

Burnett Lands - 3370 Greenbank Road

**Site Serviceability and Stormwater
Management Report**

BURNETT LANDS
3370 GREENBANK ROAD
SITE SERVICEABILITY AND STORMWATER
MANAGEMENT REPORT

Prepared for:

Claridge Homes

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Novatech File: 111117

Ref: R-2016-170

December 9, 2016

City of Ottawa
Planning, Infrastructure and Economic Development Department
Planning Services Branch
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Ottawa, Ontario
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Attention: **Mr. Don Herweyer, Manager of Development Review South**

Reference: **Burnett Lands - 3370 Greenbank Road**
Site Serviceability and Stormwater Management Report
Novatech File No.: 111117

Enclosed herein are six (6) copies of the "Site Serviceability and Stormwater Management Report" for the proposed development of the Burnett Lands located at 3370 Greenbank Road, Ottawa. The report is submitted in support of applications for Official Plan Amendment, Zoning By-Law Amendment and Draft Plan of Subdivision. It will address how the subject development will be serviced with sanitary sewer, storm sewers, watermain and stormwater management.

Should you have any questions or comments, please do not hesitate to contact us.

Sincerely,

NOVATECH

Greg MacDonald, P.Eng.
Director, Land Development and Public Sector Projects

Encl.



TABLE OF CONTENTS

1.0	INTRODUCTION	1
1.1	Background	1
1.2	Additional Reports	2
2.0	EXISTING CONDITIONS	3
2.1	Topography & Drainage.....	3
2.2	Subsurface Conditions.....	3
3.0	SANITARY SERVICING.....	4
4.0	WATERMAIN	5
5.0	STORMWATER MANAGEMENT CRITERIA.....	7
6.0	BURNETT MUNICIPAL DRAIN	8
7.0	STORMWATER MANAGEMENT DESIGN	9
7.1	Storm Sewer Design (Minor System).....	9
7.2	Inlet Control Devices.....	10
7.3	Overland Flow Path (Major System).....	10
7.4	Street 'B' / Caivan Lands	10
7.5	Water Quality.....	10
8.0	HYDROLOGIC & HYDRAULIC MODELING.....	11
8.1	Design Storms	11
8.2	Model Development.....	11
8.2.1	Storm Drainage Areas	11
8.2.2	Subcatchment Model Parameters	11
8.2.3	Minor System.....	14
8.2.4	Major System.....	15
8.2.5	Modeling Files / Schematic	16
8.3	PCSWMM Model Results	16
8.3.1	Minor System.....	16
8.3.2	Major System.....	16
8.3.3	Hydraulic Grade Line	18
9.0	UTILITIES	20
10.0	EROSION AND SEDIMENT CONTROL	20
11.0	CONCLUSIONS AND RECOMMENDATIONS	21

LIST OF TABLES

- Table 4.1: Water Demand Summary (Pre Watermain Reconfiguration)
Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)
Table 7.1: Storm Sewer Design Parameters
Table 8.1: Hydrologic Modeling Parameters
Table 8.2: ICD Parameters
Table 8.3: Minor System Peak Flows at Outlets
Table 8.4: Ponding Depths at Catchbasins (100yr Event)
Table 8.5: Storm Sewer Hydraulic Grade Line: 5yr-4hr Chicago Distribution, 100-yr WL in Jock River

List of Figures

- Figure 1 Site Location Plan
Figure 2 Concept Plan
Figure 3 Realigned Greenbank Road Watermain
Figure 4 Watermain Layout
Figure 5 Burnett Municipal Drain

Appendices

- Appendix A Design Sheets
Appendix B Sanitary Report Excerpts
Appendix C Watermain Boundary Conditions, FUS Calculations, and Modelling Results
Appendix D SWM Calculations & PCSWMM Model
Appendix E Burnett Municipal Drain Analysis
Appendix F Engineering Drawings

Drawings

- 111117 – GP General Plan of Services (revision 1)
111117 – GR Grading Plan (revision 1)
111117 – SAN Sanitary Drainage Area Plan (revision 1)
111117 – STM Storm Drainage Area Plan (revision 1)

Enclosed CD

- PCSWMM Model Files
PCSWMM Model Output

1.0 INTRODUCTION

Novatech has been retained by Claridge Homes to prepare a Site Serviceability & Stormwater Management Report for the lands located at 3370 Greenbank Road (herein referred to as the “Burnett Lands”). This report has been prepared in support of the application for Official Plan Amendment, Zoning By-Law Amendment, and Draft Plan of Subdivision, and outlines the servicing and proposed storm drainage and stormwater management strategy for the site.

1.1 Background

The subject site is approximately 15.5 hectares in area and is located immediately north of the Jock River, south of Strandherd Drive and between the Kennedy Burnett Stormwater Management Facility and the existing Greenbank Road as shown on **Figure 1**. The Burnett Municipal Drain is tributary to the Jock River and travels through the subject site. The property currently has farm and accessory structures located near its southern boundary with an existing gravel access on to Greenbank Road. The remainder of the site is currently used for passive agriculture activities. The topography is generally flat with a gentle slope from the northeastern corner to the southwestern corner.



Figure 1: Site Location (Base Map Source: GeoOttawa)

The following describes the existing and planned land uses adjacent to the subject site:

North: Lands to the north, owned by Caivan Communities, are currently under the development approval process and have recently obtained OPA and ZBLA approval (Amendment #144) from the City to permit High Rise and Mid Rise Mixed-Use Residential developments, Mid Rise Residential Dwellings, and a Neighbourhood Park as per *Schedule 1- Land Use Plan, South Nepean Secondary Plan (Area 7)*. Further north of the Caivan Communities' development is the planned Barrhaven Town Centre which will include a variety of retail uses to service the surrounding existing and planned residential developments.

East: Lands east of the subject site contain a mixture of low density residential dwellings (single detached houses), a secondary school (St. Joseph Catholic High School), and an existing vegetated area. Greenbank Road currently forms the eastern boundary of the site. The realigned Greenbank Road will bisect the site as per the design by the City.

South: The Jock River flows west – east along the majority of the southern boundary of the property until it turns south near the southeastern corner of the site. The lands south of Jock River are within the *Barrhaven South Community Design Plan* and are intended for a future district park and residential uses as shown on *Figure 17* of the *Barrhaven South Community Design Plan*.

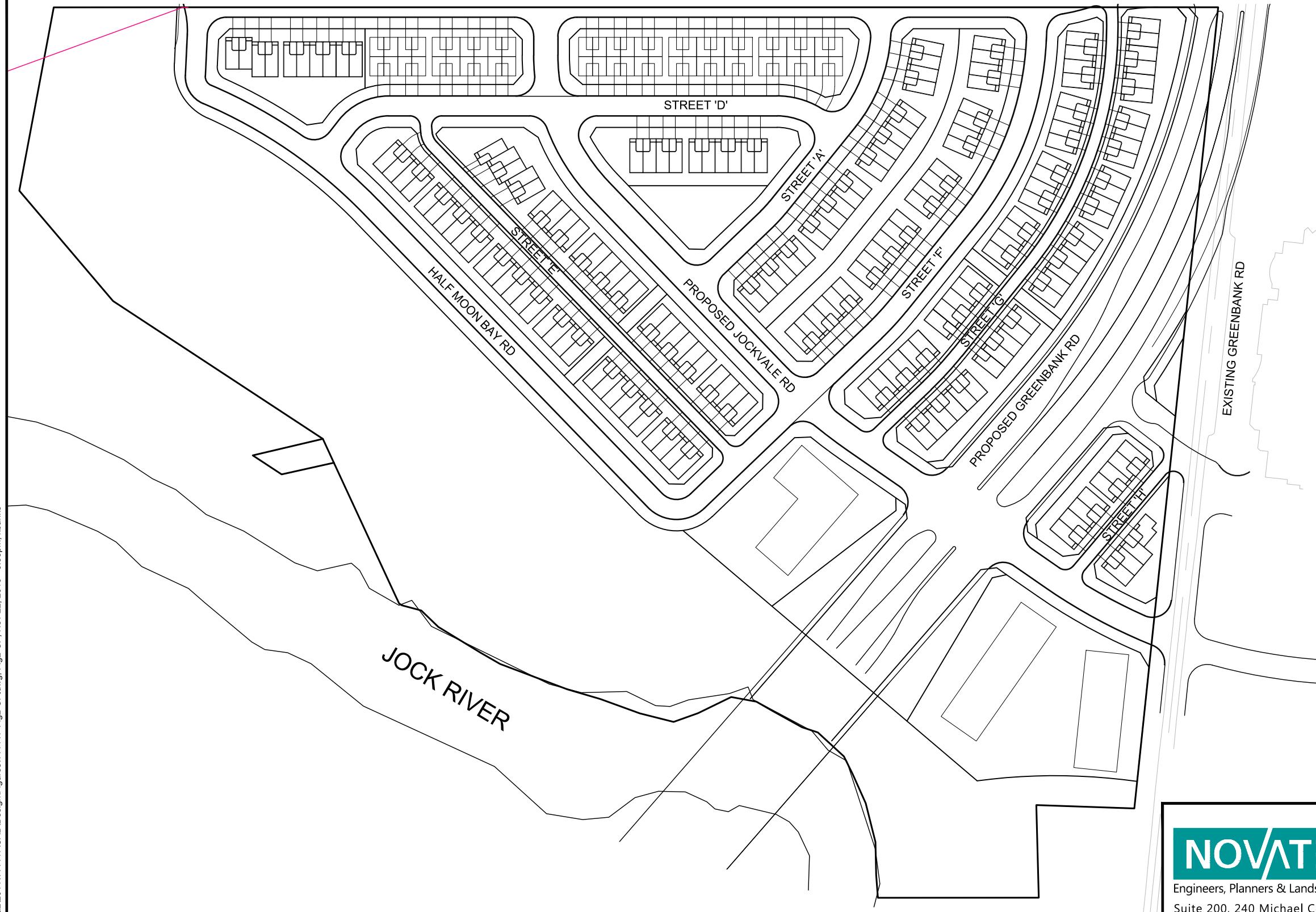
West: The Kennedy Burnett stormwater management facility is located north west of the subject site and drains into the Jock River. Lands immediately west are currently vacant and intended for mostly conservation and residential uses as identified in Schedule B of the Official Plan.

Plans are to develop a residential subdivision, as shown on **Figure 2 – Concept Plan**, which will consist of 247 townhome units and 420 condo units for a total of 667 units.

1.2 Additional Reports

This report provides information on the considerations and approach by which Novatech has designed and evaluated the proposed servicing for the Burnett Lands. This report should be read in conjunction with the following:

- *South Nepean Collector Sewer Alignment & Finalization Report – Phase 2* prepared by Novatech, dated June 2014
- *Hydrology Report – July 2004: Jock River Flood Risk Mapping (within the City of Ottawa)* prepared by the Rideau Valley Conservation Authority, dated July 2004
- *Hydraulics Report – November 2004: Jock River Flood Risk Mapping (within the City of Ottawa)* prepared by the Rideau Valley Conservation Authority, dated November 2004
- *Greenbank (Burnett Municipal Drain) Headwaters Report*, prepared by Bowfin Environmental Consulting and Muncaster Environmental Planning Inc., dated March 2016.
- *Geotechnical Investigation Proposed Residential Development Burnett Lands, Greenbank Road at the Jock River, Ottawa Ontario*, Prepared by Golder Associates Ltd., dated May 2016 (Report No. 1523044-1000)



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**3370 GREENBANK RD.
BURNETT LANDS**

CONCEPT PLAN

SCALE 1 : 2000 0 20 40 60 80

DATE NOV 2016 JOB 111117 FIGURE FIGURE 2

2.0 EXISTING CONDITIONS

2.1 Topography & Drainage

The proposed site is currently undeveloped and consists of agricultural lands. Access to the site is currently provided at Greenbank Road. The site has a gentle slope from north to south, with most overland flow being directed to the existing Burnett Municipal Drain, which flows into the Jock River at the south end of the site boundary. The Burnett Municipal Drain transects the site in a north to south fashion.

2.2 Subsurface Conditions

Golder Associates Ltd. has completed a geotechnical investigation in support of the proposed development. The report is titled "Geotechnical Investigation, Proposed Residential Development, Burnett Lands, Greenbank Road at the Jock River, Ottawa Ontario, dated May 2016 (Report No. 1523044-1000)." The fieldwork for this investigation was carried out between February 18th and 23rd, 2016. The work consisted of advancing nine (9) boreholes to depths ranging from 1.7m to 8.2 m below ground surface. The principal findings of the Geotechnical Investigation are as follows:

- The site was divided into two distinct areas (A and B), with Area 'A' having a thick deposit of silty clay up to 8.2m deep and Area 'B' having a very stiff to stiff layer of silty clay overlying glacial till, or glacial till near the ground surface.
- Area 'A' generally consists of a 1.3 to 3.1 m layer of a silty clay deposit beneath the topsoil layer, which has been weathered to a grey brown crust. Below this is a grey silty clay, which extends to a depth of 3.1 to 8.2 m.
 - In some of the boreholes within Area 'A', glacial till was encountered beneath the silty clay deposit at a depth of 3.1 to 4.3 m.
- Area 'B' generally consists of a 0.6 to 2.5 m layer of a silty clay deposit beneath the topsoil layer, which has been weathered to a grey brown crust. Below this is a silty sand layer which ranges from 0.3 to 0.9 m thick. In all boreholes, glacial till exists below the silty clay and silty sand at depths ranging from 0.3 to 2.5 m.
- Groundwater inflow was observed in some of the boreholes at depths of between 0.92 m and 2.17 m below ground surface.

The report provides engineering guidelines based on Golder Associates interpretation of the borehole information and project requirements. Refer to the above-noted report for complete details.

3.0 SANITARY SERVICING

As per the South Nepean Collector Functional Design Update (FDU) prepared by Dillon Consulting (July 2012), the South Nepean Collector (SNC) is the sanitary outlet for the proposed development and has been sized to accommodate the peak sanitary flows from the proposed Burnett Lands development. Refer to Figure 1, Existing Sanitary Network and Collection Areas, and Table 5.1, Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area, of the FDU, located in **Appendix A**. The noted figure and table confirms the development is located within the Sanitary Drainage Area 8A of the SNC.

The design criteria used to determine the sanitary flows are based on the City of Ottawa's sewer design guidelines and are as follows:

- Residential Average Flow = 350L/capita/day
- Peaking Factor = Harmon Equation (max peaking factor = 4.0)
- Peak Extraneous Flows (Infiltration) = 0.28L/s/ha
- Population Density = 2.7 people/townhouse, 1.8 people/condo
- Minimum Pipe Slope (200mm/250mm) = 0.32% / 0.24%
- Minimum Full Flow Velocity = 0.6m/s
- Maximum Full Flow Velocity = 3.0m/s

In addition to the peak sanitary flows from the proposed Burnett Lands development, the on-site sanitary sewers are sized to accommodate a portion of the peak sanitary flows from the adjacent residential lands located immediately north of the site. The peak sanitary flow from the adjacent lands is based on 100 townhouses per hectare as per the *South Nepean Town Centre Community Design Plan* (July 2006) prepared by the City of Ottawa. For reference a copy of the South Nepean Town Centre Community Design Plan is included in **Appendix B**.

The proposed sanitary sewer system is shown on the General Plan of Services (Drawing **111117-GP**). The Sanitary Drainage Area Plan (Drawing **111117-SAN**) confirms the sanitary drainage areas assumed to outlet in the proposed onsite sanitary sewers. Both drawings are included in **Appendix F**.

The calculated peak sanitary design flow for the development is 39.1L/s: 16.5L/s will outlet into the SNC at the Street D/Jockvale Road intersection and 22.6L/s will outlet into the SNC at the Street F/Jockvale Road intersection. For detailed calculations refer to the Sanitary Sewer Design Sheet located in **Appendix A**.

4.0 WATERMAIN

Ultimately, the Burnett Lands will be serviced with a 250mm and 300 mm looped watermain with connections at both the northeast and northwest limits of the site. At the northeast limits, the watermain will connect to a 400 mm watermain to be located in the realigned Greenbank Road, and at the northwest limits of the site, to a future watermain located within the lands to the north of the site. Refer to the following figures:

- **Figure 3 – Realigned Greenbank Road Watermain**
- **Figure 4 – Watermain Layout**

As the ultimate watermain connections have not been constructed to date, it is proposed to construct approximately 335 m of the 400 mm realigned Greenbank Road watermain from the Greenbank and Jockvale intersection to south of the northeast limits of the site. It is also proposed to install approximately 255 m of watermain between Jockvale Road and the realigned Greenbank Road to provide a looped watermain for the proposed development prior to the connection to the northwest.

It is noted the proposed watermain works are located in a future Zone 3C pressure zone. The realignment of the pressure zone will be completed by the City of Ottawa and once complete will alter the boundary conditions for the development. The realignment is tentatively scheduled for 2018. The City of Ottawa has provided boundary conditions for the pre and post-realignment conditions. This report considers both conditions.

Fire flow demands have been calculated as per the Fire Underwriter's Survey (FUS) and are included in **Appendix C**. However as per the City of Ottawa's technical bulletin ISDTB-2014-02 (Revisions to Ottawa Design Guidelines – Water), the fire flow is capped at 10,000 L/min (167 L/s). Watermain analysis was completed based on the following criteria:

Demands:

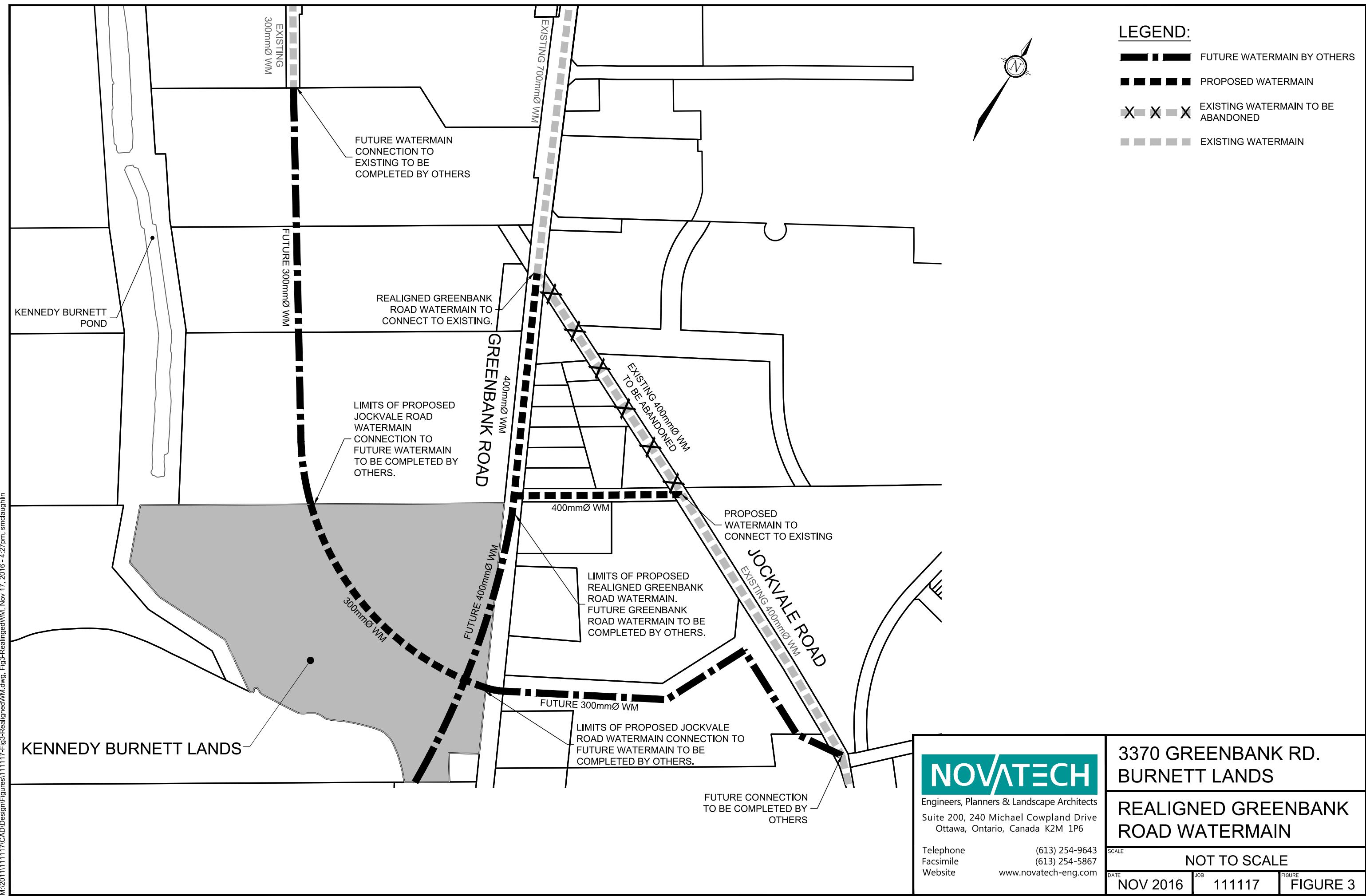
- | | |
|------------------------|----------------------------|
| • Townhouse Density | 2.7 persons/unit |
| • Average Daily Demand | 350 L/capita/day |
| • Max. Daily Demand | 2.5 x Average Daily Demand |
| • Peak Hour Demand | 2.2 x Maximum Daily Demand |
| • Fire Flow Demand | Fire Underwriters Survey |

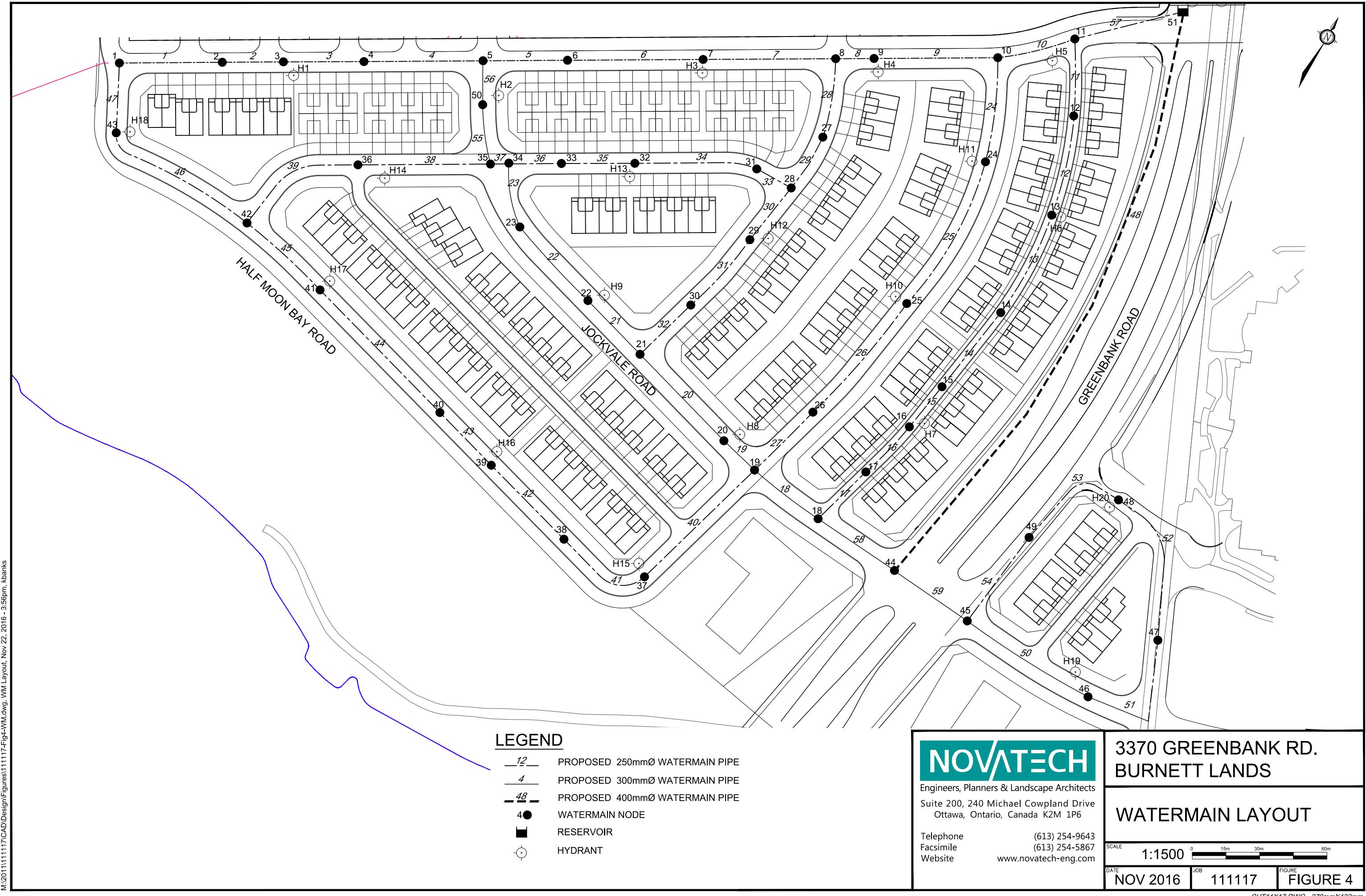
System Requirements:

- | | |
|---------------------------|---------------------------------------|
| • Max. Pressure (System) | 690 kPa (100 psi) |
| • Max. Pressure (Service) | 552 kPa (80 psi) |
| • Min. Pressure | 276 kPa (40 psi) excluding fire flows |
| • Min. Pressure (Fire) | 138 kPa (20 psi) including fire flows |
| • Max. Age (Quality) | 192 hours (onsite) |

Friction Factors:

- | | |
|------------------|----------|
| • Watermain Size | C-Factor |
| • 200-250 mm | 110 |
| • 300-400 mm | 120 |





The high pressure condition (average daily demand) was analyzed to ensure the system meets the design criteria for maximum pressure and quality. The maximum daily demand plus fire flow and peak hour conditions were analyzed to ensure the system meets the design criteria for maximum flow and minimum pressure.

The hydraulic modelling results for the development prior to the City reconfiguring the watermain are listed in **Table 4.1**.

Table 4.1: Water Demand Summary (Pre Watermain Reconfiguration)

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	5.8	N/A	690/100 (Max)	669.5/97.1	116
Maximum Daily Demand	14.5	167	138/20 (Min)	142.0/20.6	N/A
Peak Hour	31.8	N/A	276/40 (Min)	424.0/61.5	N/A

The analysis confirms the proposed watermain can service the Burnett Lands prior to the site being included into the realigned Zone 3C pressure zone. It is noted that pressure in the main is greater than 552 kPa/80psi therefore the use of pressure reducing values will be considered during detailed design.

The hydraulic modelling results for the development after the watermain realignment of the watermain are listed in **Table 4.2**.

Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	5.8	N/A	690/100 (Max)	533.0/77.3	116
Maximum Daily Demand	14.5	167	138/20 (Min)	142.0/20.6	N/A
Peak Hour	31.8	N/A	276/40 (Min)	424.0/61.5	N/A

The analysis confirms the proposed watermain can service the Burnett Lands after the watermain realignment. It is noted that pressure in the main is greater than 552 kPa/80psi therefore the use of pressure reducing values will be considered during detailed design.

A copy of the boundary conditions provided by the City of Ottawa, fire flow calculations, detailed hydraulic analysis results, and watermain layout figure are included in **Appendix C**.

5.0 STORMWATER MANAGEMENT CRITERIA

The Burnett Lands are tributary to the Jock River, which falls under the jurisdiction of the Rideau Valley Conservation Authority (RVCA). The following stormwater management criteria have been developed based on the requirements of the RVCA and the City of Ottawa Sewer Design Guidelines (October 2012) and Technical Bulletin PIEDTB-2016-01 (September 2016).

Minor System (Storm Sewers)

- Storm sewers are to be designed using the Rational Method for a 1:5 year return period;
- Inlet control devices (ICDs) will be installed in road and rearyard catchbasins to control inflows to the storm sewers;
- The 100-year hydraulic grade line in the storm sewer shall be at least 0.3 m below the underside of footing (USF) elevations for the proposed development.

Major System (Overland Flow)

- Overland flows are to be confined within the right-of-ways and/ or defined drainage easements for all storms up to and including the 1:100-year event;
- Maximum depth of flow (static + dynamic) on local and collector streets shall not exceed 0.35 m and shall be confined to the road right-of-way, as well as not touch any part of the building envelope and must remain below the lowest building opening during the stress test event;
- Storm runoff that exceeds the capacity of the minor system is to be stored within road sags;
 - Runoff that exceeds the capacity of the road sags is to be conveyed overland along defined major system flow routes towards the proposed major system outlet to the Jock River.
- ICD flow rates are to be calculated for each drainage area to ensure that the following stormwater management (SWM) objectives are satisfied:
 - Surface water accumulation at street low points, during a 5-year event, shall not be present by the end of the rainfall event;
 - Major system storage in backyards is not to be included/ accounted for in design computations;
 - Maximum flow depths and elevations on streets shall not exceed 0.30 m and shall be confined to the road right-of-way as well as not be within 0.30 m (vertical) to the nearest building opening;
 - The product of the 100-year flow depth (m) on street and flow velocity (m/s) shall not exceed 0.60;
 - The 100-year hydraulic gradeline within the storm sewers shall not be within 0.30 m to adjacent building underside of footing elevations.

Water Quality & Quantity Control

- An *Enhanced* (80% TSS removal) level of quality control is required for storm outfalls to the Jock River;
- Lot level and conveyance Best Management Practices should be to promote infiltration and treatment of storm runoff.
- Quantity control of post-development runoff to pre-development levels is not required for lands outletting directly to the Jock River, provided that there are no adverse impacts on downstream watercourses, structures, or property resulting from the proposed development.

Erosion and Sediment Control

- Erosion and sediment control measures are to be implemented during construction in accordance with the “Guidelines on Erosion and Sediment Control for Urban Construction Sites” (Government of Ontario, May 1987).
- All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.

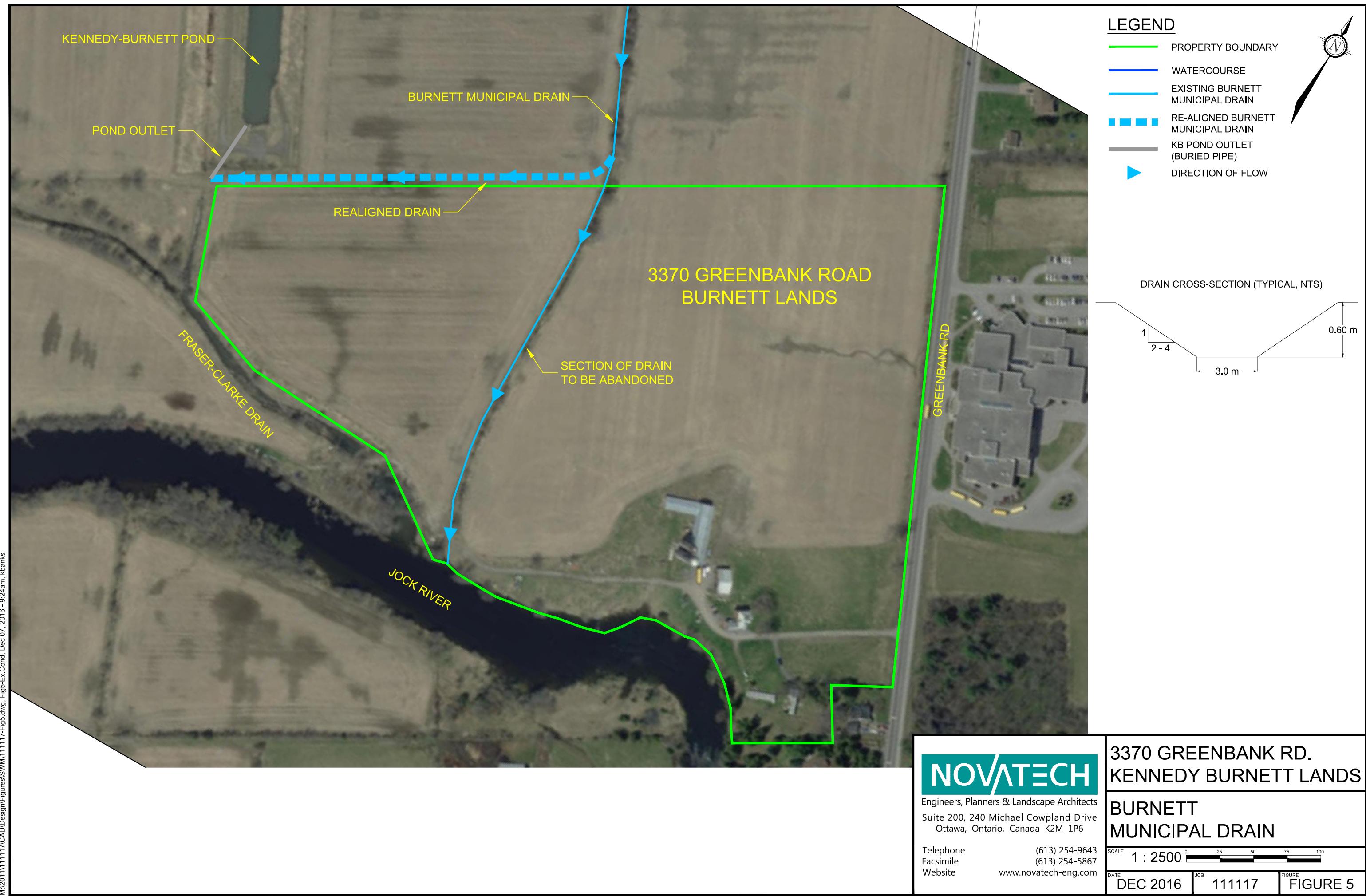
6.0 BURNETT MUNICIPAL DRAIN

Bisecting the site north-to-south is the existing Burnett Municipal Drain – Refer to **Figure 5**. The drain is a tributary to the Jock River, with a total length of approximately 1.3 km. A significant portion of the original Burnett Drain has been replaced by storm sewers. The drain has been fully enclosed and/or abandoned north of the Barrhaven Town Centre commercial plaza. The Burnett Municipal Drain is primarily an open channel between the Barrhaven Town Centre and the confluence with the Jock River, but a portion of the drain is piped across an existing driving range to the north of the Caivan Lands for a distance of approximately 170 m.

Novatech has prepared a memo (South Nepean Collector Culvert Crossings, June 10, 2016 – included in **Appendix E**) that outlines the sizing of culvert crossings to be installed as a part of the Phase 2 South Nepean Collector project. This memo provides an assessment of the design flows and capacity of the existing Burnett Municipal Drain. During the 100-year storm, the peak flow through the drain is approximately 2.7 m³/s. The existing drain has a cross-section consisting of a 3.0 m bottom width, 0.6 m depth, and side slopes ranging from 2:1 to 4:1. This cross-section gives a bankfull capacity of approximately 3.9 m³/s, based on Manning’s equation.

It is recommended that the drain be re-directed to the west, around the boundary of the site, connecting with the existing outlet for the KB Pond – as shown on **Figure 5**. To maintain the capacity of the drain, the realigned channel should maintain a cross-section consistent with the existing drain. Ultimately, it is anticipated that the drain will be abandoned as part of planned future development upstream.

Once construction of the subdivision and surrounding developments has been completed, it is anticipated that the municipal drain will be formally abandoned, as the flows originally directed to the drain will be captured by the proposed storm sewer systems.



As a part of the Municipal Drain Headwaters report, the Burnett Drain was classified using the four step process of the Headwater Guidelines:

- Hydrology Classification
 - Municipal drain provides valued hydrologic function (due to flow in spring);
 - Tributary drains are constructed agricultural drains with limited hydrologic function.
- Riparian Classification
 - Municipal Drain and Tributary 1 have limited riparian functions;
 - Tributaries 2 & 3 have limited to contributing riparian functions.
- Fish and Fish Habitat Classification
 - Municipal Dain is considered to have a contributing fish habitat;
 - Tributaries have no value as fish habitat.
- Terrestrial Classification
 - Municipal Drain and Tributaries have limited terrestrial functions.

As a result of these classifications, it is recommended that there is no management required for the tributaries of the Burnett Municipal Drain and that they can be abandoned. It is recommended that the Burnett Municipal Drain be managed through mitigation. Additional information on the Burnett Drain is available in Headwater Report referenced in **Section 1.2** of this report.

7.0 STORMWATER MANAGEMENT DESIGN

Storm servicing for the subject development will be provided using a dual drainage system: Runoff from frequent events will be conveyed by storm sewers (minor system), while flows from large storm events which exceed the capacity of the minor system will be conveyed overland along defined overland flow routes (major system).

The stormwater management design for the Burnett Lands conforms to the recommended stormwater management strategy for the planned Environmental Assessment update for the Kennedy-Burnett SWM Facility – refer to **Figure 113221 FIG-6** in **Appendix D**.

7.1 Storm Sewer Design (Minor System)

The proposed storm sewers have been designed using the Rational Method to convey peak flows associated with a 5-year return period. The storm sewer design sheet is provided in **Appendix A**. The corresponding Storm Drainage Area Plan (Drawing **111117-STM**) is provided in **Appendix F**. The design criteria used to size the storm sewers are summarized in **Table 7.1**.

Table 7.1: Storm Sewer Design Parameters

Parameter	Design Criteria
Local and Collector Roads	5 Year Return Period
Storm Sewer Design	Rational Method/Modeling
IDF Rainfall Data	Ottawa Sewer Design Guidelines (Oct. 2012)
Initial Time of Concentration (T_c)	15 minutes (rearyards) / 10 minutes (roads)
Minimum Velocity	0.8 m/s
Maximum Velocity	3.0 m/s
Minimum Diameter	250 mm

7.2 Inlet Control Devices

Inlet control devices (ICDs) will be installed in all catchbasins to limit inflows to the minor system during large (>1:5 year) storm events.

All catchbasins will have a single connection to the storm sewers. ICDs will be round orifice plugs with diameters of either 83mm, 94mm, 102mm, 127mm, 152mm, or 178mm.

7.3 Overland Flow Path (Major System)

The site has been graded to provide an engineered overland flow route (major system) for large, infrequent storms or in the event that the storm sewer system becomes obstructed. Flows will be directed to the Jock River at the low point in the system. The design of the major system conforms to the design standards outlined in Section 5.5 of the Sewer Design Guidelines.

7.4 Street 'B'/ Caivan Lands

Street 'B' is a shared road between the Burnett Lands and the Caivan Lands to the north. Approximately 1.69 ha of the Burnett Lands development and approximately 2.61 ha of the Caivan Lands will drain to Street 'B'. Since Street 'B' will be constructed as a part of the Burnett Lands Development, the street and tributary areas have been included in the design.

Storm runoff from the Caivan Lands and Street 'B' will be directed to the outlet channel for Kennedy-Burnett SWM facility by a shared storm sewer. Water quality treatment upstream of this outfall will be provided using a hydrodynamic separator at the western end of Street 'B' (refer to the General Plan of services, 111117-GP in **Appendix F**).

7.5 Water Quality

An *Enhanced* (80% TSS removal) level of water quality control will be provided by using hydrodynamic separators (HDS) upstream of the storm outfalls.

- Storm runoff from the majority of the site will be treated by an HDS unit upstream of the outfall to the Jock River near the southeast corner of the site.
- Storm runoff from Street B will be treated by an HDS unit located upstream of the outfall to the Kennedy-Burnett SWM Facility outlet channel near the northwest corner of the site. This storm outlet and HDS unit will also serve as the storm outlet for the adjacent Caivan development to the north.

In addition to the HDS units, lot level and conveyance Best Management Practices should be considered at the detailed design stage to promote infiltration and treatment of storm runoff;

- Perforated pipes for rear-yard catchbasin leads;
- Direct roof leaders to rear-yard areas;
- Infiltration trenches underlying swales in rear-yard areas;
- The use of fine sandy loam topsoil on residential lawns;

8.0 HYDROLOGIC & HYDRAULIC MODELING

The *City of Ottawa Sewer Design Guidelines* (October 2012) requires hydrologic modeling for all dual drainage systems. The performance of the proposed storm drainage system for the Burnett Lands was evaluated using the PCSWMM hydrologic/hydraulic model.

8.1 Design Storms

The hydrologic analysis was completed using the following synthetic design. The IDF parameters used to generate the design storms were taken from the *City of Ottawa Sewer Design Guidelines* (October 2012).

4 Hour Chicago Storms:

25mm 4hr Chicago storm
2-year 4hr Chicago storm
5-year 4hr Chicago storm
100-year 4hr Chicago storm
100-year 4hr +20% Chicago storm

The 4-hour Chicago distribution generates the highest peak flows for both the minor and major systems and was determined to be the critical storm distribution for the design of the storm drainage system.

The proposed drainage system has also been stress tested using a 4-hour Chicago design storm that has a 20% higher intensity and total volume compared to the 100-year event.

Modeling files are provided on the enclosed CD.

8.2 Model Development

The PCSWMM model has been developed to account for both minor and major system flows from the development, and ensure no adverse impacts on the downstream drainage system. The results of the analysis were used to:

- Determine the total major and minor system runoff from the site;
- Calculate the storm sewer hydraulic grade line for the 100-year storm event;
- Evaluate overland flow depths and ponding volumes during the 100-year event; and
- Ensure no ponding in the right-of-ways remains at the end of all storm events;

8.2.1 Storm Drainage Areas

For modeling purposes, the site has been divided into subcatchments based on the drainage areas tributary to each inlet of the proposed storm sewer system. The catchment areas are shown on the Storm Drainage Area Plan (**111117-STM**) in **Appendix F**.

8.2.2 Subcatchment Model Parameters

The hydrologic parameters for each subcatchment were developed based on the Concept Plan (**Figure 2**) and the Storm Drainage Area Plan (**111117-STM**). An overview of the modeling parameters is provided in

Table 8.1.

Table 8.1: Hydrologic Modeling Parameters

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Imp. (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
Burnett Lands - Claridge						
A-01	0.235	0.65	64%	50%	43	0.75%
A-02	0.249	0.65	64%	50%	36	0.75%
A-03	0.127	0.65	64%	75%	36	0.75%
A-04	0.302	0.65	64%	50%	34	0.75%
A-05	0.275	0.65	64%	75%	37	0.75%
A-06	0.274	0.65	64%	60%	37	0.75%
A-07	0.289	0.65	64%	50%	39	0.75%
A-08	0.246	0.65	64%	75%	41	0.75%
A-09	0.081	0.65	64%	50%	23	0.75%
A-10	0.187	0.65	64%	50%	37	0.75%
A-11	0.225	0.65	64%	50%	25	0.75%
A-12	0.036	0.65	64%	100%	24	0.75%
A-13	0.200	0.65	64%	50%	24	0.75%
A-14	0.195	0.65	64%	50%	23	0.75%
A-15	0.105	0.65	64%	100%	30	0.75%
A-16	0.128	0.65	64%	100%	32	0.75%
A-17	0.246	0.65	64%	50%	49	0.75%
A-18	0.272	0.65	64%	50%	32	0.75%
A-19	0.122	0.65	64%	100%	31	0.75%
A-20	0.130	0.65	64%	100%	29	0.75%
A-21	0.217	0.65	64%	50%	27	0.75%
A-22	0.060	0.65	64%	100%	24	0.75%
A-23	0.457	0.65	64%	75%	46	0.75%
A-24	0.209	0.65	64%	75%	28	0.75%
A-25	0.214	0.65	64%	75%	29	0.75%
A-26	0.448	0.65	64%	70%	45	0.75%
A-27	0.190	0.65	64%	75%	32	0.75%
A-28	0.119	0.65	64%	0%	18	0.75%
A-29	0.231	0.65	64%	60%	46	0.75%
A-30	0.498	0.65	64%	50%	71	0.75%
A-31	0.061	0.65	64%	0%	20	0.75%
A-32	0.450	0.65	64%	75%	56	0.75%
A-33	0.816	0.65	64%	50%	117	0.75%

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Imp. (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
A-34	0.773	0.65	64%	0%	55	0.75%
A-35	0.983	0.65	64%	0%	70	0.75%
A-36	0.450	0.65	64%	0%	60	0.75%
A-37	0.372	0.65	64%	0%	53	0.75%
Street B - Caivan Lands						
B-01	0.092	0.65	64%	50%	28	0.75%
B-02	0.149	0.65	64%	0%	23	0.75%
B-03	0.097	0.65	64%	10%	32	0.75%
B-04	0.825	0.65	64%	50%	110	0.75%
B-05	0.064	0.65	64%	100%	27	0.75%
B-06	0.076	0.65	64%	0%	19	0.75%
B-07	0.127	0.65	64%	40%	32	0.75%
B-08	0.225	0.65	64%	50%	32	0.75%
B-09	0.245	0.65	64%	50%	34	0.75%
B-10	1.115	0.30	14%	0%	149	0.75%
B-11	0.049	0.65	64%	0%	13	0.75%
B-12	0.088	0.65	64%	50%	20	0.75%
B-13	0.097	0.65	64%	50%	22	0.75%
B-14	0.320	0.65	64%	50%	73	0.75%
B-15	0.120	0.65	64%	50%	27	0.75%
B-16	0.136	0.65	64%	50%	31	0.75%
B-17	0.247	0.65	64%	50%	33	0.75%
B-18	0.121	0.65	64%	50%	28	0.75%
B-19	0.107	0.65	64%	50%	22	0.75%
CaivanLands	8.101	0.65	64%	50%	200	0.50%

Infiltration

Infiltration losses for all catchment areas were modeled using Horton's infiltration equation, which defines the infiltration capacity of the soil over the duration of a precipitation event using a decay function that ranges from an initial maximum infiltration rate to a minimum rate as the storm progresses. The default values for the City of Ottawa were used for all catchments.

Horton's Equation:

$$f(t) = f_c + (f_o - f_c)e^{-k(t)}$$

Initial infiltration rate: $f_o = 76.2 \text{ mm/hr}$
Final infiltration rate: $f_c = 13.2 \text{ mm/hr}$
Decay Coefficient: $k = 4.14/\text{hr}$

Depression Storage

The default values for depression storage in the City of Ottawa were used for all catchments.

- Depression Storage (pervious areas): 4.67 mm
- Depression Storage (impervious areas): 1.57 mm

Residential rooftops are assumed to provide no depression storage and all rainfall is converted to runoff.

Equivalent Width

'Equivalent Width' refers to the width of the sub-catchment flow path. This parameter (Table 5.1) is calculated as described in the *Sewer Design Guidelines, October 2012, Section 5.4.5.6*.

Impervious Values

Impervious (%IMP) values for each subcatchment area were calculated based on the concept plan (**Figure 2**). The impervious values correspond to the Runoff Coefficients (C) used in the Rational Method calculations using the equation:

$$\%IMP = \frac{C - 0.2}{0.7}$$

For the Storm Sewer Design spreadsheet, typical lots were analyzed with respect to the concept plan and runoff coefficients were determined based on the proposed land use. For development consisting of primarily medium-density residential (multi-unit attached dwellings) a runoff coefficient (C) of 0.65 was selected.

8.2.3 Minor System

Inflows to the storm sewer were modeled based on the ICD specified for the inlet and the maximum depth of ponding. Storage volumes within the right-of-way are based on the grading design. ICD parameters are outlined as follows in **Table 8.2**.

Table 8.2: ICD Parameters

Structure	ICD Size & Inlet Rate			5-year Peak Flow*
	Diameter (mm)	Max Head (m)	5-year Capture Rate* (L/s)	
Burnett Lands - Claridge				
CB01-02	127	1.14	37.74	50.15
CB03-04	127	1.23	38.84	48.26
CB05-06	152	1.12	52.87	58.43
CB07-08	127	1.14	37.29	50.69
CB09-10	127	1.04	4.66	4.80
CB11-12	108	1.19	27.44	54.07
CB13-14	102	1.15	23.96	27.65
CB15-16	108	1.15	28.06	56.86
CB17-18	102	1.15	24.77	49.60
CB19	83	1.16	15.85	17.43
CB20-21	156	1.12	57.10	80.41
CB22	108	1.15	27.33	37.97
CB23	108	1.15	26.97	38.91
CB24-25	127	1.14	36.88	44.09

Structure	ICD Size & Inlet Rate			5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	5-year Capture Rate* (L/s)	
CB26-27	178	1.11	72.35	86.47
CB28	127	1.15	37.09	47.94
CB29	94	1.15	20.45	29.12
CB30-31	127	1.14	36.75	38.46
CB32-33	127	1.14	37.03	44.92
CB34-35	127	1.14	37.01	44.34
CB40-41	178	1.11	72.83	87.24
CB42-43	152	1.12	51.29	52.25
CB44-45	127	1.23	39.15	50.39
CB59	83	1.16	6.67	6.67
CB60	102	1.15	23.68	24.03
CB61	83	1.16	11.03	11.11
CB62	108	1.15	22.43	22.56
CB63	108	1.15	23.50	23.67
CB64	102	1.15	19.31	19.43
CB66	83	1.16	12.35	13.15
CB67-68-69	178	1.17	74.36	85.77
Street B - Caivan Lands				
CB36	83	1.46	15.93	18.08
CB37	102	1.45	24.08	29.14
CB39	83	1.46	16.11	19.63
CB46-47	83	1.46	14.13	15.06
CB48-49	83	1.44	16.16	24.54
CB50-51	127	1.44	37.25	42.30
CB52-53	178	1.41	73.84	92.98
CB54	108	1.45	9.23	9.67
CB55-56	178	1.41	53.29	91.54
CB57-58	178	1.31	49.98	50.85
CB65	83	1.46	11.78	11.90

8.2.4 Major System

The proposed road network was input into the PCSWMM model to calculate the total inflow into the storm sewers (minor system), and to calculate the overland flows and flow depths within the right-of-way (major system).

The roads are represented in the model as open channels. Model input includes:

- Right-of-way cross-sections;

- Length and slope of the road between each high and low point;
- The location of all storm inlets and whether the inlets are in a sag or on-grade.

The elevations used to define the road network in the PCSWMM model are based on the gutter elevations, as opposed to the centerline of road elevations shown on the Grading Plans (**111117-GR1-3** as provided in **Appendix F**). Right-of-way cross sections used in the PCSWMM model are provided in **Appendix D**.

8.2.5 Modeling Files / Schematic

The PCSWMM model schematics and 100-year model output data are provided in **Appendix D**. Digital copies of the modeling files and model output for all storm events are provided on the enclosed CD.

8.3 PCSWMM Model Results

8.3.1 Minor System

The performance of the Burnett Lands storm sewer system has been evaluated using various storm distributions and historical storms. The 4-hour Chicago distribution was found to generate the highest peak flows in the storm sewers and was selected as the critical storm distribution. The results are summarized in **Table 8.3**.

The 100-year 4-hour Chicago storm was also increased by 20% (intensity + total precipitation) to evaluate the impact of an extreme event on the performance of the minor system. The results of this analysis indicate there is no significant impact on the minor system peak flows, due to the fact that inflows to the storm sewer system will be controlled using ICDs.

Table 8.3: Minor System Peak Flows at Outlets

Storm Distribution->	4hr Chicago				
Return Period->	25mm	2yr	5yr	100yr	100yr +20%
Burnett Lands - Flows to Jock River					
Minor System - HW-01	591	843	1028	1328	1346
Major System - OverlandOutlet	0	0	0	4	32
Burnett & Caivan Lands - Flows to Fraser-Clarke Drain					
Outlet from Street B - HW-02	720	1063	1531	1596	1622
Offsite Flows					
Greenbank Road - GreenbankOut	237	331	465	867	1083

8.3.2 Major System

The major system network was evaluated using the PCSWMM model to ensure that the ponding depths conform to City standards. A summary of ponding depths and volumes for the 100-year event is provided in **Table 8.4**. Ponding volumes and depths for all storm events (including the 100-yr+20% event and 5-year event) are provided in **Appendix D**.

Table 8.4: Ponding Depths at Catchbasins (100yr Event)

Structure	T/G (m)	Max. Static Ponding (Spill Depth)		100-yr Event (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Spill Flow (L/s)
Burnett Lands - Claridge							
CB01-02	93.25	93.44	0.19	93.41	0.16	N	0
CB03-04	93.18	93.44	0.26	93.42	0.24	N	0
CB05-06	93.05	93.34	0.29	93.19	0.14	N	0
CB07-08	92.92	93.34	0.42	93.08	0.16	N	0
CB09-10	93.01	93.34	0.33	93.26	0.25	N	0
CB15-16	93.20	93.37	0.17	93.43	0.23	Y	56
CB17-18	93.51	93.76	0.25	93.71	0.20	N	0
CB19	93.44	93.64	0.20	93.56	0.12	N	0
CB20-21	93.34	93.60	0.26	93.56	0.22	N	0
CB22	93.49	93.64	0.15	93.64	0.15	N	0
CB23	93.28	93.51	0.23	93.37	0.09	N	0
CB24-25	93.25	93.48	0.23	93.35	0.10	N	0
CB26-27	93.08	93.36	0.28	93.24	0.16	N	0
CB28	92.99	93.34	0.35	93.08	0.09	N	0
CB29	92.98	93.34	0.36	93.08	0.10	N	0
CB30-31	93.31	93.58	0.27	93.60	0.29	Y	77
CB40-41	93.35	93.65	0.30	93.52	0.17	N	0
CB42-43	93.47	93.72	0.25	93.63	0.16	N	0
CB44-45	93.62	93.87	0.25	93.77	0.15	N	0
CB59	93.70	93.91	0.21	93.14	0.00	N	0
CB60	93.50	93.80	0.30	93.81	0.31	Y	1
CB61	93.50	93.78	0.28	93.64	0.14	N	0
CB62	93.49	93.78	0.29	93.77	0.28	N	0
CB63	93.55	93.84	0.29	93.85	0.30	Y	2
CB64	93.66	93.95	0.29	93.91	0.25	N	0
CB66	93.90	94.01	0.11	93.97	0.07	N	0
CB67-68-69	93.66	94.15	0.49	93.83	0.17	N	0
Street B - Caivan Lands							
CB39	93.78	93.86	0.08	93.92	0.14	Y	52
CB46-47	93.67	93.77	0.10	93.84	0.17	Y	52
CB48-49	93.72	93.86	0.14	93.87	0.15	Y	9
CB50-51	93.59	93.84	0.25	93.74	0.15	N	0

Structure	T/G (m)	Max. Static Ponding (Spill Depth)		100-yr Event (4hr)				
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Spill Flow (L/s)	
CB52-53	93.52	93.76	0.24	93.77	0.25	Y	4	
CB55-56	93.38	93.67	0.29	93.59	0.21	N	0	
CB57-58	93.30	93.56	0.26	93.47	0.17	N	0	

8.3.3 Hydraulic Grade Line

The results of the analysis were used to ensure that a minimum freeboard of 0.30m is provided between the 100-year HGL and the designed underside of footing elevations. The 100-year HGL is indicated on the Plan and Profile Drawings (**111117-P1-9**). The HGL analysis confirms that all dwellings within the Burnett Lands will have at least 0.30m of freeboard between the modeled hydraulic grade line and the underside of footing elevation.

Storm runoff from the Burnett Lands is to be conveyed to the Jock River, after flowing through a Vortechs unit for water quality treatment.

Peak flows from the subdivision, tributary to the Jock River will not coincide with peak flows in the river. Consequently, a combined frequency analysis was used to assess the maximum HGL in the storm sewers. The hydraulic analysis was initially modeled for two scenarios:

- 1) 5-year flows in the storm sewers, 100-year flood elevation at the outlet.
- 2) 100-year flows in the storm sewers, 5-year flood elevation at the outlet.

The 5-year peak flow combined with the 100-year flood elevation of 91.28 at the Jock River and 91.58 at the Fraser-Clarke Drain produced the highest HGL values, and this represents the worst-case scenario. This scenario was used for all subsequent analysis.

Table 8.5 provides a summary of the 5-year peak flow combined with the 100-year flood elevation HGL elevation at each storm manhole within the proposed development. HGL elevations for the 100-year flows in the storm sewers, 5-year flood elevation at the outlet as well as a summary of the HGL elevations for a 20% increase (rainfall intensity and total precipitation) in the 100-year design event. The results of this stress testing indicates that, even under this scenario, the hydraulic grade line in the sewers will only slightly increase (max increase of 0.03m), ensuring that the HGL will remain below the undersides of footing of the proposed units.

**Table 8.5: Storm Sewer Hydraulic Grade Line:
5yr-4hr Chicago Distribution, 100-yr WL in Jock River**

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elevation (m)	Design USF (m)	Clearance from USF (m)
Burnett Lands - Claridge					
102 (STM)	90.52	93.78	92.04	92.62	0.58
104 (STM)	90.37	93.93	92.03	92.60	0.57

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elevation (m)	Design USF (m)	Clearance from USF (m)
106 (STM)	90.27	93.87	92.04	92.60	0.56
108 (STM)	90.10	93.75	92.01	92.51	0.50
110 (STM)	89.94	93.46	92.00	92.43	0.43
304 (STM)	90.56	93.67	91.72	92.25	0.53
306 (STM)	90.35	93.58	91.72	92.22	0.50
308 (STM)	90.11	93.41	91.70	92.17	0.47
310 (STM)	89.74	93.46	91.66	92.06	0.40
312 (STM)	89.26	93.36	91.65	92.06	0.41
314 (STM)	89.19	92.61	91.58	-	-
316 (STM)	89.00	93.09	91.49	-	-
324 (STM)	88.96	93.34	91.41	-	-
326 (STM)	89.95	93.38	91.35	-	-
328 (STM)	88.91	92.16	91.30	-	-
330 (STM)	88.94	92.53	91.38	-	-
402 (STM)	90.62	93.82	92.02	92.40	0.38
404 (STM)	90.37	93.80	92.01	92.45	0.44
406 (STM)	90.46	93.71	92.04	92.55	0.51
408 (STM)	90.36	93.80	92.04	92.55	0.51
608 (STM)	90.25	93.71	91.99	92.55	0.56
610 (STM)	89.58	93.30	91.79	92.38	0.59
612 (STM)	89.82	93.93	91.64	-	-
614 (STM)	89.92	93.76	91.71	-	-
616 (STM)	90.23	94.27	91.72	92.65	0.93
618 (STM)	90.03	93.65	91.80	92.25	0.45
902 (STM)	90.34	93.98	91.82	92.55	0.73
904 (STM)	90.29	93.76	91.83	92.35	0.52
906 (STM)	90.12	93.53	91.82	92.35	0.53
908 (STM)	89.97	93.57	91.81	92.25	0.44
Street B - Caivan Lands					
200 (STM)	91.55	94.54	92.38	-	-
202 (STM)	91.40	94.14	92.37	-	-
204 (STM)	91.08	93.94	92.26	92.55	0.29
206 (STM)	90.84	93.62	92.14	92.45	0.31
208 (STM)	90.69	93.82	92.06	92.45	0.39
210 (STM)	90.53	93.52	91.89	92.45	0.56
212 (STM)	90.40	93.73	91.80	92.40	0.60
214 (STM)	90.30	93.47	91.73	92.25	0.52
216 (STM)	90.01	93.37	91.65	92.25	0.60

9.0 UTILITIES

The development will be serviced by hydro, phone, cable, and gas from the existing services on Greenbank Road. During detailed design the works will be coordinated with local utilities companies.

10.0 EROSION AND SEDIMENT CONTROL

Erosion and sediment control measures will be implemented during construction in accordance with the "Guidelines on Erosion and Sediment Control for Urban Construction Sites" (Government of Ontario, May 1987). Detailed plans will be provided at the detailed design stage.

Typical erosion and sediment control measures recommended include, but are not limited to, the use of silt fences around perimeter of site (OPSD 219.110), filter fabric or inserts under catch basin/maintenance hole lids, heavy duty silt fence barrier (OPSD 219.130), straw bale check dams (OPSD 219.180), rock check dams (219.210 or OPSD 219.211), turbidity curtain (OPSD 219.260), dewatering trap (OPSD 219.240), temporary water passage system (OPSD 221.030), riprap (OPSS 511), mud mats, silt bags for dewatering operations, topsoil and sod to disturbed areas and natural grassed waterways. Dewatering and sediment control techniques will be developed for the individual situations based on the above guidelines and utilizing typical measures to ensure erosion and sediment control is controlled in an acceptable manner and there is no negative impact to adjacent lands, water bodies or water treatment/conveyance facilities.

It will be the responsibility of the Contractor to submit a detailed construction schedule and appropriate staging, dewatering and erosion and sediment control plans to the Contract Administrator for review and approval prior to the commencement of work.

All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.

- A qualified inspector should conduct daily visits during construction to ensure that the contractor is working in accord with the design drawings and that mitigation measures are being implemented as specified.
 - A light duty silt fence barrier is to be installed in the locations shown on the Erosion and Sediment Control Plan.
 - Straw bale barriers are to be installed in drainage ditches
 - Inserts are to be placed under the grates of all proposed and existing catchbasins and structures.
 - After complete build-out, all sewers are to be inspected and cleaned and all sediment and construction fencing is to be removed.
- The contractor shall ensure that proper dust control is provided with the application of water (and if required, calcium chloride) during dry periods.
- The contractor shall immediately report to the engineer or inspector any accidental discharges of sediment material into any ditch or sewer system. Appropriate response measures shall be carried out by the contractor without delay.
- The contractor acknowledges that failure to implement erosion and sediment control measures may result in penalties imposed by any applicable regulatory agency.

11.0 CONCLUSIONS AND RECOMMENDATIONS

Sanitary Servicing

The analysis of the proposed sanitary servicing conforms the following:

- The site is located within Sanitary Drainage Area 8A of Phase 2 of the SNC.
- The on-site sanitary sewers have been sized to accommodate a portion of the peak sanitary flows from the adjacent lands to the north, to be developed by others.
- The proposed sanitary sewers have adequate capacity to accommodate the peak sanitary flow.

Watermain

The analysis of the proposed watermain network conforms the following:

- Approximately 560m of the 400mm Greenbank Road watermain is to be installed and connected to the existing 400mm Jockvale Road watermain to provide a looped watermain feed for the proposed site.
- Ultimately the site will connect to future watermain, to be completed by others, to the northwest.
- The site is located in a future Zone 3C pressure zone that once complete will increase the pressure in the system onsite.
- This report confirms the proposed watermain can service the site pre and post zone reconfiguration.
- During detailed design the use of pressure reducing valves will be explored.

Stormwater Management

The stormwater management design for the Burnett Lands development conforms to the criteria established as part of this report.

The conclusions based on the results of the stormwater management analysis are as follows:

Storm Drainage / Conveyance

- Storm sewers (minor system) have been designed to convey the uncontrolled 5-year peak flow using the Rational Method.
- Inflows to the minor system will be controlled using inlet control devices (ICDs). Proposed ICDs will consist of round orifice plates with various standard diameters (83mm, 94mm, 102mm, 108mm, 127mm, 152mm, and 178mm).
- The site has been graded to provide surface storage at low points along roadways, and a major system outlet to the Jock River for flows which exceed the capacity of the road sags.
- Ponding depths will not exceed 0.30m for all storms up to and including the 100-year event.
- The post-development peak flows from the site will have no adverse impact on the Jock River downstream.
- A minimum clearance of 0.30m will be provided between the 100-year hydraulic grade line (HGL) and the designed underside of footing elevations.

Stormwater Management

- An *Enhanced* level of water quality control (80% TSS removal) will be provided using hydrodynamic separators upstream of the storm outfalls.
- Quantity control is not required for the Burnett Lands, as they are located adjacent to the Jock River.

Erosion and Sediment control

- Erosion and sediment control measures (i.e. filter fabric, silt fences, etc.) will be implemented prior to construction and are to remain in place until vegetation is established.

The preceding report is respectfully submitted for review and approval. Please contact the undersigned should you have questions or require additional information.

NOVATECH

Stormwater Management and Modeling

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Approved by:



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Appendix A

Design Sheets

SANITARY SEWER DESIGN SHEET

CLARIDGE SUBDIVISION
DEVELOPER: CLARIDGE HOMES

PROJECT : 111117
DESIGNED BY: LSC
CHECKED BY: GJM
DATE PREPARED: Dec. 2015
DATE REVISED: Nov. 25/16



LOCATION							CUMULATIVE		PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER								
STREET	FROM MH	TO MH	Condos	TOWNS	POPULATION (in 1000's)	AREA (ha.)	POPULATION (in 1000's)	AREA (ha.)					LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	Roughness Coef.	GRADE (%)	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/ Qcap
Future Street B	Stub 201	203			0.0608	0.225	0.061	0.225	4.00	0.98	0.06	1.05	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.05
Future Street B	201	203		3	0.0081	0.268	0.069	0.493	4.00	1.12	0.14	1.25	36.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.04
Future Street B	Stub 203	205		6	0.0805	0.298	0.080	0.298	4.00	1.30	0.08	1.39	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.07
Future Street B	203	205		6	0.0162	0.315	0.166	1.106	4.00	2.68	0.31	2.99	35.5	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.10
Future Street B	Stub 205	207		4	0.0878	0.325	0.088	0.325	4.00	1.42	0.09	1.51	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.08
Future Street B	205	207		4	0.0108	0.107	0.264	1.538	4.00	4.28	0.43	4.71	35.5	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.16
Future Street B	Stub 207	209		7	0.0791	0.293	0.079	0.293	4.00	1.28	0.08	1.36	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.07
Future Street B	207	209		7	0.0189	0.183	0.362	2.014	4.00	5.87	0.56	6.43	57.8	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.21
Future Street B	Stub 209				0.0510	0.189	0.051	0.189	4.00	0.83	0.05	0.88	12.0	250	251.5	DR 35	0.013	0.35	35.7	0.72	0.02
Future Street B	209				0.5219	1.933	0.522	1.933	3.96	8.38	0.54	8.92	12.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.45
Future Street B	213	211		12	0.0324	0.312	0.554	2.245	3.95	8.87	0.63	9.50	100.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.31
Future Street B	211	209		6	0.0162	0.172	0.571	2.417	3.94	9.12	0.68	9.79	58.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.32
Jockvale	209	601		0	0.0000	0.064	0.984	4.684	3.80	15.16	1.31	16.47	39.4	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.55
Jockvale/Street D	601	SNC		0	0.0000	0.000	0.984	4.684	3.80	15.16	1.31	16.47	3.0	250	251.5	DR 35	0.013	1.00	60.4	1.22	0.27
Street D	401	603		10	0.0270	0.259	0.027	0.259	4.00	0.44	0.07	0.51	79.2	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.03
Jockvale	603	605		5	0.0135	0.224	0.041	0.483	4.00	0.66	0.14	0.79	32.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.03
Jockvale	605	607		11	0.0297	0.460	0.070	0.943	4.00	1.14	0.26	1.40	79.8	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.05
Street D	403	405		25	0.0675	0.600	0.068	0.600	4.00	1.09	0.17	1.26	103.5	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.06
Street D	405	105		3	0.0081	0.078	0.076	0.678	4.00	1.23	0.19	1.41	21.8	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.07
Street A	101	103		4	0.0108	0.213	0.011	0.213	4.00	0.18	0.06	0.23	24.4	200	201.2	DR 35	0.013	0.65	26.9	0.84	0.01
Street A	103	105		5	0.0135	0.147	0.024	0.360	4.00	0.39	0.10	0.49	30.7	200	201.2	DR 35	0.013	0.65	26.9	0.84	0.02
Street A	105	107		7	0.0189	0.219	0.119	1.257	4.00	1.93	0.35	2.28	47.8	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.12
Street A	107	607		6	0.0162	0.272	0.135	1.529	4.00	2.19	0.43	2.62	51.4	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.13
Jockvale	607	609		9	0.0243	0.328	0.230	2.800	4.00	3.72	0.78	4.50	72.5	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.15
Half Moon Bay	301	303		14	0.0378	0.551	0.038	0.551	4.00	0.61	0.15	0.77	91.3	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.04
Half Moon Bay	303	305		16	0.0432	0.594	0.081	1.145	4.00	1.31	0.32	1.63	120.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.08
Half Moon Bay	305	307		0	0.0000	0.016	0.081	1.161	4.00	1.31	0.33	1.64	9.3	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.08
Half Moon Bay	307	609		0	0.0000	0.135	0.081	1.296	4.00	1.31	0.36	1.68	73.2	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.09
Street F	901	903		10	0.0270	0.429	0.027	0.429	4.00	0.44	0.12	0.56	41.9	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.03
Street F	903	905		10	0.0270	0.278	0.054	0.707	4.00	0.88	0.20	1.07	39.9	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.05
Street F	905	907		20	0.0540	0.555	0.108	1.262	4.00	1.75	0.35	2.10	76.3	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.11
Street F	907	609		11	0.0297	0.325	0.138	1.587	4.00	2.23	0.44	2.68	57.0	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.14
Jockvale	609	611		0	0.0000	0.000	0.448	5.683	4.00	7.26	1.59	8.85	6.0	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.29
Jockvale	Stub 615	280																			

SANITARY SEWER DESIGN SHEET

CLARIDGE SUBDIVISION

DEVELOPER: CLARIDGE HOMES

PROJECT : 111117
 DESIGNED BY: LSC
 CHECKED BY: GJM
 DATE PREPARED: Dec. 2015
 DATE REVISED: Nov. 25/16



LOCATION							CUMULATIVE		PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER								
STREET	FROM MH	TO MH	Condos	TOWNS	POPULATION (in 1000's)	AREA (ha.)	POPULATION (in 1000's)	AREA (ha.)					LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	Roughness Coef.	GRADE (%)	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/ Qcap
Greenbank	705	707		6	0.0162	0.447	0.054	1.279	4.00	0.88	0.36	1.23	60.5	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.06
Greenbank	707	613		11	0.0297	0.690	0.084	1.969	4.00	1.36	0.55	1.91	93.1	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.10
Jockvale	613	611	140	0	0.2520	0.539	0.872	3.837	3.84	13.56	1.07	14.63	71.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.48
Jockvale/Steet F	611	SNC		0	0.0000	0.000	1.320	9.520	3.72	19.89	2.67	22.56	2.8	250	251.5	DR 35	0.013	1.00	60.4	1.22	0.37

14.20

Notes: 1 Q(d) = Design Flow (L/sec)
 Q(p) = Population Flow (L/sec)
 Q(i) = Extraneous Flow (L/sec)

Q(p) = (PxqxM/86,400), where P = Population (2.7 persons per town/semi, 1.8 persons per condo)
 q = Average per capita flow = 350 L/cap/day
 M = Harmon Formula (maximum of 4.0)

2 Future population based on zoning density

Storm Sewer Design Sheet

STREET	FROM	TO	AREA ID			FLOW					PROPOSED SEWER								Q/Qfull
				R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)	EXCESS CAPACITY (l/s)	
Half Moon Bay	MH 304	MH 306	A1		0.235	0.42	0.42	10.00	104.19	44.25	600	609.6	0.30	65.5	351.20	1.20	0.91	306.96	0.13
Half Moon Bay	MH 306	MH 308	A2		0.249	0.45	0.87	10.91	99.63	87.13	600	609.6	0.25	89.4	320.60	1.10	1.36	233.47	0.27
Street E	CB 14	MH 308	A3		0.127	0.23	0.23	10.00	104.19	23.91	250	254.0	1.00	39.4	62.10	1.22	0.54	38.19	0.39
Half Moon Bay	MH 308	MH 310	A4		0.302	0.55	1.65	12.27	93.58	154.38	600	609.6	0.25	73.3	320.60	1.10	1.11	166.22	0.48
Street E	CB 12	MH 310	A5		0.275	0.50	0.50	10.00	104.19	51.78	250	254.0	1.00	39.4	62.10	1.22	0.54	10.33	0.83
Half Moon Bay	MH 310	MH 312	A6		0.274	0.50	2.64	13.38	89.19	235.62	750	762.0	0.15	41.4	450.27	0.99	0.70	214.65	0.52
Street D	MH 402	MH 404	A7-A9		0.616	1.11	1.11	10.00	104.19	115.98	600	609.6	0.25	78.0	320.60	1.10	1.18	204.62	0.36
Jockvale	MH 404	MH 608	A10, A11		0.412	0.74	1.86	11.18	98.33	182.65	600	609.6	0.25	34.2	320.60	1.10	0.52	137.95	0.57
Jockvale	MH 608	MH 110	A12, A13		0.236	0.43	2.28	11.70	95.98	219.22	600	609.6	0.20	79.6	286.76	0.98	1.35	67.54	0.76
Street D	MH 406	MH 408	A14		0.195	0.35	0.35	10.00	104.19	36.71	600	609.6	0.20	105.5	286.76	0.98	1.79	250.04	0.13
Street D	MH 408	MH 106				0.00	0.35	11.79	95.60	33.68	600	609.6	0.20	20.5	286.76	0.98	0.35	253.07	0.12
Street A	MH 102	MH 104				0.00	0.00	10.00	104.19	0.00	600	609.6	0.50	24.3	453.40	1.55	0.26	453.40	0.00
Street A	MH 104	MH 106	A15		0.105	0.19	0.19	10.26	102.83	19.51	600	609.6	0.30	28.2	351.20	1.20	0.39	331.69	0.06
Street A	MH 106	MH 108	A16-A18		0.646	1.17	1.71	12.14	94.10	160.86	600	609.6	0.20	47.2	286.76	0.98	0.80	125.89	0.56
Street A	MH 108	MH 110	A19-A21		0.469	0.85	2.56	12.94	90.86	232.32	675	685.8	0.15	55.4	339.98	0.92	1.00	107.66	0.68
Jockvale	MH 110	MH 610	A22		0.060	0.11	4.95	13.06	90.41	447.50	750	762.0	0.21	76.5	532.76	1.17	1.09	85.27	0.84
Street F	MH 902	MH 904				0.00	0.00	10.00	104.19	0.00	600	609.6	0.31	37.7	357.01	1.22	0.51	357.01	0.00
Street F	MH 904	MH 906	A23		0.457	0.83	0.83	10.51	101.55	83.86	600	609.6	0.20	40.1	286.76	0.98	0.68	202.89	0.29
Street F	MH 906	MH 908	A24		0.209	0.38	1.20	11.20	98.28	118.27	600	609.6	0.20	70.5	286.76	0.98	1.20	168.48	0.41
Street F	MH 908	MH 610	A25, A26		0.662	1.20	2.40	12.39	93.05	223.30	675	685.8	0.15	67.3	339.98	0.92	1.22	116.68	0.66
Jockvale	MH 618	MH 610	A27		0.190	0.34	0.34	10.00	104.19	35.77	600	609.6	0.30	41.1	351.20	1.20	0.57	315.43	0.10
Half Moon Bay	MH 610	MH 312	A28, A29		0.350	0.63	8.32	14.15	86.41	719.38	975	990.6	0.14	79.4	875.66	1.14	1.17	156.28	0.82
Pathway Blk	MH 312	MH 314	A30		0.498	0.90	11.87	15.31	82.56	979.66	1200	1219.2	0.10	58.1	1287.49	1.10	0.88	307.84	0.76
Pathway Blk	MH 314	MH 316				0.00	11.87	16.19	79.89	948.03	1200	1219.2	0.12	120.0	1410.38	1.21	1.66	462.35	0.67
Street H	MH 616	MH 614	A31		0.061	0.11	0.11	10.00	104.19	11.48	450	457.2	0.20	73.5	133.15	0.81	1.51	121.67	0.09
Jockvale	MH 614	MH 612	A32		0.450	0.81	0.92	11.51	96.83	89.41	600	609.6	0.21	24.1	293.84	1.01	0.40	204.43	0.30
Apartment Blk	MH 612	MH 316	A33		0.816	1.47	2.40	11.91	95.08	227.98	600	609.6	0.28	81.4	339.29	1.16	1.17	111.31	0.67
Outlet	MH 316	MH 324				0.00	14.26	17.85	75.35	1074.86	1200	1219.2	0.10	15.3	1287.49	1.10	0.23	212.63	0.83
Outlet	MH 324	MH 326				0.00	14.26	18.08	74.76	1066.46	1200	1219.2	0.28	3.6	2154.39	1.84	0.03	1087.92	0.50
Outlet	MH 326	HW1				0.00	14.26	18.11	74.68	1065.29	1200	1219.2	0.10	47.0	1287.49	1.10	0.71	222.20	0.83

STREET	FROM	TO	AREA ID	FLOW							PROPOSED SEWER									Q/Qfull
				R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)	EXCESS CAPACITY (l/s)		
Street B	MH200	MH202	B01, B02		0.241	0.44	0.44	10.00	104.19	45.37	675	685.8	0.20	37.4	392.57	1.06	0.59	347.20	0.12	
Street B	MH202	MH204	B03, B04		0.922	1.67	2.10	10.59	101.19	212.66	675	685.8	0.20	82.6	392.57	1.06	1.30	179.92	0.54	
Street B	MH204	MH206	B05, B06, B07		0.267	0.48	2.58	11.88	95.19	245.98	750	762.0	0.20	120.0	519.92	1.14	1.76	273.94	0.47	
Street B	MH206	MH208	B08, B09, B10	1.115	0.470	1.78	4.36	13.64	88.23	384.95	750	762.0	0.20	38.5	519.92	1.14	0.56	134.98	0.74	
Caivan Lands	CAP	MH208	B12		0.088	0.16	0.16	10.00	104.19	16.57	450	457.2	0.40	12.0	188.30	1.15	0.17	171.73	0.09	
Street B	MH208	MH210	B11		0.049	0.09	4.45	14.20	86.22	383.84	750	762.0	0.15	29.3	450.27	0.99	0.50	66.42	0.85	
Caivan Lands	CAP	MH210	B13		0.097	0.18	0.18	10.00	104.19	18.26	300	304.8	0.40	12.0	63.87	0.87	0.23	45.60	0.29	
Street B	MH210	MH212	B14		0.320	0.58	5.03	14.70	84.54	425.26	750	762.0	0.15	28.5	450.27	0.99	0.48	25.01	0.94	
Caivan Lands	CAP	MH212	B15		0.120	0.22	0.22	10.00	104.19	22.59	300	304.8	0.50	12.0	71.41	0.98	0.20	48.81	0.32	
Street B	MH212	MH214			0.00	5.03	15.18	82.98	417.39	825	838.2	0.15	35.5	580.56	1.05	0.56	163.17	0.72		
Caivan Lands	CAP	MH214	B16		0.136	0.25	0.25	10.00	104.19	25.61	300	304.8	0.50	12.0	71.41	0.98	0.20	45.80	0.36	
Street B	MH214	MH216			0.00	5.03	15.74	81.23	408.60	900	914.4	0.10	35.5	597.83	0.91	0.65	189.23	0.68		
Caivan Lands	CAP	MH216	B18		0.121	0.22	0.22	10.00	104.19	22.78	300	304.8	0.50	12.0	71.41	0.98	0.20	48.62	0.32	
Street B	MH216	MH218	B17		0.274	0.50	5.53	16.39	79.31	438.20	975	990.6	0.10	36.1	740.07	0.96	0.63	301.87	0.59	
Caivan Lands	CAP	MH218	B19, Caivan Lands		7.607	13.75	13.75	10.00	104.19	1432.22	975	990.6	0.50	12.0	1654.85	2.15	0.09	222.63	0.87	
Outlet	MH218	H2			0.00	5.53	17.02	77.55	428.48	1500	1524.0	0.14	79.8	2762.07	1.51	0.88	2333.60	0.16		

Definitions

Q = 2.78 AIR

Q = Peak Flow, in Litres per second (L/s)

A = Area in hectares (ha)

I = Rainfall Intensity (mm/h)

Notes:

1) Ottawa Rainfall-Intensity Curve

2) Min Velocity = 0.80 m/sec.

Includes the remaining 7.5ha portion of the Caivan Lands not included in Area IDs B01-B19 (4.33ha). The total area of the Caivain Lands is 11.83ha.

Appendix B
Sanitary Report Excerpts



City of Ottawa
South Nepean Collector

Figure 01
Existing Sanitary Network and Collection Areas

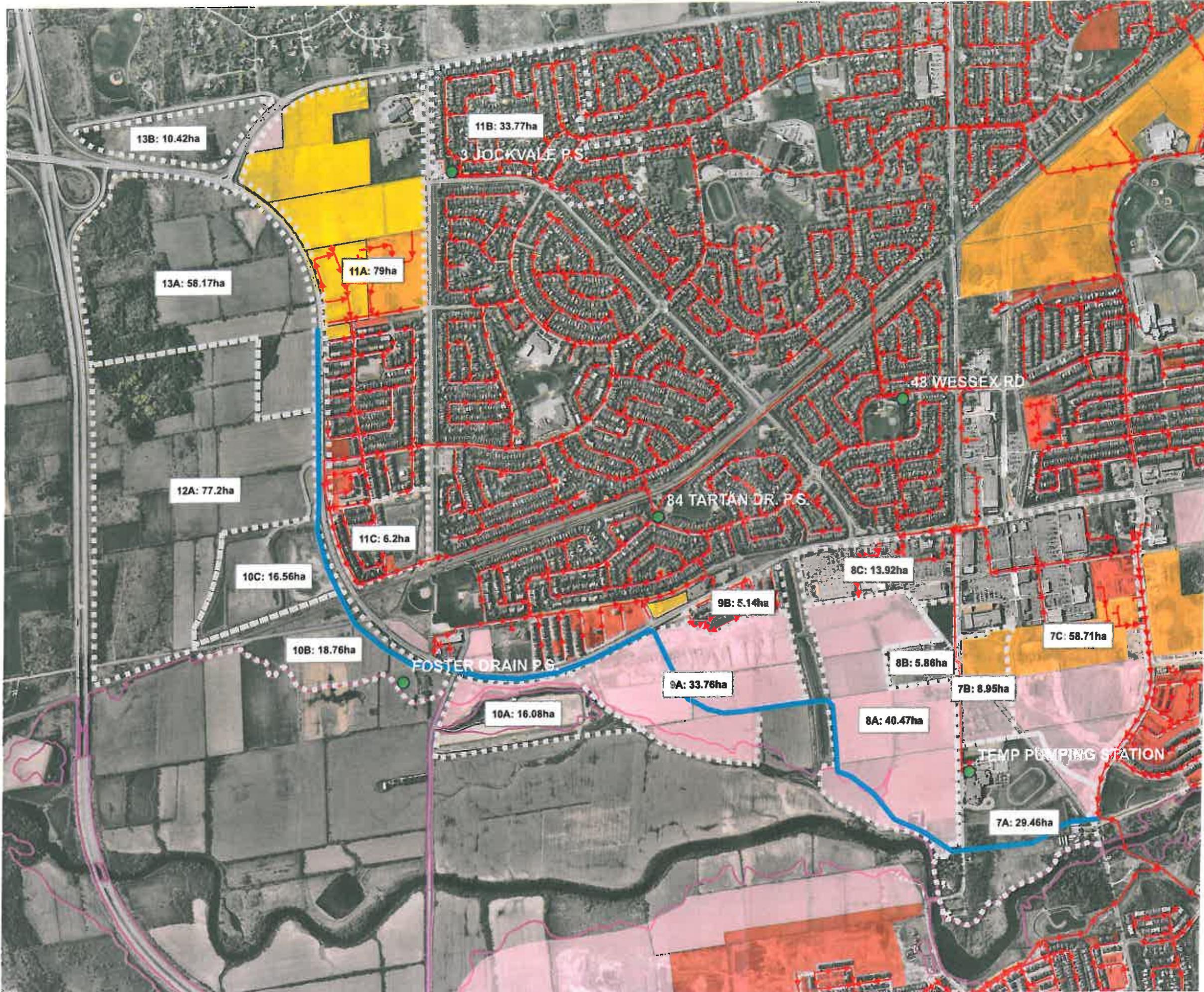


Table 5.1: Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area

Collection Area	Discharging Node	Estimated from GIS			City of Ottawa VURL Data			Other Space ¹ (ha)	Population (PE)	Residential Density (PE/net ha)	Comments	Additional Source(s)
		Gross Institutional/Commercial (ha)	Gross Residential (ha)	Gross Area (ha)	Net Residential (ha)	Units (#)	Unit Density (#/ha)					
7A	70	13.5	7.4	29.5	4.0	605	0.3	9.1	1637	4.25	Flow calculations include St Joseph H.S. Pump Station firm capacity of 7.0 L/s Additional 600 units (TAC)	3.4ppu (TAC)
7B		0.0	9.24	9.24	6.23	1474	136.7	3.0	3321	638.8	Population from split VURL allocated by area. VURL parcel id 323 - inconsistency between net and gross reported area.	2.7ppu (TAC)
8A		0.0	40.0	40.0	24.1	4462	185.1	15.9	12047.4	499.9		2.7ppu (TAC)
8B		5.9	0.0	5.9	0.0	0	0	0.0			Future Commercial area	
8C		13.9	0.0	13.9	0.0	0	0	0.0			Commercial area includes Home Depot	
9A	80	0.0	33.8	33.8	18.6	635	34.1	15.2	2210	116.2		3.4ppu (TAC)
10A	90	0.0	16.1	16.1	9.7	451	28.0	6.4	1533.4	158.0	Assume net population = 60% gross.	3.4ppu (TAC)
10B	100	18.8	0.0	35.3	0.0	0	0	16.5			Allocated as potential future I/C use as directed by TAC	
10C	110	16.6	0.0	35.3	0.0	0	0	18.7			Area includes current Municipal Snow Dump. Flow allowance is made for potential future I/C use	
11C		0.0	6.2	6.2	Note 2			2.5	306	82.7	This area is south of '11 block' in the existing development	From IBI Apr 2010 Report Figure 1
11A		12.5	66.5	79.0				26.6	3923	98.3	Institutional includes 4.38ha church site and 6.89 ha institution at northeast corner, as well and Claridge Commercial (0.56ha) and DCR/Phoenix Commercial (0.64ha)	From IBI Apr 2010 Report Figure 1
11B	120	0.0	37.0	37.0				14.8	1550	69.8	Presently serviced by Jockvale pump station; to be redirected to SNC.	Estimated from 2011 Census Block data
12A		77.2	0.0	77.2				0.0			Allow sanitary peak flow 79.0 L/s	Novatech, Employment Lands Report, Revised Jan 2012
13A		58.5	0.0	58.5				0.0			Allow sanitary peak flow 62.8 L/s plus Collection Area 13B, total 82.2 L/s	
13B	130	12.5	0.0	12.5				0.0			Allow sanitary peak flow 19.4 L/s; gravity discharge to Collection Area 13A	IBI/Novatech

Notes:

1. Other space includes other residential space accounting for the difference between gross area (measured with GIS) and net area (provided in VURL data), such as sidewalks, roads, greenspace, etc.
2. Collection Area 11A and 11B population and land use as identified under Additional Source(s). Other space reported as 60% of gross residential area, consistent with VURL average.

South Nepean Town Centre Community Design Plan



Planning and Growth Management Department
Community Planning and Design Division
July 2006
Publication #03-14

**OTTAWA CITY COUNCIL
28 AND 29 JUNE 2006
ANDREW S. HAYDON HALL
9:00 a.m.**

MINUTES 61

PLANNING AND ENVIRONMENT COMMITTEE REPORT 50

**13. SOUTH NEPEAN TOWN CENTRE COMMUNITY
DESIGN PLAN
AND OFFICIAL PLAN AMENDMENT**

COMMITTEE RECOMMENDATIONS

That Council:

1. Approve the South Nepean Town Centre Community Design Plan in Document 8, which has been submitted under separate cover.
2. Adopt Official Plan Amendment No. XX to the City of Ottawa Official Plan (2003), as detailed in Document 5, to implement the Community Design Plan.

BY-LAW NO. 2006 - 260

A by-law of the City of Ottawa to amend the Official Plan for the City of Ottawa to change the designation of the lands which are the subject of the South Nepean Town Centre Community Design Plan.

WHEREAS Planning and Environment Committee convened a public meeting to consider the adoption of the aforementioned official plan amendment;

AND WHEREAS Planning and Environment Committee recommends the adoption of the aforementioned official plan amendment;

AND WHEREAS Council on June 28, 2006 carried the recommendation of Planning and Environment Committee;

THEREFORE the Council of the City of Ottawa enacts as follows:

1. Attachment A, being Official Plan Amendment No. 44 to the Official Plan for the City of Ottawa is hereby adopted.
2. This by-law shall come into force in accordance with the provisions of the Planning Act, R.S.O. 1990, c.P.13, as amended.

ENACTED AND PASSED this 29th day of June, 2006.

CARRIED

CITY CLERK

MAYOR

Land Use

4.4 Policy Area – High Rise Residential

The High Rise Residential policy area identifies sites within the Town Centre that will accommodate the highest density residential uses, located in proximity to the transit “hub”. Apartment buildings will be the only residential type permitted.

Policies

For the High Rise Residential policy area:

- (1) Apartments are the only permitted use. As part of an apartment building, retail, office and commercial uses at grade are also permitted.
- (2) The minimum building height is 6 storeys and the maximum building height is 12 storeys.
- (3) The net density target for residential uses is 300 units per hectare.
- (4) At least 90% of required parking for each development must be provided in parking structures, either above-grade or below-grade.

- (2) The minimum building height is 2 storeys and the maximum building height is 4 storeys.
- (3) The net density target for residential uses is 100 units per hectare.

4.6 Policy Area – Neighbourhood Park

The Neighbourhood Park policy area will include both public parks and public plazas. The five public parks within the Town Centre will be designed to support the area’s urban nature, as accessible amenities with open frontages and clearly defined entrances that work within the grid pattern of streets and blocks. The two public plazas within the Town Centre will be designed as predominately hard surfaced areas fronted by buildings in order to create a built form edge and generate pedestrian activity.

Policies

For the Neighbourhood Park policy area:

- (1) Public parks, public plazas, community facilities, and conservation uses are permitted.
- (2) The locations of the five public parks and the two public plazas are identified on Schedule 5 of this CDP.
- (3) Acquisition of all neighbourhood parks will be as per Section 8.4 and Schedule 5 of this CDP.
- (4) Public parks must:
 - (a) Generally be between 0.4 and 1.0 hectares of level land;
 - (b) Have at least three sides that are entirely open to the street;
 - (c) Not have rear or side yards adjacent on their fourth side;
 - (d) Be designed with an emphasis on hard surfacing and seating areas as compared to traditional suburban neighbourhood parks; and
 - (e) Not contain sports fields.
- (5) Public plazas must:
 - (a) Generally be less than 0.4 hectares of level land;

4.5 Policy Area – Mid Rise Residential

The Mid Rise Residential policy area is intended to accommodate the majority of the Town Centre’s ground-oriented multiple unit dwellings. This policy area will provide an appropriate transition between the low density neighbourhoods surrounding the Town Centre to the higher intensity uses within it.

Policies

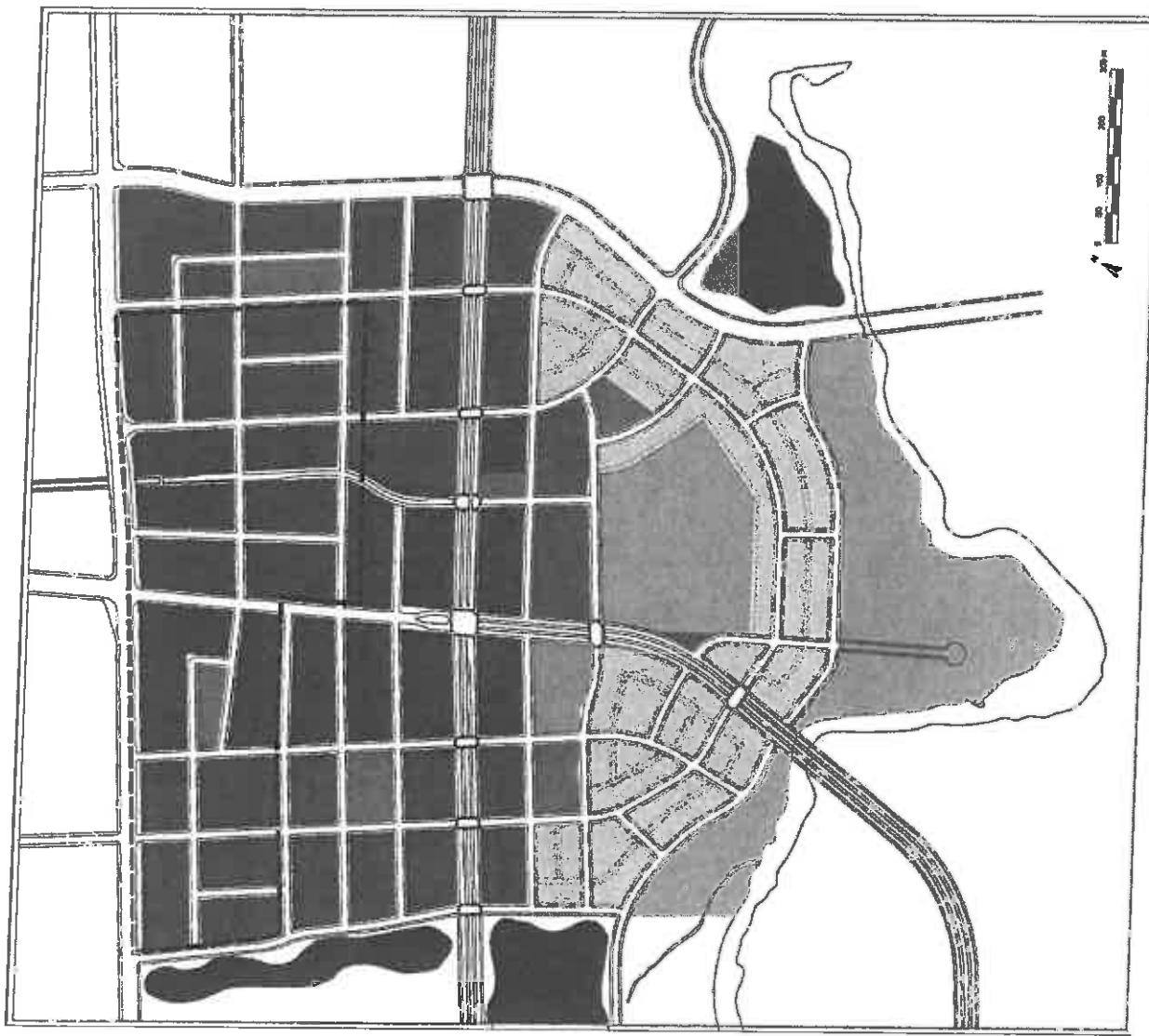
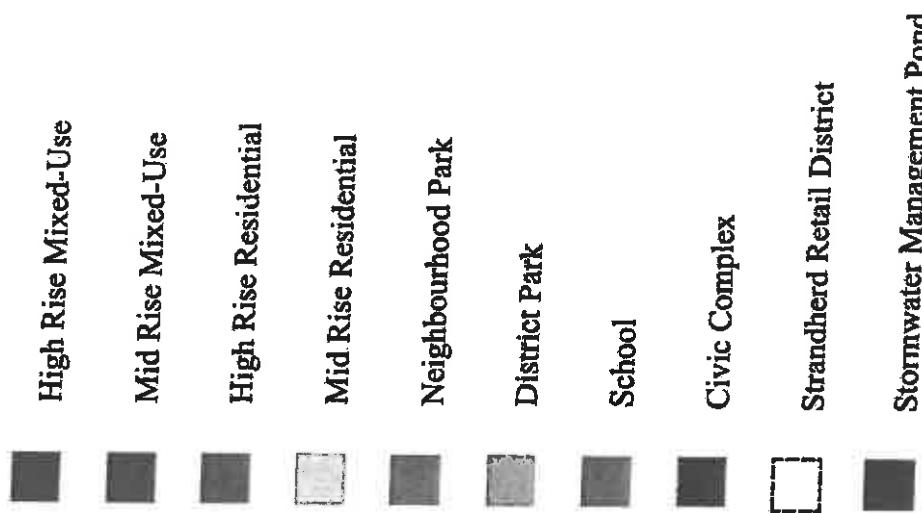
For the Mid Rise Residential policy area:

- (1) Apartments, street, block and stacked townhouses, public and institutional uses, schools, places of worship and community facilities are permitted. Ground floor retail uses within a mixed-use building with residential uses above are permitted along Greenbank Road.

Section 9.0

Schedules

Schedule 1 - Land Use Plan

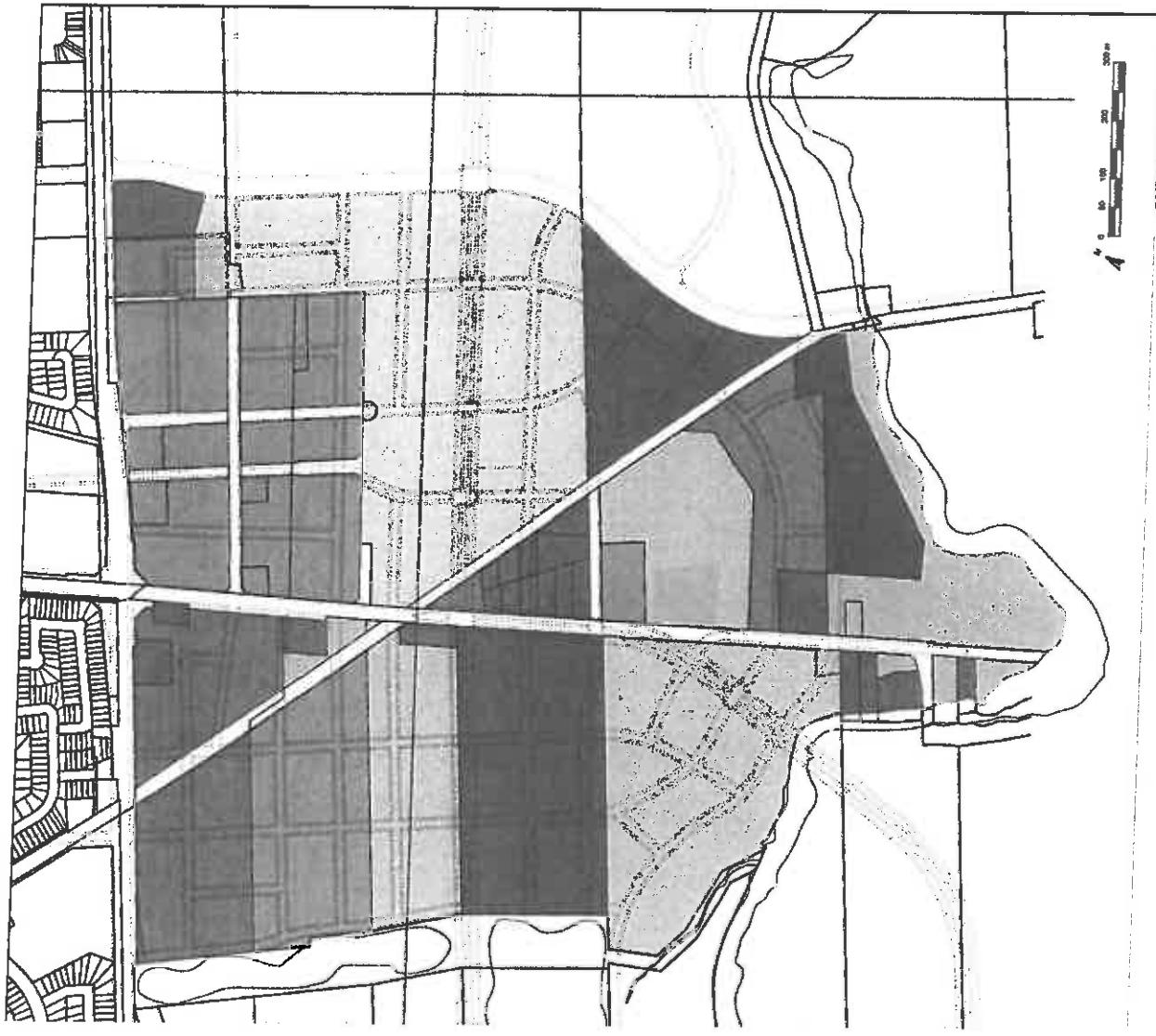


Appendix A

Property Boundaries

Note:

The colour of different parcels illustrates the boundaries of different parcels, or groups of parcels, and does not indicate land ownership.



Appendix C

Watermain Boundary Conditions, FUS Calculations, and
Modelling Results

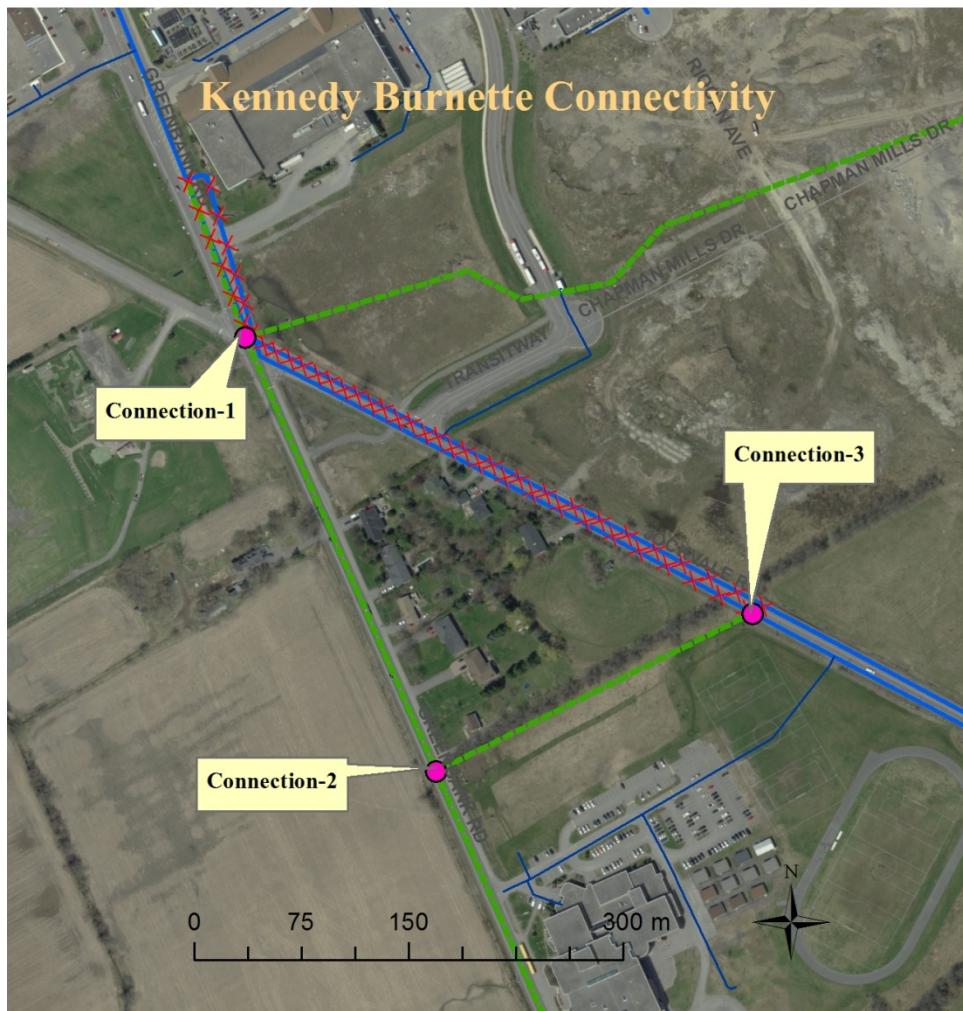
Boundary Conditions at Kennedy Brunette

Information Provided:

Date provided: 19 April 2016

For Residential and School	
Criteria	Demand (L/s)
Average Demand	2.7
Maximum Daily Demand	6.7
Peak Hourly Demand	14.8
Fire Flow Demand	250
Maximum Daily + Fire Flow Demand	256.7

Location:



Results:

Connection-1: Pre Zone Configuration

Criteria	Head (m)	Pressure (psi)
Max HGL	162.1	92.9
PKHR	138.5	59.4
MXDY + Fire Flow (250 L/s)	112.3	22.2

Connection-2:

Criteria	Head (m)	Pressure (psi)
Max HGL	161.7	95.3
PKHR	137.9	61.6
MXDY + Fire Flow (250 L/s)	110.5	22.6

Connection-3:

Criteria	Head (m)	Pressure (psi)
Max HGL	161.4	88.2
PKHR	137.6	54.4
MXDY + Fire Flow (250 L/s)	110.1	22.3

Results: Post zone-reconfiguration

Connection-1:

Criteria	Head (m)	Pressure (psi)
Max HGL	174.7	72.6
PKHR	146.3	70.5
MXDY + Fire Flow (250 L/s)	145.4	69.2

Connection-2:

Criteria	Head (m)	Pressure (psi)
Max HGL	147.7	75.5
PKHR	145.7	72.7
MXDY + Fire Flow (250 L/s)	144.3	70.6

Connection-3:

Criteria	Head (m)	Pressure (psi)
Max HGL	147.7	75.8
PKHR	145.6	72.8
MXDY + Fire Flow (250 L/s)	144.1	70.6

Considerations:

1. According to the City of Ottawa Water Design Guidelines as well as the Ontario Building Code, the maximum pressure at any point within a distribution system shall not exceed 80 psi in occupied areas. In scenario-2, measures should be taken to try to reduce the residual pressure below 80 psi without the use of special pressure control equipment. In circumstances where the residual pressure cannot be reduced below 80 psi without the use of pressure control equipment, a pressure reducing valve (**PRV**) should be installed at site.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 3

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	440		880			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	> 45.1m	4,675	0%			
		East Side	0 - 3 m		25%			
		South Side	30.1 - 45 m		5%			
		West Side	0 - 3 m		25%			
				Cumulative Total	55%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	13,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or	L/s 217			
				or	USGPM 3,435			
Required Duration of Fire Flow (hours)				Hours	2.5			
Required Volume of Fire Flow (m ³)				m ³	1950			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 5

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	415		830			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	20.1 - 30 m	10%	4,250			
		East Side	20.1 - 30 m	10%				
		South Side	30.1- 45 m	5%				
		West Side	0 - 3 m	25%				
				Cumulative Total	50%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	13,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	217			
				or USGPM	3,435			
Required Duration of Fire Flow (hours)				Hours	2.5			
Required Volume of Fire Flow (m ³)				m ³	1950			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 6

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	506		1,012			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
	F	Base fire flow without reductions			10,000			
		$F = 220 C (A)^{0.5}$						
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	3.1 - 10 m	20%	5,525			
		East Side	10.1 - 20 m					
		South Side	3.1 - 10 m					
		West Side	20.1 - 30 m	10%				
		Cumulative Total		65%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	14,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	233			
				or USGPM	3,699			
Required Duration of Fire Flow (hours)				Hours	3			
Required Volume of Fire Flow (m ³)				m ³	2520			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 7

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	678		1,356			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					12,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 10,200			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	> 45.1m	0% 25% 10% 25%	6,120			
		East Side	0 - 3 m					
		South Side	20.1 - 30 m					
		West Side	0 - 3 m					
				Cumulative Total	60%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	16,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	267			
				or USGPM	4,227			
Required Duration of Fire Flow (hours)				Hours	3.5			
Required Volume of Fire Flow (m ³)				m ³	3360			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 9

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	252		504			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					7,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 5,950			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	30.1 - 45 m	5%	2,678			
		East Side	10.1 - 20 m	15%				
		South Side	3.1 - 10 m	20%				
		West Side	30.1 - 45 m	5%				
		Cumulative Total		45%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	9,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	150			
				or USGPM	2,378			
Required Duration of Fire Flow (hours)				Hours	2			
Required Volume of Fire Flow (m ³)				m ³	1080			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 10

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	415		830			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	30.1- 45 m	5% 0% 25% 15%	3,825			
		East Side	> 45.1m					
		South Side	0 - 3 m					
		West Side	10.1 - 20 m					
		Cumulative Total		45%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	12,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	200			
				or USGPM	3,170			
Required Duration of Fire Flow (hours)				Hours	2.5			
Required Volume of Fire Flow (m ³)				m ³	1800			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 13

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	275		550			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					8,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 6,800			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	3.1 - 10 m	20% 0% 25% 15%	4,080			
		East Side	> 45.1m					
		South Side	0 - 3 m					
		West Side	10.1 - 20 m					
		Cumulative Total		60%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	11,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	183			
				or USGPM	2,906			
Required Duration of Fire Flow (hours)				Hours	2			
Required Volume of Fire Flow (m ³)				m ³	1320			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 16

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	575		1,150			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					11,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 9,350			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	3.1 - 10 m		4,675			
		East Side	10.1 - 20 m					
		South Side	30.1- 45 m					
		West Side	20.1 - 30 m					
				Cumulative Total	50%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	14,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	233			
				or USGPM	3,699			
Required Duration of Fire Flow (hours)				Hours	3			
Required Volume of Fire Flow (m ³)				m ³	2520			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 20

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	575		1,150			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					11,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 9,350			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	3.1 - 10 m		4,675			
		East Side	20.1 - 30 m					
		South Side	30.1- 45 m					
		West Side	10.1 - 20 m					
				Cumulative Total	50%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	14,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	233			
				or USGPM	3,699			
Required Duration of Fire Flow (hours)				Hours	3			
Required Volume of Fire Flow (m ³)				m ³	2520			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 22

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	450		900			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	0 - 3 m	25%	5,100			
		East Side	30.1 - 45 m	5%				
		South Side	3.1 - 10 m	20%				
		West Side	20.1 - 30 m	10%				
				Cumulative Total	60%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	14,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	233			
				or USGPM	3,699			
Required Duration of Fire Flow (hours)				Hours	3			
Required Volume of Fire Flow (m ³)				m ³	2520			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 25 (6 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	500		1,000			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
	F	Base fire flow without reductions			10,000			
		$F = 220 C (A)^{0.5}$						
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	0 - 3 m	25%	6,375			
		East Side	20.1 - 30 m	10%				
		South Side	0 - 3 m	25%				
		West Side	10.1 - 20 m	15%				
		Cumulative Total		75%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	15,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	250			
				or USGPM	3,963			
Required Duration of Fire Flow (hours)				Hours	3			
Required Volume of Fire Flow (m ³)				m ³	2700			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 32(6 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	678		1,356			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					12,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 10,200			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	> 45.1m	0% 25% 10% 25%	6,120			
		East Side	0 - 3 m					
		South Side	20.1 - 30 m					
		West Side	0 - 3 m					
				Cumulative Total	60%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	16,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	267			
				or USGPM	4,227			
Required Duration of Fire Flow (hours)				Hours	3.5			
Required Volume of Fire Flow (m ³)				m ³	3360			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 36

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	415		830			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	30.1 - 45 m	5%	4,250			
		East Side	10.1 - 20 m					
		South Side	30.1 - 45 m					
		West Side	0 - 3 m	25%				
				Cumulative Total	50%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	13,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or	217			
				or	USGPM 3,435			
Required Duration of Fire Flow (hours)				Hours	2.5			
Required Volume of Fire Flow (m ³)				m ³	1950			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 39

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	442		884			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	3.1 - 10 m	20%	4,675			
		East Side	20.1 - 30 m					
		South Side	0 - 3 m					
		West Side	> 45.1m	0%				
		Cumulative Total		55%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	13,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	217			
				or USGPM	3,435			
Required Duration of Fire Flow (hours)				Hours	2.5			
Required Volume of Fire Flow (m ³)				m ³	1950			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 41 (6 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	443		886			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					10,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	8,500			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	20.1 - 30 m	10%	5,100			
		East Side	0 - 3 m					
		South Side	> 45.1m					
		West Side	0 - 3 m					
				Cumulative Total	60%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	14,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or	L/s 233			
				or	USGPM 3,699			
Required Duration of Fire Flow (hours)				Hours	3			
Required Volume of Fire Flow (m ³)				m ³	2520			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 43

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	286		572			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					8,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 6,800			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	> 45.1m	0% 25% 0% 0%	1,700			
		East Side	0 - 3 m					
		South Side	> 45.1m					
		West Side	> 45.1m					
				Cumulative Total	25%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	9,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	150			
				or USGPM	2,378			
Required Duration of Fire Flow (hours)				Hours	2			
Required Volume of Fire Flow (m ³)				m ³	1080			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description:Node 46

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	375		750			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					9,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 7,650			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
				Cumulative Total	0%			
Exposure surcharge (cumulative (%))								
5	(3)	North Side	> 45.1m		1,530			
		East Side	> 45.1m					
		South Side	30.1- 45 m	5%				
		West Side	10.1 - 20 m	15%				
				Cumulative Total	20%			
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	9,000			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	150			
				or USGPM	2,378			
Required Duration of Fire Flow (hours)				Hours	2			
Required Volume of Fire Flow (m ³)				m ³	1080			

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 48 (4 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)			
Required Fire Flow								
Construction Material								
1	Coefficient related to type of construction C	Wood frame	Yes	1.5	1.5			
		Ordinary construction		1				
		Non-combustible construction		0.8				
		Fire resistive construction (< 3 hrs)		0.7				
		Fire resistive construction (> 3 hrs)		0.6				
Floor Area								
2	A	Building Footprint (m ²)	270		540			
		Number of Floors/Storeys	2					
		Area of structure considered (m ²)						
F								
Base fire flow without reductions					8,000			
$F = 220 C (A)^{0.5}$								
Reductions or Surcharges								
Occupancy hazard reduction or surcharge								
3	(1)	Non-combustible		-25%	-15% 6,800			
		Limited combustible	Yes	-15%				
		Combustible		0%				
		Free burning		15%				
		Rapid burning		25%				
Sprinkler Reduction								
4	(2)	Adequately Designed System (NFPA 13)	No	-30%	0			
		Standard Water Supply	No	-10%				
		Fully Supervised System	No	-10%				
		Cumulative Total		0%				
Exposure surcharge (cumulative (%))								
5	(3)	North Side	> 45.1m		2,720			
		East Side	10.1 - 20 m					
		South Side	0 - 3 m					
		West Side	> 45.1m					
		Cumulative Total		40%				
(1) + (2) + (3)								
Total Required fire Flow, rounded to nearest 1000L/min				L/min	9,520			
(2,000 L/min < Fire Flow < 45,000 L/min)				or L/s	159			
				or USGPM	2,515			
Required Duration of Fire Flow (hours)				Hours	2			
Required Volume of Fire Flow (m ³)				m ³	1142.4			

Table 1
Watermain Demand Calculations

Node	Unit		Pop.	Demand (L/s)		
	Town	Condo		High Pres.	Max Daily	Peak Hour
1	0	0	0	0.00	0.00	0.00
2	10	0	27	0.11	0.27	0.60
3	0	0	0	0.00	0.00	0.00
4	10	0	27	0.11	0.27	0.60
5	0	0	0	0.00	0.00	0.00
6	8	0	22	0.09	0.22	0.49
7	10	0	27	0.11	0.27	0.60
8	0	0	0	0.00	0.00	0.00
9	0	0	0	0.00	0.00	0.00
10	0	0	0	0.00	0.00	0.00
11	0	0	0	0.00	0.00	0.00
12	10	0	27	0.11	0.27	0.60
13	16	0	44	0.18	0.45	0.98
14	10	0	27	0.11	0.27	0.60
15	10	0	27	0.11	0.27	0.60
16	0	0	0	0.00	0.00	0.00
17	12	0	33	0.13	0.33	0.74
18	0	140	252	1.02	2.55	5.61
19	0	0	0	0.00	0.00	0.00
20	4	0	11	0.04	0.11	0.25
21	6	0	17	0.07	0.17	0.38
22	6	0	17	0.07	0.17	0.38
23	9	0	25	0.10	0.25	0.56
24	8	0	22	0.09	0.22	0.49
25	10	0	27	0.11	0.27	0.60
26	6	0	17	0.07	0.17	0.38
27	7	0	19	0.08	0.19	0.42
28	3	0	9	0.04	0.09	0.20
29	6	0	17	0.07	0.17	0.38

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

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Table 1
Watermain Demand Calculations

Node	Unit		Pop.	Demand (L/s)		
	Town	Condo		High Pres.	Max Daily	Peak Hour
30	6	0	17	0.07	0.17	0.38
31	6	0	17	0.07	0.17	0.38
32	11	0	30	0.12	0.30	0.67
33	9	0	25	0.10	0.25	0.56
34	0	0	0	0.00	0.00	0.00
35	0	0	0	0.00	0.00	0.00
36	10	0	27	0.11	0.27	0.60
37	0	0	0	0.00	0.00	0.00
38	10	0	27	0.11	0.27	0.60
39	0	0	0	0.00	0.00	0.00
40	10	0	27	0.11	0.27	0.60
41	10	0	27	0.11	0.27	0.60
42	0	0	0	0.00	0.00	0.00
43	0	0	0	0.00	0.00	0.00
44	0	0	0	0.00	0.00	0.00
45	0	0	0	0.00	0.00	0.00
46	0	0	0	0.00	0.00	0.00
47	4	0	11	0.04	0.11	0.25
48	0	0	0	0.00	0.00	0.00
49	8	0	22	0.09	0.22	0.49
50	0	280	504	2.04	5.10	11.23
				5.78	14.46	31.82

1. Population density: 2.7 people/town
2. High Pressure demand = 350L/s/p/d
3. Maximum Daily demand = 2.5 x High Pressure Demand
4. Peak Hour Demand = 2.2 x Maximum Daily Demand

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Date: November 14, 2016

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Table 2
Pipe Data

Pipe	Length (m)	Diameter (mm)	Roughness
1	43	250	110
2	31	250	110
3	32	250	110
4	56	250	110
5	43	250	110
6	50	250	110
7	62	250	110
8	17	250	110
9	50	250	110
10	36	250	110
11	30	250	110
12	45	250	110
13	47	250	110
14	43	250	110
15	20	250	110
16	25	250	110
17	34	250	110
18	26	300	120
19	14	300	120
20	54	300	120
21	33	300	120
22	48	300	120
23	21	300	120
24	44	250	110
25	71	250	110
26	61	250	110
27	34	250	110
28	33	250	110
29	34	250	110

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Date:November 14 2016

Table 2
Pipe Data

Pipe	Length (m)	Diameter (mm)	Roughness
30	18	250	110
31	42	250	110
32	31	250	110
33	27	250	110
34	46	250	110
35	29	250	110
36	26	250	110
37	7	300	120
38	59	250	110
39	56	250	110
40	71	250	110
41	46	250	110
42	47	250	110
43	36	250	110
44	75	250	110
45	45	250	110
46	73	250	110
47	27	250	110
48	270	400	120
50	54	300	120
51	61	250	110
52	69	250	110
53	51	250	110
54	42	250	110
55	34	300	120
56	13	300	120
57	50	250	110
58	41	300	120
59	36	300	120

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Date:November 14 2016

Table 3
Pre Configuration Condition
High Pressure Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
1	93.5	0.00	161.7	68.2	96.7	68
2	93.6	0.11	161.7	68.1	96.6	50
3	93.8	0.00	161.7	67.9	96.3	27
4	93.6	0.11	161.7	68.1	96.6	21
5	93.9	0.00	161.7	67.8	96.1	17
6	93.7	0.09	161.7	68.0	96.4	7
7	93.8	0.11	161.7	67.9	96.3	4
8	94.0	0.00	161.7	67.7	96.0	2
9	93.8	0.00	161.7	67.9	96.3	2
10	94.1	0.00	161.7	67.6	95.9	1
11	94.5	0.00	161.7	67.2	95.3	0
12	94.5	0.11	161.7	67.2	95.3	1
13	94.4	0.18	161.7	67.3	95.4	2
14	94.0	0.11	161.7	67.7	96.0	3
15	93.8	0.11	161.7	67.9	96.3	4
16	93.7	0.00	161.7	68.0	96.4	5
17	93.6	0.13	161.7	68.1	96.6	6
18	93.6	1.02	161.7	68.1	96.6	4
19	93.3	0.00	161.7	68.4	97.0	6
20	93.5	0.04	161.7	68.2	96.7	6
21	93.4	0.07	161.7	68.3	96.8	8
22	93.6	0.07	161.7	68.1	96.6	9
23	93.6	0.10	161.7	68.1	96.6	12
24	93.8	0.09	161.7	67.9	96.3	2
25	93.8	0.11	161.7	67.9	96.3	4
26	93.4	0.07	161.7	68.3	96.8	7
27	94.1	0.08	161.7	67.6	95.9	3
28	93.8	0.04	161.7	67.9	96.3	5
29	93.7	0.07	161.7	68.0	96.4	21

Table 3
Pre Configuration Condition
High Pressure Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
30	93.8	0.07	161.7	67.9	96.3	11
31	93.8	0.07	161.7	67.9	96.3	6
32	93.9	0.12	161.7	67.8	96.1	10
33	93.8	0.10	161.7	67.9	96.3	19
34	93.6	0.00	161.7	68.1	96.6	14
35	93.7	0.00	161.7	68.0	96.4	14
36	93.7	0.11	161.7	68.0	96.4	19
37	93.2	0.00	161.7	68.5	97.1	9
38	93.2	0.11	161.7	68.5	97.1	11
39	93.3	0.00	161.7	68.4	97.0	14
40	93.3	0.11	161.7	68.4	97.0	17
41	93.4	0.11	161.7	68.3	96.8	35
42	93.6	0.00	161.7	68.1	96.6	31
43	93.6	0.00	161.7	68.1	96.6	58
44	94.1	0.00	161.7	67.6	95.9	3
45	94.3	2.04	161.7	67.4	95.6	3
46	94.0	0.00	161.7	67.7	96.0	23
47	94.0	0.04	161.7	67.7	96.0	39
48	94.5	0.00	161.7	67.2	95.3	116
49	94.4	0.09	161.7	67.3	95.4	28
50	93.8	0.00	161.7	67.9	96.3	36
51*	N/A	N/A	161.7	N/A	N/A	N/A

* Boundary Condition

[] Maximum Pressure

[] Maximum Time

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Date: November 14, 2016

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Table 4a
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 3

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	109.2	15.7	22.3
2	93.6	0.3	108.8	15.2	21.5
3	93.8	167.0	108.5	14.7	20.8
4	93.6	0.3	109.1	15.5	22.0
5	93.9	0.0	110.4	16.5	23.3
6	93.7	0.2	110.5	16.8	23.8
7	93.8	0.3	110.7	16.9	23.9
8	94.0	0.0	110.9	16.9	23.9
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.2
29	93.7	0.2	110.9	17.2	24.3

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4a
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 3

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.4	16.7	23.6
37	93.2	0.0	111.0	17.8	25.3
38	93.2	0.3	110.9	17.7	25.0
39	93.3	0.0	110.7	17.4	24.7
40	93.3	0.3	110.6	17.3	24.5
41	93.4	0.3	110.4	17.0	24.0
42	93.6	0.0	110.2	16.6	23.6
43	94.1	0.0	111.8	17.7	25.1
44	93.6	0.0	109.5	15.9	22.5
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.4	16.6	23.5
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4b
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 5

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	110.4	16.9	23.9
2	93.6	0.3	110.3	16.7	23.7
3	93.8	0.0	110.2	16.4	23.3
4	93.6	0.3	110.2	16.6	23.5
5	93.9	167.0	110.1	16.2	23.0
6	93.7	0.2	110.3	16.6	23.6
7	93.8	0.3	110.5	16.7	23.7
8	94.0	0.0	110.8	16.8	23.9
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.3
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.8	16.7	23.7
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.3

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4b
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 5

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.6	16.7	23.7
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.5	16.9	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.5	16.8	23.8
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.2
39	93.3	0.0	110.9	17.6	24.9
40	93.3	0.3	110.8	17.5	24.8
41	93.4	0.3	110.6	17.2	24.4
42	93.6	0.0	110.5	16.9	24.0
43	93.6	0.0	110.4	16.8	23.8
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.2	16.4	23.3
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4c
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 6

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	110.5	17.0	24.2
2	93.6	0.3	110.5	16.9	24.0
3	93.8	0.0	110.5	16.7	23.6
4	93.6	0.3	110.4	16.8	23.9
5	93.9	0.0	110.4	16.5	23.4
6	93.7	167.2	109.5	15.8	22.4
7	93.8	0.3	110.1	16.3	23.0
8	94.0	0.0	110.7	16.7	23.7
9	93.8	0.0	110.9	17.1	24.2
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4c
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 6

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.3
39	93.3	0.0	110.9	17.6	25.0
40	93.3	0.3	110.9	17.6	24.9
41	93.4	0.3	110.7	17.3	24.6
42	93.6	0.0	110.6	17.0	24.2
43	93.6	0.0	110.6	17.0	24.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.6
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4d
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 7

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.6	16.8	23.9
4	93.6	0.3	110.6	17.0	24.1
5	93.9	0.0	110.6	16.7	23.7
6	93.7	0.2	110.1	16.4	23.2
7	93.8	167.3	109.5	15.7	22.2
8	94.0	0.0	110.6	16.6	23.6
9	93.8	0.0	110.8	17.0	24.1
10	94.1	0.0	111.3	17.2	24.3
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.9	17.3	24.5
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4d
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 7

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.7	17.2	24.4
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.3
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.1	17.9	25.4
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.7	17.1	24.2
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.6	16.8	23.9
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4e
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 9

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.0	17.5	24.8
2	93.6	0.3	111.0	17.4	24.7
3	93.8	0.0	111.0	17.2	24.4
4	93.6	0.3	111.0	17.4	24.7
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	110.9	17.2	24.3
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.6	16.6	23.5
9	93.8	167.0	110.3	16.5	23.4
10	94.1	0.0	111.2	17.1	24.2
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.2
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.6
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.2	17.4	24.7
25	93.8	0.3	111.3	17.5	24.7
26	93.4	0.2	111.3	17.9	25.4
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4e
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 9

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	111.0	17.1	24.2
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.7
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	111.0	17.3	24.6
37	93.2	0.0	111.3	18.1	25.6
38	93.2	0.3	111.2	18.0	25.6
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.1	17.8	25.3
41	93.4	0.3	111.1	17.7	25.1
42	93.6	0.0	111.0	17.4	24.7
43	93.6	0.0	111.0	17.4	24.7
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.0	17.2	24.4
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4f
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 10

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.3	17.8	25.3
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.7
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.2	17.4	24.7
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.1	17.3	24.5
10	94.1	167.0	110.9	16.8	23.8
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	111.5	17.0	24.1
13	94.4	0.5	111.5	17.1	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.6	17.8	25.2
16	93.7	0.0	111.6	17.9	25.3
17	93.6	0.3	111.6	18.0	25.5
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.4	18.1	25.7
20	93.5	0.1	111.4	17.9	25.4
21	93.4	0.2	111.4	18.0	25.5
22	93.6	0.2	111.4	17.8	25.2
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	111.0	17.2	24.4
25	93.8	0.3	111.2	17.4	24.6
26	93.4	0.2	111.4	18.0	25.5
27	94.1	0.2	111.2	17.1	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	25.0

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4f
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 10

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.3	17.5	24.9
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.7
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	25.0
36	93.7	0.3	111.3	17.6	25.0
37	93.2	0.0	111.4	18.2	25.8
38	93.2	0.3	111.4	18.2	25.8
39	93.3	0.0	111.4	18.1	25.7
40	93.3	0.3	111.4	18.1	25.6
41	93.4	0.3	111.4	18.0	25.5
42	93.6	0.0	111.3	17.7	25.2
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4g
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node13

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.6	18.1	25.6
2	93.6	0.3	111.6	18.0	25.5
3	93.8	0.0	111.6	17.8	25.2
4	93.6	0.3	111.6	18.0	25.5
5	93.9	0.0	111.6	17.7	25.0
6	93.7	0.2	111.6	17.9	25.3
7	93.8	0.3	111.6	17.8	25.2
8	94.0	0.0	111.6	17.6	24.9
9	93.8	0.0	111.6	17.8	25.2
10	94.1	0.0	111.5	17.4	24.7
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	110.9	16.4	23.2
13	94.4	167.5	109.9	15.5	22.0
14	94.0	0.3	110.4	16.4	23.2
15	93.8	0.3	110.8	17.0	24.1
16	93.7	0.0	111.0	17.3	24.5
17	93.6	0.3	111.2	17.6	25.0
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.6	18.3	25.9
20	93.5	0.1	111.6	18.1	25.6
21	93.4	0.2	111.6	18.2	25.8
22	93.6	0.2	111.6	18.0	25.5
23	93.6	0.3	111.6	18.0	25.5
24	93.8	0.2	111.5	17.7	25.2
25	93.8	0.3	111.6	17.8	25.2
26	93.4	0.2	111.6	18.2	25.8
27	94.1	0.2	111.6	17.5	24.8
28	93.8	0.1	111.6	17.8	25.2
29	93.7	0.2	111.6	17.9	25.3

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4g
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node13

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.6	17.8	25.2
31	93.8	0.2	111.6	17.8	25.2
32	93.9	0.3	111.6	17.7	25.0
33	93.8	0.3	111.6	17.8	25.2
34	93.6	0.0	111.6	18.0	25.5
35	93.7	0.0	111.6	17.9	25.3
36	93.7	0.3	111.6	17.9	25.3
37	93.2	0.0	111.6	18.4	26.0
38	93.2	0.3	111.6	18.4	26.0
39	93.3	0.0	111.6	18.3	25.9
40	93.3	0.3	111.6	18.3	25.9
41	93.4	0.3	111.6	18.2	25.8
42	93.6	0.0	111.6	18.0	25.5
43	93.6	0.0	111.6	18.0	25.5
44	94.1	0	111.84	17.74	25.2
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.6	17.8	25.2
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4h
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 16

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.5	18.0	25.6
2	93.6	0.3	111.5	17.9	25.4
3	93.8	0.0	111.5	17.7	25.1
4	93.6	0.3	111.5	17.9	25.4
5	93.9	0.0	111.5	17.6	25.0
6	93.7	0.2	111.5	17.8	25.3
7	93.8	0.3	111.5	17.7	25.1
8	94.0	0.0	111.5	17.5	24.9
9	93.8	0.0	111.5	17.7	25.2
10	94.1	0.0	111.6	17.5	24.8
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.3	16.8	23.9
13	94.4	0.5	111.0	16.6	23.5
14	94.0	0.3	110.6	16.6	23.5
15	93.8	0.3	110.2	16.4	23.3
16	93.7	167.0	110.1	16.4	23.2
17	93.6	0.3	110.7	17.1	24.2
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.5	18.2	25.8
20	93.5	0.1	111.5	18.0	25.6
21	93.4	0.2	111.5	18.1	25.7
22	93.6	0.2	111.5	17.9	25.4
23	93.6	0.3	111.5	17.9	25.4
24	93.8	0.2	111.6	17.8	25.2
25	93.8	0.3	111.5	17.7	25.1
26	93.4	0.2	111.5	18.1	25.7
27	94.1	0.2	111.5	17.4	24.7
28	93.8	0.1	111.5	17.7	25.1
29	93.7	0.2	111.5	17.8	25.3

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4h
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 16

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.5	17.7	25.1
31	93.8	0.2	111.5	17.7	25.1
32	93.9	0.3	111.5	17.6	25.0
33	93.8	0.3	111.5	17.7	25.1
34	93.6	0.0	111.5	17.9	25.4
35	93.7	0.0	111.5	17.8	25.3
36	93.7	0.3	111.5	17.8	25.3
37	93.2	0.0	111.5	18.3	26.0
38	93.2	0.3	111.5	18.3	26.0
39	93.3	0.0	111.5	18.2	25.8
40	93.3	0.3	111.5	18.2	25.8
41	93.4	0.3	111.5	18.1	25.7
42	93.6	0.0	111.5	17.9	25.4
43	93.6	0.0	111.5	17.9	25.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.5	17.7	25.1
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

 Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4i
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 20

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.1	17.6	25.0
2	93.6	0.3	111.1	17.5	24.9
3	93.8	0.0	111.1	17.3	24.6
4	93.6	0.3	111.1	17.5	24.9
5	93.9	0.0	111.1	17.2	24.4
6	93.7	0.2	111.2	17.5	24.8
7	93.8	0.3	111.2	17.4	24.6
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.4	17.3	24.6
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.4	17.8	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	167.1	111.0	17.5	24.8
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.1	17.5	24.8
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.2	17.1	24.2
28	93.8	0.1	111.1	17.3	24.6
29	93.7	0.2	111.1	17.4	24.7

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4i
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 20

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.6
32	93.9	0.3	111.1	17.2	24.4
33	93.8	0.3	111.1	17.3	24.6
34	93.6	0.0	111.1	17.5	24.8
35	93.7	0.0	111.1	17.4	24.7
36	93.7	0.3	111.1	17.4	24.7
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.2	18.0	25.5
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.2	17.9	25.3
41	93.4	0.3	111.1	17.7	25.2
42	93.6	0.0	111.1	17.5	24.9
43	93.6	0.0	111.1	17.5	24.9
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.7	17.4	24.7
46	94.0	0.0	111.7	17.7	25.2
47	94.0	0.1	111.7	17.7	25.2
48	94.5	0.0	111.7	17.2	24.4
49	94.4	0.2	111.7	17.3	24.6
50	93.8	0.0	111.1	17.3	24.6
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4j
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 22

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.8	17.3	24.5
2	93.6	0.3	110.8	17.2	24.3
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.3
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.2
7	93.8	0.3	110.9	17.1	24.2
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.1	17.6	25.0
21	93.4	0.2	110.8	17.4	24.6
22	93.6	167.2	110.5	16.9	23.9
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.2

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4j
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 22

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.8	17.0	24.1
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.2
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.8	17.2	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4k
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 25

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.3	17.8	25.2
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.6
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.3	17.5	24.8
8	94.0	0.0	111.3	17.3	24.5
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.2	17.1	24.3
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.3	17.9	25.4
22	93.6	0.2	111.3	17.7	25.1
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	110.6	16.8	23.9
25	93.8	167.3	109.7	15.9	22.5
26	93.4	0.2	110.7	17.3	24.5
27	94.1	0.2	111.3	17.2	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	24.9

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4k
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 25

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.3	17.5	24.8
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.6
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	24.9
36	93.7	0.3	111.3	17.6	24.9
37	93.2	0.0	111.3	18.1	25.7
38	93.2	0.3	111.3	18.1	25.7
39	93.3	0.0	111.3	18.0	25.5
40	93.3	0.3	111.3	18.0	25.5
41	93.4	0.3	111.3	17.9	25.4
42	93.6	0.0	111.3	17.7	25.1
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4I
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 32

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.2
5	93.9	0.0	110.7	16.8	23.8
6	93.7	0.2	110.7	17.0	24.1
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.8	16.8	23.8
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.7
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.6	16.8	23.8
29	93.7	0.2	110.6	16.9	24.0

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NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4I
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 32

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.8	17.0	24.0
31	93.8	0.2	110.2	16.4	23.3
32	93.9	167.3	109.6	15.7	22.3
33	93.8	0.3	110.2	16.4	23.2
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.7	17.1	24.3
43	93.6	0.0	110.7	17.1	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	23.9
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4m
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 36

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.2	16.7	23.6
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.3	16.5	23.4
4	93.6	0.3	110.4	16.8	23.8
5	93.9	0.0	110.5	16.6	23.6
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.3

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4m
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 36

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.9
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.9
36	93.7	167.3	109.3	15.6	22.1
37	93.2	0.0	111.0	17.8	25.2
38	93.2	0.3	110.8	17.6	24.9
39	93.3	0.0	110.6	17.3	24.5
40	93.3	0.3	110.4	17.1	24.3
41	93.4	0.3	110.1	16.7	23.7
42	93.6	0.0	109.9	16.3	23.2
43	93.6	0.0	110.1	16.5	23.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4n
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 39

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.6	17.1	24.2
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.8	17.2	24.4
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	111.0	17.3	24.5
7	93.8	0.3	111.1	17.3	24.5
8	94.0	0.0	111.1	17.1	24.3
9	93.8	0.0	111.2	17.4	24.7
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.1	17.0	24.1
28	93.8	0.1	111.1	17.3	24.5
29	93.7	0.2	111.1	17.4	24.7

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4n
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 39

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.5
32	93.9	0.3	111.0	17.1	24.3
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.6
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	109.9	16.7	23.7
38	93.2	0.3	109.1	15.9	22.6
39	93.3	167.0	108.3	15.0	21.3
40	93.3	0.3	108.8	15.5	21.9
41	93.4	0.3	109.8	16.4	23.2
42	93.6	0.0	110.3	16.7	23.7
43	93.6	0.0	110.5	16.9	24.0
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.7
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	111.0	17.2	24.3
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4o
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 41

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.1	16.6	23.5
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.4	16.6	23.5
4	93.6	0.3	110.5	16.9	24.0
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.3
7	93.8	0.3	110.9	17.1	24.3
8	94.0	0.0	111.0	17.0	24.2
9	93.8	0.0	111.1	17.3	24.6
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.0	17.6	25.0
22	93.6	0.2	110.9	17.3	24.6
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.0	16.9	24.0
28	93.8	0.1	111.0	17.2	24.4
29	93.7	0.2	111.0	17.3	24.5

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4o
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 41

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	110.9	17.0	24.0
33	93.8	0.3	110.8	17.0	24.1
34	93.6	0.0	110.8	17.2	24.4
35	93.7	0.0	110.7	17.0	24.2
36	93.7	0.3	110.2	16.5	23.4
37	93.2	0.0	110.6	17.4	24.6
38	93.2	0.3	110.1	16.9	24.0
39	93.3	0.0	109.7	16.4	23.2
40	93.3	0.3	109.4	16.1	22.8
41	93.4	167.3	108.7	15.3	21.7
42	93.6	0.0	109.7	16.1	22.8
43	93.6	0.0	110.0	16.4	23.2
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4p
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 43

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	108.7	15.2	21.5
2	93.6	0.3	109.1	15.5	22.0
3	93.8	0.0	109.5	15.7	22.2
4	93.6	0.3	109.9	16.3	23.1
5	93.9	0.0	110.5	16.6	23.5
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4p
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 43

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.3
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	23.9
36	93.7	0.3	110.2	16.5	23.3
37	93.2	0.0	110.9	17.7	25.1
38	93.2	0.3	110.7	17.5	24.8
39	93.3	0.0	110.5	17.2	24.4
40	93.3	0.3	110.3	17.0	24.1
41	93.4	0.3	110.0	16.6	23.5
42	93.6	0.0	109.8	16.2	23.0
43	93.6	167.0	108.4	14.8	20.9
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4q
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 46

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

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Table 4q
Pre Confirmation Condition
Max Daily Demand and Fire Flow at Node 46

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	167.0	110.2	16.2	22.9
47	94.0	0.1	110.3	16.3	23.2
48	94.5	0.0	110.5	16.0	22.7
49	94.4	0.2	110.7	16.3	23.1
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 4r
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 48

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

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Table 4r
Pre Configuration Condition
Max Daily Demand and Fire Flow at Node 48

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	0.0	110.6	16.6	23.5
47	94.0	0.1	109.9	15.9	22.5
48	94.5	167.0	109.1	14.6	20.6
49	94.4	0.2	110.0	15.6	22.2
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016 | File: 20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

Table 5
Pre Configuration Condition
Peak Hour Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.00	137.87	44.4	62.9
2	93.6	0.60	137.87	44.3	62.8
3	93.8	0.00	137.87	44.1	62.5
4	93.6	0.60	137.87	44.3	62.8
5	93.9	0.00	137.87	44.0	62.3
6	93.7	0.49	137.87	44.2	62.6
7	93.8	0.60	137.87	44.1	62.5
8	94.0	0.00	137.87	43.9	62.2
9	93.8	0.00	137.87	44.1	62.5
10	94.1	0.00	137.87	43.8	62.1
11	94.5	0.00	137.88	43.4	61.5
12	94.5	0.60	137.88	43.4	61.5
13	94.4	0.98	137.88	43.5	61.7
14	94.0	0.60	137.87	43.9	62.2
15	93.8	0.60	137.87	44.1	62.5
16	93.7	0.00	137.87	44.2	62.6
17	93.6	0.74	137.87	44.3	62.8
18	93.6	5.61	137.87	44.3	62.8
19	93.3	0.00	137.87	44.6	63.2
20	93.5	0.25	137.87	44.4	62.9
21	93.4	0.38	137.87	44.5	63.1
22	93.6	0.38	137.87	44.3	62.8
23	93.6	0.56	137.87	44.3	62.8
24	93.8	0.49	137.87	44.1	62.5
25	93.8	0.60	137.87	44.1	62.5
26	93.4	0.38	137.87	44.5	63.1
27	94.1	0.42	137.87	43.8	62.1
28	93.8	0.20	137.87	44.1	62.5
29	93.7	0.38	137.87	44.2	62.6

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 12, 2016 | Job No. 111117 | DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\PeakHour.xls

Table 5
Pre Configuration Condition
Peak Hour Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.38	137.87	44.1	62.5
31	93.8	0.38	137.87	44.1	62.5
32	93.9	0.67	137.87	44.0	62.3
33	93.8	0.56	137.87	44.1	62.5
34	93.6	0.00	137.87	44.3	62.8
35	93.7	0.00	137.87	44.2	62.6
36	93.7	0.60	137.87	44.2	62.6
37	93.2	0.00	137.87	44.7	63.3
38	93.2	0.60	137.87	44.7	63.3
39	93.3	0.00	137.87	44.6	63.2
40	93.3	0.60	137.87	44.6	63.2
41	93.4	0.60	137.87	44.5	63.1
42	93.6	0.00	137.87	44.3	62.8
43	93.6	0.00	137.87	44.3	62.8
44	94.1	0.00	137.87	43.8	62.1
45	94.3	11.23	137.87	43.6	61.8
46	94.0	0.00	137.87	43.9	62.2
47	94.0	0.25	137.87	43.9	62.2
48	94.5	0.00	137.87	43.4	61.5
49	94.4	0.49	137.87	43.5	61.6
50	93.8	0.00	137.87	44.1	62.5
51*	N/A	N/A	137.9	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 12, 2016 | Job No. 111117 | DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\PeakHour.xls

Table 6
Post Configuration Condition
High Pressure Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
1	93.5	0.00	147.7	54.2	76.9	68
2	93.6	0.11	147.7	54.1	76.7	50
3	93.8	0.00	147.7	53.9	76.4	27
4	93.6	0.11	147.7	54.1	76.7	21
5	93.9	0.00	147.7	53.8	76.3	17
6	93.7	0.09	147.7	54.0	76.6	7
7	93.8	0.11	147.7	53.9	76.4	4
8	94.0	0.00	147.7	53.7	76.1	2
9	93.8	0.00	147.7	53.9	76.4	2
10	94.1	0.00	147.7	53.6	76.0	1
11	94.5	0.00	147.7	53.2	75.4	0
12	94.5	0.11	147.7	53.2	75.4	1
13	94.4	0.18	147.7	53.3	75.6	2
14	94.0	0.11	147.7	53.7	76.1	3
15	93.8	0.11	147.7	53.9	76.4	4
16	93.7	0.00	147.7	54.0	76.6	5
17	93.6	0.13	147.7	54.1	76.7	6
18	93.6	1.02	147.7	54.1	76.7	4
19	93.3	0.00	147.7	54.4	77.1	6
20	93.5	0.04	147.7	54.2	76.9	6
21	93.4	0.07	147.7	54.3	77.0	8
22	93.6	0.07	147.7	54.1	76.7	9
23	93.6	0.10	147.7	54.1	76.7	12
24	93.8	0.09	147.7	53.9	76.4	2
25	93.8	0.11	147.7	53.9	76.4	4
26	93.4	0.07	147.7	54.3	77.0	7
27	94.1	0.08	147.7	53.6	76.0	3
28	93.8	0.04	147.7	53.9	76.4	5
29	93.7	0.07	147.7	54.0	76.6	21

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NOVATECH ENGINEERING CONSULTANTS LTD.
Date: November 14, 2016

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Table 6
Post Configuration Condition
High Pressure Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
30	93.8	0.07	147.7	53.9	76.4	11
31	93.8	0.07	147.7	53.9	76.4	6
32	93.9	0.12	147.7	53.8	76.3	10
33	93.8	0.10	147.7	53.9	76.4	19
34	93.6	0.00	147.7	54.1	76.7	14
35	93.7	0.00	147.7	54.0	76.6	14
36	93.7	0.11	147.7	54.0	76.6	19
37	93.2	0.00	147.7	54.5	77.3	9
38	93.2	0.11	147.7	54.5	77.3	11
39	93.3	0.00	147.7	54.4	77.1	14
40	93.3	0.11	147.7	54.4	77.1	17
41	93.4	0.11	147.7	54.3	77.0	35
42	93.6	0.00	147.7	54.1	76.7	31
43	93.6	0.00	147.7	54.1	76.7	58
44	94.1	0.00	147.7	53.6	76.0	3
45	94.3	2.04	147.7	53.4	75.7	3
46	94.0	0.00	147.7	53.7	76.1	23
47	94.0	0.04	147.7	53.7	76.1	39
48	94.5	0.00	147.7	53.2	75.4	116
49	94.4	0.09	147.7	53.3	75.6	28
50	93.8	0.00	147.7	53.9	76.4	36
51*	N/A	N/A	147.7	N/A	N/A	N/A

* Boundary Condition

[] Maximum Pressure

[] Maximum Time

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Date: November 14, 2016

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Table 7a
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 3

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	109.2	15.7	22.3
2	93.6	0.3	108.8	15.2	21.5
3	93.8	167.0	108.5	14.7	20.8
4	93.6	0.3	109.1	15.5	22.0
5	93.9	0.0	110.4	16.5	23.3
6	93.7	0.2	110.5	16.8	23.8
7	93.8	0.3	110.7	16.9	23.9
8	94.0	0.0	110.9	16.9	23.9
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.2
29	93.7	0.2	110.9	17.2	24.3

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Date: November 14, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7a
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 3

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.4	16.7	23.6
37	93.2	0.0	111.0	17.8	25.3
38	93.2	0.3	110.9	17.7	25.0
39	93.3	0.0	110.7	17.4	24.7
40	93.3	0.3	110.6	17.3	24.5
41	93.4	0.3	110.4	17.0	24.0
42	93.6	0.0	110.2	16.6	23.6
43	94.1	0.0	111.8	17.7	25.1
44	93.6	0.0	109.5	15.9	22.5
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.4	16.6	23.5
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7b
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 5

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	110.4	16.9	23.9
2	93.6	0.3	110.3	16.7	23.7
3	93.8	0.0	110.2	16.4	23.3
4	93.6	0.3	110.2	16.6	23.5
5	93.9	167.0	110.1	16.2	23.0
6	93.7	0.2	110.3	16.6	23.6
7	93.8	0.3	110.5	16.7	23.7
8	94.0	0.0	110.8	16.8	23.9
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.3
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.8	16.7	23.7
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7b
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 5

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.6	16.7	23.7
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.5	16.9	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.5	16.8	23.8
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.2
39	93.3	0.0	110.9	17.6	24.9
40	93.3	0.3	110.8	17.5	24.8
41	93.4	0.3	110.6	17.2	24.4
42	93.6	0.0	110.5	16.9	24.0
43	93.6	0.0	110.4	16.8	23.8
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.2	16.4	23.3
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

 Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7c
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 6

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.5	17.0	24.2
2	93.6	0.3	110.5	16.9	24.0
3	93.8	0.0	110.5	16.7	23.6
4	93.6	0.3	110.4	16.8	23.9
5	93.9	0.0	110.4	16.5	23.4
6	93.7	167.2	109.5	15.8	22.4
7	93.8	0.3	110.1	16.3	23.0
8	94.0	0.0	110.7	16.7	23.7
9	93.8	0.0	110.9	17.1	24.2
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

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Date: November 2016

Table 7c
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 6

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.3
39	93.3	0.0	110.9	17.6	25.0
40	93.3	0.3	110.9	17.6	24.9
41	93.4	0.3	110.7	17.3	24.6
42	93.6	0.0	110.6	17.0	24.2
43	93.6	0.0	110.6	17.0	24.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.6
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 12, 2016

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Table 7d
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 7

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.6	16.8	23.9
4	93.6	0.3	110.6	17.0	24.1
5	93.9	0.0	110.6	16.7	23.7
6	93.7	0.2	110.1	16.4	23.2
7	93.8	167.3	109.5	15.7	22.2
8	94.0	0.0	110.6	16.6	23.6
9	93.8	0.0	110.8	17.0	24.1
10	94.1	0.0	111.3	17.2	24.3
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.9	17.3	24.5
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

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Date: November 2016

Table 7d
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 7

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.7	17.2	24.4
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.3
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.1	17.9	25.4
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.7	17.1	24.2
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.6	16.8	23.9
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7e
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 9

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.0	17.5	24.8
2	93.6	0.3	111.0	17.4	24.7
3	93.8	0.0	111.0	17.2	24.4
4	93.6	0.3	111.0	17.4	24.7
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	110.9	17.2	24.3
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.6	16.6	23.5
9	93.8	167.0	110.3	16.5	23.4
10	94.1	0.0	111.2	17.1	24.2
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.2
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.6
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.2	17.4	24.7
25	93.8	0.3	111.3	17.5	24.7
26	93.4	0.2	111.3	17.9	25.4
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7e
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 9

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	111.0	17.1	24.2
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.7
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	111.0	17.3	24.6
37	93.2	0.0	111.3	18.1	25.6
38	93.2	0.3	111.2	18.0	25.6
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.1	17.8	25.3
41	93.4	0.3	111.1	17.7	25.1
42	93.6	0.0	111.0	17.4	24.7
43	93.6	0.0	111.0	17.4	24.7
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.0	17.2	24.4
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 14, 2016

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Table 7f
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 10

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.3	17.8	25.3
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.7
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.2	17.4	24.7
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.1	17.3	24.5
10	94.1	167.0	110.9	16.8	23.8
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	111.5	17.0	24.1
13	94.4	0.5	111.5	17.1	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.6	17.8	25.2
16	93.7	0.0	111.6	17.9	25.3
17	93.6	0.3	111.6	18.0	25.5
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.4	18.1	25.7
20	93.5	0.1	111.4	17.9	25.4
21	93.4	0.2	111.4	18.0	25.5
22	93.6	0.2	111.4	17.8	25.2
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	111.0	17.2	24.4
25	93.8	0.3	111.2	17.4	24.6
26	93.4	0.2	111.4	18.0	25.5
27	94.1	0.2	111.2	17.1	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	25.0

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Date: November 2016\20161110\Post Configuration\MaxDailyFF.xls

Table 7f
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 10

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.3	17.5	24.9
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.7
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	25.0
36	93.7	0.3	111.3	17.6	25.0
37	93.2	0.0	111.4	18.2	25.8
38	93.2	0.3	111.4	18.2	25.8
39	93.3	0.0	111.4	18.1	25.7
40	93.3	0.3	111.4	18.1	25.6
41	93.4	0.3	111.4	18.0	25.5
42	93.6	0.0	111.3	17.7	25.2
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

 Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7g
Post Configuration Condition
Max Daily Demand and Fire Flow at Node13

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.6	18.1	25.6
2	93.6	0.3	111.6	18.0	25.5
3	93.8	0.0	111.6	17.8	25.2
4	93.6	0.3	111.6	18.0	25.5
5	93.9	0.0	111.6	17.7	25.0
6	93.7	0.2	111.6	17.9	25.3
7	93.8	0.3	111.6	17.8	25.2
8	94.0	0.0	111.6	17.6	24.9
9	93.8	0.0	111.6	17.8	25.2
10	94.1	0.0	111.5	17.4	24.7
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	110.9	16.4	23.2
13	94.4	167.5	109.9	15.5	22.0
14	94.0	0.3	110.4	16.4	23.2
15	93.8	0.3	110.8	17.0	24.1
16	93.7	0.0	111.0	17.3	24.5
17	93.6	0.3	111.2	17.6	25.0
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.6	18.3	25.9
20	93.5	0.1	111.6	18.1	25.6
21	93.4	0.2	111.6	18.2	25.8
22	93.6	0.2	111.6	18.0	25.5
23	93.6	0.3	111.6	18.0	25.5
24	93.8	0.2	111.5	17.7	25.2
25	93.8	0.3	111.6	17.8	25.2
26	93.4	0.2	111.6	18.2	25.8
27	94.1	0.2	111.6	17.5	24.8
28	93.8	0.1	111.6	17.8	25.2
29	93.7	0.2	111.6	17.9	25.3

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NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016

Table 7g
Post Configuration Condition
Max Daily Demand and Fire Flow at Node13

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.6	17.8	25.2
31	93.8	0.2	111.6	17.8	25.2
32	93.9	0.3	111.6	17.7	25.0
33	93.8	0.3	111.6	17.8	25.2
34	93.6	0.0	111.6	18.0	25.5
35	93.7	0.0	111.6	17.9	25.3
36	93.7	0.3	111.6	17.9	25.3
37	93.2	0.0	111.6	18.4	26.0
38	93.2	0.3	111.6	18.4	26.0
39	93.3	0.0	111.6	18.3	25.9
40	93.3	0.3	111.6	18.3	25.9
41	93.4	0.3	111.6	18.2	25.8
42	93.6	0.0	111.6	18.0	25.5
43	93.6	0.0	111.6	18.0	25.5
44	94.1	0	111.84	17.74	25.2
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.6	17.8	25.2
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7h
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 16

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.5	18.0	25.6
2	93.6	0.3	111.5	17.9	25.4
3	93.8	0.0	111.5	17.7	25.1
4	93.6	0.3	111.5	17.9	25.4
5	93.9	0.0	111.5	17.6	25.0
6	93.7	0.2	111.5	17.8	25.3
7	93.8	0.3	111.5	17.7	25.1
8	94.0	0.0	111.5	17.5	24.9
9	93.8	0.0	111.5	17.7	25.2
10	94.1	0.0	111.6	17.5	24.8
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.3	16.8	23.9
13	94.4	0.5	111.0	16.6	23.5
14	94.0	0.3	110.6	16.6	23.5
15	93.8	0.3	110.2	16.4	23.3
16	93.7	167.0	110.1	16.4	23.2
17	93.6	0.3	110.7	17.1	24.2
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.5	18.2	25.8
20	93.5	0.1	111.5	18.0	25.6
21	93.4	0.2	111.5	18.1	25.7
22	93.6	0.2	111.5	17.9	25.4
23	93.6	0.3	111.5	17.9	25.4
24	93.8	0.2	111.6	17.8	25.2
25	93.8	0.3	111.5	17.7	25.1
26	93.4	0.2	111.5	18.1	25.7
27	94.1	0.2	111.5	17.4	24.7
28	93.8	0.1	111.5	17.7	25.1
29	93.7	0.2	111.5	17.8	25.3

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NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016

Table 7h
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 16

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
30	93.8	0.2	111.5	17.7	25.1
31	93.8	0.2	111.5	17.7	25.1
32	93.9	0.3	111.5	17.6	25.0
33	93.8	0.3	111.5	17.7	25.1
34	93.6	0.0	111.5	17.9	25.4
35	93.7	0.0	111.5	17.8	25.3
36	93.7	0.3	111.5	17.8	25.3
37	93.2	0.0	111.5	18.3	26.0
38	93.2	0.3	111.5	18.3	26.0
39	93.3	0.0	111.5	18.2	25.8
40	93.3	0.3	111.5	18.2	25.8
41	93.4	0.3	111.5	18.1	25.7
42	93.6	0.0	111.5	17.9	25.4
43	93.6	0.0	111.5	17.9	25.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.5	17.7	25.1
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

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Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7i
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 20

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.1	17.6	25.0
2	93.6	0.3	111.1	17.5	24.9
3	93.8	0.0	111.1	17.3	24.6
4	93.6	0.3	111.1	17.5	24.9
5	93.9	0.0	111.1	17.2	24.4
6	93.7	0.2	111.2	17.5	24.8
7	93.8	0.3	111.2	17.4	24.6
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.4	17.3	24.6
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.4	17.8	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	167.1	111.0	17.5	24.8
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.1	17.5	24.8
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.2	17.1	24.2
28	93.8	0.1	111.1	17.3	24.6
29	93.7	0.2	111.1	17.4	24.7

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016

Table 7i
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 20

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.6
32	93.9	0.3	111.1	17.2	24.4
33	93.8	0.3	111.1	17.3	24.6
34	93.6	0.0	111.1	17.5	24.8
35	93.7	0.0	111.1	17.4	24.7
36	93.7	0.3	111.1	17.4	24.7
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.2	18.0	25.5
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.2	17.9	25.3
41	93.4	0.3	111.1	17.7	25.2
42	93.6	0.0	111.1	17.5	24.9
43	93.6	0.0	111.1	17.5	24.9
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.7	17.4	24.7
46	94.0	0.0	111.7	17.7	25.2
47	94.0	0.1	111.7	17.7	25.2
48	94.5	0.0	111.7	17.2	24.4
49	94.4	0.2	111.7	17.3	24.6
50	93.8	0.0	111.1	17.3	24.6
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Table 7j
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 22

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.8	17.3	24.5
2	93.6	0.3	110.8	17.2	24.3
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.3
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.2
7	93.8	0.3	110.9	17.1	24.2
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.1	17.6	25.0
21	93.4	0.2	110.8	17.4	24.6
22	93.6	167.2	110.5	16.9	23.9
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.2

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NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016\11\10\20161110\Post Configuration\MaxDailyFF.xls

Table 7j
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 22

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.8	17.0	24.1
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.2
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.8	17.2	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

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Date: November 2016\11\10\POST CONFIGURATION\MAX DAILY FF.XLS

Table 7k
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 25

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.3	17.8	25.2
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.6
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.3	17.5	24.8
8	94.0	0.0	111.3	17.3	24.5
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.2	17.1	24.3
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.3	17.9	25.4
22	93.6	0.2	111.3	17.7	25.1
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	110.6	16.8	23.9
25	93.8	167.3	109.7	15.9	22.5
26	93.4	0.2	110.7	17.3	24.5
27	94.1	0.2	111.3	17.2	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	24.9

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Date: November 2016\11\10\20161110\Post Configuration\MaxDailyFF.xls

Table 7k
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 25

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.3	17.5	24.8
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.6
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	24.9
36	93.7	0.3	111.3	17.6	24.9
37	93.2	0.0	111.3	18.1	25.7
38	93.2	0.3	111.3	18.1	25.7
39	93.3	0.0	111.3	18.0	25.5
40	93.3	0.3	111.3	18.0	25.5
41	93.4	0.3	111.3	17.9	25.4
42	93.6	0.0	111.3	17.7	25.1
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7I
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 32

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.2
5	93.9	0.0	110.7	16.8	23.8
6	93.7	0.2	110.7	17.0	24.1
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.8	16.8	23.8
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.7
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.6	16.8	23.8
29	93.7	0.2	110.6	16.9	24.0

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Date: November 2016\20161110\Post Configuration\MaxDailyFF.xls

Table 7I
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 32

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.8	17.0	24.0
31	93.8	0.2	110.2	16.4	23.3
32	93.9	167.3	109.6	15.7	22.3
33	93.8	0.3	110.2	16.4	23.2
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.7	17.1	24.3
43	93.6	0.0	110.7	17.1	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	23.9
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

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Date: November 2016\20161110\Post Configuration\MaxDailyFF.xls

Table 7m
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 36

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.2	16.7	23.6
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.3	16.5	23.4
4	93.6	0.3	110.4	16.8	23.8
5	93.9	0.0	110.5	16.6	23.6
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016

Table 7m
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 36

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.9
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.9
36	93.7	167.3	109.3	15.6	22.1
37	93.2	0.0	111.0	17.8	25.2
38	93.2	0.3	110.8	17.6	24.9
39	93.3	0.0	110.6	17.3	24.5
40	93.3	0.3	110.4	17.1	24.3
41	93.4	0.3	110.1	16.7	23.7
42	93.6	0.0	109.9	16.3	23.2
43	93.6	0.0	110.1	16.5	23.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016\11\10\POST CONFIGURATION\MAX DAILY FF.XLS

Table 7n
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 39

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.6	17.1	24.2
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.8	17.2	24.4
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	111.0	17.3	24.5
7	93.8	0.3	111.1	17.3	24.5
8	94.0	0.0	111.1	17.1	24.3
9	93.8	0.0	111.2	17.4	24.7
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.1	17.0	24.1
28	93.8	0.1	111.1	17.3	24.5
29	93.7	0.2	111.1	17.4	24.7

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016\11\10\20161110\Post Configuration\MaxDailyFF.xls

Table 7n
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 39

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.5
32	93.9	0.3	111.0	17.1	24.3
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.6
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	109.9	16.7	23.7
38	93.2	0.3	109.1	15.9	22.6
39	93.3	167.0	108.3	15.0	21.3
40	93.3	0.3	108.8	15.5	21.9
41	93.4	0.3	109.8	16.4	23.2
42	93.6	0.0	110.3	16.7	23.7
43	93.6	0.0	110.5	16.9	24.0
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.7
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	111.0	17.2	24.3
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7o
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 41

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	110.1	16.6	23.5
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.4	16.6	23.5
4	93.6	0.3	110.5	16.9	24.0
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.3
7	93.8	0.3	110.9	17.1	24.3
8	94.0	0.0	111.0	17.0	24.2
9	93.8	0.0	111.1	17.3	24.6
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.0	17.6	25.0
22	93.6	0.2	110.9	17.3	24.6
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.0	16.9	24.0
28	93.8	0.1	111.0	17.2	24.4
29	93.7	0.2	111.0	17.3	24.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016\20161110\Post Configuration\MaxDailyFF.xls

Table 7o
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 41

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	110.9	17.0	24.0
33	93.8	0.3	110.8	17.0	24.1
34	93.6	0.0	110.8	17.2	24.4
35	93.7	0.0	110.7	17.0	24.2
36	93.7	0.3	110.2	16.5	23.4
37	93.2	0.0	110.6	17.4	24.6
38	93.2	0.3	110.1	16.9	24.0
39	93.3	0.0	109.7	16.4	23.2
40	93.3	0.3	109.4	16.1	22.8
41	93.4	167.3	108.7	15.3	21.7
42	93.6	0.0	109.7	16.1	22.8
43	93.6	0.0	110.0	16.4	23.2
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

 Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7p
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 43

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	108.7	15.2	21.5
2	93.6	0.3	109.1	15.5	22.0
3	93.8	0.0	109.5	15.7	22.2
4	93.6	0.3	109.9	16.3	23.1
5	93.9	0.0	110.5	16.6	23.5
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

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NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016\11\10\20161110\Post Configuration\MaxDailyFF.xls

Table 7p
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 43

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.3
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	23.9
36	93.7	0.3	110.2	16.5	23.3
37	93.2	0.0	110.9	17.7	25.1
38	93.2	0.3	110.7	17.5	24.8
39	93.3	0.0	110.5	17.2	24.4
40	93.3	0.3	110.3	17.0	24.1
41	93.4	0.3	110.0	16.6	23.5
42	93.6	0.0	109.8	16.2	23.0
43	93.6	167.0	108.4	14.8	20.9
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7q
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 46

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

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Date: November 2016\11\10\20161110\Post Configuration\MaxDailyFF.xls

Table 7q
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 46

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	167.0	110.2	16.2	22.9
47	94.0	0.1	110.3	16.3	23.2
48	94.5	0.0	110.5	16.0	22.7
49	94.4	0.2	110.7	16.3	23.1
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 7r
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 48

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

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Date: November 2016\11\10\20161110\Post Configuration\MaxDailyFF.xls

Table 7r
Post Configuration Condition
Max Daily Demand and Fire Flow at Node 48

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	0.0	110.6	16.6	23.5
47	94.0	0.1	109.9	15.9	22.5
48	94.5	167.0	109.1	14.6	20.6
49	94.4	0.2	110.0	15.6	22.2
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

File: 20161110\20161117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

Table 8
Post Configuration Condition
Peak Hour Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.00	137.87	44.4	62.9
2	93.6	0.60	137.87	44.3	62.8
3	93.8	0.00	137.87	44.1	62.5
4	93.6	0.60	137.87	44.3	62.8
5	93.9	0.00	137.87	44.0	62.3
6	93.7	0.49	137.87	44.2	62.6
7	93.8	0.60	137.87	44.1	62.5
8	94.0	0.00	137.87	43.9	62.2
9	93.8	0.00	137.87	44.1	62.5
10	94.1	0.00	137.87	43.8	62.1
11	94.5	0.00	137.88	43.4	61.5
12	94.5	0.60	137.88	43.4	61.5
13	94.4	0.98	137.88	43.5	61.7
14	94.0	0.60	137.87	43.9	62.2
15	93.8	0.60	137.87	44.1	62.5
16	93.7	0.00	137.87	44.2	62.6
17	93.6	0.74	137.87	44.3	62.8
18	93.6	5.61	137.87	44.3	62.8
19	93.3	0.00	137.87	44.6	63.2
20	93.5	0.25	137.87	44.4	62.9
21	93.4	0.38	137.87	44.5	63.1
22	93.6	0.38	137.87	44.3	62.8
23	93.6	0.56	137.87	44.3	62.8
24	93.8	0.49	137.87	44.1	62.5
25	93.8	0.60	137.87	44.1	62.5
26	93.4	0.38	137.87	44.5	63.1
27	94.1	0.42	137.87	43.8	62.1
28	93.8	0.20	137.87	44.1	62.5
29	93.7	0.38	137.87	44.2	62.6

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 20, 2016 | Job No. 111117 | DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\PeakHour.xls

Table 8
Post Configuration Condition
Peak Hour Check

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.38	137.87	44.1	62.5
31	93.8	0.38	137.87	44.1	62.5
32	93.9	0.67	137.87	44.0	62.3
33	93.8	0.56	137.87	44.1	62.5
34	93.6	0.00	137.87	44.3	62.8
35	93.7	0.00	137.87	44.2	62.6
36	93.7	0.60	137.87	44.2	62.6
37	93.2	0.00	137.87	44.7	63.3
38	93.2	0.60	137.87	44.7	63.3
39	93.3	0.00	137.87	44.6	63.2
40	93.3	0.60	137.87	44.6	63.2
41	93.4	0.60	137.87	44.5	63.1
42	93.6	0.00	137.87	44.3	62.8
43	93.6	0.00	137.87	44.3	62.8
44	94.1	0.00	137.87	43.8	62.1
45	94.3	11.23	137.87	43.6	61.8
46	94.0	0.00	137.87	43.9	62.2
47	94.0	0.25	137.87	43.9	62.2
48	94.5	0.00	137.87	43.4	61.5
49	94.4	0.49	137.87	43.5	61.6
50	93.8	0.00	137.87	44.1	62.5
51*	N/A	N/A	137.9	N/A	N/A

* Boundary Condition

[Redacted] Minimum Pressure

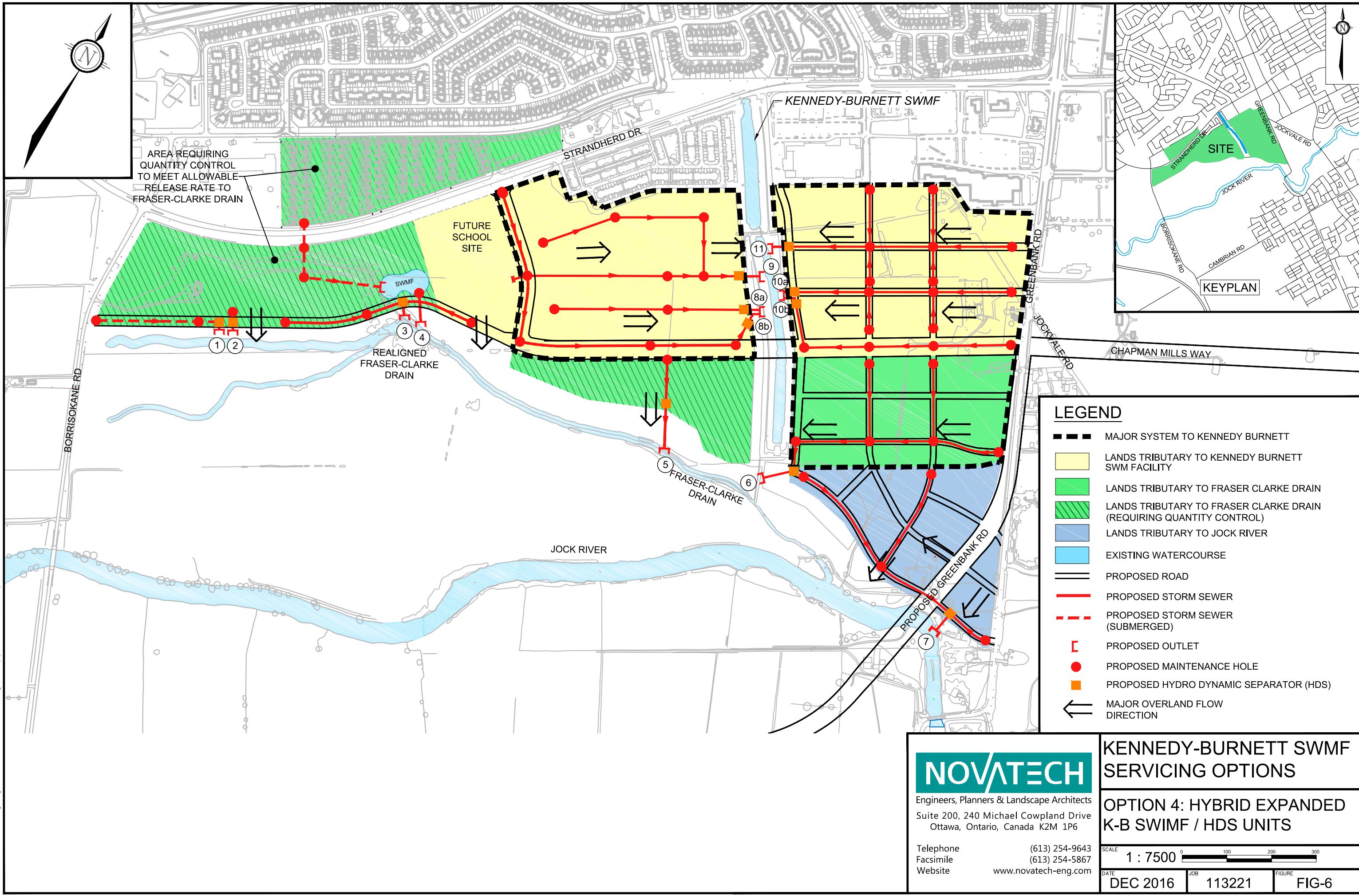
Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 20, 2016 | Job No. 111117 | DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\PeakHour.xls

Appendix D

SWM Calculations & PCSWMM Model



Burnett Lands - 3370 Greenbank Road
Post-Development Model Parameters

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Impervious (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
Burnett Lands - Claridge						
A-01	0.235	0.65	64%	50%	43	0.75%
A-02	0.249	0.65	64%	50%	36	0.75%
A-03	0.127	0.65	64%	75%	36	0.75%
A-04	0.302	0.65	64%	50%	34	0.75%
A-05	0.275	0.65	64%	75%	37	0.75%
A-06	0.274	0.65	64%	60%	37	0.75%
A-07	0.289	0.65	64%	50%	39	0.75%
A-08	0.246	0.65	64%	75%	41	0.75%
A-09	0.081	0.65	64%	50%	23	0.75%
A-10	0.187	0.65	64%	50%	37	0.75%
A-11	0.225	0.65	64%	50%	25	0.75%
A-12	0.036	0.65	64%	100%	24	0.75%
A-13	0.200	0.65	64%	50%	24	0.75%
A-14	0.195	0.65	64%	50%	23	0.75%
A-15	0.105	0.65	64%	100%	30	0.75%
A-16	0.128	0.65	64%	100%	32	0.75%
A-17	0.246	0.65	64%	50%	49	0.75%
A-18	0.272	0.65	64%	50%	32	0.75%
A-19	0.122	0.65	64%	100%	31	0.75%
A-20	0.130	0.65	64%	100%	29	0.75%
A-21	0.217	0.65	64%	50%	27	0.75%
A-22	0.060	0.65	64%	100%	24	0.75%
A-23	0.457	0.65	64%	75%	46	0.75%
A-24	0.209	0.65	64%	75%	28	0.75%
A-25	0.214	0.65	64%	75%	29	0.75%
A-26	0.448	0.65	64%	70%	45	0.75%
A-27	0.190	0.65	64%	75%	32	0.75%
A-28	0.119	0.65	64%	0%	18	0.75%
A-29	0.231	0.65	64%	60%	46	0.75%
A-30	0.498	0.65	64%	50%	71	0.75%
A-31	0.061	0.65	64%	0%	20	0.75%
A-32	0.450	0.65	64%	75%	56	0.75%
A-33	0.816	0.65	64%	50%	117	0.75%
A-34	0.773	0.65	64%	0%	55	0.75%
A-35	0.983	0.65	64%	0%	70	0.75%
A-36	0.450	0.65	64%	0%	60	0.75%
A-37	0.372	0.65	64%	0%	53	0.75%

Burnett Lands - 3370 Greenbank Road
Post-Development Model Parameters

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Impervious (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
Street B - Caivan Lands						
B-01	0.092	0.65	64%	50%	28	0.75%
B-02	0.149	0.65	64%	0%	23	0.75%
B-03	0.097	0.65	64%	10%	32	0.75%
B-04	0.825	0.65	64%	50%	110	0.75%
B-05	0.064	0.65	64%	100%	27	0.75%
B-06	0.076	0.65	64%	0%	19	0.75%
B-07	0.127	0.65	64%	40%	32	0.75%
B-08	0.225	0.65	64%	50%	32	0.75%
B-09	0.245	0.65	64%	50%	34	0.75%
B-10	1.115	0.30	14%	0%	149	0.75%
B-11	0.049	0.65	64%	0%	13	0.75%
B-12	0.088	0.65	64%	50%	20	0.75%
B-13	0.097	0.65	64%	50%	22	0.75%
B-14	0.320	0.65	64%	50%	73	0.75%
B-15	0.120	0.65	64%	50%	27	0.75%
B-16	0.136	0.65	64%	50%	31	0.75%
B-17	0.247	0.65	64%	50%	33	0.75%
B-18	0.121	0.65	64%	50%	28	0.75%
B-19	0.107	0.65	64%	50%	22	0.75%
CaivanLands	8.101	0.65	64%	50%	200	0.50%

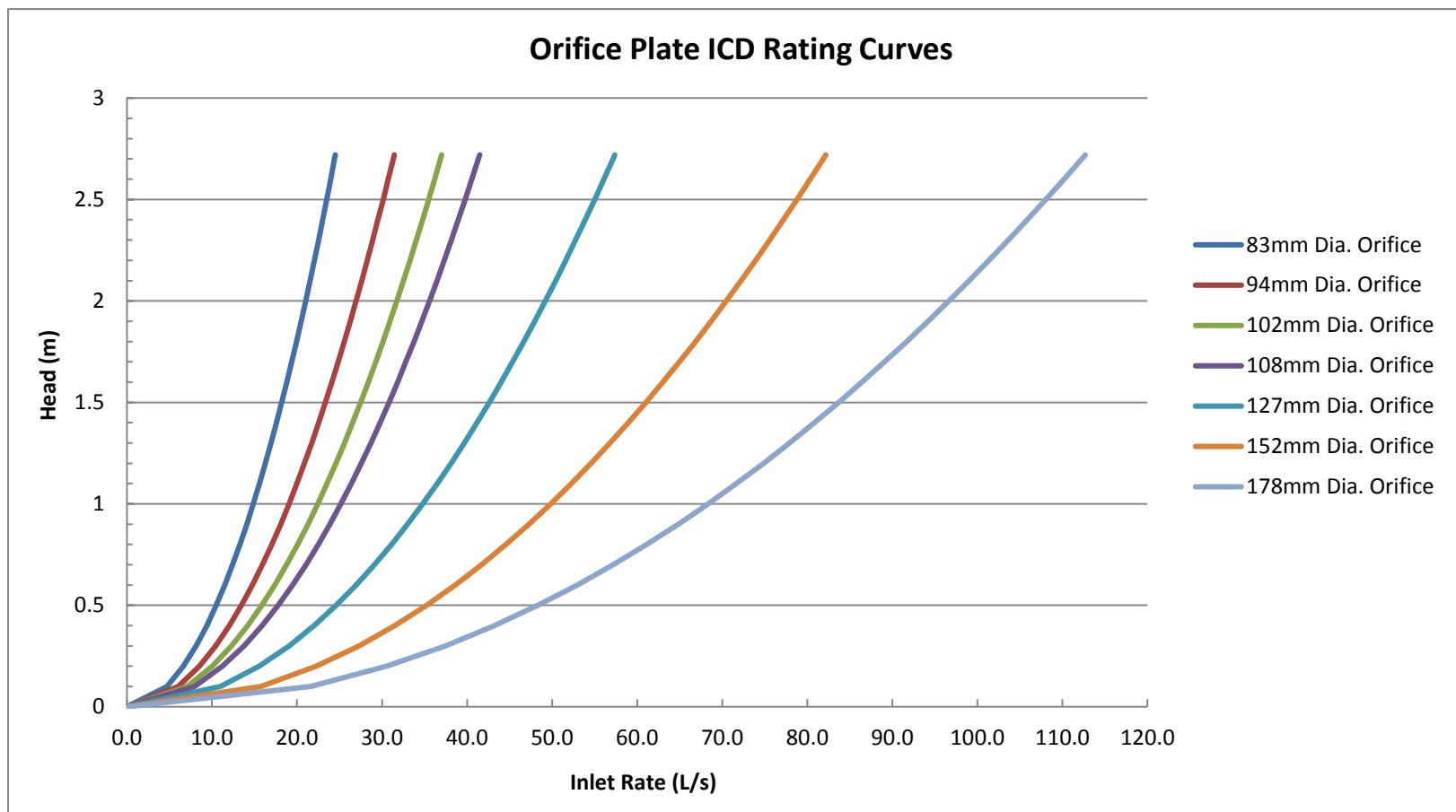
Burnett Lands - 3370 Greenbank Road
Inlet Control Device Parameters

Structure	ICD Size & Inlet Rate			5-year Peak Flow*
	Diameter (mm)	Max Head (m)	5-year Inlet Capture (L/s)	
Burnett Lands - Claridge				
CB01-02	127	1.14	37.09	50.15
CB03-04	127	1.23	38.53	48.26
CB05-06	152	1.12	52.83	58.43
CB07-08	127	1.14	37.09	50.69
CB09-10	127	1.04	35.42	4.80
CB11-12	108	1.19	27.40	54.07
CB13-14	102	1.15	24.05	27.65
CB15-16	108	1.15	26.93	56.86
CB17-18	102	1.15	24.05	49.60
CB19	83	1.16	15.99	17.43
CB20-21	156	1.12	55.60	80.41
CB22	108	1.15	26.93	37.97
CB23	108	1.15	26.93	38.91
CB24-25	127	1.14	37.09	44.09
CB26-27	178	1.11	72.03	86.47
CB28	127	1.15	37.25	47.94
CB29	94	1.15	20.46	29.12
CB30-31	127	1.14	37.09	38.46
CB32-33	127	1.14	37.09	44.92
CB34-35	127	1.14	37.09	44.34
CB40-41	178	1.11	72.03	87.24
CB42-43	152	1.12	52.83	52.25
CB44-45	127	1.23	38.53	50.39
CB59	83	1.16	15.99	6.67
CB60	102	1.15	24.05	24.03
CB61	83	1.16	15.99	11.11
CB62	108	1.15	26.93	22.56
CB63	108	1.15	26.93	23.67
CB64	102	1.15	24.05	19.43
CB66	83	1.16	15.99	13.15
CB67-68-69	178	1.17	73.95	85.77

Burnett Lands - 3370 Greenbank Road
Inlet Control Device Parameters

Structure	ICD Size & Inlet Rate			5-year Peak Flow*
	Diameter (mm)	Max Head (m)	5-year Inlet Capture (L/s)	
Street B - Caivan Lands				
CB36	83	1.46	17.94	18.08
CB37	102	1.45	27.01	29.14
CB39	83	1.46	17.94	19.63
CB46-47	83	1.46	17.94	15.06
CB48-49	83	1.44	17.82	24.54
CB50-51	127	1.44	41.70	42.30
CB52-53	178	1.41	81.18	92.98
CB54	108	1.45	30.25	9.67
CB55-56	178	1.41	81.18	91.54
CB57-58	178	1.31	78.25	50.85
CB65	83	1.46	17.94	11.90

*From SSA Model, 5-year 4-hour Chicago storm distribution

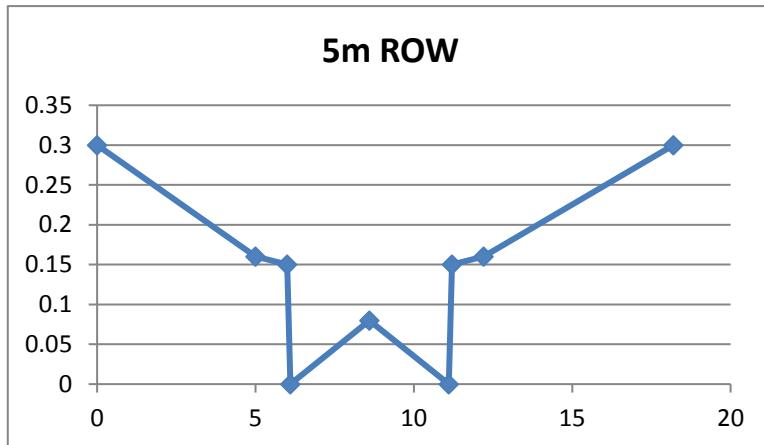


Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elev - 5yr 4hr (100-yr Fixed) (m)	HGL Elev - 100yr 4hr (5-yr Fixed) (m)	HGL Elev - 100yr 4hr +20% (5-yr Fixed) (m)	Design USF (m)	Clearance 5yr 4hr (100-yr Fixed) (m)	Clearance 100yr 4hr (5-yr Fixed) (m)	Clearance 100yr 4hr +20% (5-yr Fixed) (m)
Burnett Lands - Claridge									
102 (STM)	90.52	93.78	92.04	91.54	91.59	92.62	0.58	1.08	1.03
104 (STM)	90.37	93.93	92.03	91.54	91.59	92.60	0.57	1.06	1.01
106 (STM)	90.27	93.87	92.04	91.53	91.59	92.60	0.56	1.07	1.01
108 (STM)	90.10	93.75	92.01	91.49	91.54	92.51	0.50	1.02	0.97
110 (STM)	89.94	93.46	92.00	91.42	91.46	92.43	0.43	1.01	0.97
304 (STM)	90.56	93.67	91.72	91.15	91.17	92.25	0.53	1.10	1.08
306 (STM)	90.35	93.58	91.72	91.14	91.17	92.22	0.50	1.08	1.05
308 (STM)	90.11	93.41	91.70	91.12	91.15	92.17	0.47	1.05	1.02
310 (STM)	89.74	93.46	91.66	91.08	91.10	92.06	0.40	0.98	0.96
312 (STM)	89.26	93.36	91.65	91.06	91.09	92.06	0.41	1.00	0.97
314 (STM)	89.19	92.61	91.58	90.98	91.00	-	-	-	-
316 (STM)	89.00	93.09	91.49	90.88	90.89	-	-	-	-
324 (STM)	88.96	93.34	91.41	90.79	90.79	-	-	-	-
326 (STM)	89.95	93.38	91.35	90.71	90.72	-	-	-	-
328 (STM)	88.91	92.16	91.30	90.66	90.66	-	-	-	-
330 (STM)	88.94	92.53	91.38	90.75	90.75	-	-	-	-
402 (STM)	90.62	93.82	92.02	91.49	91.53	92.40	0.38	0.91	0.87
404 (STM)	90.37	93.80	92.01	91.48	91.52	92.45	0.44	0.97	0.93
406 (STM)	90.46	93.71	92.04	91.53	91.67	92.55	0.51	1.02	0.88
408 (STM)	90.36	93.80	92.04	91.53	91.60	92.55	0.51	1.02	0.95
608 (STM)	90.25	93.71	91.99	91.46	91.50	92.55	0.56	1.09	1.05
610 (STM)	89.58	93.30	91.79	91.24	91.27	92.38	0.59	1.14	1.11
612 (STM)	89.82	93.93	91.64	91.06	91.08	-	-	-	-
614 (STM)	89.92	93.76	91.71	91.15	91.16	-	-	-	-
616 (STM)	90.23	94.27	91.72	91.15	91.17	92.65	0.93	1.50	1.48
618 (STM)	90.03	93.65	91.80	91.24	91.27	92.25	0.45	1.01	0.98
902 (STM)	90.34	93.98	91.82	91.28	91.31	92.55	0.73	1.27	1.24
904 (STM)	90.29	93.76	91.83	91.28	91.31	92.35	0.52	1.07	1.04
906 (STM)	90.12	93.53	91.82	91.27	91.31	92.35	0.53	1.08	1.04
908 (STM)	89.97	93.57	91.81	91.25	91.29	92.25	0.44	1.00	0.96
Street B - Caivan Lands									
200 (STM)	91.55	94.54	92.38	92.34	92.37	-	-	-	-
202 (STM)	91.40	94.14	92.37	92.33	92.36	-	-	-	-
204 (STM)	91.08	93.94	92.26	91.92	91.95	92.55	0.29	0.63	0.60
206 (STM)	90.84	93.62	92.14	91.69	91.70	92.45	0.31	0.76	0.75
208 (STM)	90.69	93.82	92.06	91.53	91.54	92.45	0.39	0.92	0.91
210 (STM)	90.53	93.52	91.89	91.33	91.34	92.45	0.56	1.12	1.11
212 (STM)	90.40	93.73	91.80	91.21	91.22	92.40	0.60	1.19	1.18
214 (STM)	90.30	93.47	91.73	91.12	91.12	92.25	0.52	1.13	1.13
216 (STM)	90.01	93.37	91.65	90.94	90.94	92.25	0.60	1.31	1.31

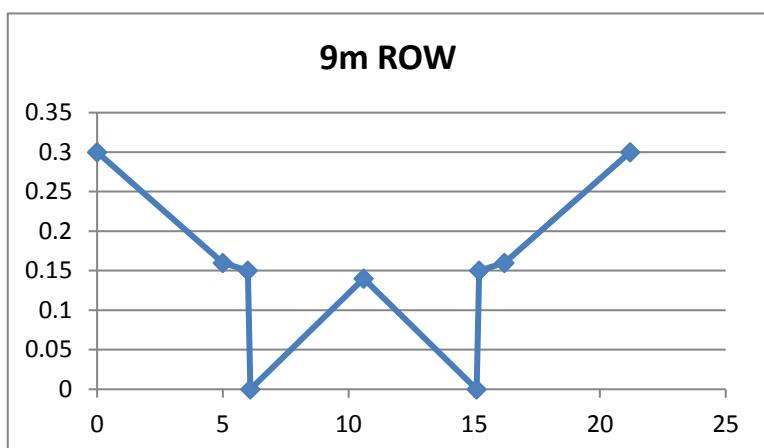
Structure	T/G	Max. Static Ponding (Spill Depth)		5-yr Event				100-yr Event (4hr)				100-yr Event (+20%) (4hr)				
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Spill Flow (L/s)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)
Burnett Lands - Claridge																
CB01-02	93.25	93.44	0.19	93.30	0.05	N	0.00	93.41	0.16	N	0.00	0	93.46	0.21	Y	0.02
CB03-04	93.18	93.44	0.26	93.22	0.04	N	0.00	93.42	0.24	N	0.00	0	93.46	0.28	Y	0.02
CB05-06	93.05	93.34	0.29	93.06	0.01	N	0.00	93.19	0.14	N	0.00	0	93.24	0.19	N	0.00
CB07-08	92.92	93.34	0.42	92.96	0.04	N	0.00	93.08	0.16	N	0.00	0	93.09	0.17	N	0.00
CB09-10	93.01	93.34	0.33	93.06	0.05	N	0.00	93.26	0.25	N	0.00	0	93.31	0.30	N	0.00
CB15-16	93.20	93.37	0.17	93.32	0.12	N	0.00	93.43	0.23	Y	0.06	56	93.46	0.26	Y	0.09
CB17-18	93.51	93.76	0.25	93.66	0.15	N	0.00	93.71	0.20	N	0.00	0	93.73	0.22	N	0.00
CB19	93.44	93.64	0.20	93.32	0.00	N	0.00	93.56	0.12	N	0.00	0	93.62	0.18	N	0.00
CB20-21	93.34	93.60	0.26	93.42	0.08	N	0.00	93.56	0.22	N	0.00	0	93.62	0.28	Y	0.02
CB22	93.49	93.64	0.15	93.54	0.05	N	0.00	93.64	0.15	N	0.00	0	93.67	0.18	Y	0.03
CB23	93.28	93.51	0.23	93.30	0.02	N	0.00	93.37	0.09	N	0.00	0	93.42	0.14	N	0.00
CB24-25	93.25	93.48	0.23	93.25	0.00	N	0.00	93.35	0.10	N	0.00	0	93.40	0.15	N	0.00
CB26-27	93.08	93.36	0.28	93.10	0.02	N	0.00	93.24	0.16	N	0.00	0	93.29	0.21	N	0.00
CB28	92.99	93.34	0.35	92.11	0.00	N	0.00	93.08	0.09	N	0.00	0	93.17	0.18	N	0.00
CB29	92.98	93.34	0.36	93.01	0.03	N	0.00	93.08	0.10	N	0.00	0	93.17	0.19	N	0.00
CB30-31	93.31	93.58	0.27	93.49	0.18	N	0.00	93.60	0.29	Y	0.02	77	93.61	0.30	Y	0.03
CB40-41	93.35	93.65	0.30	93.39	0.04	N	0.00	93.52	0.17	N	0.00	0	93.58	0.23	N	0.00
CB42-43	93.47	93.72	0.25	93.37	0.00	N	0.00	93.63	0.16	N	0.00	0	93.70	0.23	N	0.00
CB44-45	93.62	93.87	0.25	93.68	0.06	N	0.00	93.77	0.15	N	0.00	0	93.78	0.16	N	0.00
CB59	93.70	93.91	0.21	92.75	0.00	N	0.00	93.14	0.00	N	0.00	0	93.39	0.00	N	0.00
CB60	93.50	93.80	0.30	93.27	0.00	N	0.00	93.81	0.31	Y	0.01	1	93.90	0.40	Y	0.10
CB61	93.50	93.78	0.28	92.90	0.00	N	0.00	93.64	0.14	N	0.00	0	93.68	0.18	N	0.00
CB62	93.49	93.78	0.29	93.16	0.00	N	0.00	93.77	0.28	N	0.00	0	93.79	0.30	Y	0.01
CB63	93.55	93.84	0.29	93.29	0.00	N	0.00	93.85	0.30	Y	0.01	2	93.85	0.30	Y	0.01
CB64	93.66	93.95	0.29	93.27	0.00	N	0.00	93.91	0.25	N	0.00	0	93.96	0.30	Y	0.01
CB66	93.90	94.01	0.11	93.35	0.00	N	0.00	93.97	0.07	N	0.00	0	94.01	0.11	N	0.00
CB67-68-69	93.66	94.15	0.49	93.70	0.04	N	0.00	93.83	0.17	N	0.00	0	93.88	0.22	N	0.00
Street B - Caivan Lands																
CB39	93.78	93.86	0.08	93.82	0.04	N	0.00	93.92	0.14	Y	0.06	52	93.93	0.15	Y	0.07
CB46-47	93.67	93.77	0.10	93.43	0.00	N	0.00	93.84	0.17	Y	0.07	52	93.90	0.23	Y	0.13
CB48-49	93.72	93.86	0.14	93.79	0.07	N	0.00	93.87	0.15	Y	0.01	9	93.90	0.18	Y	0.04
CB50-51	93.59	93.84	0.25	93.62	0.03	N	0.00	93.74	0.15	N	0.00	0	93.80	0.21	N	0.00
CB52-53	93.52	93.76	0.24	93.60	0.08	N	0.00	93.77	0.25	Y	0.01	4	93.81	0.29	Y	0.05
CB55-56	93.38	93.67	0.29	93.42	0.04	N	0.00	93.59	0.21	N	0.00	0	93.66	0.28	N	0.00
CB57-58	93.30	93.56	0.26	93.30	0.00	N	0.00	93.47	0.17	N	0.00	0	93.53	0.23	N	0.00

Burnett Lands - 3370 Greenbank Road
Roadway Cross-Sections

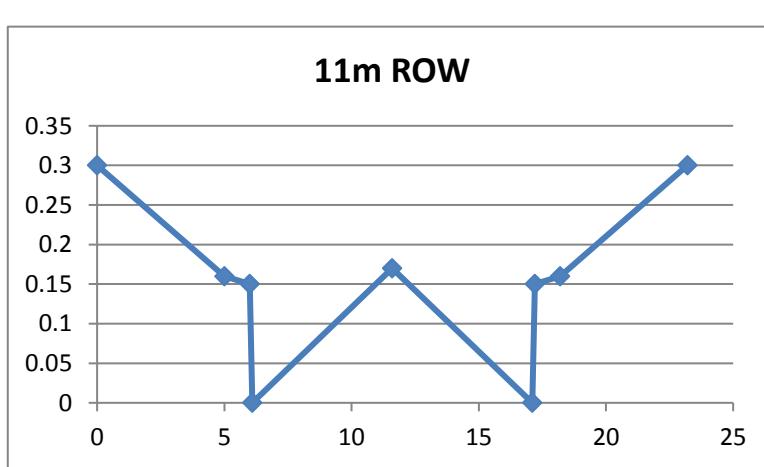
5m - ROW	
0	0.3
5	0.16
6	0.15
6.1	0
8.6	0.08
11.1	0
11.2	0.15
12.2	0.16
18.2	0.3



9m - ROW	
0	0.3
5	0.16
6	0.15
6.1	0
10.6	0.14
15.1	0
15.2	0.15
16.2	0.16
21.2	0.3



11m - ROW	
0	0.3
5	0.16
6	0.15
6.1	0
11.6	0.17
17.1	0
17.2	0.15
18.2	0.16
23.2	0.3



Burnett Lands - 3370 Greenbank Road
Design Storm Time Series Data
Chicago Design Storms



C25mm-4.stm		C2-4.stm		C5-4.stm	
Duration	Intensity	Duration	Intensity	Duration	Intensity
min	mm/hr	min	mm/hr	min	mm/hr
0:00	0	0:00	0	0:00	0
0:10	1.34	0:10	1.98	0:10	2.49
0:20	1.49	0:20	2.23	0:20	2.77
0:30	1.69	0:30	2.58	0:30	3.14
0:40	1.96	0:40	3.06	0:40	3.62
0:50	2.33	0:50	3.81	0:50	4.31
1:00	2.91	1:00	5.1	1:00	5.37
1:10	3.91	1:10	7.91	1:10	7.19
1:20	6.1	1:20	19.04	1:20	11.14
1:30	14.53	1:30	76.81	1:30	26.25
1:40	58.72	1:40	23.64	1:40	104.19
1:50	17.11	1:50	11.91	1:50	30.86
2:00	8.32	2:00	7.98	2:00	15.15
2:10	5.5	2:10	6.03	2:10	10.07
2:20	4.13	2:20	4.87	2:20	7.58
2:30	3.32	2:30	4.1	2:30	6.11
2:40	2.79	2:40	3.55	2:40	5.14
2:50	2.41	2:50	3.14	2:50	4.45
3:00	2.12	3:00	2.82	3:00	3.93
3:10	1.9	3:10	2.57	3:10	3.53
3:20	1.73	3:20	2.35	3:20	3.21
3:30	1.58	3:30	2.18	3:30	2.94
3:40	1.46	3:40	2.03	3:40	2.72
3:50	1.36	3:50	1.9	3:50	2.53
4:00	1.27	4:00	1.79	4:00	2.37

Burnett Lands - 3370 Greenbank Road
Design Storm Time Series Data
Chicago Design Storms



C100-4.stm		C100-4+20%.stm	
Duration	Intensity	Duration	Intensity
min	mm/hr	min	mm/hr
0:00	0	0:00	0
0:10	4.07	0:10	4.88
0:20	4.54	0:20	5.45
0:30	5.14	0:40	7.14
0:40	5.95	0:50	8.51
0:50	7.09	1:00	10.62
1:00	8.85	1:10	14.28
1:10	11.9	1:20	22.25
1:20	18.54	1:30	53.03
1:30	44.19	1:40	214.27
1:40	178.56	1:50	62.45
1:50	52.04	2:00	30.37
2:00	25.31	2:10	20.08
2:10	16.73	2:20	15.07
2:20	12.56	2:30	12.11
2:30	10.09	2:40	10.16
2:40	8.47	2:50	8.78
2:50	7.32	3:00	7.75
3:00	6.46	3:10	6.95
3:10	5.79	3:20	6.3
3:20	5.25	3:30	5.78
3:30	4.82	3:40	5.34
3:40	4.45	3:50	4.97
3:50	4.14	4:00	4.66
4:00	3.88		



Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

M:\2011\111117\CAD\Design\111117 - GP.DWG
M:\2011\111117\CAD\Design\111117 - GP.DWG

NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
Rainfall/Runoff YES
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Starting Date JAN-21-2016 00:00:00
Ending Date JAN-21-2016 12:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 01:00:00
Routing Time Step 5.00 sec

WARNING 03: negative offset ignored for Link CB10-11ROADLINK
WARNING 04: minimum elevation drop used for Conduit CB12-13ROADLINK
WARNING 04: minimum elevation drop used for Conduit CB27-28ROADLINK
WARNING 04: minimum elevation drop used for Conduit CB29-30ROADLINK
WARNING 02: maximum depth increased for Node CB07-08
WARNING 02: maximum depth increased for Node CB09-10
WARNING 02: maximum depth increased for Node CB13-14
WARNING 02: maximum depth increased for Node CB13-14ROAD
WARNING 02: maximum depth increased for Node CB32-33
WARNING 02: maximum depth increased for Node CB32-33 ROAD
WARNING 02: maximum depth increased for Node CB34-35
WARNING 02: maximum depth increased for Node CB34-35 ROAD
WARNING 02: maximum depth increased for Node CB57-58
WARNING 02: maximum depth increased for Node CB60
WARNING 02: maximum depth increased for Node CB61

Element Count

Number of rain gages 1

Number of subcatchments ... 57
Number of nodes 162
Number of links 220
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
RAIN	C5-4	INTENSITY	10 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
A-01	0.23	43.00	64.00	0.7500	RAIN	CB01-02
A-02	0.25	36.00	64.00	0.7500	RAIN	CB03-04
A-03	0.13	36.00	64.00	0.7500	RAIN	CB13-14ROAD
A-04	0.30	34.00	64.00	0.7500	RAIN	CB05-06
A-05	0.28	37.00	64.00	0.7500	RAIN	CB11-12ROAD
A-06	0.27	37.00	64.00	0.7500	RAIN	CB07-08
A-07	0.29	39.00	64.00	0.7500	RAIN	CB15-16
A-08	0.25	41.00	64.00	0.7500	RAIN	CB17-18
A-09	0.08	23.00	64.00	0.7500	RAIN	CB19
A-10	0.19	37.00	64.00	0.7500	RAIN	CB20-21
A-11	0.23	25.00	64.00	0.7500	RAIN	CB20-21
A-12	0.04	24.00	64.00	0.7500	RAIN	CB59
A-13	0.20	24.00	64.00	0.7500	RAIN	CB23
A-14	0.20	23.00	64.00	0.7500	RAIN	CB22
A-15	0.11	30.00	64.00	0.7500	RAIN	CB64
A-16	0.13	32.00	64.00	0.7500	RAIN	CB63
A-17	0.25	49.00	64.00	0.7500	RAIN	CB44-45
A-18	0.27	32.00	64.00	0.7500	RAIN	CB42-43
A-19	0.12	31.00	64.00	0.7500	RAIN	CB62
A-20	0.13	29.00	64.00	0.7500	RAIN	CB60
A-21	0.22	27.00	64.00	0.7500	RAIN	CB24-25
A-22	0.06	24.00	64.00	0.7500	RAIN	CB61
A-23	0.46	46.00	64.00	0.7500	RAIN	CB40-41
A-24	0.21	28.00	64.00	0.7500	RAIN	CB34-35 ROAD
A-25	0.21	29.00	64.00	0.7500	RAIN	CB32-33 ROAD
A-26	0.45	45.00	64.00	0.7500	RAIN	CB26-27
A-27	0.19	32.00	64.00	0.7500	RAIN	CB30-31
A-28	0.23	18.00	64.00	0.7500	RAIN	CB29
A-29	0.23	46.00	64.00	0.7500	RAIN	CB09-10
A-30	0.50	71.00	64.00	0.7500	RAIN	A30-STOR
A-31	0.06	20.00	64.00	0.7500	RAIN	CB66
A-32	0.45	56.00	64.00	0.7500	RAIN	CB67-68-69
A-33	0.82	117.00	64.00	0.7500	RAIN	A33-STOR
A-34	0.77	55.00	64.00	0.7500	RAIN	4-GreenbankOut
A-35	0.98	70.00	64.00	0.7500	RAIN	4-GreenbankOut
A-36	0.45	60.00	64.00	0.7500	RAIN	4-GreenbankOut
A-37	0.37	53.00	64.00	0.7500	RAIN	4-GreenbankOut
B-01	0.09	28.00	64.00	0.7500	RAIN	CB36
B-02	0.15	23.00	64.00	0.7500	RAIN	CB37
B-03	0.10	32.00	64.00	0.7500	RAIN	CB39
B-04	0.83	110.00	64.00	0.7500	RAIN	B04_Stor
B-05	0.06	27.00	64.00	0.7500	RAIN	CB-65
B-06	0.08	19.00	64.00	0.7500	RAIN	CB46-47
B-07	0.13	32.00	64.00	0.7500	RAIN	CB48-49
B-08	0.23	32.00	64.00	0.7500	RAIN	CB50-51
B-09	0.25	34.00	64.00	0.7500	RAIN	CB52-53
B-10	1.12	149.00	14.00	0.1000	RAIN	CB52-53
B-11	0.05	13.00	64.00	0.7500	RAIN	CB54

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

B-12	0.09	20.00	64.00	0.7500	RAIN	CB100
B-13	0.10	22.00	64.00	0.7500	RAIN	CB102
B-14	0.32	73.00	64.00	0.7500	RAIN	CB55-56
B-15	0.12	27.00	64.00	0.7500	RAIN	CB104
B-16	0.14	31.00	64.00	0.7500	RAIN	CB106
B-17	0.27	33.00	64.00	0.7500	RAIN	CB57-58
B-18	0.12	28.00	64.00	0.7500	RAIN	CB108
B-19	0.11	22.00	64.00	0.7500	RAIN	CB110
CaiwanLands	8.10	200.00	64.00	0.5000	RAIN	Caiwan-Stor

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
01+011	JUNCTION	93.48	0.30	0.0	
01+051	JUNCTION	93.76	0.30	0.0	
01+118	JUNCTION	93.72	0.30	0.0	
01+143	JUNCTION	93.93	0.30	0.0	
02+196	JUNCTION	93.67	0.30	0.0	
02+288	JUNCTION	93.76	0.30	0.0	
02+365	JUNCTION	93.84	0.30	0.0	
02+432	JUNCTION	93.86	0.30	0.0	
02+458	JUNCTION	93.77	0.30	0.0	
02+512	JUNCTION	93.86	0.30	0.0	
02+564	JUNCTION	94.40	0.30	0.0	
03+026	JUNCTION	93.56	0.30	0.0	
03+054	JUNCTION	93.61	0.30	0.0	
03+109	JUNCTION	93.44	0.30	0.0	
03+127	JUNCTION	93.37	0.30	0.0	
03+200	JUNCTION	93.42	0.30	0.0	
03+291	JUNCTION	93.34	0.30	0.0	
03+384	JUNCTION	93.34	0.30	0.0	
04+078	JUNCTION	93.65	0.30	0.0	
04+089	JUNCTION	93.76	0.30	0.0	
04+129	JUNCTION	93.64	0.30	0.0	
04+207	JUNCTION	93.87	0.30	0.0	
05+116	JUNCTION	93.93	0.30	0.0	
05+200	JUNCTION	93.49	0.30	0.0	
06+023	JUNCTION	93.64	0.30	0.0	
06+043	JUNCTION	93.74	0.30	0.0	
06+108	JUNCTION	93.60	0.30	0.0	
06+194	JUNCTION	93.51	0.30	0.0	
06+273	JUNCTION	93.40	0.30	0.0	
06+309	JUNCTION	93.88	0.30	0.0	
06+332	JUNCTION	94.51	0.30	0.0	
06+355	JUNCTION	94.15	0.30	0.0	
09+000	JUNCTION	93.36	0.30	0.0	
09+097	JUNCTION	93.65	0.30	0.0	
09+203	JUNCTION	93.89	0.30	0.0	
10+011	JUNCTION	93.58	0.30	0.0	
10+232	JUNCTION	94.36	0.30	0.0	
102 (STM)	JUNCTION	90.82	2.95	0.0	
104 (STM)	JUNCTION	90.67	3.27	0.0	
106 (STM)	JUNCTION	90.57	3.30	0.0	
106a	JUNCTION	90.50	3.29	0.0	
108 (STM)	JUNCTION	90.40	3.35	0.0	
108a	JUNCTION	90.34	3.21	0.0	
11+000	JUNCTION	94.01	0.30	0.0	
11+086	JUNCTION	94.26	0.30	0.0	
110 (STM)	JUNCTION	90.24	3.22	0.0	
110a	JUNCTION	90.17	3.21	0.0	
200 (STM)	JUNCTION	91.33	3.21	0.0	
202 (STM)	JUNCTION	91.25	2.89	0.0	
202a	JUNCTION	91.14	2.87	0.0	
204 (STM)	JUNCTION	91.01	2.93	0.0	
204a	JUNCTION	90.91	2.90	0.0	

206 (STM)	JUNCTION	90.77	2.85	0.0
208 (STM)	JUNCTION	90.69	3.13	0.0
208a	JUNCTION	90.65	3.02	0.0
210 (STM)	JUNCTION	90.53	2.99	0.0
212 (STM)	JUNCTION	90.40	3.33	0.0
214 (STM)	JUNCTION	90.30	3.17	0.0
216 (STM)	JUNCTION	90.01	3.36	0.0
304 (STM)	JUNCTION	90.86	2.51	0.0
306 (STM)	JUNCTION	90.65	2.93	0.0
306a (STM)	JUNCTION	90.52	2.96	0.0
308 (STM)	JUNCTION	90.41	3.00	0.0
310 (STM)	JUNCTION	90.07	3.39	0.0
312 (STM)	JUNCTION	89.56	3.80	0.0
314 (STM)	JUNCTION	89.49	3.15	0.0
316 (STM)	JUNCTION	89.36	3.90	0.0
324 (STM)	JUNCTION	89.35	3.30	0.0
326 (STM)	JUNCTION	89.30	3.24	0.0
328 (STM)	JUNCTION	88.91	3.25	0.0
330 (STM)	JUNCTION	89.33	3.21	0.0
402 (STM)	JUNCTION	90.91	2.91	0.0
402a (STM)	JUNCTION	90.84	2.97	0.0
404 (STM)	JUNCTION	90.67	3.13	0.0
404a	JUNCTION	90.63	3.13	0.0
406 (STM)	JUNCTION	90.45	3.25	0.0
408 (STM)	JUNCTION	90.36	3.44	0.0
608 (STM)	JUNCTION	90.55	3.16	0.0
608a	JUNCTION	90.49	3.13	0.0
610 (STM)	JUNCTION	89.88	3.42	0.0
610a	JUNCTION	89.84	3.48	0.0
612 (STM)	JUNCTION	90.12	4.03	0.0
614 (STM)	JUNCTION	90.22	3.50	0.0
616 (STM)	JUNCTION	90.53	3.74	0.0
618 (STM)	JUNCTION	90.36	3.28	0.0
902 (STM)	JUNCTION	90.70	3.27	0.0
904 (STM)	JUNCTION	90.58	3.18	0.0
906 (STM)	JUNCTION	90.49	3.04	0.0
908 (STM)	JUNCTION	90.27	3.30	0.0
908a	JUNCTION	90.19	3.17	0.0
C100 (STM)	JUNCTION	90.89	2.61	0.0
C102 (STM)	JUNCTION	90.85	2.65	0.0
C104 (STM)	JUNCTION	90.81	2.69	0.0
C106 (STM)	JUNCTION	90.75	2.75	0.0
C108 (STM)	JUNCTION	90.72	2.78	0.0
C110 (STM)	JUNCTION	90.67	2.83	0.0
CB01-02	JUNCTION	92.05	1.50	0.0
CB03-04	JUNCTION	91.89	1.59	0.0
CB05-06	JUNCTION	91.85	1.50	0.0
CB07-08	JUNCTION	91.72	1.65	0.0
CB09-10	JUNCTION	91.91	1.40	0.0
CB100	JUNCTION	92.60	1.75	0.0
CB102	JUNCTION	92.60	1.75	0.0
CB104	JUNCTION	92.60	1.75	0.0
CB106	JUNCTION	92.60	1.75	0.0
CB108	JUNCTION	92.60	1.75	0.0
CB110	JUNCTION	92.60	1.75	0.0
CB11-12	JUNCTION	92.38	1.54	0.0
CB11-12ROAD	JUNCTION	93.62	0.30	0.0
CB13-14	JUNCTION	92.64	1.61	0.0
CB13-14ROAD	JUNCTION	93.84	0.41	0.0
CB15-16	JUNCTION	92.00	1.50	0.0
CB17-18	JUNCTION	92.31	1.50	0.0
CB19	JUNCTION	92.24	1.50	0.0
CB20-21	JUNCTION	92.14	1.50	0.0
CB22	JUNCTION	92.29	1.50	0.0
CB23	JUNCTION	92.08	1.50	0.0
CB24-25	JUNCTION	92.05	1.50	0.0
CB26-27	JUNCTION	91.88	1.50	0.0
CB28	JUNCTION	91.78	1.51	0.0
CB29	JUNCTION	91.78	1.50	0.0

Burnett Lands – 3370 Greenbank Road 5-year Storm, 100-year Fixed Outlet Elevations Model Output

Burnett Lands – 3370 Greenbank Road

5-year Storm, 100-year Fixed Outlet Elevations

Model Output

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

{STM}.102-104	CIRCULAR	0.61	0.29	0.15	0.61	1	470.15
{STM}.104-106	CIRCULAR	0.61	0.29	0.15	0.61	1	352.37
{STM}.106-106a	CIRCULAR	0.61	0.29	0.15	0.61	1	299.48
{STM}.108-108a	CIRCULAR	0.69	0.37	0.17	0.69	1	350.83
{STM}.110-110a	CIRCULAR	0.76	0.46	0.19	0.76	1	515.75
{STM}.306-306a	CIRCULAR	0.61	0.29	0.15	0.61	1	326.79
{STM}.308-310	CIRCULAR	0.61	0.29	0.15	0.61	1	326.80
{STM}.310-312	CIRCULAR	0.76	0.46	0.19	0.76	1	449.67
{STM}.312-314	CIRCULAR	1.22	1.17	0.30	1.22	1	1291.42
{STM}.314-316	CIRCULAR	1.22	1.17	0.30	1.22	1	1305.31
{STM}.316-324	CIRCULAR	1.22	1.17	0.30	1.22	1	1272.17
{STM}.326-328	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1	2378.14
{STM}.328-HW-1	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1	2436.50
{STM}.330-336	CIRCULAR	1.22	1.17	0.30	1.22	1	2874.95
{STM}.402-402a	CIRCULAR	0.61	0.29	0.15	0.61	1	269.05
{STM}.404-404a	CIRCULAR	0.61	0.29	0.15	0.61	1	328.85
{STM}.406-408	CIRCULAR	0.61	0.29	0.15	0.61	1	286.99
{STM}.408-106	CIRCULAR	0.61	0.29	0.15	0.61	1	287.22
{STM}.608-608a	CIRCULAR	0.61	0.29	0.15	0.61	1	284.66
{STM}.610-610a	CIRCULAR	0.99	0.77	0.25	0.99	1	811.34
{STM}.612-316	CIRCULAR	0.61	0.29	0.15	0.61	1	287.33
{STM}.614-312	CIRCULAR	0.61	0.29	0.15	0.61	1	286.33
{STM}.616-614	CIRCULAR	0.46	0.16	0.11	0.46	1	133.04
{STM}.618-610	CIRCULAR	0.61	0.29	0.15	0.61	1	350.87
{STM}.902-904	CIRCULAR	0.61	0.29	0.15	0.61	1	358.92
{STM}.904-906	CIRCULAR	0.61	0.29	0.15	0.61	1	286.54
{STM}.906-908	CIRCULAR	0.61	0.29	0.15	0.61	1	287.07
{STM}.908-908a	CIRCULAR	0.69	0.37	0.17	0.69	1	337.70
106a-108	CIRCULAR	0.61	0.29	0.15	0.61	1	285.94
108a-110	CIRCULAR	0.69	0.37	0.17	0.69	1	360.11
110a-610	CIRCULAR	0.76	0.46	0.19	0.76	1	512.86
200-202	CIRCULAR	0.69	0.37	0.17	0.69	1	351.75
202-204	CIRCULAR	0.69	0.37	0.17	0.69	1	343.69
202a-204	CIRCULAR	0.69	0.37	0.17	0.69	1	347.79
204-206	CIRCULAR	0.76	0.46	0.19	0.76	1	455.54
204a-206	CIRCULAR	0.76	0.46	0.19	0.76	1	446.46
206-208	CIRCULAR	0.76	0.46	0.19	0.76	1	458.92
208-208a	CIRCULAR	0.76	0.46	0.19	0.76	1	378.04
208a-210	CIRCULAR	0.76	0.46	0.19	0.76	1	371.17
210-212	CIRCULAR	0.84	0.55	0.21	0.84	1	486.79
212-214	CIRCULAR	0.91	0.66	0.23	0.91	1	475.29
214-216	CIRCULAR	0.91	0.66	0.23	0.91	1	638.35
216-H2	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1	2791.51
304-306	CIRCULAR	0.61	0.29	0.15	0.61	1	354.55
306a-308	CIRCULAR	0.61	0.29	0.15	0.61	1	323.83
324-330	CIRCULAR	1.22	1.17	0.30	1.22	1	2491.62
402a-404	CIRCULAR	0.61	0.29	0.15	0.61	1	277.88
404a-608	CIRCULAR	0.61	0.29	0.15	0.61	1	329.20
608a-110	CIRCULAR	0.61	0.29	0.15	0.61	1	289.47
610a-312	CIRCULAR	0.99	0.77	0.25	0.99	1	844.32
908a-610	CIRCULAR	0.69	0.37	0.17	0.69	1	340.19
C100-208	CIRCULAR	0.61	0.29	0.15	0.61	1	248.72
C102-208a	CIRCULAR	0.61	0.29	0.15	0.61	1	248.72
C104-210	CIRCULAR	0.61	0.29	0.15	0.61	1	248.72
C106-212	CIRCULAR	0.61	0.29	0.15	0.61	1	248.72
C108-214	CIRCULAR	0.61	0.29	0.15	0.61	1	248.72
C110-216	CIRCULAR	0.61	0.29	0.15	0.61	1	248.72
CB10-11ROADLINK	RECT_OPEN	0.30	3.00	0.28	10.00	1	6953.33
CB12-13ROADLINK	RECT_OPEN	0.30	6.00	0.29	20.00	1	909.75
CB27-28ROADLINK	RECT_OPEN	0.30	3.00	0.28	10.00	1	631.09
CB29-30ROADLINK	RECT_OPEN	0.30	3.00	0.28	10.00	1	631.09
CB53-ROAD	TRIANGULAR	0.30	0.27	0.14	1.80	1	118.77
CB57-ROAD	TRIANGULAR	0.30	0.27	0.14	1.80	1	118.77
CB59-ROAD	TRIANGULAR	0.30	0.27	0.14	1.80	1	187.79
CST-01	9m-ROW	0.30	3.10	0.14	21.20	1	7844.93
CST-02	9m-ROW	0.30	3.10	0.14	21.20	1	9421.17
CST-03	9m-ROW	0.30	3.10	0.14	21.20	1	6872.13
CST-04	9m-ROW	0.30	3.10	0.14	21.20	1	6872.13
CST-05	9m-ROW	0.30	3.10	0.14	21.20	1	10011.21

CST-06	9m-ROW	0.30	3.10	0.14	21.20	1	10011.21
RY-00	TRIANGULAR	0.30	0.27	0.14	1.80	1	291.33
RY-00a	TRIANGULAR	0.30	0.27	0.14	1.80	1	118.98
RY-01	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.93
RY-02	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.78
RY-03	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.88
RY-04	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.78
RY-05	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.74
RY-06	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.78
RY-07	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.32
RY-08	TRIANGULAR	0.30	0.27	0.14	1.80	1	281.70
RY-09	TRIANGULAR	0.30	0.27	0.14	1.80	1	291.08
RY-10	TRIANGULAR	0.30	0.27	0.14	1.80	1	281.70
S01-00	9m-ROW	0.30	3.10	0.14	21.20	1	3963.85
S01-01	9m-ROW	0.30	3.10	0.14	21.20	1	4762.26
S01-02	9m-ROW	0.30	3.10	0.14	21.20	1	4584.08
S01-03	9m-ROW	0.30	3.10	0.14	21.20	1	4560.54
S01-04	9m-ROW	0.30	3.10	0.14	21.20	1	4613.09
S01-05	9m-ROW	0.30	3.10	0.14	21.20	1	6349.97
S01-06	RECT_OPEN	0.30	3.00	0.28	10.00	1	12260.05
S01-07	RECT_OPEN	0.30	3.00	0.28	10.00	1	11432.19
S03-01a	9m-ROW	0.30	3.10	0.14	21.20	1	545

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

S10-02	5m-ROW	0.30	2.40	0.13	18.20	1	2729.26
S10-03	5m-ROW	0.30	2.40	0.13	18.20	1	2733.56
S10-04	RECT_OPEN	0.30	3.00	0.28	10.00	1	6642.05
S10-05	RECT_OPEN	0.30	3.00	0.28	10.00	1	5422.66
S11-01	5m-ROW	0.30	2.40	0.13	18.20	1	3310.53
S11-02	5m-ROW	0.30	2.40	0.13	18.20	1	5989.76
S11-03	5m-ROW	0.30	2.40	0.13	18.20	1	3313.94
ST10-00	5m-ROW	0.30	2.40	0.13	18.20	1	2467.78
ST6-01a	11m-ROW	0.30	3.40	0.14	23.20	1	5368.01
ST6-01b	11m-ROW	0.30	3.40	0.14	23.20	1	4305.74
ST6-01c	11m-ROW	0.30	3.40	0.14	23.20	1	8291.98
STB-00	9m-ROW	0.30	3.10	0.14	21.20	1	5778.40
STB-01	9m-ROW	0.30	3.10	0.14	21.20	1	5810.07
STB-02	9m-ROW	0.30	3.10	0.14	21.20	1	4168.40
STB-03	9m-ROW	0.30	3.10	0.14	21.20	1	4188.12
STB-04	9m-ROW	0.30	3.10	0.14	21.20	1	4621.40
STB-05	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
STB-06	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
STB-07	9m-ROW	0.30	3.10	0.14	21.20	1	4643.63
STB-08	9m-ROW	0.30	3.10	0.14	21.20	1	4562.20
STB-09	9m-ROW	0.30	3.10	0.14	21.20	1	4745.50
STB-10	9m-ROW	0.30	3.10	0.14	21.20	1	5040.78
STB-11	9m-ROW	0.30	3.10	0.14	21.20	1	5040.78
STB-12	9m-ROW	0.30	3.10	0.14	21.20	1	4158.17
STB-13	9m-ROW	0.30	3.10	0.14	21.20	1	4512.57

Transect Summary

Transect 11m-ROW

Area:

0.0003	0.0014	0.0031	0.0056	0.0087
0.0126	0.0171	0.0224	0.0283	0.0350
0.0423	0.0503	0.0591	0.0685	0.0787
0.0895	0.1010	0.1133	0.1262	0.1398
0.1542	0.1692	0.1849	0.2014	0.2185
0.2374	0.2590	0.2824	0.3070	0.3324
0.3586	0.3856	0.4133	0.4417	0.4710
0.5009	0.5317	0.5632	0.5954	0.6284
0.6622	0.6967	0.7319	0.7680	0.8047
0.8423	0.8806	0.9196	0.9594	1.0000

Hrad:

0.0203	0.0406	0.0608	0.0811	0.1014
0.1217	0.1420	0.1623	0.1825	0.2028
0.2231	0.2434	0.2637	0.2840	0.3042
0.3245	0.3448	0.3651	0.3854	0.4056
0.4259	0.4462	0.4665	0.4868	0.5071
0.4758	0.4659	0.4779	0.4994	0.5251
0.5505	0.5757	0.6006	0.6253	0.6498
0.6741	0.6982	0.7221	0.7459	0.7696
0.7931	0.8165	0.8398	0.8630	0.8860
0.9090	0.9319	0.9547	0.9774	1.0000

Width:

0.0171	0.0342	0.0512	0.0683	0.0854
0.1025	0.1196	0.1366	0.1537	0.1708
0.1879	0.2049	0.2220	0.2391	0.2562
0.2733	0.2903	0.3074	0.3245	0.3416
0.3587	0.3757	0.3928	0.4099	0.4270
0.4954	0.5528	0.5880	0.6121	0.6305
0.6490	0.6675	0.6860	0.7044	0.7229
0.7414	0.7599	0.7783	0.7968	0.8153
0.8337	0.8522	0.8707	0.8892	0.9076
0.9261	0.9446	0.9631	0.9815	1.0000

Transect 5m-ROW

Area:

0.0005	0.0019	0.0043	0.0076	0.0119
0.0172	0.0234	0.0306	0.0387	0.0478
0.0578	0.0688	0.0807	0.0934	0.1062
0.1190	0.1318	0.1446	0.1574	0.1703
0.1832	0.1961	0.2090	0.2220	0.2349
0.2494	0.2668	0.2857	0.3058	0.3271
0.3496	0.3732	0.3981	0.4241	0.4512
0.4796	0.5091	0.5398	0.5717	0.6048
0.6390	0.6744	0.7110	0.7487	0.7877
0.8278	0.8691	0.9115	0.9552	1.0000

Hrad:

0.0225	0.0450	0.0676	0.0901	0.1126
0.1351	0.1577	0.1802	0.2027	0.2252
0.2477	0.2703	0.2928	0.3297	0.3737
0.4176	0.4612	0.5047	0.5480	0.5912
0.6342	0.6770	0.7197	0.7622	0.8045
0.6979	0.6515	0.6566	0.6637	0.6724
0.6826	0.6940	0.7064	0.7197	0.7339
0.7487	0.7642	0.7802	0.7967	0.8137
0.8310	0.8487	0.8668	0.8851	0.9037
0.9225	0.9416	0.9609	0.9804	1.0000

Width:

0.0210	0.0421	0.0631	0.0842	0.1052
0.1263	0.1473	0.1684	0.1894	0.2104
0.2315	0.2525	0.2736	0.2809	0.2813
0.2818	0.2822	0.2826	0.2831	0.2835
0.2840	0.2844	0.2848	0.2853	0.2857
0.3516	0.4042	0.4301	0.4560	0.4819
0.5078	0			

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

Control Actions Taken

Runoff Quantity	Continuity	Volume hectare-m	Depth mm
Total Precipitation	1.039	45.162	
Evaporation Loss	0.000	0.000	
Infiltration Loss	0.376	16.351	
Surface Runoff	0.650	28.232	
Final Surface Storage	0.012	0.542	
Continuity Error (%)	0.082		
Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr	
Dry Weather Inflow	0.000	0.000	
Wet Weather Inflow	0.650	6.496	
Groundwater Inflow	0.000	0.000	
RDII Inflow	0.000	0.000	
External Inflow	0.006	0.055	
External Outflow	0.659	6.592	
Internal Outflow	0.000	0.000	
Storage Losses	0.000	0.000	
Initial Stored Volume	0.114	1.141	
Final Stored Volume	0.114	1.138	
Continuity Error (%)	-0.494		
Highest Continuity Errors			
Node 02+196 (99.64%)			
Node CB11-12ROAD (-32.99%)			
Node 04+078 (9.02%)			
Node 02+512 (8.12%)			
Node C100 (STM) (6.41%)			
Time-Step Critical Elements			
None			
Highest Flow Instability Indexes			
Link C100-208 (104)			
Link C110-216 (103)			
Link C108-214 (103)			
Link 206-208 (102)			
Link C104-210 (101)			
Minimum Time Step : 1.82 sec			
Average Time Step : 4.99 sec			
Maximum Time Step : 5.00 sec			
Percent in Steady State : 0.00			
Average Iterations per Step : 6.82			

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
A-01	45.16	0.00	0.00	14.73	29.94	0.07	44.76	0.663
A-02	45.16	0.00	0.00	14.87	29.80	0.07	46.82	0.660
A-03	45.16	0.00	0.00	14.49	30.44	0.04	24.81	0.674
A-04	45.16	0.00	0.00	15.02	29.65	0.09	55.90	0.657
A-05	45.16	0.00	0.00	14.91	30.01	0.08	51.50	0.664
A-06	45.16	0.00	0.00	14.91	29.86	0.08	51.32	0.661
A-07	45.16	0.00	0.00	14.91	29.76	0.09	54.12	0.659
A-08	45.16	0.00	0.00	14.79	30.14	0.07	46.63	0.667
A-09	45.16	0.00	0.00	14.49	30.19	0.02	15.82	0.669
A-10	45.16	0.00	0.00	14.69	29.99	0.06	35.77	0.664
A-11	45.16	0.00	0.00	15.03	29.64	0.07	41.61	0.656
A-12	45.16	0.00	0.00	14.11	28.94	0.01	6.67	0.641
A-13	45.16	0.00	0.00	14.98	29.69	0.06	37.18	0.657
A-14	45.16	0.00	0.00	14.99	29.68	0.06	36.21	0.657
A-15	45.16	0.00	0.00	14.49	28.92	0.03	19.43	0.640
A-16	45.16	0.00	0.00	14.56	28.92	0.04	23.68	0.640
A-17	45.16	0.00	0.00	14.69	29.99	0.07	47.07	0.664
A-18	45.16	0.00	0.00	14.99	29.68	0.08	50.50	0.657
A-19	45.16	0.00	0.00	14.55	28.92	0.04	22.57	0.640
A-20	45.16	0.00	0.00	14.62	28.92	0.04	24.03	0.640
A-21	45.16	0.00	0.00	14.96	29.71	0.06	40.44	0.658
A-22	45.16	0.00	0.00	14.32	28.93	0.02	11.11	0.641
A-23	45.16	0.00	0.00	15.08	29.83	0.14	83.88	0.661
A-24	45.16	0.00	0.00	14.92	30.01	0.06	39.13	0.664
A-25	45.16	0.00	0.00	14.91	30.01	0.06	40.09	0.665
A-26	45.16	0.00	0.00	15.09	29.78	0.13	82.21	0.659
A-27	45.16	0.00	0.00	14.78	30.14	0.06	36.04	0.667
A-28	45.16	0.00	0.00	15.23	28.93	0.07	41.39	0.641
A-29	45.16	0.00	0.00	14.68	30.09	0.07	44.16	0.666
A-30	45.16	0.00	0.00	14.88	29.79	0.15	93.52	0.660
A-31	45.16	0.00	0.00	14.42	29.77	0.02	11.99	0.659
A-32	45.16	0.00	0.00	14.96	29.96	0.13	83.95	0.663
A-33	45.16	0.00	0.00	14.88	29.80	0.24	153.37	0.660
A-34	45.16	0.00	0.00	15.28	28.87	0.22	137.01	0.639
A-35	45.16	0.00	0.00	15.28	28.87	0.28	174.10	0.639
A-36	45.16	0.00	0.00	14.92	29.25	0.13	84.22	0.648
A-37	45.16	0.00	0.00	14.88	29.29	0.11	69.89	0.649
B-01	45.16	0.00	0.00	14.46	30.23	0.03	18.11	0.669
B-02	45.16	0.00	0.00	14.83	29.34	0.04	28.07	0.650
B-03	45.16	0.00	0.00	14.42	29.87	0.03	19.11	0.661
B-04	45.16	0.00	0.00	14.92	29.75	0.25	154.40	0.659
B-05	45.16	0.00	0.00	14.30	28.93	0.02	11.90	0.641
B-06	45.16	0.00	0.00	14.56	29.62	0.02	14.72	0.656
B-07	45.16	0.00	0.00	14.55	30.03	0.04	24.57	0.665
B-08	45.16	0.00	0.00	14.88	29.79	0.07	42.35	0.660
B-09	45.16	0.00	0.00	14.90	29.78	0.07	45.99	0.659
B-10	45.16	0.00	0.00	37.87	7.07	0.08	47.11	0.157
B-11	45.16	0.00	0.00	14.52	29.66	0.01	9.45	0.657
B-12	45.16	0.00	0.00	14.61	30.07	0.03	16.82	0.666
B-13	45.16	0.00	0.00	14.61	30.07	0.03	18.72	0.666
B-14	45.16	0.00	0.00	14.61	30.07	0.10	61.62	0.666
B-15	45.16	0.00	0.00	14.62	30.06	0.04	23.16	0.666
B-16	45.16	0.00	0.00	14.61	30.07	0.04	26.16	0.666
B-17	45.16	0.00	0.00	14.98	29.69	0.08	50.86	0.657
B-18	45.16	0.00	0.00	14.60	30.08	0.04	23.42	0.666
B-19	45.16	0.00	0.00	14.67	30.01	0.03	20.56	0.664
CaivanLands	45.16	0.00	0.00	15.86	28.74	2.33	1044.59	0.636

Node Depth Summary

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Occurrence days	Max hr:min
01+011	JUNCTION	0.00	0.00	93.48	0	00:00
01+051	JUNCTION	0.00	0.00	93.76	0	00:00
01+118	JUNCTION	0.00	0.00	93.72	0	00:00
01+143	JUNCTION	0.00	0.00	93.93	0	00:00
02+196	JUNCTION	0.00	0.00	93.67	0	01:30
02+288	JUNCTION	0.00	0.00	93.76	0	00:00
02+365	JUNCTION	0.00	0.00	93.84	0	00:00
02+432	JUNCTION	0.00	0.00	93.86	0	00:00
02+458	JUNCTION	0.00	0.00	93.77	0	00:00
02+512	JUNCTION	0.00	0.01	93.87	0	01:31
02+564	JUNCTION	0.00	0.00	94.40	0	00:00
03+026	JUNCTION	0.00	0.00	93.56	0	00:00
03+054	JUNCTION	0.00	0.00	93.61	0	00:00
03+109	JUNCTION	0.00	0.00	93.44	0	00:00
03+127	JUNCTION	0.00	0.00	93.37	0	00:00
03+200	JUNCTION	0.00	0.00	93.42	0	00:00
03+291	JUNCTION	0.00	0.00	93.34	0	00:00
03+384	JUNCTION	0.00	0.00	93.34	0	00:00
04+078	JUNCTION	0.00	0.01	93.66	0	01:39
04+089	JUNCTION	0.00	0.00	93.76	0	00:00
04+129	JUNCTION	0.00	0.00	93.64	0	00:00
04+207	JUNCTION	0.00	0.00	93.87	0	00:00
05+116	JUNCTION	0.00	0.00	93.93	0	00:00
05+200	JUNCTION	0.00	0.03	93.52	0	01:31
06+023	JUNCTION	0.00	0.00	93.64	0	00:00
06+043	JUNCTION	0.00	0.00	93.74	0	00:00
06+108	JUNCTION	0.00	0.00	93.60	0	00:00
06+194	JUNCTION	0.00	0.00	93.51	0	00:00
06+273	JUNCTION	0.00	0.00	93.40	0	00:00
06+309	JUNCTION	0.00	0.00	93.88	0	00:00
06+332	JUNCTION	0.00	0.00	94.51	0	00:00
06+355	JUNCTION	0.00	0.00	94.15	0	00:00
09+000	JUNCTION	0.00	0.00	93.36	0	00:00
09+097	JUNCTION	0.00	0.00	93.65	0	00:00
09+203	JUNCTION	0.00	0.00	93.89	0	00:00
10+011	JUNCTION	0.00	0.00	93.58	0	00:00
10+232	JUNCTION	0.00	0.00	94.36	0	00:00
102 (STM)	JUNCTION	0.47	1.21	92.04	0	01:29
104 (STM)	JUNCTION	0.63	1.37	92.04	0	01:30
106 (STM)	JUNCTION	0.73	1.47	92.04	0	01:30
106a	JUNCTION	0.80	1.52	92.02	0	01:30
108 (STM)	JUNCTION	0.90	1.61	92.01	0	01:30
108a	JUNCTION	0.96	1.64	91.98	0	01:30
11+000	JUNCTION	0.00	0.00	94.01	0	00:00
11+086	JUNCTION	0.00	0.00	94.26	0	00:00
110 (STM)	JUNCTION	1.06	1.71	91.95	0	01:30
110a	JUNCTION	1.13	1.72	91.89	0	01:30
200 (STM)	JUNCTION	0.27	1.04	92.37	0	01:24
202 (STM)	JUNCTION	0.35	1.12	92.37	0	01:24
202a	JUNCTION	0.45	1.17	92.31	0	01:24
204 (STM)	JUNCTION	0.58	1.25	92.26	0	01:24
204a	JUNCTION	0.68	1.30	92.21	0	01:30
206 (STM)	JUNCTION	0.82	1.36	92.13	0	01:24
208 (STM)	JUNCTION	0.90	1.37	92.06	0	01:24
208a	JUNCTION	0.94	1.34	91.99	0	01:24
210 (STM)	JUNCTION	1.06	1.38	91.91	0	01:24
212 (STM)	JUNCTION	1.18	1.41	91.81	0	01:24
214 (STM)	JUNCTION	1.28	1.44	91.74	0	01:24
216 (STM)	JUNCTION	1.57	1.64	91.65	0	01:28
304 (STM)	JUNCTION	0.43	0.87	91.73	0	01:30
306 (STM)	JUNCTION	0.64	1.07	91.72	0	01:30
306a (STM)	JUNCTION	0.77	1.19	91.71	0	01:30

308 (STM)	JUNCTION	0.88	1.30	91.71	0	01:30
310 (STM)	JUNCTION	1.22	1.60	91.67	0	01:29
312 (STM)	JUNCTION	1.73	2.09	91.65	0	01:30
314 (STM)	JUNCTION	1.80	2.10	91.58	0	01:30
316 (STM)	JUNCTION	1.92	2.13	91.49	0	01:29
324 (STM)	JUNCTION	1.93	2.06	91.41	0	01:30
326 (STM)	JUNCTION	1.98	2.06	91.36	0	01:30
328 (STM)	JUNCTION	2.37	2.39	91.30	0	01:28
330 (STM)	JUNCTION	1.96	2.06	91.38	0	01:28
402 (STM)	JUNCTION	0.39	1.12	92.03	0	01:30
402a (STM)	JUNCTION	0.46	1.18	92.02	0	01:30
404 (STM)	JUNCTION	0.63	1.35	92.02	0	01:30
404a	JUNCTION	0.67	1.38	92.01	0	01:30
406 (STM)	JUNCTION	0.84	1.75	92.20	0	01:22
408 (STM)	JUNCTION	0.94	1.68	92.04	0	01:30
608 (STM)	JUNCTION	0.75	1.45	92.00	0	01:30
608a	JUNCTION	0.80	1.49	91.98	0	01:30
610 (STM)	JUNCTION	1.41	1.92	91.80	0	01:30
610a	JUNCTION	1.45	1.90	91.74	0	01:29
612 (STM)	JUNCTION	1.17	1.52	91.64	0	01:30
614 (STM)	JUNCTION	1.07	1.49	91.71	0	01:30
616 (STM)	JUNCTION	0.76	1.19	91.72	0	01:29
618 (STM)	JUNCTION	0.93	1.43	91.80	0	01:29
902 (STM)	JUNCTION	0.59	1.13	91.83	0	01:30
904 (STM)	JUNCTION	0.71	1.26	91.84	0	01:30
906 (STM)	JUNCTION	0.81	1.34	91.83	0	01:30
908 (STM)	JUNCTION	1.02	1.54	91.81	0	01:30
908a	JUNCTION	1.10	1.62	91.81	0	01:30
C100 (STM)	JUNCTION	0.70	1.18	92.07	0	01:24
C102 (STM)	JUNCTION	0.74	1.14	91.99	0	01:24
C104 (STM)	JUNCTION	0.78	1.10	91.91	0	01:24
C106 (STM)	JUNCTION	0.83	1.07	91.82	0	01:24
C108 (STM)	JUNCTION	0.86	1.03	91.75	0	01:24
C110 (STM)	JUNCTION	0.95	2.24	92.91	0	01:31
CB01-02	JUNCTION	0.04	1.25	93.30	0	01:30
CB03-04	JUNCTION	0.04	1.33	93.22	0	01:30
CB05-06	JUNCTION	0.04	1.21	93.06	0	01:30
CB07-08	JUNCTION	0.05	1.24	92.96	0	01:31
CB09-10	JUNCTION	0.04	1.15	93.06	0	01:30
CB100	JUNCTION	0.04	1.41	94.01	0	01:30
CB102	JUNCTION	0.03	1.40	94.00	0	01:30
CB104	JUNCTION	0.03	1.40	94.00	0	01:30
CB106	JUNCTION	0.04	1.41	94.01	0	01:30
CB108	JUNCTION	0.03	1.40	94.00	0	01:30
CB110	JUNCTION	0.04	1.41	94.01	0	01:30
CB11-12	JUNCTION	0.06				

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	06+108 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	06+194 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	06+273 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	06+309 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	06+332 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	06+355 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	09+000 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	09+097 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	09+203 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	10+011 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	10+232 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	102 (STM) JUNCTION 0.00 15.78 0 01:22 0.000 0.009	104 (STM) JUNCTION 0.00 26.26 0 01:22 0.000 0.045	106 (STM) JUNCTION 0.00 110.37 0 01:29 0.000 0.229	106a JUNCTION 0.00 158.35 0 01:30 0.000 0.296	108 (STM) JUNCTION 0.00 204.68 0 01:29 0.000 0.371	108a JUNCTION 0.00 241.07 0 01:30 0.000 0.436	11+000 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	11+086 JUNCTION 0.00 0.00 0 00:00 0.000 0.000	110 (STM) JUNCTION 0.00 400.38 0 01:30 0.000 0.860	110a JUNCTION 0.00 410.40 0 01:30 0.000 0.869	200 (STM) JUNCTION 0.00 73.36 0 01:23 0.000 0.036	202 (STM) JUNCTION 0.00 209.46 0 01:29 0.000 0.360	202a JUNCTION 0.00 234.99 0 01:30 0.000 0.402	204 (STM) JUNCTION 0.00 251.01 0 01:30 0.000 0.441	204a JUNCTION 0.00 288.23 0 01:30 0.000 0.505	206 (STM) JUNCTION 0.00 362.04 0 01:30 0.000 0.656	208 (STM) JUNCTION 0.00 386.50 0 01:30 0.000 0.699	208a JUNCTION 0.00 403.87 0 01:30 0.000 0.734	210 (STM) JUNCTION 0.00 479.30 0 01:30 0.000 0.864	212 (STM) JUNCTION 0.00 504.98 0 01:30 0.000 0.907	214 (STM) JUNCTION 0.00 577.26 0 01:30 0.000 1.023	216 (STM) JUNCTION 0.00 1521.63 0 01:30 0.000 3.378	304 (STM) JUNCTION 0.00 37.48 0 01:30 0.000 0.080	306 (STM) JUNCTION 0.00 76.89 0 01:31 0.000 0.160	306a (STM) JUNCTION 0.00 77.25 0 01:31 0.000 0.157	308 (STM) JUNCTION 0.00 130.13 0 01:30 0.000 0.245	310 (STM) JUNCTION 0.00 151.15 0 01:30 0.000 0.334	312 (STM) JUNCTION 0.00 965.09 0 01:30 0.000 2.037	314 (STM) JUNCTION 0.00 964.54 0 01:30 0.000 2.039	316 (STM) JUNCTION 0.00 1200.09 0 01:30 0.000 2.438	324 (STM) JUNCTION 0.00 1200.47 0 01:30 0.000 2.542	326 (STM) JUNCTION 0.00 1208.78 0 01:30 0.000 2.510	328 (STM) JUNCTION 0.00 1199.35 0 01:30 0.000 2.504	330 (STM) JUNCTION 0.00 729.67 0 01:30 0.000 1.723	402 (STM) JUNCTION 0.00 41.46 0 01:21 0.000 0.096	402a (STM) JUNCTION 0.00 57.98 0 01:38 0.000 0.219	404 (STM) JUNCTION 0.00 68.63 0 01:31 0.000 0.243	404a JUNCTION 0.00 127.08 0 01:31 0.000 0.364	406 (STM) JUNCTION 0.00 57.61 0 01:22 0.000 0.069	408 (STM) JUNCTION 0.00 44.12 0 01:22 0.000 0.082	608 (STM) JUNCTION 0.00 128.74 0 01:31 0.000 0.361	608a JUNCTION 0.00 135.10 0 01:31 0.000 0.371	610 (STM) JUNCTION 0.00 634.86 0 01:31 0.000 1.401	610a JUNCTION 0.00 691.16 0 01:31 0.000 1.479	612 (STM) JUNCTION 0.00 237.19 0 01:30 0.000 0.407	614 (STM) JUNCTION 0.00 237.08 0 01:30 0.000 0.402	616 (STM) JUNCTION 0.00 11.51 0 01:30 0.000 0.019	618 (STM) JUNCTION 0.00 39.57 0 01:42 0.000 0.186	902 (STM) JUNCTION 0.00 4.58 0 03:51 0.000 0.009	904 (STM) JUNCTION 0.00 5.96 0 01:22 0.000 0.019	906 (STM) JUNCTION 0.00 74.39 0 01:31 0.000 0.149	908 (STM) JUNCTION 0.00 74.82 0 01:31 0.000 0.144	908a JUNCTION 0.00 147.25 0 01:31 0.000 0.282	C100 (STM) JUNCTION 0.00 15.20 0 01:30 0.000 0.030	C102 (STM) JUNCTION 0.00 17.57 0 01:30 0.000 0.033	C104 (STM) JUNCTION 0.00 22.00 0 01:30 0.000 0.040	C106 (STM) JUNCTION 0.00 24.91 0 01:30
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Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

C110 (STM)	JUNCTION	0.00	948.05	0	01:33	0.000	2.360
CB01-02	JUNCTION	44.76	44.76	0	01:30	0.070	0.070
CB03-04	JUNCTION	46.82	46.82	0	01:30	0.074	0.074
CB05-06	JUNCTION	55.89	55.89	0	01:30	0.090	0.090
CB07-08	JUNCTION	51.31	51.31	0	01:30	0.082	0.082
CB09-10	JUNCTION	44.16	64.91	0	01:30	0.069	0.088
CB100	JUNCTION	16.82	16.82	0	01:30	0.026	0.026
CB102	JUNCTION	18.72	18.72	0	01:30	0.029	0.029
CB104	JUNCTION	23.16	23.16	0	01:30	0.036	0.036
CB106	JUNCTION	26.15	26.15	0	01:30	0.041	0.041
CB108	JUNCTION	23.41	23.41	0	01:30	0.037	0.037
CB110	JUNCTION	20.56	20.56	0	01:30	0.032	0.032
CB11-12	JUNCTION	0.00	21.84	0	01:30	0.000	0.092
CB11-12ROAD	JUNCTION	51.50	51.50	0	01:30	0.083	0.083
CB13-14	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
CB13-14ROAD	JUNCTION	24.80	24.80	0	01:30	0.039	0.039
CB15-16	JUNCTION	54.12	54.12	0	01:30	0.086	0.086
CB17-18	JUNCTION	46.63	70.57	0	01:30	0.074	0.114
CB19	JUNCTION	15.81	15.81	0	01:30	0.024	0.024
CB20-21	JUNCTION	77.37	77.37	0	01:30	0.123	0.123
CB22	JUNCTION	36.21	36.21	0	01:30	0.058	0.058
CB23	JUNCTION	37.18	37.18	0	01:30	0.059	0.059
CB24-25	JUNCTION	40.43	40.43	0	01:30	0.064	0.064
CB26-27	JUNCTION	82.20	82.20	0	01:30	0.133	0.133
CB28	JUNCTION	0.00	17.93	0	01:30	0.000	0.007
CB29	JUNCTION	41.38	41.38	0	01:30	0.067	0.067
CB30-31	JUNCTION	36.04	106.80	0	01:30	0.057	0.183
CB32-33	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
CB32-33 ROAD	JUNCTION	40.09	76.62	0	01:30	0.064	0.127
CB34-35	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
CB34-35 ROAD	JUNCTION	39.13	39.13	0	01:30	0.063	0.063
CB36	JUNCTION	18.11	18.11	0	01:30	0.028	0.028
CB37	JUNCTION	28.06	29.24	0	01:30	0.044	0.044
CB39	JUNCTION	19.10	19.68	0	01:30	0.029	0.029
CB40-41	JUNCTION	83.87	83.87	0	01:30	0.136	0.136
CB42-43	JUNCTION	50.49	50.49	0	01:30	0.081	0.081
CB44-45	JUNCTION	47.07	47.07	0	01:30	0.074	0.074
CB46-47	JUNCTION	14.72	15.09	0	01:30	0.023	0.023
CB48-49	JUNCTION	24.57	24.57	0	01:30	0.038	0.038
CB50-51	JUNCTION	42.34	42.34	0	01:30	0.067	0.067
CB52-53	JUNCTION	93.09	93.09	0	01:30	0.152	0.152
CB54	JUNCTION	9.45	9.63	0	01:30	0.014	0.014
CB55-56	JUNCTION	61.62	61.62	0	01:30	0.096	0.096
CB57-58	JUNCTION	50.86	53.08	0	01:30	0.081	0.081
CB59	JUNCTION	6.67	6.67	0	01:30	0.010	0.010
CB60	JUNCTION	24.03	24.03	0	01:30	0.038	0.038
CB61	JUNCTION	11.11	11.11	0	01:30	0.017	0.017
CB62	JUNCTION	22.57	22.57	0	01:30	0.035	0.035
CB63	JUNCTION	23.68	23.68	0	01:30	0.037	0.037
CB64	JUNCTION	19.43	19.43	0	01:30	0.030	0.030
CB-65	JUNCTION	11.90	11.90	0	01:30	0.019	0.019
CB66	JUNCTION	11.99	11.99	0	01:30	0.018	0.018
CB67-68-69	JUNCTION	83.94	83.94	0	01:30	0.135	0.135
HP-01	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
HP-03	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
RYP-01	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
RYP-02	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
RYP-03	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
RYP-04	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
1-HW-01 (STM)	OUTFALL	0.00	1199.64	0	01:30	0.000	2.521
2-OverlandOutlet	OUTFALL	0.00	0.00	0	00:00	0.000	0.000
3-HW-02 (STM)	OUTFALL	0.00	1521.75	0	01:30	0.000	3.379
4-GreenbankOut	OUTFALL	465.16	465.16	0	01:30	0.747	0.747
A30-STOR	STORAGE	93.51	93.51	0	01:30	0.148	0.148
A33-STOR	STORAGE	153.35	153.35	0	01:30	0.243	0.243
B04_Stor	STORAGE	154.38	154.38	0	01:30	0.245	0.245
Caivan-Stor	STORAGE	1044.25	1044.25	0	01:30	2.328	2.328

Node Surcharge Summary					

Surcharging occurs when water rises above the top of the highest conduit.					

Node	Type	Hours Surcharged	Max. Height Meters	Min. Depth Meters	
102 (STM)	JUNCTION	0.32	0.604	1.740	
104 (STM)	JUNCTION	0.70	0.723	1.896	
106 (STM)	JUNCTION	12.00	0.809	1.837	
106a	JUNCTION	12.00	0.913	1.767	
108 (STM)	JUNCTION	12.00	0.919	1.747	
108a	JUNCTION	12.00	0.957	1.567	
110 (STM)	JUNCTION	12.00	0.949	1.510	
110a	JUNCTION	12.00	0.958	1.494	
200 (STM)	JUNCTION	0.15	0.359	2.165	
202 (STM)	JUNCTION	0.17	0.433	1.771	
202a	JUNCTION	0.21	0.482	1.702	
204 (STM)	JUNCTION	0.24	0.486	1.682	
204a	JUNCTION	0.35	0.537	1.601	
206 (STM)	JUNCTION	12.00	0.600	1.488	
208 (STM)	JUNCTION	12.00	0.611	1.757	
208a	JUNCTION	12.00	0.579	1.679	
210 (STM)	JUNCTION	12.00	0.535	1.613	
212 (STM)	JUNCTION	12.00	0.497	1.915	
214 (STM)	JUNCTION	12.00	0.458	1.728	
216 (STM)	JUNCTION	12.00	0.420	1.720	
304 (STM)	JUNCTION	0.20	0.261	1.639	
306 (STM)	JUNCTION	12.00	0.451	1.856	
306a (STM)	JUNCTION	12.00	0.582	1.768	
308 (STM)	JUNCTION	12.00	0.676	1.701	
310 (STM)	JUNCTION	12.00	0.834	1.790	
312 (STM)	JUNCTION	12.00	0.873	1.706	
314 (STM)	JUNCTION	12.00	0.865	1.054	
316 (STM)	JUNCTION	12.00	0.876	1.775	
324 (STM)	JUNCTION	12.00	0.714	1.238	
326 (STM)	JUNCTION	12.00	0.708	1.182	
328 (STM)	JUNCTION	12.00	0.845	0.857	
330 (STM)	JUNCTION	12.00	0.83		

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

B04_Stor	STORAGE	0.35	1.150	0.298																																																																																																																															
Caivan-Stor	STORAGE	0.21	0.690	0.320																																																																																																																															

Node Flooding Summary																																																																																																																																			

No nodes were flooded.																																																																																																																																			

Storage Volume Summary																																																																																																																																			

<table border="1"> <thead> <tr> <th>Storage Unit</th> <th>Average Volume 1000 m³</th> <th>Avg Pcnt Full</th> <th>E&I Loss</th> <th>Maximum Volume 1000 m³</th> <th>Max Pcnt Full</th> <th>Time of Max Occurrence days hr:min</th> <th>Maximum Outflow LPS</th> </tr> </thead> <tbody> <tr> <td>A30-STOR</td> <td>0.000</td> <td>0</td> <td>0</td> <td>0.001</td> <td>0</td> <td>0 01:30</td> <td>92.73</td> </tr> <tr> <td>A33-STOR</td> <td>0.000</td> <td>0</td> <td>0</td> <td>0.001</td> <td>1</td> <td>0 01:30</td> <td>151.28</td> </tr> <tr> <td>B04_Stor</td> <td>0.000</td> <td>0</td> <td>0</td> <td>0.001</td> <td>0</td> <td>0 01:30</td> <td>153.25</td> </tr> <tr> <td>Caivan-Stor</td> <td>0.000</td> <td>0</td> <td>0</td> <td>0.013</td> <td>1</td> <td>0 01:31</td> <td>933.92</td> </tr> </tbody> </table>						Storage Unit	Average Volume 1000 m ³	Avg Pcnt Full	E&I Loss	Maximum Volume 1000 m ³	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS	A30-STOR	0.000	0	0	0.001	0	0 01:30	92.73	A33-STOR	0.000	0	0	0.001	1	0 01:30	151.28	B04_Stor	0.000	0	0	0.001	0	0 01:30	153.25	Caivan-Stor	0.000	0	0	0.013	1	0 01:31	933.92																																																																																						
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B04_Stor	0.000	0	0	0.001	0	0 01:30	153.25																																																																																																																												
Caivan-Stor	0.000	0	0	0.013	1	0 01:31	933.92																																																																																																																												

Outfall Loading Summary																																																																																																																																			

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System	68.15	163.01	3181.45	6.647																																																																																																																															

Link Flow Summary																																																																																																																																			

<table border="1"> <thead> <tr> <th>Link</th> <th>Type</th> <th>Maximum Flow LPS</th> <th>Time of Max Occurrence days hr:min</th> <th>Maximum Veloc m/sec</th> <th>Max/Full Flow</th> <th>Max/Depth</th> </tr> </thead> <tbody> <tr> <td>{STM}.102-104</td> <td>CONDUIT</td> <td>15.78</td> <td>0 01:22</td> <td>0.05</td> <td>0.03</td> <td>1.00</td> </tr> <tr> <td>{STM}.104-106</td> <td>CONDUIT</td> <td>20.75</td> <td>0 01:28</td> <td>0.07</td> <td>0.06</td> <td>1.00</td> </tr> <tr> <td>{STM}.106-106a</td> <td>CONDUIT</td> <td>109.95</td> <td>0 01:30</td> <td>0.38</td> <td>0.37</td> <td>1.00</td> </tr> <tr> <td>{STM}.108-108a</td> <td>CONDUIT</td> <td>204.29</td> <td>0 01:30</td> <td>0.55</td> <td>0.58</td> <td>1.00</td> </tr> <tr> <td>{STM}.110-110a</td> <td>CONDUIT</td> <td>400.00</td> <td>0 01:30</td> <td>0.88</td> <td>0.78</td> <td>1.00</td> </tr> <tr> <td>{STM}.306-306a</td> <td>CONDUIT</td> <td>77.25</td> <td>0 01:31</td> <td>0.26</td> <td>0.24</td> <td>1.00</td> </tr> <tr> <td>{STM}.308-310</td> <td>CONDUIT</td> <td>130.12</td> <td>0 01:30</td> <td>0.45</td> <td>0.40</td> <td>1.00</td> </tr> <tr> <td>{STM}.310-312</td> <td>CONDUIT</td> <td>151.95</td> <td>0 01:30</td> <td>0.33</td> <td>0.34</td> <td>1.00</td> </tr> <tr> <td>{STM}.312-314</td> <td>CONDUIT</td> <td>964.54</td> <td>0 01:30</td> <td>0.83</td> <td>0.75</td> <td>1.00</td> </tr> <tr> <td>{STM}.314-316</td> <td>CONDUIT</td> <td>966.20</td> <td>0 01:30</td> <td>0.83</td> <td>0.74</td> <td>1.00</td> </tr> <tr> <td>{STM}.316-324</td> <td>CONDUIT</td> <td>1200.47</td> <td>0 01:30</td> <td>1.03</td> <td>0.94</td> <td>1.00</td> </tr> <tr> <td>{STM}.326-328</td> <td>CONDUIT</td> <td>1199.35</td> <td>0 01:30</td> <td>0.64</td> <td>0.50</td> <td>1.00</td> </tr> <tr> <td>{STM}.328-HW-1</td> <td>CONDUIT</td> <td>1199.64</td> <td>0 01:30</td> <td>0.64</td> <td>0.49</td> <td>1.00</td> </tr> <tr> <td>{STM}.330-336</td> <td>CONDUIT</td> <td>731.87</td> <td>0 01:30</td> <td>0.63</td> <td>0.25</td> <td>1.00</td> </tr> <tr> <td>{STM}.402-402a</td> <td>CONDUIT</td> <td>32.64</td> <td>0 01:38</td> <td>0.12</td> <td>0.12</td> <td>1.00</td> </tr> <tr> <td>{STM}.404-404a</td> <td>CONDUIT</td> <td>70.05</td> <td>0 01:31</td> <td>0.24</td> <td>0.21</td> <td>1.00</td> </tr> <tr> <td>{STM}.406-408</td> <td>CONDUIT</td> <td>41.01</td> <td>0 01:22</td> <td>0.14</td> <td>0.14</td> <td>1.00</td> </tr> </tbody> </table>						Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/Full Flow	Max/Depth	{STM}.102-104	CONDUIT	15.78	0 01:22	0.05	0.03	1.00	{STM}.104-106	CONDUIT	20.75	0 01:28	0.07	0.06	1.00	{STM}.106-106a	CONDUIT	109.95	0 01:30	0.38	0.37	1.00	{STM}.108-108a	CONDUIT	204.29	0 01:30	0.55	0.58	1.00	{STM}.110-110a	CONDUIT	400.00	0 01:30	0.88	0.78	1.00	{STM}.306-306a	CONDUIT	77.25	0 01:31	0.26	0.24	1.00	{STM}.308-310	CONDUIT	130.12	0 01:30	0.45	0.40	1.00	{STM}.310-312	CONDUIT	151.95	0 01:30	0.33	0.34	1.00	{STM}.312-314	CONDUIT	964.54	0 01:30	0.83	0.75	1.00	{STM}.314-316	CONDUIT	966.20	0 01:30	0.83	0.74	1.00	{STM}.316-324	CONDUIT	1200.47	0 01:30	1.03	0.94	1.00	{STM}.326-328	CONDUIT	1199.35	0 01:30	0.64	0.50	1.00	{STM}.328-HW-1	CONDUIT	1199.64	0 01:30	0.64	0.49	1.00	{STM}.330-336	CONDUIT	731.87	0 01:30	0.63	0.25	1.00	{STM}.402-402a	CONDUIT	32.64	0 01:38	0.12	0.12	1.00	{STM}.404-404a	CONDUIT	70.05	0 01:31	0.24	0.21	1.00	{STM}.406-408	CONDUIT	41.01	0 01:22	0.14	0.14	1.00
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{STM}.406-408	CONDUIT	41.01	0 01:22	0.14	0.14	1.00																																																																																																																													

{STM}.408-106	CONDUIT	44.12	0 01:22	0.15	0.15	1.00
{STM}.608-608a	CONDUIT	130.29	0 01:31	0.45	0.46	1.00
{STM}.610-610a	CONDUIT	636.09	0 01:31	0.82	0.78	1.00
{STM}.612-316	CONDUIT	237.04	0 01:30	0.81	0.82	1.00
{STM}.614-312	CONDUIT	237.19	0 01:30	0.81	0.83	1.00
{STM}.616-614	CONDUIT	11.63	0 01:30	0.07	0.09	1.00
{STM}.618-610	CONDUIT	40.08	0 01:44	0.14	0.11	1.00
{STM}.902-904	CONDUIT	4.79	0 01:07	0.02	0.01	1.00
{STM}.904-906	CONDUIT	5.96	0 01:22	0.02	0.02	1.00
{STM}.906-908	CONDUIT	74.82	0 01:31	0.26	0.26	1.00
{STM}.908-908a	CONDUIT	75.53	0 01:33	0.20	0.22	1.00
106a-108	CONDUIT	159.11	0 01:30	0.54	0.56	1.00
108a-110	CONDUIT	241.16	0 01:29	0.65	0.67	1.00
110a-610	CONDUIT	410.64	0 01:30	0.90	0.80	1.00
200-202	CONDUIT	60.78	0 01:23	0.17	0.17	1.00
202-204	CONDUIT	209.14	0 01:30	0.57	0.61	1.00
202a-204	CONDUIT	234.88	0 01:30	0.64	0.68	1.00
204-206	CONDUIT	250.98	0 01:30	0.55	0.55	1.00
204a-206	CONDUIT	288.23	0 01:30	0.63	0.65	1.00
206-208	CONDUIT	361.97	0 01:30	0.79	0.79	1.00
208-208a	CONDUIT	386.43	0 01:30	0.85	1.02	1.00
208a-210	CONDUIT	404.11	0 01:30	0.89	1.09	1.00
210-212	CONDUIT	479.86	0 01:30	0.87	0.99	1.00
212-214	CONDUIT	504.58	0 01:30	0.77	1.06	1.00
214-216	CONDUIT	577.32	0 01:30	0.88	0.90	1.00
216-H2	CONDUIT	1521.75	0 01:30	0.81	0.55	1.00
304-306	CONDUIT	38.09	0 01:31	0.13	0.11	1.00
306a-308	CONDUIT	78.12	0 01:31	0.27	0.24	1.00
324-330	CONDUIT	729.67	0 01:30	0.63	0.29	1.00
402a-404	CONDUIT	61.28	0 01:43	0.21	0.22	1.00
404a-608	CONDUIT	128.74	0 01:31	0.44	0.39	1.00
608a-110	CONDUIT	135.82	0 01:31	0.46	0.47	1.00
610a-312	CONDUIT	692.07	0 01:31	0.90	0.82	1.00
908a-610	CONDUIT	148.29	0 01:31	0.40	0.44	1.00
C100-208	CONDUIT	16.52	0 09:00	0.06	0.07	1.00
C102-208a	CONDUIT	17.99	0 01:30	0.06	0.07	1.00
C104-210	CONDUIT	22.45	0 01:29	0.08	0.09	1.00
C106-212	CONDUIT	25.56	0 01:29	0.09	0.10	1.00
C108-214	CONDUIT	22.68	0 01:30	0.08	0.09	1.00
C110-216	CONDUIT	947.68	0 01:32	3.24	3.81	1.00
CB10-11ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.07
CB12-13ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB27-28ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB29-30ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB53-ROAD	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB57-ROAD	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB59-ROAD	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CST-01	CHANNEL	0.34	0 01:30	0.21	0.00	0.02
CST-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.07
CST-03	CHANNEL	0.07	0 01:30	0.00	0.00	0.01
CST-04	CHANNEL	0.32	0 01:30	0.43	0.00	0.02
CST-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.01
CST-06	CHANNEL	2.36	0 01:30	0.39	0.00	0.04
RY-00	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-00a	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-01	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-02	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-03	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-04	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-05	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-06	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-07	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-08	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-09	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-10	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
S01-00	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
S01-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
S01-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S01-03						

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

S01-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		STB-06	CHANNEL	0.00	0 00:00	0.00	0.00	0.05
S01-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.01		STB-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.05
S01-06	CONDUIT	0.00	0 00:00	0.00	0.00	0.01		STB-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
S01-07	CONDUIT	0.00	0 00:00	0.00	0.00	0.03		STB-09	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
S03-01a	CHANNEL	0.00	0 00:00	0.00	0.00	0.01		STB-10	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S03-01b	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		STB-11	CHANNEL	0.00	0 00:00	0.00	0.00	0.07
S03-01c	CHANNEL	0.00	0 00:00	0.00	0.00	0.08		STB-12	CHANNEL	0.00	0 01:30	0.00	0.00	0.07
S03-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.08		STB-13	CHANNEL	0.00	0 01:30	0.00	0.00	0.01
S03-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB01-02	ORIFICE	37.48	0 01:30			1.00
S03-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB03-04	ORIFICE	38.81	0 01:30			1.00
S03-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB05-06	ORIFICE	52.67	0 01:30			1.00
S03-06	CHANNEL	0.00	0 00:00	0.00	0.00	0.02		OCB07-08	ORIFICE	37.39	0 01:31			1.00
S03-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.02		OCB09-10	ORIFICE	55.08	0 01:31			1.00
S03-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB11-12	ORIFICE	21.36	0 01:31			1.00
S03-09	CONDUIT	0.00	0 00:00	0.00	0.00	0.07		OCB13-14	ORIFICE	0.00	0 00:00			0.00
S03-10	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB15-16	ORIFICE	27.98	0 01:33			1.00
S03-OUT	CONDUIT	0.00	0 00:00	0.00	0.00	0.00		OCB17-18	ORIFICE	25.35	0 01:40			1.00
S04-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.19		OCB19	ORIFICE	14.97	0 01:30			1.00
S04-02	CHANNEL	0.85	0 01:39	0.01	0.00	0.20		OCB20-21	ORIFICE	57.03	0 01:31			1.00
S04-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.02		OCB22	ORIFICE	27.28	0 01:31			1.00
S04-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB23	ORIFICE	26.89	0 01:31			1.00
S04-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB24-25	ORIFICE	36.79	0 01:27			1.00
S04-06	CHANNEL	0.00	0 00:00	0.00	0.00	0.09		OCB26-27	ORIFICE	72.07	0 01:30			1.00
S04-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.09		OCB28	ORIFICE	17.50	0 01:30			1.00
S04-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.09		OCB28-Stor	ORIFICE	92.73	0 01:30			1.00
S04-09	CHANNEL	0.00	0 00:00	0.00	0.00	0.09		OCB29	ORIFICE	20.56	0 01:30			1.00
S05-01	CONDUIT	11.15	0 01:33	0.03	0.00	0.27		OCB30-31	ORIFICE	39.57	0 01:42			1.00
S05-02	CHANNEL	23.99	0 01:30	0.32	0.01	0.31		OCB31-Stor	ORIFICE	151.28	0 01:30			1.00
S05-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.08		OCB32-33	ORIFICE	0.00	0 00:00			0.00
S05-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB34-35	ORIFICE	0.00	0 00:00			0.00
S05-05	CHANNEL	22.53	0 01:30	0.49	0.01	0.13		OCB36	ORIFICE	15.93	0 01:30			1.00
S05-06	CHANNEL	22.36	0 01:31	0.70	0.00	0.13		OCB37	ORIFICE	24.08	0 01:30			1.00
S05-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.08		OCB39	ORIFICE	16.11	0 01:30			1.00
S06-01d	CONDUIT	0.00	0 00:00	0.00	0.00	0.14		OCB40-41	ORIFICE	72.64	0 01:30			1.00
S06-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.14		OCB42-43	ORIFICE	49.84	0 01:30			1.00
S06-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.03		OCB44-45	ORIFICE	39.02	0 01:30			1.00
S06-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.03		OCB46-47	ORIFICE	14.13	0 01:30			1.00
S06-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB48-49	ORIFICE	16.15	0 01:31			1.00
S06-06	CONDUIT	17.93	0 01:30	0.09	0.01	0.07		OCB50-51	ORIFICE	37.26	0 01:30			1.00
S06-07	CHANNEL	0.00	0 00:00	0.00	0.00	0.06		OCB52-53	ORIFICE	73.85	0 01:30			1.00
S06-08	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB54	ORIFICE	9.22	0 01:30			1.00
S06-09	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB55-56	ORIFICE	53.30	0 01:30			1.00
S06-10	CHANNEL	0.00	0 00:00	0.00	0.00	0.00		OCB57-58	ORIFICE	50.08	0 01:30			1.00
S06-11	CHANNEL	0.00	0 00:00	0.00	0.00	0.06		OCB59	ORIFICE	6.66	0 01:30			1.00
S09-00	CHANNEL	1.19	0 01:31	0.17	0.00	0.09		OCB60	ORIFICE	23.83	0 01:30			1.00
S09-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB61	ORIFICE	11.02	0 01:30			1.00
S09-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB62	ORIFICE	22.44	0 01:30			1.00
S09-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.07		OCB63	ORIFICE	23.52	0 01:30			1.00
S09-04	CHANNEL	0.00	0 00:00	0.00	0.00	0.04		OCB64	ORIFICE	19.32	0 01:30			1.00
S09-05	CONDUIT	0.00	0 00:00	0.00	0.00	0.04		OCB65	ORIFICE	11.78	0 01:30			1.00
S09-06	CONDUIT	0.00	0 00:00	0.00	0.00	0.00		OCB66	ORIFICE	11.51	0 01:30			1.00
S10-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.09		OCB67-68	ORIFICE	74.32	0 01:30			1.00
S10-02	CHANNEL	36.74	0 01:30	0.30	0.01	0.21		OCB-B04	ORIFICE	153.25	0 01:30			1.00
S10-03	CHANNEL	71.73	0 01:30	0.44	0.03	0.39		OCB-C100	ORIFICE	15.20	0 01:30			1.00
S10-04	CONDUIT	0.00	0 00:00	0.00	0.00	0.30		OCB-C102	ORIFICE	17.57	0 01:30			1.00
S10-05	CONDUIT	0.00	0 00:00											

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class										Avg. Froude Number	Avg. Flow Change
		Dry Up Dry	Up Sub Sup	Down Crit	Up Crit	Down Crit	Class	Avg.					
{STM}.102-104	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0008	
{STM}.104-106	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0010	
{STM}.106-106a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0020	
{STM}.108-108a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0020	
{STM}.110-110a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0016	
{STM}.306-306a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0011	
{STM}.308-310	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0011	
{STM}.310-312	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0015	
{STM}.312-314	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0012	
{STM}.314-316	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0014	
{STM}.316-324	1.48	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0029	
{STM}.326-328	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0014	
{STM}.328-HW-1	2.49	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0020	
{STM}.330-336	5.92	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0013	
{STM}.402-402a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0011	
{STM}.404-404a	1.27	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0023	
{STM}.406-408	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0005	
{STM}.408-106	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0017	
{STM}.608-608a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0025	
{STM}.610-610a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0018	
{STM}.612-316	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0021	
{STM}.614-312	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0022	
{STM}.616-614	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0008	
{STM}.618-610	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0010	
{STM}.902-904	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0009	
{STM}.904-906	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0014	
{STM}.906-908	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0011	
{STM}.908-908a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0013	
106a-108	1.13	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0032	
108a-110	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0030	
110a-610	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0017	
200-202	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0004	
202-204	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0004	
202a-204	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0007	
204-206	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0005	
204a-206	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0010	
206-208	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0019	
208-208a	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.00 0.00	0.0028	
208a-210	1.00	0.00 0.00	0.00 1.00	0.00 0.00	0.00 0.00	0.0							

Burnett Lands – 3370 Greenbank Road
5-year Storm, 100-year Fixed Outlet Elevations
Model Output

S09-01	1.14	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-02	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-03	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-04	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-05	1.08	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-06	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S10-01	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S10-02	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.37	0.0000	
S10-03	1.00	0.00	0.00	0.07	0.00	0.00	0.93	0.68	0.0000	
S10-04	1.96	0.92	0.08	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S10-05	1.76	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S11-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S11-02	1.21	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S11-03	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST10-00	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST6-01a	1.66	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST6-01b	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST6-01c	1.21	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-00	1.27	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-01	1.00	0.99	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.0000
STB-02	1.00	0.00	0.99	0.00	0.00	0.01	0.00	0.01	0.00	0.0000
STB-03	1.00	0.00	0.00	0.00	0.00	0.00	1.00	0.46	0.0000	
STB-04	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-05	1.12	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-06	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-07	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-08	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-09	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-10	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-11	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-12	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.0000
STB-13	1.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.0000

{STM}.906-908	12.00	12.00	12.00	0.01	0.01
{STM}.908-908a	12.00	12.00	12.00	0.01	0.01
106a-108	12.00	12.00	12.00	0.01	0.23
108a-110	12.00	12.00	12.00	0.01	0.12
110a-610	12.00	12.00	12.00	0.01	0.12
200-202	0.15	0.15	0.15	0.01	0.01
202-204	0.17	0.17	0.17	0.01	0.01
202a-204	0.21	0.21	0.21	0.01	0.01
204-206	0.24	0.24	0.24	0.01	0.01
204a-206	0.35	0.35	0.35	0.01	0.01
206-208	12.00	12.00	12.00	0.01	0.04
208-208a	12.00	12.00	12.00	0.04	0.26
208a-210	12.00	12.00	12.00	0.10	0.22
210-212	12.00	12.00	12.00	0.01	0.15
212-214	12.00	12.00	12.00	0.09	0.20
214-216	12.00	12.00	12.00	0.01	0.13
216-H2	12.00	12.00	12.00	0.01	0.01
304-306	0.20	0.20	0.20	0.01	0.01
306a-308	12.00	12.00	12.00	0.01	0.01
324-330	12.00	12.00	12.00	0.01	0.14
402a-404	0.33	0.33	0.33	0.01	0.01
404a-608	12.00	12.00	12.00	0.01	0.01
608a-110	12.00	12.00	12.00	0.01	0.01
610a-312	12.00	12.00	12.00	0.01	0.17
908a-610	12.00	12.00	12.00	0.01	0.07
C100-208	12.00	12.00	12.00	0.01	0.09
C102-208a	12.00	12.00	12.00	0.02	
C104-210	12.00	12.00	12.00	0.01	0.01
C106-212	12.00	12.00	12.00	0.01	0.01
C108-214	12.00	12.00	12.00	0.01	0.01
C110-216	12.00	12.00	12.00	0.70	0.87

 Conduit Surcharge Summary

Analysis begun on: Wed Nov 23 10:02:12 2016
 Analysis ended on: Wed Nov 23 10:02:15 2016
 Total elapsed time: 00:00:03

Conduit	Hours Full		Above Normal	Capacity Limited	
	Both Ends	Upstream	Dnstream	Flow	
{STM}.102-104	0.32	0.32	0.32	0.01	0.01
{STM}.104-106	11.86	11.86	11.88	0.01	0.01
{STM}.106-106a	12.00	12.00	12.00	0.01	0.01
{STM}.108-108a	12.00	12.00	12.00	0.01	0.01
{STM}.110-110a	12.00	12.00	12.00	0.01	0.01
{STM}.306-306a	12.00	12.00	12.00	0.01	0.01
{STM}.308-310	12.00	12.00	12.00	0.01	0.01
{STM}.310-312	12.00	12.00	12.00	0.01	0.01
{STM}.312-314	12.00	12.00	12.00	0.01	0.11
{STM}.314-316	12.00	12.00	12.00	0.01	0.04
{STM}.316-324	12.00	12.00	12.00	0.01	1.21
{STM}.326-328	12.00	12.00	12.00	0.01	0.01
{STM}.328-HW-1	12.00	12.00	12.00	0.01	0.20
{STM}.330-336	12.00	12.00	12.00	0.01	0.14
{STM}.402-402a	0.27	0.27	0.27	0.01	0.01
{STM}.404-404a	10.02	10.02	10.85	0.01	0.01
{STM}.406-408	0.25	0.25	0.25	0.01	0.01
{STM}.408-106	11.99	11.99	11.99	0.01	0.01
{STM}.608-608a	12.00	12.00	12.00	0.01	0.01
{STM}.610-610a	12.00	12.00	12.00	0.01	0.16
{STM}.612-316	12.00	12.00	12.00	0.01	0.01
{STM}.614-312	12.00	12.00	12.00	0.01	0.12
{STM}.616-614	12.00	12.00	12.00	0.01	0.01
{STM}.618-610	12.00	12.00	12.00	0.01	0.01
{STM}.902-904	0.64	0.64			

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

M:\2011\111117\CAD\Design\111117 - GP.DWG
M:\2011\111117\CAD\Design\111117 - GP.DWG

NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
Rainfall/Runoff YES
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Starting Date JAN-21-2016 00:00:00
Ending Date JAN-21-2016 12:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 01:00:00
Routing Time Step 5.00 sec

WARNING 03: negative offset ignored for Link CB10-11ROADLINK
WARNING 04: minimum elevation drop used for Conduit CB12-13ROADLINK
WARNING 04: minimum elevation drop used for Conduit CB27-28ROADLINK
WARNING 04: minimum elevation drop used for Conduit CB29-30ROADLINK
WARNING 02: maximum depth increased for Node CB07-08
WARNING 02: maximum depth increased for Node CB09-10
WARNING 02: maximum depth increased for Node CB13-14
WARNING 02: maximum depth increased for Node CB13-14ROAD
WARNING 02: maximum depth increased for Node CB32-33
WARNING 02: maximum depth increased for Node CB32-33 ROAD
WARNING 02: maximum depth increased for Node CB34-35
WARNING 02: maximum depth increased for Node CB34-35 ROAD
WARNING 02: maximum depth increased for Node CB57-58
WARNING 02: maximum depth increased for Node CB60
WARNING 02: maximum depth increased for Node CB61

Element Count

Number of rain gages 1

Number of subcatchments ... 57
Number of nodes 162
Number of links 220
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
RAIN	C100-4	INTENSITY	10 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
A-01	0.23	43.00	64.00	0.7500	RAIN	CB01-02
A-02	0.25	36.00	64.00	0.7500	RAIN	CB03-04
A-03	0.13	36.00	64.00	0.7500	RAIN	CB13-14ROAD
A-04	0.30	34.00	64.00	0.7500	RAIN	CB05-06
A-05	0.28	37.00	64.00	0.7500	RAIN	CB11-12ROAD
A-06	0.27	37.00	64.00	0.7500	RAIN	CB07-08
A-07	0.29	39.00	64.00	0.7500	RAIN	CB15-16
A-08	0.25	41.00	64.00	0.7500	RAIN	CB17-18
A-09	0.08	23.00	64.00	0.7500	RAIN	CB19
A-10	0.19	37.00	64.00	0.7500	RAIN	CB20-21
A-11	0.23	25.00	64.00	0.7500	RAIN	CB20-21
A-12	0.04	24.00	64.00	0.7500	RAIN	CB59
A-13	0.20	24.00	64.00	0.7500	RAIN	CB23
A-14	0.20	23.00	64.00	0.7500	RAIN	CB22
A-15	0.11	30.00	64.00	0.7500	RAIN	CB64
A-16	0.13	32.00	64.00	0.7500	RAIN	CB63
A-17	0.25	49.00	64.00	0.7500	RAIN	CB44-45
A-18	0.27	32.00	64.00	0.7500	RAIN	CB42-43
A-19	0.12	31.00	64.00	0.7500	RAIN	CB62
A-20	0.13	29.00	64.00	0.7500	RAIN	CB60
A-21	0.22	27.00	64.00	0.7500	RAIN	CB24-25
A-22	0.06	24.00	64.00	0.7500	RAIN	CB61
A-23	0.46	46.00	64.00	0.7500	RAIN	CB40-41
A-24	0.21	28.00	64.00	0.7500	RAIN	CB34-35 ROAD
A-25	0.21	29.00	64.00	0.7500	RAIN	CB32-33 ROAD
A-26	0.45	45.00	64.00	0.7500	RAIN	CB26-27
A-27	0.19	32.00	64.00	0.7500	RAIN	CB30-31
A-28	0.23	18.00	64.00	0.7500	RAIN	CB29
A-29	0.23	46.00	64.00	0.7500	RAIN	CB09-10
A-30	0.50	71.00	64.00	0.7500	RAIN	A30-STOR
A-31	0.06	20.00	64.00	0.7500	RAIN	CB66
A-32	0.45	56.00	64.00	0.7500	RAIN	CB67-68-69
A-33	0.82	117.00	64.00	0.7500	RAIN	A33-STOR
A-34	0.77	55.00	64.00	0.7500	RAIN	4-GreenbankOut
A-35	0.98	70.00	64.00	0.7500	RAIN	4-GreenbankOut
A-36	0.45	60.00	64.00	0.7500	RAIN	4-GreenbankOut
A-37	0.37	53.00	64.00	0.7500	RAIN	4-GreenbankOut
B-01	0.09	28.00	64.00	0.7500	RAIN	CB36
B-02	0.15	23.00	64.00	0.7500	RAIN	CB37
B-03	0.10	32.00	64.00	0.7500	RAIN	CB39
B-04	0.83	110.00	64.00	0.7500	RAIN	B04_Stor
B-05	0.06	27.00	64.00	0.7500	RAIN	CB-65
B-06	0.08	19.00	64.00	0.7500	RAIN	CB46-47
B-07	0.13	32.00	64.00	0.7500	RAIN	CB48-49
B-08	0.23	32.00	64.00	0.7500	RAIN	CB50-51
B-09	0.25	34.00	64.00	0.7500	RAIN	CB52-53
B-10	1.12	149.00	14.00	0.1000	RAIN	CB52-53
B-11	0.05	13.00	64.00	0.7500	RAIN	CB54

Burnett Lands – 3370 Greenbank Road 100-year Storm, 5-year Fixed Outlet Elevations Model Output

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

CB30-31	JUNCTION	92.11	1.50	0.0
CB32-33	JUNCTION	92.42	1.84	0.0
CB32-33 ROAD	JUNCTION	93.62	0.64	0.0
CB34-35	JUNCTION	92.80	1.85	0.0
CB34-35 ROAD	JUNCTION	94.00	0.65	0.0
CB36	JUNCTION	93.06	1.50	0.0
CB37	JUNCTION	92.83	1.50	0.0
CB39	JUNCTION	92.58	1.50	0.0
CB40-41	JUNCTION	92.15	1.50	0.0
CB42-43	JUNCTION	92.27	1.50	0.0
CB44-45	JUNCTION	92.33	1.59	0.0
CB46-47	JUNCTION	92.47	1.50	0.0
CB48-49	JUNCTION	92.54	1.48	0.0
CB50-51	JUNCTION	92.39	1.50	0.0
CB52-53	JUNCTION	92.32	1.50	0.0
CB54	JUNCTION	92.37	1.50	0.0
CB55-56	JUNCTION	92.18	1.50	0.0
CB57-58	JUNCTION	92.20	1.40	0.0
CB59	JUNCTION	92.50	1.50	0.0
CB60	JUNCTION	92.30	1.80	0.0
CB61	JUNCTION	92.30	1.55	0.0
CB62	JUNCTION	92.29	1.50	0.0
CB63	JUNCTION	92.35	1.50	0.0
CB64	JUNCTION	92.46	1.50	0.0
CB-65	JUNCTION	92.50	1.50	0.0
CB66	JUNCTION	92.70	1.50	0.0
CB67-68-69	JUNCTION	92.40	1.56	0.0
HP-01	JUNCTION	93.91	0.30	0.0
HP-03	JUNCTION	93.80	0.30	0.0
RYP-01	JUNCTION	94.00	0.30	0.0
RYP-02	JUNCTION	93.95	0.30	0.0
RYP-03	JUNCTION	93.84	0.30	0.0
RYP-04	JUNCTION	93.78	0.30	0.0
1-HW-01 (STM)	OUTFALL	89.20	1.22	0.0
2-OverlandOutlet	OUTFALL	93.00	0.30	0.0
3-HW-02 (STM)	OUTFALL	89.90	1.22	0.0
4-GreenbankOut	OUTFALL	0.00	0.00	0.0
A30-STOR	STORAGE	92.30	1.70	0.0
A33-STOR	STORAGE	92.30	1.70	0.0
B04_Stor	STORAGE	92.75	1.70	0.0
Caivan-Stor	STORAGE	92.10	1.75	0.0

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
{STM}.102-104	102 (STM)	104 (STM)	CONDUIT	22.5	0.5368	0.0130
{STM}.104-106	104 (STM)	106 (STM)	CONDUIT	28.2	0.3015	0.0130
{STM}.106-106a	106 (STM)	106a	CONDUIT	32.1	0.2178	0.0130
{STM}.108-108a	108 (STM)	108a	CONDUIT	37.5	0.1598	0.0130
{STM}.110-110a	110 (STM)	110a	CONDUIT	37.5	0.1972	0.0130
{STM}.306-306a	306 (STM)	306a (STM)	CONDUIT	50.1	0.2593	0.0130
{STM}.308-310	308 (STM)	310 (STM)	CONDUIT	73.3	0.2593	0.0130
{STM}.310-312	310 (STM)	312 (STM)	CONDUIT	41.4	0.1499	0.0130
{STM}.312-314	312 (STM)	314 (STM)	CONDUIT	56.5	0.1009	0.0130
{STM}.314-316	314 (STM)	316 (STM)	CONDUIT	83.4	0.1031	0.0130
{STM}.316-324	316 (STM)	324 (STM)	CONDUIT	15.3	0.0979	0.0130
{STM}.326-328	326 (STM)	328 (STM)	CONDUIT	60.0	0.1000	0.0130
{STM}.328-HW-1	328 (STM)	1-HW-01 (STM)	CONDUIT	9.5	0.1050	0.0130
{STM}.330-336	330 (STM)	326 (STM)	CONDUIT	5.0	0.5000	0.0130
{STM}.402-402a	402 (STM)	402a (STM)	CONDUIT	29.9	0.2340	0.0150
{STM}.404-404a	404 (STM)	404a	CONDUIT	14.1	0.2626	0.0130
{STM}.406-408	406 (STM)	408 (STM)	CONDUIT	105.5	0.2000	0.0130
{STM}.408-106	408 (STM)	106 (STM)	CONDUIT	20.5	0.2003	0.0130
{STM}.608-608a	608 (STM)	608a	CONDUIT	28.5	0.1968	0.0130
{STM}.610-610a	610 (STM)	610a	CONDUIT	33.3	0.1202	0.0130
{STM}.612-316	612 (STM)	316 (STM)	CONDUIT	78.3	0.2005	0.0130

{STM}.614-312	614 (STM)	612 (STM)	CONDUIT	25.1	0.1991	0.0130
{STM}.616-614	616 (STM)	614 (STM)	CONDUIT	80.3	0.2005	0.0130
{STM}.618-610	618 (STM)	610 (STM)	CONDUIT	41.1	0.2989	0.0130
{STM}.902-904	902 (STM)	904 (STM)	CONDUIT	37.7	0.3128	0.0130
{STM}.904-906	904 (STM)	906 (STM)	CONDUIT	40.1	0.1994	0.0130
{STM}.906-908	906 (STM)	908 (STM)	CONDUIT	70.5	0.2001	0.0130
{STM}.908-908a	908 (STM)	908a	CONDUIT	50.7	0.1480	0.0130
106a-108	106a	108 (STM)	CONDUIT	15.1	0.1985	0.0130
108a-110	108a	110 (STM)	CONDUIT	17.8	0.1684	0.0130
110a-610	110a	610 (STM)	CONDUIT	39.0	0.1950	0.0130
200-202	200 (STM)	202 (STM)	CONDUIT	37.4	0.2138	0.0150
202-204	202 (STM)	202a	CONDUIT	53.9	0.2042	0.0150
202a-204	202a	204 (STM)	CONDUIT	28.7	0.2091	0.0150
204-206	204 (STM)	204a	CONDUIT	48.8	0.2048	0.0150
204a-206	204a	206 (STM)	CONDUIT	71.2	0.1967	0.0150
206-208	206 (STM)	208 (STM)	CONDUIT	38.5	0.2078	0.0150
208-208a	208 (STM)	208a	CONDUIT	28.4	0.1410	0.0150
208a-210	208a	210 (STM)	CONDUIT	29.4	0.1360	0.0150
210-212	210 (STM)	212 (STM)	CONDUIT	35.5	0.1408	0.0150
212-214	212 (STM)	214 (STM)	CONDUIT	35.5	0.0845	0.0150
214-216	214 (STM)	216 (STM)	CONDUIT	36.1	0.1524	0.0150
216-H2	216 (STM)	3-HW-02 (STM)	CONDUIT	79.8	0.1378	0.0130
304-306	304 (STM)	306 (STM)	CONDUIT	65.5	0.3053	0.0130
306a-308	306a (STM)	308 (STM)	CONDUIT	39.3	0.2546	0.0130
324-330	324 (STM)	330 (STM)	CONDUIT	5.0	0.5000	0.0150
402a-404	402a (STM)	404 (STM)	CONDUIT	48.1	0.2496	0.0150
404a-608	404a	608 (STM)	CONDUIT	20.1	0.2632	0.0130
608a-110	608a	110 (STM)	CONDUIT	51.1	0.2035	0.0130
610a-312	610a	312 (STM)	CONDUIT	46.1	0.1301	0.0130
908a-610	908a	610 (STM)	CONDUIT	16.6	0.1502	0.0130
C100-208	C100 (STM)	208 (STM)	CONDUIT	25.0	0.2000	0.0150
C102-208a	C102 (STM)	208a	CONDUIT	25.0	0.2000	0.0150
C104-210	C104 (STM)	210 (STM)	CONDUIT	25.0	0.2000	0.0150
C106-212	C106 (STM)	212 (STM)	CONDUIT	25.0	0.2000	0.0150
C108-214	C108 (STM)	214 (STM)	CONDUIT	25.0	0.2000	0.0150
C110-216	C110 (STM)	216 (STM)	CONDUIT	25.0	0.2000	0.0150
CB10-11ROADLINK	CB11-12ROAD	CB11-12	CONDUIT	5.0	0.7400	0.0160
CB12-13ROADLINK	CB13-14ROAD	CB13-14ROAD	CONDUIT	10.0	0.0030	0.0160
CB27-28ROADLINK	CB32-33	CB32-33 ROAD	CONDUIT	5.0	0.0061	0.0160
CB29-30ROADLINK	CB34-35	CB34-35	CONDUIT	5.0	0.0061	0.0160
CB53-ROAD	CB60	01+051	CONDUIT	15.0</td		

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

{STM}.102-104	CIRCULAR	0.61	0.29	0.15	0.61	1	470.15		CST-06	9m-ROW	0.30	3.10	0.14	21.20	1	10011.21
{STM}.104-106	CIRCULAR	0.61	0.29	0.15	0.61	1	352.37		RY-00	TRIANGULAR	0.30	0.27	0.14	1.80	1	291.33
{STM}.106-106a	CIRCULAR	0.61	0.29	0.15	0.61	1	299.48		RY-00a	TRIANGULAR	0.30	0.27	0.14	1.80	1	118.98
{STM}.108-108a	CIRCULAR	0.69	0.37	0.17	0.69	1	350.83		RY-01	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.93
{STM}.110-110a	CIRCULAR	0.76	0.46	0.19	0.76	1	515.75		RY-02	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.78
{STM}.306-306a	CIRCULAR	0.61	0.29	0.15	0.61	1	326.79		RY-03	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.88
{STM}.308-310	CIRCULAR	0.61	0.29	0.15	0.61	1	326.80		RY-04	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.78
{STM}.310-312	CIRCULAR	0.76	0.46	0.19	0.76	1	449.67		RY-05	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.74
{STM}.312-314	CIRCULAR	1.22	1.17	0.30	1.22	1	1291.42		RY-06	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.78
{STM}.314-316	CIRCULAR	1.22	1.17	0.30	1.22	1	1305.31		RY-07	TRIANGULAR	0.30	0.27	0.14	1.80	1	257.32
{STM}.316-324	CIRCULAR	1.22	1.17	0.30	1.22	1	1272.17		RY-08	TRIANGULAR	0.30	0.27	0.14	1.80	1	281.70
{STM}.326-328	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1	2378.14		RY-09	TRIANGULAR	0.30	0.27	0.14	1.80	1	291.08
{STM}.328-HW-1	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1	2436.50		RY-10	TRIANGULAR	0.30	0.27	0.14	1.80	1	281.70
{STM}.330-336	CIRCULAR	1.22	1.17	0.30	1.22	1	2874.95		S01-00	9m-ROW	0.30	3.10	0.14	21.20	1	3963.85
{STM}.402-402a	CIRCULAR	0.61	0.29	0.15	0.61	1	269.05		S01-01	9m-ROW	0.30	3.10	0.14	21.20	1	4762.26
{STM}.404-404a	CIRCULAR	0.61	0.29	0.15	0.61	1	328.85		S01-02	9m-ROW	0.30	3.10	0.14	21.20	1	4584.08
{STM}.406-408	CIRCULAR	0.61	0.29	0.15	0.61	1	286.99		S01-03	9m-ROW	0.30	3.10	0.14	21.20	1	4560.54
{STM}.408-106	CIRCULAR	0.61	0.29	0.15	0.61	1	287.22		S01-04	9m-ROW	0.30	3.10	0.14	21.20	1	4613.09
{STM}.608-608a	CIRCULAR	0.61	0.29	0.15	0.61	1	284.66		S01-05	9m-ROW	0.30	3.10	0.14	21.20	1	6349.97
{STM}.610-610a	CIRCULAR	0.99	0.77	0.25	0.99	1	811.34		S01-06	RECT_OPEN	0.30	3.00	0.28	10.00	1	12260.05
{STM}.612-316	CIRCULAR	0.61	0.29	0.15	0.61	1	287.33		S01-07	RECT_OPEN	0.30	3.00	0.28	10.00	1	11432.19
{STM}.614-312	CIRCULAR	0.61	0.29	0.15	0.61	1	286.33		S03-01a	9m-ROW	0.30	3.10	0.14	21.20	1	5455.67
{STM}.616-614	CIRCULAR	0.46	0.16	0.11	0.46	1	133.04		S03-01b	9m-ROW	0.30	3.10	0.14	21.20	1	2405.43
{STM}.618-610	CIRCULAR	0.61	0.29	0.15	0.61	1	350.87		S03-01c	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
{STM}.902-904	CIRCULAR	0.61	0.29	0.15	0.61	1	358.92		S03-02	9m-ROW	0.30	3.10	0.14	21.20	1	5552.13
{STM}.904-906	CIRCULAR	0.61	0.29	0.15	0.61	1	286.54		S03-03	9m-ROW	0.30	3.10	0.14	21.20	1	2403.77
{STM}.906-908	CIRCULAR	0.61	0.29	0.15	0.61	1	287.07		S03-04	9m-ROW	0.30	3.10	0.14	21.20	1	3684.20
{STM}.908-908a	CIRCULAR	0.69	0.37	0.17	0.69	1	337.70		S03-05	9m-ROW	0.30	3.10	0.14	21.20	1	4041.05
106a-108	CIRCULAR	0.61	0.29	0.15	0.61	1	285.94		S03-06	9m-ROW	0.30	3.10	0.14	21.20	1	4995.65
108a-110	CIRCULAR	0.69	0.37	0.17	0.69	1	360.11		S03-07	9m-ROW	0.30	3.10	0.14	21.20	1	4149.10
110a-610	CIRCULAR	0.76	0.46	0.19	0.76	1	512.86		S03-08	9m-ROW	0.30	3.10	0.14	21.20	1	5038.93
200-202	CIRCULAR	0.69	0.37	0.17	0.69	1	351.75		S03-09	RECT_OPEN	0.30	1.50	0.27	5.00	1	2218.23
202-204	CIRCULAR	0.69	0.37	0.17	0.69	1	343.69		S03-10	9m-ROW	0.30	3.10	0.14	21.20	1	5731.78
202a-204	CIRCULAR	0.69	0.37	0.17	0.69	1	347.79		S03-OUT	RECT_OPEN	0.30	3.00	0.28	10.00	1	1952.23
204-206	CIRCULAR	0.76	0.46	0.19	0.76	1	455.54		S04-01	9m-ROW	0.30	3.10	0.14	21.20	1	4397.38
204a-206	CIRCULAR	0.76	0.46	0.19	0.76	1	446.46		S04-02	9m-ROW	0.30	3.10	0.14	21.20	1	5410.17
206-208	CIRCULAR	0.76	0.46	0.19	0.76	1	458.92		S04-03	9m-ROW	0.30	3.10	0.14	21.20	1	5399.02
208-208a	CIRCULAR	0.76	0.46	0.19	0.76	1	378.04		S04-04	9m-ROW	0.30	3.10	0.14	21.20	1	5935.05
208a-210	CIRCULAR	0.76	0.46	0.19	0.76	1	371.17		S04-05	9m-ROW	0.30	3.10	0.14	21.20	1	6924.09
210-212	CIRCULAR	0.84	0.55	0.21	0.84	1	486.79		S04-06	9m-ROW	0.30	3.10	0.14	21.20	1	5832.45
212-214	CIRCULAR	0.91	0.66	0.23	0.91	1	475.29		S04-07	9m-ROW	0.30	3.10	0.14	21.20	1	4406.79
214-216	CIRCULAR	0.91	0.66	0.23	0.91	1	638.35		S04-08	9m-ROW	0.30	3.10	0.14	21.20	1	4310.86
216-H2	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1	2791.51		S04-09	9m-ROW	0.30	3.10	0.14	21.20	1	6189.83
304-306	CIRCULAR	0.61	0.29	0.15	0.61	1	354.55		S05-01	RECT_OPEN	0.30	1.50	0.27	5.00	1	3260.12
306a-308	CIRCULAR	0.61	0.29	0.15	0.61	1	323.83		S05-02	5m-ROW	0.30	2.40	0.13	18.20	1	2792.80
324-330	CIRCULAR	1.22	1.17	0.30	1.22	1	2491.62		S05-03	5m-ROW	0.30	2.40	0.13	18.20	1	2706.63
402a-404	CIRCULAR	0.61	0.29	0.15	0.61	1	277.88		S05-04	5m-ROW	0.30					

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

S10-02	5m-ROW	0.30	2.40	0.13	18.20	1	2729.26
S10-03	5m-ROW	0.30	2.40	0.13	18.20	1	2733.56
S10-04	RECT_OPEN	0.30	3.00	0.28	10.00	1	6642.05
S10-05	RECT_OPEN	0.30	3.00	0.28	10.00	1	5422.66
S11-01	5m-ROW	0.30	2.40	0.13	18.20	1	3310.53
S11-02	5m-ROW	0.30	2.40	0.13	18.20	1	5989.76
S11-03	5m-ROW	0.30	2.40	0.13	18.20	1	3313.94
ST10-00	5m-ROW	0.30	2.40	0.13	18.20	1	2467.78
ST6-01a	11m-ROW	0.30	3.40	0.14	23.20	1	5368.01
ST6-01b	11m-ROW	0.30	3.40	0.14	23.20	1	4305.74
ST6-01c	11m-ROW	0.30	3.40	0.14	23.20	1	8291.98
STB-00	9m-ROW	0.30	3.10	0.14	21.20	1	5778.40
STB-01	9m-ROW	0.30	3.10	0.14	21.20	1	5810.07
STB-02	9m-ROW	0.30	3.10	0.14	21.20	1	4168.40
STB-03	9m-ROW	0.30	3.10	0.14	21.20	1	4188.12
STB-04	9m-ROW	0.30	3.10	0.14	21.20	1	4621.40
STB-05	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
STB-06	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
STB-07	9m-ROW	0.30	3.10	0.14	21.20	1	4643.63
STB-08	9m-ROW	0.30	3.10	0.14	21.20	1	4562.20
STB-09	9m-ROW	0.30	3.10	0.14	21.20	1	4745.50
STB-10	9m-ROW	0.30	3.10	0.14	21.20	1	5040.78
STB-11	9m-ROW	0.30	3.10	0.14	21.20	1	5040.78
STB-12	9m-ROW	0.30	3.10	0.14	21.20	1	4158.17
STB-13	9m-ROW	0.30	3.10	0.14	21.20	1	4512.57

Transect Summary

Transect 11m-ROW

Area:

0.0003	0.0014	0.0031	0.0056	0.0087
0.0126	0.0171	0.0224	0.0283	0.0350
0.0423	0.0503	0.0591	0.0685	0.0787
0.0895	0.1010	0.1133	0.1262	0.1398
0.1542	0.1692	0.1849	0.2014	0.2185
0.2374	0.2590	0.2824	0.3070	0.3324
0.3586	0.3856	0.4133	0.4417	0.4710
0.5009	0.5317	0.5632	0.5954	0.6284
0.6622	0.6967	0.7319	0.7680	0.8047
0.8423	0.8806	0.9196	0.9594	1.0000

Hrad:

0.0203	0.0406	0.0608	0.0811	0.1014
0.1217	0.1420	0.1623	0.1825	0.2028
0.2231	0.2434	0.2637	0.2840	0.3042
0.3245	0.3448	0.3651	0.3854	0.4056
0.4259	0.4462	0.4665	0.4868	0.5071
0.4758	0.4659	0.4779	0.4994	0.5251
0.5505	0.5757	0.6006	0.6253	0.6498
0.6741	0.6982	0.7221	0.7459	0.7696
0.7931	0.8165	0.8398	0.8630	0.8860
0.9090	0.9319	0.9547	0.9774	1.0000

Width:

0.0171	0.0342	0.0512	0.0683	0.0854
0.1025	0.1196	0.1366	0.1537	0.1708
0.1879	0.2049	0.2220	0.2391	0.2562
0.2733	0.2903	0.3074	0.3245	0.3416
0.3587	0.3757	0.3928	0.4099	0.4270
0.4954	0.5528	0.5880	0.6121	0.6305
0.6490	0.6675	0.6860	0.7044	0.7229
0.7414	0.7599	0.7783	0.7968	0.8153
0.8337	0.8522	0.8707	0.8892	0.9076
0.9261	0.9446	0.9631	0.9815	1.0000

Transect 5m-ROW

Area:

0.0005	0.0019	0.0043	0.0076	0.0119
0.0172	0.0234	0.0306	0.0387	0.0478
0.0578	0.0688	0.0807	0.0934	0.1062
0.1190	0.1318	0.1446	0.1574	0.1703
0.1832	0.1961	0.2090	0.2220	0.2349
0.2494	0.2668	0.2857	0.3058	0.3271
0.3496	0.3732	0.3981	0.4241	0.4512
0.4796	0.5091	0.5398	0.5717	0.6048
0.6390	0.6744	0.7110	0.7487	0.7877
0.8278	0.8691	0.9115	0.9552	1.0000

Hrad:

0.0225	0.0450	0.0676	0.0901	0.1126
0.1351	0.1577	0.1802	0.2027	0.2252
0.2477	0.2703	0.2928	0.3297	0.3737
0.4176	0.4612	0.5047	0.5480	0.5912
0.6342	0.6770	0.7197	0.7622	0.8045
0.6979	0.6515	0.6566	0.6637	0.6724
0.6826	0.6940	0.7064	0.7197	0.7339
0.7487	0.7642	0.7802	0.7967	0.8137
0.8310	0.8487	0.8668	0.8851	0.9037
0.9225	0.9416	0.9609	0.9804	1.0000

Width:

0.0210	0.0421	0.0631	0.0842	0.1052
0.1263	0.1473	0.1684	0.1894	0.2104
0.2315	0.2525	0.2736	0.2809	0.2813
0.2818	0.2822	0.2826	0.2831	0.2835
0.2840	0.2844	0.2848	0.2853	0.2857
0.3516	0.4042	0.4301	0.4560	0.4819
0.5078	0.5338	0.5597	0.5856	0.6115
0.6374	0.6633	0.6892	0.7151	0.7410
0.7669	0.7928	0.8187	0.8446	0.8705
0.8964	0.9223	0.9482	0.9741	1.0000

Transect 9m-ROW

Area:

0.0004	0.0015	0.0034	0.0061	0.0095
0.0137	0.0186	0.0243	0.0308	0.0380
0.0460	0.0548	0.0643	0.0746	0.0856
0.0974	0.1099	0.1232	0.1373	0.1522
0.1678	0.1841	0.2012	0.2189	0.2367
0.2557	0.2768	0.2992	0.3223	0.3463
0.3711	0.3968	0.4232	0.4505	0.4787
0.5076	0.5374	0.5680	0.5995	0.6317
0.6648	0.6988	0.7335	0.7691	0.8055
0.8428	0.8808	0.9197	0.9594	1.0000

Hrad:

0.0203	0.0406	0.0609	0.0812	0.1015
0.1218	0.1421	0.1624	0.1827	0.2031
0.2234	0.2437	0.2640	0.2843	0.3046
0.3249	0.3452	0.3655	0.3858	0.4061
0.4264	0.4467	0.4670	0.5004	0.5401
0.5171	0.5140	0.5356	0.5570	0.5784
0.5998	0.6211	0.6423	0.6636	0.6848
0.7059	0.7271	0.7482	0.7692	0.7903
0.8114	0.8324	0.8534	0.8744	0.8953
0.9163	0.9372	0.9582	0.9791	1.0000

Width:

0.0186	0.0371	0
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Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

Control Actions Taken

	Volume hectare-m	Depth mm
Total Precipitation	1.749	76.002
Evaporation Loss	0.000	0.000
Infiltration Loss	0.486	21.107
Surface Runoff	1.245	54.106
Final Surface Storage	0.012	0.543
Continuity Error (%)	0.323	
*****	Volume hectare-m	Volume 10^6 ltr
Flow Routing Continuity		
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	1.245	12.450
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.002	0.025
External Outflow	1.264	12.642
Internal Outflow	0.000	0.000
Storage Losses	0.000	0.000
Initial Stored Volume	0.070	0.696
Final Stored Volume	0.070	0.696
Continuity Error (%)	-1.268	

Highest Continuity Errors		
Node 06+194 (-22.16%)		
Node CB11-12ROAD (-16.70%)		
Node 02+196 (-12.32%)		
Node 02+512 (-3.77%)		
Node 316 (STM) (-2.66%)		

Time-Step Critical Elements		
Link 208-208a (2.63%)		
Link C110-216 (1.39%)		

Highest Flow Instability Indexes		
Link VORTECHS (114)		
Link 324-330 (109)		
Link {STM}.330-336 (96)		
Link {STM}.316-324 (90)		
Link {STM}.314-316 (88)		

Routing Time Step Summary		
Minimum Time Step : 0.50 sec		
Average Time Step : 4.76 sec		
Maximum Time Step : 5.00 sec		
Percent in Steady State : 0.00		
Average Iterations per Step : 6.68		

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
A-01	76.00	0.00	0.00	17.96	57.58	0.14	86.67	0.758
A-02	76.00	0.00	0.00	18.19	57.34	0.14	89.11	0.755
A-03	76.00	0.00	0.00	17.62	58.18	0.07	49.91	0.766
A-04	76.00	0.00	0.00	18.48	57.05	0.17	104.88	0.751
A-05	76.00	0.00	0.00	18.27	57.52	0.16	97.73	0.757
A-06	76.00	0.00	0.00	18.26	57.37	0.16	97.29	0.755
A-07	76.00	0.00	0.00	18.27	57.27	0.17	102.56	0.753
A-08	76.00	0.00	0.00	18.05	57.74	0.14	89.84	0.760
A-09	76.00	0.00	0.00	17.62	57.93	0.05	31.75	0.762
A-10	76.00	0.00	0.00	17.89	57.65	0.11	69.70	0.759
A-11	76.00	0.00	0.00	18.50	57.03	0.13	78.02	0.750
A-12	76.00	0.00	0.00	17.23	48.71	0.02	11.43	0.641
A-13	76.00	0.00	0.00	18.40	57.13	0.11	69.98	0.752
A-14	76.00	0.00	0.00	18.42	57.11	0.11	68.09	0.751
A-15	76.00	0.00	0.00	17.62	48.69	0.05	33.33	0.641
A-16	76.00	0.00	0.00	17.71	48.68	0.06	40.62	0.641
A-17	76.00	0.00	0.00	17.88	57.66	0.14	91.77	0.759
A-18	76.00	0.00	0.00	18.43	57.10	0.16	94.95	0.751
A-19	76.00	0.00	0.00	17.70	48.68	0.06	38.72	0.641
A-20	76.00	0.00	0.00	17.79	48.68	0.06	41.25	0.641
A-21	76.00	0.00	0.00	18.36	57.17	0.12	76.26	0.752
A-22	76.00	0.00	0.00	17.43	48.69	0.03	19.05	0.641
A-23	76.00	0.00	0.00	18.62	57.15	0.26	156.84	0.752
A-24	76.00	0.00	0.00	18.27	57.51	0.12	74.24	0.757
A-25	76.00	0.00	0.00	18.26	57.52	0.12	76.12	0.757
A-26	76.00	0.00	0.00	18.63	57.10	0.26	153.65	0.751
A-27	76.00	0.00	0.00	18.04	57.75	0.11	69.48	0.760
A-28	76.00	0.00	0.00	18.99	56.02	0.13	76.79	0.737
A-29	76.00	0.00	0.00	17.88	57.76	0.13	86.15	0.760
A-30	76.00	0.00	0.00	18.20	57.33	0.29	177.82	0.754
A-31	76.00	0.00	0.00	17.53	57.52	0.03	24.31	0.757
A-32	76.00	0.00	0.00	18.36	57.42	0.26	158.56	0.756
A-33	76.00	0.00	0.00	18.20	57.33	0.47	291.72	0.754
A-34	76.00	0.00	0.00	19.13	55.88	0.43	254.07	0.735
A-35	76.00	0.00	0.00	19.13	55.88	0.55	322.85	0.735
A-36	76.00	0.00	0.00	18.28	56.75	0.26	159.31	0.747
A-37	76.00	0.00	0.00	18.21	56.82	0.21	132.74	0.748
B-01	76.00	0.00	0.00	17.58	57.98	0.05	36.54	0.763
B-02	76.00	0.00	0.00	18.12	56.91	0.08	53.60	0.749
B-03	76.00	0.00	0.00	17.53	57.63	0.06	38.76	0.758
B-04	76.00	0.00	0.00	18.28	57.25	0.47	292.34	0.753
B-05	76.00	0.00	0.00	17.41	48.70	0.03	20.40	0.641
B-06	76.00	0.00	0.00	17.71	57.34	0.04	29.20	0.754
B-07	76.00	0.00	0.00	17.70	57.75	0.07	48.83	0.760
B-08	76.00	0.00	0.00	18.21	57.32	0.13	80.50	0.754
B-09	76.00	0.00	0.00	18.24	57.30	0.14	87.29	0.754
B-10	76.00	0.00	0.00	51.72	24.07	0.27	97.39	0.317
B-11	76.00	0.00	0.00	17.66	57.39	0.03	18.84	0.755
B-12	76.00	0.00	0.00	17.77	57.78	0.05	33.18	0.760
B-13	76.00	0.00	0.00	17.78	57.77	0.06	36.88	0.760
B-14	76.00	0.00	0.00	17.77	57.77	0.18	121.50	0.760
B-15	76.00	0.00	0.00	17.79	57.76	0.07	45.60	0.760
B-16	76.00	0.00	0.00	17.77	57.77	0.08	51.58	0.760
B-17	76.00	0.00	0.00	18.39	57.13	0.16	95.77	0.752
B-18	76.00	0.00	0.00	17.77	57.78	0.07	46.21	0.760
B-19	76.00	0.00	0.00	17.86	57.68	0.06	40.18	0.759
CaivanLands	76.00	0.00	0.00	21.68	53.76	4.36	2057.72	0.707

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

Node Depth Summary						
Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days	Time of Max hr:min
01+011	JUNCTION	0.00	0.00	93.48	0	00:00
01+051	JUNCTION	0.00	0.00	93.76	0	01:31
01+118	JUNCTION	0.00	0.04	93.76	0	01:31
01+143	JUNCTION	0.00	0.00	93.93	0	00:00
02+196	JUNCTION	0.00	0.04	93.71	0	01:30
02+288	JUNCTION	0.00	0.01	93.77	0	01:41
02+365	JUNCTION	0.00	0.00	93.84	0	00:00
02+432	JUNCTION	0.00	0.01	93.87	0	01:38
02+458	JUNCTION	0.00	0.07	93.84	0	01:37
02+512	JUNCTION	0.00	0.06	93.92	0	01:30
02+564	JUNCTION	0.00	0.00	94.40	0	00:00
03+026	JUNCTION	0.00	0.00	93.56	0	00:00
03+054	JUNCTION	0.00	0.00	93.61	0	00:00
03+109	JUNCTION	0.00	0.00	93.44	0	00:00
03+127	JUNCTION	0.00	0.06	93.43	0	01:39
03+200	JUNCTION	0.00	0.00	93.42	0	00:00
03+291	JUNCTION	0.00	0.00	93.34	0	00:00
03+384	JUNCTION	0.00	0.00	93.34	0	00:00
04+078	JUNCTION	0.00	0.06	93.71	0	01:31
04+089	JUNCTION	0.00	0.00	93.76	0	00:00
04+129	JUNCTION	0.00	0.00	93.64	0	00:00
04+207	JUNCTION	0.00	0.00	93.87	0	00:00
05+116	JUNCTION	0.00	0.00	93.93	0	00:00
05+200	JUNCTION	0.00	0.05	93.54	0	01:30
06+023	JUNCTION	0.00	0.00	93.64	0	00:00
06+043	JUNCTION	0.00	0.00	93.74	0	00:00
06+108	JUNCTION	0.00	0.00	93.60	0	00:00
06+194	JUNCTION	0.00	0.01	93.52	0	01:31
06+273	JUNCTION	0.00	0.05	93.45	0	01:37
06+309	JUNCTION	0.00	0.00	93.88	0	00:00
06+332	JUNCTION	0.00	0.00	94.51	0	00:00
06+355	JUNCTION	0.00	0.00	94.15	0	00:00
09+000	JUNCTION	0.00	0.00	93.36	0	00:00
09+097	JUNCTION	0.00	0.00	93.65	0	00:00
09+203	JUNCTION	0.00	0.03	93.92	0	01:31
10+011	JUNCTION	0.00	0.02	93.60	0	01:36
10+232	JUNCTION	0.00	0.00	94.36	0	00:00
102 (STM)	JUNCTION	0.04	0.71	91.54	0	01:34
104 (STM)	JUNCTION	0.07	0.87	91.54	0	01:34
106 (STM)	JUNCTION	0.13	0.96	91.53	0	01:34
106a	JUNCTION	0.20	1.02	91.52	0	01:34
108 (STM)	JUNCTION	0.29	1.09	91.49	0	01:34
108a	JUNCTION	0.35	1.12	91.46	0	01:34
11+000	JUNCTION	0.00	0.00	94.01	0	00:00
11+086	JUNCTION	0.00	0.00	94.26	0	00:00
110 (STM)	JUNCTION	0.45	1.18	91.42	0	01:35
110a	JUNCTION	0.52	1.18	91.34	0	01:35
200 (STM)	JUNCTION	0.04	0.48	91.81	0	01:40
202 (STM)	JUNCTION	0.07	0.56	91.81	0	01:40
202a	JUNCTION	0.08	0.61	91.75	0	01:40
204 (STM)	JUNCTION	0.09	0.69	91.70	0	01:40
204a	JUNCTION	0.10	0.74	91.65	0	01:40
206 (STM)	JUNCTION	0.22	0.80	91.57	0	01:39
208 (STM)	JUNCTION	0.28	0.79	91.48	0	01:38
208a	JUNCTION	0.31	0.74	91.39	0	01:38
210 (STM)	JUNCTION	0.42	0.77	91.30	0	01:38
212 (STM)	JUNCTION	0.54	0.80	91.20	0	01:37
214 (STM)	JUNCTION	0.64	0.82	91.12	0	01:36
216 (STM)	JUNCTION	0.92	0.99	91.00	0	01:36
304 (STM)	JUNCTION	0.03	0.29	91.15	0	01:37
306 (STM)	JUNCTION	0.06	0.49	91.14	0	01:37

306a (STM)	JUNCTION	0.15	0.61	91.13	0	01:36
308 (STM)	JUNCTION	0.26	0.71	91.12	0	01:36
310 (STM)	JUNCTION	0.59	1.01	91.08	0	01:35
312 (STM)	JUNCTION	1.11	1.51	91.06	0	01:35
314 (STM)	JUNCTION	1.17	1.50	90.98	0	01:35
316 (STM)	JUNCTION	1.28	1.52	90.88	0	01:35
324 (STM)	JUNCTION	1.29	1.43	90.78	0	01:35
326 (STM)	JUNCTION	1.34	1.41	90.71	0	01:32
328 (STM)	JUNCTION	1.72	1.75	90.66	0	01:33
330 (STM)	JUNCTION	1.31	1.43	90.75	0	01:33
402 (STM)	JUNCTION	0.05	0.59	91.50	0	01:33
402a (STM)	JUNCTION	0.07	0.66	91.50	0	01:33
404 (STM)	JUNCTION	0.08	0.82	91.49	0	01:34
404a	JUNCTION	0.09	0.85	91.48	0	01:34
406 (STM)	JUNCTION	0.50	1.08	91.54	0	01:34
408 (STM)	JUNCTION	0.37	1.18	91.54	0	01:34
608 (STM)	JUNCTION	0.15	0.92	91.47	0	01:34
608a	JUNCTION	0.20	0.96	91.45	0	01:33
610 (STM)	JUNCTION	0.80	1.36	91.24	0	01:35
610a	JUNCTION	0.83	1.33	91.17	0	01:35
612 (STM)	JUNCTION	0.54	0.98	91.10	0	01:22
614 (STM)	JUNCTION	0.45	1.02	91.24	0	01:22
616 (STM)	JUNCTION	0.13	1.20	91.73	0	01:22
618 (STM)	JUNCTION	0.31	0.88	91.24	0	01:35
902 (STM)	JUNCTION	0.04	0.58	91.28	0	01:35
904 (STM)	JUNCTION	0.10	0.70	91.28	0	01:35
906 (STM)	JUNCTION	0.19	0.79	91.28	0	01:35
908 (STM)	JUNCTION	0.41	0.99	91.26	0	01:35
908a	JUNCTION	0.48	1.06	91.25	0	01:35
C100 (STM)	JUNCTION	0.08	0.59	91.48	0	01:39
C102 (STM)	JUNCTION	0.11	0.54	91.39	0	01:38
C104 (STM)	JUNCTION	0.14	0.49	91.30	0	01:37
C106 (STM)	JUNCTION	0.19	0.45	91.20	0	01:37
C108 (STM)	JUNCTION	0.22	0.40	91.12	0	01:36
C110 (STM)	JUNCTION	0.46	2.19	92.86	0	01:39
CB01-02	JUNCTION	0.13	1.36	93.41	0	01:34
CB03-04	JUNCTION	0.22	1.52	93.41	0	01:51
CB05-06	JUNCTION	0.11	1.34	93.19	0	01:33
CB07-08	JUNCTION	0.16	1.36	93.08	0	01:35
CB09-10	JUNCTION	0.13	1.35	93.26	0	01:37
CB100	JUNCTION	0.09	1.43	94.03	0	01:30
CB102	JUNCTION	0.09	1.43	94.03	0	01:30
CB104	JUNCTION	0.09	1.44	94.04	0	01:30
CB106	JUNCTION	0.09	1.44	94.04	0	01:30
CB108	JUNCTION	0.09	1.43	94.03	0	01:30
CB110	JUNCTION	0.10	1.43	94.03	0	01:30
CB11-12	JUNCTION</					

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

CB42-43	JUNCTION	0.11	1.36	93.63	0	01:35
CB44-45	JUNCTION	0.12	1.44	93.77	0	01:30
CB46-47	JUNCTION	0.14	1.37	93.84	0	01:36
CB48-49	JUNCTION	0.17	1.33	93.87	0	01:39
CB50-51	JUNCTION	0.12	1.35	93.74	0	01:34
CB52-53	JUNCTION	0.21	1.45	93.77	0	01:42
CB54	JUNCTION	0.06	1.24	93.61	0	01:30
CB55-56	JUNCTION	0.16	1.41	93.59	0	01:35
CB57-58	JUNCTION	0.14	1.27	93.47	0	01:35
CB59	JUNCTION	0.03	0.64	93.14	0	01:30
CB60	JUNCTION	0.08	1.51	93.81	0	01:30
CB61	JUNCTION	0.05	1.34	93.64	0	01:30
CB62	JUNCTION	0.07	1.48	93.77	0	01:30
CB63	JUNCTION	0.07	1.50	93.85	0	01:30
CB64	JUNCTION	0.07	1.45	93.91	0	01:30
CB-65	JUNCTION	0.09	1.33	93.83	0	01:37
CB66	JUNCTION	0.07	1.27	93.97	0	01:31
CB67-68-69	JUNCTION	0.13	1.43	93.83	0	01:34
HP-01	JUNCTION	0.00	0.00	93.91	0	00:00
HP-03	JUNCTION	0.00	0.01	93.81	0	01:31
RYP-01	JUNCTION	0.00	0.00	94.00	0	00:00
RYP-02	JUNCTION	0.00	0.00	93.95	0	00:00
RYP-03	JUNCTION	0.00	0.01	93.85	0	01:31
RYP-04	JUNCTION	0.00	0.00	93.78	0	00:00
1-HW-01 (STM)	OUTFALL	1.43	1.43	90.63	0	00:00
2-OverlandOutlet	OUTFALL	0.00	0.01	93.01	0	01:36
3-HW-02 (STM)	OUTFALL	1.02	1.02	90.92	0	00:00
4-GreenbankOut	OUTFALL	0.00	0.00	0.00	0	00:00
A30-STOR	STORAGE	0.12	1.52	93.82	0	01:32
A33-STOR	STORAGE	0.13	1.65	93.95	0	01:32
B04_Stor	STORAGE	0.13	1.53	94.28	0	01:32
Caiyan-Stor	STORAGE	0.22	1.58	93.68	0	01:41

 Node Inflow Summary

Node	Type	Maximum	Maximum	Lateral	Total	Inflow	Inflow
		Lateral	Total	Time of Max	Inflow		
		LPS	LPS	days	hr:min	10^6 ltr	10^6 ltr
01+011	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
01+051	JUNCTION	0.00	0.00	0	01:30	0.000	0.000
01+118	JUNCTION	0.00	33.41	0	01:30	0.000	0.013
01+143	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
02+196	JUNCTION	0.00	49.06	0	01:30	0.000	0.020
02+288	JUNCTION	0.00	4.15	0	01:38	0.000	0.001
02+365	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
02+432	JUNCTION	0.00	9.91	0	01:31	0.000	0.002
02+458	JUNCTION	0.00	11.98	0	01:30	0.000	0.004
02+512	JUNCTION	0.00	52.02	0	01:30	0.000	0.026
02+564	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
03+026	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
03+054	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
03+109	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
03+127	JUNCTION	0.00	55.11	0	01:36	0.000	0.059
03+200	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
03+291	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
03+384	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
04+078	JUNCTION	0.00	77.25	0	01:30	0.000	0.057
04+089	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
04+129	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
04+207	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
05+116	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
05+200	JUNCTION	0.00	61.34	0	01:30	0.000	0.056
06+023	JUNCTION	0.00	0.00	0	00:00	0.000	0.000

06+043	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
06+108	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
06+194	JUNCTION	0.00	1.24	0	01:30	0.000	0.000
06+273	JUNCTION	0.00	74.81	0	01:36	0.000	0.061
06+309	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
06+332	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
06+355	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
09+000	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
09+097	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
09+203	JUNCTION	0.00	15.59	0	01:28	0.000	0.004
10+011	JUNCTION	0.00	79.32	0	01:33	0.000	0.062
10+232	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
102 (STM)	JUNCTION	0.00	23.33	0	01:30	0.000	0.004
104 (STM)	JUNCTION	0.00	40.45	0	01:30	0.000	0.056
106 (STM)	JUNCTION	0.00	122.89	0	01:34	0.000	0.376
106a	JUNCTION	0.00	179.06	0	01:34	0.000	0.544
108 (STM)	JUNCTION	0.00	236.47	0	01:34	0.000	0.674
108a	JUNCTION	0.00	274.75	0	01:34	0.000	0.803
11+000	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
11+086	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
110 (STM)	JUNCTION	0.00	438.37	0	01:34	0.000	1.569
110a	JUNCTION	0.00	447.33	0	01:34	0.000	1.599
200 (STM)	JUNCTION	0.00	24.26	0	01:21	0.000	0.046
202 (STM)	JUNCTION	0.00	215.60	0	01:32	0.000	0.641
202a	JUNCTION	0.00	247.71	0	01:39	0.000	0.739
204 (STM)	JUNCTION	0.00	264.15	0	01:40	0.000	0.810
204a	JUNCTION	0.00	303.37	0	01:41	0.000	0.941
206 (STM)	JUNCTION	0.00	384.56	0	01:43	0.000	1.350
208 (STM)	JUNCTION	0.00	405.50	0	01:40	0.000	1.423
208a	JUNCTION	0.00	421.54	0	01:39	0.000	1.472
210 (STM)	JUNCTION	0.00	497.79	0	01:38	0.000	1.740
212 (STM)	JUNCTION	0.00	519.92	0	01:38	0.000	1.808
214 (STM)	JUNCTION	0.00	595.08	0	01:36	0.000	2.055
216 (STM)	JUNCTION	0.00	1672.13	0	01:37	0.000	6.459
304 (STM)	JUNCTION	0.00	39.29	0	01:35	0.000	0.135
306 (STM)	JUNCTION	0.00	86.73	0	01:54	0.000	0.334
306a (STM)	JUNCTION	0.00	103.08	0	01:53	0.000	0.349
308 (STM)	JUNCTION	0.00	154.90	0	01:51	0.000	0.544
310 (STM)	JUNCTION	0.00	174.39	0	01:50	0.000	0.699
312 (STM)							

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

C108 (STM)	JUNCTION	0.00	38.69	0	02:03	0.000	0.062
C110 (STM)	JUNCTION	0.00	1077.82	0	01:39	0.000	4.405
CB01-02	JUNCTION	86.67	86.67	0	01:30	0.135	0.135
CB03-04	JUNCTION	89.11	90.06	0	01:30	0.143	0.201
CB05-06	JUNCTION	104.87	104.87	0	01:30	0.172	0.172
CB07-08	JUNCTION	97.28	97.28	0	01:30	0.157	0.157
CB09-10	JUNCTION	86.15	145.07	0	01:30	0.133	0.189
CB100	JUNCTION	33.18	33.18	0	01:30	0.050	0.050
CB102	JUNCTION	36.88	36.88	0	01:30	0.056	0.056
CB104	JUNCTION	45.59	45.59	0	01:30	0.069	0.069
CB106	JUNCTION	51.57	51.57	0	01:30	0.078	0.078
CB108	JUNCTION	46.20	46.20	0	01:30	0.070	0.070
CB110	JUNCTION	40.18	40.18	0	01:30	0.062	0.062
CB11-12	JUNCTION	0.00	30.01	0	01:30	0.000	0.129
CB11-12ROAD	JUNCTION	97.73	97.73	0	01:30	0.158	0.158
CB13-14	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
CB13-14ROAD	JUNCTION	49.90	49.90	0	01:30	0.074	0.074
CB15-16	JUNCTION	102.55	149.94	0	01:30	0.165	0.224
CB17-18	JUNCTION	89.83	137.38	0	01:30	0.142	0.218
CB19	JUNCTION	31.75	62.74	0	01:29	0.047	0.059
CB20-21	JUNCTION	147.70	147.70	0	01:30	0.236	0.241
CB22	JUNCTION	68.09	68.09	0	01:30	0.111	0.111
CB23	JUNCTION	69.98	70.02	0	01:30	0.114	0.114
CB24-25	JUNCTION	76.25	76.25	0	01:30	0.124	0.124
CB26-27	JUNCTION	153.65	153.65	0	01:30	0.256	0.256
CB28	JUNCTION	0.00	78.41	0	01:37	0.000	0.089
CB29	JUNCTION	76.79	106.76	0	01:37	0.129	0.191
CB30-31	JUNCTION	69.48	207.92	0	01:30	0.110	0.354
CB32-33	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
CB32-33 ROAD	JUNCTION	76.12	145.75	0	01:30	0.123	0.243
CB34-35	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
CB34-35 ROAD	JUNCTION	74.23	74.23	0	01:30	0.120	0.120
CB36	JUNCTION	36.54	36.54	0	01:30	0.054	0.054
CB37	JUNCTION	53.60	73.11	0	01:30	0.085	0.093
CB39	JUNCTION	38.75	50.49	0	01:26	0.056	0.060
CB40-41	JUNCTION	156.84	161.92	0	01:30	0.261	0.264
CB42-43	JUNCTION	94.94	114.29	0	01:30	0.155	0.168
CB44-45	JUNCTION	91.76	91.76	0	01:30	0.142	0.143
CB46-47	JUNCTION	29.20	73.72	0	01:30	0.044	0.072
CB48-49	JUNCTION	48.83	48.83	0	01:30	0.073	0.073
CB50-51	JUNCTION	80.49	80.49	0	01:30	0.129	0.130
CB52-53	JUNCTION	184.67	184.67	0	01:30	0.409	0.409
CB54	JUNCTION	18.84	36.44	0	01:30	0.028	0.036
CB55-56	JUNCTION	121.49	178.44	0	01:30	0.185	0.210
CB57-58	JUNCTION	95.76	162.61	0	01:30	0.156	0.188
CB59	JUNCTION	11.43	11.43	0	01:29	0.018	0.018
CB60	JUNCTION	41.25	41.25	0	01:29	0.063	0.063
CB61	JUNCTION	19.05	19.05	0	01:29	0.029	0.029
CB62	JUNCTION	38.72	38.72	0	01:29	0.059	0.059
CB63	JUNCTION	40.62	40.62	0	01:29	0.062	0.062
CB64	JUNCTION	33.33	33.33	0	01:29	0.051	0.051
CB-65	JUNCTION	20.40	20.40	0	01:29	0.031	0.044
CB66	JUNCTION	24.30	24.30	0	01:30	0.035	0.035
CB67-68-69	JUNCTION	158.55	158.55	0	01:30	0.259	0.259
HP-01	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
HP-03	JUNCTION	0.00	1.43	0	01:30	0.000	0.000
RYP-01	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
RYP-02	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
RYP-03	JUNCTION	0.00	2.70	0	01:30	0.000	0.000
RYP-04	JUNCTION	0.00	0.00	0	00:00	0.000	0.000
1-HW-01 (STM)	OUTFALL	0.00	1303.10	0	01:36	0.000	4.758
2-OverlandOutlet	OUTFALL	0.00	4.17	0	01:36	0.000	0.002
3-HW-02 (STM)	OUTFALL	0.00	1672.18	0	01:37	0.000	6.459
4-GreenbankOut	OUTFALL	868.92	868.92	0	01:30	1.447	1.447
A30-STOR	STORAGE	177.81	177.81	0	01:30	0.285	0.285
A33-STOR	STORAGE	291.70	291.70	0	01:30	0.468	0.468
B04_Stor	STORAGE	292.32	292.32	0	01:30	0.472	0.472
Caivan-Stor	STORAGE	2057.67	2057.67	0	01:30	4.355	4.355

Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Meters	Min. Depth Meters
102 (STM)	JUNCTION	0.12	0.102	2.242
104 (STM)	JUNCTION	0.28	0.223	2.396
106 (STM)	JUNCTION	0.40	0.309	2.337
106a	JUNCTION	0.50	0.408	2.272
108 (STM)	JUNCTION	0.51	0.408	2.258
108a	JUNCTION	0.57	0.430	2.094
110 (STM)	JUNCTION	0.58	0.418	2.041
110a	JUNCTION	0.62	0.416	2.036
206 (STM)	JUNCTION	0.22	0.036	2.052
208 (STM)	JUNCTION	0.20	0.025	2.343
306a (STM)	JUNCTION	0.02	0.002	2.348
308 (STM)	JUNCTION	0.31	0.093	2.284
310 (STM)	JUNCTION	0.59	0.248	2.376
312 (STM)	JUNCTION	0.67	0.287	2.292
314 (STM)	JUNCTION	0.73	0.266	1.653
316 (STM)	JUNCTION	12.00	0.262	2.389
324 (STM)	JUNCTION	0.56	0.084	1.868
326 (STM)	JUNCTION	0.72	0.061	1.829
328 (STM)	JUNCTION	12.00	0.199	1.503
330 (STM)	JUNCTION	12.00	0.206	1.780
402a (STM)	JUNCTION	0.09	0.046	2.314
404 (STM)	JUNCTION	0.24	0.158	2.314
404a	JUNCTION	0.40	0.241	2.276
406 (STM)	JUNCTION	0.05	0.028	2.171
408 (STM)	JUNCTION	0.32	0.236	2.266
608 (STM)	JUNCTION	0.43	0.278	2.244
608a	JUNCTION	0.50	0.346	2.170
610 (STM)	JUNCTION	0.63	0.369	2.055
610a	JUNCTION	0.65	0.339	2.150
612 (STM)	JUNCTION	0.57	0.323	3.050
614 (STM)	JUNCTION	0.55	0.413	2.474
616 (STM)	JUNCTION	0.40	0.740	2.544
618 (STM)	JUNCTION	0.52	0.269	2.405
904 (STM)	JUNCTION	0.19	0.083	2.482
906 (STM)	JUNCTION	0.37	0.167	2.256
908 (STM)	JUNCTION	0.54	0.303	2.314

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

Storage Unit	1000 m3	Full	Loss	1000 m3	Full	days	hr:min	LPS
A30-STOR	0.001	1	0	0.034	15	0	01:32	96.77
A33-STOR	0.002	2	0	0.053	70	0	01:32	164.92
B04_Stor	0.002	1	0	0.054	18	0	01:32	160.61
Caivan-Stor	0.024	1	0	0.483	28	0	01:41	1063.47

Outfall Loading Summary								

Flow Avg. Max. Total								
Freq. Flow Flow Volume								
Outfall Node	Pcnt.	LPS	LPS	10^6 ltr				

1-HW-01 (STM)	99.90	152.35	1303.10	4.758				
2-OverlandOutlet	2.78	2.85	4.17	0.002				
3-HW-02 (STM)	99.83	211.96	1672.18	6.459				
4-GreenbankOut	76.38	61.99	868.92	1.447				

System	69.72	429.14	3635.40	12.667				

Link Flow Summary								

Maximum Time of Max Maximum Max/ Max/								
Flow Occurrence Veloc Full Full								
Link	Type	LPS	days	hr:min	m/sec	Flow	Depth	

{STM}.102-104	CONDUIT	23.33	0	01:30	0.17	0.05	1.00	
{STM}.104-106	CONDUIT	40.73	0	01:30	0.18	0.12	1.00	
{STM}.106-106a	CONDUIT	123.23	0	01:34	0.43	0.41	1.00	
{STM}.108-108a	CONDUIT	236.59	0	01:34	0.64	0.67	1.00	
{STM}.110-110a	CONDUIT	438.22	0	01:34	0.96	0.85	1.00	
{STM}.306-306a	CONDUIT	103.08	0	01:53	0.72	0.32	0.90	
{STM}.308-310	CONDUIT	158.01	0	01:50	0.54	0.48	1.00	
{STM}.310-312	CONDUIT	174.62	0	01:50	0.38	0.39	1.00	
{STM}.312-314	CONDUIT	1044.05	0	01:36	0.89	0.81	1.00	
{STM}.314-316	CONDUIT	1044.12	0	01:36	0.89	0.80	1.00	
{STM}.316-324	CONDUIT	1302.91	0	01:36	1.12	1.02	1.00	
{STM}.326-328	CONDUIT	1303.09	0	01:36	0.69	0.55	1.00	
{STM}.328-HW-1	CONDUIT	1303.10	0	01:36	0.69	0.53	1.00	
{STM}.330-336	CONDUIT	790.95	0	01:36	0.68	0.28	1.00	
{STM}.402-402a	CONDUIT	33.74	0	01:57	0.45	0.13	0.98	
{STM}.404-404a	CONDUIT	94.06	0	01:56	0.68	0.29	1.00	
{STM}.406-408	CONDUIT	48.26	0	01:54	0.48	0.17	1.00	
{STM}.408-106	CONDUIT	64.54	0	01:53	0.35	0.22	1.00	
{STM}.608-608a	CONDUIT	159.68	0	01:56	0.69	0.56	1.00	
{STM}.610-610a	CONDUIT	695.97	0	01:35	0.90	0.86	1.00	
{STM}.612-316	CONDUIT	260.45	0	01:33	0.89	0.91	1.00	
{STM}.614-312	CONDUIT	260.28	0	01:33	0.89	0.91	1.00	
{STM}.616-614	CONDUIT	51.03	0	01:22	0.35	0.38	1.00	
{STM}.618-610	CONDUIT	45.81	0	02:04	0.23	0.13	1.00	
{STM}.902-904	CONDUIT	20.55	0	01:24	0.17	0.06	0.97	
{STM}.904-906	CONDUIT	51.59	0	01:25	0.28	0.18	1.00	
{STM}.906-908	CONDUIT	104.70	0	01:56	0.37	0.36	1.00	
{STM}.908-908a	CONDUIT	106.16	0	01:56	0.29	0.31	1.00	
106a-108	CONDUIT	179.21	0	01:34	0.61	0.63	1.00	
108a-110	CONDUIT	274.68	0	01:34	0.74	0.76	1.00	
110a-610	CONDUIT	447.24	0	01:34	0.98	0.87	1.00	
200-202	CONDUIT	37.74	0	01:50	0.17	0.11	0.75	
202-204	CONDUIT	214.03	0	01:39	0.86	0.62	0.85	
202a-204	CONDUIT	247.42	0	01:40	1.01	0.71	0.89	
204-206	CONDUIT	264.48	0	01:41	0.80	0.58	0.94	

204a-206	CONDUIT	305.67	0	01:43	0.70	0.68	0.99
206-208	CONDUIT	385.01	0	01:43	0.84	0.84	1.00
208-208a	CONDUIT	405.64	0	01:40	0.89	1.07	0.99
208a-210	CONDUIT	421.70	0	01:39	0.95	1.14	0.94
210-212	CONDUIT	497.87	0	01:38	0.96	1.02	0.89
212-214	CONDUIT	520.04	0	01:38	0.87	1.09	0.85
214-216	CONDUIT	595.16	0	01:37	1.07	0.93	0.79
216-H2	CONDUIT	1672.18	0	01:37	0.99	0.60	0.83
304-306	CONDUIT	45.12	0	01:54	0.63	0.13	0.63
306a-308	CONDUIT	110.63	0	01:52	0.52	0.34	1.00
324-330	CONDUIT	790.90	0	01:36	0.68	0.32	1.00
402a-404	CONDUIT	69.48	0	01:56	0.80	0.25	1.00
404a-608	CONDUIT	156.29	0	01:56	0.83	0.47	1.00
608a-110	CONDUIT	163.60	0	01:58	0.57	0.57	1.00
610a-312	CONDUIT	756.15	0	01:35	0.98	0.90	1.00
908a-610	CONDUIT	162.77	0	01:55	0.44	0.48	1.00
C100-208	CONDUIT	15.84	0	01:33	0.09	0.06	0.98
C102-208a	CONDUIT	17.57	0	01:37	0.08	0.07	0.93
C104-210	CONDUIT	21.77	0	01:34	0.13	0.09	0.84
C106-212	CONDUIT	24.71	0	01:34	0.16	0.10	0.78
C108-214	CONDUIT	37.21	0	02:04	0.26	0.15	0.70
C110-216	CONDUIT	1077.82	0	01:39	3.69	4.33	1.00
CB10-11ROADLINK	CONDUIT	0.00	0	00:00	0.00	0.00	0.12
CB12-13ROADLINK	CONDUIT	0.00	0	00:00	0.00	0.00	0.00
CB27-28ROADLINK	CONDUIT	0.00	0	00:00	0.00		

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

S04-02	CHANNEL	67.64	0	01:31	0.11	0.01	0.48		OCB20-21	ORIFICE	60.15	0	01:37		1.00
S04-03	CHANNEL	0.00	0	00:00	0.00	0.00	0.10		OCB22	ORIFICE	28.35	0	01:38		1.00
S04-04	CHANNEL	0.00	0	00:00	0.00	0.00	0.19		OCB23	ORIFICE	27.74	0	01:40		1.00
S04-05	CHANNEL	0.00	0	00:00	0.00	0.00	0.19		OCB24-25	ORIFICE	38.16	0	01:34		1.00
S04-06	CHANNEL	0.00	0	00:00	0.00	0.00	0.25		OCB26-27	ORIFICE	76.21	0	01:34		1.00
S04-07	CHANNEL	0.00	0	00:00	0.00	0.00	0.25		OCB28	ORIFICE	38.24	0	01:46		1.00
S04-08	CHANNEL	0.00	0	00:00	0.00	0.00	0.24		OCB28-Stor	ORIFICE	96.77	0	01:32		1.00
S04-09	CHANNEL	33.41	0	01:30	0.15	0.01	0.32		OCB29	ORIFICE	21.09	0	01:46		1.00
S05-01	CONDUIT	77.25	0	01:30	0.12	0.02	0.43		OCB30-31	ORIFICE	41.20	0	01:36		1.00
S05-02	CHANNEL	47.72	0	01:30	0.32	0.02	0.43		OCB31-Stor	ORIFICE	164.92	0	01:32		1.00
S05-03	CHANNEL	0.00	0	00:00	0.00	0.00	0.10		OCB32-33	ORIFICE	0.00	0	00:00		0.00
S05-04	CHANNEL	0.00	0	00:00	0.00	0.00	0.11		OCB34-35	ORIFICE	0.00	0	00:00		0.00
S05-05	CHANNEL	61.34	0	01:30	0.64	0.02	0.18		OCB36	ORIFICE	16.09	0	01:30		1.00
S05-06	CHANNEL	60.85	0	01:30	0.51	0.01	0.48		OCB37	ORIFICE	24.42	0	01:30		1.00
S05-07	CHANNEL	0.00	0	00:00	0.00	0.00	0.42		OCB39	ORIFICE	16.75	0	01:30		1.00
S06-01d	CONDUIT	31.86	0	01:29	0.08	0.01	0.55		OCB40-41	ORIFICE	76.63	0	01:35		1.00
S06-02	CHANNEL	0.00	0	00:00	0.00	0.00	0.36		OCB42-43	ORIFICE	55.84	0	01:35		1.00
S06-03	CHANNEL	0.00	0	00:00	0.00	0.00	0.16		OCB44-45	ORIFICE	40.38	0	01:30		1.00
S06-04	CHANNEL	0.10	0	01:31	0.04	0.00	0.16		OCB46-47	ORIFICE	16.98	0	01:36		1.00
S06-05	CHANNEL	0.10	0	01:31	0.05	0.00	0.15		OCB48-49	ORIFICE	16.73	0	01:39		1.00
S06-06	CONDUIT	78.35	0	01:37	0.16	0.06	0.30		OCB50-51	ORIFICE	39.03	0	01:34		1.00
S06-07	CHANNEL	72.72	0	01:37	0.63	0.01	0.23		OCB52-53	ORIFICE	78.89	0	01:42		1.00
S06-08	CHANNEL	0.00	0	00:00	0.00	0.00	0.09		OCB54	ORIFICE	16.09	0	01:30		1.00
S06-09	CHANNEL	0.00	0	00:00	0.00	0.00	0.00		OCB55-56	ORIFICE	56.93	0	01:35		1.00
S06-10	CHANNEL	0.00	0	00:00	0.00	0.00	0.00		OCB57-58	ORIFICE	53.88	0	01:35		1.00
S06-11	CHANNEL	0.00	0	00:00	0.00	0.00	0.28		OCB59	ORIFICE	11.37	0	01:30		1.00
S09-00	CHANNEL	16.63	0	01:26	0.13	0.00	0.32		OCB60	ORIFICE	30.03	0	01:30		1.00
S09-01	CHANNEL	15.59	0	01:28	0.13	0.00	0.27		OCB61	ORIFICE	16.75	0	01:30		1.00
S09-02	CHANNEL	6.89	0	01:31	0.11	0.00	0.32		OCB62	ORIFICE	29.78	0	01:30		1.00
S09-03	CHANNEL	0.00	0	00:00	0.00	0.00	0.28		OCB63	ORIFICE	29.95	0	01:30		1.00
S09-04	CHANNEL	0.00	0	00:00	0.00	0.00	0.26		OCB64	ORIFICE	26.28	0	01:30		1.00
S09-05	CONDUIT	0.00	0	00:00	0.00	0.00	0.26		OCB65	ORIFICE	16.72	0	01:37		1.00
S09-06	CONDUIT	0.00	0	00:00	0.00	0.00	0.14		OCB66	ORIFICE	16.32	0	01:31		1.00
S10-01	CHANNEL	0.00	0	00:00	0.00	0.00	0.11		OCB67-68	ORIFICE	78.33	0	01:34		1.00
S10-02	CHANNEL	69.99	0	01:30	0.36	0.03	0.26		OCB-B04	ORIFICE	160.61	0	01:32		1.00
S10-03	CHANNEL	139.54	0	01:30	0.43	0.05	0.62		OCB-C100	ORIFICE	15.34	0	01:30		1.00
S10-04	CONDUIT	79.32	0	01:33	0.05	0.01	0.53		OCB-C102	ORIFICE	17.76	0	01:30		1.00
S10-05	CONDUIT	74.81	0	01:36	0.26	0.01	0.13		OCB-C104	ORIFICE	22.25	0	01:30		1.00
S11-01	CHANNEL	0.00	0	00:00	0.00	0.00	0.12		OCB-C106	ORIFICE	25.17	0	01:30		1.00
S11-02	CHANNEL	0.00	0	00:00	0.00	0.00	0.12		OCB-C108	ORIFICE	22.68	0	01:30		1.00
S11-03	CHANNEL	0.00	0	00:00	0.00	0.00	0.28		OCB-C110	ORIFICE	19.31	0	01:23		1.00
ST10-00	CHANNEL	0.00	0	00:00	0.00	0.00	0.06		Orifice-55	ORIFICE	1063.47	0	01:45		1.00
ST6-01a	CHANNEL	0.00	0	00:00	0.00	0.00	0.06		VORTECHS	WEIR	513.77	0	01:35		1.00
ST6-01b	CHANNEL	0.00	0	00:00	0.00	0.00	0.00		10-11	DUMMY	30.01	0	01:30		
ST6-01c	CHANNEL	0.00	0	00:00	0.00	0.00	0.19		12-13	DUMMY	0.00	0	00:00		
STB-00	CHANNEL	0.00	0	00:00	0.00	0.00	0.06		27-28	DUMMY	0.00	0	00:00		
STB-01	CHANNEL	19.60	0	01:30	0.27	0.00	0.16		29-30	DUMMY	0.00	0	00:00		
STB-02	CHANNEL	47.84	0	01:30	0.57	0.01	0.19								
STB-03	CHANNEL	46.86	0	01:30	0.20	0.01	0.36								
STB-04	CHANNEL	11.98	0	01:30	0.10	0.00	0.41								
STB-05	CHANNEL	1.10	0	01:38	0.05	0.00	0.14								
STB-06	CHANNEL	1.10	0	01:38	0.10	0.00	0.26								
STB-07	CHANNEL	0.00	0	00:00	0.00	0.00	0.24								
STB-08	CHANNEL	0.00	0	00:00	0.00	0.00	0.41								
STB-09	CHANNEL	4.15	0	01:38	0.01	0.00	0.42								
STB-10	CHANNEL	0.22	0	01:41	0.14	0.00	0.06								
STB-11	CHANNEL	91.27	0												

Burnett Lands – 3370 Greenbank Road 100-year Storm, 5-year Fixed Outlet Elevations Model Output

Burnett Lands – 3370 Greenbank Road
100-year Storm, 5-year Fixed Outlet Elevations
Model Output

STB-00	1.27	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-01	1.00	0.96	0.00	0.00	0.03	0.00	0.00	0.00	0.02	0.0000
STB-02	1.00	0.00	0.96	0.00	0.03	0.01	0.00	0.00	0.03	0.0000
STB-03	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.43	0.0000
STB-04	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.81	0.0000
STB-05	1.12	0.00	0.00	0.00	0.95	0.05	0.00	0.00	0.68	0.0000
STB-06	1.00	0.00	0.00	0.00	0.07	0.00	0.00	0.93	0.38	0.0000
STB-07	1.00	0.93	0.07	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-08	1.00	0.87	0.13	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-09	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.37	0.0000
STB-10	1.00	0.00	0.00	0.00	0.02	0.00	0.00	0.98	0.48	0.0000
STB-11	1.00	0.90	0.06	0.00	0.02	0.00	0.03	0.00	0.00	0.0000
STB-12	1.00	0.00	0.00	0.00	0.10	0.00	0.00	0.90	0.88	0.0000
STB-13	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	1.00	0.0000

Analysis ended on: Wed Nov 23 10:04:49 2016
 Total elapsed time: 00:00:04

 Conduit Surcharge Summary

Conduit	Hours Full		Above Full Capacity	Hours Limited
	Both Ends	Upstream		
{STM}.102-104	0.12	0.12	0.12	0.01
{STM}.104-106	0.35	0.35	0.35	0.01
{STM}.106-106a	0.44	0.44	0.44	0.01
{STM}.108-108a	0.51	0.51	0.52	0.01
{STM}.110-110a	0.58	0.58	0.58	0.01
{STM}.308-310	0.33	0.33	0.33	0.01
{STM}.310-312	0.59	0.59	0.59	0.01
{STM}.312-314	0.67	0.67	0.67	0.01
{STM}.314-316	0.76	0.76	0.76	0.01
{STM}.316-324	12.00	12.00	12.00	0.11
{STM}.326-328	12.00	12.00	12.00	0.01
{STM}.328-HW-1	12.00	12.00	12.00	0.01
{STM}.330-336	12.00	12.00	12.00	0.01
{STM}.404-404a	0.35	0.35	0.36	0.01
{STM}.406-408	0.05	0.05	0.05	0.01
{STM}.408-106	0.37	0.37	0.37	0.01
{STM}.608-608a	0.46	0.46	0.46	0.01
{STM}.610-610a	0.63	0.63	0.63	0.01
{STM}.612-316	0.66	0.66	0.66	0.01
{STM}.614-312	0.55	0.55	0.56	0.01
{STM}.616-614	0.40	0.40	0.41	0.01
{STM}.618-610	0.52	0.52	0.52	0.01
{STM}.904-906	0.21	0.21	0.21	0.01
{STM}.906-908	0.40	0.40	0.40	0.01
{STM}.908-908a	0.54	0.54	0.54	0.01
106a-108	0.50	0.50	0.50	0.01
108a-110	0.57	0.57	0.57	0.01
110a-610	0.62	0.62	0.62	0.01
206-208	0.20	0.20	0.20	0.01
208-208a	0.01	0.01	0.01	0.33
208a-210	0.01	0.01	0.01	0.39
210-212	0.01	0.01	0.01	0.17
212-214	0.01	0.01	0.01	0.33
306a-308	0.02	0.02	0.02	0.01
324-330	12.00	12.00	12.00	0.01
402a-404	0.09	0.09	0.09	0.01
404a-608	0.40	0.40	0.40	0.01
608a-110	0.50	0.50	0.50	0.01
610a-312	0.65	0.65	0.65	0.01
908a-610	0.62	0.62	0.62	0.01
C110-216	0.65	0.65	0.65	1.25

Analysis begun on: Wed Nov 23 10:04:45 2016

Appendix E

Burnett Municipal Drain Analysis

MEMORANDUM

DATE: JUNE 10, 2016
TO: BOB DOWDELL, E.I.T. & EDSON DONNELLY, C.E.T.
FROM: CONRAD STANG, M.A.SC., P.ENG.
RE: SOUTH NEPEAN COLLECTOR CULVERT CROSSINGS
CC: MIKE PETEPIECE, P.ENG.

This technical memorandum provides details on the sizing and location of the proposed access road culvert crossings which will be installed as part of Phase 2 of the South Nepean Collector (SNC). It is anticipated that the proposed culverts will remain in place until such time that the subject lands are developed. The location of the proposed culvert crossings and corresponding drainage areas are shown on the attached figure (DSK54):

- Culvert C1: Burnett Municipal Drain
- Culvert C2: Ditch draining Mion property west of K-B SWMF
- Culvert C3: Ditch adjacent the K-B SWMF

Design Criteria

The culverts are to convey the 10-year peak flows from their respective upstream drainage areas without overtopping the access road, as per Section 6.4.2 of the City of Ottawa Sewer Design Guidelines (October, 2012). For the 600mm culverts this corresponds to a Headwater / Depth (HW/D) of 1.5 (300mm cover), which corresponds to the maximum HW/D ratio as recommended by MTO. Excess flows will overtop the access road and graded back towards the downstream watercourse, as shown in the detail on the attached figure (DSK54).

Design Flows

The culvert crossings have been designed based on current City of Ottawa standards and rainfall data. Peak flows were estimated using the Visual Otthymo hydrologic model; modeling parameters and results are attached. The 12-hour SCS distribution generated the highest peak flows and was selected as the critical storm distribution for sizing the proposed culverts. Simulated peak flows at the proposed culvert crossings are provided in Table 1.

Table 1: Simulated Peak Flows at Proposed Culvert Crossings

Culvert	Culvert Dimensions	Return Period (years)	Peak Flow (m³/s)	HW/D (m)	Freeboard (m)
Culvert C1 Burnett Municipal Drain (29.27 ha)	3x 600mm Dia. CSP Culverts L = 8.0m S = 0.75% *Inv. = 91.10m	2-year	0.63	0.82	0.41
		5-year	1.10	1.13	0.22
		10-year	1.44	1.37	0.08
		25-year	1.90	1.65	0.09 Overtopping
		50-year	2.26	1.75	0.15 Overtopping
		100-year	2.66	1.84	0.20 Overtopping
Culvert C2 Mion/Pavic Ditch (5.11 ha)	1x 600mm Dia. CSP Culvert L = 8.0m S = 0.50% *Inv. = 92.30m	2-year	0.09	0.46	0.63
		5-year	0.16	0.64	0.51
		10-year	0.21	0.77	0.44
		25-year	0.28	0.93	0.34
		50-year	0.34	1.07	0.26
		100-year	0.40	1.21	0.17
Culvert C3 KB-SWMF Ditch (1.36 ha)	1x 1000mm Dia. CSP Culvert L = 12.0m S = 0.60% *Inv. = 92.35m	2-year	0.04	0.15	1.85
		5-year	0.07	0.20	1.80
		10-year	0.09	0.23	1.77
		25-year	0.12	0.26	1.74
		50-year	0.15	0.30	1.70
		100-year	0.17	0.32	1.68

*Inverts to be confirmed in the field.

Culvert Crossings

The proposed culvert crossings were designed using Autodesk Hydraflow Express culvert sizing software – supporting calculations are attached. A summary of the proposed culvert crossings is provided below.

Culvert C1: Burnett Municipal Drain (3x 600mm CSP Culverts)

The Burnett Municipal Drain was established in the late 1960's after the passing of By-Law No. 107-68 (Township of Nepean). A copy of the Burnett Municipal Drain By-Law and October 16th, 1968 Engineers report is attached. Land use in the watershed has changed substantially since the adoption of the By-Law and a significant portion of the upstream drainage area has been redirected to the Kennedy-Burnett SWM Facility. It is anticipated that this drain will be abandoned in the near future as development proceeds within the remaining undeveloped areas south of Strandherd Drive.

The Burnett Municipal Drain is a trapezoidal channel with a 3m bottom width, 0.60m depth and side slopes ranging from 2:1 to 4:1. Based on Manning's equation, the Burnett Municipal Drain has a bankfull capacity of 3.88 m³/s, which is sufficient to convey the 100-year peak flow from the current upstream drainage area (which is considerably smaller than the original drainage area from the 1968 Engineer's Report).

As per the 1968 by-law, an existing 900mm diameter CSP culvert was installed downstream of the proposed SNC crossing to provide access to the Kelvin Burnett property. The proposed crossing should therefore provide at least a similar flow capacity (approximately 900 L/s).

Based on current design standards, the proposed culvert crossing should be three (3) 600mm diameter CSP culverts, which will provide capacity for a 5-year return period flowing full and a 10-

year return period without over topping the access road. Refer to the attached detailed calculations. Storm events greater than the 10-year return period will overtop the access road, but the grading will confine excess flows to the downstream ditch. The middle 600mm CSP culvert will be countersunk 0.10m.

Culvert C2: Mion Ditch (600mm CSP Culvert)

The west watercourse is an intermittent ditch that runs north-to-south through the Mion property. The ditch is a V-bottom ditch with a 0.30m depth and 3:1 / 6:1 side slopes. Based on Manning's equation, the capacity of the ditch is $0.23 \text{ m}^3/\text{s}$, which corresponds to a 10-year storm.

A 600mm diameter CSP culvert has capacity to convey storm events up-to and including the 100-year storm event ($0.40 \text{ m}^3/\text{s}$) without overtopping the access road. Refer to the attached detailed calculations. The 600mm CSP culvert will be countersunk 0.10m.

Culvert C3: KB-SWMF Ditch (1000mm CSP Culvert)

The watercourse west of the KB-SWMF is a deep intermittent ditch that runs north-to-south adjacent the K-B SWMF. The ditch is a V-bottom ditch with a 2.0m depth and 2.5:1 side slopes. Based on Manning's equation, the capacity of the ditch is $21.06 \text{ m}^3/\text{s}$. The estimated 100-year peak flows from the 1.36 ha catchment are $0.17 \text{ m}^3/\text{s}$; therefore, the ditch has ample capacity to convey large peak flows.

Due to the dimensions of the ditch, a 1000mm diameter CSP culvert is recommended. The capacity of the culvert ($\text{HW/D}=1.0$) is $1.10 \text{ m}^3/\text{s}$. Flows should not overtop the culvert, but if they do they will spill into the KB-SWMF. Refer to the attached detailed calculations.

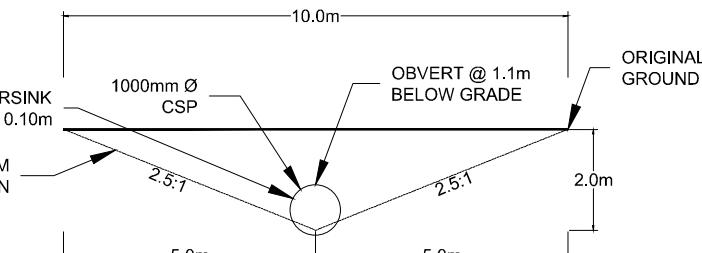
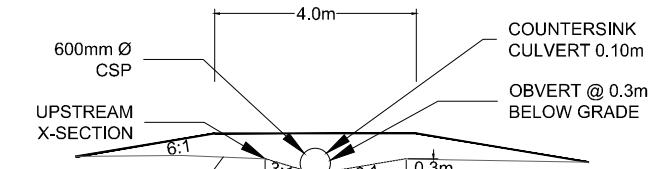
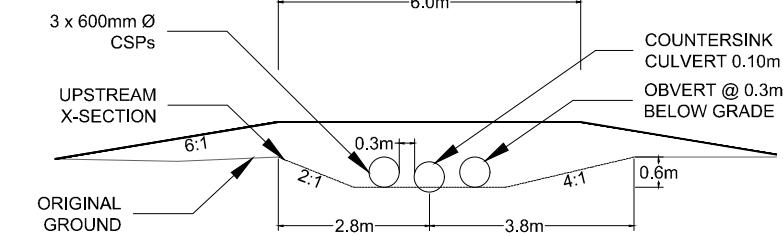
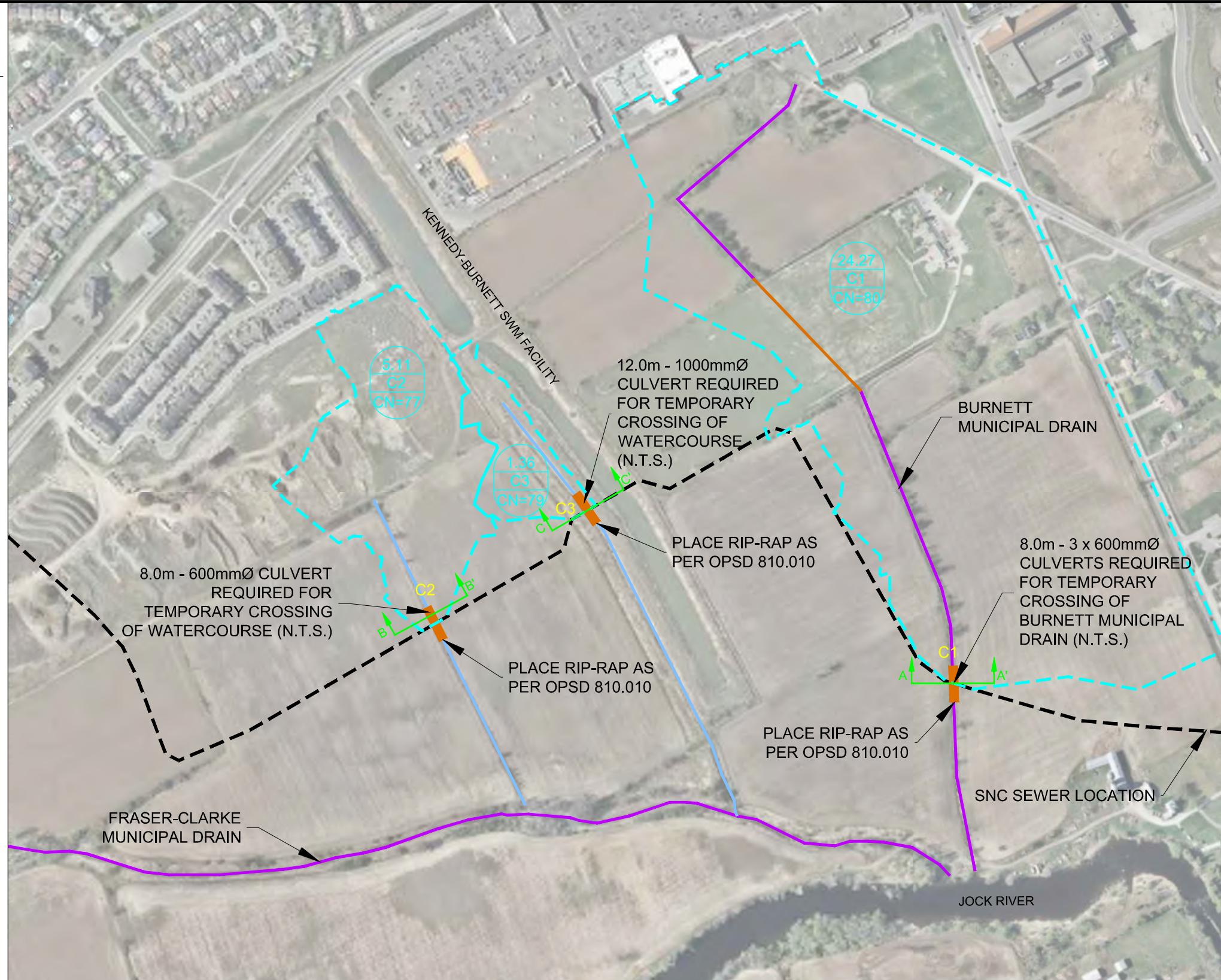
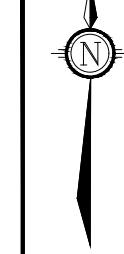
Access Road

For culvert crossings C1 and C2, the 6.0m wide access road will be graded adjacent the culvert crossing in order to have two (2) 3.0m wide depressions, which will act as a weir for flows in excess of the culvert capacity. Refer to the cross-section detail on the attached Figure (DSK54).

Attachments:

- Culvert Locations and Sizes (Figure DSK54)
- Visual Ottymo Modeling Parameters and Results
- Detailed Culvert and Ditch Calculations
- Burnett Municipal Drain By-Law





M:\2015\115075\CAD\Design\Figures\DSK54_Culverts.dwg, SNC, Jun 07, 2016 - 10:01am, nsmit

LEGEND

- CULVERT**
- WATERCOURSE**
- MUNICIPAL DRAIN**
- PROPOSED ALIGNMENT OF THE SOUTH NEPEAN COLLECTOR (SNC)**
- DRAINAGE AREA BOUNDARY**
- CATCHMENT AREA (ha)**
- AREA ID**
- RUNOFF CURVE NUMBER**

24.27
C1
CN=80

NOVATECH

Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6
Telephone (613) 254-9643
Facsimile (613) 254-5867
Website www.novatech-eng.com

SOUTH NEPEAN COLLECTOR SEWER PHASE 2

REQUIRED CULVERT LOCATIONS AND SIZES

SCALE 1 : 5000 0 50 100 150 200

DATE JUN 2016 JOB 115075 FIGURE DSK54

QUT11Y17.DIM/2 270mmx120mm

South Nepean Collector Phase 2: Culvert Crossings
Existing Conditions Hydrologic Model Parameters and Results
115075

Time to Peak Calculations

(Uplands Overland Flow Method)

Area ID	Area (ha)	CN	Ia	Overland Flow					Concentrated Overland Flow					Channel Flow			Overall					
				Length (m)	Elevation U/S (m)	Elevation D/S (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Length (m)	Elevation U/S (m)	Elevation D/S (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Length (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Time of Concentration (min)	Time to Peak (min)	Time to Peak (min)
C1: Burnett MD	29.27	80	6.4	100	95.50	94.50	1.0%	0.28	5.95	150	94.50	92.75	1.2%	0.30	8.33	400	0.7%	0.38	18	32	21	0.36
C2: West Ditch	5.11	77	7.6	100	93.30	93.00	0.3%	0.15	11.11	125	93.00	92.75	0.2%	0.13	16.03	150	0.5%	0.32	8	35	23	0.39
C3: KB SWMF Ditch	1.36	79	6.8	50	93.35	93.15	0.4%	0.17	4.90	25	93.15	93.05	0.4%	0.16	2.60	150	0.6%	0.35	7	15	10	0.17

$$Ia = 0.10 \times S$$

Model Results: 12-hour SCS Storm Distribution

Area ID	Peak Flow (m³/s) (Return Period)					
	2-year	5-year	10-year	25-year	50-year	100-year
C1: Burnett MD	0.63	1.10	1.44	1.90	2.26	2.66
C2: West Ditch	0.09	0.16	0.21	0.28	0.34	0.40
C3: KB SWMF Ditch	0.04	0.07	0.09	0.12	0.15	0.17

Culvert Report

2-year 12-hour SCS Storm

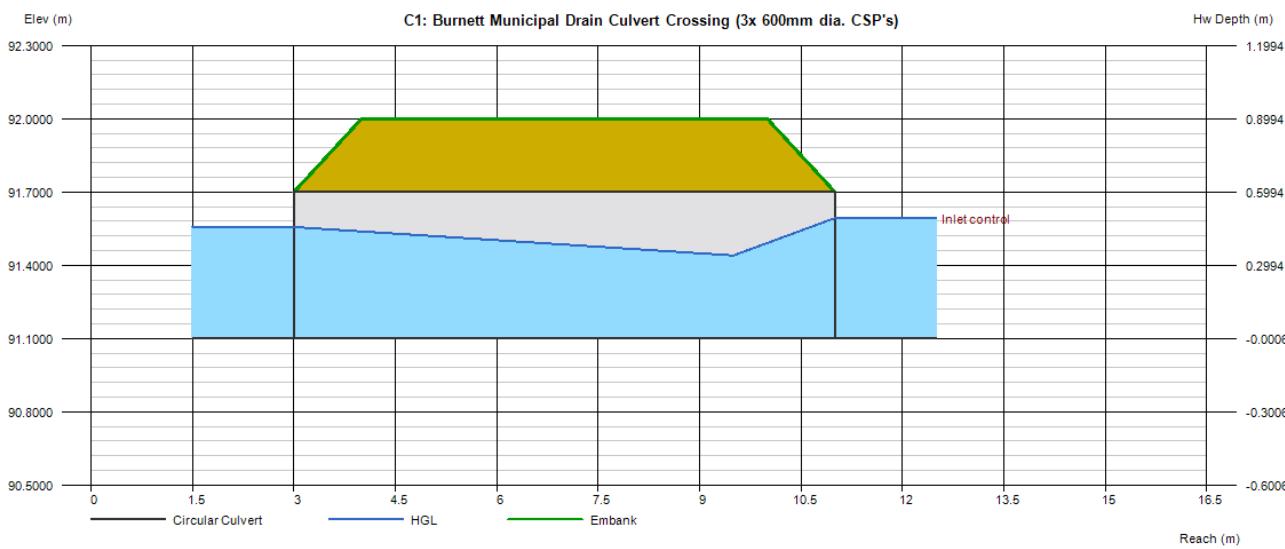
Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9
Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 2.0000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtotals (cms)	= 0.7000
Qpipe (cms)	= 0.7000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.0109
Veloc Up (m/s)	= 1.5645
HGL Dn (m)	= 91.5565
HGL Up (m)	= 91.4135
Hw Elev (m)	= 91.5931
Hw/D (m)	= 0.8208
Flow Regime	= Inlet Control



Culvert Report

5-year 12-hour SCS Storm

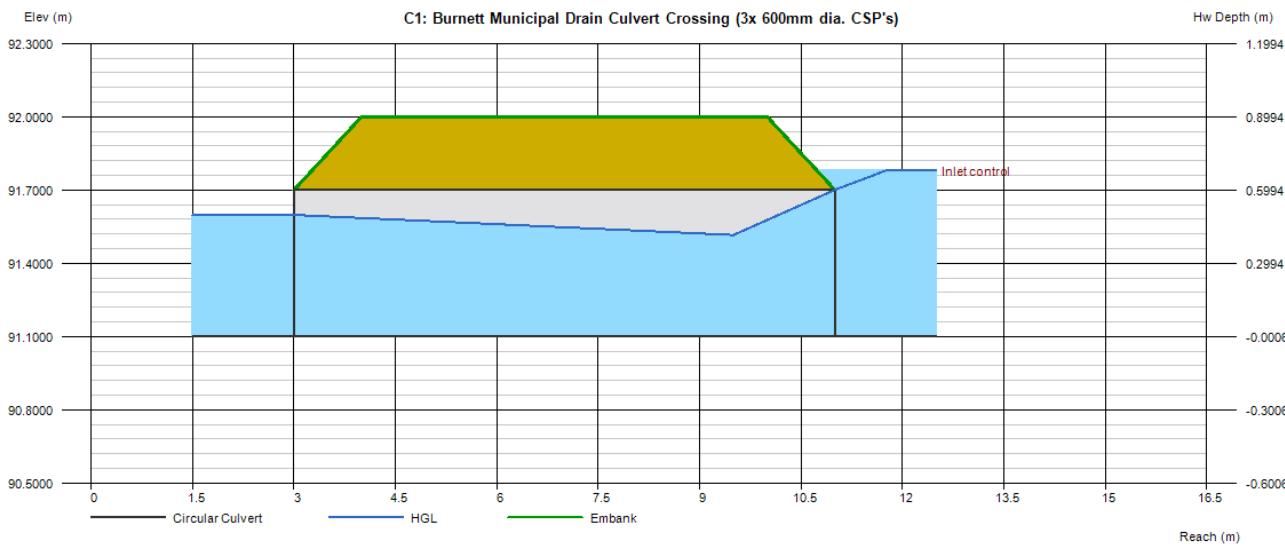
Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9
Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 2.0000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtotals (cms)	= 1.1000
Qpipe (cms)	= 1.1000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.4614
Veloc Up (m/s)	= 1.8514
HGL Dn (m)	= 91.5981
HGL Up (m)	= 91.4967
Hw Elev (m)	= 91.7793
Hw/D (m)	= 1.1311
Flow Regime	= Inlet Control



Culvert Report

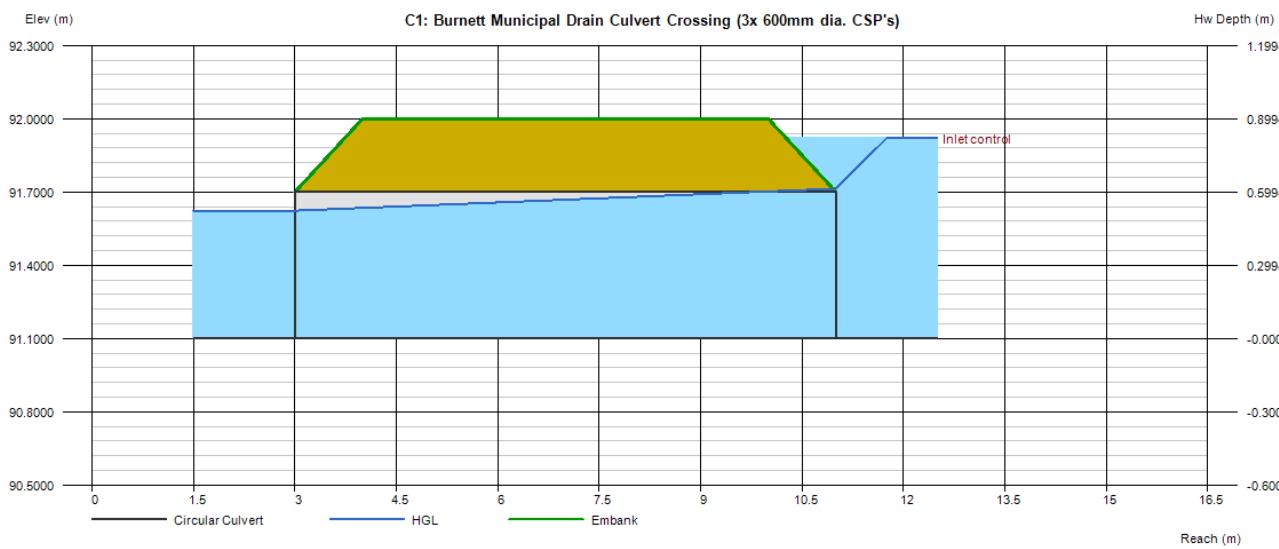
10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000	Calculations	
Pipe Length (m)	= 8.0000	Qmin (cms)	= 0.0000
Slope (%)	= 0.0076	Qmax (cms)	= 2.0000
Invert Elev Up (m)	= 91.1006	Tailwater Elev (m)	= (dc+D)/2
Rise (mm)	= 600.0		
Shape	= Circular	Highlighted	
Span (mm)	= 600.0	Qtot (cms)	= 1.4000
No. Barrels	= 3	Qpipe (cms)	= 1.4000
n-Value	= 0.016	Qovertop (cms)	= 0.0000
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 1.7823
Culvert Entrance	= Projecting	Veloc Up (m/s)	= 1.6505
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 91.6237
Embankment		HGL Up (m)	= 91.7136
Top Elevation (m)	= 92.0000	Hw Elev (m)	= 91.9199
Top Width (m)	= 6.0000	Hw/D (m)	= 1.3655
Crest Width (m)	= 6.0000	Flow Regime	= Inlet Control



Culvert Report

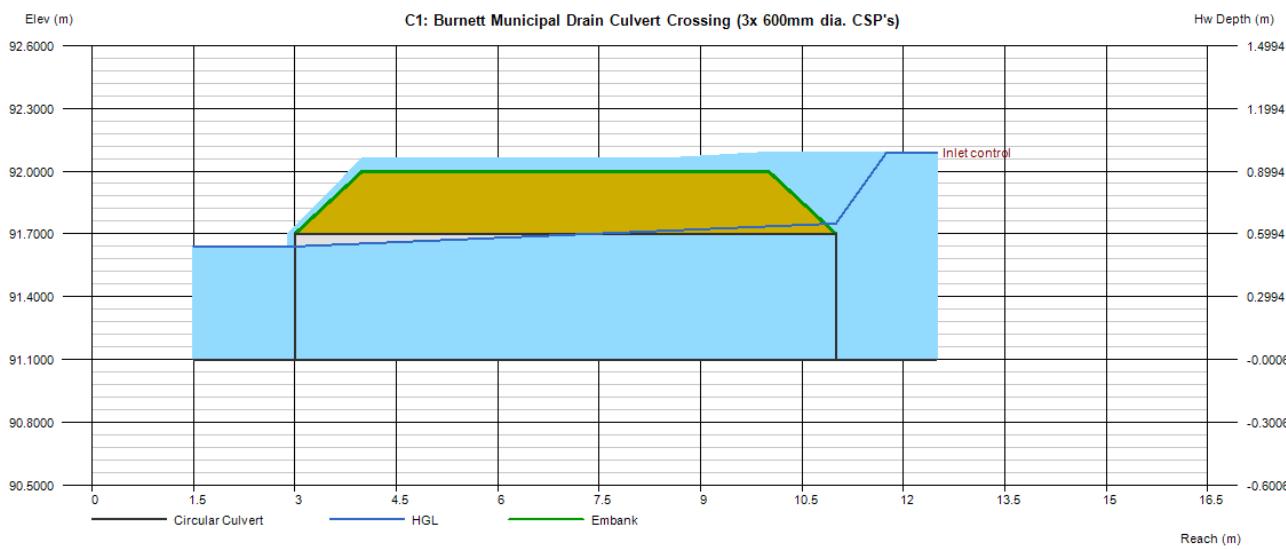
25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	=	91.1000	Calculations	
Pipe Length (m)	=	8.0000	Qmin (cms)	= 0.0000
Slope (%)	=	0.0076	Qmax (cms)	= 2.0000
Invert Elev Up (m)	=	91.1006	Tailwater Elev (m)	= (dc+D)/2
Rise (mm)	=	600.0		
Shape	=	Circular	Highlighted	
Span (mm)	=	600.0	Qtotals (cms)	= 1.9000
No. Barrels	=	3	Qpipe (cms)	= 1.6221
n-Value	=	0.016	Qovertop (cms)	= 0.2779
Culvert Type	=	Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 2.0174
Culvert Entrance	=	Projecting	Veloc Up (m/s)	= 1.9123
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 91.6400
Embankment			HGL Up (m)	= 91.7494
Top Elevation (m)	=	92.0000	Hw Elev (m)	= 92.0880
Top Width (m)	=	6.0000	Hw/D (m)	= 1.6457
Crest Width (m)	=	6.0000	Flow Regime	= Inlet Control



Culvert Report

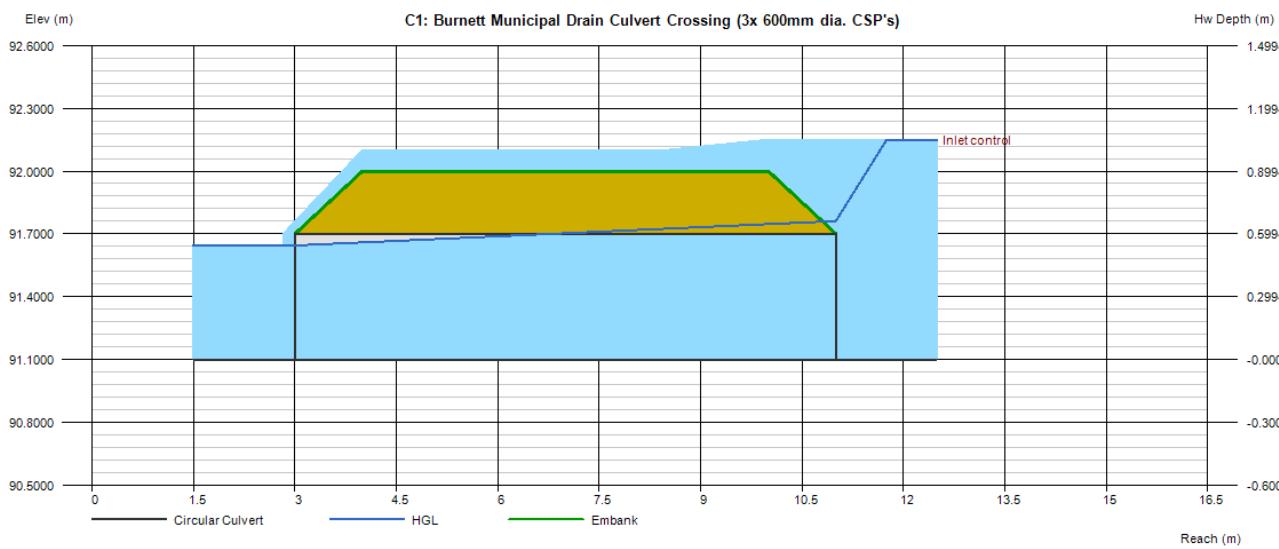
50-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000	Calculations	
Pipe Length (m)	= 8.0000	Qmin (cms)	= 2.0000
Slope (%)	= 0.0076	Qmax (cms)	= 3.0000
Invert Elev Up (m)	= 91.1006	Tailwater Elev (m)	= $(dc+D)/2$
Rise (mm)	= 600.0		
Shape	= Circular	Highlighted	
Span (mm)	= 600.0	Qtotal (cms)	= 2.3000
No. Barrels	= 3	Qpipe (cms)	= 1.6952
n-Value	= 0.016	Qovertop (cms)	= 0.6048
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 2.0950
Culvert Entrance	= Projecting	Veloc Up (m/s)	= 1.9985
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 91.6448
Embankment		HGL Up (m)	= 91.7612
Top Elevation (m)	= 92.0000	Hw Elev (m)	= 92.1492
Top Width (m)	= 6.0000	Hw/D (m)	= 1.7477
Crest Width (m)	= 6.0000	Flow Regime	= Inlet Control



Culvert Report

100-year 12-hour SCS Storm

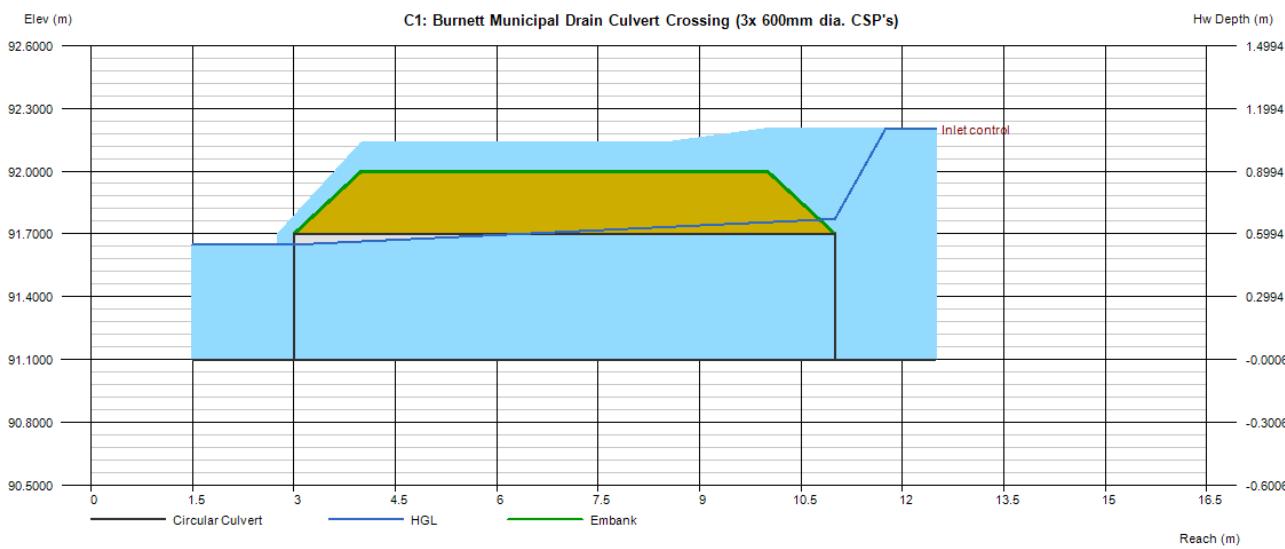
Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9
Embankment	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 2.0000
Qmax (cms)	= 3.0000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 2.7000
Qpipe (cms)	= 1.7556
Qovertop (cms)	= 0.9444
Veloc Dn (m/s)	= 2.1591
Veloc Up (m/s)	= 2.0697
HGL Dn (m)	= 91.6486
HGL Up (m)	= 91.7709
Hw Elev (m)	= 92.2017
Hw/D (m)	= 1.8352
Flow Regime	= Inlet Control



Culvert Report

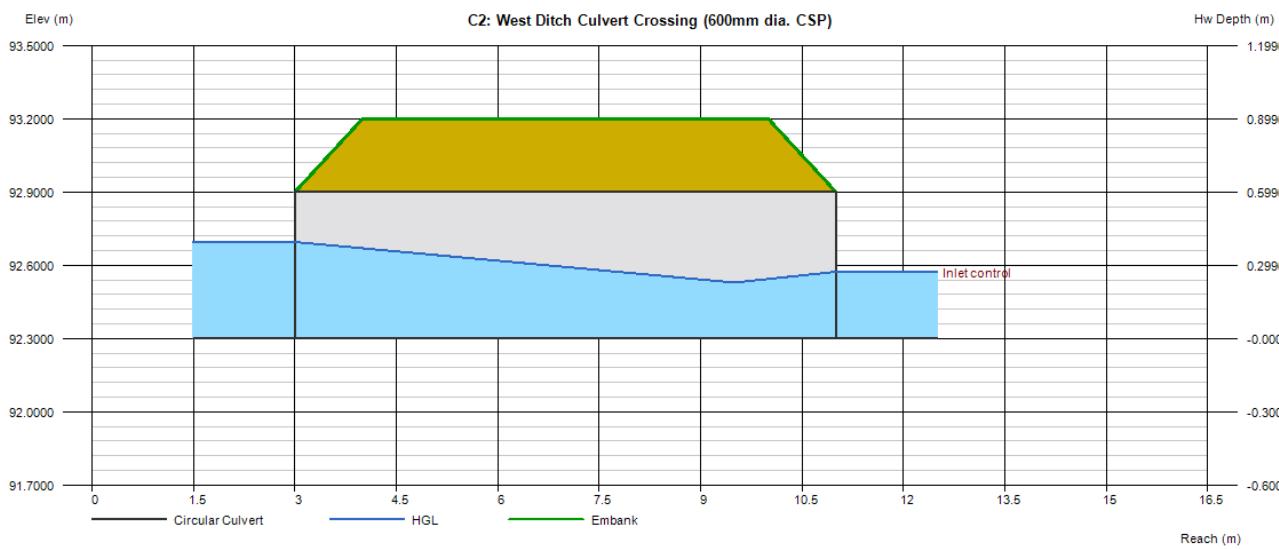
2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000	Calculations	
Pipe Length (m)	= 8.0000	Qmin (cms)	= 0.0000
Slope (%)	= 0.0049	Qmax (cms)	= 0.4000
Invert Elev Up (m)	= 92.3004	Tailwater Elev (m)	= $(dc+D)/2$
Rise (mm)	= 600.0		
Shape	= Circular	Highlighted	
Span (mm)	= 600.0	Qtotal (cms)	= 0.0900
No. Barrels	= 1	Qpipe (cms)	= 0.0900
n-Value	= 0.016	Qovertop (cms)	= 0.0000
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 0.4555
Culvert Entrance	= Projecting	Veloc Up (m/s)	= 1.1656
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 92.6953
Embankment		HGL Up (m)	= 92.4910
Top Elevation (m)	= 93.2000	Hw Elev (m)	= 92.5734
Top Width (m)	= 6.0000	Hw/D (m)	= 0.4550
Crest Width (m)	= 6.0000	Flow Regime	= Inlet Control



Culvert Report

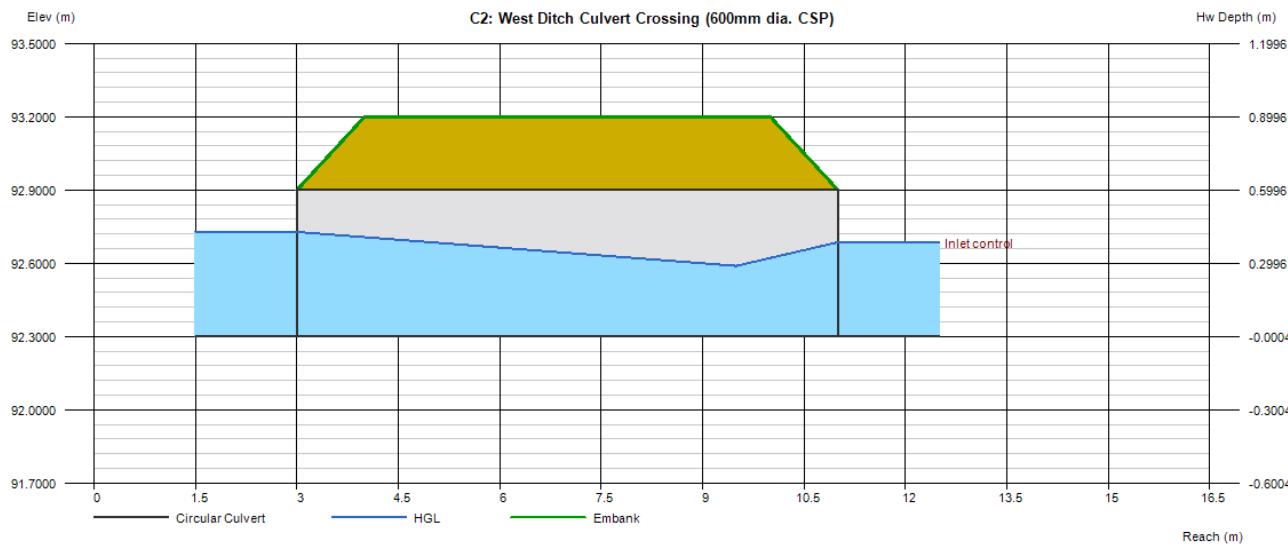
5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	=	92.3000	Calculations	
Pipe Length (m)	=	8.0000	Qmin (cms)	= 0.0000
Slope (%)	=	0.0049	Qmax (cms)	= 0.4000
Invert Elev Up (m)	=	92.3004	Tailwater Elev (m)	= (dc+D)/2
Rise (mm)	=	600.0		
Shape	=	Circular	Highlighted	
Span (mm)	=	600.0	Qtotal (cms)	= 0.1600
No. Barrels	=	1	Qpipe (cms)	= 0.1600
n-Value	=	0.016	Qovertop (cms)	= 0.0000
Culvert Type	=	Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 0.7407
Culvert Entrance	=	Projecting	Veloc Up (m/s)	= 1.3844
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 92.7284
Embankment			HGL Up (m)	= 92.5573
Top Elevation (m)	=	93.2000	Hw Elev (m)	= 92.6860
Top Width (m)	=	6.0000	Hw/D (m)	= 0.6428
Crest Width (m)	=	6.0000	Flow Regime	= Inlet Control



Culvert Report

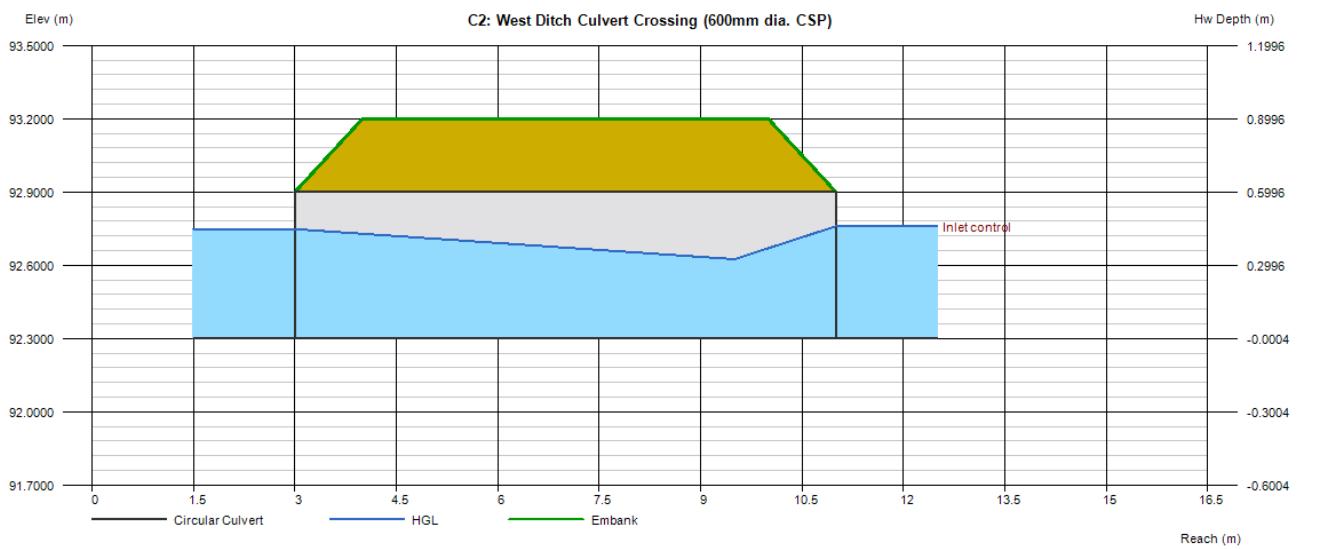
10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000	Calculations	
Pipe Length (m)	= 8.0000	Qmin (cms)	= 0.0000
Slope (%)	= 0.0049	Qmax (cms)	= 0.4000
Invert Elev Up (m)	= 92.3004	Tailwater Elev (m)	= $(dc+D)/2$
Rise (mm)	= 600.0		
Shape	= Circular	Highlighted	
Span (mm)	= 600.0	Qtotal (cms)	= 0.2100
No. Barrels	= 1	Qpipe (cms)	= 0.2100
n-Value	= 0.016	Qovertop (cms)	= 0.0000
Culvert Type	= Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 0.9273
Culvert Entrance	= Projecting	Veloc Up (m/s)	= 1.5101
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 92.7481
Embankment		HGL Up (m)	= 92.5966
Top Elevation (m)	= 93.2000	Hw Elev (m)	= 92.7595
Top Width (m)	= 6.0000	Hw/D (m)	= 0.7652
Crest Width (m)	= 6.0000	Flow Regime	= Inlet Control



Culvert Report

25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

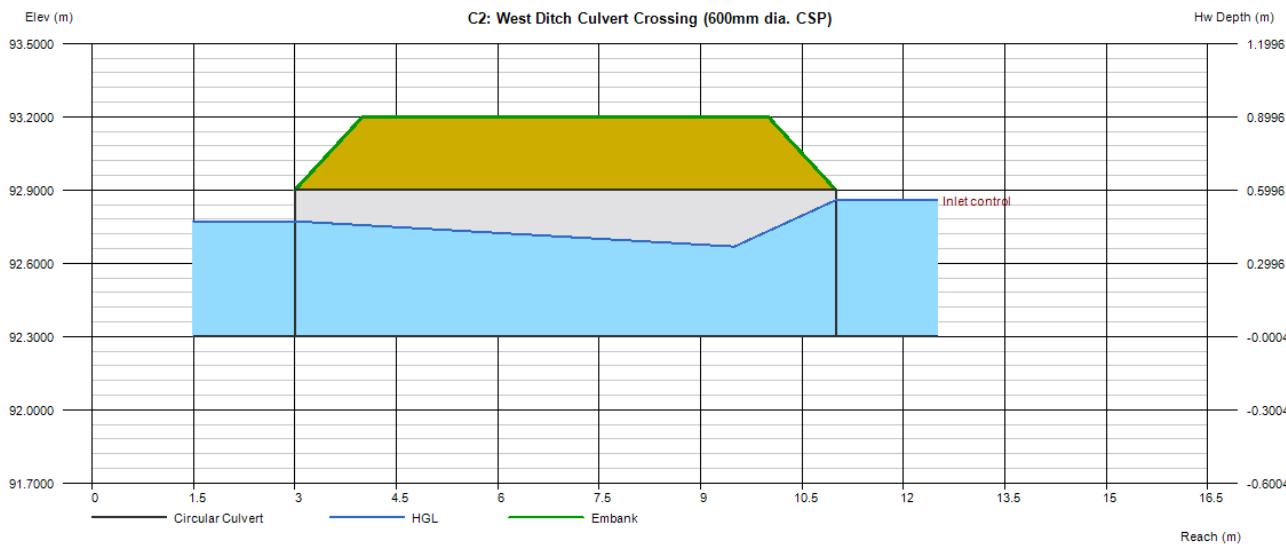
C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0049
Invert Elev Up (m)	= 92.3004
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 1
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 93.2000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.4000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtotals (cms)	= 0.2800
Qpipe (cms)	= 0.2800
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.1731
Veloc Up (m/s)	= 1.6682
HGL Dn (m)	= 92.7722
HGL Up (m)	= 92.6447
Hw Elev (m)	= 92.8585
Hw/D (m)	= 0.9302
Flow Regime	= Inlet Control



Culvert Report

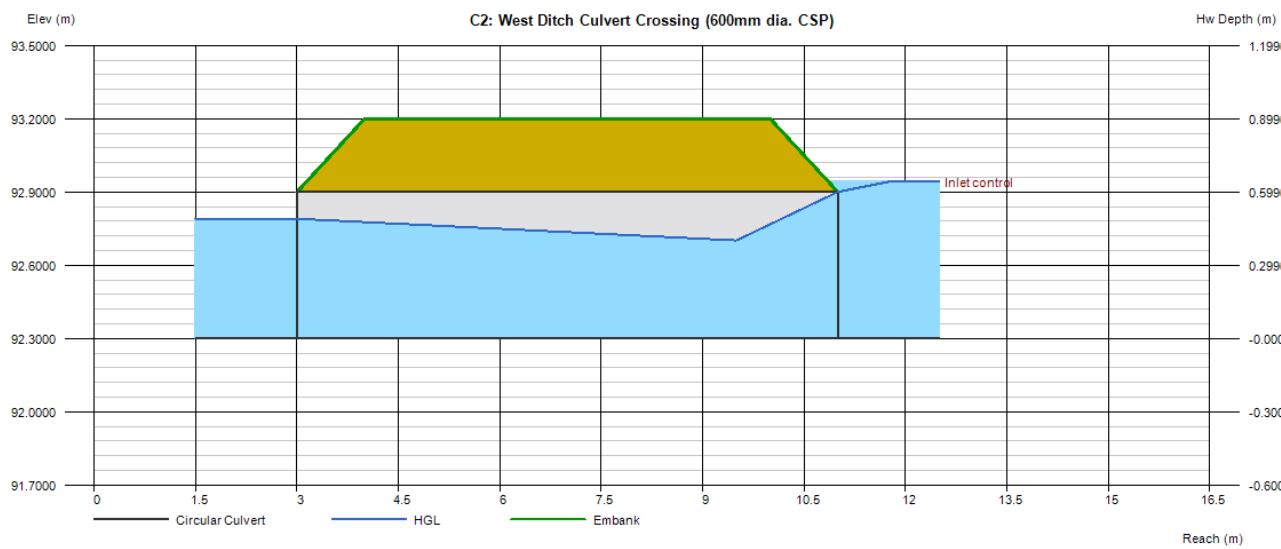
50-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	=	92.3000	Calculations	
Pipe Length (m)	=	8.0000	Qmin (cms)	= 0.0000
Slope (%)	=	0.0049	Qmax (cms)	= 0.4000
Invert Elev Up (m)	=	92.3004	Tailwater Elev (m)	= $(dc+D)/2$
Rise (mm)	=	600.0		
Shape	=	Circular	Highlighted	
Span (mm)	=	600.0	Qtotal (cms)	= 0.3400
No. Barrels	=	1	Qpipe (cms)	= 0.3400
n-Value	=	0.016	Qovertop (cms)	= 0.0000
Culvert Type	=	Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 1.3741
Culvert Entrance	=	Projecting	Veloc Up (m/s)	= 1.7957
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 92.7905
Embankment			HGL Up (m)	= 92.6813
Top Elevation (m)	=	93.2000	Hw Elev (m)	= 92.9420
Top Width (m)	=	6.0000	Hw/D (m)	= 1.0693
Crest Width (m)	=	6.0000	Flow Regime	= Inlet Control



Culvert Report

100-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

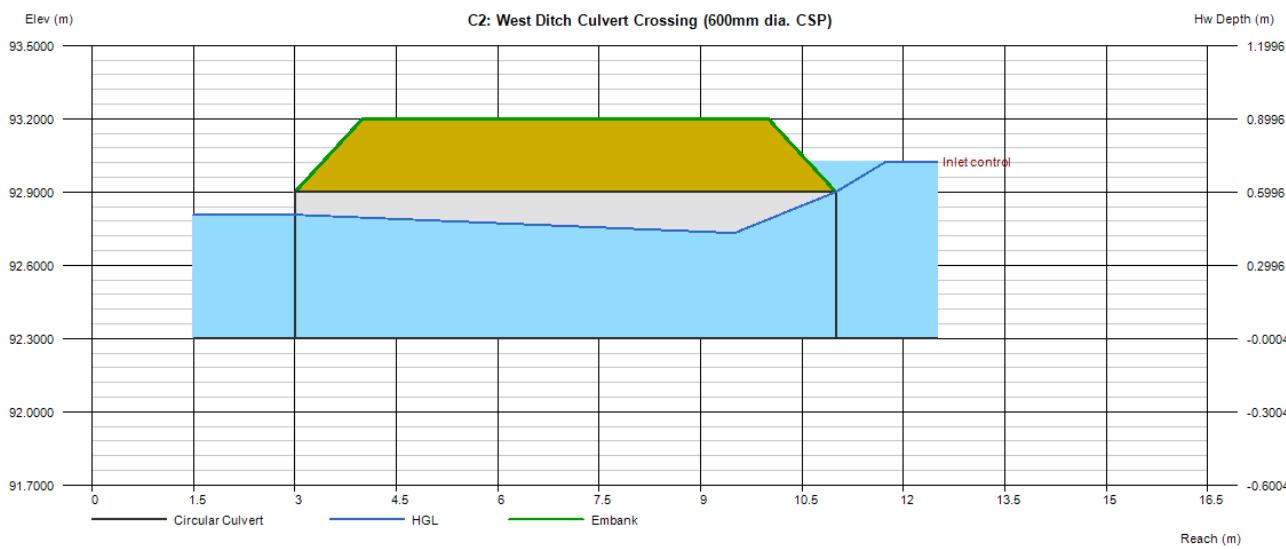
C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0049
Invert Elev Up (m)	= 92.3004
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 1
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 93.2000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.4000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtotals (cms)	= 0.4000
Qpipe (cms)	= 0.4000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.5694
Veloc Up (m/s)	= 1.9217
HGL Dn (m)	= 92.8071
HGL Up (m)	= 92.7145
Hw Elev (m)	= 93.0256
Hw/D (m)	= 1.2087
Flow Regime	= Inlet Control



Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

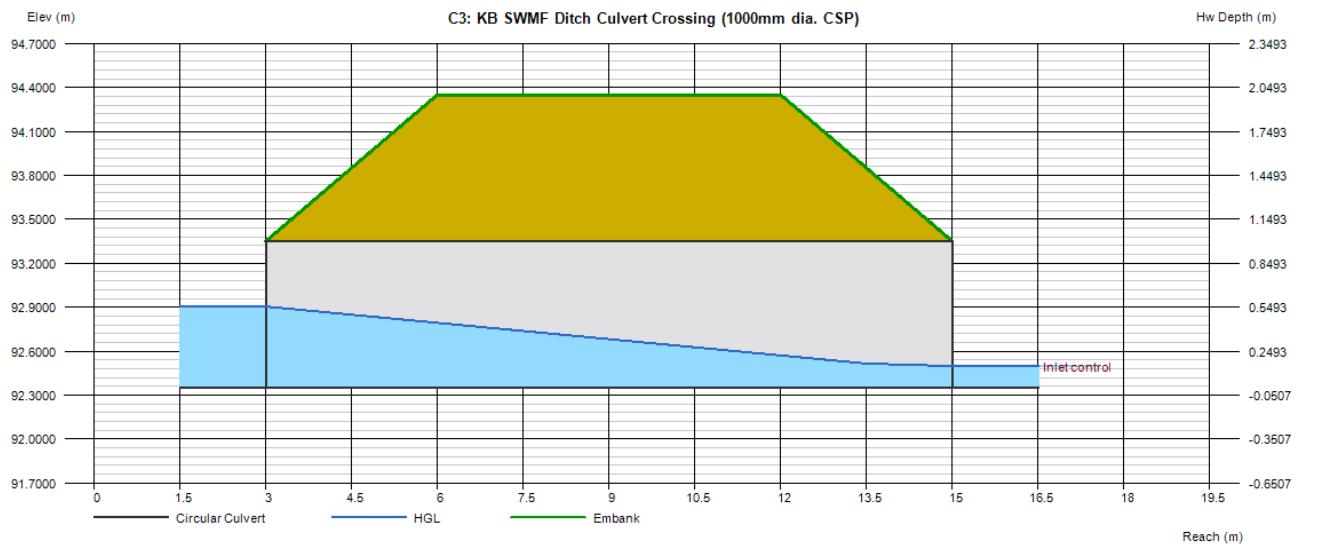
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 0.0400
Qpipe (cms)	= 0.0400
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.0894
Veloc Up (m/s)	= 0.8567
HGL Dn (m)	= 92.9048
HGL Up (m)	= 92.4602
Hw Elev (m)	= 92.4986
Hw/D (m)	= 0.1479
Flow Regime	= Inlet Control



Culvert Report

5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

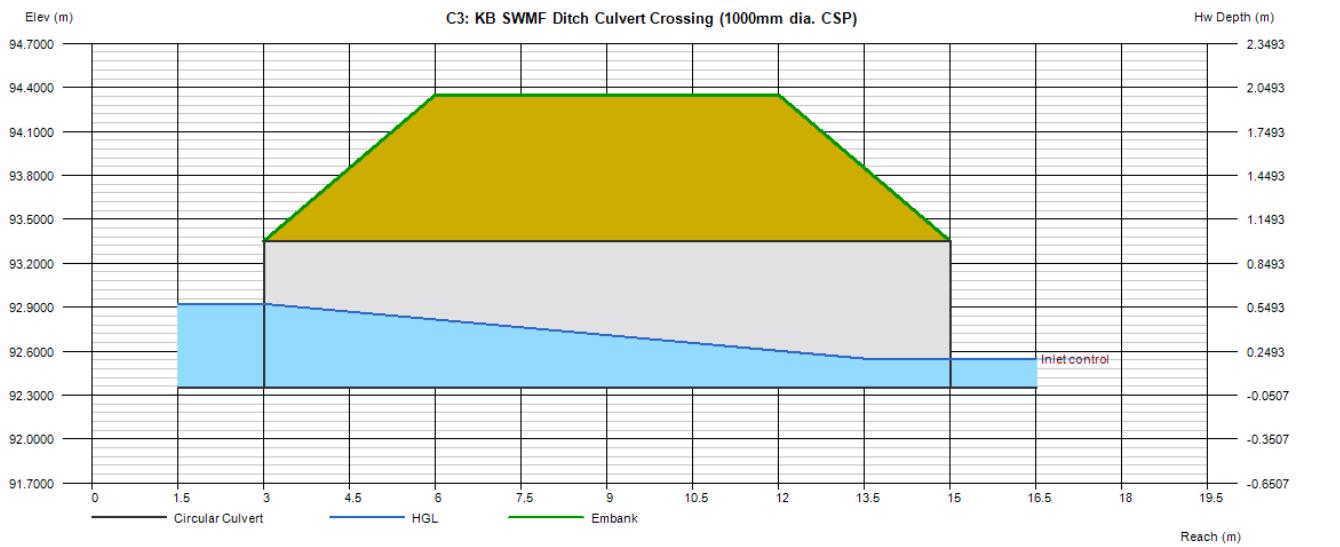
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 0.0700
Qpipe (cms)	= 0.0700
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.1505
Veloc Up (m/s)	= 0.9911
HGL Dn (m)	= 92.9227
HGL Up (m)	= 92.4961
Hw Elev (m)	= 92.5484
Hw/D (m)	= 0.1977
Flow Regime	= Inlet Control



Culvert Report

10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

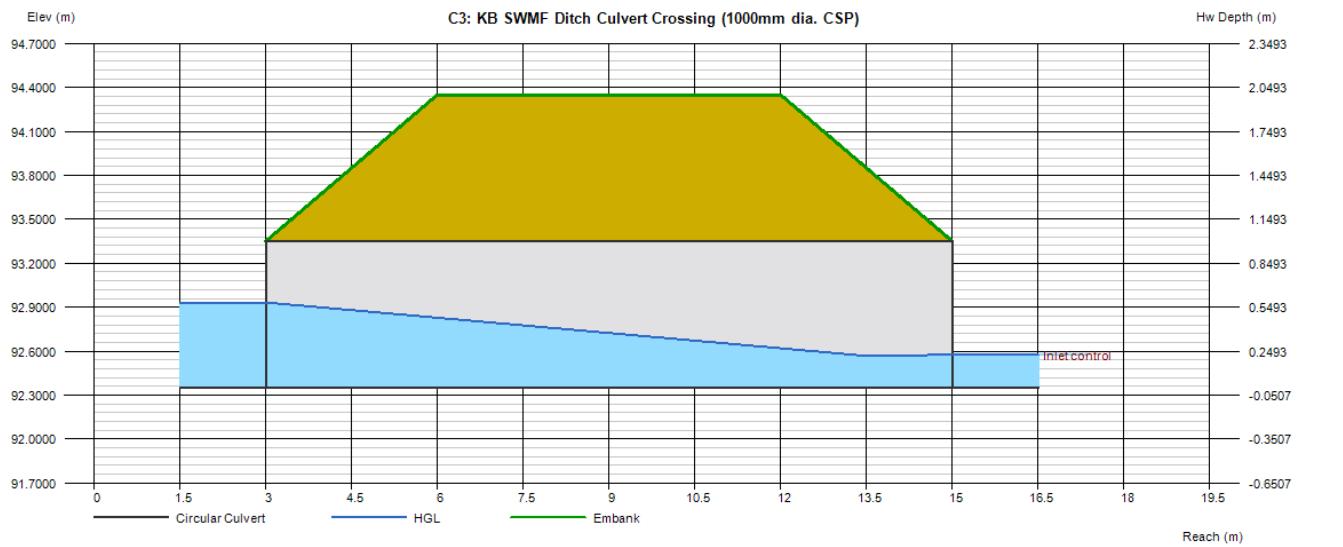
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 0.0900
Qpipe (cms)	= 0.0900
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.1895
Veloc Up (m/s)	= 1.0598
HGL Dn (m)	= 92.9326
HGL Up (m)	= 92.5159
Hw Elev (m)	= 92.5763
Hw/D (m)	= 0.2256
Flow Regime	= Inlet Control



Culvert Report

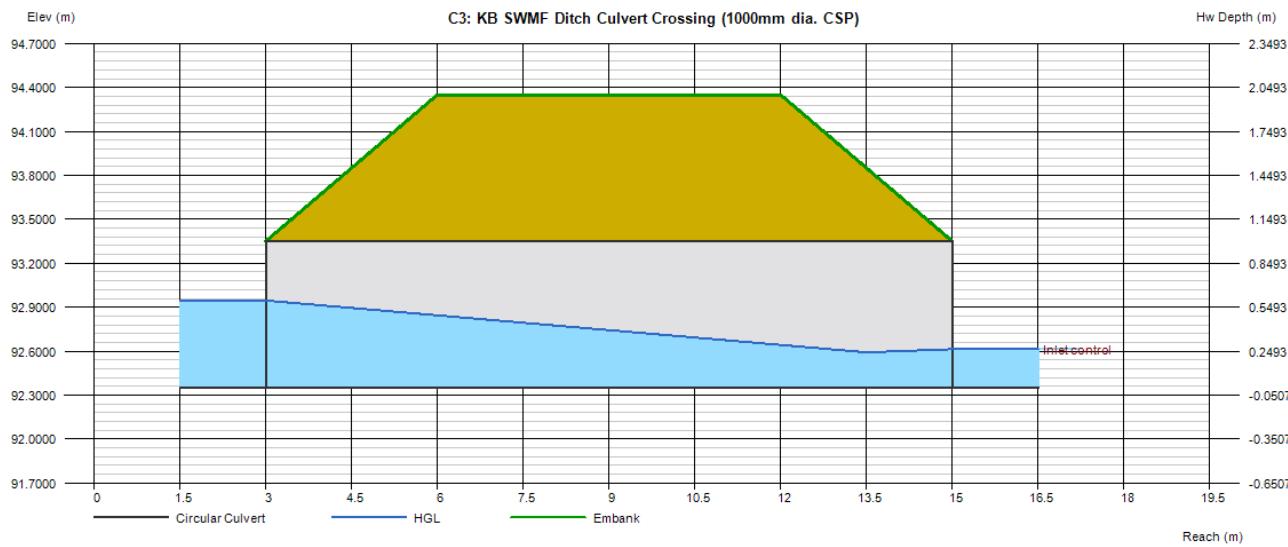
25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	=	92.3500	Calculations	
Pipe Length (m)	=	12.0000	Qmin (cms)	= 0.0000
Slope (%)	=	0.0058	Qmax (cms)	= 0.2000
Invert Elev Up (m)	=	92.3507	Tailwater Elev (m)	= (dc+D)/2
Rise (mm)	=	1000.0		
Shape	=	Circular	Highlighted	
Span (mm)	=	1000.0	Qtotal (cms)	= 0.1200
No. Barrels	=	1	Qpipe (cms)	= 0.1200
n-Value	=	0.020	Qovertop (cms)	= 0.0000
Culvert Type	=	Circular Corrugate Metal Pipe	Veloc Dn (m/s)	= 0.2461
Culvert Entrance	=	Projecting	Veloc Up (m/s)	= 1.1449
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9	HGL Dn (m)	= 92.9456
Embankment			HGL Up (m)	= 92.5419
Top Elevation (m)	=	94.3500	Hw Elev (m)	= 92.6136
Top Width (m)	=	6.0000	Hw/D (m)	= 0.2629
Crest Width (m)	=	6.0000	Flow Regime	= Inlet Control



Culvert Report

50-year 12-hour SCS Storm

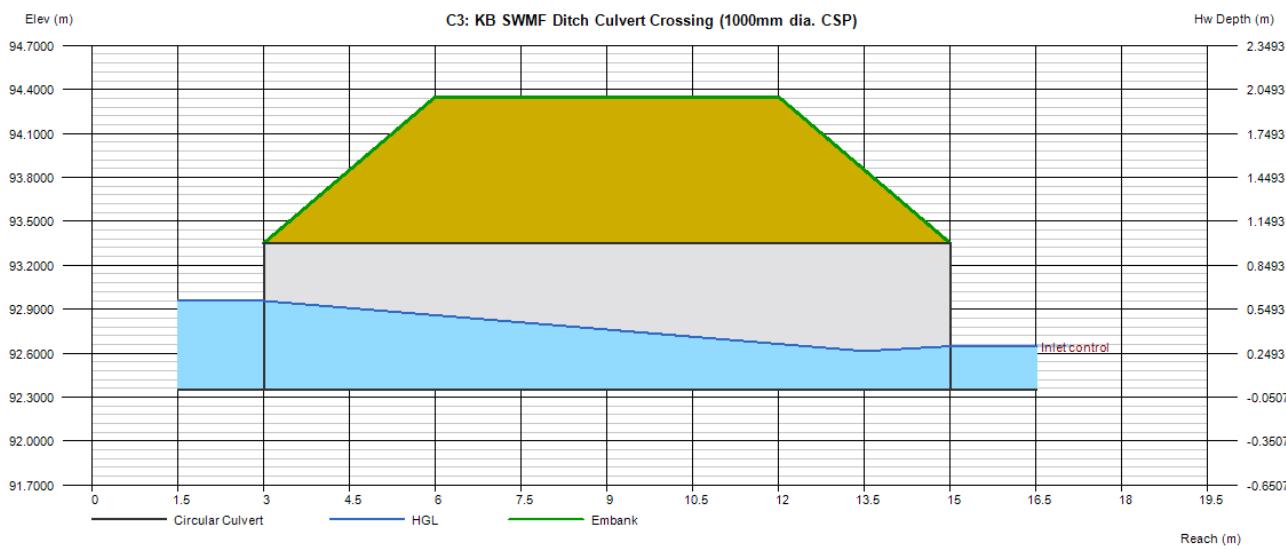
Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9
Embankment	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 0.1500
Qpipe (cms)	= 0.1500
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.3006
Veloc Up (m/s)	= 1.2156
HGL Dn (m)	= 92.9571
HGL Up (m)	= 92.5650
Hw Elev (m)	= 92.6472
Hw/D (m)	= 0.2965
Flow Regime	= Inlet Control



Culvert Report

100-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

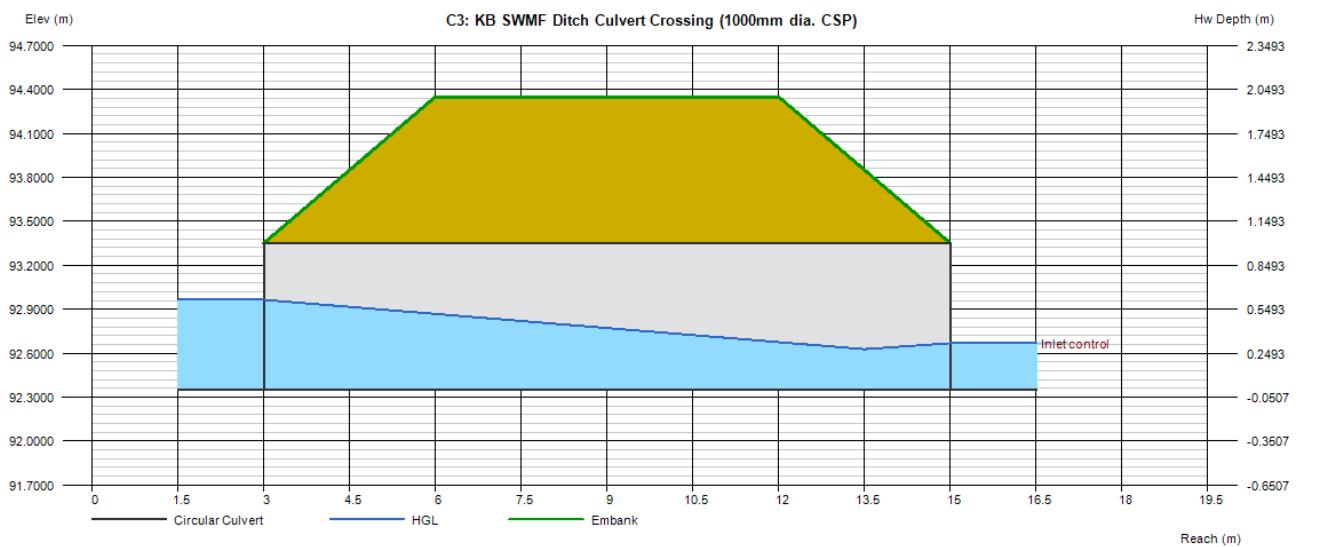
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 0.1700
Qpipe (cms)	= 0.1700
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.3360
Veloc Up (m/s)	= 1.2582
HGL Dn (m)	= 92.9642
HGL Up (m)	= 92.5791
Hw Elev (m)	= 92.6682
Hw/D (m)	= 0.3175
Flow Regime	= Inlet Control



Culvert Report

Full Flow Capacity (HW/D = 1.0)

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

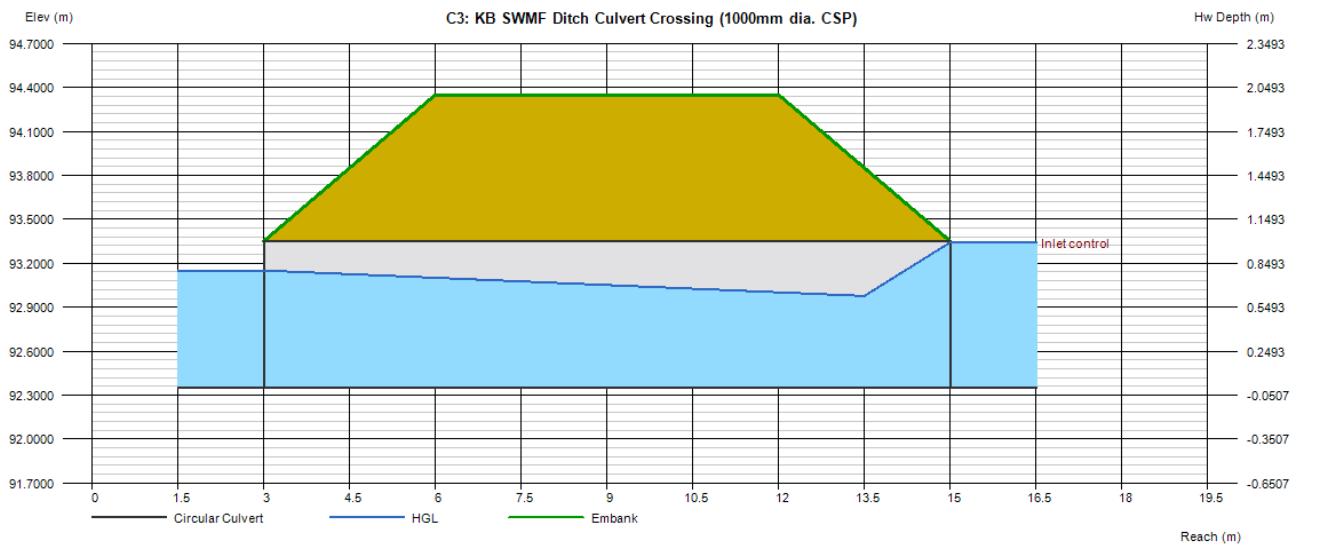
C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

Embankment

Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

Calculations	
Qmin (cms)	= 0.0000
Qmax (cms)	= 1.3000
Tailwater Elev (m)	= $(dc+D)/2$
Highlighted	
Qtot (cms)	= 1.1000
Qpipe (cms)	= 1.1000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.6315
Veloc Up (m/s)	= 2.2284
HGL Dn (m)	= 93.1508
HGL Up (m)	= 92.9523
Hw Elev (m)	= 93.3430
Hw/D (m)	= 0.9923
Flow Regime	= Inlet Control



**South Nepean Collector Phase 2: Culvert Crossings
Ditch Capacities (Manning's Equation)**

C1: Burnett Municipal Drain		
Parameter	Units	Value
Depth	m	0.60
Bottom Width	m	3.00
Side slope (L)	1 to X	2.0
Side slope (R)	1 to X	4.0
Top Width (L)	m	1.20
Top Width (R)	m	2.40
Area	m^2	2.880
Perimeter	m	6.82
R=A/P	m	0.42
n		0.035
Slope	m/m	0.007
Q_{\max}	m^3/s	3.877
V_{\max}	m/s	1.346

Trapezoidal Channel (different side slopes)

C2: West Ditch		
Parameter	Units	Value
Depth	m	0.30
Side slope (L)	1 to X	3.0
Side slope (R)	1 to X	6.0
Top Width (L)	m	0.90
Top Width (R)	m	1.80
Area	m^2	0.405
Perimeter	m	2.77
R=A/P	m	0.15
n		0.035
Slope	m/m	0.005
Q_{\max}	m^3/s	0.227
V_{\max}	m/s	0.560

V-bottom ditch (different side slopes)

C3: KB SWMF Ditch		
Parameter	Units	Value
Depth	m	2.00
Side slope (L)	1 to X	2.5
Side slope (R)	1 to X	2.5
Top Width (L)	m	5.00
Top Width (R)	m	5.00
Area	m^2	10.000
Perimeter	m	10.77
R=A/P	m	0.93
n		0.035
Slope	m/m	0.006
Q_{\max}	m^3/s	21.063
V_{\max}	m/s	2.106

V-bottom ditch (same side slopes)

CORPORATION OF THE TOWNSHIP OF NEPEAN

By-Law No. 107-68

(The Drainage Act, 1962-63, Section 27, Form 4)

A By-law to provide for Drainage Work in the Township of Nepean, in the County of Carleton and for borrowing on the credit of the Municipality the sum of Five Thousand, Six Hundred and Thirty-Three (\$5,633.00) Dollars, for completing the same.

- - - - -

WHEREAS the requisite number of owners, as shown by the last revised Assessment Roll, of the property hereinafter set forth requiring drainage have petitioned the Council of the said Township of Nepean praying that the following lands and roads may be drained by a drainage works.

Plan and Profile:

The accompanying plan, profile and specifications dated October 16th, 1968 are to form a part of this report. The plan will show the area of the watershed, and the drain. The profile and specifications will show the extent of work, the bench marks, grades and disposal of materials, etc.

Recommendation:

It is our recommendation that this drain be reconstructed from station 0 + 00 to station 15 + 31 where it enters the present road ditch. At this location the present road ditch will carry the water to the south side of the forced road where construction is to continue from station 19 + 98. The drain then flows east to the present railway culvert which is of sufficient size and the drain has been designed to flow through it at its present elevation. From this location the drain still flows easterly to the Township Road between lots 15 and 16, Concession 3 at station 43 + 70. The present culvert through this road is to be lowered to the designed grade for this location and from here the drain flows east to its outlet in the Jock River at station 86 + 25.

AND WHEREAS the Council has procured a report made by Alex J. Graham, C.E., hereto attached and marked Schedule "C" of By-Law No. 107-68.

AND WHEREAS the Council is of opinion that the drainage of the area described is desirable:

THEREFORE the Council of the Township of Nepean pursuant to The Drainage Act, 1962-63, enacts as follows:

1. The report is hereby adopted, and the drainage works as therein indicated and set forth are hereby authorized and shall be completed in accordance therewith.

2. The Corporation of the Township of Nepean may borrow on the credit or the Corporation the sum of \$5,633.00, being the funds necessary for the drainage works, not otherwise provided for, provided that such sum shall be reduced by the amount of grants and commuted payments with respect to the land and roads assessed, and may issue Debentures of the Corporation to that amount in sums of not less than Fifty Dollars each, and payable within ten years from the date of the said Debentures, with interest at the rate of 8 per centum per annum, that is to say annually with coupons.

3. For paying the sum of \$2,914.65 the amount charged against such lands and roads for benefit, and the sum of \$2,718.35 the amount charged against such lands and roads for outlet liability, and the sum of \$Nil the amount charged against such lands and roads for injuring liability, and the sum of Nil the amount charged against lands and roads for improvement, apart from lands and roads belonging to or controlled by the Municipality, and for covering interest thereon for ten years at the rate of 8 per centum per annum, the following total special rates, over and above all other rates, shall be assessed, levied, and collected (in the same manner and at the same time as other taxes are levied and collected) upon and from the under-mentioned parcels of land and parts of parcels and roads, and the amount of the total special rates and interest against each parcel or part of parcel respectively shall be divided into ten equal parts and one such part shall be assessed, levied and collected as aforesaid, in each year, for ten years after the passing of this by-law during which the Debentures have to run, provided that no greater amount shall be levied than is required after taking into account and crediting the amount of grants under subsection 3 of section 64 of The Drainage Act, 1962-63, the amount of moneys paid under a by-law passed under subsection 4 of section 40 of that Act and commuted payments with respect to lands and roads assessed.

3.

4. For paying the sum of \$590.43 the amount assessed against such roads and lands of the Municipality, and for covering interest thereon for ten years at the rate of 8 per cent per annum, a special rate, sufficient to produce the required yearly amount therefor, shall, over and above all other rates, be levied and collected (in the same manner and at the same time as other taxes are levied and collected) upon and from the whole ratesable property in the Township of Nepean in each year for ten years, after the passing of this by-law, during which the Debentures have to run.

5. This by-law comes into force on the passing thereof and may be cited as the "Burnett Municipal Drain By-Law".

First Reading December 9th, 1968.

Second Reading December 9th, 1968.

Third Reading January 6th, 1969.

Enacted this 6th day of January, 1969.

D. A. Moodie

D. A. Moodie

Reeve

J. E. Robbs

J. E. Robbs

Clerk

TOWNSHIP OF NEPEAN
BURNETT MUNICIPAL DRAIN
SCHEDULE OF ASSESSMENT

SCHEDULE OF ASSESSMENT

No.	Lot	Name	Acreage Assessed	Main Drain Benefit	Main Drain Outlet	Total	Esti- mated Grant	Balance Payable	To Cover Interest For 10 yrs. at 8%	Total Special Rate	Annual Assessment During Each Year For 10 years
S $\frac{1}{2}$	Lot 19	Don Fraser	12		198.18	198.18	132.13	66.06	29.36	98.11	9.84
N $\frac{1}{2}$ S. pt.	Lot 18	Lorne Burnett	43	549.77	517.25	1,067.02	711.31	355.71	174.29	550.00	53.00
S Pt.	Lot 18	Robert Mowat	6		74.28	74.28	49.54	24.74	12.13	36.87	3.69
Pt. W $\frac{3}{4}$ / N $\frac{1}{2}$	Lot 17	Mrs. E. Monk	8	96.25	99.84	196.09	130.72	65.37	30.05	97.42	9.74
Pt. W $\frac{3}{4}$ / N $\frac{1}{2}$	Lot 17	Sala & Ricciuto	11	76.23	136.18	212.41	141.62	70.79	34.69	105.48	10.55
S $\frac{1}{2}$	Lot 17	Carl Fraser	20	160.93	149.37	310.50	206.99	103.51	50.72	154.23	15.42
N $\frac{1}{2}$ & Pt S $\frac{2}{3}$ L16	Patrick Moloughney	15	249.10	223.80	472.90	315.27	157.63	77.24	234.67	23.49	
S $\frac{2}{3}$ Less 20ac L16	Fergus Houlshen	49	291.15	492.36	872.51	582.30	291.21	122.69	433.90	43.39	
E $\frac{2}{3}$ N $\frac{1}{2}$	Lot 15	Carl Fraser	19	204.05	107.16	311.21	207.47	103.74	50.83	154.57	15.46
S $\frac{1}{2}$	Lot 15	Wm. Clark	18	204.05	73.98	278.03	185.35	92.68	45.41	138.09	13.81
Pt. E $\frac{7}{15}$ N $\frac{3}{8}$ L.14	Mrs. H. Houlshen	16	233.70	50.72	284.42	189.63	94.79	46.45	141.24	14.12	
S. 5/8	Lot 14	John Houlshen	13	294.52	34.71	329.23	219.49	109.74	53.77	163.51	16.35
Pt. E $\frac{7}{15}$	Lot 13	Kelvin Burnett	9	208.10	--	208.10	138.76	69.34	33.98	103.32	10.33
Forced Road in Concession 3			--	122.82	251.64	377.36	--	--	--	--	--
Township Road Between Lots 15 & 16			--	76.23	136.94	213.07	--	--	--	--	--
Canadian National Railways			--	57.75	168.94	226.69	--	--	--	--	--
Totals				\$2,912.65	\$2,718.35	\$5,633.00	\$3,210.58	\$1,605.30	\$784.71	\$2,391.91	\$239.19

ENGINEER'S REPORT

BURNETT MUNICIPAL DRAIN

TOWNSHIP OF NEPEAN

Graham, Berman and Associates Ltd.,
Consulting Engineers,
Ottawa 3, Ontario.

October 16th, 1968.

Schedule C By-Law 107-68

October 16th, 1968.

The Reeve and Members of Council,
Township of Napanee,
3895 Richmond Road,
Ottawa 6, Ontario.

Gentlemen:

In answer to the prayer of over half the property owners concerned, requesting that the Burnett Award Ditch be reconstructed to relieve the present flooding conditions and provide adequate outlet for tile drains. This drain is to be reconstructed under section 3 of the Drainage Act 1962-63 and by reason of a motion passed in Council, I have caused a survey to be made, prepared plan, profile and schedule of assessment, and beg to report as follows:

The drainage works shall be known as the Burnett Municipal Drain.

Plan and Profile:

The accompanying plan, profile and specifications dated October 16th, 1968 are to form a part of this report. The plan will show the area of the watershed, and the drain. The profile and specifications will show the extent of work, the bench marks, grades and disposal of materials, etc.

Recommendation:

It is our recommendation that this drain be reconstructed from station 0 + 00 to station 15 + 31 where it enters the present road ditch. At this location the present road ditch will carry the water to the south side of the forced road where construction is to continue from station 19 + 98. The drain then flows east to the present railway culvert which is of sufficient size and the drain has been designed to flow through it at its present elevation. From this location the drain still flows easterly to the Township Road between lots 15 and 16, Concession 3 at station 43 + 70. The present culvert through this road is to be lowered to the designed grade for this location and from here the drain flows east to its outlet in the Jack River at station 86 + 25.

Farm Crossings:

A Severance Allowance is made in this report under Section 8 (6) of the Drainage Act 1962-63, culvert dimensions are stated in the following Schedule of Allowance.

Township of Napanee:

Concession 3:

N ₂	Lot 16	Lorne Burnett	1-24 ^{11/16} " x 16' x 16 gauge CWP	\$31.52
S ₃	Lot 18	Lorne Burnett	1-24 ^{11/16} " x 16' x 16 gauge CWP	\$1.52
Pt. N3/4				
N ₂	Lot 17	Mrs. E. Monk	1-24 ^{11/16} " x 16' x 16 gauge CWP	\$1.52
W3/4 N ₂	Lot 17	see a previous	1-24 ^{11/16} " x 16' x 16 gauge CWP	\$1.52
S ₂	Lot 17	Clark Fraser	1-24 ^{11/16} " x 16' x 16 gauge CWP	\$1.52
W 1/3	Lot 16	Patrick McLaughney	1-30 ^{11/16} " x 16' x 16 gauge CWP	\$2.08
PT S2/3	Lot 16	Vergus Houleben	1-30 ^{11/16} " x 16' x 16 gauge CWP	\$2.08
E2/3 N ₂	Lot 15	Carl Fraser	1-36 ^{11/16} " x 16' x 16 gauge CWP	104.88
S ₂	Lot 15	Mr. Clark	1-36 ^{11/16} " x 16' x 16 gauge CWP	104.88

Pt E7/15	Lot 14	Mrs. H. Houlehen	1-36" x 16' x 16 gauge CWP \$104.88
S 5/8	Lot 14	John Houlehen	1-36" x 16' x 16 gauge CWP 104.88
E Pt	Lot 13	Kelvin Burnett	1-36" x 16' x 16 gauge CWP 104.88
Total			<u>\$1,116.16</u>

These allowances will, in my opinion, adequately compensate the above property owners for the cost and installation of the above culverts.

Road Bridges and Culverts:

The present 24" x 18' CWP through the forced road in Concession 3 is sufficient in size, and the drain has been designed to flow through it at its present grade.

The present 30" x 26' CWP through the Township Road between lots 15 and 16, Concession 3 is sufficient in size and requires only to be lowered to the designed grade at this location.

Estimated Costs \$203.00

These estimated costs have not been included in the Estimated cost of construction as it is expected that the Township of Mono's Road Department will accept this responsibility as part of their maintenance program.

Railroad Culvert:

The present culvert through the Canadian National Railway which consists of a 24" CWP on the south side, and a 2' x 2.75' timber culvert on the north side of the tracks is sufficient in size, and the drain has been designed to flow through at the present grade.

Land Damage:

The amounts shown in the following allowances will, in my opinion, adequately compensate the property owners indicated for land or crop damage (if any) under Section 8 (1) of the Drainage Act 1960-63.

N ₂	Lot 18	Lorne Burnett	366.00
S ₁ ₂	Lot 18	Lorne Burnett	62.00
Pt W3/4			
N ₂	Lot 17	Mrs. E. Monk	9.00
W3/4 N ₂	Lot 17	Sala & Ricciuto	18.00
S ₁ ₂	Lot 17	Clark Fraser	38.00
N 1/3	Lot 16	Patrick MacCugnay	59.00
Pt S2/3	Lot 16	Fergus Houlehen	90.00
E2/3 N ₂	Lot 15	Carl Fraser	81.00
S ₁ ₂	Lot 15	Mr. Clark	81.00
Pt E7/15 &			
N 3/8	Lot 14	Mrs. Helen Houlehen	63.00
S 5/8	Lot 14	John Houlehen	63.00
Ept	Lot 13	Kelvin Burnett	96.00
Total			<u>3726.00</u>

Distribution of Costs:

The estimated costs for this construction are apportioned to the properties responsible for benefit and outlet as determined by their areas, locations, and run-off.

In my opinion, no liability for injury will exist because of this construction, and for this reason no injury liability column will appear in the Schedule of Assessment, this being in my estimation a fair distribution of costs.

Future Maintenance:

This drain will be maintained by the Township of Nepean, and the costs of such future maintenance will be apportioned to the property owners in the same proportioned to the property owners in the same proportions as in the attached Schedule of Assessment.

The "Estimated Costs" and incidental expenses are as follows:

Earth Excavation and Spreading of 1,992 c.y. @ .60¢	\$1,195.20
Hard Pan Excavation 282 c.y. @ \$1.60	451.20
Brushing	40.00
Fern Crossings	1,116.16
Land Damage Section 8 (1) of the Drainage Act 1962-63	726.00
Engineer's Fees and Expenses	884.44
Contingencies	470.00
Clerk's Fees	200.00
Printing and Publishing By-Law	125.00
Advertising and Letting Contract	85.00
Court of Revision	150.00
Supervision of Construction	190.00
<hr/>	
Total Estimated Cost	<u>\$5,633.00</u>

Grants:

Under Section 61, 63 and 64 of the Drainage Act 1962-63, a Provincial Grant of 33 1/3% of the cost of construction for Agricultural Lands may be obtained.

A subsequent grant by Federal A.R.D.A. through the Provincial grant administration media will further reduce Agricultural assessments by another one-third.

The assessments are then payable two-thirds by grant, and one-third by property assessment.

Respectfully submitted this 16th day of October, 1968.



GRAHAM, BERNAN AND ASSOCIATES LTD.

John S. Morrison
John S. Morrison,
Drainage Manager.

PAGE 1

SCHEDULE OF ASSESSMENT

JOB NO. 1862

DATE: Oct. 16/68

BURNSTI MUNICIPAL DRAIN

TOWNSHIP OF NEPEAN

Conc.	Lot	Name	Acreage Assessed	Main Drain Benefit	Main Drain Outlet	Total
3	S $\frac{1}{2}$ Lot 19	Don Fraser	13		198.18	198.18
	N $\frac{1}{2}$ Spt Lot 18	Lorne Burnett	43	549.77	517.25	1,067.02
	S Pt. Lot 18	Robert Mowat	6		74.28	74.28
	Pt W $\frac{3}{4}$ /4 N $\frac{1}{2}$ L.17	Mrs. E. Monk	8	96.25	99.84	196.09
	Pt W $\frac{3}{4}$ /4 N $\frac{1}{2}$ Lot 17	Sals & Ricciuto	11	76.23	136.18	212.41
	S $\frac{1}{2}$ Lot 17	Carl Fraser	20	160.93	149.37	310.50
	N $\frac{1}{2}$ /3 & Pt S $\frac{2}{3}$ /3 L.16	Patrick Maloughney	15	249.10	225.00	474.20
	S $\frac{2}{3}$ /3 Less 30ac L. 16	Fergus Houlahan	49	381.15	492.36	673.51
	E $\frac{2}{3}$ /3 N $\frac{1}{2}$ Lot 15	Carl Fraser	19	204.05	107.16	311.21
	S $\frac{1}{2}$ Lot 15	Mn. Clerk	18	204.05	73.96	278.03
	Pt E $\frac{7}{15}$ N $\frac{3}{8}$ L.14	Mrs. H. Houlahan	16	233.70	50.72	284.42
	S $\frac{5}{8}$ Lot 14	John Houlahan	13	294.52	34.71	329.23
	Pt E $\frac{1}{2}$ Lot 13	Kelvin Burnett	9	208.10		208.10
	Forced Road in Concession 3		122.82	254.64		377.36
	Township Road Between Lots 15 and 16		76.23	136.94		213.07
	Canadian National Railways		57.75	168.94		226.69
Totals			\$2,914.65	\$2,718.35		\$5,633.00

SUMMARY OF ASSESSMENT

LANDS:

Township Roads	\$ 590.43
Non Agricultural Lands	\$ 226.69
Land used for Agricultural Purposes	\$1,815.88

GRANTS ON AGRICULTURAL LANDS

Estimated Provincial Grant of 33 1/3%	\$1,605.29
Estimated Federal A.R.D.A. Grant of 33 1/3%	\$1,605.29
Estimated Property Assessments	\$1,605.30

SUMMARY OF ASSESSMENT

LANDS

Township roads	\$ 590.43
Non Agricultural Lands	622.78
Land used for Agricultural Purposes	\$1,619.79

Grants on Agricultural Lands

Estimated Provincial Grant of 33 1/3 %	\$1,539.93
Estimated Federal A.R.D.A. Grant of 33 1/3%	\$1,539.93
Estimated Property Assessments	\$1,539.93

SUPPLEMENT TO THE GENERAL SPECIFICATIONS

BURNETT MUNICIPAL DRAIN

TOWNSHIP OF NORPEAN

Graham, Berman and Associates Ltd.,
Consulting Engineers,
Ottawa 8, Ontario.

October 16th, 1968.

Meaning of Terms:

- "Municipal Council" - shall mean the Municipal Council of Napan Township.
- "Reeve" - shall mean the Reeve of Napan Township.
- "Engineer" - shall mean the Engineer in charge of the works, or his authorized representative.
- "Contractor" - shall mean the Contractor or Contractors performing the work, or their foremen on the grounds.

Details of Work:

The accompanying plan, profile and specifications dated October 16th, 1968 apply to and govern this construction.

8,625 lineal feet of open drain as follows:

Earth Excavation and Spreading of 1,992 c.y.

Hardpan Excavation and Spreading of 262 c.y.

Brushing:

A Severance Allowance under Section 6 (6) of the Drainage Act 1962-63 is aploied on this drain.

Center-line:

The present watercourse is to be the center-line of construction.

Important:

- (a) The Engineer must be notified at least 5 days prior to the starting of work on this contract.
- (b) Fences may be opened to allow construction equipment to go through them, and are to be closed immediately after that piece of equipment passes through, if fences are found to be left opened, they will be closed at the "Contractor's Expense".

GENERAL SPECIFICATIONS

Municipal Drainage (Open Drains)

Graham, Barman & Associates Ltd.
St. Thomas & Ottawa, Ontario.

1. These specifications are drawn up to cover the work as outlined in the Engineering Report on the drain, and as further outlined in the supplement to General Specifications.

Where there is any doubt as to the meaning or intention of the specifications, it shall be the Contractor's duty to obtain a ruling in writing from the Engineer before proceeding with the work.

2. Supply of Labour and Materials:

The Contractor shall supply all materials, labour, equipment, tools, machinery etc. for the full and proper completion of this work in accordance with the specifications, plan and profile. All work must be done in a neat and workmanlike manner, and to the satisfaction of the Engineer.

3. Roads to be kept open:

All roads, public and private are to be kept open and in passable condition during the continuance of this work.

4. Relief Ditches:

Should the Contractor deem it necessary to dig relief ditches on any part of this work, he shall do so and refill same entirely at his own expense.

5. Damages:

a) In case of damage being done to any farm or other property along the line of work by blasting or other operations, the Contractor shall be held liable for such damage.

b) The Owner or Occupant of the property on which the drainage works is located shall be responsible for the protection of all livestock on said property during construction, and shall be liable for any damages caused to or by such livestock.

c) The Contractor shall also rebuild and leave in as good condition as before construction, all fences removed in order to execute this work.

i) Fences crossing the drainage works must be closed immediately after the construction equipment has passed through them.

ii) Fences parallel to the drain must be replaced immediately after the work is completed in each section.

d) When hay or other produce is growing on lands adjacent to the proposed work, the Contractor must give the Owner ten days' notice

notice in writing to remove the same before he begins work on that portion, otherwise he shall be held liable for any damage caused.

6. Clearing and Grubbing:

a) Trees or brush growing in or on the banks of the drain are to be grubbed out clean. Trees having a stump diameter of 6" or over are to be cut into log lengths and piled clear of the spread materials. The stumps are to be piled in a corner of the field from which they were taken, adjacent to, but not closer than 4 feet to the edge of the drain. Branches and brush under 6" in diameter are to be piled on the excavated materials for disposal by the Property Owner.

b) At locations where the drain passes through brush or wood-lots, it is necessary that a strip of land be cleared along one side of the drain; the locations and dimensions will be given in the Supplement Specifications. However it is not intended that large trees growing within this specified area should be cut unless it is apparent that excessive damage will be caused to them. Stumps are not to exceed 1 ft. in height, and brush and branches are to be disposed of as in section (a) above.

Payment for this work will be made under the lump sum tender item "Cutting Brush".

7. Disposal of Materials:

The excavated materials shall be disposed of so as to do as little damage to lands and crops as possible.

a) Earth excavated from the drain is to be taken back a distance of 10 feet, leaving a clean berm 10 feet wide along the edge of the drain; and to be spread over the adjoining lands in such a manner that the elevation of the completed work does not exceed the elevation of the adjoining lands by more than 6" on cultivated lands and 12" on unworked or bush lands.

The completed work is to have a neat appearance and to be comparatively smooth.

b) Hard Pan and Rock excavated from the drain is to be taken back 4 feet from the edge of the drain and left in a pile so that it may be disposed of by the Property Owner.

c) Water Courses where necessary will be cut through the spread materials every 200 feet or in the low spots along the drain to allow surface drainage of the surrounding areas.

d) Re-location of Drain: At locations where the drain is to be removed from a road allowance, materials excavated from the new drain may be used to fill the road ditch in such a manner so as to allow the water from the road to enter the new drain. Excess materials are to be spread on the adjoining lands as above.

e) Straightening: At locations where straightening occurs the upper end of the old water course is to be filled level to the shoulders for a distance of 20 feet, and the excavated material put in the old drain when the intervening distance does not exceed 100 feet. Where the distance exceeds 100, the shoulders / are

are to be pushed into the bottom of the remainder of the abandoned watercourse, and so shaped that the water will run out of the low end, and in such a manner as to allow the Property Owner to travel through the drain and gradually reclaim the land.

f) Boulders: All boulders having a cubic content of 1 cubic foot or more are to be neatly placed along the edge of the drain at a distance of approximately 4 feet from it.

The price for the above materials disposal is to be included in the bid price per cubic yard for excavation.

8. Description of Ditch:

The ditch is to be constructed to the grades, widths and side slopes as shown on the accompanying profile.

The bottom width shall not be increased without maintaining the specified side slopes.

The grade is to be constructed to provide a constant slope to the end of the ditch so that no water will be stagnant therein.

9. Centre Line:

The Centre Line shall be the Centre Line of the present ditch, provided the fences are far enough back from the shoulders to allow for the proper width of bottom and side slope as shown on the accompanying profile. In locations where fences are too close, the Centre Line may be moved away from the fence a sufficient distance so that bottom widths and side slopes may be maintained.

At locations where the drain is to be removed from a road allowance, the centre line will be staked by the Engineer to allow for a clear berm of 3 feet between the property line and the edge of the drain.

At locations where excessive meanderings of the present stream take place, straightening may be required; in such cases the Centre Line will be staked in the field by the Engineer.

10. Grades and Centre Line:

Grades and Centre Line will be given by the Engineer upon receiving 5 clear days notice in writing that such Grades and Centre Line are required.

The Contractor will take precautions to ensure that Grades and Line so set will not be disturbed during construction.

Any subsequent setting of Line and Grade on the project will be charged to the Contractor.

11. Inspection:

Upon completion of the work there should be a continuous ditch or water course of the size and dimensions according to the plan, profile and specifications.

Any foreign material accumulated in the drain will be removed by the Contractor, unless he can clearly show that he is not responsible for the foreign material being in the drain.

The Contractor, when he considers all work to have been so completed, must notify the Engineer that he requires an inspection of the works to be made and he, or his representative, will accompany the Engineer on this inspection.

12. Classification of materials:

Earth shall mean clay, loam, sand, small stones, gravel and muck, etcetera.

Hard Pan shall mean materials other than rock that require the use of picks, bars, dynamite, etc. for their removal.

Rock shall mean strata rock or boulders having a cubic content of 1½ cubic feet or more.

Prices are to be submitted separately to cover each type of material.

13. Farm Bridges and Culverts:

The Contractor shall, as part of this contract, clean out or lower Farm Bridges and Culverts considered to be large enough for their locations.

The Contractor shall notify the Owner four clear days in writing to remove farm bridges that are not large enough to meet specifications; if the bridges are not removed, the Contractor may, after four days, remove same, exercising normal caution so as not to unduly damage the materials, piling same neatly 15 feet from the edge of the drain.

The Contractor may remove the flooring to clean out under a bridge that is of sufficient size; however, he must replace this flooring in as good condition as before it was removed.

The responsibility for the replacement of culverts or the construction of new ones is set forth in the Supplement Specifications.

Payment for the above work is to be included in the bid price per cubic yard for earth excavation.

14. Road Bridges and Culverts:

The Contractor shall notify the road Superintendent concerned as to the date and time the excavating equipment will be at the site of the road bridge or culvert.

The Contractor will construct the ditch to the ends of the present bridge or culvert.

The Contractor will clean all culverts that are of sufficient size and set at the required grade.

The Contractor will lower the grade and otherwise clear out under bridges having sufficient size for their locations. Payment for this work is deemed to be included in the bid price per cubic yard for earth excavation.

14. Road Bridges and Culverts. (Continued)

Work other than the above that may be required by the Road Superintendent will be paid for on an hourly basis by the Road Department responsible for the upkeep of this section of road.

15. Right to Increase or Decrease:

The Municipal Council reserves the right to increase the work as it deems necessary and the contract price per cubic yard shall remain the same.

16. Sub-Letting:

No portion of the work is to be sub-let without the consent of the Municipal Council and the Engineer.

17. Deposit:

A cash deposit or certified cheque on a chartered bank in the amount of 10% of the tender price must accompany each tender, such deposit will be returned to the unsuccessful bidders within 7 days of tender closing.

18. Date of Completion:

The whole work shall be completed on or before the day of _____, 196____ and when the Contractor considers that the work is completed, he must notify the Engineer in writing that he requires a final inspection thereof.

19. Payment:

Cash payment will be made monthly equal to 90% of the value of the work completed on the certificate of the Engineer, when the value of work completed within the month amounts to Three Hundred Dollars (\$300.00) or more. The remaining 10% will be retained until 60 days after the whole work has been accepted as finished.

20. The Municipal Council reserves the right to reject any and all tenders.

21. The Contractor shall comply with the regulations of the Workers Compensation Board of Ontario.

22. Damages caused to public utilities installation shall be the responsibility of the General Contractor.

Prior to starting this work, he shall obtain from the public utilities (i.e. telephone, hydro, gas) the locations, if any, of all their installations along these works.

Township of Monoona

TENDER FORM

I/AE
do hereby tender and agree to construct the
in accordance with the attached specifications and drawings.

I/AE have examined the site of the above work and are thoroughly
familiar with the work that is to be done.

I/AE tender and agree to perform the above mentioned work for the
following prices:

EXTENT OF WORK:TOTAL PRICE

Earth Excavation and Spreading of 1,992 c.y.....

Hardpan Excavation and Spreading of 232 c.y.....

Brushing.....

TOTAL CONTRACT PRICE

I/AE guarantee that the above work will be completed on the
day of _____, 196_____.

I/AE enclose a certified cheque or cash deposit for the sum of
(\$ _____), being 10% of the tender
price, and further agree to furnish a suitable bond for 100% of the
contract price within 7 days of notification of acceptance of tender,
if so requested. In such case, the deposit will be returned on the
signing of the contract.

The deposit or bonds of the successful bidder may be retained by the
Clerk until 60 days after the above work has been completed and accepted,
or in the event the tender is unsuccessful, it shall be returned within
7 days.

Offered on behalf of Contractor:Accepted on behalf of the Municipality

NAME: _____

RECEIVED: _____

ADDRESS: _____

CLERK: _____

POSITION: _____

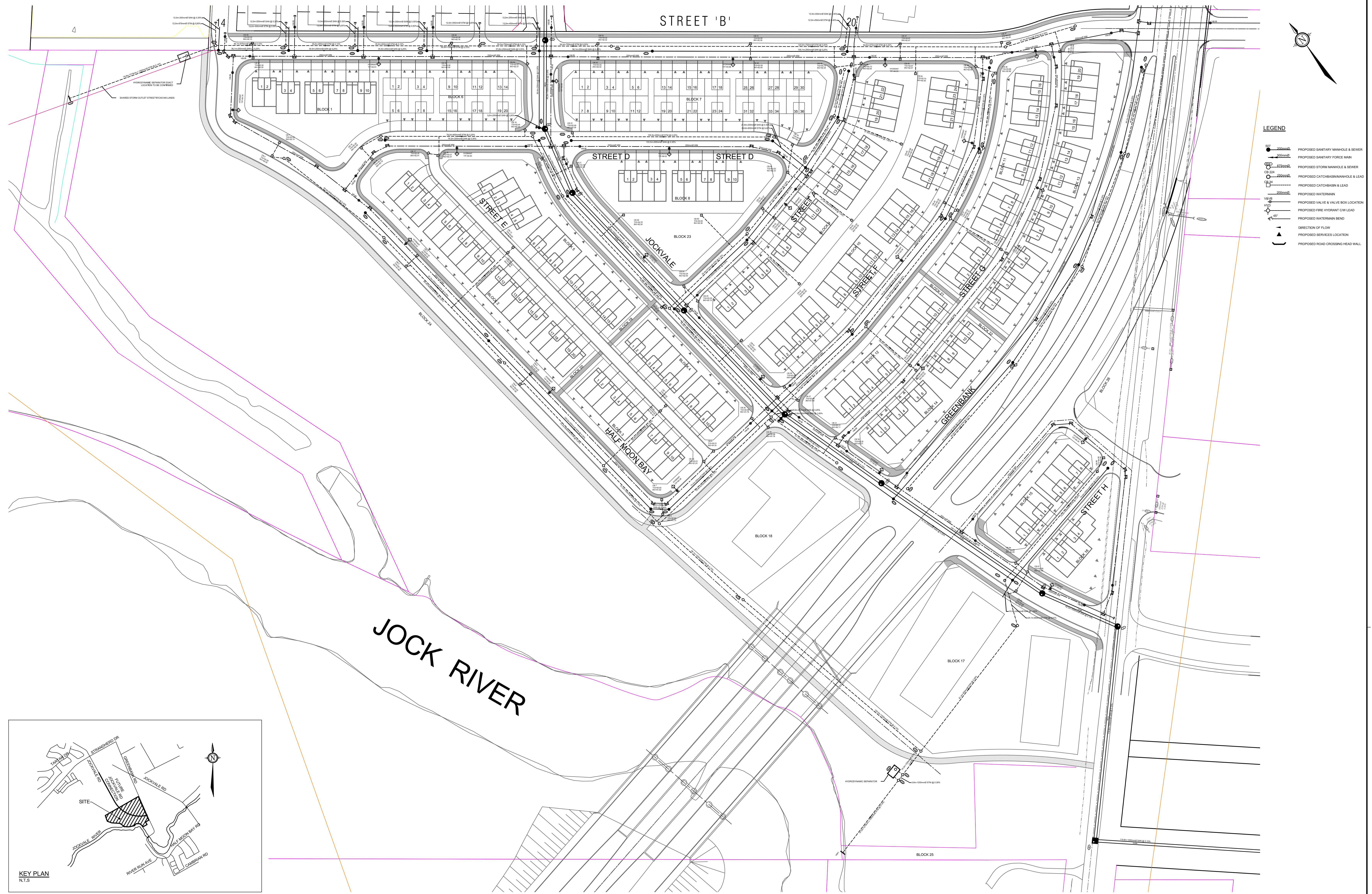
DATE: _____

DATE: _____

Appendix F

Engineering Drawings

<i>General Plan of Services</i>	<i>111117-GP</i>	<i>(revision 1)</i>
<i>Grading Plan</i>	<i>111117-GR</i>	<i>(revision 1)</i>
<i>Sanitary Drainage Area Plan</i>	<i>111117-SAN</i>	<i>(revision 1)</i>
<i>Storm Drainage Area Plan</i>	<i>111117-STM</i>	<i>(revision 1)</i>

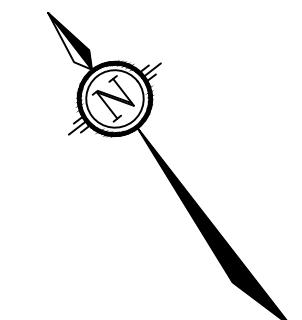


N-20111117-GP DRAFT OVERLAY 13.2016 - 1107am.mifig

NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS,
WATERMAINS, SEWERS AND OTHER
UNDERGROUND AND SURFACE UTILITIES AND
STRUCTURES IS NOT NECESSARILY SHOWN ON
THE CONTRACT DRAWINGS, AND WHERE SHOWN,
THE ACCURACY OF THE POSITION OF SUCH
UTILITIES AND STRUCTURES IS NOT GUARANTEED.
BEFORE STARTING WORK, DETERMINE THE EXACT
LOCATION OF ALL SUCH UTILITIES AND
STRUCTURES AND ASSUME ALL LIABILITY FOR
DAMAGE TO THEM.

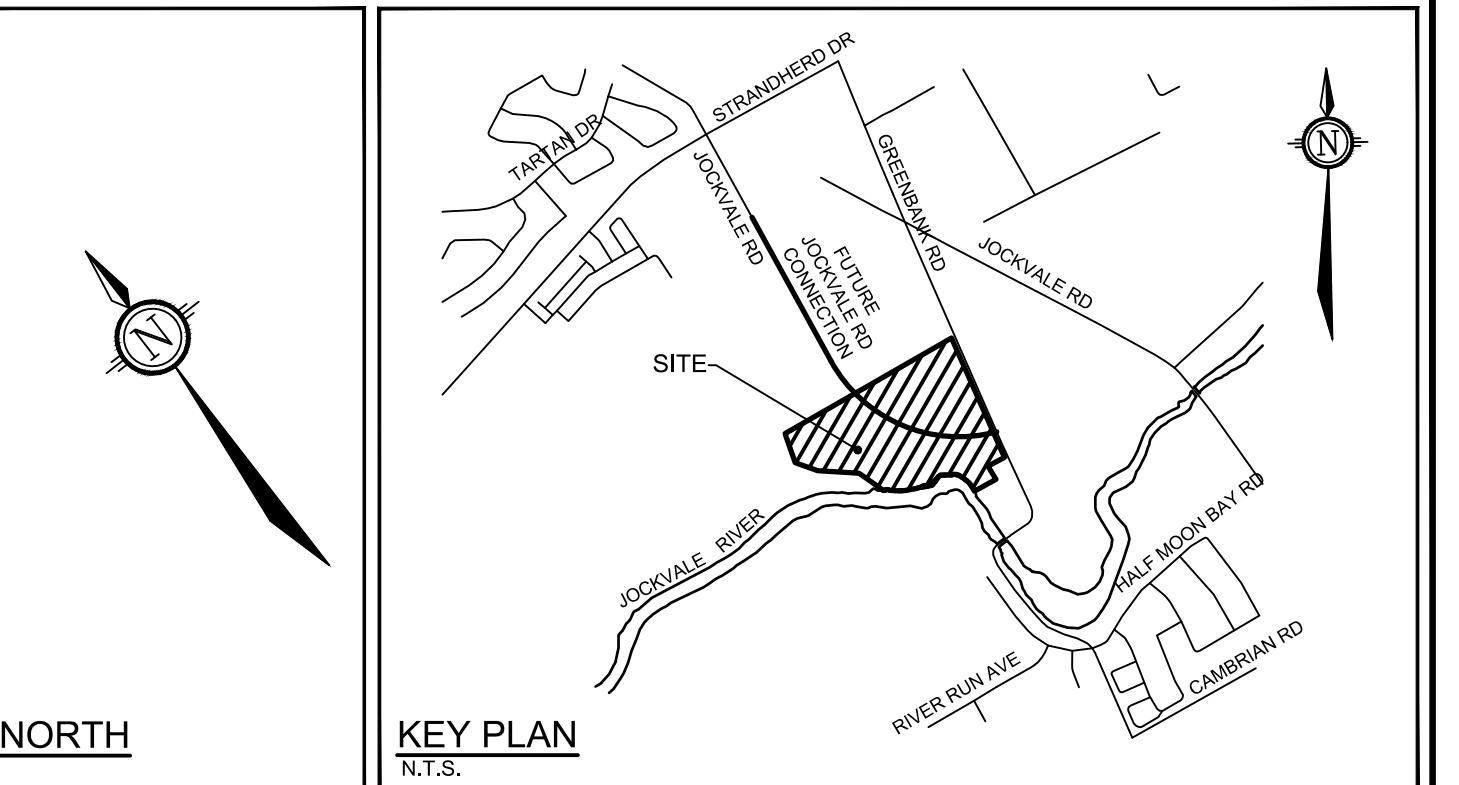
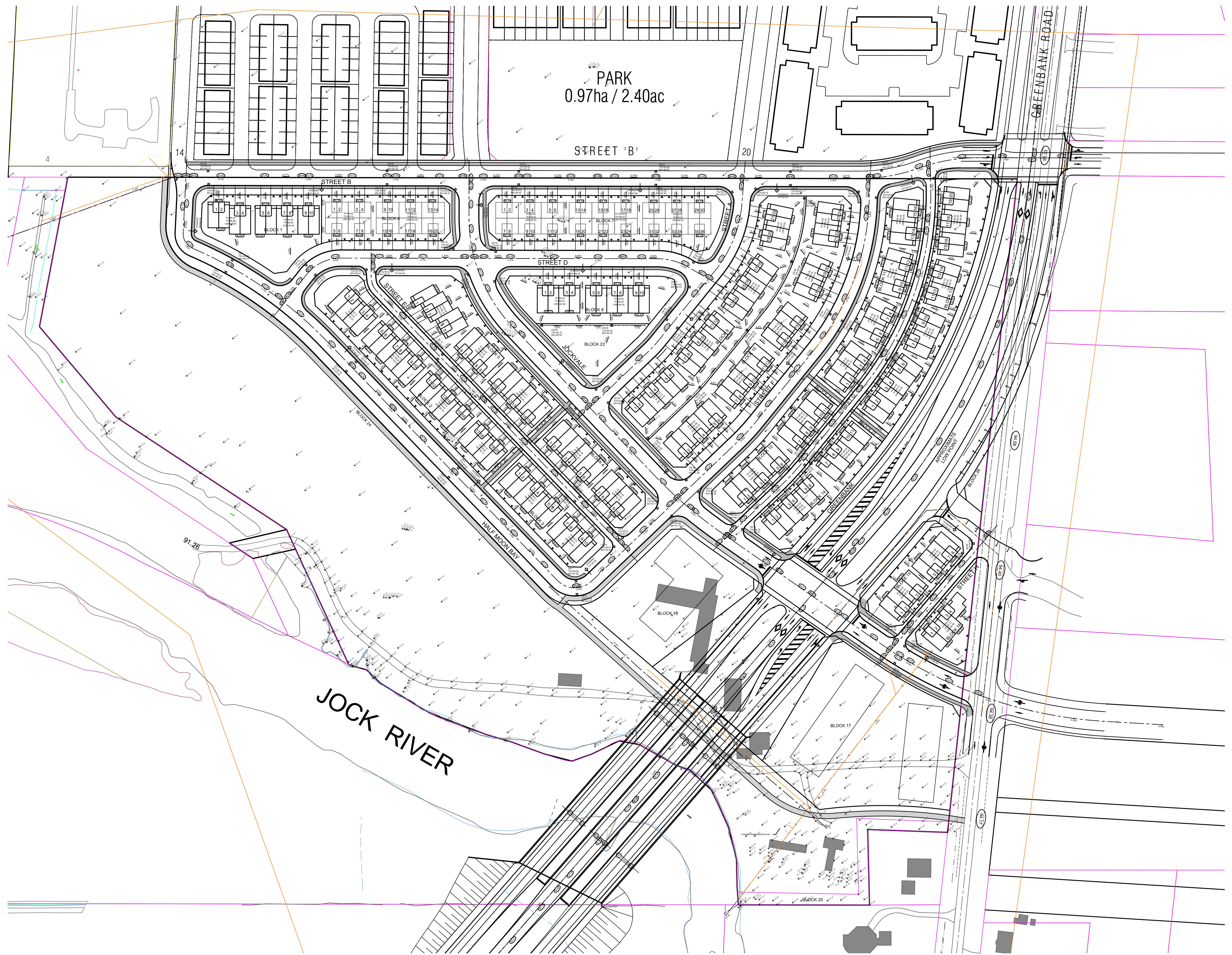
DVIEW 51.17"

STREET 'B'



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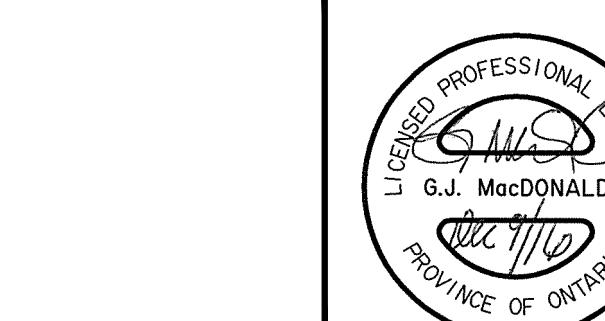
LEGEND

	IRON BAR & PROPERTY LINE
	LEGAL ADJACENT
(130.71)	PROPOSED ROAD ELEVATION
131.35	PROPOSED GARAGE ELEVATION
(127.25)	PROPOSED TERRACE ELEVATION
+ 130.49	<i>EXISTING TOPO SURFACE ELEVATION</i>
3.2%	PROPOSED SLOPE
FF=132.35	PROPOSED FINISHED FLOOR ELEVATION
TF=132.05	PROPOSED TOP OF FOUNDATION ELEVATION
USF=129.40	PROPOSED UNDERSIDE OF FOOTING ELEVATION

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NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6

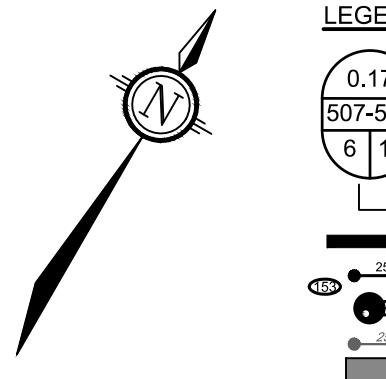
Telephone (613) 254-9643
Facsimile (613) 254-5867
Website www.novatech-eng.com

LOCATION
CITY OF OTTAWA
3370 GREENBANK ROAD

DRAWING NAME

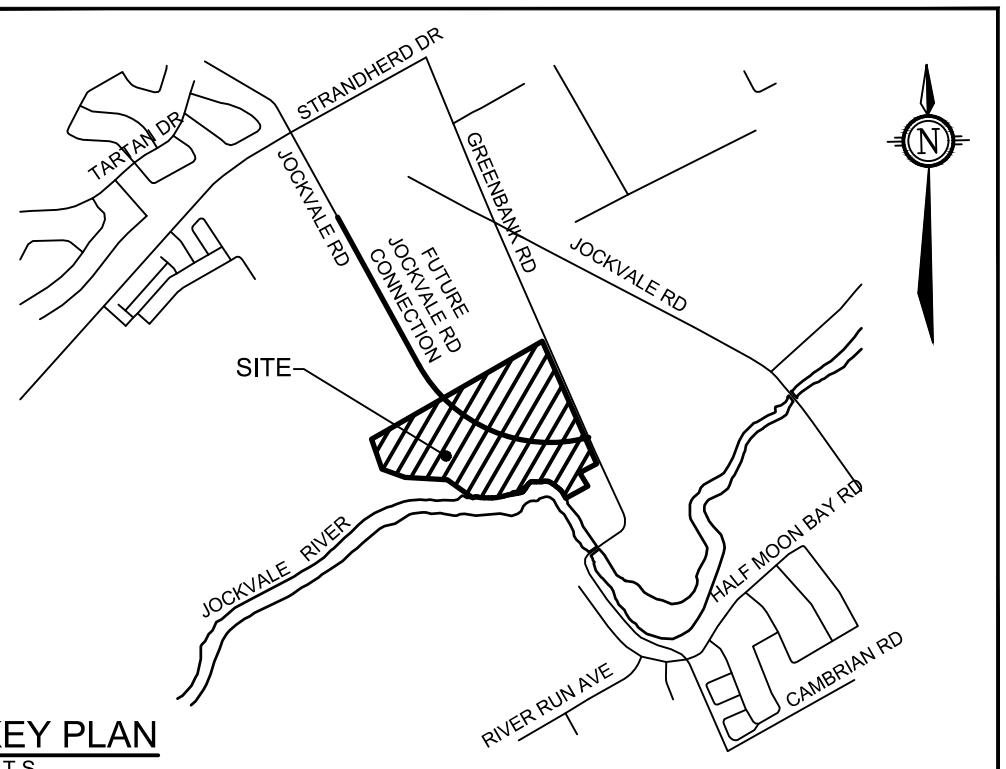
GRADING PLAN

	PROJECT No.
	111117
	REV
	REV # 1
	DRAWING No.
	111117-GR



LEGEND

- 0.17 DRAINAGE AREA (HECTARES)
- 507-509 DRAINAGE AREA NUMBER
- 6 111 POPULATION EQUIVALENT
- NUMBER OF UNITS
- SANITARY DRAINAGE AREA BOUNDARY
- PROPOSED SANITARY MANHOLE & SEWER
- EXISTING SANITARY MANHOLE & SEWER
- FUTURE DEVELOPMENT BY OTHER



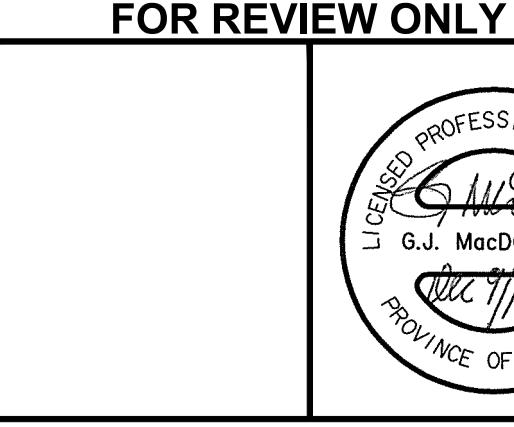
NOTE: SANITARY SAN. DES. 12-2010-3.37m. using N.T.S.

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NOTE:
CONTRACTOR TO CONFIRM ELEVATIONS OF INFRASTRUCTURE IN
THE STREET PRIOR TO EXTENDING SERVICES INTO THE SITE AND
SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IMMEDIATELY.

1	ISSUED WITH DRAFT PLAN OF SUBDIVISION	DEC 6/16	GJM
No.	REVISION	DATE	BY

SCALE	DESIGN
1:750	GJM/LSC
	CHECKED
	GJM
	DRAWN
	LSC
	CHECKED
	GJM
0 15 30	APPROVED
	GJM



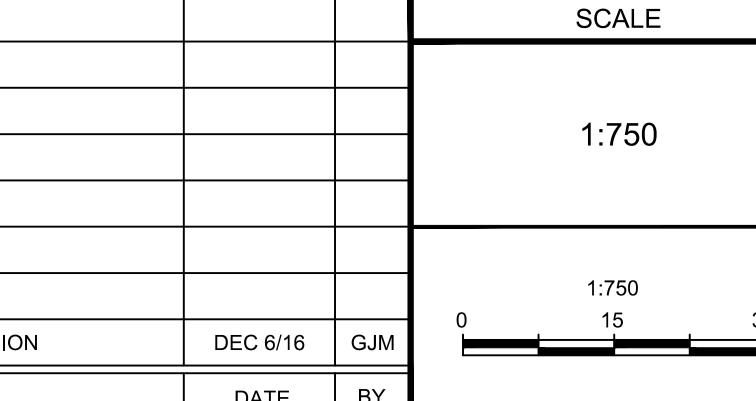
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DRAWING NAME SANITARY DRAINAGE AREA PLAN	REV REV # 1
	DRAWING No. 111117-SAN1



NOTE:
CONTRACTOR TO CONFIRM ELEVATIONS OF INFRASTRUCTURE IN
THE STREET PRIOR TO EXTENDING SERVICES INTO THE SITE AND
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1	ISSUED WITH DRAFT PLAN OF SUBDIVISION	
No.		REVISION



FOR REVIEW ONLY

