

Site Servicing and Stormwater Management Report for Site Plan Control Application

Proposed New Public High School Barrhaven Centre 4005 Strandherd Drive Barrhaven, Ontario

Prepared for

Conseil des écoles publiques de l'Est de l'Ontario

Attention: Mrs Carolyn Jones, MRAIC

September 5, 2019 Revision 03

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LRL File No.: 170364



TABLE OF CONTENTS

1	INT	RODUCTION	3
2	SIT	E DESCRIPTION	3
3	SC	OPE OF WORK	4
4	WA	TER SUPPLY AND FIRE PROTECTION	5
	4.1	Existing Water Supply Services	5
	4.2	Water Supply Demand	5
	4.3	Water supply servicing design	6
	4.4	Boundary Conditions	6
5	SAI	NITARY DRAINAGE	7
	5.1	Municipal Sanitary Sewer Services	7
	5.2	Sanitary Sewer Servicing Design	7
6	STO	DRMWATER MANAGEMENT	7
	6.1	Municipal Stormwater Infrastructure	7
	6.2	Stormwater Management Concept	8
	6.3	Design Criteria	8
	6.3.	1 Water Quality	8
	6.3.	2 Water Quantity	8
	6.4	Method of Analysis	8
	6.5	Allowable Release Rate	8
	6.6	Stormwater Quantity Controls	9
	6.7	Stormwater Quality Management	10
7	ER	OSION AND SEDIMENT CONTROL	10
8	CO	NCLUSIONS	10
9	LIM	IITATIONS AND USE OF REPORT	11

LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 2 of 11

LIST OF FIGURES

Figure 1 - Aerial view of the location of the proposed development (Google Earth)....... 4

APPENDICES

Appendix A	Domestic Water Demand and Fire Flow Calculations & Boundary Conditions
Appendix B	Sanitary Sewer Calculation Sheet
Appendix C	Stormwater Management Design Sheets
Appendix D	Supporting Documents
Appendix E	Engineering Drawings



LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 3 of 11

1 Introduction

LRL Associates Ltd. (LRL) has been retained by the Conseil des écoles publiques de L'Est de l'Ontario (CEPEO) to prepare a site servicing and stormwater management report in support of their site plan control application for a proposed new Public High School in Barrhaven. This report presents the proposed servicing plan of the new development for water and sanitary services, as well as stormwater management.

This report has been prepared in consideration of the survey carried out by Annis O'Sullivan Vollebekk Ltd. (AOV) in August 2017 and the design brief prepared by J.L. Richards & Associates Limited and dated July 2017. Should there be any discrepancies in the existing infrastructure and/or connections to the existing services, which may relate to the site servicing considerations, LRL should be advised in order to review the report recommendations. This report should be read in conjunction with the grading and drainage, site servicing, and stormwater management plans prepared by LRL.

2 SITE DESCRIPTION

The subject property is currently vacant land and is located within the urban boundary of the City of Ottawa, Ontario. As illustrated in Figure 1, the development is located south on Strandherd Drive. The total area of the property measures approximately 4.86 ha.

Chapman Mills is to be developed and constructed by others in accordance with the design prepared by J.L. Richards & Associates Limited as part of Harmony Stage 1 Development for Minto Communities, dated July 2017.

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Figure 1 - Aerial view of the location of the proposed development (Google Earth)

The proposed development is located within the proposed Minto Harmony Community area that has yet to be fully constructed by others. The land surface has a minimal grade change with elevations ranging between 92.09m and 94.57m.

The proposed development consists of a three-storey, slab-on grade, public high school with access roadway, parking areas, playground areas, a soccer field and a future portable classes area. Refer to Appendix D for the proposed site plan (Drawing C401 – Servicing Plan).

3 SCOPE OF WORK

As per applicable guidelines, the scope of work includes the following:

Water services

- Calculate the expected domestic water demands at average and peak flow conditions.
- Calculate the fire flow as per the Fire Underwriters Survey (FUS) method.
- Describe the proposed water distribution network and connection to the existing watermain.

Sanitary services

- Describe the existing sanitary sewers available to receive wastewater from the building.
- Calculate the peak flow rates from the development.
- Describe the proposed sanitary sewer system.

LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 5 of 11

- Verify the available capacity in the downstream sanitary sewer.
- Verify the capacity of the existing lateral sanitary sewer

Stormwater management

- Calculate the allowable stormwater release rate.
- Calculate the anticipated post development stormwater release rates.
- Demonstrate how the target quality and quantity objectives will be achieved.
- Verify the capacity of the existing lateral storm sewer

4 WATER SUPPLY AND FIRE PROTECTION

4.1 Existing Water Supply Services

The 300mm diameter watermain on Chapman Mills Drive will be constructed by others. According to J.L. Richards & Associates Limited design, the site will have a 200mm diameter water service stub provided along Chapman Mills Drive, approximately 240m southeast of Strandherd Drive. There will be four (4) fire hydrants along Chapman Mills Drive on the east side near the proposed site. Refer to C401 – Servicing Plan for the proposed municipal infrastructure design.

4.2 Water Supply Demand

As per the AWWA Standards and the City of Ottawa Design Guidelines, the average domestic water demand was calculated using 850 equivalent fixture units and for daily and hourly peaking factors of **1.5** and **1.8**, respectively. Thus, the average daily domestic water demand for the proposed building is **12.33** L/s, the maximum daily flow rate is **18.50** L/s and the maximum hourly flow rate is **22.20** L/s. Refer to Appendix A for the domestic water demand calculation sheet.

The fire flow requirement was evaluated in accordance with the Fire Underwriters Survey (FUS). This method is based on the floor area of the building to be protected, type and combustibility of the structural frame and the separation distances with adjoining buildings. The fire flow demand was calculated to be **116.7L/s**. Refer to Appendix A – Fire Flow Calculations, for the fire flow

However, to meet the minimum requirement of a 90m radius distance between the fire hydrant and the building, as required by the City of Ottawa, a private fire hydrant on the said property is added to service the building. In addition, a second private hydrant has been proposed south corner of the school for future portable expansion planning. Refer to LRL drawing C401 Rev.01 – *Servicing Plan* for the layout of the proposed water services and connections.

LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 6 of 11

4.3 Water supply servicing design

The proposed building will be serviced by a 200mm dia. water service which will connect to the 200mm diameter stub along Chapman Mills Drive. The proposed service will be located on the east side of the building. Refer to LRL drawing C401 – Servicing Plan for the layout of the proposed water service.

A fire department siamese connection is proposed on the east side near the building main student entrance. The siamese connection is located adjacent to the proposed sidewalk to facilitate the access for the fire department year-round. To meet the minimum requirement of a 45m radius distance between the fire hydrant and the building main entrance, as required by the OBC, a private fire hydrant will be installed at the north-east corner of the building. Refer to LRL drawing C401 Rev.01 – Servicing Plan for the layout of the proposed water service connection and the private fire hydrant.

4.4 Boundary Conditions

The existing boundary conditions provided by the City of Ottawa for the site are as follow:

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	156.5	89.5
Peak Hour	144.7	72.7
Max Day Plus Fire (7,000) L/min	127.8	48.7

As the available fire flow provided by the City of Ottawa is above the minimum fire flow requirement for the proposed development, no supplementary fire protection and storage are required for the site. Refer to Appendix A for the provided city boundary conditions dated 2018-Dec-17.

LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 7 of 11

5 SANITARY DRAINAGE

5.1 Municipal Sanitary Sewer Services

Along Chapman Mills Drive, a 300mm diameter sanitary sewer flowing easterly on Clarity Street will be constructed by others. J.L. Richards & Associates Limited design indicates that a 250mm diameter sanitary service stub will be provided to service this site with a total allowable flow 62.0 L/s. Refer to drawing C401 – Servicing Plan for the layout.

5.2 Sanitary Sewer Servicing Design

The new building will be serviced with a 250mm dia. sanitary service that will be installed south of the new building. A sanitary maintenance manhole, SAN MH01 will be installed just north of the property line before connecting to the 250mm stub and discharging to the municipality sewer system. The proposed 250mm PVC DR35 sanitary service will be installed at a minimum slope of 1.00%. Refer to LRL drawing C401 – Servicing Plan for the proposed sanitary servicing.

The sanitary peak load was calculated using the Ontario Building Code (2012), Part 7 – Plumbing and estimated at **12.33 L/s.**

Also, LRL used the City of Ottawa Sanitary Design Guidelines to estimate the allocated sanitary peak flow to ensure the most conservative demand is used. Using the criteria as shown in Table 1 and the City specific design parameters, the site anticipated sanitary flow was calculated at 5.58 L/s for the 4.86 ha area. Refer to Appendix B for the site sanitary sewer design sheet. Since this estimate is lower than the total sanitary peak flow estimated using the Ontario Building Code (2012) Part 7, the total sanitary flow for the proposed elementary school was estimated at 12.33 L/s, as this flow rate is more accurate and conservative.

Table 1 Sanitary Sewer Design Criteria

Design Parameter	Value
Minimum service connection size (diameter)	135 mm
Manning roughness coefficient (n)	0.013
Minimum velocity (full)	0.6 m/s
Maximum velocity (full)	3.0 m/s

6 STORMWATER MANAGEMENT

6.1 Municipal Stormwater Infrastructure

Along Chapman Mills Drive, a 675mm diameter storm sewer flowing easterly and draining into the 1,350mm diameter storm sewer on Clarity Street will be constructed by others. J.L. Richards

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LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 8 of 11

& Associates Limited show that a 1,050mm diameter storm service stub will be provided to service the site. Refer to drawing C401 – Servicing Plan for the layout.

6.2 Stormwater Management Concept

Drainage from the site will be captured by a series of roof drains, catchbasins, catchbasin manholes and perforated subdrains which will convey the stormwater to the existing 1,050mm diameter storm sewer stub on Chapman Mills Drive. Refer to LRL drawing C401 for the proposed storm servicing and drawings C301 and C702 for the grading and drainage plan and stormwater management. Refer to Appendix C – Stormwater Management Design Sheets for the proposed site storm sewer design.

6.3 Design Criteria

Stormwater quantity control measures are proposed for this site to reduce the post-development stormwater runoff to allowable levels.

6.3.1 Water Quality

Enhanced (80% Total Suspended Solids removal) quality control is to be provided by others. The stormwater retaining pond has been designed as part of Minto Harmony Community project, Phase 1 which was prepared by J.L. Richards & Associates Limited.

6.3.2 Water Quantity

All storm events up to and including the 100-year event will be controlled to the 5-year predevelopment level. The site major overland flow route has been designed to ensure that storm events beyond the 100-year design storm can be safely conveyed overland towards the Chapman Mills Drive right-of-way. The minor system (storm sewer) within the site is sized to convey the 5-year storm event flows from the site to the municipal storm sewer on Chapman Mills Drive.

6.4 Method of Analysis

The Rational Method was used to calculate the runoff from the development. The Intensity-Duration-Frequency (IDF) curve formulas for the MacDonald-Cartier International Airport, in the city of Ottawa, were used to calculate the peak storm flows for the site.

6.5 Allowable Release Rate

This site is subject to stormwater management control where the allowable flow for the 5 and 100-year storm events are estimated at 1,055.8 L/s as per the design prepared by J.L. Richards & Associates Limited. The total allowable release rate will be restricted with an undersized pipe

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LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 9 of 11

at the outlet that will throttle the total allowable release rate to **884L/s** as per J.L. Richards & Associates Limited design sheet. Refer to Appendix D - Supporting Documents.

6.6 Stormwater Quantity Controls

The proposed stormwater management quantity control for this development will be accomplished through the use of: undersized piping to throttle the flow rate, Zurn Control-Flo roof drains, roof top water storage, pipe and maintenance holes structure underground. The proposed site storm sewer and stormwater management system are shown on drawing C401 – Servicing Plan and detailed calculations including the design sheet are attached in Appendix C.

The collected stormwater from catchment area WS-15 (0.711ha), will first be regulated using thirty-five (35) one notch Zurn Control-Flo roof drains with a total maximum release rate of **14.91 L/s*m.** With the thirty-five (35) roof drains, a maximum ponding volume of **213** m³ of water will be stored on the roof during the 100-year storm event to minimize the water directed to MH02. The controlled roof water will be captured by the proposed storm manhole MH02 before outletting to Chapman Mills Drive. The two future expansions have been added to the design calculations in order to obtain the maximum value for the roof top ponding and flow generated by the hard surface. The roof drains will be connected downstream of the controlled manhole (MH03) to prevent a double control measurement as per the City of Ottawa guidelines. The total anticipated flow of the roof drains is **63.47L/s**.

WS-01, WS-02, WS-03, WS-04, WS-05, WS-06, WS-07, WS-08, WS-09, WS-10, WS-11, WS-12, WS-13 and WS-14 areas, (0.059ha, 0.105ha, 0.281ha, 0.221ha, 0.161ha, 0.095ha, 0.169ha, 0.201ha, 0.130ha, 1.033ha, 0.929ha, 0.333ha, 0.285ha and 0.147ha respectively) consist of parking and driving area, landscaped area and playground area. These catchments area will be captured through a number of catchbasins and subdrains before being directed to the proposed 825mm outlet on Chapman Mills Drive and controlled using an undersized 825mm diameter reinforced concrete pipe. The 825mm pipe will be installed at a 0.29% slope that has a full capacity of **769 L/s**.

In order to control the 100-year storm event, **224.25m³** of on-site storage will be required. This storage will be provided with the use of some pipe/structure storage and an infiltration gallery up to the expected high water level of 93.00m MASL. The stormwater storage will be provided as follows: **115.84** m³ from on-site pipes and maintenance structures and **180.31** m³ from the infiltration gallery. Refer to C401 – Servicing Plan and Appendix C for stormwater management design details. Thus, the outlet to Chapman Mills Drive is able of achieving the required stormwater quantity control of **884L/s**.

LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 10 of 11

6.7 Stormwater Quality Management

A Stormwater Management Facility Pond design by J.L. Richards & Associates Limited is located southeast of the subject site at the downstream end of Minto Harmony Community and will provide enhanced protection, 80% Total Suspended Solids removal. Hence, no stormwater quality management is required on the site. Refer to Appendix D – Supporting Documents for the location of the proposed stormwater management pond.

7 EROSION AND SEDIMENT CONTROL

During the construction, erosion and sediment controls will be required primarily via a sediment control fence to be erected along the perimeter of the site where runoff has the potential of leaving the site. Inlet sediment control devices are also to be provided in any catchbasin and/or manholes on and around the site that may be impacted by the site construction activities. Construction and maintenance requirements for erosion and sediment controls are to comply with Ontario Provincial Standard Specification OPSS 577. Refer to LRL drawing C101 – Erosion and Sediment Control Plan for details.

8 Conclusions

In accordance with this report objectives, the analyses for the proposed development can be summarized as follows:

Water Service

- The anticipated maximum domestic water demand for the site is 22.20 L/s.
- The required fire flow was calculated at 116.7 L/s using the FUS method.
- A new fire hydrant will be installed on-site within the m radius from the siamese connection.
- The new development will be serviced with a 200mm dia. watermain connected to the proposed 300mm dia. watermain on Chapman Mills Drive.

Sanitary Service

- The anticipated sanitary flow from the proposed development is 12.33 L/s.
- The proposed building will be serviced by a 250mm sanitary service connection to the existing 900mm dia. sanitary sewer on Chapman Mills Drive.
- A new monitoring manhole will be installed on the new 250mm sanitary service that is connected to the proposed 900mm dia. sanitary sewer on Chapman Mills Drive.

Stormwater Management

 The stormwater release rates from the proposed development will meet the predevelopment allowable release rate of 884 L/s onto Chapman Mills Drive.

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LRL File: 170364 Original September 14, 2018 Revised September 5, 2019 Page 11 of 11

- Stormwater quantity control objectives will be achieved through on-site storage.
- Stormwater quality control objectives will be achieved off-site through the future Stormwater Management Pond designed by J.L. Richards & Associates Limited.

9 LIMITATIONS AND USE OF REPORT

The report conclusions are applicable only to this project described in this report. Any changes may require a review by LRL Associates Ltd. to insure compatibility with the recommendations contained in this report. We trust the information presented meets your current requirements. Please do not hesitate to contact us should you have any questions or concerns.

Prepared by:

LRL Associates Ltd.

Virginia/Johnson, P.Eng

Civil Engineer



APPENDIX A

Domestic Water Demand and Fire Flow Calculations & Boundary Conditions



Domestic Water Supply Calculations LRL File No.

Project: Public High School Barrhaven Centre Location: 4005 Strandherd Drive, Barrhaven, Ottawa

Date: January 16, 2019 G. Brunet Designed:

Verified: J.C. Lalonde

Domestic Commercial Flow Demand

5,956 $\,m^2\,$ Total Building Floor Area = (includes existing and proposed building)

Site Total Area = 4.86 ha

Total Proposed Fixture Unit = 850 Average Demand Per Fixture Unit = L/min As per AWWA Standard 0.8704

Average Commercial Water Demand = 740 L/min 12.33

> Maximum Daily Peak Factor = 1.5 * As per City of Ottawa

Maximum Daily Commercial = 18.50 L/s 1,110 L/min

* As per City of Ottawa Maximum Hourly Peak Factor = 1.8

Maximum Hourly Commercial = 1,332 L/min 22.20 L/s

Therefore,

Domestic Peak Hourly Flow Rate 22.20 L/s Required Fire Flow rate= 116.7 L/s



Fire Flow Calculations

LRL File No. 170364

Project Public High School Barrhaven Centre

Date January 16, 2019

Method Fire Underwriters Survey (FUS)

Designed by G. Brunet

Multi-level Development	9281	
	9.281	m ²

Step	Task	Term	Options	Multiplier	Choose:	Value	unit	Fire Flow					
			Structural Framing Ma	aterial									
			Wood Frame	1.5									
	Choose frame used for	Coefficient C	Ordinary Construction	1.0									
1	building	related to the type of	Non-combustible construction	0.8	Non-combustible construction	0.8		1					
	ballaring	construction	Fire resistive construction <2 hrs	0.7									
			Fire resistive construction >2 hrs	0.6									
			Floor Space Are	a									
			Single family dwelling	0	_								
2	Choose type of housing	Type of housing	Townhouse - no. of units	0	Building - no. of units per floor	1	units						
			Building - no. of units per floor	1									
	Enter no. of storeys	Number of floors/storey	s for the building (excluding the basement)			3	floors						
3	Enter area	Enter floor space area		1		9,281	sq.m.						
4	Obtain fire flow before	Required fire flow	Fire Flo	w = 220 x C x	Area A 0.5		L/min	16,955					
	reductions	Trequired into now	THETIC	W - 220 X O X	Alea		L/s	282.6					
	Reductions or surcharge due to factors affecting burning												
			Non-combustible	-0.25									
	Choose combustibility of contents	Occupancy hazard	Limited combustible	-0.15									
5		reduction or surcharge	Combustible	0	Limited combustible	-0.15							
	or contonio	- Caucatan or Caronaryo	Free burning	0.15			L/min	14,412					
			Rapid burning	0.25			L/s	240.2					
			Sprinklers (NFPA13)	-0.30	True	-0.3							
6	Choose reduction for sprinklers	Sprinkler reduction	Water supply is standard for both the system and fire department hose lines	-0.10	True	-0.1	L/min	7,206					
			Fully supervised system	-0.10	True	-0.1	L/s	120.1					
			North side	Over 45m	0								
7	Choose separation	Exposure distance	East side	Over 45m	0								
′	Choose separation	between units	South side	Over 45m	0		L/min	7,206					
			West side	Over 45m	0	0	L/s	120.1					
Net required fire flow													
	Obtain fire flow.		Minimum require	ed fire flow rate	e (rounded to nearest 1000 as per Cit	y of Ottawa)	L/min	7,000					
8	duration, and volume				Minimum required	fire flow rate	L/s	116.7					
					Required duratio	n of fire flow	hr	2					

BOUNDARY CONDITIONS



Boundary Conditions For: 4005 Strandherd Dr.

Date of Boundary Conditions: 2018-Dec-17

Provided Information:

Scenario	Den	nand
	L/min	L/s
Average Daily Demand	739.8	12.3
Maximum Daily Demand	1,110	18.5
Peak Hour	1332	22.2
Fire Flow #1 Demand	7,000	116.7

Number of Connections: 1

Location:



BOUNDARY CONDITIONS



Results:

Pre-Configuration

Connection #: 1

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	156.5	89.5
Peak Hour	144.7	72.7
Max Day Plus Fire (7,000) L/min	127.8	48.7

¹Elevation: **94.640 m**

Notes:

1) As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:

- a) If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
- b) Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.
- 2) Both HGL and Pressures at Junction reflect the interm condition of watermain connections and not the ultimate condition of watermain connections which involves a 400 mm diameter main at Strandherd Dr. that will connect the network providing higher HGL at near by connections.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

APPENDIX B

Sanitary Servicing Calculation Sheet

LRL Associates Ltd. Storm Design Sheet

Average Daily Flow = 350 L/p/day Commercial & Institutional Flow = 50000 L/ha/day

Light Industrial Flow = 35000 L/ha/day

Heavy Industrial Flow = 55000 L/ha/day Maximum Residential Peak Factor = 4.0

Commercial & Institutional Peak Factor = 1.5

LRJ

LRL File No. 170364

Project: Public High School Barrhaven Centre
Location: 4005 Strandherd Drive, Barrhaven, Ottawa

Date: January 16, 2019
Designed: G. Brunet
Verified: J.C. Lalonde

Sanitary Design Parameters

Industrial Peak Factor = as per Appendix 4-B = 7

Extraneous Flow = 0.28 L/s/gross ha

Pipe Design Parameters

Minimum Velocity = 0.60 m/s Manning's n = 0.013

LC	CATION			RES	SIDENTIAL AI	REA AND PO	OPULATION		COMMER	CIAL	INDUSTR	IAL	INSTIT	UTIONAL	C+I+I		NFILTRATION					PIF	E			MANH	IOLE
OTDEET	50011111	TO 1411		POP.	CUMMU	JLATIVE	BEAU 5407	PEAK FLOW		ACCU. AREA	ACCU. AREA	PEAK		ACCU. F	U. PEAK FLOW	TOTAL AREA	TOTAL AREA ACCU. AREA	INFILT.	TOTAL FLOW (I/s)		DIA.	SLOPE	MATERA	CAP.	VEL.	UP	
STREET	FROM MH	TO MH	AREA (Ha)	POP.	AREA (Ha)	POP.	PEAK FACT.	(l/s)	AREA (Ha)	AREA (Ha) (Ha)	(Ha) (Ha) FACT.	AREA (Ha)	AREA (Ha)	(l/s)	(Ha)	(Ha)	FLOW (I/s)	(1/8)	LENGTH (m)	(mm)	(%)	IL	(FULL) (I/s)	(FULL) (m/s)	INVERT (m)	(m)	
SITE	PROP. BLDG	MH01	0.000	0.0	0.00	0.0	0.0	0.00	0.000	0.000	0.00	7.0	4.9	4.9	4.22	4.86	4.86	1.36	5.58	91.3	250	1.00%	PVC	59.47	1.21	91.43	90.52
SITE	MH01	STUB	0.000	0.0	0.0	0.0	4.0	0.00	0.000	0.000	0.00	7.0	0.0	4.9	4.22	0.00	4.86	1.36	5.58	26.8	250	1.00%	PVC	59.47	1.21	90.46	90.19
SITE	STUB	CITY	0.000	0.0	0.0	0.0	4.0	0.00	0.000	0.000	0.00	7.0	0.0	4.9	4.22	0.00	4.86	1.36	5.58	1.0	250	1.00%	PVC	59.47	1.21	90.19	90.18
																			(use 12.33 L/s)							1	
						NO	OTES						Designed:			PROJECT:											
					are to be confi	irmed on-site	9.						G.B.						Public High School Barrhaven Centre								
	Inverts provided by J.L. Richards & Associates Ltd									Checked:							LOCATIO	N:									
	* Use average flow rate of 12.33 L/s for design											J.C.L.					40	05 Standherd Dr	rive, Otta	awa							
												Dwg. Refer			File Ref.:				Date:				1	Shee	et No.		
													С	401 - Servicin	g Plan		170	0364		Se	eptembe	er-14-18		<u> </u>	1 0	of 1	

APPENDIX C

Stormwater Management Design Sheets

LRL Associates Ltd. Storm Design Sheet

Average Daily Flow = 350 L/p/day Commercial & Institutional Flow = 50000 L/ha/day

Light Industrial Flow = 35000 L/ha/day

Heavy Industrial Flow = 55000 L/ha/day Maximum Residential Peak Factor = 4.0

Commercial & Institutional Peak Factor = 1.5

LRJ

LRL File No. 170364

Project: Public High School Barrhaven Centre
Location: 4005 Strandherd Drive, Barrhaven, Ottawa

Date: January 16, 2019
Designed: G. Brunet
Verified: J.C. Lalonde

Sanitary Design Parameters

Industrial Peak Factor = as per Appendix 4-B = 7

Extraneous Flow = 0.28 L/s/gross ha

Pipe Design Parameters

Minimum Velocity = 0.60 m/s Manning's n = 0.013

LC	CATION			RES	SIDENTIAL AI	REA AND PO	OPULATION		COMMER	CIAL	INDUSTR	IAL	INSTIT	UTIONAL	C+I+I		NFILTRATION					PIF	E			MANH	IOLE
OTDEET	50011111	TO 1411		POP.	CUMMU	JLATIVE	BEAU 5407	PEAK FLOW		ACCU. AREA	ACCU. AREA	PEAK		ACCU. F	U. PEAK FLOW	TOTAL AREA	TOTAL AREA ACCU. AREA	INFILT.	TOTAL FLOW (I/s)		DIA.	SLOPE	MATERA	CAP.	VEL.	UP	
STREET	FROM MH	TO MH	AREA (Ha)	POP.	AREA (Ha)	POP.	PEAK FACT.	(l/s)	AREA (Ha)	AREA (Ha) (Ha)	(Ha) (Ha) FACT.	AREA (Ha)	AREA (Ha)	(l/s)	(Ha)	(Ha)	FLOW (I/s)	(1/8)	LENGTH (m)	(mm)	(%)	IL	(FULL) (I/s)	(FULL) (m/s)	INVERT (m)	(m)	
SITE	PROP. BLDG	MH01	0.000	0.0	0.00	0.0	0.0	0.00	0.000	0.000	0.00	7.0	4.9	4.9	4.22	4.86	4.86	1.36	5.58	91.3	250	1.00%	PVC	59.47	1.21	91.43	90.52
SITE	MH01	STUB	0.000	0.0	0.0	0.0	4.0	0.00	0.000	0.000	0.00	7.0	0.0	4.9	4.22	0.00	4.86	1.36	5.58	26.8	250	1.00%	PVC	59.47	1.21	90.46	90.19
SITE	STUB	CITY	0.000	0.0	0.0	0.0	4.0	0.00	0.000	0.000	0.00	7.0	0.0	4.9	4.22	0.00	4.86	1.36	5.58	1.0	250	1.00%	PVC	59.47	1.21	90.19	90.18
																			(use 12.33 L/s)							1	
						NO	OTES						Designed:			PROJECT:											
					are to be confi	irmed on-site	9.						G.B.						Public High School Barrhaven Centre								
	Inverts provided by J.L. Richards & Associates Ltd									Checked:							LOCATIO	N:									
	* Use average flow rate of 12.33 L/s for design											J.C.L.					40	05 Standherd Dr	rive, Otta	awa							
												Dwg. Refer			File Ref.:				Date:				1	Shee	et No.		
													С	401 - Servicing	g Plan		170	0364		Se	eptembe	er-14-18		<u> </u>	1 0	of 1	



LRL File No. 170364

Project: Public High School Barrhaven Centre
Location: 4005 Strandherd Drive, Barrhaven, Ottawa

Date: January 16, 2019

Designed:G. BrunetVerified:J.C. LalondeDrawing Reference:C701 and C702

Post-Devlopment Catchments

WATERSHED	C = 0.20	C = 0.80	C = 0.90	Total Area (ha)	Combined C
EWS-01	0.00	0.00	0.06	0.059	0.90
EWS-02	0.10	0.00	0.00	0.105	0.23
EWS-03	0.07	0.00	0.21	0.281	0.73
EWS-04	0.00	0.00	0.22	0.221	0.90
EWS-05	0.15	0.01	0.00	0.161	0.24
EWS-06	0.10	0.00	0.00	0.095	0.20
EWS-07	0.15	0.02	0.00	0.169	0.27
EWS-08	0.03	0.00	0.17	0.201	0.80
EWS-09	0.01	0.00	0.12	0.130	0.86
EWS-10	0.94	0.00	0.10	1.033	0.27
EWS-11	0.80	0.00	0.13	0.929	0.30
EWS-12	0.19	0.00	0.14	0.333	0.50
EWS-13	0.26	0.00	0.03	0.285	0.26
EWS-14	0.03	0.00	0.12	0.147	0.76
EWS-15 (ROOF)	0.00	0.00	0.71	0.711	0.90
TOTAL	2.819	0.030	2.011	4.860	0.49



LRL File No.

170364 Public High School Barrhaven Centre Project: Location: 4005 Strandherd Drive, Barrhaven, Ottawa

January 16, 2019 G. Brunet

Designed: J.C. Lalonde Checked: Drawing Ref.: C401

Stormwater Management Design Sheet

STORM - 5 YEAR

Runoff Equation

Q = 2.78CIA (L/s)

C = Runoff coefficient

I = Rainfall intensity (mm/hr)

 $= A / (Td + C)^B$

A = Area (ha)

 T_c = Time of concentration (min)

Pre-Devlopment Catchments within Development Area

Allowable Release Rate

5 Year Pre-Development Flow Rate

 $I_5 = 998.071 / (Td + 6.053)^{0.814}$

a = 998.071

C = 6.053

Allowable Release Rate= 884.00 L/s

As Per JL Richards, Harmony Stage 1 Report Dated July 2017

b = 0.814

Post-development Stormwater Management

					∑R₅	∑R ₁₀₀
	Total Site Area =	4.860	ha	∑R=	0.49	0.62
	WS-01	0.059	ha	R=	0.90	1.00
	WS-02	0.105	ha	R=	0.23	0.28
	WS-03	0.281	ha	R=	0.73	0.91
	WS-04	0.221	ha	R=	0.90	1.00
	WS-05	0.161	ha	R=	0.24	0.30
	WS-06	0.095	ha	R=	0.20	0.25
	WS-07	0.169	ha	R=	0.27	0.34
Controlled	WS-08	0.201	ha	R=	0.80	0.99
	WS-09	0.130	ha	R=	0.86	1.00
	WS-10	1.033	ha	R=	0.27	0.33
	WS-11	0.929	ha	R=	0.30	0.37
	WS-12	0.333	ha	R=	0.50	0.63
	WS-13	0.285	ha	R=	0.26	0.33
	WS-14	0.147	ha	R=	0.76	0.95
	Total Flow to Storm Stub =	4.149	ha	∑R=	0.42	0.53
Roof Top	WS-15 (Controlled Rooftop Area)	0.711	ha	R=	0.90	1.00
Koof Top	Total Un-Contolled =	0.711	ha	ΣR=	0.90	1.00

Post-development Stormwater Management

 $I_{100} = 1735.688 / (Td + 6.014)^{0.820}$

a = 1735.688

b = 0.82

C = 6.014

			Rooftop Storage	е		Overland Stor	age			
Time (min)	Intensity (mm/hr)	Controlled Runoff (L/s)	Storage Volume (m³)	Controlled Release Rate (L/s)	Controlled Runoff** (L/s)	Storage Volume (m³)	Controlled Release Rate (L/s)	Uncontrolled Runoff (L/s)	Total Release Rate (L/s)	Height on Roof (m)
10	178.6	352.94	180	52.89	1143.76	224	770.0	0.00	823	0.101
20	120.0	237.09	210	61.88	794.69	30	770.0	0.00	832	0.118
30	91.9	181.58	213	63.47	624.72	0	770.0	0.00	833	0.120
40	75.1	148.53	205	62.94	522.02	0	770.0	0.00	833	0.116
50	64.0	126.41	195	61.35	452.07	0	770.0	0.00	831	0.110
60	55.9	110.48	188	58.18	399.65	0	770.0	0.00	828	0.106
70	49.8	98.41	180	55.53	359.71	0	770.0	0.00	826	0.101
80	45.0	88.93	173	52.89	327.75	0	770.0	0.00	823	0.097
90	41.1	81.26	167	50.24	301.40	0	770.0	0.00	820	0.094
100	37.9	74.92	164	47.60	279.16	0	770.0	0.00	818	0.092
110	35.2	69.58	152	46.54	261.60	0	770.0	0.00	817	0.086
120	32.9	65.02	148	44.43	245.39	0	770.0	0.00	814	0.083
130	30.9	61.07	171	39.14	227.90	0	770.0	0.00	809	0.096

Infiltration Gallery - 280m

Granular Storage

Total Available Storage =

 m^3 Pipe Storage 13.74 ${\sf m}^3$

166.6

180.31 m³

40% Void refer to Drawing C401 for detail Rooftop Controls

Control-Flo Roof Drain Rate = 136 L/min Max HWL = 0.150 Control-Flo Roof Drain Rate = 15.11 # of roof drains = 35 m^3 212.6

Max Roof Storage = Height = 0.120 Max Roof Rate =

Onsite Stormwater Retention

224.25 m³ Total Storage Required = 212.61 m³ Rooftop Ponding =

104.88 m³ Pipe Storage = refer to Storm Sewer Design Sheet 16.55 m³ CB/MH Storage = refer to Storm Sewer Design Sheet Infiltration Gallery = 180.00 m³ refer to Drawing C401

301.42 m³ Total Available Storage =



LRL File No. 170364

Project: Public High School Barrhaven Centre Location: 4005 Strandherd Drive, Barrhaven, Ottawa

January 16, 2019 Date:

G. Brunet Designed: Checked: J.C. Lalonde Drawing Ref.: C401

Stormwater Management Design Sheet

STORM - 5 YEAR

Runoff Equation

Q = 2.78CIA (L/s)

C = Runoff coefficient

I = Rainfall intensity (mm/hr)

 $= A / (Td + C)^B$

A = Area (ha)

 T_c = Time of concentration (min)

Pre-Devlopment Catchments within Development Area

Allowable Release Rate

5 Year Pre-Development Flow Rate

 $I_5 = 998.071 / (Td + 6.053)^{0.814}$

a = 998.071

b = 0.814

C = 6.053

Allowable Release Rate= 884.00 L/s

As Per JL Richards, Harmony Stage 1 Report Dated July 2017

Post-development Stormwater Management

					∑R₅	ΣR ₁₀₀
	Total Site Area =	4.860	ha	∑R=	0.49	0.62
	WS-01	0.059	ha	R=	0.90	1.00
	WS-02	0.105	ha	R=	0.23	0.28
	WS-03	0.281	ha	R=	0.73	0.91
	WS-04	0.221	ha	R=	0.90	1.00
	WS-05	0.161	ha	R=	0.24	0.30
	WS-06	0.095	ha	R=	0.20	0.25
	WS-07	0.169	ha	R=	0.27	0.34
Controlled	WS-08	0.201	ha	R=	0.80	0.99
	WS-09	0.130	ha	R=	0.86	1.00
	WS-10	1.033	ha	R=	0.27	0.33
	WS-11	0.929	ha	R=	0.30	0.37
	WS-12	0.333	ha	R=	0.50	0.63
	WS-13	0.285	ha	R=	0.26	0.33
	WS-14	0.147	ha	R=	0.76	0.95
	Total Flow to Storm Stub =	4.149	ha	∑R=	0.42	0.53
Poof Ton	WS-15 (Controlled Rooftop Area)	0.711	ha	R=	0.90	1.00
Roof Top	Total Un-Contolled =	0.711	ha	∑R=	0.90	1.00

5 Year Stormwater Management Calculations

 $I_5 = 998.071 / (Td + 6.053)^{0.814}$

a = 998.071

b = 0.814

C = 6.053

I /min

			Rooftop Storag	е		Overland Stora	ge			
Time (min)	Intensity (mm/hr)	Controlled Runoff (L/s)	Storage Volume (m³)	Controlled Release Rate (L/s)	Controlled Runoff** (L/s)	Storage Volume (m³)	Controlled Release Rate (L/s)	Uncontrolled Runoff (L/s)	Total Release Rate (L/s)	Height on Roof (m)
10	104.2	185.35	95	27.14	536.38	0	770.00	0.00	797.14	0.053
20	70.3	124.97	111	32.36	375.71	0	770.00	0.00	802.36	0.063
30	53.9	95.93	113	32.88	296.45	0	770.00	0.00	802.88	0.064
40	44.2	78.60	111	32.36	248.31	0	770.00	0.00	802.36	0.062
50	37.7	66.98	107	31.32	215.34	0	770.00	0.00	801.32	0.060
60	32.9	58.60	102	30.27	191.28	0	770.00	0.00	800.27	0.057
70	29.4	52.25	97	29.23	172.78	0	770.00	0.00	799.23	0.054
80	26.6	47.25	94	27.66	157.48	0	770.00	0.00	797.66	0.053
90	24.3	43.21	90	26.62	145.33	0	770.00	0.00	796.62	0.050
100	22.4	39.86	86	25.57	135.09	0	770.00	0.00	795.57	0.048
110	20.8	37.04	83	24.53	126.30	0	770.00	0.00	794.53	0.046
120	19.5	34.63	80	23.49	118.63	0	770.00	0.00	793.49	0.045
130	18.3	32.54	79	22.44	111.86	0	770.00	0.00	792.44	0.044
260	10.6	18.85	0	19.83	71.63	0	770.00	0.00	789.83	0.000

Onsite Stormwater Retention Rooftop Controls Control-Flo Roof Drain Rate =

			Control i lo recoi Brain reate =	100	L/111111
Total Storage Required =	0.00 m ³		Max HWL =	0.152	m
Rooftop Ponding =	113.49 m ³		Control-Flo Roof Drain Rate =	14.91	L/s⋅m
Pipe Storage =	104.88 m ³	refer to Storm Sewer Design Sheet	# of roof drains =	35	
CB/MH Storage =	16.55 m ³	refer to Storm Sewer Design Sheet	Max Roof Storage =	113.5	m^3
Infiltration Gallery =	180.00 m ³	refer to Drawing C401	Height =	0.064	m
Total Available Storage =	301.42 m ³		Max Roof Rate =	32.88	L/s

APPENDIX D
Supporting Documents



HARMONY STAGE 1 4005 STRANDHERD DRIVE CITY OF OTTAWA MINTO COMMUNITIES INC. JLR NO. 24051-01

STORM SEWER DESIGN SHEET

Designed by: J.W.
Checked by: K.F.

Date: July 2017

Manning's Coefficient, n = 0.013 IDF CURVE = 2 Year

Denotes Existing Storm Sewer (Mattamy Barrhaven Mews) - Per IBI As-Constructed Drawings dated February 2010 PLUG FLOW Flow Time Peak ICD Flow Cum. Area 5 Peak Flow Cum. 2.78AR STREET 0.53 0.75 Q full V full 0.72 2.78AR Drop 0.30 Area 2 YR (ha) Intensity Intensity Flow Length 2 Yr 5 Yr MH120 235.0 0.85 94 24 Commercial (Block 105) Stub 198 13 248.09 102.01 306.2 10.42 Chapman Mills Drive MH120 MH121 2.16 306.28 135.0 339.63 0.92 0.25 1424.40 1.59 13.8 368.6 Future Institutional Block Stub MH121 0.00 4.86 10.00 0.14 10.13 10.13 104.19 1055. 0 1055.80 884.0 94.41 92.001 90.934 2.41 94.32 0.00 91.967 90.900 2.35 Chapman Mills Drive 0.20 76.10 15.23 MH125 0.20 16.0 MH123 10.19 0.88 0.71 103.22 73.46 88.69 108.21 0.95 19.5 Clarity (Street No. 5) MH121 MH115 0.44 6.68 12.58 1.22 0.68 0.88 68.13 14.23 92.28 1312.88 1372.88 54.0 2156.55 1.46 783.7 94.32 91.967 MH115 132.95 84.0 Park (Block 117) 1/18 72 0.83 13.80 0.88 1.44 15.50 87.65 1358.87 1509.12 94.22 91.807 90.435 Clarity (Street No. 5) MH115 1.27 8.21 2.32 64.76 150.24 66.0 2156.55 1.46 77.3 647.4 2.41 94.10 -0.08 91.691 MH118 MH117 10.00 1.49 MH117 1.03 79 19 62.0 286.47 0.98 52.0 Easement (Block 110) MH116 MH115A 12 70 116.03 116.03 339.63 0.92 MH113 0.46 14.68 0.43 0.68 4.71 62.53 15.50 84.60 1311.58 1606.07 2856.14 1.57 40.4 1250.1 Clarity (Street No. 5) 286.47 0.98 96.9 209.5 MH114 MH113 0.53 0.00 10.00 1.65 1.00 1.00 76.81 77.02 77.02 94.17 91.603 90.994 Namaste (Street No. 4) 0.53 66.0 83.20 1670.76 Clarity (Street No. 5) 19.99 18.0 Waterlilly Way
 0.48
 0.00
 11.65
 0.96
 0.72
 0.98
 71.01
 69.65

 0.70
 0.00
 12.60
 1.27
 0.44
 1.42
 68.07
 96.74
 69.65 94.77 92.110 91.805 95.00 91.508 91.051 34.0
 0.00
 0.00
 10.00
 1.30

 0.80
 0.00
 11.30
 1.16
 1.46
 0.00 105.23 1.46 72.14 105.23 4.90 8.21 15.95 0.34 5.13 8.21 16.29 0.47 0.46 15.50 80.61 1249.67 15.50 79.62 1234.34 0.0 34.0 2856.14 1.57 31.6 1065.6 2856.14 1.57 44.0 1060.4 Clarity (Street No. 5) Clarity (Street No. 5) 9.07 59.61 9.53 58.89 0.23 MH108 MH107
 0.33
 0.00
 10.00
 1.30
 0.49
 0.49
 76.81
 37.34

 0.89
 0.00
 11.30
 1.33
 1.12
 1.61
 72.14
 115.94
 37.34 115.94
 0.25
 91.46
 0.80
 62.7
 54.1

 0.16
 179.46
 0.80
 64.3
 63.5
 MH106 90.0 Octave (Street No. 2) MH106 MH102 0.64 6.66 8.21 16.75 0.73 1.07 12.21 57.91 707.11 15.50 78.29 1213.69 1920.81 36.0 1585 2495 0.15 5749.47 1.87 82.2 3828.7 93.94 91.513 89.913 2.43 93.86 0.00 91.390 89.790 2.47 Clarity (Street No. 5) 57.95 0.80 89.05 0.80 96.96 0.85 148.72 0.91 76.19 51.77 71.27 48.43 66.49 70.46 96.96 Waterlilly Way Hamsa (Street No. 48.43 0.38 28.0 MH102 70.46 15.50 76.29 1182.80 17.49 0.33 1956.79 32.0 3792.7 Hamsa (Steet No. 1) 7.40 8.21 13.71 Hamsa (Street No. 1) MH103 74.65 75.44 1169.60 74.61 1156.77 5749.47 1.87 5749.47 1.87 3760.1 3781.9 93.37 91.716 89.735 93.50 91.661 89.680 1.65 1.84 93.50 0.00 91.661 89.680 93.50 0.00 91.621 89.640 MH103 MH104 17.82 819.72 1989.32 0.0 MH104 18.14 810.82 1967.59



HARMONY STAGE 1
4005 STRANDHERD DRIVE
CITY OF OTTAWA
MINTO COMMUNITIES INC.
JLR NO. 24051-001

SANITARY SEWER DESIGN SHEET

Designed by: J.W.

Checked by: K.F.

Date : May 2017

Γ	Single Family	3.4	pers/unit	q =	350	L/cap/day
	Semi-Detached/Townhouse (row)	2.7	pers/unit	l =	0.280	L/s/ha
				Inst. =	50000	L/ha/day
	Manning's Coeff. N =	0.013	1			

Denotes Existing Sanitary Sewer (South Nepean Collector) - In accordance with City of Ottawa Drawings Contract No. ISD14-2033

Denotes Existing Sanitary Sewer (Mattamy Barrhaven Mews) - Per IBI As-Constructed Drawings dated February 2010

Denotes Future External Lands (Refer to South Nepean Collector - Phase 2 & 3 Sanitary Sewer Design Sheet prepared by Novatech, dated August 2015)

	RESIDENTIAL INSTITUTIONAL/COMMERCIAL																												
	T		NU	IMBER OF U		_	LATIVE	PEAKING	POPUL.	iii to i i i o	CUMM.	INST.	PEAK EXTR.	PEAK DES.	1	SEWE	R DATA			RESIDUAL		UPST	REAM			D	OWNSTRE/	AM	
STREET	M.I	1. #	SING.	MULT.	AREA	POPUL.	AREA	FACTOR	FLOW	AREA	AREA	FLOW	FLOW	FLOW	DIA.	SLOPE	CAPAC.	VEL.	LENGTH	CAP.	Center	Obvert	Invert	Cover	Center	Obvert	Obvert	Invert	Cover
	FROM	TO			ha	peop.	ha		l/s	ha	ha	l/s	l/s	l/s	mm	%	l/s	m/s	m	l/s	Line				Line	Drop			
Hamsa Street (Street No. 1)	MH1	MH2	10	0	0.48	34	0.48	4.00	0.55	0.00	0.00	0.00	0.13	0.69	200	0.35	20.2	0.62	64.5	19.56	94.06	92.191	91.988	1.87	93.86	1.00	91.965	91.762	1.89
Hamsa Street (Street No. 1)	MH5	MH3	15	0	0.69	51	0.69	4.00	0.83	0.00	0.00	0.00	0.19	1.02	200	0.35	20.2	0.62	88.9	19.22	93.88	91.462	91.259	2.42	93.47		91.151	90.947	2.32
Hamsa Street (Street No. 1)	MH3	MH2	5	0	0.26	68	0.95	4.00	1.10	0.00	0.00	0.00	0.27	1.37	200	0.35	20.2	0.62	37.0	18.87	93.47	91.151	90.947	2.32	93.86	0.06	91.021	90.818	2.84
Clarity Street (Street No. 5)	MH2	МН6	4	0	0.25	116	1.68	4.00	1.87	0.00	0.00	0.00	0.47	2.34	200	0.35	20.2	0.62	76.1	17.90	93.86	90.961	90.758	2.90	93.94		90.695	90.491	3.25
Octave Street (Street No. 2)	MH8	MH7	12	0	0.51	41	0.51	4.00	0.66	0.00	0.00	0.00	0.14	0.80	200	0.35	20.2	0.62	62.6	19.44	94.07	91.194	90.991	2.88	93.77		90.975	90.772	2.79
Octave Street (Street No. 2)	MH7	MH6	10	0	0.41	75	0.92	4.00	1.21	0.00	0.00	0.00	0.26	1.47	200	0.35	20.2	0.62	63.1	18.77	93.77	90.975	90.772	2.79	93.94	0.06	90.755	90.551	3.19
Clarity Street (Street No. 5)	мн6	MH10A	4	0	0.24	204	2.84	4.00	3.31	0.00	0.00	0.00	0.80	4.10	200	0.35	20.2	0.62	51.0	16.14	93.94	90.695	90.491	3.25	93.76		90.516	90.313	3.24
Clarity Street (Street No. 5)	MH10A	MH10	0	0	0.00	204	2.84	4.00	3.31	0.00	0.00	0.00	0.80	4.10	200	0.35	20.2	0.62	25.4	16.14	93.76	90.516	90.313	3.24	94.02		90.427	90.224	3.59
Lilith Street (Street No. 3)	MH9	MH10	0	14	0.46	38	0.46	4.00	0.61	0.00	0.00	0.00	0.13	0.74	200	0.35	20.2	0.62	65.5	19.50	95.00	92.343	92.139	2.66	94.02	1.69	92.113	91.910	1.91
Lilith Street (Street No. 3)	MH12	MH11	10	0	0.46	34	0.46	4.00	0.55	0.00	0.00	0.00	0.13	0.68	200	0.35	20.2	0.62	62.8	19.56	94.15	90.919	90.716	3.23	93.84		90.699	90.496	3.14
Lilith Street (Street No. 3)	MH11	MH10	9	0	0.41	65	0.87	4.00	1.05	0.00	0.00	0.00	0.24	1.29	200	0.35	20.2	0.62	60.5	18.95	93.84	90.699	90.496	3.14	94.02	0.06	90.487	90.284	3.53
Clarity Street (Street No. 5)	MH10	MH13	0	6	0.27	323	4.44	4.00	5.23	0.00	0.00	0.00	1.24	6.47	200	0.35	20.2	0.62	78.5	13.77	94.02	90.427	90.224	3.59	94.10		90.153	89.949	3.95
Namaste (Street No. 4)	MH14	MH13	14	0	0.69	48	0.69	4.00	0.77	0.00	0.00	0.00	0.19	0.96	200	0.35	20.2	0.62	97.1	19.28	94.20	90.552	90.349	3.65	94.10	0.06	90.213	90.009	3.89
Clarity Street (Street No. 5)	MH13	MH15	0	17	0.59	416	5.72	4.00	6.74	0.00	0.00	0.00	1.60	8.34	200	0.35	20.2	0.62	117.4	11.90	94.10	90.153	89.949	3.95	94.22	-0.05	89.742	89.538	4.48
Park (Block 117)	MH16	MH15	0	0	1.67	0	1.67	4.00	0.00	0.00	0.00	0.00	0.47	0.47	150	0.50	11.2	0.62	12.9	10.77	94.32	90.357	90.205	3.96	94.22	0.50	90.293	90.140	3.93
Clarity Street (Street No. 5)	MH15	EX. CM 4	0	12	0.46	449	7.85	4.00	7.26	0.00	0.00	0.00	2.20	9.46	300	2.00	142.7	1.96	106.9	133.21	94.22	89.793	89.488	4.43	94.32	0.01	87.655	87.350	6.66
Future Institutional (Block 127)	STUB	EX. CM 4	0	0	0.00	0	0.00	4.00	0.00	4.86 *	4.86	4.22	1.36	5.58	250	1.00	62.0	1.22	15.7	56.46	94.41	90.431	90.177	3.98	94.32	2.63	90.274	90.020	4.05
Upstream Mixed Use Development	-	EX. CM 1			105.84	10974	105.84	2.91	129.56	175.19	175.19	152.07	78.69	360.33															
Chapman Mills Drive	EX. CM 1	EX. CM 2	0	0	0.16	10974	106.00	2.91	129.56	0.00	175.19	152.07	78.73	360.37	900	0.10	597.2	0.91	60.9	236.85	95.03	87.833	86.918	7.20	94.43		87.772	86.857	6.66
Future Commercial (Block 105)	STUB	EX. CM 2	0	0	0.00	0	0.00	4.00	0.00	0.76	0.76	0.66	0.21	0.87	250	1.00	62.0	1.22	15.0	61.17	94.53	90.624	90.370	3.91	94.43	2.70	90.474	90.220	3.96
Chapman Mills Drive	EX. CM 2	EX. CM 3	0	0	0.16	10974	106.16	2.91	129.56	0.00	175.95	152.73	78.99	361.29	900	0.10	597.2	0.91	47.4	235.94	94.43	87.772	86.857	6.66	94.41		87.724	86.810	6.69
Waterlilly Way	EX. 22A	17	0	22	0.56	59	0.56	4.00	0.96	0.00	0.00	0.00	0.16	1.12	200	1.00	34.2	1.06	84.2	33.10	94.59	92.702	92.499	1.89	94.40	0.55	91.860	91.657	2.54
Waterlilly Way	17	EX. CM 3	5	17	0.70	122	1.26	4.00	1.98	0.00	0.00	0.00	0.35	2.33	250	1.00	62.0	1.22	105.6	59.70	94.40	91.310	91.056	3.09	94.41	2.53	90.254	90.000	4.16
Chapman Mills Drive	EX. CM 3	EX. CM 4	0	0	0.21	11096	107.63	2.91	130.79	0.00	175.95	152.73	79.40	362.93	900	0.10	597.2	0.91	76.1	234.30	94.41	87.724	86.810	6.69	94.32		87.648	86.734	6.67
Chapman Mills Drive	EX. CM 4	EX. CM 5	0	0	0.09	11545	115.57	2.89	135.27	0.00	180.81	156.95	82.99	375.21	900	0.10	597.2	0.91	32.0	222.01	94.32	87.648	86.734	6.67	94.20		87.616	86.702	6.58
Chapman Mills Drive	EX. CM 5	EX. CM 6	0	0	0.03	11545	115.68	2.89	135.27	0.00	180.81	156.95	83.02	375.24	900	0.10	597.2	0.91	42.3	221.98	94.20	87.616	86.702	6.58	94.30		87.574	86.660	6.73
·																													
High Density Residential (Block 118)	STUB	18	0	24	0.32	65	0.32	4.00	1.05	0.00	0.00	0.00	0.09	1.14	200	0.35	20.2	0.62	9.0	19.10	94.39	90.090	89.887	4.30	94.27	0.00	90.059	89.855	4.21
Chakra Street (Street No. 6)	18	EX. CM 6	0	0	0.10	65	0.42	4.00	1.05	0.00	0.00	0.00	0.12	1.17	300	4.00	201.8	2.77	46.8	200.60	94.27	90.059	89.754	4.21	94.30	0.61	88.185	87.880	6.11
Chapman Mills Drive	EX. CM 6	EX. CM 7	0	0	0.21	11610	116.31	2.89	135.92	0.00	180.81	156.95	83.19	376.06	900	0.10	597.2	0.91	74.0	221.16	94.30	87.574	86.660	6.73	93.40	0.01	87.500	86.586	5.90
Future Residential Development	STUB	EX. CM 7	0	0	8.83 *	1430 **	8.83	3.69	21.41	5.35	5.35	4.64	3.97	30.02	250	0.25	31.0	0.61	12.0	1.00	93.61	87.434	87.180	6.18	93.40	-0.09	87.404	87.150	6.00
				1		•												ī	1			i e				l	1		5.00
Chapman Mills Drive	EX. CM 7	EX. CM 8	0	0	0.00	13040	125.14	2.84	149.99	0.00	186.16	161.59	87.16	398.75	900	0.10	597.2	0.91	25.7	198.48	93.40	87.490	86.576	5.91	93.38		87.464	86.550	5.92
Chapman Mills Drive	EX. CM 7	EX. CM 8	0	0	0.00	13040	125.14	2.84	149.99	0.00	186.16	161.59	87.16	398.75	900	0.10	597.2	0.91	25.7	198.48	93.40	87.490	86.576	5.91	93.38		87.464 INV =	86.550 86.550	5.92

Notes:

institutional, Commercial and Residential Area taken from Draft Plan of Subdivision prepared by Annis, O'Sullivan, Vollebekk Ltd. (Residential Area inclusive of Future Transit Corridor (2.279 Ha) and Servicing Easement (0.112 Ha))

** Cumulative Population based on Medium Density Residential Land Use (162 pers/ha) as per Novatech's South Nepean Collector - Phase 2 & 3 Sanitary Sewer Design Sheet dated August 2015



HARMONY STAGE 1
4005 STRANDHERD DRIVE
CITY OF OTTAWA
MINTO COMMUNITIES INC.
JLR NO. 24051-001

SANITARY SEWER DESIGN SHEET

Designed by: J.W.

Checked by: K.F.

Date : May 2017

Single Family	3.4	pers/unit	q =	350	L/cap/day
Semi-Detached/Townhouse (row)	2.7	pers/unit	I =	0.280	L/s/ha
			Inst. =	50000	L/ha/day
Manning's Coeff. N =	0.01	3			

Denotes Existing Sanitary Sewer (South Nepean Collector) - In accordance with City of Ottawa Drawings Contract No. ISD14-2033

Denotes Existing Sanitary Sewer (Mattamy Barrhaven Mews) - Per IBI As-Constructed Drawings dated February 2010

Denotes Future External Lands (Refer to South Nepean Collector - Phase 2 & 3 Sanitary Sewer Design Sheet prepared by Novatech, dated August 2015)

						RESIDENTIA				INSTITU	ITIONAL/COM	IMERCIAL																	
	м	H. #	NU	JMBER OF U	NITS	CUMU	JLATIVE	PEAKING	POPUL.		CUMM.	INST.	PEAK EXTR.	PEAK DES.		SEWE	R DATA			RESIDUAL		UPST	ΓREAM			D	OWNSTREA	MA	
STREET			SING.	MULT.	AREA	POPUL.	AREA	FACTOR	FLOW	AREA	AREA	FLOW	FLOW	FLOW	DIA.	SLOPE	CAPAC.	VEL.	LENGTH	CAP.	Center	Obvert	Invert	Cover	Center	Obvert	Obvert	Invert	Cover
	FROM	то			na	peop.	ha		l/s	ha	ha	l/s	l/s	l/s	mm	%	I/s	m/s	m	l/s	Line				Line	Drop			
Hamsa Street (Street No. 1)	MH1	MH2	10	0	0.48	34	0.48	4.00	0.55	0.00	0.00	0.00	0.13	0.69	200	0.35	20.2	0.62	64.5	19.56	94.06	92.191	91.988	1.87	93.86	1.00	91.965	91.762	1.89
riamod outot (outot rio. 1)														1.00															
Hamsa Street (Street No. 1)	MH5	MH3	15	0	0.69	51	0.69	4.00	0.83	0.00	0.00	0.00	0.19	1.02	200	0.35	20.2	0.62	88.9	19.22	93.88	91.462	91.259	2.42	93.47		91.151	90.947	2.32
Hamsa Street (Street No. 1)	MH3	MH2	5	0	0.26	68	0.95	4.00	1.10	0.00	0.00	0.00	0.27	1.37	200	0.35	20.2	0.62	37.0	18.87	93.47	91.151	90.947	2.32	93.86	0.06	91.021	90.818	2.84
					2.05	440	4.00		4.0=											47.00	22.22		00 750		20.04		00.005	20.404	
Clarity Street (Street No. 5)	MH2	MH6	4	0	0.25	116	1.68	4.00	1.87	0.00	0.00	0.00	0.47	2.34	200	0.35	20.2	0.62	76.1	17.90	93.86	90.961	90.758	2.90	93.94		90.695	90.491	3.25
Octave Street (Street No. 2)	MH8	MH7	12	0	0.51	41	0.51	4.00	0.66	0.00	0.00	0.00	0.14	0.80	200	0.35	20.2	0.62	62.6	19.44	94.07	91.194	90.991	2.88	93.77		90.975	90.772	2.79
Octave Street (Street No. 2)	MH7	MH6	10	0	0.41	75	0.92	4.00	1.21	0.00	0.00	0.00	0.26	1.47	200	0.35	20.2	0.62	63.1	18.77	93.77	90.975	90.772	2.79	93.94	0.06	90.755	90.551	3.19
Clarity Street (Street No. 5)	MH6	MH10A	4	0	0.24	204	2.84	4.00	3.31	0.00	0.00	0.00	0.80	4.10	200	0.35	20.2	0.62	51.0	16.14	93.94	90.695	90.491	3.25	93.76		90.516	90.313	3.24
Clarity Street (Street No. 5)	MH10A	MH10	0	0	0.00	204	2.84	4.00	3.31	0.00	0.00	0.00	0.80	4.10	200	0.35	20.2	0.62	25.4	16.14	93.76	90.516	90.313	3.24	94.02		90.427	90.224	3.59
Lilith Street (Street No. 3)	MH9	MH10	0	14	0.46	38	0.46	4.00	0.61	0.00	0.00	0.00	0.13	0.74	200	0.35	20.2	0.62	65.5	19.50	95.00	92.343	92.139	2.66	94.02	1.69	92.113	91.910	1.91
Emili Greet (Greet No. 5)	1411 15	WIITIO			0.10		0.10		0.01	0.00	0.00	0.00	0.10	5.7 1	200	0.00	20.2	0.02	00.0	10.00	00.00	02.010	02.100	2.00	01.02	1.00	02.110	01.010	
Lilith Street (Street No. 3)	MH12	MH11	10	0	0.46	34	0.46	4.00	0.55	0.00	0.00	0.00	0.13	0.68	200	0.35	20.2	0.62	62.8	19.56	94.15	90.919	90.716	3.23	93.84		90.699	90.496	3.14
Lilith Street (Street No. 3)	MH11	MH10	9	0	0.41	65	0.87	4.00	1.05	0.00	0.00	0.00	0.24	1.29	200	0.35	20.2	0.62	60.5	18.95	93.84	90.699	90.496	3.14	94.02	0.06	90.487	90.284	3.53
																													
Clarity Street (Street No. 5)	MH10	MH13	0	6	0.27	323	4.44	4.00	5.23	0.00	0.00	0.00	1.24	6.47	200	0.35	20.2	0.62	78.5	13.77	94.02	90.427	90.224	3.59	94.10		90.153	89.949	3.95
Namaste (Street No. 4)	MH14	MH13	14	0	0.69	48	0.69	4.00	0.77	0.00	0.00	0.00	0.19	0.96	200	0.35	20.2	0.62	97.1	19.28	94.20	90.552	90.349	3.65	94.10	0.06	90.213	90.009	3.89
Hamado (oliocitio, 1)									****				-																
Clarity Street (Street No. 5)	MH13	MH15	0	17	0.59	416	5.72	4.00	6.74	0.00	0.00	0.00	1.60	8.34	200	0.35	20.2	0.62	117.4	11.90	94.10	90.153	89.949	3.95	94.22	-0.05	89.742	89.538	4.48
																													
Park (Block 117)	MH16	MH15	0	0	1.67	0	1.67	4.00	0.00	0.00	0.00	0.00	0.47	0.47	150	0.50	11.2	0.62	12.9	10.77	94.32	90.357	90.205	3.96	94.22	0.50	90.293	90.140	3.93
Clarity Street (Street No. 5)	MH15	EX. CM 4	0	12	0.46	449	7.85	4.00	7.26	0.00	0.00	0.00	2.20	9.46	300	2.00	142.7	1.96	106.9	133.21	94.22	89.793	89.488	4.43	94.32	0.01	87.655	87.350	6.66
Clarity Caroot (Caroot No. 0)		2,4, 0,					1																						
Future Institutional (Block 127)	STUB	EX. CM 4	0	0	0.00	0	0.00	4.00	0.00	4.86 *	4.86	4.22	1.36	5.58	250	1.00	62.0	1.22	15.7	56.46	94.41	90.431	90.177	3.98	94.32	2.63	90.274	90.020	4.05
																													—
Upstream Mixed Use Development		EX. CM 1			105.84	10974	105.84	2.91	129.56	175.19	175.19	152.07	78.69	360.33															\vdash
Chapman Mills Drive	EX. CM 1	EX. CM 2	0	0	0.16	10974	106.00	2.91	129.56	0.00	175.19	152.07	78.73	360.37	900	0.10	597.2	0.91	60.9	236.85	95.03	87.833	86.918	7.20	94.43		87.772	86.857	6.66
Chapman Nine Bhite	2/4 0111 1	274 0111 2			0.10				120.00	0.00						-		0.01			-	0.1000		1.20					
Future Commercial (Block 105)	STUB	EX. CM 2	0	0	0.00	0	0.00	4.00	0.00	0.76	0.76	0.66	0.21	0.87	250	1.00	62.0	1.22	15.0	61.17	94.53	90.624	90.370	3.91	94.43	2.70	90.474	90.220	3.96
																													<u> </u>
Chapman Mills Drive	EX. CM 2	EX. CM 3	0	0	0.16	10974	106.16	2.91	129.56	0.00	175.95	152.73	78.99	361.29	900	0.10	597.2	0.91	47.4	235.94	94.43	87.772	86.857	6.66	94.41		87.724	86.810	6.69
Waterlilly Way	EX. 22A	17	0	22	0.56	59	0.56	4.00	0.96	0.00	0.00	0.00	0.16	1.12	200	1.00	34.2	1.06	84.2	33.10	94.59	92.702	92.499	1.89	94.40	0.55	91.860	91.657	2.54
Waterlilly Way	17	EX. CM 3	5	17	0.70	122	1.26	4.00	1.98	0.00	0.00	0.00	0.35	2.33	250	1.00	62.0	1.22	105.6	59.70	94.40	91.310	91.056	3.09	94.41	2.53	90.254	90.000	4.16
Chapman Mills Drive	EX. CM 3	EX. CM 4	0	0	0.21	11096	107.63	2.91	130.79	0.00	175.95	152.73	79.40	362.93	900	0.10	597.2	0.91	76.1	234.30	94.41	87.724	86.810	6.69	94.32		87.648	86.734	6.67
Ol Mail D	5V 0144	EV 014 5	0	0	0.00	44545	445.57	0.00	405.07	0.00	400.04	450.05	00.00	075.04	000	0.40	507.0	0.04	20.0	000.04	04.00	87.648	00.704	6.67	04.00		87.616	00.700	0.50
Chapman Mills Drive Chapman Mills Drive	EX. CM 4 EX. CM 5	EX. CM 5 EX. CM 6	0	0	0.09	11545 11545	115.57 115.68	2.89 2.89	135.27 135.27	0.00	180.81 180.81	156.95 156.95	82.99 83.02	375.21 375.24	900	0.10	597.2 597.2	0.91	32.0 42.3	222.01 221.98	94.32 94.20	87.648	86.734 86.702	6.58	94.20 94.30		87.574	86.702 86.660	6.58 6.73
Спартнат мініз Бліче	EX. CIVI 5	EX. CIVI 0	Ü		0.11	11040	110.00	2.00	100.27	0.00	100.01	100.00	00.02	07 U.Z4	300	0.10	001.2	0.51	42.0	221.00	34.20	07.010	00.702	0.00	54.50		01.014	00.000	0.70
High Density Residential (Block 118)	STUB	18	0	24	0.32	65	0.32	4.00	1.05	0.00	0.00	0.00	0.09	1.14	200	0.35	20.2	0.62	9.0	19.10	94.39	90.090	89.887	4.30	94.27	0.00	90.059	89.855	4.21
Chakra Street (Street No. 6)	18	EX. CM 6	0	0	0.10	65	0.42	4.00	1.05	0.00	0.00	0.00	0.12	1.17	300	4.00	201.8	2.77	46.8	200.60	94.27	90.059	89.754	4.21	94.30	0.61	88.185	87.880	6.11
0 1 1 1 1	EV 211	EV 2000	^	_	0.04	44040	110.01	2.00	105.00	0.00	100.04	150.05	02.40	276.00	000	0.40	E07.0	0.04	74.0	224.40	04.00	07.574	00.000	6.70	02.40	0.04	07.500	06 500	F 00
Chapman Mills Drive	EX. CM 6	EX. CM 7	0	0	0.21	11610	116.31	2.89	135.92	0.00	180.81	156.95	83.19	376.06	900	0.10	597.2	0.91	74.0	221.16	94.30	87.574	86.660	6.73	93.40	0.01	87.500	86.586	5.90
Future Residential Development	STUB	EX. CM 7	0	0	8.83 *	1430 **	8.83	3.69	21.41	5.35	5.35	4.64	3.97	30.02	250	0.25	31.0	0.61	12.0	1.00	93.61	87.434	87.180	6.18	93.40	-0.09	87.404	87.150	6.00
. Laro Rootsoniai Dovolopinoit	3105	LAN OW 7				1						-					1							1			1		
Chapman Mills Drive	EX. CM 7	EX. CM 8	0	0	0.00	13040	125.14	2.84	149.99	0.00	186.16	161.59	87.16	398.75	900	0.10	597.2	0.91	25.7	198.48	93.40	87.490	86.576	5.91	93.38		87.464	86.550	5.92
																											INV =	86.550	
	1	I	I	1	1	1	1		1	1	1	1	1	1	I		1	1	1	I	I	1	1	1	1	1	1	1	(

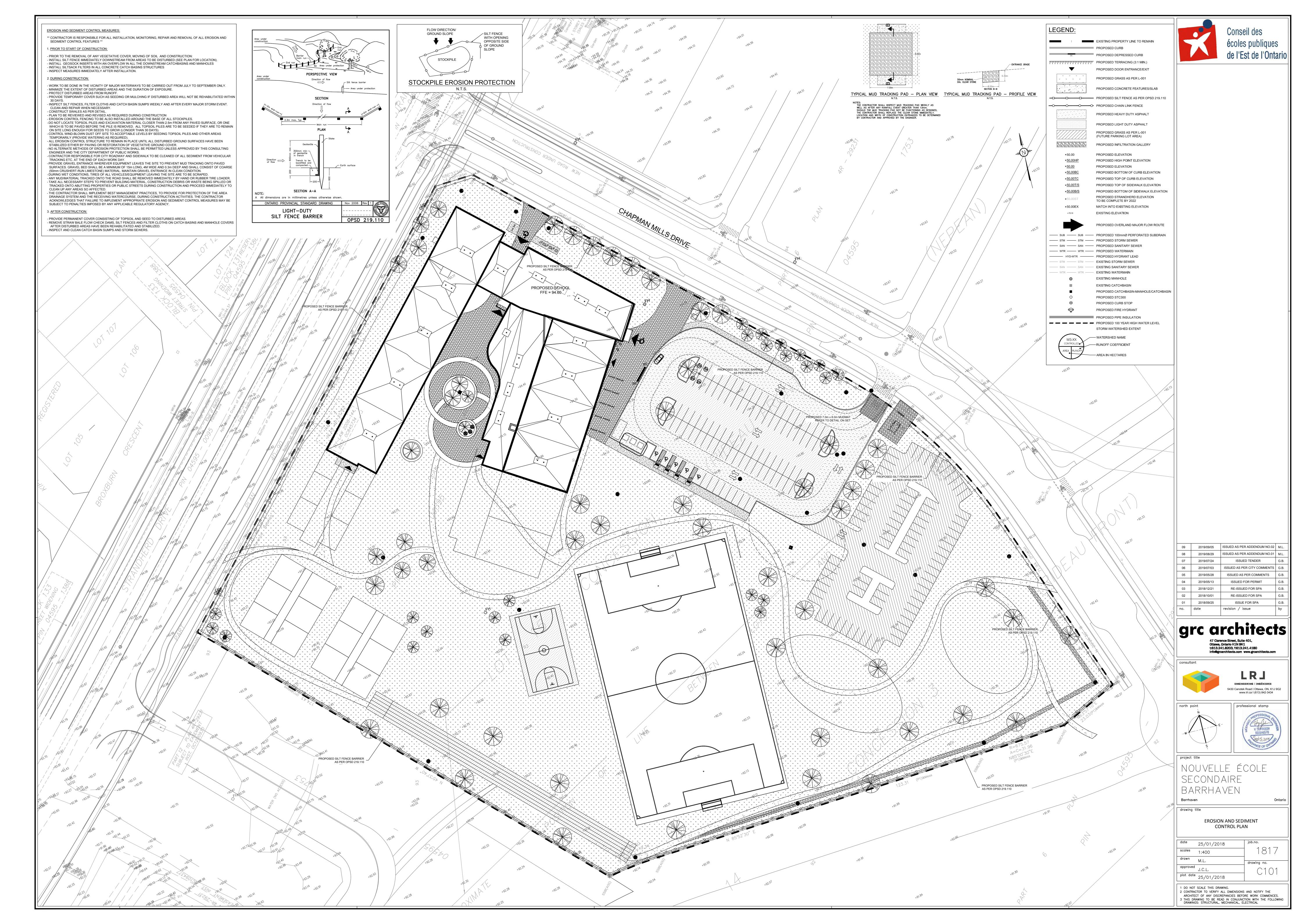
Notes:

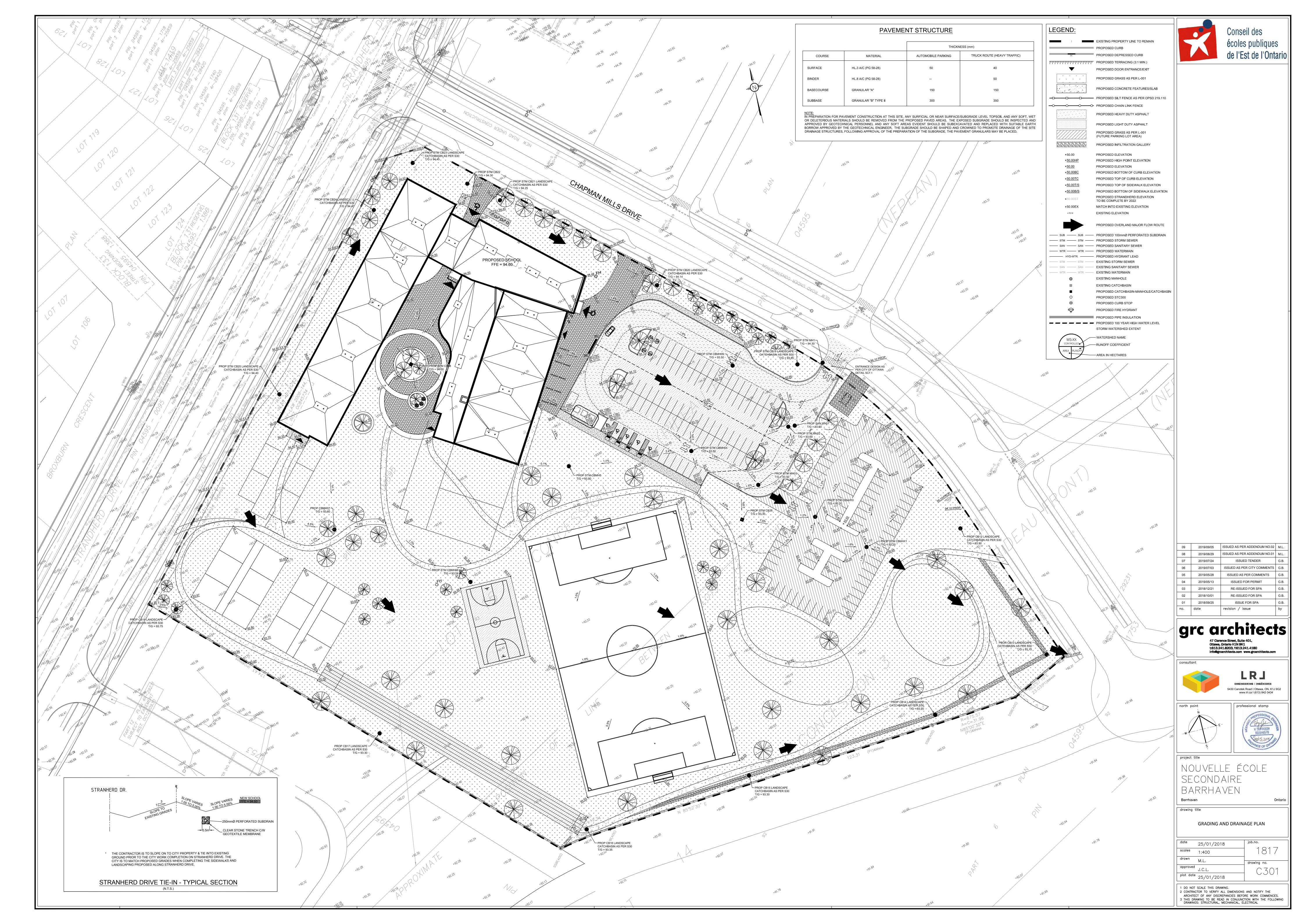
institutional, Commercial and Residential Area taken from Draft Plan of Subdivision prepared by Annis, O'Sullivan, Vollebekk Ltd. (Residential Area inclusive of Future Transit Corridor (2.279 Ha) and Servicing Easement (0.112 Ha))

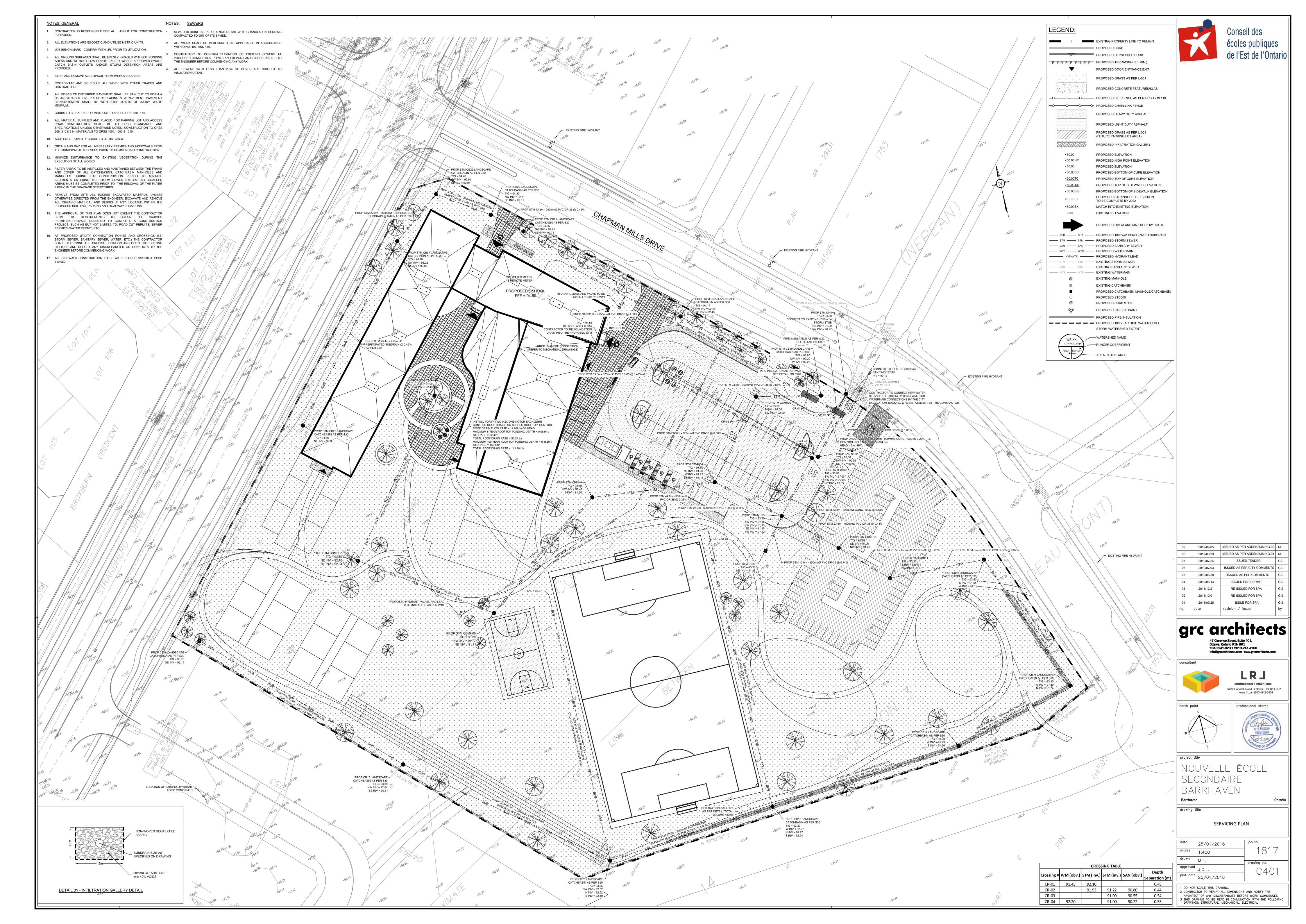
** Cumulative Population based on Medium Density Residential Land Use (162 pers/ha) as per Novatech's South Nepean Collector - Phase 2 & 3 Sanitary Sewer Design Sheet dated August 2015

APPENDIX E

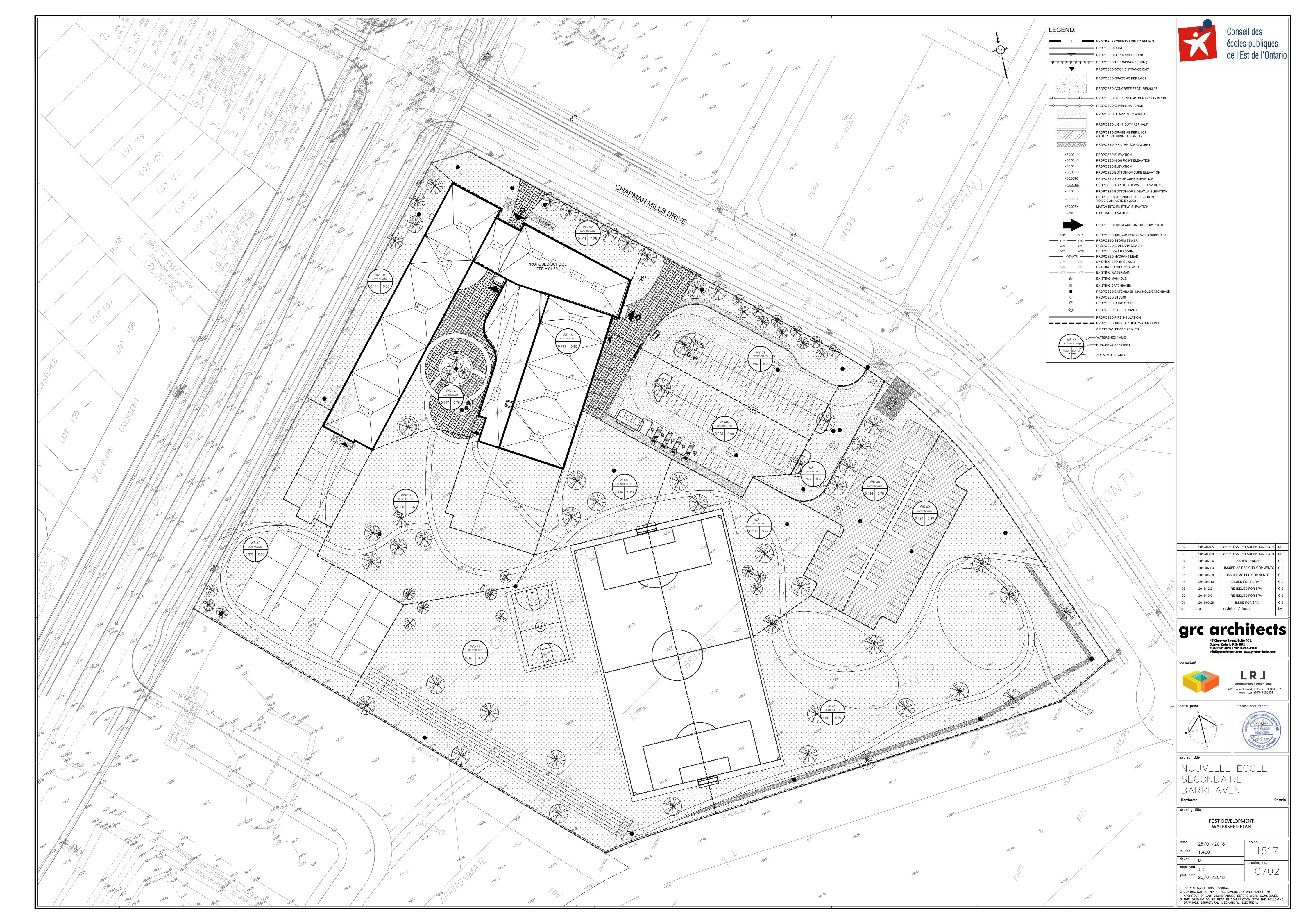
Engineering Drawings

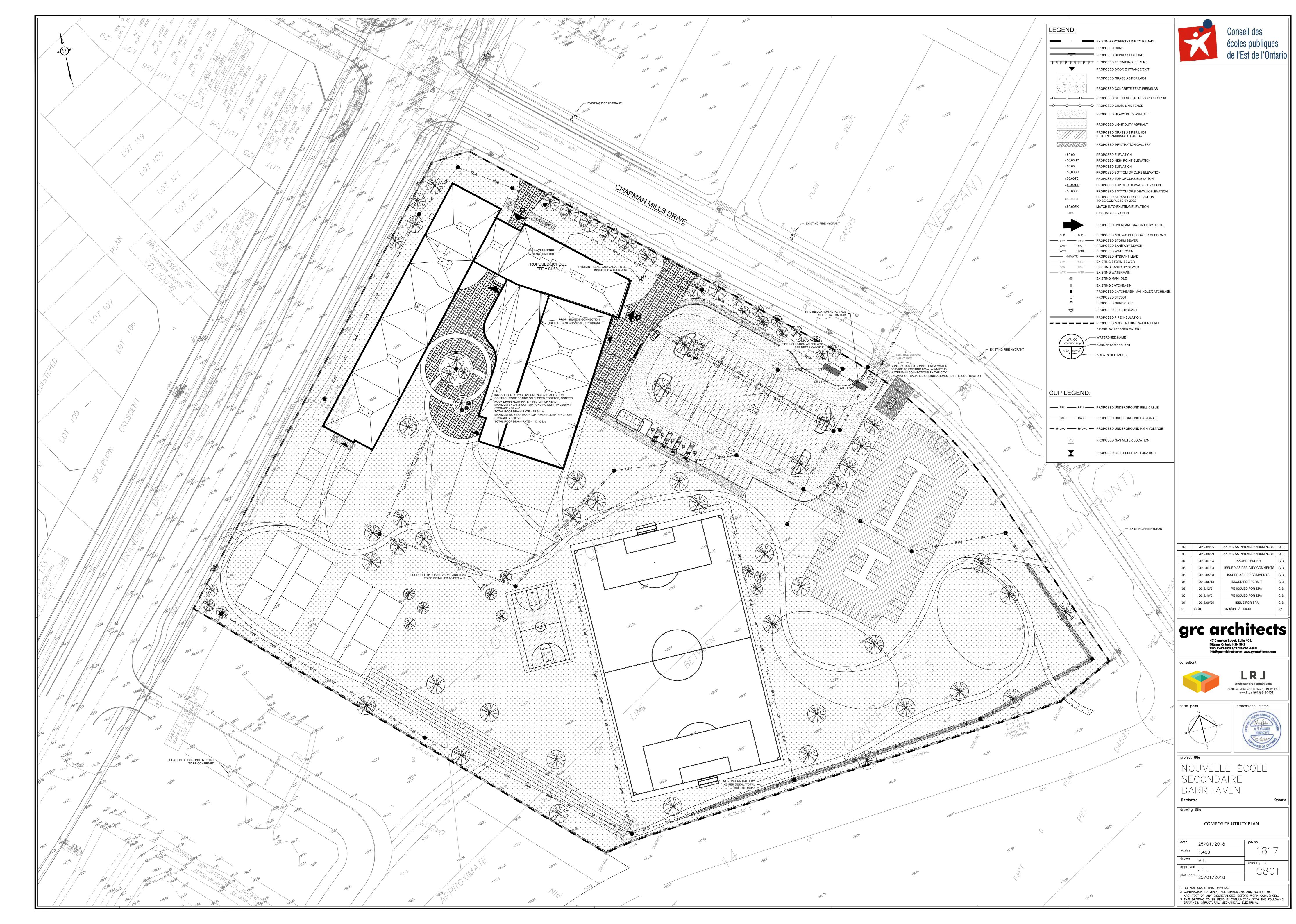


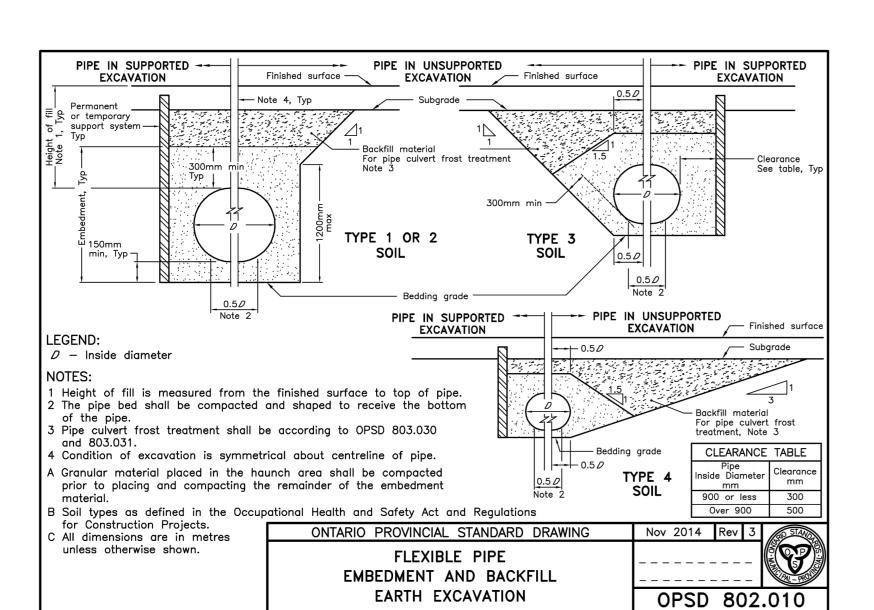


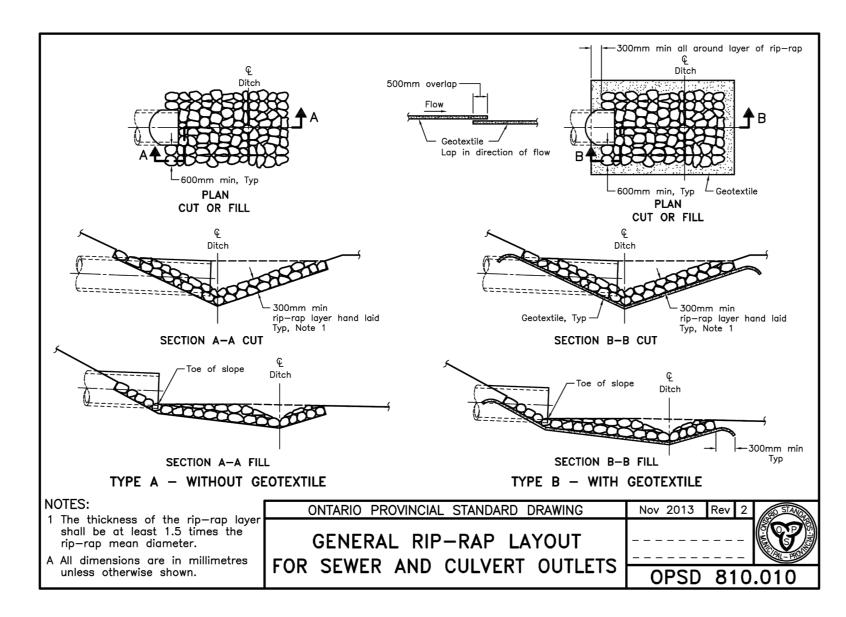


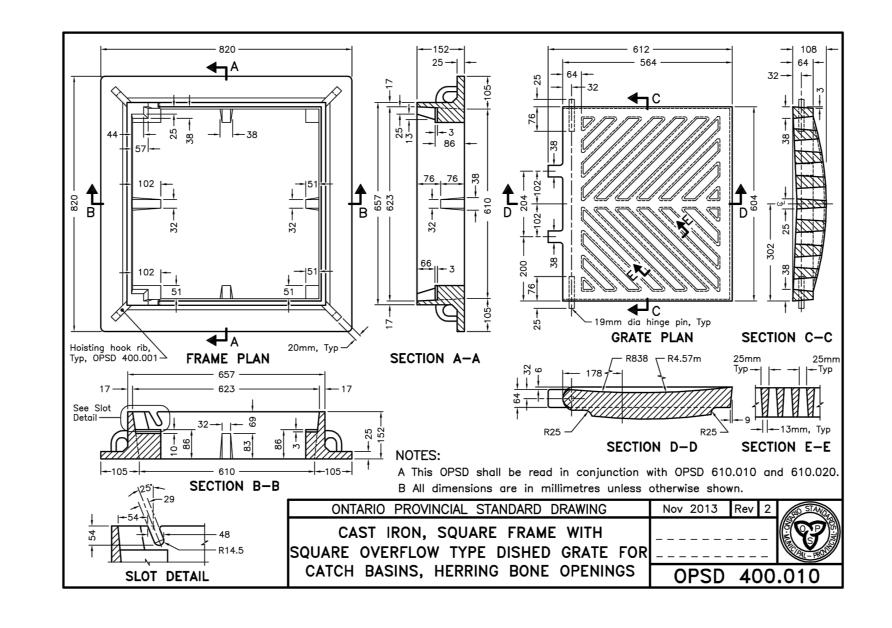


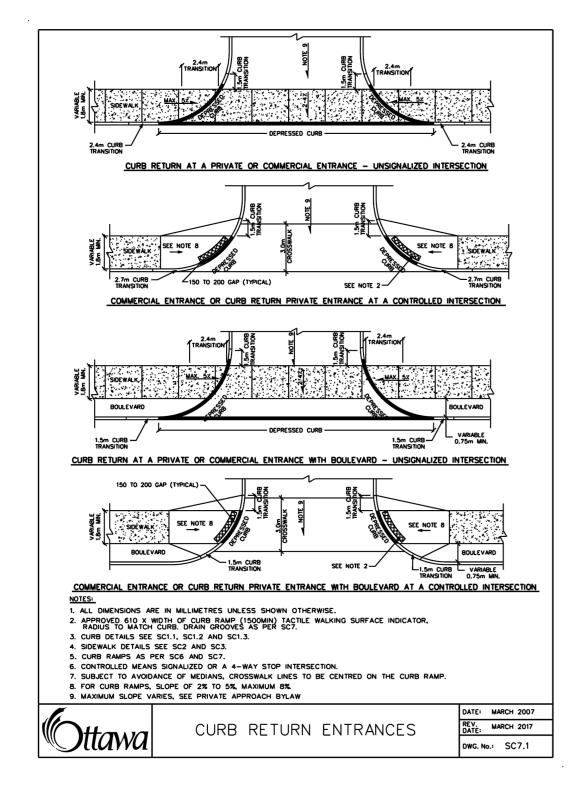


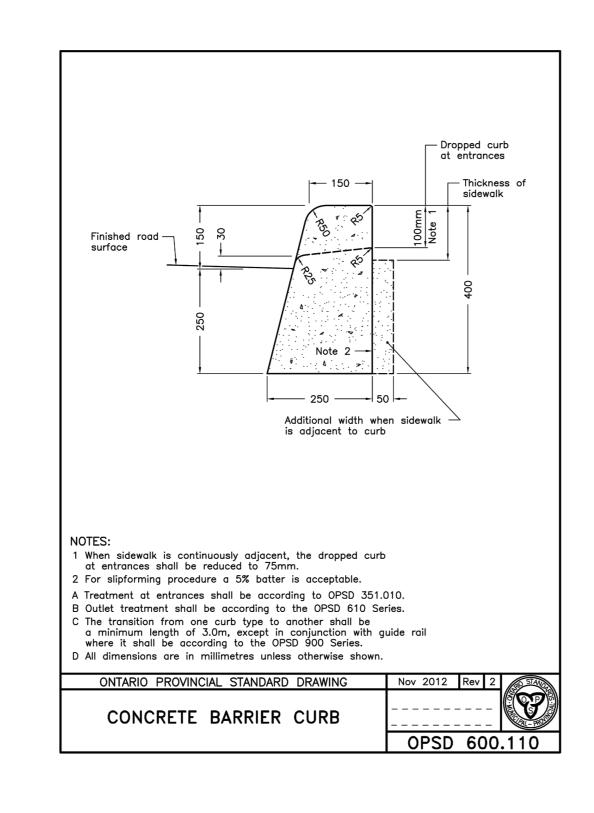


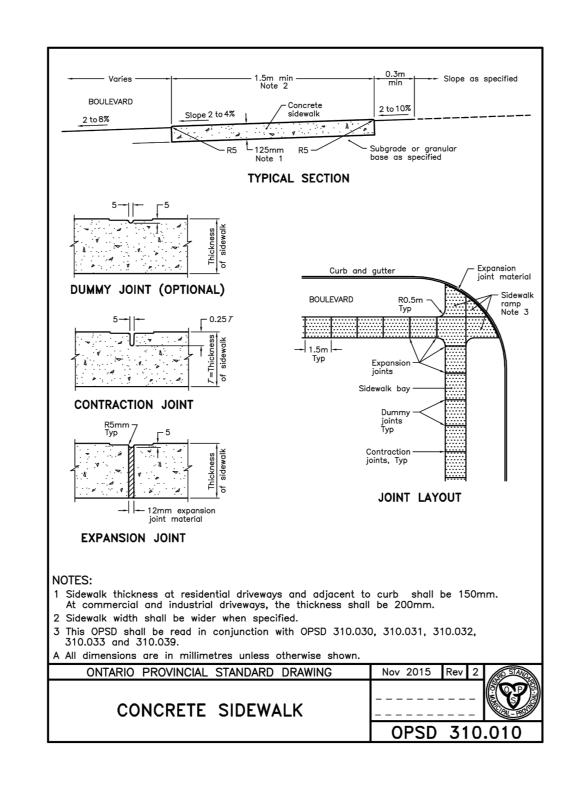


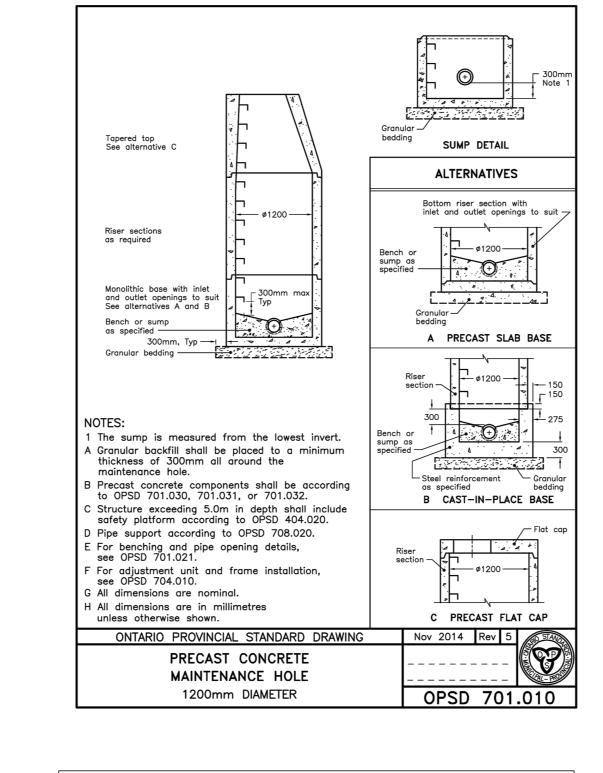


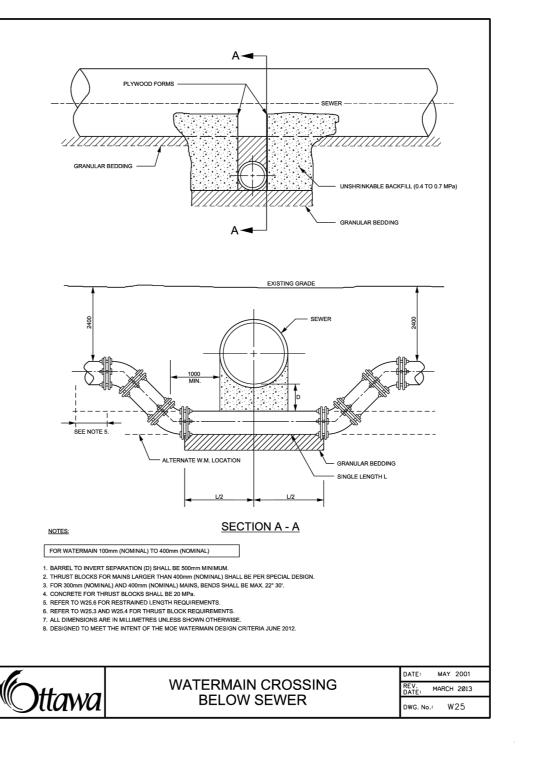


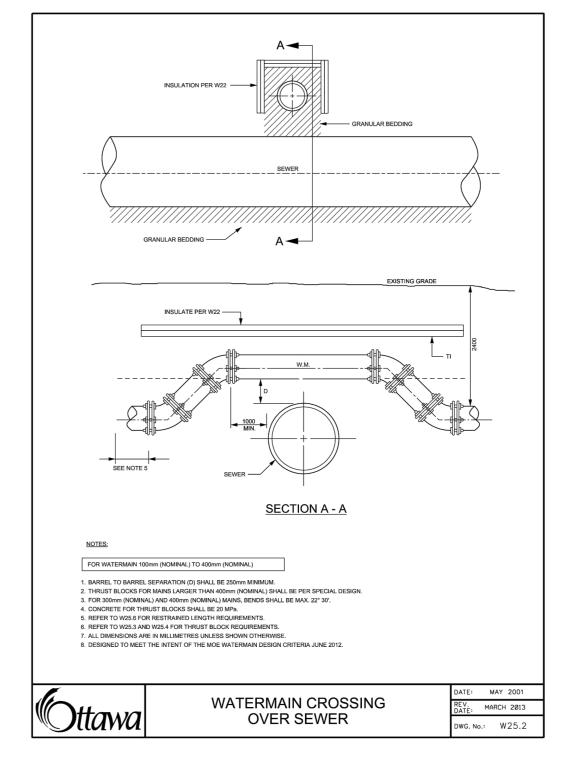


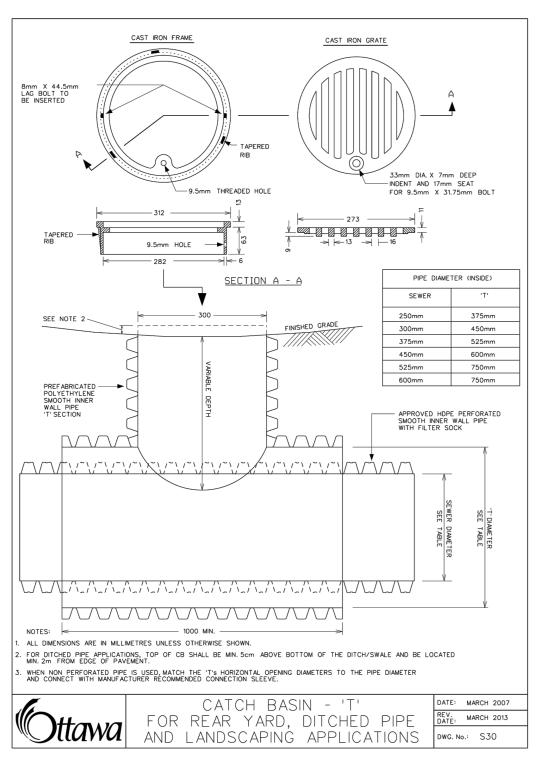


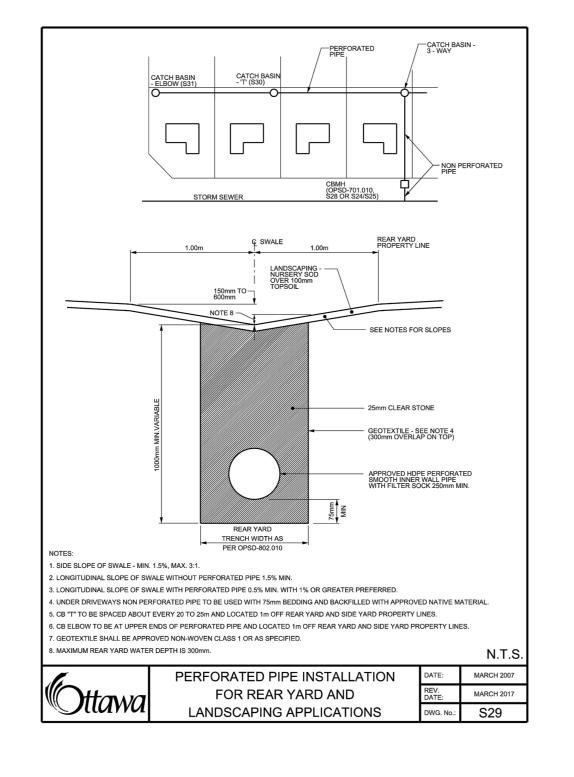


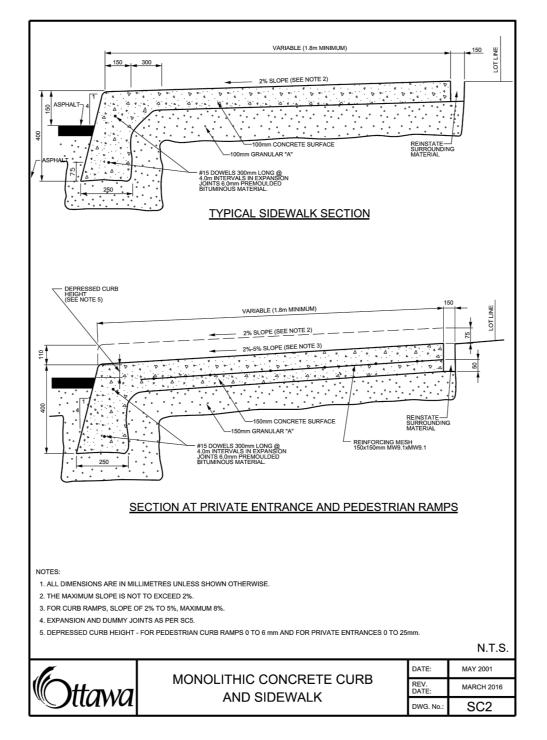


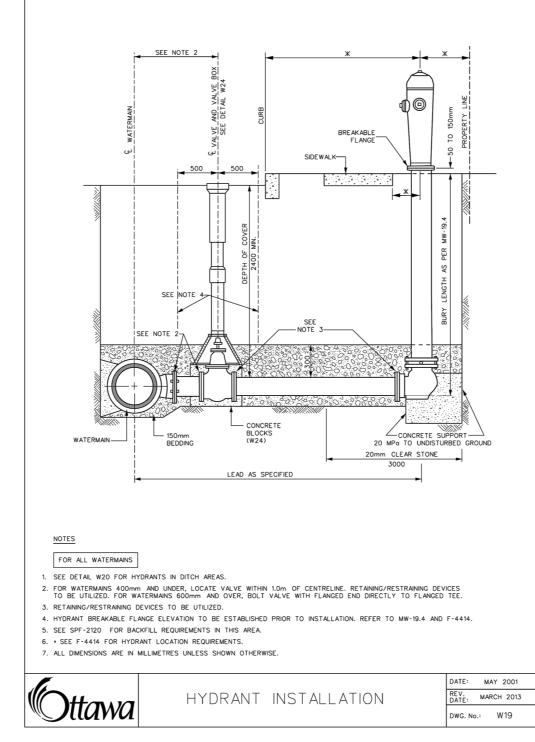


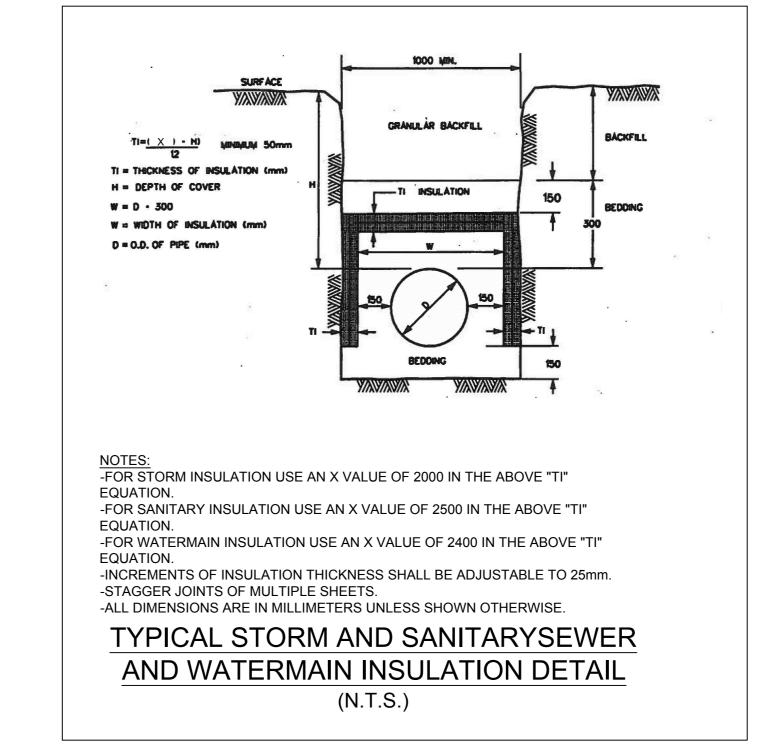








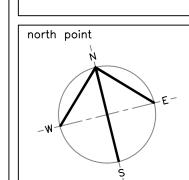




09	2019/09/05	ISSUED AS PER ADDENDUM NO.02	M.L.
08	2019/08/29	ISSUED AS PER ADDENDUM NO.01	M.L.
07	2019/07/24	ISSUED TENDER	G.B.
06	2019/07/03	ISSUED AS PER CITY COMMENTS	G.B.
05	2019/05/28	ISSUED AS PER COMMENTS	G.B.
04	2019/05/13	ISSUED FOR PERMIT	G.B.
03	2018/12/21	RE-ISSUED FOR SPA	G.B.
02	2018/10/01	RE-ISSUED FOR SPA	G.B.
01	2018/09/25	ISSUE FOR SPA	G.B.
no.	date	revision / issue	by
1	1		









project title

NOUVELLE ÉCOLE

SECONDAIRE

BARRHAVEN

drawing title

CONSTRUCTION DETAILS PLAN

date	25/01/2018	, , , , ,
scales	1:400	181/
drawn	M.L.	drawing no.
approved	J.C.L.	
plot date	25/01/2018	

DO NOT SCALE THIS DRAWING.
 CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE WORK COMMENCES.
 THIS DRAWING TO BE READ IN CONJUNCTION WITH THE FOLLOWING DRAWINGS: STRUCTURAL, MECHANICAL, ELECTRICAL