

### TECHNICAL MEMORANDUM

DATE: JANUARY 2018

TO: MURRAY CHOWN

FROM: CARA RUDDLE

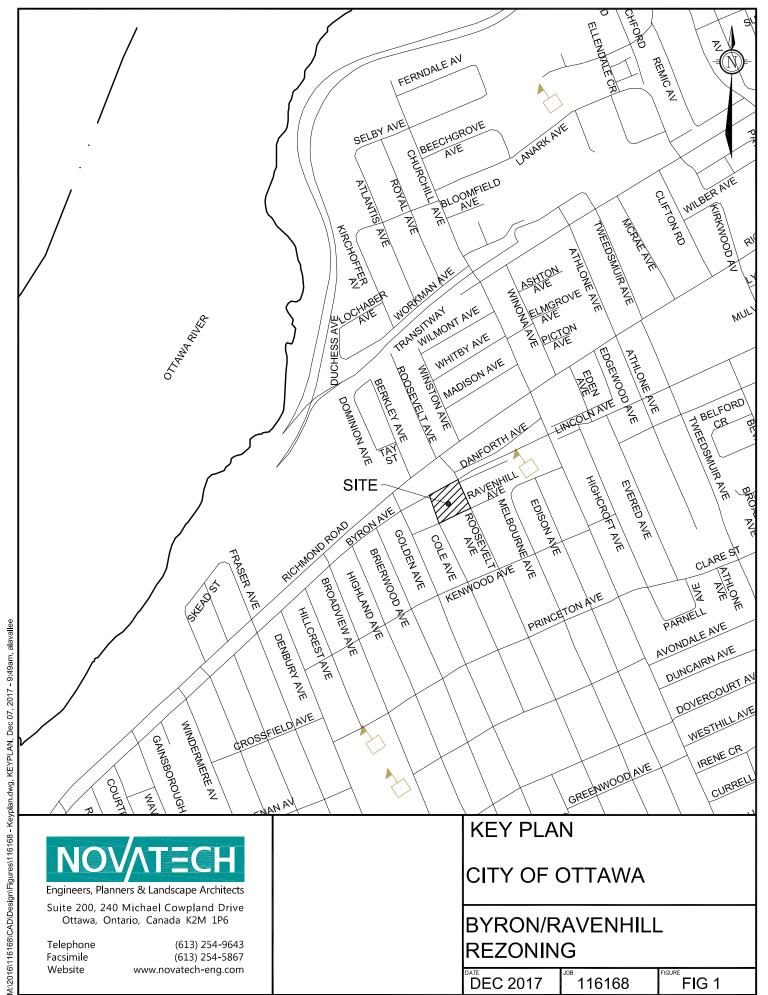
RE: BYRON / RAVENHILL AVENUE REZONING

Novatech has been retained to review the adequacy of existing services for the re-zoning of the existing properties 556-576 Byron Avenue, 436-446 Roosevelt Avenue and 411-425 Ravenhill Avenue within the City of Ottawa. Refer to **Figure 1** – Key plan for the site location. The subject site consists of 8 existing lots with a total area of approximately 0.489 ha. Currently, seven of the eight lots are developed with residential dwellings and a commercial development (Westboro Dental Center). Refer to **Figure 2** – Existing Conditions. The rezoning would allow for the severance of 8 existing lots in half to create 16 lots for the construction of 16 low-rise residential apartment buildings. Each building would be limited to 3 storeys and a maximum of 4 units with an approximate footprint of 1500sqft. Refer to **Figure 3** – Potential Development Plan.

This technical memorandum is being submitted in support of a rezoning application. The memo will review the water, sanitary and storm servicing requirements for the potential development of 16 low-rise apartment buildings and will provide an analysis on the existing infrastructure surrounding the site to ensure there is adequate capacity.

#### **Water Servicing**

There is an existing public 150mm diameter watermain within the Byron, Roosevelt and Ravenhill Avenue right-of-way's. The existing 150mm diameter watermain currently services the existing residential and commercial developments on the subject site. It is proposed to service the potential 16 low-rise apartment buildings directly from the existing watermain along Byron and Ravenhill Avenue. Refer to **Figure 4** – Existing Services Figure. The water demands for the potential development were calculated and provided to the City to obtain boundary conditions to confirm serviceability. The domestic water demand calculations are based on a theoretical population for the potential development based on criteria provided in the City of Ottawa Water Design Guidelines. The required fire flow was calculated using the Fire Underwriter's Survey method and is based on a 3-storey above ground wood frame construction. The water demand calculations, boundary conditions and hydraulic analysis calculation for the existing public infrastructure are provided in **Appendix A** for reference. The results of the hydraulic analysis are summarized below in **Table 1**.



SHT8X11.DWG - 216mmx279mm



CUT11V17 DIA/C 270mmV122mm





Engineers, Planners & Landscape Architects

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6

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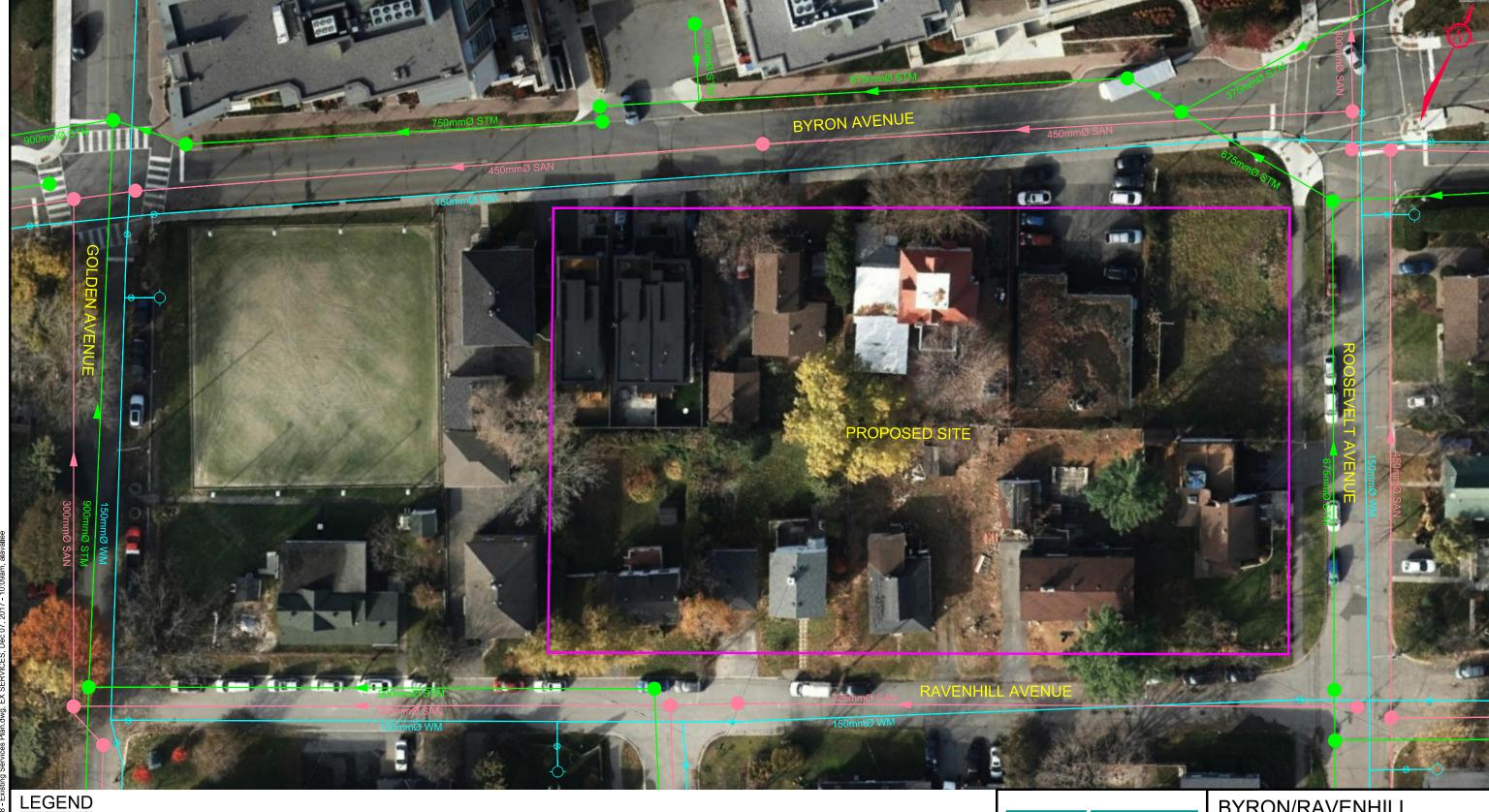
(613) 254-9643 (613) 254-5867 www.novatech-eng.com



City of Ottawa **BYRON & RAVENHILL** POTENTIAL DEVELOPMENT **PLAN** 1:750 FEB, 2018 FIG 3

116168

SHT8X11.DWG - 216mmx279mm





EXISTING SANITARY SEWER AND MANHOLE WITH DIRECTION OF FLOW

EXISTING STORM SEWER AND MANHOLE WITH DIRECTION OF FLOW

**∞** 

EXISTING VALVE AND HYDRANT

EXISTING WATERMAIN

NOVATECH

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Telephone Facsimile Website (613) 254-9643 (613) 254-5867 www.novatech-eng.com BYRON/RAVENHILL REZONING

**EXISTING SERVICES PLAN** 

CUT11V17 DIA/C 270mm V122m

Table1: Water Analysis Results Summary

Condition	Street	Demand (L/s)	Min/Max Allowable Operating Pressures (psi)	Limits of Design Operating Pressures (psi)
High Pressure	Ravenhill / Byron	0.55	80psi (Max)	67.3
Maximum Daily	Ravenhill	152.46	20noi (Min)	24.5
Demand and <i>Fire</i> <i>Flow</i>	Byron	152.46	20psi (Min)	23.3
Peak Hour	Ravenhill / Byron	3.72	40psi (Min)	58.0

The results of the water analysis show there is adequate flow and pressure in the existing 150mm watermain in Byron Avenue and Ravenhill Avenue to meet the required domestic demands and pressures for fire flow.

#### **Sanitary Servicing**

There is an existing 225mm and 300mm diameter sanitary sewer in Ravenhill Avenue and a 450mm diameter sanitary sewer in Byron Avenue. The total sanitary flows from the existing development is calculated to be 2.68 L/s. It is proposed to service the potential 16 low-rise apartment buildings directly from the existing sewers in Ravenhill and Byron Avenue. Refer to **Figure 4** – Existing Services Figure. The sanitary flows generated by the potential development are calculated to be 2.39 L/s (calculations below). There is a total decrease in sanitary flow of 0.29 L/s in the proposed condition. This is due to the City of Ottawa requirement for fully separated systems in all new developments.

#### **Existing Development Flows**

Number of Existing Single-Family Units = 10 units

Population = 10 units x 3.4 people/unit = 34 people

Residential Peaking Factor = 4 (Harmon Formula)

Domestic Sanitary Flow = (280L/person/day x 34 people) / 86400 sec/day x 4 = 0.44 L/s

Institutional Area = 0.06ha Institutional Peaking Factor = 1.5 Domestic Sanitary Flow = (50000L/ha/day x 0.06ha) / 86400 sec/day x 1.5 = 0.05 L/s

Foundation drain allowance for unseparated systems = 3 L/s/ha Site Drainage Area = 0.73 ha Foundation Drainage = 3 L/s/ha x 0.73 ha = 2.19 L/s

Total Existing Sanitary Flow = 0.44 + 0.05 + 2.19 = 2.68 L/s

#### Potential Development Flows

Number of 2 Bedroom Units = 16 buildings x 4 units/building = 64 units

Population = 64 units x 2.1 people/unit = 135 people

Residential Peaking Factor = 4 (Harmon Formula)

Domestic Sanitary Flow = (350L/person/day x 135 people) / 86400 sec/day x 4 = 2.19 L/s

Extraneous flow allowance = 0.28 L/s/ha
Site Drainage Area = 0.73ha
Extraneous Flow = 0.28 L/s/ha x 0.73ha = 0.20 L/s

Total Potential Development Sanitary Flow = 2.19 + 0.20 = 2.39 L/s

A downstream analysis of the 525mm diameter sanitary trunk sewer to the west of the Byron and Golden Avenue intersection was completed to confirm the capacity in the existing sanitary system. The sanitary sewer design sheets for the existing and proposed conditions and drainage area figures are provided in **Appendix B** for reference.

A review of the downstream analysis, in the existing condition, shows that the one section of the 300mm diameter sanitary sewer in Ravenhill Avenue is at capacity (99.8% full). The remainder of the sanitary sewer system has excess capacity. The total peak design flows are not likely indicative of the actual flows since the flows generated in the spreadsheets assume that the entire drainage area is serviced by a non-separated system. As per the City of Ottawa Sewer Design Guidelines the sanitary analysis incorporates a 3.0 L/s/ha foundation drain allowance for existing developments where there is potential for partially separated systems.

In the proposed condition, the total peak design flow to the 300mm diameter sewer in Ravenhill Avenue is reduced by 0.29 L/s since the potential development will require a separated foundation drain system. Therefore, since the potential development decreases the flows from the existing condition, there are no concerns that the potential development flows will have any adverse effects on the existing infrastructure.

#### **Storm Servicing & Stormwater Management**

There is an existing 675mm diameter storm sewer along the north side of the Byron Avenue and an existing 900mm diameter storm sewer along Ravenhill Avenue to the west of the potential development. There is no existing storm sewer in Ravenhill Avenue fronting the potential development. Refer to **Figure 4** – Existing Services Figure. The potential development foundation drainage is required to have full separation from the sanitary system. This will require all new development in the Ravenhill area to utilize sump pumps to the surface for foundation drainage.

Through the site plan approval process stromwater management will be required on a site by site basis. Quality control requirements should be confirmed with the Conservation Authority during detailed design. Quantity control will be required to meet pre-development conditions for the site.

#### **Conclusion**

Based on the foregoing, the existing sanitary sewer and watermain infrastructure have capacity to service the potential development. Stormwater management, including quantity and quality control of stormwater will be required on a site by site basis. Quantity control will ensure that storm flows from each site will not exceed existing conditions. The existing storm sewer system will have adequate capacity to service the potential development.

#### **NOVATECH**

Prepared by:

Reviewed by:

Matthew Hrehoriak, EIT

Cara Ruddle, P.Eng. Senior Project Manager

#### **List of Appendices:**

Appendix A: Water Calculations
Appendix B: Sanitary Calculations

### **APPENDIX A**

Water Calculations



# BYRON / RAVENHILL AVENUE HYDRAULIC ANALYSIS

JOB NO. 116168 DATE: NOVEMBER 2, 2017

		٧	Table 1 Vater Dema	and		
Node	Resid	lential Pop	ulation	Reside	ntial Demar	nd (L/s)
Node	Uni	ts	Total	A D	Max.	Deal-Harr
	Units	POP	Population	Avg Day	Daily	Peak Hour
Proposed Development	64	135	135	0.55	2.46	3.72
Total Pop	64	135	135	0.55	2.46	3.72

#### **Design Parameters:**

- 2 Bed Apartment	2.1	pop/unit
Section 4.0 Ottawa Sewer Design Guidelines		
- Average Domestic Flow	350	L/day
Peaking Factors: Table 3-3 Moe Guideline		•
Max. Daily Demand:		
- Residential	4.5	
Peak Hourly Demand:		
- Residential	6.8	

### **FUS - Fire Flow Calculations**

As per 1999 Fire Underwriter's Survey Guidelines



**Novatech #:** 116168

Project Name: Byron / Ravenhill Avenue

Date: Nov 2/17
Input By: Matt Hrehoriak

Legend

Input by User

No Information or Input Required

Building Description: 3 Storey above ground 4 Unit apartment

Step			Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)
		Required Fire	Flow			,
	Construction Ma	•				
1	Coefficient related to type of construction	Wood frame Ordinary construction Non-combustible construction	Yes	1.5 1 0.8	1.5	
	С	Fire resistive construction (< 3 hrs) Fire resistive construction (> 3 hrs)		0.7 0.6		
	Floor Area	Gross Floor Area (m <sup>2</sup> )	140			
2	A	Number of Floors/Storeys	3			
		Area of structure considered (m <sup>2</sup> )			420	
	F	Base fire flow without reductions				7,000
	_	$F = 220 \text{ C (A)}^{0.5}$				,
		Reductions or Su	ırcharges			
	Occupancy haza	ard reduction or surcharge				
3		Non-combustible Limited combustible	Yes	-25% -15%		
	(1)	Combustible Free burning		0% 15%	-25%	5,250
	O i . I I D . I	Rapid burning		25%		
	Sprinkler Reduc		No	200/		
4		Adequately Designed System (NFPA 13)	No	-30% -10%		
7	(2)	Standard Water Supply Fully Supervised System	No	-10%		0
		Fully Supervised System		ulative Total	0%	
	Exposure surch	arge (cumulative (%))			-70	
	•	North Side	10.1 - 20 m		15%	
5		East Side	0 - 3 m		25%	
3	(3)	South Side	20.1 - 30 m		10%	3,938
		West Side	0 - 3 m		25%	
		Total Denvined fine Floor moved of the		lative Total	75%	0.000
		Total Required fire Flow, rounded to near (2,000 L/min < Fire Flow < 45,000 L/min)	est 1000L/m	in or	L/min L/s	9,000 150
	(1) + (2) + (3)	(2,000 Dillill < 1 lie 1 low < 45,000 Dillill)		or	USGPM	2,378
	(1) + (2) + (3)	Required Duration of Fire Flow (hours)		1	Hours	2
		Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1080

#### **Matthew Hrehoriak**

From: Wu, John < John.Wu@ottawa.ca>
Sent: November-16-17 9:01 AM

Matthew Urahariak

**To:** Matthew Hrehoriak

Subject: RE: Byron Ravenhill Avenue Watermain Boundary Condition Request

**Attachments:** 116168 - WM-FIG 1 - Marked Up PDF.pdf

#### Here is the result:

## \*\*\*\*The following information may be passed on to the consultant, but do NOT forward this e-mail directly.\*\*\*\*

The following are boundary conditions, HGL, for hydraulic analysis at 556-576 Byron, 436-446 Roosevelt, and 411-425 Ravenhill (zone 1W) assumed to be connected to the 152 mm on Byron, 152 mm on Roosevelt, and 152 mm on Ravenhill (see attached PDF for location).

Minimum HGL = 108.8 m (All connections)

Maximum HGL = 115.3 m (All connections)

Max Day + Fire Flow = 85.9 m (Connection 1)

Max Day + Fire Flow = 83.9 m (Connection 2)

Max Day + Fire Flow = 83.7 m (Connection 3)

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

#### John

**From:** Matthew Hrehoriak [mailto:m.hrehoriak@novatech-eng.com]

Sent: Monday, November 06, 2017 9:10 AM

To: Wu, John < John. Wu@ottawa.ca>

Cc: Cara Ruddle <c.ruddle@novatech-eng.com>

Subject: Byron Ravenhill Avenue Watermain Boundary Condition Request

Hi John,

Please find below water demand information for the proposed development at the existing 566-576 Bryon Ave (even only), 436 & 446 Roosevelt Ave and 411-425 Ravenhill Ave (odd only) properties. Also, attached is a key plan showing the site location. Please provide boundary conditions for the existing watermain infrastructure highlighted on the attached plan so we can confirm the existing infrastructure has capacity for the proposed development.

Water Demands proposed development:

AVG DAY = 0.55L/s MAX DAY = 2.46L/s PEAK HOUR = 3.72L/s MAX DAY + FIRE =152.46L/s

Thanks.

Matthew Hrehoriak, B.Eng., EIT

**NOVATECH** Engineers, Planners & Landscape Architects

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# BYRON / RAVENHILL AVENUE HYDRAULIC ANALYSIS

JOB #: 116168 DATE: NOVEMBER 20, 2017

#### **CALCULATED WATER DEMNADS:**

PROPOSED DEVELOPMENT (16-4UNIT APRTMENT BUILDINGS)

AVERAGE DAY = 0.550 L/sMAXIMUM DAY = 2.460 L/sPEAK HOUR = 3.720 L/sMAX DAY + FIRE = 152.460 L/s

#### **CITY OF OTTAWA BOUNDARY CONDITIONS:**

BOUNDAY CONDITIONS BASED ON (ZONE 1W) CONNECTION TO 150mm DIA. WATERMAIN ON BYRON AVE.

MINIMUM HGL = 108.8 mMAXIMUM HGL = 115.3 mMAX DAY + FIRE = 85.9 m

BOUNDAY CONDITIONS BASED ON (ZONE 1W) CONNECTION TO 150mm DIA. WATERMAIN ON RAVENHILL AVE.

MINIMUM HGL = 108.8 m MAXIMUM HGL = 115.3 m MAX DAY + FIRE = 83.7 m



## BYRON / RAVENHILL AVENUE HYDRAULIC ANALYSIS

JOB #: 116168 DATE: NOVEMBER 20, 2017

#### **WATERMAIN ANALYSIS:**

#### BYRON AVE WATERMAIN CONNECTIONS

AVERAGE GROUND ELEVATION = 68.0 m APROX BYRON HYDRANT ELEV = 69.5 m

HIGH PRESSURE TEST = MAX HGL - AVG GROUND ELEV x 1.42197 PSI/m < 80 PSI

HIGH PRESSURE = 67.3 PSI

LOW PRESSURE TEST = MIN HGL - AVG GROUND ELEV x 1.42197 PSI/m > 40 PSI

LOW PRESSURE = 58.0 PSI

MAX DAY + FIRE TEST = MAX DAY + FIRE - AVG GROUND ELEV x 1.42197 PSI/m > 20 PSI

LOW PRESSURE = 23.3 PSI

#### **RAVENHILL AVE WATERMAIN CONNECTIONS**

AVERAGE GROUND ELEVATION = 68.0 m APROX RAVENHILL HYDRANT ELEV = 66.5 m

HIGH PRESSURE TEST = MAX HGL - AVG GROUND ELEV x 1.42197 PSI/m < 80 PSI

HIGH PRESSURE = 67.3 PSI

LOW PRESSURE TEST = MIN HGL - AVG GROUND ELEV x 1.42197 PSI/m > 40 PSI

LOW PRESSURE = 58.0 PSI

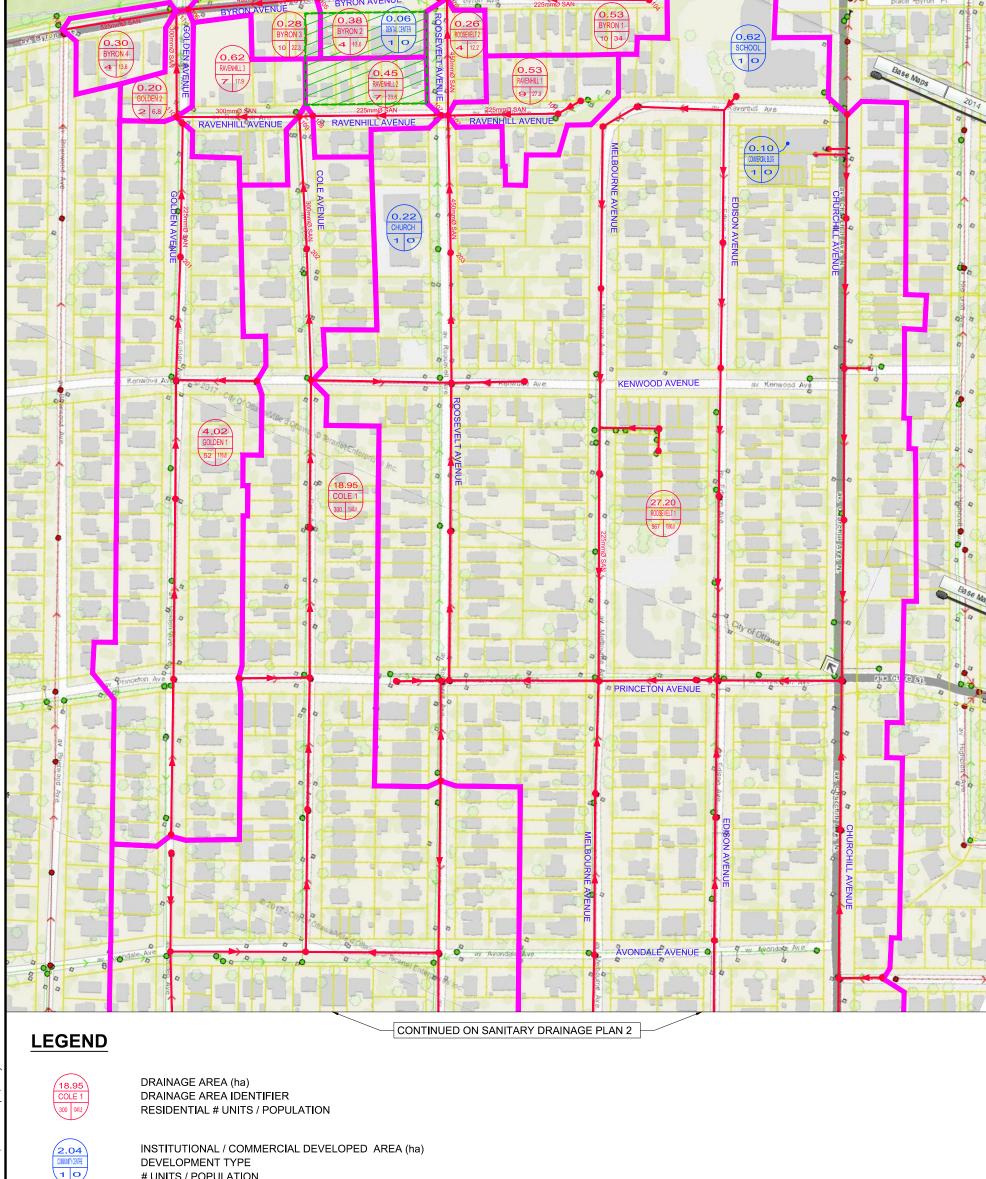
MAX DAY + FIRE TEST = MAX DAY + FIRE - AVG GROUND ELEV x 1.42197 PSI/m > 20 PSI

LOW PRESSURE = 24.5 PSI

THE EXISTING 150mm DIAMETER WATERMAIN IN BYRON AND RAVENHILL AVENUE PASSES ALL THREE ANALYLIS TESTS, THEREFORE THERE IS CAPCITY IN THE EXISTING INFRASTRUCTURE FOR THE PROPOSED ADDITION.

### **APPENDIX B**

Sanitary Calculations



Calcs\SAN\SAN DRAINAGE.dwg, SAN 1, Dec 08, 2017 - 12:02pm, Ise

# UNITS / POPULATION

SANITARY SEWER c/w MANHOLE AND FLOW DIRECTION

DRAINAGE AREA BOUNDARY

SUBJECT SITE BOUNDARY



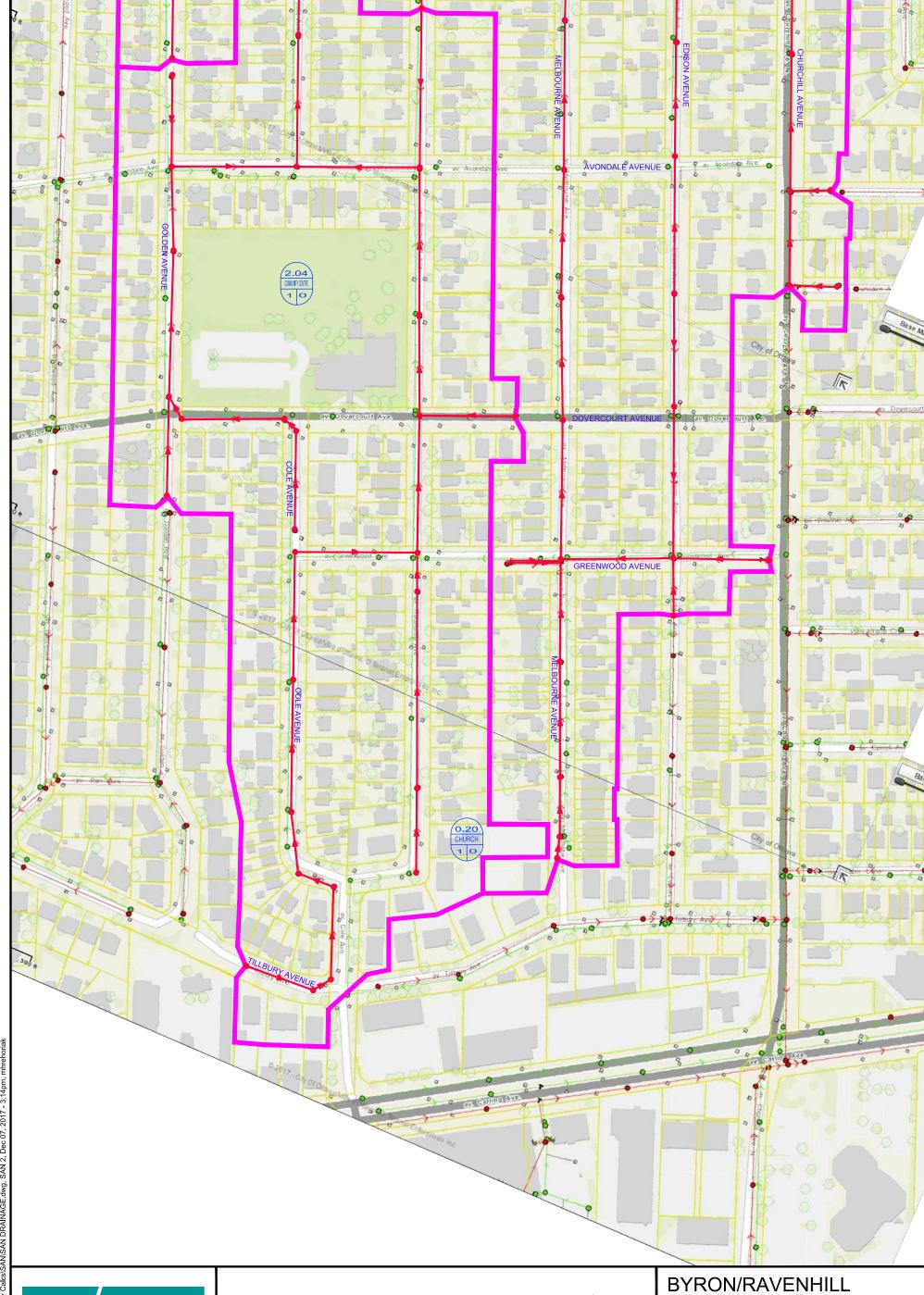
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BYRON/RAVENHILL **REZONING** 

SANITARY DRAINAGE PLAN 1

1 : 2500° NOV 2017 116168 **A1** 





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BYRON/RAVENHILL REZONING

SANITARY DRAINAGE PLAN 2

 PROJECT #: 116168 PROJECT NAME: BYRON RAVENHILL AVENUE LOCATION: CITY OF OTTAWA



#### **Existing Condition Sanitary Flows**

	Location		Resid	lential	Institu	tional	Cum	ulative	tive Peak Factor		Instit	tutional	Residential	Infiltr	ation	Found	lation	Darda Dardana	PIPE						
Street / Area	From	То	Population	Area (ha)	Area (ha)	Accu. Area (ha)	Pop.	Area (ha)	Res Peak Factor	Insti Peak Factor	Peak Flow (I/s)	Accu. Peak Flow	Peak Flow (I/s)	Infilt. Flow (I/s)	Accu Infil. Flow	Found. Flow (I/s)	Accu Found. Flow	Peak Design Flow (I/s)	Size (mm)	Slope (%)	Length (m)	Capacity (I/s)	Full Flow Vel. (m/s)	Q/Q <sub>full</sub> (%)	
Roosevelt Ave 1	203	102	1590.8	27.20	0.94	0.94	1590.8	27.20	3.7	1.5	0.8	0.8	18.87			81.60	81.60	101.29	450	0.84	129.3	261.1	1.64	38.8%	
Ravenhill Ave 1	101	102	27.3	0.53			27.3	0.53	4.0	1.5	0.0	0.0	0.35			1.59	1.59	1.94	225	1.68	72.9	58.1	1.46	3.3%	
1101011111117110			27.0	0.00			27.10	0.00	0		0.0	0.0	0.00			1.00	1.00			1.00	12.0	00.1		0.070	
Roosevelt Ave 2	102	103	12.2	0.26			1630.3	27.99	3.7	1.5	0.0	0.8	19.30			0.78	83.97	104.09	450	1.18	110.3	309.5	1.95	33.6%	
Byron Ave 1	104	103	34.0	0.53			34.0	0.53	4.0	1.5	0.0	0.0	0.44			1.59	1.59	2.03	225	0.50	190.3	31.7	0.80	6.4%	
Bylon Ave 1	104	103	34.0	0.55			34.0	0.55	4.0	1.5	0.0	0.0	0.44			1.59	1.09	2.03	223	0.50	190.3	31.7	0.60	0.476	
Byron Ave 2	103	105	13.6	0.38	0.06	0.06	1677.9	28.90	3.6	1.5	0.1	0.9	19.81			1.14	86.70	107.38	450	1.68	114.5	369.3	2.32	29.1%	
Byron Ave 3	105	106	22.3	0.28			1700.2	29.28	3.6	1.5	0.0	0.9	20.05			0.84	87.54	108.46	450	1.18	122.2	309.5	1.95	35.0%	
			1																						
Ravenhill Ave 2	107	108	23.8	0.45			23.8	0.45	4.0	1.5	0.0	0.0	0.31			1.35	1.35	1.66	225	3.10	121.2	79.0	1.99	2.1%	
Cole Ave 1	202	109	1045.8	18.95	2.24	2.24	1045.8	18.95	3.8	1.5	1.9	1.9	12.84			56.85	56.85	71.63	300	1.40	124.8	114.3	1.62	62.7%	
Ravenhill Ave 3	109	110	17.9	0.62			1087.5	20.02	3.8	1.5	0.0	1.9	13.31			1.86	60.06	75.31	300	0.61	80.9	75.5	1.07	99.8%	
Golden Ave 1	201	110	176.8	4.02			176.8	4.02	4.0	1.5	0.0	0.0	2.29			12.06	12.06	14.35	300	0.61	88.0	75.5	1.07	19.0%	
Golden Ave 2	110	111	6.8	0.20			1271.1	24.24	3.7	1.5	0.0	1.9	15.37			0.60	72.72	90.03	300	1.20	70.0	105.8	1.50	85.1%	
COIGOIT/WC Z	.10		3.0	0.20			12/1.1	27.27	5.7	1.5	0.0	1.5	10.07			0.00	12.12	55.05	300	1.20	7 0.0	100.0	1.50	55.170	
Byron TRUNK	111	112	13.6	0.30			2984.9	53.8	3.4	1.5	0.0	2.8	33.32			0.90	161.16	197.29	525	0.30	75.0	235.4	1.09	83.8%	

Design Parameters

Single Family 3.4 persons/unit Average Apartments
Section 4.0 Ottawa Sewer Design Guidelines
- Average Domestic Flow Existing Development
- Average Domestic Flow Proposed Development 1.8 persons/unit 280 350 l/person/day l/person/day - Institutional / Commercial Flow 50000 l/ha/day - Extraneous Flows (Only used in separated systems) 0.28 l/ha/day - Foundation Drain Allowance l/ha/day 3.0 Residential Peaking Factor Institutional / Commercial Peaking Factor Harmon Equation

Notes:

Used the Average Apt./Persons Per Unit Value of 1.8 when determining the apartment populations.

1.5

The number of units in an apartment buildings are assumed values.

Pipe information taken from Geo Ottawa

0.28L/s/ha Infiltration Allowance accounted for in 3 L/s/ha foundation drainage allowance



EXISTING DRAINAGE AREA (ha)
DRAINAGE AREA IDENTIFIER
RESIDENTIAL # UNITS / POPULATION



INSTITUTIONAL / COMMERCIAL DEVELOPED AREA (ha) DEVELOPMENT TYPE # UNITS / POPULATION



SANITARY SEWER c/w MANHOLE AND FLOW DIRECTION

DRAINAGE AREA BOUNDARY



PROPOSED DRAINAGE AREA (ha) DRAINAGE AREA IDENTIFIER RESIDENTIAL # UNITS / POPULATION



SUBJECT SITE BOUNDARY



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BYRON/RAVENHILL REZONING

SANITARY DRAINAGE PLAN 3 (PROPOSED DEVELOPMENT)

1:2500° 25 50 75 100 DATE NOV 2017 JOB 116168 FIGURE A3

SHT11X17.DWG - 279mmX432mm



#### **Proposed Condition Sanitary Flows**

	Location		Proposed	pposed Residential Existing Residential			Institu	utional	Cum	nulative	Peak	Factor	Insti	tutional	Residential	Infilt	ration	Foundation		Peak	PIPE					
Street / Area	From	То	Population	Area (ha)	Population	Area (ha)	Area (ha)	Accu. Area (ha)	Pop.	Area (ha)	Res Peak Factor	Insti Peak Factor	Peak Flow (I/s)	Accu. Peak Flow	Peak Flow (I/s)	Infilt. Flow (I/s)	Accu Infil. Flow	Found. Flow (I/s)	Accu Found. Flow	Design Flow (I/s)	Size (mm)	Slope (%)	Length (m)	Capacity (I/s)	Full Flow Vel. (m/s)	Q/Q <sub>full</sub> (%)
Roosevelt Ave 1	203	102			1590.8	27.20	0.94	0.94	1590.8	27.20	3.7	1.5	0.8	0.8	18.87		0.00	81.60	81.60	101.29	450	0.84	129.3	261.1	1.64	38.8%
Ravenhill Ave 1	101	102			27.3	0.53			27.3	0.53	4.0	1.5			0.35		0.00	1.59	1.59	1.94	225	1.68	72.9	58.1	1.46	3.3%
Roosevelt Ave 2	102	103			12.2	0.26			1630.3	27.99	3.7	1.5		0.8	19.30		0.00	0.78	83.97	104.09	450	1.18	110.3	309.5	1.95	33.6%
Byron Ave 1	104	103			34.0	0.53			34.0	0.53	4.0	1.5			0.44		0.00	1.59	1.59	2.03	225	0.50	190.3	31.7	0.80	6.4%
Byron Ave 2 Byron Ave 3	103 105	105 106	58.8 8.4	0.38	0.0	0.00	0.06	0.06	1723.1	28.52	3.6	1.5	0.1	0.9	20.47	0.11	0.11	0.00	85.56	107.01	450	1.68	114.5	369.3	2.32	29.0%
Byron Ave 3	105	100	8.4	0.04	22.3	0.24			1753.8	28.52	3.6	1.5			20.70	0.01	0.12	0.72	86.28	107.10	450	1.18	122.2	309.5	1.95	34.6%
Ravenhill Ave 2	107	108	67.2	0.31	3.4	0.14			70.6	0.14	4.0	1.5			1.13	0.09	0.09	0.42	0.42	1.64	225	3.10	121.2	79.0	1.99	2.1%
Cole Ave 1	202	109			1045.8	18.95	2.24	2.24	1045.8	18.95	3.8	1.5	1.9	1.9	12.84		0.00	56.85	56.85	71.63	300	1.40	124.8	114.3	1.62	62.7%
Ravenhill Ave 3	109	110			17.9	0.62			1134.3	19.71	3.8	1.5		1.9	14.04		0.09	1.86	59.13	75.20	300	0.61	80.9	75.5	1.07	99.7%
Golden Ave 1	201	110			176.8	4.02			176.8	4.02	4.0	1.5			2.29		0.00	12.06	12.06	14.35	300	0.61	88.0	75.5	1.07	19.0%
Golden Ave 2	110	111			6.8	0.20			1317.9	23.93	3.7	1.5		1.9	16.09		0.09	0.60	71.79	89.91	300	1.20	70.0	105.8	1.50	84.9%
Byron 4	111	112			13.6	0.30			3085.3	52.8	3.4	1.5		2.8	34.69		0.20	0.90	158.97	196.68	525	0.30	75.0	235.4	1.09	83.6%

Design I	Parameter:
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Single Family Average Apartments

Section 4.0 Ottawa Sewer Design Guidelines
- Average Domestic Flow
- Average Domestic Flow Proposed Development

- Institutional / Commercial Flow

- Extraneous Flows

- Foundation Drain Allowance

Residential Peaking Factor
Institutional / Commercial Peaking Factor

Used the Average Apt./Persons Per Unit Value of 1.8 when determining the apartment populations. The number of units in an apartment buildings are assumed values. Pipe information taken from Geo Ottawa Notes:

3.4 persons/unit 1.8 persons/unit

280 l/person/day 350 l/person/day 50000 l/ha/day

0.28 I/ha/day l/ha/day 3.0

Harmon Equation

1.5

Foundation drain allowance not required for proposed developments as systems are required to be fully separated. Foundation drain allowance replaced by extraneous flow in proposed development condition.