Clifton Property Development Inc.

Assessment of Adequacy of Public Services

316-332 Clifton Road

City of Ottawa, Ontario





CIMA+ file number: A001062 September 4, 2020

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1. Introduction

CIMA+ was retained by Clifton Property Development Inc. to prepare an Assessment of Adequacy of Public Services Report for the proposed construction of a multi-unit residential development comprised of six (6) townhouse blocks, with a total of twenty-nine (29) residential dwellings, at 316-332 Clifton Road in Ottawa, Ontario.

The purpose of this assessment is to confirm that the proposed development can be adequately serviced by the existing municipal infrastructure (water, sanitary, and storm) surrounding the site. This assessment shall be used in support of a Zoning By-law Amendment (ZBLA) prior to proceeding with Site Plan Control.

1.1 Site Description and Proposed Development

The site is located along the west side of Clifton Road, between Wilber Avenue and Scott Street (refer to **Figure 1** below). The subject site is currently comprised of five (5) private residential dwellings. The combined site area (316-332 Clifton Road) measures approximately 0.29 ha.

Generally, the site is bounded by a private residential dwelling (314 Clifton Road) to the north, Clifton Road to the east, Wilber Avenue to the south, and a mixed-use multi-storey residential/commercial building (319 McRae Avenue) to the west.



Figure 1: Site Location - Plan View.



The proposed development is a six (6) block townhouse development with 29 residential units, expected to include approximately 79 residents, with parking available in private garages at each unit. Refer to **Figure 2** for a conceptual site plan of the proposed development (prepared by Hobin Architecture Inc.).



Figure 2: Conceptual Site Plan.

1.2 Review of Available Background Documentation

The following design guidelines have been used to estimate the theoretical servicing requirements for the proposed development; while geoOttawa, a detailed topographic survey prepared by Annis, O'Sullivan, Vollebekk Ltd. (**Appendix B**), and the available as-built drawings (**Appendix A**) provided by the City of Ottawa Information Centre have been used to determine the existing municipal services location, size, material and inverts fronting the site.

- + Ottawa Sewer Design Guidelines (October 2012), including;
 - Technical Bulletins ISTB-2018-01;
- + Ottawa Design Guidelines Water Distribution (2010), including;
 - Technical Bulletins ISTB-2018-02, ISDTB-2014-02 and ISD 2010-02;
- Ministry of the Environment Design Guidelines for Sewage Works (2008);
- + Ministry of the Environment Stormwater Management Planning and Design Manual (2003);
- + Ministry of the Environment Design Guidelines for Drinking-Water Systems (2008); and
- + Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection (1999).



1.3 Existing Infrastructure

As identified using the detailed topographic survey, geoOttawa and the available Utility Record Drawings provided by the City of Ottawa Information Centre, the following municipal infrastructure are available within the right-of-way fronting the proposed development site (refer to **Appendix B** for Existing Conditions Plan).

Clifton Road

- + 152 mm diameter UCI watermain (preferred water connection point);
- + 225 mm diameter Concrete sanitary sewer (preferred sanitary connection point);
- + 450 mm diameter Concrete storm sewer (preferred storm connection point).

1.4 Consultation and Permits

In response to the pre-consultation requirements defined in the City's Development Servicing Study Checklist, the following agencies were consulted in support of the preparation of this report. The Development Servicing Study Checklist as well as all relevant correspondence with the consulted agencies can be found in **Appendix A**.

City of Ottawa

The City of Ottawa Information Centre was contacted to obtain any Reports, Studies, Engineering, and/or Utility Plans including sanitary sewer, storm sewer, watermain, gas, etc. within or adjacent to the site location. The available as-built plans were obtained, while no existing reports or studies were available. Given a detailed utility survey was previously completed by Annis, O'Sullivan, Vollebekk Ltd. for the project the UCC drawings were not obtained.

CIMA+ also contacted Shawn Wessel from the City of Ottawa's Planning, Infrastructure and Economic Development Department to obtain any site-specific servicing and stormwater management design criteria for the proposed development. The provided comments and criteria relevant to the Assessment of Adequacy of Public Services are referenced within the appropriate sections of this report, while the remaining requirements for design will be assessed and addressed at the detailed design stage (Site Plan Control).

Rideau Valley Conservation Authority (RVCA)

The subject site falls under the jurisdiction of the Rideau Valley Conservation Authority (RVCA). CIMA+ contacted Jamie Batchelor from the RVCA to identify any Natural Heritage/Hazards features that may impact the development as well as any Storm Water Management Criteria for the site and required approvals/permits. These criteria are addressed in *Section 4.2* of this Report.

Ministry of the Environment, Conservation and Parks (MECP)

CIMA+ expects that the proposed development will require an Environmental Compliance Approval (ECA) as the development does not meet the exemption requirements per O.Reg. 525/98, section 3(a), when considering the proposed sewage works and stormwater management facility will service multiple owners.

It is expected that the application can be submitted to the MECP through the City of Ottawa's Transfer of Review (ToR) Program. Correspondence has been provided to the City project manager (refer to **Appendix A**).



2. Water Servicing

2.1 Water Supply Design Criteria

The design criteria for determining the water demand requirements for the proposed development follow the parameters outlined in the Ottawa Design Guidelines – Water Distribution (2010) and associated technical bulletins, as well as the MOE Design Guidelines for Drinking-Water Systems (2008). Namely, the following parameters have been used in determining the water demands:

Design Criterion ¹	Residential Areas		
Average Day Demand	350 L/capita/day		
Maximum Daily Demand	7.6 × average daily demand ¹		
Maximum (Peak) Hour Demand	11.5 × average daily demand ¹		
Populations – 3 Bedroom Townhome	2.7 Persons Per Unit		
Desired Operating Pressure under Normal Operating Conditions	50 to 70 psi		
Minimum Operating Pressure under Normal Operating Conditions	40 psi		
Maximum Operating Pressure under Normal Operating Conditions	80 psi		
Minimum Operating Pressure under Maximum Daily Demand + Fire Flow	20 psi		

Table 2-1: Water Supply Design Criteria

In addition to those design criteria identified in **Table 2-1**, the following comments and criteria must be considered in the water supply servicing strategy in accordance with City Guidelines:

- + The subject site is located within the 1W pressure zone;
- Residential buildings with a basic day demand greater than 50 m³/day (0.57 L/s) are required to be connected to a minimum of two (2) water services separated by an isolation valve to avoid a vulnerable service area;
- + Fire flow demand requirements shall be based on the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection 1999 and Technical Bulletin ISTB-2018-02;
- Exposure separation distances shall be defined on a figure to support the FUS calculation and required fire flow (RFF);
- + Hydrant capacity shall be assessed if relying on any public hydrants to provide fire protection, particularly if high design fire flows are being proposed, to demonstrate the Required Fire Flow

¹ Note that residential peaking factors were selected from **Table 3-3** of the MOE Design Guidelines for Drinking-Water Systems for 0 to 500 persons.



(RFF) can be achieved. Identification of which hydrants are being considered to meet the RFF on a fire hydrant coverage figure is required as part of the boundary conditions request.

2.2 Proposed Water Supply Servicing and Calculations

Water Demands

The water supply demands for the proposed development are presented in **Table 2-2** below. The demands were developed utilizing the development statistics provided by Hobin Architects Inc. and those design criteria identified in *Section 2.1*. Refer to **Appendix D** for detailed calculations.

Table 2-2: Water Demands			
Demand Type	Average Daily Demand (L/s)	Maximum Daily Demand (L/s)	Maximum (Peak) Hour Demand (L/s)
Residential	0.32	2.43	3.68
Total	0.32	2.43	3.68

Given the basic day demand is less than 50 m³/day (0.57 L/s), only one connection is required. However, the need for a second connection to the watermain on Clifton Road will be assessed at the detailed design stage in order to provide redundant supply, reduce hydraulic losses, and avoid stagnant water associated with a dead-end water main.

Proposed Water Supply Connection Point(s)

A primary water service to Clifton Road at the north access and a potential secondary connection to Clifton Road (to be confirmed in detailed design) at the south access is proposed. Refer to **Appendix C** for proposed connection points.

Required Fire Flow (RFF)

The required fire flow for the site was developed using the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection 1999 and Technical Bulletin ISTB-2018-02. It was determined that an RFF of **10,000 L/min (167 L/s)** would be required to provide adequate protection.

It was assumed that multiple municipal hydrants would be required to meet the fire flow requirements and a fire hydrant coverage figure was prepared is support of the boundary conditions request from the City.

Refer to **Appendix D** for detailed calculations, including supporting figures for exposure distances and hydrant coverage.

Municipal Boundary Conditions

Using the proposed demands, required fire flow and supporting figures the City provided boundary conditions for hydraulic analysis for current conditions, based on computer model simulation. The boundary conditions are as follows:



Hydraulic Condition (HGL = Hydraulic Grade Line)	Boundary Condition (Head) (m) Clifton Rd. 152mm dia.
Minimum HGL	108.5
Maximum HGL	114.7

Table 2-3: Watermain Boundary Conditions

A Multi-Hydrant Analysis was performed by the City utilizing the two nearest available hydrants on Clifton Road as identified on the Hydrant Coverage Figure prepared by CIMA+ (refer to **Appendix D**). The total available flow from these hydrants was calculated as follows:

..

Table 2-4: Available Hydrant Flows			
Hydrant Location	Available Flow (L/s)		
South of Site	85		
North of Site	70		
Total	155		

. .

Hydraulic Analysis – Water Supply Adequacy

A hydraulic analysis was completed utilizing the boundary condition information provided by the City for the proposed development in order to confirm that there is adequate flow and pressure in the water distribution system to meet the required water demands. The following Table summarizes the available flow and pressure in the system under each demand scenario:

		Available Flow/Pressure			
Demand Type	Proposed Demand (L/s)	Design Operating Pressure (Relative Head) (m)	Design Operating Pressure (psi)	Desired Flow/Pressure Objective	Flow/Pressure Objective Achieved?
Average Daily Demand	0.32	50.5	72	50 to 70 psi	Yes
Maximum Day Demand + Fire Flow	169.10	155 L/s @ 20 psi		≥ 20 psi	No
Maximum (Peak) Hour Demand	3.68	44.3	63	50 to 70 psi	Yes

Table 2-5: Water Supply Adequacy - Hydraulic Analysis



NOTES:

- 1. Required fire flow demand was calculated as **10,000 L/min (167 L/s)**.
- 2. The minimum HGL elevation at Connection Points 1 and 2 is **108.5 m** and the maximum HGL elevation is **114.7 m**.
- 3. Boundary conditions for Connection 1 (North) to Clifton Road assumes a ground elevation of 64.20 m.
- 4. Boundary conditions for Connection 2 (South) to Clifton Road assumes a ground elevation of 64.40 m.

2.3 Water Supply Summary and Conclusions

The water supply design for the proposed development follows the parameters outlined in the Ottawa Design Guidelines – Water Distribution (2010) and associated technical bulletins, as well as the MOE Design Guidelines for Drinking-Water Systems (2008).

Based on the boundary conditions provided by the City an additional private hydrant will be required on site to provide adequate fire flow. There is adequate flow and pressure in the water distribution system to meet the required potable water demands for the proposed development.

3. Sanitary Servicing

3.1 Sanitary Servicing Design Criteria

The design criteria for determining the sanitary peak flow rates for the proposed development follow the parameters outlined in the City of Ottawa Sewer Design Guidelines, 2012 and City of Ottawa Technical Bulletin ISTB-2018-01. Namely, the following parameters have been used in determining the peak sanitary flow rates:

Design Criterion	Residential Areas	
Base Flow	280 L/capita/day	
Populations – 3 Bedroom Townhome	2.7 Persons Per Unit	
	Determined by Harmon Equation	
Peaking Factor	$P.F. = 1 + \left[\frac{1}{4 + \left(\frac{P}{1,000}\right)^{\frac{1}{2}}}\right] \times 0.8$ (P = population; P.F. = peaking factor) Maximum P.F. = 4.0	
	Minimum P.F. = 2.0	
Dry Weather Infiltration Rate	0.05 L/s/effective gross hectare (for all areas)	
Wet Weather Infiltration	0.28 L/s/effective gross hectare (for all areas)	
Total Infiltration Allowance	0.33 L/s/effective gross hectare (for all areas)	

Table 3-1: Sanitary Peak Flow Determ	nination Design Criteria
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3.2 **Proposed Sanitary Servicing and Calculations**

Proposed Sanitary Peak Flows

The estimated peak flows from the proposed development based on the design criteria listed in **Table 3-2** are outlined in the following Table.

Flow Type	Total Flow Rate (L/s)
Total Estimated Average Dry Weather Flow Rate	0.26
Total Estimate Peak Dry Weather Flow Rate	0.93
Total Estimate Peak Wet Weather Flow Rate	1.02

Refer to **Appendix E** for detailed calculations.

Proposed Sanitary Service Connection Point

The proposed sanitary service will connect to the existing 225 mm diameter Concrete sanitary sewer within the right-of-way of Clifton Road. Wastewater flows to the West Nepean Trunk Collector sewer system. Refer to **Appendix C** for proposed connection points.

3.3 Sanitary Servicing Summary and Conclusions

The sanitary servicing design for the proposed development conforms to the requirements of the City of Ottawa Sewer Design Guidelines, 2012, and Technical Bulletin ISTB-2018-01.

Peak wastewater demands were provided to the City, who confirmed that there is adequate residual capacity in the city system to accommodate the proposed wastewater flow (refer to **Appendix E**).

4. Storm Servicing and Stormwater Management

4.1 Background

As previously mentioned, the subject site of 316-332 Clifton Road currently occupies five (5) residential dwellings. The site is approximately 67% pervious with no existing stormwater measures on site (i.e. catch basins, sewers, etc.) and it is thus assumed that there are no current stormwater management controls on site. Based on available recent survey information the site is relatively flat. As such storm runoff from the front yards of the existing properties generally flows to the storm system on Clifton Road, and the rear yards of the existing properties generally flow to Wilber Avenue, which connects into the storm system on Clifton Road.

Considering there are no current stormwater systems on site and that it is assumed that there are no flow attenuation controls the anticipated peak flows for the existing site are as follows (refer to **Appendix F**):



Storm Event	Release Flow (L/s)	
5-year	38.4	
100-year	81.9	

Table 4-1: Pre-Development Peak Release Flows – Existing Site

Storm runoff from the site enters the municipal system along Clifton Road and ultimately discharges to the Ottawa River approximately 3.4 km downstream from the site. Refer to **Appendix F** for sketch demonstrating the flow path to the ultimate outlet.

4.2 Storm Servicing Strategy and Design Criteria

The design of the major and minor storm systems must ensure that the following criteria are upheld under post-development conditions, in keeping with the requirements of the City and the Rideau Valley Conservation Authority (refer to **Appendix A**).

- The allowable release rate for the site shall coincide with the 5-year storm event under predevelopment conditions;
- + The allowable release rate shall take into consideration any increase in uncontrolled runoff from increases in hard surface in the boulevard (concrete, interlocking paving stone, etc.).
- The pre-development runoff coefficient (C) shall be a maximum equivalent 'C' of 0.50, or the actual existing site runoff coefficient, whichever is less;
- The pre-development Time of Concentration (Tc) shall be calculated using an appropriate method and must not be less than 10 minutes;
- + A Tc of 10 minutes shall be used for all post-development calculations;
- + Storm runoff in excess of the allowable 5-year pre-development release rate, up to and including the 100-year storm event, must be detained on site;
- Where an underground storage tank or cistern is proposed and calculated utilizing the Modified Rational Method an average release rate equal to 50% of the peak allowable rate shall be applied to estimate the required volume to account for fluctuating head and release rate;
- Given the receiving storm sewer is small (450 mm diameter) and this is an uncontrolled system, gravity connections of foundations to the storm sewer are discouraged and the City recommends that sump pumps be used;
- + Overland flow will generally be directed to Clifton Road;
- Based on the distance of the site from the ultimate outlet at the Ottawa River being more that 2 km the RVCA would not insist on additional on-site water quality treatment.



4.3 Proposed Storm Servicing and Stormwater Management Design and Calculations

Proposed Storm Service Connection Point

Based on communications with the City, it is understood that the preferred and anticipated stormwater connection from the proposed development will discharge to the existing 450 mm concrete storm sewer on Clifton Road. Refer to **Appendix C** for proposed connection points.

Pre-development (Allowable) Release Rates

The pre-development release rates are summarized in the following Table:

Catchment ID	Area (ha)	Runoff Coefficient (C)	Time of Concentration (Tc) (minutes)	Rainfall Intensity (mm/hr)	Release Rate (L/s)
Subject Site	0.295	0.45	10	104.19	38.4

Table 4-2: Pre-development (Allowable) Release Rate (2-year event)

The storm runoff under post-development conditions for the site area must be controlled to the allowable 5-year pre-development release rate of **38.4 L/s**, up to and including the 100-year storm event.

Post Development Flow Rates and Stormwater Quantity Control

The anticipated post-development flow rates and required storage when controlled to the allowable pre-development release rate are summarized in the following Table.

Control Area	100-year Release Rate (L/s)	100-year Surface Storage Volume (m³)	Modified 100-year Release Rate for Underground Retention (L/s)	100-year Underground Retention Storage Volume (m³)	
Roof Areas	8.0	45.2	-	-	
Attenuated Areas (not to tank)	3.5	0.9	-	-	
Attenuated Areas (to tank)	13.0	-	6.5	36.0	
Unattenuated Areas	13.9	0	-	-	
Total	38.4	46.1	6.5	36.0	

Table 4-3: Post-development Flow Rate and Storage Summary

The total post development release rate takes into consideration the increase in uncontrolled runoff from the boulevard (unattenuated areas) being converted to a hard surface (concrete,



interlocking paving stone, etc.). Thus, the remaining allowable release rate for the attenuated site area is **24.5 L/s**.

When underground storage is used (such as a storage tank) the release rate will fluctuate as the head rises within the tank, which can have a significant impact on the storage requirements. To account for this fluctuation the City requests that storage volumes be calculated using 50% of the peak flow rate when using the Modified Rational Method or using a dynamic computer model. For this preliminary design phase, the Modified Rational Method is utilized, using a modified release rate of **6.5 L/s** for the determination of underground storage requirements.

As demonstrated in **Table 4-3** an anticipated storage volume of **36.0** m³ shall be required on-site via an underground storage tank, with **46.1** m³ of storage proposed on the roofs and surface to restrict stormwater discharge to the allowable release rate of **38.4** L/s. Refer to **Appendix F** for detailed stormwater storage calculations.

Stormwater Quality Control

Through consultation with the Rideau Valley Conservation Authority (RVCA) (refer to **Appendix A**) it was confirmed that they would not require any onsite water quality control measures save and except best management practices.

4.4 Storm Servicing and Stormwater Management Summary and Conclusions

The storm servicing design for the proposed development conforms to the requirements of the City of Ottawa Sewer Design Guidelines, 2012, and associated Technical Bulletins.

The allowable release rate for the site post-development was calculated to be **38.4 L/s**. It is expected that this can be achieved via surface storage, roof retention and underground storage.

5. Conclusion

The purpose of this assessment is to confirm that the proposed development can be adequately serviced using the existing municipal infrastructure (water, sanitary, and storm) surrounding the site. This assessment shall be used in support of a Zoning By-law Amendment (ZBLA) to allow for the construction of six (6) townhouse blocks at 316-332 Clifton Road.

The important information and findings as a result of this assessment are as follows:

- The proposed development is expected to include six (6) townhouse blocks comprising 29 units, with a population of approximately 79 persons;
- The proposed development is expected to require an Environmental Compliance Approval (ECA) as per the Ontario Water Resources Act;
- The anticipated water demands for the proposed site are 0.32 L/s (average day), 169.10 L/s (max day + fire flow), and 3.68 L/s (peak hour). Based on the boundary conditions provided by the City an additional private hydrant will be required on site to provide adequate fire flow. There is adequate flow and pressure in the water distribution system to meet the required potable water demands for the proposed development.
- The estimated sanitary flow for the proposed development is 0.26 L/s (average dry weather),
 0.93 L/s (peak dry weather), and 1.02 L/s (peak wet weather). The City of Ottawa has



indicated that the existing sanitary sewer network near the proposed site can accept the peak wet weather sanitary flow of the proposed development;

- Storm runoff in excess of the allowable 5-year pre-development release rate, up to and including the 100-year storm event, will be detained on site via surface, roof and underground retention prior to being discharged to the municipal storm sewer system;
- + The allowable stormwater release rate for the proposed site is **38.4** L/s. It is expected that this will be achieved via roof retention, surface storage and an underground retention tank.
- The site is approximately 67% pervious with no existing stormwater measures on site (i.e. catch basins, sewers, etc.) and it is thus assumed that there are no current stormwater management controls on site. As such storm runoff from the front yards of the existing properties generally flows to the storm system on Clifton Road, and the rear yards of the existing properties generally flow to Wilber Avenue, which connects into the storm system on Clifton Road;
- Quality control of stormwater is not required for the site given its distance from the receiving watercourse;
- + As a result of the conclusions drawn by the previous points, it is expected that the proposed development can be serviced by the existing municipal services network surrounding the site.

We trust this Assessment of Adequacy of Public Services Report is to your satisfaction. If you have any questions regarding this report, please do not hesitate to contact any of the signatories.





Appendix A Pre-consultation Correspondence





Servicing Study Guidelines for Development Applications				
4. Develop	ment Servicing Study Checklist			
4.1 Genera	l Content			
Required Co	ntent	Reference Location		
	Executive Summary (for larger reports only).	N/A		
\checkmark	Date and revision number of the report.	Cover Sheet		
_	Location map and plan showing municipal address, boundary, and layout of proposed development.	Report Figures		
_	Plan showing the site and location of all existing services.	Appendix B		
\checkmark	Development statistics, land use, density, adherence to zoning and official plan, and reference to	Section 1.1		
	applicable subwatershed and watershed plans that provide context to which individual developments			
	Summary of Pre-consultation Meetings with City and other approval agencies.	Section 1.4		
\checkmark	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies,	Section 1.2 & 1.4		
	Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the			
	proponent must provide justification and develop a detendable design criteria.	Section 1 0 2 1 3 1 & 4 2		
	Identification of existing and proposed infrastructure available in the immediate area	Section 1.3 & Annendix B		
	Identification of Environmentally Significant Areas, watercourses and Municipal Drains natentially			
	impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	N/A		
	Concept level master grading plan to confirm existing and proposed grades in the development. This is	N/A		
	required to commit the reasibility of proposed stormwater management and drainage, soil removal and the constraints, and notential impacts to neighbouring properties. This is also required to confirm that the			
	proposed grading will not impede existing major system flow paths.			
	Identification of potential impacts of proposed piped services on private services (such as wells and septic	N/A		
	fields on adjacent lands) and mitigation required to address potential impacts.	1		
	Proposed phasing of the development, if applicable.	N/A		
	Reference to geotechnical studies and recommendations concerning servicing.	N/A		
	All preliminary and formal site plan submissions should have the following information:	N/A		
	- Metric scale; North Arrow (including construction North):			
	- Key Plan			
	- Name and contact information of applicant and property owner;			
	- Property limits including bearings and dimensions;			
	- Existing and proposed structures and parking areas;			
	 Easements, road widening and rights-of-way; 			
	- Adjacent street names.			
4.2 Develo	pment Servicing Report: Water			
Required Co	ntent	Reference Location		
	Confirm consistency with Master Servicing Study, if available	N/A		
	Availability of public infrastructure to service proposed development	Section 1.3 & Appendix B		
	Identification of system constraints	Section 2.1 & 2.2		
✓	Identify boundary conditions	Section 2.2		
	Confirmation of adequate domestic supply and pressure	Section 2.2 & 2.3		
	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Section 2.2 & 2.3		
	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	N/A		
	Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A		
Image: A start of the start	Address reliability requirements such as appropriate location of shut-off valves	Section 2.2 & Appendix C		
	Check on the necessity of a pressure zone boundary modification.	N/A		
✓	Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient	Table 2-5		
	water for the proposed land use. This includes data that shows that the expected demands under average			
	day, peak nour and fire flow conditions provide water within the required pressure range	NI/A		
	the existing system, provisions for necessary looning, and annurtenances (values, pressure reducing values)	IN/A		
	valve chambers, and fire hydrants) including special metering provisions.			

Servicing Study Guidelines for Development Applications				
	Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A		
I	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Section 2.3		
	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference	N/A		
4 3 Develo	nment Servicing Report: Wastewater			
Required Co	ntont	Reference Location		
	Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of	Section 3.1		
	Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used	Section 3.1		
	to justify capacity requirements for proposed infrastructure).			
	Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A		
	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A		
✓	Description of existing sanitary sewer available for discharge of wastewater from proposed development	Section 1.3, 3.2 & Appendix B		
	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Section 3.3		
✓	Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Section 3.2 & Appendix E		
I	Description of proposed sewer network including sewers, pumping stations, and forcemains.	Section 3.2		
	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality.	N/A		
	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new numping station to service development	N/A		
	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A		
	Identification and implementation of the emergency overflow from sanitary pumping stations in relation	N/A		
	to the hydraulic grade line to protect against basement flooding.	Ν/Δ		
	pmont Servicing Depart: Stormwater Checklict	1975		
4.4 Develo	phient Servicing Report: Stormwater Checknst	Deferrer es Lesetien		
Required Co.	Description of drainage outlets and downstream constraints including logality of outlets (i.e. municipal	Section 4.1		
	drain, right-of-way, watercourse, or private property)	Section 4.1		
\checkmark	Analysis of available capacity in existing public infrastructure.	Section 4.1		
I	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Appendix C & F		
	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 4.2		
	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 4.2		
7	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 4.3, 4.4 & Appendix C		
	Set-back from private sewage disposal systems.	N/A		
	Watercourse and hazard lands setbacks.	N/A		
	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that	Appendix A		
	Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A		
	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year cature period) and major events (1:100 year return period)	Section 4.3 & Appendix F		
	Identification of watercourses within the proposed development and how watercourses will be protected,	N/A		
	Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 4.1 & 4.3		

Servicing Study Guidelines for Development Applications			
	Any proposed diversion of drainage catchment areas from one outlet to another.	N/A	
	Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	N/A	
	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.	N/A	
	Identification of potential impacts to receiving watercourses	N/A	
	Identification of municipal drains and related approval requirements.	N/A	
I	Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 4.3 and 4.4	
	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	N/A	
	Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A	
	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	N/A	
	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A	
	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A	
4.5 Approv	val and Permit Requirements: Checklist		
Required Co	ntent	Reference Location	
	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/A	
1	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A	
	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. Changes to Municipal Drains.	N/A N/A	
	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. Changes to Municipal Drains. Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A N/A N/A	
4.6 Conclu	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. Changes to Municipal Drains. Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) sion Checklist	N/A N/A N/A	
4.6 Conclu Required Co	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. Changes to Municipal Drains. Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) sion Checklist ntent	N/A N/A N/A Reference Location	
□ □ 4.6 Conclu Required Co ☑	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. Changes to Municipal Drains. Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) sion Checklist ntent Clearly stated conclusions and recommendations	N/A N/A N/A Reference Location Section 5.0	
□ □ 4.6 Conclu Required Co ☑	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. Changes to Municipal Drains. Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) sion Checklist ntent Clearly stated conclusions and recommendations Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	N/A N/A N/A Reference Location Section 5.0	

Jaymeson Adams

From:	Wessel, Shawn <shawn.wessel@ottawa.ca></shawn.wessel@ottawa.ca>
Sent:	August 27, 2020 10:36 AM
То:	Tim Kennedy; Deiaco, Simon
Cc:	Christian Lavoie-Lebel; Jaymeson Adams
Subject:	RE: 316 - 332 Clifton Road - City Servicing Requirements and Design Criteria

Good morning Mr. Kennedy.

Please find boundary conditions as requested:

****The following information may be passed on to the consultant, but do NOT forward this e-mail directly.****

The following are boundary conditions, HGL, for hydraulic analysis at 316-332 Clifton Road (zone 1W) assumed to be both connected to the 152mm on Clifton Road (see attached PDF for location).

Minimum HGL = 108.5m

Maximum HGL = 114.7m

Connection 1: Available Fire Flow @ 20 psi = 85 L/s

Connection 2: Available Fire Flow @ 20 psi = 70 L/s

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji Project Manager - Infrastructure Approvals Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale Planning, Infrastructure and Economic Development Department | Direction générale de la planification de l'infrastructure et du développement économique City of Ottawa | Ville d'Ottawa 110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1 (613) 580 2424 Ext. | Poste 33017 Int. Mail Code | Code de Courrier Interne 01-14 <u>shawn.wessel@ottawa.ca</u>

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Please note that, while my work hours may be affected by the current situation, I still have access to email, video conferencing and telephone. Feel free to schedule video conferences and/or telephone calls, as necessary.

From: Tim Kennedy <Tim.Kennedy@cima.ca>
Sent: August 20, 2020 8:38 AM
To: Wessel, Shawn <shawn.wessel@ottawa.ca>; Deiaco, Simon <Simon.Deiaco@ottawa.ca>
Cc: Christian Lavoie-Lebel <Christian.Lavoie-Lebel@cima.ca>; Jaymeson Adams <Jaymeson.Adams@cima.ca>
Subject: 316 - 332 Clifton Road - City Servicing Requirements and Design Criteria

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Hi Shawn and Simon,

I was given your contact information by a team member for this file who noted you would be the engineer and planner on file respectively. CIMA+ will be leading the civil design and I am hoping you have some availability this week to discuss the servicing requirements and design criteria for the above mentioned development. Would sometime on Friday between 9:30 and 1:30 work?

We have completed some preliminary analysis in regards to (1) Sanitary Demands and (2) Water/Fire Demands as follows:

- 1) <u>SANITARY DEMANDS</u>: Please find the proposed development information below and detailed calculations attached (I have also attached the Architectural Concept Plans for reference):
 - A. <u>Type of Development and Units</u>: The proposed development involves the construction of six (6) townhouse blocks comprising thirty-one (31) residential units with private garages.
 - B. Site Address: 316–322 Clifton Road.
 - C. <u>Location of Services:</u> connection to existing 225 mm diameter concrete sanitary sewer on Clifton Road with new maintenance hole on sewer main anticipated.
 - D. Total Estimated Average Dry Weather Flow Rate: 0.27 L/s
 - E. Total Estimated Peak Dry Weather Flow Rate: 0.98 L/s
 - F. Total Estimated Peak Wet Weather Flow Rate: 1.08 L/s

Could you please confirm if there is enough capacity in the City system to accommodate the proposed wastewater flow.

<u>WATER AND FIRE FLOW DEMANDS</u>: Please find the proposed development information below and detailed calculations and associated figures attached, including: (1) Water Demand Calculations, (2) Fire Flow Calculations, (3) Figure 1 – Proposed Water Service Connection Locations, (4) Figure 2 – Exposure Separation Distances, (5) Figure 3 – Fire Hydrant Coverage.

- 1. <u>Type of Development and Units:</u> The proposed development involves the construction of six (6) townhouse blocks comprising thirty-one (31) residential units with private garages.
- 2. Site Address: 316–322 Clifton Road.
- 3. **Location of Services:** Please see attached Figure 1.
- 4. <u>Plan showing Proposed Water Connections:</u> Please see attached Figure 1.
- 5. Average Daily Demand: 0.34 L/s
- 6. Maximum Daily Demands: 2.52 L/s
- 7. Peak Hour Demand: 3.81 L/s
- 8. Required Fire Flow (RFF): 10,000 L/min

Could you please provide the boundary conditions for the proposed development.

If you have any questions or require anything further, please do not hesitate to contact me on my cell (working from home these days).

Looking forward to collaborating on this one.

Regards,

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TIM KENNEDY, P.Eng. Project Manager / Infrastructure

T 613-860-2462 ext. 6620 M 613-462-3627 F 613-860-1870 110-240 Catherine Street, Ottawa, ON K2P 2G8 CANADA

Notice to our customers on the COVID-19





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Jaymeson Adams

From: Wessel, Shawn <shawn.wessel@ottawa.ca> Sent: August 24, 2020 1:41 PM To: Tim Kennedy Cc: Jaymeson Adams Subject: RE: 316 - 322 Clifton Road - City Pre-consult

I apologize, we had provided you with the incorrect criteria for this site.

Please see the following:

The impact to the sanitary is negligible and as for storm they will can to do SWM on site since this appears to be a site plan. The storm system will need to be controlled to the 5-year using the lesser of C=0.5 or existing. Note that the storm pipe in the street is small (300 mm) and this is an uncontrolled system. I would highly discourage any gravity connection of foundations to the storm sewer and would instead ask that they use sump pumps.

We apologize for the inconvenience

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji **Project Manager - Infrastructure Approvals** Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale Planning, Infrastructure and Economic Development Department | Direction générale de la planification de l'infrastructure et du développement économique City of Ottawa | Ville d'Ottawa 110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1 (613) 580 2424 Ext. | Poste 33017 Int. Mail Code | Code de Courrier Interne 01-14 shawn.wessel@ottawa.ca



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From: Wessel, Shawn
Sent: August 24, 2020 11:19 AM
To: Tim Kennedy <Tim.Kennedy@cima.ca>
Cc: Jaymeson Adams <Jaymeson.Adams@cima.ca>
Subject: 316 - 322 Clifton Road - City Pre-consult

Good morning Mr. Kennedy.

Message from Water Resources Dept:

Storm should not be an issue since SWM will deal with any excess flows. We would ask that consultant provide us with the proposed sanitary flows and we can enter that in our model to see if there will be any impact. Infrastructure is pre 1970, therefore Control to 2-year C=0.5

Please let me know if you want to discuss this further, or wait for Water Resources Dept. to review your sanitary flows and allow them to provide comments.

I have provided Jaymeson with some details regarding seperating sewer laters for foundation and roof drains, that being two storm and one sanitary and water service for each TH, if using flat roofs. Alternatively, sloped roofs with either a cistern in each unit, or downspouts are also permitted. DS must discharge 1.5 metres from property line as well as foundation walls and not allow for City SWs to become a hazzard in winter time, due to ice build up.

For flat roofs, a roof plan illustrating 5 and 100 year ponding, scuppers and roof drain locations is required as well as a table including, but not limited to, roof drain number, drain type (manufacturer, product name & number), weir opening etc.

In addtion to above, the apllicant must have their mechanical and structural engineer complete, stamp date and sign a Roof Flow Control Declaration Form.

2020 Roof Flow Control Declaration Fo

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji Project Manager - Infrastructure Approvals

Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale Planning, Infrastructure and Economic Development Department | Direction générale de la planification de l'infrastructure et du développement économique City of Ottawa | Ville d'Ottawa 110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1 (613) 580 2424 Ext. | Poste 33017 Int. Mail Code | Code de Courrier Interne 01-14 shawn.wessel@ottawa.ca



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Jaymeson Adams

ISD Information Centre / Centre Information <informationcentre@ottawa.ca></informationcentre@ottawa.ca>
August 26, 2020 10:13 AM
Jaymeson Adams
20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request
1246p&p1-ab.pdf 8273-08.pdf; 20-0892.xlsx

Good morning Jaymeson,

Attached are the plans and work order for the locations requested.

The City of Ottawa's Financial Services Branch will send out an invoice at the end of the month. <u>Please retain the attached work order for your records</u>. A copy will not be included with the <u>invoice sent at the end of the month</u>.

If you have any concerns regarding this information, please refer to the contact information below.

I hope this helps.

Thank you.

Kind regards,

Brenda

For further information, please contact;

Geospatial Analytics Technology & Solutions Branch - Information Centre Unit

Phone: 613-580-2424 x 44455 Email: <u>informationcentre@ottawa.ca</u>

From: Jaymeson Adams <Jaymeson.Adams@cima.ca>

Sent: August 26, 2020 8:50 AM

To: ISD Information Centre / Centre Information <informationcentre@ottawa.ca>

Cc: Tim Kennedy <tim.kennedy@cima.ca>; Christian Lavoie-Lebel <Christian.Lavoie-Lebel@cima.ca> Subject: RE: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

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Good morning Brenda,

Thank you for the information. In that case, please keep CIMA as the recipient in the header of the estimate.

Please proceed.

Thanks,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Sent: August 26, 2020 8:47 AM
To: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
Subject: RE: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Good morning Jaymeson,

Your previous email had asked if we could bill Clifton Property Developments.

We cannot not do third party billing.

Brenda

From: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
Sent: August 26, 2020 8:24 AM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Cc: Tim Kennedy <<u>tim.kennedy@cima.ca</u>>; Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

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Good morning Brenda,

In a follow-up to below, are you able to proceed with providing the two (2) PDF plan & profile drawings while the estimate is being revised?

Thanks,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: Jaymeson Adams
Sent: August 26, 2020 8:12 AM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Cc: Tim Kennedy <<u>Tim.Kennedy@cima.ca</u>>; Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>>
Subject: RE: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Good morning Brenda,

Thank you for the updated estimate.

I require one more change to the header of the estimate before we can proceed. The cost should be billed to the following instead of CIMA+:

Clifton Property Development Inc. 100 Smirle Avenue K1Y 0S3

Please revise accordingly.

Thanks,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Sent: August 25, 2020 7:14 PM
To: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Hi Jaymeson,

Attached is an updated estimate as per your request. Please advise if we can proceed.

Thank you.

Brenda

From: Jaymeson Adams <Jaymeson.Adams@cima.ca Sent: August 25, 2020 3:48 PM To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>> Cc: Tim Kennedy <<u>tim.kennedy@cima.ca</u>>; Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>> Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

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Hi Brenda,

Thank you very much. After discussion with my Project Managers, the UCC info will not be required in any format (PDF or CAD).

Please see below for my revised request:

COST ESTIMATE

Project: 316-332 Clifton Rd.: Scott St. to Richmond Rd.

Description	Quantity	Unit Price	Amount
Administration Fee	1	\$78.00	\$78.00
Engineering Planpdf	2	\$16.00	\$32.00
UCC Central Registry Utility Planspdf	2	\$45.00	\$90.00
		Sub-Total	\$200.00 <mark>\$110.00</mark>
		HST	\$26.00 <mark>\$14.30</mark>
		Total	\$226.00 <mark>\$124.30</mark>

Would you be able to provide me a revised cost estimate using the above table?

Thanks,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>
 Sent: August 25, 2020 1:36 PM
 To: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>
 Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Hello Jaymeson,

Attached is an updated estimate. The cost for 2 UCC pdf files is shown in the estimate.

Looking forward to your reply on how you would like to proceed.

Thanks so much.

Brenda

From: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>> Sent: August 21, 2020 1:09 PM To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>> Cc: Tim Kennedy <<u>tim.kennedy@cima.ca</u>> Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

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Hi Brenda,

Thank you for the reply and update. Have a great weekend!

Best regards,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>> Sent: August 21, 2020 1:07 PM To: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>> Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Good afternoon Jaymeson,

Thank you for your reply. I am awaiting confirmation on an updated estimate. Once I have this I can forward for your approval to proceed.

Apologies for the delay, I hope to hear back soon.

Kind regards,

Brenda

From: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
Sent: August 19, 2020 3:05 PM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Cc: Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>>; Tim Kennedy <<u>tim.kennedy@cima.ca</u>>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

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Hi Brenda,

Thanks for your patience in my response. <u>We would like to proceed with obtaining the two (2) PDF's but <mark>it is not</mark> necessary to obtain the CAD utility drawing.</u>

Therefore, we expect that the cost for the required information will be **\$124.30 (including HST).**

The drawings can be invoiced to the following:

Clifton Property Development Inc. 100 Smirle Avenue K1Y 0S3

Please prepare an updated Work Order with this information.

Thank you,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+ From: Jaymeson Adams
Sent: August 14, 2020 8:12 AM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Subject: RE: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Hi Brenda,

I will be discussing the cost estimate you provided with my Project Manager. I will let you know as soon as a decision is made.

Thanks,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Sent: August 12, 2020 2:32 PM
To: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
Cc: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Good afternoon Jaymeson,

Please note for the UCC portion of request UCC MH and Fronting in .dwg format would be the same $cost - 0.5 \times 143.00 = \71.50 .

Plan and profiles are an additional fee as shown on the attached along with the administration fee.

Looking forward to your reply on how you would like to proceed.

Thank you.

Brenda

For further information, please contact;

Geospatial Analytics Technology & Solutions Branch - Information Centre Unit

Phone: 613-580-2424 x 44455 Email: <u>informationcentre@ottawa.ca</u> From: Jaymeson Adams <Jaymeson.Adams@cima.ca>
Sent: August 12, 2020 11:29 AM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Cc: Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>>; Hugues Bisson <<u>Hugues.Bisson@cima.ca</u>>; Tim Kennedy
<<u>tim.kennedy@cima.ca</u>>; Greg Santyr <<u>Greg.Santyr@cima.ca</u>>
Subject: RE: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

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Hi Brenda,

We will only require UCC plans and plan & profiles.

Could you send us a cost estimate for (1) manhole to manhole and (2) fronting property?

Ideally any information you have on Clifton Avenue between Scott Street and Richmond Road would be perfect for the estimates.

Thanks,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures T 613-860-2462 ext. 6659 F 613-860-1870 CIMA+

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Sent: August 12, 2020 11:07 AM
To: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
Cc: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

Hello Jaymeson,

Will you only need UCC plans and plan and profiles?

With that information we can provide an estimate as per requested.

Looking forward to your reply.

Brenda

From: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Sent: August 12, 2020 10:56 AM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

For UCC – fronting the site only. Mh to Mh, whole street segment? Please confirm for estimate [JA Comment: Please see above for clarification]

From: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>
Sent: August 12, 2020 9:44 AM
To: ISD Information Centre / Centre Information <<u>informationcentre@ottawa.ca</u>
Cc: Tim Kennedy <<u>tim.kennedy@cima.ca</u>
; Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>
; Hugues Bisson
<<u>Hugues.Bisson@cima.ca</u>
; Greg Santyr <<u>Greg.Santyr@cima.ca</u>
Subject: 20-0892 - 316–332 Clifton Road - Servicing Capacity Assessment - Information Request

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We are working with a client on a servicing capacity assessment for a zoning by-law amendment at 316–332 Clifton Road (refer to the attached Key Plan).

Our client is considering a servicing connection for the proposed development to Clifton Road. Could you please provide any available background information for the existing services and utilities that may be present at these locations, including but not limited to watermain, storm, and sanitary sewer, gas, hydro, streetlighting, Bell, Rogers, etc.? If you could provide a list of any available information you have on file and the associated fees for obtaining these, it would be much appreciated.

Also, CAD versions of utility plans would be preferable.

Thank you,

JAYMESON ADAMS, EIT Engineering Trainee / Infrastructure Ingénieur en formation / Infrastructures

T 613-860-2462 ext. 6659 F 613-860-1870 110–240 Catherine Street, Ottawa, ON K2P 2G8 CANADA



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Jaymeson Adams

From:	Wessel, Shawn <shawn.wessel@ottawa.ca></shawn.wessel@ottawa.ca>
Sent:	September 1, 2020 10:28 AM
То:	Tim Kennedy
Cc:	Jaymeson Adams
Subject:	RE: 316 - 322 Clifton Road - City Pre-consult
Attachments:	UPDATED_City of Ottawa_TOR 2_0 Additional Works Schedule A ver 20180309_Feb
	2020.pdf; 2020 ECA Expanded Works Doc.docx

Good morning Mr. Kennedy.

Generally, a ToR MECP ECA application took anywhere from 2-6 months, depending on how busy the MECP were at the time.

During Covid19, it appears the MECP are taking longer to review applications and proponents are advised to check in with them, once submitted, on a regular basis to get status updates. The MECP appears to be understaffed and/or funded and are currently swamped with applications, even with the new ToR agreements with municipalities.

For ToR the following must be confirmed:

City can now be considered for approval of private works that will remain private as long as they meet conditions 1 and 2 of the attached schedule A.

If it does meet these conditions and you want to proceed under the ToR then the proposal needs approval from the MECP supervisor.

The test is if these works were considered public would they fall under the ToR. If the only reason that they are going as direct submission is because of private ownership then we should be able to process under ToR

Each proposal needs to have approval from the MECP supervisor.

For the supervisors' approval process please provide the following:

The applicants name and address.

The site name and location.

Who will own the sewer works.

A brief description of the proposed sewage works.

Confirmation that outside of the fact the proposed sewage works will remain privately owned the works do meet all the requirements of items 1) and 2) of schedule A of the City's ToR agreement. A request that the application be processed under the ToR for additional works not listed.

Further to above, please note this will need to be provided to MECP:

The applicants name and address:

- 123456 Ontario Inc. (c.o.b. as West East North South Developments)
- 1234 Somewhere Avenue, Ottawa, Ontario, K1A 4N9

The site name and location:

- Somewhere Phase I
- 1234 Somewhere Avenue, Ottawa, Ontario, K1A 4N9

Who will own the sewer works:

• Common elements condominium will own both the sanitary sewers and the storm sewers.

A brief description of the proposed sewage works:

• Storm sewers and small bore sanitary sewers to connect to the existing sewer networks within Phase 1A of the residential development.

Confirmation that outside of the fact the proposed sewage works will remain privately owned the works do meet all the requirements of items 1) and 2) of schedule A of the City's ToR agreement:

- The sanitary sewers and the storm sewers will remain privately owned.
- The sanitary sewers meet the requirements of Schedule A Item 1) Standard Works Allowed, as follows.
 - New private sanitary sewers
 - i. Designed in accordance the Ministry document Design Guidelines for Sewage Works 2008 (section 5.15.4)
 - ii. Not combined sewers
 - iii. Do not discharge directly to a sewage treatment plant
- The storm sewers meet the requirements of Schedule A Item 1) Standard Works Allowed, as follows.
 - New private storm sewers
 - i. Designed in accordance the Ministry document Design Guidelines for Sewage Works 2008
 - ii. Designed primarily for the collection and transmission of stormwater
 - iii. Discharge to existing storm sewers
 - iv. Not drainage works under the Drainage Act
 - v. Not combined sewers or superpipes and do not connect to a combined sewer
 - vi. Not located on industrial land or designed to service industrial land
 - vii. Do not collect, store or discharge stormwater containing substances or pollutants (other than TSS, or oil and grease) detrimental to the environment or human health. Note that there is an existing Vortech unit and dry pond downstream of the Phase 1A storm sewer system.
 - viii. Do not require the establishment and monitoring of effluent quality criteria.

In addition to MECP application and supplemental information including, but not limited to, proof of identification <u>or</u> certificate of Incorporation, pipe data form, calc. sheets, drainage plans, approved civil plans, Site Servicing and SWM Report, Geotech Report, Correspondence with RVCA, MECP, Ministry of Natural Resources (if applicable) and City of Ottawa and Expanded Works (Draft ECA) which I have attached.

To summarize ToR info above, please complete your package, submit to me to review and once the City is satisfied with the package, we will contact the MECP to ensure ToR is permitted. Please use the Schedules and info above as a test as to whether ToR would be permitted by MECP.

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji Project Manager - Infrastructure Approvals Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale Planning, Infrastructure and Economic Development Department | Direction générale de la planification de l'infrastructure et du développement économique City of Ottawa | Ville d'Ottawa 110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1 (613) 580 2424 Ext. | Poste 33017 Int. Mail Code | Code de Courrier Interne 01-14 shawn.wessel@ottawa.ca

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Please note that I will be away from the office September 10-15 inclusive, returning on the 16th.

Please also note that, while my work hours may be affected by the current situation, I still have access to email, video conferencing and telephone. Feel free to schedule video conferences and/or telephone calls, as necessary.

From: Tim Kennedy <Tim.Kennedy@cima.ca> Sent: September 01, 2020 6:50 AM To: Wessel, Shawn <shawn.wessel@ottawa.ca> Cc: Jaymeson Adams <Jaymeson.Adams@cima.ca> Subject: RE: 316 - 322 Clifton Road - City Pre-consult

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Good morning Shawn,

The RVCA has confirmed that quality control of stormwater will not be required on site given its distance from the outlet. This being the case can you confirm that the ECA process can be completed through the City's Transfer of Review program?

Also what is the expected timeline these days for ECA approval through Transfer of Review?

Thanks,

TIM KENNEDY, P.Eng. Project Manager / Infrastructure T 613 860-2462 ext. 6620 M 613 462-3627 CIMA+

From: Wessel, Shawn <<u>shawn.wessel@ottawa.ca</u>> Sent: Wednesday, August 26, 2020 9:36 AM To: Tim Kennedy <<u>Tim.Kennedy@cima.ca</u>> Cc: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>> Subject: RE: 316 - 322 Clifton Road - City Pre-consult

Good morning Mr. Kennedy

I sent a message to Water Distribution.

They are busy at the moment and will forward to you once we have obtained the conditions.

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji Project Manager - Infrastructure Approvals Gestionnaire de projet – Approbation des demandes d'infrastructures

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From: Tim Kennedy <<u>Tim.Kennedy@cima.ca</u>> Sent: August 26, 2020 9:09 AM To: Wessel, Shawn <<u>shawn.wessel@ottawa.ca</u>> Cc: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>> Subject: RE: 316 - 322 Clifton Road - City Pre-consult

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Hi Shawn,

Just wanted to touch base and see if you had any idea on when the boundary conditions might be available?

Thanks,

TIM KENNEDY, P.Eng. Project Manager / Infrastructure T 613 860-2462 ext. 6620 M 613 462-3627 CIMA+

From: Wessel, Shawn <<u>shawn.wessel@ottawa.ca</u>> Sent: Monday, August 24, 2020 1:41 PM To: Tim Kennedy <<u>Tim.Kennedy@cima.ca</u>> Cc: Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>> Subject: RE: 316 - 322 Clifton Road - City Pre-consult

I apologize, we had provided you with the incorrect criteria for this site.

Please see the following:

The impact to the sanitary is negligible and as for storm they will can to do SWM on site since this appears to be a site plan. The storm system will need to be controlled to the 5-year using the lesser of C=0.5 or existing. Note that the storm pipe in the street is small (300 mm) and this is an uncontrolled system. I would highly discourage any gravity connection of foundations to the storm sewer and would instead ask that they use sump pumps.

We apologize for the inconvenience

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji **Project Manager - Infrastructure Approvals** Gestionnaire de projet – Approbation des demandes d'infrastructures

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From: Wessel, Shawn Sent: August 24, 2020 11:19 AM To: Tim Kennedy <Tim.Kennedy@cima.ca> Cc: Jaymeson Adams < Jaymeson.Adams@cima.ca> Subject: 316 - 322 Clifton Road - City Pre-consult

Good morning Mr. Kennedy.

Message from Water Resources Dept:

Storm should not be an issue since SWM will deal with any excess flows. We would ask that consultant provide us with the proposed sanitary flows and we can enter that in our model to see if there will be any impact. Infrastructure is pre 1970, therefore Control to 2-year C=0.5

Please let me know if you want to discuss this further, or wait for Water Resources Dept. to review your sanitary flows and allow them to provide comments.

I have provided Jaymeson with some details regarding seperating sewer laters for foundation and roof drains, that being two storm and one sanitary and water service for each TH, if using flat roofs. Alternatively, sloped roofs with either a cistern in each unit, or downspouts are also permitted. DS must discharge 1.5 metres from property line as well as foundation walls and not allow for City SWs to become a hazzard in winter time, due to ice build up.

For flat roofs, a roof plan illustrating 5 and 100 year ponding, scuppers and roof drain locations is required as well as a table including, but not limited to, roof drain number, drain type (manufacturer, product name & number), weir opening etc.

In addition to above, the apllicant must have their mechanical and structural engineer complete, stamp date and sign a Roof Flow Control Declaration Form.



If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

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Shawn Wessel, A.Sc.T.,rcji Project Manager - Infrastructure Approvals Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale Planning, Infrastructure and Economic Development Department | Direction générale de la planification de l'infrastructure et du développement économique City of Ottawa | Ville d'Ottawa 110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1 (613) 580 2424 Ext. | Poste 33017 Int. Mail Code | Code de Courrier Interne 01-14 shawn.wessel@Ottawa.ca

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Works allowed to be submitted under the TOR program by the Municipality are described in Sections 1 and 2 below. The works must also meet any requirements in the applicable section. Works that are not described in Section 1 or 2, do not meet any applicable requirements or to which Section 3 applies are not allowed to be submitted under the TOR program.

<u>1. Standard Works Allowed</u>

i) Allowed Sanitary Sewage Works

Unless specified in Section 3 of this Schedule, only ECA applications for the following sanitary sewage works are allowed to be submitted by the Municipality under the TOR Program:

a. New or modified, municipal or private sanitary sewers, forcemains or siphons that:

- i. are designed in accordance with the Ministry document *Design Guidelines for Sewage Works*, 2008 (PIBS 6879) as amended from time to time;
- ii. are not combined sewers; and
- iii. do not discharge directly to a sewage treatment plant.
- b. New or modified, municipal or private sanitary sewage pumping stations that:
 - i. are designed in accordance with the Ministry document *Design Guidelines for Sewage Works*, 2008 (PIBS 6879) as amended from time to time; and
 - ii. do not discharge directly to a sewage treatment plant.

For greater clarity, any sanitary sewage works that provide any treatment of sanitary sewage are not allowed to be submitted under the TOR program.

ii) Allowed Stormwater Works

Unless specified in Section 3 of this Schedule, only ECA applications for the following stormwater works are allowed to be submitted by the Municipality under the TOR Program:

- a. New or modified municipal or private storm sewers, ditches, culverts and grassed swales that:
 - i. are designed in accordance with the Ministry document *Stormwater Management Planning and Design Manual, 2003* (PIBS 4329e) as amended from time to time;
 - ii. are designed primarily for the collection and transmission of stormwater;
 - iii. discharge to existing storm sewers, other existing stormwater conveyance works, an approved stormwater management facility, or a Municipal Drain;
 - iv. for drainage works under the *Drainage Act*, approval of a petition for the modifications must be obtained under the *Drainage Act* prior to submitting an application for an ECA;
 - v. are not combined sewers or superpipes and does not connect to a combined sewer;
 - vi. are not located on industrial land or designed to service industrial land;
 - vii. do not propose to collect, store or discharge stormwater containing substances or pollutants (other than Total Suspended Solids, or oil and grease) detrimental to the environment or human health; and
 - viii. do not require the establishment and monitoring of effluent quality criteria.

- b. New or modified, municipal or private oil/grit separators that:
 - i. are designed in accordance with the Ministry document *Stormwater Management Planning and Design Manual*, 2003 (PIBS 4329e) as amended from time to time;
 - ii. discharge to existing storm sewers, other existing stormwater conveyance, an approved stormwater management facility, or a Municipal Drain;
 - iii. for drainage works under the *Drainage Act*, approval of a petition for the modifications must be obtained under the *Drainage Act* prior to submitting an application for an ECA;
 - iv. are not located on industrial land or designed to service industrial land;
 - v. do not propose to collect, store or discharge stormwater containing substances or pollutants (other than Total Suspended Solids, or oil and grease) detrimental to the environment or human health; and
 - vi. do not require the establishment and monitoring of effluent quality criteria.

2. Additional Works Allowed

The Municipality may submit ECA applications for sanitary and/or stormwater works other than those allowed in Section 1 as described below and in accordance with any listed requirements.

The Municipality's TOR Program is expanded to include:

- a. Combined Sewers
 - the rehabilitation of existing combined sewers where there is no increase in combined sewage overflow (CSO).
- b. Stormwater Management Facilities (wet ponds, wetlands, hybrid ponds, dry ponds)
 - altering, modifying, adding, optimizing or expanding the retention capacity for existing approved stormwater management facilities, including stormwater outfalls, provided that:
 - if the proposed works are required to provide quality control, the works are designed to achieve Enhanced Level water quality control and erosion protection (i.e. 80% TSS removal); and
 - o any attenuation design requirements are satisfied;
 - installing new stormwater management facilities, including stormwater outfalls, provided that:
 - if the proposed works are required to provide quality control, the works are designed to achieve Enhanced Level water quality control and erosion protection (i.e. 80% TSS removal); and
 - any attenuation design requirements are satisfied;
 - stormwater pumping stations.
- c. Lot Level and Conveyance Control (Low Impact Development) Measures
 - altering, modifying, adding, optimizing or expanding the retention capacity for existing approved low impact development (LID) measures, including stormwater outfalls, provided that:

- if the proposed works are required to provide quality control, the LID measures are designed to achieve Enhanced Level water quality control and erosion protection (i.e. 80% TSS removal); and
- any attenuation design requirements are satisfied;
- installing new LID measures, including stormwater outfalls, provided that:
 - if the proposed works are required to provide quality control, the LID measures are designed to achieve Enhanced Level water quality control and erosion protection (i.e. 80% TSS removal);
 - o any attenuation design requirements are satisfied; and
 - the design considers corrective and remediation measures in the event of lack of performance of the LID measures;
- rooftop, surface and underground storage with inlet control devices or orifices.

For Works listed in 2a through 2c the following requirements must be met:

- the Works must be designed in accordance with the Ministry documents *Design Guidelines for Sewage Works*, 2008 (PIBS 6879) and *Stormwater Management Planning and Design Manual*, 2003 (PIBS 4329e), as amended from time to time;
- the Works must receive drainage only from non-industrial lands, where industrial lands are defined by *Ontario Regulation 525/98*;
- any stormwater management pond listed in 2b above shall not be used as a snowmelt facility;
- for Works that are designed to partially infiltrate or exfiltrate into the surrounding soils during high flow conditions:
 - based on the type of works, the vertical separation distance between the highest groundwater table (i.e. spring runoff) and the lowest elevation of the works shall adhere to Table 4.1 of the Ministry document *Stormwater Management Planning and Design Manual*, 2003 (PIBS 4329e); and
 - groundwater must not be utilized as a potable water resource anywhere drainage is captured by the stormwater management works;
- infiltration or exfiltration stormwater works include:
 - pervious pipes and catch-basins;
 - filtering systems, and infiltration trenches, such as, soak away pits attached to pervious catchbasins and sand filter beds;
 - infiltration basins;
 - pervious pipes and catch-basins with infiltration trench systems, rainwater and snow melt into the surrounding soils during high flow conditions; and
 - open channels, ditches, swale drainage systems, bio-swales, tree pits, and infiltration trenches on public roads, or right-of-ways, designed to exfiltrate part or all of the stormwater runoff from the adjacent road into the surrounding soils. These types of works are to include vegetative surfaces;
- for stormwater pumping stations, high level alarm systems, appropriate response time during emergency conditions, and redundancy in pumping arrangement must be provided;

- for the rehabilitation of existing combined sewers, the Works must conform to *Ministry Procedure F-5-5, Determination of Treatment Requirements for Municipal and Private Combined and Partially Separated Sewer Systems*, as amended from time to time;
- for drainage works under the *Drainage Act*, approval of a petition for the modifications must be obtained under the *Drainage Act* prior to submitting an application for an ECA;
- the description of the works for a new or replacement outfall will identify the receiving watercourse if it discharges into any of the provincially recognized critical receivers and/or their tributaries;
- the applicant has consulted with the local Conservation Authority and obtained necessary clearance as required, if the works discharge to a surface water body;
- as part of the Letter of Recommendation, the Municipality has clearly identified all of the works which fall under this Section of Schedule A;
- the Municipality has notified all applicants for works allowed in this Section that the ECA may contain conditions requiring the development of an operation and maintenance program, including a spill contingency plan for the works; the Municipality shall include in their Letter of Recommendation any other conditions related to operation and maintenance of the works if applicable; and
- the Municipality shall maintain a report with detailed records of all the stormwater management works constructed during the year.

The report and records noted above are to include, but not be limited to, the approval number, date of approval, location, description of the stormwater management works, information about what, how, when, why and who operates and maintains the works.

The report must also include a summary of the operation and maintenance program activities, any trouble shooting activities, reports of any flooding conditions and/or any complaints received from the public. The report must also include a statement concerning the potential for these stormwater management systems to impact groundwater quality, which will be based upon the available evidence from inspection and maintenance activities.

The Ministry may require the submission of this report upon request. Further instructions on where and to whom the report is to be submitted will be provided by the Ministry.

In most cases, private works included in this Section will be subject to the requirements under the Environmental Bill of Rights (EBR), which includes mandatory posting of the project proposal on the Environmental Registry for a minimum of forty-five (45) days prior to the issuance of the Environmental Compliance Approval. Ontario Regulation 681/94 under the EBR sets forth the types of ECAs that are classified as Class I or II proposals which require posting on the Environmental Registry. All private wastewater ECAs are subject to posting on the Environmental Registry unless they relate to a discharge point which is already subject to an ECA approval and the proposed ECA would not permit an increase in the discharge of any specific contaminant from the discharge point. In addition, as per section 30 of the EBR, a proposal may be exempt from EBR requirements if the proposal has been considered in a substantially equivalent process of public participation.

3. Works Not Allowed To Be Submitted

Under no circumstances are the following applications for Works identified in either Section 1 or 2 to be submitted under the TOR program:

- a. applications that are identified by the local Ministry District Office as being proposed within the zone of influence of a landfill area;
- b. applications for sanitary sewage works that provide any treatment of sanitary sewage;
- c. applications for Regional Stormwater Control Facilities or Regional Flood Control Facilities consisting of storm water management ponds that are designed to provide quality control or contain floods **greater than** the 100 year flood event;
- d. applications that are for airports or airparks;
- e. applications that are for pumping stations that service combined sewer systems;
- f. applications for projects that have received a Part II Order request, until the request has been decided;
- g. applications for projects that have undertaken an individual Environmental Assessment; and
- h. applications that are likely to trigger the Duty to Consult.

In addition, if the Municipality determines that the works listed in an application have been constructed or are being constructed before an Environmental Compliance Approval has been issued, the Municipality shall:

- i. immediately notify the local Ministry District Office; and
- ii. confirm with the Supervisor, Transfer of Review Program (Supervisor) that the application must be submitted directly to the Ministry for review. Once this confirmation is obtained, the municipality shall return the application and all associated documents and fees to the applicant and advise them that the application will not be reviewed under the TOR program. With written permission from the Supervisor, the municipality may be allowed to proceed with the review of the application.

4. 2020 Program Update: Proposed Consolidated Linear Infrastructure Approach

In view of the Ministry's plan to move to a consolidated permissions approach to linear infrastructure in the near future and subject to the written permission of the Supervisor, the municipality may be allowed in the interim to review additional works currently not listed in this schedule (including private works that may not be covered at the time of the application by an agreement pursuant to the Planning Act under section 1 of this Agreement).

Jaymeson Adams

From:	Jamie Batchelor <jamie.batchelor@rvca.ca></jamie.batchelor@rvca.ca>
Sent:	August 31, 2020 1:38 PM
То:	Tim Kennedy
Cc:	Christian Lavoie-Lebel; Jaymeson Adams
Subject:	RE: 316 - 332 Clifton Road - RVCA Pre-Consultation - SWM Criteria

Good Afternoon Tim,

I also tracked the sewer network and came up with similar results. Based on the distance from the outlet being over 2km, the RVCA would not insist on additional on-site water quality treatment. However, if you are proposing water quality treatment via a mechanical separator, the RVCA would view this favorably, and an improvement for the site.

Jamie Batchelor, MCIP, RPP Planner, ext. 1191 Jamie.batchelor@rvca.ca



3889 Rideau Valley Drive PO Box 599, Manotick ON K4M 1A5 T 613-692-3571 | 1-800-267-3504 F 613-692-0831 | www.rvca.ca

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From: Tim Kennedy <Tim.Kennedy@cima.ca>
Sent: Monday, August 31, 2020 6:47 AM
To: Jamie Batchelor <jamie.batchelor@rvca.ca>
Cc: Christian Lavoie-Lebel <Christian.Lavoie-Lebel@cima.ca>; Jaymeson Adams <Jaymeson.Adams@cima.ca>
Subject: RE: 316 - 332 Clifton Road - RVCA Pre-Consultation - SWM Criteria

Good morning Jamie,

Just wanted to follow up on my email below.

Thanks,

TIM KENNEDY, P.Eng. Project Manager / Infrastructure T 613 860-2462 ext. 6620 M 613 462-3627 CIMA+

From: Tim Kennedy Sent: Friday, August 21, 2020 6:47 AM To: Jamie Batchelor <<u>jamie.batchelor@rvca.ca</u>>
 Cc: Christian Lavoie-Lebel <<u>Christian.Lavoie-Lebel@cima.ca</u>>; Jaymeson Adams <<u>Jaymeson.Adams@cima.ca</u>>
 Subject: 316 - 332 Clifton Road - RVCA Pre-Consultation - SWM Criteria

Good morning Jamie,

We are working on another file in the City of Ottawa and I wanted to get your input on Natural Heritage/Hazards features that may impact the development as well as any Storm Water Management Criteria for the site and required approvals/permits.

The proposed development involves the construction of six (6) townhouse blocks comprising thirty-one (31) residential units with private garages and a private access road through the site. It is anticipated that storm water will be stored in underground tanks and a mechanical separator will be used to achieve quality control requirements.

I have attached a key plan with the site location as well as the ultimate flow path for the storm sewer.

If you need anything further please let me know. Feel free to call me on my cell if you would like to discuss.

Best Regards,

TIM KENNEDY, P.Eng. Project Manager / Infrastructure

T 613-860-2462 ext. 6620 **M** 613-462-3627 **F** 613-860-1870 110–240 Catherine Street, Ottawa, ON K2P 2G8 CANADA

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Appendix B Existing Conditions Plan







<u>Insight Properties INC.</u> ("The Client"), their solicitors, mortgagees, and other related parties, permission to use original, signed, sealed copies of the Surveyor's Real Property Report in transactions involving The Client. — B — B — B ----- W ¢450mm Storm Sew ----- ST ------ ST ----26° 29' (Reference Bearing) Closest OHV Elev=75.0 Closest OHW Grass Concrete Grass 74.18 Meas. (74.14 Pl) Grass × 53 Wooden Porch & Steps Concrete Steps Door Sill J Elev=65.66 Door Sill Elev=65.35 No. 328 Bungalow Stucco Sided (Foundation Noted) No. 324 No. 320 2 Storey Vinyl Sided Bungalow Stucco Sided No. 332 2 Storey Brick Sided (Foundation Noted) (Foundation Noted) (Foundation Noted) LOT 19 Wooden Steps LOT -18 LOT 22 04021 - 0046 -Door Sill Elev=65.72 - PIN Door Sill Elev=65.27 Då Wooden Deck 20 LOT 21 64.60 Sill 55.35 Vinyl Addition Wooden Deck & Steps Wooden Deck & Steps inyl Additio PLAN 0.54 64.40 Wood Sided Garage - PIN 04021 0045 --P'N04021 0047-3x30ø Wood Sided Garage 2x0.2¢ 04021 0044 PIN WPF 0.28 South 6^{4.11} CA.23 64.14 O PO-W 0.5ø 164.65 (P6)&Meas. 77.50 (PI)&Meas. BF (North Face Noted) FRONT) (OTTAWA PART BF _/ 0.09 West _____ CONCESSION 04021 PIN _____

PLAN

UTILITY NOTES

ANNIS, O'SULLIVAN, VOLLEBEKK LTD. grants to

- be the responsibility of the user to contact the respective utility authorities for confirmation.
- 2. Only visible surface utilities were located. 3. Underground utility data derived from City of Ottawa utility sheet reference E-05-02 & E-05-06
- 4. Sanitary and storm sewer grades and inverts were derived from City of Ottawa
- 5. A field location of underground plant by the pertinent utility authority is
- mandatory before any work involving breaking ground, probing, excavating etc.

ELEVATION NOTES

1. Elevations shown are geodetic and are referred to the CGVD28 geodetic datum. 2. It is the responsibility of the user of this information to verify that the job benchmark has not been altered or disturbed and that it's relative elevation and description agrees with the information shown on this drawing.



elevation of some topographical data that is otherwise visible.

Land Surveyors Job No. 18018-19 Lt 18 To 23 PL 369 T F



Appendix C

Conceptual Site Servicing and Stormwater Management Plan







	ςίτε θατα	
	SITE DATA <u>SITE STATISTICS</u> (NUMBER OF UNITS & GROSS BUILDING AREA)	
	BLOCK 1 4 UNITS 6,300 SQ.FT (585m ²) BLOCK 2 6 UNITS 9,450 SQ.FT. (878m ²) BLOCK 3 4 UNITS 5.802 SQ.FT (547m ²)	
G	BLOCK 5 4 UNITS 5,892 SQ.FT. $(547m^2)$ BLOCK 5 4 UNITS 5,892 SQ.FT. $(547m^2)$ BLOCK 5 4 UNITS 5,892 SQ.FT. $(547m^2)$ BLOCK 6 7 UNITS 11.025 SO.FT. $(1.024m^2)$	
n	TOTAL 29 UNITS 44,451 SQ.FT. (4,130M ²)	
в в		
	LOT COVERAGE TOTAL LOT AREA: 2.949m ²	
MH-S T\G=64.59	TOTAL GROSS BUILDING AREA: 1.358m ² TOTAL LOT COVERAGE 46%	
	TOTAL LANDSCAPE AREA: XXXm ² TOTAL LOT COVERAGE XX%	
W W -	TOTAL LOT COVERAGE XX%	
CB CB		
" Γ\G ┿── st ── st − 62.9 :63.0		
P P F	ANNIS O SOLLIVAN, VOLLEBERK LID. 14 CONCOURSE GATE, SUITE 500 OTTAWA, ON K2E 7S6 ANNIS O SOLLIVAN, VOLLEBERK LID. SURVEYOR'S REAL PROPERTY REPORT PART 1 PLAN OF LOTS 18,19, 20, 21, 22 & 23	
онw — (CONTACT: TEL. REGISTERED PLAN 369 CITY OF OTTAWA	
ω		
Sewer		
25mm (PLANNED UNIT DEVELOPMENT ZONING MECHANISM	
s \$2	ZUNING: DWELLING TYPE: PUD – 31 UNITS REQUIRED PROPOSED	
σ	MIN. WIDTH OF PRIVATE DRIVEWAY XXm 4.5m	
- ω	MIN. SEPARATION AREA BETWEEN BUILDINGS 1.2m MIN. 2.4m MIN.	
()	SETBACKS	
ν Ι		
	MIN. LOT WIDTH MIN. LOT AREA MAX. BUILDING HEIGHT	
\leq	PARKING REQUIREMENTS - RESIDENTS	
	(PARKING PROVISIONS 2008-250 SECTION 106)	
	RESIDENTS REQUIREPROVIDED10RESIDENT SPACES31[31. (31-12)x0.5]RESIDENT SPACES	no. date revision
\sim	XX_ VISITORS XX_DRIVEWAYS @ 3mx5.2m	It is the responsibility of the appropriate contractor to check and verify all dimen-
<	<u>z</u> visitoks [0.1x(31-12)]	or omissions to the architect.
P	12 SPACES 31 TOTAL	pertinent codes and by-laws. Do not scale drawings.
	CONSULTANTS	This drawing may not be used for construction until signed.
	ARCHITECT HOBIN ARCHITECTURE INC.	Copyright reserved.
	63 PAMILLA STREET, OTTAWA, ON K1S 3K7	
	EUNTACT: BILL RITCEY TEL. 613–236–7200 FAX. 613–265–2005	
20		Hobin Architecture
		Incorporated 63 Pamilla Street
		Ottawa, Ontario Canada K1S 3K7
2	APPROVAL SIGNATURE	T: 613-238-7200 F: 613-235-2005
	APPROVED REFUSED	E: mail@nobinarc.com HOBIN hobinarc.com ARCHITECTURE
		project title
	REVIEW, SUBURBAN SERVICES	CLIFTON TOWNS
		clifton road, ottawa, ontario
	LOCATION MAP N.T.S.	
	site	
	NORTH WESTBORD	HL AUG/20 1:150
	BEACH SCOTT Stanta Bakery Westbore	project 2030
	Any Net Official Superstore Co	drawing no.
	And	SP-1
	LAURENTIAN VIEW	
]		revision no.



Appendix D Water Supply Design Calculations







CIMA+ PROJECT NUMBER: A001062 CLIENT: Clifton Pro PROJECT STATUS: Prelimina

316-332 Clifton Road Property Development A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

WATER CONSUMPTION CALCULATIONS

APPLICABLE DESIGN GUIDELINES:

1. Ottawa Design Guidelines - Water Distribution (2010)

2. City of Ottawa Technical Bulletin ISTB-2018-02, ISDTB-2014-02 and ISD-2010-02

3. MOE Design Guidelines for Drinking-Water Systems

RESIDENTIAL AND COMMERCIAL WATER DEMANDS: RESIDENTIAL DESIGN CRITERIA:

Residential Average Day Demand:	350	L/c/day
Maximum Day Peaking Factor:	7.6	x Average Daily Demand
Maximum (Peak Hour) Peaking Factor:	11.5	x Average Daily Demand

EQUIVALENT POPULATION :

Unit Type	Number of Units	Persons Per Unit	Population
Block 1 Townhomes	4	2.7	11
Block 2 Townhomes	6	2.7	16
Block 3 Townhomes	4	2.7	11
Block 4 Townhomes	4	2.7	11
Block 5 Townhomes	4	2.7	11
Block 6 Townhomes	7	2.7	19
Total	29		79

Per Unit Populations:

Table 4.1 Per Unit Populations Unit Type Persons Per Unit Single Family 3.4 2.7 Semi-detached Duplex 2.3 Townhouse (row) 2.7 **Apartments:** Bachelor 1.4 1 Bedroom 1.4 2 Bedroom 2.1 3 Bedroom 3.1 Average Apt. 1.8

WATER DEMANDS:

Demand Type	Average Daily Demand (L/s)	Maximum Daily Demand (L/s)	Maximum (Peak) Hour Demand (L/s)
Residential	0.32	2.43	3.68
Total	0.32	2.43	3.68

NOTES:

1. Maximum Day and Maximum Hour residential peaking factors determined using Table 3-3 of the MOE Design Guidelines for Drinking-Water System for 0 to 500 persons.

2. Given basic day demand less than 50 m3/day (0.57 L/s), only one connection is required. However, the need for a second connection to the watermain on Clifton will be assessed at the detailed design stage in order to provide redundant supply, reduce hydraulic losses and avoid stagnant water associated with a dead end water main.

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-24

Verified by: Tim Kennedy, P.Eng. PEO# 100173201 Date: 2020-08-24



CIMA+ PROJECT NUMBER: A001062 CLIENT: Clifton Pro PROJECT STATUS: Prelimina

316-332 Clifton RoadProperty DevelopmentA001062Clifton Property Development Inc.Preliminary Design (Assessment of Adequacy of Public Services)

FIRE FLOW ASSESSMEN

APPLICABLE DESIGN GUIDELINES:

- 1. Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection, 1999
- 2. Ottawa Design Guidelines Water Distribution (2010) including Appendix H per ISTB-2018-02
- 3. City of Ottawa Technical Bulletins ISTB-2018-02 and ISDTB-2014-02
- 4. MOE Design Guidelines for Drinking-Water Systems

STEP A - DETERMINE THE TYPE OF CONSTRUCTION

Type of Construction	Coefficient (C)	Value Selected (C)
Fire-resistive Construction (> 3 hours)	0.6	
Non-combustible Construction	0.8	15
Ordinary Construction	1	1.5
Wood Frame Construction	1.5	

STEP B - DETERMINE THE FLOOR AREA

Block	Gross Floor Area (m ²)
Block 3	547
Block 4	547
Block 5	547
TOTAL FLOOR AREA (A):	1,641

STEP C - DETERMINE THE HEIGHT IN STOREYS

Floor/Level Per Block	Number of Storeys
Ground Level:	1
Level 2:	1
Level 3:	1
HEIGHT IN STOREYS:	3

STEP D - DETERMINE BASE FIRE FLOW (ROUND TO NEAREST 1,000 L/min)

$$F = 220C\sqrt{A}$$

Where:

F is the required fire flow in L/min

C is the coefficient related to the type of construction, and;

A is the total floor area of the building in m²

Coefficient Related to Type of Construction (C) =	1.5
Floor Area Considered (A) =	1,641 m ²

REQUIRED (BASE) FIRE FLOW (F) =

13000 L/min (Rounded to Nearest 1,000 L/min)



CIMA+ PROJECT NUMBER: A001062 CLIENT: Clifton Pro PROJECT STATUS: Prelimina

316-332 Clifton Road
Property Development
A001062
Clifton Property Development Inc.
Preliminary Design (Assessment of Adequacy of Public Services)

FIRE FLOW ASSESSMENT

STEP E - DETERMINE THE INCREASE OR DECREASE FOR OCCUPANCY AND APPLY TO STEP D (STEP D x STEP E, DO NOT ROUND)

Occupancy Class	Occupancy Factor	Value Selected (C)
Non-combustible	0.75	
Limited combustible	0.85	
Combustible	1.00	0.85
Free burning	1.15	
Rapid burning	1.25	

REQUIRED (BASE) FIRE FLOW (F) = 11050 L/min (Not rounded)

STEP F - DETERMINE THE DECREASE, IF ANY, FOR AUTOMATIC SPRINKLER PROTECTION AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

Sprinkler System Design	Sprinkler Design Charge	Value Selected (C)	Total Charge
Automatic sprinkler system conforming to NFPA standards	-30%	No	0%
Standard water supply	-10%	No	0%
Fully supervised system	-10%	No	0%
TOTAL CHARGE FOR SPRINKLER SYSTEM			0%

DECREASE FOR SPRINKLER PROTECTION = 0 L/min (Not rounded)

STEP G - DETERMINE THE TOTAL INCREASE FOR EXPOSURES AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

Façade	Separation Distance (m)	Length-height Factor of Exposed Wall (m-storeys)	Assumed Construction of Exposed Wall of Adiacent	Total Charge
North Façade	9.1	18	Wood Frame	17%
East Façade	10.1	210	Wood Frame	15%
South Façade	8.5	48	Wood Frame	18%
West Façade	9.5	472	Fire Resistive or Ordinary with Unprotected Openings	20%
TOTAL CHARGE FOR EXPOSURES				70%

INCREASE FOR EXPOSURES =

9100 L/min (Not rounded)

STEP H - DETERMINE FIRE FLOW INCLUDING ALL INCREASES AND REDUCTIONS ((STEP E + STEP F + STEP G, ROUND TO NEAREST 1,000 L/min)

TOTAL REQUIRED FIRE FLOW (RFF) =	20000 L/min (Rounded to Nearest 1,000 L/min)
	333 L/s
	5283 USGPM

MODIFICATIONS FOR APPLICATION IN THE CITY OF OTTAWA (per City of Ottawa Technical Bulletin ISTB-2018-02 and ISDTB-2014-02)

TOTAL REQUIRED FIRE FLOW (RFF) =	10000 L/min
	167 L/s
	2642 USGPM



CLIENT:

316-332 Clifton Road **Property Development** CIMA+ PROJECT NUMBER: A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

NOTES/COMMENTS:

STEP A - DETERMINE THE TYPE OF CONSTRUCTION

1. The buildings do not qualify as ordinary construction as less than 2/3 of exterior walls are constructed from stone/masonry.

PROJECT STATUS:

STEP B - DETERMINE THE FLOOR AREA

1.Blocks 3, 4 and 5 to the west of the proposed development are separated by less than 3 m, thus the fire area for the FUS calculation is the sum of the floor area of all three buildings.

STEP C - DETERMINE THE HEIGHT IN STOREYS

1. Garage/parking level located at least 50% below grade and thus not considered.

STEP D - DETERMINE BASE FIRE FLOW (ROUND TO NEAREST 1,000 L/min)

1. No notes or comments.

STEP E - DETERMINE THE INCREASE OR DECREASE FOR OCCUPANCY AND APPLY TO STEP D (STEP D x STEP E, DO NOT ROUND)

1. Residential buildings have a limited combustible occupancy.

STEP F - DETERMINE THE DECREASE, IF ANY, FOR AUTOMATIC SPRINKLER PROTECTION AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND) 1. No sprinklers.

STEP G - DETERMINE THE TOTAL INCREASE FOR EXPOSURES AND APPLY TO VALUE IN STEP D ABOVE (DO NOT ROUND)

1. The length height factor of the exposed walls to the east and west includes Blocks 3, 4 and 5 as they are separated by less than 3 m. The total exposure charge is below the maximum value of 75% ...

STEP H - DETERMINE FIRE FLOW INCLUDING ALL INCREASES AND REDUCTIONS ((STEP E + STEP F + STEP G, ROUND TO NEAREST 1,000 L/min) 1. No notes or comments.

MODIFICATIONS FOR APPLICATION IN THE CITY_OF OTTAWA (per City of Ottawa Technical Bulletin ISTB-2018-02 and ISDTB-2014-02)

1. The City of Ottawa cap of 10,000 L/min applies given town house blocks contain no more than 7 units, the total footprint of the fire area is less than 600 m² and town homes are not back to back.

> Prepared by: Jaymeson Adams, EIT

Date: 2020-08-24

Verified by: Tim Kennedy, P.Eng. PEO# 100173201

Date: 2020-08-25



T: ST3-ST0-ST0 THE SHE Catherine Street, Change, Cit. K2P SEG CANADIA VIN BY: Designed BY: Deproved BY:

PROPOSED WATER SERVICE CONNECTION LOCATION(S)

DRAWN BY:	DESIGNED BY:	APPROVED BY:	SCALE:	DATE:	PROJECT No:	SHEET No:	FIGURE No:
J. Adams		T. Kennedy	NTS	2020/08/25	A001062	1 of 1	1





EXPOSURE SEPARATION DISTANCES

DRAWN BY:	DESIGNED BY:	APPROVED BY:	SCALE:	DATE:	PROJECT No:	SHEET No:	FIGURE No:
J. Adams		T. Kennedy	NTS	2020/08/24	A001062	1 of 1	2

PROPOSED HYDRANT CLASS AA DISTANCE TO BUILDING ≤ 75 m CONTRIBUTING FLOW 5,700 L/min

> EXISTING HYDRANT CLASS AA DISTANCE TO BUILDING > 75 m AND \leq 150 m CONTRIBUTING FLOW 3,800 L/min

WILBER AVE

FIRE HYDRANT COVERAGE

DRAWN BY:	DESIGNED BY:	APPROVED BY:	SCALE:	DATE:	PROJECT No:	SHEET No:	FIGURE No:
J.Adams		T. Kennedy	NTS	2020/08/25	A001062	1 of 1	3

EXISTING HYDRANT CLASS AA DISTANCE TO BUILDING > 75 m AND ≤ 150 m CONTRIBUTING FLOW 3,800 L/min

IFTON RD

SCOTTST



CIMA+ PROJECT NUMBER: CLIENT: **PROJECT STATUS:**

316-332 Clifton Road **Property Development** A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

HYDRAULIC ANALYSIS - WATER

APPLICABLE DESIGN GUIDELINES:

- 1. Ottawa Design Guidelines Water Distribution (2010)
- 2. City of Ottawa Technical Bulletin ISDTB-2014-02 and ISD-2010-02
- 3. MOE Design Guidelines for Drinking-Water Systems

MUNICIPAL BOUNDARY CONDITIONS

HYDRAULIC BOUNDARY CONDITIONS (PROVIDED BY THE CITY OF OTTAWA):

Hydraulic Condition (HGL = Hydraulic Grade Line)	Boundary Condition (Head) (m) Clifton Rd. 152 mm dia.
Minimum HGL	108.5
Maximum HGL	114.7
Maximum Day + Fire Flow	N/A

AVAILABLE HYDRANT FLOWS - MULTI-HYDRANT ANALYSIS (PROVIDED BY THE CITY OF OTTAWA):

Hydrant ID	Available Flow (L/s)
North Hydrant	70
South Hydrant	85
Total	155

HYDRAULIC ANALYSIS - WATER SUPPLY ADEQUACY (FLOW AND PRESSURE) **DESIGN CRITERIA - WATERMAIN PRESSURE AND DEMAND OBJECTIVES:**

Demand Type	Minimum Pressure (psi)	Desired Minimum Pressure (psi)	Desired Maximum Pressure (psi)	Maximum Pressure (psi)
Average Daily Demand	40.0	50.0	70.0	80.0
Maximum Daily Demand + Fire Flow	20.0			
Maximum (Peak) Hour Demand	40.0			

WATERMAIN PRESSURE AND DEMAND ANALYSIS SUMMARY - PROPOSED DEVELOPMENT:

		Available Fl	ow/Pressure		
Demand Type	Proposed Demand (L/s)	Design Operating Pressure ¹ (Relative Head) (m)	Design Operating Pressure (psi)	Flow/Pressure Objective Achieved?	
Average Daily Demand	0.32	50.5	72	YES	
Maximum Daily Demand + Fire Flow	169.10	155 L/s (@ 20 psi	NO	*See note
Maximum (Peak) Hour Demand	3.68	44.3	63	YES	-

1. Assumed ground elevation for northern connection (m) = 64.2

NOTES:

1. Based on the boundary conditions provided by the City and additional private hydrant will be required on site to provide adequate fire flow.

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-28

Verified by: Tim Kennedy, P.Eng.

1.

PEO# 100173201

Date: 2020-08-31

Z:Cima-C10/Ott Projects/A/A001000-A001499/A001062 Clifton Property Development/300/360 Civil/200811 Adequacy of Servicing Calculations/03 WM/200831 Water Demands and Analysis.xlsx/Hvdraulic Analysis

Jaymeson Adams

From:	Wessel, Shawn <shawn.wessel@ottawa.ca></shawn.wessel@ottawa.ca>
Sent:	August 27, 2020 10:36 AM
То:	Tim Kennedy; Deiaco, Simon
Cc:	Christian Lavoie-Lebel; Jaymeson Adams
Subject:	RE: 316 - 332 Clifton Road - City Servicing Requirements and Design Criteria

Good morning Mr. Kennedy.

Please find boundary conditions as requested:

****The following information may be passed on to the consultant, but do NOT forward this e-mail directly.****

The following are boundary conditions, HGL, for hydraulic analysis at 316-332 Clifton Road (zone 1W) assumed to be both connected to the 152mm on Clifton Road (see attached PDF for location).

Minimum HGL = 108.5m

Maximum HGL = 114.7m

Connection 1: Available Fire Flow @ 20 psi = 85 L/s

Connection 2: Available Fire Flow @ 20 psi = 70 L/s

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji Project Manager - Infrastructure Approvals Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale Planning, Infrastructure and Economic Development Department | Direction générale de la planification de l'infrastructure et du développement économique City of Ottawa | Ville d'Ottawa 110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1 (613) 580 2424 Ext. | Poste 33017 Int. Mail Code | Code de Courrier Interne 01-14 <u>shawn.wessel@ottawa.ca</u>

Please consider the environment before printing this email

Please note that, while my work hours may be affected by the current situation, I still have access to email, video conferencing and telephone. Feel free to schedule video conferences and/or telephone calls, as necessary.

From: Tim Kennedy <Tim.Kennedy@cima.ca>
Sent: August 20, 2020 8:38 AM
To: Wessel, Shawn <shawn.wessel@ottawa.ca>; Deiaco, Simon <Simon.Deiaco@ottawa.ca>
Cc: Christian Lavoie-Lebel <Christian.Lavoie-Lebel@cima.ca>; Jaymeson Adams <Jaymeson.Adams@cima.ca>
Subject: 316 - 332 Clifton Road - City Servicing Requirements and Design Criteria

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Hi Shawn and Simon,

I was given your contact information by a team member for this file who noted you would be the engineer and planner on file respectively. CIMA+ will be leading the civil design and I am hoping you have some availability this week to discuss the servicing requirements and design criteria for the above mentioned development. Would sometime on Friday between 9:30 and 1:30 work?

We have completed some preliminary analysis in regards to (1) Sanitary Demands and (2) Water/Fire Demands as follows:

- 1) <u>SANITARY DEMANDS</u>: Please find the proposed development information below and detailed calculations attached (I have also attached the Architectural Concept Plans for reference):
 - A. <u>Type of Development and Units</u>: The proposed development involves the construction of six (6) townhouse blocks comprising thirty-one (31) residential units with private garages.
 - B. Site Address: 316–322 Clifton Road.
 - C. <u>Location of Services:</u> connection to existing 225 mm diameter concrete sanitary sewer on Clifton Road with new maintenance hole on sewer main anticipated.
 - D. Total Estimated Average Dry Weather Flow Rate: 0.27 L/s
 - E. Total Estimated Peak Dry Weather Flow Rate: 0.98 L/s
 - F. Total Estimated Peak Wet Weather Flow Rate: 1.08 L/s

Could you please confirm if there is enough capacity in the City system to accommodate the proposed wastewater flow.

<u>WATER AND FIRE FLOW DEMANDS</u>: Please find the proposed development information below and detailed calculations and associated figures attached, including: (1) Water Demand Calculations, (2) Fire Flow Calculations, (3) Figure 1 – Proposed Water Service Connection Locations, (4) Figure 2 – Exposure Separation Distances, (5) Figure 3 – Fire Hydrant Coverage.

- 1. <u>Type of Development and Units:</u> The proposed development involves the construction of six (6) townhouse blocks comprising thirty-one (31) residential units with private garages.
- 2. Site Address: 316–322 Clifton Road.
- 3. **Location of Services:** Please see attached Figure 1.
- 4. <u>Plan showing Proposed Water Connections:</u> Please see attached Figure 1.
- 5. Average Daily Demand: 0.34 L/s
- 6. Maximum Daily Demands: 2.52 L/s
- 7. Peak Hour Demand: 3.81 L/s
- 8. Required Fire Flow (RFF): 10,000 L/min

Could you please provide the boundary conditions for the proposed development.

If you have any questions or require anything further, please do not hesitate to contact me on my cell (working from home these days).

Looking forward to collaborating on this one.

Regards,

ı

TIM KENNEDY, P.Eng. Project Manager / Infrastructure

T 613-860-2462 ext. 6620 M 613-462-3627 F 613-860-1870 110-240 Catherine Street, Ottawa, ON K2P 2G8 CANADA

Notice to our customers on the COVID-19





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Appendix E Sanitary Servicing Design Calculations







CIMA+ PROJECT NUMBER: A001062 CLIENT: Clifton Pro PROJECT STATUS: Preliminal

316-332 Clifton Road Property Development A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

WASTEWATER PEAK FLOW DETERMINATION

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

2. City of Ottawa Technical Bulletin ISTB-2018-01

DOMESTIC CONTRIBUTIONS: RESIDENTIAL DESIGN CRITERIA:

Residential Average Flow: (1) Residential Peak Factor (P.F.): 280 L/c/dayHarmon Equation (Min 2.0 and Max 4.0) $P.F.=1 + \left(14 \\ 14 \\ K \right) * K$



Per Unit Populations:

Unit Type	Persons Per Unit
Single Family	3.4
Semi-detached	2.7
Duplex	2.3
Townhouse (row)	2.7
Apartments:	
Bachelor	1.4
1 Bedroom	1.4
2 Bedroom	2.1
3 Bedroom	3.1
Average Apt.	1.8

AVERAGE FLOW - DOMESTIC:

Unit Type	Number of Units	Persons Per Unit	Population	Average Flow (L/s)
Block 1 Townhomes	4	2.7	11	0.04
Block 2 Townhomes	6	2.7	16	0.05
Block 3 Townhomes	4	2.7	11	0.04
Block 4 Townhomes	4	2.7	11	0.04
Block 5 Townhomes	4	2.7	11	0.04
Block 6 Townhomes	7	2.7	19	0.06
Total	29		79	0.26

PEAK FLOW - DOMESTIC:

Population: (2)	79	persons
Average Dry Weather Flow: $(3) = (1) \times (2)$	0.26	L/s
Peaking Factor (P.F.): (4)	3.62	
Peak Domestic Flow: (5) = (3) x (4)	0.93	L/s

EXTRANEOUS FLOW CONTRIBUTION - INFLOW AND INFILTRATION:

EXTRANEOUS DESIGN CRITERIA:		
Dry Weather Infiltration:	0.05	L/s/effective gross ha (for all areas)
Wet Weather Infiltration:	0.28	L/s/effective gross ha (for all areas)
PEAK FLOW - EXTRANEOUS:		
Effective Gross Area: (11)	0.2950	ha
Total Infiltration Allowance: (12)	0.33	L/s/effective gross ha (for all areas)
Peak Extraneous Flow: (13) = (11) x (12)	0.10	L/s


CIMA+ PROJECT NUMBER: A001062 CLIENT: Clifton Pro PROJECT STATUS: Preliminar

316-332 Clifton Road
Property Development
A001062
Clifton Property Development Inc.
Preliminary Design (Assessment of Adequacy of Public Services)

WASTEWATER PEAK FLOW DETERMINATION

Total Estimated Avg. Dry Weather Flow Rate:	0.26	L/s
Total Estimated Peak Dry Weather Flow Rate:	0.93	L/s
Total Estimated Peak Wet Weather Flow Rate:	1.02	L/s
Total Estimated Peak Wet Weather Flow Rate:	1.02	L/s

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-24

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



Appendix F Storm Servicing and Stormwater Management Calculations







CIMA+ PROJECT NUMBER: CLIENT: PROJECT STATUS: 316-332 Clifton RoadProperty DevelopmentA001062Clifton Property Development Inc.Preliminary Design (Assessment of Adequacy of Public Services)

STORM RUNOFF COEFFICIENT DETERMINATION (PRE-DEVELOPMENT)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

PRE-DEVELOPMENT RUNOFF COEFFICIENT DETERMINATION:

Area	Pervious Area	Pervious Area Runoff Coefficient	Impervious Area m²	Impervious Area Runoff Coefficient	Total Area m²	Weighted Runoff Coefficient (5-year)	Weighted Runoff Coefficient (100-year)
Attenuated Area	1893	0.20	1057	0.90	2950	0.45	0.56
Unattenuated Area - Clifton (NC1)	192	0.20	70	0.90	262	0.39	0.48
Unattenuated Area - Wilber (NC2)	166	0.20	2	0.90	168	0.21	0.26
TOTAL	2251	0.20	1129	0.90	3380	0.43	0.54

NOTES:

For 25 year storms add 10% to C value

For 50 year storms add 20% to C value

For 100 year storms add 25% to C value

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-25

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



CIMA+ PROJECT NUMBER: CLIENT: PROJECT STATUS: 316-332 Clifton RoadProperty DevelopmentA001062Clifton Property Development Inc.Preliminary Design (Assessment of Adequacy of Public Services)

STORM PRE-DEVELOPMENT FLOW

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

PRE-DEVELOPMENT FLOW DETERMINATION: DESIGN CRITERIA:

Design Storm (year):	5	
IDF Regression Constants: (a)	998.071	
(b)	6.053	
(c)	0.814	
IDF Curve Equation (mm/hr):	l = a / (Time	e in min + b) ^c
		where: $Q = Flow (L/s)$
		C = Runoff Coefficient
Rational Formula (L/s):	Q = 2.78C*I*A	I = Rainfall Intensity (mm/hr)
		A = Area (hectares)

ALLOWABLE RELEASE RATE - SUMMARY:

Catchment ID	Area	Runoff Coefficient	Time of Concentration	Intensity	Allowable Release Rate	Release Flow Per Unit Area
	(A) ha	(C)	(tc) min	(l) mm/hr	(Q) L/s	(Q/ha) L/s/ha
A1	0.295	0.45	10	104.19	38.4	130.2
Total	0.295				38.4	130.2

NOTES:

1. Calculated Time of Concentration (tc) using Bransby Williams (C > 0.4) is 7 min. Minimum Tc of 10 min used per City Standard.

2. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)

Prepared by: Jaymeson Adams, EIT

Date: 2020-09-03

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



CIMA+ PROJECT NUMBER: CLIENT: PROJECT STATUS: 316-332 Clifton Road Property Development A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

STORM PRE-DEVELOPMENT FLOW - EXISTING SITE FLOWS

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

PRE-DEVELOPMENT FLOW DETERMINATION - 100-YEAR EVENT:

DESIGN CRITERIA:	
------------------	--

Design Storm (year):	100	
IDF Regression Constants: (a)	1735.688	1
(b)	6.014	
(c)	0.820	
IDF Curve Equation (mm/hr):	I = a / (Time	e in min + b) ^c
		where: $Q = Flow (L/s)$
		C = Runoff Coefficient
Rational Formula (L/s):	Q = 2.78C*I*A	I = Rainfall Intensity (mm/hr)
		A = Area (hectares)

ALLOWABLE RELEASE RATE - SUMMARY:

Catchment ID	Area	Runoff Coefficient	Time of Concentration	Intensity	Release Rate	Release Flow Per Unit Area
	(A) ha	(C)	(tc) min	(I) mm/hr	(Q) L/s	(Q/ha) _{L/s/ha}
A1	0.295	0.56	10	178.56	81.9	277.8
Total	0.295				81.9	277.8

NOTES:

1. Calculated Time of Concentration (tc) using Bransby Williams (C > 0.4) is 7 min. Minimum Tc of 10 min used per City Standard.

2. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)

Prepared by: Jaymeson Adams, EIT

Date: 2020-09-03

Verified by: Tim Kennedy, P.Eng. PEO# 100173201







CIMA+ PROJECT NUMBER: CLIENT: PROJECT STATUS: 316-332 Clifton RoadProperty DevelopmentA001062Clifton Property Development Inc.Preliminary Design (Assessment of Adequacy of Public Services)

STORM RUNOFF COEFFICIENT DETERMINATION (POST-DEVELOPMENT)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

POST-DEVELOPMENT RUNOFF COEFFICIENT DETERMINATION:

Area	Pervious Area m²	Pervious Area Runoff Coefficient	Impervious Area ^{m²}	Impervious Area Runoff Coefficient	Total Area m²	Weighted Runoff Coefficient (5-year)	Weighted Runoff Coefficient (100-year)
Block 1 (A1)	0	0.20	184	0.90	184	0.90	0.95
Block 2 (A2)	0	0.20	276	0.90	276	0.90	0.95
Block 3 (A3)	0	0.20	190	0.90	190	0.90	0.95
Block 4 (A4)	0	0.20	190	0.90	190	0.90	0.95
Block 5 (A5)	0	0.20	190	0.90	190	0.90	0.95
Block 6 (A6)	0	0.20	321	0.90	321	0.90	0.95
Remainder of Site (A7)	371	0.20	922	0.90	1293	0.70	0.87
Unattenuated Area - Clifton (NC1)	271	0.20	214	0.90	485	0.51	0.64
Unattenuated Area - Wilber (NC2)	165	0.20	86	0.90	251	0.44	0.55
TOTAL	807	0.20	2573	0.90	3380	0.73	0.92

NOTES:

For 25 year storms add 10% to C value

For 50 year storms add 20% to C value

For 100 year storms add 25% to C value

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-28

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



CIMA+ PROJECT NUMBER: CLIENT: PROJECT STATUS: 316-332 Clifton Road Property Development A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

STORM PRE- VS POST DEVELOPMENT FLOW - UNATTENUATED AREAS (100-YEAR)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

PRE- vs POST-DEVELOPMENT FLOW DETERMINATION FOR UNATTENUATED AREAS: DESIGN CRITERIA:

Design Storm (year):	100	
IDF Regression Constants: (a) (b) (c)	1735.688 6.014 0.820	
IDF Curve Equation (mm/hr):	I = a / (Time	∋ in min + b) ^c
Rational Formula (L/s):	Q = 2.78C*I*A	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area (hectares)

RELEASE RATE SUMMARY - UNATTENUATED AREAS - PRE- vs POST-DEVELOPMENT (100-year):

Design Event	Area (A) _{ha}	Runoff Coefficient (C)	Time of Concentration (tc) ^{min}	Intensity (I) ^{mm/hr}	Allowable Release Rate (Q) ^{L/s}	Release Flow Per Unit Area (Q/ha) L/s/ha
Pre-development (Clifton)	0.0262	0.48	10	178.56	6.2	238.08
Post-development (Clifton)	0.0485	0.64	10	178.56	15.4	317.44
Variance (Post minus Pre)					9.2	

NOTES:

1. Calculated Time of Concentration (tc) using Bransby Williams (C > 0.4) is 7 min. Minimum Tc of 10 min used per City Standard.

2. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-28

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



CIMA+ PROJECT NUMBER: CLIENT: PROJECT STATUS: 316-332 Clifton Road Property Development A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

STORM PRE- VS POST DEVELOPMENT FLOW - UNATTENUATED AREAS (100-YEAR)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

PRE- vs POST-DEVELOPMENT FLOW DETERMINATION FOR UNATTENUATED AREAS: DESIGN CRITERIA:

Design Storm (year):	100	
IDF Regression Constants: (a) (b) (c)	1735.688 6.014 0.820	
IDF Curve Equation (mm/hr):	I = a / (Time	∋ in min + b) ^c
Rational Formula (L/s):	Q = 2.78C*I*A	where: Q = Flow (L/s) C = Runoff Coefficient I = Rainfall Intensity (mm/hr) A = Area (hectares)

RELEASE RATE SUMMARY - UNATTENUATED AREAS - PRE- vs POST-DEVELOPMENT (100-year):

Design Event	Area (A) _{ha}	Runoff Coefficient (C)	Time of Concentration (tc) ^{min}	Intensity (I) ^{mm/hr}	Allowable Release Rate (Q) ^{L/s}	Release Flow Per Unit Area (Q/ha) L/s/ha
Pre-development (Wilber)	0.0168	0.26	10	178.56	2.2	128.96
Post-development (Wilber)	0.0251	0.55	10	178.56	6.8	272.80
Variance (Post minus Pre)					4.7	

NOTES:

1. Calculated Time of Concentration (tc) using Bransby Williams (C > 0.4) is 7 min. Minimum Tc of 10 min used per City Standard.

2. IDF Parameters per City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier International Airport)

Prepared by: Jaymeson Adams, EIT

Date: 2020-08-28

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



PROJECT NAME:316-332 Clifton Road Property DevelopmentCIMA+ PROJECT NUMBER:A001062CLIENT:Clifton Property Development Inc.PROJECT STATUS:Preliminary Design (Assessment of Adequacy of Public Services)

STORMWATER MANAGEMENT – PRELIMINARY RETENTION CALCULATIONS

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

STORMWATER MANAGEMENT SUMMARY - STORAGE AND DRAWDOWN: DESIGN CRITERIA: Rainfall event 100.0 years Roof Flows 8.0 L/s Attenuated Flows (Not to Tank) 3.5 L/s

Attenuated Flows (Not to Tank)	3.5 L/s
Attenuated Flows (To Tank)	13.0 L/s
Unattenuated Flow (100 year)	13.9 L/s
Allowable Release Rate	38.4 L/s

Sub-Area	Total Area (m ²)	Available Storage Area (m ²)	Catchbasin/ Roof Drain Elevation (m)	Maximum Ponding Elevation (m)	Y _{max} (m)	V _{max} (m ³)	V _{rain} (m ³)	V _{acc} (m ³)	Y _{rain} (m)	Elev _{rain} (m)	A _{rain} (m ²)	Q (L/s)	Q _{1/2} for Storage Calculation (L/s)	Drawdown Time (min)	Comments
A1	184	184	100.00	100.15	0.15	9.2	6.4	6.4	0.12	100.12	153	1.0	N/A	106	Block 1
A2	276	276	100.00	100.15	0.15	13.8	8.4	8.4	0.12	100.12	216	2.0	N/A	70	Block 2
A3	190	190	100.00	100.15	0.15	9.5	6.6	6.6	0.13	100.13	159	1.0	N/A	111	Block 3
A4	190	190	100.00	100.15	0.15	9.5	6.6	6.6	0.13	100.13	159	1.0	N/A	111	Block 4
A5	190	190	100.00	100.15	0.15	9.5	6.6	6.6	0.13	100.13	159	1.0	N/A	111	Block 5
A6	321	321	100.00	100.15	0.15	16.1	10.5	10.5	0.12	100.12	260	2.0	N/A	87	Block 6
A7.1	110	28	64.20	64.30	0.10	0.9	0.9	0.9	0.10	64.30	27	3.5	N/A	4	Site Area not to Tank
A7.2	1183	-	-	-	0.76	37.8	36.0	36.0	-	-	-	13.0	6.5	92	Site Area to Tank
NC1	485	0	0.00	0.00	0.00	0.0	0.0	0.0	0.00	0.00	0.00	9.2	N/A	0.00	to Clifton
NC2	251	0	0.00	0.00	0.00	0.0	0.0	0.0	0.00	0.00	0.00	4.7	N/A	0.00	to Wilber
Total	3380	1379				106.3	82.1	82.1				38.4	6.5		

DEFINITIONS OF ABBREVIATIONS USED IN CALCULATION TABLE:	
NC = Area is not controlled (unattenuated)	V _{acc} = Total volume of water accumulated within the sub-area in the event of a specific rainfall.
Available Area = Area of water accumulated in sub-area at Max. Elev.	Y _{rain} = Depth of water generated by rainfall.
Catchbasin Elev. = Elevation of catchbasin inlet (top of grate).	Elev _{rain} = Elevation of water generated by rainfall.
Max. Elev. = Maximum elevation of water that may be accumulated within sub-area.	A _{rain} = Area of water generated by rainfall.
Y_{max} = Maximum depth of water that may be accumulated within the sub-area.	Q = Release flow rate.
V_{max} = Maximum volume of water (capacity) that may be accumulated within the sub-area.	Tank Release Rate = Release rate from the underground storage tank equal to 1/2 the allowable release rate.
V_{rain} = Volume of water generated by rainfall.	Drawdown Time = Time required for the total volume of water accumulated within sub-area to subside.
Prepared by:Jaymeson Adams, EIT Date:2020-09-03	

Verified by: Tim Kennedy, P.Eng. PEO# 100173201



316-332 Clifton Road Property DevelopmentA001062Clifton Property Development Inc.Preliminary Design (Assessment of Adequacy of Public Services)

RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A1 (BLOCK 1)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)				
Release Rate Per Unit Area (Q/ha):	54.35 L/s/ha				
Area (A):	0.0184 ha				
Runoff Coefficient (C):	0.95				
Rainfall Event:	100 year				
Release Rate (Q):	0.0010 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
Α	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

6.4 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m ³)	(m³)	(m ³)
T	I I	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	3.5	0.3	3.2
10.0	178.6	5.2	0.6	4.6
15.0	142.9	6.2	0.9	5.3
20.0	120.0	7.0	1.2	5.8
25.0	103.8	7.6	1.5	6.1
30.0	91.9	8.0	1.8	6.2
35.0	82.6	8.4	2.1	6.3
40.0	75.1	8.8	2.4	6.4
45.0	69.1	9.1	2.7	6.4
50.0	64.0	9.3	3.0	6.3
55.0	59.6	9.6	3.3	6.3
60.0	55.9	9.8	3.6	6.2
65.0	52.6	10.0	3.9	6.1
70.0	49.8	10.2	4.2	6.0
75.0	47.3	10.3	4.5	5.8
80.0	45.0	10.5	4.8	5.7
85.0	43.0	10.6	5.1	5.5
90.0	41.1	10.8	5.4	5.4
95.0	39.4	10.9	5.7	5.2
100.0	37.9	11.0	6.0	5.0
105.0	36.5	11.2	6.3	4.9
110.0	35.2	11.3	6.6	4.7
115.0	34.0	11.4	6.9	4.5
120.0	32.9	11.5	7.2	4.3
125.0	31.9	11.6	7.5	4.1
130.0	30.9	11.7	7.8	3.9
Design Volume:				6.4









316-332 Clifton Road Property Development A001062 Clifton Property Development Inc. Preliminary Design (Assessment of Adequacy of Public Services)

RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A2 (BLOCK 2)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)				
Release Rate Per Unit Area (Q/ha):	72.46 L/s/ha				
Area (A):	0.0276 ha				
Runoff Coefficient (C):	0.95				
Rainfall Event:	100 year				
Release Rate (Q):	0.0020 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
А	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

8.4 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m³)	(m³)	(m³)
Т	1	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	5.3	0.6	4.7
10.0	178.6	7.8	1.2	6.6
15.0	142.9	9.4	1.8	7.6
20.0	120.0	10.5	2.4	8.1
25.0	103.8	11.3	3.0	8.3
30.0	91.9	12.0	3.6	8.4
35.0	82.6	12.6	4.2	8.4
40.0	75.1	13.1	4.8	8.3
45.0	69.1	13.6	5.4	8.2
50.0	64.0	14.0	6.0	8.0
55.0	59.6	14.3	6.6	7.7
60.0	55.9	14.7	7.2	7.5
65.0	52.6	15.0	7.8	7.2
70.0	49.8	15.2	8.4	6.8
75.0	47.3	15.5	9.0	6.5
80.0	45.0	15.7	9.6	6.1
85.0	43.0	16.0	10.2	5.8
90.0	41.1	16.2	10.8	5.4
95.0	39.4	16.4	11.4	5.0
100.0	37.9	16.6	12.0	4.6
105.0	36.5	16.7	12.6	4.1
110.0	35.2	16.9	13.2	3.7
115.0	34.0	17.1	13.8	3.3
120.0	32.9	17.3	14.4	2.9
125.0	31.9	17.4	15.0	2.4
130.0	30.9	17.6	15.6	2.0
135.0	30.0	17.7	16.2	1.5
140.0	29.2	17.8	16.8	1.0
145.0	28.4	18.0	17.4	0.6
150.0	27.6	18.1	18.0	0.1
155.0	26.9	18.2	18.6	-0.4
160.0	26.2	18.3	19.2	-0.9
Design Volume:				0 /







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RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A3 (BLOCK 3)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airpor				
Release Rate Per Unit Area (Q/ha):	52.63 L/s/ha				
Area (A):	0.0190 ha				
Runoff Coefficient (C):	0.95				
Rainfall Event:	100 year				
Release Rate (Q):	0.0010 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
А	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

6.6 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m³)	(m³)	(m ³)
T		CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	3.7	0.3	3.4
10.0	178.6	5.4	0.6	4.8
15.0	142.9	6.4	0.9	5.5
20.0	120.0	7.2	1.2	6.0
25.0	103.8	7.8	1.5	6.3
30.0	91.9	8.3	1.8	6.5
35.0	82.6	8.7	2.1	6.6
40.0	75.1	9.0	2.4	6.6
45.0	69.1	9.3	2.7	6.6
50.0	64.0	9.6	3.0	6.6
55.0	59.6	9.9	3.3	6.6
60.0	55.9	10.1	3.6	6.5
65.0	52.6	10.3	3.9	6.4
70.0	49.8	10.5	4.2	6.3
75.0	47.3	10.7	4.5	6.2
80.0	45.0	10.8	4.8	6.0
85.0	43.0	11.0	5.1	5.9
90.0	41.1	11.1	5.4	5.7
95.0	39.4	11.3	5.7	5.6
100.0	37.9	11.4	6.0	5.4
105.0	36.5	11.5	6.3	5.2
110.0	35.2	11.6	6.6	5.0
115.0	34.0	11.8	6.9	4.9
120.0	32.9	11.9	7.2	4.7
125.0	31.9	12.0	7.5	4.5
130.0	30.9	12.1	7.8	4.3
135.0	30.0	12.2	8.1	4.1
140.0	29.2	12.3	8.4	3.9
145.0	28.4	12.4	8.7	3.7
150.0	27.6	12.5	9.0	3.5
Design Volume:				6.6







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RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A4 (BLOCK 4)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)				
Release Rate Per Unit Area (Q/ha):	52.63 L/s/ha				
Area (A):	0.0190 ha				
Runoff Coefficient (C):	0.95				
Rainfall Event:	100 year				
Release Rate (Q):	0.0010 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
А	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

6.6 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m ³)	(m³)	(m³)
T	I I	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	3.7	0.3	3.4
10.0	178.6	5.4	0.6	4.8
15.0	142.9	6.4	0.9	5.5
20.0	120.0	7.2	1.2	6.0
25.0	103.8	7.8	1.5	6.3
30.0	91.9	8.3	1.8	6.5
35.0	82.6	8.7	2.1	6.6
40.0	75.1	9.0	2.4	6.6
45.0	69.1	9.3	2.7	6.6
50.0	64.0	9.6	3.0	6.6
55.0	59.6	9.9	3.3	6.6
60.0	55.9	10.1	3.6	6.5
65.0	52.6	10.3	3.9	6.4
70.0	49.8	10.5	4.2	6.3
75.0	47.3	10.7	4.5	6.2
80.0	45.0	10.8	4.8	6.0
85.0	43.0	11.0	5.1	5.9
90.0	41.1	11.1	5.4	5.7
95.0	39.4	11.3	5.7	5.6
100.0	37.9	11.4	6.0	5.4
105.0	36.5	11.5	6.3	5.2
110.0	35.2	11.6	6.6	5.0
115.0	34.0	11.8	6.9	4.9
120.0	32.9	11.9	7.2	4.7
125.0	31.9	12.0	7.5	4.5
130.0	30.9	12.1	7.8	4.3
135.0	30.0	12.2	8.1	4.1
140.0	29.2	12.3	8.4	3.9
Design Volume:				6.6







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RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A5 (BLOCK 5)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)				
Release Rate Per Unit Area (Q/ha):	52.63 L/s/ha				
Area (A):	0.0190 ha				
Runoff Coefficient (C):	0.95				
Rainfall Event:	100 year				
Release Rate (Q):	0.0010 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
А	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

6.6 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m³)	(m³)	(m ³)
Т	1	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	3.7	0.3	3.4
10.0	178.6	5.4	0.6	4.8
15.0	142.9	6.4	0.9	5.5
20.0	120.0	7.2	1.2	6.0
25.0	103.8	7.8	1.5	6.3
30.0	91.9	8.3	1.8	6.5
35.0	82.6	8.7	2.1	6.6
40.0	75.1	9.0	2.4	6.6
45.0	69.1	9.3	2.7	6.6
50.0	64.0	9.6	3.0	6.6
55.0	59.6	9.9	3.3	6.6
60.0	55.9	10.1	3.6	6.5
65.0	52.6	10.3	3.9	6.4
70.0	49.8	10.5	4.2	6.3
75.0	47.3	10.7	4.5	6.2
80.0	45.0	10.8	4.8	6.0
85.0	43.0	11.0	5.1	5.9
90.0	41.1	11.1	5.4	5.7
95.0	39.4	11.3	5.7	5.6
100.0	37.9	11.4	6.0	5.4
105.0	36.5	11.5	6.3	5.2
110.0	35.2	11.6	6.6	5.0
115.0	34.0	11.8	6.9	4.9
120.0	32.9	11.9	7.2	4.7
125.0	31.9	12.0	7.5	4.5
130.0	30.9	12.1	7.8	4.3
135.0	30.0	12.2	8.1	4.1
140.0	29.2	12.3	8.4	3.9
145.0	28.4	12.4	8.7	3.7
Design Volume:				6.6







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RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A6 (BLOCK 6)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)				
Release Rate Per Unit Area (Q/ha):	62.31 L/s/ha				
Area (A):	0.0321 ha				
Runoff Coefficient (C):	0.95				
Rainfall Event:	100 year				
Release Rate (Q):	0.0020 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
А	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

10.5 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m³)	(m³)	(m³)
Т	1	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	6.2	0.6	5.6
10.0	178.6	9.1	1.2	7.9
15.0	142.9	10.9	1.8	9.1
20.0	120.0	12.2	2.4	9.8
25.0	103.8	13.2	3.0	10.2
30.0	91.9	14.0	3.6	10.4
35.0	82.6	14.7	4.2	10.5
40.0	75.1	15.3	4.8	10.5
45.0	69.1	15.8	5.4	10.4
50.0	64.0	16.3	6.0	10.3
55.0	59.6	16.7	6.6	10.1
60.0	55.9	17.0	7.2	9.8
65.0	52.6	17.4	7.8	9.6
70.0	49.8	17.7	8.4	9.3
75.0	47.3	18.0	9.0	9.0
80.0	45.0	18.3	9.6	8.7
85.0	43.0	18.6	10.2	8.4
90.0	41.1	18.8	10.8	8.0
95.0	39.4	19.0	11.4	7.6
100.0	37.9	19.3	12.0	7.3
105.0	36.5	19.5	12.6	6.9
110.0	35.2	19.7	13.2	6.5
115.0	34.0	19.9	13.8	6.1
120.0	32.9	20.1	14.4	5.7
125.0	31.9	20.2	15.0	5.2
130.0	30.9	20.4	15.6	4.8
135.0	30.0	20.6	16.2	4.4
140.0	29.2	20.7	16.8	3.9
145.0	28.4	20.9	17.4	3.5
Design Volume:				10.5







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RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A7.1 (GROUND LEVEL)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:					
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)				
Release Rate Per Unit Area (Q/ha):	313.81 L/s/ha				
Area (A):	0.0110 ha				
Runoff Coefficient (C):	0.87				
Rainfall Event:	100 year				
Release Rate (Q):	0.0035 m ³ /s				
Discharge Factor (K):	1				

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
A	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

0.9 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m ³)	(m³)	(m³)
Т	1	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	1.9	1.0	0.9
10.0	178.6	2.8	2.1	0.8
15.0	142.9	3.4	3.1	0.3
20.0	120.0	3.8	4.1	-0.3
25.0	103.8	4.1	5.2	-1.0
30.0	91.9	4.4	6.2	-1.8
35.0	82.6	4.6	7.2	-2.6
40.0	75.1	4.8	8.3	-3.5
45.0	69.1	5.0	9.3	-4.4
50.0	64.0	5.1	10.4	-5.3
55.0	59.6	5.2	11.4	-6.2
60.0	55.9	5.3	12.4	-7.1
65.0	52.6	5.5	13.5	-8.0
70.0	49.8	5.6	14.5	-8.9
75.0	47.3	5.7	15.5	-9.9
80.0	45.0	5.7	16.6	-10.8
85.0	43.0	5.8	17.6	-11.8
90.0	41.1	5.9	18.6	-12.7
95.0	39.4	6.0	19.7	-13.7
100.0	37.9	6.0	20.7	-14.7
105.0	36.5	6.1	21.7	-15.6
110.0	35.2	6.2	22.8	-16.6
115.0	34.0	6.2	23.8	-17.6
120.0	32.9	6.3	24.9	-18.6
125.0	31.9	6.4	25.9	-19.5
130.0	30.9	6.4	26.9	-20.5
135.0	30.0	6.5	28.0	-21.5
140.0	29.2	6.5	29.0	-22.5
Design Volume:				0.9







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RETENTION CALCULATIONS FOR FOR SUB-CATCHMENT AREA A7.2 (GROUND LEVEL)

APPLICABLE DESIGN GUIDELINES:

1. City of Ottawa Sewer Design Guidelines, 2012

REQUIRED STORAGE VOLUME DETERMINATION:

DESIGN CRITERIA:							
Rainfall Station:	City of Ottawa Sewer Design Guidelines, 2012 (Macdonald-Cartier Airport)						
Release Rate Per Unit Area (Q/ha):	109.89 L/s/ha						
Area (A):	0.1183 ha						
Runoff Coefficient (C):	0.87						
Rainfall Event:	100 year						
Release Rate (Q):	0.0065 m³/s						
Discharge Factor (K):	1						

Regression Constants	2 year	5 year	10 year	25 year	50 year	100 year
A	732.951	998.071	1174.184	1402.844	1569.58	1735.688
В	6.199	6.053	6.014	6.018	6.014	6.014
С	0.810	0.814	0.816	0.819	0.82	0.82

Required Retention Volume:

36.0 m³

Rainfall	Rainfall	Runoff	Output	Retention
Duration	Intensity	Volume	Volume	Volume
(min)	(mm/h)	(m ³)	(m³)	(m³)
Т	1	CIAT	kQT	(3)-(4)
(1)	(2)	(3)	(4)	(5)
5.0	242.7	20.8	2.0	18.9
10.0	178.6	30.6	3.9	26.7
15.0	142.9	36.8	5.9	30.9
20.0	120.0	41.2	7.8	33.4
25.0	103.8	44.5	9.8	34.8
30.0	91.9	47.3	11.7	35.6
35.0	82.6	49.6	13.7	35.9
40.0	75.1	51.6	15.6	36.0
45.0	69.1	53.3	17.6	35.8
50.0	64.0	54.9	19.5	35.4
55.0	59.6	56.3	21.5	34.8
60.0	55.9	57.5	23.4	34.1
65.0	52.6	58.7	25.4	33.3
70.0	49.8	59.8	27.3	32.5
75.0	47.3	60.8	29.3	31.5
80.0	45.0	61.7	31.2	30.5
85.0	43.0	62.6	33.2	29.5
90.0	41.1	63.5	35.1	28.4
95.0	39.4	64.3	37.1	27.2
100.0	37.9	65.0	39.0	26.0
105.0	36.5	65.7	41.0	24.8
110.0	35.2	66.4	42.9	23.5
115.0	34.0	67.1	44.9	22.2
120.0	32.9	67.7	46.8	20.9
125.0	31.9	68.3	48.8	19.6
130.0	30.9	68.9	50.7	18.2
135.0	30.0	69.5	52.7	16.8
140.0	29.2	70.0	54.6	15.4
Design Volume:				36.0





Jaymeson Adams, EIT Prepared by: Date: 2020-09-03 Tim Kennedy, P.Eng. PEO# 100173201 Verified by:

