

1 Canfield Road St. Mary Coptic Orthodox Church Expansion

Transportation Impact Assessment

Strategy Report

December 17, 2019

Prepared for:

St. Mary Coptic Orthodox Church

Prepared by:

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# 1.0 SCREENING

# 1.1 SUMMARY OF DEVELOPMENT

Municipal Address	1 Canfield Road
Description of Location	Northwest quadrant of Greenbank Road / Canfield Road / Craig Henry Drive
Land Use Classification	Institutional – Place of Worship (Church)
Development Size (units)	Existing: 1 building Proposed Expansion: 1 additional building
Development Size (ft²)	Existing: 12,920 ft <sup>2</sup> Proposed Expansion: 31,710 ft <sup>2</sup> Total: 44,630 ft <sup>2</sup>
Number of Accesses and Locations	1 existing access on Canfield Road (location will be shifted further west as part of this development application)
Phase of Development	1 of 1 total
Buildout Year	2021

If available, please attach a sketch of the development or site plan to this form.

# 1.2 TRIP GENERATION TRIGGER

Considering the development's land use type and size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size	Triggered
Single-family homes	40 units	*
Townhomes or apartments	90 units	*
Office	3,500 m <sup>2</sup>	*
Industrial	5,000 m <sup>2</sup>	×
Fast-food restaurant or coffee shop	100 m <sup>2</sup>	*
Destination retail	1,000 m <sup>2</sup>	*
Gas station or convenience market	75 m²	*
Development will generate more than 60 pe	rson trips	✓

<sup>\*</sup> If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, <u>the Trip Generation Trigger is satisfied.</u>



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# 1.3 LOCATION TRIGGERS

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?		×
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone? *		×

<sup>\*</sup>DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

# 1.4 SAFETY TRIGGERS

	Yes	No
Are posted speed limits on a boundary street 80 km/hr or greater?		×
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?		×
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	✓	
Is the proposed driveway within auxiliary lanes of an intersection?		×
Does the proposed driveway make use of an existing median break that serves an existing site?		×
Is there a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?		×
Does the development include a drive-thru facility?		×

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

# 1.5 SUMMARY

	Yes	No
Does the development satisfy the Trip Generation Trigger?	✓	
Does the development satisfy the Location Trigger?		×
Does the development satisfy the Safety Trigger?	✓	

If none of the triggers are satisfied, the TIA Study is complete. If one or more of the triggers is satisfied, the TIA Study must continue into the next stage (Screening and Scoping).



If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

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# 2.0 SCOPING

# 2.1 EXISTING AND PLANNED CONDITIONS

# 2.1.1 Proposed Development

The subject site is located in the Trend-Arlington neighbourhood of Ottawa, in the former City of Nepean. The site is located at the northwest quadrant of the intersection of Greenbank Road and Canfield Road / Craig Henry Drive and is bound by residential land uses to the north and to the west, Greenbank Road to the east, and Canfield Road to the south. The existing site includes the St. Mary Coptic Orthodox Church with a gross floor area of 12,920 ft<sup>2</sup>.

**Figure** 1 illustrates the location of the proposed site.

The subject site is currently zoned as Minor Institutional Zone (I1 B[428]) Zone; the purpose of the I1 – Minor Institutional Zone, according to the City of Ottawa Official Plan, is to:

- "Permit a range of community uses, institutional accommodation and emergency service uses to locate in areas designated as General Urban Area or Central Area in the official plan; and
- Minimize the impact of these minor institutional uses located in close proximity to residential uses by ensuring that the such uses are of a scale and intensity that is compatible with neighbourhood character."

**Figure 2** illustrates the proposed site plan. The existing site access is located on Canfield Road, approximately 60m west of Greenbank Road, and is a full-movements access. As part of the development application, this site access will be shifted approximately 2.5m west of the existing access. A total of 99 parking spaces are planned as part of the proposed expansion. The completion of the church expansion building is anticipated to occur by 2021.

The subject church expansion building is proposed to be a multi-purpose, two-storey building with subgrade basement as per the plans prepared by N45 Architecture Inc, dated October 10, 2019. The main floor will consist of a gym / hall that will be able to accommodate 672 people, a multimedia room, lounge, kitchen, as well as showers and change rooms. The second floor includes a study, lounges, a studio, as well as storage space. The basement includes 15 classrooms that will predominately be used on Sundays, a chapel that can seat 81 people, as well as meeting and office space. Within the site plan, there is one existing residential dwelling. This residential dwelling will be owned by the church, however, the function and use of this house will not change from existing.

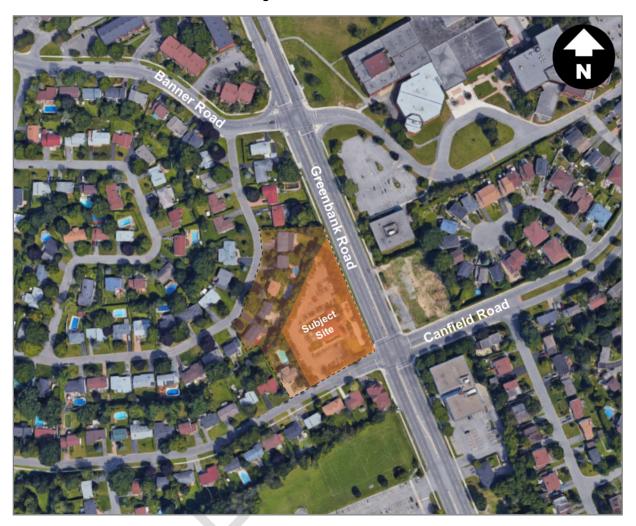
Due to the nature of the existing land use, the existing church building currently generates the most amount of traffic on Sundays. The proposed expansion building will generally be used by the congregation before / after the Sunday service, therefore, the proposed expansion building is not anticipated to generate any additional traffic above what is already being generated by the existing church on a typical Sunday.



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Figure 1 - Site Location





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TOPOGRAPHIC PLAN of SURVEY of LOTS 19, 38, 39, 40, 41 AND 42 AND PART OF BLOCK J REGISTERED PLAN 485324 CITY OF OTTAWA SURVEYOR'S CERTIFICATE ARY COPTIC ORTHODOX CHURCH 7797.96 m<sup>4</sup> 68.64 m 18.18 m (EXISTING 7.5 m MINL CORNER YARD
MINI. REAR YARD
MINI. INTER. SIDE YARD
MAX. BUILDING HEIGHT
PROPERSED PARKONS SPA TOTAL 4129.8 m² (44,451 s.f) N45 ARCHITECTURE INC. 71 Bank Street, 7th Floor - Otseva, Ontario, KTP 6N2 101. 813.224,0896 fee: 613.224,9811 ST. MARY COPTIC ORTHODOX CHURCH EXPANSION 1 CANFIELD RD, OTTAWA, ON 26×52 m 3.4 x 5.2 m TYPE A (B-F) 2 24×5.2 m TYPEB (B-F) 2 2.4 x 5.2 m REDUCED 2.6 x 4.8 m REDUCED drawing title SITE PLAN - NEW WORK CANFIELD ROAD 15-134 A-002 SITE PLAN SCALE 1990

Figure 2 - Proposed Site Plan



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## 2.1.2 Existing Conditions

#### 2.1.2.1 Roads and Traffic Control

The roadways under consideration in the study area are described as follows:

Greenbank Road Greenbank Road is a municipally owned, four-lane divided arterial roadway with a posted

speed limit of 60 km/hr across the frontage of the proposed site. Sidewalks are provided

along both sides of Greenbank Road.

Canfield Road Canfield Road is a municipally owned, two-lane undivided collector roadway with a posted

speed limit of 40 km/hr across the frontage of the proposed site. Sidewalks are provided along both sides of Canfield Road, east of the existing St. Mary Church entrance. The intersection with Greenbank Road is signalized with left turn auxiliary lanes on all

approaching legs.

Access to the site is achieved via the existing access on Canfield Road, approximately 60m west of Greenbank Road. Within 200m of the proposed access, there are numerous existing residential buildings and driveways along both sides of Canfield Road. In addition, Elvaston Avenue (local road) is located approximately 170m east of the existing site access.

The St. Mary Coptic Church community also operates a daycare on the southwest quadrant of the Greenbank Road at Canfield Road intersection. This daycare centre is not part of the subject development application, however, it will be taken into consideration when determining the internal circulation within the subject site.

In addition, access driveways are present on Greenbank Road, approximately 110m south of Canfield Road, to Knoxdale Public School on the west side of Greenbank Road, as well as a low-rise office building and Weefolk daycare facility on the east side of Greenbank Road.

Figure 3 illustrates the existing lane configuration and traffic control.



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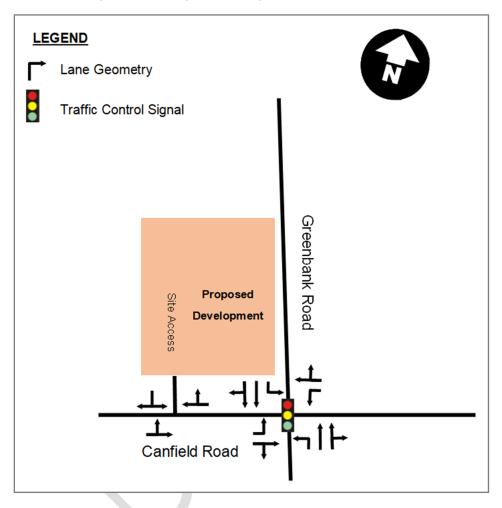


Figure 3 - Existing Lane Configuration and Traffic Control

## 2.1.2.2 Walking and Cycling

There are currently sidewalks along both sides of Greenbank Road and along both sides of Canfield Road, east of the subject site access. There are no multi-use pathways or community trails in the vicinity of the subject site that can be accessed by pedestrians and cyclists.

In general, cycling facilities in the Trend-Arlington community are developing but limited at present. Greenbank Road and Canfield Road have no cycling facilities as of October 2019. As per the City of Ottawa's Cycling Plan, Canfield Road Drive is designated as a suggested cycling route. The City of Ottawa's Ultimate Cycling Network designates Greenbank Road as a spine cycling route and Canfield Road as a local route.

Figure 4 illustrates the existing and planned pedestrian and cycling facilities within the vicinity of the subject site.

**Figure 5** illustrates the pedestrian and cyclist traffic identified in the City of Ottawa's traffic movement counts at the subject intersections during the PM and Sunday peak hours.



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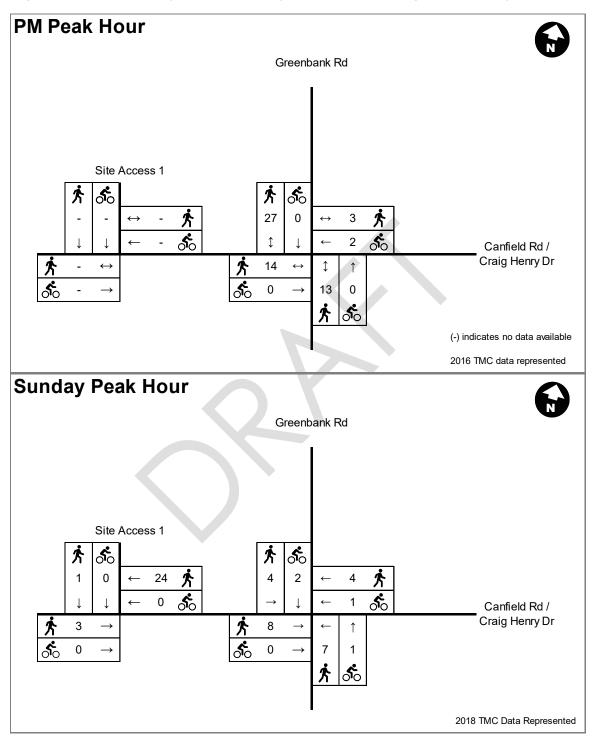
Figure 4 - Existing and Planned Active Modes Facilities

Source: geoOttawa, accessed October 2019



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Figure 5 - Pedestrian and Cyclist Traffic at Subject Intersections during PM and Sunday Peak Hours





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### 2.1.2.3 Transit

The subject site is currently serviced by transit through the following routes:

Route 82 Route 82 is a local bus route that provides service from Lincoln Fields / Tunney's Pasture to Bayshore / Craig Henry and operates throughout the day, 7 days per week. Service between Lincoln Fields and Tunney's Pasture operates during weekday peak-periods only.

Route 173 Route 173 is a local bus route that provides service between Barrhaven Centre and Bayshore Station and operates during the day from Monday to Friday.

Route 282 is a Connexion route providing bus service during peak hours (i.e. 6:00 – 9:00 AM and 3:00 -6:00 PM) from Monday to Friday. This route provides service between Tunney's Pasture and Trend-Arlington via Pinecrest Station.

Figure 6 illustrates nearby transit routes and stops in proximity to the subject site.

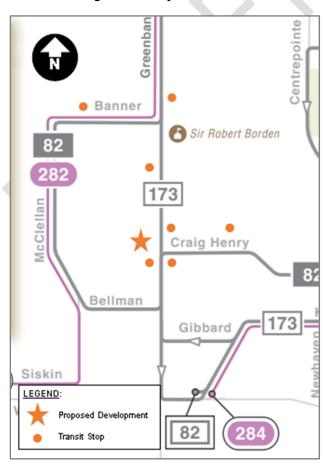


Figure 6 - Study Area Transit

Source: OC Transpo System Map, accessed October 11th, 2019



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## 2.1.2.4 Traffic Management Measures

No existing traffic management measures were identified near the subject site.

## 2.1.2.5 Traffic Volumes

Turning movement counts at the Greenbank Road at Canfield Road intersection were collected by the City of Ottawa for the PM peak hour (2016) and the Sunday peak hour (2018). Turning movement counts at the Canfield Road at Site Access intersection were collected by Stantec for the PM peak hour (2019) and by the City for the Sunday peak hour (2018).

**Figure 7 and Figure 8** below illustrates the traffic counts during the PM and Sunday peak hours adjusted for 2019 using a growth rate of 2%.

**Appendix A** contains the turning movement count data and is provided for reference.

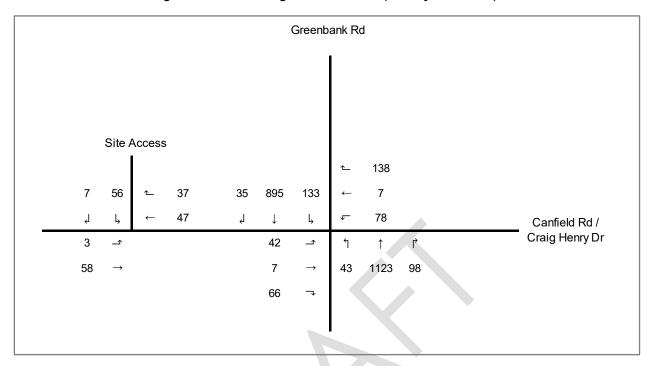
Greenbank Rd Site Access 118 2 11 19 1241 180 22 ٤ Ļ 67 Ļ 66 Canfield Rd / Craig Henry Dr 0 15 ٦ 7 \_ 1 33 23 11 912 85 18

Figure 7 - 2019 Existing Traffic Volumes (PM Peak)



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Figure 8 - 2019 Existing Traffic Volumes (Sunday Peak Hour)





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## 2.1.2.6 Collision History

Collision data was provided by the City of Ottawa for the period January 2014 to December 2018 in the vicinity of the subject site. The data was reviewed to determine if any intersections or road segments exhibited an identifiable collision pattern during the five (5) year period.

Table 1 summarizes the collision class and impact types for each road segment and intersection in the study area.

**Table 1 - Collision Summary** 

		IMPACT TYPE				
LOCATION	CLASS	Sideswipe	Angle / Turning	Rear End	Single Vehicle	Other
Greenbank Road	Property Damage	2		1		
between Banner Road and Canfield	Non-Fatal Injury				1	
Greenbank Road at	Property Damage	2	9	11		
Canfield Road / Craig Henry Drive	Non-Fatal Injury		8	5	3	
Greenbank Road	Property Damage	2	5	2		
between Canfield and Bellman Drive	Non-Fatal Injury		2	1		
Total	Property Damage	4	29	26	2	0
	Non-Fatal Injury	0	13	8	5	1

Based on the collision data summarized in **Table 1** above it was found that the majority of the collisions resulted in property damage only (69%), which suggests that the collisions occurred at low enough speeds to not cause serious injury to people. The Greenbank Road at Canfield Road / Craig Henry Drive intersections experienced 38 collisions. These collisions were further reviewed to determine if there are any discernable patters and can be seen in **Table 2** below.



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Table 2 - Collision Summary at Study Intersections

		Greenbank Road at Canfield Road / Craig Henry Drive
	Unknown	0
	Daylight	28
Time of Day	Dusk	0
	Dark	9
	Dawn	1
	Clear	31
	Rain	5
Environment	Snow	1
	Strong wind	0
	Fog, mist, smoke, dust	1
	Dry	28
	Wet	6
Surface Condition	Packed snow	0
Surface Condition	Ice	1
	Slush	1
	Loose snow	2

Most collisions occurred during clear environmental conditions (82%) on dry surface conditions (65%) during daylight hours (74%). Only two collisions in 2014 involved a pedestrian although, it should be noted that the City of Ottawa's OpenData database does not report pedestrian involvement in collisions for the year 2017 and as such, is not reflected.

## 2.1.3 Planned Conditions

## 2.1.3.1 Road Network Modifications

There are no planned transportation improvements in the vicinity of the subject development as per the City of Ottawa's 2013 Transportation Master Plan (TMP). The closest improvement to the subject site is the Baseline Road Bus Rapid Transit (BRT). As part of this transit priority project, transit signal priority and queue jump lanes and new bus stations will be introduced on Baseline Road between Baseline Station and Richmond Road, including a new station near the intersection of Baseline Road at Greenbank Road, which is approximately 1.5km north of the subject development. This transit project is scheduled to occur by 2023, as per direction from the City of Ottawa.

## 2.1.3.2 Future Background Developments

There are no planned developments outlined on the City of Ottawa's development applications website within the vicinity of the subject site that would affect the study area intersections.



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# 2.2 STUDY AREA AND TIME PERIODS

## 2.2.1 Study Area

The proposed study area includes the following intersections:

- 1. Greenbank Road at Canfield Road / Craig Henry Drive; and
- 2. Canfield Road at Site Access.

## 2.2.2 Time Periods

The proposed scope of the transportation assessment includes the following analysis time periods:

- Weekday PM peak hour of roadway; and
- Sunday peak hour generator.

It should be noted that the subject development is most active on Sundays, and as such, the Canfield Road at Site Access intersection will only be assessed during the Sunday peak hour.

## 2.2.3 Horizon Years

The scope of the transportation assessment proposes the following horizon years:

- 2019 existing conditions;
- 2021 future background conditions;
- 2021 total future conditions (site build-out); and
- 2026 total future conditions (5 years beyond build-out).



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# 2.3 EXEMPTIONS REVIEW

**Table 3** summarizes the Exemptions Review table from the City of Ottawa's 2017 Transportation Impact Assessment Guidelines.

Table 3 - Exemptions Review

Module	Element	Exemption Considerations	Exempted?
Design Review Component			
4.4 December and Decima	4.1.2 Circulation and Access	Only required for site plans	No
4.1 Development Design	4.1.3 New Street Networks	Only required for plans of subdivision	Yes
	4.2.1 Parking Supply	Only required for site plans	No
4.2 Parking	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	No
Network Impact Component	'		'
4.5 Transportation Demand Management	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	No
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	No
4.8 Network Concept		Only required when proposed development generates more than 200 person-trips during the peak hour in excess of the equivalent volume permitted by established zoning	Yes
4.9 Intersection Design	All Elements	Not required if site generation trigger is not met.	No



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# 3.0 FORECASTING

The Step 3.0 – Forecasting section has been reviewed by the City of Ottawa and was subject to revision as per the comments prepared by the City, dated November 27<sup>th</sup>, 2019. The comment responses reflected are herein. Further detail can be found in **Appendix B**.

# 3.1 DEVELOPMENT GENERATED TRAVEL DEMAND

## 3.1.1 Trip Generation and Mode Shares

On a typical Sunday, there will be no activity occurring in the expansion building during service in the existing church. Based on this, the trips generated by the entire site (i.e. existing church + proposed expansion building) on a Sunday will be based on the existing church only. As the church is currently operating at capacity during Sunday service, there will not be an increase in the number of people who attend Sunday service once the expansion building is complete. As such, a trip generation for the expansion building during the Sunday peak hour was not included in the subject TIA.

During the PM peak hour, the proposed expansion building may contain events that generate traffic. As outlined in the *Institute of Transportation Engineers (10<sup>th</sup> Edition) Trip Generation Manual,* Land-Use Code 560 – Church includes meeting rooms, classrooms, and dining, catering, or party rooms. As such, this land use is the most representative of the proposed expansion building. The aforementioned rationale was confirmed with the City of Ottawa as part of the Step 1, 2, and 3 TIAs.

Given the relatively negligible traffic that the proposed expansion building is anticipated to generate during the PM peak hour, it was conservatively assumed that all site trips will be auto trips (i.e. 100% auto mode share).

Table 4 outlines the projected site trips generated by the proposed expansion building during the PM peak hour.

Table 4 – Site Generated Traffic

LUC	Land Use	Size	Weekday PM Peak Hour				
LUC	Land Ose Size		In	Out	Rate		
Rates	Rates						
560	Church	31,475 sq. ft.	45%	55%	0.49		
Conversion to Person Trips							
	Church	Auto Trip Gen	7	9	16		
560		Person Trip Conversion	1.28	1.28	1.28		
		Person Trips	9	12	21		
Auto Trips							
560	560 Church 31,475 sq. ft.		9	12	21		



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# 3.1.2 Trip Assignment

Site generated trips were assigned to the study area road network based on the existing traffic distribution, as seen in **Figure 9**. It should be noted that the values in red represent 'outbound' trips and the values in black represent 'inbound' trips.

Figure 10 illustrates the site generated trips for the proposed development during the PM peak hour.

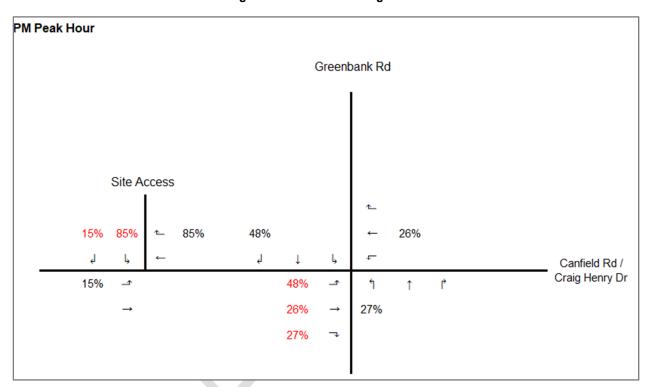


Figure 9 - Site Traffic Assignment



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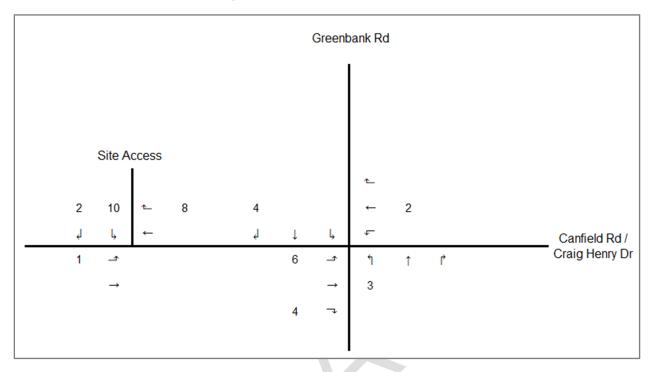


Figure 10 - Site Trips (PM Peak Hour)

# 3.2 BACKGROUND NETWORK TRAVEL DEMAND

# 3.2.1 Transportation Network Plans

There are no transit projects scheduled to occur within the time horizons of the subject study that will affect the study area intersections. As outlined in **Section 2.1.2.2**, Greenbank Road is included in the City of Ottawa's Ultimate Cycling Plan as a spine cycling route and Canfield Road is included as a local cycling route.

# 3.2.2 Background Growth

The City of Ottawa provided **Figure 11** below, which outlines the average annual growth rates based on trend lines. The average annual growth in the Trend-Arlington neighbourhood is in the range of -4.0% to 2.0%. To be conservative, a 2% annual background growth rate was used in the subject analysis.



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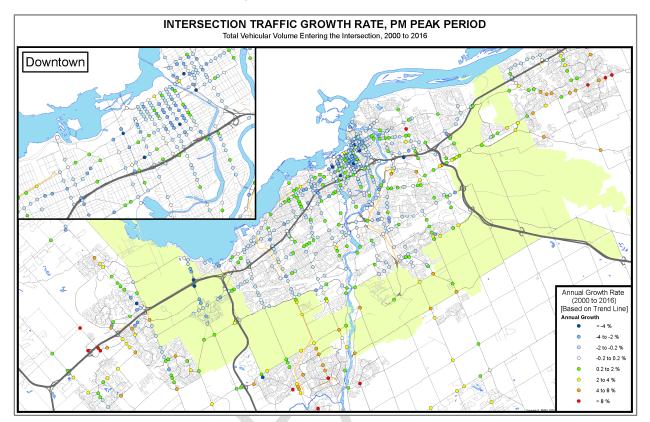


Figure 11 - Annual Growth Rates

# 3.2.3 Other Developments

As outlined in **Section 2.1.3.2**, there are no background developments scheduled to occur within the time horizons of the subject study that will affect the study area intersections.

# 3.3 DEMAND RATIONALIZATION

The proposed site is not anticipated to encounter any capacity restrictions that cannot be resolved through roadway improvements and therefore no demand rationalization is required.



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# 4.0 STRATEGY REPORT

## 4.1 DEVELOPMENT DESIGN

## 4.1.1 Design for Sustainable Modes

To facilitate walking in the vicinity of the subject site, there are existing sidewalks along both sides of Greenbank Road as well as along both sides of Canfield Road, along the frontage of the subject site. West of the subject site, the sidewalk on Canfield Road is only on the south side of the road.

There are no existing cycling facilities within the vicinity of the subject site, as vehicles operate in mixed traffic on both Greenbank Road and Canfield Road. A bicycle rack will be provided next to the existing church building that will serve both the existing church as well as the proposed expansion building.

The subject site is currently well serviced by transit along Greenbank Road. There are existing transit stops at the intersection of Greenbank Road at Canfield Road, approximately 100m from the front door of the existing church. These transit stops are serviced by routes 82 and 173, as described in **Section 2.1.2.3**.

### 4.1.2 Circulation and Access

The existing church is currently operational with one site access located on Canfield Road, approximately 50m west of Greenbank Road. The proposed site plan includes shifting this site access approximately 2.5m further west to maximize the circulation on-site. This site access will be stop-controlled along the site access approach and no turning restrictions will be included.

The existing sidewalk on the north side of Canfield Road is proposed to be extended until the proposed site access location. In addition, a new walkway is being proposed at the southwest quadrant of the site plan which will provide a more direct connection from Greenbank Road to the subject site.

## 4.1.3 New Street Networks

Not applicable; exempted during screening and scoping.

## 4.2 PARKING

## 4.2.1 Parking Supply

**Auto Parking** – The subject development is located within 'Area C: Suburban' as outlined in the City of Ottawa's Zoning By-Law Schedule 1A. Based on this classification, as per City of Ottawa Zoning By-law 2008-250 (Section 101 and 102), the minimum parking space requirement for a place of worship is 10 auto parking spaces per 100m<sup>2</sup> of gross floor area of assembly area and 2.4 parking spaces per 100m<sup>2</sup> of office space. The place of worship, gym, and office spaces are the only uses within the proposed site plan that requires dedicated auto parking spaces.



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The existing church has approximately 275m<sup>2</sup>, which equates to a parking requirement of 28 spaces. The proposed gym / hall has approximately 570m<sup>2</sup>, which equates to a parking requirement of 57 parking spaces. The proposed office has approximately 100m<sup>2</sup>, which equates to approximately 4 parking spaces. The total required number of auto parking spaces is therefore 89. The proposed site plan includes 99 parking spaces, which meets the minimum requirements.

**Bicycle Parking** – As per City of Ottawa Zoning By-law 2008-250 (Section 111), the minimum bicycle parking rate of 1 bicycle parking space per 1500m<sup>2</sup> of gross floor area is required. The existing church and the entire expansion building contribute to the bicycle parking for the proposed site. The total area for both buildings is approximately 4,100m<sup>2</sup>, which equates to roughly 3 bicycle parking spaces. The proposed site plan includes 4 bicycle parking spaces which meets the minimum requirement.

## 4.2.2 Spillover Parking

Stantec conducted parking surveys on Sunday, April 29<sup>th</sup>, 2018 between 8:30 AM and 2:00 PM. **Table 5** outlines the findings from this parking survey.

**Number of Parking Spaces** % Utilized Location **Maximum Occupied** Capacity St. Mary Coptic Church 75 113% 85 (existing) Canfield Road 34 17 50% Knoxdale Public School 5% 78 4 Parkmount Crescent 119 9 8% 20 Banner Road 15 75% 6% Keppler Crescent 67 4

**Table 5 - Parking Survey Results** 

The existing church was found to have a maximum of 85 vehicles parked on site. Information was provided to Stantec by the church that there are approximately an additional 10 vehicles who currently park on-street on Sundays, for a total demand of 95 parking spaces. The proposed site plan includes 99 parking spaces, which exceeds the demand of 95 parking spaces.

Despite the proposed parking supply exceeding the anticipated demand, the Church is pro-actively putting an agreement in place with the nearby Knoxdale Public School whereby members of the church can park in the school parking lot on Sundays as an alternative to parking on-street. This school is located approximately 160m from the church, or approximately a two-minute walk. There will be a sign on the church property indicating that once the church parking lot is full, motorists are to park in the Knoxdale parking lot and walk over. This should eliminate any neighbourhood concerns regarding parking along Canfield Road. As there will not be a pathway connecting the church to Parkmount Crescent, members of the church will likely not park along Parkmount Crescent as it would involve walking



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greater than 400m from their car to church, which is more than double the distance from the church to Knoxdale Public School.

During the parking survey, it was found that the number of parked vehicles along Keppler Crescent, Banner Road, and Parkmount Crescent were consistent between 8:30 AM – 2:00 PM. This suggests that the parked cars along these three roads are attributed to the residents and not members of the St. Mary Coptic Church.

## 4.3 BOUNDARY STREET DESIGN

### 4.3.1 Multi Modal Level of Service

The multi-modal level of service (MMLOS) was evaluated for Greenbank Road and Canfield Road to assist with developing a design concept that maximizes the achievement of the MMLOS objectives. Based on the proximity of these two roads to Knoxdale Public School, it was determined that both subject roads, across the frontages of the subject site, fall under the 'within 300m of a school' Policy Area designation. This policy area dictates the following MMLOS targets that will be applied to the two roadways.

As Greenbank Road (arterial) and Canfield Road (collector) are within 300m of a school, these roadway segments are subject to a pedestrian level of service (PLOS) target of A.

The 2013 Ottawa Transportation Master Plan designates Greenbank Road as a spine route and Canfield Road as a local cycling route. As such, Greenbank Road has a Bicycle Level of Service (BLOS) target of C and Canfield Road has a BLOS target of B.

For Transit Level of Service (TLOS), both Greenbank Road has a TLOS target of D. As Canfield Road, across the frontage of the subject site, does not currently have any transit routes, the TLOS does not apply for this roadway segment.

Greenbank Road is a truck route and is therefore subject to a Truck Level of Service (TkLOS) target of D. As Canfield Road is a collector road and is not a truck route, TkLOS does not apply.

**Table 6** presents the MMLOS for the two roadway segments.

#### **Greenbank Road**

Greenbank Road, fronting the proposed development, achieves a PLOS of E, which fails to meet the PLOS target of A. To meet the PLOS target along Greenbank Road, the sidewalk on the west side would need to be widened to 1.8m while maintaining a boulevard of greater than 2.0m coupled with a reduction in the speed limit to 40 km/hr and a reduction in the curb lane AADT to less than 3000. Although the pedestrian facilities are well-developed along Greenbank Road (i.e., sidewalks and boulevards along both sides), proximity to Knoxdale Elementary School results in an unattainably high PLOS target given the nature of Greenbank Road being an arterial roadway with high speeds and high volumes.

As cyclists operate in mixed traffic along Greenbank Road, it operates with a BLOS of F, which does not meet the target of C. Keeping the mixed-use lanes, the speed limit would need to be reduced to 40km/hr in order to meet the BLOS target. An alternative to achieving the BLOS target of D would be to implement curbside bicycle lanes along both sides



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of Greenbank Road, however, this could have spatial and financial constraints. A second alternative to achieving the BLOS target would be to implement physically separated cycling facilities (i.e. a cycle track) along both sides of Greenbank Road. Similar to implementing on-street bicycle lanes, this could also have a spatial and financial constraints.

Greenbank Road, across the frontage of the subject site, meets both the TLOS and TkLOS targets.

### **Canfield Road**

While Canfield Road, across the frontage of the subject site, includes sidewalks along both sides, it currently does not meet the PLOS target of A. Implementing a 0.5m boulevard would allow the PLOS target of A to be met. Another alternative to meet the PLOS target would be to reduce the posted speed limit.

Canfield Road, across the frontage of the subject site, meets the BLOS target of B.

As Canfield Road is not a transit route nor truck route, the TLOS and TkLOS targets do not apply.

**PLOS** BLOS TI OS **TkLOS Roadway Segment** Target **Actual Target** Actual **Target Actual Target** Actual Greenbank Road along Ε С D D Α Α D property line Canfield Road along N/A N/A Α В В Α property line

Table 6 - Roadway Segment MMLOS

Appendix C contains the detailed roadway segment MMLOS analysis.

# 4.4 ACCESS INTERSECTION DESIGN

## 4.4.1 Access Location

There is an existing site access to the church, located on Canfield Road approximately 50m west of Greenbank Road. The proposed site plan includes shifting this site access approximately 2.5m west. The access will be stop-controlled on the site access approach without any turning restrictions. No secondary accesses are proposed.

### 4.4.2 Intersection Control

#### **Greenbank Road at Canfield Road**

The existing intersection of Greenbank Road at Canfield Road is signalized with auxiliary left turn lanes in all directions.

### **Canfield Road at Site Access**

The existing intersection of Canfield Road at Site Access does not currently have a traffic control device; however, it operates as a stop-controlled intersection along the Site Access approach.



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## 4.5 TRANSPORTATION DEMAND MANAGEMENT

City of Ottawa TDM Checklists were used to determine what TDM measures could be implemented based on the available information. Based on the checklists, the following TDM measures have been incorporated into the site plan:

- Building entrances are located in order to minimize walking distances to sidewalks and transit stops;
- Safe, direct, and attractive pedestrian access between the public sidewalks and building entrances have been provided;
- Sidewalks will be smooth, well-drained surfaces and will be easily accessible;
- Links to the existing sidewalk network will be included;
- Safe, direct, and attractive walking routes will be provided between building entrances and nearby transit stops;
- Bicycle parking will be provided in highly visible areas;
- The number of bicycle parking meets the requirements per the Zoning By-Law;
- The bicycle parking racks will be securely anchored; and
- The number of vehicle parking spaces meets the requirements per the Zoning By-Law.

The TDM checklists are contained in Appendix D.

## 4.6 NEIGHBHOURHOOD TRAFFIC MANAGEMENT

# 4.6.1 Adjacent Neighbourhoods

As only one site access is proposed, all subject development traffic will use Canfield Road to access Greenbank Road. **Table 7** summarizes the PM and Sunday peak two-way traffic volume forecasts for Canfield Road at the build-out of the subject site.

Table 7 - AM & PM 2021 Traffic Volume Forecasts for Canfield Road

Road	2021 Total Traffic Volume PM Peak	2021 Total Traffic Volume SUN Peak
Canfield Road	160 veh/hr	225 veh/hr

The traffic volumes along Canfield Road at the build-out of the subject development are not projected to exceed the threshold of 300 vehicles/hour (veh/hr) for collector roadways.



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# 4.7 TRANSIT

# 4.7.1 Route Capacity

As the proposed development is anticipated to generate a negligible number of trips during the PM peak hour (and the Sunday peak hour will be the same as existing), a conservative 100% auto modal share was applied. As such, the proposed development is anticipated to generate a negligible amount of new transit trips.

Section 2.1.2.3 outlines the three existing transit routes within the vicinity of the subject site.

## 4.8 REVIEW OF NETWORK CONCEPT

Not applicable; exempted during screening and scoping.

## 4.9 INTERSECTION DESIGN

### 4.9.1 Intersection Control

The intersection controls for the three study area intersections were discussed in **Section 4.4.2** and the analysis of the intersections can be seen in **Section 4.9.2**.

## 4.9.2 Intersection Design

An assessment of the study area intersections was undertaken to determine the operational characteristics under the various horizons identified in the Screening and Scoping report. Intersection operational analysis was facilitated with Synchro 10.0<sup>TM</sup> software package and the MMLOS analysis was completed for all modes and compared against the City of Ottawa's MMLOS targets.

### 4.9.2.1 2019 Existing Conditions

Figure 7 and Figure 8 illustrate 2019 existing PM and Sunday peak hour traffic volumes at the study area intersection.

### **Intersection Capacity Analysis**

Table 8 summarizes the results of the Synchro analysis for 2019 existing intersection operations.

Both study area intersections are currently operating satisfactorily, and as such, no improvements are required to supplement existing conditions.

Figure 3 illustrates the existing intersection control and lane configurations at the two study area intersections.

Appendix E contains detailed intersection performance worksheets.



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Table 8 - 2019 Existing Intersection Operations

Intersection	Intersection Control	Ар	proach / Movement	LOS	V/C	Delay (s)	Queue 95 <sup>th</sup> (m)		
		EB	Left		0.15 (0.24)	52.1 (33.8)	6.3 (11.9)		
		ED	Through / Right	A (A)	0.15 (0.30)	42.2 (27.5)	11.2 (18.2)		
	Signalized	WB	Left	A (A)	0.32 (0.33)	46.0 (31.1)	25.9 (21.0)		
Greenbank		VVD	Through / Right	C (A)	0.74 (0.60)	50.8 (30.6)	58.1 (39.9)		
Road at Canfield Road		NB	Left	A (A)	0.09 (0.13)	11.3 (10.5)	4.9 (6.3)		
			Through / Right	A (B)	0.43 (0.61)	6.1 (9.7)	64.4 (80.5)		
		SB	Left	A (A)	0.52 (0.55)	17.2 (27.1)	46.9 (37.8)		
			Through / Right	A (A)	0.55 (0.46)	7.5 (7.6)	89.6 (56.7)		
			Overall Intersection	-	-	11.4 (12.5)	-		
		EB	Left / Through	A (A)	0.00 (0.00)	0.0 (0.4)	0.0 (0.0)		
Canfield Road	Minor Stop- Control	WB	Through / Right	A (A)	0.00 (0.00)	0.0 (0.0)	0.0 (0.0)		
at Site Access		SB	Left / Right	A (A)	0.02 (0.08)	9.1 (9.5)	0.6 (1.8)		
		(	Overall Intersection	A (A)	-	0.9 (3.0)	-		
Notes: 1. Table format: PM (SUN) 2. v/c – represents the anticipated volume divided by the predicted capacity									

#### Multi-Modal Level of Service Assessment

Based on the proximity of this intersection to the nearby Knoxdale Elementary School, it was determined that the intersection of Greenbank Road at Canfield Road falls under the 'within 300m of a school' Policy Area designation. As such, the intersection is subject to a Pedestrian Level of Service (PLOS) target of A.

The 2013 Ottawa Transportation Master Plan designates Greenbank Road as a spine cycling route and Canfield Road as a local cycling route in the Ultimate Cycling Network. As such, the Churchill Avenue at Richmond Road intersection has a bicycle level of service (BLOS) target of B.

Greenbank Road has transit routes along it, and therefore, the Transit Level of Service (TLOS) target for the intersection is D.

Greenbank Road is also designated as a truck route; however, Canfield Road is not. As such, trucks will not be turning at the intersection of Greenbank Road and Canfield Road, thus, the Truck Level of Service (TkLOS) does not apply to this intersection.

Table 9 summarizes the MMLOS at the intersection of Greenbank Road at Canfield Road under 2019 existing conditions.

Appendix C contains the detailed MMLOS analysis.

**Table 9 - Existing Signalized Intersection MMLOS** 

Signalized	PLOS		BLOS		TLOS		TkLOS	
Intersection	Target	Actual	Target	Actual	Target	Actual	Target	Actual
Greenbank Road at Canfield Road	А	E	В	F	D	F	N	/A



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The Pedestrian Level of Service at the intersection of Greenbank Road at Canfield Road currently operates with a PLOS of E, which is below the desired target. The number of lanes required to cross Greenbank Road results in the PLOS operating below the target. Measures to improve the PLOS at this intersection include reducing the number of lanes on Greenbank Road, allocating more green time to the east / west movements for vehicles (and thus, pedestrians), implementing medians along Greenbank Road that are more than 2.4m wide, protecting the left turn phases, prohibiting the right turns on red, and raising the crosswalks.

The Bicycle Level of Service at the intersection of Greenbank Road at Canfield Road currently operates with a BLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection BLOS is influenced by the availability of dedicated cycling amenities, number of lanes cyclists must cross to negotiate a turn at intersections, and roadway operating speeds. Due to the nature of arterial roads, the posted speed limit is typically 60 km/hr or greater, which limits the potential BLOS at intersections. Reducing the posted speed limit is not a feasible option for Greenbank Road. Implementing a higher order bicycle facility (i.e. curbside bicycle lanes with two-stage left turn bike boxes) would allow the BLOS to meet the target.

The Transit Level of Service at the intersection of Greenbank Road at Canfield Road currently operates with a TLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection transit level of service is governed by the delay at the intersection. The westbound approach experiences the highest delay, which governs the TLOS at the intersection. Allocating more green time to the east / west movements for vehicles would improve the delay on the westbound approach, however, it would be to the detriment of the northbound and southbound vehicles.

## 4.9.2.2 2021 Future Background Conditions

Figure 12 illustrates 2021 future background PM and Sunday peak hour traffic volumes at the study area intersections.

**Table 10** summarizes the results of the Synchro analysis under 2021 future background conditions. Both study area intersections are anticipated to operate acceptably under 2021 future background conditions.

**Appendix E** contains detailed intersection performance worksheets.

Table 10 – 2021 Future Background Intersection Operations

Intersection	Intersection Control	Ар	proach / Movement	LOS	V/C	Delay (s)	Queue 95 <sup>th</sup> (m)		
		EB	Left	A (A)	0.14 (0.23)	52.1 (33.9)	6.3 (11.2)		
		ED	Through / Right	A (A)	0.15 (0.30)	42.8 (28.0)	10.5 (17.5)		
	Signalized	WB	Left	A (A)	0.31 (0.31)	46.4 (31.4)	25.2 (19.6)		
Greenbank Road at Canfield Road		WB	Through / Right	A (A)	0.73 (0.60)	51.2 (31.1)	55.3 (37.1)		
		NB	Left	A (A)	0.08 (0.11)	9.6 (9.3)	4.2 (5.6)		
		IND	Through / Right	A (B)	0.40 (0.56)	5.6 (8.6)	58.1 (71.4)		
		SB	Left	A (B)	0.44 (0.46)	13.5 (20.4)	37.8 (29.4)		
			Through / Right	A (A)	0.51 (0.42)	6.7 (6.9)	79.8 (51.1)		
			Overall Intersection	-	-	10.7 (11.4)	-		
	Minor Stop- Control	EB	Left / Through	A (A)	0.00 (0.00)	0.0 (0.4)	0.0 (0.0)		
Canfield Road		WB	Through / Right	A (A)	0.00 (0.00)	0.0 (0.0)	0.0 (0.0)		
at Site Access		SB	Left / Right	A (A)	0.02 (0.07)	9.1 (9.5)	0.0 (1.4)		
		Overall Intersection		A (A)	-	0.9 (2.9)	-		
Notes:  1. Table format: PM (SUN)  2. V/G. represents the anticipated volume divided by the predicted capacity.									

v/c – represents the anticipated volume divided by the predicted capacity



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#### **Multi-Modal Level of Service Assessment**

Based on the proximity of this intersection to the nearby Knoxdale Elementary School, it was determined that the intersection of Greenbank Road at Canfield Road falls under the 'within 300m of a school' Policy Area designation. As such, the intersection is subject to a Pedestrian Level of Service (PLOS) target of A.

The 2013 Ottawa Transportation Master Plan designates Greenbank Road as a spine cycling route and Canfield Road as a local cycling route in the Ultimate Cycling Network. As such, the Churchill Avenue at Richmond Road intersection has a bicycle level of service (BLOS) target of B.

Greenbank Road has transit routes along it, and therefore, the Transit Level of Service (TLOS) target for the intersection is D.

Greenbank Road is also designated as a truck route; however, Canfield Road is not. As such, trucks will not be turning at the intersection of Greenbank Road and Canfield Road, thus, the Truck Level of Service (TkLOS) does not apply to this intersection.

**Table 12** summarizes the MMLOS at the intersection of Greenbank Road at Canfield Road under 2021 future background conditions.

Appendix C contains the detailed MMLOS analysis.

Table 11 – 2021 Future Background Signalized Intersection MMLOS

		A000000	1000					
Signalized	PLOS		BLOS		TLOS		TkLOS	
Intersection	Target	Actual	Target	Actual	Target	Actual	Target	Actual
Greenbank Road at Canfield Road	A	E	В	F	D	F	N,	/A

The Pedestrian Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a PLOS of E, which is below the desired target. The number of lanes required to cross Greenbank Road results in the PLOS operating below the target. Measures to improve the PLOS at this intersection include reducing the number of lanes on Greenbank Road, allocating more green time to the east / west movements for vehicles (and thus, pedestrians), implementing medians along Greenbank Road that are more than 2.4m wide, protecting the left turn phases, prohibiting the right turns on red, and raising the crosswalks.

The Bicycle Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a BLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection BLOS is influenced by the availability of dedicated cycling amenities, number of lanes cyclists must cross to negotiate a turn at intersections, and roadway operating speeds. Due to the nature of arterial roads, the posted speed limit is typically 60 km/hr or greater, which limits the potential BLOS at intersections. Reducing the posted speed limit is not a feasible option for Greenbank Road. Implementing a higher order bicycle facility (i.e. curbside bicycle lanes with two-stage left turn bike boxes) would allow the BLOS to meet the target.

The Transit Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a TLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection transit level of service is



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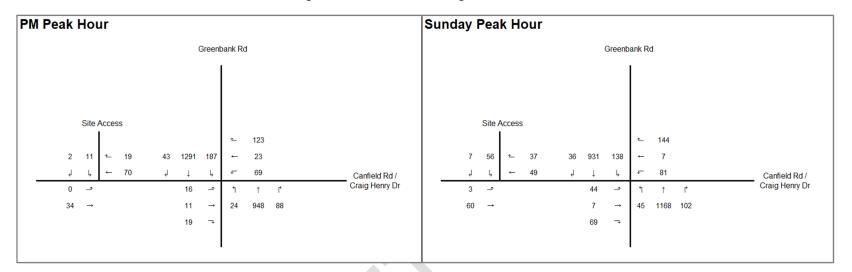
governed by the delay at the intersection. The westbound approach experiences the highest delay, which governs the TLOS at the intersection. Allocating more green time to the east / west movements for vehicles would improve the delay on the westbound approach, however, it would be to the detriment of the northbound and southbound vehicles.





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Figure 12 - 2021 Future Background Volumes





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#### 4.9.2.3 2021 Total Future Conditions

Figure 13 illustrates 2021 total future PM and Sunday peak hour traffic volumes at the study area intersections.

Table 12 summarizes the results of the Synchro analysis for 2021 total future intersection operations.

Both study area intersections are projected to operate acceptably under 2021 total future conditions.

**Appendix E** contains detailed intersection performance worksheets.

Table 12 – 2021 Total Future Intersection Operations

Intersection	Intersection Control	Ар	proach / Movement	LOS	V/C	Delay (s)	Queue 95 <sup>th</sup> (m)		
		EB Left		A (A)	0.20 (0.24)	52.6 (33.8)	9.1 (11.9)		
		ED	Through / Right	A (A)	0.17 (0.32)	42.9 (28.0)	11.9 (18.9)		
	Signalized	WB	Left	A (A)	0.32 (0.31)	46.6 (31.4)	25.2 (19.6)		
Greenbank		VVD	Through / Right	A (A)	0.73 (0.59)	50.9 (30.8)	55.3 (37.1)		
Road at		NB	Left	A (A)	0.09 (0.12)	10.0 (9.6)	4.9 (6.3)		
Canfield Road			Through / Right	A (B)	0.40 (0.56)	5.6 (8.8)	58.1 (72.1)		
		SB	Left	A (B)	0.45 (0.46)	13.6 (20.8)	37.8 (29.4)		
			Through / Right	A (A)	0.52 (0.43)	6.8 (7.1)	80.5 (51.8)		
		(	Overall Intersection	-	-	10.9 (11.6)	-		
		EB	Left / Through	A (A)	0.00 (0.00)	0.2 (0.5)	0.0 (0.0)		
Canfield Road	Minor Stop- Control	WB	Through / Right	A (A)	0.00 (0.00)	0.0 (0.0)	0.0 (0.0)		
at Site Access		SB	Left / Right	A (A)	0.03 (0.10)	9.2 (9.6)	0.7 (2.1)		
		(	Overall Intersection	A (A)	-	1.7 (3.4)	-		
Notes:  1. Table format: PM (SUN)  2. v/c – represents the anticipated volume divided by the predicted capacity									

# Multi-Modal Level of Service Assessment

Based on the proximity of this intersection to the nearby Knoxdale Elementary School, it was determined that the intersection of Greenbank Road at Canfield Road falls under the 'within 300m of a school' Policy Area designation. As such, the intersection is subject to a Pedestrian Level of Service (PLOS) target of A.

The 2013 Ottawa Transportation Master Plan designates Greenbank Road as a spine cycling route and Canfield Road as a local cycling route in the Ultimate Cycling Network. As such, the Churchill Avenue at Richmond Road intersection has a bicycle level of service (BLOS) target of B.

Greenbank Road has transit routes along it, and therefore, the Transit Level of Service (TLOS) target for the intersection is D.

Greenbank Road is also designated as a truck route; however, Canfield Road is not. As such, trucks will not be turning at the intersection of Greenbank Road and Canfield Road, thus, the Truck Level of Service (TkLOS) does not apply to this intersection.

**Table 13** summarizes the MMLOS at the intersection of Greenbank Road at Canfield Road under 2021 total future conditions.



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Appendix C contains the detailed MMLOS analysis.

Table 13 - 2021 Total Future Signalized Intersection MMLOS

Signalized	PLOS		BLOS		TLOS		TkLOS	
Intersection	Target	Actual	Target	Actual	Target	Actual	Target	Actual
Greenbank Road at Canfield Road	Α	E	В	F	D	F	N/A	

The Pedestrian Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a PLOS of E, which is below the desired target. The number of lanes required to cross Greenbank Road results in the PLOS operating below the target. Measures to improve the PLOS at this intersection include reducing the number of lanes on Greenbank Road, allocating more green time to the east / west movements for vehicles (and thus, pedestrians), implementing medians along Greenbank Road that are more than 2.4m wide, protecting the left turn phases, prohibiting the right turns on red, and raising the crosswalks.

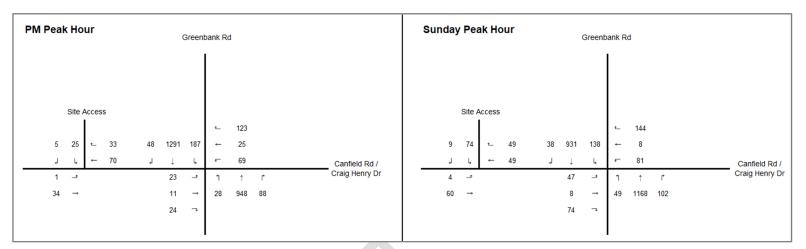
The Bicycle Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a BLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection BLOS is influenced by the availability of dedicated cycling amenities, number of lanes cyclists must cross to negotiate a turn at intersections, and roadway operating speeds. Due to the nature of arterial roads, the posted speed limit is typically 60 km/hr or greater, which limits the potential BLOS at intersections. Reducing the posted speed limit is not a feasible option for Greenbank Road. Implementing a higher order bicycle facility (i.e. curbside bicycle lanes with two-stage left turn bike boxes) would allow the BLOS to meet the target.

The Transit Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a TLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection transit level of service is governed by the delay at the intersection. The westbound approach experiences the highest delay, which governs the TLOS at the intersection. Allocating more green time to the east / west movements for vehicles would improve the delay on the westbound approach, however, it would be to the detriment of the northbound and southbound vehicles.



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Figure 13 - 2021 Total Future Volumes





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#### 4.9.2.4 2026 Ultimate Conditions

Figure 14 illustrates 2026 ultimate PM and Sunday peak hour traffic volumes at the study area intersections.

#### **Intersection Capacity Analysis**

Table 14 summarizes the results of the Synchro analysis for 2026 ultimate intersection operations.

Both study area intersections are projected to operate acceptably under 2026 ultimate conditions.

**Appendix E** contains detailed intersection performance worksheets.

Table 14 - 2026 Ultimate Intersection Operations

Intersection	Intersection Control	Ар	proach / Movement	LOS	V/C	Delay (s)	Queue 95 <sup>th</sup> (m)
		EB	Left	A (A)	0.20 (0.25)	52.5 (33.7)	9.1 (12.6)
		ED	Through / Right	A (A)	0.18 (0.31)	42.0 (27.3)	13.3 (19.6)
		WB	Left	A (A)	0.33 (0.33)	46.0 (31.0)	27.3 (21.7)
Greenbank		VVD	Through / Right	A (A)	0.75 (0.60)	50.5 (30.3)	60.2 (40.6)
Road at	Signalized	NB	Left	A (A)	0.11 (0.14)	12.5 (11.3)	5.6 (7.7)
Canfield Road		IND	Through / Right	A (C)	0.45 (0.63)	6.5 (10.3)	67.9 (84.7)
		SB	Left	C (D)	0.55 (0.59)	19.4 (31.0)	51.8 (42.0)
		SD	Through / Right	B (A)	0.57 (0.47)	8.0 (7.9)	95.2 (59.5)
		(	Overall Intersection	-	-	12.1 (13.1)	-
		EB	Left / Through	A (A)	0.00 (0.00)	0.2 (0.4)	0.0 (0.0)
Canfield Road	Minor Stop-	WB	Through / Right	A (A)	0.00 (0.00)	0.0 (0.0)	0.0 (0.0)
at Site Access	Control	SB	Left / Right	A (A)	0.03 (0.10)	9.3 (9.7)	0.7 (2.1)
		(	Overall Intersection	A (A)	-	1.6 (3.3)	-
	nat: PM (SUN) esents the anticipated	volume d	ivided by the predicted capacity				

#### **Multi-Modal Level of Service Assessment**

Based on the proximity of this intersection to the nearby Knoxdale Elementary School, it was determined that the intersection of Greenbank Road at Canfield Road falls under the 'within 300m of a school' Policy Area designation. As such, the intersection is subject to a Pedestrian Level of Service (PLOS) target of A.

The 2013 Ottawa Transportation Master Plan designates Greenbank Road as a spine cycling route and Canfield Road as a local cycling route in the Ultimate Cycling Network. As such, the Churchill Avenue at Richmond Road intersection has a bicycle level of service (BLOS) target of B.

Greenbank Road has transit routes along it, and therefore, the Transit Level of Service (TLOS) target for the intersection is D.

Greenbank Road is also designated as a truck route; however, Canfield Road is not. As such, trucks will not be turning at the intersection of Greenbank Road and Canfield Road, thus, the Truck Level of Service (TkLOS) does not apply to this intersection.



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**Table 15** summarizes the MMLOS at the intersection of Greenbank Road at Canfield Road under 2021 total future conditions.

Appendix C contains the detailed MMLOS analysis.

Table 15 – 2026 Ultimate Signalized Intersection MMLOS

Signalized	PL	os	BL	os	TL	os	TkL	.os
Intersection	Target	Actual	Target	Actual	Target	Actual	Target	Actual
Greenbank Road at Canfield Road	Α	E	В	F	D	F	N	/A

The Pedestrian Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a PLOS of E, which is below the desired target. The number of lanes required to cross Greenbank Road results in the PLOS operating below the target. Measures to improve the PLOS at this intersection include reducing the number of lanes on Greenbank Road, allocating more green time to the east / west movements for vehicles (and thus, pedestrians), implementing medians along Greenbank Road that are more than 2.4m wide, protecting the left turn phases, prohibiting the right turns on red, and raising the crosswalks.

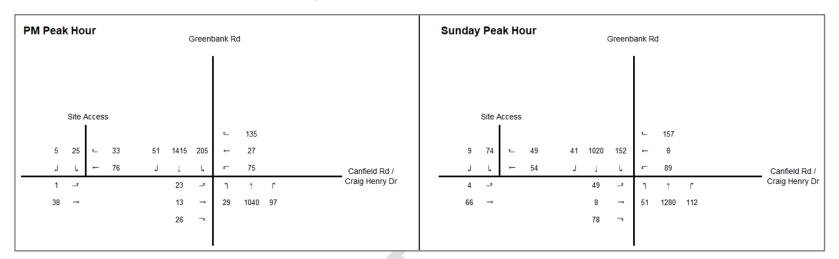
The Bicycle Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a BLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection BLOS is influenced by the availability of dedicated cycling amenities, number of lanes cyclists must cross to negotiate a turn at intersections, and roadway operating speeds. Due to the nature of arterial roads, the posted speed limit is typically 60 km/hr or greater, which limits the potential BLOS at intersections. Reducing the posted speed limit is not a feasible option for Greenbank Road. Implementing a higher order bicycle facility (i.e. curbside bicycle lanes with two-stage left turn bike boxes) would allow the BLOS to meet the target.

The Transit Level of Service at the intersection of Greenbank Road at Canfield Road is projected to operate with a TLOS of F, which is below the desired target. Based on the MMLOS guidelines, intersection transit level of service is governed by the delay at the intersection. The westbound approach experiences the highest delay, which governs the TLOS at the intersection. Allocating more green time to the east / west movements for vehicles would improve the delay on the westbound approach, however, it would be to the detriment of the northbound and southbound vehicles



Strategy Report December 17, 2019

Figure 14 - 2026 Ultimate Traffic Volumes





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### 5.0 SUMMARY AND CONCLUSIONS

This Transportation Impact Assessment (TIA) was prepared in support of a Zoning By-Law Amendment and Site Plan Control application for the proposed church expansion building on the St. Mary Coptic Church property, located at 1 Canfield Road. The site is located at the northwest quadrant of the Greenbank Road at Canfield Road intersection and is bound by Greenbank Road to the east, Canfield Road to the south, and existing residential houses to the west and north.

The subject site currently includes one access to Canfield Road, approximately 60m west of Greenbank Road. The proposed site plan includes shifting this access approximately 2.5m west to allow for improved on-site circulation. The proposed expansion building includes a gym / hall, a multimedia room, lounge, kitchen, study, studio, classrooms for Sunday school, a small chapel, as well as meeting / office space.

Due to the nature of the existing land use, the existing church currently generates the most amount of traffic on Sundays. The proposed expansion building will generally be used by the congregation before / after the Sunday service and it will be prohibited to have activities in this building during Sunday service; therefore, the proposed expansion building is not anticipated to generate any additional traffic above what is already being generated by the existing church on a typical Sunday. The proposed site (i.e. existing church plus expansion building) is anticipated to generate 21 two-way auto trips during the PM peak hour.

Both study area intersections are projected to operate acceptably under all study horizons.

The multi-modal level of service (MMLOS) assessment for segments found the following:

- Greenbank Road, fronting the proposed development, achieves a PLOS of E, which fails to meet the PLOS target of A. To meet the PLOS target along Greenbank Road, the sidewalk on the west side would need to be widened to 1.8m while maintaining a boulevard of greater than 2.0m coupled with a reduction in the speed limit to 40 km/hr and a reduction in the curb lane AADT to less than 3000. Although the pedestrian facilities are well-developed along Greenbank Road (i.e., sidewalks and boulevards along both sides), proximity to Knoxdale Elementary School results in an unattainably high PLOS target given the nature of Greenbank Road being an arterial roadway with high speeds and high volumes.
- Greenbank Road, fronting the subject site, currently achieves a Bicycle Level of service (BLOS) of F. To achieve the BLOS target of C, the speed limit would need to be reduced to 40km/hr while maintaining the existing mixed-use lanes. An alternative to achieving the BLOS target of D would be to implement curbside bicycle lanes along both sides of Greenbank Road, however, this could have spatial and financial constraints. A second alternative to achieving the BLOS target would be to implement physically separated cycling facilities (i.e. a cycle track) along both sides of Greenbank Road. Similar to implementing on-street bicycle lanes, this could also have a spatial and financial constraints.
- Greenbank Road, across the frontage of the subject site, currently meets both the Transit and Truck Level of Service (TLOS and TkLOS) targets.



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- While Canfield Road, across the frontage of the subject site, includes sidewalks along both sides, it currently
  does not meet the PLOS target of A. Implementing a 0.5m boulevard would allow the PLOS target of A to be
  met. Another alternative to meet the PLOS target would be to reduce the posted speed limit.
- Canfield Road, across the frontage of the subject site, currently meets the BLOS target.
- As Canfield Road is not a transit nor truck route, the TLOS and TkLOS targets do not apply.

The MMLOS assessment for the signalized intersection of Greenbank Road at Canfield Road found the following:

- The PLOS currently operates with a PLOS of E, which is below the desired target. Measures to improve the PLOS at this intersection include reducing the number of lanes on Greenbank Road, allocating more green time to the east / west movements for vehicles (and thus, pedestrians), implementing medians along Greenbank Road that are more than 2.4m wide, protecting the left turn phases, prohibiting the right turns on red, and raising the crosswalks. This PLOS is anticipated to remain consistent throughout all study horizons.
- The BLOS currently operates with a BLOS of F, which is also below target. Due to the nature of arterial roads, the posted speed limit is typically 60 km/hr or greater, which limits the potential BLOS at intersections. Reducing the posted speed limit is not a feasible option for Greenbank Road. Implementing a higher order bicycle facility (i.e. curbside bicycle lanes with two-stage left turn bike boxes) would allow the BLOS to meet the target. This BLOS is anticipated to remain consistent throughout all study horizons.
- The TLOS currently operates with a TLOS of F, which is also below the desired target. Based on the MMLOS guidelines, intersection transit level of service is governed by the delay at the intersection. The westbound approach experiences the highest delay, which governs the TLOS at the intersection. Allocating more green time to the east / west movements for vehicles would improve the delay on the westbound approach, however, it would be to the detriment of the northbound and southbound vehicles. This TLOS is anticipated to remain consistent throughout all study horizons.

Stantec conducted parking surveys on Sunday, April 29<sup>th</sup>, 2018 between 8:30 AM and 2:00 PM. The existing church was found to have a maximum of 85 vehicles parked on site. Information was provided to Stantec by the church that there are approximately an additional 10 vehicles who currently park on-street on Sundays, for a total demand of 95 parking spaces. The proposed site plan includes 99 parking spaces, which exceeds the demand of 95 parking spaces.

Despite the proposed parking supply exceeding the anticipated demand, the Church is pro-actively putting an agreement in place with the nearby Knoxdale Public School whereby members of the church can park in the school parking lot on Sundays as an alternative to parking on-street. This school is located approximately 160m from the church, or approximately a two-minute walk. There will be a sign on the church property indicating that once the church parking lot is full, motorists are to park in the Knoxdale parking lot and walk over. This should eliminate any neighbourhood concerns regarding parking along Canfield Road. As there will not be a pathway connecting the church to Parkmount Crescent, members of the church will likely not park along Parkmount Crescent as it would involve walking greater than 400m from their car to church, which is more than double the distance from the church to Knoxdale Public School.



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During the parking survey, it was found that the number of parked vehicles along Keppler Crescent, Banner Road, and Parkmount Crescent were consistent between 8:30~AM - 2:00~PM. This suggests that the parked cars along these three roads are attributed to the residents and not members of the St. Mary Coptic Church.

Based on the transportation evaluation presented in this study, the proposed site located at 1 Canfield Road can be supported and should be permitted to proceed from a transportation perspective.





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# Appendix A TRAFFIC COUNT DATA



Sun Apr 29, 2018

Full Length (7AM-2PM)

All Classes (Lights and Motorcycles, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517824, Location: 45.329734, -75.782399, Site Code: 37731103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA

Leg	West					East					North					
Dire ction	Eastbo	ound				Westbo	und				Southbo	und				
Time	I	. T	U	App	Pe d*	Т	R	U	App	Ped*	L	R	U	App	Pe d*	Int
2018-04-29 7:00 <i>P</i>	M (	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 <i>A</i>	M (	) 1	. 0	1	0	0	0	0	0	0	0	0	0	0	0	1
7:30 <i>A</i>	M (	) 3	0	3	0	0	1	0	1	0	0	0	0	0	0	4
7:45 <i>A</i>	M (	) 7	0	7	0	1	7	0	8	0	1	0	0	1	0	16
Hourly To	al (	) 11	. 0	11	0	1	8	0	9	0	1	0	0	1	0	21
8:00 <i>A</i>	M (	) 7	0	7	0	2	7	0	9	0	1	0	0	1	0	17
8:15 <i>A</i>	M (	) 6	0	6	0	4	7	0	11	0	0	0	0	0	0	17
8:30 <i>A</i>	M (	8	0	8	0	6	11	0	17	0	2	0	0	2	0	27
8:45 <i>A</i>	M :	. 9	0	10	0	3	11	0	14	1	3	0	0	3	1	27
Hourly To	al	30	0	31	0	15	36	0	51	1	6	0	0	6	1	88
9:00 <i>A</i>	M :	L 7	0	8	0	8	21	0	29	0	2	1	0	3	0	40
9:15 <i>A</i>	M (	16	0	16	0	2	18	0	20	0	0	0	0	0	0	36
9:30 <i>A</i>	M (	) 11	. 0	11	0	8	11	0	19	0	0	1	0	1	0	31
9:45 <i>A</i>	M (	) 11	. 0	11	0	13	8	0	21	3		4	0	5	0	37
Hourly To	al :	45	0	46	0	31	58	0	89	3	3	6	0	9	0	144
10:00 <i>A</i>	M (	16	0	16	0	9	8	0	17	0	4	1	0	5	3	38
10:15 <i>A</i>	M :	1 9	0	10	0	7	2	0	9	0	1	3	0	4	0	23
10:30 <i>A</i>	M (	16	0	16	0	10	2	0	12	0	0	0	0	0	0	28
10:45 <i>A</i>		l 13		14	0	13	3	0	16	0	1	2	0	3	0	33
Hourly To	al 2	54	. 0	56	0	39	15	0	54	0	6	6	0	12	3	122
11:00 <i>A</i>	M (	14	0	14	0	8	3	0	11	0	0	3	0	3	0	28
11:15 <i>A</i>	M :	11	. 0	12	1	8	2	1	11	16	18	0	0	18	0	41
11:30 <i>A</i>	M 2	! 14	0	16	0	15	7	0	22	0	22	2	0	24	0	62
11:45 <i>A</i>	M (	16	0	16	0	7	6	0	13	4	13	1	0	14	0	43
Hourly To	_	55		58	1	38	18	1	57	20		6	0	59	0	174
12:00F	M :	12	0	13	1	15	11	0	26	9	9	3	0	12	1	51
12:15F		15		15	2	9	13	0	22	11		1	0	13	0	50
12:30F	M (	16	0	16	0	8	0	0	8	2		0	0	7	0	31
12:45F		13		14	1	15	5	0	20	0	5	1		6	1	40
Hourly To				58	4	47	29	0	76	22	33	5	0	38	2	172
1:00F	_			16	0	10	6	0	16	3		0	0	4	0	36
1:15F		21		22	0	11	4	0	15	2		2	0	15	0	52
1:30F	_			21	0	14	3		17	3		0	0	12	0	50
1:45F				3	0	7	3	0	10	0		1		12	0	25
Hourly To	al [	61	. 0	62	0	42	16	0	58	8	40	3	0	43	0	163
To	<b>al</b> 10	312	0	322	5	213	180	1	394	54	142	26	0	168	6	884
% Approa	<b>h</b> 3.1%	96.9%	0%	-	-	54.1%	45.7%	0.3%	-	-	84.5%	15.5%	0%	-	-	-
% То				36.4 %	-	24.1%			44.6%	-	16.1%	2.9%		19.0%	-	-
Lights and Motorcycl	_			316	-	209	179	1		-	141	26	0	167	-	872
% Lights and Motorcycl				98.1%	-	98.1%			98.7%	-	99.3%	100%		99.4 %	-	98.6%
He a	-			0	-	0	1	0	1	-	1	0		1	-	2
% He a	-		0%	0 %	-	0%	0.6%	0%	0.3%	-	0.7%		0%	0.6%	-	0.2%
Bicycles on Ro	_			6	-	4	0	0	4	-	0	0	0	0	-	10
% Bicycles on Ro	_	1.9%	0%	1.9 %	-	1.9%	0%	0%	1.0 %		0%	0%	0%	0 %	-	1.1%
Pe de stria	_			-	5	-	-	-		54	-	-	-		6	
% Pedestria	_			-	100%	-	-	-		100%	-	-	-		100%	-
Bicycles on Crosswa				-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswa	lk		-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-

<sup>\*</sup>Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sun Apr 29, 2018

Full Length (7AM-2PM)

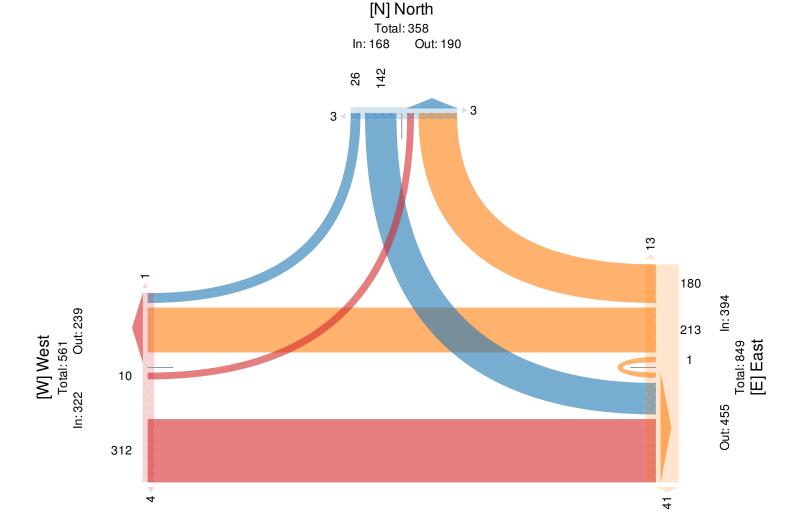
All Classes (Lights and Motorcycles, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517824, Location: 45.329734, -75.782399, Site Code: 37731103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA



Sun Apr 29, 2018

AM Peak (WKND) (10:45AM - 11:45AM)

All Classes (Lights and Motorcycles, Heavy, Pedestrians, Bicycles on Road,

Bicycles on Crosswalk)

All Movements

ID: 517824, Location: 45.329734, -75.782399, Site Code: 37731103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA

Le g	West					East					North					
Dire ction	Eastbou	ınd				We s tbo	ınd				Southbo	und				
Time	L	T	U	App	Pe d*	Т	R	U	App	Pe d*	L	R	U	App	Ped*	Int
2018-04-29 10:45AM	1	13	0	14	0	13	3	0	16	0	1	2	0	3	0	33
11:00AM	0	14	0	14	0	8	3	0	11	0	0	3	0	3	0	28
11:15 AM	1	11	0	12	1	8	2	1	11	16	18	0	0	18	0	41
11:30AM	2	14	0	16	0	15	7	0	22	0	22	2	0	24	0	62
Total	4	52	0	56	1	44	15	1	60	16	41	7	0	48	0	164
% Approach	7.1%	92.9%	0%	-	-	73.3%	25.0%	1.7%	-	-	85.4%	14.6%	0%	-	-	-
% Total	2.4%	31.7%	0%	34.1%	-	26.8%	9.1%	0.6%	36.6%	-	25.0%	4.3%	0%	29.3%	-	-
PHF	0.500	0.929	-	0.875	-	0.733	0.536	0.250	0.682	-	0.466	0.583	-	0.500	-	0.661
Lights and Motorcycles	4	50	0	54	-	43	15	1	59	-	41	7	0	48	-	161
% Lights and Motorcycles	100%	96.2%	0%	96.4 %	-	97.7%	100%	100%	98.3%	-	100%	100%	0%	100%	-	98.2%
He a vy	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% He avy	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%
Bicycles on Road	0	2	0	2	-	1	0	0	1	-	0	0	0	0	-	3
% Bicycles on Road	0%	3.8%	0%	3.6%	-	2.3%	0%	0%	1.7 %	-	0%	0%	0%	0%	-	1.8%
Pe de strians	-	-	-	-	1	-	-	-	-	16	-	-	-	-	0	
% Pedestrians	-	-	-	-	100%	-	-	-	-	100%	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	0%	-	-	-	-	0%	-	-	-	-	-	-

<sup>\*</sup>Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sun Apr 29, 2018

AM Peak (WKND) (10:45AM - 11:45AM)

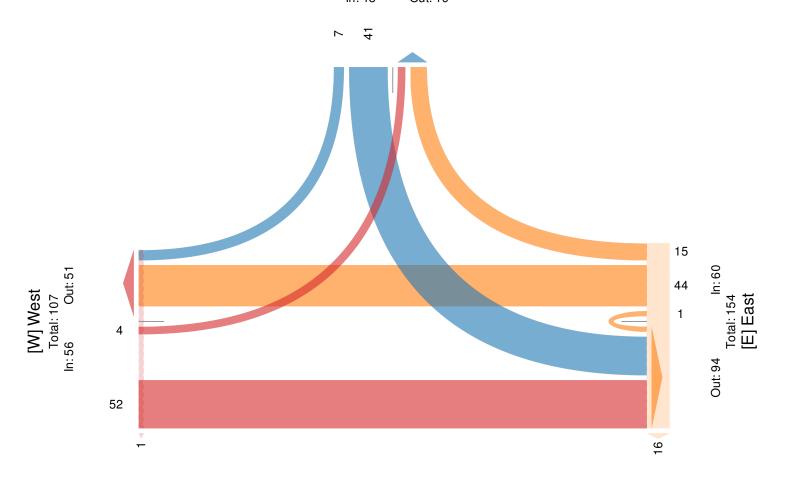
All Classes (Lights and Motorcycles, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517824, Location: 45.329734, -75.782399, Site Code: 37731103







Sun Apr 29, 2018

Midday Peak (WKND) (11:30AM - 12:30PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy, Pedestrians, Bicycles on Road,

Bicycles on Crosswalk)

All Movements

ID: 517824, Location: 45.329734, -75.782399, Site Code: 37731103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA

Leg	West					East					North					
Dire ction	Eastbou	ınd				Westbou	ınd				Southbo	und				
Time	L	T	U	App	Pe d*	T	R	U	App	Pe d*	L	R	U	App	Pe d*	Int
2018-04-29 11:30AM	2	14	0	16	0	15	7	0	22	0	22	2	0	24	0	62
11:45AM	0	16	0	16	0	7	6	0	13	4	13	1	0	14	0	43
12:00PM	1	12	0	13	1	15	11	0	26	9	9	3	0	12	1	51
12:15PM	0	15	0	15	2	9	13	0	22	11	12	1	0	13	0	50
Total	3	57	0	60	3	46	37	0	83	24	56	7	0	63	1	206
% Approach	5.0%	95.0%	0%	-	-	55.4%	44.6%	0%	-	-	88.9%	11.1%	0%	-	-	-
% Total	1.5%	27.7%	0%	29.1%	-	22.3%	18.0%	0%	40.3%	-	27.2%	3.4%	0%	30.6%	-	-
PHF	0.375	0.891	-	0.938	-	0.767	0.712	-	0.798	-	0.636	0.583	-	0.656	-	0.831
Lights and Motorcycles	3	57	0	60	-	44	37	0	81	-	56	7	0	63	-	204
% Lights and Motorcycles	100%	100%	0%	100%	-	95.7%	100%	0%	97.6%	-	100%	100%	0%	100%	-	99.0%
He a vy	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% He avy	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0%
Bicycles on Road	0	0	0	0	-	2	0	0	2	-	0	0	0	0	-	2
% Bicycles on Road	0%	0%	0%	0 %	-	4.3%	0%	0%	2.4 %	-	0%	0%	0%	0 %	-	1.0%
Pedestrians	-	-	-	-	3	-	-	-	-	24	-	-	-	-	1	
% Pedestrians	-	-	-	-	100%	-	-	-	-	100%	-	-	-	-	100%	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-

<sup>\*</sup>Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sun Apr 29, 2018

Midday Peak (WKND) (11:30AM - 12:30PM) - Overall Peak Hour

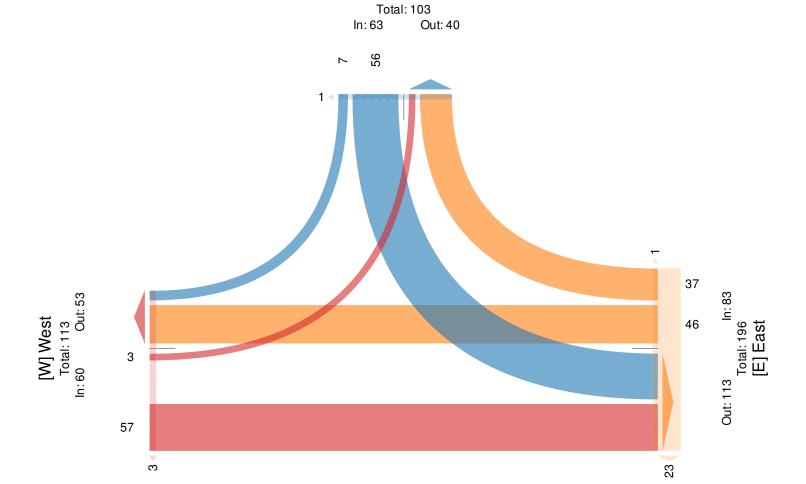
All Classes (Lights and Motorcycles, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517824, Location: 45.329734, -75.782399, Site Code: 37731103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA



[N] North

5363811 - Greenbank and Canfield/Craig Henry - Apr 29th - TMC
Sun Apr 29, 2018
Full Length (7AM-2PM)
All Classes (Lights and Motorcycles, Lights on Crosswalk and Motorcycles on Crosswalk, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements
ID: 517820, Location: 45.329982, -75.781649, Site Code: 37729103



Leg	West					East						South						North						
Direction	Eastbou	ınd				Westbo	und					Northbo	und					Southbo	und					
Time	L	T	R	U App	Ped*	L	T	R	U	App	Ped*	L	T	R	U	App	Pe d*	L	T	R	U	App	Ped*	Int
2018-04-29 7:00AM	0	0	0	0 0	0	1	0	0	0	1	2	0	34	1	0	35	0	1	31	0	0	32	1	68
7:15AM	0	0	0	0 0	) 2	2	0	8	0	10	0	0	39	0	0	39	0	3	31	0	0	34	0	83
7:30AM	1 2	0	1	0 3	0	2	0	9	0	11	0	1	60	3	0	64	0	3	42	0	0	45	0	123
7:45AM	1 4	0	3	0 7	, 0	4	0	19	1	24	1	1	59	6	0	66	2	5	64	3	0	72	0	169
Hourly Total	1 6	0	4	0 10	) 2	9	0	36	1	46	3	2	192	10	0	204	2	12	168	3	0	183	1	443
8:00AM	1 2	2		0 5	0	3	1	3	0	7	0	3	51	3	0	57	0	3	58	4	0	65	0	134
8:15AM	1 2	1	4	0 7	2	6	1	9	0	16	0	6	44	1	0	51	0	5	88	3	0	96	0	170
8:30AM	1 2	2		0 7	1 1	10	0	20	0	30	1	9	84	3	0	96	0	1	108	6	0	115	0	248
8:45AM	7	1	6	0 14	. 5	10	1	14	0	25	2	8	91	8	0	107	2	9	140	7	0	156	2	302
Hourly Total	1 13	6		0 33			3	46	0	78	3	26	270	15	0	311	2	18	394	20	0	432	2	854
9:00 AM	1 1	3		0 6		10	2	22	0	34	2	13	116	3	0	132	1	9	111	10	0	130	0	302
	_					_					- 4	_					- 1	_				122	- 1	_
9:15AM	12	1		0 16			2	21	0	32	1	16	116	8	0	140	0	10	106	5	1		1	310
9:30AM	9	1		0 15			2	29	0	43	2	14	152	9	0	175	2	8	119	8	0	135	1	368
9:45AM	1 3	1		0 9		8	2	30	0	40	1	14	171	9	0	194	1	10	161	5	0	176	2	4 19
Hourly Total	1 25	6	15				8	102	0	149	6	57	555	29	0	641	4		497	28	1	563	4	1399
10:00AM	14	1		0 23			0	30	0	39	0	9	153	10	0	172	3	16	146	4	0	166	0	400
10:15AM	6	0	6	0 12		. 10	3	32	0	45	2	7	189	5	0	201	1	13	153	4	0	170	1	428
10:30AM	7	1	5	0 13	1	. 16	1	25	0	42	1	7	238	10	0	255	0	10	180	4	1	195	0	505
10:45AM	1 4	2	7	0 13	2	16	1	34	0	51	1	3	258	14	1	276	4	21	192	12	0	225	0	565
Hourly Total	31	4	26	0 61	L 7	51	5	121	0	177	4	26	838	39	1	904	8	60	671	24	1	756	1	1898
11:00AM	1 4	1	8	0 13	1	. 9	1	23	0	33	0	6	194	11	0	211	3	21	204	4	1	230	0	487
11:15AM	1 7	0	11	0 18	1 4	10	3	34	0	47	0	4	192	9	0	205	1	26	211	5	0	242	0	512
11:30AM	1 14	2		0 48	1 2	_	2	24	0	36	1	8	178	12	0	198	0	15	200	6	0	221	0	503
11:45AM	1 8	2		0 28		. 12	1	25	0	38	0	10	224	14	0	248	0	_	234	10	1	269	0	583
Hourly Total	1 33	5		0 107		_	7	106	0		1	28	788	46	0	862	4		849	25	2	962	0	2085
12:00PM	1 11	1		0 26		12	1	26	0	39	3	10	211	11	0	232	2	30	211	9	0	250	1	547
12:15PM	11	3		0 24		10	0	30	0	40	1	16	214	21	0	251	- 0	25	219	6	0	250	0	565
12:30PM	1 13	1		0 27		. 11	1	22	0	34	2	9	247	18	0	274	0	25	248	4	0	277	0	612
12:45PM	_	2		0 21		_	1	24	0	42	3	7	312	25	1	345	4	_	260	6	0	294	2	702
						_		102		155		42				1102						1071	2	2426
Hourly Total	43	7					3		0		9		984	75	1		6		938	25	0		3	
1:00PM	10	0		0 18		27	0	42	0	69	0	8	280	30	0	318	0	31	203	11	0	245	- 2	650
1:15PM	11	1		0 29			3	37	0	55	1	10	251	23	0	284	2	32	197	8	0	237	0	605
1:30PM	1 9	3		0 38		. 17	2	32	0	51	1	7	258	18	0	283	2	39	217	1	0	257	2	629
1:45PM	5	0		0 14			1	22	0	40	5	4	244	23	0	271	0		207	10	1	244	0	569
Hourly Total	35	4	60	0 99	7	76	6	133	0	215	7	29	1033	94	0	1156	4	128	824	30	1	983	4	2453
Tota	186	32	236	0 454	58	295	32	646	1	974	33	210	4660	308	2	5180	30	449	4341	155	5	4950	15	11558
% Approach	41.0%	7.0%	52.0% 0	%	-	30.3%	3.3%	66.3%	0.1%	-	-	4.1% 9	90.0%	5.9%	0%	-	-	9.1% 8	7.7%	3.1%	0.1%	-	-	-
% Total	1.6%	0.3%	2.0% 0	% 3.9%	, .	2.6%	0.3%	5.6%	0%	8.4%	-	1.8%	40.3%	2.7%	0% 4	4.8%	-	3.9% 3	7.6%	1.3%	0% 4	12.8%	-	-
Lights and Motorcycles	183	31	236	0 450	) .	287	31	640	1	959	-	209	4647	297	2	5155	-	447	4323	154	5	4929	-	11493
% Lights and Motorcycles	98.4%	96.9%	100% 0	% 99.1%		97.3%	96.9%	99.1%	100% 9	98.5%	-	99.5%	99.7% 9	96.4% 1	00% 9	99.5%		99.6% 9	9.6% 9	99.4% 1	00% 9	99.6%		99.4%
Heav	/ 1	0		0 1		. 8	0	6	0	14		0	13	11	0	24	_	2	18	1	0	21	_	60
% He av	0.5%	0%	0% 0			2.7%	0%	0.9%	0%	1.4 %		0%	0.3%	3.6%	0%	0.5%	_	0.4%		0.6%	0%	0.4%	_	0.5%
Bicycles on Road	_	1		0 3		0	1	0.570	0	1		1	0.570	0	0	1		0.170	0.170	0.070	0	0	$\rightarrow$	5
% Bicycles on Road		3.1%	0% 0			0%	3.1%	0%		0.1%		0.5%	0%	0%	0%	0%		0%	0%	0%	0%	0%	-	0%
	1.1/0	3.1/0	0 /0 0	, U. / 70	, .	0 70	3.1/0	U /0	0 /0	V.1 /0	- 1	0.570	U /0	0 /0	0 /0	0 /0	-	0 70	U /U	0 /0	U /U	0 /0	-	U /0
Lights on Crosswalk and Motorcycles on Crosswalk	-			-	- 0%	1 -					2 00′	-					0%	-					0%	
% Lights on Crosswalk and Motorcycles on Crosswalk	`					-		-	-		3.0%			-		-						-	$\rightarrow$	_
Pedestrians	-	-	-		- 54	-	-	-	-	-	29	-	-	-	-	-	23	-	-	-	-	-	13	
% Pedestrians	-	-	-		- 93.1%	-	-	-	-		7.9%	-	-	-	-		6.7%	-	-	-	-		5.7%	-
Bicycles on Crosswall	- 1	-	-	-	- 4	-	-	-	-	-	3	-	-	-	-	-	7	-	-	-	-	-	2	
% Bicycles on Crosswall	-	-	-	-	- 6.9%	-	-	-	-	-	9.1%	-	-	-	-	- 2	3.3%	-	-	-	-	- 13	3.3%	-

<sup>\*</sup>Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sun Apr 29, 2018

Full Length (7AM-2PM)

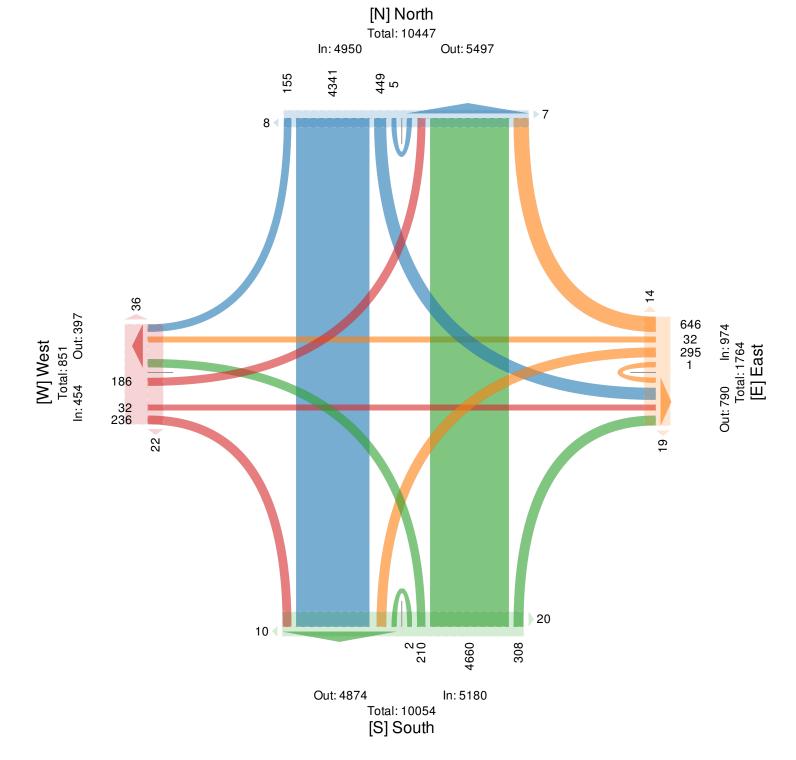
All Classes (Lights and Motorcycles, Lights on Crosswalk and Motorcycles on Crosswalk, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517820, Location: 45.329982, -75.781649, Site Code: 37729103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA



Sun Apr 29, 2018

AM Peak (WKND) (10:30AM - 11:30AM)

All Classes (Lights and Motorcycles, Lights on Crosswalk and Motorcycles on Crosswalk, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements

ID: 517820, Location: 45.329982, -75.781649, Site Code: 37729103



Leg	West						East						South						North						
Direction	Eastbo	ınd					Westbo	und					Northb	ound					Southbo	ound					
Time	L	T	R	U	App	Pe d*	L	T	R	U	App	Ped*	L	T	R	U	App	Pe d*	L	T	R	U	App 1	Ped*	Int
2018-04-29 10:30AM	7	1	5	0	13	1	16	1	25	0	42	1	7	238	10	0	255	0	10	180	4	1	195	0	505
10:45AM	4	2	7	0	13	2	16	1	34	0	51	1	3	258	14	1	276	4	21	192	12	0	225	0	565
11:00AM	4	1	8	0	13	1	9	1	23	0	33	0	6	194	11	0	211	3	21	204	4	1	230	0	487
11:15AM	7	0	11	0	18	4	10	3	34	0	47	0	4	192	9	0	205	1	26	211	5	0	242	0	512
Total	22	4	31	0	57	8	51	6	116	0	173	2	20	882	44	1	947	8	78	787	25	2	892	0	2069
% Approach	38.6%	7.0%	54.4% (	)%	-	-	29.5%	3.5%	67.1% (	0%	-	-	2.1%	93.1%	4.6%	0.1%	-	-	8.7%	88.2%	2.8%	0.2%	-		-
% Total	1.1%	0.2%	1.5% (	)% 2	2.8%	-	2.5%	0.3%	5.6% (	0%	8.4 %	-	1.0%	42.6%	2.1%	0% -	45.8%	-	3.8%	38.0%	1.2%	0.1%	43.1%		-
PHF	0.786	0.500	0.705	- 0	.792	-	0.797	0.500	0.853	-	0.848	-	0.714	0.855	0.786	0.250	0.858	-	0.750	0.932	0.521	0.500	0.921		0.915
Lights and Motorcycles	20	4	31	0	55	-	50	6	116	0	172	-	20	881	42	1	944	-	77	785	25	2	889	-	2060
% Lights and Motorcycles	90.9%	100%	100% (	)% <b>96</b>	6.5%	-	98.0%	100%	100% (	0% 9	9.4 %	-	100%	99.9%	95.5%	100%	99.7%	-	98.7%	99.7%	100%	100%	99.7%		99.6%
He a vy	0	0	0	0	0	-	1	0	0	0	1	-	0	1	2	0	3	-	1	2	0	0	3	-	7
% He a vy	0%	0%	0% (	)%	0%	-	2.0%	0%	0% (	0%	0.6%	-	0%	0.1%	4.5%	0%	0.3%	-	1.3%	0.3%	0%	0%	0.3%		0.3%
Bicycles on Road	2	0	0	0	2	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0		2
% Bicycles on Road	9.1%	0%	0% (	)% 3	3.5%	-	0%	0%	0% (	0%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0 %		0.1%
Lights on Crosswalk and Motorcycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Lights on Crosswalk and Motorcycles on Crosswalk	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-		-
Pedestrians	-	-	-	-	-	6	-	-	-	-	-	2	-	-	-	-	-	8	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	- 1	75.0%	-	-	-	-	-	100%	-	-	-	-	- 1	00%	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	- :	25.0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	-	-

<sup>\*</sup>Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sun Apr 29, 2018

AM Peak (WKND) (10:30AM - 11:30AM)

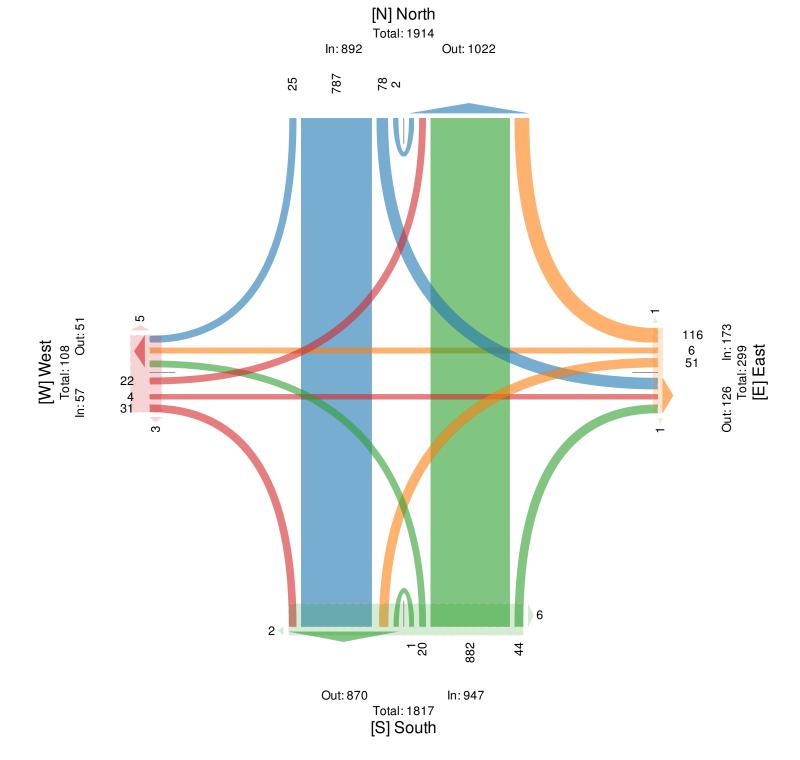
All Classes (Lights and Motorcycles, Lights on Crosswalk and Motorcycles on Crosswalk, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517820, Location: 45.329982, -75.781649, Site Code: 37729103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA



Sun Apr 29, 2018

Suit Apr 29, 2016
Midday Peak (WKND) (12:45PM - 1:45PM) - Overall Peak Hour
All Classes (Lights and Motorcycles, Lights on Crosswalk and Motorcycles on Crosswalk, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements

ID: 517820, Location: 45.329982, -75.781649, Site Code: 37729103



Leg	West						East						South						North						
Direction	Eastbo	und					Westbo	und					Northl	bound					Southb	ound					
Time	L	T	R	U	App	Pe d*	L	T	R	U	App	Pe d*	L	T	R	U	App	Ped*	L	T	R	U	App	Ped*	Int
2018-04-29 12:45PM	8	2	11	0	21	5	17	1	24	0	42	3	7	312	25	1	345	4	28	260	6	0	294	2	702
1:00PM	10	0	8	0	18	2	27	0	42	0	69	0	8	280	30	0	318	0	31	203	11	0	245	2	650
1:15PM	11	1	17	0	29	0	15	3	37	0	55	1	10	251	23	0	284	2	32	197	8	0	237	0	605
1:30PM	9	3	26	0	38	1	17	2	32	0	51	1	7	258	18	0	283	2	39	217	1	0	257	2	629
Total	38	6	62	0	106	8	76	6	135	0	217	5	32	1101	96	1	1230	8	130	877	26	0	1033	6	2586
% Approach	35.8%	5.7%	58.5% (	)%	-	-	35.0%	2.8%	62.2%	0%	-	-	2.6%	89.5%	7.8%	0.1%	-	-	12.6%	84.9%	2.5%	0%	-	-	-
% Total	1.5%	0.2%	2.4% (	)% -	4.1%	-	2.9%	0.2%	5.2%	0%	8.4 %	-	1.2%	42.6%	3.7%	0% 4	17.6%	-	5.0%	33.9%	1.0%	0%:	39.9%	-	-
PHF	0.864	0.500	0.596	- (	0.697	-	0.704	0.500	0.804	-	0.786	-	0.800	0.882	0.800	0.250	0.891	-	0.833	0.843	0.591	-	0.878	-	0.921
Lights and Motorcycles	38	6	62	0	106	-	74	6	134	0	214	-	32	1096	94	1	1223	-	130	872	26	0	1028	-	2571
% Lights and Motorcycles	100%	100%	100% (	)% 1	100%	-	97.4%	100%	99.3%	0% 9	98.6%	-	100%	99.5%	97.9%	100% 9	99.4 %	-	100%	99.4%	100%	0% :	99.5%	-	99.4%
He avy	0	0	0	0	0	-	2	0	1	0	3	-	0	5	2	0	7	-	0	5	0	0	5	-	15
% He avy	0%	0%	0% (	)%	0%	-	2.6%	0%	0.7%	0%	1.4 %	-	0%	0.5%	2.1%	0%	0.6%	-	0%	0.6%	0%	0%	0.5%	-	0.6%
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0% (	)%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%
Lights on Crosswalk and Motorcycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Lights on Crosswalk and Motorcycles on Crosswalk	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-
Pe de strians	-	-	-	-	-	8	-	-	-	-	-	4	-	-	-	-	-	7	-	-	-	-	-	4	
% Pedestrians	-	-	-	-	- 1	100%	-	-	-	-	- 8	30.0%	-	-	-	-	- 8	37.5%	-	-	-	-	-	66.7%	-
Bicycles on Crosswalk	-	-	_	-	-	0	-	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	2	
% Bicycles on Crosswalk	-	-	_	-	-	0%	-	-	-	-	- 2	20.0%	-	-	-	-	- :	12.5%	-	-	-	-	- 1	33.3%	-

<sup>\*</sup>Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sun Apr 29, 2018

Midday Peak (WKND) (12:45PM - 1:45PM) - Overall Peak Hour

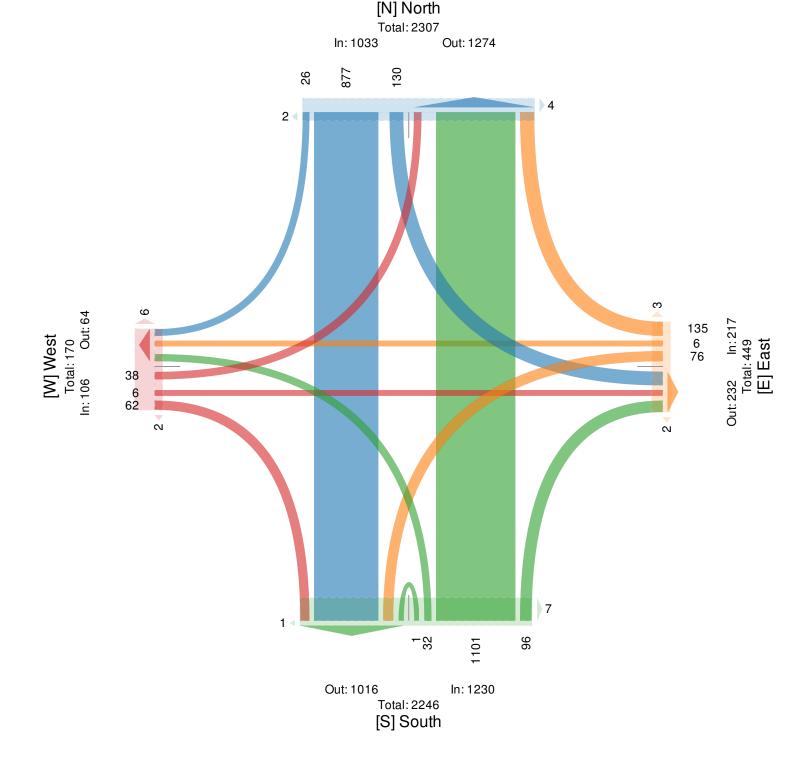
All Classes (Lights and Motorcycles, Lights on Crosswalk and Motorcycles on Crosswalk, Heavy, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 517820, Location: 45.329982, -75.781649, Site Code: 37729103



Provided by: City of Ottawa 100 Constellation Dr, Nepean, ON, K2G 5J9, CA





W.O.

35823

### **Turning Movement Count - 15 Minute Summary Report**

### **GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR**

Survey Date: Wednesday, March 30, 2016

**Total Observed U-Turns** 

Northbound: 2 Southbound: 9 Eastbound: 0 Westbound: 0

#### **GREENBANK RD**

#### **CANFIELD RD/CRAIG HENRY DR**

			'	GKEE	INDAN	IN KL	,				CAN	IFIELI	יטא ט	CKAI	3 HEN	ואו ט	'K			
		١	Northbo	und		So	uthboun	ıd			Eas	stbound	I		Wes	stbound	b			
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	3	219	3	225	10	129	3	142	367	4	0	6	10	8	0	27	35	45	412
07:15	07:30	2	295	3	300	7	148	2	157	457	10	2	12	24	10	2	31	43	67	524
07:30	07:45	2	325	7	334	12	176	3	191	525	8	3	6	17	11	1	55	67	84	609
07:45	08:00	1	340	17	358	16	220	5	241	599	20	1	11	32	29	2	44	75	107	706
08:00	08:15	4	360	12	376	18	178	0	196	572	17	5	10	32	12	1	46	59	91	663
08:15	08:30	4	344	14	362	21	155	1	178	540	11	3	8	22	8	2	54	64	86	626
08:30	08:45	2	290	13	305	22	158	4	184	489	15	4	7	26	21	2	37	60	86	575
08:45	09:00	3	253	15	271	18	183	4	206	477	12	2	7	21	8	0	45	53	74	551
09:00	09:15	1	210	15	227	23	143	2	168	395	12	0	4	16	11	1	32	44	60	455
09:15	09:30	1	202	6	209	19	150	4	173	382	7	2	7	16	11	0	34	45	61	443
09:30	09:45	3	190	6	199	16	130	1	147	346	10	0	5	15	6	1	42	49	64	410
09:45	10:00	4	175	9	188	10	161	2	174	362	4	1	5	10	15	0	31	46	56	418
11:30	11:45	6	171	4	181	22	158	7	188	369	8	1	3	12	4	1	29	34	46	415
11:45	12:00	7	186	11	204	25	201	7	234	438	4	0	1	5	9	1	17	27	32	470
12:00	12:15	7	192	20	219	29	206	5	240	459	4	0	5	9	12	1	27	40	49	508
12:15	12:30	5	208	18	231	22	206	7	235	466	6	1	6	13	11	1	19	31	44	510
12:30	12:45	3	227	7	237	17	181	9	207	444	5	2	4	11	5	1	29	35	46	490
12:45	13:00	8	213	12	234	14	184	4	203	437	4	1	3	8	18	2	32	52	60	497
13:00	13:15	4	177	11	192	23	182	1	206	398	13	3	12	28	10	1	25	36	64	462
13:15	13:30	1	180	21	202	23	173	7	203	405	4	2	3	9	12	1	20	33	42	447
15:00	15:15	1	221	16	238	30	274	6	310	548	8	1	4	13	17	0	29	46	59	607
15:15	15:30	9	205	18	232	42	329	8	379	611	5	3	4	12	12	2	24	38	50	661
15:30	15:45	5	197	19	221	37	245	9	293	514	6	2	5	13	10	3	33	46	59	573
15:45	16:00		229	13	245	34	300	12	346	591	3	3	6	12	13	5	26	44	56	647
16:00			222	26	255	38	300	7	346	601	6	1	6	13	18	1	31	50	63	664
16:15			204	25	233	55	285	7	347	580	2	2	0	4	16	2	29	47	51	631
16:30			205	18	229	43	286	9	338	567	1	3	3	7	17	11	31	59	66	633
16:45			191	24	221	40	293	13	346	567	5	4	5	14	12	2	38	52	66	633
17:00			205	27	240	59	283	15	357	597	10	2	5	17	18	2	30	50	67	664
17:15			188	25	217	58	262	5	325	542	1	1	8	10	22	5	43	70 57	80	622
17:30			187	20	216	47	283	10	340	556	8	2	7	17	19	3	35	57	74 50	630
17:45			210	16	230	53	284	9	346	576	5	0	4	9	12	3	34	49	58	634
TOTAL	_:	137	7221	471	7831	903	6846	188	7946	15777	238	57	182	477	417	60	105	9 <b>15</b> 3	86 2013	17790

Note: U-Turns are included in Totals.

Comment:



# **Turning Movement Count - Cyclist Volume Report**

Work Order 35823

Start Time: 07:00

### **GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR**

Count Date: Wednesday, March 30, 2016

**GREENBANK RD** 

#### **CANFIELD RD/CRAIG HENRY DR**

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	0	0	0	2	0	2	2
08:00 09:00	0	0	0	2	0	2	2
09:00 10:00	0	0	0	0	0	0	0
11:30 12:30	0	0	0	0	0	0	0
12:30 13:30	0	0	0	0	0	0	0
15:00 16:00	1	0	1	0	1	1	2
16:00 17:00	0	0	0	0	1	1	1
17:00 18:00	1	1	2	0	1	1	3
Total	2	1	3	4	3	7	10

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.

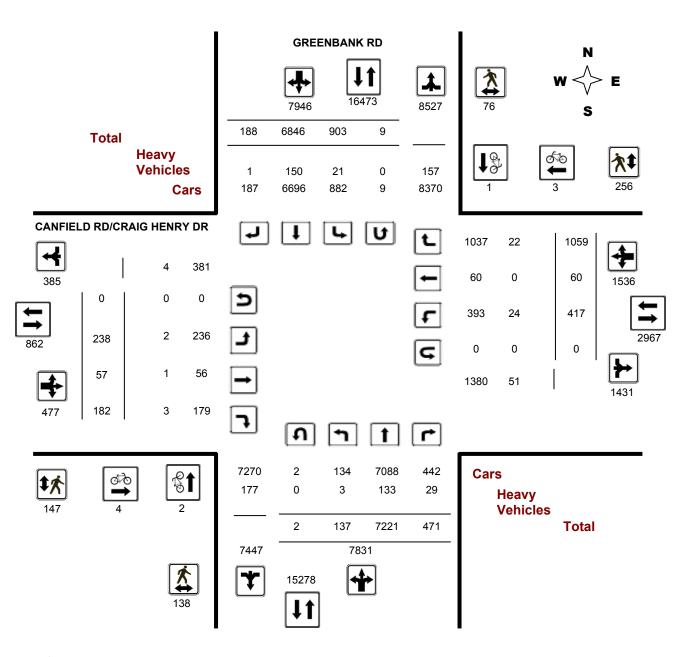


**Turning Movement Count - Full Study Diagram** 

# GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Survey Date: Wednesday, March 30, 2016 WO#: 35823

**Device:** Miovision



Comments



W.O.

35823

### **Turning Movement Count - Heavy Vehicle Report**

# GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Survey Date: Wednesday, March 30, 2016

#### GREENBANK RD CANFIELD RD/CRAIG HENRY DR

		Northb	ound		(	Southb	ound	_			Eastb	ound		١	Westbo	ound				
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	1	21	6	28	7	23	0	30	58	0	0	0	0	4	0	1	5	5	63
08:00	09:00	1	20	5	26	5	22	0	27	53	1	0	1	2	2	0	2	4	6	59
09:00	10:00	0	15	2	17	4	30	0	34	51	1	0	0	1	4	0	3	7	8	59
11:30	12:30	1	16	4	21	1	16	1	18	39	0	0	0	0	2	0	0	2	2	41
12:30	13:30	0	22	4	26	1	22	0	23	49	0	0	0	0	3	0	2	5	5	54
15:00	16:00	0	13	4	17	0	16	0	16	33	0	1	1	2	3	0	2	5	7	40
16:00	17:00	0	10	2	12	1	15	0	16	28	0	0	1	1	2	0	6	8	9	37
17:00	18:00	0	16	2	18	2	6	0	8	26	0	0	0	0	4	0	6	10	10	36
Sub 1	Γotal	3	133	29	165	21	150	1	172	337	2	1	3	6	24	0	22	46	52	389
U-Turn	s (Heav	y Vel	nicles)		0				0	0				0				0	0	0
Tot	al	3	133	29	0	21	150	1	172	337	2	1	3	6	24	0	22	46	52	389

Heavy Vehicles include Buses, Single-Unit Trucks and Articulated Trucks. Further, they ARE included in the Turning Movement Count Summary.



Work Order 35823

### **Turning Movement Count - Pedestrian Volume Report**

### GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Count Dat	e: Wednesday,	March 30, 2016				Start Time:	07:00
Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	1	5	6	0	1	1	7
07:15 07:30	0	0	0	2	4	6	6
07:30 07:45	3	2	5	4	1	5	10
07:45 08:00	24	6	30	16	5	21	51
07:00 08:00	28	13	41	22	11	33	74
08:00 08:15	6	1	7	3	2	5	12
08:15 08:30	2	2	4	5	3	8	12
08:30 08:45	5	11	16	7	5	12	28
08:45 09:00	0	12	12	7	6	13	25
08:00 09:00	13	26	39	22	16	38	77
09:00 09:15	2	0	2	0	2	2	4
09:15 09:30	0	0	0	0	1	1	1
09:30 09:45	1	1	2	2	3	5	7
09:45 10:00	1	1	2	0	3	3	5
09:00 10:00	4	2	6	2	9	11	17
11:30 11:45	2	1	3	1	5	6	9
11:45 12:00	8	4	12	8	30	38	50
12:00 12:15	3	0	3	3	21	24	27
12:15 12:30	3	8	11	6	21	27	38
11:30 12:30	16	13	29	18	77	95	124
12:30 12:45	0	1	1	2	5	7	8
12:45 13:00	2	0	2	2	7	9	11
13:00 13:15	3	0	3	6	4	10	13
13:15 13:30	1	1	2	1	3	4	6
12:30 13:30	6	2	8	11	19	30	38
15:00 15:15	2	5	7	3	5	8	15
15:15 15:30	29	4	33	6	67	73	106
15:30 15:45	10	1	11	6	13	19	30
15:45 16:00	4	1	5	8	4	12	17
15:00 16:00	45	11	56	23	89	112	168
16:00 16:15	3	0	3	12	2	14	17
16:15 16:30	1	1	2	5	2	7	9
16:30 16:45	6	1	7	2	5	7	14
16:45 17:00	3	2	5	12	4	16	21
16:00 17:00	13	4	17	31	13	44	61
17:00 17:15	5	3	8	4	6	10	18
17:15 17:30	5	0	5	4	6	10	15
17:30 17:45	3	2	5	7	5	12	17
17:45 18:00	0	0	0	3	5	8	8
17:00 18:00	13	5	18	18	22	40	58
Total	138	76	214	147	256	403	617

Comment:



**Work Order** 

35823

### **Turning Movement Count - Full Study Summary Report**

### **GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR**

Survey Date: Wednesday, March 30, 2016

#### **Total Observed U-Turns**

**AADT Factor** 

1.00

Northbound: 2

Eastbound:

Southbound: 9

Westbound: 0

Full Study

								F	Full St	udy									
			GR	EENB	ANK F	RD					CANF	IELD	RD/CF	AIG H	IENRY	/ DR			
_		Northb	ound		,	Southb	ound		_		Eastbo	ound		,	Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	8	1179	30	1217	45	673	13	731	1948	42	6	35	83	58	5	157	220	303	2251
08:00 09:00	13	1247	54	1314	79	674	9	762	2076	55	14	32	101	49	5	182	236	337	2413
09:00 10:00	9	777	36	822	68	584	9	661	1483	33	3	21	57	43	2	139	184	241	1724
11:30 12:30	25	757	53	835	98	771	26	895	1730	22	2	15	39	36	4	92	132	171	1901
12:30 13:30	16	797	51	864	77	720	21	818	1682	26	8	22	56	45	5	106	156	212	1894
15:00 16:00	18	852	66	936	143	1148	35	1326	2262	22	9	19	50	52	10	112	174	224	2486
16:00 17:00	23	822	93	938	176	1164	36	1376	2314	14	10	14	38	63	16	129	208	246	2560
17:00 18:00	25	790	88	903	217	1112	39	1368	2271	24	5	24	53	71	13	142	226	279	2550
Sub Total	137	7221	471	7829	903	6846	188	7937	15766	238	57	182	477	417	60	1059	1536	2013	17779
U Turns				2				9	11				0				0	0	11
Total	137	7221	471	7831	903	6846	188	7946	15777	238	57	182	477	417	60	1059	1536	2013	17790
EQ 12Hr	190	10037	655	10885	1255	9516	261	11045	21930	331	79	253	663	580	83	1472	2135	2798	24728
Note: These	values a	are calcu	lated b	y multipl	lying the	e totals b	y the a	ppropria	te expans	sion fact	tor.		1	.39					
AVG 12Hr	190	10037	655	10885	1255	9516	261	11045	21930	331	79	253	663	580	83	1472	2135	2798	24728
Note: These	volumes	s are cald	culated	by mult	iplying t	he Equiv	alent 1	12 hr. tota	als by the	AADT	factor.		•	1.00					
AVG 24Hr	249	13149	858	14259	1644	12466	342	14469	28728	433	104	331	869	759	109	1928	2797	3666	32394
Note: These	volumes	s are cal	culated	by mult	iplying t	he Avera	age Da	ily 12 hr.	totals by	12 to 2	4 expans	sion fac	tor.	1.31					

#### Comments:

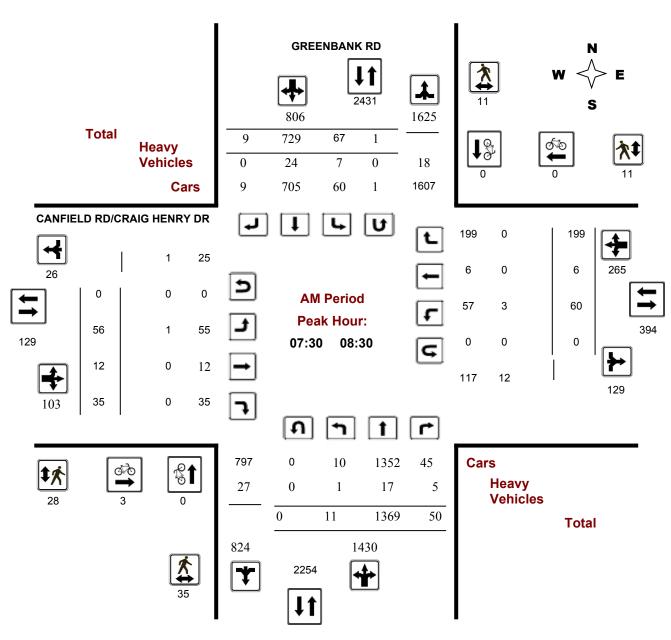
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



# **Turning Movement Count - Full Study Peak Hour Diagram**

### **GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR**

Survey Date: Wednesday, March 30, 2016 WO No: 35823
Start Time: 07:00 Device: Miovision



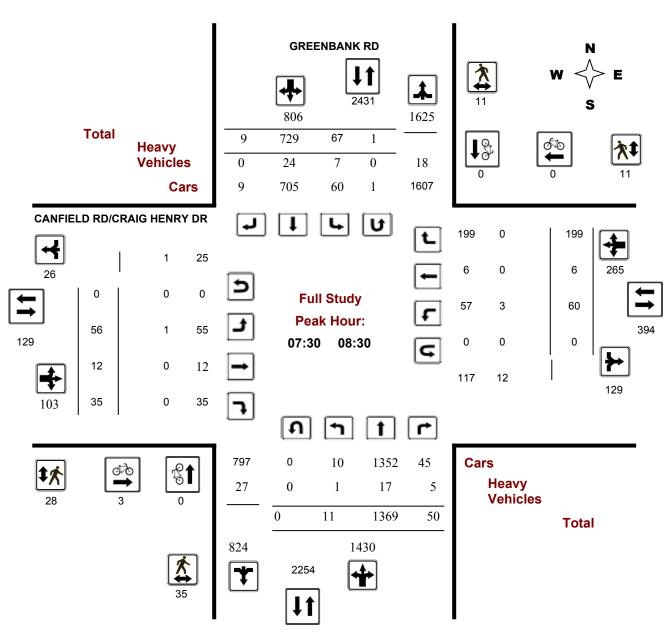
**Comments** 



# **Turning Movement Count - Full Study Peak Hour Diagram**

### GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Survey Date: Wednesday, March 30, 2016 WO No: 35823
Start Time: 07:00 Device: Miovision



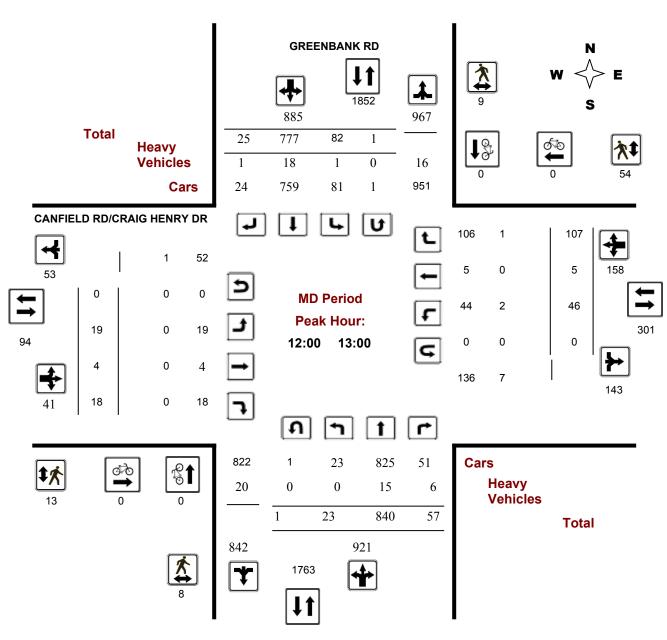
**Comments** 



# **Turning Movement Count - Full Study Peak Hour Diagram**

### GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Survey Date: Wednesday, March 30, 2016 WO No: 35823
Start Time: 07:00 Device: Miovision



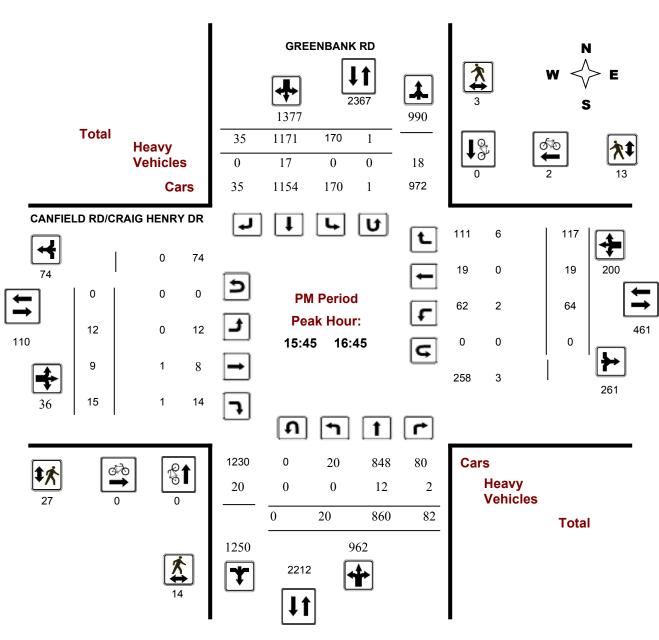
**Comments** 



# **Turning Movement Count - Full Study Peak Hour Diagram**

### GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Survey Date: Wednesday, March 30, 2016 WO No: 35823
Start Time: 07:00 Device: Miovision



**Comments** 



### **Turning Movement Count - 15 Min U-Turn Total Report**

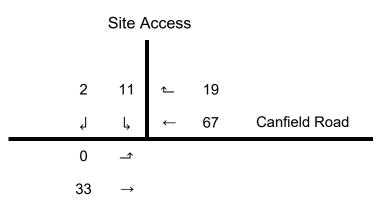
### GREENBANK RD @ CANFIELD RD/CRAIG HENRY DR

Survey Date: Wednesday, March 30, 2016

_		•					
Time I	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total	
07:00	07:15	0	0	0	0	0	
07:15	07:30	0	0	0	0	0	
07:30	07:45	0	0	0	0	0	
07:45	08:00	0	0	0	0	0	
08:00	08:15	0	0	0	0	0	
08:15	08:30	0	1	0	0	1	
08:30	08:45	0	0	0	0	0	
08:45	09:00	0	1	0	0	1	
09:00	09:15	1	0	0	0	1	
09:15	09:30	0	0	0	0	0	
09:30	09:45	0	0	0	0	0	
09:45	10:00	0	1	0	0	1	
11:30	11:45	0	1	0	0	1	
11:45	12:00	0	1	0	0	1	
12:00	12:15	0	0	0	0	0	
12:15	12:30	0	0	0	0	0	
12:30	12:45	0	0	0	0	0	
12:45	13:00	1	1	0	0	2	
13:00	13:15	0	0	0	0	0	
13:15	13:30	0	0	0	0	0	
15:00	15:15	0	0	0	0	0	
15:15	15:30	0	0	0	0	0	
15:30	15:45	0	2	0	0	2	
15:45	16:00	0	0	0	0	0	
16:00	16:15	0	1	0	0	1	
16:15	16:30	0	0	0	0	0	
16:30	16:45	0	0	0	0	0	
16:45	17:00	0	0	0	0	0	
17:00	17:15	0	0	0	0	0	
17:15	17:30	0	0	0	0	0	
17:30	17:45	0	0	0	0	0	
17:45	18:00	0	0	0	0	0	
То	otal	2	9	0	0	11	

Date: November 7, 2019

# PM peak hour



Strategy Report December 17, 2019

# Appendix B STEP 3.0 - FORECASTING COMMENT RESPONSE



From: Gervais, Josiane
To: O"Grady, Lauren

Cc:Meloshe, Nancy; Smith, Molly; Dickinson, MarySubject:RE: 1 Canfield - Step 3 TIA - TPM CommentsDate:Friday, November 29, 2019 12:00:59 PM

Hi Lauren,

The responses below are adequate. Please incorporate the comments/responses below within your report, and proceed to Step 4.

Regards,

#### Josiane Gervais, P.Eng.

Project Manager, Infrastructure Approvals | GPRJ Approbation des demandes d'infrastructure Development Review Branch | Dir Examen des projets d'aménagement

City of Ottawa | Ville d'Ottawa

Tel |Tél.: 613-580-2424 ext. | poste 21765

web | Site Web : www.ottawa.ca

From: O'Grady, Lauren < Lauren. OGrady@stantec.com>

Sent: November 28, 2019 3:19 PM

**To:** Gervais, Josiane <josiane.gervais@ottawa.ca>

**Cc:** Meloshe, Nancy <Nancy.Meloshe@stantec.com>; Smith, Molly <Molly.Smith@stantec.com>;

Dickinson, Mary <mary.dickinson@ottawa.ca>

Subject: RE: 1 Canfield - Step 3 TIA - TPM Comments

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#### Good morning Josiane,

Please see below my comment responses in green. If these comments are acceptable, I recommend proceeding with the Step 4 TIA as there is very little that needs to be changed in the Step 3 TIA that adds value to the overall study.

Please let me know if you and TES concur and if I can proceed with the Step 4 TIA.

Feel free to give me a call to discuss.

Have a great day

Lauren O'Grady P.Eng. Transportation Engineer

Direct: 613-784-2264

lauren.o'grady@stantec.com

Stantec 400 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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From: Gervais, Josiane < josiane.gervais@ottawa.ca>
Sent: Wednesday, November 27, 2019 1:07 PM
To: O'Grady, Lauren < Lauren.OGrady@stantec.com>
Subject: RE: 1 Canfield - Step 3 TIA - TPM Comments

Hi Lauren,

Please find comments below regarding the Forecasting Report:

#### **Transportation Engineering Services**

- Confirm whether there is a planned church expansion and/or phase 2. In the report, only one phase is indicated, but the site plan shows otherwise. The site plan is also inconsistent regarding the size of the office. The proposed site includes one phase.
- Clarify the use/ownership of the existing houses that remain within the Subject Site. There is one house that is within the subject site that will be owned by the church. The function and use of this house will not change from existing.
- Provide further justification that the expansion building will not increase trip generation during the Sunday peak hour. It seems likely that the size of the church's congregation may increase with people attracted to the church by the additional facilities in the expansion building (gym, kitchen, classrooms, chapel, etc...). As the size of the church is not anticipating to increase, the number of people who attend Sunday service will not increase (the current church is currently at capacity for Sunday service). The way the operations will work on a Sunday is that the attendees will go to the service on Sunday and then people will typically head over to the expansion building for socialization / refreshments / Sunday school. There will be no activities happening in the expansion building during Sunday service, therefore, the number of trips the site will generate will not be more than what is there currently on a Sunday. This information was provided to us by the client.
- Consider contacting The Met (Prince of Wales/Hunt Club) to obtain their weekday PM trip generation for similar functions. Using the Church land use code for weekday PM peak is likely to underestimate person-trips to the multi-purpose facility. Using the church land use for the expansion building was agreed upon by the City during the preparation of the Step 3 TIA. This land use includes auxiliary uses such as classrooms, dining facilities, party rooms, etc. In addition, the majority of the events (i.e. birthday parties, community meetings) that will occur in the expansion building will not overlap with the roadway peak. It will likely occur once the PM peak hour has slowed down.

- Provide at least the minimum number of parking spaces required by the by-law. Providing fewer than required will require an exemption through the Committee of Adjustment. With a gym/stage capably of accommodating 672 people, 96 parking spaces will be insufficient. Attention will need to be paid to the parking spillover module in step 4, particularly with the potential for spillover parking on Parkmount Crescent (which the expanded Site is now directly adjacent to). There will be an agreement in place between the church and an adjacent school that will allow spillover parking to use the school parking lot on Sundays. The Step 4 TIA will address parking and provide more details.
- Correct the number of parking spaces shown in section 3.3.2. the number of parking spaces will be confirmed as part of the Step 4 TIA.
- Include the traffic counts referenced in 2.1.2.5 in an appendix. The appendices will be provided as part of the Step 4 TIA.
- The proponent must re-submit Step 3 to address the issues noted above. See response to the second Development Review comment below.

#### **Traffic Signal Operations**

No comments, Noted.

#### **Development Review - TPM**

- Revise Table 4 (and subsequent figures). According to the site plan, the Gross Floor Area of Phase 1 is +/- 2,948 m² (31,710 s.f.). As agreed upon by the City during the preparation of the Step 3 TIA, the GFA of the 'gym / hall' area was used to calculate the number of trips. The rationale being that the gym / hall is the main reason people will use the expansion building. The auxiliary uses (office, etc.) will be used by people who are already attending the functions / events in the gym / hall. Despite this, the trip generation was rerun with the entire expansion building area (31,710 GFA) and it was found that the number of trips during the PM peak hour is 20 two-way trips, which is a negligible increase compared to using the 6000 GFA of the gym. The 31,710 GFA will be used to be conservative in the trip generation and can be reflected in the Step 4 TIA.
- Section 3.2.1 should comment on network plans for all travel modes. Based on the above comment responses, Stantec recommends proceeding with the Step 4 TIA as there is very little to change in the Step 3 TIA that will add value.

As requested by TES, please update and re-submit the forecasting report.

Regards,

#### Josiane Gervais, P.Eng.

Project Manager, Infrastructure Approvals | GPRJ Approbation des demandes d'infrastructure Development Review Branch | Dir Examen des projets d'aménagement City of Ottawa | Ville d'Ottawa

Tel |Tél.: 613-580-2424 ext. | poste 21765

web | Site Web : www.ottawa.ca

**From:** O'Grady, Lauren < <u>Lauren.OGrady@stantec.com</u>>

Sent: November 08, 2019 2:38 PM

**To:** Gervais, Josiane < <u>josiane.gervais@ottawa.ca</u>>

**Cc:** Meloshe, Nancy < <u>Nancy.Meloshe@stantec.com</u>>; bishoy samy < <u>bishoy\_samy@hotmail.com</u>>

Subject: 1 Canfield - Step 3 TIA

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Hi Josiane,

Please see attached the Step 3 TIA for the proposed development located at 1 Canfield. Please let me know if you have any questions or comments.

Have a great weekend,

### Lauren O'Grady P.Eng.

Transportation Engineer

Direct: 613-784-2264 lauren.o'grady@stantec.com

Stantec

400 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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,

Strategy Report December 17, 2019

# Appendix C MULTI-MODAL LEVEL OF SERVICE ANALYSIS



ConsultantStantecProject1 Canfield RoadScenario2019 ExistingDateDecember 5th, 2019

Permissive   Per		INTERSECTIONS		Greenbank Roa	nd at Canfield Road	
Median		Crossing Side	NORTH	SOUTH	EAST	WEST
Permissive or yield control control control RTOR allowed RTOR allowe			_	-	-	3 No Median - 2.4 m
Right Turns on Red (RTOR)?   Right Turn on Red (RTOR)?   Right Turn on Red (RTOR)?   Right Turn channel   RTOR allowed   RTO		Conflicting Left Turns				
Right Turns on Red (RTOR) ?   Ped Signal Leading Interval?   No		Conflicting Right Turns	•	•	•	,
Right Turn Channel   No Channel   10-15m   10-		, ,				
Comer Radius						
PETSI Score Ped. Exposure to Traffic LoS Ped.		S				-
Cycle Length   Effective Walk Time	an	Corner Radius			10-15m	
Cycle Length   Effective Walk Time	stri	•	markings	markings		markings
Cycle Length   Effective Walk Time	əpe		-			-
Effective Walk Time  Average Pedestrian Delay  Pedestrian Delay LoS  E  E  B  B  C  C  C  Level of Service  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE <bahrebreau mixed="" or="" separated="" th="" tra<="" traffic=""><th>Pe</th><th></th><th></th><th></th><th>-</th><th></th></bahrebreau>	Pe				-	
Average Pedestrian Delay Pedestrian Delay LoS  E  E  Approach From North South Fast West  Approach From North South Fast West  F Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic  Left Turn Approach One lane crossed One lane crossed No lane crossed No lane crossed No lane crossed Separated or Mixed Traffic F F F B B B B B B B B B B B B B B B B						
Pedestrian Delay LoS    E						
E   E   C   C					-	
Approach From NORTH SOUTH EAST WEST  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement Separated or Mixed Traffic Mixed Traffi		Pedestrian Delay Los	_	li e		
Approach From North South East West  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement Separated or Mixed Traffic  Left Turn Approach Operating Speed  Coperating Speed  Left Turning Cyclist  F F B B B Level of Service  Effective Corner Radius		Level of Comice	E	E	С	С
Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE <blank> Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic  Mixe</blank>		Level of Service			E	
IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE   Dedicated Right Turning Speed   Cyclist Through Movement   Separated or Mixed Traffic   Mixed Traf			NORTH	SOUTH	EAST	WEST
Cyclist Through Movement  Separated or Mixed Traffic  Left Turn Approach Operating Speed  Left Turning Cyclist  Average Signal Delay  Average Signal Delay  Effective Corner Radius  Cyclist Through Movement  Mixed Traffic  No lane crossed		IF Dedicated Right Turn Lane, THEN Right Turn Configuration,	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
Separated or Mixed Traffic  Left Turn Approach Operating Speed  Left Turning Cyclist  F F B B B Level of Service  Average Signal Delay  Separated or Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic One lane crossed One lane crossed One lane crossed No lane cross		Dedicated Right Turning Speed				
Operating Speed         ≥ 60 km/h         ≥ 60 km/h         ≤ 40 km/h         ≤ 40 km/h           Left Turning Cyclist         F         F         B         B           F         F         B         B           Level of Service         F         B         B           Average Signal Delay         ≤ 30 sec         ≤ 20 sec         > 40 sec           D         C         F         -           Effective Corner Radius         F         -	d)	Cyclist Through Movement				
Operating Speed         ≥ 60 km/h         ≥ 60 km/h         ≤ 40 km/h         ≤ 40 km/h           Left Turning Cyclist         F         F         B         B           F         F         B         B           Level of Service         F         B         B           Average Signal Delay         ≤ 30 sec         ≤ 20 sec         > 40 sec           D         C         F         -           Effective Corner Radius         F         -	<u> </u>	Separated or Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
Left Turning Cyclist  F F B B B F F B B B C F F B B B B C C F   Average Signal Delay   Average Signal Delay   Signal Delay  Sign	Bicy	Left Turn Approach	One lane crossed	One lane crossed	No lane crossed	No lane crossed
F F B B B   B   C F	_	Operating Speed	≥ 60 km/h	≥ 60 km/h	≤ 40 km/h	≤ 40 km/h
Level of Service  Average Signal Delay  Solve Service  Average Signal Delay  Level of Service  D C F  Effective Corner Radius		Left Turning Cyclist	F	F	В	В
Average Signal Delay  Solve Signal Delay  Average Signal Delay  Solve So			F	F	В	В
Level of Service  D C F  F  Effective Corner Radius		Level of Service	F			
Effective Corner Radius	i	Average Signal Delay	≤ 30 sec	≤ 20 sec	> 40 sec	
Effective Corner Radius	ns		D	С	F	-
	Tra	Level of Service			F	
		Effective Corner Radius				
Number of Receiving Lanes on Departure from Intersection	쑹	Number of Receiving Lanes on Departure from Intersection				
	[2		-	-	-	-
Level of Service -		Level of Service			-	

Consultant Stantec Project 1 Canfield Road
Scenario 2021 Future Background Date December 5th, 2019

Conflicting Left Turns		INTERSECTIONS		Greenbank Roa	nd at Canfield Road	
Median - 2.4 m Permissive or yield control RTOR allowed RTOR allowed No No No No Channel Corner Radius 10-15m 10-15		Crossing Side	NORTH	SOUTH	EAST	WEST
Conflicting Left Turns		Lanes	5	5	3	3
Permissive or yield control   Permissive or yield controls   Permissive or yield contentsore   Permissive or yield contents or yeld contentsore   Permiss			No Median - 2.4 m		No Median - 2.4 m	No Median - 2.4 m
Conflicting Right Turns on Red (RTOR) ?   Right Turns on Red (RTOR) ?   Ped Signal Leading Interval?   Right Turn Channel   No Chann		Conflicting Left Turns				
Right Turns on Red (RTOR) ?   Ped Signal Leading Interval?   No   No   No   No   No   No   No   N		Conflicting Right Turns	,	•	•	,
Ped Signal Leading Interval?   No No No No No No Channel   10-15m   Std transverse   markings   Std transverse markings   Std tran		Right Turns on Red (RToR) ?				RTOR allowed
Comer Radius		` ,	No	No	No	No
Crosswalk Type  PETSI Score  Ped. Exposure to Traffic LoS  E  E  Co  Cycle Length  Effective Walk Time  Pedestrian Delay  Pedestrian Delay LoS  E  E  C  Co  Co  Co  Co  Co  Co  Co  C		Right Turn Channel	No Channel	No Channel	No Channel	No Channel
Cycle Length Effective Walk Time 9 9 9 57  Average Pedestrian Delay 46 46 13 13  Pedestrian Delay LoS E E  Level of Service  E  Approach From North South East West  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic  Mixed Traffic Mixed T	_	Corner Radius	10-15m	10-15m	10-15m	10-15m
Cycle Length Effective Walk Time 9 9 9 57  Average Pedestrian Delay 46 46 13 13  Pedestrian Delay LoS E E  Level of Service  E  Approach From North South East West  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Tr	stria	Crosswalk Type			Std transverse markings	Std transverse markings
Cycle Length Effective Walk Time 9 9 9 57  Average Pedestrian Delay 46 46 13 13  Pedestrian Delay LoS E E  Level of Service  E  Approach From North South East West  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic  Mixed Traffic Mixed T	de	PETSI Score	37	37	70	70
Effective Walk Time  Average Pedestrian Delay  Pedestrian Delay LoS  E  E  B  B  B  B  E  E  C  C  C  Level of Service  E  Approach From North South EAST WEST  Approach From North South EAST West  HEN Right Turn Lane, THEN Right Turn Configuration, ELSE ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Traf	Pe	Ped. Exposure to Traffic LoS	E	E	С	С
Average Pedestrian Delay 46 46 13 13 13  Pedestrian Delay LoS E E E B B B  Level of Service  E  Approach From North South East West  Approach From North South East West  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE ELSE Cyclist Through Movement Separated or Mixed Traffic Mixed Tra			110	110	110	110
Pedestrian Delay LoS   E   E   B   B			9	9	57	57
E E C C  Level of Service    E		Average Pedestrian Delay	46	46	13	13
Level of Service   E		Pedestrian Delay LoS	E	E	B	В
Approach From NORTH SOUTH EAST WEST  Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Tra			E	E	С	С
Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic		Level of Service			E	
IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE <blank> Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic  Left Turn Approach One lane crossed No lane crossed No lane crossed  Operating Speed ≥ 60 km/h ≥ 60 km/h ≤ 40 km/h  Left Turning Cyclist F F B B</blank>	Approach From		NORTH	SOUTH	EAST	WEST
THEN Right Turn Configuration, ELSE Dedicated Right Turning Speed  Cyclist Through Movement  Separated or Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic Mixed Traffic  Left Turn Approach One lane crossed No lane crossed No lane crossed  Operating Speed \$\geq 60 \text{ km/h} \geq 60 \text{ km/h}  \square 40 \text{ km/h}  \square 40 \text{ km/h}  \square 40 \text{ km/h}			Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
Cyclist Through Movement  Separated or Mixed Traffic Mixe		THEN Right Turn Configuration,				
Separated or Mixed Traffic Mi		Dedicated Right Turning Speed				
Operating Speed         ≥ 60 km/h         ≥ 60 km/h         ≤ 40 km/h         ≤ 40 km/h           Left Turning Cyclist         F         F         B         B	0	Cyclist Through Movement				
Operating Speed         ≥ 60 km/h         ≥ 60 km/h         ≤ 40 km/h         ≤ 40 km/h           Left Turning Cyclist         F         F         B         B	) JO	Separated or Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
Left Turning Cyclist F F B B	Вісу	Left Turn Approach	One lane crossed	One lane crossed	No lane crossed	No lane crossed
		Operating Speed	≥ 60 km/h	≥ 60 km/h	≤ 40 km/h	≤ 40 km/h
		Left Turning Cyclist	F	F	В	В
			F	F	В	В
Level of Service F		Level of Service	·			
Average Signal Delay ≤ 40 sec ≤ 20 sec > 40 sec	Ħ	Average Signal Delay	≤ 40 sec	≤ 20 sec	> 40 sec	
E C F -	ns		E	С	F	-
Level of Service  E  C  F	Tra	Level of Service	F			
Effective Corner Radius		Effective Corner Radius				
Number of Receiving Lanes on Departure from Intersection	×					
from Intersection	2		-	-	-	-
Level of Service -	Ē	Level of Service			-	

ConsultantStantecProject1 Canfield RoadScenario2021 Total FutureDateDecember 5th, 2019

	INTERSECTIONS		Greenbank Roa	Greenbank Road at Canfield Road			
	Crossing Side	NORTH	SOUTH	EAST	WEST		
	Lanes Median Conflicting Left Turns	5 No Median - 2.4 m Permissive	5 No Median - 2.4 m Permissive	3 No Median - 2.4 m Permissive	3 No Median - 2.4 m Permissive		
	Conflicting Right Turns Right Turns on Red (RToR) ? Ped Signal Leading Interval? Right Turn Channel	Permissive or yield control RTOR allowed No No Channel	Permissive or yield control RTOR allowed No No Channel	Permissive or yield control RTOR allowed No No Channel	Permissive or yield control RTOR allowed No No Channel		
Pedestrian	Corner Radius Crosswalk Type	10-15m Std transverse markings	10-15m Std transverse markings	10-15m Std transverse markings	10-15m Std transverse markings		
de	PETSI Score	37	37	70	70		
Pe	Ped. Exposure to Traffic LoS	E	E	С	С		
	Cycle Length	110	110	110	110		
	Effective Walk Time	9	9	57	57		
	Average Pedestrian Delay	46	46	13	13		
	Pedestrian Delay LoS	E	E	В	В		
		E	E	С	С		
	Level of Service			E			
Approach From		NORTH	SOUTH	EAST	WEST		
	Bicycle Lane Arrangement on Approach IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE <blank></blank>	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic		
	Dedicated Right Turning Speed						
Φ	Cyclist Through Movement						
Į,	Separated or Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic		
Bicycle	Left Turn Approach	One lane crossed	One lane crossed	No lane crossed	No lane crossed		
	Operating Speed	≥ 60 km/h	≥ 60 km/h	≤ 40 km/h	≤ 40 km/h		
	Left Turning Cyclist	F	F	В	В		
		F	F	В	В		
	Level of Service	F					
Ħ	Average Signal Delay	≤ 40 sec	≤ 20 sec	> 40 sec			
nsi		E	С	F	-		
Transit	Level of Service	F					
쏭	Effective Corner Radius Number of Receiving Lanes on Departure from Intersection						
Truck	Level of Service	-	-	-	-		

ConsultantStantecProject1 Canfield RoadScenario2026 UltimateDateDecember 5th, 2019

	INTERSECTIONS		Greenbank Roa	d at Canfield Road	
	Crossing Side	NORTH	SOUTH	EAST	WEST
	Lanes Median	5 No Median - 2.4 m	5 No Median - 2.4 m	3 No Median - 2.4 m	3 No Median - 2.4 m
	Conflicting Left Turns	Permissive	Permissive	Permissive	Permissive
	Conflicting Right Turns	Permissive or yield	Permissive or yield	Permissive or yield	Permissive or yield
	Right Turns on Red (RToR) ?	control RTOR allowed	control RTOR allowed	control RTOR allowed	control RTOR allowed
	Ped Signal Leading Interval?	No	No	No	No
	Right Turn Channel	No Channel	No Channel	No Channel	No Channel
<u>_</u>	Corner Radius	10-15m	10-15m	10-15m	10-15m
Pedestrian	Crosswalk Type	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings
de	PETSI Score	37	37	70	70
Pe	Ped. Exposure to Traffic LoS	E	E	С	С
	Cycle Length	110	110	110	110
	Effective Walk Time	9	9	57	57
	Average Pedestrian Delay	46	46	13	13
	Pedestrian Delay LoS	E	E	В	В
		E	E	С	С
	Level of Service			E	
Approach From		NORTH	SOUTH	EAST	WEST
	Bicycle Lane Arrangement on Approach	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
	IF Dedicated Right Turn Lane, THEN Right Turn Configuration, ELSE <blank></blank>				
	Dedicated Right Turning Speed				
Φ	Cyclist Through Movement				
ΰ	Separated or Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
Bicycle	Left Turn Approach	One lane crossed	One lane crossed	No lane crossed	No lane crossed
	Operating Speed	≥ 60 km/h	≥ 60 km/h	≤ 40 km/h	≤ 40 km/h
	Left Turning Cyclist	F	F	В	В
		F	F	В	В
	Level of Service	F			
ij	Average Signal Delay	> 40 sec	≤ 20 sec	> 40 sec	
nsi		F	С	F	-
Transit	Level of Service			F	
	Effective Corner Radius				
Truck	Number of Receiving Lanes on Departure from Intersection				
[2		-	-	-	-
_	Level of Service			-	

# **Multi-Modal Level of Service - Segments Form**

ConsultantStantecProject1 Canfield RoadScenario2019 ExistingDateDecember 5th, 2019

SEGMENTS		Greenbank	Canfield
	Sidewalk Width	along PL 1.5 m	along PL 1.8 m
⊑	Boulevard Width	> 2 m	< 0.5 m
ria	Avg Daily Curb Lane Traffic Volume	> 3000	≤ 3000
Pedestrian	Operating Speed On-Street Parking	> 60 km/h no	> 30 to 50 km/h yes
) မင	Exposure to Traffic PLoS	E	В
	Level of Service	Е	В
	Type of Cycling Facility	Mixed Traffic	Mixed Traffic
	Number of Travel Lanes	2-3 lanes total	≤ 2 (no centreline)
	Operating Speed	≥ 60 km/h	≤ 40 km/h
cle	# of Lanes & Operating Speed LoS	F	Α
Bicycle	Bike Lane (+ Parking Lane) Width		
	Bike Lane Width LoS	-	-
	Bike Lane Blockages		
	Blockage LoS	-	-
	Level of Service	F	Α
Ħ	Facility Type	Mixed Traffic	
<b>Fransit</b>	Friction or Ratio Transit:Posted Speed	Vt/Vp ≥ 0.8	
T I	Level of Service	D	-
J	Truck Lane Width	≤ 3.5 m	
Ż	Travel Lanes per Direction	> 1	
Truck	Level of Service	Α	-

Strategy Report December 17, 2019

# Appendix D TRANSPORTATION DEMAND MANAGEMENT



# **TDM-Supportive Development Design and Infrastructure Checklist:**

Non-Residential Developments (office, institutional, retail or industrial)

Legend				
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed			
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users			
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance			

	TDM-s	supportive design & infrastructure measures:  Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	<b>∡</b>
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures:  Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	✓
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	upportive design & infrastructure measures:  Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	<b>▼</b>
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	
BETTER	2.1.5	Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	Not Applicable. Site Plan does not provide more than 50 bicycle spaces
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	
	2.3	Shower & change facilities	
BASIC	2.3.1	Provide shower and change facilities for the use of active commuters	
BETTER	2.3.2	In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	
	2.4	Bicycle repair station	
BETTER	2.4.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	

	TDM-s	supportive design & infrastructure measures:  Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	4.2	Carpool parking	
BASIC	4.2.1	Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	
BETTER	4.2.2	At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide carshare parking spaces in permitted non-residential zones, occupying either required or provided parking spaces (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	

	TDM-s	supportive design & infrastructure measures:  Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	♥
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	
	7.	OTHER	
	7.1	On-site amenities to minimize off-site trips	
BETTER	7.1.1	Provide on-site amenities to minimize mid-day or mid-commute errands	

# **TDM Measures Checklist:**

Non-Residential Developments (office, institutional, retail or industrial)

# BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	★ 1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & destin	ations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances	
	2.2	Bicycle skills training	
		Commuter travel	
BETTER	★ 2.2.1	Offer on-site cycling courses for commuters, or subsidize off-site courses	
	2.3	Valet bike parking	
		Visitor travel	
BETTER	2.3.1	Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances	
BASIC	3.1.2	Provide online links to OC Transpo and STO information	
BETTER	3.1.3	Provide real-time arrival information display at entrances	
	3.2	Transit fare incentives	
		Commuter travel	
BETTER	3.2.1	Offer preloaded PRESTO cards to encourage commuters to use transit	
BETTER ★	3.2.2	Subsidize or reimburse monthly transit pass purchases by employees	
		Visitor travel	
BETTER	3.2.3	Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	
	3.3	Enhanced public transit service	
		Commuter travel	
BETTER	3.3.1	Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.3.2	Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	
	3.4	Private transit service	
		Commuter travel	
BETTER	3.4.1	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.4.2	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for festivals, concerts, games)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	4.	RIDESHARING	
	4.1	Ridematching service	
		Commuter travel	
BASIC	4.1.1	Provide a dedicated ridematching portal at OttawaRideMatch.com	
	4.2	Carpool parking price incentives	
		Commuter travel	
BETTER	4.2.1	Provide discounts on parking costs for registered carpools	
	4.3	Vanpool service	
		Commuter travel	
BETTER	4.3.1	Provide a vanpooling service for long-distance commuters	
	5.	CARSHARING & BIKESHARING	
	5.1	Bikeshare stations & memberships	
BETTER	5.1.1	Contract with provider to install on-site bikeshare station for use by commuters and visitors	
		Commuter travel	
BETTER	5.1.2	Provide employees with bikeshare memberships for local business travel	
	5.2	Carshare vehicles & memberships	
		Commuter travel	
BETTER	5.2.1	Contract with provider to install on-site carshare vehicles and promote their use by tenants	
BETTER	5.2.2	Provide employees with carshare memberships for local business travel	
	6.	PARKING	
	6.1	Priced parking	
		Commuter travel	
BASIC *	6.1.1	Charge for long-term parking (daily, weekly, monthly)	
BASIC	6.1.2	Unbundle parking cost from lease rates at multi-tenant sites	
		Visitor travel	
BETTER	6.1.3	Charge for short-term parking (hourly)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	7.	TDM MARKETING & COMMUNICATIONS	
	7.1	Multimodal travel information	
		Commuter travel	
BASIC ★	7.1.1	Provide a multimodal travel option information package to new/relocating employees and students	
	_	Visitor travel	
BETTER ★	7.1.2	Include multimodal travel option information in invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)	
	7.2	Personalized trip planning	
		Commuter travel	
BETTER ★	7.2.1	Offer personalized trip planning to new/relocating employees	
	7.3	Promotions	
		Commuter travel	
BETTER	7.3.1	Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes	
	8.	OTHER INCENTIVES & AMENITIES	
	8.1	Emergency ride home	
		Commuter travel	:
BETTER ★	8.1.1	Provide emergency ride home service to non-driving commuters	
BETTER ★	8.1.1 <b>8.2</b>		
BETTER *		commuters	
	8.2	commuters  Alternative work arrangements	
	<b>8.2</b> 8.2.1	commuters  Alternative work arrangements  Commuter travel	
BASIC ★	<b>8.2</b> 8.2.1 8.2.2	Commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours	
BASIC ★ BETTER	<b>8.2</b> 8.2.1 8.2.2	Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks	
BASIC ★ BETTER	8.2.1 8.2.2 8.2.3	Commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework	
BASIC ★ BETTER	8.2.1 8.2.2 8.2.3	Alternative work arrangements  Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options	
BASIC ★ BETTER BETTER ★	8.2.1 8.2.2 8.2.3 8.3	Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the	
BASIC ★ BETTER BETTER ★	8.2.1 8.2.2 8.2.3 8.3 8.3.1	Alternative work arrangements  Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options  Commuter travel Provide local business travel options that minimize the need for employees to bring a personal car to work	
BASIC ★ BETTER BETTER ★	8.2.1 8.2.2 8.2.3 8.3 8.3.1 8.4	Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work  Commuter incentives	
BASIC ★ BETTER ★ BASIC ★	8.2.1 8.2.2 8.2.3 8.3 8.3.1 8.4	Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work  Commuter travel  Offer employees a taxable, mode-neutral commuting	
BASIC ★ BETTER ★ BASIC ★	8.2.1 8.2.2 8.2.3 8.3 8.3.1 8.4.1	Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work  Commuter incentives  Commuter travel  Offer employees a taxable, mode-neutral commuting allowance	

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# Appendix E INTERSECTION PERFORMANCE WORKSHEETS



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# **E.1 2019 EXISTING CONDITIONS**



	۶	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	~	<b>/</b>	<b>+</b>	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	₽		ሻ	Դ		ሻ	<b>∱</b> ኈ		*	<b>∱</b> ∱	
Traffic Volume (veh/h)	15	11	18	66	22	118	23	912	85	180	1241	41
Future Volume (veh/h)	15	11	18	66	22	118	23	912	85	180	1241	41
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	17	12	20	73	24	131	26	1013	94	200	1379	46
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	114	81	135	228	32	177	282	2340	217	387	2498	83
Arrive On Green	0.14	0.14	0.14	0.14	0.14	0.14	0.75	0.75	0.75	0.75	0.75	0.75
Sat Flow, veh/h	1232	597	995	1377	238	1300	376	3114	289	509	3325	111
Grp Volume(v), veh/h	17	0	32	73	0	155	26	547	560	200	697	728
Grp Sat Flow(s),veh/h/ln	1232	0	1593	1377	0	1538	376	1683	1720	509	1683	1752
Q Serve(g_s), s	1.5	0.0	1.9	5.4	0.0	10.7	3.5	13.2	13.2	26.2	19.3	19.4
Cycle Q Clear(g_c), s	12.1	0.0	1.9	7.4	0.0	10.7	22.9	13.2	13.2	39.4	19.3	19.4
Prop In Lane	1.00		0.63	1.00		0.85	1.00		0.17	1.00		0.06
Lane Grp Cap(c), veh/h	114	0	216	228	0	209	282	1265	1292	387	1265	1316
V/C Ratio(X)	0.15	0.00	0.15	0.32	0.00	0.74	0.09	0.43	0.43	0.52	0.55	0.55
Avail Cap(c_a), veh/h	252	0	395	383	0	382	282	1265	1292	387	1265	1316
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.5	0.0	41.9	45.2	0.0	45.7	10.7	5.0	5.0	12.3	5.8	5.8
Incr Delay (d2), s/veh	0.6	0.0	0.3	0.8	0.0	5.1	0.6	1.1	1.1	4.9	1.7	1.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	0.9	0.0	1.6	3.7	0.0	8.3	0.7	9.1	9.2	6.7	12.4	12.8
Unsig. Movement Delay, s/veh	1											
LnGrp Delay(d),s/veh	52.1	0.0	42.2	46.0	0.0	50.8	11.3	6.1	6.1	17.2	7.5	7.5
LnGrp LOS	D	Α	D	D	Α	D	В	Α	Α	В	Α	Α
Approach Vol, veh/h		49			228			1133			1625	
Approach Delay, s/veh		45.7			49.2			6.2			8.7	
Approach LOS		D			D			Α			Α	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		88.4		21.6		88.4		21.6				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 70		* 27		* 70		* 27				
Max Q Clear Time (g c+l1), s		24.9		14.1		41.4		12.7				
Green Ext Time (p_c), s		27.4		0.3		25.6		2.3				
Intersection Summary												
HCM 6th Ctrl Delay			11.4									
HCM 6th LOS			В									
Notos												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	0.9					
		EDT	WDT	WED	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	•	4	<b>1</b>	40	¥	•
Traffic Vol, veh/h	0	33	67	19	11	2
Future Vol, veh/h	0	33	67	19	11	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	37	74	21	12	2
William 10 W		O.	• •			_
	Major1		Major2		Minor2	
Conflicting Flow All	95	0	-	0	122	85
Stage 1	-	-	-	-	85	-
Stage 2	-	-	-	-	37	-
Critical Hdwy	4.12	-	_	-	6.42	6.22
Critical Hdwy Stg 1	-	_	-	-	5.42	_
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_	_		3 318
Pot Cap-1 Maneuver	1499	_	_	_	873	974
Stage 1	- 100	_	_	_	938	-
Stage 2	_				985	-
Platoon blocked, %	_	_			303	-
	1400	-	-	-	070	074
Mov Cap-1 Maneuver	1499	-	_	-	873	974
Mov Cap-2 Maneuver	-	-	-	-	873	-
Stage 1	-	-	-	-	938	-
Stage 2	-	-	-	-	985	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		9.1	
HCM LOS	U		U		9.1 A	
HCIVI LOS					А	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1499	_	_	_	
HCM Lane V/C Ratio		-	_	_		0.016
HCM Control Delay (s)		0	_	_	_	9.1
HCM Lane LOS		A	_	_	_	9.1 A
	١	0			-	0.1
HCM 95th %tile Q(veh	)	U	-	-	-	U. I

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>/</b>	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	₽		ሻ	₽		ሻ	<b>ተ</b> ኈ		ሻ	<b>∱</b> ∱	
Traffic Volume (veh/h)	42	7	66	78	7	138	43	1123	98	133	895	35
Future Volume (veh/h)	42	7	66	78	7	138	43	1123	98	133	895	35
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	47	8	73	87	8	153	48	1248	109	148	994	39
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	192	27	242	268	13	254	375	2062	180	269	2174	85
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.66	0.66	0.66	0.66	0.66	0.66
Sat Flow, veh/h	1225	151	1374	1317	75	1438	546	3133	273	401	3302	130
Grp Volume(v), veh/h	47	0	81	87	0	161	48	669	688	148	507	526
Grp Sat Flow(s),veh/h/ln	1225	0	1525	1317	0	1513	546	1683	1723	401	1683	1749
Q Serve(g_s), s	2.8	0.0	3.5	4.6	0.0	7.4	3.5	16.9	17.0	24.9	11.0	11.0
Cycle Q Clear(g_c), s	10.1	0.0	3.5	8.1	0.0	7.4	14.6	16.9	17.0	42.0	11.0	11.0
Prop In Lane	1.00		0.90	1.00		0.95	1.00		0.16	1.00		0.07
Lane Grp Cap(c), veh/h	192	0	269	268	0	267	375	1108	1134	269	1108	1151
V/C Ratio(X)	0.24	0.00	0.30	0.33	0.00	0.60	0.13	0.60	0.61	0.55	0.46	0.46
Avail Cap(c_a), veh/h	422	0	555	515	0	551	375	1108	1134	269	1108	1151
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.1	0.0	26.9	30.4	0.0	28.5	9.8	7.3	7.3	19.2	6.3	6.3
Incr Delay (d2), s/veh	0.7	0.0	0.6	0.7	0.0	2.2	0.7	2.4	2.4	7.9	1.4	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	1.7	0.0	2.6	3.0	0.0	5.7	0.9	11.3	11.5	5.4	7.9	8.1
Unsig. Movement Delay, s/veh		0.0	2.0	0.0	0.0	0.7	0.0	11.0	11.0	0.1	7.0	0.1
LnGrp Delay(d),s/veh	33.8	0.0	27.5	31.1	0.0	30.6	10.5	9.7	9.7	27.1	7.6	7.6
LnGrp LOS	C	A	C	C	A	C	В	A	A	C	Α	Α
Approach Vol, veh/h		128			248			1405			1181	
Approach Delay, s/veh		29.8			30.8			9.7			10.0	
Approach LOS		29.0 C			30.0 C			9.7 A			В	
Approach LOS					C						D	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		55.1		19.9		55.1		19.9				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 35		* 27		* 35		* 27				
Max Q Clear Time (g_c+l1), s		19.0		12.1		44.0		10.1				
Green Ext Time (p_c), s		14.3		1.1		0.0		2.7				
Intersection Summary												
HCM 6th Ctrl Delay			12.5									
HCM 6th LOS			В									
Notes												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	3					
	EDI	EDT	WDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	•	<u>ન</u>	ĵ.	07	<b>Y</b>	-
Traffic Vol, veh/h	3	58	47	37	56	7
Future Vol, veh/h	3	58	47	37	56	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	э,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	64	52	41	62	8
					_	
	Major1		Major2		Minor2	
Conflicting Flow All	93	0	-	0	143	73
Stage 1	-	-	-	-	73	-
Stage 2	-	-	-	-	70	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	-	_	3.518	3.318
Pot Cap-1 Maneuver	1501	_	_	_	850	989
Stage 1	-	_	_	_	950	-
Stage 2	_	_	_	_	953	_
Platoon blocked, %		_		_	300	
	1501		-		848	989
Mov Cap-1 Maneuver		-	_	-		
Mov Cap-2 Maneuver	-	-	-	-	848	-
Stage 1	-	-	-	-	948	-
Stage 2	-	-	-	-	953	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.4		0		9.5	
HCM LOS	0.4		U		3.5 A	
TIOWI LOS						
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1501	_	_	_	
HCM Lane V/C Ratio		0.002	_	-	_	0.081
HCM Control Delay (s)	\	7.4	0	_	_	9.5
HCM Lane LOS		Α	A	_	_	Α
HCM 95th %tile Q(veh	1	0	-	_	_	0.3
HOW SOUL WILLE CLASSE	1	U	_	-	-	0.5

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# **E.2 2021 FUTURE BACKGROUND CONDITIONS**



# Greenbank Road & Canfield Road 2021 Future Background - PM Peak Hour

	•	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	~	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ»		ሻ	₽		7	ħβ		7	<b>∱</b> ∱	
Traffic Volume (veh/h)	16	11	19	69	23	123	24	948	88	187	1291	43
Future Volume (veh/h)	16	11	19	69	23	123	24	948	88	187	1291	43
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	16	11	19	69	23	123	24	948	88	187	1291	43
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	113	75	130	221	31	168	313	2360	219	420	2520	84
Arrive On Green	0.13	0.13	0.13	0.13	0.13	0.13	0.76	0.76	0.76	0.76	0.76	0.76
Sat Flow, veh/h	1242	583	1007	1380	242	1296	410	3114	289	545	3325	111
Grp Volume(v), veh/h	16	0	30	69	0	146	24	512	524	187	653	681
Grp Sat Flow(s), veh/h/ln	1242	0	1591	1380	0	1539	410	1683	1720	545	1683	1752
Q Serve(g_s), s	1.4	0.0	1.8	5.1	0.0	10.0	2.7	11.7	11.7	20.0	16.9	16.9
Cycle Q Clear(g_c), s	11.4	0.0	1.8	7.0	0.0	10.0	19.6	11.7	11.7	31.7	16.9	16.9
Prop In Lane	1.00	0.0	0.63	1.00	0.0	0.84	1.00	11.7	0.17	1.00	10.5	0.06
Lane Grp Cap(c), veh/h	113	0	206	221	0	199	313	1276	1304	420	1276	1328
V/C Ratio(X)	0.14	0.00	0.15	0.31	0.00	0.73	0.08	0.40	0.40	0.44	0.51	0.51
Avail Cap(c_a), veh/h	260	0.00	395	385	0.00	382	313	1276	1304	420	1276	1328
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.6	0.00	42.5	45.6	0.00	46.1	9.2	4.6	4.6	10.1	5.3	5.3
Incr Delay (d2), s/veh	0.6	0.0	0.3	0.8	0.0	5.2	0.5	0.9	0.9	3.4	1.5	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	0.9	0.0	1.5	3.6	0.0	7.9	0.6	8.2	8.3	5.4	11.0	11.4
Unsig. Movement Delay, s/veh		0.0	1.0	3.0	0.0	1.3	0.0	0.2	0.5	J. <del>4</del>	11.0	11.4
LnGrp Delay(d),s/veh	52.1	0.0	42.8	46.4	0.0	51.2	9.6	5.6	5.6	13.5	6.7	6.7
LnGrp LOS	52.1 D	Α	42.0 D	40.4 D	Α	51.2 D	9.0 A	3.0 A	3.0 A	13.5 B	Α	0.7 A
	U		U	U		U	^		^	Ь		
Approach Vol, veh/h		46			215			1060			1521	
Approach Delay, s/veh		46.1			49.7			5.7			7.6	
Approach LOS		D			D			Α			Α	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		89.1		20.9		89.1		20.9				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 70		* 27		* 70		* 27				
Max Q Clear Time (g_c+I1), s		21.6		13.4		33.7		12.0				
Green Ext Time (p_c), s		26.2		0.3		30.1		2.2				
Intersection Summary												
HCM 6th Ctrl Delay			10.7									
HCM 6th LOS			В									
Notos												

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	0.9					
		ERT	MOT	MDD	ODI	ODD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्न	f)		¥	
Traffic Vol, veh/h	0	34	70	19	11	2
Future Vol, veh/h	0	34	70	19	11	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	_
Grade, %	_	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	34	70	19	11	2
IVIVIIICT IOW	U	04	10	10		
Major/Minor	Major1	N	Major2		Minor2	
Conflicting Flow All	89	0	-	0	114	80
Stage 1	_	-	-	-	80	-
Stage 2	-	-	-	-	34	-
Critical Hdwy	4.12	_	-	_	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	-	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	3 318
Pot Cap-1 Maneuver	1506	_		_	882	980
Stage 1	1500	_		_	943	-
	-		_			
Stage 2	-	-	-	-	988	-
Platoon blocked, %	4500	-	-	-	000	000
Mov Cap-1 Maneuver	1506	-	-	-	882	980
Mov Cap-2 Maneuver	-	-	-	-	882	-
Stage 1	-	-	-	-	943	-
Stage 2	-	-	-	-	988	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		9.1	
HCM LOS	U		U		9.1 A	
HOW LOS					A	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1506	_			
HCM Lane V/C Ratio		-	_	_		0.015
HCM Control Delay (s)	\	0	_	_	_	9.1
HCM Lane LOS		A	_	_	_	9.1 A
	1	0		-	-	0
HCM 95th %tile Q(veh	)	U	-	-	-	U

	۶	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	~	<b>/</b>	Ţ	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>₽</b>		ሻ	₽		ሻ	<b>∱</b> β		ሻ	<b>ተ</b> ኈ	
Traffic Volume (veh/h)	44	7	69	81	7	144	45	1168	102	138	931	36
Future Volume (veh/h)	44	7	69	81	7	144	45	1168	102	138	931	36
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	44	7	69	81	7	144	45	1168	102	138	931	36
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	189	23	231	259	12	241	408	2091	182	301	2205	85
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.67	0.67	0.67	0.67	0.67	0.67
Sat Flow, veh/h	1236	140	1383	1323	70	1442	581	3133	273	436	3304	128
Grp Volume(v), veh/h	44	0	76	81	0	151	45	627	643	138	474	493
Grp Sat Flow(s),veh/h/ln	1236	0	1523	1323	0	1512	581	1683	1723	436	1683	1749
Q Serve(g_s), s	2.6	0.0	3.3	4.3	0.0	6.9	2.9	14.8	14.9	18.4	9.8	9.8
Cycle Q Clear(g_c), s	9.5	0.0	3.3	7.6	0.0	6.9	12.7	14.8	14.9	33.3	9.8	9.8
Prop In Lane	1.00		0.91	1.00		0.95	1.00		0.16	1.00		0.07
Lane Grp Cap(c), veh/h	189	0	255	259	0	253	408	1123	1150	301	1123	1167
V/C Ratio(X)	0.23	0.00	0.30	0.31	0.00	0.60	0.11	0.56	0.56	0.46	0.42	0.42
Avail Cap(c_a), veh/h	432	0	554	520	0	550	408	1123	1150	301	1123	1167
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.3	0.0	27.4	30.7	0.0	28.9	8.7	6.6	6.6	15.5	5.8	5.8
Incr Delay (d2), s/veh	0.6	0.0	0.6	0.7	0.0	2.2	0.5	2.0	2.0	5.0	1.2	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	1.6	0.0	2.5	2.8	0.0	5.3	0.8	10.0	10.2	4.2	7.1	7.3
Unsig. Movement Delay, s/veh	1											
LnGrp Delay(d),s/veh	33.9	0.0	28.0	31.4	0.0	31.1	9.3	8.6	8.6	20.4	6.9	6.9
LnGrp LOS	С	Α	С	С	Α	С	Α	Α	Α	С	Α	Α
Approach Vol, veh/h		120			232			1315			1105	
Approach Delay, s/veh		30.2			31.2			8.6			8.6	
Approach LOS		C			C			A			A	
Timer - Assigned Phs		2		4		6		8			, ,	
Phs Duration (G+Y+Rc), s		55.8		19.2		55.8		19.2				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 35		* 27		* 35		* 27				
Max Q Clear Time (g c+l1), s		16.9		11.5		35.3		9.6				
Green Ext Time (p_c), s		15.4		1.1		0.0		2.6				
Intersection Summary												
			11.4									
HCM 6th Ctrl Delay HCM 6th LOS			11.4 B									
Notes			D									

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	2.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	f)		14	
Traffic Vol, veh/h	3	60	49	37	56	7
Future Vol, veh/h	3	60	49	37	56	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	_
Grade, %	_	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	60	49	37	56	7
WWW.CT IOW		00	10	O1	00	•
	Major1	N	//ajor2		Minor2	
Conflicting Flow All	86	0	-	0	134	68
Stage 1	-	-	-	-	68	-
Stage 2	-	-	-	-	66	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	_	_	-	_	5.42	_
Critical Hdwy Stg 2	-	_	_	-	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	3.318
Pot Cap-1 Maneuver	1510	_	_	_	860	995
Stage 1	-	_	_	_	955	-
Stage 2	_	_	_	_	957	_
Platoon blocked, %		_	_	_	331	
Mov Cap-1 Maneuver	1510	-	-	_	858	995
Mov Cap-2 Maneuver	-	-	-	-	858	-
Stage 1	-	-	-	-	953	-
Stage 2	-	-	-	-	957	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.4		0		9.5	
HCM LOS	0.1				A	
TIOM EGG						
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1510	-	-	-	871
HCM Lane V/C Ratio		0.002	-	-	-	0.072
HCM Control Delay (s)		7.4	0	-	_	9.5
HCM Lane LOS		Α	A	-	_	Α
HCM 95th %tile Q(veh	)	0	_	_	-	0.2
	1	U				J.2

Strategy Report December 17, 2019

# **E.3 2021 TOTAL FUTURE CONDITIONS**



# HCM 6th Signalized Intersection Summary 1: Greenbank Road & Canfield Road

	۶	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ»		ሻ	ĵ»		ሻ	ħβ		ሻ	<b>∱</b> }	
Traffic Volume (veh/h)	23	11	24	69	23	123	28	948	88	187	1291	48
Future Volume (veh/h)	23	11	24	69	23	123	28	948	88	187	1291	48
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	23	11	24	69	23	123	28	948	88	187	1291	48
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	114	65	141	218	32	169	311	2357	219	420	2505	93
Arrive On Green	0.13	0.13	0.13	0.13	0.13	0.13	0.76	0.76	0.76	0.76	0.76	0.76
Sat Flow, veh/h	1242	496	1082	1373	242	1296	408	3114	289	545	3310	123
Grp Volume(v), veh/h	23	0	35	69	0	146	28	512	524	187	656	683
	1242	0	1577	1373	0	1539	408	1683	1720	545	1683	1750
Grp Sat Flow(s),veh/h/ln	2.0	0.0	2.2	5.2	0.0	10.0	3.2	11.7	11.7	20.1	17.1	17.1
Q Serve(g_s), s	12.0		2.2	7.3						31.8		
Cycle Q Clear(g_c), s		0.0			0.0	10.0	20.4	11.7	11.7		17.1	17.1
Prop In Lane	1.00	0	0.69	1.00	0	0.84	1.00	4074	0.17	1.00	4074	0.07
Lane Grp Cap(c), veh/h	114	0	206	218	0	201	311	1274	1302	420	1274	1324
V/C Ratio(X)	0.20	0.00	0.17	0.32	0.00	0.73	0.09	0.40	0.40	0.45	0.51	0.52
Avail Cap(c_a), veh/h	260	0	391	379	0	382	311	1274	1302	420	1274	1324
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.7	0.0	42.5	45.8	0.0	45.9	9.4	4.7	4.7	10.2	5.3	5.3
Incr Delay (d2), s/veh	0.9	0.0	0.4	0.8	0.0	5.0	0.6	0.9	0.9	3.4	1.5	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	1.3	0.0	1.7	3.6	0.0	7.9	0.7	8.2	8.3	5.4	11.1	11.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	52.6	0.0	42.9	46.6	0.0	50.9	10.0	5.6	5.6	13.6	6.8	6.8
LnGrp LOS	D	Α	D	D	Α	D	Α	Α	Α	В	Α	A
Approach Vol, veh/h		58			215			1064			1526	
Approach Delay, s/veh		46.7			49.5			5.7			7.6	
Approach LOS		D			D			Α			Α	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		89.0		21.0		89.0		21.0				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 70		* 27		* 70		* 27				
Max Q Clear Time (g_c+l1), s		22.4		14.0		33.8		12.0				
Green Ext Time (p_c), s		26.1		0.3		30.0		2.2				
Intersection Summary												
HCM 6th Ctrl Delay			10.9									
HCM 6th LOS			10.9 B									
1 IOM OUI LOO												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	Þ		- W	
Traffic Vol, veh/h	1	34	70	33	25	5
Future Vol, veh/h	1	34	70	33	25	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	_	0	-
Grade, %	_	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	34	70	33	25	5
IVIVIII( I IOVV		04	70	00	20	J
Major/Minor	Major1	N	//ajor2		Minor2	
Conflicting Flow All	103	0	-	0	123	87
Stage 1	-	-	-	-	87	-
Stage 2	_	-	-	_	36	-
Critical Hdwy	4.12	_	_	_	6.42	6.22
Critical Hdwy Stg 1	_	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	3 318
Pot Cap-1 Maneuver	1489	_	_	_	872	971
Stage 1	-	_	_	_	936	-
Stage 2	_	_	_	_	986	_
Platoon blocked, %	_	_		_	300	_
	1400				071	971
Mov Cap-1 Maneuver	1489	-	-	-	871	
Mov Cap-2 Maneuver	-	-	-	-	871	-
Stage 1	-	-	-	-	935	-
Stage 2	-	-	-	-	986	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.2		0		9.2	
HCM LOS	0.2		U		A	
TIOWI LOO						
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1489	_	_	_	886
HCM Lane V/C Ratio		0.001	-	-	_	0.034
HCM Control Delay (s)		7.4	0	-	-	9.2
HCM Lane LOS		Α	A	-	-	Α
HCM 95th %tile Q(veh	)	0	-	_	_	0.1
TOW JOHN JOHN Q VOID	1	U				0.1

2021 Total Future - Sunday Peak Hour

Movement Lane Configurations	EBL	EBT										
Lane Configurations	-		EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	7	₽		ሻ	f)		7	<b>∱</b> ⊅		ሻ	ተኈ	
Traffic Volume (veh/h)	47	8	74	81	7	144	49	1168	102	138	931	39
Future Volume (veh/h)	47	8	74	81	7	144	49	1168	102	138	931	39
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	47	8	74	81	7	144	49	1168	102	138	931	39
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	192	25	234	258	12	245	405	2082	182	299	2188	92
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.66	0.66	0.66	0.66	0.66	0.66
Sat Flow, veh/h	1236	149	1376	1316	70	1442	579	3133	273	436	3292	138
Grp Volume(v), veh/h	47	0	82	81	0	151	49	627	643	138	476	494
Grp Sat Flow(s), veh/h/ln	1236	0	1524	1316	0	1512	579	1683	1723	436	1683	1747
Q Serve(g_s), s	2.7	0.0	3.5	4.3	0.0	6.9	3.2	14.9	15.0	18.6	9.9	9.9
Cycle Q Clear(g_c), s	9.6	0.0	3.5	7.9	0.0	6.9	13.2	14.9	15.0	33.6	9.9	9.9
Prop In Lane	1.00	0.0	0.90	1.00	0.0	0.95	1.00	17.5	0.16	1.00	9.9	0.08
Lane Grp Cap(c), veh/h	192	0	259	258	0	257	405	1119	1145	299	1119	1161
V/C Ratio(X)	0.24	0.00	0.32	0.31	0.00	0.59	0.12	0.56	0.56	0.46	0.43	0.43
Avail Cap(c_a), veh/h	432	0.00	555	513	0.00	550	405	1119	1145	299	1119	1161
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)												1.00
Uniform Delay (d), s/veh	33.1	0.0	27.3	30.7	0.0	28.7	9.0	6.7	6.7	15.7	5.9	5.9
Incr Delay (d2), s/veh	0.7	0.0	0.7	0.7	0.0	2.1	0.6	2.0	2.0	5.1	1.2	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	1.7	0.0	2.7	2.8	0.0	5.3	0.9	10.1	10.3	4.2	7.2	7.4
Unsig. Movement Delay, s/veh	00.0	0.0	00.0	04.4	0.0	00.0	0.0	0.0	0.7	00.0	- 4	7.0
LnGrp Delay(d),s/veh	33.8	0.0	28.0	31.4	0.0	30.8	9.6	8.8	8.7	20.8	7.1	7.0
LnGrp LOS	С	Α	С	С	Α	С	Α	Α	Α	С	Α	A
Approach Vol, veh/h		129			232			1319			1108	
Approach Delay, s/veh		30.1			31.0			8.8			8.8	
Approach LOS		С			С			Α			Α	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		55.5		19.5		55.5		19.5				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 35		* 27		* 35		* 27				
Max Q Clear Time (g c+l1), s		17.0		11.6		35.6		9.9				
Green Ext Time (p_c), s		15.1		1.1		0.0		2.5				
Intersection Summary												
HCM 6th Ctrl Delay			11.6									
HCM 6th LOS			11.0 B									
Notes			D									

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	3.4					
		FDT	MOT	MDD	ODL	ODD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	,	<u>ન</u>	<b>^</b>	40	¥	^
Traffic Vol, veh/h	4	60	49	49	74	9
Future Vol, veh/h	4	60	49	49	74	9
Conflicting Peds, #/hr	_ 0	0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	60	49	49	74	9
Major/Minor I	Major1	N	Major2		Minor2	
Conflicting Flow All	98	0	-	0	142	74
Stage 1	90	-		-	74	- 14
	_			-	68	_
Stage 2	4.12	-	-		6.42	6.22
Critical Hdwy		-	-	-		
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1495	-	-	-	851	988
Stage 1	-	-	-	-	949	-
Stage 2	-	-	-	-	955	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1495	-	-	-	848	988
Mov Cap-2 Maneuver	-	-	-	-	848	-
Stage 1	-	-	-	-	946	-
Stage 2	-	-	-	-	955	-
Annroach	EB		WB		SB	
Approach						
HCM Control Delay, s	0.5		0		9.6	
HCM LOS					Α	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1495	_	_	_	861
HCM Lane V/C Ratio		0.003	-	_	_	0.096
HCM Control Delay (s)		7.4	0	_	_	9.6
HCM Lane LOS		A	A	-	_	3.0 A
HCM 95th %tile Q(veh)	\	0	-			0.3
HOW SOUL YOUR W(VELL)	)	U	_	_	_	0.5

Strategy Report December 17, 2019

# **E.4 2026 ULTIMATE CONDITIONS**



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₽		7	₽		*	ተኈ		ሻ	<b>∱</b> ∱	
Traffic Volume (veh/h)	23	13	26	75	27	135	29	1040	97	205	1415	51
Future Volume (veh/h)	23	13	26	75	27	135	29	1040	97	205	1415	51
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	4==0	No	4==0	4	No	4	4	No	4==0	4==0	No	4==0
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	23	13	26	75	27	135	29	1040	97	205	1415	51
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	114	74	148	228	36	181	267	2324	217	372	2475	89
Arrive On Green	0.14	0.14	0.14	0.14	0.14	0.14	0.75	0.75	0.75	0.75	0.75	0.75
Sat Flow, veh/h	1224	527	1055	1368	257	1284	362	3113	290	495	3315	119
Grp Volume(v), veh/h	23	0	39	75	0	162	29	562	575	205	718	748
Grp Sat Flow(s),veh/h/ln	1224	0	1582	1368	0	1541	362	1683	1720	495	1683	1750
Q Serve(g_s), s	2.0	0.0	2.4	5.6	0.0	11.1	4.2	14.0	14.0	29.6	20.7	20.8
Cycle Q Clear(g_c), s	13.1	0.0	2.4	8.0	0.0	11.1	25.1	14.0	14.0	43.6	20.7	20.8
Prop In Lane	1.00	•	0.67	1.00	•	0.83	1.00	4057	0.17	1.00	4057	0.07
Lane Grp Cap(c), veh/h	114	0	223	228	0	217	267	1257	1284	372	1257	1307
V/C Ratio(X)	0.20	0.00	0.18	0.33	0.00	0.75	0.11	0.45	0.45	0.55	0.57	0.57
Avail Cap(c_a), veh/h	246	0	393	375	0	382	267	1257	1284	372	1257	1307
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.7	0.0	41.6	45.2	0.0	45.4	11.7	5.3	5.3	13.6	6.2	6.2
Incr Delay (d2), s/veh	0.9	0.0	0.4	0.8	0.0	5.1	0.8	1.2	1.1	5.8	1.9	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	1.3	0.0	1.9	3.9	0.0	8.6	0.8	9.5	9.7	7.4	13.1	13.6
Unsig. Movement Delay, s/veh		0.0	40.0	40.0	0.0	<b>50 5</b>	40.5	C F	C 4	10.1	0.0	0.0
LnGrp Delay(d),s/veh	52.5	0.0	42.0	46.0	0.0	50.5	12.5	6.5	6.4	19.4	8.0	8.0
LnGrp LOS	D	A	D	D	A	D	В	A	Α	В	A	A
Approach Vol, veh/h		62			237			1166			1671	
Approach Delay, s/veh		45.9			49.0			6.6			9.4	
Approach LOS		D			D			Α			Α	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		87.8		22.2		87.8		22.2				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 70		* 27		* 70		* 27				
Max Q Clear Time (g_c+l1), s		27.1		15.1		45.6		13.1				
Green Ext Time (p_c), s		27.5		0.3		22.5		2.4				
Intersection Summary												
HCM 6th Ctrl Delay			12.1									
HCM 6th LOS			В									

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	1.6					
		EDT	WDT	MDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	<b>\$</b>		¥	
Traffic Vol, veh/h	1	38	76	33	25	5
Future Vol, veh/h	1	38	76	33	25	5
Conflicting Peds, #/hr	_ 0	_ 0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	э,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	38	76	33	25	5
NA ' (NA'			4 . 0		4: 0	
	Major1		Major2		Minor2	
Conflicting Flow All	109	0	-	0	133	93
Stage 1	-	-	-	-	93	-
Stage 2	-	-	-	-	40	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1481	-	-	-	861	964
Stage 1	-	-	-	-	931	-
Stage 2	_	-	-	-	982	-
Platoon blocked, %		_	-	_		
Mov Cap-1 Maneuver	1481	-	_	-	860	964
Mov Cap-2 Maneuver	-	_	-	_	860	-
Stage 1	_	_	_	_	930	_
Stage 2	_	_	_	_	982	_
Olago Z					302	
Approach	EB		WB		SB	
HCM Control Delay, s	0.2		0		9.3	
HCM LOS					Α	
NA' I /NA - ' NA	. 1	EDI	EDT	MOT	MDD	0DL .4
Minor Lane/Major Mvn	nt .	EBL	EBT	WBT	WBR	
Capacity (veh/h)		1481	-	-	-	876
HCM Lane V/C Ratio		0.001	-	-	-	0.034
HCM Control Delay (s)		7.4	0	-	-	9.3
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh	1)	0	-	-	-	0.1

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>/</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	₽		ሻ	₽		ሻ	<b>∱</b> β		ሻ	<b>∱</b> ⊅	
Traffic Volume (veh/h)	49	8	78	89	8	157	51	1280	112	152	1020	41
Future Volume (veh/h)	49	8	78	89	8	157	51	1280	112	152	1020	41
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772	1772
Adj Flow Rate, veh/h	49	8	78	89	8	157	51	1280	112	152	1020	41
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	195	26	250	269	13	261	361	2047	179	256	2155	87
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.65	0.65	0.65	0.65	0.65	0.65
Sat Flow, veh/h	1221	142	1382	1311	73	1439	532	3133	273	388	3299	133
Grp Volume(v), veh/h	49	0	86	89	0	165	51	686	706	152	520	541
Grp Sat Flow(s), veh/h/ln	1221	0	1523	1311	0	1513	532	1683	1723	388	1683	1748
Q Serve(g_s), s	2.9	0.0	3.7	4.7	0.0	7.5	4.0	17.9	18.0	28.3	11.6	11.6
Cycle Q Clear(g_c), s	10.4	0.0	3.7	8.4	0.0	7.5	15.6	17.9	18.0	46.4	11.6	11.6
Prop In Lane	1.00	0.0	0.91	1.00	0.0	0.95	1.00	17.5	0.16	1.00	11.0	0.08
Lane Grp Cap(c), veh/h	195	0	276	269	0	274	361	1100	1126	256	1100	1142
V/C Ratio(X)	0.25	0.00	0.31	0.33	0.00	0.60	0.14	0.62	0.63	0.59	0.47	0.47
Avail Cap(c_a), veh/h	418	0.00	554	509	0.00	551	361	1100	1126	256	1100	1142
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.0	0.00	26.6	30.3	0.00	28.2	10.4	7.6	7.6	21.3	6.5	6.5
	0.7	0.0	0.6	0.7	0.0	2.1	0.8	2.7	2.6	9.7	1.5	1.4
Incr Delay (d2), s/veh	0.7		0.0		0.0		0.0			0.0		
Initial Q Delay(d3),s/veh	1.8	0.0	2.8	0.0 3.1	0.0	0.0 5.8	1.1	0.0 11.8	0.0 12.1	6.0	0.0 8.3	0.0 8.5
%ile BackOfQ(95%),veh/ln		0.0	2.0	ا . ۱	0.0	5.0	1.1	11.0	12.1	0.0	0.3	0.0
Unsig. Movement Delay, s/veh		0.0	07.0	24.0	0.0	20.2	44.0	40.0	40.0	24.0	0.0	7.0
LnGrp Delay(d),s/veh	33.7	0.0	27.3	31.0	0.0	30.3	11.3	10.3	10.3	31.0	8.0	7.9
LnGrp LOS	С	A	С	С	Α	С	В	В	В	С	Α	A
Approach Vol, veh/h		135			254			1443			1213	
Approach Delay, s/veh		29.6			30.6			10.3			10.8	
Approach LOS		С			С			В			В	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		54.7		20.3		54.7		20.3				
Change Period (Y+Rc), s		* 5.7		* 6.7		* 5.7		* 6.7				
Max Green Setting (Gmax), s		* 35		* 27		* 35		* 27				
Max Q Clear Time (g c+l1), s		20.0		12.4		48.4		10.4				
Green Ext Time (p_c), s		13.6		1.2		0.0		2.8				
Intersection Summary												
HCM 6th Ctrl Delay			13.1									
HCM 6th LOS			В									
Notes												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	3.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EBL			WBR		SBR
Lane Configurations	4	<del>ન</del>	<b>}</b>	40	74	٥
Traffic Vol, veh/h	4	66	54	49	74	9
Future Vol, veh/h	4	66	54	49	74	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	66	54	49	74	9
Major/Mina-	N 1 = i = = 1		1-is=0		\	
	Major1		Major2		Minor2	
Conflicting Flow All	103	0	-	0	153	79
Stage 1	-	-	-	-	79	-
Stage 2	-	-	-	-	74	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1489	-	-	-	839	981
Stage 1	-	-	-	-	944	-
Stage 2	_	_	_	_	949	_
Platoon blocked, %		_	-	_		
Mov Cap-1 Maneuver	1489	_	_	_	836	981
Mov Cap-2 Maneuver	-	_	_	_	836	-
Stage 1	_				941	_
Stage 2	_	_	_	_	949	_
Staye 2	_	_	-	_	343	_
Approach	EB		WB		SB	
HCM Control Delay, s	0.4		0		9.7	
HCM LOS					Α	
Minor Lane/Major Mvn	<u>nt</u>	EBL	EBT	WBT	WBR:	
Capacity (veh/h)		1489	-	-	-	
HCM Lane V/C Ratio		0.003	-	-	-	0.098
HCM Control Delay (s)	)	7.4	0	-	-	9.7
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh	)	0	-	-	-	0.3
75	,	_				