



VIA Email: lloyd@lloydphillips.com

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The Hindu Temple of Ottawa Carlton Inc.
c/o Lloyd Phillips & Associates Ltd.
1827 Woodward Drive, Suite 109,
Ottawa, Ontario K2C 0P9

Attention: Mr. Lloyd Phillips

Subject: Terrain Analysis and Private Sewage Disposal System Impact Assessment
- Proposed Assembly Hall
The Hindu Temple of Ottawa Carlton, 4835 Bank Street, Ottawa, Ontario

Dear Mr. Phillips,

LRL Associates Ltd. (LRL) has conducted a Terrain Analysis and Private Sewage Disposal System Impact Study for the proposed Hindu Temple of Ottawa Carlton Assembly Hall to be constructed on the property located at 4835 Bank Street, Ottawa, Ontario (herein referred to as the "Site"). It is understood that it is proposed that a 2,000 m² Assembly Hall will be constructed at the eastern portion of the existing developed property which will have an available capacity of approximately 600 individuals, increasing the total occupancy of the site to an estimated 850.

The proposed Assembly Hall will be supplied by municipal water supply and a private septic as is the existing development on the Site.

The assessment was carried out to determine if the proposed development:

- Has soil conditions that are suitable for onsite sewage disposal; and
- Will not impair the use of groundwater resources on the Site or on adjacent lands.

The proposed sewage system will be designed for approximately 21.6 m³/day of wastewater, as outlined below, which requires approval under Section 53 of the Ontario Water Resources Act (systems greater than 10,000 L/day). Large systems are subject to the Ontario Ministry of the Environment and Climate Change's (MOECC) Guideline B-7¹ which defines the acceptable levels of groundwater impacts of the system on the neighbouring properties.

¹ Incorporation of the Reasonable Use Concept into MOEE Groundwater Management Activities, Guideline B-7, April 1994

Guideline B-7 establishes the amount of contamination that may be discharged to the adjacent property that will have no more than a negligible effect on the present or potential ~~reasonable~~ use of that property. For health-related parameters, the groundwater quality must not be degraded by an amount in excess of 25% of the difference between background and the Ontario Drinking Water Objectives. For the purposes of this Site, the levels of nitrates must be less than 2.5 mg/L at the limits of the property.

Reasonable current and potential uses shall be established with respect to Site specific soil and water bearing units in the subsurface and would apply to all of the ground lying beneath a particular property. Such a decision based on the present use of groundwater in the vicinity.

The assessment involved a desktop review of available information on the geology and hydrogeology of the Site and adjacent lands in addition to an instructive subsurface investigation (test pitting program). The Site is serviced by municipal water supply, however, neighbouring properties within 500 m of the Site were found to have records of supply wells present.

1 SITE AND AREA DESCRIPTION

The property is situated at the southern extents of the City of Ottawa at 4385 Bank Street. The property is legally described as Part Lot 22, Concession 5RF Gloucester Parts 1 & 2, 5R3156. The location of the subject site is shown in **Figure 1**. The Site's area is approximately 3.8 hectares (9.4 acres). The property is currently occupied by the Hindu Temple of Ottawa Carleton. The footprint of the existing temple building is approximately 1,060 m². The existing temple is located at the western extent of the Site with the associated septic systems to the north and south of the temple. Based on the previously prepared Use Permit, dated December 5, 1985, issued by the MOECC, and associated application, the existing septic systems were designed for 3,750 L/day, assuming occupancy of 250 individuals and the use of 15 L/day per individual.

The neighbouring land use is as follows:

- Bank street, followed by light industrial/commercial business to the west; and
- Vacant/treed land to the north, south and east.

The topography of the land is generally flat with an approximate elevation of 97 m above mean sea level.

These site features are shown in the **Figure 2**.

2 PROPOSED DEVELOPMENT

It is anticipated that an assembly hall be constructed at the eastern portion of the Site with the associated septic along the south of the proposed structure. Water supply will be obtained from municipal services. According to the City of Ottawa by-law requirements, and the proposed size of the new development and existing development (2,000 and 1,060 m², respectively), a total of 162 parking spaces will be required. No additional parking spaces are proposed as the current development on the Site is equipped with 173 parking spaces.

The proposed assembly hall is anticipated to include a dining area, a kitchen, lobby and two (2) halls. The proposed development will be equipped with a full basement as well.

The proposed development plan is shown in the **Figure 3**.

3 FIELDWORK

On May 8th, 2017, eight (8) test pits were advanced across the Site. The test pits were placed around the general perimeter of the Site so not to disrupt existing Site activities and services. The rationale for the test pits were to determine the general upper soil and perched water conditions. The test pits were advanced using a backhoe operated by a local contractor (Yelle Excavation, Ottawa) and under direct supervision by LRL field staff. The locations of the test pits are presented in **Figure 4** with the Test Pit Logs included in **Appendix A**.

An open tube piezometer was installed in select test pits locations (TP1, TP3, TP5 and TP7) to allow for the groundwater elevation measurement and sampling of the perched water found in the overburden, herein referred to as groundwater. Groundwater samples were collected from each of the piezometers on May 8th, 2017, with the exception to TP5 which was found to have insufficient water available for sampling (i.e. dry). The samples collected were submitted for laboratory analyses for select nitrate species parameters. The laboratory Certificate of Analysis is included in **Appendix B**.

A soil sample from select test pits was submitted to LRL's material testing laboratory for sieve and hydrometer analyses. The sieve and hydrometer analysis certificates are included in **Appendix C**.

A ground surface elevation survey was carried out at each test pit location to obtain the elevation of the test pit ground surface and the piezometer stick-up. These elevations would aid in determining the groundwater elevations across the Site. An arbitrary benchmark was established at the top of the east arm of the hydrant located along the west of the southern entrance to the Site. The benchmark was given an elevation of 100.00 m. The elevations of summarized in **Table 1** and are presented in the Test Pit logs included in **Appendix A**.

4 TOPOGRAPHY, GEOLOGY AND HYDROGEOLOGY

Local topography indicates that the inferred overburden groundwater flow direction is east towards the North Castor River. The nearest open water body to the Site is an unnamed tributary that flows into the North Castor River, approximately 1.1 km east of the Site.

Surficial soil deposit mapping² indicates that the overburden consists of till, plain with local relief less than 5 m. Bedrock mapping³ indicates that the underlying bedrock consists of dolomite and limestone, part of the Oxford Formation.

The test pits completed across the Site were found to have a thin layer of topsoil over fill material which extended to depths between 0.7 and 1.5 m thick. The fill was underlain with silty sand in TP1. The fill layer generally extended to bedrock refusal, encounter at depths from 0.8 to 2.1 m bgs. Waste debris was observed in the fill material in TP2, TP3 and TP5, which included metal, tire debris and asphalt.

A representative till sample collected during the test pitting activities (TP3-6) was submitted for sieve analysis. The till sample was measured to be 39% Silt & Clay, 40% Sand, and 21% Gravel. A second representative till sample collected (TP1-3) was submitted for hydrometer

² St-Onge, D.A. (compilation), 2009: Surficial geology, lower Ottawa valley, Ontario-Quebec; Geological Survey of Canada, Map 2140A, scale 1:125000

³ Harrison, J.E., 1976. Geological Survey of Canada, Generalized Bedrock Geology, Ottawa-Hull, Ontario and Quebec, Map 1508A, scale 1:125000.

analysis. The sample was measured to be 22% clay, 64% silt, 9% sand and 5% gravel. These results are presented in the sieve and hydrometer analysis certificates are included in **Appendix C** and are summarized in **Table 2**.

A search was conducted of the available well records from the MOECC Water Well Record Department. The search by UTM coordinates covered a 500 m radius from the site. The search returned records for twenty-three (23) wells. The well records are included in **Appendix D** and their locations are presented in **Figure 5**.

Review of the records of the wells within 500 m of the site retrieved revealed that the wells are drilled wells extending to depths between 8.2 and 67.1 m. The well records shows that that the geological conditions within 500 m are relatively similar, and consist generally of mixed till materials including sand, clay, gravel and boulders from 0 . 8.0 m. Unidentified soil conditions, %muck+and %soil+were found as overburden descriptions in a couple of well records, as noted in the table below. Bedrock conditions varied slightly between limestone, sandstone and occasionally shale. Bedrock starting depths also vary from 0.6 to 7.9 m.

The general subsurface conditions indicated in the well records within 500 m of the site are as follows:

MOE Well Number	Distance and Direction from Site (m)	Depth (m)	Overburden Details			Bedrock Details	Groundwater Encountered (m)	Static Water Level (m)	Type of water
			Sand/ Fill (m)	Clay/ Loam (m)	Gravel/ Till (m)				
1502181	210 N	14.0	--	--	0 . 6.4	6.4- 14.0 (Limestone)	14.0	2.4	Fresh
7112950	485 N	52.7	--	0 . 3.3	--	3.3 . 52.7 (Limestone)	51.5	4.7	Unspecified
1533566	385 N	67.1	0 . 2.1	--	--	2.1 . 29.8 (Sandstone)	65.8	4.8	Unspecified
						29.8 - 38.7 (Limestone)			
						38.7 - 67.1 (Sandstone)			
1531693	385 N	67.1	--	--	0 . 0.9	0.9 . 67.1 (Sandstone)	62.7	9.1	Fresh
1502249	370 N	25.9	0 . 1.2	--	--	1.2 . 25.9 (Sandstone)	25.2	4.5	Unspecified
1502248	330 N	29.9	0 . 0.3	0.3 . 1.8	--	1.8 . 29.9 (Sandstone)	24.3, 29.5	4.2	Fresh
1502246	335 N	24.4	--	--	0 . 1.5	1.5 . 24.4 (Sandstone)	9.1, 18.2, 30.1	1.5	Fresh
1517349	260 N	8.2	0 . 2.4	--	--	2.4 . 8.2 (Granite)	8.2	1.5	Fresh
1509925	215 N	19.2	--	--	0 . 3.9 %Boulders+	3.9 . 19.2 (Sandstone)	18.2	0.6	Fresh
1502175	360 NW	18.3	0 . 6.0	--	--	6.0 . 18.3 (Sandstone)	18.3	3.0	Fresh
1502176	250 NM	13.7	--	0 . 5.4	--	5.4 . 13.7 (Limestone)	13.7	1.8	Fresh



1502179	50 W	27.1	--	--	0 . 4.8	4.8- 7.62 (Limestone) 7.62 . 27.1 (Sandstone)	27.1	6.1	Fresh
1513436	100 SW	15.0	--	0 . 3.6 %soil+	3.6 . 4.8	4.8 . 15 (Limestone)	14.6	4.3	Fresh
1502180	140 S	16.8	--	0 . 1.8 %oam+	--	1.8 . 16.8 (Limestone)	16.8	1.8	Fresh
1502177	195 S	18.2	0 . 2.1	--	2.1 . 6.1	6.1 . 18.2 (Sandstone)	18.2	1.8	Fresh
1512375	230 S	22.5	0 . 2.7	--	--	2.7 . 22.5 (Sandstone)	22.5	3.6	Fresh
1512265	245 S	14.6	--	0 . 0.9	--	0.9 . 14.6 (Limestone)	2.4, 6.4, 10.3	1.2	Fresh
1514664	220 SW	15.2	--	--	0 . 3.9	3.9 . 9.1 (Shale) 9.1 . 38.1 (Limestone)	9.7, 16.7	6.1	Fresh
1516052	15 S	54.2	0 - 2.8	--	2.8 . 7.9	7.9 . 13.1 (Limestone) 13.1 . 54.4 (Sandstone)	53.3	9.1	Fresh
1502178	310 SW	15.2	--	--	0 . 5.4	5.4 . 15.2 (Limestone)	14.6	3.9	Fresh
1510717	400 S	15.8	0 . 1.8	--	--	1.8 . 15.8 (Limestone)	15.2	2.1	Fresh
1514840	370 S	41.1	0 . 0.9 %topsoil+	--	--	0.9 . 41.1 (Limestone)	32.0	6.0	Fresh
1502250	370 S	24.1	--	0 . 0.6 %oam+	--	0.6 . 19.8 (Sandstone) 19.8 . 24.0 (Granite)	18.2, 24.0	6.0	Fresh

4.1 Groundwater from Test Pits

Table 3 summarizes the water quality analysis from the test pit piezometers for nitrates, nitrites, ammonia and total kjeldahl nitrogen (TKN). The Laboratory Certificate of Analysis are included in **Appendix B**.

Nitrites were not detected (<0.05 mg/L) in any groundwater samples collected. Nitrate levels were found to be 0.5 mg/L in TP3 and <0.1 mg/L in both TP1 and TP7, below the ODWS of 10 mg/L. Ammonia was measured to be 0.28, 0.39 and 1.66 mg/L in TP1, TP3 and TP7, respectively. There are no set ODWS for ammonia.

TKN values were reported as 78.1, 65.3 and 131 mg/L, in TP1, TP3 and TP7, respectively. There are no set ODWS for TKN; however, based on the measured groundwater elevations which suggest that the overburden groundwater flow direction is to the north, TP7 can be considered a background location. This would indicate that the background levels of TKN are higher than those downgradient (TP1 and TP3). TP7 is located along the extents of one of the existing septic beds on the property, and additionally, during the advancement of TP7, organic material including a tree stump was encountered. Both of which (septic and other organic decomposition) could contribute to the elevated levels of TKN across the central portion of the Site.

5 REASONABLE USE OF GROUNDWATER

According to Guideline B-7, reasonable current and potential uses shall be established with respect to specific soil and water bearing units in the subsurface and would apply to all of the ground lying beneath a particular property. A decision on the reasonable use is based on the present use of groundwater in the vicinity and its potential use. The current and potential uses of the aquifers identified in Section 4 are identified below.

5.1 Surficial Sand/Clay/Till Groundwater

The surficial sand/clay/till groundwater is unlikely to be used as a water supply based on the following:

- The Site and the adjacent properties are currently serviced by municipal water although water well records were identified in the area.
- Based on the well records reviewed and the shallow overburden conditions, no shallow wells were identified on the subject site or adjacent lands. Generally, the overburden conditions are not suitable for construction of a well.
- The buildings in this area are serviced by private septic systems; therefore, the current use of the overburden groundwater is for the attenuation of the septic system effluent.

5.2 Bedrock/Till Aquifer

Twenty-three (23) well records were available for properties located within a 500 m radius of the Site. The records indicate that all twenty-three (23) wells tap into bedrock aquifer. Although it is our understanding that municipal water is available for the neighbouring properties, it is unknown at this time if these wells are still present or continue to be used for potable purposes.

6 TERRAIN ANALYSIS AND SEPTIC DESIGN

The terrain analysis was conducted to demonstrate that the unconsolidated material on the Site is appropriate for the construction of an on-site subsurface sewage disposal system, with consideration taken regarding the existing installation.

The subsurface conditions indicated for the Site are considered suitable for a Class IV sewage disposal system with a fully raised leaching bed depending on the lot specific soil and groundwater conditions at the actual location of the proposed septic system leaching bed. The leaching bed should be constructed to conform to the specifications set out in the Ontario Building Code (OBC). As part of this assessment, an analysis was carried out to ensure that sufficient space exists at the Site for the construction of a third septic system in accordance with the OBC which will service the proposed assembly hall.

As previously mentioned, the existing temple is serviced with two (2) sewage disposal systems located at the north and south sides of the buildings, respectively. Both are constructed with 9,000 L fibreglass septic tanks and 8 runs of 13.3 m in length piping. The existing septic systems were designed for a combined sewage flow of 3,750 L/day, assuming occupancy of 250 individuals and the use of 15 L/day per individual.



As a conservative approach to determining the expected largest septic system envelope required to service the proposed assembly hall, a septic system envelope size was calculated assuming a fully raised bed with mantle, a percolation rate of 12 min/cm for the imported sand required and a daily sewage flow of 21,600 L as calculated below.

The daily sewage flow was calculated assuming 850 persons visit the Site daily, of which approximately 600 individuals will occupy the proposed Assembly Hall. In accordance with Schedule 8 of the OBC, it is assumed that each individual which occupies the Site will discharge 36 L/day into the septic system. This is the set value for an Assembly Hall equipped with a kitchen facility. Both the existing and the proposed buildings are equipped, or will be equipped with a kitchen. As previously mentioned, the existing Temple is serviced with two (2) septic systems located at the north and south sides of the buildings, each constructed with 9,000 L fibreglass septic tanks and 8 runs of 13.3 m in length piping. The existing septic systems were designed for a combined sewage flow of 3,750 L/day, assuming occupancy of 250 individuals and the use of 15 L/day per individual.

The total length of pipe required for the proposed septic bed for the proposed Assembly Hall, assuming imported fill, was calculated as approximately 1,300 m:

$$L = QT/200$$

where L = length of pipe (m)

Q = daily sewage flow for the proposed assembly hall (L/day)

T = percolation rate of the imported sand fill material (min/cm)

Therefore an area of approximately 2,090 m² is required for the septic bed assuming 87 pipes each having a length of 15 m and a spacing of 1.6 m between the pipes. A mantle of 15 m in length would be required along the down gradient portion of the bed. Based on the total coverage of the septic bed (raised portion and mantle plus a replacement area) would be approximately 5,440 m².

The Site has a total area of 38,000 m². However, when the area of the proposed and existing buildings, septic systems and other site features (parking facility), are taken into consideration, an area of approximately 32,000 m² is available for the installation of a septic system in accordance with the OBC to service an assembly hall with a design sewage flow of up to 21,600 L/day. The proposed site development plan is shown in **Figure 3**.

6.1 Average Daily Water Demand Variance

It should be noted that the average daily water demand presented in the Site Servicing Report prepared by LRL, dated September 18, 2017 was calculated for the entire property using Section 7 of the OBC. The demand was calculated assuming a worst case scenario where all fixtures at the property, both the existing and the proposed buildings, are turned on simultaneously at the applicable flowrate for each fixture is specified in the OBC. The purpose of this calculation is to size the piping required to service the site.



7 PRIVATE SEWAGE DISPOSAL SYSTEM IMPACT STUDY

The bedrock (sandstone, limestone, granite) aquifer has the potential to be used as a potable water source and therefore must be protected from the septic system effluent. The attenuation method was used to determine the impact of the effluent on this aquifer. The groundwater impact assessment addresses the ability of the land to attenuate the sewage effluent created by the development. Three (3) methods for conducting the assessment are outlined in MOECC's *Procedure D-5-4 Technical Guideline for Individual On-Site Sewage Systems: Water Quality Impact Risk Assessment* (1996):

- *Lot Size Consideration* for lot greater than 10 000 m² (1 hectare);
- *System Isolation Consideration* for areas where the septic system is hydrogeologically isolated from the potable water source; and
- *Contaminate Attenuation Consideration* for sites that do not meet the above two points.

Based on the review of the available information and site visit (above), bedrock was encountered at depths less than 2.0 m across the Site, therefore the Site is considered hydrogeologically sensitive with areas of thin soil over bedrock.

As mentioned above, the lot size is 38,000 m² with approximately 32,000 m² available for the installation of the proposed septic system. The lot size consideration for lots greater than 10 000 m² does not apply since the system is considered a large sewage disposal system based on the anticipated sewage demand of 25,350 L/day, including the existing system. Therefore, **Contamination Attenuation** was considered in this terrain analysis. The critical contaminant identified was nitrates.

No bedrock outcrops were encountered on the Site at the time of the study. As mentioned above in Section 4, a minimum of between 0.8 and 2.1 m of overburden was encountered over the bedrock across the Site. The overburden material generally consisted of a fill material in the test pits with a stratum of till (TP3) or silty sand (TP1) above the bedrock in areas. The receiving groundwater for the septic system effluent is identified as the fill, silty sand and till. This groundwater is not considered an aquifer as it was encountered at depths less than 2.0 m below grade. As stated in Section 5.1, this groundwater is not a suitable supply aquifer for potable water based on its assumed poor yield, poor quality, shallow depth and likely used for the attenuation of the Site's existing and the neighbouring properties septic systems. This groundwater is considered a suitable attenuation zone because alternative sources of water are available (i.e. municipal water or bedrock aquifer).

The isolating feature between the receiving groundwater encountered in this overburden stratum and the bedrock aquifer is inferred to be the native till material. The well records within 500 m of the Site indicate that a minimum of 0.6 m of overburden is present. Due to the shallow depth to bedrock it is recommended that on-going monitoring be carried out to confirm the treated effluent nitrate levels as well as monitoring to ensure that reasonable use criteria of 2.5 mg/L for nitrates at the property boundaries are met. Further details of this recommendation are provided below in Section 10.

7.1 Contaminant Attenuation Method (Predictive Assessment)

The Contaminant Attenuation Method (Predictive Assessment) was used to determine the potential impact of the proposed on-Site septic systems at the boundary of the Site. This procedure assesses the risk that the individual on-site systems will cause the concentration of

the nitrate-nitrogen exceed 2.5 mg/L at the property boundaries. Dilution is the attenuation mechanism considered for nitrates, with precipitation being the only source of infiltration. The following parameters and assumptions were used in the nitrate attenuation calculations:

- The MOECC's Design Guidelines for Sewage Works, Chapter 22: Large Subsurface Disposal Systems, defines the infiltration factor as 250 mm;
- The average background nitrate concentration was calculated to be 0.17 mg/L based on the laboratory analytical results of the groundwater samples collected from TP1, TP3 and TP7;
- Impervious areas (existing and proposed) were calculated to be of 3,168 m² for the buildings and 5,975 m² of paved driveway and parking areas; and
- That further treatment, dosage and filtration systems will be required as part of the septic design to have a nitrate concentration of 2.5 mg/L for the sewage.

Based on the total proposed sewage volume for the entire Site of 25,350 L/day, the existing lot size, soil conditions, a nitrate concentration of the sewage of 40 mg/L, the calculated levels of nitrates at the property limits is estimated as 22.5 mg/L. presented in the attached **Table 4A**. This is above the Procedure B-7 Reasonable Use limit of 2.5 mg/L at the properties boundaries. Based on the *Contaminant Attenuation Method* the current lot size and soil conditions are not suitable to attenuate the nitrate impacts generated by the septic systems of the development in accordance with the Procedure B-7 requirements.

The above calculations are based on the current D-5-4 guideline which requires the use of 40 mg/L as the contaminant source as per Section 5.6.2 (a). A wastewater treatment system such as *Bionest* is an advanced tertiary treatment system which has reported reductions to the waste water effluent total nitrogen levels of 68.75%. These results were obtained through the BNQ 3680-910 certification for Bionest system models (SA-3 to SA-6). This particular system is approved by the OBC and the Building Materials Evaluation Commission of the Ontario Ministry of Municipal Affairs and Housing. Furthermore, Section 5.7 of the D-5-4 guideline states that the Ministry recognises that as research continues, information and technologies may become available which warrant minor or substantial revisions to this guideline.

It is our understanding that additional treatment and components to a *Bionest* systems (or equivalent), can reduce the nitrate level in sewage effluent from 40 mg/L to 2.5 mg/L (93.75% reduction). For the purpose of this report, we will refer to this as an advanced tertiary system. It is understood that as part of the sewage design for this Site, which is not LRL's mandate at this time, sufficient documentation will be provided to support this. Based on a treated nitrate level in the proposed system effluent of 2.5 mg/L, the proposed assembly hall the nitrogen levels at the property boundaries is reduced to 4.5 mg/L, which is still above the reasonable use limit of 2.5 mg/L. The calculations are presented in **Table 4B**.

The nitrates can be further reduced if a standard Bionest system is added to the two (2) existing septic systems with a reported reduction from 40 mg/L to 12.5 mg/L. This is in conjunction with advanced tertiary system as noted above with a reported reduction from 40 mg/L to 2.5 mg/L. Based on this configuration, the calculated nitrate level at the property boundary is 2.2 mg/L, below the Procedure B-7 Reasonable Use limit of 2.5 mg/L at the property limits. The calculations are presented in **Table 4C**.

8 DISCUSSION

Based on a review of geological maps and MOECC well record database and conducting a test pitting program, two (2) aquifers/groundwater zones were identified: within the overburden (fill,

sand, till) and within the bedrock (limestone, sandstone, granite) at depths ranging from 8.2 to 65.8 m in depth.

According to Guideline B-7, reasonable current and potential uses shall be established with respect to specific soil and water bearing units in the subsurface and would apply to all of the ground lying beneath a particular property. Even though the site is service by municipal water, the bedrock aquifer's potential use as a potable water source must be considered. The overburden groundwater stratum was not considered a potential source of potable water since it is too shallow for the proper construction of a dug well and it is currently used to attenuate the septic system effluent the Site, and likely neighbouring properties. The bedrock aquifer has the potential to be used as a water supply source based on the well records reviewed.

Since the bedrock aquifer has the potential to be used as a potable water source it must be protected from the septic system effluent. The system contamination attenuation method was used to determine the impact of the effluent on this aquifer. The isolating feature, and attenuation zone, between the receiving groundwater stratum and the bedrock aquifer is the overburden (fill, till, clay) which separates them. This layer is found to have a thickness of at least 0.6 m.

Based on the hydrogeological assessment sensitive conditions are present on the site due to thin overburden. The overburden generally consists of fill to bedrock, with till or silty sand observed at two (2) of the test pits. Due to the thin soils and sensitive site conditions it is recommended that the leaching bed of the proposed system be fully raised and an appropriate groundwater monitoring program be implemented. As a precautionary measure, the on-going monitoring program should include the bedrock aquifer.

9 CONCLUSION

Based on our review of available information and the results of the groundwater sampling and laboratory analytical programs, we conclude the following:

1. Sufficient area exists on the property for the installation of a septic system in accordance with the OBC to service the proposed Assembly Hall with a design sewage flow of up to 21,600 L/day.
2. In accordance with the D-5-4 guideline, without pre-treatment the lot area of the Site is not of sufficient size to attenuate the impacts of the proposed septic system based on the **%Contaminant Attenuation Method** using 40 mg/L as the contaminant source as per Section 7.
3. Pre-treatment of the sewage from the existing sewage disposal systems with a Bionest system model (SA-3 to SA-6) certified treatment system, which has a documented and measured output of 12.5 mg/L and pre-treatment of the proposed system with an augmented Bionest system with a reported output of 2.5 mg/L yields a calculated nitrate concentration at the property line of 2.2 mg/L, based on the **"Contaminant Attenuation Method"**.
4. Records of domestic wells were retrieved within 500 m of the site. The potable water source of these wells is the bedrock aquifer. A thin layer of either clay, gravel or till, with some sand in areas, being between 0.9 and 7.6 m thick over bedrock.



10 RECOMMENDATIONS

1. Pre-consultation with the Ontario Ministry of the Environment and Climate Change is recommended at the planning stage of this project with respect to the proposed septic design and treatment options.
2. It is recommended that a plan showing which properties within 500 m of the Site are serviced with municipal water are obtained from the City of Ottawa. If properties without municipal services are identified, it is recommended that the property owners be contacted to confirm if they are supplied by a private well and if so, obtain details regarding their well, where possible. It is recommended that the findings be provided at the time of an Application for Site Plan Control.
3. The daily volume, calculated using the 36 L/day per individual value set out in the OBC, is in excess of 10,000 L. Therefore, it is recommended that an application for an Environmental Compliance Approval be submitted to the MOECC at the time of an Application for Site Plan Control. Note that pre-consultation with the MOECC is required prior to submission.
4. The septic system should be placed at least 15 m from any drilled wells/water service and 30 m from any dug well. It is recommended that the water table be surveyed prior to installation. The 20 m setback from the normal high water mark of the identified stream east of the proposed development footprint.
5. The treatment system required for septic should be designed accordingly and monitored/maintained.
6. It is recommended that groundwater monitoring wells be installed in compliance with *O. Reg. 903: Wells* to aid in the interpretation of groundwater flow direction and monitoring potential impacts to the identified supply aquifers. The findings would be submitted at the time of the Application for Site Plan Control.
7. It is recommended that a geodetic benchmark be used for further investigations on the site, including the proposed monitoring wells and groundwater elevations.
8. Due to the thin soils and sensitive site conditions it is recommended that the leaching bed of the proposed system be fully raised and an appropriate groundwater monitoring program be implemented.

11 LIMITATIONS

The findings contained in this report are based on data and information collected during the Terrain Analysis of the subject property conducted by LRL Associates Ltd. The conclusions and recommendations are based solely on site conditions encountered at the time of our fieldwork on May 8th, 2017, supplemented by historical information and data obtained as described in this report. The information presented in this report represents the groundwater conditions at the locations sampled. Due to natural variations in geological conditions, no inference is made to the soil or groundwater conditions between sampling points. No assurance is made regarding changes in conditions subsequent to the time of this investigation. If additional information is discovered or obtained, LRL Associates Ltd. should be requested to re-evaluate the conclusions presented in this report and to provide amendments as required.

In evaluating the subject property, LRL Associates Ltd. has relied in good faith on information provided by individuals as noted in this report. We assume that the information provided is

factual and accurate. We accept no responsibility for any deficiencies, misstatements or inaccuracies contained in this report as a result of omissions, misinterpretation or fraudulent acts of the persons contacted.

Yours truly,
LRL Associates Ltd.



Jessica Arthurs
Senior Environmental Technician



Matthew Whitney, P. Eng.



Encl.

- Figure 1 – Site Location
- Figure 2 – Site Plan
- Figure 3 – Proposed Development Plan
- Figure 4 – Test Pit Locations, Groundwater Elevations and Groundwater Contours
- Figure 5 – Well Locations, Ontario Well Records
- Table 1 – Summary of Groundwater Elevations in Test Pits
- Table 2 – Summary of Sieve & Hydrometer Analyses
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FIGURES



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PROJECT

TERRAIN ANALYSIS AND PRIVATE SEWAGE DISPOSAL
SYSTEM IMPACT STUDY
PROPOSED ASSEMBLY HALL
4835 BANK STREET, OTTAWA, ONTARIO

DRAWING TITLE

SITE LOCATION
(NOT TO SCALE)
SOURCE: GEOOTTAWA

CLIENT

THE HINDU TEMPLE OF OTTAWA CARLTON

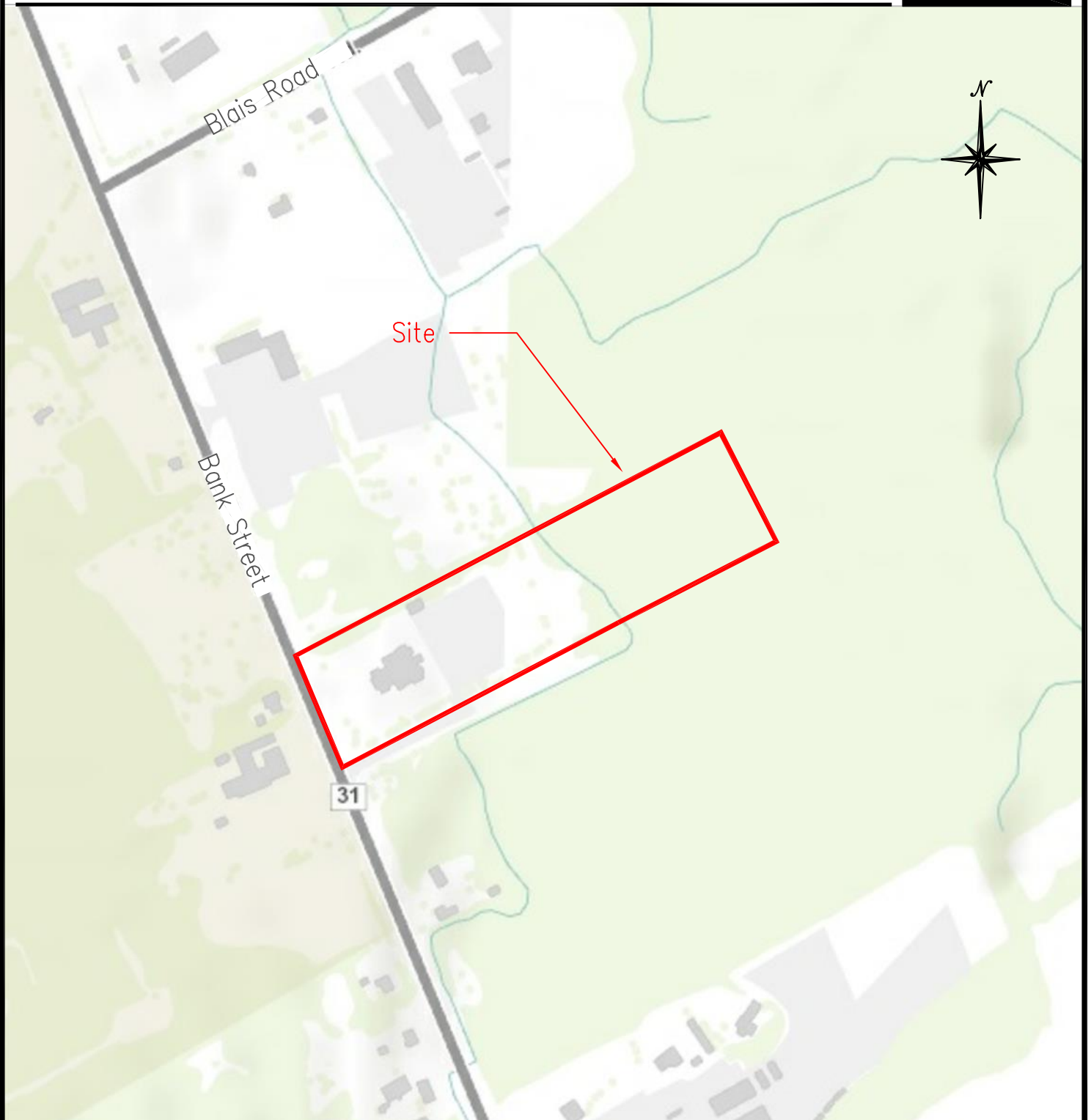
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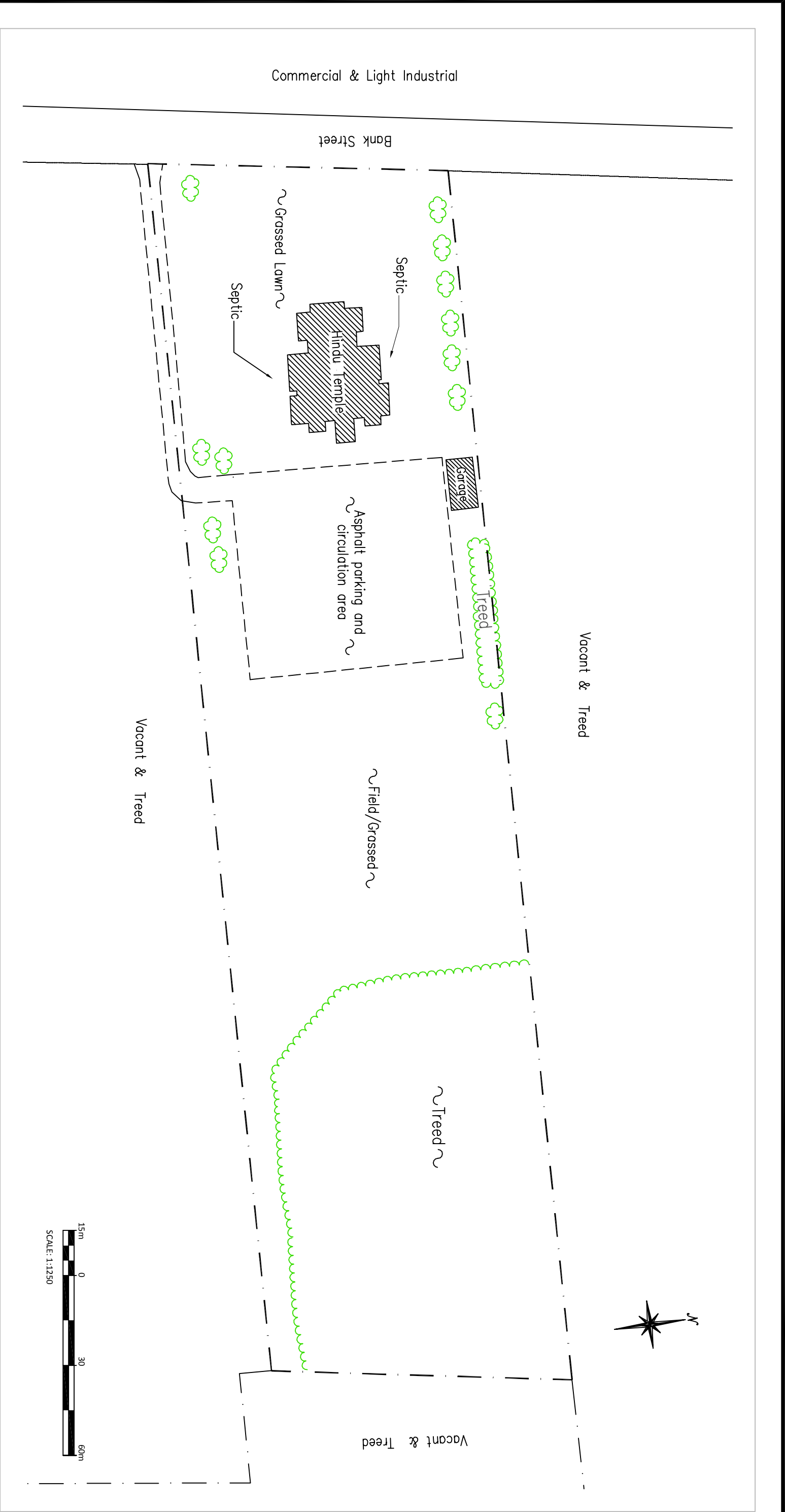
JANUARY 2018

PROJECT

170132

FIGURE1






LEGEND

- Existing Building
- Division between various surface materials
- Property Line
- Tree Line
- Tree

01	ISSUED FOR REVIEW	A.S	05/18/17
No.	REVISIONS	BY	DATE



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CLIENT

THE HINDU TEMPLE OF OTTAWA

CARLTON

DESIGNED BY:	DRAWN BY:	APPROVED BY:
--	A.S	M.W


PROJECT

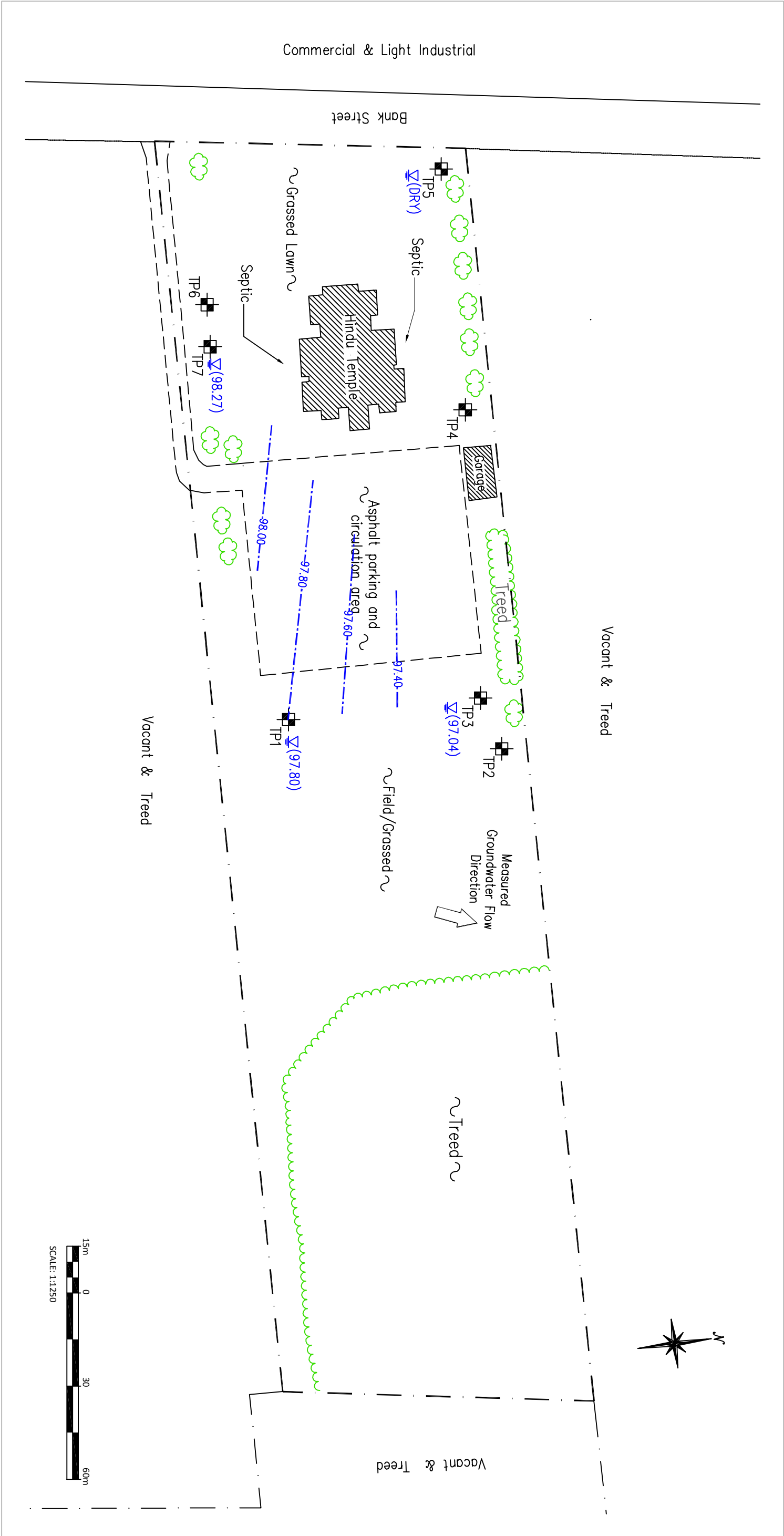
TERRAIN ANALYSIS AND PRIVATE SEWAGE DISPOSAL SYSTEM IMPACT STUDY - PROPOSED ASSEMBLY HALL 4835 BANK STREET, OTTAWA, ONTARIO

DRAWING TITLE

SITE PLAN

PROJECT NO.	170132
DATE	JANUARY 2018





LEGEND

Existing Building

Division between various surface materials

Property Line

Tree Line

Tree

Test Pit (May 2017)

Groundwater Elevation (May 8, 2017)

Groundwater Contour Line

TP99

$\nabla(99.99)$

01

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CARLTON

DESIGNED BY:

DRAWN BY:

APPROVED BY:

PROJECT

A.S

M.W

TERRAIN ANALYSIS AND PRIVATE SEWAGE DISPOSAL SYSTEM IMPACT STUDY - PROPOSED ASSEMBLY HALL 4835 BANK STREET, OTTAWA, ONTARIO

DRAWING TITLE

TEST PIT LOCATIONS, GROUNDWATER ELEVATIONS AND GROUNDWATER CONTOUR LINES

PROJECT NO.

170132

DATE

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FIGURE4



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PROJECT

TERRAIN ANALYSIS AND PRIVATE SEWAGE DISPOSAL
SYSTEM IMPACT STUDY
PROPOSED ASSEMBLY HALL
4835 BANK STREET, OTTAWA, ONTARIO

DRAWING TITLE

WELL LOCATIONS
ONTARIO WELL RECORDS WITHIN 500 M OF THE SITE
(NOT TO SCALE)

CLIENT

THE HINDU TEMPLE OF OTTAWA CARLTON

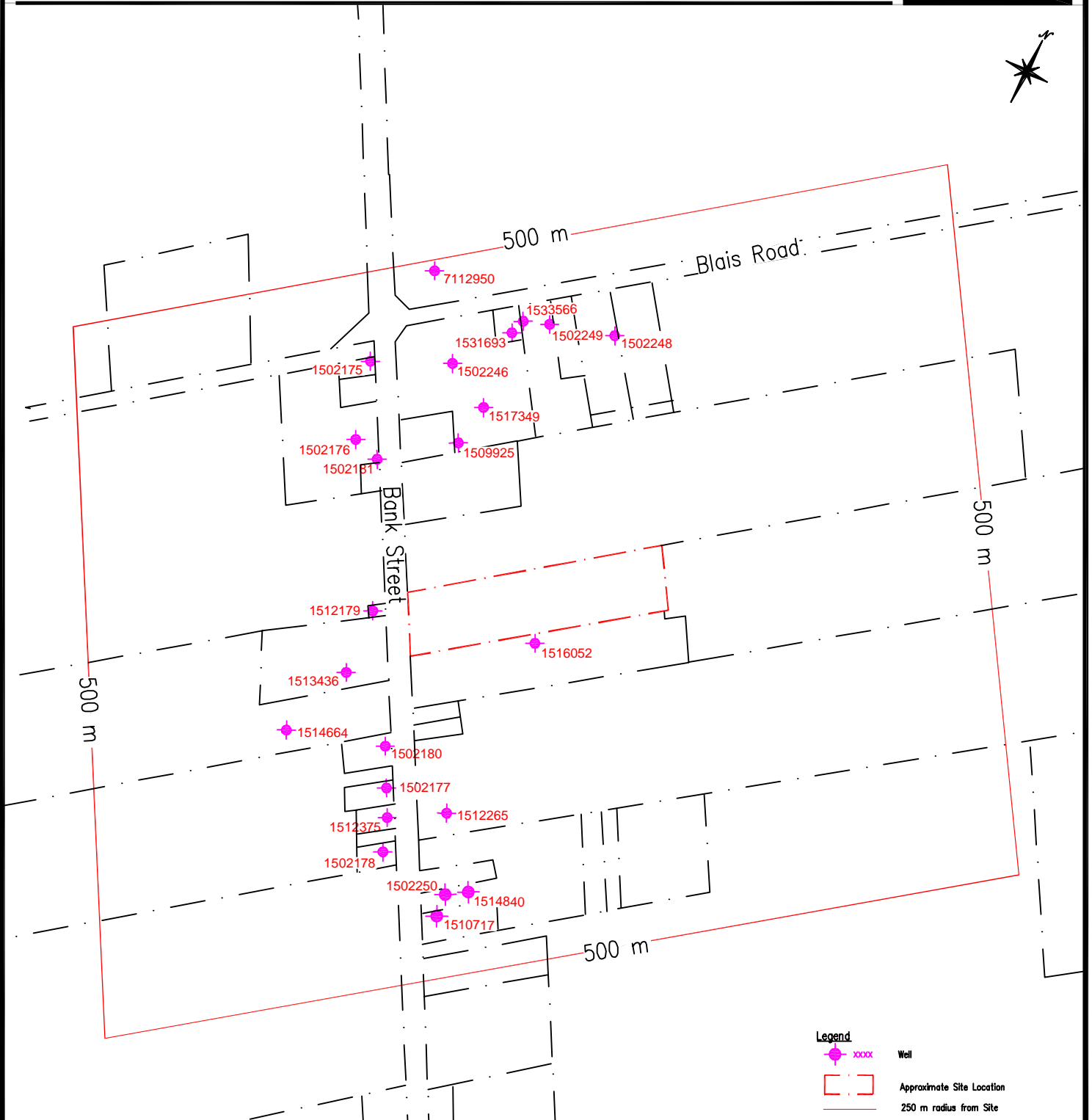
DATE

JANUARY 2018

PROJECT

170132

FIGURE 5



TABLES

Table 1
Summary of Groundwater Elevations in Test Pits

Terrain Analysis and Private Sewage Disposal System Impact Study - Proposed Assembly Hall
4835 Bank Street, Ottawa, Ontario
LRL File: 170132

Test Pit	Ground Surface Elevation ¹ (m)	Reference Elevation ² (m)	Depth To Water Table (m)		Groundwater Elevation (m)
			Reference Point	Ground Surface	
TP1	98.21	99.15	1.35	0.41	97.80
TP2	97.09	--	--	--	--
TP3	97.75	98.98	1.94	0.71	97.04
TP4	99.54	--	--	--	--
TP5	98.78	99.02	DRY	--	--
TP6	99.38	--	--	--	--
TP7	99.60	100.79	2.52	1.33	98.27

NOTES

¹ Elevations are based off of a temporary benchmark established at the top of the east arm of the fire hydrant along the south of the Site (100.00 m).

² Reference elevation is top of piezometer.

Table 2
Summary of Sieve & Hydrometer Analyses
Terrain Analysis and Private Sewage Disposal System Impact Study - Proposed Assembly Hall
4835 Bank Street, Ottawa, Ontario
LRL File: 170132

Sample		Sample Gradation ¹						
		Percent Particles in Each Fraction						
		Gravel	Sand			Silt	Clay	Soil Texture Classification
			Coarse	Medium	Fine			
Depth (m)	>4.75 mm	2.0 - 4.75 mm	425 µm - 2.0 mm	75 - 425 µm	2 - 75 µm	< 2µm		
TP1-3	1.8 - 2.0	4.8	1.2	1.8	6.5	63.8	22.0	Silt Loam
TP3-6	1.4 - 1.6	21.3	7.0	12.7	20.1	39.0		Fine Silty Sand

NOTES:

¹ Unified Soil Classification System

Table 3
Summary of analysis of water samples collected from the test pits.
Terrain Analysis and Private Sewage Disposal System Impact Study - Proposed Assembly Hall
4835 Bank Street, Ottawa, Ontario
LRL File: 170132

Parameter	Units	MRL	Ontario Drinking Water Standards		Sample		
			Standard	Type	TP1	TP3	TP7
Sample Date (d/m/y)					05/08/2017	05/08/2017	05/08/2017
Ammonia	mg/L	0.01			0.28	0.39	1.66
Total Kjeldahl Nitrogen	mg/L	0.1			78.1	65.3	131
Nitrate as N	mg/L	0.1	10	MAC	<0.1	0.5	<0.1
Nitrite as N	mg/L	0.05	1	MAC	<0.05	<0.05	<0.05

NOTES

MAC Maximum Acceptable Concentration

MRL Minimum Reportable Limit

Table 4A
Nitrate Attenuation Calculations (Standard Disposal System)

Terrain Analysis - Proposed Assembly Hall
4835 Bank Street, Ottawa, Ontario
LRL File: 170132

1. Area Available for Infiltration

Number of Lots	n	1
Approximate footprint of existing temple	H	1068 m ²
Approximate footprint of existing garage	H	100 m ²
Approximate footprint of proposed assembly hall	H	2000 m ²
Approximate area of paved driveways and parking (proposed and existing)	d ¹	5975 m ²
Approximate Length of Road	L	0 m
Approximate Width of Road	w	0 m
Total Area of Property		38000 m ²
Impervious Area		9143.0 m ²
Roads	I x w	0 m ²
Driveway	n x d	5975 m ²
Building	n x H	3168 m ²
Area available Infiltration	A	28,857 m²

2. Nitrate Dilution Calculations

Nitrate Concentration of Infiltration	C _i	0.0 mg/L ⁶
Site Infiltration	Q _i = A*PI	7214 m ³
Daily Sewage Volume - Existing	Q _{d,1}	3.75 m ³
Maximum Yearly Sewage Volume - Existing (water)	Q _{e,1} = 365*n*Q _d	1369 m ³
Nitrate Concentration in Sewage - Existing	C _{e,1}	40 mg/L
Daily Sewage Volume - Proposed New Development	Q _{d,2}	21.6 m ³
Maximum Yearly Sewage Volume (water) - Proposed New Development	Q _{e,2} = 365*n*Q _d	7884 m ³
Nitrate Concentration in Sewage - Proposed New Development	C _{e,2}	40 mg/L
Maximum Allowable Nitrate Concentration at Boundary	C _m	2.5 mg/L
Increase in Nitrate Concentration at Boundaries	C = (Q _{e,1} C _{e,1} + Q _{e,2} C _{e,2} + Q _i C _i) / (Q _{e,1} + Q _{e,2} + Q _i)	22.5 mg/L

NOTES

- Table 2: Infiltration Factors, *Hydrological Technical Information Requirements for Land Development Applications*, Ministry of the Energy and Environment, April 1995.
- Thornthwaite and Mather² (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance.
- Moisture surplus for data for Mason Anger (Environment Canada Meteorological Service of Canada, 2010).
- Area based on proposed development plan
- As per *Technical Guideline for Individual On-Site Sewage Systems: Water Quality and Impact Risk Assessment*, Ministry of the Energy and Environment, August 1996.
- Average of nitrate concentrations from test pits water sample collected on May 8, 2017

Table 4B
Nitrate Attenuation Calculations (Tertiary Disposal System)

Terrain Analysis - Proposed Assembly Hall
4835 Bank Street, Ottawa, Ontario
LRL File: 170132

1. Area Available for Infiltration

Number of Lots	n	1
Approximate footprint of existing temple	H	1068 m ²
Approximate footprint of existing garage	H	100 m ²
Approximate footprint of proposed assembly hall	H	2000 m ²
Approximate area of paved driveways and parking (proposed and existing)	d ¹	5975 m ²
Approximate Length of Road	L	0 m
Approximate Width of Road	w	0 m
Total Area of Property		38000 m ²
Impervious Area		9143.0 m ²
Roads	I x w	0 m ²
Driveway	n x d	5975 m ²
Building	n x H	3168 m ²
Area available Infiltration	A	28,857 m²

2. Nitrate Dilution Calculations

Nitrate Concentration of Infiltration	C _i	0.0 mg/L ⁶
Site Infiltration	Q _i = A*PI	7214 m ³
Daily Sewage Volume - Existing	Q _{d,1}	3.75 m ³
Maximum Yearly Sewage Volume - Existing (water)	Q _{e,1} = 365*n*Q _d	1369 m ³
Nitrate Concentration in Sewage - Existing	C _{e,1}	40 mg/L
Daily Sewage Volume - Proposed New Development	Q _{d,2}	21.6 m ³
Maximum Yearly Sewage Volume (water) - Proposed New Development	Q _{e,2} = 365*n*Q _d	7884 m ³
Nitrate Concentration in Sewage - Proposed New Development	C _{e,2}	2.5 mg/L
Maximum Allowable Nitrate Concentration at Boundary	C _m	2.5 mg/L
Increase in Nitrate Concentration at Boundaries	C = (Q _{e,1} C _{e,1} + Q _{e,2} C _{e,2} + Q _i C _i) / (Q _{e,1} + Q _{e,2} + Q _i)	4.5 mg/L

NOTES

- Table 2: Infiltration Factors, *Hydrological Technical Information Requirements for Land Development Applications*, Ministry of the Energy and Environment, April 1995.
- Thornthwaite and Mather² (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance.
- Moisture surplus for data for Mason Anger (Environment Canada Meteorological Service of Canada, 2010).
- Area based on proposed development plan
- As per *Technical Guideline for Individual On-Site Sewage Systems: Water Quality and Impact Risk Assessment*, Ministry of the Energy and Environment, August 1996.
- Average of nitrate concentrations from test pits water sample collected on May 8, 2017

Table 4C
Nitrate Attenuation Calculations (Advanced Tertiary System)

Terrain Analysis - Proposed Assembly Hall
4835 Bank Street, Ottawa, Ontario
LRL File: 170132

1. Area Available for Infiltration

Number of Lots	n	1
Approximate footprint of existing temple	H	1068 m ²
Approximate footprint of existing garage	H	100 m ²
Approximate footprint of proposed assembly hall	H	2000 m ²
Approximate area of paved driveways and parking (proposed and existing)	d ^v	5975 m ²
Approximate Length of Road	L	0 m
Approximate Width of Road	w	0 m
Total Area of Property		38000 m ²
Impervious Area		9143.0 m ²
Roads	l x w	0 m ²
Driveway	n x d	5975 m ²
Building	n x H	3168 m ²
Area available Infiltration	A	28,857 m²

2. Nitrate Dilution Calculations

Nitrate Concentration of Infiltration	C _i	0.0 mg/L ⁶
Site Infiltration	Q _i = A*PI	7214 m ³
Daily Sewage Volume - Existing	Q _{d,1}	3.75 m ³
Maximum Yearly Sewage Volume - Existing (water)	Q _{e,1} = 365*n*Q _d	1369 m ³
Nitrate Concentration in Sewage - Existing	C _{e,1}	12.5 mg/L
Daily Sewage Volume - Proposed New Development	Q _{d,2}	21.6 m ³
Maximum Yearly Sewage Volume (water) - Proposed New Development	Q _{e,2} = 365*n*Q _d	7884 m ³
Nitrate Concentration in Sewage - Proposed New Development	C _{e,2}	2.5 mg/L
Maximum Allowable Nitrate Concentration at Boundary	C _m	2.5 mg/L
Increase in Nitrate Concentration at Boundaries	C = (Q _{e,1} C _{e,1} + Q _{e,2} C _{e,2} + Q _i C _i) / (Q _{e,1} + Q _{e,2} + Q _i)	2.2 mg/L

NOTES

- Table 2: Infiltration Factors, *Hydrological Technical Information Requirements for Land Development Applications*, Ministry of the Energy and Environment, April 1995.
- Thornthwaite and Mather⁵ (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance.
- Moisture surplus for data for Mason Anger (Environment Canada Meteorological Service of Canada, 2010).
- Area based on proposed development plan
- As per *Technical Guideline for Individual On-Site Sewage Systems: Water Quality and Impact Risk Assessment*, Ministry of the Energy and Environment, August 1996.
- Average of nitrate concentrations from test pits water sample collected on May 8, 2017

APPENDIX A
Test Pit Logs



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe



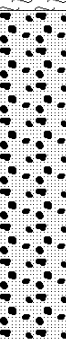



Test Pit Log: TP1

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

SUBSURFACE PROFILE		SAMPLE DATA					Water Level (Standpipe or Open Excavation)
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)	Water Content (%) Liquid Limit (%)	
0 ft m 0	Ground Surface	98.21					 0.4 m bgs (08/05/17)
	TOPSOIL Sandy, dark brown, dry.	0.00					
		98.01					
1	FILL Sandy clay, dark brown, dry.	0.20					
2							
3	Silty Sand Trace clay, with clay seam from 1.7 to 1.8 m bgs, brown, dry.	97.31		1			
1	Sieve analysis completed.	0.90					
4							
5				2			
6							
7	End of Test Pit Refusal over inferred bedrock.	96.11		3			
2		2.10					
8							

Easting: N/M

Northing: N/M

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 98.21

Top of Riser Elev.: 99.15

Excavation Width: 1.2 m

Excavation Length: 1.5 m

NOTES:

BGS- Below Ground Surface



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

Test Pit Log: TP2

SUBSURFACE PROFILE		SAMPLE DATA					Water Level (Standpipe or Open Excavation)
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)	Water Content (%) Liquid Limit (%)	
0 ft m 0	Ground Surface	97.09 0.00					
	FILL Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs.						
1	Buried metal structure/waste at approximately 0.9 m bgs.						
2							
3							
3 ft m 1	End of Test Pit	96.19 0.90		4			
4							
5							
6							
7							
8							

Easting: N/M

Northing: N/M

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 97.09

Top of Riser Elev.: --

Excavation Width: 1.2 m

Excavation Length: 1.5 m

NOTES:

Test pit terminated at 0.9 meters due to volume of water in pit.
BGS- Below Ground Surface



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Test Pit Log: TP3

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

SUBSURFACE PROFILE			SAMPLE DATA					Water Content			Water Level (Standpipe or Open Excavation)
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)	Liquid Limit (%)					
						25	50	75			
ft m					50	150					
0	Ground Surface	97.75									
	TOPSOIL Sandy loam, dark brown, dry.	0.00									
	Brick debris found in top 0.2 m bgs.	97.55									
	FILL Sandy silt, trace boulders, brown, dry.	0.20									
1	Tire debris found at approximately 0.8 m bgs.			5							
2											
	TILL Silty sand, trace gravel, cobbles and boulders, brown, dry.	96.95									
3	Sieve analysis completed.	0.80									
4											
5				6							
	End of Test Pit	96.05									
6	Refusal at 1.7 m bgs over inferred bedrock.	1.70									
7											
8											

0.71 m bgs (08/05/17)

Easting: 0454091

Northing: 5017670

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 97.75

Top of Riser Elev.: 98.98

Excavation Width: 1.2 m

Excavation Length: 1.5 m

NOTES:

BGS- Below Ground Surface



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

Test Pit Log: TP4

SUBSURFACE PROFILE		SAMPLE DATA					Water Level (Standpipe or Open Excavation)
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)	Water Content (%) Liquid Limit (%)	
0	Ground Surface	99.54					
0	TOPSOIL Silty loam, trace clay, dark brown, dry.	0.00					
1							
2	FILL Silty sand, trace cobbles and gravel, light brown, dry. Changing to dark brown sandy fill with trace boulders at approximately 0.8 m bgs.	99.04 0.50		7			
3							
4				8			
5	End of Test Pit Refusal at 1.4 m bgs over inferred bedrock or large concrete structure.	98.14 1.40					
6							
7							
8							

Easting: 0454005

Northing: 5017628

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 99.54

Top of Riser Elev.: --

Excavation Width: N/M

Excavation Length: N/M

NOTES:

BGS- Below Ground Surface



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Test Pit Log: TP5

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

SUBSURFACE PROFILE		SAMPLE DATA					Water Level (Standpipe or Open Excavation)
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)	Water Content (%) Liquid Limit (%)	
0 ft 0 m	Ground Surface	98.78					<p>▼ Dry at 1.53 m bgs</p>
	TOPSOIL Silty loam some sand, dark brown, dry.	0.00					
		98.63		10			
	FILL Sand, some silt, trace cobbles, brown, dry.	0.15					
	Waste debris of metal and asphalt pieces at approximately 0.9 m bgs.						
1				9			
2							
3							
4							
5				11			
6							
7							
8							
	End of Test Pit Refusal at 1.5 m bgs over inferred bedrock.	97.28 1.50					

Easting: 0453945

Northing: 5017595

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 98.78

Top of Riser Elev.: 99.02

Excavation Width: N/M

Excavation Length: N/M

NOTES:

BGS- Below Ground Surface



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Test Pit Log: TP6

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

SUBSURFACE PROFILE			SAMPLE DATA							Water Content			Water Level (Standpipe or Open Excavation)		
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)				Liquid Limit (%)						
									25	50	75				
0	Ground Surface	99.38													
0	TOPSOIL Sandy loam, dark brown, dry.	0.00													
		99.23													
1	FILL Sand, some gravel, cobbles, boulders, silty seam at 0.7 m bgs, brown, dry.	0.15													
	Refusal at 0.8 m bgs over inferred bedrock.														
2				12											
		98.58		13											
	End of Test Pit	0.80													
3															
4															
5															
6															
7															
8															

Easting: 0454003

Northing: 5017542

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 99.38

Top of Riser Elev.: --

Excavation Width: N/M

Excavation Length: N/M

NOTES:

BGS- Below Ground Surface



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Test Pit Log: TP7

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation Ltd.

SUBSURFACE PROFILE		SAMPLE DATA					Water Level (Standpipe or Open Excavation)
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa)	Water Content (%) Liquid Limit (%)	
0 ft m 0	Ground Surface	99.60					
	TOPSOIL Sandy loam, dark brown, dry.	0.00					
1	FILL Sand, brown, trace metal debris, dry.	99.40 0.20					
2							
3	TILL Silty sand, trace clay, boulders, grey, organics including tree stump, roots, black Refusal due to obstruction (tree stump).	98.90 0.70					
4							
5							
6							
7							
8							
	End of Test Pit	97.80 1.80					

1.33 m bgs (08/05/17)

Easting: 0454051

Northing: 5017564

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

Groundsurface Elevation: 99.60

Top of Riser Elev.: 100.79

Excavation Width: N/M

Excavation Length: N/M

NOTES:

BGS- Below Ground Surface



Symbols and Terms Used on Borehole and Test Pit Logs

The following explains the data presented in the borehole and test pit logs.

1. Soil Description

The soil descriptions presented in this report are based on commonly accepted methods of classification and identification employed in geotechnical practice. Classification and identification of soil involves some judgement and LRL Associates Ltd. does not guarantee descriptions as exact, but infers accuracy to the extent that is common in current geotechnical practice. Boundaries between zones on the logs are often not distinct but transitional and were interpreted.

a. Proportion

The proportion of each constituent part, as defined by the grain size distribution, is denoted by the following terms:

Term	Proportions
"trace"	1% to 10%
"some"	10% to 20%
prefix (i.e. "sandy" silt)	20% to 35%
"and" (i.e. sand "and" gravel)	35% to 50%

b. Compactness and Consistency

The state of compactness of granular soils is defined on the basis of the Standard Penetration Test. See Section 2c for more details. The consistency of clayey or cohesive soils is based on the shear strength of the soil, as determined by field vane tests and by a visual and tactile assessment of the soil strength.

The state of compactness of granular soils is defined by the following terms:

State of Compactness Granular Soils	Standard Penetration Number "N"
Very loose	0 – 4
Loose	4 – 10
Compact or medium	10 - 30
Dense	30 - 50
Very dense	over - 50

The consistency of cohesive soils is defined by the following terms:

Consistency Cohesive Soils	Undrained Shear Strength (Cu) (kPa)
Very soft	under 10
Soft	10 - 25
Medium or firm	25 - 50
Stiff	50 - 100
Very stiff	100 - 200
Hard	over - 200

2. Sample Data

a. Elevation depth

This is a reference to the geodesic elevation of the soil or to a benchmark of an arbitrary elevation at the location of the borehole or test pit. The depth of geological boundaries is measured from ground surface.

b. Type

Symbol	Type	Letter Code
	Auger	AU
	Split spoon	SS
	Shelby tube	ST
	Rock Core	RC

c. Sample Number

Each sample taken from the borehole is numbered in the field as shown in this column.

LETTER CODE (as above) – Sample Number

d. Blows (N) or RQD

This column indicates the Standard Penetration Number (N) as per ASTM D-1586. This is used to determine the state of compactness of the soil sampled. It corresponds to the number of blows



required to drive 300 mm of the split spoon sampler using a $622 \text{ kg}\cdot\text{m/s}^2$ hammer falling freely from a height of 760 mm. For a 600 mm long split spoon, the blow counts are recorded for every 150 mm. The "N" index is obtained by adding the number of blows from the 2nd and 3rd count. Technical refusal indicates a number of blows greater than 50.

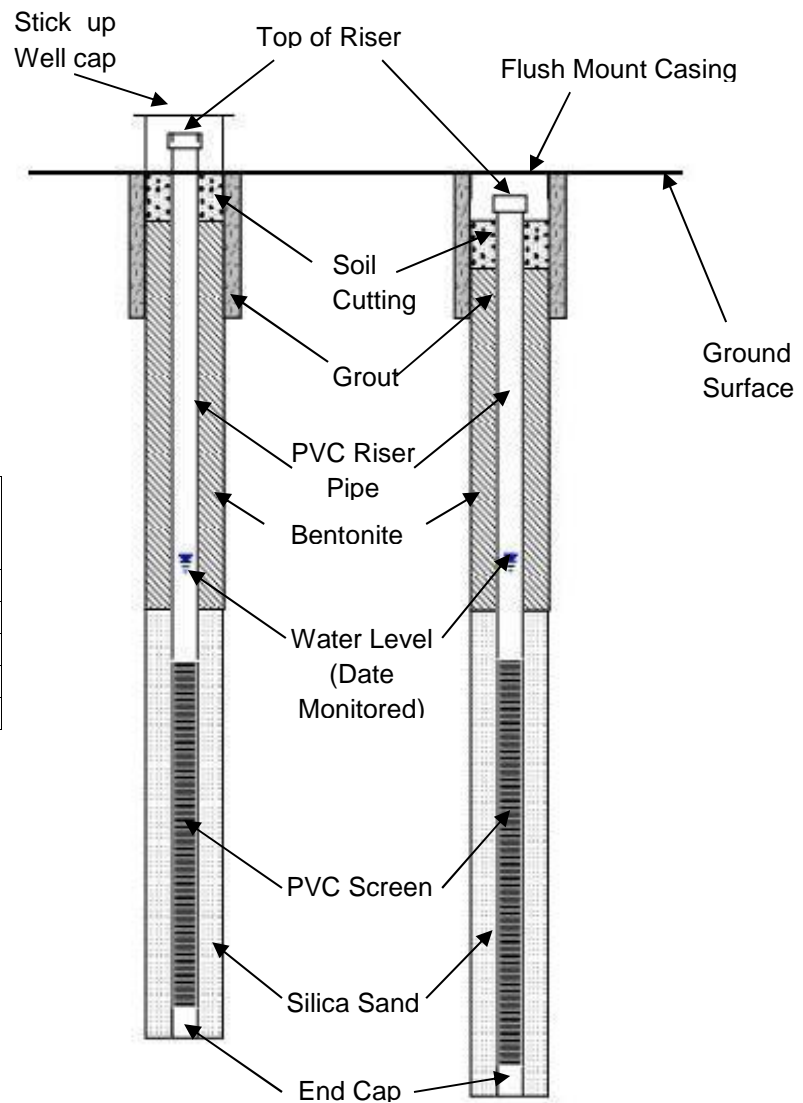
In the case of rock, this column presents the Rock Quality Designation (RQD). The RQD is calculated as the cumulative length of rock pieces recovered having lengths of 10 cm or more divided by the length of coring. The qualitative description of the bedrock based on RQD is given below.

Rock Quality Designation (RQD) (%)	Description of Rock Quality
0 – 25	very poor
25 – 50	poor
50 – 75	fair
75 – 90	good
90 – 100	excellent

e. Recovery (%)

For soil samples this is the percentage of the recovered sample obtained versus the length sampled. In the case of rock, the percentage is the length of rock core recovered compared to the length of the drill run.

3. General Monitoring Well Data



APPENDIX B
Laboratory Certificates of Analysis

Certificate of Analysis

LRL Associates Ltd.

5430 Canotek Road
Ottawa, ON K1J 9G2
Attn: Jessica Arthurs

Client PO:
Project: 170132
Custody: 32310

Report Date: 15-May-2017
Order Date: 11-May-2017

Order #: 1719377

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1719377-01	TP1
1719377-02	TP3
1719377-03	TP7

Approved By:



Dale Robertson, BSc
Laboratory Director

Certificate of Analysis
Client: LRL Associates Ltd.
Client PO:

Report Date: 15-May-2017

Order Date: 11-May-2017

Project Description: 170132

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Ammonia, as N	EPA 351.2 - Auto Colour	12-May-17	12-May-17
Anions	EPA 300.1 - IC	12-May-17	12-May-17
Total Kjeldahl Nitrogen	EPA 351.2 - Auto Colour, digestion	12-May-17	15-May-17

Certificate of Analysis
Client: LRL Associates Ltd.
Client PO:

Report Date: 15-May-2017

Order Date: 11-May-2017

Project Description: 170132

Client ID:	TP1	TP3	TP7	-
Sample Date:	08-May-17	08-May-17	08-May-17	-
Sample ID:	1719377-01	1719377-02	1719377-03	-
MDL/Units	Water	Water	Water	-

General Inorganics

Ammonia as N	0.01 mg/L	0.28	0.39	1.66	-
Total Kjeldahl Nitrogen	0.1 mg/L	78.1	65.3	131	-

Anions

Nitrate as N	0.1 mg/L	<0.1	0.5	<0.1	-
Nitrite as N	0.05 mg/L	<0.05	<0.05	<0.05	-

Certificate of Analysis
Client: LRL Associates Ltd.
Client PO:

Report Date: 15-May-2017

Order Date: 11-May-2017

Project Description: 170132

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Nitrate as N	ND	0.1	mg/L						
Nitrite as N	ND	0.05	mg/L						
General Inorganics									
Ammonia as N	ND	0.01	mg/L						
Total Kjeldahl Nitrogen	ND	0.1	mg/L						

Certificate of Analysis
Client: LRL Associates Ltd.
Client PO:

Report Date: 15-May-2017

Order Date: 11-May-2017

Project Description: 170132

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Nitrate as N	ND	0.1	mg/L	ND				20	
Nitrite as N	ND	0.05	mg/L	ND				20	
General Inorganics									
Ammonia as N	0.021	0.01	mg/L	0.022			2.4	8	
Total Kjeldahl Nitrogen	1.50	0.1	mg/L	1.52			1.8	10	

Certificate of Analysis
Client: LRL Associates Ltd.
Client PO:

Report Date: 15-May-2017

Order Date: 11-May-2017

Project Description: 170132

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Nitrate as N	1.01	0.1	mg/L	ND	101	81-112			
Nitrite as N	1.02	0.05	mg/L	ND	102	76-117			
General Inorganics									
Ammonia as N	0.280	0.01	mg/L	0.022	103	81-124			
Total Kjeldahl Nitrogen	1.91	0.1	mg/L		95.7	81-126			

Certificate of Analysis
Client: LRL Associates Ltd.
Client PO:

Report Date: 15-May-2017
Order Date: 11-May-2017
Project Description: 170132

Qualifier Notes:***Login Qualifiers :***

Samples received submerged in water, possibly melted ice. This condition can compromise sample integrity.

Applies to samples: TP1, TP3, TP7

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable

ND: Not Detected

MDL: Method Detection Limit

Source Result: Data used as source for matrix and duplicate samples

%REC: Percent recovery.

RPD: Relative percent difference.

APPENDIX C
Sieve & Hydrometer Analysis



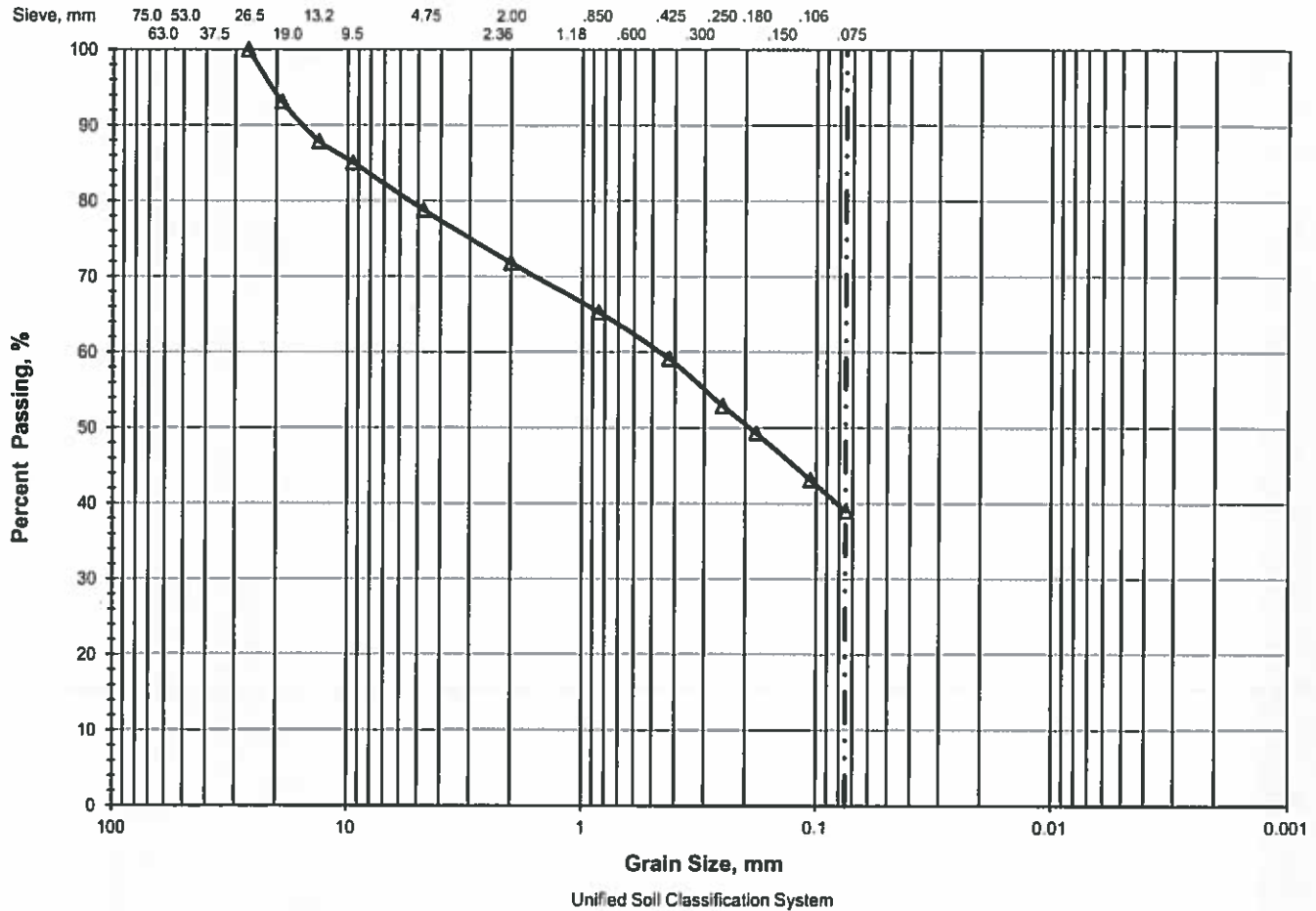
LRL Associates Ltd.

PARTICLE SIZE ANALYSIS

ASTM D 422 / LS-702

Client: Lloyd Phillips & Associates Ltd.
 Project: Hydrogeological Assessment & Terrain Analysis
 Location: 4835 Bank Street., Ottawa, ON.

File No.: 170132
 Report No.: 2
 Date: May 8, 2017



> 75 mm	% GRAVEL		% SAND			% FINES
	Coarse	Fine	Coarse	Medium	Fine	Silt & Clay
0.0	6.0	15.3	7.0	12.7	20.1	39.0

Location	Sample	Depth, m	D ₆₀	D ₅₀	D ₃₀	D ₁₅	D ₁₀	C _c	C _u
Δ	TP3	6	1.4 - 1.6	0.4855	0.1932				

APPENDIX D
Ontario Well Record Printouts

055,38

316/52



UTM 1182 453760E

N 15R 56117560N

The Ontario Water Resources Commission Act

Elev: 4R 03115

WATER WELL RECORD

Basin 251 CARLETON

County or District

Township, Village, Town or City

GLOUCESTER

Con. 4RF

Lot

21

Date completed

20

(day)

JULY

month

62

year)

Address BILLINGS BRIDGE

Casing and Screen Record

Inside diameter of casing 18 1/4

Total length of casing -

Type of screen -

Length of screen -

Depth to top of screen -

Diameter of finished hole 4

Pumping Test

Static level 6

Test-pumping rate 6 G.P.M.

Pumping level 8

Duration of test pumping 1 HR

Water clear or cloudy at end of test CC

Recommended pumping rate 6 G.P.M.

with pump setting of 30 feet below ground surface

Well Log

Water Record

Overburden and Bedrock Record

From
ft.To
ft.Depth(s) at
which water(s)
foundKind of water
(fresh, salty,
sulphur)

CLAY

0

18

Limestone

18

45

45

F

For what purpose(s) is the water to be used?

Home

Is well on upland, in valley, or on hillside?

Drilling or Boring Firm

M MEASHER

Address

6700

Licence Number

618

Name of Driller or Borer

311415

Address

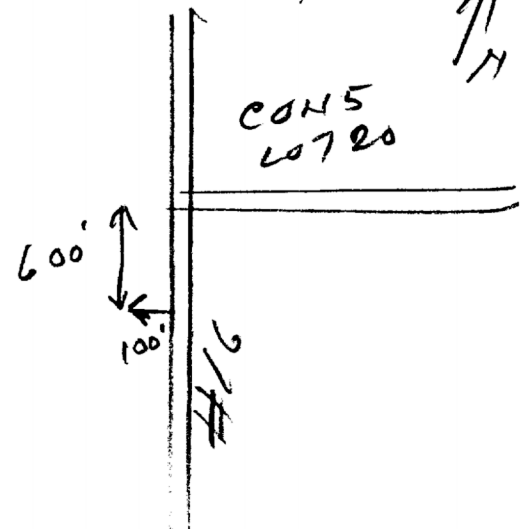
Date

Aug 28

(Signature of Licensed Drilling or Boring Contractor)

Location of Well

In diagram below show distances of well from
road and lot line. Indicate north by arrow.

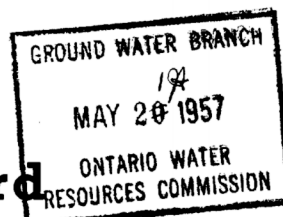


316/52
864
UTM 1182 4531710 E
Rideau Front
15R 50717101810 N
Elev. 4R 103310
Basin 215 221



The Water-well Drillers Act, 1954
Department of Mines

15 No 2177



Water-Well Record

County or Territorial District Carleton Township, Village, Town or City Gloucester

in Village, Town or City
Address 46 Lawrence St Ottawa

(day) (month) (year)

Pipe and Casing Record

Pumping Test

Casing diameter(s) <u>2"</u>	Static level <u>6</u>
Length(s) <u>21</u>	Pumping rate <u>800 G.P.H</u>
Type of screen	Pumping level <u>2.5 ft</u>
Length of screen	Duration of test <u>2 hr</u>

Well Log

Water Record

Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
<u>Sand</u>	<u>0</u>	<u>7</u>	<u>60</u>	<u>54</u>	<u>Fresh</u>
<u>Boulders and Sand</u>	<u>7</u>	<u>20</u>			
<u>Wt Sand Stone</u>	<u>20</u>	<u>60</u>			

For what purpose(s) is the water to be used?

House

Is water clear or cloudy? clear

Is well on upland, in valley, or on hillside? Upland

Drilling firm F.R. Corbett

Address 1252 Baseline Rd

City of Ottawa

Name of Driller F.R. Corbett

Address

Licence Number 395

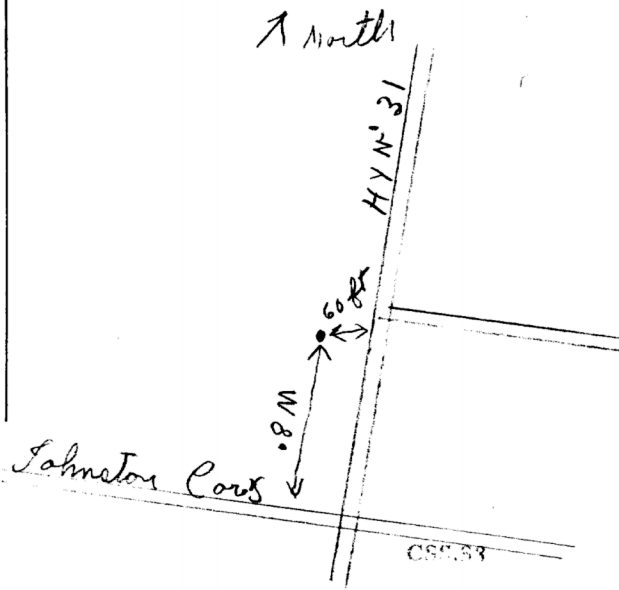
I certify that the foregoing statements of fact are true.

Date May 14/57 F.R. Corbett

Signature of Licensee

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



UTM 1182 415131040E
19R 510116121910N

Elev. 19R 0173.54
Basin 215
Con IV
104 22



The Water-well Drillers Act, 1954
Department of Mines

15
AUG 19 1957
ONTARIO WATER
RESOURCES COMMISSION

Water-Well Record

County or Territorial District Carleton Township, Village, Town or City Gloucester
Address Bellings Bridge

Pipe and Casing Record

Pumping Test

Casing diameter (s) 4"
Length (s) 23 feet
Type of screen 1
Length of screen
Static level 13
Pumping rate 240 gal PH
Pumping level 50 feet
Duration of test 1 hour

Well Log

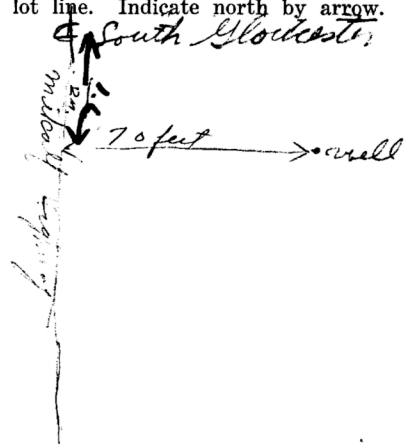
Water Record

Overburden and Bedrock Record	From ft.	To ft.	Depth (s) at which water (s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
<u>hard gravel</u>	<u>0</u>	<u>18</u>	<u>48 feet</u>	<u>37'</u>	<u>fresh</u>
<u>Clay & Sand</u>					<u>very hard</u>
<u>& boulders</u>					
<u>Very hard lime</u>	<u>18</u>	<u>50</u>			
<u>Stone Rock</u>					

For what purpose(s) is the water to be used?
house, hillside, etc.
Is water clear or cloudy? Clear
Is well on upland, in valley, or on hillside?
uplands
Drilling firm
Address
Name of Driller James Kettle
Address (Perryville)
Licence Number 337
I certify that the foregoing statements of fact are true.
Date August 5 James Kettle
Signature of Licensee

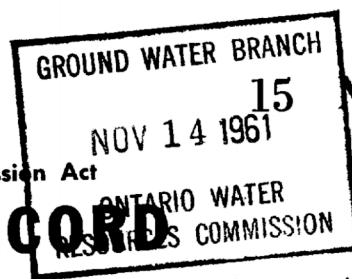
Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



L.P.

316/52



2178

UTM 18 Z 4153810 EElev. 15 R 10117330 NBasin 215 1 CarletonCounty or District CarletonCon 4 R FLot P.T.22Township, Village, Town or City GloucesterDate completed 6 10 1961Address 28 Clarence St. Ottawa 2, Ont.

WATER WELL RECORD

Casing and Screen Record

Inside diameter of casing 6 3/16
 Total length of casing 21'
 Type of screen NONE
 Length of screen NONE
 Depth to top of screen NONE
 Diameter of finished hole 6"

Pumping Test

Static level 20'
 Test-pumping rate 80 G.P.M.
 Pumping level 70'
 Duration of test pumping 1 hr.
 Water clear or cloudy at end of test clear
 Recommended pumping rate 80 G.P.M.
 with pump setting of 80 feet below ground surface

Well Log

Overburden and Bedrock Record

	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
Till and Boulder, grey hard lime stone and sand stone.	0	16	85	fresh
SAND STONE	16	25		
Boulder Till	25	89		
<u>Boulder Till</u>	<u>0</u>	<u>16</u>		
<u>HARD GREY LIMESTONE</u>	<u>16</u>	<u>25</u>		
<u>SANDSTONE</u>	<u>25</u>	<u>89</u>	<u>85</u>	<u>FRESH</u>

Water Record

For what purpose(s) is the water to be used?

Co-operativeIs well on upland, in valley, or on hillside? ValleyDrilling or Boring Firm J. B. Dufresne Co. Ltd.Address Ottawa, Ontario.Licence Number 194Name of Driller or Borer W. RoyAddress HillDate Oct 10/60

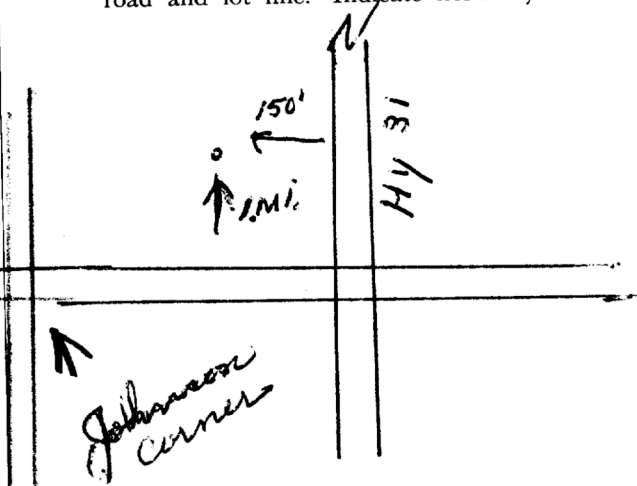
(Signature of Licensed Drilling or Boring Contractor)

Form 7 15M Sets 60-5930

OWRC COPY

Location of Well

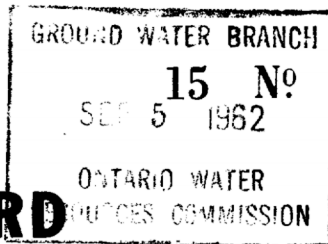
In diagram below show distances of well from road and lot line. Indicate north by arrow.



CS-153

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UTM 118^Z 4513181010^E



5^R 5101175310^N

The Ontario Water Resources Commission Act

Elev. 4^R 213115

WATER WELL RECORD

Basin 25 CHARLETON

County or District

Township, Village, Town or City CLARENDON

Con. 4RF Lot 2122

Date completed 26 JULY 62
(day month year)

Address BILLINGS BRIDGE

Casing and Screen Record

Inside diameter of casing 4
Total length of casing 21
Type of screen -
Length of screen -
Depth to top of screen -
Diameter of finished hole 4

Pumping Test

Static level 8
Test-pumping rate 5 G.P.M.
Pumping level 10
Duration of test pumping 1 HR
Water clear or cloudy at end of test cc
Recommended pumping rate 5 G.P.M.
with pump setting of 30 feet below ground surface

Well Log

Overburden and Bedrock Record

CLAY
Limestone

From ft.

To ft.

Depth(s) at which water(s) found

Kind of water (fresh, salty, sulphur)

0 21

21 46

46 F

For what purpose(s) is the water to be used?

Home

Is well on upland, in valley, or on hillside?

Drilling or Boring Firm

MEAGHER

Address OTTAWA

Licence Number 618

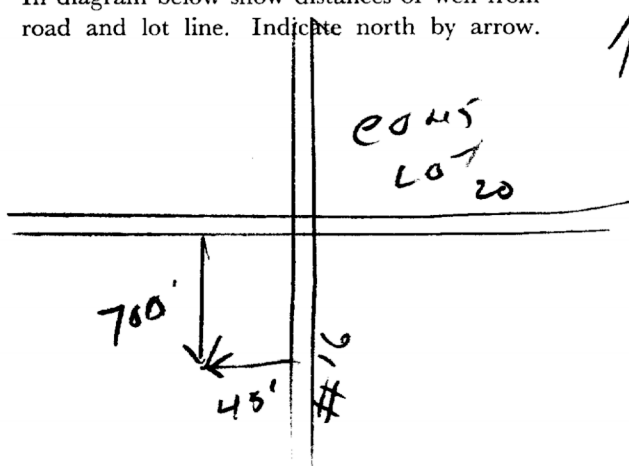
Name of Driller or Borer SDME

Address 70624

Date 1962
(Signature of Licensed Drilling or Boring Contractor)

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



020,52

C55.58

UTM 1182 4539610 E



WATER RESOURCES
DIVISION
15 No 2249
DEC 14 1966
ONTARIO WATER
RESOURCES COMMISSION

5R 5017880N The Ontario Water Resources Commission Act

Elev. 4R 0300

WATER WELL RECORD

Basin 125 11 Carl

County or District 5 1P Lot 21

Township, Village, Town or City G. Loughester

Date completed 19 Nov. 1966
(day month year)

Address RR #3 Metcalfe Ont

Casing and Screen Record

Inside diameter of casing 5"
Total length of casing 20'
Type of screen
Length of screen
Depth to top of screen
Diameter of finished hole 5"

Pumping Test

Static level 15
Test-pumping rate 5 G.P.M.
Pumping level 45
Duration of test pumping 1 hr
Water clear or cloudy at end of test cloudy
Recommended pumping rate 5 G.P.M.
with pump setting of 75' feet below ground surface

Well Log

Overburden and Bedrock Record

sand fill
sandstone

From
ft.

To
ft.

Depth(s) at
which water(s)
found

Kind of water
(fresh, salty,
sulphur)

0

4

83

fresh

4

85

For what purpose(s) is the water to be used?

old house

Is well on upland, in valley, or on hillside?

upland

Drilling or Boring Firm

Capital Water
Supply

Address

14 Ashford Dr
Ottawa 6

Licence Number

2158

Name of Driller or Borer

A Scott

Address

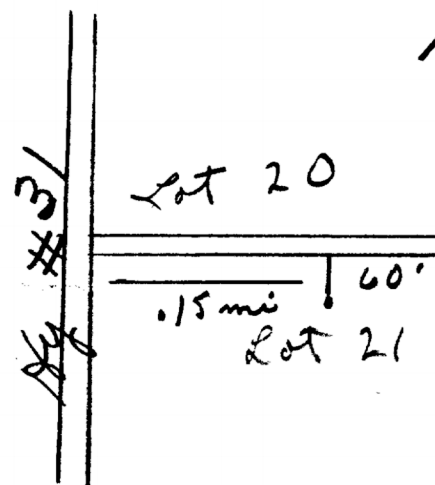
Date Nov 19, 1966

Walter Lavanagh

(Signature of Licensed Drilling or Boring Contractor)

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



316/52



WATER RESOURCES	
DIVISION	
15	No. 2250
JAN 19 1965	
ONTARIO WATER RESOURCES COMMISSION	

UTM 118Z 4541110E

Radian 5R 56116920N

Elev. 423

Basin 251 Curleton

The Ontario Water Resources Commission Act

WATER WELL RECORD

County or District

Township, Village, Town or City

Con. V R F Lot 23

Date completed 14 Dec 1964

Address Box 254 RR6, Ottawa

Casing and Screen Record

Inside diameter of casing 5"
 Total length of casing 10'
 Type of screen none
 Length of screen —
 Depth to top of screen —
 Diameter of finished hole 5"

Pumping Test

Static level 20'
 Test-pumping rate 4 G.P.M.
 Pumping level 65'
 Duration of test pumping 1 1/2 hrs
 Water clear or cloudy at end of test cloudy
 Recommended pumping rate 4 G.P.M.
 with pump setting of 75 feet below ground surface

Well Log

Water Record

Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
loam	0	2		
Hard Sandstone	2	65		
Red Granite	65	79	60-79	fresh

For what purpose(s) is the water to be used?

house

Is well on upland, in valley, or on hillside? hillside

Drilling or Boring Firm

McLean Water Supply Ltd

Address 1532 Raven Ave

Ottawa

Licence Number 1328

Name of Driller or Borer H. Sally

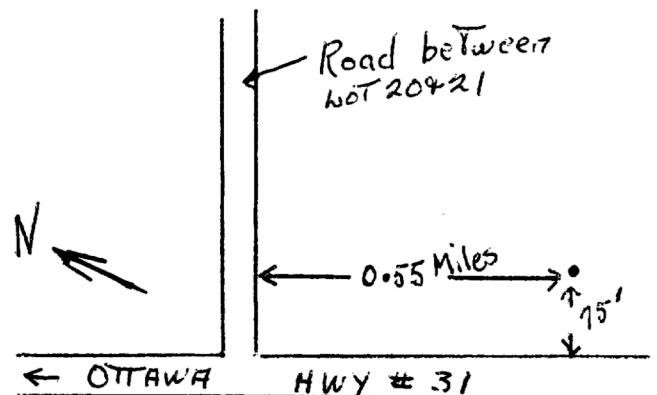
Address

Date Dec 17, 1964

(Signature of Licensed Drilling or Boring Contractor)

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



UTM 18 453 890
54 50 17 040
Elev. 4 0308
RF
Can. 21



09925

CODED
The Ontario Water Resources Commission Act

WATER WELL RECORD

County or District **Carleton** Township, Village, Town or City **Gloucester**
Con. **RF 5** Lot **2021** Date completed **6 December 1968**
(day month year)
Address **Long Sault, Ontario**

Casing and Screen Record

Inside diameter of casing **6"**
Total length of casing **15'**
Type of screen **nil**
Length of screen **n/a**
Depth to top of screen **n/a**
Diameter of finished hole **6"**

Pumping Test

Static level **2'**
Test-pumping rate **10** G.P.M.
Pumping level **5'**
Duration of test pumping **1 Hour**
Water clear or cloudy at end of test **cloudy**
Recommended pumping rate **10** G.P.M.
with pump setting of **25'** feet below ground surface

Well Log

Overburden and Bedrock Record

Closely packed Boulders
Very Abrasive Sandstone

From
ft.

To
ft.

Depth(s) at
which water(s)
found

Kind of water
(fresh, salty,
sulphur)

0'

13'

13'

63'

60'

fresh

For what purpose(s) is the water to be used?

Trailer Sales Depot

Is well on upland, in valley, or on hillside? **Valley**

Drilling or Boring Firm

Blair Phillips Drilling Co. Ltd.,

Address **1119 Balise Road, Ottawa 5, Ontario.**

Licence Number **2779**

Name of Driller or Borer **J. Moore**

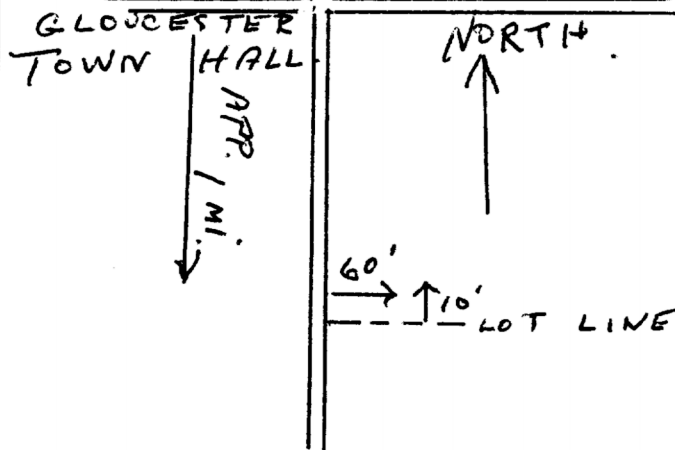
Address **Kars, Ontario**

Date **6 December 1968**

(Signature of Licensed Drilling or Boring Contractor)

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



WATER WELL RECORD

Water management in Ontario 1. PRINT ONLY IN SPACES PROVIDED

2. CHECK ☒ CORRECT BOX WHERE APPLICABLE

11

1510717

MUNICIP.

CON.

CONF
RF

at last

COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	CON., BLOCK, TRACT, SURVEY, ETC.	LOT
CARLETON	GLOUCESTER	9 RF	023
31 HIGHWAY LEITRUM		DATE COMPLETED	FE-53
		DAY 15	MO. 10 YR. 70
ING	RC.	ELEVATION	RC.
2169.20	4	2134.2	4
		25	

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

[illegible]

31	0000012	0053215						
32								

41 WATER RECORD			
WATER FOUND AT - FEET		KIND OF WATER	
10-13	1 <input checked="" type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL	14
15-18	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL	19
20-23	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL	24
25-28	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL	29
30-33	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL	34

51 CASING & OPEN HOLE RECORD				
INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
06-10-11	1 <input checked="" type="checkbox"/> STEEL	12		13-10
	2 <input checked="" type="checkbox"/> GALVANIZED			0020
	3 <input checked="" type="checkbox"/> CONCRETE	188	0	20
	4 <input checked="" type="checkbox"/> OPEN HOLE			
17-18	1 <input type="checkbox"/> STEEL	19		20-21
	2 <input type="checkbox"/> GALVANIZED			
	3 <input type="checkbox"/> CONCRETE			
	4 <input checked="" type="checkbox"/> OPEN HOLE			0052
24-25	1 <input type="checkbox"/> STEEL	26		27-30
	2 <input type="checkbox"/> GALVANIZED			
	3 <input type="checkbox"/> CONCRETE			
	4 <input type="checkbox"/> OPEN HOLE			

SCREEN	54	65	75	39-42
	SIZE(S) OF OPENING (SLOT NO.)	31-33	DIAMETER	34-38
	INCHES			FEET
	MATERIAL AND TYPE	DEPTH TO TOP OF SCREEN		41-44
				FEET

61 PLUGGING & SEALING RECORD			
DEPTH SET AT - FEET		MATERIAL AND TYPE	CEMENT GROUT, LEAD PACKER, ETC.
FROM	TO		
10-13	14-17		
18-21	22-25		
26-29	30-33	80	

71	PUMPING TEST METHOD		TO	PUMPING RATE		11-14	DURATION OF PUMPING		17-18
	<input checked="" type="checkbox"/> X PUMP <input type="checkbox"/> BAILER			0006		GPM.	22 15-16 00		MINS.
	STATIC LEVEL		25	WATER LEVELS DURING		1		2 RECOVERY	
	19-21	22-24	15 MINUTES	26-28	30 MINUTES	29-31	45 MINUTES	32-34	60 MINUTES
	007	015	007	007	007	007	007	007	
	FEET	FEET	FEET	FEET	FEET	FEET	FEET	FEET	
	IF FLOWING, GIVE RATE		38-41	PUMP INTAKE SET AT		WATER AT END OF TEST		42	
			GPM.	30		FEET	<input checked="" type="checkbox"/> X CLEAR <input type="checkbox"/> CLOUDY		
	RECOMMENDED PUMP TYPE			RECOMMENDED PUMP SETTING		43-45	RECOMMENDED PUMPING RATE		
	<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> X DEEP			040		FEET	0005		
	50-53		0008		GPM./FT. SPECIFIC CAPACITY				

<p>FINAL STATUS OF WELL</p>	<p>54</p> <p>1 <input checked="" type="checkbox"/> WATER SUPPLY 2 <input type="checkbox"/> OBSERVATION WELL 3 <input type="checkbox"/> TEST HOLE 4 <input type="checkbox"/> RECHARGE WELL</p>	<p>5 <input type="checkbox"/> ABANDONED, INSUFFICIENT FLUID 6 <input type="checkbox"/> ABANDONED, POOR QUALITY 7 <input type="checkbox"/> UNFINISHED</p>
<p>WATER USE</p>	<p>55-56</p> <p>1 <input type="checkbox"/> DOMESTIC 2 <input type="checkbox"/> STOCK 3 <input type="checkbox"/> IRRIGATION 4 <input type="checkbox"/> INDUSTRIAL <input type="checkbox"/> OTHER _____</p>	<p>5 <input checked="" type="checkbox"/> COMMERCIAL 6 <input type="checkbox"/> MUNICIPAL 7 <input type="checkbox"/> PUBLIC SUPPLY 8 <input type="checkbox"/> COOLING OR AIR CONDITIONING 9 <input type="checkbox"/> NOT USED</p>
<p>METHOD OF DRILLING</p>	<p>57</p> <p>1 <input checked="" type="checkbox"/> CABLE TOOL 2 <input type="checkbox"/> ROTARY (CONVENTIONAL) 3 <input type="checkbox"/> ROTARY (REVERSE) 4 <input type="checkbox"/> ROTARY (AIR) 5 <input type="checkbox"/> AIR PERCUSSION</p>	<p>6 <input type="checkbox"/> BORING 7 <input type="checkbox"/> DIAMOND 8 <input type="checkbox"/> JETTING 9 <input type="checkbox"/> DRIVING</p>

LOCATION OF WELL

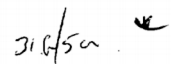
IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.

The diagram illustrates the location of a well (marked with a circle and the number 31) relative to a road and lot lines. The well is located 300' from the road and 30' from the lot line. A distance of 7.0m is also indicated. A north arrow points towards the top right. The lots are labeled LOT 25, LOT 26, and LOT 22.

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	W. MOLOUGHNEY	3701
	ADDRESS	
	1110 FISHER	
	NAME OF DRILLER OR BORE	LICENCE NUMBER
	W. MOLOUGHNEY	
	SIGNATURE OF CONTRACTOR	SUBMISSION DATE
	<i>[Signature]</i>	DAY 11 MO. FEB. YR 2

OFFICE USE ONLY	DATA SOURCE	58 CONTRACTOR	59-62	DATE RECEIVED	63-68
	1	3701		230271	
	DATE OF INSPECTION		INSPECTOR		
REMARKS:					P <i>[Signature]</i> WI

OWRC COPY



105

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

XC COPY

Well ID Number: 1512375
Well Audit Number:
Well Tag Number:

This table contains information from the original well record and any subsequent updates.

Well Location

Address of Well Location	
Township	GLOUCESTER TOWNSHIP
Lot	022
Concession	RF 04
County/District/Municipality	OTTAWA-CARLETON
City/Town/Village	
Province	ON
Postal Code	n/a
UTM Coordinates	NAD83 — Zone 18
	Easting: 454020.70
	Northing: 5017262.00
Municipal Plan and Sublot Number	
Other	

Overburden and Bedrock Materials Interval

General Colour	Most Common Material	Other Materials	General Description	Depth From	Depth To
BRWN	OBDN	SAND		0 ft	9 ft
WHIT	SNDS			9 ft	74 ft

Annular Space/Abandonment Sealing Record

Depth From	Depth To	Type of Sealant Used (Material and Type)	Volume Placed
------------	----------	--	---------------

Method of Construction & Well Use

Method of Construction	Well Use
Diamond	Domestic

Status of Well

Water Supply

Construction Record - Casing

Inside Diameter	Open Hole or material	Depth From	Depth To
2 inch	GALVANIZED		20 ft
	OPEN HOLE		74 ft

Construction Record - Screen

Outside Diameter	Material	Depth From	Depth To
------------------	----------	------------	----------

Well Contractor and Well Technician Information

Well Contractor's Licence Number: 1703

Results of Well Yield Testing

After test of well yield, water was	CLEAR
If pumping discontinued, give reason	
Pump intake set at	
Pumping Rate	8 GPM
Duration of Pumping	2 h:0 m
Final water level	12 ft
If flowing give rate	
Recommended pump depth	35 ft
Recommended pump rate	8 GPM
Well Production	PUMP
Disinfected?	

Draw Down & Recovery

Draw Down Time(min)	Draw Down Water level	Recovery Time(min)	Recovery Water level
SWL	6 ft		
1		1	
2		2	
3		3	
4		4	
5		5	
10		10	
15	12 ft	15	
20		20	
25		25	
30	12 ft	30	
40		40	
45	12 ft	45	
50		50	
60	12 ft	60	

Water Details

Water Found at Depth	Kind
74 ft	Fresh

Hole Diameter

Depth From	Depth To	Diameter
------------	----------	----------

Audit Number:

Date Well Completed: November 27, 1972

Date Well Record Received by MOE: March 07, 1973

Updated: February 2, 2018

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Tags

- [Environment and energy.](#)



WATER WELL RECORD

316/5a

11

1514664.

MUNICIP. 15002 CON. RF

CON.
RF

0.4

21 18 453793 5017090 4 0340 30 26

[illegible][illegible]

WATER FOUND AT - FEET		KIND OF WATER			
0032	10-15	1 <input checked="" type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	14	
	2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERAL			
0111	15-18	1 <input checked="" type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	19	
	2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERAL			
	20-23	1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	24	
	2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERAL			
	25-28	1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	29	
	2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERAL			
	30-33	1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	34	
	2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERAL			

INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
10-11	1 <input checked="" type="checkbox"/> STEEL	12		13-16
14-16	2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	18.5	0	22.0022
17-18	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input checked="" type="checkbox"/> OPEN HOLE	19	22	20-23 0125
22-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	26		27-30

SIZE(S) OF OPENING (SLOT NO.)	31-33	DIAMETER	34-38	LENGTH	39-40
		INCHES		FEET	
MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN		41-44	8
				FEET	

DEPTH SET AT - FEET		MATERIAL AND TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
FROM	TO	
10-13	14-17	
18-21	22-25	
26-29	30-33	80


PUMPING TEST	PUMPING TEST METHOD		10	PUMPING RATE		11-14	DURATION OF PUMPING	
	1 <input checked="" type="checkbox"/> PUMP	2 <input type="checkbox"/> BAILER		0012		GPM	01	15-16 HOURS 15
	STATIC LEVEL	WATER LEVEL END OF PUMPING	25	WATER LEVELS DURING			1 <input checked="" type="checkbox"/> PUMPING	17-18 MINS
							2 <input type="checkbox"/> RECOVERY	
	19-21	22-24	15 MINUTES	30 MINUTES	45 MINUTES	60 MINUTES		
	020	020	020	020	020	020		
IF FLOWING, GIVE RATE		38-41	PUMP INTAKE SET AT			WATER AT END OF TEST		42
		GPM	80		FEET	1 <input checked="" type="checkbox"/> CLEAR		2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE			43-45	RECOMMENDED PUMP SETTING		RECOMMENDED PUMPING RATE		46-48
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP			080		FEET	0008		GPM
50-53			GPM / SET. SPECIFIC CAPACITY					

WATER USE 04	22-55	1 <input type="checkbox"/> DOMESTIC 2 <input type="checkbox"/> STOCK 3 <input type="checkbox"/> IRRIGATION 4 <input checked="" type="checkbox"/> INDUSTRIAL <input type="checkbox"/> OTHER	5 <input type="checkbox"/> COMMERCIAL 6 <input type="checkbox"/> MUNICIPAL 7 <input type="checkbox"/> PUBLIC SUPPLY 8 <input type="checkbox"/> COOLING OR AIR CONDITIONING <input type="checkbox"/> NOT USED
	57	1 <input type="checkbox"/> CABLE TOOL 2 <input type="checkbox"/> ROTARY (CONVENTIONAL) 3 <input type="checkbox"/> ROTARY (REVERSE) 4 <input type="checkbox"/> ROTARY (AIR) 5 <input checked="" type="checkbox"/> AIR PERCUSSION	6 <input type="checkbox"/> BORING 7 <input type="checkbox"/> DIAMOND 8 <input type="checkbox"/> JETTING 9 <input type="checkbox"/> DRIVING
METHOD OF DRILLING 5			

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.

Hand-drawn sketch map of the study area. A vertical line represents the 'Hy #31' well. A horizontal line represents the 'South Gloucester' road. A small rectangle is drawn near the intersection, labeled '750' and '20'.

DRILLERS REMARKS

CONTRACTOR	NAME OF WELL CONTRACTOR		LICENCE NUMBER
	Hawthorne Drilling Ltd		2558
	ADDRESS		
	PO Box 4218 Stat E.		
	NAME OF DRILLER OR BORER		LICENCE NUMBER
	A. Emond		2558
	SIGNATURE OF CONTRACTOR		SUBMISSION DATE
			DAY 24 MO. 2 YR. 2

OFFICE USE ONLY	DATA SOURCE	58	CONTRACTOR	52	DATE RECEIVED	63-68
	1	2558	2	205	75	
	DATE OF INSPECTION		INSPECTOR			
REMARKS:						P
C55.48						WI



WATER WELL RECORD

The Ontario Water Resources Act

Ontario

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11

1574664

COUNTY OR DISTRICT Carleton	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE Gloucester	CON., BLOCK, TRACT, SURVEY, ETC. 111	LOT 22
OWNER (SURNAME FIRST) Canadian Industries Ltd.	ADDRESS Highway #31	DATE COMPLETED DAY 20 NO. 2 YR. 25	
ZONING ZONE 21 EASTING 10 NORTHING 12 ELEVATION 25 BASIN CODE 11			

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET
Brown	Sand Gravel	Boulders	Dense	0 13
Black	Shale		Loose	13 30
Grey	Limestone		Sand	30 111
White	Sandstone		Sand	111 125

31	32
----	----

41 WATER RECORD	
WATER FOUND AT - FEET	KIND OF WATER
32	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 14
111	2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD			
INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET
6 1/4	1 <input checked="" type="checkbox"/> STEEL	1.185	0 22
5 7/8	2 <input type="checkbox"/> GALVANIZED		22 125

SCREEN	SIZE(S) OF OPENING (SLOT NO.)	DIAMETER	LENGTH

61 PLUGGING & SEALING RECORD	
DEPTH SET AT - FEET	MATERIAL AND TYPE
FROM TO	

71 PUMPING TEST	10 PUMPING RATE	11-14 DURATION OF PUMPING
1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> BAILEY	12 GPM	15-16 HOURS 17-18 MINS
STATIC LEVEL	WATER LEVELS DURING	1 <input type="checkbox"/> PUMPING 2 <input type="checkbox"/> RECOVERY
20	20 20 20 20	

LOCATION OF WELL	
IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.	

54 FINAL STATUS OF WELL	55-56 WATER USE	57 METHOD OF DRILLING
1 <input checked="" type="checkbox"/> WATER SUPPLY	1 <input type="checkbox"/> DOMESTIC	1 <input type="checkbox"/> CABLE TOOL
2 <input type="checkbox"/> OBSERVATION WELL	2 <input type="checkbox"/> STOCK	2 <input type="checkbox"/> ROTARY (CONVENTIONAL)
3 <input type="checkbox"/> TEST HOLE	3 <input type="checkbox"/> IRRIGATION	3 <input type="checkbox"/> ROTARY (REVERSE)
4 <input type="checkbox"/> RECHARGE WELL	4 <input checked="" type="checkbox"/> INDUSTRIAL	4 <input type="checkbox"/> ROTARY (AIR)

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	Haithorne Drilling Ltd.	2558
	ADDRESS	
	PO Box 4218 Stat. E.	
	NAME OF DRILLER OR BORER	LICENCE NUMBER
	A. Emond	2558
	SIGNATURE OF CONTRACTOR	SUBMISSION DATE
		DAY 24 MO. 2 YR. 25

OFFICE USE ONLY	DATA SOURCE	CONTRACTOR	DATE RECEIVED
	DATE OF INSPECTION	INSPECTOR	
	REMARKS		



316/5a

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1514840

MUNICIP
15002

CON. 12 E

105

COUNTY OR DISTRICT

TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE

10	14
CON. BLOCK TRACT SURVEY ETC	

15

	22	23
LOT		25

OWNER (SURNAME FIRST)

20-47

ADDRESS _____

Done

023

14. 500 000 110

200 -- RR #1

DATE COMPLETED
11

2-53 2.

21

U 17

LASTING
454143

NORTHING
5016952

ELEVATION
10345

RC.	BASIN COD
111	111

11

114

IV

1

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

OFFICE USE ONLY	DATA SOURCE	58	CONTRACTOR	DATE RECEIVED	63-58	80
	1		2537	06 08 75		
	DATE OF INSPECTION		INSPECTOR			
			[Signature]			
	REMARKS:					P ✓
						WI



Ontario

Ministry
of the
Environment

The Ontario Water Resources Act

WATER WELL RECORD

1517349

MUNICIP

15002

CODE

021

05

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11

COUNTY OR DISTRICT

Ottawa C.D.

TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE

Shawville

CON. BLOCK, TRACT, SURVEY, ETC.

5 R.F.

LOT

25-27

DATE COMPLETED

09

06

YR 80

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	POST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
Brown	Sandy soil	stone		0	8
Brown	hard	granite rock		8	27

31 00086021281 00276211373

32

41 WATER RECORD	
WATER FOUND AT - FEET	KIND OF WATER
10-13	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR
15-18	2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR
25-28	2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR
	2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD	
INSIDE DIAM. INCHES	MATERIAL
06 10 11 12	1 <input checked="" type="checkbox"/> STEEL
17-18	2 <input type="checkbox"/> GALVANIZED
24-25	3 <input type="checkbox"/> CONCRETE
	4 <input type="checkbox"/> OPEN HOLE

SIZE(S) OF OPENING (SLOT NO.)	31-33
DIAMETER	34-38
LENGTH	39-40
MATERIAL AND TYPE	
DEPTH TO TOP OF SCREEN	41-44
	FEET

61 PLUGGING & SEALING RECORD	
DEPTH SET AT - FEET	MATERIAL AND TYPE (CEMENT GROUT LEAD PACKER, ETC.)
FROM	TO
10-13	14-17
18-21	22-25
26-29	30-33

71 PUMPING TEST METHOD	10 PUMPING RATE	11-14 DURATION OF PUMPING
1 <input type="checkbox"/> PUMP 2 <input checked="" type="checkbox"/> BAILEY	0015	GPM 01
STATIC LEVEL	WATER LEVELS DURING	1 <input checked="" type="checkbox"/> PUMPING 2 <input type="checkbox"/> RECOVERY
19-21	15 MINUTES 30 MINUTES	45 MINUTES 60 MINUTES
005	014	012
IF FLOWING, GIVE RATE	PUMP INTAKE SET AT	WATER AT END OF TEST
	GPM 27	FEET 014
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP SETTING	RECOMMENDED PUMPING RATE
1 <input checked="" type="checkbox"/> SHALLOW 2 <input type="checkbox"/> DEEP	023	0007

LOCATION OF WELL	
IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.	
DRILLER'S REMARKS	

FINAL STATUS OF WELL	1 <input checked="" type="checkbox"/> WATER SUPPLY 5 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
	2 <input type="checkbox"/> OBSERVATION WELL 6 <input type="checkbox"/> ABANDONED, POOR QUALITY
	3 <input type="checkbox"/> TEST HOLE 7 <input type="checkbox"/> UNFINISHED
	4 <input type="checkbox"/> RECHARGE WELL
WATER USE	1 <input checked="" type="checkbox"/> DOMESTIC 5 <input checked="" type="checkbox"/> COMMERCIAL
05	2 <input type="checkbox"/> STOCK 6 <input type="checkbox"/> MUNICIPAL
	3 <input type="checkbox"/> IRRIGATION 7 <input type="checkbox"/> PUBLIC SUPPLY
	4 <input type="checkbox"/> INDUSTRIAL 8 <input type="checkbox"/> COOLING OR AIR CONDITIONING
	9 <input type="checkbox"/> NOT USED
METHOD OF DRILLING	1 <input checked="" type="checkbox"/> RIGID POOL 6 <input type="checkbox"/> BORING
1	2 <input type="checkbox"/> ROTARY (CONVENTIONAL) 7 <input type="checkbox"/> DIAMOND
	3 <input type="checkbox"/> ROTARY (REVERSE) 8 <input type="checkbox"/> JETTING
	4 <input type="checkbox"/> ROTARY (AIR) 9 <input type="checkbox"/> DRIVING
	5 <input type="checkbox"/> AIR PERCUSSION

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	Maxime Cyr Ltd.	1517
	ADDRESS	
	Casabon Ont.	
	NAME OF DRILLER OR BORE	LICENCE NUMBER
	SIGNATURE OF CONTRACTOR	SUBMISSION DATE
	Maxime Cyr	DAY MO YR

OFFICE USE ONLY	DATA SOURCE	58 CONTRACTOR	59-62 DAY RECEIVED	63-68
	1	1517	020980	80
	DATE OF INSPECTION	INSPECTOR		
	REMARKS			

Print only in spaces provided.

Mark correct box with a checkmark, where applicable.

11

1531693

Municipality

Con.

15002

CON

05

County or District <i>Other</i>		Township/Borough/City/Town/Village <i>Gloucester</i>		Con block tract survey, etc. <i>5</i>		Lot <i>6</i>	
Address <i>Greely St</i>				Date completed <i>25</i> <i>10</i> <i>00</i> day month year			
Northing		RC		Elevation		Basin Code	
<div style="border: 1px solid black; width: 100px; height: 15px;"></div>		<div style="border: 1px solid black; width: 100px; height: 15px;"></div>		<div style="border: 1px solid black; width: 100px; height: 15px;"></div>		<div style="border: 1px solid black; width: 100px; height: 15px;"></div>	

LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions)

[illegible]

41 WATER RECORD				
Water found at - feet		Kind of water		
10-13	1 <input checked="" type="checkbox"/> Fresh 2 <input checked="" type="checkbox"/> Salty	3 <input checked="" type="checkbox"/> Sulphur 4 <input checked="" type="checkbox"/> Minerals 5 <input checked="" type="checkbox"/> Gas	14	
15-18	1 <input checked="" type="checkbox"/> Fresh 2 <input checked="" type="checkbox"/> Salty	3 <input checked="" type="checkbox"/> Sulphur 4 <input checked="" type="checkbox"/> Minerals 5 <input checked="" type="checkbox"/> Gas	19	
20-23	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 5 <input type="checkbox"/> Gas	24	
25-28	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 5 <input type="checkbox"/> Gas	29	
30-33	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 5 <input type="checkbox"/> Gas	34	35

CASING & OPEN HOLE RECORD					
Inside diam inches	Material	Wall thickness inches	Depth - feet		
			From	To	
10-11	1 <input checked="" type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	12		13-16	
6 1/4		1 88	0	22	
17-18	1 <input type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input checked="" type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	19		20-23	
8 3/4			0	20	
24-25	1 <input type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input checked="" type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	26		27-30	
6			20	220	

SCREEN	Sizes of opening (Slot No.)	31-33	Diameter	34-38	Length	39-40
			inches		feet	
	Material and type			Depth at top of screen		
				41-44		
				feet		

61 PLUGGING & SEALING RECORD			
<input checked="" type="checkbox"/> Annular space		<input type="checkbox"/> Abandonment	
Depth set at - feet		Material and type (Cement grout, bentonite, etc.)	
From	To		
2 ¹³ 18-21	22 ¹⁷ 22-25	Cement grout	
26-29	30-33	80	

PUMPING TEST	71	Pumping test method ¹⁰ <input checked="" type="checkbox"/> Pump <input type="checkbox"/> Bailor	Pumping rate ¹¹⁻¹⁴ 10 GPM	Duration of pumping ¹⁵⁻¹⁶ 1 Hours 17-18 Mins
	Static level ¹⁹⁻²¹	Water level end of pumping ²²⁻²⁴	Water levels during ²⁵ <input type="checkbox"/> Pumping <input checked="" type="checkbox"/> Recovery	
	30 feet	120 feet	15 minutes ²⁶⁻²⁸ 30 minutes ²⁹⁻³¹ 45 minutes ³²⁻³⁴ 60 minutes ³⁵⁻³⁷	
			30 feet 30 feet 30 feet 30 feet	
	If flowing give rate ³⁸⁻⁴¹	Pump intake set at ⁴²⁻⁴³	Water at end of test ⁴⁴⁻⁴⁵	
	GPM	feet <input type="checkbox"/> Clear <input checked="" type="checkbox"/> Cloudy		
	Recommended pump type	Recommended pump setting ⁴⁶⁻⁴⁸	Recommended pump rate ⁴⁹⁻⁵⁰	
	<input type="checkbox"/> Shallow <input checked="" type="checkbox"/> Deep	120 feet	10 GPM	

FINAL STATUS OF WELL 54

1 <input checked="" type="checkbox"/> Water supply	5 <input type="checkbox"/> Abandoned, insufficient supply	9 <input type="checkbox"/> Unfinished
2 <input type="checkbox"/> Observation well	6 <input type="checkbox"/> Abandoned, poor quality	10 <input type="checkbox"/> Replacement well
3 <input type="checkbox"/> Test hole	7 <input type="checkbox"/> Abandoned (Other)	
4 <input type="checkbox"/> Recharge well	8 <input type="checkbox"/> Dewatering	

WATER USE 55-56

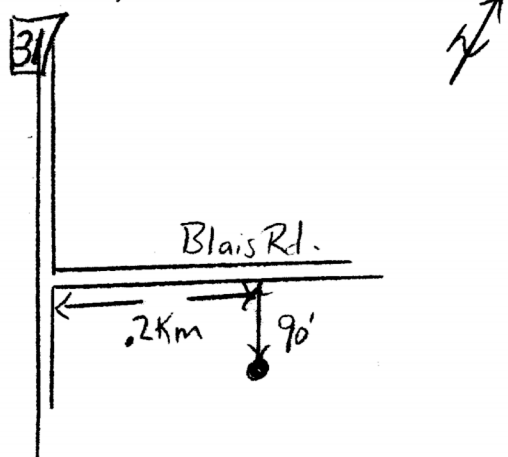
1 <input checked="" type="checkbox"/> Domestic	5 <input type="checkbox"/> Commercial	9 <input type="checkbox"/> Not use
2 <input type="checkbox"/> Stock	6 <input type="checkbox"/> Municipal	10 <input type="checkbox"/> Other
3 <input type="checkbox"/> Irrigation	7 <input type="checkbox"/> Public supply	
4 <input type="checkbox"/> Industrial	8 <input type="checkbox"/> Cooling & air conditioning	

METHOD OF CONSTRUCTION 57

1 <input type="checkbox"/> Cable tool	5 <input checked="" type="checkbox"/> Air percussion	9 <input type="checkbox"/> Driving
2 <input type="checkbox"/> Rotary (conventional)	6 <input type="checkbox"/> Boring	10 <input type="checkbox"/> Digging
3 <input type="checkbox"/> Rotary (reverse)	7 <input type="checkbox"/> Diamond	11 <input type="checkbox"/> Other
4 <input type="checkbox"/> Rotary (air)	8 <input type="checkbox"/> Jetting	

LOCATION OF WELL

In diagram below show distances of well from road and lot line. Indicate north by arrow.



222862

Name of Well Contractor <i>Air-Rock Drilling Co Ltd</i>	Well Contractor's Licence No <i>1119</i>
Address <i>RR # 2 Jasper Dt</i>	
Name of Well Technician <i>Shannon Purcell</i>	Well Technician's Licence No <i>T2122</i>
Signature of Technician/Contractor <i>[Signature]</i>	Submission date <i>03 to 06</i>

MINISTRY USE ONLY	Data source	58 Contractor	59-62	Date received	63-68
		11 19		JAN 03 2001	
	Date of inspection	Inspector			
	Remarks				
	CSS.ES				

Print only in spaces provided.

Mark correct box with a checkmark, where applicable.

11

153356.6




Municipality
15002

Con.
RF

05

County or District Ottawa Carleton		Township/Borough/City/Town/Village Gloucester		Con block 5	tract survey, etc.	Lot 21	25-27
Address [REDACTED]		Gloucester, Ont			Date completed 07 02 03		18-23
					day	month	year
21	U M	Northings	RC	Elevation	RC	Basin Code	ii iii iv

[illegible]

31     

41 WATER RECORD			
Water found at - feet		Kind of water	
19-13	<input type="checkbox"/> Fresh <input checked="" type="checkbox"/> Salty	<input type="checkbox"/> Sulphur <input checked="" type="checkbox"/> Minerals <input type="checkbox"/> Gas	14
216			
15-18	<input type="checkbox"/> Fresh <input checked="" type="checkbox"/> Salty	<input type="checkbox"/> Sulphur <input checked="" type="checkbox"/> Minerals <input type="checkbox"/> Gas	19
20-23	<input type="checkbox"/> Fresh <input type="checkbox"/> Salty	<input type="checkbox"/> Sulphur <input type="checkbox"/> Minerals <input type="checkbox"/> Gas	24
25-28	<input type="checkbox"/> Fresh <input type="checkbox"/> Salty	<input type="checkbox"/> Sulphur <input type="checkbox"/> Minerals <input type="checkbox"/> Gas	29
30-33	<input type="checkbox"/> Fresh <input type="checkbox"/> Salty	<input type="checkbox"/> Sulphur <input type="checkbox"/> Minerals <input type="checkbox"/> Gas	34

51 CASING & OPEN HOLE RECORD				
Inside diam inches	Material	Wall thickness inches	Depth - feet	
			From	To
10-11	<input checked="" type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	12		13-14
69		188	0	22
17-18	1 <input type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input checked="" type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	19		20-21
83			0	20
24-25	1 <input type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input checked="" type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	26		27-28
6			20	220

SCREEN	Sizes of opening (Slot No.)	31-33	Diameter	34-38	Length	39-40
			inches		feet	
	Material and type	Depth at top of screen			41-44	30
					feet	

61		PLUGGING & SEALING RECORD	
<input checked="" type="checkbox"/> Annular space		<input type="checkbox"/> Abandonment	
Depth set at - feet From To 10-13 17 22 22 18-21 22-25		Material and type (Cement grout, bentonite, etc.) Cement grout	
26-29	30-33	36	

PUMPING TEST	71	Pumping test method <input checked="" type="checkbox"/> Pump <input type="checkbox"/> Bailor	10	Pumping rate 9	11-14 GPM	Duration of pumping 15-18 Hours 17-18 Mins
	Static level 19-21 feet	Water level end of pumping 22-24 feet	25	Water levels during 15 minutes 25-28 feet	<input type="checkbox"/> Pumping <input checked="" type="checkbox"/> Recovery	
	16	120	16	16	45 minutes 32-34 feet	60 minutes 35-37 feet
	16	120	16	16	45 minutes 32-34 feet	60 minutes 35-37 feet
	16	120	16	16	45 minutes 32-34 feet	60 minutes 35-37 feet
	II flowing give rate	38-41 GPM	Pump intake set at	feet	Water at end of test	42
	Recommended pump type <input checked="" type="checkbox"/> Shallow <input type="checkbox"/> Deep		Recommended pump setting 120	43-45 feet	Recommended pump rate 9	46-49 GPM

FINAL STATUS OF WELL 54

<input checked="" type="checkbox"/> 1 Water supply	<input type="checkbox"/> 5 Abandoned, insufficient supply	<input type="checkbox"/> 9 Unfinished
<input type="checkbox"/> 2 Observation well	<input type="checkbox"/> 6 Abandoned, poor quality	<input type="checkbox"/> 10 Replacement well
<input type="checkbox"/> 3 Test hole	<input type="checkbox"/> 7 Abandoned (Other)	
<input type="checkbox"/> 4 Recharge well	<input type="checkbox"/> 8 Dewatering	

WATER USE		
1 <input type="checkbox"/> Domestic	5 <input type="checkbox"/> Commercial	9 <input type="checkbox"/> Not use
2 <input type="checkbox"/> Stock	6 <input type="checkbox"/> Municipal	10 <input type="checkbox"/> Other
3 <input type="checkbox"/> Irrigation	7 <input type="checkbox"/> Public supply	
4 <input type="checkbox"/> Industrial	8 <input type="checkbox"/> Cooling & air conditioning	

METHOD OF CONSTRUCTION 57

1 <input type="checkbox"/> Cable tool	5 <input checked="" type="checkbox"/> Air percussion	9 <input type="checkbox"/> Driving
2 <input type="checkbox"/> Rotary (conventional)	6 <input type="checkbox"/> Boring	10 <input type="checkbox"/> Digging
3 <input type="checkbox"/> Rotary (reverse)	7 <input type="checkbox"/> Diamond	11 <input type="checkbox"/> Other
4 <input type="checkbox"/> Rotary (air)	8 <input type="checkbox"/> Jetting	

LOCATION OF WELL

In diagram below show distances of well from road and lot line.
Indicate north by arrow.

↑ N

248869

Name of Well Contractor A. Kach D. Uglott	Well Contractor's Licence No. 1119
Address RR#1 Richmond, Ont	
Name of Well Technician Shannon Purcell	Well Technician's Licence No. 1212
Signature of Technician/Contractor [Signature]	Submission date 25 Oct 03

MINISTRY USE ONLY	Data source	58 Contractor	59-62	Date received	63-66
		1119		MAR 31 2003	
	Date of inspection	Inspector			
	Remarks				
	CSS.ES3				

Well Owner's Information

First Name Airport Golfland		Last Name		E-mail Address				<input type="checkbox"/> Well Constructed by Well Owner		
Mailing Address (Street Number/Name, RR) 6357 Emerald Links			Municipality Greely		Province Ontario		Postal Code K 4 P 1M 4 613 850 5468		Telephone No. (inc. area code)	

Part A Construction and/or Major Alteration of a Well

Address of Well Location (Street Number/Name, RR)		Township		Lot		Concession	
Hwy 31		Gloucester		20		5	
County/District/Municipality		City/Town/Village				Province	
Ottawa Carleton		Gloucester				Ontario	
UTM Coordinates		Zone		Easting		Northing	
NAD 83		18		453794		5018088	
GPS Unit Make		Model		Mode of Operation:			
Garmin				<input type="checkbox"/> Undifferentiated <input type="checkbox"/> Differentiated, specify _____			
				<input checked="" type="checkbox"/> Averaged			

Overburden and Bedrock Materials (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth From	Depth (Metres) To
Brown	Clay	Stones	Packed	0	3.35
Grey	Limestone		Broken	3.35	4.57
Grey	Limestone		Medium Hard	4.57	42.66
Grey	Limestone	Sandstone Layers	Hard	42.66	52.72

Annular Space/Abandonment Sealing Record

Depth Set at (Metres)		Type of Sealant Used (Material and Type)	Volume Placed (Cubic Metres)
From	To		
6.40	0	Grouted Bentonite Slurry	.132m ³

Method of Construction

<input type="checkbox"/> Cable Tool	<input type="checkbox"/> Diamond	<input type="checkbox"/> Public	<input type="checkbox"/> Commercial	<input type="checkbox"/> Not used
<input type="checkbox"/> Rotary (Conventional)	<input type="checkbox"/> Jetting	<input checked="" type="checkbox"/> Domestic	<input type="checkbox"/> Municipal	<input type="checkbox"/> Dewatering
<input type="checkbox"/> Rotary (Reverse)	<input type="checkbox"/> Driving	<input type="checkbox"/> Livestock	<input type="checkbox"/> Test Hole	<input type="checkbox"/> Monitoring
<input checked="" type="checkbox"/> Rotary (Air)	<input type="checkbox"/> Digging	<input type="checkbox"/> Irrigation	<input type="checkbox"/> Cooling & Air Conditioning	
<input type="checkbox"/> Air percussion	<input type="checkbox"/> Boring	<input type="checkbox"/> Industrial		
<input type="checkbox"/> Other, specify _____		<input type="checkbox"/> Other, specify _____		

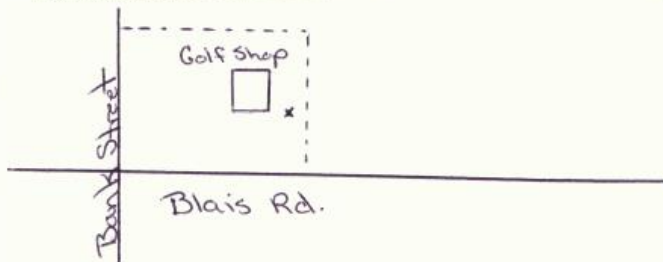
Status of Well

<input checked="" type="checkbox"/> Water Supply	<input type="checkbox"/> Dewatering Well	<input type="checkbox"/> Observation and/or Monitoring Hole
<input type="checkbox"/> Replacement Well	<input type="checkbox"/> Abandoned, Insufficient Supply	<input type="checkbox"/> Alteration (Construction)
<input type="checkbox"/> Test Hole	<input type="checkbox"/> Abandoned, Poor Water Quality	<input type="checkbox"/> Other, specify _____
<input type="checkbox"/> Recharge Well	<input type="checkbox"/> Abandoned, other, specify _____	

Location of Well

Please provide a map below showing:

- all property boundaries, and measurements sufficient to locate the well in relation to fixed points,
- an arrow indicating the North direction
- detailed drawings can be provided as attachments no larger than legal size (8.5" by 14")
- digital pictures of inside of well can also be provided



Results of Well Yield Testing

Check box if after test of well yield, water was: <input checked="" type="checkbox"/> Clear and sand free <input type="checkbox"/> Cannot develop to sand-free state	Draw Down		Recovery	
	Time (Min)	Water Level (Metres)	Time (Min)	Water Level (Metres)
	Static Level	4.75	Static Level	
If pumping discontinued, give reason:	1	6.42	1	18.19
Pumping test method	2	8.55	2	17.26
Submersible	3	9.96	3	15.67
Pump intake set at (Metres)	4	11.18	4	14.50
45.71	5	12.29	5	13.32
Pumping rate (Litres/min)	10	16.10	10	9.44
54.6	15	18.20	15	7.38
Duration of pumping	20	19.51	20	6.24
3 hrs + min	25	20.36	25	5.61
Final water level end of pumping	30	20.94	30	5.18
(Metres)	40	21.64	40	4.75
Recommended pump type	50	22.01	50	
<input type="checkbox"/> Shallow <input checked="" type="checkbox"/> Deep	60	22.14	60	
Recommended pump depth				
30.47 Metres				
Recommended pump rate				
(Litres/min)				
45.5				
If flowing give rate				
(Litres/min)				

Water Details

Water found at Depth 51.50 Metres <input type="checkbox"/> Gas	Kind of Water <input type="checkbox"/> Fresh <input type="checkbox"/> Salty <input type="checkbox"/> Sulphur <input type="checkbox"/> Minerals
Water found at Depth Metres <input type="checkbox"/> Gas	Kind of Water <input type="checkbox"/> Fresh <input type="checkbox"/> Salty <input type="checkbox"/> Sulphur <input type="checkbox"/> Minerals
Water found at Depth Metres <input type="checkbox"/> Gas	Kind of Water <input type="checkbox"/> Fresh <input type="checkbox"/> Salty <input type="checkbox"/> Sulphur <input type="checkbox"/> Minerals

Casing Used

☐ Galvanized
☒ Steel
☐ Fibreglass
☐ Plastic
☐ Concrete

Screen Used

☐ Galvanized
☐ Steel
☐ Fibreglass
☐ Plastic
☐ Concrete

Casing and Well Details

Diameter of the Hole (Centimetres)	15.39
Depth of the Hole (Metres)	52.72
Wall Thickness (Metres)	.48

No Casing and Screen Used

<input type="checkbox"/> Open Hole	Inside Diameter of the Casing (Metres) 15.86
Disinfected? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	Depth of the Casing (Metres) + .45 to 6.40

Ministry Use Only

Audit No. z 77392	Well Contractor No.
Date Received (yyyy/mm/dd) OCT 14 2009	Date of Inspection (yyyy/mm/dd)
Remarks	

Date Well Completed (yyyy/mm/dd)	Was the well owner's information package delivered? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	Date the Well Record and Package Delivered to Well Owner (yyyy/mm/dd)
2008/07/14		2008/07/15

Well Contractor and Well Technician Information

Business Name of Well Contractor		Well Contractor's Licence No.	
Capital Water Supply Ltd.		1 5 5 8	
Business Address (Street No./Name, number, RR)		Municipality	
Box 490		Stittsville	
Province	Postal Code	Business E-mail Address	
Ontario	K2S 1A6	office@capitalwater.ca	
Bus.Telephone No. (inc. area code)		Name of Well Technician (Last Name, First Name)	
613 836 1766		Miller, Stephen	
Well Technician's Licence No.		Signature of Technician	
0 0 0 0 7			
		Date Submitted (yyyy/mm/dd)	
		2008/07/16	