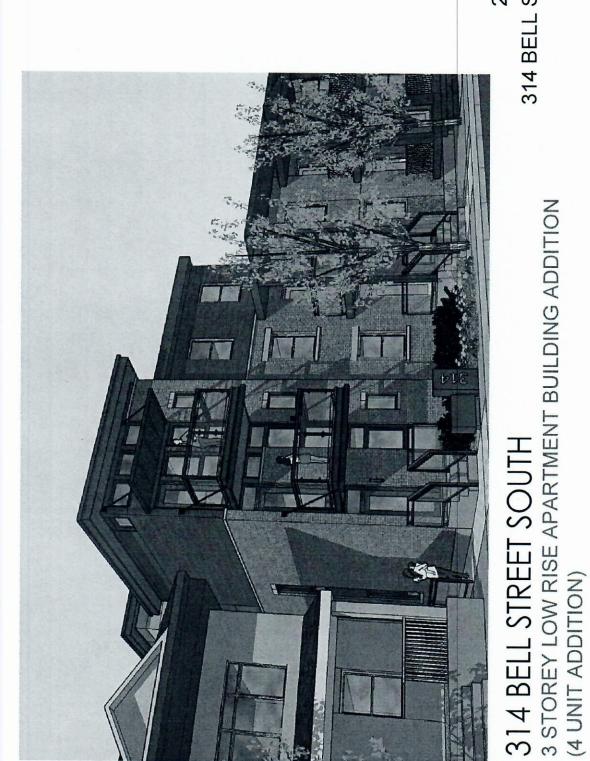
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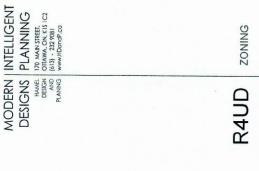
#### APPENDIX D

#### **CITY OF OTTAWA**

- SITE PLAN AND TOPOGRAPHIC PLAN
- FUS FIRE FLOW CALCULATIONS
- FUS EXPOSURE DISTANCE (FIGURE 1)
- WATER BOUNDARY CONDITIONS
- SUPPORTING HYDRAULIC CALCULATIONS
- HYDRANT SPACING (FIGURE 2)





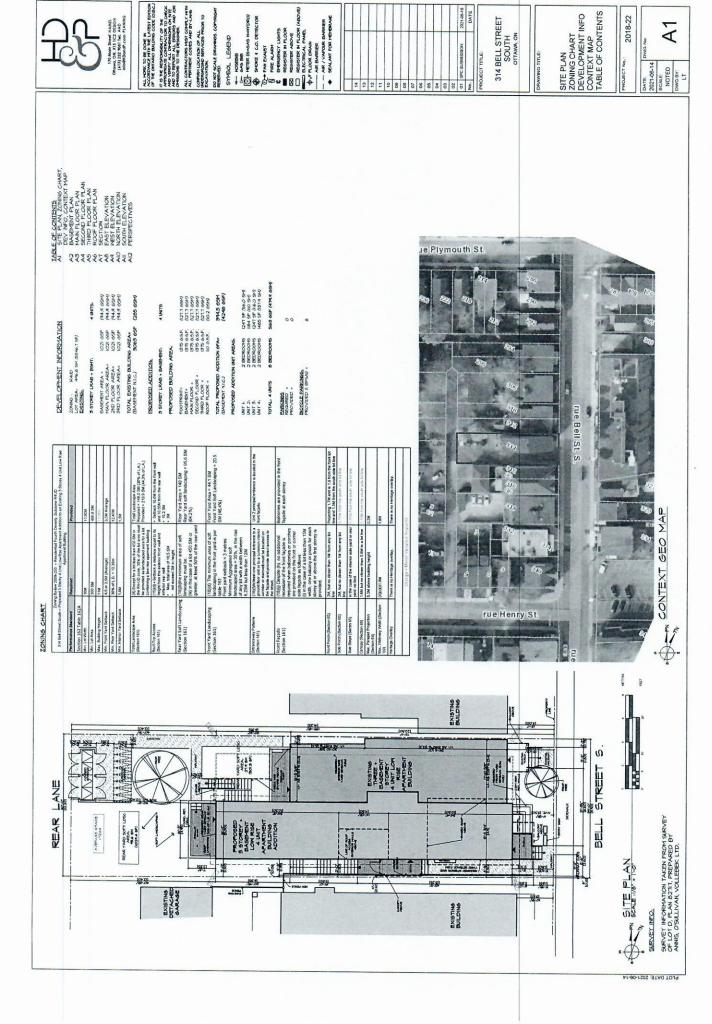


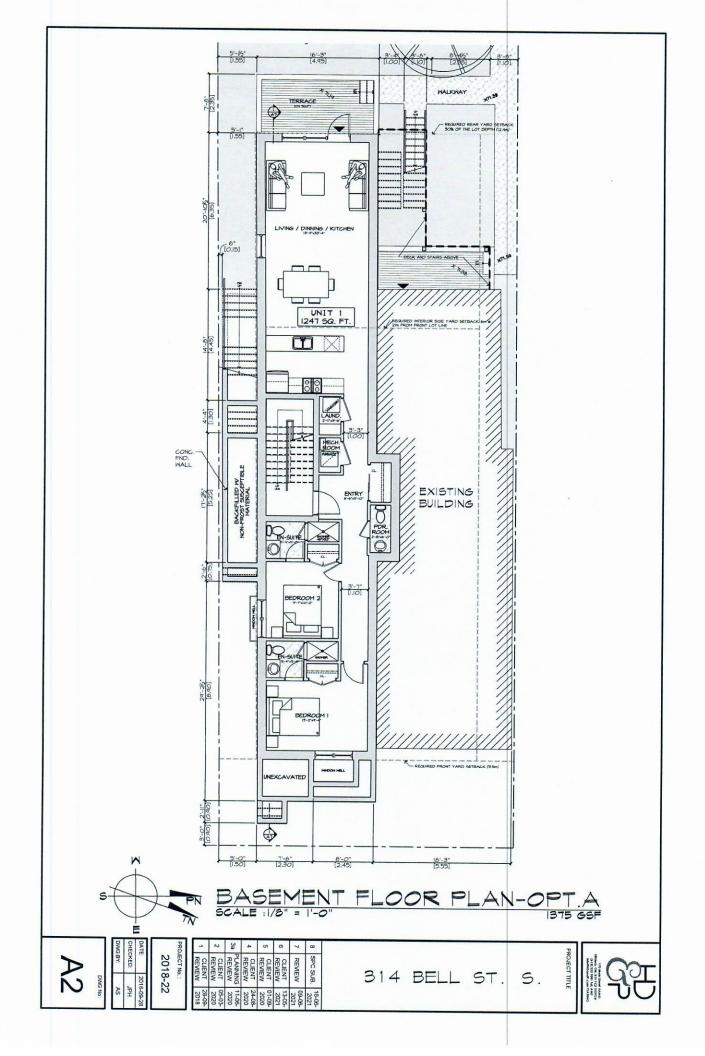
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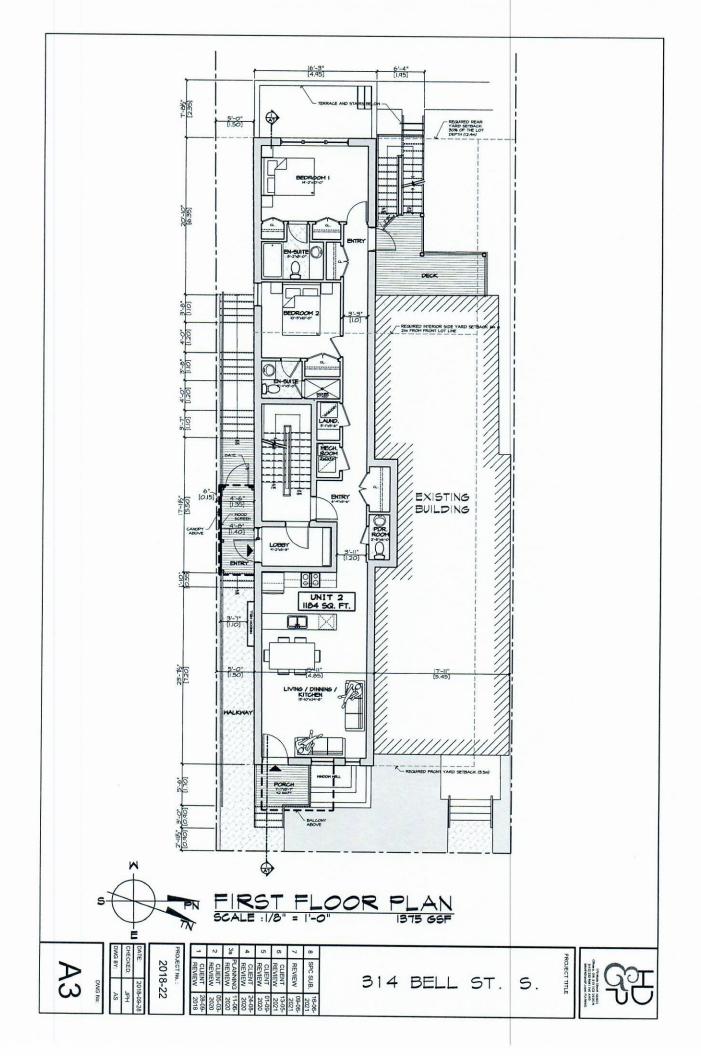
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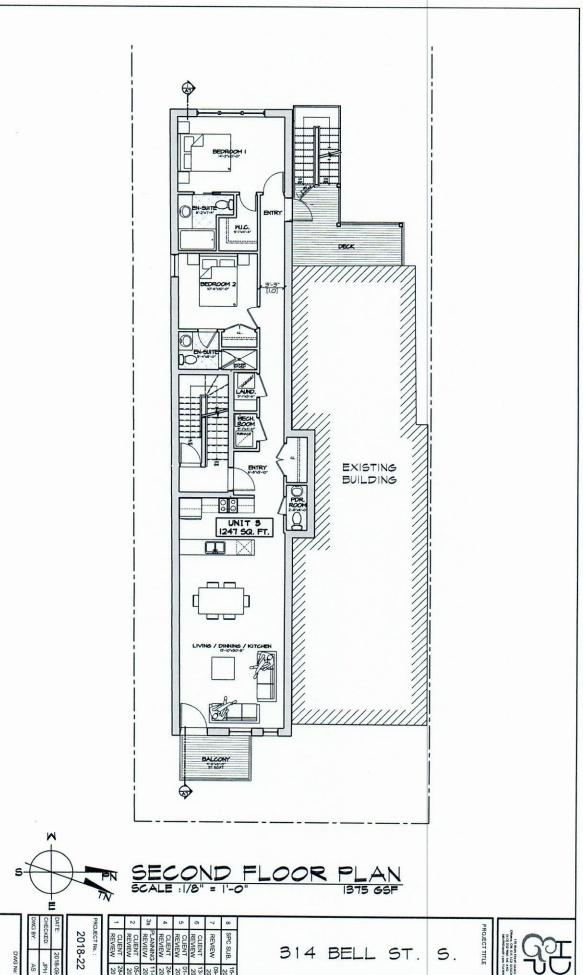
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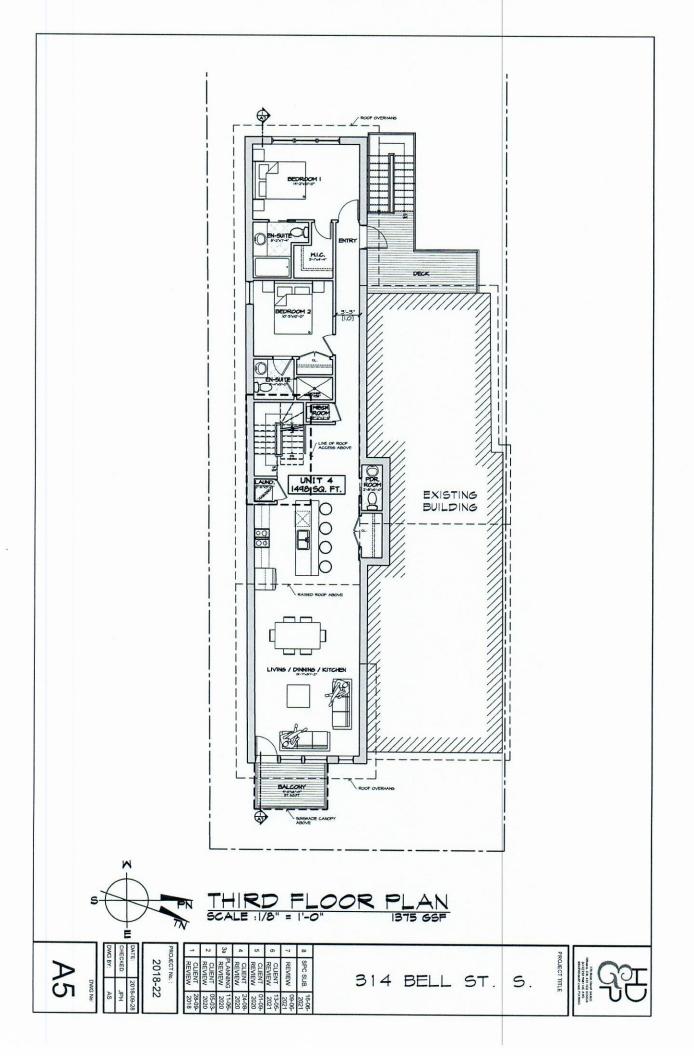
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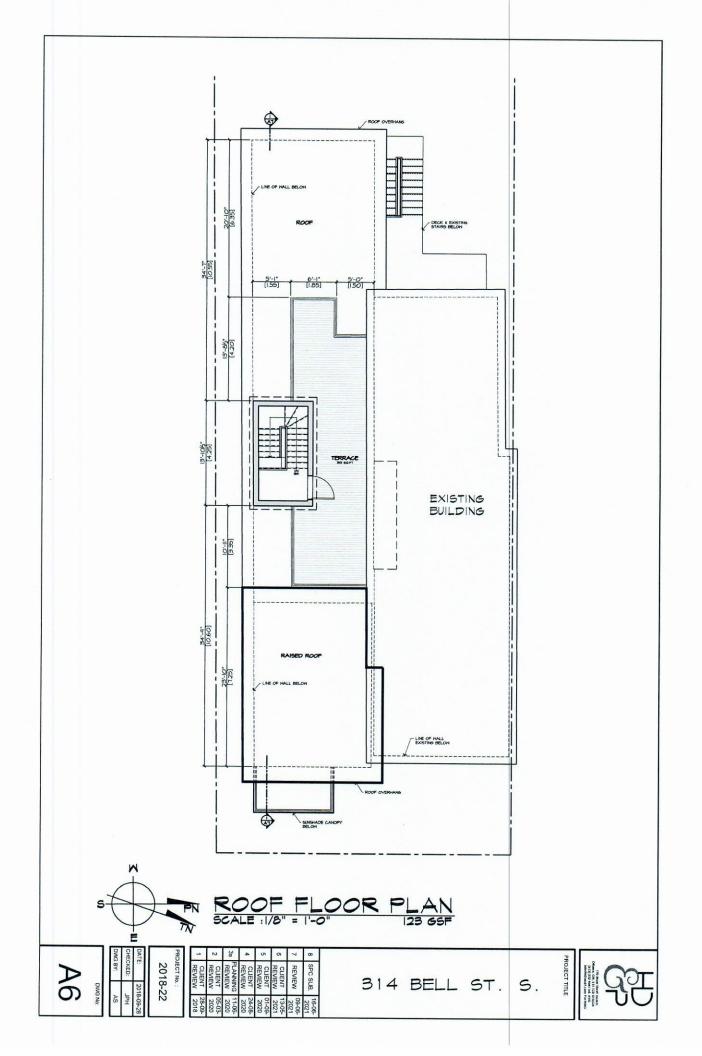


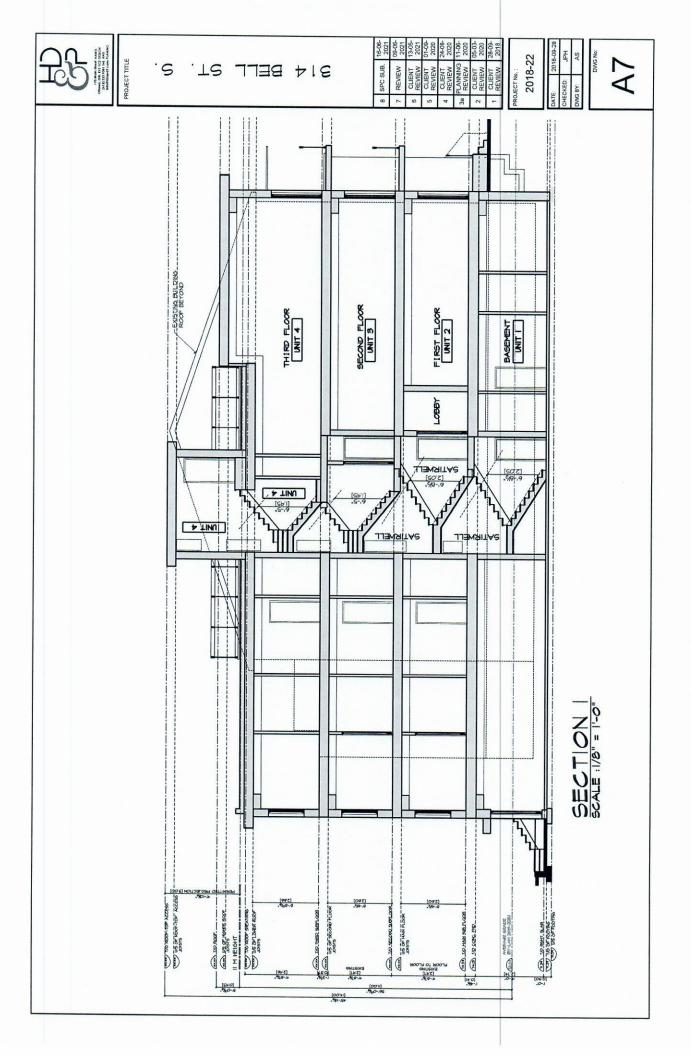












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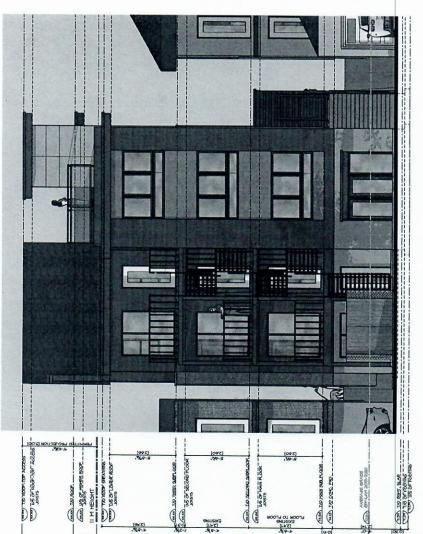
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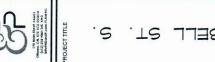
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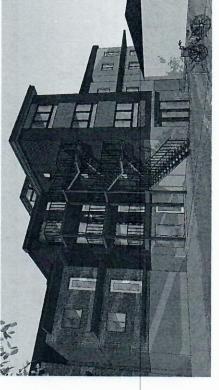
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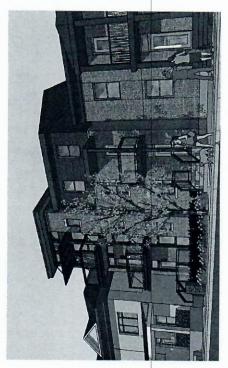
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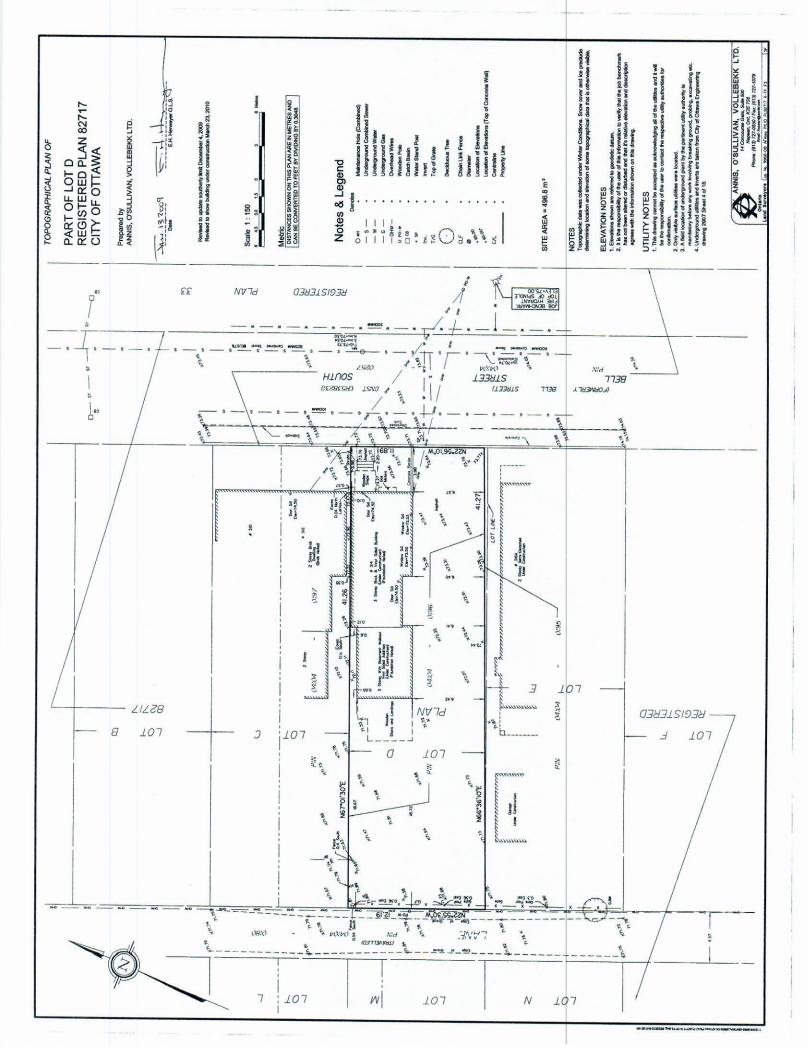
NORTH WEST ELEVATION



SOUTH EAST ELEVATION



SOUTH WEST ELEVATION



**ATTACHMENT 2: FUS FIRE FLOW CALCULATIONS** 



# **FUS Fire Flow Calculation**

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401084 Project Name: 314 Bell St S

Date: October 1, 2021

Fire Flow Calculation #: 1 Building Type/Description/Name: Residential

Data inputted by: Christène Razafimaharo, M.Sc., EIT Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

Building addition. Fire separation to be provided between building addition and existing building.

Notes: Basement total height = 74.13 ft - 71.39 ft = 2.74 ft; basement height below grade = 72.64 ft - 71.39 ft = 1.25 ft. Basement is therefore 45% below grade,

i.e. less than 50% below grade.

Step	Task	Term	Options	Choose:	Value Used	Unit	Total Fire Flow (L/min)						
		Framing Material											
	Choose Frame Used for Construction of		Wood Frame 1.5										
1		Coefficient related to	Ordinary construction 1										
	Unit	type of construction	Non-combustible construction	0.8	Wood Frame	1.5	m						
		(C)	Fire resistive construction (< 2 hrs)	0.7									
			Fire resistive construction (> 2 hrs)	0,6									
	Choose Type of Housing (if TH,		FI	oor Space Area									
2	Enter Number of		Single Family	1									
	Units Per TH	Type of Housing	Townhouse - indicate # of units	1	Other (Comm, Ind, Apt	4	Units						
	Block)		Other (Comm, Ind, Apt etc.)	4	etc.)								
2.2	# of Storeys	Number of Floors/S	Storeys in the Unit (do not include basement if	50% below grade):	4	4	Storeys						
3	Enter Ground Floor Area of One Unit	Average I	Floor Area (A) based total floor area of all floor	s (non-fire resistive construction):	128	511	Area in Square						
4	Obtain Required Fire Flow without Reductions		Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A)  Round to nearest 1,000 L/min										
5	Apply Factors	Reductions/Increases Due to Factors Affecting Burning											
	Affecting Burning		Reductions/Increases	s Due to Factors	Affecting Burning								
	Affecting Burning		Reductions/Increases Non-combustible		Affecting Burning								
	Affecting Burning Choose	Occupancy content		-0.25	Affecting Burning								
5.1	Choose Combustibility of	Occupancy content hazard reduction or	Non-combustible			-0.15	N/A	5 950					
5.1	Choose		Non-combustible Limited combustible	-0.25 -0.15 0	Affecting Burning  Limited combustible	-0.15	N/A	5,950					
5.1	Choose Combustibility of	hazard reduction or	Non-combustible Limited combustible Combustible	-0.25 -0.15 0 0.15		-0.15	N/A	5,950					
5.1	Choose Combustibility of	hazard reduction or surcharge	Non-combustible Limited combustible Combustible Free burning	-0.25 -0.15 0	Limited combustible								
5.1	Choose Combustibility of	hazard reduction or	Non-combustible Limited combustible Combustible Free burning Rapid burning	-0.25 -0.15 0 0.15 0.25		-0.15	N/A N/A	5,950					
5.1	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of	hazard reduction or surcharge	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13	-0.25 -0.15 0 0.15 0.25 -0.3	Limited combustible  None  Water supply is not		N/A	0					
	Choose Combustibility of Building Contents  Choose Reduction	hazard reduction or surcharge Sprinkler reduction	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and	-0.25 -0.15 0 0.15 0.25 -0.3	Limited combustible  None	0							
	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept, hose line	-0.25 -0.15 0 0.15 0.25 -0.3 0	Limited combustible  None  Water supply is not	0	N/A N/A	0					
	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of	hazard reduction or surcharge Sprinkler reduction Water Supply Credit	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept, hose line Water supply is not standard or N/A	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1	None  Water supply is not standard or N/A	0	N/A	0					
	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1	None  Water supply is not standard or N/A  Sprinkler not fully	0	N/A N/A	0					
5.2	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers  Choose Separation	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision Credit  Exposure Distance	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1 0	None  Water supply is not standard or N/A  Sprinkler not fully supervised or N/A	0 0	N/A N/A N/A	0 0					
5.2	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision Credit	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A North Side	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1 0 Fire Wall	None  Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  0.1	0	N/A N/A	0					
5.2	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers  Choose Separation Distance Between	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision Credit  Exposure Distance	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A North Side East Side	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1 0 Fire Wall 20.1 to 30.1m	None  Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  0.1  0.1	0 0	N/A N/A N/A	0 0					
5.2	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers  Choose Separation Distance Between	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision Credit  Exposure Distance	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A North Side East Side South Side	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1 0 Fire Wall 20.1 to 30.1m 0 to 3.0m 10.1 to 20.0m	None  Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  0.1  0.1  0.25  0.15	0 0 0 0.6	N/A N/A N/A	0 0					
5.2	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers  Choose Separation Distance Between Units  Obtain Required	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision Credit  Exposure Distance	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A North Side East Side South Side West Side	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1 0 Fire Wall 20.1 to 30.1m 0 to 3.0m 10.1 to 20.0m	None  Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  0.1  0.1  0.25  0.15	0 0 0 0.6	N/A N/A N/A m	0 0 0 3,570					
5.2	Choose Combustibility of Building Contents  Choose Reduction Due to Presence of Sprinklers  Choose Separation Distance Between Units	hazard reduction or surcharge  Sprinkler reduction  Water Supply Credit  Sprinkler Supervision Credit  Exposure Distance	Non-combustible Limited combustible Combustible Free burning Rapid burning Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A North Side East Side South Side West Side	-0.25 -0.15 0 0.15 0.25 -0.3 0 -0.1 0 Fire Wall 20.1 to 30.1m 0 to 3.0m 10.1 to 20.0m	None  Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  0.1  0.1  0.25  0.15	0 0 0 0.6	N/A N/A N/A m its applied:	0 0 0 3,570					



## **FUS Fire Flow Calculation**

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401084 Project Name: 314 Bell St S

Date: October 1, 2021

Fire Flow Calculation #: 2
Building Type/Description/Name: Residential

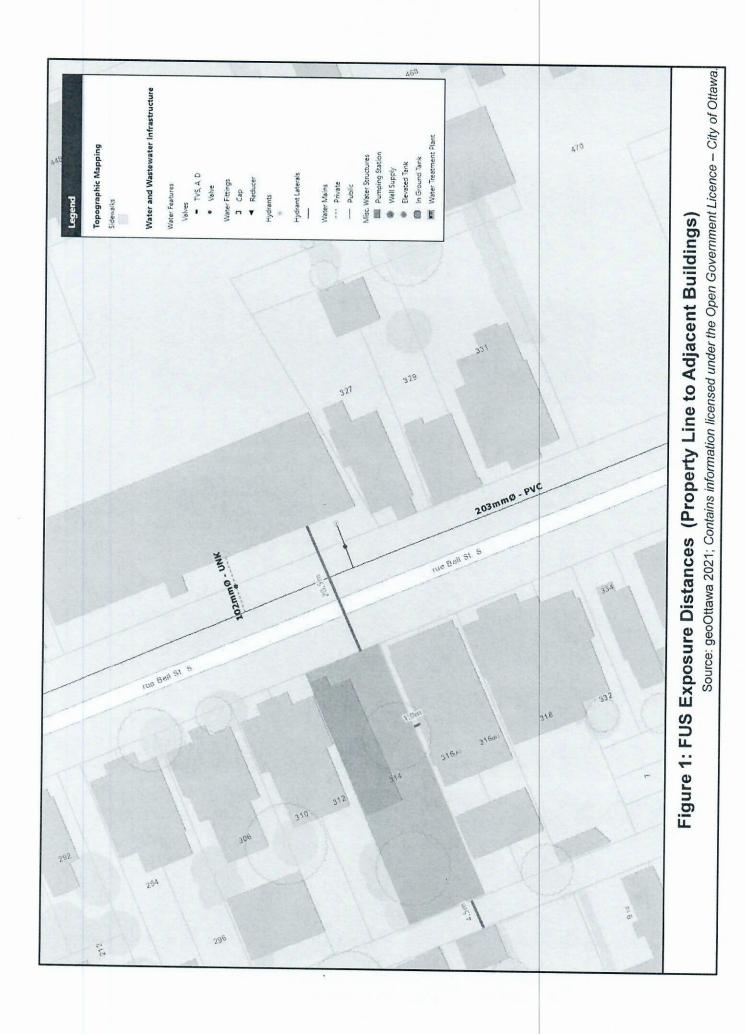
Data inputted by: Christène Razafimaharo, M.Sc., EIT Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

Notes: Building addition. Fire separation to be provided between building addition and existing building. Assuming basement is built more than 50% below grade.

Step	Task	Term	Options	Choose:	Value Used	Unit	Total Fire Flow (L/min)					
		Framing Material										
	Choose Frame		Wood Frame	1.5								
1	Used for Construction of	Coefficient related to	Ordinary construction									
	Unit	type of construction (C)	Non-combustible construction	0.8	Wood Frame	1.5	m					
		(C)	Fire resistive construction (< 2 hrs)	0.7								
			Fire resistive construction (> 2 hrs)	0.6								
	Choose Type of Housing (if TH,		FI	oor Space Area				W				
2	Enter Number of		Single Family	1		- No. 100						
	Units Per TH	Type of Housing	Townhouse - indicate # of units	1	Other (Comm, Ind, Apt	4	Units					
	Block)		Other (Comm, Ind, Apt etc.)	4	etc.)		- Crinto					
2.2	# of Storeys	Number of Floors/	Storeys in the Unit (do not include basement if	50% below grade):	3	3	Storeys					
3	Enter Ground Floor Area of One		Floor Area (A) based total floor area of all floor		128	383	Area in Square					
	Unit			Square Metres (m2)	363	Meters (m <sup>2</sup> )	10 mag					
4	Obtain Required Fire Flow without Reductions		Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A)  Round to nearest 1,000 L/min									
5	Apply Factors Affecting Burning		Reductions/Increases	Due to Factors	Affecting Burning							
	Choose	100	Non-combustible	-0.25								
		Occupancy content	Limited combustible	-0.15			N/A					
5.1	Combustibility of	hazard reduction or	Combustible	0	Limited combustible	-0.15		5,100				
	Building Contents	Iding Contents surcharge	Free burning	0.15				0,100				
			Rapid burning	0.25								
1220		Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3			Versa III					
		Sprinkler reduction	None	0	None	0	N/A	0				
5.2	Choose Reduction Due to Presence of	Water Supply Credit	Water supply is standard for sprinkler and fire dept, hose line	-0.1	Water supply is not	0	N/A	0				
	Sprinklers		Water supply is not standard or N/A	0	standard or N/A		197	Ū				
		Sprinkler Supervision	Sprinkler system is fully supervised	-0.1	Sprinkler not fully	-						
		Credit	Sprinkler not fully supervised or N/A	0	supervised or N/A	0	N/A	0				
			North Side	Fire Wall	0.1							
	01 0 11	Exposure Distance	East Side	20.1 to 30.1m	0.1							
5.3	Choose Separation			0 to 3.0m	0.25	0.6	m	3,060				
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	South Side	0 10 3.0111	0.25							
5.3	Distance Between		South Side West Side	10.1 to 20.0m	0.15							
5.3	Distance Between			10.1 to 20.0m	0.15	/min lim	its applied:	8,000				
	Distance Between Units  Obtain Required		West Side	10.1 to 20.0m ed to nearest 1,0	0.15			8,000 133				
5.3	Distance Between Units		West Side	10.1 to 20.0m ed to nearest 1,0	0.15 000 ☑min, with max	Flow (ab	ove) in L/s:	-				

ATTACHMENT 3: FIGURE 1 – FUS EXPO	SURE DISTANCES

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**ATTACHMENT 4: WATER BOUNDARY CONDITIONS** 

### Razafimaharo, Christene

From:

TL MaK <tlmakecl@bellnet.ca>

Sent:

Thursday, October 28, 2021 11:15 AM

To:

Alemany, Kevin

Cc:

Razafimaharo, Christene

Subject:

FW: 314 Bell Street - Water Boundary Conditions Request

Attachments:

314 Bell Street South October 2021.pdf

Hi Kevin,

Attached please find water boundary conditions received on October 28, 2021 from the City of Ottawa regarding 314 Bell Street.

Could you please proceed with your calculations at your earliest convenience for our serviceability report preparation.

Let us know if you have any questions or comments.

Regards,

Tony Mak

T.L. Mak Engineering Consultants Ltd. 1455 Youville Drive, Suite 218 Ottawa, ON. K1C 6Z7 Tel. 613-837-5516 | Fax: 613-837-5277

E-mail: tlmakecl@bellnet.ca

From: Bakhit, Reza [mailto:reza.bakhit@ottawa.ca]

Sent: October 28, 2021 10:10 AM

To: TL MaK

Subject: 314 Bell Street - Water Boundary Conditions Request

Hi Tony,

The following are boundary conditions, HGL, for hydraulic analysis at 314 Bell Street South North (zone 1W) assumed to be connected to the 203 mm watermain on Bell Street South (see attached PDF for location).

Minimum HGL: 106.7 m Maximum HGL: 115.1 m

Max Day + FF (133 L/s): 104.6 m Max Day + FF (167 L/s): 102.0 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be

assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Regards,

#### Reza Bakhit, P.Eng, C.E.T

Project Manager

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - Centeral Branch

City of Ottawa | Ville d'Ottawa

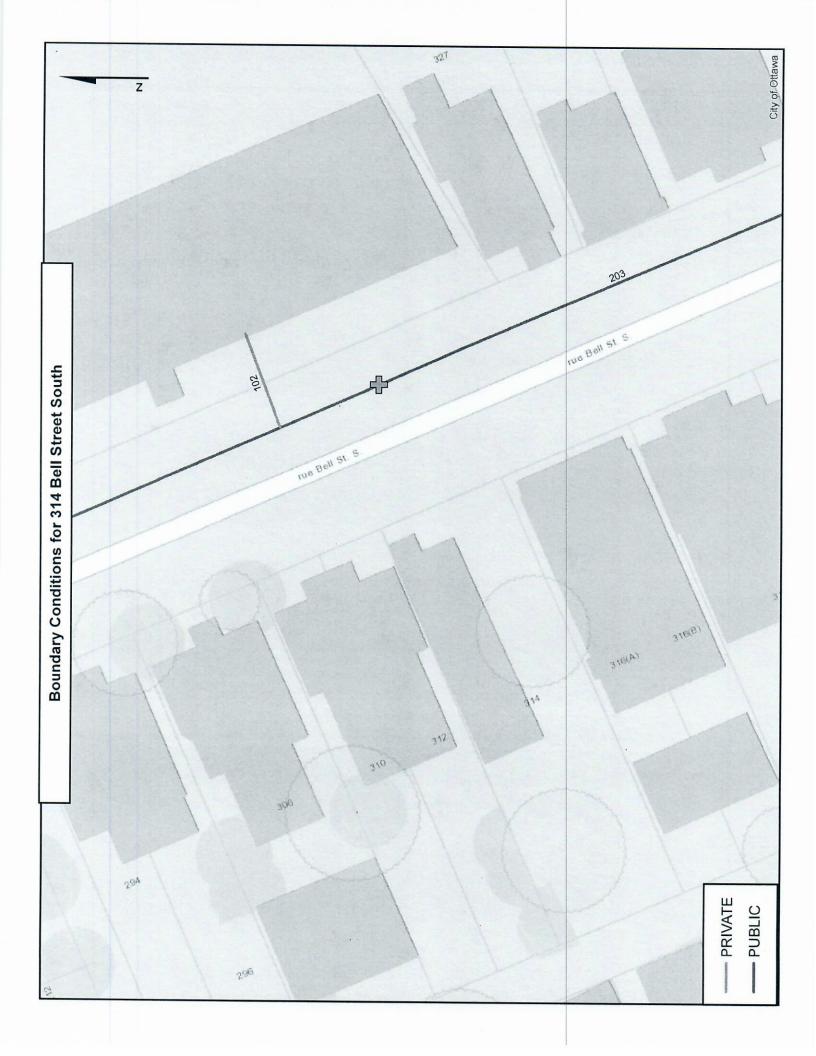
110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2400 ext./poste 19346, reza.bakhit@ottawa.ca

Please note: Given the current pandemic, I will be working from home until further notice; reaching me by email is the easiest. I will be checking my voicemail, just not as frequently as I normally would be.

This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par son destinataire prévu est interdite. Je vous remercie de votre collaboration.



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ATTACHBAENT E CHEROCETING HIVER ALLING	
ATTACHMENT 5: SUPPORTING HYDRAULIC C	ALCULATIONS
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#### Supporting Hydraulic Calculations

Stantec Project #: 163401084 Project Name: 314 Bell St S

Date: October 28, 2021

Data inputted by: Christène Razafimaharo, M.Sc., EIT Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

### Boundary Conditions provided by the City:

Scenario 1: Peak Hour (Min HGL): 106.7 m;

Scenario 2: Average Day (Max HGL): 115.1 m; and Scenario 3: Maximum Day plus Fire Flow: 102.0 m.

#### **Sample Calculations**

HGL(m) = hp + hz

(1)

where: hp = Pressure Head (m); and hz = Elevation Head (m), estimated from topography.

For Scenario 1, we have:

HGL(m) = 106.7 and hz (m) = 73.98.

Rearranging Equation 1, we can calculate the Pressure Head (hp) as follow:

hp (m) = HGL - hz $\therefore hp = 106.7 - 74.0 \text{ m} = 32.7 \text{ m}.$ 

To convert from Pressure Head (m) to a pressure value (kPa), the following equation can be used:

P(kPa) = (p \* g \* hp) / 1000 (2)

where:  $\rho$  = density of water = 1000 kg/m<sup>3</sup>; and g = gravitational acceleration = 9.81 m/s<sup>2</sup>.

Using Equation 2, we can calculate the Pressure (P) as follow:

P (kPa) = (1000 \* 9.81 \* 32.72) / 1000 ∴ P = 321 kPa.

Considering that 1 kPa = 0.145 psi, the pressure under Scenario 1 is equal to:

P = 47 psi.

Applying the same procedures, the pressures under Scenario 2 and Scenario 3 are calculated as follows: Scenario 2: P = 58 psi; and Scenario 3: P = 40 psi.

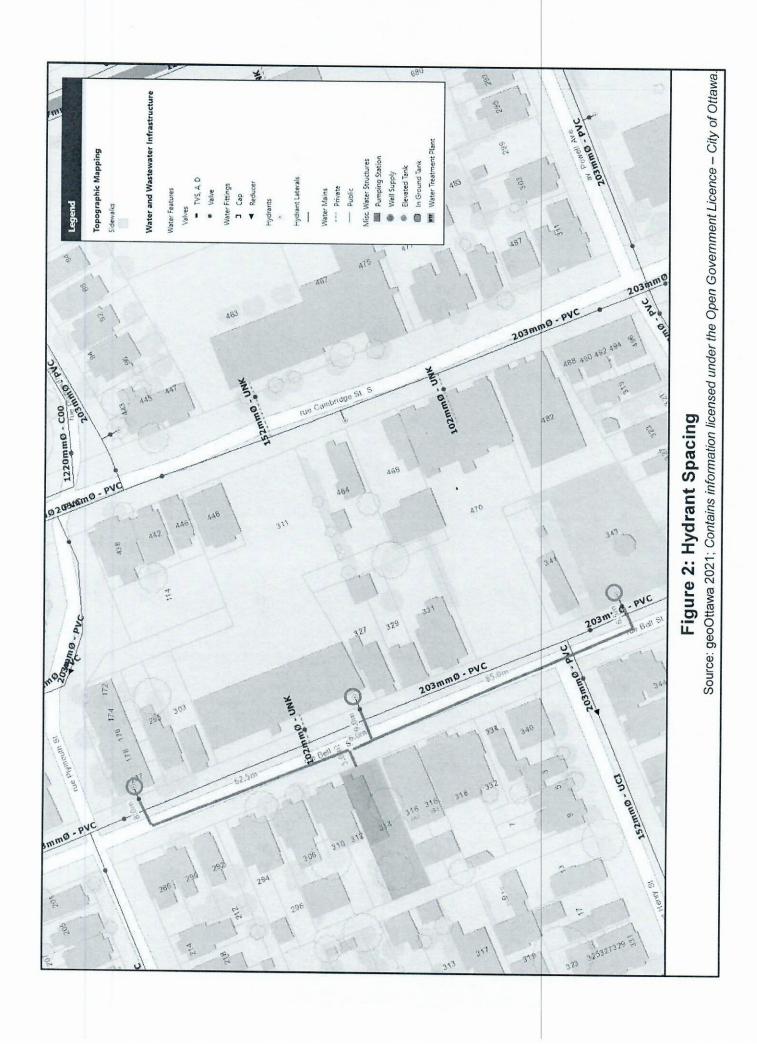
#### To summarize:

Scenario 1: Minimum Pressure under Peak Hour Demand: 321 kPa (47 psi)

Scenario 2: Maximum Pressure under Average Day Demand: 403 kPa (58 psi)

Scenario 3: Minimum Pressure under Maximum Day + Fire Flow Demand: 275 kPa (40 psi)

ATTACHMENT 6: FIGURE 2 – HYDRANT SPACING



# **PROPOSED**

# THREE-STOREY APARTMENT BUILDING ADDITION

**LOT RE-DEVELOPEMENT SITE** 

PART OF LOT "D"

R-PLAN 82717

314 BELL STREET SOUTH

**CITY OF OTTAWA** 

APPENDIX E

CITY OF OTTAWA

SANITARY SEWER DESIGN SHEET

SHEET No. 1 OF 1

. ;	2000.s	Actual velocity at O(d)			-										, e.				SHEET No.	-
•	population in 1000's	full flow velocity (m/s)	1.12																SHE	
	7	PROPOSED SEWER  Type Grade Capacity  of % (L.a)  pipe n=0.03	8.61																	MENT
	$O(p) = \frac{1+\frac{14}{4}}{86.4}  \text{where P}$ $O(p) = \frac{PqM}{86.4}  (L/s)$ $O(1) = 1A  (L/s)  \text{where A}$ $O(d) = O(p) + O(1)  (L/s)$	Grade	1-0 (min)																13	RPARTY
	$0(p) = \frac{1+14}{4+\sqrt{p}}$ $0(p) = \frac{PqM}{86.4}$ $0(1) = 1A (1/4)$ $0(d) = 0(p) +$		BVC			$\parallel$													LSTARE	
۲	M = Q(p)	Pipe size (nm)	202		$\parallel$	$\prod$	$\prod$	$\prod$	$\parallel$	$\parallel$			$\frac{1}{1}$	$\parallel$		4	<del>  </del> -	$\parallel$	+ BEL	3 STOREY
SHEET		Length (m)	2.5			1		#		+						$\frac{1}{1}$	-		17 314	DSEA
		Peak design 110w Q(d) (L/s)	0.13	75.0		$\prod$		$\prod$	$\prod$	$\prod$			4			$\frac{1}{1}$	-		PROJECT	prioros
DESIGN		Peak extraneou (low Q(i) (L/s)	0.02						Ц	$\prod$						4			1	2
		1 Pop. 110w 0(p) (1./s)	0.11	0															TLM	TUE
SEWER		Peaking Factor M	4	.4					.							$\coprod$			DESIGN	CHECKED
		ATIVE Area A (hectares)	50.0			Ì.													DE	CHEC
TARY	z <sup>1</sup>	CUMUI Pop.	\$ <del>1</del> 8	8.4																
SANIT		DUAL Area A hectares	808																	T # 1
	cap, d) ha. s)	Area Pop. hectar	8.4	8.4																1
	i flow (201, 1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	10	Combines Combines Sewert										G)	NEE	~					
	ily per capit k extraneous stor 4.0 (MA) opulation the xtraneous fit esign flow	FROM	SITE MEN BLOG	DX-BUDY							or different	ESSIONA	14/20	I M MAK	(A).		OP OF			
	q = average daily per capita flow (201,1cap, d) I = unit of peak extraneous flow (201,1ha. s) Mapeaking factor A.O (1000) Q (p) = peak population flow (1,1s) Q (i) = peak extraneous flow (1,1s) Q (d) = peak design flow	STREET	STREET									ONO	100 00	TOWN	1	200	O.I.			

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## **PROPOSED**

# THREE-STOREY APARTMENT BUILDING ADDITION

LOT RE-DEVELOPEMENT SITE

PART OF LOT "D"

R-PLAN 82717

314 BELL STREET SOUTH

**CITY OF OTTAWA** 

# **APPENDIX F**

**DEVELOPMENT SERVICING STUDY CHECKLIST SUMMARY** 

# 4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.7	General Content
	Executive Summary (for larger reports only).
$\boxtimes$	Date and revision number of the report.
X	Location map and plan showing municipal address, boundary, and layout of proposed development.
\	Plan showing the site and location of all existing services.
	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.
	Summary of Pre-consultation Meetings with City and other approval agencies.
	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defendable design criteria.
M,	Statement of objectives and servicing criteria.
Ø	Identification of existing and proposed infrastructure available in the immediate area.
	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).

	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.
□.	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.
$\Box$	Proposed phasing of the development, if applicable.
XI.	Reference to geotechnical studies and recommendations concerning servicing.
X	All preliminary and formal site plan submissions should have the following information:
	<ul> <li>Metric scale</li> <li>North arrow (including construction North)</li> <li>Key plan</li> <li>Name and contact information of applicant and property owner</li> <li>Property limits including bearings and dimensions</li> <li>Existing and proposed structures and parking areas</li> <li>Easements, road widening and rights-of-way</li> <li>Adjacent street names</li> </ul>
4.2	Development Servicing Report: Water
	Confirm consistency with Master Servicing Study, if available
$\boxtimes$	Availability of public infrastructure to service proposed development
X	Identification of system constraints
X	Identify boundary conditions
図	Confirmation of adequate domestic supply and pressure
	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.
A	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.
	Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design
-	Address reliability requirements such as appropriate location of shut-off valves  Check on the necessity of a pressure zone boundary modification.

	Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range
<b>区</b>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.
	Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.
<b>Y</b>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.
	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.
4.3	Development Servicing Report: Wastewater
X	Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).
	Confirm consistency with Master Servicing Study and/or justifications for deviations.
	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.
Ø	Description of existing sanitary sewer available for discharge of wastewater from proposed development.
	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)
M	Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.
	Description of proposed sewer network including sewers, pumping stations, and forcemains.

	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation soil cover, as well as protecting against water quantity and quality).
	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.
	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.
	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.
	Special considerations such as contamination, corrosive environment etc.
4.4	Development Servicing Report: Stormwater Checklist
X	Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)
	Analysis of available capacity in existing public infrastructure.
X´	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.
₩.	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.
	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.
A	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.
	Set-back from private sewage disposal systems:
	Watercourse and hazard lands setbacks.
	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.
□ ; 1	Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.

M	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).
	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.
Ä	Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.
	Any proposed diversion of drainage catchment areas from one outlet to another.
Ø	Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.
	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.
	Identification of potential impacts to receiving watercourses
	Identification of municipal drains and related approval requirements.
Ø.	Descriptions of how the conveyance and storage capacity will be achieved for the development.
Ø	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.
	Inclusion of hydraulic analysis including hydraulic grade line elevations.
	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.
	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.
П	Identification of fill constraints related to floodplain and geotechnical investigation.

# 4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, exceptin cases of dams as defined in the Act.
	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.
	Changes to Municipal Drains.
	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)
4.6	Conclusion Checklist
M	Clearly stated conclusions and recommendations
	Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.
X	All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario