### Wurtemburg Tower 101 Wurtemburg Street Ottawa, Ontario

**Servicing Design Brief** 

#### WURTEMBURG TOWER 101 WURTEMBURG STREET OTTAWA, ONTARIO

**SERVICING DESIGN BRIEF** 

#### Prepared by:

NOVATECH ENGINEERING CONSULTANTS LTD.
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File No.: 111013
Report Reference No.: R-2011-033
February 25, 2011



February 25, 2011

City of Ottawa Planning and Growth Management Department Development Review (Urban) Services Branch Infrastructure Approvals Division 110 Laurier Avenue West, 4<sup>th</sup> Floor Ottawa ON, K1P 1J1

Attention:

Mr. Bruce Coombe

Dear Sir:

Reference: Wurtemburg Tower – 101 Wurtemburg Street

Servicing Design Brief Our File No.: 111013

Enclosed herein is the Servicing Design Brief for the proposed residential development at 101 Wurtemburg Street, located on the east side of the intersection of Wurtemburg Street and Clarence Street. This brief is submitted in support of the rezoning and site plan applications and outlines how the site will be serviced, and demonstrates that adequate services are available for sanitary, storm and water.

Trusting this report is adequate for your purposes. Should you have any questions, or require additional information, please contact us.

Yours truly,

**NOVATECH ENGINEERING CONSULTANTS LTD.** 

Greg MacDonald, P.Eng Senior Project Manager

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#### 1.0 INTRODUCTION

The proposed Wurtemburg Tower residential development at 101 Wurtemburg Street is located on the east side of the intersection of Wurtemburg Street and Clarence Street in the City of Ottawa, as shown in Figure 1 - Key Plan. The proposed development will consist of an 18 storey tower with 66 condominium units. Also, a total of 54 parking spaces will be provided on 3 levels of underground parking. Refer to Figure 2 - Site Plan for details.

As identified in the City of Ottawa's Zoning By-law, this site is currently designated as R5C [926] F(2.5) – Residential Fifth Density Subzone C, Exception 926, FSI of 2.5; a mix of medium and high density residential units. The zoning amendment will revise the 926 exception to permit the proposed development, as shown in the site plan. Specific details are provided in a Planning Rationale submitted as part of the ZBL Amendment application.

The subject site consists of approximately 0.0795 ha and is currently occupied by a two-storey residential building, as shown in Figure 3 – Existing Conditions.

This servicing design brief will outline how the site will be serviced with sanitary, storm and water, as well will demonstrate that adequate municipal capacity is available to service the development.

#### 2.0 SANITARY SEWER

The development will be serviced by a 200 mm dia. sanitary service that will connect to the existing 300 mm dia. sanitary sewer on Wurtemburg Street. All proposed sanitary services will be equipped with full-port backwater valves.

Proposed development flows, based on the City's guidelines for residential units, retail, and office space are presented below and are compared to the design flows based on current zoning.

#### Proposed Average Sanitary Flows Under Proposed Development

#### Wurtemburg Tower

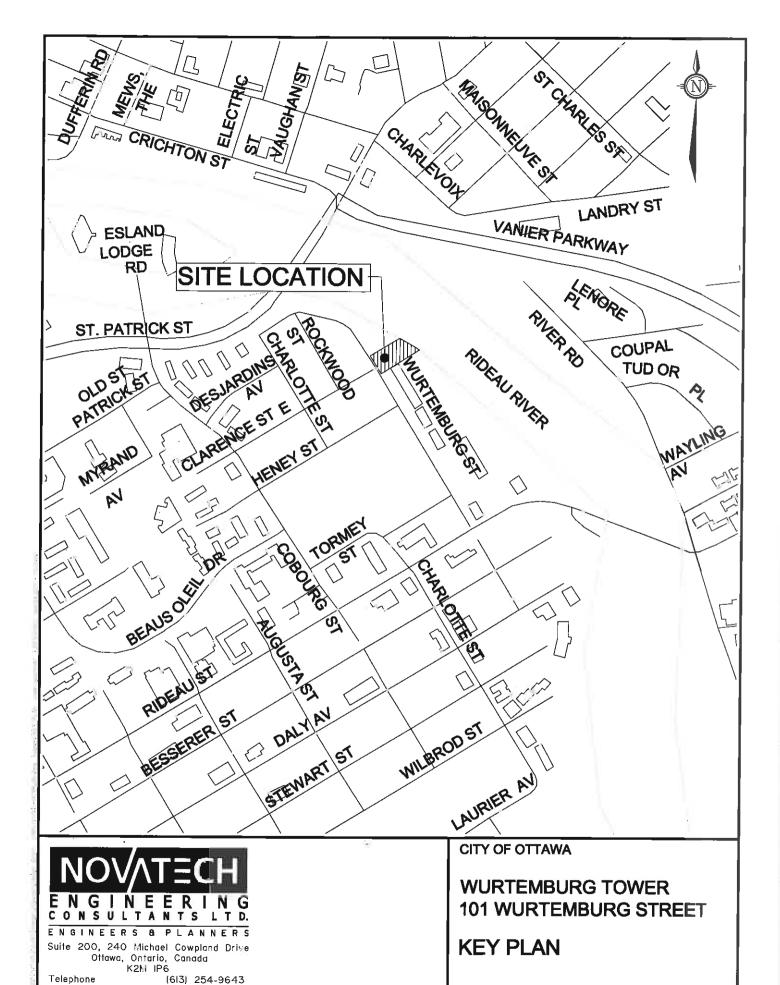
Residential: Q<sub>SAN</sub> = 66 units x 1.8 persons/unit x 350 L/cap/day = 41,580 L/day

Total Wurtemburg Tower Average Sanitary Flow = 41,580 L/day = 0.48 L/sec

Total Wurtemburg Tower Peak Sanitary Flow = 1.92 L/sec (with PF = 4.22 ∴ use 4.0 ⇒ max for residential)

Therefore,

Total Site Average Sanitary Flow = 0.48 L/sec Total Site Peak Sanitary Flow = 1.92 L/sec (with PF)



Facsimile

Email:

(613) 254-5867

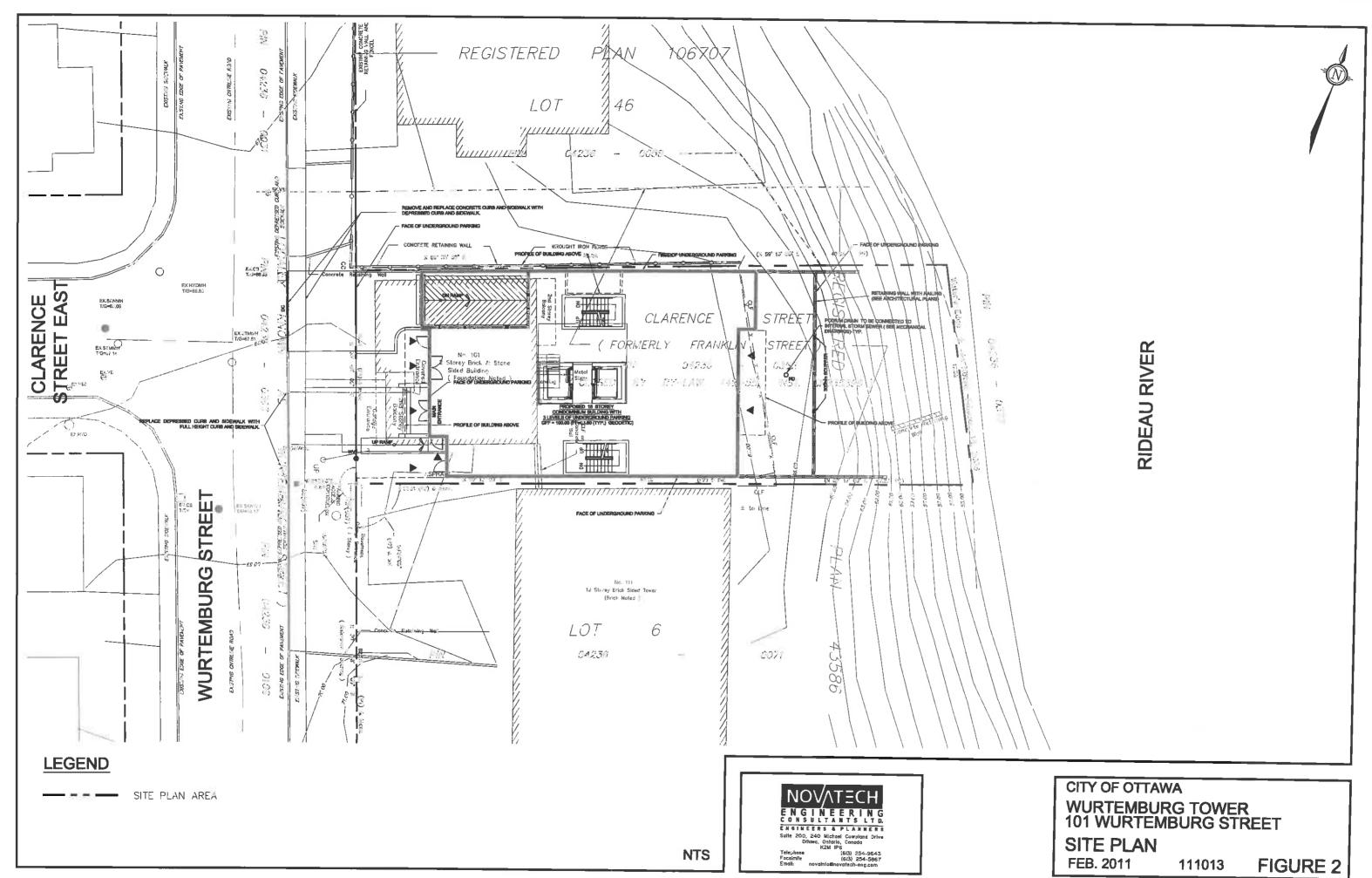
novainto@novatech-eng.com

SHT&X11.DWG - 218mmX278mm

FIGURE 1

111013

FEB. 2011



SHT11X17 DWG - 278mmX43in:m



LEGEND

SITE PLAN AREA

CITY OF OTTAWA WURTEMBURG TOWER 101 WURTEMBURG STREET **EXISTING CONDITIONS** 

FEB. 2011

111013

FIGURE 3

#### **Calculated Average Sanitary Flows Under Current Zoning**

Currently, the site is zoned R5C [926] F(2.5), a mix of medium and high density residential units. Zoning flows are calculated using the City's general population densities from Table 4.1 of the City of Ottawa Sewer Design Guidelines and are as follows:

Site Area = 0.0795 ha
Population Density = 1800 people/ha
Population = 0.0795 ha \* 1800 people/ha = 143 people

Average Sanitary flow = 143 people \* (350 L/person/day) / (86,400 s/day) = 0.58 L/sec

Total Sanitary flow = 2.32 L/sec (with PF = 4.20  $\therefore$  use 4.0  $\Rightarrow$  max for residential)

Therefore, the proposed development sanitary flows are less than the flows calculated under the existing zoning.

#### 3.0 STORMWATER

Stormwater flows from the site are currently conveyed to the existing storm sewer system via road catchbasins by overland flows and to the river. As part of this development, all stormwater will be controlled on site and discharged via a 200 mm dia. storm service that will connect to the existing 300 mm dia. storm sewer on Wurtemburg Street. All proposed storm services will be equipped with backwater valves.

The City will require that on-site stormwater management be implemented to control post-development stormwater discharge for both the 5 & 100 year storm events based on an allowable runoff coefficient (C) of 0.50, a time of concentration ( $t_c$ ) of 20 minutes, and 5-year storm control. Stormwater management will be achieved through the use of rooftop controls. Should surplus storage be required, stormwater management alternatives such as storage tanks will be implemented.

The site will be graded such that flows in excess of the 100-year storm event will be conveyed overland to Wurtemburg Street and the river (as per existing conditions). Erosion and sediment control measures will be implemented during all phases of construction and inspected regularly. A separate stormwater management report will also be submitted as part of the site plan application.

#### 4.0 WATERMAIN

The proposed development will be serviced by a 150 mm dia. water service that will connect to the existing 200mm dia. watermain on Wurtemburg Street. A shut off valve will be provided on the property line per City of Ottawa Specifications. The water meter will be located in the basement level mechanical room of the building. Similarly, a remote receptacle will be located at the surface near the entrance to the building on the exterior.

Estimated domestic water demands for the development are roughly the same as the proposed development sanitary flows listed above in Section 2.0.

#### Wurtemburg Tower

Average daily demand (L/sec): Q<sub>WATER</sub> = 41,580 L/day ÷ 86,400 sec/day = 0.48 L/sec

Using a peak factor of 2.5, the required maximum daily demand yields:

Q<sub>WATER</sub> = 1.20 L/sec

Using a peak factor of 2.2, the required maximum hour demand yields:

Q<sub>WATER</sub> = 2.64 L/sec

Estimated water demands for the development are as follows (based on fixture units):

#### Wurtemburg Tower

Estimated domestic demand (max peak day) = 100 igpm = 7.58 L/sec

Based on the data provided by the City, the existing watermains in the area are adequate to service this development. A copy of the watermain data is attached.

The building will be provided with standpipe and sprinkler system for fire protection. Typically the fire demand for a development of this type is in the order of 750 US gal/min (approx. 625 igpm). The available supply at 20 psi is in the neighbourhood of 2100 igpm around the site, based on the hydrant data supplied by the City.

#### 5.0 CONCLUSIONS

Based on the foregoing, adequate sanitary, storm and water services are available to support this development.

#### **NOVATECH ENGINEERING CONSULTANTS LTD.**

Prepared by:

Justin Gauthier, B.Eng.

Junior Engineer

Reviewed by:

G.J. MocDONALD TO

Greg MacDonald, P.Eng. Senior Project Manager

## APPENDIX A Excerpts from Ottawa Sewer Design Guidelines

specifically the downtown core. The construction of new combined sewer systems is no longer permitted in the City of Ottawa other than for the replacement of existing combined sewers within the City's defined Combined Sewer Area (see Section 5.1.6).

New storm drainage systems cannot be connected to existing combined sewers except as an interim measure where sewer separation is to be ultimately implemented or where circumstances allow no other alternative.

Section 5 discusses combined sewers in greater detail since their design must consider peak storm flows.

#### 4.2.4 Private Sanitary Servicing

Private servicing consists mainly of private sewage collection and treatment systems on individual lots and do not form part of these guidelines.

#### 4.3 GENERAL POPULATION DENSITIES

**Pre-zoned Land** – When lands are zoned for a specific residential use and detailed information is not available, the following population densities shall apply:

**Table 4.1 Population Densities** 

Unit Type (Min Lot Area M²)	Zoning (And all similar zonings)	Persons per unit <sup>1</sup>	Units per net ha	Persons (per net ha) <sup>3</sup>	Persons (per gross
Res-Single Family (600)	R1A-B*	2.4	avg. 2	67	ha) 4
		3.4	16.7	57	34
Res-Single Family (501)	R1C-E*	3.4	20.0	68	41
Res-Single Family (441)	R1F-H*	3.4	22.7	77	47
Res-Single Family (360)	R1I-K*	3.4	27.8	94	57
Res-Single Family (270)	R1L-N*	3.4	37.1	126	76
Res-Single Family (197)	R1P-Q*	3.4	50.8	173	105
Res-Semi-detached (278)	R2A-B*	2.7	36.0	97	59
Res-Semi-detached (232)	R2C*	2.7	43.1	116	71
Res-Semi-detached (180)	R2D-E*, G*	2.7	55.6	150	91
Res-Semi-detached (135)	R2F*	2.7	74.1	200	121
Townhouse (170)	R3F*, R4A-B*	2.7	58.8	159	96
Townhouse (110)	R3M*	2.7	90.9	246	149
Res-Duplex (441)	R2A-C*	2.3	45.4	104	63
Res-Duplex (360)	R2D*, R3F-G*	2.3	55.6	128	77
Res-Duplex (270)	R2E-F*, R3K*	2.3	74.1	170	103
Res-Duplex (197)	R2G*, R4F*	2.3	101.6	234	141
Res-Triplex (557)	R3A-C*	2.3	53.9	124	75

Unit Type (Min Lot Area M <sup>2</sup> )	Zoning (And all similar zonings)	Persons per unit <sup>1</sup>	Units per net ha avg. <sup>2</sup>	Persons (per net ha) <sup>3</sup>	Persons (per gross ha) 4
Res-Triplex (330)	R3D-E*, H-J*, L*, N*, R4C-E*	2.3	90.9	209	127
Apartments:					
Low Density		1.8	100	180	
Medium Density		1.8	300	540	
High Density		1.8	1000	1800	
Very High Density <sup>5</sup>		1.8	1000 +	1800 +	

- \*) former City of Ottawa zoning designation.
- 1) from 1996 census data.
- 2) new suburban construction, 5-year average (1997-2001), except apartments data which is based on site plans & duplex density which is an assumed average.
- 3) "net ha" refers to population densities per hectare of purely residential land (i.e. area of the building lots only including private parking and roads but excluding all public road rights-of-way and all other non-residential uses such as parks, stormwater management facilities, commercial developments, schools, community centres, etc.).
- 4) "gross ha" refers to population densities per hectare of residential and all other non-residential land uses such as streets, schools and parks. Numbers provided apply to large subdivision situations. For smaller residential situations the persons per gross ha will be higher, about 75% of the persons per net ha.
- 5) apartment densities in the downtown have been as high as 4,000 units/net ha. Proposals with a units/net ha density greater than 1000 will be evaluated on a case-by-case basis.

**Development Proposed Land** – When the number and type of housing units within a proposed development are known, the calculation of population for the proposed development shall be based on the following:

**Table 4.2 Per Unit Populations** 

Unit Type	Persons Per Unit
Single Family	3.4
Semi-detached	2.7
Duplex	2.3
Townhouse (row)	2.7
Apartments:	
Bachelor	1.4
1 Bedroom	1.4
2 Bedroom	2.1
3 Bedroom	3.1
Average Apt.	1.8

#### 4.4.1 Calculation of Peak Design Flows

The formulae and parameters to be applied in the calculation of peak design flows (standard peak flow design parameters) for new or infill developments are illustrated in Figure 4.3 and described as follows:

Figure 4.3 Peak Flow Design Parameters Summary

#### **AVERAGE WASTEWATER FLOWS:**

Residential Average Flow:

350 L/c/day

Commercial Average Flow:

50,000 L/gross ha/d

**Institutional Average Flow:** Average Light Industrial Flow: 35,000 L/gross ha/d

50,000 L/gross ha/d

Average Heavy Industrial Flow 55,000 L/gross ha/d

PEAKING FACTORS:

Residential Peak factor: Harmon Equation

$$P.F.=1+\left(\frac{14}{4+\left(\frac{P}{1000}\right)^{\frac{1}{2}}}\right)*K$$

where: P=Population

K=Correction Factor = 1

Commercial Peak factor: Institutional Peak factor: 1.5

1.5

**Industrial Peak Factor:** 

Per Figure in Appendix 4-B

PEAK EXTRANEOUS FLOWS: (design event)

**Infiltration Allowance:** 

0.28 L/s/effective gross ha (for all areas)

Less than 10 ha.

**Foundation Drain Allowance:** 

5.0 L/s/gross ha (if necessary for existing partially separated and

combined

areas only)

10 ha - 100 ha

**Foundation Drain Allowance:** 

3.0 L/s/gross ha (if necessary for existing partially separated and

combined

areas only)

Greater than 100 ha

**Foundation Drain Allowance:** 

2.0 L/s/gross ha (if necessary for existing partially separated and

combined

areas only)

APPENDIX B
Hydrant Flow Data

# Justin Gauthier

Crowder, Murray [Murray.Crowder@ottawa.ca] Thursday, February 03, 2011 8:34 AM From: Sent: ë

Subject:

j.gauthier@novatech-eng.com RE: Hydrant Flow Data Request (Wurtemburg Street)

Wurtemburg & Clarence.pdf

Attachments:

Follow up Completed Follow Up Flag: Flag Status:

Note: the computed flows are approximate and performed for hydrant colour coding purposes,

thus these values are not intended for design purposes.

Novatech Engineering Consultants Ltd. **Justin Gauthier** Company: <u>등</u>

254-9643x217

Wurtemburg @ Clarence 11-02-03-08:17:42 254-5867 Request\_dt: Location: Fax:

gauthier@novatech-eng.com 3 Email:

(igpm)									46 950	863
										>52 3
Pressure (psi)	Static	56	56	64	64	56	58	56	90	09
Residual	Hydrant	6832186	6832186	6832161	6832161	6832186	6832162	6832186	6832136	6832188
Flow	Hydrant	6833125	6833127	6832160	6832173	6832184	6832174	6832185	6832186	6832187
Inspection	Date	2009/06/01	2009/06/01	2009/06/02	2009/06/02	2009/06/01	2009/06/01	2009/06/01	2009/06/01	2009/06/01

@ 20 psi 2275 2093 2333 2110

Flow

2045 2155 1945 2265 2059

Murray Crowder

**Technical Support** 

**Environmental Services Department Drinking Water Operations Branch** 

City of Ottawa 951 Clyde Avenue, Ottawa, On K1Z 5A6

Mail Code 06-65 Tel: (613) 580-2424 x 22231

(613) 728-4183 Fax:

e-mail: murray.crowder@ottawa.ca

From: Justin Gauthier [mailto:j.gauthier@novatech-eng.com]

Sent: February 02, 2011 3:57 PM

To: Crowder, Murray

Subject: Hydrant Flow Data Request (Wurtemburg Street)

Importance: High

Hi Murray,

We are currently working on a Site Plan located at 101 Wurtemburg Street. I send this e-mail requesting that you please provide me with hydrant flow data (static, fire flows @ 20 psi, etc.) for the following hydrants:

H125, H127, H186, H187 on Wurtemburg;

H160, H184 on Clarence;

H174, H185 on Heney;

H173 on Charlotte.

Refer to attached sketch for hydrant locations, they are located on City of Ottawa grid 368-032 & 368-033.

Could you please also send me the sketch from your program showing the watermain sizes you have on file for the above-mentioned streets around the site, as you have in the past (if you have it for SAN & STM as well), thanks in advance.

I'm in a little bit of a rush for the information, so a quick response would be greatly appreciated.

Should you have any questions, or require additional information, don't hesitate to contact me.

Regards,

Justin Gauthier, B.A.Sc.

Novatech Engineering Consultants Ltd.

Suite 200, 240 Michael Cowpland Drive

Kanata, Ontario, Canada, K2M 1P6 Phone: 613.254.9643 x217

Fax: 613.254.5867

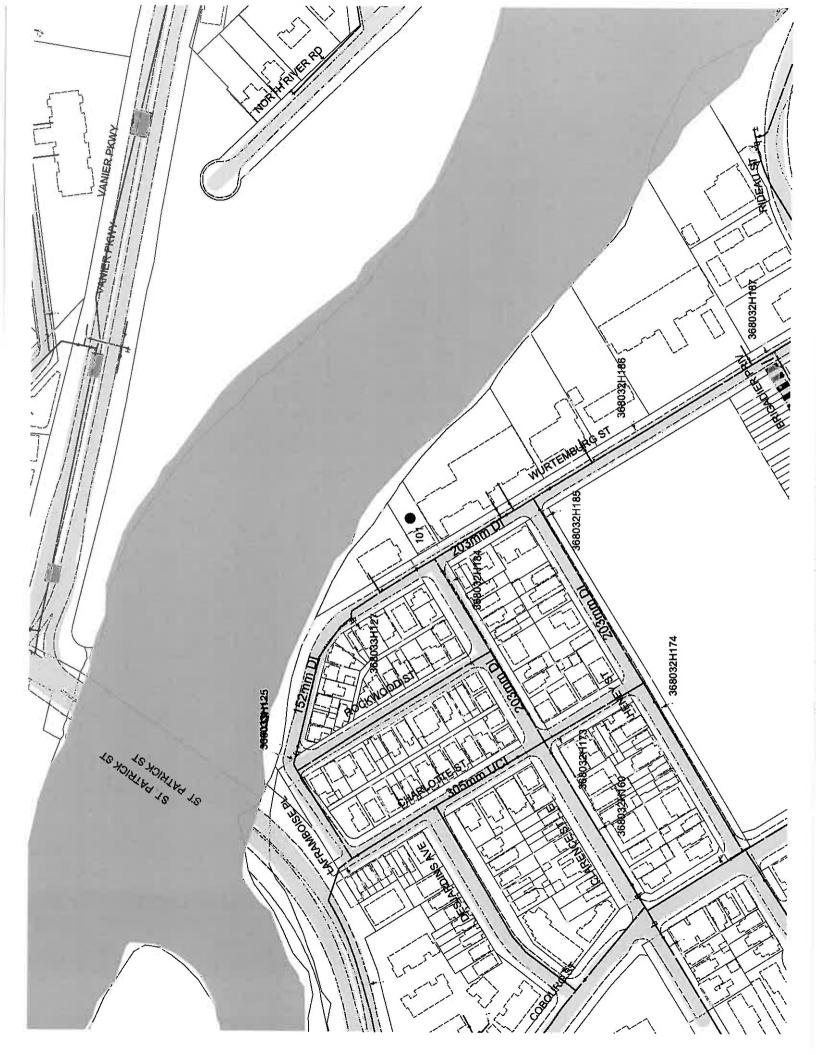
Email: j.gauthier@novatech-eng.com

Website: www.novatech-eng.com

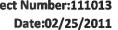
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## APPENDIX C Servicing Study Guidelines Checklist





4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Executive Summary (for larger reports only).	NA		
Date and revision number of the report.	Υ	Cover	
Location map and plan showing municipal address,			
boundary, and layout of proposed development.	Υ		Figures 1, 2 and 3
Plan showing the site and location of all existing services.	Υ		Figures 2 and 3
Development statistics, land use, density, adherence to			
zoning and official plan, and reference to applicable	v	1.0	
subwatershed and watershed plans that provide context	Υ	1.0	
to which individual developments must adhere.			
Summary of Pre-consultation Meetings with City and			
other approval agencies.	N		
Reference and confirm conformance to higher level			
studies and reports (Master Servicing Studies,			
Environmental Assessments, Community Design Plans),	, v		
or in the case where it is not in conformance, the	Υ	2.0 - 4.0	
proponent must provide justification and develop a			
defendable design criteria.			
Statement of objectives and servicing criteria.	Y		Addressed in Section 2.0, 3.0. 4.0.
Identification of existing and proposed infrastructure			
available in the immediate area.	Y		Figures 2 and 3
Identification of Environmentally Significant Areas,			
watercourses and Municipal Drains potentially impacted			
by the proposed development (Reference can be made to	NA		
the Natural Heritage Studies, if available).			
Concept level master grading plan to confirm existing and			
proposed grades in the development. This is required to			
confirm the feasibility of proposed stormwater	N		
management and drainage, soil removal and fill			MOUTE and the first transfer
constraints, and potential impacts to neighboring			Will be addressed in Site plan application.
properties. This is also required to confirm that the			
proposed grading will not impede existing major system			
flow paths.			



#### **Project Name: Wurtemburg Tower** Project Number:111013

Date:02/25/2011

4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Identification of potential impacts of proposed piped			
services on private services (such as wells and septic			
fields on adjacent lands) and mitigation required to	NA		
address potential impacts.			_
Proposed phasing of the development, if applicable.	NA		
Reference to geotechnical studies and recommendations			
concerning servicing.	N		
All preliminary and formal site plan submissions should			
have the following information:			
Metric scale	Υ	ALL	
North arrow (including construction North)	Υ	ALL	
Key plan	Υ	ALL	
Name and contact information of applicant	v		
and property owner	Y	ALL	
Property limits including bearings and	Υ	ALL	
Existing and proposed structures and parking	Υ	ALL	
Easements, road widening and rights-of-way	Υ	ALL	
Adjacent street names	γ	ALL	



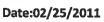
	Addressed		
4.2 Water	(Y/N/NA)	Section	Comments
Confirm consistency with Master Servicing Study, if	,,	F 0	All for A Both to the second
available.	Υ	5.0	Also refer to Appendix B for Hydrant Flow data.
Availability of public infrastructure to service proposed	.,		
development.	Υ		Figures 2 and 3
Identification of system constraints.	Υ	4.0	
Identify boundary conditions.	Υ	4.0	
Confirmation of adequate domestic supply and pressure.	Υ	4.0	
Confirmation of adequate fire flow protection and			
confirmation that fire flow is calculated as per the Fire			
Underwriter's Survey. Output should show available fire	Y	4.0	
flow at locations throughout the development.	;		
Provide a check of high pressures. If pressure is found to			
be high, an assessment is required to confirm the	Υ	4.0	
application of pressure reducing valves.			
Definition of phasing constraints. Hydraulic modeling is			
required to confirm servicing for all defined phases of the	NA		
project including the ultimate design.			
Address reliability requirements such as appropriate	, , , , , , , , , , , , , , , , , , ,	4.0	
location of shut-off valves.	Υ	4.0	
Check on the necessity of a pressure zone boundary			
modification.	NA		
Reference to water supply analysis to show that major			
infrastructure is capable of delivering sufficient water for			
the proposed land use. This includes data that shows that	.,		
the expected demands under average day, peak hour and	Y	4.0	
fire flow conditions provide water within the required			
pressure range.			
Description of the proposed water distribution network,			
including locations of proposed connections to the			
existing system, provisions for necessary looping, and			
appurtenances (valves, pressure reducing valves, valve	Y	4.0	
chambers, and fire hydrants) including special metering			
provisions.			
Description of off-site required feedermains, booster			
pumping stations, and other water infrastructure that will		į	
be ultimately required to service proposed development,	NA		
including financing, interim facilities, and timing of			
implementation.			
Confirmation that water demands are calculated based		4.0	
on the City of Ottawa Design Guidelines.	Y	4.0	
Provision of a model schematic showing the boundary			
conditions locations, streets, parcels, and building	N	l	Figure 3 shows main.
locations for reference.		l	



Project Name: Wurtemburg Tower
Project Number:111013

Date:02/25/2011

4.3 Wastewater	Addressed	Section	Comments
	(Y/N/NA)	55555	Commence
Summary of proposed design criteria (Note: Wet-weather	1		
flow criteria should not deviate from the City of Ottawa			
Sewer Design Guidelines. Monitored flow data from	Υ	2.0	
relatively new infrastructure cannot be used to justify			
capacity requirements for proposed infrastructure).			
Confirm consistency with Master Servicing Study and/or	Y	2.0	
justifications for deviations.	'	2.0	
Consideration of local conditions that may contribute to	1		
extraneous flows that are higher than the recommended	Y	2.0	
flows in the guidelines. This includes groundwater and		2.0	
soil conditions, and age and condition of sewers.			
Description of existing sanitary sewer available for	Υ	2.0	
discharge of wastewater from proposed development.	Y	2.0	
Verify available capacity in downstream sanitary sewer			
and/or identification of upgrades necessary to service the			
proposed development. (Reference can be made to	Υ	2.0	
previously completed Master Servicing Study if			
applicable)			
Calculations related to dry-weather and wet-weather			
flow rates from the development in standard MOE	Y	2.0	
sanitary sewer design table (Appendix 'C') format.			
Description of proposed sewer network including sewers,			
pumping stations, and forcemains.	Y	2.0	Figures 2 and 3
Discussion of previously identified environmental			
constraints and impact on servicing (environmental			
constraints are related to limitations imposed on the			
development in order to preserve the physical condition	NA	i	
of watercourses, vegetation, soil cover, as well as	!		
protecting against water quantity and quality).			
Pumping stations: impacts of proposed development on			
existing pumping stations or requirements for new	NA		
pumping station to service development.			
Forcemain capacity in terms of operational redundancy,			
surge pressure and maximum flow velocity.	NA		
	-		
dentification and implementation of the emergency			
overflow from sanitary pumping stations in relation to the	NA		
nydraulic grade line to protect against basement flooding.			
Special considerations such as contamination, corrosive			
environment etc.	NΑ		





	Addressed		
4.4 Stormwater	(Y/N/NA)	Section	Comments
Description of drainage outlets and downstream			
constraints including legality of outlet (i.e. municipal	Y	3.0	
drain, right-of-way, watercourse, or private property).			
Analysis of the available capacity in existing public		2.0	
infrastructure.	Y	3.0	
A drawing showing the subject lands, its surroundings,			
the receiving watercourse, existing drainage patterns and	Y		Figures 2 and 3
proposed drainage patterns.			
Water quantity control objective (e.g. controlling post-			
development peak flows to pre-development level for			
storm events ranging from the 2 or 5 year event	<b>i</b> 1		
(dependent on the receiving sewer design) to 100 year			
return period); if other objectives are being applied, a	Y	3.0	
rationale must be included with reference to hydrologic			
analyses of the potentially affected subwatersheds,	l i		
taking into account long-term cumulative effects.			
Water Quality control objective (basic, normal or			
enhanced level of protection based on the sensitivities of	NA		
the receiving watercourse) and storage requirements.			
Description of stormwater management concept with			
facility locations and descriptions with references and	. ү	3.0	
supporting information.	'	3.0	
Set-back from private sewage disposal systems.	NA NA		
Watercourse and hazard lands setbacks.	NA NA		
Record of pre-consultation with the Ontario Ministry of	INA		
Environment and the Conservation Authority that has	NA		
jurisdiction on the affected watershed.	l NA		
Confirm consistency with sub-watershed and Master			
Servicing Study, if applicable study exists.	Υ	3.0	
Storage requirements (complete with calcs) and			
conveyance capacity for 5 yr and 100 yr events.	Υ ]	3.0	
Identification of watercourse within the proposed			_
development and how watercourses will be protected, or,			
if necessary, altered by the proposed development with	NA		
applicable approvals.			
Calculate pre and post development peak flow rates			
including a description of existing site conditions and			
	Υ	3.0	
proposed impervious areas and drainage catchments in	ľ		
comparison to existing conditions.			
Any proposed diversion of drainage catchment areas from one outlet to another.	NA		
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and SWM facilities.	Υ	3.0	
If quantity control is not proposed, demonstration that			
downstream system has adequate capacity for the post-	i		
	NA		
development flows up to and including the 100-year return period storm event.	ļ		
return period storm event.	_		



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4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Identification of municipal drains and related approval requirements.	NA		
Description of how the conveyance and storage capacity will be achieved for the development.	Υ	3.0	
100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Υ	3.0	
Inclusion of hydraulic analysis including HGL elevations.	N		
Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Υ	3.0	
Identification of floodplains — proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	NA		
Identification of fill constrains related to floodplain and geotechnical investigation.	NA		



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4.5 Approval and Permit Requirements	Addressed (Y/N/NA)	Section	Comments
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	NA		
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N		
Changes to Municipal Drains.	N		
Other permits (National Capital Commission, Parks			
Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	NA 		

4.6 Conclusion	Addressed (Y/N/NA)	Section	Comments
Clearly stated conclusions and recommendations.	Υ	5.0	
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	NA		
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario.	Υ	5.0	