

GENERAL NOTES

- ANY DEVIATION FROM THE CONDITIONS SHOWN ON THESE DRAWINGS MUST BE REPORTED TO THE ENGINEER.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF PART 4 OF THE 2024 O.B.C. ONTARIO REGULATION 203/24
- STANDARDS**
 - CSA STANDARD A23.3-19 DESIGN OF CONCRETE STRUCTURES
 - CSA STANDARD A23.1-19 CONCRETE MATERIALS & METHODS OF CONCRETE CONSTRUCTION
 - CSA-S16-19 LIMIT STATES DESIGNS OF STEEL STRUCTURES
- ANY MODIFICATIONS TO EXISTING STRUCTURES ARE TO BE LIMITED TO WORK NOTED ON THESE DRAWINGS. ANY ADDITIONAL OR PROPOSED MODIFICATIONS TO EXISTING STRUCTURES MUST BE APPROVED BY THE ENGINEER

- FOUNDATIONS**
 - ALL FOOTINGS ARE TO BEAR ON UNDISTURBED COMPACT/STIFF GLACIAL TILL, OR ENGINEERED FILL. CONFIRM WITH GEOTECHNICAL REPORT/ENGINEER ANY REQUIREMENTS FOR MUD SLAB TO PROTECT IN-SITU SOILS.
 - BEARING CAPACITY USED IN THE FOOTING DESIGN IS ASSUMED TO BE $SLS=150 \text{ kPa}$ / $ULS=225 \text{ kPa}$
 - BEARING SURFACE IS TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
 - FOR FURTHER INFORMATION SEE GEOTECHNICAL REPORT No. PG6694-1 PREPARED BY PATERSON GROUP
 - STEP FOOTINGS WHERE INDICATED ON PLAN AT THE RATE OF 1 HORIZONTAL TO 1 VERTICAL. PROVIDE A MINIMUM OF 4-200 x 2400 Lg DOWELS FROM EACH PILE TO PILE CAP.

- MATERIALS**
 - CONCRETE STRENGTH AT 28 DAYS TO BE AS NOTED ON THESE DRAWINGS AND SPECIFICATIONS.
 - REINFORCING STEEL TO BE DEFORMED GRADE 400W WITH $F_y=400 \text{ MPa}$.
 - HOLLOW STRUCTURAL STEEL SECTIONS TO BE ASTM A500 GRADE C OR G40.21 350W CLASS C.
 - ALL "W", "C", "L" & "WWF" SHAPE STEEL SECTIONS TO BE GRADE G40.21 350W WITH $F_y=350 \text{ MPa}$.
 - ALL OTHER STRUCTURAL STEEL TO BE GRADE G40.21 300W WITH $F_y=300 \text{ MPa}$ UNLESS NOTED OTHERWISE.
 - ALL STRUCTURAL STEEL TO RECEIVE 1 SHOP APPLIED COAT OF PRIMER UNLESS NOTED.
 - ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR IS TO BE HOT DIP GALVANIZED UNLESS NOTED.
 - ANCHOR BOLTS TO BE A307.
 - ALL OTHER BOLTS TO BE A325.
 - A325 BOLTS EXPOSED TO EXTERIOR ARE TO BE GALVANIZED U/N.
 - A307 BOLTS EXPOSED TO EXTERIOR ARE TO BE GALVANIZED U/N.
 - ALL WOOD STUDS TO BE SPF NO.2 OR BETTER.
 - ALL PLYWOOD TO BE D-FIR PLYWOOD TO CSA 0121 OR CANADIAN SOFTWOOD PLYWOOD TO 0151.
 - ALL OSB TO MEET CSA 0325.
 - ALL WOOD TO BE DRY SEASONED, WITH A MOISTURE CONTENT LESS THAN 15%.

- CONCRETE COVER**
 - FOOTINGS 75 mm BOTTOM 50 mm SIDES
 - WALLS 50 mm UNLESS NOTED OTHERWISE

- DOWELS**
 - DOWELS TO FOOTINGS TO BE OF SAME DIAMETER AS THE LOWEST LIFT OF VERTICAL REINFORCING IN COLUMNS, PIERS OR WALLS.

9. LOADS

ALL LOADS & FORCES INDICATED ON THESE DRAWINGS ARE UNFACTORED WORKING LOADS UNLESS NOTED.

10. LEGEND

B = BOTTOM
 B1 = BOTTOM LOWER LAYER
 B2 = BOTTOM UPPER LAYER
 BLL = BOTTOM LOWER LAYER
 BUL = BOTTOM UPPER LAYER
 CONT = CONTINUOUS
 DP = DEPTH
 DWL = DOWELS
 EE = EACH END
 EF = EACH FACE
 EL = ELEVATION
 ES = EACH SIDE
 EW = EACH WAY
 H = HORIZONTAL
 (H) = HOOKED BAR
 O/C = ON CENTER
 T = TOP
 T1 = TOP UPPER LAYER
 T2 = TOP LOWER LAYER
 TLL = TOP LOWER LAYER
 TUL = TOP UPPER LAYER
 UM = UNLESS NOTED OTHERWISE
 V = VERTICAL

DESIGN & DETAILING CRITERIA FOR SUPPLIERS

1. MISCELLANEOUS METALS & STEEL STAIRS
 MISC METALS & STEEL STAIRS ARE TO BE DESIGNED AND DETAILED BY MISC METALS & STEEL STAIRS SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL MISC METAL & STEEL STAIR WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE MISC METALS & STEEL STAIRS DESIGN ENGINEER.

2. GUARDS & HANDRAILS
 GUARDS & HANDRAILS ARE TO BE DESIGNED AND DETAILED BY STEEL SUPPLIER IN ACCORDANCE WITH THE CURRENT BUILDING CODE REQUIREMENTS. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL GUARDS & HANDRAIL WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE GUARD & HANDRAIL DESIGN ENGINEER. ALL GLASS IN GUARDS IS TO BE IN COMPLIANCE WITH SB-13 OF THE ONTARIO BUILDING CODE. GLASS GUARD DESIGN IS TO MEET THE REQUIREMENTS OF PART 4.1.5.14 OF THE ONTARIO BUILDING CODE. GLASS GUARDS ARE TO COMPLY WITH CAN/CSG-12.20-M89. PROVIDE A CONTINUOUS TOP RAIL ON ALL FREE-STANDING GLASS GUARDS.

3. TEMPORARY BRACING
 EACH TRADE SHALL SUBMIT TEMPORARY BRACING PLANS AND SAFE ERECTION PROCEDURES NECESSARY FOR THE COMPLETION OF THEIR WORK. THESE PLANS ARE TO BE COORDINATED WITH OTHER TRADES ON SITE TO ENSURE NO OVERLAP OR INTERFERENCE, AND SITE WORK IS NOT INTERRUPTED. DRAWINGS AND PROCEDURES ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, AND PROVIDED TO THE DESIGN TEAM FOR REVIEW. ALL TEMPORARY BRACING IS TO BE REVIEWED ON SITE BY THE BRACING DESIGN ENGINEER, AND REPORTS PROVIDED TO THE DESIGN TEAM. ALL BRACING REMOVAL IS ONLY TO OCCUR AFTER RECEIVING WRITTEN PERMISSION BY THE BRACING ENGINEER CONFIRMING THE FINAL DESIGN CONDITION HAS BEEN MET AND IS STABLE. SIGNOFF LETTERS ARE TO BE PROVIDED TO THE DESIGN TEAM. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING COORDINATION OF BRACING AND TEMPORARY WORKS MEASURES BY SUB TRADES.

NOTE:
 INSPECTION REPORTS CREATED AS A RESULT OF THE ABOVE NOTED WORK MUST BE SUBMITTED TO THE CONSTRUCTION MANAGER. CONSTRUCTION MANAGER IS TO PROVIDE COPIES TO THE CONSULTANTS.

REINFORCING BAR LAP LENGTH TABLE

CONCRETE STRENGTH (MPa)	REINFORCING BAR LAP LENGTH (mm)				
	10M	15M	20M	30M	35M
25	425	600	750	1200	1400
30	400	550	675	1100	1275
35	375	525	625	1000	1200
40	350	475	600	950	1125
45	325	450	550	900	1050
50	300	425	525	850	1000

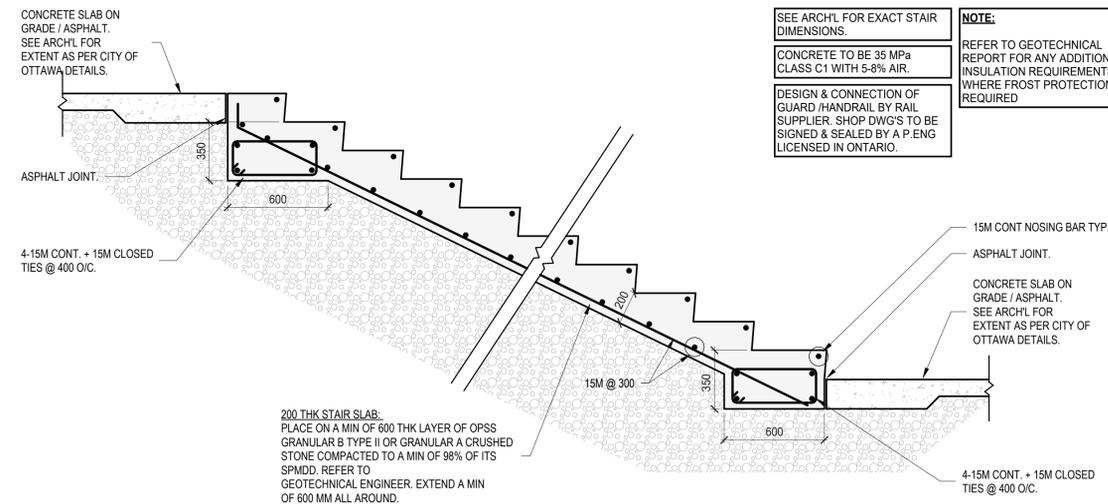
CONCRETE COMPRESSIVE STRENGTH:

35 MPa WITH 5-8% AIR ENTRAINMENT. (CLASS C1 EXPOSURE)

NOTE:

USE NEW FORM PLY ON ALL EXPOSED ARCH'L FINISHES. REFER TO ARCHITECTURAL DRAWINGS FOR FINISHES AND REVEALS.

REFER TO GEOTECHNICAL REPORT FOR ANY ADDITIONAL INSULATION REQUIREMENTS WHERE FROST PROTECTION IS LESS THAN 2100mm



TYPICAL CAST-IN-PLACE CONCRETE STAIR

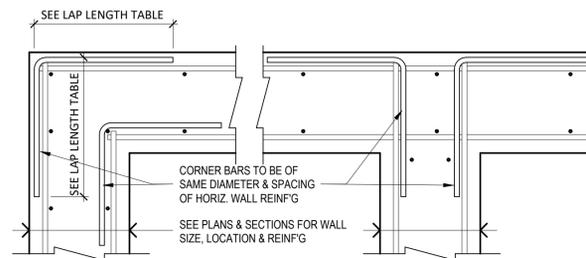
SEE ARCH'L FOR EXACT STAIR DIMENSIONS.

CONCRETE TO BE 35 MPa CLASS C1 WITH 5-8% AIR.

DESIGN & CONNECTION OF GUARD / HANDRAIL BY RAIL SUPPLIER. SHOP DWGS TO BE SIGNED & SEALED BY A P. ENG LICENSED IN ONTARIO.

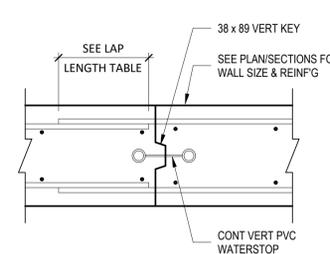
NOTE:

REFER TO GEOTECHNICAL REPORT FOR ANY ADDITIONAL INSULATION REQUIREMENTS WHERE FROST PROTECTION IS REQUIRED



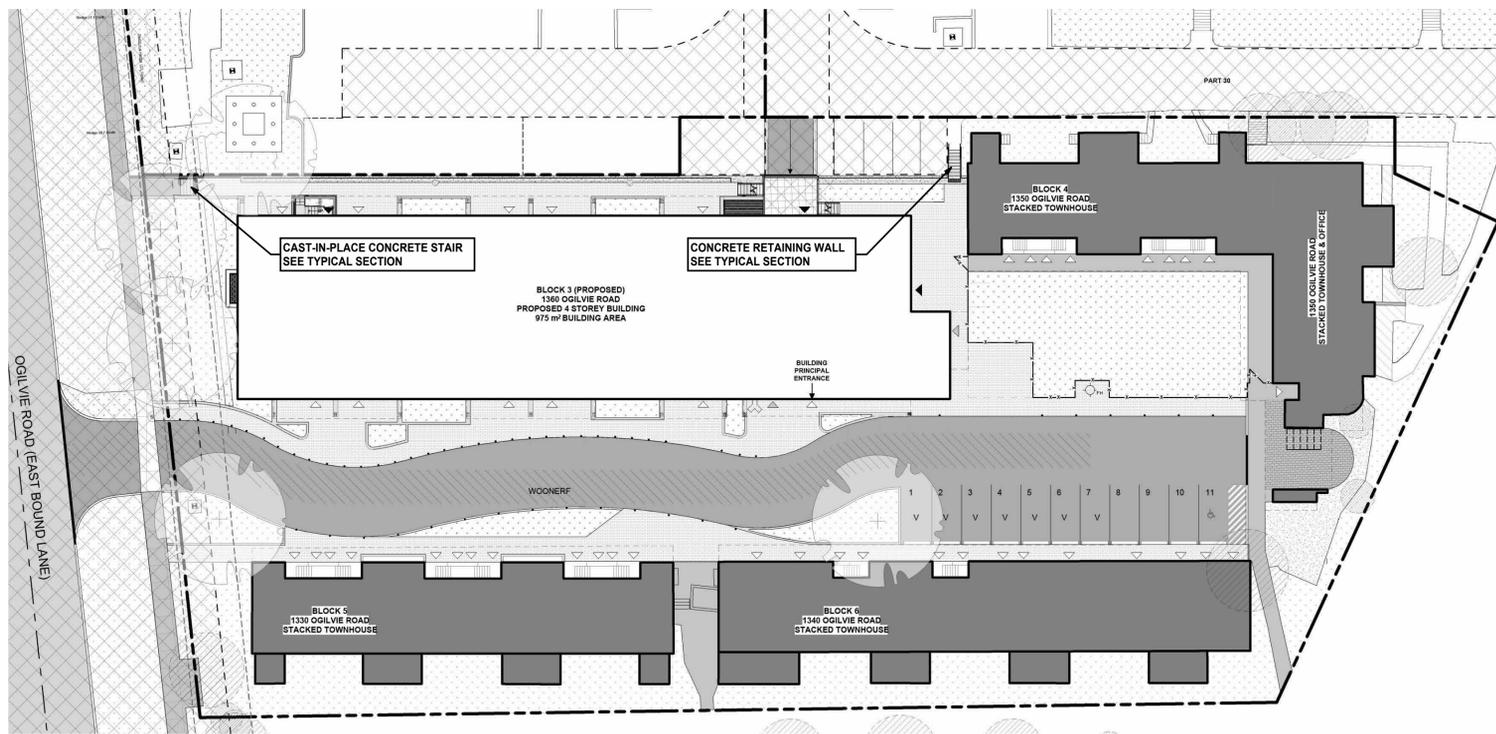
TYPICAL WALL INTERSECTION REINFORCEMENT

CONCRETE WALLS WITH 2 SHEETS OF REINFORCING (WALL THICKNESS GREATER THAN 215 mm) NOT APPLICABLE TO SHEARWALLS. SEE SHEARWALL ELEVATION DRAWINGS.



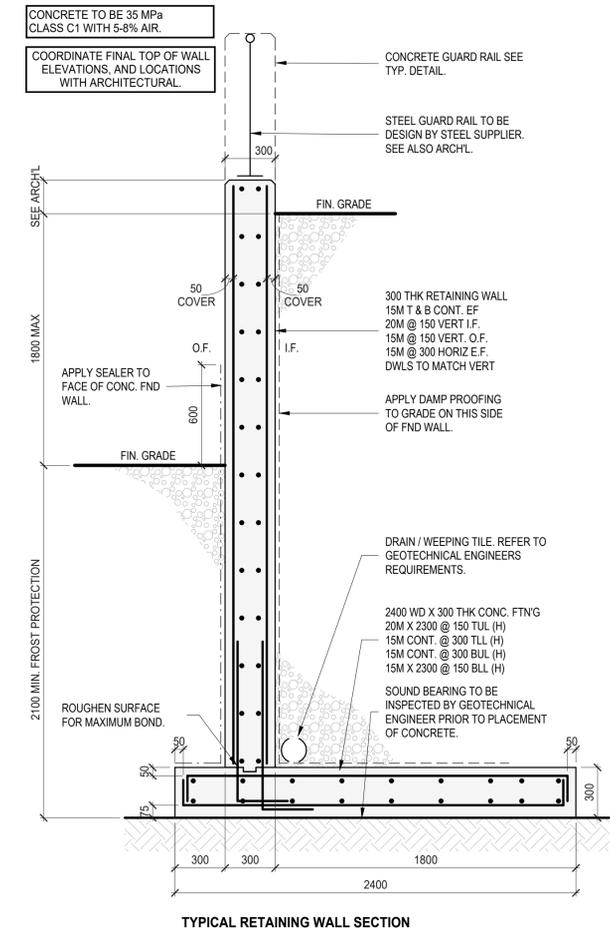
TYPICAL WALL CONSTRUCTION JOINT DETAIL

MAXIMUM SPACING OF CONSTRUCTION JOINTS TO BE 20 meters



PROPOSED ARCHITECTURAL SITE PLAN

THE WALL HAS BEEN DESIGNED WITH A FACTOR OF SAFETY GREATER THAN 1.5 FOR STATIC LOADS, AND 1.1 FOR SEISMIC LOADS



TYPICAL RETAINING WALL SECTION

ISSUED FOR: SITE PLAN CONTROL 2026-02-03

No. Revision Description Date

- THE CONTRACTOR IS RESPONSIBLE FOR CHECKING AND VERIFYING ALL DIMENSIONS. ANY DISCREPANCY SHALL BE REPORTED TO THE ENGINEER.
- THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL MATERIAL RELEVANT TO THE PROJECT.
- ADDITIONAL INFORMATION MAY BE ISSUED FOR CLARIFICATION TO ASSIST PROPER EXECUTION OF WORK. SUCH DRAWINGS WILL HAVE THE SAME MEANING AND INTENT AS IF THEY WERE INCLUDED WITH THE DRAWINGS IN THE CONTRACT DOCUMENT.
- DO NOT SCALE DRAWINGS

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PROJECT
BLOCK 3 REDEVELOPMENT
1360 OGLVIE ROAD

ARCHITECT
CSV ARCHITECTS

DRAWING TITLE
SITE PLAN DETAILS

DRAWN
 A.M.

REVIEWED
 J.C.

SCALE
 As indicated

ENGINEERS SEAL
 PROJECT No.
 23-064

SHEET No.
 SP.S1

REVISION No. 1