

NOVATECH

Engineers, Planners & Landscape Architects

Engineering

Land / Site
Development

Municipal
Infrastructure

Environmental /
Water Resources

Traffic /
Transportation

Structural

Recreational

Planning

Land / Site
Development

Planning Application
Management

Municipal Planning
Documents &
Studies

Expert Witness
(OMB)

Wireless Industry

Landscape Architecture

Urban Design &
Streetscapes

Open Space, Parks &
Recreation Planning

Community &
Residential
Developments

Commercial &
Institutional Sites

Environmental
Restoration

35
Years

Burnett Lands

3370 Greenbank Road, Ottawa

Site Serviceability and Stormwater Management Report

Engineering excellence. Planning precision. Inspired landscapes.

BURNETT LANDS
3370 GREENBANK ROAD
SITE SERVICEABILITY AND STORMWATER
MANAGEMENT REPORT

Prepared for:

Claridge Homes

Prepared By:

NOVATECH
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
K2M 1P6

December 9, 2016

Revised: January 26, 2018

Novatech File: 111117
Ref: R-2016-170

January 26, 2018

City of Ottawa
Planning, Infrastructure and Economic Development Department
Planning Services Branch
110 Laurier Ave. West, 4th Floor
Ottawa, Ontario
K1P 1J1

Attention: Mr. Don Herweyer, Manager of Development Review South

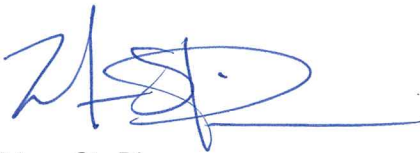
**Reference: Burnett Lands - 3370 Greenbank Road
Site Serviceability and Stormwater Management Report
Novatech File No.: 111117**

Enclosed herein are three (3) copies of the "Site Serviceability and Stormwater Management Report" for the proposed development of the Burnett Lands located at 3370 Greenbank Road, Ottawa. The report is submitted in support of applications for Official Plan Amendment, Zoning By-Law Amendment and Draft Plan of Subdivision. It will address how the subject development will be serviced with sanitary sewer, storm sewers, watermain and stormwater management.

Should you have any questions or comments, please do not hesitate to contact us.

Sincerely,

NOVATECH



Marc St. Pierre
Senior Project Manager

Encl.

TABLE OF CONTENTS

1.0	INTRODUCTION.....	1
1.1	Background	1
1.2	Additional Reports	2
2.0	EXISTING CONDITIONS	3
2.1	Topography & Drainage.....	3
2.2	Subsurface Conditions.....	3
3.0	SANITARY SERVICING.....	4
4.0	WATERMAIN	5
5.0	STORMWATER MANAGEMENT CRITERIA.....	7
6.0	BURNETT MUNICIPAL DRAIN	9
7.0	STORMWATER MANAGEMENT DESIGN	10
7.1	Storm Sewer Design (Minor System).....	10
7.2	Inlet Control Devices.....	10
7.3	Overland Flow Path (Major System)	10
7.4	Street 'B'/ Caivan Lands	10
7.5	Water Quantity & Quality Control.....	11
8.0	HYDROLOGIC & HYDRAULIC MODELING.....	12
8.1	Design Storms.....	12
8.2	Model Development.....	12
8.2.1	<i>Storm Drainage Areas</i>	<i>12</i>
8.2.2	<i>Subcatchment Model Parameters.....</i>	<i>12</i>
8.2.3	<i>Minor System.....</i>	<i>15</i>
8.2.4	<i>Major System.....</i>	<i>16</i>
8.2.5	<i>Modeling Files / Schematic.....</i>	<i>17</i>
8.3	PCSWMM Model Results	17
8.3.1	<i>Minor System.....</i>	<i>17</i>
8.3.2	<i>Major System.....</i>	<i>17</i>
8.3.3	<i>Hydraulic Grade Line</i>	<i>18</i>
8.3.4	<i>On-Site Storage (Future Development Blocks)</i>	<i>20</i>
9.0	UTILITIES	20
10.0	EROSION AND SEDIMENT CONTROL	21
11.0	CONCLUSIONS AND RECOMMENDATIONS	22

LIST OF TABLES

Table 4.1: Water Demand Summary (Pre Watermain Reconfiguration)

Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)

Table 7.1: Storm Sewer Design Parameters

Table 8.1: Hydrologic Modeling Parameters

Table 8.2: ICD Parameters

Table 8.3: Minor System Peak Flows at Outlets

Table 8.4: Ponding Depths at Catchbasins (100yr Event)

Table 8.5: Storm Sewer Hydraulic Grade Line: 5yr-4hr Chicago Distribution, 100-yr WL in Jock River

List of Figures

- Figure 1 Site Location Plan
- Figure 2 Concept Plan
- Figure 3 Realigned Greenbank Road Watermain
- Figure 4 Watermain Layout
- Figure 5 Burnett Municipal Drain

Appendices

- Appendix A Design Sheets
- Appendix B Sanitary Report Excerpts
- Appendix C Watermain Boundary Conditions, FUS Calculations, and Modelling Results
- Appendix D SWM Calculations & PCSWMM Model
- Appendix E Burnett Municipal Drain Analysis
- Appendix F Correspondence
- Appendix G Engineering Drawings

Drawings

- 111117 – GP General Plan of Services (revision 2)
- 111117 – GR Grading Plan (revision 2)
- 111117 – SAN Sanitary Drainage Area Plan (revision 2)
- 111117 – STM Storm Drainage Area Plan (revision 2)

Enclosed CD

- PCSWMM Model Files
- PCSWMM Model Output

1.0 INTRODUCTION

Novatech has been retained by Claridge Homes to prepare a Site Serviceability & Stormwater Management Report for the lands located at 3370 Greenbank Road (herein referred to as the “Burnett Lands”). This report has been prepared in support of the application for Official Plan Amendment, Zoning By-Law Amendment, and Draft Plan of Subdivision, and outlines the servicing and proposed storm drainage and stormwater management strategy for the site.

1.1 Background

The subject site is approximately 15.5 hectares in area and is located immediately north of the Jock River, south of Strandherd Drive and between the Kennedy Burnett Stormwater Management Facility and the existing Greenbank Road as shown on **Figure 1**. The Burnett Municipal Drain is tributary to the Jock River and travels through the subject site. The property currently has farm and accessory structures located near its southern boundary with an existing gravel access on to Greenbank Road. The remainder of the site is currently used for passive agriculture activities. The topography is generally flat with a gentle slope from the northeastern corner to the southwestern corner.



Figure 1: Site Location (Base Map Source: GeoOttawa)

The following describes the existing and planned land uses adjacent to the subject site:

North: Lands to the north, owned by Caivan Communities, are currently under the development approval process and have recently obtained OPA and ZBLA approval (Amendment #144) from the City to permit High Rise and Mid Rise Mixed-Use Residential developments, Mid Rise Residential Dwellings, and a Neighbourhood Park as per *Schedule 1- Land Use Plan, South Nepean Secondary Plan (Area 7)*. Further north of the Caivan Communities' development is the planned Barrhaven Town Centre which will include a variety of retail uses to service the surrounding existing and planned residential developments.

East: Lands east of the subject site contain a mixture of low density residential dwellings (single detached houses), a secondary school (St. Joseph Catholic High School), and an existing vegetated area. Greenbank Road currently forms the eastern boundary of the site. The realigned Greenbank Road will bisect the site as per the design by the City.

South: The Jock River flows west – east along the majority of the southern boundary of the property until it turns south near the southeastern corner of the site. The lands south of Jock River are within the Barrhaven *South Community Design Plan* and are intended for a future district park and residential uses as shown on *Figure 17* of the *Barrhaven South Community Design Plan*.

West: The Kennedy Burnett stormwater management facility is located north west of the subject site and drains into the Jock River. Lands immediately west are currently vacant and intended for mostly conservation and residential uses as identified in Schedule B of the Official Plan.

Plans are to develop a residential subdivision, as shown on **Figure 2 – Concept Plan**, which will consist of 247 townhome units and 420 condo units for a total of 667 units.

1.2 Additional Reports

This report provides information on the considerations and approach by which Novatech has designed and evaluated the proposed servicing for the Burnett Lands. This report should be read in conjunction with the following:

- *South Nepean Collector Sewer Alignment & Finalization Report – Phase 2*, prepared by Novatech, dated June 2014;
- *South Nepean Collector: Phase 2, Hydraulics Review/Assessment, Technical Memorandum*, prepared by Novatech, dated August 20, 2015
- *South Nepean Collector Sewer– Phase 2, Preliminary Design Report*, prepared by Novatech, dated March 2, 2016;
- *Hydrology Report – July 2004: Jock River Flood Risk Mapping (within the City of Ottawa)* prepared by the Rideau Valley Conservation Authority, dated July 2004;
- *Jock River Reach 1 Subwatershed Study and Barrhaven South Master Servicing Study*;
- *Hydraulics Report – November 2004: Jock River Flood Risk Mapping (within the City of Ottawa)* prepared by the Rideau Valley Conservation Authority, dated November 2004;



M:\2011\1117\CAD\Design\Figures\111117-Fig2-CP.dwg, Fig2-CP, Jan 18, 2018 - 10:02am, szorgel



Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
Facsimile (613) 254-5867
Website www.novatech-eng.com

3370 GREENBANK RD.
BURNETT LANDS

CONCEPT PLAN

SCALE 1 : 2000

DATE JAN 2018 JOB 111117 FIGURE 2

CUT1117.DWG 270mm x 182mm

- *Kennedy-Burnett Potable Water Master Servicing Study, by Stantec, dated April 29, 2014;*
- *Greenbank (Burnett Municipal Drain) Headwaters Report, prepared by Bowfin Environmental Consulting and Muncaster Environmental Planning Inc., dated March 2016;*
- *Geotechnical Investigation Proposed Residential Development Burnett Lands, Greenbank Road at the Jock River, Ottawa Ontario, Prepared by Golder Associates Ltd., dated May 2016 (Report No. 1523044-1000)*

2.0 EXISTING CONDITIONS

2.1 Topography & Drainage

The proposed site is currently undeveloped and consists of agricultural lands. Access to the site is currently provided at Greenbank Road. The site has a gentle slope from north to south, with most overland flow being directed to the existing Burnett Municipal Drain, which flows into the Jock River at the south end of the site boundary. The Burnett Municipal Drain transects the site in a north to south fashion.

2.2 Subsurface Conditions

Golder Associates Ltd. has completed a geotechnical investigation in support of the proposed development. The report is titled "Geotechnical Investigation, Proposed Residential Development, Burnett Lands, Greenbank Road at the Jock River, Ottawa Ontario, dated December 2016 (Report No. 1523044-1000)." The fieldwork for this investigation was carried out between February 18th and 23rd, 2016. The work consisted of advancing ten (10) boreholes to depths ranging from 1.7m to 8.2 m below ground surface. The principal findings of the Geotechnical Investigation are as follows:

- The site was divided into two distinct areas (A and B), with Area 'A' having a thick deposit of silty clay up to 8.2m deep and Area 'B' having a very stiff to stiff layer of silty clay overlying glacial till, or glacial till near the ground surface.
- Area 'A' generally consists of a 1.3 to 3.1 m layer of a silty clay deposit beneath the topsoil layer, which has been weathered to a grey brown crust. Below this is a grey silty clay, which extends to a depth of 3.1 to 8.2 m.
 - In some of the boreholes within Area 'A', glacial till was encountered beneath the silty clay deposit at a depth of 3.1 to 4.3 m.
- Area 'B' generally consists of a 0.6 to 2.5 m layer of a silty clay deposit beneath the topsoil layer, which has been weathered to a grey brown crust. Below this is a silty sand layer which ranges from 0.3 to 0.9 m thick. In all boreholes, glacial till exists below the silty clay and silty sand at depths ranging from 0.3 to 2.5 m.
- Groundwater inflow was observed in some of the boreholes at depths of between 0.91 m and 2.17 m below ground surface.

The report provides engineering guidelines based on Golder Associates interpretation of the borehole information and project requirements. Refer to the above-noted report for complete details.

3.0 SANITARY SERVICING

As per the South Nepean Collector Functional Design Update (FDU) prepared by Dillon Consulting (July 2012), the South Nepean Collector (SNC) is the sanitary outlet for the proposed development and has been sized to accommodate the peak sanitary flows from the proposed Burnett Lands development. Refer to Figure 1, Existing Sanitary Network and Collection Areas, and Table 5.1, Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area, of the FDU, located in **Appendix A**. The noted figure and table confirms the development is located within the Sanitary Drainage Area 8A of the SNC.

The design criteria used to determine the sanitary flows are based on the City of Ottawa's sewer design guidelines and are as follows:

- Residential Average Flow = 350L/capita/day
- Residential Peaking Factor = Harmon Equation (max peaking factor = 4.0)
- Institutional Demand = 50,000L/gross ha/day
- Institutional Peaking Factor = 1.5
- Peak Extraneous Flows (Infiltration) = 0.28L/s/ha
- Population Density = 2.7 people/townhouse, 1.8 people/condo
- Minimum Pipe Slope (200mm/250mm) = 0.32% / 0.24%
- Minimum Full Flow Velocity = 0.6m/s
- Maximum Full Flow Velocity = 3.0m/s

In addition to the peak sanitary flows from the proposed Burnett Lands development, the on-site sanitary sewers are sized to accommodate a portion of the peak sanitary flows from the adjacent residential lands located immediately north of the site as well as the existing high school (St. Joseph) immediately east of Greenbank Road. The peak sanitary flow from the adjacent lands is based on 100 townhouses per hectare as per the *South Nepean Town Centre Community Design Plan* (July 2006) prepared by the City of Ottawa. For reference a copy of the South Nepean Town Centre Community Design Plan is included in **Appendix B**. The peak sanitary flow from the high school is based on 50,000 L/gross ha/day as per *South Nepean Collector: Phase 2, Hydraulics Review/Assessment, Technical Memorandum, prepared by Novatech, dated August 20, 2015*. Refer to **Appendix B** for technical memo excerpts.

The proposed sanitary sewer system is shown on the General Plan of Services (Drawing **111117-GP**). The Sanitary Drainage Area Plan (Drawing **111117-SAN**) confirms the sanitary drainage areas assumed to outlet in the proposed onsite sanitary sewers. Both drawings are included in **Appendix G**.

The calculated peak sanitary design flow for the development (including St. Joseph high school, 12.08L/s) is 43.14/s: 12.8L/s will outlet into the SNC at the Half Moon Bay/Jockvale Road intersection and 30.34/s will outlet into the SNC at the Street 5/Jockvale Road intersection. For detailed calculations refer to the Sanitary Sewer Design Sheet located in **Appendix A**. There is a large difference in invert elevation between the proposed sewers within the Burnett lands and the SNC at the Burnett outlets (4.17m). During construction of the SNC, both outlet manholes have been equipped with cast in place drop structures as per 1003.010 to accommodate the drop and sanitary stub for connection.

4.0 WATERMAIN

Ultimately, the Burnett Lands will be serviced with a combination of 50mm, 200mm and 300 mm looped watermain with connections at both the northeast, northwest and southeast limits of the site. At the northeast limits, the watermain will connect to a 300 mm watermain to be located in the realigned Greenbank Road, at the southeast limits, to a future 300mm watermain (by others) located within Jockvale Road and at the northwest limits of the site, to a future watermain located within the lands to the north of the site. Refer to the following figures:

- **Figure 3** – Realigned Greenbank Road Watermain
- **Figure 4** – Watermain Layout

The ultimate watermain connections are consistent with Stantec's *Kennedy-Burnett Potable Water Master Servicing Study* (April 29, 2014) and have not been constructed to date. It has been noted by the City that the southeast connection (by others) should advance and be constructed prior to development of the Burnett Lands. It is proposed to construct approximately 335 m of the 300 mm realigned Greenbank Road watermain from the Greenbank and Jockvale intersection to south of the northeast limits of the site to provide a second connection. These two connections will serve as a loop system for the proposed Burnett development.

It is noted the proposed watermain works are located in a future Zone 3C pressure zone. The realignment of the pressure zone will be completed by the City of Ottawa and once complete will alter the boundary conditions for the development. The realignment is unknown at this time. The City of Ottawa has provided boundary conditions for the pre and post-realignment conditions. This report considers both conditions.

Fire flow demands have been calculated as per the Fire Underwriter's Survey (FUS) and are included in **Appendix C**. However as per the City of Ottawa's technical bulletin ISDTB-2014-02 (Revisions to Ottawa Design Guidelines – Water), the majority of the townhouse fireflows have been capped at 10,000 L/min (167 L/s). Watermain analysis was completed based on the following criteria:

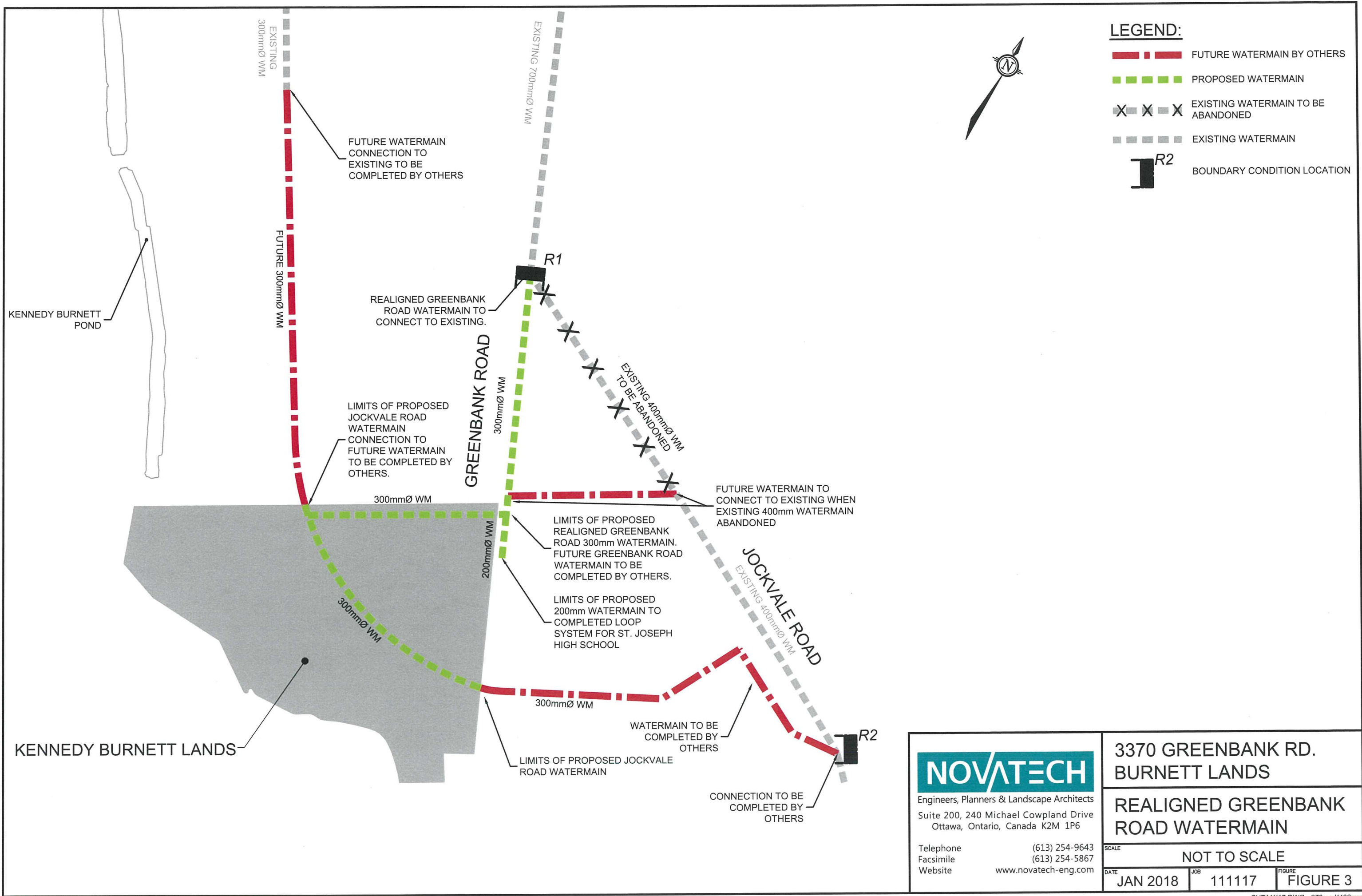
Demands:

- | | |
|---------------------------|----------------------------|
| • Townhouse Density | 2.7 persons/unit |
| • Condo/Apartment Density | 1.8 persons/unit |
| • Average Daily Demand | 350 L/capita/day |
| • Max. Daily Demand | 2.5 x Average Daily Demand |
| • Peak Hour Demand | 2.2 x Maximum Daily Demand |
| • Fire Flow Demand | Fire Underwriters Survey |

System Requirements:

- | | |
|------------------------------------|---------------------------------------|
| • Max. Pressure (Unoccupied Areas) | 690 kPa (100 psi) |
| • Max. Pressure (Occupied Areas) | 552 kPa (80 psi) |
| • Min. Pressure | 276 kPa (40 psi) excluding fire flows |
| • Min. Pressure (Fire) | 138 kPa (20 psi) including fire flows |
| • Max. Age (Quality) | 192 hours (onsite) |

M:\2011\1117\CAD\Design\Figures\111117-Fig3-RealignedWM.dwg, Fig3-RealignedWM, Jan 18, 2018 - 10:08am, szorgel



LEGEND:

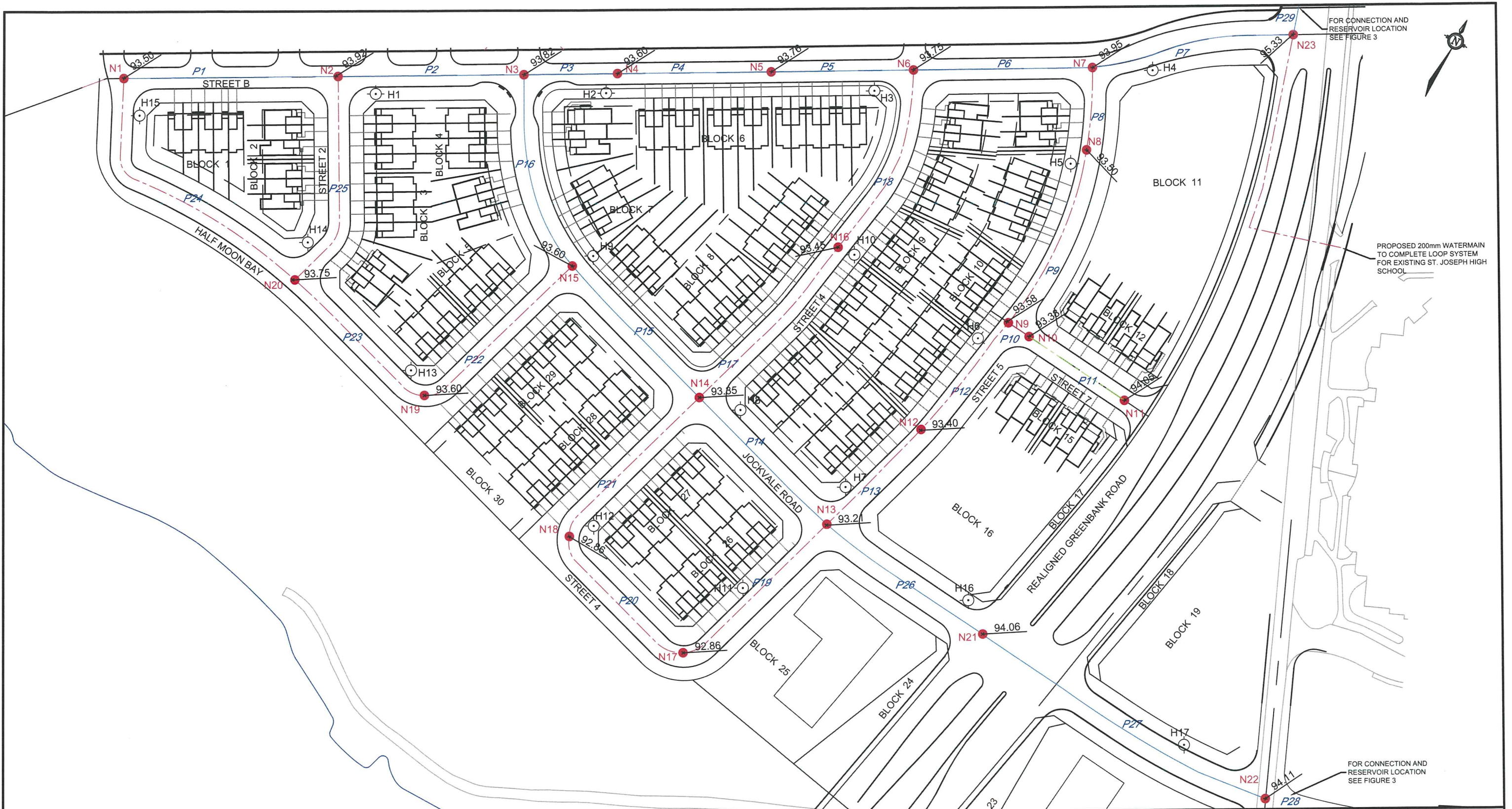
- - - FUTURE WATERMAIN BY OTHERS
- - - PROPOSED WATERMAIN
- - - X X X EXISTING WATERMAIN TO BE ABANDONED
- - - EXISTING WATERMAIN
- R1 BOUNDARY CONDITION LOCATION



<p>Engineers, Planners & Landscape Architects Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6</p> <p>Telephone (613) 254-9643 Facsimile (613) 254-5867 Website www.novatech-eng.com</p>	<p>3370 GREENBANK RD. BURNETT LANDS</p>	
	<p>REALIGNED GREENBANK ROAD WATERMAIN</p>	
<p>SCALE NOT TO SCALE</p>		
DATE	JOB	FIGURE
JAN 2018	111117	FIGURE 3

CUT1117.DWG 330mm x 420mm

M:\2011\1117\CAD\Design\Figures\111117-Fig4-WM.dwg, WM Layout, Jan 18, 2018 - 10:13am, szorgel



FOR CONNECTION AND RESERVOIR LOCATION SEE FIGURE 3

PROPOSED 200mm WATERMAIN TO COMPLETE LOOP SYSTEM FOR EXISTING ST. JOSEPH HIGH SCHOOL

FOR CONNECTION AND RESERVOIR LOCATION SEE FIGURE 3

LEGEND

- 12 PROPOSED 50mmØ WATERMAIN PIPE
- 12 PROPOSED 200mmØ WATERMAIN PIPE
- 4 PROPOSED 300mmØ WATERMAIN PIPE
- WATERMAIN NODE
- RESERVOIR
- HYDRANT

- x 97.75 GROUND ELEVATION
- x FIREFLOWS CAN'T BE CAPPED

NOVATECH
 Engineers, Planners & Landscape Architects
 Suite 200, 240 Michael Cowpland Drive
 Ottawa, Ontario, Canada K2M 1P6
 Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

**3370 GREENBANK RD.
 BURNETT LANDS**

WATERMAIN LAYOUT

SCALE 1:1500

DATE JAN 2018 JOB 111117 FIGURE FIGURE 4

Friction Factors:

- Watermain Size C-Factor
- 50mm 100
- 200-250 mm 110
- 300-400 mm 120

Hydraulic modelling of the proposed 3370 Greenbank Road was completed using EPANET 2.0. EPANET is public domain software capable of modeling municipal water distribution systems by performing simulations of the water movement within a pressurized system. EPANET utilized the Hazen-Williams equation to predict the performance of the proposed watermain and considered the following input parameters: water demand, pipe length, pipe diameter, pipe roughness, and pipe elevation. Table 1 (Water Demand Calculations) in Appendix C confirms the water demands at each node in the system. Table 2 (Pipe Data) in Appendix C confirms the length, diameter, and roughness of each pipe in the system. Tables 3-8 in Appendix C confirms the elevation of each node in the system.

The high pressure condition (average daily demand) was analyzed to ensure the system meets the design criteria for maximum pressure and quality. The maximum daily demand plus fire flow and peak hour conditions were analyzed to ensure the system meets the design criteria for maximum flow and minimum pressure.

The hydraulic modelling results for the development prior to the City reconfiguring the watermain are listed in **Table 4.1**.

Table 4.1: Water Demand Summary (Pre Watermain Reconfiguration)

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	4.53	N/A	690/100 (Max)	628.8/91.2	38
Maximum Daily Demand	11.33	167, 200	138/20 (Min)	326.8/47.4	N/A
Peak Hour	24.93	N/A	276/40 (Min)	514.3/74.6	N/A

The analysis confirms the proposed watermain can service the Burnett Lands prior to the site being included into the realigned Zone 3C pressure zone. It is noted that pressure in the main is greater than 552 kPa/80psi therefore the use of pressure reducing values will be considered during detailed design.

The hydraulic modelling results for the development after the watermain realignment of the watermain are listed in **Table 4.2**.

Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	4.53	N/A	690/100 (Max)	536.4/77.8	38
Maximum Daily Demand	11.33	167, 200	138/20 (Min)	408.9/59.3	N/A
Peak Hour	24.93	N/A	276/40 (Min)	504.7/73.2	N/A

The analysis confirms the proposed watermain can service the Burnett Lands after the watermain realignment under all operating conditions.

A copy of the boundary conditions provided by the City of Ottawa, fire flow calculations, detailed hydraulic analysis results, and watermain layout figure are included in **Appendix C**.

Deviations from the City of Ottawa Design Guidelines – Water Distribution (2010) include:

- Isolation valves are to be located 2.0m away from the intersection, from the point where the projection of the property line intersects the watermain. This distance has been increased to accommodate intersection narrowing along the collector road to improve pedestrian crossings and to ensure no valve chamber is located under curb and located within the roadway. This occurs in the Jockvale/Street B intersection.

5.0 STORMWATER MANAGEMENT CRITERIA

The Burnett Lands are tributary to the Jock River, which falls under the jurisdiction of the Rideau Valley Conservation Authority (RVCA). The following stormwater management criteria have been developed based on the requirements of the RVCA and the City of Ottawa Sewer Design Guidelines (October 2012) and Technical Bulletin PIEDTB-2016-01 (September 2016).

Minor System (Storm Sewers)

- Storm sewers are to be designed using the Rational Method for the 1:5-year return period;
 - Where necessary, pipes are to be oversized to accommodate the HGL elevation;
- Inlet control devices (ICDs) will be installed in road and rearyard catchbasins to control inflows to the storm sewers;
- The 100-year hydraulic grade line in the storm sewer shall be at least 0.3 m below the underside of footing (USF) elevations for the proposed development.

Major System (Overland Flow)

- Maximum depth of flow (static + dynamic) on local and collector streets shall not exceed 0.35 m. The depth of flow may extend adjacent to the right-of-way, provided that the water level must not touch any part of the building envelope and must remain below the lowest building opening during the stress test event;
- Storm runoff that exceeds the capacity of the minor system is to be stored within road sags and conveyed overland along defined major system flow routes towards the proposed major system outlet to the Jock River;
- Surface water accumulation at street low points, during a 2-year event, shall not be present by the end of the rainfall event;
- Major system storage in backyards is not to be included/ accounted for in design computations;
- The product of the 100-year flow depth (m) on street and flow velocity (m/s) shall not exceed 0.60;
- ICD flow rates are to be calculated for each drainage area to ensure that the following criteria are satisfied.

Water Quality & Quantity Control

- An *Enhanced* (80% TSS removal) level of quality control is required for storm outfalls to the Jock River;
- Lot level and conveyance Best Management Practices should be to promote infiltration and treatment of storm runoff.
- Quantity control of post-development runoff to pre-development levels is not required for lands outletting directly to the Jock River, provided that there are no adverse impacts on downstream watercourses, structures, or property resulting from the proposed development. (*Refer to Jock River Reach One Subwatershed Study – Stantec, 2007*)

Erosion and Sediment Control

- Erosion and sediment control measures are to be implemented during construction in accordance with the “Guidelines on Erosion and Sediment Control for Urban Construction Sites” (Government of Ontario, May 1987).
- All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.

6.0 BURNETT MUNICIPAL DRAIN

The existing Burnett Municipal Drain bisects the site from north to south – Refer to **Figure 5**. The drain is a tributary to the Jock River, with a total length of approximately 1.3 km. A significant portion of the original Burnett Drain has been replaced by storm sewers. The drain has been fully enclosed and/or abandoned north of the Barrhaven Town Centre commercial plaza. The Burnett Municipal Drain is primarily an open channel between the Barrhaven Town Centre and the confluence with the Jock River, but a portion of the drain is piped across an existing driving range to the north of the Caivan Lands for a distance of approximately 170 m.

Novatech has prepared a memo (South Nepean Collector Culvert Crossings, June 10, 2016 – included in **Appendix E**) that outlines the sizing of culvert crossings to be installed as a part of the Phase 2 South Nepean Collector project. This memo provides an assessment of the design flows and capacity of the existing Burnett Municipal Drain. During the 100-year storm, the peak flow through the drain is approximately 2.7 m³/s. The existing drain has a cross-section consisting of a 3.0 m bottom width, 0.6 m depth, and side slopes ranging from 2:1 to 4:1. This cross-section gives a bankfull capacity of approximately 3.9 m³/s, based on Manning's equation.

Under interim conditions, the drain will be re-directed to the west, around the boundary of the site, connecting with the existing outlet for the KB Pond – as shown on **Figure 5**. To maintain the capacity of the drain, the realigned channel should maintain a cross-section consistent with the existing drain. Ultimately, it is anticipated that the drain will be abandoned as part of planned future development upstream.

Once construction of the subdivision and surrounding developments have been completed the municipal drain will be formally abandoned, as all areas originally directed to the drain will be captured by the proposed storm sewer systems.

As a part of the Municipal Drain Headwaters report, the Burnett Drain was classified using the four-step process of the Headwater Guidelines:

- Hydrology Classification
 - Municipal drain provides valued hydrologic function (due to flow in spring);
 - Tributary drains are constructed agricultural drains with limited hydrologic function.
- Riparian Classification
 - Municipal Drain and Tributary 1 have limited riparian functions;
 - Tributaries 2 & 3 have limited to contributing riparian functions.
- Fish and Fish Habitat Classification
 - Municipal Dain is considered to have a contributing fish habitat;
 - Tributaries have no value as fish habitat.
- Terrestrial Classification
 - Municipal Drain and Tributaries have limited terrestrial functions.

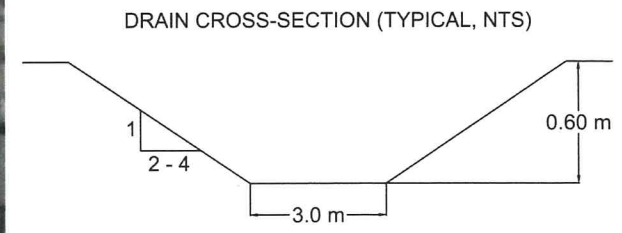
Because of these classifications, it is recommended that there is no management required for the tributaries of the Burnett Municipal Drain and that they can be abandoned. It is recommended that the Burnett Municipal Drain be managed through mitigation, which can include replicating the function of the drain through lot level conveyance best management practices (i.e. directing roof leaders to grassed areas). Additional information on the Burnett Drain is available in Headwater Report referenced in **Section 1.2** of this report. Further information on possible mitigation measures are outlined in the Evaluation, Classification and Management of Headwater Drainage Features Guidelines (CVC & TRCA, January 2014).

I:\2011\1117\CAD\Design\Figures\SWM\11117-Fig5.dwg, Figs-Ex Cond, Jan 12, 2018 - 8:44am, szorgel



LEGEND

- PROPERTY BOUNDARY
- WATERCOURSE
- EXISTING BURNETT MUNICIPAL DRAIN
- - - RE-ALIGNED BURNETT MUNICIPAL DRAIN
- KB POND OUTLET (BURIED PIPE)
- ▶ DIRECTION OF FLOW



NOVATECH
 Engineers, Planners & Landscape Architects
 Suite 200, 240 Michael Cowpland Drive
 Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

3370 GREENBANK RD.
 KENNEDY BURNETT LANDS

BURNETT MUNICIPAL DRAIN

SCALE 1 : 2500

DATE JAN 2018 JOB 111117 FIGURE 5

7.0 STORMWATER MANAGEMENT DESIGN

Storm servicing for the subject development will be provided using a dual drainage system: Runoff from frequent events will be conveyed by storm sewers (minor system), while flows from large storm events which exceed the capacity of the minor system will be conveyed overland along defined overland flow routes (major system).

The stormwater management design for the Burnett Lands conforms to the recommended stormwater management strategy for the planned Environmental Assessment update for the Kennedy-Burnett SWM Facility – refer to **Figure 113221 FIG-6** in **Appendix D**.

7.1 Storm Sewer Design (Minor System)

The proposed storm sewers have been designed using the Rational Method to convey peak flows associated with a 5-year return period. The storm sewer design sheet is provided in **Appendix A**. The corresponding Storm Drainage Area Plan (Drawing **111117-STM**) is provided in **Appendix G**. The design criteria used to size the storm sewers are summarized in **Table 7.1**.

Table 7.1: Storm Sewer Design Parameters

Parameter	Design Criteria
Local & Collector Roads	5-year Return Period
Storm Sewer Design	Rational Method/Modeling
IDF Rainfall Data	Ottawa Sewer Design Guidelines (Oct. 2012)
Initial Time of Concentration (T_c)	15 minutes (rearyards) / 10 minutes (roads)
Minimum Velocity	0.8 m/s
Maximum Velocity	3.0 m/s
Minimum Diameter	250 mm

7.2 Inlet Control Devices

Inlet control devices (ICDs) will be installed in all catchbasins to limit inflows to the minor system during large (>1:5 year) storm events.

All catchbasins will have a single connection to the storm sewers. ICDs will be round orifice plugs with diameters of either 83mm, 94mm, 102mm, 127mm, 152mm, or 178mm.

7.3 Overland Flow Path (Major System)

The site has been graded to provide an engineered overland flow route (major system) for large, infrequent storms or in the event that the storm sewer system becomes obstructed. Flows will be directed to the Jock River at the low point in the system. The design of the major system conforms to the design standards outlined in Section 5.5 of the Sewer Design Guidelines.

7.4 Street 'B' / Caivan Lands

Street 'B' is a shared road between the Burnett Lands and the Caivan Lands to the north. Approximately 1.66 ha of the Burnett Lands development and approximately 2.61 ha of the Caivan Lands will drain to Street 'B'. Since Street 'B' will be constructed as a part of the Burnett Lands Development, the street and tributary areas have been included in the design.

Storm runoff from the Caivan Lands and Street 'B' will be directed to the outlet channel for Kennedy-Burnett SWM facility by a shared storm sewer. Water quality treatment upstream of this outfall will be provided using a hydrodynamic separator at the western end of Street 'B' (refer to the General Plan of Services, **111117-GP** in **Appendix G**).

7.5 Water Quantity & Quality Control

Quantity Control

As outlined in the Jock River Reach One Subwatershed Study (Stantec, June 2007); "No quantity control storage required for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River."

Quality Control

An *Enhanced* (80% TSS removal) level of water quality control will be provided by using hydrodynamic separators (HDS) upstream of the storm outfalls.

- Storm runoff from the majority of the site will be treated by an HDS unit upstream of the outfall to the Jock River near the southeast corner of the site.
- Storm runoff from Street B will be treated by an HDS unit located upstream of the outfall to the Kennedy-Burnett SWM Facility outlet channel near the northwest corner of the site. This storm outlet and HDS unit will also serve as the storm outlet for the adjacent Caivan development to the north.

In addition to the HDS units, lot level and conveyance Best Management Practices should be considered at the detailed design stage to promote infiltration and treatment of storm runoff;

- Perforated pipes for rear-yard catchbasin leads;
- Direct roof leaders to rear-yard areas;
- Infiltration trenches underlying swales in rear-yard areas;
- The use of fine sandy loam topsoil on residential lawns;

8.0 HYDROLOGIC & HYDRAULIC MODELING

The *City of Ottawa Sewer Design Guidelines* (October 2012) requires hydrologic modeling for all dual drainage systems. The performance of the proposed storm drainage system for the Burnett Lands was evaluated using the PCSWMM hydrologic/hydraulic model.

Modeling files are provided on the enclosed CD.

8.1 Design Storms

The hydrologic analysis was completed using the following synthetic design. The IDF parameters used to generate the design storms were taken from the *City of Ottawa Sewer Design Guidelines* (October 2012).

4 Hour Chicago Storms:

- 25mm 4hr Chicago storm
- 2-year 4hr Chicago storm
- 5-year 4hr Chicago storm
- 100-year 4hr Chicago storm
- 100-year 4hr +20% Chicago storm

The 4-hour Chicago distribution generates the highest peak flows for both the minor and major systems and was determined to be the critical storm distribution for the design of the storm drainage system.

The proposed drainage system has also been stress tested using a 4-hour Chicago design storm that has a 20% higher intensity and total volume compared to the 100-year event.

8.2 Model Development

The PCSWMM model has been developed to account for both minor and major system flows from the development, and ensure no adverse impacts on the downstream drainage system. The results of the analysis were used to:

- Determine the total major and minor system runoff from the site.
- Calculate the storm sewer hydraulic grade line for the 100-year storm event;
- Evaluate overland flow depths and ponding volumes during the 100-year event; and
- Ensure no ponding in the right-of-ways remains at the end of all storm events;

8.2.1 Storm Drainage Areas

For modeling purposes, the site has been divided into subcatchments based on the drainage areas tributary to each inlet of the proposed storm sewer system. The catchment areas are shown on the Storm Drainage Area Plan (111117-STM) in **Appendix G**.

8.2.2 Subcatchment Model Parameters

The hydrologic parameters for each subcatchment were developed based on the Concept Plan (**Figure 2**) and the Storm Drainage Area Plan (111117-STM). An overview of the modeling parameters is provided in **Table 8.1**.

Table 8.1: Hydrologic Modeling Parameters

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Impervious (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
Burnett Lands - Claridge						
A01	0.163	0.65	64%	50%	12	0.85
A02	0.106	0.65	64%	100%	24	1.50
A03	0.352	0.65	64%	50%	21	0.75
A04	0.258	0.65	64%	50%	14	0.75
A05	0.032	0.65	64%	100%	26	1.50
A06	0.287	0.65	64%	50%	22	0.75
A07	0.178	0.65	64%	100%	27	2.00
A08	0.218	0.65	64%	50%	15	0.70
A09	0.265	0.65	64%	50%	27	1.30
A10	0.108	0.65	64%	50%	13	0.85
A11	0.147	0.65	64%	100%	26	1.80
A12	0.276	0.65	64%	50%	22	0.55
A13	0.143	0.65	64%	100%	36	1.50
A14	0.161	0.65	64%	50%	19	0.75
A15	0.215	0.65	64%	100%	28	1.50
A16	0.200	0.65	64%	50%	18	0.70
A17	0.135	0.65	64%	100%	37	1.50
A18	0.219	0.65	64%	50%	22	0.85
A19	0.285	0.65	64%	50%	19	0.75
A20	0.195	0.65	64%	100%	27	1.50
A21	0.154	0.65	64%	50%	15	1.00
A22	0.474	0.65	64%	50%	52	1.00
A23	0.070	0.65	64%	100%	15	1.50
A24	0.185	0.65	64%	50%	17	0.85
A25	0.174	0.65	64%	100%	29	1.50
A26	0.239	0.65	64%	50%	22	1.70
A27	0.071	0.65	64%	100%	14	1.50
A28	0.192	0.65	64%	50%	18	1.00
A29	0.317	0.65	64%	50%	55	1.00
A30	0.269	0.65	64%	50%	14	1.00
A31	0.269	0.65	64%	50%	14	0.80
A32	0.476	0.65	64%	50%	64	1.00
A33	0.145	0.65	64%	50%	15	1.30
A34-35	0.588	0.65	64%	50%	32	1.30
A-36	0.845	0.65	64%	50%	99	1.00
A-37	0.734	0.65	64%	0%	24	1.00
A-38	0.744	0.65	64%	0%	27	1.00

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Impervious (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
A-39	0.376	0.65	64%	0%	26	1.00
A-40	0.303	0.65	64%	0%	22	1.00
Street B - Caivan Lands						
B-01	0.135	0.65	64%	50%	10	2.00
B-02	0.824	0.65	64%	50%	62	1.00
B-03	0.095	0.65	64%	100%	26	1.80
B-04	0.137	0.65	64%	50%	10	0.60
B-05	0.226	0.65	64%	50%	15	0.75
B-06	0.200	0.65	64%	100%	31	1.50
B-07	0.204	0.65	64%	50%	13	1.00
B-08	1.111	0.65	64%	0%	70	0.85
B-09	0.090	0.65	64%	50%	22	1.00
B-10	0.186	0.65	64%	50%	13	0.85
B-11	0.097	0.65	64%	50%	11	1.00
B-12	0.111	0.65	64%	100%	25	2.00
B-13	0.120	0.65	64%	50%	14	1.00
B-14	0.136	0.65	64%	50%	15	1.00
B-15	0.026	0.65	64%	100%	12	1.80
B-16	0.121	0.65	64%	50%	14	1.00
B-17	0.342	0.65	64%	50%	19	0.85
B-18	0.107	0.65	64%	50%	22	1.00
CaivanLands	8.251	0.30	14%	50%	200	1.00

Infiltration

Infiltration losses for all catchment areas were modeled using Horton's infiltration equation, which defines the infiltration capacity of the soil over the duration of a precipitation event using a decay function that ranges from an initial maximum infiltration rate to a minimum rate as the storm progresses. The default values for the City of Ottawa were used for all catchments.

Horton's Equation:

$$f(t) = f_c + (f_o - f_c)e^{-k(t)}$$

Initial infiltration rate: $f_o = 76.2$ mm/hr
 Final infiltration rate: $f_c = 13.2$ mm/hr
 Decay Coefficient: $k = 4.14$ /hr

Depression Storage

The default values for depression storage in the City of Ottawa were used for all catchments.

- Depression Storage (pervious areas): 4.67 mm
- Depression Storage (impervious areas): 1.57 mm

Residential rooftops are assumed to provide no depression storage and all rainfall is converted to runoff.

Equivalent Width

'Equivalent Width' refers to the width of the sub-catchment flow path. This parameter (Table 5.1) is calculated as described in the *Sewer Design Guidelines, October 2012, Section 5.4.5.6*.

Impervious Values

Impervious (%IMP) values for each subcatchment area were calculated based on the concept plan (**Figure 2**). The impervious values correspond to the Runoff Coefficients (C) used in the Rational Method calculations using the equation:

$$\%IMP = \frac{C - 0.2}{0.7}$$

For the Storm Sewer Design spreadsheet, typical lots were analyzed with respect to the concept plan and runoff coefficients were determined based on the proposed land use. For development consisting of primarily medium-density residential (multi-unit attached dwellings) a runoff coefficient (C) of 0.65 was selected.

8.2.3 Minor System

Inflows to the storm sewer were modeled based on the ICD specified for the inlet and the maximum depth of ponding. Storage volumes within the right-of-way are based on the grading design. ICD parameters are outlined as follows in **Table 8.2**.

Table 8.2: ICD Parameters

Structure	ICD Size & Inlet Rate				5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	Calculated Inlet Capture Rate (L/s)	Actual 5-year Capture Rate* (L/s)	
Burnett Lands - Claridge					
CB01-02	83	1.56	18.57	19.80	29.70
CB03-04	94	1.56	23.77	25.56	44.59
CB05-06	108	1.55	31.31	33.70	52.18
CB07-08	102	1.55	27.96	30.46	61.87
CB09-10	108	1.54	31.23	33.68	56.00
CB11-12	94	1.55	23.71	25.67	47.24
CB13-14	94	1.56	23.77	25.45	37.03
CB15-16	102	1.55	27.96	30.06	44.36
CB17-18	108	1.55	31.31	33.51	53.44
CB19-20	94	1.55	23.72	25.52	41.77
CB21-22	108	1.55	31.31	34.14	61.32
CB23-24	83	1.56	18.57	19.69	25.32
CB25-26	83	1.56	18.56	19.85	28.06
CB27-28	108	1.55	31.32	33.67	46.27
CB29-30	108	1.54	31.25	33.24	50.10
CB31-32	94	1.56	23.77	25.44	36.38
CB33-34	108	1.55	31.30	33.55	46.44

Structure	ICD Size & Inlet Rate				5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	Calculated Inlet Capture Rate (L/s)	Actual 5-year Capture Rate* (L/s)	
CB35-36	94	1.56	23.77	25.66	44.72
CB37-38	83	1.56	18.57	19.86	29.23
RYCB01	83	1.61	18.85	18.98	4.82
RYCB02	83	1.56	18.55	4.82	18.98
RYCB03	83	1.96	20.79	19.85	19.85
RYCB04	83	1.64	19.02	20.45	20.45
RYCB05	83	1.46	17.94	5.93	5.93
RYCB06	83	2.07	21.37	22.82	22.82
RYCB07	83	1.85	20.20	21.74	21.74
RYCB08	83	2.13	21.68	23.31	23.31
RYCB09	83	1.97	20.85	22.19	22.18
RYCB10	83	2.07	21.37	22.72	22.72
RYCB11	83	1.77	19.76	17.22	17.22
RYCB12	83	2.01	21.06	22.61	22.61
RYCB13	83	2.82	24.95	25.14	25.13
RYCB14	83	1.85	20.20	12.96	12.96
RYCB15	83	1.83	20.09	12.80	12.80
Street B - Caivan Lands					
CB39-40	127	1.54	43.16	45.54	59.94
CB41-42	94	1.56	23.77	25.05	33.57
CB43-44	127	1.54	43.16	46.48	73.10
CB45-46	102	1.55	27.96	29.79	40.21
CB47-48	83	1.56	18.56	19.62	24.27
CB49-50	83	1.56	18.57	19.71	25.99

*From PCSWMM Model, 5-year 4-hour Chicago storm distribution

8.2.4 Major System

The proposed road network was input into the PCSWMM model to calculate the overland flows and flow depths within the right-of-way (major system).

The roads are represented in the model as open channels. Model input includes:

- Right-of-way cross-sections;
- Length and slope of the road between each high and low point;
- The location of all storm inlets and whether the inlets are in a sag or on-grade.

The elevations used to define the road network in the PCSWMM model are based on the gutter elevations, as opposed to the centerline of road elevations shown on the Grading Plans (**111117-GR1-3** as provided in **Appendix G**). Right-of-way cross sections used in the PCSWMM model are provided in **Appendix D**.

8.2.5 Modeling Files / Schematic

The PCSWMM model schematics and 100-year model output data are provided in **Appendix D**. Digital copies of the modeling files and model output for all storm events are provided on the enclosed CD.

8.3 PCSWMM Model Results

8.3.1 Minor System

The minor system peak flows were evaluated using the 4-hour Chicago distribution for a full range of return periods. The results are summarized in **Table 8.3**.

Table 8.3: Minor System Peak Flows at Outlets

Storm Distribution->	4hr Chicago				
Return Period->	25mm	2yr	5yr	100yr	100yr +20%
Burnett Lands - Flows to Jock River					
Minor System (HW-01)	643	904	1077	1282	1306
Major System (OVF-OUT)	0	0	0	1	6
Burnett & Caivan Lands - Flows to Fraser-Clarke Drain					
Outlet from Street B (HW-02)	789	1194	1611	1752	1786
Offsite Flows					
Greenbank Road (GRBK-OUT)	176	255	364	689	854

8.3.2 Major System

The major system network was evaluated using the PCSWMM model to ensure that the ponding depths conform to City standards. A summary of ponding depths and volumes for the 100-year event is provided in **Table 8.4**. Ponding volumes and depths for all storm events (including the 100-yr+20% event and 5-year event) are provided in **Appendix D**.

Table 8.4: Ponding Depths at Catchbasins (100yr Event)

Structure	T/G (m)	Max. Static Ponding		100-yr Event (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)
Burnett Lands - Claridge							
CB01-02	93.39	93.65	0.26	93.56	0.17	N	0.00
CB03-04	93.27	93.58	0.31	93.46	0.19	N	0.00
CB05-06	93.17	93.50	0.33	93.38	0.21	N	0.00
CB07-08	93.33	93.69	0.36	93.59	0.26	N	0.00
CB09-10	93.26	93.50	0.24	93.49	0.23	N	0.00
CB11-12	93.24	93.43	0.20	93.46	0.22	Y	0.03
CB13-14	93.30	93.56	0.26	93.48	0.18	N	0.00
CB15-16	93.32	93.52	0.20	93.55	0.23	Y	0.03

Structure	T/G (m)	Max. Static Ponding		100-yr Event (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)
CB17-18	93.10	93.39	0.29	93.28	0.18	N	0.00
CB19-20	92.93	93.12	0.19	93.14	0.21	Y	0.02
CB21-22	92.73	92.89	0.16	92.97	0.24	Y	0.08
CB23-24	92.69	92.89	0.20	92.94	0.25	Y	0.05
CB25-26	92.73	92.89	0.16	92.94	0.21	Y	0.05
CB27-28	92.85	92.92	0.07	93.02	0.17	Y	0.10
CB29-30	92.88	92.92	0.04	93.02	0.14	Y	0.10
CB31-32	93.07	93.27	0.20	93.26	0.19	N	0.00
CB33-34	93.25	93.45	0.20	93.46	0.21	Y	0.01
CB35-36	93.27	93.45	0.18	93.48	0.21	Y	0.03
CB37-38	93.35	93.55	0.20	93.53	0.18	N	0.00
Street B - Caivan Lands							
CB39-40	93.17	93.45	0.28	93.25	0.08	N	0.00
CB41-42	93.44	93.69	0.25	93.51	0.07	N	0.00
CB43-44	93.40	93.69	0.29	93.74	0.34	Y	0.05
CB45-46	93.41	93.62	0.21	93.58	0.17	N	0.00
CB47-48	93.47	93.62	0.15	93.59	0.12	N	0.00
CB49-50	93.67	93.82	0.15	93.80	0.13	N	0.00

8.3.3 Hydraulic Grade Line

Peak flows from the subdivision will not coincide with peak flows in the Jock River. Consequently, a combined frequency analysis was used to assess the maximum HGL in the storm sewers. The hydraulic analysis was initially modeled for two scenarios:

- 1) 5-year flows in the storm sewers, 100-year flood elevation at the outlet (Jock River)
- 2) 100-year flows in the storm sewers, 5-year flood elevation at the outlet (Jock River)

The 5-year peak flow combined with the 100-year flood elevation of 91.28 at the Jock River and 91.58 at the Fraser-Clarke Drain produced the highest HGL values, representing the worst-case scenario. This scenario was used for all subsequent analysis.

Table 8.5 provides a summary of the 5-year peak flow combined with the 100-year flood elevation HGL elevation at each storm manhole within the proposed development. These HGL elevations have also been included on the Plan & Profile Drawings (**11117-P1-9**).

HGL elevations for the 100-year flows in the storm sewers, 5-year flood elevation at the outlet as well as a summary of the HGL elevations for a 20% increase (rainfall intensity and total precipitation) in the 100-year design event have been included in **Appendix D**. The results of this stress testing indicate that, even under this scenario, the hydraulic grade line in the sewers will only slightly increase (max increase of 0.03m), ensuring that the HGL will remain below the undersides of footing of the proposed units.

**Table 8.5: Storm Sewer Hydraulic Grade Line:
5yr-4hr Chicago Distribution, 100-yr WL in Jock River**

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elevation (m)	Design USF (m)	USF Clearance (m)
Burnett Lands - Claridge					
200 (STM)	90.92	93.70	91.71	92.03	0.32
202 (STM)	90.81	93.39	91.71	92.02	0.31
204 (STM)	90.78	93.41	91.72	92.02	0.30
300 (STM)	90.38	93.50	91.72	92.02	0.30
302 (STM)	90.28	93.67	91.71	92.02	0.31
304 (STM)	90.05	93.73	91.68	92.01	0.33
306 (STM)	90.90	93.26	91.68	92.02	0.34
400 (STM)	90.50	93.51	91.79	92.34	0.55
402 (STM)	90.38	93.41	91.78	92.34	0.56
404 (STM)	90.31	93.58	91.78	92.33	0.55
406 (STM)	90.11	93.57	91.78	92.32	0.54
408 (STM)	90.94	93.01	91.70	92.00	0.30
410 (STM)	90.12	92.99	91.65	91.99	0.34
412 (STM)	89.26	92.65	91.61	92.02	0.41
500 (STM)	90.74	93.62	91.82	92.12	0.30
502 (STM)	90.57	93.49	91.82	92.12	0.30
504 (STM)	90.42	93.37	91.82	92.12	0.30
506 (STM)	90.43	93.50	91.80	92.12	0.32
508 (STM)	89.93	93.31	91.76	92.10	0.34
600 (STM)	90.85	93.50	91.78	92.35	0.57
602 (STM)	90.64	93.33	91.78	92.32	0.54
604 (STM)	90.26	93.45	91.78	92.32	0.54
606 (STM)	89.95	93.26	91.76	92.32	0.56
608 (STM)	89.58	93.08	91.71	92.08	0.37
700 (STM)	88.65	94.14	91.81	92.12	0.31
802 (STM)	89.92	93.82	91.61	-	-
804 (STM)	89.82	93.93	91.60	-	-
806 (STM)	89.28	93.34	91.50	-	-
808 (STM)	89.25	93.38	91.54	-	-
810 (STM)	89.27	93.40	91.32	-	-
900 (STM)	89.18	92.61	91.57	-	-
902 (STM)	89.00	93.09	91.44	-	-
Street B - Caivan Lands					
100 (STM)	89.71	93.32	91.64	92.03	0.39
102 (STM)	89.86	93.37	91.67	92.03	0.36
104 (STM)	89.89	93.66	91.70	92.03	0.33

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elevation (m)	Design USF (m)	USF Clearance (m)
106 (STM)	90.14	93.75	91.73	-	-
108 (STM)	90.18	93.55	91.77	-	-
110 (STM)	90.31	93.71	91.79	-	-
112 (STM)	90.44	93.51	91.83	92.42	0.59
114 (STM)	90.62	93.66	91.89	92.49	0.60
116 (STM)	91.02	93.83	91.94	-	-
118 (STM)	90.99	94.51	91.94	-	-

8.3.4 On-Site Storage (Future Development Blocks)

Within the Burnett Lands are several blocks that are to be developed at a later date. Storm runoff from these blocks will be captured by the Burnett Lands storm sewer system. To simulate the stormwater management for these blocks, the subcatchment areas are directed to storage nodes, which are connected to the downstream sewers via orifices which have been sized for the 5-year uncontrolled peak flow from the block. Peak flows and required storage volumes are outlined in **Table 8.6**.

Table 8.6: Peak Flows & Storage Volumes – Future Development Blocks

Block ID	Drainage Area ID	Area (ha)	Allowable Release Rate* (L/s)	Storage Required (100-year event)	
				Total Volume (m ³)	Per-Hectare Volume (m ³ /ha)
Block 11	A22	0.474	91	38	80
Block 16	A29	0.317	64	27	85
Block 19	A34-35	0.588	105	35	60
Block 20	A36	0.845	164	61	72
Block 25	A32	0.476	93	35	74

*The allowable release rate is the 5-year uncontrolled flow calculated using the PCSWMM model

Storage nodes have been sized to provide enough storage for the 100-year event, without overflow or flooding. It is assumed that with storage volumes under 100m³/ha, the required storage can be achieved on-site through a combination of low-point ponding in parking lots and roadways and rooftop storage.

9.0 UTILITIES

The development will be serviced by hydro, phone, cable, and gas from the existing services on Greenbank Road. During detailed design, the works will be coordinated with local utility companies.

10.0 EROSION AND SEDIMENT CONTROL

Erosion and sediment control measures will be implemented during construction in accordance with the "Guidelines on Erosion and Sediment Control for Urban Construction Sites" (Government of Ontario, May 1987). Detailed plans will be provided at the detailed design stage.

Typical erosion and sediment control measures recommended include, but are not limited to, the use of silt fences around perimeter of site (OPSD 219.110), filter fabric or inserts under catch basin/maintenance hole lids, heavy duty silt fence barrier (OPSD 219.130), straw bale check dams (OPSD 219.180), rock check dams (219.210 or OPSD 219.211), turbidity curtain (OPSD 219.260), dewatering trap (OPSD 219.240), temporary water passage system (OPSD 221.030), riprap (OPSS 511), mud mats, silt bags for dewatering operations, topsoil and sod to disturbed areas and natural grassed waterways. Dewatering and sediment control techniques will be developed for the individual situations based on the above guidelines and utilizing typical measures to ensure erosion and sediment control is controlled in an acceptable manner and there is no negative impact to adjacent lands, water bodies or water treatment/conveyance facilities.

It will be the responsibility of the Contractor to submit a detailed construction schedule and appropriate staging, dewatering and erosion and sediment control plans to the Contract Administrator for review and approval prior to the commencement of work.

All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.

- A qualified inspector should conduct daily visits during construction to ensure that the contractor is working in accord with the design drawings and that mitigation measures are being implemented as specified.
 - A light duty silt fence barrier is to be installed in the locations shown on the Erosion and Sediment Control Plan.
 - Straw bale barriers are to be installed in drainage ditches
 - Inserts are to be placed under the grates of all proposed and existing catchbasins and structures.
 - After complete build-out, all sewers are to be inspected and cleaned and all sediment and construction fencing is to be removed.
- The contractor shall ensure that proper dust control is provided with the application of water (and if required, calcium chloride) during dry periods.
- The contractor shall immediately report to the engineer or inspector any accidental discharges of sediment material into any ditch or sewer system. Appropriate response measures shall be carried out by the contractor without delay.
- The contractor acknowledges that failure to implement erosion and sediment control measures may result in penalties imposed by any applicable regulatory agency.

11.0 CONCLUSIONS AND RECOMMENDATIONS

Sanitary Servicing

The analysis of the proposed sanitary servicing conforms the following:

- The site is located within Sanitary Drainage Area 8A of Phase 2 of the SNC.
- The on-site sanitary sewers have been sized to accommodate a portion of the peak sanitary flows from the adjacent lands to the north, to be developed by others, and St. Joseph high school located east of the development across Greenbank Road.
- The proposed sanitary sewers have adequate capacity to accommodate the peak sanitary flow.

Watermain

The analysis of the proposed watermain network confirms the following:

- Approximately 335m of the 300mm Greenbank Road watermain is to be installed and connected to the proposed 300mm at the northeast limits of the site. A second connection will be provided by connecting to a future 300mm watermain (by others) on Jockvale Road at the southeast limits of the site. These connections will provide a loop system for the development.
- Ultimately the site will connect to a future watermain, to be completed by others, to the northwest.
- The site is located in a future Zone 3C pressure zone that once complete will increase the pressures in the system onsite for the maximum daily and peak hour conditions. The average day pressures will decrease and therefore improve the system by lowering pressures within acceptable standards (<80psi).
- This report confirms the proposed watermain can service the site pre and post zone reconfiguration.
- During detailed design the use of pressure reducing valves will be explored.

Stormwater Management

The stormwater management design for the Burnett Lands development conforms to the criteria established as part of this report.

The conclusions based on the results of the stormwater management analysis are as follows:

Storm Drainage / Conveyance

- Storm sewers (minor system) have been designed to convey the uncontrolled 5-year peak flow using the Rational Method.
- Inflows to the minor system will be controlled using inlet control devices (ICDs). Proposed ICDs will consist of round orifice plates with various standard diameters (83mm, 94mm, 102mm, 108mm, 127mm, 152mm, and 178mm).
- The site has been graded to provide surface storage at low points along roadways, and a major system outlet to the Jock River for flows which exceed the capacity of the road sags.
- Ponding depths will not exceed 0.35m for all storms up to and including the 100-year event.

- The post-development peak flows from the site will have no adverse impact on the Jock River downstream.
- A minimum clearance of 0.30m will be provided between the 100-year hydraulic grade line (HGL) and the designed underside of footing elevations.

Stormwater Management

- An *Enhanced* level of water quality control (80% TSS removal) will be provided using hydrodynamic separators upstream of the storm outfalls.
- Quantity control is not required for the Burnett Lands, as they are located adjacent to the Jock River.

Erosion and Sediment control

- Erosion and sediment control measures (i.e. filter fabric, silt fences, etc.) will be implemented prior to construction and are to remain in place until vegetation is established.

The preceding report is respectfully submitted for review and approval. Please contact the undersigned should you have questions or require additional information.

NOVATECH

Stormwater Management and Modeling
Prepared by:

Sanitary and Water Modeling
Prepared by:

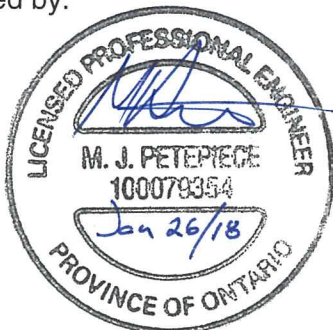


Kallie Auld, P.Eng.
Project Coordinator, Water Resources

Steve Zorgel, P.Eng.
Project Coordinator, Engineering

Reviewed by:

Approved by:



A handwritten signature in blue ink, appearing to read "Marc St. Pierre".

Michael Petepiece, P.Eng.
Senior Project Manager, Water Resources

Marc St. Pierre
Senior Project Manager

Appendix A
Design Sheets

Project: 111117
 Location: 3370 Greenbank Rd.
 Client: Claridge Homes



DATE: November 2016
 REVISED: January 2018



Storm Sewer Design Sheet

STREET	FROM	TO	AREA ID	FLOW							PROPOSED SEWER							Q/Qfull	
				R=0.30	R=0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)		EXCESS CAPACITY (l/s)
Half Moon Bay	MH 300	MH 302	A1		0.163	0.29	0.29	10.00	104.19	50.65	600	609.6	0.23	42.8	307.51	1.05	0.68	256.87	0.16
Half Moon Bay			A2		0.106	0.19	0.49												
Half Moon Bay	MH 200	MH 202	Services Only																
Half Moon Bay	MH 202	MH 204	A3		0.352	0.64	0.64	10.00	104.19	66.27	600	609.6	0.37	5.4	390.03	1.33	0.07	323.76	0.17
Half Moon Bay	MH 204	MH 302				0.64	0.64	10.07	103.84	66.05	600	609.6	0.20	25.1	286.76	0.98	0.43	220.71	0.23
								10.49											
Half Moon Bay	MH 302	MH 304	A4		0.258	0.47	1.59	10.68	100.74	165.84	600	609.6	0.20	74.0	286.76	0.98	1.26	120.91	0.58
Half Moon Bay			A5		0.032	0.06	1.65												
Half Moon Bay	MH 306	MH 304	A6		0.287	0.52	0.52	10.00	104.19	54.04	600	609.6	0.30	69.0	351.20	1.20	0.96	297.17	0.15
								10.96											
Block 28/29/30	MH 304	MH 410	A7		0.178	0.32	2.49	11.93	94.98	236.15	675	685.8	0.20	70.0	392.57	1.06	1.10	156.42	0.60
								13.03											
Street 4	MH 408	MH 410	A8		0.218	0.39	0.39	10.00	104.19	90.94	600	609.6	0.30	67.9	351.20	1.20	0.94	260.27	0.26
			A9		0.265	0.48	0.87												
Street 4	MH 410	MH 412	A10		0.108	0.20	3.55	13.03	90.50	345.72	750	762.0	0.15	78.0	450.27	0.99	1.32	104.55	0.77
Block 26/27			A11		0.147	0.27	3.82												
Jockvale	MH 600	MH 602	A12		0.276	0.50	0.50	10.00	104.19	51.96	600	609.6	0.57	35.0	484.10	1.66	0.35	432.14	0.11
Jockvale	MH 602	MH 604	A13		0.143	0.26	0.76	10.35	102.37	77.51	600	609.6	0.20	35.1	286.76	0.98	0.60	209.25	0.27
Jockvale	MH 604	MH 606	A14		0.161	0.29	1.05	10.95	99.44	142.85	600	609.6	0.20	79.6	286.76	0.98	1.35	143.91	0.50
			A15		0.215	0.39	1.44												
Street 4	MH 400	MH 402	A16		0.200	0.36	0.36	10.00	104.19	37.66	600	609.6	0.45	24.3	430.13	1.47	0.28	392.48	0.09
Street 4	MH 402	MH 404	A17		0.135	0.24	0.61	10.28	102.76	62.21	600	609.6	0.25	28.2	320.60	1.10	0.43	258.40	0.19
Street 4	MH 404	MH 406	A18		0.219	0.40	1.00	10.70	100.62	100.73	600	609.6	0.20	47.2	286.76	0.98	0.80	186.03	0.35
Street 4	MH 406	MH 606	A19		0.285	0.51	1.52	11.50	96.86	146.85	675	685.8	0.15	55.4	339.98	0.92	1.00	193.12	0.43
								12.51											
Jockvale	MH 606	MH 608	A20		0.195	0.35	3.31	12.51	92.58	305.96	750	762.0	0.21	78.0	532.76	1.17	1.11	226.80	0.57
								13.62											

STREET	FROM	TO	AREA ID	FLOW							PROPOSED SEWER							Q/Qfull	
				R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)		EXCESS CAPACITY (l/s)
Street 5	MH 500	MH 502	A21		0.154	0.28	0.28	10.00	104.19	118.24	600	609.6	0.47	34.0	439.59	1.50	0.38	321.35	0.27
			A22		0.474	0.86	1.13												
Street 5	MH 502	MH 504	A23		0.070	0.13	1.26	10.38	102.24	128.96	675	685.8	0.18	38.2	372.43	1.01	0.63	243.47	0.35
Street 5	MH 504	MH 506	A24		0.185	0.33	1.60	11.01	99.15	189.38	675	685.8	0.22	26.9	411.73	1.11	0.40	222.36	0.46
			A25		0.174	0.31	1.91												
								11.41											
Street 7	MH 700	MH 506	A26		0.239	0.43	0.43	10.00	104.19	45.00	600	609.6	0.25	56.5	320.60	1.10	0.86	275.61	0.14
								10.86											
Street 5	MH 506	MH 508	A27		0.071	0.13	2.47	11.41	97.29	274.06	675	685.8	0.22	55.5	411.73	1.11	0.83	137.67	0.67
			A28		0.192	0.35	2.82												
Street 5	MH 508	MH 608	A29		0.317	0.57	3.39	12.24	93.67	363.08	750	762.0	0.19	61.7	506.76	1.11	0.93	143.67	0.72
			A30		0.269	0.49	3.88												
								13.17											
Half Moon Bay	MH 608	MH 412	A31		0.269	0.49	7.67	13.62	88.29	700.04	975	990.6	0.14	79.3	875.66	1.14	1.16	175.63	0.80
			A33		0.145	0.26	7.93												
								14.79											
Pathway Blk	MH 412	MH 900	A32		0.476	0.86	12.61	14.79	84.25	1062.33	1200	1219.2	0.12	56.5	1410.38	1.21	0.78	348.04	0.75
Pathway Blk	MH 900	MH 902				0.00	12.61	15.57	81.77	1031.02	1200	1219.2	0.11	120.0	1350.33	1.16	1.73	319.31	0.76
								17.30											
Jockvale	MH 802	MH 804	A34		0.071	0.13	0.13	10.00	104.19	110.71	600	609.6	0.21	24.1	293.84	1.01	0.40	183.13	0.38
			A35		0.517	0.93	1.06												
Apartment Block	MH 804	MH 902	A36		0.845	1.53	2.59	10.40	102.13	264.46	600	609.6	0.28	81.4	339.29	1.16	1.17	74.84	0.78
								11.57											
Outlet	MH 902	MH 806				0.00	15.20	17.30	76.80	1167.23	1200	1219.2	0.12	15.3	1410.38	1.21	0.21	243.14	0.83
Outlet	MH 806	MH 808				0.00	15.20	17.51	76.24	1158.71	1200	1219.2	0.28	3.6	2154.39	1.84	0.03	995.68	0.54
Outlet	MH 808	HW1				0.00	15.20	17.54	76.15	1157.41	1500	1524.0	0.10	47.0	2334.38	1.28	0.61	1176.97	0.50
								18.16											

STREET	FROM	TO	AREA ID	FLOW							PROPOSED SEWER							Q/Qfull	
				R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)		EXCESS CAPACITY (l/s)
Street B	MH 118	MH 116	B01		0.135	0.24	0.24	10.00	104.19	25.42	675	685.8	0.20	37.4	392.57	1.06	0.59	367.15	0.06
Street B	MH 116	MH 114	B02		0.824	1.49	1.73	10.59	101.19	217.78	675	685.8	0.20	82.6	392.57	1.06	1.30	174.80	0.55
			B03		0.095	0.17	1.90												
			B04		0.137	0.25	2.15												
Street B	MH 114	MH 112	B05		0.226	0.41	2.56	11.88	95.19	278.15	750	762.0	0.20	120.0	519.92	1.14	1.76	241.77	0.53
			B06		0.200	0.36	2.92												
Street B	MH 112	MH 110	B07		0.204	0.37	3.29	13.64	88.23	372.06	750	762.0	0.20	39.0	519.92	1.14	0.57	147.87	0.72
			B08	1.111		0.93	4.22												
								14.21											
Caivan Lands	CAP	MH 110	B09		0.090	0.16	0.16	10.00	104.19	16.94	450	457.2	0.40	12.0	188.30	1.15	0.17	171.36	0.09
								10.17											
Street B	MH 110	MH108	B10		0.186	0.34	4.72	14.21	86.20	406.50	825	838.2	0.15	28.8	580.56	1.05	0.46	174.07	0.70
								14.67											
Caivan Lands	CAP	MH 108	B11		0.097	0.18	0.18	10.00	104.19	18.26	300	304.8	0.50	12.0	71.41	0.98	0.20	53.14	0.26
								10.20											
Street B	MH 108	MH 106	B12		0.111	0.20	5.09	14.67	84.65	431.00	825	838.2	0.15	28.5	580.56	1.05	0.45	149.56	0.74
								15.12											
Caivan Lands	CAP	MH 106	B13		0.120	0.22	0.22	10.00	104.19	22.59	300	304.8	0.50	12.0	71.41	0.98	0.20	48.81	0.32
								10.20											
Street B	MH 106	MH 104				0.00	5.31	15.12	83.17	441.53	825	838.2	0.14	35.5	560.88	1.02	0.58	119.34	0.79
								15.70											
Caivan Lands	CAP	MH 104	B14		0.136	0.25	0.25	10.00	104.19	25.61	300	304.8	0.50	12.0	71.41	0.98	0.20	45.80	0.36
								10.20											
Street B	MH 104	MH 102	B15		0.026	0.05	5.60	15.70	81.36	455.70	900	914.4	0.10	35.5	597.83	0.91	0.65	142.13	0.76
								16.35											
Caivan Lands	CAP	MH 102	B16		0.121	0.22	0.22	10.00	104.19	22.78	300	304.8	0.50	12.0	71.41	0.98	0.20	48.62	0.32
								10.20											
Street B	MH 102	MH 100	B17		0.342	0.62	6.44	16.35	79.43	511.35	975	990.6	0.10	36.1	740.07	0.96	0.63	228.72	0.69
								16.98											
Caivan Lands	CAP	MH 100	B18		0.107	0.19	0.19	20.00	70.25	1184.00	1200	1219.2	0.15	12.0	1576.85	1.35	0.15	392.84	0.75
			Caivan Lands		9.220	16.66	16.85												
								20.15											
Outlet	MH 100	H2				0.00	23.29	20.15	69.93	1628.74	1500	1524.0	0.14	79.8	2762.07	1.51	0.88	1133.34	0.59

			FLOW								PROPOSED SEWER								
STREET	FROM	TO	AREA ID	R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)	EXCESS CAPACITY (l/s)	Q/Qfull

Definitions

Q = 2.78 AIR

Q = Peak Flow, in Litres per second (L/s)

A = Area in hectares (ha)

I = Rainfall Intensity (mm/h)

Notes:

1) Ottawa Rainfall-Intensity Curve

2) Min Velocity = 0.80 m/sec.

TC calculated based on an average travel time of 1.0m/s x 600m

Includes the remaining 9.22ha portion of the Caivan Lands not included in Area IDs B03,B09-B10,B-12,B14-B15, B17,B19 (2.61ha). The total area of the Caivan Lands is 11.83ha.



SANITARY SEWER DESIGN SHEET
 BURNETT LANDS SUBDIVISION
 DEVELOPER: CLARIDGE HOMES



PROJECT : 113191 111117
 DESIGNED BY: LSC LSC
 CHECKED BY: CJR GJM
 DATE PREPARED: MAR, 2014 Dec. 2015
 DATE REVISED: Nov. 25/16
 DATE REVISED: Jan. 26/18

LOCATION				CUMULATIVE								PROPOSED SEWER													
STREET	AREA	FROM MH	TO MH	CONDOS	TOWNS	FUTURE TOWNS*	SCHOOL AREA (ha)	POPULATION (in 1000's)	AREA (ha.)	POPULATION (in 1000's)	AREA (ha.)	PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	Roughness Coef.	GRADE (%)	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap	
Future Street B	B01	Stub	101			23		0.0621	0.225	0.062	0.225	4.00	1.01	0.06	1.07	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.05	
	101	101	103		2			0.0054	0.261	0.068	0.486	4.00	1.09	0.14	1.23	36.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.04	
Future Street B	B02	Stub	103			30		0.0810	0.298	0.081	0.298	4.00	1.31	0.08	1.40	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.07	
	103	103	105		3			0.0081	0.189	0.157	0.973	4.00	2.54	0.27	2.81	35.5	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.09	
Future Street B	B03	Stub	105			33		0.0891	0.325	0.089	0.325	4.00	1.44	0.09	1.53	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.08	
	105	105	107					0.0000	0.076	0.246	1.374	4.00	3.98	0.38	4.37	35.5	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.14	
Future Street B	B04/B05	Stub	107			54		0.1458	0.527	0.146	0.527	4.00	2.36	0.15	2.51	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.13	
	107	107	109					0.0000	0.110	0.392	2.011	4.00	6.34	0.56	6.91	57.8	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.23	
Future Street B	B06	Stub	109			19		0.0513	0.189	0.051	0.189	4.00	0.83	0.05	0.88	12.0	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.03	
Future** Street B	B07**	Stub	113			83		0.2241	1.933	0.224	1.933	4.00	3.63	0.54	4.22	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.21	
	113	113	111		14			0.0378	0.561	0.262	2.494	4.00	4.24	0.70	4.99	100.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.17	
	111	111	109					0.0000	0.109	0.262	2.603	4.00	4.24	0.73	5.02	58.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.17	
Jockvale	109	109	601		7			0.0189	0.243	0.724	5.046	3.89	11.39	1.41	12.85	39.4	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.43	
	Jockvale	601	SNC					0.0000		0.724	5.046	3.89	11.39	1.41	12.85	3.0	250	251.5	DR 35	0.013	1.00	60.4	1.22	0.21	
Jockvale	601	601	603		7			0.0189	0.261	0.019	0.261	4.00	0.31	0.07	0.38	38.0	250	251.5	DR 35	0.013	0.26	30.8	0.62	0.01	
	Jockvale	603	307					0.0000		0.019	0.261	4.00	0.31	0.07	0.38	5.8	250	251.5	DR 35	0.013	0.38	37.2	0.75	0.01	
Street 2	201	201	203		14			0.0378	0.402	0.038	0.402	4.00	0.61	0.11	0.73	46.9	200	201.2	DR 35	0.013	0.38	20.5	0.65	0.04	
	Street 2	203	205		1			0.0027	0.005	0.041	0.407	4.00	0.66	0.11	0.77	6.9	200	201.2	DR 35	0.013	0.46	22.6	0.71	0.03	
	Street 2	205	301		2			0.0054	0.127	0.046	0.534	4.00	0.74	0.15	0.89	21.5	200	201.2	DR 35	0.013	0.37	20.3	0.64	0.04	
Half Moon Bay	301	301	303					0.0000	0.122	0.046	0.656	4.00	0.74	0.18	0.93	66.3	200	201.2	DR 35	0.013	0.36	20.0	0.63	0.05	
	Half Moon Bay	303	305		1			0.0027	0.088	0.049	0.744	4.00	0.79	0.21	1.00	10.9	200	201.2	DR 35	0.013	0.37	20.3	0.64	0.05	
	Half Moon Bay	305	307		17			0.0459	0.493	0.095	1.237	4.00	1.53	0.35	1.88	73.0	200	201.2	DR 35	0.013	0.37	20.3	0.64	0.09	
Jockvale	307	307	605		4			0.0108	0.259	0.124	1.757	4.00	2.01	0.49	2.50	74.0	250	251.5	DR 35	0.013	0.27	31.4	0.63	0.08	
Street 4	409	409	605		18			0.0486	0.612	0.049	0.612	4.00	0.79	0.17	0.96	66.8	200	201.2	DR 35	0.013	0.40	21.1	0.66	0.05	
Street 4	401	401	403		4			0.0108	0.211	0.011	0.211	4.00	0.18	0.06	0.23	24.4	200	201.2	DR 35	0.013	0.66	27.1	0.85	0.01	
	Street 4	403	405		5			0.0135	0.152	0.024	0.363	4.00	0.39	0.10	0.50	30.7	200	201.2	DR 35	0.013	0.39	20.8	0.65	0.02	
	Street 4	405	407		14			0.0378	0.392	0.062	0.755	4.00	1.01	0.21	1.22	47.8	200	201.2	DR 35	0.013	0.38	20.5	0.65	0.06	
	Street 4	407	605		11			0.0297	0.315	0.092	1.070	4.00	1.49	0.30	1.79	51.4	200	201.2	DR 35	0.013	0.37	20.3	0.64	0.09	
Jockvale	605	605	607					0.0000	0.130	0.265	3.569	4.00	4.29	1.00	5.29	74.0	250	251.5	DR 35	0.013	0.27	31.4	0.63	0.17	
Street 4	411	411	607		8			0.0216	0.377	0.022	0.377	4.00	0.35	0.11	0.46	66.6	200	201.2	DR 35	0.013	0.71	28.1	0.88	0.02	
Street 5	501	501	503		30			0.0810	0.877	0.081	0.877	4.00	1.31	0.25	1.56	32.1	200	201.2	DR 35	0.013	0.72	28.3	0.89	0.06	
	Street 5	503	505		4			0.0108	0.144	0.092	1.021	4.00	1.49	0.29	1.77	37.6	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.09	
	Street 5	505	507		3			0.0081	0.106	0.100	1.127	4.00	1.62	0.32	1.93	26.8	200	201.2	DR 35	0.013	0.37	20.3	0.64	0.10	
ST. JOSEPH SCHOOL***	Offsite***	EX. SA 365	701				10.52					1.50	6.09	2.95	12.08										
Street 7	701	701	507		12			0.0324	0.405	0.032	0.405	4.00	0.53	0.11	12.72	56.7	250	251.5	DR 35	0.013	0.37	36.7	0.74	0.35	
Street 5	507	507	509		8			0.0216	0.250	0.154	1.782	4.00	2.49	0.50	15.07	55.3	200	201.2	DR 35	0.013	0.34	19.4	0.61	0.78	
	Street 5	509	607	60	6			0.1242	0.542	0.278	2.324	4.00	4.51	0.65	17.23	57.1	200	201.2	DR 35	0.013	0.36	20.0	0.63	0.86	
Jockvale	607	607	609					0.0000		0.564	6.270	3.95	9.02	1.76	22.85	4.6	250	251.5	DR 35	0.013	0.44	40.1	0.81	0.57	
Street H	801	801	613	60				0.1080	0.352	0.108	0.352	4.00	1.75	0.10	1.85	75.6	200	201.2	DR 35	0.013	0.50	23.6	0.74	0.08	
Jockvale	615	Stub	613	130				0.2340	0.734	0.234	0.734	4.00	3.79	0.21	4.00	10.0	200	201.2	DR 35	0.013	1.00	33.3	1.05	0.12	

SANITARY SEWER DESIGN SHEET

BURNETT LANDS SUBDIVISION
DEVELOPER: CLARIDGE HOMES



PROJECT : 113191 111117
DESIGNED BY: LSC LSC
CHECKED BY: CJR GJM
DATE PREPARED: MAR, 2014 Dec. 2015
DATE REVISED: Nov. 25/16
DATE REVISED: Jan. 26/18

LOCATION				CUMULATIVE								PROPOSED SEWER												
STREET	AREA	FROM MH	TO MH	CONDOS	TOWNS	FUTURE TOWNS*	SCHOOL AREA (ha)	POPULATION (in 1000's)	AREA (ha.)	POPULATION (in 1000's)	AREA (ha.)	PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	Roughness Coef.	GRADE (%)	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap
Jockvale	613	613	611					0.0000	0.273	0.342	1.359	4.00	5.54	0.38	5.92	74.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.20
Jockvale	611	611	609	75				0.1350	0.530	0.477	1.889	3.98	7.70	0.53	8.23	71.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.27
Jockvale/Street 4/5		609	SNC					0.0000		1.041	8.159	3.79	15.98	2.28	30.34	2.8	250	251.5	DR 35	0.013	1.00	60.4	1.22	0.50

Notes:
1. $Q(d) = Q(p) + Q(i)$, where

1 Q(d) = Design Flow (L/sec)
Q(p) = Population Flow (L/sec)
Q(i) = Extraneous Flow (L/sec)

2* Future townhouse population based on zoning density 100 units/ha.

3** Offsite parkland and half of fronting roadway excluded (1.111ha) from residential population calculations. Parkland contribution accounted for as per Appendix 4-A.4.

4*** Flows taken from South Nepean Collector: Phase 2, Hydraulics Review/Assessment, Technical Memo, completed by Novatech, dated August 20, 2015.

Residential

$Q(p) = (P \times q \times M / 86,400)$, where
P = Population (2.7 persons per town/semi, 2.1 person per multi-unit zen, 1.8 persons per apartment)
q = Average per capita flow = 350 L/cap/day
M = Harmon Formula (maximum of 4.0)

Institutional

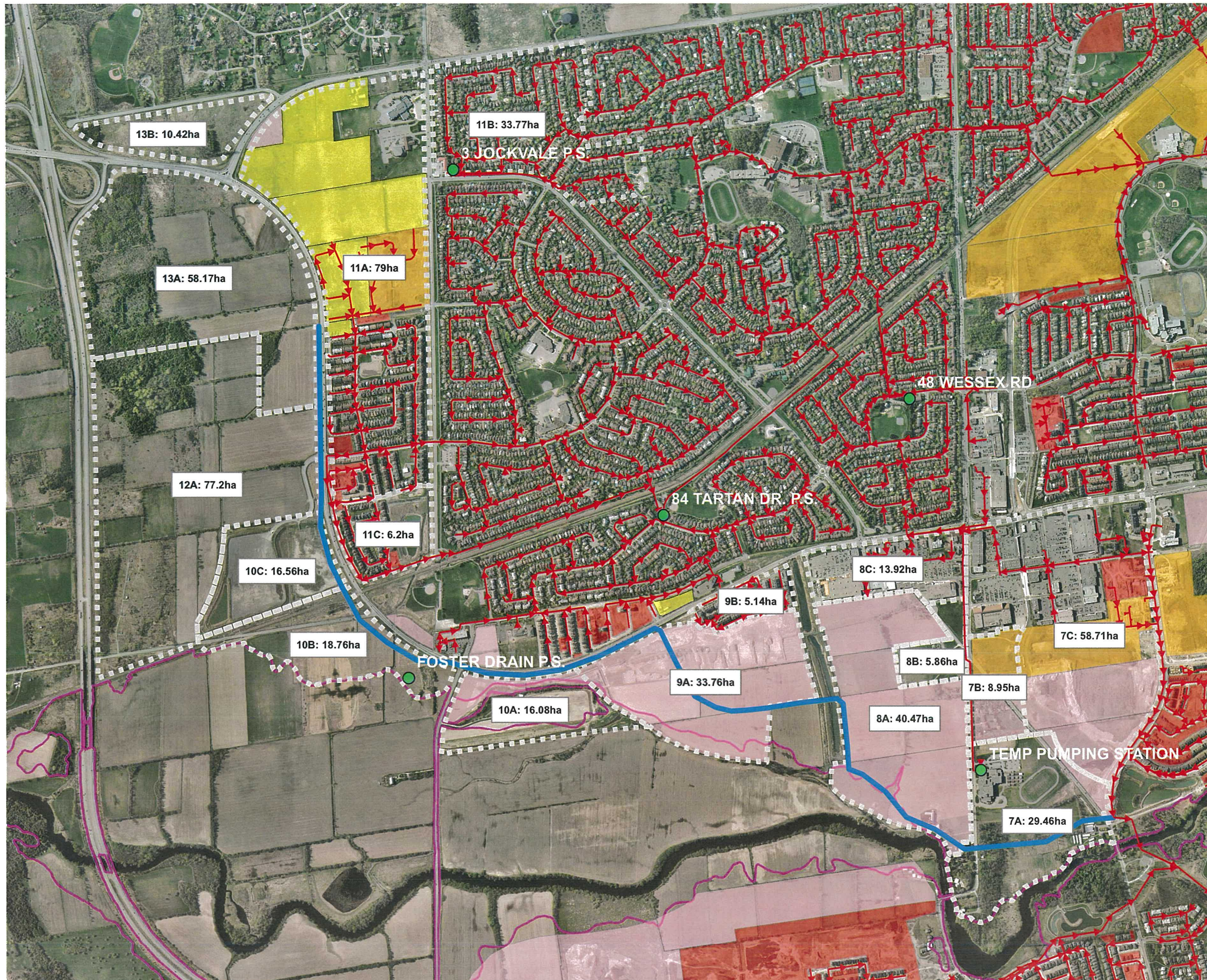
$Q(p) = (P \times q \times M / 86,400)$, where
P = Population Demand = 50,000L/gross ha/day
M = 1.5

Parkland (as per Appendix 4-A.4)

$Q(p) = (P \times q \times M / 86,400)$, where
P = Population = 75 persons/acre/day
q = 20L/person/day
M = 1.5

Appendix B
Sanitary Report Excerpts

Figure 01
Existing Sanitary Network and Collection Areas



- Pump Station
 - Existing Sanitary Main (With Flow Direction)
 - Proposed Alignment for South Nepean Collector
 - Collection Area
- DEVELOPMENT STATUS**
- Registered
 - Draft Approved
 - Pending
 - No Plan
 - Floodplain

NOT TO SCALE



MAP DRAWING INFORMATION:
DATA PROVIDED BY THE CITY OF OTTAWA

MAP CREATED BY: BC
MAP CHECKED BY: MBM
MAP PROJECTION: NO PROJECTION

FILE LOCATION: \\dillon.ca\dillon_dfs\Ottawa\Ottawa CA\CAD\2011\115681\Design_GIS\MXD\Figure01c_ExistingSanitaryNetwork.mxd

Table 5.1: Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area

Collection Area	Discharging Node	Estimated from GIS			City of Ottawa VURL Data			Other Space ¹ (ha)	Population (PE)	Residential Density (PE/net ha)	Comments	Additional Source(s)
		Gross Institutional/Commercial (ha)	Gross Residential (ha)	Gross Area (ha)	Net Residential (ha)	Units (#)	Unit Density (#/ha)					
7A	70	13.5	7.4	29.5	4.0	605	0.3	9.1	1637	4.25	Flow calculations include St Joseph H.S. Pump Station firm capacity of 7.0 L/s Additional 600 units (TAC)	3.4ppu (TAC)
7B		0.0	9.24	9.24	6.23	1474	136.7	3.0	3321	638.8	Population from split VURL allocated by area. VURL parcel id 323 - inconsistency between net and gross reported area.	2.7ppu (TAC)
8A		0.0	40.0	40.0	24.1	4462	185.1	15.9	12047.4	499.9		2.7ppu (TAC)
8B		5.9	0.0	5.9	0.0	0	0	0.0			Future Commercial area	
8C		13.9	0.0	13.9	0.0	0	0	0.0			Commercial area includes Home Depot	
9A	80	0.0	33.8	33.8	18.6	635	34.1	15.2	2210	116.2		3.4ppu (TAC)
10A	90	0.0	16.1	16.1	9.7	451	28.0	6.4	1533.4	158.0	Assume net population = 60% gross.	3.4ppu (TAC)
10B	100	18.8	0.0	35.3	0.0	0	0	16.5			Allocated as potential future I/C use as directed by TAC	
10C	110	16.6	0.0	35.3	0.0	0	0	18.7			Area includes current Municipal Snow Dump. Flow allowance is made for potential future I/C use	
11C		0.0	6.2	6.2	Note 2			2.5	306	82.7	This area is south of '11 block' in the existing development	From IBI Apr 2010 Report Figure 1
11A		12.5	66.5	79.0				26.6	3923	98.3	Institutional includes 4.38ha church site and 6.89 ha institution at northeast corner, as well and Claridge Commercial (0.56ha) and DCR/Phoenix Commercial (0.64ha)	From IBI Apr2010 Report Figure 1
11B	0.0	37.0	37.0	14.8				1550	69.8	Presently serviced by Jockvale pump station; to be redirected to SNC.	Estimated from 2011 Census Block data	
12A	120	77.2	0.0	77.2	0.0			Allow sanitary peak flow 79.0 L/s	Novatech, Employment Lands Report, Revised Jan 2012			
13A	130	58.5	0.0	58.5	Note 2			0.0			Allow sanitary peak flow 62.8 L/s plus Collection Area 13B, total 82.2 L/s	
13B		12.5	0.0	12.5	0.0			Allow sanitary peak flow 19.4 L/s; gravity discharge to Collection Area 13A	IBI/Novatech			

Notes:

1. Other space includes other residential space accounting for the difference between gross area (measured with GIS) and net area (provided in VURL data), such as sidewalks, roads, greenspace, etc.
2. Collection Area 11A and 11B population and land use as identified under Additional Source(s). Other space reported as 60% of gross residential area, consistent with VURL average.

Table 5.1: Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area

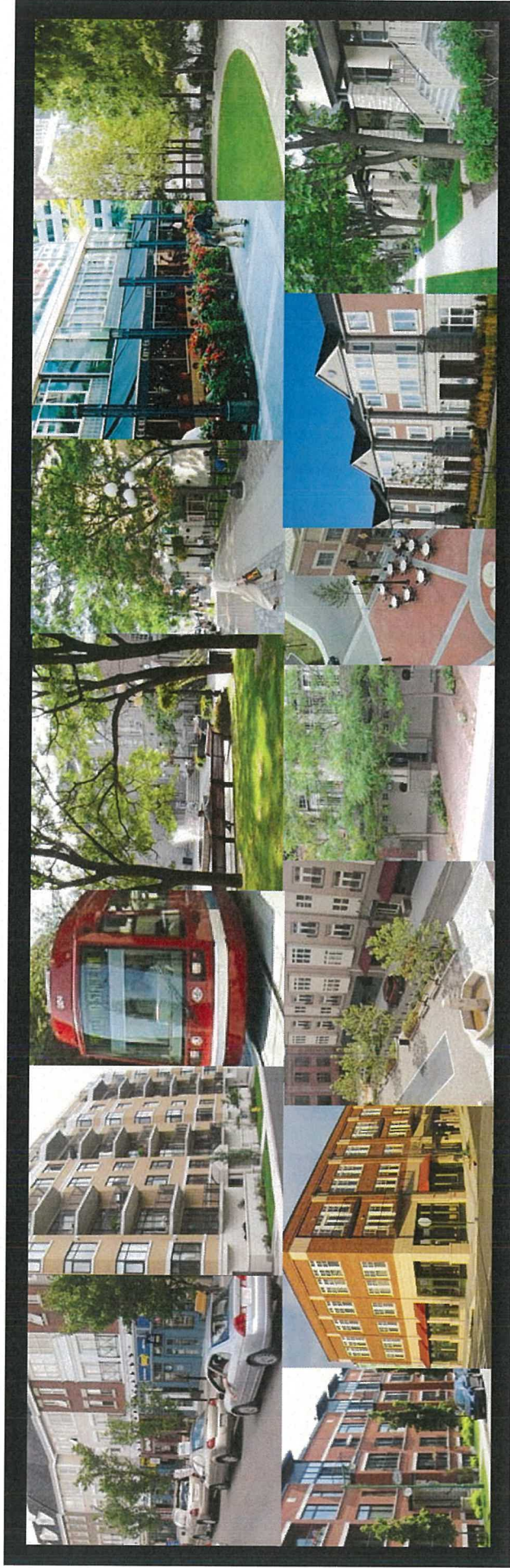
Collection Area	Discharging Node	Estimated from GIS			City of Ottawa VURL Data			Other Space ¹ (ha)	Population (PE)	Residential Density (PE/net ha)	Comments	Additional Source(s)
		Gross Institutional/Commercial (ha)	Gross Residential (ha)	Gross Area (ha)	Net Residential (ha)	Units (#)	Unit Density (#/ha)					
7A	70	13.5	7.4	29.5	4.0	605	0.3	9.1	1637	4.25	Flow calculations include St Joseph H.S. Pump Station firm capacity of 7.0 L/s Additional 600 units (TAC)	3.4ppu (TAC)
7B		0.0	9.24	9.24	6.23	1474	136.7	3.0	3321	638.8	Population from split VURL allocated by area. VURL parcel id 323 - inconsistency between net and gross reported area.	2.7ppu (TAC)
8A		0.0	40.0	40.0	24.1	4462	185.1	15.9	12047.4	499.9		2.7ppu (TAC)
8B		5.9	0.0	5.9	0.0	0	0	0.0			Future Commercial area	
8C		13.9	0.0	13.9	0.0	0	0	0.0			Commercial area includes Home Depot	
9A	80	0.0	33.8	33.8	18.6	635	34.1	15.2	2210	116.2		3.4ppu (TAC)
10A	90	0.0	16.1	16.1	9.7	451	28.0	6.4	1533.4	158.0	Assume net population = 60% gross.	3.4ppu (TAC)
10B	100	18.8	0.0	35.3	0.0	0	0	16.5			Allocated as potential future I/C use as directed by TAC	
10C	110	16.6	0.0	35.3	0.0	0	0	18.7			Area includes current Municipal Snow Dump. Flow allowance is made for potential future I/C use	
11C		0.0	6.2	6.2	Note 2			2.5	306	82.7	This area is south of '11 block' in the existing development	From IBI Apr 2010 Report Figure 1
11A	12.5	66.5	79.0	26.6				3923	98.3	Institutional includes 4.38ha church site and 6.89 ha institution at northeast corner, as well and Claridge Commercial (0.56ha) and DCR/Phoenix Commercial (0.64ha)	From IBI Apr2010 Report Figure 1	
11B	0.0	37.0	37.0	14.8				1550	69.8	Presently serviced by Jockvale pump station; to be redirected to SNC.	Estimated from 2011 Census Block data	
12A	77.2	0.0	77.2	0.0						Allow sanitary peak flow 79.0 L/s	Novatech, Employment Lands Report, Revised Jan 2012	
13A	58.5	0.0	58.5	0.0						Allow sanitary peak flow 62.8 L/s plus Collection Area 13B, total 82.2 L/s		
13B	130	12.5	0.0	12.5	0.0		Allow sanitary peak flow 19.4 L/s; gravity discharge to Collection Area 13A	IBI/Novatech				

Notes:

1. Other space includes other residential space accounting for the difference between gross area (measured with GIS) and net area (provided in VURL data), such as sidewalks, roads, greenspace, etc.
2. Collection Area 11A and 11B population and land use as identified under Additional Source(s). Other space reported as 60% of gross residential area, consistent with VURL average.



South Nepean Town Centre Community Design Plan



Planning and Growth Management Department
Community Planning and Design Division
July 2006
Publication #03-14

OTTAWA CITY COUNCIL
28 AND 29 JUNE 2006
ANDREW S. HAYDON HALL
9:00 a.m.

MINUTES 61

PLANNING AND ENVIRONMENT COMMITTEE REPORT 50

**13. SOUTH NEPEAN TOWN CENTRE COMMUNITY
DESIGN PLAN
AND OFFICIAL PLAN AMENDMENT**

COMMITTEE RECOMMENDATIONS

That Council:

1. Approve the South Nepean Town Centre Community Design Plan in Document 8, which has been submitted under separate cover.
2. Adopt Official Plan Amendment No. XX to the City of Ottawa Official Plan (2003), as detailed in Document 5, to implement the Community Design Plan.

CARRIED

BY-LAW NO. 2006 - 260

A by-law of the City of Ottawa to amend the Official Plan for the City of Ottawa to change the designation of the lands which are the subject of the South Nepean Town Centre Community Design Plan.

WHEREAS Planning and Environment Committee convened a public meeting to consider the adoption of the aforementioned official plan amendment;

AND WHEREAS Planning and Environment Committee recommends the adoption of the aforementioned official plan amendment;

AND WHEREAS Council on June 28, 2006 carried the recommendation of Planning and Environment Committee;

THEREFORE the Council of the City of Ottawa enacts as follows:

1. Attachment A, being Official Plan Amendment No. 44 to the Official Plan for the City of Ottawa is hereby adopted.
2. This by-law shall come into force in accordance with the provisions of the Planning Act, R.S.O. 1990, c.P.13, as amended.

ENACTED AND PASSED this 29th day of June, 2006.

CITY CLERK

MAYOR

Land Use

4.4 Policy Area – High Rise Residential

The High Rise Residential policy area identifies sites within the Town Centre that will accommodate the highest density residential uses, located in proximity to the transit “hub”. Apartment buildings will be the only residential type permitted.

Policies

For the High Rise Residential policy area:

- (1) Apartments are the only permitted use. As part of an apartment building, retail, office and commercial uses at grade are also permitted.
- (2) The minimum building height is 6 storeys and the maximum building height is 12 storeys.
- (3) The net density target for residential uses is 300 units per hectare.
- (4) At least 90% of required parking for each development must be provided in parking structures, either above-grade or below-grade.

4.5 Policy Area – Mid Rise Residential

The Mid Rise Residential policy area is intended to accommodate the majority of the Town Centre’s ground-oriented multiple unit dwellings. This policy area will provide an appropriate transition between the low density neighbourhoods surrounding the Town Centre to the higher intensity uses within it.

Policies

For the Mid Rise Residential policy area:

- (1) Apartments, street, block and stacked townhouses, public and institutional uses, schools, places of worship and community facilities are permitted. Ground floor retail uses within a mixed-use building with residential uses above are permitted along Greenbank Road.

- (2) The minimum building height is 2 storeys and the maximum building height is 4 storeys.

- (3) The net density target for residential uses is 100 units per hectare.

4.6 Policy Area – Neighbourhood Park

The Neighbourhood Park policy area will include both public parks and public plazas. The five public parks within the Town Centre will be designed to support the area’s urban nature, as accessible amenities with open frontages and clearly defined entrances that work within the grid pattern of streets and blocks. The two public plazas within the Town Centre will be designed as predominately hard surfaced areas fronted by buildings in order to create a built form edge and generate pedestrian activity.

Policies

For the Neighbourhood Park policy area:

- (1) Public parks, public plazas, community facilities, and conservation uses are permitted.
- (2) The locations of the five public parks and the two public plazas are identified on Schedule 5 of this CDP.
- (3) Acquisition of all neighbourhood parks will be as per Section 8.4 and Schedule 5 of this CDP.
- (4) Public parks must:
 - (a) Generally be between 0.4 and 1.0 hectares of level land;
 - (b) Have at least three sides that are entirely open to the street;
 - (c) Not have rear or side yards adjacent on their fourth side;
 - (d) Be designed with an emphasis on hard surfacing and seating areas as compared to traditional suburban neighbourhood parks; and
 - (e) Not contain sports fields.
- (5) Public plazas must:
 - (a) Generally be less than 0.4 hectares of level land;



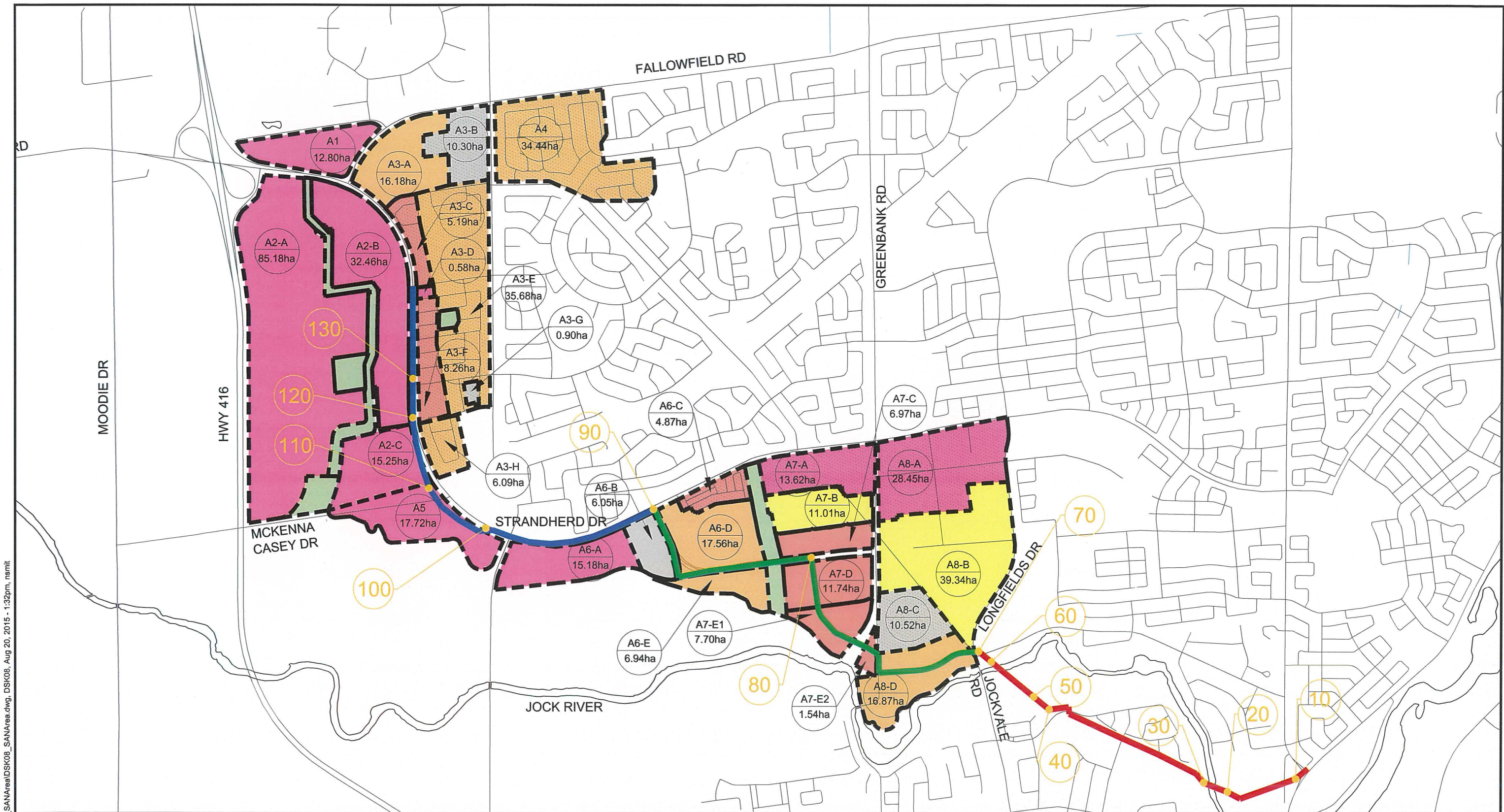


Note:

The colour of different parcels illustrates the boundaries of different parcels, or groups of parcels, and does not indicate land ownership.









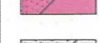

Table 5: Updated Allocation of Commercial, Institutional and Residential Demands to Phases 2 & 3 (Nodes 70 – 130) of the SNC by Collection Area (Novatech, 2015)

Collection Area	Upstream Node	Existing / Proposed Development	Existing / Proposed Land Use	Area (ha)	Estimated Number of Residential Units	Population Density (persons / ha)	Comment	Reference
A1	130	Proposed	Commercial	12.80	-	-	O'Keefe Court – Conceptual site plan shows proposed commercial.	Conceptual Plans for O'Keefe Court
A2-A	130	Proposed	Commercial	85.18	-	-	CitiGate – Analysis uses same approach as the design for CitiGate.	Detailed Servicing and SWM Report (Phase 1) (Novatech, 2014)
A2-B	130	Proposed	Commercial	32.46	-	-		
A2-C	120	Proposed	Commercial (ex. Snow dump)	15.25	-	-	Existing snow dump facility assumed to be future commercial.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)
A3-A	130	Proposed	Low Density Residential	16.48	461	95.2	Havencrest – Existing single family units.	Havencrest Design Report (IBI, 2013)
A3-B	130	Existing	Institutional	10.30	-	-	Cedarview Middle School and Cedarview Alliance Church.	Aerial Photos / Site Visits
A3-C	130	Existing	Medium Density Residential	5.19	311	162	Existing townhouse units.	
A3-D	130	Existing	Commercial	0.58	-	-	Existing commercial buildings.	
A3-E	130	Existing	Low Density Residential	35.68	999	95.2	Existing single family units.	
A3-F	130	Existing	Medium Density Residential	8.26	496	162.0	Existing townhouse units.	
A3-G	130	Existing	Institutional	0.90	-	-	Ottawa Torah Centre Chibad.	
A3-H	120	Existing	Low Density Residential	6.09	171	95.2	Existing single family units.	
A4	130	Existing	Low Density Residential	34.44	964	95.2	Existing single family units currently serviced by Jockvale pump station; to be redirected to SNC.	
A5	110	Proposed	Commercial	17.72	-	-	Proposed commercial south of McKenna Casey Drive.	Site Visits
A6-A	100	Proposed	Institutional	20.70	-	-	Proposed school site on Minto property.	Conceptual Plan for Lands Adjacent the Kennedy-Burnett SWMF provided by Minto (2015)
A6-B	90	Existing	Medium Density Residential	4.87	292	162.0	Existing townhouse units.	Aerial Photos / Site Visits
A6-C	90	Proposed	Low Density Residential	10.11	283	95.2	Proposed single family units on lands owned by Minto.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A6-D	90	Proposed	Low Density Residential	5.59	157	95.2	Proposed single family units on lands owned by Mion.	
A6-E	90	Proposed	Low Density Residential	7.24	203	95.2	Proposed single family units on lands owned by Pavic / Braovac.	
A7-A	80	Existing	Commercial	13.62	-	-	Existing large retail stores (commercial).	Aerial Photos
A7-B	80	Proposed	High Density Residential	11.01	826	135.0	Proposed high density units on lands owned by Richcraft / Trinity.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A7-C	80	Proposed	Medium Density Residential	6.97	418	162.0	Proposed Medium density units on lands owned by Mion.	
A7-D	80	Proposed	Medium Density Residential	11.74	704	162.0	Proposed Medium density units on lands owned by Caivan.	
A7-E1/E2	80	Proposed	Medium Density Residential	9.24	554	162.0	Proposed Medium density units on lands owned by Claridge.	
A8-A	80	Existing	Commercial	28.45	-	-	Existing Barrhaven Market Place (commercial).	Aerial Photos / Site Visits
A8-B	80	Proposed	High Density Residential	39.34	2951	135.0	Future development similar to Ampersands development.	Site Visits
A8-C	80	Existing	Institutional	10.52	-	-	Existing St. Joseph High School.	Aerial Photos / Site Visits
A8-D	80	Proposed	Low Density Residential	16.87	1012	162.0	Proposed 600 low density residential units.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)



M:\2015\115075\CAD\Design\Figures\DSK\DSK08_SANAREA.dwg, DSK08, Aug 20, 2015 - 1:32pm, nsm1

LEGEND

- | | | | |
|-------------------------------------------------------------------------------------|------------------------------------------------|---------------------------------------------------------------------------------------|--------------------------------------------|
|  | EXISTING / PROPOSED HIGH DENSITY RESIDENTIAL |  | OTHER LANDS (OPEN SPACE, PARKS, AND SWMFS) |
|  | EXISTING / PROPOSED MEDIUM DENSITY RESIDENTIAL |  | SOUTH NEPEAN COLLECTOR PHASE 1 |
|  | EXISTING / PROPOSED LOW DENSITY RESIDENTIAL |  | SOUTH NEPEAN COLLECTOR PHASE 2 |
|  | EXISTING / PROPOSED COMMERCIAL |  | SOUTH NEPEAN COLLECTOR PHASE 3 |
|  | EXISTING / PROPOSED INSTITUTIONAL |  | SOUTH NEPEAN COLLECTOR NODE ID |



NOVATECH
 Engineers, Planners & Landscape Architects
 Suite 200, 240 Michael Cowpland Drive
 Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

SOUTH NEPEAN COLLECTOR SEWER

SANITARY DRAINAGE AREAS AND LAND USE

SCALE 1:20 000

DATE AUG 2015 JOB 115075 FIGURE FIG. 1

PROJECT #: 115075
 DESIGNED BY: CMS
 CHECKED BY: MJP
 DATE: August 20, 2015

SANITARY SEWER DESIGN SHEET
South Nepean Collector - Phase 2 & 3

Theoretical Current Operational Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (10,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.05 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (300 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-B	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-B	Institutional	130		10.30		10.30					0.0	1.2	0.5	0.0	1.2	0.5	0.0	1.7
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	841	2.71	0.0	0.0	0.3	0.0	1.2	0.8	7.9	9.9
A3-D	Commercial	130	0.58			0.58		841	841	2.71	0.1	0.0	0.0	0.1	1.2	0.8	7.9	10.0
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	4238	2.39	0.0	0.0	1.8	0.1	1.2	2.6	35.1	39.0
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	5576	2.32	0.0	0.0	0.4	0.1	1.2	3.0	44.9	49.2
A3-G	Institutional	130		0.90		0.90			5576	2.32	0.0	0.1	0.0	0.1	1.3	3.0	44.9	49.4
A4	Low Density Residential*	130				0.00			5576	2.32	0.0	0.0	0.0	0.1	1.3	3.0	44.9	49.4
A2-C	Snow Dump Facility	120				0.00			5576	2.32	0.0	0.0	0.0	0.1	1.3	3.0	44.9	49.4
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	6155	2.30	0.0	0.0	0.3	0.1	1.3	3.4	49.1	53.8
A5	Open Space	110				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-A	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-B	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	6944	2.27	0.0	0.0	0.2	0.1	1.3	3.6	54.6	59.6
A6-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	1.3	3.6	54.6	59.6
A6-E	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	1.3	3.6	54.6	59.6
A7-A	Commercial	90	13.62			13.62			6944	2.27	2.7	0.0	0.7	2.8	1.3	4.3	54.6	63.0
A7-B	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-C	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-E1/E2	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A8-A	Commercial	80	28.45			28.45			6944	2.27	5.6	0.0	1.4	8.4	1.3	5.7	54.6	70.0
A8-B	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	1.3	5.7	54.6	70.0
A8-C	Institutional	80		10.52		10.52			6944	2.27	0.0	1.2	0.5	8.4	2.5	6.2	54.6	71.8
A8-D	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	2.5	6.2	54.6	71.8
ROW Along SNC Sewer Alignment	-	80				14.34			6944	2.27	0.0	0.0	0.7	8.4	2.5	6.9	54.6	72.5
TOTAL		80	42.65	21.72	60.09	138.80	-	6944	6944	2.27	8.4	2.5	6.9	8.4	2.5	6.9	54.6	72.5

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 - 28 (28 used)	2.7 - 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 - 60 (60 used)	2.7	162.0
High Density (apartments)	60 - 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 0.6
- Institutional / Commercial Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station; currently not directed to SNC

PROJECT #: 115075
 DESIGNED BY: CMS
 CHECKED BY: MJP
 DATE: August 20, 2015

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Future Full Service Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing / Proposed Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.28 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (350 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Commercial	130	12.80			12.80					11.1	0.0	3.6	11.1	0.0	3.6	0.0	14.7
A2-A	Commercial	130	85.18			85.18					73.9	0.0	23.9	85.1	0.0	27.4	0.0	112.5
A2-B	Commercial	130	32.46			32.46					28.2	0.0	9.1	113.2	0.0	36.5	0.0	149.8
A3-A	Low Density Residential	130			16.18	16.18	95.2	1540	1540	3.67	0.0	0.0	4.5	113.2	0.0	41.1	22.9	177.2
A3-B	Institutional	130		10.30		10.30				3.67	0.0	8.9	2.9	113.2	8.9	43.9	22.9	189.0
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	2381	3.53	0.0	0.0	1.5	113.2	8.9	45.4	34.0	201.6
A3-D	Commercial	130	0.58			0.58			2381	3.53	0.5	0.0	0.2	113.7	8.9	45.6	34.0	202.2
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	5778	3.19	0.0	0.0	10.0	113.7	8.9	55.5	74.6	252.8
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	7116	3.10	0.0	0.0	2.3	113.7	8.9	57.9	89.4	269.9
A3-G	Institutional	130		0.90		0.90			7116	3.10	0.0	0.8	0.0	113.7	9.7	58.1	89.4	270.9
A4	Low Density Residential	130			34.44	34.44	95.2	3279	10395	2.94	0.0	0.0	9.6	113.7	9.7	67.8	123.7	314.9
A2-C	Commercial (ex. snow dump)	120	15.25			15.25			10395	2.94	13.2	0.0	4.3	127.0	9.7	72.0	123.7	332.4
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	10974	2.91	0.0	0.0	1.7	127.0	9.7	73.7	129.6	340.0
A5	Commercial	110	17.72			17.72			10974	2.91	15.4	0.0	5.0	142.4	9.7	78.7	129.6	360.3
A6-A	Commercial	100	15.18			15.18			10974	2.91	13.2	0.0	4.3	155.5	9.7	82.9	129.6	377.8
A6-B	Institutional	100		6.05		6.05			10974	2.91	0.0	5.3	1.7	155.5	15.0	84.6	129.6	384.7
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	11763	2.88	0.0	0.0	1.4	155.5	15.0	86.0	137.4	393.9
A6-D	Low Density Residential	90			17.56	17.56	95.2	1672	13435	2.83	0.0	0.0	4.9	155.5	15.0	90.9	153.8	415.2
A6-E	Low Density Residential	90			6.94	6.94	95.2	661	14096	2.81	0.0	0.0	1.9	155.5	15.0	92.9	160.2	423.6
A7-A	Commercial	90	13.62			13.62			14096	2.81	11.8	0.0	3.8	167.4	15.0	96.7	160.2	439.2
A7-B	High Density Residential	90			11.01	11.01	135.0	1486	15582	2.76	0.0	0.0	3.1	167.4	15.0	99.8	174.3	456.4
A7-C	Medium Density Residential	90			6.97	6.97	162.0	1129	16711	2.73	0.0	0.0	2.0	167.4	15.0	101.7	184.9	468.9
A7-D	Medium Density Residential	90			11.74	11.74	162.0	1902	18613	2.68	0.0	0.0	3.3	167.4	15.0	105.0	202.4	489.7
A7-E1/E2	Medium Density Residential	90			9.24	9.24	162.0	1497	20110	2.65	0.0	0.0	2.6	167.4	15.0	107.6	215.9	505.8
A8-A	Commercial	80	28.45			28.45			20110	2.65	24.7	0.0	8.0	192.0	15.0	115.5	215.9	538.5
A8-B	High Density Residential	80			39.34	39.34	135.0	5311	25421	2.55	0.0	0.0	11.0	192.0	15.0	126.6	262.4	596.0
A8-C	Institutional	80		10.52		10.52			25421	2.55	0.0	9.1	2.9	192.0	24.1	129.5	262.4	608.1
A8-D	Low Density Residential	80			16.87	16.87	120.9	2040	27461	2.52	0.0	0.0	4.7	192.0	24.1	134.2	279.8	630.2
ROW Along SNC Sewer Alignment	-	80				14.34												
TOTAL		80	221.24	27.77	230.38	493.73	-	27461	27461	2.52	192.0	24.1	134.2	192.0	24.1	138.2	279.8	634.2

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 - 28 (28 used)	2.7 - 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 - 60 (60 used)	2.7	162.0
High Density (apartments)	60 - 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 1.0
- Institutional / Commercial Peaking Factor = 1.5

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

Appendix C

Watermain Boundary Conditions, FUS Calculations, and
Modelling Results

Boundary Conditions 3370 Greenbank Road (Burnett Lands)

Information Provided

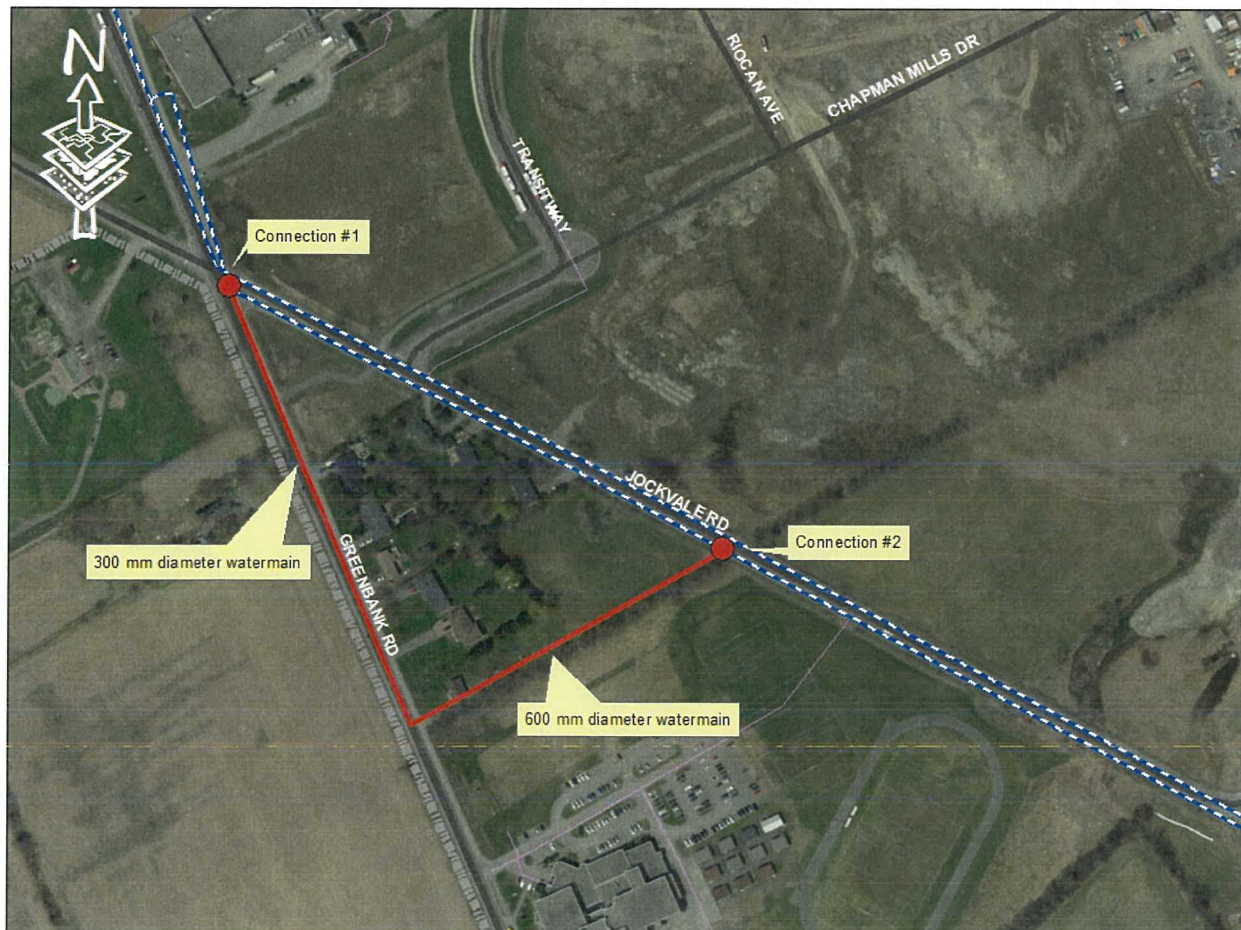
Date provided: 27 December 2017

Scenario	Demand	
	L/min	L/s
Average Daily Demand	348.6	5.81
Maximum Daily Demand	871.2	14.52
Peak Hour	1917	32.0
Fire Flow Demand # 1	12000	200.0
Fire Flow Demand # 2	15000	250.0
Fire Flow Demand # 3	18000	300.0

of connections

2

Connection Location Scenario 1



Connection Location Scenario 2



Results

SCENARIO 1

PRE-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.2	97.2
Peak Hour	141.2	62.6
Max Day plus Fire (12,000 l/min)	138.2	58.4
Max Day plus Fire (15,000 l/min)	133.7	52.0
Max Day plus Fire (18,000 l/min)	129.3	45.6

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Private Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	153.0	95.0
Peak Hour	140.9	60.2
Max Day plus Fire (12,000 l/min)	137.1	54.8
Max Day plus Fire (15,000 l/min)	132.2	47.8
Max Day plus Fire (18,000 l/min)	127.3	40.8

¹ Ground Elevation = 108.2 m

SCENARIO 2

PRE-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.2	97.2
Peak Hour	141.2	62.6
Max Day plus Fire (12,000 l/min)	138.2	58.4
Max Day plus Fire (15,000 l/min)	133.7	52.0
Max Day plus Fire (18,000 l/min)	129.2	45.6

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Bren-Maur Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.2	104.1
Peak Hour	140.5	69.0
Max Day plus Fire (12,000 l/min)	136.8	63.7
Max Day plus Fire (15,000 l/min)	131.8	56.7
Max Day plus Fire (18,000 l/min)	126.9	49.7

¹ Ground Elevation = 108.2 m

SCENARIO 1

POST-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	72.1
Peak Hour	146.3	70.0
Max Day plus Fire (12,000 l/min)	146.4	70.2
Max Day plus Fire (15,000 l/min)	146.2	69.9
Max Day plus Fire (18,000 l/min)	145.9	69.5

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Private Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	70.0
Peak Hour	146.0	67.5
Max Day plus Fire (12,000 l/min)	145.2	66.5
Max Day plus Fire (15,000 l/min)	144.6	65.5
Max Day plus Fire (18,000 l/min)	143.8	64.5

¹ Ground Elevation = 108.2 m

SCENARIO 2

POST-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	72.1
Peak Hour	146.3	70.0
Max Day plus Fire (12,000 l/min)	146.4	70.2
Max Day plus Fire (15,000 l/min)	146.3	69.9
Max Day plus Fire (18,000 l/min)	145.9	69.5

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Bren-Maur Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	79.2
Peak Hour	145.7	76.2
Max Day plus Fire (12,000 l/min)	145.4	75.9
Max Day plus Fire (15,000 l/min)	144.9	75.1
Max Day plus Fire (18,000 l/min)	144.3	74.2

¹ Ground Elevation = 108.2 m

Notes:

- 1) As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a) If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.

- b) Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 111117

Project Name: Kennedy Burnett

Date: 19/07/2017

Input By: Steve Zorgel

Reviewed By: Drew Blair

Legend

Input by User

No Information or Input Required

Building Description: 3 Unit Townhouse (can't be capped as per Technical Bulletin ISDTB-2014-02)

Wood frame

Step			Input	Multiplier Options	Value Used	Total Fire Flow (L/min)
Base Fire Flow						
1	Construction Material					
	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
		Fire resistive construction (> 3 hrs)		0.6		
Floor Area						
2	A	Building Footprint (m ²)	270			
		Number of Floors/Storeys	2			
		Area of structure considered (m ²)			540	
	F	Base fire flow without reductions				8,000
		$F = 220 C (A)^{0.5}$				
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge					
	(1)	Non-combustible		-25%	-15%	6,800
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
		Rapid burning		25%		
Sprinkler Reduction						
4	(2)	Adequately Designed System (NFPA 13)		-30%	0	
		Standard Water Supply		-10%		
		Fully Supervised System		-10%		
			Cumulative Total		0%	
5	Exposure Surcharge (cumulative %)					
	(3)	North Side	3.1 - 10 m		20%	4,080
		East Side	20.1 - 30 m		10%	
		South Side	20.1 - 30 m		10%	
		West Side	3.1 - 10 m		20%	
			Cumulative Total		60%	
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	11,000
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	183
				or	USGPM	2,906
7	Storage Volume	Required Duration of Fire Flow (hours)			Hours	2
		Required Volume of Fire Flow (m ³)			m ³	1320

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 111117

Project Name: Kennedy Burnett

Date: 19/07/2017

Input By: Steve Zorgel

Reviewed By: Drew Blair

Legend

Input by User

No Information or Input Required

Building Description: 4 Unit Townhouse (can't be capped as per Technical Bulletin ISDTB-2014-02)

Wood frame

Step			Input	Multiplier Options	Value Used	Total Fire Flow (L/min)	
Base Fire Flow							
1	Construction Material						
	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5		
		Ordinary construction		1			
		Non-combustible construction		0.8			
		Fire resistive construction (< 3 hrs)		0.7			
Fire resistive construction (> 3 hrs)			0.6				
2	Floor Area						
	A	Building Footprint (m ²)	380		760		
		Number of Floors/Storeys	2				
		Area of structure considered (m ²)					
	F	Base fire flow without reductions				9,000	
$F = 220 C (A)^{0.5}$							
Reductions or Surcharges							
3	Occupancy hazard reduction or surcharge						
	(1)	Non-combustible		-25%	-15%	7,650	
		Limited combustible	Yes	-15%			
		Combustible		0%			
		Free burning		15%			
Rapid burning			25%				
4	Sprinkler Reduction						
	(2)	Adequately Designed System (NFPA 13)		-30%	0		
		Standard Water Supply		-10%			
		Fully Supervised System		-10%			
Cumulative Total				0%			
5	Exposure Surcharge (cumulative %)						
	(3)	North Side	30.1 - 45 m		5%	4,208	
		East Side	20.1 - 30 m		10%		
		South Side	3.1 - 10 m		20%		
		West Side	3.1 - 10 m		20%		
Cumulative Total				55%			
Results							
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	12,000	
		(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	200
					or	USGPM	3,170
7	Storage Volume		Required Duration of Fire Flow (hours)		Hours	2.5	
			Required Volume of Fire Flow (m ³)		m ³	1800	

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 111117

Project Name: Kennedy Burnett

Date: 19/07/2017

Input By: Steve Zorgel

Reviewed By: Drew Blair

Legend

Input by User

No Information or Input Required

Building Description: Condo Unit - 4 Storey Blk 25

Non-combustible construction

Step			Input	Multiplier Options	Value Used	Total Fire Flow (L/min)
Base Fire Flow						
1	Construction Material					
	Coefficient related to type of construction C	Wood frame		1.5	0.8	
		Ordinary construction		1		
		Non-combustible construction	Yes	0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	Floor Area					
	A	Building Footprint (m ²)	1800		7,200	
		Number of Floors/Storeys	4			
		Area of structure considered (m ²)				
	F	Base fire flow without reductions				15,000
$F = 220 C (A)^{0.5}$						
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge					
	(1)	Non-combustible		-25%	-15%	12,750
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	Sprinkler Reduction					
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%	-30%	-6,375
		Standard Water Supply	Yes	-10%	-10%	
		Fully Supervised System	Yes	-10%	-10%	
Cumulative Total				-50%		
5	Exposure Surcharge (cumulative %)					
	(3)	North Side	30.1 - 45 m		5%	1,913
		East Side	20.1 - 30 m		10%	
		South Side	> 45.1m		0%	
		West Side	> 45.1m		0%	
Cumulative Total				15%		
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	8,000
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	133
				or	USGPM	2,114
7	Storage Volume		Required Duration of Fire Flow (hours)		Hours	2
			Required Volume of Fire Flow (m ³)		m ³	960

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 111117

Project Name: Kennedy Burnett

Date: 19/07/2017

Input By: Steve Zorgel

Reviewed By: Drew Blair

Legend

Input by User

No Information or Input Required

Building Description: Condo Unit - 4 Storey Blk 20

Non-combustible construction

Step			Input	Multiplier Options	Value Used	Total Fire Flow (L/min)
Base Fire Flow						
1	Construction Material					
	Coefficient related to type of construction C	Wood frame		1.5	0.8	
		Ordinary construction		1		
		Non-combustible construction	Yes	0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	Floor Area					
	A	Building Footprint (m ²)	2500			
		Number of Floors/Storeys	4			
		Area of structure considered (m ²)			10,000	
F	Base fire flow without reductions				18,000	
		$F = 220 C (A)^{0.5}$				
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge					
	(1)	Non-combustible		-25%	-15%	15,300
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	Sprinkler Reduction					
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%	-30%	-7,650
		Standard Water Supply	Yes	-10%	-10%	
		Fully Supervised System	Yes	-10%	-10%	
Cumulative Total				-50%		
5	Exposure Surcharge (cumulative %)					
	(3)	North Side	30.1- 45 m		5%	1,530
		East Side	> 45.1m		0%	
		South Side	30.1- 45 m		5%	
		West Side	> 45.1m		0%	
Cumulative Total				10%		
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	9,000
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	150
				or	USGPM	2,378
7	Storage Volume		Required Duration of Fire Flow (hours)		Hours	2
			Required Volume of Fire Flow (m ³)		m ³	1080

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 111117

Project Name: Kennedy Burnett

Date: 19/07/2017

Input By: Steve Zorgel

Reviewed By: Drew Blair

Legend

Input by User

No Information or Input Required

Building Description: Condo Unit - 4 Storey Blk 16

Non-combustible construction

Step			Input	Multiplier Options	Value Used	Total Fire Flow (L/min)
Base Fire Flow						
1	Construction Material					
	Coefficient related to type of construction C	Wood frame		1.5	0.8	
		Ordinary construction		1		
		Non-combustible construction	Yes	0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	Floor Area					
	A	Building Footprint (m ²)	1600		6,400	
		Number of Floors/Storeys	4			
		Area of structure considered (m ²)				
F	Base fire flow without reductions				14,000	
		$F = 220 C (A)^{0.5}$				
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge					
	(1)	Non-combustible		-25%	-15%	11,900
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	Sprinkler Reduction					
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%	-30%	-5,950
		Standard Water Supply	Yes	-10%	-10%	
		Fully Supervised System	Yes	-10%	-10%	
		Cumulative Total		-50%		
5	Exposure Surcharge (cumulative %)					
	(3)	North Side	20.1 - 30 m		10%	3,570
		East Side	> 45.1m		0%	
		South Side	30.1- 45 m		5%	
		West Side	10.1 - 20 m		15%	
		Cumulative Total		30%		
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	10,000
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	167
				or	USGPM	2,642
7	Storage Volume		Required Duration of Fire Flow (hours)		Hours	2
			Required Volume of Fire Flow (m ³)		m ³	1200

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 111117

Project Name: Kennedy Burnett

Date: 19/07/2017

Input By: Steve Zorgel

Reviewed By: Drew Blair

Legend

Input by User

No Information or Input Required

Building Description: Condo Unit - 4 Storey Blk 19

Non-combustible construction

Step			Input	Multiplier Options	Value Used	Total Fire Flow (L/min)	
Base Fire Flow							
1	Construction Material						
	Coefficient related to type of construction C	Wood frame			1.5	0.8	
		Ordinary construction			1		
		Non-combustible construction	Yes		0.8		
		Fire resistive construction (< 3 hrs)			0.7		
Fire resistive construction (> 3 hrs)				0.6			
2	Floor Area						
	A	Building Footprint (m ²)	2000				
		Number of Floors/Storeys	4				
		Area of structure considered (m ²)			8,000		
	F	Base fire flow without reductions				16,000	
$F = 220 C (A)^{0.5}$							
Reductions or Surcharges							
3	Occupancy hazard reduction or surcharge						
	(1)	Non-combustible			-25%	-15%	13,600
		Limited combustible	Yes		-15%		
		Combustible			0%		
		Free burning			15%		
Rapid burning				25%			
4	Sprinkler Reduction						
	(2)	Adequately Designed System (NFPA 13)	Yes		-30%	-6,800	
		Standard Water Supply	Yes		-10%		
		Fully Supervised System	Yes		-10%		
				Cumulative Total	-50%		
5	Exposure Surcharge (cumulative %)						
	(3)	North Side	> 45.1m		0%	680	
		East Side	> 45.1m		0%		
		South Side	30.1- 45 m		5%		
		West Side	> 45.1m		0%		
				Cumulative Total	5%		
Results							
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	7,000	
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	117	
				or	USGPM	1,849	
7	Storage Volume		Required Duration of Fire Flow (hours)		Hours	2	
			Required Volume of Fire Flow (m ³)		m ³	840	

Table 1						
Watermain Demand Calculations						
Node	Number of Units		Pop.	Demand (L/s)		
	Town	Apartment Condo		High Pres.	Max Daily	Peak Hour
1	5		14	0.06	0.14	0.31
2	7		19	0.08	0.19	0.42
3	10		27	0.11	0.27	0.60
4	4		11	0.04	0.11	0.25
5	7		19	0.08	0.19	0.42
6	6		17	0.07	0.17	0.38
7	16		44	0.18	0.45	0.98
8	18		49	0.20	0.50	1.09
9	8		22	0.09	0.22	0.49
10	6		17	0.07	0.17	0.38
11	6		17	0.07	0.17	0.38
12	9	60	133	0.54	1.35	2.96
13	4	75	146	0.59	1.48	3.25
14	20		54	0.22	0.55	1.20
15	18		49	0.20	0.50	1.09
16	19		52	0.21	0.53	1.16
17	4		11	0.04	0.11	0.25
18	10		27	0.11	0.27	0.60
19	8		22	0.09	0.22	0.49
20	10		27	0.11	0.27	0.60
21			0	0.00	0.00	0.00
22		190	342	1.39	3.46	7.62
23			0	0.00	0.00	0.00
				4.53	11.33	24.93

1. Residential Population density: 2.7 people/town, 2.1 people/apartment
2. Residential High Pressure demand = 350L/s/p/d
3. Residential Maximum Daily demand = 2.5 x High Pressure Demand
4. Residential Peak Hour Demand = 2.2 x Maximum Daily Demand

Table 2
Pipe Data

Pipe	Length (m)	Diameter (mm)	Roughness
1	87	300	120
2	75	300	120
3	44	300	120
4	55	300	120
5	59	300	120
6	72	300	120
7	80	300	120
8	38	200	110
9	74	200	110
10	10	200	110
11	45	50	100
12	56	200	110
13	54	200	110
14	74	300	120
15	74	300	120
16	81	300	120
17	84	200	110
18	79	200	110
19	83	200	110
20	68	200	110
21	76	200	110
22	80	200	110
23	72	200	110
24	120	200	110
25	89	200	110
26	75	300	120
27	130	300	120
28	610	300	120
29	330	300	120

Table 3
Pre Configuration Condition
High Pressure Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
Resvr R1*	157.2	-2.45	157.20	0.0	0.0	0.0
Resvr R2*	157.2	-2.10	157.20	0.0	0.0	0.0
Junc N1	93.5	0.06	157.2	63.7	90.3	24.5
Junc N2	93.9	0.08	157.2	63.3	89.7	12.0
Junc N3	93.8	0.11	157.2	63.4	89.9	7.3
Junc N4	93.6	0.04	157.2	63.6	90.2	6.4
Junc N5	93.8	0.08	157.2	63.4	90.0	5.3
Junc N6	93.8	0.07	157.2	63.5	90.0	4.2
Junc N7	94.0	0.18	157.2	63.3	89.7	3.3
Junc N8	93.5	0.20	157.2	63.7	90.3	3.8
Junc N9	93.6	0.09	157.2	63.6	90.2	5.0
Junc N10	93.4	0.07	157.2	63.8	90.5	5.6
Junc N11	94.1	0.07	157.19	63.1	89.5	6.0
Junc N12	93.4	0.54	157.2	63.8	90.5	10.8
Junc N13	93.2	0.59	157.2	64.0	90.7	13.3
Junc N14	93.4	0.22	157.2	63.9	90.5	12.7
Junc N15	93.6	0.20	157.2	63.7	90.3	10.4
Junc N16	93.5	0.21	157.2	63.8	90.4	5.8
Junc N17	92.9	0.04	157.2	64.3	91.2	25.7
Junc N18	92.9	0.11	157.2	64.3	91.2	26.2
Junc N19	93.6	0.09	157.2	63.6	90.2	38.0
Junc N20	93.8	0.11	157.2	63.5	90.0	27.6
Junc N21	94.1	0.00	157.2	63.1	89.5	9.3
Junc N22	94.1	1.39	157.2	63.1	89.5	5.7
Junc N23	95.3	0.00	157.2	61.9	87.7	2.6

* Boundary Condition

***** Maximum Pressure
***** Maximum Time

Prepared By:
NOVATECH
Date: January 12, 2018

Table 4a
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 1

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-110.7	138.2	0.0	0.0
Resvr R2*	137.1	-67.6	137.1	0.0	0.0
Junc N1	93.5	0.1	133.4	39.9	56.6
Junc N2	93.9	0.2	133.4	39.5	56.0
Junc N3	93.8	0.3	133.4	39.6	56.2
Junc N4	93.6	167.1	133.2	39.6	56.2
Junc N5	93.8	0.2	133.6	39.9	56.5
Junc N6	93.8	0.2	134.0	40.3	57.1
Junc N7	94.0	0.5	134.5	40.6	57.5
Junc N8	93.5	0.5	134.5	41.0	58.1
Junc N9	93.6	0.2	134.4	40.8	57.8
Junc N10	93.4	0.2	134.4	41.0	58.1
Junc N11	94.1	0.2	134.3	40.3	57.1
Junc N12	93.4	1.4	134.3	40.9	58.0
Junc N13	93.2	1.5	134.2	41.0	58.2
Junc N14	93.4	0.6	134.0	40.6	57.6
Junc N15	93.6	0.5	133.7	40.1	56.9
Junc N16	93.5	0.5	134.0	40.6	57.5
Junc N17	92.9	0.1	134.1	41.3	58.5
Junc N18	92.9	0.3	134.1	41.2	58.4
Junc N19	93.6	0.2	133.6	40.0	56.7
Junc N20	93.8	0.3	133.5	39.7	56.3
Junc N21	94.1	0.0	134.5	40.4	57.3
Junc N22	94.1	3.5	134.9	40.8	57.8
Junc N23	95.3	0.0	135.2	39.9	56.6

* **Boundary Condition**

..... Minimum Pressure

Table 4b
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 2

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-120.4	138.2	0.0	0.0
Resvr R2*	136.8	-73.9	136.8	0.0	0.0
Junc N1	93.5	0.1	131.2	37.7	53.4
Junc N2	93.9	183.2	131.2	37.2	52.8
Junc N3	93.8	0.3	132.4	38.5	54.6
Junc N4	93.6	0.1	132.6	39.0	55.3
Junc N5	93.8	0.2	133.0	39.2	55.6
Junc N6	93.8	0.2	133.3	39.6	56.1
Junc N7	94.0	0.5	133.9	39.9	56.6
Junc N8	93.5	0.5	133.8	40.3	57.1
Junc N9	93.6	0.2	133.6	40.1	56.8
Junc N10	93.4	0.2	133.6	40.3	57.1
Junc N11	94.1	0.2	133.6	39.6	56.1
Junc N12	93.4	1.4	133.5	40.1	56.9
Junc N13	93.2	1.5	133.4	40.2	57.0
Junc N14	93.4	0.6	133.1	39.8	56.4
Junc N15	93.6	0.5	132.6	39.1	55.4
Junc N16	93.5	0.5	133.2	39.8	56.4
Junc N17	92.9	0.1	133.3	40.4	57.3
Junc N18	92.9	0.3	133.2	40.4	57.2
Junc N19	93.6	0.2	132.0	38.4	54.4
Junc N20	93.8	0.3	131.4	37.6	53.4
Junc N21	94.1	0.0	133.7	39.6	56.2
Junc N22	94.1	3.5	134.2	40.1	56.8
Junc N23	95.3	0.0	134.7	39.4	55.9

* **Boundary Condition**

Minimum Pressure

Prepared By:
NOVATECH
Date: January 12, 2018

Table 4c
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 4

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-112.6	138.2	0.0	0.0
Resvr R2*	136.8	-65.7	136.8	0.0	0.0
Junc N1	93.5	0.1	133.3	39.8	56.4
Junc N2	93.9	0.2	133.3	39.4	55.8
Junc N3	93.8	0.3	133.3	39.5	56.0
Junc N4	93.6	167.1	133.1	39.5	56.0
Junc N5	93.8	0.2	133.5	39.7	56.3
Junc N6	93.8	0.2	133.9	40.1	56.9
Junc N7	94.0	0.5	134.4	40.4	57.3
Junc N8	93.5	0.5	134.3	40.8	57.9
Junc N9	93.6	0.2	134.2	40.6	57.6
Junc N10	93.4	0.2	134.2	40.8	57.9
Junc N11	94.1	0.2	134.2	40.2	56.9
Junc N12	93.4	1.4	134.1	40.7	57.8
Junc N13	93.2	1.5	134.1	40.9	58.0
Junc N14	93.4	0.6	133.9	40.5	57.4
Junc N15	93.6	0.5	133.5	40.0	56.7
Junc N16	93.5	0.5	133.9	40.4	57.3
Junc N17	92.9	0.1	134.0	41.1	58.3
Junc N18	92.9	0.3	133.9	41.1	58.2
Junc N19	93.6	0.2	133.4	39.8	56.5
Junc N20	93.8	0.3	133.3	39.6	56.1
Junc N21	94.1	0.0	134.3	40.3	57.1
Junc N22	94.1	3.5	134.7	40.6	57.6
Junc N23	95.3	0.0	135.1	39.8	56.4

* Boundary Condition

***** Minimum Pressure

Table 4d
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 6

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-114.6	138.2	0.0	0.0
Resvr R2*	136.8	-63.7	136.8	0.0	0.0
Junc N1	93.5	0.1	133.9	40.4	57.2
Junc N2	93.9	0.2	133.9	40.0	56.6
Junc N3	93.8	0.3	133.9	40.0	56.8
Junc N4	93.6	0.1	133.8	40.2	57.0
Junc N5	93.8	0.2	133.7	40.0	56.7
Junc N6	93.8	167.2	133.6	39.9	56.6
Junc N7	94.0	0.5	134.3	40.3	57.2
Junc N8	93.5	0.5	134.3	40.8	57.8
Junc N9	93.6	0.2	134.2	40.7	57.7
Junc N10	93.4	0.2	134.2	40.9	57.9
Junc N11	94.1	0.2	134.2	40.2	57.0
Junc N12	93.4	1.4	134.2	40.8	57.9
Junc N13	93.2	1.5	134.2	41.0	58.2
Junc N14	93.4	0.6	134.1	40.7	57.7
Junc N15	93.6	0.5	134.0	40.4	57.3
Junc N16	93.5	0.5	133.8	40.4	57.3
Junc N17	92.9	0.1	134.2	41.3	58.6
Junc N18	92.9	0.3	134.1	41.3	58.5
Junc N19	93.6	0.2	133.9	40.3	57.2
Junc N20	93.8	0.3	133.9	40.1	56.9
Junc N21	94.1	0.0	134.5	40.4	57.3
Junc N22	94.1	3.5	134.8	40.7	57.7
Junc N23	95.3	0.0	135.0	39.7	56.3

* Boundary Condition

Minimum Pressure

Table 4e
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 8

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-114.9	138.2	0.0	0.0
Resvr R2*	136.8	-63.4	136.8	0.0	0.0
Junc N1	93.5	0.1	134.3	40.8	57.8
Junc N2	93.9	0.2	134.3	40.3	57.2
Junc N3	93.8	0.3	134.3	40.4	57.3
Junc N4	93.6	0.1	134.3	40.7	57.6
Junc N5	93.8	0.2	134.3	40.5	57.4
Junc N6	93.8	0.2	134.3	40.5	57.4
Junc N7	94.0	0.5	134.3	40.3	57.1
Junc N8	93.5	167.5	131.0	37.5	53.2
Junc N9	93.6	0.2	132.3	38.7	54.9
Junc N10	93.4	0.2	132.3	38.9	55.1
Junc N11	94.1	0.2	132.3	38.2	54.2
Junc N12	93.4	1.4	133.3	39.9	56.5
Junc N13	93.2	1.5	134.3	41.0	58.2
Junc N14	93.4	0.6	134.3	40.9	58.0
Junc N15	93.6	0.5	134.3	40.7	57.7
Junc N16	93.5	0.5	134.3	40.8	57.9
Junc N17	92.9	0.1	134.3	41.4	58.7
Junc N18	92.9	0.3	134.3	41.4	58.7
Junc N19	93.6	0.2	134.3	40.7	57.6
Junc N20	93.8	0.3	134.3	40.5	57.4
Junc N21	94.1	0.0	134.5	40.4	57.3
Junc N22	94.1	3.5	134.9	40.7	57.8
Junc N23	95.3	0.0	135.0	39.7	56.3

* Boundary Condition

 Minimum Pressure

Table 4f
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 10

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-113.6	138.2	0.0	0.0
Resvr R2*	136.8	-64.7	136.8	0.0	0.0
Junc N1	93.5	0.1	134.2	40.7	57.7
Junc N2	93.9	0.2	134.2	40.3	57.1
Junc N3	93.8	0.3	134.2	40.4	57.3
Junc N4	93.6	0.1	134.2	40.6	57.6
Junc N5	93.8	0.2	134.3	40.5	57.4
Junc N6	93.8	0.2	134.3	40.5	57.5
Junc N7	94.0	0.5	134.3	40.4	57.3
Junc N8	93.5	0.5	132.6	39.1	55.4
Junc N9	93.6	0.2	129.2	35.6	50.4
Junc N10	93.4	167.2	127.5	34.1	48.4
Junc N11	94.1	0.2	127.5	33.4	47.4
Junc N12	93.4	1.4	131.7	38.3	54.3
Junc N13	93.2	1.5	134.2	40.9	58.1
Junc N14	93.4	0.6	134.2	40.8	57.9
Junc N15	93.6	0.5	134.2	40.7	57.6
Junc N16	93.5	0.5	134.2	40.8	57.8
Junc N17	92.9	0.1	134.2	41.3	58.6
Junc N18	92.9	0.3	134.2	41.3	58.6
Junc N19	93.6	0.2	134.2	40.6	57.6
Junc N20	93.8	0.3	134.2	40.5	57.4
Junc N21	94.1	0.0	134.4	40.3	57.2
Junc N22	94.1	3.5	134.8	40.7	57.7
Junc N23	95.3	0.00	135.1	39.8	56.4

* Boundary Condition

Minimum Pressure

Table 4g
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 12

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-111.3	138.2	0.0	0.0
Resvr R2*	136.8	-67.1	136.8	0.0	0.0
Junc N1	93.5	0.1	134.2	40.7	57.7
Junc N2	93.9	0.2	134.2	40.3	57.1
Junc N3	93.8	0.3	134.2	40.4	57.2
Junc N4	93.6	0.1	134.2	40.6	57.6
Junc N5	93.8	0.2	134.3	40.5	57.5
Junc N6	93.8	0.2	134.3	40.6	57.6
Junc N7	94.0	0.5	134.5	40.5	57.5
Junc N8	93.5	0.5	133.5	40.0	56.7
Junc N9	93.6	0.2	131.6	38.0	53.8
Junc N10	93.4	0.2	131.6	38.2	54.1
Junc N11	94.1	0.2	131.5	37.5	53.1
Junc N12	93.4	168.4	130.1	36.7	52.1
Junc N13	93.2	1.5	134.0	40.8	57.8
Junc N14	93.4	0.6	134.1	40.7	57.7
Junc N15	93.6	0.5	134.1	40.6	57.5
Junc N16	93.5	0.5	134.2	40.8	57.8
Junc N17	92.9	0.1	134.0	41.1	58.3
Junc N18	92.9	0.3	134.0	41.2	58.4
Junc N19	93.6	0.2	134.2	40.6	57.5
Junc N20	93.8	0.3	134.2	40.4	57.3
Junc N21	94.1	0.0	134.2	40.2	56.9
Junc N22	94.1	3.5	134.6	40.5	57.5
Junc N23	95.3	0.0	135.2	39.9	56.5

* Boundary Condition

Minimum Pressure

Table 4h
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 15

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-128.6	138.2	0.0	0.0
Resvr R2*	136.8	-82.8	136.8	0.0	0.0
Junc N1	93.5	0.1	131.8	38.3	54.3
Junc N2	93.9	0.2	131.8	37.9	***53.7***
Junc N3	93.8	0.3	131.8	38.0	53.9
Junc N4	93.6	0.1	132.1	38.5	54.6
Junc N5	93.8	0.2	132.4	38.6	54.8
Junc N6	93.8	0.2	132.7	39.0	55.3
Junc N7	94.0	0.5	133.3	39.4	55.8
Junc N8	93.5	0.5	133.2	39.7	56.3
Junc N9	93.6	0.2	132.9	39.4	55.8
Junc N10	93.4	0.2	132.9	39.6	56.1
Junc N11	94.1	0.2	132.9	38.9	55.1
Junc N12	93.4	1.4	132.8	39.4	55.8
Junc N13	93.2	1.5	132.6	39.4	55.9
Junc N14	93.4	0.6	132.2	38.9	55.1
Junc N15	93.6	200.5	131.5	38.0	53.8
Junc N16	93.5	0.5	132.5	39.0	55.3
Junc N17	92.9	0.1	132.5	39.6	56.2
Junc N18	92.9	0.3	132.3	39.5	56.0
Junc N19	93.6	0.2	131.6	38.0	53.9
Junc N20	93.8	0.3	131.8	38.0	53.9
Junc N21	94.1	0.0	133.0	38.9	55.2
Junc N22	94.1	3.5	133.6	39.5	56.0
Junc N23	95.3	0.0	134.3	39.0	55.2

* Boundary Condition

*****53.7***** Minimum Pressure

Prepared By:
NOVATECH
Date: January 12, 2018

Table 4i
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 16

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-112.5	138.2	0.0	0.0
Resvr R2*	136.8	-65.8	136.8	0.0	0.0
Junc N1	93.5	0.1	133.9	40.4	57.2
Junc N2	93.9	0.2	133.9	39.9	56.6
Junc N3	93.8	0.3	133.9	40.0	56.8
Junc N4	93.6	0.1	133.9	40.3	57.1
Junc N5	93.8	0.2	133.9	40.1	56.9
Junc N6	93.8	0.2	133.9	40.1	56.9
Junc N7	94.0	0.5	134.4	40.5	57.4
Junc N8	93.5	0.5	134.3	40.8	57.9
Junc N9	93.6	0.2	134.2	40.6	57.6
Junc N10	93.4	0.2	134.2	40.8	57.9
Junc N11	94.1	0.2	134.2	40.1	56.9
Junc N12	93.4	1.4	134.1	40.7	57.8
Junc N13	93.2	1.5	134.1	40.9	57.9
Junc N14	93.4	0.6	133.8	40.5	57.4
Junc N15	93.6	0.5	133.9	40.3	57.1
Junc N16	93.5	167.5	130.2	36.7	52.1
Junc N17	92.9	0.1	134.0	41.1	58.3
Junc N18	92.9	0.3	133.9	41.1	58.2
Junc N19	93.6	0.2	133.9	40.3	57.1
Junc N20	93.8	0.3	133.9	40.1	56.9
Junc N21	94.1	0.0	134.3	40.2	57.1
Junc N22	94.1	3.5	134.7	40.6	57.6
Junc N23	95.3	0.0	135.1	39.8	56.5

* Boundary Condition

***** Minimum Pressure

Table 4j
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 18

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-107.3	138.2	0.0	0.0
Resvr R2*	136.8	-71.0	136.8	0.0	0.0
Junc N1	93.5	0.1	133.9	40.4	57.3
Junc N2	93.9	0.2	133.9	40.0	56.7
Junc N3	93.8	0.3	134.0	40.1	56.9
Junc N4	93.6	0.1	134.1	40.5	57.4
Junc N5	93.8	0.2	134.2	40.4	57.3
Junc N6	93.8	0.2	134.4	40.6	57.6
Junc N7	94.0	0.5	134.7	40.8	57.8
Junc N8	93.5	0.5	134.5	41.0	58.2
Junc N9	93.6	0.2	134.2	40.6	57.5
Junc N10	93.4	0.2	134.2	40.8	57.8
Junc N11	94.1	0.2	134.1	40.1	56.8
Junc N12	93.4	1.4	133.9	40.5	57.4
Junc N13	93.2	1.5	133.7	40.4	57.3
Junc N14	93.4	0.6	133.6	40.3	57.1
Junc N15	93.6	0.5	133.8	40.3	57.1
Junc N16	93.5	0.5	134.0	40.5	57.5
Junc N17	92.9	0.1	131.1	38.2	54.2
Junc N18	92.9	167.3	128.9	36.1	-51.1
Junc N19	93.6	0.2	133.9	40.3	57.1
Junc N20	93.8	0.3	133.9	40.2	56.9
Junc N21	94.1	0.0	133.9	39.9	56.5
Junc N22	94.1	3.5	134.4	40.3	57.1
Junc N23	95.3	0.0	135.4	40.1	56.8

* Boundary Condition

***** Minimum Pressure

Table 4k
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 20

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-111.5	138.2	0.0	0.0
Resvr R2*	136.8	-66.9	136.8	0.0	0.0
Junc N1	93.5	0.1	132.3	38.8	55.0
Junc N2	93.9	0.2	132.5	38.6	54.7
Junc N3	93.8	0.3	133.2	39.4	55.9
Junc N4	93.6	0.1	133.4	39.8	56.5
Junc N5	93.8	0.2	133.7	39.9	56.6
Junc N6	93.8	0.2	134.0	40.2	57.1
Junc N7	94.0	0.5	134.5	40.5	57.4
Junc N8	93.5	0.5	134.4	40.9	58.0
Junc N9	93.6	0.2	134.2	40.6	57.6
Junc N10	93.4	0.2	134.2	40.8	57.9
Junc N11	94.1	0.2	134.2	40.1	56.9
Junc N12	93.4	1.4	134.1	40.7	57.7
Junc N13	93.2	1.5	134.0	40.8	57.8
Junc N14	93.4	0.6	133.7	40.4	57.3
Junc N15	93.6	0.5	133.3	39.7	56.4
Junc N16	93.5	0.5	133.9	40.4	57.3
Junc N17	92.9	0.1	133.9	41.0	58.2
Junc N18	92.9	0.3	133.8	41.0	58.1
Junc N19	93.6	0.2	131.7	38.1	54.0
Junc N20	93.8	167.3	130.2	36.4	51.6
Junc N21	94.1	0.0	134.2	40.2	57.0
Junc N22	94.1	3.5	134.6	40.5	57.5
Junc N23	95.3	0.0	135.2	39.9	56.5

* Boundary Condition

***** Minimum Pressure

Table 4I
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 22

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	138.2	-85.9	138.2	0.0	0.0
Resvr R2*	136.8	-75.5	136.8	0.0	0.0
Junc N1	93.5	0.1	135.4	41.9	59.4
Junc N2	93.9	0.2	135.4	41.5	58.8
Junc N3	93.8	0.3	135.4	41.6	59.0
Junc N4	93.6	0.1	135.5	41.9	59.4
Junc N5	93.8	0.2	135.6	41.8	59.3
Junc N6	93.8	0.2	135.7	41.9	59.4
Junc N7	94.0	0.5	135.9	42.0	59.5
Junc N8	93.5	0.5	135.7	42.2	59.9
Junc N9	93.6	0.2	135.5	41.9	59.4
Junc N10	93.4	0.2	135.5	42.1	59.7
Junc N11	94.1	0.2	135.4	41.4	58.7
Junc N12	93.4	1.4	135.2	41.8	59.3
Junc N13	93.2	1.5	135.1	41.9	59.3
Junc N14	93.4	0.6	135.2	41.9	59.4
Junc N15	93.6	0.5	135.3	41.8	59.2
Junc N16	93.5	0.5	135.5	42.0	59.6
Junc N17	92.9	0.1	135.1	42.3	59.9
Junc N18	92.9	0.3	135.2	42.3	60.0
Junc N19	93.6	0.2	135.4	41.8	59.2
Junc N20	93.8	0.3	135.4	41.7	59.1
Junc N21	94.1	0.0	134.7	40.7	57.6
Junc N22	94.1	153.5	134.1	40.0	56.7
Junc N23	95.3	0.0	136.4	41.0	58.2

* Boundary Condition

Minimum Pressure

Prepared By:
NOVATECH
Date: January 12, 2018

Table 5
Pre Configuration Condition
Peak Hour Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	157.2	-143.66	157.2	0.0	0.0
Resvr R2*	140.5	118.74	140.5	0.0	0.0
Junc N1	93.5	0.31	149.98	56.5	80.1
Junc N2	93.9	0.42	149.98	56.1	79.5
Junc N3	93.8	0.60	149.99	56.2	79.6
Junc N4	93.6	0.25	150.17	56.6	80.2
Junc N5	93.8	0.42	150.4	56.6	80.3
Junc N6	93.8	0.38	150.64	56.9	80.7
Junc N7	94.0	0.98	151.22	57.3	81.2
Junc N8	93.5	1.09	150.82	57.3	81.3
Junc N9	93.6	0.49	150.07	56.5	80.1
Junc N10	93.4	0.38	150.07	56.7	80.4
Junc N11	94.1	0.38	149.98	55.9	79.3
Junc N12	93.4	2.96	149.54	56.1	79.6
Junc N13	93.2	3.25	149.11	55.9	79.3
Junc N14	93.4	1.20	149.49	56.1	79.6
Junc N15	93.6	1.09	149.77	56.2	79.7
Junc N16	93.5	1.16	150.07	56.6	80.3
Junc N17	92.9	0.25	149.24	56.4	79.9
Junc N18	92.9	0.60	149.36	56.5	80.1
Junc N19	93.6	0.49	149.86	56.3	79.8
Junc N20	93.8	0.60	149.94	56.2	79.7
Junc N21	94.1	0.00	148.24	54.2	76.8
Junc N22	94.1	7.62	146.75	52.6	74.6
Junc N23	95.3	0.00	152.39	57.1	80.9

* Boundary Condition

Minimum Pressure

Table 6
Post Configuration Condition
High Pressure Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
Resvr R1*	147.7	-2.45	147.70	0.0	0.0	0.0
Resvr R2*	147.7	-2.10	147.70	0.0	0.0	0.0
Junc N1	93.5	0.06	147.7	54.2	76.9	24.5
Junc N2	93.9	0.08	147.7	53.8	76.3	12.0
Junc N3	93.8	0.11	147.7	53.9	76.4	7.3
Junc N4	93.6	0.04	147.7	54.1	76.7	6.4
Junc N5	93.8	0.08	147.7	53.9	76.5	5.3
Junc N6	93.8	0.07	147.7	54.0	76.5	4.2
Junc N7	94.0	0.18	147.7	53.8	76.2	3.3
Junc N8	93.5	0.20	147.7	54.2	76.9	3.8
Junc N9	93.6	0.09	147.7	54.1	76.7	5.0
Junc N10	93.4	0.07	147.7	54.3	77.0	5.6
Junc N11	94.1	0.07	147.69	53.6	76.1	6.0
Junc N12	93.4	0.54	147.7	54.3	77.0	10.8
Junc N13	93.2	0.59	147.7	54.5	77.3	13.3
Junc N14	93.4	0.22	147.7	54.4	77.1	12.7
Junc N15	93.6	0.20	147.7	54.2	76.8	10.4
Junc N16	93.5	0.21	147.7	54.3	76.9	5.8
Junc N17	92.9	0.04	147.7	54.8	** 77.8 **	25.7
Junc N18	92.9	0.11	147.7	54.8	** 77.8 **	26.2
Junc N19	93.6	0.09	147.7	54.1	76.7	38.0
Junc N20	93.8	0.11	147.7	54.0	76.5	27.6
Junc N21	94.1	0.00	147.7	53.6	76.1	9.3
Junc N22	94.1	1.39	147.7	53.6	76.0	5.7
Junc N23	95.3	0.00	147.7	52.4	74.3	2.6

* Boundary Condition

***** Maximum Pressure
***** Maximum Time

Prepared By:
NOVATECH
Date: January 12, 2018

Table 7a
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 1

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-109.2	146.4	0.0	0.0
Resvr R2*	145.4	-69.1	145.4	0.0	0.0
Junc N1	93.5	167.1	139.5	46.0	65.2
Junc N2	93.9	0.2	140.6	46.6	66.1
Junc N3	93.8	0.3	141.5	47.7	67.7
Junc N4	93.6	0.1	141.8	48.2	68.3
Junc N5	93.8	0.2	142.0	48.3	68.4
Junc N6	93.8	0.2	142.3	48.6	68.9
Junc N7	94.0	0.5	142.8	48.9	69.3
Junc N8	93.5	0.5	142.7	49.2	69.8
Junc N9	93.6	0.2	142.6	49.0	69.5
Junc N10	93.4	0.2	142.6	49.2	69.8
Junc N11	94.1	0.2	142.6	48.5	68.8
Junc N12	93.4	1.4	142.5	49.1	69.6
Junc N13	93.2	1.5	142.4	49.2	69.8
Junc N14	93.4	0.6	142.2	48.8	69.2
Junc N15	93.6	0.5	141.7	48.2	68.3
Junc N16	93.5	0.5	142.2	48.8	69.2
Junc N17	92.9	0.1	142.3	49.5	70.1
Junc N18	92.9	0.3	142.2	49.4	70.0
Junc N19	93.6	0.2	141.1	47.5	67.4
Junc N20	93.8	0.3	140.5	46.8	66.3
Junc N21	94.1	0.0	142.7	48.6	68.9
Junc N22	94.1	3.5	143.1	49.0	69.5
Junc N23	95.3	0.0	143.5	48.2	68.3

* **Boundary Condition**

*****65.2***** Minimum Pressure

Table 7b
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 2

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-118.1	146.4	0.0	0.0
Resvr R2*	145.4	-76.2	145.4	0.0	0.0
Junc N1	93.5	0.1	139.6	46.1	65.3
Junc N2	93.9	183.2	139.5	45.6	***64.7***
Junc N3	93.8	0.3	140.7	46.9	66.5
Junc N4	93.6	0.1	141.0	47.4	67.2
Junc N5	93.8	0.2	141.3	47.6	67.5
Junc N6	93.8	0.2	141.7	47.9	68.0
Junc N7	94.0	0.5	142.2	48.3	68.5
Junc N8	93.5	0.5	142.2	48.7	69.0
Junc N9	93.6	0.2	142.0	48.4	68.7
Junc N10	93.4	0.2	142.0	48.6	68.9
Junc N11	94.1	0.2	142.0	47.9	68.0
Junc N12	93.4	1.4	141.9	48.5	68.8
Junc N13	93.2	1.5	141.8	48.6	68.9
Junc N14	93.4	0.6	141.5	48.2	68.3
Junc N15	93.6	0.5	141.0	47.4	67.3
Junc N16	93.5	0.5	141.6	48.1	68.3
Junc N17	92.9	0.1	141.7	48.8	69.2
Junc N18	92.9	0.3	141.6	48.7	69.1
Junc N19	93.6	0.2	140.4	46.8	66.3
Junc N20	93.8	0.3	139.8	46.0	65.3
Junc N21	94.1	0.0	142.1	48.1	68.1
Junc N22	94.1	3.5	142.7	48.5	68.8
Junc N23	95.3	0.0	143.1	47.7	67.7

* Boundary Condition

*** Minimum Pressure

Table 7c
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 4

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-110.1	146.4	0.0	0.0
Resvr R2*	145.4	-68.3	145.4	0.0	0.0
Junc N1	93.5	0.1	141.7	48.2	68.3
Junc N2	93.9	0.2	141.7	47.8	67.7
Junc N3	93.8	0.3	141.7	47.9	67.9
Junc N4	93.6	167.1	141.5	47.9	67.9
Junc N5	93.8	0.2	141.9	48.1	68.2
Junc N6	93.8	0.2	142.3	48.5	68.8
Junc N7	94.0	0.5	142.8	48.8	69.2
Junc N8	93.5	0.5	142.7	49.2	69.8
Junc N9	93.6	0.2	142.6	49.0	69.5
Junc N10	93.4	0.2	142.6	49.2	69.8
Junc N11	94.1	0.2	142.6	48.5	68.8
Junc N12	93.4	1.4	142.5	49.1	69.7
Junc N13	93.2	1.5	142.5	49.3	69.9
Junc N14	93.4	0.6	142.2	48.9	69.3
Junc N15	93.6	0.5	141.9	48.4	68.6
Junc N16	93.5	0.5	142.2	48.8	69.2
Junc N17	92.9	0.1	142.4	49.5	70.2
Junc N18	92.9	0.3	142.3	49.5	70.1
Junc N19	93.6	0.2	141.8	48.2	68.4
Junc N20	93.8	0.3	141.7	48.0	68.0
Junc N21	94.1	0.0	142.7	48.7	69.0
Junc N22	94.1	3.5	143.2	49.1	69.6
Junc N23	95.3	0.0	143.5	48.1	68.2

* Boundary Condition

***** Minimum Pressure

Table 7d
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 6

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-111.9	146.4	0.0	0.0
Resvr R2*	145.4	-66.4	145.4	0.0	0.0
Junc N1	93.5	0.1	142.3	48.8	69.1
Junc N2	93.9	0.2	142.3	48.3	68.5
Junc N3	93.8	0.3	142.3	48.4	68.7
Junc N4	93.6	0.1	142.2	48.6	68.9
Junc N5	93.8	0.2	142.1	48.3	68.5
Junc N6	93.8	167.2	142.0	48.3	68.4
Junc N7	94.0	0.5	142.6	48.7	69.0
Junc N8	93.5	0.5	142.6	49.1	69.7
Junc N9	93.6	0.2	142.6	49.1	69.6
Junc N10	93.4	0.2	142.6	49.3	69.8
Junc N11	94.1	0.2	142.6	48.6	68.8
Junc N12	93.4	1.4	142.6	49.2	69.8
Junc N13	93.2	1.5	142.6	49.4	70.1
Junc N14	93.4	0.6	142.5	49.1	69.6
Junc N15	93.6	0.5	142.3	48.8	69.2
Junc N16	93.5	0.5	142.2	48.8	69.2
Junc N17	92.9	0.1	142.6	49.7	70.5
Junc N18	92.9	0.3	142.5	49.7	70.4
Junc N19	93.6	0.2	142.3	48.7	69.1
Junc N20	93.8	0.3	142.3	48.5	68.8
Junc N21	94.1	0.0	142.9	48.8	69.2
Junc N22	94.1	3.5	143.3	49.2	69.7
Junc N23	95.3	0.0	143.4	48.0	68.1

* Boundary Condition

***** Minimum Pressure

Table 7e
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 8

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-112.1	146.4	0.0	0.0
Resvr R2*	145.4	-66.2	145.4	0.0	0.0
Junc N1	93.5	0.1	142.6	49.1	69.7
Junc N2	93.9	0.2	142.6	48.7	69.1
Junc N3	93.8	0.3	142.6	48.8	69.2
Junc N4	93.6	0.1	142.6	49.0	69.5
Junc N5	93.8	0.2	142.6	48.9	69.3
Junc N6	93.8	0.2	142.6	48.9	69.3
Junc N7	94.0	0.5	142.6	48.7	69.0
Junc N8	93.5	167.5	139.4	45.9	65.0
Junc N9	93.6	0.2	140.7	47.1	66.7
Junc N10	93.4	0.2	140.7	47.3	67.0
Junc N11	94.1	0.2	140.6	46.6	66.1
Junc N12	93.4	1.4	141.6	48.2	68.4
Junc N13	93.2	1.5	142.6	49.4	70.1
Junc N14	93.4	0.6	142.6	49.3	69.9
Junc N15	93.6	0.5	142.6	49.1	69.6
Junc N16	93.5	0.5	142.6	49.2	69.7
Junc N17	92.9	0.1	142.6	49.8	70.6
Junc N18	92.9	0.3	142.6	49.8	70.6
Junc N19	93.6	0.2	142.6	49.0	69.5
Junc N20	93.8	0.3	142.6	48.9	69.3
Junc N21	94.1	0.0	142.9	48.8	69.2
Junc N22	94.1	3.5	143.3	49.2	69.7
Junc N23	95.3	0.0	143.4	48.0	68.1

* Boundary Condition

***** Minimum Pressure

Table 7f
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 10

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-110.9	146.4	0.0	0.0
Resvr R2*	145.4	-67.4	145.4	0.0	0.0
Junc N1	93.5	0.1	142.6	49.1	69.6
Junc N2	93.9	0.2	142.6	48.7	69.0
Junc N3	93.8	0.3	142.6	48.8	69.2
Junc N4	93.6	0.1	142.6	49.0	69.5
Junc N5	93.8	0.2	142.6	48.9	69.3
Junc N6	93.8	0.2	142.7	48.9	69.3
Junc N7	94.0	0.5	142.7	48.8	69.1
Junc N8	93.5	0.5	140.9	47.4	67.3
Junc N9	93.6	0.2	137.5	44.0	62.3
Junc N10	93.4	167.2	135.9	42.5	60.3
Junc N11	94.1	0.2	135.9	41.8	59.3
Junc N12	93.4	1.4	140.0	46.6	66.1
Junc N13	93.2	1.5	142.6	49.3	70.0
Junc N14	93.4	0.6	142.6	49.2	69.8
Junc N15	93.6	0.5	142.6	49.0	69.5
Junc N16	93.5	0.5	142.6	49.2	69.7
Junc N17	92.9	0.1	142.6	49.7	70.5
Junc N18	92.9	0.3	142.6	49.7	70.5
Junc N19	93.6	0.2	142.6	49.0	69.5
Junc N20	93.8	0.3	142.6	48.9	69.3
Junc N21	94.1	0.0	142.8	48.7	69.1
Junc N22	94.1	3.5	143.2	49.1	69.6
Junc N23	95.3	0.00	143.4	48.1	68.2

* Boundary Condition

Minimum Pressure

Table 7g
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 12

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-108.7	146.4	0.0	0.0
Resvr R2*	145.4	-69.6	145.4	0.0	0.0
Junc N1	93.5	0.1	142.6	49.1	69.6
Junc N2	93.9	0.2	142.6	48.6	69.0
Junc N3	93.8	0.3	142.6	48.7	69.1
Junc N4	93.6	0.1	142.6	49.0	69.5
Junc N5	93.8	0.2	142.7	48.9	69.3
Junc N6	93.8	0.2	142.7	49.0	69.4
Junc N7	94.0	0.5	142.8	48.9	69.3
Junc N8	93.5	0.5	141.8	48.3	68.5
Junc N9	93.6	0.2	139.9	46.3	65.7
Junc N10	93.4	0.2	139.9	46.5	66.0
Junc N11	94.1	0.2	139.9	45.9	65.0
Junc N12	93.4	168.4	138.5	45.1	64.0
Junc N13	93.2	1.5	142.4	49.2	69.7
Junc N14	93.4	0.6	142.5	49.1	69.6
Junc N15	93.6	0.5	142.5	49.0	69.4
Junc N16	93.5	0.5	142.6	49.1	69.7
Junc N17	92.9	0.1	142.4	49.5	70.2
Junc N18	92.9	0.3	142.4	49.6	70.3
Junc N19	93.6	0.2	142.5	48.9	69.4
Junc N20	93.8	0.3	142.6	48.8	69.2
Junc N21	94.1	0.0	142.6	48.6	68.9
Junc N22	94.1	3.5	143.1	49.0	69.4
Junc N23	95.3	0.0	143.5	48.2	68.3

* Boundary Condition

 Minimum Pressure

Table 7h
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 15

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-126.5	146.4	0.0	0.0
Resvr R2*	145.4	-84.9	145.4	0.0	0.0
Junc N1	93.5	0.1	140.2	46.7	66.2
Junc N2	93.9	0.2	140.2	46.3	65.6
Junc N3	93.8	0.3	140.2	46.4	65.8
Junc N4	93.6	0.1	140.5	46.9	66.4
Junc N5	93.8	0.2	140.8	47.0	66.6
Junc N6	93.8	0.2	141.1	47.3	67.1
Junc N7	94.0	0.5	141.7	47.7	67.7
Junc N8	93.5	0.5	141.6	48.1	68.1
Junc N9	93.6	0.2	141.3	47.7	67.7
Junc N10	93.4	0.2	141.3	47.9	68.0
Junc N11	94.1	0.2	141.3	47.2	67.0
Junc N12	93.4	1.4	141.2	47.8	67.7
Junc N13	93.2	1.5	141.0	47.8	67.8
Junc N14	93.4	0.6	140.6	47.3	67.0
Junc N15	93.6	200.5	139.9	46.3	65.7
Junc N16	93.5	0.5	140.9	47.4	67.2
Junc N17	92.9	0.1	140.9	48.0	68.0
Junc N18	92.9	0.3	140.7	47.9	67.9
Junc N19	93.6	0.2	140.0	46.4	65.8
Junc N20	93.8	0.3	140.1	46.4	65.8
Junc N21	94.1	0.0	141.4	47.3	67.1
Junc N22	94.1	3.5	142.1	47.9	68.0
Junc N23	95.3	0.0	142.6	47.3	67.0

* Boundary Condition

Minimum Pressure

Table 7i
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 16

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-110.0	146.4	0.0	0.0
Resvr R2*	145.4	-68.4	145.4	0.0	0.0
Junc N1	93.5	0.1	142.2	48.7	69.1
Junc N2	93.9	0.2	142.2	48.3	68.5
Junc N3	93.8	0.3	142.2	48.4	68.7
Junc N4	93.6	0.1	142.3	48.7	69.0
Junc N5	93.8	0.2	142.3	48.5	68.8
Junc N6	93.8	0.2	142.3	48.5	68.8
Junc N7	94.0	0.5	142.8	48.8	69.2
Junc N8	93.5	0.5	142.7	49.2	69.8
Junc N9	93.6	0.2	142.6	49.0	69.5
Junc N10	93.4	0.2	142.6	49.2	69.8
Junc N11	94.1	0.2	142.6	48.5	68.8
Junc N12	93.4	1.4	142.5	49.1	69.7
Junc N13	93.2	1.5	142.5	49.3	69.9
Junc N14	93.4	0.6	142.2	48.9	69.3
Junc N15	93.6	0.5	142.2	48.7	69.0
Junc N16	93.5	167.5	138.5	45.1	63.9
Junc N17	92.9	0.1	142.4	49.5	70.2
Junc N18	92.9	0.3	142.3	49.5	70.1
Junc N19	93.6	0.2	142.2	48.6	69.0
Junc N20	93.8	0.3	142.2	48.5	68.8
Junc N21	94.1	0.0	142.7	48.7	69.0
Junc N22	94.1	3.5	143.2	49.0	69.5
Junc N23	95.3	0.0	143.5	48.1	68.3

* **Boundary Condition**

Minimum Pressure

Prepared By:
NOVATECH
Date: January 12, 2018

Table 7j
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 18

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-104.9	146.4	0.0	0.0
Resvr R2*	145.4	-73.4	145.4	0.0	0.0
Junc N1	93.5	0.1	142.3	48.8	69.2
Junc N2	93.9	0.2	142.3	48.4	68.6
Junc N3	93.8	0.3	142.3	48.5	68.8
Junc N4	93.6	0.1	142.4	48.8	69.2
Junc N5	93.8	0.2	142.6	48.8	69.2
Junc N6	93.8	0.2	142.7	49.0	69.4
Junc N7	94.0	0.5	143.1	49.1	69.6
Junc N8	93.5	0.5	142.9	49.4	70.0
Junc N9	93.6	0.2	142.5	48.9	69.4
Junc N10	93.4	0.2	142.5	49.1	69.7
Junc N11	94.1	0.2	142.5	48.5	68.7
Junc N12	93.4	1.4	142.3	48.9	69.3
Junc N13	93.2	1.5	142.1	48.8	69.3
Junc N14	93.4	0.6	142.0	48.7	69.0
Junc N15	93.6	0.5	142.2	48.6	69.0
Junc N16	93.5	0.5	142.4	48.9	69.4
Junc N17	92.9	0.1	139.4	46.6	66.1
Junc N18	92.9	167.3	137.3	44.5	63.0
Junc N19	93.6	0.2	142.2	48.6	69.0
Junc N20	93.8	0.3	142.3	48.5	68.8
Junc N21	94.1	0.0	142.3	48.3	68.4
Junc N22	94.1	3.5	142.8	48.7	69.1
Junc N23	95.3	0.0	143.7	48.4	68.6

* Boundary Condition

Minimum Pressure

Table 7k
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 20

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
Resvr R1*	146.4	-109.0	146.4	0.0	0.0
Resvr R2*	145.4	-69.3	145.4	0.0	0.0
Junc N1	93.5	0.1	140.7	47.2	66.9
Junc N2	93.9	0.2	140.9	47.0	66.6
Junc N3	93.8	0.3	141.6	47.8	67.7
Junc N4	93.6	0.1	141.8	48.2	68.3
Junc N5	93.8	0.2	142.1	48.3	68.5
Junc N6	93.8	0.2	142.4	48.6	68.9
Junc N7	94.0	0.5	142.8	48.9	69.3
Junc N8	93.5	0.5	142.7	49.2	69.8
Junc N9	93.6	0.2	142.6	49.0	69.5
Junc N10	93.4	0.2	142.6	49.2	69.8
Junc N11	94.1	0.2	142.6	48.5	68.8
Junc N12	93.4	1.4	142.5	49.1	69.6
Junc N13	93.2	1.5	142.4	49.2	69.7
Junc N14	93.4	0.6	142.1	48.8	69.2
Junc N15	93.6	0.5	141.7	48.1	68.2
Junc N16	93.5	0.5	142.2	48.8	69.2
Junc N17	92.9	0.1	142.3	49.4	70.1
Junc N18	92.9	0.3	142.2	49.4	70.0
Junc N19	93.6	0.2	140.0	46.4	65.8
Junc N20	93.8	167.3	138.6	44.8	63.5
Junc N21	94.1	0.0	142.7	48.6	68.9
Junc N22	94.1	3.5	143.1	49.0	69.5
Junc N23	95.3	0.0	143.5	48.2	68.3

* **Boundary Condition**

 Minimum Pressure

Table 7I
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 22

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
Resvr R1*	146.4	-83.3	146.4	0.0	0.0
Resvr R2*	145.4	-78.0	145.4	0.0	0.0
Junc N1	93.5	0.1	143.8	50.3	71.3
Junc N2	93.9	0.2	143.8	49.9	70.7
Junc N3	93.8	0.3	143.8	50.0	70.8
Junc N4	93.6	0.1	143.8	50.2	71.2
Junc N5	93.8	0.2	143.9	50.2	71.1
Junc N6	93.8	0.2	144.0	50.3	71.3
Junc N7	94.0	0.5	144.2	50.3	71.3
Junc N8	93.5	0.5	144.1	50.6	71.7
Junc N9	93.6	0.2	143.8	50.2	71.2
Junc N10	93.4	0.2	143.8	50.4	71.5
Junc N11	94.1	0.2	143.8	49.7	70.5
Junc N12	93.4	1.4	143.6	50.2	71.2
Junc N13	93.2	1.5	143.4	50.2	71.2
Junc N14	93.4	0.6	143.6	50.2	71.2
Junc N15	93.6	0.5	143.7	50.1	71.1
Junc N16	93.5	0.5	143.8	50.4	71.4
Junc N17	92.9	0.1	143.5	50.6	71.8
Junc N18	92.9	0.3	143.5	50.7	71.9
Junc N19	93.6	0.2	143.7	50.1	71.1
Junc N20	93.8	0.3	143.8	50.0	70.9
Junc N21	94.1	0.0	143.1	49.0	69.5
Junc N22	94.1	153.5	142.5	48.4	68.7
Junc N23	95.3	0.0	144.7	49.3	69.9

* Boundary Condition

Minimum Pressure

Table 8
Post Configuration Condition
Peak Hour Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
Resvr R1*	146.3	-33.22	146.3	0.0	0.0
Resvr R2	145.7	8.30	145.7	0.0	0.0
Junc N1	93.5	0.31	145.83	52.3	74.2
Junc N2	93.9	0.42	145.83	51.9	73.6
Junc N3	93.8	0.60	145.83	52.0	73.8
Junc N4	93.6	0.25	145.84	52.2	74.1
Junc N5	93.8	0.42	145.85	52.1	73.9
Junc N6	93.8	0.38	145.87	52.1	73.9
Junc N7	94.0	0.98	145.9	52.0	73.7
Junc N8	93.5	1.09	145.87	52.4	74.3
Junc N9	93.6	0.49	145.83	52.3	74.1
Junc N10	93.4	0.38	145.83	52.5	74.4
Junc N11	94.1	0.38	145.73	51.7	73.3
Junc N12	93.4	2.96	145.8	52.4	74.3
Junc N13	93.2	3.25	145.8	52.6	74.6
Junc N14	93.4	1.20	145.81	52.5	74.4
Junc N15	93.6	1.09	145.82	52.3	74.1
Junc N16	93.5	1.16	145.83	52.4	74.3
Junc N17	92.9	0.25	145.8	52.9	75.1
Junc N18	92.9	0.60	145.8	52.9	75.1
Junc N19	93.6	0.49	145.82	52.2	74.0
Junc N20	93.8	0.60	145.83	52.1	73.8
Junc N21	94.1	0.00	145.78	51.7	73.3
Junc N22	94.1	7.62	145.75	51.6	73.2
Junc N23	95.3	0.00	145.98	50.7	71.8

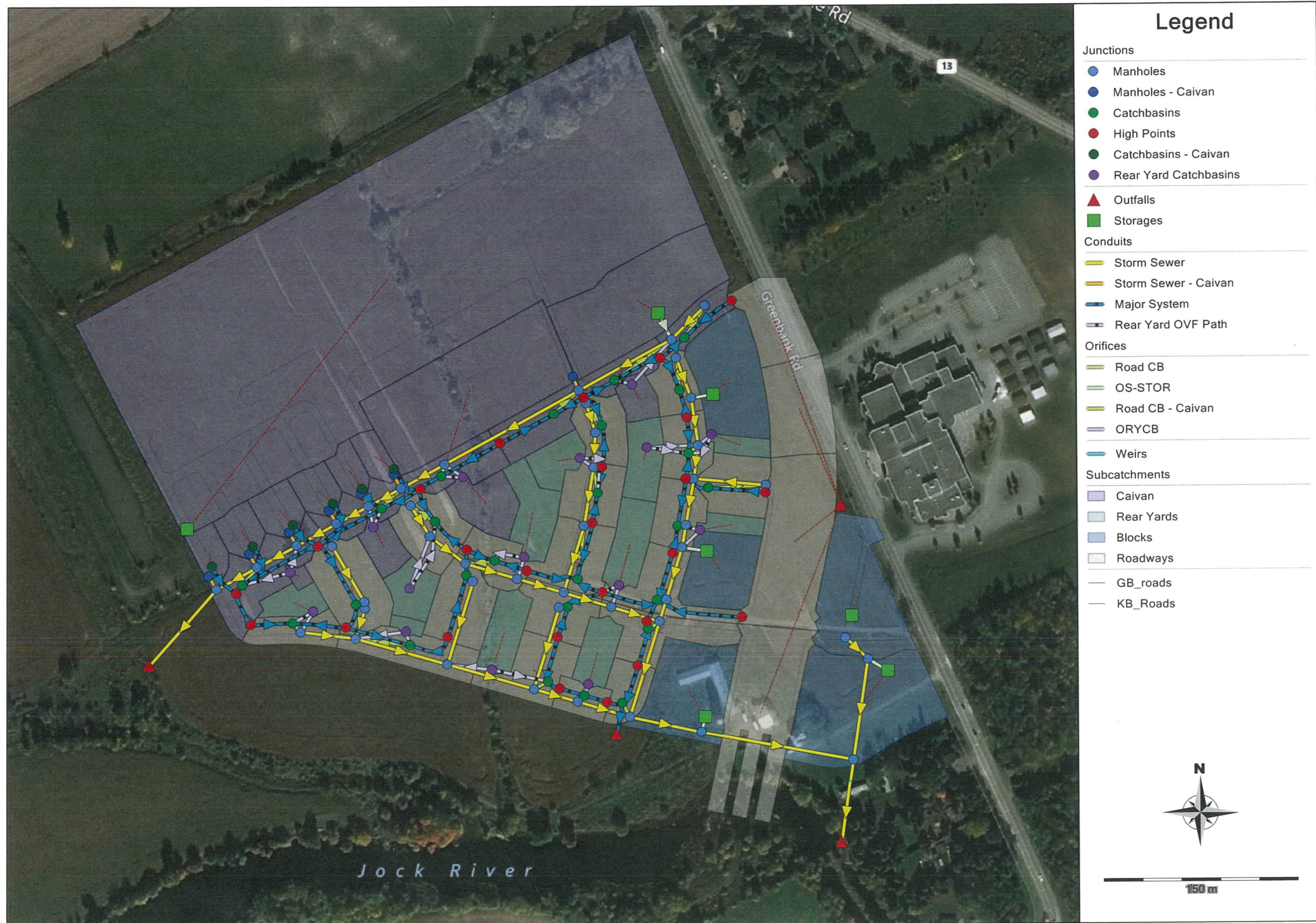
 Minimum Pressure

Prepared By:
NOVATECH
Date: January 12, 2018

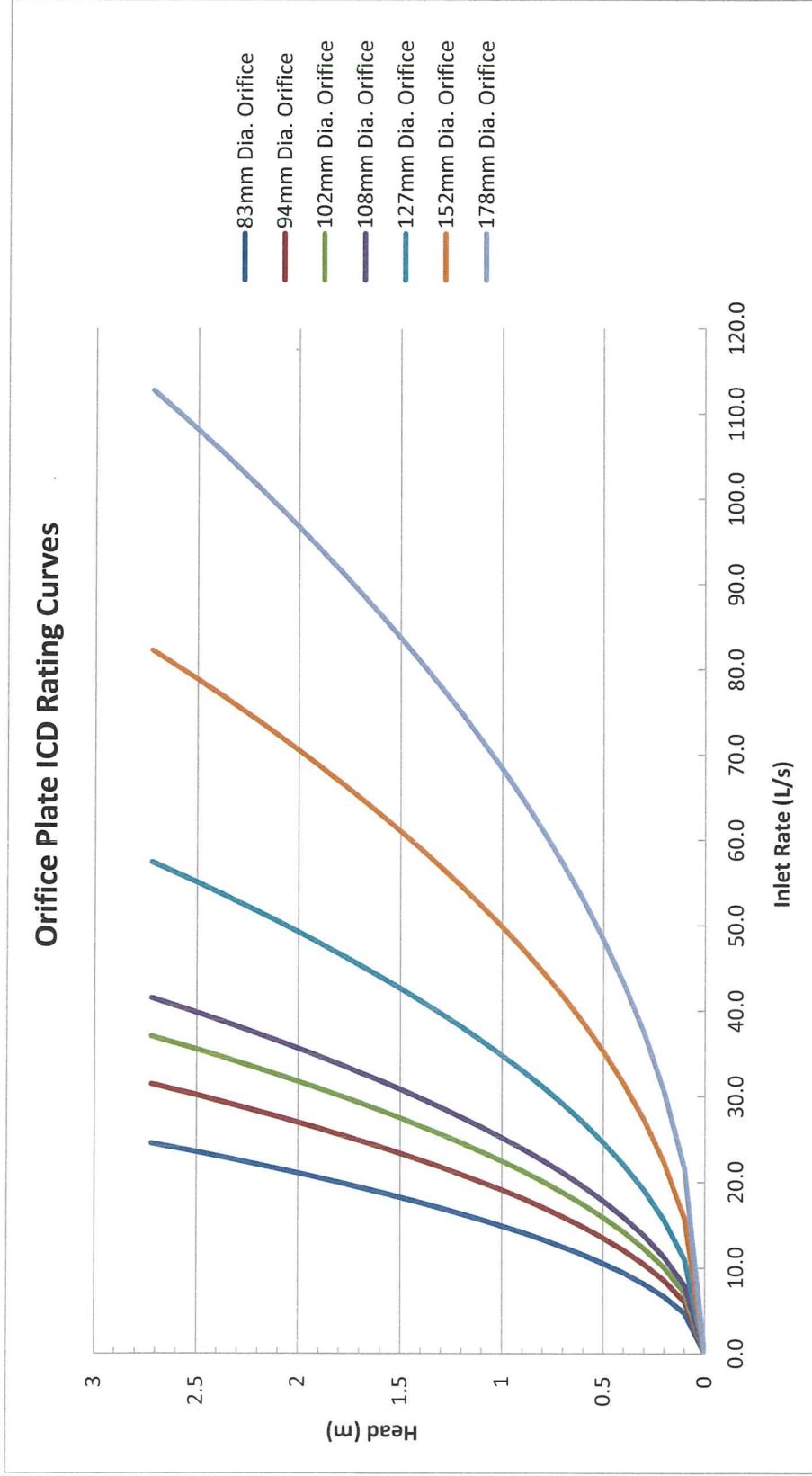
M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20180106\Scenario 2 - Post-Configured
Zone\PeakHour.xls

Appendix D

SWM Calculations & PCSWMM Model



Burnett Lands - 3370 Greenbank Road ICD Rating Curves



Burnett Lands - 3370 Greenbank Road
Ponding in Road Calculations

Structure	T/G (m)	Max. Static Ponding (Spill Depth)		2-yr Event				5-yr Event				100-yr Event (4hr)				100-yr Event (+20%) (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)
Burnett Lands - Claridge																			
CB01-02	93.39	93.65	0.26	93.31	0.00	N	0.00	93.45	0.06	N	0.00	93.56	0.17	N	0.00	93.61	0.22	N	0.00
CB03-04	93.27	93.58	0.31	93.30	0.03	N	0.00	93.35	0.08	N	0.00	93.46	0.19	N	0.00	93.50	0.23	N	0.00
CB05-06	93.17	93.50	0.33	93.19	0.02	N	0.00	93.26	0.09	N	0.00	93.38	0.21	N	0.00	93.45	0.28	N	0.00
CB07-08	93.33	93.69	0.36	93.40	0.07	N	0.00	93.46	0.13	N	0.00	93.59	0.26	N	0.00	93.64	0.31	N	0.00
CB09-10	93.26	93.50	0.24	93.26	0.00	N	0.00	93.34	0.08	N	0.00	93.49	0.23	N	0.00	93.54	0.28	Y	0.04
CB11-12	93.24	93.43	0.20	93.20	0.00	N	0.00	93.34	0.11	N	0.00	93.46	0.22	Y	0.03	93.49	0.25	Y	0.06
CB13-14	93.30	93.56	0.26	93.23	0.00	N	0.00	93.37	0.07	N	0.00	93.48	0.18	N	0.00	93.53	0.23	N	0.00
CB15-16	93.32	93.52	0.20	93.18	0.00	N	0.00	93.40	0.08	N	0.00	93.55	0.23	Y	0.03	93.57	0.25	Y	0.05
CB17-18	93.10	93.39	0.29	93.11	0.01	N	0.00	93.17	0.07	N	0.00	93.28	0.18	N	0.00	93.36	0.26	N	0.00
CB19-20	92.93	93.12	0.19	92.93	0.00	N	0.00	93.01	0.08	N	0.00	93.14	0.21	Y	0.02	93.16	0.23	Y	0.04
CB21-22	92.73	92.89	0.16	92.75	0.02	N	0.00	92.86	0.13	N	0.00	92.97	0.24	Y	0.08	92.99	0.26	Y	0.10
CB23-24	92.69	92.89	0.20	91.93	0.00	N	0.00	92.73	0.04	N	0.00	92.94	0.25	Y	0.05	92.99	0.30	Y	0.10
CB25-26	92.73	92.89	0.16	92.55	0.00	N	0.00	92.79	0.06	N	0.00	92.94	0.21	Y	0.05	92.99	0.26	Y	0.10
CB27-28	92.85	92.92	0.07	92.64	0.00	N	0.00	92.93	0.08	Y	0.01	93.02	0.17	Y	0.10	93.07	0.22	Y	0.15
CB29-30	92.88	92.92	0.04	92.76	0.00	N	0.00	92.92	0.04	N	0.00	93.02	0.14	Y	0.10	93.07	0.19	Y	0.15
CB31-32	93.07	93.27	0.20	92.97	0.00	N	0.00	93.14	0.07	N	0.00	93.26	0.19	N	0.00	93.31	0.24	Y	0.04
CB33-34	93.25	93.45	0.20	93.13	0.00	N	0.00	93.32	0.07	N	0.00	93.46	0.21	Y	0.01	93.50	0.25	Y	0.05
CB35-36	93.27	93.45	0.18	93.05	0.00	N	0.00	93.37	0.10	N	0.00	93.48	0.21	Y	0.03	93.51	0.24	Y	0.06
CB37-38	93.35	93.55	0.20	93.26	0.00	N	0.00	93.42	0.07	N	0.00	93.53	0.18	N	0.00	93.57	0.22	Y	0.02
Street B - Caivan Lands																			
CB39-40	93.17	93.45	0.28	92.87	0.00	N	0.00	93.19	0.02	N	0.00	93.25	0.08	N	0.00	93.27	0.10	N	0.00
CB41-42	93.44	93.69	0.25	93.11	0.00	N	0.00	93.46	0.02	N	0.00	93.51	0.07	N	0.00	93.55	0.11	N	0.00
CB43-44	93.40	93.69	0.29	92.89	0.00	N	0.00	93.49	0.09	N	0.00	93.74	0.34	Y	0.05	93.75	0.35	Y	0.06
CB45-46	93.41	93.62	0.21	93.14	0.00	N	0.00	93.46	0.05	N	0.00	93.58	0.17	N	0.00	93.62	0.21	N	0.00
CB47-48	93.47	93.62	0.15	92.92	0.00	N	0.00	93.50	0.03	N	0.00	93.59	0.12	N	0.00	93.63	0.16	Y	0.01
CB49-50	93.67	93.82	0.15	93.31	0.00	N	0.00	93.71	0.04	N	0.00	93.80	0.13	N	0.00	93.83	0.16	Y	0.01

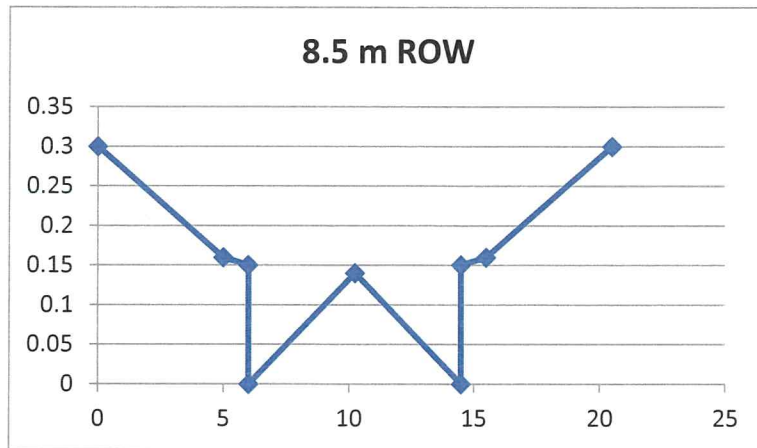
Burnett Lands - 3370 Greenbank Road

Roadway Cross-Sections



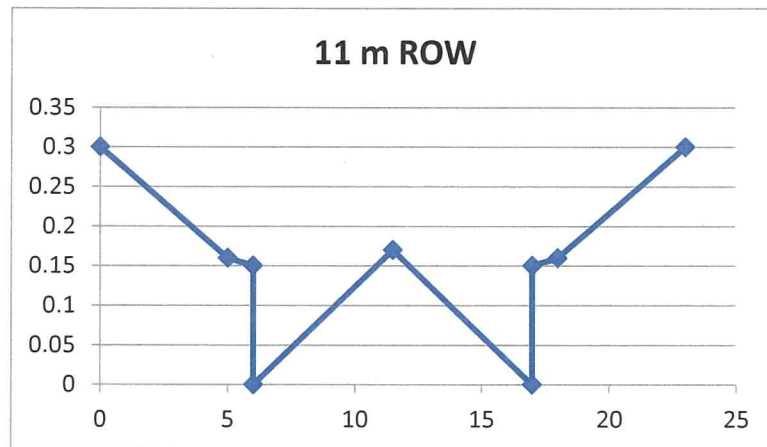
8.5m - ROW

0	0.3
5	0.16
6	0.15
6.01	0
10.25	0.14
14.49	0
14.5	0.15
15.5	0.16
20.5	0.3



11m - ROW

0	0.3
5	0.16
6	0.15
6.01	0
11.5	0.17
16.99	0
17	0.15
18	0.16
23	0.3



Burnett Lands - 3370 Greenbank Road
HGL Elevations

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elev - 5yr 4hr (100-yr Fixed) (m)	Design USF (m)	Clearance 5yr 4hr (100-yr Fixed) (m)	Clearance 100yr 4hr (5-yr Fixed) (m)	Clearance 100yr 4hr +20% (5-yr Fixed) (m)
Burnett Lands - Claridge							
200 (STM)	90.92	93.70	91.71	92.03	0.32	0.77	0.77
202 (STM)	90.81	93.39	91.71	92.02	0.31	0.76	0.76
204 (STM)	90.78	93.41	91.72	92.02	0.30	0.78	0.78
300 (STM)	90.38	93.50	91.72	92.02	0.30	0.94	0.92
302 (STM)	90.28	93.67	91.71	92.02	0.31	0.94	0.92
304 (STM)	90.05	93.73	91.68	92.01	0.33	0.95	0.93
306 (STM)	90.90	93.26	91.68	92.02	0.34	0.68	0.68
400 (STM)	90.50	93.51	91.79	92.34	0.55	1.14	1.12
402 (STM)	90.38	93.41	91.78	92.34	0.56	1.14	1.12
404 (STM)	90.31	93.58	91.78	92.33	0.55	1.14	1.11
406 (STM)	90.11	93.57	91.78	92.32	0.54	1.13	1.11
408 (STM)	90.94	93.01	91.70	92.00	0.30	0.94	0.93
410 (STM)	90.12	92.99	91.65	91.99	0.34	0.95	0.94
412 (STM)	89.26	92.65	91.61	92.02	0.41	1.02	1.01
500 (STM)	90.74	93.62	91.82	92.12	0.30	0.87	0.84
502 (STM)	90.57	93.49	91.82	92.12	0.30	0.87	0.84
504 (STM)	90.42	93.37	91.82	92.12	0.30	0.88	0.85
506 (STM)	90.43	93.50	91.80	92.12	0.32	0.90	0.87
508 (STM)	89.93	93.31	91.76	92.10	0.34	0.92	0.90
600 (STM)	90.85	93.50	91.78	92.35	0.57	1.16	1.13
602 (STM)	90.64	93.33	91.78	92.32	0.54	1.13	1.10
604 (STM)	90.26	93.45	91.78	92.32	0.54	1.13	1.11
606 (STM)	89.95	93.26	91.76	92.32	0.56	1.15	1.13
608 (STM)	89.58	93.08	91.71	92.08	0.37	0.96	0.94
700 (STM)	88.65	94.14	91.81	92.12	0.31	0.89	0.87
802 (STM)	89.92	93.82	91.61	-	-	-	-
804 (STM)	89.82	93.93	91.60	-	-	-	-
806 (STM)	89.28	93.34	91.50	-	-	-	-
808 (STM)	89.25	93.38	91.54	-	-	-	-
810 (STM)	89.27	93.40	91.32	-	-	-	-
900 (STM)	89.18	92.61	91.57	-	-	-	-
902 (STM)	89.00	93.09	91.44	-	-	-	-

Burnett Lands - 3370 Greenbank Road
HGL Elevations

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elev - 5yr 4hr (100-yr Fixed) (m)	Design USF (m)	Clearance 5yr 4hr (100-yr Fixed) (m)	Clearance 100yr 4hr (5-yr Fixed) (m)	Clearance 100yr 4hr +20% (5-yr Fixed) (m)
Street B - Caivan Lands							
100 (STM)	89.71	93.32	91.64	92.03	0.39	1.05	1.04
102 (STM)	89.86	93.37	91.67	92.03	0.36	1.00	1.00
104 (STM)	89.89	93.66	91.70	92.03	0.33	0.96	0.95
106 (STM)	90.14	93.75	91.73	-	-	-	-
108 (STM)	90.18	93.55	91.77	-	-	-	-
110 (STM)	90.31	93.71	91.79	-	-	-	-
112 (STM)	90.44	93.51	91.83	92.42	0.59	1.09	1.09
114 (STM)	90.62	93.66	91.89	92.49	0.60	1.04	1.04
116 (STM)	91.02	93.83	91.94	-	-	-	-
118 (STM)	90.99	94.51	91.94	-	-	-	-

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output



EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

111117 - Kennedy Burnett subdivision
Draft plan for subdivision, located north of the Jock River and west of Greenbank Road.
Second submission, after site plan change.

WARNING 03: negative offset ignored for Link 100-H2
 WARNING 03: negative offset ignored for Link 408-410
 WARNING 03: negative offset ignored for Link 410-412.1
 WARNING 03: negative offset ignored for Link 502-504
 WARNING 03: negative offset ignored for Link OR1
 WARNING 02: maximum depth increased for Node CB05-06
 WARNING 02: maximum depth increased for Node CB09-10
 WARNING 02: maximum depth increased for Node CB11-12
 WARNING 02: maximum depth increased for Node CB19-20
 WARNING 02: maximum depth increased for Node CB21-22
 WARNING 02: maximum depth increased for Node CB25-26
 WARNING 02: maximum depth increased for Node CB39-40
 WARNING 02: maximum depth increased for Node CB39-44
 WARNING 02: maximum depth increased for Node RYCB01
 WARNING 02: maximum depth increased for Node RYCB05
 WARNING 02: maximum depth increased for Node RYCB08
 WARNING 02: maximum depth increased for Node RYCB09
 WARNING 02: maximum depth increased for Node RYCB10
 WARNING 02: maximum depth increased for Node RYCB11
 WARNING 02: maximum depth increased for Node RYCB13
 WARNING 02: maximum depth increased for Node RYCB14
 WARNING 02: maximum depth increased for Node RYCB15

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 58
 Number of nodes 136
 Number of links 184
 Number of pollutants 0
 Number of land uses 0

 Rainage Summary

 Name Data Source Area Width %Imperv %Slope Rain Gage Outlet
 Rainage C5yr-4hr

Name	Data Source	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
F01		0.16	11.80	64.00	0.8500	Rainage	CB01-02
A02		0.11	24.21	64.00	1.5000	Rainage	RYCB02
A03		0.35	21.21	64.00	0.7500	Rainage	CB07-08
A04		0.26	14.10	64.00	0.7500	Rainage	CB03-04
A05		0.03	26.25	64.00	1.5000	Rainage	RYCB05
A06		0.29	21.72	64.00	0.7500	Rainage	CB05-06
A07		0.18	26.78	64.00	2.0000	Rainage	RYCB06
A08		0.22	15.22	64.00	0.7000	Rainage	CB19-20
A09		0.27	26.66	64.00	1.3000	Rainage	CB21-22
A10		0.11	13.25	64.00	0.8500	Rainage	CB23-24
A11		0.15	25.77	64.00	1.8000	Rainage	RYCB10
A12		0.28	22.02	64.00	0.5500	Rainage	CB09-10
A13		0.14	35.91	64.00	1.5000	Rainage	RYCB04
A14		0.16	18.98	64.00	0.7500	Rainage	CB11-12
A15		0.22	27.90	64.00	1.5000	Rainage	RYCB08
A16		0.20	18.14	64.00	0.7000	Rainage	CB13-14
A17		0.24	22.45	64.00	1.8500	Rainage	RYCB03
A18		0.14	22.45	64.00	0.7500	Rainage	CB17-18
A19		0.28	18.70	64.00	1.5000	Rainage	RYCB13
A20		0.20	26.95	64.00	1.0000	Rainage	CB37-38
A21		0.15	14.65	64.00	1.0000	Rainage	RYCB14
A22		0.47	51.97	64.00	1.0000	Rainage	A22-STOR
A23		0.07	14.83	64.00	1.5000	Rainage	RYCB15
A24		0.19	17.24	64.00	0.8500	Rainage	CB35-36

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
A25	JUNCTION	29.45	64.00	1.5000	Rainage
A26	JUNCTION	22.17	64.00	1.7000	Rainage
A27	JUNCTION	14.30	64.00	1.5000	Rainage
A28	JUNCTION	17.97	64.00	1.0000	Rainage
A29	JUNCTION	14.50	64.00	1.0000	Rainage
A30	JUNCTION	14.50	64.00	1.0000	Rainage
A31	JUNCTION	13.86	64.00	0.8000	Rainage
A32	JUNCTION	64.23	64.00	1.0000	Rainage
A33	JUNCTION	14.74	64.00	1.3000	Rainage
A34-35	JUNCTION	32.45	64.00	1.0360	Rainage
A36	JUNCTION	95.43	64.00	1.0000	Rainage
A37	JUNCTION	24.17	64.00	1.0000	Rainage
A38	JUNCTION	27.21	64.00	1.0000	Rainage
A39	JUNCTION	25.59	64.00	1.0000	Rainage
A40	JUNCTION	22.22	64.00	1.0000	Rainage
B01	JUNCTION	10.49	64.00	2.0000	Rainage
B02	JUNCTION	61.55	64.00	1.0000	Rainage
B03	JUNCTION	26.32	64.00	1.8000	Rainage
B04	JUNCTION	9.69	64.00	0.6000	Rainage
B05	JUNCTION	14.70	64.00	0.7500	Rainage
B06	JUNCTION	30.77	64.00	1.5000	Rainage
B07	JUNCTION	13.20	64.00	1.0000	Rainage
B08	JUNCTION	70.21	64.00	0.8500	Rainage
B09	JUNCTION	12.55	64.00	0.8500	Rainage
B10	JUNCTION	11.12	64.00	1.0000	Rainage
B11	JUNCTION	11.12	64.00	2.0000	Rainage
B12	JUNCTION	24.56	64.00	1.0000	Rainage
B13	JUNCTION	13.88	64.00	1.0000	Rainage
B14	JUNCTION	15.49	64.00	1.0000	Rainage
B15	JUNCTION	12.38	64.00	1.8000	Rainage
B16	JUNCTION	14.33	64.00	1.0000	Rainage
B17	JUNCTION	18.85	64.00	0.8500	Rainage
B18	JUNCTION	22.13	64.00	1.0000	Rainage
CAV-Lands	JUNCTION	200.00	64.00	1.0000	Rainage

 Node Summary

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output



Station	Structure	Material	Flow	Depth	Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
RYV0F01	RYV0B01	CONDUIT	0.0350	15.0	3.1816				
RYV0F02	RYV0B02	CONDUIT	0.0350	15.0	1.5802				
RYV0F03	RYV0B03	CONDUIT	0.0350	15.0	0.3857				
RYV0F04	RYV0B04	CONDUIT	0.0350	15.0	0.2468				
RYV0F05	RYV0B05	CONDUIT	0.0350	15.0	1.2468				
RYV0F06	RYV0B06	CONDUIT	0.0350	15.0	3.2484				
RYV0F07	RYV0B07	CONDUIT	0.0350	15.0	2.3807				
RYV0F08	RYV0B08	CONDUIT	0.0350	15.0	1.6336				
RYV0F09	RYV0B09	CONDUIT	0.0350	15.0	1.1801				
RYV0F10	RYV0B10	CONDUIT	0.0350	15.0	3.3152				
RYV0F11	RYV0B11	CONDUIT	0.0350	15.0	0.2800				
RYV0F12	RYV0B12	CONDUIT	0.0350	15.0	1.3134				
RYV0F13	RYV0B13	CONDUIT	0.0350	15.0	0.6000				
RYV0F14	RYV0B14	CONDUIT	0.0350	15.0	2.1805				
RYV0F15	RYV0B15	CONDUIT	0.0350	15.0	0.8467				
ST1-01	ST1-01	CONDUIT	0.0350	66.8	3.1874				
ST1-02	ST1-02	CONDUIT	0.0350	58.0	0.6000				
ST1-03	ST1-03	CONDUIT	0.0350	34.3	0.7229				
ST1-04	ST1-04	CONDUIT	0.0350	42.5	0.7368				
ST1-05	ST1-05	CONDUIT	0.0350	26.7	0.7163				
ST1-06	ST1-06	CONDUIT	0.0350	28.0	0.5281				
ST1-07	ST1-07	CONDUIT	0.0350	40.0	0.8705				
ST1-08	ST1-08	CONDUIT	0.0350	24.8	0.5930				
ST1-09	ST1-09	CONDUIT	0.0350	47.2	2.7469				
ST1-10	ST1-10	CONDUIT	0.0350	56.1	0.8147				
ST1-11	ST1-11	CONDUIT	0.0350	20.9	1.7084				
ST1-12	ST1-12	CONDUIT	0.0350	27.7	0.7225				
ST2-01	ST2-01	CONDUIT	0.0350	32.3	0.7949				
ST2-02	ST2-02	CONDUIT	0.0350	41.7	0.7120				
ST2-03	ST2-03	CONDUIT	0.0350	51.7	0.8061				
ST2-04	ST2-04	CONDUIT	0.0350	36.7	0.8373				
ST2-05	ST2-05	CONDUIT	0.0350	51.6	0.7881				
ST2-06	ST2-06	CONDUIT	0.0350	20.4	1.6070				
ST2-07	ST2-07	CONDUIT	0.0350	28.2	1.1241				
ST2-08	ST2-08	CONDUIT	0.0350	33.2	0.7736				
ST2-09	ST2-09	CONDUIT	0.0350	21.2	1.1939				
ST2-10	ST2-10	CONDUIT	0.0350	44.5	0.8822				
ST2-11	ST2-11	CONDUIT	0.0350	30.4	0.9792				
ST2-12	ST2-12	CONDUIT	0.0350	30.4	1.5269				
ST2-13	ST2-13	CONDUIT	0.0350	25.6	0.7579				
ST2-14	ST2-14	CONDUIT	0.0350	36.5	1.0589				
ST2-15	ST2-15	CONDUIT	0.0350	10.8	1.4498				
ST2-16	ST2-16	CONDUIT	0.0350	20.1	0.9803				
ST2-17	ST2-17	CONDUIT	0.0350	18.7	1.0538				
ST2-18	ST2-18	CONDUIT	0.0350	12.1	1.3012				
ST2-19	ST2-19	CONDUIT	0.0350	32.6	1.0354				
ST2-20	ST2-20	CONDUIT	0.0350	30.6	0.7070				
ST2-21	ST2-21	CONDUIT	0.0350	6.2	1.0699				
ST2-22	ST2-22	CONDUIT	0.0350	30.4	1.5364				
ST2-23	ST2-23	CONDUIT	0.0350	22.6	0.8712				
ST2-24	ST2-24	CONDUIT	0.0350	29.0	0.9557				
ST2-25	ST2-25	CONDUIT	0.0350	20.4	0.8663				
ST2-26	ST2-26	CONDUIT	0.0350	37.9	0.9952				
ST2-27	ST2-27	CONDUIT	0.0350	22.4	0.8803				
ST2-28	ST2-28	CONDUIT	0.0350	18.7	0.9266				
ST2-29	ST2-29	CONDUIT	0.0350	19.7	0.2249				
ST2-30	ST2-30	CONDUIT	0.0350	29.5	1.4750				
ST2-31	ST2-31	CONDUIT	0.0350	33.5	0.7307				
ST2-32	ST2-32	CONDUIT	0.0350	23.1	1.1456				
ST2-33	ST2-33	CONDUIT	0.0350	23.1	0.7937				
ST2-34	ST2-34	CONDUIT	0.0350	26.4	0.7903				
ST2-35	ST2-35	CONDUIT	0.0350	23.1	1.2412				
ST2-36	ST2-36	CONDUIT	0.0350	44.8	1.1459				
ST2-37	ST2-37	CONDUIT	0.0350	68.4	1.6273				
ST2-38	ST2-38	CONDUIT	0.0350	44.7	1.6515				
ST2-39	ST2-39	CONDUIT	0.0350	19.7	1.0044				
ST2-40	ST2-40	CONDUIT	0.0350						
ST2-41	ST2-41	CONDUIT	0.0350						
ST2-42	ST2-42	CONDUIT	0.0350						
ST2-43	ST2-43	CONDUIT	0.0350						
ST2-44	ST2-44	CONDUIT	0.0350						
ST2-45	ST2-45	CONDUIT	0.0350						
ST2-46	ST2-46	CONDUIT	0.0350						
ST2-47	ST2-47	CONDUIT	0.0350						
ST2-48	ST2-48	CONDUIT	0.0350						
ST2-49	ST2-49	CONDUIT	0.0350						
ST2-50	ST2-50	CONDUIT	0.0350						
ST2-51	ST2-51	CONDUIT	0.0350						
ST2-52	ST2-52	CONDUIT	0.0350						
ST2-53	ST2-53	CONDUIT	0.0350						
ST2-54	ST2-54	CONDUIT	0.0350						
ST2-55	ST2-55	CONDUIT	0.0350						
ST2-56	ST2-56	CONDUIT	0.0350						
ST2-57	ST2-57	CONDUIT	0.0350						
ST2-58	ST2-58	CONDUIT	0.0350						
ST2-59	ST2-59	CONDUIT	0.0350						
ST2-60	ST2-60	CONDUIT	0.0350						
ST2-61	ST2-61	CONDUIT	0.0350						
ST2-62	ST2-62	CONDUIT	0.0350						
ST2-63	ST2-63	CONDUIT	0.0350						
ST2-64	ST2-64	CONDUIT	0.0350						
ST2-65	ST2-65	CONDUIT	0.0350						
ST2-66	ST2-66	CONDUIT	0.0350						
ST2-67	ST2-67	CONDUIT	0.0350						
ST2-68	ST2-68	CONDUIT	0.0350						
ST2-69	ST2-69	CONDUIT	0.0350						
ST2-70	ST2-70	CONDUIT	0.0350						
ST2-71	ST2-71	CONDUIT	0.0350						
ST2-72	ST2-72	CONDUIT	0.0350						
ST2-73	ST2-73	CONDUIT	0.0350						
ST2-74	ST2-74	CONDUIT	0.0350						
ST2-75	ST2-75	CONDUIT	0.0350						
ST2-76	ST2-76	CONDUIT	0.0350						
ST2-77	ST2-77	CONDUIT	0.0350						
ST2-78	ST2-78	CONDUIT	0.0350						
ST2-79	ST2-79	CONDUIT	0.0350						
ST2-80	ST2-80	CONDUIT	0.0350						
ST2-81	ST2-81	CONDUIT	0.0350						
ST2-82	ST2-82	CONDUIT	0.0350						
ST2-83	ST2-83	CONDUIT	0.0350						
ST2-84	ST2-84	CONDUIT	0.0350						
ST2-85	ST2-85	CONDUIT	0.0350						
ST2-86	ST2-86	CONDUIT	0.0350						
ST2-87	ST2-87	CONDUIT	0.0350						
ST2-88	ST2-88	CONDUIT	0.0350						
ST2-89	ST2-89	CONDUIT	0.0350						
ST2-90	ST2-90	CONDUIT	0.0350						
ST2-91	ST2-91	CONDUIT	0.0350						
ST2-92	ST2-92	CONDUIT	0.0350						
ST2-93	ST2-93	CONDUIT	0.0350						
ST2-94	ST2-94	CONDUIT	0.0350						
ST2-95	ST2-95	CONDUIT	0.0350						
ST2-96	ST2-96	CONDUIT	0.0350						
ST2-97	ST2-97	CONDUIT	0.0350						
ST2-98	ST2-98	CONDUIT	0.0350						
ST2-99	ST2-99	CONDUIT	0.0350						
ST2-100	ST2-100	CONDUIT	0.0350						
ST2-101	ST2-101	CONDUIT	0.0350						
ST2-102	ST2-102	CONDUIT	0.0350						
ST2-103	ST2-103	CONDUIT	0.0350						
ST2-104	ST2-104	CONDUIT	0.0350						
ST2-105	ST2-105	CONDUIT	0.0350						
ST2-106	ST2-106	CONDUIT	0.0350						
ST2-107	ST2-107	CONDUIT	0.0350						
ST2-108	ST2-108	CONDUIT	0.0350						
ST2-109	ST2-109	CONDUIT	0.0350						
ST2-110	ST2-110	CONDUIT	0.0350						
ST2-111	ST2-111	CONDUIT	0.0350						
ST2-112	ST2-112	CONDUIT	0.0350						
ST2-113	ST2-113	CONDUIT	0.0350						
ST2-114	ST2-114	CONDUIT	0.0350						
ST2-115	ST2-115	CONDUIT	0.0350						
ST2-116	ST2-116	CONDUIT	0.0350						
ST2-117	ST2-117	CONDUIT	0.0350						
ST2-118	ST2-118	CONDUIT	0.0350						
ST2-119	ST2-119	CONDUIT	0.0350						
ST2-120	ST2-120	CONDUIT	0.0350						
ST2-121	ST2-121	CONDUIT	0.0350						
ST2-122	ST2-122	CONDUIT	0.0350						
ST2-123	ST2-123	CONDUIT	0.0350						
ST2-124	ST2-124	CONDUIT	0.0350						
ST2-125	ST2-125	CONDUIT	0.0350						
ST2-126	ST2-126	CONDUIT	0.0350						
ST2-127	ST2-127	CONDUIT	0.0350						
ST2-128	ST2-128	CONDUIT	0.0350						
ST2-129	ST2-129	CONDUIT	0.0350						
ST2-130	ST2-130	CONDUIT	0.0350						
ST2-131	ST2-131	CONDUIT	0.0350						
ST2-132	ST2-132	CONDUIT	0.0350						
ST2-133	ST2-133	CONDUIT	0.0350						
ST2-134	ST2-134	CONDUIT	0.0350						
ST2-135	ST2-135	CONDUIT	0.0350						
ST2-136	ST2-136	CONDUIT	0.0350						
ST2-137	ST2-137	CONDUIT	0.0350						
ST2-138	ST2-138	CONDUIT	0.0350						
ST2-139									

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output



0.1244 0.1451 0.1658 0.1866 0.2073
 0.2286 0.2491 0.2695 0.2902 0.3109
 0.3317 0.3524 0.3731 0.3938 0.4146
 0.4354 0.4561 0.4768 0.4976 0.5183
 0.5391 0.5598 0.5806 0.6013 0.6221
 0.6428 0.6635 0.6843 0.7050 0.7258
 0.7465 0.7673 0.7880 0.8088 0.8295
 0.8503 0.8710 0.8918 0.9126 0.9333
 0.9541 0.9748 0.9956 1.0164 1.0371
 1.0579 1.0787 1.0994 1.1202 1.1410

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

```

*****
Analysis Options
Flow Units ..... LPS
Process Model .....
Rainfall/Runoff ..... YES
ROII ..... NO
Snowmelt ..... NO
Groundwater ..... NO
Flow Routing ..... YES
Ponding Allowed ..... NO
Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Starting Date ..... 01/02/2018 00:00:00
Ending Date ..... 01/03/2018 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:01:00
Dry Time Step ..... 00:01:00
Routing Time Step ..... 2.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 8
Head Tolerance ..... 0.001500 m
    
```

```

*****
Runoff Quantity Continuity
*****
Initial LID Storage ..... 0.012
Total Precipitation ..... 1.043
Evaporation Loss ..... 0.000
Infiltration Loss ..... 0.326
Surface Runoff ..... 0.710
Final Storage ..... 0.012
Continuity Error (%) ..... 0.665
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000
Wet Weather Inflow ..... 0.710
Groundwater Inflow ..... 0.000
ROII Inflow ..... 0.000
External Inflow ..... 0.000
External Outflow ..... 0.710
Flooding Loss ..... 0.000
Evaporation Loss ..... 0.000
Infiltration Loss ..... 0.000
Initial Stored Volume ..... 0.112
Final Stored Volume ..... 1.145
Continuity Error (%) ..... -0.326
    
```

```

*****
Highest Continuity Errors
*****
Node 014217 (99.97%)
Node 806 (STM) (4.31%)
Node 808 (STM) (-3.38%)
Node 285 (STM) (-1.85%)
Node 810 (STM) (-1.12%)
    
```

```

*****
Time-Step Critical Elements
*****
Link 202-204 (L.52%)
*****
*****
Highest Flow Instability Indices
*****
Link ORYCB03 (106)
Link 110-108 (99)
Link 104-102 (92)
Link OCB21-22 (91)
Link OCB23-24 (91)
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step ..... 0.50 sec
Average Time Step ..... 1.99 sec
Maximum Time Step ..... 2.00 sec
Percent in Steady State ..... 0.00
Average Iterations per Step ..... 6.26
Percent Not Converging ..... 49.07
*****
Subcatchment Runoff Summary
*****
    
```

Runoff Coeff	Subcatchment	Total Precip		Total Runoff		Total Evap		Total Infil		Total Runoff		Total Peak	
		mm	mm	mm	mm	mm	mm	mm	mm	mm	mm	mm	mm
0.701	A01	45.16	0.00	0.00	0.00	0.00	13.51	31.66	0.05	29.70	0.05	29.70	
0.640	A02	45.16	0.00	0.00	0.00	0.00	12.28	28.93	0.03	19.63	0.03	19.63	
0.695	A03	45.16	0.00	0.00	0.00	0.00	13.77	31.40	0.11	61.87	0.11	61.87	
0.693	A04	45.16	0.00	0.00	0.00	0.00	13.88	31.29	0.08	44.59	0.08	44.59	
0.641	A05	45.16	0.00	0.00	0.00	0.00	11.76	28.94	0.01	5.93	0.01	5.93	
0.701	A06	45.16	0.00	0.00	0.00	0.00	13.53	31.64	0.09	52.18	0.09	52.18	
0.640	A07	45.16	0.00	0.00	0.00	0.00	12.46	28.92	0.05	32.92	0.05	32.92	
0.698	A08	45.16	0.00	0.00	0.00	0.00	13.65	31.52	0.07	39.00	0.07	39.00	
0.713	A09	45.16	0.00	0.00	0.00	0.00	12.97	32.21	0.09	51.23	0.09	51.23	
0.640	A10	45.16	0.00	0.00	0.00	0.00	12.99	32.19	0.03	20.85	0.03	20.85	
0.640	A11	45.16	0.00	0.00	0.00	0.00	12.40	28.93	0.04	27.20	0.04	27.20	
0.688	A12	45.16	0.00	0.00	0.00	0.00	13.64	31.53	0.09	49.47	0.09	49.47	
0.641	A13	45.16	0.00	0.00	0.00	0.00	12.23	28.93	0.04	26.48	0.04	26.48	
0.711	A14	45.16	0.00	0.00	0.00	0.00	13.08	32.10	0.05	30.78	0.05	30.78	
0.640	A15	45.16	0.00	0.00	0.00	0.00	12.69	28.92	0.06	39.65	0.06	39.65	
0.704	A16	45.16	0.00	0.00	0.00	0.00	13.38	31.80	0.06	37.03	0.06	37.03	
0.641	A17	45.16	0.00	0.00	0.00	0.00	12.18	28.93	0.04	25.00	0.04	25.00	
0.718	A18	45.16	0.00	0.00	0.00	0.00	13.16	32.02	0.07	41.54	0.07	41.54	
0.687	A19	45.16	0.00	0.00	0.00	0.00	13.68	31.49	0.09	50.78	0.09	50.78	
0.640	A20	45.16	0.00	0.00	0.00	0.00	12.64	28.92	0.06	36.00	0.06	36.00	

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output



Node	45-16	0-00	13-15	32-03	0-05	29-23	Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
A21	45-16	0-00	13-15	32-03	0-05	29-23	01+217	JUNCTION	0.00	0.00	93.79	0 01:38	0.00
A22	45-16	0-00	13-01	32-16	0-15	91-24	01+292	JUNCTION	0.00	0.00	93.93	0 00:00	0.00
A23	45-16	0-00	12-33	28-93	0-02	12-96	01+370	JUNCTION	0.00	0.00	93.79	0 00:00	0.00
A24	45-16	0-00	13-25	31-92	0-06	34-74	01+449	JUNCTION	0.00	0.00	93.62	0 00:00	0.00
A25	45-16	0-00	12-48	28-92	0-05	32-18	01+521	JUNCTION	0.00	0.00	93.82	0 00:00	0.00
A26	45-16	0-00	12-92	32-25	0-08	46-44	01+586	JUNCTION	0.00	0.00	94.97	0 00:00	0.00
A27	45-16	0-00	12-36	28-93	0-02	13-14	03+000	JUNCTION	0.00	0.00	93.45	0 01:32	0.00
A28	45-16	0-00	13-17	32-01	0-06	36-38	03+048	JUNCTION	0.00	0.00	93.65	0 00:00	0.00
A29	45-16	0-00	12-62	32-56	0-10	63-73	03+118	JUNCTION	0.00	0.00	93.69	0 00:00	0.00
A30	45-16	0-00	13-74	31-43	0-08	47-53	03+214	JUNCTION	0.00	0.00	93.58	0 00:00	0.00
A31	45-16	0-00	12-83	32-35	0-15	93-45	04+069	JUNCTION	0.00	0.00	93.56	0 00:00	0.00
A32	45-16	0-00	12-96	32-22	0-05	28-06	04+118	JUNCTION	0.00	0.00	93.52	0 00:00	0.00
A33	45-16	0-00	13-69	31-48	0-19	104-57	04+213	JUNCTION	0.00	0.00	93.12	0 00:00	0.00
A34-35	45-16	0-00	12-95	32-23	0-27	163-76	04+264	JUNCTION	0.00	0.00	92.89	0 00:00	0.00
A36	45-16	0-00	14-27	30-90	0-23	117-41	04+305	JUNCTION	0.00	0.00	92.89	0 00:00	0.00
A37	45-16	0-00	14-16	31-01	0-23	122-04	04+357	JUNCTION	0.00	0.03	92.95	0 01:32	0.03
A38	45-16	0-00	13-49	31-68	0-12	68-69	04+393	JUNCTION	0.00	0.00	93.55	0 00:00	0.00
A39	45-16	0-00	13-41	31-76	0-10	55-87	05+067	JUNCTION	0.00	0.00	93.45	0 00:00	0.00
A40	45-16	0-00	13-01	32-17	0-04	25-99	05+112	JUNCTION	0.00	0.00	93.77	0 00:00	0.00
A41	45-16	0-00	13-39	31-78	0-26	152-30	05+172	JUNCTION	0.00	0.00	93.50	0 00:00	0.00
A42	45-16	0-00	12-12	28-93	0-03	17-59	06+075	JUNCTION	0.00	0.00	93.50	0 00:00	0.00
A43	45-16	0-00	13-72	31-45	0-07	44-27	06+132	JUNCTION	0.00	0.00	93.43	0 00:00	0.00
A44	45-16	0-00	13-69	31-48	0-04	40-21	06+193	JUNCTION	0.00	0.03	93.42	0 01:30	0.03
A45	45-16	0-00	12-55	28-92	0-06	36-96	06+300	JUNCTION	0.00	0.00	93.99	0 00:00	0.00
A46	45-16	0-00	13-54	31-63	0-06	37-02	07+067	JUNCTION	0.00	0.00	93.99	0 00:00	0.00
A47	45-16	0-00	37-16	8-01	0-09	28-11	100 (STM)	JUNCTION	1.87	1.93	91.64	0 01:28	1.93
A48	45-16	0-00	12-38	32-81	0-03	18-76	102 (STM)	JUNCTION	1.73	1.81	91.67	0 01:27	1.81
A49	45-16	0-00	13-59	31-59	0-06	33-57	104 (STM)	JUNCTION	1.69	1.83	91.72	0 00:02	1.81
A50	45-16	0-00	12-97	32-21	0-03	18-75	106 (STM)	JUNCTION	1.45	1.65	91.79	0 00:02	1.60
A51	45-16	0-00	12-22	28-93	0-03	20-56	108 (STM)	JUNCTION	1.40	1.58	91.76	0 01:29	1.58
A52	45-16	0-00	12-97	32-21	0-04	23-22	110 (STM)	JUNCTION	1.28	1.49	91.79	0 01:29	1.49
A53	45-16	0-00	12-98	32-20	0-04	26-28	112 (STM)	JUNCTION	1.14	1.39	91.83	0 01:29	1.39
A54	45-16	0-00	11-89	28-94	0-01	4-82	114 (STM)	JUNCTION	0.97	1.27	91.89	0 01:29	1.27
A55	45-16	0-00	12-94	32-24	0-04	23-46	116 (STM)	JUNCTION	0.57	0.92	91.94	0 01:30	0.91
A56	45-16	0-00	13-80	31-37	0-11	59-85	118 (STM)	JUNCTION	0.60	0.95	91.94	0 01:30	0.95
A57	45-16	0-00	12-49	32-70	0-03	21-92	120 (STM)	JUNCTION	0.39	0.79	91.71	0 01:27	0.78
A58	45-16	0-00	12-38	32-80	2-71	1288-70	122 (STM)	JUNCTION	0.37	0.79	91.71	0 01:27	0.78
CalvanLands	45-16	0-00	12-38	32-80	2-71	1288-70	124 (STM)	JUNCTION	0.36	0.81	91.72	0 01:27	0.80
*****							126 (STM)	JUNCTION	0.51	0.91	91.72	0 01:27	0.90
*****							204 (STM)	JUNCTION	2.20	2.50	91.79	0 01:29	2.50
*****							282 (STM)	JUNCTION	2.20	2.50	91.79	0 01:29	2.50
*****							283 (STM)	JUNCTION	2.53	2.74	91.79	0 01:29	2.74
*****							284 (STM)	JUNCTION	2.47	2.79	91.90	0 00:02	2.63
*****							285 (STM)	JUNCTION	2.52	2.64	91.71	0 01:27	2.64
*****							286 (STM)	JUNCTION	2.55	2.65	91.68	0 01:27	2.65
*****							287 (STM)	JUNCTION	2.66	2.96	91.88	0 01:31	2.96
*****							288 (STM)	JUNCTION	2.42	2.81	91.97	0 00:02	2.62
*****							300 (STM)	JUNCTION	0.91	1.33	91.71	0 01:28	1.32
*****							302 (STM)	JUNCTION	1.01	1.43	91.71	0 01:28	1.42
*****							304 (STM)	JUNCTION	1.24	1.63	91.68	0 01:29	1.62
*****							306 (STM)	JUNCTION	0.39	0.78	91.68	0 01:28	0.78
*****							400 (STM)	JUNCTION	0.79	1.28	91.78	0 01:29	1.28
*****							402 (STM)	JUNCTION	0.91	1.40	91.78	0 01:29	1.40
*****							404 (STM)	JUNCTION	0.98	1.47	91.78	0 01:29	1.47
*****							406 (STM)	JUNCTION	1.18	1.66	91.77	0 01:29	1.66
*****							408 (STM)	JUNCTION	0.34	0.75	91.00	0 01:26	0.70
*****							410a (STM)	JUNCTION	1.33	1.56	91.65	0 01:29	1.50
*****							412 (STM)	JUNCTION	2.03	2.35	91.61	0 01:29	2.35
*****							500 (STM)	JUNCTION	0.55	1.09	91.83	0 01:29	1.08
*****							502 (STM)	JUNCTION	0.72	1.26	91.83	0 01:29	1.25
*****							504 (STM)	JUNCTION	0.87	1.40	91.82	0 01:29	1.39
*****							506 (STM)	JUNCTION	0.86	1.37	91.80	0 01:29	1.37
*****							506a (STM)	JUNCTION	0.94	1.43	91.78	0 01:29	1.42
*****							508 (STM)	JUNCTION	1.36	1.83	91.76	0 01:29	1.83
*****							508a (STM)	JUNCTION	1.14	1.58	91.72	0 01:29	1.57
*****							600 (STM)	JUNCTION	0.44	0.93	91.78	0 01:29	0.93
*****							602 (STM)	JUNCTION	0.65	1.14	91.78	0 01:29	1.13
*****							604 (STM)	JUNCTION	1.03	1.52	91.78	0 01:29	1.51
*****							604a (STM)	JUNCTION	0.82	1.30	91.77	0 01:29	1.29
*****							606 (STM)	JUNCTION	1.34	1.81	91.76	0 01:29	1.81
*****							606a (STM)	JUNCTION	1.12	1.57	91.74	0 01:29	1.57
*****							800 (STM)	JUNCTION	1.21	1.73	91.71	0 01:29	1.73
*****							802 (STM)	JUNCTION	1.37	1.91	91.81	0 01:29	1.91
*****							804 (STM)	JUNCTION	1.37	1.69	91.61	0 01:30	1.69
*****							806 (STM)	JUNCTION	1.46	1.78	91.60	0 01:30	1.78
*****							808 (STM)	JUNCTION	2.00	2-24	91.52	0 19:05	2-06
*****							810 (STM)	JUNCTION	2.03	2-28	91.53	0 04:55	2-07
*****							810	JUNCTION	2.01	2-06	91.33	0 01:26	2-05

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output



Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Occurrence hrs:min	Lateral Inflow Volume 10 ⁶ ltr	Total Inflow Volume 10 ⁶ ltr	Flow Balance Error Percent
900 (STM)	JUNCTION	2.11	2.39	91:57	0	0.1:29	2.38
902 (STM)	JUNCTION	2.39	2.45	91:44	0	0.1:29	2.44
CB03-01	JUNCTION	0.04	1.66	93:35	0	0.1:32	1.66
CB03-04	JUNCTION	0.05	1.68	93:35	0	0.1:32	1.66
CB05-06	JUNCTION	0.04	1.69	93:26	0	0.1:32	1.69
CB07-08	JUNCTION	0.06	1.73	93:46	0	0.1:37	1.73
CB09-10	JUNCTION	0.04	1.68	93:34	0	0.1:32	1.68
CB11-12	JUNCTION	0.04	1.70	93:34	0	0.1:31	1.70
CB13-14	JUNCTION	0.04	1.67	93:37	0	0.1:32	1.67
CB15-16	JUNCTION	0.04	1.69	93:41	0	0.1:31	1.69
CB17-18	JUNCTION	0.05	1.68	93:18	0	0.1:33	1.68
CB19-20	JUNCTION	0.05	1.69	93:02	0	0.1:34	1.69
CB21-22	JUNCTION	0.19	1.75	92:88	0	0.1:32	1.75
CB23-24	JUNCTION	0.22	1.68	92:77	0	0.1:31	1.68
CB25-26	JUNCTION	0.18	1.70	92:95	0	0.1:32	1.70
CB27-28	JUNCTION	0.07	1.70	92:88	0	0.1:32	1.70
CB-283	JUNCTION	0.02	1.28	93:88	0	0.1:30	1.27
CB-284	JUNCTION	0.03	1.41	94:01	0	0.1:30	1.41
CB-285	JUNCTION	0.03	1.42	94:02	0	0.1:31	1.42
CB-286	JUNCTION	0.03	1.41	94:01	0	0.1:31	1.41
CB-287	JUNCTION	0.03	1.41	94:01	0	0.1:30	1.41
CB-288	JUNCTION	0.02	1.28	92:98	0	0.1:30	1.27
CB-289	JUNCTION	0.02	1.28	92:98	0	0.1:31	1.27
CB29-30	JUNCTION	0.05	1.66	92:84	0	0.1:34	1.66
CB31-32	JUNCTION	0.04	1.67	93:14	0	0.1:31	1.67
CB33-34	JUNCTION	0.04	1.67	93:32	0	0.1:31	1.67
CB35-36	JUNCTION	0.04	1.70	93:37	0	0.1:31	1.70
CB37-38	JUNCTION	0.04	1.67	93:42	0	0.1:31	1.67
CB39-40	JUNCTION	0.05	1.62	93:19	0	0.1:31	1.62
CB41-42	JUNCTION	0.04	1.62	93:46	0	0.1:31	1.62
CB43-44	JUNCTION	0.06	1.69	93:49	0	0.1:33	1.69
CB45-46	JUNCTION	0.04	1.65	93:46	0	0.1:31	1.65
CB47-48	JUNCTION	0.03	1.63	93:50	0	0.1:31	1.63
CB49-50	JUNCTION	0.03	1.64	93:71	0	0.1:31	1.64
RYCB01	JUNCTION	0.00	0.14	92:14	0	0.1:30	0.14
RYCB02	JUNCTION	0.02	1.52	93:55	0	0.1:30	1.52
RYCB03	JUNCTION	0.10	1.85	93:35	0	0.1:30	1.85
RYCB04	JUNCTION	0.03	1.76	93:52	0	0.1:28	1.76
RYCB05	JUNCTION	0.01	1.19	92:15	0	0.1:30	0.19
RYCB06	JUNCTION	0.11	2.39	93:50	0	0.1:29	2.19
RYCB07	JUNCTION	0.04	2.09	93:60	0	0.1:29	2.28
RYCB08	JUNCTION	0.05	2.29	93:60	0	0.1:29	2.28
RYCB09	JUNCTION	0.03	2.08	93:57	0	0.1:28	2.08
RYCB10	JUNCTION	0.23	2.18	93:26	0	0.1:29	2.18
RYCB11	JUNCTION	0.02	1.48	93:10	0	0.1:30	1.47
RYCB12	JUNCTION	0.04	2.15	93:57	0	0.1:29	2.15
RYCB13	JUNCTION	0.88	3.09	93:53	0	0.1:30	3.09
RYCB14	JUNCTION	0.01	0.92	92:43	0	0.1:30	0.92
RYCB15	JUNCTION	0.01	0.93	92:46	0	0.1:30	0.93
GRBK-OUT	OUTFALL	0.00	0.00	0:00	0	0.0:00	0.00
HW-01	OUTFALL	2.08	2.08	91:28	0	0.0:00	2.08
HW-02	OUTFALL	1.68	1.68	91:58	0	0.0:00	1.68
OVF-OUT	OUTFALL	0.00	0.00	92:78	0	0.0:00	0.00
A22-STOR	STORAGE	0.03	1.42	93:92	0	0.1:30	1.42
A29-STOR	STORAGE	0.03	1.43	93:93	0	0.1:30	1.42
A32-STOR	STORAGE	0.03	1.42	93:42	0	0.1:30	1.42
A34-35-STOR	STORAGE	0.03	1.40	94:40	0	0.1:30	1.40
B06-STOR	STORAGE	0.03	1.42	94:12	0	0.1:30	1.42
B07-STOR	STORAGE	0.03	1.41	94:16	0	0.1:30	1.41
CAV-STOR	STORAGE	0.06	1.44	93:54	0	0.1:32	1.44

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Occurrence hrs:min	Lateral Inflow Volume 10 ⁶ ltr	Total Inflow Volume 10 ⁶ ltr	Flow Balance Error Percent
01+217	JUNCTION	0.00	0.07	0 01:31	0	2.28e+005	22.809
01+292	JUNCTION	0.00	0.00	0 00:00	0	0	0.000
01+370	JUNCTION	0.00	0.00	0 00:00	0	0	0.000
01+449	JUNCTION	0.00	0.00	0 00:00	0	0	0.000
01+521	JUNCTION	0.00	0.00	0 00:00	0	0	0.000
ltr							

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output



606a (STM)	0.00	229.63	0	01:31	0	0.585	-0.036	106 (STM)	JUNCTION	24.00	0.337	1.965
700 (STM)	0.00	581.63	0	01:32	0	0.000	-0.000	108 (STM)	JUNCTION	24.00	0.267	1.785
800 (STM)	0.00	33.57	0	01:32	0	0.082	-0.000	110 (STM)	JUNCTION	24.00	0.253	1.919
804 (STM)	0.00	103.20	0	01:30	0	0.193	-0.031	114 (STM)	JUNCTION	24.00	0.268	1.680
806 (STM)	0.00	261.69	0	01:30	0	0.478	0.363	116 (STM)	JUNCTION	0.12	0.084	1.770
808 (STM)	0.00	1196.94	0	01:29	0	4.47	4.499	118 (STM)	JUNCTION	0.08	0.045	2.670
810 (STM)	0.00	1198.08	0	01:29	0	4.51	4.268	120 (STM)	JUNCTION	0.12	0.045	2.670
900 (STM)	0.00	458.96	0	01:29	0	1.18	-1.109	122 (STM)	JUNCTION	24.00	0.284	0.547
902 (STM)	0.00	1196.90	0	01:29	0	2.27	-0.599	124 (STM)	JUNCTION	24.00	0.487	0.277
CB01-02	0.00	936.17	0	01:29	0	2.81	0.228	126 (STM)	JUNCTION	24.00	0.344	0.423
CB03-04	0.00	29.70	0	01:30	0	0.0516	-0.059	128 (STM)	JUNCTION	24.00	0.355	0.418
CB05-06	44.59	44.59	0	01:30	0	0.0807	0.095	130 (STM)	JUNCTION	24.00	0.507	0.264
CB07-08	52.18	52.18	0	01:30	0	0.0908	-0.053	132 (STM)	JUNCTION	1.40	0.419	1.791
CB09-10	61.87	61.87	0	01:30	0	0.111	0.004	134 (STM)	JUNCTION	0.10	0.067	1.963
CB11-12	45.47	55.90	0	01:30	0	0.087	-0.037	136 (STM)	JUNCTION	0.12	0.078	2.052
CB13-14	30.78	49.58	0	01:30	0	0.0888	-0.208	138 (STM)	JUNCTION	0.39	0.375	1.725
CB15-16	37.03	37.03	0	01:30	0	0.0636	-0.066	140 (STM)	JUNCTION	0.99	0.484	1.626
CB17-18	41.54	45.81	0	01:30	0	0.0701	0.044	142 (STM)	JUNCTION	24.00	0.559	1.797
CB19-20	50.78	55.39	0	01:30	0	0.0897	0.0918	144 (STM)	JUNCTION	0.14	0.142	1.314
CB21-22	31.39	31.39	0	01:30	0	0.0687	0.0708	146 (STM)	JUNCTION	0.44	0.297	1.343
CB23-24	51.23	61.19	0	01:30	0	0.0348	-0.124	148 (STM)	JUNCTION	24.00	0.806	0.798
CB25-26	20.85	28.19	0	01:30	0	0.0435	-0.034	150 (STM)	JUNCTION	24.00	0.811	1.793
CB27-28	46.27	46.27	0	01:30	0	0.0467	-0.034	152 (STM)	JUNCTION	0.39	0.471	1.664
CB-283	18.76	18.76	0	01:30	0	0.0841	0.057	154 (STM)	JUNCTION	24.00	0.571	1.552
CB-284	23.22	23.22	0	01:30	0	0.0295	-0.235	156 (STM)	JUNCTION	24.00	0.642	1.552
CB-285	26.28	26.28	0	01:30	0	0.0387	-0.050	158 (STM)	JUNCTION	24.00	0.687	1.596
CB-286	23.46	23.46	0	01:30	0	0.0438	-0.079	160 (STM)	JUNCTION	24.00	0.741	1.547
CB-287	21.92	21.92	0	01:30	0	0.039	-0.370	162 (STM)	JUNCTION	24.00	0.767	1.547
CB-288	18.75	18.75	0	01:30	0	0.0335	-0.079	164 (STM)	JUNCTION	24.00	0.816	1.424
CB29-30	47.53	52.33	0	01:30	0	0.0312	0.18	166 (STM)	JUNCTION	0.35	0.320	1.720
CB31-32	36.38	36.38	0	01:30	0	0.0846	0.088	168 (STM)	JUNCTION	24.00	0.519	1.551
CB33-34	46.44	46.44	0	01:30	0	0.0615	0.048	170 (STM)	JUNCTION	24.00	0.596	1.674
CB35-36	34.74	46.72	0	01:30	0	0.0771	-0.001	172 (STM)	JUNCTION	24.00	0.686	1.577
CB37-38	29.23	29.23	0	01:30	0	0.0591	-0.224	174 (STM)	JUNCTION	24.00	0.743	1.501
CB39-40	59.85	59.93	0	01:30	0	0.0493	0.026	176 (STM)	JUNCTION	24.00	0.809	1.429
CB41-42	33.57	33.57	0	01:30	0	0.109	0.276	178 (STM)	JUNCTION	24.00	0.837	1.371
CB43-44	20.11	31.0	0	01:30	0	0.0587	-0.082	180 (STM)	JUNCTION	0.47	0.465	2.334
CB45-46	40.27	40.27	0	01:30	0	0.135	-0.062	182 (STM)	JUNCTION	24.00	0.784	2.206
CB47-48	24.27	24.27	0	01:30	0	0.0431	-0.032	184 (STM)	JUNCTION	24.00	0.818	1.820
CB49-50	25.99	25.99	0	01:30	0	0.0434	-0.006	186 (STM)	JUNCTION	24.00	1.021	2.188
RYCB01	4.82	4.82	0	01:30	0	0.00752	-0.011	188 (STM)	JUNCTION	24.00	1.065	1.849
RYCB02	19.63	19.63	0	01:30	0	0.0307	0.007	190 (STM)	JUNCTION	24.00	0.837	2.074
RYCB03	20.56	20.56	0	01:30	0	0.0321	-1.237	192 (STM)	JUNCTION	24.00	0.870	1.041
RYCB04	26.48	26.48	0	01:30	0	0.0414	-0.241	194 (STM)	JUNCTION	24.00	0.875	1.646
RYCB05	5.93	5.93	0	01:30	0	0.00926	-0.009	196 (STM)	JUNCTION	24.00	0.875	1.646
RYCB06	32.92	32.92	0	01:30	0	0.0515	0.079	198 (STM)	JUNCTION	24.00	0.875	1.646
RYCB07	36.96	36.96	0	01:30	0	0.0578	0.220	200 (STM)	JUNCTION	24.00	0.875	1.646
RYCB08	39.65	39.65	0	01:30	0	0.0622	0.344	202 (STM)	JUNCTION	24.00	0.875	1.646
RYCB09	25.00	25.00	0	01:30	0	0.039	0.018	204 (STM)	JUNCTION	24.00	0.875	1.646
RYCB10	27.20	27.20	0	01:30	0	0.0425	0.156	206 (STM)	JUNCTION	24.00	0.875	1.646
RYCB11	17.59	17.63	0	01:30	0	0.0425	0.0492	208 (STM)	JUNCTION	24.00	0.875	1.646
RYCB12	32.18	32.18	0	01:30	0	0.0275	0.007	210 (STM)	JUNCTION	24.00	0.875	1.646
RYCB13	36.00	36.00	0	01:30	0	0.0503	0.321	212 (STM)	JUNCTION	24.00	0.875	1.646
RYCB14	13.14	13.14	0	01:30	0	0.0564	-0.047	214 (STM)	JUNCTION	24.00	0.875	1.646
RYCB15	12.96	12.96	0	01:30	0	0.0205	0.008	216 (STM)	JUNCTION	24.00	0.875	1.646
GRBK-OUT	364.00	364.00	0	01:30	0	0.0202	0.008	218 (STM)	JUNCTION	24.00	0.875	1.646
HR-01	194.00	194.00	0	01:30	0	0.1673	0.000	220 (STM)	JUNCTION	24.00	0.875	1.646
HR-02	0.00	1631.15	0	01:31	0	3.7	0.000	222 (STM)	JUNCTION	24.00	0.875	1.646
OUTFALL	0.00	1631.15	0	01:31	0	3.8	0.000	224 (STM)	JUNCTION	24.00	0.875	1.646
OUTFALL	0.00	0.00	0	00:00	0	0.000	0.000	226 (STM)	JUNCTION	24.00	0.875	1.646
OUTFALL	0.00	0.00	0	01:30	0	0.000	0.000	228 (STM)	JUNCTION	24.00	0.875	1.646
A22-STOR	91.24	91.24	0	01:30	0	0.152	0.007	230 (STM)	JUNCTION	24.00	0.875	1.646
A29-STOR	63.73	63.73	0	01:30	0	0.103	0.007	232 (STM)	JUNCTION	24.00	0.875	1.646
A32-STOR	93.45	93.45	0	01:30	0	0.154	0.008	234 (STM)	JUNCTION	24.00	0.875	1.646
A34-35-STOR	104.57	104.57	0	01:30	0	0.185	0.008	236 (STM)	JUNCTION	24.00	0.875	1.646
A36-STOR	163.76	163.76	0	01:30	0	0.272	0.008	238 (STM)	JUNCTION	24.00	0.875	1.646
B02-STOR	152.30	152.30	0	01:30	0	0.262	0.007	240 (STM)	JUNCTION	24.00	0.875	1.646
CAV-STOR	1288.70	1288.70	0	01:30	0	2.71	0.008	242 (STM)	JUNCTION	24.00	0.875	1.646

Node Surgecharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours	Max. Height Above Crown	Min. Depth Below Rim
			Meters	Meters
102 (STM)	JUNCTION	24.00	0.386	1.701
104 (STM)	JUNCTION	24.00	0.402	1.940

Node Loading Summary

Storage Unit	Average Volume 1000 m3	Avg Full	Exp Full	Exfil Loss	Max Full	Max Occurrence days hr:min	Maximum Volume 1000 m3	Max Full	Total Volume 10^6 ltr
A22-STOR	0.000	0	0	0	0.001	0 01:30	2	88.22	
A29-STOR	0.000	0	0	0	0.001	0 01:30	3	60.71	
A32-STOR	0.000	0	0	0	0.001	0 01:30	2	90.19	
A34-35-STOR	0.000	0	0	0	0.001	0 01:30	1	103.20	
A36-STOR	0.000	0	0	0	0.001	0 01:30	1	158.44	
B02-STOR	0.000	0	0	0	0.001	0 01:30	1	149.32	
CAV-STOR	0.000	0	0	0	0.021	0 01:32	2	1125.86	

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Outfall Node	Flow Pcnt	Avg Flow LFS	Max Flow LFS	Total Volume 10^6 ltr
GRBK-OUT	45.45	19.90	364.00	0.673

Burnett Lands - 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output

HR-01 99.95 43.61 1198.14 3.371
 HR-02 96.11 52.22 1631.15 3.605
 OVF-OUT 0.10 0.00 0.00 0.000
 System 60.53 115.72 3186.08 7.849

 Link Flow Summary

Link	Type	Maximum Flow LFS	Time of Max Occurrence days hr:min	Maximum Vel oc m/sec		Max/ Full Flow Depth
				Full	Max/ Flow	
100-H2	CONDUIT	1631.15	0 01:31	0.86	0.57	1.00
102-100	CONDUIT	424.26	0 01:30	0.63	0.65	1.00
104-102	CONDUIT	487.00	0 01:30	0.65	0.71	1.00
106-104	CONDUIT	400.95	0 01:30	0.73	0.71	1.00
108-106	CONDUIT	382.67	0 01:30	0.69	0.68	1.00
110-108	CONDUIT	393.91	0 01:31	0.63	0.62	1.00
112-110	CONDUIT	303.91	0 01:31	0.57	0.57	1.00
114-112	CONDUIT	235.70	0 01:31	0.52	0.45	1.00
116-114	CONDUIT	207.63	0 01:30	0.57	0.54	0.98
118-116	CONDUIT	31.45	0 01:31	0.10	0.08	0.89
200-202	CONDUIT	35.05	0 01:23	0.27	0.12	0.88
202-204	CONDUIT	46.25	0 01:33	0.34	0.12	1.00
204-302b	CONDUIT	48.05	0 01:33	0.29	0.17	1.00
282-114	CONDUIT	10.74	0 01:26	0.07	0.06	1.00
283-110	CONDUIT	17.41	0 01:30	0.11	0.09	1.00
284-106	CONDUIT	18.65	0 01:27	0.26	0.27	1.00
285-104	CONDUIT	18.56	0 01:36	0.26	0.27	1.00
286-102	CONDUIT	18.32	0 01:27	0.26	0.27	1.00
287-100	CONDUIT	1144.12	0 01:32	1.33	0.73	0.69
288-108	CONDUIT	17.38	0 01:30	0.25	0.25	1.00
300-302	CONDUIT	38.73	0 01:30	0.33	0.12	1.00
302-304	CONDUIT	114.39	0 01:33	0.39	0.40	1.00
304-410	CONDUIT	180.09	0 01:33	0.49	0.46	1.00
400-202	CONDUIT	34.58	0 01:33	0.22	0.13	0.89
402-404	CONDUIT	25.68	0 01:31	0.09	0.08	1.00
404-406	CONDUIT	76.58	0 01:31	0.26	0.27	1.00
406-606	CONDUIT	108.20	0 01:31	0.29	0.32	1.00
408-410	CONDUIT	32.81	0 01:25	0.12	0.09	1.00
410-412_1	CONDUIT	235.71	0 01:32	0.52	0.48	1.00
410-412_2	CONDUIT	270.24	0 01:32	0.59	0.68	1.00
412-900	CONDUIT	846.11	0 01:29	0.72	0.53	1.00
500-502	CONDUIT	19.98	0 01:36	0.07	0.05	1.00
502-504	CONDUIT	107.91	0 01:30	0.29	0.27	1.00
504-506	CONDUIT	166.09	0 01:30	0.45	0.40	1.00
506-508_1	CONDUIT	189.70	0 01:30	0.54	0.49	1.00
506-508_2	CONDUIT	222.11	0 01:30	0.60	0.55	1.00
508-608_1	CONDUIT	295.60	0 01:30	0.65	0.58	1.00
508-608_2	CONDUIT	324.61	0 01:30	0.71	0.63	1.00
600-602	CONDUIT	35.50	0 01:43	0.12	0.07	1.00
602-604	CONDUIT	53.75	0 01:25	0.18	0.19	1.00
604-606_1	CONDUIT	107.32	0 01:34	0.27	0.28	1.00
604-606_2	CONDUIT	107.32	0 01:34	0.27	0.28	1.00
606-608_1	CONDUIT	208.74	0 01:31	0.46	0.40	1.00
606-608_2	CONDUIT	229.64	0 01:31	0.50	0.44	1.00
608-412	CONDUIT	581.66	0 01:30	0.75	0.67	1.00
700-506	CONDUIT	34.13	0 01:32	0.12	0.07	1.00
802-804	CONDUIT	103.25	0 01:30	0.35	0.35	1.00
804-902	CONDUIT	261.75	0 01:30	0.90	0.78	1.00
808-HW1	CONDUIT	1198.14	0 01:29	0.64	0.49	1.00
900-902	CONDUIT	936.21	0 01:29	0.80	0.70	1.00
902-806	CONDUIT	1196.94	0 01:29	1.03	0.81	1.00
C1	CHANNEL	0.01	0 01:30	0.02	0.00	0.01
C2	CHANNEL	0.11	0 01:31	0.01	0.00	0.04
C3	CHANNEL	0.06	0 01:31	0.01	0.00	0.02
C4	CHANNEL	0.01	0 01:31	0.01	0.00	0.03
C5	CHANNEL	0.00	0 00:00	0.00	0.00	0.02
C6	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
C7	CONDUIT	459.06	0 01:29	0.39	0.23	1.00
COF-OUT	CONDUIT	458.96	0 01:29	0.39	0.23	1.00
RYOVF01	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RYOVF02	CONDUIT	0.00	0 00:00	0.00	0.00	0.03
RYOVF03	CONDUIT	0.00	0 00:00	0.00	0.00	0.09
RYOVF04	CONDUIT	6.56	0 01:29	0.38	0.03	0.26
RYOVF05	CONDUIT	0.00	0 00:00	0.00	0.00	0.13



Engineers, Planners & Landscape Architects

RYOVF06	CONDUIT	13.00	0 01:29	0.61	0.03	0.37
RYOVF07	CONDUIT	15.20	0 01:30	0.59	0.05	0.31
RYOVF08	CONDUIT	18.82	0 01:30	0.56	0.07	0.36
RYOVF09	CONDUIT	4.60	0 01:29	0.32	0.02	0.24
RYOVF10	CONDUIT	7.38	0 01:29	0.54	0.02	0.23
RYOVF11	CONDUIT	11.92	0 01:31	0.07	0.00	0.06
RYOVF12	CONDUIT	11.92	0 01:31	0.07	0.00	0.06
RYOVF13	CONDUIT	15.13	0 01:30	0.31	0.05	0.32
RYOVF14	CONDUIT	0.00	0 00:00	0.00	0.00	0.12
RYOVF15	CONDUIT	0.00	0 00:00	0.00	0.00	0.17
ST1-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.03
ST1-02	CHANNEL	0.00	0 01:36	0.00	0.00	0.03
ST1-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.02
ST1-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.02
ST1-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
ST1-06	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
ST1-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.07
ST1-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.07
ST1-09	CHANNEL	0.00	0 00:00	0.00	0.00	0.04
ST1-10	CHANNEL	0.00	0 00:00	0.00	0.00	0.04
ST1-11	CHANNEL	0.00	0 00:00	0.00	0.00	0.05
ST1-12	CHANNEL	0.00	0 01:38	0.00	0.00	0.05
ST2-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.18
ST2-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.18
ST3-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.08
ST3-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.08
ST3-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.08
ST3-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST3-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST3-06	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
ST3-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
ST4-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.09
ST4-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.09
ST4-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
ST4-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
ST4-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST4-06	CHANNEL	4.88	0 01:30	0.09	0.00	0.17
ST4-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.14
ST4-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.21
ST4-09	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST4-10	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST4-11	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST4-12	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST4-13	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
ST4-14	CHANNEL	0.00	0 00:00	0.00	0.00	0.14
ST4-15	CHANNEL	4.82	0 01:30	0.07	0.00	0.18
ST5-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.09
ST5-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.09
ST5-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.14
ST5-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.10
ST5-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.10
ST5-06	CHANNEL	0.00	0 00:00	0.00	0.00	0.08
ST5-07	CHANNEL	2.18	0 01:32	0.04	0.00	0.13
ST6-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
ST6-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
ST6-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.16
ST6-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.16
ST6-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.14
ST6-06	CHANNEL	4.70	0 01:30	0.11	0.00	0.14
ST6-07	CHANNEL	4.52	0 01:30	0.14	0.00	0.12
ST6-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.08
ST7-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.10
ST7-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.10
OCB01-02	ORIFICE	19.80	0 01:32	0.00	0.00	1.00
OCB03-04	ORIFICE	25.56	0 01:35	0.00	0.00	1.00
OCB05-06	ORIFICE	33.71	0 01:32	0.00	0.00	1.00
OCB07-08	ORIFICE	30.46	0 01:37	0.00	0.00	1.00
OCB09-10	ORIFICE	33.69	0 01:32	0.00	0.00	1.00
OCB11-12	ORIFICE	25.73	0 01:32	0.00	0.00	1.00
OCB13-14	ORIFICE	30.10	0 01:31	0.00	0.00	1.00
OCB15-16	ORIFICE	33.49	0 01:39	0.00	0.00	1.00
OCB17-18	ORIFICE	25.08	0 01:45	0.00	0.00	1.00
OCB19-20	ORIFICE	31.69	0 01:47	0.00	0.00	1.00
OCB21-22	ORIFICE	18.05	0 01:36	0.00	0.00	1.00
OCB23-24	ORIFICE	18.05	0 01:41	0.00	0.00	1.00
OCB25-26	ORIFICE	31.51	0 01:41	0.00	0.00	1.00
OCB27-28	ORIFICE	17.34	0 01:30	0.00	0.00	1.00
OCB28A	ORIFICE	18.24	0 01:31	0.00	0.00	1.00
OCB28B	ORIFICE	18.29	0 01:31	0.00	0.00	1.00
OCB28C	ORIFICE	18.23	0 01:31	0.00	0.00	1.00

**Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output**

ST5-06	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST5-07	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST5-08	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-01	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-02	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-03	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-04	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-05	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-06	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-07	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST6-08	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST7-01	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST7-02	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Conduit Surcharge Summary

Conduit	Hours Full		Hours Full		Hours Full		Hours Full	
	Both	Ends	Upstream	Downstream	Abv. Normal	Full Normal	Flow	Exceeded
100-H2	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
102-100	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
104-102	24.00	24.00	24.00	24.00	0.01	0.01	0.12	0.12
106-104	24.00	24.00	24.00	24.00	0.01	0.01	0.16	0.16
108-106	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
110-108	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
112-110	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
114-112	0.13	0.13	0.13	0.13	0.01	0.01	0.01	0.01
116-114	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
202-204	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
204-302b	0.03	0.03	0.03	0.10	0.01	0.01	0.01	0.01
282-114	0.08	0.08	0.08	0.13	0.01	0.01	0.01	0.01
283-110	24.00	24.00	24.00	24.00	0.01	0.01	0.48	0.48
283-106	24.00	24.00	24.00	24.00	0.01	0.01	0.13	0.13
285-104	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
286-102	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
288-108	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
300-302	1.39	1.39	1.39	24.00	0.01	0.01	0.01	0.01
302-304	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
304-410	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
306-304	0.01	0.01	0.01	0.12	0.01	0.01	0.01	0.01
400-402	0.39	0.39	0.98	0.01	0.01	0.01	0.01	0.01
402-404	1.53	1.53	24.00	0.01	0.01	0.01	0.01	0.01
404-406	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
406-606	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
408-410	0.14	0.14	0.44	0.01	0.01	0.01	0.01	0.01
410-412.1	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
410-412.2	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
412-900	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
500-502	0.39	0.39	24.00	0.01	0.01	0.01	0.01	0.01
502-504	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
504-506	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
506-508.1	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
508-608.1	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
508-608.2	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
600-602	0.35	0.35	24.00	0.01	0.01	0.01	0.01	0.01
602-604	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
604-606.1	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
604-606.2	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
606-608.1	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
606-608.2	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
608-412	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
700-506	0.47	0.47	24.00	0.01	0.01	0.01	0.01	0.01
802-804	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
804-502	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
808-HW1	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
900-502	24.00	24.00	24.00	24.00	0.01	0.01	0.01	0.01
902-806	24.00	24.00	24.00	24.00	0.01	0.01	3.77	3.77
C7	24.00	24.00	24.00	24.00	0.01	0.01	1.89	1.89
C8	24.00	24.00	24.00	24.00	0.01	0.01	5.63	5.63

Analysis begun on: Tue Jan 16 10:07:24 2018
Analysis ended on: Tue Jan 16 10:07:37 2018
Total elapsed time: 00:00:13

Burnett Lands - 3370 Greenbank Road 100-year Storm, 5-year Fixed Outlet Elevations Model Output



EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.0.12)

111117 - Kennedy Burnett subdivision
Draft plan for subdivision, located north of the Joek River and west of Greenbank Road.
Second submission, after site plan change.

WARNING 03: negative offset ignored for Link 100-02
WARNING 03: negative offset ignored for Link 408-410
WARNING 03: negative offset ignored for Link 410-412.1
WARNING 03: negative offset ignored for Link 502-504.1
WARNING 03: negative offset ignored for Link ORI
WARNING 02: maximum depth increased for Node CB05-06
WARNING 02: maximum depth increased for Node CB09-10
WARNING 02: maximum depth increased for Node CB11-12
WARNING 02: maximum depth increased for Node CB19-20
WARNING 02: maximum depth increased for Node CB21-22
WARNING 02: maximum depth increased for Node CB25-26
WARNING 02: maximum depth increased for Node CB39-40
WARNING 02: maximum depth increased for Node CB43-44
WARNING 02: maximum depth increased for Node RYCB01
WARNING 02: maximum depth increased for Node RYCB05
WARNING 02: maximum depth increased for Node RYCB06
WARNING 02: maximum depth increased for Node RYCB08
WARNING 02: maximum depth increased for Node RYCB09
WARNING 02: maximum depth increased for Node RYCB11
WARNING 02: maximum depth increased for Node RYCB13
WARNING 02: maximum depth increased for Node RYCB14
WARNING 02: maximum depth increased for Node RYCB15

Element Count

Number of rain gages 1
Number of subcatchments ... 58
Number of nodes 136
Number of links 184
Number of pollutants 0
Number of land uses 0

Node Summary

Name	Data Source	Area	Width	Imperv	Slope	Rain Gage	Outlet
Raingage	CI00yr-4hr				INTENSITY	10 min.	
A01		0.16	11.80	64.00	0.8500	Raingage	CB01-02
A02		0.11	24.21	64.00	1.5000	Raingage	RYCB02
A03		0.35	21.21	64.00	0.7500	Raingage	CB07-08
A04		0.26	24.10	64.00	1.7500	Raingage	CB03-04
A05		0.26	26.95	64.00	1.5000	Raingage	RYCB05
A06		0.28	26.72	64.00	1.5000	Raingage	RYCB06
A07		0.18	26.78	64.00	2.0000	Raingage	RYCB06
A08		0.22	15.22	64.00	0.7000	Raingage	CB19-20
A09		0.27	26.66	64.00	1.3000	Raingage	CB21-22
A10		0.11	13.25	64.00	0.8500	Raingage	CB23-24
A11		0.15	25.77	64.00	1.8000	Raingage	RYCB10
A12		0.28	22.02	64.00	0.5500	Raingage	CB09-10
A13		0.14	35.91	64.00	1.5000	Raingage	RYCB04
A14		0.16	18.88	64.00	1.5000	Raingage	CB11-12
A15		0.22	27.90	64.00	1.5000	Raingage	RYCB08
A16		0.20	18.14	64.00	0.7000	Raingage	CB13-14
A17		0.14	36.79	64.00	0.8500	Raingage	RYCB09
A18		0.22	22.45	64.00	0.8500	Raingage	CB15-16
A19		0.28	18.70	64.00	0.7500	Raingage	CB17-18
A20		0.20	26.95	64.00	1.5000	Raingage	RYCB13
A21		0.15	14.65	64.00	1.0000	Raingage	CB37-38
A22		0.27	11.87	64.00	1.5000	Raingage	RYCB07
A23		0.17	11.83	64.00	1.5000	Raingage	RYCB07
A24		0.09	17.24	64.00	0.8500	Raingage	CB35-36
A25		0.17	29.45	64.00	1.5000	Raingage	RYCB12

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
01+217	JUNCTION	93.79	0.35	0.0	0.0
01+292	JUNCTION	93.69	0.35	0.0	0.0
01+370	JUNCTION	93.79	0.35	0.0	0.0
01+449	JUNCTION	93.62	0.35	0.0	0.0
01+521	JUNCTION	93.82	0.35	0.0	0.0
01+600	JUNCTION	94.77	0.35	0.0	0.0
03+000	JUNCTION	93.45	0.35	0.0	0.0
03+048	JUNCTION	93.65	0.35	0.0	0.0
03+118	JUNCTION	93.69	0.35	0.0	0.0
03+214	JUNCTION	93.58	0.35	0.0	0.0
04+069	JUNCTION	93.56	0.35	0.0	0.0
04+118	JUNCTION	93.52	0.35	0.0	0.0
04+213	JUNCTION	93.12	0.35	0.0	0.0
04+264	JUNCTION	92.89	0.35	0.0	0.0
04+305	JUNCTION	92.89	0.35	0.0	0.0
04+357	JUNCTION	93.07	0.35	0.0	0.0
04+393	JUNCTION	92.92	0.35	0.0	0.0
05+067	JUNCTION	93.55	0.35	0.0	0.0
05+122	JUNCTION	93.45	0.35	0.0	0.0
05+177	JUNCTION	93.27	0.35	0.0	0.0
06+075	JUNCTION	93.50	0.35	0.0	0.0
06+132	JUNCTION	93.43	0.35	0.0	0.0
06+206	JUNCTION	93.39	0.35	0.0	0.0
06+300	JUNCTION	93.08	0.35	0.0	0.0
07+067	JUNCTION	93.99	0.35	0.0	0.0
100 (STM)	JUNCTION	89.71	3.61	0.0	0.0
102 (STM)	JUNCTION	89.86	3.51	0.0	0.0
104 (STM)	JUNCTION	89.89	3.77	0.0	0.0
106 (STM)	JUNCTION	90.14	3.62	0.0	0.0
108 (STM)	JUNCTION	90.18	3.37	0.0	0.0
110 (STM)	JUNCTION	90.31	3.40	0.0	0.0
112 (STM)	JUNCTION	90.44	3.07	0.0	0.0
114 (STM)	JUNCTION	90.62	3.04	0.0	0.0
116 (STM)	JUNCTION	91.02	2.81	0.0	0.0
118 (STM)	JUNCTION	90.99	3.52	0.0	0.0
200 (STM)	JUNCTION	90.92	2.78	0.0	0.0
202 (STM)	JUNCTION	90.81	2.58	0.0	0.0
204 (STM)	JUNCTION	90.78	2.63	0.0	0.0
206 (STM)	JUNCTION	89.59	3.10	0.0	0.0
282 (STM)	JUNCTION	89.82	3.06	0.0	0.0
284 (STM)	JUNCTION	89.12	3.06	0.0	0.0
285 (STM)	JUNCTION	89.06	3.07	0.0	0.0

**Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output**

Station	Shape	0.61	0.29	0.15	0.61	534.15	11m-ROW	0.35	4.26	0.22	23.00	1	4690.09
600-602	CIRCULAR	0.61	0.29	0.15	0.61	534.15	11m-ROW	0.35	4.26	0.22	23.00	1	4690.09
602-604	CIRCULAR	0.61	0.29	0.15	0.61	286.51	11m-ROW	0.35	4.26	0.22	23.00	1	3768.79
604-606_1	CIRCULAR	0.61	0.29	0.15	0.61	287.09	11m-ROW	0.35	4.26	0.22	23.00	1	3895.53
604-606_2	CIRCULAR	0.61	0.29	0.15	0.61	288.59	11m-ROW	0.35	4.26	0.22	23.00	1	4881.87
606-608_1	CIRCULAR	0.76	0.46	0.19	0.76	526.60	11m-ROW	0.35	4.26	0.22	23.00	1	4690.72
608-412	CIRCULAR	0.99	0.76	0.35	0.99	525.42	11m-ROW	0.35	4.26	0.22	23.00	1	5589.75
700-506	CIRCULAR	0.61	0.29	0.15	0.61	474.94	8.5m-ROW	0.35	3.72	0.22	20.50	1	4930.15
802-804	CIRCULAR	0.61	0.29	0.15	0.61	292.37	8.5m-ROW	0.35	3.72	0.22	20.50	1	3844.89
804-902	CIRCULAR	0.61	0.29	0.15	0.61	333.66							
808-HW1	HORIZ ELLIPSE	1.22	1.17	0.30	1.22	1453.79							
900-902	CIRCULAR	1.22	1.17	0.30	1.22	1338.22							
502-806	CIRCULAR	1.22	1.17	0.30	1.22	1469.98							
C1	8.5m-ROW	0.35	3.72	0.22	0.35	1422.59							
C2	8.5m-ROW	0.35	3.72	0.22	0.35	1747.58							
C3	8.5m-ROW	0.35	3.72	0.22	0.35	879.04							
C4	8.5m-ROW	0.35	3.72	0.22	0.35	1432.90							
C5	8.5m-ROW	0.35	3.72	0.22	0.35	1068.02							
C6	8.5m-ROW	1.22	1.17	0.30	1.22	2032.89							
C7	8.5m-ROW	1.22	1.17	0.30	1.22	1818.27							
C8	8.5m-ROW	1.22	1.17	0.30	1.22	134.32							
OVF-00T	TRIANGULAR	0.30	0.27	0.14	0.30	375.08							
RYOVF01	TRIANGULAR	0.30	0.27	0.14	0.30	264.33							
RYOVF02	TRIANGULAR	0.30	0.27	0.14	0.30	130.76							
RYOVF03	TRIANGULAR	0.30	0.27	0.14	0.30	233.54							
RYOVF04	TRIANGULAR	0.30	0.27	0.14	0.30	378.99							
RYOVF05	TRIANGULAR	0.30	0.27	0.14	0.30	324.45							
RYOVF06	TRIANGULAR	0.30	0.27	0.14	0.30	268.76							
RYOVF07	TRIANGULAR	0.30	0.27	0.14	0.30	228.43							
RYOVF08	TRIANGULAR	0.30	0.27	0.14	0.30	382.87							
RYOVF09	TRIANGULAR	0.30	0.27	0.14	0.30	111.27							
RYOVF10	TRIANGULAR	0.30	0.27	0.14	0.30	240.99							
RYOVF11	TRIANGULAR	0.30	0.27	0.14	0.30	162.88							
RYOVF12	TRIANGULAR	0.30	0.27	0.14	0.30	310.51							
RYOVF13	TRIANGULAR	0.30	0.27	0.14	0.30	183.49							
RYOVF14	TRIANGULAR	0.30	0.27	0.14	0.30	6849.24							
RYOVF15	TRIANGULAR	0.30	0.27	0.14	0.30	2971.71							
ST1-01	8.5m-ROW	0.35	3.72	0.22	0.35	3261.97							
ST1-02	8.5m-ROW	0.35	3.72	0.22	0.35	4655.23							
ST1-03	8.5m-ROW	0.35	3.72	0.22	0.35	3293.17							
ST1-04	8.5m-ROW	0.35	3.72	0.22	0.35	3377.63							
ST1-05	8.5m-ROW	0.35	3.72	0.22	0.35	2787.97							
ST1-06	8.5m-ROW	0.35	3.72	0.22	0.35	3579.35							
ST1-07	8.5m-ROW	0.35	3.72	0.22	0.35	2954.22							
ST1-08	8.5m-ROW	0.35	3.72	0.22	0.35	6358.39							
ST1-09	8.5m-ROW	0.35	3.72	0.22	0.35	3462.79							
ST1-10	8.5m-ROW	0.35	3.72	0.22	0.35	5014.41							
ST1-11	8.5m-ROW	0.35	3.72	0.22	0.35	3261.05							
ST1-12	8.5m-ROW	0.35	3.72	0.22	0.35	3420.41							
ST2-01	8.5m-ROW	0.35	3.72	0.22	0.35	3237.16							
ST2-02	8.5m-ROW	0.35	3.72	0.22	0.35	3444.54							
ST2-03	8.5m-ROW	0.35	3.72	0.22	0.35	3510.43							
ST2-04	8.5m-ROW	0.35	3.72	0.22	0.35	3405.88							
ST2-05	8.5m-ROW	0.35	3.72	0.22	0.35	4863.36							
ST2-06	8.5m-ROW	0.35	3.72	0.22	0.35	4067.53							
ST2-07	8.5m-ROW	0.35	3.72	0.22	0.35	3374.38							
ST2-08	8.5m-ROW	0.35	3.72	0.22	0.35	4059.91							
ST2-09	8.5m-ROW	0.35	3.72	0.22	0.35	2633.24							
ST2-10	8.5m-ROW	0.35	3.72	0.22	0.35	4740.66							
ST2-11	8.5m-ROW	0.35	3.72	0.22	0.35	3339.82							
ST2-12	8.5m-ROW	0.35	3.72	0.22	0.35	3947.76							
ST2-13	8.5m-ROW	0.35	3.72	0.22	0.35	4619.40							
ST2-14	8.5m-ROW	0.35	3.72	0.22	0.35	3788.44							
ST2-15	8.5m-ROW	0.35	3.72	0.22	0.35	3938.20							
ST2-16	8.5m-ROW	0.35	3.72	0.22	0.35	4376.16							
ST2-17	8.5m-ROW	0.35	3.72	0.22	0.35	3903.75							
ST2-18	8.5m-ROW	0.35	3.72	0.22	0.35	3225.82							
ST2-19	8.5m-ROW	0.35	3.72	0.22	0.35	3968.30							
ST2-20	8.5m-ROW	0.35	3.72	0.22	0.35	4755.26							
ST2-21	8.5m-ROW	0.35	3.72	0.22	0.35	3580.80							
ST2-22	8.5m-ROW	0.35	3.72	0.22	0.35	3750.51							
ST2-23	8.5m-ROW	0.35	3.72	0.22	0.35	3570.81							
ST2-24	8.5m-ROW	0.35	3.72	0.22	0.35	3827.20							
ST2-25	8.5m-ROW	0.35	3.72	0.22	0.35	3897.65							
ST2-26	8.5m-ROW	0.35	3.72	0.22	0.35	3897.65							
ST2-27	8.5m-ROW	0.35	3.72	0.22	0.35	1839.68							
ST2-28	8.5m-ROW	0.35	3.72	0.22	0.35	5321.74							
ST2-29	11m-ROW	0.35	4.26	0.22	0.35	3745.58							
ST2-30	11m-ROW	0.35	4.26	0.22	0.35	3745.58							

**Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output**

0.9281 0.9461 0.9820 1.0000

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LFS
Process Models:
Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed NO
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Starting Date 01/02/2018 00:00:00
Ending Date 01/03/2018 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Routing Time Step 2.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 4
Head Tolerance 0.001500 m

Runoff Quantity Continuity

Initial LID Storage 0.012
Total Precipitation 1.755
Evaporation Loss 0.000
Infiltration Loss 0.401
Surface Runoff 1.331
Final Storage 0.012
Continuity Error (%) 1.269

Flow Routing Continuity

Dry Weather Inflow 0.000
Wet Weather Inflow 1.331
Groundwater Inflow 0.000
RDII Inflow 0.000
External Inflow 0.000
External Outflow 1.341
Flooding Loss 0.000
Evaporation Loss 0.000
Exfiltration Loss 0.000
Initial Stored Volume 0.070
Final Stored Volume 0.747
Continuity Error (%) -0.986

Highest Continuity Errors

Node 01+217 (53.75%)
Node 03+000 (47.87%)
Node 01+292 (6.97%)
Node CB39-40 (1.54%)
Node 808 (STM) (-1.51%)

Time-Step Critical Elements

None

Highest Flow Instability Indexes

Link ORYCB13 (14)
Link W1 (2)
Link C8 (1)
Link C7 (1)
Link 902-806 (1)

Routing Time Step Summary

Minimum Time Step : 0.50 sec
Average Time Step : 1.99 sec
Maximum Time Step : 2.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 2.09
Percent Not Converging : 1.18

Subcatchment Runoff Summary

Coeff	Subcatchment	Total		Total		Total		Total		Total		Peak	
		Precip	Runoff	Runoff	Evap	Runoff	Runoff	Runoff	Runoff	Runoff	Runoff	Runoff	LFS
		mm	mm	mm	mm	mm	mm	mm	mm	mm	mm	mm	10 ⁶ ltr
A01		76.00	0.00	0.00	0.00	16.58	59.45	0.10	55.95				
0.782		76.00	0.00	0.00	0.00	15.04	48.69	0.05	33.65				
0.641		76.00	0.00	0.00	0.00	17.02	59.01	0.21	116.37				
0.776		76.00	0.00	0.00	0.00	17.21	58.82	0.15	83.93				
0.774		76.00	0.00	0.00	0.00	14.60	48.72	0.02	10.16				
0.641		76.00	0.00	0.00	0.00	16.61	59.42	0.17	98.26				
A06		76.00	0.00	0.00	0.00	15.22	48.68	0.09	56.49				
0.782		76.00	0.00	0.00	0.00	16.81	59.22	0.13	73.35				
0.641		76.00	0.00	0.00	0.00	15.81	60.23	0.16	98.44				
0.779		76.00	0.00	0.00	0.00	15.83	60.21	0.07	40.04				
0.792		76.00	0.00	0.00	0.00	15.15	48.69	0.07	46.66				
A10		76.00	0.00	0.00	0.00	16.79	59.24	0.16	93.05				
A11		76.00	0.00	0.00	0.00	14.99	48.69	0.07	45.39				
A12		76.00	0.00	0.00	0.00	15.96	60.08	0.10	58.79				
0.779		76.00	0.00	0.00	0.00	15.47	48.68	0.10	68.18				
A13		76.00	0.00	0.00	0.00	16.37	59.66	0.12	69.96				
A14		76.00	0.00	0.00	0.00	14.95	48.69	0.07	42.85				
A15		76.00	0.00	0.00	0.00	16.06	59.98	0.13	79.08				
0.640		76.00	0.00	0.00	0.00	16.86	59.17	0.17	95.49				
0.785		76.00	0.00	0.00	0.00	15.41	48.68	0.09	61.85				
A17		76.00	0.00	0.00	0.00	16.05	59.99	0.09	55.66				
0.641		76.00	0.00	0.00	0.00	15.87	60.17	0.29	174.91				
A18		76.00	0.00	0.00	0.00	15.09	48.69	0.03	22.22				
0.789		76.00	0.00	0.00	0.00	16.19	59.84	0.11	65.89				
A19		76.00	0.00	0.00	0.00	15.24	48.68	0.08	55.22				
A20		76.00	0.00	0.00	0.00	16.86	59.17	0.17	95.49				
A21		76.00	0.00	0.00	0.00	15.41	48.68	0.09	61.85				
A22		76.00	0.00	0.00	0.00	16.05	59.99	0.09	55.66				
A23		76.00	0.00	0.00	0.00	15.87	60.17	0.29	174.91				
A24		76.00	0.00	0.00	0.00	15.09	48.69	0.03	22.22				
A25		76.00	0.00	0.00	0.00	16.19	59.84	0.11	65.89				
0.641		76.00	0.00	0.00	0.00	15.24	48.68	0.08	55.22				

Burnett Lands – 3370 Greenbank Road 100-year Storm, 5-year Fixed Outlet Elevations Model Output

Node	76.00	0.00	0.00	15.75	60.29	0.14	89.48	03+000	0.00	0.01	93.46	0	01:55	0.01
A26	76.00	0.00	0.00	15.75	60.29	0.14	89.48	JUNCTION	0.00	0.01	93.46	0	01:55	0.01
0.793	76.00	0.00	0.00	15.12	48.69	0.03	22.54	JUNCTION	0.00	0.00	93.65	0	00:00	0.00
A27	76.00	0.00	0.00	16.07	59.97	0.12	69.23	JUNCTION	0.00	0.00	93.69	0	00:00	0.00
0.641	76.00	0.00	0.00	16.96	60.66	0.16	89.38	JUNCTION	0.00	0.00	93.58	0	00:00	0.00
0.786	76.00	0.00	0.00	17.26	58.76	0.16	87.12	JUNCTION	0.00	0.00	93.56	0	00:00	0.00
A29	76.00	0.00	0.00	15.63	60.41	0.29	181.16	JUNCTION	0.00	0.02	93.55	0	01:34	0.02
0.798	76.00	0.00	0.00	16.88	59.15	0.35	196.65	JUNCTION	0.00	0.02	93.14	0	01:41	0.02
A30	76.00	0.00	0.00	17.99	58.03	0.43	223.07	JUNCTION	0.00	0.05	92.96	0	01:31	0.05
0.777	76.00	0.00	0.00	17.75	58.27	0.43	230.97	JUNCTION	0.00	0.00	92.94	0	00:00	0.00
A31	76.00	0.00	0.00	16.35	59.48	0.22	129.33	JUNCTION	0.00	0.00	93.07	0	00:00	0.00
0.773	76.00	0.00	0.00	16.43	59.60	0.18	105.36	JUNCTION	0.00	0.10	93.02	0	01:41	0.10
A32	76.00	0.00	0.00	15.86	60.18	0.08	49.83	JUNCTION	0.00	0.03	93.33	0	00:00	0.00
0.795	76.00	0.00	0.00	16.40	59.63	0.49	287.57	JUNCTION	0.00	0.03	93.48	0	00:00	0.00
A33	76.00	0.00	0.00	14.90	48.70	0.05	30.16	JUNCTION	0.00	0.00	93.27	0	00:00	0.00
0.793	76.00	0.00	0.00	16.88	59.15	0.35	196.65	JUNCTION	0.00	0.03	93.50	0	00:00	0.00
A34-35	76.00	0.00	0.00	15.78	60.26	0.51	315.08	JUNCTION	0.00	0.04	93.46	0	01:34	0.04
0.778	76.00	0.00	0.00	17.99	58.03	0.43	223.07	JUNCTION	0.00	0.00	93.99	0	00:00	0.00
A36	76.00	0.00	0.00	17.75	58.27	0.43	230.97	JUNCTION	0.00	0.00	93.99	0	00:00	0.00
0.793	76.00	0.00	0.00	16.35	59.48	0.22	129.33	JUNCTION	1.21	0.00	90.98	0	01:38	1.27
A37	76.00	0.00	0.00	16.43	59.60	0.18	105.36	JUNCTION	1.07	1.17	91.03	0	01:35	1.17
0.764	76.00	0.00	0.00	15.86	60.18	0.08	49.83	JUNCTION	1.04	1.18	91.07	0	01:34	1.18
0.767	76.00	0.00	0.00	16.40	59.63	0.49	287.57	JUNCTION	0.79	1.00	91.14	0	01:34	1.00
A39	76.00	0.00	0.00	14.90	48.70	0.05	30.16	JUNCTION	0.75	1.03	91.21	0	01:34	1.03
0.783	76.00	0.00	0.00	16.35	59.48	0.22	129.33	JUNCTION	0.63	0.96	91.27	0	01:35	0.96
A40	76.00	0.00	0.00	16.43	59.60	0.18	105.36	JUNCTION	0.50	0.88	91.33	0	01:35	0.88
0.784	76.00	0.00	0.00	15.86	60.18	0.08	49.83	JUNCTION	0.45	0.83	91.45	0	01:35	0.83
B01	76.00	0.00	0.00	16.40	59.63	0.49	287.57	JUNCTION	0.29	0.65	91.67	0	01:34	0.65
0.792	76.00	0.00	0.00	16.40	59.63	0.49	287.57	JUNCTION	0.29	0.65	91.67	0	01:34	0.65
0.785	76.00	0.00	0.00	14.90	48.70	0.05	30.16	JUNCTION	0.30	0.38	91.56	0	01:52	0.38
0.641	76.00	0.00	0.00	16.88	59.15	0.35	196.65	JUNCTION	0.30	0.38	91.56	0	01:52	0.38
0.778	76.00	0.00	0.00	17.99	58.03	0.43	223.07	JUNCTION	0.32	0.45	91.52	0	01:52	0.45
0.778	76.00	0.00	0.00	15.32	48.68	0.10	63.46	JUNCTION	0.32	0.46	91.52	0	01:52	0.46
0.641	76.00	0.00	0.00	16.63	59.40	0.12	69.71	JUNCTION	2.00	2.06	91.45	0	01:36	2.06
0.782	76.00	0.00	0.00	46.59	29.43	0.33	92.56	JUNCTION	2.01	2.22	91.27	0	01:35	2.22
0.387	76.00	0.00	0.00	15.13	60.93	0.05	37.42	JUNCTION	2.01	2.13	91.25	0	01:38	2.13
0.802	76.00	0.00	0.00	16.70	59.33	0.11	63.18	JUNCTION	2.01	2.13	91.20	0	01:40	2.13
B10	76.00	0.00	0.00	16.70	59.33	0.11	63.18	JUNCTION	2.01	2.13	91.16	0	01:34	2.13
B11	76.00	0.00	0.00	15.81	60.23	0.06	36.03	JUNCTION	2.08	2.97	91.89	0	01:43	2.97
0.792	76.00	0.00	0.00	14.98	48.69	0.05	35.23	JUNCTION	2.01	2.13	91.29	0	01:34	2.13
0.641	76.00	0.00	0.00	15.80	60.24	0.07	44.63	JUNCTION	0.37	0.70	91.08	0	01:36	0.70
0.793	76.00	0.00	0.00	15.82	60.22	0.08	50.47	JUNCTION	0.59	1.01	91.06	0	01:36	1.01
0.792	76.00	0.00	0.00	14.70	48.71	0.01	8.25	JUNCTION	0.31	0.44	91.34	0	01:43	0.44
0.641	76.00	0.00	0.00	15.77	60.27	0.07	45.16	JUNCTION	0.31	0.70	91.20	0	01:36	0.69
0.753	76.00	0.00	0.00	17.07	58.96	0.20	112.58	JUNCTION	0.32	0.81	91.20	0	01:36	0.81
0.776	76.00	0.00	0.00	15.25	60.81	0.07	43.45	JUNCTION	0.34	0.89	91.19	0	01:36	0.89
0.800	76.00	0.00	0.00	15.13	60.90	0.05	35.23	JUNCTION	0.53	1.09	91.18	0	01:36	1.09
0.801	76.00	0.00	0.00	15.13	60.90	5.02	2791.50	JUNCTION	0.01	0.12	91.06	0	02:03	0.12
								JUNCTION	0.52	0.91	91.04	0	01:36	0.91
								JUNCTION	0.69	1.06	91.02	0	01:35	1.06
								JUNCTION	1.38	1.74	91.00	0	01:35	1.74
								JUNCTION	0.07	0.51	91.25	0	01:35	0.51
								JUNCTION	0.08	0.68	91.25	0	01:35	0.68
								JUNCTION	0.23	0.82	91.24	0	01:35	0.82
								JUNCTION	0.22	0.79	91.22	0	01:35	0.79
								JUNCTION	0.30	0.84	91.19	0	01:35	0.84
								JUNCTION	0.72	1.25	91.18	0	01:35	1.25
								JUNCTION	0.50	0.99	91.13	0	01:35	0.99
								JUNCTION	0.02	0.34	91.19	0	01:35	0.34
								JUNCTION	0.03	0.55	91.19	0	01:35	0.55
								JUNCTION	0.39	0.93	91.19	0	01:35	0.93
								JUNCTION	0.18	0.71	91.18	0	01:35	0.71
								JUNCTION	1.22	1.86	91.17	0	01:35	1.86
								JUNCTION	0.48	0.71	91.15	0	01:35	0.71
								JUNCTION	0.07	0.15	91.20	0	01:36	0.15
								JUNCTION	2.10	2.58	91.23	0	01:36	2.58
								JUNCTION	0.82	1.12	91.04	0	01:21	1.12
								JUNCTION	1.35	1.64	90.92	0	00:21	1.64
								JUNCTION	1.36	1.56	90.77	0	00:12	1.43
								JUNCTION	1.46	1.78	90.83	0	00:12	1.41
								JUNCTION	1.64	1.83	90.83	0	00:12	1.78
								JUNCTION	0.09	0.11	93.56	0	00:12	0.11
								JUNCTION	0.11	1.79	93.46	0	01:41	1.79
								JUNCTION	0.09	1.81	93.38	0	01:42	1.81
								JUNCTION	0.13	1.86	93.59	0	01:49	1.86
								JUNCTION	0.12	1.83	93.49	0	01:59	1.83
								JUNCTION	0.08	1.82	93.46	0	01:55	1.82
								JUNCTION	0.08	1.86	93.56	0	01:56	1.86
								JUNCTION	0.10	1.78	93.28	0	01:44	1.78
								JUNCTION	0.09	1.81	93.14	0	01:42	1.81

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
01+217	JUNCTION	0.00	0.01	0 02:05	0.01
01+292	JUNCTION	0.00	0.05	0 01:56	0.05
01+370	JUNCTION	0.00	0.00	93.74	0.00
01+449	JUNCTION	0.00	0.00	93.62	0.00
01+521	JUNCTION	0.00	0.00	93.82	0.00
01+586	JUNCTION	0.00	0.00	94.97	0.00

Burnett Lands – 3370 Greenbank Road

100-year Storm, 5-year Fixed Outlet Elevations

Model Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Occurrence days hr:min	Lateral Inflow Volume 10 ⁶ Ltr	Total Inflow Volume 10 ⁶ Ltr	Flow Balance Error Percent
CB21-22	JUNCTION	0.08	1.84	92.57	0	0.0131	1.84
CB23-24	JUNCTION	0.09	1.85	92.94	0	0.0139	1.85
CB25-26	JUNCTION	0.08	1.81	92.94	0	0.0139	1.81
CB27-28	JUNCTION	0.08	1.77	93.02	0	0.0141	1.77
CB-28A	JUNCTION	0.05	1.44	94.04	0	0.0133	1.44
CB-28B	JUNCTION	0.06	1.45	94.05	0	0.0138	1.45
CB-28C	JUNCTION	0.06	1.46	94.06	0	0.0139	1.46
CB-28E	JUNCTION	0.06	1.46	94.06	0	0.0139	1.46
CB-28F	JUNCTION	0.06	1.46	94.06	0	0.0139	1.46
CB-28G	JUNCTION	0.05	1.45	94.05	0	0.0135	1.45
CB-28H	JUNCTION	0.04	1.43	94.03	0	0.0133	1.43
CB29-30	JUNCTION	0.09	1.74	93.02	0	0.0141	1.74
CB31-32	JUNCTION	0.08	1.79	93.26	0	0.0142	1.79
CB33-34	JUNCTION	0.08	1.81	93.46	0	0.0141	1.81
CB35-36	JUNCTION	0.08	1.81	93.48	0	0.0132	1.81
CB37-38	JUNCTION	0.08	1.78	93.53	0	0.0141	1.78
CB39-40	JUNCTION	0.08	1.68	93.25	0	0.0141	1.68
CB41-42	JUNCTION	0.10	1.67	93.51	0	0.0200	1.67
CB43-44	JUNCTION	0.17	1.94	93.74	0	0.0156	1.94
CB45-46	JUNCTION	0.08	1.72	93.58	0	0.0140	1.72
CB47-48	JUNCTION	0.06	1.72	93.59	0	0.0132	1.72
CB49-50	JUNCTION	0.07	1.73	93.80	0	0.0140	1.73
RYCB01	JUNCTION	0.01	0.32	92.32	0	0.0130	0.32
RYCB02	JUNCTION	0.03	1.70	93.73	0	0.0125	1.70
RYCB03	JUNCTION	0.04	2.14	93.64	0	0.0130	2.14
RYCB04	JUNCTION	0.04	2.14	93.64	0	0.0125	2.14
RYCB05	JUNCTION	0.01	0.47	93.23	0	0.0130	0.47
RYCB06	JUNCTION	0.05	2.23	93.34	0	0.0130	2.23
RYCB07	JUNCTION	0.05	2.03	93.60	0	0.0126	2.03
RYCB08	JUNCTION	0.06	2.33	93.64	0	0.0125	2.33
RYCB09	JUNCTION	0.04	2.14	93.63	0	0.0124	2.14
RYCB10	JUNCTION	0.04	2.22	93.30	0	0.0130	2.22
RYCB11	JUNCTION	0.04	1.98	93.60	0	0.0130	1.98
RYCB12	JUNCTION	0.04	2.20	93.62	0	0.0125	2.20
RYCB13	JUNCTION	0.26	3.15	93.59	0	0.0130	3.15
RYCB14	JUNCTION	0.02	1.92	93.43	0	0.0130	1.92
RYCB15	JUNCTION	0.03	1.94	93.47	0	0.0130	1.94
GRK-OUT	OUTFALL	0.00	0.00	0.00	0	0.0000	0.00
HW-01	OUTFALL	1.43	1.43	90.63	0	0.0000	1.43
HW-02	OUTFALL	1.02	1.02	90.52	0	0.0000	1.02
OVF-OUT	OUTFALL	0.00	0.03	92.91	0	0.0139	0.03
AZ2-STOR	STORAGE	0.05	1.68	94.18	0	0.0133	1.68
AZ3-STOR	STORAGE	0.05	1.69	94.19	0	0.0133	1.69
AZ4-STOR	STORAGE	0.05	1.70	94.19	0	0.0133	1.70
AZ5-STOR	STORAGE	0.05	1.70	94.19	0	0.0133	1.70
AZ6-STOR	STORAGE	0.05	1.70	94.45	0	0.0133	1.70
AZ7-STOR	STORAGE	0.05	1.70	94.45	0	0.0133	1.70
CAV-STOR	STORAGE	0.09	1.65	93.75	0	0.0143	1.65

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Occurrence days hr:min	Lateral Inflow Volume 10 ⁶ Ltr	Total Inflow Volume 10 ⁶ Ltr	Flow Balance Error Percent
01+217	JUNCTION	0.00	2.24	0	0.0140	0	0.00354
01+292	JUNCTION	0.00	4.24	0	0.0145	0	7.456
01+370	JUNCTION	0.00	0.00	0	0.0000	0	0.000
01+449	JUNCTION	0.00	0.00	0	0.0000	0	0.000
01+521	JUNCTION	0.00	0.00	0	0.0000	0	0.000
01+586	JUNCTION	0.00	0.00	0	0.0000	0	0.000
03+000	JUNCTION	0.00	1.22	0	0.0138	0	91.829
03+048	JUNCTION	0.00	0.00	0	0.0000	0	0.000
03+118	JUNCTION	0.00	0.00	0	0.0000	0	0.000
03+214	JUNCTION	0.00	0.00	0	0.0000	0	0.000
04+069	JUNCTION	0.00	0.00	0	0.0000	0	0.000
04+118	JUNCTION	0.00	24.21	0	0.0130	0	6.378
04+213	JUNCTION	0.00	10.02	0	0.0133	0	38.581
04+264	JUNCTION	0.00	57.42	0	0.0130	0	-21.123
04+305	JUNCTION	0.00	22.85	0	0.0135	0	1.433

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Occurrence days hr:min	Lateral Inflow Volume 10 ⁶ Ltr	Total Inflow Volume 10 ⁶ Ltr	Flow Balance Error Percent
04+357	JUNCTION	0.00	0.00	0	0.0000	0	0.000
04+383	JUNCTION	0.00	38.17	0	0.0130	0	0.000
05+067	JUNCTION	0.00	0.00	0	0.0000	0	0.000
05+122	JUNCTION	0.00	26.75	0	0.0130	0	0.000
05+177	JUNCTION	0.00	0.00	0	0.0000	0	0.000
06+075	JUNCTION	0.00	0.00	0	0.0000	0	0.000
06+132	JUNCTION	0.00	24.39	0	0.0130	0	0.00541
06+183	JUNCTION	0.00	36.98	0	0.0130	0	-14.253
06+300	JUNCTION	0.00	0.00	0	0.0000	0	0.000
07+067	JUNCTION	0.00	0.00	0	0.0000	0	0.000
100 (STM)	JUNCTION	0.00	1751.86	0	0.0139	0	-0.001
102 (STM)	JUNCTION	0.00	509.34	0	0.0134	0	2.005
104 (STM)	JUNCTION	0.00	444.72	0	0.0134	0	1.777
106 (STM)	JUNCTION	0.00	423.54	0	0.0134	0	1.688
108 (STM)	JUNCTION	0.00	405.08	0	0.0133	0	1.61
110 (STM)	JUNCTION	0.00	371.83	0	0.0136	0	1.5
112 (STM)	JUNCTION	0.00	327.73	0	0.0136	0	1.3
114 (STM)	JUNCTION	0.00	257.38	0	0.0134	0	0.83
116 (STM)	JUNCTION	0.00	226.87	0	0.0133	0	0.699
118 (STM)	JUNCTION	0.00	20.27	0	0.0140	0	0.0812
120 (STM)	JUNCTION	0.00	1.25	0	0.0122	0	0.00284
122 (STM)	JUNCTION	0.00	31.63	0	0.0147	0	0.209
124 (STM)	JUNCTION	0.00	31.59	0	0.0146	0	0.205
126 (STM)	JUNCTION	0.00	31.59	0	0.0146	0	0.205
128 (STM)	JUNCTION	0.00	18.42	0	0.0133	0	-0.0843
130 (STM)	JUNCTION	0.00	18.49	0	0.0138	0	0.0719
132 (STM)	JUNCTION	0.00	18.45	0	0.0140	0	0.0805
134 (STM)	JUNCTION	0.00	18.41	0	0.0134	0	0.0714
136 (STM)	JUNCTION	0.00	1249.60	0	0.0143	0	5.09
138 (STM)	JUNCTION	0.00	18.34	0	0.0133	0	0.0598
140 (STM)	JUNCTION	0.00	40.40	0	0.0130	0	0.149
142 (STM)	JUNCTION	0.00	105.01	0	0.0130	0	0.527
144 (STM)	JUNCTION	0.00	151.62	0	0.0129	0	0.776
146 (STM)	JUNCTION	0.00	34.97	0	0.0142	0	0.17
148 (STM)	JUNCTION	0.00	26.34	0	0.0141	0	0.119
150 (STM)	JUNCTION	0.00	29.18	0	0.0156	0	0.119
152 (STM)	JUNCTION	0.00	77.75	0	0.0136	0	0.314
154 (STM)	JUNCTION	0.00	114.27	0	0.0152	0	0.502
156 (STM)	JUNCTION	0.00	26.52	0	0.0142	0	0.133
158 (STM)	JUNCTION	0.00	27.86	0	0.0151	0	0.108
160 (STM)	JUNCTION	0.00	115.1	0	0.0142	0	0.606
162 (STM)	JUNCTION	0.00	894.38	0	0.0136	0	3.62
164 (STM)	JUNCTION	0.00	20.53	0	0.0141	0	0.0923
166 (STM)	JUNCTION	0.00	123.96	0	0.0138	0	0.379
168 (STM)	JUNCTION	0.00	182.60	0	0.0134	0	0.604
170 (STM)	JUNCTION	0.00	216.28	0	0.0136	0	0.758
172 (STM)	JUNCTION	0.00	242.51	0	0.0136	0	0.882
174 (STM)	JUNCTION	0.00	321.48	0	0.0133	0	1.11
176 (STM)	JUNCTION	0.00	355.75	0	0.0133	0	1.28
178 (STM)	JUNCTION	0.00	35.18	0	0.0159	0	0.216
180 (STM)	JUNCTION	0.00	54.83	0	0.0135	0	0.28
182 (STM)	JUNCTION	0.00	81.64	0	0.0152	0	0.399
184 (STM)	JUNCTION	0.00	104.96	0	0.0139	0	0.485
186 (STM)	JUNCTION	0.00	225.99	0	0.0152	0	0.997
188 (STM)	JUNCTION	0.00	240.04	0	0.0139	0	1.08
190 (STM)	JUNCTION	0.00	622.92	0	0.0135	0	2.52
192 (STM)	JUNCTION	0.00	14.42	0	0.0131	0	0.54
194 (STM)	JUNCTION	0.00	114.06	0	0.0133	0	0.773
196 (STM)	JUNCTION	0.00	288.42	0	0.0133	0	0.973
198 (STM)	JUNCTION	0.00	1280.93	0	0.0136	0	5.03
200 (STM)	JUNCTION	0.00	1281.31	0	0.0136	0	5.08
202 (STM)	JUNCTION	0.00	491.03	0	0.0136	0	1.95
204 (STM)	JUNCTION	0.00	993.29	0	0.0136	0	4.14
206 (STM)	JUNCTION	0.00	1280.92	0	0.0136	0	5
208 (STM)	JUNCTION	55.95	69.54	0	0.0130	0.0969	0.103
210 (STM)	JUNCTION	83.93	83.93	0	0.0130	0.152	-0.054
212 (STM)	JUNCTION	98.26	98.26	0	0.0130	0.171	0.056
214 (STM)	JUNCTION	116.37	116.37	0	0.0130	0.208	-0.148
216 (STM)	JUNCTION	93.05	117.73	0	0.0130	0.164	-0.513
218 (STM)	JUNCTION	59.79	103.42	0	0.0130	0.0967	-0.399
220 (STM)	JUNCTION	69.96	69.96	0	0.0130	0.119	0.052
222 (STM)	JUNCTION	95.41	95.41	0	0.0130	0.143	-0.509
224 (STM)	JUNCTION	73.9	73.9	0	0.0130	0.165	0.349
226 (STM)	JUNCTION	88.81	88.81	0	0.0130	0.181	-0.325
228 (STM)	JUNCTION	98.44	131.87	0	0.0130	0.116	0.181
230 (STM)	JUNCTION	40.04	104.23	0	0.0130	0.065	0.571
232 (STM)	JUNCTION	53.95	53.95	0	0.0130	0.0873	-0.022

**Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output**

Node	Type	Hours Surcharged	Max. Height Above Crown	Min. Depth Below Rim	Elevation
CB27-28	JUNCTION	87.12	87.12	0.1130	0.158
CB-283	JUNCTION	37.42	37.42	0.1130	0.0548
CB-284	JUNCTION	44.63	44.63	0.1130	0.0723
CB-285	JUNCTION	50.47	50.47	0.1130	0.0819
CB-286	JUNCTION	45.16	45.16	0.1130	0.0729
CB-287	JUNCTION	36.03	36.03	0.1130	0.0651
CB-288	JUNCTION	89.38	89.38	0.1130	0.0934
CB29-30	JUNCTION	69.23	71.19	0.1130	0.112
CB31-32	JUNCTION	89.48	90.93	0.1130	0.144
CB33-34	JUNCTION	55.89	103.46	0.1127	0.136
CB35-36	JUNCTION	55.66	55.66	0.1127	0.0924
CB37-38	JUNCTION	112.58	115.53	0.1130	0.208
CB39-40	JUNCTION	63.18	76.85	0.1130	0.148
CB41-42	JUNCTION	147.27	188.76	0.1130	0.473
CB43-44	JUNCTION	75.62	75.62	0.1130	0.134
CB45-46	JUNCTION	45.64	53.41	0.1130	0.0835
CB47-48	JUNCTION	49.83	49.83	0.1130	0.0812
CB49-50	JUNCTION	8.25	8.25	0.1127	0.0127
RYCB01	JUNCTION	33.65	33.65	0.1130	0.0516
RYCB02	JUNCTION	35.23	35.23	0.1130	0.054
RYCB03	JUNCTION	45.39	45.39	0.1130	0.0696
RYCB04	JUNCTION	10.16	10.16	0.1130	0.0156
RYCB05	JUNCTION	26.49	26.49	0.1130	0.0867
RYCB06	JUNCTION	68.18	68.18	0.1130	0.0574
RYCB07	JUNCTION	42.85	42.85	0.1130	0.0693
RYCB08	JUNCTION	46.66	46.66	0.1130	0.0529
RYCB09	JUNCTION	30.16	30.16	0.1130	0.0872
RYCB10	JUNCTION	55.22	55.22	0.1130	0.0847
RYCB11	JUNCTION	61.85	61.85	0.1130	0.0949
RYCB12	JUNCTION	22.54	22.54	0.1130	0.0346
RYCB13	JUNCTION	22.22	28.64	0.1130	0.0341
RYCB14	JUNCTION	688.73	688.73	0.1130	1.26
RYCB15	JUNCTION	0.00	1281.79	0.1136	0.000
HW-01	OUTFALL	0.00	1751.91	0.1139	0.000
HW-02	OUTFALL	0.00	1.03	0.1139	0.000
OVF-OUT	STORAGE	174.91	174.91	0.1130	0.285
A22-STOR	STORAGE	125.24	125.24	0.1130	0.192
A29-STOR	STORAGE	191.16	191.16	0.1130	0.288
A32-STOR	STORAGE	196.65	196.65	0.1130	0.348
A34-35-STOR	STORAGE	237.59	335.99	0.1130	0.509
B02-STOR	STORAGE	291.57	291.57	0.1130	0.491
CANV-STOR	STORAGE	2791.50	2791.50	0.1130	5.02

Node Surcharging Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown	Min. Depth Below Rim	Elevation
406 (STM)	JUNCTION	0.21	0.058	2.382	2.382
410a (STM)	JUNCTION	0.60	0.197	1.367	1.367
412 (STM)	JUNCTION	0.68	0.213	1.651	1.651
404 (STM)	JUNCTION	0.21	0.067	2.127	2.127
506a (STM)	JUNCTION	0.35	0.109	2.175	2.175
508 (STM)	JUNCTION	0.43	0.182	2.132	2.132
508a (STM)	JUNCTION	0.52	0.225	2.015	2.015
604 (STM)	JUNCTION	0.07	0.008	2.262	2.262
604a (STM)	JUNCTION	0.31	0.098	2.165	2.165
606 (STM)	JUNCTION	0.41	0.154	2.090	2.090
606a (STM)	JUNCTION	0.51	0.219	2.019	2.019
608 (STM)	JUNCTION	0.58	0.245	1.963	1.963
802 (STM)	JUNCTION	0.48	0.213	2.777	2.777
804 (STM)	JUNCTION	0.49	0.226	2.924	2.924
806 (STM)	JUNCTION	24.00	0.416	2.423	2.423
808 (STM)	JUNCTION	24.00	0.296	2.618	2.618
900 (STM)	JUNCTION	24.00	0.344	2.567	2.567
902 (STM)	JUNCTION	0.96	0.258	1.653	1.653
902 (STM)	JUNCTION	23.99	0.262	2.259	2.259

Node Flooding Summary

No nodes were flooded.

Storage Unit	Average Volume 1000 m3	Avgt Pent Full	Evap Loss	Exfil Pent Loss	Maximum Volume 1000 m3	Max Pent Full	Time of Occurrence days hr:min	Maximum Outflow IFS
A22-STOR	0.000	1	0	0	0.033	96	0 01:33	96.44
A29-STOR	0.000	1	0	0	0.025	83	0 01:33	96.38
A32-STOR	0.000	1	0	0	0.034	98	0 01:33	99.2
A34-35-STOR	0.000	1	0	0	0.033	93	0 01:33	114.06
A36-STOR	0.001	1	0	0	0.059	97	0 01:33	174.38
B02-STOR	0.001	1	0	0	0.049	99	0 01:33	165.01
CANV-STOR	0.020	2	0	0	0.909	71	0 01:43	1231.15

Outfall Loading Summary

Outfall Node	Flow Fed IFS	Max Flow IFS	Total Volume 10^6 I/cr
GRBK-OUT	45.94	33.38	688.73
HW-01	99.28	62.68	1281.79
HW-02	91.05	95.35	1751.91
OVF-OUT	2.11	0.49	1.03
System	59.59	191.90	3572.26

Link Flow Summary

Link	Type	Maximum Flow IFS	Time of Occurrence days hr:min	Maximum Veloc m/sec	Max/Full Flow	Max/Full Depth
100-42	CONDUIT	1751.91	0 01:39	1.05	0.62	0.82
102-100	CONDUIT	509.26	0 01:34	0.89	0.68	0.74
104-102	CONDUIT	444.67	0 01:34	0.87	0.75	0.73
106-104	CONDUIT	423.51	0 01:34	1.00	0.75	0.72
108-106	CONDUIT	405.05	0 01:34	1.07	0.72	0.65
110-108	CONDUIT	372.08	0 01:37	0.96	0.67	0.67
112-110	CONDUIT	327.98	0 01:36	0.94	0.62	0.72
114-112	CONDUIT	257.01	0 01:36	0.90	0.49	0.61
116-114	CONDUIT	226.63	0 01:34	1.21	0.59	0.52
118-116	CONDUIT	39.83	0 01:50	0.29	0.11	0.51
200-202	CONDUIT	1.26	0 02:49	0.06	0.00	0.14
202-204	CONDUIT	31.63	0 01:48	0.59	0.08	0.24
204-302b	CONDUIT	31.63	0 01:52	0.65	0.11	0.22
282-114	CONDUIT	0.91	0 01:50	0.21	0.00	0.18
283-110	CONDUIT	19.33	0 01:52	0.61	0.10	0.53
285-106	CONDUIT	18.45	0 01:38	0.71	0.27	0.39
285-104	CONDUIT	18.45	0 01:34	0.71	0.27	0.39
286-102	CONDUIT	18.41	0 01:34	0.71	0.27	0.39
287-100	CONDUIT	1249.60	0 01:43	1.57	0.79	0.65
288-108	CONDUIT	18.34	0 01:33	0.71	0.27	0.41
300-302	CONDUIT	38.12	0 01:31	0.40	0.12	0.73
302-304	CONDUIT	104.70	0 01:52	0.73	0.36	0.91
304-410	CONDUIT	156.35	0 01:51	0.49	0.40	1.00
306-304	CONDUIT	34.96	0 01:43	0.77	0.10	0.21
400-402	CONDUIT	29.18	0 01:56	0.62	0.07	0.74
402-404	CONDUIT	35.12	0 01:56	0.31	0.11	0.90
404-406	CONDUIT	79.62	0 01:52	0.52	0.28	0.98
406-606	CONDUIT	121.72	0 01:52	0.36	0.36	1.00
408-410	CONDUIT	26.52	0 01:42	0.73	0.08	0.34
410-412_1	CONDUIT	227.49	0 01:51	0.50	0.46	1.00
410-412_2	CONDUIT	255.29	0 01:51	0.56	0.64	1.00
412-900	CONDUIT	894.51	0 01:36	0.77	0.56	1.00
500-502	CONDUIT	34.15	0 01:31	0.49	0.08	0.87
502-504	CONDUIT	182.98	0 01:38	0.61	0.42	1.00
504-506	CONDUIT	216.17	0 01:36	0.58	0.53	1.00
506-508_1	CONDUIT	242.45	0 01:36	0.66	0.60	1.00
506-508_2	CONDUIT	242.45	0 01:36	0.66	0.60	1.00

Burnett Lands – 3370 Greenbank Road

100-year Storm, 5-year Fixed Outlet Elevations

Model Output

Conduit	Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Normal Flow	Hours Full Capacity Limited
ST3-02	1.00	0.96	0.04	0.00	0.00
ST3-03	1.00	0.96	0.04	0.00	0.00
ST3-04	1.00	0.95	0.05	0.00	0.00
ST3-05	1.00	0.95	0.05	0.00	0.00
ST3-06	1.00	0.95	0.05	0.00	0.00
ST3-07	1.00	0.96	0.04	0.00	0.00
ST4-01	1.00	0.96	0.04	0.00	0.00
ST4-02	1.00	0.96	0.04	0.00	0.00
ST4-03	1.00	0.96	0.04	0.00	0.00
ST4-04	1.00	0.96	0.04	0.00	0.00
ST4-05	1.00	0.96	0.04	0.00	0.00
ST4-06	1.00	0.96	0.04	0.00	0.00
ST4-07	1.00	0.96	0.04	0.00	0.00
ST4-08	1.00	0.96	0.04	0.00	0.00
ST4-09	1.00	0.96	0.04	0.00	0.00
ST4-10	1.00	0.96	0.04	0.00	0.00
ST4-11	1.00	0.96	0.04	0.00	0.00
ST4-12	1.00	0.96	0.04	0.00	0.00
ST4-13	1.00	0.96	0.04	0.00	0.00
ST4-14	1.00	0.96	0.04	0.00	0.00
ST4-15	1.00	0.96	0.04	0.00	0.00
ST5-01	1.00	0.96	0.04	0.00	0.00
ST5-02	1.00	0.96	0.04	0.00	0.00
ST5-03	1.00	0.96	0.04	0.00	0.00
ST5-04	1.00	0.96	0.04	0.00	0.00
ST5-05	1.00	0.96	0.04	0.00	0.00
ST5-06	1.00	0.96	0.04	0.00	0.00
ST5-07	1.00	0.96	0.04	0.00	0.00
ST5-08	1.00	0.96	0.04	0.00	0.00
ST6-01	1.00	0.94	0.06	0.00	0.00
ST6-02	1.00	0.94	0.06	0.00	0.00
ST6-03	1.00	0.96	0.04	0.00	0.00
ST6-04	1.00	0.96	0.04	0.00	0.00
ST6-05	1.00	0.96	0.04	0.00	0.00
ST6-06	1.00	0.96	0.04	0.00	0.00
ST6-07	1.00	0.96	0.04	0.00	0.00
ST6-08	1.00	0.96	0.04	0.00	0.00
ST7-01	1.00	0.96	0.04	0.00	0.00
ST7-02	1.00	0.96	0.04	0.00	0.00

 Conduit Surge Summary

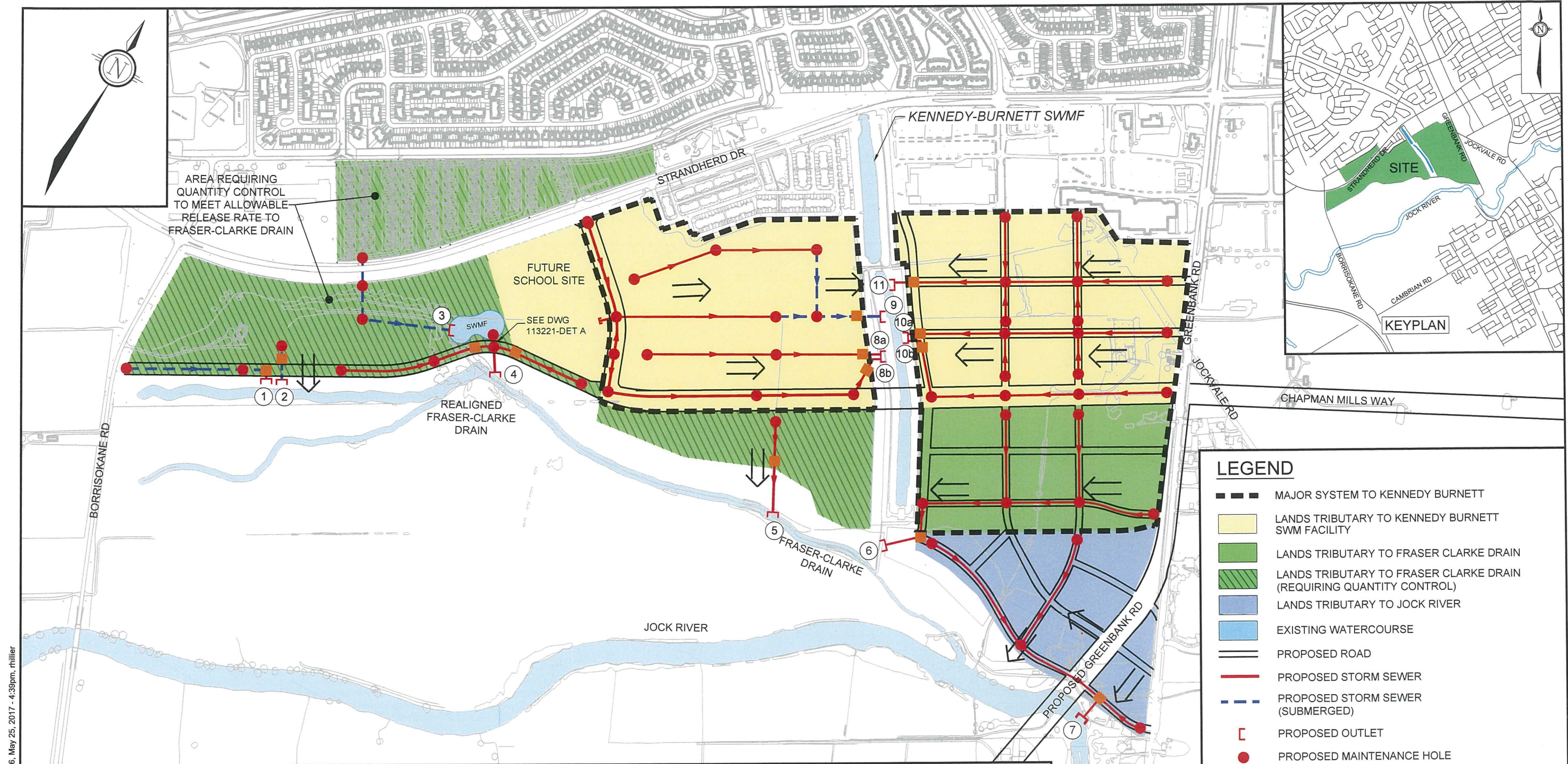
Conduit	Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Normal Flow	Hours Full Capacity Limited
302-304	0.01	0.01	0.01	0.15	0.01
304-410	0.17	0.17	0.46	0.01	0.01
404-406	0.01	0.01	0.21	0.01	0.01
406-606	0.29	0.29	0.41	0.01	0.01
410-412_1	0.48	0.48	0.60	0.01	0.01
410-412_2	0.60	0.60	0.75	0.01	0.01
412-900	0.68	0.68	0.96	0.01	0.01
500-502	0.01	0.01	0.02	0.01	0.01
502-504	0.01	0.01	0.21	0.01	0.01
504-506	0.21	0.21	0.33	0.01	0.01
506-508_1	0.33	0.33	0.40	0.01	0.01
506-508_2	0.43	0.43	0.53	0.01	0.01
508-608_1	0.43	0.43	0.52	0.01	0.01
508-608_2	0.52	0.52	0.56	0.01	0.01
602-604	0.01	0.01	0.07	0.01	0.01
604-606_1	0.11	0.11	0.31	0.01	0.01
604-606_2	0.31	0.31	0.42	0.01	0.01
606-608_1	0.42	0.42	0.51	0.01	0.01
606-608_2	0.51	0.51	0.64	0.01	0.01
608-412	0.59	0.59	0.82	0.01	0.15
700-506	0.01	0.01	0.41	0.01	0.01
802-804	0.48	0.48	0.49	0.01	0.01
804-902	0.61	0.61	24.00	0.01	0.01
808-HW1	24.00	24.00	24.00	0.01	0.02
900-902	0.96	0.96	23.99	0.01	1.07
902-806	24.00	24.00	24.00	0.01	0.06
C7	24.00	24.00	24.00	0.01	0.59
CR007	24.00	24.00	24.00	0.01	0.01
RYOVF07	0.01	0.01	22.38	0.01	0.01

Analysis begun on: Tue Jan 16 10:03:06 2018
 Analysis ended on: Tue Jan 16 10:03:15 2018

**Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output**

Total elapsed time: 00:00:09





LEGEND

- MAJOR SYSTEM TO KENNEDY BURNETT
- LANDS TRIBUTARY TO KENNEDY BURNETT SWM FACILITY
- LANDS TRIBUTARY TO FRASER CLARKE DRAIN
- LANDS TRIBUTARY TO FRASER CLARKE DRAIN (REQUIRING QUANTITY CONTROL)
- LANDS TRIBUTARY TO JOCK RIVER
- EXISTING WATERCOURSE
- PROPOSED ROAD
- PROPOSED STORM SEWER
- PROPOSED STORM SEWER (SUBMERGED)
- PROPOSED OUTLET
- PROPOSED MAINTENANCE HOLE
- PROPOSED HYDRO DYNAMIC SEPARATOR (HDS)
- MAJOR OVERLAND FLOW DIRECTION

ID	WATER COURSE	DRAINAGE AREA	OUTLET			PIPE SIZE	PEAK FLOW *CONTROLLED	SUBMERGED SEWERS	DEPTH TO OBVERT	MAX GRADE RAISE	STM HGL (D/S - U/S)
			NWL	PIPE INVERT							
1	FRASER-CLARKE DRAIN	0.97 ha	90.25	90.15	600mm	28 L/s*	75m	1.5m	1.1m	91.75m - 92.30m	
2	FRASER-CLARKE DRAIN	5.34 ha	90.25	89.85	965 x 1525mm ELLIPTICAL	187 L/s*	400m	1.5m	0.9m	91.75m - 92.20m	
3	MINTO SWM POND	14.64 ha	90.00	89.81	1220mm x 1930mm ELLIPTICAL	1,785 L/S	195m	1.5m	0.8m	91.65m - 92.23m	
4	FRASER-CLARKE DRAIN	1.29 ha + 14.64 ha	89.90	89.90	1050mm	692 L/S*	0m	1.5m	1.0m	91.65m - 92.25m	
5	FRASER-CLARKE DRAIN	6.49 ha	89.87	89.87	965 x 1525mm ELLIPTICAL	363 L/S*	0m	1.8m	0.9m	91.65m - 92.00m	
6	FRASER-CLARKE DRAIN	11.83 ha	89.90	89.90	1220mm x 1930mm ELLIPTICAL	1,649 L/S	0m	1.8m	0.8m	91.75m - 92.65m	
7	JOCK RIVER	9.24 ha	89.20	89.20	965 x 1525mm ELLIPTICAL	1,252 L/S	0m	1.8m	0.1m	91.60m - 92.45m	
8A	KENNEDY-BURNETT SWMF	6.58 ha	90.20	90.20	1050mm	915 L/S	0m	1.5m	0.9m	91.80m - 92.80m	
8B	KENNEDY-BURNETT SWMF	2.44 ha	90.20	90.20	825mm	444 L/S	0m	1.8m	0.9m	91.80m - 92.76m	
9	KENNEDY-BURNETT SWMF	15.49 ha	90.20	90.00	1220mm x 1930mm ELLIPTICAL	2,034 L/S	200m	1.5m	1.2m	91.90m - 93.18m	
10A	KENNEDY-BURNETT SWMF	6.68 ha	90.20	90.20	1050mm	928 L/S	0m	2.0m	0m	91.80m - 92.80m	
10B	KENNEDY-BURNETT SWMF	2.07 ha	90.20	90.20	825 mm	365 L/S	0m	2.0m	0m	91.80m - 92.78m	
11	KENNEDY-BURNETT SWMF	10.90 ha	90.20	90.20	1220mm x 1930mm ELLIPTICAL	1,892 L/S	0m	1.8m	0.3m	91.90m - 92.78m	

NOVATECH
 Engineers, Planners & Landscape Architects
 Suite 200, 240 Michael Cowpland Drive
 Ottawa, Ontario, Canada K2M 1P6
 Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

**KENNEDY-BURNETT SWMF
SERVICING OPTIONS**

**OPTION 3b: HYBRID EXPANDED
K-B SWIMF / HDS UNITS**

SCALE 1 : 7500

DATE JAN 2017 JOB 113221 FIGURE FIG-6

M:\2013\113221\CAD\Design\Figures\SWM\113221-Figs 5-6.dwg, FIG-6, May 25, 2017 - 4:39pm, rhiller

Burnett Lands - 3370 Greenbank Road
Design Storm Time Series Data
Chicago Design Storms



C25mm-4.stm		C2-4.stm		C5-4.stm	
Duration	Intensity	Duration	Intensity	Duration	Intensity
min	mm/hr	min	mm/hr	min	mm/hr
0:00	0	0:00	0	0:00	0
0:10	1.34	0:10	1.98	0:10	2.49
0:20	1.49	0:20	2.23	0:20	2.77
0:30	1.69	0:30	2.58	0:30	3.14
0:40	1.96	0:40	3.06	0:40	3.62
0:50	2.33	0:50	3.81	0:50	4.31
1:00	2.91	1:00	5.1	1:00	5.37
1:10	3.91	1:10	7.91	1:10	7.19
1:20	6.1	1:20	19.04	1:20	11.14
1:30	14.53	1:30	76.81	1:30	26.25
1:40	58.72	1:40	23.64	1:40	104.19
1:50	17.11	1:50	11.91	1:50	30.86
2:00	8.32	2:00	7.98	2:00	15.15
2:10	5.5	2:10	6.03	2:10	10.07
2:20	4.13	2:20	4.87	2:20	7.58
2:30	3.32	2:30	4.1	2:30	6.11
2:40	2.79	2:40	3.55	2:40	5.14
2:50	2.41	2:50	3.14	2:50	4.45
3:00	2.12	3:00	2.82	3:00	3.93
3:10	1.9	3:10	2.57	3:10	3.53
3:20	1.73	3:20	2.35	3:20	3.21
3:30	1.58	3:30	2.18	3:30	2.94
3:40	1.46	3:40	2.03	3:40	2.72
3:50	1.36	3:50	1.9	3:50	2.53
4:00	1.27	4:00	1.79	4:00	2.37

Burnett Lands - 3370 Greenbank Road
Design Storm Time Series Data
Chicago Design Storms



C100-4.stm		C100-4+20%.stm	
Duration	Intensity	Duration	Intensity
min	mm/hr	min	mm/hr
0:00	0	0:00	0
0:10	4.07	0:10	4.88
0:20	4.54	0:20	5.45
0:30	5.14	0:40	7.14
0:40	5.95	0:50	8.51
0:50	7.09	1:00	10.62
1:00	8.85	1:10	14.28
1:10	11.9	1:20	22.25
1:20	18.54	1:30	53.03
1:30	44.19	1:40	214.27
1:40	178.56	1:50	62.45
1:50	52.04	2:00	30.37
2:00	25.31	2:10	20.08
2:10	16.73	2:20	15.07
2:20	12.56	2:30	12.11
2:30	10.09	2:40	10.16
2:40	8.47	2:50	8.78
2:50	7.32	3:00	7.75
3:00	6.46	3:10	6.95
3:10	5.79	3:20	6.3
3:20	5.25	3:30	5.78
3:30	4.82	3:40	5.34
3:40	4.45	3:50	4.97
3:50	4.14	4:00	4.66
4:00	3.88		

CDS Average Annual Efficiency For TSS Removal

Area = 10.57 ha

C = 0.67

CDS Model: CDS PMSU56_68

Flowrate: 538

IDF Data: Ottawa

Engineer: Novatech

Contact: Kallie Auld, P.Eng.

Date: 15-Jan-18

Project: Burnett Lands - 3370 Greenbank Rd.

Location: Ottawa, ON

OGS ID: OGS 1

Return	Period	Flow	TSS Percentage Captured	Treated Flow Volume	Total Flow Volume	Frequency of Occurrence	System Flow	CDS Flow	By-Pass Flow	Volume Percentage Treated
month / yr	Yr	l/s	%	litres	litres	%	l/s	l/s	l/s	%
1m	0.08	128.72	94.2	185681	185681	91.70	128.72	129	0.00	100.00
2m	0.17	212.26	91.3	306184	306185	83.33	212.26	212	0.00	100.00
3m	0.25	282.70	88.7	407793	407793	75.00	282.70	283	0.00	100.00
4m	0.33	347.46	86.4	501206	501206	66.70	347.46	347	0.00	100.00
5m	0.42	402.91	84.6	574440	574440	58.30	402.91	403	0.00	100.00
6m	0.50	449.00	82.8	647674	647674	50.00	449.00	449	0.00	100.00
7m	0.58	491.75	81.3	701021	703549	41.70	491.75	492	0.00	99.69
8m	0.67	530.95	79.8	754367	759423	33.30	530.95	531	0.00	99.38
9m	0.75	565.20	78.2	807713	815297	25.00	565.20	538	27.18	99.07
10m	0.83	598.19	76.5	836069	860049	16.70	598.19	538	60.17	97.39
11m	0.92	629.36	74.8	864425	904801	8.30	629.36	538	91.34	95.70
1y	1	658.27	73.1	892781	949552	1.00	658.27	538	120.25	94.02
2y	2	921.70	60.5	1050620	1329544	0.50	921.70	538	383.68	79.02
5y	5	1074.82	55.0	1121754	1550420	0.20	1074.82	538	536.80	72.35
10y	10	1105.05	54.1	1135330	1594024	0.10	1105.05	538	567.03	71.22
25y	25	1144.69	53.0	1153135	1651208	0.04	1144.69	538	606.67	69.84
50y	50	1783.93	39.4	1353175	2573312	0.02	1783.93	538	1245.91	52.58
100y	100	1269.07	49.5	1197749	1830625	0.01	1269.07	538	731.05	65.43
Q-ave		441.40	l/s	14491117.3	18144785.1					

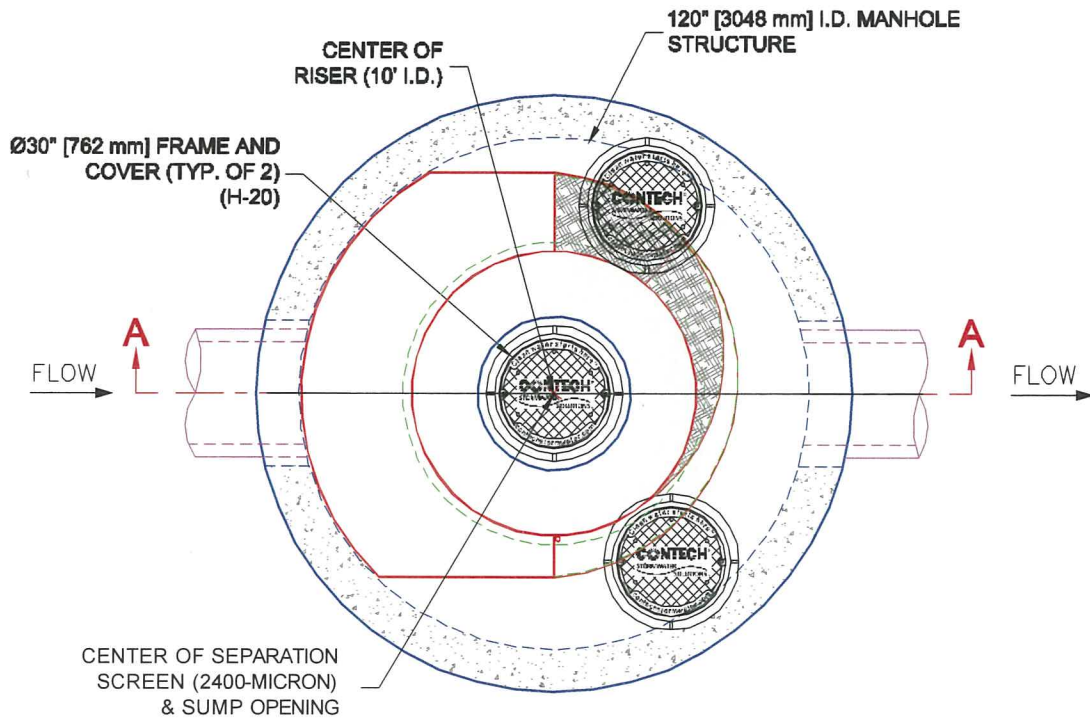
Average Annual TSS Removal Efficiency [%]: **82.9**

Ave. Ann. T. Volume [%]:

97.8



PLAN VIEW



**CDS MODEL PMSU56_68_10, 538 L/S TREATMENT CAPACITY
STORM WATER TREATMENT UNIT**

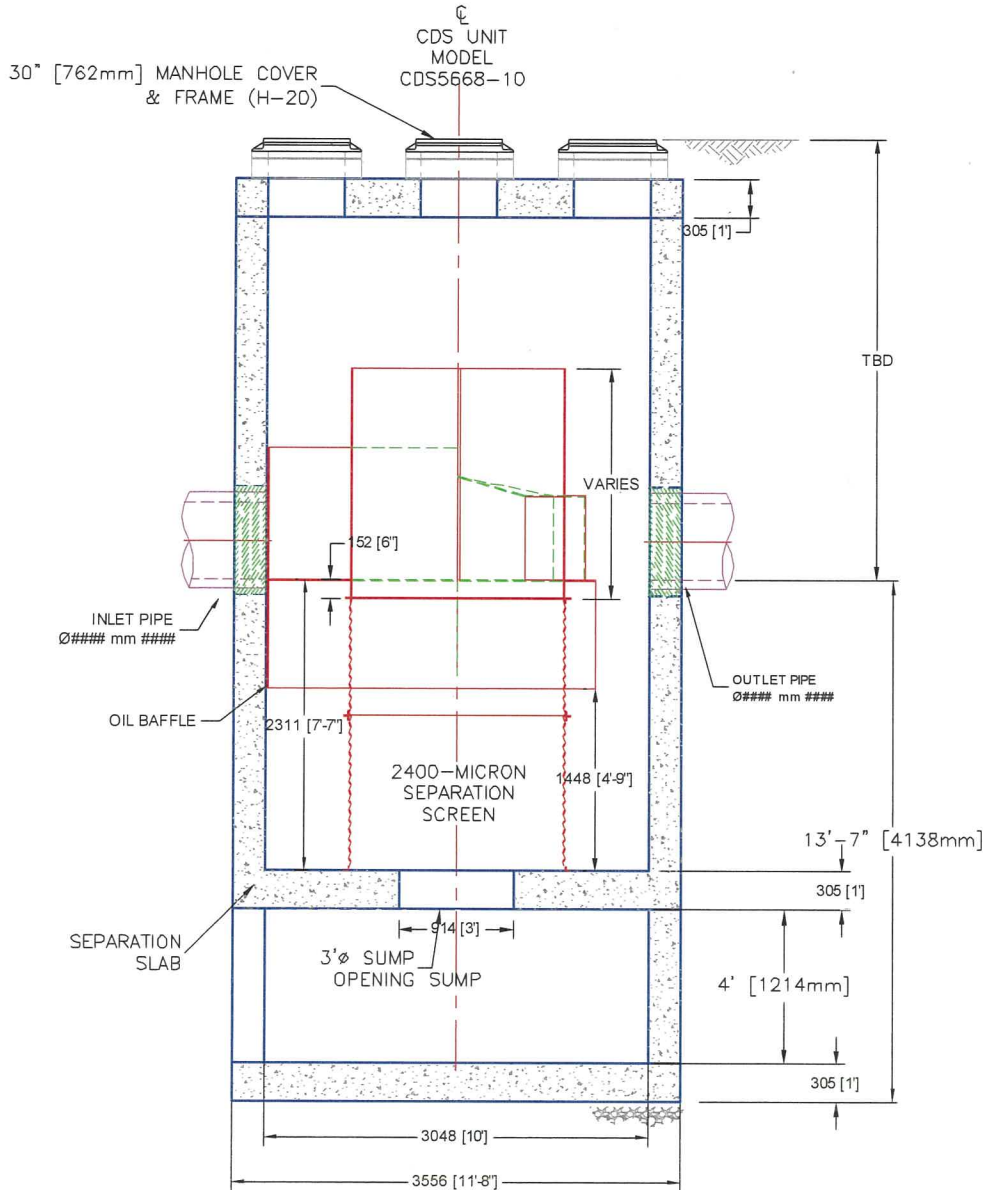


PROJECT NAME
CITY, STATE

JOB#	###/##-##-###	SCALE 1" = 5'
DATE	##/##/##	SHEET 1
DRAWN	INITIALS	
APPROV.		



SECTION A-A VIEW



CDS MODEL PMSU56_68_10, 538 L/S TREATMENT CAPACITY
STORM WATER TREATMENT UNIT



PROJECT NAME
CITY, STATE

JOB#	###/##-##-###
DATE	##/##/##
DRAWN	INITIALS
APPROV.	

SCALE
1" = 6'

SHEET

2



**VORTECHS SYSTEM® ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
 BASED ON AN AVERAGE PARTICLE SIZE OF TYPICAL MICRONS
 BURNETT LANDS 3370 GREENBANK RD
 OTTAWA, ON
 MODEL PC1319 OFF-LINE**

Design Ratio¹ = $\frac{(10.57 \text{ hectares}) \times (0.67) \times (2.775)}{(12.3 \text{ m}^2)} = 1.59$

Bypass occurs at an elevation of 90.16m (at approximately 26 l/s/m²)

Rainfall Intensity mm/hr	Operating Rate ² % of capacity	Flow Treated (l/s)	% Total Rainfall Volume ³	Rmvl. Effcy ⁴ (%)	Rel. Effcy (%)
0.5	1.2	9.9	9.2%	98.0%	9.0%
1.0	2.3	19.8	10.6%	98.0%	10.4%
1.5	3.5	29.7	9.9%	98.0%	9.7%
2.0	4.7	39.6	8.4%	98.0%	8.2%
2.5	5.8	49.5	7.7%	98.0%	7.5%
3.0	7.0	59.4	5.9%	98.0%	5.8%
3.5	8.2	69.3	4.4%	96.9%	4.2%
4.0	9.3	79.2	4.7%	96.3%	4.5%
4.5	10.5	89.1	3.3%	96.0%	3.2%
5.0	11.7	99.0	3.0%	95.3%	2.9%
6.0	14.0	118.8	5.4%	93.8%	5.1%
7.0	16.3	138.6	4.4%	90.6%	3.9%
8.0	18.6	158.4	3.5%	88.8%	3.1%
9.0	21.0	178.2	2.8%	87.3%	2.5%
10.0	23.3	198.0	2.2%	85.7%	1.9%
15.0	35.0	297.0	7.0%	80.0%	5.6%
20.0	46.6	396.0	4.1%	69.0%	2.9%
25.0	58.3	495.0	1.2%	59.3%	0.7%
30.0	69.9	594.1	0.5%	50.0%	0.3%
35.0	81.6	693.1	0.4%	33.7%	0.1%
40.0	93.2	792.1	0.4%	14.4%	0.1%
					91.5%

Predicted Annual Runoff Volume Treated = 92.5%
Assumed removal efficiency for bypassed flows = 0.0%
Estimated reduction in efficiency⁵ = 6.5%
Predicted Net Annual Load Removal Efficiency = 85%

1 - Design Ratio = (Total Drainage Area) x (Runoff Coefficient) x (Rational Method Conversion) / Grit Chamber Area

- The Total Drainage Area and Runoff Coefficient are specified by the site engineer.
- The rational method conversion based on the units in the above equation is 2.775.

2 - Operating Rate (% of capacity) = percentage of peak operating rate of 68 l/s/m².

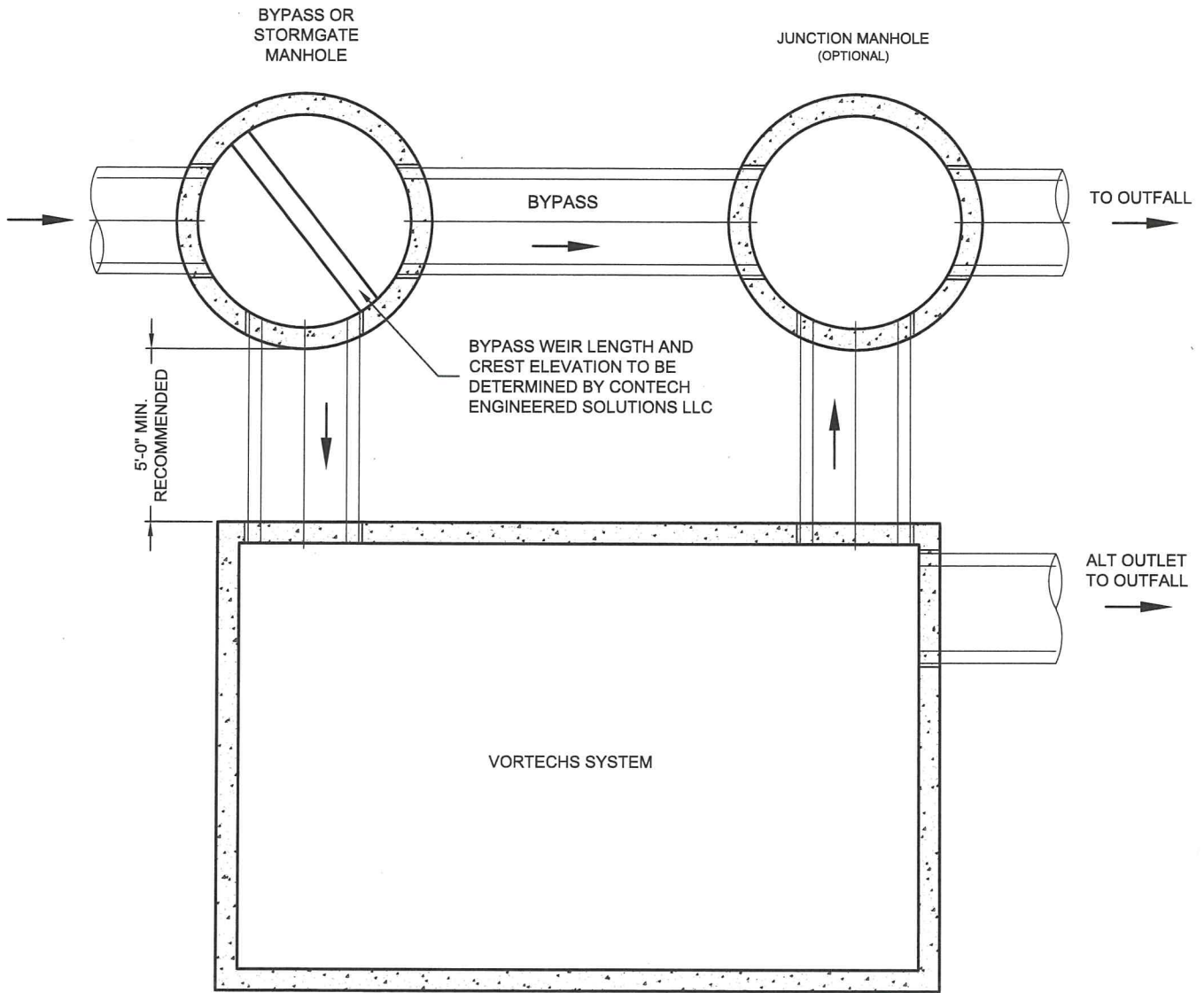
3 - Based on 42 years of hourly rainfall data from Canadian Station 6105976, Ottawa CDA, ON

4 - Based on Contech Construction Products laboratory verified removal of a TYPICAL particle size gradation (see Technical Bulletin #1).

5- Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

Calculated by: JAK 1/16 Checked by:

FOR INFORMATIONAL PURPOSES ONLY
 NOT INTENDED AS A CONSTRUCTION DOCUMENT
- BYPASS AND JUNCTION STRUCTURES MAY OR MAY NOT BE SUPPLIED BY CONTECH -



ACTUAL ORIENTATION AND LAYOUT MAY VARY DUE TO
 SITE SPECIFIC CONSIDERATIONS



THIS PRODUCT MAY BE PROTECTED BY THE FOLLOWING
 U.S. PATENT: 5,759,415, RELATED FOREIGN PATENTS.

The design and information shown on this drawing is provided as a service to the project owner, engineer and contractor by Contech Engineered Solutions LLC ("Contech"). Neither this drawing, nor any part thereof, may be used, reproduced or modified in any manner without the prior written consent of Contech. Failure to comply is done at the user's own risk and Contech expressly disclaims any liability or responsibility for such use.

If discrepancies between the supplied information upon which the drawing is based and actual field conditions are encountered as site work progresses, these discrepancies must be reported to Contech immediately for re-evaluation of the design. Contech accepts no liability for designs based on missing, incomplete or inaccurate information supplied by others.

CONTECH
 ENGINEERED SOLUTIONS LLC
 www.ContechES.com

200 Enterprise Drive, Scarborough, ME 04074
 877-907-8676 207-885-9830 207-885-9825 FAX

TYPICAL BYPASS LAYOUT
 VORTECHS® STORMWATER TREATMENT SYSTEM

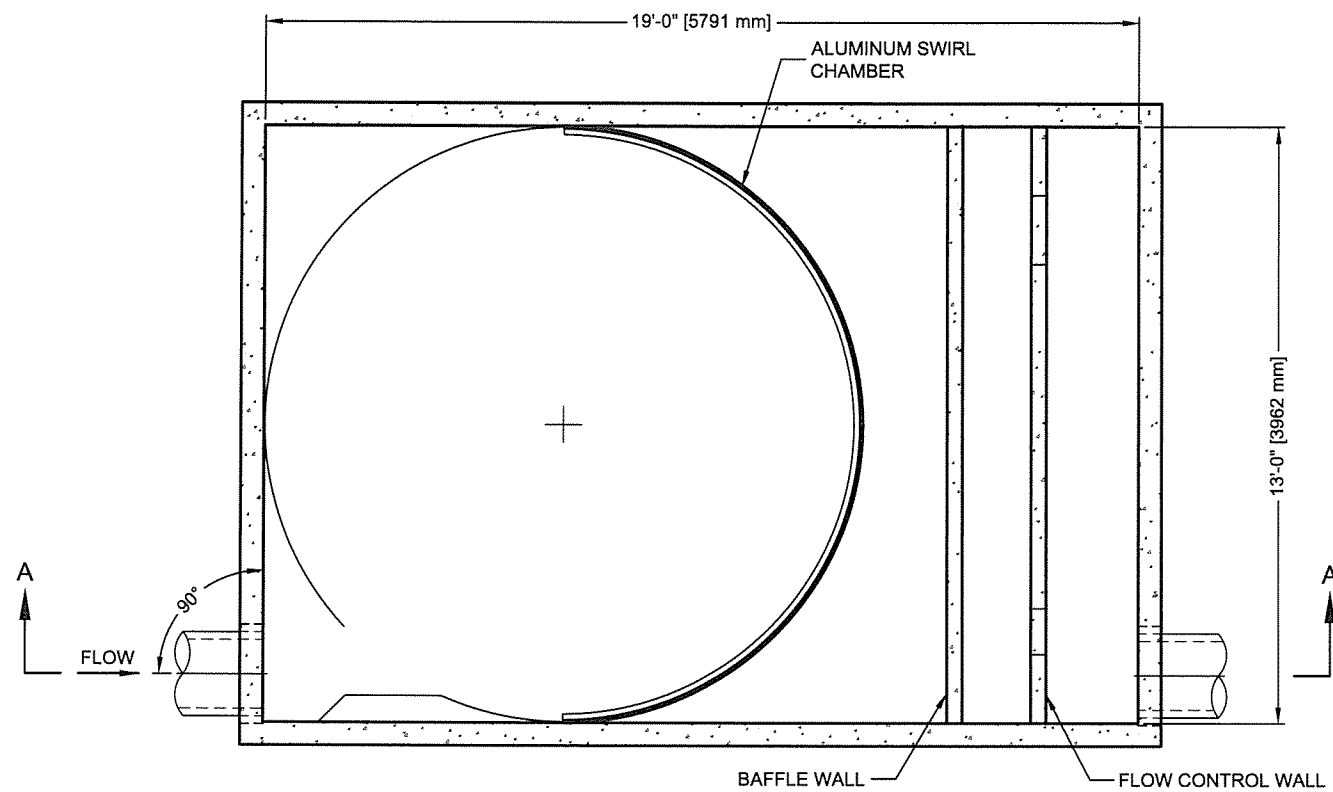
DATE: 3/8/13	SCALE: NONE	PROJECT No.: TYPVXBPLOR	SEQ. No.: N/A	DRAWN: SCF	CHECKED: NDG
--------------	-------------	-------------------------	---------------	------------	--------------

I:\COMM\CAD\TREATMENT\20 VORTECHS\40 STANDARD DRAWINGS\TYPICAL DETAILS\TYPVXBPLOR.DWG 3/8/2013 3:11 PM

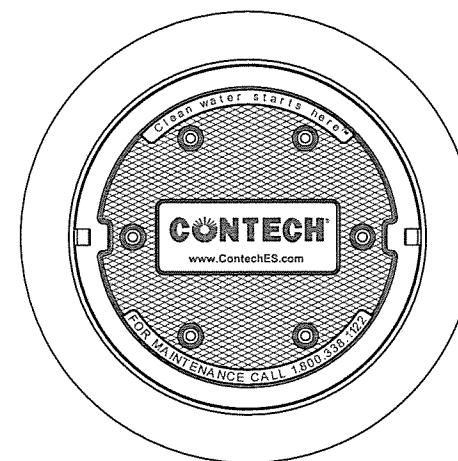
VORTECHS PC1319 DESIGN NOTES

VORTECHS PC1319 RATED TREATMENT CAPACITY IS 30 CFS, OR PER LOCAL REGULATIONS. IF THE SITE CONDITIONS EXCEED RATED TREATMENT CAPACITY, AN UPSTREAM BYPASS STRUCTURE IS REQUIRED.

THE STANDARD INLET/OUTLET CONFIGURATION IS SHOWN. FOR OTHER CONFIGURATION OPTIONS, PLEASE CONTACT YOUR CONTECH REPRESENTATIVE. www.ContechES.com

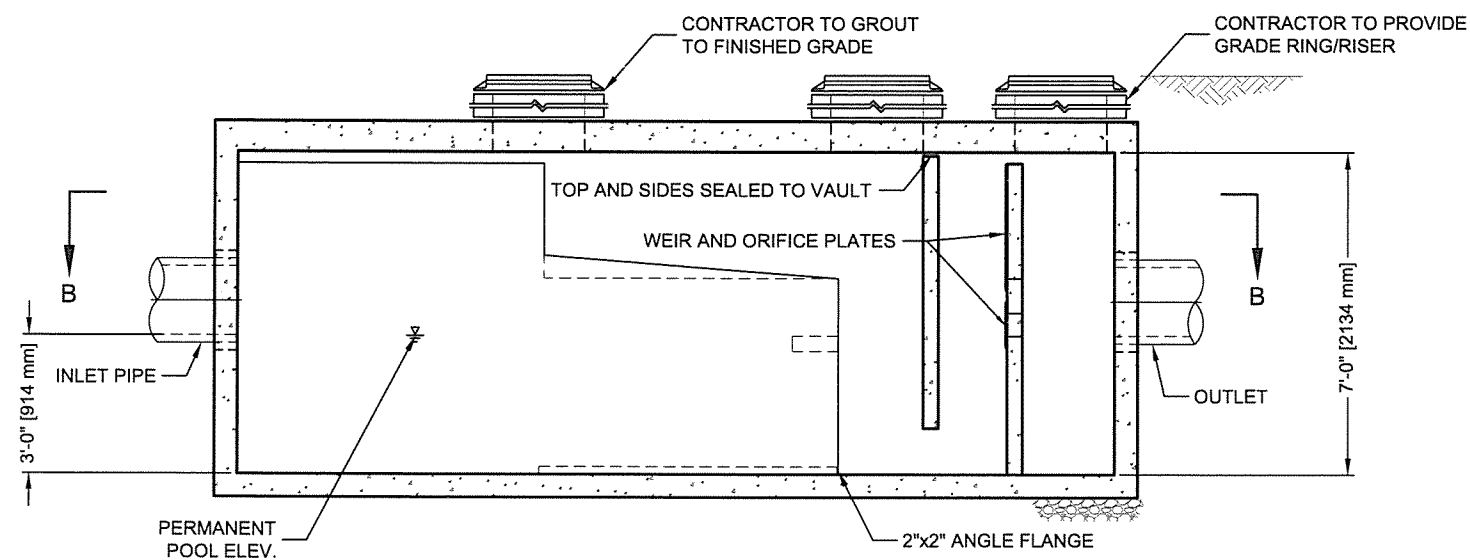


SECTION B-B



FRAME AND COVER
(DIAMETER VARIES)
N.T.S.

SITE SPECIFIC DATA REQUIREMENTS			
STRUCTURE ID	*		
WATER QUALITY FLOW RATE (CFS)	*		
PEAK FLOW RATE (CFS)	*		
RETURN PERIOD OF PEAK FLOW (YRS)	*		
PIPE DATA:	I.E.	MATERIAL	DIAMETER
INLET PIPE 1	*	*	*
INLET PIPE 2	*	*	*
OUTLET PIPE	*	*	*
RIM ELEVATION			
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT	
	*	*	
NOTES/SPECIAL REQUIREMENTS:			
* PER ENGINEER OF RECORD			



SECTION A-A

GENERAL NOTES

- CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- DIMENSIONS MARKED WITH () ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
- FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
- VORTECHS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING.
- STRUCTURE SHALL MEET AASHTO HS20 AND CASTINGS SHALL MEET AASHTO M306 LOAD RATING, ASSUMING GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION.
- INLET PIPE(S) MUST BE PERPENDICULAR TO THE VAULT AND AT THE CORNER TO INTRODUCE THE FLOW TANGENTIALLY TO THE SWIRL CHAMBER. DUAL INLETS NOT TO HAVE OPPOSING TANGENTIAL FLOW DIRECTIONS.
- OUTLET PIPE(S) MUST BE DOWN STREAM OF THE FLOW CONTROL BAFFLE AND MAY BE LOCATED ON THE SIDE OR END OF THE VAULT. THE FLOW CONTROL WALL MAY BE TURNED TO ACCOMODATE OUTLET PIPE KNOCKOUTS ON THE SIDE OF THE VAULT.

INSTALLATION NOTES

- ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE VORTECHS STRUCTURE (LIFTING CLUTCHES PROVIDED).
- CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
- CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.



THIS PRODUCT MAY BE PROTECTED BY THE FOLLOWING
U.S. PATENT: 5,759,415, RELATED FOREIGN PATENTS

CONTECH
ENGINEERED SOLUTIONS LLC

www.ContechES.com

9025 Centre Pointe Dr., Suite 400, West Chester, OH 45069

800-338-1122 513-645-7000 513-645-7993 FAX

VORTECHS PC1319
STANDARD DETAIL

Transmittal



Engineers, Planners & Landscape Architects

ATTN: Emmanuelle Van Rutten, Director	DATE: January 18 th , 2018
MORIYAMA & TESHIMA ARCHITECTS 109 Murray Street, Unit 3, Ottawa, ON K1N 5M5 613 562 2908 x194	OUR FILE No: P17173
	RE: OPS South Campus Security Clearance Application Forms
FROM: François Thauvette, P. Eng.	Return to Sender: Yes: No: X

Non Rush:	Rush:	Hot-shot:	Gold:	Over-night:	Pick-up:	Mail:	Hand Deliver: X
------------------	--------------	------------------	--------------	--------------------	-----------------	--------------	------------------------

COMMENTS

Hi Emmanuelle,

As requested, please find enclosed the completed OPS Security Application Forms for the following Novatech staff:

First submission (priority)

- François Simon Thauvette (Senior PM/Civil Eng.)
- Stephen Charles Matthews (Senior Design Technologist)
- J. Lee Sheets (Director)
- Lindsey Katherine Seely (CAD Technologist)

Second submission

- Daniel Gerald Vaughan (Manager - Construction Services)
- Steven Michael Lopes (Inspector)

We may add others to the list, if necessary.

Regards,

François Thauvette, P. Eng., Senior Project Manager | Land Development & Public-Sector Engineering

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 219 | Cell: 613.276.0310 | Fax: 613.254.5867

Appendix E

Burnett Municipal Drain Analysis

MEMORANDUM

DATE: JUNE 10, 2016
TO: BOB DOWDELL, E.I.T. & EDSON DONNELLY, C.E.T.
FROM: CONRAD STANG, M.A.SC., P.ENG.
RE: SOUTH NEPEAN COLLECTOR CULVERT CROSSINGS
CC: MIKE PETEPIECE, P.ENG.

This technical memorandum provides details on the sizing and location of the proposed access road culvert crossings which will be installed as part of Phase 2 of the South Nepean Collector (SNC). It is anticipated that the proposed culverts will remain in place until such time that the subject lands are developed. The location of the proposed culvert crossings and corresponding drainage areas are shown on the attached figure (DSK54):

- Culvert C1: Burnett Municipal Drain
- Culvert C2: Ditch draining Mion property west of K-B SWMF
- Culvert C3: Ditch adjacent the K-B SWMF

Design Criteria

The culverts are to convey the 10-year peak flows from their respective upstream drainage areas without overtopping the access road, as per Section 6.4.2 of the City of Ottawa Sewer Design Guidelines (October, 2012). For the 600mm culverts this corresponds to a Headwater / Depth (HW/D) of 1.5 (300mm cover), which corresponds to the maximum HW/D ratio as recommended by MTO. Excess flows will overtop the access road and graded back towards the downstream watercourse, as shown in the detail on the attached figure (DSK54).

Design Flows

The culvert crossings have been designed based on current City of Ottawa standards and rainfall data. Peak flows were estimated using the Visual Otthymo hydrologic model; modeling parameters and results are attached. The 12-hour SCS distribution generated the highest peak flows and was selected as the critical storm distribution for sizing the proposed culverts. Simulated peak flows at the proposed culvert crossings are provided in Table 1.

Table 1: Simulated Peak Flows at Proposed Culvert Crossings

Culvert	Culvert Dimensions	Return Period (years)	Peak Flow (m ³ /s)	HW/D (m)	Freeboard (m)
Culvert C1 Burnett Municipal Drain (29.27 ha)	3x 600mm Dia. CSP Culverts L = 8.0m S = 0.75% *Inv. = 91.10m	2-year	0.63	0.82	0.41
		5-year	1.10	1.13	0.22
		10-year	1.44	1.37	0.08
		25-year	1.90	1.65	0.09 Overtopping
		50-year	2.26	1.75	0.15 Overtopping
		100-year	2.66	1.84	0.20 Overtopping
Culvert C2 Mion/Pavic Ditch (5.11 ha)	1x 600mm Dia. CSP Culvert L = 8.0m S = 0.50% *Inv. = 92.30m	2-year	0.09	0.46	0.63
		5-year	0.16	0.64	0.51
		10-year	0.21	0.77	0.44
		25-year	0.28	0.93	0.34
		50-year	0.34	1.07	0.26
		100-year	0.40	1.21	0.17
Culvert C3 KB-SWMF Ditch (1.36 ha)	1x 1000mm Dia. CSP Culvert L = 12.0m S = 0.60% *Inv. = 92.35m	2-year	0.04	0.15	1.85
		5-year	0.07	0.20	1.80
		10-year	0.09	0.23	1.77
		25-year	0.12	0.26	1.74
		50-year	0.15	0.30	1.70
		100-year	0.17	0.32	1.68

*Inverts to be confirmed in the field.

Culvert Crossings

The proposed culvert crossings were designed using Autodesk Hydraflow Express culvert sizing software – supporting calculations are attached. A summary of the proposed culvert crossings is provided below.

Culvert C1: Burnett Municipal Drain (3x 600mm CSP Culverts)

The Burnett Municipal Drain was established in the late 1960's after the passing of By-Law No. 107-68 (Township of Nepean). A copy of the Burnett Municipal Drain By-Law and October 16th, 1968 Engineers report is attached. Land use in the watershed has changed substantially since the adoption of the By-Law and a significant portion of the upstream drainage area has been redirected to the Kennedy-Burnett SWM Facility. It is anticipated that this drain will be abandoned in the near future as development proceeds within the remaining undeveloped areas south of Strandherd Drive.

The Burnett Municipal Drain is a trapezoidal channel with a 3m bottom width, 0.60m depth and side slopes ranging from 2:1 to 4:1. Based on Manning's equation, the Burnett Municipal Drain has a bankfull capacity of 3.88 m³/s, which is sufficient to convey the 100-year peak flow from the current upstream drainage area (which is considerably smaller than the original drainage area from the 1968 Engineer's Report).

As per the 1968 by-law, an existing 900mm diameter CSP culvert was installed downstream of the proposed SNC crossing to provide access to the Kelvin Burnett property. The proposed crossing should therefore provide at least a similar flow capacity (approximately 900 L/s).

Based on current design standards, the proposed culvert crossing should be three (3) 600mm diameter CSP culverts, which will provide capacity for a 5-year return period flowing full and a 10-

year return period without over topping the access road. Refer to the attached detailed calculations. Storm events greater than the 10-year return period will overtop the access road, but the grading will confine excess flows to the downstream ditch. The middle 600mm CSP culvert will be countersunk 0.10m.

Culvert C2: Mion Ditch (600mm CSP Culvert)

The west watercourse is an intermittent ditch that runs north-to-south through the Mion property. The ditch is a V-bottom ditch with a 0.30m depth and 3:1 / 6:1 side slopes. Based on Manning's equation, the capacity of the ditch is 0.23 m³/s, which corresponds to a 10-year storm.

A 600mm diameter CSP culvert has capacity to convey storm events up-to and including the 100-year storm event (0.40 m³/s) without overtopping the access road. Refer to the attached detailed calculations. The 600mm CSP culvert will be countersunk 0.10m.

Culvert C3: KB-SWMF Ditch (1000mm CSP Culvert)

The watercourse west of the KB-SWMF is a deep intermittent ditch that runs north-to-south adjacent the K-B SWMF. The ditch is a V-bottom ditch with a 2.0m depth and 2.5:1 side slopes. Based on Manning's equation, the capacity of the ditch is 21.06 m³/s. The estimated 100-year peak flows from the 1.36 ha catchment are 0.17 m³/s; therefore, the ditch has ample capacity to convey large peak flows.

Due to the dimensions of the ditch, a 1000mm diameter CSP culvert is recommended. The capacity of the culvert (HW/D=1.0) is 1.10 m³/s. Flows should not overtop the culvert, but if they do they will spill into the KB-SWMF. Refer to the attached detailed calculations.

Access Road

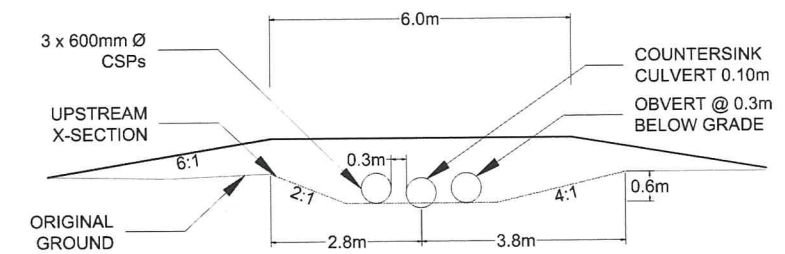
For culvert crossings C1 and C2, the 6.0m wide access road will be graded adjacent the culvert crossing in order to have two (2) 3.0m wide depressions, which will act as a weir for flows in excess of the culvert capacity. Refer to the cross-section detail on the attached Figure (DSK54).

Attachments:

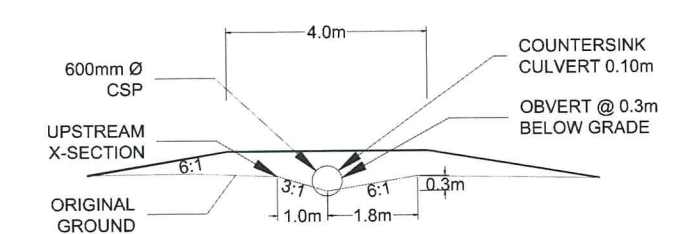
- Culvert Locations and Sizes (Figure DSK54)
- Visual Otthymo Modeling Parameters and Results
- Detailed Culvert and Ditch Calculations
- Burnett Municipal Drain By-Law



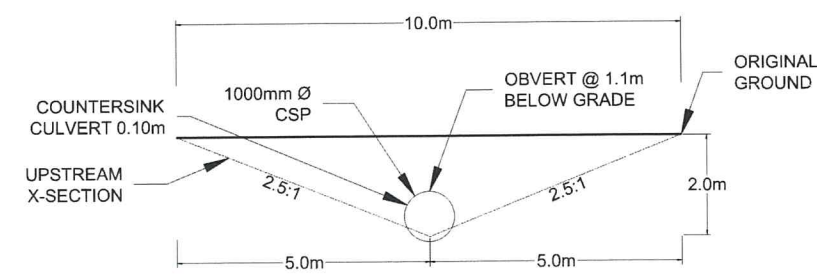
20151115075\CADD\Design\Figures\DSK54_Culverts.dwg, SNC, Jun 07, 2016 - 10:01am, nsmi



**TEMPORARY CROSSING
CROSS SECTION A-A' (C1)**
SCALE: 1:150



**TEMPORARY CROSSING
CROSS SECTION B-B' (C2)**
SCALE: 1:150



**TEMPORARY CROSSING
CROSS SECTION C-C' (C3)**
SCALE: 1:150

LEGEND

- CULVERT
- WATERCOURSE
- MUNICIPAL DRAIN
- PROPOSED ALIGNMENT OF THE SOUTH NEPEAN COLLECTOR (SNC)
- DRAINAGE AREA BOUNDARY
- CATCHMENT AREA (ha)
AREA ID
RUNOFF CURVE NUMBER

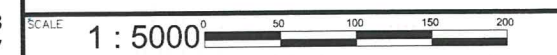


Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
Facsimile (613) 254-5867
Website www.novatech-eng.com

**SOUTH NEPEAN COLLECTOR
SEWER PHASE 2**

**REQUIRED CULVERT
LOCATIONS AND SIZES**



DATE JUN 2016 | JOB 115075 | FIGURE DSK54

South Nepean Collector Phase 2: Culvert Crossings
Existing Conditions Hydrologic Model Parameters and Results
115075



Time to Peak Calculations
(Uplands Overland Flow Method)

Area ID	Area (ha)	CN	Ia	Overland Flow						Concentrated Overland Flow						Channel Flow				Overall			
				Length (m)	Elevation U/S (m)	Elevation D/S (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Length (m)	Elevation U/S (m)	Elevation D/S (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Length (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Time of Concentration (min)	Time to Peak (min)	Time to Peak (min)	Time to Peak (hrs)
C1: Burnett MD	29.27	80	6.4	100	95.50	94.50	1.0%	0.28	5.95	150	94.50	92.75	1.2%	0.30	8.33	400	0.7%	0.38	18	32	21	21	0.36
C2: West Ditch	5.11	77	7.6	100	93.30	93.00	0.3%	0.15	11.11	125	93.00	92.75	0.2%	0.13	16.03	150	0.5%	0.32	8	35	23	23	0.39
C3: KB SWMF Ditch	1.36	79	6.8	50	93.35	93.15	0.4%	0.17	4.90	25	93.15	93.05	0.4%	0.16	2.60	150	0.6%	0.35	7	15	10	10	0.17

$Ia = 0.10 \times S$

Model Results: 12-hour SCS Storm Distribution

Area ID	Peak Flow (m ³ /s) (Return Period)					
	2-year	5-year	10-year	25-year	50-year	100-year
C1: Burnett MD	0.63	1.10	1.44	1.90	2.26	2.66
C2: West Ditch	0.09	0.16	0.21	0.28	0.34	0.40
C3: KB SWMF Ditch	0.04	0.07	0.09	0.12	0.15	0.17

Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

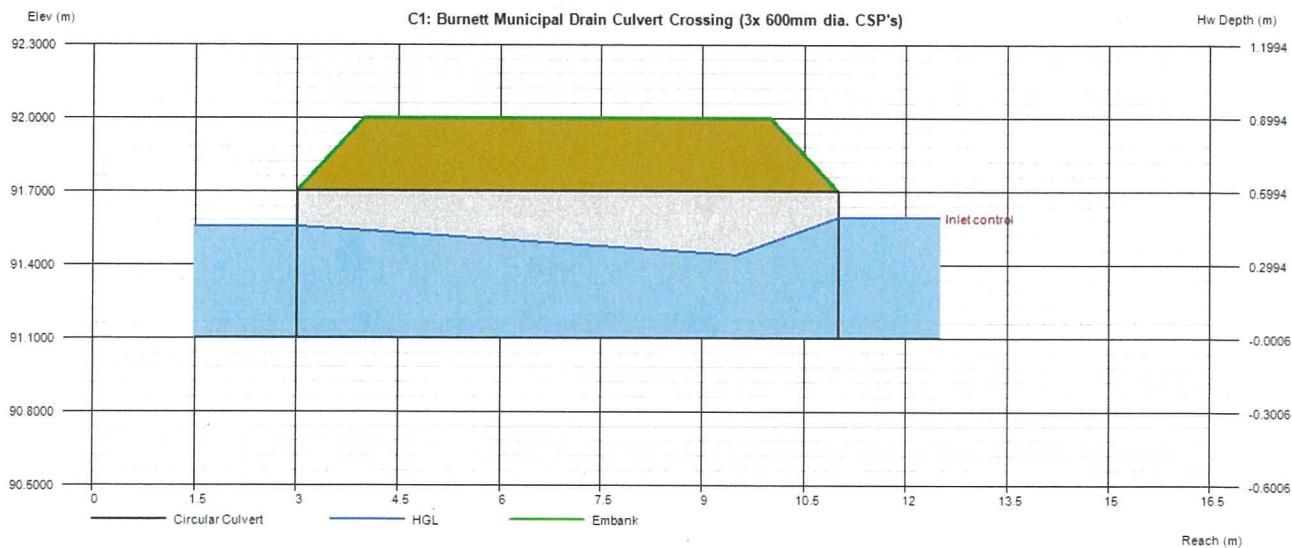
C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 2.0000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 0.7000
Qpipe (cms)	= 0.7000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.0109
Veloc Up (m/s)	= 1.5645
HGL Dn (m)	= 91.5565
HGL Up (m)	= 91.4135
Hw Elev (m)	= 91.5931
Hw/D (m)	= 0.8208
Flow Regime	= Inlet Control



Culvert Report

5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m) = 91.1000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0076
 Invert Elev Up (m) = 91.1006
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 3
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

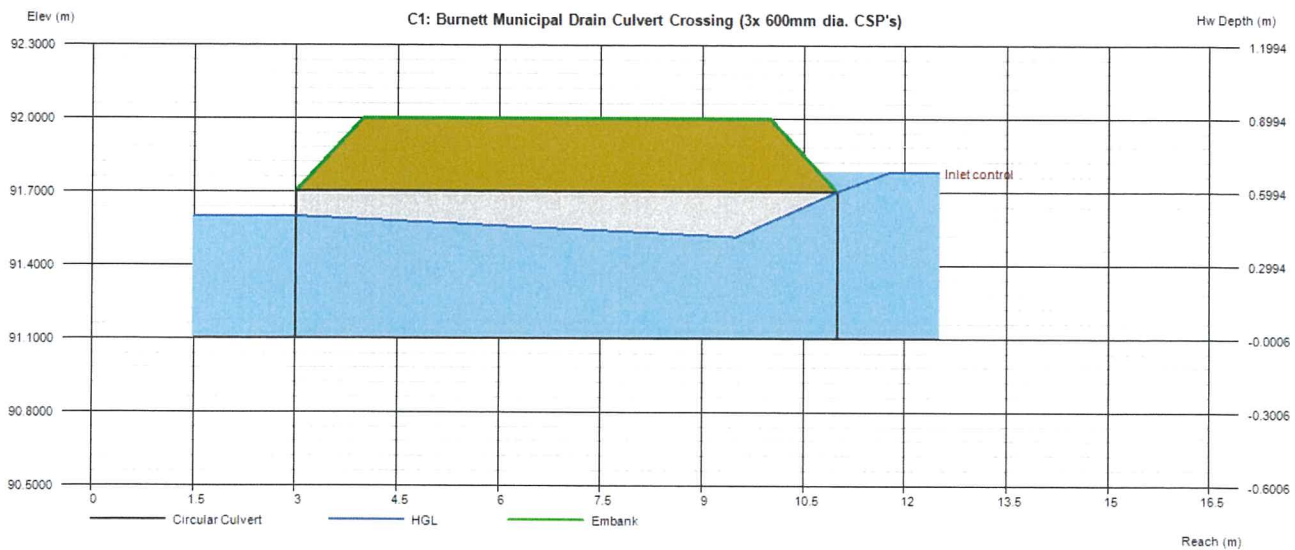
Top Elevation (m) = 92.0000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations

Qmin (cms) = 0.0000
 Qmax (cms) = 2.0000
 Tailwater Elev (m) = (dc+D)/2

Highlighted

Qtotal (cms) = 1.1000
 Qpipe (cms) = 1.1000
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 1.4614
 Veloc Up (m/s) = 1.8514
 HGL Dn (m) = 91.5981
 HGL Up (m) = 91.4967
 Hw Elev (m) = 91.7793
 Hw/D (m) = 1.1311
 Flow Regime = Inlet Control



Culvert Report

10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

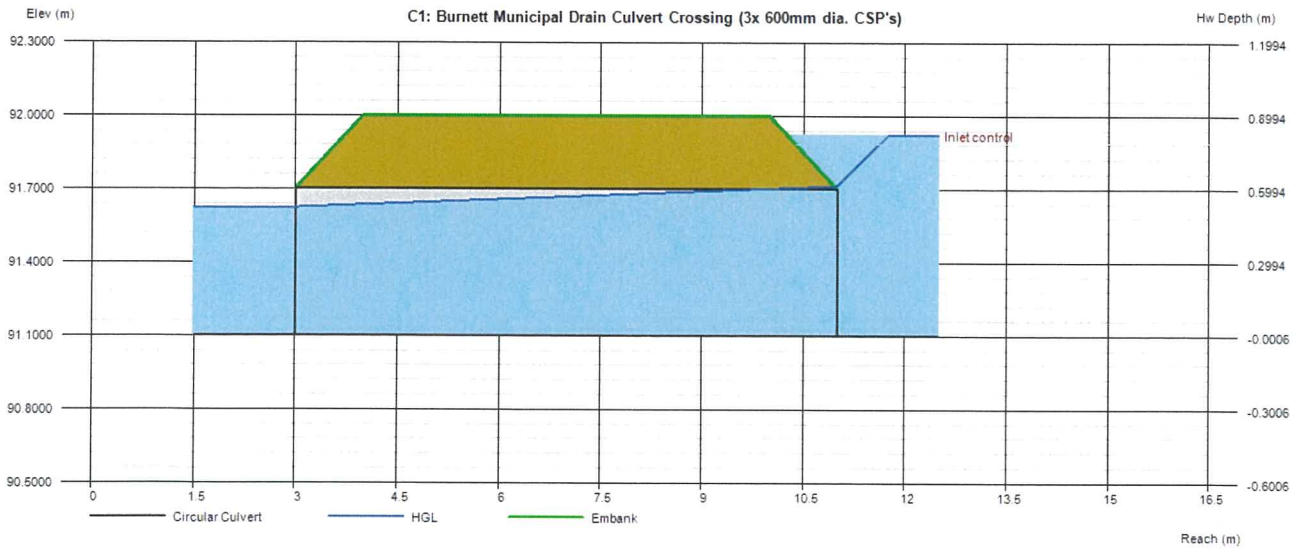
C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m) = 91.1000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0076
 Invert Elev Up (m) = 91.1006
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 3
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment
 Top Elevation (m) = 92.0000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations
 Qmin (cms) = 0.0000
 Qmax (cms) = 2.0000
 Tailwater Elev (m) = (dc+D)/2

Highlighted
 Qtotal (cms) = 1.4000
 Qpipe (cms) = 1.4000
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 1.7823
 Veloc Up (m/s) = 1.6505
 HGL Dn (m) = 91.6237
 HGL Up (m) = 91.7136
 Hw Elev (m) = 91.9199
 Hw/D (m) = 1.3655
 Flow Regime = Inlet Control



Culvert Report

25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

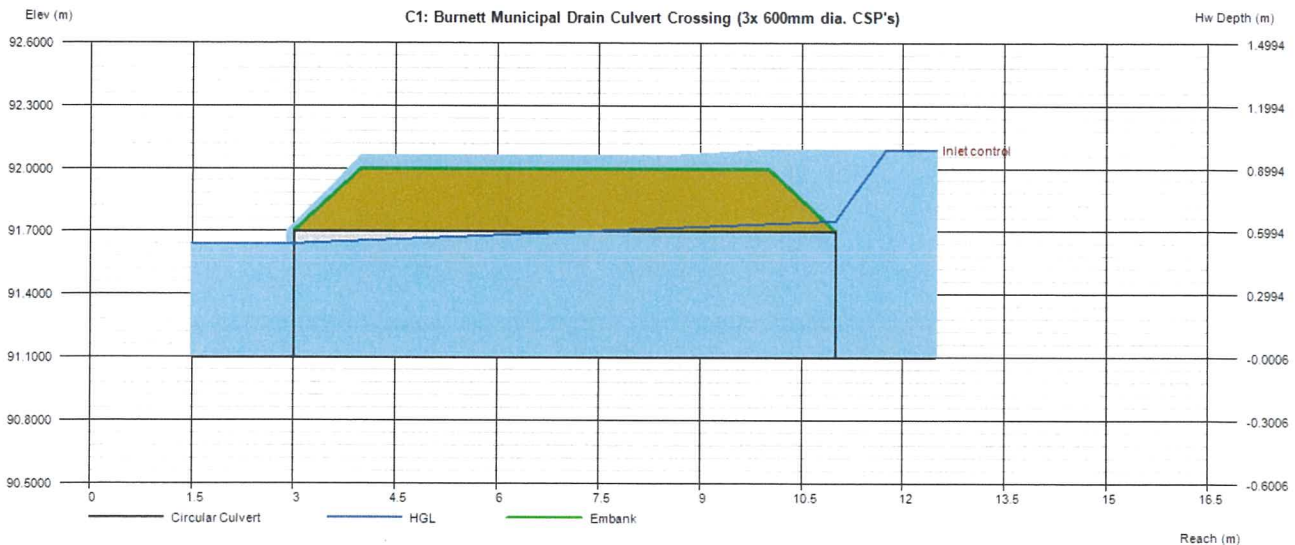
C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 2.0000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 1.9000
Qpipe (cms)	= 1.6221
Qovertop (cms)	= 0.2779
Veloc Dn (m/s)	= 2.0174
Veloc Up (m/s)	= 1.9123
HGL Dn (m)	= 91.6400
HGL Up (m)	= 91.7494
Hw Elev (m)	= 92.0880
Hw/D (m)	= 1.6457
Flow Regime	= Inlet Control



Culvert Report

50-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

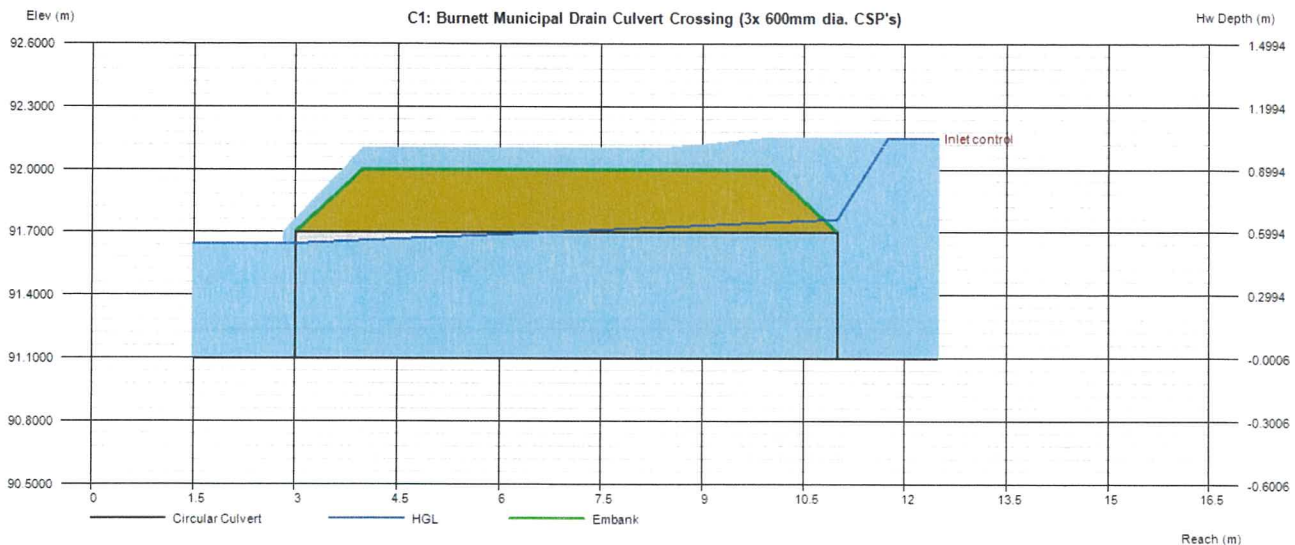
Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations

Qmin (cms)	= 2.0000
Qmax (cms)	= 3.0000
Tailwater Elev (m)	= (dc+D)/2

Highlighted

Qtotal (cms)	= 2.3000
Qpipe (cms)	= 1.6952
Qovertop (cms)	= 0.6048
Veloc Dn (m/s)	= 2.0950
Veloc Up (m/s)	= 1.9985
HGL Dn (m)	= 91.6448
HGL Up (m)	= 91.7612
Hw Elev (m)	= 92.1492
Hw/D (m)	= 1.7477
Flow Regime	= Inlet Control



Culvert Report

100-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

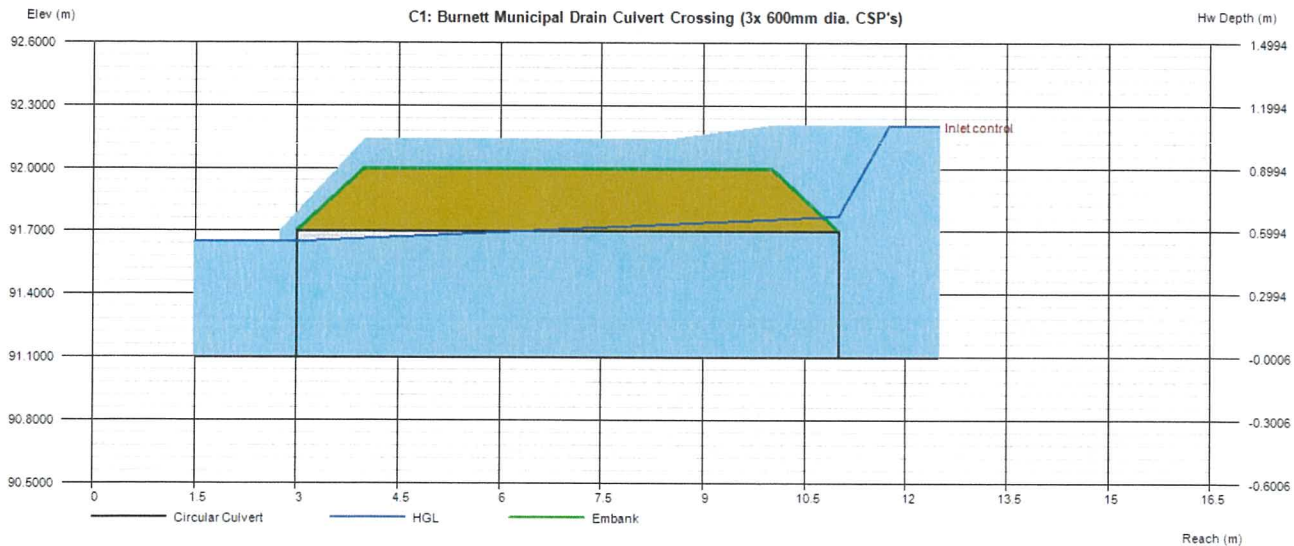
C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 2.0000
Qmax (cms)	= 3.0000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 2.7000
Qpipe (cms)	= 1.7556
Qovertop (cms)	= 0.9444
Veloc Dn (m/s)	= 2.1591
Veloc Up (m/s)	= 2.0697
HGL Dn (m)	= 91.6486
HGL Up (m)	= 91.7709
Hw Elev (m)	= 92.2017
Hw/D (m)	= 1.8352
Flow Regime	= Inlet Control



Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

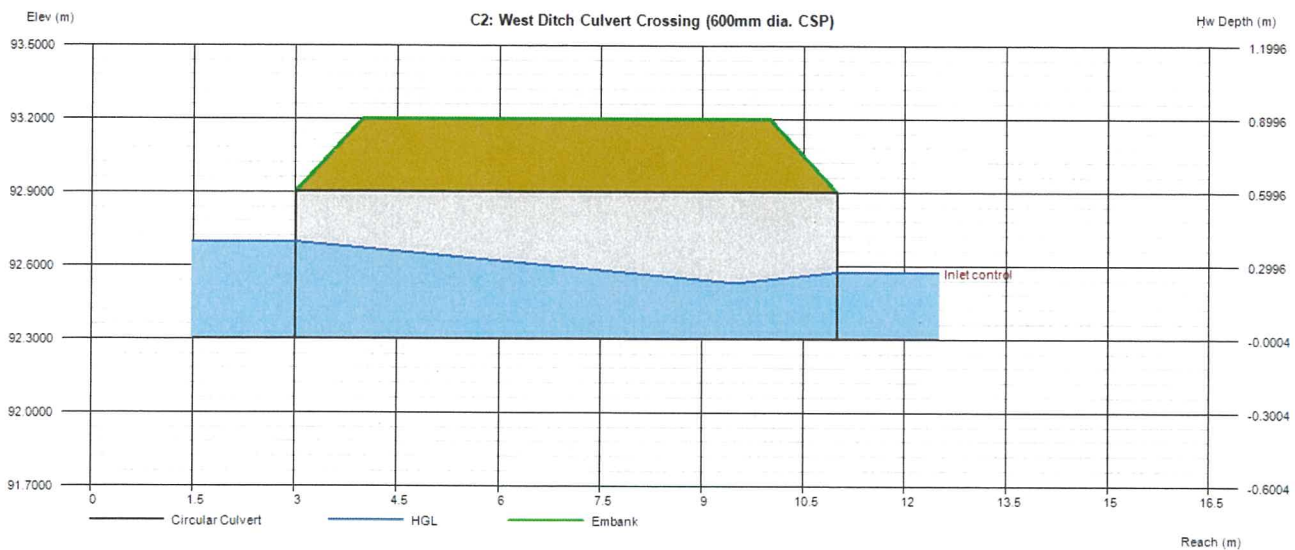
C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0049
 Invert Elev Up (m) = 92.3004
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 1
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment
 Top Elevation (m) = 93.2000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations
 Qmin (cms) = 0.0000
 Qmax (cms) = 0.4000
 Tailwater Elev (m) = (dc+D)/2

Highlighted
 Qtotal (cms) = 0.0900
 Qpipe (cms) = 0.0900
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 0.4555
 Veloc Up (m/s) = 1.1656
 HGL Dn (m) = 92.6953
 HGL Up (m) = 92.4910
 Hw Elev (m) = 92.5734
 Hw/D (m) = 0.4550
 Flow Regime = Inlet Control



Culvert Report

5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

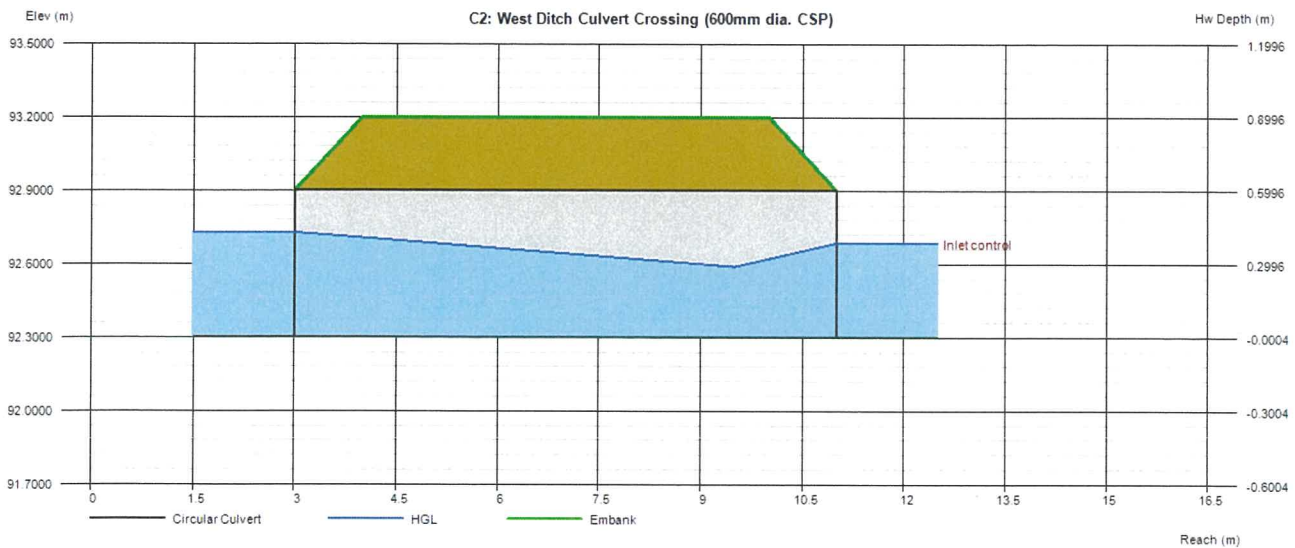
C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0049
 Invert Elev Up (m) = 92.3004
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 1
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment
 Top Elevation (m) = 93.2000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations
 Qmin (cms) = 0.0000
 Qmax (cms) = 0.4000
 Tailwater Elev (m) = (dc+D)/2

Highlighted
 Qtotal (cms) = 0.1600
 Qpipe (cms) = 0.1600
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 0.7407
 Veloc Up (m/s) = 1.3844
 HGL Dn (m) = 92.7284
 HGL Up (m) = 92.5573
 Hw Elev (m) = 92.6860
 Hw/D (m) = 0.6428
 Flow Regime = Inlet Control



Culvert Report

10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

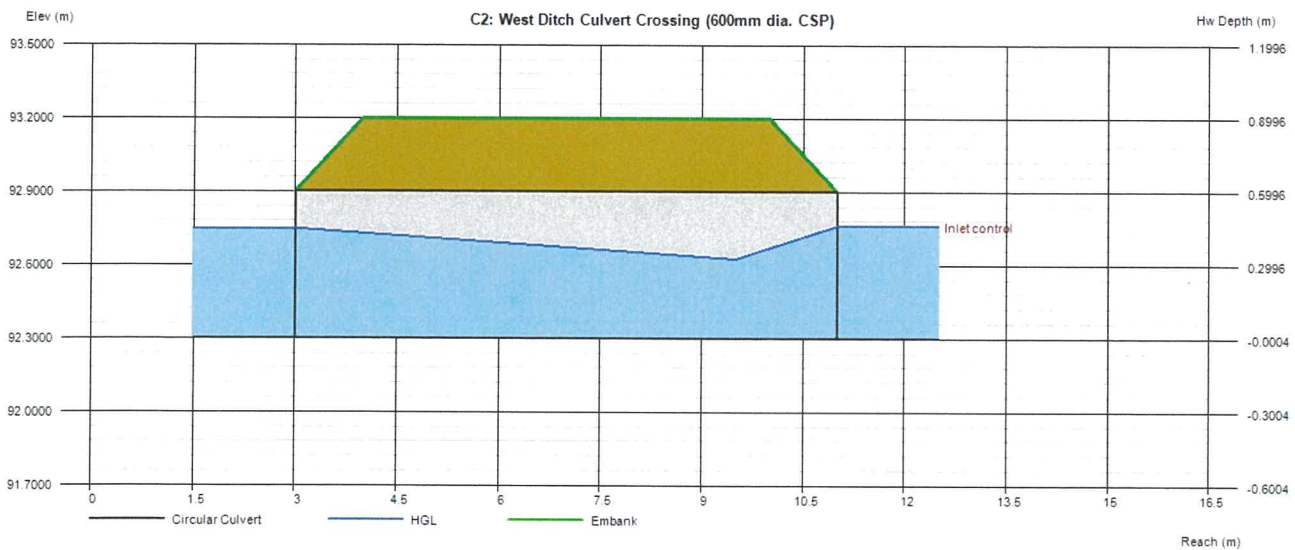
C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0049
Invert Elev Up (m)	= 92.3004
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 1
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 93.2000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.4000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 0.2100
Qpipe (cms)	= 0.2100
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.9273
Veloc Up (m/s)	= 1.5101
HGL Dn (m)	= 92.7481
HGL Up (m)	= 92.5966
Hw Elev (m)	= 92.7595
Hw/D (m)	= 0.7652
Flow Regime	= Inlet Control



Culvert Report

25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

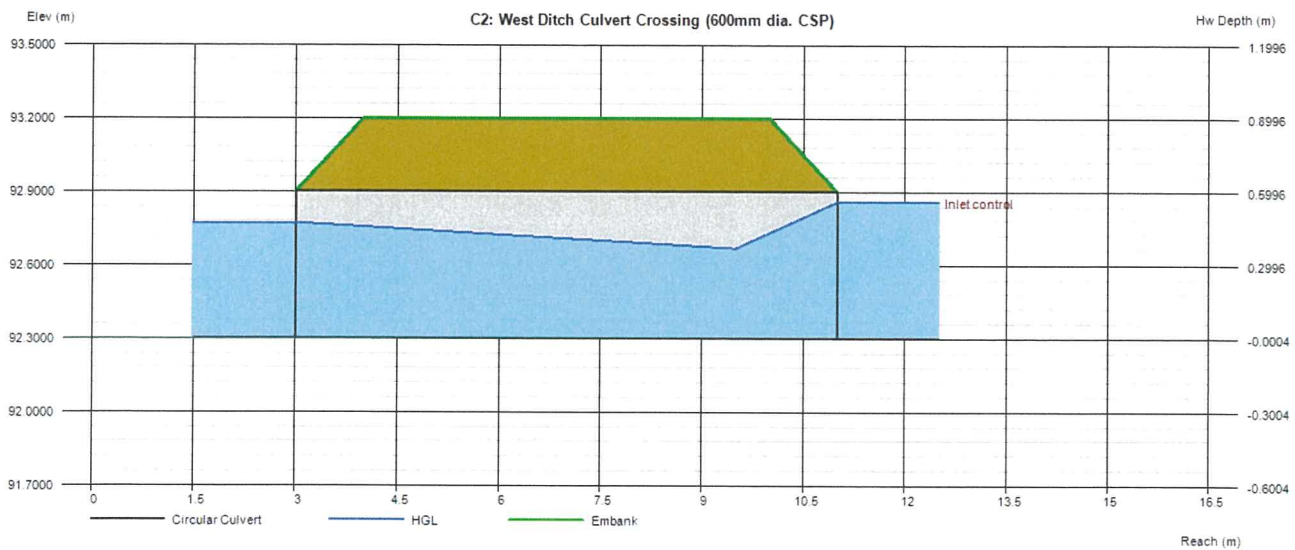
C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0049
 Invert Elev Up (m) = 92.3004
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 1
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment
 Top Elevation (m) = 93.2000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations
 Qmin (cms) = 0.0000
 Qmax (cms) = 0.4000
 Tailwater Elev (m) = (dc+D)/2

Highlighted
 Qtotal (cms) = 0.2800
 Qpipe (cms) = 0.2800
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 1.1731
 Veloc Up (m/s) = 1.6682
 HGL Dn (m) = 92.7722
 HGL Up (m) = 92.6447
 Hw Elev (m) = 92.8585
 Hw/D (m) = 0.9302
 Flow Regime = Inlet Control



Culvert Report

50-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0049
 Invert Elev Up (m) = 92.3004
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 1
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

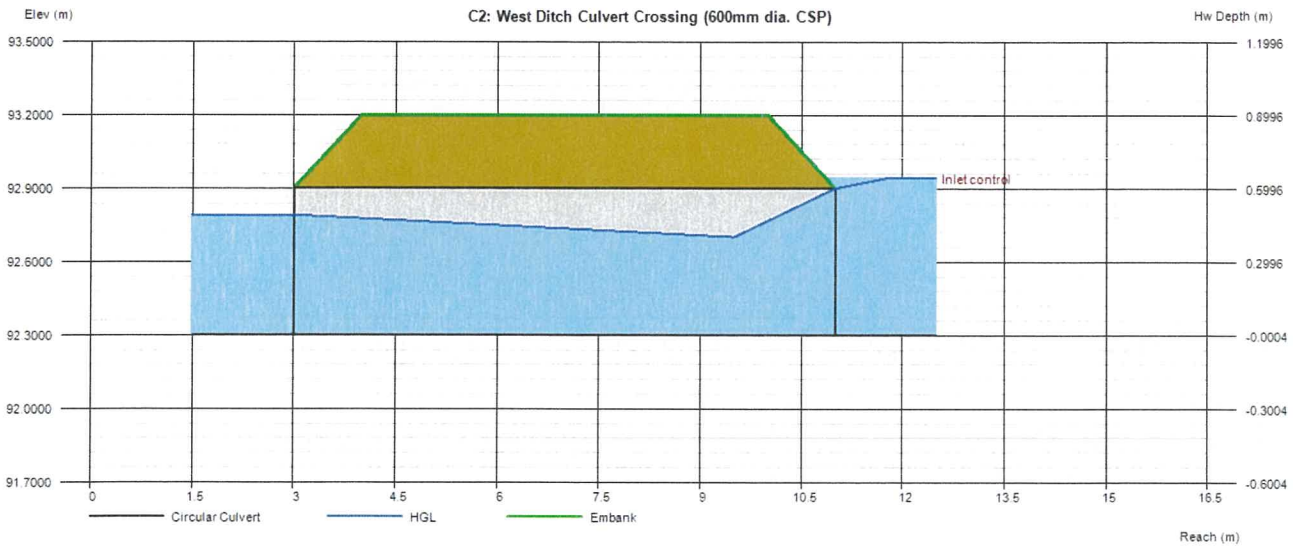
Embankment
 Top Elevation (m) = 93.2000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations

Qmin (cms) = 0.0000
 Qmax (cms) = 0.4000
 Tailwater Elev (m) = (dc+D)/2

Highlighted

Qtotal (cms) = 0.3400
 Qpipe (cms) = 0.3400
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 1.3741
 Veloc Up (m/s) = 1.7957
 HGL Dn (m) = 92.7905
 HGL Up (m) = 92.6813
 Hw Elev (m) = 92.9420
 Hw/D (m) = 1.0693
 Flow Regime = Inlet Control



Culvert Report

100-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000
 Pipe Length (m) = 8.0000
 Slope (%) = 0.0049
 Invert Elev Up (m) = 92.3004
 Rise (mm) = 600.0
 Shape = Circular
 Span (mm) = 600.0
 No. Barrels = 1
 n-Value = 0.016
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

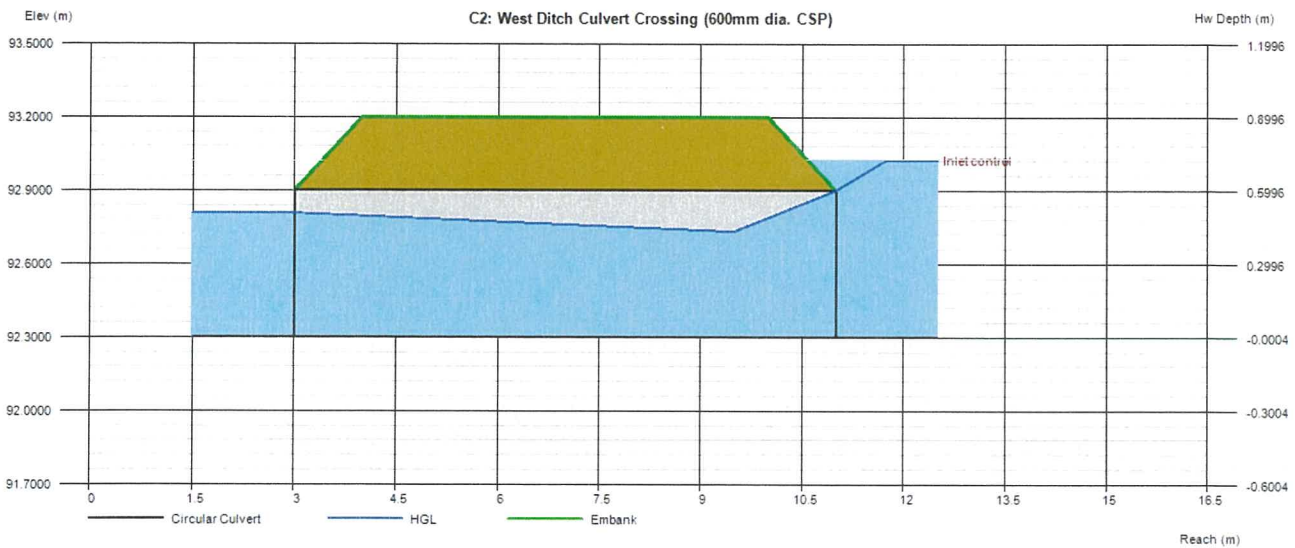
Embankment
 Top Elevation (m) = 93.2000
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000

Calculations

Qmin (cms) = 0.0000
 Qmax (cms) = 0.4000
 Tailwater Elev (m) = (dc+D)/2

Highlighted

Qtotal (cms) = 0.4000
 Qpipe (cms) = 0.4000
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 1.5694
 Veloc Up (m/s) = 1.9217
 HGL Dn (m) = 92.8071
 HGL Up (m) = 92.7145
 Hw Elev (m) = 93.0256
 Hw/D (m) = 1.2087
 Flow Regime = Inlet Control



Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

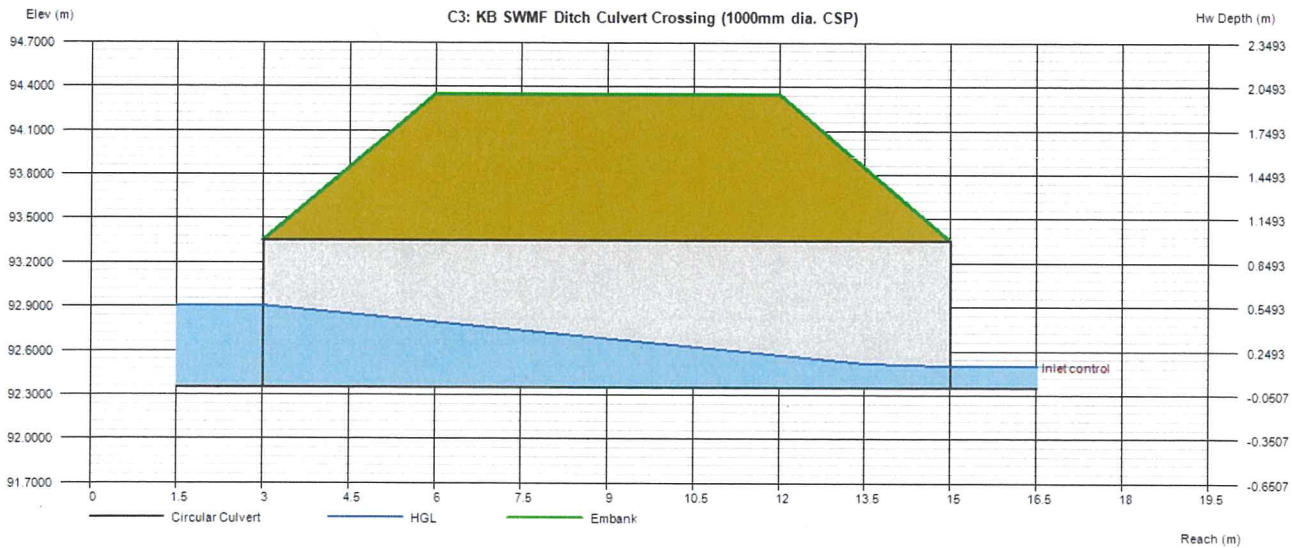
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	=	92.3500
Pipe Length (m)	=	12.0000
Slope (%)	=	0.0058
Invert Elev Up (m)	=	92.3507
Rise (mm)	=	1000.0
Shape	=	Circular
Span (mm)	=	1000.0
No. Barrels	=	1
n-Value	=	0.020
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Projecting
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 0.0400
Qpipe (cms)	= 0.0400
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.0894
Veloc Up (m/s)	= 0.8567
HGL Dn (m)	= 92.9048
HGL Up (m)	= 92.4602
Hw Elev (m)	= 92.4986
Hw/D (m)	= 0.1479
Flow Regime	= Inlet Control



Culvert Report

5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

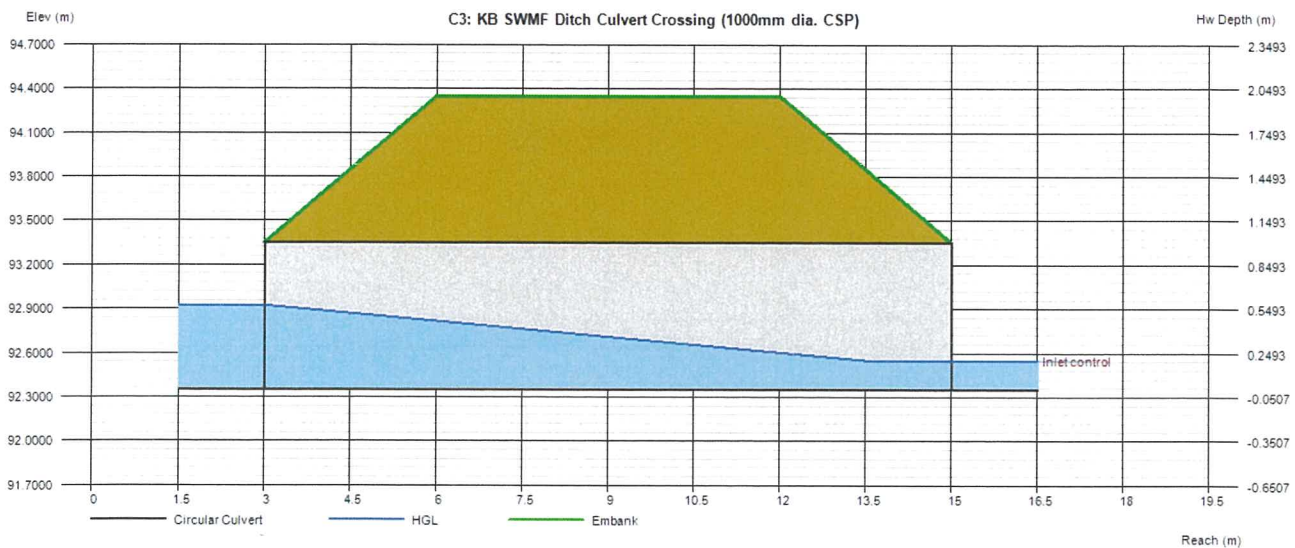
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m) = 92.3500
 Pipe Length (m) = 12.0000
 Slope (%) = 0.0058
 Invert Elev Up (m) = 92.3507
 Rise (mm) = 1000.0
 Shape = Circular
 Span (mm) = 1000.0
 No. Barrels = 1
 n-Value = 0.020
 Culvert Type = Circular Corrugate Metal Pipe
 Culvert Entrance = Projecting
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

Calculations
 Qmin (cms) = 0.0000
 Qmax (cms) = 0.2000
 Tailwater Elev (m) = (dc+D)/2

Highlighted
 Qtotal (cms) = 0.0700
 Qpipe (cms) = 0.0700
 Qovertop (cms) = 0.0000
 Veloc Dn (m/s) = 0.1505
 Veloc Up (m/s) = 0.9911
 HGL Dn (m) = 92.9227
 HGL Up (m) = 92.4961
 Hw Elev (m) = 92.5484
 Hw/D (m) = 0.1977
 Flow Regime = Inlet Control

Embankment
 Top Elevation (m) = 94.3500
 Top Width (m) = 6.0000
 Crest Width (m) = 6.0000



Culvert Report

10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

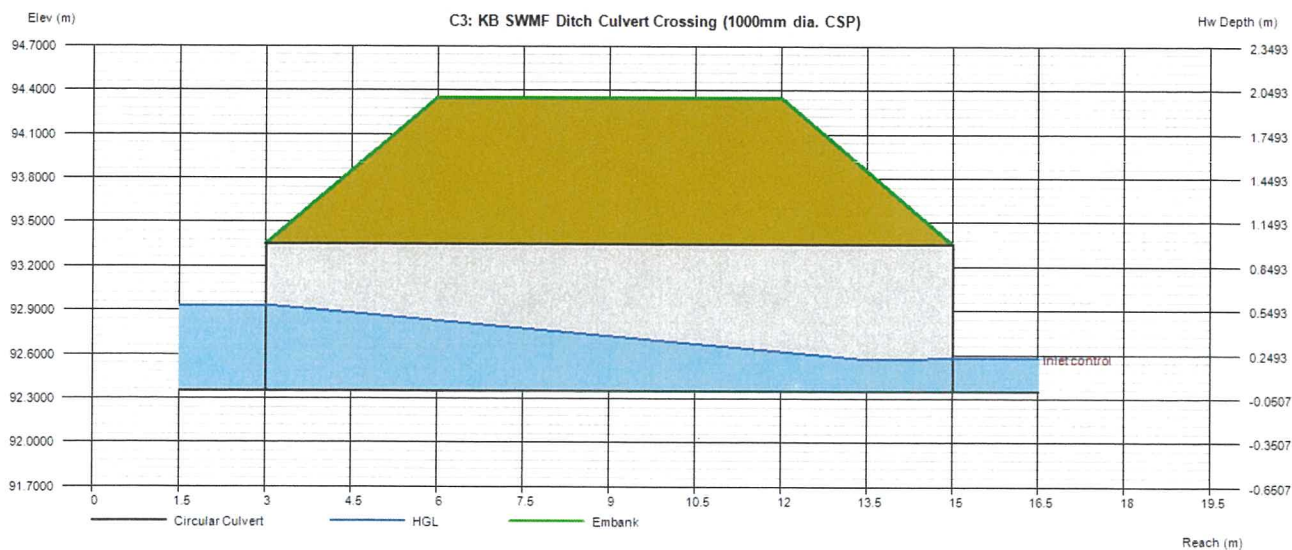
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 0.0900
Qpipe (cms)	= 0.0900
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.1895
Veloc Up (m/s)	= 1.0598
HGL Dn (m)	= 92.9326
HGL Up (m)	= 92.5159
Hw Elev (m)	= 92.5763
Hw/D (m)	= 0.2256
Flow Regime	= Inlet Control



Culvert Report

25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	=	92.3500
Pipe Length (m)	=	12.0000
Slope (%)	=	0.0058
Invert Elev Up (m)	=	92.3507
Rise (mm)	=	1000.0
Shape	=	Circular
Span (mm)	=	1000.0
No. Barrels	=	1
n-Value	=	0.020
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Projecting
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9

Calculations

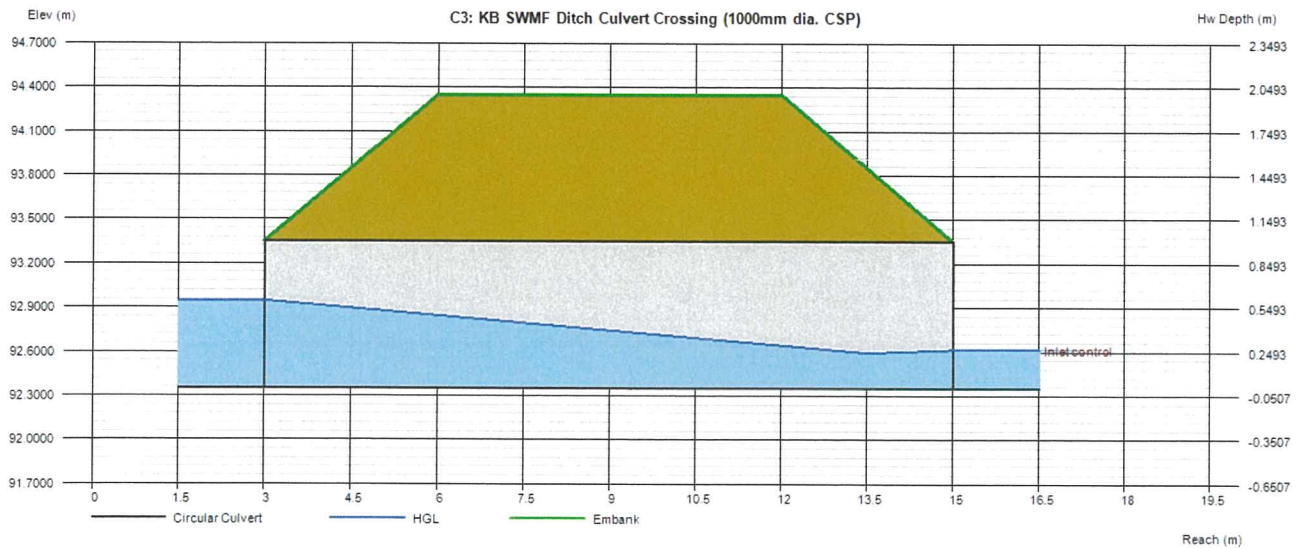
Qmin (cms)	=	0.0000
Qmax (cms)	=	0.2000
Tailwater Elev (m)	=	(dc+D)/2

Highlighted

Qtotal (cms)	=	0.1200
Qpipe (cms)	=	0.1200
Qovertop (cms)	=	0.0000
Veloc Dn (m/s)	=	0.2461
Veloc Up (m/s)	=	1.1449
HGL Dn (m)	=	92.9456
HGL Up (m)	=	92.5419
Hw Elev (m)	=	92.6136
Hw/D (m)	=	0.2629
Flow Regime	=	Inlet Control

Embankment

Top Elevation (m)	=	94.3500
Top Width (m)	=	6.0000
Crest Width (m)	=	6.0000



Culvert Report

50-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	=	92.3500
Pipe Length (m)	=	12.0000
Slope (%)	=	0.0058
Invert Elev Up (m)	=	92.3507
Rise (mm)	=	1000.0
Shape	=	Circular
Span (mm)	=	1000.0
No. Barrels	=	1
n-Value	=	0.020
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Projecting
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9

Calculations

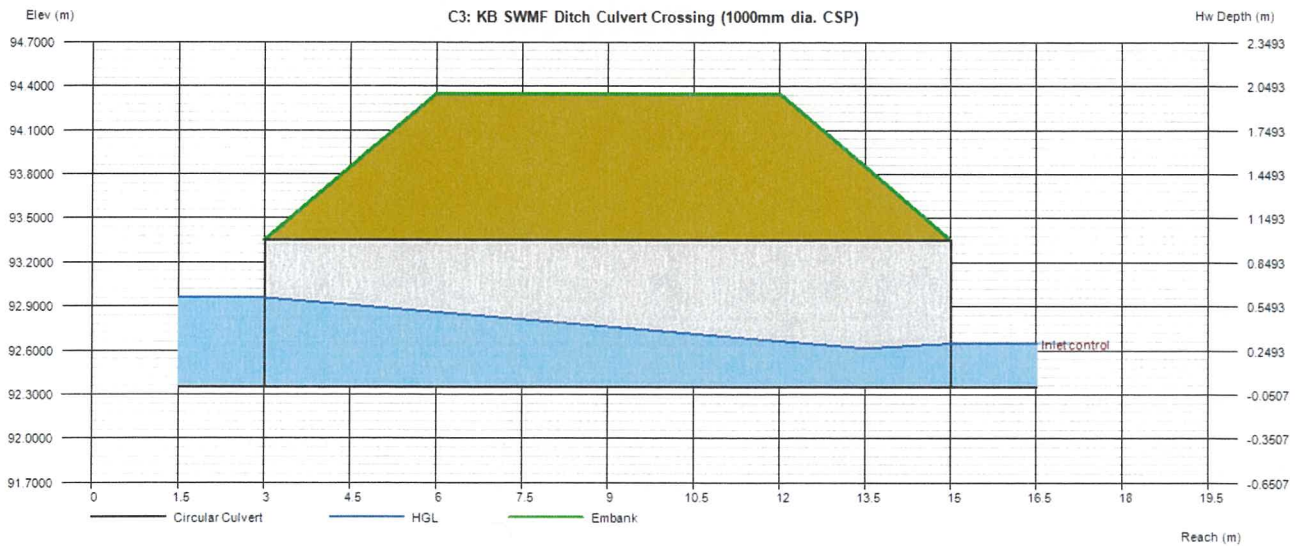
Qmin (cms)	=	0.0000
Qmax (cms)	=	0.2000
Tailwater Elev (m)	=	(dc+D)/2

Highlighted

Qtotal (cms)	=	0.1500
Qpipe (cms)	=	0.1500
Qovertop (cms)	=	0.0000
Veloc Dn (m/s)	=	0.3006
Veloc Up (m/s)	=	1.2156
HGL Dn (m)	=	92.9571
HGL Up (m)	=	92.5650
Hw Elev (m)	=	92.6472
Hw/D (m)	=	0.2965
Flow Regime	=	Inlet Control

Embankment

Top Elevation (m)	=	94.3500
Top Width (m)	=	6.0000
Crest Width (m)	=	6.0000



Culvert Report

100-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Calculations

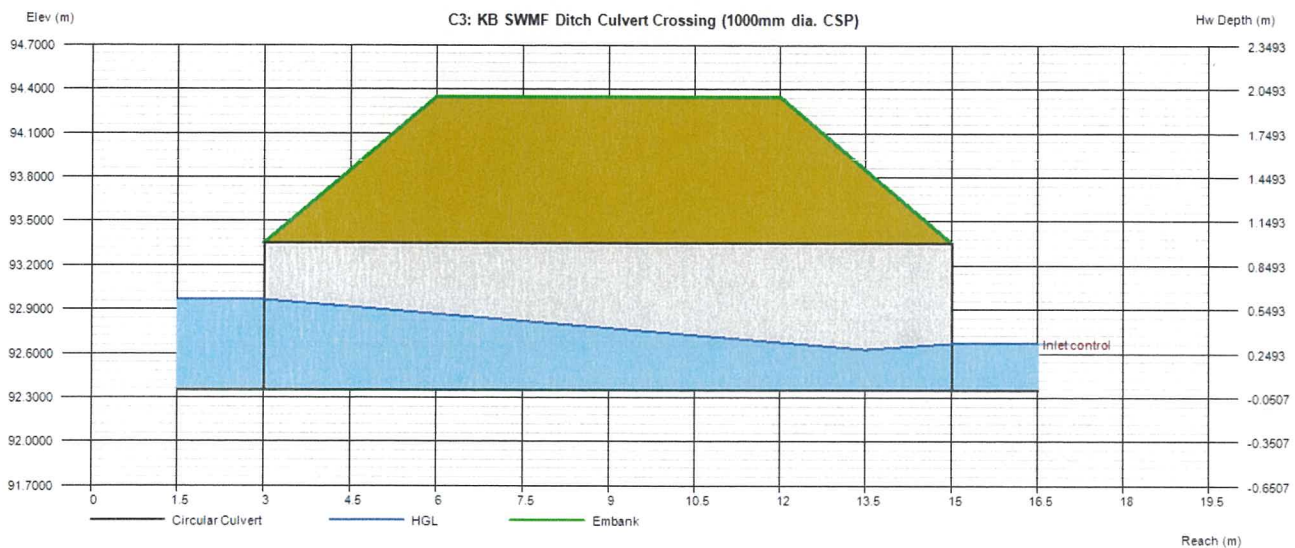
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2

Highlighted

Qtotal (cms)	= 0.1700
Qpipe (cms)	= 0.1700
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.3360
Veloc Up (m/s)	= 1.2582
HGL Dn (m)	= 92.9642
HGL Up (m)	= 92.5791
Hw Elev (m)	= 92.6682
Hw/D (m)	= 0.3175
Flow Regime	= Inlet Control

Embankment

Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000



Culvert Report

Full Flow Capacity (HW/D = 1.0)

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

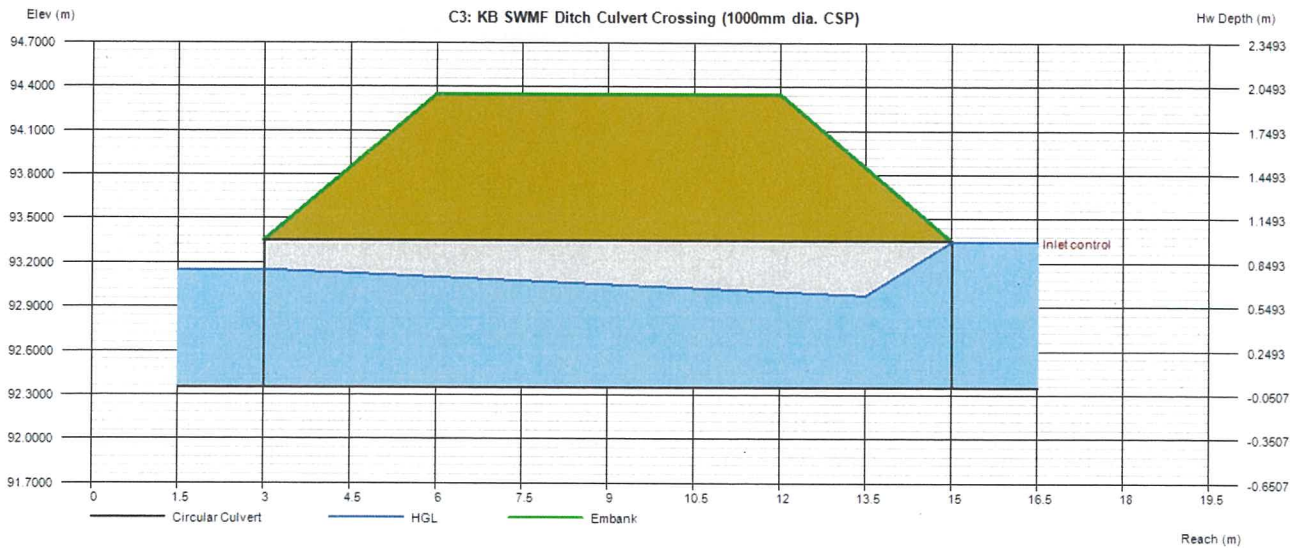
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	=	92.3500
Pipe Length (m)	=	12.0000
Slope (%)	=	0.0058
Invert Elev Up (m)	=	92.3507
Rise (mm)	=	1000.0
Shape	=	Circular
Span (mm)	=	1000.0
No. Barrels	=	1
n-Value	=	0.020
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Projecting
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9

Embankment	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 1.3000
Tailwater Elev (m)	= (dc+D)/2

Highlighted	
Qtotal (cms)	= 1.1000
Qpipe (cms)	= 1.1000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.6315
Veloc Up (m/s)	= 2.2284
HGL Dn (m)	= 93.1508
HGL Up (m)	= 92.9523
Hw Elev (m)	= 93.3430
Hw/D (m)	= 0.9923
Flow Regime	= Inlet Control



**South Nepean Collector Phase 2: Culvert Crossings
Ditch Capacities (Manning's Equation)**

C1: Burnett Municipal Drain		
Parameter	Units	Value
Depth	m	0.60
Bottom Width	m	3.00
Side slope (L)	1 to X	2.0
Side slope (R)	1 to X	4.0
Top Width (L)	m	1.20
Top Width (R)	m	2.40
Area	m ²	2.880
Perimeter	m	6.82
R=A/P	m	0.42
n		0.035
Slope	m/m	0.007
Q _{max}	m ³ /s	3.877
V _{max}	m/s	1.346

Trapezoidal Channel (different side slopes)

C2: West Ditch		
Parameter	Units	Value
Depth	m	0.30
Side slope (L)	1 to X	3.0
Side slope (R)	1 to X	6.0
Top Width (L)	m	0.90
Top Width (R)	m	1.80
Area	m ²	0.405
Perimeter	m	2.77
R=A/P	m	0.15
n		0.035
Slope	m/m	0.005
Q _{max}	m ³ /s	0.227
V _{max}	m/s	0.560

V-bottom ditch (different side slopes)

C3: KB SWMF Ditch		
Parameter	Units	Value
Depth	m	2.00
Side slope (L)	1 to X	2.5
Side slope (R)	1 to X	2.5
Top Width (L)	m	5.00
Top Width (R)	m	5.00
Area	m ²	10.000
Perimeter	m	10.77
R=A/P	m	0.93
n		0.035
Slope	m/m	0.006
Q _{max}	m ³ /s	21.063
V _{max}	m/s	2.106

V-bottom ditch (same side slopes)

CORPORATION OF THE TOWNSHIP OF NEPEAN

By-Law No. 107-68

(The Drainage Act, 1962-63, Section 27, Form 4)

A By Law to provide for Drainage Work in the Township of Nepean, in the County of Carleton and for borrowing on the credit of the Municipality the sum of Five Thousand, Six Hundred and Thirty-Three (\$5,633.00) Dollars, for completing the same.

WHEREAS the requisite number of owners, as shown by the last revised Assessment Roll, of the property hereinafter set forth requiring drainage have petitioned the Council of the said Township of Nepean praying that the following lands and roads may be drained by a drainage works.

Plan and Profile:

The accompanying plan, profile and specifications dated October 16th, 1968 are to form a part of this report. The plan will show the area of the watershed, and the drain. The profile and specifications will show the extent of work, the bench marks, grades and disposal of materials, etc.

Recommendation:

It is our recommendation that this drain be reconstructed from station 0 + 00 to station 15 + 31 where it enters the present road ditch. At this location the present road ditch will carry the water to the south side of the forced road where construction is to continue from station 19 + 98. The drain then flows east to the present railway culvert which is of sufficient size and the drain has been designed to flow through it at its present elevation. From this location the drain still flows easterly to the Township Road between lots 15 and 16, Concession 3 at station 43 + 70. The present culvert through this road is to be lowered to the designed grade for this location and from here the drain flows east to its outlet in the Jock River at station 86 + 25.

AND WHEREAS the Council has procured a report made by Alex J. Graham, C.E., hereto attached and marked Schedule "C" of By-Law No. 107-68.

AND WHEREAS the Council is of opinion that the drainage of the area described is desirable:

THEREFORE the Council of the Township of Nepean pursuant to The Drainage Act, 1962-63, enacts as follows:

1. The report is hereby adopted, and the drainage works as therein indicated and set forth are hereby authorized and shall be completed in accordance therewith.

2. The Corporation of the Township of Nepean may borrow on the credit of the Corporation the sum of \$5,000.00, being the funds necessary for the drainage works, not otherwise provided for, provided that such sum shall be reduced by the amount of grants and commuted payments with respect to the land and roads assessed, and may issue Debentures of the Corporation to that amount in sums of not less than Fifty Dollars each, and payable within ten years from the date of the said Debentures, with interest at the rate of 8 per centum per annum, that is to say annually with coupons.

3. For paying the sum of \$2,914.65 the amount charged against such lands and roads for benefit, and the sum of \$2,718.35 the amount charged against such lands and roads for outlet liability, and the sum of \$Nil the amount charged against such lands and roads for injuring liability, and the sum of Nil the amount charged against lands and roads for improvement, apart from lands and roads belonging to or controlled by the Municipality, and for covering interest thereon for ten years at the rate of 8 per centum per annum, the following total special rates, over and above all other rates, shall be assessed, levied, and collected (in the same manner and at the same time as other taxes are levied and collected) upon and from the under-mentioned parcels of land and parts of parcels and roads, and the amount of the total special rates and interest against each parcel or part of parcel respectively shall be divided into ten equal parts and one such part shall be assessed, levied and collected as aforesaid, in each year, for ten years after the passing of this by-law during which the Debentures have to run, provided that no greater amount shall be levied than is required after taking into account and crediting the amount of grants under subsection 3 of section 64 of The Drainage Act, 1962-63, the amount of moneys paid under a by-law passed under subsection 4 of section 40 of that Act and commuted payments with respect to lands and roads assessed.

4. For paying the sum of \$590.43 the amount assessed against such roads and lands of the Municipality, and for covering interest thereon for ten years at the rate of 8 per cent per annum, a special rate, sufficient to produce the required yearly amount therefor, shall, over and above all other rates, be levied and collected (in the same manner and at the same time as other taxes are levied and collected) upon and from the whole rateable property in the Township of Hopewell in each year for ten years, after the passing of this by-law, during which the Debentures have to run.

5. This by-law comes into force on the passing thereof and may be cited as the "Burnett Municipal Drain By-law".

First Reading December 9th, 1968.

Second Reading December 9th, 1968.

Third Reading January 6th, 1969.

Enacted this 6th day of January, 1969.

D. A. Moodie
D. A. Moodie Reeve

J. E. Hobbs
J. E. Hobbs Clerk

.....

.....

.....

.....

.....

.....

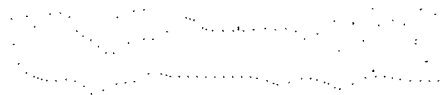
TOWNSHIP OF NEPEAN

BURNETT MUNICIPAL DRAIN

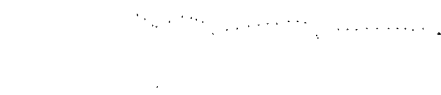
SCHEDULE OF ASSESSMENT

Lot	Name	Acres	Assessed	Main Drain	Benefit	Outlet	Total	Esti- mated	Grant	Balance	Payable	To Cover Interest For 10 yrs. at 8%	Total Special Rate	Annual Assessment During Each Year For 10 years
St 19	Don Fraser	12		296.18			198.18	132.19		66.06		30.36	98.41	9.84
St S. pt.	Lorne Burnett	43		517.25	549.77	74.28	1,067.02	711.31		355.71		174.29	530.00	53.00
S Pt.	Robert Mowat	6					74.28	49.54		24.74		12.13	36.87	3.69
St 16	Robert Mowat	6		99.84			196.09	130.72		65.37		30.05	97.42	9.74
St 17	Mrs. E. Monk	8		136.18	96.25		212.41	141.62		70.79		34.69	105.48	10.55
St 17	Mrs. E. Monk	8		149.37	76.23		310.50	206.99		103.51		50.72	154.23	15.42
St 17	Sale & Ricciotto	11		160.93	160.93		472.90	315.27		157.63		77.24	234.87	23.49
St 17	Carl Fraser	20		249.10	249.10		872.57	582.30		291.27		142.69	433.90	43.39
Rt 1/3 & Pt S2/3 L16	Patrick Moloughney	15		381.15	204.05	107.16	311.21	207.47		103.74		50.83	154.57	15.46
Rt 1/3 L16	Pequo Houlehen	49		204.05	204.05	73.98	278.03	185.35		92.68		45.41	138.09	13.81
Rt 1/3 L16	Carl Fraser	19		233.70	233.70	50.72	284.42	189.63		94.79		46.45	141.24	14.12
St 15	Mrs. Clerk	18		294.52	294.52	34.71	329.23	219.49		109.74		53.77	163.51	16.35
Pt. 27/15 N3/8 L. 24	Mrs. H. Houlehen	16		208.10	208.10	--	208.10	138.76		69.34		33.98	103.52	10.35
St 14	John Houlehen	13		--	--	--	--	--		--		--	--	--
Pt. E. 2	Kevin Burnett	9		122.82	122.82	254.64	377.36	--		--		--	--	--
Forced Road in Concession J		--		76.23	76.23	136.94	213.07	--		--		--	--	--
Township Road Between Lots 15 & 16		--		57.75	57.75	168.94	226.69	--		--		--	--	--
Canadian National Railways		--		\$2,914.65	\$2,914.65	\$2,718.35	\$5,633.00	\$3,210.58	\$1,605.30	\$784.71	\$2,391.91	\$239.19	\$2,391.91	\$239.19

Total \$2,914.65 \$2,718.35 \$5,633.00 \$3,210.58 \$1,605.30 \$784.71 \$2,391.91 \$239.19



ENGINEER'S REPORT
BURNETT MUNICIPAL DRAIN
TOWNSHIP OF NEPEAN



Graham, Berman and Associates Ltd.,
Consulting Engineers,
Ottawa 3, Ontario.

October 16th, 1968.



Richard C. A. [unclear]

October 16th, 1968.

The Reeve and Members of Council,
Township of Moness,
3895 Richmond Road,
Ottawa 6, Ontario.

Gentlemen:

In answer to the prayer of a petition of over half the property owners concerned, requesting that the Burnett Acre Ditch be reconstructed to relieve the present flooding conditions and provide adequate outlet for tile drains. This drain is to be reconstructed under section 3 of the Drainage Act 1962-63 and by reason of a motion passed in Council, I have caused a survey to be made, prepared plan, profile and schedule of assessment, and beg to report as follows:

The drainage works shall be known as the Burnett Municipal Drain.

Plan and Profile:

The accompanying plan, profile and specifications dated October 16th, 1968 are to form a part of this report. The plan will show the area of the watershed, and the drain. The profile and specifications will show the extent of work, the bench marks, grades and disposal of materials, etc.

Recommendation:

It is our recommendation that this drain be reconstructed from station 0 + 00 to station 15 + 31 where it enters the present road ditch. At this location the present ditch will carry the water to the south side of the forced road where construction is to continue from station 19 + 93. The drain then flows east to the present railway culvert which is of sufficient size and the drain has been designed to flow through it at its present elevation. From this location the drain still flows easterly to the Township Road between lots 15 and 16, Concession 3 at station 43 + 70. The present culvert through this road is to be lowered to the designed grade for this location and from here the drain flows east to its outlet in the Jack River at station 85 + 25.

Farm Crossings:

A Severance Allowance is made in this report under Section 3 (6) of the Drainage Act 1962-63, culvert dimensions are stated in the following Schedule of Allowance.

Township of Moness:

Concession 3:

N $\frac{1}{2}$	Lot 18	Lorne Burnett	1-24" x 16' x 16 gauge CMP	\$81.52
S $\frac{1}{2}$	Lot 18	Lorne Burnett	1-24" x 16' x 16 gauge CMP	81.52
Pt. W $\frac{3}{4}$				
N $\frac{1}{2}$	Lot 17	Mrs. E. Monk	1-24" x 16' x 16 gauge CMP	81.52
W $\frac{3}{4}$ N $\frac{1}{2}$	Lot 17	John & Elizabeth	1-24" x 16' x 16 gauge CMP	81.52
S $\frac{1}{2}$	Lot 17	Clark Fraser	1-24" x 16' x 16 gauge CMP	81.52
N 1/3	Lot 16	Patrick McLoughney	1-30" x 16' x 16 gauge CMP	\$2.08
PT E $\frac{2}{3}$	Lot 16	Vergus Houbahan	1-30" x 16' x 16 gauge CMP	92.08
E $\frac{2}{3}$ N $\frac{1}{2}$	Lot 15	Carl Fraser	1-36" x 16' x 16 gauge CMP	104.88
S $\frac{1}{2}$	Lot 15	Wm. Clark	1-36" x 16' x 16 gauge CMP	104.88

Pt E7/15	Lot 14	Mrs. W. Houlahan	1-36" x 16' x 16' gauge CMP	\$102.88
S 5/8	Lot 14	John Houlahan	1-36" x 16' x 16' gauge CMP	104.88
E Pt	Lot 13	Kelvin Burnett	1-36" x 16' x 16' gauge CMP	104.88
Total				<u>\$1,116.16</u>

These allowances will, in my opinion, adequately compensate the above property owners for the cost and installation of the above culverts.

Road Bridges and Culverts:

The present 24" x 18' CMP through the forced road in Concession 3 is sufficient in size, and the drain has been designed to flow through it at its present grade.

The present 30" x 26' CMP through the Township Road between lots 15 and 16, Concession 3 is sufficient in size and requires only to be lowered to the designed grade at this location.

Estimated Costs \$208.00

These estimated costs have not been included in the Estimated cost of construction as it is expected that the Township of Mendon's Road Department will accept this responsibility as part of their maintenance program.

Railroad Culvert:

The present culvert through the Canadian National Railway which consists of a 24" CMP on the south side, and a 2' x 2.75' timber culvert on the north side of the tracks is sufficient in size, and the drain has been designed to flow through at the present grade.

Land Damage:

The amounts shown in the following allowances will, in my opinion, adequately compensate the property owners indicated for land or crop damage (if any) under Section 8 (1) of the Drainage Act 1962 65.

N $\frac{1}{2}$	Lot 18	Lorne Burnett		366.00
S $\frac{1}{2}$	Lot 18	Lorne Burnett		62.00
Pt W3/4				
N $\frac{1}{2}$	Lot 17	Mrs. E. Monk		9.00
W3/4 N $\frac{1}{2}$	Lot 17	Sala & Ricevito		18.00
S $\frac{1}{2}$	Lot 17	Clark Fraser		38.00
N 1/3	Lot 16	Patrick McLaughney		59.00
Pt E2/3	Lot 16	Fergus Houlahan		90.00
E2/3 N $\frac{1}{2}$	Lot 15	Carl Fraser		61.00
S $\frac{1}{2}$	Lot 15	Ma. Clark		61.00
Pt E7/15 &				
N 3/8	Lot 14	Mrs. Helen Houlahan		63.00
S 5/8	Lot 14	John Houlahan		63.00
E Pt	Lot 13	Kelvin Burnett		96.00
Total				<u>\$726.00</u>

Distribution of Costs:

The estimated costs for this construction are apportioned to the properties responsible for benefit and outlet as determined by their areas, locations, and run-off.

In my opinion, no liability for injury will exist because of this construction, and for this reason no injury liability column will appear in the Schedule of Assessment, this being in my estimation a fair distribution of costs.

Future Maintenance:

This drain will be maintained by the Township of Nepean, and the costs of such future maintenance will be apportioned to the property owners in the same proportions to the property owners in the same proportions as in the attached Schedule of Assessment.

The "Estimated Costs" and incidental expenses are as follows:

Earth Excavation and Spreading of 1,992 c.y. @ .60¢	\$1,195.20
Hard Pan Excavation 282 c.y. @ \$1.60	451.20
Brushing	40.00
Fern Crossings	1,116.16
Land Damage Section 8 (1) of the Drainage Act 1962-63	726.00
Engineer's Fees and Expenses	884.44
Contingencies	470.00
Clerk's Fees	200.00
Printing and Publishing By-Law	125.00
Advertising and Letting Contract	85.00
Court of Revision	150.00
Supervision of Construction	190.00
Total Estimated Cost	<u>\$5,633.00</u>

Grants:

Under Section 61, 63 and 64 of the Drainage Act 1962-63, a Provincial Grant of 33 1/3% of the cost of construction for Agricultural Lands may be obtained.

A subsequent grant by Federal A.R.D.A. through the Provincial grant administration media will further reduce Agricultural assessments by another one-third.

The assessments are then payable two-thirds by grant, and one-third by property assessment.

Respectfully submitted this 16th day of October, 1968.



GRAHAM, BERMAN AND ASSOCIATES LTD.

John S. Morrison
John S. Morrison,
Drainage Manager.

SCHEDULE OF ASSESSMENT
MUNICIPAL DRAIN

JOB NO. 1862

DATE: Oct. 16/58

TOWNSHIP OF NEPEAN

Comp.	Lot	Name	Acreage Assessed	Main Drain Benefit	Outlet	Total
3	3 1/2 Lot 19	Don Fraser	13		198.18	198.18
	1 1/2 Spt Lot 18	Lorne Burnett	43	549.77	517.25	1,067.02
	3 Pt. Lot 18	Robert Neust	6		74.28	74.28
	Pt W3/4 N 1/2 L. 17	Mrs. E. Monk	8	96.25	99.84	196.09
	Pt W3/4 N 1/2 Lot 17	Sala & Ricciotto	11	76.23	136.18	212.41
	3 1/2 Lot 17	Carl Fraser	20	160.93	149.37	310.30
	N1/3 & P1/2 2/3 L. 16	Patrick Neighbour	15	249.40	223.00	472.40
	S2/3 Less 30ac L. 16	Fergus Houlahan	49	181.15	492.36	673.51
	E2/3 N 1/2 Lot 15	Carl Fraser	19	204.05	107.16	311.21
	3 1/2 Lot 15	Mrs. Clary	18	204.05	73.98	278.03
	Pt E7/15 W3/8 L. 14	Mrs. H Houlahan	16	233.70	50.72	284.42
	S5/8 Lot 14	John Houlahan	13	294.52	34.71	329.23
	Pt E 1/2 Lot 13	Kelvin Burnett	9	208.10		208.10
	Forced Road in Concession 3			122.82	254.64	377.46
	Township Road Between Lots 15 and 16			76.23	136.94	213.07
	Canadian National Railways			57.75	168.94	226.69
	Totals			\$2,914.65	\$2,718.35	\$5,633.00

SUMMARY OF ASSESSMENT

LANDS:

Township Roads	\$ 590.43
Non Agricultural Lands	\$ 226.69
Land used for Agricultural Purposes	\$4,815.88

GRANTS ON AGRICULTURAL LANDS

Estimated Provincial Grant of 33 1/3%	\$1,605.29
Estimated Federal A.R.D.A. Grant of 33 1/3%	\$1,605.29
Estimated Property Assessments	\$1,605.30

SUMMARY OF ASSESSMENT

LANDS

Township roads \$ 590.43
Non Agricultural lands 422.78
Land used for Agricultural Purposes \$1,619.79

Grants on Agricultural Lands

Estimated Provincial Grant of 33 1/3 % \$1,539.93
Estimated Federal A.A.D.A. Grant of 33 1/3% \$1,539.93
Estimated Property Assessments \$1,539.93

SUPPLEMENT TO THE GENERAL SPECIFICATIONS

BURKETT MUNICIPAL DRAIN

TOWNSHIP OF KEPLAN

Graham, Borden and Associates Ltd.,
Consulting Engineers,
Ottawa 8, Ontario.

October 16th, 1968.

Meaning of Terms:

- "Municipal Council" - shall mean the Municipal Council of Heron Township.
- "Reeve" - shall mean the Reeve of Heron Township.
- "Engineer" - shall mean the Engineer in charge of the works, or his authorized representatives.
- "Contractor" - shall mean the Contractor or Contractors performing the work, or their foreman on the grounds.

Nature of Work:

The accompanying plan, profile and specifications dated October 16th, 1968 apply to and govern this construction.

8,625 lineal feet of open drain as follows:

Earth Excavation and Spreading of 1,972 c.y.

Hardpan Excavation and Spreading of 282 c.y.

Brushing

A Severance Allowance under Section 8 (6) of the Drainage Act 1962-63 is applied on this drain.

Center-line:

The present watercourse is to be the center-line of construction.

Important:

- (a) The Engineer must be notified at least 5 days prior to the starting of work on this contract.
- (b) Fences may be opened to allow construction equipment to go through them, and are to be closed immediately after that piece of equipment passes through, if fences are found to be left opened, they will be closed at the "Contractor's Expense".

GENERAL SPECIFICATIONS

Municipal Drainage (Open-Drains)

Graham, Barman & Associates Ltd.
St. Thomas & Ottawa, Ontario.

1. These specifications are drawn up to cover the work as outlined in the Engineer's Report on the drain, and as further outlined in the supplement to General Specifications.

Where there is any doubt as to the meaning or intention of the specifications, it shall be the Contractor's duty to obtain a ruling in writing from the Engineer before proceeding with the work.

2. Supply of Labour and Materials:

The Contractor shall supply all materials, labour, equipment, tools, machinery etc. for the full and proper completion of this work in accordance with the specifications, plan and profile. All work must be done in a neat and workmanlike manner, and to the satisfaction of the Engineer.

3. Roads to be kept open:

All roads, public and private are to be kept open and in passable condition during the continuance of this work.

4. Relief Ditches:

Should the Contractor deem it necessary to dig relief ditches on any part of this work, he shall do so and refill same entirely at his own expense.

5. Damages:

a) In case of damage being done to any farm or other property along the line of work by blasting or other operations, the Contractor shall be held liable for such damage.

b) The Owner or Occupant of the property on which the drainage works is located shall be responsible for the protection of all livestock on said property during construction, and shall be liable for any damages caused to or by such livestock.

c) The Contractor shall also rebuild and leave in as good condition as before construction, all fences removed in order to execute this work.

i) Fences crossing the drainage works must be closed immediately after the construction equipment has passed through them.

ii) Fences parallel to the drain must be replaced immediately after the work is completed in each section.

d) When hay or other produce is growing on lands adjacent to the proposed work, the Contractor must give the Owner ten days' /notice

notice in writing to remove the same before he begins work on that portion, otherwise he shall be held liable for any damage caused.

6. Clearing and Grubbing:

a) Trees or brush growing in or on the banks of the drain are to be grubbed out clean. Trees having a stump diameter of 6" or over are to be cut into log lengths and piled clear of the spread materials. The stumps are to be piled in a corner of the field from which they were taken, adjacent to, but not closer than 4 feet to the edge of the drain. Branches and brush under 6" in diameter are to be piled on the excavated materials for disposal by the Property Owner.

b) At locations where the drain passes through brush or wood-lots, it is necessary that a strip of land be cleared along one side of the drain; the locations and dimensions will be given in the Supplement Specifications. However it is not intended that large trees growing within this specified area should be cut unless it is apparent that excessive damage will be caused to them. Stumps are not to exceed 1 ft. in height, and brush and branches are to be disposed of as in section (a) above.

Payment for this work will be made under the lump sum tender item "Cutting Brush".

7. Disposal of Materials:

The excavated materials shall be disposed of so as to do as little damage to lands and crops as possible.

a) Earth excavated from the drain is to be taken back a distance of 10 feet, leaving a clean berm 10 feet wide along the edge of the drain; and to be spread over the adjoining lands in such a manner that the elevation of the completed work does not exceed the elevation of the adjoining lands by more than 6" on cultivated lands and 12" on unworked or bush lands.

The completed work is to have a neat appearance and to be comparatively smooth.

b) Hard Pan and Rock excavated from the drain is to be taken back 4 feet from the edge of the drain and left in a pile so that it may be disposed of by the Property Owner.

c) Water Courses where necessary will be cut through the spread materials every 200 feet or in the low spots along the drain to allow surface drainage of the surrounding areas.

d) Re-location of Drain: At locations where the drain is to be removed from a road allowance, materials excavated from the new drain may be used to fill the road ditch in such a manner so as to allow the water from the road to enter the new drain. Excess materials are to be spread on the adjoining lands as above.

e) Straightening: At locations where straightening occurs the upper end of the old water course is to be filled level to the shoulders for a distance of 20 feet, and the excavated material put in the old drain when the intervening distance does not exceed 100 feet. Where the distance exceeds 100, the shoulders / are

are to be pushed into the bottom of the remainder of the abandoned watercourse, and so shaped that the water will run out of the low end, and in such a manner as to allow the Property Owner to travel through the drain and gradually reclaim the land.

f) Boulders: All boulders having a cubic content of 1 cubic foot or more are to be neatly placed along the edge of the drain at a distance of approximately 4 feet from it.

The price for the above materials disposal is to be included in the bid price per cubic yard for excavation.

8. Description of Ditch:

The ditch is to be constructed to the grades, widths and side slopes as shown on the accompanying profile.

The bottom width shall not be increased without maintaining the specified side slopes.

The grade is to be constructed to provide a constant slope to the end of the ditch so that no water will be stagnant therein.

9. Centre Line:

The Centre Line shall be the Centre Line of the present ditch, provided the fences are far enough back from the shoulders to allow for the proper width of bottom and side slope as shown on the accompanying profile. In locations where fences are too close, the Centre Line may be moved away from the fence a sufficient distance so that bottom widths and side slopes may be maintained.

At locations where the drain is to be removed from a road allowance, the centre line will be staked by the Engineer to allow for a clear berm of 3 feet between the property line and the edge of the drain.

At locations where excessive meanderings of the present stream take place, straightening may be required; in such cases the Centre Line will be staked in the field by the Engineer.

10. Grades and Centre Line:

Grades and Centre Line will be given by the Engineer upon receiving 5 clear days notice in writing that such Grades and Centre Line are required.

The Contractor will take precautions to ensure that Grades and Line so set will not be disturbed during construction.

Any subsequent setting of Line and Grade on the project will be charged to the Contractor.

11. Inspection:

Upon completion of the work there should be a continuous ditch or water course of the size and dimensions according to the plan, profile and specifications.

Any foreign material accumulated in the drain will be removed by the Contractor, unless he can clearly show that he is not responsible for the foreign material being in the drain.

The Contractor, when he considers all work to have been so completed, must notify the Engineer that he requires an inspection of the works to be made and he, or his representative, will accompany the Engineer on this inspection.

12. Classification of Materials:

Earth shall mean clay, loam, sand, small stones, gravel and muck, etcetera.

Hard Pan shall mean materials other than rock that require the use of picks, bars, dynamite, etc. for their removal.

Rock shall mean strata rock or boulders having a cubic content of 14 cubic feet or more.

Prices are to be submitted separately to cover each type of material.

13. Farm Bridges and Culverts:

The Contractor shall, as part of this contract, clean out or lower Farm Bridges and Culverts considered to be large enough for their locations.

The Contractor shall notify the Owner four clear days in writing to remove farm bridges that are not large enough to meet specifications; if the bridges are not removed, the Contractor may, after four days, remove same, exercising normal caution so as not to unduly damage the materials, piling same neatly 15 feet from the edge of the drain.

The Contractor may remove the flooring to clean out under a bridge that is of sufficient size; however, he must replace this flooring in as good condition as before it was removed.

The responsibility for the replacement of culverts or the construction of new ones is set forth in the Supplement Specifications.

Payment for the above work is to be included in the bid price per cubic yard for earth excavation.

14. Road Bridges and Culverts:

The Contractor shall notify the road Superintendent concerned as to the date and time the excavating equipment will be at the site of the road bridge or culvert.

The Contractor will construct the ditch to the ends of the present bridge or culvert.

The Contractor will clean all culverts that are of sufficient size and set at the required grade.

The Contractor will lower the grade and otherwise clear out under bridges having sufficient size for their locations. Payment for this work is deemed to be included in the bid price per cubic yard for earth excavation.

14. Road Bridges and Culverts: (Continued)

Work other than the above that may be required by the Road Superintendent will be paid for on an hourly basis by the Road Department responsible for the upkeep of this section of road.

15. Right to Increase or Decrease:

The Municipal Council reserves the right to increase the work as it deems necessary and the contract price per cubic yard shall remain the same.

16. Sub-Letting:

No portion of the work is to be sub-let without the consent of the Municipal Council and the Engineer.

17. Deposit:

A cash deposit or certified cheque on a chartered bank in the amount of 10% of the tender price must accompany each tender, such deposit will be returned to the unsuccessful bidders within 7 days of tender closing.

18. Date of Completion:

The whole work shall be completed on or before the _____ day of _____, 196____ and when the Contractor considers that the work is completed, he must notify the Engineer in writing that he requires a final inspection thereof.

19. Payment:

Cash payment will be made monthly equal to 90% of the value of the work completed on the certificate of the Engineer, when the value of work completed within the month amounts to Three Hundred Dollars (\$300.00) or more. The remaining 10% will be retained until 60 days after the whole work has been accepted as finished.

20. The Municipal Council reserves the right to reject any and all tenders.

21. The Contractor shall comply with the regulations of the Workmen's Compensation Board of Ontario.

22. Damages caused to public utilities installation shall be the responsibility of the General Contractor.

Prior to starting this work, he shall obtain from the public utilities (i.e. telephone, hydro, gas) the locations, if any, of all their installations along these works.

TENDER FORM

I/WE _____ OF _____
do hereby tender and agree to construct the _____
in accordance with the attached specifications and drawings.

I/WE have examined the site of the above work and are thoroughly
familiar with the work that is to be done.

I/WE tender and agree to perform the above mentioned work for the
following prices:

EXTENT OF WORK:

TOTAL PRICE

Earth Excavation and Spreading of 1,992 c.y.

Hardpan Excavation and Spreading of 282 c.y.

Brushing
.....
.....

TOTAL CONTRACT PRICE _____

I/WE guarantee that the above work will be completed on the
day of _____, 196 _____.

I/WE enclose a certified cheque or cash deposit for the sum of
(\$ _____), being 10% of the tender
price, and further agree to furnish a suitable bond for 100% of the
contract price within 7 days of notification of acceptance of tender,
if so requested. In such case, the deposit will be returned on the
signing of the contract.

The deposit or bonds of the successful bidder may be retained by the
Clerk until 60 days after the above work has been completed and accepted,
or in the event the tender is unsuccessful, it shall be returned within
7 days.

Offered on behalf of Contractor:

Accepted on behalf of the Municipality

NAME: _____

RECEIVED: _____

ADDRESS: _____

CLERK: _____

DATE: _____

POSITION: _____

DATE: _____

Appendix F
Correspondence

Greg Winters

From: Xu, Lily <Lily.Xu@ottawa.ca>
Sent: Friday, June 26, 2015 4:27 PM
To: 'jim.burghout@claridgehomes.com'; Greg Winters
Cc: Hall, James; Young, Mark; Carter, Riley; Krabicka, Jeannette; Rehman, Sami; McDonald, Glen; Greg MacDonald; jocelyn.chandler@rvca.ca; Xu, Lily
Subject: 3370 Jockvale (SNTC Claridge) pre-con - follow up
Attachments: Study and Plan ID List.docx

Hi Jim and Greg,

This is a follow-up of the Pre-Application Consultation on June 19, 2015 for Claridge's property at 3370 Jockvale Road. The attached "Applicant's Study and Plan Identification List" identifies the number of copies required for each report and plan in order to deem the application(s) complete. PDF files are needed for all required reports and plans. Guidance on preparing the studies and plans can be found [online](#).

Further, please note Staff's preliminary comments:

Servicing and Engineering

- Please note the right of way (ROW) for Jockvale Road extension may need to be widened to 22-24 metres due to the additional space needed for the underground truck sewer.
- For Half Moon Bay Drive, Staff is open to other non-standard ROW cross section options provided the objectives of the secondary plan will still be achieved.
- In general, servicing is not permitted along lane ways. Please note the right angle of 8 metre laneways are discouraged due to maintenance challenges.

RVCA

The RVCA will be looking for:

- Plan showing:
 - 1:100 year floodplain (91.59 to 91.45) based on site specific geodetic elevations
 - 30 m from the NHWM of both the Jock River and the Clarke Drain
 - 15 m from top of bank
 - Geotechnical slope stability limit in accordance with MNR Technical Slope Stability Guidelines
 - Meander belt
- Detailed cut and fill proposal for floodplain boundary revision under O.Reg 174/06
- Location and detailed design of proposed stormwater outlets (should be based on a site visit and concurrence with RVCA watercourse regulations staff (Hal Stimson)
- Headwater Assessment of tributary proposed to be closed and potential offset plan for any values identified to be lost in Headwaters Assessment.
- Detailed cross section of area under bridge and proposed path.
- Hydrogeological report assessing risk to adjacent private water wells and recommendations for mitigation and pre-post construction monitoring.
- Stormwater management report including water budget and source control measures to promote infiltration.
- Identification of areas that would benefit from riparian planting.

Transportation

- Staff in general has no objection towards the proposed removal of the planned Half Moon Bay Drive under the Greenbank Road/Bridge, provided that:
 - the submission can demonstrate that the proposed road network can function properly;

- pedestrian and cyclist connections will be continued under the bridge and connected conveniently with other pedestrian and cyclist facilities within the Town Centre;
- sufficient information on the engineering and environmental benefits can be presented.
- Note streets at signalized intersections may need to be widened to accommodate turning lanes.
- Before starting the Community Transportation Study, please provide a Terms of Reference to Project Manager, Transportation, Riley Carter, at Riley.Carter@ottawa.ca or 613 580 2424 ext 14304, for review and concurrence.

Environment

- The required Environmental Impact Statement (EIS) shall address
 - Potential habitat for threatened or endangered species, OP Section 4.7.4
 - impacts from stormwater discharge, which may include but not limited to, potential erosion, suspended solids, changes in thermal regime, etc.
 - establishing the appropriate setback requirement, OP Section 4.7.3,
 - findings and recommendations from the headwater features study
 - slope stability and floodplain analyses from Geotechnical Study
 - recommendations for riparian plantings, as required under the Jock River Reach One Subwatershed Study.
- Please contact the Kemptville Office of the Ministry of Natural Resources and Forestry to determine their obligations under the Endangered Species Act and to identify which species should be covered in their field investigations.
- To assist with the riparian planting recommendations requested as part of the EIS, the authors can refer to and draw information from the MNR's extension notes, such as "Preserving and restoring natural shorelines" or "Buffers Protect the Environment" for example. http://lrconline.com/Extension_Notes_English/water/water_index.html
Another valuable source is "Ecological Buffer Guideline Review" by Beacon Environmental (Dec 2012). <http://www.creditvalleyca.ca/wp-content/uploads/2013/08/Ecological-Buffer-Guideline-Review.pdf>
- A joint report of the Tree Conservation Report (TCR) and EIS is acceptable.
- For further clarifications, please contact Environmental Planner, Sami Rehman, at Sami.Rehman@ottawa.ca or 613 580 2424 ext 13364.

Planning and Land Use

- The policies in the most recent OP encourage mixed-uses within Town Centres. The Secondary Plan also suggests none-residential uses along major roads. The ground floor along Greenbank and other collectors shall be constructed to be able to accommodate non-residential uses. Considerations will be implemented into zoning details (and through subdivision, site plan and condominium). These considerations, in addition to permitted uses, may include:
 - minimum ground floor height of 4 m
 - minimum window glazing of 40%
 - maximum grading difference from the street to the ground floor is 0.45 m
 - no restriction of signage and commercial vehicles
 - warning clause of non-residential uses on title, etc
- The most recent policies in the PPS and OP encourage density surrounding transit stations and routes. Therefore the block along Greenbank has the potential to accommodate unit types of higher density. It is understood that other aspects such as servicing options, density targets, and marketability have to come into play.
- An OPA is required in order to achieve the requested change in building height and road removal. Road right of way justifications can be dealt with the Subdivision.
- Please ensure to comply with the [Terms of Reference](#) when preparing the Planning Rationale.

Parkland

- 5% parkland dedication is applicable to the site as per the Parkland Dedication Bylaw. Based on the site area of 15.4 ha, the required parkland dedication is 0.77 ha.

- Staff is willing to take a parkette as parkland dedication in combination with Cash-in-Lieu for the area. Suggest consider part of the triangular shaped block in the middle of the subdivision for a parkette.

Urban Design

- Please note the site is subject to the [Urban Design Review Panel](#) (UDRP). Please refer to the online link for [Panel Meeting Schedule](#) and [Submission requirements](#).
- It is noted that the Design Brief package will not be a mandatory requirement for deeming the application(s) complete. However, to your advantage, it is highly recommended that the applications shall go to the UDRP for review and comment prior to staff providing the 1st round of comments.

I hope the above is of some assistance. Please let me know if there are further questions or any clarification will be helpful.

Yours truly,

Lily Xu, MPL, MCIP, RPP, LEED Green Assoc.
Planner II, Suburban Services
Urbaniste II, Services suburbains



City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West. Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1
tel/tél:613.580.2424 ext./poste **27505**, fax/télé:613-580-2576, email/courriel:Lily.Xu@ottawa.ca
ottawa.ca/planning / ottawa.ca/urbanisme

This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

APPLICANT'S STUDY AND PLAN IDENTIFICATION LIST

 Legend: **S** indicates that the study or plan is required with application submission.

A indicates that the study or plan may be required to satisfy a condition of approval/draft approval.

For information and guidance on preparing required studies and plans refer to:

<http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans>

S/A	Number of copies	ENGINEERING		S/A	Number of copies
S	6	1. Site Servicing Plan	2. Site Servicing Study	S	6
S	6	3. Grade Control and Drainage Plan	4. Geotechnical Study (Incl. Slope Stability Study)	S	4
	2	5. Composite Utility Plan	6. Groundwater Impact Study		6
	5	7. Servicing Options Report	8. Headwater tributary Assessment	S	6
S	9	9. Community Transportation Study	10. Erosion and Sediment Control Plan	S	6
S	6	11. Storm water Management Report	12. Hydro-geological Off Site Risk Assessment	S	6
S	3	13. Hydraulic Water main Analysis	14. Noise Feasibility Study	S	3
	35/50/55	15. Roadway Modification Design Plan	16. Confederation Line Proximity Study		9

S/A	Number of copies	PLANNING / DESIGN / SURVEY		S/A	Number of copies
S	55	17. Draft Plan of Subdivision	18. Plan Showing Layout of Parking Garage		2
	30	19. Draft Plan of Condominium	20. Planning Rationale	S	3
	35/55	21. Site Plan	22. Minimum Distance Separation (MDS)		3
S	55	23. Concept Plan Showing Proposed Land Uses and Landscaping	24. Agrology and Soil Capability Study		5
	3	25. Concept Plan Showing Ultimate Use of Land	26. Cultural Heritage Impact Statement		3
	35/55	27. Landscape Plan	28. Archaeological Resource Assessment Requirements: S (site plan) A (subdivision, condo)	A	3
S	2	29. Survey Plan	30. Shadow Analysis	A	3
S	3	31. Architectural Building Elevation Drawings (dimensioned) all unit types	32. Design Brief (includes the Design Review Panel Submission Requirements)	A	3
	6	33. Wind Analysis			

S/A	Number of copies	ENVIRONMENTAL		S/A	Number of copies
S	4	34. Phase 1 Environmental Site Assessment	35. Impact Assessment of Adjacent Waste Disposal/Former Landfill Site		6
	5	36. Phase 2 Environmental Site Assessment (depends on the outcome of Phase 1)	37. Assessment of Landform Features		7
	4	38. Record of Site Condition	39. Mineral Resource Impact Assessment		4
S	8	40. Tree Conservation Report	41. Environmental Impact Statement / Impact Assessment of Endangered Species	S	8
	4	42. Mine Hazard Study / Abandoned Pit or Quarry Study			

S/A	Number of copies	ADDITIONAL REQUIREMENTS		S/A	Number of copies
		43.	44.		

Meeting Date: June 19, 2015

Application Type: OPA/Zoning/Subdivision

File Lead (Assigned Planner): Lily Xu

Infrastructure Approvals Project Manager: James Hall

Site Address (Municipal Address): 3370 Greenbank

 *Preliminary Assessment: 1 2 3 4 5

*One (1) indicates that considerable major revisions are required before a planning application is submitted, while five (5) suggests that proposal appears to meet the City's key land use policies and guidelines. **This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.**

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, the Planning and Growth Management Department will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the Planning and Growth Management Department.

Steve Zorgel

Subject: FW: Claridge submission to RVCA for 3370 Greenbank Rd

From: Jocelyn Chandler [<mailto:jocelyn.chandler@rvca.ca>]
Sent: Monday, September 26, 2016 1:43 PM
To: Greg Winters <G.Winters@novatech-eng.com>
Subject: Fw: Claridge submission to RVCA for 3370 Greenbank Rd

As requested. Jocelyn

From: Jocelyn Chandler
Sent: March 31, 2016 11:55 AM
To: Greg MacDonald
Cc: Jim Burghout (jim.burghout@claridgehomes.com)
Subject: Claridge submission to RVCA for 3370 Greenbank Rd

Hello Greg,

As discussed, we are in receipt of a plan and an application form under O.Reg 174/06 for i) the future closure of a tributary to the Jock River and ii) floodplain cut/fill works on the subject property.

I understand from our conversation that the actual work is unlikely to occur in the next two year, and that Claridge is seeking 'support in principle' on the plans relating to the imminent application for development under the Planning Act. Based on this information and our discussion, I will proceed as follows;

- 1) Return the application form for a regulatory permit. This can be submitted later when the date of the works is known as the permits have limited 2 year expiry dates.
- 2) Circulate the supporting documentation to our technical staff for review and support in principle.
- 3) To complete this review, we require the headwaters assessment report and a supporting technical memo to explain the cut/fill details.

Claridge will be invoiced for these two technical reviews only, which will be used in support of their future planning and regulatory applications. Please note that should the application under O.Reg 174/06 not be submitted within two years, it is possible the documents and related review will need to be updated as unfortunately we cannot provide support in principle indefinitely.

Please call me with any questions. Jocelyn

Jocelyn Chandler M.Pl. MCIP, RPP

Planner, RVCA

t) 613-692-3571 x1137

f) 613-692-0831

jocelyn.chandler@rvca.ca

www.rvca.ca

mail: Box 599 3889 Rideau Valley Dr., Manotick, ON K4M 1A5

courier: 3889 Rideau Valley Dr., Nepean, ON K2C 3H1

This message may contain information that is privileged or confidential and is intended for the use of the individual(s) or entity named above. This material may contain confidential or personal information which may be subject to the provisions of the Municipal Freedom of Information & Protection of Privacy Act. If you are not the intended recipient of this email, any use, review, revision, retransmission, distribution, dissemination, copying, printing or otherwise use of, or taking any action in reliance upon this email, is strictly prohibited. If you have received this email in error, please contact the sender and delete the original and any copy of the email and any print out thereof, immediately. Your cooperation is appreciated.

Steve Zorgel

Subject: FW: RVCA Burnett

From: Renaud, Jean-Charles [<mailto:Jean-Charles.Renaud@ottawa.ca>]
Sent: Friday, September 30, 2016 9:54 AM
To: Greg Winters <G.Winters@novatech-eng.com>
Cc: Jocelyn Chandler <jocelyn.chandler@rvca.ca>; Rehman, Sami <Sami.Rehman@ottawa.ca>; Weeks, Gwendolyn <Gwendolyn_Weeks@golder.com>; Hall, James <James.Hall@ottawa.ca>
Subject: RE: Terms of Reference

Good morning Greg,

As it relates to the cut/fill, staff will require that an agreement in principle be reached prior to submission in order to ensure that the application review moves ahead with limited impacts as a result of unknown elements. Cut/fill approval will be conditioned in the draft agreement.

JC

Jean-Charles Renaud, MCIP/MICU, RPP/UPC

Planner | *Urbaniste*

Development Review, Suburban Services | *Examen des projets d'aménagement, Services suburbains*

Planning, Infrastructure and Economic Development Department | *Services de la planification, de l'infrastructure et du développement économique*

City of Ottawa | *Ville d'Ottawa*

110 Laurier Avenue West. Ottawa, ON | *110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1*

613.580.2424 ext./poste 27629

ottawa.ca/planning / ottawa.ca/urbanisme

From: Greg Winters [<mailto:G.Winters@novatech-eng.com>]

Sent: Friday, September 23, 2016 1:29 PM

To: Jocelyn Chandler; Rehman, Sami; Weeks, Gwendolyn

Cc: Renaud, Jean-Charles

Subject: RE: Terms of Reference

All good questions.

To clarify...

Tribs 2,3, 4 on the image from the headwater study that I sent you are actually on Caivan lands, north of the lands owned by Claridge.

I've highlighted the approximate limits of the Claridge – Burnett Lands with a red hatch.

We are only proposing to pipe the Burnett Drain for the portion through the developable portion of the subdivision up to the flood plain along the Jock River.

The KB Pond outlet does not appear to be adjacent to the area of the subdivision proposed for development. The KB Pond outlet is adjacent to a large Open Space (flood plain) Block that will be dedicated to the City at registration.

The application has not been submitted yet. We are still completing the necessary studies. Greg MacDonald will be completing any missing information.

The application (when we file it) should not be put on hold or deemed incomplete due to information related to the a cut/fill. We recognize that this will get resolved during the (months) of review and that the City will not issue DP approval unless the cut/fill has been approved. Not dismissing this, but it is also not a “required study” at submission on the City’s list for a DP application.

There will be a multi-use pathway under the bridge as discussed at the last pre-consult meeting. The road has been eliminated as there is not enough vertical clearance under the structure for a full road.

The pathway is identified as a requirement on Schedule 5 – Greenspace Network South Nepean Town Centre CDP. I don’t know the exact location or the setback. It would be getting too far in the “weeds” at this point to have a discussion about setbacks and design of the pathway. I’m sure we’ll figure this out as we proceed with the application and into detailed design post DP approval.

Hope this helps as it has been a while since we looked at this together.

Gregory Winters, MCIP, RPP, Project Manager-Planner

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x241 | Cell: 613.261.4990 | Fax: 613.254.5867

<http://ca.linkedin.com/in/gregwinters/>

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Jocelyn Chandler [<mailto:jocelyn.chandler@rvca.ca>]

Sent: Friday, September 23, 2016 12:48 PM

To: Rehman, Sami <Sami.Rehman@ottawa.ca>; Greg Winters <G.Winters@novatech-eng.com>; Weeks, Gwendolyn <Gwendolyn.Weeks@golder.com>

Cc: Renaud, Jean-Charles <Jean-Charles.Renaud@ottawa.ca>

Subject: RE: Terms of Reference

OK but outlet to KB Pond is a watercourse too...filled with fish. I presume you have the setbacks on that?

Also we have never done a preliminary review of the cut/fill proposal because information was missing (email to Greg McDonald March 31, 2016). It should be reviewed in principle before application for sub is deemed complete.

Also what is the proposed infrastructure under eth bridge at this time? A path or a road? Does it meet Jock R setbacks too?

Thanks ,j

Jocelyn Chandler M.Pl. MCIP, RPP
Planner, RVCA

t) 613-692-3571 x1137

f) 613-692-0831

jocelyn.chandler@rvca.ca

www.rvca.ca

mail: Box 599 3889 Rideau Valley Dr., Manotick, ON K4M 1A5

courier: 3889 Rideau Valley Dr., Nepean, ON K2C 3H1

This message may contain information that is privileged or confidential and is intended for the use of the individual(s) or entity named above. This material may contain confidential or personal information which may be subject to the provisions of the Municipal Freedom of Information & Protection of Privacy Act. If you are not the intended recipient of this email, any use, review, revision, retransmission, distribution, dissemination, copying, printing or otherwise use of, or taking any action in reliance upon this email, is strictly prohibited. If you have received this email in error, please contact the sender and delete the original and any copy of the email and any print out thereof, immediately. Your cooperation is appreciated.

From: Rehman, Sami [<mailto:Sami.Rehman@ottawa.ca>]

Sent: Friday, September 23, 2016 12:46 PM

To: Greg Winters <G.Winters@novatech-eng.com>; Jocelyn Chandler <jocelyn.chandler@rvca.ca>; Weeks, Gwendolyn <Gwendolyn_Weeks@golder.com>

Cc: Renaud, Jean-Charles <Jean-Charles.Renaud@ottawa.ca>

Subject: RE: Terms of Reference

Thanks for the clarification.

Can you please explain what is a preliminary EIS and what is the purpose of it? Thank you. sami

Sami Rehman

Environmental Planner | Planificateur environnemental

Development Review Services | Services d'examen demandes d'aménagements

Planning, Infrastructure and Economic Development Department | Service de planification, d'Infrastructure et de Développement économique

City of Ottawa | Ville d'Ottawa

☎ 613.580.2424 ext./poste 13364

From: Greg Winters [<mailto:G.Winters@novatech-eng.com>]

Sent: September 23, 2016 12:38 PM

To: Jocelyn Chandler; Weeks, Gwendolyn; Rehman, Sami

Subject: RE: Terms of Reference

Jocelyn

See attached and below from the HWS from Muncaster

You are correct. There are three tributaries/watercourses on site. All three will be replaced by the on-site storm sewers through the development of the subdivision. We are not looking for exceptions to setbacks as they will disappear.

We are also not looking for an exception for the Jock. There will be some adjustments to the flood plain through your review of a application to fill. Otherwise all the development is outside the 30 metre from nwl of the Jock.



Gregory Winters, MCIP, RPP, Project Manager-Planner

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x241 | Cell: 613.261.4990 | Fax: 613.254.5867

<http://ca.linkedin.com/in/gregwinters/>

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Jocelyn Chandler [mailto:jocelyn.chandler@rvca.ca]

Sent: Friday, September 23, 2016 12:24 PM

To: Weeks, Gwendolyn <Gwendolyn.Weeks@golder.com>; Rehman, Sami <Sami.Rehman@ottawa.ca>

Cc: Greg Winters <G.Winters@novatech-eng.com>

Subject: RE: Terms of Reference

Hello Gwendolyn,

I haven't seen the HDF prepared by Muncaster so I don't know what it covers/contains.

If Claridge is going to be looking for any exceptions to watercourse setbacks as proposed in the OP (and there are three watercourses currently affecting the lands) that would need to be covered in the EIS.

Jocelyn

Jocelyn Chandler M.Pl. MCIP, RPP
Planner, RVCA
t) 613-692-3571 x1137
f) 613-692-0831
jocelyn.chandler@rvca.ca

www.rvca.ca

mail: Box 599 3889 Rideau Valley Dr., Manotick, ON K4M 1A5

courier: 3889 Rideau Valley Dr., Nepean, ON K2C 3H1

This message may contain information that is privileged or confidential and is intended for the use of the individual(s) or entity named above. This material may contain confidential or personal information which may be subject to the provisions of the Municipal Freedom of Information & Protection of Privacy Act. If you are not the intended recipient of this email, any use, review, revision, retransmission, distribution, dissemination, copying, printing or otherwise use of, or taking any action in reliance upon this email, is strictly prohibited. If you have received this email in error, please contact the sender and delete the original and any copy of the email and any print out thereof, immediately. Your cooperation is appreciated.

From: Weeks, Gwendolyn [<mailto:Gwendolyn.Weeks@golder.com>]

Sent: Friday, September 16, 2016 2:58 PM

To: Rehman, Sami <Sami.Rehman@ottawa.ca>; Jocelyn Chandler <jocelyn.chandler@rvca.ca>

Cc: Greg Winters <G.Winters@novatech-eng.com>

Subject: Terms of Reference

Hello Sami and Jocelyn,

I have been retained by Claridge to help with their proposed development at 3370 Greenbank Road. Attached is a Terms of Reference to complete a preliminary EIS for the Site. Please review at your convenience, and If you have any questions, please give me a call.

Many thanks,

-Gwendolyn

Gwendolyn Weeks (H.B.Sc.Env) | Ecologist | Golder Associates Ltd.

1931 Robertson Road, Ottawa, Ontario, Canada, K2H 5B7

 We Have Moved!

T: +1 (613) 592 9600 | D: +1 (613) 592-9600 x4234 | F: +1 (613) 592 9601 | C: +1 (613) 913-1179 | E:

Gwendolyn.Weeks@golder.com | www.golder.com

Work Safe, Home Safe

This e-mail and any files transmitted with it are confidential and intended only for the individual named. If you have received this e-mail by mistake, please notify the sender immediately by e-mail. If you are not the named addressee you should not disseminate, distribute or copy this e-mail. Please notify the sender immediately by e-mail if you have received this e-mail by mistake and delete this e-mail if you have received this e-mail by accident. If you have any questions about this e-mail please contact the sender. Thank you.

Golder is a multi-national firm and the G&A globe design are trademarks of Golder Associates Corporation

Please consider the environment before printing this email.

This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

Appendix G

Engineering Drawings

<i>General Plan of Services</i>	<i>111117-GP</i>	<i>(revision 2)</i>
<i>Grading Plan</i>	<i>111117-GR</i>	<i>(revision 2)</i>
<i>Sanitary Drainage Area Plan</i>	<i>111117-SAN</i>	<i>(revision 2)</i>
<i>Storm Drainage Area Plan</i>	<i>111117-STM</i>	<i>(revision 2)</i>