

OUR REF: TO3131TOI00

TB Check List

Report Context

prepared for: Bronson Inc.

789 King Street West Toronto, ON M5V 1M6

V	Municipal address;
	No inclusion rational:
V	Location relative to major elements of the existing transportation system (e.g., the
	site is located in the southwest quadrant of the intersection of Main Street/ First
	Street, 600metres from the Maple Street Rapid Transit Station);
	No inclusion rational:
V	Existing land uses or permitted use provisions in the Official Plan, Zoning By-law,
	etc.;
	No inclusion rational:
✓	Proposed land uses and relevant planning regulations to be used in the analysis;
	No inclusion rational:
V	Proposed development size (building size, number of residential units, etc.) and
	location on site;
	No inclusion rational:
	Estimated date of occupancy;
	No inclusion rational: Unknown
V	Planned phasing of development;
	No inclusion rational:
V	Proposed number of parking spaces (not relevant for Draft Plans of Subdivision);
	No inclusion rational:
V	Proposed access points and type of access (full turns, right-in/ right-out, turning
	restrictions, etc.
	No inclusion rational:
V	Study area;
	No inclusion rational:



Existing Conditions

V	Existing roads and ramps in the study area, including jurisdiction, classification,
	number of lanes, and posted speed limit;
	No inclusion rational:
V	Existing intersections, indicating type of control, lane configurations, turning
	restrictions, and any other relevant data (e.g., extraordinary lane widths, grades,
	etc.);
	No inclusion rational:
V	Existing access points to adjacent developments (both sides of all roads bordering
	the site);
	No inclusion rational:
V	Existing transit system, including stations and stops;
	No inclusion rational:
V	Existing on- and off-road bicycle facilities and pedestrian sidewalks and pathway
	networks;
	No inclusion rational:
V	Existing system operations (V/C, LOS);
	No inclusion rational:
V	Major trip generators/ attractors within the Study Area should be indicated.
	No inclusion rational:
Dem	and Forecasting
	General background growth;
	No inclusion rational: Not required for Transportation Brief
	Other study area developments;
	No inclusion rational: Not required for Transportation Brief
	Changes to the study area road network;
	No inclusion rational: Not required for Transportation Brief
	Future background system operations (V/C, LOS, queue lengths):
	No inclusion rational: Not required for Transportation Brief
V	Trip generation rates;
	No inclusion rational:
V	Trip distribution and assignment.
	No inclusion rational



Impact Analysis

V	Total future system operations (V/C, LOS, queue lengths);	
	No inclusion rational:	
V	Signal and auxiliary lane (device) warrants;	
	No inclusion rational:	
V	Operational/ safety assessment (e.g., sight line assessment where grades are a	ın
	issue);	
	No inclusion rational:	
V	Storage analysis for closely spaced intersections;	
	No inclusion rational:	
V	Pedestrian and bicycle network connections and continuity;	
	No inclusion rational:	
V	On-site circulation and design;	
	No inclusion rational:	
V	Potential for neighbourhood impacts; and TDM.	
	No inclusion rational:	
V	Synchro Files	
	No inclusion rational:	
CTS		
Impa	act Analysis	
	Network Capacity Analysis;	
	No inclusion rational:	
	Non-auto network connections and continuity;	
	No inclusion rational:	
	Potential for community impacts, and TDM.	
	No inclusion rational:	
	Synchro Files	
	No inclusion rational:	
	Screenline Analysis	
	No inclusion rational:	



192 & 196 Bronson Avenue/31 Cambridge Street

Transportation Brief

prepared for:

Bronson Inc.

786 King Street West Toronto, ON M5V 1M6

prepared by:

Delcan

1223 Michael Street

Suite 100 Ottawa, ON K1J 7T2

22 July 2013

TO3131TOI00

Table o	f Contents	
1. Intro	oduction	1
2. Scop	e of Work	1
3. Exist	ing Conditions	3
3.1	Study Area	3
	Area Road Network	
	Existing Study Area Intersections	
	Transit Network	
3.5	Bicycle and Pedestrian Facilities	7
3.6	Existing Intersection Operations	7
4. Dem	and Forecasting	9
4.1	Site Vehicle Trip Generation	9
5. Futu	re Traffic Operations	3
5.1	Neighbourhood Impacts	4
6. Tran	sportation Demand Management1	5
7. Site	Plan Review 1	5
8. Findi	ngs and Recommendations 1	7
lict of D	Eiguros	
List of F	Local Context	1
•	Preliminary Site Plan	
O	Area Transit Network	
•	Existing Peak Hour Traffic Volumes	
•	'New' and 'Pass-by' Site-Generated Vehicle Trips	
-	Projected Traffic Volumes	
List of T	Tables	
	Existing Performance at Study Area Intersections	7
	ITE Trip Generation Rates	
	Modified Person Trip Generation	
Table 4:	High-Rise Condominium Site Trip Generation	0
Table 5:	Retail Site Trip Generation	0
Table 6:	Total Site Vehicle Trip Generation	1
Table 7:	Projected Performance at Study Area Intersections	4
Append	lices	
Appendix	A – Current Peak Hour Volumes	
Appendix	B – SYNCHRO Capacity Analysis: Existing Conditions	
Appendix	C - SYNCHRO Capacity Analysis: Projected Conditions	
Appendix	D – SYNCHRO Capacity Analysis: Modified Bronson/Slater Intersection	



1. Introduction

From the information provided, a mixed-use 18 storey residential condominium building (218 units) with ground floor retail (9,379 ft²), with access to/from Cambridge Street North, is being proposed for a site located on the west side of Bronson Avenue approximately 75 m north of the Bronson/Primrose intersection. The site, which is municipally known as 192 and 196 Bronson Avenue/31 Cambridge Street, is currently occupied by an office building and an approximate 40 space parking lot. The site's local context is depicted as Figure 1 and the preliminary Site Plan is depicted as Figure 2.

Figure 1: Local Context



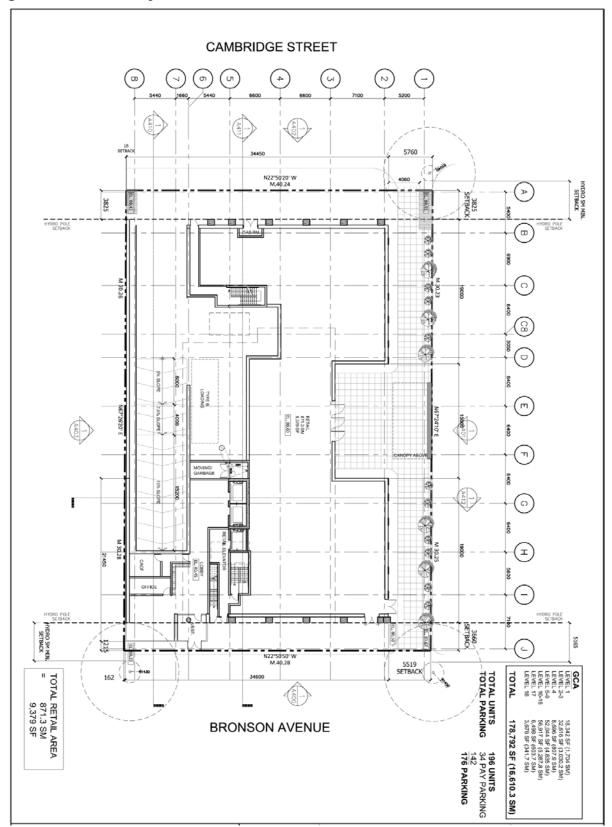
2. SCOPE OF WORK

Based on the ensuing trip generation and our review of the City's Transportation Impact Assessment Guidelines (TIA), the proposed development is projected to generate less than the City's threshold for requiring a transportation impact assessment. As such, no further traffic analysis is required. However, this modified Transportation Brief has been prepared to assist in the application/review process. Following discussions with City Staff we have prepared this report that captures only the relevant transportation issues, which are as follows:

- existing traffic conditions at adjacent intersections;
- future site trip generation; and
- Site Plan issues, including pedestrian access, proposed vehicle access, parking, loading and circulation layout.



Figure 2: Preliminary Site Plan





3. EXISTING CONDITIONS

3.1 Study Area

At the preconsultation meeting, it was agreed with City Staff that the study area should consist of the signalized intersections of Bronson/Slater, Bronson/Laurier, Bronson/Gloucester, Bronson/Primrose, Bronson/Somerset and the unsignalized Cambridge/Primrose intersections.

3.2 Area Road Network

Bronson Avenue is a north-south arterial roadway with a four lane cross-section. The unposted speed limit is understood to be 50 km/h. Within the study area, on-street parking is permitted in certain areas during evenings and weekends only (parking not permitted from 7 AM to 6 PM).

Slater Street is an arterial roadway, which operates as a one-way in the eastbound direction. Its cross-section consists of two passenger vehicle travel lane and one transit/taxi only travel lane. On-street parking, located along the north side of the road, is permitted during evenings and weekends only (parking is not permitted from 7 AM to 6 PM). Its unposted speed limit is understood to be 50 km/h.

Laurier Avenue is an east-west arterial roadway with a two lane cross-section. Within the study area, there are no auxiliary turn lanes, and on-street parking is not permitted. Its unposted speed limit is understood to be 50 km/h.

Gloucester Street is a local roadway, which operates as a one-way in the westbound direction. Its cross-section consists of a single travel lane with on-street parking located along the south side of the road. Its unposted speed limit is understood to be 50 km/h.

Primrose Avenue is a local roadway with an unposted speed limit of 50 km/h. Its cross-section consists of a single travel lane in each direction with on-street parking located along the north side of the road.

Cambridge Street N is a local roadway with an unposted speed limit of 50 km/h. Its cross-section consists of a single travel lane in each direction with on-street parking located along both sides of the road.

Somerset Street W is an east-west arterial roadway with an unposted speed limit of 50 km/h. It has a four lane cross-section with on-street parking provided along both sides of the road. With the completion of the Bronson Reconstruction, auxiliary left-turn lanes will be provided at the Bronson/Somerset intersection.



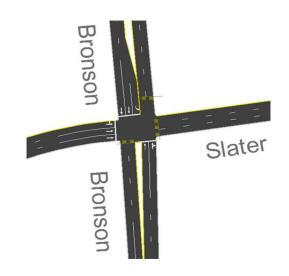
3.3 Existing Study Area Intersections

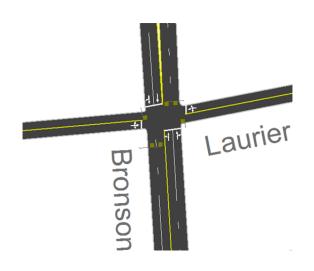
Bronson/Slater

The Bronson/Slater intersection is а signalized four-legged intersection. The northbound approach consists of a single through lane and a shared through/rightturn lane. The southbound approach consists of two through lanes and a left-turn lane. The eastbound approach consists of a shared through/left-turn lane and two through lanes (one through lane is for transit only). Eastbound right-turns are not permitted, however traffic counts reveal that approximately 30 vehicles turn right in an 8 hour period. As Slater Street operates as a one-way roadway in the eastbound direction, the northbound left-turn and southbound right-turn movements are not permitted.



The Bronson/Laurier intersection signalized four-legged intersection. northbound approach consists of a shared through/left-turn lane and through/right-turn lane. The southbound approach consists of a single through lane and a shared through/right-turn lane. The eastbound approach consists of a single shared left-turn/right-turn lane. The westbound approach consists of a single full-movement lane. The southbound leftturn and the eastbound through movements are not permitted at this location. However, traffic counts reveal that approximately 20 vehicles turn southbound left and approximately 40 vehicles go eastbound through during an 8-hour period.







Bronson/Gloucester

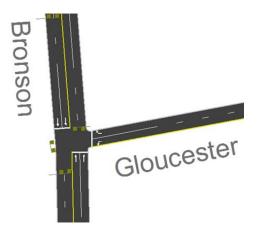
The Bronson/Gloucester intersection is a signalized three-legged intersection. The northbound and southbound approaches consist of two through lanes. The westbound approach consists of a right-turn lane and a left-turn lane. As Gloucester Street operates as a one-way roadway in the westbound direction, the southbound left-turn and northbound right-turn movements are not permitted at this location.

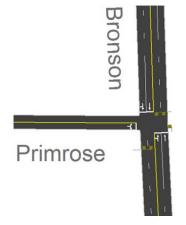
Bronson/Primrose

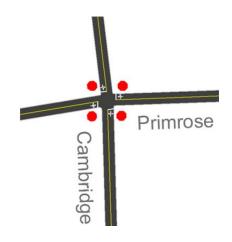
The Bronson/Primrose intersection is a signalized three-legged intersection. The northbound approach consists of a single through lane and a shared through/left-turn lane. The southbound approach consists of a single through lane and a shared through/right-turn lane. The eastbound approach consists of a single full movement lane. All movements are permitted at this location.

Cambridge/Primrose

The Cambridge/Primrose intersection is an unsignalized four-legged intersection with STOP control at all four approaches. All approaches consist of single full-movement lanes. The south leg of the intersection has a narrow cross-section, with caution signs indicating the narrow roadway and a recommended speed of 20 km/h. All movements are permitted at this location.









Bronson/Somerset W

The Bronson/Somerset W intersection is a signalized four-legged intersection. north and southbound approaches consist of shared through/right-turn lanes and shared through/left-turn lanes. When the Bronson Avenue reconstruction is complete, the east and westbound approaches will consist of single left-turn lanes and shared through/right-turn lanes. All movements are permitted at this location, however, at all four approaches right-turns-on-red are not permitted.



3.4 Transit Network

The proposed development is in close proximity (approximately 300 m) from the existing rapid transit corridor along Slater Street/Albert Street. The rapid transit corridor provides easy access to employment in both the east and west directions and to the downtown core. In the future, this Transitway will be replaced by the Confederation Line LRT project, which will have a station located approximately 600 m walking distance from the site. Bus stops for regular (Black) Route #4 located on Bronson Avenue approximately 75 m walking distance north of the site. Figure 3 depicts the existing transit within the vicinity of the site.

Albert 16

Slater A 9 9 12

Albert 16

St. Vincent

St. Vincent

Somerset

AH

Gladstone

Argyle

Argyle

Argyle

Figure 3: Area Transit Network



3.5 Bicycle and Pedestrian Facilities

According to the City's 2008 Official Cycling Plan (OCP), Slater Street, Laurier Avenue/Cambridge Street and Somerset Street are classified as "Spine or City-wide" cycling routes. Within the study area, shared use lanes currently exist along Slater Street and Cambridge Street and bicycle lanes are provided along Laurier Avenue. The bike lanes along Laurier Avenue are to be made permanent with potential additional links to create a segregated cross-town cycling spine. In the future, shared-use lanes are proposed along Somerset Street by 2018.

Connecting pedestrians to transit service and other adjacent development, sidewalks are currently provided along both sides of all study area roadways.

3.6 Existing Intersection Operations

Illustrated in Figure 4, and included as Appendix A, are the most recent weekday morning and afternoon peak hour traffic volumes obtained from the City of Ottawa for the Bronson/Slater, Bronson/Laurier, Bronson/Gloucester, Bronson/Primrose, Bronson/Somerset and Cambridge/Primrose intersections.

Table 1 provides a summary of existing traffic operations at key study area intersections, based on the Synchro (V8) traffic analysis software. The subject intersections were assessed in terms of the volume-to-capacity (v/c) ratio and the corresponding Level of Service (LoS) for the 'critical movement(s)'. The intersections 'as a whole' were assessed based on a weighted v/c ratio. The Synchro model output of existing conditions is provided within Appendix B.

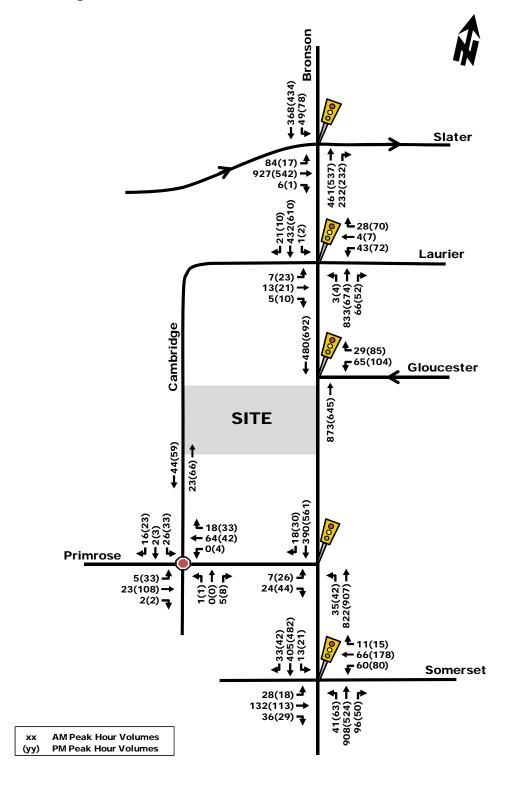
Table 1: Existing Performance at Study Area Intersections

		1	Weekday AM	Peak (PM Pe	ak)	
		Critical Mov	ement	Intersec	tion 'as	a whole'
Intersection	LoS	max. v/c or avg. delay (s)	Movement	Delay (s)	LoS	v/c
Bronson/Slater	E(B)	0.96(0.67)	EBT(NBT)	25.4(14.0)	C(A)	0.79(0.60)
Bronson/Laurier	A(A)	0.43(0.57)	NBT(WBT)	4.5(6.8)	A(A)	0.35(0.40)
Bronson/Gloucester	A(A)	0.39(0.35)	NBT(SBT)	2.7(3.3)	A(A)	0.38(0.35)
Bronson/Primrose	A(A)	0.38(0.49)	NBT(NBT)	4.8(7.9)	A(A)	0.37(0.47)
Bronson/Somerset	D(A)	0.82(0.55)	NBT(NBT)	24.7(21.7)	B(A)	0.64(0.49)
Cambridge/Primrose	A(A)	7.3(8.1)	EBT(EBT)	7.3(7.8)	-	-

Note: Analysis of signalized intersections assumes a PHF of 0.95, a saturation flow rate of 1800 veh/h/lane, and a CBD type area.



Figure 4: Existing Peak Hour Traffic Volumes





As shown in Table 1, study area intersections 'as a whole' are operating at an acceptable LoS 'C' or better during the weekday morning and afternoon peak hours, with respect to the City of Ottawa operating standards of LoS 'D' or better (0.90 > v/c > 0.00).

With regard to 'critical movements' at study area intersections, they are operating at an acceptable LoS 'D' or better during the weekday morning and afternoon peak hours, with the exception of the eastbound through movement at the Bronson/Slater intersection during the morning peak hour, which is operating at capacity (LoS 'E').

4. DEMAND FORECASTING

4.1 Site Vehicle Trip Generation

Summarized in Table 2, are the appropriate vehicle trip generation rates for the proposed land uses obtained from the 9th Edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

Table 2: ITE Trip Generation Rates

Landilloo	Data	Trip I	Rates
Land Use	Source	AM Peak	PM Peak
High-Rise	ITE	T = 0.34(du);	T = 0.38(du);
Condominium	232	T = 0.29(du) + 28.86	T= 0.34(du) + 15.47
Specialty Retail	ITE	T = 1.36(X);	T = 2.71(X);
Centre	826	T = 1.20(X) + 10.74	T = 2.40(X) + 21.48

Notes: T = Average Vehicle Trip Ends

 $X = 1,000 \text{ ft}^2 \text{ Gross Floor Area}$

du = dwelling units

Specialty Retail AM Peak is assumed to be 50% of the PM Peak

As ITE trip generation surveys only record vehicle trips and typically reflect highly suburban locations (with little to no access by travel modes other than private automobiles), adjustment factors appropriate to the more urban study area context were applied to attain estimates of person trips for the proposed development. This approach is considered appropriate within the industry for urban infill developments.

To convert ITE vehicle trip rates to person trips, an auto occupancy factor and a non-auto trip factor were applied to the ITE vehicle trip rates. Our review of available literature suggests that a combined factor of approximately 1.3 is considered reasonable to account for typical North American auto occupancy values of approximately 1.15 and combined transit and non-motorized modal shares of less than 10%. As such, the person trip generation for the proposed site is summarized in Table 3.



Table 3: Modified Person Trip Generation

Land Use	Data	Area	AM Pe	ak (pe	rsons)	PM Pe	eak (pe	rsons)
Land USe	Source	Alea	In	Out	Total	In	Out	Total
High-Rise	ITE	218	22	98	120	71	45	116
Condominium	232	Units	22	90	120	7 1	40	110
Specialty Retail	ITE	9,379 ft ²	16	13	29	25	32	57
Specially Retail	826	9,37911	10	13	29	25	32	37
	Total Per	rson Trips	38	111	149	96	77	173

Note: 1.3 factor to account for typical North American auto occupancy values of approximately 1.15 and combined transit and non-motorized modal shares of less than 10%

The person trips shown in Table 3 for the proposed site were then reduced by modal share values based on the 2011 TRANS O-D survey to reflect the site's location and proximity to employment, shopping uses and transit availability. As the site is located in the urban area, close to the downtown core, a high non-auto modal split has been applied, although it is noteworthy that higher non-auto mode splits are likely being realized in highly urban areas of the City such as the location of the proposed development. Modal share values for the proposed uses are summarized in Table 4 and 5, with the total site vehicle trip generation summarized in Table 6.

Table 4: High-Rise Condominium Site Trip Generation

			AM Peak	(PM Peak	(
Travel Mode	Mode Share	(Pe	ersons/	hr)	(Pe	ersons/	hr)
		In	Out	Total	In	Out	Total
Auto Driver	50%	11	49	60	36	23	59
Auto Passenger	10%	2	9	11	7	4	11
Transit	25%	5	25	30	18	11	29
Non-motorized	15%	4	15	19	10	7	17
Total Person Trips	100%	22	98	120	71	45	116
Total 'N	ew' Auto Trips	11	49	60	36	23	59

Table 5: Retail Site Trip Generation

			AM Peak	(l	PM Peak	(
Travel Mode	Mode Share	(Pe	ersons/	hr)	(Pe	ersons/	hr)
		In	Out	Total	In	Out	Total
Auto Driver	50%	8	7	15	13	16	29
Auto Passenger	10%	1	1	2	2	3	5
Transit	25%	4	4	8	7	8	15
Non-motorized	15%	3	1	4	3	5	8
Total Person Trips	100%	16	13	29	25	32	57
Less Retai	l Pass-by (30%)	-2	-2	-4	-4	-4	-8
Total 'N	ew' Auto Trips	6	5	11	9	12	21



Table 6: Total Site Vehicle Trip Generation

Landillas	AM F	Peak (ve	h/h)	PM F	Peak (ve	h/h)
Land Use	In	Out	Total	In	Out	Total
High-Rise Condominiums	11	49	60	36	23	59
Specialty Retail	8	7	15	13	16	29
Retail Pass-By (30%)	-2	-2	-4	-4	-4	-8
Total 'New' Auto Trips	17	54	71	45	35	80

As shown in Table 6, the resulting number of potential 'new' two-way vehicle trips for the proposed development is approximately 71 and 80 veh/h during the weekday morning and afternoon peak hours, respectively. However, the 'net' increase in site traffic generation will be less than this, as the site is currently occupied by an approximate 40 space parking lot. For this size of parking lot, an estimated 20 veh/h two-way can be assumed during both the morning and afternoon peak hours. Therefore the 'net' two-way vehicle trip generation for the proposed development is approximately 50 and 60 veh/h during the weekday morning and afternoon peak hours, respectively. This projected number of two-way vehicle trips is less than the City's 75 veh/h threshold for requiring a transportation impact assessment.

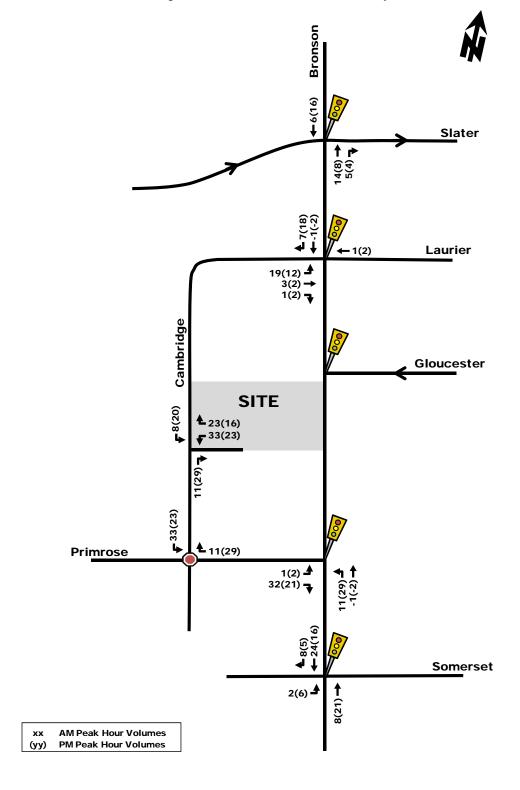
Given the existing site driveway connection is to Bronson Avenue, and in an abundance of caution, the ensuing analysis assumes the total vehicle trips of 71 and 80 veh/h. These 'new' trips were then distributed based on the site's connectivity to the existing road network and our knowledge of the surrounding area. The resultant distribution is assumed to be:

- 45% to/from the south via Bronson Avenue towards HWY 417;
- 25% to/from the north via Bronson;
- 15% to/from the west via Somerset Street; and
- 15% to/from the east Albert Street/Slater Street and Laurier Avenue;
 100%

'New' and 'Pass-by' site-generated trips are illustrated in Figure 5.



Figure 5: 'New' and 'Pass-by' Site-Generated Vehicle Trips

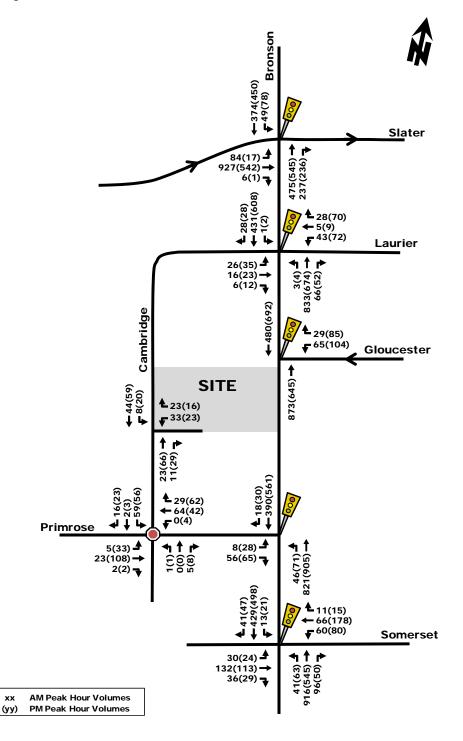




5. FUTURE TRAFFIC OPERATIONS

For the purpose of this study, the total projected traffic volumes were derived by superimposing 'new' and 'pass-by' site-generated traffic volumes (Figure 5) onto existing volumes (Figure 4). The resulting total projected traffic volumes are illustrated as Figure 6.

Figure 6: Projected Traffic Volumes





With no signal timing plan or roadway modifications, the signalized study area intersections 'as a whole' are projected to operate similar to existing conditions summarized in Table 1. The Synchro model output of projected conditions without roadway or signal modifications are provided within Appendix C.

Table 7: Projected Performance at Study Area Intersections

			Mookdov AM	Dook (DM Do	ماد)	
			Weekday AM	·		
		Critical Mov	ement	Intersec	tion 'as	a whole'
Intersection	LoS	max. v/c or avg. delay (s)	Movement	Delay (s)	LoS	v/c
Bronson/Slater	E(B)	0.96(0.68)	EBT(NBT)	25.3(14.4)	C(B)	0.80(0.61)
Bronson/Laurier	A(A)	0.43(0.57)	NBT(WBT)	4.8(6.9)	A(A)	0.35(0.40)
Bronson/Gloucester	A(A)	0.39(0.35)	NBT(SBT)	2.2(3.0)	A(A)	0.38(0.35)
Bronson/Primrose	A(A)	0.43(0.59)	NBT(NBT)	5.8(9.8)	A(A)	0.41(0.56)
Bronson/Somerset	D(A)	0.83(0.57)	NBT(NBT)	24.9(21.8)	B(A)	0.65(0.50)
Cambridge/Primrose	A(A)	7.7(8.3)	SBT(EBT)	7.5(8.0)	-	-
Cambridge/Site	A(A)	9.0(9.4)	WBL(WBL)	4.0(2.5)	-	-

Note: Analysis of signalized intersections assumes a PHF of 0.95, a saturation flow rate of 1800 veh/h/lane, and a CBD type area.

As the proposed development is projected to contribute approximately 1 to 2 new vehicles a minute to the area road network during the weekday morning and afternoon peak hours, it has no off-site transportation impacts or requirements. As previously mentioned, this projected impact is the 'worst case scenario' as the existing site's traffic has not been removed from the area road network.

The proposed site driveway connection to Cambridge Street is projected to operate with acceptable delays of approximately 9 seconds with 95th percentile queues of approximately 1.5 metres (no more than 1 vehicle in queue) during peak hours.

Possible mitigative measures to improve performance at the Bronson/Slater intersection during the morning peak hour include optimizing splits in Synchro. This will result in an LoS 'D' or better for all movements. The Synchro model output of this modification is included as Appendix D.

5.1 Neighbourhood Impacts

With regard to Primrose Avenue and Cambridge Street south of the proposed site driveway, which are classified as local roadways, the proposed development is projected to add approximately 44 and 52 veh/h to current two-way traffic volumes during the weekday morning and afternoon peak hours, which equates to approximately 1 new vehicle every minute. The total amount of two-way vehicle traffic (existing plus site-generated traffic)



equates to approximately 2 to 3 vehicles every minute travelling along Cambridge Street and 3 to 5 vehicles every minute travelling along Primrose Avenue during peak hours.

With regard to Cambridge Street north of the site, the proposed development is projected to add approximately 31 and 36 veh/h to current two-way traffic volumes during the weekday morning and afternoon peak hours, which equates to approximately 1 new vehicle every 2 minutes. The total amount of two-way vehicle traffic along Cambridge Street north of the site equates to approximately 2 to 3 vehicles every minute. The current volumes along these local streets are relatively low and the increase in vehicle volumes along these roads from the proposed development is considered negligible.

The site driveway has been proposed to Cambridge Street in part based on the recommendation of the City's Urban Design Review Panel. As a result of the site driveway connection to Cambridge Street, traffic along Cambridge Street and Primrose Avenue will increase as outlined above. However, it is our opinion that the total expected two-way traffic volumes along these two roads are acceptable for local roadways.

With respect to neighbourhood transit, the site is projected to generate an approximate total of 38 and 44 'new' two-way person transit trips during the weekday morning and afternoon peak hours, respectively. This amount of demand will be spread between local bus routes and rapid transit facilities (both existing and proposed) and can be easily accommodated.

6. Transportation Demand Management

Depending on the nature of a development, Transportation Demand Management (TDM) strategies have the potential to be an integral part of a planned development in order to address and support the City of Ottawa policies with regard to TDM. For this particular site, its proximity to the existing transit service is considered very advantageous in lessening the reliance on the private automobile. A number of TDM measures could also be considered, including:

- improving the quality and safety of pedestrian facilities, such as enhanced sidewalks/lighting;
- improving bicycle facilities, such as provision of secure on-site bicycle storage;
- provide change/shower area for any on-site staff; and
- inclusion of dedicated car-sharing parking spaces.

These are important TDM strategies to encourage active modes of transportation to/from the site.

7. SITE PLAN REVIEW

This section provides an overview of site access, parking requirements, pedestrian circulation and transit accessibility. The Site Plan was previously illustrated in Figure 2.

Parking

A total of 142 residential parking spaces and 34 retail/visitor parking spaces are proposed to serve the subject site. This amount of residential parking is sufficient with respect to the



City's Zoning By-Law requirements for Area B, identified in Schedule 1 of the City's Zoning By-Law. The proponent will be seeking a blended parking space rate for the 34 parking spaces proposed for the retail land use and visitor parking.

With regard to parking space dimensioning, they are noted as being a 2.6 m in width and 5.2 m in length which meets the City's By-Law requirements.

Site Circulation

With regard to on-site circulation, the proposed drive aisle widths are noted as 6.7 m in width which meets the City's By-Law requirements.

The ramp providing access to the lower level parking is noted as having a 5% to 15% grade. The City's Private Approach By-Law states that a private approach may be greater than 6% but shall not exceed 12% provided that a subsurface melting device sufficient to keep the private approach free of ice at all times is installed and properly maintained. In addition, our review of the available industry literature indicates that ramp grades should ideally not exceed 12%, however, up to 15% is acceptable if pedestrians are specifically excluded from using the ramp, and appropriate transition grades are utilized. We have also observed ramps in the 15% to 20% range working acceptably indoors with low vehicular volumes. Given the low amount of projected vehicles entering/existing the proposed development and provided appropriate pedestrian signage is installed, the ramp grade is considered acceptable.

Access Requirements

Based on projected volumes and proximity to adjacent intersections, additional traffic control/auxiliary turn lanes are not required at the proposed driveway connection.

The proposed width of the site driveway is noted as approximately 6.7 m which meets the City's By-Law requirements.

With regard to the proposed location of the site driveway, it is noted as being less than 3.0 metres from the adjacent property line, which does not meet the City's Private Approach By-Law requirements. The adjacent site (an approximate 30 space parking lot) and the proposed site are both expected to generate low vehicular volumes, Cambridge Street is relatively flat with good visibility for drivers to/from the proposed site and the existing two-way vehicle volumes along Cambridge Street are relatively low. In addition, the site driveway consists of approximately 20 m of roadway with 0% grade which will provide good visibility for drivers of the sidewalk, on-coming traffic along Cambridge Road and vehicles exiting the adjacent site. Therefore, based on the above, the proposed location of the site driveway is considered to be both safe and acceptable.

As for heavy vehicles, sufficient turning radii should be provided to accommodate garbage/recycling collection vehicles.



Pedestrians/Transit

An east-west pedestrian pathway is proposed from Bronson Avenue to Cambridge Street along the north side of the site connecting pedestrians to transit service and other adjacent development. This connection will be publicly accessible and allow pedestrian movement through the site between Cambridge Street and Bronson Avenue.

Based on the projected site-generated transit person trips to/from the proposed site, the current area transit service will be able to accommodate the additional person traffic.

Bicycles

A total of 202 below grade bicycle parking spaces and 24 above grade bicycle parking spaces are proposed to serve the development, which is sufficient with respect to the City's By-Law requirement. The location of the above grade bicycle parking stalls is not identified on the attached Site Plan, however, with respect to the City's By-Law requirements, bicycle parking should be located in well-lit areas, close to main building entrances.

8. FINDINGS AND RECOMMENDATIONS

Based on the foregoing analysis of the proposed site, the following transportation-related conclusions are offered:

- Study area intersections are currently operating at an acceptable Level of Service during the weekday morning and afternoon peak hours with the exception of the 'critical movement' at the Bronson/Slater intersection during the morning peak hour;
- The proposed development is projected to generate approximately 71 and 80 veh/h
 during the weekday morning and afternoon peak hours, respectively. These volumes
 equate to approximately 1 to 2 new vehicles every minute, and this amount of new
 traffic is considered relatively insignificant with no measureable impact on the
 operations of adjacent intersections;
- Future traffic conditions at study area intersections are projected to operate similar to existing conditions;
- A mitigative measure to improve both the existing and projected performance of the Bronson/Slater intersection during the morning peak hour is to optimize splits in Synchro;
- The site's on-site circulation and parking layout is well arranged and meets By-Law requirements;
- The proposed location of the site driveway is noted as being less than 3.0 m from the
 adjacent property line, which does not meet the City's minimum requirement.
 However, the proposed driveway is flat for approximately 20 m which provides good
 visibility for drivers of the sidewalk, the roadway and the adjacent site driveway.
 Therefore, there will be no vehicular conflicts as a result of driveways being too close
 in proximity;



- The vehicle residential parking supply meets By-Law requirements and the proponent will be seeking a blended rate for the retail and visitor parking requirements. The parking stalls are noted as meeting By-Law requirements in terms of dimensioning; and
- The parking garage access/egress ramp grades are noted as 15% (with transitions), which is acceptable provided appropriate signage is installed.

The proposed development fits well into the context of the surrounding area, and its location and design serves to promote use of walking, cycling, and transit modes, thus supporting City of Ottawa policies, goals and objectives with respect to redevelopment, intensification and modal share.

Therefore, based on the foregoing, approval of the proposed 192 Bronson Avenue residential development is recommended from a transportation perspective.

Prepared By:

André J. Sponder, B.A.Sc.

Analyst, Transportation

Ottawa Operations

Reviewed By:

Paul Croft, MCIP, RPP

Senior Transportation Planner

Ottawa Operations



MIRRIBIAN

Appendix A Current Peak Hour Volumes



BRONSON AVE and SLATER ST

(ULRS Listing BRONSON & SLATER)

Survey Date: Friday 27 May 2011

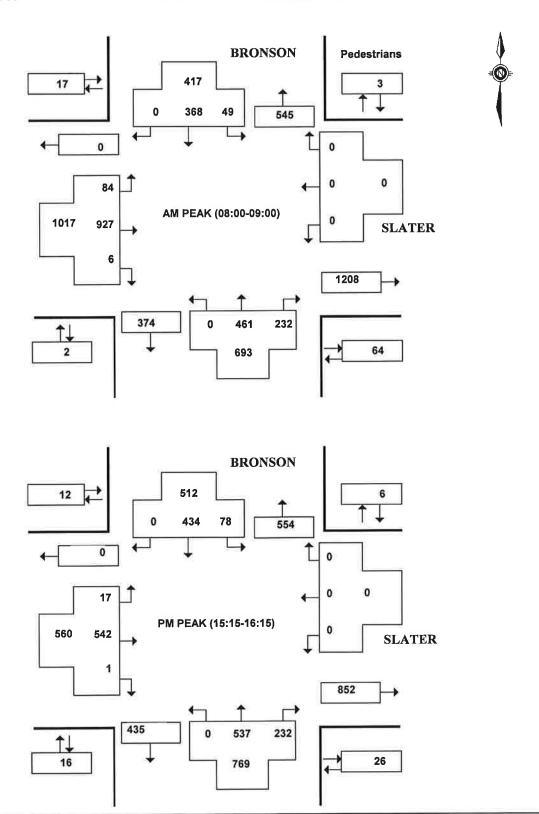
Conditions: wet Start Time: 0700 **Total Observed U-Turns**

Northbound: 0
Eastbound: 0

O Southbound: O Westbound: O

AADT Factor Friday in May is

8

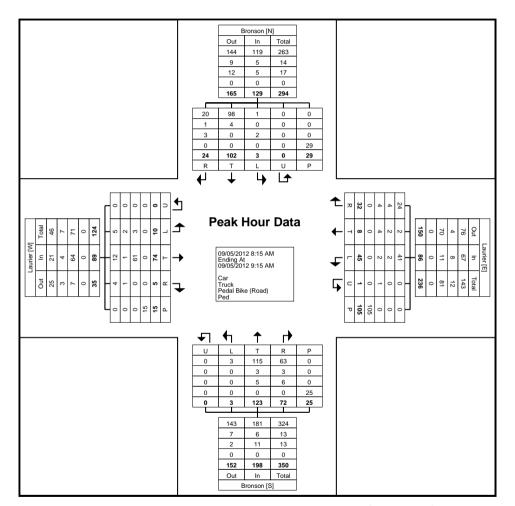


Approved by: JM Printed on: 23/09/2011

City of Ottawa 110 Laurier Ave West

Ottawa, Ontario, Canada K1P 1J1 613-580-2424 ashley.viau@ottawa.ca

Count Name: Bronson & Laurier Site Code: Start Date: 09/05/2012 Page No: 5

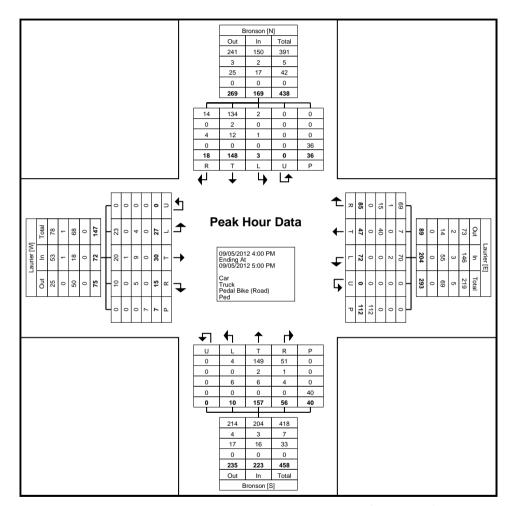


Turning Movement Peak Hour Data Plot (8:15 AM)

City of Ottawa 110 Laurier Ave West

Ottawa, Ontario, Canada K1P 1J1 613-580-2424 ashley.viau@ottawa.ca

Count Name: Bronson & Laurier Site Code: Start Date: 09/05/2012 Page No: 9



Turning Movement Peak Hour Data Plot (4:00 PM)



BRONSON AVE and GLOUCESTER ST

(ULRS Listing BRONSON & GLOUCEST)

Survey Date: Friday 10 June 2011 Conditions:

dry **Start Time:**

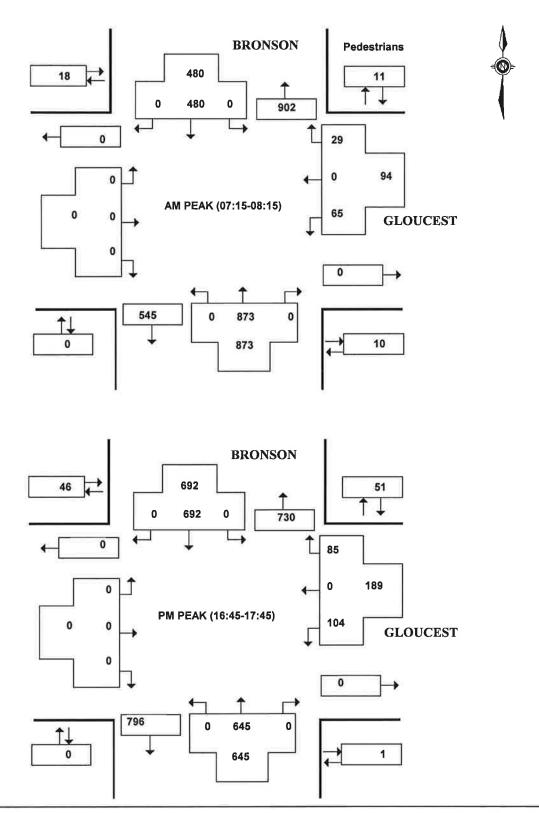
0700

Total Observed U-Turns

Northbound: O Southbound: Eastbound:

0 0 Westbound:

AADT Factor Friday in June is





BRONSON AVE and PRIMROSE AVE

(ULRS Listing BRONSON & PRIMROSE)

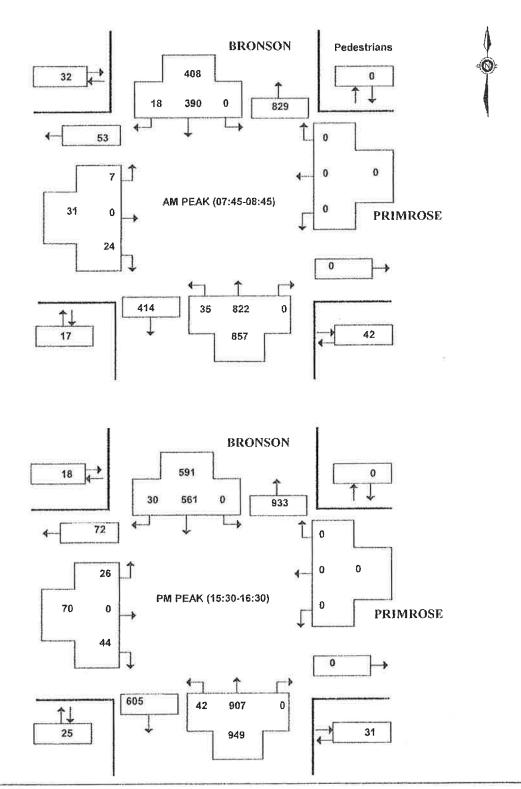
Survey Date: Friday 10 June 2011

Conditions: dry Start Time: 0700 Total Observed U-Turns

Northbound: O Southbound: Eastbound: Westbound:

AADT Factor Friday in June is

8





BRONSON AVE and SOMERSET ST

(ULRS Listing BRONSON & SOMERSET)

Survey Date: Friday 10 June 2011 Conditions:

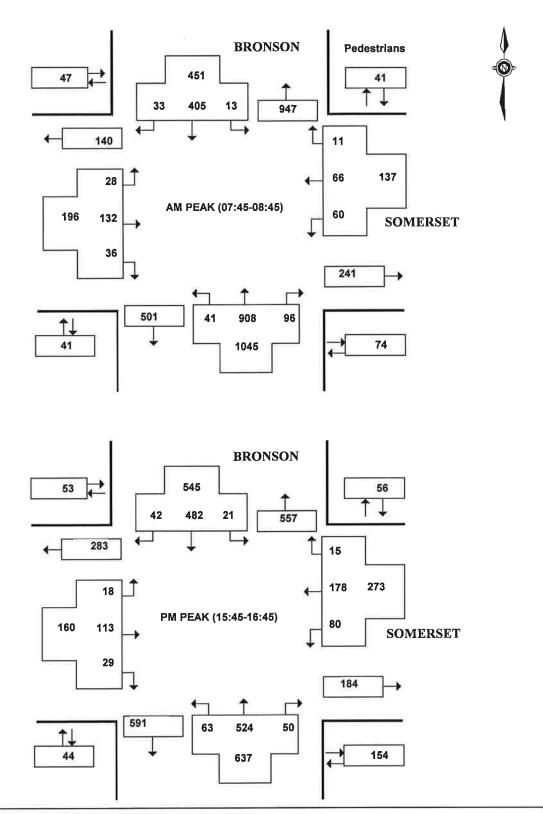
dry

Start Time: 0700 **Total Observed U-Turns**

O Southbound: Northbound: 0 Eastbound:

0 Westbound:

AADT Factor Friday in June is





CAMBRIDGE ST and PRIMROSE AVE

(ULRS Listing CAMBRIDG & PRIMROSE)

Survey Date: Tuesday 19 May 1998 Conditions:

Start Time:

Dry 0700

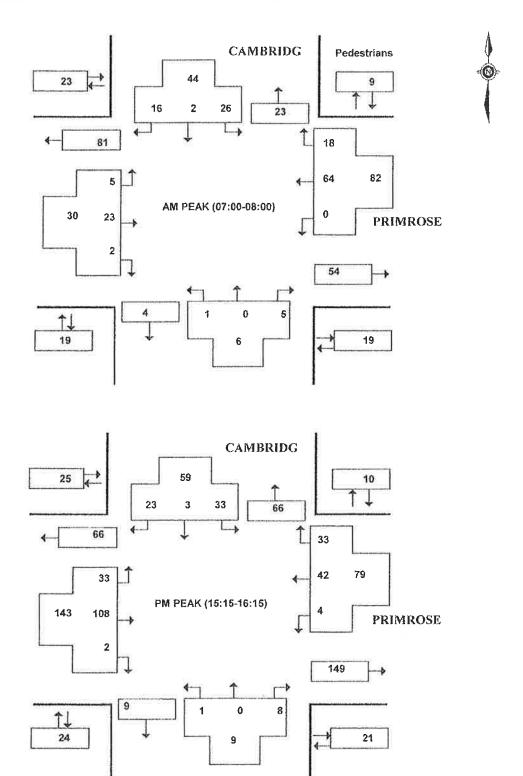
Total Observed U-Turns

Northbound: Eastbound:

0 Southbound:

4 Westbound:

AADT Factor Tuesday in May is



Appendix B SYNCHRO Capacity Analysis: Existing Conditions

Existing AM 1: Bronson & Slater

	→	<u>†</u>	/	
Lane Group	EBT	NBT	SBL	SBT
Lane Configurations	414	† 15	*	44
Volume (vph)	927	461	49	368
Lane Group Flow (vph)	1070	729	52	387
Turn Type	NA	NA	Perm	NA
Protected Phases	4	2		6
Permitted Phases	7		6	J
Detector Phase	4	2	6	6
Switch Phase		2	J	0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.9	26.0	26.0	26.0
Total Split (s)	28.0	32.0	32.0	32.0
Total Split (%)	46.7%	53.3%	53.3%	53.3%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.6	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	6.0	6.0	6.0
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	Max	C-Max	C-Max	C-Max
Act Effct Green (s)	22.1	26.0	26.0	26.0
Actuated g/C Ratio	0.37	0.43	0.43	0.43
v/c Ratio	0.96	0.58	0.23	0.29
Control Delay	39.6	12.4	14.0	11.8
Queue Delay	0.0	0.1	0.0	0.0
Total Delay	39.6	12.5	14.0	11.8
LOS	D	В	В	В
Approach Delay	39.6	12.5		12.1
Approach LOS	D	В		В
Queue Length 50th (m)	58.6	10.8	3.4	13.7
Queue Length 95th (m)	#98.0	28.5	10.2	22.0
Internal Link Dist (m)	157.6	69.3	10.2	35.9
	137.0	09.3	17.0	33.9
Turn Bay Length (m)	1116	1249	227	1322
Base Capacity (vph)				
Starvation Cap Reductn	0	43	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.96	0.60	0.23	0.29
Intersection Summary				
Cycle Length: 60				

Cycle Length: 60

Actuated Cycle Length: 60
Offset: 13 (22%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 60

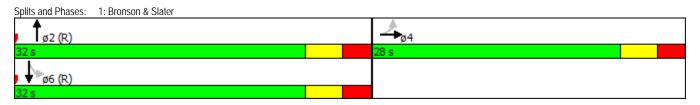
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.96
Intersection Signal Delay: 25.4
Intersection Capacity Utilization 83.5%

Intersection LOS: C ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

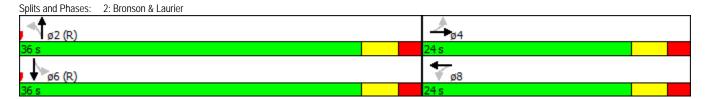
Queue shown is maximum after two cycles.



Delcan Synchro 8 - Report

	•	→	•	←	1	†	/	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		∠î_		43-		aî îs		∳ Љ
Volume (vph)	7	4 13	43	4	3	833	1	432
Lane Group Flow (vph)	0	26	0	78	0	949	0	478
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		4		8		2		6
Permitted Phases	4		8		2		6	
Detector Phase	4	4	8	8	2	2	6	6
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	23.4	23.4	23.4	23.4	30.5	30.5	30.5	30.5
Total Split (s)	24.0	24.0	24.0	24.0	36.0	36.0	36.0	36.0
Total Split (%)	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	60.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.1	2.1	2.1	2.1	2.2	2.2	2.2	2.2
Lost Time Adjust (s)		0.0		0.0		0.0		0.0
Total Lost Time (s)		5.4		5.4		5.5		5.5
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)		11.8		11.8		45.7		45.7
Actuated g/C Ratio		0.20		0.20		0.76		0.76
v/c Ratio		0.10		0.34		0.43		0.22
Control Delay		16.4		24.2		4.0		1.6
Queue Delay		0.0		0.0		0.0		0.0
Total Delay		16.4		24.2		4.0		1.6
LOS		В		С		А		А
Approach Delay		16.4		24.2		4.0		1.6
Approach LOS		В		С		А		А
Queue Length 50th (m)		2.0		7.8		10.9		2.3
Queue Length 95th (m)		6.2		15.4		22.4		m4.7
nternal Link Dist (m)		67.7		479.7		47.5		69.3
Turn Bay Length (m)								
Base Capacity (vph)		425		359		2188		2198
Starvation Cap Reductn		0		0		26		0
Spillback Cap Reductn		0		0		0		0
Storage Cap Reductn		0		0		0		0
Reduced v/c Ratio		0.06		0.22		0.44		0.22
Intersection Summary								
Cycle Length: 60								
Actuated Cycle Length: 60								
Offset: 13 (22%), Referenced to pha	ase 2:NBTL a	ınd 6:SBTL,	Start of Gre	en				
Natural Cycle: 55								
Control Type: Actuated-Coordinated	d							
Maximum v/c Ratio: 0.43								
Intersection Signal Delay: 4.5					ersection L			
Intersection Capacity Utilization 55.	7%			IC	U Level of S	Service B		

Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.



Delcan Synchro 8 - Report

₩ ø6 (R)

	•	4	†	ţ	
Lane Group	WBL	WBR	NBT	SBT	
Lane Configurations	*	#	44	44	
Volume (vph)	65	29	873	480	
Lane Group Flow (vph)	68	31	919	505	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8		2	6	
Permitted Phases		8			
Detector Phase	8	8	2	6	
Switch Phase					
Minimum Initial (s)	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.1	25.1	25.1	22.1	
Total Split (s)	22.0	22.0	38.0	38.0	
Total Split (%)	36.7%	36.7%	63.3%	63.3%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.8	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.1	5.1	5.1	5.1	
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	None	None	C-Max	C-Max	
Act Effct Green (s)	11.4	11.4	46.5	46.5	
Actuated g/C Ratio	0.19	0.19	0.78	0.78	
v/c Ratio	0.24	0.11	0.39	0.21	
Control Delay	21.8	8.5	1.8	1.3	
Queue Delay	0.0	0.0	0.0	0.1	
Total Delay	21.8	8.5	1.8	1.3	
LOS	С	А	А	А	
Approach Delay	17.7		1.8	1.3	
Approach LOS	В		Α	Α	
Queue Length 50th (m)	6.6	0.0	20.7	3.3	
Queue Length 95th (m)	13.6	5.0	3.0	4.6	
Internal Link Dist (m)	364.2		104.0	47.5	
Turn Bay Length (m)					
Base Capacity (vph)	429	396	2364	2364	
Starvation Cap Reductn	0	0	0	613	
Spillback Cap Reductn	0	0	0	0	
Storage Cap Reductn	0	0	0	0	
Reduced v/c Ratio	0.16	0.08	0.39	0.29	
Intersection Summers					
Intersection Summary					
Cycle Length: 60					
Actuated Cycle Length: 60		L CORT OF			
Offset: 16 (27%), Referenced to	phase 2:NBT and	d 6:SB1, Si	art of Greer	า	
Natural Cycle: 55					
Control Type: Actuated-Coordina	ited				
Maximum v/c Ratio: 0.39					
Intersection Signal Delay: 2.7					tersection LOS: A
Intersection Capacity Utilization 4	18.9%			10	CU Level of Service A
Analysis Period (min) 15					
Splits and Phases: 3: Bronson	& Gloucester				
. † (D)					
ø2 (R)					
30 S					

Delcan Synchro 8 - Report

₩ ø6 (R)

Existing AM 4: Bronson & Primrose

	۶	1	1	↓
Lane Group	EBL	NBL	NBT	SBT
Lane Configurations	W		413	ቀ ሴ
Volume (vph)	7	35	822	390
Lane Group Flow (vph)	32	0	902	430
Turn Type	NA	Perm	NA	NA
Protected Phases	4		2	6
Permitted Phases		2		
Detector Phase	4	2	2	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	25.2	23.2	23.2	23.2
Total Split (s)	26.0	34.0	34.0	34.0
Total Split (%)	43.3%	56.7%	56.7%	56.7%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.2		5.2	5.2
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	C-Max	C-Max	C-Max
Act Effct Green (s)	12.0		49.8	49.8
Actuated g/C Ratio	0.20		0.83	0.83
v/c Ratio	0.11		0.38	0.17
Control Delay	9.8		5.3	3.4
Queue Delay	0.0		0.0	0.0
Total Delay	9.8		5.3	3.4
LOS	А		А	А
Approach Delay	9.8		5.3	3.4
Approach LOS	А		А	А
Queue Length 50th (m)	0.7		0.0	0.0
Queue Length 95th (m)	5.2		53.1	16.5
Internal Link Dist (m)	71.5		171.7	104.0
Turn Bay Length (m)				
Base Capacity (vph)	490		2346	2513
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.07		0.38	0.17
Intersection Summary				
Cycle Length: 60				
Actuated Cycle Length: 60				
Offset: 12 (20%), Referenced to p	haca 2-MDTL a	nd 6.CDT	Start of Crox	n.
	iliase z.indil a	iiiu 0.3D1, .	Start of Gree	#II
Natural Cycle: 55 Control Type: Actuated-Coordinat	od			
Maximum v/c Ratio: 0.38	.eu			
Intersection Signal Delay: 4.8				In
Intersection Capacity Utilization 70	0.5%			IC
Analysis Period (min) 15	0.370			IC
Analysis Feriod (IIIII) 15				
Splits and Phases: 4: Bronson	& Primrosa			
Splits and i mases. 4. Biolison	Q I IIIIII USE			
√ Tø2 (R)				
1 92 (R)				
3 4 s				

	•	-	•	•	4	†	-	ļ				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	ø1	ø3	ø5	ø7
Lane Configurations	7	ĵ.	*	ĵ.		4Tb		4Tb				
Volume (vph)	28	132	60	66	41	908	13	405				
Lane Group Flow (vph)	29	177	63	81	0	1100	0	475				
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Protected Phases		4		8		2		6	1	3	5	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	2	2	6	6				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	1.0	1.0	1.0	1.0
Minimum Split (s)	18.9	18.9	18.9	18.9	18.8	18.8	18.8	18.8	7.0	20.0	7.0	20.0
Total Split (s)	32.0	32.0	32.0	32.0	53.0	53.0	53.0	53.0	5.0	5.0	5.0	5.0
Total Split (%)	33.7%	33.7%	33.7%	33.7%	55.8%	55.8%	55.8%	55.8%	5%	5%	5%	5%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	2.0	2.0	2.0	2.0
All-Red Time (s)	2.6	2.6	2.6	2.6	2.5	2.5	2.5	2.5	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0				
Total Lost Time (s)	5.9	5.9	5.9	5.9		5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max	Max	Max	C-Max	C-Max	C-Max	C-Max	Min	Min	Min	Min
Act Effct Green (s)	26.1	26.1	26.1	26.1		47.2		47.2				
Actuated g/C Ratio	0.27	0.27	0.27	0.27		0.50		0.50				
v/c Ratio	0.10	0.43	0.27	0.19		0.82		0.35				
Control Delay	27.0	32.3	30.8	27.9		26.8		15.5				
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0				
Total Delay	27.0	32.3	30.8	27.9		26.8		15.5				
LOS	С	С	С	С		С		В				
Approach Delay		31.6	-	29.2		26.8		15.5				
Approach LOS		С		С		С		В				
Queue Length 50th (m)	4.0	26.7	9.0	11.4		86.3		26.7				
Queue Length 95th (m)	10.9	46.0	20.2	22.9		115.3		37.7				
Internal Link Dist (m)		306.8		381.0		115.2		171.7				
Turn Bay Length (m)		000.0		000								
Base Capacity (vph)	281	411	235	424		1339		1347				
Starvation Cap Reductn	0	0	0	0		0		0				
Spillback Cap Reductn	0	0	0	0		0		0				
Storage Cap Reductn	0	0	0	0		0		0				
Reduced v/c Ratio	0.10	0.43	0.27	0.19		0.82		0.35				
Intersection Summary												

Intersection Summary

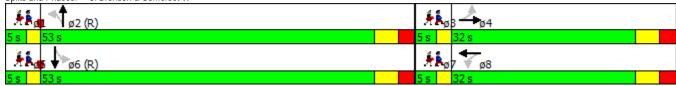
Cycle Length: 95

Actuated Cycle Length: 95
Offset: 22 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 90

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.82
Intersection Signal Delay: 24.7
Intersection Capacity Utilization 89.4%
Analysis Period (min) 15

Intersection LOS: C ICU Level of Service E

Splits and Phases: 6: Bronson & Somerset W



Existing AM 5: Cambridge & Primrose

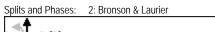
	•	→	*	•	+	•	•	†	~	/	 	-√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			43-			43-			43-	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	5	23	2	0	64	18	1	0	5	26	2	16
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	5	24	2	0	67	19	1	0	5	27	2	17
Direction, Lane #	EB1	WB 1	NB 1	SB 1								
Volume Total (vph)	32	86	6	46								
Volume Left (vph)	5	0	1	27								
Volume Right (vph)	2	19	5	17								
Hadj (s)	0.03	-0.10	-0.43	-0.07								
Departure Headway (s)	4.1	3.9	3.8	4.1								
Degree Utilization, x	0.04	0.09	0.01	0.05								
Capacity (veh/h)	854	895	915	853								
Control Delay (s)	7.3	7.4	6.8	7.3								
Approach Delay (s)	7.3	7.4	6.8	7.3								
Approach LOS	А	Α	Α	Α								
Intersection Summary												
Delay			7.3									
Level of Service			Α									
Intersection Capacity Utilization			26.7%	ICI	U Level of S	ervice			А			
Analysis Period (min)			15									

Existing PM 1: Bronson & Slater

	→	†	-	ļ
Lane Group	EBT	NBT	SBL	SBT
Lane Configurations	413	♠ ₺	*	44
Volume (vph)	542	537	78	434
Lane Group Flow (vph)	590	809	82	457
Turn Type	NA	NA	Perm	NA
Protected Phases	4	2		6
Permitted Phases			6	
Detector Phase	4	2	6	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.9	26.0	26.0	26.0
Total Split (s)	26.0	29.0	29.0	29.0
Total Split (%)	47.3%	52.7%	52.7%	52.7%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.6	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	6.0	6.0	6.0
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	Max	C-Max	C-Max	C-Max
Act Effct Green (s)	20.1	23.0	23.0	23.0
Actuated g/C Ratio	0.37	0.42	0.42	0.42
v/c Ratio	0.53	0.67	0.43	0.36
Control Delay	15.9	13.3	19.9	12.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	15.9	13.3	19.9	12.0
LOS	В	В	В	В
Approach Delay	15.9	13.3	-	13.2
Approach LOS	В	В		В
Queue Length 50th (m)	23.5	16.6	5.5	15.5
Queue Length 95th (m)	36.2	35.9	16.7	24.7
Internal Link Dist (m)	157.6	69.3		35.9
Turn Bay Length (m)	. 37.0	37.0	17.0	30.7
Base Capacity (vph)	1112	1212	192	1275
Starvation Cap Reductn	0	4	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.53	0.67	0.43	0.36
	0.55	0.07	0.43	0.50
Intersection Summary				
Cycle Length: 55				
Actuated Cycle Length: 55				
Offset: 48 (87%), Referenced to phase	se 2:NBT an	d 6:SBTL, S	Start of Gree	en
Natural Cycle: 50				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.67				
Intersection Signal Delay: 14.0				In
Intersection Capacity Utilization 71.1	%			IC
Analysis Period (min) 15				
• , ,				
Splits and Phases: 1: Bronson & S	Slater			
*				



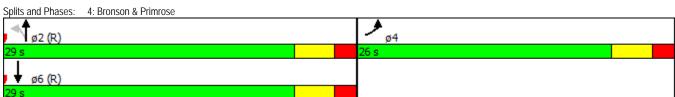
	٠	→	•	—	•	†	\	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		474		412		4Tb		Αt₃
Volume (vph)	23	4 21	72	4	4	674	2	610
Lane Group Flow (vph)	0	57	0	157	0	768	0	655
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		4		8		2		6
Permitted Phases	4		8		2	_	6	
Detector Phase	4	4	8	8	2	2	6	6
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	23.4	23.4	23.4	23.4	30.5	30.5	30.5	30.5
Total Split (s)	24.0	24.0	24.0	24.0	31.0	31.0	31.0	31.0
Total Split (%)	43.6%	43.6%	43.6%	43.6%	56.4%	56.4%	56.4%	56.4%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.1	2.1	2.1	2.1	2.2	2.2	2.2	2.2
Lost Time Adjust (s)		0.0		0.0		0.0		0.0
Total Lost Time (s)		5.4		5.4		5.5		5.5
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)		13.1		13.1		35.2		35.2
Actuated g/C Ratio		0.24		0.24		0.64		0.64
v/c Ratio		0.18		0.57		0.42		0.35
Control Delay		14.2		26.3		5.9		2.5
Queue Delay		0.0		0.0		0.1		0.0
Total Delay		14.2		26.3		5.9		2.5
LOS		В		С		А		Α
Approach Delay		14.2		26.3		5.9		2.5
Approach LOS		В		С		А		Α
Queue Length 50th (m)		3.7		14.0		8.6		4.1
Queue Length 95th (m)		9.5		25.6		26.1		7.6
Internal Link Dist (m)		67.7		479.7		47.5		69.3
Turn Bay Length (m)								
Base Capacity (vph)		444		392		1838		1856
Starvation Cap Reductn		0		0		146		0
Spillback Cap Reductn		0		0		0		0
Storage Cap Reductn		0		0		0		0
Reduced v/c Ratio		0.13		0.40		0.45		0.35
Intersection Summary								
Cycle Length: 55								
Actuated Cycle Length: 55								
Offset: 52 (95%), Referenced to pha	se 2:NBTL a	ınd 6:SBTL,	Start of Gre	een				
Natural Cycle: 55								
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.57								
Intersection Signal Delay: 6.8					tersection L			
Intersection Capacity Utilization 52.2	2%			IC	U Level of S	Service A		
Analysis Period (min) 15								





	•	•	†	Ţ
Lane Group	WBL	WBR	NBT	SBT
Lane Configurations	*	#	44	44
Volume (vph)	104	85	645	692
Lane Group Flow (vph)	109	89	679	728
Turn Type	NA	Perm	NA	NA
Protected Phases	8	. 3	2	6
Permitted Phases		8		J
Detector Phase	8	8	2	6
Switch Phase	U	U	۷	U
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	25.1	25.1	25.1	22.1
	22.0		33.0	33.0
Total Split (s)	40.0%	22.0	60.0%	60.0%
Total Split (%)		40.0%		
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	1.8	1.8	1.8	1.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.1	5.1	5.1	5.1
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	C-Max	C-Max
Act Effct Green (s)	11.5	11.5	37.3	37.3
Actuated g/C Ratio	0.21	0.21	0.68	0.68
v/c Ratio	0.34	0.26	0.33	0.35
Control Delay	20.9	6.6	1.4	2.0
Queue Delay	0.0	0.0	0.0	0.1
Total Delay	20.9	6.6	1.4	2.0
LOS	20.9 C	0.0 A	1.4 A	2.0 A
	14.4	A	1.4	
Approach LOS				2.0
Approach LOS	B	0.0	A	A
Queue Length 50th (m)	9.7	0.0	0.6	5.5
Queue Length 95th (m)	17.7	7.8	1.3	7.7
Internal Link Dist (m)	364.2		104.0	47.5
Turn Bay Length (m)				
Base Capacity (vph)	468	460	2070	2070
Starvation Cap Reductn	0	0	0	237
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.23	0.19	0.33	0.40
Intersection Summary				
Cycle Length: 55				
Actuated Cycle Length: 55				
Offset: 54 (98%), Referenced to ph	nase 2:NBT an	d 6:SBT, St	art of Greer	1
Natural Cycle: 55				
Control Type: Actuated-Coordinate	ed			
Maximum v/c Ratio: 0.35				
Intersection Signal Delay: 3.3				Int
Intersection Capacity Utilization 45.	.8%			IC
Analysis Period (min) 15				
Splits and Phases: 3: Bronson &	Gloucester			
A	. 5.04000101			
T ø2 (R)				
1 Ø2 (K)				
33 s				
1				

•	4	†	↓
FRI	NBI	NBT	SBT
	NDL		↑ 13
	12		T → 561
			623
			NA
	Pellii		NA 6
4	2	2	D
4		2	,
4	2	2	6
10.0	40.0	40.0	40.0
			10.0
			23.2
			29.0
47.3%	52.7%	52.7%	52.7%
3.3	3.3	3.3	3.3
1.9	1.9	1.9	1.9
0.0			0.0
			5.2
0.2		0.2	0.2
None	C-Max	C-Max	C-Max
	O-IVIAX		40.8
			0.74
			0.74
			7.0
			0.0
			7.0
			A
			7.0
			А
			13.4
8.2		#68.3	29.4
71.5		171.7	104.0
561		2052	2240
0		0	0
			0
			0
0.13		0.49	0.28
nhaca 2-MDTL a	nd 6.CDT	Start of Cross	an .
ліаse z:NBTL al	110 0:2B1, 3	olati ul G186	:11
h. d			
ted			
			Int
77.0%			
77.0% eds capacity, que			Ini IC
	26 73 NA 4 10.0 25.2 26.0 47.3% 3.3 1.9 0.0 5.2 None 12.0 0.22 0.21 9.6 0.0 9.6 A 2.3 8.2 71.5 561 0 0 0.13	EBL NBL 26 42 73 0 NA Perm 4 2 4 2 10.0 10.0 25.2 23.2 26.0 29.0 47.3% 52.7% 3.3 3.3 1.9 1.9 0.0 5.2 None C-Max 12.0 0.22 0.21 9.6 0.0 9.6 A 9.6 A 2.3 8.2 71.5 561 0 0 0 0.13	EBL NBL NBT 26 42 907 73 0 999 NA Perm NA 4 2 2 4 2 2 10.0 10.0 10.0 25.2 23.2 23.2 26.0 29.0 29.0 47.3% 52.7% 52.7% 3.3 3.3 3.3 1.9 1.9 1.9 1.9 0.0 0.0 5.2 5.2 None C-Max C-Max 12.0 40.8 0.22 0.74 0.21 0.49 9.6 8.3 0.0 0.0 9.6 8.3 A A 9.6 8.3 A O 9.7 9.0 9.8 9



	•	→	1	•	4	†	-	ļ				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	ø1	ø3	ø5	ø7
Lane Configurations	7	ĵ.	7	Î.		413		4îb				
Volume (vph)	18	113	80	178	63	524	21	482				
Lane Group Flow (vph)	19	150	84	203	0	671	0	573				
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Protected Phases		4		8		2		6	1	3	5	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	2	2	6	6				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	1.0	1.0	1.0	1.0
Minimum Split (s)	18.9	18.9	18.9	18.9	18.8	18.8	18.8	18.8	7.0	7.0	7.0	7.0
Total Split (s)	32.0	32.0	32.0	32.0	53.0	53.0	53.0	53.0	5.0	5.0	5.0	5.0
Total Split (%)	33.7%	33.7%	33.7%	33.7%	55.8%	55.8%	55.8%	55.8%	5%	5%	5%	5%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	2.0	2.0	2.0	2.0
All-Red Time (s)	2.6	2.6	2.6	2.6	2.5	2.5	2.5	2.5	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	2.0	0.0	2.10	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	5.9	5.9	5.9		5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max	Max	Max	C-Max	C-Max	C-Max	C-Max	Min	Min	Min	Min
Act Effct Green (s)	26.1	26.1	26.1	26.1	O Wax	47.2	O Wax	47.2	IVIIII	IVIIII	141111	14111
Actuated g/C Ratio	0.27	0.27	0.27	0.27		0.50		0.50				
v/c Ratio	0.27	0.27	0.27	0.27		0.55		0.30				
Control Delay	26.9	31.2	34.5	33.0		18.8		16.5				
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0				
Total Delay	26.9	31.2	34.5	33.0		18.8		16.5				
LOS	20.9 C	C C	34.5 C	33.0 C		В		10.5 B				
Approach Delay	C	30.8	C	33.4		18.8		16.5				
		30.6 C		33.4 C		10.0 B		10.3 B				
Approach LOS	2.6	22.2	12.5	31.0		43.0		33.7				
Queue Length 50th (m)												
Queue Length 95th (m)	8.1	39.5	26.4	51.7		59.5		46.7				
Internal Link Dist (m)		306.8		381.0		115.2		171.7				
Turn Bay Length (m)	220	200	21/	401		1010		104/				
Base Capacity (vph)	228	399	216	431		1212		1346				
Starvation Cap Reductn	0	0	0	0		0		0				
Spillback Cap Reductn	0	0	0	0		0		0				
Storage Cap Reductn	0	0	0	0		0		0				
Reduced v/c Ratio	0.08	0.38	0.39	0.47		0.55		0.43				
Intersection Summary												
Cycle Length: 95												
Actuated Cycle Length: 95	alassa O NIDTI		C1 1 C									
Offset: 24 (25%), Referenced to	pnase 2:NBTL a	nd 6:SBTL,	Start of Gre	een								
Natural Cycle: 60												
Control Type: Actuated-Coordina	ated											
Maximum v/c Ratio: 0.55												

Maximum v/c Ratio: 0.55
Intersection Signal Delay: 21.7
Intersection Capacity Utilization 79.7%
Analysis Period (min) 15 Intersection LOS: C ICU Level of Service D



Existing PM 5: Cambridge & Primrose

	۶	→	*	1	+	•	•	†	~	/	 	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43-			₩.			43-	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	33	108	2	4	42	33	1	0	8	33	3	23
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	35	114	2	4	44	35	1	0	8	35	3	24
Direction, Lane #	EB1	WB 1	NB 1	SB 1								
Volume Total (vph)	151	83	9	62								
Volume Left (vph)	35	4	1	35								
Volume Right (vph)	2	35	8	24								
Hadj (s)	0.07	-0.21	-0.48	-0.09								
Departure Headway (s)	4.2	4.0	4.0	4.3								
Degree Utilization, x	0.18	0.09	0.01	0.07								
Capacity (veh/h)	835	874	837	777								
Control Delay (s)	8.1	7.4	7.0	7.7								
Approach Delay (s)	8.1	7.4	7.0	7.7								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			7.8									
Level of Service			Α									
Intersection Capacity Utilization			33.8%	IC	U Level of S	ervice			А			
Analysis Period (min)			15									

Appendix C SYNCHRO Capacity Analysis: Projected Conditions

Projected AM 1: Bronson & Slater

	→	†	/	+
Lane Group	EBT	NBT	SBL	SBT
Lane Configurations	414	† 13	ሻ	44
Volume (vph)	927	475	49	374
Lane Group Flow (vph)	1070	749	52	394
Turn Type	NA NA	NA	Perm	NA
Protected Phases	4	2		6
Permitted Phases	· ·	_	6	
Detector Phase	4	2	6	6
Switch Phase				_
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.9	26.0	26.0	26.0
Total Split (s)	28.0	32.0	32.0	32.0
Total Split (%)	46.7%	53.3%	53.3%	53.3%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.6	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	6.0	6.0	6.0
Lead/Lag	5.7	0.0	0.0	0.0
Lead-Lag Optimize?				
Recall Mode	Max	C-Max	C-Max	C-Max
Act Effct Green (s)	22.1	26.0	26.0	26.0
	0.37	0.43	0.43	0.43
Actuated g/C Ratio v/c Ratio	0.37	0.43	0.43	0.43
Control Delay	39.6	12.7	14.3	11.8
Queue Delay	0.0	0.1	0.0	0.0
Total Delay	39.6	12.8	14.3	11.8
LOS	D	В	В	В
Approach Delay	39.6	12.8		12.1
Approach LOS	D	В		В
Queue Length 50th (m)	58.6	12.1	3.4	14.1
Queue Length 95th (m)	#98.0	29.5	10.3	22.4
Internal Link Dist (m)	157.6	69.3		35.9
Turn Bay Length (m)			17.0	
Base Capacity (vph)	1116	1250	220	1322
Starvation Cap Reductn	0	43	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.96	0.62	0.24	0.30
Intersection Summary				

Intersection Summary

Cycle Length: 60

Actuated Cycle Length: 60
Offset: 13 (22%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 60

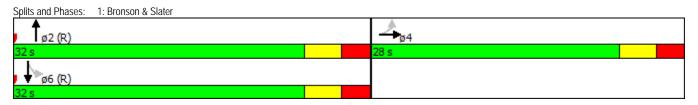
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.96
Intersection Signal Delay: 25.3
Intersection Capacity Utilization 84.1%

Intersection LOS: C ICU Level of Service E

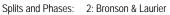
Analysis Period (min) 15

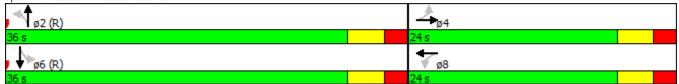
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



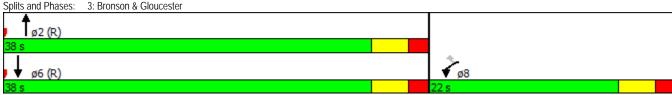
	۶	→	•	←	4	†	-	ļ
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		4 16		412		ፈቤ		∳ ሴ
Volume (vph)	26	16	43	4 5	3	833	1	431
Lane Group Flow (vph)	0	50	0	79	0	949	0	484
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		4		8		2		6
Permitted Phases	4		8		2		6	
Detector Phase	4	4	8	8	2	2	6	6
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	23.4	23.4	23.4	23.4	30.5	30.5	30.5	30.5
Total Split (s)	24.0	24.0	24.0	24.0	36.0	36.0	36.0	36.0
Total Split (%)	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	60.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.1	2.1	2.1	2.1	2.2	2.2	2.2	2.2
Lost Time Adjust (s)		0.0		0.0		0.0		0.0
Total Lost Time (s)		5.4		5.4		5.5		5.5
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)		11.8		11.8		45.7		45.7
Actuated g/C Ratio		0.20		0.20		0.76		0.76
v/c Ratio		0.21		0.35		0.43		0.22
Control Delay		19.2		24.4		4.1		1.5
Queue Delay		0.0		0.0		0.0		0.0
Total Delay		19.2		24.4		4.1		1.5
LOS		В		С		Α		Α
Approach Delay		19.3		24.4		4.1		1.5
Approach LOS		В		С		Α		Α
Queue Length 50th (m)		4.3		7.9		11.0		2.3
Queue Length 95th (m)		10.1		15.5		23.7		m4.5
Internal Link Dist (m)		67.7		479.7		47.5		69.3
Turn Bay Length (m)								
Base Capacity (vph)		376		354		2187		2192
Starvation Cap Reductn		0		0		26		0
Spillback Cap Reductn		0		0		0		0
Storage Cap Reductn		0		0		0		0
Reduced v/c Ratio		0.13		0.22		0.44		0.22
Intersection Summary								
Cycle Length: 60								
Actuated Cycle Length: 60								
Offset: 13 (22%), Referenced to phase	2·NRTI a	nd 6:SRTI	Start of Gre	en				
Natural Cycle: 55	,DIL (I	0.0D1L,	Ciair of Oft					
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.43								
ntersection Signal Delay: 4.8				Int	tersection L0	DS: A		
Intersection Signal Belay: 4.0 Intersection Capacity Utilization 55.3%					U Level of S			
Analysis Period (min) 15	•			10	O LOVOI OI C	OI VICE D		
m Volume for 95th percentile queue	is matered	hy unstream	n signal					





Projected AM 3: Bronson & Gloucester

	•	•	†	↓
Lane Group	WBL	WBR	NBT	SBT
Lane Configurations	*	#	44	44
Volume (vph)	65	29	873	480
Lane Group Flow (vph)	68	31	919	505
Turn Type	NA	Perm	NA	NA
Protected Phases	8		2	6
Permitted Phases		8		
Detector Phase	8	8	2	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	25.1	25.1	25.1	22.1
Total Split (s)	22.0	22.0	38.0	38.0
Total Split (%)	36.7%	36.7%	63.3%	63.3%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	1.8	1.8	1.8	1.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.1	5.1	5.1	5.1
Lead/Lag	J. I	J. I	J. I	J. I
Lead-Lag Optimize?				
Recall Mode	None	None	C-Max	C-Max
Act Effct Green (s)	11.4	11.4	46.5	46.5
Actuated g/C Ratio	0.19	0.19	0.78	0.78
v/c Ratio	0.19	0.19	0.78	0.78
Control Delay	21.8	8.5	0.39	1.4
Queue Delay	0.0	0.0	0.9	0.1
Total Delay	21.8	8.5	0.0	1.4
LOS	21.0 C	6.5 A	0.9 A	1.4 A
Approach Delay	17.7	А	0.9	1.4
Approach LOS	17.7		0.9 A	1.4 A
Queue Length 50th (m)	6.6	0.0	1.9	3.6
Queue Length (Eth (m)		5.0	3.0	5.0 5.1
Queue Length 95th (m)	13.6	5.0		
Internal Link Dist (m)	364.2		104.0	47.5
Turn Bay Length (m)	400	207	22/4	2274
Base Capacity (vph)	429	396	2364	2364
Starvation Cap Reductn	0	0	0	616
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.16	0.08	0.39	0.29
Intersection Summary				
Cycle Length: 60				
Actuated Cycle Length: 60				
Offset: 16 (27%), Referenced to pl	naca 2-NDT an	4 6.CDT C	tart of Croos	1
	iase z:inb i an	u 0:381, S	iai i Ui Greer	I
Natural Cycle: 55	od.			
Control Type: Actuated-Coordinate	eu			
Maximum v/c Ratio: 0.39				
Intersection Signal Delay: 2.2	2004			Inf
Intersection Capacity Utilization 48	3.9%			IC
Analysis Period (min) 15				
Splits and Phases: 3: Bronson &	& Gloucester			
•				
T _{ø2 (R)}				
20 -				



Projected AM 4: Bronson & Primrose

Lane Group				
	EBL	NBL	NBT	SBT
Lane Configurations	W		414	∳ ሴ
Volume (vph)	8	46	821	390
Lane Group Flow (vph)	67	0	912	430
Turn Type	NA	Perm	NA	NA
Protected Phases	4	1 01111	2	6
Permitted Phases	7	2		U
Detector Phase	4	2	2	6
Switch Phase	7	2	2	U
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	25.2	23.2	23.2	23.2
Total Split (s)	26.0	34.0	34.0	34.0
Total Split (%)	43.3%	56.7%	56.7%	56.7%
	43.3%		3.3	
Yellow Time (s)		3.3 1.9	3.3 1.9	3.3 1.9
All-Red Time (s)	1.9	1.9		
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.2		5.2	5.2
Lead/Lag				
Lead-Lag Optimize?	Nama	C Mass	C Mass	C Man
Recall Mode	None	C-Max	C-Max	C-Max
Act Effet Green (s)	12.0		45.8	45.8
Actuated g/C Ratio	0.20		0.76	0.76
v/c Ratio	0.21		0.43	0.19
Control Delay	8.2		6.5	4.0
Queue Delay	0.0		0.0	0.0
Total Delay	8.2		6.5	4.0
LOS	Α		A	Α
Approach Delay	8.2		6.5	4.0
Approach LOS	А		А	Α
Queue Length 50th (m)	0.8		21.7	4.6
Queue Length 95th (m)	7.3		54.5	16.6
Internal Link Dist (m)	71.5		171.7	104.0
Turn Bay Length (m)				
Base Capacity (vph)	505		2122	2308
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.13		0.43	0.19
Intersection Summany				
Intersection Summary				
Cycle Length: 60				
Actuated Cycle Length: 60	O NIDTL -		2116-0	
Offset: 12 (20%), Referenced to pha	ase 2:NBTL a	nd 6:SB1, S	Start of Gree	en
Natural Cycle: 55				
Control Type: Actuated-Coordinated	j			
Maximum v/c Ratio: 0.43				
Intersection Signal Delay: 5.8				Ir
Intersection Capacity Utilization 70.8	8%			[(
Analysis Period (min) 15				
Splits and Phases: 4: Bronson &	Primrose			
Splits and Phases: 4: Bronson &	Primrose			



Lane Configurations		•	-	•	←	4	†	-	ļ				
Volume (vph) 30 132 60 66 41 916 13 129	Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	ø1	ø3	ø5	ø7
Volume (vph) 30 132 60 66 41 916 13 129	Lane Configurations	*	Ť.	*	î.		412		417.				
Lane Group Flow (ynh)			132		66	41	916	13	429				
Protected Phases		32	177	63	81	0	1108	0	509				
Permitted Phases	Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Detector Phase 4	Protected Phases		4		8		2		6	1	3	5	7
Switch Phase Swit	Permitted Phases	4		8		2		6					
Minimum Initial (s)	Detector Phase	4	4	8	8	2	2	6	6				
Minimum Split (s) 18.9 18.9 18.9 18.9 18.8 18.8 18.8 18.8 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0 7.0 20.0	Switch Phase												
Total Split (s) 32.0 32.0 32.0 32.0 32.0 53.0 53.0 53.0 53.0 50 50 50 50 50 50 50 50 50 50 50 50 50	Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	1.0	1.0	1.0	1.0
Total Split (%) 33.7% 33.7% 33.7% 33.7% 33.7% 55.8% 55.8% 55.8% 55.8% 55%	Minimum Split (s)	18.9	18.9	18.9	18.9	18.8	18.8	18.8	18.8	7.0	20.0	7.0	20.0
Total Split (%) 33.7% 33.7% 33.7% 33.7% 55.8% 55.8% 55.8% 55.8% 55% 55% 55% 55% 55% 55% 56% 56% 56% 55%	Total Split (s)	32.0	32.0	32.0	32.0	53.0	53.0	53.0	53.0	5.0	5.0	5.0	5.0
Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0		33.7%	33.7%	33.7%	33.7%	55.8%	55.8%	55.8%	55.8%	5%	5%	5%	5%
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.9 5.8 5.8 5.8 Lead		3.3		3.3	3.3	3.3	3.3	3.3	3.3	2.0	2.0	2.0	2.0
Total Lost Time (s) 5.9 5.9 5.9 5.9 5.9 5.8 5.8 Lead/Lag Lag Lag <td>All-Red Time (s)</td> <td>2.6</td> <td>2.6</td> <td>2.6</td> <td>2.6</td> <td>2.5</td> <td>2.5</td> <td>2.5</td> <td>2.5</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td>	All-Red Time (s)	2.6	2.6	2.6	2.6	2.5	2.5	2.5	2.5	0.0	0.0	0.0	0.0
Total Lost Time (s) 5.9 5.9 5.9 5.9 5.9 5.8 Lag	Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0				
Lead-Lag Optimize? Yes		5.9	5.9	5.9	5.9		5.8		5.8				
Recall Mode Max Max Max Max C-Max C-Max C-Max Min	Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead
Act Effct Green (s) 26.1 26.1 26.1 26.1 47.2 47.2 Actuated g/C Ratio 0.27 0.27 0.27 0.50 0.50 v/c Ratio 0.11 0.43 0.27 0.19 0.83 0.38 Control Delay 27.2 32.3 30.8 27.9 27.2 15.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 32.3 30.8 27.9 27.2 15.8 LOS C C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) 8ase Capacity (vph) 281 411 235 424	Lead-Lag Optimize?			Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Actuated g/C Ratio 0.27 0.27 0.27 0.50 0.50 v/c Ratio 0.11 0.43 0.27 0.19 0.83 0.38 Control Delay 27.2 32.3 30.8 27.9 27.2 15.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 32.3 30.8 27.9 27.2 15.8 LOS C C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 0	Recall Mode	Max	Max	Max	Max	C-Max	C-Max	C-Max	C-Max	Min	Min	Min	Min
v/c Ratio 0.11 0.43 0.27 0.19 0.83 0.38 Control Delay 27.2 32.3 30.8 27.9 27.2 15.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 32.3 30.8 27.9 27.2 15.8 LOS C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn <td>Act Effct Green (s)</td> <td>26.1</td> <td>26.1</td> <td>26.1</td> <td>26.1</td> <td></td> <td>47.2</td> <td></td> <td>47.2</td> <td></td> <td></td> <td></td> <td></td>	Act Effct Green (s)	26.1	26.1	26.1	26.1		47.2		47.2				
Control Delay 27.2 32.3 30.8 27.9 27.2 15.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 32.3 30.8 27.9 27.2 15.8 LOS C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Actuated g/C Ratio	0.27	0.27	0.27	0.27		0.50		0.50				
Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 32.3 30.8 27.9 27.2 15.8 LOS C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0		0.11	0.43	0.27	0.19		0.83		0.38				
Total Delay 27.2 32.3 30.8 27.9 27.2 15.8 LOS C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Control Delay	27.2	32.3	30.8	27.9		27.2		15.8				
LOS C C C C C C B Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0				
Approach Delay 31.5 29.2 27.2 15.8 Approach LOS C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Total Delay	27.2	32.3	30.8	27.9		27.2		15.8				
Approach LOS C C C B Queue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0	LOS	С	С	С	С		С		В				
Oueue Length 50th (m) 4.4 26.7 9.0 11.4 87.4 29.1 Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Approach Delay		31.5		29.2		27.2		15.8				
Queue Length 95th (m) 11.5 46.0 20.2 22.9 117.0 40.7 Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Approach LOS		С		С		С		В				
Internal Link Dist (m) 306.8 381.0 115.2 171.7 Turn Bay Length (m) 306.8 381.0 115.2 171.7 Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Queue Length 50th (m)	4.4	26.7	9.0	11.4		87.4		29.1				
Turn Bay Length (m) Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Queue Length 95th (m)	11.5	46.0	20.2	22.9		117.0		40.7				
Base Capacity (vph) 281 411 235 424 1337 1345 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Internal Link Dist (m)		306.8		381.0		115.2		171.7				
Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0													_
Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0	Base Capacity (vph)	281	411	235	424		1337		1345				
Storage Cap Reductn 0 0 0 0 0 0		0	0	0	0		0		0				
Storage Cap Reductn 0 0 0 0 0 0	Spillback Cap Reductn	0	0	0	0		0		0				
		0	0	0	0		0		0				
		0.11	0.43	0.27	0.19		0.83		0.38				

Intersection Summary

Cycle Length: 95

Actuated Cycle Length: 95
Offset: 22 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 90

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.83
Intersection Signal Delay: 24.9
Intersection Capacity Utilization 90.8%
Analysis Period (min) 15

Intersection LOS: C ICU Level of Service E





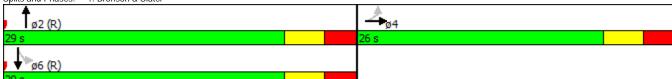
Projected AM 5: Cambridge & Primrose

	•	→	•	•	+	•	•	†	<u> </u>	/	+	-√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43-			43-			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	5	23	2	0	64	29	1	0	5	59	2	16
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	5	24	2	0	67	31	1	0	5	62	2	17
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	32	98	6	81								
Volume Left (vph)	5	0	1	62								
Volume Right (vph)	2	31	5	17								
Hadj (s)	0.03	-0.15	-0.43	0.06								
Departure Headway (s)	4.2	4.0	3.8	4.2								
Degree Utilization, x	0.04	0.11	0.01	0.10								
Capacity (veh/h)	828	883	894	821								
Control Delay (s)	7.4	7.5	6.9	7.7								
Approach Delay (s)	7.4	7.5	6.9	7.7								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			7.5									
Level of Service			Α									
Intersection Capacity Utilization			29.1%	ICI	U Level of S	ervice			Α			
Analysis Period (min)			15									

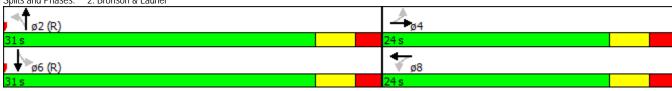
	•	•	+	<u> </u>		Ι
	•	_	ı		•	*
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĥ			વ
Volume (veh/h)	33	23	23	11	8	44
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	35	24	24	12	8	46
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						203
pX, platoon unblocked						
vC, conflicting volume	93	30			36	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	93	30			36	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	96	98			99	
cM capacity (veh/h)	902	1044			1575	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	59	36	55			
Volume Left	35	0	33 8			
Volume Right	24	12	0			
cSH	955	1700	1575			
Volume to Capacity	0.06	0.02	0.01			
Queue Length 95th (m)	1.5	0.02	0.01			
Control Delay (s)	9.0	0.0	1.2			
Lane LOS	9.0 A	0.0	1.2 A			
Approach Delay (s)	9.0	0.0	1.2			
Approach LOS	9.0 A	0.0	1.2			
• •	А					
Intersection Summary						
Average Delay			4.0			
Intersection Capacity Utilization			19.7%	IC	U Level of Servi	ice
Analysis Period (min)			15			

Projected PM 1: Bronson & Slater

	→	†	/	
Lane Group	EBT	NBT	SBL	SBT
Lane Configurations	413	♦ %	*	44
Volume (vph)	542	545	78	450
Lane Group Flow (vph)	590	822	82	474
Turn Type	NA	NA	Perm	NA
Protected Phases	4	2		6
Permitted Phases			6	
Detector Phase	4	2	6	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.9	26.0	26.0	26.0
Total Split (s)	26.0	29.0	29.0	29.0
Total Split (%)	47.3%	52.7%	52.7%	52.7%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.6	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	6.0	6.0	6.0
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	Max	C-Max	C-Max	C-Max
Act Effct Green (s)	20.1	23.0	23.0	23.0
Actuated g/C Ratio	0.37	0.42	0.42	0.42
v/c Ratio	0.53	0.68	0.44	0.37
Control Delay	15.9	13.9	20.4	12.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	15.9	14.0	20.4	12.1
LOS	В	В	С	В
Approach Delay	15.9	14.0	_	13.3
Approach LOS	В	В		В
Queue Length 50th (m)	23.5	18.7	5.5	16.2
Queue Length 95th (m)	36.2	36.8	16.9	25.7
Internal Link Dist (m)	157.6	69.3	.0.7	35.9
Turn Bay Length (m)	107.0	57.5	17.0	00.7
Base Capacity (vph)	1112	1212	188	1275
Starvation Cap Reductn	0	4	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.53	0.68	0.44	0.37
Intersection Summary	0.55	3.00	5.77	3.37
Cycle Length: 55				
Actuated Cycle Length: 55	haca 2.NDT an	44.CDTL 0	Start of Cross	n .
Offset: 48 (87%), Referenced to p	onase z:NBT an	0 0:5BTL, 3	Start of Gree	en
Natural Cycle: 50	t1			
Control Type: Actuated-Coordinal	tea			
Maximum v/c Ratio: 0.68				
Intersection Signal Delay: 14.4	11 50/			In
Intersection Capacity Utilization 7	1.5%			IC
Analysis Period (min) 15				
Splits and Phases: 1: Bronson	8. Slator			
Spins and Friases. 1. DIUIISUII	a Siaicí			



	ᄼ	→	•	←	4	†	-	↓
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		43-				ፈቴ		∳ Љ
Volume (vph)	35	23	72	4	4	674	2	608
Lane Group Flow (vph)	0	74	0	159	0	768	0	671
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		4		8		2		6
Permitted Phases	4	•	8		2		6	
Detector Phase	4	4	8	8	2	2	6	6
Switch Phase			-					-
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	23.4	23.4	23.4	23.4	30.5	30.5	30.5	30.5
Total Split (s)	24.0	24.0	24.0	24.0	31.0	31.0	31.0	31.0
Total Split (%)	43.6%	43.6%	43.6%	43.6%	56.4%	56.4%	56.4%	56.4%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.1	2.1	2.1	2.1	2.2	2.2	2.2	2.2
Lost Time Adjust (s)		0.0		0.0		0.0		0.0
Total Lost Time (s)		5.4		5.4		5.5		5.5
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)		13.2		13.2		35.1		35.1
Actuated g/C Ratio		0.24		0.24		0.64		0.64
v/c Ratio		0.24		0.57		0.42		0.36
Control Delay		15.2		26.2		5.9		2.4
Queue Delay		0.0		0.0		0.1		0.0
Total Delay		15.2		26.2		6.0		2.4
LOS		В		С		А		А
Approach Delay		15.2		26.2		6.0		2.4
Approach LOS		В		С		А		А
Queue Length 50th (m)		5.0		14.2		8.7		4.0
Queue Length 95th (m)		11.6		25.6		26.1		7.4
Internal Link Dist (m)		66.2		479.7		47.5		69.3
Turn Bay Length (m)								
Base Capacity (vph)		430		390		1831		1844
Starvation Cap Reductn		0		0		144		0
Spillback Cap Reductn		0		0		0		0
Storage Cap Reductn		0		0		0		0
Reduced v/c Ratio		0.17		0.41		0.46		0.36
Intersection Summary								
Cycle Length: 55								
Actuated Cycle Length: 55	o D.MDTL o	nd (CDTI	Ctart of Cra	on				
Offset: 52 (95%), Referenced to phas	e Z:NBTL a	110 0:281L,	Start of Gre	een				
Natural Cycle: 55 Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.57								
ntersection Signal Delay: 6.9				Int	ersection L	OC. A		
	/				U Level of S			
Intersection Capacity Utilization 51.49	Ó			IC	U Level of S	service A		
Analysis Period (min) 15								
Splits and Phases: 2: Bronson & La	urier							
-4								
ø2 (R)							14	



	•	•	†	↓
Lane Group	WBL	WBR	NBT	SBT
Lane Configurations	*	#	44	44
Volume (vph)	104	85	645	692
Lane Group Flow (vph)	109	89	679	728
Turn Type	NA	Perm	NA	NA
Protected Phases	8	. 31111	2	6
Permitted Phases		8		J
Detector Phase	8	8	2	6
Switch Phase	U	U		U
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	25.1	25.1	25.1	22.1
	22.0		33.0	33.0
Total Split (s)	40.0%	22.0	60.0%	60.0%
Total Split (%)		40.0%		
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	1.8	1.8	1.8	1.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.1	5.1	5.1	5.1
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	C-Max	C-Max
Act Effct Green (s)	11.5	11.5	37.3	37.3
Actuated g/C Ratio	0.21	0.21	0.68	0.68
v/c Ratio	0.34	0.26	0.33	0.35
Control Delay	20.9	6.6	0.6	2.1
Queue Delay	0.0	0.0	0.0	0.1
Total Delay	20.9	6.6	0.6	2.1
LOS	C C	Α	Α	Α
Approach Delay	14.4	А	0.6	2.1
Approach LOS	В		Α	Α
Queue Length 50th (m)	9.7	0.0	0.6	5.7
		7.8		8.0
Queue Length 95th (m)	17.7	7.8	1.3	
Internal Link Dist (m)	364.2		104.0	47.5
Turn Bay Length (m)				
Base Capacity (vph)	468	460	2070	2070
Starvation Cap Reductn	0	0	0	235
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.23	0.19	0.33	0.40
Intersection Summary				
-				
Cycle Length: 55				
Actuated Cycle Length: 55				
Offset: 54 (98%), Referenced to ph	nase 2:NBT and	d 6:SBT, St	tart of Greer	1
Natural Cycle: 55				
Control Type: Actuated-Coordinate	ed			
Maximum v/c Ratio: 0.35				
Intersection Signal Delay: 3.0				Int
Intersection Capacity Utilization 45	.8%			IC
Analysis Period (min) 15				
-				
Splits and Phases: 3: Bronson &	Gloucester			
A				
ø2 (R)				
33 s				
1				

Projected PM 4: Bronson & Primrose

4. Diolison & Filmose	•	1	†	ı		_
				*		
Lane Group	EBL	NBL	NBT	SBT		
Lane Configurations	À		413	∳ ሴ		
Volume (vph)	28	71	905	561		
Lane Group Flow (vph)	97	0	1028	623		
Turn Type	NA	Perm	NA	NA		
Protected Phases	4		2	6		
Permitted Phases		2				
Detector Phase	4	2	2	6		
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0		
Minimum Split (s)	25.2	23.2	23.2	23.2		
Total Split (s)	26.0	29.0	29.0	29.0		
Total Split (%)	47.3%	52.7%	52.7%	52.7%		
Yellow Time (s)	3.3	3.3	3.3	3.3		
All-Red Time (s)	1.9	1.9	1.9	1.9		
Lost Time Adjust (s)	0.0		0.0	0.0		
Total Lost Time (s)	5.2		5.2	5.2		
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	C-Max	C-Max	C-Max		
Act Effct Green (s)	12.0		36.7	36.7		
Actuated g/C Ratio	0.22		0.67	0.67		
v/c Ratio	0.27		0.59	0.31		
Control Delay	9.1		10.8	8.1		
Queue Delay	0.0		0.0	0.0		
Total Delay	9.1		10.8	8.1		
LOS	А		В	Α		
Approach Delay	9.1		10.8	8.1		
Approach LOS	А		В	А		
Queue Length 50th (m)	2.5		27.0	13.8		
Queue Length 95th (m)	9.3		#81.8	29.7		
Internal Link Dist (m)	71.5		171.7	104.0		
Turn Bay Length (m)						
Base Capacity (vph)	569		1754	2017		
Starvation Cap Reductn	0		0	0		
Spillback Cap Reductn	0		0	0		
Storage Cap Reductn	0		0	0		
Reduced v/c Ratio	0.17		0.59	0.31		
Intersection Summary						
Cycle Length: 55						
Actuated Cycle Length: 55						
Offset: 49 (89%), Referenced to pha	ase 2:NBTL a	nd 6:SBT. S	Start of Gree	en		
Natural Cycle: 60						
Control Type: Actuated-Coordinated	1					
Maximum v/c Ratio: 0.59						
Intersection Signal Delay: 9.8				In	tersection LOS: A	
Intersection Capacity Utilization 77.9	9%				U Level of Service D)
Analysis Period (min) 15						
# 95th percentile volume exceeds	capacity que	eue may he	longer			
Ougus shown is maximum after t		Juo may be	.ongor.			

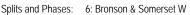
Queue shown is maximum after two cycles.

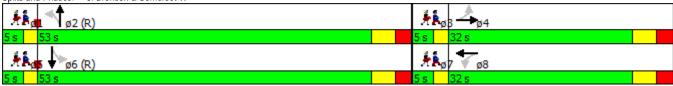
Splits and Phases: 4: Bronson & Primrose <u>▶</u>
ø4 ø6 (R)

	•	-	•	+	*	†	/	+				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	ø1	ø3	ø5	ø7
Lane Configurations	7	ĵ₃	*	ĵ,		4Tb		4Tb				
Volume (vph)	24	113	80	178	63	545	21	498				
Lane Group Flow (vph)	25	150	84	203	0	693	0	595				
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Protected Phases		4		8		2		6	1	3	5	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	2	2	6	6				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	1.0	1.0	1.0	1.0
Minimum Split (s)	18.9	18.9	18.9	18.9	18.8	18.8	18.8	18.8	7.0	7.0	7.0	7.0
Total Split (s)	32.0	32.0	32.0	32.0	53.0	53.0	53.0	53.0	5.0	5.0	5.0	5.0
Total Split (%)	33.7%	33.7%	33.7%	33.7%	55.8%	55.8%	55.8%	55.8%	5%	5%	5%	5%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	2.0	2.0	2.0	2.0
All-Red Time (s)	2.6	2.6	2.6	2.6	2.5	2.5	2.5	2.5	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0				
Total Lost Time (s)	5.9	5.9	5.9	5.9		5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max	Max	Max	C-Max	C-Max	C-Max	C-Max	Min	Min	Min	Min
Act Effct Green (s)	26.1	26.1	26.1	26.1		47.2		47.2				
Actuated g/C Ratio	0.27	0.27	0.27	0.27		0.50		0.50				
v/c Ratio	0.11	0.38	0.39	0.47		0.57		0.44				
Control Delay	27.4	31.2	34.5	33.0		19.1		16.7				
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0				
Total Delay	27.4	31.2	34.5	33.0		19.1		16.7				
LOS	С	С	С	С		В		В				
Approach Delay		30.7		33.4		19.1		16.7				
Approach LOS		С		С		В		В				
Queue Length 50th (m)	3.4	22.2	12.5	31.0		44.9		35.4				
Queue Length 95th (m)	9.9	39.5	26.4	51.7		62.1		48.8				
Internal Link Dist (m)		306.8		381.0		115.2		171.7				
Turn Bay Length (m)												
Base Capacity (vph)	228	399	216	431		1213		1345				
Starvation Cap Reductn	0	0	0	0		0		0				
Spillback Cap Reductn	0	0	0	0		0		0				
Storage Cap Reductn	0	0	0	0		0		0				
Reduced v/c Ratio	0.11	0.38	0.39	0.47		0.57		0.44				
Intersection Summary												
Cycle Length: 95												
Actuated Cycle Length: 95												

Actuated Cycle Length: 95
Offset: 24 (25%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.57
Intersection Signal Delay: 21.8
Intersection Capacity Utilization 81.1%
Analysis Period (min) 15

Intersection LOS: C ICU Level of Service D





Projected PM 5: Cambridge & Primrose

	•	→	*	•	+	•	•	†	<u> </u>	/	 	-√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			₩.			4			43-	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	33	108	2	4	42	62	1	0	8	56	3	23
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	35	114	2	4	44	65	1	0	8	59	3	24
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	151	114	9	86								
Volume Left (vph)	35	4	1	59								
Volume Right (vph)	2	65	8	24								
Hadj (s)	0.07	-0.30	-0.48	0.00								
Departure Headway (s)	4.3	4.0	4.1	4.5								
Degree Utilization, x	0.18	0.13	0.01	0.11								
Capacity (veh/h)	812	874	810	749								
Control Delay (s)	8.3	7.6	7.1	8.0								
Approach Delay (s)	8.3	7.6	7.1	8.0								
Approach LOS	А	Α	А	А								
Intersection Summary												
Delay			8.0									
Level of Service			Α									
Intersection Capacity Utilization			34.2%	ICI	J Level of S	ervice			Α			
Analysis Period (min)			15									

	•	•	†	/	\	Ţ
Movement	₩BL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	···bit	ĵ.	11511	002	4
Volume (veh/h)	23	16	66	29	20	59
Sign Control	Stop	10	Free	27		Free
Grade	0%		0%			0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
	24	17	69	31	21	62
Hourly flow rate (vph) Pedestrians	24	17	09	31	21	02
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						198
pX, platoon unblocked						
vC, conflicting volume	189	85			100	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	189	85			100	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	97	98			99	
cM capacity (veh/h)	789	974			1493	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	41	100	83			
Volume Left	24	0	21			
Volume Right	17	31	0			
cSH	856	1700	1493			
Volume to Capacity	0.05	0.06	0.01			
Queue Length 95th (m)	1.1	0.00	0.01			
	9.4	0.0				
Control Delay (s)		0.0	2.0			
Lane LOS	Α	0.0	A			
Approach Delay (s)	9.4	0.0	2.0			
Approach LOS	А					
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utilization			21.1%	IC	U Level of Servi	ce
Intersection Capacity Utilization Analysis Period (min)			21.1% 15	IC	U Level of Servi	ce

Appendix D SYNCHRO Capacity Analysis: Modified Bronson/Slater Intersection

	→	†	\	
Lane Group	EBT	NBT	SBL	SBT
Lane Configurations	414	A 13	*	44
Volume (vph)	927	475	49	374
Lane Group Flow (vph)	1070	749	52	394
Turn Type	NA	NA	Perm	NA
Protected Phases	4	2		6
Permitted Phases			6	
Detector Phase	4	2	6	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.9	26.0	26.0	26.0
Total Split (s)	32.0	28.0	28.0	28.0
Total Split (%)	53.3%	46.7%	46.7%	46.7%
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.6	2.7	2.7	2.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.9	6.0	6.0	6.0
Lead/Lag	317	0.0	0.0	0.0
Lead-Lag Optimize?				
Recall Mode	Max	C-Max	C-Max	C-Max
Act Effct Green (s)	26.1	22.0	22.0	22.0
Actuated g/C Ratio	0.44	0.37	0.37	0.37
v/c Ratio	0.81	0.71	0.31	0.35
Control Delay	21.3	18.5	19.6	15.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	21.3	18.5	19.6	15.0
LOS	C C	В	В	В
Approach Delay	21.3	18.5	D	15.5
Approach LOS	C C	В		В
Queue Length 50th (m)	51.2	21.8	4.0	16.1
Queue Length 95th (m)	#76.7	38.5	12.2	25.5
Internal Link Dist (m)	157.6	69.3	12.2	35.9
Turn Bay Length (m)	137.0	07.3	17.0	33.7
Base Capacity (vph)	1318	1057	17.0	1118
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductin	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.81	0.71	0.31	0.35
Intersection Commons	0.01	0.71	0.01	0.55

Intersection Summary

Cycle Length: 60

Actuated Cycle Length: 60
Offset: 13 (22%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.81 Intersection Signal Delay: 19.2 Intersection Capacity Utilization 84.1%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

