

# **REPORT ON**

**Geotechnical Investigation Proposed Residential Development Burnett Lands** Greenbank Road at the Jock River Ottawa, Ontario

#### Submitted to:

Jim Burghout Claridge Homes Corporation 2001-210 Gladstone Ottawa, Ontario K2P 0Y6

Report Number: 1523044 - 1000

Distribution:

7 copies - Novatech 1 e-copy - Claridge Homes Corporation 1 copy - Golder Associates Ltd.







# **Table of Contents**

| 1.0 | INTRODUCTION                    |  |    |  |  |  |  |  |  |
|-----|---------------------------------|--|----|--|--|--|--|--|--|
| 2.0 | DESCRIPTION OF PROJECT AND SITE |  |    |  |  |  |  |  |  |
| 3.0 | PROCEDURE                       |  |    |  |  |  |  |  |  |
| 4.0 | SUBSURFACE CONDITIONS           |  |    |  |  |  |  |  |  |
|     | 4.1                             | General                                    | 2  |  |  |  |  |  |  |
|     | 4.2                             | Area A                                     | 2  |  |  |  |  |  |  |
|     | 4.3                             | Area B                                     | 6  |  |  |  |  |  |  |
| 5.0 | DISCU                           | SSION                                      | 9  |  |  |  |  |  |  |
|     | 5.1                             | General                                    | 9  |  |  |  |  |  |  |
|     | 5.2                             | Site Grading                               | 9  |  |  |  |  |  |  |
|     | 5.3                             | Foundations                                | 10 |  |  |  |  |  |  |
|     | 5.4                             | Seismic Design                             | 11 |  |  |  |  |  |  |
|     | 5.5                             | Frost Protection                           | 11 |  |  |  |  |  |  |
|     | 5.6                             | Basement Excavations                       | 11 |  |  |  |  |  |  |
|     | 5.7                             | Basement and Garage Floor Slabs            | 12 |  |  |  |  |  |  |
|     | 5.8                             | Basement Wall and Foundation Wall Backfill | 12 |  |  |  |  |  |  |
|     | 5.9                             | Site Servicing                             | 13 |  |  |  |  |  |  |
|     | 5.10                            | Pavement Design                            | 14 |  |  |  |  |  |  |
|     | 5.11                            | Pools, Decks and Additions                 | 15 |  |  |  |  |  |  |
|     | 5.11.1                          | Above Ground and In Ground Pools           | 15 |  |  |  |  |  |  |
|     | 5.11.2                          | Decks                                      | 16 |  |  |  |  |  |  |
|     | 5.11.3                          | Additions                                  | 16 |  |  |  |  |  |  |
|     | 5.12                            | Corrosion and Cement Type                  | 16 |  |  |  |  |  |  |
|     | 5.13                            | Trees                                      | 16 |  |  |  |  |  |  |
| 6.0 | ADDITI                          | ONAL CONSIDERATIONS                        | 17 |  |  |  |  |  |  |
| 7.0 | CLOSU                           | IRE  | 18 |  |  |  |  |  |  |
| Imp | ortant In                       | formation and Limitations of This Report   |    |  |  |  |  |  |  |

i





#### **TABLES**

Table 1 - Some Common Trees in Decreasing order of Water Demand

#### **FIGURES**

Figure 1 – Site Plan

Figure 2 - Consolidation Test Results

Figure 3 – Grain Size Distribution

Figure 4 – Seismic Site Classification

#### **APPENDICES**

#### **APPENDIX A**

List of Abbreviations and Symbols Record of Borehole Sheets Current Investigation

#### **APPENDIX B**

Record of Borehole Sheets and Laboratory Test Results Previous Investigations

#### **APPENDIX C**

Results of Basic Chemical Analysis EXOVA Environmental Ontario Report No. 1603027





# 1.0 INTRODUCTION

This report presents the results of a geotechnical investigation carried out for the proposed residential development site to be located west of Greenbank Road, adjacent to the Jock River, in Ottawa, Ontario.

The purpose of this geotechnical investigation was to determine the general soil and groundwater conditions across this site by means of seven boreholes. Based on an interpretation of the factual information obtained during this current investigation, along with the existing data available for the area from the previous investigations, engineering guidelines are provided on the geotechnical design aspects of the project, including construction considerations which could affect design decisions.

The reader is referred to the "Important Information and Limitations of This Report" which follows the text but forms an integral part of this document.





# 2.0 DESCRIPTION OF PROJECT AND SITE

Consideration is being given to developing a residential subdivision on a site located west of Greenbank Road, adjacent to the Jock River, in Ottawa, Ontario. The approximate location of the site is shown on the Key Map insert provided on the Site Plan, Figure 1.

The following is known about the site and project:

- The site is located immediately west of Greenbank Road, approximately 400 metres south of the intersection with Jockvale Road. The southwest boundary of the site is adjacent to the Jock River and consists of a low-lying flood plain.
- The site is trapezoid-shaped and measures approximately 500 metres by up to 400 metres long.
- The site topography is relatively flat with a gentle downward slope from east to west. A shallow ditch (Burnett Municipal Drain) running north-south crosses the middle of the site.
- The majority of the site is currently undeveloped and predominately consists of agricultural land with localized vegetation and trees.
- It is understood that the proposed development will include conventional residential dwellings (semi-detached and townhouse) as well as access roads and services within the subdivision.
- Greenbank Road will be shifted to the west and will cross the eastern portion of the site.

Golder Associates completed a preliminary geotechnical investigation at this site in 2011, as part of the "due diligence" process associated with Claridge Homes acquiring this property. That investigation included a limited number of very widely spaced testholes.

Golder Associates also previously completed several geotechnical investigations within or in the vicinity of the site, including a preliminary investigation for a residential development to be located immediately north of this property and for the proposed South Nepean Collector sewer, which is proposed to cross the site.

Based on a review of those previous studies and published geological mapping, the subsurface conditions on northern and western portions of the site are expected to consist of a thick deposit of clay, but the clay is expected to thin towards the central and southern portions of the site where glacial till is expected at shallow depths.

Published geological mapping indicates that the depth to the bedrock surface on this site should be in the order of 5 to 15 metres.





# 3.0 PROCEDURE

The fieldwork for this investigation was carried out on February 18 through February 23, 2016. At that time, the following boreholes were put down at the approximate locations shown on the Site Plan, Figure 1.

- Seven boreholes (numbered BH16-101 to 16-107) were put down across the site.
- Two additional boreholes (numbered 16-102A and 16-104A) were put down adjacent to boreholes 16-102 and 16-104 to retrieve a Shelby tube sample of the "softer" silty clay.
- One additional borehole (numbered 16-107A) was put down adjacent to borehole 16-107 due to shallow refusal to augering.

All of the boreholes were advanced to depths ranging from about 1.7 (practical refusal to augering) to 8.2 metres below the existing ground surface. The boreholes were advanced using a track-mounted continuous flight hollow-stem auger drill rig, supplied and operated by CCC Geotechnical and Environmental Drilling Ltd. of Ottawa, Ontario.

Standard Penetration Tests (SPTs) were carried out in the boreholes at regular intervals of depth and samples of the soils encountered were recovered using split spoon sampling equipment. In situ vane testing was carried out, where possible, in the cohesive deposits to determine the undrained shear strength of these soils. Four relatively undisturbed 73 millimetre diameter thin walled Shelby tube samples of the silty clay were obtained from boreholes 16-102, 16-102A, 16-104, and 16-104A using a fixed piston sampler.

Groundwater monitoring wells were installed into boreholes 16-101 and 16-103 to allow for subsequent measurement of the groundwater level across the site. The groundwater levels in these monitoring wells were measured on March 7, 2016.

The fieldwork was supervised by an experienced technician from our staff who located the boreholes, directed the drilling operations and in situ testing, logged the boreholes and samples, and took custody of the soil samples retrieved.

Upon completion of the drilling operations, samples of the soils encountered in the boreholes were transported to our laboratory for further examination by the project engineer and for laboratory testing. The laboratory testing included natural water content determinations, Atterberg limit testing, grain size distribution testing, and oedometer consolidation testing.

Two samples of soil, one each from boreholes 16-105 and 16-107, were submitted to EXOVA Environmental Ontario for basic chemical analysis related to potential sulphate attack on buried concrete element and corrosion of buried ferrous elements.

The borehole locations were selected, picketed, and surveyed in the field by Golder Associates personnel. The borehole locations and elevations were surveyed using a Trimble R8 Global Positioning System (GPS) unit. The elevations are referenced to Geodetic datum.



# 4.0 SUBSURFACE CONDITIONS

## 4.1 General

The subsurface conditions encountered in the boreholes put down for the current investigation are shown on the Record of Borehole Sheets in Appendix A. The results of the laboratory water content and Atterberg limit testing are provided on the Record of Borehole Sheets. The oedometer consolidation test results are provided on Figure 2. The results of grain size distribution testing on selected sample of soil are provided on Figure 3.

The subsurface conditions encountered in the relevant boreholes put down on this site for the previous investigations are provided on the Record of Borehole Sheets in Appendix B.

The results of the basic chemical analyses carried out on two soil samples, one each from boreholes 16-105 and 16-107, are provided in Appendix C.

Based on the results of the current and previous investigations, the site has been divided into two assessment areas (see Site Plan, Figure 2), and the subsurface conditions within each area generally consist of:

- Area A (western and northern portions of the site): A thick deposit of silty clay which extends to depths of up to more than 8.2 metres below the existing ground surface.
- Area B (central and eastern portions of the site): Very stiff to stiff layer of silty clay overlying glacial till, or glacial till near the ground surface.

The following sections present a more detailed overview of the subsurface conditions for each assessment area.

#### 4.2 Area A

Area A includes the northern and western portions of the site. Boreholes 16-101, 16-102/16-102A and 16-104/16-104A for the current investigation and boreholes 11-1/11-1A, 11-2/11-2A, 11-5, 11-6, and 15-9/15-9A from the previous investigation were put down within this area.

#### **Topsoil**

Topsoil exists at the ground surface at all of the borehole locations. The topsoil varies from about 150 to 300 millimetres in thickness.

# **Sensitive Silty Clay**

The topsoil is underlain by a deposit of silty clay.

The upper 1.3 to 3.1 metres of the silty clay deposit have been weathered to a grey brown crust. SPT "N" values within the weathered crust ranged from 1 to 7 blows per 0.3 metres of penetration. In situ vane testing measured undrained shear strengths of about 59 to greater than 96 kilopascals. The results of this in situ testing indicate the crust to have a stiff to very stiff consistency.

The measured water contents of the weathered deposit were about 32 to 56 percent.

The silty clay below the depth of weathering is grey in colour. The silty clay deposit in boreholes 16-102, 11-2, 11-5, and 15-9/15-9A was fully penetrated and extends to depths ranging from about 3.1 to 4.3 metres below the existing ground surface. The unweathered grey silty clay in the remaining boreholes was not fully penetrated but was proven to depths of about 3.4 to 8.2 metres below the ground surface prior to the boreholes being terminated.





The undrained shear strength within the unweathered deposit is typically in the range of 30 to 50 kilopascals, which indicates a firm consistency.

The sensitivity of the silty clay was measured to predominantly vary between about 5 and 14, indicating this clay to be sensitive to extra-sensitive in accordance with the Canadian Foundation Engineering Manual (CFEM).

The results of Atterberg limit testing carried out on three samples of the grey silty clay indicate plasticity index values between 30 and 50 percent and liquid limit values between 51 and 75 percent, which indicates a high plasticity soil. The measured water content of the samples from the unweathered soils ranged from about 48 to 86 percent, except one sample at the bottom of borehole 16-104, which measured water content of 24 percent. This silty clay sample contains a trace to some sand and gravel and is likely near surface of the glacial till.

Oedometer consolidation testing was carried out on two Shelby tube samples of the grey silty clay. The results of this testing are provided on Figure 2 and on Figure 3 in Appendix B, as well as summarized below.

| Borehole/<br>Sample No. | Sample<br>Depth<br>(m) | Unit Wt.<br>(kN/m³) | σ <sub>P</sub> ′<br>(kPa) | σνο′<br>(kPa) | Cc   | Cr    | e <sub>o</sub> | OCR |
|-------------------------|------------------------|---------------------|---------------------------|---------------|------|-------|----------------|-----|
| 11-1A / 1               | 3.4                    | 16.2                | 225                       | 74            | 0.95 | 0.018 | 1.46           | 4.8 |
| 16-104A / 1             | 5.0                    | 14.9                | 110                       | 50            | 3.07 | 0.003 | 2.44           | 2.2 |

#### Notes:

σρ' - Apparent preconsolidation pressure σνο' - Computed existing vertical effective stress

 $C_c$  - Compression index  $C_r$  - Recompression index  $e_o$  - Initial void ratio OCR - Overconsolidation ratio

## **Glacial Till**

Glacial till was encountered beneath the silty clay deposit at boreholes 16-102, 11-2, 11-5 and 15-9 at depths of about 3.1 to 4.3 metres below the ground surface. The glacial till consists of a heterogeneous mixture of gravel, cobbles, and boulders in a matrix of silty sand to sandy silt. The glacial till was proven to depths of about 5.2 to 7.6 metres below the ground surface.

SPT "N" values of 1 to greater than 50 blows per 0.3 metres of penetration were measured in the glacial till, indicating a very loose to very dense state of packing. However the higher "N" values likely reflect the presence of cobbles and/or boulders within the glacial till, rather than actual state of packing of the soil matrix.

A layer of sandy soil was encountered below the glacial till in boreholes 11-2 and 15-9A. The sandy layer was proven to be at least about 0.3 and 1.7 metres thick, extending to depths of at least 5.5 and 9.1 metres below the ground surface respectively.

#### **Groundwater**

The groundwater levels in the monitoring well or piezometers sealed in the current and previous boreholes were measured at various times. The observed groundwater levels are summarized in the table below:





| Borehole<br>Number | Geological Unit | Ground<br>Surface<br>Elevation (m) | Groundwater<br>Depth (m) | Groundwater<br>Level (m) | Date of<br>Measurement |
|--------------------|-----------------|------------------------------------|--------------------------|--------------------------|------------------------|
| 16-101             | Silty Clay      | 91.82                              | 1.09                     | 90.73                    | Mar 7, 2016            |
| 11-1A              | Silty Clay      | -                                  | 1.23                     | -                        | Feb 7, 2011            |
| 11-2A              | Silty Clay      | -                                  | 0.95                     | -                        | Feb 7, 2011            |

It should be noted that groundwater levels are expected to fluctuate seasonally. Higher groundwater levels are expected during wet periods of the year, such as spring.

## 4.3 Area B

Area B includes the central and eastern parts of the site. Boreholes 16-103, 16-105, 16-106, 16-107/16-107A from the current investigation and boreholes 11-3 and 11-4 from the previous investigation were put down within this part of the site. In addition, boreholes 15-7, 15-8, 15-17/15-17A, 15-103, 16-301, and 16-302 from the South Nepean Collector investigation were advanced on this portion of the site.

# **Fill and Topsoil**

About 2.0 metres of fill exists at borehole 15-17. The fill consists of sandy silt and clayey silt, with varying amounts of gravel and organic matter.

SPT "N" values of 7 to 15 blows per 0.3 metres of penetration were measured within the fill, indicating a loose to compact state of packing.

Topsoil exists at the ground surface or below the fill. The topsoil varies from about 150 to 370 millimetres in thickness at the borehole locations.

## Silty Clay

At most of the borehole locations, the fill and topsoil are underlain by a deposit of silty clay. Where encountered, the entire silty clay deposit has been weathered to a grey brown crust and extends to depths of about 0.6 to 2.5 metres below the existing ground surface.

SPT "N" values within the weathered silty clay ranged from 2 to 10 blows per 0.3 metres of penetration. The results of one in situ vane test measured an undrained shear strength of about 59 kilopascals. The results of the in situ testing indicate the crust to have a stiff to very stiff consistency.

The measured water content of two samples of the weathered silty clay was about 36 and 41 percent.

#### **Upper Sandy Silt to Silty Sand**

A layer of silty sand with some silt exists beneath the silty clay at borehole 16-103. A possible sand layer was also encountered at the ground surface at boreholes 16-301 and 16-302. The sandy soil is about 0.3 to 0.9 metres thick.



#### **Glacial Till**

Glacial till exists below the silty clay and silty sand deposits or below the topsoil in all boreholes, at depths varying from about 0.3 to 2.5 metres below the existing ground surface. The glacial till generally consists of a heterogeneous mixture of gravel, cobbles, and boulders in a matrix of silty sand to sandy silt.

SPT "N" values within the glacial till of 1 to greater than 100 blows per 0.3 metres of penetration indicates a very loose to very dense state of packing. However, the higher blow counts likely reflect the presence of cobbles and/or boulders within the glacial till, rather than the actual state of packing of the soil matrix.

The measured water content of samples from the glacial till ranged from about 9 to 14 percent

A layer of sandy soil was encountered within the glacial till deposit at boreholes 16-106, 11-3, and 15-7 at depths of about 3.2 to 4.6 metres below the ground surface. The sandy soil consists of sand and silty sand and was proven to be about 0.3 to 1.4 metres thick. SPT "N" values within the sandy soil of 9 to 22 blows per 0.3 metre of penetration indicates this sandy soil to have a loose to compact state of packing.

The results of grain size distribution on one sample of the sandy soil retrieved from the current and previous investigations are provided on Figure 3.

## **Auger Refusal and Bedrock**

Refusal to auger advancement was encountered at boreholes 16-103, 16-105, 16-106, 16-107/16-107A, 15-17A, and 15-103 at depths of about 1.7 to 7.3 metres below the ground surface. Refusal may indicate the bedrock surface, but it likely reflects the presence of cobbles and/or boulders in the glacial till deposit.

Bedrock was encountered at boreholes 15-7, 15-8, 16-301 and 16-302. The boreholes were extended into the bedrock for depths of about 1.8 to 3.2 metres using rotary diamond drilling techniques while retrieving NQ or HQ sized core.

The following table summarizes the bedrock surface depths and elevations encountered at the borehole locations.

| Borehole<br>Number | Ground Surface<br>Elevation (m) | Depth to<br>Bedrock (m) | Bedrock Surface<br>Elevation (m) |
|--------------------|---------------------------------|-------------------------|----------------------------------|
| 15-7               | 92.84                           | 6.20                    | 86.64                            |
| 15-8               | 92.19                           | 8.89                    | 83.30                            |
| 16-301             | 93.16                           | 9.80                    | 83.36                            |
| 16-302             | 93.06                           | 8.02                    | 85.04                            |

The bedrock encountered in the boreholes consists of grey limestone and dolomite, with black shale interbeds. The bedrock is fresh and thinly to medium bedded. The Rock Quality Designation (RQD) values measured on the recovered bedrock core samples generally ranged from about 50 to 100 percent, indicating a fair to excellent rock quality.





## **Groundwater**

The groundwater levels in the monitoring well or piezometers sealed in the current and previous boreholes were measured at various times. The observed groundwater levels are summarized in the table below:

| Borehole<br>Number | Geologic<br>Unit   | Ground<br>Surface<br>Elevation<br>(m) | Groundwater<br>Depth (m) | Groundwater<br>Level (m) | Date of<br>Measurement | Estimated<br>Hydraulic<br>Conductivity<br>(cm/s) |
|--------------------|--|---------------------------------------|--------------------------|--------------------------|------------------------|--|
| 16-103             | Silty Clay/<br>Clay/Silty<br>Sand Till                   | 93.51                                 | 0.91                     | 92.60                    | Mar 7, 2016            | -  |
| 15-7               | Bedrock  | 92.84                                 | 2.17                     | 90.67                    | Apr 24, 2015           | 5x10 <sup>-5</sup>                               |
| 15-9A              | Gravelly sand<br>(interbedded<br>within glacial<br>till) | 92.27                                 | 0.92                     | 91.35                    | Aug 25, 2015           | 3x10 <sup>-3</sup>                               |
| 11-3               | Glacial Till   | -                                     | 1.53                     | -                        | Feb 7, 2011            | -  |

It should be noted that groundwater levels are expected to fluctuate seasonally. Higher groundwater levels are expected during wet periods of the year, such as spring.



## 5.0 DISCUSSION

## 5.1 General

This section of the report provides engineering guidelines on the geotechnical design aspects of the project based on our interpretation of the borehole information and the project requirements. The reader is referred to the "Important Information and Limitations of This Report" which follows the text but forms an integral part of this document.

# 5.2 Site Grading

The subsurface conditions on this site generally consist of silty clay overlying glacial till at depths varying from about 3.1 metres on the north part of the site and to more than 8.2 metres beneath the west part of the site. The lower portions of the deeper silty clay deposit are compressible beneath the northern and western parts of the site. On the central and eastern parts of the site, the subsurface conditions consist of very stiff weathered silty clay underlain by glacial till, or glacial till near the ground surface.

The "softer" grey silty clay deposit has limited capacity to accept additional load from the weight of grade raise fill and from the foundations of houses without undergoing consolidation settlements. This condition is particularly true for the northern and western portions of the site where the silty clay deposit is much thicker and more compressible. Therefore, to leave sufficient remaining capacity for the silty clay to support house foundations, with reasonable footing sizes, the thicknesses of grade raise fill will need to be limited.

The following table provides the maximum grade raises which are permitted for each of the assessment areas as indicated on Figure 1. These grade raise limitations have been assessed based on leaving sufficient remaining capacity in the silty clay deposit such that footings up to 0.6 metres in size can be designed using an allowable bearing pressure of at least 75 kilopascals, consistent with design in accordance with Part 9 of the Ontario Building Code (OBC).

| Assessment<br>Area | Maximum Permissible Grade<br>Raise (m) |
|--------------------|--|
| А                  | 1.7                                    |
| В                  | NA¹                                    |

Note 1: NA = Not Applicable.

For Area B, there is no practical grade raise restriction since this area is underlain by very stiff weathered silty clay overlying glacial till deposit, or just glacial till.

The above maximum grade raise for Area A was selected based on the following considerations:

- The houses will have conventional depth basements, with founding levels in the range of 2.4 metres below finished grade.
- Any fill required for site grading (above original grade) and the backfill within the garages would have a unit weight of no more than 20 kilonewtons per cubic metre. Crushed clear stone, uniform fine sand, as well as silty clay (such as present on this site) may be suitable for this purpose. Sand and gravel, glacial till, and crushed stone typically have a higher unit weight and, if these materials are to be used, the maximum permissible grade raises would be reduced and would need to be re-evaluated.



If the grading restriction given for Area A cannot be accommodated, the following three options could be considered:

- 1) The additional required grade raising (above the limits given in the previous table) could be accomplished using expanded polystyrene (EPS) light weight fill.
- 2) The area could be pre-loaded and allowed to settle in advance of house construction. The subgrade settlements would need to be monitored to establish when sufficient settlements had occurred such that house construction could proceed. To reduce the time required for the pre-loading, it is likely that a temporary surcharge above the existing grade would need to be considered, however in either case the pre-load time could be months or years.
- 3) The houses could be supported on driven steel piles, which derive their support from more competent bearing soils (i.e., glacial till) below the "softer" layers of silty clay.

Additional geotechnical guidelines would need to be provided if any of the three above options are selected. Additional investigation could also be required (in particular for Options 2 and 3) before those guidelines could be finalized.

As a general guideline regarding the site grading, the preparation for filling of the site should include stripping the topsoil or buried topsoil (near borehole 15-17) for predictable performance of structures and services. The topsoil is not suitable as engineered fill and should be stockpiled separately for re-use in landscaping applications only. In areas with no proposed structures, services, or roadways, the topsoil may be left in-place provided some settlement of the ground surface following filling can be tolerated.

## 5.3 Foundations

It is considered that the proposed residences may be supported on spread footings founded on or within the weathered silty clay or glacial till (where the surface of the till is shallow).

For Area A, the unweathered silty clay deposit beneath this portion of the site has limited capacity to accept the combined load from site grading fill and foundation loads. The allowable bearing pressures for spread footing foundations are therefore based on limiting the stress increases on the unweathered compressible silty clay at depth to an acceptable level so that foundation settlements do not become excessive. Four important parameters in calculating the stress increase on the grey silty clay are:

- The thickness of soil below the underside of the footings and above the "softer" unweathered silty clay;
- The size (dimensions) of the footings;
- The amount of surcharge in the vicinity of the foundations due to landscape fill, underslab fill, floor loads, etc., as described in Section 5.2; and,
- The effects of groundwater lowering caused by this or other construction.

Provided that the grade raises are restricted to those indicated in Section 5.2, strip footing foundations up to 0.6 metre in width can be designed using a maximum allowable bearing pressure of 75 kilopascals. As such, the house footings may be sized in accordance with Part 9 of the OBC.



The post-construction total and differential settlements of footings sized using the above maximum allowable bearing pressure should be less than about 25 and 15 millimetres, respectively, provided that the subgrade at or below founding level is not disturbed during construction.

Further, the provided maximum allowable bearing pressure for footings founded within the silty clay corresponds to settlement resulting from consolidation of these deposits. Consolidation of the silty clay is a process which takes months or longer and, as such, results from sustained loading. Therefore, the foundation loads to be used in conjunction with the allowable bearing pressure should be the full dead load plus <u>sustained</u> live load.

The glacial till on this site contains cobbles and boulders. Any cobbles or boulders in the footing areas which have been loosened by the excavation process should be removed and the cavity filled with lean concrete (the cobble and boulders should not be pushed back in place).

# 5.4 Seismic Design

For design in accordance with Part 4 of the 2012 OBC, where required, a seismic site response classification of Site Class E could be assigned to the northern and western portions of the site, and a Site Class D could be assigned for the eastern portion of the site, acknowledging that this requirement does not apply to ground oriented residential structures designed per Part 9 of the OBC. The boundary between these two Site Class areas are shown on Figure 4.

This Site Class assessment was carried out using in situ testing data (as permitted by the OBC), and therefore it could be conservative. Geophysical measurement of the shear wave velocity for the upper 30 metres of soil and/or bedrock below founding level on this site would allow for a more accurate seismic Site Class to be specified, and to help assess whether more favourable Site Class values could be assigned to parts of the site.

The soils on this site are not considered to be susceptible to liquefaction during seismic events.

#### 5.5 Frost Protection

All exterior perimeter foundation elements or foundation elements in unheated areas should be provided with a minimum of 1.5 metres of earth cover for frost protection purposes. Isolated and/or unheated exterior footings adjacent to surfaces which are cleared of snow cover during winter months should be provided with a minimum of 1.8 metres of earth cover.

Insulating the bearing surface with high density insulation could be considered as an alternative to earth over for frost protection. Further details could be provide if required.

## 5.6 Basement Excavations

Excavations for basements will be through the fill, topsoil, sandy soil, silty clay, and glacial till (where the till surface is shallow). No unusual problems are anticipated in excavating in the overburden soils using conventional hydraulic excavating equipment, recognizing that large boulders will likely be encountered within the glacial till. Boulders larger than 0.3 metres in size should be removed from the walls of the excavations for worker safety.

Side slopes in the overburden materials should be stable in the short term at 1 horizontal to 1 vertical (1H:1V) in accordance with the Occupational Health and Safety Act (OHSA) of Ontario for Type 3 soils.



Some groundwater inflow into the excavations could be expected. However, for the planned basement excavation depths, it should be possible to handle the groundwater inflow by pumping from well filtered sumps in the excavations. Where the subgrade is found to be wet and sensitive to disturbance, consideration should be given to placing a mud slab of lean concrete over the subgrade (following inspection and approval by geotechnical personnel) or a 150 millimetre thick layer of OPSS Granular A underlain by a non-woven geotextile, to protect the subgrade from construction traffic.

# 5.7 Basement and Garage Floor Slabs

In preparation for the construction of the basement floor slabs, all loose, wet and disturbed materials should be removed from beneath the floor slabs. Provision should be made for at least 200 millimetres of 19 millimetre crushed clear stone to form the base of the basement floor slabs.

To prevent hydrostatic pressure build up beneath the basement floor slabs, it is suggested that the granular base material be positively drained. This could be achieved by providing a hydraulic link between the underslab fill material and the exterior drainage system.

The backfill material inside the garage should have a unit weight no greater than 20 kilonewtons per cubic metre (i.e., uniform fine sand or clear crushed stone). The garage backfill should be placed in maximum 300 millimetre thick lifts and be compacted to at least 95 percent of the material's standard Proctor maximum dry density using suitable compaction equipment. The granular base for the garage floor slab should consist of at least 150 millimetres of Granular A compacted to at least 95 percent of the material's Standard Proctor maximum dry density using suitable compaction equipment.

## 5.8 Basement Wall and Foundation Wall Backfill

The soils at this site are frost susceptible and should not be used as backfill directly against exterior, unheated, or well insulated foundation elements. To avoid problems with frost adhesion and heaving, a bond break such as Platon system sheeting should be placed against the foundation walls. Alternatively, these foundation elements could be backfilled with non-frost susceptible sand, provided it meets the unit weight restriction of 20 kilonewtons per cubic metre.

Drainage of the wall backfill should be provided by means of a perforated pipe subdrain in a surround of 19 millimetre clear stone, fully wrapped in geotextile, which leads by gravity drainage to an adjacent storm sewer or sump pit. Conventional damp proofing of the basement walls is appropriate with the above design approach.

Where design of basement walls in accordance with Part 4 of the 2012 OBC is required, walls backfilled with granular material and effectively drained as described above should be designed to resist lateral earth pressures calculated using a triangular distribution of the stress with a base magnitude of K<sub>0</sub>γH, where:

- $K_o$  = The lateral earth pressure coefficient in the 'at rest' state, use 0.5;
- $\gamma$  = The unit weight of the granular backfill, use 22 kilonewtons per cubic metre; and,
- H = The height of the basement wall in metres.

If Platon System sheeting or similar water barrier product is used against the foundation walls, then hydrostatic groundwater pressures should also be considered in the calculation of the lateral earth pressures.



# 5.9 Site Servicing

Excavations for the installation of site services will be made through the fill, topsoil, sandy soil, silty clay, and glacial till (at least on parts of the site). No unusual problems are anticipated in excavating in the overburden using conventional hydraulic excavating equipment, recognizing that cobbles and boulders will likely be encountered within the glacial till. Boulders larger than 0.3 metres in size should be removed from the excavation side slopes for worker safety.

In accordance with the OHSA of Ontario, the silty clay and the sandy soils above the water table would generally be classified as Type 3 soils and side slopes in the entire deposit of silty clay as well as the sandy soils above the water table in the short term may be sloped at 1H:1V. Excavation side slopes in the sandy soil below the groundwater level would need to be cut back at 3H:1V (i.e., Type 4 soils). This includes the sandy deposits within the glacial till. Alternatively, excavations within the overburden could also be carried out within a fully braced steel trench box, which would minimize the width of the excavation.

Stockpiling of soil beside the excavations made in the silty clay should be avoided; the weight of the stock piled soil could lead to basal instability of braced excavations or slope instability for unsupported excavations.

Some groundwater inflow into the excavations should be expected from the grey clay and glacial till. However, it should generally be possible to handle the groundwater inflow by pumping from well filtered sumps in the excavations provided suitably sized pumps are used.

Higher groundwater inflows should be expected from the more permeable sandy layers within the glacial till, and some sloughing and subgrade disturbance should be expected. It is expected that passive dewatering will be required to prevent destabilization (i.e., disturbance) of the excavation base in areas where basal heave or subgrade disturbance will be potential issues (i.e., where high permeability sandy soils are present at or in close proximity to the base of the excavations such as borehole 15-9A). As a preliminary guideline, it is envisaged that passive relief wells could be installed at periodic intervals along the alignment of "deeper" site services alignment in advance of excavation to allow for lowering the piezometric level. Alternatively, pumping from sumps installed within pits just ahead (and off-line) of the sewer trenches could also be considered. If this option is chosen, pits should be located just off-line of the site services alignment to avoid the potential for disturbance of the subgrade below pipe bedding level. The design of the dewatering system should ultimately be the responsibility of the contractor.

The actual rate of groundwater inflow to the trench (or the required pumping from sumps) will depend on many factors, including: the contractor's schedule and rate of excavation, the size of the excavation, the number of working areas being excavated at one time, and the time of year at which the excavation is made. Also, there may be instances where significant volumes of precipitation, surface runoff and/or groundwater collects in an open excavation, and must be pumped out. If the pumping volumes exceed 400,000 litres per day, a Category 3 Permit-To-Take-Water (PTTW) will be required from the Ministry of the Environment and Climate Change (MOECC). For expected groundwater inflows greater than 50,000 litres per day but less than 400,000 litres per day (equivalent to a formerly Category 2 PTTW), a registration in the Environmental Activity and Sector Registry (EASR) will be required under the new regulations, which came into force on March 29, 2016.

At least 150 millimetres of OPSS Granular A should be used as pipe bedding for sewer and water pipes. Where unavoidable disturbance to the subgrade surface does occur, it may be necessary to place a sub-bedding layer consisting of compacted OPSS Granular B Type II beneath the Granular A, or to thicken the Granular A bedding.





The bedding material should, in all cases, extend to the spring line of the pipe and should be compacted to at least 95 percent of the material's standard Proctor maximum dry density. The use of clear crushed stone as a bedding layer should not be permitted anywhere on this project since fine particles from the sandy backfill materials or sandy soils on the trench walls could potentially migrate into the voids in the clear crushed stone and cause loss of lateral pipe support.

Cover material, from the spring line of the pipe to at least 300 millimetres above the top of pipe, should consist of OPSS Granular A or Granular B Type I with a maximum particle size of 25 millimetres. The cover material should be compacted to at least 95 percent of the material's standard Proctor maximum dry density.

It should generally be possible to re-use the drier silty clay and glacial till as trench backfill.

However, the high moisture content of the "deeper" silty clay deposits (i.e., below the weathered zone) makes these soils difficult to handle and compact. If these materials are excavated during installation of the site services, they should be wasted or should only be used as backfill in the lower portion of the trenches to limit the amount of long term settlement of the roadway surface. If the unweathered silty clay is used in trenches under roadways, long term settlement of the pavement surface should be expected.

Where the trench will be covered with hard surfaced areas, the type of native material placed in the frost zone (between subgrade level and 1.8 metres depth) should match the soil exposed on the trench walls for frost heave compatibility. Trench backfill should be placed in maximum 300 millimetre thick lifts and should be compacted to at least 95 percent of the material's standard Proctor maximum dry density using suitable compaction equipment.

Long term groundwater level lowering at this site could lead to overstressing of the silty clay and the settlement of overlying structures. Impervious dykes or cut-offs should be constructed at 100 metre intervals in the service trenches to reduce groundwater lowering at the site due to the "french drain" effect of the granular bedding and surround for the service pipes. It is important that these barriers extend from trench wall to trench wall and that they fully penetrate the granular materials to the trench bottom. The dykes should be at least 1.5 metres wide and could be constructed using relatively dry (i.e., compactable) grey brown silty clay from the weathered zone.

# 5.10 Pavement Design

In preparation for pavement construction, all topsoil, disturbed, or otherwise deleterious materials (i.e., those materials containing organic material) should be removed from the roadway areas.

Pavement areas requiring grade raising to proposed subgrade level should be filled using acceptable (compactable and inorganic) earth borrow or OPSS Select Subgrade Material. These materials should be placed in maximum 300 millimetre thick lifts and should be compacted to at least 95 percent of the material's standard Proctor maximum dry density using suitable compaction equipment.

The surface of the pavement subgrade should be crowned to promote drainage of the roadway granular structure. Perforated pipe sub-drains should be provided at subgrade level extending from the catch basins for a distance of at least 3 metres longitudinally, parallel to the curb in two directions.





The pavement structure for local roads should consist of:

| Pavement Component              | Thickness<br>(millimetres) |
|---------------------------------|----------------------------|
| Asphaltic Concrete              | 90                         |
| OPSS Granular A Base            | 150                        |
| OPSS Granular B Type II Subbase | 375                        |

The pavement structure for collector roadways should consist of:

| Pavement Component  | Thickness<br>(millimetres) |
|---|----------------------------|
| Asphaltic Concrete OPSS Granular A Base OPSS Granular B Type II Subbase | 90<br>150<br>450           |

For arterial roadway, the subbase thickness should be increased to 600 millimetres.

The granular base and subbase materials should be uniformly compacted to at least 100 percent of the materials' standard Proctor maximum dry density using suitable vibratory compaction equipment. The asphaltic concrete should be compacted in accordance with Table 10 of OPSS 310.

The composition of the asphaltic concrete pavement should be as follows:

Superpave 12.5 mm Surface Course – 40 millimetres

Superpave 19 mm Base Course - 50 millimetres

The asphaltic cement should consist of PG 58-34 and the design of the mixes should be based on a Traffic Category B for local roads and Category D for collector roads.

The above pavement designs are based on the assumption that the pavement subgrade has been acceptably prepared (i.e., where the trench backfill and grade raise fill have been adequately compacted to the required density and the subgrade surface not disturbed by construction operations or precipitation). Depending on the actual conditions of the pavement subgrade at the time of construction, it could be necessary to increase the thickness of the subbase and/or to place a woven geotextile beneath the granular materials.

# 5.11 Pools, Decks and Additions

#### 5.11.1 Above Ground and In Ground Pools

For Area A, no special geotechnical considerations are necessary for the installation of in-ground pools, provided that the pool (including piping) does not extend deeper than the house footing level. A geotechnical assessment will be required if the pool extends deeper than the house foundations.

For Area A, due to the additional loads that would be imposed by the construction of *above-ground pools*, these should be located no closer than 2 metres from the outside wall of the house. In addition, the installation of an above-ground pool should not be permitted to alter the existing grades within 3 metres of the house. Provided these restrictions are adhered to, no further geotechnical assessment should be required for above-ground pools.



For Area B, no special geotechnical considerations are necessary for the installation of in-ground or above ground pools.

#### 5.11.2 Decks

For Area A, a geotechnical evaluation/assessment will be necessary for future decks that:

- are attached to the house;
- require changes to the existing grades; or,
- are heavily loaded and require spread footing or drilled pier foundations.

The geotechnical evaluation must consider the proposed grading, foundation types and sizes, depths of foundations, and design bearing pressures. Written approval from a geotechnical engineer should be required by the City of Ottawa prior to a building permit being issued.

For Area B, no special geotechnical considerations area necessary for decks.

#### 5.11.3 Additions

For both Areas A and B, any proposed addition to a house (regardless of size) will require a geotechnical assessment. The geotechnical assessment must consider the proposed grading, foundation types and sizes, depths of foundations, and design bearing pressures. Written approval from a geotechnical engineer should be required by the City of Ottawa prior to the building permit being issued.

# 5.12 Corrosion and Cement Type

Two samples of soil, one each from boreholes 16-105 and 16-107, were submitted to EXOVA Environmental Ontario for basic chemical analysis related to potential corrosion of buried steel elements and potential sulphate attack on buried concrete elements. The results of this testing are provided in Appendix C. The results indicate that concrete made with Type GU Portland cement should be acceptable for substructures. The results also indicate a moderate to elevated potential for corrosion of exposed ferrous metal, which should be considered in the design of substructures.

#### **5.13** Trees

The clayey soils encountered within portions of the site are highly sensitive to water depletion by trees of high water demand during periods of dry weather. When trees draw water from the clayey soil, the clay undergoes shrinkage which can result in settlement of adjacent structures. The zone of influence of a tree is considered to be approximately equal to the height of the tree. Therefore trees which have a high water demand should not be planted closer to structures than the ultimate height of the trees. Table 1 provides a list of the common trees in decreasing order of water demand and, accordingly, decreasing risk of potential effects on structures.

The restriction on trees can be waived in those areas where the footings are founded on glacial till.





## 6.0 ADDITIONAL CONSIDERATIONS

The guidelines in this report have been developed on the basis of the structures on this site being designed in accordance with Part 9 of the OBC. For any portions of the site where the structures will need to be designed in accordance with Part 4 of the OBC, additional geotechnical investigation may be required and additional guidelines would need to be provided.

The soils at this site are sensitive to disturbance from ponded water, construction traffic and frost.

All footing and subgrade areas should be inspected by experienced geotechnical personnel prior to filling or concreting to ensure that soil having adequate bearing capacity has been reached and that the bearing surfaces have been properly prepared. The placing and compaction of any engineered fill as well as sewer bedding and backfill should be inspected to ensure that the materials used conform to the specifications from both a grading and compaction point of view.

At the time of the writing of this report, only preliminary details for the proposed subdivision were available. Golder Associates should be retained to review the final drawings and specifications for this project prior to construction to ensure that the guidelines in this report have been adequately interpreted.

The groundwater level monitoring devices (i.e., standpipe piezometers or wells) installed at the site will require decommissioning at the time of construction in accordance with Ontario Regulation 128/03. However, it is expected that most of the wells will either be destroyed during construction or can be more economically abandoned as part of the construction. If that is not the case or is not considered feasible, abandonment of the monitoring wells can be carried out separately.





# 7.0 CLOSURE

We trust this report contains sufficient information for your present requirements. If you have any questions concerning this report, or if we can be of further service to you on this project, please call us.

Yours truly,

**GOLDER ASSOCIATES LTD.** 

Troy Skinner, P.Eng.

Associate, Senior Geotechnical Engineer

KM/CK/TMS/ob

Geotechnical Engineer

\\golder.gds\gal\ottawa\active\2015\3 proj\1523044 claridge burnett lands ottawa\08\_reports\1000-geotech\final\1523044-1000 geotech report\_final.docx

100103870

# IMPORTANT INFORMATION AND LIMITATIONS OF THIS REPORT

**Standard of Care:** Golder Associates Ltd. (Golder) has prepared this report in a manner consistent with that level of care and skill ordinarily exercised by members of the engineering and science professions currently practicing under similar conditions in the jurisdiction in which the services are provided, subject to the time limits and physical constraints applicable to this report. No other warranty, expressed or implied is made.

**Basis and Use of the Report:** This report has been prepared for the specific site, design objective, development and purpose described to Golder by the Client, **Claridge Homes Corporation.** The factual data, interpretations and recommendations pertain to a specific project as described in this report and are not applicable to any other project or site location. Any change of site conditions, purpose, development plans or if the project is not initiated within eighteen months of the date of the report may alter the validity of the report. Golder cannot be responsible for use of this report, or portions thereof, unless Golder Ts requested to review and, if necessary, revise the report.

The information, recommendations and opinions expressed in this report are for the sole benefit of the Client. No other party may use or rely on this report or any portion thereof without Golder's express written consent. If the report was prepared to be included for a specific permit application process, then the client may authorize the use of this report for such purpose by the regulatory agency as an Approved User for the specific and identified purpose of the applicable permit review process, provided this report is not noted to be a draft or preliminary report, and is specifically relevant to the project for which the application is being made. Any other use of this report by others is prohibited and is without responsibility to Golder. The report, all plans, data, drawings and other documents as well as all electronic media prepared by Golder are considered its professional work product and shall remain the copyright property of Golder, who authorizes only the Client and Approved Users to make copies of the report, but only in such quantities as are reasonably necessary for the use of the report by those parties. The Client and Approved Users may not give, lend, sell, or otherwise make available the report or any portion thereof to any other party without the express written permission of Golder. The Client acknowledges that electronic media is susceptible to unauthorized modification, deterioration and incompatibility and therefore the Client cannot rely upon the electronic media versions of Golder's report or other work products.

The report is of a summary nature and is not intended to stand alone without reference to the instructions given to Golder by the Client, communications between Golder and the Client, and to any other reports prepared by Golder for the Client relative to the specific site described in the report. In order to properly understand the suggestions, recommendations and opinions expressed in this report, reference must be made to the whole of the report. Golder cannot be responsible for use of portions of the report without reference to the entire report.

Unless otherwise stated, the suggestions, recommendations and opinions given in this report are intended only for the guidance of the Client in the design of the specific project. The extent and detail of investigations, including the number of test holes, necessary to determine all of the relevant conditions which may affect construction costs would normally be greater than has been carried out for design purposes. Contractors bidding on, or undertaking the work, should rely on their own investigations, as well as their own interpretations of the factual data presented in the report, as to how subsurface conditions may affect their work, including but not limited to proposed construction techniques, schedule, safety and equipment capabilities.

Soil, Rock and Groundwater Conditions: Classification and identification of soils, rocks, and geologic units have been based on commonly accepted methods employed in the practice of geotechnical engineering and related disciplines. Classification and identification of the type and condition of these materials or units involves judgment, and boundaries between different soil, rock or geologic types or units may be transitional rather than abrupt. Accordingly, Golder does not warrant or guarantee the exactness of the descriptions.

# IMPORTANT INFORMATION AND LIMITATIONS OF THIS REPORT (cont'd)

Special risks occur whenever engineering or related disciplines are applied to identify subsurface conditions and even a comprehensive investigation, sampling and testing program may fail to detect all or certain subsurface conditions. The environmental, geologic, geotechnical, geochemical and hydrogeologic conditions that Golder interprets to exist between and beyond sampling points may differ from those that actually exist. In addition to soil variability, fill of variable physical and chemical composition can be present over portions of the site or on adjacent properties. **The professional services retained for this project include only the geotechnical aspects of the subsurface conditions at the site, unless otherwise specifically stated and identified in the report.** The presence or implication(s) of possible surface and/or subsurface contamination resulting from previous activities or uses of the site and/or resulting from the introduction onto the site of materials from off-site sources are outside the terms of reference for this project and have not been investigated or addressed.

Soil and groundwater conditions shown in the factual data and described in the report are the observed conditions at the time of their determination or measurement. Unless otherwise noted, those conditions form the basis of the recommendations in the report. Groundwater conditions may vary between and beyond reported locations and can be affected by annual, seasonal and meteorological conditions. The condition of the soil, rock and groundwater may be significantly altered by construction activities (traffic, excavation, groundwater level lowering, pile driving, blasting, etc.) on the site or on adjacent sites. Excavation may expose the soils to changes due to wetting, drying or frost. Unless otherwise indicated the soil must be protected from these changes during construction.

**Sample Disposal:** Golder will dispose of all uncontaminated soil and/or rock samples 90 days following issue of this report or, upon written request of the Client, will store uncontaminated samples and materials at the Client's expense. In the event that actual contaminated soils, fills or groundwater are encountered or are inferred to be present, all contaminated samples shall remain the property and responsibility of the Client for proper disposal.

**Follow-Up and Construction Services:** All details of the design were not known at the time of submission of Golder's report. Golder should be retained to review the final design, project plans and documents prior to construction, to confirm that they are consistent with the intent of Golder's report.

During construction, Golder should be retained to perform sufficient and timely observations of encountered conditions to confirm and document that the subsurface conditions do not materially differ from those interpreted conditions considered in the preparation of Golder's report and to confirm and document that construction activities do not adversely affect the suggestions, recommendations and opinions contained in Golder's report. Adequate field review, observation and testing during construction are necessary for Golder to be able to provide letters of assurance, in accordance with the requirements of many regulatory authorities. In cases where this recommendation is not followed, Golder's responsibility is limited to interpreting accurately the information encountered at the borehole locations, at the time of their initial determination or measurement during the preparation of the Report.

Changed Conditions and Drainage: Where conditions encountered at the site differ significantly from those anticipated in this report, either due to natural variability of subsurface conditions or construction activities, it is a condition of this report that Golder be notified of any changes and be provided with an opportunity to review or revise the recommendations within this report. Recognition of changed soil and rock conditions requires experience and it is recommended that Golder be employed to visit the site with sufficient frequency to detect if conditions have changed significantly.

Drainage of subsurface water is commonly required either for temporary or permanent installations for the project. Improper design or construction of drainage or dewatering can have serious consequences. Golder takes no responsibility for the effects of drainage unless specifically involved in the detailed design and construction monitoring of the system.



#### TABLE 1

# SOME COMMON TREES IN DECREASING ORDER OF WATER DEMAND

#### **Broad Leaved Deciduous**

Poplar

Alder

Aspen

Willow

Elm

Maple

Birch

Ash

Beech

Oak

## **Deciduous Conifer**

Larch

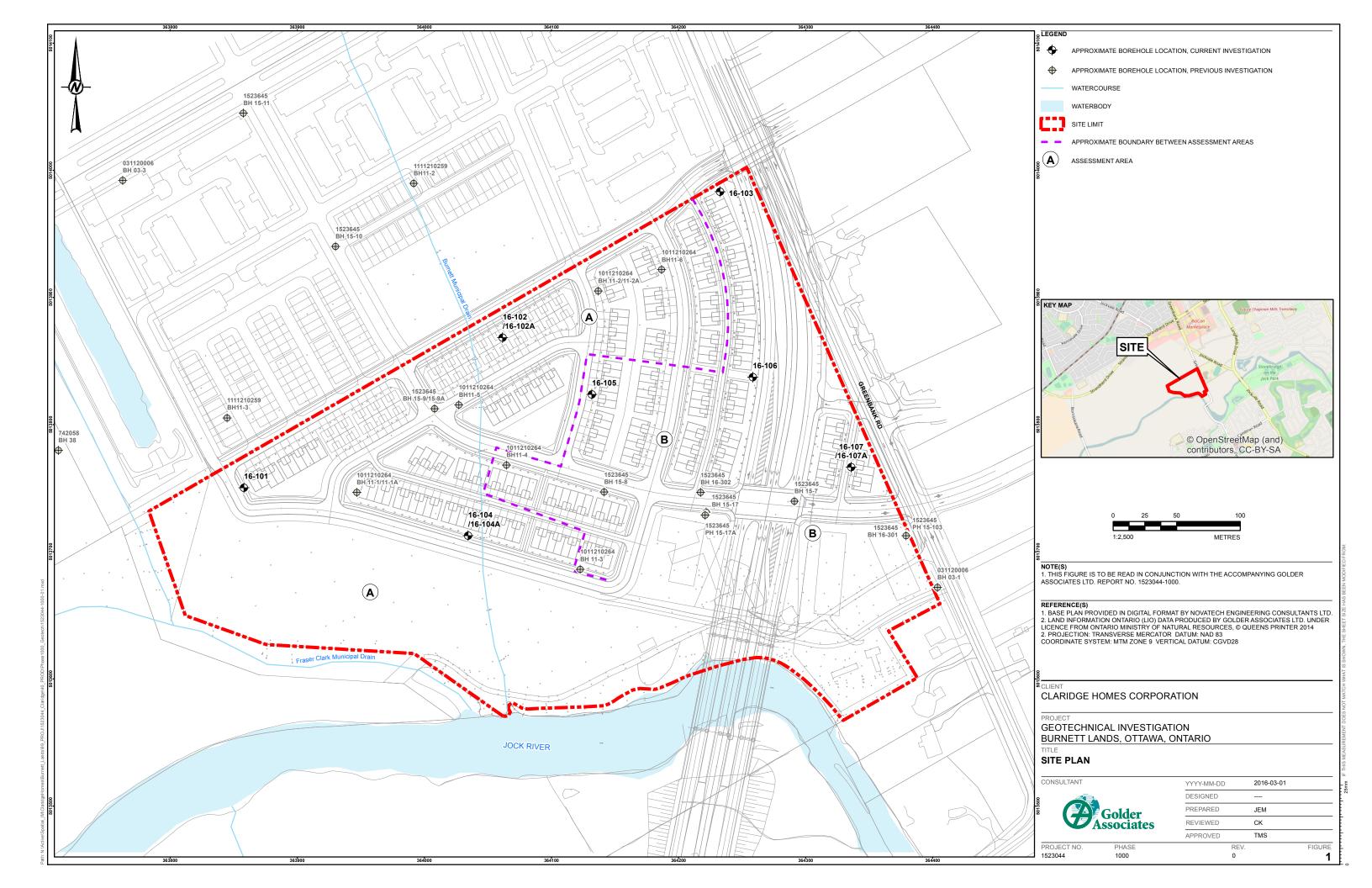
# **Evergreen Conifers**

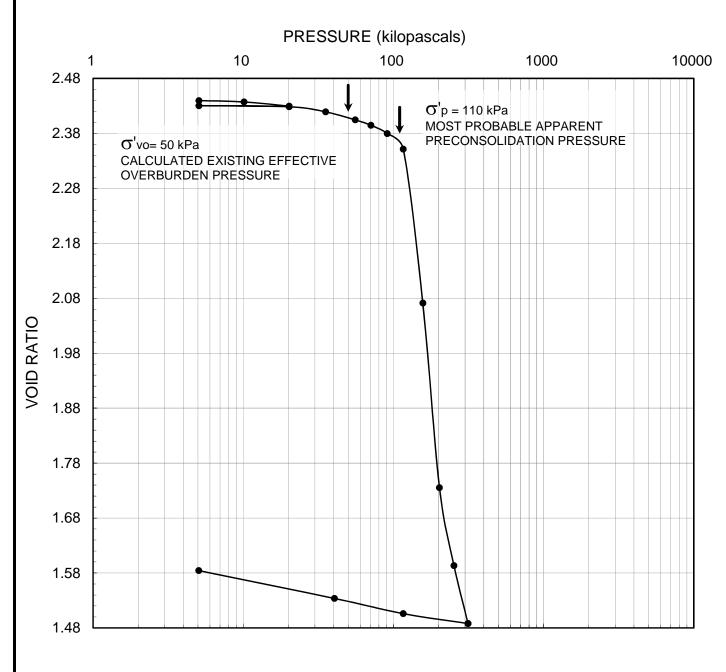
Spruce

Fir

Pine







# **LEGEND**

Borehole: 16-104A  $W_i = 86\%$ 

 $S_0 = 99\%$ 

 $\gamma = 14.9 \text{ kN/m}^3$ 

Sample: 1

 $W_f = 57\%$ 

 $e_0 = 2.44$ 

 $G_s = 2.81$ 

Depth (m): 5.0

 $W_1 = 75\%$ 

 $C_c = 3.07$ 

Elevation (m): 86.3

 $W_p = 25\%$ 

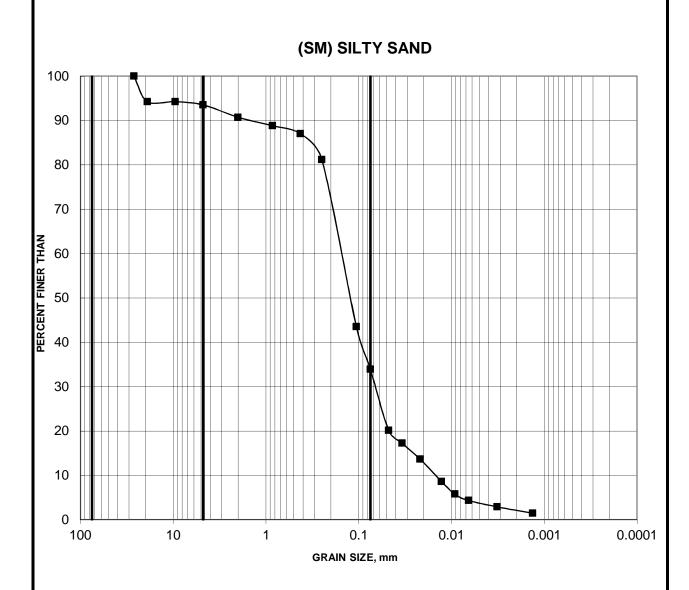
 $C_r = 0.003$ 



PROJECT No.

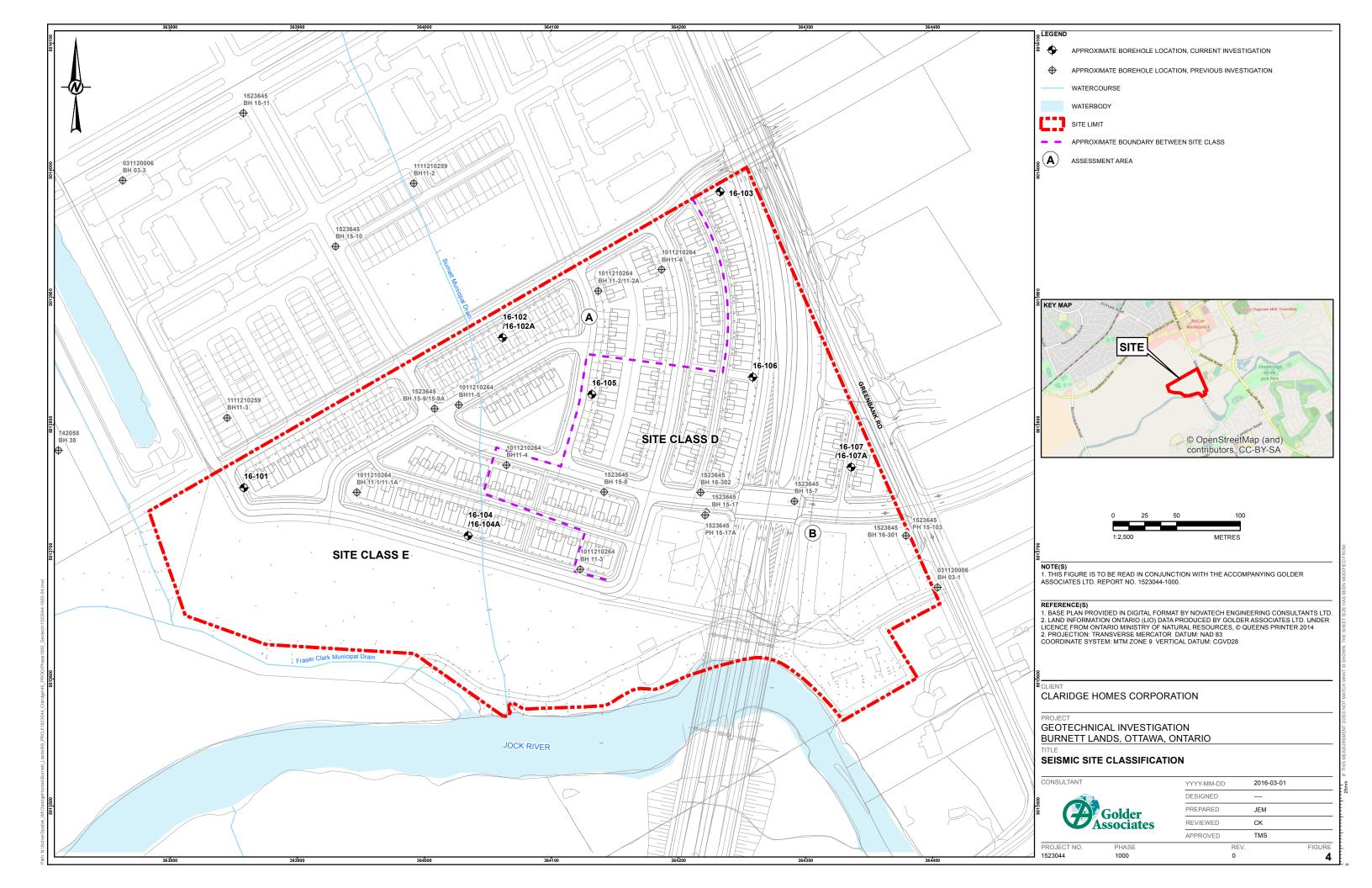
| SCALE   | AS SHOWN | TITLE |
|---------|----------|-------|
| DATE    | 04/19/16 |       |
| CADD    | N/A      | C     |
| ENTERED | CW       |       |
| CHECK   | CNIM     |       |

| Consolidatio | n summar      | CHECK  | CNM    |        |   |
|--------------|---------------|--------|--------|--------|---|
| Consolidatio | ili Sullillai | ,      | CIVIVI | FIGURI |   |
| 1523044 F    | REV.          | REVIEW | CK     |        | 2 |



| Cobble | coarse        | fine | coarse                | medium | fine | SILT AND CLAY |
|--------|---------------|------|-----------------------|--------|------|---------------|
| Size   | e GRAVEL SIZE |      | GRAVEL SIZE SAND SIZE |        | ZE   | SILT AND CLAT |
|        |               |      |                       |        |      |               |

| Borehole          | Sample | Depth (m) |
|-------------------|--------|-----------|
| <b>-■</b> -16-106 | 8      | 4.57-5.18 |
|                   |        |           |







# **APPENDIX A**

List of Abbreviations and Symbols Record of Borehole Sheets Current Investigation





# ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES AND TEST PITS

#### PARTICLE SIZES OF CONSTITUENTS

| Soil        | Particle Size            | Millimetres    | Inches               |
|-------------|--------------------------|----------------|----------------------|
| Constituent | Description              |                | (US Std. Sieve Size) |
| BOULDERS    | Not<br>Applicable        | >300           | >12                  |
| COBBLES     | Not<br>Applicable        | 75 to 300      | 3 to 12              |
| GRAVEL      | Coarse                   | 19 to 75       | 0.75 to 3            |
|             | Fine                     | 4.75 to 19     | (4) to 0.75          |
| SAND        | Coarse                   | 2.00 to 4.75   | (10) to (4)          |
|             | Medium                   | 0.425 to 2.00  | (40) to (10)         |
|             | Fine                     | 0.075 to 0.425 | (200) to (40)        |
| SILT/CLAY   | Classified by plasticity | <0.075         | < (200)              |

#### MODIFIERS FOR SECONDARY AND MINOR CONSTITUENTS

|                    | N GEGGREART AND MINIOR GGREATH GERTS   |  |  |  |  |  |  |  |
|--------------------|--|--|--|--|--|--|--|--|
| Percentage by Mass | Modifier   |  |  |  |  |  |  |  |
| >35                | Use 'and' to combine major constituents (i.e., SAND and GRAVEL, SAND and CLAY) |  |  |  |  |  |  |  |
| > 12 to 35         | Primary soil name prefixed with "gravelly, sandy, SILTY, CLAYEY" as applicable |  |  |  |  |  |  |  |
| > 5 to 12          | some   |  |  |  |  |  |  |  |
| ≤ 5                | trace  |  |  |  |  |  |  |  |

#### PENETRATION RESISTANCE

#### Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) split-spoon sampler for a distance of 300 mm (12 in.).

#### **Cone Penetration Test (CPT)**

An electronic cone penetrometer with a  $60^{\circ}$  conical tip and a project end area of  $10~\text{cm}^2$  pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (q<sub>i</sub>), porewater pressure (u) and sleeve frictions are recorded electronically at 25 mm penetration intervals.

#### Dynamic Cone Penetration Resistance (DCPT); N<sub>d</sub>:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter,  $60^{\circ}$  cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

PH: Sampler advanced by hydraulic pressure
PM: Sampler advanced by manual pressure
WH: Sampler advanced by static weight of hammer
WR: Sampler advanced by weight of sampler and rod

#### SAMPLES

| AS       | Auger sample   |
|----------|--|
| BS       | Block sample   |
| CS       | Chunk sample   |
| DO or DP | Seamless open ended, driven or pushed tube sampler – note size |
| DS       | Denison type sample  |
| FS       | Foil sample  |
| RC       | Rock core  |
| SC       | Soil core  |
| SS       | Split spoon sampler – note size                                |
| ST       | Slotted tube   |
| ТО       | Thin-walled, open – note size                                  |
| TP       | Thin-walled, piston – note size                                |
| WS       | Wash sample  |

#### SOIL TESTS

| SUIL TESTS          |   |
|---------------------|---|
| w                   | water content   |
| PL , w <sub>p</sub> | plastic limit   |
| $LL$ , $W_L$        | liquid limit  |
| С                   | consolidation (oedometer) test  |
| CHEM                | chemical analysis (refer to text)   |
| CID                 | consolidated isotropically drained triaxial test <sup>1</sup>                                       |
| CIU                 | consolidated isotropically undrained triaxial test with porewater pressure measurement <sup>1</sup> |
| D <sub>R</sub>      | relative density (specific gravity, Gs)   |
| DS                  | direct shear test   |
| GS                  | specific gravity  |
| М                   | sieve analysis for particle size  |
| MH                  | combined sieve and hydrometer (H) analysis  |
| MPC                 | Modified Proctor compaction test  |
| SPC                 | Standard Proctor compaction test  |
| OC                  | organic content test  |
| SO <sub>4</sub>     | concentration of water-soluble sulphates  |
| UC                  | unconfined compression test   |
| UU                  | unconsolidated undrained triaxial test  |
| V (FV)              | field vane (LV-laboratory vane test)  |
| γ                   | unit weight   |

 Tests which are anisotropically consolidated prior to shear are shown as CAD, CAU.

#### NON-COHESIVE (COHESIONLESS) SOILS

#### Compactness<sup>2</sup>

| Term       | SPT 'N' (blows/0.3m) <sup>1</sup> |
|------------|-----------------------------------|
| Very Loose | 0 - 4                             |
| Loose      | 4 to 10                           |
| Compact    | 10 to 30                          |
| Dense      | 30 to 50                          |
| Very Dense | >50                               |

SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects.
 Definition of compactness descriptions based on SPT 'N' ranges from

#### Field Moisture Condition

| Term  | Description   |
|-------|---|
| Dry   | Soil flows freely through fingers.                            |
| Moist | Soils are darker than in the dry condition and may feel cool. |
| Wet   | As moist, but with free water forming on hands when handled.  |

#### COHESIVE SOILS

|            | Consistency                       |                                      |
|------------|-----------------------------------|--------------------------------------|
| Term       | Undrained Shear<br>Strength (kPa) | SPT 'N' <sup>1</sup><br>(blows/0.3m) |
| Very Soft  | <12                               | 0 to 2                               |
| Soft       | 12 to 25                          | 2 to 4                               |
| Firm       | 25 to 50                          | 4 to 8                               |
| Stiff      | 50 to 100                         | 8 to 15                              |
| Very Stiff | 100 to 200                        | 15 to 30                             |
| Hard       | >200                              | >30                                  |

 SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects; approximate only.

#### Water Content

| Term   | Description  |
|--------|--|
| w < PL | Material is estimated to be drier than the Plastic Limit.  |
| w ~ PL | Material is estimated to be close to the Plastic Limit.    |
| w > PL | Material is estimated to be wetter than the Plastic Limit. |

January 2013 G-2



Definition of compactness descriptions based on SPT 'N' ranges fron Terzaghi and Peck (1967) and correspond to typical average N<sub>60</sub> values.



# **LIST OF SYMBOLS**

Unless otherwise stated, the symbols employed in the report are as follows:

| I.                             | GENERAL   | (a)<br>w                         | Index Properties (continued) water content                                   |
|--------------------------------|---|----------------------------------|--|
| π                              | 3.1416  | w <sub>i</sub> or LL             | liquid limit   |
| ln x                           | natural logarithm of x  | $w_p$ or PL                      | plastic limit  |
| log <sub>10</sub>              | x or log x, logarithm of x to base 10   | I <sub>p</sub> or PI             | plasticity index = $(w_l - w_p)$   |
| g<br>t                         | acceleration due to gravity   | Ws<br>L                          | shrinkage limit  |
| ι                              | time  | I <sub>L</sub><br>I <sub>C</sub> | liquidity index = $(w - w_p) / I_p$<br>consistency index = $(w_l - w) / I_p$ |
|                                |   | e <sub>max</sub>                 | void ratio in loosest state  |
|                                |   | $e_{min}$                        | void ratio in densest state  |
|                                |   | $I_D$                            | density index = $(e_{max} - e) / (e_{max} - e_{min})$                        |
| II.                            | STRESS AND STRAIN   |                                  | (formerly relative density)  |
| γ                              | shear strain  | (b)                              | Hydraulic Properties   |
| Δ                              | change in, e.g. in stress: $\Delta \sigma$                                      | h                                | hydraulic head or potential  |
| 3                              | linear strain volumetric strain   | q                                | rate of flow velocity of flow  |
| ε <sub>v</sub>                 | coefficient of viscosity  | v<br>i                           | hydraulic gradient   |
| η<br>υ                         | Poisson's ratio   | k                                | hydraulic conductivity   |
| σ                              | total stress  |                                  | (coefficient of permeability)  |
| σ′                             | effective stress ( $\sigma' = \sigma - u$ )                                     | j                                | seepage force per unit volume  |
| $\sigma'_{vo}$                 | initial effective overburden stress   |                                  |  |
| $\sigma_1,  \sigma_2,$         | principal stress (major, intermediate,  |                                  |  |
| $\sigma_3$                     | minor)  | (c)                              | Consolidation (one-dimensional)  |
|                                |   | Cc                               | compression index  |
| $\sigma_{\rm oct}$             | mean stress or octahedral stress  | $C_{r}$                          | (normally consolidated range) recompression index                            |
| σ.                             | = $(\sigma_1 + \sigma_2 + \sigma_3)/3$<br>shear stress                          | C <sub>r</sub>                   | (over-consolidated range)  |
| τ<br>u                         | porewater pressure  | $C_s$                            | swelling index   |
| Ĕ                              | modulus of deformation  | Cα                               | secondary compression index  |
| G                              | shear modulus of deformation  | $m_v$                            | coefficient of volume change   |
| K                              | bulk modulus of compressibility   | C <sub>V</sub>                   | coefficient of consolidation (vertical direction)                            |
|                                |   | C <sub>h</sub>                   | coefficient of consolidation (horizontal direction)                          |
| III.                           | COUL DRODEDTIES   | T <sub>v</sub>                   | time factor (vertical direction)   |
| III.                           | SOIL PROPERTIES   | U<br>~'                          | degree of consolidation<br>pre-consolidation stress                          |
| (a)                            | Index Properties  | σ′ <sub>p</sub><br>OCR           | over-consolidation ratio = $\sigma'_p / \sigma'_{vo}$                        |
| ρ(γ)                           | bulk density (bulk unit weight)*  | 33.1                             |  |
| $\rho_d(\gamma_d)$             | dry density (dry unit weight)   | (d)                              | Shear Strength   |
| $\rho_{\rm w}(\gamma_{\rm w})$ | density (unit weight) of water  | $\tau_p,  \tau_r$                | peak and residual shear strength   |
| $\rho_{\rm s}(\gamma_{\rm s})$ | density (unit weight) of solid particles  | φ′<br>δ                          | effective angle of internal friction   |
| $\gamma'$                      | unit weight of submerged soil   |                                  | angle of interface friction  |
| D-                             | $(\gamma' = \gamma - \gamma_w)$<br>relative density (specific gravity) of solid | μ<br>C'                          | coefficient of friction = $tan \delta$ effective cohesion                    |
| $D_R$                          | particles ( $D_R = \rho_s / \rho_w$ ) (formerly $G_s$ )                         | Cu, Su                           | undrained shear strength ( $\phi = 0$ analysis)                              |
| е                              | void ratio  | p                                | mean total stress $(\sigma_1 + \sigma_3)/2$                                  |
| n                              | porosity  | p'                               | mean effective stress $(\sigma_1' + \sigma_3)/2$                             |
| S                              | degree of saturation  | q                                | $(\sigma_1 - \sigma_3)/2$ or $(\sigma'_1 - \sigma'_3)/2$                     |
|                                |   | $q_{\rm u}$                      | compressive strength $(\sigma_1 - \sigma_3)$                                 |
|                                |   | $S_{t}$                          | sensitivity  |
| * Dens                         | ity symbol is $\rho$ . Unit weight symbol is $\gamma$                           | Notes: 1                         | $\tau = c' + \sigma' \tan \phi'$   |
| where                          | e $\gamma = \rho g$ (i.e. mass density multiplied by                            | 2                                | shear strength = (compressive strength)/2                                    |
| accel                          | eration due to gravity)   |                                  |  |

January 2013 G-3



# RECORD OF BOREHOLE: 16-101

SHEET 1 OF 1

LOCATION: See Site Plan

BORING DATE: February 23, 2016

DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| DEPTH SCALE METRES SORING METHOD |   | SOIL PROFILE  | SAMPLES DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m |                |        |      |             |                 |            | ,    | IIIDKA          | k, cm/s            | ONDUCT | IVIII,  |            | 2 بـ | PIEZOMETER |                |  |                    |                 |                  |                            |    |
|----------------------------------|---|---|--|----------------|--------|------|-------------|-----------------|------------|------|-----------------|--------------------|--------|---------|------------|------|------------|----------------|--|--------------------|-----------------|------------------|----------------------------|----|
| SCA                              | BORING METHOD                             |   | , LOT  |                |        |      |             | LOT             |            |      |                 | 30m                | 2      | 0 4     | 0          | 60   | 80         | 10             |  | ) <sup>-5</sup> 10 | 0 <sup>-4</sup> | 10 <sup>-3</sup> | ADDITIONAL<br>LAB. TESTING | OR |
| AET E                            | ING                                       | DESCRIPTION   | STRATA PLOT  | ELEV.<br>DEPTH | NUMBER | TYPE | BLOWS/0.30m | SHEAF<br>Cu. kP | STREN      | IGTH | nat V.<br>rem V | + Q - ●<br>⊕ U - ○ | WA     | ATER CO |            |      |            | DOIT<br>B. TE  | STANDPIPE<br>INSTALLATION                              |                    |                 |                  |                            |    |
| DE                               | BOR                                       |   | STRA   | (m)            | ⊋      |      | 3LOV        |                 |            |      |                 |                    | vvp    |         |            |      | WI         | <del>Z</del> Z |  |                    |                 |                  |                            |    |
|                                  |   | GROUND SURFACE  | - 0,   | 91.82          |        |      |             | 2               | υ <u>4</u> | 0    | 60              | 80                 | 20     | ) 4     | <u>u b</u> | 0    | 80         |                |  |                    |                 |                  |                            |    |
| - 0                              |   | TOPSOIL - (ML) sandy SILT; brown                                      | EEE  | 0.00           |        | GRAE | -           |                 |            |      |                 |                    |        |         |            |      |            |                | ij   |                    |                 |                  |                            |    |
|                                  |   | (CI/CH) SILTY CLAY to CLAY, trace sand; grey brown (Weathered Crust); |  | 0.15           |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   | cohesive, w>PL, stiff to very stiff                                   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                | 2      | GRAE | -           |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| . 1                              |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| ·                                |   |   |  |                | 3      | SS   | 4           |                 |            |      |                 |                    |        | 0       |            |      |            |                | <u></u> ¥₩   |                    |                 |                  |                            |    |
|                                  |   |   |  |                | _      |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                | 4      | SS   | 4           |                 |            |      |                 |                    |        | d       | )          |      |            |                | Native Backfill  |                    |                 |                  |                            |    |
| 2                                |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 | >96                |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| ١                                |   |   |  | 88.77          |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| 3                                |   | (CI/CH) SILTY CLAY to CLAY; grey; cohesive, w>PL, firm                |  | 3.05           |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                | V V V V V V V V V V V V V V V V V V V                  |                    |                 |                  |                            |    |
|                                  | stem)                                     | conesive, w>PL, firm  |  |                | 5      | SS   | 1           |                 |            |      |                 |                    |        |         | 0          |      |            |                |  |                    |                 |                  |                            |    |
|                                  | Power Auger<br>200 mm Diam. (Hollow Stem) |   |  |                | L      |      |             |                 |            |      |                 |                    |        |         |            |      |            |                | Bentonite Seal   |                    |                 |                  |                            |    |
|                                  | Power Auger<br>Diam. (Hollo               |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                | Demonite Seal  |                    |                 |                  |                            |    |
| 4                                | Pow<br>n Diar                             |   |  |                |        |      |             | Φ               |            | +    |                 |                    |        |         |            |      |            |                | â  |                    |                 |                  |                            |    |
|                                  | 00 mr                                     |   |  |                |        |      |             | <b>⊕</b>        | +          |      |                 |                    |        |         |            |      |            |                | Silica Sand  |                    |                 |                  |                            |    |
|                                  | 2   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| . 5                              |   |   |  |                | 6      | ss   | РН          |                 |            |      |                 |                    |        |         |            | 0    |            |                |  |                    |                 |                  |                            |    |
| . 5                              |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                | 51 mm Diam. PVC<br>#10 Slot Screen                     |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             | Φ               | +          |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             | Φ               | +          |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| - 6                              |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                | W.L. in Screen at                                      |                    |                 |                  |                            |    |
|                                  |   |   |  |                | 7      | SS   | WH          |                 |            |      |                 |                    |        |         |            |      |            |                | W.L. in Screen at<br>Elev. 90.73 m on<br>March 7, 2016 |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        | _    |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| _                                |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| - 7                              |   |   |  |                |        |      |             | Φ               | +          |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             | Φ               | +          |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   | End of Borehole   |  | 84.20<br>7.62  |        |      |             | Φ               | +          |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   | End of Dorellole  |  | 7.02           |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| - 8                              |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| . 9                              |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| Ĭ                                |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| 10                               |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
|                                  |   |   |  |                |        |      |             |                 |            |      |                 |                    |        |         |            |      |            |                |  |                    |                 |                  |                            |    |
| DF                               | PTH S                                     | SCALE   |  |                |        |      | 4           |                 |            |      |                 |                    |        |         |            |      |            | 10             | OGGED: CG  |                    |                 |                  |                            |    |
| DE                               | 1113                                      |   |  |                |        |      | - (         |                 | GG<br>Ass  | Alde | r.              |                    |        |         |            |      |            | L              | JULES. UU  |                    |                 |                  |                            |    |

# RECORD OF BOREHOLE: 16-102

SHEET 1 OF 1

LOCATION: See Site Plan

SAMPLER HAMMER, 64kg; DROP, 760mm

BORING DATE: February 22, 2016

DATUM: Geodetic

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| » ALE  | ,<br>G        |           | SOIL PROFILE   | L           | ı                     | SA     | MPL         |             | DYNAMIC PENETI<br>RESISTANCE, BL  |               | ,                        |    | k, cm/s | NDUCTIVI |    | NG NG                      | PIEZOMETER                      |
|--------|---------------|-----------|--|-------------|-----------------------|--------|-------------|-------------|-----------------------------------|---------------|--------------------------|----|---------|----------|----|----------------------------|---------------------------------|
| METRES | PODING METHOD |           | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE        | BLOWS/0.30m | 20 40<br>SHEAR STRENGT<br>Cu, kPa |               | 80<br>+ Q - ●<br>∌ U - ○ | Wp | TER CON | NTENT PE | WI | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
|        |               | _         | GROUND SURFACE   | S           | <u> </u>              |        | $\vdash$    | В           | 20 40                             | 60            | 80                       | 20 | 40      | 60       | 80 | ++                         |                                 |
| 0      | $\vdash$      | Н         | TOPSOIL - (ML) CLAYEY SILT; dark   |             | 92.01                 | 1      | GRAB        |             |                                   |               |                          |    |         |          | +  | ++                         |                                 |
|        |               |           | brown (CI/CH) SILTY CLAY to CLAY; grey   |             | 91.78<br>0.23         |        | GRAB        |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           | brown (Weathered Crust); cohesive, w>PL, very stiff  |             |                       |        | GKAB        | -           |                                   |               |                          |    |         |          |    |                            |                                 |
| 1      |               |           |  |             | 90.49                 | 3      | ss          | 6           |                                   |               |                          |    |         |          |    |                            |                                 |
| 2      |               |           | (CI/CH) SILTY CLAY to CLAY; grey; cohesive, w~PL, firm   |             | 1.52                  | 4      | SS          | 1           |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             | ⊕ +                               |               |                          |    |         |          |    |                            |                                 |
| 3      |               |           | (SM) SILTY SAND, some gravel; grey,  |             | 88.96<br>3.05         |        |             | D           | <b>+</b> +                        |               |                          |    |         |          |    |                            |                                 |
|        | Auger         |           | (SM) SILTY SAND, some gravel; grey, contains cobbles and boulders (GLACIAL TILL); non-cohesive, wet, very loose to compact |             |                       | 5      | TP          | PH          |                                   |               |                          |    |         |          |    |                            |                                 |
| 4      | ower          | Jiam.,    |  |             | 1                     |        | 1           |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 7      | Power Auger   | 200 mm    |  |             |                       | 6      | SS          | 7           |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        | 1           |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 5      |               |           |  |             |                       | 7      | ss          | 4           |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        | 1           |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       | 8      | ss          | 8           |                                   |               |                          |    |         |          |    |                            |                                 |
| •      |               |           |  |             |                       |        | .           |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 6      |               |           |  |             |                       |        | 1           |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       | 9      | ss          | 17          |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       | -      |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        | 1           |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 7      |               |           |  |             | 1                     | _      | $  \cdot  $ |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       | 10     | ss          | 2           |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             | 84.39                 |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        | П             | $\exists$ | End of Borehole  | 1/17%       | 7.62                  |        | 1           |             |                                   |               |                          |    |         |          |    |                            |                                 |
| . 8    |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
| Ū      |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 9      |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 10     |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
| 10     |               |           |  |             |                       |        |             |             |                                   |               |                          |    |         |          |    |                            |                                 |
|        |               |           |  |             | I                     |        | ш           |             |                                   |               |                          | L  |         |          |    |                            |                                 |
| DE     | PTI           | H S       | CALE   |             |                       |        |             | 1           | COL                               | der<br>ciates |                          |    |         |          |    | LOC                        | GGED: DG                        |
| 1:     | 50            |           |  |             |                       |        |             | 1           | ASSO                              | ciatos        |                          |    |         |          |    | CHE                        | CKED: CK                        |

# RECORD OF BOREHOLE: 16-102A

SHEET 1 OF 1 DATUM: Geodetic

BORING DATE: February 22, 2016 LOCATION: See Site Plan

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| ا پ                   | 무   | SOIL PROFILE  | 1, 1        | s            | AMPL |             | DYNAMIC PENETRATION RESISTANCE, BLOWS | 0.3m                           |                  | cm/s             |         |                  | NG &                       | PIEZOMETER      |
|-----------------------|---|---|-------------|--------------|------|-------------|---------------------------------------|--------------------------------|------------------|------------------|---------|------------------|----------------------------|-----------------|
| DEPTH SCALE<br>METRES | BORING METHOD                             |   | STRATA PLOT | _   #        |      | BLOWS/0.30m |                                       | 0 80                           | 10 <sup>-6</sup> | 10 <sup>-5</sup> | 10-4    | 10 <sup>-3</sup> | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE |
| Ē                     | ING                                       | DESCRIPTION   | TA F        | EV. PTH MBER | TYPE | /S/0        | SHEAR STRENGTH r<br>Cu, kPa r         | at V. + Q - ●<br>em V. ⊕ U - ○ | WATE             | R CONTE          | NT PERC | ENT              | DDI1                       | INSTALLATION    |
| 7                     | BOR                                       |   | TRA (       | m) ∏ ≥       | [    | 3101        |                                       |                                | Wp ⊢             |                  |         |                  | 44                         |                 |
| $\dashv$              |   | GROUND SURFACE  |             | -            | +    | ш           | 20 40 6                               | 0 80                           | 20               | 40               | 60      | 80               | ++                         |                 |
| 0                     |   | TOPSOIL - (ML) CLAYEY SILT; dark                                    | E551        | 0.00         | +    |             |                                       |                                |                  |                  |         |                  | ++                         |                 |
|                       |   | brown   | ت (بربرب    | 0.23         |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   | (CI/CH) SILTY CLAY to CLAY; grey brown (Weathered Crust); cohesive, |             | 0.20         |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   | w>PL, very stiff  |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       | ĺε  |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 1                     | Ste                                       |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       | Power Auger<br>Diam. (Holloy              |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       | wer A                                     |   |             | 0.49         |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       | Power Auger<br>200 mm Diam. (Hollow Stem) | (CI/CH) SILTY CLAY to CLAY; grey; cohesive, w~PL, firm              |             | 1.52         |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       | 8   | conesive, w~PL, firm  |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 2                     | 2   | 1   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             | 1            | TP   | PH          |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             | 39.27        |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| ŀ                     |   | End of Borehole   |             | 2.74         |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 3                     |   | Note: Soil stratigraphy inferred from BH                            |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   | 16-102  |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 4                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 5                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 6                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 7                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 8                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 9                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
|                       |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| 10                    |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| Ш                     |   |   |             |              |      |             |                                       |                                |                  |                  |         |                  |                            |                 |
| DEF                   | PTH                                       | SCALE   |             |              |      | 4           | Golder                                | _                              |                  |                  |         |                  | LOC                        | GGED: DG        |
|                       |   |   |             |              |      |             | <b>Gorde</b> i                        | -                              |                  |                  |         |                  |                            | CKED: CK        |

# RECORD OF BOREHOLE: 16-103

SHEET 1 OF 1

LOCATION: See Site Plan

BORING DATE: February 19, 2016

DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| 1 TO bro (Ci. bro w>)  (Sh nor (Gi. (Sh cor (Gi. (Gi. (Gi. (Gi. (Gi. (Gi. (Gi. (Gi.               | DESCRIPTION  ROUND SURFACE  OPSOIL - (ML) CLAYEY SILT; dark rown  CIVCH) SILTY CLAY to CLAY; grey rown (Weathered Crust); cohesive, PPL, very stiff  SM) SILTY SAND, some gravel; brown; on-cohesive, wet, loose  SM) SILTY SAND, some gravel; grey, ontains cobbles and boulders GLACIAL TILL); non-cohesive, wet, loose to very dense | STRATA  | 0.15  | SRAE SSS SS SS SS SS SS | 10 4 4 | 20 40 60 80  SHEAR STRENGTH nat V. + Q - ● Cu, kPa  20 40 60 80  20 40 60 80 | 10 <sup>6</sup> 10 <sup>5</sup> 10 <sup>4</sup> 10 <sup>3</sup> WATER CONTENT PERCENT Wp | PIEZOMETER OR STANDPIPE INSTALLATION  Bentonite Seal  Silica Sand  51 mm Diam. PVC #10 Slot Screen  Silica Sand  W.L. in Screen at Elev. 92.60 m on March 7, 2016 |
|---|---|---------|---|-------------------------|--------|--|--|---|
| TO book (Ci. brown North Magnet Auger (Alollow Stem) (Alollow Stem) 2000 mm Diam. (Hollow Stem) 4 | OPSOIL - (ML) CLAYEY SILT; dark rown  CI/CH) SILTY CLAY to CLAY; grey rown (Weathered Crust); cohesive,  PL, very stiff  SM) SILTY SAND, some gravel; brown; on-cohesive, wet, loose  SM) SILTY SAND, some gravel; grey, ontains cobbles and boulders  SIACIAL TILL); non-cohesive, wet,  | 93      | 0.00 1 2 2 2 3 3 3 3 3 3 3 4 1.69 4 5 5 5 5 6 6 | SRAE SSS SS SS SS SS SS | 10 4   |  | 0  | Silica Sand  51 mm Diam. PVC #10 Slot Screen  |
| 200 mm Diam. (Hollow Stem)  200 mm Diam. (Hollow Stem)  | SM) SILTY SAND, some gravel; grey, ontains cobbles and boulders GLACIAL TILL); non-cohesive, wet,   | 9       | 91.69<br>1.82<br>4<br>91.38<br>2.13             | SS SS SS                | 11 4   |  |  | 51 mm Diam. PVC<br>#10 Slot Screen  |
| Power Auger<br>200 mm Diam. (Hollow Stem)   |   |         |   |                         |        |  |  | W.L. in Screen at<br>Elev. 92.60 m on<br>March 7. 2016  |
|   |   | (4/1/4) | 8   | ss                      |        |  |  |   |
|   |   |         | 9   |                         |        |  |  |   |
| Enc<br>Aug  | ind of Borehole<br>uger Refusal   |         | 1:<br>7.26                                      | 1 SS                    | >50    |  |  |   |
| 9   |   |         |   |                         |        |  |  |   |

# RECORD OF BOREHOLE: 16-104

SHEET 1 OF 1

LOCATION: See Site Plan BORING DATE: February 23, 2016

DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| DEPTH SCALE<br>METRES | ИЕТНОВ                                    | SOIL PROFILE   | LOT         |                       |        | MPLI |             | DYNAMI<br>RESIST |   | NETRA<br>, BLOW<br>40 | FION<br>/S/0.3m<br>60 | ,<br>80 | ,                     |   | AULIC C<br>k, cm/s<br>0 <sup>-6</sup> 1 | •               |          | 10 <sup>-3</sup> | ONAL                       | PIEZOMETER<br>OR          |
|-----------------------|---|--|-------------|-----------------------|--------|------|-------------|------------------|---|-----------------------|-----------------------|---------|-----------------------|---|---|-----------------|----------|------------------|----------------------------|---------------------------|
| META                  | BORING METHOD                             | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE | BLOWS/0.30m | SHEAR<br>Cu, kPa |   |                       |                       |         | ) - <b>•</b><br>) - O | W | ATER C                                  | —⊖ <sup>W</sup> | <u>'</u> | WI               | ADDITIONAL<br>LAB. TESTING | STANDPIPE<br>INSTALLATION |
| 0                     |   | GROUND SURFACE  TOPSOIL - (ML) CLAYEY SILT to CLAY;  |             | 91.25                 | 1      | GRAB |             | 20               |   | 40                    | 60                    | 80      |                       | 2 | 20 4                                    | 10              | 60       | 80               |                            |                           |
|                       |   | dark brown  (CI/CH) SILTY CLAY, trace sand; grey brown, with red mottling (Weathered Crust); cohesive, w>PL, stiff to very stiff |             | 90.95<br>0.30         |        | GRAB |             |                  |   |                       |                       |         |                       |   |   |                 |          |                  |                            |                           |
| 1                     |   | ,  |             |                       | 3      | ss   | 4           |                  |   |                       |                       |         |                       |   | 0                                       |                 |          |                  |                            |                           |
|                       |   |  |             |                       | 4      | TP   | PH          |                  |   |                       |                       |         |                       |   |   | 0               |          |                  |                            |                           |
| 2                     |   |  |             |                       |        |      |             | Φ                |   |                       |                       |         | +                     |   |   |                 |          |                  |                            |                           |
|                       |   |  |             |                       |        |      |             | Φ                |   |                       |                       | +       |                       |   |   |                 |          |                  |                            |                           |
| 3                     |   | (CI/CH) SILTY CLAY to CLAY; grey; cohesive, w>PL, soft to firm   |             | 88.20<br>3.05         | 5      | ss   | PH          |                  |   |                       |                       |         |                       |   |   |                 | 0        |                  |                            |                           |
| 4                     | Power Auger<br>200 mm Diam. (Hollow Stem) |  |             |                       |        | _    |             | ₽                |   | +                     |                       |         |                       |   |   |                 |          |                  |                            |                           |
|                       | Power<br>200 mm Diam.                     |  |             |                       |        |      |             | Φ                |   | +                     |                       |         |                       |   |   |                 |          |                  |                            |                           |
| 5                     |   |  |             |                       | 6      | ss   | PH          |                  |   |                       |                       |         |                       |   |   |                 | 0        |                  |                            |                           |
|                       |   |  |             |                       |        |      |             | ⊕                | + | +                     |                       |         |                       |   |   |                 |          |                  |                            |                           |
| 6                     |   |  |             |                       | 7      | ss   | PH          |                  |   |                       |                       |         |                       |   |   | 0               |          |                  |                            |                           |
| 7                     |   | (Cl/CH) SILTY CLAY, trace to some sand, trace gravel; grey; cohesive, w>PL, stiff to very stiff                                  |             | 84.54<br>6.71         |        | -    |             |                  |   |                       |                       |         |                       |   |   |                 |          |                  |                            |                           |
| 7                     |   | W>PL, Stiff to Very Stiff  |             |                       |        |      |             | Φ                |   | +                     |                       | >       | 96                    |   |   |                 |          |                  |                            |                           |
| 8                     |   |  |             |                       | 8      | SS   | PH          |                  |   |                       |                       |         |                       |   | 0                                       |                 |          |                  |                            |                           |
|                       |   | End of Borehole  |             | 83.0 <u>1</u><br>8.24 |        |      |             |                  |   |                       |                       |         |                       |   |   |                 |          |                  |                            |                           |
| 9                     |   |  |             |                       |        |      |             |                  |   |                       |                       |         |                       |   |   |                 |          |                  |                            |                           |
|                       |   |  |             |                       |        |      |             |                  |   |                       |                       |         |                       |   |   |                 |          |                  |                            |                           |
| 10                    |   |  |             |                       |        |      |             |                  |   |                       |                       |         |                       |   |   |                 |          |                  |                            |                           |
| DE<br>1:              |   | SCALE  | •           | •                     | •      |      |             |                  | G | olde<br>soci          | er                    | 1       |                       |   | 1                                       | '               | 1        |                  |                            | GGED: CG                  |

### RECORD OF BOREHOLE: 16-104A

SHEET 1 OF 1

DATUM: Geodetic

LOCATION: See Site Plan SAMPLER HAMMER, 64kg; DROP, 760mm BORING DATE: February 23, 2016

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| ļ (^   | 兒   | SOIL PROFILE   | <b> </b>    |                       | SA     | MPLE     |             | DYNAMIC PENETRATION<br>RESISTANCE, BLOWS/0.3m      | ١.                       |     | k, cm/s  |              |    | Α̈́В                       | PIEZOMETER                      |
|--------|---|--|-------------|-----------------------|--------|----------|-------------|--|--------------------------|-----|----------|--------------|----|----------------------------|---------------------------------|
| METRES | BORING METHOD                             | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE     | BLOWS/0.30m | 20 40 60<br>SHEAR STRENGTH nat V.<br>Cu, kPa rem V | 80<br>+ Q - ●<br>⊕ U - ○ | W   | /ATER Co | ONTENT PERCE | WI | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
|        | ٣   | GROUND SURFACE   | S           |                       |        |          | Ф           | 20 40 60   | 80                       | 1 2 | 20 4     | 0 60         | 80 |                            |                                 |
| 0      | $\vdash$                                  | TOPSOIL - (ML) CLAYEY SILT to CLAY;  | EEE         | 91.25<br>0.00         |        | $\vdash$ | +           |  | +                        |     |          |              |    | +                          |                                 |
| 1      |   | dark brown  (CI/CH) SILTY CLAY, trace sand; grey brown, with red mottling (Weathered Crust); cohesive, w>PL, stiff to very stiff |             | 90.95                 |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 2      | Power Auger<br>200 mm Diam. (Hollow Stem) | (CI/CH) SILTY CLAY to CLAY; grey; cohesive, w>PL, soft to firm   |             | 88.20<br>3.05         |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 4      |   | Coriesive, w/FL, soit to iiiiii  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 5      |   |  |             | 96 12                 | 1      | TP I     | РН          |  |                          |     | -        |              | 0  | С                          |                                 |
|        |   | End of Borehole  | - XXXX      | 86.12<br>5.13         |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   | Note: Soil stratigraphy inferred from BH 16-104  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   | 10-10-   |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 6      |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 7      |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 8      |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 9      |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
| 10     |   |  |             |                       |        |          |             |  |                          |     |          |              |    |                            |                                 |
|        | <u> </u>                                  |  | 1           |                       |        |          |             |  |                          | I   |          |              |    |                            |                                 |
| DE     | ртц                                       | SCALE  |             |                       |        |          | _           | Golder<br>Associate                                |                          |     |          |              |    |                            | GGED: CG                        |

### RECORD OF BOREHOLE: 16-105

SHEET 1 OF 1

LOCATION: See Site Plan

BORING DATE: February 22, 2016

DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| ALE<br>S              | HOD           | 2                          | SOIL PROFILE   | 1 -         |                       | SAM    | IPLES               | RESISTANCE, BLOWS/0.3ffi   | HYDRAULIC CONDUCTIVITY, k, cm/s         | PIEZOMETER                           |
|-----------------------|---------------|----------------------------|--|-------------|-----------------------|--------|---------------------|--|---|--------------------------------------|
| DEPTH SCALE<br>METRES | BORING METHOD |                            | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE<br>BLOWS/0.30m | 20 40 60 80<br>SHEAR STRENGTH nat V. + Q - €<br>Cu, kPa rem V. ⊕ U - C | 10° 10° 10° 10° 10° 10° 10° 10° 10° 10° | PIEZOMETER OR STANDPIPE INSTALLATION |
|                       |               | 1                          | GROUND SURFACE   |             | 92.39                 |        |                     |  |   |                                      |
| - 0                   |               |                            | TOPSOIL - (ML) CLAYEY SILT; dark<br>brown<br>(CL/CI) SILTY CLAY; grey brown<br>(Weathered Crust); cohesive, w>PL, stiff<br>to very stiff |             | 0.00<br>0.15          | 1 6    | RAB -               |  |   |                                      |
| 1                     |               |                            |  |             |                       | 3      | SS 5                | 5  |   |                                      |
| 2                     |               |                            |  |             |                       | 4 :    | SS 3                | 3  |   |                                      |
|                       |               |                            | (SM) SILTY SAND, some gravel; grey, contains cobbles and boulders (GLACIAL TILL); non-cohesive, wet, loose to compact                    |             | 90.03                 |        | SS 7                | ,  |   |                                      |
| 3                     | Power Auger   | 200 mm Diam. (Hollow Stem) |  |             |                       | 6      | SS 3                | 3  |   |                                      |
| 4                     |               | 200 mm [                   |  |             |                       | 7 :    | SS 6                |  |   |                                      |
| 5                     |               |                            |  |             |                       | 8 :    | SS 7                | ,  |   |                                      |
| 6                     |               |                            |  |             |                       | 9 :    | SS 7                | ,  |   |                                      |
| Ü                     |               |                            |  |             |                       | 10     | SS 26               | 6  |   |                                      |
| 7                     |               |                            | End of Borehole<br>Auger Refusal   |             | 85.38<br>7.01         |        |                     |  |   |                                      |
| 8                     |               |                            |  |             |                       |        |                     |  |   |                                      |
| 9                     |               |                            |  |             |                       |        |                     |  |   |                                      |
| 10                    |               |                            |  |             |                       |        |                     |  |   |                                      |
| DE<br>1:              |               | <u> </u>                   | CALE   | 1           | <u> </u>              |        |                     | Golder   |   | LOGGED: CG CHECKED: CK               |

### RECORD OF BOREHOLE: 16-106

SHEET 1 OF 1

LOCATION: See Site Plan

BORING DATE: February 18, 2016

DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| 1 | ,   | THOD                |  | SOIL PROFILE   | 10          |                       |        | MPLI         |             | DYNAMIC PENETR<br>RESISTANCE, BLO<br>20 40 | ATION<br>WS/0.3m<br>60 | 80                 | HYDRAU<br>k | LIC CONDI<br>, cm/s<br>10 <sup>-5</sup> | JCTIVITY<br>10⁻⁴ | 10 <sup>-3</sup> | TING                       | PIEZOMETER<br>OR          |
|---|-----|---------------------|--|--|-------------|-----------------------|--------|--------------|-------------|--|------------------------|--------------------|-------------|---|------------------|------------------|----------------------------|---------------------------|
| 1 |     | BORING METHOD       |  | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE         | BLOWS/0.30m | SHEAR STRENGTH<br>Cu, kPa                  | nat V.<br>rem V.       | + Q - ●<br>⊕ U - ○ | WAT<br>Wp F | ER CONTE                                | ENT PERO         | ENT<br>WI        | ADDITIONAL<br>LAB. TESTING | STANDPIPE<br>INSTALLATION |
|   | 0 - |                     | brown<br>(CL/CI) SILTY                   | FACE IIL) CLAYEY SILT; dark CLAY; grey brown rust); cohesive, w>PL, stif         |             | 92.78<br>0.00<br>0.15 | 1      | GRAB<br>GRAB | -           | 20 40                                      | 60                     | 80                 | 20          | 40                                      | 60               | 80               |                            |                           |
|   | 1   |                     |  |  |             |                       | 3      | SS           | 6           |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 2   |                     |  |  |             | 90.49                 | 4      | ss           | 4           |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 3   | Auger (Hollow Stem) |  | AND, some gravel; grey<br>contains cobbles and<br>kCIAL TILL); non-cohesive<br>e | .,          | 2.29                  | 5      | ss           | 2           |  |                        |                    |             |   |                  |                  |                            |                           |
|   |     | 200 mm Diam (Hollow |  |  |             | 88.97                 | 6      | SS           | 1           |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 4   |                     | contains cobb<br>(GLACIAL TIL<br>compact | SILT, some gravel; grey,<br>les and boulders<br>L); non-cohesive, wet,           |             | 3.81<br>88.21         | 7      | SS           | 15          |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 5   |                     | (SM) SILTY S. non-cohesive,              | AND, fine; grey;<br>wet, compact   |             | 4.57                  | 8      | SS           | 15          |  |                        |                    |             |   |                  |                  | МН                         |                           |
|   | 6   |                     | contains cobb                            | AND, some gravel; grey,<br>les and boulders<br>L); non-cohesive, wet<br>le       |             | 86.84<br>5.94<br>6.10 |        | SS           | 19          |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 7   |                     |  |  |             |                       |        |              |             |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 8   |                     |  |  |             |                       |        |              |             |  |                        |                    |             |   |                  |                  |                            |                           |
|   | 9   |                     |  |  |             |                       |        |              |             |  |                        |                    |             |   |                  |                  |                            |                           |
| 1 | 0   |                     |  |  |             |                       |        |              |             |  |                        |                    |             |   |                  |                  |                            |                           |
|   |     | ·ΤΗ                 | SCALE                                    |  |             |                       |        |              |             | Gold                                       | er                     |                    |             |   |                  |                  |                            | LOC                       |

#### RECORD OF BOREHOLE: 16-107

SHEET 1 OF 1

BORING DATE: February 18, 2016 LOCATION: See Site Plan DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| 7 .v                  |       | дон.                       | SOIL PROFILE   | 1 _         |                       | SA    | MPL  |             | DYNAMIC F<br>RESISTAN     |                | TION<br>S/0.3m | ), | 1                        | :m/s                         | CTIVITY | ,  | AL<br>NG                   | PIEZOMETER                      |
|-----------------------|-------|----------------------------|--|-------------|-----------------------|-------|------|-------------|---------------------------|----------------|----------------|----|--------------------------|------------------------------|---------|----|----------------------------|---------------------------------|
| DEPTH SCALE<br>METRES |       | BORING METHOD              | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH        | . =   | TYPE | BLOWS/0.30m | 20<br>SHEAR ST<br>Cu, kPa | 40<br>RENGTH   |                | 80 | 10 <sup>-6</sup><br>WATE | 10 <sup>-5</sup><br>R CONTEN |         |    | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
|                       |       | BC                         |  | STF         | (m)                   | _     |      | BLC         | 20                        | 40             | 60             | 80 | 20                       | 40                           | 60      | 80 |                            |                                 |
| 0                     | L     | $\Box$                     | GROUND SURFACE   | ļ           | 93.14                 |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| -                     |       |                            | TOPSOIL - (ML) CLAYEY SILT; dark<br>\brown   |             | 0.00                  | 1 1 1 | GRAE | -           |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            | (CL/CI) SILTY CLAY: grev brown   |             |                       |       | GRAE | -           |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       | 200 mm Diam. (Hollow Stem) | (Weathered Crust); cohesive, w>PL, stiff to very stiff   |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       | der   | Molic                      |  |             |                       | _     | 1    |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 1                     | er Au | F F                        |  |             |                       | 3     | SS   | 4           |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       | Pow   | n Diar                     |  |             |                       |       | 33   | 4           |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       | 00 mu                      | (SM) SILTY SAND; grey, contains  |             | 91.77<br>1.37         |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       | 30                         | (SM) SILTY SAND; grey, contains<br>cobbles and boulders (GLACIAL TILL);<br>non-cohesive, wet, very dense |             |                       |       | 1    |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            | non-conesive, wet, very derise   |             | 1                     | l     | SS   | >50         |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 2                     |       | щ                          | End of Borehole  |             | 91.1 <u>6</u><br>1.98 |       | 1    |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            | Auger Refusal  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 3                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 4                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 5                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| Ū                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 6                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 7                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 8                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 9                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| J                     |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
| 10                    |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       |       |                            |  |             |                       |       |      |             |                           |                |                |    |                          |                              |         |    |                            |                                 |
|                       | _     |                            |  | -           |                       | -     | -    |             |                           | - 1            |                | 1  |                          |                              |         |    |                            |                                 |
| DE                    | P     | TH S                       | CALE   |             |                       |       |      | 1           |                           | Golde<br>ssoci | r              |    |                          |                              |         |    | LC                         | OGGED: CG                       |
| 1:                    | 50    | )                          |  |             |                       |       |      | ,           | <b>V</b> JA               | SSOCI          | ates           |    |                          |                              |         |    | CH                         | ECKED: CK                       |

#### **RECORD OF BOREHOLE: 16-107A**

SHEET 1 OF 1

LOCATION: See Site Plan

BORING DATE: February 18, 2016

DATUM: Geodetic

SAMPLER HAMMER, 64kg; DROP, 760mm PENETRATION TEST HAMMER, 64kg; DROP, 760mm HYDRAULIC CONDUCTIVITY, k, cm/s DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER STRATA PLOT 10<sup>-5</sup> NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT BLOWS/0. DESCRIPTION DEPTH -<del>0</del>W Wp -(m) GROUND SURFACE 93.14 TOPSOIL - (ML) CLAYEY SILT; dark brown 0.15 (CL/CI) SILTY CLAY; grey brown (Weathered Crust); cohesive, w>PL, stiff to very stiff Power / 91.77 (SM) SILTY SAND; grey, contains cobbles and boulders (GLACIAL TILL); 91.46 non-cohesive, wet, very dense End of Borehole Auger Refusal 2 Note: Soil stratigraphy inferred from BH 16-107 1523044.GPJ GAL-MIS.GDT 05/24/16 JEM 9 10

Golder

MIS-BHS 001



# **APPENDIX B**

**Record of Borehole Sheets and Laboratory Test Results Previous Investigations** 



#### RECORD OF BOREHOLE: BH 11-1

SHEET 1 OF 1

LOCATION: See Site Plan

1:50

BORING DATE: January 25, 2011

DATUM: Local

CHECKED:

PENETRATION TEST HAMMER, 64kg; DROP, 760mm SAMPLER HAMMER, 64kg; DROP, 760mm HYDRAULIC CONDUCTIVITY, k, cm/s DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER STRATA PLOT BLOWS/0.30m NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT DESCRIPTION DEPTH −OW Wp H (m) GROUND SURFACE TOPSOIL 0.00 Very stiff grey brown SILTY CLAY, some silty fine sand layers (Weathered Crust) 0.23 50 DO 5 0 50 DO 2 3 2 Firm grey SILTY CLAY 3.05 50 DO 0 3 Power Auger n Diam. (Hollow Ф  $\oplus$ 50 DO 0 Ф Ф  $\oplus$  $\oplus$ 50 DO WH 5 0  $\oplus$ End of Borehole 7.62 MIS-BHS 001 1011210264.GPJ GAL-MIS.GDT 03/01/16 JEM 9 10 DEPTH SCALE LOGGED: PAH

Golder

#### RECORD OF BOREHOLE: BH 11-1A

SHEET 1 OF 1

CHECKED:

LOCATION: See Site Plan

1:50

BORING DATE: January 25, 2011

DATUM: Local

SAMPLER HAMMER, 64kg; DROP, 760mm PENETRATION TEST HAMMER, 64kg; DROP, 760mm HYDRAULIC CONDUCTIVITY, k, cm/s DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER STRATA PLOT BLOWS/0.30m NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT DESCRIPTION DEPTH −OW Wp ⊢ (m) GROUND SURFACE TOPSOIL 0.00 Very stiff grey brown SILTY CLAY, some silty fine sand layers (Weathered Crust) 0.23 Native Backfill Bentonite Seal Silica Sand Firm grey SILTY CLAY 3.05 76 TP PH Standpipe 0 End of Borehole 3.66 W.L. in Standpipe at 1.23 m depth below ground surface on February 7, 2011 Note: Soil profile inferred from RECORD OF BOREHOLE BH 11-1. MIS-BHS 001 1011210264.GPJ GAL-MIS.GDT 03/01/16 JEM 9 10 DEPTH SCALE LOGGED: PAH Golder

SAMPLER HAMMER, 64kg; DROP, 760mm

### RECORD OF BOREHOLE: BH 11-2

SHEET 1 OF 1
DATUM: Local

LOCATION: See Site Plan

BORING DATE: January 28, 2011

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| H F                   | 문             | SOIL PROFILE   | 1.          | ı                     | SA     | MPLE             |             | DYNAMIC PENETRA<br>RESISTANCE, BLOV     | TION<br>/S/0.3m | HYDRAULIC CONDUCTIVITY, k, cm/s | 무위                         | PIEZOMETER                      |
|-----------------------|---------------|--|-------------|-----------------------|--------|------------------|-------------|---|-----------------|---------------------------------|----------------------------|---------------------------------|
| DEPTH SCALE<br>METRES | BORING METHOD | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE             | BLOWS/0.30m | 20 40<br>L HEAR STRENGTH<br>Cu, kPa     |                 | Wp I WI                         | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
| _                     | ш             | GROUND SURFACE   | S           | <u> </u>              |        | H                | B           | 20 40                                   | 60 80           | 20 40 60 80                     |                            |                                 |
| - 0                   |               | TOPSOIL  Very stiff to stiff grey brown SILTY CLAY (Weathered Crust)       |             | 0.00                  |        |                  |             |   |                 |                                 |                            |                                 |
| 1                     |               |  |             |                       | 1      | 50<br>DO         | 7           |   |                 | 0                               |                            |                                 |
| - 2                   | (ma           |  |             |                       | 2      | 50<br>DO         | 4           |   |                 | 0                               |                            |                                 |
| 3                     | Power Auger   | Firm grey SILTY CLAY   |             | 3.05                  |        | -                |             | <ul><li>⊕</li><li>⊕</li><li>H</li></ul> | +               |                                 |                            |                                 |
|                       | 2006          | ingley old i obdi  |             | 3.03                  | 3      | 50<br>DO         |             | <ul><li>+</li><li>+</li><li>+</li></ul> |                 | <u> </u>                        |                            |                                 |
| 4                     |               | Loose to compact grey SILTY SAND, some gravel, with cobbles (GLACIAL TILL) |             | 3.96                  | 4      | 50<br>DO         | 8           | ⊕ +                                     |                 |                                 |                            |                                 |
| 5                     |               | Loose grey fine to medium SAND   |             | 5.18                  | 5      | 50<br>DO<br>GRAB | 22          |   |                 |                                 |                            |                                 |
| 6                     |               | End of Borehole  |             | 5.49                  |        |                  |             |   |                 |                                 |                            |                                 |
| 7                     |               |  |             |                       |        |                  |             |   |                 |                                 |                            |                                 |
| 8                     |               |  |             |                       |        |                  |             |   |                 |                                 |                            |                                 |
| 9                     |               |  |             |                       |        |                  |             |   |                 |                                 |                            |                                 |
| 10                    |               |  |             |                       |        |                  |             |   |                 |                                 |                            |                                 |
| DE                    | PTH           | SCALE  |             |                       |        |                  | 4           | Gold                                    | <b></b>         |                                 | LOG                        | GED: PAH                        |

#### RECORD OF BOREHOLE: BH 11-2A

SHEET 1 OF 1

LOCATION: See Site Plan

1:50

BORING DATE: January 28, 2011

DATUM: Local

CHECKED:

SAMPLER HAMMER, 64kg; DROP, 760mm PENETRATION TEST HAMMER, 64kg; DROP, 760mm HYDRAULIC CONDUCTIVITY, k, cm/s DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER STRATA PLOT BLOWS/0.30m NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT DESCRIPTION DEPTH −OW Wp -(m) GROUND SURFACE TOPSOIL 0.00 Very stiff to stiff grey brown SILTY CLAY (Weathered Crust) 0.30 Native Backfill 8 Bentonite Seal Silica Sand Firm grey SILTY CLAY 3.05 88 TP PH Standpipe End of Borehole W.L. in Standpipe at 0.95 m depth below ground surface on February 7, 2011 Soil profile inferred from RECORD OF BOREHOLE BH 11-2. MIS-BHS 001 1011210264.GPJ GAL-MIS.GDT 03/01/16 JEM 9 10 DEPTH SCALE LOGGED: PAH Golder

#### RECORD OF BOREHOLE: BH 11-3

SHEET 1 OF 1 DATUM: Local

BORING DATE: January 25, 2011 LOCATION: See Site Plan

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| H<br>F | 2        | 요                  | SOIL PROFILE   |             |                | SA       | MPL      |             | DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m   | HYDRAULIC CONDUCTIVITY,<br>k, cm/s                                  | وٰڐ                        | PIEZOMETER                                     |
|--------|----------|--------------------|--|-------------|----------------|----------|----------|-------------|--|---|----------------------------|--|
| METRES | E        | BORING MEI HOD     |  | STRATA PLOT |                | l K      |          | BLOWS/0.30m | 20 40 60 80                                  | 10 <sup>-6</sup> 10 <sup>-5</sup> 10 <sup>-4</sup> 10 <sup>-3</sup> | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE                                |
| MET    |          | ן פּ               | DESCRIPTION  | TAF         | ELEV.<br>DEPTH | NUMBER   | TYPE     | VS/0.       | SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ | WATER CONTENT PERCENT   | DDIT<br>B. TE              | STANDPIPE<br>INSTALLATION                      |
| 7      | 2        | ğ                  |  | TRA         | (m)            | ≥        | -        | 3LOV        |  | Wp I → W I WI   | ¥₹                         |  |
|        | ۲        | $\overline{+}$     | GROUND SURFACE   | S           |                |          | $\vdash$ | ш           | 20 40 60 80                                  | 20 40 60 80   | 1                          |  |
| 0      | H        | $\dashv$           | TOPSOIL  | EEE         | 0.00           | $\vdash$ | $\vdash$ |             |  |   | $\vdash$                   | <b>├</b>                                       |
|        |          |                    |  |             | 0.25           |          |          |             |  |   |                            |  |
|        |          |                    | Very stiff grey brown SILTY CLAY (Weathered Crust)                                     |             | 0.25           |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            | l 🛚  |
|        |          |                    |  |             |                |          | 1        |             |  |   |                            |  |
| 1      |          |                    |  |             |                | 1        | 50<br>DO | 6           |  |   |                            |  |
|        |          | H                  | Loose to compact grey brown SANDY  |             | 1.22           |          |          |             |  |   |                            |  |
|        |          |                    | Loose to compact grey brown SANDY<br>SILT, some gravel, with cobbles<br>(GLACIAL TILL) |             |                |          |          |             |  |   |                            | l  |
|        |          |                    | (GLACIAL TILL)   |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                | 2        | 50<br>DO | 18          |  |   |                            | Native Backfill                                |
| 2      |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          | ]        |             |  |   |                            |  |
|        |          | Stem)              | Loose grey SANDY SILT, some gravel, with cobbles (GLACIAL TILL)                        |             | 2.29           |          | 50       |             |  |   |                            | l 🛚  |
|        | Je.      | woll o             |  |             |                | 3        | 50<br>DO | 8           |  |   |                            |  |
|        | ∍r Aug   | ا<br>ا             |  |             |                |          |          |             |  |   |                            |  |
| 3      | Powe     | mm Diam. (Hollow S |  |             |                |          |          |             |  |   |                            |  |
|        |          | 0 mm               |  |             | 1              | 4        | 50<br>DO | 8           |  |   |                            |  |
|        |          | 200                |  |             | 1              | -        | DO       | ٦           |  |   |                            |  |
|        |          |                    |  |             |                |          | -        |             |  |   |                            |  |
|        |          |                    |  |             | 1              |          |          |             |  |   |                            | Bentonite Seal                                 |
| 4      |          |                    |  |             |                |          |          |             |  |   |                            | S. ROING GER                                   |
|        |          |                    |  |             | 1              |          |          |             |  |   |                            |  |
|        |          |                    |  |             | . <u>.</u>     |          |          |             |  |   |                            | [8   |
|        |          |                    | Loose grey fine to medium SAND   |             | 4.57           |          | _        |             |  |   |                            | Silica Sand                                    |
| 5      |          |                    | Loose to compact grey SANDY SILT, some gravel, with cobbles (GLACIAL                   |             | 4.88           | 5        | 50<br>DO | 9           |  |   |                            |  |
|        |          |                    | some gravel, with cobbles (GLACIAL TILL)   |             |                |          | 4        |             |  |   |                            |  |
|        |          |                    | ,  |             |                |          | 50       |             |  |   |                            | X  |
|        |          |                    |  |             | 1              | 6        | 50<br>DO | 27          |  |   |                            | Standpipe                                      |
|        |          | Ц                  | End of Borehole  |             | 5.79           |          | -        |             |  |   |                            | W.L. in Standpipe                              |
| 6      |          |                    | Lita di Borendie   |             | 00             |          |          |             |  |   |                            | W.L. in Standpipe at 1.53 m depth below ground |
|        |          |                    |  |             |                |          |          |             |  |   |                            | surface on<br>February 7, 2011                 |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
| 7      |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
| 8      |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
| ^      |          |                    |  |             |                |          |          |             |  |   |                            |  |
| 9      |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        |          |                    |  |             |                |          |          |             |  |   |                            |  |
| 10     |          |                    |  |             |                |          |          |             |  |   |                            |  |
| 10     |          |                    |  |             |                |          |          |             |  |   |                            |  |
|        | <u> </u> | 1                  |  | 1           | 1              | _        | Ш        |             |  |   | <u> </u>                   | 1  |
| DE     | PT       | ΉS                 | CALE   |             |                |          |          | 1           | Golder<br>Associates                         |   | L                          | OGGED: PAH                                     |
|        | 50       |                    |  |             |                |          |          |             | 7 JUNIEL                                     |   |                            | ECKED:   |

#### RECORD OF BOREHOLE: BH 11-4

SHEET 1 OF 1

LOCATION: See Site Plan

1:50

BORING DATE: January 28, 2011

DATUM: Local

CHECKED:

PENETRATION TEST HAMMER, 64kg; DROP, 760mm SAMPLER HAMMER, 64kg; DROP, 760mm DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m HYDRAULIC CONDUCTIVITY, k, cm/s SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER STRATA PLOT BLOWS/0.30m 10<sup>-5</sup> NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT DESCRIPTION DEPTH −OW Wp I (m) GROUND SURFACE TOPSOIL 0.00 Very stiff to stiff grey brown SILTY CLAY (Weathered Crust) 0.22 50 DO 6 Power Auger ) mm Diam. (Hollow 50 DO 2 2  $\oplus$ Loose grey SANDY SILT, some gravel (GLACIAL TILL) 2.51 50 DO 7 50 DO 5 End of Borehole 3.66 MIS-BHS 001 1011210264.GPJ GAL-MIS.GDT 03/01/16 JEM 9 10 DEPTH SCALE LOGGED: PAH

#### RECORD OF BOREHOLE: BH 11-5

SHEET 1 OF 1 DATUM: Local

BORING DATE: January 25, 2011 LOCATION: See Site Plan

SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| ا<br>ا | GOH.          | SOIL PROFILE   | 1 -         | ı     | SA     | MPLI     |             | DYNAMIC PENETE<br>RESISTANCE, BLO | OWS/0       | .3m              | ζ.             |            | k, cm/s |        |    |                  | l RG                    | PIEZOMETER      |
|--------|---------------|--|-------------|-------|--------|----------|-------------|-----------------------------------|-------------|------------------|----------------|------------|---------|--------|----|------------------|-------------------------|-----------------|
| METRES | BORING METHOD |  | STRATA PLOT | ELEV. | Ä      |          | BLOWS/0.30m | 20 40                             | 60          |                  |                | 10-6       |         |        |    | 10 <sup>-3</sup> | ADDITIONAL LAB. TESTING | OR<br>STANDPIPE |
| ≅<br>∐ | RING          | DESCRIPTION  | ATA         | DEPTH | NUMBER | TYPE     | WS/C        | SHEAR STRENGT<br>Cu, kPa          | H na<br>rei | t V. +<br>n V. ⊕ | Q - •<br>U - O |            |         | ONTENT |    |                  | ABDI.                   | INSTALLATION    |
| נ      | BOF           |  | STR         | (m)   | Ž      |          | BLO         | 20 40                             | 60          |                  |                | Wp I<br>20 |         |        | 60 | ₩I<br>80         | "                       |                 |
| _      |               | GROUND SURFACE   |             |       |        | П        |             | Ī                                 | Ĭ           |                  |                | Ī          | j       |        |    |                  | 1 1                     |                 |
| 0      |               | TOPSOIL  | EEE         | 0.00  |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               | Very stiff to stiff grey brown SILTY CLAY, some sand seams (Weathered  |             | 0.25  |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               | CLAY, some sand seams (weathered Crust)  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        | - 1      |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 1      |               |  |             |       | 1      | 50<br>DO | 7           |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       | l .    | DO       | <i>'</i>    |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       | 2      | 50<br>DO | 2           |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 2      | (mg)          |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        | ler ler       |  |             | 1     |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        | er Aug        | Stiff grey SILTY CLAY, some silt seams   |             | 2.59  |        |          |             | Φ                                 |             | +                |                |            |         |        |    |                  |                         |                 |
|        | Powe          | Suri grey SILTT CLAT, SOME SIIT SEAMS  |             | 2.59  | 3      | 50<br>DO | wн          |                                   |             |                  |                |            |         |        | 0  |                  |                         |                 |
| 3      | Power Auger   |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        | 1             |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               | Lance to constitute of the Con |             | 2     |        |          |             | <b>+</b>   -                      | -           |                  |                |            |         |        |    |                  |                         |                 |
|        |               | Loose to very loose grey SANDY SILT, some gravel (GLACIAL TILL)  |             | 3.51  |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        | 1        |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 4      |               |  |             | 1     | 4      | 50<br>DO | 5           |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             | 1     |        | $  \  $  |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       | 5      | 50<br>DO | 2           |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 5      |               |  |             |       |        | טט       |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               | End of Borehole  | - rxxx      | 5.18  |        | 1        |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 6      |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 7      |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 8      |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 9      |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| J      |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| 10     |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
|        |               |  |             |       |        |          |             |                                   |             |                  |                |            |         |        |    |                  |                         |                 |
| DE     | PTH           | SCALE  |             |       |        |          | 4           |                                   | <b>.</b>    |                  |                |            |         |        |    |                  | LO                      | GGED: PAH       |
| _      | 50            |  |             |       |        |          |             | Gol                               | aer         |                  |                |            |         |        |    |                  |                         | CKED:           |

#### RECORD OF BOREHOLE: BH 11-6

SHEET 1 OF 1 DATUM: Local

CHECKED:

LOCATION: See Site Plan

1:50

SAMPLER HAMMER, 64kg; DROP, 760mm

BORING DATE: January 28, 2011

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m HYDRAULIC CONDUCTIVITY, k, cm/s SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER STRATA PLOT BLOWS/0.30m NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT DESCRIPTION DEPTH −OW Wp ⊢ (m) GROUND SURFACE TOPSOIL 0.00 Very stiff to stiff grey brown SILTY CLAY (Weathered Crust) 0.25 200 mm Diam 50 DO  $\oplus$ Ф 3.35 End of Borehole MIS-BHS 001 1011210264.GPJ GAL-MIS.GDT 03/01/16 JEM 9 10 DEPTH SCALE LOGGED: PAH

# RECORD OF BOREHOLE: 15-7

SHEET 1 OF 2

LOCATION: N 5013739.7 ;E 364291.3 SAMPLER HAMMER, 64kg; DROP, 760mm BORING DATE: August 12, 2015

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

DATUM: CGVD28

| Ц.                    | 3              | 2                      | SOIL PROFILE  | ١.          |                       | SF     | MPL  |             | DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m                               | k, cm/s   | 무일                         | PIEZOMETER                      |
|-----------------------|----------------|------------------------|---|-------------|-----------------------|--------|------|-------------|--|---|----------------------------|---------------------------------|
| DEPTH SCALE<br>METRES | DOUTTHW OWIGOR | בסאוואס ואובו          | DESCRIPTION   | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE | BLOWS/0.30m | 20 40 60 80<br>SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○  20 40 60 80 | 10 <sup>8</sup> 10 <sup>6</sup> 10 <sup>4</sup> 10 <sup>2</sup> WATER CONTENT PERCENT  Wp I → W I WI  20 40 60 80 | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
| . 0                   |                |                        | GROUND SURFACE  |             | 92.84                 |        |      |             |  |   |                            | NO.                             |
| Ü                     |                |                        | TOPSOIL - (ML/SM) sandy SILT to<br>SILTY SAND; dark brown; moist<br>(SM) gravelly SILTY SAND; grey brown,<br>with oxidation staining, presence of<br>cobbles and boulders inferred from |             | 0.00<br>92.56<br>0.28 | 1      | AS   | -           |  |   |                            | Native Backfill  Bentonite Seal |
| 1                     |                | ΙI                     | cobbles and boulders inferred from<br>auger resistance (GLACIAL TILL);<br>non-cohesive, moist to wet, compact to<br>very dense  |             |                       | 2      | ss   | >50         |  |   |                            |                                 |
| 2                     |                |                        |   |             |                       | 3      | ss   | 17          |  |   |                            | ∑<br>Native Backfill            |
|                       | uger           | Hollow Stem)           |   |             |                       | 4      | ss   | 46          |  |   |                            |                                 |
| 3                     | Power A        | 200 mm Diam. (Hollow S | (SM) SILTY SAND, fine, trace gravel; brown; non-cohesive, wet, compact  |             | 89.64<br>3.20         | 5      | ss   | 12          |  |   |                            | Native Backfill                 |
| 4                     |                | 200                    | (SM) gravelly SILTY SAND; grey<br>(GLACIAL TILL); non-cohesive, wet,  |             | 88.72<br>4.12         | 6      | ss   | 22          |  |   | М                          |                                 |
| 5                     |                | -                      | compact  (ML) gravelly sandy SILT; grey (GLACIAL TILL); non-cohesive, wet,  |             | 87.81<br>5.03         | 7      | ss   | 21          |  |   |                            |                                 |
|                       |                |                        | (GLAČIAL TILL); non-cohesive, wet, compact  |             |                       | 8      | ss   | 11          |  |   |                            |                                 |
| 6                     |                |                        | Borehole continued on RECORD OF DRILLHOLE 15-7  |             | 86.64<br>6.2          | 9      | ss   | >50         |  |   |                            |                                 |
| 7                     |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 8                     |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
|                       |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 9                     |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 10                    |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 11                    |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
|                       |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 12                    |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 13                    |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 14                    |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
|                       |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| 13<br>14<br>15        |                |                        |   |             |                       |        |      |             |  |   |                            |                                 |
| DE                    | PT             | H S                    | CALE  |             |                       |        |      | _           | Golder<br>Associates   |   | L                          | DGGED: PAH                      |

INCLINATION: -90°

LOCATION: N 5013739.7 ;E 364291.3

AZIMUTH: ---

#### RECORD OF DRILLHOLE: 15-7

DRILL RIG: CME 850

DRILLING CONTRACTOR: Marathon Drilling

DRILLING DATE: August 12, 2015 DATUM: CGVD28

SHEET 2 OF 2

BD - Bedding FO - Foliation CO - Contact OR - Orthogonal CL - Cleavage PL - Planar CU- Curved UN- Undulating ST - Stepped IR - Irregular PO- Polished
K - Slickensided
SM- Smooth
Ro - Rough
MB- Mechanical Break

BR - Broken Rock
NOTE: For additional abbreviations refer to list of abbreviations & symbols. JN - Joint FLT - Fault SHR- Shear VN - Vein CJ - Conjugate DRILLING RECORD DEPTH SCALE METRES SYMBOLIC LOG ELEV. DESCRIPTION FRACT. R.Q.D. INDEX PER 0.25 m 86848 45248 HYDRAULIC CONDUCTIVITY K, cm/sec DEPTH RECOVERY (m) TOTAL SOLID CORE % 0000 80 80 80 80 80 BEDROCK SURFACE 86.64 Fresh, thinly to medium bedded, grey LIMESTONE Peltonite Seal Granitic Sand Rotary Drill 32 mm Diam. PVC #10 Slot Screen 3 End of Drillhole WL in Screen at Elev. 90.67 m on Aug. 24, 2015 10 11 12 13 14 15 16 17 18 1523645.GPJ GAL-MISS.GDT 09/23/15 JM 19 20 21 DEPTH SCALE LOGGED: PAH

1:75

Golder CHECKED: SD

# RECORD OF BOREHOLE: 15-8

BORING DATE: August 11-12, 2015

SHEET 1 OF 2 DATUM: CGVD28

LOCATION: N 5013747.0 ;E 364141.5 SAMPLER HAMMER, 64kg; DROP, 760mm

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| SALE   | THOE                                      | SOIL PROFILE   | <del> </del> |                       |          | MPL  |             | DYNAMIC PENETRATION RESISTANCE, BLOWS | `,                             | k, cm/s            |                         | PIEZOMETER                      |
|--------|---|--|--------------|-----------------------|----------|------|-------------|---------------------------------------|--------------------------------|--------------------|-------------------------|---------------------------------|
| METRES | BORING METHOD                             | DESCRIPTION  | STRATA PLOT  | ELEV.<br>DEPTH<br>(m) | NUMBER   | TYPE | BLOWS/0.30m | 20 40 6 SHEAR STRENGTH r Cu, kPa r    | at V. + Q - ●<br>em V. ⊕ U - ○ | WATER CONTENT PERC | ADDITIONAL LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
| 0      |   | GROUND SURFACE TOPSOIL - (CL/ML) CLAYEY SILT; dark brown; moist  |              | 92.19<br>0.00<br>0.20 | 1        | AS   | -           | 20 40 6                               | 5 50                           | 20 40 00           |                         |                                 |
| 1      |   | (CI) sandy SILTY CLAY; grey brown, contains sifty sand seams (WEATHERED CRUST); cohesive, w>PL, very stiff             |              |                       | 2        | ss   | 6           |                                       |                                |                    |                         |                                 |
|        |   | (SM) gravelly SILTY SAND; grey brown, presence of cobbles and boulders inferred from auger resistance (GLACIAL         |              | 90.88                 |          |      |             |                                       |                                |                    |                         |                                 |
| 2      |   | TILL); non-cohesive, wet, compact  (SM) gravelly SILTY SAND; grey (GLACIAL TILL); non-cohesive, wet,                   |              | 90.06                 |          | SS   | 19          |                                       |                                |                    |                         |                                 |
| 3      |   | very loose to compact  |              |                       | 4        | SS   | 2           |                                       |                                |                    |                         |                                 |
| 4      | / Stem)                                   |  |              |                       | 5        | SS   | 3           |                                       |                                |                    |                         |                                 |
| 7      | Power Auger<br>200 mm Diam. (Hollow Stem) |  |              |                       | 6        | SS   | 1           |                                       |                                |                    | МН                      |                                 |
| 5      | 200 mm [                                  |  |              |                       | 7        | SS   | 2           |                                       |                                |                    |                         |                                 |
| 6      |   |  |              |                       | 8        | SS   | 5           |                                       |                                |                    |                         |                                 |
| 7      |   |  |              |                       | 9        | SS   | 5           |                                       |                                |                    |                         |                                 |
| ,      |   | (SM) gravelly SILTY SAND; dark grey,   |              | 84.57<br>7.62         | 10       | ss   | 10          |                                       |                                |                    |                         |                                 |
| 8      |   | presence of cobbles and boulders inferred from auger resistance (GLACIAL TILL); non-cohesive, wet, dense to very dense |              |                       | 11       | ss   | 48          |                                       |                                |                    | М                       |                                 |
| 9      |   | Borehole continued on RECORD OF DRILLHOLE 15-8   |              | 83.30<br>8.89         | 12       | ss   | 90          |                                       |                                |                    |                         |                                 |
| 10     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
| 10     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
| 11     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
| 12     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
| 13     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
|        |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
| 14     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
| 15     |   |  |              |                       |          |      |             |                                       |                                |                    |                         |                                 |
|        | PTH S                                     | CALE   |              | I                     | <u> </u> |      |             | Golder                                |                                |                    | 10                      | GGED: PAH                       |

INCLINATION: -90°

LOCATION: N 5013747.0 ;E 364141.5

AZIMUTH: ---

#### RECORD OF DRILLHOLE: 15-8

DRILLING DATE: August 11-12, 2015

DRILL RIG: CME 850

DRILLING CONTRACTOR: Marathon Drilling

SHEET 2 OF 2 DATUM: CGVD28

BD - Bedding FO - Foliation CO - Contact OR - Orthogonal CL - Cleavage PL - Planar CU- Curved UN- Undulating ST - Stepped IR - Irregular PO- Polished
K - Slickensided
SM- Smooth
Ro - Rough
MB- Mechanical Break

BR - Broken Rock
NOTE: For additional abbreviations refer to list of abbreviations & symbols. JN - Joint FLT - Fault SHR- Shear VN - Vein CJ - Conjugate DRILLING RECORD DEPTH SCALE METRES SYMBOLIC LOG ELEV. DESCRIPTION FRACT. R.Q.D. INDEX PER 0.25 m 86848 45248 HYDRAULIC CONDUCTIVITY K, cm/sec DEPTH RECOVERY (m) TOTAL SOLID CORE %  $\begin{array}{c} 10^{-8} \\ 10^{-2} \end{array}$ 8848 BEDROCK SURFACE 83.30 Fresh, thinly to medium bedded, grey LIMESTONE Rotary Drill NQ Core 10 81.52 10.67 End of Drillhole 11 12 13 14 15 16 17 18 19 20 MIS-RCK 004B 1523645.GPJ GAL-MISS.GDT 09/23/15 JM 21 22 23 Golder

DEPTH SCALE 1:75

LOGGED: PAH CHECKED: SD

# **RECORD OF BOREHOLE: 15-9**

SHEET 1 OF 1 BORING DATE: August 10, 2015 DATUM: CGVD28

SAMPLER HAMMER, 64kg; DROP, 760mm

LOCATION: N 5013812.4 ;E 364008.0

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| S                     | ТНОВ          | SOIL PROFILE  | - 1    |                       | SA     | MPLI |             | DYNAMIC PENE<br>RESISTANCE, E  |       |                    | , |      | cm/s |         |               | ING<br>ING                 | PIEZOMETER                      |
|-----------------------|---------------|---|--------|-----------------------|--------|------|-------------|--------------------------------|-------|--------------------|---|------|------|---------|---------------|----------------------------|---------------------------------|
| DEPTH SCALE<br>METRES | BORING METHOD | DESCRIPTION Exp   | E DI   | EPTH<br>(m)           | NUMBER | TYPE | BLOWS/0.30m | 20 4<br>SHEAR STREN<br>Cu, kPa | GTH r | at V. +<br>em V. ⊕ |   | Wp ⊦ |      | NTENT F | PERCENT<br>WI | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
| - 0                   |               | GROUND SURFACE  TOPSOIL - (ML) sandy SILT; dark brown; moist (CI) sandy SILTY CLAY; grey brown, contains silty sand seams (WEATHERED CRUST); cohesive, w>PL, very stiff | $\neg$ | 92.27<br>0.00<br>0.22 | 1      | SS   | 4           | 20 4                           | 0 6   | 0 8                | 0 | 20   | 40   | 60      | 80            |                            |                                 |
| 2                     | Power Auger   | (Cl/ML) SILTY CLAY to CLAYEY SILT; grey; cohesive, w>PL, firm to stiff  |        | 89.83<br>2.44         | 2      | SS   | 1           | Φ                              | ++    |                    |   |      |      |         |               |                            |                                 |
| 3                     | 200           | Probable Glacial Till   |        | 87.95<br>4.32         | 3      | SS   | 2           | ⊕                              | +     |                    |   |      |      |         |               |                            |                                 |
| 5                     | 1             | End of Borehole Auger Refusal on Probable Boulder Note: Refer to Record of Borehole 15-9A for deeper stratigraphy.  |        | 4.50                  | 4      | SS   | >50         |                                |       |                    |   |      |      |         |               |                            |                                 |
| 7                     |               |   |        |                       |        |      |             |                                |       |                    |   |      |      |         |               |                            |                                 |
| 8                     |               |   |        |                       |        |      |             |                                |       |                    |   |      |      |         |               |                            |                                 |
| 10                    |               |   |        |                       |        |      |             |                                |       |                    |   |      |      |         |               |                            |                                 |
| 11                    |               |   |        |                       |        |      |             |                                |       |                    |   |      |      |         |               |                            |                                 |
|                       |               |   |        |                       |        |      |             |                                |       |                    |   |      |      |         |               |                            |                                 |
| 13                    |               |   |        |                       |        |      |             |                                |       |                    |   |      |      |         |               |                            |                                 |
|                       | PTH           | SCALE   |        |                       |        |      |             | <b>J</b> AGO                   | lde   | •                  |   |      |      |         |               | LO                         | GGED: PAH                       |

# RECORD OF BOREHOLE: 15-9A

SHEET 1 OF 1 DATUM: CGVD28

SAMPLER HAMMER, 64kg; DROP, 760mm

LOCATION: N 5013812.4 ;E 364008.0

BORING DATE: August 10-11, 2015

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

|        | OC.                                      | SOIL PROFILE                                       |             |                | SA     | MPL  | ES          | DYNAMIC PENETRAT<br>RESISTANCE, BLOW | TION \ | HYDRAULIC C                   | ONDUCTIVITY,                           |                            |  |
|--------|--|--|-------------|----------------|--------|------|-------------|--------------------------------------|--------|-------------------------------|--|----------------------------|--|
| METRES | BORING METHOD                            | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH | NUMBER | TYPE | BLOWS/0.30m | 20 40<br>SHEAR STRENGTH<br>Cu, kPa   | 60 80  | 10 <sup>-8</sup> 1<br>WATER C | 0°6 10°4 10°2<br>CONTENT PERCENT  W  W | ADDITIONAL<br>LAB. TESTING | PIEZOMETER<br>OR<br>STANDPIPE<br>INSTALLATION        |
| 1      | B0                                       | GROUND SURFACE                                     | STR         | (m)            | _      |      | BLC         | 20 40                                | 60 80  |                               | 40 60 80                               |                            |  |
| 1 2    |  | Refer to Record of Borehole 15-9 for stratigraphy  |             | 92.27          |        |      |             |                                      |        |                               |  |                            | Bentonite Seal                                       |
| 3 4 5  | Power Auger<br>200 mm Diam (Hollow Stem) | (ML) gravelly sandy SILT; grey,                    |             | 87.70<br>4.57  | 1 2    | SS   | 1           |                                      |        |                               |  |                            | Native Backfill                                      |
| 6      |  |  |             |                | 3      | SS   | 2           |                                      |        |                               |  |                            | Bentonite Seal                                       |
| 7      |  | (SP) gravelly SAND; grey; non-cohesive, wet, loose |             | 84.80<br>7.47  | 4      | ss   | 17          |                                      |        |                               |  |                            | Native Backfill and<br>Granitic Sand                 |
| 8      |  |  |             |                | 6      | SS   | -           |                                      |        |                               |  | М                          | 38 mm Diam. PVC #10 Slot Screen                      |
| 10     |  | End of Borehole                                    |             | 83.13<br>9.14  |        | -    |             |                                      |        |                               |  |                            | WL in Screen at<br>Elev. 91.35 m on<br>Aug. 24, 2015 |
| 11     |  |  |             |                |        |      |             |                                      |        |                               |  |                            |  |
| 12     |  |  |             |                |        |      |             |                                      |        |                               |  |                            |  |
| 13     |  |  |             |                |        |      |             |                                      |        |                               |  |                            |  |
| 114    |  |  |             |                |        |      |             |                                      |        |                               |  |                            |  |
|        | PTH                                      | SCALE  |             |                |        |      |             | Golde                                |        |                               |  | LC                         | DGGED: PAH   |

MIS-BHS 001 1523645.GPJ GAL-MIS.GDT 09/23/15 JM

1:75

# RECORD OF BOREHOLE: 15-17

SHEET 1 OF 1 DATUM: CGVD28

LOCATION: N 5013731.1 ;E 364221.7 SAMPLER HAMMER, 64kg; DROP, 760mm BORING DATE: August 11, 2015

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

| ALE<br>S              | 2     | ₽                      | SOIL PROFILE   | 1 -         |                       | 37     | MPL      | _           | RESISTANCE, BLO                    | `,             | k, cm/s   |           | ₹8                         | PIEZOMETER                      |
|-----------------------|-------|------------------------|--|-------------|-----------------------|--------|----------|-------------|------------------------------------|----------------|---|-----------|----------------------------|---------------------------------|
| DEPTH SCALE<br>METRES |       | BORING MEI HOD         | DESCRIPTION  | STRATA PLOT | ELEV.<br>DEPTH<br>(m) | NUMBER | TYPE     | BLOWS/0.30m | 20 40<br>SHEAR STRENGTH<br>Cu, kPa | rem V. ⊕ U - ○ | Wp <del>· · · · · · · · · · · · · · · · · · ·</del> | ERCENT WI | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE<br>INSTALLATION |
|                       |       |                        | GROUND SURFACE   | o           | 93.79                 |        |          | ш           | 20 40                              | 60 80          | 20 40 60  | 80        |                            |                                 |
| 0                     |       |                        | FILL - (ML) gravelly sandy SILT; dark<br>brown and red brown, contains organic<br>matter; non-cohesive, moist, compact                           |             | 0.00                  | 1      | SS       | 15          |                                    |                |   |           |                            |                                 |
| 1                     |       |                        | FILL - (ML) CLAYEY SILT, some gravel; dark grey; cohesive, w>PL  |             | 92.57<br>1.22         | 3      | ss<br>ss | 15<br>7     |                                    |                |   |           |                            |                                 |
| 2                     | Auger | (Hollow Stem)          | TOPSOIL - (OL) ORGANIC SILT; black;<br>\moist  |             | 91.75<br>2.04<br>2.23 | 4      | SS       | 11          |                                    |                |   |           |                            |                                 |
| 3                     | Power | 200 mm Diam. (Hollow S | (ML) gravelly sandy SILT; grey brown, presence of cobbles and boulders inferred from auger resistance (GLACIAL TILL); non-cohesive, wet, compact |             |                       | 5      | SS       |             |                                    |                |   |           |                            |                                 |
| 4                     |       | •                      | (SM) gravelly SILTY SAND; grey,<br>presence of cobbles and boulders<br>inferred from auger resistance (GLACIAL                                   |             | 90.13<br>3.66         | 7      | SS       |             |                                    |                |   |           |                            |                                 |
|                       |       |                        | TILL); non-cohesive, wet, compact  |             | 88.9 <u>1</u><br>4.88 | 8      | SS       | 13          |                                    |                |   |           |                            |                                 |
| - 5                   |       |                        | End of Borehole  |             | 4.00                  |        |          |             |                                    |                |   |           |                            |                                 |
| 6                     |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 7                     |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 8                     |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| . 9                   |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
|                       |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 10                    |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| - 11                  |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 12                    |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 13                    |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 14                    |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
| 15                    |       |                        |  |             |                       |        |          |             |                                    |                |   |           |                            |                                 |
|                       | PT    | HS                     | CALE   |             |                       |        |          |             | Gold                               |                |   |           | LOG                        | GGED: PAH                       |

LOCATION: N 5013728.8 ;E 364221.0

#### RECORD OF PROBEHOLE: 15-17A

BORING DATE: August 12, 2015

DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m HYDRAULIC CONDUCTIVITY, k, cm/s SOIL PROFILE SAMPLES BORING METHOD DEPTH SCALE METRES ADDITIONAL LAB. TESTING PIEZOMETER BLOWS/0.30m STRATA PLOT 10<sup>-6</sup> NUMBER STANDPIPE INSTALLATION ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - ○ WATER CONTENT PERCENT DESCRIPTION DEPTH -o<sup>W</sup> Wp ⊢ (m) GROUND SURFACE 93.62 Refer to Record of Borehole 15-17 for stratigraphy Power Auger 2 Probable Glacial Till End of Probehole Auger Refusal 10 11 12 MIS-BHS 001 1523645.GPJ GAL-MIS.GDT 09/23/15 JM 13 15

DEPTH SCALE 1:75

LOGGED: PAH CHECKED: SD

SHEET 1 OF 1

DATUM: CGVD28

### **RECORD OF PROBEHOLE: 15-103**

BORING DATE: August 13, 2015

SAMPLER HAMMER, 64kg; DROP, 760mm

LOCATION: N 5013713.1 ;E 364379.7

PENETRATION TEST HAMMER, 64kg; DROP, 760mm

SHEET 1 OF 1

DATUM: CGVD28

| i i    | 면<br>오                                    | SOIL PROFILE  |             |  | SA     | AMPL |             | YNAMIC PENETRATION   HYDRAULIC CONDUCTIVITY, ESISTANCE, BLOWS/0.3m   HYDRAULIC CONDUCTIVITY, k, cm/s | ا ي ا                                   | PIEZOMETER                      |
|--------|---|---|-------------|--|--------|------|-------------|--|---|---------------------------------|
| METRES | BORING METHOD                             | DESCRIPTION   | STRATA PLOT | ELEV.<br>DEPTH<br>(m)                  | NUMBER | TYPE | BLOWS/0.30m | HEAR STRENGTH nat V. + Q - ● WATER CONTENT PERCE U, kPa  | S Z L L L L L L L L L L L L L L L L L L | OR<br>STANDPIPE<br>INSTALLATION |
| 0      |   | GROUND SURFACE  |             | 94.22                                  |        |      |             | 1 1 1 1 1 1  |   |                                 |
| 1      | Power Auger<br>200 mm Diam. (Hollow Stem) | TOPSOIL  Probable Silty Clay (Weathered Crust)  Probable Glacial Till, presence of cobbles and boulders inferred from auger refusal and auger resistance  |             | 0.00<br>93.92<br>0.30<br>93.61<br>0.61 |        |      |             |  |   |                                 |
| 3      | DCPT                                      | Probable Glacial Till   |             | 90.87<br>3.35                          |        |      |             | 127  |   |                                 |
| 5      |   | End of Probehole<br>Auger Refusal at 3.35 m<br>Dynamic Cone Refusal at 5.79 m   |             | 88.43<br>5.79                          |        |      |             | > 220<br>> 200<br>> 200<br>> 200   |   |                                 |
| 7      |   | Note:  The AW drill rods used to advance the DCPT were angled upon completion at an incline of about 10 degrees from vertical and were immovable/stuck. The top 1.5 metres was removed, but about 4.5 metres of the drill rods remain in the ground from about 1.5 to 6 metres depth. |             |  |        |      |             |  |   |                                 |
| 9      |   |   |             |  |        |      |             |  |   |                                 |
| 10     |   |   |             |  |        |      |             |  |   |                                 |
| 11     |   |   |             |  |        |      |             |  |   |                                 |
| 12     |   |   |             |  |        |      |             |  |   |                                 |
| 13     |   |   |             |  |        |      |             |  |   |                                 |
| 14     |   |   |             |  |        |      |             |  |   |                                 |
|        | DTU C                                     | CCALE   |             |  |        |      |             | <b>S</b> Golder<br>Associates  |   | GGED: PAH                       |

LOCATION: N 5013712.6 ;E 364379.1

# RECORD OF BOREHOLE: 16-301

BORING DATE: March 4-7, 2016

SHEET 1 OF 2 DATUM: CGVD28

| S                                | ТНОБ                     | SOIL PROFILE  | <br> -      |                       | SA     | MPL  |             | DYNAMIC<br>RESISTAN |              |             | ,             |                          | cm/s                        |    |                          | ING<br>ING                 | PIEZOMETER  |
|----------------------------------|--------------------------|---|-------------|-----------------------|--------|------|-------------|---------------------|--------------|-------------|---------------|--------------------------|-----------------------------|----|--------------------------|----------------------------|---|
| DEPTH SCALE<br>METRES            | BORING METHOD            | DESCRIPTION   | STRATA PLOT | ELEV.                 | NUMBER | TYPE | BLOWS/0.30m | 20<br>SHEAR ST      | 40<br>RENGTH | 60<br>nat V | 80<br>+ Q - ● | 10 <sup>-8</sup><br>WATE | 10 <sup>-6</sup><br>R CONTE |    | 10 <sup>-2</sup><br>CENT | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE   |
| 7<br>F <u>≅</u>                  | ORIN                     | DESCRIPTION   | IRAT/       | DEPTH<br>(m)          | NOM    | Ĕ    | OWS         | SHEAR ST<br>Cu, kPa |              | rem V.      | ⊕ Ū-Ŏ         | l ∧∧b ⊢                  |                             | W  | ⊣ wı                     | ADE<br>LAB.                | INSTALLATION  |
|                                  | В                        | GROUND SURFACE                                      | S           |                       |        |      | B           | 20                  | 40           | 60          | 80            | 20                       | 40                          | 60 | 80                       |                            |   |
| - 0                              |                          | Probable Sand                                       |             | 93.16<br>0.00         |        |      | $\vdash$    |                     |              | +           |               |                          |                             |    | _                        | +                          |   |
| - 1                              |                          | Probable Glacial Till                               |             | 92.2 <u>5</u><br>0.91 |        |      |             |                     |              |             |               |                          |                             |    |                          |                            | Ā   |
| - 2                              | Wash Boring<br>NW Casing |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| - 5<br>- 6                       | Wash                     |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| - 8                              |                          |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            | WL in open<br>borehole at 0.78 m<br>depth below<br>ground surface<br>upon completion of<br>drilling |
|                                  |                          |   |             | 83.36                 |        |      |             |                     |              |             |               |                          |                             |    |                          |                            | drilling  |
| - 10                             |                          | Borehole continued on RECORD OF<br>DRILLHOLE 16-301 | ***         | 9.8                   |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| - 11                             |                          |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| - 12                             |                          |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| - 13<br>- 14<br>- 15<br>DE<br>1: |                          |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| · 14                             |                          |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| - 15                             |                          |   |             |                       |        |      |             |                     |              |             |               |                          |                             |    |                          |                            |   |
| DE                               | PTH:                     | SCALE   |             |                       |        |      |             |                     | Gold<br>ssoc | ( <b>-)</b> |               |                          |                             |    |                          | L                          | OGGED: DWM  |

### RECORD OF DRILLHOLE: 16-301

DRILLING DATE: March 4-7, 2016

DRILL RIG: CME 850

SHEET 2 OF 2 DATUM: CGVD28

LOCATION: N 5013712.6 ;E 364379.1 INCLINATION: -90° AZIMUTH: ---

DRILLING CONTRACTOR: CCC

| SCALE  | RECORD                  |  | CLOG         | ELEV.         | No.     | COLOUR<br>% RETURN | JN<br>FL<br>SH<br>VN | - Jo<br>T - Fa<br>IR- SI<br>I - Vo<br>- C | hear<br>ein |                 | F     | 3D - E<br>50 - F<br>50 - C<br>5R - C | Beddin<br>oliatio<br>Contac<br>Orthog<br>Cleava | g<br>on<br>et<br>onal |     | PL<br>CU<br>UN<br>ST | - Pla<br>I- Cu<br>I- Un<br>- Ste | anar<br>irved<br>idulating<br>epped<br>egular | PO-<br>K -<br>SM-<br>Ro-<br>MR- | Slick<br>Smo<br>Rou | shed<br>cension<br>oth<br>gh | ded      | trea!                | NO<br>abb              | TE: Foreviat | Broke<br>for add<br>tions re<br>viations | itional<br>efer to         | list             |   |
|--------|-------------------------|--|--------------|---------------|---------|--------------------|----------------------|---|-------------|-----------------|-------|--------------------------------------|---|-----------------------|-----|----------------------|----------------------------------|---|---------------------------------|---------------------|------------------------------|----------|----------------------|------------------------|--------------|--|----------------------------|------------------|---|
| METRES | DRILLING RECORD         | DESCRIPTION  BEDROCK SURFACE                           | SYMBOLIC LOG | DEPTH<br>(m)  | RUN No. | FLUSH %            | TO<br>COF            | ECO<br>TAL<br>RE %                        | VER         | Y<br>.ID<br>E % | R.Q.D | ). FF<br>IN<br>F<br>0.               | ACT.  | B Ar                  | gle |                      | SCO<br>w.r.t.<br>RE<br>IS        | TYPE AND<br>DESCRI                            | / DATA                          | _                   | hanio                        | CO<br>CO | IYDR<br>NDU<br>K, cr | RAUL<br>JCTIV<br>m/sec | IC [         | Oiame                                    | etral<br>oadr<br>x<br>a) A | MC<br>-Q'<br>VG. |   |
| 10     |                         | Fresh, thinly to thickly bedded, grey DOLOMITE BEDROCK |              | 83.36<br>9.80 | 1       | 001                | 3                    |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 11     | Rotary Drill<br>NQ Core |  |              |               | 2       | 100                | 3                    |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 12     |                         |  |              | 80.16         | 3       | 100                | 2                    |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 13     | _                       | End of Drillhole                                       | <del></del>  | 13.00         |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            | - 11             | NL in open<br>corehole at 0.78 m<br>depth below<br>ground surface<br>upon completion of<br>drilling |
| 14     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  | g   |
| 15     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 16     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 17     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 18     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 19     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 20     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 21     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 22     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 23     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| 24     |                         |  |              |               |         |                    |                      |   |             |                 |       |                                      |   |                       |     |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            |                  |   |
| DE     | PTH S                   | CALE   | 1            |               |         |                    |                      |   |             |                 |       | <br> -                               | iiii  | Ш                     | Ш   |                      |                                  |   |                                 |                     |                              |          |                      |                        |              |  |                            | LO               | GGED: DWM   |

### RECORD OF BOREHOLE: 16-302

SHEET 1 OF 2 DATUM: CGVD28

LOCATION: N 5013746.6 ;E 364217.4 BORING DATE: March 4, 2016

| щ           |     | QQ                       | SOIL PROFILE                                     |             |               | SA     | MPL  | ES          | DYNAMIC PENETRA<br>RESISTANCE, BLOV | TION \ VS/0.3m | HYDRAULIC CONDUCTIVITY, k, cm/s   | ٥٦                         | PIEZOMETER                              |
|-------------|-----|--------------------------|--|-------------|---------------|--------|------|-------------|-------------------------------------|----------------|---|----------------------------|---|
| DEPTH SCALE | 2   | BORING METHOD            | _  | STRATA PLOT | ELEV.         | 3ER    | й    | BLOWS/0.30m | 20 40                               | 60 80          | 10 <sup>8</sup> 10 <sup>6</sup> 10 <sup>-4</sup> 10 <sup>-2</sup> WATER CONTENT PERCENT | ADDITIONAL<br>LAB. TESTING | OR<br>STANDPIPE                         |
| DEPTI       | M   | ORING                    | DESCRIPTION                                      | RATA        | DEPTH<br>(m)  | NUMBER | TYPE | OWS/        | SHEAR STRENGTH<br>Cu, kPa           | rem V. ⊕ U - ○ | WP I WI   | ADDI<br>LAB.               | INSTALLATION                            |
|             | +   | ă                        | GROUND SURFACE                                   | ST          |               |        |      | 표           | 20 40                               | 60 80          | 20 40 60 80   |                            |   |
| -           | ٥   | Т                        | Probable Sand                                    | , A. S.     | 93.06<br>0.00 |        |      |             |                                     |                |   |                            | =                                       |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           | 1   |                          | Probable Glacial Till                            |             | 92.15<br>0.91 |        |      |             |                                     |                |   |                            | 3                                       |
| Ē           | 1   |                          | Probable Glacial Till                            |             | 0.51          |        |      |             |                                     |                |   |                            | =                                       |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| F           | 2   |                          |  |             |               |        |      |             |                                     |                |   |                            | =                                       |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            | =                                       |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ē           | 3   |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ē           |     | D _                      |  |             |               |        |      |             |                                     |                |   |                            |   |
| _           | 4   | Wash Boring<br>NW Casing |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           |     | Was                      |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| E           | 5   |                          |  |             |               |        |      |             |                                     |                |   |                            | =                                       |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| E           | 6   |                          |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           | 7   |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
|             | 8   |                          |  |             | 85.04         |        |      |             |                                     |                |   |                            |   |
| Ė           |     |                          | Borehole continued on RECORD OF DRILLHOLE 16-302 |             | 8.02          |        |      |             |                                     |                |   |                            | =                                       |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           | 9   |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė,          | 10  |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            | =                                       |
| <u>.</u>    | 11  |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė,          | 12  |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ė (         | 14  |                          |  |             |               |        |      |             |                                     |                |   |                            | = |
| Ė           |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ē.,         | 13  |                          |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
| F           |     |                          |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
| E           |     |                          |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
| <u> </u>    | 14  |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| Ē           |     |                          |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
| Ε.          | 15  |                          |  |             |               |        |      |             |                                     |                |   |                            | ]                                       |
|             |     |                          |  |             |               |        |      |             |                                     |                |   |                            |   |
| ı           | DEF | TH S                     | SCALE  |             |               |        |      |             | Cold                                | O.F.           |   | L                          | OGGED: DWM                              |
|             | 1:7 | 5                        |  |             |               |        |      | ١           | Gold                                | iates          |   | СН                         | ECKED:                                  |

MIS-BHS 001 1523645.GPJ GAL-MIS.GDT 05/13/16 JM

INCLINATION: -90°

1:75

LOCATION: N 5013746.6 ;E 364217.4

AZIMUTH: ---

# RECORD OF DRILLHOLE: 16-302

DRILLING DATE: March 4, 2016

DRILL RIG: CME 850

DRILLING CONTRACTOR: CCC

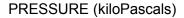
DATUM: CGVD28

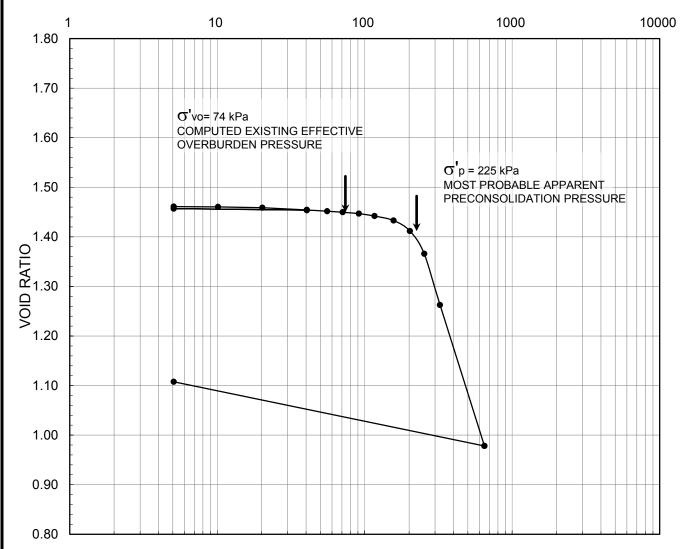
SHEET 2 OF 2

CHECKED:

BD - Bedding FO - Foliation CO - Contact OR - Orthogonal CL - Cleavage PO- Polished
K - Slickensided
SM- Smooth
Ro - Rough
MB- Mechanical Break

BR - Broken Rock
NOTE: For additional abbreviations refer to list of abbreviations & symbols. JN - Joint FLT - Fault SHR- Shear VN - Vein CJ - Conjugate PL - Planar CU- Curved UN- Undulating ST - Stepped IR - Irregular DRILLING RECORD DEPTH SCALE METRES SYMBOLIC LOG ELEV. DESCRIPTION R.Q.D. INDEX PER 0.25 m RUN HYDRAULIC CONDUCTIVITY K, cm/sec DEPTH RECOVERY DISCONTINUITY DATA Diametra Point Loa Index (MPa) DIP w.r.t. CORE AXIS (m) TOTAL SOLID CORE %  $\begin{array}{c} 10^{-8} \\ 10^{-2} \end{array}$ 8848 BEDROCK SURFACE 85.04 Fresh, thinly to thickly bedded, grey DOLOMITE BEDROCK Rotary Drill 3 11 81.86 11.20 End of Drillhole 12 13 14 15 16 17 18 19 20 MIS-RCK 004 1523645.GPJ GAL-MISS.GDT 05/13/16 JM 21 22 23 Golder DEPTH SCALE LOGGED: DWM





#### **LEGEND**

Borehole: 11-1A

 $w_i = 51\%$ 

 $S_0 = 97\%$ 

Sample: 1

 $W_f = 39\%$ 

Consolidation summary

10-1121-0264

 $C_c = 0.95$ 

Depth (m): 3.4

 $w_1 = 61\%$ 

 $C_r = 0.018$ 

 $w_p = 23\%$ 

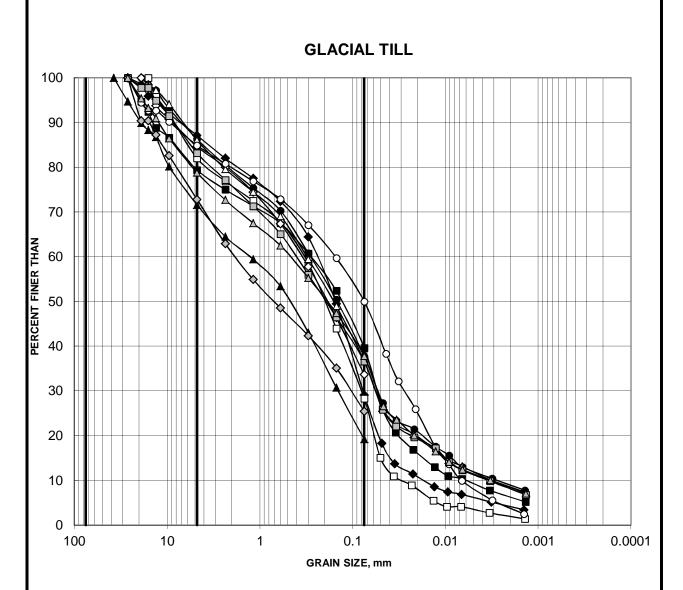


PROJECT No.

|   | SCALE  | AS SHOWN | TITLE |
|---|--------|----------|-------|
|   | DATE   | 04/15/16 |       |
|   | DESIGN | NA       | (     |
|   | CADD   | NA       |       |
|   | CHECK  |          |       |
| - | REVIEW |          |       |

# **CONSOLIDATION TEST RESULTS**

3



| Cobble | coarse | fine     | coarse | medium  | fine | SILT AND CLAY |
|--------|--------|----------|--------|---------|------|---------------|
| Size   | GRAV   | 'EL SIZE |        | SAND SI | ZE   | SILT AND CLAT |

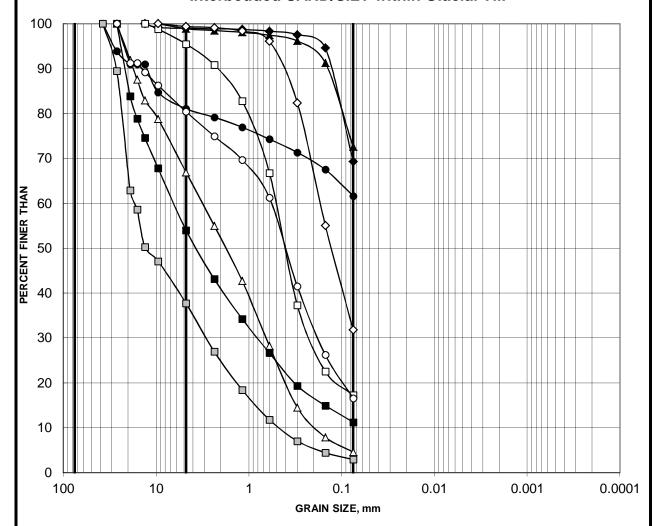
| Borehole             | Sample | Depth (m)   |
|----------------------|--------|-------------|
| <del></del> 15-1     | 10     | 6.86-7.47   |
| <del>-</del> 15-1    | 12     | 8.38-8.99   |
| <del></del> 15-1     | 14     | 10.67-11.28 |
| <b>-</b> ← 15-2      | 5      | 2.29-2.90   |
| <b>-</b> □ 15-2      | 12     | 7.62-7.77   |
| <b>-</b> ≎- 15-3     | 7      | 4.57-5.18   |
| <i>-</i> △ 15-3A     | 5      | 3.05-3.66   |
| > 15-6B              | 9      | 7.62-8.23   |
| <b>-</b> □ 15-8      | 6      | 3.81-4.42   |
| <b>-</b> ≎ 15-8      | 11     | 7.62-8.23   |
| <del>-</del> Δ 15-12 | 13     | 12.20-12.96 |

Created by: MI

Project: <u>1523645</u> **Golder Associates** 

# **GRAIN SIZE DISTRIBUTION**

# Interbedded SAND/SILT within Glacial Till



| Cobble | coarse | fine     | coarse | medium  | fine | SILT AND CLAY |
|--------|--------|----------|--------|---------|------|---------------|
| Size   | GRAV   | 'EL SIZE |        | SAND SI | ZE   | SILT AND CLAT |

| Borehole            | Sample | Depth (m) |
|---------------------|--------|-----------|
| <b></b> 15-2        | 9      | 5.34-5.79 |
| <b>→</b> 15-4       | 10     | 5.98-6.59 |
| <del>-▲-</del> 15-5 | 10     | 7.62-8.23 |
| <b></b> 15-6A       | 9      | 7.62-8.23 |
| —□— 15-6B           | 7      | 4.57-5.18 |
| <b>-</b> ≎ 15-7     | 6      | 3.81-4.12 |
| <i>-</i> △ 15-9A    | 5      | 7.62-8.23 |
| <b>-</b> ≎- 15-10   | 9      | 8.54-9.15 |
| <b>□-</b> 15-11     | 9      | 8.54-9.15 |
|                     |        |           |





# **APPENDIX C**

Results of Basic Chemical Analysis EXOVA Environmental Ontario Report No. 1603027



#### **EXOVA** ENVIRONMENTAL ONTARIO

# **Certificate of Analysis**



Client: Golder Associates Ltd. (Ottawa)

1931 Robertson Road

Ottawa, ON

K2H 5B7
Attention: Ms. Christine Ko

PO#:

Invoice to: Golder Associates Ltd. (Ottawa)

Report Number: 1603027
Date Submitted: 2016-03-02
Date Reported: 2016-03-08
Project: 1523044/1000
COC #: 805851

| Group             | Analyte                 | MRL   | Units  | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D.  Guideline | 1229119<br>Soil<br>2016-02-22<br>16-105 SA3/2.5-4.5 | 1229120<br>Soil<br>2016-02-22<br>16-107 SA4/5-6.4 |
|-------------------|-------------------------|-------|--------|---|---|---|
| Agri Soil         | рН                      | 2.0   |        |   | 7.4   | 8.3   |
| General Chemistry | CI                      | 0.002 | %      |   | 0.010   | 0.004   |
|                   | Electrical Conductivity | 0.05  | mS/cm  |   | 0.33  | 0.15  |
|                   | Resistivity             | 1     | ohm-cm |   | 3030  | 6670  |
|                   | SO4                     | 0.01  | %      |   | <0.01   | 0.01  |

Guideline = \* = Guideline Exceedence

All analysis completed in Ottawa, Ontario (unless otherwise indicated by \*\* which indicates analysis was completed in Mississauga, Ontario).

Results relate only to the parameters tested on the samples submitted.

Methods references and/or additional QA/QC information available on request.

MRL = Method Reporting Limit, AO = Aesthetic Objective, OG = Operational Guideline, MAC = Maximum Acceptable Concentration, IMAC = Interim Maximum Acceptable Concentration, STD = Standard, PWQO = Provincial Water Quality Guideline, IPWQO = Interim Provincial Water Quality Objective, TDR = Typical Desired Range

As a global, employee-owned organisation with over 50 years of experience, Golder Associates is driven by our purpose to engineer earth's development while preserving earth's integrity. We deliver solutions that help our clients achieve their sustainable development goals by providing a wide range of independent consulting, design and construction services in our specialist areas of earth, environment and energy.

For more information, visit golder.com

Africa + 27 11 254 4800
Asia + 86 21 6258 5522
Australasia + 61 3 8862 3500
Europe + 44 1628 851851
North America + 1 800 275 3281
South America + 56 2 2616 2000

solutions@golder.com www.golder.com

Golder Associates Ltd. 1931 Robertson Road Ottawa, Ontario, K2H 5B7 Canada

T: +1 (613) 592 9600

