

VIA Email: lloyd@lloydphillips.com

June 22, 2017

Our File Ref.: 170132.02

The Hindu Temple of Ottawa Carlton Inc. c/o Lloyd Phillips & Associates Ltd. 1827 Woodward Drive, Suite 109, Ottawa. Ontario K2C 0P9

Attention: Mr. Lloyd Phillips

Subject: Terrain Analysis - Proposed Assembly Hall

The Hindu Temple of Ottawa Carlton, 4835 Bank Street, Ottawa, Ontario

Dear Mr. Phillips,

LRL Associates Ltd. (LRL) has conducted a Terrain Analysis for the proposed Hindu Temple of Ottawa Carlton Assembly Hall to be construction on the property located at 4835 Bank Street, Ottawa, Ontario (herein referred to as the "Site"). It is understood that it is proposed that a 2,100 m² Assembly Hall will be constructed at the eastern portion of the existing developed property which will have an available capacity of approximately 600 individuals, increasing the total occupancy of the site to an estimated 850.

The proposed Assembly Hall will be supplied by municipal water supply and a private septic as is the existing development on the Site.

The assessment was carried out to determine if the proposed development:

- Has soil conditions that are suitable for onsite sewage disposal; and
- Will not impair the use of groundwater resources on the Site or on adjacent lands.

The assessment was conducted according to Ontario Ministry of the Environment's and Climate Change (MOECC) "Hydrogeological Technical Information Requirements for Land Development Applications" (April 1995), which include the following guidelines and procedures:

- Guideline D-5 Planning for Sewage and Water Services (August 1996); and
- Procedure D-5-4 Technical Guideline for Individual On-site Sewage Systems: Water Quality Impact Risk Assessment (August 1996).

The assessment involved a desktop review of available information on the geology and hydrogeology of the Site and adjacent lands. The Site is serviced by municipal water supply, therefore a review of the local potential supply aquifer was not carried out. Although deemed not



Mr. L. Phillips

LRL File: 170132.04

June 22, 2017

Page 2 of 9

necessary, neighbouring properties within 250 m of the Site were found to have records of supply wells present.

### 1 SITE AND AREA DESCRIPTION

The property is situated at the southern extents of the City of Ottawa at 4385 Bank Street. The property is legally described as Part Lot 22, Concession 5RF Gloucester Parts 1 & 2, 5R3156. The location of the subject site is shown in **Figure 1**. The Site's area is approximately 3.8 hectares (9.4 acres). The property is currently occupied by The Hindu Temple of Ottawa Carleton, approximately 1,060 m², which encompasses the western and central portions of the Site. The existing Temple is located at the western extent of the Site with the associated septic systems to the north and south of the Temple. Based on the previously prepared Use Permit, dated December 5, 1985, issued by the Ontario Ministry of the Environmental and Climate Change, and associated application, the existing septic systems were designed for 3,750 L/day, assuming occupancy of 250 individuals and the use of 15 L/day per individual.

The neighbouring land use is as follows:

- Bank street, followed by light industrial/commercial business to the west; and
- Vacant/treed land to the north, south and east.

The topography of the land is generally flat with an approximate elevation of 97 m above mean sea level.

These site features are shown in the **Figure 2**.

### 2 PROPOSED DEVELOPMENT

It is anticipated that an Assembly Hall be constructed at the eastern portion of the Site with the associated septic along the south of the proposed structure. Water supply will be obtained from municipal services. According to the City of Ottawa by-law requirements, and the proposed size of the new development and existing development (2,100 and 1,060 m², respectively), a total of 153 parking spaces will be required. No additional parking spaces are proposed as the current development on the Site is equipped with 176 parking spaces.

The proposed Assembly Hall is anticipated to include a dining area, a kitchen, lobby and two (2) halls. The proposed development will be equipped with a full basement as well.

The proposed development plan is shown in the **Figure 3**.

### 3 FIELDWORK

On May 8<sup>th</sup>, 2017, eight (8) test pits were advanced across the Site. The test pits were placed around the general perimeter of the Site so not to disrupt existing Site activities and services. The rational for the test pits were to determine the general upper soil and perched water conditions. The test pits were advanced using a backhoe operated by a local contractor (Yelle Excavation, Ottawa) and under direct supervision by LRL field staff. The locations of the test pits are presented in **Figure 4** with the Test Pit Logs included in **Appendix A**.

An open tube piezometer was installed in select test pits locations (TP1, TP3, TP5 and TP7) to allow for the groundwater elevation measurement and sampling of the perched water found in the overburden, herein referred to as groundwater. Groundwater samples were collected from each

Mr. L. Phillips LRL File: 170132.04
June 22, 2017 Page 3 of 9

of the piezometers on May 8th, 2017, with the exception to TP5 which was found to have insufficient water available for sampling (i.e. dry). The samples collected were submitted for laboratory analyses for select nitrate species parameters. The laboratory Certificate of Analysis is included in Appendix B.

A soil sample from select test pits was submitted to LRL's material testing laboratory for sieve and hydrometer analyses. The sieve and hydrometer analysis certificates are included in Appendix C.

A ground surface elevation survey was carried out at each test pit location to obtain the elevation of the test pit ground surface and the piezometer stick-up. These elevations would aid in determining the groundwater elevations across the Site. An arbitrary benchmark was established at the top of the east arm of the hydrant located along the west of the southern entrance to the Site. The benchmark was given an elevation of 100.00 m. The elevations of summarized in **Table 1** and are presented in the Test Pit logs included in **Appendix A**.

### 4 Topography, Geology and Hydrogeology

Local topography indicates that the inferred overburden groundwater flow direction is east towards the North Castor River. The nearest open water body to the Site is an unnamed tributary that flows into the North Castor River, approximately 1.1 km east of the Site.

Surficial soil deposit mapping<sup>1</sup> indicates that the overburden consists of till, plain with local relief less than 5 m. Bedrock mapping<sup>2</sup> indicates that the underlying bedrock consists of dolomite and limestone, part of the Oxford Formation.

The test pits completed across the Site were found to have a thin layer of topsoil over fill material which extended to depths between 0.7 and 1.5 m thick. The fill was followed by silty sandy till with the exception to TP2, TP4, TP5 and TP6. TP2 was terminated in the fill stratum at a depth of 0.9 m bgs due to significant amounts of water infiltration, TP4 and TP5 were terminated over bedrock at a depth of 1.4 and 1.5 m, respectively, following the fill stratum and TP6 which encountered bedrock at a depth of 0.8 m bgs following the fill material. The remaining test pits extended into till followed by bedrock refusal at depths between 1.7 and 2.1 m bgs. With the exception of TP7 which extended to 1.8 m bgs, and was terminated due to a large stump limiting the excavation to extend further.

A representative till sample collected during the test pitting activities (TP3-6) was submitted for sieve analysis. The till sample was measured to be 39% Silt & Clay, 40% Sand, and 21% Gravel. A second representative till sample collected (TP1-3) was submitted for hydrometer analysis. The sample was measured to be 22% clay, 64% silt, 9% sand and 5% gravel. These results are presented in the sieve and hydrometer analysis certificates are included in **Appendix C** and are summarized in **Table 2**.

A search was conducted of the available well records from the MOECC Water Well Record Department. The search by UTM coordinates covered a 250 m radius from the site. The search

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<sup>&</sup>lt;sup>1</sup> St-Onge, D.A. (compilation), 2009: Surficial geology, lower Ottawa valley, Ontario-Quebec; Geological Survey of Canada, Map 2140A, scale 1:125000

<sup>&</sup>lt;sup>2</sup> Harrison, J.E., 1976. Geological Survey of Canada, Generalized Bedrock Geology, Ottawa-Hull, Ontario and Quebec, Map 1508A, scale 1:125000.

Mr. L. Phillips

LRL File: 170132.04

June 22, 2017

Page 4 of 9

returned records for nine (9) wells. The well records are included in **Appendix D** and their locations are presented in **Figure 5**.

Review of the records of the wells within 250 m of the site retrieved revealed that the wells are drilled wells extending to depths between 13.7 and 27.1 m. The well records shows that that the geological conditions within 250 m are relatively similar, and consist generally of mixed till materials including sand, clay, gravel and boulders from 0-7.6 m. Unidentified soil conditions, "muck" and "soil" were found as overburden descriptions in a couple of well records, as noted in the table below. Bedrock conditions varied slightly between limestone, sandstone and occasionally shale. Bedrock starting depths also vary from 1.8 to 7.6 m.

The general subsurface conditions indicated in the well records within 500 m of the site are as follows:

MOE			(	Overburden Detail	s	Bedrock Details	Groundwater	Static Water	Type of
Well Number	Direction from Site (m)	(m)	Sand/ Fill (m)	Clay/ Loam (m)	Gravel/ Till (m)	Bedrock	Encountered (m)	Level (m)	water
1502181	110 N	14.0			0 – 6.4	6.4- 14.0 (Limestone)	14.0	2.4	Fresh
1502179	50 W	27.1			0 – 4.8	4.8- 7.62 (Limestone) 7.62 – 27.1 (Sandstone)	27.1	6.1	Fresh
1513436	100 SW	15		0 – 3.6 "Soil"	3.6 – 4.8	4.8 – 15 (Limestone)	14.6	4.3	Fresh
1502180	140 S	16.8		0 – 1.8 "Loam"		1.8 – 16.8 (Limestone)	16.8	1.8	Fresh
1502177	195 S	18.2	0 – 2.1		2.1 – 6.1	6.1 – 18.2 (Sandstone)	18.2	1.8	Fresh
1512375	230 S	22.5	0 – 2.7			2.7 – 22.5 (Sandstone)	22.5	3.6	Fresh
1502176	160 NW	13.7		0 – 5.4		5.4 – 13.7 (Limestone)	13.7	1.8	Fresh
1512265	245 S	14.6		0 – 0.9		0.9 – 14.6 (Limestone)	2.4, 6.4, 10.3	1.2	Fresh
1514664	220 SW	15.2			0 – 3.9	3.9 – 9.1 (Shale) 9.1 – 38.1 (Limestone)	9.7, 16.7	6.1	Fresh

### 4.1 Groundwater from Test Pits

**Table 3** summarizes the water quality analysis from the test pit piezometers for nitrates, nitrites, ammonia and total kjeldahl nitrogen (TKN). The Laboratory Certificate of Analysis are included in **Appendix B**.

Nitrites were not detected (<0.05 mg/L) in any groundwater samples collected. Nitrate levels were found to be 0.5 mg/L in TP3 and <0.1 mg/L in both TP1 and TP5, below the ODWS of 10 mg/L. Ammonia was measured to be 0.28, 0.39 and 1.66 mg/L in TP2, TP3 and TP5, respectively. TKN values were reported as 78.1, 65.3 and 131 mg/L, in TP2, TP3 and TP5, respectively. There are no set ODWS for ammonia or TKN.

Mr. L. Phillips LRL File: 170132.04
June 22, 2017 Page 5 of 9

### 5 TERRAIN ANALYSIS

The terrain analysis was conducted to demonstrate that the unconsolidated material on the Site is appropriate for the construction of an on-site subsurface sewage disposal system, with consideration taken regarding the existing installation.

The subsurface conditions indicated for the site are considered suitable for a Class IV septic sewage disposal system with a fully raised leaching bed depending on the lot specific soil and groundwater conditions at the actual location of the proposed septic system leaching bed. The leaching bed should be constructed to conform to the specifications set out in the Ontario Building Code (OBC).

As part of this assessment, an analysis was carried out to ensure that sufficient space exists at the Site for the construction of a third septic system in accordance with the OBC which will service the proposed Assembly Hall.

As a conservative approach to determining the expected largest septic system envelope required to service the proposed Assembly Hall, a septic system envelope size was calculated assuming a fully raised bed with mantle, a percolation rate of 12 min/cm for the imported sand required and a daily sewage flow of 21,600 L.

The daily sewage flow was calculated assuming 850 persons visit the Site daily, of which approximately 600 individuals will occupy the proposed Assembly Hall. In accordance with Schedule 8 of the OBC, it is assumed that each individual which occupies the Site will discharge 36 L/day into the septic system. This is the set value for an Assembly Hall equipped with a kitchen facility. Both the existing and the proposed buildings are equipped, or will be equipped with a kitchen. As previously mentioned, the existing Temple is serviced with two (2) septic systems located at the north and south sides of the buildings, each constructed with 9,000 L fibreglass septic tanks and 8 runs of 13.3 m in length piping. The existing septic systems were designed for a combined sewage flow of 3,750 L/day, assuming occupancy of 250 individuals and the use of 15 L/day per individual.

The total length of pipe required for the proposed septic bed for the proposed Assembly Hall, assuming imported fill, was calculated as approximately 1,300 m:

L = QT/200

where L = length of pipe (m)

Q = daily sewage flow for the proposed assembly hall (L/day)

T = percolation rate of the imported sand fill material (min/cm)

Therefore an area of approximately 2,090 m² is required for the septic bed assuming 87 pipes each having a length of 15 m and a spacing of 1.6 m between the pipes. A mantle of 15 m in length would be required along the down gradient portion of the bed. Based on the total coverage of the septic bed (raised portion and mantle plus a replacement area) would be approximately 4,190 m².

The Site has a total area of 38,000 m<sup>2</sup>. However, when the area of the proposed and existing buildings, septic systems and other site features (parking facility), are taken into consideration, an area of approximately 32,000 m<sup>2</sup> is available for the installation of a septic system in

Mr. L. Phillips LRL File: 170132.04
June 22, 2017 Page 6 of 9

accordance with the OBC to service an assembly hall with a design sewage flow of up to 21,600 L/day. The proposed site development plan is shown in **Figure 3**.

### **6** GROUNDWATER IMPACT ASSESSMENT

The groundwater impact assessment addresses the ability of the land to attenuate the sewage effluent created by the development. Three methods for conducting the assessment are outlined in MOE's *Procedure D-5-4 Technical Guideline for Individual On-Site Sewage Systems: Water Quality Impact Risk Assessment* (1996):

- Lot Size Consideration for lot greater than 10 000 m<sup>2</sup> (1 hectare);
- *System Isolation Consideration* for areas where the septic system is hydrogeologically isolated from the potable water source; and
- Contaminate Attenuation Consideration for sites that do not meet the above two points.

Based on the review of the available information and site visit (above), the site is not obviously hydrogeologically sensitive (i.e. karstic areas, areas of fractured bedrock at the surface). Bedrock was encountered at depths between less than 2.0 m across the site, therefore the site is considered hydrogeologically sensitive with areas of thin soil over highly permeable soils (i.e. bedrock).

As mentioned above, the lot size is 38,000 m² with approximately 32,000 m² available for the installation of the proposed septic system. The lot size consideration for lots greater than 10 000 m² does not apply since the system is considered a large sewage disposal system based on the anticipated sewage demand of 25,350 L/day, including the existing system. Therefore, "Contamination Attenuation" was considered in this terrain analysis,

### **6.1 Contaminant Attenuation Method (Predictive Assessment)**

The Contaminant Attenuation Method (Predictive Assessment) was used to determine the impact of the proposed on-Site septic systems at the boundary of the Site. This procedure assesses the risk that the individual on-site systems will cause the concentration of the nitrate-nitrogen exceed 10 mg/L at the property boundaries. Dilution is the attenuation mechanism considered for nitrates, with precipitation being the only source of infiltration. The following parameters and assumptions were used in the nitrate attenuation calculations:

- Infiltration factors for the site;
  - Flat topography;
  - Infiltration Factors:
    - i. An assumption of Clay Loam was used for this calculation;
    - ii. Approximately 11,360 m<sup>2</sup> of the site is Wooded and the remaining 26,400 m<sup>2</sup> is considered Cultivated Land:
  - o Moisture Surplus:
    - i. The wooded land across an area of approximately 11,360 m² of the site is considered Closed Mature Forest cover, while the remaining cultivated land is considered Shallow Rooted Crops;
    - ii. Silt loam as defined by the sieve and hydrometer testing.
  - The average background nitrate concentration was calculated to be 0.2 mg/L;

Mr. L. Phillips LRL File: 170132.04
June 22, 2017 Page 7 of 9

o Impervious areas (existing and proposed) were calculated to be of 3,240 m<sup>2</sup> for the buildings and 1,790 m<sup>2</sup> of paved driveway and parking areas; and

 Moisture surplus values from the Ottawa weather station (Environment Canada, 2011).

The moisture surplus printout is included in **Appendix E**. This value is considered representative of the site located at the south-central extent of the City of Ottawa, Ontario.

The detailed calculations for the proposed development are presented in the attached **Table 4A**.

Based on the total proposed sewage volume for the entire Site of 25,350 L/day, the existing lot size and soil conditions, the calculated levels of nitrates at the property limits is estimated as 23.03 mg/L. This is above the procedure's guideline of 10.0 mg/L at the properties boundaries. Based on the "*Contaminant Attenuation Method*" the current lot size and soil conditions are not suitable to attenuate the nitrate impacts generated by the septic systems on the development in accordance with current D-5-4 guidelines..

The above calculations are based on the current D-5-4 guideline which requires the use of 40 mg/L as the contaminant source as per section 5.6.2 (a). A wastewater treatment system such as "Bionest" is an advanced tertiary treatment system which has reported reductions to the waste water effluent total nitrogen levels of 68.75%. These results were obtained through the BNQ 3680-910 certification for Bionest system models (SA-3 to SA-6). This particular system is approved by the OBC and the Building Materials Evaluation Commission of the Ontario Ministry of Municipal Affairs and Housing. Furthermore, section 5.7 of the D-5-4 guideline states that the ministry recognises "that as research continues, information and technologies may become available which warrant minor of substantial revisions to this guideline". It is unknown at this time whether revisions are to be considered because of this available technology; however it is LRL's professional opinion that this is a reasonable approach and should be considered as a possible solution.

If "Bionest" systems (or equivalent), which can achieve the reduced levels of total nitrogen of 12.5 mg/L versus the D-5-4 guideline of 40 mg/L are mandated for the proposed assembly hall the nitrogen levels at the property boundaries is reduced to 9.58 mg/L which is more satisfactory in regards to the 10.0 mg/L requirement, **Table 4B**. It is thus the **professional opinion of LRL that if the "Bionest" systems (or equivalent) are mandated for the proposed development the current lot size and soil conditions are suitable to attenuate the nitrogen impacts generated by the treatment systems on the development.** 

### 7 Conclusions

Based on our review of available information and the results of the groundwater sampling and laboratory analytical programs, we conclude the following:

- Sufficient area exists on the property for the installation of a septic system in accordance with the OBC to service the proposed Assembly Hall with a design sewage flow of up to 20,600 L/day.
- 2. In accordance with the D-5-4 guideline, the lot area of the Site is not of sufficient size to attenuate the impacts of the proposed septic system based on the "*Contaminant Attenuation Method*" using 40 mg/L as the contaminant source as per section 7.1.

Mr. L. Phillips LRL File: 170132.04
June 22, 2017 Page 8 of 9

3. If one is to consider mandating the installation of BNQ 3680-910 certified treatment systems of Bionest system models (SA-3 to SA-6) which has a documented and measured output of 12.5 mg/L nitrate-nitrogen concentrations, the lot size is of sufficient size to attenuate the impacts of the proposed septic system based on the "Contaminant Attenuation Method" using the 12.5 mg/L instead of the 40 mg/L as the contaminant source as per section 5.6.2(a). The Site would have calculated nitrate-nitrogen concentration at the boundary of 9.58 mg/L.

4. Records of domestic wells were retrieved within 250 m of the site. The potable water source of these wells is the bedrock aquifer. A thin layer of either clay, gravel or till, with some sand in areas, being between 0.9 and 7.6 m thick over bedrock.

### 8 RECOMMENDATIONS

- 1. As new technologies are available, such as the Bionest system discuss above, mandating the installation of BNQ 3680-910 certified treatment systems of Bionest system models (SA-3 to SA-6) or equivalent, which have had a documented and measured output of 12.5 mg/L nitrate-nitrogen concentrations, the Site will be of sufficient size to attenuate the impacts of the proposed septic system based on the "Contaminant Attenuation Method" using the 12.5 mg/L instead of the 40 mg/L as the contaminant source as per section 5.6.2 (a). The property would have calculated nitrate-nitrogen concentration at the boundary of 9.58 mg/L. It should be noted that even if the proposed system fails by a factor of 1.2 it still meets the D-5-4 guideline of 10 mg/L at the properties boundary.
- 2. The reviewing authorities should consider that the level of conservatism is quite significant barring that treatment systems with measured and quantifiable reductions in nitrate-nitrogen have been measured and approved. The evaluation of nitrate-nitrogen the "Contaminant Attenuation Method" is based on many factors which are typically used, which LRL has completed, in a conservative manner.
- 3. The septic system should be placed at least 15 m from any drilled wells/water service and 30 m from any dug well. It is recommended that the water table be surveyed prior to installation.
- 4. The daily volume, calculated using the 36 L/day per individual value set out in the OBC, is in excess of 10,000 L. Therefore an Environmental Compliance Approval, issued by the MOECC, may be required.

### 9 LIMITATIONS

The findings contained in this report are based on data and information collected during the Terrain Analysis of the subject property conducted by LRL Associates Ltd. The conclusions and recommendations are based solely on site conditions encountered at the time of our fieldwork on May 8<sup>th</sup>, 2017, supplemented by historical information and data obtained as described in this report. The information presented in this report represents the groundwater conditions at the locations sampled. Due to natural variations in geological conditions, no inference is made to the soil or groundwater conditions between sampling points. No assurance is made regarding changes in conditions subsequent to the time of this investigation. If additional information is discovered or obtained, LRL Associates Ltd. should be requested to re-evaluate the conclusions presented in this report and to provide amendments as required.

LRL File: 170132.04 Page 9 of 9

In evaluating the subject property, LRL Associates Ltd. has relied in good faith on information provided by individuals as noted in this report. We assume that the information provided is factual and accurate. We accept no responsibility for any deficiencies, misstatements or inaccuracies contained in this report as a result of omissions, misinterpretation or fraudulent acts of the persons contacted.

Yours truly,

LRL Associates Ltd.

Jessica Arthurs

Senior Environmental Technician

Matthew Whitney, P. Eng.



Encl.

Figure 1 – Site Location

Figure 2 - Site Plan

Figure 3 – Proposed Development Plan

Figure 4 - Test Pit Locations, Groundwater Elevations and Groundwater Contours

Figure 5 - Well Locations, Ontario Well Records

Table 1 – Summary of Groundwater Elevations in Test Pits

Table 2 – Summary of Sieve & Hydrometer Analyses

Table 3 - Summary of Analysis of Water Samples Collected From the Test Pits

Table 4A – Nitrate Attenuation Calculations

Table 4B - Nitrate Attenuation Calculations

Appendix A – Test Pit Logs

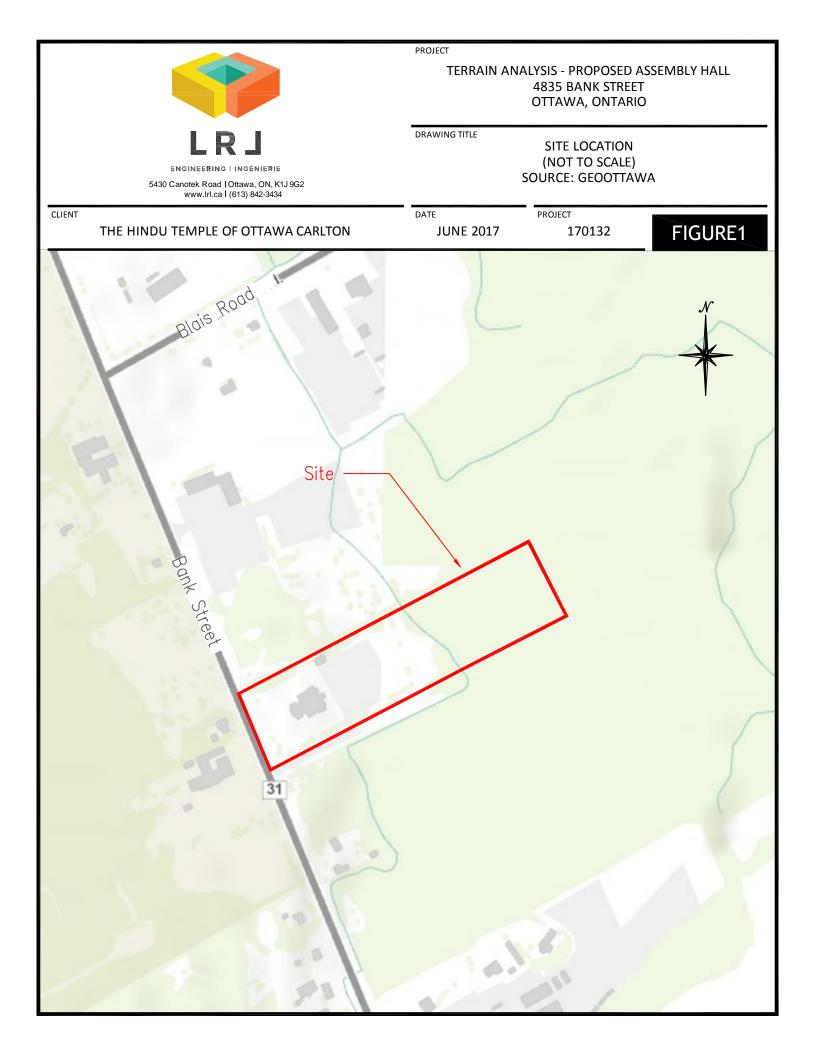
Appendix B – Laboratory Certificates of Analysis

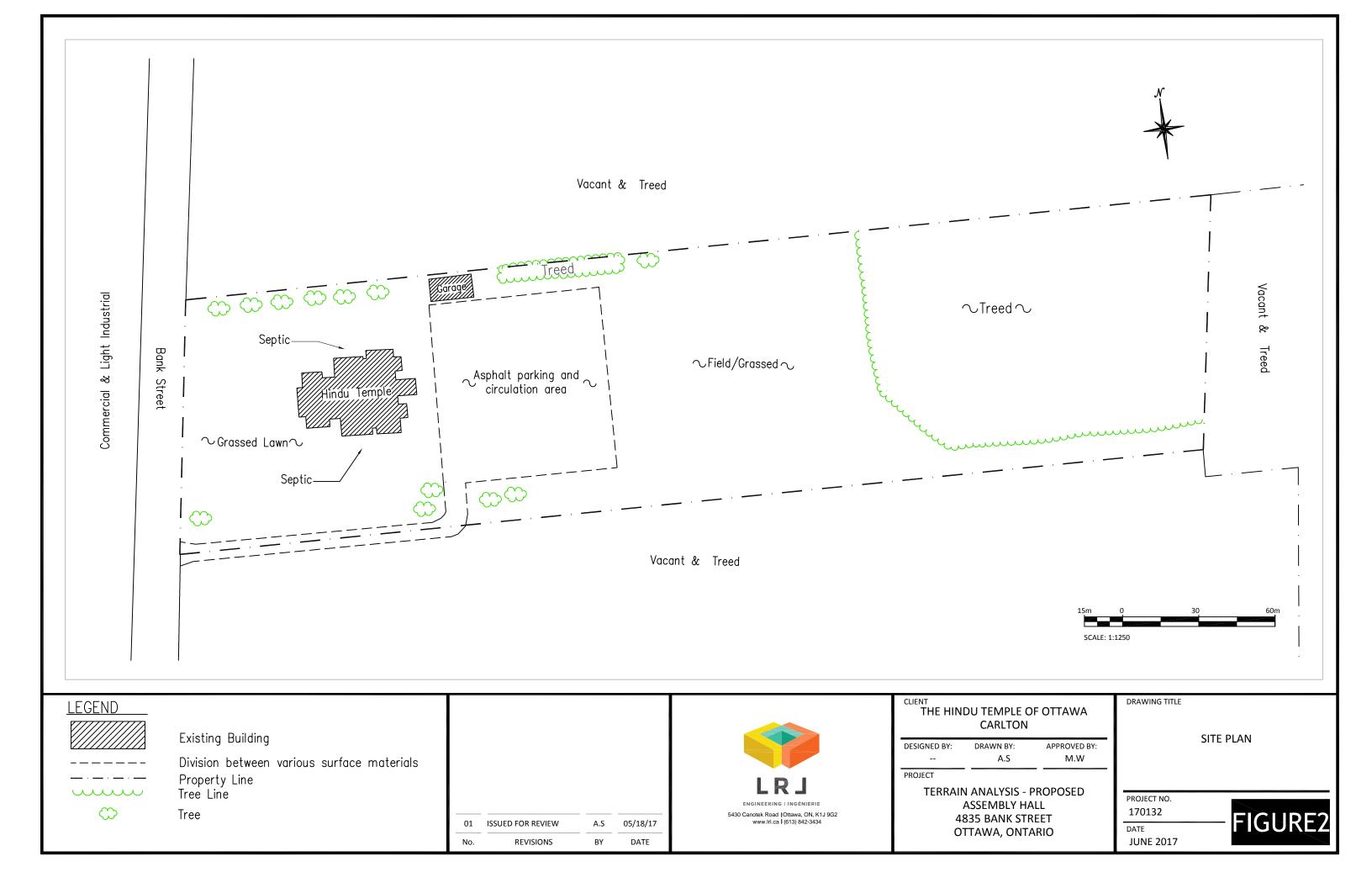
Appendix C – Sieve & Hydrometer Analysis

Appendix D – Ontario Well Record Printouts

Appendix E - Moisture Surplus Printouts









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CLIENT

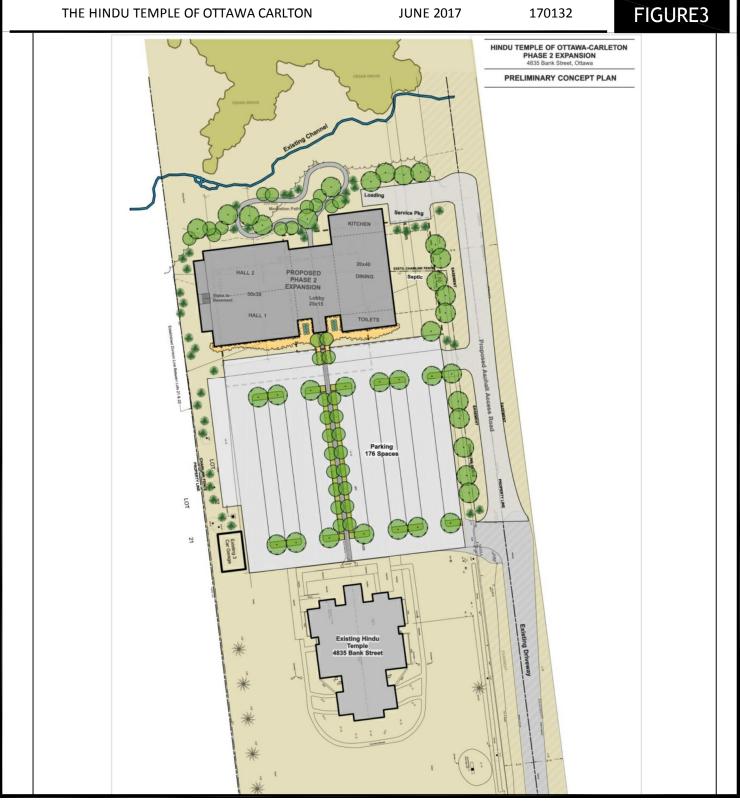
PROJECT

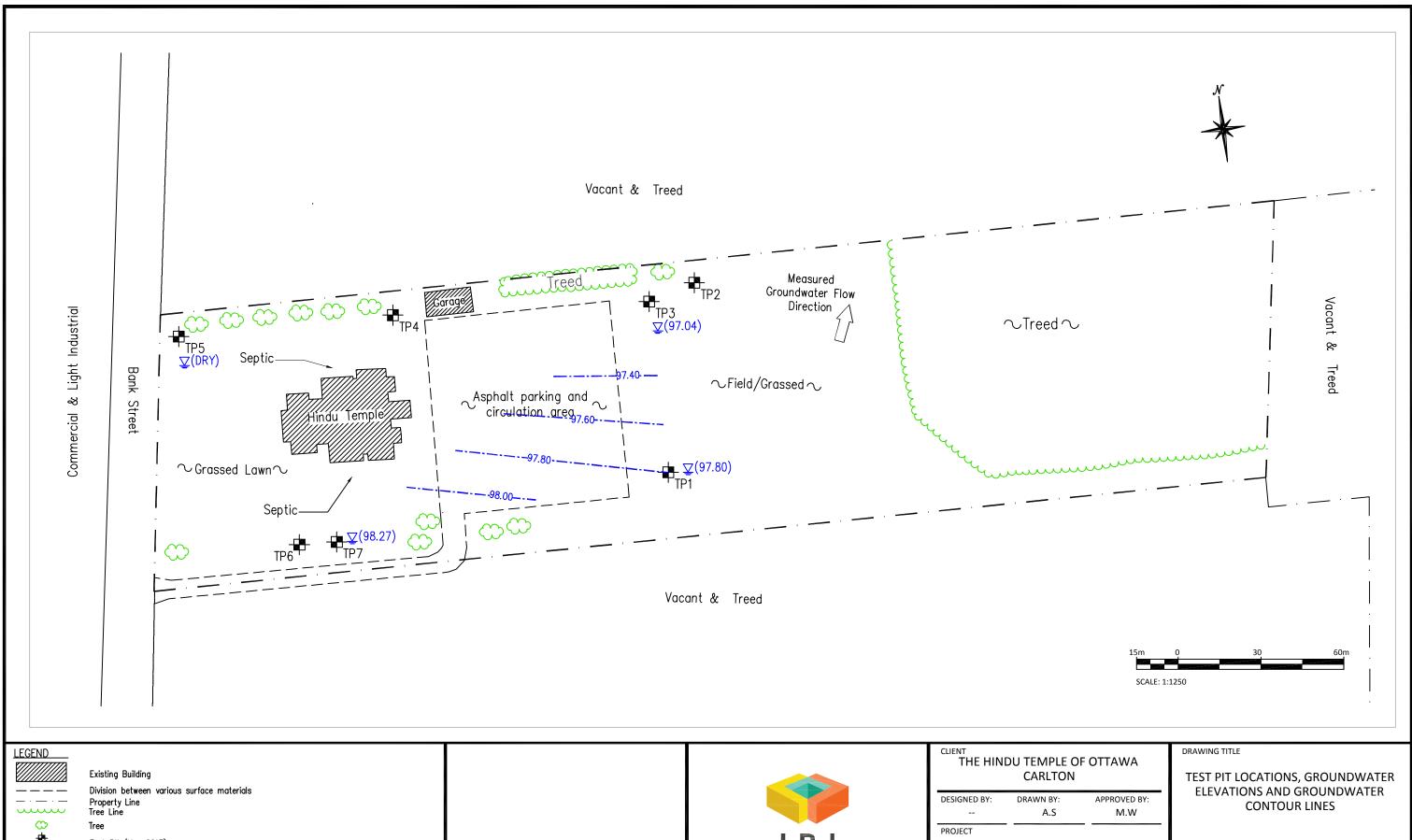
### TERRAIN ANALYSIS - PROPOSED ASSEMBLY HALL **4835 BANK STREET** OTTAWA, ONTARIO

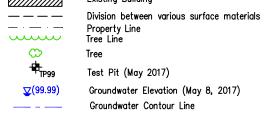
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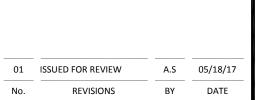
PROPOSED DEVELOPMENT PLAN SOURCE: LLOYD PHILLIPS & ASSOCIATES LTD. (NOT TO SCALE)

DATE PROJECT











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DESIGNED BY:	DRAWN BY:	APPROVED BY:								
	A.S M.W									
PROJECT										
TERRAIN	TERRAIN ANALYSIS - PROPOSED									
ASSEMBLY HALL										
4	4835 BΔNK STREET									

4835 BANK STREET OTTAWA, ONTARIO

PROJECT NO. 170132 DATE JUNE 2017





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### TERRAIN ANALYSIS - PROPOSED ASSEMBLY HALL 4835 BANK STREET OTTAWA, ONTARIO

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WELL LOCATIONS ONTARIO WELL RCORDS (NOT TO SCALE)

DATE PROJECT

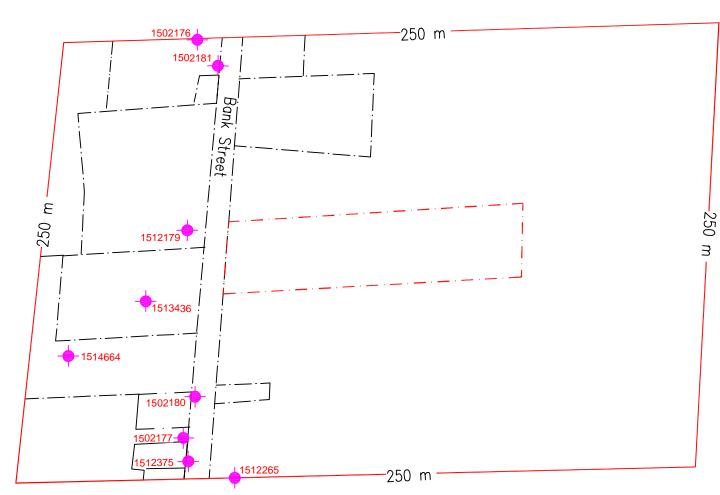
THE HINDU TEMPLE OF OTTAWA CARLTON

**JUNE 2017** 

170132

FIGURE 5





### <u>Legend</u>

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Well



Approximate Site Location

250 m radius from Site

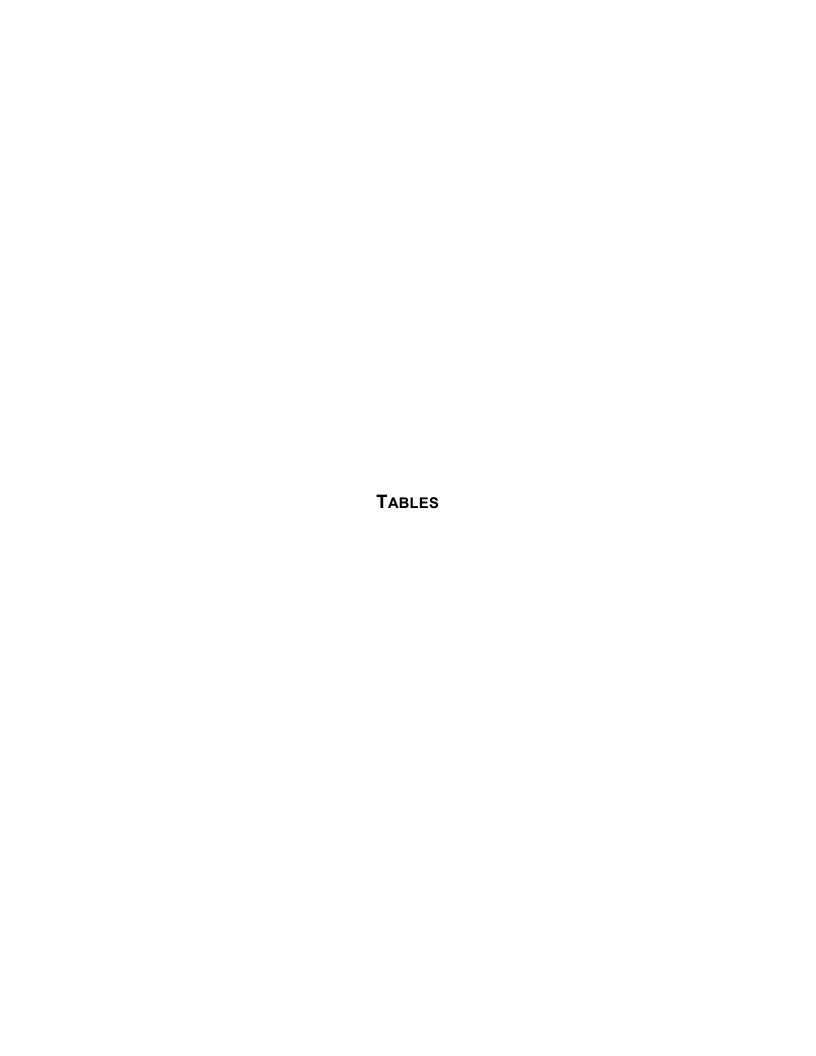


Table 1
Summary of Groundwater Elevations in Test Pits

Terrain Analysis - Proposed Assembly Hall 4835 Bank Street, Ottawa, Ontario

LRL File: 170132

	Ground Surface Elevation <sup>1</sup>	Reference Elevation <sup>2</sup>	Depth To Water Table (m)		Groundwater Elevation
Test Pit	(m)	(m)	Reference Point	Ground Surface	(m)
TP1	98.21	99.15	1.35	0.41	97.80
TP2	97.09				
TP3	97.75	98.98	1.94	0.71	97.04
TP4	99.54				
TP5	98.78	99.02	DRY		
TP6	99.38				
TP7	99.60	100.79	2.52	1.33	98.27

### NOTES

<sup>&</sup>lt;sub>1</sub> Elevations are based off of a temporary benchmark established at the top of the east arm of the fire hydrant along the southof the Site (100.00 m).

<sup>&</sup>lt;sup>2</sup> Reference elevation is top of piezometer.

## Table 2 Summary of Sieve & Hydrometer Analyses

Terrain Analysis - Proposed Assembly Hall Part of Lot 16, Concession 7, Hammond, Ontario

LRL File: 160833

			Perce					
		Gravel	Coarse	Sand Medium	Fine	Silt	Clay	
Sample	Depth (m)	>4.75 mm	2.0 - 4.75 mm	425 μm - 2.0 mm	75 - 425 μm	2 - 75 μm	< 2μm	Soil Texture Classification
TP1-3	1.8 - 2.0	4.8	1.2	1.8	6.5	63.8	22.0	Silt Loam
TP3-6	1.4 - 1.6	21.3	7.0	12.7	20.1	39	.0	Fine Silty Sand

NOTES:

<sup>&</sup>lt;sup>1</sup> Unified Soil Classification System

## Table 3 Summary of analysis of water samples collected from the test pits.

Terrain Analysis - Proposed Assembly Hall 4835 Bank Street, Ottawa, Ontario

LRL File: 170132

			Ontario Drir Stand	•	Sample		
Parameter	Units	MRL	Standard	Type	TP1	TP3	TP5
Sample Date (d/m/y)					05/08/2017	05/08/2017	05/08/2017
Ammonia	mg/L	0.01			0.28	0.39	1.66
Total Kjeldahl Nitrogen	mg/L	0.1			78.1	65.3	131
Nitrate as N	mg/L	0.1	10	MAC	<0.1	0.5	<0.1
Nitrite as N	mg/L	0.05	1	MAC	< 0.05	< 0.05	< 0.05

### NOTES

MAC Maximum Acceptable Concentration

MRL Minimum Reportable Limit

### Table 4A

#### **Nitrate Attenuation Calculations**

Terrain Analysis - Proposed Assembly Hall 4835 Bank Street, Ottawa, Ontario LRL File: 170132

### 1. Potential Infiltration

Weather Station Ottawa

			Infiltration Factor (IF) <sup>1</sup>						Moisture Surplus (MS)				Potential Infiltration (PI) (IF*MS) (mm)	
Part No.	Section Area (m²)	Topography	Value	Soil	Value	Cover	Value	Total	Ground Cover	Soil Type	Moisture Retention <sup>2</sup> (mm)	Moisture Surplus <sup>3</sup> (mm)		Weighted
												` ,		
Α	26,400	Flat	0.3	Clay Loam	0.2	Cultivated Land	0.1	0.6	Shallow Rooted Crops	3 Silt Loam	125	349	209.4	145.5
В	11,360	Flat	0.3	Clay Loam	0.2	Woodland	0.2	0.7	Closed Mature Forest	3 Silt Loam	400	301	210.7	63.0
Total	38,000												Total	208.5

### 2. Area Available for Infiltration

Number of Lots			n	1
Approximate footpi	rint of existing t	emple	Н	1060 m <sup>2</sup>
Approximate footpi	rint of existing of	garage	Н	80 m <sup>2</sup>
Approximate footpi	rint of propsed	Н	2100 m <sup>2</sup>	
Approximate area	of paved drivev	ď	1790 m²	
Approximate Lengt	th of Road	L	0 m	
Approximate Width	of Road		W	0 m
Total Area of Prope	erty			38000 m <sup>2</sup>
Impervious Area				5030.0 m <sup>2</sup>
	Roads	l x w	$0 \text{ m}^2$	
	Driveway	n x d	1790 m <sup>2</sup>	
	Buidling	n x H	3240 m <sup>2</sup>	
Area available Inf	iltration		A	32,970 m <sup>2</sup>

### 3. Nitrate Diluation Calculations

Nitrate Concentration of Infiltration	C <sub>i</sub>	0.2 mg/L <sup>6</sup>
Site Infiltration	$Q_i = A^*PI$	6873 m <sup>3</sup>
Daily Sewage Volume - Existing	$Q_{d,1}$	3.75 m <sup>3</sup>
Maximum Yearly Sewage Volume - Existing (water)	Q <sub>e,1</sub> =365*n*Q <sub>d</sub>	1369 m <sup>3</sup>
Nitrate Concentration in Sewage - Existing	C <sub>e,1</sub>	40 mg/L
Daily Sewage Volume - Proposed New Development	$Q_{d,2}$	21.6 m <sup>3</sup>
Maximum Yearly Sewage Volume (water) - Proposed New Developme	ent Q <sub>e,2</sub> =365*n*Q <sub>d</sub>	7884 m <sup>3</sup>
Nitrate Concentration in Sewage - Proposed New Development	C <sub>e,2</sub>	40 mg/L
Maximum Allowable Nitrate Concentration at Boundary	$C_m$	10.0 mg/L
Increase in Nitrate Concentration at Boundaries	$C = (Q_{e,1}C_{e,1} + Q_{e,2}C_{e,2} + Q_iC_i)/(Q_{e,1} + Q_{e,2} + Q_i)$	23.03 mg/L

### NOTES

- 1 Table 2: Infiltration Factors, Hydrogical Technical Information Requirements for Land Development Applications, Ministry of the Energy and Environment, April 1995.
- Thornthwaite and Mather's (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance.
- Moisture surplus for data for Mason Anger (Environment Canada Meteorological Service of Canada, 2010).

### Table 4B Nitrate Attenuation Calculations

Terrain Analysis - Proposed Assembly Hall 4835 Bank Street, Ottawa, Ontario LRL File: 170132

### 1. Potential Infiltration

Weather Station Ottawa

vvouin	Ci Otation	Ollawa												
			Infiltration Factor (IF) <sup>1</sup>						Moisture Surplus (MS)				Potential Infiltration (PI) (IF*MS) (mm)	
Part No.	Section	Tonography	Value	Soil	Value	Cover	Value	Total			Moisture Retention <sup>2</sup> (mm)	Moisture Surplus <sup>3</sup> (mm)	II I	Weighted
NO.	Area (m²)	Topography	value	5011	value	Cover	value	Total	Ground Cover	Soil Type	(mm)	(mm)	Section	weighted
Α	26,400	Flat	0.3	Clay Loam	0.2	Cultivated Land	0.1	0.6	Shallow Rooted Crops	3 Silt Loam	125	349	209.4	145.5
В	11,360	Flat	0.3	Clay Loam	0.2	Woodland	0.2	0.7	Closed Mature Forest	3 Silt Loam	400	301	210.7	63.0
Total	38,000												Total	208.5

### 2. Area Available for Infiltration

Number of Lots			n	1
Approximate footpi	rint of existing t	emple	Н	1060 m <sup>2</sup>
Approximate footpi	rint of existing of	garage	Н	80 m <sup>2</sup>
Approximate footpi	rint of propsed	Н	2100 m <sup>2</sup>	
Approximate area	of paved drivev	ď	1790 m²	
Approximate Lengt	th of Road	L	0 m	
Approximate Width	of Road		W	0 m
Total Area of Prope	erty			38000 m <sup>2</sup>
Impervious Area				5030.0 m <sup>2</sup>
	Roads	l x w	$0 \text{ m}^2$	
	Driveway	n x d	1790 m <sup>2</sup>	
	Buidling	n x H	3240 m <sup>2</sup>	
Area available Inf	iltration		A	32,970 m <sup>2</sup>

### 3. Nitrate Diluation Calculations

Nitrate Concentration of Infiltration	C <sub>i</sub>	0.2 mg/L <sup>6</sup>
Site Infiltration	$Q_i = A^*PI$	6873 m <sup>3</sup>
Daily Sewage Volume - Existing	$Q_{d,1}$	$3.75 \text{ m}^3$
Maximum Yearly Sewage Volume - Existing (water)	Q <sub>e,1</sub> =365*n*Q <sub>d</sub>	1369 m <sup>3</sup>
Nitrate Concentration in Sewage - Existing	C <sub>e,1</sub>	40 mg/L
Daily Sewage Volume - Proposed New Development	$Q_{d,2}$	21.6 m <sup>3</sup>
Maximum Yearly Sewage Volume (water) - Proposed New Developme	ent Q <sub>e,2</sub> =365*n*Q <sub>d</sub>	7884 m <sup>3</sup>
Nitrate Concentration in Sewage - Proposed New Development	C <sub>e,2</sub>	12.5 mg/L
Maximum Allowable Nitrate Concentration at Boundary	$C_m$	10.0 mg/L
Increase in Nitrate Concentration at Boundaries	$C = (Q_{e,1}C_{e,1} + Q_{e,2}C_{e,2} + Q_iC_i)/(Q_{e,1} + Q_{e,2} + Q_i)$	9.58 mg/L

### NOTES

- 1 Table 2: Infiltration Factors, Hydrogical Technical Information Requirements for Land Development Applications, Ministry of the Energy and Environment, April 1995.
- Thornthwaite and Mather's (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance.
- Moisture surplus for data for Mason Anger (Environment Canada Meteorological Service of Canada, 2010).

APPENDIX A
Test Pit Logs



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

**Project:** Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation ltd.

;	SUBSURFACE PROFILE	SAI	MPLE D	ATA		Water Content	
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa) 50 150	Valer Content  v (%)  25 50 75   Liquid Limit  (%)  25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m	Ground Surface	98.21 0.00					[ 6
	TOPSOIL Sandy, dark brown, dry.	98.01					Js (08/05/1
1	FILL Sandy clay, dark brown, dry.	97.31					
3-	Silty Sand Trace clay, with clay seam from 1.7	97.31 0.90			-		
<u> </u> 1	to 1.8 m bgs, brown, dry.		-	1			
4	Sieve analysis completed.						
-			_	2			
6							
-		96.11		3			
7	End of Test Pit	2.10					
_	Refusal over inferred bedrock.						
8-							
Easti	ing: N/M	Northing:	: N/M		NOTES:	I	I
	Datum: Top east arm of hydrant at south e				BGS- E	Below Ground Surface	
		Top of Ri					
Exca	vation Width: 1.2 m	Excavation	on Length	<b>1:</b> 1.5 m			



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

**Excavation Contractor:** Maurice Yelle Excavation ltd.

Soil Description  Soil Description  Ground Surface FILL Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs.  Burled metal structure/waste at approximately 0.9 m bgs.  End of Test Pit  Soil Description  Ground Surface FILL Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs.  Burled metal structure/waste at approximately 0.9 m bgs.  End of Test Pit  Soil Description  Ground Surface FILL Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs.  Burled metal structure/waste at approximately 0.9 m bgs.	SUE	BSURFACE PROFILE	SAI	MPLE D	ATA								
Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs.  Buried metal structure/waste at approximately 0.9 m bgs.  End of Test Pit  4	Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	near Str (kPa 1	rength ) 50	25 L	(%) 50 .iquid Li	75 <sup>▽</sup>	Water Level (Standpipe o Open Excavati	r on)	
End of Test Pit  End of Test Pit  5	- S S N N - E A	FILL Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs. Buried metal structure/waste at	97.09										
		End of Test Pit	96.19		4								
	- - - 2 - 7- - - - -												

Site Datum: Top east arm of hydrant at south entrance (100.00 m)

**Groundsurface Elevation: 97.09** Top of Riser Elev.: --

Excavation Width: 1.2 m Excavation Length: 1.5 m Test pit terminated at 0.9 meters due to volume of water in

pit. BGS- Below Ground Surface



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

**Project:** Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Excavation Contractor: Maurice Yelle Excavation ltd.

S	UBSURFACE PROFILE	SAI	MPLE C	ATA			Motor Contont	
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear S (kP	<b>trength</b> (a) 150	Water Content  ∇ (%)  25 50 75  Liquid Limit  (%)  25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m	Ground Surface	97.75						
0 — 0 — 1 — 1 — 1 — 1 — 1 — 1 — 1 — 1 —	TOPSOIL Sandy loam, dark brown, dry. Brick debris found in top 0.2 m bgs. FILL Sandy silt, trace boulders, brown, dry. Tire debris found at approximately 0.8 m bgs.  TILL Silty sand, trace gravel, cobbles and boulders, brown, dry. Sieve analysis completed.  End of Test Pit Refusal at 1.7 m bgs over inferred bedrock.	96.95 0.80 96.05 1.70		6				▲ 0.71 m bgs (08/05/17)
Factin	ig: 0454091	Northing	: 5017670	)		NOTES:		
Site D Grour	atum: Top east arm of hydrant at south endsurface Elevation: 97.75	ntrance (1		: 98.98			ow Ground Surface	



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

La of Ottoma Ocaleta and La ocalism 4005 D

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

**Project:** Terrain Analysis

Excavation Contractor: Maurice Yelle Excavation Itd.

S	UBSURFACE PROFILE	SAMPLE DATA					Water Content		
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear S (kF 50	Strength Pa) 150	⊽ 25 Liqu	(%) \$\sigma\$ 50 75  id Limit (%) \$\sigma\$ 50 75	Water Level (Standpipe or Open Excavation)
0 ft m	Ground Surface	99.54							
1-	TOPSOIL Silty loam, trace clay,dark brown, dry.	0.00	<i>ખુખખુખખુખખુખ</i>						
2—	FILL Silty sand, trace cobbles and gravel, light brown, dry.	99.04 0.50							
<u> </u>	Changing to dark brown sandy fill with trace boulders at approximately 0.8 m bgs.			7					
3-1									
4-		08 14		8	-				
5	End of Test Pit Refusal at 1.4 m bgs over inferred bedrock or large concrete structure.	98.14 1.40							
6-									
7-									
8—									
Ecoti-	Faction 0454005					NOTES:			
	Easting: 0454005 Northing: 5017628  Site Datum: Top east arm of hydrant at south entrance (100.00 m)					S- Below G	round Surfac	e	
Groun	dsurface Elevation: 99.54	Γορ of Ri	ser Elev	.:					
Excava	Excavation Width: N/M Excavation Length: N/M								



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

\_\_\_\_

Field Personnel: JA

**Project:** Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

**Excavation Method:** Backhoe **Excavation Contractor:** Maurice Yelle Excavation Itd.

	SUBSURFACE PROFILE	SAI	MPLE [	DATA			Water Content	
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear St (kP)	rength a) 150	Water Content  ∇ (%) ∇ 25 50 75  Liquid Limit  □ (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m	Ground Surface	98.78 0.00	~ ~ .					
-	TOPSOIL Silty loam some sand, dark brown, dry.	98.63 0.15		10	_			
1-	FILL Sand, some silt, trace cobbles, brown, dry.  Waste debris of metal and asphalt pieces at approximately 0.9 m bgs.	0.15		10				
3- 3- 1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	1			9	-			it 1.53 m bgs
5-	End of Test Pit Refusal at 1.5 m bgs over inferred bedrock.	97.28 1.50		11	-			☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐☐
6	2							
8-								
Site	Easting: 0453945 Northing: 5017595  Site Datum: Top east arm of hydrant at south entrance (100.00 m)			<u> </u>	NOTES: BGS	- Below Ground Surface		
	Groundsurface Elevation: 98.78 Top of Riser Elev.: 99.02  Excavation Width: N/M Excavation Length: N/M							



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

Project: Terrain Analysis

Excavation Contractor: Maurice Yelle Excavation ltd.

S	SUBSURFACE PROFILE SAMPLE DATA				Water Content			
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear Strength (kPa) 50 150	▼ (%) ▼ 25 50 75	Water Level (Standpipe or Open Excavation)	
0 ft m	Ground Surface	99.38						
0 — 0 - - - - - 1 —	TOPSOIL Sandy loam, dark brown, dry.  FILL Sand, some gravel, cobbles, boulders, silty seam at 0.7 m bgs, brown, dry.	0.00 99.23 0.15						
2-	Refusal at 0.8 m bgs over inferred bedrock.							
-				12	-			
	End of Test Pit	98.58 0.80	7:7:	13				
5								
2 7 								
Easting	<b>Easting:</b> 0454003 <b>Northing:</b> 5017542				I	NOTES:		
	atum: Top east arm of hydrant at south					BG:	S- Below Ground Surf	ace
		Top of Ri						



**Project No.:** 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

**Project:** Terrain Analysis

Excavation Method: Backhoe Excavation Contractor: Maurice Yelle Excavation ltd.

	SUBSURFACE PROFILE	SAI	MPLE DATA		Water Content	
Depth	Soil Description	Elev./Depth (m)	Lithology Sample Number	<b>Shear Stre</b> (kPa) 50 15	water Content  √ (%) √ 25 50 75  congth  Liquid Limit  (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
o ft m	Ground Surface	99.60				
1— 1— 2— 3— 4— 5— 6— —————————————————————————————	FILL Sand, brown, trace metal debris, dry.  TILL Silty sand, trace clay, boulders, grey, organics including tree stump, roots, blanes including tree stump, roots, bgstump).	99.40 0.20 98.90 0.70 97.80 1.80				
7						
East	ting: 0454051	\ Northina	: 5017564	NC	DTES:	
Site Gro	Datum: Top east arm of hydrant at south er undsurface Elevation: 99.60	ntrance (1			BGS- Below Ground Surfac	e



# Symbols and Terms Used on Borehole and Test Pit Logs

The following explains the data presented in the borehole and test pit logs.

### 1. Soil Description

The soil descriptions presented in this report are based on commonly accepted methods of classification and identification employed in geotechnical practice. Classification and identification of soil involves some judgement and LRL Associates Ltd. does not guarantee descriptions as exact, but infers accuracy to the extent that is common in current geotechnical practice. Boundaries between zones on the logs are often not distinct but transitional and were interpreted.

### a. Proportion

The proportion of each constituent part, as defined by the grain size distribution, is denoted by the following terms:

Term	Proportions
%bace+	1% to 10%
‰me+	10% to 20%
prefix	20% to 35%
(i.e. ‰andy+silt)	
%and+	35% to 50%
(i.e. sand %and+gravel)	

### b. Compactness and Consistency

The state of compactness of granular soils is defined on the basis of the Standard Penetration Test. See Section 2c for more details. The consistency of clayey or cohesive soils is based on the shear strength of the soil, as determined by field vane tests and by a visual and tactile assessment of the soil strength.

The state of compactness of granular soils is defined by the following terms:

State of	Standard
Compactness	Penetration
Granular Soils	Number "N"
Very loose	0.4
Loose	4 . 10
Compact or medium	10 - 30
Dense	30 - 50
Very dense	over - 50

The consistency of cohesive soils is defined by the following terms:

Consistency Cohesive Soils	Undrained Shear Strength (Cu) (kPa)
Very soft	under 10
Soft	10 - 25
Medium or firm	25 - 50
Stiff	50 - 100
Very stiff	100 - 200
Hard	over - 200

### 2. Sample Data

### a. Elevation depth

This is a reference to the geodesic elevation of the soil or to a benchmark of an arbitrary elevation at the location of the borehole or test pit. The depth of geological boundaries is measured from ground surface.

### b. Type

Symbol	Туре	Letter Code
1	Auger	AU
X	Split spoon	SS
	Shelby tube	ST
И	Rock Core	RC

### c. Sample Number

Each sample taken from the borehole is numbered in the field as shown in this column.

LETTER CODE (as above) . Sample Number

### d. Blows (N) or RQD

This column indicates the Standard Penetration Number (N) as per ASTM D-1586. This is used to determine the state of compactness of the soil sampled. It corresponds to the number of blows

required to drive 300 mm of the split spoon sampler using a 622 kg\*m/s² hammer falling freely from a height of 760 mm. For a 600 mm long split spoon, the blow counts are recorded for every 150 mm. The %N+ index is obtained by adding the number of blows from the 2<sup>nd</sup> and 3<sup>rd</sup> count. Technical refusal indicates a number of blows greater than 50.

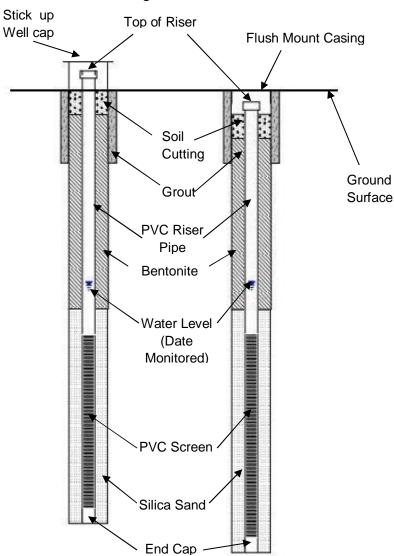
In the case of rock, this column presents the Rock Quality Designation (RQD). The RQD is calculated as the cumulative length of rock pieces recovered having lengths of 10 cm or more divided by the length of coring. The qualitative description of the bedrock based on RQD is given below.

Rock Quality Designation (RQD) (%)	Description of Rock Quality
0 . 25	very poor
25 . 50	poor
50 . 75	fair
75 . 90	good
90 . 100	excellent

### e. Recovery (%)

For soil samples this is the percentage of the recovered sample obtained versus the length sampled. In the case of rock, the percentage is the length of rock core recovered compared to the length of the drill run.

### 3. General Monitoring Well Data



# APPENDIX B Laboratory Certificates of Analysis



300 - 2319 St. Laurent Blvd Ottawa, ON, K1G 4J8 1-800-749-1947 www.paracellabs.com

### Certificate of Analysis

### LRL Associates Ltd.

5430 Canotek Road Ottawa, ON K1J 9G2 Attn: Jessica Arthurs

Client PO:

Project: 170132 Report Date: 15-May-2017 Custody: 32310 Order Date: 11-May-2017

Order #: 1719377

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1719377-01	TP1
1719377-02	TP3
1719377-03	TP7

Approved By:



Dale Robertson, BSc Laboratory Director



Certificate of Analysis

Client PO:

Client: LRL Associates Ltd.

Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017

**Project Description: 170132** 

### **Analysis Summary Table**

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Ammonia, as N	EPA 351.2 - Auto Colour	12-May-17	12-May-17
Anions	EPA 300.1 - IC	12-May-17	12-May-17
Total Kjeldahl Nitrogen	EPA 351.2 - Auto Colour, digestion	12-May-17	15-May-17



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017

**Project Description: 170132** 

Certificate of Analysis Client: LRL Associates Ltd. Client PO:

	Client ID: Sample Date: Sample ID:	TP1 08-May-17 1719377-01	TP3 08-May-17 1719377-02	TP7 08-May-17 1719377-03	- - -
	MDL/Units	Water	Water	Water	-
General Inorganics					-
Ammonia as N	0.01 mg/L	0.28	0.39	1.66	-
Total Kjeldahl Nitrogen	0.1 mg/L	78.1	65.3	131	-
Anions					
Nitrate as N	0.1 mg/L	<0.1	0.5	<0.1	-
Nitrite as N	0.05 mg/L	<0.05	<0.05	<0.05	-



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017 **Project Description: 170132** 

Certificate of Analysis Client: LRL Associates Ltd. Client PO:

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Nitrate as N Nitrite as N	ND ND	0.1 0.05	mg/L mg/L						
General Inorganics			_						
Ammonia as N Total Kjeldahl Nitrogen	ND ND	0.01 0.1	mg/L mg/L						



Certificate of Analysis

Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017 **Project Description: 170132** 

Client: LRL Associates Ltd.
Client PO:

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions Nitrate as N	ND	0.1	mg/L	ND				20	
Nitrite as N	ND	0.05	mg/L	ND				20	
General Inorganics									
Ammonia as N Total Kjeldahl Nitrogen	0.021 1.50	0.01 0.1	mg/L mg/L	0.022 1.52			2.4 1.8	8 10	



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017 **Project Description: 170132** 

Certificate of Analysis Client: LRL Associates Ltd. Client PO:

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Nitrate as N	1.01	0.1	mg/L	ND	101	81-112			
Nitrite as N	1.02	0.05	mg/L	ND	102	76-117			
General Inorganics									
Ammonia as N	0.280	0.01	mg/L	0.022	103	81-124			
Total Kjeldahl Nitrogen	1.91	0.1	mg/L		95.7	81-126			



Order #: 1719377

Report Date: 15-May-2017 Certificate of Analysis Order Date: 11-May-2017 Client: LRL Associates Ltd. Client PO: **Project Description: 170132** 

#### **Qualifier Notes:**

**Login Qualifiers:** 

Samples received submerged in water, possibly melted ice. This condition can compromise sample integrity. Applies to samples: TP1, TP3, TP7

#### **Sample Data Revisions**

None

#### **Work Order Revisions / Comments:**

None

#### **Other Report Notes:**

n/a: not applicable ND: Not Detected

MDL: Method Detection Limit

Source Result: Data used as source for matrix and duplicate samples

%REC: Percent recovery.

RPD: Relative percent difference.

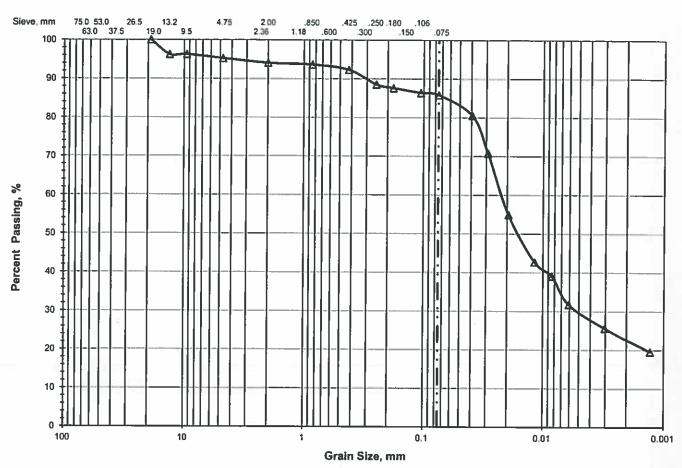
# APPENDIX C Sieve & Hydrometer Analysis





#### **PARTICLE SIZE ANALYSIS**

Client:Lloyd Phillips & Associates Ltd.File No.:170132Project:Hydrogeological Assessment & Terrain AnalysisReport No.:1Location:4835 Bank Street., Ottawa, ON.Date:May 8, 2017



Unified Soil Classification System

	> 75 mm	% GR	AVEL		% SAND	_	% FINES		
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
Δ	0.0	0.0	4.8	1.2	1.8	6.5	63.8	22.0	

	Location	Sample	Depth, m	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	Cu
Δ	TP-1	3	1.80 - 2.00	0.0226	0.0164	0.0052				





#### **PARTICLE SIZE ANALYSIS**

ASTM D 422 / LS-702

Client: Project:

Location:

Lloyd Phillips & Associates Ltd.

Hydrogeological Assessment & Terrain Analysis

4835 Bank Street., Ottawa, ON.

File No.:

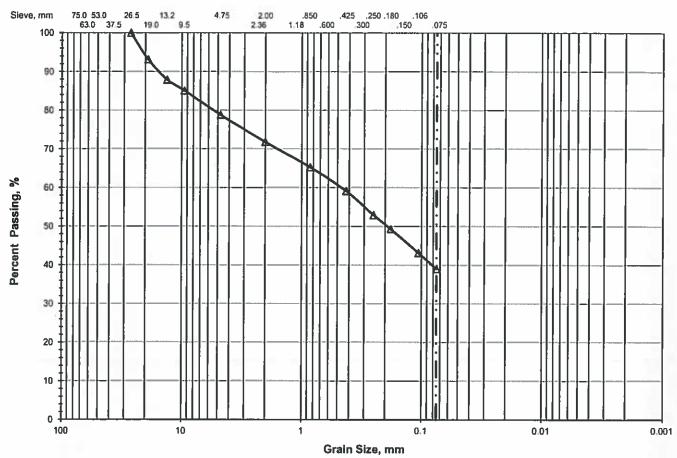
Date:

170132

2

Report No.:

May 8, 2017



Unified Soil Classification System

	> 75 mm	% GR	AVEL		% SANI	D <sub>.</sub>	% FINES		
		Coarse	Fine	Coarse	Medium	Fine	Silt & Clay		
Δ	0.0	6.0	15.3	7.0	12.7	20.1	39.0		
					10.				

	Location	Sample	Depth, m	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	Cc	Cu
Δ	TP3	6	1.4 - 1.6	0.4855	0.1932					
								"-		
ĺ										



## APPENDIX D Ontario Well Record Printouts



No

Elev. 4 R 0131310

The Water-well Drillers Act, 1954 Department of Mines

GROUND WATER BRANCH MAY 20 1957 ONTARIO WATER

in Village, Town or City).....

Static level .....

Depth(s)

at which

water(s)

found

60

Address 40 Farrance It Mana

**Pumping Test** 

Water Record

No. of feet

water rises

Basin |2|5| |2|1 Water-Well Record COMMISSION Carleton Township, Village, Town or City Slove County or Territorial District. (month) (day) (year) Pipe and Casing Record Casing diameter(s) Length(s) Type of screen ..... Length of screen ..... Well Log From Overburden and Bedrock Record ft. Sand 20 60 For what purpose(s) is the water to be used? touse Is water clear or cloudy? Is well on upland, in valley, or on hillside?.... Kipland Drilling firm F. R. Const. Address 2 Bareline RO City diese Name of Driller Z. R. f. J. H. Address .... Licence Number 3 7 3

> I certify that the foregoing statements of fact are true.

Date 24 any 14/50 St R Constit

Pumping rate 800 J. P. f/ Duration of test 2 hr

Kind of water

(fresh, salty, or sulphur)

Jonest

Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.

A Month Tohnston Cors CSC.ST

Form 5

UTM 18   4   5   3   8   6   0   E		GRO	OUND WATER	15 Nº	21
SR SIOI 713 310 N The Ontario Water Resou	ırces	Commission A	NOV 14 1		/ \
Elev. 92 R TO131215 WATER WEL			STRES COA	MISSION	•
Basin 215   Chrleton T	ownsh	ip, Village, To	wn or CityG	loucester 10	1961
Con 4 R F Lot P. T. 22	ate co	( )	iaj	month	year)
	lress	28 Cla	rence St	• Ottawa	2, Ont.
Casing and Screen Record			Pumping	Test	
Inside diameter of casing 6 3/16	Stat	ic level			~ . <b>H</b>
Total length of casing 211	Tes	t-pumping rat	e <b>5</b> 4/		G.P. <b>A</b> .
Type of screen	Pur	nping level	//	1 hn	
Length of screen	Dui	ration of test p	umping	cles	
Depth to top of screen	Wa	ter clear or clo	udy at end of to	80	H G.P. <b>M</b> .
Diameter of finished hole 6"	Re	commended pi	imping rate 80		w ground surface
	wit	h pump setting	g of		r Record
Well Log				Depth(s) at	Kind of water
Overburden and Bedrock Record		From ft.	To ft.	which water(s) found	(fresh, salty, sulphur)
Till and Milder rest. Grey hard lime a	ton	9 9	16-	85	fresh
and sand stone	PERSON SALARIAN	25	89		
SAN A TO THE STATE OF THE STATE	THE PERSON NAMED IN				
BOULDER TILL		0	16		
HARD GREY LIMESTONE		16	25		
5ANDS Tone		25	89	85	FBESH.
Jij Aug Esta					
For what purpose(s) is the water to be used?  Co-operative		In diagrar	<b>Location</b> n below show	distances of we	ell from
Is well on upland, in valley, or on hillside?  Valley  In the same Co. Little.		road and	lot line Ind	icate north by	arrow.
Is well on upland, in valley, or on hillsider.	l		100 11110. 1110	<i>'</i>	
T R INTRESTE CO. Liu.			Not mine: Ind.	,	
Drilling or Boring Firm J. B. Bullesiic 55. House	2		150'	,	
Drilling or Boring Firm J. B. Dullesiic Oc. Louis	Programme of the second	,	$\mathcal{N}$	in in	
Drilling or Boring Firm J. B. Dulleshe Go. Box.  Address Ottawa, Ontario.			$\mathcal{N}$	Ну зі	
Drilling or Boring Firm J. B. Dullesing Os. Box.  Address Ottawa, Ontario.	. orange	,	$\mathcal{N}$	Hy 31	<u>agangangan an</u> 1 - mag <b>g</b> 1
Drilling or Boring Firm J. B. Dullesite Oct.  Address Ottawa, Ontario.  Licence Number 194	Control of Management (Control of Control of	,	$\mathcal{N}$	Hy 31	g*
Address Ottawa, Ontario.  Licence Number 194  Name of Driller or Borer W. Roy	Control of Management (Control of Control of		150'	Hy 31	
Drilling or Boring Firm J. B. Bullesho So. B	Control of Management (Control of Control of		150'	Hy 31	,
Drilling or Boring Firm J. B. Bullesho So. B	Control of Management (Control of Control of	A Jahrana	150'	Hy 31	
Drilling or Boring Firm J. B. Dullesho S. B. Dulles	Control of Management (Control of Control of		150'	Hy 31	

316/52 GROUND WATER BRANC Ontario Water Resources Commission Act ONTARIO WATER RESOURCES COMMISSION TER WELL MLE Tony Township, Village, Town or City G-Love Es TER Date completed 29 June 6/ ddress BILLINGS BRIDGE **Pumping Test** Casing and Screen Record Inside diameter of casing Test-pumping rate G.P.M. Total length of casing // / Pumping level Type of screen Duration of test pumping / HR Length of screen Water clear or cloudy at end of test ZCEAR Depth to top of screen Recommended pumping rate 4 G.P.M. Diameter of finished hole with pump setting of \_\_\_\_\_\_ feet below ground surface **Water Record** Well Loa Kind of water Depth(s) at  $\mathbf{From}$ (fresh, salty, sulphur) which water(s) Overburden and Bedrock Record found LOAM 0 FAEY Lomest ME 55 Location of Well For what purpose(s) is the water to be used? In diagram below show distances of well from road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? Drilling or Boring Firm IN MEAGHER OTTAND Licence Number 245 Name of Driller or Borer 5 Am &

OWRC COPY

Form 7 15M Sets 60-5930

C50.03

GROUND WATER BRANCH UTM 118 2 41513181010 E 510117151310 N Ontario Water Resources Commission Act ONTARIO WATER ER WELL RECORDINGES COMMISSION .....Township, Village, Town or City.... Date completed 26 BILLINGS BRIDGE **Pumping Test** Casing and Screen Record Static level Inside diameter of casing Test-pumping rate Total length of casing Pumping level Type of screen Duration of test pumping Length of screen Water clear or cloudy at end of test Depth to top of screen Recommended pumping rate Diameter of finished hole with pump setting of. feet below ground surface Water Record Well Log Kind of water Depth(s) at From To ft. which water(s) (fresh, salty, Overburden and Bedrock Record found sulphur) CLAY LIMESTON 46 Location of Well For what purpose(s) is the water to be used? In diagram below show distances of well from road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? otrana Address Licence Number Name of Driller or Borer Address Form 7 10M-62-1152 CSS.58 OWRC COPY



The Ontario Water Resources Commission Act
WATER WELL RECORD

316/50 ·

	Water management in C	Ontario 1. PRINT ONLY IN S 2. CHECK 🔀 CORRE	PACES PROVIDED CT BOX WHERE APPLICABLE TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	ie į	151226		CON.  ACT, SURVEY, ETC.	F	22 23 24 LOT 25-27
	Carl	eton	Gloucester			5 /	F DATE COI	S He	1£022 8-53
					EVATION	RC. BASIN CODE	DAY_ 2	<u>4 мо. Жеч</u>	<u>VR. 72</u>
Į				25 2	0336	30 31			47
•	GENERAL COLOUR	MOST	G OF OVERBURDEN AND BED  OTHER MATERIALS	ROCK		(SEE INSTRUCTION OF THE CONTROL OF T		DEPTH	- FEET
		COMMON MATERIAL						FROM	то
	Brown	Clay	Sand & Stones		1	Clay & Si ray limes		3	<u>48</u>
		***							
							··· · · · ·		
							0.00		
			- 14-14-7						
	31 ) 00030	6 ast 28/12 lao4	a.2/15t			 	+	<u> </u>	<u> </u>
٦	32	4 15 21	32	43		54	65		75 80
	WATER JOUND	R RECORD	51 CASING & OPEN HO	LE RE		·	31-33 DIAMI	ETER 34-38 L	ENGTH 39-40 FEET
	10:13	RESH 3 SULPHUR	INCHES HICKNESS INCHES 10-11   STEEL 12	FROM	TO 13-16	MATERIAL AND TY	PE	DEPTH TO TOP OF SCREEN	41-44 80
4	15-18 1 FF	RESH 3 SULPHUR 19	06 2	ን . ሩ የት	0012		NG & SEA	LING RE	CORD
*.	20-23 1 FF	RESH 3 SULPHUR	17-18 1 STEEL 19 2 GALVANIZED	J+0	20-23	DEPTH SET AT - FEE	MATERIAL AND		MENT GROUT, PACKER, ETC.)
	25-28 1		3 ☐ CONCRETE 4 ☐ OPEN HOLE 24-25 1 ☐ STEEL 26		0048		-25		
~	30-33 1 F	RESH 3 SULPHUR 34 80	2 GALVANIZED 3 CONCRETE			26-29 30	-33 80		
1	MIMPING TEST METHO		4 OPEN HOLE	7 [		LOCATIO	ON OF WE		
V	STATIC	BAILER  WATER LEVEL 25  WATER	1 PUMPING	S.	LOT LINE		STANCES OF WELL FR		
	LEVEL 19-21	22-24 15 MINUTES	2 RECOVERY	·   <del> </del>	20 LOI LINE				
	Z IF FLOWING,	1.007	FEET FEET FEET		21 7		/~		
	RECOMMENDED PUMP	GPM. TYPE RECOMMENDED	8 FEET CLEAR 2 CLOUDY 43-45 RECOMMENDED 46-4		-515		/		
	SHALLOW 50-53	DEEP SETTING C	30 FEET PUMPING OOO8 GPN		i June	11.04	102		
[	FINAL 54	WATER SUPPLY	5 🗆 ABANDONED, INSUFFICIENT SUPPLY	<u> </u>		Hwy-	x	/	
	STATUS OF WELL	OBSERVATION WELL  TEST HOLE  CHAPTER WELL	6 ABANDONED, POOR QUALITY 7 UNFINISHED	1		3/	1/20	4'	
Ì	55-56		5 COMMERCIAL	1			_   _	LOT LI	NE_
	WATER USE O	3   IRRIGATION 4   INDUSTRIAL	7 DPUBLIC SUPPLY 8 COOLING OR AIR CONDITIONING				- Erozan	_ Eu	-
	57	1 CABLE TOOL	9  NOT USED 6  BORING	-	:				
	METHOD OF	2 ROTARY (CONVENTI 3 ROTARY (REVERSE)	ONAL) 7 🗌 DIAMOND 8 🗋 JETTING		ļ				
إ	DRILLING	ROTARY (AIR)	9 DRIVING	DRILL	LERS REMARKS:				
.1	NAME OF WELL CON	tractor <b>hnston Dril</b>	ling Co. 3002	NE S	DATA SOURCE  DATE OF INSPECTION	58 CONTRACTOR 3002	59-62 DATE RECEIVE	50173	63-68 80
	ADDRESS PA		HE" Ottawa, Ont		ATE OF INSPECTION	INSI	PECTOR (		
	E OF DRILLER C	OR BORER	LICENCE NUMBER	1 1	REMARKS:	1	3	Р	K
			SURMISSION DATE  DAY MO YR.	OFFICE			For the second	W	' I
	AC C	OPY	1	للا					Δ

Well ID Number: 1512375 Well Audit Number: Well Tag Number:

This table contains information from the original well record and any subsequent updates.

#### **Well Location**

Address of Well Location	
Township	GLOUCESTER TOWNSHIP
Lot	022
Concession	RF 04
County/District/Municipality	OTTAWA-CARLETON
City/Town/Village	
Province	ON
Postal Code	n/a
UTM Coordinates	NAD83 — Zone 18 Easting: 454020.70 Northing: 5017262.00
Municipal Plan and Sublot Number	_
Other	

#### Overburden and Bedrock Materials Interval

General Colour	Most Common Material	Other Materials	General Description	Depth From	Depth To
BRWN	OBDN	SAND		0 ft	9 ft
WHIT	SNDS			9 ft	74 ft

### **Annular Space/Abandonment Sealing Record**

Depth	Depth	Type of Sealant Used	Volume
From	To	(Material and Type)	Placed

#### **Method of Construction & Well Use**

<b>Method of Construction</b>	Well Use
Diamond	Domestic

#### Status of Well

Water Supply

### **Construction Record - Casing**

Inside Diameter	Open Hole or material	Depth From	Depth To
2 inch	GALVANIZED		20 ft
	OPEN HOLE		74 ft

#### **Construction Record - Screen**

Outside Diameter Material Pepth Depth From To

#### Well Contractor and Well Technician Information

Well Contractor's Licence Number: 1703

## **Results of Well Yield Testing**

After test of well yield, water was	CLEAR
If pumping discontinued, give reason	
Pump intake set at	
Pumping Rate	8 GPM
<b>Duration of Pumping</b>	2 h:0 m
Final water level	12 ft
If flowing give rate	_
Recommended pump depth	35 ft
Recommended pump rate	8 GPM
Well Production	PUMP
Disinfected?	_

#### **Draw Down & Recovery**

Draw Down Time(min)	Draw Down Water level	Recovery Time(min)	Recovery Water level
SWL	6 ft		
1		1	
2		2	
3		3	
4		4	
5		5	
10		10	
15	12 ft	15	
20		20	
25		25	
30	12 ft	30	
40		40	
45	12 ft	45	
50		50	
60	12 ft	60	

#### **Water Details**

Water Found at Depth	Kind
74 ft	Fresh

#### **Hole Diameter**

Depth		Diameter
From	To	

**Audit Number:** 

Date Well Completed: November 27, 1972

**Date Well Record Received by MOE:** March 07, 1973

Updated: March 20, 2017

Rate Rate

Share <u>facebook twitter Print</u>

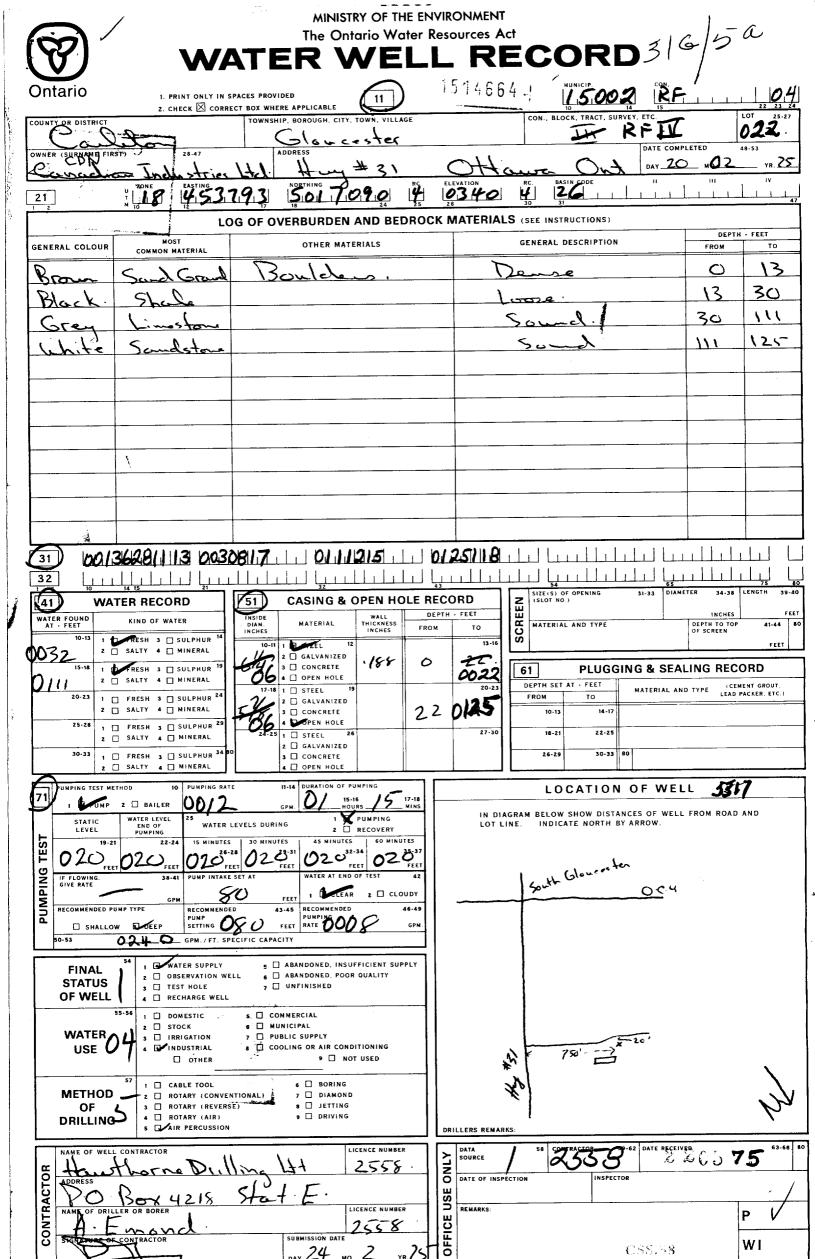
Tags

• Environment and energy,

#### MINISTRY OF THE ENVIRONMENT The Ontario Water Resources Act TER WELL RECORD 1513436 - Culita LETRIM Ottan GLOUCESTER ONT UNITED CO - OF OF CATARIO R. R. #6 OFFAWA. ONTARIO. BASIN CODE 4.53.850 " [ ZONE LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS) GENERAL DESCRIPTION OTHER MATERIALS GENERAL COLOUR FROM то Soft 0 4 Top Seil Brown 4 12 Brown Soil Boulder Hard 12 Soft Porous 16 Grey Limestone Clay 50 Medium Hard 16 White Limestone Grey Limestone 1000HG92 11 DOV CHG122/31 1 DOV 6/21/5/05 1 Dasid/V5 11 11 11 11 11 11 11 11 11 31 SIZE(S) OF OPENING CASING & OPEN HOLE RECORD WATER RECORD [51] (41) WATER FOUND KIND OF WATER WALL THICKNESS MATERIAL FROM 1 T FRESH 3 SULPHUR 2 SALTY 4 MINERAL 00 48 22 13-16 STEEL 2 GALVANIZED -188 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL 3 CONCRETE 61 **PLUGGING & SEALING RECORD** 0022 4 OPEN HOLE DEPTH SET AT - FEET 1 | STEEL 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL 2 GALVANIZED 3 T CONCRETE 4 OPEN HOLE 1 🗆 FRESH 3 🗆 SULPHUR 4 MINERAL 1 🗆 STEEL Z SALTY 2 GALVANIZED 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL 3 CONCRETE LOCATION OF WELL 1 | PUMP IN DIAGRAM BELOW SHOW LISTANCES OF WELL FROM BOAD AND LOT LINE. INDICATE NORTH BY ARROW. PUMPING 2 | RECOVER WATER LEVEL END OF PUMPING WATER LEVELS DURING 15 MINUTES 30 MINUTES 29-22-24 MINUTES (30 FEET $030_{\frac{\text{feet}}{}}$ **30** FEET 014 0.35 m 2 CLOUDY RECOMMENDED PUMP SETTING 30 RECOMMENDED PUMP TYPE FEET RATE UOS PARKING 5 ABANDONED, INSUFFICIENT SUPPLY WATER SUPPLY FINAL OBSERVATION WELL 6 ABANDONED, POOR QUALITY LOT. **STATUS** 3 TEST HOLE 7 UNFINISHED OF WELL 1 DOMESTIC 2 STOCK 6 MUNICIPAL WATER IRRIGATION PUBLIC SUPPLY USE ().\ 8 COOLING OR AIR CONDITIONING 4 | INDUSTRIAL ☐ OTHER 9 🗌 NOT USED CABLE TOOL 6 T BORING **METHOD** ROTARY (CONVENTIONAL) 7 DIAMOND 2 🔲 OF 3 🗍 ROTARY (REVERSE) 8 | JETTING **DRILLING** 5 AIR PERCUSSION DATA SOURCE ONLY 2557 28 HAWTHORNE DRILLING LIMITED ... DATE OF INSPECTIO OFFICE USE Box 4218 STATION FIET OTTAWA ONTARIO REMARKS (188.138

MINISTRY OF THE ENVIRONMENT COPY

07-091





# The Ontario Water Resources Act WATER WELL RECORD

Ontario	1. PRINT ONLY IN S 2. CHECK 🗵 CORRI	SPACES PROVIDED 11 11 ECT BOX WHERE APPLICABLE	I	51461	64   MUNICIP		1	1 1 37 21
OWNER (SURNAME FIL	RST) 28 47	TOWNSHIP, BOROUGH, CITY, TOWN. VILLA	AGE		CON., BLOCK, TRACT, SUR	VEY, ETC.	LETED	22 ·
Canadi	an Industries	Hel. Huy # 31	. (	HC	aura Ont	DAY_2C		vn.25
21	ZONE EASTING	NORTHING 0	RC EL	EVATION	RC BASIN CODE	<u> </u>	. 1	.   .   .   .
	LC	OG OF OVERBURDEN AND BE	DROCK M	MATERIA	LS (SEE INSTRUCTIONS)			
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS			GENERAL DESCRIPTION		FROM	TO TO
Brown	Sand Grand	Boulders.		7	Dense		<u> </u>	13
Black.	Shala			<u> </u>	Loose.		_13_	30
Grey	Limostone				Sound		<u>30</u>	111
white	Sandstone				کی سیک		_///	125
	1		-					
31							1 ] ] i ]	
32	14 15 21		سبها لسا		54   SIZE(S) OF OPENING	55 31-33 DIAME	ER 34-38	LENGTH 39
WATER FOUND	TER RECORD	51 CASING & OPEN HO	DEPTH		Z (SLOT NO )		INCHES	F
10-13	FRESH 3 SULPHUR 14	INCHES INCHES	FROM	TO 13 -16	MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN	41-44 FEET
15-18 1 (	TRESH 3 SULPHUR 19	64   GALVANIZED 188	0	22.	61 PLUGGI	NG & SEAL	ING REC	ORD
20-23 1	SALTY 4 MINERAL  FRESH 3 SULPHUR 24	17-18 1		20-23	DEPTH SET AT - FEET FROM TO	MATERIAL AND		PACKER, ETC.)
l	SALTY 4 MINERAL  FRESH 3 SULPHUR 29	5/8 3 CONCRETE	22	125	10:13 14:17			
<del></del>	☐ SALTY 4 ☐ MINERAL ☐ FRESH 3 ☐ SULPHUR 34 11	24-25 I  STEEL 26 2  GALVANIZED 3  CONCRETE		27.30	18-21 22-25 26-29 30-33 8	10	· · · · · · · · · · · · · · · · · · ·	
	SALTY 4 MINERAL  ETHOD 10 PUMPING RAT	4 G OPEN HOLE						
71 PUMPING TEST MI	2 D BAILER 12		17-18 MINS		LOCATION			
STATIC LEVEL	PUMPING	LEVELS DURING  1  PUMPING 2  RECOVERY  1 30 MINUTES   45 MINUTES   60 MINU			AGRAM BELOW SHOW DISTAN LINE. INDICATE NORTH BY		FHOM ROAD	ANU
20,	20 10	$\frac{28}{2}$ $\frac{20^{31}}{2}$ $\frac{20^{32-34}}{2}$	Ö37		۱.	_		
IF FLOWING. GIVE RATE  RECOMMENDED P	38-41 PUMP INTAKE	SET AT WATER AT END OF TEST	42		South Gloureste	004		
RECOMMENDED P	UMP TYPE RECOMMENDE		46-49					
SO-53		ECIFIC CAPACITY	GPM.					
FINAL STATUS OF WELL	1 WATER SUPPLY 2 OBSERVATION WE 3 TEST HOLE	5 ABANDONED. INSUFFICIENT SUP 6 ABANDONED. POOR QUALITY 7 UNFINISHED	PLY					
	55-56   DOMESTIC	S COMMERCIAL 6 MUNICIPAL						
WATER USE	3 ☐ IRRIGATION 4	7 DUBLIC SUPPLY B COOLING OR AIR CONDITIONING 9 NOT USED			AN 180' -	-20'		`
METHOD OF DRILLING	3   ROTARY (REVERS		DR	ILLERS REMA	Ap)			R
1 1 1	L CONTRACTOR	LICENCE NUMBER		DATA SOURCE	<del></del>	DATE RECEIVE کے رک		61-6
ADDRESS	Thorne Dil	1mg 4 2558	-   ē	DATE OF INS				
NAME OF DRIE	SOX 4218	Statiti		REMARKS:		1		P
DDRESS DAME OF DRILL	CONTRACTOR .	SUBMISSION DATE	25					
	<del></del>	DAY 24 MO. 2 Y	<u>، 2</u> 5 ا		- 5	11:30 . HŠ		WI
MINISTR	RY OF THE ENVI	RONMENT COPY					FOR	M 7 MOE C

# APPENDIX E Moisture Surplus Printouts

Ottawa\_50mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY... 50 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . 1.075 LOWER ZONE..... 30 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 -1 30- 6 -19 18.3 31- 7 20.8 -41 31-8 19.5 -34 30- 9 14.6 -9 31-10 8. 1 -1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -105 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN PΕ DATE RAIN MELT ΑE **DEF** SURP SNOW SOI L ACC P 31- 1 3.0 2. 6 2. 3 28- 2 31- 3 30- 4 1.7 31- 5 1.9 30-6 1.2 1.2 31- 7 31- 8 1.3 7 30- 9 1.5 31-10 1.4 30-11 1.7 31-12 3.0 

Ottawa\_75mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY... 75 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . 1.075 LOWER ZONE..... 45 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 -10 31- 7 20.8 -32 31-8 19.5 -32 30- 9 14.6 -9 31-10 8. 1 -1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -84 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOI L ACC P 31- 1 3.0 2. 6 2. 3 28- 2 31- 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 30- 9 1.5 31-10 1.4 30-11 1.7 31-12 3.0 

Ottawa Airport, C	N			100mm_\ GET MEA		D. txt R THE P	ERI OD	1950-2	010	DC20492
LAT 45.32 LONG 75.67				CAPACI		100 MM 60 MM		AT IND		36. 41 1. 075
DATE TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOI L	ACC P
04 4 40 /		4.0	4-	_	_	_	~=			

DATE	TEMP (C)	PCPN	RAIN	MELT	PE	ΑE	DEF	SURP	SNOW	SOI L	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-10.6 -8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1 1.3 -7.0 5.9 TTL	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26	15 18 80 69 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1	0 1 5 32 80 112 115 88 66 35 10 1 545	0 0 0 0 0 -4 -21 -29 -8 -1 0 0 -63	25 28 106 104 13 4 2 1 3 10 34 33 363	83 110 64 0 0 0 0 0 0 0 0 9 47	99 99 100 100 81 47 19 18 32 63 91 97	299 356 422 494 568 651 740 827 912 77 157 236

Ottawa	Airport, 0	N	STAN	DARD [	DEVI ATI	ONS FO	OR THE	PERI OD	1950-	2010	DC20492	
DATE	TEMP (C)	PCPN	RAI N	MELT	PE	ΑE	DEF	SURP	SNOW	SOI L	ACC P	
31- 1 28- 2	3. 0 2. 6	26 29	16 15	18 27	1 1	1 1	0 0	30 37	43 59	5 3	55 59	
31- 3 30- 4	2. 3 1. 7	28 31	22 31	47 84	4 8	4 8	0 0	53 84	83 0	0 2	65 74	
31- 5 30- 6	1. 9 1. 2	32 38	32 38	0	12 9	12 12	0 11	21 17	0	22 34	85 93	
31- 7 31- 8	1. 2 1. 3	42 39	42 39	0	8 8	25 29	26 30	11 5	0	30 30	93 107	
30- 9 31-10	1. 5 1. 4	38 37	38 37	0 2	8 7	14 6	13 2	15 21	0 0	35 36	110 37	
30-11 31-12	1. / 3. 0	27 30	28 22	9 14	4 1	4 1	0	34 30	13 34	19 8	45 56	

Ottawa_125mm_WBNRMSD.txt									
WATER BUDGET MEANS FOR THE PERIOD 1950-2010	DC20492								
ATED HOLDING CADACITY 125 MM HEAT INDEY	26 /11								

Uttawa	Airport, o	'IN	WAIL	טעטט א.		ANS FUR	I III E	EKIOD	1930-2	UIU	DC20492
	45. 32 75. 67					TY 1			AT IND		36. 41 1. 075
DATE	TEMP (C)	PCPN	RAI N	MELT	PE	AE	DEF	SURP	SNOW	SOI L	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-10. 6 -8. 8 -2. 7 5. 9 13. 0 18. 3 20. 8 19. 5 14. 6 8. 1 1. 3 -7. 0 5. 9 TTL	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26	15 18 80 69 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 115 122 92 67 35 10 1 560	0 0 0 0 -1 -13 -25 -7 -1 0 0	24 28 105 104 13 4 2 1 3 9 27 29 349	83 110 64 0 0 0 0 0 0 0 0 9 47	122 123 125 125 106 69 33 28 41 74 108 119	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, O	N	STAN	DARD D	EVI ATI	ONS FO	R THE	PERI OD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAI N	MELT	PE	AE	DEF	SURP	SNOW	SOI L	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12	3. 0 2. 6 2. 3 1. 7 1. 9 1. 2 1. 3 1. 5 1. 4 1. 7 3. 0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9	1 1 4 8 12 9 8 8 8 7 4 1	1 4 8 12 9 21 26 13 6 4	0 0 0 0 4 23 28 11 2 0	31 37 54 84 21 17 11 5 14 20 32 30	43 59 83 0 0 0 0 0 0 13 34	10 8 0 2 22 39 37 38 42 42 25 14	55 59 65 74 85 93 107 110 37 45 56

Ottawa\_150mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...150 MM HEAT INDEX... 36.41 A..... 1.075 LOWER ZONE..... 90 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 31- 3 30- 4 -2.7 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -8 31-8 19.5 -19 30- 9 14.6 -6 31-10 8. 1 -1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -34 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 3.0 28- 2 31- 3 2. 6 2. 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 

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Ottawa\_200mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...200 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . 1.075 LOWER ZONE.....120 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -3 31-8 19.5 -11 7 30- 9 14.6 -4 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -18 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 3.0 2. 6 2. 3 28- 2 31- 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 30- 9 1.5 

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Ottawa\_225mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...225 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . 1.075 LOWER ZONE.....135 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -2 31-8 19.5 -8 30- 9 14.6 -4 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -14 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 3.0 28- 2 31- 3 2. 6 2. 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 

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Ottawa\_250mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...250 MM HEAT INDEX. . . 36.41 LOWER ZONE.....150 MM A. . . . . . . . . . . . 1.075 DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -1 31-8 19.5 -6 30- 9 14.6 -3 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -10 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOI L ACC P 31- 1 3.0 2. 6 2. 3 28- 2 31- 3 30- 4 1.7 31- 5 1.9 30-6 1.2 ģ 31- 7 1.2 31- 8 1.3 

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Ottawa\_265mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...265 MM HEAT INDEX... 36.41 A..... 1.075 DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -1 31-8 19.5 -5 30- 9 -3 14.6 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -9 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ **DEF** SURP SNOW SOIL ACC P ΑE 31- 1 3.0 28- 2 31- 3 2. 6 2. 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 

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Ottawa\_275mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...275 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . . . 1.075 DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -1 31-8 19.5 -4 30- 9 14.6 -2 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -7 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 3.0 2. 6 2. 3 28- 2 31- 3 30- 4 1.7 31- 5 1.9 30-6 1.2 

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Ottawa\_280mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...280 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . 1.075 LOWER ZONE.....168 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 -2.7 31- 3 30- 4 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 -1 31-8 19.5 -4 30- 9 14.6 -2 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -7 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 3.0 2. 6 2. 3 28- 2 31- 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 

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Ottawa_300mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC											DC20492
	45.32 G 75.67		TER HO					_	AT IND		36. 41 1. 075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	ΑE	DEF	SURP	SNOW	SOI L	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-10. 6 -8. 8 -2. 7 5. 9 13. 0 18. 3 20. 8 19. 5 14. 6 8. 1 1. 3 -7. 0 5. 9 TTL	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 116 135 114 73 36 10 1 603	0 0 0 0 0 0 0 -3 -2 0 0 0	19 23 95 101 13 4 2 1 3 7 18 20 306	83 110 64 0 0 0 0 0 0 0 0 47	279 285 297 300 281 243 194 167 176 209 252 272	299 356 422 494 568 651 740 827 912 77 157 236

Ottawa	STAN	DARD [	DEVI ATI	ONS FO	OR THE	PERI OD	1950-	2010	DC20492		
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	ΑE	DEF	SURP	SNOW	SOI L	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8	3. 0 2. 6 2. 3 1. 7 1. 9 1. 2 1. 2	26 29 28 31 32 38 42	16 15 22 31 32 38 42	18 27 47 84 0 0	1 1 4 8 12 9 8	1 1 4 8 12 9 8	0 0 0 0 0 0 2 8	29 36 57 81 21 17 11	43 59 83 0 0	37 33 13 2 22 41 52	55 59 65 74 85 93 93
30- 9 31-10	1. 3 1. 5 1. 4	39 38 37	39 38 37	0 2	8 7	10 9 7	5 1	14 19	0 0 0	65 71 65	110 37
30-11 31-12	1. 7 3. 0	27 30	28 22	9 14	4 1	4 1	0 0	29 28	13 34	52 41	45 56

Ottawa\_400mm\_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20492 LAT. . . . 45. 32 LONG. . . 75. 67 WATER HOLDING CAPACITY...400 MM HEAT INDEX. . . 36.41 A. . . . . . . . . . 1.075 LOWER ZONE.....240 MM DATE TEMP (C) **PCPN** RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 -10.6 -8.8 28- 2 31- 3 30- 4 -2.7 5. 9 31- 5 13.0 30- 6 18.3 31- 7 20.8 31-8 19.5 -1 7 30- 9 14.6 -1 31-10 8. 1 30-11 1. 3 31-12 -7.0 5.9 TTL AVE -2 Ottawa Airport, ON STANDARD DEVIATIONS FOR THE PERIOD 1950-2010 DC20492 TEMP (C) PCPN DATE RAIN MELT PΕ ΑE **DEF** SURP SNOW SOIL ACC P 31- 1 3.0 28- 2 31- 3 2. 6 2. 3 30- 4 1.7 31- 5 1.9 30-6 1.2 31- 7 1.2 31- 8 1.3 

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