

TECHNICAL MEMORANDUM

DATE September 11, 2018 **Project No.** 07-1121-0232-7100

TO Vincent Denomme

CC Rvan Magladry

Ryan Magladry IBI Group

Claridge Homes

FROM Sean Spanik, EIT EMAIL sspanik@golder.com

INFILTRATION RATE ASSESSMENT 380 ROLLING MEADOW CRESCENT, OTTAWA, ONTARIO

Golder Associates Ltd. (Golder), under contract with Claridge Homes (Claridge), undertook an assessment of the infiltration characteristics of three soil strata within the footprint of a proposed development. The tests were completed at 380 Rolling Meadow Crescent Ottawa, Ontario in the vicinity of BH 08-206. Figure 1, attached, shows the site and testing location.

Method

Measurement of the field-saturated hydraulic conductivity (K_{fs}) of near surface soils was carried out using a Guelph Permeameter apparatus (Model 2800K1) by Golder personnel between September 4 and September 6, 2018. The testing methodology was based on the stormwater infiltration best management practices described in the Stormwater Management Criteria: Appendix B: Water Balance and Recharge document prepared by the Credit Valley Conservation Authority (CVCA, 2012).

At each testing location, the Guelph Permeameter was installed in a 6 centimeter (cm) diameter hand-augered hole at a depth ranging from 0.57 to 2.05 metres below ground surface. All tests were completed in unsaturated soils (above the water table) near borehole 08-206. The soils encountered during the hand augering were documented. The Guelph Permeameter was operated according to the single head method. The outflow of water at the testing depth was monitored until it was determined that it had reached steady-state. The field-saturated hydraulic conductivity of the soil was estimated using the following equation (Elrick et al., 1989):

$$K_{fs} = \frac{C_1 Q_1}{2H_1^2 + \pi a^2 C_1 + 2\pi \frac{H_1}{a^*}}$$

Where: C_1 = shape factor

 $Q_1 = \text{flow rate (cm}^3/\text{s)}$

 H_1 = water column height (cm)

a = well radius (cm)

 a^* = alpha factor (0.12 cm⁻¹)

In accordance with CVCA, 2012, the percolation rate ("T-time" in min/cm) corresponding to each K_{fs} was estimated based on information presented in Tables 2 and 3 of the Supplementary Standard SB-6 Percolation Time and Soil Descriptions, of the Ontario Building Code (OMMAH, 2012) summarized below:

Field-Saturated Hydraulic Conductivity K _{fs} (cm/s)	Estimated Percolation Time, T (min/cm)	Infiltration Rate, 1/T (mm/hr)
10 ⁻¹	2	300
10-2	4	150
10 ⁻³	8	75
10 ⁻⁴	12	50
10 ⁻⁵	20	30
10 ⁻⁶	50	12

Design infiltration rates were then determined in accordance with Table B3 of the Stormwater Management Criteria: Appendix B: Water Balance and Recharge document prepared by the Credit Valley Conservation Authority (CVCA, 2012).

Results

The soils encountered during the hand augering for the Guelph Permeameter testing were as follows:

Depth (m)	Description
0 – 0.85	SILTY SAND; brown; non-cohesive; contains cobbles and gravel (Fill)
0.85 – 1.05	SAND; yellow-brown; non-cohesive; moist
1.05 – 1.80	SILTY SAND; grey-brown; non-cohesive; moist
1.80 – 2.05	SILTY CLAY; grey-brown; cohesive; moist (Weathered Crust)

The results of the infiltration tests are summarized in the attached Table 1. Based on the testing results, the field-saturated hydraulic conductivity of the native soils ranges from 2×10^{-3} cm/s (silty sand) to 4×10^{-5} cm/s (silty clay). One test was completed in the silty sand fill. Testing results indicate that the saturated hydraulic conductivity of the fill is 6×10^{-5} cm/s.

The percolation times (T-time) and the infiltration rates associated with each test result were determined, and are also summarized in Table 1. Percolation times range from 6 min/cm to 17 min/cm. Infiltration rates (the inverse of the T-times) were calculated to range from 36 mm/hr to 105 mm/hr.

Because the results of the Guelph Permeameter testing and the record of borehole 08-206 indicate that there are less permeable soil horizons (silty clay) below the more permeable sand and silty sand, the recommended "safety correction factor" to calculate the design infiltration rate is 3.5 (CVCA, 2012). Therefore, the design infiltration rates for the native soil were calculated to range from 12 mm/hr to 29 mm/hr.



Closure

We trust the information included meets your current needs. Should you require clarification, please do not hesitate to contact us.

Yours truly,

Golder Associates Ltd.

Sean Spanik, M.A.Sc., EIT

Environmental Consultant

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Attachments: Table 1 - Guelph Permeameter Infiltration Rate Testing Results September 2018

Figure 1 – Field Investigation Locations Attachment 1 - Borehole Record

References

Elrick, D.E., W.D. Reynolds, and K.A. Tan. 1989. Hydraulic conductivity measurements in the unsaturated zone using improved well analyses. Ground Water Monitoring Review. 9:184-193.

Credit Valley Conservation Authority (CVCA). 2012. Stormwater Management Criteria: Appendix B: Water Balance and Recharge document.

Ontario Ministry of Municipal Affairs and Housing (OMMAH). 2012. Supplementary Guidelines to the Ontario Building Code 2012. SG-6 Percolation Time and Soil Descriptions. Toronto, Ontario.



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Table 1: Guelph Permeameter Infiltration Rate Testing Results September 2018

Test ID	Test Depth (m)	Material	K _{sat} (cm/s)	T-Time (min/cm)	1/T (Infiltration Rate mm/hr)	Design Infiltration Rate (mm/hr)
GP-01	0.57	silty sand with gravel and cobbles (fill)	6E-05	13	46	13
GP-02	1.27	silty sand	2E-03	6	100	29
GP-03	2.05	silty clay (weathered crust)	4E-05	14	43	12

Notes: All auger holes were dry upon completion



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ATTACHMENT 1

Borehole Record



PROJECT: 07-1121-0232

1:50

RECORD OF BOREHOLE: 08-206

SHEET 1 OF 1

LOCATION: N 5032572.3 ;E 381786.6

BORING DATE: October 9, 2008

DATUM: Geodetic

CHECKED: S.A.T.

SAMPLER HAMMER, 64kg; DROP, 760mm PENETRATION TEST HAMMER, 64kg; DROP, 760mm DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m HYDRAULIC CONDUCTIVITY, k, cm/s SOIL PROFILE SAMPLES BORING METHOD ADDITIONAL LAB. TESTING DEPTH SCALE METRES PIEZOMETER STRATA PLOT NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - O WATER CONTENT PERCENT BLOWS/0. DESCRIPTION DEPTH -OW Wp H (m) GROUND SURFACE 78.91 TOPSOIL 78.61 Bentonite Seal Yellow brown SILTY SAND 0.30 78.36 Loose, brown SAND, trace silt 77.8<u>1</u> SS 12 Compact, brown SILTY SAND Very stiff to stiff, grey brown SILTY CLAY (WEATHERED CRUST) SS 2 Stiff to firm, grey SILTY CLAY, with occasional sand seams Native Backfill Φ \oplus Firm, grey SILTY CLAY Power Auger Ф Ф Benonite Seal æ Silica Sand Ф æ Standpipe SS wн Ф Cave Ф Ф End of Borehole 07-1121-0232.GPJ GAL-MIS.GDT 7/16/18 JM 9 10 MIS-BHS 001 GOLDER DEPTH SCALE LOGGED: D.J.S.