



Engineers, Planners & Landscape Architects

Engineering

- Land/Site Development
- Municipal Infrastructure
- Environmental/Water Resources
- Traffic/Transportation
- Recreational

Planning

- Land/Site Development
- Planning Application Management
- Municipal Planning
- Urban Design
- Expert Witness (LPAT)
- Wireless Industry

Landscape Architecture

- Streetscapes & Public Amenities
- Open Space, Parks & Recreation
- Community & Residential
- Commercial & Institutional
- Environmental Restoration

Proposed Residential High-Rise 829 Carling Avenue, Ottawa Transportation Impact Assessment



Engineering excellence.

Planning progress.

Liveable landscapes.

**Proposed Residential High-Rise
829 Carling Avenue
Transportation Impact Assessment**

Prepared By:

NOVATECH
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
K2M 1P6

May 2021

Novatech File: 121008
Ref: R-2021-015

May 11, 2021

City of Ottawa
Planning and Growth Management Department
110 Laurier Ave. W., 4th Floor,
Ottawa, Ontario K1P 1J1

Attention: Mr. Mike Giampa
Project Manager, Infrastructure Approvals

Dear Mr. Giampa:

Reference: 829 Carling Avenue
Transportation Impact Assessment
Novatech File No. 121008

We are pleased to submit the following Transportation Impact Assessment, in support of Official Plan Amendment, Zoning By-Law Amendment, and Site Plan Control applications at 829 Carling Avenue. The structure and format of this report is in accordance with the City of Ottawa Transportation Impact Assessment Guidelines (June 2017).

If you have any questions or comments regarding this report, please feel free to contact Brad Byvelds, or the undersigned.

Yours truly,

NOVATECH



Joshua Audia, B.Sc.
E.I.T. | Transportation/Traffic



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check ☒ appropriate field(s)] is either transportation engineering ☒ or transportation planning ☐.

1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Dated at Ottawa this 11th day of May, 2021.
(City)

Name: Brad Byvelds, P.Eng.
(Please Print)

Professional Title: Project Coordinator, Transportation/Traffic

B. Byvelds

Signature of Individual certifier that s/he meets the above four criteria

Office Contact Information (Please Print)	
Address:	240 Michael Cowpland Drive, Suite 200
City / Postal Code:	Ottawa, ON, K2M 1P6
Telephone / Extension:	613-254-9643 x 286
E-Mail Address:	b.byvelds@novatech-eng.com

TABLE OF CONTENTS

EXECUTIVE SUMMARY	I
1.0 SCREENING	1
1.1 INTRODUCTION	1
1.2 PROPOSED DEVELOPMENT	1
1.3 SCREENING FORM	1
2.0 SCOPING.....	2
2.1 EXISTING CONDITIONS	2
2.1.1 Roadways.....	2
2.1.2 Intersections	4
2.1.3 Driveways.....	7
2.1.4 Pedestrian and Cycling Facilities	8
2.1.5 Transit	8
2.1.6 Area Traffic Management	10
2.1.7 Existing Traffic Volumes	10
2.1.8 Collision Records.....	11
2.2 PLANNED CONDITIONS	15
2.2.1 Planned Roadway and Transit Projects	15
2.2.2 Other Area Developments	15
2.3 STUDY AREA AND TIME PERIODS	18
2.4 EXEMPTIONS REVIEW.....	19
3.0 FORECASTING.....	20
3.1 DEVELOPMENT-GENERATED TRAVEL DEMAND.....	20
3.1.1 Trip Generation.....	20
3.1.2 Trip Distribution.....	24
3.1.3 Trip Assignment.....	25
3.2 BACKGROUND TRAFFIC	26
3.2.1 Other Area Developments	26
3.2.2 General Background Growth Rate	29
3.2.3 Existing Traffic Volume Balancing.....	29
3.3 FUTURE TRAFFIC CONDITIONS.....	30
3.4 DEMAND RATIONALIZATION.....	30
3.4.1 Existing Intersection Operations	30
3.4.2 2028 Background Intersection Operations	42
3.4.3 2033 Background Intersection Operations	45
4.0 ANALYSIS	48
4.1 DEVELOPMENT DESIGN	48
4.1.1 Design for Sustainable Modes	48
4.1.2 Circulation and Access	49
4.2 PARKING	49
4.3 BOUNDARY STREETS	49
4.4 ACCESS DESIGN	51
4.5 TRANSPORTATION DEMAND MANAGEMENT.....	52
4.5.1 Context for TDM	52
4.5.2 Need and Opportunity.....	52
4.5.3 TDM Program	55
4.6 NEIGHBOURHOOD TRAFFIC MANAGEMENT.....	55

4.7	TRANSIT	55
4.8	INTERSECTION DESIGN.....	56
4.8.1	Intersection MMLOS Review.....	56
4.8.2	2028 Total Intersection Operations	61
4.8.3	2033 Total Intersection Operations	62
5.0	CONCLUSIONS AND RECOMMENDATIONS	64

Tables

Table 1:	Reported Collisions	11
Table 2:	TIA Exemptions.....	19
Table 3:	Mode Shares for Commercial and Residential Uses.....	21
Table 4:	Existing Development – Trip Generation	21
Table 5:	Existing Development – Trips by Mode Share	21
Table 6:	Existing Development – Primary and Pass-by Trips	22
Table 7:	Proposed Retail – Trip Generation	22
Table 8:	Proposed Retail – Trips by Mode Share	22
Table 9:	Proposed Residential – Peak Period Trip Generation.....	23
Table 10:	Proposed Residential – Peak Period Trips by Mode Share	23
Table 11:	Proposed Residential – Peak Hour Trips by Mode Share	23
Table 12:	Trip Generation Summary	24
Table 13:	Ottawa Civic Hospital Expansion – Trip Generation	28
Table 14:	Existing Traffic Operations	41
Table 15:	Existing Queues	41
Table 16:	2028 Background Traffic Operations	43
Table 17:	2028 Background Queues.....	43
Table 18:	2033 Background Traffic Operations	45
Table 19:	2033 Background Queues.....	46
Table 20:	Parking Requirements.....	49
Table 21:	Segment MMLOS Summary.....	50
Table 22:	Intersection MMLOS Summary.....	56
Table 23:	2028 Total Traffic Operations	61
Table 24:	2028 Total Queues.....	62
Table 25:	2033 Total Traffic Operations	62
Table 26:	2033 Total Queues.....	63

Figures

Figure 1: View of the Subject Site	2
Figure 2: OC Transpo Bus Stop Locations	9
Figure 3: Existing Network Traffic Volumes	12
Figure 4: Carling Avenue Transit Priority Measures – Functional Design	16
Figure 5: LRT Phase 2	17
Figure 6: Balanced Existing Traffic Volumes	31
Figure 7: Existing Site-Generated Traffic Volumes	32
Figure 8: Proposed Site-Generated Traffic Volumes	33
Figure 9: Net Site-Generated Traffic Volumes	34
Figure 10: 2028 Other Area Development-Generated Traffic Volumes	35
Figure 11: 2033 Other Area Development-Generated Traffic Volumes	36
Figure 12: 2028 Background Traffic Volumes	37
Figure 13: 2033 Background Traffic Volumes	38
Figure 14: 2028 Total Traffic Volumes	39
Figure 15: 2033 Total Traffic Volumes	40
Figure 16: Inbound Turning Movements	53
Figure 17: Outbound Turning Movements	54
Figure 18: Desirable Cycling Facility Pre-Selection Nomograph	60

Appendices

Appendix A:	Proposed Site Plan
Appendix B:	TIA Screening Form
Appendix C:	OC Transpo Route Maps
Appendix D:	Traffic Count Data
Appendix E:	Collision Records
Appendix F:	Relevant Excerpts of <i>TRANS Trip Generation Manual</i> (WSP, 2020)
Appendix G:	Other Area Developments
Appendix H:	Strategic Long-Range Model and Intersection Growth Rate Figures
Appendix I:	Signal Timing Plans
Appendix J:	Existing Synchro Analysis
Appendix K:	Background Synchro Analysis
Appendix L:	Transportation Demand Management
Appendix M:	MMLOS Review
Appendix N:	Total Synchro Analysis

EXECUTIVE SUMMARY

This Transportation Impact Assessment (TIA) has been prepared for the property located at 829 Carling Avenue, in support of Official Plan Amendment, Zoning By-Law Amendment, and Site Plan Control applications. The subject site is approximately 0.38 acres in size and is currently occupied by a CIBC banking centre. The subject site is currently served by one driveway to Carling Avenue, and one driveway to Sidney Street. Both driveways are approximately 30m west of Preston Street.

The subject site is designated as 'Mixed Use Centre' on Schedule B of the City of Ottawa's Official Plan. The implemented zoning for the property is 'Arterial Mainstreet' (AM1), and the site is within the Preston-Carling District Secondary Plan.

The proposed development consists of a single 61-storey high-rise residential building with 459 dwellings and approximately 2,792 ft² GFA of ground-floor retail. A total of 386 parking spaces will be provided in six levels of underground parking and seven levels of above ground parking. Access to the parking garage will be provided through two one-way accesses separated by a 2m median along Sidney Street. The development will be constructed in a single phase, with a buildout year of 2028.

The study area for this report includes the boundary roadways Carling Avenue, Preston Street, and Sidney Street, and the intersections of Carling Avenue/Sherwood Drive, Carling Avenue/Champagne Avenue, Carling Avenue/Trillium Pathway, Carling Avenue/Preston Street, Carling Avenue/Booth Street, Preston Street/Beech Street, Preston Street/Pamilla Street, Preston Street/Adeline Street, Preston Street/Sidney Street, and Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway. The selected time periods for the analysis are the weekday AM and PM peak hours, as they represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2028 build-out year and 2033 horizon year.

The conclusions and recommendations of this TIA can be summarized as follows:

Forecasting

- The proposed development is anticipated to generate an additional 152 person trips during the AM peak hour (including 19 vehicle trips), and an additional 101 person trips during the PM peak hour (including 13 vehicle trips).

Development Design

- Concrete sidewalks will be provided around the north, south, and east sides of the proposed building, and will connect to existing sidewalks on Sidney Street, Preston Street, and Carling Avenue. Fifteen bicycle parking spaces will be provided in three outside parking areas, as well as 226 bicycle parking spaces within different levels of the parking garage.
- OC Transpo stops #2397, #6655, #6657, #7369, #8013, #8014, #8023, and the Carling O-Train Station are within 400m walking distance of all entrances to the proposed development.
- All required TDM-supportive design and infrastructure measures in the TDM checklist are met.

Parking

- The proposed development includes 386 vehicle parking spaces, meeting the minimum number of required parking spaces and maximum number of permitted parking spaces, as outlined in the City's *Zoning By-Law* (ZBL).
- The proposed development includes 241 bicycle parking spaces, meeting the minimum number of required spaces as outlined in the City's ZBL. Section 111 outlines a requirement to provide at least 25% of bicycle spaces within a secure area or structure, which is also met by the proposed development.

Boundary Streets

- Sidney Street meets the target pedestrian level of service (PLOS) A and the target bicycle level of service (BLOS) D.
- Preston Street does not meet the target PLOS A or BLOS B, meets the target truck level of service (TkLOS) D, and achieves a transit level of service (TLOS) F, but has no target.
- The best possible PLOS for Preston Street is a PLOS C, which would require sidewalks with a minimum width of 2.0m and a minimum boulevard width of 2.0m. This is identified for the City's consideration. Along the site's frontage, a sidewalk width greater than 4m is proposed. Considering 2m of this width to be boulevard width, the best possible PLOS C will be achieved.
- The target BLOS B for Preston Street can be achieved by reducing the speed limit to 40 km/h, or by implementing curbside bike lanes with a minimum width of 1.5m. In areas with on-street parking, a 4.25m-wide bike plus parking lane would also achieve the target BLOS B. This is identified for the City's consideration.
- A sidewalk of approximately 2m width is proposed along the site's frontage to Sidney Street. This will maintain the PLOS of Sidney Street at the target PLOS A.

Access Design

- The existing depressed curbs to the subject site will be removed as part of the proposed development, and full-height curb and sidewalks will be reinstated per City standards. Curbs will be depressed and continuous across the proposed accesses to Sidney Street.
- The proposed accesses meet the requirements of Sections 25(a), 25(d), 25(h), 25(i), and 25(u) of the *Private Approach By-Law*, and Section 107(1) of the ZBL.
- It is requested that the requirements of Sections 25(m)(ii), 25(p), and 25(u) of the *Private Approach By-Law* be waived, as the accesses are proposed as far from Preston Street as possible, will maintain proper sightlines, and will not create a traffic hazard.
- The layout of the proposed accesses and parking garage ramps result in an on-site weave zone. To ensure safe operations, convex mirrors are proposed on the wall west of the inbound access, the wall east of the outbound access, and on the median between the two accesses. Additionally, stop signs are recommended at the inbound access and at both ramps for outbound vehicles.

- The proposed accesses will be stop-controlled, with free flow on Sidney Street. It is anticipated that the proposed accesses will operate acceptably during both peak hours.

Transportation Demand Management

- The proponent has committed to providing the following TDM measures:
 - Display local area maps with walking/cycling access routes and key destinations at major entrances;
 - Display relevant transit schedules and route maps at entrances;
 - Unbundle parking cost from monthly rent;
 - Provide a multimodal travel option information package to new residents.

Neighbourhood Traffic Management

- The proposed development relies on the local roadway Sidney Street for direct vehicular access. No neighbourhood traffic management measures are required, as Sidney Street is a short, dead-end roadway that only provides access to four different lots (7 Sidney Street, 490 Preston Street, 829 Carling Avenue, and 845 Carling Avenue).

Transit

- The proposed development is anticipated to generate an additional 63 transit trips during the AM peak hour and an additional 54 transit trips during the PM peak hour. It is anticipated that the proposed development will not have a significant impact on operations at the Carling O-Train Station and surrounding bus stops.

Intersection MMLOS

- The results of the intersection MMLOS analysis can be summarized as follows:
 - All study area intersections do not meet the target PLOS;
 - All study area intersections do not meet the target BLOS, except for Carling Avenue/Trillium Pathway and Preston Street/Pamilla Street;
 - All study area intersections with targets meet the target TLOS, except for Carling Avenue/Preston Street and Carling Avenue/Booth Street;
 - All study area intersections do not meet the target TkLOS, except for Carling Avenue/Sherwood Drive and Carling Avenue/Preston Street;
 - All study area intersections meet the target vehicular level of service (Auto LOS), except for Carling Avenue/Preston Street.
- Pedestrian Level of Service
 - All approaches at Carling Avenue/Sherwood Drive, Carling Avenue/Champagne Avenue, Carling Avenue/Preston Street, and Carling Avenue/Booth Street, and the east and west approach at Carling Avenue/Trillium Pathway, do not meet the target PLOS A. The functional design for the Carling Avenue Transit Priority Measures outlines various measures to improve the level of comfort for pedestrians, but the target PLOS A will not be achieved at any approach.
 - All approaches of Preston Street/Beech Street do not meet the target PLOS A. There is limited opportunity in improving the PLOS without the removal of auxiliary turn lanes.

- The north, south, and west approaches of Preston Street/Pamilla Street do not meet the target PLOS A. The north and south approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. Therefore, textured crosswalks similar to the east and west approaches at this intersection could be considered. Curb bulbouts could be considered to reduce crossing distance.
- All approaches of Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target PLOS A. The north, east, and west approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. This is identified for the City's consideration.
- Bicycle Level of Service
 - The west approach of Carling Avenue/Sherwood Drive, the north and west approaches of Carling Avenue/Champagne Avenue, and all approaches of Carling Avenue/Booth Street do not meet the target BLOS. The functional design for the Carling Avenue Transit Priority Measures identify segregated cycling facilities and protected intersections at these locations, which will allow all left turns for cyclists to take place off-road, and improve these approaches to a BLOS A.
 - All approaches of Carling Avenue/Preston Street does not meet the target BLOS B. The functional design for the Carling Avenue Transit Priority Measures identify segregated cycling facilities and two-stage left-turn bike boxes for eastbound and westbound cyclists, which would improve these approaches to a BLOS A. Two-stage bike boxes could be considered for northbound/southbound cyclists as well, and is identified for the City's consideration.
 - The north, south, and east approaches of Preston Street/Beech Street do not meet the target BLOS B. The *Ontario Traffic Manual – Book 18* identifies that mixed traffic lanes are appropriate for Beech Street, and designated bike lanes are appropriate on Preston Street. This is identified for the City's consideration. Alternatively, a reduction of the speed limit from 50 km/h to 40 km/h on both roadways would improve the BLOS to the target.
 - The north and west approaches of Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target BLOS B. The target BLOS can be achieved for these approaches with the implementation of two-stage left-turn bike boxes. This is identified for the City's consideration.
- Transit Level of Service
 - The north, east, and west approaches at Carling Avenue/Preston Street, and the north and east approaches at Carling Avenue/Booth Street, do not meet the target TLOS C during the AM and PM peak hours. The transit priority measures on Carling Avenue are anticipated to allow the east and west approaches to operate at a TLOS C or better. The north approaches at both intersections are anticipated to continue operating below the target TLOS in future conditions.

- Truck Level of Service
 - The east approach of Carling Avenue/Champagne Avenue does not meet the target TkLOS D. As Champagne Avenue is a local roadway and not a truck route, no modifications are recommended.
 - The east approach of Carling Avenue/Booth Street does not meet the target TkLOS D. Based on the functional design for the Carling Avenue Transit Priority Measures, the receiving lane for this movement will be a wider lane, and may accommodate trucks turning right from the east approach. Therefore, no further modifications are recommended.
 - All approaches of Preston Street/Beech Street and Preston Street/Pamilla Street do not meet the target TkLOS D. While the target TkLOS could be met by increase the curb radii, Beech Street and Pamilla Street are local roadways and not truck routes. Therefore, no modifications are recommended.
 - The south and west approaches of Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target TkLOS D. As these approaches involve heavy vehicles turning right into or out of the Dow's Lake Pavilion, no modifications are recommended.

Existing Intersection Operations

- At Carling Avenue/Preston Street, the northbound left turn, southbound through/right turn, and westbound left turn movements do not meet the target Auto LOS E during the PM peak hour.
- At Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway, the southbound left turn/through movement does not meet the target Auto LOS E during the AM and PM peak hours.
- During the AM and PM peak hours, southbound queueing at Carling Avenue/Preston Street extends through the upstream intersection at Preston Street/Sidney Street.

Background Intersection Operations

- Traffic throughout the study area could be displaced or alleviated through a combination of increase use of non-auto modes of transportation, alternate times of travel for drivers, and alternate routes of travel. It is assumed that the Carling Avenue Transit Priority Measures project will increase the transit modal share and decrease the auto modal share by the buildout year 2028.
- As congestion increases within the study area, some motorists may alter their travel times to occur outside of the peak hours and alter their routes to other roadways within proximity of the study area.
- At Carling Avenue/Preston Street, a reduction of 10 southbound through/right turning vehicles during the AM peak hour, and 80 northbound left turning vehicles, 50 southbound through/right turning vehicles, 70 eastbound through/right turning vehicles, and 90 westbound left turning vehicles during the PM peak hour would be required to meet the target Auto LOS E in the 2033 background conditions.

- At Carling Avenue/Booth Street, a reduction of 10 eastbound left turning vehicles during the AM peak hour and 30 westbound through vehicles during the PM peak hour would be required to meet the target Auto LOS E in the 2033 background conditions.
- At Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway, a reduction of 40 southbound left turning vehicles during the AM peak hour and 50 southbound left turning vehicles during the PM peak hour would be required to meet the target Auto LOS E in the 2033 background conditions.

Total Intersection Operations

- Traffic generated by the proposed development is anticipated to have marginal operational effects for most movements at the study area intersections.
- During the AM peak hour, the southbound through/right turn movement at Carling Avenue/Preston Street and the eastbound left turn movement at Carling Avenue/Booth Street marginally downgrade to an Auto LOS F as a result of site-generated traffic (i.e. eight additional southbound through/right turn vehicles at Carling Avenue/Preston Street and two additional eastbound left turning vehicles at Carling Avenue/Booth Street).
- It is anticipated that northbound left turns and eastbound left/right turns at Preston Street/Sidney Street will rely on courtesy from queued drivers on Preston Street to complete their turns during the peak hours. As there are two northbound lanes approaching Sidney Street, northbound through vehicles can use the curbside lane to bypass a northbound left turning vehicle. This is consistent with the existing intersection operations.
- The proposed development will add:
 - Six to twelve northbound left turning vehicles (equivalent to one vehicle every five to ten minutes during the peak hours);
 - One to four eastbound left turning vehicles (equivalent to one vehicle every 15 minutes during the AM peak hour), and;
 - Eight to thirteen eastbound right turning vehicles (equivalent to one vehicle every five to eight minutes during the peak hours).
- Based on the foregoing, the proposed development can be recommended from a transportation perspective.

1.0 SCREENING

1.1 Introduction

This Transportation Impact Assessment (TIA) has been prepared for the property located at 829 Carling Avenue, in support of Official Plan Amendment, Zoning By-Law Amendment, and Site Plan Control applications. The subject site is approximately 0.38 acres in size and is currently occupied by a CIBC banking centre. The subject site is currently served by one driveway to Carling Avenue, and one driveway to Sidney Street. Both driveways are approximately 30m west of Preston Street.

The subject site is surrounded by the following:

- Sidney Street and future high-rise residential development to the north,
- Carling Avenue and Dow's Lake Public Parking to the south,
- Preston Street and future high-rise residential development to the east, and
- An existing auto dealership to the west.

The site context is shown in **Figure 1**.

1.2 Proposed Development

The subject site is designated as 'Mixed Use Centre' on Schedule B of the City of Ottawa's Official Plan. The implemented zoning for the property is 'Arterial Mainstreet' (AM1), and the site is within the Preston-Carling District Secondary Plan.

The proposed development consists of a single 61-storey high-rise residential building with 459 dwellings and approximately 2,792 ft² GFA of ground-floor retail. A total of 386 parking spaces will be provided in six levels of underground parking and seven levels of above ground parking. Access to the parking garage will be provided through two one-way accesses separated by a 2m median along Sidney Street. The development will be constructed in a single phase, with a buildout year of 2028.

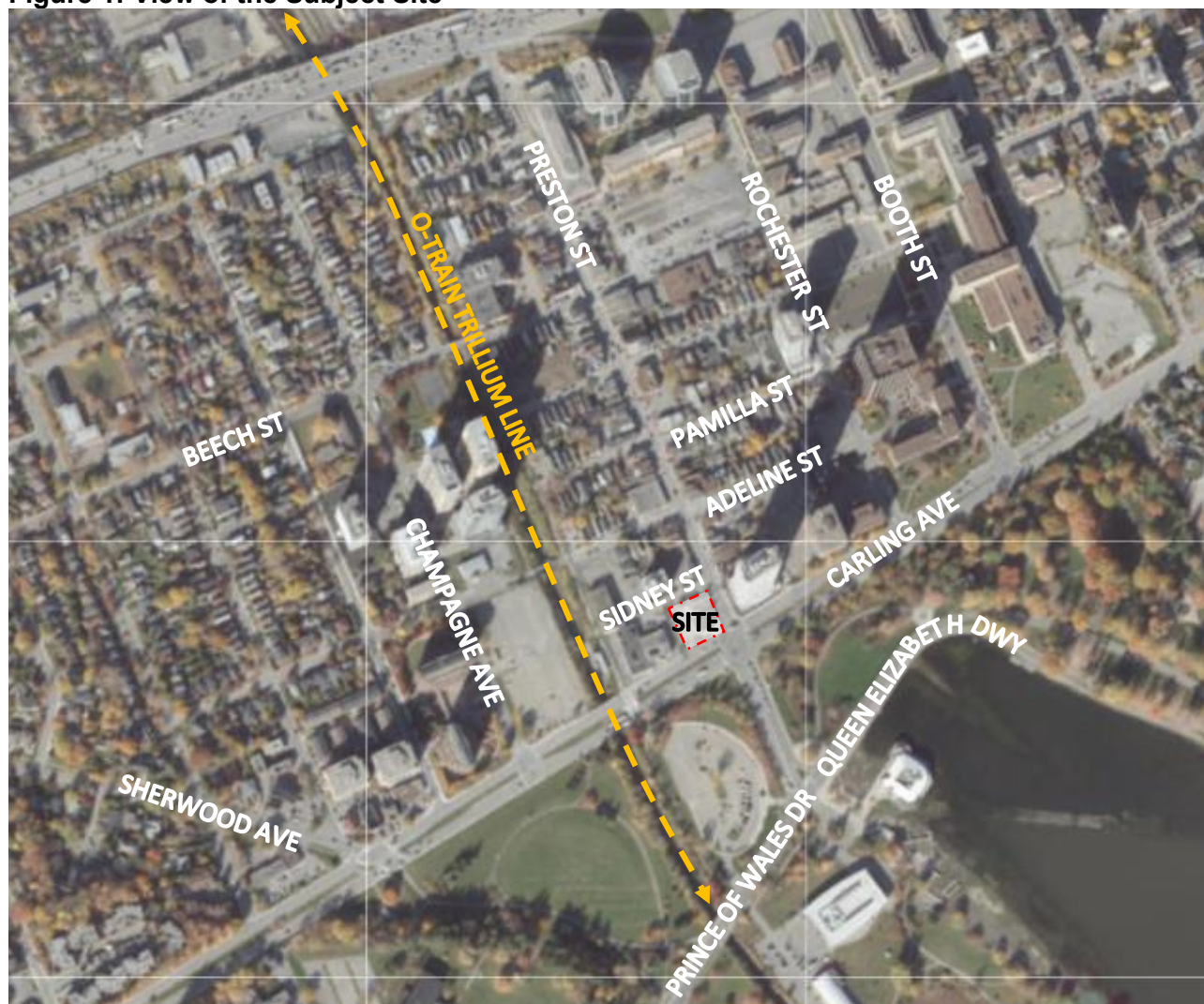
A copy of the proposed site plan is included in **Appendix A**.

1.3 Screening Form

The City's *2017 TIA Guidelines* identify three triggers for completing a TIA report, including trip generation, location, and safety. The criteria for each trigger are outlined in the City's TIA Screening Form. The trigger results are as follows:

- Trip Generation Trigger – The development is expected to generate over 60 person trips/peak hour; further assessment is required based on this trigger.
- Location Triggers – The development is located within a Transit Oriented Development Zone; further assessment is required based on this trigger.
- Safety Triggers – The proposed driveway is located within the area of influence of an adjacent traffic signal; further assessment is required based on this trigger.

A copy of the TIA Screening Form is included in **Appendix B**.

Figure 1: View of the Subject Site

2.0 SCOPING

2.1 Existing Conditions

2.1.1 Roadways

All roadways within the study area fall under the jurisdiction of the City of Ottawa.

Carling Avenue is an arterial roadway that generally runs on an east-west alignment between March Road and Bronson Avenue. Within the study area, it has a six-lane divided urban cross-section, concrete sidewalks on both sides of the roadway, and a posted speed limit of 60 km/h. Carling Avenue is classified as a truck route allowing full loads. Street parking is not permitted.

Preston Street is an arterial roadway that generally runs on a north-south alignment between Albert Street and Prince of Wales Drive. Within the study area, Preston Street has a four-lane undivided urban cross-section south of Carling Avenue, and a two-lane undivided urban cross-section north of

Carling Avenue. Concrete sidewalks are provided on both sides of Preston Street, and the roadway has an unposted regulatory speed limit of 50 km/h. Preston Street is classified as a truck route allowing full loads. On-street parking lanes are provided in select locations on both sides of Preston Street north of Carling Avenue. Street parking is not permitted south of Carling Avenue.

Prince of Wales Drive is an arterial roadway that generally runs on a north-south alignment between Preston Street and Fourth Line Road in North Gower. Within the study area, it has a two-lane undivided urban cross section, a concrete sidewalk on the north side of the road and an asphalt sidewalk on the south side, and a posted speed limit of 60km/h. Prince of Wales Drive is classified as a truck route allowing full loads.

Queen Elizabeth Driveway is a federally owned roadway that travels between Preston Street and Laurier Avenue. It has a two-lane undivided urban cross section, a multi-use pathway on the south side of the road, and a posted speed limit of 40km/h. Queen Elizabeth Driveway is not classified as a truck route.

Booth Street is a major collector roadway that runs on a north-south alignment between Carling Avenue and north of the Sir John A MacDonald Parkway/Wellington Street West, where it continues as the Chaudiere Crossing interprovincial bridge to Gatineau. Within the study area it has a two-lane undivided urban cross section, sidewalks on both sides, and a regulatory speed limit of 50km/h. Booth Street is classified as a truck route allowing full loads between Carling Avenue and Raymond Street.

Rochester Street runs on a north-south alignment between Carling Avenue and north of Primrose Avenue. It is classified as a major collector roadway between Carling Avenue and Gladstone Avenue, where it continues north as a local roadway. Within the study area it has a two-lane undivided urban cross section, sidewalks on both sides, and a regulatory speed limit of 50km/h. Rochester Street is classified as a truck route allowing full loads between Carling Avenue and Gladstone Avenue.

Sherwood Drive is a collector roadway that generally runs on an east-west alignment between Holland Avenue and Reid Avenue, before running on a southeast-northwest alignment between Reid Avenue and Carling Avenue. Within the study area, Sherwood Drive has a two-lane undivided urban cross-section, concrete sidewalks on both sides of the roadway, and a posted speed limit of 40 km/h. Sherwood Drive is not classified as a truck route, and street parking is not permitted.

Champagne Avenue is a local roadway that generally runs on a north-south alignment between Young Street and Carling Avenue. Within the study area, Champagne Avenue has a two-lane undivided urban cross-section, concrete sidewalks on both sides of the roadway north of Beech Street, a concrete sidewalk on the west side of the roadway south of Beech Street, and a posted speed limit of 40 km/h. Champagne Avenue is not classified as a truck route. Street parking is generally permitted on both sides of Champagne Avenue north of the subject site, with one hour restrictions for non-permit holders on weekdays between 8:00am and 5:00pm. South of the subject site, street parking is generally permitted on the west side of Champagne Avenue.

Beech Street is a local roadway that generally runs on an east-west alignment between Lynwood Avenue and Rochester Street. Within the study area, Beech Street has a two-lane undivided urban cross-section, sidewalks on both sides of the roadway, and an unposted regulatory speed limit of 50 km/h. Between Loretta Avenue and Preston Street, traffic calming measures such as curb extensions and flex posts have been implemented at select locations. Beech Street is not classified as a truck route. Street parking is generally permitted on the north side of Beech Street.

Pamilla Street is a local roadway that runs on an east-west alignment between Rochester Street and west of Preston Street. West of Preston Street it functions as a two-lane two-way roadway with parking permitted on the north side. East of Preston Street it functions as a one-way roadway with parking permitted on the south side. Sidewalks are provided on both sides of the entire length of the roadway. Pamilla Street has a regulatory speed limit of 50km/h and is not classified as a truck route.

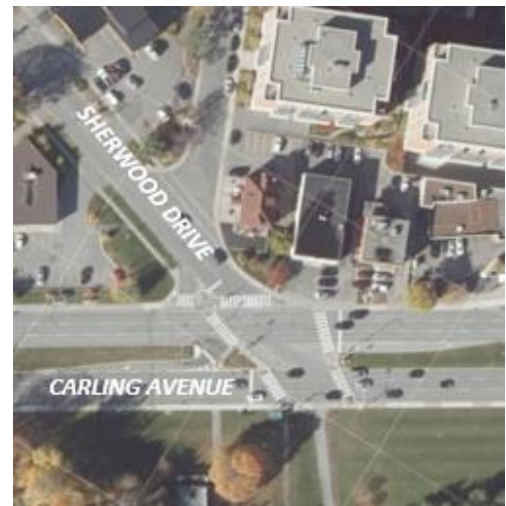
Adeline Street is a local roadway that runs on an east-west alignment between Rochester Street and west of Preston Street. It has a two-lane undivided urban cross section, sidewalks on both sides, and parking permitted on the south side. Adeline Street is not classified as a truck route.

Sidney Street is a local roadway that runs on an east-west alignment west of Preston Street. It has a two-lane undivided urban cross section, sidewalks on both sides, and parking permitted on the south side. Sidney Street is not classified as a truck route.

2.1.2 Intersections

Carling Ave/Sherwood Dr

- Signalized three-legged intersection
- North Approach: one left turn lane and one channelized right turn lane
- East Approach: one left turn lane, two through lanes, and one shared through/right turn lane
- West Approach: one left turn lane, two through lanes, and one transit-only through lane (not shown in aerial)
- Zebra-striped crosswalks implemented for all approaches in 2018
- The left turn lane on the east approach previously facilitated left turn movements for a development south of Carling Avenue. Since the development was removed, this turn lane is used to facilitate U-turn movements at this intersection



Carling Ave/Champagne Ave

- Signalized three-legged intersection
- North Approach: one left turn lane and one right turn lane
- East Approach: three through lanes and one right turn lane
- West Approach: one left turn lane, two through lanes, and one transit-only through lane (not shown in aerial)
- Standard crosswalks are provided on all approaches



Carling Ave/Trillium Pathway

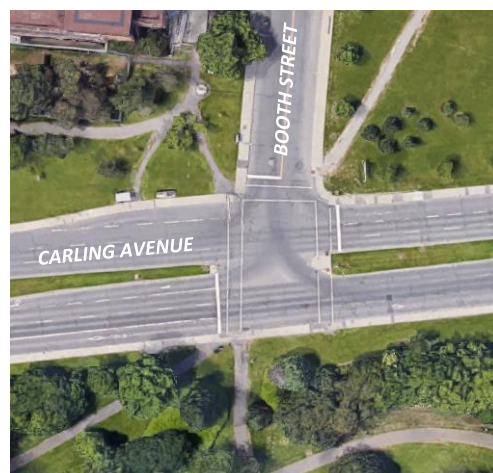
- Signalized pedestrian/cyclist crossing
- North/South Approaches: a single, bidirectional multi-use pathway (MUP)
- East Approach: three through lanes
- West Approach: two through lanes and one transit-only through lane (not shown in aerial)
- Zebra-striped crosswalks provided for all approaches
- Crossride provided for cyclists crossing Carling Avenue

Carling Ave/Preston St

- Signalized four-legged intersection
- North Approach: one left turn lane and one shared through/right turn lane
- South Approach: one left turn lane, one through lane, and one shared through/right turn lane
- East Approach: one left turn lane, two through lanes, and one shared through/right turn lane
- West Approach: one left turn lane, two through lanes, one transit-only through lane, and one right turn lane (not shown in aerial)
- Standard crosswalks are provided on all approaches

Carling Ave/Booth St

- Signalized three-legged intersection
- North Approach: one left turn and one right turn lane
- East Approach: two through lanes, and one shared right turn lane/transit-only through lane
- West Approach: one left turn lane, two through lanes, and one transit-only through lane
- Standard crosswalks are provided on all approaches



Preston St/Beech St

- Signalized four-legged intersection
- North/South Approaches: one left turn lane and one shared through/right turn lane
- East Approach: one shared left turn/through lane and one right turn lane
- West Approach: one shared left turn/through/right turn lane
- Concrete textured crosswalks are provided on all approaches

Preston St/Pamilla St

- Signalized four-legged intersection
- North/South/West Approaches: one shared left turn/through/right turn lane
- East approach is on-way eastbound
- Concrete textured crosswalks are provided on the east and west approaches
- Standard crosswalks are provided on the north and south approaches

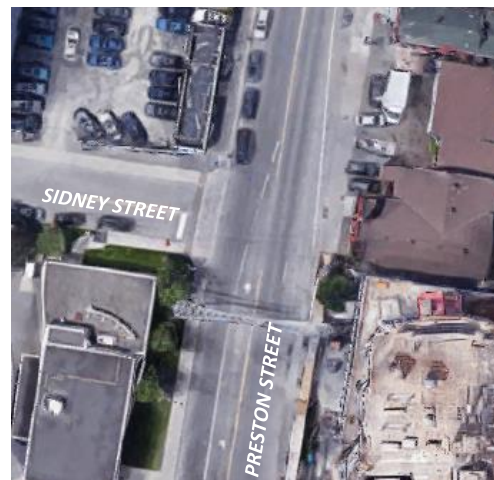
Preston St/Adeline St

- Unsignalized four-legged intersection; stop control on east/west approaches
- All Approaches: one shared left turn/through/right turn lane
- East approach is on-way eastbound
- Concrete textured crosswalks are provided on the east and west approaches



Preston St/Sidney St

- Unsignalized three-legged intersection; stop control on west approaches
- All Approaches: one shared lane
- A standard crosswalk is provided on the west approach

Preston St/Prince of Wales Dr/Queen Elizabeth Dwy

- Signalized four-legged intersection
- North Approach: one shared left turn/through lane and one right turn lane
- South Approach: one shared left/through/right turn lane
- East Approach: one left turn lane, one through lane, and one channelized right turn lane
- West Approach: one left turn lane and one shared through/right turn lane
- Standard crosswalks are provided on all legs
- Bike lanes are provided on the west leg
- A MUP is provided on south side of the road on the east leg

**2.1.3 Driveways**

In accordance with the *2017 TIA Guidelines*, a review of adjacent driveways along the boundary roads are provided as follows:

Sidney Street, North Side:

- One access to a residential development at 7 Sidney Street
- One future access to a residential development at 500 Preston Street (under construction)
- One access to a car dealership at 845 Carling Avenue

Sidney Street, South Side:

- None

Carling Avenue, North Side:

- Two accesses to a car dealership at 845 Carling Avenue

Carling Avenue, South Side

- None

Preston Street, East Side:

- One access to an auto repair shop at 495 Preston Street
- One access to the Claridge ICON Sales Centre at 485 Preston Street

Preston Street, West Side:

- None

2.1.4 Pedestrian and Cycling Facilities

Sidewalks are provided on both sides of Carling Avenue, Prince of Wales Drive, Sidney Street, Adeline Street, Pamilla Street, Beech Street, Booth Street, Preston Street, and Sherwood Drive, and on the west side of Champagne Avenue north of Carling Avenue. A multi-use pathway is provided on the east side of the Trillium Rail Corridor, referred to as the Trillium Pathway, and on the south side of Queen Elizabeth Driveway east of Preston Street. A multi-use pathway network is also provided through Commissioners Park east of Preston Street between Carling Avenue and Queen Elizabeth Driveway, and ties into the southeast corner of the Carling Avenue/Preston Street intersection. On-street bike lanes are provided on Prince of Wales Drive.

In the City of Ottawa's primary cycling network, Carling Avenue and Prince of Wales Drive are classified as Spine Routes, Preston Street and Adeline Street west of Preston Street are classified as Local Routes, and Sherwood Drive is classified as both a Local Route and Neighbourhood Bikeway. The Trillium Pathway is classified as a Crosstown Bikeway.

2.1.5 Transit

The Carling O-Train Station is located at a walking distance of approximately 120m from the proposed development. This station is currently under construction as part of the City's Phase 2 Light Rail Transit (LRT) Trillium Line extension and is scheduled to be reopen in 2022. Further details of Phase 2 LRT are provided in Section 2.2. While the Carling Avenue O-Train Station is closed for the Trillium Line Extension construction, Route 2 trains have been replaced by buses along Preston Street.

OC Transpo bus stops in proximity of the subject site are summarized as follows:

Carling Avenue/Carling O-Train Station

- Stop #7369 – for routes 55, 56, and 85
(located on the south side of Carling Avenue, approximately 50m west of the Trillium Pathway)
- Stop #8014 – for routes 55, 56, and 85
(located on the north side of Carling Avenue, approximately 50m west of the Trillium Pathway)

Carling Avenue/Preston Street

- Stop #2397 – for route 85
(located on the west side of Preston Street, approximately 10m north of Carling Avenue)
- Stop #6657 – for routes 85 and 2
(located on the east side of Preston Street, approximately 45m north of Carling Avenue)
- Stop #8023 – for routes 55 and 56
(located on the south side of Carling Avenue at Preston Street)

Preston Street/Adeline Street

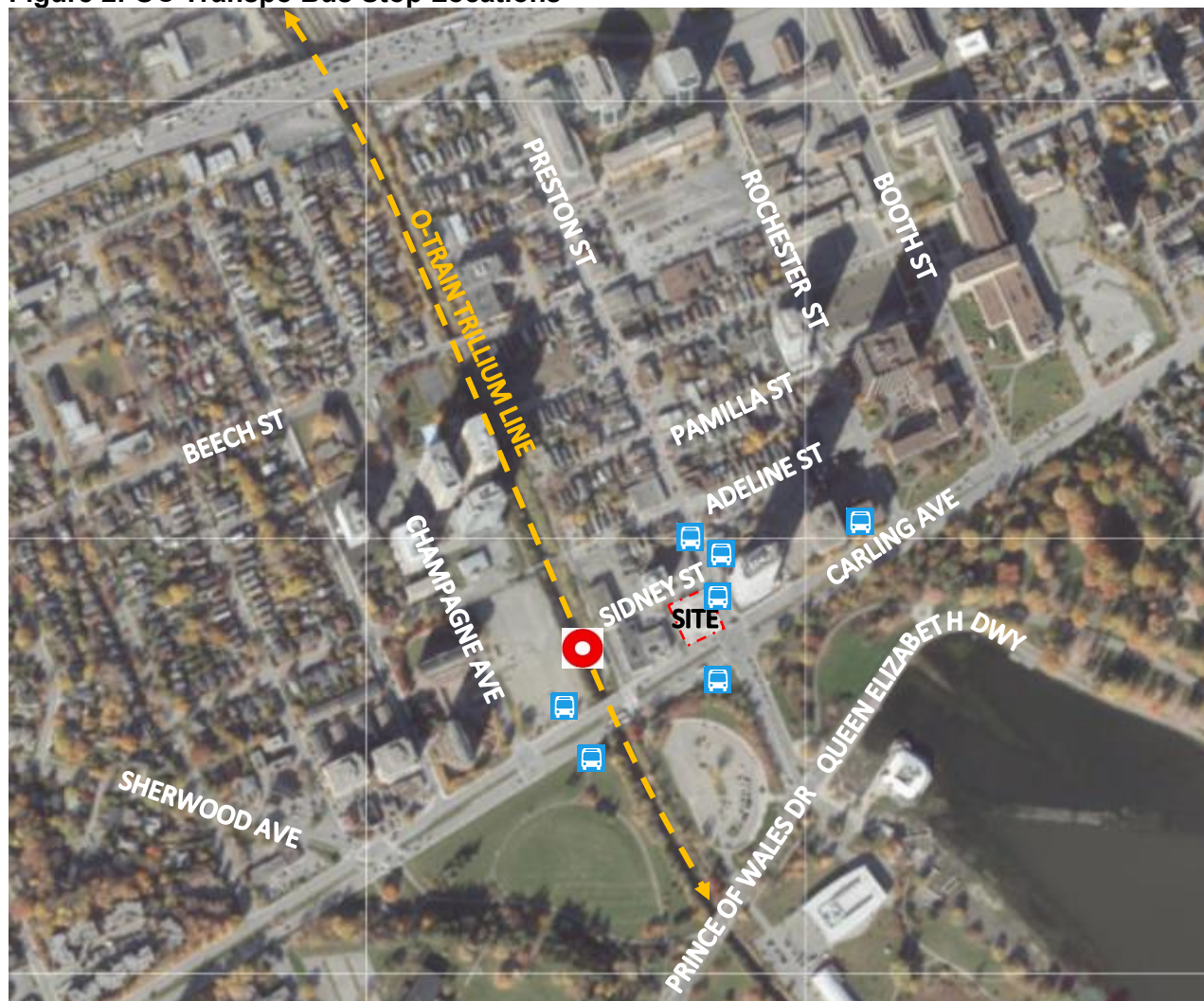
- Stop #6655 – for route 2
(located on the west side of Preston Street, approximately 10m south of Adeline Street)

Carling Avenue/Norfolk Avenue

- Stop #8013 – for routes 55, 56, and 85
(located on the north side of Carling Avenue, approximately 15m east of the Norfolk Avenue)

Locations of bus stops in proximity of the site are shown in **Figure 2**.

Figure 2: OC Transpo Bus Stop Locations



OC Transpo Route 2 travels between Bayview O-Train Station and Greenboro O-Train Station. The route operates on 10 to 15 minute headways, seven days a week.

OC Transpo Route 55 travels between Elmvale and the Bayshore Transit Station. The route operates on 15 minute headways on weekdays and 30 minute headways on weekends.

OC Transpo Route 56 travels between Tunney's Pasture Station and Hurdman Station. The route operates every 15 to 30 minute headways on weekdays. The route does not operate on weekends.

OC Transpo Route 85 travels between either Mackenzie King Station or Lees Station and Bayshore Station. The route operates every 15 to 30 minute, seven days a week.

Detailed route information and an excerpt from the OC Transpo System Map are included in **Appendix C**.

2.1.6 Area Traffic Management

There are no Area Traffic Management (ATM) studies within the study area that have been completed or are currently in progress. Traffic calming measures such as curb extensions and flex posts have been implemented at select locations along Beech Street between Loretta Avenue and Preston Street.

2.1.7 Existing Traffic Volumes

Weekday traffic counts completed by the City of Ottawa were used to determine the existing pedestrian, cyclist, and vehicular traffic volumes at the study area intersections. These counts were completed on the dates listed below by the following sources:

• Carling Avenue/Sherwood Drive	August 25, 2016
• Carling Avenue/Champagne Avenue	February 4, 2016
• Carling Avenue/Trillium Pathway	July 13, 2016
• Carling Avenue/Preston Street	June 20, 2017
• Carling Avenue/Booth Street	September 12, 2019
• Preston Street/Beech Street	September 7, 2016
• Preston Street/Pamilla Street	September 7, 2016
• Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway	January 10, 2018

It is noted that the City of Ottawa does not have traffic counts at the Preston Street/Adeline Street and Preston Street/Sidney Street intersections. Due to COVID-19 restrictions, new traffic counts at these intersections would not be reflective of typical traffic conditions, and therefore have not been conducted.

As part of the 500 Preston Street Community Transportation Study dated June 2011, Delcan conducted a traffic count at the Preston Street/Sidney Street intersection. At the time of the 2011 traffic count, the 845 Carling Avenue site contained a car dealership. As this site is still occupied by a car dealership, vehicular traffic to/from Sidney Street from the 2011 Delcan traffic count is considered representative of traffic along Sidney Street. Northbound and southbound through traffic volumes along Preston Street have been drawn from the Carling Avenue/Preston Street intersection.

As part of the 505 Preston Street Community Transportation Study dated December 2012, IBI Group conducted a traffic count at the Preston Street/Adeline Street intersection. As newer traffic counts were unavailable at the time of writing of this report, the 2012 IBI Group count has been used to reflect traffic to/from Adeline Street. Northbound and southbound through traffic volumes along Preston Street have been drawn from the Carling Avenue/Preston Street intersection.

All traffic count data previously discussed are included in **Appendix D**. Traffic volumes within the study area are shown in **Figure 3**.

2.1.8 Collision Records

Historical collision data from the last five years was obtained from the City's Public Works and Service Department for the study area intersections. Copies of the collision summary reports are included in **Appendix E**.

The collision data has been evaluated to determine if there are any identifiable collision patterns. The number of collisions at each intersection from January 1, 2015 to December 31, 2019 is summarized in **Table 1**.

Table 1: Reported Collisions

Intersection	Impact Types						Total
	Angle	Sideswipe	Rear End	Turning Movement	Approach	SMV ⁽¹⁾ / Other	
Carling Avenue/ Sherwood Drive	-	-	5	2	-	1	8
Carling Avenue/ Champagne Avenue	1	4	4	2	-	-	11
Carling Avenue/ Trillium Pathway	-	-	1	-	-	3	4
Carling Avenue/ Preston Street	6	10	24	10	-	6	56
Carling Avenue/ Booth Street	1	2	8	9	-	-	20
Preston Street/ Beech Street	1	2	-	3	-	1	7
Preston Street/ Pamilla Street	1	-	3	1	1	1	7
Preston Street/ Adeline Street	1	-	1	-	-	2	4
Preston Street/ Sidney Street	6	1	1	1	-	-	9
Preston St/Prince of Wales Drive/Queen Elizabeth Dwy	5	4	11	5	-	1	26

1. SMV = Single Motor Vehicle

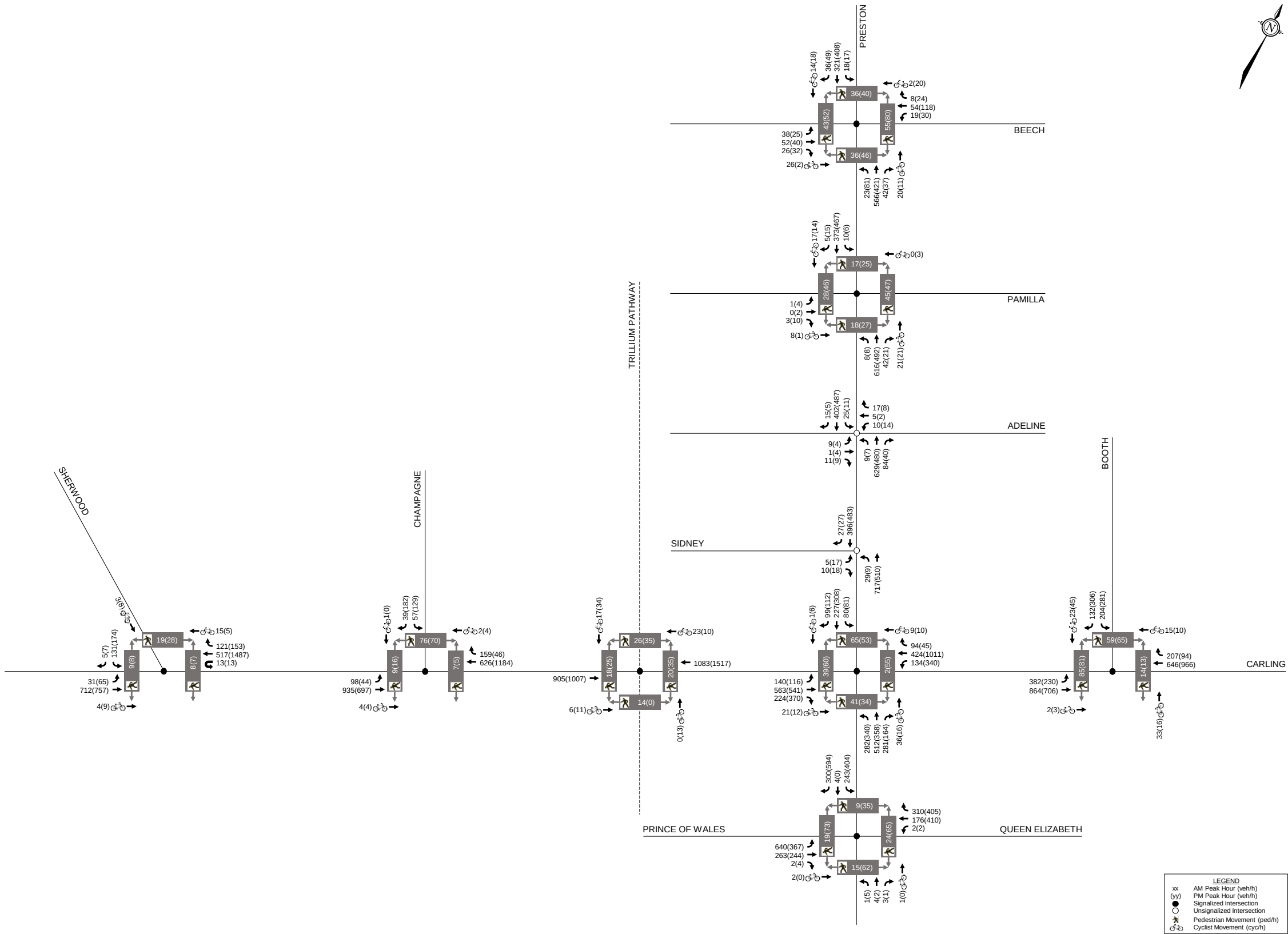
Carling Avenue/Sherwood Drive

A total of eight collisions were reported at this intersection over the last five years, of which there were five rear-end impacts, two turning movement impacts, and one single vehicle/other impact. Three of the collisions resulted in personal injuries, but none caused fatalities. Four of the collisions occurred in poor driving conditions. None of the collisions involved pedestrians or cyclists.

Carling Avenue/Champagne Avenue

A total of eleven collisions were reported at this intersection over the last five years, of which there were four rear-end impacts, four sideswipe impacts, two turning movement impacts, and one angle impact. Three of the collisions resulted in personal injuries, but none caused fatalities. Five of the collisions occurred in poor driving conditions. None of the collisions involved pedestrians or cyclists.

Figure 3: Existing Network Traffic Volumes



Carling Avenue/Trillium Pathway

A total of four collisions were reported at this intersection over the last five years, of which there was one rear-end impact and three impacts involving a vehicle and a cyclist. All four collisions resulted in personal injuries, but none caused fatalities. No collisions occurred in poor driving conditions.

Carling Avenue/Preston Street

A total of 56 collisions were reported at this intersection over the last five years, of which there were 24 rear-end impacts, ten turning movement impacts, ten sideswipe impacts, six angle impacts, and six single vehicle/other impacts. Nineteen of the 56 collisions resulted in injuries, but none caused fatalities. Twenty-four of the collisions occurred under poor driving conditions. Eight of the collisions involved cyclists and one involved a pedestrian.

Of the 24 rear-end impacts, three occurred at the northbound approach (two left turn incident and one through vehicle incident), three occurred at the southbound approach (all through vehicle incidents), ten occurred at the eastbound approach (nine through vehicle incidents and one left turn incident), and eight occurred at the westbound approach (one left turn incident, six through vehicle incidents, and one right turn incident). Eight of the 24 collision occurred in poor driving conditions.

Of the nine turning movement impacts, six involved southbound left turning vehicles (four of which involved cyclists), two involved northbound right turning vehicles (one of which involved a cyclist), one involved a southbound right turning vehicle and a cyclist, and one involved a westbound left turning vehicle. Six of the collisions resulted in personal injuries, but none caused fatalities. Six of the collisions occurred under poor driving conditions.

It is likely that some or all of the northbound cyclists entered the intersection from the MUP approach at the southeast corner of Carling Avenue/Preston Street. Cyclists coming from the MUP enter the intersection on an angle, rather than parallel with northbound or westbound traffic. As such, drivers may not recognize which direction cyclists are heading until the cyclist has entered the intersection. It is noted that as crossrides are not provided for cyclists at this intersection, cyclists are required to dismount when crossing. The Carling Avenue Transit Priority Measures functional design may address this pattern of collisions, as a realignment of the MUP approach at this intersection is identified in the design. The functional design does not include crossrides for cyclists crossing Carling Avenue from the MUP, and cyclists will still be required to dismount when crossing.

Of the ten sideswipe impacts, one occurred at the northbound approach, four occurred at the southbound approach, two occurred at the eastbound approach, and three occurred at the westbound approach. All of the collisions resulted in property damage only. Five collisions occurred in poor driving conditions.

Carling Avenue/Booth Street

A total of 20 collisions were reported at this intersection over the last five years, of which there were nine turning movement impacts, eight rear-end impacts, two sideswipe impacts, and one angle impact. Six of the collisions resulted in personal injuries, but none caused fatalities. Seven of the collisions occurred in poor driving conditions. One of the collisions involved a cyclist and none of the involved pedestrians.

Of the nine turning movement impacts, seven involved eastbound left turning vehicles, one involved a southbound left turning vehicle, and one involved a westbound left turning vehicle making a U-turn. Three of the collisions caused personal injuries and four occurred under poor driving conditions.

Of the eight rear-end impacts, four included westbound vehicles, three included eastbound vehicles, and one included southbound vehicles. Three caused personal injuries and two occurred under in poor driving conditions.

Preston Street/Beech Street

A total of seven collisions were reported at this intersection over the last five years, of which there were three turning movement impacts, two sideswipe impact, one angle impact, and one single vehicle/other impact. Only one of the collisions resulted in personal injuries. Two of the collisions occurred in poor driving conditions. None of the collisions involved pedestrians or cyclists.

Preston Street/Pamilla Street

A total of seven collisions were reported at this intersection over the last five years, of which there were three rear-end impacts, one angle impact, one turning movement impact, one approach impact, and one single vehicle/other impact. Only one of the collisions resulted in personal injuries. Two of the collisions occurred in poor driving conditions. One of the collisions involved a pedestrian and none involved cyclists.

Preston Street/Adeline Street

A total of four collisions were reported at this intersection over the last five years, of which there were two single vehicle/other impacts, one angle impact, and one rear-end impact. All of the collisions caused property damage only. Three of the collisions occurred under poor driving conditions. None of the collisions involved pedestrians or cyclists. None of the collisions involved pedestrians or cyclists.

Preston Street/Sidney Street

A total of nine collisions were reported at this intersection over the last five years, of which there were six angle impacts, one sideswipe impact, one rear-end impact, and one turning movement impact. All of the collisions caused property damage only. Five of the collisions occurred in poor driving conditions. None of the collisions involved pedestrians or cyclists.

Of the six angle impacts, four involved eastbound left turning vehicles, one involved an eastbound right turning vehicle and one involved an eastbound vehicle performing an unknown maneuver.

Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway

A total of 26 collisions were reported at this intersection over the last five years, of which there were 11 rear-end impacts, five turning movement impacts, five angle impacts, four sideswipe impacts, and one single vehicle/other impact. Six of the collisions resulted in personal injuries, but none caused fatalities. Ten of the collisions occurred in poor driving conditions. Four of the collisions involved a cyclists and none of the involved pedestrians.

Of the 11 rear-end impacts, five included southbound vehicles, three involved eastbound vehicles, and three involved westbound vehicles. One caused personal injuries and four occurred under in poor driving conditions.

2.2 Planned Conditions

2.2.1 Planned Roadway and Transit Projects

Within the study area, the 2013 Ottawa Cycling Plan identifies the planned Westboro Neighbourhood Bikeway which includes shared use lanes on Sherwood Drive between Fairmont Avenue and Carling Avenue as a Phase 1 (2014-2019) project. The 2013 Ottawa Pedestrian Plan does not identify any improvements within the study area.

The City's 2013 Transportation Master Plan (TMP) does not identify any roadway projects within the study area in its Affordable Road Network. The Carling Avenue Transit Priority Measures project is identified in the 2013 TMP as an improvement in the Affordable Rapid Transit and Transit Priority (RTTP) Network.

The TMP indicates that between Lincoln Fields Station and Carling O-Train Station, exclusive bus lanes will be made available via reallocation of existing traffic lanes. Between Carling O-Train Station and Bronson Avenue, transit signal priority and queue jump lanes will be implemented at select intersections. The preliminary functional design of the Carling Avenue Transit Priority Measures project for the section within the study area is shown in **Figure 4**.

Construction for Phase 2 of the LRT began in 2019. Phase 2 of LRT will extend the Confederation Line east and west and will extend the Trillium Line south. The Trillium Line extension will continue the Trillium Line from Greenboro Station to Limebank Road in Riverside South, along with a link to the Ottawa Macdonald-Cartier International Airport. Revenue service for this extension is planned for 2022. A map of the planned Phase 2 LRT extensions are shown in **Figure 5**.

2.2.2 Other Area Developments

In proximity of the proposed development, there are multiple other residential and mixed-use developments are under construction, approved, or in the approval process. Other developments in the area include:

A residential expansion containing 197 residential units is proposed for 17 Aberdeen Street. A Transportation Overview dated August 2016 was prepared in support of this development.

A mixed-use development containing 1,000 residential units and 142,200 ft² of commercial space is proposed at 552 Booth Street. A TIA dated December 2018 was prepared in support of a Zoning By-law Amendment and Official Plan Amendment application for this development.

A mixed-use building containing 40 residential units and 1,000 ft² of Office space is proposed at 289 Carling Avenue. A TIA dated August 2019 was prepared in support of this development.

A mixed-use development containing 1,123 units and 16,255 ft² of commercial space is proposed at 845 Carling Avenue. A Community Transportation Study/Transportation Impact Study was prepared in support of a Zoning By-law Amendment application for this development in April 2013.

A residential development containing 236 units is proposed at 90 Champagne Avenue. A TIA dated November 2019 was prepared in support of this development.

Figure 4: Carling Avenue Transit Priority Measures – Functional Design

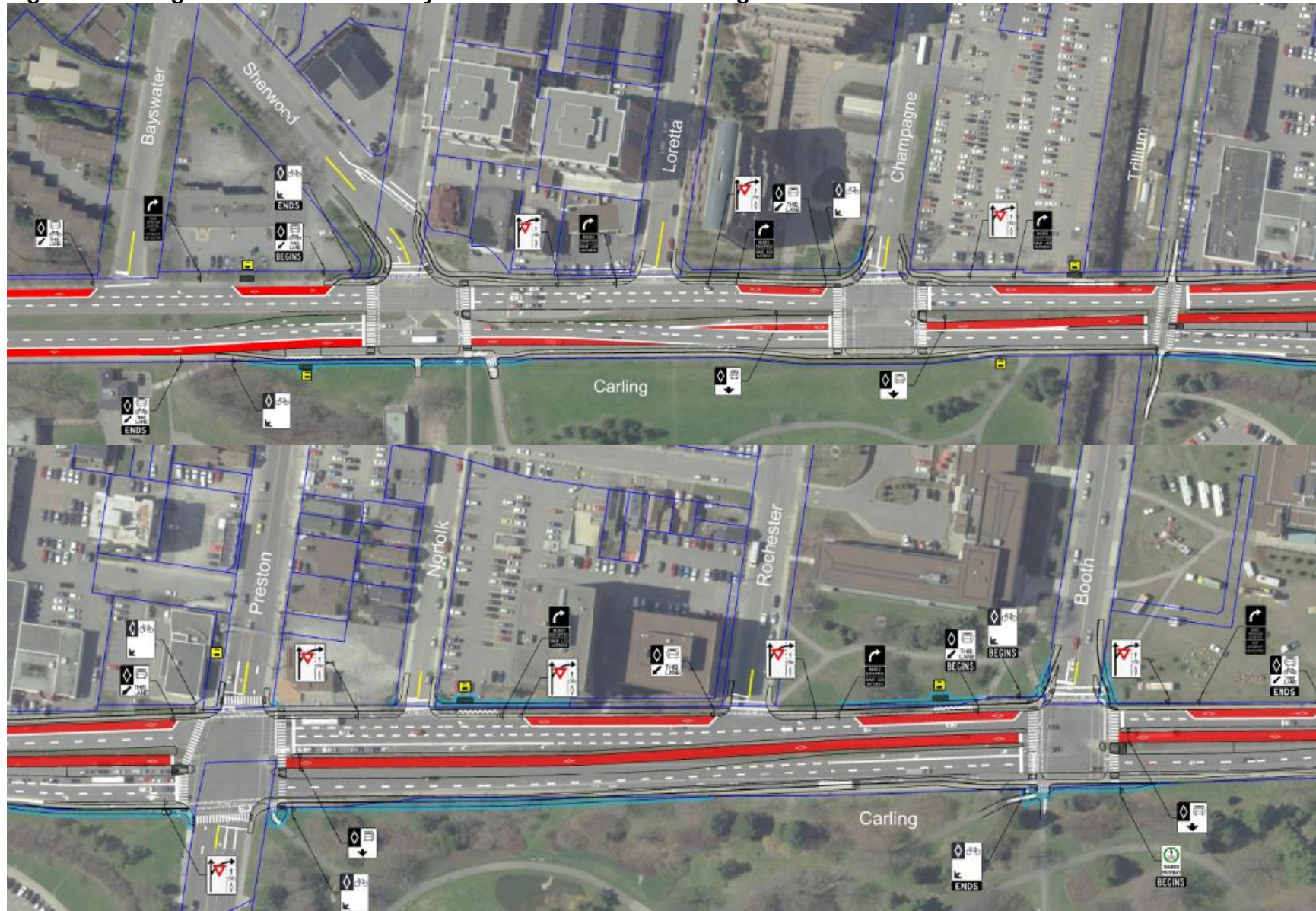
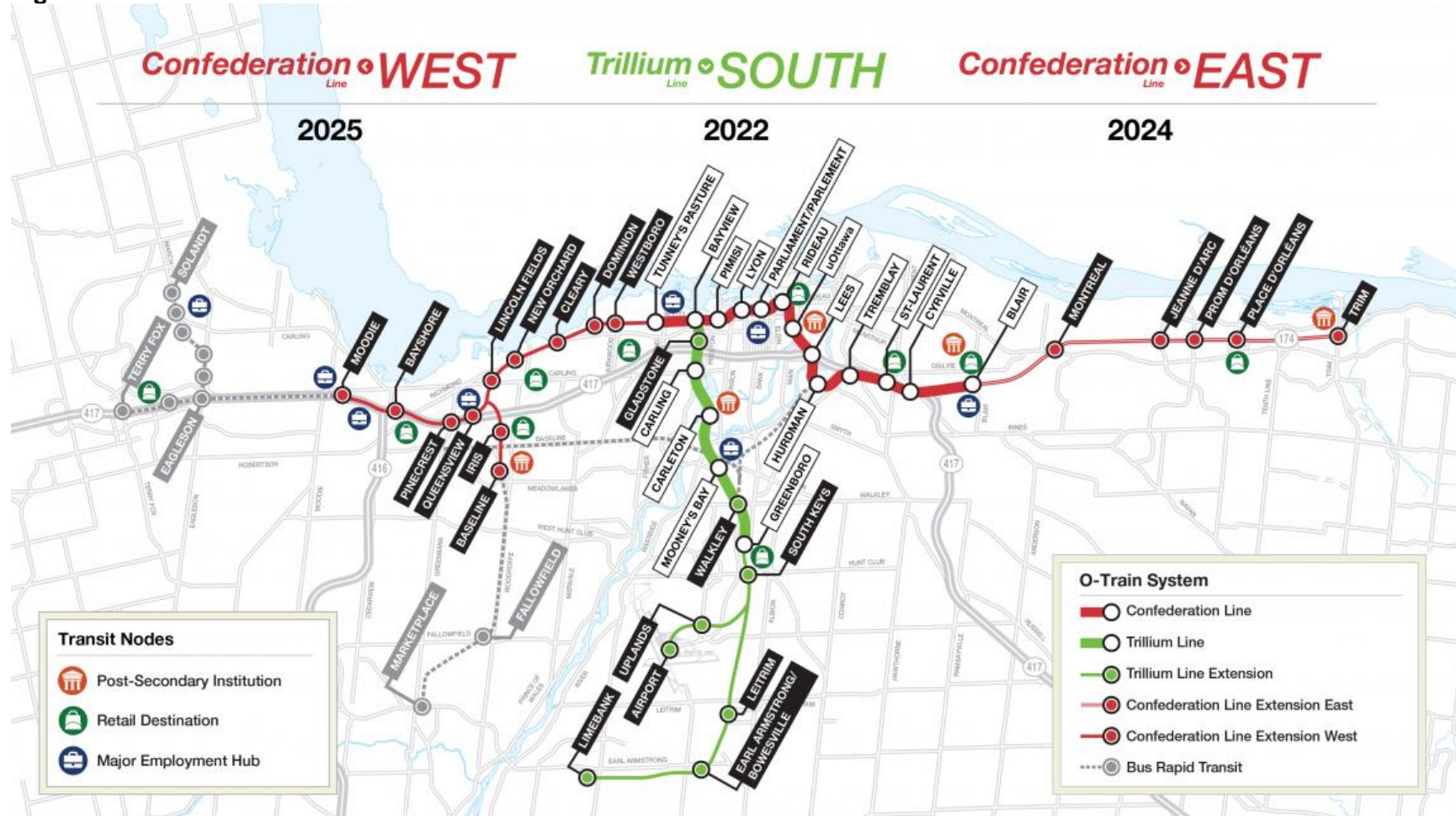


Figure 5: LRT Phase 2



Two high-rise residential towers containing a combined 540 units are currently under construction at 101 and 105 Champagne Avenue. A Transportation Overview dated November 2011 and Parking Requirements Study dated November 2015 were prepared in support of this development.

A residential development containing 117 units is proposed for 93-105 Norman Street. A Transportation Brief Addendum 1 dated October 2013 was prepared in support of this application.

A mixed-use development containing 280 residential units and 10,441 ft² of commercial space is currently under construction at 500 Preston Street. A Community Transportation Study dated June 2011, Addendum dated July 2013, and Addendum 2 dated October 2013 in support of this application.

A mixed-use development containing 252 residential units, 4,786 ft² of retail space, and 16,047 ft² of office space is currently under construction at 505 Preston Street. A Community Transportation Study dated December 2012 and Transportation Overview dated May 2013 were prepared in support of this development.

A mixed-use development containing 540 residential units and 59,182 ft² of commercial space is proposed at 450 Rochester Street. A TIA dated October 2019 was prepared in support of this report.

An expansion of the Ottawa Civic Hospital is planned at 930 Carling Avenue (north and west of Prince of Wales Drive, south of Carling Avenue, and east of Birch Drive), and is anticipated to include 800 hospital beds and 2,000,000 ft² GFA of floor space. A traffic study has not yet been submitted to the City of Ottawa.

2.3 Study Area and Time Periods

The study area for this report includes the boundary roadways Carling Avenue, Preston Street, and Sidney Street, as well as the following intersections:

- Carling Avenue/Sherwood Drive
- Carling Avenue/Champagne Avenue
- Carling Avenue/Trillium Pathway
- Carling Avenue/Preston Street
- Carling Avenue/Booth Street
- Preston Street/Beech Street
- Preston Street/Pamilla Street
- Preston Street/Adeline Street
- Preston Street/Sidney Street
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway

The selected time periods for the analysis are the weekday AM and PM peak hours, as they represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2028 build-out year and 2033 horizon year.

2.4 Exemptions Review

This module reviews possible exemptions from the final Transportation Impact Assessment, as outlined in the *2017 TIA Guidelines*. The applicable exemptions for this site are shown in **Table 2**.

Table 2: TIA Exemptions

Module	Element	Exemption Criteria	Status
Design Review Component			
4.1 Development Design	4.1.2 Circulation and Access	• Only required for site plans	Not Exempt
	4.1.3 New Street Networks	• Only required for plans of subdivision	Exempt
4.2 Parking	4.2.1 Parking Supply	• Only required for site plans	Not Exempt
	4.2.2 Spillover Parking	• Only required for site plans where parking supply is 15% below unconstrained demand	Exempt
Network Impact Component			
4.5 Transportation Demand Management	<i>All elements</i>	• Not required for non-residential site plans expected to have fewer than 60 employees and/or students on location at any given time	Not Exempt
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	• Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Not Exempt
4.8 Network Concept	<i>All elements</i>	• Only required when proposed development generates more than 200 person-trips during the peak hour in excess of the equivalent volume permitted by the established zoning	Exempt

Based on the foregoing, the following modules will be included in the TIA report:

- Module 4.1: Development Design
- Module 4.2: Parking
- Module 4.3: Boundary Streets
- Module 4.4: Access Design
- Module 4.5: Transportation Demand Management
- Module 4.6: Neighbourhood Traffic Management
- Module 4.7: Transit
- Module 4.9: Intersection Design

3.0 FORECASTING

3.1 Development-Generated Travel Demand

3.1.1 Trip Generation

The subject site is currently occupied with a CIBC banking centre (approximately 4,300 ft² GFA in size). The proposed development will replace the existing banking centre with a total of 459 high-rise dwellings and approximately 2,792 ft² GFA of ground-floor retail. The methodology and results of estimating the trip generation for each land use are discussed in the subsequent sections.

The *TRANS Trip Generation Manual Summary Report*, prepared in October 2020 by WSP, includes data to estimate the mode shares for commercial trip generators (in Table 13 of the manual) and high-rise multifamily housing (in Table 8 of the manual) for the AM and PM peak periods, based on district. The *TRANS Trip Generation Manual* identifies the subject site as being located within the Ottawa Inner Area district, and outlines the following mode shares for commercial and residential developments in the Ottawa Inner Area.

Commercial Mode Shares

- Auto Driver: 39% AM, 22% PM
- Auto Passenger: 2% AM, 4% PM
- Transit: 16% AM, 12% PM
- Cyclist: 3% AM, 4% PM
- Pedestrian: 40% AM, 58% PM

Residential Mode Shares

- Auto Driver: 26% AM, 25% PM
- Auto Passenger: 6% AM, 8% PM
- Transit: 28% AM, 21% PM
- Cyclist: 5% AM, 6% PM
- Pedestrian: 34% AM, 39% PM

As the site is located within 600m of the Carling O-Train Station, the proposed development is considered a Transit-Oriented Development (TOD). In TOD zones, the transit share is assumed to increase significantly compared to any TRANS O-D district. The City has outlined sustainable mode share targets for transit-oriented developments, which can be summarized as follows:

- Auto Driver: 15% during peak periods;
- Auto Passenger: 5% during peak periods;
- Transit: 65% during peak periods;
- Non-Auto (Active): 15% during peak periods.

Given the subject site's proximity to amenities and destinations such as Preston Street and Carleton University, the assumed mode shares for the existing and proposed land uses reflect a higher non-auto mode share and a lower transit share than the TOD mode share targets described above. It is also assumed that the non-auto mode share for the existing bank and proposed retail uses is higher than the non-auto mode share for the proposed high-rise dwellings.

The mode shares carried forward in the trip generation estimates for each land use are included in **Table 3**. Relevant excerpts of the *TRANS Trip Generation Manual* are included in **Appendix F**.

Table 3: Mode Shares for Commercial and Residential Uses

Mode	Existing Bank		Proposed Retail		Proposed Residential	
	AM	PM	AM	PM	AM	PM
Auto Driver	15%	15%	15%	15%	15%	15%
Auto Passenger	5%	5%	5%	5%	5%	5%
Transit	35%	15%	35%	15%	40%	35%
Cyclist	5%	5%	5%	5%	5%	5%
Pedestrian	40%	60%	40%	60%	35%	40%

3.1.1.1 Existing Bank Trip Generation

Trips generated by the existing bank have been estimated using the Drive-in Bank land use rates included in the *ITE Trip Generation Manual, 10th Edition*. The estimated number of person trips generated by the existing development are shown in **Table 4**.

Table 4: Existing Development – Trip Generation

Land Use	ITE Code	GFA	AM Peak Hour (pph ⁽¹⁾)			PM Peak Hour (pph)		
			IN	OUT	TOT	IN	OUT	TOT
Drive-in Bank	912	4,300 ft ²	30	22	52	56	56	112

1. pph: Person Trips per Hour – Calculated using an ITE Trip to Person Trip factor of 1.28, consistent with the 2017 TIA Guidelines

Based on the previous table, the existing banking centre is estimated to generate 52 person trips during the AM peak hour and 112 person trips during the PM peak hour. A breakdown of these trips by modal share is shown in **Table 5**.

Table 5: Existing Development – Trips by Mode Share

Travel Mode	Mode Share		AM Peak Hour			PM Peak Hour		
	AM	PM	IN	OUT	TOT	IN	OUT	TOT
Peak Hour Person Trips			30	22	52	56	56	112
Auto Driver	15%	15%	5	3	8	8	8	16
Auto Passenger	5%	5%	1	1	2	3	3	6
Transit	35%	15%	11	8	19	8	8	16
Cyclist	5%	5%	1	1	2	3	3	6
Pedestrian	40%	60%	12	9	21	34	34	68

From the previous table, the existing banking centre is estimated to generate eight vehicle trips during the AM peak hour and 16 vehicle trips during the PM peak hour.

This land use is anticipated to generate two types of external peak hour trips: primary and pass-by trips. Primary trips are made for the specific purpose of visiting the site, while pass-by trips are made as intermediate stops on the way to another destination. Peak hour pass-by trips for the existing development are estimated to be approximately 35%, based on the average rate identified in the *ITE Trip Generation Handbook* for the Drive-in Bank land use. The primary and pass-by trips generated by the existing banking centre are summarized in **Table 6**.

Table 6: Existing Development – Primary and Pass-by Trips

Trip Type	AM Peak Hour (vph ⁽¹⁾)			PM Peak Hour (vph)		
	IN	OUT	TOT	IN	OUT	TOT
Existing Development Vehicle Trips	5	3	8	8	8	16
Pass-by	1	1	2	3	3	6
Primary	4	2	6	5	5	10

1. vph: Vehicle Trips per Hour

3.1.1.2 Proposed Retail Trip Generation

Since the retail uses are not known at this time, the trips generated by the proposed retail uses have been estimated using the Shopping Center land use rates included in the *ITE Trip Generation Manual, 10th Edition*. The estimated number of person trips generated by the proposed ground-floor retail are shown in **Table 7**.

Table 7: Proposed Retail – Trip Generation

Land Use	ITE Code	GFA	AM Peak Hour (pph)			PM Peak Hour (pph)		
			IN	OUT	TOT	IN	OUT	TOT
Shopping Centre	820	2,792 ft ²	3	1	4	6	8	14

Based on the previous table, the proposed retail is estimated to generate four person trips during the AM peak hour and 14 person trips during the PM peak hour. A breakdown of these trips by modal share is shown in **Table 8**.

Table 8: Proposed Retail – Trips by Mode Share

Travel Mode	Mode Share		AM Peak Hour			PM Peak Hour		
	AM	PM	IN	OUT	TOT	IN	OUT	TOT
Peak Hour Person Trips			3	1	4	6	8	14
Auto Driver	15%	15%	1	0	1	1	1	2
Auto Passenger	5%	5%	0	0	0	0	0	0
Transit	35%	15%	1	0	1	1	2	3
Cyclist	5%	5%	0	0	0	0	0	0
Pedestrian	40%	60%	1	1	2	4	5	9

From the previous table, the proposed retail is estimated to generate one vehicle trip during the AM peak hour and two vehicle trips during the PM peak hour.

Given the low projected vehicular volumes generated by the proposed retail, it is not assumed to generate any pass-by trips. This is further justified since the site will only be accessed via Sidney Street, whereas the existing banking centre includes an access to Carling Avenue as well as an access to Sidney Street.

3.1.1.3 Proposed Residential Trip Generation

The trips generated by the 459 proposed dwellings have been estimated using the *TRANS Trip Generation Manual*, which present peak hour trip generation rates for different types of housing for the AM and PM peak periods. For the High-Rise Multifamily Housing land use, the process of converting the trip generation estimates from peak period to peak hour is shown in the following tables.

The estimated number of person trips generated by the proposed dwellings for the AM and PM peak periods are shown in **Table 9**. A breakdown of these trips by modal share is shown in **Table 10**.

Table 9: Proposed Residential – Peak Period Trip Generation

Land Use	TRANS Rate	Units	AM Peak Period (ppp ⁽¹⁾)			PM Peak Period (ppp)		
			IN	OUT	TOT	IN	OUT	TOT
High-Rise Multifamily Housing	AM: 0.80 PM: 0.90	459	114	253	367	240	173	413

1. ppp: Person Trips per Peak Period

Table 10: Proposed Residential – Peak Period Trips by Mode Share

Travel Mode	Mode Share		AM Peak Period			PM Peak Period		
	AM	PM	IN	OUT	TOT	IN	OUT	TOT
Peak Period Person Trips			114	253	367	240	173	413
Auto Driver	15%	15%	17	38	55	36	26	62
Auto Passenger	5%	5%	6	12	18	12	9	21
Transit	40%	35%	46	101	147	84	60	144
Cyclist	5%	5%	5	13	18	12	9	21
Pedestrian	35%	40%	40	89	129	96	69	165

Table 4 of the *TRANS Trip Generation Manual* includes adjustment factors to convert the estimated number of trips generated for each mode from peak period to peak hour. A breakdown of the peak hour trips by mode is shown in **Table 11**.

Table 11: Proposed Residential – Peak Hour Trips by Mode Share

Travel Mode	Adj. Factor ⁽¹⁾		AM Peak Hour			PM Peak Hour		
	AM	PM	IN	OUT	TOT	IN	OUT	TOT
Auto Driver	0.48	0.44	8	18	26	16	11	27
Auto Passenger	0.48	0.44	3	6	9	5	4	9
Transit	0.55	0.47	25	56	81	39	28	67
Cyclist	0.58	0.48	3	7	10	6	4	10
Pedestrian	0.58	0.52	23	51	74	50	36	86
Peak Hour Person Trips			62	138	200	116	83	199

1. Adjustment Factors are included in Table 4 of the *TRANS Trip Generation Manual*

From the previous table, the proposed high-rise dwellings are estimated to generate 200 person trips (including 26 vehicle trips) during the AM peak hour and 199 person trips (including 27 vehicle trips) during the PM peak hour.

3.1.1.4 Summary of Trip Generation Estimates

A summary of the peak hour person trips generated by the existing banking centre, proposed retail, and proposed residential are included in **Table 12**.

Table 12: Trip Generation Summary

Travel Mode	AM Peak Hour			PM Peak Hour		
	IN	OUT	TOT	IN	OUT	TOT
Existing Development						
Banking Centre Trips	30	22	52	56	56	112
Auto Driver	5	3	8	8	8	16
Auto Passenger	1	1	2	3	3	6
Transit	11	8	19	8	8	16
Cyclist	1	1	2	3	3	6
Pedestrian	12	9	21	34	34	68
Proposed Redevelopment						
Retail Trips	3	1	4	6	8	14
Auto Driver	1	0	1	1	1	2
Auto Passenger	0	0	0	0	0	0
Transit	1	0	1	1	2	3
Cyclist	0	0	0	0	0	0
Pedestrian	1	1	2	4	5	9
Residential Trips	62	138	200	116	83	199
Auto Driver	8	18	26	16	11	27
Auto Passenger	3	6	9	5	4	9
Transit	25	56	81	39	28	67
Cyclist	3	7	10	6	4	10
Pedestrian	23	51	74	50	36	86
Net Additional Trips	35	117	152	66	35	101
Auto Driver	4	15	19	9	4	13
Auto Passenger	2	5	7	2	1	3
Transit	15	48	63	32	22	54
Cyclist	2	6	8	3	1	4
Pedestrian	12	43	55	20	7	27

From the previous table, the proposed development is estimated to generate an additional 152 person trips (including 19 vehicle trips) during the AM peak hour and 101 person trips (including 13 vehicle trips) during the PM peak hour.

It is likely that a percentage of the trips generated by the proposed development will be internally captured (for example, residents of the building making a trip to any of the businesses on the ground floor). No deduction has been made to account for internally captured trips, as the proposed retail trip generation is relatively low. Therefore, all trips generated by the proposed retail is assumed to have an origin or destination beyond the subject site. This simplifying assumption also allows for a more conservative analysis.

3.1.2 Trip Distribution

The assumed distribution of trips generated by the existing and proposed developments have been derived from existing traffic patterns within the study area and logical trip routing. Site-generated retail trips are anticipated to follow the two-way traffic patterns of the PM peak hour, and site-generated residential trips are anticipated to follow the traffic patterns associated with the typical commute (i.e. departing the study area during the AM peak and arriving during the PM peak). The distribution of site-generated trips can be described as follows.

Existing Bank and Proposed Retail Distribution

- 5% to/from the north via Champagne Avenue;
- 15% to/from the north via Preston Street;
- 5% to/from the north via Booth Street;
- 20% to/from the east via Carling Avenue;
- 10% to/from the east via Queen Elizabeth Driveway;
- 5% to/from the west via Sherwood Drive;
- 25% to/from the west via Carling Avenue;
- 15% to/from the west via Prince of Wales Drive.

Proposed Residential Distribution

- 10% to/from the north via Preston Street;
- 10% to/from the north via Booth Street;
- 20% to/from the east via Carling Avenue;
- 10% to/from the east via Beech Street;
- 15% to/from the east via Queen Elizabeth Driveway;
- 5% to/from the west via Sherwood Drive;
- 15% to/from the west via Carling Avenue;
- 15% to/from the west via Prince of Wales Drive.

3.1.3 Trip Assignment

Trips generated by the existing banking centre have been assigned to the Carling Avenue and Sidney Street accesses. Based on the origin/destination, some trips are anticipated to arrive at one access but exit at the other. This reflects the existing condition that the Carling Avenue access is restricted to right-in/right-out (RIRO) only, whereas the Preston Street/Sidney Street intersection allows all movements. The trip assignment for the existing banking centre trips can be summarized as follows.

Carling Avenue Access

- 100% of pass-by trips;
- 100% of inbound trips from the north via Booth Street, the east via Carling Avenue and Queen Elizabeth Driveway, and the west via Prince of Wales Drive;
- 100% of outbound trips to the north via Champagne Avenue, and the west via Sherwood Drive and Carling Avenue.

Sidney Street Access

- 100% of inbound trips from the north via Preston Street and Champagne Avenue, and the west via Sherwood Drive and Carling Avenue;
- 100% of outbound trips to the north via Preston Street and Booth Street, the east via Carling Avenue and Queen Elizabeth Driveway, and the west via Prince of Wales Drive.

All trips generated by the proposed development will enter and exit the site via the two proposed one-way accesses to Sidney Street.

3.2 Background Traffic

3.2.1 Other Area Developments

A review of other area development traffic has been conducted, per the developments listed in Section 2.2.2. Traffic generated by the following other area developments that under construction, approved, or are in the approval process have been considered for this report. Relevant excerpts of the traffic studies associated with the developments below are included in **Appendix G**.

17 Aberdeen Street

A residential expansion including 197 dwellings is proposed at 17 Aberdeen Street. A Transportation Overview was prepared by IBI Group in August 2016, in support of a Site Plan Control application for this development. The study identified that the expansion would generate an increase of 40 vph during the AM peak hour and 50 vph during the PM peak hour. No trip distribution or site-generated traffic figures were developed as part of the Transportation Overview. Traffic generated by this development have been added to the 2028 and 2033 background volumes, based on the trip distribution assumptions described in Section 3.1.2.

552 Booth Street

A mixed-use development including 1,000 dwellings and 142,200 ft² GFA of retail/office space is proposed at 552 Booth Street. A TIA was prepared by Parsons in December 2018, in support of Official Plan and Zoning By-Law Amendment applications for the development. The TIA presented trip generation projections for the 2025 buildout year using existing mode shares and the 2030 horizon year using target mode shares to reflect the City's initiative to increase transit ridership. In this scenario, the development is projected to generate approximately 175 vph during the peak hours. For the purposes of this TIA, the trip generation projections associated with the target mode shares have been added to the 2028 and 2033 background volumes.

289 Carling Avenue

A mixed-use development including 40 dwellings and 1,000 ft² GFA of ground-floor office space is proposed at 289 Carling Avenue. A TIA was prepared by CGH Transportation in August 2019, in support of the development. The TIA identified that the development would have negligible impact on the road network, and has not been added to the 2028 or 2033 background volumes.

845 Carling Avenue

A mixed-use development including 1,123 dwellings and 16,000 ft² GFA of ground-floor retail is proposed at 845 Carling Avenue. A Community Transportation Study/Transportation Impact Study (CTS/TIS) was prepared by Delcan in April 2013, in support of a Zoning By-Law Amendment application for the development. The CTS/TIS identified that the development would generate a net increase of approximately 150 vph during the AM peak hour and 175 vph during the PM peak hour. The study identified that construction of the development would be phased over a 15 to 20 year period, and the project has not advanced to the Site Plan Control stage. For the purposes of this TIA, 50% of traffic generated by this development has been added to the 2028 background volumes and 100% of traffic generated by this development has been added to the 2033 background volumes.

90 Champagne Avenue

A residential development including 236 dwellings is proposed at 90 Champagne Avenue. A TIA was prepared by Novatech in November 2019, in support of Zoning By-Law Amendment and Site Plan Control applications for the development. The TIA identified that the development would generate an increase of approximately 25 vph during the AM and PM peak hours. Traffic generated by this development has been added to the 2028 and 2033 background volumes.

101-105 Champagne Avenue

A residential development including 540 dwellings is currently under construction at 101 and 105 Champagne Avenue. A Transportation Overview and Parking Requirements Study were prepared by Delcan/Parsons in November 2011 and November 2015, respectively, in support of Official Plan Amendment, Zoning By-Law Amendment, and Site Plan Control applications for the development. The studies identified that the development would generate a net increase of approximately 55 vph during the peak hours. Traffic generated by this development has been added to the 2028 and 2033 background volumes.

93-105 Norman Street

A residential development including 117 dwellings is proposed at 93-105 Norman Street. A Transportation Brief and Addendum were prepared by Delcan in October 2013, in support of a Site Plan Control application for the development. The study identified that the development would generate a net increase of approximately 70 to 80 person trips during the peak hours, equating to approximately 15 vph assuming a 20% driver share. Traffic generated by this development have been added to the 2028 and 2033 background volumes, based on the trip distribution assumptions described in Section 3.1.2.

500 Preston Street

A mixed-use development including 280 dwellings and 10,000 ft² GFA of ground-floor retail is currently under construction at 500 Preston Street. A CTS dated June 2011, and subsequent addenda dated December 2012 and October 2013, were prepared by Delcan in support of a Site Plan Control for the development. The reports identified that the development would generate a net increase of approximately 95 vph during the AM peak hour and 110 vph during the PM peak hour. Traffic generated by this development has been added to the 2028 and 2033 background volumes.

505 Preston Street

A mixed-use development including 252 dwellings and 20,800 ft² GFA of office/retail uses is currently under construction at 505 Preston Street. A CTS dated December 2012, and Transportation Overview dated May 2013, were prepared by IBI Group in support of Zoning By-Law Amendment and Site Plan Control applications for the development. The studies identified that the development would generate a net increase of approximately 50 vph during the AM peak hour and 60 vph during the PM peak hour. Traffic generated by this development has been added to the 2028 and 2033 background volumes.

450 Rochester Street

A mixed-use development including 540 dwellings and 59,182 ft² GFA of commercial space is proposed at 450 Rochester Street. A TIA was prepared in October 2019 by Parsons, in support of Zoning By-Law Amendment and Site Plan Control applications for the development. The TIA identified that the development would generate a net increase of approximately 260 vph during the peak hours. Traffic generated by this development has been added to the 2028 and 2033 background volumes.

Ottawa Civic Hospital Expansion

An expansion of the Ottawa Civic Hospital is planned at 930 Carling Avenue. The development is anticipated to include approximately 800 beds and 2,000,000 ft² GFA of floor area. Buildout of the expansion is anticipated to occur in 2028. A traffic study has not been submitted to the City at the time of writing. Per discussions with City staff, high-level assumptions have been made to account for trips generated by the hospital expansion, which have been added to the 2028 and 2033 background volumes.

Similar to the process shown in Sections 3.1.1.1 and 3.1.1.2, the number of person trips generated by the new hospital expansion have been estimated using the Hospital land use rates included in the *ITE Trip Generation Manual, 10th Edition* and the City-prescribed ITE Trip to Person Trip Factor of 1.28, per the *2017 TIA Guidelines*. The number of vehicle trips generated by the new hospital has then been estimated using a 35% driver share, which is consistent with the driver share outlined in the *2011 TRANS O-D Survey Report* for the Ottawa Inner Area. A summary of the estimated number of vehicle trips generated by the hospital expansion is included in **Table 13**.

Table 13: Ottawa Civic Hospital Expansion – Trip Generation

Land Use	ITE Code	Units	AM Peak Hour (vph)			PM Peak Hour (vph)		
			IN	OUT	TOT	IN	OUT	TOT
Hospital	610	800 beds	470	180	650	200	500	700

From the previous table and for the purposes of this analysis, the hospital expansion is assumed to generate approximately 650 vehicle trips during the AM peak hour and 700 vehicle trips during the PM peak hour.

Based on conceptual plans released in 2018, it is assumed that all vehicle trips generated by the hospital expansion will access the hospital via a new south approach at Carling Avenue/Sherwood Drive or via a new access to Prince of Wales Drive west of Preston Street (outside of the study area). These assumed trips have been distributed to the road network using the same distribution as the existing/proposed commercial distribution described in Section 3.1.2.

Trips generated by the hospital expansion have been assigned to the assumed access locations as follows:

Carling Avenue/Sherwood Drive Intersection

- 100% of trips to/from the north via Champagne Avenue and Booth Street, the east via Carling Avenue, and the west via Sherwood Drive and Carling Avenue.

Prince of Wales Drive Access

- 100% of trips to/from the north via Preston Street, the east via Queen Elizabeth Driveway, and the west via Prince of Wales Drive.

3.2.2 General Background Growth Rate

A review of snapshots of the City's *Strategic Long-Range Model* and *Intersection Traffic Growth Rates (2000-2016)* has been conducted. Both resources are included in **Appendix H**. Comparing snapshots of the 2011 and 2031 AM peak hour traffic volumes, the *Strategic Long-Range Model* suggests positive growth on all arterial roadways, ranging from approximately 0.5% per annum on Preston Street to 4% per annum of Prince of Wales Drive. The *Intersection Traffic Growth Rates* figures, which determine growth rates based on total vehicular volumes entering the intersection, identify the following growth rates between 2000 and 2016:

- Carling Avenue/Sherwood Drive
 - AM Peak Hour: negative growth between -0.2% and -2% per annum;
 - PM Peak Hour: positive growth between +0.2% and +2% per annum.
- Carling Avenue/Champagne Avenue
 - AM Peak Hour: negative growth between -0.2% and -2% per annum;
 - PM Peak Hour: positive growth between +0.2% and +2% per annum.
- Carling Avenue/Preston Street
 - AM Peak Hour: negative growth between -0.2% and -2% per annum;
 - PM Peak Hour: negative growth between -0.2% and -2% per annum.
- Carling Avenue/Booth Street
 - AM Peak Hour: negative growth between -0.2% and -2% per annum;
 - PM Peak Hour: negative growth between -0.2% and -2% per annum.
- Preston Street/Beech Street
 - AM Peak Hour: negative growth between -0.2% and -2% per annum;
 - PM Peak Hour: negative growth between -0.2% and -2% per annum.
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - AM Peak Hour: negative growth between -0.2% and -2% per annum;
 - PM Peak Hour: no growth (i.e. between -0.2% and +2% per annum).

It is anticipated that background growth along the study area roadways will be captured through the addition of traffic generated by other area developments, as described in the previous section. Therefore, no background growth rates have been applied to any of the study area roadways.

East-west traffic along Carling Avenue is forecasted to decrease by as much as 20% in the peak direction and 15% in the off-peak direction by 2031, through implementation of the Carling Avenue transit priority measures shown in **Figure 4**. It is anticipated that the measures will be implemented by the buildout year 2028, and will progressively reduce east-west traffic volumes. A reduction of 10% in east-west traffic on Carling Avenue has been assumed for the 2028 background traffic conditions for both the peak and off-peak directions. The full reductions of 20% in the peak direction and 15% in the off-peak direction have been assumed for the 2033 background traffic conditions.

3.2.3 Existing Traffic Volume Balancing

To account for discrepancies in the different traffic counts conducted, the existing through traffic volumes have been balanced throughout the study area for any through movements with discrepancies of greater than 10%. Volumes between the study area intersections along the Carling Avenue and Preston Street corridors have therefore been adjusted to be within 10% of each other. The balanced existing traffic volumes are shown in **Figure 6**.

3.3 Future Traffic Conditions

The figures listed below present the following future traffic conditions:

- Existing site-generated traffic volumes are shown in **Figure 7**;
- Proposed site-generated traffic volumes are shown in **Figure 8**;
- Net site-generated traffic volumes are shown in **Figure 9**;
- Other area development-generated traffic volumes in 2028 are shown in **Figure 10**;
- Other area development-generated traffic volumes in 2033 are shown in **Figure 11**;
- Background traffic volumes in 2028 are shown in **Figure 12**;
- Background traffic volumes in 2033 are shown in **Figure 13**;
- Total traffic volumes in 2028 are shown in **Figure 14**;
- Total traffic volumes in 2033 are shown in **Figure 15**.

3.4 Demand Rationalization

A review of the existing and background intersection operations has been conducted to determine if and when traffic volumes exceed capacity within the study area. The intersection parameters used in the analysis are consistent with the *2017 TIA Guidelines* (Saturated Flow Rate: 1,800 vphpl, Peak Hour Factor: 0.9 in existing conditions and 1.0 in future conditions). Per Exhibit 22 of the *Multi-Modal Level of Service (MMLOS) Guidelines*, the target vehicular level of service (Auto LOS) at all study area intersections is an Auto LOS E, which equates to a vehicle-to-capacity (v/c) ratio of 1.00.

Intersection and lane geometry for all future conditions is consistent with the planned roadway modifications within the study area (for example, lane reductions on Carling Avenue and a new northbound approach at Carling Avenue/Sherwood Drive are included). Signal timing plans were obtained from the City, and are included in **Appendix I**.

3.4.1 Existing Intersection Operations

Intersection capacity analysis has been conducted for the existing traffic conditions. The results of the analysis are summarized in **Table 14** and **Table 15** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix J**.

Figure 6: Balanced Existing Traffic Volumes

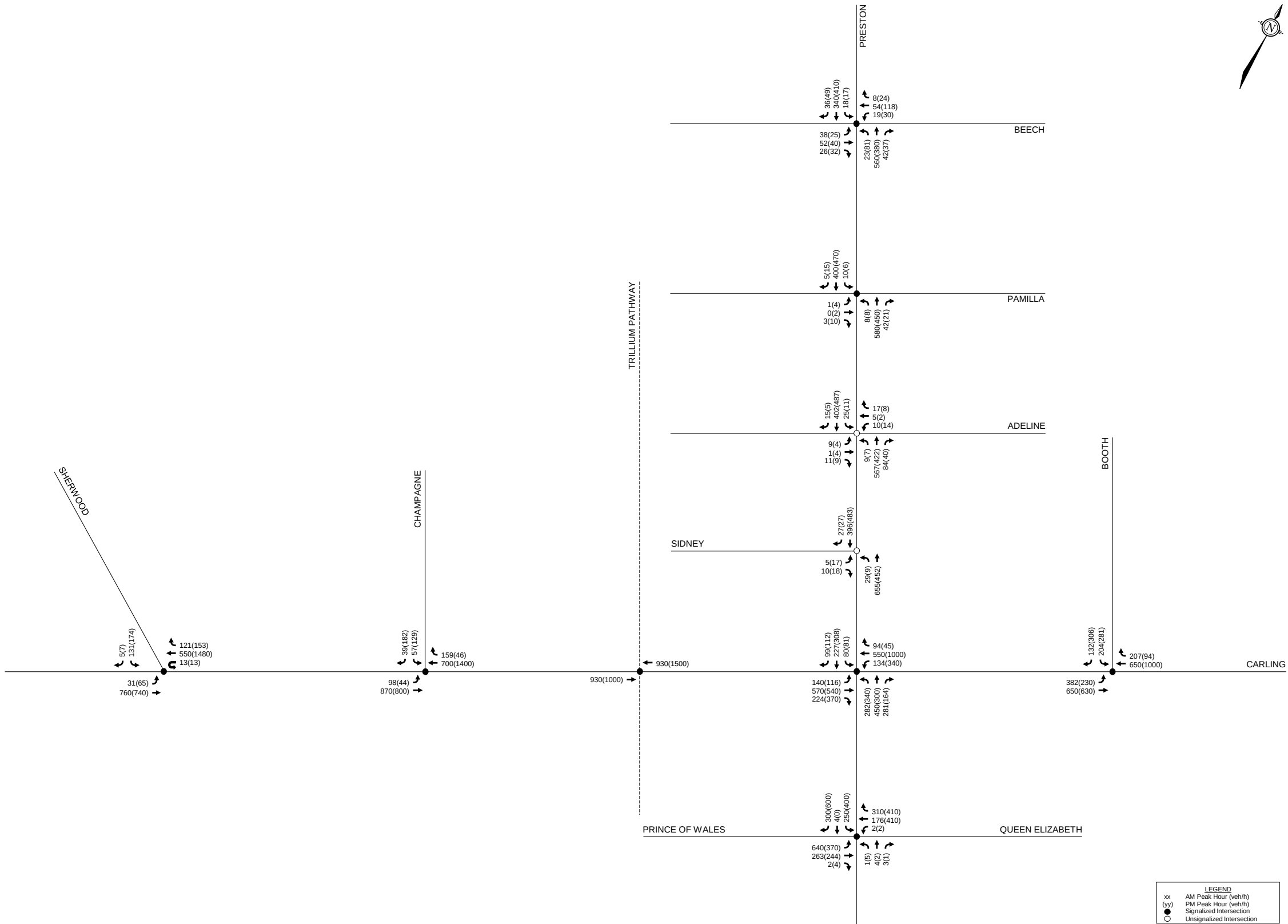


Figure 7: Existing Site-Generated Traffic Volumes

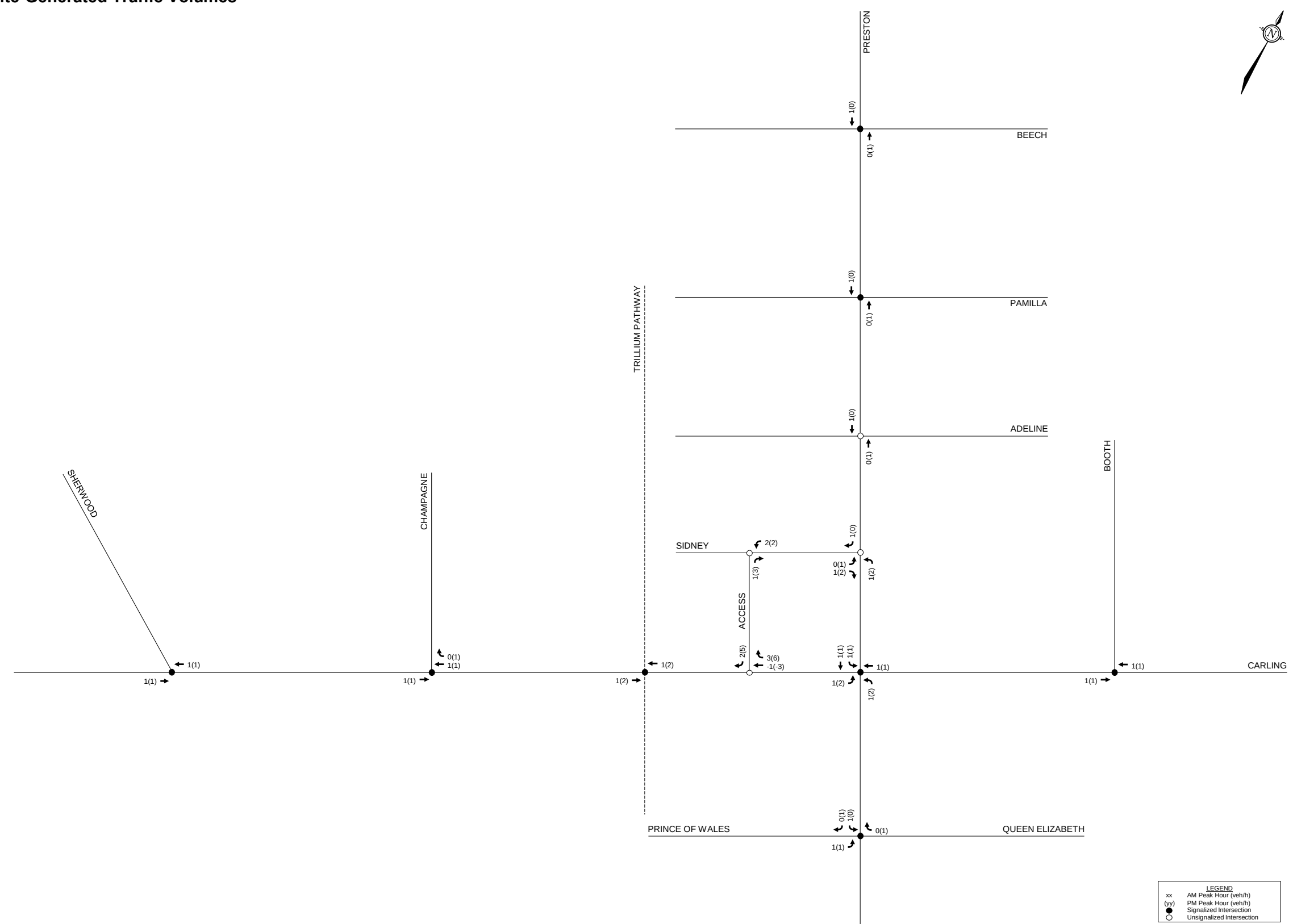


Figure 8: Proposed Site-Generated Traffic Volumes

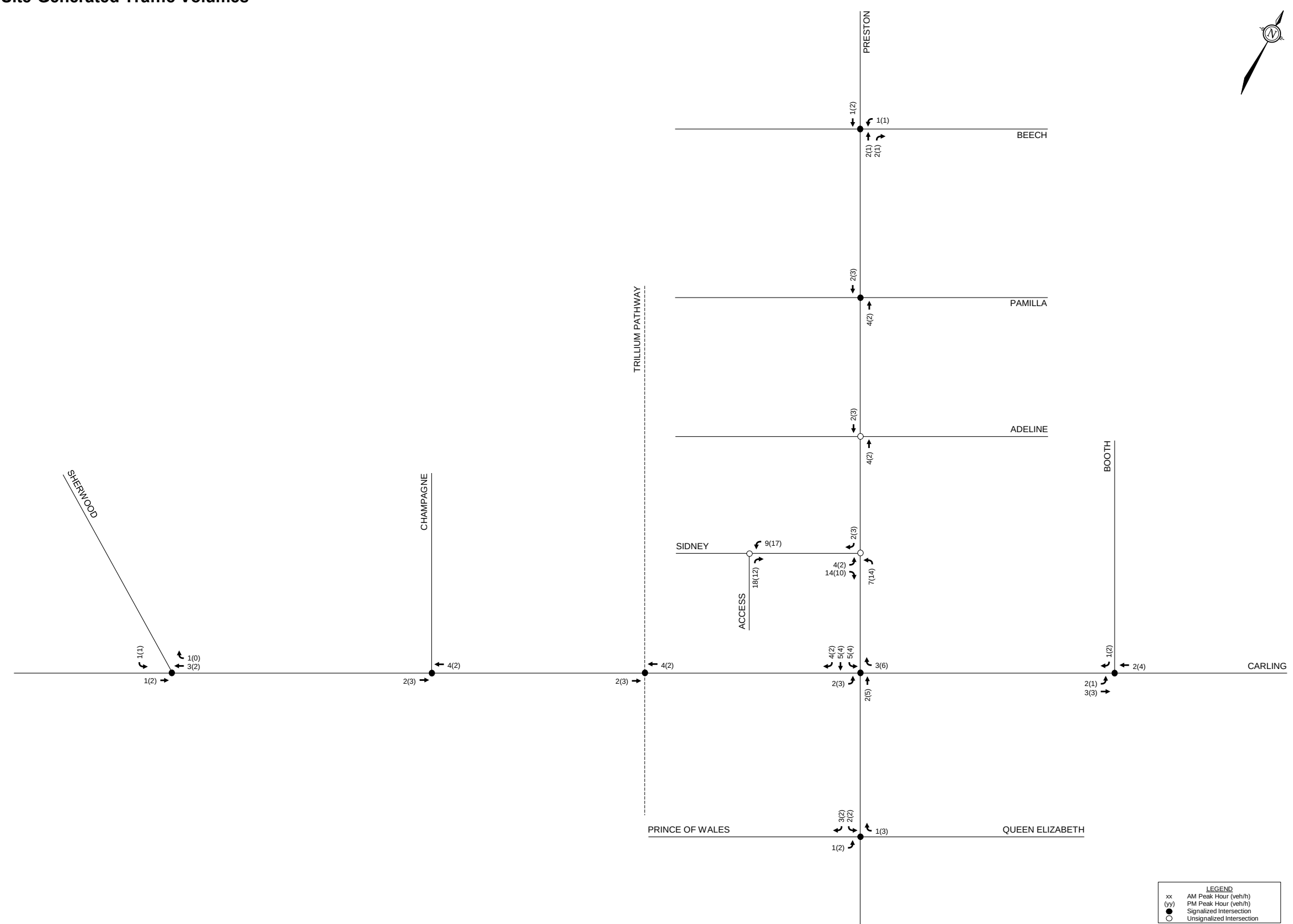


Figure 9: Net Site-Generated Traffic Volumes

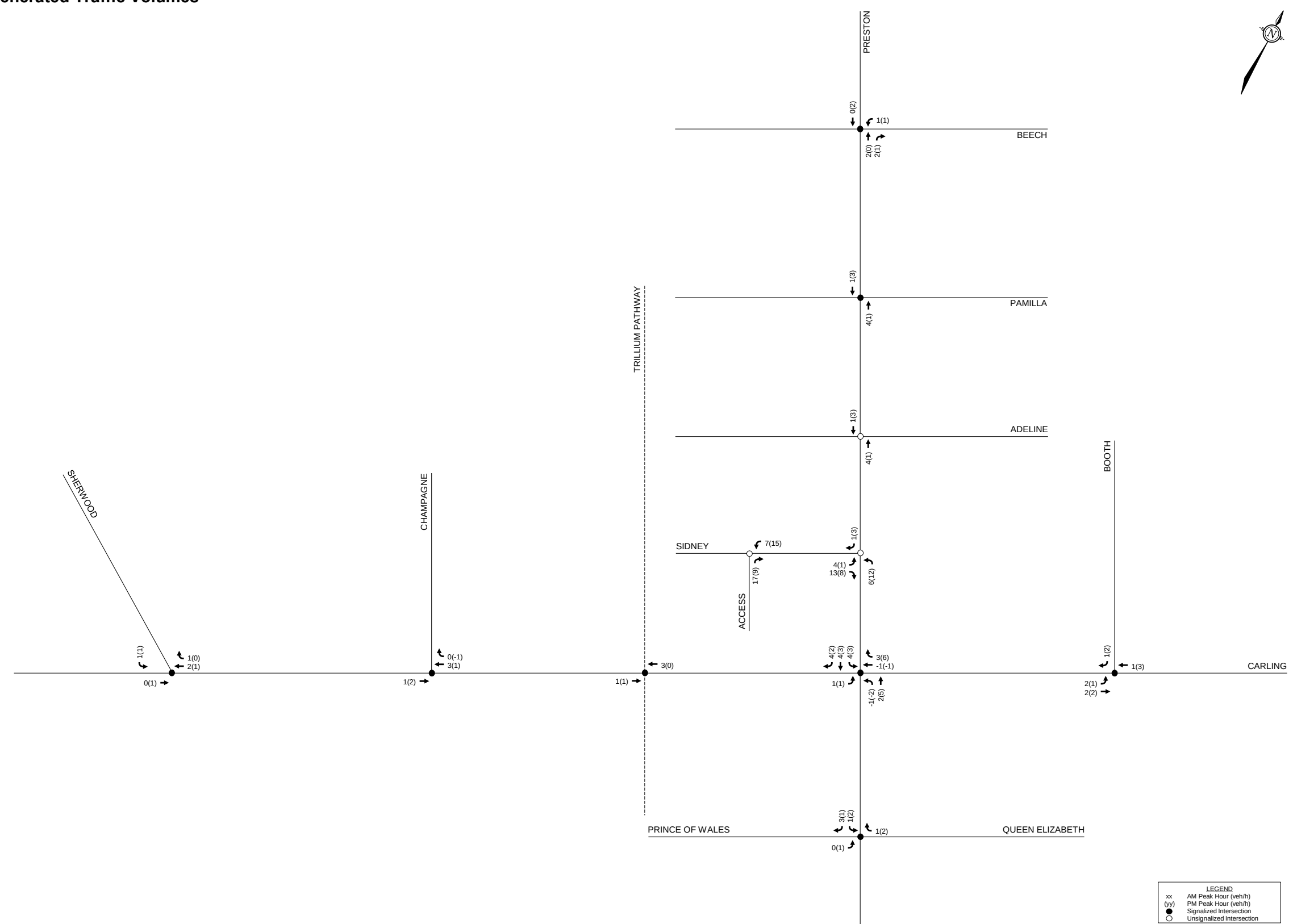


Figure 10: 2028 Other Area Development-Generated Traffic Volumes

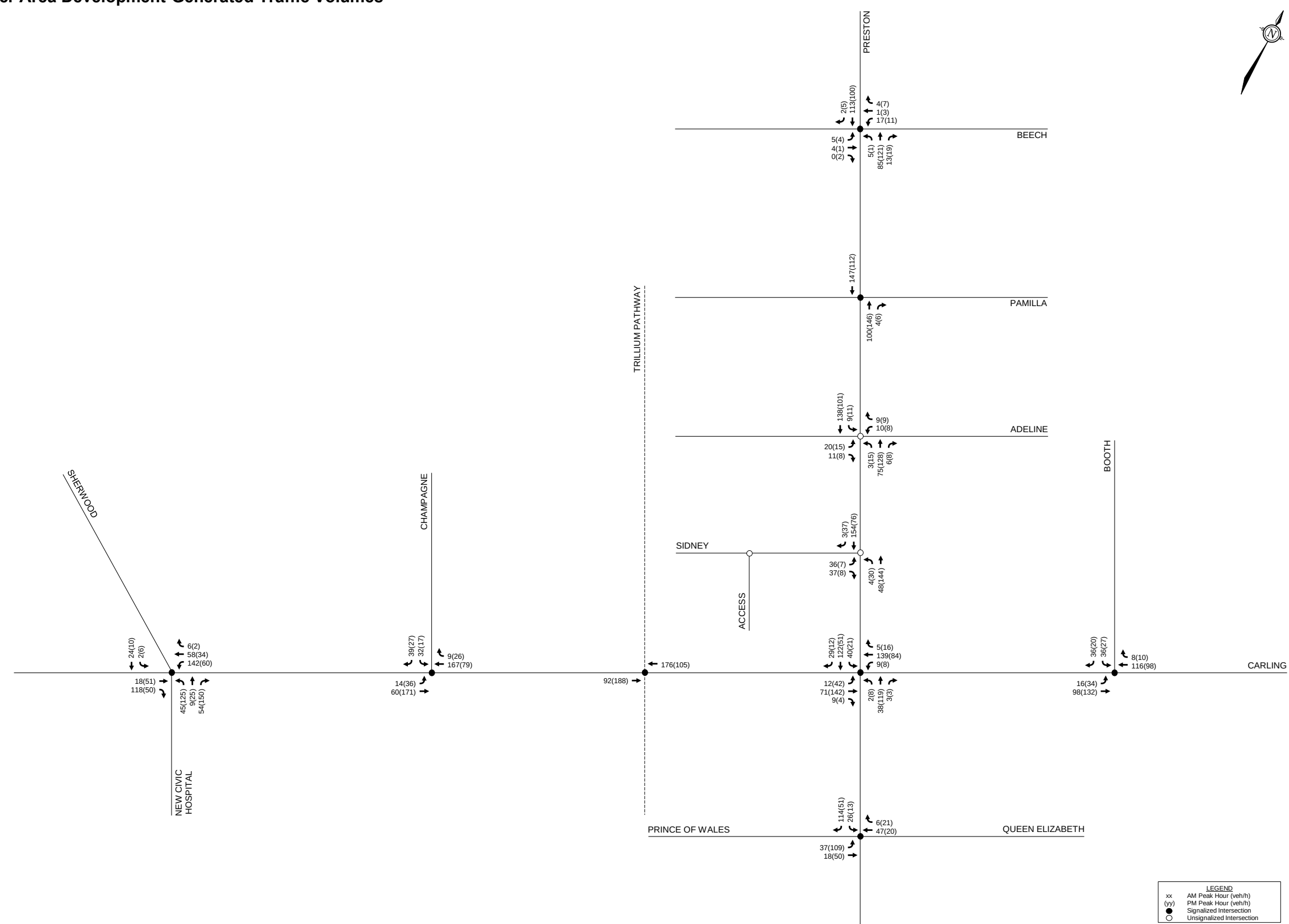


Figure 11: 2033 Other Area Development-Generated Traffic Volumes

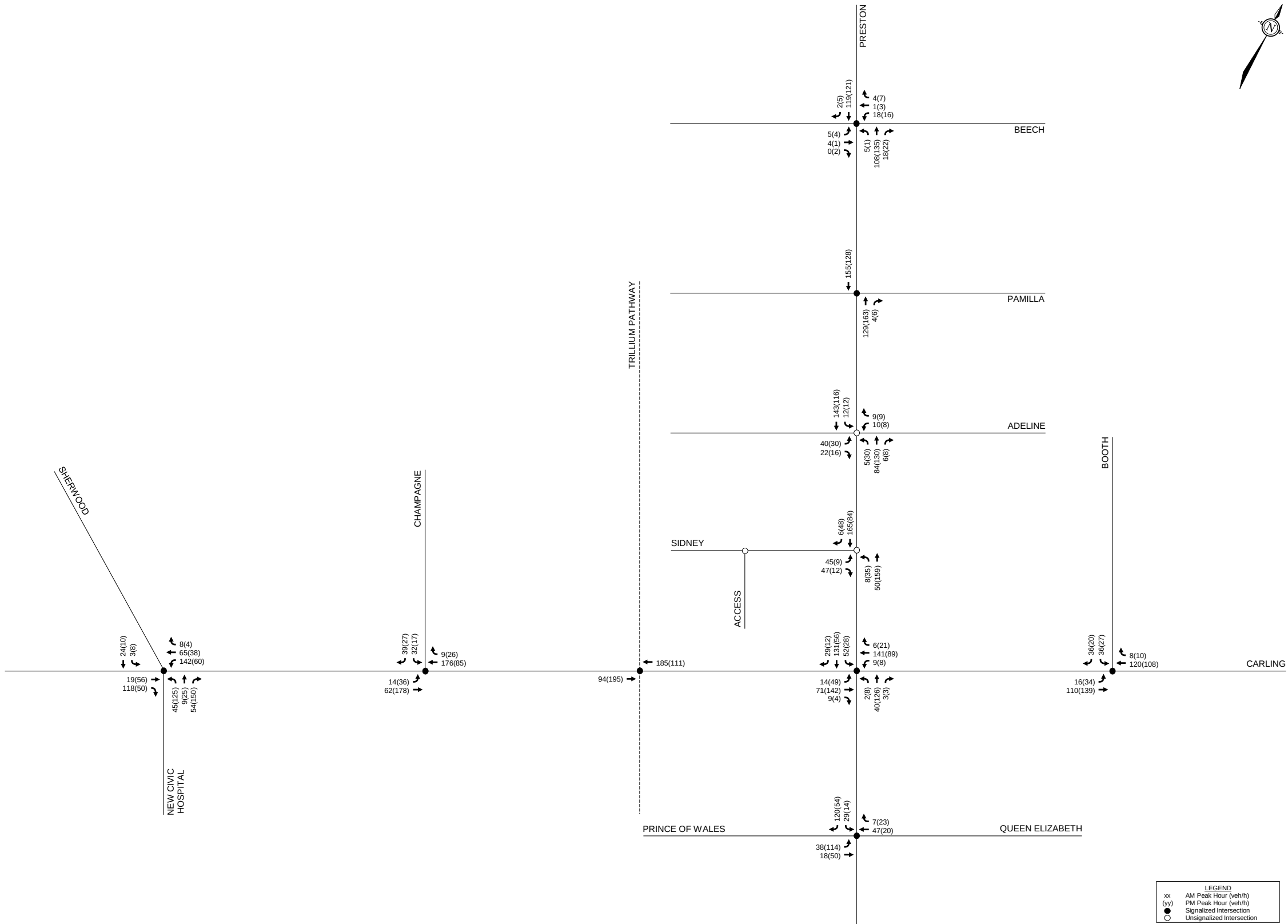


Figure 12: 2028 Background Traffic Volumes

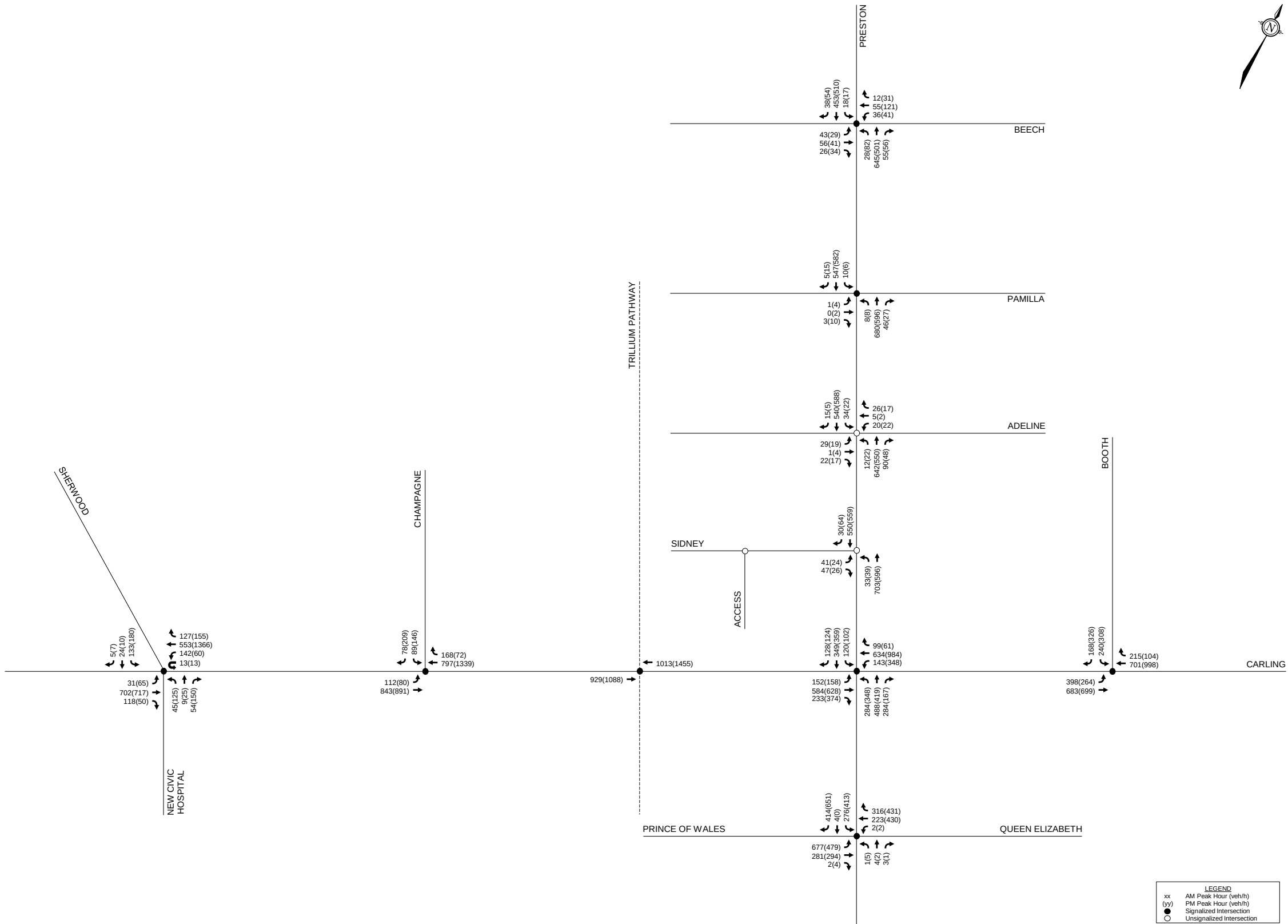


Figure 13: 2033 Background Traffic Volumes

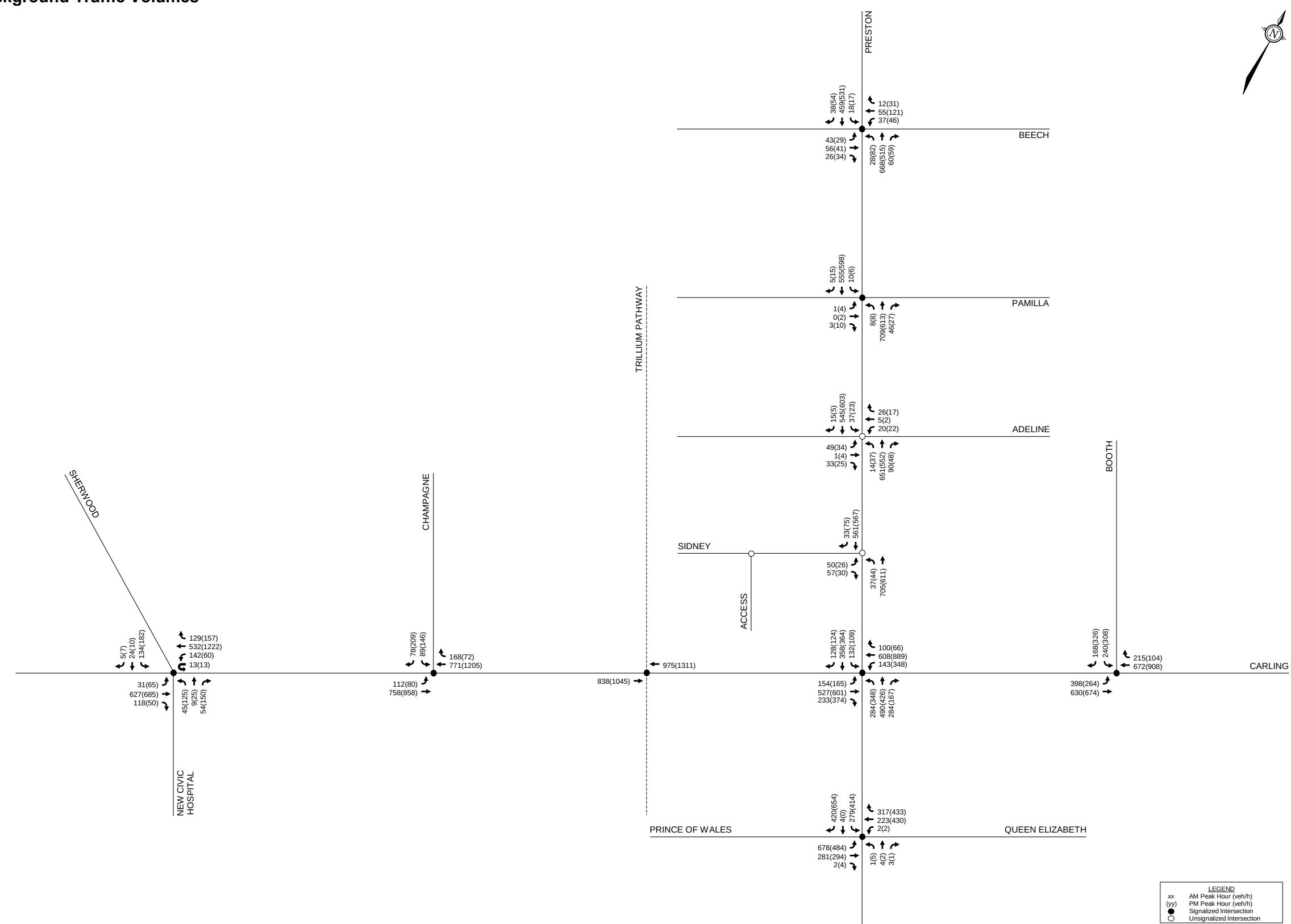


Figure 14: 2028 Total Traffic Volumes

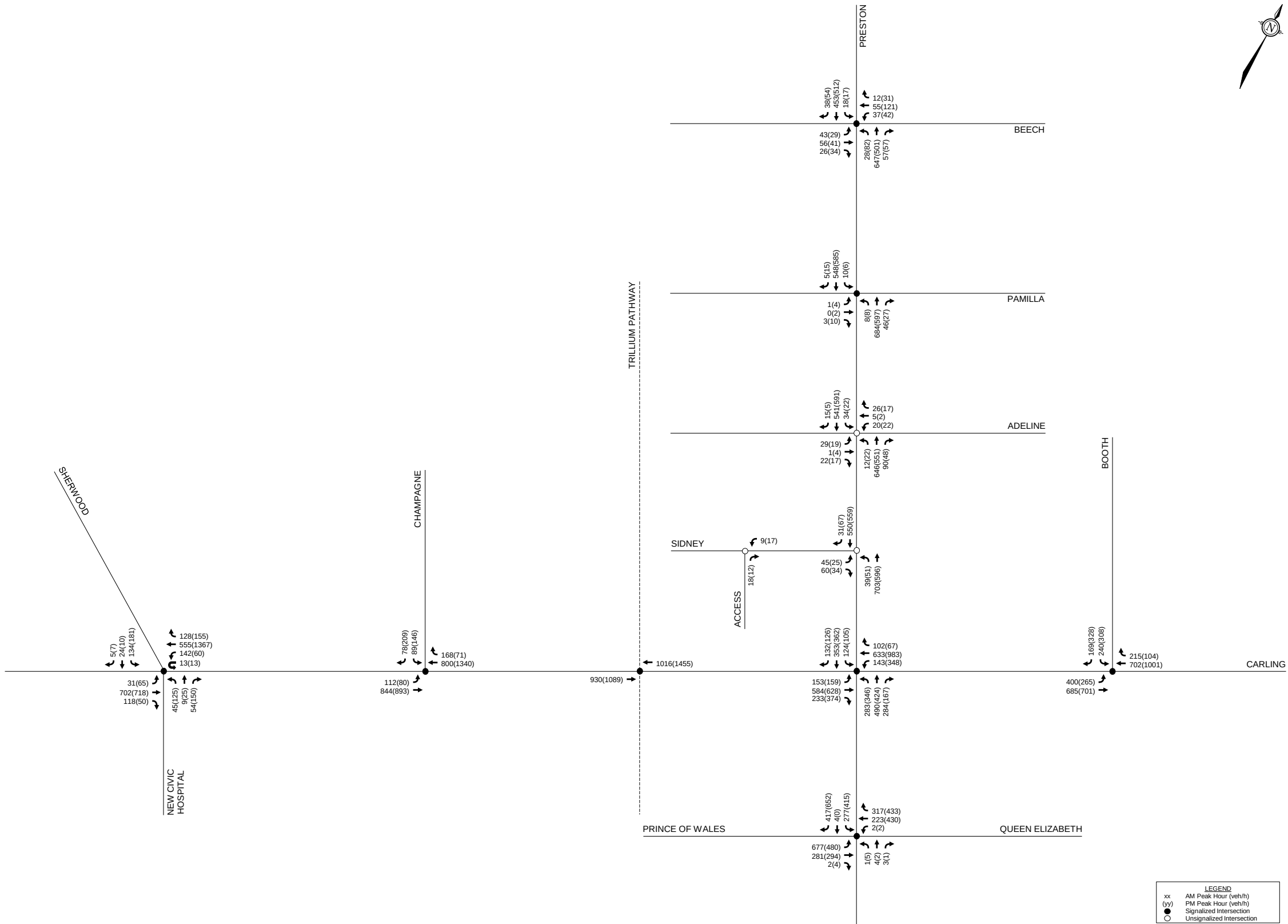


Figure 15: 2033 Total Traffic Volumes

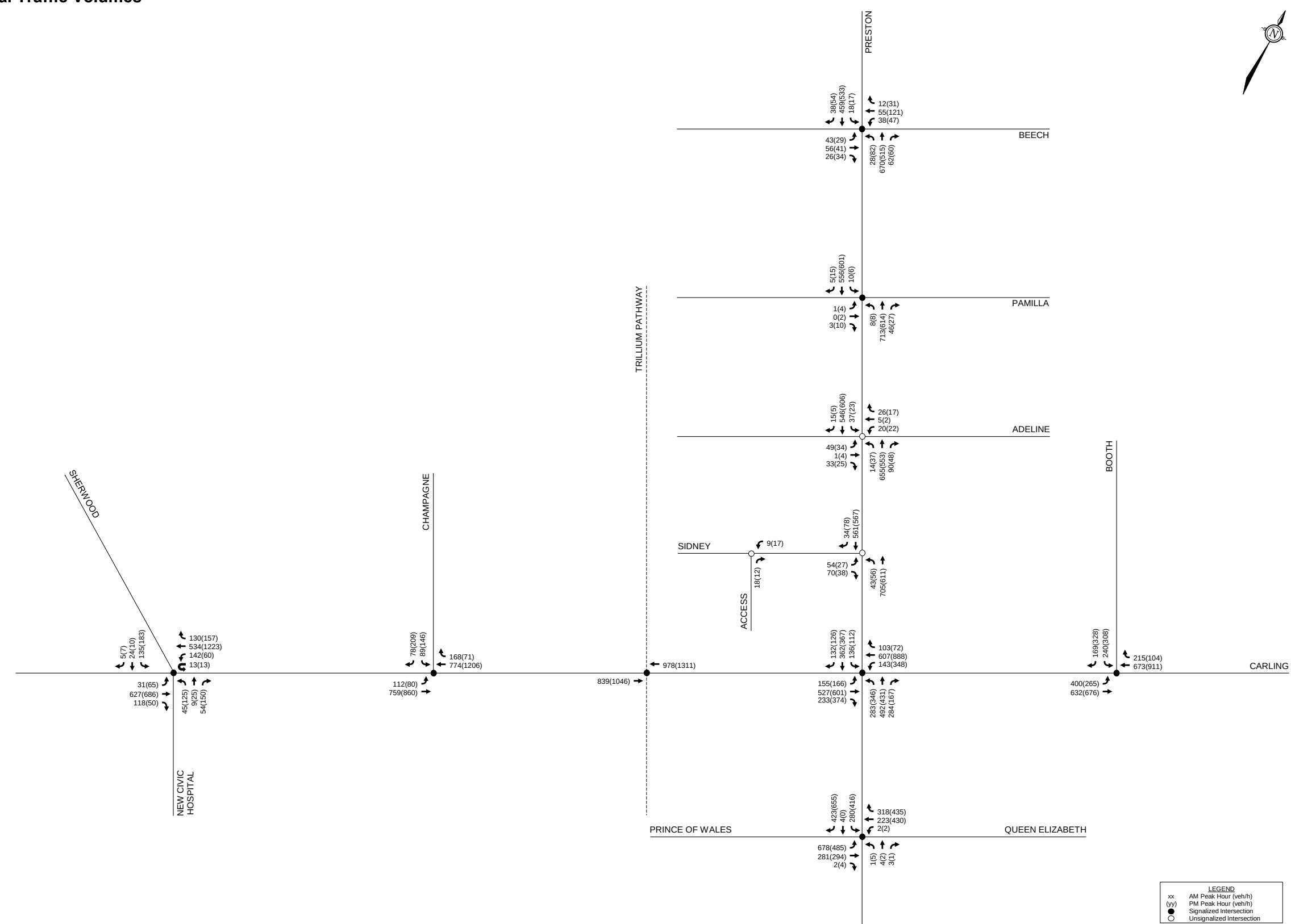


Table 14: Existing Traffic Operations

Intersection	Period	Critical Movements			Intersection		
		Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS
Carling Avenue/ Sherwood Drive ⁽¹⁾	AM	0.52	A	SBL	0.30	12 sec	A
	PM	0.69	B	SBL	0.63	19 sec	B
Carling Avenue/ Champagne Avenue ⁽¹⁾	AM	0.39	A	EBT	0.22	6 sec	A
	PM	0.59	A	WBT	0.58	10 sec	A
Carling Avenue/ Trillium Pathway ⁽²⁾	AM	0.39	A	EBT	0.34	3 sec	A
	PM	0.45	A	WBT	0.45	7 sec	A
Carling Avenue/ Preston Street ⁽¹⁾	AM	0.85	D	NBL	0.79	41 sec	C
	PM	1.44	F	NBL	1.08	83 sec	F
		1.03	F	SBT/R			
		1.33	F	WBL			
Carling Avenue/ Booth Street ⁽¹⁾	AM	0.84	D	EBL	0.59	25 sec	A
	PM	0.85	D	SBL	0.59	23 sec	A
Preston Street/ Beech Street ⁽¹⁾	AM	0.61	B	NBT	0.54	12 sec	A
	PM	0.54	B	WBL/T	0.42	12 sec	A
Preston Street/ Pamilla Street ⁽¹⁾	AM	0.47	A	NBT	0.41	4 sec	A
	PM	0.38	A	SBT	0.37	4 sec	A
Preston Street/ Adeline Street ⁽³⁾	AM	23 sec	C	WBL/T/R	-		
	PM	20 sec	C	WBL/T/R			
Preston Street/ Sidney Street ⁽³⁾	AM	15 sec	B	EBL/R	-		
	PM	17 sec	C	EBL/R			
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway ⁽¹⁾	AM	1.16	F	SBL/T	0.84	30 sec	D
	PM	1.21	F	SBL/T	0.98	60 sec	E

1. Signalized intersection

2. Signalized pathway crossing

3. Unsignalized intersection

Table 15: Existing Queues

Intersection	Mvmt	Storage/ Spacing ⁽¹⁾	AM Peak			PM Peak		
			v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Carling Avenue/ Preston Street	NBL	75m	0.85 [D]	55	m#74	1.44 [F]	~116	#174
	SBT/R	35m	0.74 [C]	67	96	1.03 [F]	~125	#186
	EBL	65m	0.83 [D]	37	#76	0.68 [B]	33	55
	WBL	75m	0.78 [C]	34	#71	1.33 [F]	~125	#182
	WBT	50m	0.65 [B]	21	51	0.81 [D]	100	#134
Carling Avenue/ Booth Street	SBL	--	0.68 [B]	47	64	0.83 [D]	68	98
	EBL	45m	0.84 [D]	75	#112	0.76 [C]	23	48
Preston Street/ Prince of Wales Drive/Queen Elizabeth Driveway	SBL/T	135m	1.16 [F]	~71	m#118	1.21 [F]	~128	#186
	EBL	55m	0.87 [D]	74	#115	0.87 [D]	68	#113

1. Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

m: volume for the 95th percentile queue is metered by an upstream signal#: volume for the 95th percentile cycle exceeds capacity

~: approach is above capacity

From the previous tables, the northbound left turn, southbound through/right turn, and westbound left turn movements at Carling Avenue/Preston Street, and the southbound left turn/through movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target Auto LOS E. All other movements within the study area currently meet the City's target. A summary of the critical queueing at the study area intersections is provided below.

During the AM peak hour, the average (50th-percentile) and maximum (95th-percentile) queue lengths of the eastbound left turn movements at Carling Avenue/Booth Street and Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway exceed the storage lengths provided for those movements. Additionally, the maximum queue length of the eastbound left turn movement at Carling Avenue/Preston Street exceeds the storage length provided. Both the average and maximum queue lengths for the southbound through/right turn movement at Carling Avenue/Preston Street extend through the upstream intersection at Preston Street/Sidney Street.

During the PM peak hour, the average and maximum queue lengths of the northbound and westbound left turn movements at Carling Avenue/Preston Street and the eastbound left turn movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway exceed the storage lengths provided for these movements. Further, the maximum queue length of the eastbound left turn movement at Carling Avenue/Booth Street exceed the storage length provided. Both the average and maximum queue lengths of the southbound through/right turn and westbound through movements at Carling Avenue/Preston Street extend through the upstream intersections of Preston Street/Adeline Street and Carling Avenue/Norfolk Avenue, respectively. Additionally, the maximum queue length of the southbound left turn/through movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway extends through the upstream intersection at Carling Street/Preston Street.

The approximate required reduction in volumes to meet the target Auto LOS for each over-capacity movement is included below.

AM Peak Hour

- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - Southbound left turn/through (v/c: 1.13): reduction of 40 vehicles required.

PM Peak Hour

- Carling Avenue/Preston Street
 - Northbound left turn (v/c: 1.44): reduction of 110 vehicles required;
 - Southbound through/right turn (v/c: 1.03): reduction of 20 vehicles required;
 - Westbound left turn (v/c: 1.33): reduction of 90 vehicles required.
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - Southbound left turn/through (v/c: 1.21): reduction of 70 vehicles required.

3.4.2 2028 Background Intersection Operations

Intersection capacity analysis has been conducted for the 2028 background traffic conditions. Signal timing plans within the study area have been optimized to reflect the planned changes in the roadway network. The results of the analysis are summarized in **Table 16** and **Table 17** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix K**.

Table 16: 2028 Background Traffic Operations

Intersection	Period	Critical Movements			Intersection		
		Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS
Carling Avenue/ Sherwood Drive ⁽¹⁾	AM	0.57	A	SBL	0.34	11 sec	A
	PM	0.73	C	SBL	0.66	16 sec	B
Carling Avenue/ Champagne Avenue ⁽¹⁾	AM	0.36	A	EBT	0.33	6 sec	A
	PM	0.69	B	SBR	0.55	11 sec	A
Carling Avenue/ Trillium Pathway ⁽²⁾	AM	0.39	A	WBT	0.38	4 sec	A
	PM	0.53	A	WBT	0.47	3 sec	A
Carling Avenue/ Preston Street ⁽¹⁾	AM	0.99	E	EBT	0.98	47 sec	E
	PM	1.37	F	NBL	1.20	96 sec	F
		1.10	F	SBT/R			
		1.14	F	EBT/R			
		1.34	F	WBL			
Carling Avenue/ Booth Street ⁽¹⁾	AM	1.01	F	EBL	0.95	42 sec	E
	PM	1.14	F	WBT	1.07	62 sec	F
Preston Street/ Beech Street ⁽¹⁾	AM	0.64	A	NBT	0.57	12 sec	A
	PM	0.55	A	WBL/T	0.47	12 sec	A
Preston Street/ Pamilla Street ⁽¹⁾	AM	0.50	A	NBT	0.46	4 sec	A
	PM	0.44	A	NBT	0.43	5 sec	A
Preston Street/ Adeline Street ⁽³⁾	AM	30 sec	C	WBL/T/R	-		
	PM	25 sec	C	WBL/T/R			
Preston Street/ Sidney Street ⁽³⁾	AM	22 sec	B	EBL/R	-		
	PM	15 sec	C	EBL/R			
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway ⁽¹⁾	AM	1.15	F	SBL/T	0.82	27 sec	D
	PM	1.13	F	SBL/T	0.98	56 sec	E

1. Signalized intersection

2. Signalized pathway crossing

3. Unsignalized intersection

Table 17: 2028 Background Queues

Intersection	Mvmt	Storage/ Spacing ⁽¹⁾	AM Peak			PM Peak		
			v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Carling Avenue/ Preston Street	NBL	75m	0.96 [E]	37	m#70	1.37 [F]	~104	#161
	SBL	35m	0.69 [B]	23	#49	0.57 [A]	23	42
	SBT/R	35m	0.99 [E]	100	#163	1.10 [F]	~137	#199
	EBL	65m	0.86 [D]	27	#63	0.93 [E]	35	#78
	EBT/R	95m	0.99 [E]	45	#123	1.14 [F]	~149	#180
	WBL	110m	0.88 [D]	28	m33	1.34 [F]	~114	m#114
	WBT	50m	0.80 [C]	76	m78	0.92 [E]	138	m126
Carling Avenue/ Booth Street	SBL	--	0.71 [C]	50	68	0.88 [D]	74	#114
	EBL	75m	1.01 [F]	~85	m#145	1.00 [E]	~77	m#84
	WBT	85m	0.99 [E]	149	#222	1.14 [F]	~296	#369
Preston Street/ Prince of Wales Drive/Queen Elizabeth Driveway	SBL/T	135m	1.15 [F]	~70	m#74	1.13 [F]	~113	#170
	EBL	55m	0.85 [D]	68	#100	0.96 [E]	88	#149

1. Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

m: volume for the 95th percentile queue is metered by an upstream signal

#: volume for the 95th percentile cycle exceeds capacity

~: approach is above capacity

From the previous tables, the northbound left turn, southbound through/right turn, eastbound through/right turn, and westbound left turn movements at Carling Avenue/Preston Street, the eastbound left turn and westbound through movements at Carling Avenue/Booth Street, and the southbound left turn/through movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target Auto LOS E. All other intersections within the study area currently meet the City's target. A summary of the critical queueing at the study area intersections is provided below.

During the AM peak hour, the average and maximum queue lengths of the eastbound left turn movements at Carling Avenue/Booth Street and Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway exceed the storage lengths provided. Additionally, the maximum queue lengths of the southbound left turn movement at Carling Avenue/Preston Street exceed the storage lengths provided. Both the average and maximum queue lengths for the southbound through/right turn and westbound through movements at Carling Avenue/Preston Street extend through at least one upstream intersection (i.e. at Sidney Street and Adeline Street for the southbound movement, and at Norfolk Avenue for the westbound movement), and the maximum queue length for the eastbound through/right turn movement at Carling Avenue/Preston Street extends through the upstream intersection with the Trillium Pathway. Both the average and maximum queue lengths for the westbound through movement at Carling Avenue/Booth Street extend through at least one upstream intersection east of the study area (i.e. at Lebreton Street and Bell Street).

During the PM peak hour, the average and maximum queue lengths of the northbound and westbound left turn movements at Carling Avenue/Preston Street and the eastbound left turn movements at Carling Avenue/Booth Street and Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway exceed the storage lengths provided. Additionally, the maximum queue lengths of the southbound left turn and eastbound left turn movements at Carling Avenue/Preston Street exceed the storage lengths provided. Both the average and maximum queue lengths of the southbound through/right turn, eastbound through/right turn, and westbound through movements at Carling Avenue/Preston Street extend through at least one upstream intersection (i.e. at Sidney Street and Adeline Street for the southbound movement, at the Trillium Pathway for the eastbound movement, and at Norfolk Avenue for the westbound movement). Additionally, both the average and maximum queue lengths for the westbound through movement at Carling Avenue/Booth Street extend through at least two upstream intersections east of the study area (i.e. at Lebreton Street, Bell Street, and Cambridge Street). The maximum queue length for the southbound left turn/through movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway extends to the upstream intersection at Carling Avenue/Preston Street.

The approximate required reduction in volumes to meet the target Auto LOS for each over-capacity movement is included below.

AM Peak Hour

- Carling Avenue/Booth Street
 - Eastbound left turn (v/c: 1.01): reduction of 10 vehicles required.
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - Southbound left turn/through (v/c: 1.15): reduction of 40 vehicles required.

PM Peak Hour

- Carling Avenue/Preston Street
 - Northbound left turn (v/c: 1.37): reduction of 80 vehicles required;
 - Southbound through/right turn (v/c: 1.10): reduction of 50 vehicles required;
 - Eastbound through/right turn (v/c: 1.14): reduction of 100 vehicles required;
 - Westbound left turn (v/c: 1.34): reduction of 90 vehicles required.
- Carling Avenue/Booth Street
 - Westbound through (v/c: 1.14): reduction of 120 vehicles required.
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - Southbound left turn/through (v/c: 1.13): reduction of 50 vehicles required.

3.4.3 2033 Background Intersection Operations

Intersection capacity analysis has been conducted for the 2033 background traffic conditions. Signal timing plans within the study area have been optimized to reflect the planned changes in the roadway network. The results of the analysis are summarized in **Table 18** and **Table 19** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix K**.

Table 18: 2033 Background Traffic Operations

Intersection	Period	Critical Movements			Intersection		
		Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS
Carling Avenue/ Sherwood Drive ⁽¹⁾	AM	0.58	A	SBL	0.32	11 sec	A
	PM	0.73	C	SBL	0.62	18 sec	B
Carling Avenue/ Champagne Avenue ⁽¹⁾	AM	0.33	A	EBT	0.32	8 sec	A
	PM	0.65	B	SBR	0.51	10 sec	A
Carling Avenue/ Trillium Pathway ⁽²⁾	AM	0.37	A	WBT	0.35	4 sec	A
	PM	0.48	A	WBT	0.44	3 sec	A
Carling Avenue/ Preston Street ⁽¹⁾	AM	1.01	F	SBT/R	0.95	54 sec	E
	PM	1.37	F	NBL	1.18	94 sec	F
		1.11	F	SBT/R			
		1.10	F	EBT/R			
		1.34	F	WBL			
Carling Avenue/ Booth Street ⁽¹⁾	AM	1.01	F	EBL	0.92	42 sec	E
	PM	1.04	F	WBT	1.00	48 sec	E
Preston Street/ Beech Street ⁽¹⁾	AM	0.66	B	NBT	0.59	12 sec	A
	PM	0.57	A	WBL/T	0.49	12 sec	A
Preston Street/ Pamilla Street ⁽¹⁾	AM	0.52	A	NBT	0.47	4 sec	A
	PM	0.45	A	NBT	0.44	5 sec	A
Preston Street/ Adeline Street ⁽³⁾	AM	40 sec	E	EBL/T/R	-		
	PM	30 sec	D	EBL/T/R			
Preston Street/ Sidney Street ⁽³⁾	AM	24 sec	C	EBL/R	-		
	PM	16 sec	C	EBL/R			
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway ⁽¹⁾	AM	1.16	F	SBL/T	0.82	28 sec	D
	PM	1.13	F	SBL/T	0.99	56 sec	E

1. Signalized intersection

2. Signalized pathway crossing

3. Unsignalized intersection

Table 19: 2033 Background Queues

Intersection	Mvmt	Storage/ Spacing ⁽¹⁾	AM Peak			PM Peak		
			v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Carling Avenue/ Preston Street	NBL	75m	0.98 [E]	39	m#73	1.37 [F]	~104	#161
	SBL	35m	0.75 [C]	26	#56	0.61 [B]	25	45
	SBT/R	35m	1.01 [F]	~104	#168	1.11 [F]	~140	#203
	EBL	65m	0.87 [D]	26	#63	0.97 [E]	36	#82
	EBT/R	95m	0.92 [E]	66	#105	1.10 [F]	~132	#173
	WBL	110m	0.88 [D]	33	m#40	1.34 [F]	~114	m#127
	WBT	50m	0.76 [C]	60	m67	0.83 [D]	122	m120
Carling Avenue/ Booth Street	SBL	--	0.71 [C]	50	68	0.87 [D]	74	#112
	EBL	75m	1.01 [F]	~90	m#155	0.99 [E]	~77	m#87
	WBT	85m	0.95 [E]	138	#208	1.04 [F]	~249	#320
Preston Street/ Prince of Wales Drive/Queen Elizabeth Driveway	SBL/T	135m	1.16 [F]	~71	m#78	1.13 [F]	~113	#171
	EBL	55m	0.85 [D]	68	#101	0.97 [E]	90	#153

1. Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

m: volume for the 95th percentile queue is metered by an upstream signal

#: volume for the 95th percentile cycle exceeds capacity

~: approach is above capacity

From the previous tables, the northbound left turn, southbound through/right turn, eastbound through/right turn, and westbound left turn movements at Carling Avenue/Preston Street, the eastbound left turn and westbound through movements at Carling Avenue/Booth Street, and the southbound left turn/through movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target Auto LOS E. All other intersections within the study area currently meet the City's target. A summary of the critical queueing at the study area intersections is provided below.

During the AM peak hour, the average and maximum queue lengths of the eastbound left turn movements at Carling Avenue/Booth Street and Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway exceed the storage lengths provided. Additionally, the maximum queue lengths of the southbound left turn movement at Carling Avenue/Preston Street exceed the storage lengths provided. Both the average and maximum queue lengths for the southbound through/right turn and westbound through movements at Carling Avenue/Preston Street extend through at least one upstream intersection (i.e. at Sidney Street and Adeline Street for the southbound movement, and at Norfolk Avenue for the westbound movement), and the maximum queue length for the eastbound through/right turn movement at Carling Avenue/Preston Street extends through the upstream intersection with the Trillium Pathway. Both the average and maximum queue lengths for the westbound through movement at Carling Avenue/Booth Street extend through at least one upstream intersection east of the study area (i.e. at Lebreton Street and Bell Street).

During the PM peak hour, the average and maximum queue lengths of the northbound and westbound left turn movements at Carling Avenue/Preston Street and the eastbound left turn movements at Carling Avenue/Booth Street and Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway exceed the storage lengths provided. Additionally, the maximum queue lengths of the southbound left turn and eastbound left turn movements at Carling Avenue/Preston Street exceed the storage lengths provided. Both the average and maximum queue lengths of the southbound through/right turn, eastbound through/right turn, and westbound through movements at Carling Avenue/Preston Street extend through at least one upstream intersection (i.e. at Sidney

Street and Adeline Street for the southbound movement, at the Trillium Pathway for the eastbound movement, and at Norfolk Avenue for the westbound movement). Additionally, both the average and maximum queue lengths for the westbound through movement at Carling Avenue/Booth Street extend through at least two upstream intersections east of the study area (i.e. at Lebreton Street, Bell Street, and Cambridge Street). The maximum queue length for the southbound left turn/through movement at Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway extends to the upstream intersection at Carling Avenue/Preston Street.

The approximate required reduction in volumes to meet the target Auto LOS for each over-capacity movement is included below.

AM Peak Hour

- Carling Avenue/Preston Street
 - Southbound through/right turn (v/c: 1.01): reduction of 10 vehicles required.
- Carling Avenue/Booth Street
 - Eastbound left turn (v/c: 1.01): reduction of 10 vehicles required.
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - Southbound left turn/through (v/c: 1.16): reduction of 40 vehicles required.

PM Peak Hour

- Carling Avenue/Preston Street
 - Northbound left turn (v/c: 1.37): reduction of 80 vehicles required;
 - Southbound through/right turn (v/c: 1.11): reduction of 50 vehicles required;
 - Eastbound through/right turn (v/c: 1.10): reduction of 70 vehicles required;
 - Westbound left turn (v/c: 1.34): reduction of 90 vehicles required.
- Carling Avenue/Booth Street
 - Westbound through (v/c: 1.04): reduction of 30 vehicles required.
- Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway
 - Southbound left turn/through (v/c: 1.13): reduction of 50 vehicles required.

Traffic throughout the study area could be displaced or alleviated through a combination of increase use of non-auto modes of transportation, alternate time to travel for drivers using the study area roadways to make use of off-peak capacity, and alternate routes for travel. A further description of each option is provided below.

Increased Use of Non-Auto Modes

It is assumed that the Carling Avenue Transit Priority Measures project will be completed by the buildout year 2028. These measures will provide more reliable transit between Lincoln Fields Station, Carling O-Train Station, and Bronson Avenue, connecting western Ottawa with the inner area and downtown core. This is anticipated to increase the transit modal share and decrease the auto modal share, thereby reducing traffic volumes within the study area. As stated in Section 3.2.2, a reduction in east-west traffic volumes on Carling Avenue is assumed starting in 2028 to reflect the implementation of the Carling Avenue Transit Priority Measures.

Alternate Travel Times

As congestion increases within the study area, some motorists may alter their travel to occur outside of the peak hours. This shift in travel times may result in a reduction of peak hour traffic volumes.

Alternate Routes of Travel

As congestion increases within the study area, some motorists may choose alternate routes of travel outside the study area. Alternative north-south routes outside of the study area include Parkdale Avenue and Bronson Avenue. Alternative east-west routes outside of the study area include Baseline Road, Highway 417, Gladstone Avenue, Somerset Street, Scott Street/Albert Street, and Sir John A. Macdonald Parkway.

4.0 ANALYSIS

4.1 Development Design

4.1.1 Design for Sustainable Modes

Concrete sidewalks will be provided around the north, south, and east sides of the proposed building, and will connect to existing sidewalks on Sidney Street, Preston Street, and Carling Avenue.

Fifteen bicycle parking spaces will be provided in three outside parking areas along Carling Avenue and Preston Street. An additional 226 bicycle parking spaces will be provided in sheltered areas, on various levels of the parking garage. Further review of the bicycle parking requirements as outlined in the City's *Zoning By-Law* (ZBL) are included in Section 4.2.

OC Transpo stops #2397, #6655, #6657, #7369, #8013, #8014, #8023, and the Carling O-Train Station are within 400m walking distance of all entrances to the proposed development. These stops provide service to OC Transpo routes 2, 55, 56, and 85.

A review of the City's *Transportation Demand Management (TDM)-Supportive Development Design and Infrastructure Checklist* has been conducted. All required TDM-supportive design and infrastructure measures in the TDM checklist are met. A copy of this checklist is included in **Appendix L**. In addition to the required measures, the proposed development also meets the following 'basic' or 'better' measures as defined in the *TDM-Supportive Development Design and Infrastructure Checklist*:

- Locate the building close to the street, and do not locate parking areas between the street and building entrances;
- Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations;
- Located building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort;
- Provide safe, direct, and attractive walking routes from building entrances to nearby transit stops;
- Provide lighting, landscaping, and benches along walking and cycling routes between building entrances and streets, sidewalks, and trails;
- Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer to encourage other cyclists and ensure adequate capacity in peak cycling season;
- Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met).

4.1.2 Circulation and Access

Pick-ups and drop-offs will occur curbside on Sidney Street. Garbage collection will also take place curbside on Sidney Street, as waste bins will be wheeled out to the curb. There is no on-site fire route proposed as part of this development. Therefore, the fire route for the development is considered to consist of Carling Avenue, Preston Street, and Sidney Street.

4.2 Parking

The subject site is located in Area B of Schedule 1 and Area Z of Schedule 1A of the City's ZBL. Vehicular and bicycle parking requirements for the proposed development are identified in Sections 101, 102, 103, and 111 of the ZBL, and are summarized in **Table 20**.

Table 20: Parking Requirements

Land Use	Rate	Units/GFA	Required
<i>Minimum Vehicle Parking Requirements</i>			
Dwelling, Mixed-Use Building	0.1 per dwelling unit after the first 12 units, up to a maximum of 30 spaces (visitors only)	459 units	30
Retail Store	No spaces required (Area Z)	286 m ²	0
Minimum Required			30
Total Parking Proposed			386
<i>Maximum Vehicle Parking Requirements</i>			
Dwelling, Mixed-Use Building	1.75 per dwelling unit (combined total of resident and visitor parking)	459 units	803
Retail Store	3.6 per 100 m ² GFA	286 m ²	10
Maximum Permitted			813
Total Parking Proposed			386
<i>Minimum Bicycle Parking Requirements</i>			
Dwelling, Mixed-Use Building	0.5 per dwelling unit	459 units	230
Retail Store	1.0 per 250 m ² GFA	286 m ²	1
Minimum Required			231
Total Bicycle Parking Proposed			241

Based on the previous table, the proposed number of vehicle and bicycle parking spaces meet all requirements of the ZBL.

Section 111(12) of the ZBL identifies that, where the number of bicycle parking spaces required for a single residential building exceeds 50 spaces, a minimum of 25% of the required total must be located within a building or structure, a secure area, or bicycle lockers. The proposed development will include 15 surface bicycle parking spaces, with the remaining 226 being located within various levels of the parking garage. Therefore, this requirement is met.

4.3 Boundary Streets

This section provides a review of the boundary streets Carling Avenue, Preston Street, and Sidney Street, using complete streets principles. The *Multi-Modal Level of Service (MMLOS) Guidelines*, produced by IBI Group in October 2015, were used to evaluate the levels of service for each alternative mode of transportation on Preston Street and Sidney Street. Per the *2017 TIA Guidelines*, an MMLOS review is not required for any streets where a complete streets concept has been

developed. This applies to Carling Avenue, based on the transit priority measures functional design shown in **Figure 4**. Given the width between the proposed building and the curblines of Carling Avenue, it is anticipated that the sidewalks and segregated cycling facilities shown in the functional design can be accommodated. The segregated cycling facilities will act as a boulevard for pedestrians.

Schedule B of the City's Official Plan identifies the boundary streets are located within the 'Mixed Use Centre' land use designation. However, due to the subject site's proximity to Carling O-Train Station, Preston Street and Sidney Street have been evaluated against the targets outlined for the 'Within 600m of a Rapid Transit Station' policy area, based on existing conditions.

A detailed MMLOS review of the boundary streets is included in **Appendix M**. A summary of the results of the segment MMLOS analysis for Preston Street and Sidney Street are provided in **Table 21**.

Table 21: Segment MMLOS Summary

Segment	PLOS		BLOS		TLOS		TkLOS	
	Actual	Target	Actual	Target	Actual	Target	Actual	Target
Preston Street	E	A	D	B	F	-	B	D
Sidney Street	A	A	A	D	-	-	-	-

The results of the segment MMLOS analysis can be summarized as follows:

- Sidney Street meets the target pedestrian level of service (PLOS), while Preston Street does not;
- Sidney Street meets the target bicycle level of service (BLOS), while Preston Street does not;
- Preston Street achieves a transit level of service (TLOS) F, but does not have a TLOS target;
- Preston Street meets the target truck level of service (TkLOS).

Preston Street currently achieves a PLOS E. The best possible score is a PLOS C without changing the operating speed of the roadway (i.e. roadways with a curb lane AADT greater than 3,000 vpd and no on-street parking can only achieve a PLOS A if the operating speed is 30 km/h). To achieve a PLOS C, sidewalks with a minimum width of 2.0m and minimum boulevard width of 2.0m are required. This is identified for the City's consideration. Along the site's frontage, a sidewalk width greater than 4m is proposed. Considering 2m of this width to be boulevard width, the best possible PLOS C is achieved.

Preston Street currently achieves a BLOS D. Based on Exhibit 11 of the MMLOS Guidelines, the target BLOS B can be achieved by reducing the speed limit to 40 km/h, or by implementing curbside bike lanes with a minimum width of 1.5m. In areas with on-street parking, a 4.25m-wide bike plus parking lane would also achieve the target BLOS B. This is identified for the City's consideration.

A sidewalk of approximately 2m width is proposed along the site's frontage to Sidney Street. This will maintain the PLOS of Sidney Street at the target PLOS A.

4.4 Access Design

The proposed development includes two adjacent one-way accesses to Sidney Street, at the northwest corner of the site. The existing depressed curbs to the subject site will be removed as part of the proposed development, and full-height curb and sidewalks will be reinstated per City standards. Curbs will be depressed and continuous across the proposed accesses to Sidney Street. The design of the proposed accesses have been evaluated using the provisions of the City's ZBL and *Private Approach By-Law* (PABL).

Section 25(a) of the PABL identifies that, for sites with 35m to 45m of frontage, a maximum of two private approaches (one-way or two-way) are permitted. The proposed pair of one-way private approaches meet this requirement.

Section 25(d) of the PABL identifies a maximum width requirement of 7.5m for any one-way private approach, as measured at the street line. Since the private approaches are approximately 4m in width, this requirement is met.

Section 25(h) of the PABL identifies that the nearest limits of any two one-way private approaches must be a minimum of 2m apart. Similarly, Section 25(i) of the PABL identifies that if a median is to be provided between two one-way private approaches, this median shall be a minimum of 2m in width. The median between the two one-way private approaches is approximately 2.3m in width, and therefore both of these requirements are met.

Section 25(m)(ii) of the PABL identifies that, for a property that abuts an arterial roadway or is within 46m of one, there is a minimum distance requirement between a private approach and the nearest intersecting street line, based on the land use and the number of parking spaces provided. For apartment buildings with 300 or more parking spaces, a minimum distance of 60m is required. The subject site does not have adequate frontage onto Carling Avenue, Preston Street, or Sidney Street to meet this requirement. Per Section 25(n) of the PABL, relief from the above requirement is permissible if the private approaches are constructed on the roadway carrying the least traffic and are located as far from the intersecting street as possible. Since the private approaches are proposed on Sidney Street, and are located as far from Preston Street as possible, it is therefore requested that the requirements of Section 25(m)(ii) be waived.

Section 25(p) of the PABL identifies a minimum separation requirement of 3m between a private approach and the nearest property line, as measured at the street line. As the proposed approach is approximately 1.4m from the western property line, this requirement is not met. Section 25(p) of the PABL also states that the 3m minimum can be reduced to as little as 0.3m, provided the proposed approach is located a safe distance from an access to the adjacent property, has adequate sight lines, and does not create a traffic hazard. As the proposed accesses are located at the terminus of Sidney Street, they are not anticipated to create a traffic hazard. Therefore, it is requested that the requirement of Section 25(p) be waived.

Section 25(u) of the PABL identifies a requirement that any private approach serving a parking area with more than 50 parking spaces shall not have a grade exceeding 2% for the first 9m inside the property line. Measuring from the property line, the grade of the proposed approaches is 4% in the direction of the roadway for approximately 11.8m before transitioning to the ramp grades. Although the grade exceeds 2% for a distance of 9m within the property, the 4% grade in the direction of the roadway will allow for driver sightlines to pedestrians walking along Sidney Street. A waiver of the PABL is requested.

Section 107(1)(a) of the ZBL identifies that for any driveway providing access to a parking garage, a minimum driveway width of 3m is required for a single traffic lane. As the width of both accesses are 4m in width before transitioning to the ramps, this requirement is met.

Section 107(1)(c) of the ZBL identifies that any drive aisles serving parking spaces within a parking garage must have a minimum width of 6.0m. As the width of all drive aisles within the parking garage have a width of 6.0m, this requirement is met.

The layout of the proposed accesses and parking garage ramps result in an on-site weave zone. Any vehicles entering the site and driving down to the underground parking garage levels and any vehicles driving from the above-ground parking levels to exit the site will need to weave through this zone. Passenger car turning movements have been conducted to confirm that vehicles can be accommodated within this weave zone, and are included in **Figure 16** and **Figure 17**. To ensure safe operations, convex mirrors are proposed on the wall west of the inbound access, the wall east of the outbound access, and on the median between the two accesses. Additionally, stop signs are recommended at the inbound access and at both ramps for outbound vehicles.

The proposed accesses will be stop-controlled, with free flow on Sidney Street. It is anticipated that the proposed accesses will operate at an Auto LOS A in both peak hours. Detailed Synchro analysis of total traffic conditions is included in Sections 4.8.2 and 4.8.3.

4.5 Transportation Demand Management

4.5.1 Context for TDM

A breakdown of the proposed development's 459 dwellings into type can be summarized as follows:

- 50 bachelor dwellings;
- 193 one-bedroom dwellings;
- 189 two-bedroom dwellings;
- 27 three-bedroom dwellings.

The proposed development also includes two opportunities for ground-floor retail, located at the southwest and northeast corners of the subject site. The tenants for these retail areas are not yet known, but are assumed to be small trip generators with few employees, given the size. Therefore, the TDM measures checklist has not been reviewed for the retail use, as it is not required.

4.5.2 Need and Opportunity

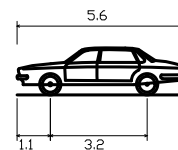
The subject site is located in the Preston-Carling District Secondary Plan, and is designated as 'Mixed Use Centre' in Schedule B of the City's Official Plan. As first shown in **Table 3**, the mode shares for the proposed uses are somewhat consistent with the mode share targets for a Transit-Oriented Development, with a lower transit share and higher non-auto share being applied. For both the proposed retail and residential uses, a 15% driver share was assumed during both the AM and PM peak hours.



NOVATECH

Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
Facsimile (613) 254-5867
Website www.novatech-eng.com



P - Passenger Car

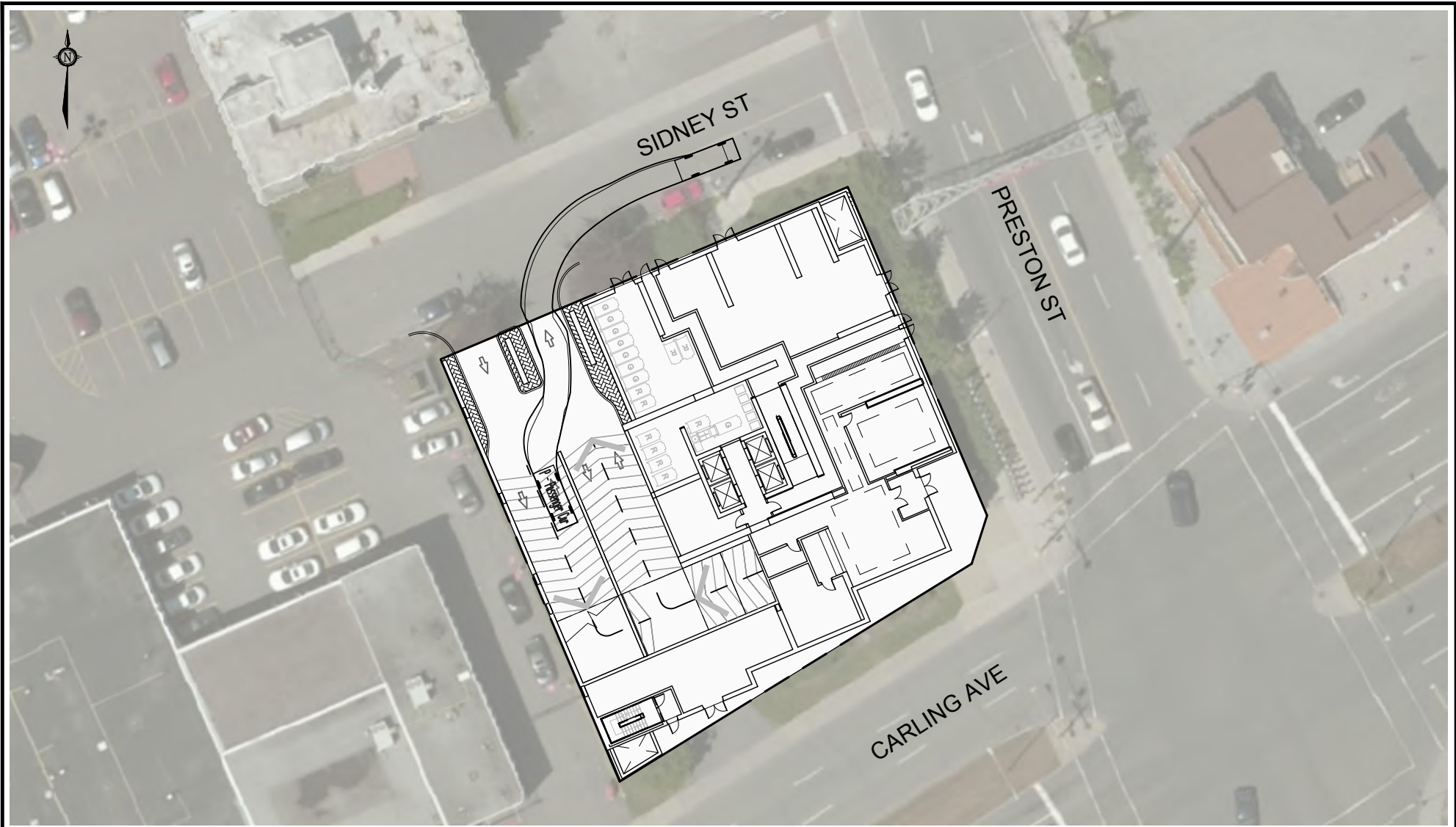
Overall Length	5.600m
Overall Width	2.000m
Overall Body Height	1.555m
Min Body Ground Clearance	0.340m
Track Width	2.000m
Lock-to-lock time	4.00s
Curb to Curb Turning Radius	6.300m

829 CARLING AVENUE

TURNING MOVEMENTS
(PARKING GARAGE)

SCALE 1 : 500

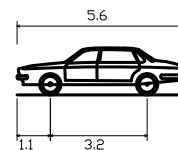
DATE FEB 2021 JOB 121008 FIGURE FIGURE-16



NOVATECH

Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
Facsimile (613) 254-5867
Website www.novatech-eng.com



P - Passenger Car

Overall Length	5.600m
Overall Width	2.000m
Overall Body Height	1.555m
Min Body Ground Clearance	0.340m
Track Width	2.000m
Lock-to-lock time	4.00s
Curb to Curb Turning Radius	6.300m

829 CARLING AVENUE

TURNING MOVEMENTS (PARKING GARAGE)

SCALE 1 : 500



DATE FEB 2021

JOB 121008

FIGURE FIGURE-17

The Ottawa Inner Area includes retail driver shares of 39% in the AM peak and 22% in the PM peak, and residential driver shares of 26% in the AM peak and 25% in the PM peak. Comparing to the mode share targets for a TOD site, failure to meet the 15% driver share target would equate an addition of approximately 20 vehicle trips during the AM peak hour and 25 vehicles during the PM peak hour.

As the planned transit lanes on Carling Avenue will often take the place of a general travel lane for all vehicles, it is anticipated that vehicular congestion will increase, and a failure to meet the proposed mode share targets may marginally increase congestion. However, the proposed development is located in close proximity to multiple bus stops as well as an LRT station, and connects to existing pedestrian and cycling networks. Further, the development is located in close proximity to many amenities, including commercial and retail areas, parks, recreation centres, and multi-use pathways.

4.5.3 TDM Program

A review of the City's *TDM Measures Checklist* has been conducted by the proponent, who has committed to providing the following TDM measures. A copy of the completed measures checklist is included in **Appendix L**.

- Display local area maps with walking/cycling access routes and key destinations at major entrances;
- Display relevant transit schedules and route maps at entrances;
- Unbundle parking cost from monthly rent;
- Provide a multimodal travel option information package to new residents.

4.6 Neighbourhood Traffic Management

The *2017 TIA Guidelines* identify two-way peak hour traffic volume thresholds for considering when a Neighbourhood Traffic Management (NTM) plan should be developed, when a site relies on local or collector roadways for access. For a local roadway, the NTM two-way volume threshold is 120 vph. This threshold is anticipated to be exceeded in all background and total traffic conditions (2028 and 2033).

The proposed development relies on Sidney Street, a local roadway, as the only road to provide direct vehicular access. No neighbourhood traffic management measures are required, as Sidney Street is a dead-end street that only provides access to four different lots (7 Sidney Street, 490 Preston Street, 829 Carling Avenue, and 845 Carling Avenue).

4.7 Transit

Based on the trip generation presented in Section 3.1.1.4, the proposed development is anticipated to generate an additional 63 transit trips during the AM peak hour (including 48 boarding and 15 alighting), and an additional 54 transit trips during the PM peak hour (including 22 boarding and 32 alighting). The area is well served with bus and light rail transit, and is served by OC Transpo Routes 2, 55, 56, and 85. It is anticipated that the proposed development will not have a significant impact on operations at the Carling O-Train Station and surrounding bus stops. Implementation of the Carling Avenue Transit Priority Measures and the extension of the Trillium Line are anticipated to be in place prior to buildout of the proposed development.

4.8 Intersection Design

4.8.1 Intersection MMLOS Review

This section provides an MMLOS review of the signalized study area intersections, using complete streets principles. All intersections have been evaluated using the MMLOS targets for intersections within 600m of a rapid transit station, and are based on existing conditions.

The full intersection MMLOS analysis is included in **Appendix M**. A summary of the results is shown in **Table 22**.

Table 22: Intersection MMLOS Summary

Intersection	PLOS		BLOS		TLOS		TkLOS		Auto LOS	
	Actual	Target	Actual	Target	Actual	Target	Actual	Target	Actual	Target
Carling Avenue/ Sherwood Drive	F	A	F	B	C	C	D	D	B	E
Carling Avenue/ Champagne Avenue	F	A	F	C	C	C	F	D	A	E
Carling Avenue/ Trillium Pathway ⁽¹⁾	F	A	A	B	B	C	-	-	A	E
Carling Avenue/ Preston Street	F	A	F	B	F	C	D	D	F	E
Carling Avenue/ Booth Street	F	A	F	C	F	C	F	D	A	E
Preston Street/ Beech Street	D	A	D	B	B	-	F	D	A	E
Preston Street/ Pamilla Street	D	A	B	B	B	-	F	D	A	E
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway	F	A	F	B	-	-	F	D	E	E
Preston Street/ Adeline Street ⁽²⁾	-	-	-	-	-	-	-	-	C	E
Preston Street/ Sidney Street ⁽²⁾	-	-	-	-	-	-	-	-	C	E

1. Signalized intersection with MUP, not evaluated for TkLOS

2. Unsignalized intersection, evaluated for Auto LOS only

The results of the intersection MMLOS analysis can be summarized as follows:

- All study area intersections do not meet the target PLOS;
- All study area intersections do not meet the target BLOS, except for Carling Avenue/Trillium Pathway and Preston Street/Pamilla Street;
- All study area intersections with targets meet the target TLOS, except for Carling Avenue/Preston Street and Carling Avenue/Booth Street;
- All study area intersections do not meet the target TkLOS, except for Carling Avenue/Sherwood Drive and Carling Avenue/Preston Street;
- All study area intersections meet the target Auto LOS, except for Carling Avenue/Preston Street.

The following includes further discussion for each intersection.

Carling Avenue/Sherwood Drive

The intersection does not meet the target PLOS A or BLOS B.

All approaches do not meet the target PLOS A, and have a cross-section equivalent to five lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes. Based on Exhibit 5 of the 2017 Addendum to the MMLOS Guidelines, any approach with a cross-section equivalent to four or more lanes crossed cannot achieve a PLOS A. Based on the functional design for the Carling Avenue Transit Priority Measures (shown in **Figure 4**), the future intersection will include shortened crossing distances at all approaches and a median refuge at the east approach. Further modifications may be possible as part of the Ottawa Civic Hospital Expansion development.

The west approach does not meet the target BLOS B, based on left turn characteristics. Based on the functional design for the Carling Avenue Transit Priority Measures, segregated cycling facilities and a protected intersection design are planned, which will allow all left turns for cyclists to take place off-road. This would improve the intersection to a BLOS A.

Carling Avenue/Champagne Avenue

The intersection does not meet the target PLOS A, BLOS C, or TkLOS D.

All approaches do not meet the target PLOS A, and have a cross-section equivalent to four lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes. Based on the functional design for the Carling Avenue Transit Priority Measures, the future intersection will include shortened crossing distances at all approaches, a median refuge at the east approach, and zebra-striped crosswalks at the east and west approaches. No other modifications are identified.

The north and west approaches do not meet the target BLOS B, based on left turn characteristics, and the east approaches does not meet the target based on right turn characteristics. Based on the functional design for the Carling Avenue Transit Priority Measures, segregated cycling facilities and a protected intersection design are planned, which will allow all left turns for cyclists to take place off-road. This would improve the intersection to a BLOS A.

The east approach does not meet the target TkLOS D. Increasing the curb radius for westbound right turns is required to meet the target. As Champagne Avenue is a local roadway and not a truck route, no modifications are recommended.

Carling Avenue/Trillium Pathway

The intersection does not meet the target PLOS A.

The east and west approaches do not meet the target PLOS A, as these approaches have a cross-section equivalent to nine lanes. There is limited opportunity in improving the PLOS without reducing the number of travel lanes on Carling Avenue. Based on the functional design for the Carling Avenue Transit Priority Measures, the future crossing will include a median refuge at the east approach. No other modifications are recommended.

Carling Avenue/Preston Street

The intersection does not meet the target PLOS A, BLOS B, TLOS C, or Auto LOS E.

All approaches do not meet the target PLOS A, and have a cross-section equivalent to four lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes. Based on the functional design for the Carling Avenue Transit Priority Measures, the future intersection will include zebra-striped crosswalks at all approaches and reduced crossing distances at the north and south approaches. No other modifications are recommended.

All approaches do not meet the target BLOS B, based on left turn characteristics. Based on the functional design for the Carling Avenue Transit Priority Measures, segregated cycling facilities are planned at the east and west approaches, and two-stage left-turn bike boxes are planned for eastbound and westbound cyclists. This will improve these approaches to a BLOS A. Two-stage left-turn bike boxes could be considered for northbound and southbound cyclists as well, and is identified for the City's consideration.

The north, east, and west approaches do not meet the target TLOS C during the AM and PM peak hours. As Preston Street is not designated within the City's RTTP networks, no recommendations are identified for the north approach. Comparing the delays of the north approach in the existing, 2033 background, and 2033 total conditions, the TLOS of the north approach is anticipated to remain at a TLOS F. It is anticipated that the transit priority measures proposed on Carling Avenue will allow the east and west approaches to operate at the target TLOS C or better (i.e. delays for buses will be 20 seconds or less for buses at these approaches).

The Auto LOS of the overall intersection does not meet the target Auto LOS E during the PM peak hour. As discussed, the northbound left turn, southbound through/right turn, and westbound left turn movements individually do not meet the target Auto LOS E. Further discussion of the existing operations at this intersection are included in Section 3.4.1.

Carling Avenue/Booth Street

The intersection does not meet the target PLOS A, BLOS C, TLOS C, or TkLOS D.

All approaches do not meet the target PLOS A, and have a cross-section equivalent to four lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes. Based on the functional design for the Carling Avenue Transit Priority Measures, the future intersection will include shortened crossing distances at all approaches, and zebra-striped crosswalks at the east and west approaches. No other modifications are recommended.

All approaches do not meet the target BLOS C, based on both left and right turn characteristics. Based on the functional design for the Carling Avenue Transit Priority Measures, segregated cycling facilities and a protected intersection design are planned, which will allow all left turns for cyclists to take place off-road. This would improve the intersection to a BLOS A.

The north and east approaches do not meet the target TLOS C during the AM and PM peak hours, and the west approach does not meet the target TLOS C during the AM peak hour only. As Booth Street is not designated within the City's RTTP networks, no recommendations are identified for the north approach. The TLOS of the north approach is anticipated to remain at a TLOS F in future conditions. It is anticipated that the transit priority measures proposed on Carling Avenue will allow

the east and west approaches to operate at the target TLOS C or better (i.e. delays for buses will be 20 seconds or less for buses at these approaches).

The east approach does not meet the target TkLOS D. Based on the functional design for the Carling Avenue Transit Priority Measures, the receiving lane for this movement will be a wider lane, and may accommodate trucks turning right from the east approach. Therefore, no further modifications are recommended.

Preston Street/Beech Street

The intersection does not meet the target PLOS A, BLOS B, or TkLOS D.

All approaches do not meet the target PLOS A, and have a cross-section equivalent to three or four lanes crossed. There is limited opportunity in improving the PLOS at each approach to the target, without the removal of auxiliary turn lanes.

The north, south, and east approaches do not meet the target BLOS B based on left turn characteristics, and the east approach does not meet the target BLOS B based on right turn characteristics. A review of the Desirable Cycling Facility Pre-Selection Nomograph included in *Ontario Traffic Manual (OTM) – Book 18* identifies that mixed-use travel lanes are appropriate on Beech Street, and designated bike lanes are appropriate on Preston Street. This is identified for the City's consideration. The pre-selection tool is included in **Figure 18**. Alternatively, a reduction of the speed limit from 50 km/h to 40 km/h on Preston Street and Beech Street would improve the BLOS to the target BLOS B based on left turn characteristics.

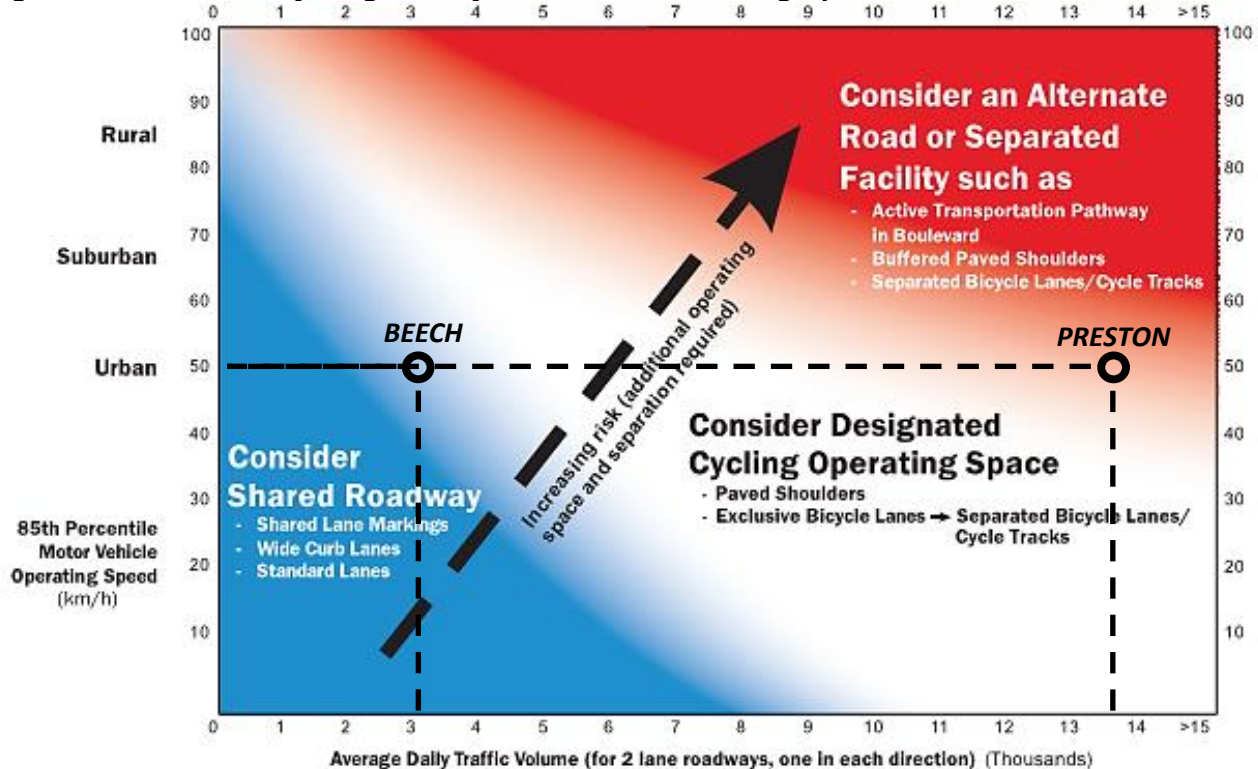
All approaches do not meet the target TkLOS D. Increasing the curb radii are required to meet the target. As Beech Street is a local roadway and not a truck route, no modifications are recommended.

Preston Street/Pamilla Street

The intersection does not meet the target PLOS A or TkLOS D.

The north, south, and west approaches do not meet the target PLOS A. There is limited opportunity in improving the PLOS at these approaches as the number of travel lanes cannot be reduced. The north and south approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks (greater than 400,000 vehicle/pedestrian conflicts over an eight-hour period). Therefore, these crosswalks could be textured similar to the crosswalks at the east and west approaches, or similar to the crosswalks at Preston Street/Beech Street. Curb bulbouts could be considered to reduce crossing distance.

All approaches do not meet the target TkLOS D. Increasing the curb radii are required to meet the target TkLOS D. As Pamilla Street is a local roadway and not a truck route, no modifications are recommended.

Figure 18: Desirable Cycling Facility Pre-Selection Nomograph**Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway**

The intersection does not meet the target PLOS A, BLOS B, or TkLOS D.

All approaches do not meet the target PLOS A, and have a cross-section equivalent to four lanes crossed or more. There is limited opportunity in improving the PLOS at each approach to the target, without the removal of auxiliary turn lanes. Based on Exhibit 5 of the 2017 Addendum to the MMLOS Guidelines, any approach with a cross-section of four or more lanes crossed cannot achieve a PLOS A. The north, east, and west approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks, and could increase the level of comfort for pedestrians. This is identified for the City's consideration.

The north and west approaches do not meet the target BLOS B based on left turn characteristics, and the north approach does not meet the target BLOS B based on right turn characteristics. Based on Exhibit 12 of the MMLOS Guidelines, the target BLOS can be achieved by implementing two-stage left-turn bike boxes for southbound and westbound cyclists. As discussed previously, curbside bike lanes on Preston Street are also identified as appropriate per the OTM Pre-Selection Tool. These improvements are identified for the City's consideration.

The south and west approaches do not meet the target TkLOS D. As these approaches involve heavy vehicles turning right into or out of the Dow's Lake Pavilion, no modifications are recommended.

4.8.2 2028 Total Intersection Operations

Intersection capacity analysis has been conducted for the 2028 total traffic conditions. Signal timings within the study area reflect the optimized conditions first described in Section 3.4.2. The results of the analysis are summarized in **Table 23** and **Table 24** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix N**.

Table 23: 2028 Total Traffic Operations

Intersection	Period	Critical Movements			Intersection		
		Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS
Carling Avenue/ Sherwood Drive ⁽¹⁾	AM	0.58	A	SBL	0.35	11 sec	A
	PM	0.74	C	SBL	0.66	16 sec	A
Carling Avenue/ Champagne Avenue ⁽¹⁾	AM	0.36	A	EBT	0.33	6 sec	A
	PM	0.69	B	SBR	0.55	11 sec	A
Carling Avenue/ Trillium Pathway ⁽²⁾	AM	0.39	A	WBT	0.38	4 sec	A
	PM	0.53	A	WBT	0.47	3 sec	A
Carling Avenue/ Preston Street ⁽¹⁾	AM	1.01	F	SBT/R	0.98	48 sec	E
	PM	1.36	F	NBL	1.20	96 sec	F
		1.11	F	SBT/R			
		1.14	F	EBT/R			
		1.34	F	WBL			
Carling Avenue/ Booth Street ⁽¹⁾	AM	1.01	F	EBL	0.94	42 sec	E
	PM	1.14	F	WBT	1.07	63 sec	F
Preston Street/ Beech Street ⁽¹⁾	AM	0.64	B	NBT	0.57	12 sec	A
	PM	0.56	A	WBL/T	0.47	12 sec	A
Preston Street/ Pamilla Street ⁽¹⁾	AM	0.50	A	NBT	0.46	4 sec	A
	PM	0.44	A	NBT	0.43	5 sec	A
Preston Street/ Adeline Street ⁽³⁾	AM	30 sec	E	WBL/T/R	-		
	PM	25 sec	D	WBL/T/R			
Preston Street/ Sidney Street ⁽³⁾	AM	23 sec	C	EBL/R	-		
	PM	16 sec	C	EBL/R			
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway ⁽¹⁾	AM	1.16	F	SBL/T	0.82	27 sec	D
	PM	1.13	F	SBL/T	0.98	56 sec	E
Sidney Street/ Site Access ⁽³⁾	AM	9 sec	A	NBR	-		
	PM	9 sec	A	NBR			

1. Signalized intersection

2. Signalized pathway crossing

3. Unsignalized intersection

Table 24: 2028 Total Queues

Intersection	Mvmt	Storage/ Spacing ⁽¹⁾	AM Peak			PM Peak		
			v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Carling Avenue/ Preston Street	NBL	75m	0.97 [E]	38	m#73	1.36 [F]	~103	#159
	SBL	35m	0.71 [C]	24	#52	0.59 [A]	23	43
	SBT/R	35m	1.01 [F]	~104	#167	1.11 [F]	~140	#202
	EBL	65m	0.86 [D]	27	#63	0.94 [E]	35	#79
	EBT/R	95m	0.99 [E]	45	#123	1.14 [F]	~148	#180
	WBL	110m	0.88 [D]	28	m33	1.34 [F]	~114	m#114
	WBT	50m	0.80 [C]	76	m77	0.92 [E]	138	m125
Carling Avenue/ Booth Street	SBL	--	0.70 [B]	50	68	0.88 [D]	74	#114
	EBL	75m	1.01 [F]	~86	m#146	1.00 [E]	~78	m#84
	WBT	85m	0.99 [E]	149	#222	1.14 [F]	~298	#371
Preston Street/ Prince of Wales Drive/Queen Elizabeth Driveway	SBL/T	135m	1.16 [F]	~70	m#74	1.13 [F]	~114	#171
	EBL	55m	0.85 [D]	68	#100	0.96 [E]	89	#151

1. Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

m: volume for the 95th percentile queue is metered by an upstream signal

#: volume for the 95th percentile cycle exceeds capacity

~: approach is above capacity

Comparing the previous tables and the 2028 background conditions, traffic generated by the proposed development is anticipated to have marginal operational effects within the study area. The discussion of over-capacity movements and queue lengths are generally consistent with those described in Section 3.4.2.

4.8.3 2033 Total Intersection Operations

Intersection capacity analysis has been conducted for the 2033 total traffic conditions. Signal timings within the study area reflect the optimized conditions first described in Section 3.4.2. The results of the analysis are summarized in **Table 25** and **Table 26** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix N**.

Table 25: 2033 Total Traffic Operations

Intersection	Period	Critical Movements			Intersection		
		Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS
Carling Avenue/ Sherwood Drive ⁽¹⁾	AM	0.58	A	SBL	0.32	11 sec	A
	PM	0.74	C	SBL	0.61	16 sec	B
Carling Avenue/ Champagne Avenue ⁽¹⁾	AM	0.33	A	WBT	0.32	6 sec	A
	PM	0.66	B	SBR	0.51	11 sec	A
Carling Avenue/ Trillium Pathway ⁽²⁾	AM	0.37	A	WBT	0.35	4 sec	A
	PM	0.48	A	WBT	0.44	3 sec	A
Carling Avenue/ Preston Street ⁽¹⁾	AM	1.02	F	SBT/R	0.96	46 sec	E
	PM	1.36	F	NBL	1.18	94 sec	F
		1.12	F	SBT/R			
		1.10	F	EBT/R			
		1.34	F	WBL			
Carling Avenue/ Booth Street ⁽¹⁾	AM	1.01	F	EBL	0.92	41 sec	E
	PM	1.04	F	WBT	1.00	49 sec	E

Intersection	Period	Critical Movements			Intersection		
		Max v/c or Delay	LOS	Mvmt	v/c	Delay	LOS
Preston Street/ Beech Street ⁽¹⁾	AM	0.67	B	NBT	0.59	12 sec	A
	PM	0.58	A	WBL/T	0.49	12 sec	A
Preston Street/ Pamilla Street ⁽¹⁾	AM	0.52	A	NBT	0.47	4 sec	A
	PM	0.45	A	NBT	0.44	5 sec	A
Preston Street/ Adeline Street ⁽³⁾	AM	40 sec	E	EBL/T/R	-		
	PM	31 sec	D	EBL/T/R			
Preston Street/ Sidney Street ⁽³⁾	AM	26 sec	C	EBL/R	-		
	PM	16 sec	C	EBL/R			
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway ⁽¹⁾	AM	1.17	F	SBL/T	0.82	28 sec	D
	PM	1.14	F	SBL/T	0.99	57 sec	E
Sidney Street/ Site Access ⁽³⁾	AM	9 sec	A	NBR	-		
	PM	9 sec	A	NBR			

1. Signalized intersection

2. Signalized pathway crossing

3. Unsignalized intersection

Table 26: 2033 Total Queues

Intersection	Mvmt	Storage/ Spacing ⁽¹⁾	AM Peak			PM Peak		
			v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)	v/c [LOS]	50 th % Queue (m)	95 th % Queue (m)
Carling Avenue/ Preston Street	NBL	75m	0.99 [E]	40	m#75	1.36 [F]	~103	#159
	SBL	35m	0.78 [C]	27	#58	0.63 [B]	25	#47
	SBT/R	35m	1.02 [F]	~111	#172	1.12 [F]	~143	#206
	EBL	65m	0.87 [D]	28	#64	0.98 [E]	37	#83
	EBT/R	95m	0.92 [E]	42	#107	1.10 [F]	~139	#173
	WBL	110m	0.88 [D]	28	m#36	1.34 [F]	~114	m#127
	WBT	50m	0.76 [C]	73	m77	0.83 [D]	122	m120
Carling Avenue/ Booth Street	SBL	--	0.71 [B]	50	68	0.88 [D]	74	#114
	EBL	75m	1.01 [F]	86	m#154	1.00 [E]	~78	m#87
	WBT	85m	0.95 [E]	139	#208	1.04 [F]	~250	#322
Preston Street/ Prince of Wales Drive/Queen Elizabeth Driveway	SBL/T	135m	1.17 [F]	~71	m#77	1.14 [F]	~114	#172
	EBL	55m	0.85 [D]	68	#101	0.97 [E]	91	#154

1. Indicates the storage length for auxiliary lanes or the spacing to the nearest upstream intersection for through lanes

m: volume for the 95th percentile queue is metered by an upstream signal#: volume for the 95th percentile cycle exceeds capacity

~: approach is above capacity

Comparing the previous tables and the 2033 background conditions, traffic generated by the proposed development is anticipated to have marginal operational effects within the study area. The discussion of over-capacity movements and queue lengths are consistent with those described in Section 3.4.3.

Based on the southbound queues at Carling Avenue/Preston Street, it is anticipated that northbound left turns and eastbound left/right turns at Preston Street/Sidney Street will rely on courtesy from queued drivers on Preston Street to complete their turns during the peak hours. As there are two northbound lanes approaching Sidney Street, northbound through vehicles can use the curbside lane to bypass a northbound left turning vehicle. This is consistent with the existing intersection operations.

Based on the site-generated traffic volumes shown in **Figure 9**, the proposed development will add:

- Six to twelve northbound left turning vehicles (equivalent to one vehicle every five to ten minutes during the peak hours);
- One to four eastbound left turning vehicles (equivalent to one vehicle every 15 minutes during the AM peak hour), and;
- Eight to thirteen eastbound right turning vehicles (equivalent to one vehicle every five to eight minutes during the peak hours).

5.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the foregoing, the conclusions and recommendations of this TIA can be summarized as follows:

Forecasting

- The proposed development is anticipated to generate an additional 152 person trips during the AM peak hour (including 19 vehicle trips), and an additional 101 person trips during the PM peak hour (including 13 vehicle trips).

Development Design

- Concrete sidewalks will be provided around the north, south, and east sides of the proposed building, and will connect to existing sidewalks on Sidney Street, Preston Street, and Carling Avenue. Fifteen bicycle parking spaces will be provided in three outside parking areas, as well as 226 bicycle parking spaces within different levels of the parking garage.
- OC Transpo stops #2397, #6655, #6657, #7369, #8013, #8014, #8023, and the Carling O-Train Station are within 400m walking distance of all entrances to the proposed development.
- All required TDM-supportive design and infrastructure measures in the TDM checklist are met.

Parking

- The proposed development includes 386 vehicle parking spaces, meeting the minimum number of required parking spaces and maximum number of permitted parking spaces, as outlined in the City's *Zoning By-Law* (ZBL).
- The proposed development includes 241 bicycle parking spaces, meeting the minimum number of required spaces as outlined in the City's ZBL. Section 111 outlines a requirement to provide at least 25% of bicycle spaces within a secure area or structure, which is also met by the proposed development.

Boundary Streets

- Sidney Street meets the target pedestrian level of service (PLOS) A and the target bicycle level of service (BLOS) D.
- Preston Street does not meet the target PLOS A or BLOS B, meets the target truck level of service (TkLOS) D, and achieves a transit level of service (TLOS) F, but has no target.
- The best possible PLOS for Preston Street is a PLOS C, which would require sidewalks with a minimum width of 2.0m and a minimum boulevard width of 2.0m. This is identified for the City's consideration. Along the site's frontage, a sidewalk width greater than 4m is proposed. Considering 2m of this width to be boulevard width, the best possible PLOS C will be achieved.
- The target BLOS B for Preston Street can be achieved by reducing the speed limit to 40 km/h, or by implementing curbside bike lanes with a minimum width of 1.5m. In areas with on-street parking, a 4.25m-wide bike plus parking lane would also achieve the target BLOS B. This is identified for the City's consideration.
- A sidewalk of approximately 2m width is proposed along the site's frontage to Sidney Street. This will maintain the PLOS of Sidney Street at the target PLOS A.

Access Design

- The existing depressed curbs to the subject site will be removed as part of the proposed development, and full-height curb and sidewalks will be reinstated per City standards. Curbs will be depressed and continuous across the proposed accesses to Sidney Street.
- The proposed accesses meet the requirements of Sections 25(a), 25(d), 25(h), 25(i), and 25(u) of the *Private Approach By-Law*, and Section 107(1) of the ZBL.
- It is requested that the requirements of Sections 25(m)(ii), 25(p), and 25(u) of the *Private Approach By-Law* be waived, as the accesses are proposed as far from Preston Street as possible, will maintain proper sightlines, and will not create a traffic hazard.
- The layout of the proposed accesses and parking garage ramps result in an on-site weave zone. To ensure safe operations, convex mirrors are proposed on the wall west of the inbound access, the wall east of the outbound access, and on the median between the two accesses. Additionally, stop signs are recommended at the inbound access and at both ramps for outbound vehicles.
- The proposed accesses will be stop-controlled, with free flow on Sidney Street. It is anticipated that the proposed accesses will operate acceptably during both peak hours.

Transportation Demand Management

- The proponent has committed to providing the following TDM measures:
 - Display local area maps with walking/cycling access routes and key destinations at major entrances;
 - Display relevant transit schedules and route maps at entrances;
 - Unbundle parking cost from monthly rent;
 - Provide a multimodal travel option information package to new residents.

Neighbourhood Traffic Management

- The proposed development relies on the local roadway Sidney Street for direct vehicular access. No neighbourhood traffic management measures are required, as Sidney Street is a short, dead-end roadway that only provides access to four different lots (7 Sidney Street, 490 Preston Street, 829 Carling Avenue, and 845 Carling Avenue).

Transit

- The proposed development is anticipated to generate an additional 63 transit trips during the AM peak hour and an additional 54 transit trips during the PM peak hour. It is anticipated that the proposed development will not have a significant impact on operations at the Carling O-Train Station and surrounding bus stops.

Intersection MMLOS

- The results of the intersection MMLOS analysis can be summarized as follows:
 - All study area intersections do not meet the target PLOS;
 - All study area intersections do not meet the target BLOS, except for Carling Avenue/Trillium Pathway and Preston Street/Pamilla Street;
 - All study area intersections with targets meet the target TLOS, except for Carling Avenue/Preston Street and Carling Avenue/Booth Street;
 - All study area intersections do not meet the target TkLOS, except for Carling Avenue/Sherwood Drive and Carling Avenue/Preston Street;
 - All study area intersections meet the target vehicular level of service (Auto LOS), except for Carling Avenue/Preston Street.
- Pedestrian Level of Service
 - All approaches at Carling Avenue/Sherwood Drive, Carling Avenue/Champagne Avenue, Carling Avenue/Preston Street, and Carling Avenue/Booth Street, and the east and west approach at Carling Avenue/Trillium Pathway, do not meet the target PLOS A. The functional design for the Carling Avenue Transit Priority Measures outlines various measures to improve the level of comfort for pedestrians, but the target PLOS A will not be achieved at any approach.
 - All approaches of Preston Street/Beech Street do not meet the target PLOS A. There is limited opportunity in improving the PLOS without the removal of auxiliary turn lanes.
 - The north, south, and west approaches of Preston Street/Pamilla Street do not meet the target PLOS A. The north and south approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. Therefore, textured crosswalks similar to the east and west approaches at this intersection could be considered. Curb bulbouts could be considered to reduce crossing distance.
 - All approaches of Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target PLOS A. The north, east, and west approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. This is identified for the City's consideration.

- Bicycle Level of Service
 - The west approach of Carling Avenue/Sherwood Drive, the north and west approaches of Carling Avenue/Champagne Avenue, and all approaches of Carling Avenue/Booth Street do not meet the target BLOS. The functional design for the Carling Avenue Transit Priority Measures identify segregated cycling facilities and protected intersections at these locations, which will allow all left turns for cyclists to take place off-road, and improve these approaches to a BLOS A.
 - All approaches of Carling Avenue/Preston Street does not meet the target BLOS B. The functional design for the Carling Avenue Transit Priority Measures identify segregated cycling facilities and two-stage left-turn bike boxes for eastbound and westbound cyclists, which would improve these approaches to a BLOS A. Two-stage bike boxes could be considered for northbound/southbound cyclists as well, and is identified for the City's consideration.
 - The north, south, and east approaches of Preston Street/Beech Street do not meet the target BLOS B. The *Ontario Traffic Manual – Book 18* identifies that mixed traffic lanes are appropriate for Beech Street, and designated bike lanes are appropriate on Preston Street. This is identified for the City's consideration. Alternatively, a reduction of the speed limit from 50 km/h to 40 km/h on both roadways would improve the BLOS to the target.
 - The north and west approaches of Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target BLOS B. The target BLOS can be achieved for these approaches with the implementation of two-stage left-turn bike boxes. This is identified for the City's consideration.
- Transit Level of Service
 - The north, east, and west approaches at Carling Avenue/Preston Street, and the north and east approaches at Carling Avenue/Booth Street, do not meet the target TLOS C during the AM and PM peak hours. The transit priority measures on Carling Avenue are anticipated to allow the east and west approaches to operate at a TLOS C or better. The north approaches at both intersections are anticipated to continue operating below the target TLOS in future conditions.
- Truck Level of Service
 - The east approach of Carling Avenue/Champagne Avenue does not meet the target TkLOS D. As Champagne Avenue is a local roadway and not a truck route, no modifications are recommended.
 - The east approach of Carling Avenue/Booth Street does not meet the target TkLOS D. Based on the functional design for the Carling Avenue Transit Priority Measures, the receiving lane for this movement will be a wider lane, and may accommodate trucks turning right from the east approach. Therefore, no further modifications are recommended.

- All approaches of Preston Street/Beech Street and Preston Street/Pamilla Street do not meet the target TkLOS D. While the target TkLOS could be met by increase the curb radii, Beech Street and Pamilla Street are local roadways and not truck routes. Therefore, no modifications are recommended.
- The south and west approaches of Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway do not meet the target TkLOS D. As these approaches involve heavy vehicles turning right into or out of the Dow's Lake Pavilion, no modifications are recommended.

Existing Intersection Operations

- At Carling Avenue/Preston Street, the northbound left turn, southbound through/right turn, and westbound left turn movements do not meet the target Auto LOS E during the PM peak hour.
- At Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway, the southbound left turn/through movement does not meet the target Auto LOS E during the AM and PM peak hours.
- During the AM and PM peak hours, southbound queueing at Carling Avenue/Preston Street extends through the upstream intersection at Preston Street/Sidney Street.

Background Intersection Operations

- Traffic throughout the study area could be displaced or alleviated through a combination of increase use of non-auto modes of transportation, alternate times of travel for drivers, and alternate routes of travel. It is assumed that the Carling Avenue Transit Priority Measures project will increase the transit modal share and decrease the auto modal share by the buildout year 2028.
- As congestion increases within the study area, some motorists may alter their travel times to occur outside of the peak hours and alter their routes to other roadways within proximity of the study area.
- At Carling Avenue/Preston Street, a reduction of 10 southbound through/right turning vehicles during the AM peak hour, and 80 northbound left turning vehicles, 50 southbound through/right turning vehicles, 70 eastbound through/right turning vehicles, and 90 westbound left turning vehicles during the PM peak hour would be required to meet the target Auto LOS E in the 2033 background conditions.
- At Carling Avenue/Booth Street, a reduction of 10 eastbound left turning vehicles during the AM peak hour and 30 westbound through vehicles during the PM peak hour would be required to meet the target Auto LOS E in the 2033 background conditions.
- At Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway, a reduction of 40 southbound left turning vehicles during the AM peak hour and 50 southbound left turning vehicles during the PM peak hour would be required to meet the target Auto LOS E in the 2033 background conditions.

Total Intersection Operations

- Traffic generated by the proposed development is anticipated to have marginal operational effects for most movements at the study area intersections.
- During the AM peak hour, the southbound through/right turn movement at Carling Avenue/Preston Street and the eastbound left turn movement at Carling Avenue/Booth Street marginally downgrade to an Auto LOS F as a result of site-generated traffic (i.e. eight additional southbound through/right turn vehicles at Carling Avenue/Preston Street and two additional eastbound left turning vehicles at Carling Avenue/Booth Street).
- It is anticipated that northbound left turns and eastbound left/right turns at Preston Street/Sidney Street will rely on courtesy from queued drivers on Preston Street to complete their turns during the peak hours. As there are two northbound lanes approaching Sidney Street, northbound through vehicles can use the curbside lane to bypass a northbound left turning vehicle. This is consistent with the existing intersection operations.
- The proposed development will add:
 - Six to twelve northbound left turning vehicles (equivalent to one vehicle every five to ten minutes during the peak hours);
 - One to four eastbound left turning vehicles (equivalent to one vehicle every 15 minutes during the AM peak hour), and;
 - Eight to thirteen eastbound right turning vehicles (equivalent to one vehicle every five to eight minutes during the peak hours).

Based on the foregoing, the proposed development can be recommended from a transportation perspective.

NOVATECH

Prepared by:



Joshua Audia, B.Sc.
E.I.T.,
Transportation/Traffic

Reviewed by:

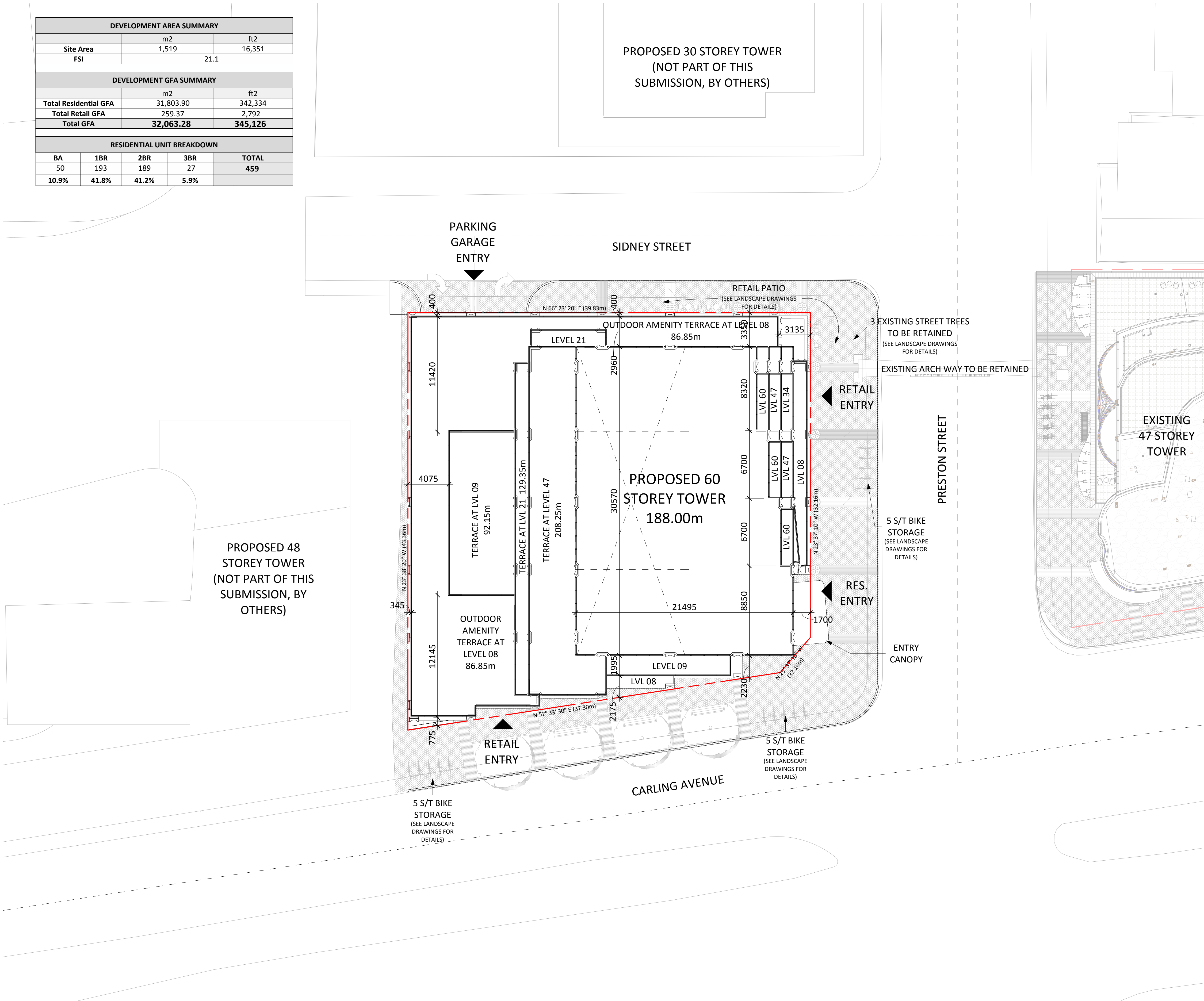


Brad Byvelds, P.Eng.
Project Coordinator,
Transportation/Traffic

APPENDIX A

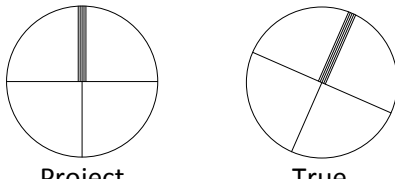
Proposed Site Plan

DEVELOPMENT AREA SUMMARY				
		m2		ft2
Site Area		1,519		16,351
FSI		21.1		
DEVELOPMENT GFA SUMMARY				
		m2		ft2
Total Residential GFA		31,803.90		342,334
Total Retail GFA		259.37		2,792
Total GFA		32,063.28		345,126
RESIDENTIAL UNIT BREAKDOWN				
BA	1BR	2BR	3BR	TOTAL
50	193	189	27	459
10.9%	41.8%	41.2%	5.9%	



General Notes

- These Contract Documents are the property of the Architect. The Architect bears no responsibility for the interpretations of these documents by the contractor. Upon written application the Architect will provide written/graphic clarification or supplementary information regarding the intent of the Contract Documents. The Architect will review Shop Drawings submitted by the Contractor for design conformance only.
- Drawings are not to be scaled for construction. Contractor to verify all existing conditions and dimensions required to perform the Work and report any discrepancies with the Contract Documents to the Architect before commencing work.
- Positions of exposed or finished mechanical or electrical devices, fittings, and fixtures are indicated on the Architectural drawings. The locations shown on the Architectural drawings govern over the Mechanical and Electrical drawings. Those items not clearly located will be located as directed by the Architect.



KEYPLAN

PROJECT TEAM

CLIENT	CLARIDGE HOMES
ARCHITECT	HARIRI PONTARINI ARCHITECTS
LANDSCAPE	JAMES B. LENNOX + ASSOCIATES
PLANNING	FOTENN PLANNING + DESIGN
STRUCTURAL	GOODEVE STRUCTURAL INC.
CIVIL/TRAFFIC	NOVATECH GROUP
GEOTECH	PATERSON GROUP INC.
WIND	GRADIENT WIND ENGINEERING
SURVEYOR	ANNIS, O'SULLIVAN, VOLLEBEKK LTD.

DRAWING STATUS

NOT FOR CONSTRUCTION

1	2021-04-15	ISSUED FOR OPA/ZBA/SPA
NO.	YYYY-MM-DD	DESCRIPTION

HARIRI PONTARINI ARCHITECTS

235 Carlaw Avenue
Suite 301
Toronto, Canada M4M 2S1
tel 416 929 4901
fax 416 929 8924
info@hp-arch.com
hariripontarini.com

Project Title:

829 Carling Ave Mixed-Use Development

829 CARLING AVE. OTTAWA, ON

SITE PLAN

Project number:	2030
Scale:	1 : 200
Sheet Start Date:	11/23/20
Drawn / Checked by:	HPA HPA
Drawing No.:	Revision:

A102



DEVELOPMENT AREA SUMMARY				
	m2		ft2	
Site Area	1,519		16,351	
FSI	21.1			
DEVELOPMENT GFA SUMMARY				
	m2		ft2	
Total Residential GFA	31,803.90		342,334	
Total Retail GFA	259.37		2,792	
Total GFA	32,063.28		345,126	
RESIDENTIAL UNIT BREAKDOWN				
BA	1BR	2BR	3BR	TOTAL
50	193	189	27	459
10.9%	41.8%	41.2%	5.9%	



General Notes

- These Contract Documents are the property of the Architect. The Architect bears no responsibility for the interpretations of these documents by the contractor. Upon written application the Architect will provide written/graphic clarification or supplementary information regarding the intent of the Contract Documents. The Architect will review Shop Drawings submitted by the Contractor for design conformance only.
- Drawings are not to be scaled for construction. Contractor to verify all existing conditions and dimensions required to perform the Work and report any discrepancies with the Contract Documents to the Architect before commencing work.
- Positions of exposed or finished mechanical or electrical devices, fittings, and fixtures are indicated on the Architectural drawings. The locations shown on the Architectural drawings govern over the Mechanical and Electrical drawings. Those items not clearly located will be located as directed by the Architect.

NORTH Project True

KEYPLAN

PROJECT TEAM	
CLIENT	CLARIDGE HOMES
ARCHITECT	HARIRI PONTARINI ARCHITECTS
LANDSCAPE	JAMES B. LENNOX + ASSOCIATES
PLANNING	FOTENN PLANNING + DESIGN
STRUCTURAL	GOODEVE STRUCTURAL INC.
CIVIL/TRAFFIC	NOVATECH GROUP
GEOTECH	PATERSON GROUP INC.
WIND	GRADIENT WIND ENGINEERING
SURVEYOR	ANNIS, O'SULLIVAN, VOLLEBEKK LTD.

DRAWING STATUS

NOT FOR CONSTRUCTION

1	2021-04-15	ISSUED FOR OPA/ZBA/SPA
NO.	YYYY-MM-DD	DESCRIPTION

HARIRI PONTARINI ARCHITECTS
235 Carlaw Avenue
Suite 301
Toronto, Canada M4M 2S1
tel 416 929 4901
fax 416 929 8924
info@hp-arch.com
hariripontarini.com

Project Title:
829 Carling Ave Mixed-Use Development

829 CARLING AVE. OTTAWA, ON

LEVEL 01	
Project number:	2030
Scale:	1 : 100
Sheet Start Date:	11/23/20
Drawn / Checked by:	HPA HPA
Drawing No.:	Revision:

A301

APPENDIX B

TIA Screening Form

City of Ottawa 2017 TIA Guidelines Screening Form

1. Description of Proposed Development

Municipal Address	829 Carling Avenue
Description of Location	Located directly north of Carling Avenue, south of Sidney Street, and west of Preston Street
Land Use Classification	Residential apartments, ground floor retail
Development Size (units)	459 dwellings
Development Size (m ²)	286 m² (3,080 ft²) GFA retail
Number of Accesses and Locations	One proposed access to Sidney Street
Phase of Development	1
Buildout Year	2028

If available, please attach a sketch of the development or site plan to this form.

2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size
Single-family homes	40 units
<i>Townhomes or apartments</i>	<i>90 units</i>
Office	3,500 m ²
Industrial	5,000 m ²
Fast-food restaurant or coffee shop	100 m ²
Destination retail	1,000 m ²
Gas station or convenience market	75 m ²

** If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.*

If the proposed development size is greater than the sizes identified above, the Trip Generation Trigger is satisfied.

3. Location Triggers

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?		✓
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*	✓	

*DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

4. Safety Triggers

	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		✓
Are there any horizontal/vertical curvatures on a boundary street limiting sight lines at a proposed driveway?		✓
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/suburban conditions)?	✓	
Is the proposed driveway within auxiliary lanes of an intersection?		✓
Does the proposed driveway make use of an existing median break that serves an existing site?		✓
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?		✓
Does the development include a drive-thru facility?		✓

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

5. Summary

	Yes	No
Does the development satisfy the Trip Generation Trigger?	✓	
Does the development satisfy the Location Trigger?	✓	
Does the development satisfy the Safety Trigger?	✓	

If none of the triggers are satisfied, the TIA Study is complete. If one or more of the triggers is satisfied, the TIA Study must continue into the next stage (Screening and Scoping).

APPENDIX C

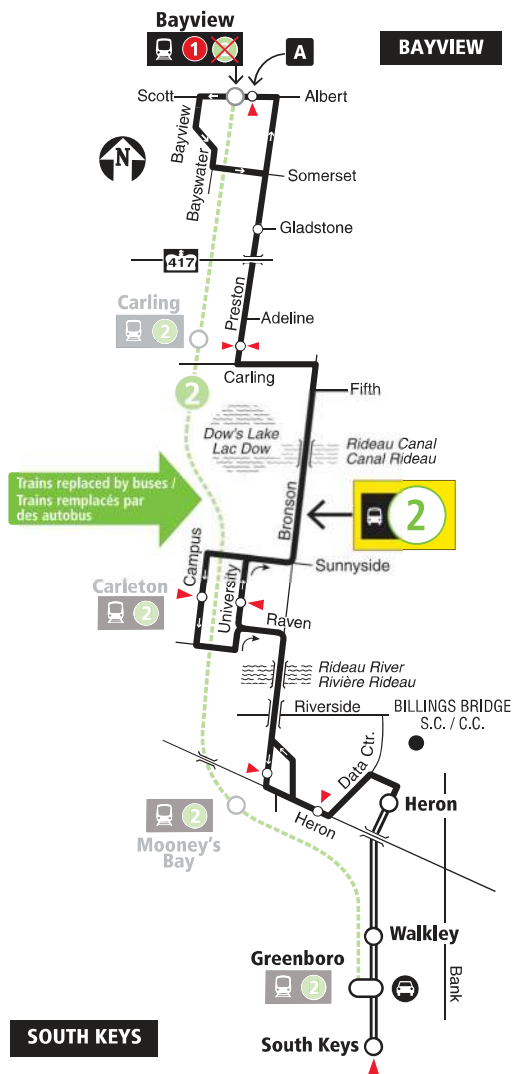
OC Transpo Route Maps



2 BAYVIEW SOUTH KEYS

Bus service during
O-Train Line 2 expansion

Service d'autobus durant le
prolongement de la Ligne 2 de l'O-Train



2020.09



Schedule / Horaire.....613-560-1000

Text / Texto 560560

plus your four digit bus stop number / plus votre numéro d'arrêt à quatre chiffres

Customer Service

Service à la clientèle 613-741-4390

Lost and Found / Objets perdus..... 613-563-4011

Security / Sécurité 613-741-2478

Effective Fall 2020

En vigueur automne 2020



INFO 613-741-4390
octranspo.com

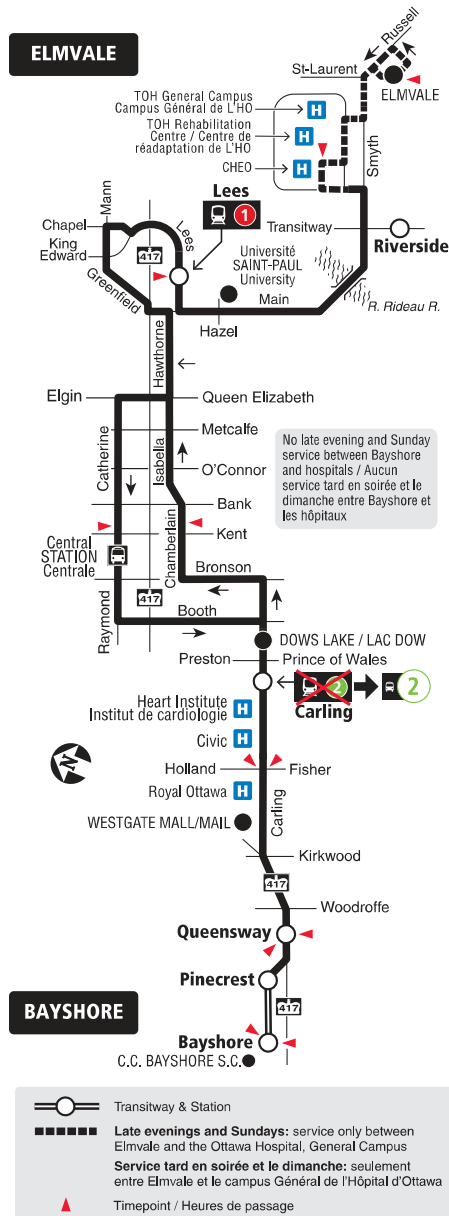
55

ELMVALE BAYSHORE

Local

7 days a week / 7 jours par semaine

On Sundays and evenings, service only between Elmvale and General campus of the Ottawa Hospital /
Service le dimanche et en soirée seulement entre Elmvale et le campus Général de l'Hôpital d'Ottawa



2020.06



Schedule / Horaire.....613-560-1000

Text / Texto 560560

plus your four digit bus stop number / plus votre numéro d'arrêt à quatre chiffres

Customer Service

Service à la clientèle 613-741-4390

Lost and Found / Objets perdus..... 613-563-4011

Security / Sécurité 613-741-2478

Effective June 29, 2020

En vigueur 29 juin 2020



INFO 613-741-4390
octranspo.com

56

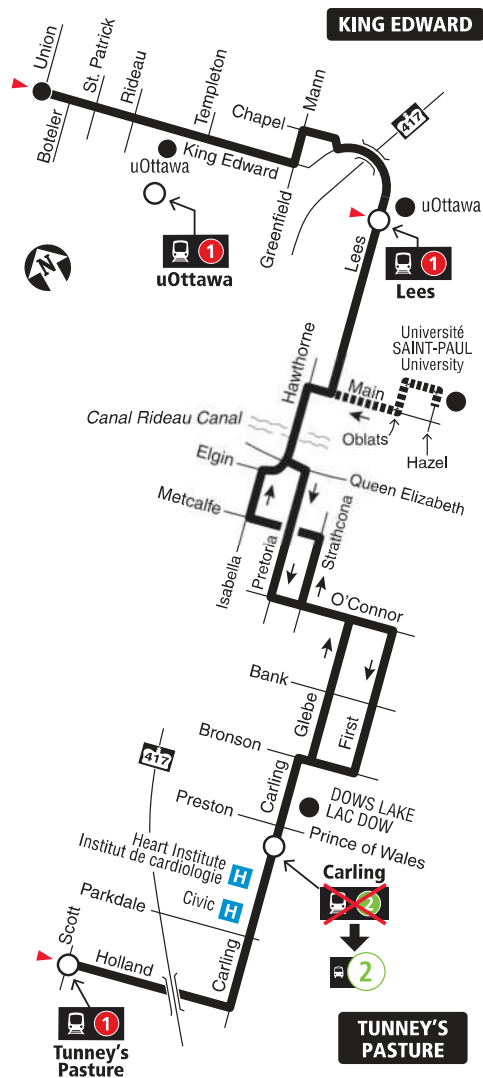
KING EDWARD TUNNEY'S PASTURE

Local

Monday to Friday / Lundi au vendredi

Peak periods only

Périodes de pointe seulement



Station
 Some trips / Certains trajets
 Timepoint / Heures de passage

2020.04



Schedule / Horaire.....613-560-1000

Text / Texto 560560

plus your four digit bus stop number / plus votre numéro d'arrêt à quatre chiffres

Customer Service

Service à la clientèle 613-741-4390

Lost and Found / Objets perdus.....613-563-4011

Security / Sécurité 613-741-2478

Effective May 3, 2020

En vigueur 3 mai 2020



INFO 613-741-4390
octranspo.com

85

GATINEAU

BAYSHORE

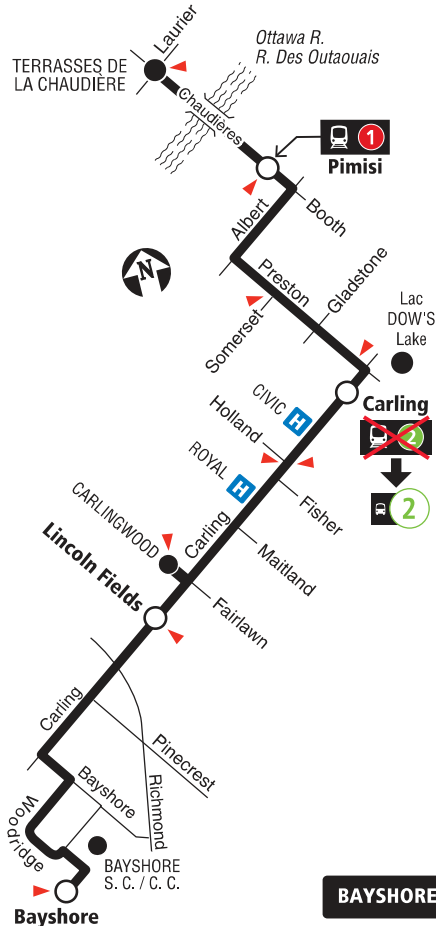
Fréquent

7 days a week / 7 jours par semaine

All day service

Service toute la journée

GATINEAU



- Station
- ▲ Timepoint / Heures de passage

2020.04



Schedule / Horaire.....613-560-1000

Text / Texto 560560

plus your four digit bus stop number / plus votre numéro d'arrêt à quatre chiffres

Customer Service

Service à la clientèle 613-741-4390

Lost and Found / Objets perdus.....613-563-4011

Security / Sécurité.....613-741-2478

Effective May 3, 2020

En vigueur 3 mai 2020



INFO 613-741-4390
octranspo.com

APPENDIX D

Traffic Count Data

Turning Movement Count - Peak Hour Diagram

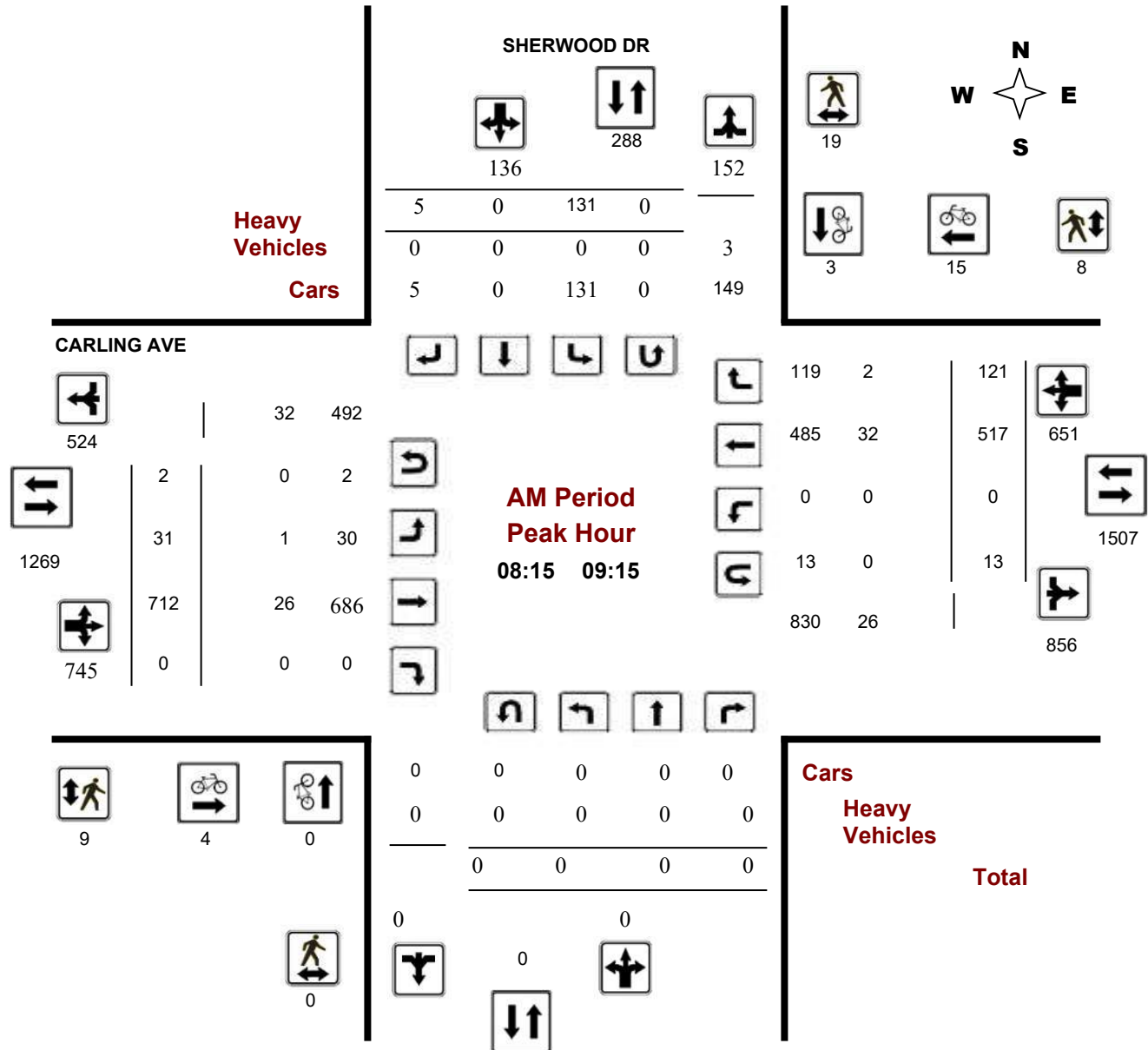
CARLING AVE @ SHERWOOD DR

Survey Date: Thursday, August 25, 2016

Start Time: 07:00

WO No: 36249

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

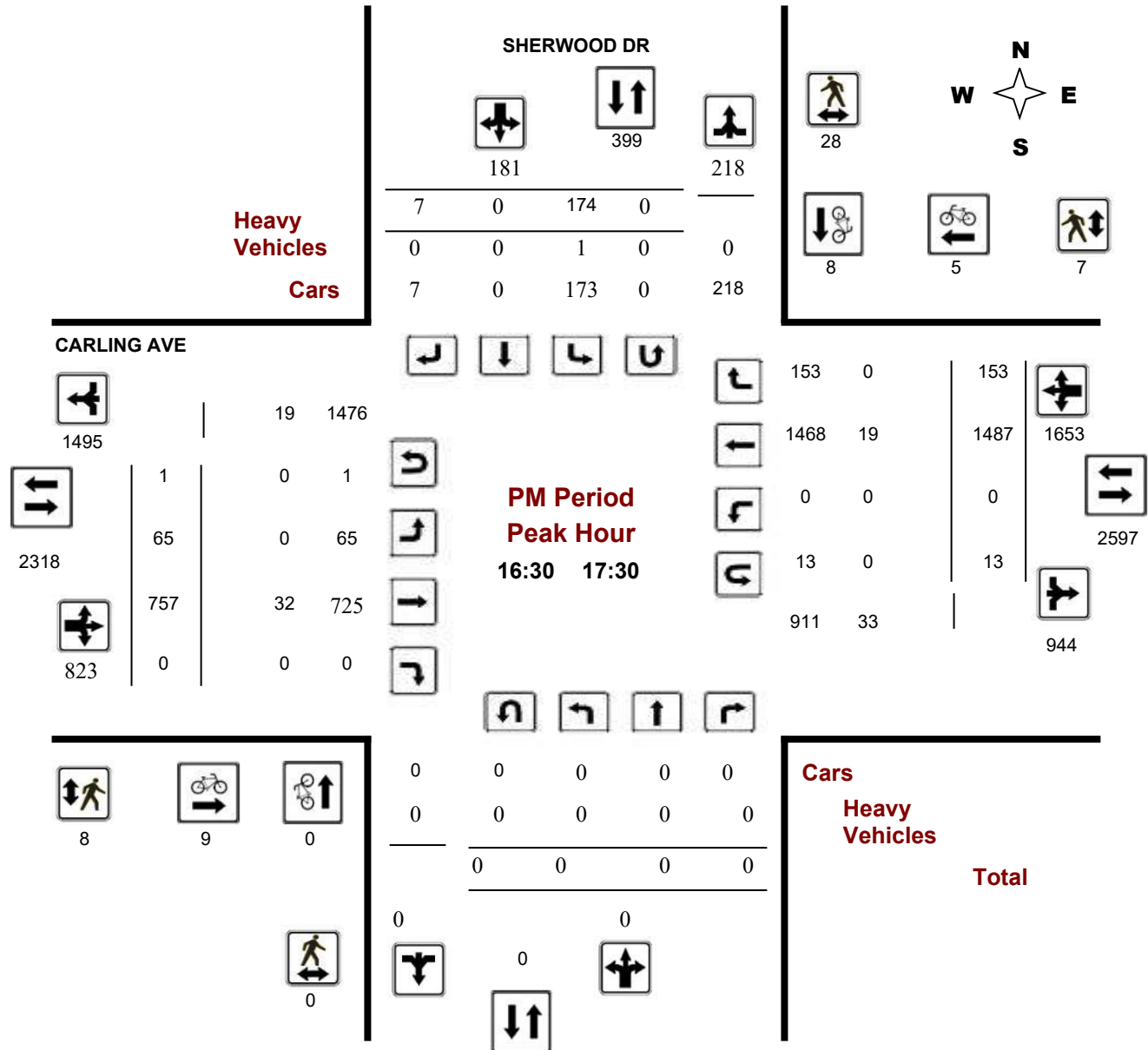
CARLING AVE @ SHERWOOD DR

Survey Date: Thursday, August 25, 2016

Start Time: 07:00

WO No: 36249

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

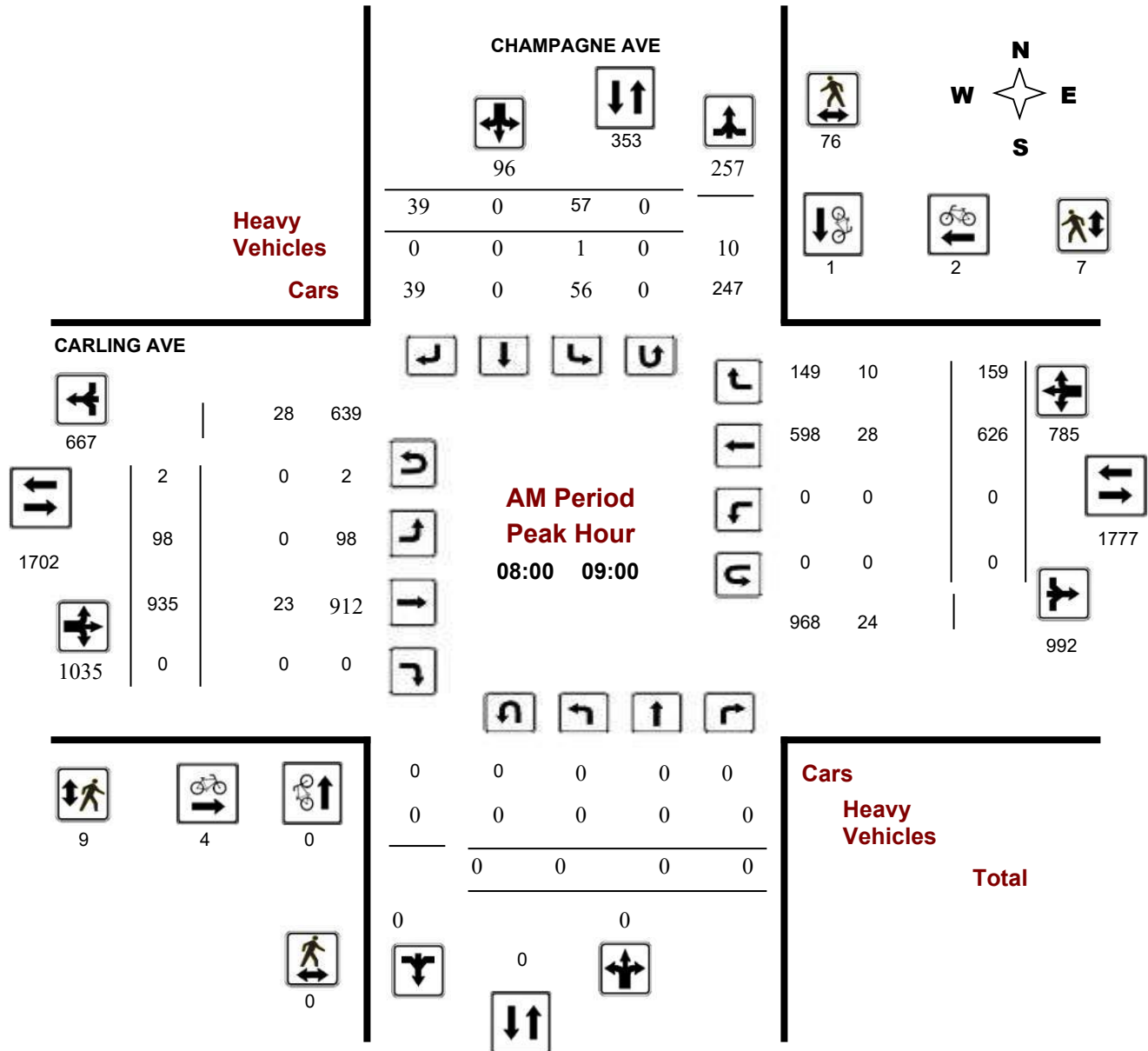
CARLING AVE @ CHAMPAGNE AVE

Survey Date: Thursday, February 04, 2016

Start Time: 07:00

WO No: 35697

Device: Miovision





Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

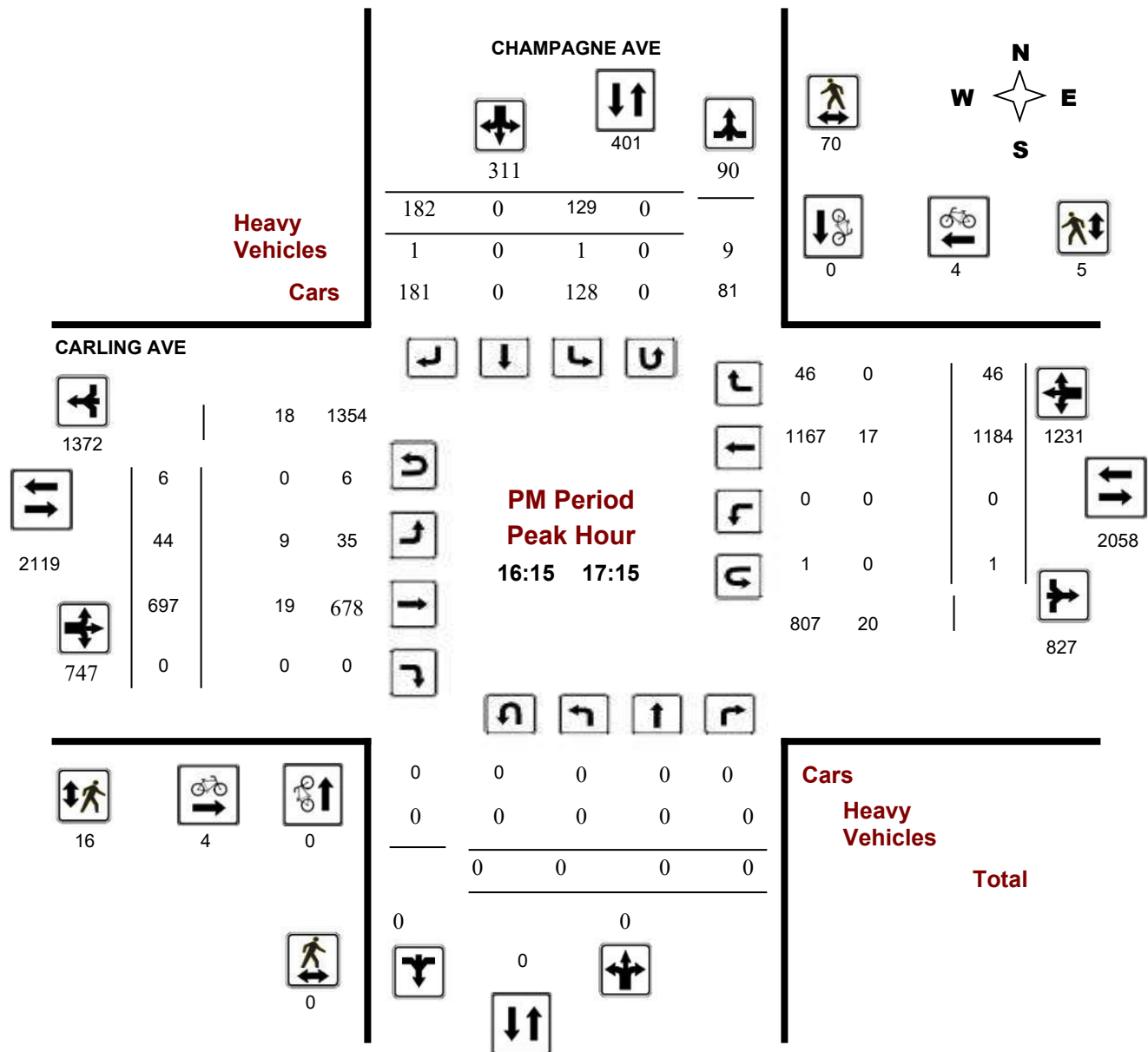
CARLING AVE @ CHAMPAGNE AVE

Survey Date: Thursday, February 04, 2016

Start Time: 07:00

WO No: 35697

Device: Miovision



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

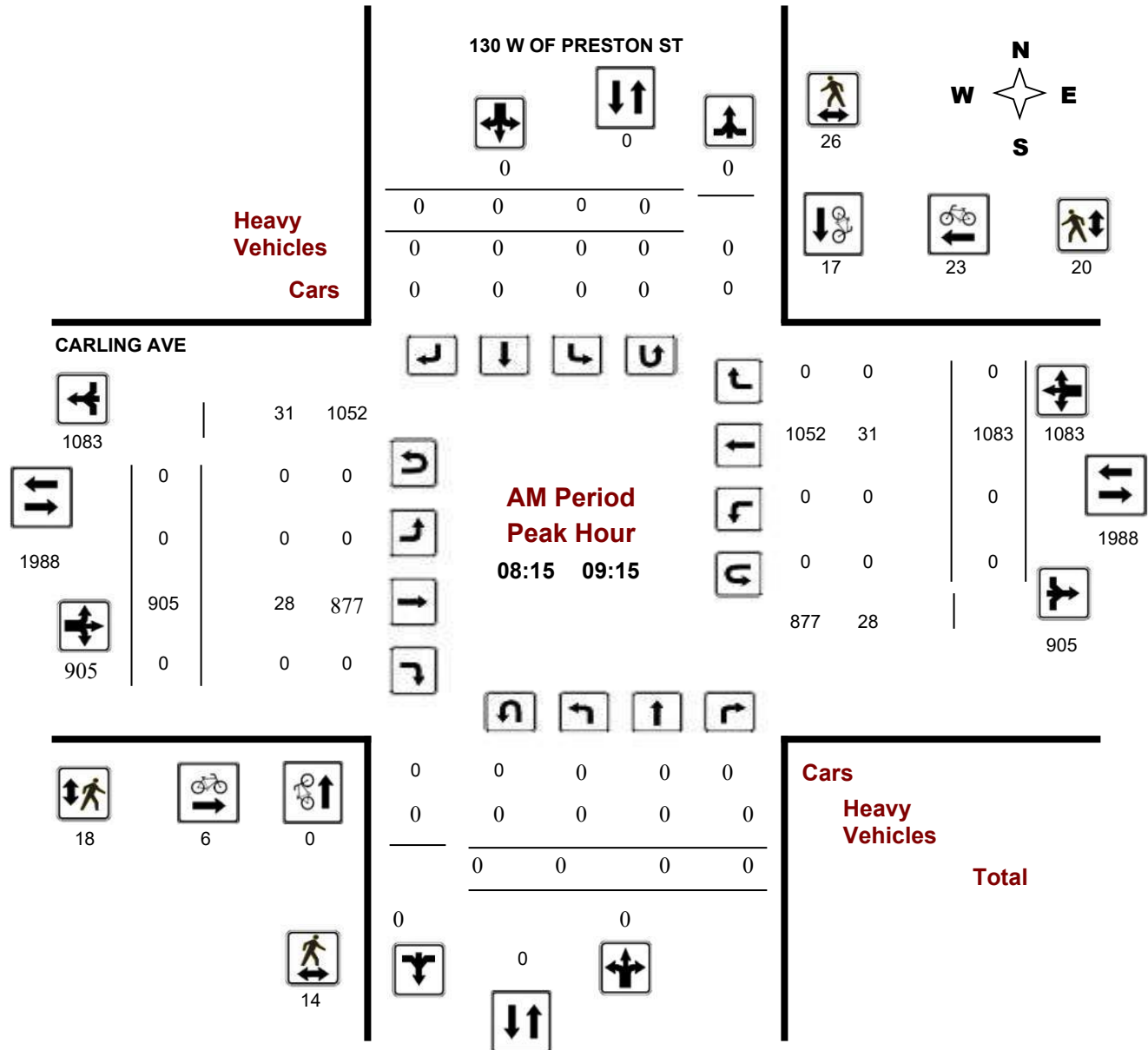
130 W OF PRESTON ST @ CARLING AVE

Survey Date: Wednesday, July 13, 2016

Start Time: 07:00

WO No: 36033

Device: Miovision



Comments INTERSECTION : CARLING AVE 130M WEST OF PRESTON ST

Turning Movement Count - Peak Hour Diagram

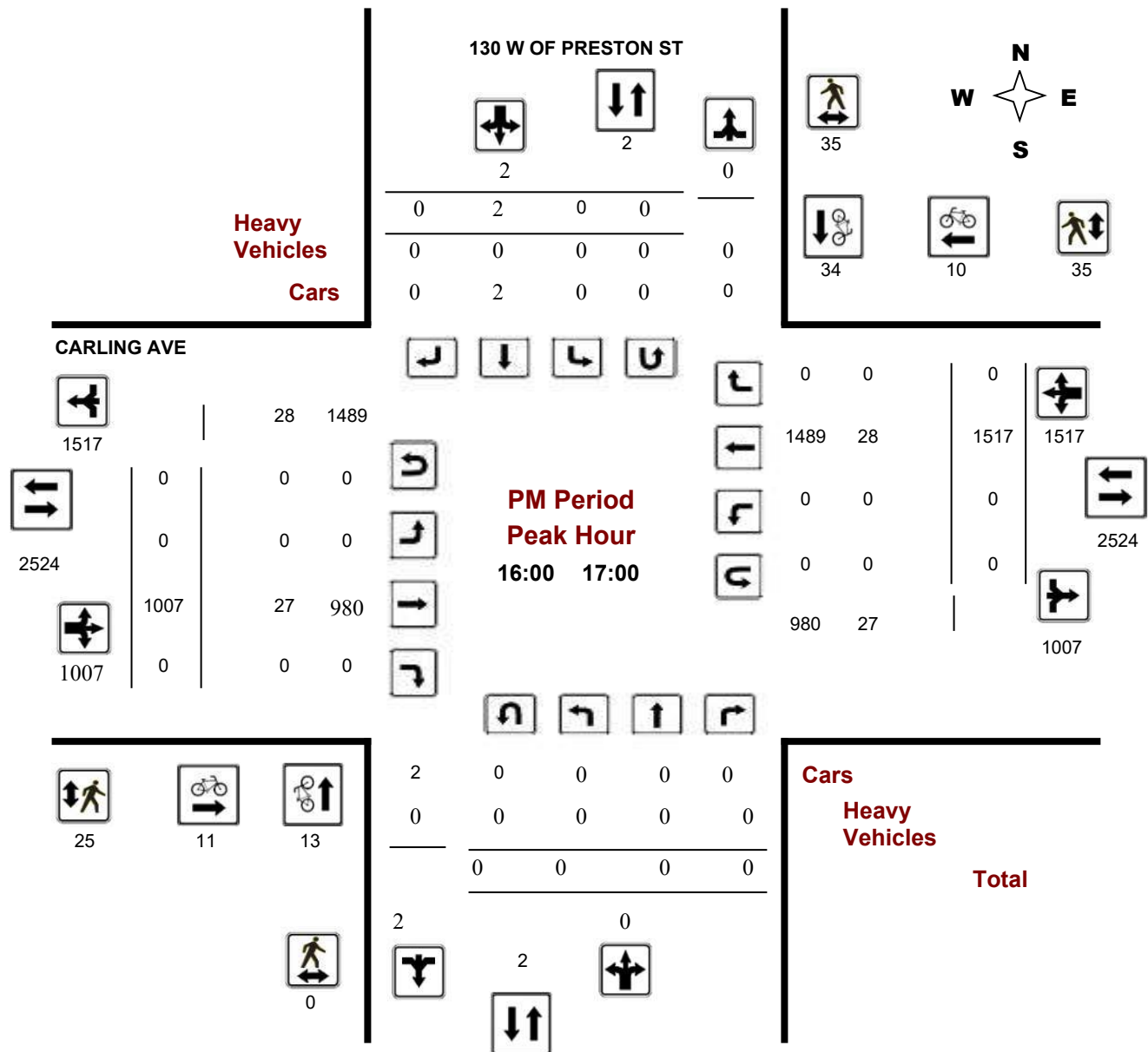
130 W OF PRESTON ST @ CARLING AVE

Survey Date: Wednesday, July 13, 2016

Start Time: 07:00

WO No: 36033

Device: Miovision





Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

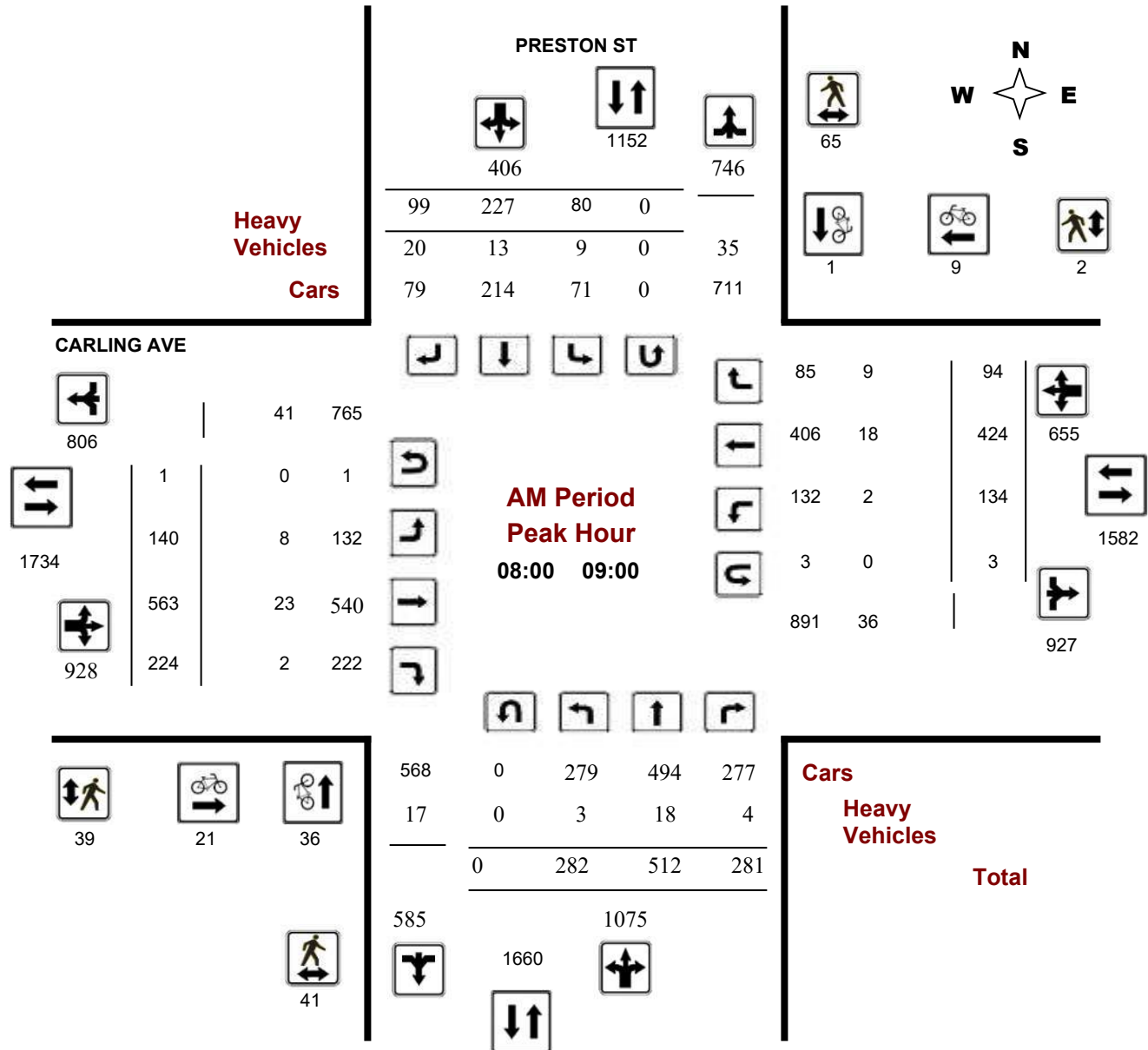
CARLING AVE @ PRESTON ST

Survey Date: Tuesday, June 20, 2017

Start Time: 07:00

WO No: 37131

Device: Miovision





Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

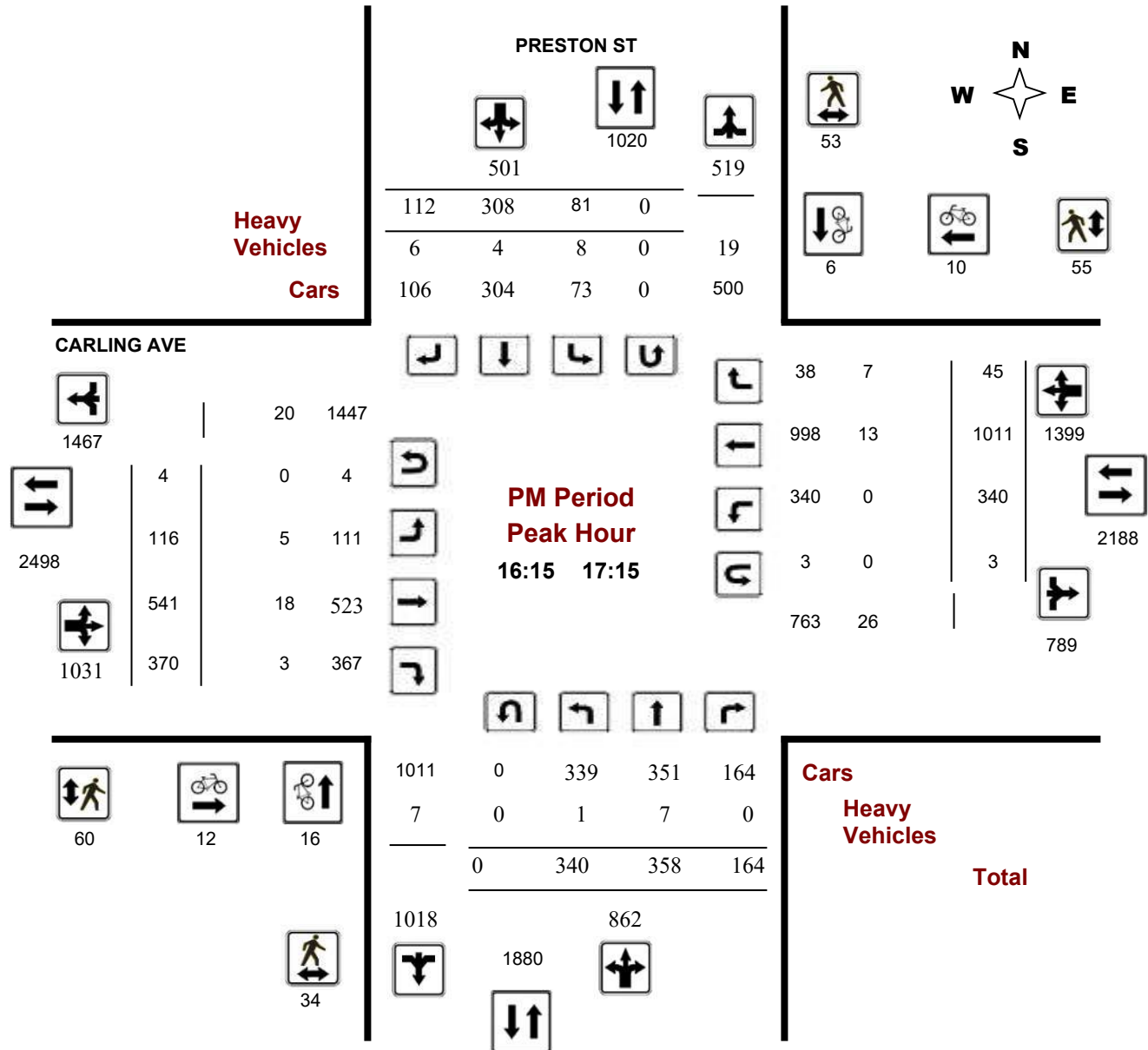
CARLING AVE @ PRESTON ST

Survey Date: Tuesday, June 20, 2017

Start Time: 07:00

WO No: 37131

Device: Miovision



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

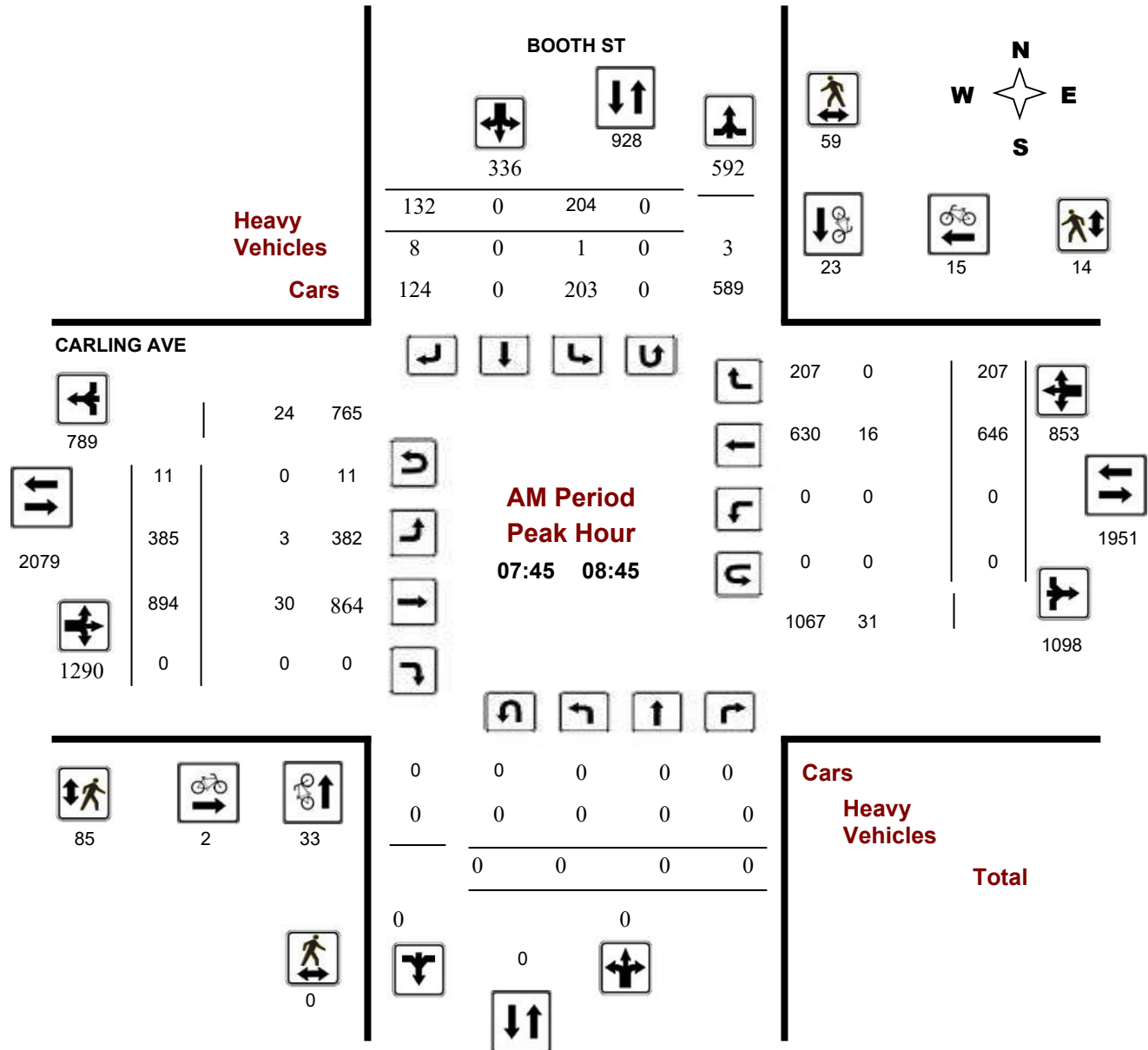
BOOTH ST @ CARLING AVE

Survey Date: Thursday, September 12, 2019

Start Time: 07:00

WO No: 38761

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

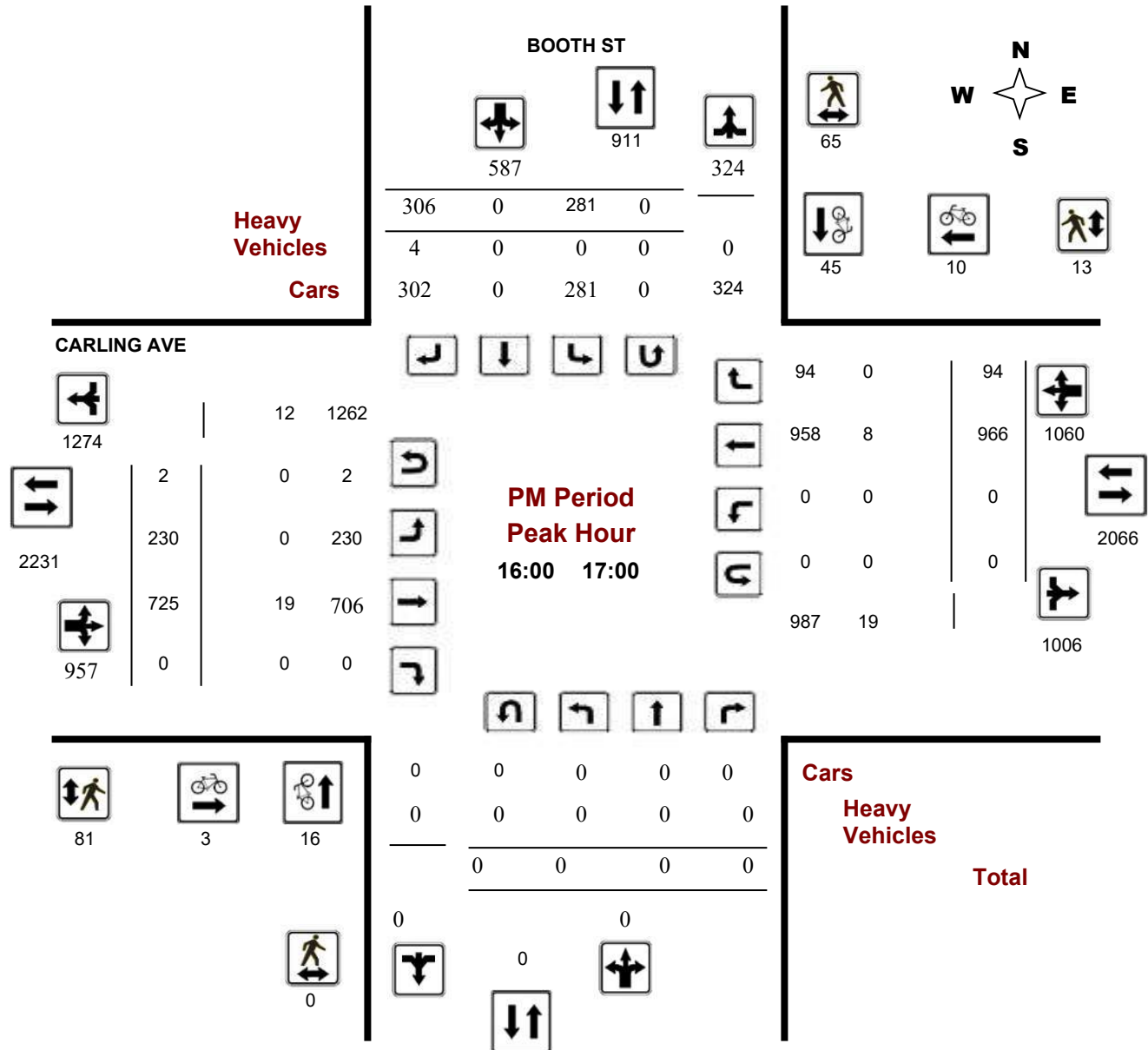
BOOTH ST @ CARLING AVE

Survey Date: Thursday, September 12, 2019

Start Time: 07:00

WO No: 38761

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

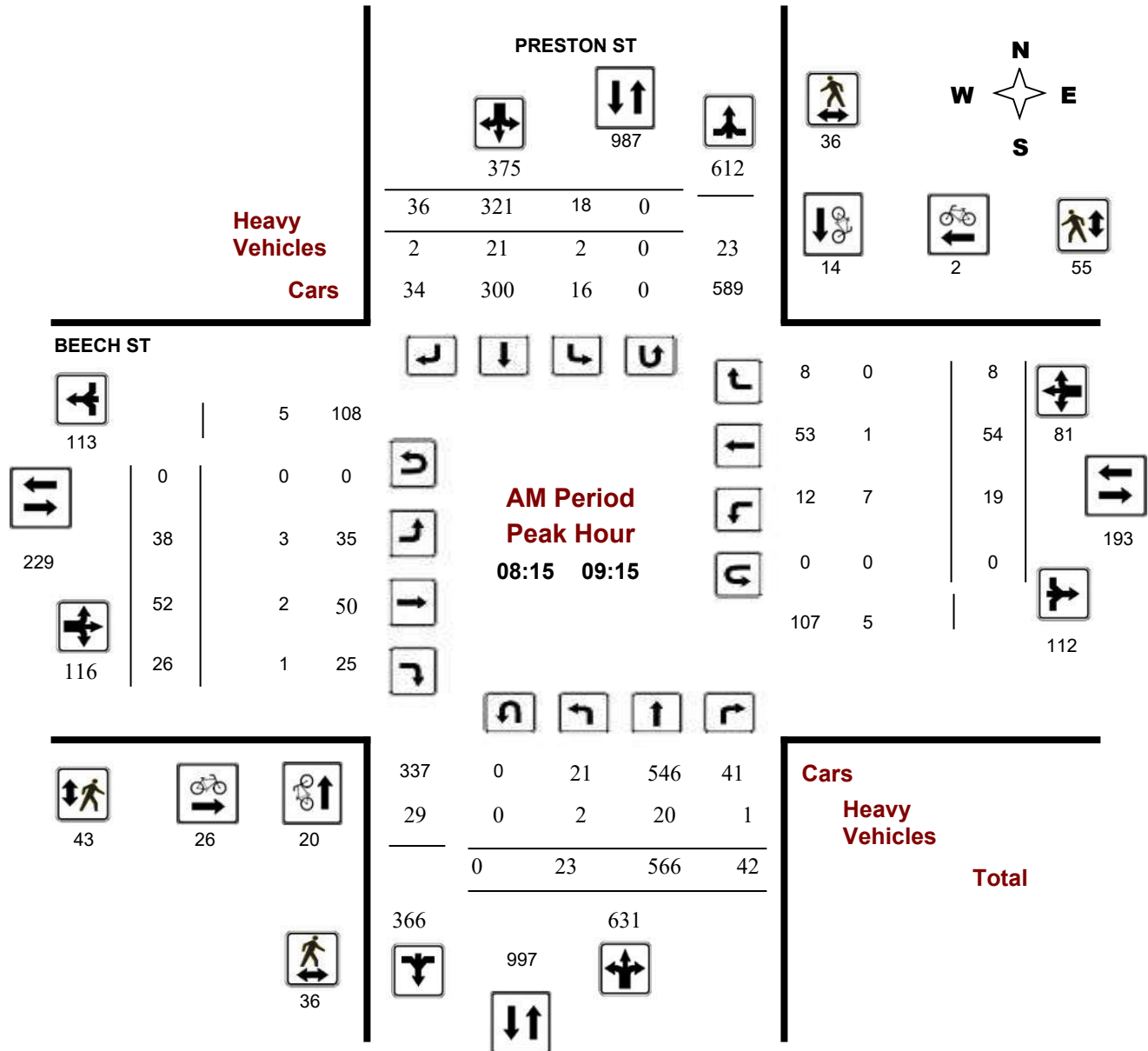
BEECH ST @ PRESTON ST

Survey Date: Wednesday, September 07, 2016

Start Time: 07:00

WO No: 36281

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

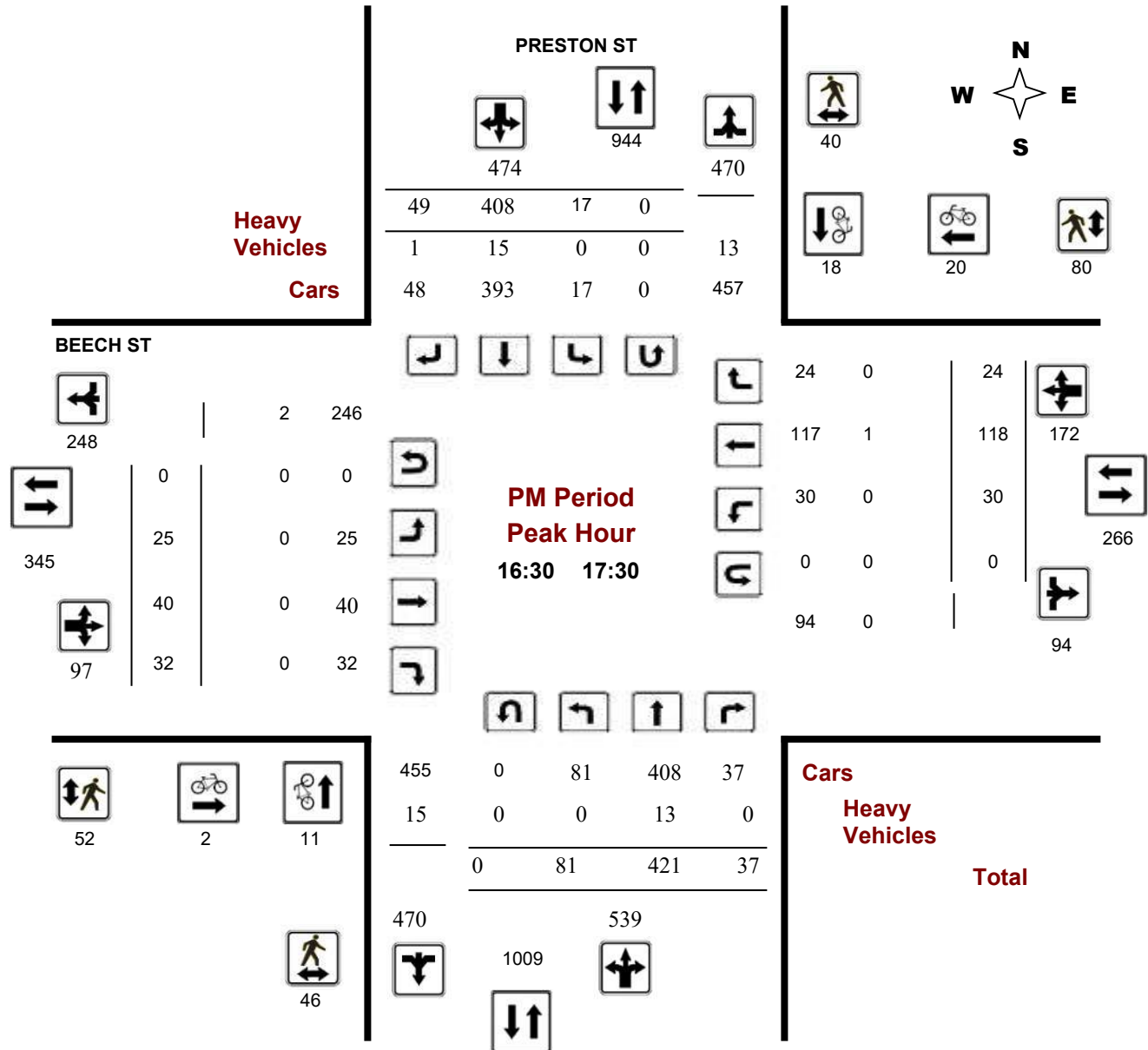
BEECH ST @ PRESTON ST

Survey Date: Wednesday, September 07, 2016

Start Time: 07:00

WO No: 36281

Device: Miovision





Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

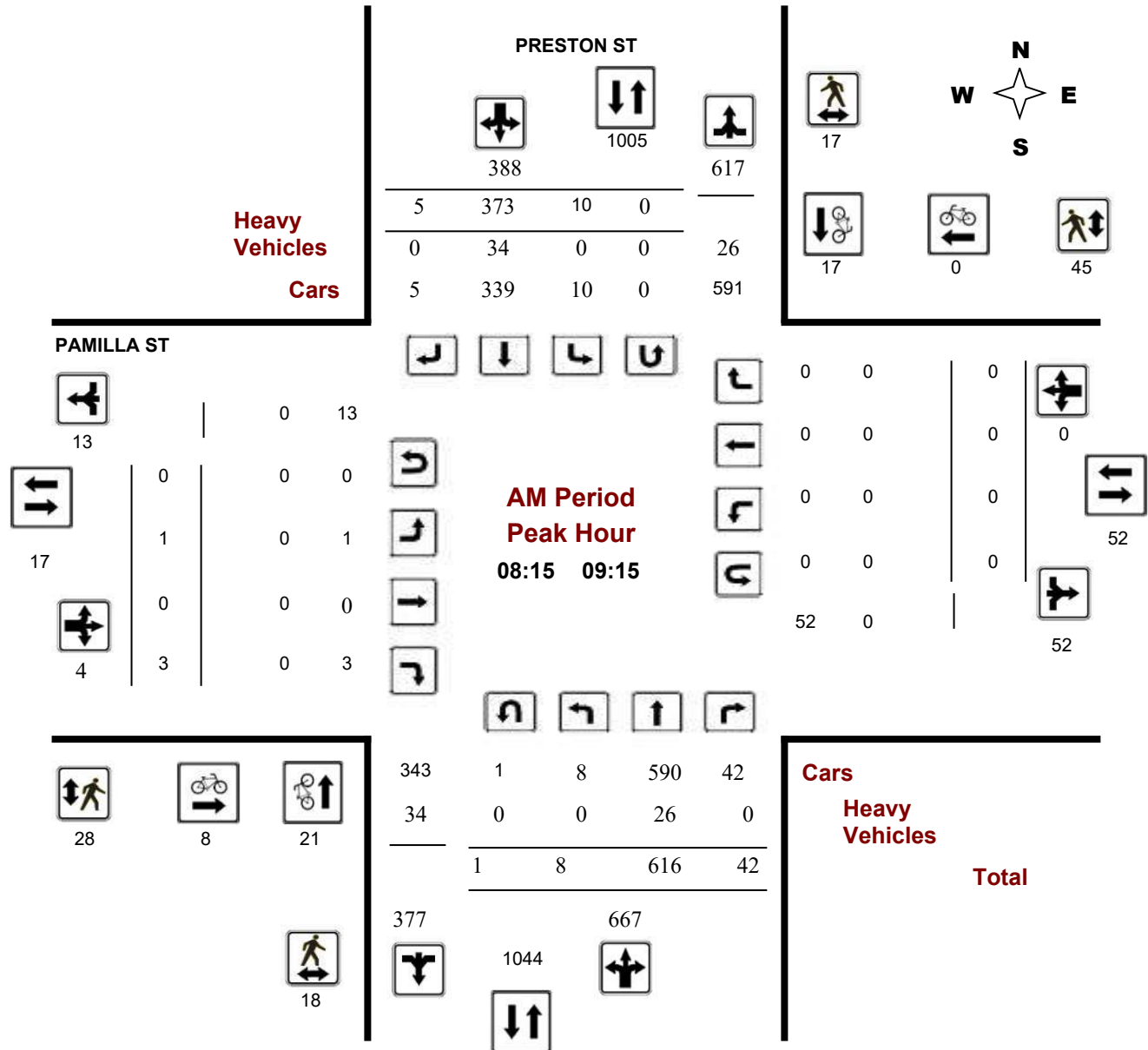
PAMILLA ST @ PRESTON ST

Survey Date: Wednesday, September 07, 2016

Start Time: 07:00

WO No: 36279

Device: Miovision





Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

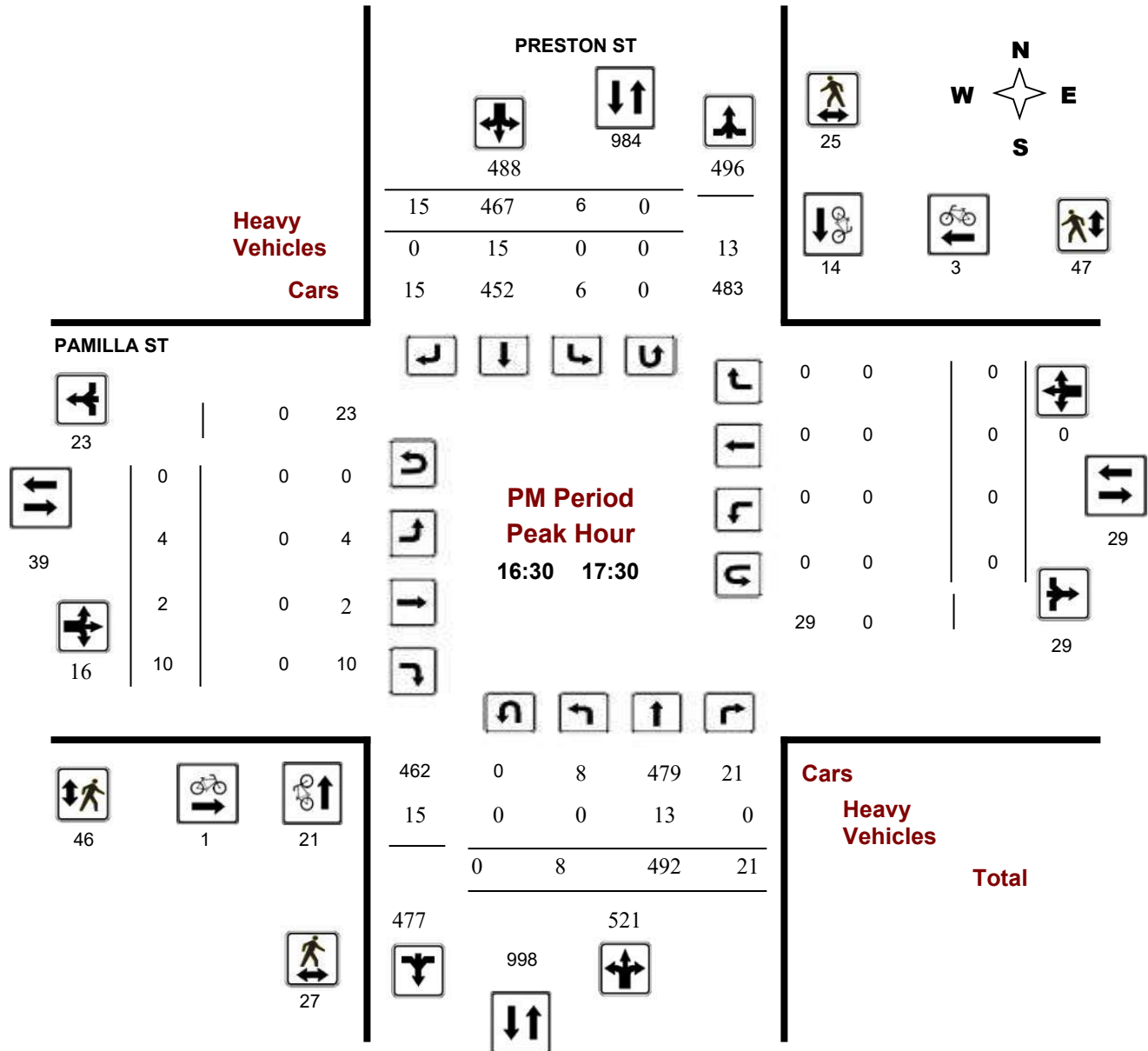
PAMILLA ST @ PRESTON ST

Survey Date: Wednesday, September 07, 2016

Start Time: 07:00

WO No: 36279

Device: Miovision



Comments

Recorded Traffic Volumes



Street 1 Preston Street

Street 2 Adeline Street

Road Conditions wet

Date March 8, 2012

Day Name Thursday

Start Time 07:30

Number of Hours 4

TIME	NORTHBOUND APPROACH ON PRESTON			SOUTHBOUND APPROACH ON PRESTON			EASTBOUND APPROACH ON ADELINE			WESTBOUND APPROACH ON ADELINE		
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT
0730-0745	1	N/A	13	4	N/A	1	0	0	0	1	0	3
0745-0800	4	N/A	12	2	N/A	3	0	1	0	3	2	4
0800-0815	3	N/A	12	8	N/A	5	1	1	6	3	0	2
0815-0830	0	N/A	18	2	N/A	4	3	0	2	3	0	4
0830-0845	4	N/A	24	13	N/A	3	4	0	2	3	3	6
0845-0900	2	N/A	30	2	N/A	3	1	0	1	1	2	5
0900-0915	0	N/A	19	2	N/A	4	1	0	3	3	1	3
0915-0930	1	N/A	12	2	N/A	4	2	0	4	3	1	4
SUB TOTAL	15	0	140	35	0	27	12	2	18	20	9	31
1530-1545	2	N/A	5	3	N/A	1	1	1	2	1	0	3
1545-1600	1	N/A	7	2	N/A	1	1	0	4	1	0	3
1600-1615	0	N/A	8	1	N/A	1	0	0	2	5	0	2
1615-1630	3	N/A	8	1	N/A	1	0	2	4	3	0	1
1630-1645	4	N/A	12	4	N/A	1	2	1	2	3	0	2
1645-1700	0	N/A	12	5	N/A	2	2	1	1	3	2	3
1700-1715	0	N/A	0	0	N/A	0	0	0	0	0	0	0
1715-1730	0	N/A	0	0	N/A	0	0	0	0	0	0	0
SUB TOTAL	10	0	52	16	0	7	6	5	15	16	2	14
TOTAL	25	0	192	51	0	34	18	7	33	36	11	45

AM
PEAK

PM
PEAK

DIRECTIONAL TRAFFIC FLOW

Intersection: Peston at Sidney

DATE: Day: 2 Month: March Year: 2011 Day of Week: Wednesday

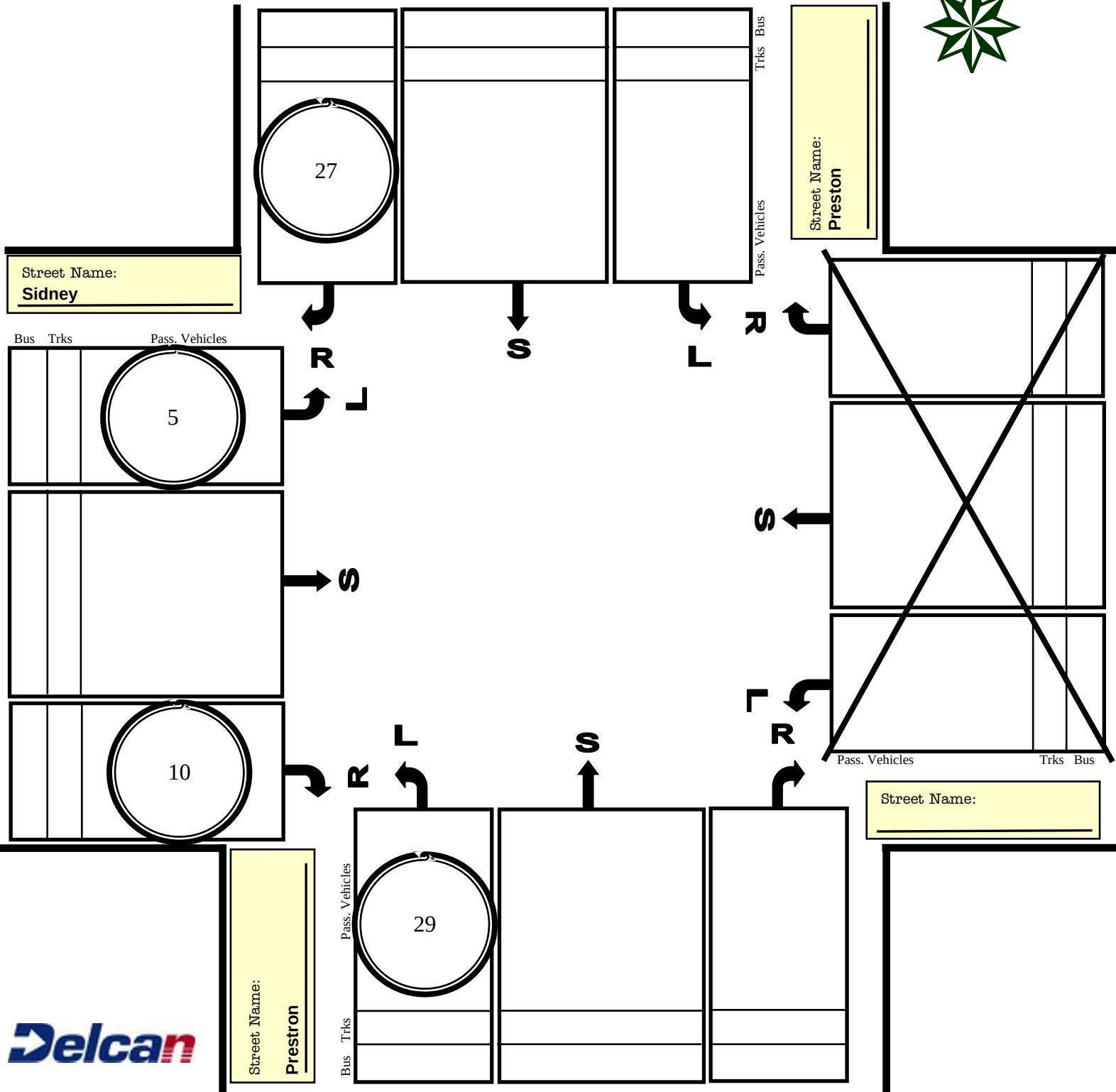
Observer: Kyle Delaney Weather: Cloudy

Chkd by: _____ Date: _____

TIME PERIOD: From: 7 : 30 To: 8 : 30

- Instructions: 1) Use tally marks to indicate vehicles.
2) Use one sheet for each 15-minute period.

N



DIRECTIONAL TRAFFIC FLOW

Intersection: **Peston** at **Sidney**

DATE: Day: **1** Month: **March** Year: **2011** Day of Week: **Tuesday**

Observer: **Kyle Delaney** Weather: **Clear**

Chkd by: _____ Date: _____

TIME PERIOD: From: **4** : **00** To: **5** : **00**

Instructions: 1) Use tally marks to indicate vehicles.

2) Use one sheet for each 15-minute period.

N

[illegible]

Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

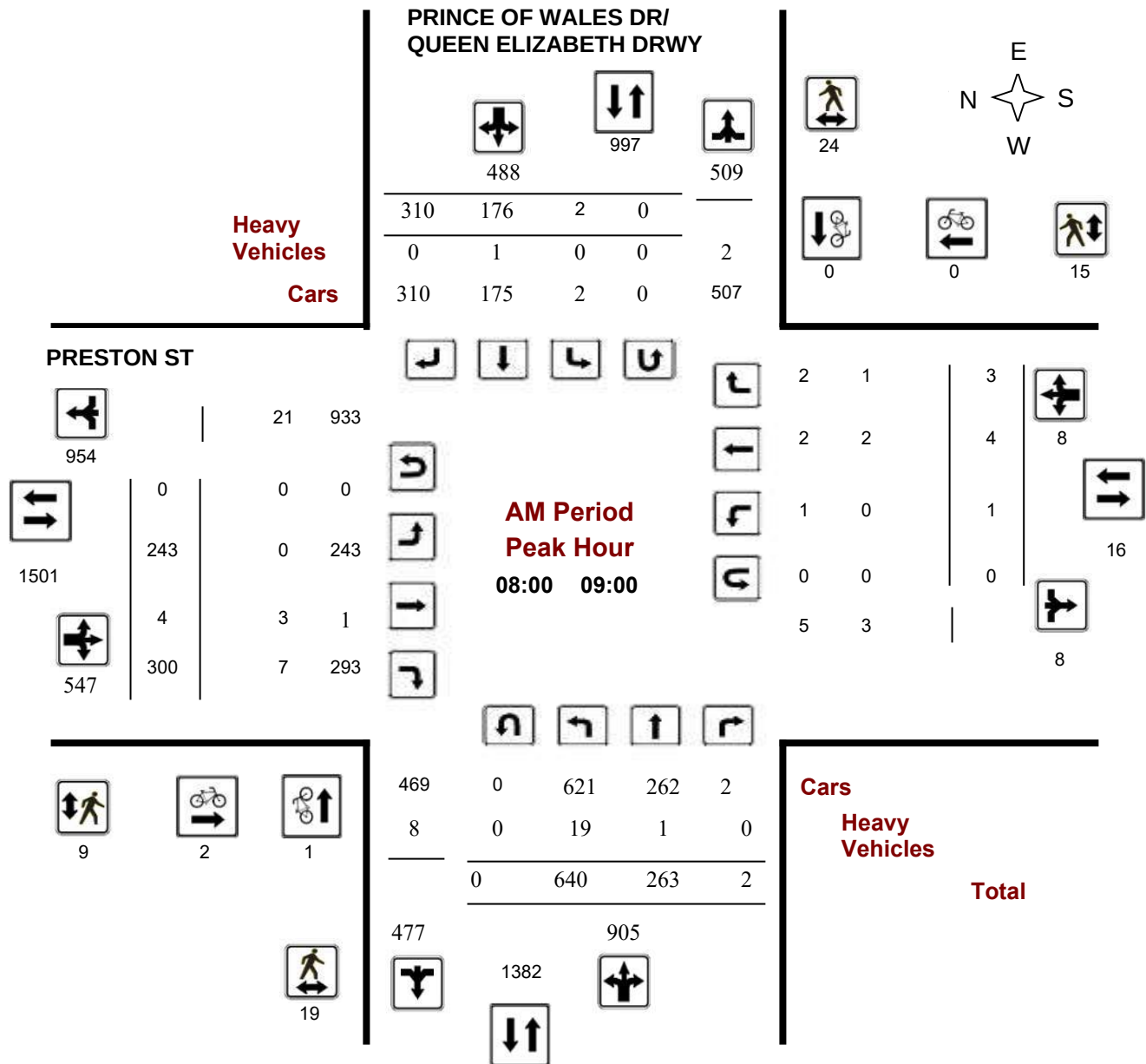
PRESTON ST @ PRINCE OF WALES DR/QUEEN ELIZABET

Survey Date: Wednesday, January 10, 2018

Start Time: 07:00

WO No: 37407

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

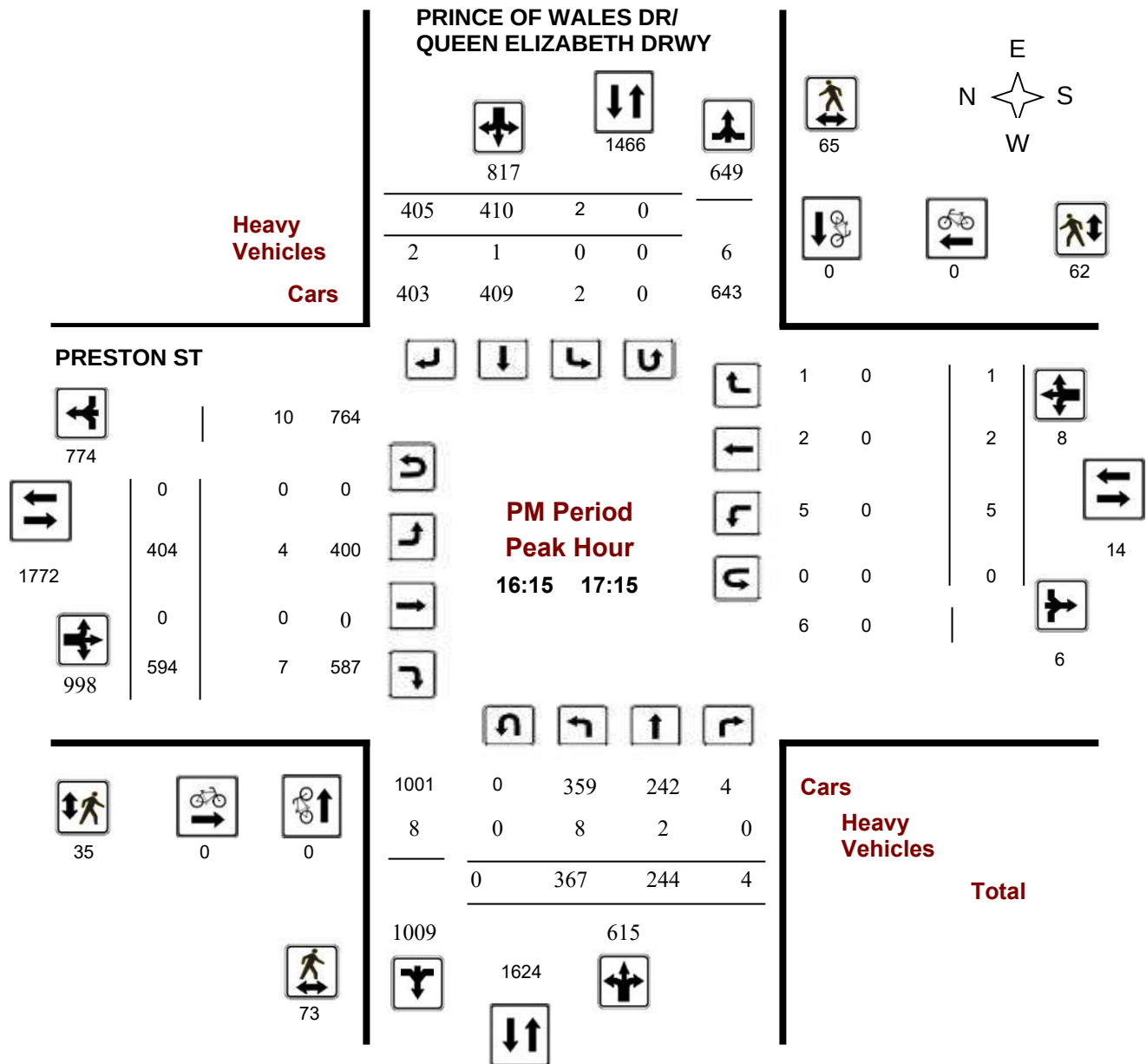
PRESTON ST @ PRINCE OF WALES DR/QUEEN ELIZABET

Survey Date: Wednesday, January 10, 2018

Start Time: 07:00

WO No: 37407

Device: Miovision



Comments

APPENDIX E

Collision Records



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: 130 W OF PRESTON ST @ CARLING AVE

Traffic Control: Traffic signal

Total Collisions: 4

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2018-Apr-12, Thu,10:02	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Passenger van	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-May-21, Mon,18:31	Clear	Angle	Non-fatal injury	Dry	North	Going ahead	Bicycle	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Cyclist	
2019-May-07, Tue,11:42	Clear	Angle	Non-fatal injury	Dry	West	Slowing or stopping	Automobile, station wagon	Cyclist	0
					North	Going ahead	Bicycle	Other motor vehicle	
2019-May-16, Thu,18:02	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Cyclist	0
					North	Going ahead	Bicycle	Other	
					North	Going ahead	Bicycle	Other motor vehicle	

Location: BEECH ST @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 7

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Jan-26, Mon,12:42	Clear	Other	P.D. only	Dry	North	Reversing	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2015-Apr-12, Sun,19:59	Clear	Turning movement	P.D. only	Dry	South	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Apr-13, Thu,11:25	Clear	Turning movement	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Oct-01, Sun,15:15	Clear	Sideswipe	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Oct-24, Tue,14:26	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: BEECH ST @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 7

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Mar-28, Thu,16:20	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Oct-24, Thu,20:49	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: BOOTH ST @ CARLING AVE

Traffic Control: Traffic signal

Total Collisions: 20

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Apr-02, Thu,16:29	Clear	Rear end	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Pick-up truck	Other motor vehicle	
2015-May-06, Wed,10:23	Clear	Rear end	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2015-Jun-30, Tue,16:26	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Jul-23, Thu,10:34	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Truck and trailer	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Oct-27, Thu,16:41	Snow	Turning movement	P.D. only	Wet	East	Turning left	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jan-10, Tue,17:30	Snow	Turning movement	P.D. only	Loose snow	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Jan-18, Thu,17:27	Snow	Rear end	P.D. only	Wet	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-07, Thu,16:17	Clear	Turning movement	P.D. only	Dry	East	Turning left	Passenger van	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: BOOTH ST @ CARLING AVE

Traffic Control: Traffic signal

Total Collisions: 20

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2018-Jun-21, Thu,15:48	Clear	Rear end	Non-fatal injury	Dry	East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jul-13, Fri,15:32	Clear	Turning movement	Non-fatal injury	Dry	South	Turning left	Automobile, station wagon	Cyclist	0
					North	Going ahead	Bicycle	Other motor vehicle	
2018-Sep-19, Wed,10:51	Clear	Sideswipe	P.D. only	Dry	South	Unknown	Automobile, station wagon	Other motor vehicle	0
					South	Unknown	Automobile, station wagon	Other motor vehicle	
2018-Sep-28, Fri,16:30	Clear	Rear end	P.D. only	Dry	West	Going ahead	School bus	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Oct-18, Thu,07:03	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jan-24, Thu,14:20	Clear	Sideswipe	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Feb-14, Thu,12:33	Clear	Turning movement	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2019-May-22, Wed,06:15	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2019-Jun-15, Sat,17:53	Rain	Turning movement	P.D. only	Wet	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jun-18, Tue,22:00	Clear	Turning movement	P.D. only	Dry	West	Making "U" turn	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Dec-01, Sun,10:44	Clear	Angle	P.D. only	Dry	South	Turning right	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: BOOTH ST @ CARLING AVE

Traffic Control: Traffic signal

Total Collisions: 20

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Dec-17, Tue,17:46	Clear	Rear end	P.D. only	Dry	East	Unknown	Unknown	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: CARLING AVE @ CHAMPAGNE AVE

Traffic Control: Traffic signal

Total Collisions: 11

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Feb-02, Mon,11:48	Snow	Angle	P.D. only	Packed snow	East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2015-Mar-27, Fri,08:25	Snow	Sideswipe	P.D. only	Wet	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Jun-16, Tue,21:15	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Pick-up truck	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Oct-05, Wed,12:52	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Pick-up truck	Other motor vehicle	
2017-Aug-15, Tue,16:57	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-15, Mon,16:15	Clear	Sideswipe	P.D. only	Packed snow	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Pick-up truck	Other motor vehicle	
2018-Nov-24, Sat,13:45	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-May-15, Wed,08:30	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ CHAMPAGNE AVE

Traffic Control: Traffic signal

Total Collisions: 11

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2019-Sep-03, Tue,21:10	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Bus (other)	Other motor vehicle	0
					West	Going ahead	Motorcycle	Other motor vehicle	
2019-Dec-04, Wed,16:28	Snow	Rear end	P.D. only	Wet	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Dec-28, Sat,14:25	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Passenger van	Other motor vehicle	

Location: CARLING AVE @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 56

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2015-Jan-16, Fri,15:45	Clear	Rear end	Non-fatal injury	Loose snow	West	Turning right	Passenger van	Other motor vehicle	0
					West	Turning right	Automobile, station wagon	Other motor vehicle	
2015-Mar-19, Thu,19:32	Clear	Sideswipe	P.D. only	Slush	South	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Apr-15, Wed,09:04	Clear	Rear end	P.D. only	Dry	East	Unknown	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Passenger van	Other motor vehicle	
2015-Apr-29, Wed,20:00	Clear	Turning movement	P.D. only	Dry	South	Turning left	Automobile, station wagon	Cyclist	0
					North	Going ahead	Bicycle	Other motor vehicle	
2015-May-12, Tue,18:50	Clear	Rear end	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2015-May-26, Tue,23:49	Clear	SMV other	P.D. only	Dry	East	Reversing	Municipal transit bus	Concrete guide rail	0
2015-Jun-06, Sat,21:44	Clear	Turning movement	Non-fatal injury	Dry	South	Turning left	Bicycle	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Cyclist	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 56

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Jun-23, Tue,23:02	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Intercity bus	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2015-Jun-24, Wed,15:39	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Municipal transit bus	Other motor vehicle	0
					West	Stopped	Pick-up truck	Other motor vehicle	
					West	Slowing or stopping	Passenger van	Other motor vehicle	
2015-Jul-30, Thu,19:39	Clear	Angle	P.D. only	Dry	North	Turning right	Automobile, station wagon	Cyclist	0
					East	Going ahead	Bicycle	Other motor vehicle	
2015-Aug-17, Mon,14:00	Clear	Sideswipe	P.D. only	Dry	South	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					South	Changing lanes	Automobile, station wagon	Other motor vehicle	
2015-Sep-22, Tue,16:56	Clear	Rear end	P.D. only	Dry	East	Unknown	Unknown	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2015-Oct-05, Mon,06:19	Clear	Turning movement	Non-fatal injury	Dry	South	Turning left	Pick-up truck	Cyclist	0
					North	Going ahead	Bicycle	Other motor vehicle	
2015-Oct-17, Sat,13:21	Clear	Rear end	P.D. only	Dry	South	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					South	Slowing or stopping	Pick-up truck	Other motor vehicle	
2016-May-18, Wed,23:39	Clear	Turning movement	Non-fatal injury	Dry	South	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Jul-06, Wed,08:39	Clear	Turning movement	Non-fatal injury	Dry	North	Turning right	Automobile, station wagon	Cyclist	0
					North	Going ahead	Bicycle	Other motor vehicle	
2016-Jul-23, Sat,23:13	Clear	Rear end	P.D. only	Dry	West	Unknown	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Nov-09, Wed,18:00	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 56

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2017-Feb-10, Fri,13:45	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Mar-06, Mon,16:53	Snow	Angle	P.D. only	Ice	South	Overtaking	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Mar-21, Tue,18:10	Clear	Turning movement	P.D. only	Dry	South	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Apr-26, Wed,16:44	Clear	Rear end	P.D. only	Dry	West	Going ahead	Pick-up truck	Other motor vehicle	0
					West	Stopped	Pick-up truck	Other motor vehicle	
					West	Stopped	Pick-up truck	Other motor vehicle	
					West	Stopped	Passenger van	Other motor vehicle	
2017-Apr-27, Thu,16:07	Clear	Rear end	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Pick-up truck	Other motor vehicle	
2017-Jun-26, Mon,08:54	Clear	Turning movement	Non-fatal injury	Dry	South	Turning right	Automobile, station wagon	Cyclist	0
					South	Going ahead	Bicycle	Other motor vehicle	
2017-Jul-29, Sat,00:57	Clear	Angle	Non-fatal injury	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Oct-14, Sat,15:47	Clear	SMV other	P.D. only	Dry	South	Turning left	Automobile, station wagon	Pole (utility, power)	0
2017-Nov-29, Wed,07:36	Clear	Sideswipe	P.D. only	Wet	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Dec-01, Fri,14:10	Rain	SMV other	Non-fatal injury	Wet	South	Turning left	Automobile, station wagon	Pedestrian	1
2018-Jan-12, Fri,10:50	Rain	Turning movement	P.D. only	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2018-Jan-15, Mon,08:54	Clear	Rear end	Non-fatal injury	Dry	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 56

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2018-Jan-26, Fri,16:00	Clear	Rear end	P.D. only	Dry	East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Feb-06, Tue,20:44	Clear	Rear end	Non-fatal injury	Slush	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Mar-01, Thu,14:30	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Mar-17, Sat,15:00	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Mar-21, Wed,18:00	Clear	Sideswipe	P.D. only	Dry	South	Overtaking	Automobile, station wagon	Other motor vehicle	0
					South	Overtaking	Automobile, station wagon	Other motor vehicle	
2018-Apr-12, Thu,17:12	Rain	Rear end	P.D. only	Wet	East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-07, Thu,16:51	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2018-Jun-14, Thu,06:57	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2018-Sep-05, Wed,16:47	Clear	Turning movement	Non-fatal injury	Dry	South	Turning left	Automobile, station wagon	Cyclist	0
					North	Going ahead	Bicycle	Other motor vehicle	
2018-Sep-21, Fri,20:22	Clear	Angle	Non-fatal injury	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Dec-05, Wed,14:47	Snow	Sideswipe	P.D. only	Slush	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 56

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Jan-12, Sat,15:14	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Passenger van	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Mar-27, Wed,08:20	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Pick-up truck	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Apr-15, Mon,15:30	Clear	Sideswipe	P.D. only	Wet	South	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jun-03, Mon,10:40	Rain	Other	P.D. only	Wet	South	Reversing	Pick-up truck	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jun-04, Tue,08:55	Clear	Rear end	P.D. only	Dry	East	Going ahead	Delivery van	Other motor vehicle	0
					East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2019-Jun-28, Fri,15:07	Clear	Rear end	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jul-16, Tue,20:45	Clear	Sideswipe	P.D. only	Dry	North	Merging	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jul-28, Sun,17:30	Clear	Turning movement	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Jul-30, Tue,15:06	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Sep-03, Tue,16:00	Clear	Rear end	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Oct-25, Fri,09:03	Clear	Other	Non-fatal injury	Dry	East	Turning right	Unknown	Cyclist	0
					West	Going ahead	Bicycle	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 56

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Nov-25, Mon,17:30	Rain	Rear end	Non-fatal injury	Wet	East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Nov-29, Fri,16:19	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Truck - open	Other motor vehicle	
2019-Dec-04, Wed,09:20	Snow	Rear end	P.D. only	Ice	East	Turning left	Automobile, station wagon	Skidding/sliding	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Dec-06, Fri,16:09	Clear	SMV other	Non-fatal injury	Wet	North	Turning left	Automobile, station wagon	Pedestrian	1

Location: CARLING AVE @ SHERWOOD DR

Traffic Control: Traffic signal

Total Collisions: 8

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Feb-28, Sat,10:25	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jan-05, Tue,09:01	Clear	Rear end	P.D. only	Ice	West	Slowing or stopping	Delivery van	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2019-Jan-22, Tue,12:30	Clear	Rear end	P.D. only	Packed snow	West	Going ahead	Pick-up truck	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Apr-09, Tue,16:57	Snow	Rear end	P.D. only	Loose snow	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-May-24, Fri,09:30	Clear	Turning movement	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Oct-31, Thu,16:19	Rain	Rear end	Non-fatal injury	Wet	East	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ SHERWOOD DR

Traffic Control: Traffic signal

Total Collisions: 8

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Nov-16, Sat,06:45	Clear	Turning movement	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Dec-04, Wed,09:27	Snow	Other	P.D. only	Wet	East	Slowing or stopping	Automobile, station wagon	Curb	0
					East	Slowing or stopping	Pick-up truck	Other motor vehicle	

Location: PAMILLA ST @ PRESTON ST

Traffic Control: Traffic signal

Total Collisions: 7

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-May-13, Wed,15:06	Clear	Turning movement	P.D. only	Dry	North	Pulling away from shoulder or curb	Pick-up truck	Other motor vehicle	0
					North	Turning right	Pick-up truck	Other motor vehicle	
2016-Dec-31, Sat,22:11	Snow	Approaching	P.D. only	Loose snow	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Feb-25, Sat,15:11	Rain	Angle	P.D. only	Wet	South	Going ahead	Pick-up truck	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Nov-20, Mon,14:15	Clear	SMV other	Non-fatal injury	Dry	North	Going ahead	Unknown	Pedestrian	1
2017-Nov-22, Wed,10:17	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-26, Tue,16:30	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2019-Jan-28, Mon,15:35	Clear	Rear end	P.D. only	Wet	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: PRESTON ST @ PRINCE OF WALES DR/QUEEN ELIZABET

Traffic Control: Traffic signal

Total Collisions: 26

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Feb-26, Thu,09:52	Clear	Rear end	P.D. only	Dry	East	Turning left	Delivery van	Other motor vehicle	0
					East	Turning left	Truck and trailer	Other motor vehicle	
2015-Apr-13, Mon,15:31	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-May-08, Fri,16:17	Clear	Rear end	Non-fatal injury	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2015-Jun-09, Tue,19:25	Rain	Other	P.D. only	Wet	East	Reversing	Pick-up truck	Other motor vehicle	0
					West	Stopped	Pick-up truck	Other motor vehicle	
2015-Oct-28, Wed,23:24	Rain	Angle	P.D. only	Wet	South	Turning right	Unknown	Other motor vehicle	0
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2015-Nov-12, Thu,13:36	Rain	Rear end	P.D. only	Wet	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2015-Nov-20, Fri,19:20	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Mar-26, Sat,18:13	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Passenger van	Other motor vehicle	
2017-Feb-23, Thu,18:13	Clear	Rear end	P.D. only	Wet	West	Turning right	Unknown	Other motor vehicle	0
					West	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Apr-08, Sat,14:47	Clear	Rear end	P.D. only	Dry	South	Turning right	Unknown	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Aug-03, Thu,11:25	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Bicycle	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: PRESTON ST @ PRINCE OF WALES DR/QUEEN ELIZABET

Traffic Control: Traffic signal

Total Collisions: 26

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2017-Aug-04, Fri,13:07	Clear	Angle	P.D. only	Dry	East	Going ahead	Bicycle	Other motor vehicle	0
					South	Going ahead	Motorcycle	Cyclist	
2017-Sep-12, Tue,09:20	Clear	Turning movement	Non-fatal injury	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Oct-02, Mon,12:45	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Nov-03, Fri,15:22	Clear	Angle	P.D. only	Dry	South	Turning right	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Pick-up truck	Other motor vehicle	
2017-Dec-21, Thu,16:00	Clear	Rear end	P.D. only	Wet	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Feb-01, Thu,11:00	Clear	Sideswipe	P.D. only	Dry	North	Unknown	Bicycle	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Cyclist	
2018-Apr-09, Mon,09:23	Clear	Angle	Non-fatal injury	Dry	South	Going ahead	Bicycle	Other motor vehicle	0
					West	Turning right	Automobile, station wagon	Cyclist	
2018-Dec-21, Fri,21:16	Rain	Turning movement	P.D. only	Wet	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-May-16, Thu,17:55	Clear	Sideswipe	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jul-02, Tue,18:27	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jul-04, Thu,11:15	Clear	Rear end	P.D. only	Dry	South	Turning right	Pick-up truck	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Other motor vehicle	
2019-Aug-26, Mon,07:45	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: PRESTON ST @ PRINCE OF WALES DR/QUEEN ELIZABET

Traffic Control: Traffic signal

Total Collisions: 26

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Oct-17, Thu,18:58	Rain	Turning movement	P.D. only	Wet	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Oct-26, Sat,15:52	Clear	Rear end	P.D. only	Dry	South	Turning right	Automobile, station wagon	Other motor vehicle	0
					South	Turning right	Pick-up truck	Other motor vehicle	
2019-Nov-14, Thu,09:50	Snow	Sideswipe	P.D. only	Loose snow	East	Overtaking	Unknown	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: PRESTON ST @ SIDNEY ST

Traffic Control: Stop sign

Total Collisions: 9

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2015-Jan-16, Fri,16:00	Snow	Angle	P.D. only	Slush	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2015-May-15, Fri,21:59	Clear	Angle	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Feb-18, Thu,13:49	Snow	Sideswipe	P.D. only	Slush	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Truck - closed	Other motor vehicle	
2017-Jan-23, Mon,11:14	Clear	Angle	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Delivery van	Other motor vehicle	
2017-Jun-23, Fri,16:43	Clear	Angle	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Jan-16, Tue,08:10	Snow	Turning movement	P.D. only	Loose snow	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Mar-07, Wed,15:27	Clear	Angle	P.D. only	Wet	East	Unknown	Automobile, station wagon	Other motor vehicle	0
					South	Unknown	Automobile, station wagon	Other motor vehicle	



Transportation Services - Traffic Services

Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: PRESTON ST @ SIDNEY ST

Traffic Control: Stop sign

Total Collisions: 9

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuvre	Vehicle type	First Event	No. Ped
2019-Jun-28, Fri,16:40	Rain	Rear end	P.D. only	Wet	South	Unknown	Unknown	Other motor vehicle	0
					South	Turning right	Pick-up truck	Other motor vehicle	
2019-Sep-09, Mon,13:30	Clear	Angle	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Passenger van	Other motor vehicle	

Intersection	Date	Time	Environment	Road Surface	Control	Collision Location	Light	Classification	Impact Type
ADELINE ST @ PRESTON ST	2/17/2016	9:52	01 - Clear	01 - Dry	02 - Stop sign	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end
ADELINE ST @ PRESTON ST	2/18/2017	12:46	01 - Clear	04 - Slush	02 - Stop sign	03 - At intersection	01 - Daylight	03 - P.D. only	06 - SMV
ADELINE ST @ PRESTON ST	2/12/2017	18:40	03 - Snow	03 - Loose snow	02 - Stop sign	03 - At intersection	07 - Dark	03 - P.D. only	02 - Angle
ADELINE ST @ PRESTON ST	2/8/2017	11:20	01 - Clear	04 - Slush	02 - Stop sign	02 - Intersection related	01 - Daylight	03 - P.D. only	06 - SMV

APPENDIX F

Relevant Excerpts of *TRANS Trip Generation Manual* (WSP, 2020)

to make use of this resource while considering the local land use context and trip characteristics for all travel modes through local and regional data.

Table 2: Person-Trip Conversion Factor

Factor	Application	Apply To	Period	Value
Person-Trip Conversion Factor	Vehicle to person-trip conversion, to normalize the measure of trip rates to account for all modes. Applicable to the ITE trip generation rates, which are mainly reported as vehicle trip rates.	Vehicle trip rates	All	1.28

3 RESIDENTIAL TRIP GENERATION RATES

3.1 Development of Residential Trip Rates

The residential trip generation rates in this manual are reflect the number of **person-trips per household** during the **peak period**. The morning peak period is from 7:00 AM to 9:30 AM, while the afternoon peak period is from 3:30 PM to 6:00 PM.

A geographic review of trip generation rates found that rates varied by dwelling type but not significantly by the geographic sectors and districts used in the 2009 TRANS Trip Generation Study¹. As such, residential trip generation rates in this manual are defined for the following three dwelling types:

- Single-Family Detached Housing
- Multifamily Housing (Low-Rise)
- Multifamily Housing (High-Rise)

Low-rise housing refers to any building that houses multiple families that is two storeys or less (e.g. semi-detached homes, townhouses). High-rise housing refers to any building that houses multiple families that is three or more storeys (e.g. apartments and condo buildings). These dwelling types are from the TRANS Origin-Destination Survey but are organized to be equivalent to the categories of the ITE *Trip Generation Manual* and local generator surveys.

¹ While person trip rates were not found to vary significantly with geographic area, location does have an impact on mode share as discussed in Section 4.2. As a result, vehicular trip rates do vary by geography as reflected in previous versions of the manual. The variation by dwelling type, in part, reflects differences in the number of persons per dwelling.

3.2 Recommended Residential Trip Generation Rates

A blended trip rate was developed from the three data sources through application of a rank-sum weighting process, considering the strengths and weaknesses of each dataset for the dwelling type in question. The recommended blended **residential person-trip rates** are presented in **Table 3**. All rates represent person-trips per dwelling unit and are to be applied to the **AM or PM peak period**.

Table 3: Recommended Residential Person-trip Rates

ITE Land Use Code	Dwelling Unit Type	Period	Person-Trip Rate
210	Single-detached	AM	2.05
		PM	2.48
220	Multi-Unit (Low-Rise)	AM	1.35
		PM	1.58
221 & 222	Multi-Unit (High-Rise)	AM	0.80
		PM	0.90

3.3 Adjustment Factors – Peak Period to Peak Hour

The various trip generation data sources require some adjustment to standardize the data for developing robust blended trip rates. The peak period conversion factor in **Table 4** may be used where applicable to develop trip generation rate estimates in the desired format.

Table 4: Adjustment Factors for Residential Trip Generation Rates

Factor	Application	Apply To	Period	Value
Peak Period Conversion Factor	Peak period to peak hour conversion. Because the 2020 TRANS Trip Generation Study reports trip generation rates by peak period, factors must be applied if the practitioner requires peak hour rates. In practice, the conversion to peak hour trip rates should occur after the application of modal shares.	Person-trip rates per peak period	AM	0.50
			PM	0.44
		Vehicle trip rates per peak period	AM	0.48
			PM	0.44
		Transit trip rates per peak period	AM	0.55
			PM	0.47
		Cycling trip rates per peak period	AM	0.58
			PM	0.48
		Walking trip rates per peak period	AM	0.58
			PM	0.52

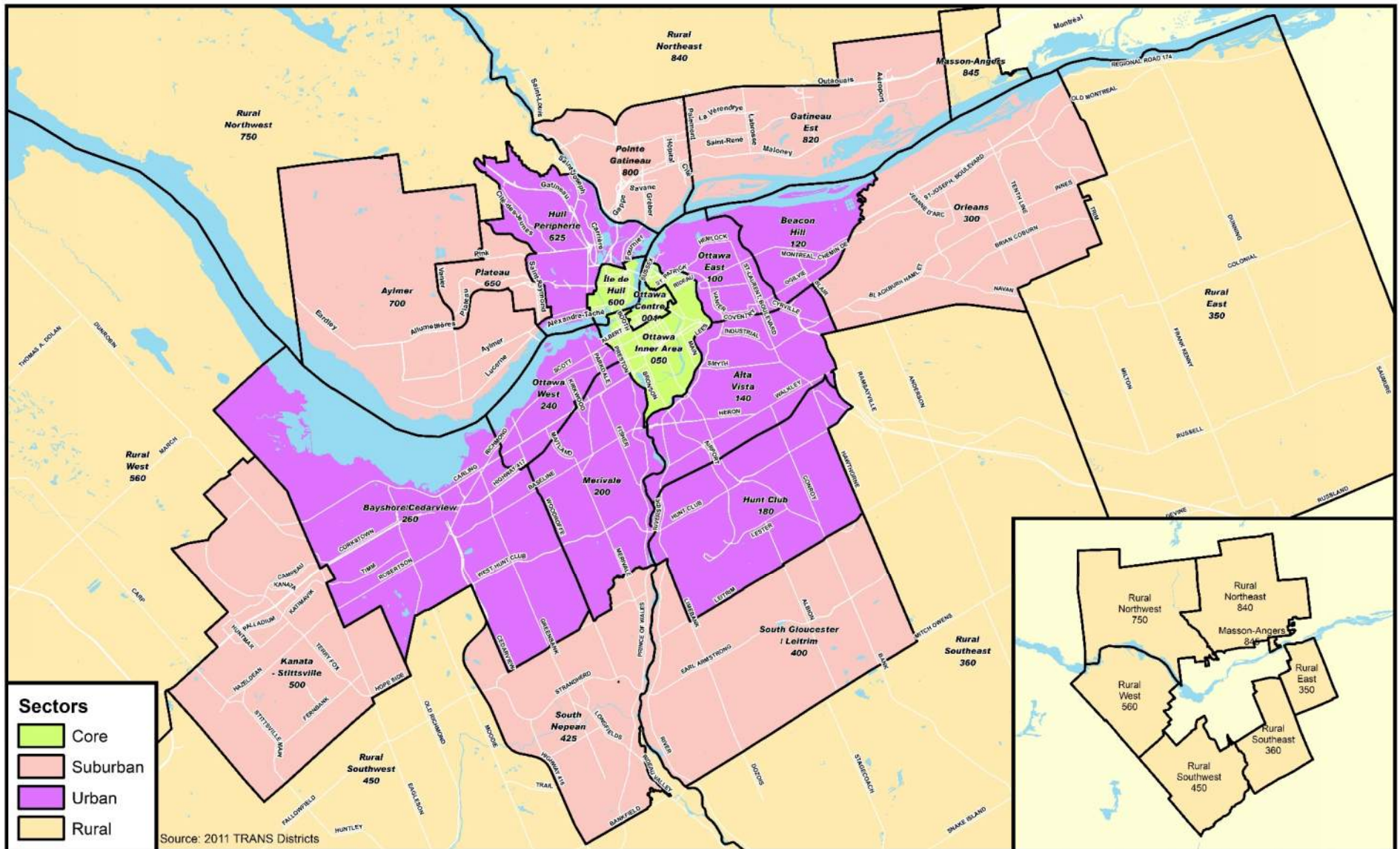


Figure 1: National Capital Region by Sector

Table 8: Residential Mode Share for High-Rise Multifamily Housing

District	Period	Mode				
		Auto Driver	Auto Pass.	Transit	Cycling	Walking
Ottawa Centre	AM	18%	2%	26%	1%	52%
	PM	17%	9%	21%	1%	52%
Ottawa Inner Area	AM	26%	6%	28%	5%	34%
	PM	25%	8%	21%	6%	39%
Île de Hull	AM	27%	3%	37%	12%	21%
	PM	26%	8%	27%	11%	28%
Ottawa East	AM	39%	7%	38%	2%	13%
	PM	40%	14%	28%	3%	15%
Beacon Hill	AM	48%	9%	30%	3%	10%
	PM	52%	16%	28%	0%	4%
Alta Vista	AM	38%	12%	42%	2%	7%
	PM	45%	16%	28%	2%	9%
Hunt Club	AM	39%	6%	44%	1%	9%
	PM	44%	11%	35%	2%	9%
Merivale	AM	41%	6%	42%	2%	8%
	PM	41%	11%	33%	2%	13%
Ottawa West	AM	28%	11%	41%	3%	16%
	PM	33%	11%	26%	7%	23%
Bayshore/Cedarview	AM	40%	12%	38%	2%	8%
	PM	40%	15%	33%	1%	11%
Hull Périphérie	AM	48%	11%	30%	1%	10%
	PM	47%	15%	23%	3%	13%
Orleans	AM	54%	7%	29%	0%	10%
	PM	61%	13%	21%	0%	6%
South Gloucester / Leitrim	AM	50%	15%	25%	1%	9%
	PM	53%	17%	21%	1%	9%
South Nepean	AM	58%	6%	30%	2%	4%
	PM	54%	15%	25%	0%	7%
Kanata - Stittsville	AM	43%	26%	28%	0%	4%
	PM	55%	19%	21%	0%	5%
Plateau	AM	53%	9%	35%	3%	1%
	PM	65%	7%	25%	2%	1%
Aylmer	AM	45%	17%	25%	0%	13%
	PM	31%	21%	23%	4%	20%
Pointe Gatineau	AM	44%	15%	24%	3%	14%
	PM	52%	15%	20%	2%	11%
Gatineau Est	AM	53%	10%	25%	0%	12%
	PM	61%	10%	25%	0%	4%
Masson-Angers	AM	63%	15%	19%	0%	3%
	PM	64%	18%	16%	0%	1%
Other Rural Districts	AM	63%	15%	19%	0%	3%
	PM	64%	18%	16%	0%	1%

5 RESIDENTIAL DIRECTIONAL SPLITS

After calculating the total person trips generated by the development and applying the appropriate modal shares, directional factors can be applied to estimate the number of inbound and outbound trips by vehicle. The vehicle trip directional splits were developed for both the AM and PM peak periods². The vehicle trip directional splits, as shown in **Table 9**, have been developed for the NCR based on a review of the local trip generator surveys as well as the latest published data in the *ITE Trip Generation Manual* (10th Edition).

Table 9: Recommended Vehicle Trip Directional Splits (Peak Period)

ITE Land Use Code	Dwelling Unit Type	Period	Inbound	Outbound
210	Single-detached	AM	30%	70%
		PM	62%	38%
220	Multi-Unit (Low-Rise)	AM	30%	70%
		PM	56%	44%
221 & 222	Multi-Unit (High-Rise)	AM	31%	69%
		PM	58%	42%

6 NON-RESIDENTIAL MODE SHARE

Mode shares were developed for three types of non-residential development: schools (elementary and high school); employment generators; and commercial (retail) generators. These mode shares were developed through data provided by the Ville de Gatineau from local school surveys as well as the TRANS Origin-Destination Survey. The non-residential mode shares presented below are limited and do not capture all development types. For data on the travel characteristics associated with colleges and universities, transportation terminals, and sports and entertainment venues in the National Capital Region, practitioners should refer to the various reports for the TRANS *Special Generators Survey* (2013), which are posted on the TRANS website. For other development types, practitioners may need to carry out their own local generator data collection where necessary.

² A directional split for active transportation was calculated based on the local generator surveys for low-rise and mid-rise land uses. The splits are mostly in-line with the vehicle directional splits, which could be used as a rough assumption for areas with lower vehicle mode share.

District	Mode				
	Auto Driver	Auto Pass.	Transit	Cycling	Walking
Masson-Angers	89%	3%	2%	1%	6%
Rural Districts	85%	5%	9%	1%	1%

6.3 Commercial Generators

All trips classified as “shopping, household maintenance” from the 2011 TRANS Origin-Destination Survey were analyzed to define the mode share for trips to commercial establishments. **Table 13** provides the mode share by district during the AM and PM peak periods for commercial generator trips. These mode shares do not include restaurant or recreation trips. Although the mode shares were calculated for trips to the generator, for most commercial developments, a similar mode share would apply for trips from the generator. In general, the sample size for shopping trips during the AM peak period tends to be low, and the results should be used with caution, particularly for districts with lower retail activity. Where the sample size for a district was less than the pre-defined cut-off, the mode share for the wider area has been applied.

Table 13: Commercial Generator Mode Share by District

District	Period	Mode				
		Auto Driver	Auto Pass.	Transit	Cycling	Walking
Ottawa Centre	AM	28%	3%	48%	1%	20%
	PM	19%	12%	30%	2%	37%
Ottawa Inner Area	AM	39%	2%	16%	3%	40%
	PM	22%	4%	12%	4%	58%
Île de Hull	AM	34%	2%	30%	2%	32%
	PM	22%	7%	18%	3%	50%
Ottawa East	AM	57%	10%	15%	1%	17%
	PM	55%	18%	11%	1%	15%
Beacon Hill	AM	67%	12%	8%	0%	14%
	PM	59%	18%	7%	1%	13%
Alta Vista	AM	64%	9%	12%	1%	14%
	PM	60%	20%	9%	0%	11%
Hunt Club	AM	70%	6%	7%	0%	17%
	PM	65%	19%	8%	1%	7%
Merivale	AM	71%	19%	1%	0%	9%
	PM	61%	16%	8%	1%	14%
Ottawa West	AM	55%	11%	11%	0%	23%
	PM	50%	16%	11%	5%	18%
Bayshore/Cedarview	AM	64%	15%	4%	0%	17%
	PM	62%	20%	6%	1%	11%
Hull Périphérie	AM	77%	8%	5%	0%	10%
	PM	60%	12%	9%	5%	14%

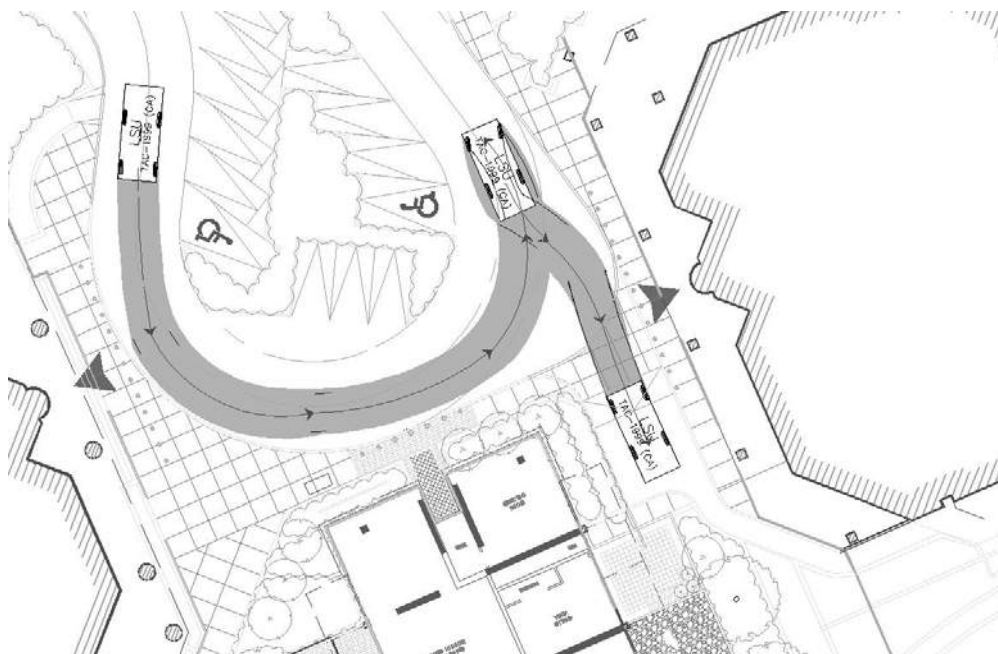
APPENDIX G

Other Area Developments

OTHER AREA DEVELOPMENTS

17 Aberdeen Street

Figure 6-4: Loading Access (Tower 1)



7 Existing Traffic Volumes

Existing (2012) traffic volumes have been obtained from the City of Ottawa. Two-way peak hour traffic volumes on Aberdeen Street are in the order of 130 to 230 vehicles per hour during the weekday morning and afternoon peak hours, respectively. Two-way peak hour traffic volumes on Rochester Street are in the order of 500 to 650 vehicles per hour during the weekday morning and afternoon peak hours, respectively. Both of these roads are well under capacity for their classification of road and number of lanes.

Based on analysis of these volumes, the intersection is operating at a Level of Service 'A' with average delays of 20 seconds on the eastbound approach during the weekday morning peak hour. During the weekday afternoon peak hour, the intersection operates at approximately Level of Service 'A' with average delays of 22 seconds on the eastbound approach. These results are consistent with field observations.

8 Trip Generation

Based on data presented in the Institute of Transportation Engineer's (ITE) Trip Generation Manual, 9th Edition, site generated traffic associated with the proposed development is expected to increase by a maximum of 94 vehicles per hour (80% exiting, 20% entering) and 119 vehicles per hour (35% exiting, 65% entering) during the weekday morning and afternoon peak hours, respectively.

The vehicular trip generation rates referenced in the Trip Generation Manual are typically for highly suburbanized locations with little to no access to public transit. The ITE rates generally capture roughly 95% of all trips to/from a development. Since auto occupancy is assumed to be in the order of 1.2 people per vehicle, the resulting Person Trip conversion factor is 1.26.

Based on local information available in the 2011 NCR Household Origin-Destination Survey, conducted by the TRANS Committee for the City of Ottawa, the breakdown of trips by transportation mode in Ottawa Inner Area is approximated as follows:

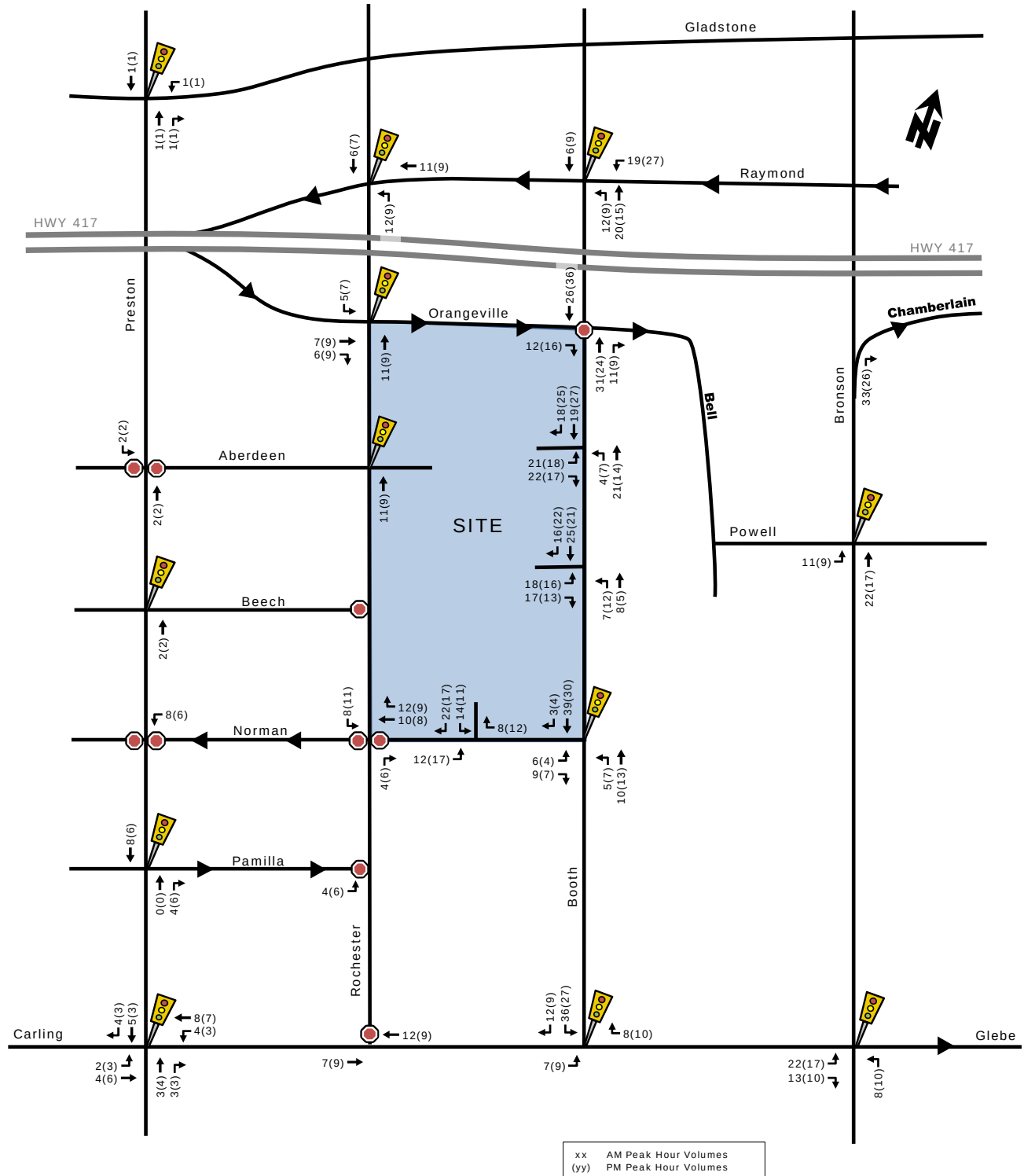
- Auto Driver: 33%
- Auto Passenger: 10%
- Transit: 19%
- Non-Auto: 39%

The local adjustment factors above therefore indicate that the development will generate approximately 39 vehicles per hour and 49 vehicles per hour during the weekday morning and afternoon peak hours, respectively. It is important to note that these are two-way trips (in and out) and will be distributed amongst each of the four active parking garage ramps. The effect on the adjacent road network is therefore expected to be insignificant.

OTHER AREA DEVELOPMENTS

552 Booth Street

Figure 13: 'New' and 'Pass-by' 2030 Site-Generated Vehicle Traffic



3.2. BACKGROUND NETWORK TRAVEL DEMANDS

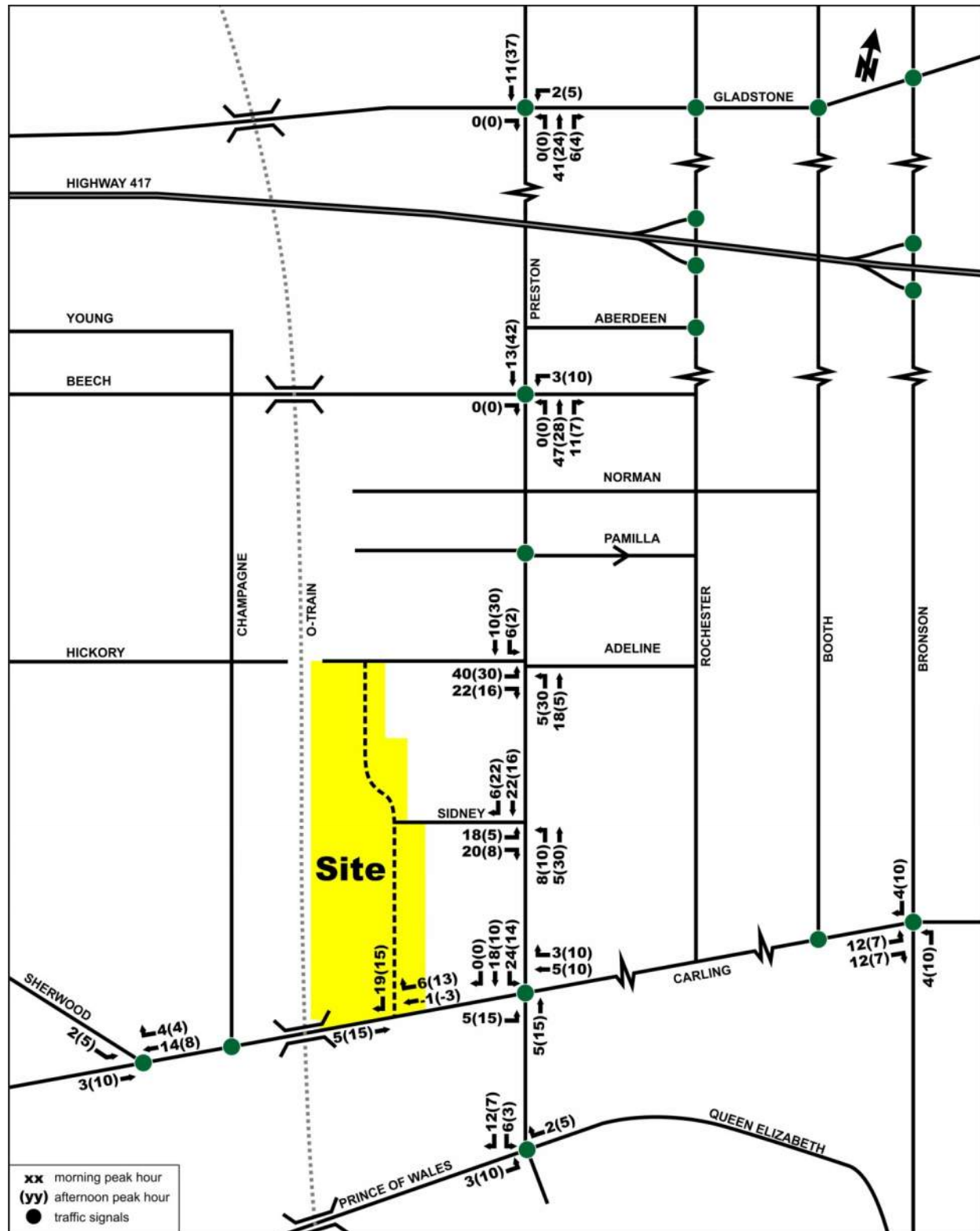
3.2.1. TRANSPORTATION NETWORK PLANS

Refer to Section 2.1.2 Planned Conditions.

OTHER AREA DEVELOPMENTS

845 Carling Avenue

Figure 6: 'New' and 'Pass-by' Site-Generated Traffic Volumes



OTHER AREA DEVELOPMENTS

90 Champagne Avenue

patterns associated with the typical commute (i.e. departing the study area during the AM peak hour, and entering the study area during the PM peak hour). The distribution of site-generated trips can be described as follows:

- 15% to/from the north via Preston Street;
- 20% to/from the south via Preston Street;
- 30% to/from the east via Carling Avenue;
- 5% to/from the east via Beech Street;
- 20% to/from the west via Carling Avenue;
- 5% to/from the west via Sherwood Avenue; and
- 5% to/from the west via Beech Street.

Trips generated by the subject site are shown in **Figure 5**.

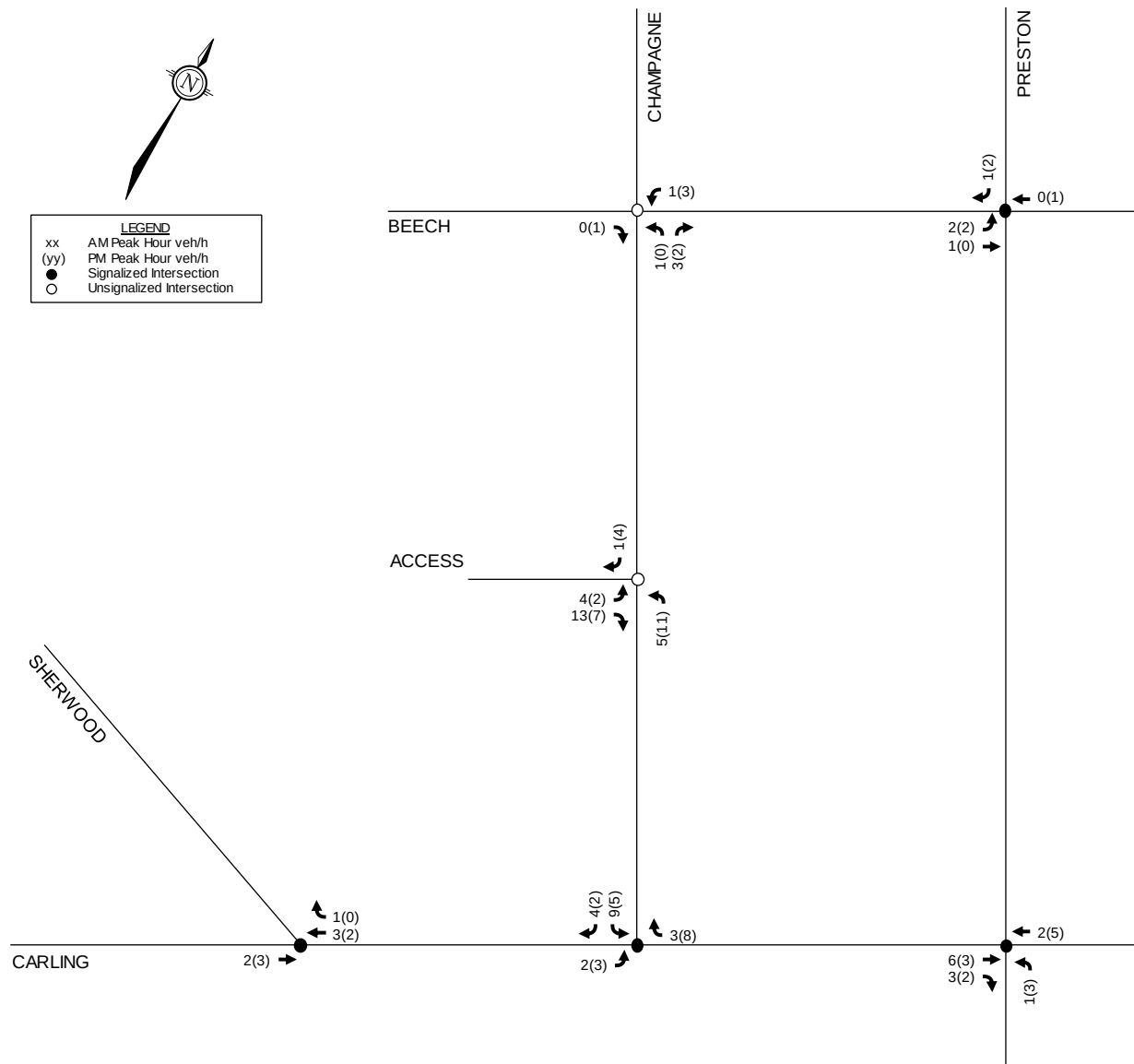
5.2 Background Traffic

5.2.1 General Background Growth Rate

A review of the City of Ottawa's Strategic Long-Range Model was conducted. Comparing snapshots of the 2011 and 2031 AM peak hour traffic volumes suggests Carling Avenue between Sherwood Drive and Preston Street will not grow significantly, while Preston Street between Carling Avenue and Beech Street will grow at a rate of 1% per annum. A review of City of Ottawa traffic count data at the Carling Avenue/Preston Street intersection from June 2015 and June 2017 suggests the Annual Average Daily Traffic (AADT) at this intersection has not grown significantly over the two years.

Based on the foregoing, no growth rate has been applied to the existing traffic volumes. As described in Section 5.2.2 below, background growth along the study area roadways will be captured by adding traffic generated by other area developments to the background traffic volumes. This approach is consistent with other traffic studies in the area.

Figure 6: Site Generated Traffic Volumes



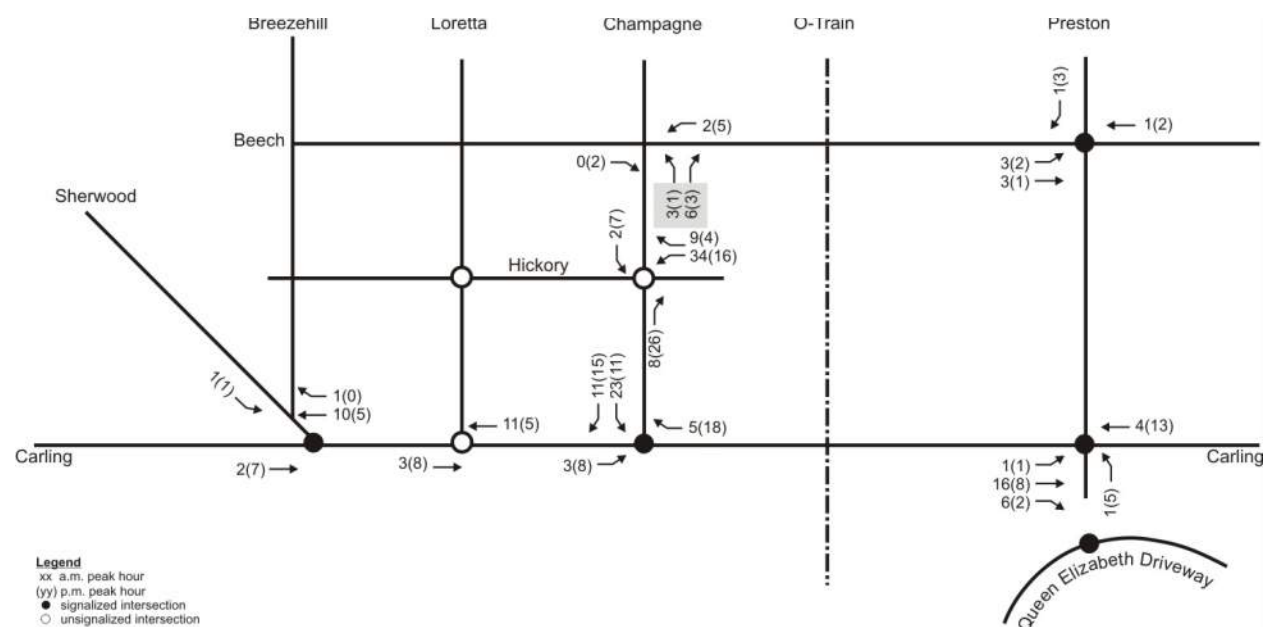
OTHER AREA DEVELOPMENTS

101-105 Champagne Avenue

**Table 10: Site-Generated Traffic Summary of Study Area Development Projects
Two-way Table (vph)**

Development	Projected Net Traffic Generated vph			
	AM Peak	%	PM Peak	%
320/330 Loretta Avenue (Domicile)	+70	9%	+85	11%
855 Champagne Avenue (Arnon)	+502	62%	+438	55%
125 Hickory Street (Mastercraft Starwood)	+147	18%	+174	22%
100 Champagne Avenue (Domicile)	+40	5%	+50	6%
101 Champagne Avenue (Ashcroft)	+53	6%	+53	6%
TOTALS	+812	100 %	+800	100 %

Figure 10: Projected Peak Hour Ashcroft Site-Generated Traffic



6.4 Analysis of Projected Future Conditions

The combined site-generated traffic for the five aforementioned projects, superimposed on current volumes (Figure 5), is depicted in Figure 11.

6.4.1 Traffic Signal Warrants

The existing traffic signals at the Carling/Champagne intersection are not warranted based on recent City of Ottawa traffic counts (August: 2009: Appendix A) and were not judged to be warranted based on the projected impact of the Arnon development alone. However, as reported in the CTS for the Mastercraft Starwood proposed development at 125 Hickory Street, the signals were judged to be warranted based on the combined impact of the surrounding developments.

OTHER AREA DEVELOPMENTS

93-105 Norman Street

However, ITE rates were adjusted based on vehicle occupancy and modal splits to develop the Modified Person Trips summarized in Table 5 of the original TB to better reflect the type of area where the subject site is located. The 1.15 vehicle occupancy value and the 10% transit/non-motorized modal share split used to calculate the 1.3 factor are based on recent available census data for the United States.

Given that the Site Plan has been revised, the total person trips have been re-calculated based on the revised number of dwelling units. The following Table 3 includes the total person trips calculated using the method outlined in the original TB and the total person trips calculated using the City's suggested method (outlined above) for the revised Site Plan.

Table 3: Modified Person Trip Generation

Land Use	Data Source	Units	AM Peak (persons)			PM Peak (persons)		
			In	Out	Total	In	Out	Total
High-Rise Condominium ⁽¹⁾	ITE 232	117 Units	15	65	80	43	27	70
High-Rise Condominium ⁽²⁾	-	117 Units	18	80	98	43	27	70
Original TB Modified Person Trip Generation								
High-Rise Condominium	ITE 232	159 Units	18	80	98	56	35	91
Note: (1) 1.3 factor to account for typical North American auto occupancy values of approximately 1.15 and combined transit and non-motorized modal shares of less than 10%. (2) The suggested rates used were as follows: 1.2 person/unit factor and 0.7 (AM peak) and 0.5 (PM peak) person trip generation rate.								

As shown in Table 3, the suggested methodology produces the same or similar results as the methodology outlined in the original TB. As both of the resultant person trip totals are less than or equal to the person trips total from the original TB, the projected Level of Service at study area intersections will be the same or better than the projected Levels of Service summarized in the original report. As such, no additional analysis is required as the original TB did not identify any required changes to the off-site roadway geometry or traffic control.

With regard to peak hour operations for this development, the analysis was performed for the hour during which the adjacent road network experiences the heaviest morning and afternoon traffic volumes. For a residential development it is appropriate to assume that this peak hour analysis will constitute the "worst case" scenario. Should the majority of person traffic from the proposed development travel outside of this peak hour, in terms of traffic operations, the impact would be less outside the peak hour, given there would be fewer vehicles overall on study area roads.

OTHER AREA DEVELOPMENTS

500 Preston Street

Figure 5: “Net” Increase in Site-Generated Traffic

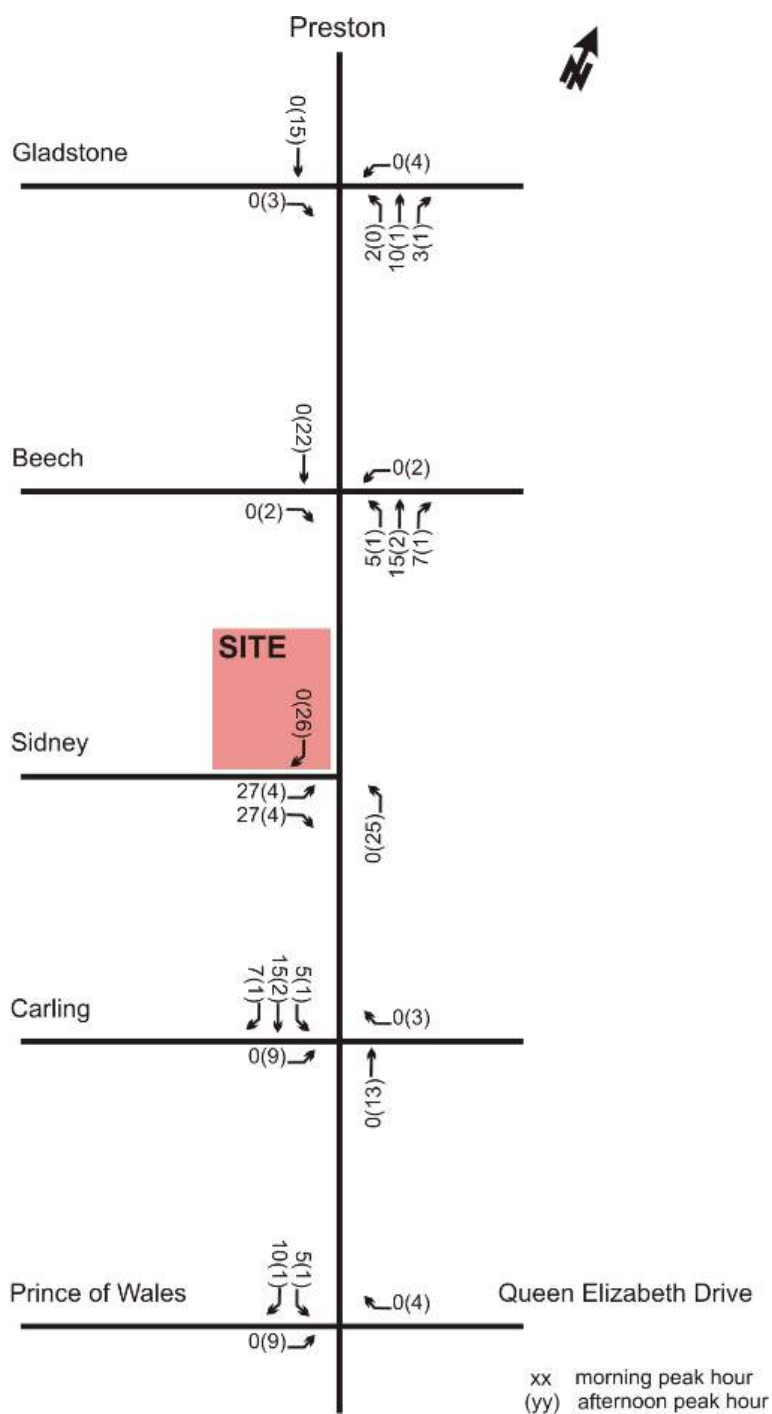


TABLE 5: High Rise Condo Trip Generation (Net Increase)

Travel Mode	Mode Share	AM Peak (Persons / hr)			PM Peak (Persons / hr)		
		In	Out	Total	In	Out	Total
Auto Driver	30%	3	12	15	6	4	10
Auto Passenger	10%	1	4	5	2	2	4
Transit	40%	3	16	19	8	5	13
Non-motorized	20%	1	7	8	3	2	5
Total Person Trips	100%	8	39	47	19	13	32
Total 'New' Auto Trips		3	12	15	6	4	10

TABLE 6: Commercial Trip Generation (Net Increase)

Travel Mode	Mode Share	AM Peak (Persons / hr)			PM Peak (Persons / hr)		
		In	Out	Total	In	Out	Total
Auto Driver	30%	8	3	11	5	11	16
Auto Passenger	10%	2	1	3	2	4	6
Transit	40%	10	2	12	5	14	19
Non-motorized	20%	5	1	6	2	7	9
Total Person Trips	100%	25	7	32	14	36	50
Total 'New' Auto Trips		8	3	11	5	11	16

TABLE 7: Total Additional Site Vehicle Trip Generation (condo + commercial / office)

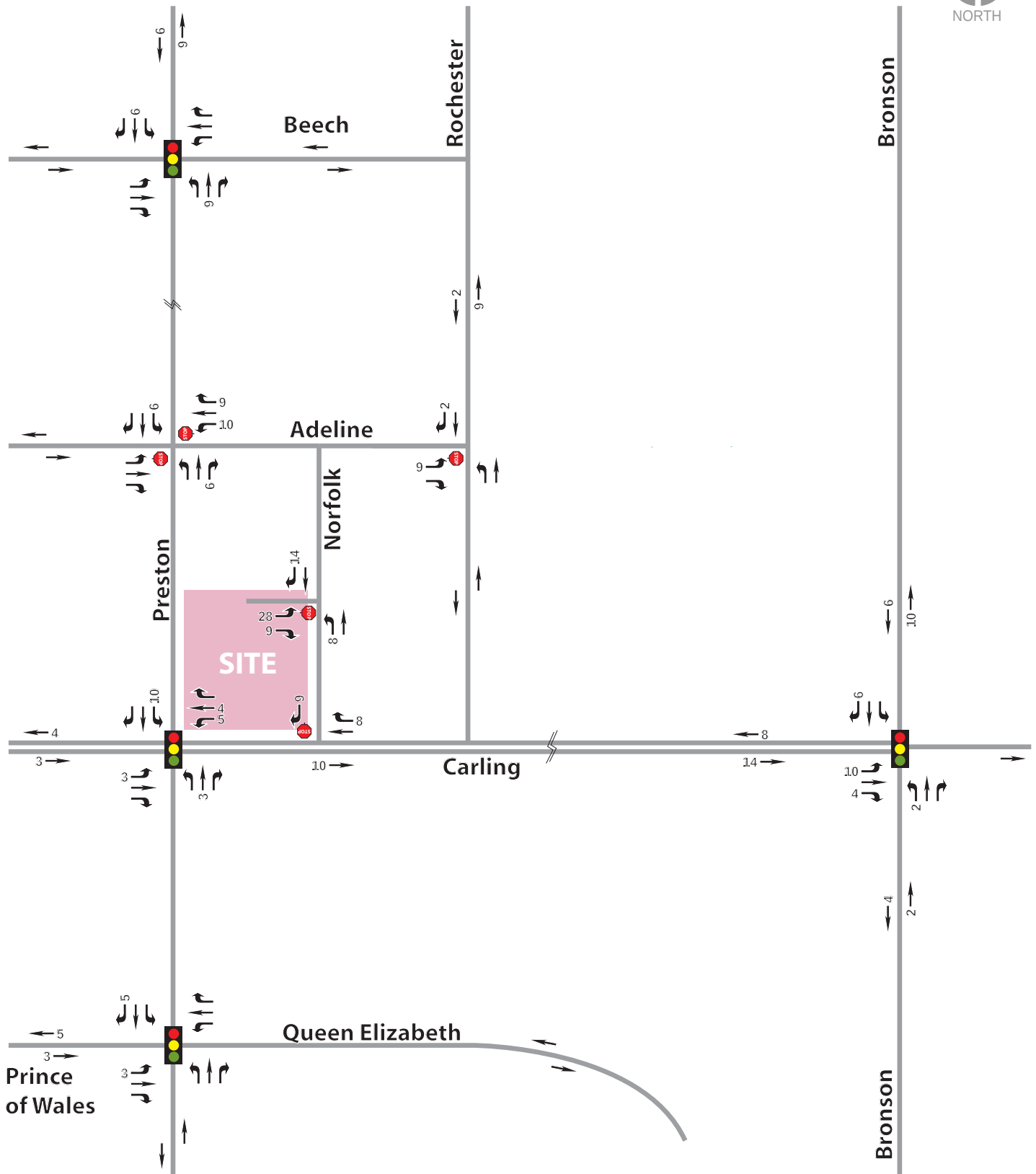
Travel Mode	AM Peak (veh / h)			PM Peak (veh / h)		
	In	Out	Total	In	Out	Total
High Rise Condo Trip Generation	3	12	15	6	4	10
Commercial/Office Trip Generation	8	3	11	5	11	16
Total 'New' Auto Trips	11	15	26	11	15	26

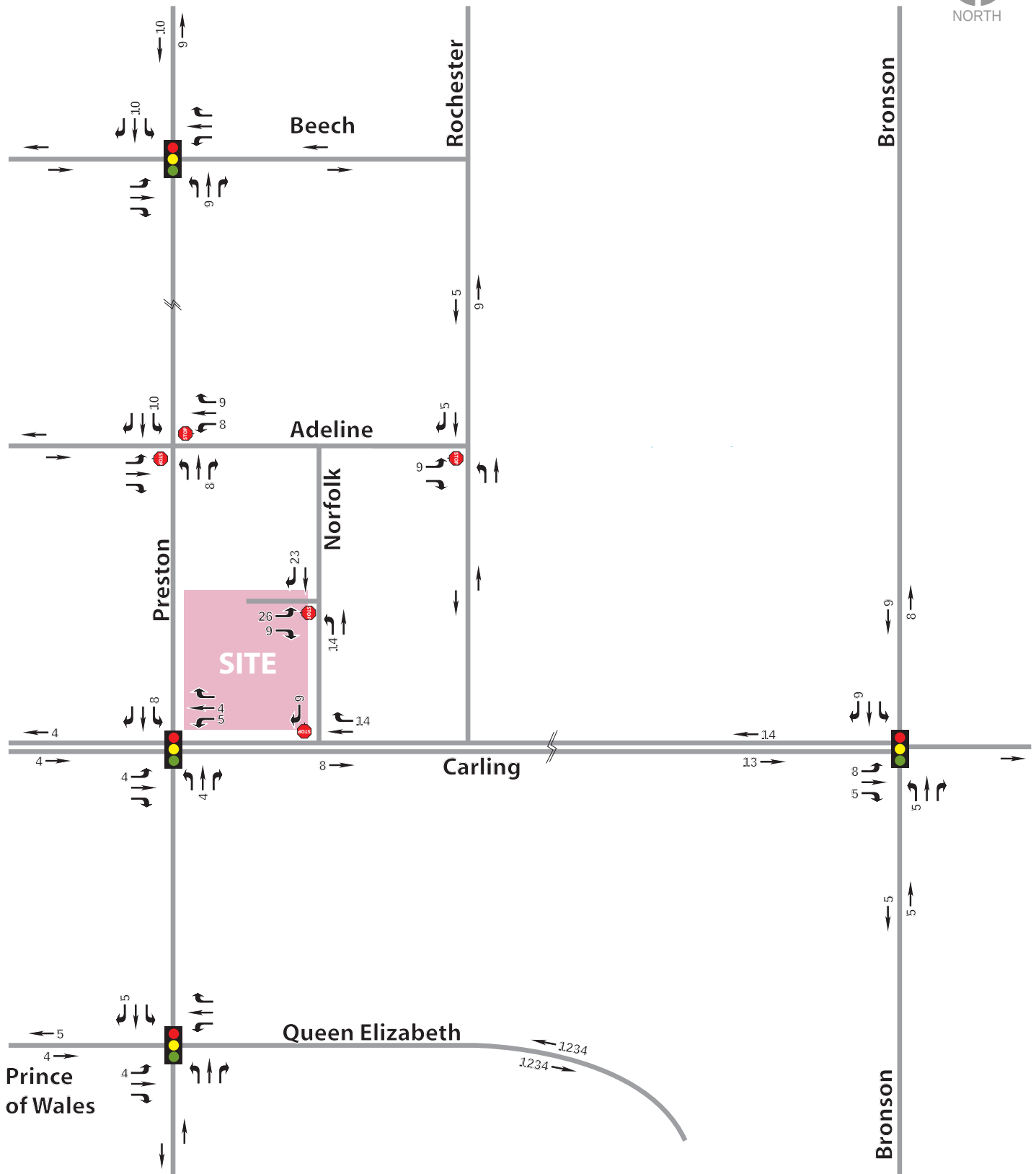
As summarized in Table 7, the net traffic increase associated with the proposed Site Plan changes is estimated at 26 vph during both peak periods, or less than one new vehicle every two minutes during peak hours.

As the initial proposal addressed in the June 2011 CTS had a "net" new traffic generation of 50 vph to 65 vph two-way total, as the changes per the December 2012 Addendum #1 added 8 vph and as the current Site Plan changes add 26 vph, the resultant total peak hour generation of the current proposal is in the range of 85 vph to 100 vph, with approximately

OTHER AREA DEVELOPMENTS

505 Preston Street





Claridge Homes Inc. – 2013-05-22

TABLE 2 - TRIP GENERATION SUMMARY – BY MODE

Travel Mode	Modal Share	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
Total Person Trips	100%	56	101	157	101	91	192
Auto Driver	31%	17	31	49	31	28	60
Auto Passenger	7%	4	7	11	7	6	13
Public Transit	35%	20	36	55	36	32	67
Non-Motorized	27%	15	27	42	27	25	52
New Auto Trips		17	31	49	31	28	60
<i>New Auto Trips (2012 CTS)</i>		<i>22</i>	<i>37</i>	<i>59</i>	<i>37</i>	<i>35</i>	<i>72</i>

Background Traffic

As analysed by Delcan in transportation studies for nearby development proposals including 101 Champagne, 505 Preston and 514-532 Rochester, traffic growth has been shown to be on a decline within the study area during the period of 2001 to 2010. It is expected that this trend will continue and that there will be a stagnant rate of background traffic growth in the study area with the exception of traffic generation from known development applications.

Since the submission of the CTS for 505 Preston Street, there have been a number of additional development applications within the study area. The following table lists all of the current applications (both in the initial planning and approval stages), along with their corresponding trip generation.

TABLE 3 - PROPOSED TRAFFIC GENERATION FROM PROPOSED DEVELOPMENTS WITHIN THE STUDY AREA

Development	Projected Net Traffic Generated		Status
	AM Peak Hour	PM Peak Hour	
855 Carling Avenue (Arnon) *UPDATED	+176	+208	Revision Likely
125 Hickory Street (Mastercraft Starwood)	+150	+175	Under Construction
100 Champagne Avenue (Domicile)	+40	+50	Under Construction
500 Preston Street (Mastercraft Starwood)	+65	+72	Approved
101 Champagne (Ashcroft)	+53	+53	Approved
93-105 Norman Street (Taggart) *NEW	+52	+56	Application On Hold
845 Carling (Richcraft) *NEW	+151	+175	Submitted for ZBA
320/330 Loretta Avenue (Domicile)	+83	+98	Recently Built-Out
350 Loretta Avenue (Domicile)	+90	+105	Recently Built-Out
514-532 Rochester Street (Domicile) *NEW	+54	+60	Submitted for SPA
774 Bronson Avenue (Samcon) *NEW	+37	+48	Submitted for SPA
265 Carling (Taggart) *NEW	+47	+43	Approved
TOTAL	998	1,143	

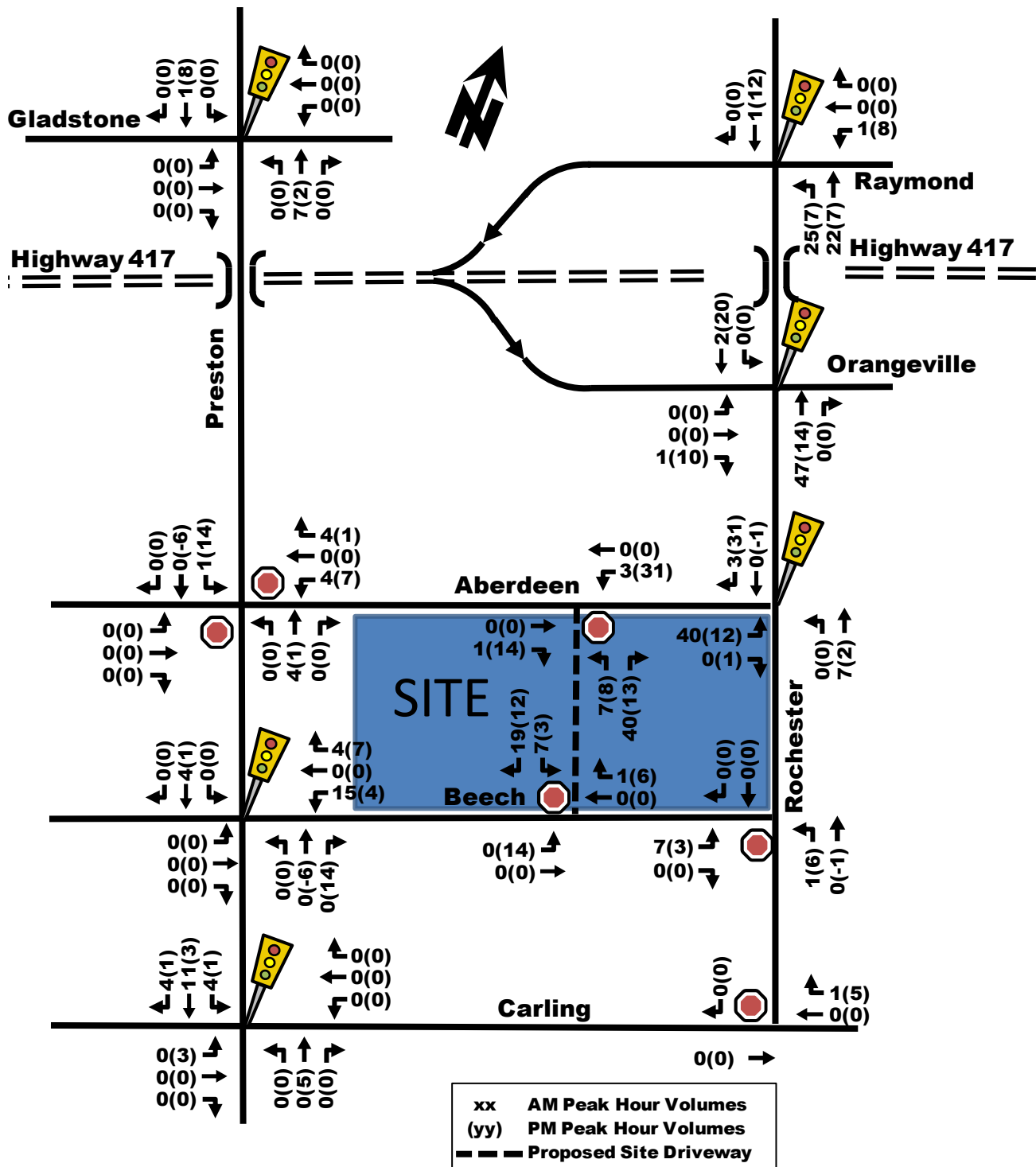
* Note: Site traffic generation values have been confirmed by IBI Group.

The update to trip generation values for each of the developments listed above suggests a net increase of 31 trips in the weekday morning peak hour and 34 trips in the weekday afternoon

OTHER AREA DEVELOPMENTS

450 Rochester Street

Figure 11: Site-Generated Traffic at Full Buildout (Phase 1 & 2)

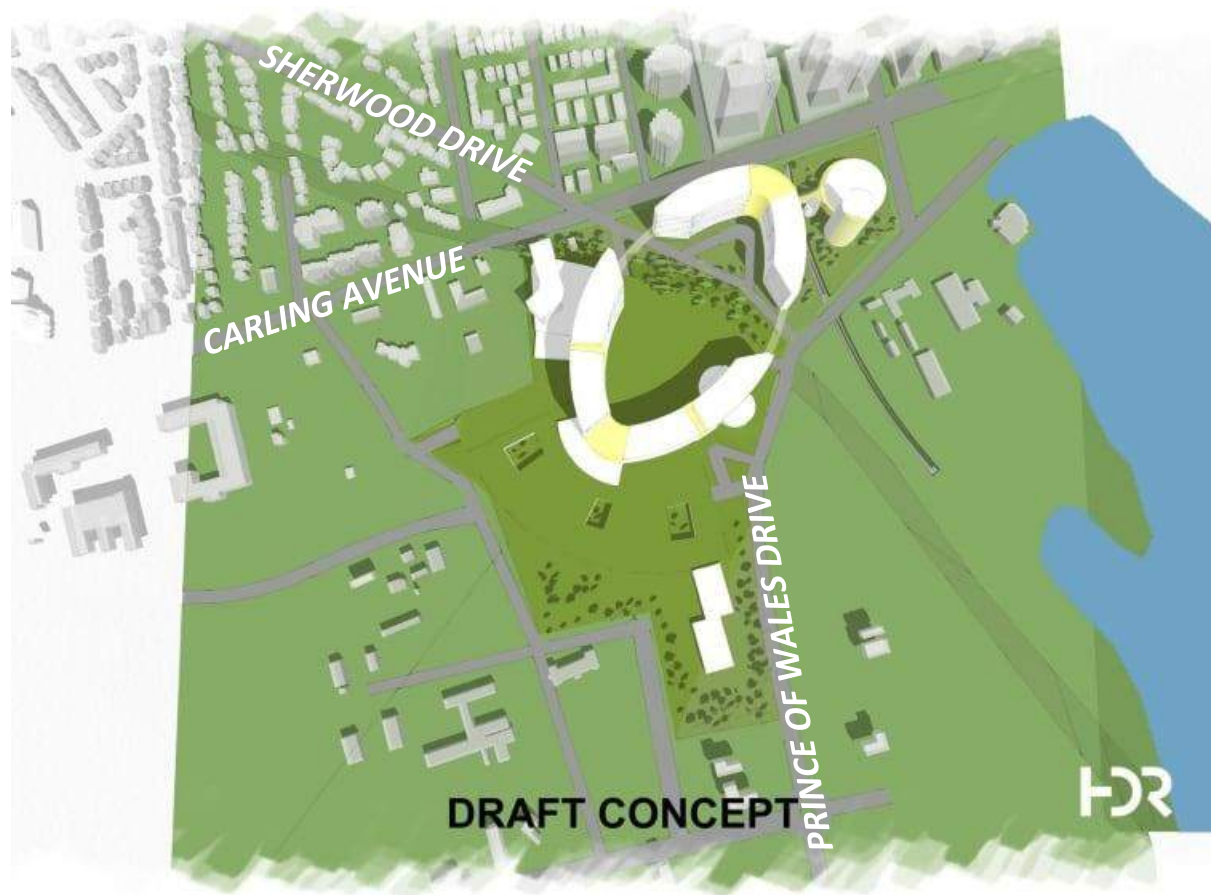


Note: values in negative reflect changes in routes based on pass-by trips or net change between trips generated and reduction in public parking lot.

OTHER AREA DEVELOPMENTS

Ottawa Civic Hospital Expansion (930 Carling Avenue)





APPENDIX H

Strategic Long-Range Model and Intersection Growth Rate Figures

TRANS Regional Model

Version 2.15 - Assigned June 16, 2020

AM Peak Hour Total Traffic Volume

Carling Preston Area

2011 Model - Basecase

N/A

User Initials: TIMW

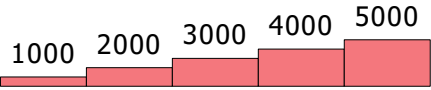
Plot Prepared: Feb 2, 2020

EMME Scenario: 21711

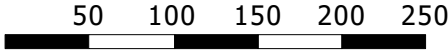


Legend

AM Peak Hour Total Traffic Volume



Distance (m)



The TRANS model is continuously refined & maintained, and all information is provided in good faith. However, model outputs are provided "as is", and no warranty or guarantee is provided as to the accuracy, reliability or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be over- or under-estimating the travel demand.

TRANS Regional Model

Version 2.15 - Assigned June 16, 2020

AM Peak Hour Total Traffic Volume

Carling Preston Area

2031 Model - Basecase

N/A

User Initials: TIMW

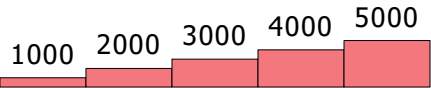
Plot Prepared: Feb 2, 2020

EMME Scenario: 21711

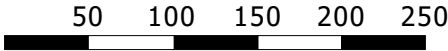


Legend

AM Peak Hour Total Traffic Volume



Distance (m)



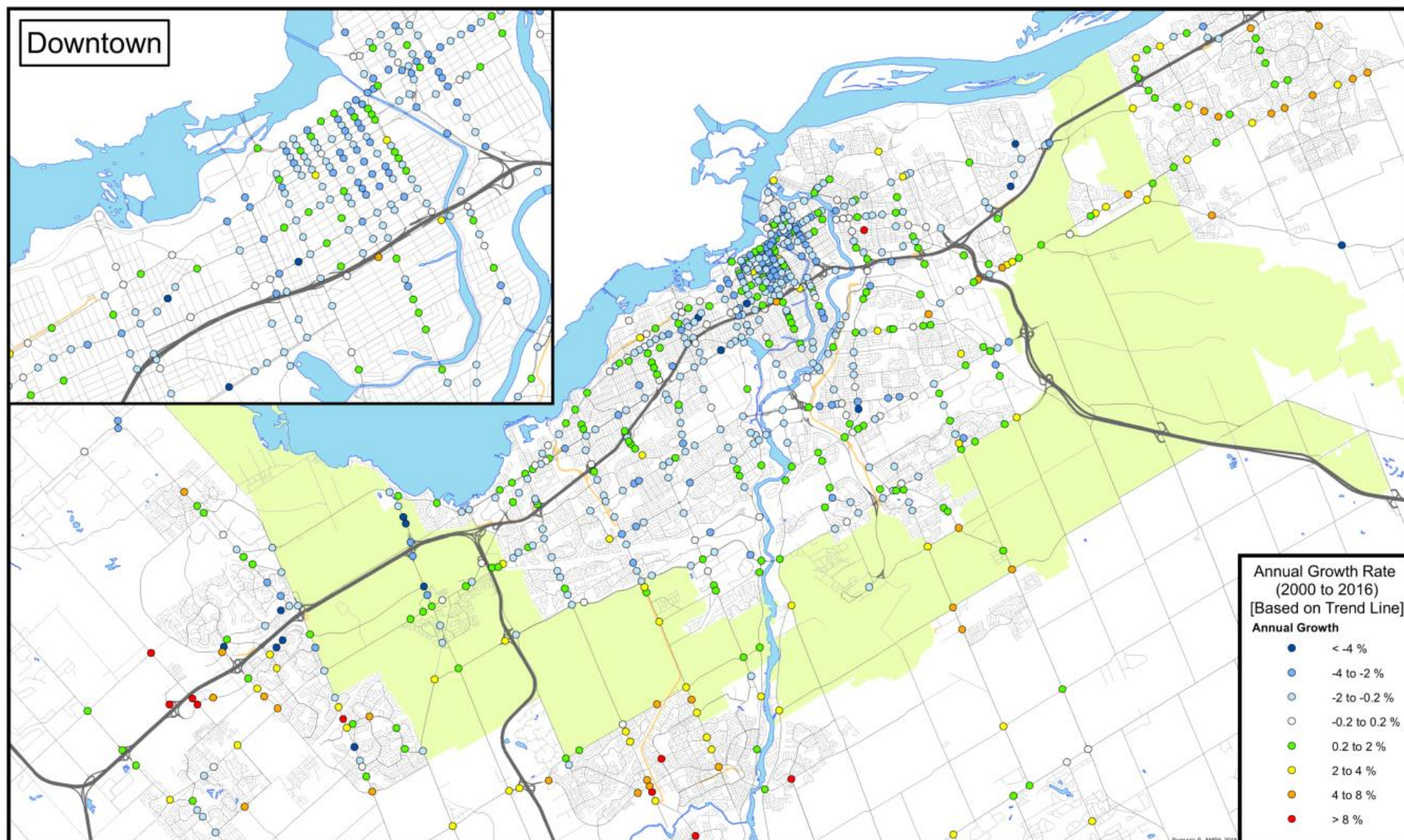
The TRANS model is continuously refined & maintained, and all information is provided in good faith. However, model outputs are provided "as is", and no warranty or guarantee is provided as to the accuracy, reliability or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be over- or under-estimating the travel demand.

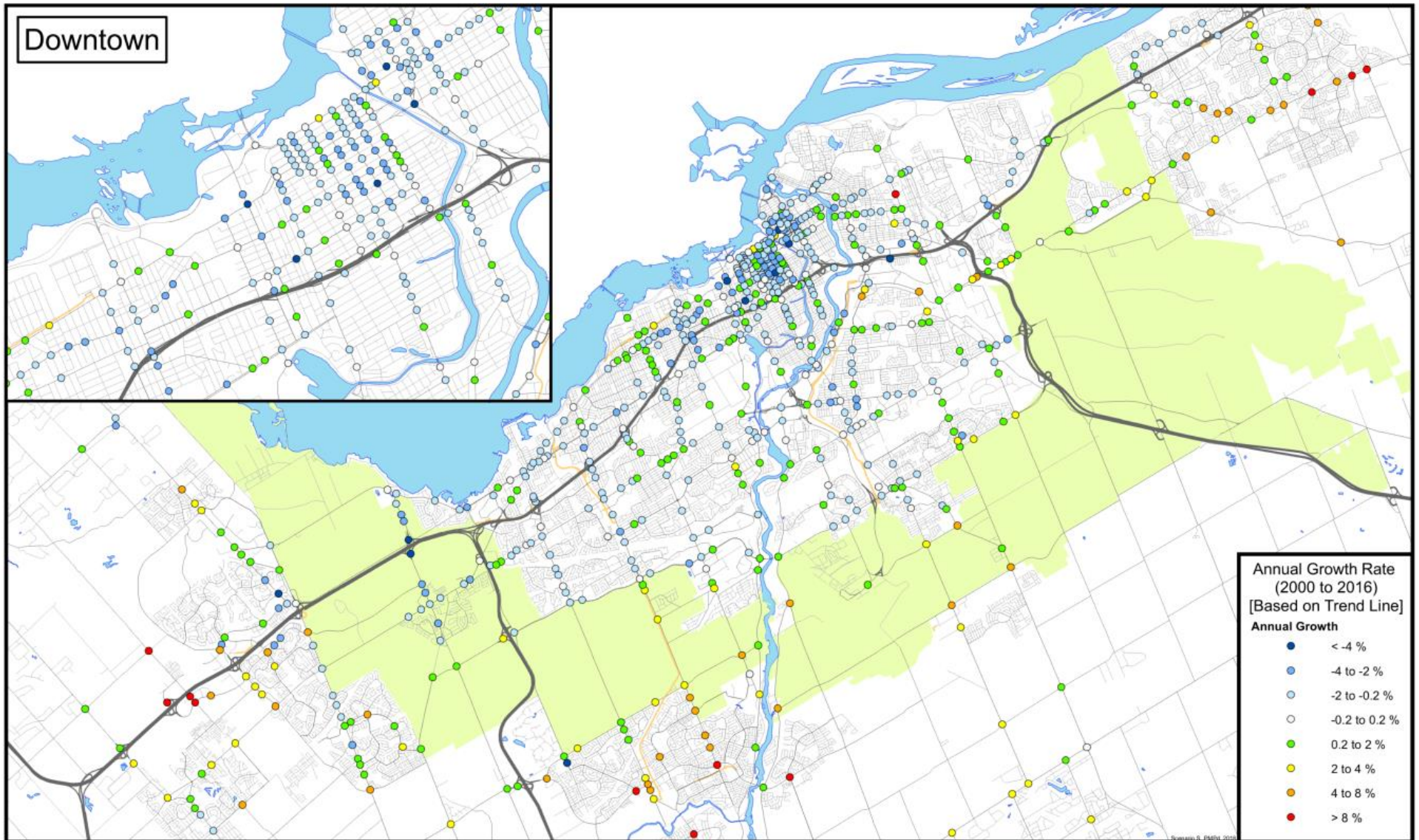
INTERSECTION TRAFFIC GROWTH RATE, AM PEAK PERIOD

Total Vehicular Volume Entering the Intersection, 2000 to 2016



INTERSECTION TRAFFIC GROWTH RATE, PM PEAK PERIOD

Total Vehicular Volume Entering the Intersection, 2000 to 2016



APPENDIX I

Signal Timing Plans

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

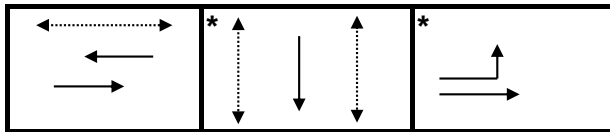
Intersection:	Main: Carling	Side: Sherwood
Controller:	ATC 3	TSD: 5135
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans[†]

	Plan				Ped Minimum Time		
	AM Peak 1	Off Peak 2	Night 9	PM Peak 13	Walk	DW	A+R
Cycle	120	130	Free	140			
Offset	112	41	X	24			
EB Thru	79	89	105.4	99	-	-	3.7+2.7
WB Thru	66	73	105.4	84	12	15	3.7+2.7
SB Thru	41	41	33.1	41	26	7	3.3+3.8
EB Left (fp)	13	16	20.2	15	-	-	3.7+1.5

Phasing Sequence[‡]

Plan: All



Notes:

- 1) In plan 9, if the NS pedestrian phases are actuated, the SB green will be extended to match.

Schedule

Weekday		Saturday		Sunday	
Time	Plan	Time	Plan	Time	Plan
0:15	9	0:15	9	0:10	9
6:30	1	7:00	2	7:30	2
9:30	2	22:30	9	22:30	9
15:00	13				
18:30	2				
22:35	9				

Notes

†: Time for each direction includes amber and all red intervals
 ‡: Start of first phase should be used as reference point for offset
 Asterisk (*) Indicates actuated phase
 (fp): Fully Protected Left Turn
 ◀.....▶ Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

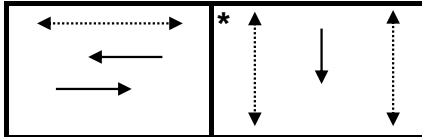
Intersection:	<i>Main:</i> Carling	<i>Side:</i> Champagne
Controller:	MS 3200	TSD: 5341
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans†

	Plan				Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 13	Night 4	Walk	DW	A+R
Cycle	120	65	70	70			
Offset	106	19	11	X			
EB Thru	82	27	32	32	-	-	3.7+1.6
WB Thru	82	27	32	32	10	10	3.7+1.6
SB Thru	38	38	38	38	7	25	3.3+2.6

Phasing Sequence‡

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
9:30	2
15:00	13
18:30	2
23:30	4

Saturday

Time	Plan
0:15	4
7:00	2
23:30	4

Sunday

Time	Plan
0:15	4
7:30	2
23:30	4

NOTES

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

◀.....▶ Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

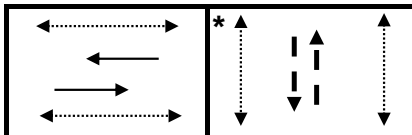
Intersection:	<i>Main:</i> Carling	<i>Side:</i> 130m W of Preston
Controller:	ATC 3	TSD: 6731
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans[†]

	Plan				Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 13	Night 4	Walk	DW	A+R
Cycle	120	65	70	70			
Offset	112	0	6	X			
EB Thru	84	30	35	35	15	5	3.7+1.4
WB Thru	84	30	35	35	15	5	3.7+1.4
NB Bike	36	35	35	35	7	22	3.0+3.6
SB Bike	36	35	35	35	7	22	3.0+3.6

Phasing Sequence[‡]

Plan: All



Schedule

Weekday		Saturday		Sunday	
Time	Plan	Time	Plan	Time	Plan
0:15	4	0:15	4	0:15	4
6:30	1	7:00	2	7:30	2
9:30	2	23:30	4	23:30	4
15:00	13				
18:30	2				
23:30	4				

Notes

[†]: Time for each direction includes amber and all red intervals

[‡]: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

←.....→ Pedestrian signal

— — ► Bike signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

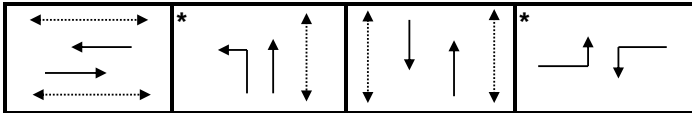
Intersection:	<i>Main:</i> Carling	<i>Side:</i> Preston
Controller:	MS 3200	TSD: 5183
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans†

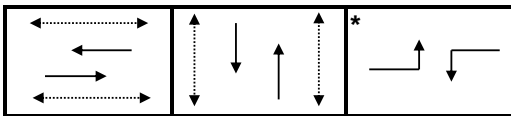
	Plan				Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 13	Night 4	Walk	DW	A+R
Cycle	120	130	140	90			
Offset	116	0	6	6			
EB Thru	35	38	41	32	7	17	3.7+2.3
WB Thru	35	38	41	32	7	17	3.7+2.3
NB Left	20	20	24	-	-	-	3.3+3.6
NB Thru	67	65	69	45	7	30	3.3+3.6
SB Thru	47	45	45	45	7	30	3.3+3.6
EB Left (fp)	18	27	30	13	-	-	3.7+2.5
WB Left (fp)	18	27	30	13	-	-	3.7+2.5

Phasing Sequence‡

Plan: 1, 2 & 13



Plan: 4



Schedule

Weekday		Saturday		Sunday	
Time	Plan	Time	Plan	Time	Plan
0:15	4	0:15	4	0:15	4
6:30	1	7:00	2	7:30	2
9:30	2	23:30	4	23:30	4
15:00	13				
18:30	2				
23:30	4				

NOTES

†: Time for each direction includes amber and all red intervals
‡: Start of first phase should be used as reference point for offset
Asterisk (*) Indicates actuated phase
(fp): Fully Protected Left Turn
◄.....► Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

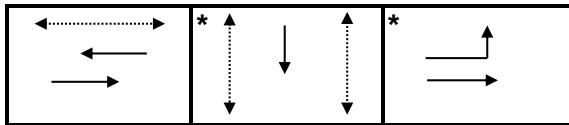
Intersection:	<i>Main:</i> Carling	<i>Side:</i> Booth
Controller:	MS 3200	TSD: 5270
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans[†]

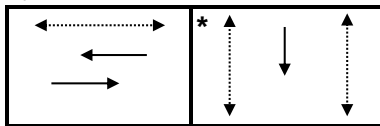
	Plan					Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 3	Night 4	Weekend 5	Walk	DW	A+R
Cycle	120	120	130	70	90			
Offset	116	85	110	X	X			
EB Thru	81	81	90	31	51	-	-	3.7+2.0
WB Thru	47	81	67	31	51	13	11	3.7+2.0
SB Thru	39	39	40	39	39	7	26	3.3+2.7
EB Left	34	-	23	-	-	-	-	3.7+2.2

Phasing Sequence[‡]

Plan: 1 & 3



Plan: 2, 4 & 5



Schedule

Weekday		Saturday		Sunday	
Time	Plan	Time	Plan	Time	Plan
0:15	4	0:15	4	0:15	4
6:30	1	7:00	5	7:00	5
9:30	2	23:30	4	23:30	4
15:00	3				
18:30	2				
23:30	4				

Notes

†: Time for each direction includes amber and all red intervals
‡: Start of first phase should be used as reference point for offset
Asterisk (*) Indicates actuated phase
(fp): Fully Protected Left Turn
◄.....► Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

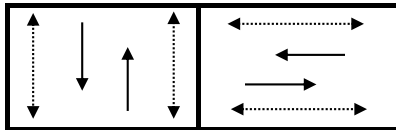
Intersection:	Main: Preston	Side: Beech
Controller:	MS 3200	TSD: 5413
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans[†]

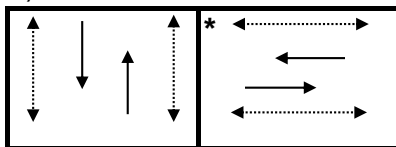
	Plan					Ped Minimum Time			
	AM Peak 1	Off Peak 2	PM Peak 3	Night 4	Weekend 5	AM Peak 11	Walk	DW	A+R
Cycle	80	80	90	70	80	80			
Offset	40	11	43	X	0	40			
NB Thru	57	57	67	47	57	57	18	10	3.3+2.2
SB Thru	57	57	67	47	57	57	18	10	3.3+2.2
EB Thru	23	23	23	23	23	23	7	10	3.3+2.3
WB Thru	23	23	23	23	23	23	7	10	3.3+2.3

Phasing Sequence[‡]

Plan: 1,2,3,5



Plan: 4,11



Schedule

Weekday

Time	Plan
0:15	4
6:00	11
7:00	1
9:30	2
15:00	3
18:00	2
22:00	4

Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

Notes

†: Time for each direction includes amber and all red intervals
†: Time for each direction includes amber and all red intervals
‡: Start of first phase should be used as reference point for offset
Asterisk (*) Indicates actuated phase
(fp): Fully Protected Left Turn
◀.....▶ Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

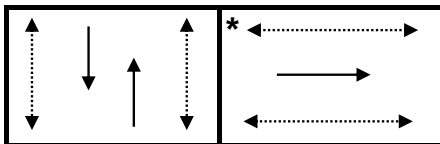
Intersection:	Main: Preston	Side: Pamilla
Controller:	MS-3200	TSD: 6150
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans†

	Plan					Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 3	Night 4	Weekend 5	Walk	DW	A+R
Cycle	80	80	90	70	80			
Offset	48	0	27	X	X			
NB Thru	59	59	69	49	59	18	5	3.3+1.8
SB Thru	59	59	69	49	59	18	5	3.3+1.8
EB Thru	21	21	21	21	21	7	8	3.3+2.2

Phasing Sequence‡

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:00	2
22:00	4

Saturday

Time	Plan
0:15	4
12:00	5
22:00	4

Sunday

Time	Plan
0:15	4
8:00	5
22:00	4

Notes

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

←.....→ Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

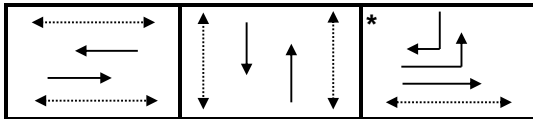
Intersection:	<i>Main:</i> Prince of Wales	<i>Side:</i> Preston
Controller:	ATC 3	TSD: 5199
Author:	Matthew Anderson	Date: 04-Mar-2021

Existing Timing Plans[†]

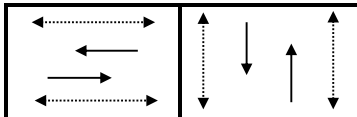
	Plan				Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 3	Night 4	Walk	DW	A+R
Cycle	120	130	130	75			
Offset	4	0	6	31			
EB Thru	90	88	81	43	7	19	3.7+2.4
WB Thru	52	52	42	43	7	19	3.7+2.4
NB Thru	30	42	49	32	12	12	3.3+2.2
SB Thru	30	42	49	32	12	12	3.3+2.2
EB Left	38	36	39	-	-	-	3.7+2.4
SB Right	38	36	39	-	-	-	3.7+2.4

Phasing Sequence[‡]

Plan: 1, 2 & 3



Plan: 4



Schedule

Weekday		Saturday		Sunday	
Time	Plan	Time	Plan	Time	Plan
0:15	4	0:15	4	0:15	4
6:30	1	7:00	2	7:30	2
9:30	2	23:30	4	23:30	4
15:00	3				
18:30	2				
23:30	4				

NOTES

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

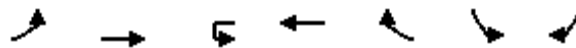
(fp): Fully Protected Left Turn

◀.....▶ Pedestrian signal

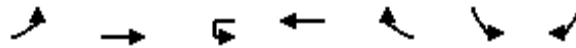
Cost is \$59.96 (\$53.06 + HST)

APPENDIX J

Existing Synchro Analysis



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	31	760	13	550	121	131	5
Future Volume (vph)	31	760	13	550	121	131	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	35.0		25.0		0.0	0.0	0.0
Storage Lanes	1		1		0	1	0
Taper Length (m)	25.0		25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	0.98			0.99		0.99	
Frt				0.973		0.995	
Flt Protected	0.950		0.950			0.954	
Satd. Flow (prot)	1642	3283	1674	4399	0	1671	0
Flt Permitted	0.950		0.335			0.954	
Satd. Flow (perm)	1617	3283	590	4399	0	1657	0
Right Turn on Red					Yes		Yes
Satd. Flow (RTOR)				57		2	
Link Speed (k/h)		60		60		40	
Link Distance (m)		196.1		162.9		242.3	
Travel Time (s)		11.8		9.8		21.8	
Confl. Peds. (#/hr)	19				19	8	9
Confl. Bikes (#/hr)					15		3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	1%	7%	2%	1%	1%
Adj. Flow (vph)	34	844	14	611	134	146	6
Shared Lane Traffic (%)							
Lane Group Flow (vph)	34	844	14	745	0	152	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	Left	Right	L NA	Right
Median Width(m)		7.0		7.0		3.5	
Link Offset(m)		0.0		0.0		0.0	
Crosswalk Width(m)		5.0		10.0		5.0	
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14		14	40	14
Number of Detectors	1	2	1	2		1	
Detector Template	Left	Thru	Left	Thru		Left	
Leading Detector (m)	6.1	30.5	6.1	30.5		6.1	
Trailing Detector (m)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Size(m)	6.1	1.8	6.1	1.8		6.1	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0		0.0	
Detector 2 Position(m)		28.7		28.7			
Detector 2 Size(m)		1.8		1.8			
Detector 2 Type		Cl+Ex		Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)		0.0		0.0			
Turn Type	Prot	NA	Perm	NA		Perm	
Protected Phases	5	2		6			
Permitted Phases			6			4	
Detector Phase	5	2	6	6		4	



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Switch Phase							
Minimum Initial (s)	5.0	10.0	10.0	10.0		10.0	
Minimum Split (s)	10.2	16.4	33.4	33.4		40.1	
Total Split (s)	13.0	79.0	66.0	66.0		41.0	
Total Split (%)	10.8%	65.8%	55.0%	55.0%		34.2%	
Maximum Green (s)	7.8	72.6	59.6	59.6		33.9	
Yellow Time (s)	3.7	3.7	3.7	3.7		3.3	
All-Red Time (s)	1.5	2.7	2.7	2.7		3.8	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	
Total Lost Time (s)	5.2	6.4	6.4	6.4		7.1	
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0	3.0		3.0	
Recall Mode	None	C-Max	C-Max	C-Max		None	
Walk Time (s)			12.0	12.0		26.0	
Flash Dont Walk (s)			15.0	15.0		7.0	
Pedestrian Calls (#/hr)			20	20		20	
Act Effct Green (s)	7.5	85.3	77.1	77.1		21.2	
Actuated g/C Ratio	0.06	0.71	0.64	0.64		0.18	
v/c Ratio	0.33	0.36	0.04	0.26		0.52	
Control Delay	62.2	8.6	8.8	6.6		48.2	
Queue Delay	0.0	0.0	0.0	0.0		0.0	
Total Delay	62.2	8.6	8.8	6.6		48.2	
LOS	E	A	A	A		D	
Approach Delay		10.7		6.7		48.2	
Approach LOS		B		A		D	
Queue Length 50th (m)	7.2	27.5	0.7	11.1		31.4	
Queue Length 95th (m)	16.8	59.2	1.9	10.7		43.2	
Internal Link Dist (m)		172.1		138.9		218.3	
Turn Bay Length (m)	35.0		25.0				
Base Capacity (vph)	112	2334	379	2848		469	
Starvation Cap Reductn	0	0	0	0		0	
Spillback Cap Reductn	0	0	0	0		0	
Storage Cap Reductn	0	0	0	0		0	
Reduced v/c Ratio	0.30	0.36	0.04	0.26		0.32	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 112 (93%), Referenced to phase 2:EBT and 6:WBTU, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 12.2

Intersection LOS: B

Intersection Capacity Utilization 51.7%

ICU Level of Service A

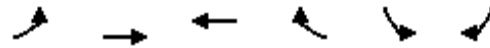
Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood



2: Carling & Champagne
AM Peak Hour

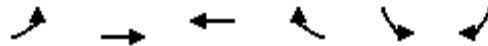
829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	98	870	700	159	57	39
Future Volume (vph)	98	870	700	159	57	39
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0			35.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.91	1.00	1.00	1.00
Ped Bike Factor	0.93			0.76	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	4672	1414	1658	1498
Flt Permitted	0.342				0.950	
Satd. Flow (perm)	559	3283	4672	1069	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				177		43
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	76			76	7	9
Confl. Bikes (#/hr)				2		1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	109	967	778	177	63	43
Shared Lane Traffic (%)						
Lane Group Flow (vph)	109	967	778	177	63	43
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	82.0	82.0	82.0	82.0	38.0	38.0
Total Split (%)	68.3%	68.3%	68.3%	68.3%	31.7%	31.7%
Maximum Green (s)	76.7	76.7	76.7	76.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	89.8	89.8	89.8	89.8	23.2	23.2
Actuated g/C Ratio	0.75	0.75	0.75	0.75	0.19	0.19
v/c Ratio	0.26	0.39	0.22	0.21	0.20	0.14
Control Delay	7.2	6.4	2.8	0.6	38.2	10.8
Queue Delay	0.0	0.1	0.0	0.0	0.0	0.0
Total Delay	7.2	6.5	2.8	0.6	38.2	10.8
LOS	A	A	A	A	D	B
Approach Delay		6.5	2.4		27.1	
Approach LOS		A	A		C	
Queue Length 50th (m)	12.4	65.0	7.4	0.0	10.5	0.0
Queue Length 95th (m)	6.8	22.0	6.9	0.3	21.0	8.1
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	45.0			35.0	20.0	
Base Capacity (vph)	418	2458	3498	844	440	423
Starvation Cap Reductn	0	322	0	0	0	0
Spillback Cap Reductn	0	11	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.45	0.22	0.21	0.14	0.10

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 106 (88%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 5.7

Intersection LOS: A

Intersection Capacity Utilization 51.8%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑↑							
Traffic Volume (vph)	0	930	0	0	930	0	0	0	0	0	0	0
Future Volume (vph)	0	930	0	0	930	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	4718	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	4718	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	26		14	14		26	18		20	20		18
Confl. Bikes (#/hr)			6			23						17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1033	0	0	1033	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1033	0	0	1033	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.40			0.27							
Control Delay		2.2			3.1							
Queue Delay		0.0			0.1							
Total Delay		2.2			3.2							
LOS		A			A							
Approach Delay		2.2			3.2							
Approach LOS		A			A							
Queue Length 50th (m)		7.9			9.7							
Queue Length 95th (m)		9.0			17.2							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			3757							
Starvation Cap Reductn		139			1273							
Spillback Cap Reductn		0			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.42			0.42							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 112 (93%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.40

Intersection Signal Delay: 2.7

Intersection LOS: A

Intersection Capacity Utilization 31.4%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 3: Trillium Pathway & Carling



Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	140	570	224	134	550	94	282	450	281	80	227	99
Future Volume (vph)	140	570	224	134	550	94	282	450	281	80	227	99
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		40.0	75.0		0.0	75.0		0.0	0.0		0.0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	0.91	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.96		0.91	0.97	0.98		0.98	0.99		1.00	0.98	
Frt			0.850		0.978			0.942			0.954	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3252	1483	1658	4456	0	1674	3051	0	1510	1517	0
Flt Permitted	0.950			0.950			0.292			0.345		
Satd. Flow (perm)	1538	3252	1350	1616	4456	0	506	3051	0	548	1517	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			199		26			163			20	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	65		41	41		65	39		2	2		39
Confl. Bikes (#/hr)			21			9			36			1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	156	633	249	149	611	104	313	500	312	89	252	110
Shared Lane Traffic (%)												
Lane Group Flow (vph)	156	633	249	149	715	0	313	812	0	89	362	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA		pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases			2				8			4		
Detector Phase	5	2	2	1	6		3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0		5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0	30.0	11.2	30.0		11.9	43.9		43.9	43.9	
Total Split (s)	18.0	35.0	35.0	18.0	35.0		20.0	67.0		47.0	47.0	
Total Split (%)	15.0%	29.2%	29.2%	15.0%	29.2%		16.7%	55.8%		39.2%	39.2%	
Maximum Green (s)	11.8	29.0	29.0	11.8	29.0		13.1	60.1		40.1	40.1	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3	2.3	2.5	2.3		3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0	6.0	6.2	6.0		6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag	Lag	Lead	Lag		Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max		None	Ped		Ped	Ped	
Walk Time (s)		7.0	7.0		7.0			7.0		7.0	7.0	
Flash Dont Walk (s)		17.0	17.0		17.0			30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20	20		20			20		20	20	
Act Effct Green (s)	14.2	29.4	29.4	13.8	29.0		57.7	57.7		37.7	37.7	
Actuated g/C Ratio	0.12	0.24	0.24	0.12	0.24		0.48	0.48		0.31	0.31	
v/c Ratio	0.83	0.80	0.52	0.78	0.65		0.85	0.52		0.52	0.74	
Control Delay	96.8	40.0	8.7	93.3	41.7		47.4	25.5		46.0	44.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	96.8	40.0	8.7	93.3	41.7		47.4	25.5		46.0	44.9	
LOS	F	D	A	F	D		D	C		D	D	
Approach Delay		41.0			50.6			31.6			45.1	
Approach LOS		D			D			C			D	
Queue Length 50th (m)	36.5	27.3	0.5	34.1	21.4		55.0	69.1		15.9	67.0	
Queue Length 95th (m)	#76.3	60.5	16.1	#70.5	51.1		m#74.6	m91.3		31.6	96.3	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0		40.0	75.0			75.0					
Base Capacity (vph)	189	796	481	190	1096		370	1609		183	520	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.83	0.80	0.52	0.78	0.65		0.85	0.50		0.49	0.70	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 116 (97%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 40.9

Intersection LOS: D

Intersection Capacity Utilization 97.2%

ICU Level of Service F

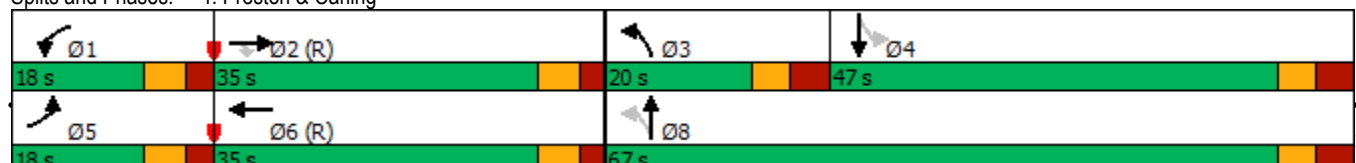
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

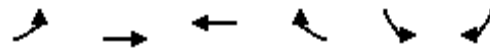
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

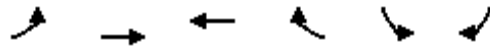
829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	382	650	650	207	204	132
Future Volume (vph)	382	650	650	207	204	132
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0			35.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	40.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor	0.98		0.96		0.98	0.88
Frt			0.964			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	4373	0	1674	1427
Flt Permitted	0.215				0.950	
Satd. Flow (perm)	370	3252	4373	0	1649	1258
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			73			147
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	59			59	14	85
Confl. Bikes (#/hr)				15		23
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	424	722	722	230	227	147
Shared Lane Traffic (%)						
Lane Group Flow (vph)	424	722	952	0	227	147
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2		1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (m)	6.1	30.5	30.5		6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0		0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0		0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8		6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA		Perm	Perm
Protected Phases	5	2	6			
Permitted Phases	2				4	4
Detector Phase	5	2	6		4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0		10.0	10.0
Minimum Split (s)	10.9	15.7	29.7		39.0	39.0
Total Split (s)	34.0	81.0	47.0		39.0	39.0
Total Split (%)	28.3%	67.5%	39.2%		32.5%	32.5%
Maximum Green (s)	28.1	75.3	41.3		33.0	33.0
Yellow Time (s)	3.7	3.7	3.7		3.3	3.3
All-Red Time (s)	2.2	2.0	2.0		2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7		6.0	6.0
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)			13.0		7.0	7.0
Flash Dont Walk (s)			11.0		26.0	26.0
Pedestrian Calls (#/hr)			20		20	20
Act Effct Green (s)	83.9	84.1	55.7		24.2	24.2
Actuated g/C Ratio	0.70	0.70	0.46		0.20	0.20
v/c Ratio	0.84	0.32	0.46		0.69	0.40
Control Delay	45.8	8.5	23.4		54.0	8.7
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	45.8	8.5	23.4		54.0	8.7
LOS	D	A	C		D	A
Approach Delay		22.3	23.4		36.2	
Approach LOS		C	C		D	
Queue Length 50th (m)	75.2	29.5	46.4		47.2	0.0
Queue Length 95th (m)	m#107.8	49.0	71.3		64.3	14.0
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	45.0					45.0
Base Capacity (vph)	564	2280	2070		453	452
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.75	0.32	0.46		0.50	0.33

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 116 (97%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 24.8

Intersection LOS: C

Intersection Capacity Utilization 83.4%

ICU Level of Service E

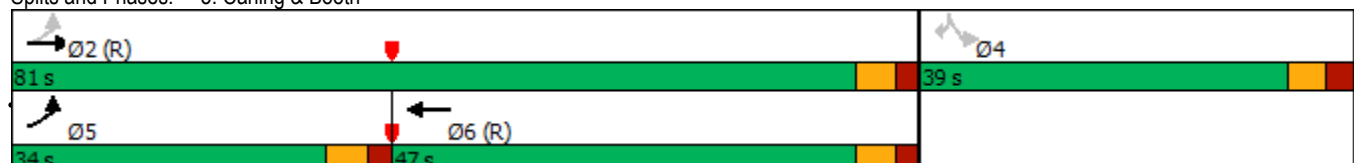
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	38	52	26	19	54	8	23	560	42	18	340	36
Future Volume (vph)	38	52	26	19	54	8	23	560	42	18	340	36
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.98	0.89	0.97	0.99		0.98	0.99	
Frt		0.970				0.850		0.989			0.986	
Flt Protected		0.984			0.987		0.950			0.950		
Satd. Flow (prot)	0	1557	0	0	1627	1498	1537	1696	0	1537	1643	0
Flt Permitted		0.876			0.904		0.488			0.322		
Satd. Flow (perm)	0	1352	0	0	1465	1339	764	1696	0	508	1643	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				34		10			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		36	36		36	43		55	55		43
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	42	58	29	21	60	9	26	622	47	20	378	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	129	0	0	81	9	26	669	0	20	418	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.43			0.26	0.03	0.05	0.61		0.06	0.39	
Control Delay		28.6			28.8	0.5	5.3	9.1		5.8	7.7	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.6			28.8	0.5	5.3	9.2		5.8	7.7	
LOS		C			C	A	A	A		A	A	
Approach Delay		28.6			26.0			9.1			7.7	
Approach LOS		C			C			A			A	
Queue Length 50th (m)		13.5			9.5	0.0	1.2	46.6		0.9	23.1	
Queue Length 95th (m)		27.9			20.1	0.5	m2.4	37.9		3.1	38.1	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		307			318	317	494	1101		329	1068	
Starvation Cap Reductn		0			0	0	0	45		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.42			0.25	0.03	0.05	0.63		0.06	0.39	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.61

Intersection Signal Delay: 11.6

Intersection LOS: B

Intersection Capacity Utilization 76.3%

ICU Level of Service D

Analysis Period (min) 15

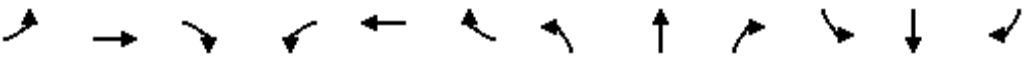
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	580	42	10	400	5
Future Volume (vph)	1	0	3	0	0	0	8	580	42	10	400	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.991			0.998	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1701	0	0	1617	0
Flt Permitted		0.988						0.995			0.984	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1694	0	0	1591	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29						10			2	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	28		45	45		28
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	9	644	47	11	444	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	700	0	0	461	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.47			0.33	
Control Delay		0.0						5.2			1.9	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.2			1.9	
LOS		A						A			A	
Approach Delay								5.2			1.9	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						71.1			14.1	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1478			1387	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.47			0.33	
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green												
Natural Cycle: 60												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.47												
Intersection Signal Delay: 3.9				Intersection LOS: A								
Intersection Capacity Utilization 58.2%				ICU Level of Service B								
Analysis Period (min) 15												

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
Existing Traffic

													
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	9	1	11	10	5	17	9	567	84	25	402	15	
Future Volume (vph)	9	1	11	10	5	17	9	567	84	25	402	15	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor													
Frt	0.930				0.929				0.983		0.995		
Flt Protected	0.979				0.985				0.999		0.997		
Satd. Flow (prot)	0	1589	0	0	1597	0	0	1699	0	0	1686	0	
Flt Permitted	0.979				0.985				0.999		0.997		
Satd. Flow (perm)	0	1589	0	0	1597	0	0	1699	0	0	1686	0	
Link Speed (k/h)	50				50				50		50		
Link Distance (m)	113.6				154.3				71.5		73.8		
Travel Time (s)	8.2				11.1				5.1		5.3		
Confl. Peds. (#/hr)							28		45		45		
Confl. Bikes (#/hr)							21						17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%	
Adj. Flow (vph)	10	1	12	11	6	19	10	630	93	28	447	17	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	23	0	0	36	0	0	733	0	0	492	0	
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No	
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	0.0				0.0				0.0		0.0		
Link Offset(m)	-2.0				-2.0				0.0		0.0		
Crosswalk Width(m)	5.0				5.0				5.0		5.0		
Two way Left Turn Lane													
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	24		14	24		14	24		14	24		14	
Sign Control	Stop				Stop				Free		Free		
Intersection Summary													
Area Type:	Other												
Control Type:	Unsignalized												
Intersection Capacity Utilization 50.2%	ICU Level of Service A												
Analysis Period (min) 15													

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
Existing Traffic

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	5	10	29	655	396	27
Future Volume (vph)	5	10	29	655	396	27
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.913				0.991	
Flt Protected	0.983			0.998		
Satd. Flow (prot)	1566	0	0	3278	1683	0
Flt Permitted	0.983			0.998		
Satd. Flow (perm)	1566	0	0	3278	1683	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	6	11	32	728	440	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	17	0	0	760	470	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	51.6%			ICU Level of Service A		
Analysis Period (min)	15					

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	640	263	2	2	176	310	1	4	3	250	4	300
Future Volume (vph)	640	263	2	2	176	310	1	4	3	250	4	300
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.93
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1668	1469
Flt Permitted	0.530			0.580				0.969			0.724	
Satd. Flow (perm)	909	1760	0	999	1762	1462	0	1177	0	0	1193	1371
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				344		3				333
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	19		24	24		19
Confl. Bikes (#/hr)			2						1			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	711	292	2	2	196	344	1	4	3	278	4	333
Shared Lane Traffic (%)												
Lane Group Flow (vph)	711	294	0	2	196	344	0	8	0	0	282	333
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		47.5	47.5	47.5		24.5			24.5	54.2
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.87	0.24		0.01	0.28	0.44		0.03			1.16	0.40
Control Delay	22.9	7.1		23.0	26.6	4.5		32.1			144.2	2.5
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	22.9	7.1		23.0	26.6	4.5		32.1			144.2	2.5
LOS	C	A		C	C	A		C			F	A
Approach Delay		18.3			12.5			32.1			67.4	
Approach LOS		B			B			C			E	
Queue Length 50th (m)	73.5	20.6		0.3	29.2	0.0		0.9			~70.9	3.0
Queue Length 95th (m)	#115.3	30.5		1.9	45.9	16.7		4.8			m#117.9	m6.5
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	830	1230		396	697	786		242			243	841
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.86	0.24		0.01	0.28	0.44		0.03			1.16	0.40

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.16

Intersection Signal Delay: 30.8

Intersection LOS: C

Intersection Capacity Utilization 95.4%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

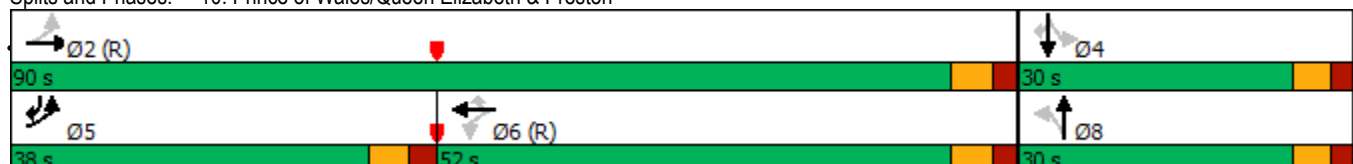
Queue shown is maximum after two cycles.

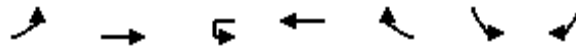
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

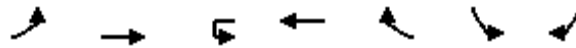
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston





Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	31	760	13	550	121	131	5
Future Volume (vph)	31	760	13	550	121	131	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	35.0		25.0		0.0	0.0	0.0
Storage Lanes	1		1		0	1	0
Taper Length (m)	25.0		25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	0.98			0.99		0.99	
Frt				0.973		0.995	
Flt Protected	0.950		0.950			0.954	
Satd. Flow (prot)	1642	3283	1674	4399	0	1671	0
Flt Permitted	0.950		0.335			0.954	
Satd. Flow (perm)	1617	3283	590	4399	0	1657	0
Right Turn on Red					Yes		Yes
Satd. Flow (RTOR)				57		2	
Link Speed (k/h)		60		60		40	
Link Distance (m)		196.1		162.9		242.3	
Travel Time (s)		11.8		9.8		21.8	
Confl. Peds. (#/hr)	19				19	8	9
Confl. Bikes (#/hr)					15		3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	1%	7%	2%	1%	1%
Adj. Flow (vph)	34	844	14	611	134	146	6
Shared Lane Traffic (%)							
Lane Group Flow (vph)	34	844	14	745	0	152	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	Left	Right	L NA	Right
Median Width(m)		7.0		7.0		3.5	
Link Offset(m)		0.0		0.0		0.0	
Crosswalk Width(m)		5.0		10.0		5.0	
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14		14	40	14
Number of Detectors	1	2	1	2		1	
Detector Template	Left	Thru	Left	Thru		Left	
Leading Detector (m)	6.1	30.5	6.1	30.5		6.1	
Trailing Detector (m)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Size(m)	6.1	1.8	6.1	1.8		6.1	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0		0.0	
Detector 2 Position(m)		28.7		28.7			
Detector 2 Size(m)		1.8		1.8			
Detector 2 Type		Cl+Ex		Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)		0.0		0.0			
Turn Type	Prot	NA	Perm	NA		Perm	
Protected Phases	5	2		6			
Permitted Phases			6			4	
Detector Phase	5	2	6	6		4	



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Switch Phase							
Minimum Initial (s)	5.0	10.0	10.0	10.0		10.0	
Minimum Split (s)	10.2	16.4	33.4	33.4		40.1	
Total Split (s)	13.0	79.0	66.0	66.0		41.0	
Total Split (%)	10.8%	65.8%	55.0%	55.0%		34.2%	
Maximum Green (s)	7.8	72.6	59.6	59.6		33.9	
Yellow Time (s)	3.7	3.7	3.7	3.7		3.3	
All-Red Time (s)	1.5	2.7	2.7	2.7		3.8	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	
Total Lost Time (s)	5.2	6.4	6.4	6.4		7.1	
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0	3.0		3.0	
Recall Mode	None	C-Max	C-Max	C-Max		None	
Walk Time (s)			12.0	12.0		26.0	
Flash Dont Walk (s)			15.0	15.0		7.0	
Pedestrian Calls (#/hr)			20	20		20	
Act Effct Green (s)	7.5	85.3	77.1	77.1		21.2	
Actuated g/C Ratio	0.06	0.71	0.64	0.64		0.18	
v/c Ratio	0.33	0.36	0.04	0.26		0.52	
Control Delay	62.2	8.6	8.8	6.6		48.2	
Queue Delay	0.0	0.0	0.0	0.0		0.0	
Total Delay	62.2	8.6	8.8	6.6		48.2	
LOS	E	A	A	A		D	
Approach Delay		10.7		6.7		48.2	
Approach LOS		B		A		D	
Queue Length 50th (m)	7.2	27.5	0.7	11.1		31.4	
Queue Length 95th (m)	16.8	59.2	1.9	10.7		43.2	
Internal Link Dist (m)		172.1		138.9		218.3	
Turn Bay Length (m)	35.0		25.0				
Base Capacity (vph)	112	2334	379	2848		469	
Starvation Cap Reductn	0	0	0	0		0	
Spillback Cap Reductn	0	0	0	0		0	
Storage Cap Reductn	0	0	0	0		0	
Reduced v/c Ratio	0.30	0.36	0.04	0.26		0.32	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 112 (93%), Referenced to phase 2:EBT and 6:WBTU, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 12.2

Intersection LOS: B

Intersection Capacity Utilization 51.7%

ICU Level of Service A

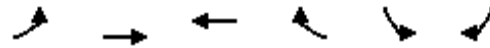
Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood



2: Carling & Champagne
AM Peak Hour

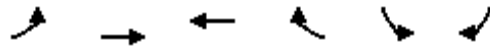
829 Carling Avenue
Existing Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	98	870	700	159	57	39
Future Volume (vph)	98	870	700	159	57	39
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0			35.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.91	1.00	1.00	1.00
Ped Bike Factor	0.93			0.76	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	4672	1414	1658	1498
Flt Permitted	0.342				0.950	
Satd. Flow (perm)	559	3283	4672	1069	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				177		43
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	76			76	7	9
Confl. Bikes (#/hr)				2		1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	109	967	778	177	63	43
Shared Lane Traffic (%)						
Lane Group Flow (vph)	109	967	778	177	63	43
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	82.0	82.0	82.0	82.0	38.0	38.0
Total Split (%)	68.3%	68.3%	68.3%	68.3%	31.7%	31.7%
Maximum Green (s)	76.7	76.7	76.7	76.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	89.8	89.8	89.8	89.8	23.2	23.2
Actuated g/C Ratio	0.75	0.75	0.75	0.75	0.19	0.19
v/c Ratio	0.26	0.39	0.22	0.21	0.20	0.14
Control Delay	7.2	6.4	2.8	0.6	38.2	10.8
Queue Delay	0.0	0.1	0.0	0.0	0.0	0.0
Total Delay	7.2	6.5	2.8	0.6	38.2	10.8
LOS	A	A	A	A	D	B
Approach Delay		6.5	2.4		27.1	
Approach LOS		A	A		C	
Queue Length 50th (m)	12.4	65.0	7.4	0.0	10.5	0.0
Queue Length 95th (m)	6.8	22.0	6.9	0.3	21.0	8.1
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	45.0			35.0	20.0	
Base Capacity (vph)	418	2458	3498	844	440	423
Starvation Cap Reductn	0	322	0	0	0	0
Spillback Cap Reductn	0	11	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.45	0.22	0.21	0.14	0.10

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 106 (88%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 5.7

Intersection LOS: A

Intersection Capacity Utilization 51.8%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑↑							
Traffic Volume (vph)	0	930	0	0	930	0	0	0	0	0	0	0
Future Volume (vph)	0	930	0	0	930	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	4718	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	4718	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	26		14	14		26	18		20	20		18
Confl. Bikes (#/hr)			6			23						17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1033	0	0	1033	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1033	0	0	1033	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.40			0.27							
Control Delay		2.2			3.1							
Queue Delay		0.0			0.1							
Total Delay		2.2			3.2							
LOS		A			A							
Approach Delay		2.2			3.2							
Approach LOS		A			A							
Queue Length 50th (m)		7.9			9.7							
Queue Length 95th (m)		9.0			17.2							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			3757							
Starvation Cap Reductn		139			1273							
Spillback Cap Reductn		0			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.42			0.42							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 112 (93%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.40

Intersection Signal Delay: 2.7

Intersection LOS: A

Intersection Capacity Utilization 31.4%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
84 s	36 s
 Ø6 (R)	
84 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	140	570	224	134	550	94	282	450	281	80	227	99
Future Volume (vph)	140	570	224	134	550	94	282	450	281	80	227	99
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		40.0	75.0		0.0	75.0		0.0	0.0		0.0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	0.91	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.96		0.91	0.97	0.98		0.98	0.99		1.00	0.98	
Frt			0.850		0.978			0.942			0.954	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3252	1483	1658	4456	0	1674	3051	0	1510	1517	0
Flt Permitted	0.950			0.950			0.292			0.345		
Satd. Flow (perm)	1538	3252	1350	1616	4456	0	506	3051	0	548	1517	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			199		26			163			20	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	65		41	41		65	39		2	2		39
Confl. Bikes (#/hr)			21			9			36			1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	156	633	249	149	611	104	313	500	312	89	252	110
Shared Lane Traffic (%)												
Lane Group Flow (vph)	156	633	249	149	715	0	313	812	0	89	362	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA		pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases			2				8			4		
Detector Phase	5	2	2	1	6		3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0		5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0	30.0	11.2	30.0		11.9	43.9		43.9	43.9	
Total Split (s)	18.0	35.0	35.0	18.0	35.0		20.0	67.0		47.0	47.0	
Total Split (%)	15.0%	29.2%	29.2%	15.0%	29.2%		16.7%	55.8%		39.2%	39.2%	
Maximum Green (s)	11.8	29.0	29.0	11.8	29.0		13.1	60.1		40.1	40.1	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3	2.3	2.5	2.3		3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0	6.0	6.2	6.0		6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag	Lag	Lead	Lag		Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max		None	Ped		Ped	Ped	
Walk Time (s)		7.0	7.0		7.0			7.0		7.0	7.0	
Flash Dont Walk (s)		17.0	17.0		17.0			30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20	20		20			20		20	20	
Act Effct Green (s)	14.2	29.4	29.4	13.8	29.0		57.7	57.7		37.7	37.7	
Actuated g/C Ratio	0.12	0.24	0.24	0.12	0.24		0.48	0.48		0.31	0.31	
v/c Ratio	0.83	0.80	0.52	0.78	0.65		0.85	0.52		0.52	0.74	
Control Delay	96.8	40.0	8.7	93.3	41.7		47.4	25.5		46.0	44.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	96.8	40.0	8.7	93.3	41.7		47.4	25.5		46.0	44.9	
LOS	F	D	A	F	D		D	C		D	D	
Approach Delay		41.0			50.6			31.6			45.1	
Approach LOS		D			D			C			D	
Queue Length 50th (m)	36.5	27.3	0.5	34.1	21.4		55.0	69.1		15.9	67.0	
Queue Length 95th (m)	#76.3	60.5	16.1	#70.5	51.1		m#74.6	m91.3		31.6	96.3	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0		40.0	75.0			75.0					
Base Capacity (vph)	189	796	481	190	1096		370	1609		183	520	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.83	0.80	0.52	0.78	0.65		0.85	0.50		0.49	0.70	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 116 (97%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 40.9

Intersection LOS: D

Intersection Capacity Utilization 97.2%

ICU Level of Service F

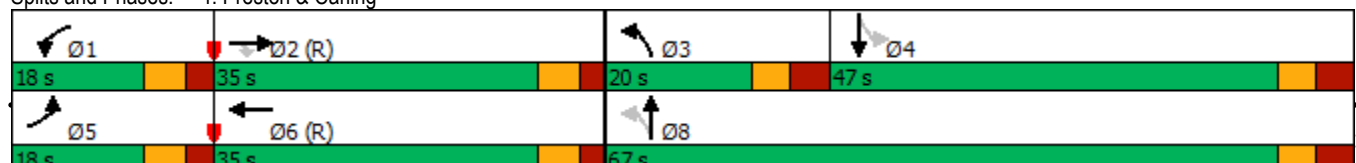
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

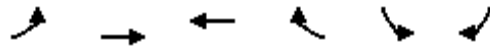
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

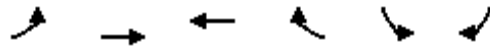
829 Carling Avenue
Existing Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	382	650	650	207	204	132
Future Volume (vph)	382	650	650	207	204	132
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0			35.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	40.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor	0.98		0.96		0.98	0.88
Frt			0.964			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	4373	0	1674	1427
Flt Permitted	0.215				0.950	
Satd. Flow (perm)	370	3252	4373	0	1649	1258
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			73			147
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	59			59	14	85
Confl. Bikes (#/hr)				15		23
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	424	722	722	230	227	147
Shared Lane Traffic (%)						
Lane Group Flow (vph)	424	722	952	0	227	147
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2		1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (m)	6.1	30.5	30.5		6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0		0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0		0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8		6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA		Perm	Perm
Protected Phases	5	2	6			
Permitted Phases	2				4	4
Detector Phase	5	2	6		4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0		10.0	10.0
Minimum Split (s)	10.9	15.7	29.7		39.0	39.0
Total Split (s)	34.0	81.0	47.0		39.0	39.0
Total Split (%)	28.3%	67.5%	39.2%		32.5%	32.5%
Maximum Green (s)	28.1	75.3	41.3		33.0	33.0
Yellow Time (s)	3.7	3.7	3.7		3.3	3.3
All-Red Time (s)	2.2	2.0	2.0		2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7		6.0	6.0
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)			13.0		7.0	7.0
Flash Dont Walk (s)			11.0		26.0	26.0
Pedestrian Calls (#/hr)			20		20	20
Act Effct Green (s)	83.9	84.1	55.7		24.2	24.2
Actuated g/C Ratio	0.70	0.70	0.46		0.20	0.20
v/c Ratio	0.84	0.32	0.46		0.69	0.40
Control Delay	45.8	8.5	23.4		54.0	8.7
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	45.8	8.5	23.4		54.0	8.7
LOS	D	A	C		D	A
Approach Delay		22.3	23.4		36.2	
Approach LOS		C	C		D	
Queue Length 50th (m)	75.2	29.5	46.4		47.2	0.0
Queue Length 95th (m)	m#107.8	49.0	71.3		64.3	14.0
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	45.0					45.0
Base Capacity (vph)	564	2280	2070		453	452
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.75	0.32	0.46		0.50	0.33

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 116 (97%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 24.8

Intersection LOS: C

Intersection Capacity Utilization 83.4%

ICU Level of Service E

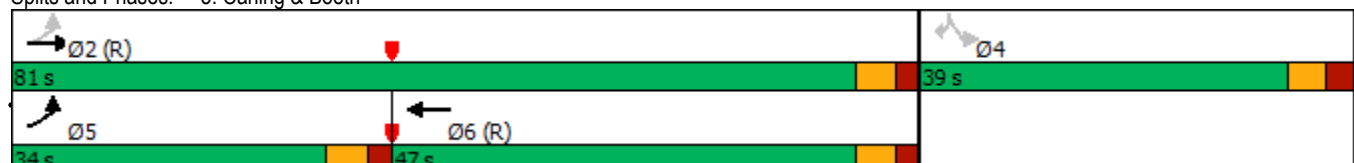
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	38	52	26	19	54	8	23	560	42	18	340	36
Future Volume (vph)	38	52	26	19	54	8	23	560	42	18	340	36
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.98	0.89	0.97	0.99		0.98	0.99	
Frt		0.970				0.850		0.989			0.986	
Flt Protected		0.984			0.987		0.950			0.950		
Satd. Flow (prot)	0	1557	0	0	1627	1498	1537	1696	0	1537	1643	0
Flt Permitted		0.876			0.904		0.488			0.322		
Satd. Flow (perm)	0	1352	0	0	1465	1339	764	1696	0	508	1643	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				34		10			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		36	36		36	43		55	55		43
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	42	58	29	21	60	9	26	622	47	20	378	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	129	0	0	81	9	26	669	0	20	418	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.43			0.26	0.03	0.05	0.61		0.06	0.39	
Control Delay		28.6			28.8	0.5	5.3	9.1		5.8	7.7	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.6			28.8	0.5	5.3	9.2		5.8	7.7	
LOS		C			C	A	A	A		A	A	
Approach Delay		28.6			26.0			9.1			7.7	
Approach LOS		C			C			A			A	
Queue Length 50th (m)		13.5			9.5	0.0	1.2	46.6		0.9	23.1	
Queue Length 95th (m)		27.9			20.1	0.5	m2.4	37.9		3.1	38.1	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		307			318	317	494	1101		329	1068	
Starvation Cap Reductn		0			0	0	0	45		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.42			0.25	0.03	0.05	0.63		0.06	0.39	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.61

Intersection Signal Delay: 11.6

Intersection LOS: B

Intersection Capacity Utilization 76.3%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	580	42	10	400	5
Future Volume (vph)	1	0	3	0	0	0	8	580	42	10	400	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.991			0.998	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1701	0	0	1617	0
Flt Permitted		0.988						0.995			0.984	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1694	0	0	1591	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29						10			2	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	28		45	45		28
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	9	644	47	11	444	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	700	0	0	461	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.47			0.33	
Control Delay		0.0						5.2			1.9	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.2			1.9	
LOS		A						A			A	
Approach Delay								5.2			1.9	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						71.1			14.1	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1478			1387	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.47			0.33	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 3.9

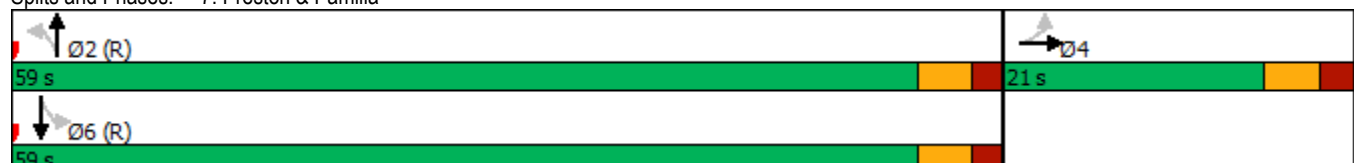
Intersection LOS: A

Intersection Capacity Utilization 58.2%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	9	1	11	10	5	17	9	567	84	25	402	15
Future Volume (vph)	9	1	11	10	5	17	9	567	84	25	402	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt	0.930				0.929				0.983		0.995	
Flt Protected	0.979				0.985				0.999		0.997	
Satd. Flow (prot)	0	1589	0	0	1597	0	0	1699	0	0	1686	0
Flt Permitted	0.979				0.985				0.999		0.997	
Satd. Flow (perm)	0	1589	0	0	1597	0	0	1699	0	0	1686	0
Link Speed (k/h)	50				50				50		50	
Link Distance (m)	113.6				154.3				71.5		73.8	
Travel Time (s)	8.2				11.1				5.1		5.3	
Confl. Peds. (#/hr)							28		45		45	
Confl. Bikes (#/hr)									21		17	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	10	1	12	11	6	19	10	630	93	28	447	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	23	0	0	36	0	0	733	0	0	492	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0				0.0				0.0		0.0	
Link Offset(m)	-2.0				-2.0				0.0		0.0	
Crosswalk Width(m)	5.0				5.0				5.0		5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control	Stop				Stop				Free		Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	50.2%											
Analysis Period (min)	15											
ICU Level of Service A												

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	5	10	29	655	396	27
Future Volume (vph)	5	10	29	655	396	27
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.913				0.991	
Flt Protected	0.983			0.998		
Satd. Flow (prot)	1566	0	0	3278	1683	0
Flt Permitted	0.983			0.998		
Satd. Flow (perm)	1566	0	0	3278	1683	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	6	11	32	728	440	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	17	0	0	760	470	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	51.6%			ICU Level of Service A		
Analysis Period (min)	15					

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	640	263	2	2	176	310	1	4	3	210	4	300
Future Volume (vph)	640	263	2	2	176	310	1	4	3	210	4	300
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.93
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1666	1469
Flt Permitted	0.530			0.580				0.972			0.724	
Satd. Flow (perm)	909	1760	0	999	1762	1462	0	1180	0	0	1192	1371
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				344		3				333
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	19		24	24		19
Confl. Bikes (#/hr)			2						1			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	711	292	2	2	196	344	1	4	3	233	4	333
Shared Lane Traffic (%)												
Lane Group Flow (vph)	711	294	0	2	196	344	0	8	0	0	237	333
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		47.5	47.5	47.5		24.5			24.5	54.2
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.87	0.24		0.01	0.28	0.44		0.03			0.98	0.40
Control Delay	22.9	7.1		23.0	26.6	4.5		32.1			93.1	2.5
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	22.9	7.1		23.0	26.6	4.5		32.1			93.1	2.5
LOS	C	A		C	C	A		C			F	A
Approach Delay		18.3			12.5			32.1			40.1	
Approach LOS		B			B			C			D	
Queue Length 50th (m)	73.5	20.6		0.3	29.2	0.0		0.9			45.9	3.0
Queue Length 95th (m)	#115.3	30.5		1.9	45.9	16.7		4.8			m#93.8	m6.5
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	830	1230		396	697	786		243			243	841
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.86	0.24		0.01	0.28	0.44		0.03			0.98	0.40

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 22.7

Intersection LOS: C

Intersection Capacity Utilization 93.8%

ICU Level of Service F

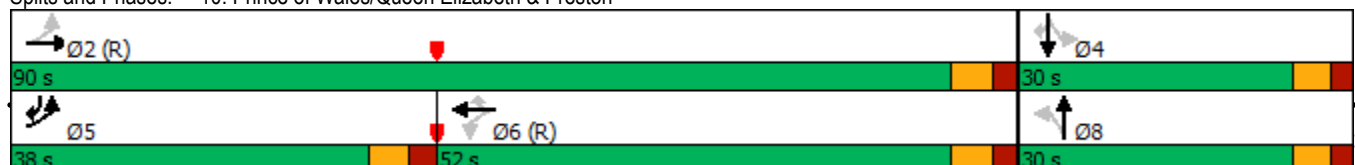
Analysis Period (min) 15

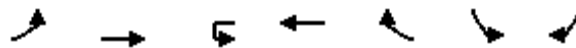
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

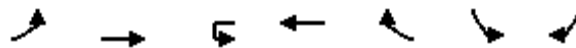
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston





Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	65	760	13	1480	153	174	7
Future Volume (vph)	65	760	13	1480	153	174	7
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	35.0		25.0		0.0	0.0	0.0
Storage Lanes	1		1		0	1	0
Taper Length (m)	25.0		25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	0.99			0.99		0.99	
Frt				0.986		0.995	
Flt Protected	0.950		0.950			0.954	
Satd. Flow (prot)	1674	3252	1674	4660	0	1671	0
Flt Permitted	0.950		0.335			0.954	
Satd. Flow (perm)	1664	3252	590	4660	0	1657	0
Right Turn on Red					Yes		Yes
Satd. Flow (RTOR)				20		1	
Link Speed (k/h)		60		60		40	
Link Distance (m)		196.1		162.9		242.3	
Travel Time (s)		11.8		9.8		21.8	
Confl. Peds. (#/hr)	28				28	7	8
Confl. Bikes (#/hr)					5		8
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	4%	1%	2%	1%	1%	1%
Adj. Flow (vph)	72	844	14	1644	170	193	8
Shared Lane Traffic (%)							
Lane Group Flow (vph)	72	844	14	1814	0	201	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	Left	Right	L NA	Right
Median Width(m)		7.0		7.0		3.5	
Link Offset(m)		0.0		0.0		0.0	
Crosswalk Width(m)		5.0		10.0		5.0	
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14		14	40	14
Number of Detectors	1	2	1	2		1	
Detector Template	Left	Thru	Left	Thru		Left	
Leading Detector (m)	6.1	30.5	6.1	30.5		6.1	
Trailing Detector (m)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Size(m)	6.1	1.8	6.1	1.8		6.1	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0		0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0		0.0	
Detector 2 Position(m)		28.7		28.7			
Detector 2 Size(m)		1.8		1.8			
Detector 2 Type		Cl+Ex		Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)		0.0		0.0			
Turn Type	Prot	NA	Perm	NA		Perm	
Protected Phases	5	2		6			
Permitted Phases			6			4	
Detector Phase	5	2	6	6		4	



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Switch Phase							
Minimum Initial (s)	5.0	10.0	10.0	10.0		10.0	
Minimum Split (s)	10.2	16.4	33.4	33.4		40.1	
Total Split (s)	15.0	99.0	84.0	84.0		41.0	
Total Split (%)	10.7%	70.7%	60.0%	60.0%		29.3%	
Maximum Green (s)	9.8	92.6	77.6	77.6		33.9	
Yellow Time (s)	3.7	3.7	3.7	3.7		3.3	
All-Red Time (s)	1.5	2.7	2.7	2.7		3.8	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	
Total Lost Time (s)	5.2	6.4	6.4	6.4		7.1	
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0	3.0		3.0	
Recall Mode	None	C-Max	C-Max	C-Max		None	
Walk Time (s)			12.0	12.0		26.0	
Flash Dont Walk (s)			15.0	15.0		7.0	
Pedestrian Calls (#/hr)			20	20		20	
Act Effct Green (s)	10.0	102.2	87.0	87.0		24.3	
Actuated g/C Ratio	0.07	0.73	0.62	0.62		0.17	
v/c Ratio	0.61	0.36	0.04	0.62		0.70	
Control Delay	84.5	8.2	14.7	16.4		65.8	
Queue Delay	0.0	0.0	0.0	0.4		0.0	
Total Delay	84.5	8.2	14.7	16.8		65.8	
LOS	F	A	B	B		E	
Approach Delay		14.2		16.7		65.8	
Approach LOS		B		B		E	
Queue Length 50th (m)	17.9	34.6	0.6	28.9		49.5	
Queue Length 95th (m)	#33.8	57.9	m2.4	155.1		68.0	
Internal Link Dist (m)		172.1		138.9		218.3	
Turn Bay Length (m)	35.0		25.0				
Base Capacity (vph)	125	2372	366	2903		401	
Starvation Cap Reductn	0	0	0	495		0	
Spillback Cap Reductn	0	0	0	0		0	
Storage Cap Reductn	0	0	0	0		0	
Reduced v/c Ratio	0.58	0.36	0.04	0.75		0.50	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 24 (17%), Referenced to phase 2:EBT and 6:WBTU, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 19.3

Intersection LOS: B

Intersection Capacity Utilization 68.4%

ICU Level of Service C

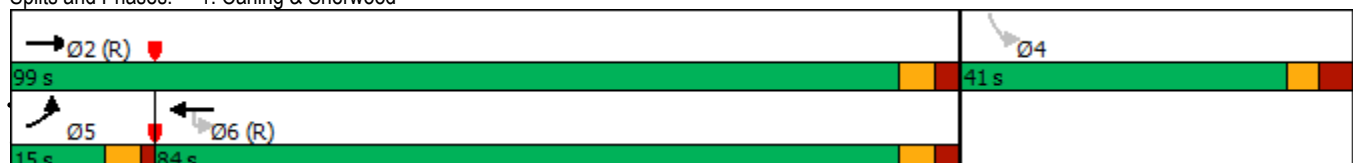
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

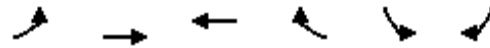
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Carling & Sherwood



2: Carling & Champagne
PM Peak Hour

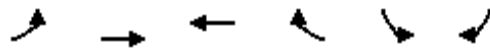
829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	44	800	1400	46	129	182
Future Volume (vph)	44	800	1400	46	129	182
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0			35.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.91	1.00	1.00	1.00
Ped Bike Factor	0.99			0.86	1.00	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	4764	1498	1674	1498
Flt Permitted	0.116				0.950	
Satd. Flow (perm)	171	3283	4764	1283	1669	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				45		2
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	70			70	5	16
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	49	889	1556	51	143	202
Shared Lane Traffic (%)						
Lane Group Flow (vph)	49	889	1556	51	143	202
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	32.0	32.0	32.0	32.0	38.0	38.0
Total Split (%)	45.7%	45.7%	45.7%	45.7%	54.3%	54.3%
Maximum Green (s)	26.7	26.7	26.7	26.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	38.5	38.5	38.5	38.5	20.3	20.3
Actuated g/C Ratio	0.55	0.55	0.55	0.55	0.29	0.29
v/c Ratio	0.52	0.49	0.59	0.07	0.30	0.48
Control Delay	39.3	9.9	6.4	1.5	18.3	21.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.1
Total Delay	39.3	9.9	6.4	1.5	18.3	21.9
LOS	D	A	A	A	B	C
Approach Delay		11.4	6.2		20.4	
Approach LOS		B	A		C	
Queue Length 50th (m)	2.7	25.3	48.6	0.3	14.9	22.0
Queue Length 95th (m)	#24.9	47.2	15.0	0.0	18.8	26.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	45.0			35.0	20.0	
Base Capacity (vph)	94	1805	2620	725	765	672
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	2	38	0	0	53
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.52	0.49	0.60	0.07	0.19	0.33

Intersection Summary

Area Type: Other

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 11 (16%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.59

Intersection Signal Delay: 9.6

Intersection LOS: A

Intersection Capacity Utilization 63.8%

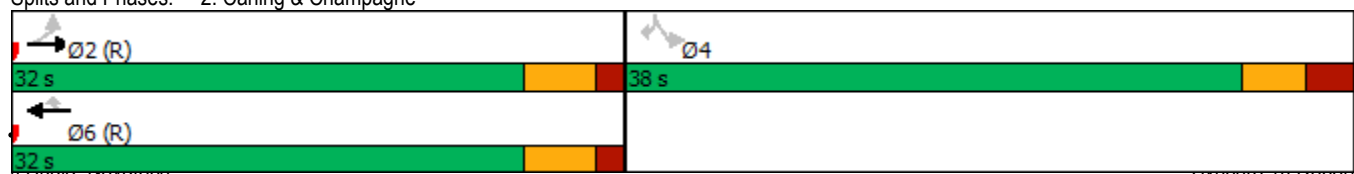
ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Carling & Champagne



0.0000, Nov 2010

Dynamic 10 Report

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑↑							
Traffic Volume (vph)	0	1000	0	0	1500	0	0	0	0	0	0	0
Future Volume (vph)	0	1000	0	0	1500	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	4764	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	4764	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	35					35	25		35	35		25
Confl. Bikes (#/hr)			11			10			13			34
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1111	0	0	1667	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1111	0	0	1667	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		35.0			35.0							
Total Split (%)		50.0%			50.0%							
Maximum Green (s)		29.9			29.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		54.0			54.0							
Actuated g/C Ratio		0.77			0.77							
v/c Ratio		0.44			0.45							
Control Delay		6.3			7.9							
Queue Delay		0.0			0.0							
Total Delay		6.3			7.9							
LOS		A			A							
Approach Delay		6.3			7.9							
Approach LOS		A			A							
Queue Length 50th (m)		0.1			0.0							
Queue Length 95th (m)		41.4			m78.0							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2531			3672							
Starvation Cap Reductn		0			0							
Spillback Cap Reductn		0			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.44			0.45							
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 6 (9%), Referenced to phase 2:EBT and 6:WBT, Start of Green												
Natural Cycle: 65												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.45												
Intersection Signal Delay: 7.3							Intersection LOS: A					
Intersection Capacity Utilization 34.8%							ICU Level of Service A					
Analysis Period (min) 15												
m Volume for 95th percentile queue is metered by upstream signal.												

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
35 s	35 s
 Ø5 (R)	
35 s	

Lane Group	Ø4
Total Split (s)	35.0
Total Split (%)	50%
Maximum Green (s)	28.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	116	540	370	340	1000	45	340	300	164	81	308	112
Future Volume (vph)	116	540	370	340	1000	45	340	300	164	81	308	112
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		40.0	75.0		0.0	75.0		0.0	0.0		0.0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	0.91	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.98		0.92	0.97	1.00		0.98	0.97		0.97	0.98	
Frt			0.850		0.994			0.947			0.960	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	3283	1483	1674	4705	0	1674	3061	0	1537	1626	0
Flt Permitted	0.950			0.950			0.104			0.462		
Satd. Flow (perm)	1581	3283	1365	1631	4705	0	180	3061	0	725	1626	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			299		5			95			13	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	53		34	34		53	60		55	55		60
Confl. Bikes (#/hr)			12			10			16			6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	129	600	411	378	1111	50	378	333	182	90	342	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	129	600	411	378	1161	0	378	515	0	90	466	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA		pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases			2				8			4		
Detector Phase	5	2	2	1	6		3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0		5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0	30.0	11.2	30.0		11.9	43.9		43.9	43.9	
Total Split (s)	30.0	41.0	41.0	30.0	41.0		24.0	69.0		45.0	45.0	
Total Split (%)	21.4%	29.3%	29.3%	21.4%	29.3%		17.1%	49.3%		32.1%	32.1%	
Maximum Green (s)	23.8	35.0	35.0	23.8	35.0		17.1	62.1		38.1	38.1	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3	2.3	2.5	2.3		3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0	6.0	6.2	6.0		6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag	Lag	Lead	Lag		Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max		None	Ped		Ped	Ped	
Walk Time (s)		7.0	7.0		7.0			7.0		7.0	7.0	
Flash Dont Walk (s)		17.0	17.0		17.0			30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20	20		20			20		20	20	
Act Effct Green (s)	16.5	35.0	35.0	23.8	42.3		62.1	62.1		38.1	38.1	
Actuated g/C Ratio	0.12	0.25	0.25	0.17	0.30		0.44	0.44		0.27	0.27	
v/c Ratio	0.68	0.73	0.73	1.33	0.81		1.44	0.37		0.46	1.03	
Control Delay	79.7	42.2	20.1	215.0	50.9		250.2	21.5		51.3	99.1	
Queue Delay	0.0	0.8	1.4	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	79.7	43.0	21.5	215.0	50.9		250.2	21.5		51.3	99.1	
LOS	E	D	C	F	D		F	C		D	F	
Approach Delay		39.4			91.2			118.3			91.3	
Approach LOS		D			F			F			F	
Queue Length 50th (m)	32.7	63.0	13.5	~124.7	100.1		~115.7	36.4		19.2	~124.5	
Queue Length 95th (m)	54.9	68.5	53.0	#181.9	#134.1		#173.7	49.3		35.9	#186.3	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0		40.0	75.0			75.0					
Base Capacity (vph)	273	820	565	284	1426		262	1410		197	451	
Starvation Cap Reductn	0	57	49	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.47	0.79	0.80	1.33	0.81		1.44	0.37		0.46	1.03	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 6 (4%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.44

Intersection Signal Delay: 82.8

Intersection LOS: F

Intersection Capacity Utilization 112.3%

ICU Level of Service H

Analysis Period (min) 15

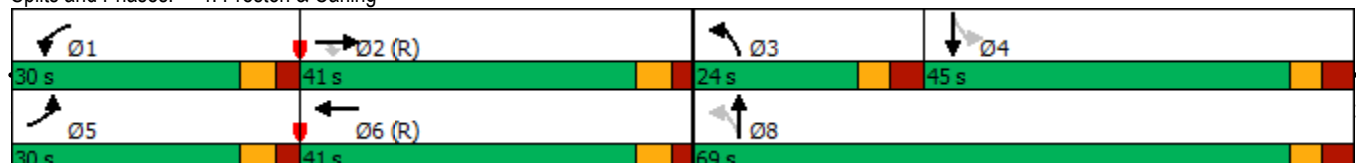
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

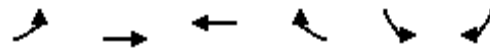
Queue shown is maximum after two cycles.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
PM Peak Hour

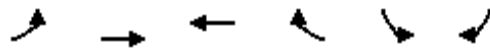
829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	230	630	1000	94	281	306
Future Volume (vph)	230	630	1000	94	281	306
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0			35.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	40.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor	0.98		0.98		0.98	0.86
Frt			0.987			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	4626	0	1674	1483
Flt Permitted	0.161				0.950	
Satd. Flow (perm)	279	3316	4626	0	1649	1279
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			16			262
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	65			65	13	81
Confl. Bikes (#/hr)				10		45
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	256	700	1111	104	312	340
Shared Lane Traffic (%)						
Lane Group Flow (vph)	256	700	1215	0	312	340
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2		1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (m)	6.1	30.5	30.5		6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0		0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0		0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8		6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA		Perm	Perm
Protected Phases	5	2	6			
Permitted Phases	2				4	4
Detector Phase	5	2	6		4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
Existing Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0		10.0	10.0
Minimum Split (s)	10.9	15.7	29.7		39.0	39.0
Total Split (s)	23.0	90.0	67.0		40.0	40.0
Total Split (%)	17.7%	69.2%	51.5%		30.8%	30.8%
Maximum Green (s)	17.1	84.3	61.3		34.0	34.0
Yellow Time (s)	3.7	3.7	3.7		3.3	3.3
All-Red Time (s)	2.2	2.0	2.0		2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7		6.0	6.0
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)			13.0		7.0	7.0
Flash Dont Walk (s)			11.0		26.0	26.0
Pedestrian Calls (#/hr)			20		20	20
Act Effct Green (s)	89.2	89.4	69.9		28.9	28.9
Actuated g/C Ratio	0.69	0.69	0.54		0.22	0.22
v/c Ratio	0.76	0.31	0.49		0.85	0.70
Control Delay	27.3	9.0	20.6		69.7	19.0
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	27.3	9.0	20.6		69.7	19.0
LOS	C	A	C		E	B
Approach Delay		13.9	20.6		43.2	
Approach LOS		B	C		D	
Queue Length 50th (m)	20.9	32.1	63.5		70.6	15.4
Queue Length 95th (m)	47.9	45.4	85.4		98.4	46.1
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	45.0					45.0
Base Capacity (vph)	374	2281	2496		431	527
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.68	0.31	0.49		0.72	0.65

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 110 (85%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 23.5

Intersection LOS: C

Intersection Capacity Utilization 77.9%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	40	32	30	118	24	81	380	37	17	410	49
Future Volume (vph)	25	40	32	30	118	24	81	380	37	17	410	49
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.98	0.84	0.96	0.99		0.94	0.99	
Frt		0.955				0.850		0.987			0.984	
Flt Protected		0.987			0.990		0.950			0.950		
Satd. Flow (prot)	0	1582	0	0	1745	1498	1674	1686	0	1674	1668	0
Flt Permitted		0.883			0.922		0.432			0.463		
Satd. Flow (perm)	0	1386	0	0	1593	1261	733	1686	0	765	1668	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		25				30		12			15	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		46	46		40	52		80	80		52
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	28	44	36	33	131	27	90	422	41	19	456	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	108	0	0	164	27	90	463	0	19	510	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	61.8	61.8		61.8	61.8	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.38			0.54	0.10	0.18	0.40		0.04	0.44	
Control Delay		28.8			40.6	11.4	2.8	4.4		4.8	7.6	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		28.8			40.6	11.4	2.8	4.4		4.8	7.6	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.8			36.4			4.1			7.5	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.5			23.8	0.0	3.5	28.8		0.8	30.5	
Queue Length 95th (m)		25.1			41.6	5.8	0.8	2.4		2.8	47.8	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		288			307	267	503	1161		525	1150	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.38			0.53	0.10	0.18	0.40		0.04	0.44	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.54

Intersection Signal Delay: 11.8

Intersection LOS: B

Intersection Capacity Utilization 66.2%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	450	21	6	470	15
Future Volume (vph)	4	2	10	0	0	0	8	450	21	6	470	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Frt		0.913						0.994			0.996	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1485	0	0	0	0	0	1709	0	0	1715	0
Flt Permitted		0.988						0.992			0.994	
Satd. Flow (perm)	0	1459	0	0	0	0	0	1696	0	0	1705	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11						6			4	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	46		47	47		46
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	11	0	0	0	9	500	23	7	522	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	17	0	0	0	0	0	532	0	0	546	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.37			0.38	
Control Delay		21.6						4.4			3.5	
Queue Delay		0.0						0.0			0.1	
Total Delay		21.6						4.4			3.6	
LOS		C						A			A	
Approach Delay		21.6						4.4			3.6	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						21.7			19.1	
Queue Length 95th (m)		6.0						44.5			30.9	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1426			1433	
Starvation Cap Reductn		0						0			109	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.07						0.37			0.41	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 50

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.38

Intersection Signal Delay: 4.2

Intersection LOS: A

Intersection Capacity Utilization 51.2%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	4	9	14	2	8	7	422	40	11	487	5
Future Volume (vph)	4	4	9	14	2	8	7	422	40	11	487	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt	0.925				0.955				0.989		0.999	
Flt Protected	0.989				0.971				0.999		0.999	
Satd. Flow (prot)	0	1596	0	0	1618	0	0	1709	0	0	1725	0
Flt Permitted	0.989				0.971				0.999		0.999	
Satd. Flow (perm)	0	1596	0	0	1618	0	0	1709	0	0	1725	0
Link Speed (k/h)	50				50				50		50	
Link Distance (m)	113.6				154.3				71.5		73.8	
Travel Time (s)	8.2				11.1				5.1		5.3	
Confl. Peds. (#/hr)							46		47		47	
Confl. Bikes (#/hr)									21		14	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%
Adj. Flow (vph)	4	4	10	16	2	9	8	469	44	12	541	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	18	0	0	27	0	0	521	0	0	559	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0				0.0				0.0		0.0	
Link Offset(m)	-2.0				-2.0				0.0		0.0	
Crosswalk Width(m)	5.0				5.0				5.0		5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control	Stop				Stop				Free		Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	43.9%											
Analysis Period (min)	15											
ICU Level of Service A												

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	17	18	9	452	483	27
Future Volume (vph)	17	18	9	452	483	27
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.931				0.993	
Flt Protected	0.976			0.999		
Satd. Flow (prot)	1586	0	0	3281	1717	0
Flt Permitted	0.976			0.999		
Satd. Flow (perm)	1586	0	0	3281	1717	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	19	20	10	502	537	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	39	0	0	512	567	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	38.8%			ICU Level of Service A		
Analysis Period (min)	15					

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	370	244	4	2	410	410	5	2	1	400	0	600
Future Volume (vph)	370	244	4	2	410	410	5	2	1	400	0	600
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.89		0.94		0.94			0.83	0.79
Frt		0.998				0.850		0.985				0.850
Flt Protected	0.950			0.950				0.968			0.950	
Satd. Flow (prot)	1642	1754	0	1674	1762	1498	0	1644	0	0	1674	1483
Flt Permitted	0.157			0.590				0.821			0.752	
Satd. Flow (perm)	271	1754	0	930	1762	1403	0	1343	0	0	1095	1166
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				175		1				87
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	73		65	65		73
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	411	271	4	2	456	456	6	2	1	444	0	667
Shared Lane Traffic (%)												
Lane Group Flow (vph)	411	275	0	2	456	456	0	9	0	0	444	667
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5
Switch Phase												

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
Existing Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		38.9	38.9	38.9		43.5			43.5	72.8
Actuated g/C Ratio	0.58	0.58		0.30	0.30	0.30		0.33			0.33	0.56
v/c Ratio	0.87	0.27		0.01	0.87	0.84		0.02			1.21	0.87
Control Delay	46.7	14.7		34.5	61.6	41.8		27.2			156.6	33.4
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	46.7	14.7		34.5	61.6	41.8		27.2			156.6	33.4
LOS	D	B		C	E	D		C			F	C
Approach Delay		33.8			51.7			27.3			82.7	
Approach LOS		C			D			C			F	
Queue Length 50th (m)	67.6	31.1		0.3	105.4	68.4		1.3			~127.8	84.5
Queue Length 95th (m)	#113.1	45.8		2.3	#163.6	#125.3		5.0			#186.2	#128.6
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	503	1010		278	527	542		450			366	796
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.82	0.27		0.01	0.87	0.84		0.02			1.21	0.84

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 59.8

Intersection LOS: E

Intersection Capacity Utilization 101.8%

ICU Level of Service G

Analysis Period (min) 15

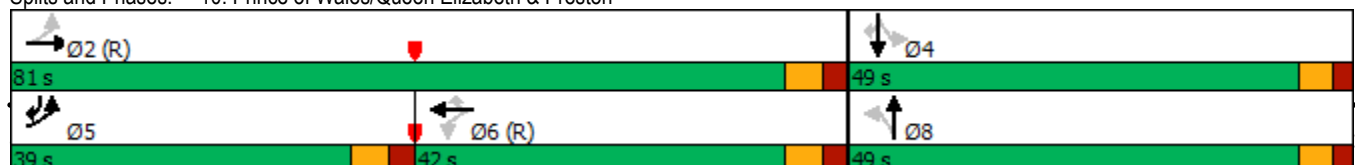
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



4: Preston & Carling
PM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	116	540	370	250	1000	45	230	300	164	81	288	112
Future Volume (vph)	116	540	370	250	1000	45	230	300	164	81	288	112
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		40.0	75.0		0.0	75.0		0.0	0.0		0.0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	0.91	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.98		0.92	0.97	1.00		0.98	0.97		0.97	0.98	
Frt			0.850		0.994			0.947			0.958	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	3283	1483	1674	4705	0	1674	3061	0	1537	1620	0
Flt Permitted	0.950			0.950			0.131			0.462		
Satd. Flow (perm)	1581	3283	1365	1631	4705	0	226	3061	0	725	1620	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			299		5			95			14	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	53		34	34		53	60		55	55		60
Confl. Bikes (#/hr)			12			10			16			6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	129	600	411	278	1111	50	256	333	182	90	320	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	129	600	411	278	1161	0	256	515	0	90	444	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA		pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases			2				8			4		
Detector Phase	5	2	2	1	6		3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0		5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0	30.0	11.2	30.0		11.9	43.9		43.9	43.9	
Total Split (s)	30.0	41.0	41.0	30.0	41.0		24.0	69.0		45.0	45.0	
Total Split (%)	21.4%	29.3%	29.3%	21.4%	29.3%		17.1%	49.3%		32.1%	32.1%	
Maximum Green (s)	23.8	35.0	35.0	23.8	35.0		17.1	62.1		38.1	38.1	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3	2.3	2.5	2.3		3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0	6.0	6.2	6.0		6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag	Lag	Lead	Lag		Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max		None	Ped		Ped	Ped	
Walk Time (s)		7.0	7.0		7.0			7.0		7.0	7.0	
Flash Dont Walk (s)		17.0	17.0		17.0			30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20	20		20			20		20	20	
Act Effct Green (s)	16.5	35.0	35.0	23.8	42.3		62.1	62.1		38.1	38.1	
Actuated g/C Ratio	0.12	0.25	0.25	0.17	0.30		0.44	0.44		0.27	0.27	
v/c Ratio	0.68	0.73	0.73	0.98	0.81		0.92	0.37		0.46	0.98	
Control Delay	79.7	42.2	20.1	105.6	50.9		69.7	21.5		51.3	87.5	
Queue Delay	0.0	0.8	1.4	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	79.7	43.0	21.5	105.6	50.9		69.7	21.5		51.3	87.5	
LOS	E	D	C	F	D		E	C		D	F	
Approach Delay		39.4			61.5			37.5			81.4	
Approach LOS		D			E			D			F	
Queue Length 50th (m)	32.7	63.0	13.5	71.4	100.1		45.4	36.4		19.2	110.2	
Queue Length 95th (m)	54.9	68.5	53.0	#123.1	#134.1		#92.7	49.3		35.9	#174.0	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0		40.0	75.0			75.0					
Base Capacity (vph)	273	820	565	284	1426		277	1410		197	451	
Starvation Cap Reductn	0	57	49	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.47	0.79	0.80	0.98	0.81		0.92	0.37		0.46	0.98	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 6 (4%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 53.0

Intersection LOS: D

Intersection Capacity Utilization 100.6%

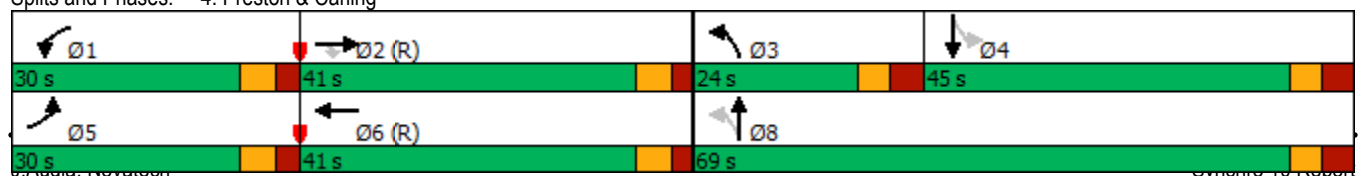
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: Preston & Carling



Signal Timing Diagram

Dynamic Traffic Report

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	370	244	4	2	410	410	5	2	1	330	0	600
Future Volume (vph)	370	244	4	2	410	410	5	2	1	330	0	600
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.89		0.94		0.93			0.83	0.79
Frt		0.998				0.850		0.985				0.850
Flt Protected	0.950			0.950				0.968			0.950	
Satd. Flow (prot)	1642	1754	0	1674	1762	1498	0	1644	0	0	1674	1483
Flt Permitted	0.157			0.590				0.836			0.752	
Satd. Flow (perm)	271	1754	0	930	1762	1403	0	1347	0	0	1095	1166
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				175		1				87
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	73		65	65		73
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	411	271	4	2	456	456	6	2	1	367	0	667
Shared Lane Traffic (%)												
Lane Group Flow (vph)	411	275	0	2	456	456	0	9	0	0	367	667
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5
Switch Phase												

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
Existing Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		38.9	38.9	38.9		43.5			43.5	72.8
Actuated g/C Ratio	0.58	0.58		0.30	0.30	0.30		0.33			0.33	0.56
v/c Ratio	0.87	0.27		0.01	0.87	0.84		0.02			1.00	0.87
Control Delay	46.7	14.7		34.5	61.6	41.8		27.2			91.2	33.4
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	46.7	14.7		34.5	61.6	41.8		27.2			91.2	33.4
LOS	D	B		C	E	D		C			F	C
Approach Delay		33.8			51.7			27.3			53.9	
Approach LOS		C			D			C			D	
Queue Length 50th (m)	67.6	31.1		0.3	105.4	68.4		1.3			~86.7	84.5
Queue Length 95th (m)	#113.1	45.8		2.3	#163.6	#125.3		5.0			#145.5	#128.6
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	503	1010		278	527	542		451			366	796
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.82	0.27		0.01	0.87	0.84		0.02			1.00	0.84

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 47.8

Intersection LOS: D

Intersection Capacity Utilization 101.8%

ICU Level of Service G

Analysis Period (min) 15

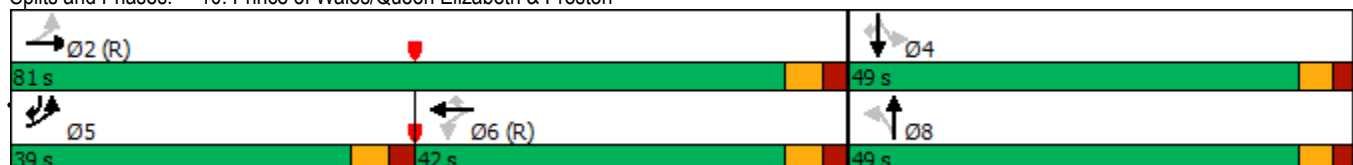
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



APPENDIX K

Background Synchro Analysis

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	31	702	118	13	142	553	127	45	9	54	133	24
Future Volume (vph)	31	702	118	13	142	553	127	45	9	54	133	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.93		0.98		0.90	0.97		0.96	0.97	0.99
Frt			0.850				0.850			0.850		0.974
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1642	3283	1483	0	1659	3161	1483	1658	1745	1483	1674	1689
Flt Permitted	0.950				0.385			0.738			0.752	
Satd. Flow (perm)	1591	3283	1385	0	661	3161	1339	1247	1745	1418	1282	1689
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			109				127			78		
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	30		20		20		30	30		30	30	
Confl. Bikes (#/hr)			5				17					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	3%	2%	1%	2%	7%	2%	2%	2%	2%	1%	2%
Adj. Flow (vph)	31	702	118	13	142	553	127	45	9	54	133	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	702	118	0	155	553	127	45	9	54	133	29
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	5
Future Volume (vph)	5
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	3
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	5
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	12.0	78.0	78.0	66.0	66.0	66.0	66.0	42.0	42.0	42.0	42.0	42.0
Total Split (%)	10.0%	65.0%	65.0%	55.0%	55.0%	55.0%	55.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Maximum Green (s)	6.8	71.6	71.6	59.6	59.6	59.6	59.6	34.9	34.9	34.9	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	7.4	84.8	84.8		76.6	76.6	76.6	21.7	21.7	21.7	21.7	21.7
Actuated g/C Ratio	0.06	0.71	0.71		0.64	0.64	0.64	0.18	0.18	0.18	0.18	0.18
v/c Ratio	0.31	0.30	0.12		0.37	0.27	0.14	0.20	0.03	0.17	0.57	0.09
Control Delay	61.3	8.3	2.3		6.1	3.4	0.6	39.5	34.0	4.5	52.5	31.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.3	8.3	2.3		6.1	3.4	0.6	39.5	34.0	4.5	52.5	31.5
LOS	E	A	A		A	A	A	D	C	A	D	C
Approach Delay		9.4				3.4			21.5			48.7
Approach LOS		A				A			C			D
Queue Length 50th (m)	6.5	23.1	0.5		1.9	3.5	0.2	8.8	1.7	0.0	27.8	4.6
Queue Length 95th (m)	15.6	47.3	7.1		12.9	17.8	0.0	16.2	5.1	4.8	40.2	10.7
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	106	2318	1010		422	2018	900	362	507	467	372	494
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.30	0.12		0.37	0.27	0.14	0.12	0.02	0.12	0.36	0.06

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 10.8

Intersection LOS: B

Intersection Capacity Utilization 82.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

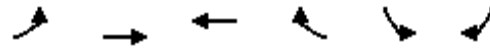




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
AM Peak Hour

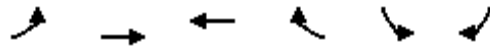
829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	112	843	797	168	89	78
Future Volume (vph)	112	843	797	168	89	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.93			0.71	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	3252	1414	1658	1498
Flt Permitted	0.329				0.950	
Satd. Flow (perm)	540	3283	3252	1009	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				140		78
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	90			90	7	9
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	112	843	797	168	89	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	843	797	168	89	78
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	78.0	78.0	78.0	78.0	42.0	42.0
Total Split (%)	65.0%	65.0%	65.0%	65.0%	35.0%	35.0%
Maximum Green (s)	72.7	72.7	72.7	72.7	36.1	36.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	85.6	85.6	85.6	85.6	23.2	23.2
Actuated g/C Ratio	0.71	0.71	0.71	0.71	0.19	0.19
v/c Ratio	0.29	0.36	0.34	0.22	0.28	0.23
Control Delay	7.9	6.2	2.6	1.8	40.4	9.0
Queue Delay	0.0	0.0	0.1	0.0	0.0	0.0
Total Delay	7.9	6.2	2.7	1.8	40.4	9.0
LOS	A	A	A	A	D	A
Approach Delay		6.4	2.6		25.7	
Approach LOS		A	A		C	
Queue Length 50th (m)	12.8	53.5	21.2	2.7	15.0	0.0
Queue Length 95th (m)	10.5	29.1	25.4	9.0	27.7	10.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	385	2342	2319	760	494	494
Starvation Cap Reductn	0	0	524	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.36	0.44	0.22	0.18	0.16

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.36

Intersection Signal Delay: 6.2

Intersection LOS: A

Intersection Capacity Utilization 58.4%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Future Volume (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3283	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3283	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40		14	14		40	18		20	20		18
Confl. Bikes (#/hr)			7			25						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.36			0.39							
Control Delay		4.4			3.3							
Queue Delay		0.0			0.1							
Total Delay		4.5			3.4							
LOS		A			A							
Approach Delay		4.5			3.4							
Approach LOS		A			A							
Queue Length 50th (m)		26.9			25.6							
Queue Length 95th (m)		31.4			m29.4							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			2615							
Starvation Cap Reductn		281			406							
Spillback Cap Reductn		80			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.40			0.46							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 57 (48%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 3.9

Intersection LOS: A

Intersection Capacity Utilization 33.8%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
34 s	36 s
 Ø5 (R)	
34 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	152	584	233	143	634	99	284	488	284	120	349	128
Future Volume (vph)	152	584	233	143	634	99	284	488	284	120	349	128
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.94	0.97		0.98		0.85	0.98	0.99		1.00	0.98	
Frt		0.957				0.850		0.945			0.960	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3050	0	1658	3252	1375	1674	3052	0	1510	1527	0
Flt Permitted	0.950			0.950			0.157			0.359		
Satd. Flow (perm)	1506	3050	0	1625	3252	1167	272	3052	0	569	1527	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		48				155		135			16	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	90		41	41		90	60		10	10		60
Confl. Bikes (#/hr)			22			10			36			5
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	152	584	233	143	634	99	284	488	284	120	349	128
Shared Lane Traffic (%)												
Lane Group Flow (vph)	152	817	0	143	634	99	284	772	0	120	477	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	20.0	37.0		18.0	35.0	35.0	21.0	65.0		44.0	44.0	
Total Split (%)	16.7%	30.8%		15.0%	29.2%	29.2%	17.5%	54.2%		36.7%	36.7%	
Maximum Green (s)	13.8	31.0		11.8	29.0	29.0	14.1	58.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	13.4	31.0		11.8	29.4	29.4	58.1	58.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.26		0.10	0.24	0.24	0.48	0.48		0.31	0.31	
v/c Ratio	0.86	0.99		0.88	0.80	0.25	0.96	0.50		0.69	0.99	
Control Delay	94.3	61.8		78.6	29.6	4.9	56.9	11.2		58.3	78.4	
Queue Delay	0.0	0.8		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	94.3	62.6		78.6	29.6	4.9	56.9	11.2		58.3	78.4	
LOS	F	E		E	C	A	E	B		E	E	
Approach Delay		67.5			34.8			23.5			74.4	
Approach LOS		E			C			C			E	
Queue Length 50th (m)	27.1	45.1		27.6	76.3	3.4	36.8	47.7		22.8	100.0	
Queue Length 95th (m)	#62.9	#122.5		m32.6	m77.7	m4.6	m#70.4	49.1		#49.2	#163.3	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	183	824		163	796	403	296	1547		175	483	
Starvation Cap Reductn	0	3		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.83	1.00		0.88	0.80	0.25	0.96	0.50		0.69	0.99	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 47.2

Intersection LOS: D

Intersection Capacity Utilization 103.4%

ICU Level of Service G

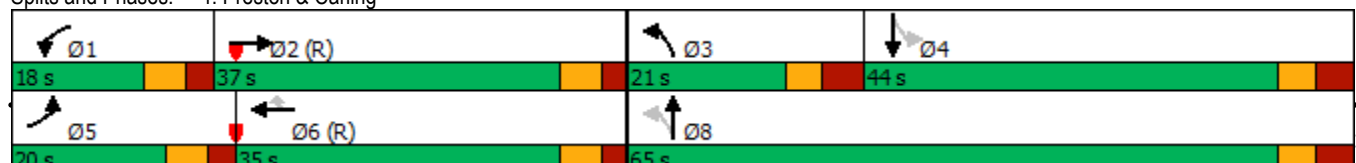
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

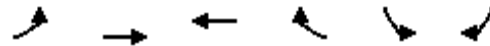
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

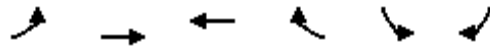
829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	398	683	701	215	240	168
Future Volume (vph)	398	683	701	215	240	168
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.93			0.80	0.98	0.80
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	1728	1498	1674	1427
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1564	3252	1728	1193	1649	1145
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				71		168
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	14	85
Confl. Bikes (#/hr)				16		23
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	398	683	701	215	240	168
Shared Lane Traffic (%)						
Lane Group Flow (vph)	398	683	701	215	240	168
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	26.0	81.0	55.0	55.0	39.0	39.0
Total Split (%)	21.7%	67.5%	45.8%	45.8%	32.5%	32.5%
Maximum Green (s)	20.1	75.3	49.3	49.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	28.5	83.7	49.3	49.3	24.6	24.6
Actuated g/C Ratio	0.24	0.70	0.41	0.41	0.20	0.20
v/c Ratio	1.01	0.30	0.99	0.40	0.71	0.46
Control Delay	77.4	7.4	66.8	18.9	55.0	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.4	7.4	66.8	18.9	55.0	9.2
LOS	E	A	E	B	E	A
Approach Delay		33.2	55.6		36.2	
Approach LOS		C	E		D	
Queue Length 50th (m)	85.4	28.6	148.5	21.1	49.8	0.0
Queue Length 95th (m)	m#144.5	m53.5	#221.6	40.0	68.0	15.6
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	396	2267	709	531	453	436
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.01	0.30	0.99	0.40	0.53	0.39

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 28 (23%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 42.2

Intersection LOS: D

Intersection Capacity Utilization 103.4%

ICU Level of Service G

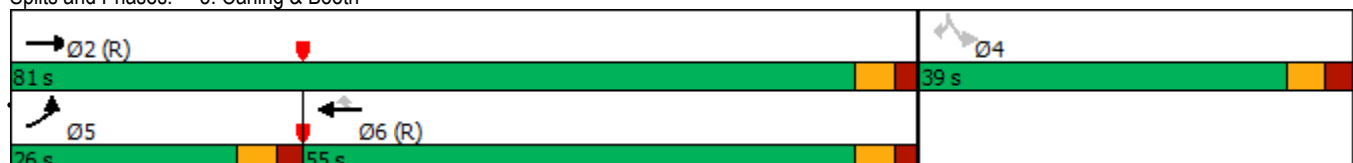
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	43	56	26	36	55	12	28	645	55	18	453	38
Future Volume (vph)	43	56	26	36	55	12	28	645	55	18	453	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.97	0.89	0.97	0.99		0.98	0.99	
Frt		0.972				0.850		0.988			0.988	
Flt Protected		0.983			0.981		0.950			0.950		
Satd. Flow (prot)	0	1558	0	0	1572	1498	1537	1693	0	1537	1648	0
Flt Permitted		0.865			0.853		0.437			0.303		
Satd. Flow (perm)	0	1336	0	0	1327	1339	684	1693	0	479	1648	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15				34		11			11	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		40	40		36	50		55	55		50
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	43	56	26	36	55	12	28	645	55	18	453	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	125	0	0	91	12	28	700	0	18	491	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.42			0.32	0.04	0.06	0.64		0.06	0.46	
Control Delay		28.9			30.3	2.8	5.5	9.6		5.8	8.6	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			30.3	2.8	5.5	9.7		5.8	8.6	
LOS		C			C	A	A	A		A	A	
Approach Delay		28.9			27.1			9.6			8.5	
Approach LOS		C			C			A			A	
Queue Length 50th (m)		13.3			10.9	0.0	1.3	50.2		0.8	29.3	
Queue Length 95th (m)		27.3			22.3	1.2	m2.6	40.8		3.0	47.5	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		302			288	317	442	1100		310	1071	
Starvation Cap Reductn		0			0	0	0	37		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.41			0.32	0.04	0.06	0.66		0.06	0.46	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 12.1

Intersection LOS: B

Intersection Capacity Utilization 81.9%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	680	46	10	547	5
Future Volume (vph)	1	0	3	0	0	0	8	680	46	10	547	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.992			0.999	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1704	0	0	1617	0
Flt Permitted		0.988						0.995			0.988	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1696	0	0	1599	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29						9			1	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	35		45	45		35
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	8	680	46	10	547	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	734	0	0	562	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.50			0.40	
Control Delay		0.0						5.4			2.5	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.4			2.5	
LOS		A						A			A	
Approach Delay								5.4			2.5	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						77.6			17.4	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1480			1394	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.50			0.40	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.50

Intersection Signal Delay: 4.1

Intersection LOS: A

Intersection Capacity Utilization 64.5%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	1	22	20	5	26	12	642	90	34	540	15
Future Volume (vph)	29	1	22	20	5	26	12	642	90	34	540	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Fr't		0.943			0.931			0.984			0.997	
Flt Protected		0.973			0.981			0.999			0.997	
Satd. Flow (prot)	0	1601	0	0	1594	0	0	1701	0	0	1689	0
Flt Permitted		0.973			0.981			0.999			0.997	
Satd. Flow (perm)	0	1601	0	0	1594	0	0	1701	0	0	1689	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							28		45	45		28
Confl. Bikes (#/hr)									21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	29	1	22	20	5	26	12	642	90	34	540	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	52	0	0	51	0	0	744	0	0	589	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	62.8%											
ICU Level of Service	B											
Analysis Period (min)	15											

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	41	41	33	703	550	30
Future Volume (vph)	41	41	33	703	550	30
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.932				0.993	
Flt Protected	0.976			0.998		
Satd. Flow (prot)	1587	0	0	3278	1686	0
Flt Permitted	0.976			0.998		
Satd. Flow (perm)	1587	0	0	3278	1686	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	41	41	33	703	550	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	82	0	0	736	580	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 57.9%	ICU Level of Service B					
Analysis Period (min) 15						

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	677	281	2	2	223	316	1	4	3	276	4	414
Future Volume (vph)	677	281	2	2	223	316	1	4	3	276	4	414
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.92
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1668	1469
Flt Permitted	0.503			0.586				0.969			0.724	
Satd. Flow (perm)	863	1760	0	1009	1762	1462	0	1176	0	0	1193	1347
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				315		3				414
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	25		24	24		25
Confl. Bikes (#/hr)			3						1			2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	677	281	2	2	223	316	1	4	3	276	4	414
Shared Lane Traffic (%)												
Lane Group Flow (vph)	677	283	0	2	223	316	0	8	0	0	280	414
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		48.2	48.2	48.2		24.5			24.5	53.5
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.85	0.23		0.00	0.32	0.41		0.03			1.15	0.48
Control Delay	21.5	7.0		23.0	26.9	4.4		32.1			120.0	3.8
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	21.5	7.0		23.0	26.9	4.4		32.1			120.0	3.8
LOS	C	A		C	C	A		C			F	A
Approach Delay		17.2			13.8			32.1			50.7	
Approach LOS		B			B			C			D	
Queue Length 50th (m)	67.5	19.7		0.3	33.8	0.1		0.9			~68.9	12.0
Queue Length 95th (m)	#99.9	29.5		1.9	52.2	16.2		4.8			m#74.0	m12.9
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	810	1230		404	706	775		242			243	880
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.84	0.23		0.00	0.32	0.41		0.03			1.15	0.47

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 27.0

Intersection LOS: C

Intersection Capacity Utilization 99.0%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

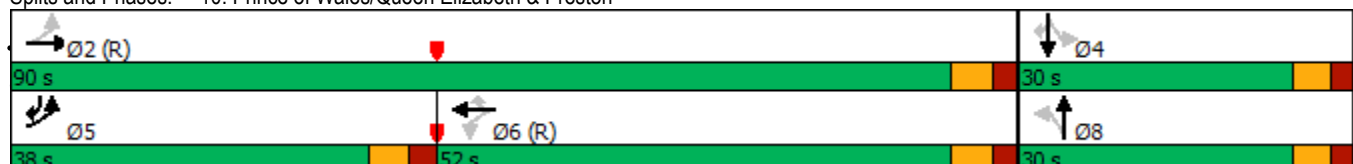
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	31	702	118	13	142	553	127	45	9	54	133	24
Future Volume (vph)	31	702	118	13	142	553	127	45	9	54	133	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.93		0.98		0.90	0.97		0.96	0.97	0.99
Frt			0.850				0.850			0.850		0.974
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1642	3283	1483	0	1659	3161	1483	1658	1745	1483	1674	1689
Flt Permitted	0.950				0.385			0.738			0.752	
Satd. Flow (perm)	1591	3283	1385	0	661	3161	1339	1247	1745	1418	1282	1689
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			109				127			78		
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	30		20		20		30	30		30	30	
Confl. Bikes (#/hr)			5				17					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	3%	2%	1%	2%	7%	2%	2%	2%	2%	1%	2%
Adj. Flow (vph)	31	702	118	13	142	553	127	45	9	54	133	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	702	118	0	155	553	127	45	9	54	133	29
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	5
Future Volume (vph)	5
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	3
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	5
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	12.0	78.0	78.0	66.0	66.0	66.0	66.0	42.0	42.0	42.0	42.0	42.0
Total Split (%)	10.0%	65.0%	65.0%	55.0%	55.0%	55.0%	55.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Maximum Green (s)	6.8	71.6	71.6	59.6	59.6	59.6	59.6	34.9	34.9	34.9	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	7.4	84.8	84.8		76.6	76.6	76.6	21.7	21.7	21.7	21.7	21.7
Actuated g/C Ratio	0.06	0.71	0.71		0.64	0.64	0.64	0.18	0.18	0.18	0.18	0.18
v/c Ratio	0.31	0.30	0.12		0.37	0.27	0.14	0.20	0.03	0.17	0.57	0.09
Control Delay	61.3	8.3	2.3		6.1	3.4	0.6	39.5	34.0	4.5	52.5	31.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.3	8.3	2.3		6.1	3.4	0.6	39.5	34.0	4.5	52.5	31.5
LOS	E	A	A		A	A	A	D	C	A	D	C
Approach Delay		9.4				3.4			21.5			48.7
Approach LOS		A				A			C			D
Queue Length 50th (m)	6.5	23.1	0.5		1.9	3.5	0.2	8.8	1.7	0.0	27.8	4.6
Queue Length 95th (m)	15.6	47.3	7.1		12.9	17.8	0.0	16.2	5.1	4.8	40.2	10.7
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	106	2318	1010		422	2018	900	362	507	467	372	494
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.30	0.12		0.37	0.27	0.14	0.12	0.02	0.12	0.36	0.06

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 10.8

Intersection LOS: B

Intersection Capacity Utilization 82.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

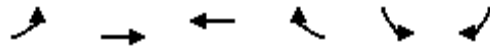




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
AM Peak Hour

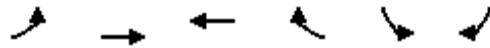
829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	112	843	797	168	89	78
Future Volume (vph)	112	843	797	168	89	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.93			0.71	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	3252	1414	1658	1498
Flt Permitted	0.329				0.950	
Satd. Flow (perm)	540	3283	3252	1009	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				140		78
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	90			90	7	9
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	112	843	797	168	89	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	843	797	168	89	78
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	78.0	78.0	78.0	78.0	42.0	42.0
Total Split (%)	65.0%	65.0%	65.0%	65.0%	35.0%	35.0%
Maximum Green (s)	72.7	72.7	72.7	72.7	36.1	36.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	85.6	85.6	85.6	85.6	23.2	23.2
Actuated g/C Ratio	0.71	0.71	0.71	0.71	0.19	0.19
v/c Ratio	0.29	0.36	0.34	0.22	0.28	0.23
Control Delay	7.9	6.2	2.6	1.8	40.4	9.0
Queue Delay	0.0	0.0	0.1	0.0	0.0	0.0
Total Delay	7.9	6.2	2.7	1.8	40.4	9.0
LOS	A	A	A	A	D	A
Approach Delay		6.4	2.6		25.7	
Approach LOS		A	A		C	
Queue Length 50th (m)	12.8	53.5	21.2	2.7	15.0	0.0
Queue Length 95th (m)	10.5	29.1	25.4	9.0	27.7	10.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	385	2342	2319	760	494	494
Starvation Cap Reductn	0	0	524	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.36	0.44	0.22	0.18	0.16

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.36

Intersection Signal Delay: 6.2

Intersection LOS: A

Intersection Capacity Utilization 58.4%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Future Volume (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3283	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3283	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40		14	14		40	18		20	20		18
Confl. Bikes (#/hr)			7			25						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	929	0	0	1013	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.36			0.39							
Control Delay		4.4			3.3							
Queue Delay		0.0			0.1							
Total Delay		4.5			3.4							
LOS		A			A							
Approach Delay		4.5			3.4							
Approach LOS		A			A							
Queue Length 50th (m)		26.9			25.6							
Queue Length 95th (m)		31.4			m29.4							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			2615							
Starvation Cap Reductn		281			406							
Spillback Cap Reductn		80			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.40			0.46							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 57 (48%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 3.9

Intersection LOS: A

Intersection Capacity Utilization 33.8%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
34 s	36 s
 Ø5 (R)	
34 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	152	584	233	143	634	99	284	488	284	120	349	128
Future Volume (vph)	152	584	233	143	634	99	284	488	284	120	349	128
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.94	0.97		0.98		0.85	0.98	0.99		1.00	0.98	
Frt		0.957				0.850		0.945			0.960	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3050	0	1658	3252	1375	1674	3052	0	1510	1527	0
Flt Permitted	0.950			0.950			0.157			0.359		
Satd. Flow (perm)	1506	3050	0	1625	3252	1167	272	3052	0	569	1527	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		48				155		135			16	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	90		41	41		90	60		10	10		60
Confl. Bikes (#/hr)			22			10			36			5
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	152	584	233	143	634	99	284	488	284	120	349	128
Shared Lane Traffic (%)												
Lane Group Flow (vph)	152	817	0	143	634	99	284	772	0	120	477	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	20.0	37.0		18.0	35.0	35.0	21.0	65.0		44.0	44.0	
Total Split (%)	16.7%	30.8%		15.0%	29.2%	29.2%	17.5%	54.2%		36.7%	36.7%	
Maximum Green (s)	13.8	31.0		11.8	29.0	29.0	14.1	58.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	13.4	31.0		11.8	29.4	29.4	58.1	58.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.26		0.10	0.24	0.24	0.48	0.48		0.31	0.31	
v/c Ratio	0.86	0.99		0.88	0.80	0.25	0.96	0.50		0.69	0.99	
Control Delay	94.3	61.8		78.6	29.6	4.9	56.9	11.2		58.3	78.4	
Queue Delay	0.0	0.8		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	94.3	62.6		78.6	29.6	4.9	56.9	11.2		58.3	78.4	
LOS	F	E		E	C	A	E	B		E	E	
Approach Delay		67.5			34.8			23.5			74.4	
Approach LOS		E			C			C			E	
Queue Length 50th (m)	27.1	45.1		27.6	76.3	3.4	36.8	47.7		22.8	100.0	
Queue Length 95th (m)	#62.9	#122.5		m32.6	m77.7	m4.6	m#70.4	49.1		#49.2	#163.3	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	183	824		163	796	403	296	1547		175	483	
Starvation Cap Reductn	0	3		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.83	1.00		0.88	0.80	0.25	0.96	0.50		0.69	0.99	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 47.2

Intersection LOS: D

Intersection Capacity Utilization 103.4%

ICU Level of Service G

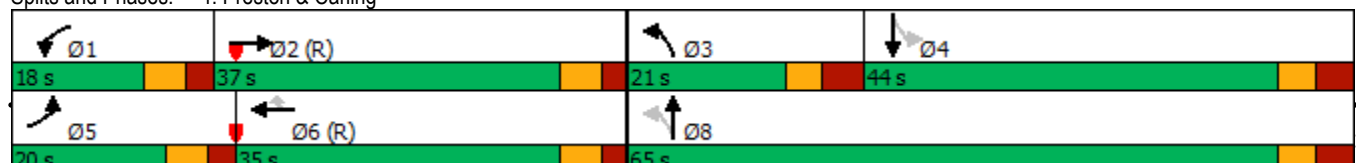
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

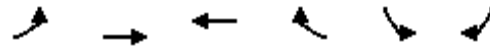
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

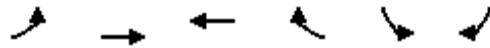
829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	388	683	701	215	240	168
Future Volume (vph)	388	683	701	215	240	168
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.93			0.80	0.98	0.80
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	1728	1498	1674	1427
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1564	3252	1728	1193	1649	1145
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				71		168
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	14	85
Confl. Bikes (#/hr)				16		23
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	388	683	701	215	240	168
Shared Lane Traffic (%)						
Lane Group Flow (vph)	388	683	701	215	240	168
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	26.0	81.0	55.0	55.0	39.0	39.0
Total Split (%)	21.7%	67.5%	45.8%	45.8%	32.5%	32.5%
Maximum Green (s)	20.1	75.3	49.3	49.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	28.5	83.7	49.3	49.3	24.6	24.6
Actuated g/C Ratio	0.24	0.70	0.41	0.41	0.20	0.20
v/c Ratio	0.98	0.30	0.99	0.40	0.71	0.46
Control Delay	71.2	7.4	66.8	18.9	55.0	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.2	7.4	66.8	18.9	55.0	9.2
LOS	E	A	E	B	E	A
Approach Delay		30.5	55.6		36.2	
Approach LOS		C	E		D	
Queue Length 50th (m)	83.1	28.6	148.5	21.1	49.8	0.0
Queue Length 95th (m)	m#138.5	m53.6	#221.6	40.0	68.0	15.6
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	396	2267	709	531	453	436
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.98	0.30	0.99	0.40	0.53	0.39

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 28 (23%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 41.1

Intersection LOS: D

Intersection Capacity Utilization 102.8%

ICU Level of Service G

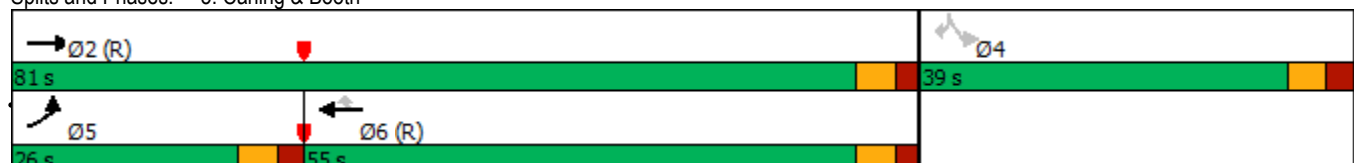
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	43	56	26	36	55	12	28	645	55	18	453	38
Future Volume (vph)	43	56	26	36	55	12	28	645	55	18	453	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.97	0.89	0.97	0.99		0.98	0.99	
Frt		0.972				0.850		0.988			0.988	
Flt Protected		0.983			0.981		0.950			0.950		
Satd. Flow (prot)	0	1558	0	0	1572	1498	1537	1693	0	1537	1648	0
Flt Permitted		0.865			0.853		0.437			0.303		
Satd. Flow (perm)	0	1336	0	0	1327	1339	684	1693	0	479	1648	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15				34		11			11	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		40	40		36	50		55	55		50
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	43	56	26	36	55	12	28	645	55	18	453	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	125	0	0	91	12	28	700	0	18	491	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.42			0.32	0.04	0.06	0.64		0.06	0.46	
Control Delay		28.9			30.3	2.8	5.5	9.6		5.8	8.6	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			30.3	2.8	5.5	9.7		5.8	8.6	
LOS		C			C	A	A	A		A	A	
Approach Delay		28.9			27.1			9.6			8.5	
Approach LOS		C			C			A			A	
Queue Length 50th (m)		13.3			10.9	0.0	1.3	50.2		0.8	29.3	
Queue Length 95th (m)		27.3			22.3	1.2	m2.6	40.8		3.0	47.5	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		302			288	317	442	1100		310	1071	
Starvation Cap Reductn		0			0	0	0	37		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.41			0.32	0.04	0.06	0.66		0.06	0.46	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 12.1

Intersection LOS: B

Intersection Capacity Utilization 81.9%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	680	46	10	547	5
Future Volume (vph)	1	0	3	0	0	0	8	680	46	10	547	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.992			0.999	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1704	0	0	1617	0
Flt Permitted		0.988						0.995			0.988	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1696	0	0	1599	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29						9			1	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	35		45	45		35
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	8	680	46	10	547	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	734	0	0	562	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.50			0.40	
Control Delay		0.0						5.4			2.5	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.4			2.5	
LOS		A						A			A	
Approach Delay								5.4			2.5	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						77.6			17.4	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1480			1394	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.50			0.40	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.50

Intersection Signal Delay: 4.1

Intersection LOS: A

Intersection Capacity Utilization 64.5%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	1	22	20	5	26	12	642	90	34	540	15
Future Volume (vph)	29	1	22	20	5	26	12	642	90	34	540	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.943			0.931			0.984			0.997	
Flt Protected		0.973			0.981			0.999			0.997	
Satd. Flow (prot)	0	1601	0	0	1594	0	0	1701	0	0	1689	0
Flt Permitted		0.973			0.981			0.999			0.997	
Satd. Flow (perm)	0	1601	0	0	1594	0	0	1701	0	0	1689	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							28		45	45		28
Confl. Bikes (#/hr)									21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	29	1	22	20	5	26	12	642	90	34	540	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	52	0	0	51	0	0	744	0	0	589	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization 62.8%												
ICU Level of Service B												
Analysis Period (min) 15												

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	41	41	33	703	550	30
Future Volume (vph)	41	41	33	703	550	30
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.932				0.993	
Flt Protected	0.976			0.998		
Satd. Flow (prot)	1587	0	0	3278	1686	0
Flt Permitted	0.976			0.998		
Satd. Flow (perm)	1587	0	0	3278	1686	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	41	41	33	703	550	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	82	0	0	736	580	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	57.9%			ICU Level of Service B		
Analysis Period (min)	15					

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	677	281	2	2	223	316	1	4	3	236	4	414
Future Volume (vph)	677	281	2	2	223	316	1	4	3	236	4	414
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.92
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1666	1469
Flt Permitted	0.503			0.586				0.972			0.724	
Satd. Flow (perm)	863	1760	0	1009	1762	1462	0	1179	0	0	1192	1347
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				315		3				414
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	25		24	24		25
Confl. Bikes (#/hr)			3						1			2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	677	281	2	2	223	316	1	4	3	236	4	414
Shared Lane Traffic (%)												
Lane Group Flow (vph)	677	283	0	2	223	316	0	8	0	0	240	414
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		48.2	48.2	48.2		24.5			24.5	53.5
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.85	0.23		0.00	0.32	0.41		0.03			0.99	0.48
Control Delay	21.5	7.0		23.0	26.9	4.4		32.1			70.2	3.8
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	21.5	7.0		23.0	26.9	4.4		32.1			70.2	3.8
LOS	C	A		C	C	A		C			E	A
Approach Delay		17.2			13.8			32.1			28.2	
Approach LOS		B			B			C			C	
Queue Length 50th (m)	67.5	19.7		0.3	33.8	0.1		0.9			46.6	12.0
Queue Length 95th (m)	#99.9	29.5		1.9	52.2	16.2		4.8			m#51.5	m12.9
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	810	1230		404	706	775		243			243	880
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.84	0.23		0.00	0.32	0.41		0.03			0.99	0.47

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 19.7

Intersection LOS: B

Intersection Capacity Utilization 96.7%

ICU Level of Service F

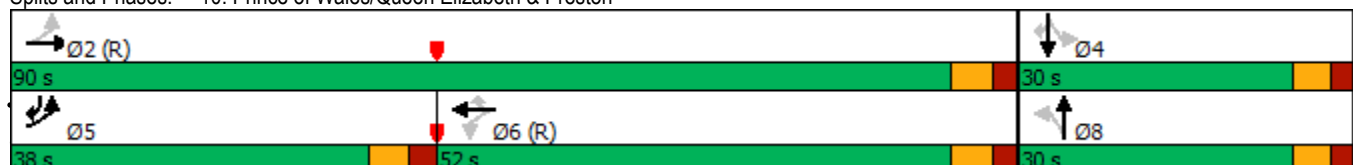
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	65	717	50	13	60	1366	155	125	25	150	180	10
Future Volume (vph)	65	717	50	13	60	1366	155	125	25	150	180	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99		0.92		0.98		0.91	0.99		0.98	0.99	0.99
Frt			0.850				0.850			0.850		0.938
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1674	3252	1483	0	1661	3316	1498	1658	1745	1483	1674	1624
Flt Permitted	0.950				0.379			0.746			0.741	
Satd. Flow (perm)	1656	3252	1371	0	650	3316	1357	1289	1745	1453	1295	1624
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			46				155			150		7
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	28		20		20		28	8		7	7	
Confl. Bikes (#/hr)			9				5					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	2%	1%	2%	2%	1%	2%	2%	2%	1%	2%
Adj. Flow (vph)	65	717	50	13	60	1366	155	125	25	150	180	10
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	717	50	0	73	1366	155	125	25	150	180	17
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4



Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	7
Future Volume (vph)	7
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	8
Confl. Bikes (#/hr)	8
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	7
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	18.0	99.0	99.0	81.0	81.0	81.0	81.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	12.9%	70.7%	70.7%	57.9%	57.9%	57.9%	57.9%	29.3%	29.3%	29.3%	29.3%	29.3%
Maximum Green (s)	12.8	92.6	92.6	74.6	74.6	74.6	74.6	33.9	33.9	33.9	33.9	33.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	10.4	99.3	99.3		86.1	86.1	86.1	27.2	27.2	27.2	27.2	27.2
Actuated g/C Ratio	0.07	0.71	0.71		0.62	0.62	0.62	0.19	0.19	0.19	0.19	0.19
v/c Ratio	0.52	0.31	0.05		0.18	0.67	0.17	0.50	0.07	0.37	0.72	0.05
Control Delay	77.0	8.8	2.6		5.5	8.0	0.5	55.8	43.0	9.1	67.7	29.9
Queue Delay	0.0	0.0	0.0		0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.0	8.8	2.6		5.5	8.1	0.5	55.8	43.0	9.1	67.7	29.9
LOS	E	A	A		A	A	A	E	D	A	E	C
Approach Delay		13.7				7.3			31.4			64.5
Approach LOS		B				A			C			E
Queue Length 50th (m)	16.2	38.1	0.3		2.4	46.0	0.0	27.2	5.0	0.0	41.0	2.0
Queue Length 95th (m)	30.2	48.1	4.3		7.4	87.2	0.3	45.1	12.1	15.9	64.4	7.8
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	153	2306	985		399	2038	893	312	422	465	313	398
Starvation Cap Reductn	0	0	0		0	97	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.31	0.05		0.18	0.70	0.17	0.40	0.06	0.32	0.58	0.04

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 15 (11%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 15.4

Intersection LOS: B

Intersection Capacity Utilization 79.2%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

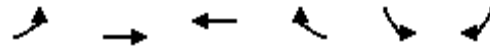




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
PM Peak Hour

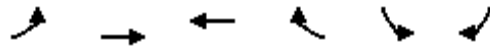
829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	80	891	1339	72	146	209
Future Volume (vph)	80	891	1339	72	146	209
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor				0.74	0.99	0.97
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	3316	1498	1674	1498
Flt Permitted	0.169				0.950	
Satd. Flow (perm)	251	3283	3316	1106	1663	1448
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				39		62
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	70			70	5	16
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	80	891	1339	72	146	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	80	891	1339	72	146	209
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	102.0	102.0	102.0	102.0	38.0	38.0
Total Split (%)	72.9%	72.9%	72.9%	72.9%	27.1%	27.1%
Maximum Green (s)	96.7	96.7	96.7	96.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	104.1	104.1	104.1	104.1	24.7	24.7
Actuated g/C Ratio	0.74	0.74	0.74	0.74	0.18	0.18
v/c Ratio	0.43	0.37	0.54	0.09	0.50	0.69
Control Delay	17.5	6.6	2.7	1.3	56.2	47.8
Queue Delay	0.0	0.1	0.3	0.0	0.0	0.0
Total Delay	17.5	6.8	3.0	1.3	56.2	47.8
LOS	B	A	A	A	E	D
Approach Delay		7.6	2.9		51.2	
Approach LOS		A	A		D	
Queue Length 50th (m)	5.1	30.0	25.0	0.6	31.7	33.3
Queue Length 95th (m)	m29.1	39.7	27.5	1.7	50.8	57.8
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	186	2441	2466	832	381	379
Starvation Cap Reductn	0	541	468	0	0	0
Spillback Cap Reductn	0	0	35	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.43	0.47	0.67	0.09	0.38	0.55

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 139 (99%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 10.9

Intersection LOS: B

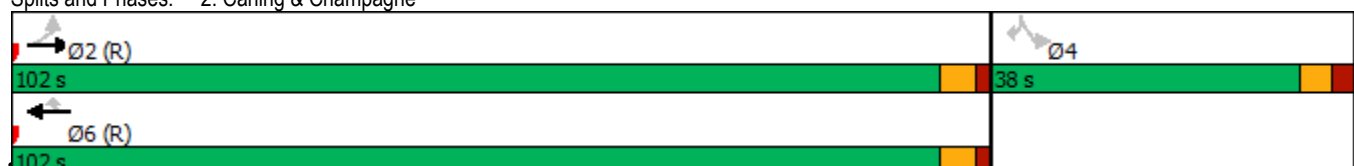
Intersection Capacity Utilization 77.2%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Future Volume (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3316	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3316	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	35					35	25		35	35		25
Confl. Bikes (#/hr)			11			10			13			34
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		104.0			104.0							
Total Split (%)		74.3%			74.3%							
Maximum Green (s)		98.9			98.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		115.6			115.6							
Actuated g/C Ratio		0.83			0.83							
v/c Ratio		0.40			0.53							
Control Delay		4.2			2.3							
Queue Delay		0.1			0.1							
Total Delay		4.3			2.4							
LOS		A			A							
Approach Delay		4.3			2.4							
Approach LOS		A			A							
Queue Length 50th (m)		43.2			26.4							
Queue Length 95th (m)		42.0			m25.9							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2710			2737							
Starvation Cap Reductn		408			380							
Spillback Cap Reductn		301			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.47			0.62							

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 114 (81%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.53

Intersection Signal Delay: 3.2

Intersection LOS: A

Intersection Capacity Utilization 46.7%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
104 s	36 s
 Ø5 (R)	
104 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	26%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	158	628	374	348	984	61	348	419	167	102	359	124
Future Volume (vph)	158	628	374	348	984	61	348	419	167	102	359	124
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.98	0.97		0.99		0.89		0.98		0.97	0.98	
Frt		0.944				0.850		0.957			0.961	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	3019	0	1674	3316	1427	1674	3109	0	1537	1629	0
Flt Permitted	0.950			0.950			0.091			0.431		
Satd. Flow (perm)	1572	3019	0	1649	3316	1272	160	3109	0	678	1629	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		86				132		54			12	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	53		34	34		53	60		55	55		60
Confl. Bikes (#/hr)			12			10			16			6
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	158	628	374	348	984	61	348	419	167	102	359	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	158	1002	0	348	984	61	348	586	0	102	483	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	21.0	44.0		28.0	51.0	51.0	24.0	68.0		44.0	44.0	
Total Split (%)	15.0%	31.4%		20.0%	36.4%	36.4%	17.1%	48.6%		31.4%	31.4%	
Maximum Green (s)	14.8	38.0		21.8	45.0	45.0	17.1	61.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	14.8	38.0		21.8	45.0	45.0	61.1	61.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.27		0.16	0.32	0.32	0.44	0.44		0.26	0.26	
v/c Ratio	0.93	1.14		1.34	0.92	0.12	1.37	0.42		0.57	1.10	
Control Delay	121.9	104.4		196.9	43.8	1.9	222.5	25.6		58.8	118.2	
Queue Delay	0.0	0.1		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	121.9	104.4		196.9	43.8	1.9	222.5	25.6		58.8	118.2	
LOS	F	F		F	D	A	F	C		E	F	
Approach Delay		106.8			80.2			99.0			107.8	
Approach LOS		F			F			F			F	
Queue Length 50th (m)	34.5	~148.5		~113.9	138.2	0.0	~103.8	48.5		22.7	~137.0	
Queue Length 95th (m)	#78.4	#180.2		m#113.9	m125.5	m0.0	#160.7	62.8		41.8	#199.2	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	170	882		260	1065	498	254	1387		179	440	
Starvation Cap Reductn	0	15		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.93	1.16		1.34	0.92	0.12	1.37	0.42		0.57	1.10	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.37

Intersection Signal Delay: 96.1

Intersection LOS: F

Intersection Capacity Utilization 125.3%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

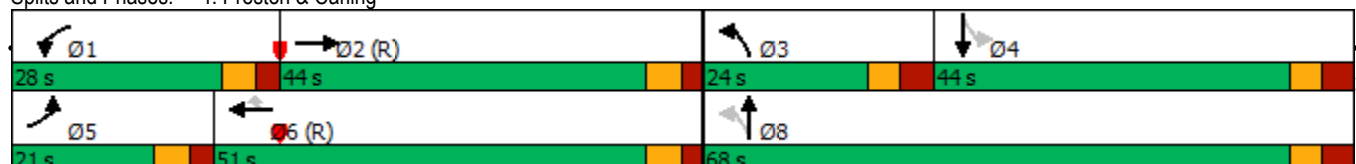
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

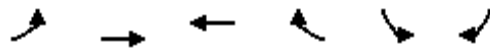
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
PM Peak Hour

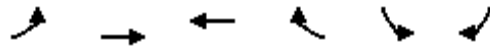
829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	264	699	998	104	308	326
Future Volume (vph)	264	699	998	104	308	326
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.95			0.79	0.98	0.75
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	1745	1498	1674	1483
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1595	3316	1745	1181	1647	1117
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				25		265
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	65			65	13	81
Confl. Bikes (#/hr)				10		45
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	264	699	998	104	308	326
Shared Lane Traffic (%)						
Lane Group Flow (vph)	264	699	998	104	308	326
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
2028 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	25.0	101.0	76.0	76.0	39.0	39.0
Total Split (%)	17.9%	72.1%	54.3%	54.3%	27.9%	27.9%
Maximum Green (s)	19.1	95.3	70.3	70.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	22.2	98.4	70.3	70.3	29.9	29.9
Actuated g/C Ratio	0.16	0.70	0.50	0.50	0.21	0.21
v/c Ratio	1.00	0.30	1.14	0.17	0.88	0.73
Control Delay	80.0	6.1	109.7	15.1	77.9	20.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	80.0	6.1	109.7	15.1	77.9	20.9
LOS	E	A	F	B	E	C
Approach Delay		26.3	100.7		48.6	
Approach LOS		C	F		D	
Queue Length 50th (m)	~77.1	44.9	~296.2	10.5	74.4	12.7
Queue Length 95th (m)	m#84.1	m43.4	#369.2	20.6	#113.7	47.4
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	265	2330	876	605	388	465
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.30	1.14	0.17	0.79	0.70

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 66 (47%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.14

Intersection Signal Delay: 61.9

Intersection LOS: E

Intersection Capacity Utilization 112.4%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	41	34	41	121	31	82	501	56	17	510	54
Future Volume (vph)	29	41	34	41	121	31	82	501	56	17	510	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.98	0.84	0.97	0.98		0.95	0.99	
Frt		0.956				0.850		0.985			0.986	
Flt Protected		0.986			0.988		0.950			0.950		
Satd. Flow (prot)	0	1584	0	0	1741	1498	1674	1680	0	1674	1673	0
Flt Permitted		0.875			0.906		0.399			0.403		
Satd. Flow (perm)	0	1374	0	0	1557	1261	680	1680	0	674	1673	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				31		14			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		46	46		40	52		80	80		52
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	29	41	34	41	121	31	82	501	56	17	510	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	104	0	0	162	31	82	557	0	17	564	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	61.8	61.8		61.8	61.8	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.37			0.55	0.12	0.18	0.48		0.04	0.49	
Control Delay		28.8			40.9	12.2	2.5	4.9		4.8	8.2	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.8			40.9	12.2	2.5	5.0		4.8	8.2	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.8			36.2			4.7			8.1	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.1			23.6	0.0	2.5	37.6		0.8	35.5	
Queue Length 95th (m)		24.5			41.4	6.6	0.5	1.8		2.6	55.4	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		285			301	268	466	1158		462	1153	
Starvation Cap Reductn		0			0	0	0	93		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.36			0.54	0.12	0.18	0.52		0.04	0.49	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.55

Intersection Signal Delay: 11.7

Intersection LOS: B

Intersection Capacity Utilization 87.3%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	596	27	6	582	15
Future Volume (vph)	4	2	10	0	0	0	8	596	27	6	582	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Frt		0.916						0.994			0.997	
Flt Protected		0.988						0.999				
Satd. Flow (prot)	0	1494	0	0	0	0	0	1709	0	0	1719	0
Flt Permitted		0.988						0.993			0.995	
Satd. Flow (perm)	0	1466	0	0	0	0	0	1698	0	0	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10						6			4	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	46		47	47		46
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	10	0	0	0	8	596	27	6	582	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	0	0	0	631	0	0	603	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.44			0.42	
Control Delay		21.9						5.0			3.7	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.9						5.0			3.7	
LOS		C						A			A	
Approach Delay		21.9						5.0			3.7	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						28.2			22.0	
Queue Length 95th (m)		5.8						57.6			33.6	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1428			1437	
Starvation Cap Reductn		0						0			64	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.06						0.44			0.44	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.44

Intersection Signal Delay: 4.6

Intersection LOS: A

Intersection Capacity Utilization 59.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

													
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	19	4	17	22	2	17	22	550	48	22	588	5	
Future Volume (vph)	19	4	17	22	2	17	22	550	48	22	588	5	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor													
Frt	0.943				0.944				0.990		0.999		
Flt Protected	0.977				0.974				0.998		0.998		
Satd. Flow (prot)	0	1608	0	0	1605	0	0	1709	0	0	1724	0	
Flt Permitted	0.977				0.974				0.998		0.998		
Satd. Flow (perm)	0	1608	0	0	1605	0	0	1709	0	0	1724	0	
Link Speed (k/h)	50				50				50		50		
Link Distance (m)	113.6				154.3				71.5		73.8		
Travel Time (s)	8.2				11.1				5.1		5.3		
Confl. Peds. (#/hr)							46		47		47		
Confl. Bikes (#/hr)							21						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%	
Adj. Flow (vph)	19	4	17	22	2	17	22	550	48	22	588	5	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	40	0	0	41	0	0	620	0	0	615	0	
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No	
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	0.0				0.0				0.0		0.0		
Link Offset(m)	-2.0				-2.0				0.0		0.0		
Crosswalk Width(m)	5.0				5.0				5.0		5.0		
Two way Left Turn Lane													
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	24		14	24		14	24		14	24		14	
Sign Control	Stop				Stop				Free		Free		
Intersection Summary													
Area Type:	Other												
Control Type:	Unsignalized												
Intersection Capacity Utilization	53.3%												
Analysis Period (min)	15												
ICU Level of Service A													

9: Preston & Sidney
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	24	26	39	596	559	64
Future Volume (vph)	24	26	39	596	559	64
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.930				0.986	
Flt Protected	0.977			0.997		
Satd. Flow (prot)	1586	0	0	3276	1706	0
Flt Permitted	0.977			0.997		
Satd. Flow (perm)	1586	0	0	3276	1706	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	24	26	39	596	559	64
Shared Lane Traffic (%)						
Lane Group Flow (vph)	50	0	0	635	623	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	58.4%			ICU Level of Service B		
Analysis Period (min)	15					

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	479	294	4	2	430	431	5	2	1	413	0	651
Future Volume (vph)	479	294	4	2	430	431	5	2	1	413	0	651
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.90		0.94		0.94			0.83	0.79
Frt		0.998				0.850		0.983				0.850
Flt Protected	0.950			0.950				0.970			0.950	
Satd. Flow (prot)	1642	1755	0	1674	1762	1498	0	1640	0	0	1674	1483
Flt Permitted	0.159			0.578				0.844			0.752	
Satd. Flow (perm)	275	1755	0	914	1762	1403	0	1370	0	0	1095	1166
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				175		1				99
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	73		65	65		73
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	479	294	4	2	430	431	5	2	1	413	0	651
Shared Lane Traffic (%)												
Lane Group Flow (vph)	479	298	0	2	430	431	0	8	0	0	413	651
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5
Switch Phase												

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2028 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		36.5	36.5	36.5		43.5			43.5	75.2
Actuated g/C Ratio	0.58	0.58		0.28	0.28	0.28		0.33			0.33	0.58
v/c Ratio	0.96	0.29		0.01	0.87	0.83		0.02			1.13	0.82
Control Delay	63.0	15.0		34.5	63.9	40.6		27.5			126.7	27.1
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	63.0	15.0		34.5	63.9	40.6		27.5			126.7	27.1
LOS	E	B		C	E	D		C			F	C
Approach Delay		44.6			52.2			27.5			65.8	
Approach LOS		D			D			C			E	
Queue Length 50th (m)	88.2	34.2		0.3	97.5	60.4		1.1			~112.7	78.5
Queue Length 95th (m)	#149.3	49.7		2.3	#149.9	#111.9		4.5			#170.0	117.0
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	504	1011		256	494	519		459			366	801
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.95	0.29		0.01	0.87	0.83		0.02			1.13	0.81

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.13

Intersection Signal Delay: 55.3

Intersection LOS: E

Intersection Capacity Utilization 106.2%

ICU Level of Service G

Analysis Period (min) 15

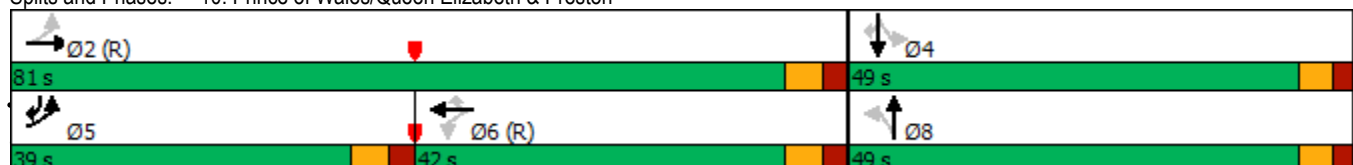
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	65	717	50	13	60	1366	155	125	25	150	180	10
Future Volume (vph)	65	717	50	13	60	1366	155	125	25	150	180	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99		0.92		0.98		0.89	0.96		0.95	0.96	0.98
Frt			0.850				0.850			0.850		0.938
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1674	3252	1483	0	1661	3316	1498	1658	1745	1483	1674	1605
Flt Permitted	0.950				0.379			0.746			0.741	
Satd. Flow (perm)	1651	3252	1370	0	650	3316	1330	1253	1745	1410	1258	1605
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			46				155			150		7
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	35		20		20		35	30		30	30	
Confl. Bikes (#/hr)			10				5					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	2%	1%	2%	2%	1%	2%	2%	2%	1%	2%
Adj. Flow (vph)	65	717	50	13	60	1366	155	125	25	150	180	10
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	717	50	0	73	1366	155	125	25	150	180	17
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	7
Future Volume (vph)	7
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	8
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	7
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	18.0	99.0	99.0	81.0	81.0	81.0	81.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	12.9%	70.7%	70.7%	57.9%	57.9%	57.9%	57.9%	29.3%	29.3%	29.3%	29.3%	29.3%
Maximum Green (s)	12.8	92.6	92.6	74.6	74.6	74.6	74.6	33.9	33.9	33.9	33.9	33.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	10.3	99.1	99.1		85.9	85.9	85.9	27.4	27.4	27.4	27.4	27.4
Actuated g/C Ratio	0.07	0.71	0.71		0.61	0.61	0.61	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.53	0.31	0.05		0.18	0.67	0.18	0.51	0.07	0.38	0.73	0.05
Control Delay	77.3	8.8	2.6		5.7	9.0	0.5	56.3	42.8	9.2	69.3	29.8
Queue Delay	0.0	0.0	0.0		0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	8.8	2.6		5.7	9.1	0.5	56.3	42.8	9.2	69.3	29.8
LOS	E	A	A		A	A	A	E	D	A	E	C
Approach Delay		13.8				8.1			31.6			65.9
Approach LOS		B				A			C			E
Queue Length 50th (m)	16.2	38.1	0.3		2.4	51.2	0.0	27.3	5.0	0.0	41.2	2.0
Queue Length 95th (m)	30.2	48.4	4.4		7.5	110.8	0.2	45.2	12.1	15.9	64.7	7.8
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	153	2302	983		398	2035	876	303	422	455	304	393
Starvation Cap Reductn	0	0	0		0	97	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.31	0.05		0.18	0.70	0.18	0.41	0.06	0.33	0.59	0.04

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 15 (11%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 16.0

Intersection LOS: B

Intersection Capacity Utilization 85.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

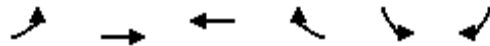




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
PM Peak Hour

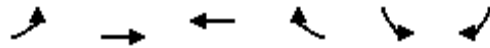
829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	80	891	1339	72	146	209
Future Volume (vph)	80	891	1339	72	146	209
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor				0.72	0.99	0.97
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	3316	1498	1674	1498
Flt Permitted	0.169				0.950	
Satd. Flow (perm)	251	3283	3316	1080	1663	1448
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				39		61
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	75			75	5	16
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	80	891	1339	72	146	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	80	891	1339	72	146	209
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	102.0	102.0	102.0	102.0	38.0	38.0
Total Split (%)	72.9%	72.9%	72.9%	72.9%	27.1%	27.1%
Maximum Green (s)	96.7	96.7	96.7	96.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	104.1	104.1	104.1	104.1	24.7	24.7
Actuated g/C Ratio	0.74	0.74	0.74	0.74	0.18	0.18
v/c Ratio	0.43	0.37	0.54	0.09	0.50	0.69
Control Delay	17.4	6.6	3.1	1.5	56.1	48.1
Queue Delay	0.0	0.1	0.3	0.0	0.0	0.0
Total Delay	17.4	6.7	3.4	1.5	56.1	48.1
LOS	B	A	A	A	E	D
Approach Delay		7.6	3.3		51.4	
Approach LOS		A	A		D	
Queue Length 50th (m)	5.1	30.0	30.0	0.9	31.7	33.5
Queue Length 95th (m)	m28.7	39.6	32.8	2.5	50.8	58.0
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	186	2441	2466	812	381	379
Starvation Cap Reductn	0	539	468	0	0	0
Spillback Cap Reductn	0	0	51	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.43	0.47	0.67	0.09	0.38	0.55

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 139 (99%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 11.0

Intersection LOS: B

Intersection Capacity Utilization 77.2%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Future Volume (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3316	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3316	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40					40	25		35	35		25
Confl. Bikes (#/hr)			11			10			13			34
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1088	0	0	1455	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		104.0			104.0							
Total Split (%)		74.3%			74.3%							
Maximum Green (s)		98.9			98.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		115.6			115.6							
Actuated g/C Ratio		0.83			0.83							
v/c Ratio		0.40			0.53							
Control Delay		4.2			3.2							
Queue Delay		0.1			0.1							
Total Delay		4.3			3.3							
LOS		A			A							
Approach Delay		4.3			3.3							
Approach LOS		A			A							
Queue Length 50th (m)		43.2			31.1							
Queue Length 95th (m)		42.1			m32.9							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2710			2737							
Starvation Cap Reductn		408			357							
Spillback Cap Reductn		172			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.47			0.61							

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 114 (81%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.53

Intersection Signal Delay: 3.7

Intersection LOS: A

Intersection Capacity Utilization 46.7%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
104 s	36 s
 Ø5 (R)	
104 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	26%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	158	528	374	258	984	61	268	419	167	102	309	124
Future Volume (vph)	158	528	374	258	984	61	268	419	167	102	309	124
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97	0.97		0.98		0.86	0.97	0.98		0.97	0.97	
Frt		0.938				0.850		0.957			0.957	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	2990	0	1674	3316	1427	1674	3104	0	1537	1607	0
Flt Permitted	0.950			0.950			0.132			0.431		
Satd. Flow (perm)	1560	2990	0	1646	3316	1230	227	3104	0	676	1607	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		126				132		54			14	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	70		34	34		70	75		60	60		75
Confl. Bikes (#/hr)			13			11			16			10
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	158	528	374	258	984	61	268	419	167	102	309	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	158	902	0	258	984	61	268	586	0	102	433	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	21.0	44.0		28.0	51.0	51.0	24.0	68.0		44.0	44.0	
Total Split (%)	15.0%	31.4%		20.0%	36.4%	36.4%	17.1%	48.6%		31.4%	31.4%	
Maximum Green (s)	14.8	38.0		21.8	45.0	45.0	17.1	61.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	14.8	38.0		21.8	45.0	45.0	61.1	61.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.27		0.16	0.32	0.32	0.44	0.44		0.26	0.26	
v/c Ratio	0.93	1.00		0.99	0.92	0.13	0.97	0.42		0.57	0.99	
Control Delay	122.2	56.0		85.4	48.1	1.7	81.5	25.6		58.9	90.8	
Queue Delay	0.0	18.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	122.2	74.0		85.4	48.1	1.7	81.5	25.6		58.9	90.8	
LOS	F	E		F	D	A	F	C		E	F	
Approach Delay		81.2			53.3			43.2			84.7	
Approach LOS		F			D			D			F	
Queue Length 50th (m)	34.0	102.2		63.9	137.7	0.0	49.5	48.5		22.7	107.9	
Queue Length 95th (m)	#78.1	#144.6		m#82.5	m140.0	m0.0	#101.4	62.8		41.8	#171.6	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	170	903		260	1065	484	275	1385		179	436	
Starvation Cap Reductn	0	49		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.93	1.06		0.99	0.92	0.13	0.97	0.42		0.57	0.99	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 63.4

Intersection LOS: E

Intersection Capacity Utilization 112.6%

ICU Level of Service H

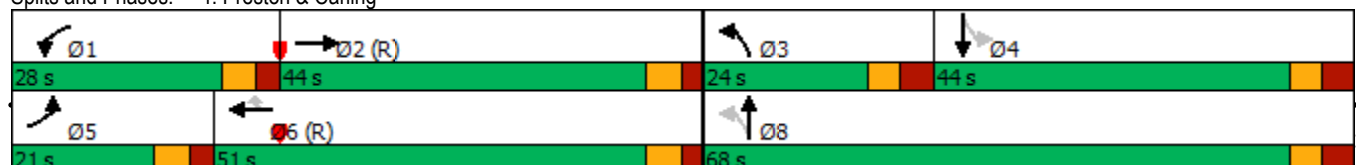
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

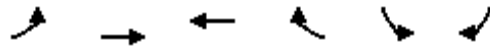
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
PM Peak Hour

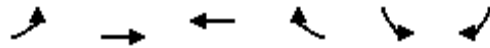
829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	264	699	878	104	308	326
Future Volume (vph)	264	699	878	104	308	326
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.94			0.77	0.98	0.75
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	1745	1498	1674	1483
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1573	3316	1745	1159	1647	1117
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				28		265
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	13	81
Confl. Bikes (#/hr)				11		45
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	264	699	878	104	308	326
Shared Lane Traffic (%)						
Lane Group Flow (vph)	264	699	878	104	308	326
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	25.0	101.0	76.0	76.0	39.0	39.0
Total Split (%)	17.9%	72.1%	54.3%	54.3%	27.9%	27.9%
Maximum Green (s)	19.1	95.3	70.3	70.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	22.2	98.4	70.3	70.3	29.9	29.9
Actuated g/C Ratio	0.16	0.70	0.50	0.50	0.21	0.21
v/c Ratio	1.00	0.30	1.00	0.17	0.88	0.73
Control Delay	94.2	6.0	65.8	14.6	77.9	20.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	94.2	6.0	65.8	14.6	77.9	20.9
LOS	F	A	E	B	E	C
Approach Delay		30.2	60.4		48.6	
Approach LOS		C	E		D	
Queue Length 50th (m)	~77.7	39.0	~220.1	10.1	74.4	12.7
Queue Length 95th (m)	m#100.3	m44.8	#304.2	20.3	#113.7	47.4
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	265	2330	876	595	388	465
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.30	1.00	0.17	0.79	0.70

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 66 (47%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 46.2

Intersection LOS: D

Intersection Capacity Utilization 105.7%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	41	34	41	121	31	82	501	56	17	510	54
Future Volume (vph)	29	41	34	41	121	31	82	501	56	17	510	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.97	0.84	0.97	0.98		0.95	0.99	
Frt		0.956				0.850		0.985			0.986	
Flt Protected		0.986			0.988		0.950			0.950		
Satd. Flow (prot)	0	1578	0	0	1741	1498	1674	1680	0	1674	1672	0
Flt Permitted		0.875			0.906		0.399			0.403		
Satd. Flow (perm)	0	1369	0	0	1554	1261	679	1680	0	674	1672	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				31		14			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		50	50		40	55		80	80		55
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	29	41	34	41	121	31	82	501	56	17	510	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	104	0	0	162	31	82	557	0	17	564	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	61.8	61.8		61.8	61.8	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.37			0.55	0.12	0.18	0.48		0.04	0.49	
Control Delay		28.9			40.9	12.2	2.5	4.9		4.8	8.2	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			40.9	12.2	2.5	5.0		4.8	8.2	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.9			36.3			4.7			8.1	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.1			23.6	0.0	2.5	37.6		0.8	35.5	
Queue Length 95th (m)		24.5			41.4	6.6	0.5	1.8		2.6	55.4	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		284			300	268	466	1158		462	1152	
Starvation Cap Reductn		0			0	0	0	93		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.37			0.54	0.12	0.18	0.52		0.04	0.49	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.55

Intersection Signal Delay: 11.7

Intersection LOS: B

Intersection Capacity Utilization 87.4%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	596	27	6	582	15
Future Volume (vph)	4	2	10	0	0	0	8	596	27	6	582	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Frt		0.916						0.994			0.997	
Flt Protected		0.988						0.999				
Satd. Flow (prot)	0	1494	0	0	0	0	0	1709	0	0	1719	0
Flt Permitted		0.988						0.993			0.995	
Satd. Flow (perm)	0	1466	0	0	0	0	0	1698	0	0	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10						6			4	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	50		47	47		50
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	10	0	0	0	8	596	27	6	582	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	0	0	0	631	0	0	603	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.44			0.42	
Control Delay		21.9						5.0			3.7	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.9						5.0			3.7	
LOS		C						A			A	
Approach Delay		21.9						5.0			3.7	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						28.2			22.0	
Queue Length 95th (m)		5.8						57.6			33.6	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1428			1437	
Starvation Cap Reductn		0						0			64	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.06						0.44			0.44	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.44

Intersection Signal Delay: 4.6

Intersection LOS: A

Intersection Capacity Utilization 59.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

													
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	19	4	17	22	2	17	22	550	48	22	588	5	
Future Volume (vph)	19	4	17	22	2	17	22	550	48	22	588	5	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor													
Frt	0.943				0.944				0.990		0.999		
Flt Protected	0.977				0.974				0.998		0.998		
Satd. Flow (prot)	0	1608	0	0	1605	0	0	1709	0	0	1724	0	
Flt Permitted	0.977				0.974				0.998		0.998		
Satd. Flow (perm)	0	1608	0	0	1605	0	0	1709	0	0	1724	0	
Link Speed (k/h)	50				50				50		50		
Link Distance (m)	113.6				154.3				71.5		73.8		
Travel Time (s)	8.2				11.1				5.1		5.3		
Confl. Peds. (#/hr)							46		47		47		
Confl. Bikes (#/hr)							21						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%	
Adj. Flow (vph)	19	4	17	22	2	17	22	550	48	22	588	5	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	40	0	0	41	0	0	620	0	0	615	0	
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No	
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	0.0				0.0				0.0		0.0		
Link Offset(m)	-2.0				-2.0				0.0		0.0		
Crosswalk Width(m)	5.0				5.0				5.0		5.0		
Two way Left Turn Lane													
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	24		14	24		14	24		14	24		14	
Sign Control	Stop				Stop				Free		Free		
Intersection Summary													
Area Type:	Other												
Control Type:	Unsignalized												
Intersection Capacity Utilization 53.3%					ICU Level of Service A								
Analysis Period (min) 15													

9: Preston & Sidney
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	24	26	39	596	559	64
Future Volume (vph)	24	26	39	596	559	64
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.930				0.986	
Flt Protected	0.977			0.997		
Satd. Flow (prot)	1586	0	0	3276	1706	0
Flt Permitted	0.977			0.997		
Satd. Flow (perm)	1586	0	0	3276	1706	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	24	26	39	596	559	64
Shared Lane Traffic (%)						
Lane Group Flow (vph)	50	0	0	635	623	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 58.4%				ICU Level of Service B		
Analysis Period (min) 15						

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	479	294	4	2	430	431	5	2	1	363	0	651
Future Volume (vph)	479	294	4	2	430	431	5	2	1	363	0	651
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.90		0.94		0.92			0.83	0.77
Frt		0.998				0.850		0.983				0.850
Flt Protected	0.950			0.950				0.970			0.950	
Satd. Flow (prot)	1642	1755	0	1674	1762	1498	0	1640	0	0	1674	1483
Flt Permitted	0.159			0.578				0.853			0.752	
Satd. Flow (perm)	275	1755	0	914	1762	1403	0	1364	0	0	1095	1137
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				175		1				99
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	80		65	65		80
Confl. Bikes (#/hr)			2									2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	479	294	4	2	430	431	5	2	1	363	0	651
Shared Lane Traffic (%)												
Lane Group Flow (vph)	479	298	0	2	430	431	0	8	0	0	363	651
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2028 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		36.5	36.5	36.5		43.5			43.5	75.2
Actuated g/C Ratio	0.58	0.58		0.28	0.28	0.28		0.33			0.33	0.58
v/c Ratio	0.96	0.29		0.01	0.87	0.83		0.02			0.99	0.83
Control Delay	63.0	15.0		34.5	63.9	40.6		27.5			88.5	28.3
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	63.0	15.0		34.5	63.9	40.6		27.5			88.5	28.3
LOS	E	B		C	E	D		C			F	C
Approach Delay		44.6			52.2			27.5			49.8	
Approach LOS		D			D			C			D	
Queue Length 50th (m)	88.2	34.2		0.3	97.5	60.4		1.1			85.0	78.5
Queue Length 95th (m)	#149.3	49.7		2.3	#149.9	#111.9		4.5			#143.0	117.0
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	504	1011		256	494	519		457			366	791
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.95	0.29		0.01	0.87	0.83		0.02			0.99	0.82

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 49.0

Intersection LOS: D

Intersection Capacity Utilization 106.5%

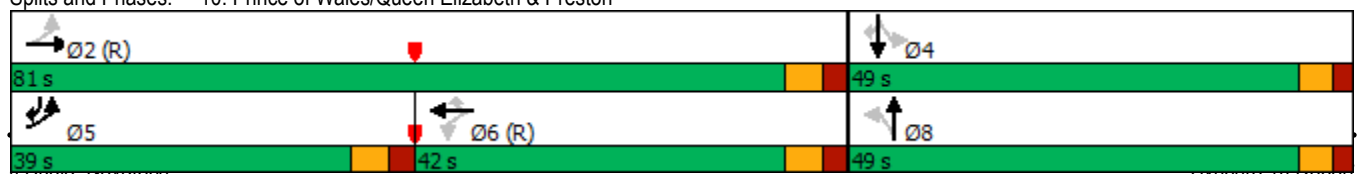
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



0.0000, Nov 2008

Dynamic 10 Report

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	31	627	118	13	142	532	129	45	9	54	134	24
Future Volume (vph)	31	627	118	13	142	532	129	45	9	54	134	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.93		0.98		0.90	0.97		0.96	0.97	0.99
Frt			0.850				0.850			0.850		0.974
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1642	3283	1483	0	1659	3161	1483	1658	1745	1483	1674	1689
Flt Permitted	0.950				0.414			0.738			0.752	
Satd. Flow (perm)	1589	3283	1385	0	710	3161	1339	1247	1745	1418	1282	1689
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			118				129			78		
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	30		20		20		30	30		30	30	
Confl. Bikes (#/hr)			5				17					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	3%	2%	1%	2%	7%	2%	2%	2%	2%	1%	2%
Adj. Flow (vph)	31	627	118	13	142	532	129	45	9	54	134	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	627	118	0	155	532	129	45	9	54	134	29
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	5
Future Volume (vph)	5
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	3
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	5
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	12.0	78.0	78.0	66.0	66.0	66.0	66.0	42.0	42.0	42.0	42.0	42.0
Total Split (%)	10.0%	65.0%	65.0%	55.0%	55.0%	55.0%	55.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Maximum Green (s)	6.8	71.6	71.6	59.6	59.6	59.6	59.6	34.9	34.9	34.9	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	7.4	84.7	84.7		76.6	76.6	76.6	21.8	21.8	21.8	21.8	21.8
Actuated g/C Ratio	0.06	0.71	0.71		0.64	0.64	0.64	0.18	0.18	0.18	0.18	0.18
v/c Ratio	0.31	0.27	0.12		0.34	0.26	0.14	0.20	0.03	0.17	0.58	0.09
Control Delay	61.3	8.0	1.9		4.8	2.9	0.4	39.5	34.0	4.5	52.6	31.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.3	8.0	1.9		4.8	2.9	0.4	39.5	34.0	4.5	52.6	31.5
LOS	E	A	A		A	A	A	D	C	A	D	C
Approach Delay		9.2				2.9			21.5			48.8
Approach LOS		A				A			C			D
Queue Length 50th (m)	6.5	20.2	0.0		2.1	3.7	0.0	8.7	1.7	0.0	28.0	4.6
Queue Length 95th (m)	15.6	41.5	6.4		5.1	7.4	0.0	16.2	5.1	4.8	40.5	10.7
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	106	2316	1012		452	2016	900	362	507	467	372	494
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.27	0.12		0.34	0.26	0.14	0.12	0.02	0.12	0.36	0.06

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.58

Intersection Signal Delay: 10.6

Intersection LOS: B

Intersection Capacity Utilization 82.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

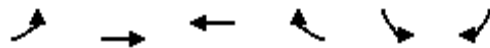




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
AM Peak Hour

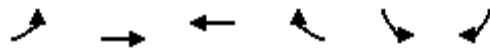
829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	112	758	771	168	89	78
Future Volume (vph)	112	758	771	168	89	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.93			0.71	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	3252	1414	1658	1498
Flt Permitted	0.340				0.950	
Satd. Flow (perm)	555	3283	3252	1009	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				144		78
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	90			90	7	9
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	112	758	771	168	89	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	758	771	168	89	78
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	78.0	78.0	78.0	78.0	42.0	42.0
Total Split (%)	65.0%	65.0%	65.0%	65.0%	35.0%	35.0%
Maximum Green (s)	72.7	72.7	72.7	72.7	36.1	36.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	85.6	85.6	85.6	85.6	23.2	23.2
Actuated g/C Ratio	0.71	0.71	0.71	0.71	0.19	0.19
v/c Ratio	0.28	0.32	0.33	0.22	0.28	0.23
Control Delay	10.3	8.0	4.2	2.7	40.4	9.0
Queue Delay	0.0	0.0	0.2	0.0	0.0	0.0
Total Delay	10.3	8.0	4.4	2.7	40.4	9.0
LOS	B	A	A	A	D	A
Approach Delay		8.3	4.1		25.7	
Approach LOS		A	A		C	
Queue Length 50th (m)	13.6	50.0	20.5	2.4	15.0	0.0
Queue Length 95th (m)	10.6	37.2	24.8	8.9	27.7	10.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	395	2342	2319	761	494	494
Starvation Cap Reductn	0	0	765	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.32	0.50	0.22	0.18	0.16

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.33

Intersection Signal Delay: 7.8

Intersection LOS: A

Intersection Capacity Utilization 57.7%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Future Volume (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3283	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3283	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40		14	14		40	18		20	20		18
Confl. Bikes (#/hr)			7			23						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.32			0.37							
Control Delay		4.4			3.3							
Queue Delay		0.1			0.1							
Total Delay		4.5			3.4							
LOS		A			A							
Approach Delay		4.5			3.4							
Approach LOS		A			A							
Queue Length 50th (m)		23.3			24.3							
Queue Length 95th (m)		27.2			m28.1							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			2615							
Starvation Cap Reductn		559			417							
Spillback Cap Reductn		46			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.41			0.44							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 57 (48%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.37

Intersection Signal Delay: 3.9

Intersection LOS: A

Intersection Capacity Utilization 32.7%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
34 s	36 s
 Ø5 (R)	
34 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	154	527	233	143	608	100	284	490	284	132	358	128
Future Volume (vph)	154	527	233	143	608	100	284	490	284	132	358	128
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.94	0.97		0.98		0.85	0.98	0.99		1.00	0.98	
Frt		0.954				0.850		0.945			0.960	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3035	0	1658	3252	1375	1674	3052	0	1510	1528	0
Flt Permitted	0.950			0.950			0.147			0.358		
Satd. Flow (perm)	1503	3035	0	1622	3252	1167	255	3052	0	567	1528	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		56				155		134			16	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	90		41	41		90	60		10	10		60
Confl. Bikes (#/hr)			22			10			36			5
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	154	527	233	143	608	100	284	490	284	132	358	128
Shared Lane Traffic (%)												
Lane Group Flow (vph)	154	760	0	143	608	100	284	774	0	132	486	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	20.0	37.0		18.0	35.0	35.0	21.0	65.0		44.0	44.0	
Total Split (%)	16.7%	30.8%		15.0%	29.2%	29.2%	17.5%	54.2%		36.7%	36.7%	
Maximum Green (s)	13.8	31.0		11.8	29.0	29.0	14.1	58.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	13.4	31.0		11.8	29.4	29.4	58.1	58.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.26		0.10	0.24	0.24	0.48	0.48		0.31	0.31	
v/c Ratio	0.87	0.92		0.88	0.76	0.25	0.98	0.50		0.75	1.01	
Control Delay	93.4	57.3		88.8	63.1	12.4	62.9	11.3		65.3	83.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	93.4	57.3		88.8	63.1	12.4	62.9	11.3		65.3	83.0	
LOS	F	E		F	E	B	E	B		E	F	
Approach Delay		63.4			61.4			25.1			79.2	
Approach LOS		E			E			C			E	
Queue Length 50th (m)	26.2	66.3		32.9	59.6	4.1	38.8	47.9		25.7	~103.7	
Queue Length 95th (m)	#63.3	#105.4		m#39.5	m67.1	m6.4	m#72.9	49.5		#56.1	#167.8	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	183	826		163	795	402	290	1546		175	483	
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.84	0.92		0.88	0.76	0.25	0.98	0.50		0.75	1.01	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 54.0

Intersection LOS: D

Intersection Capacity Utilization 101.8%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

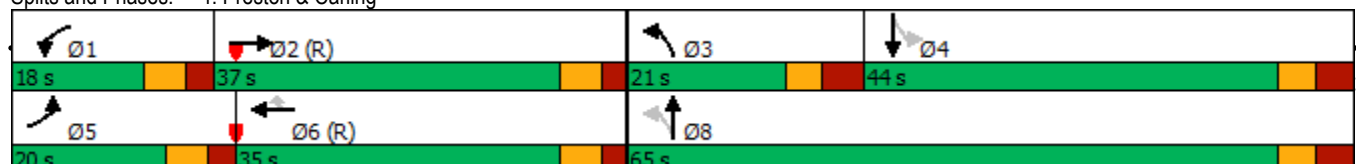
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

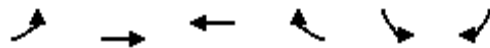
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

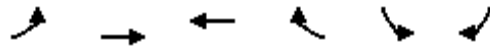
829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	398	630	672	215	240	168
Future Volume (vph)	398	630	672	215	240	168
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.93			0.80	0.98	0.80
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	1728	1498	1674	1427
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1559	3252	1728	1193	1649	1145
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				74		168
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	14	85
Confl. Bikes (#/hr)				16		23
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	398	630	672	215	240	168
Shared Lane Traffic (%)						
Lane Group Flow (vph)	398	630	672	215	240	168
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	26.0	81.0	55.0	55.0	39.0	39.0
Total Split (%)	21.7%	67.5%	45.8%	45.8%	32.5%	32.5%
Maximum Green (s)	20.1	75.3	49.3	49.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	28.5	83.7	49.3	49.3	24.6	24.6
Actuated g/C Ratio	0.24	0.70	0.41	0.41	0.20	0.20
v/c Ratio	1.01	0.28	0.95	0.40	0.71	0.46
Control Delay	87.0	6.8	58.1	18.5	55.0	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	87.0	6.8	58.1	18.5	55.0	9.2
LOS	F	A	E	B	E	A
Approach Delay		37.8	48.5		36.2	
Approach LOS		D	D		D	
Queue Length 50th (m)	90.1	20.6	138.3	20.6	49.8	0.0
Queue Length 95th (m)	m#155.2	m31.0	#207.6	39.4	68.0	15.6
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	396	2267	709	533	453	436
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.01	0.28	0.95	0.40	0.53	0.39

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 41.6

Intersection LOS: D

Intersection Capacity Utilization 101.8%

ICU Level of Service G

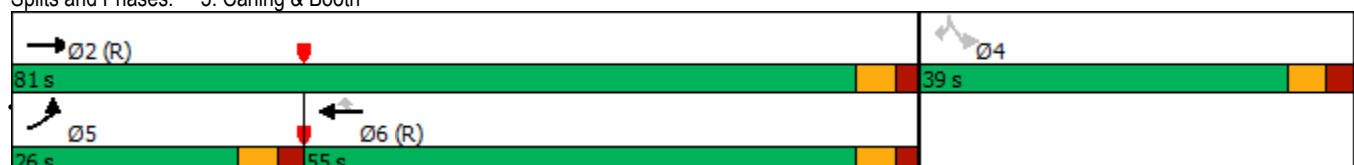
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	43	56	26	37	55	12	28	668	60	18	459	38
Future Volume (vph)	43	56	26	37	55	12	28	668	60	18	459	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.97	0.89	0.97	0.99		0.98	0.99	
Frt		0.972				0.850		0.988			0.989	
Flt Protected		0.983			0.980		0.950			0.950		
Satd. Flow (prot)	0	1558	0	0	1568	1498	1537	1692	0	1537	1650	0
Flt Permitted		0.865			0.853		0.433			0.286		
Satd. Flow (perm)	0	1336	0	0	1325	1339	678	1692	0	453	1650	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15				34		11			10	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		40	40		36	50		55	55		50
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	43	56	26	37	55	12	28	668	60	18	459	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	125	0	0	92	12	28	728	0	18	497	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.42			0.33	0.04	0.06	0.66		0.06	0.46	
Control Delay		28.9			30.4	2.8	5.8	10.2		5.9	8.7	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			30.4	2.8	5.8	10.4		5.9	8.7	
LOS		C			C	A	A	B		A	A	
Approach Delay		28.9			27.2			10.2			8.6	
Approach LOS		C			C			B			A	
Queue Length 50th (m)		13.3			11.0	0.0	1.3	53.8		0.8	29.9	
Queue Length 95th (m)		27.3			22.7	1.2	m2.7	45.4		3.0	48.3	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		302			288	317	438	1099		293	1071	
Starvation Cap Reductn		0			0	0	0	32		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.41			0.32	0.04	0.06	0.68		0.06	0.46	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 12.4

Intersection LOS: B

Intersection Capacity Utilization 83.5%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	709	46	10	555	5
Future Volume (vph)	1	0	3	0	0	0	8	709	46	10	555	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.992			0.999	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1704	0	0	1617	0
Flt Permitted		0.988						0.995			0.988	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1697	0	0	1599	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29						9			1	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	35		45	45		35
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	8	709	46	10	555	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	763	0	0	570	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.52			0.41	
Control Delay		0.0						5.7			2.5	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.7			2.5	
LOS		A						A			A	
Approach Delay								5.7			2.5	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						83.6			17.5	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1481			1394	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.52			0.41	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 4.3

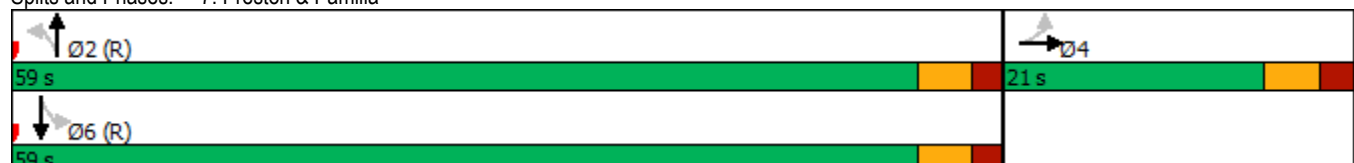
Intersection LOS: A

Intersection Capacity Utilization 66.1%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	49	1	33	20	5	26	14	651	90	37	545	15
Future Volume (vph)	49	1	33	20	5	26	14	651	90	37	545	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Fr't		0.946			0.931			0.984			0.997	
Flt Protected		0.971			0.981			0.999			0.997	
Satd. Flow (prot)	0	1603	0	0	1594	0	0	1701	0	0	1689	0
Flt Permitted		0.971			0.981			0.999			0.997	
Satd. Flow (perm)	0	1603	0	0	1594	0	0	1701	0	0	1689	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							28		45	45		28
Confl. Bikes (#/hr)									21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	49	1	33	20	5	26	14	651	90	37	545	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	83	0	0	51	0	0	755	0	0	597	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization 67.0%	ICU Level of Service C											
Analysis Period (min) 15												

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	50	57	37	705	561	33
Future Volume (vph)	50	57	37	705	561	33
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.928			0.992		
Flt Protected	0.977			0.998		
Satd. Flow (prot)	1582	0	0	3279	1684	0
Flt Permitted	0.977			0.998		
Satd. Flow (perm)	1582	0	0	3279	1684	0
Link Speed (k/h)	30			50		
Link Distance (m)	68.0			65.2		
Travel Time (s)	8.2			4.7		
Confl. Peds. (#/hr)				28		28
Confl. Bikes (#/hr)						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	50	57	37	705	561	33
Shared Lane Traffic (%)						
Lane Group Flow (vph)	107	0	0	742	594	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0		
Link Offset(m)	0.0			0.0		
Crosswalk Width(m)	5.0			2.0		
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24	14		
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 62.9%				ICU Level of Service B		
Analysis Period (min) 15						

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	678	281	2	2	223	317	1	4	3	279	4	420
Future Volume (vph)	678	281	2	2	223	317	1	4	3	279	4	420
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.92
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1668	1469
Flt Permitted	0.503			0.586				0.969			0.724	
Satd. Flow (perm)	863	1760	0	1009	1762	1462	0	1176	0	0	1193	1347
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				316		3				420
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	25		24	24		25
Confl. Bikes (#/hr)			3						1			2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	678	281	2	2	223	317	1	4	3	279	4	420
Shared Lane Traffic (%)												
Lane Group Flow (vph)	678	283	0	2	223	317	0	8	0	0	283	420
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		48.1	48.1	48.1		24.5			24.5	53.6
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.85	0.23		0.00	0.32	0.41		0.03			1.16	0.49
Control Delay	21.5	7.0		23.0	26.9	4.4		32.1			125.0	4.0
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	21.5	7.0		23.0	26.9	4.4		32.1			125.0	4.0
LOS	C	A		C	C	A		C			F	A
Approach Delay		17.3			13.8			32.1			52.7	
Approach LOS		B			B			C			D	
Queue Length 50th (m)	67.6	19.7		0.3	33.8	0.1		0.9			~70.7	12.3
Queue Length 95th (m)	#101.0	29.5		1.9	52.2	16.2		4.8			m#77.7	m14.4
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	810	1230		404	706	775		242			243	883
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.84	0.23		0.00	0.32	0.41		0.03			1.16	0.48

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.16

Intersection Signal Delay: 27.7

Intersection LOS: C

Intersection Capacity Utilization 99.3%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

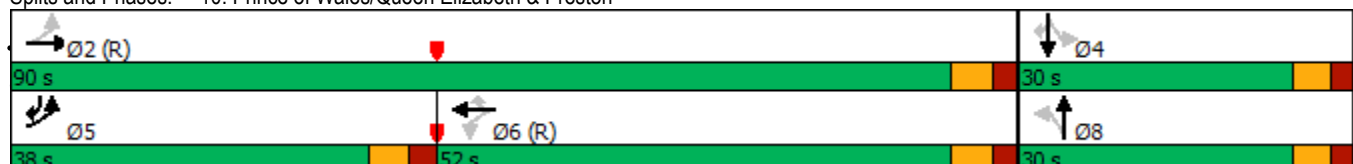
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	31	627	118	13	142	532	129	45	9	54	134	24
Future Volume (vph)	31	627	118	13	142	532	129	45	9	54	134	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.93		0.98		0.90	0.97		0.96	0.97	0.99
Frt			0.850				0.850			0.850		0.974
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1642	3283	1483	0	1659	3161	1483	1658	1745	1483	1674	1689
Flt Permitted	0.950				0.414			0.738			0.752	
Satd. Flow (perm)	1589	3283	1385	0	710	3161	1339	1247	1745	1418	1282	1689
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			118				129			78		
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	30		20		20		30	30		30	30	
Confl. Bikes (#/hr)			5				17					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	3%	2%	1%	2%	7%	2%	2%	2%	2%	1%	2%
Adj. Flow (vph)	31	627	118	13	142	532	129	45	9	54	134	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	627	118	0	155	532	129	45	9	54	134	29
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	5
Future Volume (vph)	5
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	3
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	5
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	12.0	78.0	78.0	66.0	66.0	66.0	66.0	42.0	42.0	42.0	42.0	42.0
Total Split (%)	10.0%	65.0%	65.0%	55.0%	55.0%	55.0%	55.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Maximum Green (s)	6.8	71.6	71.6	59.6	59.6	59.6	59.6	34.9	34.9	34.9	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	7.4	84.7	84.7		76.6	76.6	76.6	21.8	21.8	21.8	21.8	21.8
Actuated g/C Ratio	0.06	0.71	0.71		0.64	0.64	0.64	0.18	0.18	0.18	0.18	0.18
v/c Ratio	0.31	0.27	0.12		0.34	0.26	0.14	0.20	0.03	0.17	0.58	0.09
Control Delay	61.3	8.0	1.9		4.7	2.8	0.4	39.5	34.0	4.5	52.6	31.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.3	8.0	1.9		4.7	2.8	0.4	39.5	34.0	4.5	52.6	31.5
LOS	E	A	A		A	A	A	D	C	A	D	C
Approach Delay		9.2				2.8			21.5			48.8
Approach LOS		A				A			C			D
Queue Length 50th (m)	6.5	20.2	0.0		1.9	3.4	0.0	8.7	1.7	0.0	28.0	4.6
Queue Length 95th (m)	15.6	41.5	6.4		5.1	7.3	0.0	16.2	5.1	4.8	40.5	10.7
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	106	2316	1012		452	2016	900	362	507	467	372	494
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.27	0.12		0.34	0.26	0.14	0.12	0.02	0.12	0.36	0.06

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.58

Intersection Signal Delay: 10.6

Intersection LOS: B

Intersection Capacity Utilization 82.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood





Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

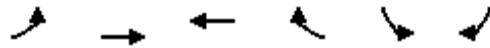
2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

						
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		 	 			
Traffic Volume (vph)	112	758	771	168	89	78
Future Volume (vph)	112	758	771	168	89	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.93			0.71	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	3252	1414	1658	1498
Flt Permitted	0.340				0.950	
Satd. Flow (perm)	555	3283	3252	1009	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				144		78
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	90			90	7	9
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	112	758	771	168	89	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	758	771	168	89	78
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	78.0	78.0	78.0	78.0	42.0	42.0
Total Split (%)	65.0%	65.0%	65.0%	65.0%	35.0%	35.0%
Maximum Green (s)	72.7	72.7	72.7	72.7	36.1	36.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	85.6	85.6	85.6	85.6	23.2	23.2
Actuated g/C Ratio	0.71	0.71	0.71	0.71	0.19	0.19
v/c Ratio	0.28	0.32	0.33	0.22	0.28	0.23
Control Delay	10.3	8.0	4.2	2.7	40.4	9.0
Queue Delay	0.0	0.0	0.2	0.0	0.0	0.0
Total Delay	10.3	8.0	4.4	2.7	40.4	9.0
LOS	B	A	A	A	D	A
Approach Delay		8.3	4.1		25.7	
Approach LOS		A	A		C	
Queue Length 50th (m)	13.6	49.9	21.0	2.8	15.0	0.0
Queue Length 95th (m)	10.6	37.2	25.4	9.4	27.7	10.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	395	2342	2319	761	494	494
Starvation Cap Reductn	0	0	765	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.32	0.50	0.22	0.18	0.16

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.33

Intersection Signal Delay: 7.8

Intersection LOS: A

Intersection Capacity Utilization 57.7%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Future Volume (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3283	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3283	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40		14	14		40	18		20	20		18
Confl. Bikes (#/hr)			7			23						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	838	0	0	975	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.32			0.37							
Control Delay		4.4			3.4							
Queue Delay		0.1			0.1							
Total Delay		4.5			3.5							
LOS		A			A							
Approach Delay		4.5			3.5							
Approach LOS		A			A							
Queue Length 50th (m)		23.3			25.3							
Queue Length 95th (m)		27.2			m29.1							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			2615							
Starvation Cap Reductn		559			417							
Spillback Cap Reductn		46			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.41			0.44							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 57 (48%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.37

Intersection Signal Delay: 3.9

Intersection LOS: A

Intersection Capacity Utilization 32.7%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
34 s	36 s
 Ø5 (R)	
34 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	154	527	233	143	608	100	284	490	284	132	348	128
Future Volume (vph)	154	527	233	143	608	100	284	490	284	132	348	128
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.94	0.97		0.98		0.85	0.98	0.99		1.00	0.98	
Frt		0.954				0.850		0.945			0.960	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3035	0	1658	3252	1375	1674	3052	0	1510	1526	0
Flt Permitted	0.950			0.950			0.158			0.358		
Satd. Flow (perm)	1503	3035	0	1622	3252	1167	273	3052	0	567	1526	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		56				155		134			16	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	90		41	41		90	60		10	10		60
Confl. Bikes (#/hr)			22			10			36			5
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	154	527	233	143	608	100	284	490	284	132	348	128
Shared Lane Traffic (%)												
Lane Group Flow (vph)	154	760	0	143	608	100	284	774	0	132	476	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	20.0	37.0		18.0	35.0	35.0	21.0	65.0		44.0	44.0	
Total Split (%)	16.7%	30.8%		15.0%	29.2%	29.2%	17.5%	54.2%		36.7%	36.7%	
Maximum Green (s)	13.8	31.0		11.8	29.0	29.0	14.1	58.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	13.4	31.0		11.8	29.4	29.4	58.1	58.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.26		0.10	0.24	0.24	0.48	0.48		0.31	0.31	
v/c Ratio	0.87	0.92		0.88	0.76	0.25	0.96	0.50		0.75	0.99	
Control Delay	93.4	57.3		88.8	63.1	12.4	56.8	11.3		65.3	78.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	93.4	57.3		88.8	63.1	12.4	56.8	11.3		65.3	78.5	
LOS	F	E		F	E	B	E	B		E	E	
Approach Delay		63.4			61.4			23.5			75.6	
Approach LOS		E			E			C			E	
Queue Length 50th (m)	26.2	66.3		32.9	59.6	4.1	36.5	47.9		25.7	99.8	
Queue Length 95th (m)	#63.3	#105.4		m#39.5	m67.1	m6.4	m#69.8	49.5		#56.1	#162.9	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	183	826		163	795	402	296	1546		175	482	
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.84	0.92		0.88	0.76	0.25	0.96	0.50		0.75	0.99	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 52.8

Intersection LOS: D

Intersection Capacity Utilization 101.8%

ICU Level of Service G

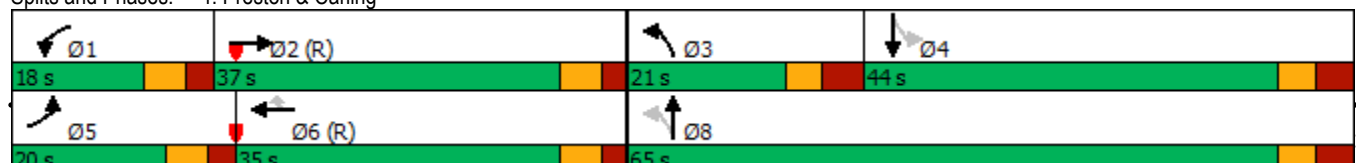
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

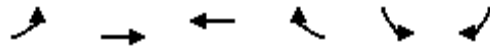
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

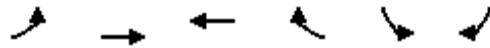
829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	388	630	672	215	240	168
Future Volume (vph)	388	630	672	215	240	168
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.93			0.80	0.98	0.80
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	1728	1498	1674	1427
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1559	3252	1728	1193	1649	1145
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				74		168
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	14	85
Confl. Bikes (#/hr)				16		23
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	388	630	672	215	240	168
Shared Lane Traffic (%)						
Lane Group Flow (vph)	388	630	672	215	240	168
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	26.0	81.0	55.0	55.0	39.0	39.0
Total Split (%)	21.7%	67.5%	45.8%	45.8%	32.5%	32.5%
Maximum Green (s)	20.1	75.3	49.3	49.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	28.5	83.7	49.3	49.3	24.6	24.6
Actuated g/C Ratio	0.24	0.70	0.41	0.41	0.20	0.20
v/c Ratio	0.98	0.28	0.95	0.40	0.71	0.46
Control Delay	80.9	6.8	58.1	18.5	55.0	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	80.9	6.8	58.1	18.5	55.0	9.2
LOS	F	A	E	B	E	A
Approach Delay		35.0	48.5		36.2	
Approach LOS		D	D		D	
Queue Length 50th (m)	87.7	20.5	138.3	20.6	49.8	0.0
Queue Length 95th (m)	m#149.5	m31.1	#207.6	39.4	68.0	15.6
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	396	2267	709	533	453	436
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.98	0.28	0.95	0.40	0.53	0.39

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 40.4

Intersection LOS: D

Intersection Capacity Utilization 101.2%

ICU Level of Service G

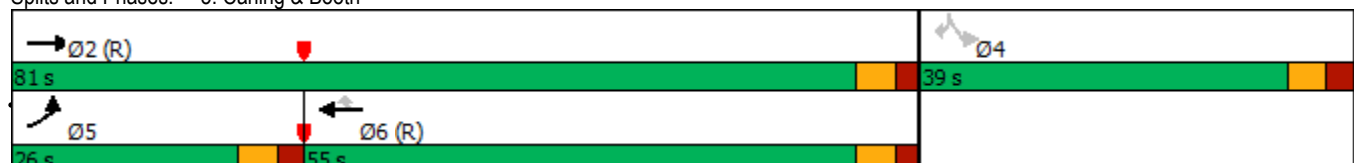
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	43	56	26	37	55	12	28	668	60	18	459	38
Future Volume (vph)	43	56	26	37	55	12	28	668	60	18	459	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.97	0.89	0.97	0.99		0.98	0.99	
Frt		0.972				0.850		0.988			0.989	
Flt Protected		0.983			0.980		0.950			0.950		
Satd. Flow (prot)	0	1558	0	0	1568	1498	1537	1692	0	1537	1650	0
Flt Permitted		0.865			0.853		0.433			0.286		
Satd. Flow (perm)	0	1336	0	0	1325	1339	678	1692	0	453	1650	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15				34		11			10	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		40	40		36	50		55	55		50
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	43	56	26	37	55	12	28	668	60	18	459	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	125	0	0	92	12	28	728	0	18	497	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.42			0.33	0.04	0.06	0.66		0.06	0.46	
Control Delay		28.9			30.4	2.8	5.8	10.2		5.9	8.7	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			30.4	2.8	5.8	10.4		5.9	8.7	
LOS		C			C	A	A	B		A	A	
Approach Delay		28.9			27.2			10.2			8.6	
Approach LOS		C			C			B			A	
Queue Length 50th (m)		13.3			11.0	0.0	1.3	53.8		0.8	29.9	
Queue Length 95th (m)		27.3			22.7	1.2	m2.7	45.4		3.0	48.3	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		302			288	317	438	1099		293	1071	
Starvation Cap Reductn		0			0	0	0	32		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.41			0.32	0.04	0.06	0.68		0.06	0.46	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 12.4

Intersection LOS: B

Intersection Capacity Utilization 83.5%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	709	46	10	555	5
Future Volume (vph)	1	0	3	0	0	0	8	709	46	10	555	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.992			0.999	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1704	0	0	1617	0
Flt Permitted		0.988						0.995			0.988	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1697	0	0	1599	0
Right Turn on Red			Yes				Yes			Yes		Yes
Satd. Flow (RTOR)		29						9			1	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	35		45	45		35
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	8	709	46	10	555	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	763	0	0	570	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.52			0.41	
Control Delay		0.0						5.7			2.5	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.7			2.5	
LOS		A						A			A	
Approach Delay								5.7			2.5	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						83.6			17.5	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1481			1394	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.52			0.41	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 4.3

Intersection LOS: A

Intersection Capacity Utilization 66.1%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	49	1	33	20	5	26	14	651	90	37	545	15
Future Volume (vph)	49	1	33	20	5	26	14	651	90	37	545	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Fr't		0.946			0.931			0.984			0.997	
Flt Protected		0.971			0.981			0.999			0.997	
Satd. Flow (prot)	0	1603	0	0	1594	0	0	1701	0	0	1689	0
Flt Permitted		0.971			0.981			0.999			0.997	
Satd. Flow (perm)	0	1603	0	0	1594	0	0	1701	0	0	1689	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							28		45	45		28
Confl. Bikes (#/hr)									21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	49	1	33	20	5	26	14	651	90	37	545	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	83	0	0	51	0	0	755	0	0	597	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization 67.0%	ICU Level of Service C											
Analysis Period (min) 15												

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (vph)	50	57	37	705	561	33
Future Volume (vph)	50	57	37	705	561	33
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.928				0.992	
Flt Protected	0.977			0.998		
Satd. Flow (prot)	1582	0	0	3279	1684	0
Flt Permitted	0.977			0.998		
Satd. Flow (perm)	1582	0	0	3279	1684	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	50	57	37	705	561	33
Shared Lane Traffic (%)						
Lane Group Flow (vph)	107	0	0	742	594	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	62.9%			ICU Level of Service B		
Analysis Period (min)	15					

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	678	281	2	2	223	317	1	4	3	239	4	420
Future Volume (vph)	678	281	2	2	223	317	1	4	3	239	4	420
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.92
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1666	1469
Flt Permitted	0.503			0.586				0.972			0.724	
Satd. Flow (perm)	863	1760	0	1009	1762	1462	0	1179	0	0	1192	1347
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				316		3				420
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	25		24	24		25
Confl. Bikes (#/hr)			3						1			2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	678	281	2	2	223	317	1	4	3	239	4	420
Shared Lane Traffic (%)												
Lane Group Flow (vph)	678	283	0	2	223	317	0	8	0	0	243	420
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
AM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		48.1	48.1	48.1		24.5			24.5	53.6
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.85	0.23		0.00	0.32	0.41		0.03			1.00	0.49
Control Delay	21.5	7.0		23.0	26.9	4.4		32.1			75.1	4.0
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	21.5	7.0		23.0	26.9	4.4		32.1			75.1	4.0
LOS	C	A		C	C	A		C			E	A
Approach Delay		17.3			13.8			32.1			30.0	
Approach LOS		B			B			C			C	
Queue Length 50th (m)	67.6	19.7		0.3	33.8	0.1		0.9			~47.2	12.3
Queue Length 95th (m)	#101.0	29.5		1.9	52.2	16.2		4.8			m#59.8	m14.3
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	810	1230		404	706	775		243			243	883
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.84	0.23		0.00	0.32	0.41		0.03			1.00	0.48

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 20.3

Intersection LOS: C

Intersection Capacity Utilization 96.9%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

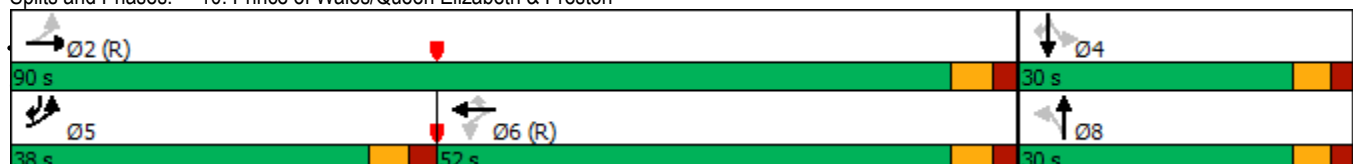
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	65	685	50	13	60	1222	157	125	25	150	182	10
Future Volume (vph)	65	685	50	13	60	1222	157	125	25	150	182	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.98		0.92		0.98		0.89	0.96		0.95	0.96	0.98
Frt			0.850				0.850			0.850		0.938
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1674	3252	1483	0	1661	3316	1498	1658	1745	1483	1674	1605
Flt Permitted	0.950				0.391			0.746			0.741	
Satd. Flow (perm)	1646	3252	1370	0	670	3316	1330	1253	1745	1410	1258	1605
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			48				157			150		7
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	35		20		20		35	30		30	30	
Confl. Bikes (#/hr)			10				5					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	2%	1%	2%	2%	1%	2%	2%	2%	1%	2%
Adj. Flow (vph)	65	685	50	13	60	1222	157	125	25	150	182	10
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	685	50	0	73	1222	157	125	25	150	182	17
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	7
Future Volume (vph)	7
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	8
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	7
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	18.0	99.0	99.0	81.0	81.0	81.0	81.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	12.9%	70.7%	70.7%	57.9%	57.9%	57.9%	57.9%	29.3%	29.3%	29.3%	29.3%	29.3%
Maximum Green (s)	12.8	92.6	92.6	74.6	74.6	74.6	74.6	33.9	33.9	33.9	33.9	33.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	10.3	99.0	99.0		85.8	85.8	85.8	27.5	27.5	27.5	27.5	27.5
Actuated g/C Ratio	0.07	0.71	0.71		0.61	0.61	0.61	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.53	0.30	0.05		0.18	0.60	0.18	0.51	0.07	0.38	0.74	0.05
Control Delay	77.3	8.7	2.4		7.9	11.9	1.0	56.2	42.8	9.1	69.8	29.8
Queue Delay	0.0	0.0	0.0		0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	8.7	2.4		7.9	11.9	1.0	56.2	42.8	9.1	69.8	29.8
LOS	E	A	A		A	B	A	E	D	A	E	C
Approach Delay		13.9				10.6			31.5			66.4
Approach LOS		B				B			C			E
Queue Length 50th (m)	16.2	36.0	0.2		4.8	115.1	0.5	27.3	5.0	0.0	41.8	2.0
Queue Length 95th (m)	30.2	45.8	4.2		11.1	145.8	1.9	45.2	12.1	15.9	65.7	7.8
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	153	2300	983		410	2033	876	303	422	455	304	393
Starvation Cap Reductn	0	0	0		0	107	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.30	0.05		0.18	0.63	0.18	0.41	0.06	0.33	0.60	0.04

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 24 (17%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 17.9

Intersection LOS: B

Intersection Capacity Utilization 86.0%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

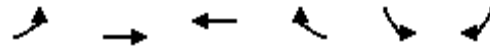




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
PM Peak Hour

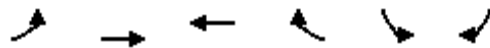
829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	80	858	1205	72	146	209
Future Volume (vph)	80	858	1205	72	146	209
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97			0.72	0.99	0.97
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	3316	1498	1674	1498
Flt Permitted	0.202				0.950	
Satd. Flow (perm)	292	3283	3316	1080	1663	1448
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				43		80
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	75			75	5	16
Confl. Bikes (#/hr)				5		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	80	858	1205	72	146	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	80	858	1205	72	146	209
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	102.0	102.0	102.0	102.0	38.0	38.0
Total Split (%)	72.9%	72.9%	72.9%	72.9%	27.1%	27.1%
Maximum Green (s)	96.7	96.7	96.7	96.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	104.4	104.4	104.4	104.4	24.4	24.4
Actuated g/C Ratio	0.75	0.75	0.75	0.75	0.17	0.17
v/c Ratio	0.37	0.35	0.49	0.09	0.51	0.66
Control Delay	12.5	6.1	2.7	1.4	56.6	41.4
Queue Delay	0.0	0.1	0.2	0.0	0.0	0.0
Total Delay	12.5	6.2	2.9	1.4	56.6	41.4
LOS	B	A	A	A	E	D
Approach Delay		6.7	2.8		47.6	
Approach LOS		A	A		D	
Queue Length 50th (m)	6.2	34.5	24.0	0.5	31.7	28.8
Queue Length 95th (m)	m25.6	45.3	26.9	1.8	50.8	53.0
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	217	2448	2473	816	381	393
Starvation Cap Reductn	0	577	470	0	0	0
Spillback Cap Reductn	0	0	78	0	0	1
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.46	0.60	0.09	0.38	0.53

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 139 (99%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 10.4

Intersection LOS: B

Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Future Volume (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3316	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3316	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40					40	25		35	35		25
Confl. Bikes (#/hr)			12			10			13			34
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		104.0			104.0							
Total Split (%)		74.3%			74.3%							
Maximum Green (s)		98.9			98.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		115.6			115.6							
Actuated g/C Ratio		0.83			0.83							
v/c Ratio		0.39			0.48							
Control Delay		4.0			2.1							
Queue Delay		0.1			0.1							
Total Delay		4.1			2.2							
LOS		A			A							
Approach Delay		4.1			2.2							
Approach LOS		A			A							
Queue Length 50th (m)		41.6			25.2							
Queue Length 95th (m)		39.3			m25.1							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2710			2737							
Starvation Cap Reductn		458			425							
Spillback Cap Reductn		231			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.46			0.57							

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 114 (81%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.48

Intersection Signal Delay: 3.0

Intersection LOS: A

Intersection Capacity Utilization 42.5%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
104 s	36 s
 Ø5 (R)	
104 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	26%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	165	601	374	348	889	66	348	426	167	109	364	124
Future Volume (vph)	165	601	374	348	889	66	348	426	167	109	364	124
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.96	0.97		0.98		0.86		0.98		0.97	0.97	
Frt		0.942				0.850		0.958			0.962	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	3009	0	1674	3316	1427	1674	3108	0	1537	1623	0
Flt Permitted	0.950			0.950			0.091			0.428		
Satd. Flow (perm)	1553	3009	0	1649	3316	1230	160	3108	0	672	1623	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		95				132		53			12	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	70		34	34		70	75		60	60		75
Confl. Bikes (#/hr)			13			11			16			10
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	165	601	374	348	889	66	348	426	167	109	364	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	165	975	0	348	889	66	348	593	0	109	488	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	21.0	44.0		28.0	51.0	51.0	24.0	68.0		44.0	44.0	
Total Split (%)	15.0%	31.4%		20.0%	36.4%	36.4%	17.1%	48.6%		31.4%	31.4%	
Maximum Green (s)	14.8	38.0		21.8	45.0	45.0	17.1	61.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	14.8	38.0		21.8	45.0	45.0	61.1	61.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.27		0.16	0.32	0.32	0.44	0.44		0.26	0.26	
v/c Ratio	0.97	1.10		1.34	0.83	0.14	1.37	0.43		0.61	1.11	
Control Delay	129.8	92.3		200.3	41.1	2.3	222.5	25.8		61.8	123.4	
Queue Delay	0.0	0.1		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	129.8	92.4		200.3	41.1	2.3	222.5	25.8		61.8	123.4	
LOS	F	F		F	D	A	F	C		E	F	
Approach Delay		97.8			81.7			98.5			112.2	
Approach LOS		F			F			F			F	
Queue Length 50th (m)	36.2	~132.4		~114.1	122.0	0.0	~103.8	49.4		24.5	~140.1	
Queue Length 95th (m)	#82.2	#172.5		m#127.4	m120.2	m0.0	#160.7	63.8		45.0	#202.9	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	170	885		260	1065	484	254	1386		178	438	
Starvation Cap Reductn	0	15		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.97	1.12		1.34	0.83	0.14	1.37	0.43		0.61	1.11	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.37

Intersection Signal Delay: 94.9

Intersection LOS: F

Intersection Capacity Utilization 124.5%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

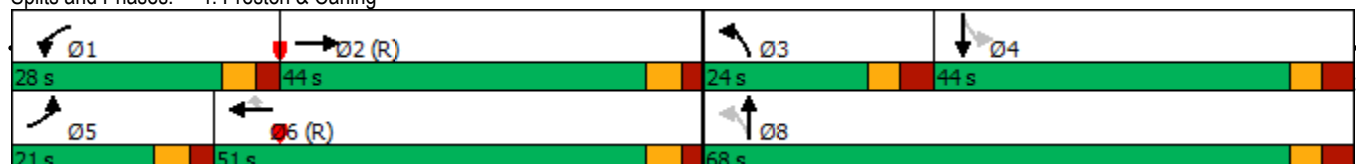
Queue shown is maximum after two cycles.

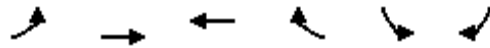
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling

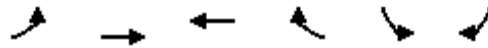




Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	264	674	908	104	308	326
Future Volume (vph)	264	674	908	104	308	326
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.94			0.77	0.98	0.75
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	1745	1498	1674	1483
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1577	3316	1745	1159	1647	1117
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				27		265
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	13	81
Confl. Bikes (#/hr)				11		45
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	264	674	908	104	308	326
Shared Lane Traffic (%)						
Lane Group Flow (vph)	264	674	908	104	308	326
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
2033 Background Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	25.0	101.0	76.0	76.0	39.0	39.0
Total Split (%)	17.9%	72.1%	54.3%	54.3%	27.9%	27.9%
Maximum Green (s)	19.1	95.3	70.3	70.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	22.2	98.4	70.3	70.3	29.9	29.9
Actuated g/C Ratio	0.16	0.70	0.50	0.50	0.21	0.21
v/c Ratio	1.00	0.29	1.04	0.17	0.88	0.73
Control Delay	82.4	5.7	74.7	14.8	77.9	20.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	82.4	5.7	74.7	14.8	77.9	20.9
LOS	F	A	E	B	E	C
Approach Delay		27.3	68.5		48.6	
Approach LOS		C	E		D	
Queue Length 50th (m)	~77.0	39.9	~248.6	10.2	74.4	12.7
Queue Length 95th (m)	m#86.9	m40.4	#320.0	20.4	#113.7	47.4
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	265	2330	876	595	388	465
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.29	1.04	0.17	0.79	0.70

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 66 (47%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.04

Intersection Signal Delay: 48.7

Intersection LOS: D

Intersection Capacity Utilization 107.4%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	41	34	46	121	31	82	515	59	17	531	54
Future Volume (vph)	29	41	34	46	121	31	82	515	59	17	531	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.97	0.84	0.97	0.98		0.95	0.99	
Frt		0.956				0.850		0.985			0.986	
Flt Protected		0.986			0.986		0.950			0.950		
Satd. Flow (prot)	0	1578	0	0	1738	1498	1674	1679	0	1674	1673	0
Flt Permitted		0.874			0.897		0.385			0.392		
Satd. Flow (perm)	0	1368	0	0	1535	1261	656	1679	0	656	1673	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				31		14			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		50	50		40	55		80	80		55
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	29	41	34	46	121	31	82	515	59	17	531	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	104	0	0	167	31	82	574	0	17	585	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.2			17.2	17.2	61.7	61.7		61.7	61.7	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.37			0.57	0.12	0.18	0.50		0.04	0.51	
Control Delay		28.8			41.8	12.2	2.5	5.1		4.9	8.5	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.8			41.8	12.2	2.5	5.2		4.9	8.5	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.8			37.1			4.9			8.4	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.1			24.4	0.0	2.4	39.5		0.8	37.6	
Queue Length 95th (m)		24.5			42.6	6.6	0.5	1.8		2.6	58.8	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		283			296	268	450	1156		450	1151	
Starvation Cap Reductn		0			0	0	0	85		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.37			0.56	0.12	0.18	0.54		0.04	0.51	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 11.9

Intersection LOS: B

Intersection Capacity Utilization 88.5%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	613	27	6	598	15
Future Volume (vph)	4	2	10	0	0	0	8	613	27	6	598	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Flt		0.916						0.994			0.997	
Flt Protected		0.988						0.999				
Satd. Flow (prot)	0	1494	0	0	0	0	0	1709	0	0	1719	0
Flt Permitted		0.988						0.993			0.995	
Satd. Flow (perm)	0	1466	0	0	0	0	0	1698	0	0	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10						6			3	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	50		47	47		50
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	10	0	0	0	8	613	27	6	598	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	0	0	0	648	0	0	619	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.45			0.43	
Control Delay		21.9						5.1			3.7	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.9						5.1			3.7	
LOS		C						A			A	
Approach Delay		21.9						5.1			3.7	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						29.4			22.6	
Queue Length 95th (m)		5.8						60.2			34.2	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1428			1437	
Starvation Cap Reductn		0						0			55	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.06						0.45			0.45	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.45

Intersection Signal Delay: 4.7

Intersection LOS: A

Intersection Capacity Utilization 60.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	34	4	25	22	2	17	37	552	48	23	603	5
Future Volume (vph)	34	4	25	22	2	17	37	552	48	23	603	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt	0.946				0.944				0.990		0.999	
Flt Protected	0.974				0.974				0.997		0.998	
Satd. Flow (prot)	0	1608	0	0	1605	0	0	1708	0	0	1724	0
Flt Permitted	0.974				0.974				0.997		0.998	
Satd. Flow (perm)	0	1608	0	0	1605	0	0	1708	0	0	1724	0
Link Speed (k/h)	50				50				50		50	
Link Distance (m)	113.6				154.3				71.5		73.8	
Travel Time (s)	8.2				11.1				5.1		5.3	
Confl. Peds. (#/hr)							46		47		47	
Confl. Bikes (#/hr)									21		14	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%
Adj. Flow (vph)	34	4	25	22	2	17	37	552	48	23	603	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	63	0	0	41	0	0	637	0	0	631	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0				0.0				0.0		0.0	
Link Offset(m)	-2.0				-2.0				0.0		0.0	
Crosswalk Width(m)	5.0				5.0				5.0		5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control	Stop				Stop				Free		Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	61.3%											
Analysis Period (min)	15											
ICU Level of Service B												

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	26	30	44	611	567	75
Future Volume (vph)	26	30	44	611	567	75
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.928				0.984	
Flt Protected	0.977			0.997		
Satd. Flow (prot)	1582	0	0	3276	1702	0
Flt Permitted	0.977			0.997		
Satd. Flow (perm)	1582	0	0	3276	1702	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	26	30	44	611	567	75
Shared Lane Traffic (%)						
Lane Group Flow (vph)	56	0	0	655	642	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 63.2%				ICU Level of Service B		
Analysis Period (min) 15						

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	484	294	4	2	430	433	5	2	1	414	0	654
Future Volume (vph)	484	294	4	2	430	433	5	2	1	414	0	654
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.90		0.94		0.93			0.83	0.77
Frt		0.998				0.850		0.983				0.850
Flt Protected	0.950			0.950				0.970			0.950	
Satd. Flow (prot)	1642	1755	0	1674	1762	1498	0	1640	0	0	1674	1483
Flt Permitted	0.156			0.578				0.844			0.752	
Satd. Flow (perm)	270	1755	0	914	1762	1403	0	1364	0	0	1095	1137
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				176		1				99
Link Speed (k/h)		60			40			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			18.3			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	80		65	65		80
Confl. Bikes (#/hr)			2									2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	484	294	4	2	430	433	5	2	1	414	0	654
Shared Lane Traffic (%)												
Lane Group Flow (vph)	484	298	0	2	430	433	0	8	0	0	414	654
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2033 Background Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		36.2	36.2	36.2		43.5			43.5	75.5
Actuated g/C Ratio	0.58	0.58		0.28	0.28	0.28		0.33			0.33	0.58
v/c Ratio	0.97	0.29		0.01	0.88	0.84		0.02			1.13	0.83
Control Delay	64.9	15.0		34.5	65.0	41.4		27.5			127.6	28.3
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	64.9	15.0		34.5	65.0	41.4		27.5			127.6	28.3
LOS	E	B		C	E	D		C			F	C
Approach Delay		45.9			53.1			27.5			66.8	
Approach LOS		D			D			C			E	
Queue Length 50th (m)	90.3	34.2		0.3	97.5	60.8		1.1			~113.1	79.3
Queue Length 95th (m)	#153.1	49.7		2.3	#149.9	#112.5		4.5			#170.5	118.2
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	502	1011		254	490	517		457			366	791
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.96	0.29		0.01	0.88	0.84		0.02			1.13	0.83

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.13

Intersection Signal Delay: 56.3

Intersection LOS: E

Intersection Capacity Utilization 106.7%

ICU Level of Service G

Analysis Period (min) 15

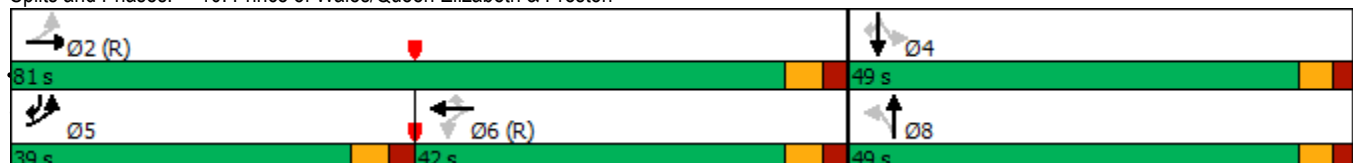
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	65	685	50	13	60	1222	157	125	25	150	182	10
Future Volume (vph)	65	685	50	13	60	1222	157	125	25	150	182	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.98		0.92		0.98		0.89	0.96		0.95	0.96	0.98
Frt			0.850				0.850			0.850		0.938
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1674	3252	1483	0	1661	3316	1498	1658	1745	1483	1674	1605
Flt Permitted	0.950				0.391			0.746			0.741	
Satd. Flow (perm)	1646	3252	1370	0	670	3316	1330	1253	1745	1410	1258	1605
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			48				157			150		7
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	35		20		20		35	30		30	30	
Confl. Bikes (#/hr)			10				5					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	2%	1%	2%	2%	1%	2%	2%	2%	1%	2%
Adj. Flow (vph)	65	685	50	13	60	1222	157	125	25	150	182	10
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	685	50	0	73	1222	157	125	25	150	182	17
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	7
Future Volume (vph)	7
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	8
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	7
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	18.0	99.0	99.0	81.0	81.0	81.0	81.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	12.9%	70.7%	70.7%	57.9%	57.9%	57.9%	57.9%	29.3%	29.3%	29.3%	29.3%	29.3%
Maximum Green (s)	12.8	92.6	92.6	74.6	74.6	74.6	74.6	33.9	33.9	33.9	33.9	33.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	10.3	99.0	99.0		85.8	85.8	85.8	27.5	27.5	27.5	27.5	27.5
Actuated g/C Ratio	0.07	0.71	0.71		0.61	0.61	0.61	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.53	0.30	0.05		0.18	0.60	0.18	0.51	0.07	0.38	0.74	0.05
Control Delay	77.3	8.7	2.4		8.2	12.9	1.1	56.2	42.8	9.1	69.8	29.8
Queue Delay	0.0	0.0	0.0		0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	8.7	2.4		8.2	13.0	1.1	56.2	42.8	9.1	69.8	29.8
LOS	E	A	A		A	B	A	E	D	A	E	C
Approach Delay		13.9				11.5			31.5			66.4
Approach LOS		B				B			C			E
Queue Length 50th (m)	16.2	36.0	0.2		5.0	140.6	0.9	27.3	5.0	0.0	41.8	2.0
Queue Length 95th (m)	30.2	45.8	4.2		12.0	167.0	2.7	45.2	12.1	15.9	65.7	7.8
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	153	2300	983		410	2033	876	303	422	455	304	393
Starvation Cap Reductn	0	0	0		0	107	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.30	0.05		0.18	0.63	0.18	0.41	0.06	0.33	0.60	0.04

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 24 (17%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 18.3

Intersection LOS: B

Intersection Capacity Utilization 86.0%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

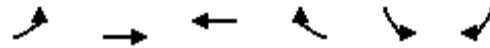




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
PM Peak Hour

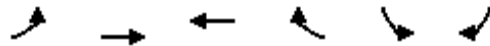
829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	80	858	1205	72	146	209
Future Volume (vph)	80	858	1205	72	146	209
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97			0.72	0.99	0.97
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	3316	1498	1674	1498
Flt Permitted	0.202				0.950	
Satd. Flow (perm)	292	3283	3316	1080	1663	1448
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				43		80
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	75			75	5	16
Confl. Bikes (#/hr)				5		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	80	858	1205	72	146	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	80	858	1205	72	146	209
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	102.0	102.0	102.0	102.0	38.0	38.0
Total Split (%)	72.9%	72.9%	72.9%	72.9%	27.1%	27.1%
Maximum Green (s)	96.7	96.7	96.7	96.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	104.4	104.4	104.4	104.4	24.4	24.4
Actuated g/C Ratio	0.75	0.75	0.75	0.75	0.17	0.17
v/c Ratio	0.37	0.35	0.49	0.09	0.51	0.66
Control Delay	12.5	6.1	2.9	1.5	56.6	41.4
Queue Delay	0.0	0.1	0.2	0.0	0.0	0.0
Total Delay	12.5	6.2	3.1	1.5	56.6	41.4
LOS	B	A	A	A	E	D
Approach Delay		6.7	3.1		47.6	
Approach LOS		A	A		D	
Queue Length 50th (m)	6.2	34.5	26.6	0.8	31.7	28.8
Queue Length 95th (m)	m25.6	45.3	29.6	2.3	50.8	53.0
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	217	2448	2473	816	381	393
Starvation Cap Reductn	0	577	470	0	0	0
Spillback Cap Reductn	0	0	193	0	0	4
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.46	0.60	0.09	0.38	0.54

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 139 (99%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 10.6

Intersection LOS: B

Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Future Volume (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3316	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3316	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40					40	25		35	35		25
Confl. Bikes (#/hr)			12			10			13			34
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1045	0	0	1311	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		104.0			104.0							
Total Split (%)		74.3%			74.3%							
Maximum Green (s)		98.9			98.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		115.6			115.6							
Actuated g/C Ratio		0.83			0.83							
v/c Ratio		0.39			0.48							
Control Delay		4.0			2.6							
Queue Delay		0.1			0.1							
Total Delay		4.1			2.7							
LOS		A			A							
Approach Delay		4.1			2.7							
Approach LOS		A			A							
Queue Length 50th (m)		41.6			27.9							
Queue Length 95th (m)		39.3			m30.4							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2710			2737							
Starvation Cap Reductn		458			372							
Spillback Cap Reductn		126			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.46			0.55							

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 114 (81%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.48

Intersection Signal Delay: 3.3

Intersection LOS: A

Intersection Capacity Utilization 42.5%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
104 s	36 s
 Ø5 (R)	
104 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	26%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	165	531	374	258	889	66	268	426	167	109	314	124
Future Volume (vph)	165	531	374	258	889	66	268	426	167	109	314	124
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.96	0.97		0.98		0.86	0.97	0.98		0.97	0.97	
Frt		0.938				0.850		0.958			0.958	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	2990	0	1674	3316	1427	1674	3108	0	1537	1610	0
Flt Permitted	0.950			0.950			0.126			0.428		
Satd. Flow (perm)	1553	2990	0	1646	3316	1230	216	3108	0	672	1610	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		125				132		53			14	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	70		34	34		70	75		60	60		75
Confl. Bikes (#/hr)			13			11			16			10
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	165	531	374	258	889	66	268	426	167	109	314	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	165	905	0	258	889	66	268	593	0	109	438	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	21.0	44.0		28.0	51.0	51.0	24.0	68.0		44.0	44.0	
Total Split (%)	15.0%	31.4%		20.0%	36.4%	36.4%	17.1%	48.6%		31.4%	31.4%	
Maximum Green (s)	14.8	38.0		21.8	45.0	45.0	17.1	61.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	14.8	38.0		21.8	45.0	45.0	61.1	61.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.27		0.16	0.32	0.32	0.44	0.44		0.26	0.26	
v/c Ratio	0.97	1.00		0.99	0.83	0.14	0.99	0.43		0.61	1.00	
Control Delay	130.0	59.0		80.1	40.8	2.5	85.4	25.8		61.8	93.5	
Queue Delay	0.0	15.9		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	130.0	74.9		80.1	40.8	2.5	85.4	25.8		61.8	93.5	
LOS	F	E		F	D	A	F	C		E	F	
Approach Delay		83.4			47.1			44.3			87.1	
Approach LOS		F			D			D			F	
Queue Length 50th (m)	35.6	~85.1		63.4	122.6	0.0	50.6	49.4		24.5	~110.0	
Queue Length 95th (m)	#82.4	#146.5		m#80.4	m123.0	m0.0	#103.3	63.8		45.0	#174.5	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	170	902		260	1065	484	272	1386		178	436	
Starvation Cap Reductn	0	43		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.97	1.05		0.99	0.83	0.14	0.99	0.43		0.61	1.00	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 62.9

Intersection LOS: E

Intersection Capacity Utilization 112.6%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

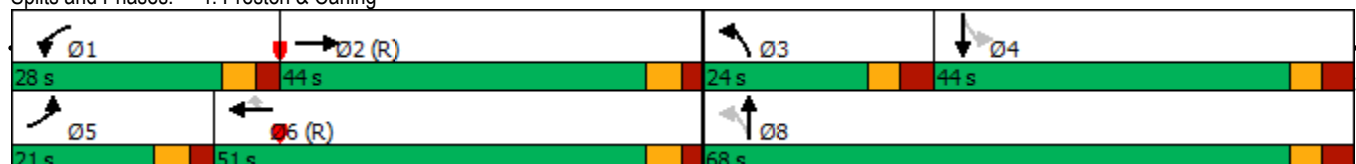
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

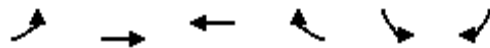
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
PM Peak Hour

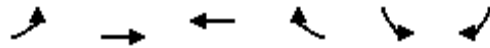
829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	264	674	878	104	308	326
Future Volume (vph)	264	674	878	104	308	326
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.94			0.77	0.98	0.75
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	1745	1498	1674	1483
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1573	3316	1745	1159	1647	1117
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				28		265
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	13	81
Confl. Bikes (#/hr)				11		45
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	264	674	878	104	308	326
Shared Lane Traffic (%)						
Lane Group Flow (vph)	264	674	878	104	308	326
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	25.0	101.0	76.0	76.0	39.0	39.0
Total Split (%)	17.9%	72.1%	54.3%	54.3%	27.9%	27.9%
Maximum Green (s)	19.1	95.3	70.3	70.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	22.2	98.4	70.3	70.3	29.9	29.9
Actuated g/C Ratio	0.16	0.70	0.50	0.50	0.21	0.21
v/c Ratio	1.00	0.29	1.00	0.17	0.88	0.73
Control Delay	92.7	5.7	65.8	14.6	77.9	20.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	92.7	5.7	65.8	14.6	77.9	20.9
LOS	F	A	E	B	E	C
Approach Delay		30.2	60.4		48.6	
Approach LOS		C	E		D	
Queue Length 50th (m)	~77.9	36.4	~220.1	10.1	74.4	12.7
Queue Length 95th (m)	m#99.5	m41.4	#304.2	20.3	#113.7	47.4
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	265	2330	876	595	388	465
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.29	1.00	0.17	0.79	0.70

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 66 (47%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 46.4

Intersection LOS: D

Intersection Capacity Utilization 105.7%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	41	34	46	121	31	82	515	59	17	531	54
Future Volume (vph)	29	41	34	46	121	31	82	515	59	17	531	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.97	0.84	0.97	0.98		0.95	0.99	
Frt		0.956				0.850		0.985			0.986	
Flt Protected		0.986			0.986		0.950			0.950		
Satd. Flow (prot)	0	1578	0	0	1738	1498	1674	1679	0	1674	1673	0
Flt Permitted		0.874			0.897		0.385			0.392		
Satd. Flow (perm)	0	1368	0	0	1535	1261	656	1679	0	656	1673	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				31		14			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		50	50		40	55		80	80		55
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	29	41	34	46	121	31	82	515	59	17	531	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	104	0	0	167	31	82	574	0	17	585	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.2			17.2	17.2	61.7	61.7		61.7	61.7	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.37			0.57	0.12	0.18	0.50		0.04	0.51	
Control Delay		28.8			41.8	12.2	2.5	5.1		4.9	8.5	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.8			41.8	12.2	2.5	5.2		4.9	8.5	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.8			37.1			4.9			8.4	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.1			24.4	0.0	2.4	39.5		0.8	37.6	
Queue Length 95th (m)		24.5			42.6	6.6	0.5	1.8		2.6	58.8	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		283			296	268	450	1156		450	1151	
Starvation Cap Reductn		0			0	0	0	85		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.37			0.56	0.12	0.18	0.54		0.04	0.51	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 11.9

Intersection LOS: B

Intersection Capacity Utilization 88.5%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	613	27	6	598	15
Future Volume (vph)	4	2	10	0	0	0	8	613	27	6	598	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Flt		0.916						0.994			0.997	
Flt Protected		0.988						0.999				
Satd. Flow (prot)	0	1494	0	0	0	0	0	1709	0	0	1719	0
Flt Permitted		0.988						0.993			0.995	
Satd. Flow (perm)	0	1466	0	0	0	0	0	1698	0	0	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10						6			3	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	50		47	47		50
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	10	0	0	0	8	613	27	6	598	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	0	0	0	648	0	0	619	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.45			0.43	
Control Delay		21.9						5.1			3.7	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.9						5.1			3.7	
LOS		C						A			A	
Approach Delay		21.9						5.1			3.7	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						29.4			22.6	
Queue Length 95th (m)		5.8						60.2			34.2	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1428			1437	
Starvation Cap Reductn		0						0			55	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.06						0.45			0.45	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.45

Intersection Signal Delay: 4.7

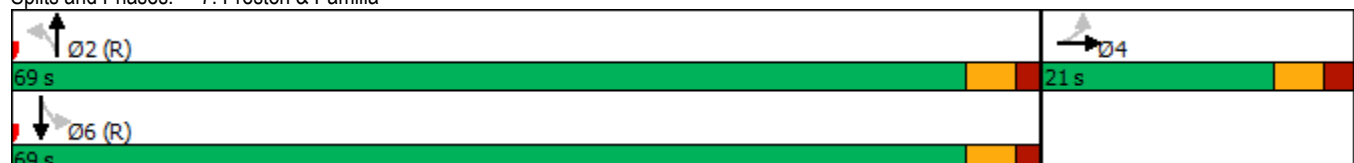
Intersection LOS: A

Intersection Capacity Utilization 60.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	34	4	25	22	2	17	37	552	48	23	603	5
Future Volume (vph)	34	4	25	22	2	17	37	552	48	23	603	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.946			0.944			0.990			0.999	
Flt Protected		0.974			0.974			0.997			0.998	
Satd. Flow (prot)	0	1608	0	0	1605	0	0	1708	0	0	1724	0
Flt Permitted		0.974			0.974			0.997			0.998	
Satd. Flow (perm)	0	1608	0	0	1605	0	0	1708	0	0	1724	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							46		47	47		46
Confl. Bikes (#/hr)									21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%
Adj. Flow (vph)	34	4	25	22	2	17	37	552	48	23	603	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	63	0	0	41	0	0	637	0	0	631	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	61.3%											
Analysis Period (min)	15											
ICU Level of Service B												

9: Preston & Sidney
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	26	30	44	611	567	75
Future Volume (vph)	26	30	44	611	567	75
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.928				0.984	
Flt Protected	0.977			0.997		
Satd. Flow (prot)	1582	0	0	3276	1702	0
Flt Permitted	0.977			0.997		
Satd. Flow (perm)	1582	0	0	3276	1702	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	26	30	44	611	567	75
Shared Lane Traffic (%)						
Lane Group Flow (vph)	56	0	0	655	642	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 63.2%				ICU Level of Service B		
Analysis Period (min) 15						

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	484	294	4	2	430	433	5	2	1	364	0	654
Future Volume (vph)	484	294	4	2	430	433	5	2	1	364	0	654
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.90		0.94		0.92			0.83	0.77
Frt		0.998				0.850		0.983				0.850
Flt Protected	0.950			0.950				0.970			0.950	
Satd. Flow (prot)	1642	1755	0	1674	1762	1498	0	1640	0	0	1674	1483
Flt Permitted	0.156			0.578				0.853			0.752	
Satd. Flow (perm)	270	1755	0	914	1762	1403	0	1365	0	0	1095	1137
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				176		1				99
Link Speed (k/h)		60			40			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			18.3			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	80		65	65		80
Confl. Bikes (#/hr)			2									2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	484	294	4	2	430	433	5	2	1	364	0	654
Shared Lane Traffic (%)												
Lane Group Flow (vph)	484	298	0	2	430	433	0	8	0	0	364	654
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

10: Prince of Wales/Queen Elizabeth & Preston
PM Peak Hour

829 Carling Avenue
2033 Background Traffic (demand rationalization)

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		36.2	36.2	36.2		43.5			43.5	75.5
Actuated g/C Ratio	0.58	0.58		0.28	0.28	0.28		0.33			0.33	0.58
v/c Ratio	0.97	0.29		0.01	0.88	0.84		0.02			0.99	0.83
Control Delay	64.9	15.0		34.5	65.0	41.4		27.5			89.2	28.3
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	64.9	15.0		34.5	65.0	41.4		27.5			89.2	28.3
LOS	E	B		C	E	D		C			F	C
Approach Delay		45.9			53.1			27.5			50.1	
Approach LOS		D			D			C			D	
Queue Length 50th (m)	90.3	34.2		0.3	97.5	60.8		1.1			85.3	79.3
Queue Length 95th (m)	#153.1	49.7		2.3	#149.9	#112.5		4.5			#143.5	118.2
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	502	1011		254	490	517		457			366	791
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.96	0.29		0.01	0.88	0.84		0.02			0.99	0.83

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 49.8

Intersection LOS: D

Intersection Capacity Utilization 106.7%

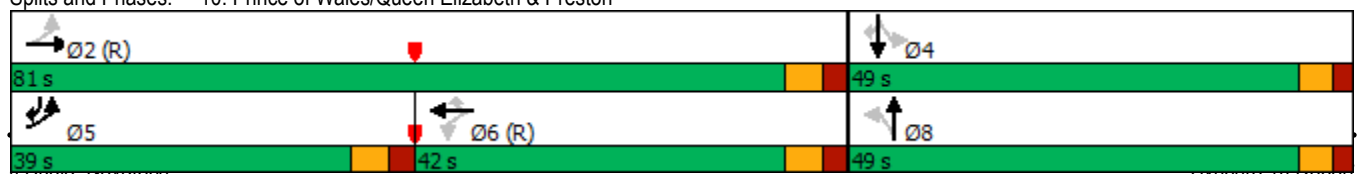
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



Signal Timing Diagram

Dynamic Report

APPENDIX L

Transportation Demand Management

TRANSPORTATION DEMAND MANAGEMENT

TDM-Supportive Development Design and Infrastructure Checklist

TDM-Supportive Development Design and Infrastructure Checklist: *Non-Residential Developments (office, institutional, retail or industrial)*

Legend	
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance

TDM-supportive design & infrastructure measures: <i>Non-residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
1. WALKING & CYCLING: ROUTES		
1.1 Building location & access points		
BASIC	1.1.1 Locate building close to the street, and do not locate parking areas between the street and building entrances	☒
BASIC	1.1.2 Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	☒
BASIC	1.1.3 Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	☒
1.2 Facilities for walking & cycling		
REQUIRED	1.2.1 Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (<i>see Official Plan policy 4.3.3</i>)	☒
REQUIRED	1.2.2 Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (<i>see Official Plan policy 4.3.12</i>)	☒

TDM-supportive design & infrastructure measures: <i>Non-residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3 Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (<i>see Official Plan policy 4.3.10</i>)	☒
REQUIRED	1.2.4 Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (<i>see Official Plan policy 4.3.10</i>)	☒
REQUIRED	1.2.5 Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (<i>see Official Plan policy 4.3.11</i>)	☒
BASIC	1.2.6 Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	☒
BASIC	1.2.7 Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	☐
BASIC	1.2.8 Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	☐
1.3 Amenities for walking & cycling		
BASIC	1.3.1 Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	☒
BASIC	1.3.2 Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	☐

TDM-supportive design & infrastructure measures: <i>Non-residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
2. WALKING & CYCLING: END-OF-TRIP FACILITIES		
2.1 Bicycle parking		
REQUIRED	2.1.1 Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see <i>Official Plan policy 4.3.6</i>)	☒
REQUIRED	2.1.2 Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see <i>Zoning By-law Section 111</i>)	☒
REQUIRED	2.1.3 Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see <i>Zoning By-law Section 111</i>)	☒
BASIC	2.1.4 Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	☐
BETTER	2.1.5 Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	☒
2.2 Secure bicycle parking		
REQUIRED	2.2.1 Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see <i>Zoning By-law Section 111</i>)	☒
BETTER	2.2.2 Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	☒
2.3 Shower & change facilities		
BASIC	2.3.1 Provide shower and change facilities for the use of active commuters	☐
BETTER	2.3.2 In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	☐
2.4 Bicycle repair station		
BETTER	2.4.1 Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	☐

TDM-supportive design & infrastructure measures: <i>Non-residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
3. TRANSIT		
3.1 Customer amenities		
BASIC	3.1.1 Provide shelters, lighting and benches at any on-site transit stops	☐
BASIC	3.1.2 Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	☐
BETTER	3.1.3 Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	☐
4. RIDESHARING		
4.1 Pick-up & drop-off facilities		
BASIC	4.1.1 Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	☐
4.2 Carpool parking		
BASIC	4.2.1 Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	☐
BETTER	4.2.2 At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	☐
5. CARSHARING & BIKESHARING		
5.1 Carshare parking spaces		
BETTER	5.1.1 Provide carshare parking spaces in permitted non-residential zones, occupying either required or provided parking spaces (<i>see Zoning By-law Section 94</i>)	☐
5.2 Bikeshare station location		
BETTER	5.2.1 Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	☐

TDM-supportive design & infrastructure measures: <i>Non-residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
6. PARKING		
6.1 Number of parking spaces		
REQUIRED	6.1.1 Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	☒
BASIC	6.1.2 Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	☐
BASIC	6.1.3 Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (<i>see Zoning By-law Section 104</i>)	☐
BETTER	6.1.4 Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (<i>see Zoning By-law Section 111</i>)	☐
6.2 Separate long-term & short-term parking areas		
BETTER	6.2.1 Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	☐
7. OTHER		
7.1 On-site amenities to minimize off-site trips		
BETTER	7.1.1 Provide on-site amenities to minimize mid-day or mid-commute errands	☐

TDM-Supportive Development Design and Infrastructure Checklist: *Residential Developments (multi-family or condominium)*

Legend	
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
1. WALKING & CYCLING: ROUTES		
1.1 Building location & access points		
BASIC	1.1.1 Locate building close to the street, and do not locate parking areas between the street and building entrances	☒
BASIC	1.1.2 Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	☒
BASIC	1.1.3 Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	☒
1.2 Facilities for walking & cycling		
REQUIRED	1.2.1 Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (<i>see Official Plan policy 4.3.3</i>)	☒
REQUIRED	1.2.2 Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (<i>see Official Plan policy 4.3.12</i>)	☒

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3 Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (<i>see Official Plan policy 4.3.10</i>)	☒
REQUIRED	1.2.4 Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (<i>see Official Plan policy 4.3.10</i>)	☒
REQUIRED	1.2.5 Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (<i>see Official Plan policy 4.3.11</i>)	☒
BASIC	1.2.6 Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	☒
BASIC	1.2.7 Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	☐
BASIC	1.2.8 Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	☐
1.3 Amenities for walking & cycling		
BASIC	1.3.1 Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	☒
BASIC	1.3.2 Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	☐

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
2. WALKING & CYCLING: END-OF-TRIP FACILITIES		
2.1 Bicycle parking		
REQUIRED	2.1.1 Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see <i>Official Plan policy 4.3.6</i>)	☒
REQUIRED	2.1.2 Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see <i>Zoning By-law Section 111</i>)	☒
REQUIRED	2.1.3 Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see <i>Zoning By-law Section 111</i>)	☒
BASIC	2.1.4 Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	☐
2.2 Secure bicycle parking		
REQUIRED	2.2.1 Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see <i>Zoning By-law Section 111</i>)	☒
BETTER	2.2.2 Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multi-family residential developments	☐
2.3 Bicycle repair station		
BETTER	2.3.1 Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	☐
3. TRANSIT		
3.1 Customer amenities		
BASIC	3.1.1 Provide shelters, lighting and benches at any on-site transit stops	☐
BASIC	3.1.2 Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	☐
BETTER	3.1.3 Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	☐

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
4. RIDESHARING		
4.1 Pick-up & drop-off facilities		
BASIC	4.1.1 Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	☐
5. CARSHARING & BIKESHARING		
5.1 Carshare parking spaces		
BETTER	5.1.1 Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see <i>Zoning By-law Section 94</i>)	☐
5.2 Bikeshare station location		
BETTER	5.2.1 Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	☐
6. PARKING		
6.1 Number of parking spaces		
REQUIRED	6.1.1 Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	☒
BASIC	6.1.2 Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	☐
BASIC	6.1.3 Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see <i>Zoning By-law Section 104</i>)	☐
BETTER	6.1.4 Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see <i>Zoning By-law Section 111</i>)	☐
6.2 Separate long-term & short-term parking areas		
BETTER	6.2.1 Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	☐

TRANSPORTATION DEMAND MANAGEMENT

TDM Measures Checklist

TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

Legend	
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance
★	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

TDM measures: <i>Residential developments</i>		Check if proposed & add descriptions
1. TDM PROGRAM MANAGEMENT		
1.1 Program coordinator		
BASIC ★	1.1.1 Designate an internal coordinator, or contract with an external coordinator	☐
1.2 Travel surveys		
BETTER	1.2.1 Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	☐
2. WALKING AND CYCLING		
2.1 Information on walking/cycling routes & destinations		
BASIC	2.1.1 Display local area maps with walking/cycling access routes and key destinations at major entrances (<i>multi-family, condominium</i>)	☒
2.2 Bicycle skills training		
BETTER	2.2.1 Offer on-site cycling courses for residents, or subsidize off-site courses	☐

TDM measures: <i>Residential developments</i>		Check if proposed & add descriptions
3. TRANSIT		
3.1 Transit information		
BASIC	3.1.1 Display relevant transit schedules and route maps at entrances (<i>multi-family, condominium</i>)	☒
BETTER	3.1.2 Provide real-time arrival information display at entrances (<i>multi-family, condominium</i>)	☐
3.2 Transit fare incentives		
BASIC ★	3.2.1 Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	☐
BETTER	3.2.2 Offer at least one year of free monthly transit passes on residence purchase/move-in	☐
3.3 Enhanced public transit service		
BETTER ★	3.3.1 Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (<i>subdivision</i>)	☐
3.4 Private transit service		
BETTER	3.4.1 Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	☐
4. CARSHARING & BIKESHARING		
4.1 Bikeshare stations & memberships		
BETTER	4.1.1 Contract with provider to install on-site bikeshare station (<i>multi-family</i>)	☐
BETTER	4.1.2 Provide residents with bikeshare memberships, either free or subsidized (<i>multi-family</i>)	☐
4.2 Carshare vehicles & memberships		
BETTER	4.2.1 Contract with provider to install on-site carshare vehicles and promote their use by residents	☐
BETTER	4.2.2 Provide residents with carshare memberships, either free or subsidized	☐
5. PARKING		
5.1 Priced parking		
BASIC ★	5.1.1 Unbundle parking cost from purchase price (<i>condominium</i>)	☐
BASIC ★	5.1.2 Unbundle parking cost from monthly rent (<i>multi-family</i>)	☒

TDM measures: <i>Residential developments</i>		Check if proposed & add descriptions
6. TDM MARKETING & COMMUNICATIONS		
6.1 Multimodal travel information		
BASIC	★ 6.1.1 Provide a multimodal travel option information package to new residents	☒
6.2 Personalized trip planning		
BETTER	★ 6.2.1 Offer personalized trip planning to new residents	☐

APPENDIX M

MMLOS Review

Segment MMLOS Analysis

This section provides a review of the boundary streets Preston Street and Sidney Street, using complete streets principles. The *Multi-Modal Level of Service (MMLOS) Guidelines*, produced by IBI Group in October 2015, were used to evaluate the levels of service for each alternative mode of transportation on the boundary streets, based on the targets for areas 'within 600m of a rapid transit station.'

Exhibit 4 of the MMLOS Guidelines has been used to evaluate the segment pedestrian level of service (PLOS) of the boundary streets. Exhibit 22 of the MMLOS Guidelines suggest a target PLOS A for all roadways within 600m of a rapid transit station. The results of the segment PLOS analysis are summarized in **Table 1**.

Exhibit 11 of the MMLOS Guidelines has been used to evaluate the segment bicycle level of service (BLOS) of the boundary streets. Within 600m of a rapid transit station, Exhibit 22 of the MMLOS Guidelines suggest a target BLOS B for roadways with a local cycling route designation (Preston Street), and a target BLOS D for roadways with no cycling route designation (Sidney Street). The results of the segment BLOS analysis are summarized in **Table 2**.

Exhibit 15 of the MMLOS Guidelines has been used to evaluate the segment transit level of service (TLOS) of Preston Street only. Despite having no TLOS target, Preston Street has been evaluated for TLOS, as it currently has transit service. The results of the segment TLOS analysis are summarized in **Table 3**.

Exhibit 20 of the MMLOS Guidelines has been used to evaluate the segment truck level of service (TkLOS) of Preston Street only. Within 600m of a rapid transit station, Exhibit 22 of the MMLOS Guidelines suggest a target TkLOS D for arterial roadways with a truck route designation. The results of the segment TkLOS analysis are summarized in **Table 4**.

Table 1: PLOS Segment Analysis

Sidewalk Width	Boulevard Width	Avg. Daily Curb Lane Traffic Volume	Presence of On-Street Parking	Operating Speed ⁽¹⁾	PLOS
Preston Street (east side, Carling Avenue to Sidney Street)					
2.0m	0m	> 3,000 vpd	No	60 km/h	E
Preston Street (west side, Carling Avenue to Sidney Street)					
2.0m	0m	> 3,000 vpd	No	60 km/h	E
Sidney Street (north side, west of Preston Street)					
1.8m	0m	≤ 3,000 vpd	No	30 km/h	A
Sidney Street (south side, west of Preston Street)					
1.8m	0m	≤ 3,000 vpd	Yes	30 km/h	A

1. Operating speed of Preston Street taken as the speed limit plus 10 km/h; operating speed of Sidney Street assumed to be 30 km/h.

Table 2: BLOS Segment Analysis

Road Class	Type of Route	Type of Bikeway	Travel Lanes	Posted Speed ⁽¹⁾	BLOS
Preston Street (Carling Avenue to Preston Street)					
Arterial	Local Route	Mixed Traffic	3	50 km/h	D
Sidney Street (west of Carling Avenue)					
Local	No Route	Mixed Traffic	2	≤ 40 km/h	A

1. No speed limit posted. Regulatory speed limit of 50 km/h assumed for Preston Street; speed limit of 30 km/h assumed for Sidney Street.

Table 3: TLOS Segment Analysis

Facility Type	Exposure to Congestion Delay, Friction, and Incidents			TLOS
	Congestion	Friction	Incident Potential	
Preston Street				
Mixed Traffic; Frequent Parking/Driveway Friction	Yes	High	High	F

Table 4: TkLOS Segment Analysis

Curb Lane Width	Number of Travel Lanes Per Direction	TkLOS
Preston Street		
> 3.7m	1	B

Intersection MMLOS Analysis

The following is a review of the MMLOS of the signalized intersections within the study area, using complete streets principles. All of these intersections have been evaluated using the MMLOS targets for intersections within 600m of a rapid transit station, and are based on existing conditions.

Exhibit 5 of the Addendum to the MMLOS Guidelines has been used to evaluate the existing PLOS at the intersections listed above. Exhibit 22 of the MMLOS Guidelines suggests a target PLOS A for all roadways within 600m of a rapid transit station. The results of the intersection PLOS analysis are summarized in **Table 5** through **Table 12**.

Exhibit 12 of the MMLOS Guidelines has been used to evaluate the existing BLOS at the intersections listed above. Within 600m of a rapid transit station, Exhibit 22 of the MMLOS Guidelines suggests a target BLOS B for local cycling routes (Sherwood Drive, Preston Street), a target BLOS C for arterial spine routes (Carling Avenue, Booth Street, Prince of Wales Drive), and a target BLOS D for roadways without a Crosstown Bikeway, Local Route, or Spine Route designation (Champagne Avenue, Trillium Pathway, Beech Street, Pamilla Street, Queen Elizabeth Driveway). The results of the intersection BLOS analysis are summarized in **Table 13**.

Exhibit 16 of the MMLOS Guidelines has been used to evaluate the existing TLOS at the intersections listed above. Exhibit 22 of the MMLOS Guidelines identifies a target TLOS C for roadways with a Transit Priority – Continuous Lanes designation (Carling Avenue), and does not identify a target TLOS for roadways without a Rapid Transit or Transit Priority designation (all others). The TLOS has been evaluated for every approach that is currently used by transit. The results of the intersection TLOS analysis are summarized in **Table 14**.

Exhibit 21 of the MMLOS Guidelines has been used to evaluate the existing TkLOS at the intersections listed above. Within 600m of a rapid transit station, Exhibit 22 of the MMLOS Guidelines identifies a target TkLOS D for arterial truck routes (Carling Avenue, Preston Street, Prince of Wales Drive) and collector truck routes (Booth Street), and no target TkLOS otherwise (Sherwood Drive, Champagne Avenue, Beech Street, Pamilla Street, Queen Elizabeth Driveway). The results of the intersection TkLOS analysis are summarized in **Table 15**.

Table 5: PLOS Intersection Analysis – Carling Avenue/Sherwood Drive

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETS I SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	72	N/A	0	No	23	No	6
Lanes Crossed (3.5m Lane Width)	5		N/A		8		9	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Protected	0	N/A	0	Permissive	-8	No Left Turn/Prohibited	0
Right Turn Conflict	Permissive or Yield	-5	N/A	0	No Right Turn/Prohibited	0	Permissive or Yield	-5
Right Turn on Red	RTOR Allowed	-3	N/A	0	RTOR Allowed	-3	N/A	0
Leading Pedestrian Interval	No	-2	N/A	0	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	> 25m	-9	N/A	0	No Right Turn	0	> 5m to 10m	-5
Parallel Right Turn Channel	No Right Turn Channel	-4	N/A	0	No Right Turn	0	Conventional without Receiving	0
Perpendicular Radius	> 5m to 10m	-5	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	Conventional without Receiving	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Zebra Stripe	-4	N/A	0	Zebra Stripe	-4	Zebra Stripe	-4
PETS I SCORE		40		-		6		-10
LOS		E		-		F		F
DELAY SCORE								
Cycle Length		120		-		140		140
Pedestrian Walk Time		44.6		-		26.9		26.9
DELAY SCORE		23.7		-		45.7		45.7
LOS		C		-		E		E
OVERALL		E		-		F		F

Table 6: PLOS Intersection Analysis – Carling Avenue/Champagne Avenue

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETS I SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	88	N/A	0	No	23	No	23
Lanes Crossed (3.5m Lane Width)	4		N/A		8		8	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Permissive	-8	N/A	0	Permissive	-8	No Left Turn/Prohibited	0
Right Turn Conflict	Permissive or Yield	-5	N/A	0	No Right Turn/Prohibited	0	Permissive or Yield	-5
Right Turn on Red	RTOR Allowed	-3	N/A	0	RTOR Allowed	-3	N/A	0
Leading Pedestrian Interval	No	-2	N/A	0	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	> 3m to 5m	-4	N/A	0	No Right Turn	0	> 10m to 15m	-6
Parallel Right Turn Channel	No Right Turn Channel	-4	N/A	0	No Right Turn	0	No Right Turn Channel	-4
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Standard	-7	N/A	0	Standard	-7	Standard	-7
PETS I SCORE		55		-		3		-1
LOS		D		-		F		F
DELAY SCORE								
Cycle Length		70		-		120		120
Pedestrian Walk Time		16.7		-		7.1		7.1
DELAY SCORE		20.3		-		53.1		53.1
LOS		C		-		E		E
OVERALL		D		-		F		F

Table 7: PLOS Intersection Analysis – Carling Avenue/Trillium Pathway

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETS I SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	120	No	120	No	23	No	23
Lanes Crossed (3.5m Lane Width)	1		1		8		8	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	No Left Turn/Prohibited	0	No Left Turn/Prohibited	0	No Left Turn/Prohibited	0	No Left Turn/Prohibited	0
Right Turn Conflict	No Right Turn/Prohibited	0	No Right Turn/Prohibited	0	No Right Turn/Prohibited	0	No Right Turn/Prohibited	0
Right Turn on Red	N/A	0	N/A	0	N/A	0	N/A	0
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	No Right Turn	0	No Right Turn	0	No Right Turn	0	No Right Turn	0
Parallel Right Turn Channel	No Right Turn	0	No Right Turn	0	No Right Turn	0	No Right Turn	0
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Zebra Stripe	-4	Zebra Stripe	-4	Zebra Stripe	-4	Zebra Stripe	-4
PETS I SCORE		114	PETS I SCORE		114	PETS I SCORE		17
LOS		A	LOS		A	LOS		F
DELAY SCORE								
Cycle Length		70		70		120		120
Pedestrian Walk Time		24.9		24.9		7.4		7.4
DELAY SCORE		14.5	DELAY SCORE		14.5	DELAY SCORE		52.8
LOS		B	LOS		B	LOS		E
OVERALL		B	OVERALL		B	OVERALL		F

Table 8: PLOS Intersection Analysis – Carling Avenue/Preston Street

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETSİ SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	88	No	55	Yes	15	Yes	15
Lanes Crossed (3.5m Lane Width)	4		6		9		9	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Protected	0	Protected	0	Permissive	-8	Perm + Prot	-8
Right Turn Conflict	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5
Right Turn on Red	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	> 3m to 5m	-4	> 3m to 5m	-4	> 10m to 15m	-6	> 3m to 5m	-4
Parallel Right Turn Channel	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn Channel	-4
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Standard	-7	Standard	-7	Standard	-7	Standard	-7
PETSİ SCORE		63			30			-18
LOS		C			E			F
DELAY SCORE								
Cycle Length		140			140		140	
Pedestrian Walk Time		18.0			18.0		32.1	
DELAY SCORE		53.2			53.2			41.6
LOS		E			E			F
OVERALL		E			E			F

Table 9: PLOS Intersection Analysis – Carling Avenue/Booth Street

CRITERIA		North Approach		South Approach		East Approach		West Approach	
PETSI SCORE									
CROSSING DISTANCE CONDITIONS									
Median > 2.4m in Width	No	88	N/A	0	Yes	30	No	23	
Lanes Crossed (3.5m Lane Width)	4		N/A		8				
SIGNAL PHASING AND TIMING									
Left Turn Conflict	Perm + Prot	-8	N/A	0	Permissive	-8	No Left Turn/Prohibited	0	
Right Turn Conflict	Permissive or Yield	-5	N/A	0	No Right Turn/Prohibited	0	Permissive or Yield	-5	
Right Turn on Red	RTOR Allowed	-3	N/A	0	RTOR Allowed	-3	N/A	0	
Leading Pedestrian Interval	No	-2	N/A	0	No	-2	No	-2	
CORNER RADIUS									
Parallel Radius	> 5m to 10m	-5	N/A	0	No Right Turn	0	> 3m to 5m	-4	
Parallel Right Turn Channel	No Right Turn Channel	-4	N/A	0	No Right Turn	0	No Right Turn Channel	-4	
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0	
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0	
CROSSING TREATMENT									
Treatment	Standard	-7	N/A	0	Standard	-7	Standard	-7	
PETSI SCORE		54		-		10		1	
LOS		D		-		F		F	
DELAY SCORE									
Cycle Length	120		-		130		130		
Pedestrian Walk Time	30.3		-		8.0		8.0		
DELAY SCORE		33.5		-		57.2		57.2	
LOS		D		-		E		E	
OVERALL		D		-		F		F	

Table 10: PLOS Intersection Analysis – Preston Street/Beech Street

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETSİ SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	88	No	88	No	88	No	105
Lanes Crossed (3.5m Lane Width)	4		4		4		3	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Permissive	-8	Permissive	-8	Permissive	-8	Permissive	-8
Right Turn Conflict	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5
Right Turn on Red	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	> 5m to 10m	-5	> 5m to 10m	-5	> 5m to 10m	-5	> 5m to 10m	-5
Parallel Right Turn Channel	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn Channel	-4
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Textured	-4	Textured	-4	Textured	-4	Textured	-4
PETSİ SCORE		57			57			74
LOS		D			D			C
DELAY SCORE								
Cycle Length		90			90			90
Pedestrian Walk Time		7.4			7.4			51.5
DELAY SCORE		37.9			37.9			8.2
LOS		D			D			A
OVERALL		D			D			C

Table 11: PLOS Intersection Analysis – Preston Street/Pamilla Street

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETSİ SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	88	No	88	No	120	No	105
Lanes Crossed (3.5m Lane Width)	4		4		2		3	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Permissive	-8	No Left Turn/Prohibited	0	Permissive	-8	Permissive	-8
Right Turn Conflict	No Right Turn/Prohibited	0	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5
Right Turn on Red	RTOR Allowed	-3	RTOR Allowed	-3	N/A	0	RTOR Allowed	-3
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	No Right Turn	0	> 5m to 10m	-5	> 3m to 5m	-4	> 5m to 10m	-5
Parallel Right Turn Channel	No Right Turn	0	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn Channel	-4
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Standard	-7	Standard	-7	Textured	-4	Textured	-4
PETSİ SCORE		68	PETSİ SCORE		93	PETSİ SCORE		74
LOS		C	LOS		A	LOS		C
DELAY SCORE								
Cycle Length		90		90		90		90
Pedestrian Walk Time		7.5		7.5		58.9		58.9
DELAY SCORE		37.8	DELAY SCORE		5.4	DELAY SCORE		5.4
LOS		D	LOS		A	LOS		A
OVERALL		D	OVERALL		A	OVERALL		C

Table 12: PLOS Intersection Analysis – Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway

CRITERIA	North Approach		South Approach		East Approach		West Approach	
PETS I SCORE								
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	23	No	88	No	72	No	72
Lanes Crossed (3.5m Lane Width)	8		4		5		5	
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Perm + Prot	-8	Permissive	-8	Permissive	-8	Permissive	-8
Right Turn Conflict	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5	Perm + Prot	-5
Right Turn on Red	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3	RTOR Allowed	-3
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	> 15m to 25m	-8	> 5m to 10m	-5	> 10m to 15m	-6	> 25m	-9
Parallel Right Turn Channel	Conventional with Receiving	-3	No Right Turn Channel	-4	No Right Turn Channel	-4	No Right Turn Channel	-4
Perpendicular Radius	N/A	0	N/A	0	> 15m to 25m	-8	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	Conventional with Receiving	-3	N/A	0
CROSSING TREATMENT								
Treatment	Standard	-7	Standard	-7	Standard	-7	Standard	-7
PETS I SCORE		-13			54			34
LOS		F			D			E
DELAY SCORE								
Cycle Length		130			130		120	120
Pedestrian Walk Time		16.9			55.9		12.5	12.5
DELAY SCORE		49.2			21.1			48.2
LOS		E			C			E
OVERALL		F			D			E

Table 13: BLOS Intersection Analysis

Approach	Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
Carling Avenue/Sherwood Drive				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared left turn/right turn lane	A
		Left Turn Accommodation	No lane crossed; ≤ 50 km/h	B
East Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	No left turn	-
West Approach	Curbside Lane (shared with transit)	Right Turn Lane Characteristics	No right turn	-
		Left Turn Accommodation	Three lanes crossed; ≥ 50 km/h	F
Carling Avenue/Champagne Avenue				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane is primary lane	A
		Left Turn Accommodation	One lane crossed; 50 km/h	D
East Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane < 50 m; turning speed < 25 km/h	D
		Left Turn Accommodation	No left turn	-
West Approach	Curbside Lane (shared with transit)	Right Turn Lane Characteristics	No right turn	-
		Left Turn Accommodation	Three lanes crossed; ≥ 50 km/h	F
Carling Avenue/Trillium Pathway				
North Approach	Mixed-Use Pathway	Right Turn Lane Characteristics	No lanes for vehicular traffic; cyclists wishing to turn onto Carling Avenue can do so during north-south phase	A
		Left Turn Accommodation		
South Approach	Mixed-Use Pathway	Right Turn Lane Characteristics	No lanes for vehicular traffic; cyclists wishing to turn onto Carling Avenue can do so during north-south phase	A
		Left Turn Accommodation		
East Approach	Mixed Traffic	Right Turn Lane Characteristics	No right turn	-
		Left Turn Accommodation	No left turn	-
West Approach	Curbside Lane (shared with transit)	Right Turn Lane Characteristics	No right turn	-
		Left Turn Accommodation	No left turn	-

Approach	Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
Carling Avenue/Preston Street				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	One lane crossed; 50 km/h	D
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	Two lanes crossed; ≥ 50 km/h	F
East Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	Three lanes crossed; ≥ 50 km/h	F
West Approach	Pocket Lane (shared with transit)	Right Turn Lane Characteristics	Right turn lane introduced to the right; lane $< 50\text{m}$, turning speed ≤ 25 km/h	B
		Left Turn Accommodation	Three lanes crossed; ≥ 50 km/h	F
Carling Avenue/Booth Street				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane $< 50\text{m}$; turning speed $\leq 25\text{km/h}$	D
		Left Turn Accommodation	One lane crossed; 50 km/h	D
East Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane $< 50\text{m}$; turning speed ≤ 25 km/h	D
		Left Turn Accommodation	No left turn	-
West Approach	Curbside Lane (shared with transit)	Right Turn Lane Characteristics	No right turn	-
		Left Turn Accommodation	Three lanes crossed; ≥ 50 km/h	F
Preston Street/Beech Street				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	One lane crossed; 50 km/h	D
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	One lane crossed; 50 km/h	D
East Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane $< 50\text{m}$; turning speed ≤ 25 km/h	D
		Left Turn Accommodation	One lane crossed; 50 km/h	D
West Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared left turn/through/right turn lane	A
		Left Turn Accommodation	No lanes crossed; ≤ 50 km/h	B

Approach	Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
Preston Street/Pamilla Street				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared left turn/through/right turn lane	A
		Left Turn Accommodation	No lanes crossed; ≤ 50 km/h	B
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared left turn/through/right turn lane	A
		Left Turn Accommodation	No lanes crossed; ≤ 50 km/h	B
West Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared left turn/through/right turn lane	A
		Left Turn Accommodation	No lanes crossed; ≤ 50 km/h	B
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway				
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane > 50m	F
		Left Turn Accommodation	One lane crossed; 50 km/h	D
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared left turn/through/right turn lane	A
		Left Turn Accommodation	No lanes crossed; ≤ 50 km/h	B
East Approach	Multi-Use Pathway	Right Turn Lane Characteristics	No impact on level of traffic stress	A
		Left Turn Accommodation	Pathway is located to the left of the roadway	A
West Approach	Curbside Bike Lane	Right Turn Lane Characteristics	Shared through/right turn lane	A
		Left Turn Accommodation	One lane crossed; ≥ 60 km/h	E

Table 14: TLOS Intersection Analysis

Approach	Delay ⁽¹⁾		TLOS
	AM Peak	PM Peak	
Carling Avenue/Sherwood Drive			
East Approach	7 sec	17 sec	C
West Approach	11 sec	14 sec	C
Carling Avenue/Champagne Avenue			
East Approach	2 sec	6 sec	B
West Approach	7 sec	11 sec	C
Carling Avenue/Trillium Pathway			
East Approach	3 sec	8 sec	B
West Approach	2 sec	6 sec	B
Carling Avenue/Preston Street			
North Approach	58 sec	91 sec	F
East Approach	47 sec	91 sec	F
West Approach	35 sec	39 sec	E
Carling Avenue/Booth Street			
North Approach	36 sec	43 sec	F
East Approach	23 sec	20 sec	D
West Approach	22 sec	14 sec	D
Preston Street/Beech Street			
North Approach	6 sec	7 sec	B
South Approach	7 sec	3 sec	B
Preston Street/Pamilla Street			
North Approach	5 sec	4 sec	B
South Approach	2 sec	4 sec	B

1. Delay based on outputs from Synchro analysis of existing conditions

Table 15: TkLOS Intersection Analysis

Approach	Effective Corner Radius	Number of Receiving Lanes Departing Intersection	TkLOS
Carling Avenue/Sherwood Drive			
North Approach	< 10m	3	D
East Approach	> 15m	1	C
Carling Avenue/Champagne Avenue			
North Approach	< 10m	3	D
East Approach	< 10m	1	F
Carling Avenue/Preston Street			
North Approach	< 10m	3	D
South Approach	10m to 15m	3	B
East Approach	< 10m	2	D
West Approach	< 10m	2	D
Carling Avenue/Booth Street			
North Approach	< 10m	3	D
East Approach	< 10m	1	F
Preston Street/Beech Street			
North Approach	< 10m	1	F
South Approach	< 10m	1	F
East Approach	< 10m	1	F
West Approach	< 10m	1	F
Preston Street/Pamilla Street			
North Approach	< 10m	1	F
South Approach	< 10m	1	F
West Approach	< 10m	1	F
Preston Street/Prince of Wales Drive/Queen Elizabeth Driveway			
North Approach	> 15m	1	C
South Approach	10m to 15m	1	E
East Approach	> 15m	2	A
West Approach	< 10m	1	F

APPENDIX N

Total Synchro Analysis

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	31	702	118	13	142	555	128	45	9	54	134	24
Future Volume (vph)	31	702	118	13	142	555	128	45	9	54	134	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.93		0.98		0.90	0.97		0.96	0.97	0.99
Frt			0.850				0.850			0.850		0.974
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1642	3283	1483	0	1659	3161	1483	1658	1745	1483	1674	1689
Flt Permitted	0.950				0.385			0.738			0.752	
Satd. Flow (perm)	1591	3283	1385	0	661	3161	1339	1247	1745	1418	1282	1689
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			109				128			78		
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	30		20		20		30	30		30	30	
Confl. Bikes (#/hr)			5				17					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	3%	2%	1%	2%	7%	2%	2%	2%	2%	1%	2%
Adj. Flow (vph)	31	702	118	13	142	555	128	45	9	54	134	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	702	118	0	155	555	128	45	9	54	134	29
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	5
Future Volume (vph)	5
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	3
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	5
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	12.0	78.0	78.0	66.0	66.0	66.0	66.0	42.0	42.0	42.0	42.0	42.0
Total Split (%)	10.0%	65.0%	65.0%	55.0%	55.0%	55.0%	55.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Maximum Green (s)	6.8	71.6	71.6	59.6	59.6	59.6	59.6	34.9	34.9	34.9	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	7.4	84.7	84.7		76.6	76.6	76.6	21.8	21.8	21.8	21.8	21.8
Actuated g/C Ratio	0.06	0.71	0.71		0.64	0.64	0.64	0.18	0.18	0.18	0.18	0.18
v/c Ratio	0.31	0.30	0.12		0.37	0.28	0.14	0.20	0.03	0.17	0.58	0.09
Control Delay	61.3	8.3	2.3		6.1	3.4	0.6	39.5	34.0	4.5	52.6	31.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.3	8.3	2.3		6.1	3.4	0.6	39.5	34.0	4.5	52.6	31.5
LOS	E	A	A		A	A	A	D	C	A	D	C
Approach Delay		9.4				3.5			21.5			48.8
Approach LOS		A				A			C			D
Queue Length 50th (m)	6.5	23.2	0.5		2.1	3.7	0.0	8.7	1.7	0.0	28.0	4.6
Queue Length 95th (m)	15.6	47.3	7.1		12.8	17.8	0.0	16.2	5.1	4.8	40.5	10.7
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	106	2316	1009		421	2016	900	362	507	467	372	494
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.30	0.12		0.37	0.28	0.14	0.12	0.02	0.12	0.36	0.06

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.58

Intersection Signal Delay: 10.8

Intersection LOS: B

Intersection Capacity Utilization 82.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

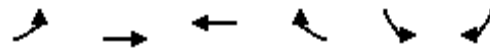




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
AM Peak Hour

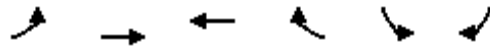
829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	112	844	800	168	89	78
Future Volume (vph)	112	844	800	168	89	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.93			0.71	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	3252	1414	1658	1498
Flt Permitted	0.328				0.950	
Satd. Flow (perm)	538	3283	3252	1009	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				139		78
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	90			90	7	9
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	112	844	800	168	89	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	844	800	168	89	78
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	78.0	78.0	78.0	78.0	42.0	42.0
Total Split (%)	65.0%	65.0%	65.0%	65.0%	35.0%	35.0%
Maximum Green (s)	72.7	72.7	72.7	72.7	36.1	36.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	85.6	85.6	85.6	85.6	23.2	23.2
Actuated g/C Ratio	0.71	0.71	0.71	0.71	0.19	0.19
v/c Ratio	0.29	0.36	0.34	0.22	0.28	0.23
Control Delay	8.0	6.2	2.6	1.8	40.4	9.0
Queue Delay	0.0	0.0	0.1	0.0	0.0	0.0
Total Delay	8.0	6.2	2.7	1.8	40.4	9.0
LOS	A	A	A	A	D	A
Approach Delay		6.4	2.6		25.7	
Approach LOS		A	A		C	
Queue Length 50th (m)	12.7	53.6	21.1	2.7	15.0	0.0
Queue Length 95th (m)	10.5	29.3	25.5	9.0	27.7	10.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	383	2342	2319	759	494	494
Starvation Cap Reductn	0	0	521	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.36	0.44	0.22	0.18	0.16

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.36

Intersection Signal Delay: 6.2

Intersection LOS: A

Intersection Capacity Utilization 58.5%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	930	0	0	1016	0	0	0	0	0	0	0
Future Volume (vph)	0	930	0	0	1016	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3283	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3283	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40		14	14		40	18		20	20		18
Confl. Bikes (#/hr)			7			25						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	930	0	0	1016	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	930	0	0	1016	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.36			0.39							
Control Delay		4.4			3.3							
Queue Delay		0.0			0.1							
Total Delay		4.5			3.4							
LOS		A			A							
Approach Delay		4.5			3.4							
Approach LOS		A			A							
Queue Length 50th (m)		27.0			25.2							
Queue Length 95th (m)		31.4			m29.2							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			2615							
Starvation Cap Reductn		280			405							
Spillback Cap Reductn		80			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.40			0.46							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 57 (48%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 3.9

Intersection LOS: A

Intersection Capacity Utilization 33.9%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
34 s	36 s
 Ø5 (R)	
34 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	153	584	233	143	633	102	283	490	284	124	353	132
Future Volume (vph)	153	584	233	143	633	102	283	490	284	124	353	132
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.94	0.97		0.98		0.85	0.98	0.99		1.00	0.98	
Frt		0.957				0.850		0.945			0.959	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3050	0	1658	3252	1375	1674	3052	0	1510	1524	0
Flt Permitted	0.950			0.950			0.149			0.358		
Satd. Flow (perm)	1506	3050	0	1625	3252	1167	258	3052	0	567	1524	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		48				155		134			16	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	90		41	41		90	60		10	10		60
Confl. Bikes (#/hr)			22			10			36			5
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	153	584	233	143	633	102	283	490	284	124	353	132
Shared Lane Traffic (%)												
Lane Group Flow (vph)	153	817	0	143	633	102	283	774	0	124	485	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	20.0	37.0		18.0	35.0	35.0	21.0	65.0		44.0	44.0	
Total Split (%)	16.7%	30.8%		15.0%	29.2%	29.2%	17.5%	54.2%		36.7%	36.7%	
Maximum Green (s)	13.8	31.0		11.8	29.0	29.0	14.1	58.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	13.4	31.0		11.8	29.4	29.4	58.1	58.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.26		0.10	0.24	0.24	0.48	0.48		0.31	0.31	
v/c Ratio	0.86	0.99		0.88	0.80	0.25	0.97	0.50		0.71	1.01	
Control Delay	94.6	61.7		78.5	29.6	5.1	61.0	11.3		60.4	83.0	
Queue Delay	0.0	0.8		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	94.6	62.5		78.5	29.6	5.1	61.0	11.3		60.4	83.0	
LOS	F	E		E	C	A	E	B		E	F	
Approach Delay		67.6			34.7			24.6			78.4	
Approach LOS		E			C			C			E	
Queue Length 50th (m)	27.2	45.1		27.9	76.2	3.7	38.4	47.9		23.8	~103.6	
Queue Length 95th (m)	#63.3	#122.5		m32.7	m77.4	m5.0	m#72.8	49.5		#51.6	#167.2	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	183	824		163	795	402	291	1546		175	482	
Starvation Cap Reductn	0	3		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.84	1.00		0.88	0.80	0.25	0.97	0.50		0.71	1.01	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 48.3

Intersection LOS: D

Intersection Capacity Utilization 103.3%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

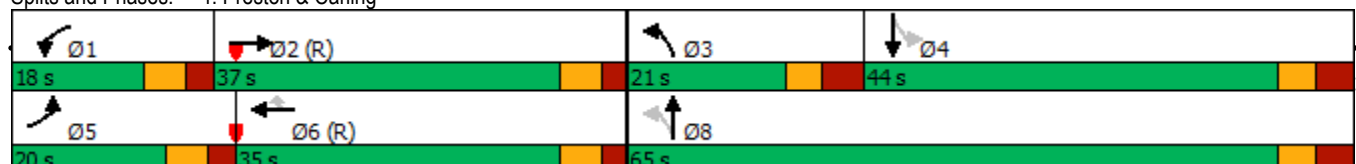
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

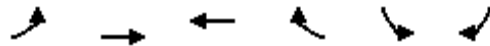
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

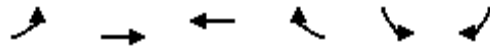
829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	400	685	702	215	240	169
Future Volume (vph)	400	685	702	215	240	169
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.93			0.80	0.98	0.80
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	1728	1498	1674	1427
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1564	3252	1728	1193	1649	1145
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				71		169
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	14	85
Confl. Bikes (#/hr)				16		23
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	400	685	702	215	240	169
Shared Lane Traffic (%)						
Lane Group Flow (vph)	400	685	702	215	240	169
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Prot	Perm
Protected Phases	5	2	6		4	
Permitted Phases				6		4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	26.0	81.0	55.0	55.0	39.0	39.0
Total Split (%)	21.7%	67.5%	45.8%	45.8%	32.5%	32.5%
Maximum Green (s)	20.1	75.3	49.3	49.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	28.6	83.8	49.3	49.3	24.5	24.5
Actuated g/C Ratio	0.24	0.70	0.41	0.41	0.20	0.20
v/c Ratio	1.01	0.30	0.99	0.40	0.70	0.46
Control Delay	77.2	7.3	67.2	18.9	54.6	9.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.2	7.3	67.2	18.9	54.6	9.3
LOS	E	A	E	B	D	A
Approach Delay		33.1	55.9		35.9	
Approach LOS		C	E		D	
Queue Length 50th (m)	85.9	27.6	148.9	21.1	49.9	0.0
Queue Length 95th (m)	m#145.5	m53.5	#222.1	40.0	67.7	15.4
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	398	2270	709	531	460	437
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.01	0.30	0.99	0.40	0.52	0.39

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 28 (23%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 42.2

Intersection LOS: D

Intersection Capacity Utilization 103.6%

ICU Level of Service G

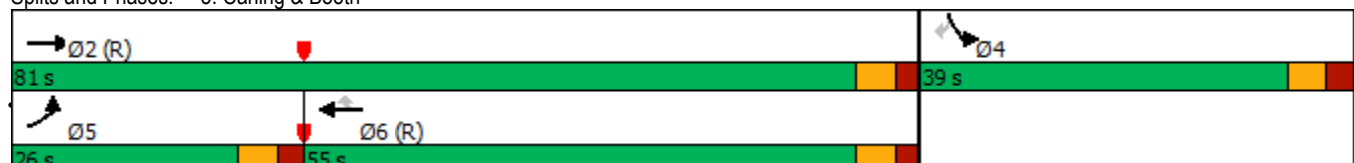
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	43	56	26	37	55	12	28	647	57	18	453	38
Future Volume (vph)	43	56	26	37	55	12	28	647	57	18	453	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.97	0.89	0.97	0.99		0.98	0.99	
Frt		0.972				0.850		0.988			0.988	
Flt Protected		0.983			0.980		0.950			0.950		
Satd. Flow (prot)	0	1558	0	0	1568	1498	1537	1693	0	1537	1648	0
Flt Permitted		0.865			0.853		0.437			0.301		
Satd. Flow (perm)	0	1336	0	0	1325	1339	684	1693	0	476	1648	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15				34		11			11	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		40	40		36	50		55	55		50
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	43	56	26	37	55	12	28	647	57	18	453	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	125	0	0	92	12	28	704	0	18	491	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.42			0.33	0.04	0.06	0.64		0.06	0.46	
Control Delay		28.9			30.4	2.8	5.6	9.7		5.8	8.6	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			30.4	2.8	5.6	9.8		5.8	8.6	
LOS		C			C	A	A	A		A	A	
Approach Delay		28.9			27.2			9.7			8.5	
Approach LOS		C			C			A			A	
Queue Length 50th (m)		13.3			11.0	0.0	1.3	50.8		0.8	29.3	
Queue Length 95th (m)		27.3			22.7	1.2	m2.6	41.5		3.0	47.5	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		302			288	317	442	1100		308	1071	
Starvation Cap Reductn		0			0	0	0	37		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.41			0.32	0.04	0.06	0.66		0.06	0.46	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 12.1

Intersection LOS: B

Intersection Capacity Utilization 82.2%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	684	46	10	548	5
Future Volume (vph)	1	0	3	0	0	0	8	684	46	10	548	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.992			0.999	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1704	0	0	1617	0
Flt Permitted		0.988						0.995			0.988	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1696	0	0	1599	0
Right Turn on Red			Yes				Yes		Yes			Yes
Satd. Flow (RTOR)		29						9			1	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	35		45	45		35
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	8	684	46	10	548	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	738	0	0	563	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.50			0.40	
Control Delay		0.0						5.5			2.5	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.5			2.5	
LOS		A						A			A	
Approach Delay								5.5			2.5	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						78.5			17.5	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1480			1394	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.50			0.40	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.50

Intersection Signal Delay: 4.2

Intersection LOS: A

Intersection Capacity Utilization 64.7%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	1	22	20	5	26	12	646	90	34	541	15
Future Volume (vph)	29	1	22	20	5	26	12	646	90	34	541	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt	0.943				0.931				0.984		0.997	
Flt Protected	0.973				0.981				0.999		0.997	
Satd. Flow (prot)	0	1601	0	0	1594	0	0	1701	0	0	1689	0
Flt Permitted	0.973				0.981				0.999		0.997	
Satd. Flow (perm)	0	1601	0	0	1594	0	0	1701	0	0	1689	0
Link Speed (k/h)	50				50				50		50	
Link Distance (m)	113.6				154.3				71.5		73.8	
Travel Time (s)	8.2				11.1				5.1		5.3	
Confl. Peds. (#/hr)							28		45		45	
Confl. Bikes (#/hr)									21		17	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	29	1	22	20	5	26	12	646	90	34	541	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	52	0	0	51	0	0	748	0	0	590	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0				0.0				0.0		0.0	
Link Offset(m)	-2.0				-2.0				0.0		0.0	
Crosswalk Width(m)	5.0				5.0				5.0		5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control	Stop				Stop				Free		Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization 62.8%					ICU Level of Service B							
Analysis Period (min) 15												

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	45	60	39	703	550	31
Future Volume (vph)	45	60	39	703	550	31
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.923				0.993	
Flt Protected	0.979			0.997		
Satd. Flow (prot)	1577	0	0	3275	1686	0
Flt Permitted	0.979			0.997		
Satd. Flow (perm)	1577	0	0	3275	1686	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	45	60	39	703	550	31
Shared Lane Traffic (%)						
Lane Group Flow (vph)	105	0	0	742	581	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	64.4%			ICU Level of Service C		
Analysis Period (min)	15					

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	677	281	2	2	223	317	1	4	3	277	4	417
Future Volume (vph)	677	281	2	2	223	317	1	4	3	277	4	417
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.92
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1668	1469
Flt Permitted	0.503			0.586				0.969			0.724	
Satd. Flow (perm)	863	1760	0	1009	1762	1462	0	1176	0	0	1193	1347
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				316		3				417
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	25		24	24		25
Confl. Bikes (#/hr)			3						1			2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	677	281	2	2	223	317	1	4	3	277	4	417
Shared Lane Traffic (%)												
Lane Group Flow (vph)	677	283	0	2	223	317	0	8	0	0	281	417
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		48.2	48.2	48.2		24.5			24.5	53.5
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.85	0.23		0.00	0.32	0.41		0.03			1.16	0.48
Control Delay	21.5	7.0		23.0	26.9	4.4		32.1			120.7	3.8
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	21.5	7.0		23.0	26.9	4.4		32.1			120.7	3.8
LOS	C	A		C	C	A		C			F	A
Approach Delay		17.2			13.8			32.1			50.9	
Approach LOS		B			B			C			D	
Queue Length 50th (m)	67.5	19.7		0.3	33.8	0.1		0.9			~69.5	11.8
Queue Length 95th (m)	#99.9	29.5		1.9	52.2	16.2		4.8			m#73.6	m12.9
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	810	1230		404	706	776		242			243	881
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.84	0.23		0.00	0.32	0.41		0.03			1.16	0.47

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.16

Intersection Signal Delay: 27.0

Intersection LOS: C

Intersection Capacity Utilization 99.1%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

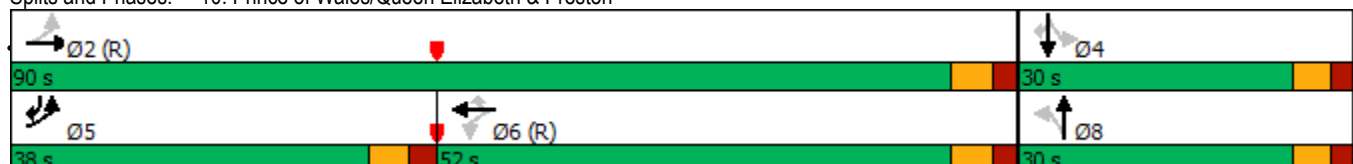
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



11: Site Access & Sidney
AM Peak Hour

829 Carling Avenue
2028 Total Traffic

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	87	0	9	61	0	18
Future Volume (vph)	87	0	9	61	0	18
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.865
Flt Protected				0.994		
Satd. Flow (prot)	1745	0	0	1735	0	1510
Flt Permitted				0.994		
Satd. Flow (perm)	1745	0	0	1735	0	1510
Link Speed (k/h)	30			30	50	
Link Distance (m)	49.5			68.0	41.2	
Travel Time (s)	5.9			8.2	3.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	87	0	9	61	0	18
Shared Lane Traffic (%)						
Lane Group Flow (vph)	87	0	0	70	0	18
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	0.0			0.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	14.8%			ICU Level of Service A		
Analysis Period (min)	15					

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	65	718	50	13	60	1367	155	125	25	150	181	10
Future Volume (vph)	65	718	50	13	60	1367	155	125	25	150	181	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99		0.92		0.98		0.89	0.96		0.95	0.96	0.98
Frt			0.850				0.850			0.850		0.938
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1674	3252	1483	0	1661	3316	1498	1658	1745	1483	1674	1605
Flt Permitted	0.950				0.379			0.746			0.741	
Satd. Flow (perm)	1652	3252	1370	0	650	3316	1330	1253	1745	1410	1258	1605
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			46				155			150		7
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	35		20		20		35	30		30	30	
Confl. Bikes (#/hr)			10				5					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	2%	1%	2%	2%	1%	2%	2%	2%	1%	2%
Adj. Flow (vph)	65	718	50	13	60	1367	155	125	25	150	181	10
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	718	50	0	73	1367	155	125	25	150	181	17
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	7
Future Volume (vph)	7
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	8
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	7
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	18.0	99.0	99.0	81.0	81.0	81.0	81.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	12.9%	70.7%	70.7%	57.9%	57.9%	57.9%	57.9%	29.3%	29.3%	29.3%	29.3%	29.3%
Maximum Green (s)	12.8	92.6	92.6	74.6	74.6	74.6	74.6	33.9	33.9	33.9	33.9	33.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	10.3	99.1	99.1		85.9	85.9	85.9	27.4	27.4	27.4	27.4	27.4
Actuated g/C Ratio	0.07	0.71	0.71		0.61	0.61	0.61	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.53	0.31	0.05		0.18	0.67	0.18	0.51	0.07	0.38	0.74	0.05
Control Delay	77.3	8.9	2.6		5.6	8.1	0.5	56.2	42.8	9.2	69.5	29.8
Queue Delay	0.0	0.0	0.0		0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	8.9	2.6		5.6	8.2	0.5	56.2	42.8	9.2	69.5	29.8
LOS	E	A	A		A	A	A	E	D	A	E	C
Approach Delay		13.8				7.4			31.6			66.1
Approach LOS		B				A			C			E
Queue Length 50th (m)	16.2	38.1	0.3		2.4	46.4	0.0	27.3	5.0	0.0	41.5	2.0
Queue Length 95th (m)	30.2	48.6	4.4		7.4	88.2	0.2	45.2	12.1	15.9	65.0	7.8
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	153	2300	982		398	2033	875	303	422	455	304	393
Starvation Cap Reductn	0	0	0		0	97	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.31	0.05		0.18	0.71	0.18	0.41	0.06	0.33	0.60	0.04

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 15 (11%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 15.7

Intersection LOS: B

Intersection Capacity Utilization 85.9%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood





Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

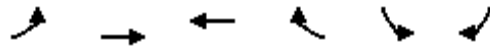
2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

						
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	80	893	1340	71	146	209
Future Volume (vph)	80	893	1340	71	146	209
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor				0.72	0.99	0.97
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	3316	1498	1674	1498
Flt Permitted	0.169				0.950	
Satd. Flow (perm)	251	3283	3316	1080	1663	1448
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				38		61
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	75			75	5	16
Confl. Bikes (#/hr)				5		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	80	893	1340	71	146	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	80	893	1340	71	146	209
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Prot	Perm
Protected Phases		2	6		4	
Permitted Phases	2			6		4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	102.0	102.0	102.0	102.0	38.0	38.0
Total Split (%)	72.9%	72.9%	72.9%	72.9%	27.1%	27.1%
Maximum Green (s)	96.7	96.7	96.7	96.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	104.1	104.1	104.1	104.1	24.7	24.7
Actuated g/C Ratio	0.74	0.74	0.74	0.74	0.18	0.18
v/c Ratio	0.43	0.37	0.54	0.09	0.49	0.69
Control Delay	17.5	6.6	2.7	1.3	56.0	48.1
Queue Delay	0.0	0.1	0.3	0.0	0.0	0.0
Total Delay	17.5	6.7	3.0	1.3	56.0	48.1
LOS	B	A	A	A	E	D
Approach Delay		7.6	2.9		51.3	
Approach LOS		A	A		D	
Queue Length 50th (m)	5.1	30.1	25.3	0.6	31.7	33.5
Queue Length 95th (m)	m28.7	39.8	27.7	1.7	50.8	58.0
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	186	2441	2466	812	383	379
Starvation Cap Reductn	0	538	468	0	0	0
Spillback Cap Reductn	0	0	37	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.43	0.47	0.67	0.09	0.38	0.55

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 139 (99%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 10.9

Intersection LOS: B

Intersection Capacity Utilization 77.2%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	1089	0	0	1455	0	0	0	0	0	0	0
Future Volume (vph)	0	1089	0	0	1455	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3316	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3316	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40					40	25		35	35		25
Confl. Bikes (#/hr)			12			11			13			34
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1089	0	0	1455	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1089	0	0	1455	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		104.0			104.0							
Total Split (%)		74.3%			74.3%							
Maximum Green (s)		98.9			98.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		115.6			115.6							
Actuated g/C Ratio		0.83			0.83							
v/c Ratio		0.40			0.53							
Control Delay		4.2			2.3							
Queue Delay		0.1			0.1							
Total Delay		4.3			2.4							
LOS		A			A							
Approach Delay		4.3			2.4							
Approach LOS		A			A							
Queue Length 50th (m)		43.2			26.7							
Queue Length 95th (m)		41.8			m26.1							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2710			2737							
Starvation Cap Reductn		405			379							
Spillback Cap Reductn		302			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.47			0.62							

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 114 (81%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.53

Intersection Signal Delay: 3.2

Intersection LOS: A

Intersection Capacity Utilization 46.7%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
104 s	36 s
 Ø5 (R)	
104 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	26%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	159	628	374	348	983	67	346	424	167	105	362	126
Future Volume (vph)	159	628	374	348	983	67	346	424	167	105	362	126
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97	0.97		0.99		0.86		0.98		0.97	0.97	
Frt		0.944				0.850		0.958			0.961	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	3018	0	1674	3316	1427	1674	3108	0	1537	1622	0
Flt Permitted	0.950			0.950			0.091			0.429		
Satd. Flow (perm)	1560	3018	0	1649	3316	1230	160	3108	0	673	1622	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		86				132		53			12	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	70		34	34		70	75		60	60		75
Confl. Bikes (#/hr)			13			11			16			6
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	159	628	374	348	983	67	346	424	167	105	362	126
Shared Lane Traffic (%)												
Lane Group Flow (vph)	159	1002	0	348	983	67	346	591	0	105	488	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	21.0	44.0		28.0	51.0	51.0	24.0	68.0		44.0	44.0	
Total Split (%)	15.0%	31.4%		20.0%	36.4%	36.4%	17.1%	48.6%		31.4%	31.4%	
Maximum Green (s)	14.8	38.0		21.8	45.0	45.0	17.1	61.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	14.8	38.0		21.8	45.0	45.0	61.1	61.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.27		0.16	0.32	0.32	0.44	0.44		0.26	0.26	
v/c Ratio	0.94	1.14		1.34	0.92	0.14	1.36	0.43		0.59	1.11	
Control Delay	123.1	104.8		196.8	43.6	2.5	219.3	25.8		60.2	123.4	
Queue Delay	0.0	0.1		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	123.1	104.9		196.8	43.6	2.5	219.3	25.8		60.2	123.4	
LOS	F	F		F	D	A	F	C		E	F	
Approach Delay		107.4			79.8			97.2			112.2	
Approach LOS		F			E			F			F	
Queue Length 50th (m)	34.7	~148.4		~114.0	138.2	0.0	~102.7	49.1		23.4	~140.1	
Queue Length 95th (m)	#78.9	#180.2		m#113.8	m125.3	m0.0	#159.0	63.7		43.2	#202.9	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	170	881		260	1065	484	254	1386		178	438	
Starvation Cap Reductn	0	15		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.94	1.16		1.34	0.92	0.14	1.36	0.43		0.59	1.11	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.36

Intersection Signal Delay: 96.3

Intersection LOS: F

Intersection Capacity Utilization 125.2%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

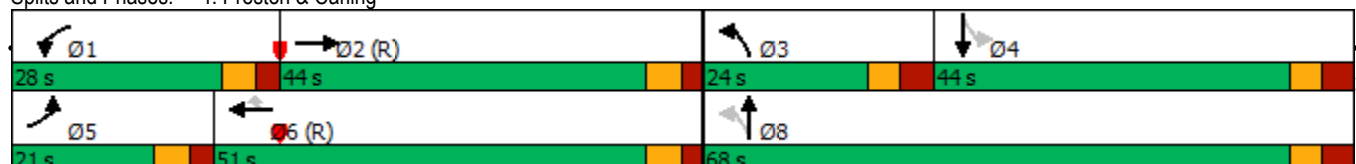
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

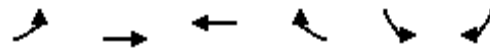
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
PM Peak Hour

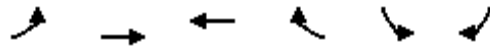
829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	265	701	1001	104	308	328
Future Volume (vph)	265	701	1001	104	308	328
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.95			0.77	0.98	0.75
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	1745	1498	1674	1483
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1589	3316	1745	1159	1647	1117
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				25		266
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	13	81
Confl. Bikes (#/hr)				11		45
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	265	701	1001	104	308	328
Shared Lane Traffic (%)						
Lane Group Flow (vph)	265	701	1001	104	308	328
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
2028 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	25.0	101.0	76.0	76.0	39.0	39.0
Total Split (%)	17.9%	72.1%	54.3%	54.3%	27.9%	27.9%
Maximum Green (s)	19.1	95.3	70.3	70.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	22.2	98.4	70.3	70.3	29.9	29.9
Actuated g/C Ratio	0.16	0.70	0.50	0.50	0.21	0.21
v/c Ratio	1.00	0.30	1.14	0.18	0.88	0.73
Control Delay	80.9	6.0	111.0	15.2	77.9	21.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	80.9	6.0	111.0	15.2	77.9	21.1
LOS	F	A	F	B	E	C
Approach Delay		26.6	102.0		48.6	
Approach LOS		C	F		D	
Queue Length 50th (m)	~77.6	44.8	~297.8	10.5	74.4	12.9
Queue Length 95th (m)	m#84.4	m43.4	#370.8	20.7	#113.7	47.7
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	265	2330	876	594	388	466
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.30	1.14	0.18	0.79	0.70

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 66 (47%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.14

Intersection Signal Delay: 62.5

Intersection LOS: E

Intersection Capacity Utilization 112.6%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	41	34	42	121	31	82	501	57	17	512	54
Future Volume (vph)	29	41	34	42	121	31	82	501	57	17	512	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.97	0.84	0.97	0.98		0.95	0.99	
Frt		0.956				0.850		0.985			0.986	
Flt Protected		0.986			0.987		0.950			0.950		
Satd. Flow (prot)	0	1578	0	0	1739	1498	1674	1679	0	1674	1672	0
Flt Permitted		0.875			0.904		0.397			0.402		
Satd. Flow (perm)	0	1369	0	0	1550	1261	675	1679	0	672	1672	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				31		14			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		50	50		40	55		80	80		55
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	29	41	34	42	121	31	82	501	57	17	512	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	104	0	0	163	31	82	558	0	17	566	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	61.8	61.8		61.8	61.8	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.37			0.56	0.12	0.18	0.48		0.04	0.49	
Control Delay		28.9			41.2	12.2	2.5	4.9		4.8	8.3	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			41.2	12.2	2.5	5.0		4.8	8.3	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.9			36.5			4.7			8.2	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.1			23.7	0.0	2.5	37.7		0.8	35.8	
Queue Length 95th (m)		24.5			41.6	6.6	0.5	1.8		2.6	55.8	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		284			299	268	463	1157		461	1152	
Starvation Cap Reductn		0			0	0	0	92		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.37			0.55	0.12	0.18	0.52		0.04	0.49	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.56

Intersection Signal Delay: 11.7

Intersection LOS: B

Intersection Capacity Utilization 87.5%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	597	27	6	585	15
Future Volume (vph)	4	2	10	0	0	0	8	597	27	6	585	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Flt		0.916						0.994			0.997	
Flt Protected		0.988						0.999				
Satd. Flow (prot)	0	1494	0	0	0	0	0	1709	0	0	1719	0
Flt Permitted		0.988						0.993			0.995	
Satd. Flow (perm)	0	1466	0	0	0	0	0	1698	0	0	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10						6			4	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	50		47	47		50
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	10	0	0	0	8	597	27	6	585	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	0	0	0	632	0	0	606	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.44			0.42	
Control Delay		21.9						5.0			3.7	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.9						5.0			3.7	
LOS		C						A			A	
Approach Delay		21.9						5.0			3.7	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						28.2			22.1	
Queue Length 95th (m)		5.8						57.9			33.8	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1428			1437	
Starvation Cap Reductn		0						0			62	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.06						0.44			0.44	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.44

Intersection Signal Delay: 4.6

Intersection LOS: A

Intersection Capacity Utilization 59.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	19	4	17	22	2	17	22	551	48	22	591	5
Future Volume (vph)	19	4	17	22	2	17	22	551	48	22	591	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt	0.943				0.944				0.990		0.999	
Flt Protected	0.977				0.974				0.998		0.998	
Satd. Flow (prot)	0	1608	0	0	1605	0	0	1709	0	0	1724	0
Flt Permitted	0.977				0.974				0.998		0.998	
Satd. Flow (perm)	0	1608	0	0	1605	0	0	1709	0	0	1724	0
Link Speed (k/h)	50				50				50		50	
Link Distance (m)	113.6				154.3				71.5		73.8	
Travel Time (s)	8.2				11.1				5.1		5.3	
Confl. Peds. (#/hr)							46		47		47	
Confl. Bikes (#/hr)									21		14	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%
Adj. Flow (vph)	19	4	17	22	2	17	22	551	48	22	591	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	40	0	0	41	0	0	621	0	0	618	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0				0.0				0.0		0.0	
Link Offset(m)	-2.0				-2.0				0.0		0.0	
Crosswalk Width(m)	5.0				5.0				5.0		5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control	Stop				Stop				Free		Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization 53.3%					ICU Level of Service A							
Analysis Period (min) 15												

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (vph)	25	34	51	596	559	67
Future Volume (vph)	25	34	51	596	559	67
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.922				0.986	
Flt Protected	0.979			0.996		
Satd. Flow (prot)	1575	0	0	3273	1706	0
Flt Permitted	0.979			0.996		
Satd. Flow (perm)	1575	0	0	3273	1706	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	25	34	51	596	559	67
Shared Lane Traffic (%)						
Lane Group Flow (vph)	59	0	0	647	626	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 68.4%				ICU Level of Service C		
Analysis Period (min) 15						

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	480	294	4	2	430	433	5	2	1	415	0	652
Future Volume (vph)	480	294	4	2	430	433	5	2	1	415	0	652
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.90		0.94		0.93			0.83	0.77
Frt		0.998				0.850		0.983				0.850
Flt Protected	0.950			0.950				0.970			0.950	
Satd. Flow (prot)	1642	1755	0	1674	1762	1498	0	1640	0	0	1674	1483
Flt Permitted	0.158			0.578				0.844			0.752	
Satd. Flow (perm)	273	1755	0	914	1762	1403	0	1365	0	0	1095	1137
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				176		1				99
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	80		65	65		80
Confl. Bikes (#/hr)			2									2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	480	294	4	2	430	433	5	2	1	415	0	652
Shared Lane Traffic (%)												
Lane Group Flow (vph)	480	298	0	2	430	433	0	8	0	0	415	652
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		36.4	36.4	36.4		43.5			43.5	75.3
Actuated g/C Ratio	0.58	0.58		0.28	0.28	0.28		0.33			0.33	0.58
v/c Ratio	0.96	0.29		0.01	0.87	0.83		0.02			1.13	0.83
Control Delay	63.7	15.0		34.5	64.1	41.0		27.5			128.5	28.3
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	63.7	15.0		34.5	64.1	41.0		27.5			128.5	28.3
LOS	E	B		C	E	D		C			F	C
Approach Delay		45.0			52.5			27.5			67.3	
Approach LOS		D			D			C			E	
Queue Length 50th (m)	88.6	34.2		0.3	97.5	60.8		1.1			~113.6	78.8
Queue Length 95th (m)	#150.6	49.7		2.3	#149.9	#112.5		4.5			#171.0	117.4
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	503	1011		256	493	519		457			366	791
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.95	0.29		0.01	0.87	0.83		0.02			1.13	0.82

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.13

Intersection Signal Delay: 56.1

Intersection LOS: E

Intersection Capacity Utilization 106.6%

ICU Level of Service G

Analysis Period (min) 15

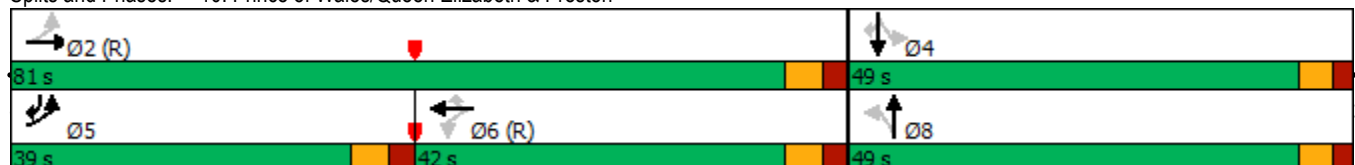
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



11: Site Access & Sidney
PM Peak Hour

829 Carling Avenue
2028 Total Traffic

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	47	0	17	101	0	12
Future Volume (vph)	47	0	17	101	0	12
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.865
Flt Protected				0.993		
Satd. Flow (prot)	1745	0	0	1733	0	1510
Flt Permitted				0.993		
Satd. Flow (perm)	1745	0	0	1733	0	1510
Link Speed (k/h)	30			30	50	
Link Distance (m)	49.5			68.0	41.2	
Travel Time (s)	5.9			8.2	3.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	47	0	17	101	0	12
Shared Lane Traffic (%)						
Lane Group Flow (vph)	47	0	0	118	0	12
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	0.0			0.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 16.6%	ICU Level of Service A					
Analysis Period (min) 15						

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	31	627	118	13	142	534	130	45	9	54	135	24
Future Volume (vph)	31	627	118	13	142	534	130	45	9	54	135	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.93		0.98		0.90	0.97		0.96	0.97	0.99
Frt			0.850				0.850			0.850		0.974
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1642	3283	1483	0	1659	3161	1483	1658	1745	1483	1674	1689
Flt Permitted	0.950				0.414			0.738			0.752	
Satd. Flow (perm)	1590	3283	1385	0	710	3161	1339	1247	1745	1418	1282	1689
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			118				130			78		
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	30		20		20		30	30		30	30	
Confl. Bikes (#/hr)			5				17					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	3%	2%	1%	2%	7%	2%	2%	2%	2%	1%	2%
Adj. Flow (vph)	31	627	118	13	142	534	130	45	9	54	135	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	627	118	0	155	534	130	45	9	54	135	29
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	5
Future Volume (vph)	5
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	3
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	5
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	12.0	78.0	78.0	66.0	66.0	66.0	66.0	42.0	42.0	42.0	42.0	42.0
Total Split (%)	10.0%	65.0%	65.0%	55.0%	55.0%	55.0%	55.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Maximum Green (s)	6.8	71.6	71.6	59.6	59.6	59.6	59.6	34.9	34.9	34.9	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	7.4	84.7	84.7		76.5	76.5	76.5	21.8	21.8	21.8	21.8	21.8
Actuated g/C Ratio	0.06	0.71	0.71		0.64	0.64	0.64	0.18	0.18	0.18	0.18	0.18
v/c Ratio	0.31	0.27	0.12		0.34	0.27	0.14	0.20	0.03	0.17	0.58	0.09
Control Delay	61.3	8.0	1.9		5.8	3.5	0.7	39.4	34.0	4.5	52.7	31.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.3	8.0	1.9		5.8	3.5	0.7	39.4	34.0	4.5	52.7	31.5
LOS	E	A	A		A	A	A	D	C	A	D	C
Approach Delay		9.2				3.5			21.5			48.9
Approach LOS		A				A			C			D
Queue Length 50th (m)	6.5	20.2	0.0		2.1	3.7	0.0	8.7	1.7	0.0	28.2	4.5
Queue Length 95th (m)	15.6	41.5	6.4		13.0	17.5	0.0	16.2	5.1	4.8	40.9	10.7
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	106	2316	1012		452	2015	900	362	507	467	372	494
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.27	0.12		0.34	0.27	0.14	0.12	0.02	0.12	0.36	0.06

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 91 (76%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.58

Intersection Signal Delay: 10.9

Intersection LOS: B

Intersection Capacity Utilization 82.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood





Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

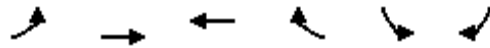
2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

						
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	112	759	774	168	89	78
Future Volume (vph)	112	759	774	168	89	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.93			0.71	0.99	0.98
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3283	3252	1414	1658	1498
Flt Permitted	0.338				0.950	
Satd. Flow (perm)	553	3283	3252	1009	1645	1464
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				144		78
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	90			90	7	9
Confl. Bikes (#/hr)				4		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	3%	4%	7%	2%	1%
Adj. Flow (vph)	112	759	774	168	89	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	759	774	168	89	78
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
AM Peak Hour

829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	78.0	78.0	78.0	78.0	42.0	42.0
Total Split (%)	65.0%	65.0%	65.0%	65.0%	35.0%	35.0%
Maximum Green (s)	72.7	72.7	72.7	72.7	36.1	36.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	85.6	85.6	85.6	85.6	23.2	23.2
Actuated g/C Ratio	0.71	0.71	0.71	0.71	0.19	0.19
v/c Ratio	0.28	0.32	0.33	0.22	0.28	0.23
Control Delay	8.2	6.3	2.6	1.8	40.4	9.0
Queue Delay	0.0	0.0	0.1	0.0	0.0	0.0
Total Delay	8.2	6.3	2.7	1.8	40.4	9.0
LOS	A	A	A	A	D	A
Approach Delay		6.5	2.6		25.7	
Approach LOS		A	A		C	
Queue Length 50th (m)	13.0	47.9	20.7	2.6	15.0	0.0
Queue Length 95th (m)	11.2	28.2	25.1	9.0	27.7	10.7
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	394	2342	2319	761	494	494
Starvation Cap Reductn	0	0	547	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.32	0.44	0.22	0.18	0.16

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.33

Intersection Signal Delay: 6.3

Intersection LOS: A

Intersection Capacity Utilization 57.8%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	839	0	0	978	0	0	0	0	0	0	0
Future Volume (vph)	0	839	0	0	978	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3283	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3283	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40		14	14		40	18		20	20		18
Confl. Bikes (#/hr)			7			25						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	839	0	0	978	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	839	0	0	978	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		84.0			84.0							
Total Split (%)		70.0%			70.0%							
Maximum Green (s)		78.9			78.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		95.6			95.6							
Actuated g/C Ratio		0.80			0.80							
v/c Ratio		0.32			0.37							
Control Delay		4.3			3.4							
Queue Delay		0.1			0.1							
Total Delay		4.4			3.4							
LOS		A			A							
Approach Delay		4.4			3.4							
Approach LOS		A			A							
Queue Length 50th (m)		24.6			24.8							
Queue Length 95th (m)		29.0			m28.0							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2615			2615							
Starvation Cap Reductn		399			416							
Spillback Cap Reductn		50			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.38			0.44							

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 57 (48%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.37

Intersection Signal Delay: 3.9

Intersection LOS: A

Intersection Capacity Utilization 32.8%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
34 s	36 s
 Ø5 (R)	
34 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	30%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	155	527	233	143	607	103	283	492	284	136	362	132
Future Volume (vph)	155	527	233	143	607	103	283	492	284	136	362	132
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.94	0.97		0.98		0.85	0.98	0.99		1.00	0.98	
Frt		0.954				0.850		0.945			0.960	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1595	3035	0	1658	3252	1375	1674	3052	0	1510	1528	0
Flt Permitted	0.950			0.950			0.139			0.358		
Satd. Flow (perm)	1503	3035	0	1622	3252	1167	241	3052	0	567	1528	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		56				155		133			16	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	90		41	41		90	60		10	10		60
Confl. Bikes (#/hr)			22			10			36			1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	4%	2%	2%	4%	10%	1%	4%	2%	12%	6%	20%
Adj. Flow (vph)	155	527	233	143	607	103	283	492	284	136	362	132
Shared Lane Traffic (%)												
Lane Group Flow (vph)	155	760	0	143	607	103	283	776	0	136	494	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	20.0	37.0		18.0	35.0	35.0	21.0	65.0		44.0	44.0	
Total Split (%)	16.7%	30.8%		15.0%	29.2%	29.2%	17.5%	54.2%		36.7%	36.7%	
Maximum Green (s)	13.8	31.0		11.8	29.0	29.0	14.1	58.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	13.5	31.0		11.8	29.3	29.3	58.1	58.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.26		0.10	0.24	0.24	0.48	0.48		0.31	0.31	
v/c Ratio	0.87	0.92		0.88	0.76	0.26	0.99	0.50		0.78	1.02	
Control Delay	95.6	47.2		80.4	29.3	5.3	67.5	11.4		68.0	86.9	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	95.6	47.2		80.4	29.3	5.3	67.5	11.4		68.0	86.9	
LOS	F	D		F	C	A	E	B		E	F	
Approach Delay		55.4			35.0			26.4			82.8	
Approach LOS		E			C			C			F	
Queue Length 50th (m)	27.8	41.9		27.6	72.7	4.2	40.4	48.2		26.8	~111.4	
Queue Length 95th (m)	#64.3	#106.8		m#35.5	m76.8	m5.8	m#75.3	49.8		#58.4	#171.5	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	183	826		163	794	402	285	1546		175	483	
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.85	0.92		0.88	0.76	0.26	0.99	0.50		0.78	1.02	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.02

Intersection Signal Delay: 46.5

Intersection LOS: D

Intersection Capacity Utilization 101.7%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

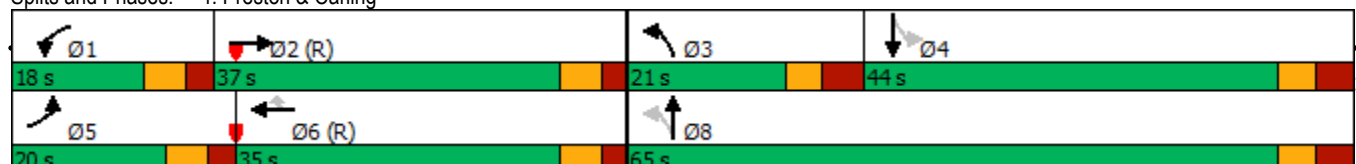
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

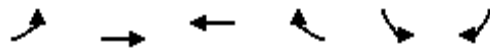
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
AM Peak Hour

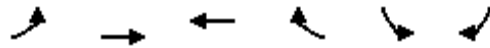
829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	400	632	673	215	240	169
Future Volume (vph)	400	632	673	215	240	169
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.93			0.80	0.98	0.80
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3252	1728	1498	1674	1427
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1559	3252	1728	1193	1649	1145
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				74		169
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	14	85
Confl. Bikes (#/hr)				16		23
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	3%	1%	1%	6%
Adj. Flow (vph)	400	632	673	215	240	169
Shared Lane Traffic (%)						
Lane Group Flow (vph)	400	632	673	215	240	169
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
AM Peak Hour

829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	26.0	81.0	55.0	55.0	39.0	39.0
Total Split (%)	21.7%	67.5%	45.8%	45.8%	32.5%	32.5%
Maximum Green (s)	20.1	75.3	49.3	49.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	28.5	83.7	49.3	49.3	24.6	24.6
Actuated g/C Ratio	0.24	0.70	0.41	0.41	0.20	0.20
v/c Ratio	1.01	0.28	0.95	0.40	0.71	0.46
Control Delay	81.3	6.7	58.3	18.5	55.0	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	81.3	6.7	58.3	18.5	55.0	9.2
LOS	F	A	E	B	E	A
Approach Delay		35.6	48.7		36.1	
Approach LOS		D	D		D	
Queue Length 50th (m)	85.6	25.3	138.8	20.6	49.8	0.0
Queue Length 95th (m)	m#153.9	m48.8	#208.1	39.4	68.0	15.4
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	396	2267	709	533	453	437
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.01	0.28	0.95	0.40	0.53	0.39

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 28 (23%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 40.7

Intersection LOS: D

Intersection Capacity Utilization 102.0%

ICU Level of Service G

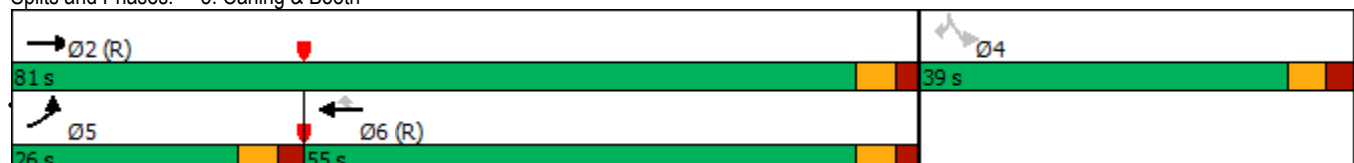
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	43	56	26	38	55	12	28	670	62	18	459	38
Future Volume (vph)	43	56	26	38	55	12	28	670	62	18	459	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.97	0.89	0.97	0.99		0.98	0.99	
Frt		0.972				0.850		0.987			0.989	
Flt Protected		0.983			0.980		0.950			0.950		
Satd. Flow (prot)	0	1558	0	0	1566	1498	1537	1690	0	1537	1650	0
Flt Permitted		0.865			0.853		0.433			0.284		
Satd. Flow (perm)	0	1336	0	0	1322	1339	678	1690	0	450	1650	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15				34		12			10	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	36		40	40		36	50		55	55		50
Confl. Bikes (#/hr)			26			2			20			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	9%	4%	4%	25%	2%	1%	10%	3%	3%	10%	6%	5%
Adj. Flow (vph)	43	56	26	38	55	12	28	670	62	18	459	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	125	0	0	93	12	28	732	0	18	497	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	57.0	57.0		57.0	57.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%	28.8%	71.3%	71.3%		71.3%	71.3%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	51.5	51.5		51.5	51.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.1			17.1	17.1	51.8	51.8		51.8	51.8	
Actuated g/C Ratio		0.21			0.21	0.21	0.65	0.65		0.65	0.65	
v/c Ratio		0.42			0.33	0.04	0.06	0.67		0.06	0.46	
Control Delay		28.9			30.5	2.8	5.8	10.3		5.9	8.7	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.9			30.5	2.8	5.8	10.4		5.9	8.7	
LOS		C			C	A	A	B		A	A	
Approach Delay		28.9			27.3			10.3			8.6	
Approach LOS		C			C			B			A	
Queue Length 50th (m)		13.3			11.1	0.0	1.3	54.5		0.8	29.9	
Queue Length 95th (m)		27.3			22.8	1.2	m2.7	46.1		3.0	48.3	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		302			287	317	438	1098		291	1071	
Starvation Cap Reductn		0			0	0	0	30		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.41			0.32	0.04	0.06	0.69		0.06	0.46	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 40 (50%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 12.4

Intersection LOS: B

Intersection Capacity Utilization 83.8%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Preston & Beech



												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	1	0	3	0	0	0	8	713	46	10	556	5
Future Volume (vph)	1	0	3	0	0	0	8	713	46	10	556	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93						0.99			1.00	
Frt		0.899						0.992			0.999	
Flt Protected		0.988						0.999			0.999	
Satd. Flow (prot)	0	1470	0	0	0	0	0	1704	0	0	1617	0
Flt Permitted		0.988						0.995			0.988	
Satd. Flow (perm)	0	1453	0	0	0	0	0	1697	0	0	1599	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29						9			1	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	17		18	18		17	35		45	45		35
Confl. Bikes (#/hr)			8						21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	10%	1%
Adj. Flow (vph)	1	0	3	0	0	0	8	713	46	10	556	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	4	0	0	0	0	0	767	0	0	571	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					59.0	59.0		59.0	59.0	
Total Split (%)	26.3%	26.3%					73.8%	73.8%		73.8%	73.8%	
Maximum Green (s)	15.5	15.5					53.9	53.9		53.9	53.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						69.8			69.8	
Actuated g/C Ratio		0.15						0.87			0.87	
v/c Ratio		0.02						0.52			0.41	
Control Delay		0.0						5.7			2.5	
Queue Delay		0.0						0.0			0.0	
Total Delay		0.0						5.7			2.5	
LOS		A						A			A	
Approach Delay								5.7			2.5	
Approach LOS								A			A	
Queue Length 50th (m)		0.0						0.0			0.0	
Queue Length 95th (m)		0.0						84.7			17.8	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		304						1481			1394	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.01						0.52			0.41	
Intersection Summary												
Area Type: Other												
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 48 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green												
Natural Cycle: 60												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.52												
Intersection Signal Delay: 4.4						Intersection LOS: A						
Intersection Capacity Utilization 66.3%						ICU Level of Service C						
Analysis Period (min) 15												

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	49	1	33	20	5	26	14	655	90	37	546	15
Future Volume (vph)	49	1	33	20	5	26	14	655	90	37	546	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Fr't		0.946			0.931			0.984			0.997	
Flt Protected		0.971			0.981			0.999			0.997	
Satd. Flow (prot)	0	1603	0	0	1594	0	0	1701	0	0	1689	0
Flt Permitted		0.971			0.981			0.999			0.997	
Satd. Flow (perm)	0	1603	0	0	1594	0	0	1701	0	0	1689	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							28		45	45		28
Confl. Bikes (#/hr)									21			17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	5%	2%
Adj. Flow (vph)	49	1	33	20	5	26	14	655	90	37	546	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	83	0	0	51	0	0	759	0	0	598	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization 67.1%	ICU Level of Service C											
Analysis Period (min) 15												

9: Preston & Sidney
AM Peak Hour

829 Carling Avenue
2033 Total Traffic

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	54	70	43	705	561	34
Future Volume (vph)	54	70	43	705	561	34
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.924				0.992	
Flt Protected	0.979			0.997		
Satd. Flow (prot)	1579	0	0	3275	1684	0
Flt Permitted	0.979			0.997		
Satd. Flow (perm)	1579	0	0	3275	1684	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			28			28
Confl. Bikes (#/hr)						17
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	5%	2%
Adj. Flow (vph)	54	70	43	705	561	34
Shared Lane Traffic (%)						
Lane Group Flow (vph)	124	0	0	748	595	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 69.0%	ICU Level of Service C					
Analysis Period (min) 15						

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	678	281	2	2	223	318	1	4	3	280	4	423
Future Volume (vph)	678	281	2	2	223	318	1	4	3	280	4	423
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		0.98		0.98		0.97			0.94	0.92
Frt		0.999				0.850		0.949				0.850
Flt Protected	0.950			0.950				0.994			0.953	
Satd. Flow (prot)	1642	1760	0	1674	1762	1498	0	1211	0	0	1668	1469
Flt Permitted	0.503			0.586				0.969			0.724	
Satd. Flow (perm)	863	1760	0	1009	1762	1462	0	1176	0	0	1193	1347
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				317		3				423
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	9		15	15		9	25		24	24		25
Confl. Bikes (#/hr)			3						1			2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	50%	25%	1%	50%	3%
Adj. Flow (vph)	678	281	2	2	223	318	1	4	3	280	4	423
Shared Lane Traffic (%)												
Lane Group Flow (vph)	678	283	0	2	223	318	0	8	0	0	284	423
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	38.0	90.0		52.0	52.0	52.0	30.0	30.0		30.0	30.0	38.0
Total Split (%)	31.7%	75.0%		43.3%	43.3%	43.3%	25.0%	25.0%		25.0%	25.0%	31.7%
Maximum Green (s)	31.9	83.9		45.9	45.9	45.9	24.5	24.5		24.5	24.5	31.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	83.9	83.9		48.1	48.1	48.1		24.5			24.5	53.6
Actuated g/C Ratio	0.70	0.70		0.40	0.40	0.40		0.20			0.20	0.45
v/c Ratio	0.85	0.23		0.00	0.32	0.41		0.03			1.17	0.49
Control Delay	21.5	7.0		23.0	26.9	4.4		32.1			126.3	4.0
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	21.5	7.0		23.0	26.9	4.4		32.1			126.3	4.0
LOS	C	A		C	C	A		C			F	A
Approach Delay		17.3			13.7			32.1			53.1	
Approach LOS		B			B			C			D	
Queue Length 50th (m)	67.6	19.7		0.3	33.8	0.1		0.9			~71.2	12.8
Queue Length 95th (m)	#101.0	29.5		1.9	52.2	16.4		4.8			m#76.9	m14.4
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	810	1230		404	706	776		242			243	885
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.84	0.23		0.00	0.32	0.41		0.03			1.17	0.48

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 4 (3%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 27.9

Intersection LOS: C

Intersection Capacity Utilization 99.3%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

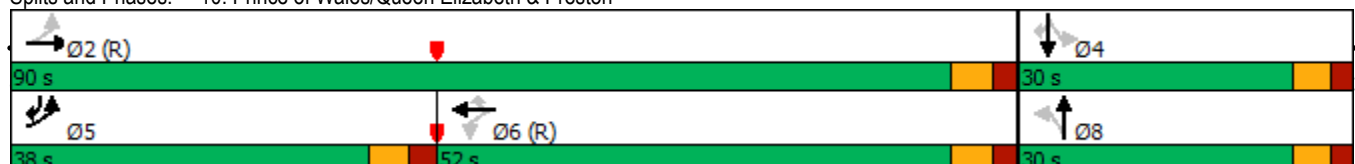
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	106	0	9	68	0	18
Future Volume (vph)	106	0	9	68	0	18
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.865
Flt Protected				0.994		
Satd. Flow (prot)	1745	0	0	1735	0	1510
Flt Permitted				0.994		
Satd. Flow (perm)	1745	0	0	1735	0	1510
Link Speed (k/h)	30			30	50	
Link Distance (m)	49.5			68.0	41.2	
Travel Time (s)	5.9			8.2	3.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	106	0	9	68	0	18
Shared Lane Traffic (%)						
Lane Group Flow (vph)	106	0	0	77	0	18
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	0.0			0.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)		14	24		24	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.9%			ICU Level of Service A		
Analysis Period (min)	15					

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	65	686	50	13	60	1223	157	125	25	150	183	10
Future Volume (vph)	65	686	50	13	60	1223	157	125	25	150	183	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		25.0		50.0		110.0	0.0		60.0	0.0	
Storage Lanes	1		1		1		1	1		1	1	
Taper Length (m)	25.0				25.0			25.0			25.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.98		0.92		0.98		0.89	0.96		0.95	0.96	0.98
Frt			0.850				0.850			0.850		0.938
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1674	3252	1483	0	1661	3316	1498	1658	1745	1483	1674	1605
Flt Permitted	0.950				0.391			0.746			0.741	
Satd. Flow (perm)	1646	3252	1370	0	670	3316	1330	1253	1745	1410	1258	1605
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)			48				157			150		7
Link Speed (k/h)		60				60			50			40
Link Distance (m)		196.1				162.9			145.2			242.3
Travel Time (s)		11.8				9.8			10.5			21.8
Confl. Peds. (#/hr)	35		20		20		35	30		30	30	
Confl. Bikes (#/hr)			10				5					
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	4%	2%	1%	2%	2%	1%	2%	2%	2%	1%	2%
Adj. Flow (vph)	65	686	50	13	60	1223	157	125	25	150	183	10
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	686	50	0	73	1223	157	125	25	150	183	17
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	L NA	Left	R NA	L NA	Left
Median Width(m)		7.0				7.0			3.5			3.5
Link Offset(m)		0.0				0.0			0.0			0.0
Crosswalk Width(m)		5.0				10.0			5.0			5.0
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	14	24		14	24		14	40	
Number of Detectors	1	2	1	1	1	2	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Thru	Right	Left	Thru
Leading Detector (m)	6.1	30.5	6.1	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7				28.7			28.7			28.7
Detector 2 Size(m)		1.8				1.8			1.8			1.8
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0
Turn Type	Prot	NA	Perm	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA
Protected Phases	5	2				6			8			4
Permitted Phases			2	6	6		6	8		8	4	
Detector Phase	5	2	2	6	6	6	6	8	8	8	4	4

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	7
Future Volume (vph)	7
Ideal Flow (vphpl)	1800
Storage Length (m)	10.0
Storage Lanes	1
Taper Length (m)	
Lane Util. Factor	1.00
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	30
Confl. Bikes (#/hr)	8
Peak Hour Factor	1.00
Heavy Vehicles (%)	1%
Adj. Flow (vph)	7
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	R NA
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	1.09
Turning Speed (k/h)	14
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	

1: Carling & Sherwood
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.2	33.4	33.4	33.4	33.4	33.4	33.4	40.1	40.1	40.1	40.1	40.1
Total Split (s)	18.0	99.0	99.0	81.0	81.0	81.0	81.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	12.9%	70.7%	70.7%	57.9%	57.9%	57.9%	57.9%	29.3%	29.3%	29.3%	29.3%	29.3%
Maximum Green (s)	12.8	92.6	92.6	74.6	74.6	74.6	74.6	33.9	33.9	33.9	33.9	33.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.5	2.7	2.7	2.7	2.7	2.7	2.7	3.8	3.8	3.8	3.8	3.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.2	6.4	6.4		6.4	6.4	6.4	7.1	7.1	7.1	7.1	7.1
Lead/Lag	Lead			Lag	Lag	Lag	Lag					
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Walk Time (s)		12.0	12.0	12.0	12.0	12.0	12.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		15.0	15.0	15.0	15.0	15.0	15.0	26.0	26.0	26.0	26.0	26.0
Pedestrian Calls (#/hr)		20	20	20	20	20	20	20	20	20	20	20
Act Effct Green (s)	10.3	99.0	99.0		85.8	85.8	85.8	27.5	27.5	27.5	27.5	27.5
Actuated g/C Ratio	0.07	0.71	0.71		0.61	0.61	0.61	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.53	0.30	0.05		0.18	0.60	0.18	0.51	0.07	0.38	0.74	0.05
Control Delay	77.3	8.8	2.4		5.8	7.4	0.6	56.1	42.8	9.1	70.1	29.8
Queue Delay	0.0	0.0	0.0		0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	8.8	2.4		5.8	7.5	0.6	56.1	42.8	9.1	70.1	29.8
LOS	E	A	A		A	A	A	E	D	A	E	C
Approach Delay		13.9				6.7			31.5			66.7
Approach LOS		B				A			C			E
Queue Length 50th (m)	16.2	36.0	0.2		2.5	43.0	0.1	27.3	5.0	0.0	42.1	2.0
Queue Length 95th (m)	30.2	46.0	4.2		7.9	71.0	0.4	45.2	12.1	15.9	65.7	7.8
Internal Link Dist (m)		172.1				138.9			121.2			218.3
Turn Bay Length (m)	40.0		25.0		50.0		110.0			60.0		
Base Capacity (vph)	153	2299	982		410	2032	876	303	422	455	304	393
Starvation Cap Reductn	0	0	0		0	107	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.30	0.05		0.18	0.64	0.18	0.41	0.06	0.33	0.60	0.04

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 15 (11%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 15.9

Intersection LOS: B

Intersection Capacity Utilization 86.0%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Carling & Sherwood

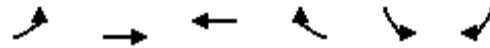




Lane Group	SBR
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2: Carling & Champagne
PM Peak Hour

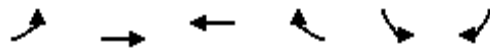
829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	80	860	1206	71	146	209
Future Volume (vph)	80	860	1206	71	146	209
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	70.0			25.0	20.0	0.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				25.0	
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97			0.72	0.99	0.97
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1409	3283	3316	1498	1674	1498
Flt Permitted	0.202				0.950	
Satd. Flow (perm)	292	3283	3316	1080	1663	1448
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				42		80
Link Speed (k/h)		60	60		50	
Link Distance (m)		162.9	117.5		178.4	
Travel Time (s)		9.8	7.1		12.8	
Confl. Peds. (#/hr)	75			75	5	16
Confl. Bikes (#/hr)				5		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	20%	3%	2%	1%	1%	1%
Adj. Flow (vph)	80	860	1206	71	146	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	80	860	1206	71	146	209
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Perm	NA	NA	Perm	Perm	Perm
Protected Phases		2	6			
Permitted Phases	2			6	4	4
Detector Phase	2	2	6	6	4	4

2: Carling & Champagne
PM Peak Hour

829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	15.3	15.3	25.3	25.3	37.9	37.9
Total Split (s)	102.0	102.0	102.0	102.0	38.0	38.0
Total Split (%)	72.9%	72.9%	72.9%	72.9%	27.1%	27.1%
Maximum Green (s)	96.7	96.7	96.7	96.7	32.1	32.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.9	5.9
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)			10.0	10.0	7.0	7.0
Flash Dont Walk (s)			10.0	10.0	25.0	25.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	104.4	104.4	104.4	104.4	24.4	24.4
Actuated g/C Ratio	0.75	0.75	0.75	0.75	0.17	0.17
v/c Ratio	0.37	0.35	0.49	0.09	0.51	0.66
Control Delay	13.8	6.6	2.7	1.4	56.6	41.4
Queue Delay	0.0	0.1	0.2	0.0	0.0	0.0
Total Delay	13.8	6.8	2.9	1.4	56.6	41.4
LOS	B	A	A	A	E	D
Approach Delay		7.4	2.8		47.6	
Approach LOS		A	A		D	
Queue Length 50th (m)	5.4	30.1	24.3	0.6	31.7	28.8
Queue Length 95th (m)	m25.8	39.8	27.2	1.8	50.8	53.0
Internal Link Dist (m)		138.9	93.5		154.4	
Turn Bay Length (m)	70.0			25.0	20.0	
Base Capacity (vph)	217	2448	2473	816	381	393
Starvation Cap Reductn	0	576	469	0	0	0
Spillback Cap Reductn	0	0	11	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.46	0.60	0.09	0.38	0.53

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 139 (99%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 10.7

Intersection LOS: B

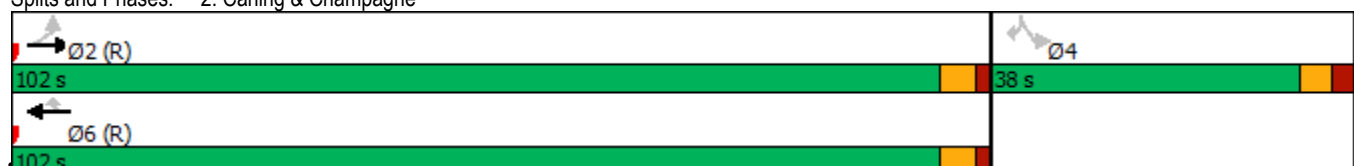
Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Carling & Champagne



3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑							
Traffic Volume (vph)	0	1046	0	0	1311	0	0	0	0	0	0	0
Future Volume (vph)	0	1046	0	0	1311	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Flt Protected												
Satd. Flow (prot)	0	3283	0	0	3316	0	0	0	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	3283	0	0	3316	0	0	0	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		117.5			124.7			157.3			54.9	
Travel Time (s)		7.1			7.5			11.3			4.0	
Confl. Peds. (#/hr)	40					40	25		35	35		25
Confl. Bikes (#/hr)			12			11			13			34
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	1046	0	0	1311	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1046	0	0	1311	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.0			7.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2			2							
Detector Template		Thru			Thru							
Leading Detector (m)		30.5			30.5							
Trailing Detector (m)		0.0			0.0							
Detector 1 Position(m)		0.0			0.0							
Detector 1 Size(m)		1.8			1.8							
Detector 1 Type		CI+Ex			CI+Ex							
Detector 1 Channel												
Detector 1 Extend (s)		0.0			0.0							
Detector 1 Queue (s)		0.0			0.0							
Detector 1 Delay (s)		0.0			0.0							
Detector 2 Position(m)		28.7			28.7							
Detector 2 Size(m)		1.8			1.8							
Detector 2 Type		CI+Ex			CI+Ex							
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0							
Turn Type		NA			NA							
Protected Phases		2			6							
Permitted Phases												
Detector Phase		2			6							
Switch Phase												
Minimum Initial (s)		10.0			10.0							
Minimum Split (s)		25.1			25.1							

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	35.6

3: Trillium Pathway & Carling
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)		104.0			104.0							
Total Split (%)		74.3%			74.3%							
Maximum Green (s)		98.9			98.9							
Yellow Time (s)		3.7			3.7							
All-Red Time (s)		1.4			1.4							
Lost Time Adjust (s)		0.0			0.0							
Total Lost Time (s)		5.1			5.1							
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0			3.0							
Recall Mode		C-Max			C-Max							
Walk Time (s)		15.0			15.0							
Flash Dont Walk (s)		5.0			5.0							
Pedestrian Calls (#/hr)		20			20							
Act Effct Green (s)		115.6			115.6							
Actuated g/C Ratio		0.83			0.83							
v/c Ratio		0.39			0.48							
Control Delay		4.1			2.1							
Queue Delay		0.1			0.1							
Total Delay		4.1			2.2							
LOS		A			A							
Approach Delay		4.1			2.2							
Approach LOS		A			A							
Queue Length 50th (m)		41.5			25.5							
Queue Length 95th (m)		39.3			m25.3							
Internal Link Dist (m)		93.5			100.7			133.3			30.9	
Turn Bay Length (m)												
Base Capacity (vph)		2710			2737							
Starvation Cap Reductn		456			424							
Spillback Cap Reductn		282			0							
Storage Cap Reductn		0			0							
Reduced v/c Ratio		0.46			0.57							

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 114 (81%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.48

Intersection Signal Delay: 3.1

Intersection LOS: A

Intersection Capacity Utilization 42.5%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trillium Pathway & Carling

 Ø2 (R)	 Ø4
104 s	36 s
 Ø5 (R)	
104 s	

Lane Group	Ø4
Total Split (s)	36.0
Total Split (%)	26%
Maximum Green (s)	29.4
Yellow Time (s)	3.0
All-Red Time (s)	3.6
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	22.0
Pedestrian Calls (#/hr)	20
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	166	601	374	348	888	72	346	431	167	112	367	126
Future Volume (vph)	166	601	374	348	888	72	346	431	167	112	367	126
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		0.0	110.0		90.0	75.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.96	0.97		0.98		0.86		0.98		0.97	0.97	
Frt		0.942				0.850		0.958			0.962	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1610	3009	0	1674	3316	1427	1674	3109	0	1537	1624	0
Flt Permitted	0.950			0.950			0.091			0.426		
Satd. Flow (perm)	1553	3009	0	1649	3316	1230	160	3109	0	669	1624	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		95				132		51			12	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		124.7			193.9			164.5			65.2	
Travel Time (s)		7.5			11.6			11.8			4.7	
Confl. Peds. (#/hr)	70		34	34		70	75		60	60		75
Confl. Bikes (#/hr)			13			11			16			6
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	5%	3%	2%	1%	2%	6%	1%	2%	1%	10%	2%	5%
Adj. Flow (vph)	166	601	374	348	888	72	346	431	167	112	367	126
Shared Lane Traffic (%)												
Lane Group Flow (vph)	166	975	0	348	888	72	346	598	0	112	493	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA		Perm	NA	
Protected Phases	5	2		1	6		3	8			4	
Permitted Phases						6	8			4		
Detector Phase	5	2		1	6	6	3	8		4	4	

4: Preston & Carling
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0	10.0	5.0	10.0		10.0	10.0	
Minimum Split (s)	11.2	30.0		11.2	30.0	30.0	11.9	43.9		43.9	43.9	
Total Split (s)	21.0	44.0		28.0	51.0	51.0	24.0	68.0		44.0	44.0	
Total Split (%)	15.0%	31.4%		20.0%	36.4%	36.4%	17.1%	48.6%		31.4%	31.4%	
Maximum Green (s)	14.8	38.0		21.8	45.0	45.0	17.1	61.1		37.1	37.1	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.3		2.5	2.3	2.3	3.6	3.6		3.6	3.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.0		6.2	6.0	6.0	6.9	6.9		6.9	6.9	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead			Lag	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max		None	C-Max	C-Max	None	Ped		Ped	Ped	
Walk Time (s)		7.0			7.0	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)		17.0			17.0	17.0		30.0		30.0	30.0	
Pedestrian Calls (#/hr)		20			20	20		20		20	20	
Act Effct Green (s)	14.8	38.0		21.8	45.0	45.0	61.1	61.1		37.1	37.1	
Actuated g/C Ratio	0.11	0.27		0.16	0.32	0.32	0.44	0.44		0.26	0.26	
v/c Ratio	0.98	1.10		1.34	0.83	0.15	1.36	0.43		0.63	1.12	
Control Delay	132.1	91.3		200.3	41.0	2.9	219.3	26.0		63.3	126.3	
Queue Delay	0.0	0.1		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	132.1	91.4		200.3	41.0	2.9	219.3	26.0		63.3	126.3	
LOS	F	F		F	D	A	F	C		E	F	
Approach Delay		97.3			81.3			96.9			114.6	
Approach LOS		F			F			F			F	
Queue Length 50th (m)	36.6	~139.2		~114.1	121.8	0.0	~102.7	50.1		25.3	~142.7	
Queue Length 95th (m)	#82.6	#173.0		m#127.1	m120.0	m0.0	#159.0	64.7		#47.4	#205.5	
Internal Link Dist (m)		100.7			169.9			140.5			41.2	
Turn Bay Length (m)	65.0			110.0		90.0	75.0					
Base Capacity (vph)	170	885		260	1065	484	254	1385		177	439	
Starvation Cap Reductn	0	15		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.98	1.12		1.34	0.83	0.15	1.36	0.43		0.63	1.12	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.36

Intersection Signal Delay: 94.6

Intersection LOS: F

Intersection Capacity Utilization 124.4%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

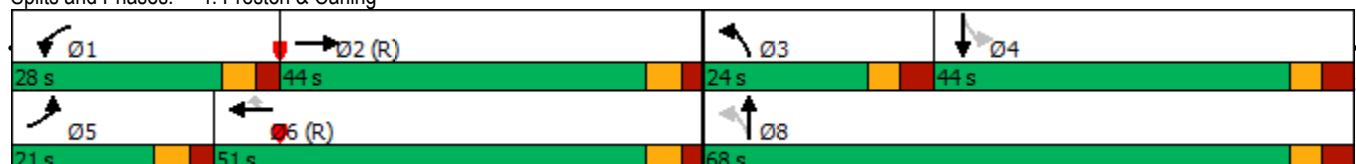
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

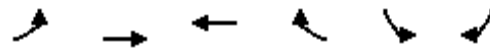
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Preston & Carling



5: Carling & Booth
PM Peak Hour

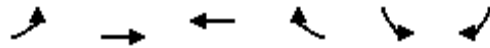
829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	265	676	911	104	308	328
Future Volume (vph)	265	676	911	104	308	328
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	75.0			25.0	0.0	45.0
Storage Lanes	1			1	1	1
Taper Length (m)	25.0				10.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Ped Bike Factor	0.94			0.77	0.98	0.75
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1674	3316	1745	1498	1674	1483
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1578	3316	1745	1159	1647	1117
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				27		266
Link Speed (k/h)		60	60		50	
Link Distance (m)		120.9	518.9		229.0	
Travel Time (s)		7.3	31.1		16.5	
Confl. Peds. (#/hr)	70			70	13	81
Confl. Bikes (#/hr)				11		45
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	2%
Adj. Flow (vph)	265	676	911	104	308	328
Shared Lane Traffic (%)						
Lane Group Flow (vph)	265	676	911	104	308	328
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		7.0	7.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	6.1	30.5	30.5	6.1	6.1	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	1.8	6.1	6.1	6.1
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7	28.7			
Detector 2 Size(m)		1.8	1.8			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA	Perm	Perm	Perm
Protected Phases	5	2	6			
Permitted Phases				6	4	4
Detector Phase	5	2	6	6	4	4

5: Carling & Booth
PM Peak Hour

829 Carling Avenue
2033 Total Traffic



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.9	15.7	29.7	29.7	39.0	39.0
Total Split (s)	25.0	101.0	76.0	76.0	39.0	39.0
Total Split (%)	17.9%	72.1%	54.3%	54.3%	27.9%	27.9%
Maximum Green (s)	19.1	95.3	70.3	70.3	33.0	33.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3
All-Red Time (s)	2.2	2.0	2.0	2.0	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.7	5.7	5.7	6.0	6.0
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	C-Max	None	None
Walk Time (s)			13.0	13.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	26.0	26.0
Pedestrian Calls (#/hr)			20	20	20	20
Act Effct Green (s)	22.2	98.4	70.3	70.3	29.9	29.9
Actuated g/C Ratio	0.16	0.70	0.50	0.50	0.21	0.21
v/c Ratio	1.00	0.29	1.04	0.17	0.88	0.73
Control Delay	83.3	5.6	75.7	14.8	77.9	21.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	83.3	5.6	75.7	14.8	77.9	21.1
LOS	F	A	E	B	E	C
Approach Delay		27.5	69.4		48.6	
Approach LOS		C	E		D	
Queue Length 50th (m)	~77.5	39.8	~250.2	10.2	74.4	12.9
Queue Length 95th (m)	m#87.3	m40.4	#322.1	20.4	#113.7	47.7
Internal Link Dist (m)		96.9	494.9		205.0	
Turn Bay Length (m)	75.0			25.0		45.0
Base Capacity (vph)	265	2330	876	595	388	466
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.29	1.04	0.17	0.79	0.70

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 66 (47%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.04

Intersection Signal Delay: 49.1

Intersection LOS: D

Intersection Capacity Utilization 107.6%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Carling & Booth



6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	41	34	47	121	31	82	515	60	17	533	54
Future Volume (vph)	29	41	34	47	121	31	82	515	60	17	533	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	15.0		0.0	25.0		0.0	25.0		0.0
Storage Lanes	0		0	1		1	1		0	1		0
Taper Length (m)	25.0			20.0			20.0			20.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.93			0.97	0.84	0.97	0.98		0.95	0.99	
Frt		0.956				0.850		0.984			0.986	
Flt Protected		0.986			0.986		0.950			0.950		
Satd. Flow (prot)	0	1578	0	0	1738	1498	1674	1677	0	1674	1673	0
Flt Permitted		0.874			0.894		0.384			0.391		
Satd. Flow (perm)	0	1368	0	0	1529	1261	654	1677	0	655	1673	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				31		15			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		101.4			151.8			160.5			163.2	
Travel Time (s)		7.3			10.9			11.6			11.8	
Confl. Peds. (#/hr)	40		50	50		40	55		80	80		55
Confl. Bikes (#/hr)			2			20			11			18
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	3%	1%	1%	4%	2%
Adj. Flow (vph)	29	41	34	47	121	31	82	515	60	17	533	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	104	0	0	168	31	82	575	0	17	587	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		6	6	

6: Preston & Beech
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6	22.6	33.5	33.5		33.5	33.5	
Total Split (s)	23.0	23.0		23.0	23.0	23.0	67.0	67.0		67.0	67.0	
Total Split (%)	25.6%	25.6%		25.6%	25.6%	25.6%	74.4%	74.4%		74.4%	74.4%	
Maximum Green (s)	17.4	17.4		17.4	17.4	17.4	61.5	61.5		61.5	61.5	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3	2.3	2.2	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6	5.6	5.5	5.5		5.5	5.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Ped	Ped		Ped	Ped	Ped	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	20	20		20	20	20	20	20		20	20	
Act Effct Green (s)		17.2			17.2	17.2	61.7	61.7		61.7	61.7	
Actuated g/C Ratio		0.19			0.19	0.19	0.69	0.69		0.69	0.69	
v/c Ratio		0.37			0.58	0.12	0.18	0.50		0.04	0.51	
Control Delay		28.8			42.0	12.2	2.5	5.1		4.9	8.6	
Queue Delay		0.0			0.0	0.0	0.0	0.1		0.0	0.0	
Total Delay		28.8			42.0	12.2	2.5	5.2		4.9	8.6	
LOS		C			D	B	A	A		A	A	
Approach Delay		28.8			37.3			4.9			8.5	
Approach LOS		C			D			A			A	
Queue Length 50th (m)		11.1			24.6	0.0	2.4	39.6		0.8	37.8	
Queue Length 95th (m)		24.5			43.0	6.6	0.5	1.8		2.6	59.0	
Internal Link Dist (m)		77.4			127.8			136.5			139.2	
Turn Bay Length (m)							25.0			25.0		
Base Capacity (vph)		283			295	268	448	1155		449	1151	
Starvation Cap Reductn		0			0	0	0	84		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.37			0.57	0.12	0.18	0.54		0.04	0.51	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.58

Intersection Signal Delay: 12.0

Intersection LOS: B

Intersection Capacity Utilization 88.6%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 6: Preston & Beech



												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	2	10	0	0	0	8	614	27	6	601	15
Future Volume (vph)	4	2	10	0	0	0	8	614	27	6	601	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.92						0.99			1.00	
Frt		0.916						0.994			0.997	
Flt Protected		0.988						0.999				
Satd. Flow (prot)	0	1494	0	0	0	0	0	1709	0	0	1719	0
Flt Permitted		0.988						0.993			0.995	
Satd. Flow (perm)	0	1466	0	0	0	0	0	1698	0	0	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10						6			3	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		114.6			152.9			73.8			160.5	
Travel Time (s)		8.3			11.0			5.3			11.6	
Confl. Peds. (#/hr)	25		27	27		25	50		47	47		50
Confl. Bikes (#/hr)			1			3			21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	2%	2%	2%	1%	3%	1%	1%	3%	1%
Adj. Flow (vph)	4	2	10	0	0	0	8	614	27	6	601	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	0	0	0	649	0	0	622	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		-2.0			-1.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2					1	2		1	2	
Detector Template	Left	Thru					Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5					6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8					6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex					CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7						28.7			28.7	
Detector 2 Size(m)		1.8						1.8			1.8	
Detector 2 Type		CI+Ex						CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0		10.0	10.0	
Minimum Split (s)	20.5	20.5					28.1	28.1		28.1	28.1	

7: Preston & Pamilla
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Split (s)	21.0	21.0					69.0	69.0		69.0	69.0	
Total Split (%)	23.3%	23.3%					76.7%	76.7%		76.7%	76.7%	
Maximum Green (s)	15.5	15.5					63.9	63.9		63.9	63.9	
Yellow Time (s)	3.3	3.3					3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2					1.8	1.8		1.8	1.8	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.5						5.1			5.1	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Recall Mode	None	None					C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0					18.0	18.0		18.0	18.0	
Flash Dont Walk (s)	8.0	8.0					5.0	5.0		5.0	5.0	
Pedestrian Calls (#/hr)	20	20					20	20		20	20	
Act Effct Green (s)		12.0						75.6			75.6	
Actuated g/C Ratio		0.13						0.84			0.84	
v/c Ratio		0.08						0.45			0.43	
Control Delay		21.9						5.1			3.7	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.9						5.1			3.7	
LOS		C						A			A	
Approach Delay		21.9						5.1			3.7	
Approach LOS		C						A			A	
Queue Length 50th (m)		0.9						29.6			22.8	
Queue Length 95th (m)		5.8						60.4			34.2	
Internal Link Dist (m)		90.6			128.9			49.8			136.5	
Turn Bay Length (m)												
Base Capacity (vph)		260						1428			1437	
Starvation Cap Reductn		0						0			55	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.06						0.45			0.45	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 27 (30%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.45

Intersection Signal Delay: 4.7

Intersection LOS: A

Intersection Capacity Utilization 60.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Preston & Pamilla



8: Preston & Adeline
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	34	4	25	22	2	17	37	553	48	23	606	5
Future Volume (vph)	34	4	25	22	2	17	37	553	48	23	606	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.946			0.944			0.990			0.999	
Flt Protected		0.974			0.974			0.997			0.998	
Satd. Flow (prot)	0	1608	0	0	1605	0	0	1708	0	0	1724	0
Flt Permitted		0.974			0.974			0.997			0.998	
Satd. Flow (perm)	0	1608	0	0	1605	0	0	1708	0	0	1724	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.6			154.3			71.5			73.8	
Travel Time (s)		8.2			11.1			5.1			5.3	
Confl. Peds. (#/hr)							46		47	47		46
Confl. Bikes (#/hr)									21			14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%
Adj. Flow (vph)	34	4	25	22	2	17	37	553	48	23	606	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	63	0	0	41	0	0	638	0	0	634	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		-2.0			-2.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	61.4%											
ICU Level of Service	B											
Analysis Period (min)	15											

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	27	38	56	611	567	78
Future Volume (vph)	27	38	56	611	567	78
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt	0.921				0.984	
Flt Protected	0.980			0.996		
Satd. Flow (prot)	1575	0	0	3273	1703	0
Flt Permitted	0.980			0.996		
Satd. Flow (perm)	1575	0	0	3273	1703	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	68.0			65.2	71.5	
Travel Time (s)	8.2			4.7	5.1	
Confl. Peds. (#/hr)			46			47
Confl. Bikes (#/hr)						14
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	2%	2%	3%	3%	2%
Adj. Flow (vph)	27	38	56	611	567	78
Shared Lane Traffic (%)						
Lane Group Flow (vph)	65	0	0	667	645	0
Enter Blocked Intersection	No	No	Yes	Yes	Yes	Yes
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	5.0			2.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	14	24			14
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 70.5%	ICU Level of Service C					
Analysis Period (min) 15						

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	485	294	4	2	430	435	5	2	1	416	0	655
Future Volume (vph)	485	294	4	2	430	435	5	2	1	416	0	655
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		0.0	30.0		25.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		1	0		0	0		1
Taper Length (m)	25.0			25.0			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00		0.90		0.94		0.93			0.83	0.77
Frt		0.998				0.850		0.983				0.850
Flt Protected	0.950			0.950				0.970			0.950	
Satd. Flow (prot)	1642	1755	0	1674	1762	1498	0	1640	0	0	1674	1483
Flt Permitted	0.154			0.578				0.844			0.752	
Satd. Flow (perm)	266	1755	0	914	1762	1403	0	1365	0	0	1095	1137
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				177		1				99
Link Speed (k/h)		60			60			30			50	
Link Distance (m)		233.9			203.3			76.1			164.5	
Travel Time (s)		14.0			12.2			9.1			11.8	
Confl. Peds. (#/hr)	35		62	62		35	80		65	65		80
Confl. Bikes (#/hr)			2									2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	2%
Adj. Flow (vph)	485	294	4	2	430	435	5	2	1	416	0	655
Shared Lane Traffic (%)												
Lane Group Flow (vph)	485	298	0	2	430	435	0	8	0	0	416	655
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			3.5	
Link Offset(m)		2.0			0.0			5.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2	1	1	2		1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5	6.1	6.1	30.5		6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8	6.1	6.1	1.8		6.1	1.8	6.1
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	5	2			6			8			4	5
Permitted Phases	2			6		6	8			4		4
Detector Phase	5	2		6	6	6	8	8		4	4	5

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	5.0
Minimum Split (s)	11.1	32.1		32.1	32.1	32.1	29.5	29.5		29.5	29.5	11.1
Total Split (s)	39.0	81.0		42.0	42.0	42.0	49.0	49.0		49.0	49.0	39.0
Total Split (%)	30.0%	62.3%		32.3%	32.3%	32.3%	37.7%	37.7%		37.7%	37.7%	30.0%
Maximum Green (s)	32.9	74.9		35.9	35.9	35.9	43.5	43.5		43.5	43.5	32.9
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.3	3.3		3.3	3.3	3.7
All-Red Time (s)	2.4	2.4		2.4	2.4	2.4	2.2	2.2		2.2	2.2	2.4
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1	6.1		5.5			5.5	6.1
Lead/Lag	Lead			Lag	Lag	Lag						Lead
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max	C-Max	Ped	Ped		Ped	Ped	None
Walk Time (s)		7.0		7.0	7.0	7.0	12.0	12.0		12.0	12.0	
Flash Dont Walk (s)		19.0		19.0	19.0	19.0	12.0	12.0		12.0	12.0	
Pedestrian Calls (#/hr)		20		20	20	20	20	20		20	20	
Act Effct Green (s)	74.9	74.9		36.1	36.1	36.1		43.5			43.5	75.6
Actuated g/C Ratio	0.58	0.58		0.28	0.28	0.28		0.33			0.33	0.58
v/c Ratio	0.97	0.29		0.01	0.88	0.84		0.02			1.14	0.83
Control Delay	65.8	15.0		34.5	65.3	41.7		27.5			129.4	28.3
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	0.0
Total Delay	65.8	15.0		34.5	65.3	41.7		27.5			129.4	28.3
LOS	E	B		C	E	D		C			F	C
Approach Delay		46.5			53.4			27.5			67.6	
Approach LOS		D			D			C			E	
Queue Length 50th (m)	91.1	34.2		0.3	97.5	61.0		1.1			~114.1	79.5
Queue Length 95th (m)	#154.2	49.7		2.3	#149.9	#113.1		4.5			#172.0	118.5
Internal Link Dist (m)		209.9			179.3			52.1			140.5	
Turn Bay Length (m)	55.0			30.0		25.0						
Base Capacity (vph)	501	1011		253	488	517		457			366	791
Starvation Cap Reductn	0	0		0	0	0		0			0	0
Spillback Cap Reductn	0	0		0	0	0		0			0	0
Storage Cap Reductn	0	0		0	0	0		0			0	0
Reduced v/c Ratio	0.97	0.29		0.01	0.88	0.84		0.02			1.14	0.83

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 6 (5%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.14

Intersection Signal Delay: 56.9

Intersection LOS: E

Intersection Capacity Utilization 106.8%

ICU Level of Service G

Analysis Period (min) 15

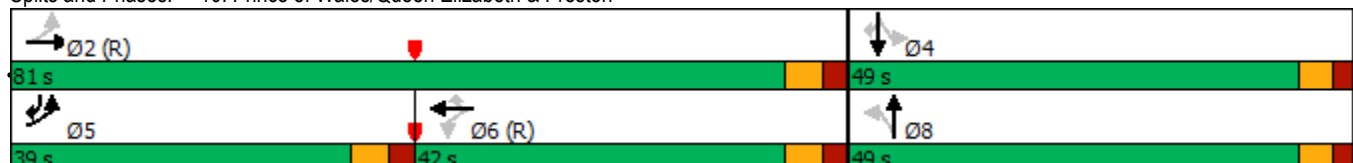
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Prince of Wales/Queen Elizabeth & Preston



11: Site Access & Sidney
PM Peak Hour

829 Carling Avenue
2033 Total Traffic

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	53	0	17	117	0	12
Future Volume (vph)	53	0	17	117	0	12
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.865
Flt Protected				0.994		
Satd. Flow (prot)	1745	0	0	1735	0	1510
Flt Permitted				0.994		
Satd. Flow (perm)	1745	0	0	1735	0	1510
Link Speed (k/h)	30			30	50	
Link Distance (m)	49.5			68.0	41.2	
Travel Time (s)	5.9			8.2	3.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	53	0	17	117	0	12
Shared Lane Traffic (%)						
Lane Group Flow (vph)	53	0	0	134	0	12
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	0.0			0.0	5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	14		24	24		14
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	17.5%			ICU Level of Service A		
Analysis Period (min)	15					