

1950 Scott Street

TIA Strategy Report



PARSONS



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check \vee appropriate field(s)] is either transportation engineering \square or transportation planning \square .
- License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.



Professional Title:

Dated at Ottawa this 30 day of April , 2019 .

(City)

Name: André Sponder

(Please Print)

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Transportation Engineer

Signature of Individual certifier that s/he meets the above four criteria

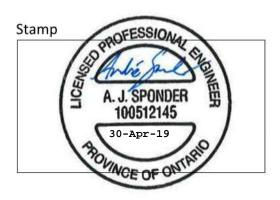
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1950 Scott Street

TIA Strategy Report

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April 30, 2019

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TIA Strategy Report

1. SCREENING FORM

The Screening Form is provided as Appendix A. The trip generation trigger was met based on the development size, the location trigger was met based on the development being in a Design Priority Area (DPA), and the safety trigger was met based on the proposed site driveway's proximity to the Scott/Lanark signalized intersection. As triggers have been met, the TIA process continued with the Scoping and Forecasting reports, provided herein.

2. SCOPING REPORT

2.1. EXISTING AND PLANNED CONDITIONS

2.1.1. PROPOSED DEVELOPMENT

Based on the proposed Site Plan, it is our understanding that the proponent is proposing a single-phase residential development located at 1950 Scott Street with an expected occupancy date in 2020. The proposed residential development will consist of approximately 141 condominium/apartment units with 162 proposed residential parking spaces and 10 visitor parking spaces. A single full-movement vehicle access is proposed to Clifton Road at the southern boundary of the site. The site is located on three property parcels, which are currently occupied by a single occupant one-story building and single-family homes and are zoned as Residential Fifth Density and Residential Third Density. The local context of the site is provided as Figure 1 and the proposed Site Plan is provided as Figure 2. The Site Plan shows temporary lay-bys for deliveries along Scott Street. These spaces are based on the future (ultimate) configuration of Scott Street and are provided for the final building.



Figure 1: Local Context







11646-1950 SCOTT STREET, Ottawa, ON

2.1.2. EXISTING CONDITIONS

Area Road Network

Scott Street is an east-west arterial, which extends from Churchill Avenue in the west to Bayview Road in the east, where it continues as Albert Street. Within the study area, Scott Street has a two-lane cross-section and auxiliary turn lanes are provided at major intersections. On-street parking bays are provided and the posted speed limit within the study area is 50 km/h.

As part of the Stage 2 LRT project, bus detours from the Transitway will be routed down Scott Street until the completion of Stage 2 LRT, expected to be in 2023.

Lanark Avenue/West Village Private is a collector roadway north of Scott Street that continues as a private roadway south of Scott Street (West Village Private). Lanark Avenue has a two-lane undivided cross-section and an unposted speed limit of 50 km/h. West Village Private has a posted speed limit of 25 km/h.

Clifton Road is a north-south local roadway that extends from Scott Street to Richmond Road. The roadway has a two-lane undivided cross-section with on-street parking provided along the east side of the roadway. The unposted speed limit is understood to be 50 km/h.

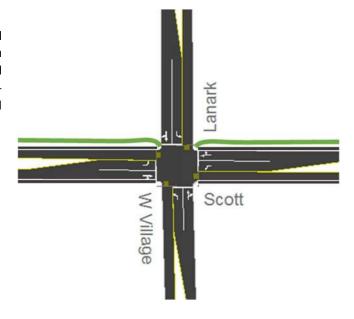
Through previous consultation with residents in the neighbourhood, we understand that there is an existing cut-through traffic issue along Clifton Road. Vehicles travel via Clifton Road, Wilber Avenue and Kirkwood Avenue to access Richmond Road and Scott Street. It is our understanding that the City is aware of this existing cut-through traffic issue and efforts have been made to minimize the amount of traffic on these local roadways. The following mitigative measures are currently installed along Clifton Road:

- Peak hour turn restrictions;
- Multiple curb bulb-outs; and
- Speed humps.

Existing Study Area Intersections

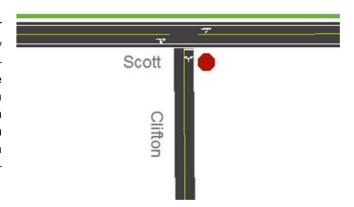
Scott/Lanark

The Scott/Lanark intersection is a 4-legged signalized intersection. All four approaches consist of a single left-turn lane and a shared through/right-turn lane. East and westbound curb bike lanes are provided along with an east-west MUP located to the north of Scott Street. All movements are permitted at this location.



Scott/Clifton

The Scott/Clifton intersection is an unsignalized T intersection with STOP control on the minor approach only (Clifton Road). All three approaches consist of a single full-movement lane. East and westbound curb bike lanes are provided along with an east-west MUP located to the north of Scott Street. There is an existing westbound left-turn restriction during the weekday morning peak period from Scott Street to Clifton Road. It is understood that this turn movement restriction is in place to help minimize cut-through traffic along Clifton Road.



Pedestrian/Cycling Network

According to the City's 2013 Official Cycling Plan (OCP), Scott Street is classified as Spine Route and a Cross-Town Bikeway, and Clifton Road between Scott Street and Wilber Avenue is classified as a "Local Route". Within the study area, a multi-use pathway exists along the north side of Scott Street in addition to on-street bicycle lanes.

Connecting pedestrians to transit service and other adjacent development, sidewalks are currently provided along both sides of Lanark Avenue, along the west side of Clifton Road, and along the south side of Scott Street, with an off-road multi-use pathway provided along the north side of Scott Street.

Transit Network

Transit service within the vicinity of the site is currently provided by OC Transpo Regular Route #50, which provides frequent all-day service. Bus stops for this route are located along Scott Street approximately 40 m from the site.

Rapid transit service (in the form of BRT) is also provided via the Westboro Station, located approximately 250 metres northwest of the proposed development, which provides convenient access to multiple routes along the Transitway. The following Figure 3 summarizes OC Transpo's Transit network system within the vicinity.

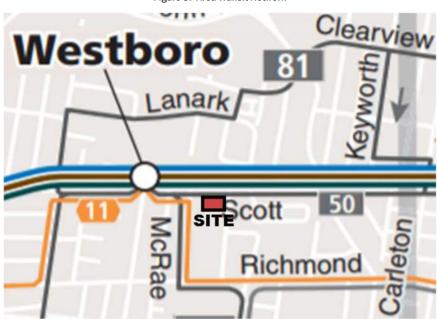
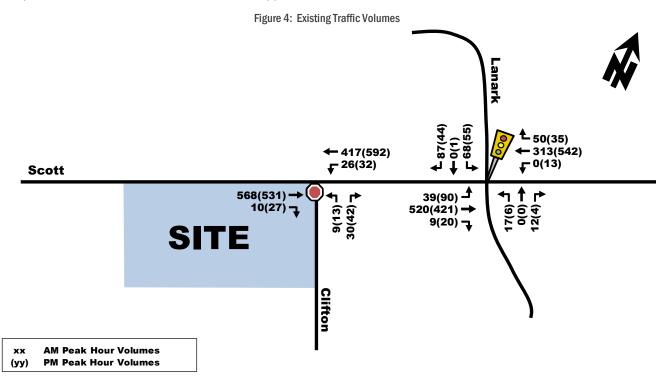


Figure 3: Area Transit Network

Phase 2 of the LRT project will divert transit onto Scott Street and the Westboro Transit Station will become the Westboro LRT Station once Phase 2 is complete by approximately 2023.

Peak Hour Travel Demands

Illustrated as Figure 4, are the most recent weekday morning and afternoon peak hour traffic volumes obtained from the City of Ottawa at the Scott/Lanark intersection and collected by Parsons (May 2018) at the Scott/Clifton intersection. These peak hour traffic volumes are included as Appendix B.



With regards to pedestrian volumes within the study area, the traffic count data indicates that there are approximately 25 to 60 pedestrians per hour along Scott Street and 10 to 30 pedestrians per hour crossing Scott Street along Lanark Road. With regard to cycling volumes, there were 2 to 17 cyclists per hour along Scott Street and 1 to 3 cyclists per hour along Lanark Road. The traffic count data was collected at the end of March 2017.

As mentioned previously, there is a westbound left-turn restriction at the Scott/Clifton intersection during the morning peak period. As shown in Figure 4, there were 26 violators of this left-turn restriction observed during one hour. In addition, there had previously been a northbound right-turn restriction at this location during the afternoon peak hour. It was observed that one of the signs (located along the north side of Scott Street) has been removed and the other sign (located along the east side of Clifton Road) is bent and requires repair as shown below. If the northbound right-turn restriction is in place, it is violated during the afternoon peak hour, as there were 42 veh/h turning northbound right onto Scott Street during the afternoon peak hour.

Figure 5: Northbound Signage along Clifton Road





Existing Road Safety Conditions

Collision history for study area roads (2011 to 2015, inclusive) was obtained from the City of Ottawa, and most collisions (82%) involved only property damage, indicating low impact speeds, and 18% involved personal injuries. The primary causes of collisions cited by police include rear end (27%), sideswipe (27%), and angle (18%) type collisions.

A standard unit of measure for assessing collisions at an intersection is based on the number collisions per million entering vehicles (MEV). At the signalized Scott/Lanark & West Village intersection reported collisions have historically taken place at a rate of 0.27/MEV.

It is noteworthy that within the vicinity of the site, there was one collision involving two pedestrians at the Scott/Lanark intersection (in 2014) and one collision involving a cyclist at the Scott/Clifton intersection (in 2013). Both resulted in nonfatal injuries. The source collision data as provided by the City of Ottawa and related analysis is included as Appendix C.

2.1.3. PLANNED CONDITIONS

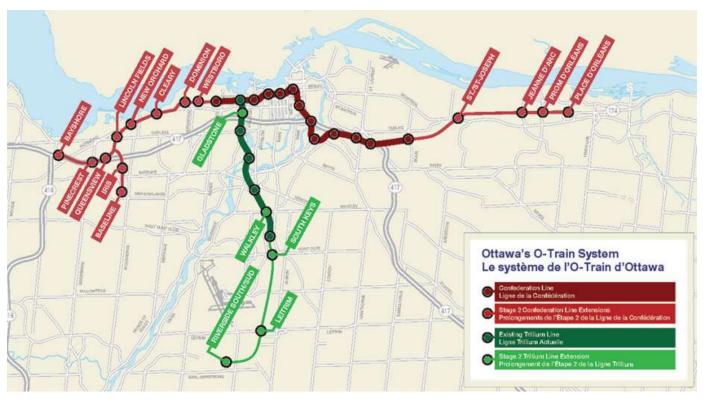
Planned Study Area Transportation Network Changes

Transit Projects

A notable transportation network change within the study area is the Phase I construction of the east-west LRT, which is the conversion of the City's existing BRT corridor to LRT between the current Blair transit station and the Tunney's Pasture station which includes a tunnel through the City's Downtown. Currently, this phase of construction is underway and is expected to be completed by the end of 2018.

Phase 2 of the LRT construction, which will extend the City's LRT further east, west and south (further improving transit within the vicinity of the site), is expected to begin by 2018 and be completed by 2023. The following Figure 6 illustrates the planned Phases 1 and 2 of the future Confederation/Trillium Lines. As mentioned previously, the subject development is located approximately 250 m from the Westboro station, which is part of Phase 2.

Figure 6: Planned LRT Phase 2



During the LRT Phase II construction, buses will be routed off the existing Transitway onto Scott Street. Approximately 210 additional busses in the morning peak and 225 additional busses in the afternoon peak are expected to be added to the existing vehicle traffic. The detour is expected to be in operation from late 2021 (October/November) to 2023.

Road Projects

As part of Stage 2 LRT, the City of Ottawa has prepared a "complete street" concept for Scott Street which is attached as Appendix D and the section directly adjacent to the site is shown as Figure 7. The proposed design will provide cycle tracks and sidewalks along both sides of Scott Street and provide fully-protected intersections at signalized intersections, including at the site's adjacent intersection (Scott/Lanark & West Village).

The planned modifications along Scott Street are expected to be constructed from May 2021 to October 2021 (some construction might begin in fall 2020). This plan will be in place for the detour of BRT buses along Scott Street during the Phase 2 LRT construction. The interim cross-section of Scott Street will include two vehicle travel lanes (curb lane is transit only) in the eastbound direction between Island Park Drive and Clifton Road. Post LRT construction (2024), it is understood that the cross-section of Scott Street will be revised to be a single vehicle travel lane in each direction.

SITE SITE

Figure 7: Planned Scott Street Interim "Complete Street" Design

Other Area Development

With respect to other area development, the following development applications have been submitted to the City of Ottawa in the vicinity of the proposed site:

1960 Scott Street

Colonnade Bridgeport is proposing the construction of a mixed-use development consisting of approximately 120 residential dwelling units, 74,153 ft² of office and 14,440 ft² of retail located at the above address. The Transportation Brief (prepared by Parsons) projected 123 veh/h and 132 veh/h in the morning and afternoon peak hours, respectively.

1946 Scott Street

Surface Developments is proposing the construction of a 9-storey residential building consisting of 60 units and 9 parking spaces. The Transportation Impact Assessment (prepared by Parsons) projected minimal vehicle impact on the transportation network. This development is currently being appealed.

320 McRae Avenue, 1976 Scott Street, and 315 Tweedsmuir Avenue

A mixed-use development consisting of approximately 242 residential dwelling units, 11,200 ft² of office space, and 23,000 ft² of retail land uses is proposed at the abovementioned address. The Transportation Study (prepared by Parsons) projected 104 and 142 veh/h in the morning and afternoon peak hours, respectively.

Tunney's Pasture

Tunney's Pasture is located approximately 1 km east of the subject site and currently consists of approximately 10,000 office/lab employees. A Master Plan for Tunney's Pasture proposes the site to be redeveloped to consist of approximately 24,000 office, lab, and retail employees and approximately 3,700 high density residential dwelling units. The Multi-Modal Transportation Study (prepared by Parsons) projected net increase in vehicle traffic of 1,135 and 1,400 veh/h during the morning and afternoon peak hours, respectively. Intersection modifications are recommended at locations that are

projected to experience capacity issues and new signalized intersections are proposed for access/egress to/from the development.

The emphasis in the City's recent Official Plan and Transportation Master Plan is to place priority on transit, encourage intensification around transit stations, encourage mixed-use developments and provide "complete streets" that better accommodate the active transportation needs of its residents and reduce the use of the private auto. The Tunney's Pasture Master Plan is reflective of these realities and is an excellent example of a sustainable plan from a transportation perspective.

2.2. STUDY AREA AND TIME PERIODS

2.2.1. STUDY AREA

The proposed study area is outlined below and highlighted in Figure 8.

- Scott/Lanark intersection;
- Scott/Clifton intersection;
- Clifton Road adjacent to the site; and
- Scott Street adjacent to the site;



Figure 8: Study Area

2.2.2. TIME PERIODS

As the proposed land use is residential, the time periods to be assessed are the weekday morning and afternoon commuter peak hours.

2.2.3. HORIZON YEARS

The expected build out date for the proposed development is assumed to be 2020. Depending on the growth rate of the study area, the horizon year 2025 will be assessed for 5-years beyond site build out.

2.3. EXEMPTION REVIEW

Based on the City's TIA guidelines and the subject site, the following sections of the TIA process will be exempt, unless otherwise directed.

Module	Element	Exemption Consideration				
4.1 Development	4.1.3 New Street	Not required for applications involving site plans.				
Design	Networks	Troc rodanos for apphositorio informig ofto pismo.				
4.2 Parking	4.2.2 Spillover Parking	The site's residential rate meets the City's By-Law requirements and visitor parking rate is deficient by only 3 spaces. Given the site's location close to transit and active mode facilities, and the option to pay for parking at adjacent sites, ie 320 McRae, the amount of visitor parking is likely sufficient.				
4.8 Review of Network Concept	All elements	This development is not expected to generate 200 person trips more than the permitted zoning for the site.				

3. FORECASTING REPORT

3.1. DEVELOPMENT-GENERATED TRAVEL DEMAND

3.1.1. TRIP GENERATION AND MODE SHARES

Appropriate trip generation rates for the proposed development consisting of approximately 141 high-rise condominiums were obtained from the City's 2009 TRANS Trip Generation – Residential Trip Rates Report. The developer is considering either rental or condominium units. Given condominium unit trip-generation rates are higher than apartment trip-generation rates, for the purposes of this report, we have used the condominium rates, as this represents a more conservative approach. These rates are summarized in Table 1.

Table 1: 2009 TRANS Residential Trip Generation Rates

Land Hoo	ITE Land Use	Trip F	Rates
Land Use	Code	AM Peak	PM Peak
High-Rise Condominiums	ITE 232	T = 0.38(du)	T = 0.34(du)
Notes: T = Average Vehicle Trip Endu = Dwelling units	nds		

Using the TRANS Trip Generation rates, the total amount of vehicle trips generated by the proposed 141 residential units was calculated. The results are summarized in Table 2.

Table 2: Projected Site Vehicle Trip Generation - TRANS Model

Land Use	Aroo	AM Peak (Veh/h)			PM Peak (Veh/h)		
Land Ose	Area	In	Out	Total	In	Out	Total
High-Rise Condominiums	141 units	15	39	54	27	21	48

As shown in Table 2, a total of 54 and 48 veh/h are projected to travel to/from the proposed development during the weekday morning and afternoon commuter peak hours. Using the TRANS Auto Trips projected in Table 2 and the mode share percentages in the TRANS Trip Generation Report (Table 3.13), the total projected number of person trips by mode for the residential development are summarized in Table 3.

Table 3: Projected Site Person Trip Generation

Travel Mode	Mode	AM Pe	ak (Person T	rips/h)	Mode	PM Pe	ak (Person Trips/h)		
Travor Modo	Share	In	Out	Total	Share	In	Out	Total	
Auto Driver	37%	15	39	54	40%	27	21	48	
Auto Passenger	8%	3	8	11	9%	6	5	11	
Transit	41%	16	44	60	37%	26	18	44	
Non-motorized	14%	5	16	21	14%	10	7	17	
Total Person Trips	100%	39	107	146	100%	69	51	120	

As shown in Table 3, based on the TRANS Trip Generation method, the proposed site is projected to generate approximately 120 to 146 person-trips per hour during the weekday commuter peak hours. The increase in two-way transit trips is estimated to be approximately 45 to 60 persons per hour, and the increase in bike/walk trips is approximately 15 to 20 persons per hour.

Existing Permitted Zoning

The proposed site is located within three different property parcels. The 1950 Scott parcel, adjacent to Scott Street, is zoned Residential Fifth Density (maximum height 18 m) and the 312 Clifton Road and 314 Clifton Road parcels, that form the southern part of the site, are zoned Residential Third Density. Based on the existing zoning, the estimated number of people trips was calculated assuming approximately 50 residential units and similar trip generation rates as outlined above. The resulting number of people trips is estimated to be 45 to 50 persons/h during the peak hour with approximately 15 to 20 of those trips being vehicle trips.

As shown, based on the proposed Site Plan, there are projected to be approximately 75 to 95 more person trips per hour during peak hours than the currently zoned land uses. However, as the City invests in improving transit and active mode transportation there is a desire to increase density in areas that are located close to these planned facilities. As the site is located within 250 m from the future Westboro LRT Station, it is expected there will be a high transit mode split for the proposed development as outlined below. The increased number of people trips from these lands is expected to be able to be accommodated given completion of Stage 2 LRT and the active mode improvements along Scott Street.

Mode Shares

The existing mode shares outlined in Table 3 were obtained from the TRANS Trip Generation Report. In comparison, the 2011 OD Survey mode shares for the Ottawa Inner Area are summarized below.

Table 4: OD Survey Trips by Primary Travel Mode – Ottawa Inner Area

Time Period	24 Hours			AM Peak Hour			Pi	Average		
Mode	From District	To District	Within District	From District	To District	Within District	From District	To District	Within District	Average
Driver	44%	44%	22%	40%	41%	20%	45%	43%	21%	36%
Passenger	12%	12%	8%	7%	9%	9%	11%	11%	8%	10%
Transit	28%	28%	10%	25%	41%	13%	33%	22%	10%	23%
Bike/Walk	13%	14%	58%	25%	7%	52%	10%	22%	60%	29%
Other	2%	3%	2%	4%	2%	6%	2%	2%	2%	3%

These existing OD Survey modal shares are similar to the ones used in Table 3 to calculate the projected person trip generation for the build-out year of the proposed development. It is noteworthy that the OD Survey records a higher number of bike/walk trips in the subject area by approximately 15% and a lower number of transit trips. However, given the location of the site, within close proximity to the Westboro Transit Station (future LRT stations), a higher transit modal share is appropriate.

For the Horizon Year 2025, which represents five-years beyond full-build out, the following future mode share are forecasted. These mode shares reflect the construction of Phase 2 LRT and site's location within close proximity to the future Westboro LRT Station.

Table 5: Future Mode Share Targets for the Development

Travel Mode	Mode Share Target	Rationale
Transit	65%	Development is located within 600 m of a future LRT station, making it a Transit-Oriented Development (TOD) which have transit targets of 65%.
Walking	10%	This is consistent with the City's TMP, TOD areas and the existing TRANS tripgeneration report.
Biking	5%	This is consistent with the City's TMP, TOD areas and the existing TRANS tripgeneration report.
Auto Passenger	5%	This is consistent with TOD targets.
Auto Driver	15%	This is consistent with TOD targets.

Based on the future mode share targets for this development, the project site-generated person trips are outlined in Table 6.

Table 6: Future Projected 2025 Site-Generated Person Trips

Tuesda Marda	Mode Share	AM Pe	ak (Person T	rips/h)	PM Pe	ak (Person Trips/h)	
Travel Mode	Widde Share	In	Out	Total	In	Out	Total
Auto Driver	15%	6	16	22	10	8	18
Auto Passenger	5%	2	5	7	3	3	6
Transit	65%	26	69	95	45	33	78
Non-motorized	15%	6	16	22	10	8	18
Total Person Trips	100%	40	106	146	68	52	120

Given the low forecasted number of vehicle trips for the Horizon Year 2025 of approximately 20 new veh/h during the peak hours, no further vehicle analysis is included for this Horizon Year 2025 with respect to the site-generated traffic volumes.

3.1.2. TRIP DISTRIBUTION

Based on the existing traffic volume counts and the location of adjacent arterial roadways and neighbourhoods, the distribution of site-generated traffic volumes is as follows:

• 50% to/from the east;

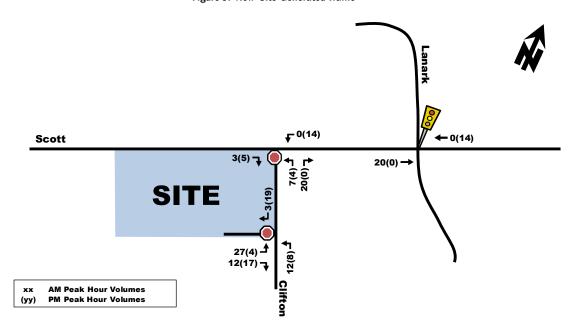
• 30% to/from the south; and

• 20% to/from the west.

3.1.3. TRIP ASSIGNMENT

A full movement driveway connection to Clifton Road is proposed to serve the subject development. This driveway is proposed at the southern boundary of the site, approximately 50 m south of Scott Street. During the morning peak hour there is a westbound left-turn restriction at the Scott/Clifton intersection and during the afternoon peak hour, there is a northbound right-turn restriction. Given the proposed driveway and existing turn restrictions, 'new' site-generated vehicle trips are assigned to the study area network and illustrated as Figure 9.

Figure 9: 'New' Site-Generated Traffic



It is noteworthy that the existing turn restrictions are understood to be in place to help prevent cut-through traffic through the neighbourhood. Based on the existing count data at the Clifton/Scott intersection, there are a number of drivers that do not comply with these existing turn restrictions. Some site-generated traffic originating/destined from/to the east will be required to travel along the southern portion on Clifton Road during the peak hours to comply with the existing turn restrictions. This is represented in Figure 9.

3.2. BACKGROUND NETWORK TRAVEL DEMANDS

3.2.1. TRANSPORTATION NETWORK PLANS

Refer to section 2.1.3 Planned Conditions - Planned Study Area Transportation Network Changes.

3.2.2. BACKGROUND GROWTH

Background traffic growth for the area is expected to grow based on significant planned area developments. However, given Stage 2 LRT construction, the City is expecting to see negative vehicle growth along Scott Street in the future (see map attached as Appendix E). As such, for background traffic projections, the projected vehicle volumes from the planned area developments (1960 Scott Street and 320 McRae) were layered onto the existing traffic volumes for the build out year 2020. As the City expects to see a significant increase in transit modes once Stage 2 LRT is constructed in this area (2023) and a decline in traffic volumes, and as there is likely to be continued development growth in the area, the vehicle traffic volumes for horizon year 2025 is assumed to be the same as year 2020.

Figure 10: Background Traffic Volumes ←0(1) Γ68(55) **1** 50(35) 472(641) **←** 368(591) **_** 26(32) 0(13) Scott 39(90) 627(623) -> 30(42) 10(27) 9(13) **579(513)** → 9(20) Clifton **AM Peak Hour Volumes** ХX

3.2.3. OTHER DEVELOPMENTS

(yy)

Refer to section 2.1.3 Planned Conditions - Other Area Developments.

3.3. DEMAND RATIONALIZATION

PM Peak Hour Volumes

Given the site's location within close proximity of Phase 2 LRT and the City's initiatives to increase density within close proximity to the LRT stations, the future travel within the study area is expected to shift modes as transit and active mode infrastructure is constructed. The future traffic volumes along Scott Street are projected to continue to increase as density in the neighbourhood increases, however higher transit modes area also expected and in the fullness of t. Given the proposed site is expected to generate less than one new vehicle per minute (less than 60 veh/h) at build-out and approximately 20 veh/h (1 new vehicle every 3 minutes) by 2025, the vehicle impact generated by the site is considered minimal.

4. STRATEGY REPORT

4.1. DEVELOPMENT DESIGN

4.1.1. DESIGN FOR SUSTAINABLE MODES

Vehicle and Bicycle Parking

Vehicle parking is proposed in an underground parking garage for residential and visitor use. A total of 148 parking spaces are proposed for residents and 13 visitor parking spaces are proposed. With regard to bicycle parking, it is located within the underground parking structure and a total of 158 bicycle parking spaces are proposed. In addition, there are 144 storage units planned within the underground structure.

Transit and Pedestrians

Transit service within the vicinity of the site is currently provided by OC Transpo Regular Route #50, which provides frequent all-day service. Bus stops for this route are located along Scott Street approximately 40 m from the site. Rapid transit service (in the form of BRT) is also provided via the Westboro Station, located approximately 250 metres northwest of the proposed development, which provides convenient access to multiple routes along the Transitway. Phase 2 of the LRT project will divert transit onto Scott Street and the Westboro Transit Station will become the Westboro LRT Station once Stage 2 is complete by approximately 2023.

Sidewalk facilities within the vicinity of the site are provided along both sides of Lanark Avenue, along the west side of Clifton Road (adjacent to the site), and along the south side of Scott Street, with an off-road MUP provided along the north side of Scott Street. Pedestrian pathways are provided connecting the building entrances/exits to the public sidewalks along Scott Street and Clifton Road.

4.1.2. CIRCULATION AND ACCESS

The full-movement driveway to Clifton Road is proposed to the underground parking garage. The width of the driveway is noted to be 6 m, and the drive aisles within the parking garage are also noted to be 6 m wide. These widths meet the City's By-Law requirements and are sufficient for the circulation of two-way traffic.

It is noteworthy that there are 6 tandem parking spaces located in the underground parking garage. We are advised by the developer that these tandem parking spaces are planned to be sold to residents of the same unit so drivers can arrange to access their vehicles.

Garbage is located adjacent to the proposed vehicle driveway along Clifton Road and it is understood that loading is planned to take place on Clifton Road.

4.2. PARKING

4.2.1. PARKING SUPPLY

Vehicle Parking

A total of 148 underground parking spaces are proposed to serve the residents of the proposed development and 13 visitor parking spaces are proposed underground. This amount of residential parking meets the City's minimum By-Law requirements for 141 units within Area Y, identified on the City's Schedule 1A. The number of visitor parking spaces required by By-Law is calculated to be 13 spaces. The total amount of residential and visitor parking does not exceed the City's maximum number of parking spaces for a development of this size within close proximity to rapid transit.

The majority of parking spaces (161 spaces) are noted to be 5.2 m in length and 2.6 m in width. There are seven (7) small parking spaces which are noted to be 4.6 m in length and 2.4 m in width. These parking space dimensions meet the City's By-Law requirements. In addition, there are six (6) tandem parking spaces, which are planned to be sold to tenants of the same unit that have more than one vehicle.

Bicycle Parking

A total of 158 bicycle parking spaces are proposed in the underground parking lot to serve the subject residential development. This amount of bicycle parking meets the City's minimum requirement with respect to the By-Law requirements. In addition, there are 144 storage lockers planned in the underground parking lot, which may also serve as bicycle parking for owners/tenants.

4.3. BOUNDARY STREET DESIGN

The boundary streets of the proposed development are Scott Street and Clifton Road. The City of Ottawa has prepared a "complete street" concept for Scott Street which is attached as Appendix D and the section directly adjacent to the site is shown as Figure 11. The subject development is not expected to have any significant impact on the future design as no driveway is proposed to Scott Street.

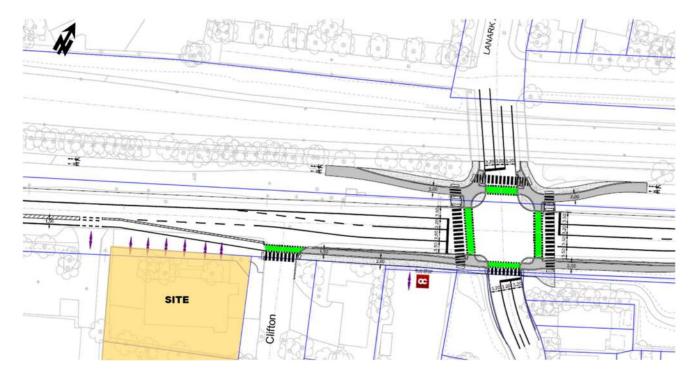


Figure 11: Planned Scott Street Interim "Complete Street" Design

The proposed design will provide cycle tracks and sidewalks along both sides of Scott Street and provide fully-protected intersections at signalized intersections, including at the site's adjacent intersection (Scott/Lanark & West Village). Transit access is provided by bus stops located adjacent to the Scott/Lanark intersection.

The planned modifications along Scott Street are expected to be constructed from May 2021 to October 2021 (some construction might begin in fall 2020). This plan will be in place for the detour of BRT busses along Scott Street during the Stage 2 LRT construction. The resulting cross-section of Scott Street will include two vehicle travel lanes (curb lane is transit only) in the eastbound direction between Island Park Drive and Clifton Road. Post construction (2024), it is understood that the cross-section of Scott Street will be revised to be a single vehicle travel lane in each direction.

4.4. ACCESS INTERSECTION DESIGN

4.4.1. LOCATION AND DESIGN OF ACCESS

The proposed access is located approximately 50 m south of Scott Street along Clifton Road. The driveway is off-set as far as possible from Scott Street, and is approximately 0.3 m from the southern property line of the site. The driveway is replacing three existing driveways and is proposed to be full-movement with a width of 6 m. The location and width of the driveway meet the City's Private Approach By-Law.

South of the proposed access there are single family home driveways, the closest being approximately 8 m south of the subject driveway. Similarly, along the east side of Clifton Road, there are multiple single-family home driveways. Given the land use type and size of the adjacent driveways, vehicle conflicts between driveways are expected to be minimal.

With respect to the ramp providing access to the underground parking garage, the ramp grade is noted to be 2% from the edge of the sidewalk to 9 m down the ramp, where it increases to 10% then further to 16%. This 2% ramp grade follows the City's guidelines to provide sufficient space for a vehicle to be at grade as the vehicle approaches the sidewalk, to ensure proper sight lines for drivers to see pedestrians and cyclists on the public sidewalks and roads. At the bottom of the ramp, appropriate transition grades should be provided to continue into the parking garage, where a 3-4% grade is provided throughout the parking garage.

4.4.2. INTERSECTION CONTROL

Based on the projected number of vehicles traveling to/from the site's driveway, STOP control on the minor approach (site driveway) only is recommended. No further traffic control or turn lanes are warranted at this site driveway.

4.5. TRANSPORTATION DEMAND MANAGEMENT

The proposed residential development is located within walking distance to transit stops located along Scott Street and the Westboro Rapid Transit Station, located within 250 m walking distance. As such, the development is expected to attract significant transit ridership, particularly post Stage 2 LRT construction (approximately 2023). Sidewalks are currently provided along adjacent City roadways and a pedestrian crossing signal is provided along Scott Street, adjacent to the Westboro Transit Station. Cycle lanes and cross-rides are proposed along Scott Street as part of the City's future works related to Stage 2 LRT construction. As such, the location of the site is ideal in promoting non-auto travel during the weekday peak hours and outside peak hours. The Transportation Demand Management (TDM) checklist is provided as Appendix F and highlighted below:

- Provide pedestrian connections to existing City sidewalks;
- Provide secure underground bicycle parking;
- Number of bicycle parking spaces exceed City's minimum requirements according to the By-Law; and
- Number of vehicle parking spaces does not exceed the City's By-Law maximum.

Given the type of development and its location adjacent to rapid transit, within the urban inner area, and given the existing and future cycling and pedestrian facilities within the area, the development is well positioned to promote travel via transit and active modes.

4.6. NEIGHBOURHOOD TRAFFIC MANAGEMENT

Through previous consultation with residents in the neighbourhood, we understand that there is an existing cut-through traffic issue along Clifton Road. It is understood that vehicles travel via Clifton Road, Wilber Avenue and Kirkwood Avenue to access Richmond Road and Scott Street. It is our understanding that the City is aware of this existing cut-through traffic issue and efforts have been made to minimize the amount of traffic on these local roadways. The following mitigative measures are currently installed along Clifton Road:

- Peak-hour turn restrictions:
- Multiple curb bulb-outs; and
- Speed humps.

Given the site's location adjacent to Scott Street, the traffic distribution shows the majority of site-generated traffic is projected to travel to/from Scott Street. A portion of the site-generated traffic is projected to travel to/from the south via Clifton Road. The increase of vehicle traffic along Clifton Road, south of the site driveway is projected to be approximately 25 veh/h during the weekday morning and afternoon peak hours. This amount of traffic equates to one new vehicle every

2 minutes. The traffic travelling to/from the proposed development along Clifton Road is considered local residential traffic and not cut-through traffic.

The City has implemented significant traffic management measures along Clifton Road to prevent cut-through traffic of vehicles travelling between Scott Street and Richmond Road. As the increase in local residential traffic along Clifton Road is projected to be low (one new vehicle every 2 minutes), no further mitigative measures are recommended. However, it is recommended that the City repair the signage along Clifton Road and additional signage along the north side of Scott Street be installed to reinforce the westbound left-turn restriction, as there are a significant number of violations of these turning movements, as noted in Section 2.1.2.

4.7. TRANSIT

The Westboro Rapid Transit Station is located within 250 m walking distance from the proposed site. Stage 2 LRT is planned to be completed by 2023 with the existing BRT station being converted into an LRT station. As shown in Section 3.1, the two-way transit people trips generated by of the proposed development at build-out year is approximately 45 to 60 persons/h during the weekday morning and afternoon peak hours. At five years beyond full-build out, this number of site-generated persons trips is expected to increase to 80 – 95 persons per hour during the peak hours. This additional amount of transit trips is expected to be able to be accommodated by the future LRT line.

4.8. REVIEW OF NETWORK CONCEPT

Exempt - See Section 2.3.

4.9. INTERSECTION DESIGN

4.9.1. EXISTING CONDITIONS

The following Table 7 provides a summary of the existing traffic operations at the study area intersections based on the SYNCHRO (V9) traffic analysis software and the existing traffic volumes (Figure 4). The subject signalized intersection was assessed in terms of the volume-to-capacity (v/c) ratio and the corresponding Level of Service (LoS) for the critical movement(s). The subject signalized intersection 'as a whole' was assessed based on weighted v/c ratio. The SYNCHRO model output of existing conditions is provided within Appendix G.

	Weekday AM Peak (PM Peak)								
Intersection		Critical Movem	nent	Intersection 'as a whole'					
	LoS	max. v/c or avg. delay (s)	Movement	Delay (s)	LoS	v/c			
Scott/Lanark	A(A)	0.39(0.41)	SBL(WBT)	6.4(6.0)	A(A)	0.37(0.37)			
Scott/Clifton (unsignalized)	C(C)	C(C) 15.3(17.8) NB(NB) 0.9(1.2) A(A)							
Note: Analysis of signalized intersections	assumes a F	PHF of 0.95 and a satu	uration flow rate of 1	.800 veh/h/lane.					

Table 7: Existing Intersection Performance

As shown in Table 7, the study area intersections 'as a whole' are currently operate at an excellent LoS 'A' during the morning and afternoon peak hours. With regard to 'critical movements' at study area intersections, they are operating at an acceptable LoS 'C' or better during peak hours with regard to City of Ottawa operating standards.

As shown in Figure 4, existing traffic volumes, the westbound left-turn and northbound right-turn restrictions are often violated. To prevent the continuation of the westbound left-turn at the Scott/Clifton intersection, it is recommended that additional signage be installed to ensure that the violation is visible to motorists. It was observed during the June 2018 site visit by Parsons' staff that the turn restriction sign on the north side of Scott Street (visible on Google Street View) has

been removed. This leaves one turn restriction sign on the south side of the road which is not as visible to drivers approaching the unsignalized intersection. The northbound right-turn sign is bent (as outlined in Section 2.1.2) and should be repaired to be visible to drivers.

Multi-Modal Level of Service - Existing Conditions

The MMLoS analysis for the Scott/Lanark signalized study area intersection is summarized in Table 8. The existing detailed MMLoS analysis is provided as Appendix H.

		Level of Service									
Intersection		Pedestrian (PLoS)		(BLoS)	Transit	(TLoS)	Truck (Truck (TkLoS) Vehicle (Lo			
	PLoS	Target	BLoS	Target	TLoS	Target	TkLoS	Target	LoS	Target	
Scott/Lanark	Е	Α	С	Α	В	No target	Е	No target	Α	E	

Table 8: MMLoS - Signalized Scott/Lanark Intersection, Existing Conditions

Given the development's location within close proximity to existing and future rapid transit and its location adjacent to a cross-town bikeway, the target levels of service for pedestrians and cyclists are high ('A'). As the BRT and future LRT are located adjacent to Scott Street and there are no transit priority measures along Scott Street (given the BRT/LRT are adjacent), there is no target for transit level of service. Scott Street is designated a truck route, however, Lanark and West Village are not part of the truck route, as such trucks are not required to turn at this intersection as part of the City's truck route, therefore there is no target level of service for trucks at this intersection. The vehicle level of service is met given the existing v/c ratio and delays.

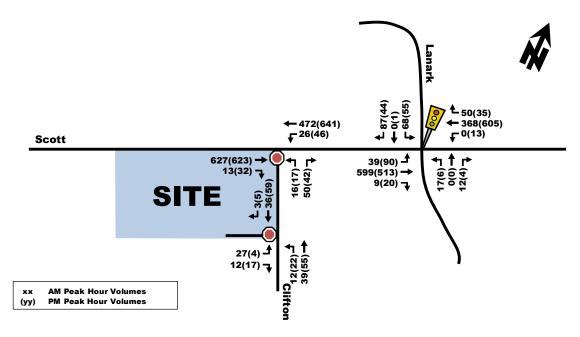
With regard to pedestrians, the PETSI (Pedestrian Exposure to Traffic at Signalized Intersection) scores are PLoS 'C' for all four legs. The delay score is PLoS 'A' for pedestrians crossing the north and south legs, however, pedestrians experience an average of 40 seconds of delay crossing the east and west legs, which results in a PLoS 'E'. Minimal changes to the signalized intersection are proposed to improve the PLoS score for the interim design (2021). Providing a zebra-stripe crosswalk type (proposed for the interim), an advanced pedestrian walk phase and increasing the effective walk time for pedestrians would improve the level of service. This crosswalk treatment has been included in the interim Scott Street design detailed in Section 4.3. Changing the existing signal timing can be done by the City Traffic Signals group if the City desires to improve the PLoS at this intersection, however, it is noteworthy that PLoS 'A' is not achievable given the MMLoS guidelines and the effective walk time calculation.

With regard to cyclists, there are currently no dedicated cycling facilities along Clifton Road and no protected left-turn for cyclists along Scott Street, resulting in a BLoS 'C'. Providing 2-stage left-turn crossing along all four legs of the intersection will improve the level of service to BLoS 'A', meeting the target for this intersection.

4.9.2. TOTAL PROJECTED 2020 CONDITIONS - FULL BUILD-OUT

The total projected 2020 traffic volumes were derived by superimposing the site-generated traffic volumes (Figure 9) onto background traffic volumes (Figure 10). The resulting total projected 2020 traffic volumes are illustrated in Figure 12.

Figure 12: Total Projected 2020 Traffic Volumes



The following Table 9 provides a summary of the total projected operations at the study area intersection based on the SYNCHRO (V10) traffic analysis software. The SYNCHRO model output of 2020 projected conditions is provided within Appendix G.

Table 9: Total Projected 2020 Performance at Study Area Intersections

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	nent	Intersection 'as a whole'					
	LoS	max. v/c or avg. delay (s)	Movement	Delay (s)	LoS	v/c			
Scott/Lanark	A(A)	0.45(0.47)	EBT(WBT)	7.6(6.6)	A(A)	0.44(0.42)			
Scott/Clifton (unsignalized)	C(C)	18.5(23.7)	NBL(NBL)	1.4(1.7)	A(A)	-			
Clifton/Site Access (unsignalized)	A(A)	9.1(8.8)	EBL(EBL)	3.5(1.5)	A(A)	-			
Note: Analysis of signalized intersections as	sumes a Ph	HF of 0.95 and a satu	uration flow rate of 1	.800 veh/h/lane.					

The Scott/Lanark and Scott/Clifton intersections are projected to operate similar to the existing conditions, with slight increases in v/c and delays due to background traffic.

The Clifton/Site Access intersection is projected to operate 'as a whole' with an excellent LoS 'A'. Critical movements at this intersection are also projected to operate at LoS 'A'.

Multi-Modal Level of Service - Projected Build-Out Conditions

For the purpose of this report, the proposed interim changes for Scott Street mentioned in Section 4.3 will be assessed for the build-out year. The following Table 10 outlines the projected multi-model level of service for the Scott/Lanark intersection and the MMLoS analysis is provided as Appendix H.

Table 10: MMLOS - Signalized Scott/Lanark Intersection, Projected 2021 Conditions

Intersection		Level of Service											
	Pedestrian (PLoS)		Bicycle (BLoS)		Transit (TLoS)		Truck (TkLoS)		Vehicle (LoS)				
	PLoS	Target	BLoS	Target	TLoS	Target	TkLoS	Target	LoS	Target			
Scott/Lanark	Е	А	А	А	В	No target	E	No target	А	E			

As shown in Table 10, the bicycle level of service meets the target BLoS 'A' with the proposed cross rides in the interim design. Similar to existing conditions, the vehicle level of service target is also achieved and there are no targets for trucks or transit at this intersection.

With regard to pedestrians, the target is not met due to the low effective walk time for pedestrians crossing the north and south legs. Signal timing can be revised at the time of the reconstruction to improve walking conditions for pedestrians, if desired, however this will reduce the amount of green time for east-west vehicle traffic. It is noteworthy that there is an existing pedestrian crossing signal location west of the Scott/Lanark intersection. Pedestrians walking from the proposed site to the Westboro Transit Station are likely to use the pedestrian signal to cross Scott Street given the shorter walking distance.

4.9.3. TOTAL PROJECTED 2025 CONDITIONS - FULL BUILD-OUT + 5 YEARS

The total projected 2025 traffic volumes are expected to be less than the 2020 total projected volumes as the vehicle site trip generation is lower than the projected site trip-generated for build out year 2020 and the background traffic is expected to experience negative growth as outlined in Section 3.2.2. As such, the vehicle level of service is expected to be similar or better at horizon year 2025 than the projected 2020 results.

Multi-Modal Level of Service - 2025 Projected Conditions

For the 2025 horizon year, it is assumed that the cross-section will be revised to two travel lanes with auxiliary turn lanes and the cross-rides and zebra stripe crosswalks implemented for the interim design will be maintained. The following Table 11 outlines the multi-model level of service for the Scott/Lanark intersection. The projected 2025 MMLoS analysis is provided as Appendix H.

Table 11: MMLOS - Signalized Scott/Lanark Intersection

	Level of Service												
Intersection	Intersection Pedestrian (PLoS)		Bicycle (BLoS)		Transit (TLoS)		Truck (TkLoS)	Vehicle (LoS)				
	PLoS	Target	BLoS	Target	TLoS	Target	TkLoS	Target	LoS	Target			
Scott/Lanark	Е	А	Α	А	В	No Target	F	No Target	А	E			

As shown in Table 11, the final Scott Street design provides the same level of service for each mode compared to the levels of service achieved as the interim design for Scott Street.

5. FINDINGS, CONCLUSIONS AND RECOMMENDATIONS

Based on the analysis herein, the following conclusions are provided:

Proposed Site

- The development will include 141 condominium/apartment units with 159 proposed underground residential parking spaces and 10 visitor parking spaces;
- Bicycle parking is proposed within the underground parking structure and a total of 79 bicycle parking spaces are proposed. In addition, there are 141 proposed lockers proposed in the underground parking garage as well;
- The proposed development will consist of one phase, with the build-out year assumed to be 2020;
- The proposed development is projected to generate 'new' two-way vehicle volumes of approximately 48 and 54 during the weekday morning and afternoon peak hours at build-out year;
- The development is located approximately 250 m from the Westboro Transit Station, which is planned to be a LRT Station with the completion of Stage 2 LRT (approximately 2023). As such, site-generated vehicle volumes are expected to decrease as transit ridership increases with the implementation of Stage 2 LRT; and
- Vehicle access to the development is proposed via a new full-movement driveway to Clifton Road.

Background Conditions

- There are several area developments planned within close proximity of the site. The vehicle volumes associated with these other area developments was layered onto the existing traffic volumes to provide background traffic volumes;
- Overall, the signalized Scott/Lanark and unsignalized Scott/Clifton intersections are projected to operate adequately during the projected background horizons;
- The levels of service achieved by mode for existing conditions are: PLoS 'E', BLoS 'C', TLoS 'B' and TkLoS 'E'; and,
- As part of Stage 2 LRT, the interim cross-section of Scott Street will include two vehicle travel lanes (curb lane is transit only) in the eastbound direction between Island Park Drive and Clifton Road;

Projected Conditions

- The signalized Scott/Lanark intersection and unsignalized Scott/Clifton intersection at horizon year 2020, are projected to operate similar to existing conditions in terms of vehicle operation;
- The new site access on Clifton Road is projected to operate 'as a whole' with a LoS 'A' throughout the horizon years;
- The bicycle level of service achieved at the Scott/Lanark intersection for the interim and post LRT Stage 2 construction of Scott Street meets the target BLoS 'A';
- The pedestrian level of service 'A' is not achieved due to low effective walk time for pedestrians crossing north-south at the signalized Scott/Clifton intersection; and,
- The approximate 45 to 60 projected transit trips for the 2020 horizon year is expected to increase to 80 to 95 trips for the 2025 horizon year. This additional amount of transit trips is expected to be able to be accommodated by the future LRT line.

Site Plan

- Cycling facilities are provided on Scott Street in the form of on-street cycle lanes and a MUP on the north side of the street:
- Pedestrian facilities include pathways connecting the building entrances/exits to the public sidewalks along Scott Street and Clifton Road;
- The proposed residential development is in a desirable location to promote active and transit modes given the type
 of development, its location adjacent to rapid transit, and the existing and future cycling and pedestrian facilities
 within the area;

- The number of vehicle and bicycle parking spaces meets the City's minimum By-Law requirement for residents, however, the visitor vehicle parking is deficient by 3 spaces. Given the site's location adjacent to transit and active mode facilities, the amount of visitor parking is likely sufficient; and,
- Vehicle access is proposed to Clifton Road and is off set as much as possible (50 m) from Scott Street.

Based on the foregoing, the proposed development fits well into the context of the surrounding area, and its location and design serves to promote use of walking, cycling, and transit modes, thus supporting City of Ottawa policies, goals and objectives with respect to redevelopment, intensification and modal share.

Therefore, approval from a transportation perspective of the proposed 1950 Scott Street development is recommended.

Prepared By:

André Jane Sponder, P.Eng. Transportation Engineer

> 100512145 2018-07-12

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Reviewed by:

Ronald Jack, P.Eng.

Senior Transportation Engineer





City of Ottawa 2017 TIA Guidelines

TIA Screening Form

Date Project May-18

Project

1950 Scott Street 476658-01000

	Project Number	476658-01000
Results of Screening	Yes/No	
Development Satisfies the Trip Generation Trigger	Yes	
Development Satisfies the Location Trigger	Yes	
Development Satisfies the Safety Trigger	Yes	

Module 1.1 - Description of Proposed Development	
Municipal Address	1950 Scott Street
Description of location	Southwest quadrant of the Scott/Clifton intersection
Land Use	Residential
Development Size	141 Units
Number of Accesses and Locations	One full-movement vehicle access on Clifton Road
Development Phasing	None
Buildout Year	Assumed 2020
Sketch Plan / Site Plan	See attached

Module 1.2 - Trip Generation Trigger	
Land Use Type	Townhomes or Apartments
Development Size	141 Units
Trip Generation Trigger Met?	Yes

Module 1.3 - Location Triggers		
Development Proposes a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit, or Spine Bicycle Networks (See Sheet 3)	No	
Development is in a Design Priority Area (DPA) or Transit- oriented Development (TOD) zone. (See Sheet 3)	Yes	
Location Trigger Met?	Yes	

Module 1.4 - Safety Triggers			
Posted Speed Limit on any boundary road	<80	km/h	
Horizontal / Vertical Curvature on a boundary street limits sight lines at a proposed driveway	No		
A proposed driveway is within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions) or within auxiliary lanes of an intersection;	Yes		
A proposed driveway makes use of an existing median break that serves an existing site	No		
There is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development	No		
The development includes a drive-thru facility	No		
Safety Trigger Met?	Yes		







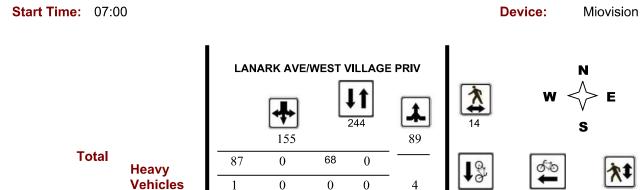
Transportation Services - Traffic Services

4 85 36807

WO No:

Turning Movement Count - Full Study Peak Hour Diagram

LANARK AVE/WEST VILLAGE PRIV @ SCOTT ST

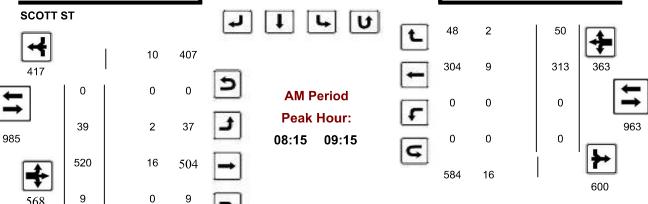


Vehicles Cars

86

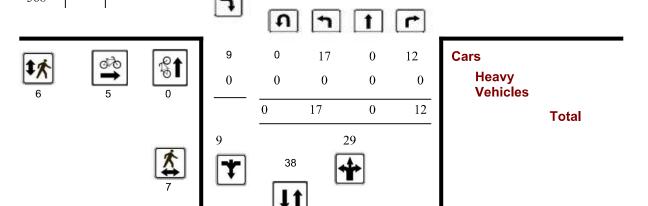
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Survey Date: Tuesday, March 28, 2017



68

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Comments

2017-Aug-16 Page 1 of 4



Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

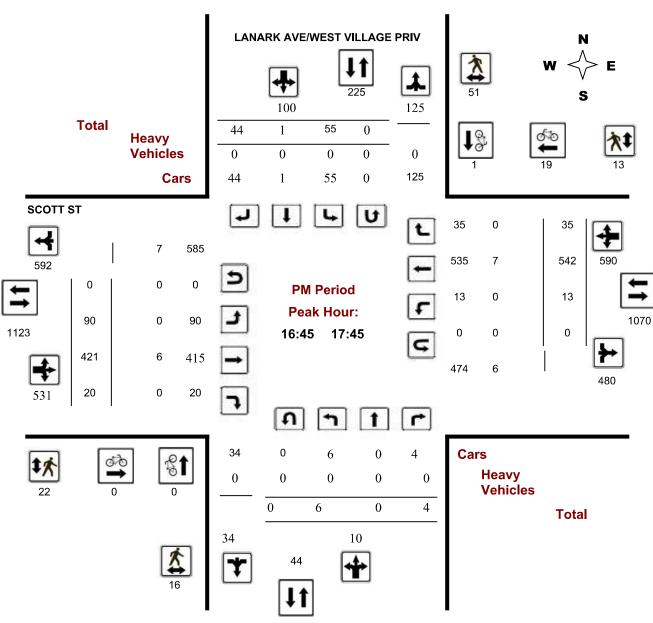
LANARK AVE/WEST VILLAGE PRIV @ SCOTT ST

Survey Date: Tuesday, March 28, 2017

Start Time: 07:00

WO No: 36807

Device: Miovision



Comments

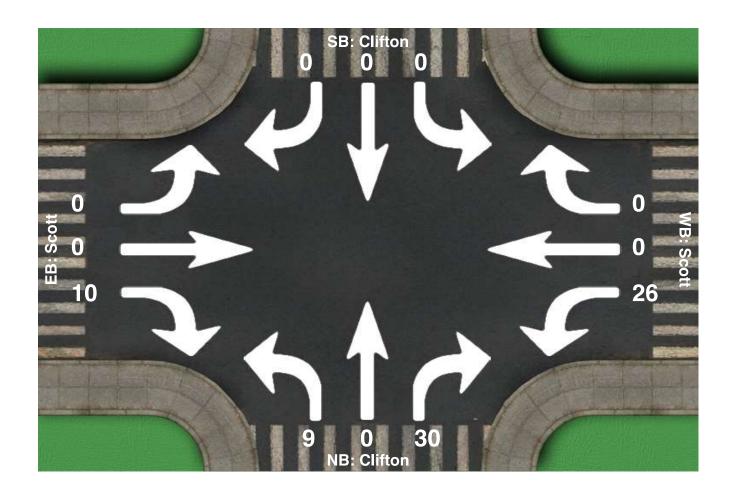
2017-Aug-16 Page 4 of 4

Intersection Peak Hour

Location: Clifton at Scott, Ottawa

GPS Coordinates:

Date: 2018-05-24
Day of week: Thursday
Weather: Sunny
Analyst: Rani Nahas



Intersection Peak Hour

08:00 - 09:00

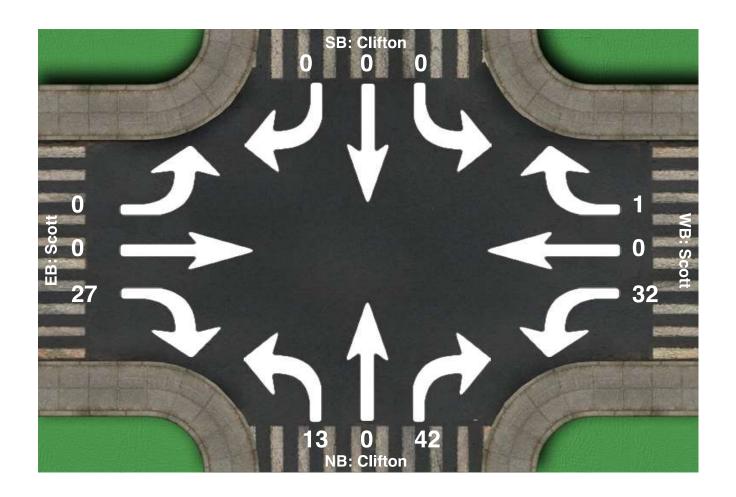
		SouthBound			Westbound			Northbound			Eastbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
Vehicle Total	0	0	0	26	0	0	9	0	30	0	0	10	75
Factor	0.00	0.00	0.00	0.72	0.00	0.00	0.56	0.00	0.54	0.00	0.00	0.50	0.78
Approach Factor		0.00			0.72			0.54			0.50		

Intersection Peak Hour

Location: Clifton at Scott, Ottawa

GPS Coordinates:

Date: 2018-05-24
Day of week: Thursday
Weather: Sunny
Analyst: Rani Nahas



Intersection Peak Hour

16:30 - 17:30

	SouthBound			Westbound			Northbound			Eastbound			Total
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	TOTAL
Vehicle Total	0	0	0	32	0	1	13	0	42	0	0	27	115
Factor	0.00	0.00	0.00	0.80	0.00	0.25	0.46	0.00	0.81	0.00	0.00	0.75	0.74
Approach Factor		0.00			0.82			0.69			0.75		



Total Area

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	3	1	1	1	1	2	0	0	9]
Non-fatal injury	0	0	0	1	0	1	0	0	2]
Non reportable	0	0	0	0	0	0	0	0	0	
Total	3	1	1	2	1	3	0	0	11] 1
	#1 or 27%	#4 or 9%	#4 or 9%	#3 or 18%	#4 or 9%	#1 or 27%	#7 or 0%	#7 or 0%		_

82% 18% 0% 100%

LANARK AVE/SCOTT ST

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2011-2015	7	14,039	1825	0.27

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	3	0	1	0	1	1	0	0	6
Non-fatal injury	0	0	0	0	0	1	0	0	1
Non reportable	0	0	0	0	0	0	0	0	0
Total	3	0	1	0	1	2	0	0	7
	43%	0%	14%	0%	14%	29%	0%	0%	-

86% 14% 0% 100%



City Operations - Transportation Services **Collision Details Report - Public Version**

From: January 1, 2014 **To:** January 1, 2016

Location: CLIFTON RD @ SCOTT ST

Traffic Control: Stop sign

Total Collisions: 1

Date/Day/Time Environment Impact Type Classification Surface Veh. Dir Vehicle Manoeuver Vehicle type First Event No. Ped 2014-Nov-24, Mon, 15:00 Clear Angle P.D. only Wet North Turning left Automobile, Other motor station wagon vehicle West Going ahead Automobile, Other motor station wagon vehicle											
Clear Angle P.D. only Wet North Turning left Automobile, station wagon West Going ahead Automobile, station wagon	Date/Day/Time		Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve		First Event	No. Ped	
Going ahead Automobile, station wagon	2014-Nov-24, Mon,15:00	Clear	Angle	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle		
						West	Going ahead) Š	Other motor vehicle		

Location: LANARK AVE/WEST VILLAGE PRIV @ SCOTT ST

Traffic Control: Traffic signal

Total Collisions: 3

Tramic Control: Hamic signal	ilic signal						וסומו כמ	otal collisions.	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Veh. Dir Vehicle Manoeuver Vehicle type		First Event	No. Ped
2014-Nov-06, Thu,15:03 Rain	Rain	Rear end	P.D. only	Wet	West	Going ahead Delivery van		Other motor vehicle	
					West	Turning left Pick-up truck		Other motor vehicle	
2014-Oct-17, Fri,09:46 Clear	Clear	SMV other	Non-fatal injury	Dry	South	Turning left	Turning left Pick-up truck Pedestrian	Pedestrian	2
2015-Feb-08, Sun,18:55 Clear	Clear	SMV other	P.D. only	Packed snow	East	Slowing or stopping Pick-up truck Ran off road	g Pick-up truck	Ran off road	

Thursday, August 17, 2017

Collision Main Detail Summary

OnTRAC Reporting System

CLIFTON RD & SCOTT ST
Former Municipality: Offawa Traffic Control:

FROM: 2011-01-01 TO: 2014-01-01

		_
	DATE	Former Municipality: Ottawa
D	DATE DAY TIME ENV LIGHT	
; -	LIGHT	I rattic Cor
	IMPACT TYPE	Traffic Control: Stop sign
)	CLASS	sign
	DIR	
ו	SURFACE COND'N	Num
	VEHICLE MANOEUVRE	Number of Collisions: 3
· ·	VEHICLE MANOEUVRE VEHICLE TYPE	
	FIRST EVENT	
,	No PE	

7	တ	Ŋ	4		Former Mu	LANARK		ယ	2		_	
2013-06-13 Thu 17:45 Clear	2013-01-07 Mo 13:39 Clear	2012-11-26 Mo 16:57 Clear	2011-08-05 Fri 15:45 Clear	DATE DAY	Former Municipality: Ottawa	LANARK AVE & SCOTT ST		2013-06-20 Thu 19:01 Clear	2012-06-08 Fri		2011-12-12 Mo 09:00 Clear	DATE DAY TIME
17:45 Clear	13:39 Clear	16:57 Clear	15:45 Clear	DAY TIME ENV				19:01 Clear	19:15 Clear		09:00 Clear	TIME ENV
Daylight Sideswipe	Daylight Rear end	Dusk Approaching	Daylight Rear end	IMPACT TYPE	Traffic Control: Traffic signal			Daylight Angle	Daylight Single vehicle		Daylight Turning	IMPACT TYPE
P.D. only V1 E V2 E	P.D. only V1 E V2 E	P.D. only V1 E V2 W	P.D. only V1 E V2 E	CLASS DIR	signal		√2 E	Non-fatal V1 N	P.D. only V1 N	√2 E	P.D. only V1 W	CLASS DIR
Dry	Slush Slush	Dy Dy	Dry Dry	SURFACE COND'N	Numb		Dry	Dry	Dry	Dry	Dry	SURFACE COND'N
Overtaking Stopped	Slowing or Slowing or	Going ahead Stopped	Going ahead Stopped	VEHICLE MANOEUVRE	Number of Collisions: 4		Going ahead	Turning right	Turning left	Going ahead	Turning left	VEHICLE MANOEUVRE
Unknown Municipal transit bus	Automobile, station Pick-up truck	Automobile, station Automobile, station	Automobile, station Pick-up truck	VEHICLE TYPE			Bicycle	Automobile, station	Pick-up truck	Automobile, station	Automobile, station	VEHICLE TYPE
Other motor vehicle Other motor vehicle	FIRST EVENT			Other motor vehicle	Cyclist	Fire hydrant	Other motor vehicle	Other motor vehicle	FIRST EVENT			
0	0	0	0	No. PED				0	0		0	No. PED

(Note: Time of Day = "00:00" represents unknown collision time Thursday, August 17, 2017

Collision Main Detail Summary

OnTRAC Reporting System

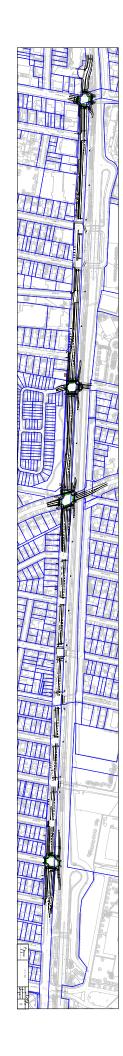
FROM: 2013-01-01 TO: 2014-01-01

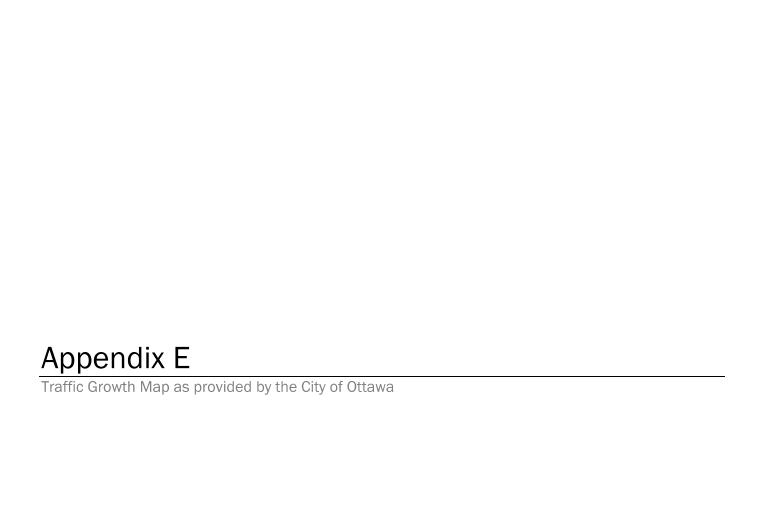
CLIFTON RD & SCOTT ST

Former Municipality: Ottawa Traffic Control: Stop sign Number of Collisions: 1

2 2013-01-07 Mo 13:39 Clear Daylight Rear end P.D. only V1 E Slush Slowing or Automobile, station Other motor vehicle V2 E Slush Slowing or Pick-up truck Other motor vehicle 3 2013-06-13 Thu 17:45 Clear Daylight Sideswipe P.D. only V1 E Dry Overtaking Unknown Other motor vehicle V2 E Dry Stopped Municipal transit bus Other motor vehicle	DATE DAY TIME ENV LIGHT TYPE CLASS DIR COND'N MANOEUVRE VEHICLE TYPE FIRST	Former Municipality: Ottawa Traffic Control: Traffic signal Number of Collisions: 2	1 2013-06-20 Thu 19:01 Clear Daylight Angle Non-fatal V1 N Dry Turning right Automobile, station Cyclist V2 E Dry Going ahead Bicycle Other n LANARK AVE & SCOTT ST	DATE DAY TIME ENV LIGHT TYPE CLASS DIR COND'N MANOEUVRE VEHICLE TYPE FIRST	Tottiet wutiteleality. Ottawa Traffic Control. Crop sign
or Automobile, station or Pick-up truck	VEHICLE MANOEUVRE VEHICLE TYPE	Number of Collisions: 2	Automobile, station Bicycle	VEHICLE MANOEUVRE VEHICLE TYPE	Natibel of Collisions.
Other motor vehicle Other motor vehicle Other motor vehicle	FIRST EVENT PED		Cyclist 0 Other motor vehicle	FIRST EVENT PED	





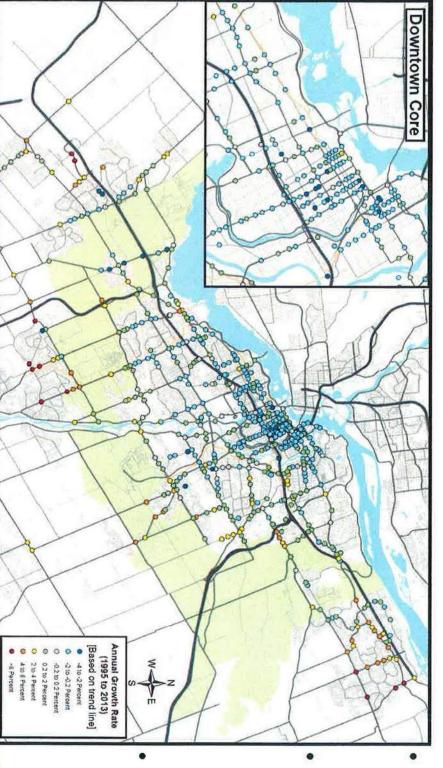




3.2 Background Traffic: Background Growth

INTERSECTION TRAFFIC GROWTH RATES, AM PEAK PERIOD (0700 to 0900)

Total Vehicular Volume Entering the Intersection, 1995 to 2013, Scenario F AM 2



 In some Growth areas, Growth been declining by location be justified traffic has rates vary rate must



TDM-Supportive Development Design and Infrastructure Checklist: Residential Developments (multi-family or condominium)

	Legend
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	Z
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	⊘
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	☑
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	□
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	Pedestrian walkways provided
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	☑
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	No on-site routes provided due to space constraints
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	□ N/A
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	N/A

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	☑
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	□2*
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	☑
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	Z
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	✓
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	□ N/A
BASIC	3.1.2	insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a	N/A
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	□ N/A

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	☑
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	N/A
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	_
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	



Existing AM 1: W Village/Lanark & Scott

	•	-	•		1	\	1	
Lane Group	EBL	EBT	WBT	NBL	NBT	SBL	SBT	
ane Configurations	7	T ₂	1	7	1	7	1	
raffic Volume (vph)	39	520	313	17	0	68	0	
uture Volume (vph)	39	520	313	17	0	68	0	
ane Group Flow (vph)	41	556	382	18	13	72	92	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	
Protected Phases		2	6		8		4	
Permitted Phases	2			8		4		
Detector Phase	2	2	6	8	8	4	4	
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	26.1	26.1	26.1	21.5	21.5	21.5	21.5	
Total Split (s)	73.0	73.0	73.0	22.0	22.0	22.0	22.0	
Total Split (%)	76.8%	76.8%	76.8%	23.2%	23.2%	23.2%	23.2%	
Yellow Time (s)	3.3	3,3	3.3	3.3	3.3	3,3	3,3	
All-Red Time (s)	2.8	2.8	2.8	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)	-2.1	-2.1	-2.1	-1.5	-1.5	-1.5	-1.5	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	77.9	77.9	77.9	13.0	13.0	13.0	13.0	
Actuated g/C Ratio	0.82	0.82	0.82	0.14	0.14	0.14	0.14	
v/c Ratio	0.05	0.38	0.27	0.11	0.02	0.39	0.13	
Control Delay	3.0	4.1	3.3	36.4	0.1	43.4	0.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	3.0	4.1	3.3	36.4	0.1	43.4	0.3	
LOS	A	Α	A	D	Α	D	Α	
Approach Delay		4.1	3.3		21.2		19.3	
Approach LOS		Α	Α		C		В	
Queue Length 50th (m)	1.3	23.6	13.3	3.0	0.0	12.4	0.0	
Queue Length 95th (m)	4.1	46.7	27.6	8.8	0.0	24.3	0.0	
Internal Link Dist (m)		52.4	109.5		44.2		56.0	
Turn Bay Length (m)	20.0			15.0		30.0		
Base Capacity (vph)	770	1460	1435	224	622	253	782	
Starvation Cap Reductn	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.05	0.38	0.27	0.08	0.02	0.28	0.12	
Intersection Summary								
Cycle Length: 95								
Actuated Cycle Length: 95 Offset: 83 (87%), Referenced to pha	ase 2:EBTL a	nd 6:WBTL,	Start of Gr	een				
Natural Cycle: 50								
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.39								
Intersection Signal Delay: 6.4					ersection L0			
Intersection Capacity Utilization 51.5 Analysis Period (min) 15	5%			IC	U Level of S	Service A		
Splits and Phases: 1: W Village/L	anark & Scot	f						
A	anan a oou	•						I Nega
73 s								♦ Ø4
+								4.
√ Ø6 (R)								Tø8

	→	•	 	•	•	/
Mayamant	EDT			WDT		
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	40	-00	€	Y	20
Traffic Volume (veh/h)	568	10	26	417	9	30
Future Volume (Veh/h)	568	10	26	417	9	30
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	598	11	27	439	9	32
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)				77		
pX, platoon unblocked					0.96	
vC, conflicting volume			609		1096	604
vC1, stage 1 conf vol			000		1000	001
vC2, stage 2 conf vol						
vCu, unblocked vol			609		1079	604
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			7.1		U. T	0,2
tF (s)			2.2		3.5	3.3
p0 queue free %			97		96	94
			•			
cM capacity (veh/h)			970		225	499
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	609	466	41			
Volume Left	0	27	9			
Volume Right	11	0	32			
cSH	1700	970	394			
Volume to Capacity	0.36	0.03	0.10			
Queue Length 95th (m)	0.0	0.7	2.6			
Control Delay (s)	0.0	0.8	15.2			
Lane LOS		Α	С			
Approach Delay (s)	0.0	0.8	15.2			
Approach LOS	****		C			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			55.6%	ICI	J Level of S	onvice
			15	ICC	Level of S	CI VICE
Analysis Period (min)			15			

	•	-	•	-	1	†	>	1	
_ane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	7	1	7	1	7	T ₂	7	1	
Traffic Volume (vph)	90	421	13	542	6	0	55	1	
Future Volume (vph)	90	421	13	542	6	0	55	1	
Lane Group Flow (vph)	95	464	14	608	6	4	58	47	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1 01111	2	1 01111	6	1 01111	8	1 01111	4	
Permitted Phases	2		6	U	8	U	4	7	
Detector Phase	2	2	6	6	8	8	4	4	
Switch Phase	2	2	U	U	U	U	7	7	
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
` '									
Minimum Split (s)	26.1	26.1	26.1	26.1	21.5	21.5	21.5	21.5	
Total Split (s)	78.0	78.0	78.0	78.0	22.0	22.0	22.0	22.0	
Total Split (%)	78.0%	78.0%	78.0%	78.0%	22.0%	22.0%	22.0%	22.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)	-2.1	-2.1	-2.1	-2.1	-1.5	-1.5	-1.5	-1.5	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	83.3	83.3	83.3	83.3	12.6	12.6	12.6	12.6	
Actuated g/C Ratio	0.83	0.83	0.83	0.83	0.13	0.13	0.13	0.13	
v/c Ratio	0.16	0.31	0.02	0.41	0.04	0.01	0.35	0.20	
Control Delay	3.3	3.3	2.5	4.0	37.8	0.0	45.5	14.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	3.3	3.3	2.5	4.0	37.8	0.0	45.5	14.1	
LOS	Α	Α	Α	Α	D	Α	D	В	
Approach Delay		3.3		3.9		22.7		31.4	
Approach LOS		A		Α		C		С	
Queue Length 50th (m)	3.1	17.6	0.4	25.9	1.1	0.0	10.6	0.2	
Queue Length 95th (m)	8.3	33.7	1.8	49.0	4.6	0.0	21.9	10.0	
Internal Link Dist (m)	0,0	52.4	110	109.5	1,0	44.2	2110	56.0	
Turn Bay Length (m)	20.0	02.17	25.0	10010	15.0	7712	30.0	00.0	
Base Capacity (vph)	602	1477	715	1474	233	687	242	311	
Starvation Cap Reductn	0	0	0	0	0	007	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.16	0.31	0.02	0.41	0.03	0.01	0.24	0.15	
	0.10	0.31	0.02	0.41	0.03	0.01	0.24	0.15	
Intersection Summary									
Cycle Length: 100									
Actuated Cycle Length: 100									
Offset: 40 (40%), Referenced to phas	e 2:EBTL a	nd 6:WBTL	, Start of Gr	een					
Natural Cycle: 55									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.41									
Intersection Signal Delay: 6.0				Int	ersection Lo	OS: A			
Intersection Capacity Utilization 60.6	%				U Level of S				
Analysis Period (min) 15					0 2010, 0. 0	.011100 B			
()									
Splits and Phases: 1: W Village/La	nark & Scot	t							
A (2) (P)									774
→ Ø2 (R)									▼ Ø4
4-									
Ø6 (R)									₹øs

	-	•		—	1	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		CDR	WDL		NDL W	NDI
Traffic Volume (veh/h)	1 5 531	27	32	₄ 1 592	13	42
Future Volume (Veh/h)	531	27	32	592	13	42
Sign Control	Free	21	32	Free	Stop	42
Grade	0%					
		0.05	0.05	0%	0%	0.05
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	559	28	34	623	14	44
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)				77		
pX, platoon unblocked					0.90	
vC, conflicting volume			587		1264	573
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			587		1239	573
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF(s)			2.2		3.5	3.3
p0 queue free %			97		92	92
cM capacity (veh/h)			988		169	519
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	587	657	58			
Volume Left	0	34	14			
Volume Right	28	0	44			
cSH	1700	988	346			
Volume to Capacity	0.35	0.03	0.17			
Queue Length 95th (m)	0.0	0.03	4.5			
Control Delay (s)	0.0	0.8	17.5			
Lane LOS	0.0		17.5 C			
	0.0	A				
Approach LOS	0.0	0.9	17.5 C			
Approach LOS			U			
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utilization			70.6%	ICU	J Level of S	ervice
Analysis Period (min)			15			

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_ane Group	EBL	EBT	WBT	NBL	NBT	SBL	SBT	
_ane Configurations	7	7>	7	7	1	7	13	
Traffic Volume (vph)	39	599	368	17	0	68	0	
Future Volume (vph)	39	599	368	17	0	68	0	
_ane Group Flow (vph)	41	640	440	18	13	72	92	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	
Protected Phases	7 01111	2	6	1 01111	8	1 01111	4	
Permitted Phases	2	_	•	8		4	•	
Detector Phase	2	2	6	8	8	4	4	
Switch Phase	_	_	•	¥	¥	•	•	
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	26.1	26.1	26.1	21.5	21.5	21.5	21.5	
Fotal Split (s)	73.0	73.0	73.0	22.0	22.0	22.0	22.0	
Total Split (%)	76.8%	76.8%	76.8%	23.2%	23.2%	23.2%	23.2%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.8	2.8	2.8	2.2	2.2	2.2	2.2	
_ost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Fotal Lost Time (s)	6.1	6.1	6.1	5.5	5.5	5.5	5.5	
Lead/Lag	0.1	0.1	0.1	0.0	0.0	0.0	3.0	
Lead-Lag Optimize?								
Recall Mode	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	75.2	75.2	75.2	12.5	12.5	12.5	12.5	
Actuated g/C Ratio	0.79	0.79	0.79	0.13	0.13	0.13	0.13	
//c Ratio	0.06	0.45	0.32	0.12	0.03	0.43	0.15	
Control Delay	4.1	6.0	4.6	36.4	0.1	45.5	0.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	4.1	6.0	4.6	36.4	0.1	45.5	0.5	
LOS	A	A	A	D	A	D	A	
Approach Delay	, ,	5.9	4.6		21.2		20.3	
Approach LOS		A	A		C		C	
Queue Length 50th (m)	1.5	34.3	19.2	3.0	0.0	12.6	0.0	
Queue Length 95th (m)	4.9	67.2	38.9	8.9	0.0	24.7	0.0	
nternal Link Dist (m)		52.4	109.5	0,0	44.2		56.0	
Turn Bay Length (m)	20.0	0	10010	15.0		30.0	00.0	
Base Capacity (vph)	689	1408	1379	204	504	218	656	
Starvation Cap Reductn	0	0	0	0	0	0	0	
Spillback Cap Reductn	ő	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.06	0.45	0.32	0.09	0.03	0.33	0.14	
ntersection Summary								
Cycle Length: 95 Actuated Cycle Length: 95								
Actuated Cycle Length: 95 Offset: 81 (85%), Referenced to phas	o 2-EDTL o	nd 6·\A/DTI	Start of Cr	oon				
Natural Cycle: 60	oc Z.EDIL d	IIU U.VVD I L	Start Of Gr	CCII				
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0,45								
ntersection Signal Delay: 7.6				Int	ersection Lo	∩ S∙ Δ		
ntersection Signal Delay: 7.5 ntersection Capacity Utilization 55.8	0/_				ersection Li U Level of S			
Analysis Period (min) 15	/0			IC	o revelors	DEL AICE D		
anaiyaa renou (milli) 10								
Splits and Phases: 1: W Village/La	nark & Scot	t						
Spino and Finasco. 1. vv vinage/La	nan a oou	•						A Common of
→ø2 (R)								♦ Ø4
73 s								22 s
₹ø6 (R)								↑ øs
106 (111)								(78

	→	•	1	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			स	W	
Traffic Volume (veh/h)	627	13	26	476	16	50
Future Volume (Veh/h)	627	13	26	476	16	50
Sign Control	Free	10		Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	660	14	27	501	17	53
Pedestrians	000	1-7		001	.,,	00
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	None			NONE		
Upstream signal (m)				77		
pX, platoon unblocked				11	0.93	
vC, conflicting volume			674		1222	667
vC1, stage 1 conf vol			0/4		1222	007
vC2, stage 2 conf vol						
vCu, unblocked vol			674		1201	667
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			4.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			97		3.5 91	3.3 88
			917		184	459
cM capacity (veh/h)					104	408
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	674	528	70			
Volume Left	0	27	17			
Volume Right	14	0	53			
cSH	1700	917	337			
Volume to Capacity	0.40	0.03	0.21			
Queue Length 95th (m)	0.0	0.7	5.8			
Control Delay (s)	0.0	8.0	18.5			
Lane LOS		Α	С			
Approach Delay (s)	0.0	8.0	18.5			
Approach LOS			С			
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utilization			59.7%	ICI	U Level of S	ervice
Analysis Period (min)			15			
radigo.or offod (filling						

o. omtori a ono	٠		4	†		4
		•			🖈	-
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			स्	13	
Traffic Volume (veh/h)	27	12	12	39	36	3
Future Volume (Veh/h)	27	12	12	39	36	3
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	28	13	13	41	38	3
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	106	40	41			
vC1, stage 1 conf vol	100	10				
vC2, stage 2 conf vol						
vCu, unblocked vol	106	40	41			
tC, single (s)	6.4	6.2	4,1			
tC, 2 stage (s)	0.4	0,2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	99	99			
cM capacity (veh/h)	884	1032	1568			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	41	54	41			
Volume Left	28	13	0			
Volume Right	13	0	3			
cSH	926	1568	1700			
Volume to Capacity	0.04	0.01	0.02			
Queue Length 95th (m)	1.1	0.2	0.0			
Control Delay (s)	9.1	1.8	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	9.1	1.8	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			3,5			
Intersection Capacity Utilization			19.5%	ICI	U Level of Serv	/ice
Analysis Period (min)			15			
			10			

	•	-	•	-	\	1	>	1	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
ane Configurations	7	1	7	T ₂	7	T ₂	7	1	
raffic Volume (vph)	90	513	13	605	6	0	55	1	
uture Volume (vph)	90	513	13	605	6	0	55	1	
ane Group Flow (vph)	95	561	14	674	6	4	58	47	
urn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1 01111	2	1 01111	6	1 01111	8	1 01111	4	
Permitted Phases	2	_	6	•	8	•	4	'	
Detector Phase	2	2	6	6	8	8	4	4	
Switch Phase	2	2	U	U	U	U	7	7	
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	26.1	26.1	26.1	26.1	21.5	21.5	21.5	21.5	
	78.0	78.0	78.0	78.0	22.0	22.0	21.5	22.0	
Fotal Split (s)									
Fotal Split (%)	78.0%	78.0%	78.0%	78.0%	22.0%	22.0%	22.0%	22.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.2	2.2	2.2	2.2	
ost Time Adjust (s)	-2.1	-2.1	-2.1	-2.1	-1.5	-1.5	-1.5	-1.5	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	82.0	82.0	82.0	82.0	13.9	13.9	13.9	13.9	
Actuated g/C Ratio	0.82	0.82	0.82	0.82	0.14	0.14	0.14	0.14	
//c Ratio	0.18	0.39	0.02	0.47	0.04	0.01	0.33	0.20	
Control Delay	4.1	4.4	3.2	5.1	35.7	0.0	43,2	13.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Fotal Delay	4.1	4.4	3.2	5.1	35.7	0.0	43.2	13.4	
LOS	A	A	A	A	D	A	D	В	
Approach Delay	,,	4.3	, ,	5.0		21.4		29.8	
Approach LOS		A		A		C		C	
Queue Length 50th (m)	3.2	23.1	0.4	30.6	1.1	0.0	10.6	0.2	
Queue Length 95th (m)	9.6	49.2	2.0	64.9	4.5	0.0	21.4	9.8	
	9.0	52.4	2.0	109.5	4.5	44.2	Z1. 4	56.0	
nternal Link Dist (m)	20.0	52.4	25.0	109.5	45.0	44.2	20.0	0.00	
Turn Bay Length (m)	20.0	4454	25.0	4447	15.0	F70	30.0	007	
Base Capacity (vph)	530	1451	610	1447	219	572	227	287	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	
educed v/c Ratio	0.18	0.39	0.02	0.47	0.03	0.01	0.26	0.16	
ntersection Summary									
ycle Length: 100									
actuated Cycle Length: 100									
Offset: 40 (40%), Referenced to phas	e 2·FRTL a	nd 6·WRTI	Start of Gr	een					
latural Cycle: 60	66 Z.LD1L a	IIIU 0.77D I L,	Otall of Of	5611					
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.47									
				14	oroodis = 1.	7C+ A			
ntersection Signal Delay: 6.6	1/				ersection LO				
ntersection Capacity Utilization 65.99	70			IC	U Level of S	ervice C			
Analysis Period (min) 15									
white and Dhanner 4.3M3///	maul: 0 O - 1								
Splits and Phases: 1: W Village/La	nark & Scot	II						_	1.1
→ Ø2 (R)									₩ Ø4
22 057									22.5
78 s									22.5
₹Ø6 (R)									₹ Ø8
70 -									22.6

	_	•		-	•	<i>></i>
Mayamant	- FDT			WDT		
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	00	40	4	Y	40
Traffic Volume (veh/h)	623	32	46	641	17	42
Future Volume (Veh/h)	623	32	46	641	17	42
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	656	34	48	675	18	44
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)				77		
pX, platoon unblocked					0.87	
vC, conflicting volume			690		1444	673
vC1, stage 1 conf vol			000			0.0
vC2, stage 2 conf vol						
vCu, unblocked vol			690		1436	673
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			7.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			95		3.5 85	90
					122	
cM capacity (veh/h)			905		122	455
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	690	723	62			
Volume Left	0	48	18			
Volume Right	34	0	44			
cSH	1700	905	254			
Volume to Capacity	0.41	0.05	0.24			
Queue Length 95th (m)	0.0	1.3	7.1			
Control Delay (s)	0.0	1.4	23.7			
Lane LOS		Α	С			
Approach Delay (s)	0.0	1.4	23.7			
Approach LOS	0,0	1,7	C C			
Intersection Summary						
Average Delay			1.7			
Intersection Capacity Utilization			85.9%	ICI	J Level of S	ervice
Analysis Period (min)			15			
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	1	
Traffic Volume (veh/h)	4	17	8	55	59	19
Future Volume (Veh/h)	4	17	8	55	59	19
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	4	18	8	58	62	20
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	710110	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	146	72	82			
vC1, stage 1 conf vol	110		<u> </u>			
vC2, stage 2 conf vol						
vCu, unblocked vol	146	72	82			
tC, single (s)	6.4	6.2	4,1			
tC, 2 stage (s)	0.1	0,2	111			
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	98	99			
cM capacity (veh/h)	842	990	1515			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	22	66	82			
Volume Left	4	8	0			
Volume Right	18	0	20			
cSH	959	1515	1700			
Volume to Capacity	0.02	0.01	0.05			
Queue Length 95th (m)	0.5	0.1	0.0			
Control Delay (s)	8.8	0.9	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	8.8	0.9	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization			20.2%	ICI	U Level of Serv	ice
Analysis Period (min)			15			
			,,,			



INTERSECTIONS	Scenario 1950 Sco	Consultant PARSONS	Multi-Modal Level of S
IONS	1950 Scott Street TIA	S	Multi-Modal Level of Service - Intersections Form
Scott/Lana	Date	Project	
Scott/Lanark - Existing	6/29/2018	476658 - 1000	

ect	4/0000 - 0000
	6/29/2018

Au	uto Truck					Tra	ans	it	Bicycle										Pedestrian											ı															
Level of Service	Volume to Capacity Ratio	Level of Service		Effective Corner Radius Number of Receiving Lanes on Departure from Intersection		Average Signal Delay Level of Service		Average Signal Delay Level of Service		Level of Service		Level of Service		Level of Service		Average Signal Delay Level of Service		Average Signal Delay Level of Service			Left Turning Cyclist	Operating Speed	Left Turn Approach	Separated or Mixed Traffic	Cyclist relative to RT motorists	Right Turning Speed	Right Turn Lane Configuration	Bicycle Lane Arrangement on Approach	Approach From	Level of Service	-	Pedestrian Delay LoS	Average Pedestrian Delay	Effective Walk Time	Cucle Length	PETSI Score	Crosswalk Type	Corner Radius	Right Turn Channel	Ped Signal Leading Interval?	Right Turns on Red (RToR)?	Conflicting Right Turns	Conflicting Left Turns	Lanes Median	INTERSECTIONS Crossing Side
			Е	1	10 - 15 m						В	> 40 to ≤ 50 km/h	No lane crossed	•	-				NORTH		C	Α	9	54	o ₅ (70	Std transverse markings	10-15m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	NORTH										
Α	0.0 - 0.60	ш	Е	1	10 - 15 m	В			C		œ	≤ 40 km/h	No lane crossed	•	-				SOUTH	Е	С	Α	9	54	05	71	Std transverse markings	5-10m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	Scott/Lanark - Existing SOUTH EAST										
	0.60	•••	E	1	10 - 15 m	3	В	≤ 10 sec	С	С	> 40 to ≤ 50 km/h	1 lane crossed	Separated	Not Applicable			Curb Bike Lane,	EAST		Е	Е	40	ο (05 0	71	Std transverse markings	5-10m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	k - Existing EAST											
			Е	1	10 - 15 m		В	≤ 10 sec		С	С	> 40 to ≤ 50 km/h	1 lane crossed	Separated	Not Applicable			Curb Bike Lane,	WEST		m	Е	40	8 %	9, 0	71	Std transverse markings	5-10m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	WEST										
			Е	1	10 - 15 m					Α	Α	> 40 to ≤ 50 km/h	2-stage, LT box	Separated	Not Applicable			ne,	NORTH		С	Α	9	54	9, 0	74	Zebra stripe hi-vis markings	5-10m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	Sc. NORTH										
Α	0.0 - 0.60	Е	Е	1	10 - 15 m	В				Α	Α	≤ 40 km/h	2-stage, LT box	Separated	Not Applicable			ne,	SOUTH	Е	В	Α	9	54	05 u	75	Zebra stripe hi-vis markings	3-5m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	Scott/Lanark - Interim Cross-Section SOUTH EAST										
	0.60		Е	1	10 - 15 m	3	В	≤ 10 sec		Α	Α	> 40 to ≤ 50 km/h	2-stage, LT box	Separated	Not Applicable			ane,	EAST		Е	Е	40	8 %	05	57	Zebra stripe hi-vis markings	5-10m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	4 No Median - 2.4 m	rim Cross-Section East										
			В	≥2	10 - 15 m		В	≤ 10 sec		A	A	> 40 to ≤ 50 km/h	2-stage, LT box	Separated	Not Applicable			ane,	WEST		ш	Е	40	8 %	95	58	Zebra stripe hi-vis markings		No Channel	N ₀	RTOR allowed	Permissive or yield control	Permissive	4 No Median - 2.4 m	on WEST										
Α	00.0-0.0					В									Α	Α	> 40 to ≤ 50 km/h	2-stage, LT box	Separated	Not Applicable		Cycletrack or MUP	ine,	NORTH		c	Α	9	54	Sp. C	74	Zebra stripe hi-vis markings	5-10m	No Channel	N _o	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	NORTH					
		F					. -	Α	Α	Α	≤ 40 km/h	2-stage, LT box	Separated	Not Applicable		Cycletrack or MUP	Curb Bike Lane, Curb Bike Lane,	SOUTH	Е	В	Α	9	54	S 0	75	Zebra stripe hi-vis markings	3-5m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	Scott/Lanark - Future 2025 SOUTH EAST											
	0.60		F	1	< 10 m		В	≤ 10 sec		Α	Α	> 40 to ≤ 50 km/h	2-stage, LT box	Separated	Not Applicable		Cycletrack or MUP	Curb Bike Lane,	EAST		Е	Е	40	æ (ο _κ (74	Zebra stripe hi-vis markings	5-10m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	- Future 2025 EAST										
			F	1	< 10 m		≤ 10 sec		A	٨	> 40 to ≤ 50 km/h	2-stage, LT box	Separated	Not Applicable		Cycletrack or MUP	Curb Bike Lane,	WEST		т	E	40	8 %	05	75	Zebra stripe hi-vis markings	3-5m	No Channel	No	RTOR allowed	Permissive or yield control	Permissive	3 No Median - 2.4 m	WEST											