

MINTO COMMUNITIES INC.

**ARCADIA RESIDENTIAL
STAGES 3, 4, 5 & 6 AND
COMMERCIAL STAGES 2 & 3
OTTAWA, ONTARIO**

**STORMWATER MANAGEMENT
STRATEGY REPORT**

April 2017

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JLR 26299-01

STORMWATER MANAGEMENT STRATEGY REPORT

MINTO COMMUNITIES INC. ARCADIA RESIDENTIAL STAGES 3, 4, 5 & 6 AND COMMERCIAL STAGES 2 & 3

- TABLE OF CONTENTS -

1.0	INTRODUCTION	1
1.1	Proposed Development	1
1.2	Background.....	4
2.0	EVALUATION OF STORM SERVICING AND SWM ALTERNATIVES	5
2.1	KWMSS POND 1	5
2.1.1	Hydrological Parameters	5
2.1.2	Catchment Storage Simulation Parameters	6
2.1.3	Preliminary Trunk Storm Sewer and Pond Design	6
2.1.4	Constraints.....	8
2.2	POND BISECTED BY RIVERCHASE ROAD WITH HYDRAULICALLY CONNECTED CELLS	10
2.2.1	Preliminary Trunk Storm Sewer and Pond Design	10
2.2.2	Constraints.....	11
2.3	POND BISECTED BY RIVERCHASE ROAD WITH HYDRAULICALLY SEPARATED CELLS	12
2.3.1	Preliminary Trunk Storm Sewer and Pond Design	13
2.3.2	Constraints.....	14
2.4	TWO PONDS: CAMPEAU DRIVE AND PAINE AVENUE	15
2.4.1	Preliminary Trunk Storm Sewer and Pond Design	16
2.4.2	Constraints.....	18
3.0	RECOMMENDED STORM SERVICING AND SWM STRATEGY	19
3.1	Advantages of Two Pond Strategy	19
3.2	Addressing Constraints of the Recommended Two Pond Strategy.....	19
3.3	Water Quality and Quantity Control for the Two Pond Strategy	20
3.4	Major System with Two Pond Strategy	21
3.5	Simulation of the Two Pond SWM Strategy.....	21
3.5.1	Pond Simulation Parameters	21
3.5.2	Simulation Results	22
4.0	SANITARY SEWER OVERFLOWS.....	23
5.0	CONCLUSION	24

- LIST OF FIGURES -

Figure 1-1: Location Plan.....	2
Figure 1-2: Staging Plan	3
Figure 2-1: KWMSS Pond 1.....	7
Figure 2-2: Pond Bisected by Riverchase with Hydraulically Connected Cells Schematic.....	11
Figure 2-3: Pond Bisected by Riverchase with Hydraulically Separated Cells Schematic.....	14
Figure 2-4: Two Pond Schematic.....	17

- LIST OF TABLES -

Table 1-2: IBI Pond 1 Concept Design (2013)	5
Table 2-1-1: Land Use Percent Impervious and Runoff Coefficient Values.....	6
Table 2-1-2: KWMSS Pond 1 Characteristics	7
Table 2-2: Pond Bisected by Riverchase with Hydraulically Connected Cells Characteristics ...	11
Table 2-3: Pond Bisected by Riverchase with Hydraulically Separated Cells Characteristics....	13
Table 2-4: Two Pond Characteristics	16
Table 3-1: Pond Storage Parameter Summary.....	22
Table 3-2: Carp River Model Results (City of Ottawa Model)	22
Table 3-3: Carp River Model Results (Two Pond Strategy)	22

- LIST OF APPENDICES -

Appendix 'A'	- Draft Plan of Subdivision
Appendix 'B'	- Background Storm Servicing and SWM Documents
Appendix 'C'	- PCSWMM Results for Storm Servicing and SWM Alternatives
	Figure C1a: KWMSS Pond 1
	Figure C1b: KWMSS Pond 1 (with 600 mm dia. Sewer on Campeau Drive)
	Figure C2: Pond Bisected by Riverchase with hydraulically connected cells
	Figure C3: Pond Bisected by Riverchase with hydraulically separate cells
	Figure C4a: Two Ponds: Campeau Drive and Paine Avenue
	Figure C4b: Two Ponds: Campeau Drive and Paine Avenue (with 600 mm dia. storm sewer on Campeau)
Appendix 'D'	- Existing Interim Pond Deficiencies noted by City
Appendix 'E'	- Water Quality Calculations
Appendix 'F'	- PCSWMM Model Outputs for Two Pond Strategy

- LIST OF DRAWINGS -

26299-01	CS1	Conceptual Site Servicing Plan
26299-01	CG1	Conceptual Grading Plan

STORMWATER MANAGEMENT STRATEGY REPORT

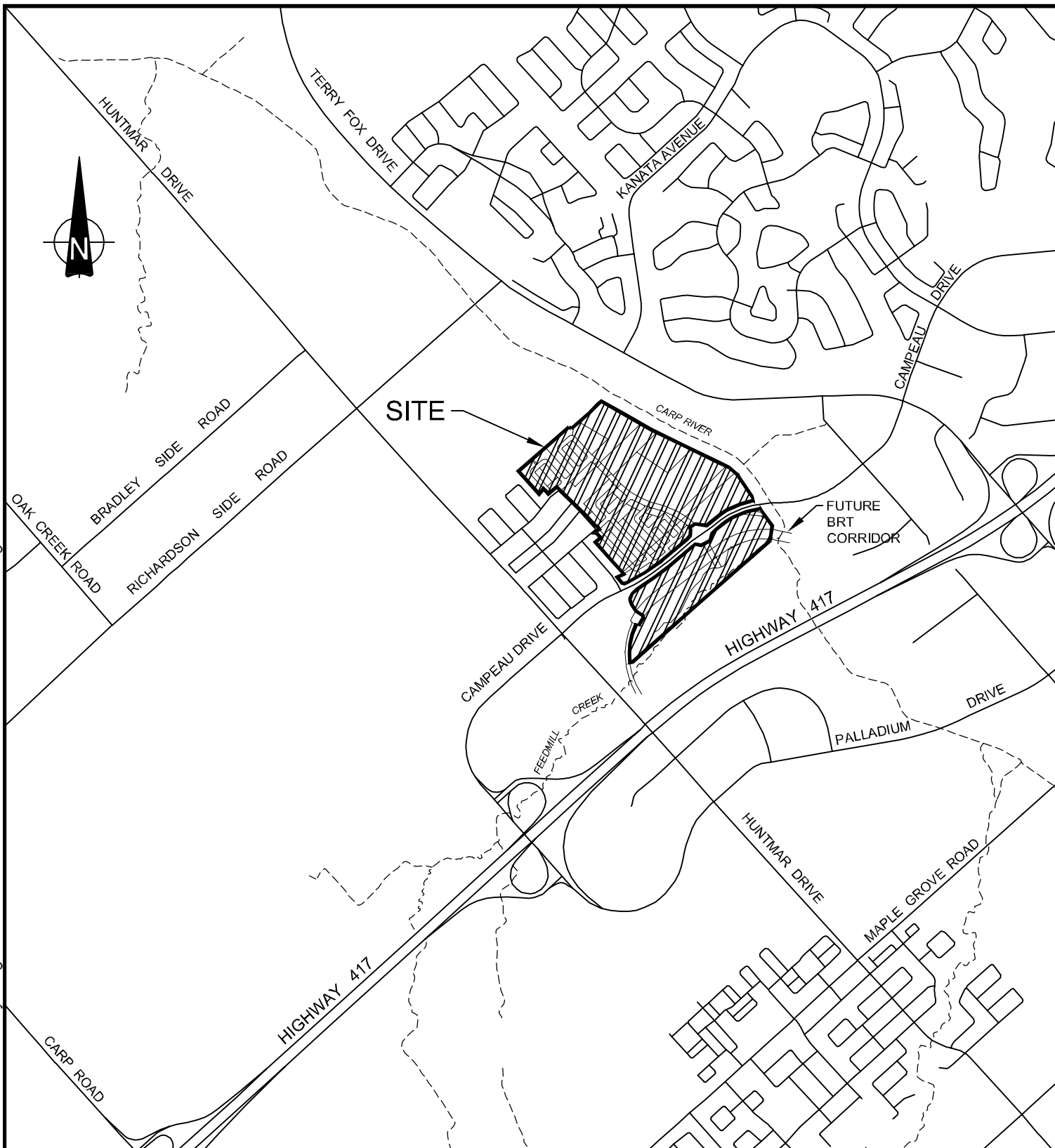
MINTO COMMUNITIES INC. ARCADIA RESIDENTIAL STAGES 3, 4, 5 & 6 AND COMMERCIAL STAGES 2 & 3

1.0 INTRODUCTION

This report is to support a proposed variation to the stormwater servicing and stormwater management (SWM) strategies recommended as part of the 2006 Kanata West Master Servicing Study (KWMSS) for Minto's Arcadia lands (located east of the Huntmar Drive and Campeau Drive intersection), a portion of the Campeau Drive right-of-way (ROW), and a portion of the future Transitway corridor located to the south of Campeau Drive. This report presents the advantages of a new servicing strategy that consists of two wet ponds outletting to the Carp River over the single pond strategy recommended for the Arcadia development area in the 2006 KWMSS.

1.1 Proposed Development

Minto's development known as Arcadia consists of six residential stages and three commercial stages. Approximately 23 ha of the Arcadia Lands known as Arcadia Stages 1 and 2 have been developed as a residential subdivision, while another ± 4 ha known as Commercial Stage 1 and a ± 4 ha external area located south of Campeau Drive, are pending approval for construction. Four residential stages (Stages 3 to 6) and two commercial stages (Stages 2 and 3) are proposed and scheduled for urbanization in Arcadia. These future stages, herein collectively referred to as the Arcadia Development, are sited on a ± 44 ha parcel of land that is bounded by the existing residential and planned commercial developments to the west, and by the Carp River to the east, as depicted on the Location Plan in **Figure 1-1**, the Staging Plan in **Figure 1-2**, and the Draft Plan of Subdivision provided in **Appendix 'A'**. The Arcadia Development is bisected near the south end by the future Campeau Drive extension and the future transitway. Currently, the land is undeveloped and generally drains easterly towards the Carp River. There is an existing drainage corridor within the Arcadia Development that serves as the dedicated outlet to the Carp River for the existing 'interim' pond servicing existing Arcadia Stages 1 and 2 and Commercial Stage 1, as well as for the western portion of the Campeau Drive ROW.



PROJECT:

MINTO COMMUNITIES INC.
ARCADIA STAGE 3,4,5,6 AND COMMERCIAL STAGES 2 AND 3
 450 HUNTMAR DRIVE, OTTAWA, ONTARIO

DRAWING:

LOCATION PLAN



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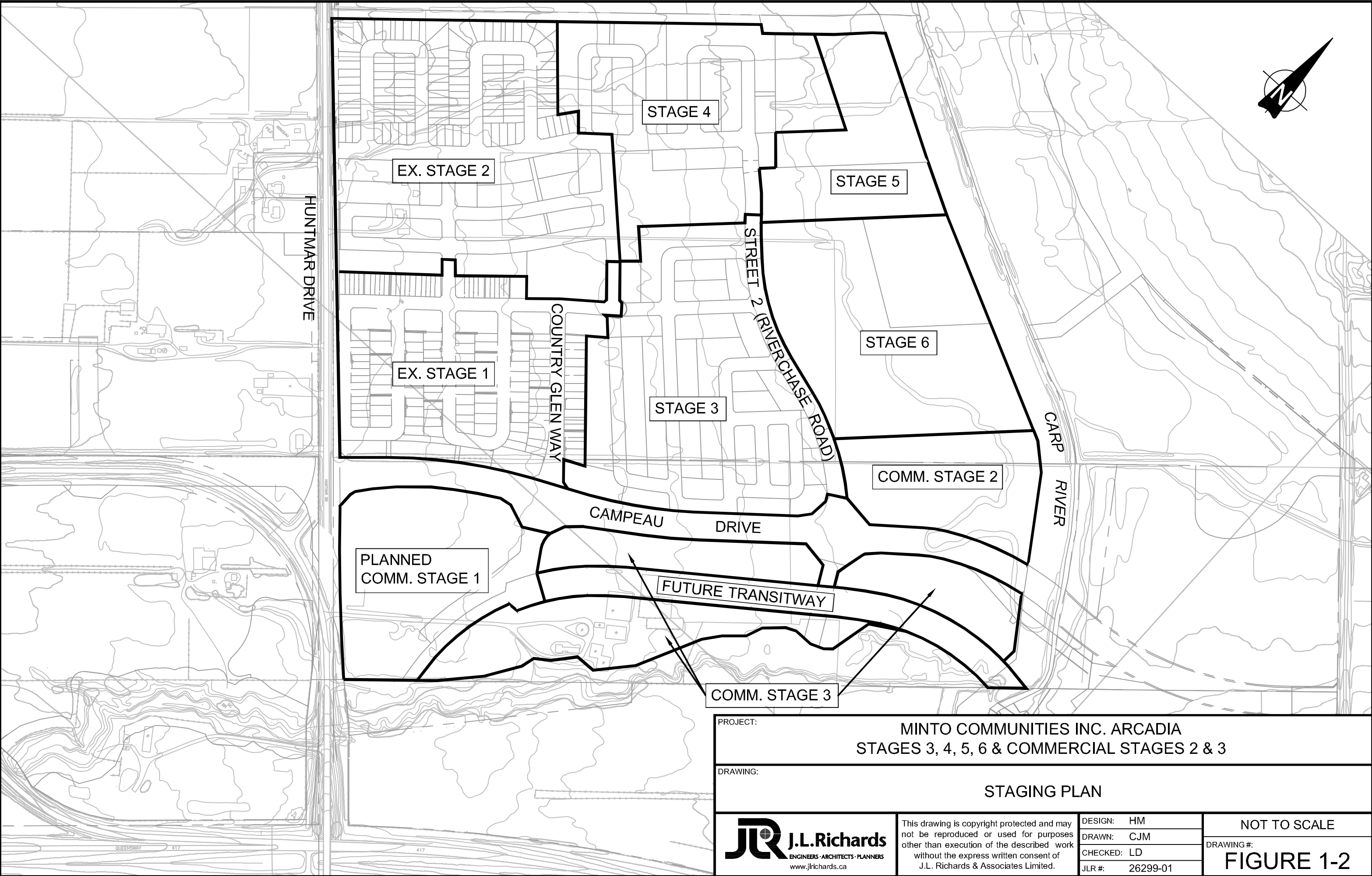
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FIGURE 1-1

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PROJECT:		MINTO COMMUNITIES INC. ARCADIA STAGES 3, 4, 5, 6 & COMMERCIAL STAGES 2 & 3	
DRAWING:		STAGING PLAN	
 J.L.Richards ENGINEERS · ARCHITECTS · PLANNERS www.jlrichards.ca	This drawing is copyright protected and may not be reproduced or used for purposes other than execution of the described work without the express written consent of J.L. Richards & Associates Limited.	DESIGN: HM	NOT TO SCALE
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1.2 Background

In 2006, the KWMSS was prepared on behalf of the Kanata West Owners Group (KWOOG) to investigate servicing requirements for a large mixed-use community referred to as Kanata West, which at build-out would include a population of approximately 17,000 persons in 6,300 households, 24,000 jobs in approximately 1 million square metres of commercial space. In terms of stormwater servicing and SWM, the KWMSS recommended that the ± 725 ha area be serviced by seven (7) water quality/quantity facilities spread over the entire Kanata West Study Area. The largest facility in Kanata West, Pond 1, was identified to serve as the dedicated stormwater outlet for approximately 77 ha, which included all of Minto's Arcadia lands, the future transitway corridor (north of Feedmill Creek), and the Campeau Drive ROW (between Huntmar Drive and the Carp River). Refer to the 2006 KWMSS Storm Sewer Minor System Drawing ST-MN and Storm Drainage Area Plan North – Pond 1 Drawing ST-PN provided in **Appendix 'B'**.

Since construction of Pond 1 could not proceed until the Carp restoration works were completed, an 'interim' wet pond facility was constructed to allow for the development of Arcadia Stage 1 residential and commercial, Stage 2, and a portion of the Campeau Drive ROW to proceed. The 'interim' pond is located to the east of Stage 2 and is equipped with two (2) separate inlets and forebays, as shown on the Interim Storm Drainage Plan prepared by IBI and provided in **Appendix 'B'**. It was intended that this existing 'interim' pond would be decommissioned once the Carp River restoration works were completed and the permanent Pond 1 was constructed.

As part of the 2006 KWMSS, the ultimate Pond 1 was conceptually sized within a 1.50 ha block, based on design standards that were applicable at that time. Since then, the Ottawa Sewer Design Guidelines were updated and re-issued in October 2012 and a number of Technical Bulletins were also subsequently issued. The 2012 Design Guidelines provide more details in terms of the modelling approach for infiltration in urban areas (Horton Method), calculation of runoff coefficients based on maximum permissible zoning, calculation of time of concentration, and assessing the performance of the infrastructure under a specific climate change event.

More recently, preliminary sizing of the ultimate Pond 1 was carried out by IBI in accordance with the 2012 Design Guidelines as part of the Report titled "Conceptual Site Servicing Study – Arcadia Stages 1, 2, 5 & 8", dated September 2013. This sizing exercise showed that the Pond 1 block needed to be increased in size from 1.50 ha to approximately 2.45 ha in order to fulfill the requirements of the 2012 Design Guidelines and meet the design objectives of the 2006 KWMSS. The IBI Conceptual Site Servicing

Study provided the following configuration details for Pond 1 (refer to cross section provided in **Appendix 'B'**).

Table 1-2: IBI Pond 1 Concept Design (2013)

Pond Normal Water Level (NWL)	92.65
Pond 1:100 year Hydraulic Grade Line (HGL)	93.54
Pond Size	2.45 ha
Pond Bottom	92.00

It should be noted that the 2.45 ha SWM Block was not confirmed with a HGL analysis as noted in Section 5.6.2.1 of IBI's 2013 Conceptual Site Servicing Study. Additionally, the pond block was sized prior to the development of the calibrated Carp River PCSWMM model by the City. Since 2013, the City has developed a PCSWMM model of the areas tributary to the Carp River, which was calibrated and validated to actual recorded data. A PCSWMM model was subsequently developed to reflect the post-development conditions and a series of hydrographs were generated at key locations that are to be matched under post-development conditions.

Due to the changes to the Design Guidelines, as well as the completion of the Carp River PCSWMM model, the SWM strategy outlined for the Arcadia Development in the 2006 KWMSS and the 2013 IBI Conceptual Site Servicing Study needed to be re-evaluated.

2.0 EVALUATION OF STORM SERVICING AND SWM ALTERNATIVES

2.1 KWMSS Pond 1

JLR developed a conceptual storm sewer design for the Arcadia Development and carried out preliminary stormwater modelling using PCSWMM to re-assess the KWMSS Pond 1 SWM strategy based on current Design Guidelines and to meet the target hydrographs of the City's February 2017 calibrated Carp River PCSWMM model. As per the 2006 KWMSS, this storm servicing strategy would consist of runoff from all existing and future Arcadia Stages being conveyed to a wet pond located east of Riverchase Road (i.e., Street No. 2 per the Draft Plan in **Appendix 'A'**) and ± 450 m north of Campeau Drive.

2.1.1 Hydrological Parameters

Catchment areas for the Arcadia Development were divided based on development stage and land use. Catchment areas were further subdivided for Stages 3, 4, 5 and 6 based on the conceptual storm sewer network developed for each Stage. The following parameters were incorporated into the PCSWMM model for the Arcadia Development:

Table 2-1-1: Land Use Percent Impervious and Runoff Coefficient Values

Land Use	Imperviousness (%)	Runoff Coefficient
Residential (plus roads)	64.3	0.65
Park Lands	14.3	0.30
Pond Blocks	35.0	0.45
Arterial Roads and Transitway	92.9	0.85
Commercial Lots	92.9	0.85

Runoff from existing Stages 1 and 2 and planned Commercial Stage 1, which drains to the existing 'interim' pond was modelled by IBI, as detailed in the Arcadia Stage 2 SWM Report and Stage 2 Inlet Design Brief, dated October 2014. As such, the existing flows to the 'interim' pond were extracted as hydrographs from the IBI XPSWMM model and incorporated in the PCSWMM model for the Arcadia Development at build-out.

As per the Carp River modelling, the Curve Number methodology was used to simulate infiltration. The curve number for each catchment was area weighted from the value used for the area in the City's Carp River model.

The model was used to simulate the 12 hour SCS storm (1:10 year and 1:100 year), which was identified as the critical storm distribution for the Carp River system.

2.1.2 Catchment Storage Simulation Parameters

The Arcadia Development is proposed to be designed with street sag storage in residential areas and on-site storage in commercial blocks. Street sag storage has been included in the model in the form of storage nodes based on a volume of 50 m³/ha, in accordance with the storage requirements outlined the 2006 KWMSS. Storage nodes in the Commercial Blocks have been modelled to contain the 1:100 year event.

The allowable release rate to the minor system was set to 200 L/s/ha for residential areas, which is equivalent to a storm event between the 1:2 to 1:5 year event. An allowable release rate to the minor system of 280 L/s/ha was set for the Campeau Drive ROW and the future transitway corridor, which is equivalent to the 1:10 year event, in accordance with the Design Guidelines for arterials.

2.1.3 Preliminary Trunk Storm Sewer and Pond Design

With the construction of Pond 1, the existing 'interim' pond servicing existing residential Stages 1 and 2 and planned commercial Stage 1 would be decommissioned and runoff from these stages, as well as that from future Stages 4 and 5 would be conveyed to Pond 1 via a trunk storm sewer along Paine Avenue. It is anticipated that this trunk

sewer would be 2750 mm in diameter at its downstream end at the intersection of Riverchase Road.

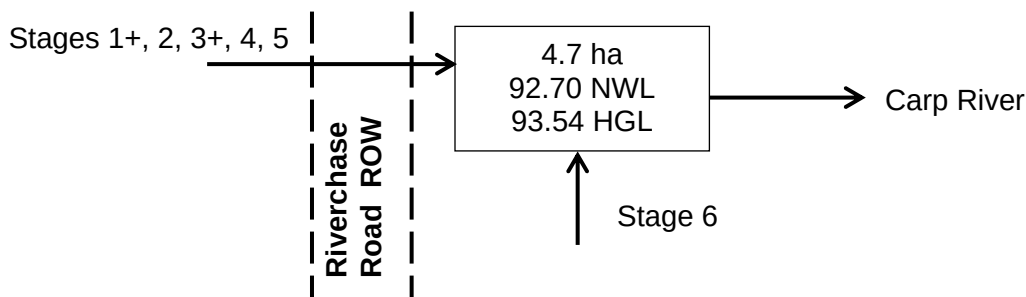
A trunk storm sewer along Riverchase Road would also be required to convey runoff from the future transitway corridor, Campeau Drive ROW, commercial Stages 2 and 3 and residential Stage 3, herein collectively referred to as 'Stage 3+'. The Riverchase Road trunk storm sewer would range in size from 1350 mm in diameter at the Campeau Drive intersection to 1650 mm in diameter at its downstream end. The Paine Avenue trunk and the Riverchase Road trunk storm sewers would ultimately converge to a single 2750 mm diameter inlet to the pond. A separate 1050 mm diameter inlet sewer to the pond would be provided and would service solely Stage 6.

The following Table and Figure provide the characteristics of the KWMSS Pond 1 servicing strategy evaluated in PCSWMM. Note that 'Stage 1+' herein refers to both existing residential Stage 1 and planned commercial Stage 1.

Table 2-1-2: KWMSS Pond 1 Characteristics

Pond (NWL)	92.70 m			
Pond 1:100 year HGL	93.54 m			
Pond Size	4.7 ha*			
Pond Inlets	Stage(s)	Diameter	Invert Elevation	Depth of Submergence
	1+,2,3+,4,5	2750 mm	90.35 m	2.35 m (85% submerged)
	6	1050 mm	92.80 m	0 m
Note *Pond size is conceptual only and does not include area required for access roads and/or clearances to adjacent properties/ROWs				

Figure 2-1: KWMSS Pond 1 Schematic



A normal water level in the pond of 92.70 m was selected based on the 1:2 year water level elevation of 92.69 m in the Carp River at the pond outlet location. A 1:100 year HGL elevation in the pond of 93.54 m was selected to be consistent with the IBI 2013 Concept Design for comparison purposes and to ensure that the HGL in existing Stage 1 and 2 sewers would be less than the current HGL elevations. With the development of a conceptual sewer design and the PCSWMM model, it was determined that a pond

footprint of approximately 4.7 ha would be required to match the post-development target hydrographs of the Carp River. As indicated in the table above, this footprint area does not include any areas required for access roads or clearances to adjacent properties and/or ROWs.

2.1.4 Constraints

In addition to the calculated Pond 1 footprint being approximately 3 ha larger than that identified in the 2006 KWMSS, and approximately 2 ha larger than that in the 2013 IBI concept design, some significant constraints were identified in completing the conceptual storm sewer design and PCSWMM model for the Arcadia Development. The following five constraints were noted:

1. Crossing conflict between Paine Avenue storm sewer and Riverchase sanitary sewer

A crossing conflict would occur between the future Paine Avenue trunk storm sewer conveying runoff from Stages 1+, 2, 4 and 5 to the pond, and the future Riverchase Road sanitary sewer conveying wastewater from Stages 4 and 5 to the existing Campeau Drive trunk sanitary sewer.

As a best case scenario, if the Paine Avenue trunk were to be extended from the existing elliptical sewer at MH 510, at minimum slope (0.15%), with invert to invert connections, it would have a minimum invert elevation of 91.16 m at the intersection of Riverchase Road. Similarly, if the Riverchase Road sanitary sewer were extended at minimum slope (0.20%) from the existing stub at Campeau Drive, it would have a minimum obvert elevation of 91.69 m at the intersection of Paine Avenue. The Riverchase Road sanitary sewer would, therefore, have to be approximately 1 meter lower to be able to cross under the Paine Avenue trunk storm sewer, which is not feasible due to the elevation of the Campeau Drive trunk sanitary sewer. Alternatively, it would have to be 2.8 m higher (with an invert elevation of 94.50 m) to cross over the storm sewer trunk, which would not provide a viable outlet for Stage 5 where existing ground ranges between 93.00 to 93.50 m.

It should also be noted that providing sanitary servicing for Stages 4 and 5 via the existing sanitary sewers on Clonrush Way and Country Glen Way is not considered a viable alternative either, since the existing sanitary sewer obvert at the intersection of Clonrush Way and Paine Avenue is 94.02 m. This obvert elevation is approximately one meter higher than existing ground in Stage 5, meaning that extensive fill works up to 5 m (exceeding the permissible grade raise) would be required to provide a gravity sanitary sewer outlet for Stage 5 via Clonrush Way.

2. HGL exceeds centreline road elevation on Campeau Drive

Stantec is currently in the process of completing the detailed design for Campeau Drive to the south of the Arcadia Development from Country Glen Way to east of the Carp River. Construction of Campeau Drive is anticipated for commence in 2018.

Based on the results of the PCSWMM model provided on Figure C1a in **Appendix 'C'**, the 1:100 year HGL along Campeau Drive ± 120 m to the east of Riverchase Road would be approximately 94.44 m, which is 0.14 m higher than the future road centreline elevation of 94.30 m at that location (see Stantec plan & profile drawing for Campeau Drive provided in **Appendix 'B'**). It is noted that only the 120 m section of the Campeau Drive storm sewer east of Riverchase Road was incorporated in the PCSWMM model as this section was deemed most critical, as it has the lowest proposed centreline elevation. Additionally, this section of storm sewer was modelled as a 750 mm diameter sewer rather than a 600 mm diameter storm sewer, as per Stantec's drawing, in order to minimize the HGL elevation. If a 600 mm diameter were to be used to service the eastern portion of Campeau Drive, then the 1:100 year HGL would increase to 94.49 m or 0.19 m above the proposed road centreline elevation, as shown on Figure C1b of **Appendix 'C'**.

3. Submergence of Riverchase Road Trunk and Campeau Drive Storm Sewers

To provide a gravity storm sewer outlet for the eastern portion of the Campeau Drive ROW, the Riverchase Road trunk storm sewer at the intersection of Campeau Drive would have to have a maximum invert elevation of 91.30 m, as per the Campeau storm sewer design prepared by Stantec (refer to the plan and profile drawing provided in **Appendix 'B'**). The Riverchase Road trunk storm sewer at this location would be 1350 mm in diameter and would have an obvert elevation of 92.67 m. Based on a normal water level of 92.70 m in the pond, the entire Riverchase Road trunk sewer from the pond to Campeau Drive (approximately 500 m) would be fully (100%) submerged, as would a ± 150 m section of Campeau Drive storm sewer east of Riverchase Road. Standing water would further extend an additional ± 20 m along the Campeau Drive storm sewer east of Riverchase Road, as well as 111 m along the Campeau Drive storm sewer west of Riverchase Road, until free flowing

4. Standing Water along Paine Avenue and Clonrush Way Trunk Storm Sewers

Although the storm sewers in the northern portion of the Arcadia Development will not be completely submerged, there will be extensive standing water along existing and future storm sewers. Based on a normal water level of 92.70 m, standing water will extend along the storm sewer system as follows:

- $\pm$ 440 m from the Pond to the west leg of Saddleback Crescent located in existing Stage 2.
- $\pm$ 145 m along Clonrush Way from Paine Avenue to south of Halyard way in Stage 2.

5. High HGL in southeastern portion of Stage 3 adjacent to Campeau Drive

Based on the results of the PCSWMM model provided on Figure C1a in **Appendix 'C'**, the 1:100 year HGL in future Stage 3 sewer northwest of the Campeau Drive and Riverchase Road intersection is approximately 95.10 m. It is noted that this HGL is almost equivalent to the adjacent Campeau Drive centreline elevation of 95.15 m, as per the Stantec Plan & Profile provided in **Appendix 'B'**. To provide the minimum required 0.30 m freeboard between the underside of footing (USF) and the HGL, a typical unit will require a minimum clearance of 2 m between the HGL and road centreline elevation. Based on this typical relationship, the most southerly street in Stage 3 would have a centreline elevation of approximately 97.10 m which would result in more than a 2 m grade difference between Stage 3 and Campeau Drive.

2.2 Pond Bisected By Riverchase Road with Hydraulically Connected Cells

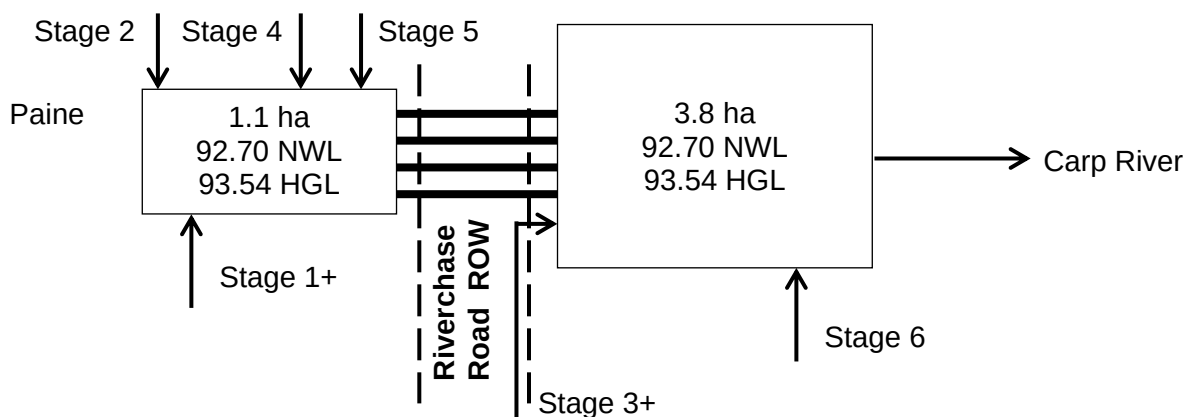
The KWMSS Pond 1 servicing concept was re-evaluated with the objective of eliminating the critical crossing constraint at the intersection of Paine Avenue and Riverchase Road, as well as reducing submergence and standing water along the storm sewer system. An alternative stormwater servicing strategy was therefore developed where the existing 'interim' pond would be retrofitted and expanded to become a permanent facility. The pond would be bisected by Riverchase Road and the forebays and inlets for Stages 1+, 2, 4 and 5 would be located on the western side of Riverchase Road, while the forebays and inlets for Stages 3+ and 6 would be located on the eastern side of Riverchase Road. The western and eastern portions of the pond would be hydraulically connected via four 2.4 m by 1.5 m box culvert crossing under Riverchase Road.

2.2.1 Preliminary Trunk Storm Sewer and Pond Design

The following table and schematic provide the key characteristics of this Option:

Table 2-2: Pond Bisected by Riverchase with Hydraulically Connected Cells Characteristics

Pond NWL	92.70 m				
Pond 1:100 year HGL	93.54 m				
Pond Size	4.9 ha (1.1 ha west of Riverchase, 3.8 ha east of Riverchase)*				
Pond Inlets	Stage	Outlet Location Relative to Riverchase	Diameter	Invert Elevation	Depth of Submergence
	1+	West	2.4 x 1.8	92.30 m	0.4 m
	2	West	2.4 x 2.2	91.39 m	1.31 m
	3+	East	1650 mm	90.35 m	2.35 m (142% submerged)
	4	West	900 mm	92.80 m	0 m
	5	West	825 mm	92.80 m	0 m
	6	East	1050 mm	92.80 m	0 m
Note *Pond size is conceptual only and does not include area required for access roads and/or clearances to adjacent properties/ROWs					

Figure 2-2: Pond Bisected by Riverchase with Hydraulically Connected Cells Schematic

By maintaining the existing Stage 1+ and 2 pond inlets on the west side of Riverchase Road, the critical crossing constraint at the intersection of Paine Avenue and Riverchase Road, as identified in the KWMSS Pond 1 option above, would be eliminated. The inclusion of separate pond inlets for Stages 4 and 5 would also eliminate any standing water along the future Paine Avenue trunk storm sewer (i.e., east of Clonrush Way).

2.2.2 Constraints

Although some critical constraints would be eliminated by making use of the existing 'interim' pond and inlets to the west of Riverchase Road, it should be noted that this servicing alternative requires that the pond be ± 0.2 ha larger than the KWMSS Pond 1 option. Additionally, the following four constraints were noted:

1. HGL exceeds centreline road elevation on Campeau Drive

As shown in the PCSWMM results presented on Figure C2 in **Appendix 'C'**, the 1:100 year HGL elevations in Stage 3+ would remain essentially the same as those determined for the KWMSS Pond 1 option presented on Figure C1a. The HGL at Campeau Drive (120 m east of Riverchase Road) would be 0.14 m higher than the future road centreline elevation, as per Stantec's design for Campeau Drive. This HGL is based on a 750 mm diameter Campeau Drive storm sewer rather than the 600 mm diameter sewer proposed by Stantec.

2. Submergence of Riverchase Road Trunk and Campeau Drive Storm Sewers

As in the KWMSS Pond 1 strategy, the ± 500 m long Riverchase Road trunk storm sewer and the ± 145 m section of the Campeau Drive storm sewer would remain fully submerged.

3. Standing Water along Paine Avenue and Clonrush Way Trunk Storm Sewers

Standing water would extend along the existing storm sewer system as follows:

- ± 220 m along Paine Avenue from the Stage 2 pond inlet to the west leg of Saddleback Crescent
- ± 145 m along Clonrush Way from Paine Avenue to south of Halyard Way
- ± 25 m along Stage 1+ outlet east of Clonrush Way

Note that the extent of standing water has been reduced by 195 m compared to the KWMSS Pond 1 alternative.

4. High HGL in southeastern portion of Stage 3 adjacent to Campeau Drive

As shown in the PCSWMM results presented on Figure C2 in **Appendix 'C'**, the 1:100 year HGL elevation in the Stage 3 sewer, north of Campeau Drive will be approximately equal to the centreline elevation of 95.15 m on Campeau Drive. To provide adequate cover over this HGL, a grade difference of at least 2 m is anticipated between Campeau Drive and the adjacent Stage 3 street to the north.

2.3 **Pond Bisected By Riverchase Road with Hydraulically Separated Cells**

The pond bisected by Riverchase Road alternative was re-evaluated with the objective of reducing the overall pond footprint. It was noted that currently the existing 'interim' pond has a 1:100 year HGL elevation of 94.21 m and, therefore, the existing storm sewer system can accommodate a higher HGL than 93.54 m, as was used in the

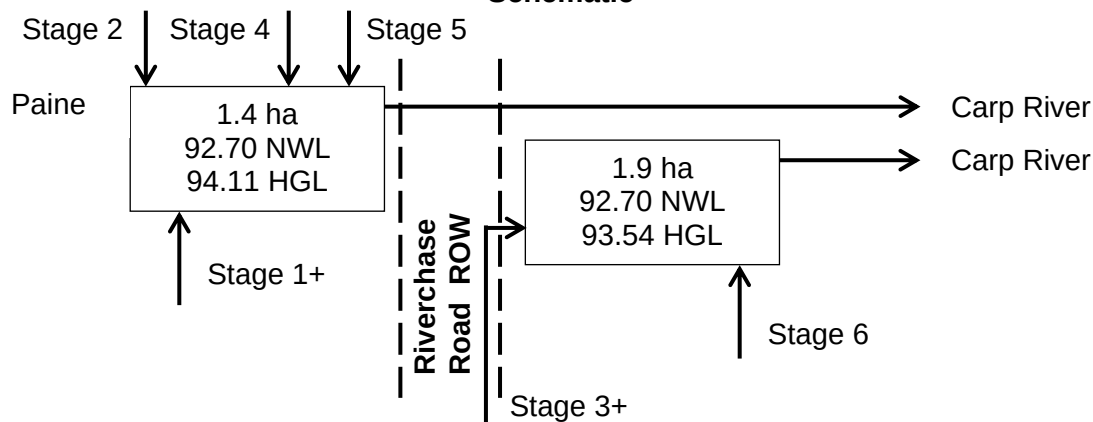
previous pond analyses. A new alternative was, therefore, developed where the pond would be bisected by Riverchase Road, but the west and east cells would be hydraulically separated and would have separate outlets to the Carp River. This alternative would allow for the 1:100 year HGL of the west cell to be maximized and the total pond footprint area to be minimized. As in the previous servicing alternative, Stages 1, 2, 4 and 5 would outlet to the west cell while Stages 3+ and 6 would outlet to the east cell.

2.3.1 Preliminary Trunk Storm Sewer and Pond Design

The following table and schematic provide the characteristics of the bisected pond option with hydraulically separated cells.

Table 2-3: Pond Bisected by Riverchase Road with Hydraulically Separated Cells Characteristics

WEST CELL				
Pond NWL	92.70 m			
Pond 1:100 year HGL	94.11 m			
Pond Size	1.4 ha*			
Pond Inlets	Stage(s)	Diameter	Invert Elevation	Depth of Submergence
	1+	2.4 x 1.8	92.30 m	0.4 m
	2	2.4 x 2.2	91.39 m	1.31 m
	4	900	92.80 m	0 m
	5	825	92.80 m	0 m
EAST CELL				
Pond NWL	92.70 m			
Pond 1:100 year HGL	93.54 m			
Pond Size	1.9 ha*			
Pond Inlets	Stage(s)	Diameter	Invert Elevation	Depth of Submergence
	3+	1650 mm	90.35	2.35 m
	6	1050 mm	92.80	0 m
Note *Pond size is conceptual only and does not include area required for access roads and/or clearances to adjacent properties/ROWs				

**Figure 2-3: Pond Bisected by Riverchase Road with Hydraulically Separated Cells
Schematic**

The 1:100 year HGL of the west cell was raised from 93.54 to 94.11 m, as this was the maximum HGL that would allow for the west cell serving as the outlet for Stages 1+, 2, 4, and 5 to be confined to a 1.4 ha block to the west of Riverchase Road. It is noted that this HGL is 0.10 m below the current HGL of 94.21 at the 'interim' pond and, therefore, does not negatively impact existing sewers. The cell serving as the outlet for Stages 3+ and 6 to the east of Riverchase Road would have a footprint area of 1.9 ha, resulting in a total pond footprint area of 3.3 ha.

2.3.2 Constraints

Although this servicing alternative successfully minimizes the total pond footprint area requirement, the constraints previously identified for the bisected pond option with hydraulically connected cells would exist. The following four constraints were noted:

1. HGL exceeds centreline road elevation on Campeau Drive

As shown in the PCSWMM results presented on Figure C3 in **Appendix 'C'**, the 1:100 year HGL elevations in Stage 3+ would be roughly the same as those determined for the KWMSS Pond 1 and the bisected hydraulically connected pond options. The 1:100 year HGL at Campeau Drive (120 m east of Riverchase Road) would be 0.14 m higher than the future road centreline elevation, as per Stantec's design for Campeau Drive. This result is based on a 750 mm diameter storm sewer on Campeau Drive rather than the 600 mm diameter sewer proposed by Stantec.

2. Submergence of Riverchase Road Trunk and Campeau Drive Storm Sewers

As in the two previous SWM analyses, the ± 500 m long Riverchase trunk storm sewer and the ± 145 m section of the Campeau Drive storm sewer east of the Riverchase Road intersection would be fully submerged.

3. Standing Water along Paine Avenue and Clonrush Way Trunk Storm Sewers

Standing water would extend along the existing storm sewer system as follows:

- ± 220 m along Paine Avenue from the Stage 2 pond inlet to the west leg of Saddleback Crescent
- ± 145 m along Clonrush Way from Paine Avenue to south of Halyard Way
- ± 25 m along Stage 1+ outlet east of Clonrush Way

Note that the extent of standing water has been reduced by 195 m compared to the KWMSS Pond 1 alternative.

4. High HGL in southeastern portion of Stage 3 adjacent to Campeau Drive

As shown in the PCSWMM results presented on Figure C3 in **Appendix 'C'**, the 1:100 year HGL elevation in the Stage 3 sewer north of Campeau Drive will be approximately equal to the centreline elevation of 95.15 m on Campeau Drive. To provide adequate cover over this HGL, a grade difference of at least 2 m is anticipated between Campeau Drive and the adjacent Stage 3 street to the north.

2.3.3 Two Ponds: Campeau Drive and Paine Avenue

The pond alternative with hydraulically separate cells was further investigated with the objectives of reducing the HGL at Campeau Drive and the southerly limit of Stage 3, as well as the submergence along the Riverchase Road trunk storm sewer. It was determined that hydraulically separating two cells of a pond is essentially equivalent to a two pond concept and that if a second pond were to be located further south, closer to the Campeau Drive, then the aforementioned constraints could be mitigated. The 'Campeau Pond' was, therefore, conceptualized to serve as the dedicated stormwater outlet for Stages 3+. The Campeau Pond would be located south of Campeau Drive and west of the Carp River Corridor, within the Commercial Stage 3 lands, and would outlet to the Carp River immediately upstream of the Campeau Bridge crossing.

In addition to the Campeau Pond, the existing 'interim' wet detention pond would be retrofitted and expanded in size to become a permanent facility, herein referred to as the 'Paine Pond'. The Paine Pond would serve as the outlet for Stages 1+, 2, 4 and 5. This

pond would be bisected by Riverchase Road and hydraulically connected by four 2.4 m x 1.5 m box culvert crossings.

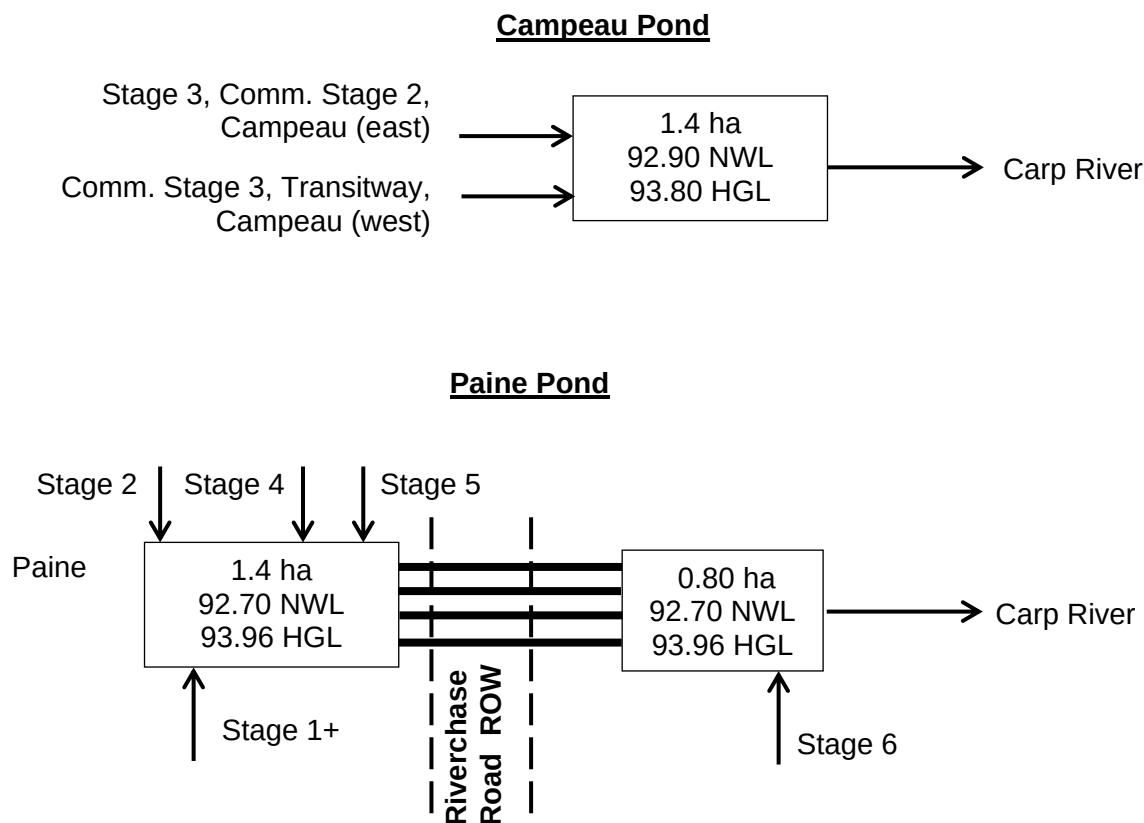
2.3.4 Preliminary Trunk Storm Sewer and Pond Design

To minimize sewer size and submergence, the Campeau Pond would have two inlet pipes: one for lands north of Campeau Drive and the eastern portion of the Campeau Drive ROW, and one for the western portion of the Campeau Drive ROW and lands to the south including the transitway corridor. The two inlet sewers would have a minimum invert elevation of 92.00 at the intersection of Riverchase Road and Campeau Drive to facilitate the crossing of the existing 600 mm diameter feedermain along Campeau Drive (as shown in the Campeau Drive As-constructed Plan & Profile prepared by IBI provided in **Appendix 'B'**).

The characteristics of the two pond strategy are summarized in the table and schematic below.

Table 2-4: Two Pond Characteristics

CAMPEAU POND				
Pond NWL	92.90 m			
Pond 1:100 year HGL	93.80 m			
Pond Size	1.4 ha*			
Pond Inlets	Stages	Diameter	Invert Elevation	Depth of Submergence
	3, Comm. 2, Campeau (east)	1650 mm	91.90	1 m (61%)
	Comm. 3, Transitway, Campeau (west)	1200 mm	91.90	1 m (83%)
RETROFITTED PAINE POND				
Pond NWL	92.70			
Pond 1:100 year HGL	93.96			
Pond Size	2.2 ha (1.4 ha west, 0.8 ha east)*			
Pond Inlets	Stage	Diameter	Invert Elevation	Depth of Submergence
	1+	2.4 x 1.8	92.30	0.4 m
	2	2.4 x 2.2	91.39	1.3 m
	4	900	92.80	0 m
	5	825	92.80	0 m
	6	1050	92.80	0 m
Note *Pond sizes are conceptual only and do not include area required for access roads and/or clearances to adjacent properties/ROWs				

Figure 2-4: Two Pond Schematic

The normal water level of 92.90 was selected for the Campeau Pond based on a 1:2 year water elevation of 92.85 m in the Carp River at the pond outlet location, upstream of the Campeau Bridge. The HGL elevation of 93.80 m was allocated at the pond as this elevation was determined to be the lowest possible HGL elevation that would result in a pond footprint that would fit within the available 1.7 ha Commercial Stage 3 block. The Paine Pond HGL elevation of 93.96 m was selected to be consistent with the 2006 KWMSS.

The results of the PCSWMM model (presented on Figure C4a of **Appendix 'C'**) indicate that with the introduction of the Campeau Pond, the 1:100 year HGL on Campeau Drive (120 m of east of the Riverchase Road intersection) would be lowered to 94.07 m, which is 0.23 m below Stantec's proposed centreline elevation and ± 0.38 m lower than the HGL determined in the previous SWM alternatives. Furthermore, the HGL at the southeastern limit of Stage 3 would be reduced to 94.23, which would reduce the grade difference between Stage 3 and Campeau Drive. Additionally, based on a NWL elevation of 92.90 m, the Riverchase Road trunk storm sewer would not be fully submerged.

It is noted that the HGL results indicated above and presented on Figure C4a are based on the use of a 750 mm diameter storm sewer on Campeau Drive, rather than a 600 mm diameter sewer as indicated on Stantec's plans for Campeau Drive. This increase in storm sewer size was incorporated to minimize the HGL on Campeau Drive. Should a 600 mm diameter sewer be constructed on Campeau Drive, then an HGL elevation of approximately 94.59 m would be anticipated, as shown on Figure C4b in **Appendix 'C'**, thus exceeding the proposed centreline elevation by 0.29m.

2.3.5 Constraints

Although the two pond strategy mitigates many of the constraints identified in the previous SWM options evaluated, such as the high HGL on Campeau Drive and in Stage 3, as well as the submergence of the Riverchase Road trunk storm sewer, the following two constraints were noted:

1. Submergence and Cover Depth of Campeau Drive storm sewer

The 120 m long section of storm sewer on Campeau Drive east of Riverchase Road would be completely submerged. Based on an invert to invert connection to the Stage 3 outlet pipe, the Campeau Drive sewer would have a downstream invert elevation of 92.03 at the intersection of Riverchase Road. As previously mentioned, a minimum invert elevation of 92.00 is required for the storm sewer outlet to cross over the existing 600 mm diameter feedermain. Extending the 750 mm diameter storm sewer at a slope of 0.13% along Campeau Drive, the obvert elevation of the pipe at the upstream end, 120 m east of Riverchase Road, would be 92.95 which is only 5 cm higher than the NWL of the pond. Furthermore, with a centreline elevation of 94.30, there would only be 1.35 m of cover above the sewer.

2. Standing Water along Paine Avenue and Clonrush Way Trunk Storm Sewers

Standing water would extend along the existing storm sewer systems outletting to the Paine Avenue Pond as follows:

- 220 m along Paine Avenue from the Stage 2 pond inlet to the west leg of Saddleback Crescent
- 145 m along Clonrush Way from Paine Avenue to south of Halyard Way
- 25 m along Stage 1+ outlet east of Clonrush Way

Note that the extent of standing water has been reduced by 195 m compared to the KWMSS Pond 1 alternative.

3.0 RECOMMENDED STORM SERVICING AND SWM STRATEGY

3.1 Advantages of Two Pond Strategy

Based on the information presented above, the two pond strategy which includes the Campeau Pond and the Paine Pond is recommended to service the Arcadia Development. Key advantages that support the implementation of the proposed two pond strategy over the other three options evaluated are as follows:

- The crossing conflicts between storm and sanitary sewer systems are eliminated.
- The Campeau Pond provides the lowest HGL on Campeau Drive, which is also below the proposed minimum centreline elevation of 94.30.
- The Campeau and Paine Ponds require a smaller total footprint area compared to the KWMSS Pond 1 Option.
- The lengths of submergence and standing water in storm sewers are reduced resulting in reduced maintenance costs.
- The Campeau Pond provides the lowest possible HGL at the southern limit of Stage 3, thus minimizing the grade difference between Stage 3 and Campeau Drive.

3.2 Addressing Constraints of the Recommended Two Pond Strategy

The constraints associated with the two pond strategy could be mitigated in a number of ways.

Regarding the shallow Campeau Drive storm sewer, a combination of insulation and a raised centreline elevation could improve the cover depth constraint associated with this sewer. Raising the centreline would also increase the cover to the HGL elevation. It is understood however, that raising the Campeau Drive centreline elevation would require additional light weight fill. Alternatively, if the Campeau Drive storm sewer were reversed to flow easterly and outlet to the pond further east, then the sewer could be lowered to improve cover depth. A lower invert elevation would be feasible at ± 180 m east of Riverchase Road since the existing Campeau Drive watermain begins to lower to cross the Carp River at that location (refer to IBI as-constructed profile drawing for Campeau Drive provided in **Appendix 'B'**). Top of watermain elevation at this location is roughly at 90.00 m compared to 92.00 m at the intersection with Riverchase Road. Reducing the elevation of the Campeau Drive sewer would, however increase the extent of standing water in the sewer.

Regarding the Paine Avenue Pond, although the extent of standing water along the existing sewers cannot be changed due to the elevation of the 1:2 year event in the Carp River, the extent of standing water along the future sewer system has been minimized by including multiple outlets and smaller diameter sewers which was made possible by the addition of the Campeau Pond.

Recently, the City shared with JLR some possible deficiencies identified by their staff in regard to the existing 'interim' pond and general concerns, should it be converted to a permanent facility. These deficiencies were provided via an e-mail correspondence dated March 9, 2017, a copy of which has been included in **Appendix 'D'**. Each of the deficiencies identified was taken into consideration and has either been addressed through the conceptual analyses completed or will be addressed at detailed design. The details associated with these deficiencies and proposed expansion and retrofitting work are summarized in Table D1 in **Appendix 'D'**.

3.3 Water Quality and Quantity Control for the Two Pond Strategy

The proposed Campeau and Paine Ponds have been sized to provide water quality control to achieve an MOECC Normal Level of Protection (i.e., 70% total suspended solids removal) before outletting to the Carp River, in accordance with the Carp River Watershed Study and the 2006 KWMSS. The Campeau Pond will, therefore, have a permanent pool of 2,150 m³ to provide quality control for Stages 3+. Preliminary water quality calculations are provided in **Appendix 'E'**.

As previously noted, the normal water level in the Paine Pond will be reduced from the current 'interim' elevation of 93.00 m to 92.70 m. The resulting loss of permanent pool from the lowered NWL will be offset with an expansion of the permanent pool. With this expansion, the permanent pool west of Riverchase will be increased from its current volume of 3,276 m³, as per IBI Report and Design Brief Arcadia Stage 2 SWM Report and Stage 2 Inlet Design Brief, dated October 2014, to 3,690m³ of storage required to provide quality control for Stages 1, 2, 4 and 5. Refer to quality control calculations provided in **Appendix 'E'**. Existing forebays for Stages 1 and 2 will be retrofitted, and new forebays for Stages 4 and 5 will be added to the expanded pond in accordance with the recommendations of the MOECC Stormwater Management Planning and Design, dated March 2003 (SMPDM).

The permanent pool for Stage 6 will be located east of Riverchase Road and will have a capacity of 630 m³. Calculations on the Stage 6 permanent pool and forebay sizing are included in **'Appendix 'E'**. Both forebay and permanent pool were sized to meet the requirements of the MOECC SWMPDM. It is noted that prior to the development of Stage 6, the portion of the pond east of Riverchase will be comprised of only extended

detention storage, the outlet berm and associated restrictor. With the development of Stage 6, an additional permanent pool will be incorporated into the eastern portion of the pond to provide water quality control for Stage 6, as well as additional storage for quantity control. A single outlet structure located at the downstream end of the eastern side of the pond will control the storm discharge to the Carp River.

The 2006 KWMSS indicated that the outlet structure for Pond 1 would consist of three outlets; a low flow french drain outlet to provide flow augmentation to the Carp River, an extended water quantity volume outlet and a weir outlet for peak flow reduction control. Both the Paine Avenue Pond and Campeau Drive Pond will have this outlet configuration.

3.4 Major System with Two Pond Strategy

Major overland flows generated in Stage 3 will be conveyed southeasterly as shown on the Conceptual Grading Plan Drawing CG1. Since major overland flow crossing the Campeau Drive is not permitted in accordance with Design Guidelines (i.e., major system flows may not cross arterial roadways), a spillover area northwest of the Campeau Drive / Riverchase Road intersection is anticipated to be required assuming that the roadway sags provide insufficient major system storage. Major system flows stored in the dry pond would then be gradually released to the minor system.

Similarly, major system flows in Stages 4 and 5 will be conveyed to a dry pond area located in the park block of Stage 5 where they will be gradually released to the minor system. Major flows generated in Stage 6 will be conveyed directly to the Paine Pond.

Conversely, it is anticipated that Commercial Stages 2 and 3 will be designed with on site storage to contain the 1:100 year event.

3.5 Simulation of the Two Pond SWM Strategy

3.5.1 Pond Simulation Parameters

The retrofitted and expanded Paine Avenue Pond was simulated as two storage nodes connected via a four barrel conduit of 2.4 m by 1.5 m box culverts. The Campeau Drive Pond was modelled as a single storage node. The ponds were simulated based on the configurations summarized in Table 3-1. The flow augmentation outlets were used to set the invert levels of the storage nodes, which represent the top of the permanent pool; however, the outlets were not included in the model as the release rate would be negligible.

Table 3-1: Pond Storage Parameter Summary

Parameter	Paine Avenue Pond (West node)	Paine Avenue Pond (East node)	Campeau Drive Pond
Storage Volume (m ³)	12,613	8,660	20,120
Maximum Active Storage Depth (m)	1.2	1.2	0.9
Surface Area at Maximum Depth (m ²)	13,700	8,300	14,200
Permanent Pool Elevation (m)	92.70	92.70	92.90
Active Storage Outlet Elevation (m)	<i>The West node connects to the East node via a 4 barrel 2.4 m by 1.5 m box culvert with an invert elevation of 92.7 m</i>	92.85	93.05
Active Storage Outlet Diameter (mm)		250	250
Weir Elevation (m)		93.65	93.70
Weir Dimensions (L x H) (m)		30 x 0.35	13 x 0.35
High Water Level Elevation (m)	93.96	93.96	93.80

3.5.2 Simulation Results

The outflows from the end-of-pipe facilities need to conform to the flows and water levels set along the Carp River. The impact of the addition of the two pond strategy to the Carp River is summarized in the model results provided in the following tables. Refer to **Appendix 'F'** for the complete PCSWMM model output files for the recommended two pond strategy.

Table 3-2: City of Ottawa Carp River Model

	1:10 year	1:100 year
Peak Flow in Carp River downstream of Campeau Drive (m ³ /s)	28.28	44.60
...at (hours)	11:40	10:40
Peak Flow in Carp River downstream of development (m ³ /s)	27.06	43.46
...at (hours)	12:00	12:00

Table 3-3: Carp River Model Results with Two Pond SWM Strategy

	1:10 year	1:100 year
Paine Avenue Pond Peak Discharge Rate (m ³ /s)	1.25	4.20
...at (hours)	6:45	6:25

	1:10 year	1:100 year
Campeau Drive Pond Peak Discharge Rate (m ³ /s)	0.10	0.55
...at (hours)	8:50	6:45
Peak Flow in Carp River downstream of Campeau Drive (m ³ /s)	28.37	44.58
...at (hours)	11:40	10:40
Peak Flow in Carp River downstream of development (m ³ /s)	26.99	43.39
...at (hours)	12:00	12:00

The results presented in Table 3-3 above show that the addition of the Campeau and Paine Ponds has a negligible impact on peak flow and time to peak in the Carp River. This is due to the low release rates from the Ponds and their timing, which occurs several hours prior to the peak flow in the Carp River.

4.0 SANITARY SEWER OVERFLOWS

The sanitary sewers in Arcadia ultimately outlet to the Signature Ridge Pump Station (SRPS). The SRPS does not have capability for an emergency overflow within the surrounds of the pump station, and therefore alternative emergency overflows are provided within the SRPS service area. The purpose of an emergency overflow is to minimize the risk of basement flooding within the sewer system in the event of a catastrophic failure, and is a last line of protection during a situation where power back-up and redundant pump features within the pumping station and inoperable.

The “Update to: Signature Ridge Pump Station Hydraulic Grade Line Analysis Arcadia Stage 2” (IBI, September 2014)” presented proposed overflow configurations which utilized stormwater management facilities as overflow points. There is an emergency sanitary overflow located at the existing ‘interim’ pond for Stages 1 and 2.

The elevation of the emergency overflow outlet impacts the HGL of the sanitary sewer system, as the design assumes the SRPS is inoperable during wet weather flow. Given that the topography within the undeveloped portion of the Arcadia Development is relatively flat, it is essential to maintain the HGL in the sanitary sewer system as low as possible. Consequently, emergency overflows are sought at both the Campeau Pond and the Paine Pond. Furthermore, with the additional forebays proposed for the storm

servicing of Stages 4, 5 and 6, there is an opportunity to incorporate additional emergency overflows which would further lower the HGL within the surrounding sanitary sewer system.

The emergency overflows will only operate during the simultaneous occurrence of the 1:100 year storm event and catastrophic failure of the SRPS.

5.0 CONCLUSION

Based on the information presented above, the two pond strategy which includes the Campeau Pond and the Paine Pond is proposed to service the Arcadia Development. This servicing and stormwater management strategy can be developed at detailed design to meet regulatory requirements, and will manage stormwater more efficiently compared to the other three options evaluated. Key advantages that support the implementation of the proposed two pond strategy over the other three options evaluated are as follows:

- The crossing conflicts between storm and sanitary sewer systems are eliminated.
- The Campeau Pond provides the lowest HGL on Campeau Drive, based on a 750 mm diameter storm sewer on Campeau Drive. Furthermore, the HGL is below the proposed minimum centreline elevation of 94.30 m whereas all other SWM alternatives evaluated resulted in the HGL being above the proposed centreline elevation.
- The Campeau and Paine Ponds require a smaller total footprint area compared to the KWMSS Pond 1 Option.
- The lengths of submergence and standing water in storm sewers are reduced, resulting in reduced maintenance costs.
- The Campeau Pond provides the lowest possible HGL at the southern limit of Stage 3, thus minimizing the grade difference between Stage 3 and Campeau Drive.

For the above reasons, we recommend that the proposed deviation to the 2006 KWMSS SWM strategy which would utilize two wet ponds to service the Arcadia Development, the Campeau Drive ROW and the future Transitway corridor, be implemented.

Prepared by:



Hilary MacKay, P.Eng.

Reviewed by:



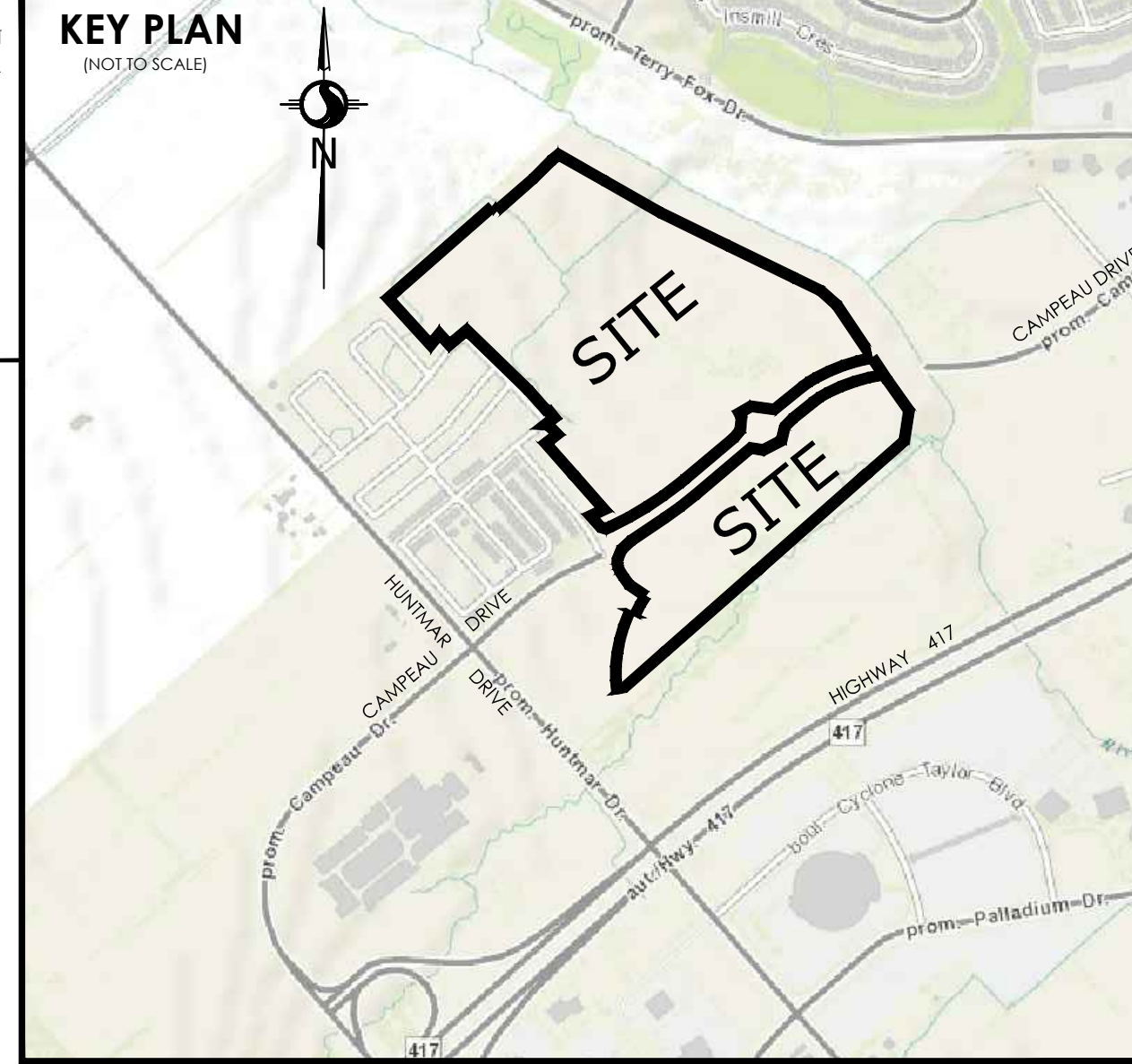
Guy Forget, P.Eng.

APPENDIX 'A'

DRAFT PLAN OF SUBDIVISION

SUBJECT TO THE CONDITIONS, IF ANY, SET FORTH IN OUR LETTER DATED 29-09-2015.
THIS DRAFT PLAN IS APPROVED BY THE CITY OF OTTAWA UNDER SECTION 51 OF THE PLANNING ACT.
THIS _____ DAY OF _____ 2016.

DERIBICK MOORE, MANAGER
DEVELOPMENT REVIEW, SUBURBAN SERVICES
PLANNING, INFRASTRUCTURE AND ECONOMIC
DEVELOPMENT DEPARTMENT, CITY OF OTTAWA



DRAFT PLAN OF SUBDIVISION

PART OF LOTS 3 AND 4 CONCESSION 1 (TOPOGRAPHIC TOWNSHIP OF MARCH) CITY OF OTTAWA

Scale 1:1250
METRIC CONVERSION
DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE
CONVERTED TO FEET BY DIVIDING BY 0.3048

SCHEDULE OF LAND USE			
BLOCKS	USE	UNITS	AREA (Hq/oc)
1 TO 31	RESIDENTIAL	-	9,922.41
32 AND 33	FUTURE RESIDENTIAL	-	8,772.32
34 AND 35	PARK	-	2,877.09
36 AND 37	SEASON POND	-	3,348.27
38 TO 42	COMMERCIAL	-	10,292.35
43 TO 46	OPEN SPACE	-	3,827.43
47	FUTURE TRANSIT WAY	-	2,805.42
48	WALKWAY	-	0.000.08
STREETS	STREET	-	3,407.83
TOTAL			47,821.17

INFORMATION: REQUIRED UNDER SECTION 51 (17) OF THE PLANNING ACT R.S.O. 1990

- SEE PLAN
- SEE PLAN
- SEE PLAN
- SEE PROPOSED LAND USE SCHEDULE (ABOVE)
- SEE PLAN
- SEE PLAN
- SEE PLAN
- CITY WATER AVAILABLE
- SEE SOIL REPORT
- SEE TOPOGRAPHICAL INFORMATION
- ALL CITY SERVICES AVAILABLE
- SUBJECT TO EASEMENT PER INST. NO. OC1529517

OWNER'S CERTIFICATE
I HEREBY AUTHORIZE STANTEC GEOMATICS LTD. TO SUBMIT THIS DRAFT PLAN OF
SUBDIVISION ON MY BEHALF

DATE

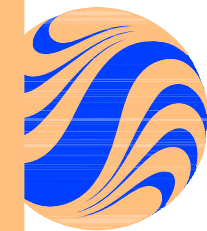
SURVEYOR'S CERTIFICATE
I HEREBY CERTIFY THAT THE BOUNDARIES OF THE SUBJECT LANDS AND THEIR RELATIONSHIP TO
ADJOINING LANDS HAVE BEEN ACCURATELY AND CORRECTLY SHOWN.

DATE

BRYAN J. WEBSTER
ONTARIO LAND SURVEYOR

APPENDIX 'B'

BACKGROUND STORM SERVICING AND SWM DOCUMENTS



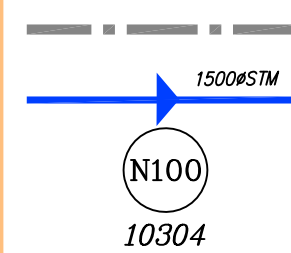
Stantec Consulting Ltd.
1505 Laperriere Avenue
Ottawa ON Canada
K1Z 7T1
Tel. 613.722.4420
Fax. 613.722.2799
www.stantec.com

Stantec

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Legend



KANATA-WEST CONCEPT
PLAN BOUNDARY
PROPOSED STORM SEWER & SIZE
STORM NODE
STORM SEWER IDENTIFICATION



2	REVISED FOR DEC.21/05 SUBMISSION	GBU	SJP	DEC.21/05
1	REVISED AS PER CITY COMMENTS (Sept.16/05)	GBU	MAF	OCT.28/05
Revision		By	Appd.	Date
File Name:	160400406	LM	MAF	MAF
		Dwn.	Chkd.	Dsgn.

Seals

Client/Project

Kanata West Concept Plan
Master Servicing Study

Ottawa, Ontario

Title

MODEL SCHEMATIC
STORM SEWER MINOR SYSTEM

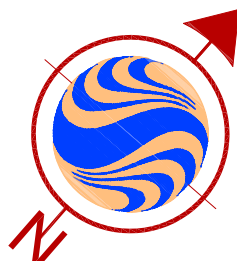
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Drawing No.	Sheet	Revision

ST-MN

6 of 7

2

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--- KANATA-WEST CONCEPT PLAN BOUNDARY
--- POND DRAINAGE BOUNDARY
--- STORM SEWER DRAINAGE LIMIT

A-1

A= 72.31 ha.
C= 0.60
I= 246 L/s
MNR= N4A01
MJR= 4A01
INF= 73 mm/yr

DRAINAGE AREA IDENTIFICATION
AREA IN HECTARES
RUNOFF COEFFICIENT
100yr. INLET CAPACITY (L/s)
MINOR SYSTEM NODE NUMBER
MAJOR SYSTEM SEGMENT NUMBER
INFILTRATION

Notes

* REFER TO FIGURE 3.2 IN KANATA WEST MASTER SERVING STUDY FOR FURTHER INFILTRATION DETAILS

1. THOSE AREAS WHICH ARE COMPLETELY SURROUNDED BY ARTERIAL ROADWAYS (SPECIFICALLY AREAS A-8, C-4, C-12, C-15, C-17, C-18) MUST PROVIDE SURFACE STORAGE IN THE AMOUNT OF 360mm/ha, OR IN SUFFICIENT QUANTITY TO DEMONSTRATE COMPLETE CONTAINMENT OF THE 100yr EVENT. (i.e. NO MAJOR SYSTEM FLOW IN THE 1:100yr EVENT)



Revision	Description	By	App'd	Date
2	REVISED FOR DEC.21/05 SUBMISSION	GBU	SJP	DEC.21/05
1	REVISED AS PER CITY COMMENTS (Sept.16/05)	GBU	MAF	OCT.26/05

Client/Project

Kanata West Concept Plan
Master Servicing Study

Ottawa, Ontario

Title

STORM DRAINAGE AREA PLAN
NORTH PONDS

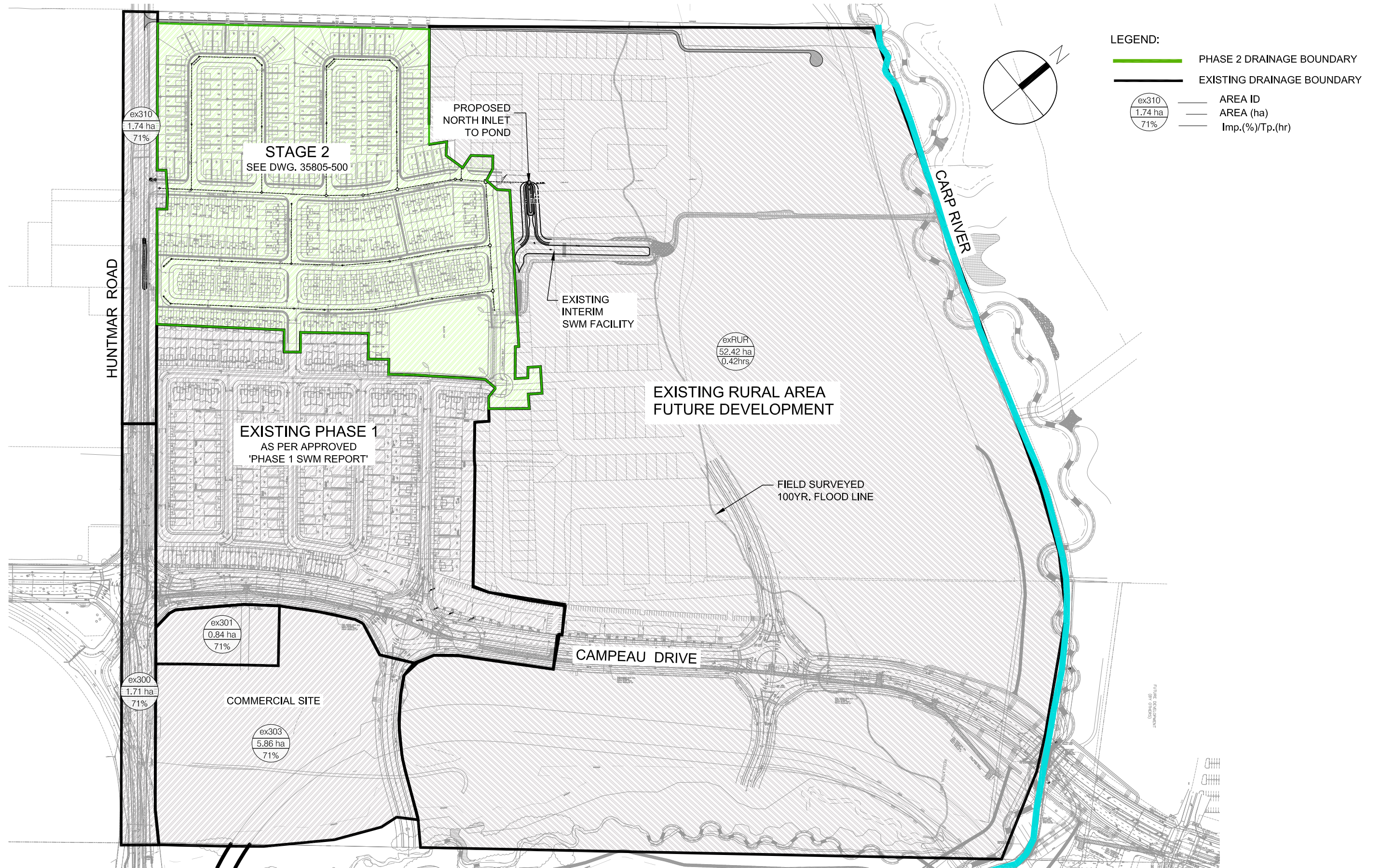
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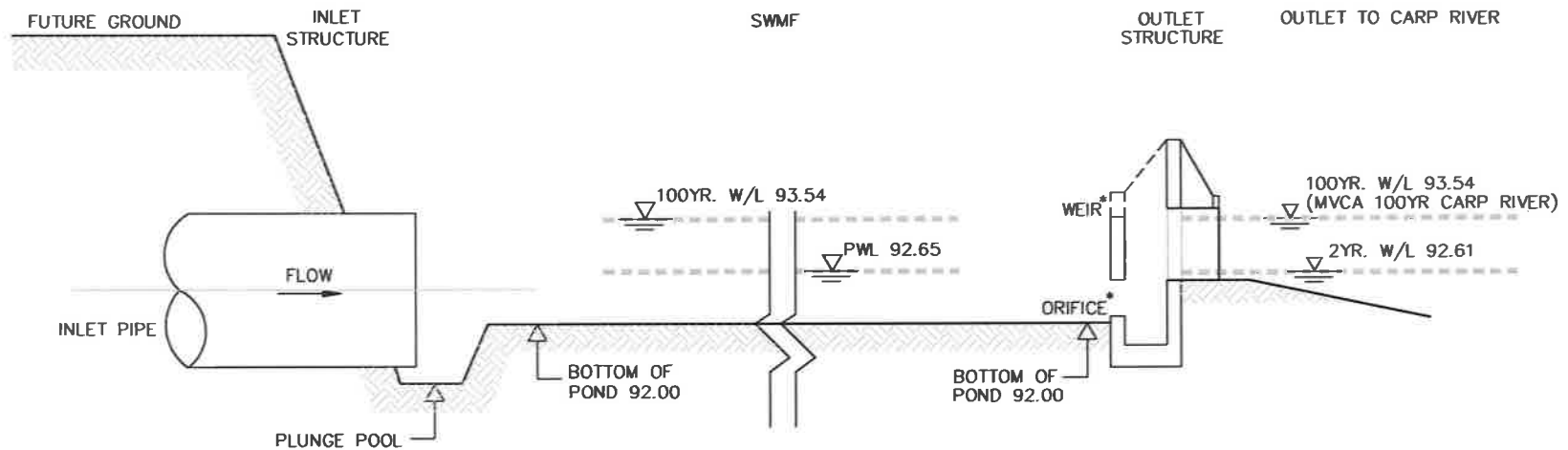
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4 of 7

2

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NOTE:

* WEIR ELEVATION AND ORIFICE SIZE TO BE ADJUSTED TO PROVIDE EXTENDED DETENTION RELEASE FOR QUALITY TREATMENT.

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Sheet No.



ULTIMATE POND CONCEPTUAL SECTION

FIGURE 6

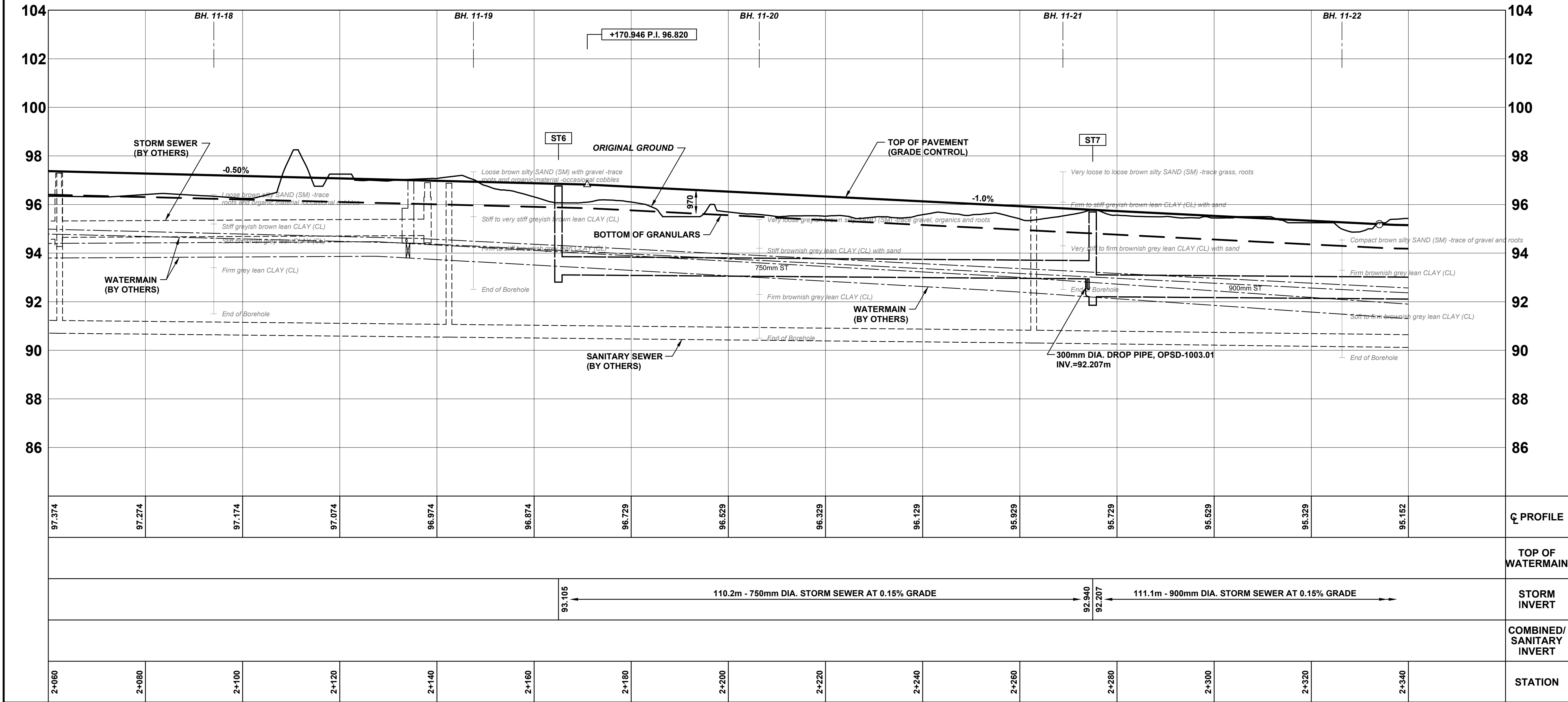
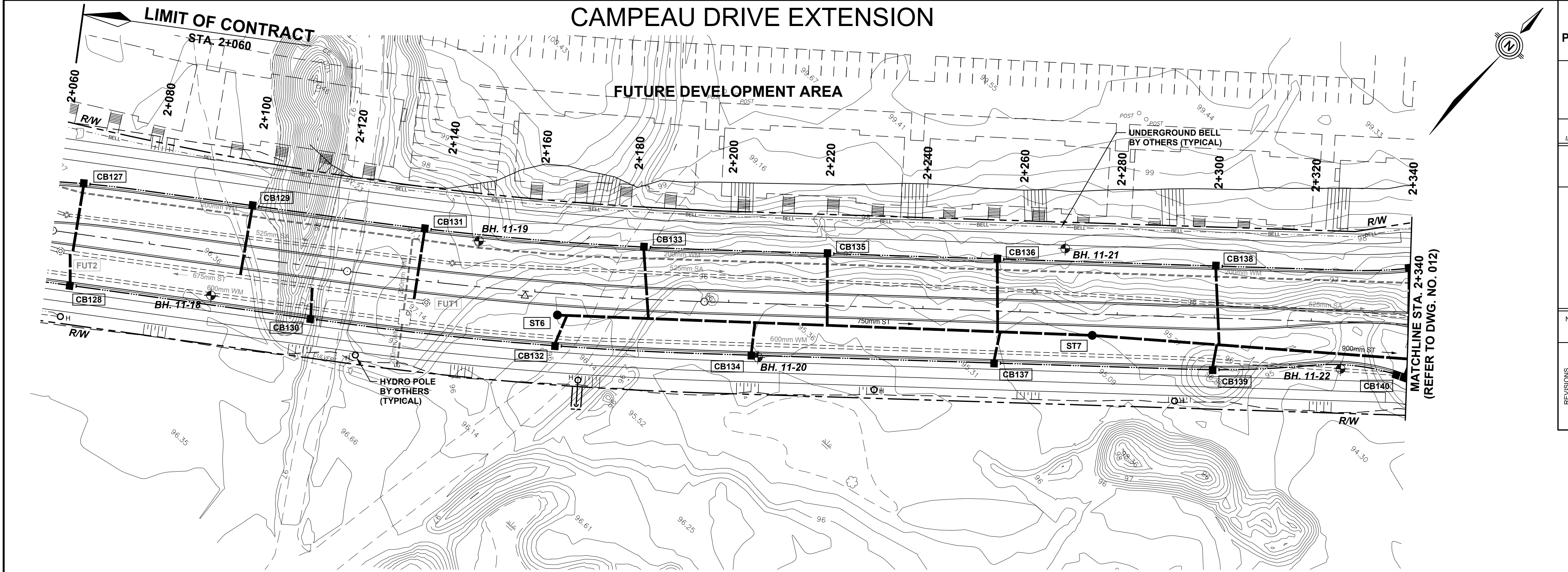
PROJECT No. 3775
DATE: 18-09-2013
SCALE: N.T.S.

ISD	001
-----	-----

Asset Group		
Des.	J.B.	CHK'd. L.G.C.
Own.	G.R.L.	CHK'd. L.G.C.
UTILITY Circ. No.	Index No.	
Const. Inspector		
Scale:		
		1:500
		1:100

to be determined by consulting
or shall prove the location
of

	By	Date (dd/mm/yyyy)



CAMPEAU DRIVE EXTENSION
PALLADIUM DRIVE TO DIDSBURY ROAD

PLAN AND PROFILE
CAMPEAU DRIVE EXTENSION
STA. 2+060 TO STA. 2+340

W. R. NEWELL, P. ENG.
General Manager
Infrastructure Services Department

M. TREMBLAY, C.E.T.
Project Manager
Infrastructure Projects

Stantec Consulting Ltd.
400 - 1031 Oakview Avenue
Ottawa, ON Canada
K2C 2S4
Tel: 613 722 4420
www.stantec.com

Contract No.
ISD12-5240

Dwg. No.
011

Sheet 11 of 40

Asset No.

Asset Group

Des.
J.B.

Chk'd.
J.B.

L.G.C.

Dwn.
G.R.L.

Utility Circ. No.

Index No.

Const. Inspector

Scale:
HORIZONTAL 1:500
VERTICAL 1:100

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

REVISIONS

No.	Description	By	Date (dd/mm/yyyy)
-	-	-	-

CATCHBASIN & MAINTENANCE HOLE DATA									
NO.	STATION	OFFSET (m)	TYPE		ICD(L/s)	ELEVATION			
			Structure	Cover		Grate	Low Inv.		
ST6	2+165.0	4.38 RT	701.011	SL	701.011	96.762	93.105		
ST7	2+275.0	4.38 RT	701.011	SL	701.011	95.692	92.207		
*CB127	2+065.0	9.72 LT	705.010	S22/S23	705.010	97.155	95.655		
*CB128	2+065.0	10.00 RT	705.010	S22/S23	705.010	97.149	95.349		
*CB129	2+100.0	10.00 LT	705.010	S22/S23	705.010	96.974	95.474		
*CB130	2+115.0	10.00 RT	705.010	S22/S23	705.010	96.899	95.099		
*CB131	2+136.0	10.00 LT	705.010	S22/S23	705.010	96.794	95.294		
*CB132	2+165.0	10.00 RT	705.010	S22/S23	705.010	96.649	95.149		
*CB133	2+182.0	10.00 LT	705.010	S22/S23	705.010	96.509	94.709		
*CB134	2+205.0	10.00 RT	705.010	S22/S23	705.010	96.279	94.479		
*CB135	2+220.0	10.00 LT	705.010	S22/S23	705.010	96.129	94.329		
*CB136	2+255.0	10.00 LT	705.010	S22/S23	705.010	95.779	93.979		
*CB137	2+255.0	10.00 RT	705.010	S22/S23	705.010	95.779	93.979		
*CB138	2+300.0	10.00 LT	705.010	S22/S23	705.010	95.329	93.529		
*CB139	2+300.0	9.94 RT	705.010	S22/S23	705.010	95.330	93.530		
*CB140	2+338.9	10.01 RT	705.020	S22/S23	705.020	94.949	94.049		
* DENOTES STRUCTURE TO BE INSULATED AS PER CITY OF OTTAWA STANDARD W23 SL DENOTES SELF-LEVELLING FRAME AND COVER									

STORM SEWER DATA						
NO. to NO. (high to low)	SIZE (mm)	LENGTH (m)	CLASS	INVERTS		
				Inlet	Outlet	
ST6	750	110.2	CONC 1000	93.105	92.940	**
ST7	900	111.1	CONC 1000	92.207	92.040	**
CB127	PIPE	200	SDR 35	95.655	95.375	*
CB128	PIPE	200	SDR 35	95.349	95.219	*
CB129	PIPE	200	SDR 35	95.474	95.184	*
CB130	PIPE	200	SDR 35	95.099	94.979	*
CB131	PIPE	200	SDR 35	95.294	95.004	*
CB132	PIPE	200	SDR 35	95.149	95.019	*
CB133	PIPE	200	SDR 35	94.709	94.564	*
CB134	PIPE	200	SDR 35	94.479	94.349	*
CB135	PIPE	200	SDR 35	94.329	94.039	*
CB136	PIPE	200	SDR 35	93.979	93.689	*
CB137	PIPE	200	SDR 35	93.979	93.859	*
CB138	PIPE	200	SDR 35	93.529	93.369	*
CB139	PIPE	200	SDR 35	93.530	93.430	*
CB140	PIPE	250	SDR 35	94.049	93.979	*
* DENOTES INVERT GIVEN AT TOP OF RISER ** DENOTES DROP PIPE AT MAINTENANCE HOLE						

2014-12-18 2:51 PM c:\01-536active\63600318_campeau_dr_extension\drawing\12-5240-011-pp1.dwg

APPENDIX 'C'

PCSWMM RESULTS FOR STORM SERVICING AND SWM ALTERNATIVES



Legend

Junctions Storages Sag Storage Manhole Pond	Conduits Storm Sewers Carp River Orifices Weirs Surface Flow Pond Outlet	Outlets Subcatchments Visible City Model Areas
--	---	--



200 m

PROJECT: **ARCADIA RESIDENTIAL STAGES 3, 4, 5 & 6 AND COMMERCIAL STAGES 2 & 3**
OTTAWA, CITY OF OTTAWA

DRAWING: **KWMSS Pond 1**



This drawing is copyright protected and may not be reproduced or use for purposes other than execution of the described work without the express written consent of J.L. Richards & Associates Limited.

DESIGN:	BP	JLR NO.:	26299
DRAWN:	BP	DRAWING NO.:	Figure C1a
CHECKED:			



● Junctions

■ Sag Storage

● Manhole

■ Pond

Conduits

— Storm Sewers

— Carp River

— Orifices

— Surface Flow

— Pond Outlet

— Outlets

Subcatchments

■ Visible

■ City Model Areas

Legend

Weirs

N

200 m

PROJECT:

ARCADIA RESIDENTIAL STAGES 3, 4, 5 & 6 AND COMMERCIAL STAGES 2 & 3
OTTAWA, CITY OF OTTAWA

DRAWING:

KWMSS Pond 1
(with 600 mm diameter sewer on Campeau Drive)

JLR

J.L.Richards

ENGINEERS · ARCHITECTS · PLANNERS

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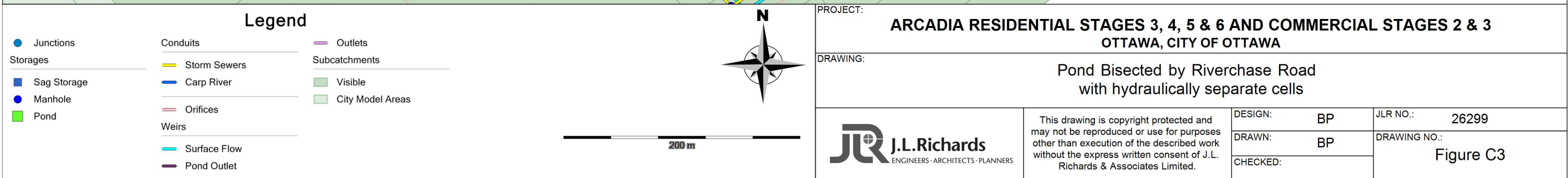
DESIGN: BP

DRAWN: BP

CHECKED:

JLR NO.: 26299

DRAWING NO.: Figure C1b





Legend

● Junctions	Conduits	— Outlets
Storages	— Storm Sewers	— Subcatchments
■ Sag Storage	— Carp River	— Visible
● Manhole	— Orifices	— City Model Areas
■ Pond	Weirs	
	— Surface Flow	
	— Pond Outlet	



200 m

PROJECT:

**ARCADIA RESIDENTIAL STAGES 3, 4, 5 & 6 AND COMMERCIAL STAGES 2 & 3
OTTAWA, CITY OF OTTAWA**

DRAWING:

**Two Ponds: Campeau Drive and Paine Avenue
(with 600 mm diameter sewer on Campeau Drive)**



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DESIGN:	BP
DRAWN:	BP
CHECKED:	

JLR NO.: 26299

DRAWING NO.:

Figure C4b

APPENDIX 'D'

EXISTING INTERIM POND DIFFICIENCIES IDENTIFIED BY CITY

Hilary MacKay

From: Guy Forget
Sent: April 25, 2017 3:46 PM
To: Hilary MacKay
Subject: FW: Campeau Rd/Arcadia Stormwater

Guy Forget, P.Eng., LEED AP
Associate
Senior Water Resources Engineer

J.L. Richards & Associates Limited
864 Lady Ellen Place, Ottawa, ON K1Z 5M2
Tel: 613-728-3571 Fax: 613-728-6012



**J.L. Richards
& Associates Limited**
ENGINEERS • ARCHITECTS • PLANNERS



From: Jolliet, Laurent [<mailto:Laurent.Jolliet@ottawa.ca>]
Sent: March 8, 2017 3:27 PM
To: Guy Forget
Cc: Lucie Dalrymple; Bobby Pettigrew; Newton, Tim; Dickinson, Mary; Moodie, Derrick; 'SMurphy@minto.com'
Subject: Campeau Rd/Arcadia Stormwater

Bonjour Guy,

Below are the comments regarding the existing interim pond.
Let me know if you have any questions.

Salutations,

Laurent Jolliet, M.A.Sc., P.Eng.
Project Manager, Infrastructure Policy
Asset Management Branch, Infrastructure Services
Planning, Infrastructure and Economic Development Department

Gestionnaire de projet, politique des infrastructures
Gestion des biens, service des infrastructures
Service de planification, d'infrastructure et de développement économique

City of Ottawa | Ville d'Ottawa
Tel. 613.580.2424 ext./poste 17149
110 Laurier Avenue West - 3rd Floor East, Ottawa, ON K1P 1J1

Comments received from Chris Melanson (Surface Water Services) on August 26, 2016:

Should the option to convert the status of this facility be entertained further, we will require a full design report and review.

From what is on site today and what this report would be required to address:

Water Quality

- Sediment forebays do not meet criteria for settlement (plunge pool, depth, length vs width ratio).
- It appears that the two sediment forebays may be treated as one, in that the check dam is located midway into the wet cell? This may be an issue if development types and rates in the two contributing areas differ greatly. We would like to minimize the overall maintenance frequency.
- Permanent pool volume and extended detention volumes of the wet cell appear inadequate.

Structural

- Presently there are no headwalls on the inlets, but the inlet design must include the means to isolate and dewater the sewer and forebay elements.
- It appears that the box pipe inlets will be submerged, the western entirely. This is to be minimized to the greatest extent possible and dewatering/sediment removal methodologies and logistics must be factored into the design.
- While there is presently no outlet structure to speak of, the outlet design must include control features that allow the greatest dewatering capacity via gravity possible and provide other operational features such as access, dewatering sumps and worker and public safety.
- Spillway is to be redesigned.

Maintenance

- In addition to the widening and deepening of the sediment forebays, the forebays require granular bottoms and the means to remove accumulated sediments - either from within or from along the sides.
- Access to the outlet structure is to be provided.
- The provision of a sediment management area is required. The required area is approximately that of the combined total surface area of the sediment forebays.

Outlet channel

- Since this facility would require a lengthy and flat channel to the Carp River, we would require this channel be located within the SWM Block and that an access road be placed adjacent to the channel to allow for the inevitable removal of vegetation.

Grading

- It appears that significant grade alterations would be required to "fit" the existing structures and water features within the lay of the land adjacent to the development. This may results in excessively steel slopes very close to the residential area. This is to be avoided.

That's all we have for now, I hope it's of assistance. Photos are available upon request.

This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

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As noted in Section 3.2 of this Report, some of the deficiencies have been taken into consideration as part of the Conceptual Paine Avenue Pond design (i.e., settling calculations, sloping, etc.); however, other concerns will be addressed as part of Detailed Design of the ultimate facility. As an example, final configuration of the inlet structures and outlet channel would be finalized during detailed design and submitted to the City for their review.

Table D1: Existing 'Interim' Pond Deficiencies identified by the City with JLR Responses

'Interim' Pond Deficiencies noted by the City per March 8, 2017 E-mail	Responses – Expanded/Retrofitted Ultimate Pond
Water Quality	
Sedimentation forebays do not meet the criteria for settlement.	Calculations associated with the expanded facility were carried out in accordance with the MOECC SMPDM and are included in Appendix 'E'.
Two sedimentation forebays treated as a single feature.	The grading of the expanded facility includes a berm between the existing forebays (Stage 1 and Stage 2). The berm will act as a deflector, isolating both forebays and minimizing turbulence. The berm will discourage short circuiting and encourage plug flow. As a result, dispersion and settling calculations were completed separately.
Location of check dam midway in the permanent pool.	The rock check dam of the expanded facility will be relocated further upstream and in accordance with the MOECC SWMPDM (i.e., based on the most critical length between Stages 1 and 2 inlets based on the 4 hour 25 mm storm event). The area upstream of rock check dam is not to exceed 30% per the MOECC.
Location of rock check dam to minimize maintenance frequency?	Proposed location will be set further upstream in accordance with the MOECC. Optimized location to minimize the frequency of sediment removal.
Permanent pool & extended detention storage appear inadequate.	Calculations were completed which demonstrate that both the minimum permanent pool & extended detention volume required as per the MOECC SWMPDM have been exceeded.
Structural	
Inlets 1 and 2 have no headwall, which makes it difficult to isolate during dewatering.	A headwall will be incorporated into the design of the expanded facility for Stages 1 & 2 pond inlets. Similarly, the expanded facility will include inlet headwalls for Stages 4 and 5.

'Interim' Pond Deficiencies noted by the City per March 8, 2017 E-mail	Responses – Expanded/Retrofitted Ultimate Pond
Box pipe inlet is submerged.	The proposed NWL will be lowered from 93.00 to 92.70, which is slightly above the recommendation of 92.65 provided in the 2006 KWMSS. The NWL will be slightly above the Carp River 1:2 year water level of 92.69. Submergence in the sewers will be reduced when compared to current conditions.
Outlet structure without control features which makes dewatering very difficult.	Outlet structures of the expanded facility will be redesigned per the recommendations of the 2006 KWMSS. Gravity dewatering is virtually impossible due to grade constraints. Dewatering during sediment removal operations will need to occur with pumps.
Spillway to be redesigned.	Spillway of the expanded facility will be designed to be more robust.
Maintenance	
Sedimentation forebays require upgrades.	Retrofit of the existing forebays will include a compacted granular base, proper length to width ratio for settlement and dispersion, etc.
Lack of sedimentation drying area	Sedimentation drying areas will be incorporated along the expanded facility. To facilitate sedimentation removal activities, one drying area will be located on either side of Riverchase.
Outlet Channel	
Outlet channel including access road to be located within the SWM Block.	A significant portion of the outlet channel will be removed. The remainder will be incorporated into the SWM Block. Access to the expanded facility can be provided via Clonrush Way, Riverchase and/or Paine Avenue.
Grading	
Grade alterations would be required to integrate the existing structures. Grading might be challenging given the proximity of the adjacent subdivision.	Structures of the expanded facility will be integrated into the design without impacting the subdivision.

APPENDIX 'E'

WATER QUALITY CONTROL CALCULATIONS

Calculation Sheet for Permanent Pool Sizing - Paine Avenue Pond - West Cell

Removal Efficiency: Normal 70% long-term S.S. removal

SWMP Type: Wet Pond

Total Drainage Area: 44.6 ha

Average Imperviousness: 64%

Storage Volume (m³/ha) 123 (See Table 3.2 from MOECC SWMPDG, copied below)Extended Detention Volume (m³/ha) 40**Permanent Pool Volume Required: 3690 m³****Extended Detention Volume Required: 1790 m³**

Protection Level	SWMP Type	Storage Volume (m ³ /ha) for Impervious Level			
		35%	55%	70%	85%
<i>Enhanced</i> 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
<i>Normal</i> 70% long-term S.S. removal	Infiltration	20	20	25	30
	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
<i>Basic</i> 60% long-term S.S. removal	Infiltration	20	20	20	20
	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for Permanent Pool Sizing - Paine Avenue Pond - East Cell

Removal Efficiency: Normal 70% long-term S.S. removal

SWMP Type: Wet Pond

Total Drainage Area: 9.7 ha

Average Imperviousness: 49%

Storage Volume (m³/ha) 104 (See Table 3.2 from MOECC SWMPDG, copied below)Extended Detention Volume (m³/ha) 40**Permanent Pool Volume Required: 630 m³****Extended Detention Volume Required: 390 m³**

Protection Level	SWMP Type	Storage Volume (m ³ /ha) for Impervious Level			
		35%	55%	70%	85%
<i>Enhanced</i> 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
<i>Normal</i> 70% long-term S.S. removal	Infiltration	20	20	25	30
	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
<i>Basic</i> 60% long-term S.S. removal	Infiltration	20	20	20	20
	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for Permanent Pool Sizing - Campeau Drive Pond

Removal Efficiency: Normal 70% long-term S.S. removal

SWMP Type: Wet Pond

Total Drainage Area: 22.9 ha

Average Imperviousness: 73%

Storage Volume (m³/ha) 134 (See Table 3.2 from MOECC SWMPDG, copied below)Extended Detention Volume (m³/ha) 40**Permanent Pool Volume Required: 2150 m³****Extended Detention Volume Required: 920 m³**

Protection Level	SWMP Type	Storage Volume (m ³ /ha) for Impervious Level			
		35%	55%	70%	85%
<i>Enhanced</i> 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
<i>Normal</i> 70% long-term S.S. removal	Infiltration	20	20	25	30
	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
<i>Basic</i> 60% long-term S.S. removal	Infiltration	20	20	20	20
	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for SWM Facility Forebay Sizing - STAGE 1

Settling Criteria

$$L_{\min} = \left(\frac{rQ_p}{V_s} \right)^{0.5}$$

Where: r = length to width ratio at the invert of the inlet pipe
 Q_p = peak outflow during design quality storm
 V_s = settling velocity

Input: l = length = 70 m
 w = width = 5 m
 r = 14.00
 Q_p = 0.02 m³/s
 V_s = 0.0003 m/s

$$L_{\min} = 27 \text{ m}$$

The peak flow rate from the pond during the quality storm is taken as the flow that would occur just below the quantity controls.

Dispersion Criteria

$$L_{\min} = \frac{8Q}{dV_f}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of permanent pool (forebay)
 V_f = desired final velocity

Input: Q = 4.05 m³/s
 d = 1.5 m
 V_f = 0.5 m/s

$$L_{\min} = 43 \text{ m}$$

The minimum forebay length is determined by the larger of the settling or dispersion criteria.

Minimum length of forebay required = 43.20 m

Length of Forebay Provided = 70.00 m

Average Forebay Velocity

$$V_{\text{avg}} = \frac{Q}{d W_{\text{avg}}}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of pond during peak 1:10 year inflow
 W_{avg} = average width of forebay

Input: Q = 4.05 m³/s
 d = 2.0 m
 W_{avg} = 14 m

$$V_{\text{avg}} = 0.15 \text{ m/s} < 0.15 \text{ m/s}$$

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for SWM Facility Forebay Sizing - STAGE 2

Settling Criteria

$$L_{\min} = \left(\frac{rQ_p}{V_s} \right)^{0.5}$$

Where: r = length to width ratio at the invert of the inlet pipe
 Q_p = peak outflow during design quality storm
 V_s = settling velocity

Input: l = length = 50 m
 w = width = 9 m
 r = 5.56
 Q_p = 0.05 m³/s
 V_s = 0.0003 m/s

$$L_{\min} = 31 \text{ m}$$

The peak flow rate from the pond during the quality storm is taken as the flow that would occur just below the quantity controls.

Dispersion Criteria

$$L_{\min} = \frac{8Q}{dV_f}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of permanent pool (forebay)
 V_f = desired final velocity

Input: Q = 2.44 m³/s
 d = 1.5 m
 V_f = 0.5 m/s

$$L_{\min} = 26 \text{ m}$$

The minimum forebay length is determined by the larger of the settling or dispersion criteria.

Minimum length of forebay required = 30.90 m

Length of Forebay Provided = 50.00 m

Average Forebay Velocity

$$V_{\text{avg}} = \frac{Q}{d W_{\text{avg}}}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of pond during peak 1:10 year inflow
 W_{avg} = average width of forebay

Input: Q = 2.44 m³/s
 d = 2.5 m
 W_{avg} = 15 m

$$V_{\text{avg}} = 0.07 \text{ m/s} < 0.15 \text{ m/s}$$

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for SWM Facility Forebay Sizing - STAGE 4 and 5

Settling Criteria

$$L_{\min} = \left(\frac{rQ_p}{V_s} \right)^{0.5}$$

Where: r = length to width ratio at the invert of the inlet pipe
 Q_p = peak outflow during design quality storm
 V_s = settling velocity

Input: l = length = 45 m
 w = width = 13 m
 r = 3.46
 Q_p = 0.05 m³/s
 V_s = 0.0003 m/s

$$L_{\min} = 24 \text{ m}$$

The peak flow rate from the pond during the quality storm is taken as the flow that would occur just below the quantity controls.

Dispersion Criteria

$$L_{\min} = \frac{8Q}{dV_f}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of permanent pool (forebay)
 V_f = desired final velocity

Input: Q = 1.134 m³/s
 d = 1.5 m
 V_f = 0.5 m/s

$$L_{\min} = 12.10 \text{ m}$$

The minimum forebay length is determined by the larger of the settling or dispersion criteria.

Minimum length of forebay required = 24.40 m

Length of Forebay Provided = 45.00 m

Average Forebay Velocity

$$V_{\text{avg}} = \frac{Q}{d W_{\text{avg}}}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of pond during peak 1:10 year inflow
 W_{avg} = average width of forebay

Input: Q = 1.134 m³/s
 d = 2.5 m
 W_{avg} = 15 m

$$V_{\text{avg}} = 0.03 \text{ m/s} < 0.15 \text{ m/s}$$

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for SWM Facility Forebay Sizing - STAGE 6

Settling Criteria

$$L_{\min} = \left(\frac{rQ_p}{V_s} \right)^{0.5}$$

Where: r = length to width ratio at the invert of the inlet pipe
 Q_p = peak outflow during design quality storm
 V_s = settling velocity

Input: l = length = 90 m
 w = width = 8 m
 r = 11.25
 Q_p = 0.05 m³/s
 V_s = 0.0003 m/s

$$L_{\min} = 43.4 \text{ m}$$

The peak flow rate from the pond during the quality storm is taken as the flow that would occur just below the quantity controls.

Dispersion Criteria

$$L_{\min} = \frac{8Q}{dV_f}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of permanent pool (forebay)
 V_f = desired final velocity

Input: Q = 1.085 m³/s
 d = 1.5 m
 V_f = 0.5 m/s

$$L_{\min} = 11.60 \text{ m}$$

The minimum forebay length is determined by the larger of the settling or dispersion criteria.

Minimum length of forebay required = 43.40 m

Length of Forebay Provided = 90.00 m

Average Forebay Velocity

$$V_{\text{avg}} = \frac{Q}{d W_{\text{avg}}}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of pond during peak 1:10 year inflow
 W_{avg} = average width of forebay

Input: Q = 1.085 m³/s
 d = 2.6 m
 W_{avg} = 15 m

$$V_{\text{avg}} = 0.03 \text{ m/s} < 0.15 \text{ m/s}$$

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

Calculation Sheet for SWM Facility Forebay Sizing - CAMPEAU POND

Settling Criteria

$$L_{\min} = \left(\frac{rQ_p}{V_s} \right)^{0.5}$$

Where: r = length to width ratio at the invert of the inlet pipe
 Q_p = peak outflow during design quality storm
 V_s = settling velocity

Input: l = length = 90 m
 w = width = 6 m
 r = 15.00
 Q_p = 0.102 m³/s
 V_s = 0.0003 m/s

$$L_{\min} = 71.50 \text{ m}$$

The peak flow rate from the pond during the quality storm is taken as the flow that would occur just below the quantity controls.

Dispersion Criteria

$$L_{\min} = \frac{8Q}{dV_f}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of permanent pool (forebay)
 V_f = desired final velocity

Input: Q = 3.696 m³/s
 d = 1.0 m
 V_f = 0.5 m/s

$$L_{\min} = 59.20 \text{ m}$$

The minimum forebay length is determined by the larger of the settling or dispersion criteria.

Minimum length of forebay required = 71.50 m

Length of Forebay Provided = 90 m

Average Forebay Velocity

$$V_{\text{avg}} = \frac{Q}{d W_{\text{avg}}}$$

Where: Q = Inlet flowrate (1:10 year, 12-hour SCS Storm)
 d = depth of pond during peak 1:10 year inflow
 W_{avg} = average width of forebay

Input: Q = 3.696 m³/s
 d = 1.5 m
 W_{avg} = 20 m

$$V_{\text{avg}} = 0.13 \text{ m/s} < 0.15 \text{ m/s}$$

Calculation by: BP

Reviewed by: _____

Date: Apr 26, 2017

APPENDIX 'F'

PCSWMM MODEL OUTPUTS FOR TWO POND STRATGEY

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

E - North Pond - 2 Connected Cells
Campeau Pond
Campeau Drive Sewer 750mm

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units CMS
Process Models:
Rainfall/Runoff YES
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Starting Date JUL-23-2009 00:00:00
Ending Date JUL-25-2009 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:05:00
Wet Time Step 00:05:00
Dry Time Step 01:00:00
Routing Time Step 2.00 sec

WARNING 04: minimum elevation drop used for Conduit C6
WARNING 04: minimum elevation drop used for Conduit CC076
WARNING 04: minimum elevation drop used for Conduit CC079
WARNING 04: minimum elevation drop used for Conduit CC083
WARNING 04: minimum elevation drop used for Conduit CC089
WARNING 04: minimum elevation drop used for Conduit CC104
WARNING 04: minimum elevation drop used for Conduit CC106
WARNING 02: maximum depth increased for Node 111-S
WARNING 02: maximum depth increased for Node CJ066
WARNING 02: maximum depth increased for Node CJ070
WARNING 02: maximum depth increased for Node CJ071
WARNING 02: maximum depth increased for Node CJ073
WARNING 02: maximum depth increased for Node CJ074
WARNING 02: maximum depth increased for Node CJ075
WARNING 02: maximum depth increased for Node CJ077
WARNING 02: maximum depth increased for Node CJ084
WARNING 02: maximum depth increased for Node CJ089
WARNING 02: maximum depth increased for Node CJ094
WARNING 02: maximum depth increased for Node CJ100
WARNING 02: maximum depth increased for Node CJ101
WARNING 02: maximum depth increased for Node CJ104
WARNING 02: maximum depth increased for Node CJ105
WARNING 02: maximum depth increased for Node CJ106
WARNING 02: maximum depth increased for Node J10

Element Count

Number of rain gages 16
Number of subcatchments ... 62
Number of nodes 125
Number of links 168
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
CHI_3hr_10	CHI_3hr_10	INTENSITY	5 min.
CHI_3hr_100	CHI_3hr_100	INTENSITY	5 min.
CHI_3hr_100CC	CHI_3hr_100CC	INTENSITY	5 min.
CHI_3hr_2	CHI_3hr_2	INTENSITY	5 min.
CHI_3hr_25	CHI_3hr_25	INTENSITY	5 min.
CHI_3hr_5	CHI_3hr_5	INTENSITY	5 min.
CHI_3hr_50	CHI_3hr_50	INTENSITY	5 min.
MT0_100CCyrSCS12hr	MT0_100CCyrSCS12hr	INTENSITY	15 min.
MT0_100yrSCS12hr	MT0_100yrSCS12hr	INTENSITY	15 min.
MT0_10mmSCS12hr	MT0_10mmSCS12hr	INTENSITY	15 min.
MT0_10yrSCS12hr	MT0_10yrSCS12hr	INTENSITY	15 min.
MT0_20mmSCS12hr	MT0_20mmSCS12hr	INTENSITY	15 min.
MT0_25yrSCS12hr	MT0_25yrSCS12hr	INTENSITY	15 min.
MT0_2yrSCS12hr	MT0_2yrSCS12hr	INTENSITY	15 min.
MT0_50yrSCS12hr	MT0_50yrSCS12hr	INTENSITY	15 min.
MT0_5yrSCS12hr	MT0_5yrSCS12hr	INTENSITY	15 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
112-115	0.27	111.23	64.30	0.5000	MT0_100yrSCS12hr	112-S
114-112	0.51	370.42	64.30	0.5000	MT0_100yrSCS12hr	114-S
114-118	0.47	238.64	64.30	0.5000	MT0_100yrSCS12hr	118-S
115-110	0.18	65.88	0.00	0.5000	MT0_100yrSCS12hr	115-S
116-115	0.47	238.64	64.30	0.5000	MT0_100yrSCS12hr	116-S
120-111	0.55	444.55	71.40	0.5000	MT0_100yrSCS12hr	120-S
122-120	0.27	174.79	71.40	0.5000	MT0_100yrSCS12hr	122-S
124-120	0.32	74.93	64.30	0.5000	MT0_100yrSCS12hr	124-S
124-126	0.31	91.35	64.30	0.5000	MT0_100yrSCS12hr	124-S
126-128	0.67	197.08	64.30	0.5000	MT0_100yrSCS12hr	126-S
131-129	0.55	119.36	64.30	0.5000	MT0_100yrSCS12hr	131-S
132-130	0.55	89.20	64.30	0.5000	MT0_100yrSCS12hr	132-S
134-132	0.77	230.33	64.30	0.5000	MT0_100yrSCS12hr	134-S
136-134	0.51	150.29	64.30	0.5000	MT0_100yrSCS12hr	136-S
138-140	0.34	107.51	64.30	0.5000	MT0_100yrSCS12hr	138-S
140-122	0.74	234.19	64.30	0.5000	MT0_100yrSCS12hr	140-S
140-142	0.64	240.84	64.30	0.5000	MT0_100yrSCS12hr	140-S
142-130	0.86	264.29	64.30	0.5000	MT0_100yrSCS12hr	142-S
144-140	0.40	119.36	64.30	0.5000	MT0_100yrSCS12hr	144-S
144-146	0.89	281.65	64.30	0.5000	MT0_100yrSCS12hr	144-S
146-122	0.25	129.84	71.40	0.5000	MT0_100yrSCS12hr	146-S
154-512	0.87	75.77	64.29	1.5000	MT0_100yrSCS12hr	St154

Minto Communities Inc
Appendix F

Arcadia Residential Stages 3, 4, 5, & 6 and Commercial Stages 2 &3
Stormwater Management Serving Report

157-158	0.74	64.47	64.29	1.5000	MT0_100yrSCS12hr	St157
158-160	0.56	38.30	64.29	1.5000	MT0_100yrSCS12hr	St158
170-156	0.36	49.05	64.29	1.5000	MT0_100yrSCS12hr	St170
172-160	0.75	61.39	64.29	1.5000	MT0_100yrSCS12hr	St172
173-174	0.36	45.36	64.29	1.5000	MT0_100yrSCS12hr	St173
512A-OUTLET	0.33	23.85	25.00	1.5000	MT0_100yrSCS12hr	St512A
513-512A	2.99	145.75	43.22	1.5000	MT0_100yrSCS12hr	St513
513-512A_PARK	0.99	70.81	25.00	1.5000	MT0_100yrSCS12hr	513
514-513	0.33	28.88	64.29	1.5000	MT0_100yrSCS12hr	St514
605-OUTLET	0.82	78.80	64.29	1.5000	MT0_100yrSCS12hr	St605
620-610_EAST	1.88	114.68	14.30	1.5000	MT0_100yrSCS12hr	620
620-610_WEST	2.06	75.95	64.29	1.5000	MT0_100yrSCS12hr	St620
630-620	0.66	62.31	64.29	1.5000	MT0_100yrSCS12hr	St630
640-630	2.57	89.39	64.29	1.5000	MT0_100yrSCS12hr	St640
CAMPEAU_DRIVE_1	1.95	60.05	92.90	0.5040	MT0_100yrSCS12hr	EXISTING_STAGE_1
COMMERCIAL_STAGE_1	5.61	168.72	70.53	0.5010	MT0_100yrSCS12hr	CAMPEAU_DRIVE_1
COMMERCIAL_STAGE_3_SOUTH_2	1.55	53.03	92.90	0.5780	MT0_100yrSCS12hr	COMMERCIAL_STAGE_1
COMMERCIAL_STAGE_3_WEST_2	0.73	56.74	92.90	0.5000	MT0_100yrSCS12hr	COMMERCIAL_STAGE_1
CS063_1	92.82	630.00	0.00	0.8000	MT0_100yrSCS12hr	CJ065
CS063_2	3.75	260.01	8.00	3.0000	MT0_100yrSCS12hr	CJ065
CS073	5.37	150.00	0.00	0.8000	MT0_100yrSCS12hr	CJ083
CS077_2	12.29	360.01	76.40	1.0000	MT0_100yrSCS12hr	CJ077
CS082_1	36.92	500.01	58.00	0.7000	MT0_100yrSCS12hr	CJ082
CS082_2	8.84	170.01	70.00	0.7000	MT0_100yrSCS12hr	CJ082
CS098	90.47	650.01	36.40	1.2000	MT0_100yrSCS12hr	CJ098
CS267_4	1.53	16.24	71.00	0.5000	MT0_100yrSCS12hr	COMMERCIAL_STAGE_1
CS267_5	1.05	15.07	71.00	0.5000	MT0_100yrSCS12hr	EXISTING_STAGE_2
CS267_7	1.71	18.97	69.00	0.5000	MT0_100yrSCS12hr	CJ065
EXISTING_STAGE_1	9.08	144.88	68.95	0.5000	MT0_100yrSCS12hr	EXISTING_STAGE_2
EXISTING_STAGE_2	12.63	289.91	64.36	0.5000	MT0_100yrSCS12hr	OF1
FUTURE_TRANSITWAY	2.22	88.80	71.43	0.5340	MT0_100yrSCS12hr	FUTURE_TRANSITWAY-S
FUTURE_TRANSITWAY_2	0.58	46.04	92.90	0.5340	MT0_100yrSCS12hr	COMMERCIAL_STAGE_1
FutureCamp-117	1.43	726.07	71.43	0.5000	MT0_100yrSCS12hr	FutureCamp-S
FutureCampEast-117	1.00	50.00	71.43	0.5000	MT0_100yrSCS12hr	117-S
FutureComm-CAMPEAU_DRIVE_POND	4.19	83.46	85.71	0.5000	MT0_100yrSCS12hr	FutureComm-S
Pond_1	1.72	287.78	35.00	0.5000	MT0_100yrSCS12hr	EX_POND
Pond_2	1.75	335.40	35.00	0.5000	MT0_100yrSCS12hr	NEW_POND
STAGE_2	0.48	500.00	25.00	0.5000	MT0_100yrSCS12hr	EXISTING_STAGE_2
STAGE_2_ROAD	0.12	500.06	25.00	0.5000	MT0_100yrSCS12hr	EXISTING_STAGE_2
Stub-110	3.56	99.05	85.71	0.5000	MT0_100yrSCS12hr	Stub-S

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
111-S	JUNCTION	96.15	0.50	0.0	
CJ065	JUNCTION	91.00	5.51	100.0	
CJ066	JUNCTION	91.02	5.60	100.0	
CJ067	JUNCTION	91.08	5.62	100.0	
CJ068	JUNCTION	91.06	6.19	100.0	
CJ069	JUNCTION	91.06	6.24	100.0	
CJ070	JUNCTION	91.10	6.21	100.0	
CJ071	JUNCTION	91.15	6.25	100.0	
CJ072	JUNCTION	91.14	6.25	100.0	
CJ073	JUNCTION	91.18	8.21	100.0	
CJ074	JUNCTION	91.20	7.69	100.0	
CJ075	JUNCTION	91.21	7.69	100.0	
CJ076	JUNCTION	91.21	6.70	100.0	
CJ077	JUNCTION	91.23	7.40	100.0	
CJ078	JUNCTION	91.22	6.38	100.0	
CJ079	JUNCTION	91.22	5.61	100.0	
CJ080	JUNCTION	91.22	5.57	100.0	
CJ081	JUNCTION	91.24	5.48	100.0	
CJ082	JUNCTION	92.27	10.00	1000.0	
CJ083	JUNCTION	91.24	5.38	100.0	
CJ084	JUNCTION	91.25	5.37	100.0	
CJ089	JUNCTION	91.25	7.01	100.0	
CJ094	JUNCTION	92.74	6.80	100.0	
CJ095	JUNCTION	92.86	3.00	100.0	
CJ096	JUNCTION	93.15	3.00	100.0	
CJ098	JUNCTION	93.40	3.00	100.0	
CJ100	JUNCTION	91.30	4.70	100.0	
CJ101	JUNCTION	91.32	4.68	100.0	
CJ103	JUNCTION	91.36	3.85	100.0	Yes
CJ104	JUNCTION	91.32	4.68	100.0	
CJ105	JUNCTION	91.35	4.65	100.0	
CJ106	JUNCTION	91.35	4.65	100.0	
CJ266	JUNCTION	92.30	3.70	100.0	
J10	JUNCTION	92.80	1.26	0.0	
J11	JUNCTION	92.90	2.10	0.0	
CJ064	OUTFALL	90.95	5.51	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
110	STORAGE	92.67	3.44	0.0	
111	STORAGE	93.31	2.84	0.0	
112	STORAGE	93.13	3.37	0.0	
112-S	STORAGE	96.50	0.50	0.0	
113	STORAGE	92.32	3.64	0.0	
114	STORAGE	93.75	2.90	0.0	
114-S	STORAGE	96.65	0.50	0.0	
115	STORAGE	93.04	3.41	0.0	
115-S	STORAGE	96.45	0.50	0.0	
116	STORAGE	93.66	2.94	0.0	
116-S	STORAGE	96.60	0.50	0.0	
117	STORAGE	92.33	3.67	0.0	
117-S	STORAGE	95.50	1.00	0.0	
118	STORAGE	93.83	2.77	0.0	
118-S	STORAGE	96.60	0.50	0.0	
120	STORAGE	93.50	2.80	0.0	
120-S	STORAGE	96.30	0.50	0.0	
122	STORAGE	93.61	2.79	0.0	
122-S	STORAGE	96.40	0.50	0.0	
124	STORAGE	94.13	2.67	0.0	
124-S	STORAGE	96.80	0.50	0.0	
126	STORAGE	93.82	2.88	0.0	
126-S	STORAGE	96.70	0.50	0.0	
128	STORAGE	93.22	3.33	0.0	
128-S	STORAGE	96.55	0.50	0.0	
129	STORAGE	93.26	3.34	0.0	
130	STORAGE	93.48	3.22	0.0	
130-S	STORAGE	96.70	0.50	0.0	
131	STORAGE	93.37	3.28	0.0	
131-S	STORAGE	96.65	0.50	0.0	
132	STORAGE	93.90	2.90	0.0	
132-S	STORAGE	96.80	0.50	0.0	
134	STORAGE	94.09	2.81	0.0	
134-S	STORAGE	96.90	0.50	0.0	
136	STORAGE	94.30	2.67	0.0	
136-S	STORAGE	96.97	0.50	0.0	
138	STORAGE	94.31	2.66	0.0	
138-S	STORAGE	96.97	0.50	0.0	
140	STORAGE	94.03	3.07	0.0	
140-S	STORAGE	97.10	0.50	0.0	
142	STORAGE	93.82	3.08	0.0	
142-S	STORAGE	96.90	0.50	0.0	
144	STORAGE	94.26	2.94	0.0	
144-S	STORAGE	97.20	0.50	0.0	
146	STORAGE	93.95	2.55	0.0	
146-S	STORAGE	96.50	0.50	0.0	
154	STORAGE	93.28	3.02	0.0	
156	STORAGE	93.84	3.16	0.0	
157	STORAGE	93.58	3.12	0.0	
158	STORAGE	93.32	3.38	0.0	
160	STORAGE	93.14	3.26	0.0	
170	STORAGE	93.97	3.03	0.0	
172	STORAGE	93.61	2.79	0.0	
173	STORAGE	93.77	2.63	0.0	
174	STORAGE	93.62	2.78	0.0	
512	STORAGE	92.87	3.43	0.0	
512A	STORAGE	92.88	3.62	0.0	
513	STORAGE	93.21	3.09	0.0	
514	STORAGE	93.94	2.36	0.0	
605	STORAGE	92.87	3.58	0.0	
610	STORAGE	92.92	3.33	0.0	

620	STORAGE	93.15	3.20	0.0			
630	STORAGE	93.46	2.99	0.0			
640	STORAGE	93.65	2.80	0.0			
CAMPEAU_DRIVE_POND	STORAGE	92.20	2.80	0.0			
CSto097	STORAGE	93.20	3.30	100.0			
EX_POND	STORAGE	92.70	1.30	0.0	Yes		
FUTURE_TRANSITWAY-S	STORAGE	95.50	0.50	0.0			
FutureCamp	STORAGE	92.50	2.30	0.0			
FutureCamp-S	STORAGE	94.30	0.50	0.0			
FutureComm	STORAGE	92.79	3.21	0.0			
FutureComm-S	STORAGE	95.50	0.50	0.0			
NEW_POND	STORAGE	92.70	1.30	0.0			
St154	STORAGE	96.30	0.50	0.0			
St157	STORAGE	96.70	0.50	0.0			
St158	STORAGE	96.70	0.50	0.0			
St170	STORAGE	97.00	0.50	0.0			
St172	STORAGE	96.40	0.50	0.0			
St173	STORAGE	96.40	0.50	0.0			
St512A	STORAGE	96.50	0.50	0.0			
St513	STORAGE	96.30	0.50	0.0			
St514	STORAGE	96.30	0.50	0.0			
St605	STORAGE	96.45	0.50	0.0			
St620	STORAGE	96.35	0.50	0.0			
St630	STORAGE	96.45	0.50	0.0			
St640	STORAGE	96.45	0.50	0.0			
Stub	STORAGE	92.71	2.09	0.0			
Stub-S	STORAGE	96.15	0.50	0.0			

Link Summary							

Name	From Node	To Node	Type	Length	%Slope	Roughness	
110-113	110	113	CONDUIT	38.1	0.1496	0.0130	
111-110	111	110	CONDUIT	75.4	0.1499	0.0130	
112-115	112	115	CONDUIT	46.1	0.1996	0.0130	
113-117	113	117	CONDUIT	16.8	0.1488	0.0130	
114-112	114	112	CONDUIT	110.7	0.1996	0.0130	
114-118	114	118	CONDUIT	39.9	0.2005	0.0130	
115-110	115	110	CONDUIT	46.1	0.1497	0.0130	
116-115	116	115	CONDUIT	107.8	0.1503	0.0130	
117-CAMPEAU_DRIVE_POND117	117	CAMPEAU_DRIVE_POND	CONDUIT	66.5	0.2000	0.0130	0.0130
118-116	118	116	CONDUIT	8.6	0.1977	0.0130	
120-111	120	111	CONDUIT	122.6	0.1501	0.0130	
122-120	122	120	CONDUIT	78.7	0.1499	0.0130	
124-120	124	120	CONDUIT	76.2	0.2493	0.0130	
124-126	124	126	CONDUIT	68.9	0.1988	0.0130	
126-114	126	114	CONDUIT	46.1	0.1500	0.0130	
126-128	126	128	CONDUIT	107.5	0.1498	0.0130	
128-112	128	112	CONDUIT	46.1	0.2017	0.0130	
129-128	129	128	CONDUIT	23.4	0.1496	0.0130	
130-131	130	131	CONDUIT	23.2	0.1509	0.0130	
131-129	131	129	CONDUIT	74.3	0.1494	0.0130	
132-130	132	130	CONDUIT	75.5	0.1497	0.0130	
134-132	134	132	CONDUIT	124.8	0.1498	0.0130	
138-134	138	134	CONDUIT	92.2	0.1497	0.0130	
138-140	138	140	CONDUIT	64.0	0.2000	0.0130	
140-122	140	122	CONDUIT	124.0	0.1500	0.0130	
140-142	140	142	CONDUIT	115.7	0.1495	0.0130	
142-130	142	130	CONDUIT	142.0	0.1494	0.0130	
144-140	144	140	CONDUIT	74.0	0.2000	0.0130	
144-146	144	146	CONDUIT	117.0	0.2000	0.0130	
146-122	146	122	CONDUIT	74.3	0.1507	0.0130	
154-512	154	512	CONDUIT	0.1500	0.1500	0.0130	
156-157	156	157	CONDUIT	13.0	0.2308	0.0130	
157-158	157	158	CONDUIT	120.0	0.1500	0.0130	
158-160	158	160	CONDUIT	70.0	0.1429	0.0130	
160-512	160	512	CONDUIT	0.1463	0.1463	0.0130	
170-156	170	156	CONDUIT	50.0	0.2600	0.0130	
172-160	172	160	CONDUIT	120.0	0.2000	0.0130	
173-174	173	174	CONDUIT	60.0	0.2500	0.0130	
174-154	174	154	CONDUIT	0.2667	0.2667	0.0130	
512A-EX_POND	512A	EX_POND	CONDUIT	55.0	0.3273	0.0130	
512-EX_POND	512	EX_POND	CONDUIT	45.0	0.3778	0.0130	
513-512A	513	512A	CONDUIT	215.0	0.1535	0.0130	
514-513	514	513	CONDUIT	110.0	0.2547	0.0130	
605-NEW_POND	605	NEW_POND	CONDUIT	46.0	0.1522	0.0130	
610-605	610	605	CONDUIT	35.0	0.1429	0.0130	
620-610	620	610	CONDUIT	150.0	0.1533	0.0130	
630-620	630	620	CONDUIT	110.0	0.1455	0.0130	
640-630	640	630	CONDUIT	70.0	0.1571	0.0130	
C12	J10	CJ266	CONDUIT	260.0	0.1923	0.0350	
C2	J11	CJ104	CONDUIT	400.0	0.3950	0.0350	
C6	EX_POND	NEW_POND	CONDUIT	40.0	0.0008	0.0130	
CC065	CJ065	CJ064	CONDUIT	14.9	0.3358	0.0350	
CC066	CJ066	CJ065	CONDUIT	9.8	0.1636	0.0350	
CC067	CJ067	CJ066	CONDUIT	39.1	0.1636	0.0350	
CC068	CJ068	CJ067	CONDUIT	151.6	0.0152	0.0350	
CC069	CJ069	CJ068	CONDUIT	9.8	0.0307	0.0350	
CC070	CJ070	CJ069	CONDUIT	9.9	0.4020	0.0350	
CC071	CJ071	CJ070	CONDUIT	39.8	0.1257	0.0350	
CC072	CJ072	CJ071	CONDUIT	9.9	0.1005	0.0350	
CC073	CJ073	CJ072	CONDUIT	114.4	0.0350	0.0350	
CC074	CJ074	CJ073	CONDUIT	54.7	0.0366	0.0350	
CC075	CJ075	CJ074	CONDUIT	9.8	0.1015	0.0350	
CC076	CJ076	CJ075	CONDUIT	64.0	0.0005	0.0350	
CC077	CJ077	CJ076	CONDUIT	363.1	0.1008	0.0400	
CC078	CJ078	CJ077	CONDUIT	24.6	0.0406	0.0350	
CC079	CJ079	CJ078	CONDUIT	137.9	0.0002	0.0350	
CC080	CJ080	CJ079	CONDUIT	408.0	0.2559	0.0130	
CC080_1	CJ080	CJ079	CONDUIT	4.9	0.0813	0.0350	
CC081	CJ081	CJ080	CONDUIT	19.7	0.0812	0.0350	
CC083	CJ083	CJ081	CONDUIT	14.8	0.0021	0.0350	
CC084	CJ084	CJ083	CONDUIT	9.8	0.1015	0.0350	
CC089	CJ089	CJ084	CONDUIT	19.8	0.0015	0.0350	
CC094	CJ094	CJ073	CONDUIT	130.5	0.1150	0.0400	
CC095	CJ095	CJ094	CONDUIT	50.0	0.2200	0.0130	
CC096	CJ096	CJ095	CONDUIT	49.0	0.5924	0.0130	
CC098	CJ098	CSto097	CONDUIT	20.0	0.5000	0.0130	
CC100	CJ100	CJ089	CONDUIT	217.6	0.0230	0.0350	
CC101	CJ101	CJ100	CONDUIT	29.4	0.0680	0.0100	
CC102	CJ102	CJ101	CONDUIT	14.5	0.0690	0.0200	
CC104	CJ104	CJ101	CONDUIT	6.4	0.0048	0.0350	
CC105	CJ105	CJ104	CONDUIT	106.1	0.0283	0.0350	
CC106	CJ106	CJ105	CONDUIT	54.0	0.0006	0.0350	
CC266	CJ266	CJ072	CONDUIT	77.4	1.1630	0.0250	
FutureCamp-117	FutureCamp	117	CONDUIT	120.0	0.1292	0.0130	
FutureComm-CAMPEAU_DRIVE_POND	FutureComm	CAMPEAU_DRIVE_POND	CONDUIT	30.0	0.1500	58.9 0.1528 0.0130	0.0130
Stub-110	Stub	110	CONDUIT				
COP_Low	CAMPEAU_DRIVE_POND	J11	ORIFICE				
COR007_1	CSto097	CJ096	ORIFICE				
COR007_2	CSto097	CJ096	ORIFICE				
NP_Low	NEW_POND	J10	ORIFICE				
10	St173	St154	WEIR				
11	St172	St152A	WEIR				
111-120-S	120-S	111-S	WEIR				
111-S-POND	111-S	CAMPEAU_DRIVE_POND	WEIR				
112-128-S	128-S	112-S	WEIR				
114-114-S	114-S	112-S	WEIR				
115-112-S	112-S	115-S	WEIR				
115-116-S	116-S	115-S	WEIR				
116-118-S	118-S	116-S	WEIR				
117-S-CAMPEAU_DRIVE_POND117-S	117-S	CAMPEAU_DRIVE_POND	WEIR				
12	St514	St513	WEIR				
120-122-S	122-S	120-S	WEIR				
122-146-S	146-S	122-S	WEIR				
126-124-S	124-S	126-S	WEIR				
128-126-S	128-S	128-S	WEIR				
130-142-S	142-S	130-S	WEIR				
131-128-S	131-S	128-S	WEIR				
131-130-S	130-S	131-S	WEIR				
132-130-S	132-S	130-S	WEIR				
132-134-S	134-S	132-S	WEIR				
134-136-S	136-S	134-S	WEIR				
138-136-S	138-S	136-S	WEIR				

14	St513	St512A	WEIR
140-144-S	144-S	140-S	WEIR
142-140-S	140-S	142-S	WEIR
15	St640	St630	WEIR
16	St630	St620	WEIR
17	St620	St605	WEIR
18	St605	NEW_POND	WEIR
5	St170	St157	WEIR
6	St157	St158	WEIR
7	St158	St512A	WEIR
8	St512A	EX_POND	WEIR
9	St154	St512A	WEIR
CDP_High	CAMPEAU_DRIVE_POND	J11	WEIR
CW097_3	CSto097	CJ096	WEIR
FUTURE_TRANSITWAY-S-CAMPEAU_DRIVE_POND	FUTURE_TRANSITWAY-S-CAMPEAU_DRIVE_POND	FUTURE_TRANSITWAY-S-CAMPEAU_DRIVE_POND	WEIR
FutureCamp-CAMPEAU_DRIVE_POND	FutureCamp-S	CAMPEAU_DRIVE_POND	WEIR
NP_High	NEW_POND	J10	WEIR
112-IC	112-S	112	OUTLET
114-IC	114-S	114	OUTLET
115-IC	115-S	115	OUTLET
116-IC	116-S	116	OUTLET
117-IC	117-S	117	OUTLET
118-IC	118-S	118	OUTLET
120-IC	120-S	120	OUTLET
122-IC	122-S	122	OUTLET
124-IC	124-S	124	OUTLET
126-IC	126-S	126	OUTLET
128-IC	128-S	128	OUTLET
130-IC	130-S	130	OUTLET
131-IC	131-S	131	OUTLET
132-IC	132-S	132	OUTLET
134-IC	134-S	134	OUTLET
136-IC	136-S	136	OUTLET
138-IC	138-S	138	OUTLET
140-IC	140-S	140	OUTLET
142-IC	142-S	142	OUTLET
144-IC	144-S	144	OUTLET
146-IC	146-S	146	OUTLET
FUTURE_TRANSITWAY-IC	FUTURE_TRANSITWAY-S	FutureComm	OUTLET
FutureCamp-Backwater	FutureCamp	FutureCamp-S	OUTLET
FutureCamp-IC	FutureCamp-S	FutureCamp	OUTLET
FutureComm-IC	FutureComm-S	FutureComm	OUTLET
0154	St154	154	OUTLET
0157	St157	157	OUTLET
0158	St158	158	OUTLET
0170	St170	170	OUTLET
0172	St172	172	OUTLET
0173	St173	173	OUTLET
0512A	St512A	512A	OUTLET
0513	St513	513	OUTLET
0514	St514	514	OUTLET
0605	St605	605	OUTLET
0620	St620	620	OUTLET
0630	St630	630	OUTLET
0640	St640	640	OUTLET
Stub-IC	Stub-S	Stub	OUTLET

Cross Section Summary							
Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
110-113	FILLED_CIRCULAR	1.01	1.15	0.28	1.35	1	1.46
111-110	CIRCULAR	0.82	0.53	0.21	0.82	1	0.56
112-115	CIRCULAR	1.05	0.87	0.26	1.05	1	1.22
113-117	FILLED_CIRCULAR	1.24	1.72	0.34	1.65	1	2.48
114-112	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
114-118	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
115-110	CIRCULAR	1.05	0.87	0.26	1.05	1	1.06
116-115	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
117-CAMPEAU_DRIVE	FILLED_CIRCULAR	1.24	1.72	0.34	1.65	1	2.88
118-116	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
120-111	CIRCULAR	0.82	0.53	0.21	0.82	1	0.56
122-120	CIRCULAR	0.82	0.53	0.21	0.82	1	0.56
124-120	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
124-126	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
126-114	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
126-128	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
128-112	CIRCULAR	1.05	0.87	0.26	1.05	1	1.23
129-128	CIRCULAR	1.05	0.87	0.26	1.05	1	1.06
130-131	CIRCULAR	0.97	0.75	0.24	0.97	1	0.87
131-129	CIRCULAR	1.05	0.87	0.26	1.05	1	1.06
132-130	CIRCULAR	0.60	0.36	0.17	0.60	1	0.33
134-132	CIRCULAR	0.60	0.36	0.17	0.60	1	0.33
136-134	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
138-140	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
140-122	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
140-142	CIRCULAR	0.75	0.44	0.19	0.75	1	0.43
142-130	CIRCULAR	0.82	0.53	0.21	0.82	1	0.55
144-140	CIRCULAR	0.45	0.16	0.11	0.45	1	0.13
144-146	CIRCULAR	0.53	0.22	0.13	0.53	1	0.19
146-122	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
154-512	CIRCULAR	0.68	0.36	0.17	0.68	1	0.33
156-157	CIRCULAR	0.38	0.11	0.09	0.38	1	0.08
157-158	CIRCULAR	0.60	0.28	0.15	0.60	1	0.24
158-160	CIRCULAR	0.60	0.36	0.17	0.60	1	0.33
160-512	CIRCULAR	0.75	0.44	0.19	0.75	1	0.43
170-156	CIRCULAR	0.38	0.11	0.09	0.38	1	0.09
172-160	CIRCULAR	0.53	0.22	0.13	0.53	1	0.19
173-174	CIRCULAR	0.38	0.11	0.09	0.38	1	0.09
174-154	CIRCULAR	0.38	0.11	0.09	0.38	1	0.09
512A-EX_POND	CIRCULAR	0.82	0.53	0.21	0.82	1	0.82
512-EX_POND	CIRCULAR	0.90	0.64	0.23	0.90	1	1.11
513-512A	CIRCULAR	0.53	0.21	0.12	0.53	1	0.56
514-513	CIRCULAR	0.38	0.11	0.09	0.38	1	0.09
605-NEW_POND	CIRCULAR	1.05	0.87	0.26	1.05	1	1.07
610-605	CIRCULAR	1.05	0.87	0.26	1.05	1	1.03
620-610	CIRCULAR	1.05	0.87	0.26	1.05	1	1.07
630-620	CIRCULAR	0.90	0.64	0.23	0.90	1	0.69
640-630	CIRCULAR	0.82	0.53	0.21	0.82	1	0.57
C12	TRAPEZOIDAL	1.00	6.00	0.64	9.00	1	5.60
C2	RECT_OPEN	1.00	1.00	0.33	1.00	1	0.06
C6	RECT_CLOSED	1.50	3.60	0.46	2.40	4	0.46
CC065	40703	5.51	886.40	1.59	214.56	1	1998.46
CC066	40713	4.98	833.59	1.48	209.62	1	1253.08
CC067	40753	5.60	983.57	1.59	224.90	1	1547.17
CC068	40900_3	4.94	922.55	1.28	228.85	1	383.30
CC069	40910	4.94	811.51	1.34	208.22	1	493.96
CC070	40919_2	6.21	1065.37	1.41	242.84	1	2429.72
CC071	40956	4.85	864.04	1.38	220.18	1	1084.41
CC072	40965_2	6.25	1072.10	1.71	220.44	1	1386.94
CC073	41071	4.82	724.41	1.37	195.56	1	478.60
CC074	41117	7.33	1337.67	2.24	220.36	1	1252.54
CC075	41125_4	7.09	1383.90	1.92	221.70	1	1945.44
CC076	41180	4.79	894.12	1.24	250.67	1	71.44
CC077	DITCH	6.80	27.78	1.91	6.00	1	33.97
CC078	41198	4.78	931.37	1.19	252.82	1	601.94
CC079	41320	4.78	1202.14	1.07	357.02	1	53.34
CC080	CIRCULAR	2.25	3.98	0.56	2.25	1	10.54
CC080_1	41323_6	4.78	1189.05	1.06	360.88	1	1006.27
CC081	41338	4.76	1246.63	1.09	376.32	1	1072.99
CC083	41349_4	5.38	1326.57	1.13	406.06	1	186.82
CC084	41357	5.37	1333.30	1.24	379.29	1	1351.26
CC089	41374_8	4.75	338.24	2.07	110.62	1	61.68
CC094	DITCH	6.80	27.78	1.91	6.00	1	36.28
CC095	CIRCULAR	1.20	1.13	0.30	1.20	1	1.83
CC096	CIRCULAR	1.20	1.13	0.30	1.20	1	3.00
CC098	CIRCULAR	1.65	2.14	0.41	1.65	1	6.45
CC100	41572	4.70	459.63	2.20	110.12	1	336.94
CC101	FutureCampeau_Culvert	4.45	93.93	1.53	28.01	1	324.82
CC102	FutureTransitway_Culvert	3.68	67.40	1.13	28.00	1	95.93
CC104	41608	4.68	454.67	1.79	110.12	1	132.22
CC105	41671	4.65	362.08	1.54	113.32	1	232.37
CC106	41725_5	4.65	782.66	1.25	227.21	1	61.63

CC266	TRAPEZOIDAL	1.00	6.00	0.64	9.00	1	19.29	
FutureCamp-117	FILLED_CIRCULAR	0.56	0.36	0.15	0.75	1	0.28	
FutureComm-CAMPEAU DRIVE	FILLED_CIRCULAR	0.79	0.90	0.91	0.25	1	0.75	1.08
Stub-110	FILLED_CIRCULAR		0.70	0.22	1.05	1		

Shape Summary

Shape FutureCampeau_Culvert

Area:	0.0060	0.0125	0.0195	0.0272	0.0353
	0.0441	0.0534	0.0632	0.0736	0.0845
	0.0960	0.1081	0.1207	0.1338	0.1476
	0.1618	0.1766	0.1929	0.2079	0.2244
	0.2415	0.2590	0.2835	0.3100	0.3365
	0.3631	0.3896	0.4162	0.4427	0.4692
	0.4958	0.5223	0.5488	0.5754	0.6019
	0.6285	0.6550	0.6815	0.7081	0.7346
	0.7612	0.7877	0.8142	0.8408	0.8673
	0.8938	0.9204	0.9469	0.9735	1.0000
Hrad:	0.0555	0.1063	0.1536	0.1980	0.2403
	0.2807	0.3196	0.3573	0.3939	0.4297
	0.4647	0.4990	0.5328	0.5661	0.5990
	0.6314	0.6636	0.6954	0.7270	0.7583
	0.7893	0.8202	0.8507	0.8809	0.9121
	0.7635	0.8144	0.8646	0.9143	0.9633
	1.0118	1.0597	1.1071	1.1539	1.2001
	1.2459	1.2911	1.3358	1.3800	1.4237
	1.4669	1.5096	1.5519	1.5937	1.6350
	1.6759	1.7164	1.7564	1.7960	1.8350
Width:	0.2351	0.2559	0.2768	0.2977	0.3185
	0.3394	0.3602	0.3811	0.4020	0.4228
	0.4437	0.4646	0.4854	0.5063	0.5271
	0.5480	0.5689	0.5897	0.6106	0.6314
	0.6523	0.6732	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000

Shape FutureTransitway_Culvert

Area:	0.0078	0.0160	0.0245	0.0332	0.0423
	0.0517	0.0614	0.0714	0.0817	0.0924
	0.1033	0.1145	0.1261	0.1380	0.1501
	0.1626	0.1754	0.1885	0.2019	0.2157
	0.2342	0.2543	0.2747	0.2953	0.3161
	0.3371	0.3583	0.3797	0.4013	0.4232
	0.4452	0.4675	0.4900	0.5127	0.5414
	0.5720	0.6025	0.6331	0.6637	0.6943
	0.7248	0.7554	0.7860	0.8166	0.8471
	0.8777	0.9083	0.9389	0.9694	1.0000
Hrad:	0.0637	0.1244	0.1827	0.2386	0.2926
	0.3448	0.3954	0.4446	0.4924	0.5391
	0.5847	0.6293	0.6730	0.7159	0.7581
	0.7996	0.8404	0.8806	0.9203	0.9594
	0.7313	0.7843	0.8366	0.8883	0.9394
	0.9898	1.0397	1.0891	1.1379	1.1861
	1.2339	1.2812	1.3280	1.3744	1.4205
	1.1512	1.2068	1.2618	1.3162	1.3702
	1.4236	1.4765	1.5289	1.5808	1.6322
	1.6831	1.7336	1.7835	1.8330	1.8822
Width:	0.2616	0.2717	0.2818	0.2919	0.3021
	0.3122	0.3223	0.3324	0.3426	0.3527
	0.3628	0.3729	0.3831	0.3932	0.4033
	0.4134	0.4236	0.4337	0.4438	0.4540
	0.6559	0.6628	0.6697	0.6766	0.6835
	0.6904	0.6973	0.7042	0.7110	0.7179
	0.7248	0.7317	0.7386	0.7455	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000

Transect Summary

Transect 40703

Area:	0.0007	0.0015	0.0026	0.0037	0.0051
	0.0066	0.0084	0.0106	0.0134	0.0167
	0.0236	0.0360	0.0569	0.0807	0.1047
	0.1290	0.1534	0.1780	0.2026	0.2274
	0.2523	0.2772	0.3022	0.3273	0.3524
	0.3776	0.4028	0.4281	0.4535	0.4789
	0.5044	0.5299	0.5555	0.5812	0.6069
	0.6327	0.6585	0.6844	0.7104	0.7364
	0.7625	0.7886	0.8148	0.8411	0.8674
	0.8938	0.9203	0.9468	0.9734	1.0000
Hrad:	0.0613	0.1128	0.1590	0.2020	0.2428
	0.2823	0.2898	0.2918	0.3049	0.3306
	0.3234	0.2755	0.2230	0.2125	0.2193
	0.2329	0.2505	0.2700	0.2906	0.3121
	0.3343	0.3570	0.3798	0.4029	0.4260
	0.4493	0.4726	0.4959	0.5192	0.5425
	0.5658	0.5891	0.6124	0.6356	0.6588
	0.6819	0.7050	0.7280	0.7510	0.7739
	0.7968	0.8196	0.8423	0.8650	0.8877
	0.9103	0.9328	0.9552	0.9777	1.0000
Width:	0.0289	0.0352	0.0415	0.0477	0.0540
	0.0603	0.0744	0.0937	0.1131	0.1429
	0.3276	0.5993	0.8851	0.8954	0.9057
	0.9148	0.9191	0.9233	0.9276	0.9315
	0.9338	0.9361	0.9384	0.9407	0.9429
	0.9452	0.9475	0.9498	0.9521	0.9544
	0.9566	0.9589	0.9612	0.9635	0.9658
	0.9681	0.9703	0.9726	0.9749	0.9772
	0.9795	0.9817	0.9840	0.9863	0.9886
	0.9909	0.9932	0.9954	0.9977	1.0000

Transect 40713

Area:	0.0006	0.0014	0.0024	0.0035	0.0057
	0.0138	0.0224	0.0317	0.0418	0.0526
	0.0650	0.0826	0.1039	0.1256	0.1478
	0.1782	0.1929	0.2159	0.2391	0.2625
	0.2860	0.3097	0.3336	0.3575	0.3815
	0.4055	0.4297	0.4540	0.4783	0.5027
	0.5271	0.5515	0.5761	0.6007	0.6253
	0.6500	0.6747	0.6996	0.7244	0.7494
	0.7744	0.7995	0.8245	0.8496	0.8747
	0.8997	0.9248	0.9499	0.9749	1.0000
Hrad:	0.0599	0.1106	0.1561	0.1984	0.1842
	0.1113	0.1107	0.1165	0.1294	0.1444
	0.1558	0.1480	0.1660	0.1856	0.2059
	0.2270	0.2483	0.2696	0.2915	0.3138
	0.3361	0.3582	0.3809	0.4036	0.4266
	0.4490	0.4716	0.4945	0.5175	0.5405
	0.5634	0.5862	0.6090	0.6317	0.6544
	0.6769	0.6992	0.7214	0.7436	0.7657
	0.7877	0.8110	0.8346	0.8583	0.8819
	0.9056	0.9292	0.9528	0.9764	1.0000
Width:	0.0288	0.0347	0.0406	0.0465	0.0524

	0.3341	0.3561	0.3866	0.4170	0.4515
	0.5537	0.8323	0.8612	0.8755	0.8897
	0.9087	0.9115	0.9222	0.9300	0.9360
	0.9420	0.9480	0.9522	0.9557	0.9592
	0.9627	0.9662	0.9688	0.9709	0.9731
	0.9752	0.9774	0.9796	0.9817	0.9839
	0.9863	0.9889	0.9915	0.9941	0.9967
	0.9993	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 40753					
Area:	0.0006	0.0014	0.0023	0.0048	0.0118
	0.0193	0.0274	0.0361	0.0485	0.0652
	0.0831	0.1045	0.1262	0.1481	0.1702
	0.1925	0.2150	0.2376	0.2603	0.2831
	0.3061	0.3291	0.3521	0.3753	0.3985
	0.4218	0.4451	0.4685	0.4920	0.5156
	0.5392	0.5628	0.5866	0.6104	0.6343
	0.6582	0.6822	0.7062	0.7304	0.7546
	0.7788	0.8031	0.8275	0.8520	0.8765
	0.9010	0.9257	0.9504	0.9751	1.0000
Hrad:					
	0.0615	0.1124	0.1580	0.1276	0.0936
	0.0991	0.1144	0.1329	0.1152	0.1370
	0.1573	0.1739	0.1932	0.2138	0.2352
	0.2569	0.2788	0.3012	0.3242	0.3472
	0.3702	0.3935	0.4169	0.4401	0.4634
	0.4866	0.5097	0.5327	0.5558	0.5788
	0.6018	0.6247	0.6475	0.6702	0.6929
	0.7155	0.7381	0.7605	0.7829	0.8052
	0.8275	0.8497	0.8713	0.8938	0.9158
	0.9377	0.9595	0.9813	1.0030	1.0000
Width:					
	0.0271	0.0337	0.0403	0.2632	0.2802
	0.3044	0.3286	0.3529	0.6282	0.6710
	0.8011	0.8454	0.8524	0.8593	0.8663
	0.8733	0.8803	0.8859	0.8895	0.8932
	0.8968	0.8996	0.9022	0.9049	0.9076
	0.9103	0.9130	0.9156	0.9182	0.9207
	0.9232	0.9257	0.9282	0.9307	0.9332
	0.9357	0.9382	0.9408	0.9433	0.9458
	0.9483	0.9508	0.9533	0.9558	0.9583
	0.9608	0.9633	0.9658	0.9683	1.0000
Transect 40900.3					
Area:	0.0005	0.0011	0.0018	0.0026	0.0068
	0.0144	0.0222	0.0304	0.0387	0.0491
	0.0656	0.0845	0.1047	0.1257	0.1474
	0.1696	0.1925	0.2160	0.2401	0.2644
	0.2889	0.3134	0.3379	0.3624	0.3869
	0.4115	0.4360	0.4605	0.4850	0.5095
	0.5341	0.5586	0.5831	0.6076	0.6322
	0.6567	0.6812	0.7057	0.7302	0.7548
	0.7793	0.8038	0.8283	0.8529	0.8774
	0.9019	0.9264	0.9510	0.9755	1.0000
Hrad:					
	0.0706	0.1323	0.1882	0.2397	0.1218
	0.1023	0.1148	0.1333	0.1538	0.1460
	0.1493	0.1600	0.1730	0.1886	0.2055
	0.2223	0.2395	0.2569	0.2751	0.2978
	0.3205	0.3436	0.3671	0.3907	0.4144
	0.4380	0.4617	0.4853	0.5090	0.5326
	0.5562	0.5798	0.6034	0.6270	0.6505
	0.6740	0.6975	0.7209	0.7444	0.7678
	0.7911	0.8145	0.8378	0.8610	0.8843
	0.9075	0.9307	0.9538	0.9769	1.0000
Width:					
	0.0230	0.0264	0.0299	0.0334	0.3036
	0.3144	0.3252	0.3360	0.3468	0.5293
	0.7388	0.7987	0.8453	0.8698	0.8945
	0.9198	0.9451	0.9703	0.9921	0.9950
	0.9979	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 40910					
Area:	0.0006	0.0013	0.0020	0.0029	0.0038
	0.0049	0.0060	0.0073	0.0086	0.0189
	0.0380	0.0590	0.0810	0.1033	0.1255
	0.1487	0.1718	0.1952	0.2189	0.2428
	0.2670	0.2914	0.3161	0.3410	0.3663
	0.3916	0.4170	0.4423	0.4677	0.4930
	0.5184	0.5437	0.5691	0.5944	0.6198
	0.6451	0.6705	0.6958	0.7212	0.7465
	0.7719	0.7972	0.8225	0.8479	0.8732
	0.8986	0.9239	0.9493	0.9746	1.0000
Hrad:					
	0.0677	0.1271	0.1809	0.2310	0.2783
	0.3236	0.3674	0.4099	0.4515	0.1497
	0.1206	0.1259	0.1425	0.1624	0.1836
	0.2053	0.2272	0.2492	0.2712	0.2931
	0.3150	0.3367	0.3582	0.3796	0.4009
	0.4249	0.4490	0.4732	0.4973	0.5215
	0.5456	0.5698	0.5939	0.6180	0.6421
	0.6661	0.6902	0.7142	0.7382	0.7621
	0.7861	0.8099	0.8338	0.8576	0.8815
	0.9052	0.9290	0.9527	0.9764	1.0000
Width:					
	0.0249	0.0286	0.0322	0.0358	0.0394
	0.0430	0.0466	0.0503	0.0539	0.0575
	0.7931	0.8006	0.8740	0.8847	0.8955
	0.9063	0.9171	0.9279	0.9384	0.9486
	0.9589	0.9691	0.9794	0.9897	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 40919.2					
Area:	0.0006	0.0012	0.0020	0.0028	0.0037
	0.0048	0.0059	0.0080	0.0113	0.0199
	0.0310	0.0443	0.0594	0.0764	0.0955
	0.1164	0.1389	0.1629	0.1871	0.2117
	0.2365	0.2617	0.2870	0.3123	0.3378
	0.3633	0.3889	0.4146	0.4403	0.4662
	0.4921	0.5181	0.5442	0.5703	0.5966
	0.6229	0.6493	0.6758	0.7024	0.7290
	0.7558	0.7820	0.8085	0.8355	0.8635
	0.8986	0.9179	0.9452	0.9725	1.0000
Hrad:					
	0.0806	0.1510	0.2148	0.2739	0.3285
	0.3799	0.4298	0.4300	0.3954	0.2704
	0.2304	0.2172	0.2166	0.2202	0.2265
	0.2364	0.2475	0.2639	0.2845	0.3061
	0.3281	0.3481	0.3717	0.3958	0.4200
	0.4445	0.4689	0.4935	0.5181	0.5427
	0.5672	0.5918	0.6163	0.6408	0.6657
	0.6896	0.7139	0.7381	0.7623	0.7865
	0.8105	0.8345	0.8584	0.8823	0.9061
	0.9298	0.9534	0.9770	1.0004	1.0000
Width:					
	0.0217	0.0249	0.0280	0.0311	0.0344
	0.0378	0.0412	0.0925	0.1670	0.3526
	0.4315	0.5025	0.5651	0.6380	0.7107
	0.7683	0.8258	0.8523	0.8630	0.8738
	0.8846	0.8914	0.8950	0.8979	0.9008
	0.9036	0.9065	0.9093	0.9122	0.9150
	0.9179	0.9208	0.9236	0.9265	0.9293

	0.9322	0.9350	0.9379	0.9407	0.9436
	0.9464	0.9493	0.9522	0.9550	0.9579
	0.9607	0.9636	0.9664	0.9693	1.0000
Transect 40956.					
Area:					
	0.0005	0.0011	0.0018	0.0045	0.0110
	0.0179	0.0250	0.0325	0.0404	0.0514
	0.0658	0.0832	0.1048	0.1272	0.1502
	0.1734	0.1966	0.2199	0.2433	0.2668
	0.2904	0.3141	0.3378	0.3616	0.3856
	0.4095	0.4336	0.4578	0.4820	0.5064
	0.5308	0.5553	0.5799	0.6046	0.6293
	0.6540	0.6787	0.7034	0.7281	0.7528
	0.7776	0.8023	0.8270	0.8517	0.8764
	0.9011	0.9259	0.9506	0.9753	1.0000
Hrad:					
	0.0648	0.1217	0.1734	0.1073	0.0824
	0.0944	0.1114	0.1299	0.1314	0.1151
	0.1210	0.1354	0.1485	0.1628	0.1840
	0.2067	0.2297	0.2528	0.2760	0.2993
	0.3225	0.3458	0.3690	0.3921	0.4152
	0.4383	0.4612	0.4841	0.5069	0.5297
	0.5523	0.5749	0.5974	0.6198	0.6438
	0.6677	0.6917	0.7156	0.7395	0.7633
	0.7871	0.8109	0.8346	0.8584	0.8821
	0.9057	0.9293	0.9529	0.9765	1.0000
Width:					
	0.0227	0.0259	0.0291	0.2591	0.2711
	0.2831	0.2952	0.3073	0.3640	0.5217
	0.6349	0.8400	0.8901	0.9273	0.9350
	0.9384	0.9418	0.9453	0.9487	0.9521
	0.9555	0.9589	0.9623	0.9657	0.9692
	0.9726	0.9760	0.9794	0.9828	0.9862
	0.9897	0.9931	0.9965	0.9999	0.9999
	0.9999	0.9999	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 40965.2					
Area:					
	0.0006	0.0014	0.0028	0.0096	0.0168
	0.0243	0.0322	0.0424	0.0569	0.0738
	0.0920	0.1114	0.1314	0.1519	0.1729
	0.1943	0.2161	0.2389	0.2621	0.2854
	0.3047	0.3271	0.3496	0.3723	0.3950
	0.4179	0.4409	0.4640	0.4873	0.5106
	0.5342	0.5578	0.5815	0.6052	0.6291
	0.6530	0.6770	0.7011	0.7253	0.7495
	0.7742	0.7988	0.8235	0.8483	0.8733
	0.8984	0.9236	0.9490	0.9744	1.0000
Hrad:					
	0.0677	0.1273	0.1243	0.0741	0.0874
	0.1057	0.1173	0.1028	0.1093	0.1288
	0.1498	0.1703	0.1913	0.2123	0.2331
	0.2538	0.2771	0.3004	0.3239	0.3474
	0.3709	0.3942	0.4174	0.4405	0.4635
	0.4864	0.5099	0.5312	0.5533	0.5753
	0.5974	0.6204	0.6433	0.6660	0.6887
	0.7113	0.7335	0.7548	0.7759	0.7969
	0.8178	0.8386	0.8593	0.8799	0.9000
	0.9207	0.9410	0.9612	0.9813	1.0000
Width:					
	0.0264	0.0299	0.2561	0.2708	0.2852
	0.2997	0.3295	0.4711	0.6264	0.6822
	0.7352	0.7693	0.7881	0.8069	0.8257
	0.8437	0.8513	0.8581	0.8624	0.8667
	0.8710	0.8753	0.8796	0.8839	0.8883
	0.8926	0.8975	0.9026	0.9078	0.9129
	0.9177	0.9207	0.9237	0.9268	0.9298
	0.9328	0.9363	0.9411	0.9458	0.9506
	0.9554	0.9602	0.9650	0.9698	0.9746
	0.9793	0.9841	0.9889	0.9937	1.0000
Transect 41071					
Area:					
	0.0013	0.0027	0.0040	0.0093	0.0165
	0.0240	0.0316	0.0395	0.0476	0.0564
	0.0663	0.0774	0.0896	0.1036	0.1203
	0.1393	0.1600	0.1820	0.2056	0.2301
	0.2547	0.2796	0.3047	0.3298	0.3549
	0.3802	0.4055	0.4308	0.4562	0.4817
	0.5072	0.5328	0.5585	0.5842	0.6100
	0.6359	0.6618	0.6877	0.7137	0.7398
	0.7658	0.7918	0.8178	0.8439	0.8699
	0.8959	0.9219	0.9480	0.9740	1.0000
Hrad:					
	0.0686	0.1347	0.1986	0.1344	0.1285
	0.1423	0.1607	0.1807	0.1980	0.2018
	0.2014	0.2055	0.2125	0.2194	0.2299
	0.2386	0.2538	0.2720	0.2879	0.3065
	0.3259	0.3459	0.3675	0.3900	0.4127
	0.4355	0.4585	0.4815	0.5046	0.5277
	0.5508	0.5739	0.5970	0.6201	0.6433
	0.6662	0.6892	0.7122	0.7358	0.7600
	0.7841	0.8082	0.8322	0.8563	0.8803
	0.9043	0.9283	0.9522	0.9761	1.0000
Width:					
	0.0512	0.0522	0.0532	0.2742	0.2821
	0.2899	0.2978	0.3057	0.3224	0.3594
	0.4030	0.4466	0.4901	0.5311	0.6005
	0.7642	0.8208	0.8690	0.9356	0.9432
	0.9527	0.9598	0.9637	0.9660	0.9684
	0.9707	0.9731	0.9754	0.9778	0.9802
	0.9825	0.9849	0.9872	0.9896	0.9920
	0.9943	0.9967	0.9990	0.9999	0.9999
	0.9999	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41117					
Area:					
	0.0010	0.0021	0.0084	0.0152	0.0221
	0.0298	0.0400	0.0519	0.0688	0.0881
	0.1087	0.1297	0.1508	0.1719	0.1934
	0.2153	0.2372	0.2593	0.2814	0.3037
	0.3259	0.3483	0.3707	0.3932	0.4157
	0.4384	0.4610	0.4838	0.5066	0.5294
	0.5523	0.5753	0.5984	0.6215	0.6447
	0.6679	0.6912	0.7146	0.7380	0.7615
	0.7851	0.8087	0.8324	0.8562	0.8800
	0.9039	0.9278	0.9518	0.9759	1.0000
Hrad:					
	0.0633	0.1231	0.0649	0.0790	0.0978
	0.1025	0.1024	0.1127	0.1184	0.1278
	0.1433	0.1653	0.1877	0.2104	0.2307
	0.2533	0.2760	0.2987	0.3213	0.3441
	0.3670	0.3898	0.4126	0.4353	0.4580
	0.4806	0.5031	0.5256	0.5479	0.5702
	0.5925	0.6146	0.6367	0.6586	0.6805
	0.7024	0.7241	0.7458	0.7674	0.7889
	0.8104	0.8317	0.8530	0.8742	0.8954
	0.9165	0.9374	0.9584	0.9792	1.0000
Width:					
	0.0425	0.0437	0.2761	0.2846	0.2930
	0.3760	0.4474	0.6000	0.7574	0.8334
	0.8658	0.8700	0.8741	0.8782	0.9040
	0.9075	0.9110	0.9146	0.9181	0.9212
	0.9238	0.9264	0.9291	0.9317	0.9343
	0.9370	0.9396	0.9422	0.9448	0.9475
	0.9501	0.9527	0.9554	0.9580	0.9606
	0.9632	0.9659	0.9685	0.9711	0.9737
	0.9764	0.9790	0.9816	0.9842	0.9869
	0.9895	0.9921	0.9947	0.9974	1.0000
Transect 41125.4					
Area:					

	0.0006	0.0015	0.0058	0.0106	0.0159
	0.0224	0.0314	0.0424	0.0551	0.0705
	0.0884	0.1093	0.1306	0.1522	0.1740
	0.1960	0.2182	0.2405	0.2631	0.2857
	0.3084	0.3312	0.3541	0.3770	0.4001
	0.4232	0.4463	0.4696	0.4930	0.5164
	0.5399	0.5635	0.5872	0.6109	0.6347
	0.6586	0.6826	0.7066	0.7308	0.7549
	0.7792	0.8035	0.8279	0.8523	0.8768
	0.9014	0.9261	0.9507	0.9754	1.0000
Hrad:	0.0753	0.1213	0.0743	0.0895	0.1080
	0.1126	0.1178	0.1323	0.1481	0.1583
	0.1700	0.1818	0.1984	0.2167	0.2361
	0.2561	0.2765	0.2970	0.3188	0.3412
	0.3637	0.3861	0.4085	0.4309	0.4533
	0.4756	0.4978	0.5200	0.5421	0.5641
	0.5861	0.6080	0.6298	0.6515	0.6732
	0.6949	0.7167	0.7384	0.7601	0.7817
	0.8032	0.8246	0.8459	0.8672	0.8884
	0.9088	0.9317	0.9545	0.9773	1.0000
Width:	0.0270	0.1636	0.1849	0.2062	0.2275
	0.3098	0.4173	0.4725	0.5604	0.6744
	0.7930	0.8618	0.8705	0.8803	0.8882
	0.8962	0.9042	0.9122	0.9169	0.9201
	0.9233	0.9266	0.9298	0.9330	0.9363
	0.9395	0.9427	0.9460	0.9492	0.9525
	0.9557	0.9589	0.9622	0.9654	0.9686
	0.9716	0.9744	0.9771	0.9799	0.9827
	0.9855	0.9882	0.9910	0.9938	0.9965
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41180					
Area:	0.0006	0.0011	0.0017	0.0071	0.0128
	0.0188	0.0251	0.0316	0.0386	0.0468
	0.0582	0.0728	0.0901	0.1105	0.1324
	0.1552	0.1784	0.2019	0.2258	0.2501
	0.2747	0.2995	0.3244	0.3494	0.3744
	0.3995	0.4245	0.4495	0.4745	0.4995
	0.5246	0.5496	0.5746	0.5996	0.6247
	0.6497	0.6747	0.6997	0.7247	0.7498
	0.7748	0.7998	0.8248	0.8499	0.8749
	0.8999	0.9249	0.9500	0.9750	1.0000
Hrad:	0.0744	0.1438	0.2092	0.0841	0.0936
	0.1115	0.1311	0.1511	0.1611	0.1578
	0.1601	0.1589	0.1585	0.1700	0.1855
	0.2035	0.2224	0.2418	0.2615	0.2813
	0.3012	0.3245	0.3481	0.3717	0.3958
	0.4202	0.4445	0.4689	0.4933	0.5176
	0.5420	0.5663	0.5906	0.6149	0.6391
	0.6634	0.6876	0.7118	0.7360	0.7601
	0.7842	0.8083	0.8324	0.8564	0.8804
	0.9044	0.9283	0.9522	0.9761	1.0000
Width:	0.0226	0.0238	0.0250	0.2237	0.2342
	0.2446	0.2551	0.2655	0.3002	0.3827
	0.5217	0.6433	0.7566	0.8451	0.9046
	0.9190	0.9334	0.9477	0.9623	0.9767
	0.9911	0.9944	0.9967	0.9990	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41198					
Area:	0.0005	0.0011	0.0021	0.0080	0.0142
	0.0207	0.0275	0.0346	0.0422	0.0510
	0.0615	0.0741	0.0887	0.1054	0.1233
	0.1423	0.1624	0.1837	0.2060	0.2295
	0.2542	0.2795	0.3049	0.3303	0.3557
	0.3812	0.4067	0.4322	0.4577	0.4833
	0.5089	0.5345	0.5602	0.5859	0.6116
	0.6373	0.6631	0.6889	0.7147	0.7406
	0.7665	0.7924	0.8184	0.8443	0.8702
	0.8962	0.9221	0.9481	0.9740	1.0000
Hrad:	0.0757	0.1444	0.1567	0.0771	0.0910
	0.1109	0.1321	0.1535	0.1634	0.1716
	0.1766	0.1816	0.1886	0.2001	0.2181
	0.2355	0.2521	0.2683	0.2840	0.2993
	0.3140	0.3340	0.3564	0.3791	0.4022
	0.4255	0.4489	0.4725	0.4961	0.5199
	0.5436	0.5674	0.5913	0.6151	0.6390
	0.6628	0.6867	0.7105	0.7344	0.7582
	0.7820	0.8058	0.8297	0.8541	0.8785
	0.9028	0.9271	0.9515	0.9757	1.0000
Width:	0.0207	0.0227	0.2236	0.2343	0.2451
	0.2558	0.2665	0.2773	0.3161	0.3672
	0.4444	0.5217	0.6177	0.6675	0.7106
	0.7537	0.7968	0.8399	0.8830	0.9261
	0.9709	0.9772	0.9783	0.9794	0.9805
	0.9816	0.9827	0.9838	0.9849	0.9860
	0.9871	0.9882	0.9893	0.9904	0.9915
	0.9926	0.9937	0.9948	0.9959	0.9970
	0.9981	0.9992	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41320					
Area:	0.0005	0.0011	0.0022	0.0084	0.0149
	0.0216	0.0286	0.0357	0.0434	0.0521
	0.0617	0.0724	0.0846	0.0986	0.1137
	0.1296	0.1465	0.1644	0.1831	0.2028
	0.2234	0.2450	0.2676	0.2909	0.3146
	0.3386	0.3629	0.3875	0.4124	0.4377
	0.4636	0.4902	0.5176	0.5457	0.5741
	0.6025	0.6309	0.6593	0.6877	0.7161
	0.7445	0.7729	0.8013	0.8296	0.8580
	0.8864	0.9148	0.9432	0.9716	1.0000
Hrad:	0.0856	0.1651	0.1796	0.0870	0.1026
	0.1254	0.1496	0.1742	0.1863	0.1976
	0.2092	0.2211	0.2205	0.2421	0.2623
	0.2815	0.2998	0.3173	0.3343	0.3508
	0.3668	0.3825	0.3970	0.4148	0.4331
	0.4519	0.4709	0.4901	0.5095	0.5282
	0.5386	0.5496	0.5610	0.5759	0.6023
	0.6287	0.6552	0.6817	0.7082	0.7347
	0.7613	0.7878	0.8144	0.8409	0.8674
	0.8940	0.9205	0.9470	0.9735	1.0000
Width:	0.0200	0.0214	0.2169	0.2247	0.2325
	0.2403	0.2480	0.2558	0.2882	0.3230
	0.3577	0.3925	0.4799	0.5128	0.5457
	0.5786	0.6115	0.6444	0.6773	0.7103
	0.7432	0.7761	0.8168	0.8280	0.8393
	0.8505	0.8618	0.8731	0.8843	0.8969
	0.9241	0.9513	0.9785	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41323.6					
Area:	0.0005	0.0011	0.0017	0.0023	0.0030
	0.0037	0.0045	0.0053	0.0062	0.0072
	0.0082	0.0093	0.0109	0.0368	0.0549
	0.0740	0.0942	0.1154	0.1375	0.1605
	0.1842	0.2088	0.2343	0.2602	0.2864
	0.3129	0.3397	0.3667	0.3940	0.4216

JLR 26299
April 2017

	0.2315	0.2644	0.2967	0.3180	0.3393
	0.3607	0.3813	0.3883	0.3697	0.3529
	0.3466	0.3313	0.2822	0.2632	0.2587
	0.2562	0.2589	0.2745	0.2950	0.3181
	0.3427	0.3682	0.3944	0.4210	0.4480
	0.4752	0.5025	0.5300	0.5576	0.5852
	0.6128	0.6405	0.6682	0.6960	0.7237
	0.7514	0.7791	0.8068	0.8345	0.8621
	0.8897	0.9173	0.9449	0.9725	1.0000
Width:	0.0599	0.0625	0.0651	0.0677	0.0703
	0.0729	0.0754	0.0777	0.0804	0.0831
	0.0948	0.1008	0.1111	0.1318	0.1570
	0.1823	0.2180	0.2989	0.3799	0.4608
	0.8300	0.9998	0.9998	0.9998	0.9998
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41572					
Area:	0.0009	0.0020	0.0033	0.0048	0.0133
	0.0299	0.0476	0.0658	0.0844	0.1035
	0.1237	0.1448	0.1668	0.1893	0.2118
	0.2343	0.2569	0.2794	0.3019	0.3244
	0.3469	0.3695	0.3920	0.4145	0.4370
	0.4595	0.4820	0.5046	0.5271	0.5496
	0.5721	0.5946	0.6172	0.6397	0.6622
	0.6847	0.7072	0.7298	0.7523	0.7748
	0.7973	0.8198	0.8424	0.8649	0.8874
	0.9099	0.9324	0.9550	0.9775	1.0000
Hrad:	0.0383	0.0710	0.1004	0.1277	0.0471
	0.0658	0.0883	0.1114	0.1341	0.1551
	0.1747	0.1934	0.2120	0.2329	0.2543
	0.2758	0.2975	0.3191	0.3408	0.3625
	0.3842	0.4059	0.4276	0.4492	0.4708
	0.4924	0.5140	0.5355	0.5570	0.5785
	0.5999	0.6213	0.6427	0.6640	0.6853
	0.7065	0.7277	0.7489	0.7701	0.7911
	0.8122	0.8332	0.8542	0.8751	0.8960
	0.9169	0.9377	0.9585	0.9793	1.0000
Width:	0.0449	0.0534	0.0619	0.0705	0.6194
	0.7752	0.7955	0.8157	0.8360	0.8730
	0.9159	0.9357	0.9557	0.9998	0.9998
	0.9998	0.9998	0.9998	0.9998	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41608					
Area:	0.0009	0.0021	0.0034	0.0049	0.0120
	0.0249	0.0413	0.0600	0.0799	0.1001
	0.1208	0.1418	0.1631	0.1846	0.2066
	0.2293	0.2520	0.2746	0.2973	0.3200
	0.3426	0.3653	0.3880	0.4106	0.4333
	0.4560	0.4786	0.5013	0.5240	0.5466
	0.5693	0.5920	0.6146	0.6373	0.6600
	0.6826	0.7053	0.7280	0.7506	0.7733
	0.7960	0.8186	0.8413	0.8640	0.8867
	0.9093	0.9320	0.9547	0.9773	1.0000
Hrad:	0.0469	0.0866	0.1224	0.1556	0.0730
	0.0853	0.0998	0.1153	0.1343	0.1543
	0.1745	0.1947	0.2161	0.2375	0.2529
	0.2744	0.2960	0.3178	0.3395	0.3613
	0.3831	0.4048	0.4266	0.4483	0.4701
	0.4918	0.5134	0.5350	0.5566	0.5782
	0.5997	0.6211	0.6426	0.6639	0.6853
	0.7066	0.7278	0.7490	0.7702	0.7913
	0.8124	0.8334	0.8544	0.8753	0.8962
	0.9171	0.9379	0.9586	0.9793	1.0000
Width:	0.0453	0.0542	0.0632	0.0721	0.4610
	0.6755	0.7747	0.8663	0.8840	0.9018
	0.9195	0.9369	0.9431	0.9494	0.9998
	0.9998	0.9998	0.9998	0.9998	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41671					
Area:	0.0021	0.0043	0.0067	0.0091	0.0117
	0.0144	0.0173	0.0202	0.0235	0.0275
	0.0323	0.0378	0.0442	0.0548	0.0688
	0.0858	0.1058	0.1269	0.1483	0.1701
	0.1922	0.2147	0.2375	0.2607	0.2843
	0.3083	0.3331	0.3597	0.3888	0.4179
	0.4470	0.4761	0.5052	0.5343	0.5634
	0.5925	0.6216	0.6507	0.6798	0.7089
	0.7381	0.7672	0.7963	0.8254	0.8545
	0.8836	0.9127	0.9418	0.9709	1.0000
Hrad:	0.0581	0.1127	0.1645	0.2138	0.2611
	0.3065	0.3504	0.3929	0.4352	0.4775
	0.3707	0.3801	0.3979	0.4093	0.4050
	0.3960	0.3876	0.3903	0.3996	0.4127
	0.4281	0.4450	0.4629	0.4813	0.5002
	0.5164	0.5296	0.5381	0.5420	0.5508
	0.5803	0.6005	0.6211	0.6421	0.6635
	0.6851	0.7069	0.7289	0.7511	0.7734
	0.7958	0.8183	0.8409	0.8635	0.8862
	0.9089	0.9316	0.9544	0.9772	1.0000
Width:	0.0745	0.0786	0.0826	0.0867	0.0908
	0.0949	0.0990	0.1030	0.1257	0.1510
	0.1764	0.2017	0.2598	0.4308	0.5336
	0.6364	0.7186	0.7306	0.7426	0.7545
	0.7665	0.7784	0.7904	0.8024	0.8143
	0.8383	0.8741	0.9080	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	0.9999	0.9999	0.9999
	0.9999	0.9999	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect 41725.5					
Area:	0.0006	0.0012	0.0020	0.0028	0.0037
	0.0089	0.0192	0.0299	0.0409	0.0522
	0.0648	0.0788	0.0930	0.1077	0.1243
	0.1424	0.1611	0.1803	0.2001	0.2204
	0.2413	0.2628	0.2848	0.3074	0.3307
	0.3548	0.3798	0.4061	0.4330	0.4600
	0.4870	0.5140	0.5410	0.5680	0.5950
	0.6220	0.6490	0.6760	0.7030	0.7300
	0.7570	0.7840	0.8110	0.8380	0.8650
	0.8920	0.9190	0.9460	0.9730	1.0000
Hrad:	0.0693	0.1310	0.1874	0.2400	0.2889
	0.0866	0.0979	0.1227	0.1495	0.1765
	0.1878	0.2117	0.2361	0.2593	0.2570
	0.2763	0.2964	0.3163	0.3358	0.3551
	0.3741	0.3926	0.4101	0.4273	0.4437
	0.4579	0.4707	0.4793	0.5020	0.5249
	0.5481	0.5713	0.5948	0.6183	0.6419

	0.6656	0.6894	0.7132	0.7370	0.7609
	0.7848	0.8087	0.8326	0.8565	0.8804
Width:	0.9044	0.9283	0.9522	0.9761	1.0000
	0.0230	0.0258	0.0286	0.0314	0.0440
	0.3582	0.3899	0.4010	0.4120	0.4231
	0.5143	0.5229	0.5316	0.5745	0.6560
	0.6813	0.7018	0.7223	0.7429	0.7634
	0.7839	0.8048	0.8275	0.8498	0.8762
	0.9093	0.9468	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
	1.0000	1.0000	1.0000	1.0000	1.0000
Transect DITCH					
Area:	0.0011	0.0044	0.0152	0.0279	0.0410
	0.0544	0.0682	0.0823	0.0968	0.1117
	0.1269	0.1425	0.1584	0.1747	0.1914
	0.2084	0.2257	0.2435	0.2615	0.2800
	0.2988	0.3180	0.3375	0.3573	0.3776
	0.3982	0.4191	0.4404	0.4621	0.4841
	0.5065	0.5293	0.5524	0.5758	0.5997
	0.6238	0.6484	0.6733	0.6985	0.7241
	0.7501	0.7764	0.8031	0.8302	0.8576
Hrad:	0.8854	0.9135	0.9420	0.9708	1.0000
	0.0304	0.0607	0.0807	0.1206	0.1588
	0.1944	0.2275	0.2586	0.2880	0.3160
	0.3427	0.3683	0.3929	0.4166	0.4396
	0.4618	0.4834	0.5044	0.5248	0.5447
	0.5641	0.5831	0.6017	0.6198	0.6376
	0.6550	0.6721	0.6888	0.7053	0.7214
	0.7373	0.7529	0.7683	0.7835	0.7984
	0.8131	0.8276	0.8418	0.8559	0.8698
	0.8835	0.8971	0.9105	0.9237	0.9368
Width:	0.9497	0.9625	0.9751	0.9876	1.0000
	0.0756	0.1511	0.4264	0.4386	0.4508
	0.4630	0.4752	0.4874	0.4996	0.5118
	0.5240	0.5362	0.5484	0.5606	0.5728
	0.5850	0.5972	0.6094	0.6216	0.6338
	0.6461	0.6583	0.6705	0.6827	0.6949
	0.7071	0.7193	0.7315	0.7437	0.7559
	0.7681	0.7803	0.7925	0.8047	0.8169
	0.8291	0.8413	0.8535	0.8657	0.8779
	0.8902	0.9024	0.9146	0.9268	0.9390
	0.9512	0.9634	0.9756	0.9878	1.0000

*****	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	34.283	96.000
Evaporation Loss	0.000	0.000
Infiltration Loss	10.062	28.177
Surface Runoff	23.172	64.887
Final Surface Storage	1.086	3.042
Continuity Error (%)	-0.110	

*****	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	23.171	231.715
Groundwater Inflow	0.000	0.000
RDI Inflow	0.000	0.000
External Inflow	272.598	2726.004
External Outflow	293.093	2930.960
Internal Outflow	0.000	0.000
Storage Losses	0.000	0.000
Initial Stored Volume	10.473	104.728
Final Stored Volume	13.177	131.775
Continuity Error (%)	-0.009	

Highest Continuity Errors

Node 514 (1.36%)
Node 116 (1.08%)

Time-Step Critical Elements

Link CC104 (73.35%)

Highest Flow Instability Indexes

Link FutureComm-IC (12)
Link Stub-IC (11)
Link FUTURE_TRANSITWAY-IC (7)
Link 117-IC (6)
Link 0640 (5)

Routing Time Step Summary

Minimum Time Step : 0.50 sec
Average Time Step : 1.44 sec
Maximum Time Step : 2.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 2.01

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
112-115	96.00	0.00	0.00	16.95	77.89	0.21	0.08	0.811
114-112	96.00	0.00	0.00	16.88	77.88	0.40	0.16	0.811
114-118	96.00	0.00	0.00	16.91	77.89	0.37	0.14	0.811
115-110	96.00	0.00	0.00	48.06	46.78	0.08	0.02	0.487
116-115	96.00	0.00	0.00	16.91	77.89	0.37	0.14	0.811
120-111	96.00	0.00	0.00	13.50	81.24	0.45	0.17	0.846
122-120	96.00	0.00	0.00	13.51	81.26	0.22	0.09	0.846
124-120	96.00	0.00	0.00	17.03	77.84	0.25	0.09	0.811
124-126	96.00	0.00	0.00	16.99	77.87	0.24	0.09	0.811
126-128	96.00	0.00	0.00	16.99	77.87	0.52	0.20	0.811
131-129	96.00	0.00	0.00	16.98	77.88	0.43	0.16	0.811
132-130	96.00	0.00	0.00	17.11	77.77	0.43	0.15	0.810
134-132	96.00	0.00	0.00	16.99	77.87	0.60	0.23	0.811
136-134	96.00	0.00	0.00	16.99	77.87	0.40	0.15	0.811
138-140	96.00	0.00	0.00	16.98	77.88	0.26	0.10	0.811
140-122	96.00	0.00	0.00	16.98	77.88	0.58	0.22	0.811
140-142	96.00	0.00	0.00	16.96	77.89	0.50	0.19	0.811
142-138	96.00	0.00	0.00	16.98	77.88	0.67	0.25	0.811
144-140	96.00	0.00	0.00	16.99	77.87	0.31	0.12	0.811
144-146	96.00	0.00	0.00	16.98	77.88	0.69	0.26	0.811
146-122	96.00	0.00	0.00	13.53	81.28	0.20	0.08	0.847
154-512	96.00	0.00	0.00	17.13	77.74	0.68	0.24	0.810
157-158	96.00	0.00	0.00	17.13	77.74	0.58	0.21	0.810
158-160	96.00	0.00	0.00	17.20	77.67	0.43	0.15	0.809
170-156	96.00	0.00	0.00	17.03	77.84	0.28	0.10	0.811
172-160	96.00	0.00	0.00	17.15	77.72	0.58	0.21	0.810

Minto Communities Inc
Appendix F

Arcadia Residential Stages 3, 4, 5, & 6 and Commercial Stages 2 & 3
Stormwater Management Serving Report

173-174	96.00	0.00	0.00	17.05	77.83	0.28	0.10	0.811
512A-OUTLET	96.00	0.00	0.00	36.64	58.16	0.19	0.05	0.606
513-512A	96.00	0.00	0.00	27.83	67.00	2.00	0.58	0.698
513-512A_PARK	96.00	0.00	0.00	36.66	58.15	0.57	0.15	0.606
514-513	96.00	0.00	0.00	17.13	77.74	0.26	0.09	0.810
605-OUTLET	96.00	0.00	0.00	17.10	77.77	0.64	0.23	0.810
620-610_EAST	96.00	0.00	0.00	42.26	52.53	0.99	0.20	0.547
620-610_WEST	96.00	0.00	0.00	17.42	77.39	1.59	0.52	0.806
630-620	96.00	0.00	0.00	17.12	77.76	0.51	0.19	0.810
640-630	96.00	0.00	0.00	17.44	77.36	1.99	0.65	0.806
CAMPEAU_DRIVE_1	96.00	426.00	0.00	3.70	517.58	10.08	2.22	0.992
COMMERCIAL_STAGE_1	96.00	68.33	0.00	15.32	148.02	8.30	2.17	0.901
COMMERCIAL_STAGE_3_SOUTH_2	96.00	0.00	0.00	3.05	92.00	1.42	0.48	0.958
COMMERCIAL_STAGE_3_WEST_2	96.00	0.00	0.00	3.48	91.68	0.67	0.24	0.955
CS063_1	96.00	0.00	0.00	27.87	60.51	56.17	1.64	0.630
CS063_2	96.00	0.00	0.00	56.62	38.17	11.74	1.21	0.398
CS073	96.00	0.00	0.00	39.35	55.43	2.97	0.22	0.577
CS077_2	96.00	0.00	0.00	6.58	88.42	10.87	3.64	0.921
CS082_1	96.00	0.00	0.00	9.62	84.17	31.08	7.42	0.877
CS082_2	96.00	0.00	0.00	10.85	84.12	7.43	2.14	0.876
CS098	96.00	0.00	0.00	39.89	54.96	49.72	10.67	0.572
CS267_4	96.00	0.00	0.00	15.43	79.44	1.21	0.29	0.827
CS267_5	96.00	0.00	0.00	15.19	79.73	0.84	0.22	0.830
CS267_7	96.00	0.00	0.00	16.52	78.36	1.34	0.32	0.816
EXISTING_STAGE_1	96.00	110.99	0.00	16.86	189.03	17.17	2.85	0.913
EXISTING_STAGE_2	96.00	145.38	0.00	19.38	220.92	27.91	4.44	0.915
FUTURE_TRANSITWAY	96.00	0.00	0.00	14.15	80.61	1.79	0.58	0.840
FUTURE_TRANSITWAY_2	96.00	0.00	0.00	3.40	91.77	0.53	0.19	0.956
FutureCamp_117	96.00	0.00	0.00	13.52	81.29	1.16	0.45	0.847
FutureCampEast_117	96.00	0.00	0.00	13.95	80.83	0.81	0.27	0.842
FutureComm-CAMPEAU_DRIVE_POND	96.00	0.00	0.00	0.00	7.02	87.61	3.67	1.08
Pond_1	96.00	0.00	0.00	31.45	63.38	1.09	0.34	0.660
Pond_2	96.00	0.00	0.00	31.37	63.46	1.11	0.35	0.661
STAGE_2	96.00	0.00	0.00	35.55	59.31	0.28	0.12	0.618
STAGE_2_ROAD	96.00	0.00	0.00	35.31	59.59	0.07	0.03	0.621
Stub-110	96.00	0.00	0.00	6.95	87.72	3.12	1.00	0.914

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min
CJ065	JUNCTION	2.03	0.34	93.51	0 05:55
CJ066	JUNCTION	2.01	2.50	93.51	0 13:25
CJ067	JUNCTION	1.95	2.43	93.51	0 13:25
CJ068	JUNCTION	1.98	2.46	93.52	0 13:25
CJ069	JUNCTION	1.97	2.46	93.52	0 13:25
CJ070	JUNCTION	1.93	2.42	93.52	0 13:25
CJ071	JUNCTION	1.88	2.37	93.52	0 13:25
CJ072	JUNCTION	1.89	2.38	93.52	0 13:21
CJ073	JUNCTION	1.86	2.35	93.53	0 13:21
CJ074	JUNCTION	1.84	2.33	93.53	0 13:21
CJ075	JUNCTION	1.83	2.33	93.54	0 13:21
CJ076	JUNCTION	1.83	2.33	93.54	0 13:21
CJ077	JUNCTION	1.90	3.97	95.20	0 06:01
CJ078	JUNCTION	1.83	2.32	93.54	0 13:19
CJ079	JUNCTION	1.83	2.33	93.55	0 13:18
CJ080	JUNCTION	1.83	2.32	93.55	0 13:18
CJ081	JUNCTION	1.81	2.31	93.55	0 13:18
CJ082	JUNCTION	0.86	7.53	99.80	0 05:50
CJ083	JUNCTION	1.82	2.31	93.55	0 13:17
CJ084	JUNCTION	1.81	2.30	93.55	0 13:17
CJ089	JUNCTION	1.83	2.32	93.57	0 13:11
CJ094	JUNCTION	0.59	0.90	93.64	0 12:20
CJ095	JUNCTION	0.48	0.79	93.65	0 12:12
CJ096	JUNCTION	0.25	0.48	93.63	0 12:01
CJ098	JUNCTION	1.77	0.93	95.17	0 10:05
CJ100	JUNCTION	1.79	2.28	93.58	0 13:08
CJ101	JUNCTION	1.80	2.34	93.66	0 12:44
CJ103	JUNCTION	1.82	2.43	93.79	0 12:20
CJ104	JUNCTION	1.80	2.34	93.66	0 12:44
CJ105	JUNCTION	1.79	2.34	93.69	0 12:40
CJ106	JUNCTION	1.79	2.35	93.70	0 12:39
CJ266	JUNCTION	0.74	1.22	93.52	0 13:21
J10	JUNCTION	0.34	1.09	93.89	0 08:24
J11	JUNCTION	0.40	0.88	93.78	0 08:05
CJ064	OUTFALL	2.08	2.56	93.51	0 13:25
OF1	OUTFALL	0.00	0.00	0.00	0 00:00
110	STORAGE	0.83	1.38	94.04	0 06:07
111	STORAGE	0.23	0.84	94.15	0 06:07
112	STORAGE	0.38	1.17	94.30	0 06:08
112-S	STORAGE	0.00	0.42	96.92	0 06:00
113	STORAGE	1.12	1.49	93.85	0 06:08
114	STORAGE	0.12	0.61	94.36	0 06:08
114-S	STORAGE	0.00	0.32	96.97	0 06:00
115	STORAGE	0.46	1.20	94.23	0 06:08
115-S	STORAGE	0.00	0.67	96.52	0 06:04
116	STORAGE	0.04	0.67	94.33	0 06:08
116-S	STORAGE	0.00	0.35	96.95	0 06:00
117	STORAGE	1.14	1.47	93.80	0 07:11
117-S	STORAGE	0.00	0.61	95.51	0 06:00
118	STORAGE	0.02	0.51	94.34	0 06:08
118-S	STORAGE	0.01	0.36	96.96	0 06:00
120	STORAGE	0.11	0.83	94.32	0 06:09
120-S	STORAGE	0.00	0.34	96.64	0 06:00
122	STORAGE	0.06	0.77	94.39	0 06:09
122-S	STORAGE	0.00	0.61	96.41	0 06:00
124	STORAGE	0.02	0.26	94.39	0 06:09
124-S	STORAGE	0.00	0.33	97.13	0 06:00
126	STORAGE	0.05	0.54	94.36	0 06:08
126-S	STORAGE	0.00	0.36	97.06	0 06:00
128	STORAGE	0.30	1.13	94.35	0 06:07
128-S	STORAGE	0.00	0.44	96.99	0 06:00
129	STORAGE	0.27	1.15	94.41	0 06:08
130	STORAGE	0.12	1.08	94.56	0 06:08
130-S	STORAGE	0.00	0.42	97.12	0 06:00
131	STORAGE	0.19	1.13	94.50	0 06:08
131-S	STORAGE	0.00	0.42	97.07	0 06:00
132	STORAGE	0.04	0.85	94.75	0 06:08
132-S	STORAGE	0.00	0.35	97.15	0 06:00
134	STORAGE	0.03	0.80	94.89	0 06:08
134-S	STORAGE	0.00	0.34	97.24	0 06:00
136	STORAGE	0.02	0.62	94.92	0 06:09
136-S	STORAGE	0.00	0.33	97.30	0 06:00
138	STORAGE	0.02	0.30	94.61	0 06:09
138-S	STORAGE	0.00	0.33	97.30	0 06:00
140	STORAGE	0.03	0.53	94.56	0 06:09
140-S	STORAGE	0.01	0.40	97.50	0 06:00
142	STORAGE	0.03	0.74	94.56	0 06:08
142-S	STORAGE	0.00	0.42	97.32	0 06:00
144	STORAGE	0.03	0.41	94.67	0 05:52
144-S	STORAGE	0.00	0.35	97.55	0 06:00
146	STORAGE	0.03	0.50	94.46	0 06:09
146-S	STORAGE	0.00	0.31	96.81	0 06:00
154	STORAGE	0.22	0.82	94.10	0 06:10
156	STORAGE	0.02	0.64	94.48	0 06:09
157	STORAGE	0.05	0.88	94.46	0 06:10
158	STORAGE	0.19	0.96	94.28	0 06:09
160	STORAGE	0.30	1.04	94.18	0 06:09
170	STORAGE	0.02	0.60	94.57	0 06:09
172	STORAGE	0.03	0.73	94.34	0 06:09
173	STORAGE	0.02	0.46	94.23	0 06:09
174	STORAGE	0.03	0.51	94.13	0 06:09
512	STORAGE	0.58	1.12	93.99	0 06:10
512A	STORAGE	0.57	1.09	93.97	0 06:22
513	STORAGE	0.28	1.26	94.47	0 06:00
514	STORAGE	0.02	0.70	94.64	0 06:01
605	STORAGE	0.58	1.07	93.94	0 06:23
610	STORAGE	0.54	1.04	93.96	0 06:23
620	STORAGE	0.33	1.28	94.43	0 06:00

630	STORAGE	0.11	1.17	94.63	0	06:01
640	STORAGE	0.04	1.12	94.77	0	06:01
CAMPEAU_DRIVE_POND	STORAGE	1.28	1.00	93.80	0	07:08
CSto097	STORAGE	1.97	1.10	95.17	0	10:06
EX_POND	STORAGE	0.74	1.24	93.94	0	06:22
FUTURE_TRANSITWAY-S	STORAGE	0.00	0.01	95.51	0	06:00
FutureCamp	STORAGE	0.98	1.57	94.07	0	06:03
FutureCamp-S	STORAGE	0.00	0.11	94.41	0	06:00
FutureComm	STORAGE	0.70	1.01	93.80	0	07:07
FutureComm-S	STORAGE	0.00	0.02	95.52	0	06:03
NEW_POND	STORAGE	0.74	1.23	93.93	0	06:23
St154	STORAGE	0.00	0.31	96.61	0	06:01
St157	STORAGE	0.00	0.31	97.01	0	06:01
St158	STORAGE	0.00	0.28	96.98	0	06:02
St170	STORAGE	0.00	0.31	97.31	0	06:00
St172	STORAGE	0.00	0.29	96.69	0	06:02
St173	STORAGE	0.00	0.31	96.71	0	06:00
St512A	STORAGE	0.00	0.01	96.51	0	06:02
St513	STORAGE	0.00	0.01	96.31	0	06:00
St514	STORAGE	0.00	0.30	96.60	0	06:02
St605	STORAGE	0.00	0.22	96.67	0	06:00
St620	STORAGE	0.00	0.01	96.36	0	06:00
St630	STORAGE	0.00	0.30	96.75	0	06:01
St640	STORAGE	0.00	0.14	96.59	0	06:02
Stub	STORAGE	0.78	1.44	94.15	0	06:07
Stub-S	STORAGE	0.00	0.22	96.37	0	06:03

Node Inflow Summary

Node	Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr
111-S	JUNCTION	0.000	0.121	0 06:00	0.000	0.092
CJ065	JUNCTION	2.866	44.046	0 12:02	69.237	2904.054
CJ066	JUNCTION	0.000	42.863	0 12:02	0.000	2835.138
CJ067	JUNCTION	0.000	43.107	0 12:02	0.000	2836.445
CJ068	JUNCTION	0.000	43.316	0 11:58	0.000	2837.507
CJ069	JUNCTION	0.000	43.342	0 11:53	0.000	2837.500
CJ070	JUNCTION	0.000	43.340	0 11:43	0.000	2837.889
CJ071	JUNCTION	0.000	43.480	0 11:43	0.000	2838.223
CJ072	JUNCTION	0.000	43.668	0 11:14	0.000	2838.884
CJ073	JUNCTION	0.000	43.649	0 10:59	0.000	2803.888
CJ074	JUNCTION	0.000	42.060	0 10:59	0.000	2759.741
CJ075	JUNCTION	0.000	43.143	0 10:54	0.000	2760.252
CJ076	JUNCTION	0.000	43.372	0 10:50	0.000	2760.858
CJ077	JUNCTION	3.637	13.199	0 06:00	10.871	49.390
CJ078	JUNCTION	9.563	0.563	0 10:45	0.000	2761.874
CJ079	JUNCTION	0.000	44.284	0 10:44	0.000	2762.729
CJ080	JUNCTION	0.000	44.374	0 10:41	0.000	2762.801
CJ081	JUNCTION	0.000	44.502	0 10:41	0.000	2762.918
CJ082	JUNCTION	9.563	0.563	0 06:00	38.511	38.516
CJ083	JUNCTION	0.219	44.587	0 10:41	2.975	2762.903
CJ084	JUNCTION	0.000	44.583	0 10:41	0.000	2759.954
CJ089	JUNCTION	0.000	44.819	0 10:40	0.000	2760.816
CJ094	JUNCTION	0.000	0.934	0 10:06	0.000	44.558
CJ095	JUNCTION	0.000	0.834	0 10:06	0.000	44.569
CJ096	JUNCTION	0.000	0.834	0 10:05	0.000	44.570
CJ098	JUNCTION	10.673	10.673	0 06:00	49.719	49.717
CJ100	JUNCTION	0.000	44.576	0 10:40	0.000	2712.571
CJ101	JUNCTION	0.000	44.588	0 10:40	0.000	2712.575
CJ103	JUNCTION	44.637	44.637	0 10:35	2697.907	2697.906
CJ104	JUNCTION	0.000	44.660	0 10:38	0.000	2712.794
CJ105	JUNCTION	0.000	44.568	0 10:38	0.000	2697.566
CJ106	JUNCTION	0.000	44.635	0 10:35	0.000	2697.902
CJ266	JUNCTION	0.000	4.203	0 06:24	0.000	35.835
J10	JUNCTION	0.000	4.283	0 06:19	0.000	35.739
J11	JUNCTION	0.000	0.596	0 06:41	0.000	15.733
CJ064	OUTFALL	0.000	44.018	0 12:02	0.000	2903.895
OF1	OUTFALL	4.441	4.441	0 06:00	27.906	27.905
110	STORAGE	0.000	2.451	0 06:06	0.000	11.193
111	STORAGE	0.000	0.549	0 06:01	0.000	2.655
112	STORAGE	0.000	1.016	0 06:06	0.000	4.080
112-S	STORAGE	0.081	0.789	0 06:00	0.210	0.500
113	STORAGE	0.000	2.451	0 06:06	0.000	11.190
114	STORAGE	0.000	0.006	0 05:58	0.000	0.447
114-S	STORAGE	0.157	0.157	0 06:00	0.397	0.397
115	STORAGE	0.000	1.206	0 06:06	0.000	5.412
115-S	STORAGE	0.025	0.926	0 06:00	0.084	0.517
116	STORAGE	0.000	0.158	0 06:09	0.000	0.822
116-S	STORAGE	0.143	0.262	0 06:00	0.366	0.485
117	STORAGE	0.000	3.031	0 06:00	0.000	13.162
117-S	STORAGE	0.268	0.268	0 06:00	0.808	0.808
118	STORAGE	0.000	0.087	0 05:58	0.000	0.477
118-S	STORAGE	0.143	0.143	0 06:00	0.366	0.366
120	STORAGE	0.000	0.565	0 06:00	0.000	2.653
120-S	STORAGE	0.175	0.175	0 05:59	0.447	0.447
122	STORAGE	0.000	0.492	0 05:59	0.000	2.094
122-S	STORAGE	0.086	0.114	0 06:00	0.219	0.238
124	STORAGE	0.000	0.094	0 05:46	0.000	0.448
124-S	STORAGE	0.184	0.184	0 06:00	0.491	0.491
126	STORAGE	0.000	0.120	0 06:10	0.000	0.763
126-S	STORAGE	0.198	0.297	0 06:00	0.565	0.565
128	STORAGE	0.000	0.896	0 06:06	0.000	3.750
128-S	STORAGE	0.000	0.815	0 06:00	0.000	0.418
129	STORAGE	0.000	0.734	0 06:08	0.000	2.916
130	STORAGE	0.000	0.638	0 05:53	0.000	2.480
130-S	STORAGE	0.000	0.780	0 06:00	0.000	0.475
131	STORAGE	0.000	0.709	0 05:54	0.000	2.913
131-S	STORAGE	0.163	0.733	0 06:00	0.428	0.701
132	STORAGE	0.000	0.388	0 06:09	0.000	1.410
132-S	STORAGE	0.154	0.280	0 06:00	0.428	0.448
134	STORAGE	0.000	0.261	0 06:09	0.000	0.981
134-S	STORAGE	0.227	0.297	0 06:00	0.600	0.611
136	STORAGE	0.000	0.102	0 05:48	0.000	0.391
136-S	STORAGE	0.150	0.179	0 06:00	0.397	0.402
138	STORAGE	0.000	0.068	0 05:48	0.000	0.260
138-S	STORAGE	0.101	0.101	0 06:00	0.265	0.265
140	STORAGE	0.000	0.232	0 06:07	0.000	1.100
140-S	STORAGE	0.411	0.613	0 06:00	1.075	1.185
142	STORAGE	0.000	0.205	0 05:51	0.000	0.916
142-S	STORAGE	0.254	0.786	0 06:00	0.670	1.085
144	STORAGE	0.000	0.178	0 05:46	0.000	0.895
144-S	STORAGE	0.382	0.382	0 06:00	1.005	1.005
146	STORAGE	0.000	0.209	0 06:00	0.000	1.052
146-S	STORAGE	0.079	0.079	0 06:00	0.293	0.293
154	STORAGE	0.000	0.246	0 05:58	0.000	0.955
156	STORAGE	0.000	0.072	0 05:49	0.000	0.277
157	STORAGE	0.000	0.219	0 05:57	0.000	0.853
158	STORAGE	0.000	0.329	0 05:57	0.000	1.287
160	STORAGE	0.000	0.472	0 05:56	0.000	1.871
170	STORAGE	0.000	0.072	0 05:48	0.000	0.277
172	STORAGE	0.000	0.150	0 05:49	0.000	0.583
173	STORAGE	0.000	0.072	0 05:48	0.000	0.278
174	STORAGE	0.000	0.712	0 05:59	0.000	0.278
512	STORAGE	0.000	0.710	0 05:56	0.000	2.818
512A	STORAGE	0.000	0.783	0 06:00	0.000	3.026
513	STORAGE	0.147	0.783	0 06:00	0.575	2.831
514	STORAGE	0.000	0.066	0 05:49	0.000	0.257
605	STORAGE	1.514	1.514	0 06:00	0.000	5.713
610	STORAGE	0.000	1.359	0 06:00	0.000	5.099
620	STORAGE	0.199	1.394	0 06:00	0.988	5.092
630	STORAGE	0.000	0.661	0 05:03	0.000	2.497
640	STORAGE	0.000	0.514	0 05:51	0.000	1.909
CAMPEAU_DRIVE_POND	STORAGE	0.000	4.553	0 06:00	0.000	26.841
CSto097	STORAGE	0.000	10.673	0 06:00	0.000	49.699
EX_POND	STORAGE	8.165	9.499	0 06:02	28.322	34.973
FUTURE_TRANSITWAY-S	STORAGE	0.500	0.580	0 05:49	1.789	1.789
FutureCamp	STORAGE	0.000	0.400	0 05:49	0.000	1.167
FutureCamp-S	STORAGE	0.451	0.451	0 06:00	1.163	1.162
FutureComm	STORAGE	0.000	1.417	0 06:00	0.000	5.466

FutureComm-S	STORAGE	1.079	1.079	0	06:00	3.671	3.671
NEW_POND	STORAGE	0.353	6.094	0	06:11	1.110	38.199
St154	STORAGE	0.242	0.258	0	06:00	0.676	0.679
St157	STORAGE	0.206	0.231	0	06:00	0.575	0.578
St158	STORAGE	0.152	0.152	0	06:00	0.435	0.437
St170	STORAGE	0.104	0.104	0	06:00	0.280	0.280
St172	STORAGE	0.208	0.208	0	06:00	0.583	0.583
St173	STORAGE	0.104	0.104	0	06:00	0.280	0.280
St512A	STORAGE	0.049	0.056	0	06:01	0.192	0.193
St513	STORAGE	0.582	0.582	0	06:00	2.003	2.003
St514	STORAGE	0.092	0.092	0	06:00	0.257	0.257
St605	STORAGE	0.230	0.230	0	06:00	0.636	0.636
St620	STORAGE	0.524	0.587	0	06:00	1.594	1.606
St630	STORAGE	0.185	0.185	0	06:00	0.513	0.513
St640	STORAGE	0.649	0.649	0	06:00	1.988	1.988
Stub	STORAGE	0.000	0.712	0	05:52	0.000	3.135
Stub-S	STORAGE	0.995	0.995	0	06:00	3.123	3.123

Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
CJ082	JUNCTION	0.37	5.282	2.468
CJ088	JUNCTION	5.86	0.125	1.225
CJ266	JUNCTION	10.88	0.223	2.477
110	STORAGE	0.13	0.017	2.059
111	STORAGE	0.10	0.012	2.001
112	STORAGE	0.23	0.108	2.201
115	STORAGE	0.30	0.138	2.216
116	STORAGE	0.16	0.065	2.274
118	STORAGE	0.16	0.062	2.260
128	STORAGE	0.16	0.069	2.200
129	STORAGE	0.20	0.099	2.192
130	STORAGE	0.18	0.097	2.142
131	STORAGE	0.17	0.079	2.150
132	STORAGE	0.22	0.174	2.050
134	STORAGE	0.15	0.127	2.010
136	STORAGE	0.04	0.017	2.052
154	STORAGE	0.24	0.143	2.202
156	STORAGE	0.16	0.265	2.520
157	STORAGE	0.16	0.271	2.244
158	STORAGE	0.20	0.276	2.424
160	STORAGE	0.67	0.289	2.216
170	STORAGE	0.13	0.221	2.434
172	STORAGE	0.12	0.210	2.055
173	STORAGE	0.05	0.083	2.172
174	STORAGE	0.09	0.131	2.274
512	STORAGE	1.04	0.217	2.308
512A	STORAGE	2.52	0.269	2.526
513	STORAGE	0.20	0.436	2.429
514	STORAGE	0.14	0.329	1.656
605	STORAGE	0.26	0.025	2.505
620	STORAGE	0.19	0.231	1.919
630	STORAGE	0.20	0.266	1.619
640	STORAGE	0.22	0.298	1.677
FutureComm-S	STORAGE	48.00	0.023	0.477
Stub	STORAGE	4.43	0.393	0.647
Stub-S	STORAGE	48.00	0.225	0.275

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	E&I Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow CMS
110	0.001	24	0	0.002	40	0 06:07	2.451
111	0.000	8	0	0.001	29	0 06:07	0.505
112	0.000	11	0	0.001	35	0 06:08	1.013
112-S	0.000	0	0	0.016	85	0 06:00	0.789
113	0.001	31	0	0.002	41	0 06:08	2.450
114	0.000	4	0	0.001	21	0 06:08	0.137
114-S	0.000	0	0	0.023	64	0 06:00	0.156
115	0.001	14	0	0.001	35	0 06:08	1.204
115-S	0.021	1	0	0.425	15	0 06:04	0.036
116	0.000	1	0	0.001	23	0 06:08	0.167
116-S	0.000	1	0	0.018	70	0 06:00	0.262
117	0.001	31	0	0.002	40	0 07:11	3.022
117-S	0.000	0	0	0.001	1	0 06:00	0.268
118	0.000	1	0	0.001	18	0 06:00	0.087
118-S	0.000	1	0	0.006	72	0 06:00	0.143
120	0.000	4	0	0.001	30	0 06:09	0.549
120-S	0.000	1	0	0.013	67	0 06:00	0.175
122	0.000	2	0	0.001	28	0 06:09	0.465
122-S	0.000	0	0	0.001	2	0 06:00	0.114
124	0.000	1	0	0.000	10	0 06:09	0.104
124-S	0.000	0	0	0.022	66	0 06:00	0.184
126	0.000	2	0	0.001	19	0 06:08	0.178
126-S	0.000	1	0	0.016	71	0 06:00	0.287
128	0.000	9	0	0.001	34	0 06:07	0.894
128-S	0.000	0	0	0.042	87	0 06:00	0.810
129	0.000	8	0	0.001	34	0 06:08	0.751
130	0.000	4	0	0.001	33	0 06:08	0.612
130-S	0.000	0	0	0.052	84	0 06:00	0.758
131	0.000	6	0	0.001	34	0 06:08	0.734
131-S	0.000	0	0	0.033	83	0 06:00	0.727
132	0.000	1	0	0.001	29	0 06:08	0.404
132-S	0.000	0	0	0.027	70	0 06:00	0.265
134	0.000	1	0	0.001	29	0 06:08	0.278
134-S	0.000	0	0	0.037	68	0 06:00	0.286
136	0.000	1	0	0.001	23	0 06:09	0.107
136-S	0.000	0	0	0.024	65	0 06:00	0.175
138	0.000	1	0	0.000	11	0 06:09	0.069
138-S	0.000	0	0	0.016	66	0 06:00	0.097
140	0.000	1	0	0.001	17	0 06:09	0.250
140-S	0.000	1	0	0.023	80	0 06:00	0.612
142	0.000	1	0	0.001	24	0 06:08	0.268
142-S	0.000	1	0	0.038	83	0 06:00	0.785
144	0.000	1	0	0.000	14	0 05:52	0.174
144-S	0.000	0	0	0.045	71	0 06:00	0.380
146	0.000	1	0	0.001	20	0 06:09	0.206
146-S	0.000	0	0	0.011	63	0 06:00	0.079
154	0.000	7	0	0.001	27	0 06:10	0.240
156	0.000	1	0	0.001	20	0 06:09	0.071
157	0.000	1	0	0.001	28	0 06:10	0.217
158	0.000	6	0	0.001	28	0 06:09	0.324
160	0.000	10	0	0.001	32	0 06:09	0.468
170	0.000	1	0	0.001	20	0 06:09	0.072
172	0.000	1	0	0.001	26	0 06:09	0.149
173	0.000	1	0	0.001	17	0 06:09	0.072
174	0.000	1	0	0.001	18	0 06:09	0.077
512	0.001	17	0	0.001	33	0 06:18	0.691
512A	0.001	16	0	0.001	30	0 06:22	0.756
513	0.000	9	0	0.001	41	0 06:00	0.736
514	0.000	1	0	0.001	30	0 06:01	0.084
605	0.001	16	0	0.001	30	0 06:23	1.503
610	0.001	16	0	0.001	31	0 06:23	1.350
620	0.000	10	0	0.001	40	0 06:00	1.359
630	0.000	4	0	0.001	39	0 06:01	0.698

640	0.000	2	0	0.001	40	0	06:01	0.529
CAMPEAU_DRIVE_POND	15.716	41	0	20.120	52	0	07:08	0.596
CSt0097	14.909	26	0	28.775	51	0	10:06	0.834
EX_POND	6.864	51	0	12.613	94	0	06:22	5.332
FUTURE_TRANSITWAY-S	0.000	0	0	0.003	2	0	06:00	0.579
FutureCamp	0.001	43	0	0.002	68	0	06:03	0.398
FutureCamp-S	0.000	0	0	0.023	23	0	06:00	0.400
FutureComm	0.001	22	0	0.001	31	0	07:07	1.415
FutureComm-S	0.002	0	0	0.127	5	0	06:03	0.838
NEW_POND	4.832	52	0	8.656	94	0	06:23	4.283
St154	0.000	0	0	0.039	62	0	06:01	0.191
St157	0.000	0	0	0.033	63	0	06:01	0.227
St158	0.000	0	0	0.022	55	0	06:02	0.112
St170	0.000	0	0	0.016	63	0	06:00	0.100
St172	0.000	0	0	0.031	57	0	06:02	0.150
St173	0.000	0	0	0.016	63	0	06:00	0.097
St512A	0.000	0	0	0.000	2	0	06:02	0.056
St513	0.000	0	0	0.004	2	0	06:00	0.581
St514	0.000	0	0	0.014	59	0	06:02	0.066
St605	0.000	0	0	0.026	45	0	06:00	0.227
St620	0.000	0	0	0.004	1	0	06:00	0.585
St630	0.000	0	0	0.029	61	0	06:01	0.142
St640	0.000	0	0	0.053	29	0	06:02	0.514
Stub	0.001	30	0	0.002	60	0	06:07	0.715
Stub-S	0.001	0	0	0.114	45	0	06:03	0.712

Outfall Loading Summary

Outfall Node	Flow Freq. Pcnt.	Avg. Flow CMS	Max. Flow CMS	Total Volume 10^6 ltr
CJ064	100.00	19.670	44.018	2903.895
OF1	99.99	0.174	4.441	27.905
System	99.99	19.844	44.255	2931.800

Link Flow Summary

Link	Type	Maximum Flow CMS	Time of Max Occurrence days hr:min	Maximum Velocity m/sec	Max/ Full Flow	Max/ Full Depth
110-113	CONDUIT	2.451	0 06:06	2.18	1.68	0.95
111-110	CONDUIT	0.585	0 06:13	1.28	1.05	1.00
112-115	CONDUIT	1.013	0 06:06	1.17	0.83	1.00
113-117	CONDUIT	2.450	0 06:05	1.63	0.99	0.84
114-112	CONDUIT	0.075	0 06:08	0.73	0.59	0.95
114-118	CONDUIT	0.064	0 05:58	0.68	0.50	1.00
115-110	CONDUIT	1.204	0 06:06	1.39	1.14	1.00
116-115	CONDUIT	0.167	0 06:09	0.95	0.70	1.00
117-CAMPEAU_DRIVE_POND	CONDUIT	3.922	0 06:00	2.30	1.05	0.90
118-116	CONDUIT	0.097	0 06:09	0.96	0.69	1.00
120-111	CONDUIT	0.549	0 06:01	1.16	0.99	1.00
122-120	CONDUIT	0.465	0 06:00	0.98	0.84	0.97
124-120	CONDUIT	0.052	0 06:06	0.85	0.60	0.82
124-126	CONDUIT	0.058	0 06:10	0.79	0.45	0.69
126-114	CONDUIT	0.072	0 06:09	0.28	0.30	0.95
126-128	CONDUIT	0.114	0 05:51	0.86	0.48	0.94
128-112	CONDUIT	0.894	0 06:06	1.07	0.73	1.00
129-128	CONDUIT	0.751	0 06:08	0.87	0.71	1.00
130-131	CONDUIT	0.612	0 06:10	0.98	0.70	1.00
131-129	CONDUIT	0.734	0 06:08	0.95	0.70	1.00
132-130	CONDUIT	0.404	0 06:09	1.13	1.24	1.00
134-132	CONDUIT	0.270	0 06:09	0.79	0.95	1.00
136-134	CONDUIT	0.107	0 06:09	0.65	0.45	1.00
138-140	CONDUIT	0.069	0 05:56	0.86	0.54	0.75
140-122	CONDUIT	0.223	0 06:08	0.84	0.94	0.90
140-142	CONDUIT	0.077	0 05:51	0.68	0.18	0.76
142-130	CONDUIT	0.268	0 06:11	0.88	0.48	0.95
144-140	CONDUIT	0.024	0 06:09	0.63	0.19	0.35
144-146	CONDUIT	0.159	0 06:00	0.99	0.82	0.78
146-122	CONDUIT	0.200	0 05:51	1.21	0.96	0.87
154-512	CONDUIT	0.245	0 05:53	1.04	0.75	1.00
156-157	CONDUIT	0.071	0 05:57	0.97	0.84	1.00
157-158	CONDUIT	0.217	0 06:10	0.89	0.91	1.00
158-160	CONDUIT	0.325	0 05:56	0.97	1.02	1.00
160-512	CONDUIT	0.468	0 05:56	1.18	1.10	1.00
170-156	CONDUIT	0.072	0 05:49	0.89	0.81	1.00
172-160	CONDUIT	0.149	0 05:50	0.90	0.77	1.00
173-174	CONDUIT	0.072	0 05:59	0.89	0.82	1.00
174-154	CONDUIT	0.077	0 06:10	1.06	0.86	1.00
512A-EX_POND	CONDUIT	0.756	0 06:01	1.41	0.92	1.00
512-EX_POND	CONDUIT	0.691	0 05:55	1.33	0.62	1.00
513-512A	CONDUIT	0.736	0 06:00	1.38	1.31	1.00
514-513	CONDUIT	0.065	0 06:05	0.86	0.95	1.00
605-NEW_POND	CONDUIT	1.503	0 06:00	2.20	1.41	1.00
610-605	CONDUIT	1.350	0 06:00	1.65	1.31	0.99
620-610	CONDUIT	1.359	0 06:00	1.57	1.27	0.99
630-620	CONDUIT	0.603	0 06:03	1.10	1.01	1.00
640-630	CONDUIT	0.529	0 06:03	1.07	0.93	1.00
C12	CONDUIT	4.203	0 06:24	1.11	0.75	0.86
C2	CONDUIT	0.547	0 06:45	0.60	0.63	0.94
C6	CONDUIT	5.332	0 06:11	0.52	2.92	0.78
CC065	CHANNEL	44.018	0 12:02	0.18	0.02	0.46
CC066	CHANNEL	42.806	0 12:02	0.14	0.03	0.50
CC067	CHANNEL	42.863	0 12:02	0.14	0.03	0.44
CC068	CHANNEL	43.107	0 12:02	0.13	0.11	0.50
CC069	CHANNEL	43.316	0 11:58	0.16	0.09	0.50
CC070	CHANNEL	43.342	0 11:53	0.27	0.02	0.39
CC071	CHANNEL	43.410	0 11:43	0.15	0.04	0.49
CC072	CHANNEL	43.480	0 11:43	0.17	0.03	0.38
CC073	CHANNEL	43.368	0 11:19	0.20	0.09	0.49
CC074	CHANNEL	42.838	0 11:04	0.17	0.03	0.32
CC075	CHANNEL	42.960	0 10:59	0.21	0.02	0.30
CC076	CHANNEL	43.143	0 10:54	0.14	0.60	0.49
CC077	CHANNEL	11.893	0 06:01	1.77	0.35	0.36
CC078	CHANNEL	43.372	0 10:50	0.17	0.07	0.49
CC079	CHANNEL	43.848	0 10:45	0.15	0.82	0.49
CC080	CONDUIT	9.563	0 06:00	2.40	0.91	1.00
CC080_1	CHANNEL	44.284	0 10:44	0.21	0.04	0.49
CC081	CHANNEL	44.374	0 10:41	0.37	0.04	0.49
CC083	CHANNEL	44.502	0 10:41	1.01	0.24	0.43
CC084	CHANNEL	44.527	0 10:41	0.74	0.03	0.43
CC089	CHANNEL	44.583	0 10:41	1.23	0.72	0.49
CC094	CHANNEL	0.833	0 10:07	0.69	0.02	0.13
CC095	CONDUIT	0.834	0 10:06	1.03	0.46	0.70
CC096	CONDUIT	0.834	0 10:06	1.43	0.28	0.53
CC098	CONDUIT	10.673	0 06:00	5.04	1.50	1.00
CC100	CHANNEL	44.372	0 10:41	0.24	0.13	0.49
CC101	CONDUIT	44.578	0 10:40	1.44	0.14	0.52
CC102	CONDUIT	44.635	0 10:35	1.44	0.47	0.65
CC104	CHANNEL	44.588	0 10:40	0.24	0.34	0.50
CC105	CHANNEL	44.444	0 10:40	0.46	0.19	0.50
CC106	CHANNEL	44.568	0 10:38	0.18	0.72	0.50
CC266	CONDUIT	4.200	0 06:26	1.11	0.22	1.00
FutureCamp-117	CONDUIT	0.398	0 06:03	1.12	1.41	1.00
FutureComm-CAMPEAU_DRIVE_POND	CONDUIT	1.415	0 06:00	2.05	1.31	0.84
Stub-110	CONDUIT	0.715	0 06:09	1.03	0.96	1.00
ODP_Low	ORIFICE	0.189	0 06:19			1.00
COR097_1	ORIFICE	0.397	0 10:06			1.00
COR097_2	ORIFICE	0.026	0 09:36			1.00
NP_Low	ORIFICE	0.123	0 06:03			1.00
10	WEIR	0.025	0 06:00			0.06
11	WEIR	0.000	0 06:00			0.00
111-120-S	WEIR	0.121	0 06:00			0.19
111-S-POND	WEIR	0.133	0 05:55			0.20
112-128-S	WEIR	0.676	0 06:00			0.68

114-112-S	WEIR	0.054	0	06:00	0.11		
115-112-S	WEIR	0.735	0	06:00	0.62		
115-116-S	WEIR	0.191	0	06:00	0.25		
116-118-S	WEIR	0.120	0	06:00	0.29		
117-S-CAMPEAU_DRIVE_POND	WEIR	0.000	0	00:00	0.00	0.00	
12	WEIR	0.000	0	00:00	0.00		
120-122-S	WEIR	0.000	0	00:00	0.00		
122-146-S	WEIR	0.029	0	06:00	0.07		
126-124-S	WEIR	0.090	0	06:00	0.15		
128-126-S	WEIR	0.225	0	06:00	0.28		
130-142-S	WEIR	0.057	0	06:00	0.58		
131-128-S	WEIR	0.617	0	06:00	0.58		
131-130-S	WEIR	0.586	0	06:00	0.61		
132-130-S	WEIR	0.155	0	06:00	0.24		
132-134-S	WEIR	0.132	0	06:00	0.20		
134-136-S	WEIR	0.532	0	06:00	0.13		
138-136-S	WEIR	0.029	0	06:00	0.14		
14	WEIR	0.000	0	00:00	0.00		
140-144-S	WEIR	0.202	0	06:00	0.26		
142-140-S	WEIR	0.532	0	06:00	0.50		
15	WEIR	0.005	0	06:01	0.02		
16	WEIR	0.005	0	06:01	0.02		
17	WEIR	0.063	0	06:00	0.12		
18	WEIR	0.000	0	06:00	0.00		
5	WEIR	0.028	0	06:00	0.07		
6	WEIR	0.025	0	06:01	0.07		
7	WEIR	0.000	0	00:00	0.00		
8	WEIR	0.000	0	06:00	0.00		
9	WEIR	0.017	0	06:02	0.05		
CDP_High	WEIR	0.554	0	06:41	0.24		
CW097-3	WEIR	0.411	0	10:06	0.45		
FUTURE TRANSITWAY-S-CAMPEAU_DRIVE_POND	WEIR	0.000	0	00:00	0.00	0.00	0.00
FutureCamp-CAMPEAU_DRIVE_POND	WEIR	0.000	0	00:00	0.00	0.00	0.00
NP_High	WEIR	4.251	0	06:19	0.92		
112-IC	DUMMY	0.054	0	05:48			
114-IC	DUMMY	0.102	0	05:48			
115-IC	DUMMY	0.036	0	05:55			
116-IC	DUMMY	0.071	0	05:45			
117-IC	DUMMY	0.268	0	06:00			
118-IC	DUMMY	0.023	0	05:32			
120-IC	DUMMY	0.054	0	05:36			
122-IC	DUMMY	0.114	0	06:00			
124-IC	DUMMY	0.094	0	05:46			
126-IC	DUMMY	0.062	0	05:43			
128-IC	DUMMY	0.134	0	05:53			
130-IC	DUMMY	0.172	0	05:49			
131-IC	DUMMY	0.110	0	05:48			
132-IC	DUMMY	0.110	0	05:49			
134-IC	DUMMY	0.154	0	05:48			
136-IC	DUMMY	0.102	0	05:48			
138-IC	DUMMY	0.068	0	05:48			
140-IC	DUMMY	0.080	0	05:33			
142-IC	DUMMY	0.128	0	05:43			
144-IC	DUMMY	0.178	0	05:46			
146-IC	DUMMY	0.050	0	05:47			
FUTURE TRANSITWAY-IC	DUMMY	0.579	0	06:00			
FutureCamp-Backwater	DUMMY	0.000	0	00:00			
FutureCamp-IC	DUMMY	0.400	0	05:49			
FutureComm-IC	DUMMY	0.838	0	05:54			
0154	DUMMY	0.174	0	05:49			
0157	DUMMY	0.148	0	05:49			
0158	DUMMY	0.112	0	05:49			
0170	DUMMY	0.072	0	05:48			
0172	DUMMY	0.150	0	05:49			
0173	DUMMY	0.077	0	05:48			
0512A	DUMMY	0.056	0	06:02			
0513	DUMMY	0.581	0	06:00			
0514	DUMMY	0.066	0	05:49			
0605	DUMMY	0.164	0	05:49			
0620	DUMMY	0.585	0	06:00			
0630	DUMMY	0.132	0	05:49			
0640	DUMMY	0.514	0	05:51			
Stub-IC	DUMMY	0.712	0	05:52			

Flow Classification Summary

Conduit	Adjusted Actual Length	--- Fraction of Time in Flow Class ---						Avg. Froude Number	Avg. Flow Change
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit		
110-113	1.00	0.01	0.00	0.00	0.32	0.00	0.00	0.07	0.0000
111-110	1.00	0.02	0.03	0.00	0.81	0.00	0.00	0.14	0.0000
112-115	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
113-117	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
114-112	1.00	0.84	0.11	0.00	0.03	0.00	0.00	0.02	0.0000
114-118	1.00	0.01	0.75	0.00	0.24	0.00	0.00	0.06	0.0000
115-110	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
116-115	1.00	0.01	0.00	0.00	0.54	0.00	0.00	0.45	0.0000
117-CAMPEAU_DRIVE_POND	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
118-116	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
120-111	1.00	0.01	0.06	0.00	0.93	0.00	0.00	0.00	0.0000
122-120	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
124-120	1.00	0.71	0.00	0.00	0.01	0.00	0.00	0.29	0.0000
124-126	1.00	0.01	0.00	0.00	0.01	0.00	0.00	0.00	0.0000
126-114	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
126-128	1.00	0.01	0.00	0.00	0.28	0.00	0.00	0.70	0.0000
128-112	1.00	0.01	0.05	0.00	0.94	0.00	0.00	0.00	0.0000
129-128	1.00	0.01	0.02	0.00	0.97	0.00	0.00	0.00	0.0000
130-131	1.00	0.01	0.00	0.00	0.60	0.00	0.00	0.39	0.0000
131-129	1.00	0.01	0.03	0.00	0.96	0.00	0.00	0.00	0.0000
132-130	1.00	0.01	0.00	0.00	0.01	0.00	0.00	0.98	0.0000
134-132	1.00	0.01	0.00	0.00	0.42	0.00	0.00	0.57	0.0000
136-134	1.00	0.01	0.00	0.00	0.02	0.00	0.00	0.97	0.0000
138-140	1.00	0.01	0.00	0.00	0.01	0.00	0.00	0.98	0.0000
140-122	1.00	0.01	0.00	0.00	0.01	0.00	0.00	0.98	0.0000
140-142	1.00	0.75	0.00	0.00	0.03	0.00	0.00	0.22	0.0000
142-130	1.00	0.01	0.00	0.00	0.37	0.00	0.00	0.62	0.0000
144-140	1.00	0.99	0.00	0.00	0.00	0.00	0.00	0.01	0.0000
144-146	1.00	0.01	0.00	0.00	0.01	0.00	0.00	0.98	0.0000
146-122	1.00	0.01	0.00	0.00	0.01	0.00	0.00	0.98	0.0000
154-512	1.00	0.01	0.03	0.00	0.88	0.00	0.00	0.08	0.0000
156-157	1.00	0.01	0.00	0.00	0.02	0.00	0.00	0.97	0.0000
157-158	1.00	0.01	0.00	0.00	0.64	0.00	0.00	0.35	0.0000
158-160	1.00	0.01	0.01	0.00	0.92	0.00	0.00	0.17	0.0000
160-512	1.00	0.01	0.00	0.00	0.91	0.00	0.00	0.08	0.0000
170-156	1.00	0.01	0.06	0.00	0.93	0.00	0.00	0.00	0.0000
172-160	1.00	0.01	0.00	0.00	0.67	0.00	0.00	0.32	0.0000
173-174	1.00	0.01	0.02	0.00	0.97	0.00	0.00	0.17	0.0000
174-154	1.00	0.01	0.00	0.00	0.38	0.00	0.00	0.61	0.0000
512A-EX_POND	1.00	0.00	0.01	0.00	0.99	0.00	0.00	0.00	0.0000
512-EX_POND	1.00	0.00	0.01	0.00	0.99	0.00	0.00	0.00	0.0000
513-512A	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
514-513	1.00	0.01	0.00	0.00	0.08	0.00	0.00	0.91	0.0000
605-NEW_POND	1.00	0.01	0.00	0.00	0.94	0.00	0.00	0.05	0.0000
610-605	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
620-610	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.0000
630-620	1.00	0.01	0.00	0.00	0.75	0.00	0.00	0.24	0.0000
640-630	1.00	0.01	0.00	0.00	0.46	0.00	0.00	0.53	0.0000
C12	1.00	0.00	0.04	0.00	0.96	0.00	0.00	0.00	0.0000
C2	1.00	0.00	0.02	0.00	0.98	0.00	0.00	0.00	0.0000
C6	1.00	0.01	0.00	0.00	0.96	0.00	0.00	0.04	0.0000
CC065	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC066	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC067	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC068	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC069	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC070	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC071	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC072	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC073	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC074	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000
CC075	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0000

CC076	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.02	0.0000
CC077	1.00	0.00	0.00	0.00	0.99	0.00	0.00	0.01	0.02	0.0000
CC078	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.03	0.0000
CC079	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.02	0.0000
CC080	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.02	0.0000
CC080_1	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.04	0.0000
CC081	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.06	0.0000
CC083	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.14	0.0000
CC084	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.11	0.0000
CC089	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.23	0.0000
CC094	1.00	0.01	0.00	0.00	0.77	0.00	0.00	0.23	0.23	0.0000
CC095	1.00	0.00	0.00	0.00	0.97	0.00	0.00	0.02	0.25	0.0000
CC096	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.50	0.0000
CC098	1.00	0.00	0.00	0.00	0.91	0.00	0.00	0.09	0.12	0.0000
CC100	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.03	0.0000
CC101	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.22	0.0000
CC102	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.21	0.0000
CC104	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.03	0.0000
CC105	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.10	0.0000
CC106	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.03	0.0000
CC266	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.02	0.0000
FutureCamp-117	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.0001
FutureComm-CAMPEAU_DRIVE_POND	1.00	0.01	0.01	0.00	0.99	0.00	0.00	0.00	0.08	0.0600
Stub-110	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.01	0.0001

Conduit Surcharge Summary

Conduit	Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
110-113	0.01	0.01	0.01	0.48	0.01
111-110	0.09	0.09	0.10	0.01	0.01
112-115	0.25	0.25	0.25	0.01	0.01
114-118	0.02	0.02	0.02	0.01	0.01
115-110	0.15	0.15	0.15	0.29	0.15
116-115	0.17	0.17	0.17	0.01	0.01
117-CAMPEAU_DRIVE_POND	0.01	0.01	0.01	0.14	0.01
118-116	0.16	0.16	0.16	0.01	0.01
120-111	0.03	0.03	0.03	0.01	0.01
128-112	0.18	0.18	0.18	0.01	0.01
129-128	0.18	0.18	0.18	0.01	0.18
130-131	0.17	0.17	0.17	0.01	0.17
131-129	0.17	0.17	0.17	0.01	0.01
132-130	0.18	0.18	0.18	0.32	0.18
134-132	0.15	0.15	0.15	0.01	0.01
136-134	0.04	0.04	0.04	0.01	0.01
154-512	0.23	0.23	0.24	0.01	0.01
156-157	0.16	0.16	0.16	0.01	0.01
157-158	0.16	0.16	0.16	0.01	0.01
158-160	0.20	0.20	0.20	0.10	0.01
160-512	0.65	0.65	0.65	0.25	0.15
170-156	0.13	0.13	0.13	0.01	0.01
172-160	0.12	0.12	0.12	0.01	0.01
173-174	0.05	0.05	0.05	0.01	0.01
174-154	0.09	0.09	0.09	0.01	0.01
512A-EX_POND	2.49	2.49	2.49	0.01	0.09
512-EX_POND	1.08	1.08	1.08	0.01	0.01
513-512A	0.15	0.15	0.15	0.23	0.12
514-513	0.14	0.14	0.14	0.01	0.03
605-NEW_POND	0.26	0.26	0.26	0.28	0.01
610-605	0.01	0.01	0.01	0.23	0.01
620-610	0.01	0.01	0.01	0.22	0.01
630-620	0.19	0.19	0.19	0.05	0.19
640-630	0.20	0.20	0.20	0.01	0.20
C6	0.01	0.01	0.01	3.57	0.01
CC080	0.37	0.37	0.37	0.01	0.01
CC098	5.78	5.78	5.78	0.27	0.01
CC266	10.88	10.88	10.88	0.01	0.01
FutureCamp-117	34.05	34.05	34.05	0.27	0.29
FutureComm-CAMPEAU_DRIVE_POND	0.01	0.01	0.01	0.01	0.28
Stub-110	4.42	4.42	4.42	0.01	0.39

Analysis begun on: Thu Apr 27 11:00:54 2017
Analysis ended on: Thu Apr 27 11:01:17 2017
Total elapsed time: 00:00:23