

Residential Building Development 1655 Carling Avenue

TIA Strategy Report

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Strategy Report

Parsons has been retained by Surface Developments to prepare a TIA in support of a Zoning By-Law Amendment Application (ZBLA) for a proposed residential building development in Ward 15: Kitchissippi. The following report represents Step 4 – Strategy, of the TIA process.

1. SCREENING FORM

The Screening Form was submitted to the City of Ottawa for review and verification of the need to complete a Transportation Impact Assessment (TIA). The Trip Generation, Location and Safety triggers of the Screening Form were all met based on the checklist provided by the TIA Guidelines. As such, a TIA Report was deemed required. The Screening Form and responses to City comments are provided in **Appendix A**.

2. SCOPING REPORT

2.1. EXISTING AND PLANNED CONDITIONS

2.1.1. PROPOSED DEVELOPMENT

The proposed development is located at 1655 Carling Ave and will consist of a total of 260 residential units within a 22-storey building. The site is currently occupied by an unpaved parking lot with an estimated maximum occupancy of 80 vehicles, which is being used by Carling Motors (1622 Carling Ave) and the Canadian Blood Services (1575 Carling Ave). The proposed residential development will be constructed in a single phase, with an anticipated full-buildout date of 2022. Figure 1 below provides the local context of the development site, while Figure 2 provides the current concept plan. The site is currently zoned as an Arterial Mainstreet (AM) zone.



Figure 1: Local Context

Figure 2: Concept Plan SURFACE 1655 CARLING AVENUE SP-1 SITE PLAN 1927 CONTRACTOR OF STREET A CONTRACT OF STREET, !!! AT ORADE COMMERCIAL 9 AVENUE CARLING (B)

1655 Carling Ave – TIA Strategy Report

As shown in **Figure 2**, one driveway access is proposed to serve the future development. The access will be located along the north side of Carling Ave, on the west end of the site, as a right-in/right-out driveway. In order to access the parking garage, residents will have to loop around the site to the east end of the building. Furthermore, the proposed development is anticipated to have 260 underground parking spaces and 24 surface parking spaces.

Given the centre median on Carling Ave along the site's frontage, vehicles destined to the site from the west would be expected to complete a U-turn at the Carling/Churchill and vehicles leaving to the site to the east would be required to complete a U-turn at the intersection Carling/Clyde/Cole intersection. Alternatively, there is a possibility that drivers elect to use Tillburry Avenue (local street situated north of the site) to avoid the U-turn manoeuvres on Carling Ave; however, this routing is considered more circuitous.

2.1.2. EXISTING CONDITIONS

Area Road Network

Carling Ave is an east-west municipal arterial roadway within the City of Ottawa, that runs from Bronson Ave in the east to March Rd in the west. Within the study area, Carling Ave has a six-lane divided cross-section, with a posted speed limit of 60 km/h and auxiliary turn lanes at major intersections.

Churchill Ave N is a north-south municipal roadway within the City of Ottawa, classified as a local roadway both south of Carling Ave and north of Lanark Ave, a major collector roadway between Carling Ave and Richmond Rd, an arterial roadway between Richmond Rd and Scott St and a collector roadway between Scott St and Lanark Ave. Churchill Ave N has a two-lane cross-section, with a posted speed limit of 50 km/h and auxiliary turn-lanes at major intersections.

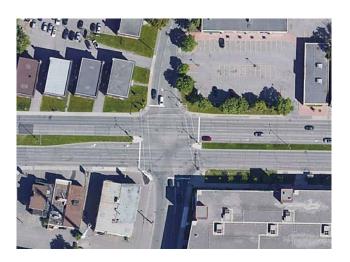
Cole Ave is a north-south municipal local roadway within the City of Ottawa, that extends from Dovercourt Ave in the north to connect to the north leg of Carling Ave in the south. Cole Ave has a two-lane cross-section and a posted speed limit of 40 km/h.

Clyde Ave is a north-south municipal local roadway within the City of Ottawa, that extends from the south leg of Carling Ave and terminates at 951 Clyde Ave. The roadway then continues at Castle Hill Crescent intersection down to Merivale Rd/Lotta Ave. Within the study area, Clyde Ave, with an assumed speed limit of 50 km/h and auxiliary turn lanes at major intersections.

Existing Study Area Intersections

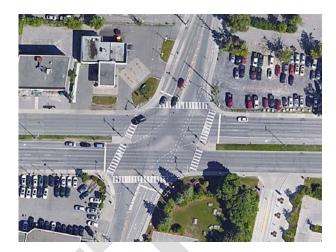
Carling/Churchill

The Carling/Churchill intersection is a four-legged full-movement signalized intersection. The east and west legs of the intersection consist of two exclusive through lanes, 1 shared through/right-turn lane and one auxiliary left-turn lane. While the north and south legs of the intersection consist of one shared through/right-turn lane and one auxiliary left-turn lane. There are no restricted movements at this intersection.



Carling/Clyde/Cole

The Carling/Clyde/Cole intersection is a four-legged full-movement signalized intersection. The east and west legs of the intersection consist of two exclusive through lanes, 1 shared through/right-turn lane and one auxiliary left-turn lane. The north leg of the intersection consists of one shared through/right-turn lane and one auxiliary left-turn lane. The south leg of the intersection consists of one exclusive through lane, one auxiliary left-turn lane and one auxiliary right-turn lane. There are no restricted movements at this intersection. However, trucks are not permitted to enter the north leg of the intersection.



Existing Driveways to Adjacent Developments

The following driveways are currently on the north side of Carling Ave, within 200 m of the proposed development driveway at 1655 Carling Ave:

- Two accesses to Shell Gas Station, approximately 45 m east of the proposed development;
- One access to a small mixed-use commercial and office building, approximately 85 m west of the proposed development; and,
- One access to commercial and office units, approximately 160 m west of the proposed development.

Pedestrian/Cycling Network

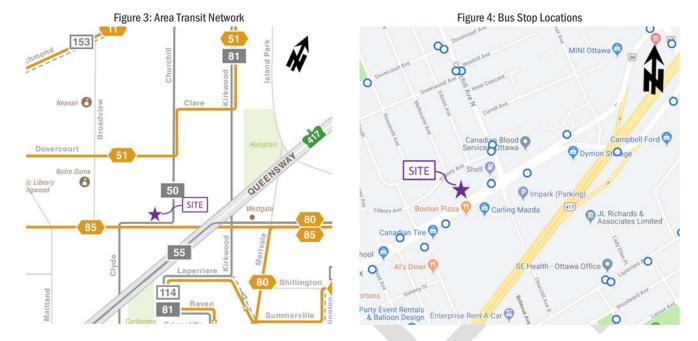
Pedestrian sidewalk facilities are provided throughout the study area, including both sides of Carling Ave, both sides of Churchill Ave, both sides of Cole Ave and the east side of Clyde Ave. With regards to cycling facilities, unidirectional cycle tracks are provided on both sides of Churchill Ave N, between Carling Ave and Byron Ave. There are no dedicated cycling facilities on Carling Ave, although it is identified as a spine route in the TMP.

Transit Network

The following OC Transpo routes currently operate along Carling Ave, at the frontage of the site:

- Route #50 (Tunney's Pasture <-> Lincoln Fields): identified by OC Transpo as a "Local Route", this route
 operates on customized routing and schedules, to serve local destinations. Route #50 operates at an
 average rate of every 30 minutes during weekdays. The nearest bus stops to the site are the
 Carling/Churchill stop, for the Lincoln Fields destination and the Carling/Clyde North stop, for the
 Tunney's Pasture destination.
- Route #85 (Lees <-> Bayshore): identified by OC Transpo as a "Frequent Route", this route operates at a high frequency along Carling Ave. Route #85 operates 7 days a week, at an average rate of every 15 minutes or less during weekday peak hours. The nearest bus stops to the site are the Carling/Churchill stop, for the Bayshore destination and the Carling/Clyde North stop, for the Lees destination.

OC Transpo route maps for routes #50 and #85 have been provided in **Appendix B**. **Figure 3** below illustrates the area transit network surrounding the subject site, while **Figure 4** provides the nearest bus stop locations to the development site in the form of blue dots. The Carling/Churchill bus stop is approximately 40 m east of the site, while the Carling/Clyde North bus stop is within approximately a 300 m walking distance of the site.



Peak Hour Travel Demand

The existing peak hour traffic volumes, which were obtained from the City of Ottawa for the intersections of Carling/Churchill and Carling/Clyde/Cole, are illustrated in **Figure 5** below. Note that the east and west traffic volumes approaching to/from the Carling/Clyde/Cole intersection were balanced according to the traffic volumes at the Carling/Churchill intersection. Similarly, the SB traffic approaching from the Tillbury/Churchill intersection was balanced based on the SB traffic volumes at the Carling/Churchill intersection. With regards to the counts conducted at the existing site, the three existing accesses were combined to assume one large access. The raw traffic count data has been provided in **Appendix C**.

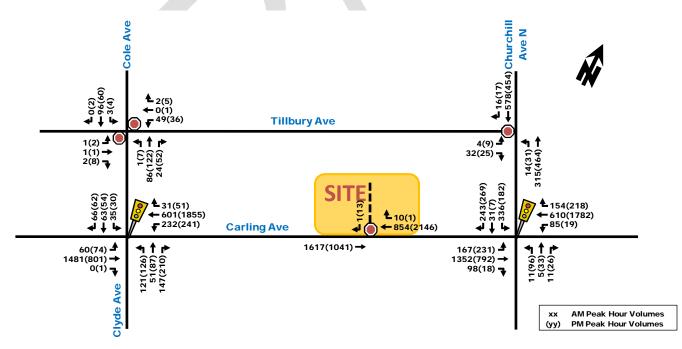


Figure 5: Existing Peak Hour Traffic Volumes

Existing Road Safety Conditions

A five-year collision history data (2014-2018, inclusive) was requested and obtained from the City of Ottawa for all intersections and road segments within the study area. Upon analyzing the collision data, the total number of collisions observed within the study area was determined to be 84 collisions within the past five-years. The majority of the collisions (83%) resulted in property damage only, while the remaining collisions resulted in a non-fatal injury. Furthermore, the type of impacts that resulted in 84 collisions were broken down into the following: 32 turning movement, 24 rear end, 24 sideswipe and 4 angle.

A standard unit of measure for assessing collisions at an intersection is based on the number of collisions per million entering vehicles (MEV). At signalized intersections within the study area, reported collisions have historically taken place at a rate of:

- 0.31 Collisions/MEV at the intersection of Carling/Churchill. A total of 22 collisions took place at this intersection
 within the past five-years. However, breakdown of collision data shows that there are no particular collision
 patterns taking place at this intersection.
- 0.77 Collisions/MEV at the intersection of Carling/Clyde/Cole. A total of 50 collisions took place at this intersection
 within the past five-years. 29 collisions occurred as a result of turning movements, the majority of which were
 between the WB/EB left-turns and the opposing EB/WB through movements, respectively. The turning movement
 collisions along Carling Ave are assumed to be due to the following:
 - Left-turning vehicles must cross 4 lanes of traffic (3 through and 1 left-turn lanes) to complete their leftturn movement.
 - Left-turn types along Carling Ave are protected-permitted during peak hours. Hence, the collisions occur during the EB/WB through/permitted left-turns phase.
 - A high volume of traffic performs a WBLT during peak hours to access the Canadian Tire store.

As such, turning movement collisions at the Carling/Clyde/Cole intersection can be potentially reduced by allowing only a protected left-turn type for EB and WB movements, during the peak hours of travel demand.

Lastly, 6 collisions were observed along Carling Ave, between Churchill Ave and Clyde Ave/Cole Ave. However, no particular collision patterns were observed.

Collision data obtained from the City of Ottawa is provided in Appendix D, along with the analysis conducted by Parsons.

2.1.3. PLANNED CONDITIONS

Planned Study Area Transportation Network Changes

Based on the City of Ottawa's TMP, the 2031 Affordable Network for Rapid Transit and Transit Priority illustrates Carling Ave as a Transit Priority Corridor (Continuous Measures) at the frontage of the site. There are no other anticipated changes to the road network surrounding the subject development's site.

Other Area Developments

A summary of other area developments is provided below based on the latest available information from the City regarding adjacent site development applications.

689 Churchill Ave

A 3-storey low-rise apartment building is proposed at 689 Churchill Ave. The building will consist of 15 residential units, which are expected to generate minimal traffic. As such, traffic generated by this development will be accounted for in the background growth rate in the forecasting section of the report.

701 Churchill Ave

A 3-storey low-rise apartment building is proposed at 701 Churchill Ave. The building will consist of 12 residential units, which are expected to generate minimal traffic. As such, traffic generated by this development will be accounted for in the background growth rate in the forecasting section of the report.

1705 Carling Ave

A TIA was submitted by Novatech in May 2018 for a 9-storey mixed-use building development, containing 68 senior's apartments and 130 residential care units at 1705 Carling Ave. As the site is currently occupied by an 80-unit motel and a 3500 ft² restaurant, the number of trips forecasted for the future development are expected to be lower than the trips generated by the existing developments at this site. Thus, trips for this development will not be added to the background traffic volumes forecasted in this report.

2.2. STUDY AREA AND TIME PERIODS

Since the proposed development consists of a residential building, the peak time periods to be assessed are the weekday morning and afternoon peak hour periods. Furthermore, the horizon years to be analyzed are the year of full-buildout of the development (2022) and five years after full-buildout (2027), as per the requirements of the TIA Guidelines. The proposed study area is shown below in **Figure 6**.



Figure 6: Study Area

Study area intersections include:

- Carling Ave/Clyde Ave/Cole Ave
- Carling Ave/Churchill Ave N

A justification letter was provided to the City of Ottawa on August 28th, 2019, explaining the rationale for using a reduced study area limit instead of the 1 km radius recommended by the TIA Guidelines. The justification letter is provided in **Appendix E**.

2.3. EXEMPTION REVIEW

Based on the City's TIA guidelines and the subject site, the following modules/elements of the TIA process, summarized in **Table 1**, are recommended to be exempt in the subsequent steps of the TIA process:

Table 1: Exemptions Review Summary

Module	Element	Exemption Consideration
4.1 - 4.4 Design Review Component	All elements	Site design requirements are omitted in a Zoning By-Law Amendment Application. These elements will be explored in detail in the future Site Plan Application.
4.8 Review of Network Concept	All elements	The site is not expected to generate 200 trips more than the established zoning.

3. FORECASTING

3.1. DEVELOPMENT GENERATED TRAVEL DEMAND

3.1.1. TRIP GENERATION AND MODE SHARES

The proposed development will consist of a high-rise apartment building containing 260 residential units. Appropriate trip generation rates were obtained from the 2009 TRANS Trip Generation Residential Trip Rates report, Table 6.3, which have been summarized in **Table 2**.

Table 2: TRANS Trip Generation Residential Trip Rates

Land Use	Data	Trip R	Rates		
Land Use	Source	AM Peak	PM Peak		
High-Rise Apartments	TRANS	T = 0.24(du);	T = 0.27(du);		
Notes: T = Average Vehicle Trip Ends du = Dwelling unit					

Using the trip rates shown in **Table 2**, along with the planned number of residential units, the number of vehicles per hour were determined as shown in **Table 3** below.

Table 3: Apartment Units Vehicle Trip Generation

Land Use		Dwelling	AM P	eak (Vehicle	PM Peak (Vehicles/h)			
	Land USE	Units	In	Out	Total	In	Out	Total
	High-Rise Apartments	260	14	48	62	43	27	70

The total vehicle trips shown in **Table 3** were then converted to total person trips using mode share percentages found in the 2009 TRANS Trip Generation Study Report. The total person trips were then used to determine person trips/h for each travel mode, based on their respective mode share percentages. **Table 4** summarizes the number of trips generated for each of the respective travel modes of the proposed development.

Table 4: Mode Shares for the Residential Buildings Development

Travel Mode	Mode	AM Peak (Person Trips/h)			Mode	PM Peak (Person Trips/h)			
Travel Mode	Share	ln (24%)	Out (76%)	Total	Share	In (62%)	Out (38%)	Total	
Auto Driver	37%	14	48	62	40%	43	27	70	
Auto Passenger	8%	3	10	13	9%	10	6	16	
Transit	41%	16	53	69	37%	40	25	65	
Non-motorized	14%	5	19	24	14%	14	10	24	
Total Person Trips	100%	38	130	168	100%	107	68	175	

As shown in **Table 4**, the resulting number of Total Person Trips expected to be generated by the proposed development are approximately 168 and 175 person trips/h in the morning and afternoon peak hours respectively.

Furthermore, the 2011 NCR Household Origin-Destination Survey provides mode share percentages based on the district where the proposed development site is located. As the site is located in the Ottawa West district, the respective mode share percentages were used in conjunction with the total person trips determined in **Table 4**, thereby providing new trip generation results as shown in **Table 5**.

Travel Mode	Mode Share	AM Pe	eak (Person T	rips/h)	PM Peak (Person Trips/h)					
Travel Mode	Widde Share	In (24%)	Out (76%)	Total	In (24%)	Out (76%)	Total			
Auto Driver	50%	20	64	84	53	34	87			
Auto Passenger	15%	6	19	25	16	10	26			
Transit	20%	7	26	33	21	14	35			
Bike	5%	2	7	9	5	4	9			
Walk	10%	4	13	17	11	7	18			
Total Person Trips	100%	39	129	168	106	69	175			
	Total Auto Trips	20	64	84	53	34	87			

Table 5: OD Survey Mode Shares - Ottawa West District

The auto trips expected to be generated by the future residential development are 84 and 87 veh/h during the morning and afternoon peak hour periods, respectively. However, considering that proposed transit movements along Carling Ave (see **Section 3.2.1**), the mode shares were adjusted as shown in **Table 6** below to reflect a higher transit usage.

Table 6. Ob Survey Wide Shares with increased transit										
Traval Mada	Made Chare	AM P	eak (Person T	rips/h)	PM Peak (Person Trips/h)					
Travel Mode	Mode Share	In (24%)	Out (76%)	Total	In (24%)	Out (76%)	Total			
Auto Driver	40%	16	52	68	43	27	70			
Auto Passenger	10%	4	12	16	10	7	17			
Transit	35%	13	45	58	37	24	61			
Bike	5%	2	7	9	5	4	9			
Walk	10%	4	13	17	11	7	18			
Total Person Trips	100%	39	129	168	106	69	175			
	Total Auto Trips	16	52	68	43	27	70			

Table 6: OD Survey Mode Shares with Increased Transit

As such, the proposed development is forecasted to generate approximately 70 veh/h during both the morning and afternoon peak hour periods. However, the proposed development site is currently occupied by an unpaved parking lot with an assumed maximum occupancy of 80 vehicles. Morning and afternoon peak hour traffic counts were conducted at the three access points of the existing parking lot. **Table 7** provides the vehicle trips to/from the existing parking lot (as shown in **Figure 5**).

AM Peak (Vehicles/h) PM Peak (Vehicles/h) Land Use Capacity In Out **Total** In Out Total **Existing Unpaved Parking Lot** 80 Vehicles 10 1 11 1 13 14

Table 7: Vehicle Trips to/from the Existing Unpaved Parking Lot

Therefore, the net differences in vehicles between future and existing development conditions are provided in **Table 8**.

Land Use	AM F	Peak (Vehicle	es/h)	PM Peak (Vehicles/h)			
Land Ose	In	Out	Total	In	Out	Total	
New Trips High-Rise Apartments	16	52	68	43	27	70	
Minus Existing Trips Unpaved Parking Lot	10	1	11	1	13	14	
Net Change	6	51	57	42	14	56	

As shown in **Table 8**, the planned residential building development is expected to result in a net increase of approximately 55 veh/h within the study area, during both morning and afternoon peak hour periods.

3.1.2. TRIP DISTRIBUTION AND ASSIGNMENT

Based on the 2011 OD Survey (Ottawa West district) and the location of adjacent arterial roadways and neighbourhoods, the distribution of site-generated traffic volumes was estimated as follows:

- 20% to/from the north;
- 10% to/from the south;
- 65% to/from the east; and,
- 5% to/from the west.

The expected site-generated auto trips in **Table 5** were then assigned to the surrounding road network as shown in **Figure 7** below. Furthermore, traffic volumes generated by the existing parking lot (see **Table 7**) are shown in **Figure 8** and the net difference in traffic (see **Table 8**) is illustrated in **Figure 9**. With regards to inbound traffic, vehicles were assumed to approach as follows:

- 65% from east Carling Ave
- 20% from north Churchill Ave N, and
- 15% from west Carling Ave (2/3 of which complete the U-turn at Carling/Churchill and 1/3 use Tillburry Ave),

Outbound vehicles were assumed to leave the site as follows:

- 15% to west Carling Ave
- 20% to north Churchill Ave N through Tillbury Ave and
- 65% to east Carling Ave (3/4 of which complete the U-turn at Carling/Clyde/Cole and 1/4 use Tillbury Ave).

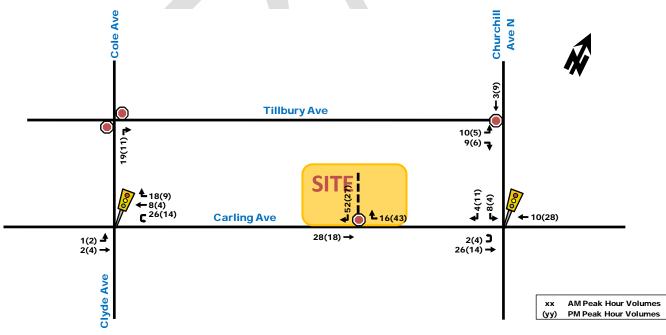
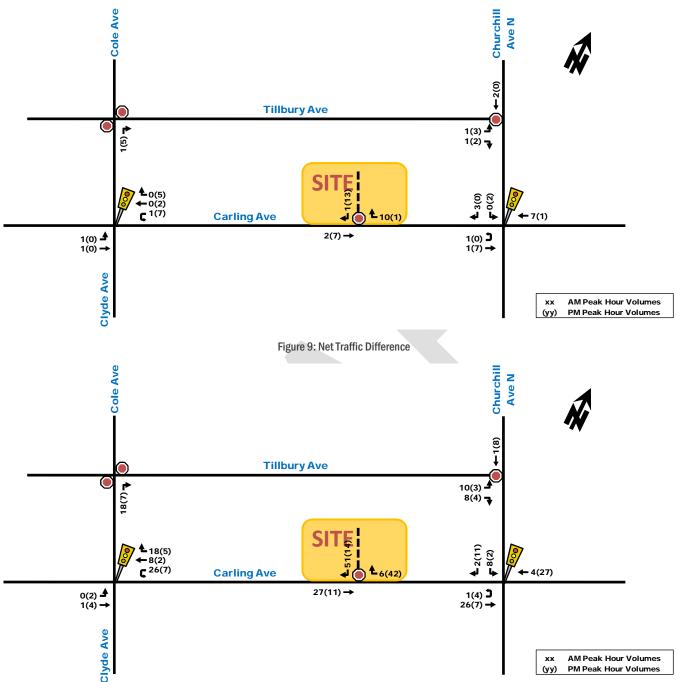


Figure 7: Planned Residential Development Site-Generated Traffic

Figure 8: Existing Parking Lot Traffic Volumes



3.2. BACKGROUND NETWORK TRAFFIC

3.2.1. TRANSPORTATION NETWORK PLANS

Refer to Section 2.1.3: Planned Study Area Transportation Network Changes. A functional design project initiated by the City of Ottawa is currently underway to provide transit priority measures along Carling Ave, from Lincoln Fields to Bronson Ave. Between Lincoln Fields and Sherwood Dr, the plan is to provide a single designated bus lane for transit by reducing the three general purpose lanes to two general purpose lanes along both sides of Carling Ave. This plan is anticipated to

be implemented by 2020. **Figure 10** below shows the functional plan at the frontage of the site, where red lines represent the designated transit bus lanes.



Figure 10: Carling Ave Transit Priority Measure

3.2.2. BACKGROUND GROWTH

The introduction of designated transit bus lanes along Carling Ave is expected to help reduce future background traffic through increasing the reliability and performance of transit services. Furthermore, since the area is well developed and there are no major other area developments planned near the subject site, traffic along Carling Ave is not anticipated to increase significantly in the next few years. As such, traffic growth is assumed to be 0% per year for the future horizon year 2022 and 1% per year thereafter for future horizon year 2027. Traffic volumes anticipated for the future background horizon year 2027 is illustrated in **Figure 11**.

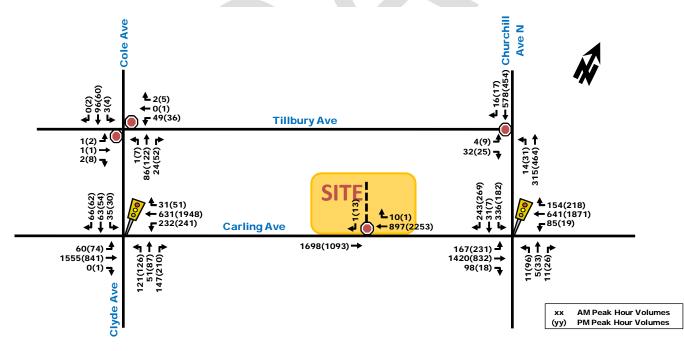


Figure 11: Future Background 2027 Traffic Volumes

3.2.3. OTHER DEVELOPMENTS

Description of other area developments taking place within the study area was provided in **Section 2.1.3 - Other Area Developments**. For the purposes of this report, there are no additional traffic volumes associated with adjacent area developments that will be included in the analysis.

3.3. DEMAND RATIONALIZATION

Given that the number of lanes along Carling Ave is anticipated to be reduced to two general-purpose lanes in each direction, capacity of the study area intersections in future horizon years will be significantly lower than the capacity of Carling Ave in existing conditions. However, based on the *Carling Avenue Transit Priority Measures and Functional Design Report (WSP, June 2017)*, east-west traffic along Carling Ave is forecasted to decrease by up to 20% due to the implementation of the continuous dedicated bus lanes. The resulting anticipated traffic volumes for future background horizon years 2022 and 2027 are illustrated in **Figure 12** and **Figure 13**. Note that, as per **Section 3.2.2**, a 1% per year background growth rate was applied to traffic volumes between horizon year 2022 and 2027.

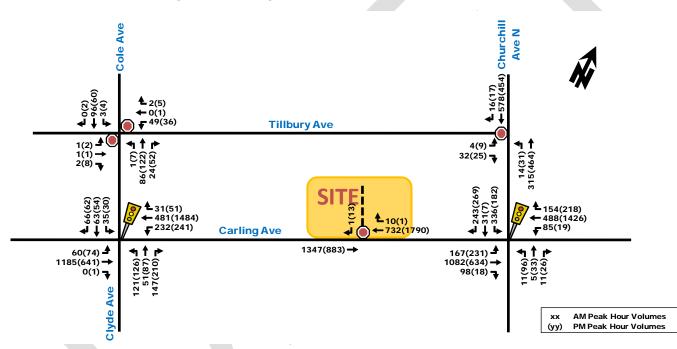
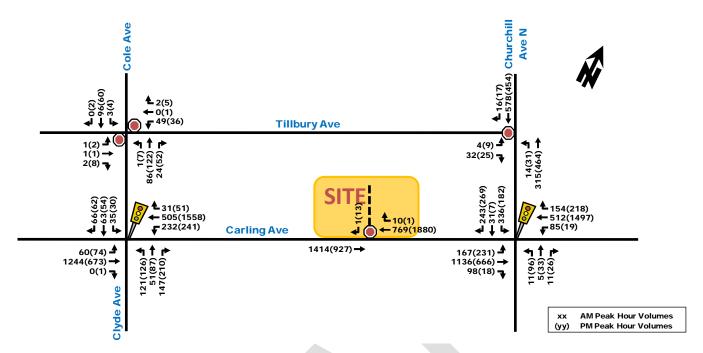


Figure 12: Future Background 2022 Traffic Volumes, with 20% East-West Reduction

Figure 13: Future Background 2027 Traffic Volumes, with 20% East-West Reduction



4. ANALYSIS

4.1. DEVELOPMENT DESIGN

Exempt – see **Section 2.3**. Note that surface parking spaces are proposed to be located on the north end of the site, while access to the underground parking is proposed on the northeast end of the site. Indoor and outdoor bicycle parking will be provided, based on the parking provisions. Pedestrian sidewalk facilities will be provided throughout the site and bus stops are located near the site (see **Section 2.1.2**).

4.2. PARKING

Exempt – see **Section 2.3**. Based on the concept plan, a total of 284 (243 underground and 24 surface) parking spaces are proposed for the development. A total of 138 bicycle parking spaces are proposed as well.

4.3. BOUNDARY STREET DESIGN

Exempt - see Section 2.3. This element will be explored in detail in the future Site Plan Application.

4.4. ACCESS INTERSECTION DESIGN

Exempt – see **Section 2.3**. As mentioned previously, the access is located on the west end of the site and can be used to access the surface and underground parking entrance at the back end of the building. STOP control is determined to be sufficient for vehicles exiting the development site.

4.5. TRANSPORTATION DEMAND MANAGEMENT

The TDM checklist is attached in Appendix F.

4.6. NEIGHBOURHOOD TRAFFIC MANAGEMENT

Within the study area, some drivers may elect to use Cole Ave and Tillbury Ave (local roads) in combination with Churchill Ave N (major collector) as part of their route to/from the proposed development. Based on the City of Ottawa's TIA Guidelines, the threshold for future traffic volumes in the peak direction is 120 veh/h for local roads and 600 veh/h for major collector roads. Based on the projected future background 2027 traffic volumes (**Figure 13**) and the net site-generated traffic (**Figure 9**):

- Along Tillbury Ave, the maximum number of traffic anticipated in the peak direction is 59 veh/h in the afternoon eastbound direction, which does not exceed the 120 veh/h threshold.
- Along Cole Ave, traffic volumes between Carling Ave and Tillbury Ave exceed the 120 veh/h threshold in peak directions for both existing and future conditions. However, there are no anticipated operational issues along Cole Ave, under normal conditions.
- Along Churchill Ave N, the maximum number of traffic anticipated in the peak direction is 579 veh/h in the morning southbound direction, which does not exceed the 600 veh/h threshold.

Therefore, changes to the existing classification of the study area roadways is not required.

4.7. TRANSIT

As previously mentioned in **Section 3.2.1**, Carling Ave is planned to be a transit priority corridor (continuous measures). In order to account for the anticipated addition of a designated bus lane along Carling Ave, the number of general purpose lanes was reduced to two lanes for the future background and future projected conditions analyzed in **Section 4.9**.

4.8. REVIEW OF NETWORK CONCEPT

Exempt - see Section 2.3.

4.9. INTERSECTION DESIGN

4.9.1. INTERSECTION CONTROL

STOP control will be provided for the proposed development access, which will be sufficient given the low site-generated traffic volumes.

4.9.2. INTERSECTION DESIGN

The Synchro 10 Trafficware was used to analyze intersection performance of intersections within the study area. Critical movements at each of the intersections were assessed based on either the movement with the highest volume-to-capacity ratio (for signalized intersections), or the movement experiencing the highest average delay (for unsignalized intersections). It should be noted that, as per the TIA Guidelines, the Peak Hour Factor (PHF) used for analysis was 0.9 in existing conditions and 1.0 in all future scenario conditions. Furthermore, the number of lanes along Carling Ave were reduced to two general purpose lanes in all future background and total projected scenarios. All Synchro report outputs for existing and future conditions have been provided in **Appendix G**.

Existing Conditions

Table 9 below summarizes the intersection performance of study area intersections, based on the existing conditions traffic volumes provided in **Figure 5**.

Table 9: Existing Conditions Intersection Performance

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moveme	nt	Intersection 'As a Whole'					
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c			
Churchill Ave N/Carling Ave (S)	D(F)	0.88(1.17)	SBL(WBT)	37.1(79.9)	B(F)	0.61(1.05)			
Clyde Ave/Cole Ave/Carling Ave (S)	C(C)	0.78(0.75)	NBL(NBL)	23.5(19.9)	B(B)	0.64(0.66)			
Carling Ave/Site Access (U)	A(B)	8.7(10.5)	SB(SB)	0.0(0.0)	-	-			
Cole Ave/Tillbury Ave (U)	B(B)	10.3(10.4)	WB(WB)	2.2(2.1)	-	-			
Tillbury Ave/Churchill Ave N (U)	B(B)	14.2(14.8)	EB(EB)	0.7(1.0)	-	-			
N. A. A. J. C.									

Note: Analysis of signalized intersections assumes a PHF of 0.90 and a saturation flow rate of 1800 veh/h/lane.

As shown in **Table 9**, the critical southbound left-turn movement at Churchill Ave N/Carling Ave operates at a LOS 'D' during the morning peak hour, while the critical westbound through movement operates at capacity with LOS 'F' during the afternoon peak hour. The critical northbound left-turn movement at Clyde Ave/Cole Ave/Carling Ave operates at a LOS 'C' during both morning and afternoon peak hour periods. Critical movements at the unsignalized intersections operate at a LOS 'B' or better during the morning and afternoon peak hour periods.

Future Background 2022 Conditions

Table 10 below summarizes the intersection performance of study area intersections, based on the future background 2022 conditions traffic volumes provided in **Figure 12**.

Table 10: Future Background 2022 Intersection Performance

	Weekday AM Peak (PM Peak)						
Intersection	Critical Movement			Intersection 'As a Whole'			
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c	
Churchill Ave N/Carling Ave (S)	D(E)	0.86(0.98)	SBL(WBT)	33.8(40.8)	A(D)	0.58(0.88)	
Clyde Ave/Cole Ave/Carling Ave (S)	C(B)	0.74(0.70)	NBL(NBL)	20.6(18.5)	B(B)	0.61(0.64)	
Carling Ave/Site Access (U)	A(B)	9.2(10.8)	SB(SB)	0.0(0.1)	-	-	
Cole Ave/Tillbury Ave (U)	B(B)	10.1(10.2)	WB(WB)	2.2(2.1)	-	-	
Tillbury Ave/Churchill Ave N (U)	B(B)	13.3(13.7)	EB(EB)	0.7(0.9)	-	-	

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As shown in **Table 10**, the critical SBL movement at Churchill Ave N/Carling Ave shows slight improvement in the morning and afternoon peak hour analysis compared to the existing conditions. Likewise, the critical movements at the Clyde Ave/Cole Ave/Carling Ave intersection show decrease in v/c ratios and operate at a LOS 'C' or better during morning and afternoon peak hour periods. Critical movements at the unsignalized intersections continue to operate at a LOS 'B' or better during the morning and afternoon peak hour periods.

Future Background 2027 Conditions

Table 11 below summarizes the intersection performance of study area intersections, based on the future background 2027 conditions traffic volumes provided in **Figure 13**.

⁽S) - Signalized intersection.

⁽U) - Unsignalized intersection.

⁽S) - Signalized intersection.

⁽U) - Unsignalized intersection.

Table 11: Future Background 2027 Interse	ection Performance

	Weekday AM Peak (PM Peak)					
Intersection	Critical Movement			Intersection 'As a Whole'		
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c
Churchill Ave N/Carling Ave (S)	D(F)	0.86(1.03)	SBL(WBT)	34.2(46.2)	A(E)	0.58(0.92)
Clyde Ave/Cole Ave/Carling Ave (S)	C(B)	0.74(0.70)	NBL(WBT)	21.5(19.0)	B(B)	0.63(0.67)
Carling Ave/Site Access (U)	A(B)	9.3(10.9)	SB(SB)	0.0(0.1)	-	-
Cole Ave/Tillbury Ave (U)	B(B)	10.1(10.2)	WB(WB)	2.2(2.1)	-	-
Tillbury Ave/Churchill Ave N (U)	B(B)	13.3(13.7)	EB(EB)	0.7(0.9)	-	-

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As shown in **Table 11**, the analysis results show slight increase in v/c ratios and intersection delays compared to the future background 2022 conditions. However, the WBT movement at the intersection of Churchill Ave N/Carling Ave is projected to operate at capacity during the afternoon peak hour.

Total Projected 2022 Conditions - Full Build-Out

The total projected 2022 traffic volumes were derived by superimposing the net site-generated traffic (**Figure 9**) onto future background 2022 traffic volumes (**Figure 12**). The resulting total projected 2022 traffic volumes are illustrated in **Figure 14**.

79(462) Churchill Cole Ave **←** 0(1) **4**9(36) **Tillbury Ave** 1(2) 🛦 (┪╽┡ 14(12) 2(8) **≜** 49(56) **←** 489(1486) **1**54(218) £ 16(43) **←** 492(1453) **₽** 85(19) 258(248) **Carling Ave** 732(1790) 60(76) 1374(894) → 168(235) 1186(645) → 0(1) **→** 1108(641) → 98(18) Clyde Ave **AM Peak Hour Volumes** PM Peak Hour Volumes

Figure 14: Total Projected 2022 Traffic Volumes

Table 12 below summarizes the intersection performance of study area intersections, based on the total projected 2022 conditions.

⁽S) - Signalized intersection.

⁽U) - Unsignalized intersection.

Table 12: Total Projected	2022 Performance a	nt Study Area Intersections

	Weekday AM Peak (PM Peak)					
Intersection	Critical Movement			Intersection 'As a Whole'		
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c
Churchill Ave N/Carling Ave (S)	D(F)	0.86(1.02)	SBL(NBL)	34.4(45.4)	A(D)	0.58(0.90)
Clyde Ave/Cole Ave/Carling Ave (S)	C(B)	0.74(0.70)	NBL(NBL)	21.4(18.6)	B(B)	0.62(0.64)
Carling Ave/Proposed Access (U)	A(B)	9.5(11.0)	SB(SB)	0.2(0.1)	-	-
Cole Ave/Tillbury Ave (U)	B(B)	10.2(10.2)	WB(WB)	2.1(2.0)	-	-
Tillbury Ave/Churchill Ave N (U)	B(B)	14.8(14.1)	EB(EB)	1.0(1.0)	-	-

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

Note that U-turn movements are analyzed in Synchro as left-turn movements. As such, U-turn traffic volumes generated by the future development were added to the left-turn traffic volumes in both the Synchro model and its corresponding traffic volumes figure (**Figure 14**). Overall, the analysis results in **Table 12** show slight increase in v/c ratios and intersection delays compared to the future background 2022 conditions. The WBT movement at the intersection of Churchill Ave N/Carling Ave is projected to operate at capacity during the afternoon peak hour.

Total Projected 2027 Conditions - Build-Out Plus Five Years

The total projected 2027 traffic volumes, shown in **Figure 15**, were derived by superimposing the net site-generated traffic (**Figure 9**) onto future background 2027 traffic volumes (**Figure 13**).

€16(17) ←579(462) Churchill Cole Ave **≜** 2(5) **←** 0(1) **Tillbury Ave** 14(12) 344(184) **♣** 66(62) **←** 63(54) **₹** 35(30) 49(56) 154(218) **1**6(43) ← 769(1880) **258(248) 85(19) Carling Ave** 60(76) **♣** 1245(677) **→** 0(1) **₹** 1441(938) 168(235) 📤 1162(673) → 98(18) **→ AM Peak Hour Volumes** (yy) PM Peak Hour Volumes

Figure 15: Total Projected 2027 Traffic Volumes

Table 13 below summarizes the intersection performance of study area intersections, based on the total projected 2027 conditions.

⁽S) - Signalized intersection.

⁽U) - Unsignalized intersection.

Table	12.	Total	Drojected	2027	Dorformanco	at Study Arga	Intersections
rabie	13.	Total	i Proiectea	2021	Periormance	at Study Area	milersections

	Weekday AM Peak (PM Peak)					
Intersection	Critical Movement			Intersection 'As a Whole'		
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c
Churchill Ave N/Carling Ave (S)	D(F)	0.86(1.06)	SBL(WBT)	34.9(52.2)	A(E)	0.59(0.94)
Clyde Ave/Cole Ave/Carling Ave (S)	C(B)	0.74(0.70)	NBL(WBT)	22.4(19.0)	B(B)	0.65(0.67)
Carling Ave/Proposed Access (U)	A(B)	9.6(11.0)	SB(SB)	0.2(0.1)	-	-
Cole Ave/Tillbury Ave (U)	B(B)	10.2(10.2)	WB(WB)	2.1(2.0)	-	-
Tillbury Ave/Churchill Ave N (U)	B(B)	14.8(14.1)	EB(EB)	1.0(1.0)	-	-

Note: Analysis of signalized intersections assumes a PHF of 1.00 and a saturation flow rate of 1800 veh/h/lane.

As shown in **Table 13**, there is a slight increase in v/c ratios compared to future background 2027 conditions. The WBT movement at the Churchill Ave N/Carling Ave intersection continues to operate at capacity during the afternoon peak hour period. However, note that the v/c ratio only slightly exceed the 1.00 threshold. As such, a simple potential mitigation measure to reduce the v/c ratio to an acceptable level is to adjust the phase timings of the intersection of Churchill Ave N/Carling Ave.

MMLOS Analysis

All required MMLOS analysis for signalized intersections and road segments within the study area will be provided in accordance to the TIA Guidelines requirements in the future Site Plan Application.

5. FINDINGS, CONCLUSIONS AND RECOMMENDATIONS

Based on the results summarized herein the following transportation related conclusions are offered:

Proposed Development

- The proposed single-phase development will consist of approximately 260 residential units within a 22storey high-rise apartment building and is expected to reach full build-out by 2022.
- A total of 260 underground parking spaces and 24 surface parking spaces are anticipated to be provided.
- A right-in/right-out access is proposed along the west side of the property, on the north side of Carling
 Ave.
- The proposed development is anticipated to generate approximately 75 vehicles/hour during both the
 morning and afternoon peak hour periods. However, due to the existing vehicle trips generated by the
 unpaved parking lot, the proposed development vehicle trips result in a net increase of approximately
 55 veh/h during the morning and afternoon peak hour periods.

Existing and Background Conditions

- In existing conditions, the SBL operates at a LOS 'D', while the WBT operates at capacity at the intersection of Churchill Ave N/Carling Ave, during the morning and afternoon peak hour periods, respectively. The NBL at the intersection of Clyde Ave/Cole Ave/Carling Ave operates at a LOS 'C' during both morning and afternoon peak hour periods. Critical movements at unsignalized intersections operate at a LOS 'B' or better during both morning and afternoon peak hours.
- Due to the absence of major other area developments near the subject site, traffic is not anticipated to grow significantly within the study area. Nonetheless, a 1% per year background growth rate was applied along Carling Ave, between horizon years 2022 and 2027.

⁽S) - Signalized intersection.

⁽U) - Unsignalized intersection.

- A designated bus lane is anticipated to be implemented along Carling Ave by year 2020, which will
 reduce the vehicle capacity significantly along Carling Ave, since a general-purpose lane in each travel
 direction will be replaced with a transit lane. However, the Carling Avenue Transit Priority Measures and
 Functional Design Report indicates that vehicle traffic along Carling Ave is expected to reduced by up to
 20% once the transit lanes are implemented. These changes are reflected in the Synchro analysis
 conducted for all future scenarios.
- The Synchro operational analysis of Future Background 2022 and 2027 conditions indicated the following:
 - o The SBL movement at the intersection of Churchill Ave N/Carling Ave operates at a LOS 'D' during the morning peak hour. While the WBT movement operates at a LOS 'E' for future background 2022 conditions and LOS 'F' for future background 2027 conditions during the afternoon peak hour.
 - The critical movements at the intersection of Clyde Ave/Cole Ave/Carling Ave operate at a LOS 'C' or better during the morning and afternoon peak hour periods.
 - Critical movements at all unsignalized intersections operate at a LOS 'B' or better during morning and afternoon peak hour periods.

Projected Conditions

- Analysis of Total Projected 2022 and 2027 conditions indicated results similar to Future Background 2022 and 2027 conditions, with slight increase in v/c ratios and intersection delays.
- Although critical movements at the intersection of Churchill Ave N/Carling Ave operate at capacity during the afternoon peak hour, the volume-to-capacity ratio exceeds the threshold only slightly. As such, the v/c ratios may be reduced to acceptable levels by adjusting the intersection's phase timings.
- Since site-generated traffic may use local roads (Cole Ave and Tillbury Ave) and a major collector road (Churchill Ave N) as part of their access route to/from the future development, anticipated future traffic volumes along these roadways was compared against the thresholds set by the TIA Guidelines. It was determined that changes to the existing classification of the existing study area roadways was not required given the very modest volume increases of 18 veh/h.
- MMLOS analysis for signalized intersections and road segments within the study area will be provided in the future Site Plan Application.

Based on the foregoing, the proposed residential development causes a slight variation to the adjacent transportation network and is recommended to proceed from a transportation perspective.

Appendix A Screening Form and City Comments



City of Ottawa 2017 TIA Guidelines Date

TIA Screening Form Project

Project Number 477272 - 01000

27-Aug-19

1655 Carling Avenue

Results of Screening	Yes/No
Development Satisfies the Trip Generation Trigger	Yes
Development Satisfies the Location Trigger	Yes
Development Satisfies the Safety Trigger	Yes

Module 1.1 - Description of Proposed Development	t en
Municipal Address	1655 Carling Avenue (Ottawa, ON)
Description of location	To replace the unpaved parking lot west of Hakim Optical
Land Use	Residential apartments tower
Development Size	260 residential units
Number of Accesses and Locations	1 Existing Access, 65 m west of Carling/Churchill intersection
Development Phasing	1 Phase
Buildout Year	2021
Sketch Plan / Site Plan	See attached

Module 1.2 - Trip Generation Trigger	
Land Use Type	Townhomes or Apartments
Development Size	260 Units
Trip Generation Trigger Met?	Yes

Module 1.3 - Location Triggers		
Development Proposes a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit, or Spine Bicycle Networks (See Sheet 3)	No	Development to continue using existing driveway
Development is in a Design Priority Area (DPA) or Transit- oriented Development (TOD) zone. (See Sheet 3)	Yes	Carling Avenue is designated as an Arterial Mainstreet
Location Trigger Met?	Yes	

Module 1.4 - Safety Triggers		
Posted Speed Limit on any boundary road	<80	km/h
Horizontal / Vertical Curvature on a boundary street limits sight lines at a proposed driveway	No	
A proposed driveway is within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions) or within auxiliary	Yes	Within 150 m of the Carling Ave/Chuchill Ave N signalized intersection
lanes of an intersection;		
A proposed driveway makes use of an existing median break that serves an existing site	No	
There is a documented history of traffic operations or safety		
concerns on the boundary streets within 500 m of the	No	
development		
The development includes a drive-thru facility	No	
Safety Trigger Met?	Yes	



Ansari, Basel

Subject: 1655 Carling Ave - Strategy Report

Responses to City comments are shown in red below.

From: Dubyk, Wally <Wally.Dubyk@ottawa.ca>
Sent: Friday, September 27, 2019 8:35 AM
To: Ansari, Basel <Basel.Ansari@parsons.com>
Cc: Deiaco, Simon <Simon.Deiaco@ottawa.ca>

Subject: [EXTERNAL] 1655 Carling Ave - Forecasting response

Basel,

Please review the following Forecasting response;

1655 Carling Avenue TIA Forecasting Report, Received September 12, 2019

Comments

General (Site Plan related)

Carling Avenue is designated as an Arterial road within the City's Official Plan with a ROW protection limit of 44.5 metres. The ROW protection limit and the offset distance (22.25 metres) are to be dimensioned from the existing centerline of pavement and shown on the drawings.

ROW interpretation – Land for a road widening will be taken equally from both sides of a road, measured from the centreline in existence at the time of the widening if required by the City. The centreline is a line running down the middle of a road surface, equidistant from both edges of the pavement. In determining the centreline, paved shoulders, bus lay-bys, auxiliary lanes, turning lanes and other special circumstances are not included in the road surface.

All underground and above ground building footprints and permanent walls need to be shown on the plan to confirm that any permanent structure does not extend either above or below into the future road widening protection limits.

The concrete sidewalks should be 2.0 metres in width and be continuous and depressed through the proposed accesses (please refer to the City's sidewalk and curb standard drawing SC7.1 for <u>unsignalized entrance</u>).

The closure of an existing private approach shall reinstate the sidewalk, shoulder, curb and boulevard to City standards.

Noted. Architect has been made aware of these comments. Updated concept plan has been provided in the report.

Transportation Engineering

Include the Cole Avenue/Tillbury Avenue and Tillbury Avenue/Churchill Avenue N intersections in the analysis given that they are a possible option for those traveling eastbound on Carling Avenue to access the development. Account for this possibility in Figures 7-9.

The Cole Avenue/Tillbury Avenue and Tillbury Avenue/Churchill Avenue intersections have been included in the report.

Adjust the target modal shares in Table 5. The existing mode shares in Table 5 are lower than what is reflected in the 2011 OD Survey for the non-motorized mode and too high for auto driver. Secondly, given the proposed transit improvements to Carling Avenue described in the report, consider a future higher transit mode share target for this development.

Target mode shares have been adjusted.

Display the walking and cycling modes separately in Tables 4 and 5.

Walking and cycling modes are now displayed in Table 5 as provided by the 2011 OD Survey. Table 4 provides a combined percentage as the 2009 TRANS Report does not provide separated walking and cycling mode shares.

To account for net change in development trips to this location as discussed in section 3.1.1, an assumption of existing trips is not accepted. Conduct a traffic count to reduce the net total vehicle trips.

Traffic counts have been conducted at the existing parking lot. Report analysis has been updated to include these volumes.

Correct the addition errors in Table 7 and reflect these changes in Figures 8 and 9. Corrections made.

Provide justification for the trip distribution shown in Section 3.1.2.

The trip distribution was obtained from the 2011 OD Survey. Nonetheless, justification for the distribution of the site-generated traffic volumes has been provided in Section 3.1.2.

Demand rationalization will be required in the buildout year as well as the ultimate horizon assuming a 1% background growth rate and the elimination of one travel lane on Carling Avenue.

Demand rationalization has been provided in Section 3.3 of the report.

Modules 4.1 - 4.4 are required, however, the review will be focused on the ability of the site to accommodate the use. Reference the TIA Guidelines Structure of a TIA Study. Module 4.6 is also required as part of this application.

Modules 4.1 to 4.4 will be explored in more detail in the site plan application. However, brief reviews of what is expected of the site have been provided. A discussion has been provided for module 4.6.

Exemption of Module 4.8 is accepted if the site plan application is submitted with the rezoning application. Otherwise, a review of the maximum possible trips that would be possible with the proposed rezoning must be included.

As shown by the Forecasting Section of the report, the development is not anticipated to generate more than 200 person-trips during peak hours. Therefore, review of network concept is not required.

Traffic Signal Operations

Provide accurate vehicular activity at the existing parking lot.

Traffic counts have been conducted for the existing parking lot and included in the analysis.

Ensure that transit priority measures along Carling Avenue are taken into account when completing the traffic impact analysis.

The transit mode share has been increased in order to account for the transit priority measures.

Left turn lane storage analysis is required.

There are no left-turns at the access. The access allows right-in/right-out movements only.

Provide a more detailed review of the Carling Avenue/Clyde Avenue intersection collision date and provide recommended mitigation measures.

A more detailed review of the collisions at Carling Avenue/Clyde Avenue has been provided in the report.

Thank you,

Wally Dubyk
Project Manager - Transportation Approvals
Development Review, Central & South Branches
613-580-2424 x13783

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3

Appendix B Transit Route Maps





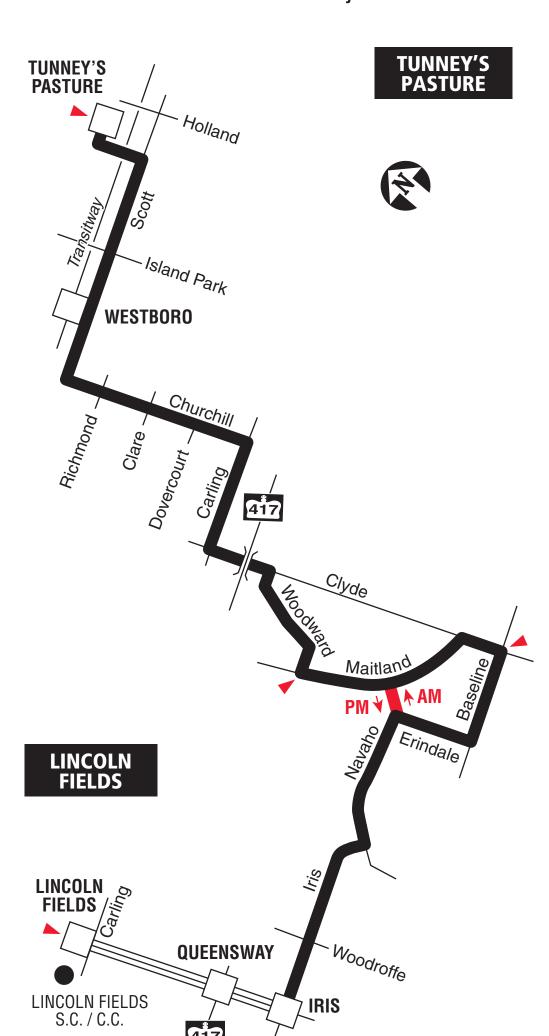


TUNNEY'S PASTURE LINCOLN FIELDS

Local

Monday to Saturday / Lundi au samedi

No service Sat. eve. or all day Sunday / Aucun service le soir le sam. ou toute la journée dimanche



■ Legend • *Légende* •



Transitway & Station



Peak Periods only /
Périodes de pointe seulement
Timepoint / Heures de passage

2017.04



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> Effective April 24, 2017 En vigueur 24 avril 2017

CC Transpo

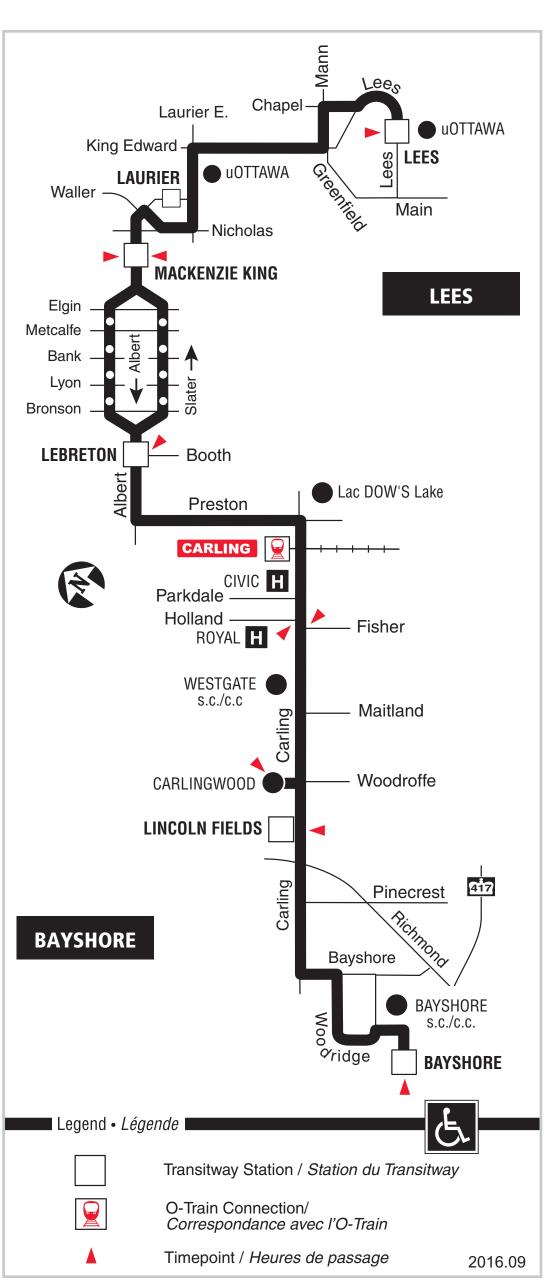
INFO 613-741-4390 octranspo.com



85 LEES BAYSHORE

7 days a week / 7 jours par semaine

All day service Service toute la journée



Effective / En vigueur Sept. 4 sept. 2016

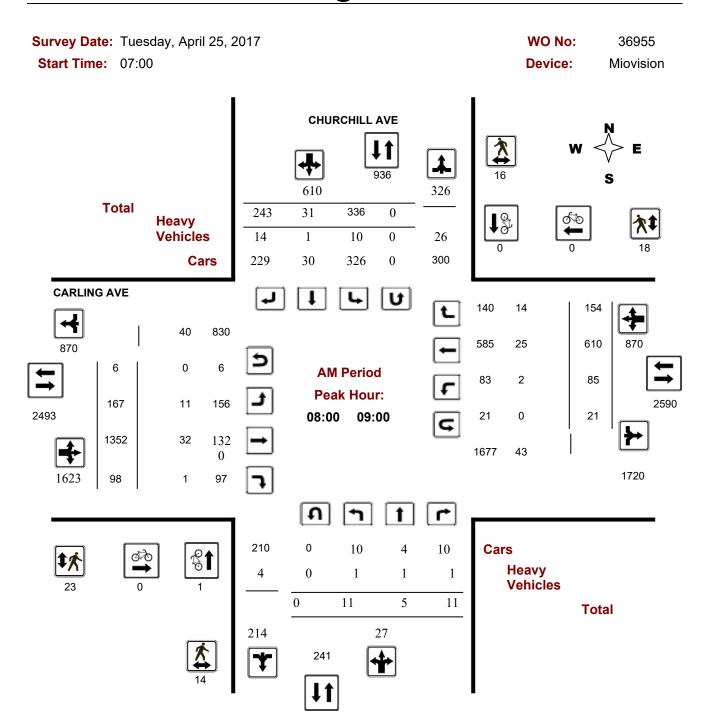
Appendix C Traffic Data



Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CHURCHILL AVE



Comments

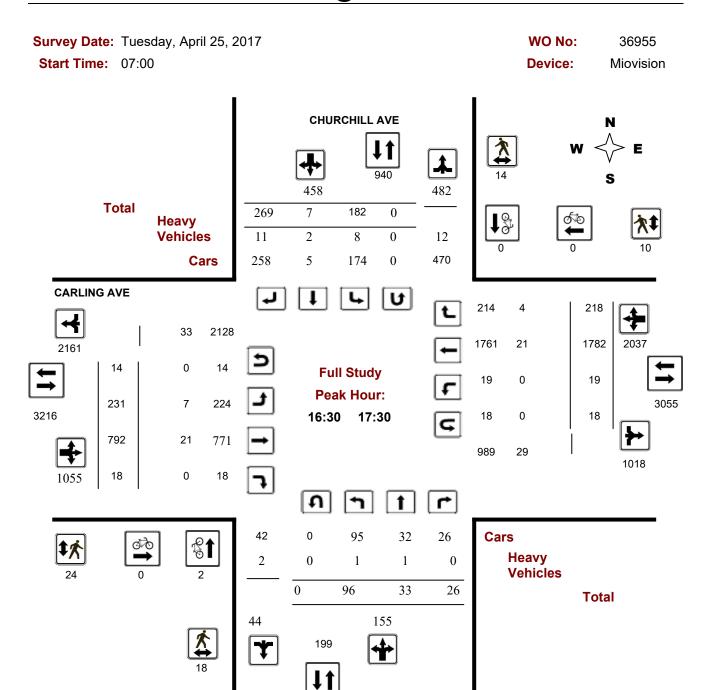
2019-Sep-04 Page 1 of 4



Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CHURCHILL AVE



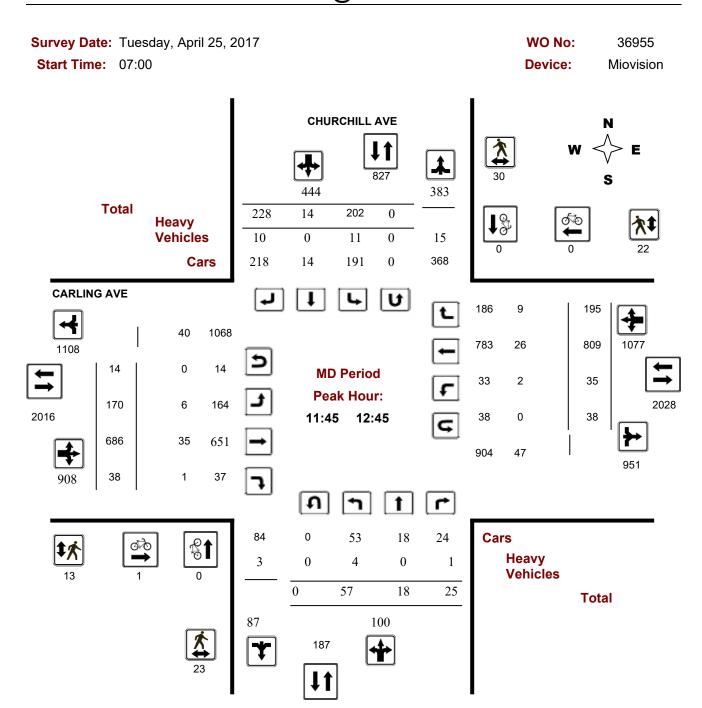
Comments

2019-Sep-04 Page 2 of 4



Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CHURCHILL AVE

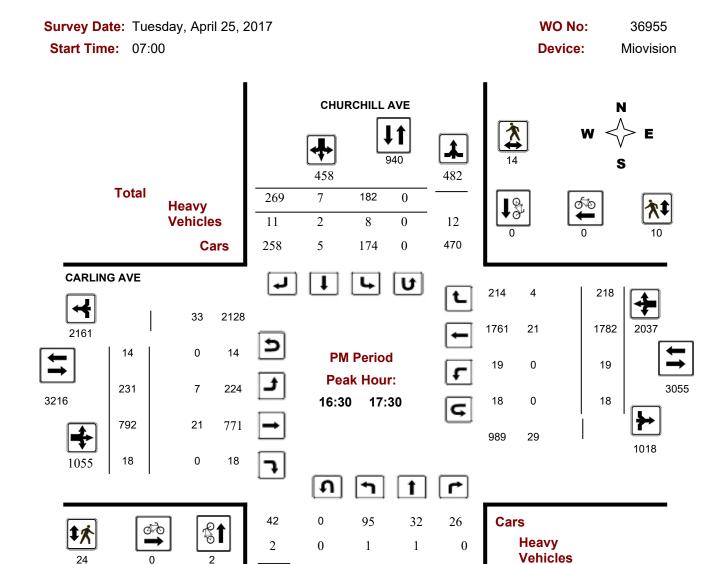


Comments



Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CHURCHILL AVE



Comments

2019-Sep-04 Page 4 of 4

0

44

96

199

33

155

*

26

Total

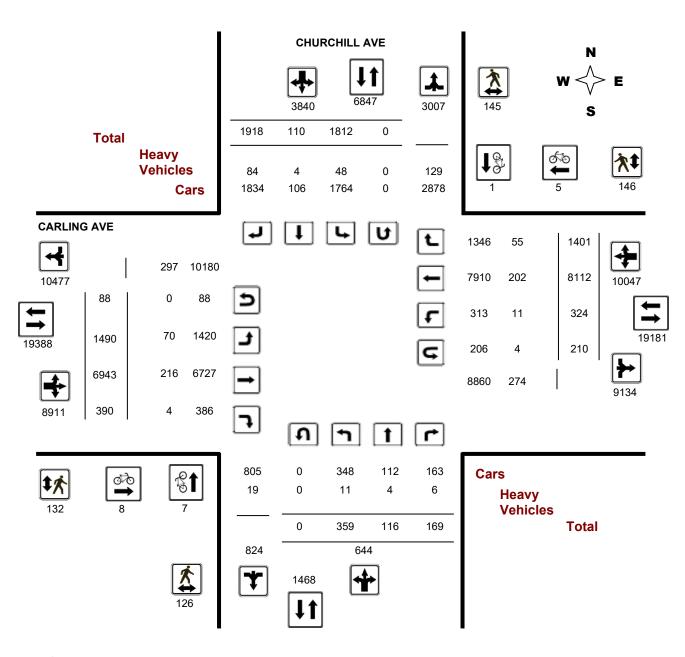


Turning Movement Count - Full Study Diagram

CARLING AVE @ CHURCHILL AVE

Survey Date: Tuesday, April 25, 2017 WO#: 36955

Device: Miovision



Comments



Work Order

36955

Turning Movement Count - Full Study Summary Report

CARLING AVE @ CHURCHILL AVE

Survey Date: Tuesday, April 25, 2017

Total Observed U-Turns

AADT Factor

.90

Northbound: 0

Eastbound:

Southbound: 0

Westbound: 210

Full Study

88

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_	N	lorthbo	ound		S	outhb	ound		_		Eastbo	ound			Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	14	6	9	29	252	11	180	443	472	144	1213	76	1433	55	416	118	589	2022	2494
08:00 09:00	11	5	11	27	336	31	243	610	637	167	1352	98	1617	85	610	154	849	2466	3103
09:00 10:00	24	5	25	54	224	19	212	455	509	171	790	77	1038	47	618	151	816	1854	2363
11:30 12:30	54	16	23	93	204	10	237	451	544	164	637	34	835	35	823	188	1046	1881	2425
12:30 13:30	46	11	30	87	188	13	241	442	529	192	749	51	992	45	713	185	943	1935	2464
15:00 16:00	38	18	21	77	220	12	259	491	568	203	709	22	934	24	1403	174	1601	2535	3103
16:00 17:00	87	24	31	142	175	11	276	462	604	239	740	17	996	22	1762	208	1992	2988	3592
17:00 18:00	85	31	19	135	213	3	270	486	621	210	753	15	978	11	1767	223	2001	2979	3600
Sub Total	359	116	169	644	1812	110	1918	3840	4484	1490	6943	390	8823	324	8112	1401	9837	18660	23144
U Turns				0				0	0				88				210	298	298
Total	359	116	169	644	1812	110	1918	3840	4484	1490	6943	390	8911	324	8112	1401	10047	18958	23442
EQ 12Hr	499	161	235	895	2519	153	2666	5338	6233	2071	9651	542	12386	450	11276	1947	13965	26351	32584
Note: These \	/alues ar	e calcul	ated by	/ multipl	ying the	totals b	y the ap	opropriat	e expans	sion fac	tor.			1.39					
AVG 12Hr	449	145	211	806	2267	138	2399	4804	5610	1864	8686	488	11148	405	10148	1753	12569	23717	29327
Note: These \	olumes :	are calc	ulated	by multi	plying th	e Equiv	/alent 1	2 hr. tota	ls by the	AADT	factor.			.90					
AVG 24Hr	588	190	277	1055	2970	180	3143	6293	7348	2442	11378	639	14603	531	13294	2296	16465	31068	38416
Note: These \	olumes :	are calc	ulated	by multi	plying th	e Avera	age Dail	y 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



Turning Movement Count - 15 Minute Summary Report

CARLING AVE @ CHURCHILL AVE

Survey Date: Tuesday, April 25, 2017

Total Observed U-Turns

Northbound: 0 Southbound: 0 Eastbound: 88 Westbound: 210

CHURCHILL AVE

CARLING AVE

36955

		N	orthbou	und		Sou	ıthbour	nd			Eas	stbound			We	stbound	d			
Time I	Pariod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00		3	2	2	7	48	2	36	86	93	23	227	16	267	14	81	25	123	390	483
07:15	07:30	1	0	1	2	43	1	40	84	86	26	295	14	337	12	94	32	140	477	563
07:30	07:45	6	1	3	10	76	4	54	134	144	39	328	15	383	15	99	25	148	531	675
07:45	08:00	4	3	3	10	85	4	50	139	149	56	363	31	451	14	142	36	197	648	797
08:00	08:15	4	0	5	9	85	7	67	159	168	29	370	26	426	11	108	35	158	584	752
08:15	08:30	1	1	2	4	73	5	53	131	135	36	330	27	395	24	162	42	235	630	765
08:30	08:45	1	2	2	5	95	9	71	175	180	50	322	18	392	24	165	38	234	626	806
08:45	09:00	5	2	2	9	83	10	52	145	154	52	330	27	410	26	175	39	243	653	807
09:00	09:15	4	2	8	14	86	8	51	145	159	46	211	26	285	9	146	32	196	481	640
09:15	09:30	5	1	3	9	34	4	43	81	90	39	213	26	280	12	147	40	205	485	575
09:30	09:45	6	1	9	16	59	5	58	122	138	44	203	12	263	16	146	42	211	474	612
09:45	10:00	9	1	5	15	45	2	60	107	122	42	163	13	221	10	179	37	236	457	579
11:30	11:45	9	1	7	17	47	0	60	107	124	44	149	7	201	8	196	47	264	465	589
11:45	12:00	15	3	5	23	63	3	58	124	147	41	165	6	217	8	195	45	263	480	627
12:00	12:15	17	6	9	32	54	2	63	119	151	37	146	12	199	12	229	48	296	495	646
12:15	12:30	13	6	2	21	40	5	56	101	122	42	177	9	232	7	203	48	267	499	621
12:30	12:45	12	3	9	24	45	4	51	100	124	50	198	11	260	8	182	54	251	511	635
12:45	13:00	17	3	10	30	46	2	69	117	147	44	164	12	224	18	176	42	243	467	614
13:00	13:15	9	4	6	19	47	5	62	114	133	51	173	14	240	7	197	46	259	499	632
13:15	13:30	8	1	5	14	50	2	59	111	125	47	214	14	278	12	158	43	227	505	630
15:00	15:15	10	3	6	19	53	3	61	117	136	44	186	6	240	9	286	32	334	574	710
15:15	15:30	11	4	5	20	51	7	69	127	147	47	172	5	229	5	317	38	367	596	743
15:30	15:45	6	9	4	19	63	0	70	133	152	68	175	7	254	5	354	48	414	668	820
15:45	16:00	11	2	6	19	53	2	59	114	133	44	176	4	227	5	446	56	510	737	870
16:00	16:15	25	4	9	38	47	2	79	128	166	63	175	4	245	7	394	45	449	694	860
16:15	16:30	26	4	9	39	54	4	63	121	160	57	182	3	243	4	476	50	533	776	936
16:30	16:45	16	6	9	31	45	2	55	102	133	59	194	2	262	6	408	59	481	743	876
16:45	17:00	20	10	4	34	29	3	79	111	145	60	189	8	259	5	484	54	547	806	951
17:00	17:15	38	10	11	59	43	1	68	112	171	67	205	4	278	3	422	54	482	760	931
17:15	17:30	22	7	2	31	65	1	67	133	164	45	204	4	256	5	468	51	527	783	947
17:30	17:45	13	6	1	20	48	0	62	110	130	58	161	3	228	2	440	49	493	721	851
17:45	18:00	12	8	5	25	57	1	73	131	156	40	183	4	229	1	437	69	514	743	899
TOTAL		250	440	400	044	4040	440	4040	20.40	4404	4 400	0040	200	0044	204	0446	140	4 400	47 40050	00440

Note: U-Turns are included in Totals.

116

1812

110

359

TOTAL:

Comment:

390

324

8911

8112 1401 **10047 18958 23442**

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1918 **3840 4484** 1490 6943



Turning Movement Count - Cyclist Volume Report

Work Order 36955

CARLING AVE @ CHURCHILL AVE

Count Date: Tuesday, April 25, 2017 Start Time: 07:00

CHURCHILL AVE

CARLING AVE

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	0	0	0	2	1	3	3
08:00 09:00	1	0	1	0	0	0	1
09:00 10:00	2	1	3	2	0	2	5
11:30 12:30	0	0	0	0	1	1	1
12:30 13:30	0	0	0	3	1	4	4
15:00 16:00	2	0	2	1	1	2	4
16:00 17:00	0	0	0	0	1	1	1
17:00 18:00	2	0	2	0	0	0	2
Total	7	1	8	8	5	13	21

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.



W.O.

36955

Turning Movement Count - Heavy Vehicle Report

CARLING AVE @ CHURCHILL AVE

Survey Date: Tuesday, April 25, 2017

CHURCHILL AVE CARLING AVE

		Northb	ound		(Southb	ound	_			Eastb	ound		١	Vestbo	ound				
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	0	0	0	0	6	0	8	14	14	16	22	0	38	1	22	9	33	71	85
08:00	09:00	1	1	1	3	10	1	14	25	28	11	32	1	44	2	25	14	41	85	113
09:00	10:00	2	1	3	6	7	0	9	16	22	9	34	2	45	6	25	5	38	83	105
11:30	12:30	5	0	0	5	10	0	10	20	25	10	29	1	40	2	34	13	49	89	114
12:30	13:30	1	0	1	2	2	0	13	15	17	6	35	0	41	0	31	3	35	76	93
15:00	16:00	0	1	1	2	4	0	11	15	17	7	24	0	31	0	26	5	31	62	79
16:00	17:00	1	0	0	1	2	3	9	14	15	7	23	0	30	0	23	3	26	56	71
17:00	18:00	1	1	0	2	7	0	10	17	19	4	17	0	21	0	16	3	19	40	59
Sub 1	Γotal	11	4	6	21	48	4	84	136	157	70	216	4	290	11	202	55	272	562	719
U-Turn	s (Heav	y Veh	icles)		0				0	0				0				4	4	4
Tot	al	11	4	6	0	48	4	84	136	157	70	216	4	290	11	202	55	276	566	723

Heavy Vehicles include Buses, Single-Unit Trucks and Articulated Trucks. Further, they ARE included in the Turning Movement Count Summary.



Work Order

Turning Movement Count - Pedestrian Volume Report

CARLING AVE @ CHURCHILL AVE Count Date: Tuesday, April 25, 2017 **Start Time:** 07:00 NB Approach SB Approach EB Approach WB Approach Time Period **Grand Total** Total **Total** (E or W Crossing) (E or W Crossing) (N or S Crossing) (N or S Crossing) 07:00 07:15 07:15 07:30 07:30 07:45 07:45 08:00 07:00 08:00 08:00 08:15 08:15 08:30 08:30 08:45 08:45 09:00 08:00 09:00 09:00 09:15 09:15 09:30 09:30 09:45 09:45 10:00 09:00 10:00 11:30 11:45 11:45 12:00 12:00 12:15 12:15 12:30 11:30 12:30 12:30 12:45 12:45 13:00 13:00 13:15 13:15 13:30 12:30 13:30 15:00 15:15 15:15 15:30 15:30 15:45 15:45 16:00 15:00 16:00 16:00 16:15 16:15 16:30 16:30 16:45 16:45 17:00 16:00 17:00 17:00 17:15 17:15 17:30 17:30 17:45 17:45 18:00

Comment:

17:00 18:00

Total

2019-Sep-04 Page 1 of 1







Turning Movement Count - 15 Min U-Turn Total Report

CARLING AVE @ CHURCHILL AVE

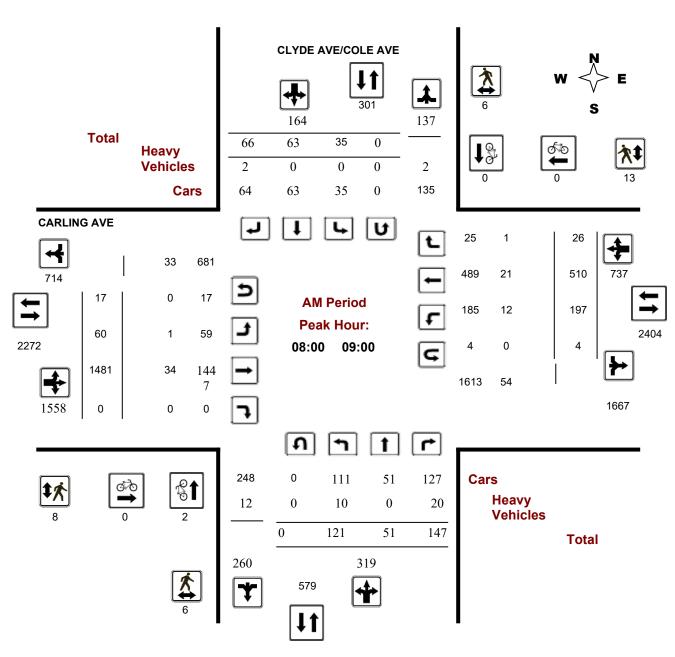
Survey Date:	Т	uesday, April 25,	2017			
Time Pe	riod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	1	3	4
07:15	07:30	0	0	2	2	4
07:30	07:45	0	0	1	9	10
07:45	08:00	0	0	1	5	6
08:00	08:15	0	0	1	4	5
08:15	08:30	0	0	2	7	9
08:30	08:45	0	0	2	7	9
08:45	09:00	0	0	1	3	4
09:00	09:15	0	0	2	9	11
09:15	09:30	0	0	2	6	8
09:30	09:45	0	0	4	7	11
09:45	10:00	0	0	3	10	13
11:30	11:45	0	0	1	13	14
11:45	12:00	0	0	5	15	20
12:00	12:15	0	0	4	7	11
12:15	12:30	0	0	4	9	13
12:30	12:45	0	0	1	7	8
12:45	13:00	0	0	4	7	11
13:00	13:15	0	0	2	9	11
13:15	13:30	0	0	3	14	17
15:00	15:15	0	0	4	7	11
15:15	15:30	0	0	5	7	12
15:30	15:45	0	0	4	7	11
15:45	16:00	0	0	3	3	6
16:00	16:15	0	0	3	3	6
16:15	16:30	0	0	1	3	4
16:30	16:45	0	0	7	8	15
16:45	17:00	0	0	2	4	6
17:00	17:15	0	0	2	3	5
17:15	17:30	0	0	3	3	6
17:30	17:45	0	0	6	2	8
17:45	18:00	0	0	2	7	9
Tota	1	0	0	88	210	298
-						



Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016 WO No: 35669
Start Time: 07:00 Device: Miovision



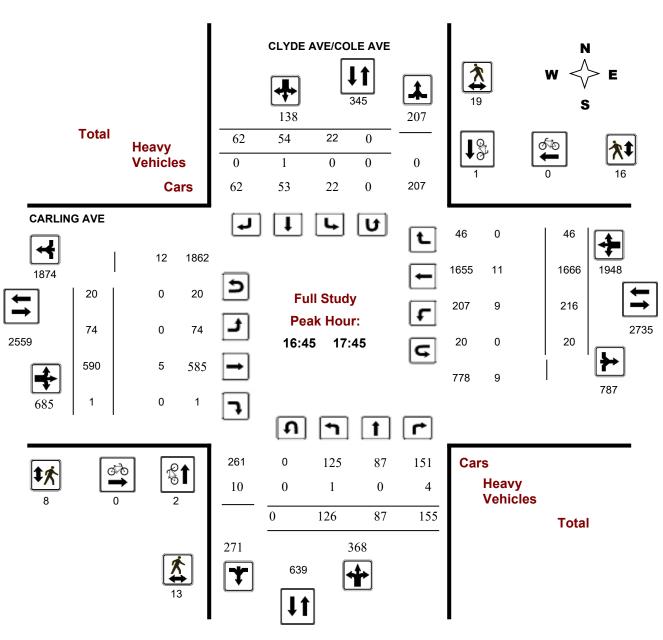
Comments



Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016 WO No: 35669
Start Time: 07:00 Device: Miovision



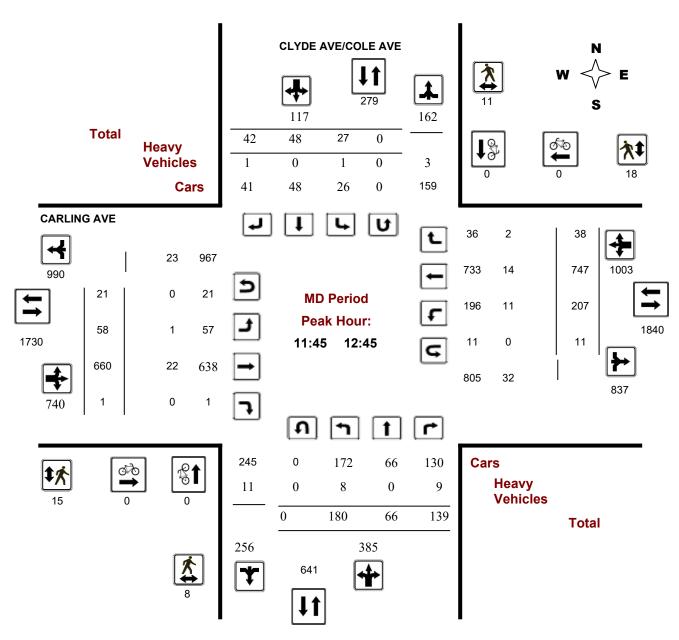
Comments



Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016 WO No: 35669
Start Time: 07:00 Device: Miovision



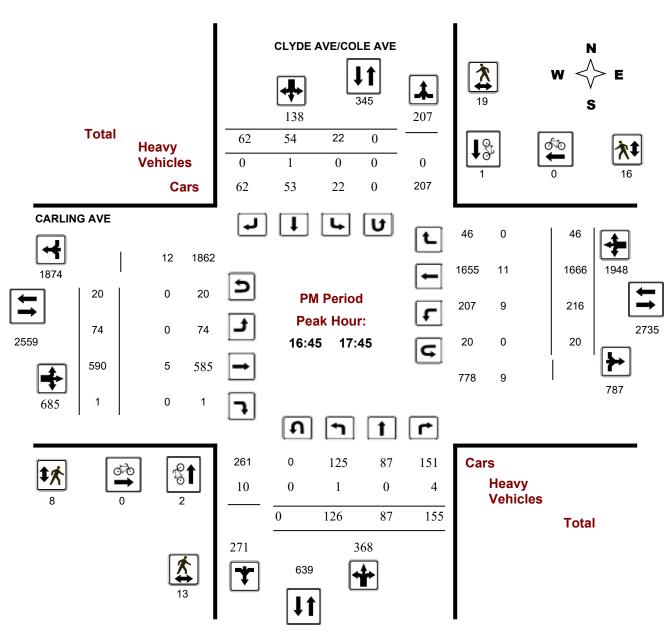
Comments



Turning Movement Count - Full Study Peak Hour Diagram

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016 WO No: 35669
Start Time: 07:00 Device: Miovision



Comments

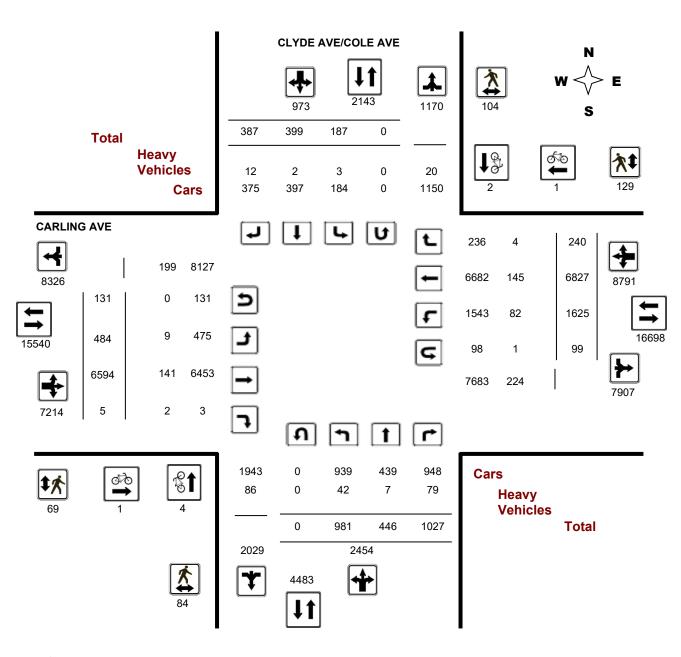


Turning Movement Count - Full Study Diagram

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016 WO#: 35669

Device: Miovision



Comments



Work Order

35669

Turning Movement Count - Full Study Summary Report

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 201

Total Observed U-Turns

AADT Factor

0 Northbound:

Southbound: 0

1.00

Eastbound:

Westbound: 131 99

Full Study

		C	CLYDE	AVE/	COLE	AVE						С	ARLIN	G AVI	Ξ				
-	N	orthb	ound		S	outhb	ound		_		Eastbo	und			Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	47	19	80	146	23	27	31	81	227	38	1209	0	1247	180	319	13	512	1759	1986
08:00 09:00	121	51	147	319	35	63	66	164	483	60	1481	0	1541	197	510	26	733	2274	2757
09:00 10:00	108	31	97	236	16	40	39	95	331	61	784	0	845	191	540	12	743	1588	1919
11:30 12:30	159	69	137	365	22	50	47	119	484	54	642	2	698	214	743	33	990	1688	2172
12:30 13:30	129	52	133	314	29	47	50	126	440	77	689	0	766	187	672	27	886	1652	2092
15:00 16:00	165	56	139	360	16	43	44	103	463	41	657	2	700	221	1117	34	1372	2072	2535
16:00 17:00	120	68	147	335	20	73	41	134	469	75	540	0	615	228	1381	51	1660	2275	2744
17:00 18:00	132	100	147	379	26	56	69	151	530	78	592	1	671	207	1545	44	1796	2467	2997
Sub Total	981	446	1027	2454	187	399	387	973	3427	484	6594	5	7083	1625	6827	240	8692	15775	19202
U Turns				0				0	0				131				99	230	230
Total	981	446	1027	2454	187	399	387	973	3427	484	6594	5	7214	1625	6827	240	8791	16005	19432
EQ 12Hr	1364	620	1428	3411	260	555	538	1352	4763	673	9166	7	10027	2259	9490	334	12219	22246	27009
Note: These	values a	re calcu	ılated by	/ multiply	ing the	totals b	y the ap	propriate	e expans	ion fac	tor.			1.39					
AVG 12Hr	1364	620	1428	3411	260	555	538	1352	4763	673	9166	7	10027	2259	9490	334	12219	22246	27009
Note: These	volumes	are cal	culated	by multip	olying th	e Equiv	alent 12	2 hr. tota	ls by the	AADT	factor.			1.00					
AVG 24Hr	1786	812	1870	4468	341	727	705	1772	6240	881	12007	9	13136	2959	12431	437	16008	29144	35384
Note: These	volumes	are cal	culated	by multip	olying th	e Avera	ige Dail	y 12 hr. t	totals by	12 to 2	4 expans	sion fac	ctor.	1.31					

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



Turning Movement Count - 15 Minute Summary Report

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016

Total Observed U-Turns

Northbound: 0 Southbound: 0 Eastbound: 131 Westbound: 99

CLYDE AVE/COLE AVE

CARLING AVE

					(VE/C								JAKI	LING A						
		N	lorthbo	und		So	uthbour	ıd	•	0.70	Eas	stbound		_	We	stbound		147	0.70	
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	6	2	13	21	3	1	3	7	28	8	221	0	229	37	46	4	89	318	346
07:15	07:30	10	5	10	25	5	7	9	21	46	10	281	0	294	50	77	4	133	427	473
07:30	07:45	13	4	21	38	8	6	8	22	60	9	344	0	357	50	86	1	141	498	558
07:45	08:00	18	8	36	62	7	13	11	31	93	11	363	0	378	43	110	4	158	536	629
08:00	08:15	34	6	29	69	6	12	17	35	104	15	393	0	409	41	112	5	159	568	672
08:15	08:30	33	16	39	88	13	10	21	44	132	12	383	0	398	48	116	2	166	564	696
08:30	08:45	23	14	49	86	12	19	16	47	133	16	399	0	421	43	146	7	198	619	752
08:45	09:00	31	15	30	76	4	22	12	38	114	17	306	0	330	65	136	12	214	544	658
09:00	09:15	25	6	18	49	8	13	13	34	83	17	269	0	290	64	138	3	206	496	579
09:15	09:30	30	7	28	65	4	16	8	28	93	19	208	0	229	55	117	4	178	407	500
09:30	09:45	24	9	30	63	1	9	8	18	81	14	152	0	172	37	130	3	171	343	424
09:45	10:00	29	9	21	59	3	2	10	15	74	11	155	0	170	35	155	2	194	364	438
11:30	11:45	18	16	30	64	4	18	12	34	98	12	169	1	189	50	155	4	212	401	499
11:45	12:00	63	14	33	110	4	6	15	25	135	15	149	1	172	55	195	9	261	433	568
12:00	12:15	39	20	40	99	7	11	12	30	129	13	148	0	168	57	204	9	274	442	571
12:15	12:30	39	19	34	92	7	15	8	30	122	14	176	0	192	52	189	11	254	446	568
12:30	12:45	39	13	32	84	9	16	7	32	116	16	187	0	208	43	159	9	214	422	538
12:45	13:00	30	11	41	82	8	12	16	36	118	19	162	0	187	36	188	6	236	423	541
13:00	13:15	29	16	31	76	5	7	9	21	97	24	172	0	198	57	151	8	220	418	515
13:15	13:30	31	12	29	72	7	12	18	37	109	18	168	0	191	51	174	4	231	422	531
15:00	15:15	38	6	35	79	4	10	13	27	106	9	181	1	195	52	224	6	285	480	586
15:15	15:30	46	15	39	100	2	13	10	25	125	11	145	0	161	60	262	9	335	496	621
15:30	15:45	29	15	33	77	4	10	9	23	100	6	181	0	190	61	303	13	382	572	672
15:45	16:00	52	20	32	104	6	10	12	28	132	15	150	1	171	48	328	6	386	557	689
16:00	16:15	33	15	33	81	3	16	10	29	110	16	150	0	169	58	387	10	460	629	739
16:15	16:30	29	23	36	88	10	28	9	47	135	26	130	0	157	43	261	13	321	478	613
16:30	16:45	32	19	38	89	5	17	7	29	118	21	138	0	162	72	309	18	402	564	682
16:45	17:00	26	11	40	77	2	12	15	29	106	12	122	0	137	55	424	10	489	626	732
17:00	17:15	38	35	50	123	4	12	18	34	157	19	171	0	201	52	461	9	527	728	885
17:15	17:30	39	15	35	89	8	12	17	37	126	18	150	0	169	58	414	12	488	657	783
17:30	17:45	23	26	30	79	8	18	12	38	117	25	147	1	178	51	367	15	444	622	739
17:45	18:00	32	24	32	88	6	14	22	42	130	16	124	0	142	46	303	8	363	505	635
TOTAL	.:	981	446	1027	2454	187	399	387	973	3427	484	6594	5	7214	1625	6827	240	879	91 16005	19432

Note: U-Turns are included in Totals.

Comment:



Turning Movement Count - Cyclist Volume Report

Work Order 35669

CARLING AVE @ CLYDE AVE/COLE AVE

Count Date: Wednesday, January 27, 2016

Start Time: 07:00

CLYDE AVE/COLE AVE

CARLING AVE

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	0	1	1	0	0	0	1
08:00 09:00	2	0	2	0	0	0	2
09:00 10:00	0	0	0	0	0	0	0
11:30 12:30	0	0	0	0	0	0	0
12:30 13:30	0	0	0	1	1	2	2
15:00 16:00	0	0	0	0	0	0	0
16:00 17:00	1	0	1	0	0	0	1
17:00 18:00	1	1	2	0	0	0	2
Total	4	2	6	1	1	2	8

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.



W.O. 35669

Turning Movement Count - Heavy Vehicle Report

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016

CLYDE AVE/COLE AVE

CARLING AVE

		Northb	ound		5	Southb	ound	_			Eastb	ound		١	Nestbo	ound	_			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	6	1	13	20	0	0	4	4	24	3	12	0	15	6	14	0	20	35	59
08:00	09:00	10	0	20	30	0	0	2	2	32	1	34	0	35	12	21	1	34	69	101
09:00	10:00	6	1	14	21	1	0	1	2	23	1	27	0	28	12	26	0	38	66	89
11:30	12:30	6	0	10	16	1	0	0	1	17	1	25	0	26	15	18	2	35	61	78
12:30	13:30	5	3	9	17	0	1	2	3	20	0	16	0	16	12	15	0	28	44	64
15:00	16:00	5	2	6	13	0	0	3	3	16	1	13	2	16	7	21	1	29	45	61
16:00	17:00	4	0	2	6	1	1	0	2	8	2	8	0	10	8	17	0	25	35	43
17:00	18:00	0	0	5	5	0	0	0	0	5	0	6	0	6	10	13	0	23	29	34
Sub 1	Γotal	42	7	79	128	3	2	12	17	145	9	141	2	152	82	145	4	232	384	529
U-Turn	s (Heav	y Veh	icles)		0				0	0				0				1	1	1
Tot	al	42	7	79	0	3	2	12	17	145	9	141	2	152	82	145	4	233	385	530

Heavy Vehicles include Buses, Single-Unit Trucks and Articulated Trucks. Further, they ARE included in the Turning Movement Count Summary.



Work Order 35669

Turning Movement Count - Pedestrian Volume Report

CARLING AVE @ CLYDE AVE/COLE AVE

Count Dat	e: Wednesday,	January 27, 2016				Start Time:	07:00
Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	1	4	5	2	1	3	8
07:15 07:30	1	2	3	1	4	5	8
07:30 07:45	2	4	6	0	2	2	8
07:45 08:00	7	3	10	2	10	12	22
07:00 08:00	11	13	24	5	17	22	46
08:00 08:15	1	2	3	2	5	7	10
08:15 08:30	2	1	3	4	4	8	11
08:30 08:45	2	2	4	2	4	6	10
08:45 09:00	1	1	2	0	0	0	2
08:00 09:00	6	6	12	8	13	21	33
09:00 09:15	5	6	11	1	3	4	15
09:15 09:30	1	2	3	1	2	3	6
09:30 09:45	0	0	0	1	1	2	2
09:45 10:00	2	2	4	4	3	7	11
09:00 10:00	8	10	18	7	9	16	34
11:30 11:45	2	7	9	1	6	7	16
11:45 12:00	3	2	5	3	1	4	9
12:00 12:15	0	4	4	5	8	13	17
12:15 12:30	1	2	3	2	0	2	5
11:30 12:30	6	15	21	11	15	26	47
12:30 12:45	4	3	7	5	9	14	21
12:45 13:00	3	1	4	1	6	7	11
13:00 13:15	5	6	11	5	8	13	24
13:15 13:30	2	2	4	2	0	2	6
12:30 13:30	14	12	26	13	23	36	62
15:00 15:15	3	4	7	1	6	7	14
15:15 15:30	2	10	12	2	5	7	19
15:30 15:45	1	1	2	4	3	7	9
15:45 16:00	5	3	8	0	4	4	12
15:00 16:00	11	18	29	7	18	25	54
16:00 16:15	7	2	9	4	3	7	16
16:15 16:30	1	3	4	1	6	7	11
16:30 16:45	5	5	10	4	5	9	19
16:45 17:00	3	3	6	0	6	6	12
16:00 17:00	16	13	29	9	20	29	58
17:00 17:15	1	3	4	1	5	6	10
17:15 17:30	5	5	10	2	2	4	14
17:30 17:45	4	8	12	5	3	8	20
17:45 18:00	2	1	3	1	4	5	8
17:00 18:00	12	17	29	9	14	23	52
Total		104	188	69	129	198	386

Comment:







Turning Movement Count - 15 Min U-Turn Total Report

CARLING AVE @ CLYDE AVE/COLE AVE

Survey Date: Wednesday, January 27, 2016

Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	2	2
07:15	07:30	0	0	3	2	5
07:30	07:45	0	0	4	4	8
07:45	08:00	0	0	4	1	5
08:00	08:15	0	0	1	1	2
08:15	08:30	0	0	3	0	3
08:30	08:45	0	0	6	2	8
08:45	09:00	0	0	7	1	8
09:00	09:15	0	0	4	1	5
09:15	09:30	0	0	2	2	4
09:30	09:45	0	0	6	1	7
09:45	10:00	0	0	4	2	6
11:30	11:45	0	0	7	3	10
11:45	12:00	0	0	7	2	9
12:00	12:15	0	0	7	4	11
12:15	12:30	0	0	2	2	4
12:30	12:45	0	0	5	3	8
12:45	13:00	0	0	6	6	12
13:00	13:15	0	0	2	4	6
13:15	13:30	0	0	5	2	7
15:00	15:15	0	0	4	3	7
15:15	15:30	0	0	5	4	9
15:30	15:45	0	0	3	5	8
15:45	16:00	0	0	5	4	9
16:00	16:15	0	0	3	5	8
16:15	16:30	0	0	1	4	5
16:30	16:45	0	0	3	3	6
16:45	17:00	0	0	3	0	3
17:00	17:15	0	0	11	5	16
17:15	17:30	0	0	1	4	5
17:30	17:45	0	0	5	11	16
17:45	18:00	0	0	2	6	8
Тс	otal	0	0	131	99	230

Appendix D City of Ottawa Collision Data

Total Area

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	19	24	20	2	0	0	0	0	65	1
Non-fatal injury	3	7	1	2	0	0	0	0	13	1
Non reportable	0	0	0	0	0	0	0	0	0	1
Total	22	31	21	4	0	0	0	0	78	1
	#2 or 28%	#1 or 40%	#3 or 27%	#4 or 5%	#5 or 0%	#5 or 0%	#5 or 0%	#5 or 0%	•	-

83% 17% 0% 100%

Carling Ave/Churchill Ave

our mig /tre/	onar onni 7100	•		
Years	Total #	24 Hr AADT	Davs	Collisions/MFV
	Collisions	Veh Volume		
2014-2018	22	38 416	1825	0.31

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	11	1	6	1	0	0	0	0	19
Non-fatal injury	2	0	1	0	0	0	0	0	3
Non reportable	0	0	0	0	0	0	0	0	0
Total	13	1	7	1	0	0	0	0	22
	E00/	E9/	220/	E0/	0%	0%	0%	0%	•

86% 14% 0% 100%

Carling Ave/Clyde Ave/Cole Ave

Years	Total #	24 Hr AADT	Days	Collisions/MEV
	Collisions	Veh Volume	,	
2014-2018	50	35 384	1825	0.77

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	6	22	11	1	0	0	0	0	40
Non-fatal injury	1	7	0	2	0	0	0	0	10
Non reportable	0	0	0	0	0	0	0	0	0
Total	7	29	11	3	0	0	0	0	50
	14%	58%	22%	6%	0%	0%	0%	0%	

80% 20% 0% 100%

Carling Ave EB, Clyde Ave to Churchill Ave N

	2, 0., 00		7110 11	
Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
	-			_
2014-2018	1 1	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	0	0	1	0	0	0	0	0	1	100%
Non-fatal injury	0	0	0	0	0	0	0	0	0	0%
Non reportable	0	0	0	0	0	0	0	0	0	0%
Total	0	0	1	0	0	0	0	0	1	100%
	0%	0%	100%	0%	0%	0%	0%	0%		•

Carling Ave WB, Cole Ave to Churchill Ave N

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2014-2018	5	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	2	1	2	0	0	0	0	0	5	10
Non-fatal injury	0	0	0	0	0	0	0	0	0	
Non reportable	0	0	0	0	0	0	0	0	0	
Total	2	1	2	0	0	0	0	0	5	1
	40%	20%	40%	0%	0%	0%	0%	0%	•	•

100% 0% 0% 100%



City Operations - Transportation Services

Collision Details Report - Public Version

From: January 1, 2014 **To:** December 31, 2018

Location: CARLING AVE @ CHURCHILL AVE

Traffic Control: Traffic signal Total Collisions: 23

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2014-Mar-04, Tue,14:04	Clear	Rear end	P.D. only	Dry	East	Going ahead	Truck and trailer	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2014-Jun-09, Mon,08:55	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2014-Sep-15, Mon,16:25	Clear	Sideswipe	Non-fatal injury	Dry	East	Changing lanes	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Pick-up truck	Other motor vehicle	
2015-Mar-28, Sat,13:18	Clear	Rear end	P.D. only	Dry	East	Turning left	Municipal transit	Other motor vehicle	
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2015-May-01, Fri,13:22	Clear	Rear end	P.D. only	Dry	South	Turning right	Pick-up truck	Other motor vehicle	
					South	Turning right	Pick-up truck	Other motor vehicle	
2015-Nov-04, Wed,16:04	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Pick-up truck	Other motor vehicle	

					West	Stopped	Automobile, station wagon	Other motor vehicle
2015-Jul-08, Wed,13:40	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor vehicle
					West	Turning left	Automobile, station wagon	Other motor vehicle
2016-May-19, Thu,13:41	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle
					West	Stopped	Automobile, station wagon	Other motor vehicle
2016-Mar-04, Fri,11:19	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
2016-Jul-08, Fri,14:38	Clear	Sideswipe	P.D. only	Dry	East	Turning right	Truck - open	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
2017-Aug-16, Wed,15:00	Clear	Turning movement	P.D. only	Dry	East	Going ahead	Delivery van	Other motor vehicle
					East	Turning right	Automobile, station wagon	Other motor vehicle
2017-Feb-10, Fri,09:52	Clear	Rear end	P.D. only	Dry	East	Slowing or stopping	Pick-up truck	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
2017-Jul-19, Wed,16:45	Clear	Angle	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle

					North	Turning left	Automobile, station wagon	Other motor vehicle
2017-Sep-20, Wed,19:20	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle
_					West	Going ahead	Automobile, station wagon	Other motor vehicle
2018-Jan-16, Tue,18:18	Clear	Rear end	P.D. only	Slush	South	Going ahead	Automobile, station wagon	Other motor vehicle
					South	Stopped	Automobile, station wagon	Other motor vehicle
2018-Feb-27, Tue,11:26	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Truck - closed	Other motor vehicle
					East	Going ahead	Automobile, station wagon	Other motor vehicle
2018-Mar-27, Tue,10:50	Clear	SMV other	Non-fatal injury	Dry	South	Turning left	Automobile, station wagon	Curb
2018-Oct-12, Fri,10:20	Clear	Rear end	P.D. only	Dry	East	Turning left	Truck - dump	Other motor vehicle
					East	Turning left	Delivery van	Other motor vehicle
2018-Sep-05, Wed,08:46	Clear	Rear end	P.D. only	Dry	East	Changing lanes	Automobile, station wagon	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
2018-Sep-04, Tue,08:30	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Truck - dump	Other motor vehicle
					West	Stopped	Automobile, station wagon	Other motor vehicle

2018-Jul-23, Mon,18:59	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
2018-Dec-25, Tue,12:19	Clear	Rear end	Non-fatal injury	Dry	South	Turning left	Automobile, station wagon	Other motor vehicle
					South	Turning left	Automobile, station wagon	Other motor vehicle
2018-Aug-27, Mon,12:02	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor
					West	Stopped	Automobile,	vehicle Other motor
							station wagon	vehicle

Location: CARLING AVE @ CLYDE AVE/COLE AVE

Traffic Control: Traffic signal Total Collisions: 55

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2014-Jan-29, Wed,15:37	Clear	Turning movement	P.D. only	Slush	North	Turning left	Automobile, station wagon	Other motor vehicle	
-					South	Going ahead	Delivery van	Other motor vehicle	
2014-Mar-12, Wed,17:04	Drifting Snow	Turning movement	P.D. only	Packed snow	West	Turning left	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2014-Apr-16, Wed,10:05	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Delivery van	Other motor vehicle	
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2014-Mar-25, Tue,15:40	Clear	Turning movement	P.D. only	Dry	East	Turning left	Pick-up truck	Other motor vehicle	

					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2014-Jan-30, Thu,13:05	Clear	SMV other	Non-fatal injury	Dry	North	Turning left	Automobile, station wagon	Pedestrian	1
2014-Jul-19, Sat,12:01	Clear	Turning movement	P.D. only	Dry	East	Making "U" turn	Automobile, station wagon	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2014-Dec-05, Fri,14:15	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Passenger van	Other motor vehicle	
2014-Nov-14, Fri,16:14	Snow	Turning movement	P.D. only	Wet	East	Making "U" turn	Automobile, station wagon	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2014-Nov-06, Thu,11:59	Clear	Sideswipe	P.D. only	Dry	East	Turning left	Truck - dump	Other motor vehicle	
					East	Stopped	Truck - dump	Other motor vehicle	
2015-Feb-10, Tue,17:34	Clear	Turning movement	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	
					West	Making "U" turn	Pick-up truck	Other motor vehicle	
2014-Sep-25, Thu,12:15	Clear	Sideswipe	P.D. only	Dry	West	Unknown	Unknown	Other motor vehicle	
					West	Turning right	Automobile, station wagon	Other motor vehicle	

2014-Oct-29, Wed,15:31	Clear	Turning movement	P.D. only	Dry	West	Turning left	Unknown	Other motor vehicle
					East		Automobile, station wagon	Other motor vehicle
2014-Dec-11, Thu,03:15	Snow	SMV other	P.D. only	Loose snow	East	Going ahead	Automobile, station wagon	Curb
2014-Sep-04, Thu,08:20	Clear	Angle	P.D. only	Dry	East	Making "U" turn	Passenger van	Other motor vehicle
					North	Turning left	Automobile, station wagon	Other motor vehicle
2014-Nov-27, Thu,11:34	Clear	Turning movement	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle
					South	•	Automobile, station wagon	Other motor vehicle
2015-Jan-05, Mon,17:51	Clear	Sideswipe	P.D. only	Wet	West	Changing lanes	Passenger van	Other motor vehicle
					West	Going ahead	Pick-up truck	Other motor vehicle
					West	Going ahead	Automobile, station wagon	Other motor vehicle
2015-Feb-19, Thu,16:10	Snow	Rear end	P.D. only	Loose snow	West	Going ahead	Unknown	Other motor vehicle
					West		Automobile, station wagon	Other motor vehicle
					West	Stopped	Automobile, station wagon	Other motor vehicle
					West	Stopped	Automobile, station wagon	Other motor vehicle
2015-Mar-27, Fri,08:15	Snow	Rear end	P.D. only	Wet	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle

					West	Stopped	Pick-up truck	Other motor vehicle
2015-Sep-15, Tue,14:46	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle
					West	Going ahead	Automobile, station wagon	Other motor vehicle
2016-Feb-12, Fri,09:41	Clear	Turning movement	Non-fatal injury	Wet	West	Turning left	Pick-up truck	Other motor vehicle
					East	Going ahead	Automobile, station wagon	Other motor vehicle
2016-Aug-02, Tue,10:00	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Passenger van	Other motor vehicle
					West	Going ahead	Automobile, station wagon	Other motor vehicle
2015-Aug-05, Wed,18:30	Clear	Turning movement	P.D. only	Dry	South	Turning left	Passenger van	Other motor vehicle
					North	Going ahead	Passenger van	Other motor vehicle
2015-Jul-23, Thu,19:14	Clear	Turning movement	P.D. only	Dry	South	Turning right	Pick-up truck	Other motor vehicle
					South	Going ahead	Automobile, station wagon	Other motor vehicle
2015-Dec-08, Tue,09:09	Clear	Turning movement	P.D. only	Dry	East	Making "U" turn	Automobile, station wagon	Other motor vehicle
					West	Going ahead	Municipal transit bus	Other motor vehicle
2016-Oct-08, Sat,15:20	Clear	SMV other	P.D. only	Dry	East	Turning right	Truck and trailer	Pole (utility, power)

2017-Jun-28, Wed,17:55	Clear	Sideswipe	P.D. only	Dry	East	Unknown	Unknown	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Nov-28, Mon,08:41	Clear	SMV other	Non-fatal injury	Dry	North	Turning right	Automobile, station wagon	Pedestrian	1
2017-May-18, Thu,08:36	Clear	Turning movement	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Jun-27, Tue,14:30	Clear	Turning movement	P.D. only	Dry	West	Turning left	Unknown	Other motor vehicle	
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Jul-07, Fri,15:43	Clear	Rear end	P.D. only	Dry	South	Slowing or stopping	g Motorcycle	Other motor vehicle	
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Nov-30, Thu,14:31	Clear	Rear end	P.D. only	Wet	North	Turning right	Automobile, station wagon	Other motor vehicle	
					North	Turning right	Pick-up truck	Other motor vehicle	
2017-Jul-26, Wed,08:34	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jul-20, Thu,15:48	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Passenger van	Other motor vehicle	
					South	Turning left	Municipal transit bus	Other motor vehicle	

2017-Oct-14, Sat,13:15	Clear	Turning movement	P.D. only	Dry	North	Turning right	Delivery van	Other motor vehicle
					South	Turning left	Automobile, station wagon	Other motor vehicle
2017-Sep-22, Fri,15:43	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Truck - dump	Other motor vehicle
					East	Going ahead	Automobile, station wagon	Other motor vehicle
2017-Oct-26, Thu,16:59	Clear	Turning movement	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle
					South	Going ahead	Automobile, station wagon	Other motor vehicle
2017-Sep-21, Thu,16:00	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle
					East	Stopped	Automobile, station wagon	Other motor vehicle
2018-Jan-13, Sat,10:12	Drifting Snow	Sideswipe	P.D. only	Ice	West	Slowing or stopping	g Automobile, station wagon	Skidding/sliding
					West	Turning left	Automobile, station wagon	Other motor vehicle
2018-Jan-09, Tue,21:41	Clear	Sideswipe	P.D. only	Slush	South	Unknown	Unknown	Other motor vehicle
					South	Stopped	Automobile, station wagon	Other motor vehicle
2018-Jan-10, Wed,21:16	Clear	Turning movement	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle
					East	Turning left	Automobile, station wagon	Other motor vehicle
	-	·				-		

2018-Feb-09, Fri,12:58	Clear	Rear end	P.D. only	Wet	West	Slowing or stopping	g Truck - dump	Other motor vehicle
					West	Turning right	Automobile, station wagon	Other motor vehicle
2018-Feb-06, Tue,18:13	Clear	Turning movement	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle
					East	Turning left	Pick-up truck	Other motor vehicle
2018-Feb-01, Thu,07:07	Snow	Sideswipe	P.D. only	Loose snow	West	Turning left	School bus	Other motor vehicle
					West	Changing lanes	Pick-up truck	Other motor vehicle
2018-May-31, Thu,08:11	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle
					West	Going ahead	Automobile, station wagon	Other motor vehicle
2018-May-18, Fri,11:37	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Truck - closed	Other motor vehicle
					East	Going ahead	Automobile, station wagon	Other motor vehicle
2018-May-12, Sat,14:30	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle
					East	Going ahead	Automobile, station wagon	Other motor vehicle
2018-Jun-29, Fri,15:48	Clear	Rear end	P.D. only	Dry	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle
					East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle
2018-Oct-19, Fri,09:19	Clear	Turning movement	P.D. only	Dry	West	Turning left	Pick-up truck	Other motor vehicle

					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Nov-23, Fri,13:08	Clear	SMV other	Non-fatal injury	Dry	North	Turning left	Automobile, station wagon	Pedestrian	1
2018-Oct-24, Wed,12:43	Clear	Turning movement	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Aug-13, Mon,16:20	Clear	Turning movement	P.D. only	Dry	East	Turning left	Pick-up truck	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Aug-11, Sat,12:51	Clear	Turning movement	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Oct-27, Sat,22:36	Snow	Sideswipe	P.D. only	Wet	East	Changing lanes	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Aug-17, Fri,10:38	Clear	Turning movement	P.D. only	Wet	West	Turning left	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Truck - closed	Other motor vehicle	
2018-Nov-16, Fri,07:23	Snow	Angle	Non-fatal injury	Loose snow	West	Going ahead	Automobile, station wagon	Other motor vehicle	
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: CARLING AVE EB btwn CLYDE AVE & CHURCHILL AVE N

Traffic Control: No control

Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Mar-27, Mon,15:30	Clear	Sideswipe	P.D. only	Wet	East	Changing lanes	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Passenger van	Other motor vehicle	

Location: CARLING AVE WB btwn COLE AVE & CHURCHILL AVE N

Traffic Control: No control Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2014-Feb-15, Sat,20:28	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	
_					West	Turning right	Automobile, station wagon	Other motor vehicle	
2015-Mar-31, Tue,10:04	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Truck and trailer	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Oct-07, Fri,14:55	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2017-May-04, Thu,16:47	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	
					West	Going ahead	Automobile, station wagon	Other motor vehicle	

2018-Aug-02, Thu,17:43	Clear	Turning movement	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle
					West	Turning right	Automobile, station wagon	Other motor vehicle

Appendix E Study Area Justification



Technical Memo

To: Wally Dubyk (City of Ottawa) Date: 28 August 2019
Copy: Mark Baker, P.Eng. Project: 477272 – 01000

From: Basel Ansari, EIT

Re: TIA 1655 Carling Avenue - Proposed Study Area

This letter is prepared with the purpose of providing justification for using a smaller study area than the 1km radius noted in the City's current TIA Guidelines for a suburban context.

The proposed development is located at 1655 Carling Avenue and is anticipated to consist of a 22-storey building containing 260 residential units, which is forecasted to generate person trips as summarized in Table 1 below. The number of vehicle trips forecasted in in the order of approximately 90 vehicles/hr during each of the morning and afternoon peak hour periods.

PM Mode PM Peak (persons/h) AM Peak (persons/h) Travel Mode **AM Mode Share** Total In Out Total In Out Share Auto Driver 50% 20 64 50% 53 34 84 87 Auto Passenger 15% 6 19 25 15% 16 10 26 20% 7 26 20% 14 35 33 21 Transit Non-motorized 15% 6 20 26 15% 16 11 27 100% 100% 69 **Total People Trips** 39 129 168 106 175 Total 'New' Residential Apartment Building Auto Trips

Table 1: Forecasted Trips

The subject site currently consists of an unpaved parking lot with an estimated maximum occupancy of 80 vehicles. Although a driveway count has not been conducted, it is estimated that half of the parking lot's capacity is generated during the commuter peak hour. On this basis, the existing parking lot generates in the order of 40 vehicles during each peak hour, thereby resulting in a net potential increase in vehicle trips of approximately 50 veh/h two-way associated with the proposed residential development.

Parsons is recommending that the TIA limit the study area for analysis to the adjacent two signalized intersections on Carling Avenue located within approximately 400m of the site, namely Carling/Clyde (to the west) and Carling/Churchill (to the east). This is consistent with an urban context according to the TIA Guidelines. Figure 1 below provides an illustration of the site location, with a 1 km radius from the site shown. Red circles within the radius indicate major intersections near the subject site that are proposed as part of the reduced study area, whereas the orange circles and rectangles represent intersections and highway ramps that would need to be included in the analysis based on the 1 km radius noted in the TIA Guidelines for a suburban context.

Given the relatively low volume of net forecasted site-generated traffic, an appropriate study area is considered to be the two signalized intersections on either side of the subject site. An evaluation of all eight signalized intersections and two highway ramps within a 1km radius is not considered of benefit to the approvals process.

PARSONS

Figure 1: Study Area



Appendix F TDM Checklist

TDM-Supportive Development Design and Infrastructure Checklist:

Residential Developments (multi-family or condominium)

Legend									
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed								
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users								
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance								

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	☑
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	No rapid transit routes within 600 meters.
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	☑
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILITY	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	♂
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	☑
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	☑
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	⊴
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	·
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	

Appendix G Synchro Analysis Reports



1: Churchill Ave N & Carling Ave

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Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø9	
Lane Configurations	ሻ	ተተ _ጉ	ሻ	ተተኈ	ሻ	f)	ሻ	1>		
Traffic Volume (vph)	167	1352	85	610	11	5	336	31		
Future Volume (vph)	167	1352	85	610	11	5	336	31		
Lane Group Flow (vph)	186	1611	94	849	12	18	373	304		
Turn Type	Prot	NA	Prot	NA	Perm	NA	Perm	NA		
Protected Phases	7	4	3	8		2		6	9	
Permitted Phases					2		6			
Detector Phase	7	4	3	8	2	2	6	6		
Switch Phase										
Minimum Initial (s)	5.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	1.0	
Minimum Split (s)	11.1	34.1	11.1	34.1	44.8	44.8	44.8	44.8	5.0	
Total Split (s)	25.0	50.0	25.0	50.0	45.0	45.0	45.0	45.0	5.0	
Total Split (%)	20.0%	40.0%	20.0%	40.0%	36.0%	36.0%	36.0%	36.0%	4%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	2.0	
All-Red Time (s)	2.4	2.4	2.4	2.4	3.5	3.5	3.5	3.5	0.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.8	6.8	6.8	6.8		
Lead/Lag	Lead	Lag	Lead	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	None	C-Max	None	None	None	None	None	
Act Effct Green (s)	17.2	53.0	12.2	48.1	40.8	40.8	40.8	40.8		
Actuated g/C Ratio	0.14	0.42	0.10	0.38	0.33	0.33	0.33	0.33		
v/c Ratio	0.80	0.79	0.57	0.46	0.05	0.03	0.88	0.45		
Control Delay	76.6	35.6	66.4	28.7	28.2	16.0	61.4	7.5		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	76.6	35.6	66.4	28.7	28.2	16.0	61.4	7.5		
LOS	E	D	Е	С	С	В	Е	Α		
Approach Delay		39.8		32.5		20.9		37.2		
Approach LOS		D		С		С		D		
Queue Length 50th (m)	44.2	127.3	22.5	56.1	2.0	1.0	83.5	5.6		
Queue Length 95th (m)	#75.1	#158.6	38.6	69.1	6.6	6.2	#134.4	26.7		
Internal Link Dist (m)		99.0		113.3		62.0		67.5		
Turn Bay Length (m)	65.0		60.0		20.0		20.0			
Base Capacity (vph)	256	2043	256	1830	252	533	436	680		
Starvation Cap Reductn	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.73	0.79	0.37	0.46	0.05	0.03	0.86	0.45		
Intersection Summary										
Cycle Length: 125										
Actuated Cycle Length: 125										
Offset: 101 (81%), Reference	ed to pha	se 4:EBT	and 8:WI	BT, Start	of Green					

Natural Cycle: 95 Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88 Intersection Signal Delay: 37.1 Intersection Capacity Utilization 80.0%

Intersection LOS: D ICU Level of Service D

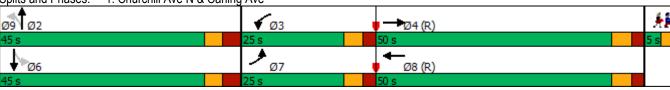
Analysis Period (min) 15

1: Churchill Ave N & Carling Ave

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Churchill Ave N & Carling Ave



2: Clyde Ave/Cole Ave & Carling Ave

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Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	*	ተተ	Ţ	ተተ	Ţ	†	7	7	ĵ»	
Traffic Volume (vph)	60	1481	232	601	121	51	147	35	63	
Future Volume (vph)	60	1481	232	601	121	51	147	35	63	
Lane Group Flow (vph)	67	1646	258	702	134	57	163	39	143	
Turn Type	pm+pt	NA	pm+pt	NA	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4	3	8		2			6	
Permitted Phases	4		8		2		2	6		
Detector Phase	7	4	3	8	2	2	2	6	6	
Switch Phase										
Minimum Initial (s)	5.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	10.4	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	23.0	60.0	23.0	60.0	37.0	37.0	37.0	37.0	37.0	
Total Split (%)	19.2%	50.0%	19.2%	50.0%	30.8%	30.8%	30.8%	30.8%	30.8%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	1.7	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.4	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lead	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	None	C-Max	None	None	None	None	None	
Act Effct Green (s)	71.4	64.1	88.4	77.9	19.6	19.6	19.6	19.6	19.6	
Actuated g/C Ratio	0.60	0.53	0.74	0.65	0.16	0.16	0.16	0.16	0.16	
v/c Ratio	0.15	0.63	0.74	0.22	0.78	0.20	0.43	0.19	0.48	
Control Delay	7.8	22.6	37.3	10.1	75.9	42.3	9.6	42.5	35.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.8	22.6	37.3	10.1	75.9	42.3	9.6	42.5	35.6	
LOS	Α	С	D	В	Е	D	Α	D	D	
Approach Delay		22.0		17.4		39.9			37.1	
Approach LOS		С		В		D			D	
Queue Length 50th (m)	3.8	99.2	35.1	23.8	30.6	11.8	0.0	8.0	21.5	
Queue Length 95th (m)	10.0	133.4	#73.6	38.3	48.5	21.8	16.7	16.7	38.0	
Internal Link Dist (m)		94.2		153.1		79.0			73.1	
Turn Bay Length (m)	20.0		120.0				5.0	20.0		
Base Capacity (vph)	604	2601	361	3138	267	451	495	320	444	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.11	0.63	0.71	0.22	0.50	0.13	0.33	0.12	0.32	
Intersection Summary										

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 84 (70%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.78 Intersection Signal Delay: 23.5 Intersection Capacity Utilization 86.6%

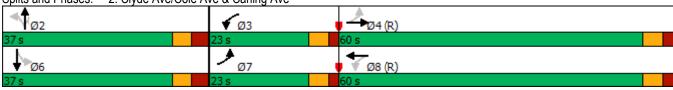
Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Clyde Ave/Cole Ave & Carling Ave



Existing AM 3: Carling Ave & Site Access

	•	→	+	•	\	4			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations		ተተተ	ተተ _ጮ			7			
Traffic Volume (veh/h)	0	1617	854	10	0	1			
Future Volume (Veh/h)	0	1617	854	10	0	1			
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90			
Hourly flow rate (vph)	0	1797	949	11	0	1			
Pedestrians									
Lane Width (m)									
Walking Speed (m/s)									
Percent Blockage									
Right turn flare (veh)									
Median type		None	None						
Median storage veh)									
Upstream signal (m)		177	123						
pX, platoon unblocked	0.89		0		0.82	0.89			
vC, conflicting volume	960				1554	322			
vC1, stage 1 conf vol	300				1004	ULL			
vC2, stage 2 conf vol									
vCu, unblocked vol	518				10	0			
tC, single (s)	4.1				6.8	6.9			
tC, 2 stage (s)	7.1				0.0	0.0			
tF (s)	2.2				3.5	3.3			
p0 queue free %	100				100	100			
cM capacity (veh/h)	928				830	964			
· · · · · · ·		ED 0	ED 2	WD 4			CD 4		
Direction, Lane # Volume Total	EB 1 599	EB 2 599	EB 3 599	WB 1 380	WB 2 380	WB 3 201	SB 1		
Volume Left	0	0	0	0	0	0	0		
Volume Right	0	0	0	0	0	11	1		
cSH	1700	1700	1700	1700	1700	1700	964		
				0.22	0.22	0.12			
Volume to Capacity	0.35	0.35	0.35			0.12	0.00		
Queue Length 95th (m)		0.0		0.0	0.0		0.0		
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	8.7		
Lane LOS	0.0			0.0			Α		
Approach LOS	0.0			0.0			8.7 A		
Approach LOS							A		
Intersection Summary			0.0						
Average Delay	C.		0.0					A	
Intersection Capacity Utiliza	ation		36.3%	IC	U Level	of Service		Α	
Analysis Period (min)			15						

Existing AM 4: Cole Ave & Tillbury Ave

	•	→	•	•	+	•	•	†	<i>></i>	/	↓	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	1	2	49	0	2	1	86	24	3	96	0
Future Volume (Veh/h)	1	1	2	49	0	2	1	86	24	3	96	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	1	1	2	54	0	2	1	96	27	3	107	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								97				
pX, platoon unblocked												
vC, conflicting volume	226	238	107	227	224	110	107			123		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	226	238	107	227	224	110	107			123		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	<u> </u>		0.0	V. <u>–</u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	726	661	947	724	673	944	1484			1464		
					0.0	011	1101			1101		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	4	56	124	110								
Volume Left	1	54	1	3								
Volume Right	2	2	27	0								
cSH	800	730	1484	1464								
Volume to Capacity	0.01	0.08	0.00	0.00								
Queue Length 95th (m)	0.1	1.9	0.0	0.0								
Control Delay (s)	9.5	10.3	0.1	0.2								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.5	10.3	0.1	0.2								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utiliza	Intersection Capacity Utilization 23.2%					of Service			Α			
Analysis Period (min)			15									

	•	`	•	<u>†</u>	1	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥	LDIX	NDL	4	<u> </u>	ODIT	
Traffic Volume (veh/h)	4	32	14	315	578	16	
Future Volume (Veh/h)	4	32	14	315	578	16	
Sign Control	Stop	32	14	Free	Free	10	
Grade	0%			0%	0%		
Peak Hour Factor	0.90	0.00	0.90	0.90		0.90	
		0.90	16		0.90 642		
Hourly flow rate (vph)	4	36	10	350	042	18	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)				91			
pX, platoon unblocked							
vC, conflicting volume	1033	651	660				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	1033	651	660				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	98	92	98				
cM capacity (veh/h)	253	469	928				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	40	366	660				
Volume Left	4	16	0				
Volume Right	36	0	18				
cSH	432	928	1700				
Volume to Capacity	0.09	0.02	0.39				
Queue Length 95th (m)	2.3	0.4	0.0				
Control Delay (s)	14.2	0.6	0.0				
Lane LOS	В	A	J.,				
Approach Delay (s)	14.2	0.6	0.0				
Approach LOS	В	0.0	0.0				
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utiliza	ation		43.1%	IC	CU Level o	of Service	Α
Analysis Period (min)	au011		15	IC.	JO LOVOI (, COI VIOG	
Analysis i Gnou (IIIII)			IJ				

Existing PM 1: Churchill Ave N & Carling Ave

Sene Group		۶	→	•	•	←	•	4	†	<i>></i>	/	ļ	4	
Traffic Volume (vph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Traffic Volume (vph)	Lane Configurations	ሻ	ተተ _ጉ		ሻ	ተ ተ ኈ		ሻ	£		ሻ	1₃		
Future Volume (vph)		231		18	19		218	96		26	182		269	
Storage Length (m)	Future Volume (vph)	231	792	18	19	1782	218	96	33	26	182	7	269	
Storage Lenes	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	
Taper Length (m)	Storage Length (m)	65.0		0.0	60.0		0.0	20.0		0.0	20.0		0.0	
Right Turn on Red	Storage Lanes	1		0	1		0	1		0	1		0	
Link Speed (k/h) 60 60 50 50 50 10.0 10.0 10.0 10.0 10.0 10.0	Taper Length (m)	25.0			15.0			20.0			25.0			
Link Distance (m) 108.4 137.3 86.0 87.9 Travel Time (s) 6.5 8.2 6.2 6.3 Lane Group Flow (vph) 257 900 0 21 2222 0 107 66 0 202 307 0 Protected Phases 7 4 3 8 2 6 6 Permitted Phases 7 4 3 8 2 2 6 6 Switch Phase <td a="" and="" annua<="" annual="" by="" committed="" park="" rows="" td=""><td>Right Turn on Red</td><td></td><td></td><td>Yes</td><td></td><td></td><td>Yes</td><td></td><td></td><td>Yes</td><td></td><td></td><td>Yes</td></td>	<td>Right Turn on Red</td> <td></td> <td></td> <td>Yes</td> <td></td> <td></td> <td>Yes</td> <td></td> <td></td> <td>Yes</td> <td></td> <td></td> <td>Yes</td>	Right Turn on Red			Yes			Yes			Yes			Yes
Travel Time (s)	Link Speed (k/h)					60			50			50		
Lane Group Flow (vph) 257 900 0 21 2222 0 107 66 0 202 307 0 Turn Type	Link Distance (m)		108.4			137.3			86.0			87.9		
Turn Type	Travel Time (s)		6.5											
Protected Phases 7	Lane Group Flow (vph)	257	900	0	21	2222	0	107	66	0	202	307	0	
Permitted Phases 7		Prot						Perm			Perm			
Detector Phase 7		7	4		3	8			2			6		
Switch Phase Minimum Initial (s) 5.0 10.0 5.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 44.0 44.8 44.8 44.8 44.8 44.8 10.0 10.0 10.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 46.5 36.0														
Minimum Initial (s) 5.0 10.0 5.0 10.0 10.0 10.0 10.0 10.0 10.0 Minimum Split (s) 11.1 34.1 14.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 45.0 15.0 0.0 20.0 25.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 46.0 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0% 36.0 </td <td></td> <td>7</td> <td>4</td> <td></td> <td>3</td> <td>8</td> <td></td> <td>2</td> <td>2</td> <td></td> <td>6</td> <td>6</td> <td></td>		7	4		3	8		2	2		6	6		
Minimum Split (s)	Switch Phase													
Total Split (s)	Minimum Initial (s)													
Total Split (%)														
Yellow Time (s) 3.7 3.7 3.7 3.7 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.2 2.2 2.2	Total Split (s)													
All-Red Time (s)	Total Split (%)													
Lost Time Adjust (s)	()													
Total Lost Time (s) 6.1 6.1 6.1 6.1 6.1 6.8 6.8 6.8 6.8 6.8 Lead/Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Recall Mode None C-Max None C-Max None None None None None Act Effet Green (s) 30.7 77.6 7.1 49.3 26.0 26.0 26.0 26.0 26.0 Act Lated g/C Ratio 0.25 0.62 0.06 0.39 0.21 0.21 0.21 0.21 0.21 v/c Ratio 0.62 0.30 0.22 1.17 1.07 0.18 0.77 0.58 Control Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	. ,													
Lead/Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Recall Mode None C-Max None None None Act Effet Green (s) 30.7 77.6 7.1 49.3 26.0 26.0 26.0 26.0 Actuated g/C Ratio 0.25 0.62 0.06 0.39 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 <td></td>														
Lead-Lag Optimize? Yes								6.8	6.8		6.8	6.8		
Recall Mode None C-Max None C-Max None None None None Act Effct Green (s) 30.7 77.6 7.1 49.3 26.0 26.0 26.0 26.0 Actuated g/C Ratio 0.25 0.62 0.06 0.39 0.21 0.21 0.21 0.21 V/c Ratio 0.62 0.30 0.22 1.17 1.07 0.18 0.77 0.58 Control Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	<u> </u>													
Act Effct Green (s) 30.7 77.6 7.1 49.3 26.0 26.0 26.0 26.0 Actuated g/C Ratio 0.25 0.62 0.06 0.39 0.21 0.21 0.21 0.21 V/c Ratio 0.62 0.30 0.22 1.17 1.07 0.18 0.77 0.58 Control Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 LOS D B E F F C E B Approach Delay 21.8 118.6 106.5 32.2 Approach LOS C F F C C E B Approach LOS C F F C C Queue Length 50th (m) 411.1 60.5 39.3														
Actuated g/C Ratio 0.25 0.62 0.06 0.39 0.21 0.21 0.21 v/c Ratio 0.62 0.30 0.22 1.17 1.07 0.18 0.77 0.58 Control Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 LOS D B E F F C E B Approach Delay 21.8 118.6 106.5 32.2 Approach LOS C F F C C E B Approach LOS C C F F C C C E B C C C E B A 24.8 7.4 47.1 5.2 Queue Length 95th (m) #111.1<														
v/c Ratio 0.62 0.30 0.22 1.17 1.07 0.18 0.77 0.58 Control Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 LOS D B E F F C E B Approach Delay 21.8 118.6 106.5 32.2 32.2 Approach LOS C F F C E B Approach LOS C F F F C C E B Approach LOS C F F F C C Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 </td <td></td>														
Control Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.														
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>														
Total Delay 51.2 13.4 60.9 119.2 157.6 23.6 65.0 10.6 LOS D B E F F C E B Approach Delay 21.8 118.6 106.5 32.2 32.2 Approach LOS C F F F C Queue Length 50th (m) 56.5 39.3 5.0 ~239.8 ~28.8 7.4 47.1 5.2 Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 62.0 63.9 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0	,													
LOS D B E F C E B Approach Delay 21.8 118.6 106.5 32.2 Approach LOS C F F C Queue Length 50th (m) 56.5 39.3 5.0 ~239.8 ~28.8 7.4 47.1 5.2 Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 62.0 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <														
Approach Delay 21.8 118.6 106.5 32.2 Approach LOS C F F C Queue Length 50th (m) 56.5 39.3 5.0 ~239.8 ~28.8 7.4 47.1 5.2 Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 62.0 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
Approach LOS C F F F C C Queue Length 50th (m) 56.5 39.3 5.0 ~239.8 ~28.8 7.4 47.1 5.2 Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 62.0 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 0 Reduced v/c Ratio 0.62 0.30 0.11 1.17 0.73 0.13 0.52 0.48 Intersection Summary		D			E			F			Е			
Queue Length 50th (m) 56.5 39.3 5.0 ~239.8 ~28.8 7.4 47.1 5.2 Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 62.0 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 Reduced v/c Ratio 0.62 0.30 0.11 1.17 0.73 0.13 0.52 0.48														
Queue Length 95th (m) #111.1 60.4 13.2 #269.0 #55.3 17.6 67.0 28.0 Internal Link Dist (m) 84.4 113.3 62.0 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
Internal Link Dist (m) 84.4 113.3 62.0 63.9 Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	•													
Turn Bay Length (m) 65.0 60.0 20.0 20.0 Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <t< td=""><td></td><td>#111.1</td><td></td><td></td><td>13.2</td><td></td><td></td><td>#55.3</td><td></td><td></td><td>67.0</td><td></td><td></td></t<>		#111.1			13.2			#55.3			67.0			
Base Capacity (vph) 416 3013 188 1892 147 524 385 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			84.4			113.3			62.0			63.9		
Starvation Cap Reductn 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 Reduced v/c Ratio 0.62 0.30 0.11 1.17 0.73 0.13 0.52 0.48 Intersection Summary														
Spillback Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 Reduced v/c Ratio 0.62 0.30 0.11 1.17 0.73 0.13 0.52 0.48 Intersection Summary														
Storage Cap Reductn 0 0 0 0 0 0 0 Reduced v/c Ratio 0.62 0.30 0.11 1.17 0.73 0.13 0.52 0.48 Intersection Summary														
Reduced v/c Ratio 0.62 0.30 0.11 1.17 0.73 0.13 0.52 0.48 Intersection Summary	•													
Intersection Summary														
	Reduced v/c Ratio	0.62	0.30		0.11	1.17		0.73	0.13		0.52	0.48		
. =	Intersection Summary													
Area Type: Other	Area Type:	Other												

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Right Turn on Red		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	9	
Detector Phase		
Switch Phase		
	1.0	
Minimum Initial (s)	5.0	
Minimum Split (s)		
Total Split (s)	5.0	
Total Split (%)	4%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Cycle Length: 125
Actuated Cycle Length: 125
Offset: 92 (74%), Referenced to phase 4:EBT and 8:WBT, Start of Green
Natural Cycle: 145
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.17
Intersection Signal Delay: 79.9 Intersection LOS: E
Intersection Capacity Utilization 111.5% ICU Level of Service H
Analysis Period (min) 15

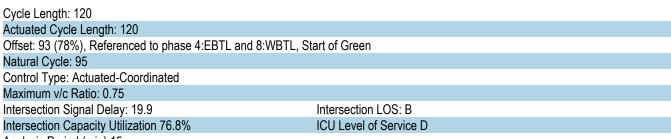
Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

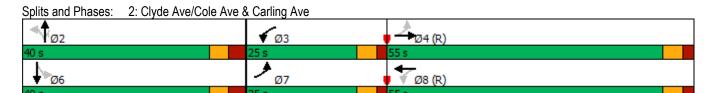
95th percentile volume exceeds capacity, gueue may be longer.

Queue shown is maximum after two cycles.

	۶	→	•	•	←	•	4	†	~	/	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ተተኈ		ሻ	ተተኈ		ሻ	1	7	ሻ	1>	
Traffic Volume (vph)	74	801	1	241	1855	51	126	87	210	30	54	62
Future Volume (vph)	74	801	1	241	1855	51	126	87	210	30	54	62
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		0.0	120.0		0.0	0.0		5.0	20.0		0.0
Storage Lanes	1		0	1		0	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			191.8			103.0			96.3	
Travel Time (s)		7.1			11.5			7.4			8.7	
Lane Group Flow (vph)	82	891	0	268	2118	0	140	97	233	33	129	0
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4			8			2		2	6		
Detector Phase	7	4		3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5		10.4	35.5		37.0	37.0	37.0	37.0	37.0	
Total Split (s)	25.0	55.0		25.0	55.0		40.0	40.0	40.0	40.0	40.0	
Total Split (%)	20.8%	45.8%		20.8%	45.8%		33.3%	33.3%	33.3%	33.3%	33.3%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1		1.7	2.1		3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8		5.4	5.8		6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	C-Max		None	C-Max		None	None	None	None	None	
Act Effct Green (s)	77.8	69.8		87.4	76.9		19.9	19.9	19.9	19.9	19.9	
Actuated g/C Ratio	0.65	0.58		0.73	0.64		0.17	0.17	0.17	0.17	0.17	
v/c Ratio	0.49	0.31		0.57	0.68		0.75	0.33	0.61	0.16	0.42	
Control Delay	27.6	14.8		11.1	17.4		70.5	45.1	20.5	41.5	30.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	27.6	14.8		11.1	17.4		70.5	45.1	20.5	41.5	30.1	
LOS	С	В		В	В		Е	D	С	D	С	
Approach Delay		15.9			16.7			40.5			32.4	
Approach LOS		В			В			D			С	
Queue Length 50th (m)	4.7	36.0		17.2	111.6		31.9	20.5	14.0	6.8	17.0	
Queue Length 95th (m)	22.5	61.8		35.6	172.7		49.5	33.0	36.3	14.7	32.2	
Internal Link Dist (m)		94.2			167.8			79.0			72.3	
Turn Bay Length (m)	20.0	/		120.0					5.0	20.0		
Base Capacity (vph)	334	2831		543	3105		313	496	528	339	486	
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	
Reduced v/c Ratio	0.25	0.31		0.49	0.68		0.45	0.20	0.44	0.10	0.27	
Intersection Summary												
Area Type:	Other											



Analysis Period (min) 15



Existing PM 3: Carling Ave & Site Access

	•	→	+	•	/	4			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations		ተተተ	↑ ↑₽			7			
Traffic Volume (veh/h)	0	1041	2146	1	0	13			
Future Volume (Veh/h)	0	1041	2146	1	0	13			
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90			
Hourly flow rate (vph)	0	1157	2384	1	0	14			
Pedestrians									
Lane Width (m)									
Walking Speed (m/s)									
Percent Blockage									
Right turn flare (veh)									
Median type		None	None						
Median storage veh)									
Upstream signal (m)		192	109						
pX, platoon unblocked	0.61				0.66	0.61			
vC, conflicting volume	2385				2770	795			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
vCu, unblocked vol	1059				1011	0			
tC, single (s)	4.1				6.8	6.9			
tC, 2 stage (s)									
tF (s)	2.2				3.5	3.3			
p0 queue free %	100				100	98			
cM capacity (veh/h)	402				155	667			
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1		
Volume Total	386	386	386	954	954	478	14		
Volume Left	0	0	0	0	0	0	0		
Volume Right	0	0	0	0	0	1	14		
cSH	1700	1700	1700	1700	1700	1700	667		
Volume to Capacity	0.23	0.23	0.23	0.56	0.56	0.28	0.02		
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.5		
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	10.5		
Lane LOS							В		
Approach Delay (s)	0.0			0.0			10.5		
Approach LOS							В		
Intersection Summary									
Average Delay			0.0						
Intersection Capacity Utiliza	tion		53.8%	IC	U Level	of Service		Α	
Analysis Period (min)			15						

Existing PM 4: Cole Ave & Tillbury Ave

	۶	→	•	€	—	•	1	†	<i>></i>	/	+	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	1	8	36	1	5	7	122	52	4	60	2
Future Volume (Veh/h)	2	1	8	36	1	5	7	122	52	4	60	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	2	1	9	40	1	6	8	136	58	4	67	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								96				
pX, platoon unblocked	0.97	0.97		0.97	0.97	0.97				0.97		
vC, conflicting volume	264	286	68	266	258	165	69			194		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	227	250	68	230	221	125	69			155		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	94	100	99	99			100		
cM capacity (veh/h)	698	629	995	693	653	899	1532			1384		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	12	47	202	73								
Volume Left	2	40	8	4								
Volume Right	9	6	58	2								
cSH	889	713	1532	1384								
Volume to Capacity	0.01	0.07	0.01	0.00								
Queue Length 95th (m)	0.3	1.6	0.1	0.1								
Control Delay (s)	9.1	10.4	0.3	0.4								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.1	10.4	0.3	0.4								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.1									
Intersection Capacity Utilizati	ion		27.8%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

Existing PM 5: Churchill Ave N & Tillbury Ave

	•	*	•	†	+	✓
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	ĵ»	
Traffic Volume (veh/h)	9	25	31	464	454	17
Future Volume (Veh/h)	9	25	31	464	454	17
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	10	28	34	516	504	19
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (m)				88		
pX, platoon unblocked						
vC, conflicting volume	1098	514	523			
vC1, stage 1 conf vol	1000	U	020			
vC2, stage 2 conf vol						
vCu, unblocked vol	1098	514	523			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	V . 1	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	95	97			
cM capacity (veh/h)	228	561	1043			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	38	550	523			
Volume Left	10	34	0			
Volume Right	28	0	19			
cSH	405	1043	1700			
Volume to Capacity	0.09	0.03	0.31			
Queue Length 95th (m)	2.3	0.8	0.0			
Control Delay (s)	14.8	0.9	0.0			
Lane LOS	В	Α				
Approach Delay (s)	14.8	0.9	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utiliz	ation		62.6%	IC	CU Level o	of Service
Analysis Period (min)			15			



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	*	^	7	ሻ	f)		ች	ĵ.	
Traffic Volume (vph)	167	1082	98	85	488	154	11	5	11	336	31	243
Future Volume (vph)	167	1082	98	85	488	154	11	5	11	336	31	243
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0			25.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		111.5			137.3			86.0			94.3	
Travel Time (s)		6.7			8.2			6.2			6.8	
Lane Group Flow (vph)	167	1082	98	85	488	154	11	16	0	336	274	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	25.0	50.0	50.0	25.0	50.0	50.0	45.0	45.0		45.0	45.0	
Total Split (%)	20.0%	40.0%	40.0%	20.0%	40.0%	40.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	16.3	57.0	57.0	11.6	52.3	52.3	37.4	37.4		37.4	37.4	
Actuated g/C Ratio	0.13	0.46	0.46	0.09	0.42	0.42	0.30	0.30		0.30	0.30	
v/c Ratio	0.76	0.70	0.14	0.54	0.34	0.23	0.05	0.03		0.86	0.44	
Control Delay	73.7	32.1	4.7	66.2	27.3	11.0	28.6	16.2		61.9	7.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	73.7	32.1	4.7	66.2	27.3	11.0	28.6	16.2		61.9	7.8	
LOS	Е	С	Α	Е	С	В	С	В		Е	Α	
Approach Delay		35.3			28.4			21.3			37.6	
Approach LOS		D			С			С			D	
Queue Length 50th (m)	39.7	112.3	0.0	20.3	44.0	7.6	1.9	0.9		76.3	5.4	
Queue Length 95th (m)	62.9	156.1	10.1	35.5	62.6	23.9	6.0	5.7		109.0	24.8	
Internal Link Dist (m)		87.5			113.3			62.0			70.3	
Turn Bay Length (m)	65.0		20.0	60.0		20.0	20.0			20.0		
Base Capacity (vph)	256	1546	713	256	1419	660	255	513		422	649	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.65	0.70	0.14	0.33	0.34	0.23	0.04	0.03		0.80	0.42	
Intersection Summary												
	Other											

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Right Turn on Red		
•		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Lane Group Flow (vph)		
Turn Type	_	
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	
Minimum Split (s)	5.0	
Total Split (s)	5.0	
Total Split (%)	4%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)	0.0	
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)	None	
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

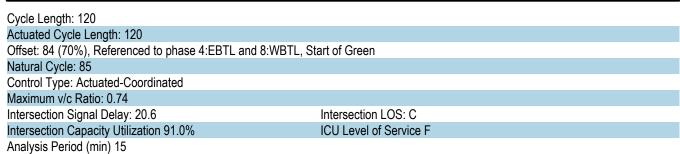
Cycle Length: 125
Actuated Cycle Length: 125
Offset: 101 (81%), Referenced to phase 4:EBT and 8:WBT, Start of Green
Natural Cycle: 95
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.86
Intersection Signal Delay: 33.8
Intersection LOS: C
Intersection Capacity Utilization 81.6%
ICU Level of Service D

Analysis Period (min) 15

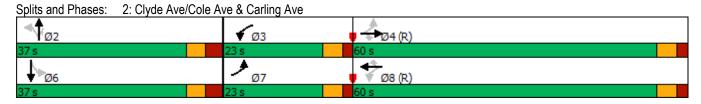
Splits and Phases: 1: Churchill Ave N & Carling Ave



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	*	^	7	ሻ		7	ች	₽	
Traffic Volume (vph)	60	1185	0	232	481	31	121	51	147	35	63	66
Future Volume (vph)	60	1185	0	232	481	31	121	51	147	35	63	66
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			188.6			103.0			97.3	
Travel Time (s)		7.1			11.3			7.4			8.8	
Lane Group Flow (vph)	60	1185	0	232	481	31	121	51	147	35	129	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	23.0	60.0	60.0	23.0	60.0	60.0	37.0	37.0	37.0	37.0	37.0	
Total Split (%)	19.2%	50.0%	50.0%	19.2%	50.0%	50.0%	30.8%	30.8%	30.8%	30.8%	30.8%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	76.4	69.3		90.2	79.9	79.9	17.8	17.8	17.8	17.8	17.8	
Actuated g/C Ratio	0.64	0.58		0.75	0.67	0.67	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.10	0.61		0.60	0.21	0.03	0.74	0.19	0.43	0.19	0.47	
Control Delay	6.5	20.3		14.1	9.5	0.1	73.5	43.8	10.4	44.2	35.2	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	6.5	20.3		14.1	9.5	0.1	73.5	43.8	10.4	44.2	35.2	
LOS	Α	С		В	Α	Α	E	D	В	D	D	
Approach Delay		19.6			10.5			39.7			37.2	
Approach LOS		В			В			D			D	
Queue Length 50th (m)	3.1	90.5		13.4	22.3	0.0	27.7	10.7	0.0	7.3	18.7	
Queue Length 95th (m)	8.5	148.4		37.0	38.2	0.0	44.8	20.5	16.3	15.6	34.7	
Internal Link Dist (m)	20.0	94.2		400.0	164.6	20.0		79.0		20.0	73.3	
Turn Bay Length (m)	20.0	1050		120.0	2057	20.0	000	4=4	5.0	20.0		
Base Capacity (vph)	733	1958		426	2257	1003	280	451	483	322	444	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.61		0.54	0.21	0.03	0.43	0.11	0.30	0.11	0.29	
Intersection Summary												
Area Type:	Other											



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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	∱ }			7
Traffic Volume (veh/h)	0	1347	732	10	0	1
Future Volume (Veh/h)	0	1347	732	10	0	1
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	1347	732	10	0	1
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)		189	112			
pX, platoon unblocked	0.91				0.82	0.91
vC, conflicting volume	742				1410	371
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	509				544	100
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	954				384	849
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	674	674	488	254	1	
Volume Left	0/4	074	0	0	0	
Volume Right	0	0	0	10	1	
cSH	1700	1700	1700	1700	849	
	0.40	0.40	0.29	0.15	0.00	
Volume to Capacity	0.40	0.40	0.29	0.15	0.00	
Queue Length 95th (m)	0.0		0.0		9.2	
Control Delay (s) Lane LOS	0.0	0.0	0.0	0.0		
	0.0		0.0		A 9.2	
Approach Delay (s) Approach LOS	0.0		0.0		9.2 A	
•					А	
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		42.6%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	1	2	49	0	2	1	86	24	3	96	0
Future Volume (Veh/h)	1	1	2	49	0	2	1	86	24	3	96	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	1	1	2	49	0	2	1	86	24	3	96	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								97				
pX, platoon unblocked												
vC, conflicting volume	204	214	96	204	202	98	96			110		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	204	214	96	204	202	98	96			110		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)			<u> </u>			<u> </u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	751	682	960	749	692	958	1498			1480		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	4	51	111	99								
Volume Left	1	49	1	3								
Volume Right	2	2	24	0								
cSH	819	756	1498	1480								
Volume to Capacity	0.00	0.07	0.00	0.00								
Queue Length 95th (m)	0.1	1.6	0.0	0.0								
Control Delay (s)	9.4	10.1	0.1	0.2								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.4	10.1	0.1	0.2								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utiliz	ation		23.2%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			स	1>	
Traffic Volume (veh/h)	4	32	14	315	578	16
Future Volume (Veh/h)	4	32	14	315	578	16
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	4	32	14	315	578	16
Pedestrians	_	<u> </u>				
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (m)				94		
pX, platoon unblocked				<u> </u>		
vC, conflicting volume	929	586	594			
vC1, stage 1 conf vol	020	000	001			
vC2, stage 2 conf vol						
vCu, unblocked vol	929	586	594			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	94	99			
cM capacity (veh/h)	293	510	982			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	36	329	594			
Volume Left	4	14	0			
Volume Right	32	0	16			
cSH	471	982	1700			
Volume to Capacity	0.08	0.01	0.35			
Queue Length 95th (m)	1.9	0.3	0.0			
Control Delay (s)	13.3	0.5	0.0			
Lane LOS	В	Α				
Approach Delay (s)	13.3	0.5	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utiliza	ation		43.1%	IC	CU Level o	of Service
Analysis Period (min)			15		. 5 25,010	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	f)		ሻ	f)	
Traffic Volume (vph)	231	634	18	19	1426	218	96	33	26	182	7	269
Future Volume (vph)	231	634	18	19	1426	218	96	33	26	182	7	269
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0			25.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		112.3			137.3			86.0			93.3	
Travel Time (s)		6.7			8.2			6.2			6.7	
Lane Group Flow (vph)	231	634	18	19	1426	218	96	59	0	182	276	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	20.0	55.0	55.0	20.0	55.0	55.0	45.0	45.0		45.0	45.0	
Total Split (%)	16.0%	44.0%	44.0%	16.0%	44.0%	44.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	28.4	82.6	82.6	7.0	53.9	53.9	23.7	23.7		23.7	23.7	
Actuated g/C Ratio	0.23	0.66	0.66	0.06	0.43	0.43	0.19	0.19		0.19	0.19	
v/c Ratio	0.60	0.28	0.02	0.20	0.98	0.32	0.96	0.18		0.76	0.56	
Control Delay	51.9	11.4	0.1	60.6	54.0	14.0	128.3	25.1		66.2	9.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	51.9	11.4	0.1	60.6	54.0	14.0	128.3	25.1		66.2	9.4	
LOS	D	В	Α	Е	D	В	F	С		Е	Α	
Approach Delay		21.8			48.9			89.0			32.0	
Approach LOS	54.7	C	0.0	4.0	D	40.7	00.7	F		40.0	C	
Queue Length 50th (m)	51.7	26.1	0.0	4.6	174.5	16.7	23.7	6.8		42.9	1.4	
Queue Length 95th (m)	#87.4	61.7	0.0	12.2		37.9	#47.1	16.7		62.0	22.6	
Internal Link Dist (m)	CE O	88.3	20.0	60.0	113.3	20.0	20.0	62.0		20.0	69.3	
Turn Bay Length (m)	65.0	2220	20.0	60.0	1460	20.0		E 22		20.0	626	
Base Capacity (vph) Starvation Cap Reductn	384	2239	978	188	1460	679	162	522		388	636	
	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn Reduced v/c Ratio	0.60	0.28	0.02	0.10	0.98	0.32	0.59	0.11		0.47	0.43	
	0.00	U.Zŏ	0.02	0.10	0.98	0.32	0.59	0.11		0.47	0.43	
Intersection Summary												
Area Type:	Other											

Baseline Synchro 10 Report

Lane Configurations Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (m) Storage Length (m) Storage Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (s) Total Split (s) Total Split (s) Sol All-Red Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay Los Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (m) Storage Lanes Taper Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (s) Total Split (w) Yellow Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay Lost Approach LoS Approach LoS Approach LoS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Future Volume (vphpl) Ideal Flow (vphpl) Storage Length (m) Storage Lanes Taper Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) 5.0 Total Split (s) 5.0 Total Split (%) 4% Yellow Time (s) Lost Time (s) Lost Time (s) Lead/Lag Lead/Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Vic Ratio Corage Cap Reductn Storage Cap Reductn Storage Cap Reductn Storage Cap Reductn				
Ideal Flow (vphpl) Storage Length (m) Storage Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (w) Yellow Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 95th (m) Internal Link Dist (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Length (m) Storage Lanes Taper Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Lanes Taper Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Solution Total Split (%) Yellow Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay LOS Approach LOS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) 4% Yellow Time (s) All-Red Time (s) Lost Time Agiust (s) Total Split (s) Total Split (%) All-Red Time (s) Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay Los Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
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Switch Phase Minimum Initial (s) 3.0 Minimum Split (s) 5.0 Total Split (s) 5.0 Total Split (%) 4% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Permitted Phases			
Switch Phase Minimum Initial (s) 3.0 Minimum Split (s) 5.0 Total Split (s) 5.0 Total Split (%) 4% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Detector Phase			
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Minimum Split (s) 5.0 Total Split (s) 5.0 Total Split (%) 4% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		3.0		
Total Split (s) 5.0 Total Split (%) 4% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Reduced v/c Ratio		0.0		
Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		.,		
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		None		
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	v/c Ratio			
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Control Delay			
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary	Reduced v/c Ratio			
intersection outfilliary	Intersection Cummers			
	intersection Summary			

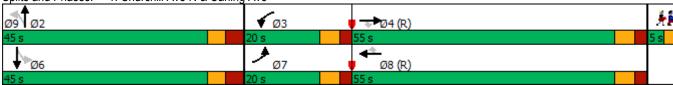
Baseline Synchro 10 Report

Cycle Length: 125
Actuated Cycle Length: 125
Offset: 92 (74%), Referenced to phase 4:EBT and 8:WBT, Start of Green
Natural Cycle: 135
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.98
Intersection Signal Delay: 40.8 Intersection LOS: D
Intersection Capacity Utilization 111.4% ICU Level of Service H
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

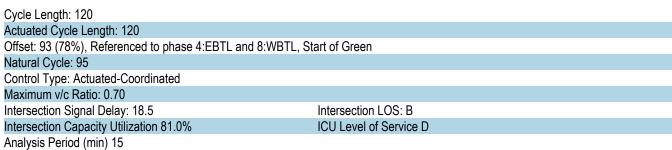
Queue shown is maximum after two cycles.

Splits and Phases: 1: Churchill Ave N & Carling Ave



Baseline Synchro 10 Report

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	*	^	7	ሻ		7	ሻ	₽	
Traffic Volume (vph)	74	641	1	241	1484	51	126	87	210	30	54	62
Future Volume (vph)	74	641	1	241	1484	51	126	87	210	30	54	62
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			187.7			103.0			97.5	
Travel Time (s)		7.1			11.3			7.4			8.8	
Lane Group Flow (vph)	74	641	1	241	1484	51	126	87	210	30	116	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	25.0	55.0	55.0	25.0	55.0	55.0	40.0	40.0	40.0	40.0	40.0	
Total Split (%)	20.8%	45.8%	45.8%	20.8%	45.8%	45.8%	33.3%	33.3%	33.3%	33.3%	33.3%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	80.5	72.9	72.9	88.6	78.9	78.9	18.4	18.4	18.4	18.4	18.4	
Actuated g/C Ratio	0.67	0.61	0.61	0.74	0.66	0.66	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.31	0.31	0.00	0.43	0.67	0.05	0.70	0.32	0.58	0.16	0.40	
Control Delay	9.5	13.3	0.0	7.8	16.6	1.0	67.0	46.3	17.6	42.8	29.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	9.5	13.3	0.0	7.8	16.6	1.0	67.0	46.3	17.6	42.8	29.6	
LOS	A	В	Α	Α	B	Α	E	D	В	D	С	
Approach Delay		12.9			15.0			38.2			32.3	
Approach LOS	2.0	В	0.0	440	B	0.0	00.0	D	0.0	0.0	С	
Queue Length 50th (m)	3.9	35.2	0.0	14.2	106.9	0.0	28.6	18.6	9.2	6.2	14.4	
Queue Length 95th (m)	10.5	61.8	0.0	30.2	173.6	2.3	45.2	30.9	30.0	13.8	29.2	
Internal Link Dist (m)	20.0	94.2	20.0	400.0	163.7	20.0		79.0	F 0	20.0	73.5	
Turn Bay Length (m)	20.0	0000	20.0	120.0	0000	20.0	200	400	5.0	20.0	400	
Base Capacity (vph)	405	2060	907	644	2228	960	328	496	528	342	486	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0 10	0	0.00	0 27	0.67	0.05	0 20	0 10	0.40	0 00	0 24	
Reduced v/c Ratio	0.18	0.31	0.00	0.37	0.67	0.05	0.38	0.18	0.40	0.09	0.24	
Intersection Summary												
Area Type:	Other											



Splits and Phases: 2: Clyde Ave/Cole Ave & Carling Ave ÿ3 Ø4 (R) Ø8 (R)

Synchro 10 Report Baseline

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	↑ ↑			7
Traffic Volume (veh/h)	0	883	1790	1	0	13
Future Volume (Veh/h)	0	883	1790	1	0	13
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	883	1790	1	0	13
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)			2			
Upstream signal (m)		188	112			
pX, platoon unblocked	0.58	.00			0.63	0.58
vC, conflicting volume	1791				2232	896
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	928				1163	0
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)					0.0	
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	98
cM capacity (veh/h)	427				118	633
		ED 0	WD 4	WD 0		
Direction, Lane # Volume Total	EB 1 442	EB 2 442	WB 1	WB 2	SB 1 13	
Volume Left	0	442	1193 0	598 0	0	
	0		0	1	13	
Volume Right		1700			633	
cSH	1700	1700	1700	1700		
Volume to Capacity	0.26	0.26	0.70	0.35	0.02	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.5	
Control Delay (s)	0.0	0.0	0.0	0.0	10.8	
Lane LOS	2.0		0.0		B	
Approach Delay (s)	0.0		0.0		10.8	
Approach LOS					В	
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliz	zation		62.3%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	1	8	36	1	5	7	122	52	4	60	2
Future Volume (Veh/h)	2	1	8	36	1	5	7	122	52	4	60	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	2	1	8	36	1	5	7	122	52	4	60	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)								110110			110110	
Upstream signal (m)								98				
pX, platoon unblocked	0.98	0.98		0.98	0.98	0.98				0.98		
vC, conflicting volume	236	257	61	240	232	148	62			174		
vC1, stage 1 conf vol	200	201	01	210	202	170	02			1, -		
vC2, stage 2 conf vol												
vCu, unblocked vol	207	228	61	210	202	116	62			143		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	7.1	0.0	0.2	7.1	0.0	0.2	7.1			7.1		
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	95	100	99	100			100		
cM capacity (veh/h)	725	651	1004	719	673	914	1541			1407		
					010	314	1041			1407		
Direction, Lane #	<u>EB 1</u>	WB 1	NB 1	SB 1								
Volume Total	11	42	181	66								
Volume Left	2	36	7	4								
Volume Right	8	5	52	2								
cSH	897	737	1541	1407								
Volume to Capacity	0.01	0.06	0.00	0.00								
Queue Length 95th (m)	0.3	1.4	0.1	0.1								
Control Delay (s)	9.1	10.2	0.3	0.5								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.1	10.2	0.3	0.5								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.1									
Intersection Capacity Utiliz	ation		27.8%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	1>	
Traffic Volume (veh/h)	9	25	31	464	454	17
Future Volume (Veh/h)	9	25	31	464	454	17
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	9	25	31	464	454	17
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				140110	110110	
Upstream signal (m)				93		
pX, platoon unblocked				30		
vC, conflicting volume	988	462	471			
vC1, stage 1 conf vol	300	702	7/1			
vC2, stage 2 conf vol						
vCu, unblocked vol	988	462	471			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	96	97			
cM capacity (veh/h)	266	599	1091			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	34	495	471			
Volume Left	9	31	0			
Volume Right	25	0	17			
cSH	450	1091	1700			
Volume to Capacity	0.08	0.03	0.28			
Queue Length 95th (m)	1.9	0.7	0.0			
Control Delay (s)	13.7	0.8	0.0			
Lane LOS	В	Α				
Approach Delay (s)	13.7	0.8	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utiliz	zation		62.6%	IC	CU Level o	of Service
Analysis Period (min)			15			



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	f)		ሻ	f.	
Traffic Volume (vph)	167	1136	98	85	512	154	11	5	11	336	31	243
Future Volume (vph)	167	1136	98	85	512	154	11	5	11	336	31	243
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0			25.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		113.4			137.3			90.4			92.0	
Travel Time (s)		6.8			8.2			6.5			6.6	
Lane Group Flow (vph)	167	1136	98	85	512	154	11	16	0	336	274	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	25.0	50.0	50.0	25.0	50.0	50.0	45.0	45.0		45.0	45.0	
Total Split (%)	20.0%	40.0%	40.0%	20.0%	40.0%	40.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	16.3	57.0	57.0	11.6	52.3	52.3	37.4	37.4		37.4	37.4	
Actuated g/C Ratio	0.13	0.46	0.46	0.09	0.42	0.42	0.30	0.30		0.30	0.30	
v/c Ratio	0.76	0.73	0.14	0.54	0.36	0.23	0.05	0.03		0.86	0.44	
Control Delay	73.7	33.2	4.7	66.2	27.6	11.0	28.6	16.2		61.9	7.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	73.7	33.2	4.7	66.2	27.6	11.0	28.6	16.2		61.9	7.8	
LOS	E	C	Α	Е	C	В	С	В		Е	A	
Approach Delay		36.1			28.5			21.3			37.6	
Approach LOS	20.7	D	0.0	20.2	C	7.0	4.0	С		70.0	D 5.4	
Queue Length 50th (m)	39.7	120.7	0.0	20.3	46.5	7.6	1.9	0.9		76.3	5.4	
Queue Length 95th (m)	62.9		10.1	35.5	66.0	23.9	6.0	5.7		109.0	24.8	
Internal Link Dist (m)	CE O	89.4	20.0	60.0	113.3	20.0	20.0	66.4		20.0	68.0	
Turn Bay Length (m)	65.0	1516	20.0	60.0	1/10	20.0	20.0	E12		20.0	640	
Base Capacity (vph)	256	1546	713	256	1419	660	255	513		422	649	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn Reduced v/c Ratio	0.65	0.73	0.14	0.33	0.36	0.23	0.04	0.03		0.80	0.42	
	0.05	0.73	0.14	0.33	0.30	0.23	0.04	0.03		0.80	0.42	
Intersection Summary												
Area Type:	Other											

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Right Turn on Red		
•		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Lane Group Flow (vph)		
Turn Type	_	
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	
Minimum Split (s)	5.0	
Total Split (s)	5.0	
Total Split (%)	4%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)	0.0	
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)	None	
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Cycle Length: 125
Actuated Cycle Length: 125
Offset: 101 (81%), Referenced to phase 4:EBT and 8:WBT, Start of Green
Natural Cycle: 95
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.86
Intersection Signal Delay: 34.2 Intersection LOS: C
Intersection Capacity Utilization 83.1% ICU Level of Service E
Analysis Period (min) 15

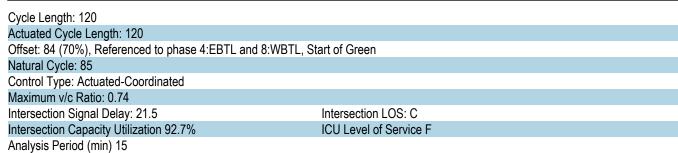
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

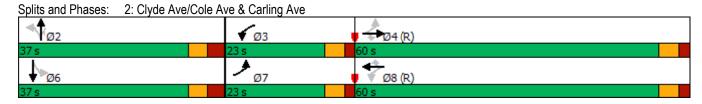
Splits and Phases: 1: Churchill Ave N & Carling Ave



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	†	7	ሻ	1>	
Traffic Volume (vph)	60	1244	0	232	505	31	121	51	147	35	63	66
Future Volume (vph)	60	1244	0	232	505	31	121	51	147	35	63	66
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			186.6			103.0			94.6	
Travel Time (s)		7.1			11.2			7.4			8.5	
Lane Group Flow (vph)	60	1244	0	232	505	31	121	51	147	35	129	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	23.0	60.0	60.0	23.0	60.0	60.0	37.0	37.0	37.0	37.0	37.0	
Total Split (%)	19.2%	50.0%	50.0%	19.2%	50.0%	50.0%	30.8%	30.8%	30.8%	30.8%	30.8%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes				N		
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	75.5	68.4		90.2	79.9	79.9	17.8	17.8	17.8	17.8	17.8	
Actuated g/C Ratio	0.63	0.57		0.75	0.67	0.67	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.11	0.64		0.62	0.22	0.03	0.74	0.19	0.43	0.19	0.47	
Control Delay	6.7	21.6		17.0	9.5	0.1	73.5	43.8	10.4	44.2	35.2	
Queue Delay Total Delay	0.0 6.7	0.0 21.6		0.0 17.0	0.0 9.5	0.0	0.0 73.5	0.0 43.8	0.0 10.4	0.0 44.2	0.0 35.2	
LOS		21.0 C		17.0 B	9.5 A		73.5 E	43.0 D	10.4 B	44.2 D	ან.2 D	
	A	20.9		В	11.4	A	<u> </u>	39.7	В	U	37.2	
Approach Delay		20.9 C			11. 4 B			39.7 D			31.2 D	
Approach LOS Queue Length 50th (m)	3.1	100.1		13.4	23.7	0.0	27.7	10.7	0.0	7.3	18.7	
Queue Length 95th (m)	8.5	159.7		42.0	40.1	0.0	44.8	20.5	16.3	15.6	34.7	
Internal Link Dist (m)	0.5	94.2		42.0	162.6	0.0	44.0	79.0	10.5	13.0	70.6	
Turn Bay Length (m)	20.0	34.2		120.0	102.0	20.0		13.0	5.0	20.0	70.0	
Base Capacity (vph)	717	1931		410	2257	1003	280	451	483	322	444	
Starvation Cap Reductn	0	0		0	0	0	0	0	403	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductin	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.64		0.57	0.22	0.03	0.43	0.11	0.30	0.11	0.29	
	0.00	0.04		0.01	0.22	0.00	0.70	0.11	0.00	U.11	0.23	
Intersection Summary	011											
Area Type:	Other											



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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	∱ }			7
Traffic Volume (veh/h)	0	1414	769	10	0	1
Future Volume (Veh/h)	0	1414	769	10	0	1
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	1414	769	10	0	1
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)		187	113			
pX, platoon unblocked	0.90				0.80	0.90
vC, conflicting volume	779				1481	390
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	534				534	102
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	927				379	841
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	707	707	513	266	1	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	10	1	
cSH	1700	1700	1700	1700	841	
Volume to Capacity	0.42	0.42	0.30	0.16	0.00	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	
Control Delay (s)	0.0	0.0	0.0	0.0	9.3	
Lane LOS					Α	
Approach Delay (s)	0.0		0.0		9.3	
Approach LOS					Α	
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliza	ition		44.6%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	1	2	49	0	2	1	86	24	3	96	0
Future Volume (Veh/h)	1	1	2	49	0	2	1	86	24	3	96	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	1	1	2	49	0	2	1	86	24	3	96	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								94				
pX, platoon unblocked												
vC, conflicting volume	204	214	96	204	202	98	96			110		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	204	214	96	204	202	98	96			110		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)						<u> </u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	751	682	960	749	692	958	1498			1480		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	4	51	111	99								
Volume Left	1	49	1	3								
Volume Right	2	2	24	0								
cSH	819	756	1498	1480								
Volume to Capacity	0.00	0.07	0.00	0.00								
Queue Length 95th (m)	0.1	1.6	0.0	0.0								
Control Delay (s)	9.4	10.1	0.1	0.2								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.4	10.1	0.1	0.2								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utiliz	ation		23.2%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	1>	
Traffic Volume (veh/h)	4	32	14	315	578	16
Future Volume (Veh/h)	4	32	14	315	578	16
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	4	32	14	315	578	16
Pedestrians	•			0.0	<u> </u>	
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				140110	140110	
Upstream signal (m)				92		
pX, platoon unblocked				52		
vC, conflicting volume	929	586	594			
vC1, stage 1 conf vol	020	000	001			
vC2, stage 2 conf vol						
vCu, unblocked vol	929	586	594			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	94	99			
cM capacity (veh/h)	293	510	982			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	36	329	594			
Volume Left	4	14	0			
Volume Right	32	0	16			
cSH	471	982	1700			
Volume to Capacity	0.08	0.01	0.35			
Queue Length 95th (m)	1.9	0.3	0.0			
Control Delay (s)	13.3	0.5	0.0			
Lane LOS	В	Α				
Approach Delay (s)	13.3	0.5	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utiliz	zation		43.1%	IC	CU Level o	of Service
Analysis Period (min)			15			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	f)		*	1>	
Traffic Volume (vph)	231	666	18	19	1497	218	96	33	26	182	7	269
Future Volume (vph)	231	666	18	19	1497	218	96	33	26	182	7	269
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0		-	25.0		-
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		111.2			137.3			86.0			92.6	
Travel Time (s)		6.7			8.2			6.2			6.7	
Lane Group Flow (vph)	231	666	18	19	1497	218	96	59	0	182	276	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	20.0	55.0	55.0	20.0	55.0	55.0	45.0	45.0		45.0	45.0	
Total Split (%)	16.0%	44.0%	44.0%	16.0%	44.0%	44.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	28.4	82.6	82.6	7.0	53.9	53.9	23.7	23.7		23.7	23.7	
Actuated g/C Ratio	0.23	0.66	0.66	0.06	0.43	0.43	0.19	0.19		0.19	0.19	
v/c Ratio	0.60	0.30	0.02	0.20	1.03	0.32	0.96	0.18		0.76	0.56	
Control Delay	51.9	11.6	0.1	60.6	65.7	14.0	128.3	25.1		66.2	9.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	51.9	11.6	0.1	60.6	65.7	14.0	128.3	25.1		66.2	9.4	
LOS	D	В	Α	Е	Е	В	F	С		Е	Α	
Approach Delay		21.5			59.1			89.0			32.0	
Approach LOS		С			Е			F			С	
Queue Length 50th (m)	51.7	27.8	0.0	4.6	190.0	16.7	23.7	6.8		42.9	1.4	
Queue Length 95th (m)	#87.4	65.5	0.0	12.2	#266.8	37.9	#47.1	16.7		62.0	22.6	
Internal Link Dist (m)		87.2			113.3			62.0			68.6	
Turn Bay Length (m)	65.0		20.0	60.0		20.0	20.0			20.0		
Base Capacity (vph)	384	2239	978	188	1460	679	162	522		388	636	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.60	0.30	0.02	0.10	1.03	0.32	0.59	0.11		0.47	0.43	
Intersection Summary												
Area Type:	Other											

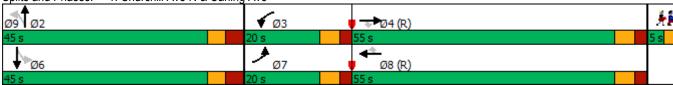
Lane Configurations Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (m) Storage Length (m) Storage Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (s) Total Split (s) Total Split (s) Sol All-Red Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay Los Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (m) Storage Lanes Taper Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (s) Total Split (w) Yellow Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay Lost Approach LoS Approach LoS Approach LoS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Storage Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Future Volume (vphpl) Ideal Flow (vphpl) Storage Length (m) Storage Lanes Taper Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) 5.0 Total Split (s) 5.0 Total Split (%) 4% Yellow Time (s) Lost Time (s) Lost Time (s) Lead/Lag Lead/Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Vic Ratio Corage Cap Reductn Storage Cap Reductn Storage Cap Reductn Storage Cap Reductn				
Ideal Flow (vphpl) Storage Length (m) Storage Length (m) Right Turn on Red Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (w) Yellow Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 95th (m) Internal Link Dist (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary	Reduced v/c Ratio			
intersection outfilliary	Intersection Cummers			
	intersection Summary			

Cycle Length: 125 Actuated Cycle Length: 125 Offset: 92 (74%), Referenced to phase 4:EBT and 8:WBT, Start of Green Natural Cycle: 145 Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.03 Intersection Signal Delay: 46.2 Intersection LOS: D Intersection Capacity Utilization 113.5% ICU Level of Service H Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

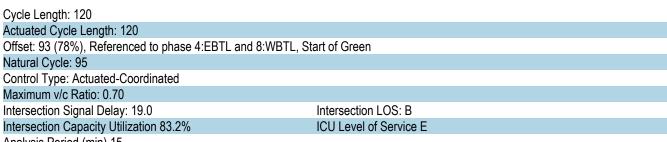
Queue shown is maximum after two cycles.

Splits and Phases: 1: Churchill Ave N & Carling Ave

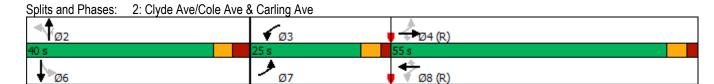


Synchro 10 Report Parsons

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	ች	^	7	ሻ	†	7	ሻ	₽	
Traffic Volume (vph)	74	673	1	241	1558	51	126	87	210	30	54	62
Future Volume (vph)	74	673	1	241	1558	51	126	87	210	30	54	62
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			188.8			103.0			93.9	
Travel Time (s)		7.1			11.3			7.4			8.5	
Lane Group Flow (vph)	74	673	1	241	1558	51	126	87	210	30	116	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	25.0	55.0	55.0	25.0	55.0	55.0	40.0	40.0	40.0	40.0	40.0	
Total Split (%)	20.8%	45.8%	45.8%	20.8%	45.8%	45.8%	33.3%	33.3%	33.3%	33.3%	33.3%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	80.5	72.9	72.9	88.6	78.9	78.9	18.4	18.4	18.4	18.4	18.4	
Actuated g/C Ratio	0.67	0.61	0.61	0.74	0.66	0.66	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.34	0.33	0.00	0.44	0.70	0.05	0.70	0.32	0.58	0.16	0.40	
Control Delay	10.4	13.5	0.0	8.0	17.6	1.0	67.0	46.3	17.6	42.8	29.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	10.4	13.5	0.0	8.0	17.6	1.0	67.0	46.3	17.6	42.8	29.6	
LOS	В	В	Α	Α	В	Α	E	D	В	D	С	
Approach Delay		13.2			15.8			38.2			32.3	
Approach LOS		В			В			D			С	
Queue Length 50th (m)	3.9	37.4	0.0	14.2	116.9	0.0	28.6	18.6	9.2	6.2	14.4	
Queue Length 95th (m)	10.5	65.4	0.0	30.2	189.8	2.3	45.2	30.9	30.0	13.8	29.2	
Internal Link Dist (m)		94.2			164.8			79.0	_		69.9	
Turn Bay Length (m)	20.0		20.0	120.0		20.0			5.0	20.0		
Base Capacity (vph)	388	2060	907	629	2228	960	328	496	528	342	486	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.19	0.33	0.00	0.38	0.70	0.05	0.38	0.18	0.40	0.09	0.24	
Intersection Summary	011											
Area Type:	Other											



Analysis Period (min) 15



Synchro 10 Report Parsons

	•	→	+	4	\	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	ħβ			#
Traffic Volume (veh/h)	0	927	1880	1	0	13
Future Volume (Veh/h)	0	927	1880	1	0	13
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	927	1880	1.00	0	13
Pedestrians		UL.	1000	•		
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		TVOTIC	NONC			
Upstream signal (m)		189	111			
pX, platoon unblocked	0.58	103	111		0.62	0.58
vC, conflicting volume	1881				2344	940
vC1, stage 1 conf vol	1001				2044	340
vC2, stage 2 conf vol						
vCu, unblocked vol	1055				1283	0
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)	4.1				0.0	0.9
tF (s)	2.2				3.5	3.3
	100				100	98
p0 queue free %					97	
cM capacity (veh/h)	377					624
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	464	464	1253	628	13	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	1	13	
cSH	1700	1700	1700	1700	624	
Volume to Capacity	0.27	0.27	0.74	0.37	0.02	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.5	
Control Delay (s)	0.0	0.0	0.0	0.0	10.9	
Lane LOS					В	
Approach Delay (s)	0.0		0.0		10.9	
Approach LOS					В	
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliz	ation		64.9%	IC	U Level	of Service
Analysis Period (min)			15	,,		

Lane Configurations		•	→	•	•	—	•	•	<u>†</u>	~	\	 	1
Traffic Volume (veh/h)	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (veh/h)	Lane Configurations		4			- 43-			- 43-			- €	
Future Volume (Veh/h) 2 1 8 36 1 5 7 122 52 4 60 25		2		8	36		5	7		52	4		2
Sign Control Stop Stop Free Free Grade O% O% O% O% O% O% O% O			1			1		7					2
Grade 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0			Stop			Stop							
Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0			•										
Hourly flow rate (vph) 2 1 8 36 1 5 7 122 52 4 60 2 Pedestrians Lane Width (m) Walking Speed (m/s) Percent Blockage Right turn flare (veh) Median type Median storage veh) Upstream signal (m) Dyx, platoon unblocked 0.98 0.98 0.98 0.98 0.98 0.98 0.98 VC, conflicting volume 236 257 61 240 232 148 62 174 VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 1 conf vol VC3 to 2 conflicting volume 2 conf vol VC4 to 2 conf vol VC9 to 3 conf vol VC9 to 3 conf vol VC9 to 4 conf vol VC9 to 5 conf vol VC9 to 4 conf vol VC9 to 5 conf vol VC9 to 6 conf v		1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Pedestrians Lane Width (m) Walking Speed (m/s)													2
Lane Width (m) Walking Speed (m/s) Percent Blockage Right turn flare (veh) Median type Median type None Median storage veh) Upstream signal (m) pX, platoon unblocked 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98		_	•			•		•		<u> </u>	•		_
Walking Speed (m/s) Percent Blockage Right turn flare (veh) Median type Median storage veh) Upstream signal (m) pX, platoon unblocked VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC3, stage 1 conf vol VC4, stage 1 conf vol VC5, stage 1 conf vol VC4, unblocked vol R 5													
Percent Blockage Right furn flare (veh) Median type None None None Median type None None None Median storage veh Upstream signal (m) PX, platoon unblocked 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0	. ,												
Right turn flare (veh) Median type Median storage veh) Upstream signal (m) pX, platoon unblocked VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC3, stage 5 conf vol VC4, unblocked vol VC5, stage 6 conf vol VC6, stage 8 conf vol VC7, stage 1 conf vol VC8, stage 9 conf vol VC9, unblocked vol VC9, unblocked vol VC9, unblocked vol VC9, stage 1 conf vol VC9, stage 1 conf vol VC9, stage 2 conf vol VC9, stage 2 conf vol VC9, unblocked vol VC9, stage 2 conf vol VC9,													
Median type Median storage veh													
Median storage veh) Upstream signal (m) 94 pX, platoon unblocked 0.98 0.98 0.98 0.98 0.98 0.98 vC, conflicting volume 236 257 61 240 232 148 62 174 vC1, stage 1 conf vol vC2, stage 2 conf vol vCU, unblocked vol 206 227 61 209 201 115 62 142 tC, single (s) 7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1 tC, 2 stage (s) tf (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 11 42 181 66 Volume Right 8 5 52 2 cSH 897 <									None			None	
Upstream signal (m)									140110			110110	
pX, platoon unblocked									94				
vC, conflicting volume 236 257 61 240 232 148 62 174 vC1, stage 1 conf vol vC2, stage 2 conf vol vCU, unblocked vol 206 227 61 209 201 115 62 142 tC, single (s) 7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1 tC, 2 stage (s) tF (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB 1 WB 1 NB 1 SB 1 SB 1 Volume Total 11 42 181 66 60 Volume Left 2 36 7 4 Volume Right 8 5 52 2 2 cSH 897 738 1541 <td< td=""><td></td><td>0.98</td><td>0.98</td><td></td><td>0 98</td><td>0.98</td><td>0.98</td><td></td><td>J-T</td><td></td><td>0.98</td><td></td><td></td></td<>		0.98	0.98		0 98	0.98	0.98		J-T		0.98		
vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 206 227 61 209 201 115 62 142 tC, single (s) 7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1 tC, 2 stage (s) tF (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB1 WB1 NB1 SB1 Volume Total 11 42 181 66 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4				61				62					
vC2, stage 2 conf vol vCu, unblocked vol 206 227 61 209 201 115 62 142 tC, single (s) 7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1 tC, 2 stage (s) tF (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB1 WB1 NB1 SB1 Volume Total 11 42 181 66 Volume Left 2 36 7 4 Volume Right 8 5 52 2 CSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 </td <td></td> <td>200</td> <td>201</td> <td>01</td> <td>240</td> <td>202</td> <td>140</td> <td>02</td> <td></td> <td></td> <td>177</td> <td></td> <td></td>		200	201	01	240	202	140	02			177		
vCu, unblocked vol 206 227 61 209 201 115 62 142 tC, single (s) 7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1 tC, 2 stage (s) tF (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 11 42 181 66 Volume Right 8 5 52 2 2 cSH 897 738 1541 1407 Volume Right 8 5 52 2 2 cSH 897 738 1541 1407 Volume Right 0.01 0.06 0.00 0.00 0.00 0.00 0.00													
tC, single (s) 7.1 6.5 6.2 7.1 6.5 6.2 4.1 4.1 tC, 2 stage (s) tF (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB1 WB1 NB1 SB1 Volume Total 11 42 181 66 Volume Left 2 36 7 4 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1		206	227	61	200	201	115	62			1/12		
tC, 2 stage (s) tF (s)													
tF (s) 3.5 4.0 3.3 3.5 4.0 3.3 2.2 2.2 p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 11 42 181 66 Volume Left 2 36 7 4 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1		7.1	0.5	0.2	7.1	0.0	0.2	7.1			7.1		
p0 queue free % 100 100 99 95 100 99 100 100 cM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 11 42 181 66 Volume Left 2 36 7 4 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach LOS A B A A Intersection Summary Average Delay 2.1		3.5	4.0	3 3	3.5	4.0	3 3	2.2			2.2		
CM capacity (veh/h) 725 652 1004 720 673 915 1541 1407 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 11 42 181 66 Volume Left 2 36 7 4 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach LOS A B A A Approach LOS A B A A Intersection Summary Average Delay 2.1													
Direction, Lane #	•												
Volume Total 11 42 181 66 Volume Left 2 36 7 4 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1	,					073	913	1341			1407		
Volume Left 2 36 7 4 Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
Volume Right 8 5 52 2 cSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
CSH 897 738 1541 1407 Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
Volume to Capacity 0.01 0.06 0.00 0.00 Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B A B Intersection Summary Average Delay 2.1													
Queue Length 95th (m) 0.3 1.4 0.1 0.1 Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
Control Delay (s) 9.1 10.2 0.3 0.5 Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
Lane LOS A B A A Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
Approach Delay (s) 9.1 10.2 0.3 0.5 Approach LOS A B Intersection Summary Average Delay 2.1													
Approach LOS A B Intersection Summary Average Delay 2.1													
Intersection Summary Average Delay 2.1				0.3	0.5								
Average Delay 2.1	Approach LOS	Α	В										
	Intersection Summary												
	Average Delay			2.1									
		ation		27.8%	IC	U Level	of Service			Α			
Analysis Period (min) 15													

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	1>	
Traffic Volume (veh/h)	9	25	31	464	454	17
Future Volume (Veh/h)	9	25	31	464	454	17
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	9	25	31	464	454	17
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				140110	110110	
Upstream signal (m)				93		
pX, platoon unblocked				30		
vC, conflicting volume	988	462	471			
vC1, stage 1 conf vol	300	702	7/1			
vC2, stage 2 conf vol						
vCu, unblocked vol	988	462	471			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	96	97			
cM capacity (veh/h)	266	599	1091			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	34	495	471			
Volume Left	9	31	0			
Volume Right	25	0	17			
cSH	450	1091	1700			
Volume to Capacity	0.08	0.03	0.28			
Queue Length 95th (m)	1.9	0.7	0.0			
Control Delay (s)	13.7	0.8	0.0			
Lane LOS	В	Α				
Approach Delay (s)	13.7	0.8	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utiliz	zation		62.6%	IC	CU Level o	of Service
Analysis Period (min)			15			



	۶	→	•	•	—	•	4	†	<i>></i>	/	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	f)		ሻ	f)	
Traffic Volume (vph)	168	1108	98	85	492	154	11	5	11	344	31	245
Future Volume (vph)	168	1108	98	85	492	154	11	5	11	344	31	245
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0			25.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		115.1			137.3			90.4			90.1	
Travel Time (s)		6.9			8.2			6.5			6.5	
Lane Group Flow (vph)	168	1108	98	85	492	154	11	16	0	344	276	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	25.0	50.0	50.0	25.0	50.0	50.0	45.0	45.0		45.0	45.0	
Total Split (%)	20.0%	40.0%	40.0%	20.0%	40.0%	40.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	16.3	56.3	56.3	11.6	51.6	51.6	38.1	38.1		38.1	38.1	
Actuated g/C Ratio	0.13	0.45	0.45	0.09	0.41	0.41	0.30	0.30		0.30	0.30	
v/c Ratio	0.76	0.73	0.14	0.54	0.35	0.24	0.05	0.03		0.86	0.44	
Control Delay	73.9	33.2	4.7	66.2	27.8	11.1	28.5	16.2		61.8	7.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	73.9	33.2	4.7	66.2	27.8	11.1	28.5	16.2		61.8	7.7	
LOS	E	С	Α	Е	С	В	С	В		Е	Α	
Approach Delay		36.2			28.7			21.2			37.7	
Approach LOS		D			С			С			D	
Queue Length 50th (m)	39.9	118.3	0.0	20.3	45.2	7.7	1.9	0.8		77.8	5.3	
Queue Length 95th (m)	63.2	161.6	10.1	35.5	63.2	23.9	6.0	5.7		#114.0	25.1	
Internal Link Dist (m)		91.1			113.3			66.4			66.1	
Turn Bay Length (m)	65.0		20.0	60.0		20.0	20.0			20.0		
Base Capacity (vph)	256	1527	706	256	1399	652	257	516		425	652	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.66	0.73	0.14	0.33	0.35	0.24	0.04	0.03		0.81	0.42	
Intersection Summary												
Area Type:	Other											

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Right Turn on Red		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	3	
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	
Minimum Split (s)	5.0	
	5.0	
Total Split (s)	4%	
Total Split (%)	2.0	
Yellow Time (s)		
All-Red Time (s)	0.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?	N	
Recall Mode	None	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

1: Churchill Ave N & Carling Ave Cycle Length: 125 Actuated Cycle Length: 125

Offset: 101 (81%), Referenced to phase 4:EBT and 8:WBT, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 34.4 Intersection LOS: C Intersection Capacity Utilization 82.5% ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

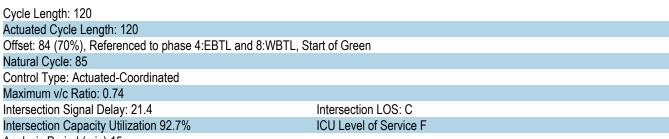
Queue shown is maximum after two cycles.

Splits and Phases: 1: Churchill Ave N & Carling Ave



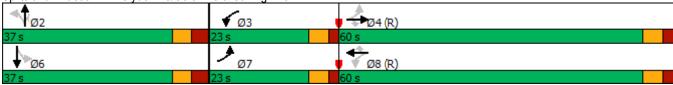
Synchro 10 Report Parsons

	٠	→	•	•	+	•	•	†	~	/	ţ	√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	*	^	7	ሻ	†	7	ሻ	f	
Traffic Volume (vph)	60	1186	0	260	489	51	121	51	147	35	63	66
Future Volume (vph)	60	1186	0	260	489	51	121	51	147	35	63	66
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			185.0			103.0			95.8	
Travel Time (s)		7.1			11.1			7.4			8.6	
Lane Group Flow (vph)	60	1186	0	260	489	51	121	51	147	35	129	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	23.0	60.0	60.0	23.0	60.0	60.0	37.0	37.0	37.0	37.0	37.0	
Total Split (%)	19.2%	50.0%	50.0%	19.2%	50.0%	50.0%	30.8%	30.8%	30.8%	30.8%	30.8%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	74.5	67.4		90.2	79.9	79.9	17.8	17.8	17.8	17.8	17.8	
Actuated g/C Ratio	0.62	0.56		0.75	0.67	0.67	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.11	0.62		0.65	0.22	0.05	0.74	0.19	0.43	0.19	0.47	
Control Delay	6.8	21.6		17.8	9.5	1.0	73.5	43.8	10.4	44.2	35.2	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	6.8	21.6		17.8	9.5	1.0	73.5	43.8	10.4	44.2	35.2	
LOS	A	С		В	Α	Α	E	D	В	D	D	
Approach Delay		20.8			11.6			39.7			37.2	
Approach LOS		С			В		_	D		_	D	
Queue Length 50th (m)	3.1	95.4		15.3	22.7	0.0	27.7	10.7	0.0	7.3	18.7	
Queue Length 95th (m)	8.5	148.4		47.9	38.8	2.2	44.8	20.5	16.3	15.6	34.7	
Internal Link Dist (m)		94.2			161.0			79.0			71.8	
Turn Bay Length (m)	20.0			120.0		20.0			5.0	20.0		
Base Capacity (vph)	720	1904		426	2257	1003	280	451	483	322	444	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.62		0.61	0.22	0.05	0.43	0.11	0.30	0.11	0.29	
Intersection Summary	Other											
Area Type:	Other											



Analysis Period (min) 15





	٠	→	+	4	\	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	↑ ↑			7
Traffic Volume (veh/h)	0	1374	732	18	0	57
Future Volume (Veh/h)	0	1374	732	18	0	57
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	1374	732	18	0	57
Pedestrians					-	
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)		185	115			
pX, platoon unblocked	0.90				0.81	0.90
vC, conflicting volume	750				1428	375
vC1, stage 1 conf vol						<u> </u>
vC2, stage 2 conf vol						
vCu, unblocked vol	513				526	98
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	93
cM capacity (veh/h)	949				389	849
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	687					
		687	488	262	57	
Volume Left	0	0	0	0	0	
Volume Right	0	1700	0	18	57	
cSH	1700	1700	1700	1700	849	
Volume to Capacity	0.40	0.40	0.29	0.15	0.07	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.6	
Control Delay (s)	0.0	0.0	0.0	0.0	9.5	
Lane LOS	2.0		2.0		A	
Approach Delay (s)	0.0		0.0		9.5	
Approach LOS					Α	
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliz	zation		43.4%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	1	2	49	0	2	1	86	44	3	96	0
Future Volume (Veh/h)	1	1	2	49	0	2	1	86	44	3	96	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	1	1	2	49	0	2	1	86	44	3	96	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								96				
pX, platoon unblocked												
vC, conflicting volume	214	234	96	214	212	108	96			130		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	214	234	96	214	212	108	96			130		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	V. <u></u>		0.0	V. <u>–</u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	740	665	960	738	683	946	1498			1455		
					000	J-10	1400			1400		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	4	51	131	99								
Volume Left	1	49	1	3								
Volume Right	2	2	44	0								
cSH	810	745	1498	1455								
Volume to Capacity	0.00	0.07	0.00	0.00								
Queue Length 95th (m)	0.1	1.7	0.0	0.0								
Control Delay (s)	9.5	10.2	0.1	0.2								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.5	10.2	0.1	0.2								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.1									
Intersection Capacity Utiliza	ation		23.9%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	ĵ.	
Traffic Volume (veh/h)	15	40	14	315	580	16
Future Volume (Veh/h)	15	40	14	315	580	16
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	15	40	14	315	580	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)				90		
pX, platoon unblocked						
vC, conflicting volume	931	588	596			
vC1, stage 1 conf vol	001	000	000			
vC2, stage 2 conf vol						
vCu, unblocked vol	931	588	596			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	95	92	99			
cM capacity (veh/h)	292	509	980			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	55	329	596			
Volume Left	15	14	0			
Volume Right	40	0	16			
cSH	423	980	1700			
Volume to Capacity	0.13	0.01	0.35			
Queue Length 95th (m)	3.4	0.3	0.0			
Control Delay (s)	14.8	0.5	0.0			
Lane LOS	В	Α				
Approach Delay (s)	14.8	0.5	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utiliz	zation		43.4%	IC	CU Level o	of Service
Analysis Period (min)			15			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	ሻ	^	7	ሻ	f)		ች	f)	
Traffic Volume (vph)	236	641	18	19	1453	218	96	33	26	185	7	281
Future Volume (vph)	236	641	18	19	1453	218	96	33	26	185	7	281
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0			25.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		106.9			137.3			86.0			92.7	
Travel Time (s)		6.4			8.2			6.2			6.7	
Lane Group Flow (vph)	236	641	18	19	1453	218	96	59	0	185	288	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	20.0	55.0	55.0	20.0	55.0	55.0	45.0	45.0		45.0	45.0	
Total Split (%)	16.0%	44.0%	44.0%	16.0%	44.0%	44.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	29.1	82.3	82.3	7.0	52.9	52.9	24.0	24.0		24.0	24.0	
Actuated g/C Ratio	0.23	0.66	0.66	0.06	0.42	0.42	0.19	0.19		0.19	0.19	
v/c Ratio	0.60	0.29	0.02	0.20	1.01	0.33	1.02	0.17		0.76	0.57	
Control Delay	51.4	11.6	0.1	60.6	63.1	14.2	146.1	24.9		66.2	9.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	51.4	11.6	0.1	60.6	63.1	14.2	146.1	24.9		66.2	9.4	
LOS	D	В	Α	Е	Е	В	F	С		Е	Α	
Approach Delay		21.9			56.8			100.0			31.6	
Approach LOS		С			Е			F			С	
Queue Length 50th (m)	52.6	26.7	0.0	4.6	183.0	16.9	~25.3	6.8		43.6	1.4	
Queue Length 95th (m)	#91.5	63.0	0.0	12.2	#255.0	37.9	#49.4	16.7		62.9	22.8	
Internal Link Dist (m)		82.9			113.3			62.0			68.7	
Turn Bay Length (m)	65.0		20.0	60.0		20.0	20.0			20.0		
Base Capacity (vph)	394	2232	974	188	1434	669	150	522		388	644	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.60	0.29	0.02	0.10	1.01	0.33	0.64	0.11		0.48	0.45	
Intersection Summary												
Area Type:	Other											

Lane Group	Ø9		
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m) Right Turn on Red			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Lane Group Flow (vph)			
Turn Type	^		
Protected Phases	9		
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	3.0		
Minimum Split (s)	5.0		
Total Split (s)	5.0		
Total Split (%)	4%		
Yellow Time (s)	2.0		
All-Red Time (s)	0.0		
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag			
Lead-Lag Optimize?			
Recall Mode	None		
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

1: Churchill Ave N & Carling Ave

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 92 (74%), Referenced to phase 4:EBT and 8:WBT, Start of Green

Natural Cycle: 145

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.02

Intersection Signal Delay: 45.4 Intersection LOS: D

Intersection Capacity Utilization 112.9% ICU Level of Service H

Analysis Period (min) 15

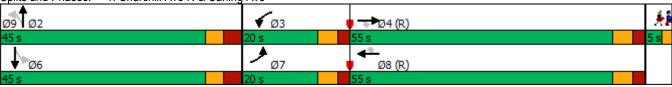
Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

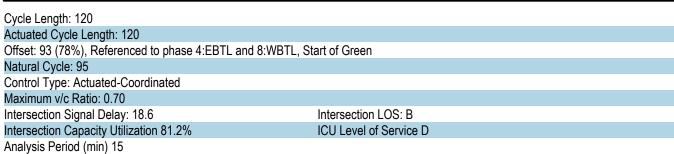
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Churchill Ave N & Carling Ave



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	*	^	7	ሻ		7	ሻ	₽	
Traffic Volume (vph)	76	645	1	250	1486	57	126	87	210	30	54	62
Future Volume (vph)	76	645	1	250	1486	57	126	87	210	30	54	62
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			193.2			103.0			96.4	
Travel Time (s)		7.1			11.6			7.4			8.7	
Lane Group Flow (vph)	76	645	1	250	1486	57	126	87	210	30	116	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	25.0	55.0	55.0	25.0	55.0	55.0	40.0	40.0	40.0	40.0	40.0	
Total Split (%)	20.8%	45.8%	45.8%	20.8%	45.8%	45.8%	33.3%	33.3%	33.3%	33.3%	33.3%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	80.3	72.7	72.7	88.7	78.8	78.8	18.4	18.4	18.4	18.4	18.4	
Actuated g/C Ratio	0.67	0.61	0.61	0.74	0.66	0.66	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.32	0.31	0.00	0.45	0.67	0.06	0.70	0.32	0.58	0.16	0.40	
Control Delay	9.6	13.5	0.0	8.0	16.7	1.4	67.0	46.3	17.6	42.8	29.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	9.6	13.5	0.0	8.0	16.7	1.4	67.0	46.3	17.6	42.8	29.6	
LOS	A	B	Α	Α	15.0	Α	Е	D	В	D	C	
Approach Delay		13.1			15.0			38.2			32.3	
Approach LOS	4.0	B	0.0	110	B 107.5	0.0	20.6	D	9.2	6.2	C 14.4	
Queue Length 50th (m)	4.0 10.7	35.6 62.9	0.0	14.8 31.3	174.6	0.0 3.3	28.6	18.6 30.9		13.8	29.2	
Queue Length 95th (m) Internal Link Dist (m)	10.7	94.2	0.0	31.3	169.2	ა.ა	45.2	79.0	30.0	13.0	72.4	
Turn Bay Length (m)	20.0	94.2	20.0	120.0	109.2	20.0		79.0	5.0	20.0	12.4	
Base Capacity (vph)	405	2053	904	641	2226	959	328	496	528	342	486	
Starvation Cap Reductn	0	2000	904	041	0	959	0	490	0	0	400	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn Reduced v/c Ratio	0.19	0.31	0.00	0.39	0.67	0.06	0.38	0.18	0.40	0.09	0.24	
	0.19	0.51	0.00	0.39	0.07	0.00	0.36	0.16	0.40	0.09	0.24	
Intersection Summary												
Area Type:	Other											



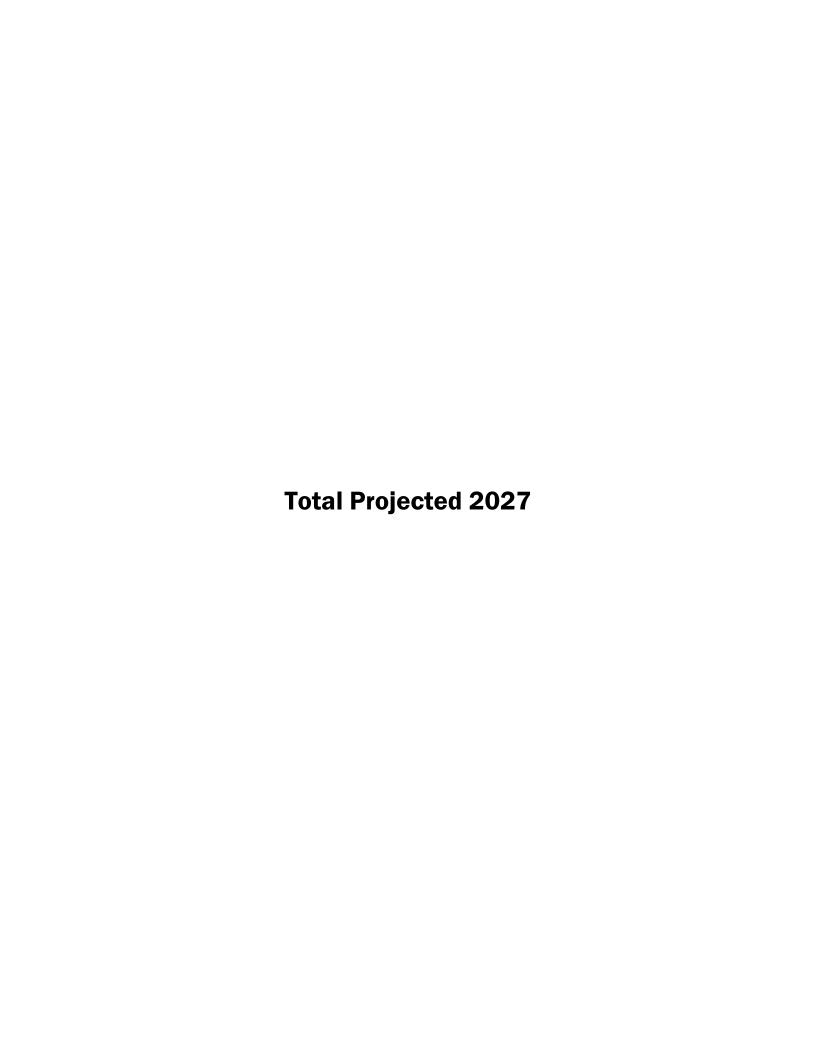


Synchro 10 Report Parsons

	٠	→	+	4	\	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	↑ ↑			7
Traffic Volume (veh/h)	0	894	1790	48	0	30
Future Volume (Veh/h)	0	894	1790	48	0	30
Sign Control	•	Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	894	1790	48	0	30
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)		193	107			
pX, platoon unblocked	0.58		.07		0.63	0.58
vC, conflicting volume	1838				2261	919
vC1, stage 1 conf vol	1000				2201	0.10
vC2, stage 2 conf vol						
vCu, unblocked vol	1007				1202	0
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)					0.0	
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	95
cM capacity (veh/h)	399				111	632
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total						
	447	447	1193	645	30	
Volume Left	0	0	0	0	0	
Volume Right	0	1700	0	48	30	
cSH	1700	1700	1700	1700	632	
Volume to Capacity	0.26	0.26	0.70	0.38	0.05	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.1	
Control Delay (s)	0.0	0.0	0.0	0.0	11.0	
Lane LOS	0.0		0.0		В	
Approach Delay (s)	0.0		0.0		11.0	
Approach LOS					В	
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utili	zation		63.8%	IC	U Level of	of Service
Analysis Period (min)			15			

	•	→	•	•	+	•	•	†	<i>></i>	/	↓	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	1	8	36	1	5	7	122	60	4	60	2
Future Volume (Veh/h)	2	1	8	36	1	5	7	122	60	4	60	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	2	1	8	36	1	5	7	122	60	4	60	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								96				
pX, platoon unblocked	0.98	0.98		0.98	0.98	0.98				0.98		
vC, conflicting volume	240	265	61	244	236	152	62			182		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	210	235	61	213	205	119	62			150		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	95	100	99	100			100		
cM capacity (veh/h)	721	645	1004	715	670	910	1541			1397		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	11	42	189	66								
Volume Left	2	36	7	4								
	8	5	60	2								
Volume Right cSH	895	733	1541	1397								
Volume to Capacity	0.01	0.06	0.00	0.00								
	0.01	1.4	0.00	0.00								
Queue Length 95th (m)	9.1											
Control Delay (s)		10.2	0.3	0.5								
Lane LOS	Α	B	A	A								
Approach LOS	9.1	10.2	0.3	0.5								
Approach LOS	Α	В										
Intersection Summary												
Average Delay		2.0										
			28.4%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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	•	•	1	Ī	¥	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥			र्स	₽		
Traffic Volume (veh/h)	12	30	31	464	463	17	
Future Volume (Veh/h)	12	30	31	464	463	17	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	12	30	31	464	463	17	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				110110	110110		
Upstream signal (m)				93			
pX, platoon unblocked							
vC, conflicting volume	998	472	480				
vC1, stage 1 conf vol	330	712	100				
vC2, stage 2 conf vol							
vCu, unblocked vol	998	472	480				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.4	0.2	7.1				
tF (s)	3.5	3.3	2.2				
p0 queue free %	95	95	97				
cM capacity (veh/h)	263	592	1082				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	42	495	480				
Volume Left	12	31	0				
Volume Right	30	0	17				
cSH	436	1082	1700				
Volume to Capacity	0.10	0.03	0.28				
Queue Length 95th (m)	2.4	0.7	0.0				
Control Delay (s)	14.1	0.8	0.0				
Lane LOS	В	Α					
Approach Delay (s)	14.1	0.8	0.0				
Approach LOS	В						
Intersection Summary							
Average Delay			1.0				
Intersection Capacity Utiliza	ation		62.6%	IC	CU Level o	of Service	В
Analysis Period (min)			15				



	٠	→	•	•	+	4	4	†	~	/	+	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	ĵ»		ሻ	1>	
Traffic Volume (vph)	168	1162	98	85	516	154	11	5	11	344	31	245
Future Volume (vph)	168	1162	98	85	516	154	11	5	11	344	31	245
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	65.0		20.0	60.0		20.0	20.0		0.0	20.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	25.0			15.0			20.0			25.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		112.5			137.3			90.4			92.6	
Travel Time (s)		6.8			8.2			6.5			6.7	
Lane Group Flow (vph)	168	1162	98	85	516	154	11	16	0	344	276	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases			4			8	2			6		
Detector Phase	7	4	4	3	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Total Split (s)	25.0	50.0	50.0	25.0	50.0	50.0	45.0	45.0		45.0	45.0	
Total Split (%)	20.0%	40.0%	40.0%	20.0%	40.0%	40.0%	36.0%	36.0%		36.0%	36.0%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3		3.3	3.3	
All-Red Time (s)	2.4	2.4	2.4	2.4	2.4	2.4	3.5	3.5		3.5	3.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	16.3	56.3	56.3	11.6	51.6	51.6	38.1	38.1		38.1	38.1	
Actuated g/C Ratio	0.13	0.45	0.45	0.09	0.41	0.41	0.30	0.30		0.30	0.30	
v/c Ratio	0.76	0.76	0.14	0.54	0.37	0.24	0.05	0.03		0.86	0.44	
Control Delay	73.9	34.5	4.7	66.2	28.0	11.1	28.5	16.2		61.8	7.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	73.9	34.5	4.7	66.2	28.0	11.1	28.5	16.2		61.8	7.7	
LOS	Е	С	Α	Ε	С	В	С	В		Е	Α	
Approach Delay		37.1			28.9			21.2			37.7	
Approach LOS		D			С			С			D	
Queue Length 50th (m)	39.9	127.1	0.0	20.3	47.7	7.7	1.9	0.8		77.8	5.3	
Queue Length 95th (m)	63.2	#183.9	10.1	35.5	66.4	23.9	6.0	5.7		#114.0	25.1	
Internal Link Dist (m)		88.5			113.3			66.4			68.6	
Turn Bay Length (m)	65.0		20.0	60.0		20.0	20.0			20.0		
Base Capacity (vph)	256	1527	706	256	1399	652	257	516		425	652	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.66	0.76	0.14	0.33	0.37	0.24	0.04	0.03		0.81	0.42	
Intersection Summary												
Area Type:	Other											
nied Type.	Other											

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Right Turn on Red		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	3	
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	
Minimum Split (s)	5.0	
	5.0	
Total Split (s)	4%	
Total Split (%)	2.0	
Yellow Time (s)		
All-Red Time (s)	0.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?	N	
Recall Mode	None	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

1: Churchill Ave N & Carling Ave

Cycle Length: 125
Actuated Cycle Length: 125
Offset: 101 (81%), Referenced to phase 4:EBT and 8:WBT, Start of Green
Natural Cycle: 105
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.86
Intersection Signal Delay: 34.9
Intersection LOS: C
Intersection Capacity Utilization 84.1%
ICU Level of Service E

Analysis Period (min) 15

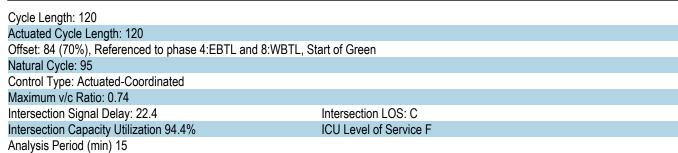
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

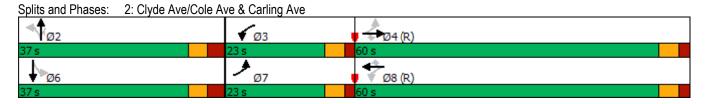
Splits and Phases: 1: Churchill Ave N & Carling Ave



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	ች	^	7	*	1	7	ሻ	₽	
Traffic Volume (vph)	60	1245	0	260	513	51	121	51	147	35	63	66
Future Volume (vph)	60	1245	0	260	513	51	121	51	147	35	63	66
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			187.6			103.0			93.4	
Travel Time (s)		7.1			11.3			7.4			8.4	
Lane Group Flow (vph)	60	1245	0	260	513	51	121	51	147	35	129	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	23.0	60.0	60.0	23.0	60.0	60.0	37.0	37.0	37.0	37.0	37.0	
Total Split (%)	19.2%	50.0%	50.0%	19.2%	50.0%	50.0%	30.8%	30.8%	30.8%	30.8%	30.8%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	73.5	66.4		90.2	79.9	79.9	17.8	17.8	17.8	17.8	17.8	
Actuated g/C Ratio	0.61	0.55		0.75	0.67	0.67	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.11	0.66		0.66	0.23	0.05	0.74	0.19	0.43	0.19	0.47	
Control Delay	6.9	23.1		21.3	9.6	1.0	73.5	43.8	10.4	44.2	35.2	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	6.9	23.1		21.3	9.6	1.0	73.5	43.8	10.4	44.2	35.2	
LOS	Α	С		С	Α	Α	Е	D	В	D	D	
Approach Delay		22.3			12.7			39.7			37.2	
Approach LOS		С			В			D			D	
Queue Length 50th (m)	3.1	105.8		20.3	24.1	0.0	27.7	10.7	0.0	7.3	18.7	
Queue Length 95th (m)	8.5	159.8		53.0	40.7	2.2	44.8	20.5	16.3	15.6	34.7	
Internal Link Dist (m)		94.2			163.6			79.0			69.4	
Turn Bay Length (m)	20.0			120.0		20.0			5.0	20.0		
Base Capacity (vph)	704	1876		413	2257	1003	280	451	483	322	444	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.09	0.66		0.63	0.23	0.05	0.43	0.11	0.30	0.11	0.29	
Intersection Summary												
Area Type:	Other											



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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	∱ }			7
Traffic Volume (veh/h)	0	1441	769	18	0	57
Future Volume (Veh/h)	0	1441	769	18	0	57
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	1441	769	18	0	57
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)		187	112			
pX, platoon unblocked	0.90				0.79	0.90
vC, conflicting volume	787				1498	394
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	538				513	100
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	93
cM capacity (veh/h)	923				386	842
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	720	720	513	274	57	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	18	57	
cSH	1700	1700	1700	1700	842	
Volume to Capacity	0.42	0.42	0.30	0.16	0.07	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.7	
Control Delay (s)	0.0	0.0	0.0	0.0	9.6	
Lane LOS					Α	
Approach Delay (s)	0.0		0.0		9.6	
Approach LOS					А	
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilizati	ion		45.4%	IC	U Level o	of Service
Analysis Period (min)			15			

	•	→	•	•	←	4	•	†	<i>></i>	\	+	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	1	2	49	0	2	1	86	44	3	96	0
Future Volume (Veh/h)	1	1	2	49	0	2	1	86	44	3	96	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	1	1	2	49	0	2	1	86	44	3	96	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								93				
pX, platoon unblocked												
vC, conflicting volume	214	234	96	214	212	108	96			130		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	214	234	96	214	212	108	96			130		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)						<u> </u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	740	665	960	738	683	946	1498			1455		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	4	51	131	99								
Volume Left	1	49	1	3								
Volume Right	2	2	44	0								
cSH	810	745	1498	1455								
Volume to Capacity	0.00	0.07	0.00	0.00								
Queue Length 95th (m)	0.1	1.7	0.0	0.0								
Control Delay (s)	9.5	10.2	0.1	0.2								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.5	10.2	0.1	0.2								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.1									
Intersection Capacity Utiliz	ation		23.9%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

	<u> </u>		•	+	ı	٦	
		*	7	ı	*	•	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥			र्स	₽		
Traffic Volume (veh/h)	15	40	14	315	580	16	
Future Volume (Veh/h)	15	40	14	315	580	16	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	15	40	14	315	580	16	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)				93			
pX, platoon unblocked							
vC, conflicting volume	931	588	596				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	931	588	596				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.1	0.2					
tF (s)	3.5	3.3	2.2				
p0 queue free %	95	92	99				
cM capacity (veh/h)	292	509	980				
		NB 1	SB 1				
Direction, Lane #	EB 1						
Volume Total	55	329	596				
Volume Left	15	14	0				
Volume Right	40	0	16				
cSH	423	980	1700				
Volume to Capacity	0.13	0.01	0.35				
Queue Length 95th (m)	3.4	0.3	0.0				
Control Delay (s)	14.8	0.5	0.0				
Lane LOS	В	A	2.5				
Approach Delay (s)	14.8	0.5	0.0				
Approach LOS	В						
Intersection Summary							
Average Delay			1.0				
Intersection Capacity Utiliza	ation		43.4%	IC	CU Level o	of Service	Α
Analysis Period (min)			15				

Lane Corolguations		٠	→	•	•	←	4	4	†	<i>></i>	/	ţ	4
Traffic Volume (vph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations	*	^	7	ች	44	7	*	f)		*	ĵ.	
Future Volume (viph)										26			281
Ideal Flow (ryphiph 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1800 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000	(, ,											7	
Storage Length (m)		1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Lanes	(, , ,	65.0		20.0	60.0		20.0			0.0	20.0		
Right Turn on Red		1		1	1		1	1		0	1		0
Link Speed (k/h)	Taper Length (m)	25.0			15.0			20.0			25.0		
Link Distance (m)	Right Turn on Red			Yes			Yes			Yes			Yes
Travel Time (s)	Link Speed (k/h)		60			60			50			50	
Lane Group Flow (vph) 236 673 18 19 1524 218 96 59 0 185 288 0 Turn Type	Link Distance (m)		111.6			137.3			86.0			93.0	
Turn Type	Travel Time (s)		6.7			8.2			6.2			6.7	
Protected Phases	Lane Group Flow (vph)	236	673	18	19	1524	218	96	59	0	185	288	0
Permitted Phases	Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Detector Phase	Protected Phases	7	4		3	8			2			6	
Switch Phase Minimum Initial (s) 5.0 10.0 10.0 5.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8 44.8	Permitted Phases			4			8	2			6		
Minimum Initial (s)	Detector Phase	7	4	4	3	8	8	2	2		6	6	
Minimum Split (s)	Switch Phase												
Total Split (s)	Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0		10.0	10.0	
Total Split (%)	Minimum Split (s)	11.1	34.1	34.1	11.1	34.1	34.1	44.8	44.8		44.8	44.8	
Yellow Time (s) 3.7 3.7 3.7 3.7 3.7 3.7 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5	Total Split (s)	20.0	55.0	55.0	20.0	55.0	55.0		45.0			45.0	
All-Red Time (s)	Total Split (%)	16.0%	44.0%	44.0%	16.0%	44.0%	44.0%	36.0%	36.0%		36.0%	36.0%	
Lost Time Adjust (s)	Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3			3.3	
Total Lost Time (s)	All-Red Time (s)		2.4		2.4		2.4					3.5	
Lead/Lag Lead Lag Lead Lag Lead Lag Lag <th< td=""><td>Lost Time Adjust (s)</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>	Lost Time Adjust (s)												
Lead-Lag Optimize? Yes	Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.8	6.8		6.8	6.8	
Recall Mode None C-Max C-Max None C-Max C-Max None	<u> </u>												
Act Effct Green (s) 29.1 82.3 82.3 7.0 52.9 52.9 24.0 24.0 24.0 24.0 Actuated g/C Ratio 0.23 0.66 0.66 0.06 0.42 0.42 0.19 0.19 0.19 0.19 v/c Ratio 0.60 0.30 0.02 0.20 1.06 0.33 1.02 0.17 0.76 0.57 Control Delay 51.4 11.7 0.1 60.6 77.6 14.2 146.1 24.9 66.2 9.4 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 <td></td>													
Actuated g/C Ratio 0.23 0.66 0.66 0.06 0.42 0.42 0.19 0.19 0.19 v/c Ratio 0.60 0.30 0.02 0.20 1.06 0.33 1.02 0.17 0.76 0.57 Control Delay 51.4 11.7 0.1 60.6 77.6 14.2 146.1 24.9 66.2 9.4 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0													
v/c Ratio 0.60 0.30 0.02 0.20 1.06 0.33 1.02 0.17 0.76 0.57 Control Delay 51.4 11.7 0.1 60.6 77.6 14.2 146.1 24.9 66.2 9.4 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0													
Control Delay 51.4 11.7 0.1 60.6 77.6 14.2 146.1 24.9 66.2 9.4 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.													
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>													
Total Delay 51.4 11.7 0.1 60.6 77.6 14.2 146.1 24.9 66.2 9.4 LOS D B A E E B F C E A Approach Delay 21.6 69.6 100.0 31.6 A A A E F C E A A A B F C E A A A B F C C E A A B A B B F C C E A A B A B B B F C C E A B A B B B C C C C E A B B B C C C B A B B B B B B B B B B B <td< td=""><td>•</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	•												
LOS D B A E E B F C E A Approach Delay 21.6 69.6 100.0 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.6 31.2 31.6 31.2 31.4 31.9 31.9 31.9 31.9 31.9 31.9 31.9 31.9 31.9 31.9													
Approach Delay 21.6 69.6 100.0 31.6 Approach LOS C E F C Queue Length 50th (m) 52.6 28.4 0.0 4.6 ~211.8 16.9 ~25.3 6.8 43.6 1.4 Queue Length 95th (m) #91.5 66.5 0.0 12.2 #274.1 37.9 #49.4 16.7 62.9 22.8 Internal Link Dist (m) 87.6 113.3 62.0 69.0 Turn Bay Length (m) 65.0 20.0 60.0 20.0 20.0 Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0													
Approach LOS C E F C Queue Length 50th (m) 52.6 28.4 0.0 4.6 ~211.8 16.9 ~25.3 6.8 43.6 1.4 Queue Length 95th (m) #91.5 66.5 0.0 12.2 #274.1 37.9 #49.4 16.7 62.9 22.8 Internal Link Dist (m) 87.6 113.3 62.0 69.0 Turn Bay Length (m) 65.0 20.0 60.0 20.0 20.0 20.0 20.0 Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		D		A	E		В	F			E		
Queue Length 50th (m) 52.6 28.4 0.0 4.6 ~211.8 16.9 ~25.3 6.8 43.6 1.4 Queue Length 95th (m) #91.5 66.5 0.0 12.2 #274.1 37.9 #49.4 16.7 62.9 22.8 Internal Link Dist (m) 87.6 113.3 62.0 69.0 Turn Bay Length (m) 65.0 20.0 60.0 20.0 20.0 20.0 Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <td></td>													
Queue Length 95th (m) #91.5 66.5 0.0 12.2 #274.1 37.9 #49.4 16.7 62.9 22.8 Internal Link Dist (m) 87.6 113.3 62.0 69.0 Turn Bay Length (m) 65.0 20.0 60.0 20.0 20.0 Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>													
Internal Link Dist (m) 87.6 113.3 62.0 69.0 Turn Bay Length (m) 65.0 20.0 60.0 20.0 20.0 20.0 Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	• ,												
Turn Bay Length (m) 65.0 20.0 60.0 20.0 20.0 20.0 Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		#91.5		0.0	12.2		37.9	#49.4			62.9		
Base Capacity (vph) 394 2232 974 188 1434 669 150 522 388 644 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	\ /		87.6			113.3			62.0			69.0	
Starvation Cap Reductn 0 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0													
Spillback Cap Reductn 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0													
Storage Cap Reductn 0 0 0 0 0 0 0 0 Reduced v/c Ratio 0.60 0.30 0.02 0.10 1.06 0.33 0.64 0.11 0.48 0.45 Intersection Summary 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 <td></td>													
Reduced v/c Ratio 0.60 0.30 0.02 0.10 1.06 0.33 0.64 0.11 0.48 0.45 Intersection Summary	•												
Intersection Summary													
•	Reduced v/c Ratio	0.60	0.30	0.02	0.10	1.06	0.33	0.64	0.11		0.48	0.45	
•	Intersection Summary												
		Other											

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Right Turn on Red		
•		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Lane Group Flow (vph)		
Turn Type	_	
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	
Minimum Split (s)	5.0	
Total Split (s)	5.0	
Total Split (%)	4%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)	0.0	
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)	None	
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

1: Churchill Ave N & Carling Ave

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 92 (74%), Referenced to phase 4:EBT and 8:WBT, Start of Green

Natural Cycle: 145

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.06

Intersection Signal Delay: 52.2 Intersection LOS: D

Intersection Capacity Utilization 115.0% ICU Level of Service H

Analysis Period (min) 15

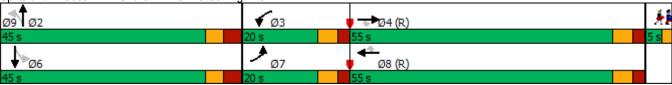
Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

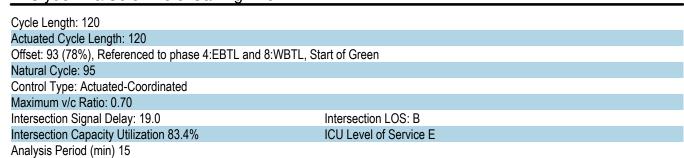
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

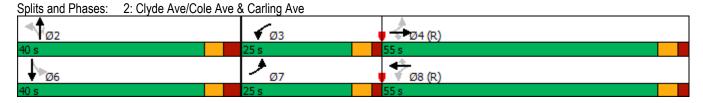
Splits and Phases: 1: Churchill Ave N & Carling Ave



	۶	→	•	•	-	•	1	†	/	/	ļ	√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7	ሻ	†	7	ሻ	1>	
Traffic Volume (vph)	76	677	1	250	1560	57	126	87	210	30	54	62
Future Volume (vph)	76	677	1	250	1560	57	126	87	210	30	54	62
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	20.0		20.0	120.0		20.0	0.0		5.0	20.0		0.0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (m)	20.0			30.0			7.6			20.0		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (k/h)		60			60			50			40	
Link Distance (m)		118.2			188.5			103.0			96.0	
Travel Time (s)		7.1			11.3			7.4			8.6	
Lane Group Flow (vph)	76	677	1	250	1560	57	126	87	210	30	116	0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	10.4	35.5	35.5	10.4	35.5	35.5	37.0	37.0	37.0	37.0	37.0	
Total Split (s)	25.0	55.0	55.0	25.0	55.0	55.0	40.0	40.0	40.0	40.0	40.0	
Total Split (%)	20.8%	45.8%	45.8%	20.8%	45.8%	45.8%	33.3%	33.3%	33.3%	33.3%	33.3%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.7	2.1	2.1	1.7	2.1	2.1	3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.8	5.8	5.4	5.8	5.8	6.6	6.6	6.6	6.6	6.6	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	80.3	72.7	72.7	88.7	78.8	78.8	18.4	18.4	18.4	18.4	18.4	
Actuated g/C Ratio	0.67	0.61	0.61	0.74	0.66	0.66	0.15	0.15	0.15	0.15	0.15	
v/c Ratio	0.35	0.33	0.00	0.46	0.70	0.06	0.70	0.32	0.58	0.16	0.40	
Control Delay	10.6	13.7	0.0	8.2	17.7	1.4	67.0	46.3	17.6	42.8	29.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	10.6	13.7	0.0	8.2	17.7	1.4	67.0	46.3	17.6	42.8	29.6	
LOS	В	42.2	Α	A	45 O	Α	Е	D	В	D	C	
Approach Delay		13.3			15.9			38.2			32.3	
Approach LOS	4.0	37.0	0.0	110	117.6	0.0	20.6	D 18.6	9.2	6.2	C 14.4	
Queue Length 50th (m)	4.0	37.8 66.3	0.0	14.8 31.3	190.8	0.0	28.6	30.9		13.8	29.2	
Queue Length 95th (m) Internal Link Dist (m)	10.7	94.2	0.0	31.3	164.5	3.3	45.2	79.0	30.0	13.0	72.0	
Turn Bay Length (m)	20.0	94.2	20.0	120.0	104.5	20.0		79.0	5.0	20.0	72.0	
	388	2053	904	625	2226	959	328	496	528	342	486	
Base Capacity (vph) Starvation Cap Reductn	0	2053	904	025	0	959	320	496	0	0	400	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.20	0.33	0.00	0.40	0.70	0.06	0.38	0.18	0.40	0.09	0.24	
	0.20	0.55	0.00	0.40	0.70	0.00	0.30	0.10	0.40	0.09	0.24	
Intersection Summary												
Area Type:	Other											



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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	∱ }			7
Traffic Volume (veh/h)	0	938	1880	48	0	30
Future Volume (Veh/h)	0	938	1880	48	0	30
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	938	1880	48	0	30
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110	110110			
Upstream signal (m)		188	112			
pX, platoon unblocked	0.58	100			0.63	0.58
vC, conflicting volume	1928				2373	964
vC1, stage 1 conf vol	1020				20.0	001
vC2, stage 2 conf vol						
vCu, unblocked vol	1162				1342	0
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)					0.0	0.0
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	95
cM capacity (veh/h)	348				90	632
		ED 0	MD 4	WDO		002
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	469	469	1253	675	30	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	48	30	
cSH	1700	1700	1700	1700	632	
Volume to Capacity	0.28	0.28	0.74	0.40	0.05	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.1	
Control Delay (s)	0.0	0.0	0.0	0.0	11.0	
Lane LOS					В	
Approach Delay (s)	0.0		0.0		11.0	
Approach LOS					В	
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utili	zation		66.5%	IC	U Level	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	1	8	36	1	5	7	122	60	4	60	2
Future Volume (Veh/h)	2	1	8	36	1	5	7	122	60	4	60	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	2	1	8	36	1	5	7	122	60	4	60	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								96				
pX, platoon unblocked	0.98	0.98		0.98	0.98	0.98				0.98		
vC, conflicting volume	240	265	61	244	236	152	62			182		
vC1, stage 1 conf vol			<u> </u>			.,,_	<u> </u>					
vC2, stage 2 conf vol												
vCu, unblocked vol	210	235	61	213	205	119	62			150		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	<u> </u>		0.0	V. <u>–</u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	95	100	99	100			100		
cM capacity (veh/h)	721	645	1004	716	670	910	1541			1397		
					070	310	1071			1001		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	11	42	189	66								
Volume Left	2	36	7	4								
Volume Right	8	5	60	2								
cSH	895	733	1541	1397								
Volume to Capacity	0.01	0.06	0.00	0.00								
Queue Length 95th (m)	0.3	1.4	0.1	0.1								
Control Delay (s)	9.1	10.2	0.3	0.5								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	9.1	10.2	0.3	0.5								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			2.0									
Intersection Capacity Utiliza	ation		28.4%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	1>	
Traffic Volume (veh/h)	12	30	31	464	463	17
Future Volume (Veh/h)	12	30	31	464	463	17
Sign Control	Stop		<u> </u>	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	12	30	31	464	463	17
Pedestrians	14		<u> </u>	101	100	.,
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				INOHE	INOTIE	
Upstream signal (m)				93		
				93		
pX, platoon unblocked	998	472	480			
vC, conflicting volume	990	412	400			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	000	470	400			
vCu, unblocked vol	998	472	480			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	2.5	2.2	0.0			
tF (s)	3.5	3.3	2.2			
p0 queue free %	95	95	97			
cM capacity (veh/h)	263	592	1082			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	42	495	480			
Volume Left	12	31	0			
Volume Right	30	0	17			
cSH	436	1082	1700			
Volume to Capacity	0.10	0.03	0.28			
Queue Length 95th (m)	2.4	0.7	0.0			
Control Delay (s)	14.1	0.8	0.0			
Lane LOS	В	Α				
Approach Delay (s)	14.1	0.8	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utiliz	zation		62.6%	ır	ا ا ا ا	of Service
Analysis Period (min)	Lation		15	IC	O LEVEL	DI OGIVICE
Analysis Pellou (IIIIII)			15			