STORMWATER MANAGEMENT REPORT

For 3055 Richmond Road, Ottawa

Prepared by:

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Revision 0 July 2022

1. Project Description:

1.1. Introduction:

Property at 3055 Richmond Road is located close to intersection of Dumaurier Avenue and Richmond Road, Ottawa, Ontario. The property is about 0.10 Hectare severed from an existing lot which contain an existing one story building.

Property at 3055 Richmond Road is currently under light residential Zoning. Due to market demand for residential, the idea initiated to use the lot to build four-story dwelling that contains 16 units.

This report will address the servicing (water, sanitary) requirements associated with the proposed development located at 3055 Richmond Road within the City of Ottawa, Ontario. This report is prepared in response to the request from City of Ottawa Planning department.

1.2. Existing Conditions:

The existing site located at 3055 Richmond Road. The property measure a total area of approximately 0.10 Hectare. The site is fronting 406mm diameter CI water main and 225mm diameter Concrete sanitary main on Richmond Road.



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1.3. Guidelines, Previous Studies, And Reports

The following studies were utilized in the preparation of this report:

- Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (City Standards)
 - Technical Bulletin ISTB-2018-01 City of Ottawa, March 21, 2018. (ISTB-2018-01)
 - Technical Bulletin ISTB-2019 Technical Bulletin ISTB-2020 City of Ottawa,
- Ottawa Design Guidelines Water Distribution City of Ottawa, July 2010.
 (Water Supply Guidelines)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 27, 2014. (ISDTB-2014-02)
 - Technical Bulletin ISTB-2018-02 City of Ottawa, March 21, 2018. (ISTB-2018-02)
 - Technical Bulletin ISTB-2019 Technical Bulletin ISTB-2020 City of Ottawa,
- Design Guidelines for Sewage Works, Ministry of the Environment, 2008.
 (MOE Design Guidelines)
- Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (SWMP Design Manual)
- ➤ Geotechnical Investigation

2. Stormwater Management

2.1. Design Criteria and Objectives

Design of the storm sewer system was completed in conformance with the City of Ottawa Design Guidelines (November 2012). Specifically, Section 5 "Storm and Combined Sewer Design" for runoff coefficients and an inlet time were referenced in this design.

The allowable release rate for the site is calculated using a runoff coefficient of 0.40, the 5 year storm event, time of concentration of 10 min and store up to the 100 years storm event. As per direction from City of Ottawa Planning Department.

During all construction activities, erosion and sediment shall be controlled by techniques outlined in Section 5 of this report.

Minor System Design Criteria

- 1. The storm water management has been designed based on the rational formula and the Manning's Equation under free flow conditions for the 2-year storm using a 10-minute inlet time.
- 2. Inflow rates of the minor system are limited to the pre-development rates for up to the 2-year storm, and are based on a time of concentration of 10 minutes.

Major System Design Criteria

- 1. The major system has been designed to accommodate on-site detention with sufficient capacity to attenuate the 100-year design storm. Excess runoff above the 100 year event will flow to Richmond Road.
- 2. On site storage is provided and calculated for up to the 100-year design storm. Calculation of the required on-site storage volumes has been supported by calculations provided in appendixes.
- 3. Calculation of the required storage volumes has been prepared based on the Modified Rational Method as identified in Section 8.3.10.3 of the City's Sewer Guidelines. The depth and extent of surface storage will be illustrated on the applicable grading plan and storm drainage plan.

2.2. Runoff Coefficients

Runoff coefficients used for either pre-development or post-development conditions were based on actual areas measured in CAD. Runoff coefficients for impervious surfaces (roofs, asphalt, and concrete) were taken as 0.90,

The allowable predevelopment runoff coefficients for the overall site is calculated as below

Area ID	Area (ha)	Runoff 'C'	AxC
Existing Dwelling	0.01	0.9	0.009
Grage and Addition	0.01	0.9	0.009
Grass or Landscape	0.08	0.1	0.008
Total Site Area (ha)	0.1		0.026

C(avg) = 0.26

Therefore predevelopment runoff C=0.50 as per Sewer Design Guideline.

Pre-development Stormwater Conditions:

Stormwater runoff from the subject property is tributary to the City of Ottawa sewer system and is located within the Rideau River Conservation Authority. As such, approvals for proposed development within this area are under the approval authority of the City of Ottawa and the Rideau River Conservation Authority for quality control.

The site is currently occupied by an existing dwellings with grass and driveway asphalt. Predevelopment conditions will be considered as directed by the City of Ottawa Infrastructure Department for runoff coefficient of 0.5 and time of concentration of 10 min and store up to the 100 years storm event as per the City of Ottawa Sewer Design Guideline.

The area for runoff coefficients used for either pre-development or post-development conditions were based on actual areas measured in CAD. Runoff coefficients for surfaces such as roofs, were taken as 0.90, for permeable landscape were taken as 0.50. Refer to appendixes for detail

It was assumed that the existing development contained no stormwater management controls for flow attenuation. The estimated combined pre-development peak flows for the 5 storm events are calculated below:

Allowable Release Rate:

- Time of Concentration = 10 minutes,
- Drainage Area = 0.10 ha

$$Q \text{ allow} = 2.78 \text{ C I A}$$

Where:

Q allow = Allowable release rate to storm sewer (L/sec)
C = Runoff Coefficient (dimensionless) =0.50
I = Average Rainfall Intensity for return period (mm/hr)
= 732.951 / (TC + 6.199) 0.810 =76.81 mm/hr (2-year)
TC = Time of concentration (minutes) =10 min
A = Drainage Area (hectares) = 0.10

Q Allow = 10.68 L/sec (5-year)

Therefore the allowable release rate from the site is 10.68 L/sec

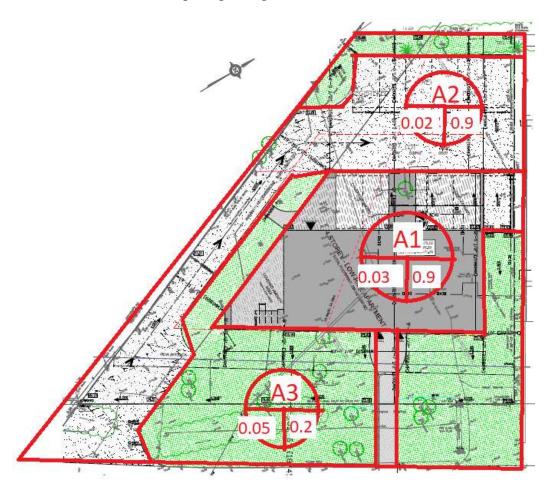
<u>Post-development Stormwater Management Target:</u>

Stormwater management requirements for the proposed development were reviewed with the City of Ottawa, generating the following requirements for the proposed development:

- ➤ Meet a total allowable release rate based on a Rational Method Coefficient of 0.50, employing the City of Ottawa IDF parameters for a 2-year storm with a time of concentration equal to or greater than 10 minutes
- Attenuate all storms up to and including the City of Ottawa 100-year design event on site. Post-development including 100-year storm event shall be controlled to the pre-development 5-year storm event.
- > During all construction activities, erosion and sediment shall be controlled by techniques outlined in section 3 this report

Storm Drain Area:

The post development storm water management design, for this site has been divided to 3 general areas; Roof area, Grass area and asphalt parking area.



Post-development Stormwater Quantity Control:

Post development storm water management design for this site includes 3 general areas; Proposed roof of development, Parking area and Grass or landscape area.

- Grass or landscape area will sheet drain to the municipal street; Richmond Road
- Parking area are able to absorb most of rainfall since green paver is proposed for these areas. Green paver is meant to aid with run-off. For the purpose of being conservative in our stormwater calculation design, C value is considered as 0.9 for parking and garbage area.
- Roof proposed building: C value for roof is considered as 0.9. Storm runoff during 100yrs storm event will be stored on the roof. Overflow runoff beyond 100 year rain fall event would be going towards Richmond Road as per original design of this subdivision.

1	C(max equiv)	I (mm/h)	Area (ha)
	0.5	76.8	0.100
	O(allow)	10.68	I/e

l (mm/h)	l (100yr) mm/h					
76.8	179					
Area ID	Area (ha)	C (5yr)	AxC	C (100yr) (Max of 1.0)	AxC	Type of Flow (Controlled/Uncontrolled)
A1: Proposed Building	0.030	0.9	0.03	1.00	0.03	
A2: Parking area	0.020	0.9	0.02	1.00	0.02	
A3: Grass area	0.050	0.2	0.01	0.25	0.01	
Total Site Area (ha)	0.1	100	0.06	=-	0.06	Total

C(avg) 5-year = 0.55 C(avg) 100-year = 0.63

As mentioned, post development storm water management design for this site includes 3 general areas; Proposed roof of building, Parking area and Grass or landscape area.

- Grass or landscape area will sheet drain to front of the property as per natural drainage pattern. During 2 year and 100 year storm event, grass area generates 2.14 L/sec and 2.67 L/sec respectively.
- Parking area will sheet drain to rear of the property as per natural drainage pattern. the release rate during 2yr and 100yr storm event are 3.84 L/sec and 4.27 L/sec, respectively.
- Proposed roof of building: Storm runoff during 2yrs and 100yrs storm event will be stored on the roof. In order to ensure that the allowable release rate to the storm sewers is not exceeded, roof drain restrictors will be installed at the roof drains by limiting the rate at which storm runoff is release to the sewers, water will tend to pond upstream of the roof drain. As ponds generally form the shape of the roof, the extend and depth of ponding resulting from the 100-year storm was determined using the following equation;

 $V=1/3 \times A \times d$

Where:

V = Storage volume (cu. m.) A = surface area of pond (sq.m.) D = pond depth at peak (m)

The pre-development allowable release is 10.68 L/sec, with presence of 100yr uncontrolled release of 2.14L/sec for landscape area and 3.84L/sec for parking area, the roof will be controlled to 4.70 L/sec.

The flow control will be done based on one roof drains at 4.70 L/sec based on the 2-year storm event.

Based on calculation, the maximum volume required for the roof at post development stage for 100yrs storm event would be 20.28m³. Considering the roof area of 298m², the maximum ponding height on the roof will come up to be 200mm. The discharge rate from above connected structure will be controlled via 4 roof drain equipped with ICD's which is selected based on design head and available manufacturer database, (see appendix) Watts RD100 (fully exposed) roof drains with adjustable flow control weirs is selected.

3. Foundation/Footing Drain

Foundation drain is independently connected to storm main on Stewart Street. Please refer to Grading and Drainage plan.

4. Erosion and Sediment Control

Following methods will be unutilized to control erosion and sediment:

- Silt fence will be installed around the perimeter of the site and will be cleaned and maintained throughout construction. Silt fence will remain in place until the working areas have been stabilized and re-vegetated.
- Catch basins will have SILTSACKs or an approved equivalent installed under the grate during construction to protect from silt entering the storm sewer system.
- A mud mat will be installed at the construction access in order to prevent mud tracking onto adjacent roads.
- Erosion and sediment controls must be in place during construction. The following recommendations to the contractor will be included in contract documents:
 - O Limit extent of exposed soils at any given time;
 - o Re-vegetate exposed areas as soon as possible;
 - o Minimize the area to be cleared and grubbed;
 - o Protect exposed slopes with plastic or synthetic mulches;
 - o Install silt fence to prevent sediment from entering existing ditches;
 - o No refueling or cleaning of equipment near existing watercourses;
 - o Provide sediment traps and basins during dewatering;

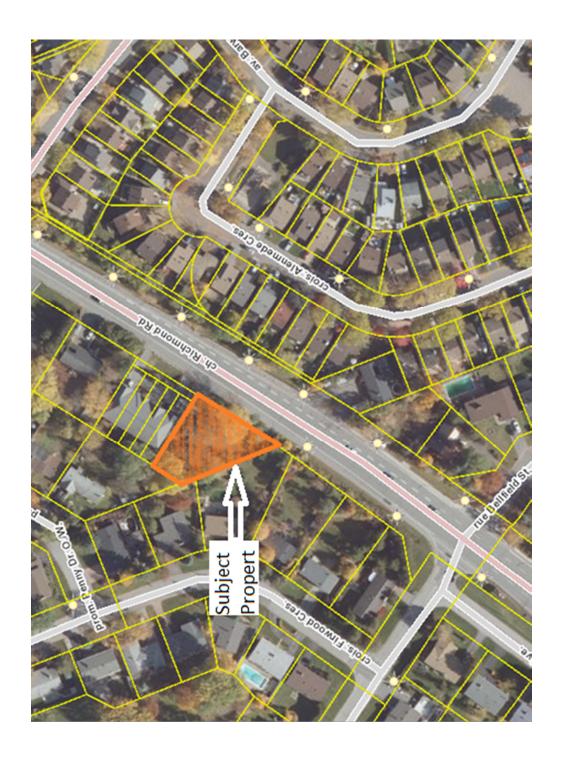
- o Install filter cloth between catch basins and frames;
- O Plan construction at proper time to avoid flooding;
- Establish material stockpiles away from watercourses, so that barriers and filters may be installed.
- The contractor will, at every rainfall, complete inspections and guarantee proper performance. The inspection is to include:
 - O Verification that water is not flowing under silt barriers;
 - O Clean and change filter cloth at catch basins.
- Construction and maintenance requirements for erosion and sediment controls to comply with Ontario Provincial Standard Specification OPSS 577, and City of Ottawa specifications.
- A visual inspection shall be completed daily on sediment control barriers and any damage repaired immediately. Care will be taken to prevent damage during construction operations.

Should you have any questions or comments, please feel free to contact undersigned.

Yours fruly Wissam Elias

P. Eng

Stormwater Management Report			
APPENDIX A:			
GeoOttawa Map			



Stormwater Management Report
APPENDIX B:

Storm water Management Calculations

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C(5 gr)	C(100 grl)	Area (ha)
0.55	0.63	0.100

Q(restricted) I/s = 4.70 ← enter restricted release rate

t(c)min	I (mm/h)	Q(unrestricted) l/s	Q(restricted) l/s	Q(stored) l/s	V(stored) m ^a
5	103.6	15.84	4.70	11.14	3.34
10	76.8	11.74	4.70	7.05	4.23
15	61.8	9.44	4.70	4.75	4.27
20	52.0	7.96	4.70	3.26	3.91
25	45.2	6.91	4.70	2.21	3.31
30	40.0	6.12	4.70	1.43	2.57
35	36.1	5.51	4.70	0.82	1.71
40	32.9	5.02	4.70	0.33	0.79
45	30.2	4.62	4.70	-0.07	-0.20
50	28.0	4.29	4.70	-0.41	-1.23
55	26.2	4.00	4.70	-0.70	-2.30
60	24.6	3.75	4.70	-0.94	-3.39
65	23.2	3.54	4.70	-1.16	-4.51
70	21.9	3.35	4.70	-1.35	-5.66
75	20.8	3.18	4.70	-1.52	-6.82
80	19.8	3.03	4.70	-1.67	-7.99
85	18.9	2.90	4.70	-1.80	-9.18
90	18.1	2.77	4.70	-1.92	-10.39
95	17.4	2.66	4.70	-2.03	-11.60
100	16.7	2.56	4.70	-2.14	-12.82
105	16.1	2.47	4.70	-2.23	-14.05
110	15.6	2.38	4.70	-2.32	-15.29

Maz Vol stored

4.27

STORAGE TABLE (100 Yr Storm)

t(c)min	l(100yr) mm/h	Q(actual) l/s	Q(restricted) l/s	Q(stored) l/s	V(stored) m ^a
5	242.7	42.2	4.7	37.5	11.24
10	178.6	31.0	4.7	26.3	15.80
15	142.9	24.8	4.7	20.1	18.12
20	120.0	20.8	4.7	16.1	19.37
25	103.8	18.0	4.7	13.3	20.02
30	91.9	16.0	4.7	11.3	20.28
35	82.6	14.3	4.7	9.7	20.27
40	75.1	13.1	4.7	8.4	20.06
45	69.1	12.0	4.7	7.3	19.71
50	64.0	11.1	4.7	6.4	19.24
55	59.6	10.4	4.7	5.7	18.69
60	55.9	9.7	4.7	5.0	18.05
65	52.6	9.1	4.7	4.4	17.35
70	49.8	8.7	4.7	4.0	16.60
75	47.3	8.2	4.7	3.5	15.81
80	45.0	7.8	4.7	3.1	14.97
85	43.0	7,5	4.7	2.8	14.11
90	41.1	7.1	4.7	2.4	13.21
95	39.4	6.9	4.7	2.2	12.28
100	37.9	6.6	4.7	1.9	11.33
105	36.5	6.3	4.7	1.6	10.36
110	35.2	6.1	4.7	1.4	9.37

Max Vol stored 20.28

Stormwater Management Report			
APPENDIX C:			
Engineering Data Sheet			



RD-100-O

Tag:

Combined Roof Drain & Secondary Overflow System

Components:





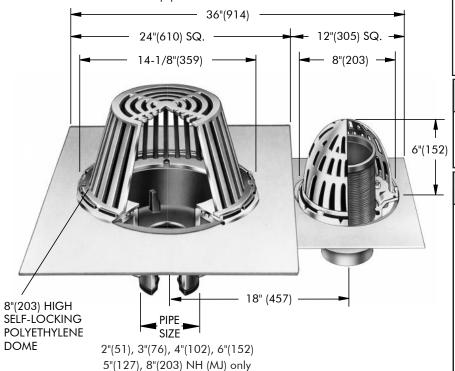






Order Code: RD-10 -O- -

SPECIFICATION: Watts Drainage Products RD-100-O combined roof drain and secondary overflow system, consisting of galvanized sump receivers; one large sump epoxy coated cast iron body, one small sump epoxy coated cast iron body, combined flashing rings and gravel stop, polyethyene dome strainers and one adjustable 4"(102) diameter ABS overflow standpipe.



Deck opening 10" (254) with sump receiver 13-1/4" (337)

> Free Area Sq. In.

Deck opening 6-1/2"(165) with sump receiver 8"(203)

Ex. RD-102P-O-K

P	ipe Sizing (Select C	ne)
Suffix	Description	
2	2"(51) Pipe Size	
3	3"(76) Pipe Size	
4	4"(102) Pipe Size	
5	5"(127) Pipe Size	
6	6"(152) Pipe Size	
8	8"(203) Pipe Size	

Outlet Type (Select One) Suffix Description

NH	No Hub (MJ)	
Р	Push On	
Т	Threaded Outlet	
Χ	Inside Caulk	

Options (Select One or More) Suffix Description

	-	
-A	Accutrol weir (specify # 1-6 slots)	
-C	Secondary Membrane Clamp	
-D	Underdeck Clamp	
-E	Adjustable Extension	
-GSS	Stainless Steel Ballast Guard	Г

-H	Adj. to 6" IRMA Ballast Guard
-K	Ductile Iron Dome

-1000	Aldininoin Donie
-L	Vandal Proof Dome
-R	2" High External Water Dam

-SO Side Outlet**

-V	Fixed Extension (1-1/2",2",3",4")
-W-1	Waterproofing Flange

-Z	Extended Integral Wide Flange	
-5	Sediment Bucket	Γ
		- 7

-5	Sedimeni Docker	
-12	Galvanized Dome	
-13	All Galvanized	
-83	Mesh Covered Dome	

-113M Special Epoxy from 3M Range

Optional Body Material (NH Only)			
Suffix	Description		
-60	PVC Body w/Socket Outlet		
4.1	A DC D = -1/C = -11 O11-1		

** Side Outlet (-SO) option only available in 2"(51), 3"(76), 4"(102) pipe sizes.

Underdeck Clamp (-BED and -D options) are not available when -SO is selected.

Job Name	Contractor
Job Location	Contractor's P.O. No.
Engineer	Representative

WATTS Drainage reserves the right to modify or change product design or construction without prior notice and without incurring any obligation to make similar changes and modifications to products previously or subsequently sold. See your WATTS Drainage representative for any clarification. Dimensions are subject to manufacturing tolerances.



CANADA: 5435 North Service Road, Burlington, ON, L7L 5H7 TEL: 905-332-6718 TOLL-FREE: 1-888-208-8927 Website: www.wattsdrainage.ca



Adjustable Accutrol	Weir
Tag:	

Adjustable Flow Control for Roof Drains

ADJUSTABLE ACCUTROL(for Large Sump Roof Drains only)

For more flexibility in controlling flow with heads deeper than 2", Watts Drainage offers the Adjustable Accutrol. The Adjustable Accutrol Weir is designed with a single parabolic opening that can be covered to restrict flow above 2" of head to less than 5 gpm per inch, up to 6" of head. To adjust the flow rate for depths over 2" of head, set the slot in the adjustable upper cone according to the flow rate required. Refer to Table 1 below. Note: Flow rates are directly proportional to the amount of weir opening that is exposed.

EXAMPLE:

For example, if the adjustable upper cone is set to cover 1/2 of the weir opening, flow rates above 2" of head will be restricted to 2-1/2 gpm per inch of head.

Therefore, at 3" of head, the flow rate through the Accutrol Weir that has 1/2 the slot exposed will be: $[5 \text{ gpm}(\text{per inch of head}) \times 2 \text{ inches of head}] + 2-1/2 \text{ gpm}(\text{for the third inch of head}) = 12-1/2 \text{ gpm}.$

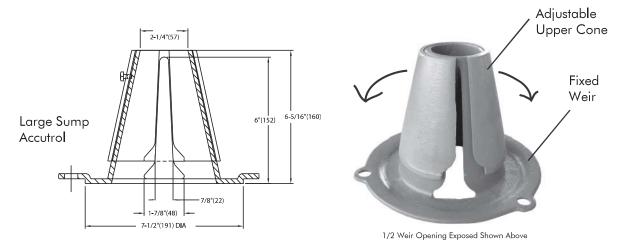


TABLE 1. Adjustable Accutrol Flow Rate Settings

			Head of Wat	er		
Weir Opening	1"	2"	3"	4"	5"	6"
Exposed	Flow Rate (gallons per minute)					
Fully Exposed	5	10	15	20	25	30
3/4	5	10	13.75	17.5	21.25	25
1/2	5	10	12.5	15	17.5	20
1/4	5	10	11.25	12.5	13.75	15
Closed	5	10	10	10	10	10

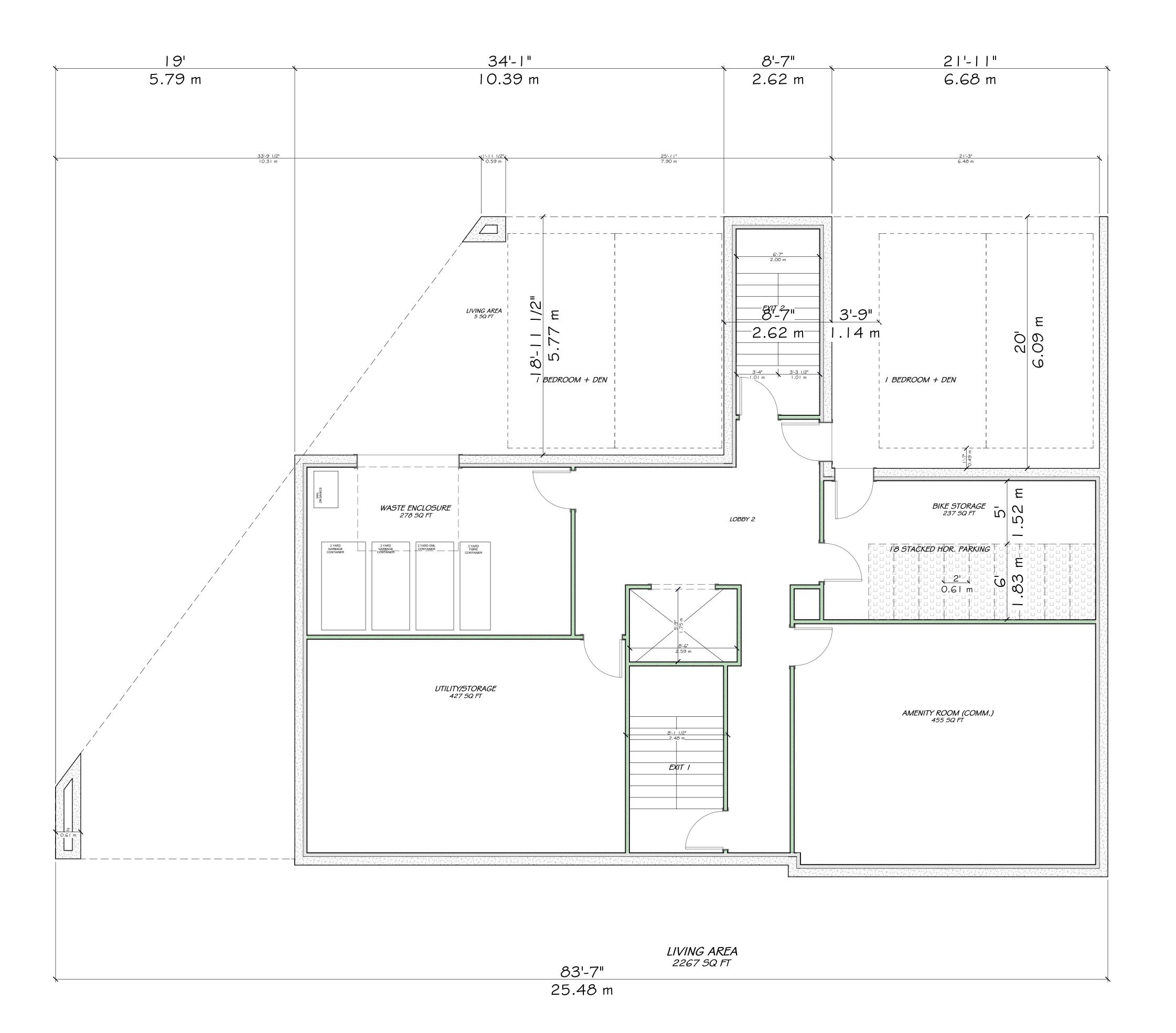
Job Name	Contractor
Job Location	Contractor's P.O. No.
Engineer	Representative

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UNPOISED ARCHITECTURE INC. 5-16 SWEETLAND AVE. OTTAWA, ON. K1N-7T6

AZUL DESIGNS OTTAWA, ON. K1H-7G2

The undersigned has reviewed and takes responsibility for this design, and has the qualifications and meets the requirements set out in the Ontario Building Code to be a designer.

RESPONSIBILITIES: OO NOT SCALE DRAWINGS

ALL DESIGN AND CONSTRUCTION TO BE IN ACCORDANCE WITH THE ONTARIO BUILDING CODE 2006

IT IS THE RESPONSIBILITY OF THE APPROPRIA' CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS ON SITE AND REPORT ALL ERROF AND OMISSIONS TO THE ARCHITECT/DESIGNEI OPYRIGHT RESERVED

GENERAL NOTES:

3055 RICHMOND ROAD SCOPE OF WORK: NEW 4 STOREY LOW RIS RENTAL BUILDING - 16 UNITS

CONSULTANTS:

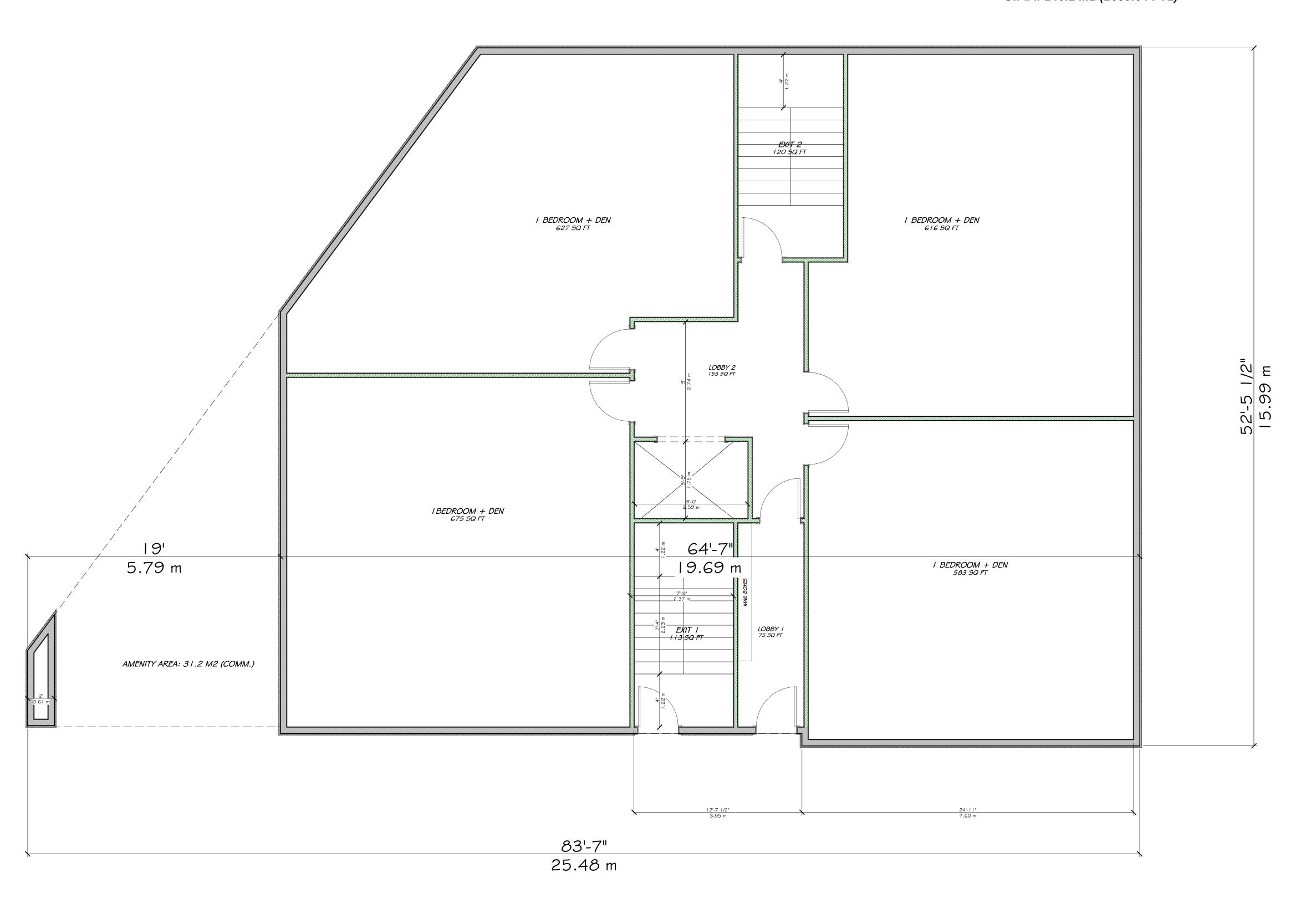
STRUCTURAL - TBD
MECHANICAL - TBD
ELECTRICAL - TBD

REVISED SITE PLAN REVISED SITE PLAN REVISION/ISSUE T: 3055 RICHMOND RD. 3055 RICHMOND RD. OTTAWA, ON K2B 6S6 613-000-0

FLOOR PLANS DATE: APRIL 12, 2022
SCALE: AS NOTED

1 BASEMENT PLAN A2 SCALE 1/4" = 1'-0"

TOTAL 1ST FLOOR AREA: 297.6 M2 (3203.28 FT2) G.F.A: 245.2 M2 (2639.54 FT2)



LIVING AREA 3204 SQ FT

2 FIRST FLOOR PLAN
A3 SCALE 1/4" = 1'-0"

UNPOISED ARCHITECTURE INC. 5-16 SWEETLAND AVE. OTTAWA, ON. K1N-7T6

AZUL DESIGNS OTTAWA, ON. K1H-7G2

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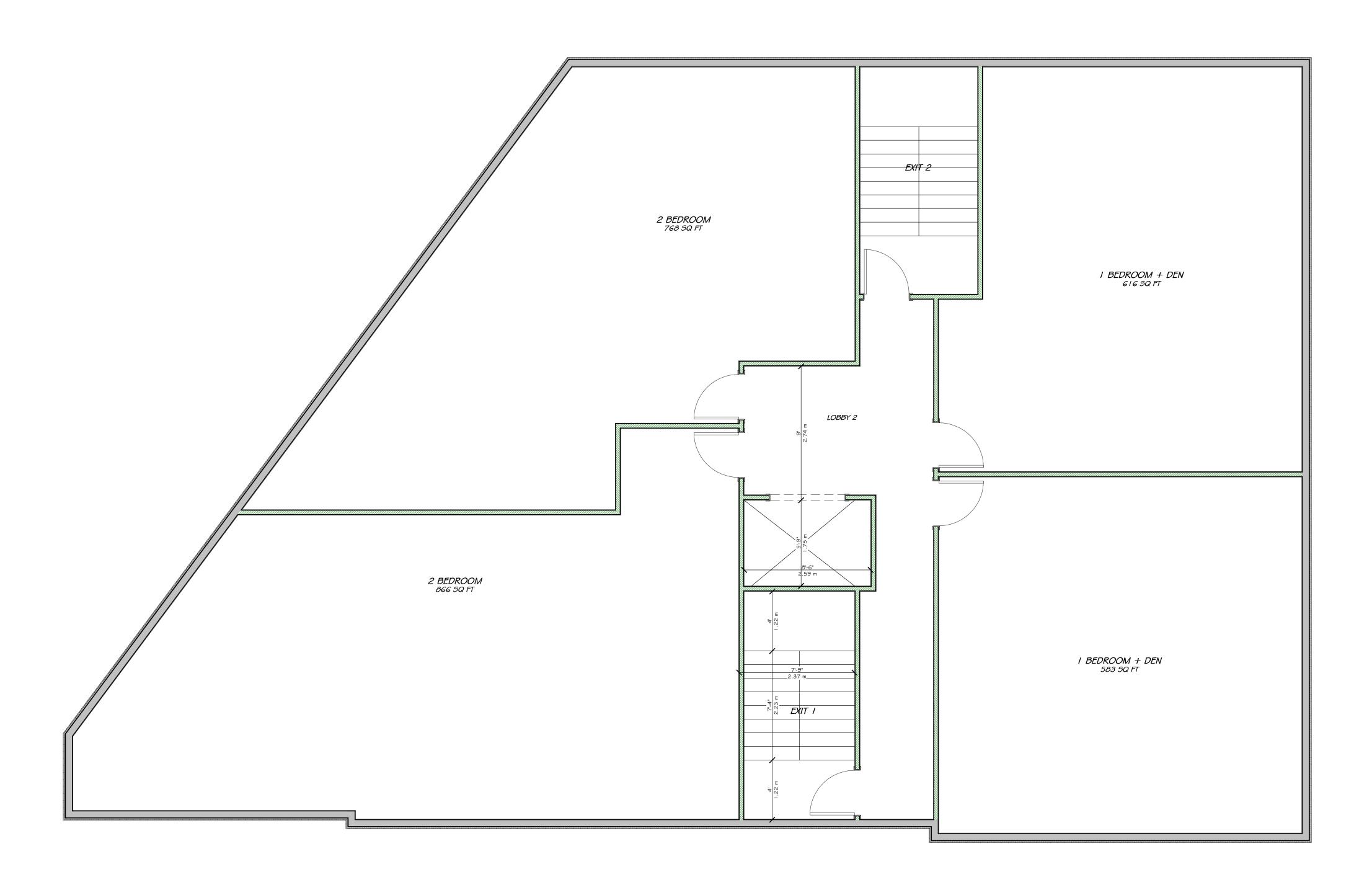
CONSULTANTS:
STRUCTURAL - TBD
MECHANICAL - TBD
ELECTRICAL - TBD

REVISED SITE PLAN REVISED SITE PLAN PRELIMINARIES NO. REVISION/ISSUE OJECT: 3055 RICHMOND RD.

3055 RICHMOND RD. OTTAWA, ON K2B 6S6 613-000-0

FLOOR PLANS

DRAWN BY: --DATE: APRIL 12, 2022
SCALE: AS NOTED



LIVING AREA 3553 SQ FT

UNPOISED ARCHITECTURE INC. 5-16 SWEETLAND AVE. OTTAWA, ON. K1N-7T6

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CONSULTANTS:
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MECHANICAL - TBD
ELECTRICAL - TBD

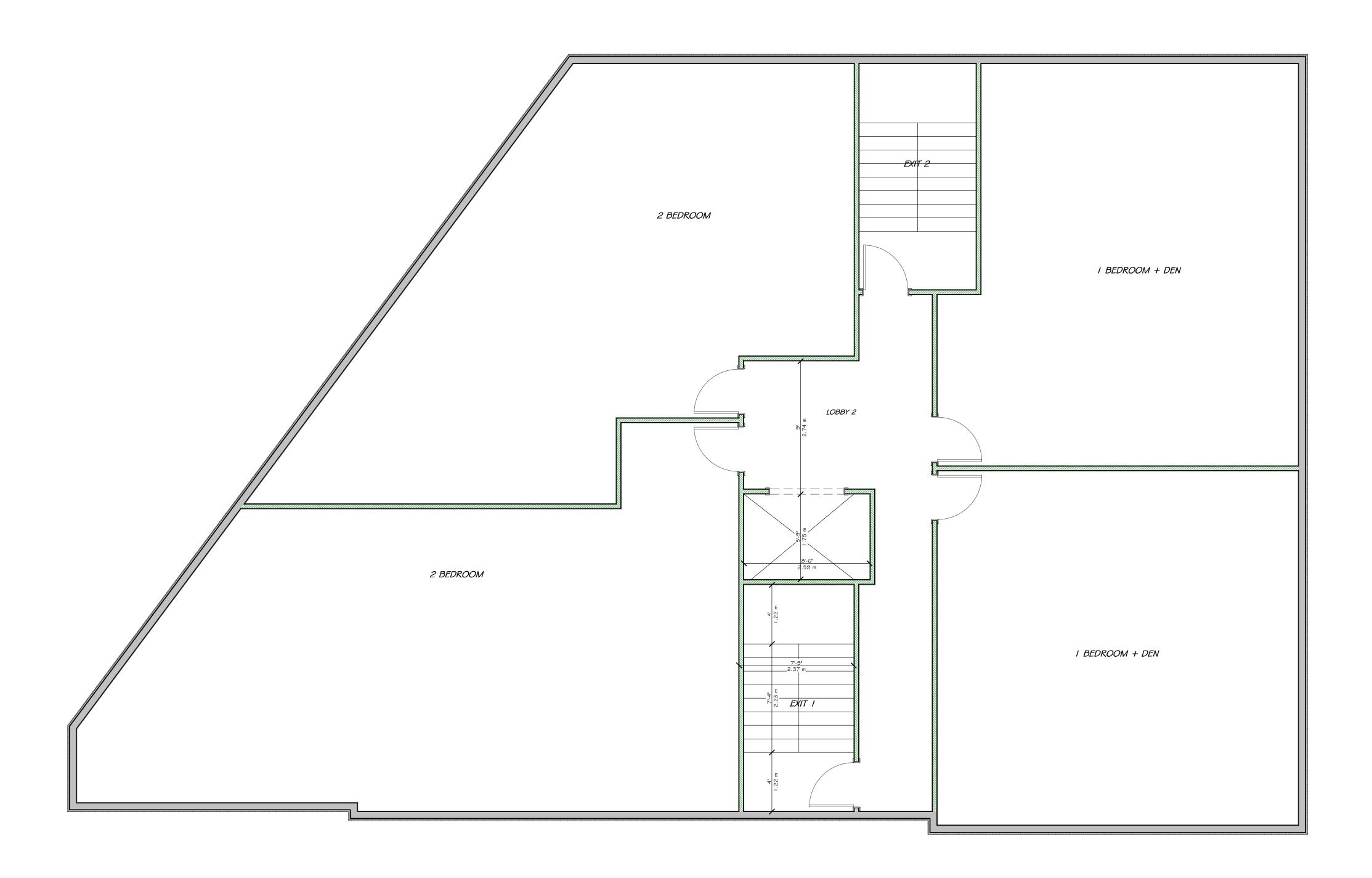
REVISED SITE PLAN REVISED SITE PLAN
PRELIMINARIES NO. REVISION/ISSUE ROJECT: 3055 RICHMOND RD.

3055 RICHMOND RD. OTTAWA, ON K2B 6S6 613-000-01

PLANS

DATE: APRIL 12, 2022
SCALE: AS NOTED

1 SECOND FLOOR PLAN
A4 SCALE 1/4" = 1'-0"



LIVING AREA 3553 SQ FT

UNPOISED ARCHITECTURE INC. 5-16 SWEETLAND AVE. OTTAWA, ON. K1N-7T6

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MECHANICAL - TBD
ELECTRICAL - TBD

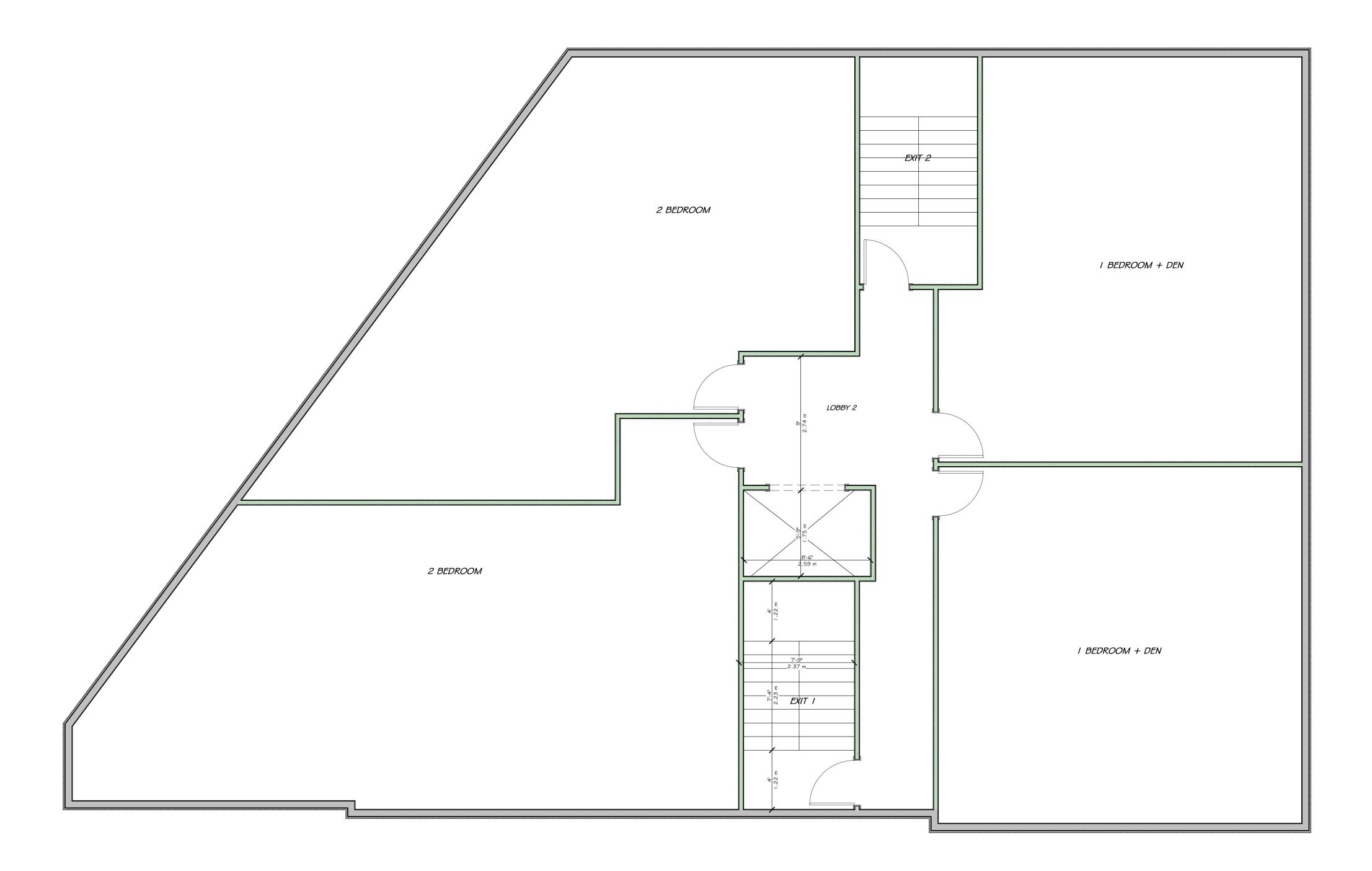
REVISED SITE PLAN REVISED SITE PLAN
PRELIMINARIES NO. REVISION/ISSUE PROJECT: 3055 RICHMOND RD.

3055 RICHMOND RD. OTTAWA, ON K2B 6S6 613-000-01

ELEVATIONS

DRAWN
BY: --
DATE: APRIL 12, 2022

SCALE: AS NOTED



LIVING AREA 3553 SQ FT

1 FOURTH FLOOR PLAN
A6 SCALE 1/4" = 1'-0"

UNPOISED ARCHITECTURE INC. 5-16 SWEETLAND AVE. OTTAWA, ON. K1N-7T6

AZUL DESIGNS OTTAWA, ON. K1H-7G2

The undersigned has reviewed and takes responsibility for this design, and has the qualifications and meets the requirements set out in the Ontario Building Code to be a designer.

RESPONSIBILITIES: DO NOT SCALE DRAWINGS

ALL DESIGN AND CONSTRUCTION TO BE IN ACCORDANCE WITH THE ONTARIO BUILDING CODE 2006 ALL CONTRACTORS MUST WORK IN ACCORDANCE WITH ALL LAWS, REGULATIONS AND BYLAWS HAVING JURISDICTION

IT IS THE RESPONSIBILITY OF THE APPROPRIAT CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS ON SITE AND REPORT ALL ERROR: AND OMISSIONS TO THE ARCHITECT/DESIGNER COPYRIGHT RESERVED GENERAL NOTES:

3055 RICHMOND ROAD SCOPE OF WORK: NEW 4 STOREY LOW RIS RENTAL BUILDING - 16 UNITS

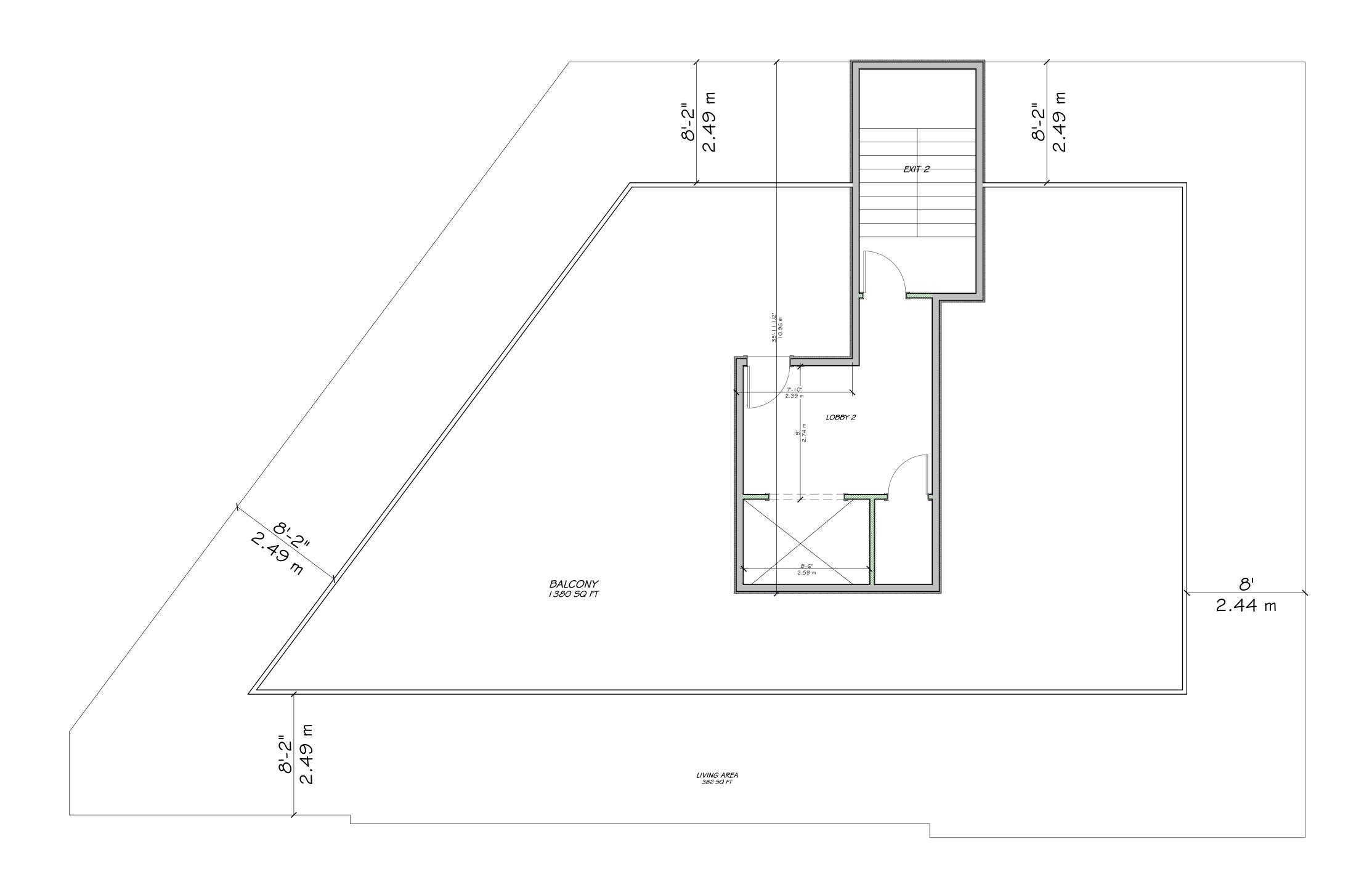
CONSULTANTS:
STRUCTURAL - TBD
MECHANICAL - TBD
ELECTRICAL - TBD

REVISED SITE PLAN REVISED SITE PLAN
PRELIMINARIES NO. REVISION/ISSUE PROJECT: 3055 RICHMOND RD.

3055 RICHMOND RD. OTTAWA, ON K2B 6S6 613-000-0

DETAILS & SECTIONS

DRAWN
BY: --DATE: APRIL 12, 2022
SCALE: AS NOTED
SHEET:
A6



LIVINGARREA

UNPOISED ARCHITECTURE INC. 5-16 SWEETLAND AVE. OTTAWA, ON. K1N-7T6

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GENERAL NOTES:

3055 RICHMOND ROAD SCOPE OF WORK: NEW 4 STOREY LOW RISE RENTAL BUILDING - 16 UNITS

CONSULTANTS:
STRUCTURAL - TBD
MECHANICAL - TBD
ELECTRICAL - TBD

4 REVISED SITE PLAN
3 REVISED SITE PLAN REVISED SITE PLAN
PRELIMINARIES NO. REVISION/ISSUE ROJECT: 3055 RICHMOND RD.

3055 RICHMOND RD. OTTAWA, ON K2B 6S6 613-000-00

FLOOR PLANS

DRAWN
BY: --
DATE: APRIL 12, 2022

SCALE: AS NOTED

1 ROOF PLAN A7 SCALE 1/4" = 1'-0"

