

301 Somme Street Industrial

Site Servicing and Stormwater Management Report



Prepared for:
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August 21, 2025

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Project/File:
160402138

301 Somme Street

Revision	Description	Author	Date	Quality Check	Date	Independent Review	Date
0	For Review	Dustin T.	2025-08-20	Dustin T.	2025-08-21	Peter M.	2025-08-21



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1 Introduction

Stantec Consulting Ltd. has been commissioned by W.O. Stinson & Son Ltd. to prepare the following Servicing and Stormwater Management Report in support of the proposed development located at 301 Somme Street in the City of Ottawa.

The 6.05ha site is located between Rideau Road to the north and Somme Street to the south and west. The site is currently zoned RH, and is currently undeveloped. Additional undeveloped parcels exist to the east of the proposed site. The site forms part of the existing Hawthorne Industrial Park subdivision, as shown in **Figure 1** below.



Figure 1: Key Plan



The proposed development will see a new tank and equipment storage building, a vehicle service building, exterior propane storage and equipment storage yard, and a private commercial gas bar constructed in addition to parking spaces and private driveways servicing the project limit area.

1.1 Objective

This site servicing and stormwater management (SWM) report presents a servicing scheme that is free of conflicts, provides on-site servicing in accordance with City of Ottawa Design Guidelines, and uses the existing municipal infrastructure in accordance with any limitations communicated during prior consultation with the City of Ottawa staff. Details of the existing infrastructure located within the Somme Street right of way (ROW) were obtained from available as-built drawings and site topographic survey.

Criteria and constraints provided by the City of Ottawa have been used as a basis for the detailed servicing design of the proposed development. Specific and potential development constraints to be addressed are as follows:

- Potable Water Servicing
 - Estimated water demands to characterize the proposed feed(s) for the proposed development which will be serviced from an on-site private well.
 - Provision of required fire flows for building exterior fire suppression activities via dry hydrant and on-site reservoir (stormwater management facility).
- Wastewater (Sanitary) Servicing
 - Define and size the sanitary service lateral which will be connected to a proposed on-site sewage system for dispersal.
- Storm Sewer Servicing
 - Define major and minor conveyance systems in conjunction with the proposed grading plan.
 - Determine the stormwater management storage requirements to meet the allowable release rate for the site.
 - Define and size the proposed storm sewers and stormwater management facility that will collect discharge from the site to the existing Somme Street roadside ditch.
- Prepare a grading plan in accordance with the proposed site plan and existing grades.

The accompanying drawings illustrate the proposed stormwater servicing for the parking lot and driveways on site.



2 Background

Documents referenced in preparing this stormwater and servicing report for 301 Somme Street include:

- *City of Ottawa Sewer Design Guidelines* (SDG), City of Ottawa, October 2012, including all subsequent technical bulletins.
- *City of Ottawa Design Guidelines – Water Distribution*, City of Ottawa, July 2010, including all subsequent technical bulletins.
- *Design Guidelines for Drinking Water Systems*, Ministry of the Environment, Conservation, and Parks (MECP), 2008.
- *Fire Protection Water Supply Guideline* for Part 3 in the Ontario Building Code, Office of the Fire Marshal (OFM), October 2020.
- *Water Supply for Public Fire Protection*, Fire Underwriters Survey (FUS), 2020.
- *Stormwater Management Planning and Design Manual*, Ministry of the Environment, Conservation, and Parks (MECP), 2003.
- *Geotechnical Investigation – Proposed Commercial Development 301 Somme Street*, Paterson Group, August 2025.
- *Terrain Analysis and Hydrogeological Study – Proposed Commercial Development 301 Somme Street*, Paterson Group, August 2025.
- *Shields Creek Subwatershed Study*, Totten Sims Hubicki Associates, June 2004.



3 Water Servicing

3.1 Background

The proposed site lies within an undeveloped block of the Hawthorne Industrial Park southeast of Rideau Road within the City of Ottawa. The proposed site does not lie within existing pressure zone boundaries of the City of Ottawa's water distribution system. The site is rural in context, and has not been provided with a piped water service connection for development, nor does an economical watermain connection option exist in proximity to the development for provision of required fire flows.

3.2 Proposed Domestic Water Servicing

Site domestic water supply is proposed to be provided through an existing on-site well dug in the immediate proximity to the proposed vehicle service bay / site office building. A detailed assessment of well water design requirements for the proposed development was performed by Paterson Group and summarized in their Terrain Analysis and Hydrogeological Study.

Paterson's water supply assessment concluded that minimum well yield capable of supplying the daily water demand for the proposed uses is readily obtainable on-site. Similarly, Paterson's investigation concluded that the existing raw water supply is considered to be treatable to meet the requirements of the drinking water regulations. Based on the raw water quality data obtained, it was recommended that chlorine be introduced to existing well water to produce a free chlorine residual of at least 50 mg/L, and a standard residential water softener be installed to remove hardness / iron concentrations.

PEX piping is recommended for domestic water supply lines in order to minimize potential for corrosion of internal piping.

3.3 Fire Flow Demands

As the proposed private site lies outside of the municipal potable water distribution system, the required fire flow for the development has been based on requirements of the Ontario Building Code (OBC). Fire flow requirements are outlined in the 2024 Building Code Compendium section A-3.2.5.7. Based on the site plan, the governing building for fire flow is anticipated to be the tank and equipment storage building intended for Group F Division 1 occupancy.

Fire flow demand was estimated based on a building of non-combustible construction type with fire-rated structural members. Although this occupancy class would anticipate a required sprinkler coverage for the entire building, it is anticipated that building sprinklers will be provided using fire suppression foam or charged by other specialized means instead of via typical fire department connection. As such, required fire flows would still remain for the exterior shell of the on-site buildings. The storage building footprint was used as the effective floor area, with an assumed building height of 6m. Given the highly structured and coordinated staffing required to operate a building under the Liquid Fuels Handling Code per Technical



Standards and Safety Authority (TSSA) requirements, a 30% fire flow reduction has been applied per OBC section A-3.2.5.7 for buildings with “well-established and practiced fire safety plans”.

Based on the above, the building’s required fire flow volume was determined to be 336m³ at a minimum rate of flow of 9,000 L/min. Detailed fire flow calculations per the OBC methodology are provided in **Appendix A**.

Per the City of Ottawa’s Technical Bulletin IWSTB-2024-05, required fire flows at the above noted rates under OBC methodology require additional review under FUS and potentially NFPA 1142 for further evaluation of required fire flows.

Resultant required fire flows under NFPA 1142 would consider an occupancy hazard class of 3 (flammable liquids) and construction classification number of 0.75 (non-combustible construction). Required fire flow volume in gallons would then be determined as the building volume in cubic feet multiplied by the construction classification and divided by the occupancy classification. This results in a volume of 184502 gallons, or approximately 698m³. It is of note that this standard is to apply to buildings without automatic sprinkler protection, which would not be the case for the proposed development.

Fire flow calculations per FUS methodology have been included in **Appendix A**, which identify a required fire flow volume of 960m³. As the building contents (flammable liquids) would require fire suppression using alternate means than water, it is felt that required fire flows under FUS methodology are overconservative for the site use given consideration of occupancy charges in the calculation methodology, and for a 2-hour duration of fire flow which would extend far beyond anticipated fire suppression activities.

3.4 Proposed Fire Water Servicing

It is proposed to provide the required fire flow volumes at the required rates determined by OBC calculations in the section above (two hydrants required). Provision of the required fire flows is to be via two dry hydrants installed in proximity to the on-site buildings. Dry hydrants are proposed to be installed as outlined in the City of Ottawa’s standard drawing W53, with schedule 40 piping from the two hydrants installed to connect to the proposed on-site stormwater management pond described in Section 5 further below.

The dry hydrant intake from the proposed pond is to be installed above a localized 600mm deep sump in the pond main cell to prevent sedimentation, and complete with a horizontal strainer and fixed end cap. Piping is to be installed at a minimum downward slope from hydrant to pond to allow for any accumulated sediments in conveyance piping to migrate back to the pond. The pond permanent pool depth of 1.7m allows for sufficient water above the intake for suction, and sufficient pond volume exists discounting anticipated ice depths for fire suppression as described in Section 5.5.10.



4 Wastewater Servicing

4.1 Background

The proposed site lies within an undeveloped block of the Hawthorne Industrial Park southeast of Rideau Road within the City of Ottawa. The site is rural in context, and has not been provided with a sanitary sewer outlet for development, nor does an economical sewer connection option exist in proximity to the development.

4.2 Wastewater Servicing Design

Domestic sanitary flows from the site are proposed to discharge to an on-site sewage system (pretreatment tank, treatment unit, and Type 'A' leaching bed per OBC requirements) within the southwest quadrant of the site. Daily sanitary sewage flows are estimated at approximately 5,130L/day. For further details on the proposed sewage system, please see Paterson Group Terrain Analysis and Hydrogeological Study under separate cover.

4.3 Proposed Servicing

A 125 mm diameter sanitary sewer at 2% slope is proposed to service the vehicle storage bays and office building and will direct wastewater peak flows to the proposed sewage system. Due to proximity of the sewer to the existing well location to be used for domestic water service, the sewer service is to be sleeved through a 200mm PVC pipe and overlaid by rigid insulation to protect the service from freezing. Final sizing of the lateral is to be confirmed by the mechanical consultant. The proposed sanitary servicing to the sewage system is shown on **Drawing SSP-1**.



5 Stormwater Management and Servicing

5.1 Objectives

The goal of this stormwater servicing and management plan is to determine the measures necessary to control the quantity and quality of stormwater released from the proposed design to meet the criteria established during the consultation process with City of Ottawa staff, and to provide sufficient details required for approval.

5.2 Stormwater Management (SWM) Criteria

The Stormwater Management (SWM) criteria were established by combining current design practices outlined by the City of Ottawa Sewer Design Guidelines (SDG) (October 2012), the Ontario Ministry of the Environment, Conservation, and Parks (MECP) Stormwater Management Planning and Design Manual, South Nation Conservation Authority (SNC) Environmental Planning Policies, and through prior consultation with City of Ottawa staff by the proponent. The following summarizes the criteria, with the source of each criterion indicated in brackets:

General

- Use of the dual drainage principle (City of Ottawa SDG).
- Wherever feasible and practical, site-level measures should be used to reduce and control the volume and rate of runoff (City of Ottawa SDG).
- Assess impact of 2-year, 5-year, 10-year, and 100-year event outlined in the City of Ottawa Sewer Design Guidelines on the major and minor drainage systems (City of Ottawa SDG).
- The post-development discharge under design storm events (2-100 year storms) from the site are to be restricted to that of the previously approved Stormwater Management Report – Hawthorne Industrial Park by J.L. Richards & Associates (City of Ottawa pre-consultation).
- Provide on-site water quality control with a minimum of 80 % TSS removal (City of Ottawa pre-consultation).

Storm Sewer and Inlet Control

- Minor system to be sized to capture at minimum runoff from the 2-year storm event (MECP, City of Ottawa SDG).
- Post-development peak discharge from the site will be limited to previously approved release rates for the 2-year, 5-year, 10-year, and 100-year design storm events (City of Ottawa pre-consultation).
- Sewer HGLs to be a minimum of 0.3m below proposed underside of footing elevations (City of Ottawa SDG).



Surface Storage and Overland Flow

- Overland flow to be directed within proposed rights-of-way to the downstream SWM pond (MECP, City of Ottawa SDG).
- 100-year surface HGL to be a minimum of 0.3 m below proposed building openings (City of Ottawa SDG).
- No surface ponding is to be permitted on roads/parking areas during the 2-year storm event (City of Ottawa SDG).
- Maximum depth of static ponding is not to exceed 0.35 m for design storm events (i.e., up to 100-year storm) (City of Ottawa SDG).
- Water must not encroach upon proposed building envelopes and must remain below all proposed building openings during the climate change event (City of Ottawa SDG).
- Provide adequate emergency overflow conveyance off-site (MECP, SDG).

5.3 Existing Conditions

The site forms part of several undeveloped parcels within the Hawthorne Industrial Park (HIP). Historically, runoff from the site sheet drained overland towards the northeast, discharging to the southerly roadside ditch along Rideau Road, and ultimately to the Findlay Creek Municipal Drain. The site has recently been subject to infilling at its northerly extents to provide a relatively flat development area and in anticipation of provision of positive drainage to the roadside ditches along Somme Street.

The *Stormwater Management Report - Hawthorne Industrial Park (HIPSWM)* (J.L Richards, 2009) identifies that runoff from the development is to be directed to the northerly roadside ditch along Somme Street, with eventual discharge to a previously constructed dry pond to the east of the development area. The dry pond was sized with sufficient capacity to control post-development peak runoff to pre-development levels for storm events ranging from the 2-year to the 100-year storm events based on IDF parameters outlined in the SDG. Ditches along Somme Street have been sized to convey 100-year runoff from serviced properties within the HIP under free-flowing conditions, and culverts within the ditch line have been sized to convey the 10-year peak flow rate without overtopping of the roadway embankment. Additionally, a 'normal' level of quality control protection (70% TSS reduction) has been previously provided for the Somme roadway via infiltration trench and perforated pipe system underlaying the ditches.

The HIPSWM report had prepared an analysis of the downstream dry pond through a SWMHYMO model with lumped catchment areas demonstrated on Figure 4 in **Appendix C**. Two scenarios were developed, Phase 1 which considers development of areas along Somme Street and further upstream, and Phase 2 that also includes development of a vacant future development block located immediately northeast of the development area. In both analyses the anticipated development was able to be serviced by a dry pond with an identical size and outlet structure, with Phase 2 incorporating an additional pond inlet solely for the purposes of servicing the additional development block.

Since issuance of the HIPSWM report, the underlying block legal fabric has been adjusted to suit development of the subject site. The current development area now forms part of Area 3 and 4 (the future



development block) in the drainage area Figure 4. Linework for the figure was overlaid on the current development plan to confirm that the site now maintains approximately 5.322ha from overall 15.6ha Area 3 (34.1% of overall) and 0.764ha from the overall 12.2ha Area 4 (6.26% of overall). In the ultimate modeled scenario for the HIP, both areas were assigned an imperviousness of 71%.

In order to develop allowable release rates for the subject site, modeled runoff for both blocks were extracted from model outputs in the HIPSWM report, weighted based on the contributing area from the development over the overall block, and summed to a single value. Allowable release rates for the site are listed in the table below:

Table 5-1: Peak Allowable Discharge Rate

Design Storm	Area 3 Runoff (L/s)	Weighted Contribution (L/s)	Area 4 Runoff (L/s)	Weighted Contribution (L/s)	Allowable Release Rate (L/s)
25mm	484	165	585	37	202
2-year	753	257	941	59	316
5-year	1176	401	1375	86	487
10-year	1475	503	1687	106	609
25-year	1879	641	2109	132	773
50-year	2237	763	2470	155	918
100-year	2612	891	2793	175	1066

5.4 Proposed Stormwater Management Plan

As site imperviousness is expected to exceed the previously assumed values within the HIPSWM report, additional quantity storage is required to restrict post-development outflows to allowable rates. In addition, the HIPSWM report identifies the need for developments within the industrial subdivision to provide their own quality control (identified in the report to 'Normal' 70% TSS reduction, now increased to 80% per City of Ottawa requirements) via on-site hydrodynamic separator, or OGS. It is of note that between issuance of the ECA for the industrial subdivision works for stormwater management and the present that there has been a change in the procedural requirement for assessing TSS reduction through use of OGS units as sole point of quality control, which would limit typical installation of OGS units to providing substantially less than the required 80% reduction. In addition, due to the nature of the industrial development, additional spills containment controls are recommended to prioritize retention of runoff in the event of a spills-related emergency.

It is proposed to provide the required quantity and quality control in conjunction with recommended spill containment within a wet pond located adjacent to the site discharge point to the roadside ditch along Somme Street.



5.4.1 Modeling Rationale

A hydrologic analysis was conducted using PCSWMM software to assess the rate and volume of runoff anticipated during post-development rainfall events for each design storm. Intensity Duration Frequency (IDF) curves were sourced from Ottawa's Sewer Design Guidelines. These IDF curves were used to develop 2-year, 5-year, 10-year, 25-year, 50-year, and 100-year 12-hour SCS Type II distributions for consistency with that modeled within the HIPSWM report, as well as to develop the 2-year and 100-year 3-hour Chicago storm events for use in sizing on-site sewer systems and culverts. Two climate change event scenarios were also developed by adding 20% to rainfall intensities for the two 100-year storms.

Surface storage estimates were based on the final grading plan design (see **Drawing GP-1**). The following assumptions were applied to the detailed model:

- Hydrologic parameters as per Ottawa Sewer Design Guidelines, including Horton infiltration, Manning's 'n', and depression storage values.
- 12-hour SCS Storm distribution for the 2 through 100-year analysis to model 'worst-case' scenario in regards to on-site storage volume and to assess proposed SWM pond performance in comparing peak site outflows to allowable release rates.
- A 4-hour 25mm storm event and 3-hour Chicago storm distributions (2, 5, and 100-year events) were used to further evaluate the proposed SWM pond performance.
- Inflows from the upstream culverts which service lands tributary to the northerly Somme Street roadside ditch have been modeled as direct constant inflow based on results of the Rational Method calculation sheets in the HIPSWM report. HIPSWM calculated Inflows for the 100-year storm have been applied conservatively to the site modeled 25-year and larger storm events, and inflows for the 10-year storm event have been applied to the modeled 2 through 10-year events.
- Percent imperviousness calculated based on actual soft and hard surfaces on each subcatchment, converted to equivalent Runoff Coefficient using the relationship $C = (\text{Imp.} \times 0.7) + 0.2$
- Subcatchment areas are defined from high-point to high-point where sags occur. Subcatchment width (average length of overland sheet flow) determined by dividing subcatchment area by subcatchment length (length of overland flow path measured from high-point to high-point), or where overland flow paths are irregular by multiplying the subcatchment area in hectares by 225.
- On-site minor system sized to convey the 2-year storm runoff without surcharging. Culverts sized to convey the 10-year storm runoff under similar conditions.
- Roadway subcatchments tributary to the Somme ditch adjacent to the development have been included within the PCSWMM model for consistency with ditch inflow calculations in the HIPSWM report.
- Number of catch basins and inlet locations based on servicing plan (**Drawing SSP-1**). Inlet top of grate elevations have been modeled 0.5m higher than physical grate elevations to allow for ponding to be modeled above the structure rim.

5.4.2 Input Parameters

Drawing SD-1 summarizes the discretized subcatchments used in the analysis of the proposed site, and outlines the major overland flow paths. The grading plans are also enclosed for review.



Appendices C3 and C4 summarize the modeling input parameters and results for the subject area; an example input and output file are provided for the 2-year and 100-year 3hr Chicago storm. For all other input files and results of storm scenarios, please examine the electronic model files provided with this report. This analysis was performed using PCSWMM, which is a front-end GUI to the EPA-SWMM engine. Model files can be examined in any program which can read EPA-SWMM files version 5.2.4.

5.4.2.1 Hydrologic Parameters

Table 5-2 presents the general subcatchment parameters used:

Table 5-2: General Subcatchment Parameters

Parameter	Value
Infiltration Method	Horton
Max. Infil. Rate (mm/hr)	76.2
Min. Infil. Rate (mm/hr)	13.2
Decay Constant (1/hr)	4.14
N Impervious	0.013
N Pervious	0.25
Dstore Imperv. (mm)	1.57
Dstore perv. (mm)	4.67
Zero Imperv. (%)	0

Table 5-3 presents the individual parameters that vary for each of the proposed subcatchments.

Table 5-3: Subcatchment Parameters

Name	Outlet	Area (ha)	Width (m)	Slope (%)	Imperv. (%)
BLDG	102A	0.183	41.1	2.0	100.0
L100A	100	0.230	51.8	2.5	42.9
L101A	105	0.366	82.3	1.5	95.7
L102A	103	0.158	35.5	1.5	100.0
L103A	103A	0.304	68.4	1.5	94.3
L104A	104	0.157	35.3	1.3	100.0
L105A	105	1.019	229.4	1.2	94.3
L107A	107	0.573	129.0	1.0	74.3
L108A	108A	0.612	137.7	1.5	77.1
L108B	108B	0.475	107.0	1.3	67.1
L110A	110	0.215	48.5	3.0	100.0
L111A	111	0.460	103.5	2.0	94.3



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L112A	112	0.285	64.2	1.5	90.0
POND	POND-S	0.285	135.0	33.0	60.0
SOMME1	C1-I	0.240	261.0	3.0	100.0
SOMME2	CULV-DS	0.030	20.0	3.0	100.0
UNC-1	RIDEAU	0.414	315.0	3.0	0.0
UNC-2	C2-I	0.353	365.0	2.0	7.1

Table 5-4 summarizes the storage node parameters used in the model. Storage curves for each node have been created based on available volumes within surface storage as applicable. Rim elevations for each node correspond to the rim elevation of the associated area's drain or catch basin plus an additional offset to allow for surface storage to be processed within the model.

Table 5-4: Storage Node Parameters

Name	Invert El. (m)	Rim Elev. (m)	Depth (m)	Curve Name	Storage Curve
100	88.91	91.00	2.09	*	FUNCTIONAL
101	88.99	92.21	3.22	*	FUNCTIONAL
102	89.60	91.74	2.14	*	FUNCTIONAL
102A	89.93	92.00	2.07	*	FUNCTIONAL
103	89.65	91.98	2.33	103V	TABULAR
103A	90.00	91.87	1.87	103AV	TABULAR
104	89.92	91.95	2.03	104V	TABULAR
105	89.45	91.11	1.66	105V	TABULAR
106	89.77	91.56	1.79	*	FUNCTIONAL
107	89.89	91.70	1.81	107V	TABULAR
108	90.08	91.63	1.55	*	FUNCTIONAL
108A	90.32	91.80	1.48	108AV	TABULAR
108B	90.32	91.80	1.48	108BV	TABULAR
109	89.74	92.21	2.47	*	FUNCTIONAL
110	89.90	91.80	1.90	110V	TABULAR
111	90.02	91.84	1.82	111V	TABULAR
112	90.07	91.87	1.80	112V	TABULAR
C1-I	88.91	90.11	1.20	*	FUNCTIONAL
C1-O	88.70	90.00	1.30	*	FUNCTIONAL
C2-I	89.58	90.86	1.28	*	FUNCTIONAL
C2-O	89.44	90.72	1.28	*	FUNCTIONAL
CULV-DS	89.82	91.07	1.25	*	FUNCTIONAL



CULV-US	89.97	91.17	1.20	*	FUNCTIONAL
POND-S	87.60	90.80	3.20	POND-V	TABULAR

5.4.2.2 Hydraulic Parameters

As per the Ottawa Sewer Design Guidelines (OSDG 2012), Manning's roughness values of 0.013 were used for sewer modeling, and a value of 0.024 used for culverts.

Storm sewers were modeled in PCSWMM to confirm flow capacities and hydraulic grade lines (HGLs) in the proposed condition with consideration of the SWM pond backwater acting on the sewers. An additional detailed storm sewer design sheet using the Rational Method is included in **Appendix B1**.

Exit losses at manholes were set for all pipe segments based on the flow angle through the structure. Exit losses were assigned as per City of Ottawa guidelines (Appendix 6b), see **Table 5.5** below.

Table 5.5: Exit Loss Coefficients for Bends at Manholes

Degrees	Coefficient
11	0.060
22	0.140
30	0.210
45	0.390
60	0.640
90	1.320
180	0.020

Entry losses for culverts have been applied using a coefficient value of 0.9 (end projecting, no headwall).

Table 5-8 below presents the parameters for the orifice and weir link objects in the model, which represent ICDs and pond outlet structure openings respectively. An additional weir represents overflow from on-site surface ponding within the southeast corner of the site, with spillage across the eastern access roadway to the Somme roadside ditch. ICDs representing circular orifice controls were assigned a discharge coefficient of 0.61, and rectangular weirs a coefficient of 1.67.

Table 5-6: Outlet/Orifice Parameters

Name	Inlet	Outlet	Inlet Elev.	Type	Diameter/Height (m)	Width (m)
107—106	107	106	89.80	CIRCULAR	0.375	-
OR1	POND-S	C1-I	89.30	CIRCULAR	0.102	-
OR2	POND-S	C1-I	89.90	RECTANGULAR	0.700	1.650



W1	105	C1-O	90.86	TRANSVERSE	0.15	6.0
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5.4.3 Model Results

The following section summarizes the key hydrologic and hydraulic model results. For detailed model results or inputs, please refer to the example input and output files in **Appendix B2 and B3**, and the electronic model files provided.

5.4.3.1 Surface Storage

The following table presents the total surface water depths (static ponding depth + dynamic flow depth, if any) on the proposed on-site access and parking areas for the 100-year design storm and climate change storm event. The worst-case modeled surface water depths have been selected when comparing the SCS storm events and modeled Chicago storm scenarios.

Table 5.7: Maximum Surface Water Depths

Storage Node / Structure ID	Top of Grate Elevation (m)	100-Year		100-Year + 20%	
		Max Water Elevation (m)	Total Surface Ponding Depth (m)	Max Water Elevation (m)	Total Surface Ponding Depth (m)
100	91.00	90.41	0.00	90.47	0.00
101	91.16	90.55	0.00	90.61	0.00
102	91.74	91.02	0.00	91.14	0.00
103	91.48	91.14	0.00	91.26	0.00
103A	91.37	91.42	0.05	91.48	0.11
104	91.45	91.41	0.00	91.48	0.03
105	90.61	90.85	0.24	90.92	0.31
106	91.56	90.97	0.00	91.03	0.00
107	91.20	91.39	0.19	91.44	0.24
108	91.63	91.46	0.00	91.52	0.00
108A	91.30	91.54	0.24	91.59	0.29
108B	91.30	91.50	0.20	91.55	0.25
109	91.46	90.89	0.00	90.99	0.00
110	91.30	91.30	0.00	91.37	0.07
111	91.34	91.48	0.14	91.56	0.22
112	91.37	91.49	0.12	91.54	0.17



5.4.3.2 Hydrologic Results

The following tables demonstrate the peak outflow from each modeled outfall during the design storm (2-100yr storm) events. Flows from UNC-2 and UNC-1 denote uncontrolled flows from the perimeter of the site that, due to grading restrictions, are captured by the existing Somme Street or Rideau Road ditches respectively. As the property had historically drained northwards to Rideau, uncontrolled flows to the Rideau Road ditch are significantly reduced from existing conditions. Uncontrolled runoff has been considered in development of overall site discharge in relation to allowable site release rates. The required SWM facility volume and outflow structure configuration was determined through iteration of each event and sized to meet the site release rate target. Results demonstrate that the SWM pond can provide adequate storage to meet restricted release rates for all events except for the 25yr 12hr SCS storm, where outflows are increased 3% above that demonstrated in the HIPSWM report. As the downstream development is able to manage runoff from the 100 year event storms where the discharge target is easily met, this minor increase is not expected to have deleterious results to downstream systems.

Table 5-8: Site Peak Discharge Rates

Event	Location	Peak Discharge Rate (L/s)	Target (L/s)
25mm, 4 Hour Chicago	OR1	14	
	OR2	0	
	W1	0	
	UNC-1	0	
	UNC-2	0	
	Total	14	202
2-Year, 3 Hour Chicago	OR1	16	
	OR2	26	
	W1	0	
	UNC-1	1	
	UNC-2	5	
	Total	48	316
5-Year, 3 Hour Chicago	OR1	17	
	OR2	129	
	W1	0	
	UNC-1	32	
	UNC-2	38	
	Total	216	487
100-Year, 3 Hour Chicago	OR1	14	
	OR2	515	



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	W1	0	
	UNC-1	139	
	UNC-2	132	
	Total	800	1066
100-Year, 3 Hour Chicago +20%	OR1	14	
	OR2	701	
	W1	146	
	UNC-1	190	
	UNC-2	174	
	Total	1225	-
2-Year, 12 Hour SCS	OR1	16	
	OR2	81	
	W1	0	
	UNC-1	14	
	UNC-2	19	
	Total	130	316
5-Year, 12 Hour SCS	OR1	17	
	OR2	279	
	W1	0	
	UNC-1	48	
	UNC-2	49	
	Total	393	487
10-Year, 12 Hour SCS	OR1	18	
	OR2	401	
	W1	0	
	UNC-1	70	
	UNC-2	65	
	Total	554	609
25-Year, 12 Hour SCS	OR1	14	
	OR2	608	
	W1	0	
	UNC-1	92	
	UNC-2	83	
	Total	797	773
50-Year, 12 Hour SCS	OR1	14	
	OR2	703	
	W1	0	



	UNC-1	107	
	UNC-2	95	
	Total	919	918
100-Year, 12 Hour SCS	OR1	14	
	OR2	786	
	W1	0	
	UNC-1	123	
	UNC-2	107	
	Total	1030	1066
100-Year, 12 Hour SCS + 20%	OR1	15	
	OR2	911	
	W1	148	
	UNC-1	153	
	UNC-2	133	
	Total	1360	-

5.4.3.3 Hydraulic Results

The City of Ottawa requires that during major storm events, the maximum hydraulic grade line be kept at least 0.30 m below the underside-of-footing (USF) of any adjacent units connected to the storm sewer during design storm events. No basements are present within the proposed design, and only one structure maintains a connection to the storm sewer for roof drainage. As the building is not anticipated to have a foundation drainage connection, the USF elevation of the building has been considered at 0.3m below the proposed finished floor of elevation 92.00 (USF=91.70). The maximum hydraulic grade line (HGL) at the building service was modeled as 91.22 for the worst case 100-year storm event, and at 91.42 for the worst case climate change storm event. The HGL elevations in both scenarios remain at least 0.30m below the proposed building USF elevation.

5.5 SWM Pond Design

As noted in sections above, the SWM pond is located in the southerly quadrant of the site, and discharges to existing roadside ditches along Somme Street. The pond has been designed to provide water quality and quantity control for the site tributary area of 5.32ha.

5.5.1 Water Quality Control

The proposed SWM Pond has been designed to achieve an “enhanced” level of treatment of urban runoff according to City of Ottawa criteria – representing an 80% removal of total suspended solids (TSS).

As the proposed SWM pond is to be privately owned and operated, the end-of-pipe facility has been designed according to the recommendations of the Ministry of the Environment Stormwater Management Planning and



Design Manual. **Table 5.9** shows the storage requirements recommended by the MECP as well as the volumes that can be provided by the proposed SWM Pond. Detailed calculations for the proposed SWM Pond are included in **Appendix B4**.

Table 5.9: SWM Pond – Water Quality Requirements

Enhanced Level of Protection – Overall Removal Efficiency of TSS 80%					
Drainage Area (ha)	Imperviousness Ratio (%)	Permanent Pool (m ³)		Extended Detention Storage (m ³)	
		Required	Provided	Required	Provided
5.32	84.0	1127	1261	213	401

For additional calculations related to drawdown time and erosion control, see Section 5.5.4.

5.5.2 Water Quantity Control

The proposed SWM Pond has been designed to attenuate post-development peak flows leaving the development site and entering the Somme Street ditch to ensure they are less than or equal to allowable flows for events up to the 100-year storm event.

Table 4.13 shows the post-development peak flow rate from the pond for the 12-hour SCS Type II distribution for the 2-year through 100-year storm events.

Table 4.10: Proposed Pond Post-Development Outflow

Scenario	Pond Outflow (L/s)
2-Year Storm	97
5-Year Storm	296
10-Year Storm	419
25-Year Storm	622
50-Year Storm	718
100-Year Storm	800

5.5.3 Pond Grading and Storage Design

The pond bottom has been set at 87.6 m in both the pond forebay and main cell. The permanent pool has been set at 89.3 m resulting in a permanent water depth of 1.7 m. The pond has been graded at maximum slope of 5:1 for 3.0 m on either side of the permanent pool, and at a maximum slope of 3:1 everywhere else, see **Drawing GP-1** for detailed grading.



The first pond outlet is a 102 mm orifice at invert at 89.3 m which was sized to retain the quality storage volume for a period greater than 24 hours as per MECP recommendations. The second pond outlet is a 1650 mm-wide by 700 mm-high weir with a weir crest at 89.90 m which was sized to meet allowable peak discharge flows to the Somme Street ditch. In event of emergency blockage of the pond outlet structure, the pond berm has been graded to provide an emergency overland flow path across the entire length of pond at elevation 90.80 to the adjacent roadside ditch line.

The stage-storage relationship for the entire facility was established using the average end area method as presented in **Appendix B4**.

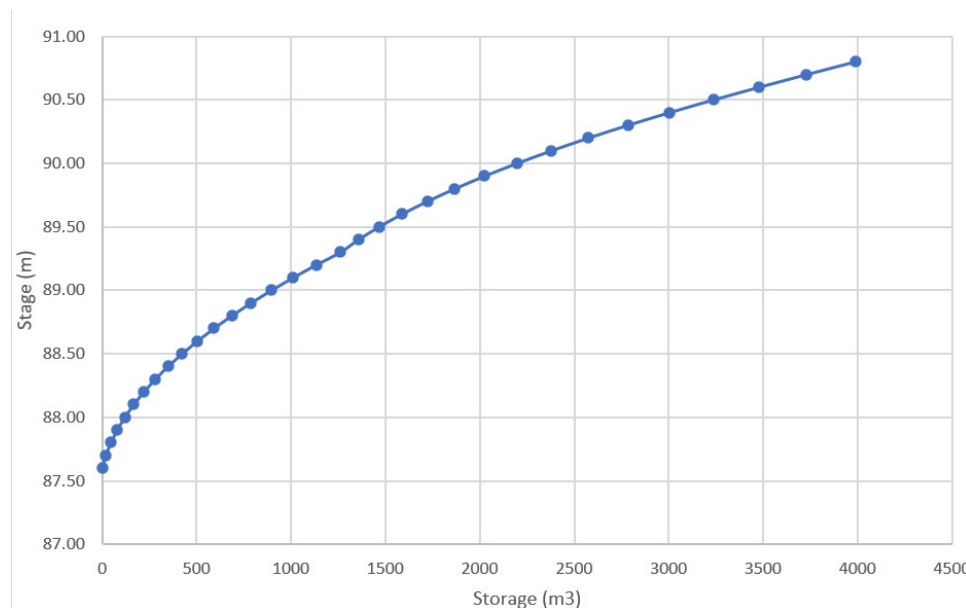


Figure 2. SWM Pond Stage-Storage Relationship

5.5.4 Slope Stability and Erosion Control

Due to the high inlet flow rates to the pond, erosion control and flow dampening measures are proposed to maintain slope stability and prevent sediment re-suspension. For side slope stability, the pond has been graded at a maximum slope of 5:1 for 3.0 m on either side of the permanent pool, and at a maximum slope of 3:1 everywhere else. Exposed slopes should be lined with biodegradable erosion control blankets and seeded with grass seed mulch once weather conditions are suitable for growth.

For erosion control purposes, as per the Ministry of the Environment Stormwater Management Planning and Design Manual, the 25 mm 4-hr Chicago event was modeled to confirm that the proposed orifice provides a minimum 24-hr drawdown time in the pond (the preferred range is 24-48 hours). Based on PCSWMM modeled results, 48 hours after the pond has reached its peak the water level has returned to within 1 cm of the permanent pool elevation, satisfying the MECP condition.



5.5.5 Inlet Design

The inlet of the SWM Pond is partially submerged, approximately 0.4 m below the permanent pool. This is due to site grading constraints and to provide sufficient cover for upstream storm sewers. A standing water elevation to the permanent pool elevation of 89.3 m has been modeled in PCSWMM to account for pond tailwater within proposed sewers.

5.5.6 Forebay Design

The purpose of the forebay is to act as the primary settling zone in the pond for the initial influx of coarse sediment and associated pollutants flushing off the sewershed. The forebay is designed to provide sufficient cross-section and length to reduce velocities and promote settling, minimize resuspension of settled solids, minimize percentage of overall permanent pool, provide sufficient sediment storage for infrequent clean out (roughly every 5 years), and have adequate accessibility and bottom-treatment for maintenance operations.

The required forebay characteristics dictate a required forebay settling length of 12.6 m. Similarly, the required forebay dispersion length is equal to 10.4 m. The provided length is approximately 17 m. The resulting average length to width ratio of the proposed configuration is approximately 3:1, also meeting MECP design recommendations. The provision of 1.7 m depth within the forebay provides for 0.7 m sediment accumulation prior to recommended cleanout while maintaining 1.0 m permanent pool depth. The designed sediment storage volume provided in the bottom 0.7 m corresponds to an estimated sediment removal frequency of approximately 5 years for the forebay. Detailed forebay calculations have been provided in **Appendix B4**.

The forebay berm to the main cell will be 0.3 m lower than the permanent pool at an elevation of 89.0 m and will be constructed of clay soil and rip-rap stone as shown on **Drawing PD-1**.

5.5.7 Outlet Design

The following subsections describe the pond outlet design. **Drawing PD-1** and **Drawing PD-2** illustrate the outlet structure in plan and sectional view.

The outlet will be located at the opposite end of the pond from the inlet and will discharge to the Somme Street ditch. A concrete outlet structure will house the required extended detention orifice, quantity control weir, and outlet pipe, including the associated maintenance infrastructure (i.e., sluice gates, etc.).

5.5.7.1 Extended Detention Control

The design of the required outlet structure incorporates a dual control configuration. Firstly, a 102 mm orifice provides an approximate 26-hour extended detention for quality control. The outlet structure maintains a reverse sloped pipe to ensure draw from deeper sections of the proposed pond for extended detention control. The entire extended detention volume is stored between 89.3 m and 89.9 m, as calculated below.

Required Storage:

Tributary Area = 5.32 ha



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Extended Detention Storage = 40 m³/ha

Storage = 5.32 x 40 = 213 m³

Provided Storage:

Forebay area at PP (89.3 m) = 420 m²

Main Cell area at PP (89.3 m) = 901 m²

Pond area at 89.9 m = 1646 m²

Provided Depth = 89.9 – 89.3 = 0.60 m

Provided Extended Detention Active Volume = 764 m³

5.5.7.2 Flow Control Weir

Quantity control of the pond discharge above the extended detention elevation is provided by a flow control weir within the outlet structure. This weir has been incorporated into the outlet structure to lower the water levels in the pond, while meeting the target peak outflows to the Somme Street ditch. A 1,650mm-long by 700 mm-high weir with invert at 89.9 m is proposed to meet quantity control requirements.

5.5.7.3 Stage-Discharge Relationship

A stage-discharge relationship was estimated using standard orifice and weir equations as outlined in **Appendix B4**. The resulting stage-discharge relationship is presented in **Figure 11** below. Note that the PCSWMM model outputs reduced discharge values for larger storm events in consideration of tailwater elevations within the receiving ditch.



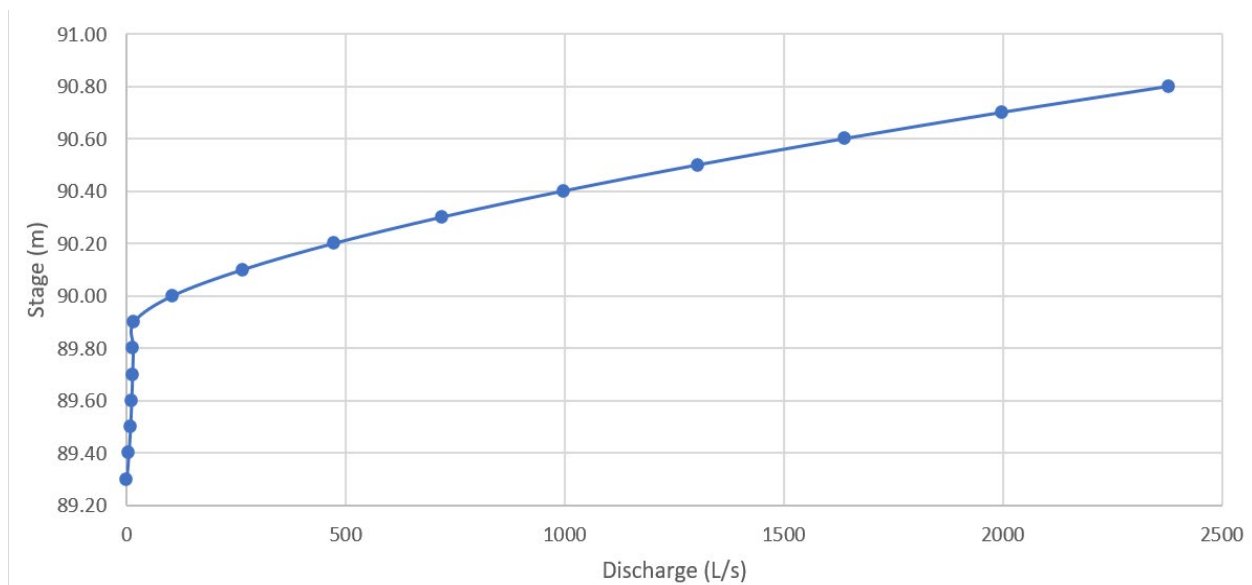


Figure 3. SWM Pond Stage-Discharge Relationship

5.5.7.4 Outlet Sewer

A 900mm x 1800mm rectangular concrete sewer conveys pond discharge from the outlet structure to the outlet headwall discharging directly to the Somme Street ditch. The receiving ditch is further lined with rip-rap stone for erosion control as shown on **Drawing PD-1**.

5.5.8 Pond Performance

As **Table 5.11** indicates, the water quality objectives of the SWM facility are met by providing extended detention of 24-48 hours and exceeding the MECP recommended water quality volumes.



Table 5.11: SWM Facility Operational Characteristics

SWM Basin Parameters	Basin Value
Total Contributing Area	5.32 ha
Imperviousness of Contributing Area (of Sewershed Area)	84.0 %
Unit Area Storage Volume Requirements as per SWMPD Manual	251.8 m ³ /ha
Required Total Water Quality Volume	1340 m ³
Wet Pond Bottom Elevation	87.6 m
Required Permanent Pool Volume	1127 m ³
Permanent Pool Volume Provided	1261 m ³
Permanent Pool Elevation	89.3 m
Permanent Pool Surface Area	1,321 m ²
Required Extended Detention Volume	213 m ³
Extended Detention Volume Provided	764 m ³
Peak Release Rate for Extended Detention	0.016 m ³ /s
Extended Detention Drawdown Time	26 hours
Extended Detention Elevation	89.9 m
5-Year Storm Maximum Ponding Level (12-hour SCS Type II)	90.12 m
5-Year Storm Peak Pond Release (12-hour SCS Type II)	0.296 m ³ /s
100-Year Storm Maximum Ponding Level (12-hour SCS Type II)	90.33 m
100-Year Storm Peak Pond Release (12-hour SCS Type II)	0.800 m ³ /s
100-Year Storm Active Volume Required (12-hour SCS Type II)	2,019 m ³
Top of Berm (minimum grade of surrounding properties)	90.8 m
Forebay Parameters	
Forebay Bottom Elevation	87.6 m
Sediment Accumulation Depth	0.7 m
Forebay Depth from Permanent Pool	1.7 m
Required Forebay Length	12.6 m
Actual Forebay Length	17 m
Clean Out Frequency	~5 years
Outlet Parameters	
Quality Orifice Size	102 mm
Quality Orifice Invert	89.30 m
Quantity Weir Size	1,650mm-W x 700mm-H
Quantity Weir Crest Invert	89.90 m



5.5.9 Spill Containment

The proposed pond is to form part of the overall spills containment strategy for the site required for compliance with requirements of the Technical Standards and Safety Authority (TSSA) including those under the Liquid Fuels Handling Code.

The proposed pond outlet structure is equipped with a reverse sloped pipe on the quality control outlet in order to ensure minor spills of floatable fuels and debris do not migrate to downstream ditches under normal operating conditions. The pond forebay provides additional settling and dispersion lengths to allow for heavier sediment or dense liquids to be trapped within the pond forebay behind the impermeable forebay berm.

The pond bottom is equipped with a 0.75m thick clay layer underlaid by filter fabric and an impermeable geomembrane to prevent groundwater ingress to the pond, and also prevent pond exfiltration to surrounding soils, limiting contaminant transport.

Finally, the pond outlet pipe is to maintain an automated sluice gate within the outlet structure for shielding from the elements. The sluice gate is to be designed by the mechanical & electrical consultants under separate cover to allow for emergency shutoff of any pond outflow should a major spill be identified by site operations staff. In this manner, runoff from fire suppression activities will be transported by on-site sewer and drainage piping to the pond for safe containment and removal without adverse effects to downstream stormwater systems.

5.5.10 Dry Hydrant Supply

Draw for two on-site dry hydrants has been provided within the pond main cell with the intent of provision of required fire flow volumes to meet City of Ottawa design criteria as outlined in the recent Technical Bulletin IWSTB-2024-05. As identified in Section 3 above, the site maintains a required fire flow volume per OBC calculations of approximately 337m³. This volume is to be provided within the pond main cell, less any ice thickness that would contribute to a reduction of available pond volume under winter conditions.

Per section 4.3.1 of the SWMPDM, ice thickness within the pond can be determined via Stefan's Equation, where the ice thickness in mm equates to the root of the annual sum of freezing degree-days for the region multiplied by a coefficient of ice growth. Coefficients of ice growth are noted within the SWMPDM as between 14-17 for average rivers with snow, 17-24 for average lakes with snow, and approaching a theoretical maximum of 34. The SWMPDM further notes that "most ponds will be small enough and will receive sufficient inflow to behave more like a river (in terms of ice build-up) than a lake". In either case, the proposed wet pond will be subject to snow accumulation.

Climate normals sourced from Environment Canada for Ottawa for the years 1991-2020 identify a mean sum of annual freezing degree-days of 909.3C.



Based on the above, the pond could reasonably expect an ice depth to occur between 513mm (using coefficient of ice growth of 17 for average rivers with snow) to 724mm (using a coefficient of ice growth of 24 for average lakes with snow).

Based on the proposed pond stage storage curve for main cell volume at elevations of 88.787 to 88.576, the available water for dry hydrant supply would be approximately 340-460m³.

5.5.11 SWM Pond Operations and Maintenance

As with any SWM facility, maintenance will be required to ensure the operational efficiency and functionality of the proposed SWM Pond. As this is a private site, the developer will be responsible for any maintenance/operation/monitoring works. The preparation of an Operations, Maintenance and Monitoring Manual for the SWM Pond will be required after issuance of an MECP ECA for the works based on criteria outlined in the ECA, and submitted under separate cover.

5.5.11.1 Maintenance Program

The following summarizes the key components to be considered for the maintenance program:

- **Inventory of Stormwater System Components** (e.g., conveyance locations, elevations, outfalls, contributing drainage area, receiving watercourse, control structure components and specifications, material types, vegetative species, and other pertinent information)
- **Periodic and Scheduled Inspections** (performed on a regular basis as determined in consultation with the City, MECP, and conservation authority)
- **Maintenance Scheduling and Performance** (e.g., vegetation/landscaping maintenance, trash and debris removal, dewatering, sediment removal and disposal, pond bank stabilization, inlet/outlet structure repairs, equipment testing/troubleshooting, etc.)
- **Documentation and Reporting** (e.g., frequent inspection reports to the municipality and conservation authority during construction).

5.5.11.2 Monitoring

Monitoring of the proposed SWM Pond will be conducted to ensure proper hydraulic and water quality performance of the facility as designed. A detailed monitoring plan will be prepared following the receipt of MECP Environmental Compliance Approval (ECA). Monitoring criteria outlined in the MECP ECA, as required by the City of Ottawa and OFS for fire suppression qualities of the pond, and per site-wide spill control procedures will be used to prepare a monitoring plan for the facility.

5.5.11.3 SWM Pond Dewatering Procedure

A 300 mm diameter opening complete with a sluice gate at elevation 87.6 m has been incorporated within the outlet structure to allow for the water level in the pond main cell to be drained completely by lowering a sump pump into the outlet structure and directing pumping discharge into the outlet pipe or other approved location for disposal. In addition, the forebay berm has been designed to be impervious with clay to allow for



water level separation between the forebay and the main cell. A drawdown structure has been provided next to the forebay berm with a 300 mm diameter pipe connecting the bottom of the forebay to the drawdown structure and a second 300 mm diameter pipe connecting the drawdown structure to the bottom of the main cell. Sluice gates for both pipes have been incorporated within the drawdown structure to facilitate maintenance. As the forebay bottom and the main cell bottom are at the same elevation, the forebay could be drained by gravity into the main cell once the main cell is empty, by opening both sluice gates. Alternatively, a sump pit has been provided in the drawdown structure to allow for the forebay to be drained with a sump pump from the drawdown structure. It is of note that full draw down of the pond will temporarily remove available site water storage for fire suppression within the pond and will dewater connected dry hydrants. Appropriate measures for emergency fire suppression must be in place before pond dewatering.

5.5.11.4 Sediment Removal and Storage

The pond forebay area is located such that sediment removal activities can be conducted with access from the north and west end of the forebay.

Sediment removal is estimated to be required at approximately 5-year intervals or at a sediment accumulation depth of 0.7 m in the forebay. Sediment accumulation within the forebay is variable and dependent on many factors including the condition of the tributary lands and the frequency of rainfall events. Therefore, sediment removal may be required at shorter or longer intervals than 5 years. Monitoring of sediment depth should be conducted regularly, and a hydrometric survey should be completed at regular intervals to confirm sediment accumulation rates. Sediment accumulation within the main cell will occur at lower rates and therefore require less frequent clean-outs than the forebay cell.

Any sediment removed from the forebay is expected to be disposed of off-site, and must be analyzed under Ontario Regulation 347 criteria. Analytical results must meet inert fill requirements if the sediment is to be used for land application and meet non-hazardous requirements as per the TCLP leachate test in O.Reg. 347 if it is to be disposed of in a municipal landfill.

5.6 Water Balance

As identified in the Shields Creek Subwatershed Study (SCSS), groundwater recharge for the overall Hawthorne Industrial Park development was intended to provide subsurface base flow into the receiving Findlay Creek. The HIPSWM report identifies that construction fill operations on site have been active since 1994. The report further clarifies that the soil component of the fill exhibits the characteristics of silty clay, with varying composition affecting its permeability in localized areas, making groundwater recharge behaviour within the HIP difficult to determine. The MECP does not endorse the use of infiltration basins on lands zoned for industrial use as there is an increased risk of groundwater contamination should a spill occur on site.

Nonetheless, options for infiltration were developed at time of preparation of the HIPSWM report that were subject to difficulty due to high groundwater tables and limited infiltration rates of subdivision soils. Representatives of the City of Ottawa and SNC were consulted, and it was concluded that groundwater



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balance targets from the SCSS would be difficult to achieve, and that on-site infiltration strategies would jeopardize groundwater quality and natural ecological integrity of receiving waters. It was decided by approval authorities for the HIP development that the requirement for water balance would be waived (see excerpts in **Appendix C**).



6 Site Grading

The project limit area of the site measures approximately 6.05 ha in area. A detailed site servicing and grading plan (see **Drawing GP-1**) has been prepared to satisfy the stormwater management requirements described in **Section 5** and to allow for positive drainage away from the face of the proposed building.

The topographic survey plan indicates that the site has been recently subject to infilling activity, with site drainage now occurring via sheet flows generally to the southeast, with a high (roughly 10m in some sections) embankment along the property's northwest boundary with Rideau Street. This embankment is to be managed under proposed conditions through construction of an on-site retaining wall with design by others (see Paterson Group Geotechnical Investigation Report PG7567 for details).

The site servicing and grading plan satisfies the grading and drainage objectives for the proposed development site. The proposed grading respects the existing grades at the property lines and provides adequate overland flow routes to municipal rights-of-way (Somme Street). The site's eastern property boundary is proposed to be raised above existing conditions with fill being placed outside of the current development block. This fill has been identified to and is acceptable to the property owner (Tomlinson Group) who is also providing the current development block for sale to W.O. Stinson & Son Ltd.. The site grading has been designed to maintain the existing drainage patterns for the non-surface storage areas.

7 Utilities

Overhead (OH) hydro-wires run parallel to the westerly portion of the site along the Somme Street right of way, adjacent to the project limit area. Utilities within the work area (pole anchors) may require temporary relocation during construction. The existing utility poles within the public right of way are to be protected during construction.

Being part of an industrial development subdivision, Hydro Ottawa, Bell, Rogers, and Enbridge servicing is readily available through existing infrastructure to service this site. The exact size, location, and routing of utilities will be finalized after design circulation. Existing overhead wires and utility plants may need to be temporarily moved/reconfigured to allow sufficient clearance for the movement of heavy machinery required for construction of the adjacent septic system. The relocation of existing utilities will be coordinated with the individual utility providers upon design circulation.



8 Approvals

The proposed development lies on a private site under singular ownership and is considered an industrial use. As such, the site is subject to the Ministry of the Environment, Conservation and Parks (MECP) Environmental Compliance Application (ECA) process. An existing ECA has been established for sewage and stormwater management works within the downstream Hawthorne Industrial Park (HIP), to which the site is to discharge. The ECA identifies that individual sites discharging to the HIP are required to not only obtain approval per section 53 of the OWRA, but also amend the existing ECA to demonstrate adequate quantity and quality of site discharge to the overall collection system.

For ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). It is possible that groundwater may be encountered during the pond excavation on this site. A minimum of two to four weeks should be allotted for completion of the EASR registration and the preparation of the Water Taking and Discharge Plan by a Qualified Person as stipulated under O.Reg. 63/16. An MECP Permit to Take Water (PTTW), which is required for dewatering volumes exceeding 400,000L/day, is not anticipated for the site.



9 Erosion and Sediment Control During Construction

To protect downstream water quality and prevent sediment build-up in catch basins and storm sewers, erosion and sediment control measures must be implemented during construction. The following recommendations will be included in the contract documents and communicated to the Contractor.

1. Implement best management practices to provide appropriate protection of the existing and proposed drainage system and the receiving water course(s).
2. Limit the extent of the exposed soils at any given time.
3. Re-vegetate exposed areas as soon as possible.
4. Minimize the area to be cleared and grubbed.
5. Protect exposed slopes with geotextiles, geogrid, or synthetic mulches.
6. Install silt barriers/fencing around the perimeter of the site as indicated in **Drawing ECDS-1** to prevent the migration of sediment offsite.
7. Install trackout control mats (mud mats) at the entrance/egress to prevent migration of sediment into the public ROW.
8. Provide sediment traps and basins during dewatering works.
9. Install sediment traps (such as SiltSack® by Terrafix) between catch basins and frames.
10. Schedule the construction works at times which avoid flooding due to seasonal rains.

The Contractor will also be required to complete inspections and guarantee the proper performance of their erosion and sediment control measures at least after every rainfall. The inspections are to include:

- Verification that water is not flowing under silt barriers.
- Cleaning and changing the sediment traps placed on catch basins.

Refer to **Drawing ECDS-1** for the proposed location of silt fences, sediment traps, and other erosion control measures.



10 Geotechnical Investigation

A geotechnical investigation report was prepared by Paterson Group on August 14, 2025, to provide an assessment of the subsurface conditions for the proposed development. The subsurface profile generally consists of approximately 2.3 m to 10 m thickness of fill, underlaid by a layer of clayey silt to silty clay and/or silty sand. Fill material was observed to consist of silty sand/silty clay with varying amounts of organics, gravel, asphalt, concrete and bedrock fragments. Bedrock is anticipated between 5.3 to 12m below ground surface.

Based on Paterson's recommendations, the site is suitable for the proposed development. The proposed industrial building is recommended to be founded on conventional spread footings placed on existing fill which has already undergone a ground improvement program. Due to presence of a silty clay deposit, the site is subject to a permissible grade raise restriction of 2 m outside of ground improvement program limits.

The recommended rigid pavement structure is further presented in **Table 10-1** below.

Table 10-1: Recommended Pavement Structure

Material	Car Only Parking Areas (mm)	Access Lanes/Local Roadways, Loading Areas, and Heavy Truck Parking Areas (mm)
Wear Course – Superpave 12.5 Asphaltic Concrete	50	40
Binder Course – Superpave 19.0 Asphaltic Concrete	-	50
BASE – OPSS Granular A Crushed Stone	150	150
SUBBASE – OPSS Granular B Type II	300	450

Refer to the geotechnical report excerpts attached in **Appendix C** and the full report as part of the site submission package for further details.



11 Conclusions

11.1 Water Servicing

An existing on-site private well has sufficient capacity to meet domestic demands for the site per Terrain Analysis and Hydrogeological by others. Required fire flows for on-site fire suppression are to be met by two on-site dry hydrants fed from a privately maintained stormwater management facility. The facility has sufficient permanent pool volume to meet fire flow requirements as determined by OBC methodology.

11.2 Sanitary Servicing

The proposed sanitary sewer service will consist of a 125 mm diameter sanitary service lateral sleeved through a 200mm pipe to direct discharge to a proposed on-site sewage treatment and disposal system. Sizing and design of the sewage system are by others and as noted in the Terrain Analysis and Hydrogeological Study.

11.3 Stormwater Servicing and Management

Surface storage at catch basins within parking areas and within a proposed wet pond stormwater management facility has been proposed to limit the stormwater discharge rate to peak allowable release rates as identified by the overarching Stormwater Management Report for the Hawthorne Industrial Park. The remaining site area not identified for redevelopment drains uncontrolled as per existing conditions and ultimately to the existing ditch along Rideau Road. The proposed wet pond provides sufficient storage to meet water quality and quantity requirements per MECP and City of Ottawa criteria, in addition to providing a permanent water source for on-site fire suppression. Spill control measures are incorporated within the proposed pond for additional protection to downstream watercourses. Amendment to the subdivision ECA as well as a separate ECA via direct submission to the MECP is required for site development in relation to storm sewers / stormwater management.

11.4 Grading

Site grading has been designed to provide an adequate emergency overland flow route and adhere to the permissible grade raise restrictions outlined in the geotechnical investigation.

11.5 Geotechnical Investigation

Based on the geotechnical investigation, the site is considered suitable for the proposed development. The proposed industrial building is recommended to be founded on conventional spread footings placed over existing fill. Due to presence of a silty clay deposit, the site is subject to a permissible grade raise restriction of 2 m.



Appendices



Appendix A Water Servicing

A.1 OBC and FUS Fire Flow Demand Calculations



Fire Flow Calculations as per Ontario Building Code 2024 (Appendix A)

Job# 160402123
Date 21-Aug-25

Designed by: DT
Checked by: PM
Description: Tank & Storage Bldg

$$Q = KVS_{\text{tot}}$$

Q = Volume of water required (L)

V = Total building volume (m³)

K = Water supply coefficient from Table 1

S_{tot} = Total of spatial coefficient values from property line exposures on all sides as obtained from the formula

$$S_{\text{tot}} = 1.0 + [S_{\text{side1}} + S_{\text{side2}} + S_{\text{side3}} + S_{\text{side4}}]$$

1	Type of construction	Building Classification		Water Supply Coefficient
	Non-Combustible with Fire-Resistance Ratings	F-1		23
2	Area of one floor (m ²)	number of floors	height of ceiling (m)	Total Building Volume (m ³)
	3483	1	6	20,898
3	Side	Exposure Distance (m)	Spatial Coefficient	Total Spatial Coefficient
	North	30	0	1
	East	30	0	
	South	30	0	
	West	36.5	0	
4	Established Fire Safety Plan?	Reduction in Volume (%)		Total Volume Reduction
	yes	30%		30%
5	Total Volume 'Q' (L)			
				336,458
				Minimum Required Fire Flow (L/min)
				9,000

*NOTE: Buildings adequately spaced to ensure no spatial coefficient contribution

Average height of 6.0m assumed.

Fire Flow Calculations as per Ontario Building Code 2024 (Appendix A)

Job# 160402123
Date 21-Aug-25

Designed by: DT
Checked by: PM
Description: Service Bays

$$Q = KVS_{\text{tot}}$$

Q = Volume of water required (L)

V = Total building volume (m³)

K = Water supply coefficient from Table 1

S_{tot} = Total of spatial coefficient values from property line exposures on all sides as obtained from the formula

$$S_{\text{tot}} = 1.0 + [S_{\text{side1}} + S_{\text{side2}} + S_{\text{side3}} + S_{\text{side4}}]$$

1	Type of construction	Building Classification		Water Supply Coefficient
	Non-Combustible with Fire-Resistance Ratings	E, F-2		17
2	Area of one floor (m ²)	number of floors	height of ceiling (m)	Total Building Volume (m ³)
	1662	1	6	9,972
3	Side	Exposure Distance (m)	Spatial Coefficient	Total Spatial Coefficient
	North	30	0	1
	East	36.5	0	
	South	30	0	
	West	30	0	
4	Established Fire Safety Plan?	Reduction in Volume (%)		Total Volume Reduction
	yes	30%		30%
5	Total Volume 'Q' (L)			
				118,667
				Minimum Required Fire Flow (L/min)
				3,600

*NOTE: Buildings adequately spaced to ensure no spatial coefficient contribution

Average height of 6.0m assumed.

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160402123
Project Name: 301 Somme
Date: 8/21/2025

Fire Flow Calculation #: 1
Description: Tank & Equipment Storage Building

Notes: OBC F-1 building per 3.2.2.73

Step	Task	Notes							Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction							0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Six Additional Floors					Vertical Openings Protected?		NO	-
		3483	0	0	0	0	0	0	3483	-
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min							-	10000
4	Determine Occupancy Charge	Rapid Burning							25%	12500
5	Determine Sprinkler Reduction	Conforms to NFPA 13							-30%	-5000
		Non-Standard Water Supply or N/A							0%	
		Fully Supervised							-10%	
		% Coverage of Sprinkler System							100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	Firewall / Sprinklered ?	-	-
		North	> 30	0	0	0-20	Type V	NO	0%	0
		East	> 30	0	0	0-20	Type V	NO	0%	
		South	> 30	0	0	0-20	Type V	NO	0%	
		West	> 30	0	0	0-20	Type V	NO	0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							8000	
		Total Required Fire Flow in L/s							133.3	
		Required Duration of Fire Flow (hrs)							2.00	
		Required Volume of Fire Flow (m³)							960	

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160402123

Project Name: 301 Somme

Date: 8/21/2025

Fire Flow Calculation #: 2

Description: Service Bays

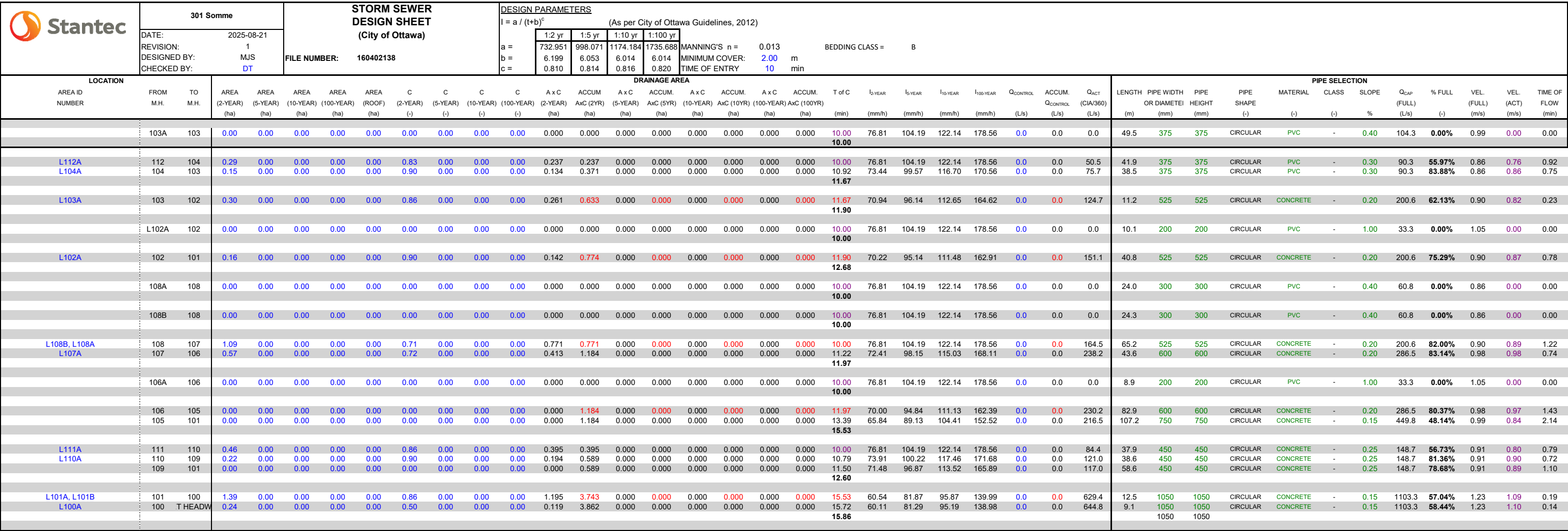
Notes: OBC F-2 building per 3.2.2.81

Step	Task	Notes							Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction							0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Six Additional Floors					Vertical Openings Protected?		NO	-
		1662	0	0	0	0	0	0	0	1662
3	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min							-	7000
4	Determine Occupancy Charge	Rapid Burning							25%	8750
5	Determine Sprinkler Reduction	Conforms to NFPA 13							-30%	-3500
		Non-Standard Water Supply or N/A							0%	
		Fully Supervised							-10%	
		% Coverage of Sprinkler System							100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	Firewall / Sprinklered ?	-	-
		North	> 30	0	0	0-20	Type V	NO	0%	0
		East	> 30	0	0	0-20	Type V	NO	0%	
		South	> 30	0	0	0-20	Type V	NO	0%	
		West	> 30	0	0	0-20	Type V	NO	0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							5000	
		Total Required Fire Flow in L/s							83.3	
		Required Duration of Fire Flow (hrs)							1.75	
		Required Volume of Fire Flow (m³)							525	

Appendix B Stormwater Servicing and Management

B.1 Storm Sewer Design Sheet





B.2 PCSWMM Model Input File (100-Year, 12-Hour SCS Storm)



[TITLE]
;;Project Title/Notes

[OPTIONS]
;;Option Value
FLOW_UNITS LPS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING YES
SKIP_STEADY_STATE NO

START_DATE 07/11/2025
START_TIME 00:00:00
REPORT_START_DATE 07/11/2025
REPORT_START_TIME 00:00:00
END_DATE 07/12/2025
END_TIME 00:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 1
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5

THREADS 8

[EVAPORATION]
;;Data Source Parameters
;;-----
CONSTANT 0.0
DRY_ONLY NO

[RAINGAGES]
;;Name Format Interval SCF Source
;;-----
RG1 INTENSITY 0:15 1.0 TIMESERIES 100S12

[SUBCATCHMENTS]
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen SnowPack
;;-----
;;-----
;0.90
BLDG RG1 102A 0.182749 100 41.119 2 0
;0.50
L100A RG1 100 0.230191 42.857 51.793 2.5 0
;0.87
L101A RG1 105 0.365744 95.714 82.292 1.5 0
;0.90
L102A RG1 103 0.157645 100 35.47 1.5 0
;0.86
L103A RG1 103A 0.303883 94.286 68.374 1.5 0
;0.90
L104A RG1 104 0.15688 100 35.298 1.3 0
;0.86
L105A RG1 105 1.019332 94.286 229.35 1.2 0

```

;0.72
L107A      RG1      107      0.573411 74.286 129.017 1      0

;0.74
L108A      RG1      108A     0.611783 77.143 137.651 1.5      0

;0.67
L108B      RG1      108B     0.475361 67.143 106.956 1.3      0

;0.90
L110A      RG1      110      0.215451 100     48.476 3      0

;0.86
L111A      RG1      111      0.459814 94.286 103.458 2      0

;0.83
L112A      RG1      112      0.28538 90      64.211 1.5      0

;0.62
POND       RG1      POND-S    0.284814 60     135     33      0

SOMME1     RG1      C1-I      0.24     100     261     3      0

SOMME2     RG1      CULV-DS   0.03     100     20      3      0

;0.20
UNC-1      RG1      RIDEAU    0.413886 0      315     3      0

;0.25
UNC-2      RG1      C2-I      0.353003 7.14    365     2      0

```

```

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted
;;-----
BLDG          0.013    0.25    1.57    4.67    0      OUTLET
L100A         0.013    0.25    1.57    4.67    0      OUTLET
L101A         0.013    0.25    1.57    4.67    0      OUTLET
L102A         0.013    0.25    1.57    4.67    0      OUTLET

```

```

L103A         0.013    0.25    1.57    4.67    0      OUTLET
L104A         0.013    0.25    1.57    4.67    0      OUTLET
L105A         0.013    0.25    1.57    4.67    0      OUTLET
L107A         0.013    0.25    1.57    4.67    0      OUTLET
L108A         0.013    0.25    1.57    4.67    0      OUTLET
L108B         0.013    0.25    1.57    4.67    0      OUTLET
L110A         0.013    0.25    1.57    4.67    0      OUTLET
L111A         0.013    0.25    1.57    4.67    0      OUTLET
L112A         0.013    0.25    1.57    4.67    0      OUTLET
POND          0.013    0.25    1.57    4.67    0      OUTLET
SOMME1        0.013    0.25    1.57    4.67    0      OUTLET
SOMME2        0.013    0.25    1.57    4.67    0      OUTLET
UNC-1         0.013    0.25    1.57    4.67    0      OUTLET
UNC-2         0.013    0.25    1.57    4.67    0      PERVIOUS 100

```

```

[INFILTRATION]
;;Subcatchment Param1 Param2 Param3 Param4 Param5
;;-----
BLDG          76.2    13.2    4.14    7      0
L100A         76.2    13.2    4.14    7      0
L101A         76.2    13.2    4.14    7      0
L102A         76.2    13.2    4.14    7      0
L103A         76.2    13.2    4.14    7      0
L104A         76.2    13.2    4.14    7      0
L105A         76.2    13.2    4.14    7      0
L107A         76.2    13.2    4.14    7      0
L108A         76.2    13.2    4.14    7      0
L108B         76.2    13.2    4.14    7      0
L110A         76.2    13.2    4.14    7      0
L111A         76.2    13.2    4.14    7      0
L112A         76.2    13.2    4.14    7      0
POND          76.2    13.2    4.14    7      0
SOMME1        76.2    13.2    4.14    7      0
SOMME2        76.2    13.2    4.14    7      0
UNC-1         76.2    13.2    4.14    7      0
UNC-2         76.2    13.2    4.14    7      0

```

```

[OUTFALLS]
;;Name Elevation Type Stage Data Gated Route To

```

```

;;-----
RIDEAU      0      FREE      NO
SOMME      88.31    FREE      NO

[STORAGE]
;;Name      Elev.  MaxDepth  InitDepth  Shape      Curve Name/Params      SurDepth  Fevap
Psi      Ksat      IMD
;;-----
;2400mm
100      88.914    2.086    0.386    FUNCTIONAL  0      0      1.13    0      0
;3000mm
101      88.993    3.217    0.307    FUNCTIONAL  0      0      1.13    0      0
;1500mm
102      89.6      2.14     0        FUNCTIONAL  0      0      1.13    0      0
;Null Structure
102A     89.926    2.074    0        FUNCTIONAL  0      0      1.13    0      0
;1500mm
103      89.652    2.328    0        TABULAR     103V      0      0
;CB
103A     90      1.87     0        TABULAR     103AV      0      0
;1200mm
104      89.917    2.033    0        TABULAR     104V      0      0
;1800mm
105      89.454    1.656    0        TABULAR     105V      0      0
;1800mm
106      89.77     1.79     0        FUNCTIONAL  0      0      1.13    0      0
;1500mm
107      89.887    1.813    0        TABULAR     107V      0      0
;1500mm
108      90.077    1.553    0        FUNCTIONAL  0      0      1.13    0      0
;CB
108A     90.323    1.477    0        TABULAR     108AV      0      0
;CB
108B     90.324    1.476    0        TABULAR     108BV      0      0
;1200mm
109      89.74     2.47     0        FUNCTIONAL  0      0      1.13    0      0
;1200mm
110      89.897    1.903    0        TABULAR     110V      0      0

```

```

;CB
111      90.022    1.818    0        TABULAR     111V      0      0
;CB
112      90.073    1.797    0        TABULAR     112V      0      0
C1-I     88.91     1.2      0        FUNCTIONAL  0      0      0      0      0
C1-O     88.7      1.3      0        FUNCTIONAL  0      0      0      0      0
C2-I     89.58     1.28     0        FUNCTIONAL  0      0      0      0      0
C2-O     89.44     1.28     0        FUNCTIONAL  0      0      0      0      0
CULV-DS  89.82     1.25     0        FUNCTIONAL  0      0      0      0      0
CULV-US  89.97     1.2      0        FUNCTIONAL  0      0      0      0      0
POND-S   87.6      3.2      1.7      TABULAR     POND-V      0      0

[CONDUITS]
;;Name      From Node      To Node      Length      Roughness  InOffset  OutOffset  InitFlow
MaxFlow
;;-----
;1050mm
100--POND-S  100      POND-S      9.071      0.013      88.914      88.9      0      0
;1050mm
101--100     101      100      12.497      0.013      88.993      88.974      0      0
;525mm
102--101     102      101      40.781      0.013      89.6      89.518      0      0
;200mm
102A--102    102A     102      10.105      0.013      89.926      89.825      0      0
;525mm
103--102     103      102      11.225      0.013      89.652      89.63      0      0
;375mm
103A--103    103A     103      49.469      0.013      90      89.802      0      0
;375mm
104--103     104      103      38.488      0.013      89.917      89.802      0      0
;750mm

```

105--101	105	101	107.237	0.013	89.454	89.293	0	0
;600mm								
106--105	106	105	82.887	0.013	89.77	89.604	0	0
;525mm								
108--107	108	107	65.244	0.013	90.077	89.947	0	0
;375mm								
108A--108	108A	108	24.002	0.013	90.323	90.227	0	0
;375mm								
108B--108	108B	108	24.344	0.013	90.324	90.227	0	0
;450mm								
109--101	109	101	58.639	0.013	89.74	89.593	0	0
;450mm								
110--109	110	109	38.618	0.013	89.897	89.8	0	0
;450mm								
111--110	111	110	37.879	0.013	90.022	89.927	0	0
;375mm								
112--104	112	104	41.886	0.013	90.073	89.947	0	0
C1	C1-I	C1-O	23.9	0.024	88.95	88.7	0	0
C2	C2-I	C2-O	21.8	0.024	89.58	89.44	0	0
C3	CULV-DS	C2-I	40.4	0.025	89.87	89.66	0	0
C4	CULV-US	CULV-DS	20.84	0.024	89.97	89.82	0	0
D1	C1-O	SOMME	74.86	0.025	88.8	88.31	0	0
D2	C2-O	C1-I	92.8	0.025	89.52	88.91	0	0

[ORIFICES]							
;;Name	From Node	To Node	Type	Offset	Qcoeff	Gated	CloseTime
;;-----							
;600mm							
107--106	107	106	SIDE	89.8	0.61	NO	0
OR1	POND-S	C1-I	SIDE	89.3	0.61	NO	0

[WEIRS]								
;;Name	From Node	To Node	Type	CrestHt	Qcoeff	Gated	EndCon	
EndCoeff	Surcharge	RoadWidth	RoadSurf	Coeff.	Curve			
;;-----								
OR2	POND-S	C1-I	TRANSVERSE	89.9	1.67	NO	0	0
YES								
W1	105	C1-O	TRANSVERSE	90.86	1.67	NO	0	0
YES								

[XSECTIONS]							
;;Link	Shape	Geom1	Geom2	Geom3	Geom4	Barrels	Culvert
;;-----							
100--POND-S	CIRCULAR	1.05	0	0	0	1	
101--100	CIRCULAR	1.05	0	0	0	1	
102--101	CIRCULAR	0.525	0	0	0	1	
102A--102	CIRCULAR	0.3	0	0	0	1	
103--102	CIRCULAR	0.525	0	0	0	1	
103A--103	CIRCULAR	0.375	0	0	0	1	
104--103	CIRCULAR	0.375	0	0	0	1	
105--101	CIRCULAR	0.75	0	0	0	1	
106--105	CIRCULAR	0.6	0	0	0	1	
108--107	CIRCULAR	0.525	0	0	0	1	
108A--108	CIRCULAR	0.375	0	0	0	1	
108B--108	CIRCULAR	0.375	0	0	0	1	
109--101	CIRCULAR	0.45	0	0	0	1	
110--109	CIRCULAR	0.45	0	0	0	1	
111--110	CIRCULAR	0.45	0	0	0	1	
112--104	CIRCULAR	0.375	0	0	0	1	
C1	ARCH	0.97	1.39	0	0	2	
C2	ARCH	0.82	1.15	0	0	2	
C3	TRIANGULAR	1.2	7.2	0	0	1	

C4	ARCH	0.82	1.15	0	0	2
D1	TRIANGULAR	1.2	7.2	0	0	1
D2	TRIANGULAR	1.2	7.2	0	0	1
107--106	CIRCULAR	0.375	0	0	0	
OR1	CIRCULAR	0.102	0	0	0	
OR2	RECT_OPEN	0.7	1.65	0	0	
W1	RECT_OPEN	0.15	6	0	0	

[LOSSES]

;;Link	Kentry	Kexit	Kavg	Flap Gate	Seepage
100--POND-S	0	0.9	0	NO	0
101--100	0	1.344	0	NO	0
102--101	0	1.344	0	NO	0
102A--102	0	1.344	0	NO	0
103--102	0	0.444	0	NO	0
103A--103	0	0.444	0	NO	0
104--103	0	0.229	0	NO	0
105--101	0	0.259	0	NO	0
106--105	0	1.344	0	NO	0
108--107	0	1.26	0	NO	0
108A--108	0	1.344	0	NO	0
108B--108	0	1.344	0	NO	0
109--101	0	0.053	0	NO	0
110--109	0	1.141	0	NO	0
111--110	0	1.344	0	NO	0
112--104	0	1.061	0	NO	0
C1	0.9	0	0	NO	0
C2	0.9	0	0	NO	0
C4	0.9	0	0	NO	0

[INFLOWS]

;;Node	Constituent	Time Series	Type	Mfactor	Sfactor	Baseline Pattern
CULV-US	FLOW	" "	FLOW	1.0	1	2581.8

[CURVES]

;;Name	Type	X-Value	Y-Value
;;			

103AV	Storage	0	1.13
103AV		1.37	1.13
103AV		1.37001	0
103AV		1.67	1037.3
103AV		1.87	1037.3
103V	Storage	0	1.13
103V		1.83	1.13
103V		1.830001	0
103V		1.98	336.6
103V		2.33	336.6
104V	Storage	0	1.13
104V		1.53	1.13
104V		1.53001	0
104V		1.73	453.2
104V		2.03	453.2
105V	Storage	0	1.13
105V		1.16	1.13
105V		1.16001	0
105V		1.41	503.1
105V		1.41001	0
105V		1.66	0
107V	Storage	0	1.13
107V		1.31	1.13
107V		1.31001	0
107V		1.61	1250
107V		1.81	1250
108AV	Storage	0	0.36
108AV		0.975	0.36
108AV		0.97501	0
108AV		1.275	1104.2
108AV		1.475	1104.2
108BV	Storage	0	0.36
108BV		0.975	0.36

108BV		0.975001	0
108BV		1.275	1232.5
108BV		1.475	1232.5
110V	Storage	0	1.13
110V		1.4	1.13
110V		1.40001	0
110V		1.7	368.3
110V		1.9	368.3
111V	Storage	0	0.36
111V		1.32	0.36
111V		1.32001	0
111V		1.62	658.5
111V		1.82	658.5
112V	Storage	0	1.13
112V		1.3	1.13
112V		1.30001	0
112V		1.6	986.1
112V		1.8	986.1
POND-V	Storage	0	163
POND-V		0.1	231
POND-V		0.2	299
POND-V		0.3	367
POND-V		0.4	435
POND-V		0.5	504
POND-V		0.6	572
POND-V		0.7	640
POND-V		0.8	708
POND-V		0.9	776
POND-V		1	844
POND-V		1.1	912
POND-V		1.2	980
POND-V		1.3	1049
POND-V		1.4	1117
POND-V		1.5	1185
POND-V		1.6	1253

POND-V	1.7	1321
POND-V	1.8	1358
POND-V	1.9	1466
POND-V	2	1588
POND-V	2.1	1721
POND-V	2.2	1867
POND-V	2.3	2025
POND-V	2.4	2196
POND-V	2.5	2379
POND-V	2.6	2575
POND-V	2.7	2783
POND-V	2.8	3003
POND-V	2.9	3236
POND-V	3	3479
POND-V	3.1	3730
POND-V	3.2	3988

```
[TIMESERIES]
;;Name      Date      Time      Value
;;-----
002C03      0:00      0
002C03      0:10      2.81
002C03      0:20      3.5
002C03      0:30      4.69
002C03      0:40      7.3
002C03      0:50      18.21
002C03      1:00      76.81
002C03      1:10      24.08
002C03      1:20      12.36
002C03      1:30      8.32
002C03      1:40      6.3
002C03      1:50      5.09
002C03      2:00      4.29
002C03      2:10      3.72
002C03      2:20      3.29
002C03      2:30      2.95
002C03      2:40      2.68
002C03      2:50      2.46
002C03      3:00      2.28
```

002S12	0:00	1.06
002S12	0:15	1.06
002S12	0:30	1.06
002S12	0:45	1.06
002S12	1:00	1.06
002S12	1:15	1.06
002S12	1:30	1.06
002S12	1:45	1.06
002S12	2:00	1.27
002S12	2:15	1.27
002S12	2:30	1.27
002S12	2:45	1.27
002S12	3:00	1.69
002S12	3:15	1.69
002S12	3:30	1.69
002S12	3:45	1.69
002S12	4:00	2.54
002S12	4:15	2.54
002S12	4:30	3.39
002S12	4:45	3.39
002S12	5:00	5.08
002S12	5:15	5.08
002S12	5:30	20.33
002S12	5:45	55.90
002S12	6:00	7.62
002S12	6:15	7.62
002S12	6:30	3.39
002S12	6:45	3.39
002S12	7:00	2.54
002S12	7:15	2.54
002S12	7:30	2.54
002S12	7:45	2.54
002S12	8:00	1.48
002S12	8:15	1.48
002S12	8:30	1.48
002S12	8:45	1.48
002S12	9:00	1.48
002S12	9:15	1.48

002S12	9:30	1.48
002S12	9:45	1.48
002S12	10:00	0.85
002S12	10:15	0.85
002S12	10:30	0.85
002S12	10:45	0.85
002S12	11:00	0.85
002S12	11:15	0.85
002S12	11:30	0.85
002S12	11:45	0.85
002S12	12:00	0
005C03	0:00	0
005C03	0:10	3.68
005C03	0:20	4.58
005C03	0:30	6.15
005C03	0:40	9.61
005C03	0:50	24.17
005C03	1:00	104.19
005C03	1:10	32.04
005C03	1:20	16.34
005C03	1:30	10.96
005C03	1:40	8.29
005C03	1:50	6.69
005C03	2:00	5.63
005C03	2:10	4.87
005C03	2:20	4.3
005C03	2:30	3.86
005C03	2:40	3.51
005C03	2:50	3.22
005C03	3:00	2.98
005S12	0:00	1.40
005S12	0:15	1.40
005S12	0:30	1.40
005S12	0:45	1.40
005S12	1:00	1.40
005S12	1:15	1.40
005S12	1:30	1.40

005S12	1:45	1.40
005S12	2:00	1.69
005S12	2:15	1.69
005S12	2:30	1.69
005S12	2:45	1.69
005S12	3:00	2.25
005S12	3:15	2.25
005S12	3:30	2.25
005S12	3:45	2.25
005S12	4:00	3.37
005S12	4:15	3.37
005S12	4:30	4.49
005S12	4:45	4.49
005S12	5:00	6.74
005S12	5:15	6.74
005S12	5:30	26.96
005S12	5:45	74.15
005S12	6:00	10.11
005S12	6:15	10.11
005S12	6:30	4.49
005S12	6:45	4.49
005S12	7:00	3.37
005S12	7:15	3.37
005S12	7:30	3.37
005S12	7:45	3.37
005S12	8:00	1.97
005S12	8:15	1.97
005S12	8:30	1.97
005S12	8:45	1.97
005S12	9:00	1.97
005S12	9:15	1.97
005S12	9:30	1.97
005S12	9:45	1.97
005S12	10:00	1.12
005S12	10:15	1.12
005S12	10:30	1.12
005S12	10:45	1.12
005S12	11:00	1.12
005S12	11:15	1.12

005S12	11:30	1.12
005S12	11:45	1.12
005S12	12:00	0
010S12	0:00	1.63
010S12	0:15	1.63
010S12	0:30	1.63
010S12	0:45	1.63
010S12	1:00	1.63
010S12	1:15	1.63
010S12	1:30	1.63
010S12	1:45	1.63
010S12	2:00	1.96
010S12	2:15	1.96
010S12	2:30	1.96
010S12	2:45	1.96
010S12	3:00	2.61
010S12	3:15	2.61
010S12	3:30	2.61
010S12	3:45	2.61
010S12	4:00	3.91
010S12	4:15	3.91
010S12	4:30	5.22
010S12	4:45	5.22
010S12	5:00	7.83
010S12	5:15	7.83
010S12	5:30	31.31
010S12	5:45	86.09
010S12	6:00	11.74
010S12	6:15	11.74
010S12	6:30	5.22
010S12	6:45	5.22
010S12	7:00	3.91
010S12	7:15	3.91
010S12	7:30	3.91
010S12	7:45	3.91
010S12	8:00	2.28
010S12	8:15	2.28
010S12	8:30	2.28

010S12	8:45	2.28
010S12	9:00	2.28
010S12	9:15	2.28
010S12	9:30	2.28
010S12	9:45	2.28
010S12	10:00	1.30
010S12	10:15	1.30
010S12	10:30	1.30
010S12	10:45	1.30
010S12	11:00	1.30
010S12	11:15	1.30
010S12	11:30	1.30
010S12	11:45	1.30
010S12	12:00	0
025M04	0:10	1.516088055
025M04	0:20	1.749115351
025M04	0:30	2.078715445
025M04	0:40	2.583625152
025M04	0:50	3.461716789
025M04	1:00	5.394996968
025M04	1:10	13.44811663
025M04	1:20	56.72433275
025M04	1:30	17.78358976
025M04	1:40	9.131254948
025M04	1:50	6.147712357
025M04	2:00	4.655383456
025M04	2:10	3.762897479
025M04	2:20	3.169361772
025M04	2:30	2.745825503
025M04	2:40	2.428071751
025M04	2:50	2.180598417
025M04	3:00	1.982179574
025M04	3:10	1.819403154
025M04	3:20	1.683310546
025M04	3:30	1.567742242
025M04	3:40	1.468311255
025M04	3:50	1.381797508
025M04	4:00	1.305793328

025S12	0:00	1.91
025S12	0:15	1.91
025S12	0:30	1.91
025S12	0:45	1.91
025S12	1:00	1.91
025S12	1:15	1.91
025S12	1:30	1.91
025S12	1:45	1.91
025S12	2:00	2.29
025S12	2:15	2.29
025S12	2:30	2.29
025S12	2:45	2.29
025S12	3:00	3.06
025S12	3:15	3.06
025S12	3:30	3.06
025S12	3:45	3.06
025S12	4:00	4.58
025S12	4:15	4.58
025S12	4:30	6.11
025S12	4:45	6.11
025S12	5:00	9.17
025S12	5:15	9.17
025S12	5:30	36.67
025S12	5:45	100.85
025S12	6:00	13.75
025S12	6:15	13.75
025S12	6:30	6.11
025S12	6:45	6.11
025S12	7:00	4.58
025S12	7:15	4.58
025S12	7:30	4.58
025S12	7:45	4.58
025S12	8:00	2.67
025S12	8:15	2.67
025S12	8:30	2.67
025S12	8:45	2.67
025S12	9:00	2.67
025S12	9:15	2.67

025S12	9:30	2.67
025S12	9:45	2.67
025S12	10:00	1.53
025S12	10:15	1.53
025S12	10:30	1.53
025S12	10:45	1.53
025S12	11:00	1.53
025S12	11:15	1.53
025S12	11:30	1.53
025S12	11:45	1.53
025S12	12:00	0
100C03	0:00	0
100C03	0:10	6.05
100C03	0:20	7.54
100C03	0:30	10.16
100C03	0:40	15.97
100C03	0:50	40.65
100C03	1:00	178.56
100C03	1:10	54.05
100C03	1:20	27.32
100C03	1:30	18.24
100C03	1:40	13.74
100C03	1:50	11.06
100C03	2:00	9.29
100C03	2:10	8.02
100C03	2:20	7.08
100C03	2:30	6.35
100C03	2:40	5.76
100C03	2:50	5.28
100C03	3:00	4.88
100S12	0:00	2.35
100S12	0:15	2.35
100S12	0:30	2.35
100S12	0:45	2.35
100S12	1:00	2.35
100S12	1:15	2.35
100S12	1:30	2.35

100S12	1:45	2.35
100S12	2:00	2.82
100S12	2:15	2.82
100S12	2:30	2.82
100S12	2:45	2.82
100S12	3:00	3.76
100S12	3:15	3.76
100S12	3:30	3.76
100S12	3:45	3.76
100S12	4:00	5.63
100S12	4:15	5.63
100S12	4:30	7.51
100S12	4:45	7.51
100S12	5:00	11.27
100S12	5:15	11.27
100S12	5:30	45.07
100S12	5:45	123.95
100S12	6:00	16.90
100S12	6:15	16.90
100S12	6:30	7.51
100S12	6:45	7.51
100S12	7:00	5.63
100S12	7:15	5.63
100S12	7:30	5.63
100S12	7:45	5.63
100S12	8:00	3.29
100S12	8:15	3.29
100S12	8:30	3.29
100S12	8:45	3.29
100S12	9:00	3.29
100S12	9:15	3.29
100S12	9:30	3.29
100S12	9:45	3.29
100S12	10:00	1.88
100S12	10:15	1.88
100S12	10:30	1.88
100S12	10:45	1.88
100S12	11:00	1.88
100S12	11:15	1.88

100S12	11:30	1.88
100S12	11:45	1.88
100S12	12:00	0
120C03	0:00	0
120C03	0:10	7.26
120C03	0:20	9.048
120C03	0:30	12.192
120C03	0:40	19.164
120C03	0:50	48.78
120C03	1:00	214.272
120C03	1:10	64.86
120C03	1:20	32.784
120C03	1:30	21.888
120C03	1:40	16.488
120C03	1:50	13.272
120C03	2:00	11.148
120C03	2:10	9.624
120C03	2:20	8.496
120C03	2:30	7.62
120C03	2:40	6.912
120C03	2:50	6.336
120C03	3:00	5.856
120S12	0:00	2.82
120S12	0:15	2.82
120S12	0:30	2.82
120S12	0:45	2.82
120S12	1:00	2.82
120S12	1:15	2.82
120S12	1:30	2.82
120S12	1:45	2.82
120S12	2:00	3.38
120S12	2:15	3.38
120S12	2:30	3.38
120S12	2:45	3.38
120S12	3:00	4.51
120S12	3:15	4.51
120S12	3:30	4.51

120S12	3:45	4.51
120S12	4:00	6.76
120S12	4:15	6.76
120S12	4:30	9.01
120S12	4:45	9.01
120S12	5:00	13.52
120S12	5:15	13.52
120S12	5:30	54.09
120S12	5:45	148.74
120S12	6:00	20.28
120S12	6:15	20.28
120S12	6:30	9.01
120S12	6:45	9.01
120S12	7:00	6.76
120S12	7:15	6.76
120S12	7:30	6.76
120S12	7:45	6.76
120S12	8:00	3.94
120S12	8:15	3.94
120S12	8:30	3.94
120S12	8:45	3.94
120S12	9:00	3.94
120S12	9:15	3.94
120S12	9:30	3.94
120S12	9:45	3.94
120S12	10:00	2.25
120S12	10:15	2.25
120S12	10:30	2.25
120S12	10:45	2.25
120S12	11:00	2.25
120S12	11:15	2.25
120S12	11:30	2.25
120S12	11:45	2.25
120S12	12:00	0

[REPORT]
;;Reporting Options
INPUT YES
CONTROLS NO

SUBCATCHMENTS ALL

NODES ALL

LINKS ALL

[TAGS]

Link	C1	MN
Link	C2	MN
Link	C3	MJ
Link	D1	MJ
Link	D2	MJ

[MAP]

DIMENSIONS	378841.1709	5018906.87675	379191.5451	5019236.75025
UNITS	Meters			

[COORDINATES]

; ;Node	X-Coord	Y-Coord
; ;-----	-----	-----
RIDEAU	378958.48	5019146.773
SOMME	379175.619	5018934.597
100	378987.558	5018973.904
101	378998.283	5018980.319
102	378977.597	5019015.465
102A	378986.306	5019020.59
103	378966.599	5019017.706
103A	378941.507	5019060.339
104	378930.23	5019005.112
105	379105.385	5018974.96
106	379105.597	5019057.847
107	379141.457	5019082.61
108	379108.372	5019138.843
108A	379087.687	5019126.669
108B	379129.356	5019151.185
109	379043.575	5019017.562
110	379023.996	5019050.848
111	379004.791	5019083.498
112	378908.995	5019041.216
C1-I	379076.977	5018938.95
C1-O	379100.821	5018937.598

C2-I	378958.572	5018946.327
C2-O	378980.22	5018944.16
CULV-DS	378921.407	5018960.863
CULV-US	378909.882	5018943.5
POND-S	379026.881	5018961.68

[VERTICES]

; ;Link	X-Coord	Y-Coord
; ;-----	-----	-----
OR2	379053.003	5018957.899

[POLYGONS]

; ;Subcatchment	X-Coord	Y-Coord
; ;-----	-----	-----
BLDG	378965.295	5019058.261
BLDG	378978.247	5019065.884
BLDG	378978.247	5019065.884
BLDG	378991.2	5019073.507
BLDG	378991.2	5019073.507
BLDG	379000.547	5019057.625
BLDG	379000.547	5019057.625
BLDG	379018.56	5019027.02
BLDG	379018.56	5019027.02
BLDG	379019.248	5019025.851
BLDG	379019.248	5019025.851
BLDG	379016.74	5019024.372
BLDG	379016.74	5019024.372
BLDG	379019.824	5019019.124
BLDG	379019.824	5019019.124
BLDG	379001.055	5019008.073
BLDG	379001.055	5019008.073
BLDG	378996.433	5019005.355
BLDG	378996.433	5019005.355
BLDG	378992.795	5019011.537
BLDG	378992.795	5019011.537
BLDG	378992.694	5019011.709
BLDG	378992.694	5019011.709
BLDG	378971.836	5019047.148
BLDG	378971.836	5019047.148

BLDG	378965.295	5019058.261
L100A	378925.833	5018980.238
L100A	378953.832	5018996.938
L100A	378953.832	5018996.938
L100A	378952.224	5018999.672
L100A	378952.224	5018999.672
L100A	378952.071	5019000.034
L100A	378964.865	5018996.485
L100A	378965.24	5018996.11
L100A	378965.24	5018996.11
L100A	378976.397	5018996.87
L100A	378977.987	5019003.028
L100A	378992.694	5019011.709
L100A	378992.694	5019011.709
L100A	378992.795	5019011.537
L100A	378992.795	5019011.537
L100A	378996.433	5019005.355
L100A	378996.433	5019005.355
L100A	379001.055	5019008.073
L100A	379001.055	5019008.073
L100A	379002.478	5019005.658
L100A	379002.478	5019005.658
L100A	379002.58	5019005.485
L100A	379002.58	5019005.485
L100A	379004.761	5018977.992
L100A	378993.617	5018975.247
L100A	378988.644	5018972.316
L100A	378979.191	5018959.028
L100A	378978.526	5018955.721
L100A	378978.526	5018955.721
L100A	378967.742	5018957.889
L100A	378967.742	5018957.889
L100A	378925.833	5018980.238
L101A	379068.081	5018974.824
L101A	379004.771	5018977.991
L101A	379004.761	5018977.992
L101A	379002.58	5019005.485
L101A	379002.58	5019005.485
L101A	379002.478	5019005.658

L101A	379002.478	5019005.658
L101A	379001.055	5019008.073
L101A	379001.055	5019008.073
L101A	379019.824	5019019.124
L101A	379019.824	5019019.124
L101A	379016.74	5019024.372
L101A	379016.74	5019024.372
L101A	379019.248	5019025.851
L101A	379019.248	5019025.851
L101A	379018.56	5019027.02
L101A	379018.56	5019027.02
L101A	379022.027	5019029.004
L101A	379022.027	5019029.004
L101A	379024.802	5019029.487
L101A	379024.802	5019029.487
L101A	379026.883	5019029.85
L101A	379026.883	5019029.85
L101A	379030.291	5019031.86
L101A	379030.291	5019031.86
L101A	379034.401	5019030.797
L101A	379034.401	5019030.797
L101A	379043.879	5019036.375
L101A	379043.879	5019036.375
L101A	379049.097	5019047.165
L101A	379049.097	5019047.165
L101A	379068.8	5019058.757
L101A	379068.8	5019058.757
L101A	379075.976	5019046.56
L101A	379075.976	5019046.56
L101A	379060.204	5019037.281
L101A	379060.204	5019037.281
L101A	379064.479	5019030.015
L101A	379064.479	5019030.015
L101A	379068.081	5018974.824
L102A	378955.075	5019037.284
L102A	378971.836	5019047.148
L102A	378971.836	5019047.148
L102A	378992.694	5019011.709
L102A	378992.694	5019011.709

L102A	378977.987	5019003.028
L102A	378976.397	5018996.87
L102A	378965.24	5018996.11
L102A	378965.24	5018996.11
L102A	378964.865	5018996.485
L102A	378952.071	5019000.034
L102A	378946.029	5019014.315
L102A	378946.029	5019014.315
L102A	378938.316	5019027.425
L102A	378938.316	5019027.425
L102A	378955.075	5019037.284
L103A	378907.914	5019079.099
L103A	378954.378	5019106.445
L103A	378954.378	5019106.445
L103A	378957.123	5019101.782
L103A	378957.123	5019101.782
L103A	378957.224	5019101.609
L103A	378957.224	5019101.609
L103A	378967.159	5019084.724
L103A	378967.159	5019084.724
L103A	378976.491	5019068.869
L103A	378976.491	5019068.869
L103A	378976.593	5019068.697
L103A	378976.593	5019068.697
L103A	378978.247	5019065.884
L103A	378978.247	5019065.884
L103A	378965.295	5019058.261
L103A	378965.295	5019058.261
L103A	378971.836	5019047.148
L103A	378971.836	5019047.148
L103A	378955.075	5019037.284
L103A	378955.075	5019037.284
L103A	378938.316	5019027.425
L103A	378938.316	5019027.425
L103A	378938.315	5019027.428
L103A	378938.315	5019027.428
L103A	378938.315	5019027.428
L103A	378924.542	5019050.841
L103A	378924.542	5019050.841
L103A	378910.755	5019074.271

L103A	378910.755	5019074.271
L103A	378910.655	5019074.441
L103A	378910.655	5019074.441
L103A	378908.929	5019077.375
L103A	378908.929	5019077.375
L103A	378907.914	5019079.099
L104A	378907.493	5018983.328
L104A	378896.172	5019002.734
L104A	378896.172	5019002.734
L104A	378896.349	5019002.837
L104A	378896.349	5019002.837
L104A	378922.558	5019018.155
L104A	378922.558	5019018.155
L104A	378938.316	5019027.425
L104A	378938.316	5019027.425
L104A	378946.029	5019014.315
L104A	378946.029	5019014.315
L104A	378952.071	5019000.034
L104A	378952.071	5019000.034
L104A	378952.224	5018999.672
L104A	378952.224	5018999.672
L104A	378953.832	5018996.938
L104A	378953.832	5018996.938
L104A	378925.833	5018980.238
L104A	378925.833	5018980.238
L104A	378907.493	5018983.328
L105A	379060.204	5019037.281
L105A	379075.976	5019046.56
L105A	379075.976	5019046.56
L105A	379095.678	5019058.154
L105A	379095.678	5019058.154
L105A	379097.568	5019058.126
L105A	379097.568	5019058.126
L105A	379107.585	5019053.35
L105A	379110.889	5019053.342
L105A	379141.455	5019053.344
L105A	379141.455	5019053.344
L105A	379172.021	5019053.341
L105A	379172.021	5019053.341

L105A	379172.968	5019053.339
L105A	379172.968	5019053.339
L105A	379172.711	5018950.211
L105A	379172.711	5018950.211
L105A	379169.706	5018950.224
L105A	379169.706	5018950.224
L105A	379110.427	5018950.367
L105A	379110.427	5018950.367
L105A	379110.65	5018959.198
L105A	379110.65	5018959.198
L105A	379109.004	5018960.855
L105A	379109.004	5018960.855
L105A	379107.873	5018960.857
L105A	379107.873	5018960.857
L105A	379080.519	5018968.934
L105A	379068.081	5018974.824
L105A	379064.479	5019030.015
L105A	379064.479	5019030.015
L105A	379060.204	5019037.281
L107A	379173.166	5019132.747
L107A	379172.968	5019053.339
L107A	379172.968	5019053.339
L107A	379172.021	5019053.341
L107A	379172.021	5019053.341
L107A	379141.455	5019053.344
L107A	379141.455	5019053.344
L107A	379110.889	5019053.342
L107A	379107.585	5019053.35
L107A	379097.568	5019058.126
L107A	379097.568	5019058.126
L107A	379095.678	5019058.154
L107A	379095.678	5019058.154
L107A	379075.976	5019046.56
L107A	379075.976	5019046.56
L107A	379068.8	5019058.757
L107A	379068.8	5019058.757
L107A	379059.191	5019075.089
L107A	379059.191	5019075.089
L107A	379078.894	5019086.681

L107A	379078.894	5019086.681
L107A	379080.618	5019087.696
L107A	379081.987	5019087.341
L107A	379082.783	5019085.986
L107A	379082.783	5019085.986
L107A	379103.839	5019098.369
L107A	379103.839	5019098.369
L107A	379124.898	5019110.755
L107A	379124.898	5019110.755
L107A	379147.44	5019121.277
L107A	379147.44	5019121.277
L107A	379172.021	5019132.747
L107A	379172.021	5019132.747
L107A	379173.166	5019132.747
L108A	379018.526	5019144.2
L108A	379082.916	5019182.096
L108A	379082.916	5019182.096
L108A	379085.67	5019177.416
L108A	379085.67	5019177.416
L108A	379085.772	5019177.243
L108A	379085.772	5019177.243
L108A	379108.378	5019138.838
L108A	379108.378	5019138.838
L108A	379124.898	5019110.755
L108A	379124.898	5019110.755
L108A	379103.839	5019098.369
L108A	379103.839	5019098.369
L108A	379082.783	5019085.986
L108A	379082.783	5019085.986
L108A	379081.987	5019087.341
L108A	379080.618	5019087.696
L108A	379078.894	5019086.681
L108A	379078.894	5019086.681
L108A	379059.191	5019075.089
L108A	379059.191	5019075.089
L108A	379051.745	5019087.745
L108A	379051.745	5019087.745
L108A	379037.335	5019112.236
L108A	379037.335	5019112.236

L108A	379031.307	5019122.478
L108A	379031.307	5019122.478
L108A	379021.377	5019139.355
L108A	379021.377	5019139.355
L108A	379018.526	5019144.2
L108B	379173.176	5019136.821
L108B	379173.166	5019132.747
L108B	379173.166	5019132.747
L108B	379172.021	5019132.747
L108B	379172.021	5019132.747
L108B	379147.44	5019121.277
L108B	379147.44	5019121.277
L108B	379124.898	5019110.755
L108B	379124.898	5019110.755
L108B	379108.378	5019138.838
L108B	379108.378	5019138.838
L108B	379085.772	5019177.243
L108B	379085.772	5019177.243
L108B	379085.67	5019177.416
L108B	379085.67	5019177.416
L108B	379082.916	5019182.096
L108B	379082.916	5019182.096
L108B	379130.946	5019210.364
L108B	379130.946	5019210.364
L108B	379131.681	5019210.787
L108B	379131.681	5019210.787
L108B	379173.176	5019136.821
L110A	379018.56	5019027.02
L110A	379000.547	5019057.625
L110A	379000.547	5019057.625
L110A	379015.007	5019066.13
L110A	379015.007	5019066.13
L110A	379032.042	5019076.153
L110A	379032.042	5019076.153
L110A	379051.745	5019087.745
L110A	379051.745	5019087.745
L110A	379059.191	5019075.089
L110A	379059.191	5019075.089
L110A	379068.8	5019058.757

L110A	379068.8	5019058.757
L110A	379049.097	5019047.165
L110A	379049.097	5019047.165
L110A	379043.879	5019036.375
L110A	379043.879	5019036.375
L110A	379034.401	5019030.797
L110A	379034.401	5019030.797
L110A	379030.291	5019031.86
L110A	379030.291	5019031.86
L110A	379026.883	5019029.85
L110A	379026.883	5019029.85
L110A	379024.802	5019029.487
L110A	379024.802	5019029.487
L110A	379022.027	5019029.004
L110A	379022.027	5019029.004
L110A	379018.56	5019027.02
L111A	378954.378	5019106.445
L111A	379018.524	5019144.198
L111A	379018.524	5019144.198
L111A	379018.526	5019144.2
L111A	379018.526	5019144.2
L111A	379021.377	5019139.355
L111A	379021.377	5019139.355
L111A	379031.307	5019122.478
L111A	379031.307	5019122.478
L111A	379037.335	5019112.236
L111A	379037.335	5019112.236
L111A	379051.745	5019087.745
L111A	379051.745	5019087.745
L111A	379032.042	5019076.153
L111A	379032.042	5019076.153
L111A	379015.007	5019066.13
L111A	379015.007	5019066.13
L111A	379000.547	5019057.625
L111A	379000.547	5019057.625
L111A	378991.2	5019073.507
L111A	378991.2	5019073.507
L111A	378978.247	5019065.884
L111A	378978.247	5019065.884

L111A	378976.593	5019068.697
L111A	378976.593	5019068.697
L111A	378976.491	5019068.869
L111A	378976.491	5019068.869
L111A	378967.159	5019084.724
L111A	378967.159	5019084.724
L111A	378957.224	5019101.609
L111A	378957.224	5019101.609
L111A	378957.123	5019101.782
L111A	378957.123	5019101.782
L111A	378954.378	5019106.445
L112A	378896.172	5019002.734
L112A	378875.03	5019038.977
L112A	378875.03	5019038.977
L112A	378871.819	5019044.481
L112A	378871.819	5019044.481
L112A	378875.945	5019060.283
L112A	378875.945	5019060.283
L112A	378907.914	5019079.099
L112A	378907.914	5019079.099
L112A	378908.929	5019077.375
L112A	378908.929	5019077.375
L112A	378910.655	5019074.441
L112A	378910.655	5019074.441
L112A	378910.755	5019074.271
L112A	378910.755	5019074.271
L112A	378924.542	5019050.841
L112A	378924.542	5019050.841
L112A	378938.315	5019027.428
L112A	378938.315	5019027.428
L112A	378938.316	5019027.425
L112A	378938.316	5019027.425
L112A	378922.558	5019018.155
L112A	378922.558	5019018.155
L112A	378896.349	5019002.837
L112A	378896.349	5019002.837
L112A	378896.172	5019002.734
POND	378981.258	5018954.623
POND	378978.526	5018955.721

POND	378979.191	5018959.028
POND	378988.644	5018972.316
POND	378993.617	5018975.247
POND	379004.771	5018977.991
POND	379068.081	5018974.824
POND	379080.519	5018968.934
POND	379085.16	5018955.977
POND	379082.788	5018951.745
POND	379072.915	5018945.228
POND	378988.363	5018949.414
POND	378981.258	5018954.623
SOMME1	378970.509	5018947.105
SOMME1	378977.015	5018946.784
SOMME1	379080.664	5018941.67
SOMME1	379172.678	5018937.13
SOMME1	379172.17	5018921.871
SOMME1	379080.873	5018928.565
SOMME1	378976.758	5018934.689
SOMME1	378969.209	5018935.116
SOMME1	378955.679	5018937.965
SOMME1	378917.223	5018954.344
SOMME1	378924.594	5018967.51
SOMME1	378970.509	5018947.105
SOMME2	378899.562	5018967.02
SOMME2	378909.195	5018978.217
SOMME2	378917.012	5018970.88
SOMME2	378924.594	5018967.51
SOMME2	378917.223	5018954.344
SOMME2	378899.562	5018967.02
UNC-1	379125.527	5019221.756
UNC-1	379131.681	5019210.787
UNC-1	379131.681	5019210.787
UNC-1	379130.946	5019210.364
UNC-1	379130.946	5019210.364
UNC-1	379082.916	5019182.096
UNC-1	379082.916	5019182.096
UNC-1	379018.526	5019144.2
UNC-1	379018.526	5019144.2
UNC-1	379018.524	5019144.198

UNC-1	379018.524	5019144.198
UNC-1	378954.378	5019106.445
UNC-1	378954.378	5019106.445
UNC-1	378907.914	5019079.099
UNC-1	378907.914	5019079.099
UNC-1	378875.945	5019060.283
UNC-1	378875.945	5019060.283
UNC-1	378871.819	5019044.481
UNC-1	378871.819	5019044.481
UNC-1	378857.097	5019052.948
UNC-1	378857.097	5019052.948
UNC-1	378860.765	5019066.686
UNC-1	378860.765	5019066.686
UNC-1	379125.527	5019221.756
UNC-2	379172.678	5018937.13
UNC-2	379080.664	5018941.67
UNC-2	378977.015	5018946.784
UNC-2	378970.509	5018947.105
UNC-2	378917.012	5018970.88
UNC-2	378888.966	5018997.203
UNC-2	378857.097	5019052.948
UNC-2	378871.819	5019044.481
UNC-2	378875.03	5019038.977
UNC-2	378896.172	5019002.734
UNC-2	378907.493	5018983.328
UNC-2	378925.833	5018980.238
UNC-2	378967.742	5018957.889
UNC-2	378978.526	5018955.721
UNC-2	378981.258	5018954.623
UNC-2	378988.363	5018949.414
UNC-2	379072.915	5018945.228
UNC-2	379082.788	5018951.745
UNC-2	379085.16	5018955.977
UNC-2	379080.519	5018968.934
UNC-2	379107.873	5018960.857
UNC-2	379109.004	5018960.855
UNC-2	379110.65	5018959.198
UNC-2	379110.427	5018950.367
UNC-2	379169.706	5018950.224

UNC-2	379172.711	5018950.211
UNC-2	379172.678	5018937.13

;;Storage Node	X-Coord	Y-Coord
;;-----	-----	-----

[SYMBOLS]		
;;Gage	X-Coord	Y-Coord
;;-----	-----	-----

B.3 PCSWMM Model Output File (100-Year, 12-Hour SCS Storm)



EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

WARNING 03: negative offset ignored for Link 107--106

Element Count

Number of rain gages 1
 Number of subcatchments ... 18
 Number of nodes 26
 Number of links 26
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
RG1	100S12	INTENSITY	15 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
BLDG	0.18	41.12	100.00	2.0000	RG1	102A
L100A	0.23	51.79	42.86	2.5000	RG1	100
L101A	0.37	82.29	95.71	1.5000	RG1	105
L102A	0.16	35.47	100.00	1.5000	RG1	103
L103A	0.30	68.37	94.29	1.5000	RG1	103A
L104A	0.16	35.30	100.00	1.3000	RG1	104
L105A	1.02	229.35	94.29	1.2000	RG1	105

L107A	0.57	129.02	74.29	1.0000	RG1	107
L108A	0.61	137.65	77.14	1.5000	RG1	108A
L108B	0.48	106.96	67.14	1.3000	RG1	108B
L110A	0.22	48.48	100.00	3.0000	RG1	110
L111A	0.46	103.46	94.29	2.0000	RG1	111
L112A	0.29	64.21	90.00	1.5000	RG1	112
POND	0.28	135.00	60.00	33.0000	RG1	POND-S
SOMME1	0.24	261.00	100.00	3.0000	RG1	C1-I
SOMME2	0.03	20.00	100.00	3.0000	RG1	CULV-DS
UNC-1	0.41	315.00	0.00	3.0000	RG1	RIDEAU
UNC-2	0.35	365.00	7.14	2.0000	RG1	C2-I

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
RIDEAU	OUTFALL	0.00	0.00	0.0	
SOMME	OUTFALL	88.31	1.20	0.0	
100	STORAGE	88.91	2.09	0.0	
101	STORAGE	88.99	3.22	0.0	
102	STORAGE	89.60	2.14	0.0	
102A	STORAGE	89.93	2.07	0.0	
103	STORAGE	89.65	2.33	0.0	
103A	STORAGE	90.00	1.87	0.0	
104	STORAGE	89.92	2.03	0.0	
105	STORAGE	89.45	1.66	0.0	
106	STORAGE	89.77	1.79	0.0	
107	STORAGE	89.89	1.81	0.0	
108	STORAGE	90.08	1.55	0.0	
108A	STORAGE	90.32	1.48	0.0	
108B	STORAGE	90.32	1.48	0.0	
109	STORAGE	89.74	2.47	0.0	
110	STORAGE	89.90	1.90	0.0	
111	STORAGE	90.02	1.82	0.0	
112	STORAGE	90.07	1.80	0.0	
C1-I	STORAGE	88.91	1.20	0.0	

C1-0	STORAGE	88.70	1.30	0.0	
C2-I	STORAGE	89.58	1.28	0.0	
C2-0	STORAGE	89.44	1.28	0.0	
CULV-DS	STORAGE	89.82	1.25	0.0	
CULV-US	STORAGE	89.97	1.20	0.0	Yes
POND-S	STORAGE	87.60	3.20	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
100--POND-S	100	POND-S	CONDUIT	9.1	0.1543	0.0130
101--100	101	100	CONDUIT	12.5	0.1520	0.0130
102--101	102	101	CONDUIT	40.8	0.2011	0.0130
102A--102	102A	102	CONDUIT	10.1	0.9996	0.0130
103--102	103	102	CONDUIT	11.2	0.1960	0.0130
103A--103	103A	103	CONDUIT	49.5	0.4003	0.0130
104--103	104	103	CONDUIT	38.5	0.2988	0.0130
105--101	105	101	CONDUIT	107.2	0.1501	0.0130
106--105	106	105	CONDUIT	82.9	0.2003	0.0130
108--107	108	107	CONDUIT	65.2	0.1993	0.0130
108A--108	108A	108	CONDUIT	24.0	0.4000	0.0130
108B--108	108B	108	CONDUIT	24.3	0.3985	0.0130
109--101	109	101	CONDUIT	58.6	0.2507	0.0130
110--109	110	109	CONDUIT	38.6	0.2512	0.0130
111--110	111	110	CONDUIT	37.9	0.2508	0.0130
112--104	112	104	CONDUIT	41.9	0.3008	0.0130
C1	C1-I	C1-0	CONDUIT	23.9	1.0461	0.0240
C2	C2-I	C2-0	CONDUIT	21.8	0.6422	0.0240
C3	CULV-DS	C2-I	CONDUIT	40.4	0.5198	0.0250
C4	CULV-US	CULV-DS	CONDUIT	20.8	0.7198	0.0240
D1	C1-0	SOMME	CONDUIT	74.9	0.6546	0.0250
D2	C2-0	C1-I	CONDUIT	92.8	0.6573	0.0250
107--106	107	106	ORIFICE			
OR1	POND-S	C1-I	ORIFICE			
OR2	POND-S	C1-I	WEIR			
W1	105	C1-0	WEIR			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
100--POND-S	CIRCULAR	1.05	0.87	0.26	1.05	1	1072.85
101--100	CIRCULAR	1.05	0.87	0.26	1.05	1	1064.83
102--101	CIRCULAR	0.53	0.22	0.13	0.53	1	192.86
102A--102	CIRCULAR	0.30	0.07	0.07	0.30	1	96.69
103--102	CIRCULAR	0.53	0.22	0.13	0.53	1	190.40
103A--103	CIRCULAR	0.38	0.11	0.09	0.38	1	110.93
104--103	CIRCULAR	0.38	0.11	0.09	0.38	1	95.85
105--101	CIRCULAR	0.75	0.44	0.19	0.75	1	431.39
106--105	CIRCULAR	0.60	0.28	0.15	0.60	1	274.80
108--107	CIRCULAR	0.53	0.22	0.13	0.53	1	191.98
108A--108	CIRCULAR	0.38	0.11	0.09	0.38	1	110.89
108B--108	CIRCULAR	0.38	0.11	0.09	0.38	1	110.68
109--101	CIRCULAR	0.45	0.16	0.11	0.45	1	142.76
110--109	CIRCULAR	0.45	0.16	0.11	0.45	1	142.90
111--110	CIRCULAR	0.45	0.16	0.11	0.45	1	142.79
112--104	CIRCULAR	0.38	0.11	0.09	0.38	1	96.17
C1	ARCH	0.97	1.06	0.29	1.39	2	1984.18
C2	ARCH	0.82	0.74	0.25	1.15	2	972.13
C3	TRIANGULAR	1.20	4.32	0.57	7.20	1	8557.37
C4	ARCH	0.82	0.74	0.25	1.15	2	1029.17
D1	TRIANGULAR	1.20	4.32	0.57	7.20	1	9602.76
D2	TRIANGULAR	1.20	4.32	0.57	7.20	1	9623.07

Analysis Options

Flow Units LPS
Process Models:
Rainfall/Runoff YES
RDII NO

Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 07/11/2025 00:00:00
Ending Date 07/12/2025 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 1.00 sec
Variable Time Step NO
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001500 m

	Volume hectare-m	Depth mm
Runoff Quantity Continuity	-----	-----
Total Precipitation	0.597	93.915
Evaporation Loss	0.000	0.000
Infiltration Loss	0.101	15.889
Surface Runoff	0.489	76.886
Final Storage	0.008	1.182
Continuity Error (%)	-0.045	

	Volume hectare-m	Volume 10^6 ltr
Flow Routing Continuity	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.489	4.890
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

External Inflow	22.306	223.067
External Outflow	22.677	226.776
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.127	1.268
Final Stored Volume	0.250	2.504
Continuity Error (%)	-0.024	

Highest Flow Instability Indexes

All links are stable.

Most Frequent Nonconverging Nodes

Convergence obtained at all time steps.

Routing Time Step Summary

Minimum Time Step : 1.00 sec
Average Time Step : 1.00 sec
Maximum Time Step : 1.00 sec
% of Time in Steady State : 0.00
Average Iterations per Step : 2.00
% of Steps Not Converging : 0.00

Subcatchment Runoff Summary

-----			Total	Total	Total	Total	Imperv	Perv	Total	
Total	Peak	Runoff	Precip	Runon	Evap	Infil	Runoff	Runoff	Runoff	
Runoff	Runoff	Coeff								
Subcatchment	Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6
Ltr	LPS									

BLDG			93.92	0.00	0.00	0.00	92.39	0.00	92.39	
0.17	62.92	0.984								
L100A			93.91	0.00	0.00	36.95	39.60	16.73	56.33	
0.13	68.21	0.600								
L101A			93.92	0.00	0.00	2.67	88.42	1.35	89.78	
0.33	125.33	0.956								
L102A			93.92	0.00	0.00	0.00	92.38	0.00	92.38	
0.15	54.28	0.984								
L103A			93.92	0.00	0.00	3.57	87.10	1.80	88.90	
0.27	103.96	0.947								
L104A			93.91	0.00	0.00	0.00	92.38	0.00	92.38	
0.14	54.01	0.984								
L105A			93.91	0.00	0.00	3.57	87.10	1.80	88.90	
0.91	348.71	0.947								
L107A			93.92	0.00	0.00	16.46	68.63	7.70	76.33	
0.44	188.09	0.813								
L108A			93.91	0.00	0.00	14.52	71.27	6.95	78.22	
0.48	203.55	0.833								
L108B			93.92	0.00	0.00	21.10	62.03	9.77	71.80	
0.34	152.83	0.765								
L110A			93.91	0.00	0.00	0.00	92.39	0.00	92.39	
0.20	74.18	0.984								
L111A			93.92	0.00	0.00	3.57	87.11	1.80	88.91	
0.41	157.31	0.947								
L112A			93.92	0.00	0.00	6.28	83.15	3.12	86.26	
0.25	97.14	0.919								
POND			93.91	0.00	0.00	24.96	55.45	12.64	68.09	
0.19	93.73	0.725								
SOMME1			93.92	0.00	0.00	0.00	92.43	0.00	92.43	

0.22	82.63	0.984								
SOMME2			93.92	0.00	0.00	0.00	92.42	0.00	92.42	
0.03	10.33	0.984								
UNC-1			93.92	0.00	0.00	63.41	0.00	30.53	30.53	
0.13	122.56	0.325								
UNC-2			93.91	0.00	0.00	61.47	6.59	32.37	32.37	
0.11	107.45	0.345								

Node Depth Summary

-----		Average	Maximum	Maximum	Time of Max	Reported
Node	Type	Depth	Depth	HGL	Occurrence	Max Depth
		Meters	Meters	Meters	days hr:min	Meters

RIDEAU	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SOMME	OUTFALL	0.69	0.77	89.08	0 06:02	0.77
100	STORAGE	0.90	1.50	90.41	0 06:01	1.50
101	STORAGE	0.83	1.56	90.55	0 06:00	1.55
102	STORAGE	0.26	1.39	90.99	0 06:00	1.39
102A	STORAGE	0.04	1.16	91.09	0 06:00	1.16
103	STORAGE	0.22	1.43	91.08	0 06:00	1.43
103A	STORAGE	0.04	1.28	91.28	0 06:00	1.27
104	STORAGE	0.05	1.40	91.32	0 06:00	1.40
105	STORAGE	0.39	1.40	90.85	0 06:00	1.39
106	STORAGE	0.14	1.20	90.97	0 06:00	1.19
107	STORAGE	0.09	1.50	91.39	0 06:02	1.50
108	STORAGE	0.08	1.38	91.46	0 06:03	1.38
108A	STORAGE	0.07	1.21	91.53	0 06:02	1.21
108B	STORAGE	0.06	1.16	91.49	0 06:04	1.16
109	STORAGE	0.15	1.16	90.90	0 06:00	1.15
110	STORAGE	0.06	1.34	91.23	0 06:00	1.34
111	STORAGE	0.05	1.37	91.39	0 06:00	1.37
112	STORAGE	0.04	1.36	91.43	0 06:00	1.36
C1-I	STORAGE	0.80	1.01	89.92	0 06:02	1.01
C1-O	STORAGE	0.84	0.92	89.62	0 06:02	0.92

C2-I	STORAGE	1.05	1.11	90.69	0	06:00	1.11
C2-O	STORAGE	0.81	0.82	90.26	0	06:00	0.82
CULV-DS	STORAGE	0.87	0.90	90.72	0	06:00	0.90
CULV-US	STORAGE	1.09	1.12	91.09	0	06:00	1.12
POND-S	STORAGE	2.22	2.73	90.33	0	06:04	2.73

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
RIDEAU	OUTFALL	122.56	122.56	0 06:00	0.126	0.126	0.000
SOMME	OUTFALL	0.00	3467.07	0 06:02	0	227	0.000
100	STORAGE	68.21	1197.02	0 06:00	0.13	4.17	0.121
101	STORAGE	0.00	1141.65	0 05:54	0	4.06	0.483
102	STORAGE	0.00	352.76	0 05:55	0	0.975	0.191
102A	STORAGE	62.92	62.92	0 06:00	0.169	0.169	0.014
103	STORAGE	54.28	294.13	0 05:53	0.146	0.806	-0.043
103A	STORAGE	103.96	103.96	0 06:00	0.27	0.27	0.088
104	STORAGE	54.01	145.72	0 05:53	0.145	0.391	0.028
105	STORAGE	474.04	667.60	0 05:54	1.23	2.49	0.493
106	STORAGE	0.00	255.56	0 06:07	0	1.26	0.078
107	STORAGE	188.09	307.40	0 06:00	0.438	1.26	-0.040
108	STORAGE	0.00	212.03	0 05:47	0	0.819	-0.024
108A	STORAGE	203.55	203.55	0 06:00	0.479	0.479	0.155
108B	STORAGE	152.83	152.83	0 06:00	0.341	0.341	0.135
109	STORAGE	0.00	225.00	0 05:54	0	0.607	-0.109
110	STORAGE	74.18	229.02	0 05:53	0.199	0.608	0.058
111	STORAGE	157.31	157.31	0 06:00	0.409	0.409	0.016
112	STORAGE	97.14	97.14	0 06:00	0.246	0.246	0.067
C1-I	STORAGE	82.63	3500.68	0 06:00	0.222	227	0.054
C1-O	STORAGE	0.00	3467.05	0 06:02	0	227	0.036
C2-I	STORAGE	107.45	2698.89	0 06:00	0.114	223	0.020

C2-O	STORAGE	0.00	2698.00	0 06:00	0	223	0.034
CULV-DS	STORAGE	10.33	2812.09	0 00:00	0.0277	223	0.038
CULV-US	STORAGE	2581.80	2581.80	0 00:00	223	223	0.008
POND-S	STORAGE	93.73	1289.61	0 06:00	0.194	5.77	0.038

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
100	0.001	43.4	0.0	0.0	0.002	71.9	0 06:01	1195.88
101	0.001	25.7	0.0	0.0	0.002	48.4	0 06:00	1133.30
102	0.000	12.3	0.0	0.0	0.002	65.2	0 06:00	350.82
102A	0.000	2.0	0.0	0.0	0.001	56.1	0 06:00	62.61
103	0.000	0.2	0.0	0.0	0.002	1.1	0 06:00	290.41
103A	0.000	0.0	0.0	0.0	0.001	0.4	0 06:00	103.11
104	0.000	0.0	0.0	0.0	0.002	0.9	0 06:00	149.87
105	0.001	1.2	0.0	0.0	0.057	89.3	0 06:00	593.87
106	0.000	7.6	0.0	0.0	0.001	66.8	0 06:00	280.58
107	0.001	0.2	0.0	0.0	0.077	17.3	0 06:02	255.56

108	0.000	5.3	0.0	0.0	0.002	88.9	0	06:03	203.67
108A	0.002	0.5	0.0	0.0	0.100	25.6	0	06:02	142.52
108B	0.001	0.3	0.0	0.0	0.074	17.2	0	06:04	95.94
109	0.000	5.9	0.0	0.0	0.001	46.8	0	06:00	221.62
110	0.000	0.1	0.0	0.0	0.002	1.1	0	06:00	225.00
111	0.000	0.0	0.0	0.0	0.003	1.4	0	06:00	159.53
112	0.000	0.0	0.0	0.0	0.007	2.1	0	06:00	117.67
C1-I	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00	3467.05
C1-O	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00	3467.07
C2-I	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00	2698.00
C2-O	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00	2697.68
CULV-DS	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00	2628.77
CULV-US	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00	2812.09
POND-S	2.095	43.0	0.0	0.0	3.280	67.4	0	06:04	800.68

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
RIDEAU	4.56	32.11	122.56	0.126
SOMME	99.91	2625.76	3467.07	226.649
System	52.23	2657.86	3565.05	226.775

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
------	------	--------------------------	--	-----------------------------	----------------------	-----------------------

100--POND-S	CONDUIT	1195.88	0 06:00	1.38	1.11	1.00
101--100	CONDUIT	1133.30	0 05:55	1.31	1.06	1.00
102--101	CONDUIT	350.82	0 05:56	1.62	1.82	1.00
102A--102	CONDUIT	62.61	0 06:00	0.89	0.65	1.00
103--102	CONDUIT	290.41	0 05:55	1.34	1.53	1.00
103A--103	CONDUIT	103.11	0 06:00	0.93	0.93	1.00
104--103	CONDUIT	149.87	0 06:02	1.36	1.56	1.00
105--101	CONDUIT	593.87	0 05:49	1.34	1.38	1.00
106--105	CONDUIT	280.58	0 06:06	0.99	1.02	1.00
108--107	CONDUIT	203.67	0 06:22	0.94	1.06	1.00
108A--108	CONDUIT	142.52	0 06:31	1.29	1.29	1.00
108B--108	CONDUIT	95.94	0 06:22	0.87	0.87	1.00
109--101	CONDUIT	221.62	0 05:54	1.39	1.55	1.00
110--109	CONDUIT	225.00	0 05:54	1.41	1.57	1.00
111--110	CONDUIT	159.53	0 06:01	1.00	1.12	1.00
112--104	CONDUIT	117.67	0 06:02	1.07	1.22	1.00
C1	CONDUIT	3467.05	0 06:02	1.64	0.87	0.97
C2	CONDUIT	2698.00	0 06:00	1.82	1.39	1.00
C3	CONDUIT	2628.77	0 00:01	1.59	0.31	0.78
C4	CONDUIT	2812.09	0 00:00	2.27	1.37	1.00
D1	CONDUIT	3467.07	0 06:02	1.83	0.36	0.66
D2	CONDUIT	2697.68	0 06:00	2.36	0.28	0.73
107--106	ORIFICE	255.56	0 06:07			1.00
OR1	ORIFICE	14.37	0 06:06			1.00
OR2	WEIR	786.35	0 06:05			0.62
W1	WEIR	0.00	0 00:00			0.00

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								Norm Ltd	Inlet Ctrl
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit			
100--POND-S	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	

101--100	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
102--101	1.00	0.03	0.00	0.00	0.86	0.00	0.00	0.11	0.02	0.00
102A--102	1.00	0.03	0.00	0.00	0.56	0.00	0.00	0.41	0.39	0.00
103--102	1.00	0.03	0.00	0.00	0.89	0.00	0.00	0.09	0.00	0.00
103A--103	1.00	0.03	0.00	0.00	0.64	0.00	0.00	0.33	0.58	0.00
104--103	1.00	0.03	0.00	0.00	0.64	0.00	0.00	0.33	0.50	0.00
105--101	1.00	0.00	0.03	0.00	0.96	0.00	0.00	0.01	0.07	0.00
106--105	1.00	0.03	0.00	0.00	0.83	0.00	0.00	0.14	0.08	0.00
108--107	1.00	0.03	0.00	0.00	0.13	0.00	0.00	0.84	0.03	0.00
108A--108	1.00	0.03	0.00	0.00	0.04	0.00	0.00	0.93	0.00	0.00
108B--108	1.00	0.03	0.00	0.00	0.04	0.00	0.00	0.93	0.00	0.00
109--101	1.00	0.03	0.00	0.00	0.84	0.00	0.00	0.13	0.05	0.00
110--109	1.00	0.03	0.00	0.00	0.64	0.00	0.00	0.33	0.38	0.00
111--110	1.00	0.03	0.00	0.00	0.20	0.00	0.00	0.77	0.05	0.00
112--104	1.00	0.03	0.00	0.00	0.14	0.00	0.00	0.84	0.04	0.00
C1	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
C2	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
C3	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
C4	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
D1	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
D2	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00

Conduit Surcharge Summary

Conduit	----- Both Ends	Hours Full Upstream	----- Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
100--POND-S	3.16	3.16	4.54	0.22	0.59
101--100	1.56	1.57	1.73	0.20	0.52
102--101	1.10	1.10	1.57	0.28	0.30
102A--102	0.80	0.80	1.10	0.01	0.01
103--102	0.91	0.91	0.97	0.26	0.29
103A--103	0.36	0.36	0.91	0.01	0.01
104--103	0.62	0.62	0.91	0.26	0.26

105--101	0.84	0.84	1.57	0.33	0.33
106--105	0.77	0.77	0.84	0.01	0.27
108--107	0.78	0.78	0.81	0.15	0.19
108A--108	0.78	0.78	0.78	0.03	0.24
108B--108	0.78	0.78	0.78	0.01	0.15
109--101	0.86	0.86	1.57	0.25	0.25
110--109	0.43	0.43	0.74	0.25	0.27
111--110	0.30	0.30	0.35	0.20	0.24
112--104	0.32	0.32	0.51	0.02	0.13
C2	0.22	23.98	0.22	23.98	0.22
C4	23.97	23.99	23.97	24.00	23.97

Analysis begun on: Wed Aug 20 10:24:05 2025
Analysis ended on: Wed Aug 20 10:24:09 2025
Total elapsed time: 00:00:04

B.4 Pond Design Calculations



Project Name: 301 Somme Street
Project No.: 160402138

Water Quality Storage Requirements

SWMP ID	SWMP Type	Area (ha)	% Impervious	Protection Level	Unit Volume Requirements (m3/ha)			Specific Volume Requirements (m3)			Volume Provided (m3)	
					Total Unit Vol.	Permanent Pool	Extended Def.	Total Vol.	Permanent Pool	Extended Def.	Permanent Pool	Extended Def.
Pond 1	Wet Pond	5.32	84.0%	Enhanced (80% TSS)	251.8	211.8	40.0	1340	1127	213	1261	764

Water quality volume requirements per Table 3.2, Stormwater Management Planning & Design Manual (MOE, 2003)

Protection Level	SWMP Type	Storage Volume (m3/ha) for Impervious Level			
		35%	55%	70%	85%
Enhanced (80% TSS)	Infiltration	25	30	35	40
Enhanced (80% TSS)	Wetland	80	105	120	140
Enhanced (80% TSS)	Hybrid Wetland	110	150	175	195
Enhanced (80% TSS)	Wet Pond	140	190	225	250
Normal (70% TSS)	Infiltration	20	20	25	30
Normal (70% TSS)	Wetland	60	70	80	90
Normal (70% TSS)	Hybrid Wetland	75	90	105	120
Normal (70% TSS)	Wet Pond	90	110	130	150
Basic (60% TSS)	Infiltration	20	20	20	20
Basic (60% TSS)	Wetland	60	60	60	60
Basic (60% TSS)	Hybrid Wetland	60	70	75	80
Basic (60% TSS)	Wet Pond	60	75	85	95
Basic (60% TSS)	Dry Pond	90	150	200	240

Project Name: 301 Somme Street
Project No.: 160402138

Outlet Structure Discharge Calculations

Control ID	Control Type	Invert	Width/Dia.	Height	Coefficient	Side Slope	End Coefficient
1	Orifice (Circ.)	89.30	0.102		0.61		
2	Weir (Rect.)	89.90	1.650	0.700	1.67		

Elevation (m)	Control 1 (L/s)	Control 2 (L/s)	Control 3 (L/s)	Control 4 (L/s)	Control 5 (L/s)	Total Discharge (L/s)
89.30	0	0				0
89.40	5	0				5
89.50	9	0				9
89.60	11	0				11
89.70	13	0				13
89.80	15	0				15
89.90	16	0				16
90.00	18	87				105
90.10	19	246				266
90.20	20	453				473
90.30	22	697				719
90.40	23	974				997
90.50	24	1281				1304
90.60	25	1614				1638
90.70	26	1972				1997
90.80	27	2353				2379

* Calculated pond discharge rates do not consider elevated HGL at pond outlet

Project Name: 301 Somme Street
Project No.: 160402138

Forebay Sizing Calculations

Settling Length per Equation 4.5 of Stormwater Management Planning and Design Manual (MOE, 2003)

$Dist = \sqrt{\frac{rQp}{v_s}}$

where:

Dist = Forebay Length (m)

r = Length-to-width ratio of forebay

Qp = Peak flow rate from the pond during design quality storm (m3/s)

Vs = Settling velocity (m/s)

r = 3 : 1

Qp = 0.016 m3/s

Vs = 0.0003 m/s

Dist = 12.6 m

Provided = 17 m

Dispersion Length per Equation 4.6 of Stormwater Management Planning and Design Manual (MOE, 2003)

$Dist = \frac{8Q}{dv_f}$

where:

Dist = Forebay Length (m)

Q = Inlet flowrate during quality (25mm) event (m3/s)

d = Depth of permanent pool in forebay less sediment depth

Vf = Desired forebay velocity (m/s)

Q = 0.648 m3/s

d = 1.00 m

Vf = 0.5 m/s

Dist = 10.4 m

Provided = 17 m

Forebay Velocity

$v = Q/A$

where:

Q = Inlet flowrate during quality (25mm) event (m3/s)

A = Forebay cross-sectional area (m2)

V = Desired forebay velocity (m/s)

Q = 0.648 m3/s

A = 19.023 m2

V = 0.03 m/s

Target <= 0.15 m/s

Forebay Deep Zone Bottom Width per Equation 4.7 of Stormwater Management Planning and Design Manual (MOE, 2003)

$width = Dist/8$

where:

width = Minimum width of forebay (m)

Dist = Forebay length (m) per Equation 4.6, 4.7

Dist = 12.6 m

width = 1.6 m

Provided = 5.7 m

Forebay Surface Area per Table 4.6 of Stormwater Management Planning and Design Manual (MOE, 2003)

$S = S A_f / S A_{pp}$

where:

SAf = Forebay surface area (m2) @ PP elevation

SApp = Permanent pool surface area (m2)

S = Target forebay size (%)

SAf = 420.0 m2

SApp = 1273.3 m2

S = 33.0 %

Target <= 33.3 %

Cleanout Frequency (with reference to Table 6.3 of Stormwater Management Planning and Design Manual (MOE, 2003))

$T_m = V_s / (L_s A_c P)$

where:

Tm = Proposed SWMF maintenance timeframe (years)

Vs = Sediment storage volume (m3)

Ls = Sediment loading rate (m3/ha)

Ac = Pond catchment area (ha)

P = Removal efficiency = Level of Protection (%)

% Impervious	35	55	70	80
Loading (m3/ha)	0.6	1.9	2.8	3.8

Vs = 81 m3

l = 84.0% Contributing area imperviousness %

Ls = 3.934 m3/ha per year

Ac = 5.32 ha

P = 80% Enhanced (80% TSS)

Tm = 4.8 years

Target >= 5.0 years

Project Name: 301 Somme Street
Project No.: 160402138

Extended Detention Drawdown Calculations

via falling head per Equation 4.10 of Stormwater Management Planning and Design Manual (MOE, 2003)

$$t = (h_1^{0.5} - h_0^{0.5}) \frac{2A_p}{CA_o(2g)^{0.5}}$$

where:

t = drawdown time (s)

h1 = starting water elevation above orifice (m)

h0 = ending water elevation above orifice (m)

Ap = pond surface area (m2)

Ao = cross-sectional area of orifice (m2)

C = orifice discharge coefficient

h1 =0.6 m

h0 =0.0 m

Ap =1321 m2

Ao =0.00817 m2

C =0.61

t =25.7 hrs

Target24-48 hrs

via falling head per Equation 4.11 of Stormwater Management Planning and Design Manual (MOE, 2003)

$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o}$$

where:

t = drawdown time (s)

C2 = slope coefficient from area-depth linear regression

C3 = intercept from area-depth linear regression

h = maximum water elevation above orifice (m)

Ao = cross-sectional area of orifice (m2)

C2 =1161.3 m/m

C3 =1321.0 m2

h =0.6 m

Ao =0.00817 m2

t =29.7 hrs

Target24-48 hrs

Project Name: 301 Somme Street
Project No.: 160402138

Stage Storage Calculations

Stage (m)	Discharge (L/s)	Storage		Depth (m)	Forebay			Main Cell		
		Active (m3)	Total (m3)		Area (m2)	Inc. Volume (m3)	Accum. Volume (m3)	Area (m2)	Inc. Volume (m3)	Accum. Volume (m3)
87.60	0	0	0	0.00	37	0	0	126	0	0
87.70	0	0	0	0.10	60	5	5	172	15	15
87.80	0	0	0	0.20	82	7	12	217	19	34
87.90	0	0	0	0.30	105	9	21	263	24	58
88.00	0	0	0	0.40	127	12	33	308	29	87
88.10	0	0	0	0.50	150	14	47	354	33	120
88.20	0	0	0	0.60	172	16	63	400	38	158
88.30	0	0	0	0.70	195	18	81	445	42	200
88.40	0	0	0	0.80	217	21	102	491	47	247
88.50	0	0	0	0.90	240	23	125	536	51	298
88.60	0	0	0	1.00	262	25	150	582	56	354
88.70	0	0	0	1.10	285	27	177	627	60	414
88.80	0	0	0	1.20	307	30	207	673	65	479
88.90	0	0	0	1.30	330	32	238	719	70	549
89.00	0	0	0	1.40	352	34	273	764	74	623
89.10	0	0	0	1.50	375	36	309	810	79	702
89.20	0	0	0	1.60	397	39	348	855	83	785
89.30	0	0	0	1.70	420	41	-	901	88	1261
89.40	5	0	0	1.80	-	-	-	1025	96	1358
89.50	9	0	0	1.90	-	-	-	1149	109	1466
89.60	11	121	121	2.00	-	-	-	1273	121	1588
89.70	13	134	255	2.10	-	-	-	1397	134	1721
89.80	15	146	401	2.20	-	-	-	1521	146	1867
89.90	16	158	559	2.30	-	-	-	1646	158	2025
90.00	105	171	730	2.40	-	-	-	1770	171	2196
90.10	266	183	913	2.50	-	-	-	1894	183	2379
90.20	473	196	1108	2.60	-	-	-	2018	196	2575
90.30	719	208	1316	2.70	-	-	-	2142	208	2783
90.40	997	220	1537	2.80	-	-	-	2266	220	3003
90.50	1304	233	1770	2.90	-	-	-	2390	233	3236
90.60	1638	243	2012	3.00	-	-	-	2468	243	3479
90.70	1997	251	2263	3.10	-	-	-	2545	251	3730
90.80	2379	258	2522	3.20	-	-	-	2623	258	3988

Appendix C External Report Excerpts



This Stormwater Management Report (SWMR) has been written to demonstrate that the subject land could be developed in compliance with the above surface water criteria and also prepared in accordance with the SWMPDM. The proposed stormwater management strategy for the HIP was developed to meet a "normal" level of protection, which corresponds to a standard approach used in land development to obtain a targeted TSS removal rate of 70%.

3.0 STORM SERVICING

3.1 General

Peak flow estimation is an important task that is carried out for any proposed development. There are several reasons that explain why flood flow rates are computed as part of site development. The main purpose of these calculations, however, is to allow for the proper configuration and sizing of the proposed conveyance systems to minimize the risk of flooding.

Drainage works are designed for a real or hypothetical storm event that may or may not happen during the lifetime of the facilities. At the onset of the design process, design criteria are adopted that may vary with the type of project, in recognition of the impacts of failure. For this particular project, the level of protection adopted (storm events up to a 1:100 year recurrence) was based on design storm characteristics of an infrequent storm event having a low probability to occur.

3.2 Description of Conveyance Systems and Design Basis

Flowing water can be conveyed to an outlet by either open-channel flow or pipe flow. Storm runoff generated by the subject lands is to be collected and conveyed by a roadside ditch/culvert system before discharging to Findlay Creek via an end-of-pipe stormwater management facility (SWMF).

Sizing of the conveyance systems was carried out using various levels of service. The open ditch system was sized with sufficient capacity to convey, under free-flowing conditions, storm runoff up to the 1:100 year recurrence, while roadway culverts were sized to provide conveyance of the 1:10 year peak flow rates without overtopping the roadway embankments.

As part of this sizing exercise, Storm Drainage Area Plans were prepared and included in this Report (refer to Drawing D-ST1 for the HIP and Drawing D-ST2 for Hawthorne and Rideau Road) that show the delineated area for each of the conveyance segments (i.e., from node location to node location), along with its assigned runoff coefficient (C-factor) based on the type of surface. Since the final development of Hawthorne Industrial Park is unknown at this time, a conservative on-site runoff coefficient (C-factor) of 0.70 was used. Table 2 illustrates the breakdown of a typical site that would generate a weighted runoff coefficient of 0.70.

Table 2 - Typical Potential Land Use Breakdown

Type of Surface	Area (%)	C-Factor
Building	10	1.0
Asphalt Parking	35	0.90
Gravel	35	0.70
Grass	20	0.20
Overall	100	0.70

It should be noted that the C-factors shown on the Storm Drainage Area Plans denote those associated with 1:10 year peak flow calculations. As recommended in Section 5.4.5.2.1 of the City of Ottawa's Sewer Design Guidelines, C-factors shown on drawings were increased by 10% and 25% for the 1:25 year and 1:100 year peak flow calculations, respectively (refer to Appendix 'A' for copies of the Rational Method Design Sheets).

3.2.1 Open Ditch System

An open ditch channel is a conduit used to convey flowing water from one location to another, with a free surface. A channel can be classified as either artificial (i.e., manmade) or natural. Artificial channels are those constructed or developed as a result of human activity. This type of conveyance system is usually implemented as a long and mild-sloped channel built in the ground, which provides conveyance of water between two points, with sections of regular geometry and shape. An open ditch system is generally designed to follow site topography and the vertical profile of the adjacent roadway. The most commonly used shapes for open channel ditches are trapezoidal and triangular, with the latter shape utilized mainly for ditches servicing small drainage areas.

The open ditches associated with the HIP and Hawthorne Road were sized with sufficient capacity to convey 1:100 year peak flow rates. As previously noted, the Rational Method Design Sheets (refer to Appendix 'A' for copy of the 1:100 year design sheet) were used to quantify the 1:100 year peak flow rates. The open ditch configuration was carried out utilizing Manning's relationship, along with the proposed geometry and slope of the channel. Two Storm Drainage Area Plans were prepared (refer to Drawings D-ST1 and D-ST2) showing proposed ditch inverts that match those shown on the Rational Method Design Sheets. Based on the ditch sizing exercise, it was determined that triangular shape ditches with 3:1 side slopes and variable depths provided the necessary conveyance of the 1:100 year peak flow rate. The Site Servicing and Grading Plan (refer to Drawing SG) was developed to provide the configuration of open ditch segments.

The existing open ditches along Rideau Road were also evaluated to ensure sufficient capacity was able to convey the 1:100 year peak flow rates resulting from upstream construction works (i.e., construction of Hawthorne Road). The Rational Method Design Sheets (refer to Appendix 'A' for copy of the 1:100 year design sheet) were used to quantify the 1:100 year peak flow rates. An existing 900 mm diameter culvert crossing under Hawthorne Road conveys flow along the north side of Rideau Road (refer to Drawing D-ST2). The capacity of this existing culvert was estimated at 1,400 L/s under a 1.5 m headwater (refer to Appendix 'B' for Culvert Design Summary Table). Upon the review of existing topography, any headwater depths greater than 1.5 m resulted in runoff being directed northerly along Hawthorne Road towards Findlay Creek. In light of the above, the existing open ditches along Rideau Road were evaluated using a conservative plug flow of 1,400 L/s in addition to surface runoff generated by the contributing areas.

3.2.2 Culvert System

The principal function of a culvert is to convey water through an embankment while, at the same time, supporting the weight of the overlying fill and vehicular movement. Culverts can be made of many different materials; steel, polyvinylchloride (PVC), high density polyethylene (HDPE) and concrete. Culverts selected for the HIP and Hawthorne Road are made of corrugated steel, in either round or arch shape. Field observations have shown that there are two major types of culvert flow conditions: inlet control and outlet control.

1. Flow Under Inlet Control

Flow with inlet control means that the discharge capacity of a culvert is controlled at the culvert entrance by the depth of headwater and by the entrance geometry, including the barrel shape, cross sectional area and the type of inlet edge. The roughness and length of the culvert barrel, and the outlet conditions are not factors in determining the culvert capacity. The longitudinal slope reduces headwater only to a small degree and can normally be neglected for conventional culverts flowing in inlet control.

2. Flow Under Outlet Control

Flow with outlet control means that the discharge capacity of a culvert is controlled by the depth of tailwater, including the velocity head within the barrel, the entrance and friction losses. The roughness, length of the culvert barrel, and slope are factors in determining the culvert capacity; the inlet geometry is of lesser importance.

To avoid having to conduct detailed hydraulic computations that would determine the type of flow under which a culvert will probably operate, the procedure recommended by the MTO (refer to MTO's Drainage Management Manual) was utilized. This methodology, referred to as the Conventional Culvert Design procedure, requires that MTO's Design Charts and Design Nomographs be used for both inlet and outlet control conditions. The higher headwater depth that is calculated from those two operating conditions would indicate the type of control and would provide the governing headwater depth. This methodology was utilized to size each culvert crossing, along with the 1:10 year peak flow rates calculated by the Rational Method Design Sheets (refer to Appendix 'A') for each of the conveyance segments. Furthermore, this calculation sheet also provides proposed culvert sizes, along with the type of control and governing depth found when using the conventional culvert design procedure. A summary of the various parameters estimated using MTO's nomographs at each of the culverts has been tabulated using MTO's Form D4-I (refer to Appendix 'B' for Conventional Culvert Design Sheet). This analysis shows that the proposed culvert crossings within the HIP and along Hawthorne Road are capable of conveying the 1:10 year peak flow rates as a minimum, without overtopping any of the roadway embankments. The hydraulic calculations were carried out assuming a roughness coefficient of 0.024 for any of the CSP and CSPA culverts. The Site Servicing and Grading Plan (Drawing SG) shows proposed culvert sizes, lengths and invert elevations at each of the crossings.

The proposed 1030 x 740 mm CSPA culvert crossing under the entrance of the pond access road was of concern due to the high flow rate during the 1:100 year storm event.

There was a possibility that the excess flow overtopping this culvert could short circuit into SWMF via the pond access road. Therefore, an analysis of the flow overtopping the proposed entrance culvert was conducted and the results confirmed that the residual flow would indeed be contained within the right-of-way corridor (refer to Appendix 'J' for desktop calculation).

4.0 WATER BALANCE

Water balance analyses are typically carried out to assess any changes in infiltration to subsurface water-bearing zones as a result of the urbanization (i.e., increase of hard surfaces) of land. The SCSS has identified the need to maintain a necessary level of quantity and quality groundwater recharge via infiltration. Groundwater recharge is required to maintain subsurface base flow to streams and wetlands in addition to maintaining groundwater levels for private and municipal wells. The Hydrogeological Study completed by Golder Associates Limited in 2008 for the HIP identified the site as being underlain by a shallow and deep aquifer separated by an impermeable rock layer. The upper aquifer provided subsurface groundwater flow to streams, while the lower aquifer was the main source for well water supply. Therefore, groundwater recharge for this site was intended to provide subsurface base flow into the receiving Findlay Creek.

Construction fill operations have been active for the HIP since 1994. The results of the geotechnical field investigation conducted by Inspec-Sol Incorporated in 2008 indicates that as much as 5.5 m of fill material (MW7-08) has been placed on parts of the site. The non-native heterogenous fill material is comprised mainly of silty clay and contains trace amounts of road and construction materials. Although the soil component of the fill material exhibits the characteristics of silty clay, the varying composition and density of the remaining portion of the fill affects its permeability in localized areas. Given the above existing conditions, it is difficult to determine how groundwater recharge will behave as subsurface flow in the existing fill matrix, particularly from individual sites within the HIP. The MOE expressed concerns about the use of infiltration strategies on the individual sites given the past history as a construction fill site. Furthermore, the MOE SWMPDM does not endorse the use of infiltration basins on lands zoned for industrial use as there is an increased risk of groundwater contamination should a spill occur on site.

An option was considered to provide infiltration for the entire site at the base of the end-of-pipe Dry Pond facility. Upon further investigation, the geotechnical report indicated

that there was a high groundwater table at the proposed pond location. In addition, in-situ soils in the area exhibited poor drainage properties which would have resulted in long retention times at the base of the pond, making it difficult to meet the water balance deficit requirements for the entire site while attempting to mimic the pre-development hydrological cycle.

Representatives from the City and SNC were consulted, and it was concluded that the SCSS groundwater balance targets for this site would be difficult to meet. It was also recognized that on-site infiltration strategies for this industrial subdivision could have a detrimental effect on groundwater quality and jeopardize the natural ecological integrity of receiving waters. In light of the above, it was decided by the approval authorities that the requirement for the water balance would be waived for the HIP development.

5.0 WATER QUALITY

5.1 General

Urbanization has been found to modify the hydrological regime of a receiving stream if inadequate stormwater management measures are implemented. The potential impacts associated with runoff arise primarily from the amount of urban area that is impervious to rain and snowmelt water. These impervious surfaces increase the amount of direct surface runoff that is generated and is conveyed more efficiently to the receiving stream. As part of the SCSS, fisheries resources have been inventoried along this watercourse, along with its associated tributaries. Given that the receiving watercourses were found to shelter fisheries, the approved document recommended that a "normal" level of protection be achieved. To fulfil this requirement, it is proposed that each individual site provide an oil/grit separator and infiltration storage be provided within the roadside open ditch system, as per the requirements presented in the SWMPDM.

5.2 Water Quality Requirement

Stormwater servicing for the HIP has been developed in accordance with the water quality recommendations of the SCSS (70% TSS removal). To fulfil this requirement, individual sites will be required to provide an oil/grit separator be installed to provide quality treatment (i.e., 70% TSS removal) of surface runoff before entering the roadside open ditch/culvert system. In addition, the oil/grit separator will be able to capture and contain hydrocarbons in the event of an on-site accidental spill.

To fulfill the water quality objectives for the paved portion of the HIP internal roads, it is proposed to provide infiltration within the open roadside ditch system to meet the storage volume requirements presented in Table 3.2 of the SWMPDM. Based on the normal level of service required and an imperviousness of 100% for the internal roads, Table 3.2 yields an extrapolated storage volume requirement of 35 m³/ha. To achieve this storage volume, a clear stone envelope complete with a 200 mm diameter perforated pipe will be installed at the base of the roadside ditches to meet the required storage volume (Refer to Appendix C for calculations).

The following table presents the calculated infiltration volume required for water quality control and those provided by the roadside open ditch system to meet the recommended MOE Design Guidelines.

Table 3 - Water Quality Infiltration Requirements

Phase	Area (ha)	Infiltration Volume Requirement (m ³)	Infiltration Method	Length of 200 mm diameter Perf. Pipe (m)	Infiltration Volume Provided (m ³)
1	1.58	55.1	Open Ditch	1760	55.3
2	0.21	7.4	Open Ditch	240	7.5
Total	1.79	62.5	Open Ditch	2000	62.8

As shown in the above Table, the infiltration volume provided by the proposed open roadside ditch network (62.8 m³) exceeds that obtained from Table 3.2 (62.5 m³) of the SWMPDM. It should be noted that additional storage within the void space of the clear stone envelope was not accounted for and would increase the actual infiltration storage volume shown in Table 3.

6.0 HYDROLOGICAL ANALYSIS

6.1 General

To satisfy the surface water objectives presented in Subsections 1.3 and 2.2, a hydrological analysis was carried out to quantify peak flow rate variations resulting from the development of the proposed HIP. To quantify this variation, the SWMHYMO Stormwater Management Hydrological Model (Version 4.02, July, 1999) was utilized to calculate peak flows during severe storm events.

To carry out the hydrological analysis, three storm drainage plans were developed; one representing the pre-development drainage conditions, one representing the post-development conditions for the current study area, Phase 1, and the other for the post-development drainage conditions, including future development, Phase 2. For each of these plans, subwatershed boundaries were delineated based on existing topography of the site and the proposed overland flow direction following development of the site (refer to Figures 2, 3 and 4 for details).

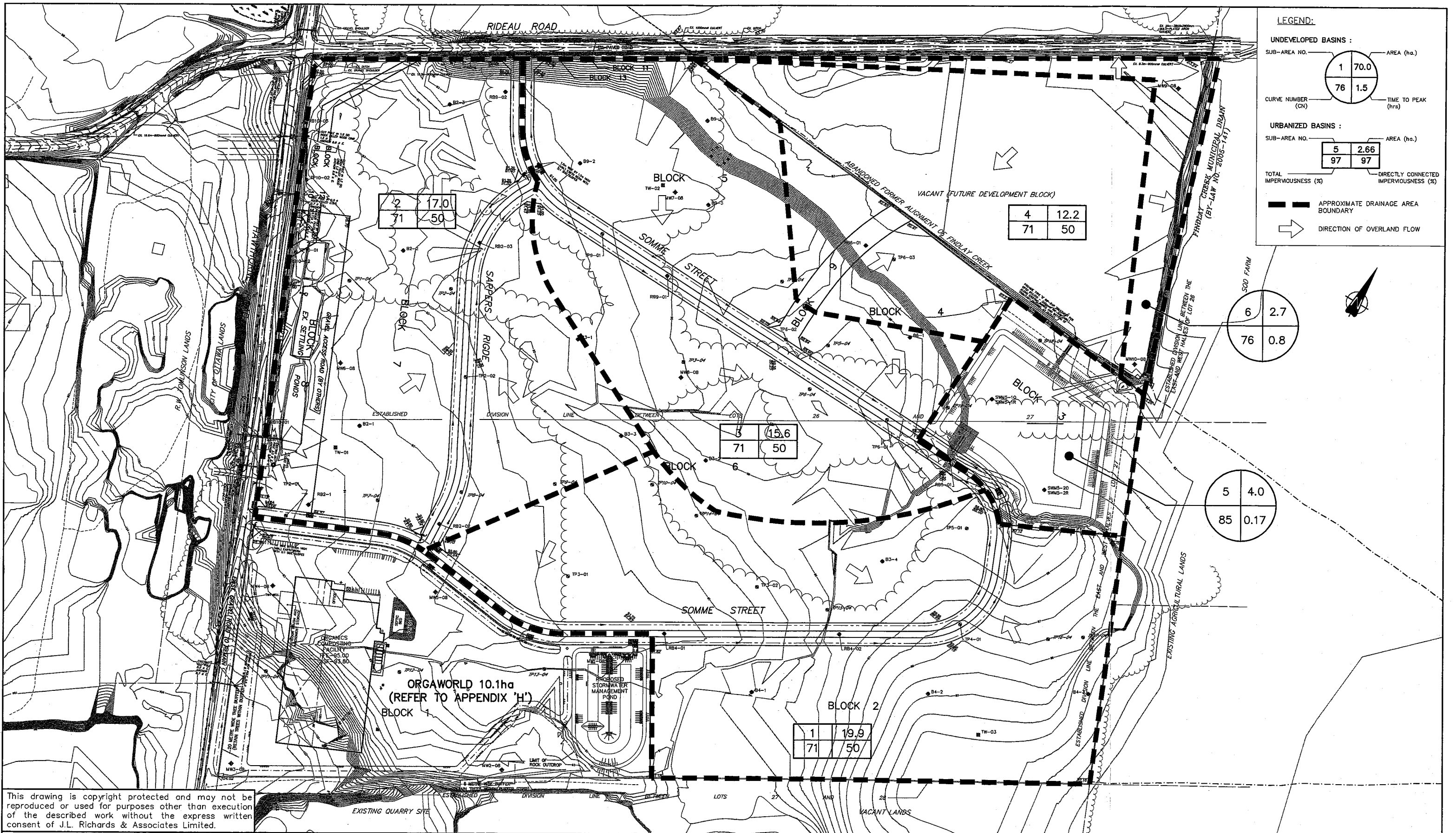
6.2 Synthetic Design Storm Simulation and Hydrological Parameters

Peak runoff rates were calculated for both pre- and post-development conditions using synthetic design storm event modelling. Peak flow rates were estimated using the 3-hour Chicago Design Storm Event, as this synthetic storm event has been recognized as the most critical event for urban runoff applications (refer to Section 5.4.3.1 of the City of Ottawa's Sewer Design Guidelines). The design storm analysis was completed using volumes derived from the Intensity-Duration-Frequency (IDF) curve equation shown in Section 5.4.2 of the City of Ottawa Sewer Design Guidelines compiled using data from 1967 to 1997.

A SWMHYMO data file was developed to represent both pre- and post-development conditions of the subject area. Simulation of surficial runoff generated from undeveloped subwatersheds was carried out using the "DESIGN NASHYD" command along with the SCS procedure to compute rainfall losses. The SCS procedure uses the Curve Number (CN) method to compute rainfall losses and the Nash unit hydrograph to simulate the hydrological response from undeveloped watersheds. To simulate surface runoff from urban subwatersheds, the "CALIB STANDHYD" command was utilized. Hydrological parameter selection and methodology is described below:

Curve Number (CN)

In order to estimate a Curve Number that represents pre-development conditions, the geotechnical investigation completed by Inspec-Sol, entitled "Geotechnical Study Subdivision Plan, Hawthorne Industrial Park, Lots 26 and 27 Concession 6, Southeast of Hawthorne and Rideau Roads, Ottawa, Ontario" dated December 19, 2008 was used. At the time of this investigation, large amounts of fill material were encountered over the majority of the site, which does not reflect the pre-development conditions. As such, only native soils encountered below fill material were used to establish pre-development condition Curve Numbers. The review of the geotechnical investigation shows native



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PROJECT:
**HAWTHORNE
INDUSTRIAL PARK**

DRAWING:
**POST DEVELOPMENT – PHASE 2
STORM DRAINAGE AREA
PLAN**

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ENGINEERS-ARCHITECTS-PLANNERS

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CHECKED: G.F.
PLOTTED: Apr 30, 2009

DRAWING NO.:
FIGURE 4
JLR NO:
20983

soils ranging from silty sand in Blocks 4 and 5, to silty clay in Blocks 3, 5, 7 and 8, to sandstone and limestone in parts of Blocks 2 and 3. These soils have been classified by Inspec-Sol as being associated with hydrologic soil groups (HSG), ranging from "B" to "D" for silty sand to silty clay, respectively. Areas where rock was encountered (i.e., Sandstone and Limestone) were classified as "Rockland." Based on this information and current land usage, as interpreted from aerial photography, a pre-development Curve Number (CN) of 76 has been calculated using the Ministry of Transportation of Ontario (MTO) Chart H2-8. Detailed calculations for the HIP have been included in Appendix 'D'.

Under post-development conditions, it is proposed to provide sufficient grade differential to allow for positive drainage to meet City of Ottawa Design Standards. As the subject lands are to be developed as an Industrial Park with a significant increase in hard surfaces (i.e., buildings, asphalt and gravel), the post-development conditions were, therefore, analysed taking into consideration the low potential of these surfaces to infiltrate storm runoff.

Imperviousness

Surface runoff under post-development conditions is greatly impacted by the imperviousness of its tributary area. Since the final development of the HIP is unknown, a conservative assumption for typical surfaces encountered in similar industrial parks was developed, as illustrated in Table 2. To determine the imperviousness based on the assumed breakdown presented in Table 2, an imperviousness calculation was carried out and is presented in Appendix 'D'. The imperviousness calculation was based on the following assumptions:

- an imperviousness of 100% was assigned for building footprints;
- an imperviousness of 100% was assigned for all asphalt parking surfaces.
- an imperviousness of 70% was assigned for all gravel surfaces; and
- it was assumed that 50% of the total imperviousness (TIMP) 50 % was modelled as directly connected imperviousness (XIMP).

Based on the above, a total imperviousness of 70% was calculated, which is equivalent to a runoff coefficient of 0.7. The hydrological analysis was, therefore, carried out using

a total imperviousness of 70%, consistent with the runoff coefficient used for sizing the open ditch/culvert system.

Time to Peak (T_p)

Time to peak calculations were carried out under pre-development conditions. Time of concentration was first estimated using the Uplands Method Chart based on the various flow paths. Once calculated, the times to peak were set to 67% (i.e., 2/3) of the time of concentration (T_c). Under pre-development conditions, a 90 minute time to peak was calculated (refer to Appendix 'D' for calculations). When modelling post-development conditions, the "CALIB STANDHYD" command was used to calculate the time to peak associated with the proposed site surfaces and grades (refer to Appendix 'E' for SWMHYMO outputs).

6.3 Simulation of Pre- and Post-Development (Uncontrolled) Conditions

The hydrological analysis was carried over the entire HIP under both the pre- and post-development conditions. As stated in Section 6.1, two post-development conditions were investigated, namely, Phase 1 and Phase 2. Phase 1 evaluates servicing for the current Study area, while Phase 2 includes the current Study area along with servicing of an additional 11.2 ha of land to the north east, shown on drawings as "Future Development Block."

Peak flow rates were computed with SWMHYMO using the procedure and parameters described in Subsection 6.2. Table 4 presents the simulated peak runoff rates under a 3 hour Chicago design storm event for both the pre- and post- (uncontrolled) development conditions for the HIP (refer to Appendix 'E' for SWMHYMO data input and output files), along with those under a 4 hour - 25 mm storm.

Table 4 - SWMHYMO Simulation Results

Return Period or Storm Depth	Peak Flow Rates (L/s)		
	Pre-Development	Phase 1 Post-Development (Uncontrolled)	Phase 2 Post-Development (Uncontrolled)
25 mm	252	1,941	2,231
2	467	3,077	3,548
5	826	4,812	5,554
10	1,097	6,135	7,029
25	1,468	7,772	9,013
50	1,767	9,240	10,588
100	2,093	10,662	12,132

Simulation results presented in the above table show that uncontrolled post-development peak flows substantially exceed those obtained under pre-development conditions. Based on the design criterion for water quantity (refer to Subsections 1.3 and 2.2 for details), post-development peak flows should be maintained to their pre-development levels for storm events ranging from a 1:5 year to a 1:100 year recurrence. In addition, the 2-year post-development peak flow should be controlled to 50% of the 2-year pre-development peak flow to satisfy the erosion criterion. Water quantity control measures were, therefore, found to be necessary for the development of this site. Details and stormwater servicing approaches proposed to fulfil the design criteria listed in Subsections 1.3 and 2.2 are presented in the following Subsections.

6.4 Simulation of Phase 1 Post-Development (Controlled) Conditions

Development of the subject lands (i.e., 70 ha, as illustrated on Figure 3) will increase the imperviousness of the subject area. To achieve the surface water objectives listed in Subsections 1.3 and 2.2, it is proposed that an end-of-pipe facility be constructed that would provide storage volume for retention of runoff.

The stormwater management criteria for the development of the HIP consist of maintaining erosion potential and peak flow rates at the pre-development levels. Storm servicing of the Subdivision was, therefore, developed such that all of these requirements were fulfilled, along with the achievement of a "normal" protection level. It

is proposed to implement the following stormwater management servicing approach for the development of the HIP:

End-of-Pipe SWMF (Block 3)

Based on the proposed grading, the end-of-pipe facility was found to generate a volume of 37,240 m³ (3.25 m depth). A low flow ditch sized for 2 year storm events was also included in the bottom of the end-of-pipe facility to convey flows to the outlet structure. The configuration of the outlet structure would be as follows:

- 1 x 150 mm diameter orifice within a 200 mm diameter Polyvinyl Chloride (PVC) pipe at elevation 82.90 m, which serves as outlet to the facility;
- 2 x 600 mm diameter Corrugated Steel Pipe culvert at elevation 84.80 m, which also serves as outlet to the facility;
- One (1) emergency overflow spillway (6.0 m wide) at elevation 86.15 m, which serves as outlet to the facility during a storm event greater than 1:100 year.

The above configuration was used to develop a Stage-Storage-Discharge relationship that relates the storativity and outlet capabilities of the proposed facility at various geodetic elevations (refer to Appendix 'F' for copy of this Table). This data (storage-discharge table) was then used as input to the SWMHYMO's ROUTE RESERVOIR command.

A SWMHYMO file, representing the post-development controlled conditions of the HIP, was developed incorporating the storage volume and the outflow capability of the proposed end-of-pipe facility. The following table presents the simulated peak runoff rates for the three (3) hour Chicago design storm under the post-development controlled conditions (refer to Appendix 'G' for SWMHYMO data input and output files), along with those under the four (4) hour - 25 mm storm.

**Table 5 - SWMHYMO Simulation Results
(Post-Development - Phase 1 Controlled Conditions)**

Return Period or Storm Depth	Peak Flow Rates (L/s)	
	Pre-Development	Phase 1 Post-Development (Controlled) ⁽¹⁾
25 mm	252	127
2 year	467	194 ⁽²⁾
5 year	826	359
10 year	1,097	589
25 year	1,468	939
50 year	1,767	1,191
100 year	2,093	1,531

Note: (1) Post-development flow is the sum of flows from the end-of-pipe facility and two uncontrolled Sub-Areas totalling 12.1 ha.

(2) 2 year post-development peak flow less than half the 2-year pre-development peak flow (233 L/s).

Simulation results presented in Table 5 show that the Phase 1 post-development controlled peak flows will be maintained below pre-development levels for the HIP. Consequently, the water quantity objective defined in Subsections 1.3 and 2.2 will be met under Phase 1.

6.5 Simulation of Phase 2 Post-Development (Controlled) Conditions

Development of Phase 2, as depicted on Figure 4, includes the Future Development Block located in the northeast corner of the HIP. This additional land could be serviced by the previously proposed end-of-pipe facility, without any modifications to facility size or outlet structure. However, a second inlet would be required in the northeast corner of the facility, which could be designed during the detailed design stage of the Future Development Block.

A SWMHYMO file, representing the Phase 2 post-development controlled conditions of the HIP, was developed incorporating the storage volume and the outflow capability of the proposed end-of-pipe facility. The following table presents the simulated peak runoff rates for the three (3) hour Chicago design storm under the Phase 2 post-development

controlled conditions (refer to Appendix 'H' for SWMHYMO data input and output files), along with those under the four (4) hour - 25 mm storm.

**Table 6 - SWMHYMO Simulation Results
(Post-Development - Phase 2 Controlled Conditions)**

Return Period or Storm Depth	Peak Flow Rates (L/s)	
	Pre-Development	Phase 2 Post-Development (Controlled) ⁽¹⁾
25 mm	252	73
2 year	467	156 ⁽²⁾
5 year	826	457
10 year	1,097	729
25 year	1,468	1,051
50 year	1,767	1,348
100 year	2,093	1,515

Note: (1) Post-development flow is the sum of flows from the end-of-pipe facility and one uncontrolled Sub-Area totalling 2.7 ha.

(2) 2-year post-development peak flow less than half the 2 year pre-development peak flow (233 L/s).

Simulation results presented in Table 6 show that the Phase 2 post-development controlled peak flows will be maintained below pre-development levels for the HIP. Consequently, the water quantity objective defined in Subsections 1.3 and 2.2 will also be met under Phase 2.

6.6 Simulation of the July 1, 1979 Historical Storm Event and Flood Potential

6.6.1 Simulation of the July 1, 1979 Historical Storm Event

In addition to designing the major drainage system to convey the 1:100 year storm event, the performance of both the open ditch system and SWMF was also assessed under the July 1, 1979 historical storm event. This historical storm event is defined as a high volume / low intensity storm event (when compared to the 1:100 year event) which

occurred mostly over a three hour period (refer to Table 5.6 in the Ottawa Sewer Design Guidelines). As shown in Table 5.6, the maximum intensity of 106.7 mm/hr only occurred for a 10 minute period (i.e., between the 85 to 95 minute time interval). The 1:100 year storm event intensities used to size the open ditch system were found to exceed the highest intensity of 106.7 mm/hr (refer to Appendix 'A' for 1:100 year Rational Method Sheet) with the exception of the most downstream ditch section (i.e., from Node 19 to Pond) where an intensity of 101.69 mm/hr was rather utilized. If an intensity of 106.7 mm/hr was used, the overall peak flow would increase from 12,814 L/s to 13,430 L/s substantially less than the free-flowing capacity of 52,735 L/s for the proposed ditch configuration. Consequently, the proposed open ditch system has the ability to convey flows generated by the July 1, 1979 storm event.

To supplement the above open ditch analysis, a hydrological analysis was also conducted to assess the performance of the SWMF under the July 1, 1979 storm event. A SWMHYMO file was, therefore, developed for the controlled Phase 2 post-development conditions of the HIP. Simulation results show that the Phase 2 post-development runoff during the July 1, 1979 storm event will be contained within the SWMF with all three of the outlet culverts flowing full in addition to approximately 210 mm of flow depth over the emergency overflow channel (refer to Appendix 'K' for SWMHYMO data input and output files). Therefore, the outlet of the SWMF has sufficient capacity to convey the July 1, 1979 historical storm event via the designated overland flow route without overtopping the banks.

6.6.2 Flood Potential

Draft approval Condition 12 of the draft subdivision conditions by the former Region of Ottawa-Carleton requires that "The owner shall complete a study indicating the extent of potential flooding on the property from Findlay Creek. The study including all models and assumptions shall be to the satisfaction of the South Nation River Conservation Authority." This condition was included as part of the original February 10, 1998 draft conditions (Gloucester File: S-RU-94-03).

Many changes have occurred on-site and adjacent to the site since Condition 12 was included in the draft approval for this site. Improvements to the roadside ditch were made along Rideau Road, immediately adjacent to the site. Surface runoff generated by the lands north of Rideau Road and conveyed to the small tributary located within the HIP site has now been re-directed toward the northeast corner of the site where the existing 3.8 m wide x 2.8 m high multi plate arch culvert crosses Rideau Road. A

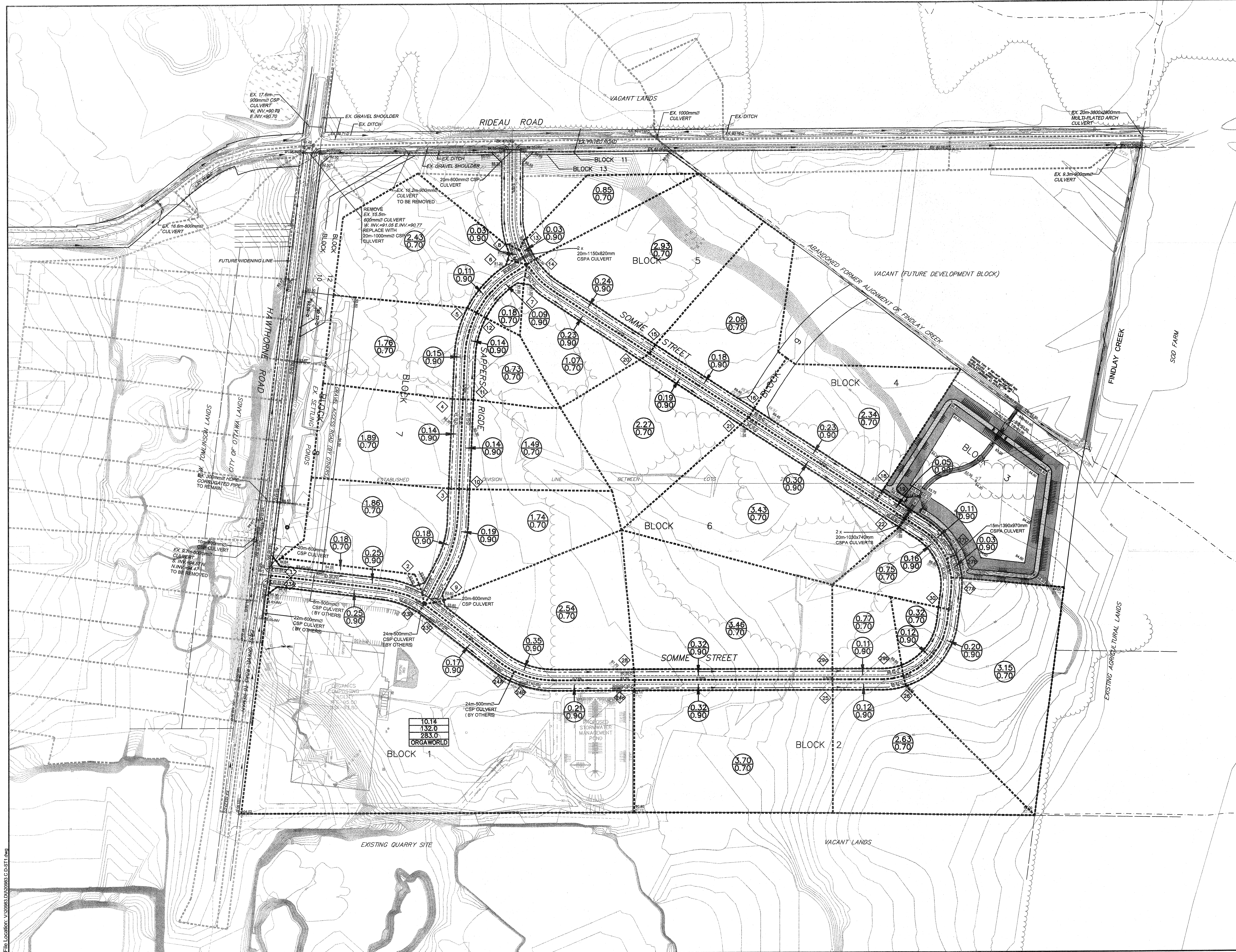
municipal drainage report was prepared by Stantec Consulting in 2004 for this section of Findlay Creek which assessed the overall geomorphological conditions and provided recommendations for future maintenance. In addition, the SCSS conducted a flood hazard analysis. The 100 year flows from the Stantec model were plotted along the creeks modelled. Floodlines were shown in Figure 6.2.3 of the report. No floodlines were indicated for the section of Findlay Creek adjacent to the HIP site.

As indicated previously in the Section 4 of this Report, as much as 5.5 m of construction fill has been added to the site since 1994. The placed fill material on the site has eliminated the natural low lying areas and raised the site grade approximately 4.5 m above the top of creek bank. The current site grades will be maintained as a minimum for the development of the HIP subdivision. Therefore, we have no concerns about flooding on the property from Findlay Creek given the above changes to the site and improvements to the adjacent drainage network. Consequently, Condition 12 of the draft approval should be considered as being satisfied on the basis that this condition is out of date based on the current site conditions.

7.0 EROSION AND SEDIMENT CONTROL MEASURES DURING CONSTRUCTION

During construction of the roadway, the collection systems (i.e., ditches, culverts, sewers, etc.) and end-of-pipe facility, appropriate erosion and sediment control measures, as outlined in MNR's "Guidelines on Erosion and Sediment Control for Urban Construction Sites," will be implemented to trap sediment on site. To ensure proper implementation, the proposed measures have been incorporated onto Drawing ESC (Drawing entitled "Erosion and Sedimentation Control Plan"). The measures shown on this Drawing were developed based on topography and site constraints. As a minimum, the following measures will be implemented during construction:

- Supply and installation of straw bale flow check dams (as per OPSD 219.180) at the upstream end of each culvert. Proposed locations of straw bale barriers are indicated on Drawing ESC.
- Supply and installation of topsoil and hydroseed along the entire open ditch system once grading has been completed for a section. Mulching will be carried out immediately after hydroseeding. This will allow for immediate bank stabilization of the system and will prevent sediment laden from occurring from exposed ditch surfaces.



Key Map

NOT TO SCALE

LEGEND

----- DRAINAGE BOUNDARY

2.91
0.70

AREA IN HECTARES
* RUNOFF COEFFICIENT (C)

10.14
132.0
283.0
ORGAWORLD

DRAINAGE AREA (ha)
10 YEAR PEAK FLOW (l/s)
100 YEAR PEAK FLOW (l/s)
ORGAWORLD SITE

28

NODE LOCATION NUMBER

PROPOSED DITCH AND FLOW DIRECTION

NOTE: RUNOFF COEFFICIENT (C) FOR DEVELOPMENT AREA IS BASED ON A WEIGHTED AVERAGE OF 0.70, WHILE ROADWAYS ARE 0.90.

03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

NO.	ISSUE	DATE
03	ISSUED FOR M.O.E. APPROVAL	28/05/09
02	REVISED PER CITY COMMENTS	30/04/09
01	ISSUED FOR CITY APPROVAL	12/02/09

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SCALE: 1:2000

0 25 50 100 150

J.L. Richards & Associates Limited
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203-863 Princess Street
Kingston, ON Canada
K7L 5N4
Tel: 613 544 1424
Fax: 613 544 5679

PROFESSIONAL STAMP

PROJECT NORTH

PROJECT:

HAWTHORNE INDUSTRIAL PARK

DRAWING:

STORM DRAINAGE AREA PLAN

There was a possibility that the excess flow overtopping this culvert could short circuit into SWMF via the pond access road. Therefore, an analysis of the flow overtopping the proposed entrance culvert was conducted and the results confirmed that the residual flow would indeed be contained within the right-of-way corridor (refer to Appendix 'J' for desktop calculation).

4.0 WATER BALANCE

Water balance analyses are typically carried out to assess any changes in infiltration to subsurface water-bearing zones as a result of the urbanization (i.e., increase of hard surfaces) of land. The SCSS has identified the need to maintain a necessary level of quantity and quality groundwater recharge via infiltration. Groundwater recharge is required to maintain subsurface base flow to streams and wetlands in addition to maintaining groundwater levels for private and municipal wells. The Hydrogeological Study completed by Golder Associates Limited in 2008 for the HIP identified the site as being underlain by a shallow and deep aquifer separated by an impermeable rock layer. The upper aquifer provided subsurface groundwater flow to streams, while the lower aquifer was the main source for well water supply. Therefore, groundwater recharge for this site was intended to provide subsurface base flow into the receiving Findlay Creek.

Construction fill operations have been active for the HIP since 1994. The results of the geotechnical field investigation conducted by Inspec-Sol Incorporated in 2008 indicates that as much as 5.5 m of fill material (MW7-08) has been placed on parts of the site. The non-native heterogenous fill material is comprised mainly of silty clay and contains trace amounts of road and construction materials. Although the soil component of the fill material exhibits the characteristics of silty clay, the varying composition and density of the remaining portion of the fill affects its permeability in localized areas. Given the above existing conditions, it is difficult to determine how groundwater recharge will behave as subsurface flow in the existing fill matrix, particularly from individual sites within the HIP. The MOE expressed concerns about the use of infiltration strategies on the individual sites given the past history as a construction fill site. Furthermore, the MOE SWMPDM does not endorse the use of infiltration basins on lands zoned for industrial use as there is an increased risk of groundwater contamination should a spill occur on site.

An option was considered to provide infiltration for the entire site at the base of the end-of-pipe Dry Pond facility. Upon further investigation, the geotechnical report indicated

that there was a high groundwater table at the proposed pond location. In addition, in-situ soils in the area exhibited poor drainage properties which would have resulted in long retention times at the base of the pond, making it difficult to meet the water balance deficit requirements for the entire site while attempting to mimic the pre-development hydrological cycle.

Representatives from the City and SNC were consulted, and it was concluded that the SCSS groundwater balance targets for this site would be difficult to meet. It was also recognized that on-site infiltration strategies for this industrial subdivision could have a detrimental effect on groundwater quality and jeopardize the natural ecological integrity of receiving waters. In light of the above, it was decided by the approval authorities that the requirement for the water balance would be waived for the HIP development.

5.0 WATER QUALITY

5.1 General

Urbanization has been found to modify the hydrological regime of a receiving stream if inadequate stormwater management measures are implemented. The potential impacts associated with runoff arise primarily from the amount of urban area that is impervious to rain and snowmelt water. These impervious surfaces increase the amount of direct surface runoff that is generated and is conveyed more efficiently to the receiving stream. As part of the SCSS, fisheries resources have been inventoried along this watercourse, along with its associated tributaries. Given that the receiving watercourses were found to shelter fisheries, the approved document recommended that a "normal" level of protection be achieved. To fulfil this requirement, it is proposed that each individual site provide an oil/grit separator and infiltration storage be provided within the roadside open ditch system, as per the requirements presented in the SWMPDM.

5.2 Water Quality Requirement

Stormwater servicing for the HIP has been developed in accordance with the water quality recommendations of the SCSS (70% TSS removal). To fulfil this requirement, individual sites will be required to provide an oil/grit separator be installed to provide quality treatment (i.e., 70% TSS removal) of surface runoff before entering the roadside open ditch/culvert system. In addition, the oil/grit separator will be able to capture and contain hydrocarbons in the event of an on-site accidental spill.

A P P E N D I X 'H'

SWMHYMO INPUT AND OUTPUT FILES (Post-Development Controlled Phase 2 Conditions)

```

00001> 2 Metric units
00002> *****
00003> # Project Name : Hawthorne Industrial Park Project Number: [20983] *
00004> # Date : January, 2009
00005> # Revised : WJA *
00006> # Developed by : Mark Buchanan, E.I.T.
00007> # Reviewed by : Guy Forget, P.Eng.
00008> # Company : J.L. Richards & Associates Limited
00009> # License # : 4418403
00010> *****
00011>
00012> #*****
00013> #*****
00014> # FILENAME: V:\20983.DU\ENG\SWMHYMO\20983PST.DAT *
00015> # FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00016> # OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00017> #*****
00018>
00019> *****
00020> # SWMHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE
00021> # PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00022> *****
00023>
00024> *****
00025> # HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00026> # FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00027> *****
00028>
00029> *****
00030> # POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00031> *****
00032>
00033> *****
00034> # CALCULATION OF 4 HR 25 MM STORM EVENT *
00035> *****
00036>
00037> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00038> # [ ] <- storm filename, one per line for NSTORM time
00039> READ STORM STORM_FILENAME=[4HR25-15.STM]
00040> #-----
00041> # DEFAULT VALUES ICASEdef=[1], read and print values
00042> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00043> #-----
00044>
00045> *****
00046> # ORGAWORLD FILE *
00047> *****
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00049> # SUB-AREA No.1
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00053> SCS curve number CN=[81],
00054> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00055> LGP=[20] (m), MNP=[0.25 ], SCP=[0.0] (m)
00056> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00057> LGI=[204.72] (m), MNI=[0.03 ], SCI=[0.0]
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00061> # SUB-AREA No.2
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00064> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00065> SCS curve number CN=[81],
00066> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00067> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00068> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00069> LGI=[244.34] (m), MNI=[0.03 ], SCI=[0.0]
00070> RAINFALL=[ , , , ] (mm/hr), END=-1
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00073> # SUB-AREA No.3
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00075> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[ 1.4 ] (ha),
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00077> SCS curve number CN=[81],
00078> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00079> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00080> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00081> LGI=[225.63] (m), MNI=[0.03 ], SCI=[0.0]
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00085> #-----
00086> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
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00089> # SUB-AREA No.4
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00091> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00092> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00093> SCS curve number CN=[81],
00094> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00095> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00096> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00097> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
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00104> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00105> SCS curve number CN=[81],
00106> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
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00108> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
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00113> #-----
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00119>
00120> TABLE of ( OUTFLOW-STORAGE ) values
00121> (cms) - (ha-m)
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00123> [ 0.008, 0.0656 ]
00124> [ 0.017, 0.1311 ]
00125> [ 0.093, 0.2831 ]
00126> [ 0.233, 0.3971 ]
00127> [ 0.337, 0.4731 ]
00128> [ 0.465, 0.5491 ]
00129> [ 0.531, 0.5871 ]
00130> [ 0.593, 0.6251 ]
00131> [ 0.654, 0.6631 ]
00132> [ 0.797, 0.7391 ]
00133> [ 0.950, 0.8274 ]
00134> [ 1.304, 0.9157 ]
00135> [ 1.880, 1.0040 ]
00136> [ 2.577, 1.0923 ]

```

```

00136> [ -1 , -1 ] (max twenty pts)
00137>
00138> *****
00139> # Remaining Hawthorne Industrial Park *
00140> *****
00141>
00142> # SUB-AREA No.1
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00145> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00146> SCS curve number CN=[81],
00147> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00148> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
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00150> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
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00159> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00160> SCS curve number CN=[81],
00161> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00162> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00163> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00164> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00165> RAINFALL=[ , , , ] (mm/hr), END=-1
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00167>
00168> # SUB-AREA No.3
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00170> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00171> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00172> SCS curve number CN=[81],
00173> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00174> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00175> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00176> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
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00180> #-----
00181> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00182> #-----
00183>
00184> # SUB-AREA No.4
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00186> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00187> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00188> SCS curve number CN=[81],
00189> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00190> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00191> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.7] (%),
00192> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
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00199> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00200> RAINFALL=[ , , , ] (mm/hr), END=-1
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00209>
00210> TABLE of ( OUTFLOW-STORAGE ) values
00211> (cms) - (ha-m)
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00213> [ 0.054, 0.2434 ]
00214> [ 0.059, 0.5834 ]
00215> [ 0.062, 0.8400 ]
00216> [ 0.064, 1.1024 ]
00217> [ 0.147, 1.3705 ]
00218> [ 0.280, 1.6444 ]
00219> [ 0.472, 1.9242 ]
00220> [ 0.724, 2.2097 ]
00221> [ 0.937, 2.5010 ]
00222> [ 1.262, 2.7981 ]
00223> [ 1.404, 3.1009 ]
00224> [ 1.532, 3.4096 ]
00225> [ 1.650, 3.7240 ]
00226> [ 2.409, 4.0442 ]
00227> [ 3.689, 4.3702 ]
00228> [ -1 , -1 ] (max twenty pts)
00229>
00230> #-----
00231>
00232> # SUB-AREA No. 6
00233>
00234> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00235> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
00236> RAINFALL=[ , , , ] (mm/hr), END=-1
00237> #-----
00238>
00239> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00240> #-----
00241>
00242>
00243> *****
00244> # CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00245> *****
00246>
00247> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00248> # [ ] <- storm filename, one per line for NSTORM time
00249> #-----
00250> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00251> ICASEcs=[1],
00252> A=[732.951], B=[6.199], and C=[0.810],
00253> #-----
00254> DEFAULT VALUES ICASEdef=[1], read and print values
00255> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00256> #-----
00257>
00258> *****
00259> # ORGAWORLD FILE *
00260> *****
00261>
00262> # SUB-AREA No.1
00263>
00264> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00265> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00266> SCS curve number CN=[81],
00267> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00268> LGP=[20] (m), MNP=[0.25 ], SCP=[0.0] (m)
00269> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00270> LGI=[204.72] (m), MNI=[0.03 ], SCI=[0.0]

```

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00271> RAINFALL=[ , , , ](mm/hr) , END=-1
00272> *%-----
00273> *
00274> * SUB-AREA No.2
00275> *
00276> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[1.54](ha),
XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[20.0](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.50](%),
LGI=[244.34](m), MNI=[0.03], SCI=[0.0](min)
00280> RAINFALL=[ , , , ](mm/hr) , END=-1
00281> *%-----
00282> *
00283> * SUB-AREA No.3
00284> *
00285> *
00286> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.51](%),
LGI=[225.63](m), MNI=[0.03], SCI=[0.0](min)
00294> RAINFALL=[ , , , ](mm/hr) , END=-1
00295> *%-----
00296> *
00297> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00298> *%-----
00299> *
00300> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
00301> *%-----
00302> *
00303> * SUB-AREA No.4
00304> *
00305> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[0.7](%),
LGP=[40](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.93](%),
LGI=[164.82](m), MNI=[0.03], SCI=[0.0](min)
00310> RAINFALL=[ , , , ](mm/hr) , END=-1
00311> *%-----
00312> *
00313> * SUB-AREA No.5
00314> *
00315> *
00316> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[20.0](m), MNP=[0.03], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
LGI=[207.25](m), MNI=[0.03], SCI=[0.0](min)
00322> RAINFALL=[ , , , ](mm/hr) , END=-1
00323> *%-----
00324> *
00325> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00326> *%-----
00327> *
00328> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00329> *%-----
00330> *
00331> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
RDT=[1.0](min)
00332> TABLE of ( OUTFLOW-STORAGE ) values
00333> (cms) - (ha-m)
00334> [ 0.000, 0.0000]
00335> [ 0.008, 0.0056]
00336> [ 0.017, 0.1311]
00337> [ 0.093, 0.2831]
00338> [ 0.233, 0.3971]
00339> [ 0.337, 0.4731]
00340> [ 0.465, 0.5491]
00341> [ 0.531, 0.5871]
00342> [ 0.593, 0.6251]
00343> [ 0.654, 0.6631]
00344> [ 0.797, 0.7391]
00345> [ 0.950, 0.8274]
00346> [ 1.304, 0.9157]
00347> [ 1.880, 1.0040]
00348> [ 2.577, 1.0923]
00349> [ -1, -1 ] (max twenty pts)
00350> *%-----
00351> *
00352> * Remaining Hawthorne Industrial Park *
00353> *
00354> *
00355> * SUB-AREA No.1
00356> *
00357> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5](min), AREA=[19.9](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.6](%),
LGI=[580](m), MNI=[0.03], SCI=[0.0](min)
00363> RAINFALL=[ , , , ](mm/hr) , END=-1
00364> *%-----
00365> *
00366> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00367> *%-----
00368> *
00369> * SUB-AREA No.2
00370> *
00371> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5](min), AREA=[17](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.65](%),
LGI=[450](m), MNI=[0.03], SCI=[0.0](min)
00378> RAINFALL=[ , , , ](mm/hr) , END=-1
00379> *%-----
00380> *
00381> * SUB-AREA No.3
00382> *
00383> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5](min), AREA=[15.6](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.5](%),
LGI=[600](m), MNI=[0.03], SCI=[0.0](min)
00390> RAINFALL=[ , , , ](mm/hr) , END=-1
00391> *%-----
00392> *
00393> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00394> *%-----
00395> *
00396> *
00397> * SUB-AREA No.4
00398> *
00399> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5](min), AREA=[12.2](ha),
XIMP=[0.50], TIMP=[0.71], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[100.0](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.7](%),
LGI=[210](m), MNI=[0.03], SCI=[0.0](min)
00405>

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00406> RAINFALL=[ , , , ](mm/hr) , END=-1
00407> *%-----
00408> *
00409> * SUB-AREA No.5
00410> *
00411> *
00412> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5](min), AREA=[4.0](ha),
DWF=[0.0](cms), CN/C=[ 85 ], TP=[0.17]hrs,
RAINFALL=[ , , , ](mm/hr) , END=-1
00415> *%-----
00416> *
00417> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00418> *%-----
00419> *
00420> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
RDT=[1.0](min)
00421> TABLE of ( OUTFLOW-STORAGE ) values
00422> (cms) - (ha-m)
00423> [ 0.0, 0.0 ]
00424> [ 0.048, 0.0574 ]
00425> [ 0.054, 0.2434 ]
00426> [ 0.059, 0.5834 ]
00427> [ 0.062, 0.8400 ]
00428> [ 0.064, 1.1024 ]
00429> [ 0.147, 1.3705 ]
00430> [ 0.280, 1.6444 ]
00431> [ 0.472, 1.9242 ]
00432> [ 0.724, 2.2097 ]
00433> [ 0.937, 2.5010 ]
00434> [ 1.262, 2.7981 ]
00435> [ 1.404, 3.1009 ]
00436> [ 1.532, 3.4096 ]
00437> [ 1.650, 3.7240 ]
00438> [ 2.409, 4.0442 ]
00439> [ 3.689, 4.3702 ]
00440> [ -1, -1 ] (max twenty pts)
00441> *%-----
00442> *
00443> *
00444> * SUB-AREA No. 6
00445> *
00446> *
00447> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5](min), AREA=[2.7](ha),
DWF=[0](cms), CN/C=[76], TP=[0.80]hrs,
RAINFALL=[ , , , ](mm/hr) , END=-1
00450> *%-----
00451> *
00452> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00453> *%-----
00454> *
00455> *
00456> * CALCULATION OF 3HR - 1.5 YEAR STORM EVENT *
00457> *
00458> *
00459> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00460> [ ] <- storm filename, one per line for NSTORM time
00461> *%-----
00462> *
00463> CHICAGO STORM IUNITS=[2], TD=[3.0](hrs), TPRAT=[0.333], CSDT=[10.0](min)
00464> ICASECS=[1],
A=[998.071], B=[6.053], and C=[0.814]
00465> *%-----
00466> *
00467> DEFAULT VALUES ICASEDEF=[1], read and print values
00468> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWHYMO\ORGA.VAL]
00469> *%-----
00470> *
00471> *
00472> *
00473> *
00474> * SUB-AREA No.1
00475> *
00476> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5](min), AREA=[2.07](ha),
XIMP=[0.84], TIMP=[0.84], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[20](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.52](%),
LGI=[204.72](m), MNI=[0.03], SCI=[0.0](min)
00483> RAINFALL=[ , , , ](mm/hr) , END=-1
00484> *%-----
00485> *
00486> * SUB-AREA No.2
00487> *
00488> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5](min), AREA=[1.54](ha),
XIMP=[0.92], TIMP=[0.92], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.50](%),
LGI=[244.34](m), MNI=[0.03], SCI=[0.0](min)
00496> RAINFALL=[ , , , ](mm/hr) , END=-1
00497> *%-----
00498> *
00499> * SUB-AREA No.3
00500> *
00501> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5](min), AREA=[1.4](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.0](%),
LGP=[5](m), MNP=[0.03], SCP=[0.0](min),
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.51](%),
LGI=[225.63](m), MNI=[0.03], SCI=[0.0](min)
00508> RAINFALL=[ , , , ](mm/hr) , END=-1
00509> *%-----
00510> *
00511> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
00512> *%-----
00513> *
00514> * SUB-AREA No.4
00515> *
00516> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5](min), AREA=[0.89](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[0.7](%),
LGP=[40](m), MNP=[0.25], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.93](%),
LGI=[164.82](m), MNI=[0.03], SCI=[0.0](min)
00524> RAINFALL=[ , , , ](mm/hr) , END=-1
00525> *%-----
00526> *
00527> * SUB-AREA No.5
00528> *
00529> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5](min), AREA=[2.66](ha),
XIMP=[0.97], TIMP=[0.97], DWF=[0.0](cms), LOSS=[2],
SCS curve number CN=[81],
Pervious surfaces: IAPer=[4.67](mm), SLPP=[1.5](%),
LGP=[20.0](m), MNP=[0.03], SCP=[0.0](min)
Impervious surfaces: IAImp=[1.57](mm), SLPI=[0.61](%),
LGI=[207.25](m), MNI=[0.03], SCI=[0.0](min)
00536> RAINFALL=[ , , , ](mm/hr) , END=-1
00537> *%-----
00538> *
00539> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
00540> *%-----
00541> *
00542> *
00543> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
00544> *%-----

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00541>
00542> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00543> RDT=[1.0] (min),
00544> TABLE of ( OUTFLOW-STORAGE ) values
00545> (cms) - (ha-m)
00546> [ 0.000, 0.0000 ]
00547> [ 0.008, 0.0656 ]
00548> [ 0.017, 0.1311 ]
00549> [ 0.093, 0.2831 ]
00550> [ 0.233, 0.3971 ]
00551> [ 0.337, 0.4731 ]
00552> [ 0.465, 0.5491 ]
00553> [ 0.531, 0.5871 ]
00554> [ 0.593, 0.6251 ]
00555> [ 0.654, 0.6631 ]
00556> [ 0.797, 0.7391 ]
00557> [ 0.950, 0.8274 ]
00558> [ 1.304, 0.9157 ]
00559> [ 1.880, 1.0040 ]
00560> [ 2.577, 1.0923 ]
00561> [ -1, -1 ] (max twenty pts)
00562>
00563> *****
00564> * Remaining Hawthorne Industrial Park *
00565> *****
00566> *
00567> * SUB-AREA No.1
00568>
00569> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00570> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00571> SCS curve number CN=[81],
00572> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00573> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00574> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00575> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00576> RAINFALL=[ , , , ] (mm/hr), END=-1
00577> *
00578> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00579> *
00580> *
00581> * SUB-AREA No.2
00582>
00583> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00584> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00585> SCS curve number CN=[81],
00586> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00587> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00588> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00589> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00590> RAINFALL=[ , , , ] (mm/hr), END=-1
00591> *
00592> *
00593> * SUB-AREA No.3
00594>
00595> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00596> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00597> SCS curve number CN=[81],
00598> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00599> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00600> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.5] (%),
00601> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
00602> RAINFALL=[ , , , ] (mm/hr), END=-1
00603> *
00604> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00605> *
00606> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00607> *
00608> *
00609> * SUB-AREA No.4
00610>
00611> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00612> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00613> SCS curve number CN=[81],
00614> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00615> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00616> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.7] (%),
00617> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
00618> RAINFALL=[ , , , ] (mm/hr), END=-1
00619> *
00620> *
00621> * SUB-AREA No.5
00622>
00623> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00624> DWF=[0] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00625> RAINFALL=[ , , , ] (mm/hr), END=-1
00626> *
00627> *
00628>
00629> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00630> *
00631> *
00632> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
00633> RDT=[1.0] (min),
00634> TABLE of ( OUTFLOW-STORAGE ) values
00635> (cms) - (ha-m)
00636> [ 0.0, 0.0 ]
00637> [ 0.048, 0.0574 ]
00638> [ 0.054, 0.2434 ]
00639> [ 0.059, 0.5834 ]
00640> [ 0.062, 0.8400 ]
00641> [ 0.064, 1.1024 ]
00642> [ 0.147, 1.3705 ]
00643> [ 0.280, 1.6444 ]
00644> [ 0.472, 1.9242 ]
00645> [ 0.724, 2.2097 ]
00646> [ 0.937, 2.5010 ]
00647> [ 1.262, 2.7981 ]
00648> [ 1.404, 3.1009 ]
00649> [ 1.532, 3.4096 ]
00650> [ 1.650, 3.7240 ]
00651> [ 2.409, 4.0442 ]
00652> [ 3.689, 4.3702 ]
00653> [ -1, -1 ] (max twenty pts)
00654>
00655> *
00656> *
00657> * SUB-AREA No. 6
00658>
00659> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00660> DWF=[0] (cms), CN=[76], TP=[0.80] hrs,
00661> RAINFALL=[ , , , ] (mm/hr), END=-1
00662> *
00663>
00664> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00665> *
00666> *
00667> *****
00668> * CALCULATION OF 3HR - 1:10 YEAR STORM EVENT *
00669> *****
00670>
00671> TZERO=[0.0], MFCOUT=[2], NSTORM=[0], NRUN=[0]
00672> *
00673> * C- storm filename, one per line for NSTORM time
00674> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00675> ICASEcs=[1],

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00676> A=[1174.184], B=[6.014], and C=[0.816],
00677> *
00678> DEFAULT VALUES ICASEdef=[1], read and print values
00679> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00680> *
00681> *****
00682> *
00683> * ORGAWORLD FILE *
00684> *****
00685> *
00686> * SUB-AREA No.1
00687>
00688> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00689> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00690> SCS curve number CN=[81],
00691> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00692> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (m)
00693> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.52] (%),
00694> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min)
00695> RAINFALL=[ , , , ] (mm/hr), END=-1
00696> *
00697> *
00698> * SUB-AREA No.2
00699>
00700> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00701> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00702> SCS curve number CN=[81],
00703> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00704> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min)
00705> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.50] (%),
00706> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0] (min)
00707> RAINFALL=[ , , , ] (mm/hr), END=-1
00708> *
00709> *
00710> * SUB-AREA No.3
00711>
00712> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00713> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00714> SCS curve number CN=[81],
00715> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00716> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min)
00717> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.51] (%),
00718> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0] (min)
00719> RAINFALL=[ , , , ] (mm/hr), END=-1
00720> *
00721> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
00722> *
00723> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
00724> *
00725> *
00726> * SUB-AREA No.4
00727>
00728> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00729> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00730> SCS curve number CN=[81],
00731> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00732> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
00733> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.93] (%),
00734> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min)
00735> RAINFALL=[ , , , ] (mm/hr), END=-1
00736> *
00737> *
00738> * SUB-AREA No.5
00739>
00740> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00741> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00742> SCS curve number CN=[81],
00743> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00744> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (m)
00745> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.61] (%),
00746> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min)
00747> RAINFALL=[ , , , ] (mm/hr), END=-1
00748> *
00749> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
00750> *
00751> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
00752> *
00753>
00754> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00755> RDT=[1.0] (min),
00756> TABLE of ( OUTFLOW-STORAGE ) values
00757> (cms) - (ha-m)
00758> [ 0.000, 0.0000 ]
00759> [ 0.008, 0.0656 ]
00760> [ 0.017, 0.1311 ]
00761> [ 0.093, 0.2831 ]
00762> [ 0.233, 0.3971 ]
00763> [ 0.337, 0.4731 ]
00764> [ 0.465, 0.5491 ]
00765> [ 0.531, 0.5871 ]
00766> [ 0.593, 0.6251 ]
00767> [ 0.654, 0.6631 ]
00768> [ 0.797, 0.7391 ]
00769> [ 0.950, 0.8274 ]
00770> [ 1.304, 0.9157 ]
00771> [ 1.880, 1.0040 ]
00772> [ 2.577, 1.0923 ]
00773> [ -1, -1 ] (max twenty pts)
00774>
00775> *****
00776> * Remaining Hawthorne Industrial Park *
00777> *****
00778> *
00779> * SUB-AREA No.1
00780>
00781> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00782> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00783> SCS curve number CN=[81],
00784> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00785> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00786> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.6] (%),
00787> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
00788> RAINFALL=[ , , , ] (mm/hr), END=-1
00789> *
00790> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
00791> *
00792> *
00793> * SUB-AREA No.2
00794>
00795> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
00796> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00797> SCS curve number CN=[81],
00798> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00799> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m)
00800> Impervious surfaces: IAIMP=[1.57] (mm), SLPI=[0.65] (%),
00801> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
00802> RAINFALL=[ , , , ] (mm/hr), END=-1
00803> *
00804> *
00805> * SUB-AREA No.3
00806>
00807> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
00808> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00809> SCS curve number CN=[81],
00810> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),

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00811> Impervious surfaces: LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00812> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.5] (%),
00813> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min
00814> RAINFALL=[ , , , ] (mm/hr), END=-1
00815> *%-----
00816> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
00817> *%-----
00818> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
00819> *%-----
00820> *
00821> * SUB-AREA No.4
00822>
00823> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
00824> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00825> SCS curve number CN=[81],
00826> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00827> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00828> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.7] (%),
00829> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min
00830> RAINFALL=[ , , , ] (mm/hr), END=-1
00831>
00832> *%-----
00833> *
00834> * SUB-AREA No.5
00835>
00836> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
00837> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
00838> RAINFALL=[ , , , ] (mm/hr), END=-1
00839> *%-----
00840>
00841> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
00842> *%-----
00843>
00844> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
00845> RDT=[1.0] (min),
00846> TABLE of ( OUTFLOW-STORAGE ) values
00847> (cms) - (ha-m)
00848> [ 0.0, 0.0 ]
00849> [ 0.048, 0.0574 ]
00850> [ 0.054, 0.2434 ]
00851> [ 0.059, 0.5834 ]
00852> [ 0.062, 0.8400 ]
00853> [ 0.064, 1.1024 ]
00854> [ 0.147, 1.3705 ]
00855> [ 0.280, 1.6444 ]
00856> [ 0.472, 1.9242 ]
00857> [ 0.724, 2.2097 ]
00858> [ 0.937, 2.5010 ]
00859> [ 1.262, 2.7981 ]
00860> [ 1.404, 3.1009 ]
00861> [ 1.532, 3.4096 ]
00862> [ 1.650, 3.7240 ]
00863> [ 2.409, 4.0442 ]
00864> [ 3.689, 4.3702 ]
00865> [ -1, -1 ] (max twenty pts)
00866>
00867> *%-----
00868> *
00869> * SUB-AREA No. 6
00870>
00871> DESIGN NASHYD ID=[ 1 ], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
00872> DWF=[0] (cms), CNC=[76], TP=[0.80] hrs,
00873> RAINFALL=[ , , , ] (mm/hr), END=-1
00874> *%-----
00875>
00876> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
00877> *%-----
00878>
00879>
00880> ***** CALCULATION OF 3HR - 1.25 YEAR STORM EVENT *****
00881> *****
00882>
00883>
00884> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
00885> *%-----
00886> * [ ] <- storm filename, one per line for NSTORM time
00887> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
00888> ICASECS=[1],
00889> A=[1402.884], B=[6.018], and C=[0.819],
00890> *%-----
00891> DEFAULT VALUES ICASEDEF=[1], read and print values
00892> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
00893> *%-----
00894>
00895> ***** ORGA WORLD FILE *****
00896> *
00897> *
00898> *
00899> * SUB-AREA No.1
00900>
00901> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
00902> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
00903> SCS curve number CN=[81],
00904> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00905> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00906> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.52] (%),
00907> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0] (min
00908> RAINFALL=[ , , , ] (mm/hr), END=-1
00909> *%-----
00910> *
00911> * SUB-AREA No.2
00912>
00913> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
00914> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
00915> SCS curve number CN=[81],
00916> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00917> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00918> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.50] (%),
00919> LGI=[244.34] (m), MNI=[0.03], SCI=[0.0] (min
00920> RAINFALL=[ , , , ] (mm/hr), END=-1
00921> *%-----
00922> *
00923> * SUB-AREA No.3
00924>
00925> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
00926> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00927> SCS curve number CN=[81],
00928> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
00929> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
00930> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.51] (%),
00931> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0] (min
00932> RAINFALL=[ , , , ] (mm/hr), END=-1
00933> *%-----
00934> ADD HYD IDsum=[4], NHYD=[ "040"], IDs to add=[1+2]
00935> *%-----
00936> ADD HYD IDsum=[5], NHYD=[ "050"], IDs to add=[3+4]
00937> *%-----
00938> *
00939> * SUB-AREA No.4
00940>
00941> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
00942> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00943> SCS curve number CN=[81],
00944> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
00945> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min
00946> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.93] (%),
00947> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0] (min
00948> RAINFALL=[ , , , ] (mm/hr), END=-1
00949> *%-----
00950> *
00951> * SUB-AREA No.5
00952>
00953> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
00954> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
00955> SCS curve number CN=[81],
00956> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00957> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min
00958> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.61] (%),
00959> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0] (min
00960> RAINFALL=[ , , , ] (mm/hr), END=-1
00961> *%-----
00962> ADD HYD IDsum=[8], NHYD=[ "080"], IDs to add=[6+7]
00963> *%-----
00964> ADD HYD IDsum=[9], NHYD=[ "090"], IDs to add=[5+8]
00965> *%-----
00966>
00967> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
00968> RDT=[1.0] (min),
00969> TABLE of ( OUTFLOW-STORAGE ) values
00970> (cms) - (ha-m)
00971> [ 0.000, 0.0000 ]
00972> [ 0.008, 0.0656 ]
00973> [ 0.017, 0.1311 ]
00974> [ 0.093, 0.2831 ]
00975> [ 0.233, 0.3971 ]
00976> [ 0.337, 0.4731 ]
00977> [ 0.465, 0.5491 ]
00978> [ 0.531, 0.5871 ]
00979> [ 0.593, 0.6251 ]
00980> [ 0.654, 0.6631 ]
00981> [ 0.797, 0.7391 ]
00982> [ 0.950, 0.8274 ]
00983> [ 1.304, 0.9157 ]
00984> [ 1.880, 1.0040 ]
00985> [ 2.577, 1.0923 ]
00986> [ -1, -1 ] (max twenty pts)
00987>
00988> ***** Remaining Hawthorne Industrial Park *****
00989> *
00990> *****
00991> *
00992> * SUB-AREA No.1
00993>
00994> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
00995> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
00996> SCS curve number CN=[81],
00997> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
00998> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
00999> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.6] (%),
01000> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min
01001> RAINFALL=[ , , , ] (mm/hr), END=-1
01002> *%-----
01003> ADD HYD IDsum=[ 2 ], NHYD=["HIP02"], IDs to add=[10+1]
01004> *%-----
01005> *
01006> * SUB-AREA No.2
01007>
01008> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01009> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01010> SCS curve number CN=[81],
01011> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01012> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
01013> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.65] (%),
01014> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min
01015> RAINFALL=[ , , , ] (mm/hr), END=-1
01016> *%-----
01017> *
01018> * SUB-AREA No.3
01019>
01020> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
01021> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01022> SCS curve number CN=[81],
01023> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01024> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
01025> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.5] (%),
01026> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min
01027> RAINFALL=[ , , , ] (mm/hr), END=-1
01028> *%-----
01029> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01030> *%-----
01031> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
01032> *%-----
01033> *
01034> * SUB-AREA No.4
01035>
01036> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
01037> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01038> SCS curve number CN=[81],
01039> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01040> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (m
01041> Impervious surfaces: IAlimp=[1.57] (mm), SLPI=[0.7] (%),
01042> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min
01043> RAINFALL=[ , , , ] (mm/hr), END=-1
01044> *%-----
01045> *
01046> * SUB-AREA No.5
01047>
01048> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
01049> DWF=[ 0 ] (cms), CN/C=[ 85 ], TP=[0.17] hrs,
01050> RAINFALL=[ , , , ] (mm/hr), END=-1
01051> *%-----
01052> *
01053>
01054> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
01055> *%-----
01056>
01057> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
01058> RDT=[1.0] (min),
01059> TABLE of ( OUTFLOW-STORAGE ) values
01060> (cms) - (ha-m)
01061> [ 0.0, 0.0 ]
01062> [ 0.048, 0.0574 ]
01063> [ 0.054, 0.2434 ]
01064> [ 0.059, 0.5834 ]
01065> [ 0.062, 0.8400 ]
01066> [ 0.064, 1.1024 ]
01067> [ 0.147, 1.3705 ]
01068> [ 0.280, 1.6444 ]
01069> [ 0.472, 1.9242 ]
01070> [ 0.724, 2.2097 ]
01071> [ 0.937, 2.5010 ]
01072> [ 1.262, 2.7981 ]
01073> [ 1.404, 3.1009 ]
01074> [ 1.532, 3.4096 ]
01075> [ 1.650, 3.7240 ]
01076> [ 2.409, 4.0442 ]
01077> [ 3.689, 4.3702 ]
01078> [ -1, -1 ] (max twenty pts)
01079>
01080> *%-----

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01081> *
01082> *SUB-AREA No. 6
01083>
01084> DESIGN NASHYD ID = [ 1 ], NHYD=["A3"], DT=[2.5]min, AREA=[2.7] (ha),
01085> DWF=[0] (cms), CNC=[76], TP=[0.80]hrs,
01086> RAINFALL=[ , , , ] (mm/hr), END=-1
01087> *%-----
01088>
01089> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
01090> *%-----
01091>
01092> *****
01093> * CALCULATION OF 3HR - 1:50 YEAR STORM EVENT *
01094> *****
01095>
01096> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01097> *%-----
01098> * [ ] <- storm filename, one per line for NSTORM time
01099>
01100> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
01101> ICASEdef=[1],
01102> A=[1569.580], B=[6.014], and C=[0.820],
01103> *%-----
01104> DEFAULT VALUES ICASEdef=[1], read and print values
01105> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
01106> *%-----
01107> *****
01108> * ORGAWORLD FILE *
01109> *****
01110>
01111> * SUB-AREA No.1
01112>
01113> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
01114> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
01115> SCS curve number CN=[81],
01116> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01117> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
01118> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01119> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01120> RAINFALL=[ , , , ] (mm/hr), END=-1
01121> *%-----
01122>
01123> * SUB-AREA No.2
01124>
01125> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
01126> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
01127> SCS curve number CN=[81],
01128> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01129> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01130> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
01131> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01132> RAINFALL=[ , , , ] (mm/hr), END=-1
01133> *%-----
01134>
01135> * SUB-AREA No.3
01136>
01137> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
01138> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01139> SCS curve number CN=[81],
01140> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01141> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01142> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
01143> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
01144> RAINFALL=[ , , , ] (mm/hr), END=-1
01145> *%-----
01146> ADD HYD IDsum=[4], NHYD=[ "040" ], IDs to add=[1+2]
01147> *%-----
01148> ADD HYD IDsum=[5], NHYD=[ "050" ], IDs to add=[3+4]
01149> *%-----
01150>
01151> * SUB-AREA No.4
01152>
01153> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
01154> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01155> SCS curve number CN=[81],
01156> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
01157> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
01158> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
01159> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
01160> RAINFALL=[ , , , ] (mm/hr), END=-1
01161> *%-----
01162>
01163> * SUB-AREA No.5
01164>
01165> CALIB STANDHYD ID=[ 7 ], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
01166> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01167> SCS curve number CN=[81],
01168> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01169> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01170> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01171> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01172> RAINFALL=[ , , , ] (mm/hr), END=-1
01173> *%-----
01174> ADD HYD IDsum=[8], NHYD=[ "080" ], IDs to add=[6+7]
01175> *%-----
01176> ADD HYD IDsum=[9], NHYD=[ "090" ], IDs to add=[5+8]
01177> *%-----
01178>
01179> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
01180> RDT=[1.0] (min),
01181> TABLE of ( OUTFLOW-STORAGE ) values
01182> ( cms ) - ( ha-m )
01183> [ 0.000, 0.0000 ]
01184> [ 0.008, 0.0656 ]
01185> [ 0.017, 0.1311 ]
01186> [ 0.093, 0.2831 ]
01187> [ 0.233, 0.3971 ]
01188> [ 0.337, 0.4731 ]
01189> [ 0.465, 0.5491 ]
01190> [ 0.531, 0.5871 ]
01191> [ 0.593, 0.6251 ]
01192> [ 0.654, 0.6631 ]
01193> [ 0.797, 0.7391 ]
01194> [ 0.950, 0.8274 ]
01195> [ 1.304, 0.9157 ]
01196> [ 1.880, 1.0040 ]
01197> [ 2.577, 1.0923 ]
01198> [ -1, -1 ] (max twenty pts)
01199>
01200> *****
01201> * Remaining Hawthorne Industrial Park *
01202> *****
01203>
01204> * SUB-AREA No.1
01205>
01206> CALIB STANDHYD ID=[ 1 ], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01207> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01208> SCS curve number CN=[81],
01209> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01210> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01211> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.6] (%),
01212> LGI=[580] (m), MNI=[0.03], SCI=[0.0] (min)
01213> RAINFALL=[ , , , ] (mm/hr), END=-1
01214> *%-----
01215> ADD HYD IDsum=[2], NHYD=["HIP02"], IDs to add=[10+1]
01216>
01217>
01218> * SUB-AREA No.2
01219>
01220> CALIB STANDHYD ID=[ 3 ], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01221> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01222> SCS curve number CN=[81],
01223> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01224> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01225> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
01226> LGI=[450] (m), MNI=[0.03], SCI=[0.0] (min)
01227> RAINFALL=[ , , , ] (mm/hr), END=-1
01228> *%-----
01229>
01230> * SUB-AREA No.3
01231>
01232> CALIB STANDHYD ID=[ 4 ], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
01233> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01234> SCS curve number CN=[81],
01235> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01236> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01237> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01238> LGI=[600] (m), MNI=[0.03], SCI=[0.0] (min)
01239> RAINFALL=[ , , , ] (mm/hr), END=-1
01240> *%-----
01241> ADD HYD IDsum=[ 5 ], NHYD=["HIP05"], IDs to add=[3+4]
01242> *%-----
01243> ADD HYD IDsum=[ 6 ], NHYD=["HIP06"], IDs to add=[5+2]
01244> *%-----
01245>
01246> * SUB-AREA No.4
01247>
01248> CALIB STANDHYD ID=[ 7 ], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
01249> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01250> SCS curve number CN=[81],
01251> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01252> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (mi)
01253> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
01254> LGI=[210] (m), MNI=[0.03], SCI=[0.0] (min)
01255> RAINFALL=[ , , , ] (mm/hr), END=-1
01256> *%-----
01257>
01258> * SUB-AREA No.5
01259>
01260> DESIGN NASHYD ID=[ 8 ], NHYD=["Pond-Block"], DT=[2.5]min, AREA=[4.0] (ha),
01261> DWF=[ 0 ] (cms), CNC=[ 85 ], TP=[0.17]hrs,
01262> RAINFALL=[ , , , ] (mm/hr), END=-1
01263> *%-----
01264>
01265> ADD HYD IDsum=[ 9 ], NHYD=["HIP08"], IDs to add=[6+7+8]
01266> *%-----
01267>
01268> ROUTE RESERVOIR IDout=[ 10 ], NHYD=["HIP-POND"], IDin=[ 9 ],
01269> RDT=[1.0] (min),
01270> TABLE of ( OUTFLOW-STORAGE ) values
01271> ( cms ) - ( ha-m )
01272> [ 0.0, 0.0 ]
01273> [ 0.048, 0.0574 ]
01274> [ 0.054, 0.2434 ]
01275> [ 0.059, 0.5834 ]
01276> [ 0.062, 0.8400 ]
01277> [ 0.064, 1.1024 ]
01278> [ 0.147, 1.3705 ]
01279> [ 0.280, 1.6444 ]
01280> [ 0.472, 1.9242 ]
01281> [ 0.724, 2.2097 ]
01282> [ 0.937, 2.5010 ]
01283> [ 1.262, 2.7981 ]
01284> [ 1.404, 3.1009 ]
01285> [ 1.532, 3.4096 ]
01286> [ 1.650, 3.7240 ]
01287> [ 2.409, 4.0442 ]
01288> [ 3.689, 4.3702 ]
01289> [ -1, -1 ] (max twenty pts)
01290>
01291>
01292> * SUB-AREA No. 6
01293>
01294> DESIGN NASHYD ID = [1], NHYD=["A3"], DT=[2.5]min, AREA=[2.7] (ha),
01295> DWF=[0] (cms), CNC=[76], TP=[0.80]hrs,
01296> RAINFALL=[ , , , ] (mm/hr), END=-1
01297> *%-----
01298>
01299> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
01300> *%-----
01301>
01302> *****
01303> * CALCULATION OF 3HR - 1:100 YEAR STORM EVENT *
01304> *****
01305>
01306> START TZERO=[0.0], METOUT=[2], NSTORM=[0], NRUN=[0]
01307> *%-----
01308> * [ ] <- storm filename, one per line for NSTORM time
01309>
01310> CHICAGO STORM IUNITS=[2], TD=[3.0] (hrs), TPRAT=[0.333], CSDT=[10.0] (min)
01311> ICASEdef=[1],
01312> A=[1735.688], B=[6.014], and C=[0.820],
01313> *%-----
01314> DEFAULT VALUES ICASEdef=[1], read and print values
01315> DEFVAL_FILENAME=[V:\22973.DU\ENG\SWMHYMO\ORGA.VAL]
01316> *%-----
01317> *****
01318> * ORGAWORLD FILE *
01319> *****
01320>
01321> * SUB-AREA No.1
01322>
01323> CALIB STANDHYD ID=[ 1 ], NHYD=["010"], DT=[2.5] (min), AREA=[ 2.07 ] (ha),
01324> XIMP=[0.84], TIMP=[0.84], DWF=[0.0] (cms), LOSS=[2],
01325> SCS curve number CN=[81],
01326> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01327> LGP=[20] (m), MNP=[0.25], SCP=[0.0] (mi)
01328> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.52] (%),
01329> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01330> RAINFALL=[ , , , ] (mm/hr), END=-1
01331> *%-----
01332>
01333> * SUB-AREA No.2
01334>
01335> CALIB STANDHYD ID=[ 2 ], NHYD=["020"], DT=[2.5] (min), AREA=[ 1.54 ] (ha),
01336> XIMP=[0.92], TIMP=[0.92], DWF=[0.0] (cms), LOSS=[2],
01337> SCS curve number CN=[81],
01338> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01339> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01340> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.50] (%),
01341> LGI=[204.72] (m), MNI=[0.03], SCI=[0.0]
01342> RAINFALL=[ , , , ] (mm/hr), END=-1
01343> *%-----
01344>
01345> * SUB-AREA No.3
01346>
01347> CALIB STANDHYD ID=[ 3 ], NHYD=["030"], DT=[2.5] (min), AREA=[1.4] (ha),
01348> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01349> SCS curve number CN=[81],
01350> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01351> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01352> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
01353> LGI=[ 225.63 ] (m), MNI=[0.03], SCI=[0.0]
01354> RAINFALL=[ , , , ] (mm/hr), END=-1
01355> *%-----

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01351> SCS curve number CN=[81],
01352> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.0] (%),
01353> LGP=[5] (m), MNP=[0.03], SCP=[0.0] (min),
01354> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.51] (%),
01355> LGI=[225.63] (m), MNI=[0.03], SCI=[0.0]
01356> RAINFALL=[ , , , ] (mm/hr), END=-1
01357> *%-----
01358> ADD HYD IDsum=[4], NHYD=["040"], IDs to add=[1+2]
01359> *%-----
01360> ADD HYD IDsum=[5], NHYD=["050"], IDs to add=[3+4]
01361> *%-----
01362> *
01363> * SUB-AREA No.4
01364>
01365> CALIB STANDHYD ID=[6], NHYD=["060"], DT=[2.5] (min), AREA=[0.89] (ha),
01366> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01367> SCS curve number CN=[81],
01368> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[0.7] (%),
01369> LGP=[40] (m), MNP=[0.25], SCP=[0.0] (min)
01370> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.93] (%),
01371> LGI=[164.82] (m), MNI=[0.03], SCI=[0.0]
01372> RAINFALL=[ , , , ] (mm/hr), END=-1
01373> *%-----
01374> *
01375> * SUB-AREA No.5
01376>
01377> CALIB STANDHYD ID=[7], NHYD=["070"], DT=[2.5] (min), AREA=[2.66] (ha),
01378> XIMP=[0.97], TIMP=[0.97], DWF=[0.0] (cms), LOSS=[2],
01379> SCS curve number CN=[81],
01380> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01381> LGP=[20.0] (m), MNP=[0.25], SCP=[0.0] (min)
01382> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.61] (%),
01383> LGI=[207.25] (m), MNI=[0.03], SCI=[0.0]
01384> RAINFALL=[ , , , ] (mm/hr), END=-1
01385> *%-----
01386> ADD HYD IDsum=[8], NHYD=["080"], IDs to add=[6+7]
01387> *%-----
01388> ADD HYD IDsum=[9], NHYD=["090"], IDs to add=[5+8]
01389> *%-----
01390>
01391> ROUTE RESERVOIR IDout=[10], NHYD=["POND"], IDin=[9],
01392> RDT=[1.0] (min),
01393> TABLE of ( OUTFLOW-STORAGE ) values
01394> (cms) - (ha-m)
01395> [ 0.000, 0.0000]
01396> [ 0.008, 0.0656]
01397> [ 0.017, 0.1311]
01398> [ 0.093, 0.2831]
01399> [ 0.233, 0.3971]
01400> [ 0.337, 0.4731]
01401> [ 0.465, 0.5491]
01402> [ 0.531, 0.5871]
01403> [ 0.593, 0.6251]
01404> [ 0.654, 0.6631]
01405> [ 0.797, 0.7391]
01406> [ 0.950, 0.8274]
01407> [ 1.304, 0.9157]
01408> [ 1.880, 1.0040]
01409> [ 2.577, 1.0923]
01410> [ -1, -1 ] (max twenty pts)
01411>
01412> *****
01413> * Remaining Hawthorne Industrial Park *
01414> *****
01415> *
01416> * SUB-AREA No.1
01417>
01418> CALIB STANDHYD ID=[1], NHYD=["HIP01"], DT=[2.5] (min), AREA=[19.9] (ha),
01419> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01420> SCS curve number CN=[81],
01421> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01422> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01423> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.6] (%),
01424> LGI=[580] (m), MNI=[0.03], SCI=[0.0]
01425> RAINFALL=[ , , , ] (mm/hr), END=-1
01426> *%-----
01427> ADD HYD IDsum=[2], NHYD=["HIP02"], IDs to add=[10+1]
01428> *%-----
01429> *
01430> * SUB-AREA No.2
01431>
01432> CALIB STANDHYD ID=[3], NHYD=["HIP03"], DT=[2.5] (min), AREA=[17] (ha),
01433> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01434> SCS curve number CN=[81],
01435> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01436> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01437> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.65] (%),
01438> LGI=[450] (m), MNI=[0.03], SCI=[0.0]
01439> RAINFALL=[ , , , ] (mm/hr), END=-1
01440> *%-----
01441> *
01442> * SUB-AREA No.3
01443>
01444> CALIB STANDHYD ID=[4], NHYD=["HIP04"], DT=[2.5] (min), AREA=[15.6] (ha),
01445> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01446> SCS curve number CN=[81],
01447> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01448> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01449> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.5] (%),
01450> LGI=[600] (m), MNI=[0.03], SCI=[0.0]
01451> RAINFALL=[ , , , ] (mm/hr), END=-1
01452> *%-----
01453> ADD HYD IDsum=[5], NHYD=["HIP05"], IDs to add=[3+4]
01454> *%-----
01455> ADD HYD IDsum=[6], NHYD=["HIP06"], IDs to add=[5+2]
01456> *%-----
01457> *
01458> * SUB-AREA No.4
01459>
01460> CALIB STANDHYD ID=[7], NHYD=["HIP07"], DT=[2.5] (min), AREA=[12.2] (ha),
01461> XIMP=[0.50], TIMP=[0.71], DWF=[0.0] (cms), LOSS=[2],
01462> SCS curve number CN=[81],
01463> Pervious surfaces: IAPER=[4.67] (mm), SLPP=[1.5] (%),
01464> LGP=[100.0] (m), MNP=[0.25], SCP=[0.0] (min)
01465> Impervious surfaces: IAImp=[1.57] (mm), SLPI=[0.7] (%),
01466> LGI=[210] (m), MNI=[0.03], SCI=[0.0]
01467> RAINFALL=[ , , , ] (mm/hr), END=-1
01468> *%-----
01469> *
01470> *
01471> * SUB-AREA No.5
01472>
01473> DESIGN NASHYD ID=[8], NHYD=["Pond-Block"], DT=[2.5] min, AREA=[4.0] (ha),
01474> DWF=[0] (cms), CN/C=[85], TP=[0.17] hrs,
01475> RAINFALL=[ , , , ] (mm/hr), END=-1
01476> *%-----
01477> *
01478> ADD HYD IDsum=[9], NHYD=["HIP08"], IDs to add=[6+7+8]
01479> *%-----
01480>
01481> ROUTE RESERVOIR IDout=[10], NHYD=["HIP-POND"], IDin=[9],
01482> RDT=[1.0] (min),
01483> TABLE of ( OUTFLOW-STORAGE ) values
01484> (cms) - (ha-m)
01485> [ 0.0, 0.0 ]

```

```

01486> [ 0.048, 0.0574 ]
01487> [ 0.054, 0.2434 ]
01488> [ 0.059, 0.5834 ]
01489> [ 0.062, 0.8400 ]
01490> [ 0.064, 1.1024 ]
01491> [ 0.147, 1.3705 ]
01492> [ 0.280, 1.6444 ]
01493> [ 0.472, 1.9242 ]
01494> [ 0.724, 2.2097 ]
01495> [ 0.937, 2.5010 ]
01496> [ 1.262, 2.7981 ]
01497> [ 1.404, 3.1009 ]
01498> [ 1.532, 3.4096 ]
01499> [ 1.650, 3.7240 ]
01500> [ 2.409, 4.0442 ]
01501> [ 3.689, 4.3702 ]
01502> [ -1, -1 ] (max twenty pts)
01503>
01504> *%-----
01505> *
01506> * SUB-AREA No. 6
01507>
01508> DESIGN NASHYD ID=[1], NHYD=["A3"], DT=[2.5] min, AREA=[2.7] (ha),
01509> DWF=[0] (cms), CN/C=[76], TP=[0.80] hrs,
01510> RAINFALL=[ , , , ] (mm/hr), END=-1
01511> *%-----
01512> *
01513> ADD HYD IDsum=[2], NHYD=["Ultimate"], IDs to add=[10+1]
01514> *%-----
01515>
01516>
01517>
01518>
01519> FINISH

```

```

00001>
00002>
00003> SSSSS W W M M H H Y Y M M O O 999 999
00004> S W W W M M M H H H Y Y M M O O 9 9 9 9
00005> SSSSS W W M M M H H H Y Y M M O O 9 9 9 9 Ver. 4.02
00006> S W W M M M H H Y Y M M O O 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9 9 # 418403
00008>
00009> StormWater Management Hydrologic Model 999 999
00010>
00011> ***** SWHYMO-99 Ver/4.02 *****
00012> ***** A single event and continuous hydrologic simulation model *****
00013> ***** based on the principles of HYMO and its successors *****
00014> ***** OTTHMO-83 and OTTHMO-89 *****
00015> *****
00016> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00017> ***** Ottawa, Ontario: (613) 727-5199 *****
00018> ***** Gatineau, Quebec: (819) 243-6858 *****
00019> ***** E-Mail: swmhyo@jfsa.com *****
00020>
00021>
00022>
00023> ++++++ Licensed user: J. L. Richards & Associates Limited ++++++
00024> ++++++ Ottawa SERIAL#418403 ++++++
00025>
00026>
00027>
00028> *****
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033> *****
00034>
00035> ***** DETAILED OUTPUT *****
00036>
00037>
00038> * DATE: 2009-05-15 TIME: 08:45:21 RUN COUNTER: 000198 *
00039>
00040> * Input filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\SWM-ALL.dat *
00041> * Output filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\SWM-ALL.out *
00042> * Summary filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\SWM-ALL.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047>
00048>
00049>
00050> 001:0001
00051> * Project Name : Hawthorne Industrial Park Project Number: [20983] *
00052> * Date : January, 2009 *
00053> * Revised : N/A *
00054> * Developed by : Mark Buchanan, E.I.T. *
00055> * Reviewed by : Guy Forget, P.Eng. *
00056> * Company : J.L. Richards & Associates Limited *
00057> * License # : 4418403 *
00058>
00059>
00060>
00061>
00062>
00063> * FILENAME: V:\20983.DU\ENG\SWHYMO\20983PST.DAT *
00064> * FILE DEVELOPED FOR SITE PLAN APPLICATION AND DETAILED DESIGN *
00065> * OF A FACILITY ASSOCIATED WITH THE OTTAWA COMPOSTING SITE *
00066>
00067>
00068>
00069> * SWHYMO FILE DEVELOPED TO INVESTIGATE FLOOD FLOWS OF THE *
00070> * PROPOSED COMPOSTING SITE UNDER POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00071>
00072>
00073> * HYDROLOGICAL ANALYSIS UNDER A 4 HR-25 MM STORM AND *
00074> * FOR DESIGN STORMS OF 1:2, 5, 10, 25, 50, AND 100 YR *
00075>
00076>
00077> * POST-DEVELOPMENT UNCONTROLLED CONDITIONS *
00078>
00079>
00080> * CALCULATION OF 4 HR 25 MM STORM EVENT *
00081>
00082>
00083> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\
00084> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\
00085> | METOUT = 2 (output = METRIC)
00086> | NRUN = 001
00087> | NSTORM = 0
00088>
00089>
00090> 001:0002
00091>
00092> | READ STORM | Filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\4HR25
00093> | Ptotal= 25.00 mm | Comments: 4hr-15 min 25 MM STORM EVENT (CHICAGO DI
00094>
00095>
00096>
00097>
00098>
00099>
00100>
00101>
00102>
00103> 001:0003
00104>
00105> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYMO-1\ORGA.VAL
00106> | FileTitle= ICASEdv = 1 (read and print data)
00107> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE
00108> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60
00109> | Horton's infiltration equation parameters:
00110> | [P= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
00111> | Parameters for PERVIOUS surfaces in STANDHYD:
00112> | [Iaimp= 4.67 mm] [LGP=40.00 m] [MNI= .250]
00113> | Parameters for IMPVIOUS surfaces in STANDHYD:
00114> | [Iaimp= 1.57 mm] [CLI= 1.50] [MNI= .035]
00115> | Parameters used in NASHYD:
00116> | [Ia= 4.67 mm] [N= 3.00]
00117>
00118> 001:0004
00119>
00120> * ORGAWORLD FILE *
00121>
00122>
00123> * SUB-AREA No.1
00124>
00125> | CALIB STANDHYD | Area (ha)= 2.07
00126> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
00127>
00128>
00129>
00130>
00131>
00132>
00133>
00134>
00135>

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00136> over (min) 10.00 30.00
00137> Storage Coeff. (min)= 10.00 (ii) 29.27 (ii)
00138> Unit Hyd. Tpeak (min)= 10.00 30.00
00139> Unit Hyd. peak (cms)= .11 .04
00140>
00141>
00142>
00143>
00144>
00145>
00146>
00147>
00148> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00149> CN* = 81.0 Ia = Dep. Storage (Above)
00150> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00151> THAN THE STORAGE COEFFICIENT.
00152> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00153>
00154> 001:0005
00155>
00156> * SUB-AREA No.2
00157>
00158> | CALIB STANDHYD | Area (ha)= 1.54
00159> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
00160>
00161>
00162>
00163>
00164>
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00263>
00264>
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00267>
00268>
00269>
00270>

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00271> CN* = 81.0 Ia = Dep. Storage (Above)
 00272> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 00273> THAN THE STORAGE COEFFICIENT.
 00274> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 00275>
 00276>
 00277> 001:0010-----
 00278> * SUB-AREA No.5
 00279>
 00280> CALIB STANDHYD | Area (ha)= 2.66
 00281> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
 00282>
 00283> IMPERVIOUS PERVIOUS (i)
 00284> Surface Area (ha)= 2.58 .08
 00285> Dep. Storage (mm)= 1.57 4.67
 00286> Average Slope (%)= .61 1.50
 00287> Length (m)= 207.25 20.00
 00288> Mannings n = .030 .250
 00289>
 00290> Max.eff.Inten.(mm/hr)= 45.63 5.66
 00291> over (min) 10.00 27.50
 00292> Storage Coeff. (min)= 10.37 (ii) 26.38 (ii)
 00293> Unit Hyd. Tpeak (min)= 10.00 27.50
 00294> Unit Hyd. peak (cms)= .11 .04
 00295>
 00296> *TOTALS*
 00297> PEAK FLOW (cms)= .24 .00
 00298> TIME TO PEAK (hrs)= 1.29 1.67
 00299> RUNOFF VOLUME (mm)= 23.43 5.17
 00300> TOTAL RAINFALL (mm)= 25.00 25.00
 00301> RUNOFF COEFFICIENT = .94 .21
 00302>
 00303> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 00304> CN* = 81.0 Ia = Dep. Storage (Above)
 00305> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 00306> THAN THE STORAGE COEFFICIENT.
 00307> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 00308>
 00309>
 00310> 001:0011-----
 00311> ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 00312> | (ha) (cms) (hrs) (mm) (cms)
 00313> ID1 06:060 .89 .089 1.25 22.88 .000
 00314> +ID2 07:070 2.66 .238 1.29 22.88 .000
 00315>
 00316> SUM 08:080 3.55 .327 1.29 22.88 .000
 00317>
 00318> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 00319>
 00320>
 00321>
 00322> 001:0012-----
 00323> ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 00324> | (ha) (cms) (hrs) (mm) (cms)
 00325> ID1 05:050 5.01 .396 1.33 21.62 .000
 00326> +ID2 08:080 3.55 .327 1.29 22.88 .000
 00327>
 00328> SUM 09:090 8.56 .716 1.29 22.14 .000
 00329>
 00330> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 00331>
 00332>
 00333>
 00334> 001:0013-----
 00335> ROUTE RESERVOIR | Requested routing time step = 1.0 min.
 00336> | IN-09: (090)
 00337> | OUT-10: (POND)
 00338>
 00339> OUTFLOW STORAGE TABLE
 00340> (cms) (ha.m.) (cms) (ha.m.)
 00341> .000 0000E+00 | .593 .625E+00
 00342> .008 .650E+01 | .654 .663E+00
 00343> .017 .131E+00 | .797 .739E+00
 00344> .093 .283E+00 | .950 .827E+00
 00345> .233 .397E+00 | 1.304 .915E+00
 00346> .337 .473E+00 | 1.880 .100E+01
 00347> .465 .549E+00 | 2.577 .109E+01
 00348> .531 .587E+00 | .000 .000E+00
 00349>
 00350> ROUTING RESULTS AREA QPEAK TPEAK R.V.
 00351> (ha) (cms) (hrs) (mm)
 00352> INFLOW>09: (090) 8.56 .716 1.29 22.14
 00353> OUTFLOW<10: (POND) 8.56 .032 3.875 22.14
 00354>
 00355> PEAK FLOW REDUCTION [Qout/qin] (%) = 4.470
 00356> TIME SHIFT OF PEAK FLOW (min) = 155.00
 00357> MAXIMUM STORAGE USED (ha.m.) = .161E+00
 00358>
 00359>
 00360> 001:0014-----
 00361> *****
 00362> * Remaining Hawthorne Industrial Park *
 00363> *****
 00364> *
 00365> * SUB-AREA No.1
 00366>
 00367> CALIB STANDHYD | Area (ha)= 19.90
 00368> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 00369>
 00370> IMPERVIOUS PERVIOUS (i)
 00371> Surface Area (ha)= 14.13 5.77
 00372> Dep. Storage (mm)= 1.57 4.67
 00373> Average Slope (%)= .60 1.50
 00374> Length (m)= 580.00 100.00
 00375> Mannings n = .030 .250
 00376>
 00377> Max.eff.Inten.(mm/hr)= 34.39 11.90
 00378> over (min) 22.50 52.50
 00379> Storage Coeff. (min)= 21.64 (ii) 52.88 (ii)
 00380> Unit Hyd. Tpeak (min)= 22.50 52.50
 00381> Unit Hyd. peak (cms)= .05 .02
 00382>
 00383> *TOTALS*
 00384> PEAK FLOW (cms)= .60 .11
 00385> TIME TO PEAK (hrs)= 1.50 2.13
 00386> RUNOFF VOLUME (mm)= 23.43 8.74
 00387> TOTAL RAINFALL (mm)= 25.00 25.00
 00388> RUNOFF COEFFICIENT = .94 .35
 00389>
 00390> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 00391> CN* = 81.0 Ia = Dep. Storage (Above)
 00392> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 00393> THAN THE STORAGE COEFFICIENT.
 00394> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 00395>
 00396>
 00397> 001:0015-----
 00398> ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 00399> | (ha) (cms) (hrs) (mm) (cms)
 00400> ID1 10:POND 8.56 .032 3.88 22.14 .000
 00401> +ID2 01:HIP01 19.90 .642 1.54 16.08 .000
 00402>
 00403> SUM 02:HIP02 28.46 .655 1.54 17.91 .000
 00404>
 00405> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00406>
 00407>
 00408> 001:0016-----
 00409> * SUB-AREA No.2
 00410>
 00411> CALIB STANDHYD | Area (ha)= 17.00
 00412> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 00413>
 00414> IMPERVIOUS PERVIOUS (i)
 00415> Surface Area (ha)= 12.07 4.93
 00416> Dep. Storage (mm)= 1.57 4.67
 00417> Average Slope (%)= .65 1.50
 00418> Length (m)= 450.00 100.00
 00419> Mannings n = .030 .250
 00420>
 00421> Max.eff.Inten.(mm/hr)= 40.81 12.73
 00422> over (min) 17.50 47.50
 00423> Storage Coeff. (min)= 16.94 (ii) 47.35 (ii)
 00424> Unit Hyd. Tpeak (min)= 17.50 47.50
 00425> Unit Hyd. peak (cms)= .07 .02
 00426>
 00427> *TOTALS*
 00428> PEAK FLOW (cms)= .60 .10
 00429> TIME TO PEAK (hrs)= 1.42 2.00
 00430> RUNOFF VOLUME (mm)= 23.43 8.74
 00431> TOTAL RAINFALL (mm)= 25.00 25.00
 00432> RUNOFF COEFFICIENT = .94 .35
 00433>
 00434> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 00435> CN* = 81.0 Ia = Dep. Storage (Above)
 00436> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 00437> THAN THE STORAGE COEFFICIENT.
 00438> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 00439>
 00440>
 00441> 001:0017-----
 00442> * SUB-AREA No.3
 00443>
 00444> CALIB STANDHYD | Area (ha)= 15.60
 00445> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 00446>
 00447> IMPERVIOUS PERVIOUS (i)
 00448> Surface Area (ha)= 11.08 4.52
 00449> Dep. Storage (mm)= 1.57 4.67
 00450> Average Slope (%)= .50 1.50
 00451> Length (m)= 600.00 100.00
 00452> Mannings n = .030 .250
 00453>
 00454> Max.eff.Inten.(mm/hr)= 34.39 11.54
 00455> over (min) 22.50 55.00
 00456> Storage Coeff. (min)= 23.33 (ii) 54.95 (ii)
 00457> Unit Hyd. Tpeak (min)= 22.50 55.00
 00458> Unit Hyd. peak (cms)= .05 .02
 00459>
 00460> *TOTALS*
 00461> PEAK FLOW (cms)= .45 .08
 00462> TIME TO PEAK (hrs)= 1.50 2.17
 00463> RUNOFF VOLUME (mm)= 23.43 8.74
 00464> TOTAL RAINFALL (mm)= 25.00 25.00
 00465> RUNOFF COEFFICIENT = .94 .35
 00466>
 00467> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 00468> CN* = 81.0 Ia = Dep. Storage (Above)
 00469> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 00470> THAN THE STORAGE COEFFICIENT.
 00471> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 00472>
 00473>
 00474> 001:0018-----
 00475> ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 00476> | (ha) (cms) (hrs) (mm) (cms)
 00477> ID1 03:HIP03 17.00 .625 1.46 16.08 .000
 00478> +ID2 04:HIP04 15.60 .484 1.54 16.08 .000
 00479>
 00480> SUM 05:HIP05 32.60 1.091 1.46 16.08 .000
 00481>
 00482> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 00483>
 00484>
 00485>
 00486> 001:0019-----
 00487> ADD HYD (HIP06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 00488> | (ha) (cms) (hrs) (mm) (cms)
 00489> ID1 05:HIP05 32.60 1.091 1.46 16.08 .000
 00490> +ID2 02:HIP02 28.46 .655 1.54 17.91 .000
 00491>
 00492> SUM 06:HIP06 61.06 1.740 1.50 16.93 .000
 00493>
 00494> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 00495>
 00496>
 00497>
 00498> 001:0020-----
 00499> * SUB-AREA No.4
 00500>
 00501> CALIB STANDHYD | Area (ha)= 12.20
 00502> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 00503>
 00504> IMPERVIOUS PERVIOUS (i)
 00505> Surface Area (ha)= 8.66 3.54
 00506> Dep. Storage (mm)= 1.57 4.67
 00507> Average Slope (%)= .70 1.50
 00508> Length (m)= 210.00 100.00
 00509> Mannings n = .030 .250
 00510>
 00511> Max.eff.Inten.(mm/hr)= 45.63 14.15
 00512> over (min) 10.00 40.00
 00513> Storage Coeff. (min)= 10.03 (ii) 39.18 (ii)
 00514> Unit Hyd. Tpeak (min)= 10.00 40.00
 00515> Unit Hyd. peak (cms)= .11 .03
 00516>
 00517> *TOTALS*
 00518> PEAK FLOW (cms)= .57 .08
 00519> TIME TO PEAK (hrs)= 1.29 1.88
 00520> RUNOFF VOLUME (mm)= 23.43 8.74
 00521> TOTAL RAINFALL (mm)= 25.00 25.00
 00522> RUNOFF COEFFICIENT = .94 .35
 00523>
 00524> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 00525> CN* = 81.0 Ia = Dep. Storage (Above)
 00526> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 00527> THAN THE STORAGE COEFFICIENT.
 00528> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 00529>
 00530>
 00531> 001:0021-----
 00532> * SUB-AREA No.5
 00533>
 00534> DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
 00535> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
 00536> U.H. Tp(hrs)= .170
 00537>
 00538> Unit Hyd Qpeak (cms)= .899
 00539>
 00540>

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00541> PEAK FLOW (cms) = .077 (i)
00542> TIME TO PEAK (hrs) = 1.375
00543> RUNOFF VOLUME (mm) = 6.343
00544> TOTAL RAINFALL (mm) = 24.999
00545> RUNOFF COEFFICIENT = .254
00546>
00547> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00548>
00549>
00550> 001:0022-----
00551>
00552> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00553> | (ha) (cms) (hrs) (mm) (cms)
00554> ID1 06:HIP06 61.06 1.740 1.50 16.93 .000
00555> ID2 07:HIP07 12.20 .585 1.29 16.08 .000
00556> ID3 08:POND-B 4.00 .077 1.38 6.34 .000
00557>
00558> SUM 09:HIP08 77.26 2.227 1.46 16.25 .000
00559>
00560> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00561>
00562>
00563> 001:0023-----
00564>
00565> | ROUTE RESERVOIR Requested routing time step = 1.0 min.
00566> | IN:09: (HIP08) |
00567> | OUT:10: (HIP-PO) |
00568>
00569> OUTFLOW STORAGE TABLE
00570> (cms) (ha.m.) (cms) (ha.m.)
00571> .000 .0000E+00 .724 .2210E+01
00572> .048 .5740E-01 .937 .2501E+01
00573> .054 .2434E+00 1.262 .2798E+01
00574> .059 .5834E+00 1.404 .3101E+01
00575> .062 .8400E+00 1.532 .3410E+01
00576> .064 .1102E+01 1.650 .3724E+01
00577> .147 .1370E+01 2.409 .4044E+01
00578> .280 .1644E+01 3.689 .4370E+01
00579> .472 .1924E+01 .000 .0000E+00
00580>
00581> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00582> (ha) (cms) (hrs) (mm)
00583> INFLOW>09: (HIP08) 77.26 2.227 1.458 16.251
00584> OUTFLOW>10: (HIP-PO) 77.26 .063 5.431 16.251
00585>
00586> PEAK FLOW REDUCTION (Qout/Qin) (%) = 2.839
00587> TIME SHIFT OF PEAK FLOW (min) = 238.33
00588> MAXIMUM STORAGE USED (ha.m.) = .1001E+01
00589>
00590> 001:0024-----
00591> *
00592> *SUB-AREA No. 6
00593>
00594> | DESIGN NASHYD | Area (ha) = 2.70 Curve Number (CN)=76.00
00595> | 01:A3 DT= 2.50 | Ia (mm) = 4.670 # of Linear Res. (N)= 3.00
00596> | U.H. Tp (hrs) = .800
00597>
00598> Unit Hyd Qpeak (cms) = .129
00599>
00600> PEAK FLOW (cms) = .013 (i)
00601> TIME TO PEAK (hrs) = 2.292
00602> RUNOFF VOLUME (mm) = 4.110
00603> TOTAL RAINFALL (mm) = 24.999
00604> RUNOFF COEFFICIENT = .164
00605>
00606> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00607>
00608>
00609> 001:0025-----
00610>
00611> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00612> | (ha) (cms) (hrs) (mm) (cms)
00613> ID1 10:HIP-PO 77.26 .063 5.43 16.25 .000
00614> ID2 01:A3 2.70 .013 2.29 4.11 .000
00615>
00616> SUM 02:Ultima 79.96 .073 2.50 15.84 .000
00617>
00618> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00619>
00620>
00621> 001:0026-----
00622> *****
00623> * CALCULATION OF 3HR - 1:2 YEAR STORM EVENT *
00624> *****
00625>
00626> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
00627> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
00628> | METOUT= 2 (output = METRIC)
00629> | NRUN = 001
00630> | NSTORM= 0
00631>
00632> 001:0002-----
00633>
00634> | CHICAGO STORM IDF curve parameters: A= 732.951
00635> | Protal= 31.86 mm B= 6.199
00636> C= .810
00637> used in: INTENSITY = A / (t + B)^C
00638>
00639> Duration of storm = 3.00 hrs
00640> Storm time step = 10.00 min
00641> Time to peak ratio = .33
00642>
00643>
00644> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00645> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00646> .17 2.815 | 1.00 76.805 | 1.83 5.095 | 2.67 2.684
00647> .33 3.498 | 1.17 24.079 | 2.00 4.291 | 2.83 2.463
00648> .50 4.687 | 1.33 12.364 | 2.17 3.718 | 3.00 2.279
00649> .67 7.305 | 1.50 8.324 | 2.33 3.288 |
00650> .83 16.209 | 1.67 6.303 | 2.50 2.953 |
00651>
00652> 001:0003-----
00653>
00654> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
00655> | ICSADrv = 1 (read and print data)
00656> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE
00657> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ---
00658>
00659> Horton's infiltration equation parameters:
00660> [F= 50.00 mm/hr] [P= 7.50 mm/hr] [DCAX= 2.00 /hr] [F= .00 mm]
00661> Parameters for PERVIOUS surfaces in STANDHYD:
00662> [Iaper= 4.67 mm] [LGP= 40.00 mm] [MNI= .250]
00663> Parameters for IMPERVIOUS surfaces in STANDHYD:
00664> [Iaimp= 1.57 mm] [CLI= 1.50] [MNI= .035]
00665> Parameters used in NASHYD:
00666> [Ia= 4.67 mm] [N= 3.00]
00667>
00668> 001:0004-----
00669> *****
00670> | ORGAWORLD FILE *
00671> *****
00672> *
00673> * SUB-AREA No.1
00674> | CALIB STANDHYD | Area (ha) = 2.07
00675>
00676> 01:010 DT= 2.50 | Total Imp(%) = 84.00 Dir. Conn.(%) = 84.00
00677>
00678> IMPERVIOUS PERVIOUS (i)
00679> Surface Area (ha) = 1.74 .33
00680> Dep. Storage (mm) = 1.57 4.67
00681> Average Slope (%) = .52 1.00
00682> Length (m) = 204.72 20.00
00683> Mannings n = .030 .250
00684>
00685> Max. eff. Inten. (mm/hr) = 76.81 11.88
00686> over (min) = 10.00 22.50
00687> Storage Coeff. (min) = 8.77 (ii) 22.21 (ii)
00688> Unit Hyd. Tpeak (min) = 10.00 22.50
00689> Unit Hyd. peak (cms) = .12 .05
00690>
00691> PEAK FLOW (cms) = .24 .01
00692> TIME TO PEAK (hrs) = 1.08 1.38
00693> RUNOFF VOLUME (mm) = 30.29 8.52
00694> TOTAL RAINFALL (mm) = 31.86 31.86
00695> RUNOFF COEFFICIENT = .95 .27
00696>
00697> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00698> CN* = 81.0 Ia = Dep. Storage (Above)
00699> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00700> THAN THE STORAGE COEFFICIENT.
00701> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00702>
00703>
00704> 001:0005-----
00705> * SUB-AREA No.2
00706>
00707> | CALIB STANDHYD | Area (ha) = 1.54
00708> | 02:020 DT= 2.50 | Total Imp(%) = 92.00 Dir. Conn.(%) = 92.00
00709>
00710> IMPERVIOUS PERVIOUS (i)
00711> Surface Area (ha) = 1.42 .12
00712> Dep. Storage (mm) = 1.57 4.67
00713> Average Slope (%) = .50 1.00
00714> Length (m) = 244.34 5.00
00715> Mannings n = .030 .030
00716>
00717> Max. eff. Inten. (mm/hr) = 76.81 15.07
00718> over (min) = 10.00 12.50
00719> Storage Coeff. (min) = 9.87 (ii) 11.36 (ii)
00720> Unit Hyd. Tpeak (min) = 10.00 12.50
00721> Unit Hyd. peak (cms) = .11 .10
00722>
00723> PEAK FLOW (cms) = .19 .00
00724> TIME TO PEAK (hrs) = 1.08 1.17
00725> RUNOFF VOLUME (mm) = 30.29 8.52
00726> TOTAL RAINFALL (mm) = 31.86 31.86
00727> RUNOFF COEFFICIENT = .95 .27
00728>
00729> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00730> CN* = 81.0 Ia = Dep. Storage (Above)
00731> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00732> THAN THE STORAGE COEFFICIENT.
00733> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00734>
00735>
00736> 001:0006-----
00737> * SUB-AREA No.3
00738>
00739> | CALIB STANDHYD | Area (ha) = 1.40
00740> | 03:030 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00
00741>
00742> IMPERVIOUS PERVIOUS (i)
00743> Surface Area (ha) = 1.36 .04
00744> Dep. Storage (mm) = 1.57 4.67
00745> Average Slope (%) = .51 1.00
00746> Length (m) = 225.63 5.00
00747> Mannings n = .030 .030
00748>
00749> Max. eff. Inten. (mm/hr) = 76.81 16.59
00750> over (min) = 10.00 10.00
00751> Storage Coeff. (min) = 9.35 (ii) 10.79 (ii)
00752> Unit Hyd. Tpeak (min) = 10.00 10.00
00753> Unit Hyd. peak (cms) = .12 .11
00754>
00755> PEAK FLOW (cms) = .18 .00
00756> TIME TO PEAK (hrs) = 1.08 1.13
00757> RUNOFF VOLUME (mm) = 30.29 8.52
00758> TOTAL RAINFALL (mm) = 31.86 31.86
00759> RUNOFF COEFFICIENT = .95 .27
00760>
00761> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00762> CN* = 81.0 Ia = Dep. Storage (Above)
00763> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00764> THAN THE STORAGE COEFFICIENT.
00765> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00766>
00767>
00768> 001:0007-----
00769>
00770> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00771> | (ha) (cms) (hrs) (mm) (cms)
00772> ID1 01:010 2.07 .245 1.08 26.81 .000
00773> ID2 02:020 1.54 .192 1.08 28.55 .000
00774>
00775> SUM 04:040 3.61 .436 1.08 27.55 .000
00776>
00777>
00778> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00779>
00780>
00781> 001:0008-----
00782>
00783> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00784> | (ha) (cms) (hrs) (mm) (cms)
00785> ID1 03:030 1.40 .186 1.08 29.64 .000
00786> ID2 04:040 3.61 .436 1.08 27.55 .000
00787>
00788> SUM 05:050 5.01 .623 1.08 28.13 .000
00789>
00790>
00791> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00792>
00793>
00794> 001:0009-----
00795> * SUB-AREA No.4
00796>
00797> | CALIB STANDHYD | Area (ha) = .89
00798> | 06:060 DT= 2.50 | Total Imp(%) = 97.00 Dir. Conn.(%) = 97.00
00799>
00800> IMPERVIOUS PERVIOUS (i)
00801> Surface Area (ha) = .86 .03
00802> Dep. Storage (mm) = 1.57 4.67
00803> Average Slope (%) = .93 .70
00804> Length (m) = 164.82 40.00
00805> Mannings n = .030 .250
00806>
00807> Max. eff. Inten. (mm/hr) = 76.81 10.24
00808> over (min) = 7.50 30.00
00809> Storage Coeff. (min) = 6.47 (ii) 30.53 (ii)
00810>

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00811> Unit Hyd. Tpeak (min)= 7.50 30.00
00812> Unit Hyd. peak (cms)= .16 .04
00813>
00814> PEAK FLOW (cms)= .14 .00 *TOTALS*
00815> TIME TO PEAK (hrs)= 1.04 1.54 1.39 (iii)
00816> RUNOFF VOLUME (mm)= 30.29 8.52 29.637
00817> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00818> RUNOFF COEFFICIENT = .95 .27 .930
00819>
00820> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00821> CN* = 81.0 Ia = Dep. Storage (Above)
00822> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00823> THAN THE STORAGE COEFFICIENT.
00824> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00825>
00826>
00827> 001:0010-----
00828> * SUB-AREA No.5
00829>
00830>
00831> | CALIB STANDHYD | Area (ha)= 2.66
00832> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
00833>
00834> IMPERVIOUS PERVIOUS (i)
00835> Surface Area (ha)= 2.58 .08
00836> Dep. Storage (mm)= 1.57 4.67
00837> Average Slope (%)= .61 1.50
00838> Length (m)= 207.25 20.00
00839> Mannings n = .030 .250
00840>
00841> Max.eff.Inten.(mm/hr)= 76.81 12.71
00842> over (min) 7.50 20.00
00843> Storage Coeff. (min)= 8.42 (ii) 20.00 (ii)
00844> Unit Hyd. Tpeak (min)= 7.50 20.00
00845> Unit Hyd. peak (cms)= .14 .06
00846>
00847> PEAK FLOW (cms)= .38 .00 *TOTALS*
00848> TIME TO PEAK (hrs)= 1.04 1.33 1.042
00849> RUNOFF VOLUME (mm)= 31.29 8.52 29.637
00850> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00851> RUNOFF COEFFICIENT = .95 .27 .930
00852>
00853> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00854> CN* = 81.0 Ia = Dep. Storage (Above)
00855> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00856> THAN THE STORAGE COEFFICIENT.
00857> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00858>
00859>
00860> 001:0011-----
00861>
00862> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00863> | 01:06:060 | (ha) (cms) (hrs) (mm) (cms)
00864> | +ID2 07:070 | 2.66 .139 1.04 29.64 .000
00865> | SUM 08:080 | 3.55 .518 1.04 29.64 .000
00866>
00867> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00868>
00869>
00870>
00871>
00872> 001:0012-----
00873> | ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00874> | 01:05:050 | (ha) (cms) (hrs) (mm) (cms)
00875> | +ID2 08:080 | 3.55 .518 1.04 29.64 .000
00876> | SUM 09:090 | 8.56 1.118 1.08 28.76 .000
00877>
00878> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00879>
00880>
00881>
00882>
00883> 001:0013-----
00884>
00885>
00886> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00887> | IN>09: (090) |
00888> | OUT<10: (POND) |
00889>
00890> OUTFLOW STORAGE TABLE
00891> (cms) (ha-m.) (cms) (ha-m.)
00892> .000 .0000E+00 .593 .6251E+00
00893> .008 .6560E-01 .654 .6631E+00
00894> .017 .1311E+00 .797 .7391E+00
00895> .093 .2831E+00 .950 .8274E+00
00896> .233 .3971E+00 1.304 .9157E+00
00897> .337 .4731E+00 1.880 .1004E+01
00898> .465 .5491E+00 2.577 .1092E+01
00899> .531 .5871E+00 .000 .0000E+00
00900>
00901> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00902> (ha) (cms) (hrs) (mm)
00903> INFLOW >09: (090) 8.56 1.118 1.083 28.757
00904> OUTFLOW<10: (POND) 8.56 .056 3.000 28.754
00905>
00906> PEAK FLOW REDUCTION [Qout/Qin](%)= 5.030
00907> TIME SHIFT OF PEAK FLOW (min)= 115.00
00908> MAXIMUM STORAGE USED (ha-m.)= .2095E+00
00909>
00910> 001:0014-----
00911> *****
00912> * Remaining Hawthorne Industrial Park *
00913> *****
00914>
00915> * SUB-AREA No.1
00916>
00917> | CALIB STANDHYD | Area (ha)= 19.90
00918> | 01:H1P01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00919>
00920> IMPERVIOUS PERVIOUS (i)
00921> Surface Area (ha)= 14.13 5.77
00922> Dep. Storage (mm)= 1.57 4.67
00923> Average Slope (%)= .60 1.50
00924> Length (m)= 580.00 100.00
00925> Mannings n = .030 .250
00926>
00927> Max.eff.Inten.(mm/hr)= 54.21 23.06
00928> over (min) 17.50 42.50
00929> Storage Coeff. (min)= 18.04 (ii) 42.02 (ii)
00930> Unit Hyd. Tpeak (min)= 17.50 42.50
00931> Unit Hyd. peak (cms)= .06 .03
00932>
00933> PEAK FLOW (cms)= .95 .21 *TOTALS*
00934> TIME TO PEAK (hrs)= 1.21 1.71 1.250
00935> RUNOFF VOLUME (mm)= 30.29 13.34 21.814
00936> TOTAL RAINFALL (mm)= 31.86 31.86 31.860
00937> RUNOFF COEFFICIENT = .95 .42 .685
00938>
00939> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00940> CN* = 81.0 Ia = Dep. Storage (Above)
00941> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00942> THAN THE STORAGE COEFFICIENT.
00943> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00944>
00945>

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00946> 001:0015-----
00947>
00948> | ADD HYD (H1P02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00949> | ID1 10:POND | (ha) (cms) (hrs) (mm) (cms)
00950> | +ID2 01:H1P01 | 8.56 .056 3.00 28.75 .000
00951> | SUM 02:H1P02 | 19.90 1.020 1.25 21.81 .000
00952>
00953> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00954>
00955>
00956>
00957>
00958> 001:0016-----
00959>
00960> * SUB-AREA No.2
00961>
00962> | CALIB STANDHYD | Area (ha)= 17.00
00963> | 03:H1P03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00964>
00965> IMPERVIOUS PERVIOUS (i)
00966> Surface Area (ha)= 12.07 4.93
00967> Dep. Storage (mm)= 1.57 4.67
00968> Average Slope (%)= .65 1.50
00969> Length (m)= 450.00 100.00
00970> Mannings n = .030 .250
00971>
00972> Max.eff.Inten.(mm/hr)= 59.23 25.04
00973> over (min) 15.00 37.50
00974> Storage Coeff. (min)= 14.60 (ii) 37.80 (ii)
00975> Unit Hyd. Tpeak (min)= 15.00 37.50
00976> Unit Hyd. peak (cms)= .08 .03
00977>
00978> PEAK FLOW (cms)= .91 .19 *TOTALS*
00979> TIME TO PEAK (hrs)= 1.17 1.63 .978 (iii)
00980> RUNOFF VOLUME (mm)= 30.29 13.34 1.167
00981> TOTAL RAINFALL (mm)= 31.86 31.86 21.814
00982> RUNOFF COEFFICIENT = .95 .42 31.860
00983>
00984> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00985> CN* = 81.0 Ia = Dep. Storage (Above)
00986> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00987> THAN THE STORAGE COEFFICIENT.
00988> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00989>
00990>
00991> 001:0017-----
00992>
00993> * SUB-AREA No.3
00994>
00995> | CALIB STANDHYD | Area (ha)= 15.60
00996> | 04:H1P04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
00997>
00998> IMPERVIOUS PERVIOUS (i)
00999> Surface Area (ha)= 11.08 4.52
01000> Dep. Storage (mm)= 1.57 4.67
01001> Average Slope (%)= .50 1.50
01002> Length (m)= 600.00 100.00
01003> Mannings n = .030 .250
01004>
01005> Max.eff.Inten.(mm/hr)= 50.44 22.17
01006> over (min) 20.00 45.00
01007> Storage Coeff. (min)= 20.01 (ii) 44.37 (ii)
01008> Unit Hyd. Tpeak (min)= 20.00 45.00
01009> Unit Hyd. peak (cms)= .06 .03
01010>
01011> PEAK FLOW (cms)= .69 .16 *TOTALS*
01012> TIME TO PEAK (hrs)= 1.25 1.79 .753 (iii)
01013> RUNOFF VOLUME (mm)= 30.29 13.34 1.292
01014> TOTAL RAINFALL (mm)= 31.86 31.86 21.814
01015> RUNOFF COEFFICIENT = .95 .42 31.860
01016>
01017> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01018> CN* = 81.0 Ia = Dep. Storage (Above)
01019> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01020> THAN THE STORAGE COEFFICIENT.
01021> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01022>
01023>
01024> 001:0018-----
01025>
01026> | ADD HYD (H1P05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01027> | ID1 03:H1P03 | (ha) (cms) (hrs) (mm) (cms)
01028> | +ID2 04:H1P04 | 17.00 .978 1.17 21.81 .000
01029> | SUM 05:H1P05 | 15.60 .753 1.29 21.81 .000
01030>
01031> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01032>
01033>
01034>
01035>
01036> 001:0019-----
01037>
01038> | ADD HYD (H1P06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01039> | ID1 05:H1P05 | (ha) (cms) (hrs) (mm) (cms)
01040> | +ID2 02:H1P02 | 32.60 1.698 1.21 21.81 .000
01041> | SUM 06:H1P06 | 28.46 1.039 1.25 23.90 .000
01042>
01043> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01044>
01045>
01046>
01047>
01048> 001:0020-----
01049>
01050> * SUB-AREA No.4
01051>
01052> | CALIB STANDHYD | Area (ha)= 12.20
01053> | 07:H1P07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
01054>
01055> IMPERVIOUS PERVIOUS (i)
01056> Surface Area (ha)= 8.26 3.54
01057> Dep. Storage (mm)= 1.57 4.67
01058> Average Slope (%)= .70 1.50
01059> Length (m)= 210.00 100.00
01060> Mannings n = .030 .250
01061>
01062> Max.eff.Inten.(mm/hr)= 76.81 29.02
01063> over (min) 7.50 30.00
01064> Storage Coeff. (min)= 8.15 (ii) 30.01 (ii)
01065> Unit Hyd. Tpeak (min)= 7.50 30.00
01066> Unit Hyd. peak (cms)= .14 .04
01067>
01068> PEAK FLOW (cms)= .91 .16 *TOTALS*
01069> TIME TO PEAK (hrs)= 1.04 1.50 .941 (iii)
01070> RUNOFF VOLUME (mm)= 30.29 13.34 1.042
01071> TOTAL RAINFALL (mm)= 31.86 31.86 21.814
01072> RUNOFF COEFFICIENT = .95 .42 31.860
01073>
01074> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01075> CN* = 81.0 Ia = Dep. Storage (Above)
01076> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01077> THAN THE STORAGE COEFFICIENT.
01078> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01079>
01080>

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01081> 001:0021-----
01082> *
01083> *SUB-AREA No.5
01084> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
01085> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01086> | U.H. Tp(hrs)= .170
01087>
01088> Unit Hyd Qpeak (cms)= .899
01089>
01090>
01091> PEAK FLOW (cms)= .145 (i)
01092> TIME TO PEAK (hrs)= 1.167
01093> RUNOFF VOLUME (mm)= 10.266
01094> TOTAL RAINFALL (mm)= 31.860
01095> RUNOFF COEFFICIENT = .322
01096>
01097> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01098>
01099>
01100> 001:0022-----
01101>
01102> | ADD HYD (H1P08 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01103> | (ha) (cms) (hrs) (mm) (cms)
01104> ID1 06:H1P06 61.06 2.733 1.21 22.79 .000
01105> +ID2 07:H1P07 12.20 .941 1.04 21.81 .000
01106> +ID3 08:Pond-B 4.00 .145 1.17 10.27 .000
01107>
01108> SUM 09:H1P08 77.26 3.542 1.21 21.98 .000
01109>
01110> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01111>
01112>
01113> 001:0023-----
01114>
01115> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01116> | IN:09: (H1P08 ) |
01117> | OUT:10: (H1P-PO) |
01118>
01119> ===== OUTFLOW STORAGE TABLE =====
01120> OUTFLOW STORAGE OUTFLOW STORAGE
01121> (cms) (ha.m.) (cms) (ha.m.)
01122> .000 .000E+00 .724 .2210E+01
01123> .049 .5740E+01 .937 .2501E+01
01124> .054 .2434E+00 1.262 .2798E+01
01125> .059 .5834E+00 1.404 .3101E+01
01126> .062 .8400E+00 1.532 .3410E+01
01127> .064 .1102E+01 1.650 .3724E+01
01128> .147 .1370E+01 2.409 .4044E+01
01129> .280 .1644E+01 3.689 .4370E+01
01130> .472 .1924E+01 .000 .0000E+00
01131>
01132> ROUTING RESULTS AREA OPEAK TPEAK R.V.
01133> (ha) (cms) (hrs) (mm)
01134> INFLOW >09: (H1P08 ) 77.26 3.542 1.208 21.985
01135> OUTFLOW <10: (H1P-PO) 77.26 .148 4.014 21.985
01136>
01137> PEAK FLOW REDUCTION [Qout/Qin] (%) = 4.179
01138> TIME SHIFT OF PEAK FLOW (min) = 168.33
01139> MAXIMUM STORAGE USED (ha.m.) = 1373E+01
01140>
01141> 001:0024-----
01142> *
01143> *SUB-AREA No. 6
01144> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
01145> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01146> | U.H. Tp(hrs)= .800
01147>
01148> Unit Hyd Qpeak (cms)= .129
01149>
01150>
01151> PEAK FLOW (cms)= .024 (i)
01152> TIME TO PEAK (hrs)= 2.083
01153> RUNOFF VOLUME (mm)= 6.883
01154> TOTAL RAINFALL (mm)= 31.860
01155> RUNOFF COEFFICIENT = .216
01156>
01157> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01158>
01159>
01160> 001:0025-----
01161> | ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01162> | (ha) (cms) (hrs) (mm) (cms)
01163> ID1 10:H1P-PO 77.26 .148 4.01 21.98 .000
01164> +ID2 01:A3 2.70 .024 2.08 6.88 .000
01165>
01166> SUM 02:Ultima 79.96 .156 3.65 21.47 .000
01167>
01168> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01169>
01170>
01171> 001:0026-----
01172> *****
01173> * CALCULATION OF 3HR - 1.5 YEAR STORM EVENT *
01174> *****
01175>
01176> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01177> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01178> | METOUT= 2 (output = METRIC)
01179> | NRUN = 001
01180> | NSTORM= 0
01181>
01182> 001:0002-----
01183>
01184> | CHICAGO STORM | IDF curve parameters: A= 998.071
01185> | Ptotal= 42.51 mm | B= 6.053
01186> | C= .814
01187>
01188> used in: INTENSITY = A / (t + B)^C
01189>
01190> Duration of storm = 3.00 hrs
01191> Storm time step = 10.00 min
01192> Time to peak ratio = .33
01193>
01194> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01195> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01196> .17 3.682 | 1.00 104.193 | 1.83 6.689 | 2.67 3.510
01197> .33 4.582 | 1.17 32.037 | 2.00 5.628 | 2.83 3.220
01198> .50 6.251 | 1.33 16.337 | 2.17 4.872 | 3.00 2.978
01199> .67 9.614 | 1.50 10.965 | 2.33 4.305 |
01200> .83 24.170 | 1.67 8.287 | 2.50 3.864 |
01201>
01202> 001:0003-----
01203>
01204> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
01205> | ICASEdv (read and print data)
01206> | ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE
01207> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----
01208>
01209> Horton's infiltration equation parameters:
01210> [F= 50.00 mm/hr] [P= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01211> Parameters for PERVIOUS surfaces in STANDHYD:
01212> [Ia= 4.67 mm] [LCP= 40.00 mm] [MNI= .250]
01213> Parameters for IMPVIOUS surfaces in STANDHYD:
01214> [Ia= 1.57 mm] [CLI= 1.50] [MNI= .035]
01215> Parameters used in NASHYD:

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01216> [Ia= 4.67 mm] [N= 3.00]
01217>
01218> 001:0004-----
01219> *****
01220> * ORGAWORD FILE *
01221> *****
01222> *
01223> * SUB-AREA No.1
01224>
01225> | CALIB STANDHYD | Area (ha)= 2.07
01226> | 01:010 DT= 2.50 | Total Imp (%) = 84.00 Dir. Conn. (%) = 84.00
01227>
01228> IMPVIOUS PERVIOUS (i)
01229> Surface Area (ha)= 1.74 .33
01230> Dep. Storage (mm)= 1.57 4.67
01231> Average Slope (%) = .52 1.00
01232> Length (m) = 204.72 20.00
01233> Mannings n = .030 .250
01234>
01235> Max.eff.Inten.(mm/hr)= 104.19 24.26
01236> over (min) 7.50 17.50
01237> Storage Coeff. (min)= 7.76 (ii) 17.86 (ii)
01238> Unit Hyd. Tpeak (min)= 7.50 17.50
01239> Unit Hyd. peak (cms)= .15 .06
01240>
01241> PEAK FLOW (cms)= .36 .01
01242> TIME TO PEAK (hrs)= 1.04 1.25
01243> RUNOFF VOLUME (mm)= 40.94 14.70
01244> TOTAL RAINFALL (mm)= 42.51 42.51
01245> RUNOFF COEFFICIENT = .96 .35
01246>
01247> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01248> CN* = 81.0 Ia = Dep. Storage (Above)
01249> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01250> THAN THE STORAGE COEFFICIENT.
01251> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01252>
01253>
01254> 001:0005-----
01255> *
01256> * SUB-AREA No.2
01257>
01258> | CALIB STANDHYD | Area (ha)= 1.54
01259> | 02:020 DT= 2.50 | Total Imp (%) = 92.00 Dir. Conn. (%) = 92.00
01260>
01261> IMPVIOUS PERVIOUS (i)
01262> Surface Area (ha)= 1.42 .12
01263> Dep. Storage (mm)= 1.57 4.67
01264> Average Slope (%) = .50 1.00
01265> Length (m) = 244.34 5.00
01266> Mannings n = .030 .030
01267>
01268> Max.eff.Inten.(mm/hr)= 104.19 31.02
01269> over (min) 7.50 10.00
01270> Storage Coeff. (min)= 8.73 (ii) 9.85 (ii)
01271> Unit Hyd. Tpeak (min)= 7.50 10.00
01272> Unit Hyd. peak (cms)= .14 .11
01273>
01274> PEAK FLOW (cms)= .28 .01
01275> TIME TO PEAK (hrs)= 1.04 1.13
01276> RUNOFF VOLUME (mm)= 40.94 14.70
01277> TOTAL RAINFALL (mm)= 42.51 42.51
01278> RUNOFF COEFFICIENT = .96 .35
01279>
01280> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01281> CN* = 81.0 Ia = Dep. Storage (Above)
01282> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01283> THAN THE STORAGE COEFFICIENT.
01284> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01285>
01286>
01287> 001:0006-----
01288> *
01289> * SUB-AREA No.3
01290>
01291> | CALIB STANDHYD | Area (ha)= 1.40
01292> | 03:030 DT= 2.50 | Total Imp (%) = 97.00 Dir. Conn. (%) = 97.00
01293>
01294> IMPVIOUS PERVIOUS (i)
01295> Surface Area (ha)= 1.36 .04
01296> Dep. Storage (mm)= 1.57 4.67
01297> Average Slope (%) = .51 1.00
01298> Length (m) = 225.63 5.00
01299> Mannings n = .030 .030
01300>
01301> Max.eff.Inten.(mm/hr)= 104.19 31.02
01302> over (min) 7.50 10.00
01303> Storage Coeff. (min)= 8.28 (ii) 9.39 (ii)
01304> Unit Hyd. Tpeak (min)= 7.50 10.00
01305> Unit Hyd. peak (cms)= .14 .12
01306>
01307> PEAK FLOW (cms)= .27 .00
01308> TIME TO PEAK (hrs)= 1.04 1.13
01309> RUNOFF VOLUME (mm)= 40.94 14.70
01310> TOTAL RAINFALL (mm)= 42.51 42.51
01311> RUNOFF COEFFICIENT = .96 .35
01312>
01313> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01314> CN* = 81.0 Ia = Dep. Storage (Above)
01315> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01316> THAN THE STORAGE COEFFICIENT.
01317> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01318>
01319>
01320> 001:0007-----
01321>
01322> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01323> | (ha) (cms) (hrs) (mm) (cms)
01324> ID1 01:010 2.07 .362 1.04 36.75 .000
01325> +ID2 02:020 1.54 .283 1.04 38.94 .000
01326>
01327> SUM 04:040 3.61 .645 1.04 37.64 .000
01328>
01329> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01330>
01331>
01332> 001:0008-----
01333>
01334> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01335> | (ha) (cms) (hrs) (mm) (cms)
01336> ID1 03:030 1.40 .274 1.04 40.16 .000
01337> +ID2 04:040 3.61 .645 1.04 37.64 .000
01338>
01339> SUM 05:050 5.01 .918 1.04 38.34 .000
01340>
01341> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01342>
01343>
01344> 001:0009-----
01345> *
01346> * SUB-AREA No.4
01347>
01348> | CALIB STANDHYD | Area (ha)= .89
01349> | 06:060 DT= 2.50 | Total Imp (%) = 97.00 Dir. Conn. (%) = 97.00
01350>

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01351> Surface Area (ha)= .86 IMPERVIOUS PERVIOUS (i)
 01352> Dep. Storage (mm)= 1.57 4.67
 01353> Average Slope (%)= .93 .70
 01354> Length (m)= 164.82 40.00
 01355> Mannings n = .030 .250
 01357> Max. eff. Inten. (mm/hr)= 104.19 20.32
 01358> over (min)= 5.00 25.00
 01359> Storage Coeff. (min)= 5.72 (ii) 24.02 (ii)
 01360> Unit Hyd. Tpeak (min)= 5.72 25.00
 01361> Unit Hyd. peak (cms)= .20 .05
 01362> *TOTALS*
 01363> PEAK FLOW (cms)= .20 .00 .205 (iii)
 01364> TIME TO PEAK (hrs)= 1.00 1.38 1.000
 01365> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
 01366> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
 01367> RUNOFF COEFFICIENT = .96 .35 .945
 01368> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01369> CN* = 81.0 Ia = Dep. Storage (Above)
 01370> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01371> THAN THE STORAGE COEFFICIENT.
 01372> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01373> 01374> 01375> 01376> 01377> 001:0010-
 01378> * SUB-AREA No.5
 01379> 01380> 01381> | CALIB STANDHYD | Area (ha)= 2.66
 01382> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
 01383> 01384> IMPERVIOUS PERVIOUS (i)
 01385> Surface Area (ha)= 2.58 .09
 01386> Dep. Storage (mm)= 1.57 4.67
 01387> Average Slope (%)= .61 1.50
 01388> Length (m)= 207.25 20.00
 01389> Mannings n = .030 .250
 01390> Max. eff. Inten. (mm/hr)= 104.19 24.26
 01391> over (min)= 7.50 17.50
 01392> Storage Coeff. (min)= 7.45 (ii) 16.40 (ii)
 01393> Unit Hyd. Tpeak (min)= 7.50 17.50
 01394> Unit Hyd. peak (cms)= .15 .07
 01395> *TOTALS*
 01396> PEAK FLOW (cms)= .54 .00 .538 (iii)
 01397> TIME TO PEAK (hrs)= 1.04 1.25 1.042
 01398> RUNOFF VOLUME (mm)= 40.94 14.70 40.157
 01399> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
 01400> RUNOFF COEFFICIENT = .96 .35 .945
 01401> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01402> CN* = 81.0 Ia = Dep. Storage (Above)
 01403> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01404> THAN THE STORAGE COEFFICIENT.
 01405> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01406> 01407> 01408> 01409> 01410> 001:0011-
 01411> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01412> (ha) (cms) (hrs) (mm) (cms)
 01413> ID1 06:060 .89 .205 1.00 40.16 .000
 01414> +ID2 07:070 2.66 .538 1.04 40.16 .000
 01415> SUM 08:080 3.55 .733 1.04 40.16 .000
 01416> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01417> 01418> 01419> 01420> 01421> 01422> 001:0012-
 01423> | ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01424> (ha) (cms) (hrs) (mm) (cms)
 01425> ID1 05:050 5.01 .918 1.04 38.34 .000
 01426> +ID2 08:080 3.55 .733 1.04 40.16 .000
 01427> SUM 09:090 8.56 1.651 1.04 39.10 .000
 01428> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01429> 01430> 01431> 01432> 01433> 01434> 001:0013-
 01435> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
 01436> | IN>09: (090) |
 01437> | OUT<10: (POND) |
 01438> ===== OUTFLOW STORAGE TABLE =====
 01439> OUTFLOW STORAGE OUTFLOW STORAGE
 01440> (cms) (ha.m.) (cms) (ha.m.)
 01441> .000 .0000E+00 .593 .6251E+00
 01442> .008 .6560E+01 .654 .6631E+00
 01443> .017 .1311E+00 .797 .7391E+00
 01444> .093 .2831E+00 .950 .8274E+00
 01445> .233 .3971E+00 1.304 .9157E+00
 01446> .337 .4731E+00 1.880 .1004E+01
 01447> .465 .5491E+00 2.577 .1092E+01
 01448> .531 .5871E+00 .000 .0000E+00
 01449> ROUTING RESULTS AREA QPEAK TPEAK R.V.
 01450> (ha) (cms) (hrs) (mm)
 01451> INFLOW >09: (POND) 8.56 1.651 1.042 39.096
 01452> OUTFLOW<10: (POND) 8.56 .089 2.625 39.093
 01453> PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.413
 01454> TIME SHIFT OF PEAK FLOW (min)= 95.00
 01455> MAXIMUM STORAGE USED (ha.m.) = .2758E+00
 01456> 01457> 01458> 01459> 01460> 001:0014-
 01461> * Remaining Hawthorne Industrial Park *
 01462> * SUB-AREA No.1
 01463> 01464> 01465> 01466> 01467> | CALIB STANDHYD | Area (ha)= 19.90
 01468> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 01469> 01470> IMPERVIOUS PERVIOUS (i)
 01471> Surface Area (ha)= 14.13 5.77
 01472> Dep. Storage (mm)= .57 4.67
 01473> Average Slope (%)= .60 1.50
 01474> Length (m)= 580.00 100.00
 01475> Mannings n = .030 .250
 01476> Max. eff. Inten. (mm/hr)= 80.14 42.65
 01477> over (min)= 15.00 35.00
 01478> Storage Coeff. (min)= 15.43 (ii) 34.18 (ii)
 01479> Unit Hyd. Tpeak (min)= 15.00 35.00
 01480> Unit Hyd. peak (cms)= .07 .03
 01481> *TOTALS*
 01482> PEAK FLOW (cms)= 1.41 .40 1.572 (iii)
 01483> TIME TO PEAK (hrs)= 1.17 1.54 1.208
 01484> RUNOFF VOLUME (mm)= 40.94 21.31 31.126

01486> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
 01487> RUNOFF COEFFICIENT = .96 .50 .732
 01488> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01489> CN* = 81.0 Ia = Dep. Storage (Above)
 01490> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01491> THAN THE STORAGE COEFFICIENT.
 01492> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01493> 01494> 01495> 01496> 001:0015-
 01497> | ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01498> (ha) (cms) (hrs) (mm) (cms)
 01499> ID1 10:POND 8.56 .089 2.63 39.09 .000
 01500> +ID2 01:HIP01 19.90 1.572 1.21 31.13 .000
 01501> SUM 02:HIP02 28.46 1.615 1.21 33.52 .000
 01502> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01503> 01504> 01505> 01506> 01507> 001:0016-
 01508> * SUB-AREA No.2
 01509> 01510> 01511> | CALIB STANDHYD | Area (ha)= 17.00
 01512> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 01513> 01514> IMPERVIOUS PERVIOUS (i)
 01515> Surface Area (ha)= 12.07 4.93
 01516> Dep. Storage (mm)= 1.57 4.67
 01517> Average Slope (%)= .65 1.50
 01518> Length (m)= 450.00 100.00
 01519> Mannings n = .030 .250
 01520> Max. eff. Inten. (mm/hr)= 89.76 47.48
 01521> over (min)= 12.50 30.00
 01522> Storage Coeff. (min)= 12.36 (ii) 30.32 (ii)
 01523> Unit Hyd. Tpeak (min)= 12.50 30.00
 01524> Unit Hyd. peak (cms)= .09 .04
 01525> *TOTALS*
 01526> PEAK FLOW (cms)= 1.36 .37 1.504 (iii)
 01527> TIME TO PEAK (hrs)= 1.13 1.46 1.167
 01528> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
 01529> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
 01530> RUNOFF COEFFICIENT = .96 .50 .732
 01531> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01532> CN* = 81.0 Ia = Dep. Storage (Above)
 01533> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01534> THAN THE STORAGE COEFFICIENT.
 01535> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01536> 01537> 01538> 01539> 01540> 001:0017-
 01541> * SUB-AREA No.3
 01542> | CALIB STANDHYD | Area (ha)= 15.60
 01543> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 01544> 01545> IMPERVIOUS PERVIOUS (i)
 01546> Surface Area (ha)= 11.08 4.52
 01547> Dep. Storage (mm)= 1.57 4.67
 01548> Average Slope (%)= .50 1.50
 01549> Length (m)= 600.00 100.00
 01550> Mannings n = .030 .250
 01551> Max. eff. Inten. (mm/hr)= 73.27 42.65
 01552> over (min)= 17.50 35.00
 01553> Storage Coeff. (min)= 17.24 (ii) 35.98 (ii)
 01554> Unit Hyd. Tpeak (min)= 17.50 35.00
 01555> Unit Hyd. peak (cms)= .07 .03
 01556> *TOTALS*
 01557> PEAK FLOW (cms)= 1.03 .30 1.176 (iii)
 01558> TIME TO PEAK (hrs)= 1.21 1.54 1.250
 01559> RUNOFF VOLUME (mm)= 40.94 21.31 31.126
 01560> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
 01561> RUNOFF COEFFICIENT = .96 .50 .732
 01562> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01563> CN* = 81.0 Ia = Dep. Storage (Above)
 01564> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01565> THAN THE STORAGE COEFFICIENT.
 01566> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01567> 01568> 01569> 01570> 01571> 01572> 01573> 001:0018-
 01574> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01575> (ha) (cms) (hrs) (mm) (cms)
 01576> ID1 03:HIP03 17.00 1.504 1.17 31.13 .000
 01577> +ID2 04:HIP04 15.60 1.176 1.25 31.13 .000
 01578> SUM 05:HIP05 32.60 2.621 1.17 31.13 .000
 01579> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01580> 01581> 01582> 01583> 01584> 01585> 001:0019-
 01586> | ADD HYD (HIP06) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01587> (ha) (cms) (hrs) (mm) (cms)
 01588> ID1 05:HIP05 32.60 2.621 1.17 31.13 .000
 01589> +ID2 02:HIP02 28.46 1.615 1.21 33.52 .000
 01590> SUM 06:HIP06 61.06 4.222 1.17 32.24 .000
 01591> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01592> 01593> 01594> 01595> 01596> 01597> 001:0020-
 01598> * SUB-AREA No.4
 01599> 01600> 01601> | CALIB STANDHYD | Area (ha)= 12.20
 01602> | 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 01603> 01604> IMPERVIOUS PERVIOUS (i)
 01605> Surface Area (ha)= 8.66 3.54
 01606> Dep. Storage (mm)= 1.57 4.67
 01607> Average Slope (%)= .70 1.50
 01608> Length (m)= 210.00 100.00
 01609> Mannings n = .030 .250
 01610> Max. eff. Inten. (mm/hr)= 104.19 52.96
 01611> over (min)= 7.50 25.00
 01612> Storage Coeff. (min)= 7.21 (ii) 24.40 (ii)
 01613> Unit Hyd. Tpeak (min)= 7.50 25.00
 01614> Unit Hyd. peak (cms)= .15 .05
 01615> *TOTALS*
 01616> PEAK FLOW (cms)= 1.28 .31 1.375 (iii)
 01617> TIME TO PEAK (hrs)= 1.04 1.38 1.042
 01618> RUNOFF VOLUME (mm)= 40.94 21.31 31.126

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01621> TOTAL RAINFALL (mm)= 42.51 42.51 42.514
01622> RUNOFF COEFFICIENT = .96 .50 .732
01623>
01624> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01625> CN* = 81.0 Ia = Dep. Storage (Above)
01626> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01627> THAN THE STORAGE COEFFICIENT.
01628> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01629>
01630>
01631> 001:0021-----
01632> *
01633> *SUB-AREA No.5
01634>
01635> | DESIGN NASHYD | Area (ha)= 4.09 Curve Number (CN)=85.00
01636> | 08:Pcond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01637> | U.H. Tp(hrs)= .170
01638>
01639> Unit Hyd Qpeak (cms)= .899
01640>
01641> PEAK FLOW (cms)= .260 (i)
01642> TIME TO PEAK (hrs)= 1.167
01643> RUNOFF VOLUME (mm)= 17.325
01644> TOTAL RAINFALL (mm)= 42.514
01645> RUNOFF COEFFICIENT = .408
01646>
01647> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01648>
01649>
01650> 001:0022-----
01651>
01652> | ADD HYD (HIP08 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01653> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01654> | IN-09: (HIP08 ) |
01655> | OUT<10: (HIP-PO) |
01656>
01657> ID1 06:HIP06 61.06 4.222 1.17 32.24 .000
01658> +ID2 07:HIP07 12.20 1.375 1.04 31.13 .000
01659> +ID3 08:Pcond-B 4.00 .260 1.17 17.32 .000
01660>
01661> SUM 09:HIP08 77.26 5.545 1.17 31.29 .000
01662>
01663> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01664>
01665> 001:0023-----
01666>
01667> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01668> | IN-09: (HIP08 ) |
01669> | OUT<10: (HIP-PO) |
01670>
01671> ===== OUTFLOW STORAGE TABLE =====
01672> OUTFLOW STORAGE OUTFLOW STORAGE
01673> (cms) (ha.m.) (cms) (ha.m.)
01674> .000 .0000E+00 .724 .2210E+01
01675> .048 .3740E+01 .937 .2501E+01
01676> .054 .2434E+00 1.262 .2798E+01
01677> .059 .5834E+00 1.404 .3101E+01
01678> .062 .8400E+00 1.532 .3410E+01
01679> .064 .1102E+01 1.650 .3724E+01
01680> .147 .1370E+01 2.409 .4044E+01
01681> .280 .1644E+01 3.689 .4370E+01
01682> .472 .1924E+01 .000 .0000E+00
01683>
01684> ROUTING RESULTS AREA OPEAK TPEAK R.V.
01685> (ha) (cms) (hrs) (mm)
01686> INFLOW>09: (HIP08 ) 77.26 5.545 1.167 31.292
01687> OUTFLOW<10: (HIP-PO) 77.26 .435 3.389 31.292
01688>
01689> PEAK FLOW REDUCTION [Qout/qin] (%) = 7.850
01690> TIME SHIFT OF PEAK FLOW (min) = 133.33
01691> MAXIMUM STORAGE USED (ha.m.)=.1871E+01
01692>
01693> 001:0024-----
01694> *
01695> *SUB-AREA No. 6
01696>
01697> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
01698> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
01699> | U.H. Tp(hrs)= .800
01700>
01701> Unit Hyd Qpeak (cms)= .129
01702>
01703> PEAK FLOW (cms)= .044 (i)
01704> TIME TO PEAK (hrs)= 2.042
01705> RUNOFF VOLUME (mm)= 12.131
01706> TOTAL RAINFALL (mm)= 42.514
01707> RUNOFF COEFFICIENT = .285
01708>
01709> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01710>
01711> 001:0025-----
01712>
01713> | ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01714> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01715> | IN-10:HIP-PO |
01716> | +ID2 01:A3 |
01717>
01718> ID1 10:HIP-PO 77.26 .435 3.39 31.29 .000
01719> +ID2 01:A3 2.70 .044 2.04 12.13 .000
01720>
01721> SUM 02:Ultima 79.96 .457 3.29 30.65 .000
01722>
01723> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01724>
01725> 001:0026-----
01726> *
01727> * CALCULATION OF 3HR = 1:10 YEAR STORM EVENT
01728>
01729> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01730> | TZERO = .00 hrs on Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
01731> | METOUT= 2 (output = METRIC)
01732> | NRUN = 001
01733> | NSTORM= 0
01734>
01735> 001:0002-----
01736>
01737> | CHICAGO STORM | IDF curve parameters: A=1174.184
01738> | Ptotal= 49.50 mm | B= 6.414
01739> | C= .816
01740>
01741> used in: INTENSITY = A / (t + B)^C
01742>
01743> Duration of storm = 3.00 hrs
01744> Storm time step = 10.00 min
01745> Time to peak ratio = .33
01746>
01747> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
01748> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
01749> .17 4.248 1.00 122.142 1.83 7.733 2.67 4.049
01750> .33 5.290 1.17 37.288 2.00 6.502 2.83 3.714
01751> .50 7.108 1.33 18.954 2.17 5.625 3.00 3.434
01752> .67 11.130 1.50 12.700 2.33 4.969
01753> .83 28.100 1.67 9.588 2.50 4.458
01754>
01755> 001:0003-----
01756>
01757> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL

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01756> ICASEdv = 1 (read and print data)
01757> Filetitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
01758> ----- PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
01759> Horton's infiltration equation parameters:
01760> [F= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [F= .00 mm]
01761> Parameters for PERVIOUS surfaces in STANDHYD:
01762> [IAPER= 4.67 mm] [LGP=40.00 m] [MNI= .250]
01763> Parameters for IMPERVIOUS surfaces in STANDHYD:
01764> [IAlmp= 1.57 mm] [CLI= 1.50] [MNI= .035]
01765> Parameters used in NASHYD:
01766> [Ia= 4.67 mm] [N= 3.00]
01767>
01768> 001:0004-----
01769> *
01770> * ORGAWORLD FILE
01771> *
01772> *
01773> * SUB-AREA No.1
01774>
01775> | CALIB STANDHYD | Area (ha)= 2.07 Dir. Conn. (%) = 84.00
01776> | 01:010 DT= 2.50 | Total Imp (%) =
01777>
01778> IMPERVIOUS PERVIOUS (i)
01779> Surface Area (ha)= 1.74 .33
01780> Dep. Storage (mm)= 1.57 4.67
01781> Average Slope (%) = 5.22 1.00
01782> Length (m) = 204.72 20.00
01783> Mannings n = .030 .250
01784>
01785> Max.eff.Inten.(mm/hr)= 122.14 34.69
01786> over (min) 7.50 15.00
01787> Storage Coeff. (min)= 7.28 (ii) 16.04 (ii)
01788> Unit Hyd. Tpeak (min)= 7.50 15.00
01789> Unit Hyd. peak (cms)= .15 .07
01790>
01791> PEAK FLOW (cms)= .43 .02 *TOTALS*
01792> TIME TO PEAK (hrs)= 1.04 1.21 .437 (iii)
01793> RUNOFF VOLUME (mm)= 47.93 19.25 43.345
01794> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01795> RUNOFF COEFFICIENT = .97 .39 .876
01796>
01797> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01798> CN* = 81.0 Ia = Dep. Storage (Above)
01799> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01800> THAN THE STORAGE COEFFICIENT.
01801> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01802>
01803> 001:0005-----
01804> *
01805> * SUB-AREA No.2
01806>
01807> | CALIB STANDHYD | Area (ha)= 1.54 Dir. Conn. (%) = 92.00
01808> | 02:020 DT= 2.50 | Total Imp (%) =
01809>
01810> IMPERVIOUS PERVIOUS (i)
01811> Surface Area (ha)= 1.42 .12
01812> Dep. Storage (mm)= 1.57 4.67
01813> Average Slope (%) = .50 1.00
01814> Length (m) = 244.34 5.00
01815> Mannings n = .030 .030
01816>
01817> Max.eff.Inten.(mm/hr)= 122.14 42.32
01818> over (min) 7.50 10.00
01819> Storage Coeff. (min)= 8.20 (ii) 9.18 (ii)
01820> Unit Hyd. Tpeak (min)= 7.50 10.00
01821> Unit Hyd. peak (cms)= .14 .12
01822>
01823> PEAK FLOW (cms)= .33 .01 *TOTALS*
01824> TIME TO PEAK (hrs)= 1.04 1.13 .341 (iii)
01825> RUNOFF VOLUME (mm)= 47.93 19.25 45.640
01826> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01827> RUNOFF COEFFICIENT = .97 .39 .922
01828>
01829> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01830> CN* = 81.0 Ia = Dep. Storage (Above)
01831> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01832> THAN THE STORAGE COEFFICIENT.
01833> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01834>
01835> 001:0006-----
01836> *
01837> * SUB-AREA No.3
01838>
01839> | CALIB STANDHYD | Area (ha)= 1.40 Dir. Conn. (%) = 97.00
01840> | 03:030 DT= 2.50 | Total Imp (%) =
01841>
01842> IMPERVIOUS PERVIOUS (i)
01843> Surface Area (ha)= 1.36 .04
01844> Dep. Storage (mm)= 1.57 4.67
01845> Average Slope (%) = .53 1.00
01846> Length (m) = 225.63 5.00
01847> Mannings n = .030 .030
01848>
01849> Max.eff.Inten.(mm/hr)= 122.14 48.18
01850> over (min) 7.50 7.50
01851> Storage Coeff. (min)= 7.77 (ii) 8.70 (ii)
01852> Unit Hyd. Tpeak (min)= 7.50 7.50
01853> Unit Hyd. peak (cms)= .15 .14
01854>
01855> PEAK FLOW (cms)= .33 .00 *TOTALS*
01856> TIME TO PEAK (hrs)= 1.04 1.08 .329 (iii)
01857> RUNOFF VOLUME (mm)= 47.93 19.25 47.074
01858> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
01859> RUNOFF COEFFICIENT = .97 .39 .951
01860>
01861> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01862> CN* = 81.0 Ia = Dep. Storage (Above)
01863> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01864> THAN THE STORAGE COEFFICIENT.
01865> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01866>
01867> 001:0007-----
01868>
01869> | ADD HYD (040 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01870> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01871> | IN-01:010 |
01872> | +ID2 02:020 |
01873>
01874> ID1 01:010 2.07 .437 1.04 43.35 .000
01875> +ID2 02:020 1.54 .341 1.04 45.64 .000
01876>
01877> SUM 04:040 3.61 .778 1.04 44.32 .000
01878>
01879> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01880>
01881> 001:0008-----
01882>
01883> | ADD HYD (050 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
01884> | ROUTE RESERVOIR | (ha) (cms) (hrs) (mm) (cms)
01885> | IN-03:030 |
01886> | +ID2 04:040 |
01887>
01888> ID1 03:030 1.40 .329 1.04 47.07 .000
01889> +ID2 04:040 3.61 .778 1.04 44.32 .000
01890>
01891> SUM 05:050 5.01 1.107 1.04 45.09 .000

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02161> Max.eff.inten.(mm/hr)= 122.14 72.53
02162> over (min) 7.50 22.50
02163> Storage Coeff. (min)= 6.77 (ii) 21.93 (ii)
02164> Unit Hyd. Tpeak (min)= 7.50 22.50
02165> Unit Hyd. peak (cms)= .16 .05
02166>
02167> *TOTALS*
02168> PEAK FLOW (cms)= 1.54 .42 1.687 (iii)
02169> TIME TO PEAK (hrs)= 1.04 1.33 1.042
02170> RUNOFF VOLUME (mm)= 47.93 26.92 37.426
02171> TOTAL RAINFALL (mm)= 49.50 49.50 49.505
02172> RUNOFF COEFFICIENT = .97 .54 .756
02173>
02174> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02175> CN* = 81.0 Ia = Dep. Storage (Above)
02176> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02177> THAN THE STORAGE COEFFICIENT.
02178> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02179>
02180> 001:0021-----
02181> *SUB-AREA No.5
02182>
02183> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
02184> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
02185> | U.H. Tp(hrs)= .170
02186>
02187> Unit Hyd Qpeak (cms)= .899
02188>
02189> PEAK FLOW (cms)= .345 (i)
02190> TIME TO PEAK (hrs)= 1.167
02191> RUNOFF VOLUME (mm)= 22.420
02192> TOTAL RAINFALL (mm)= 49.505
02193> RUNOFF COEFFICIENT = .453
02194>
02195> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02196>
02197> 001:0022-----
02198>
02199> | ADD HYD (HIP08 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02200> | ID1 05:HIP06 | (ha) (cms) (hrs) (mm) (cms)
02201> | ID2 07:HIP07 | 61.06 5.358 1.17 38.61 .000
02202> | ID3 08:Pond-B | 12.20 1.687 1.04 37.43 .000
02203> | ID3 08:Pond-B | 4.00 .345 1.17 22.42 .000
02204>
02205> SUM 09:HIP08 77.26 7.016 1.17 37.59 .000
02206>
02207> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02208>
02209> 001:0023-----
02210>
02211> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
02212> | IN:09: (HIP08 ) |
02213> | OUT:10: (HIP-PO) |
02214>
02215> ===== OUTFLOW STORAGE TABLE =====
02216> OUTFLOW STORAGE OUTFLOW STORAGE
02217> (cms) (ha.m.) (cms) (ha.m.)
02218> .000 .000E+00 724 .2210E+01
02219> .048 .5740E-01 .937 .2501E+01
02220> .054 .2434E+00 1.262 .2798E+01
02221> .059 .5830E+00 1.404 .3101E+01
02222> .062 .8400E+00 1.532 .3410E+01
02223> .064 .1102E+01 1.650 .3724E+01
02224> .147 .1370E+01 2.409 .4044E+01
02225> .280 .1644E+01 3.689 .4370E+01
02226> .472 .1924E+01 .000 .0000E+00
02227>
02228> ROUTING RESULTS AREA QPEAK TPEAK R.V.
02229> (ha) (cms) (hrs) (mm)
02230> INFLOW>09: (HIP08 ) 77.26 7.016 1.167 37.588
02231> OUTFLOW<10: (HIP-PO) 77.26 .696 3.208 37.588
02232>
02233> PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.919
02234> TIME SHIFT OF PEAK FLOW (min) = 122.50
02235> MAXIMUM STORAGE USED (ha.m.) = .2178E+01
02236>
02237> 001:0024-----
02238>
02239> *SUB-AREA No. 6
02240>
02241> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
02242> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
02243> | U.H. Tp(hrs)= .800
02244>
02245> Unit Hyd Qpeak (cms)= .129
02246>
02247> PEAK FLOW (cms)= .059 (i)
02248> TIME TO PEAK (hrs)= 2.000
02249> RUNOFF VOLUME (mm)= 16.075
02250> TOTAL RAINFALL (mm)= 49.505
02251> RUNOFF COEFFICIENT = .325
02252>
02253> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02254>
02255> 001:0025-----
02256>
02257> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02258> | ID1 10:HIP-PO | (ha) (cms) (hrs) (mm) (cms)
02259> | ID2 01:A3 | 77.26 .696 3.21 37.59 .000
02260> | SUM 02:Ultima | 2.70 .059 2.00 16.08 .000
02261>
02262> SUM 02:Ultima 79.96 .729 3.15 36.86 .000
02263>
02264> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02265>
02266> 001:0026-----
02267>
02268> ***** CALCULATION OF 3HR - 1:25 YEAR STORM EVENT *****
02269>
02270> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
02271> | TZERO = .00 hrs on 0 Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
02272> | METOUT= 2 (output = METRIC)
02273> | NRUN = 001
02274> | NSTORM= 0
02275>
02276> 001:0002-----
02277>
02278> | CHICAGO STORM | IDF curve parameters: A=1402.884
02279> | Ptotal= 58.23 mm | B= 6.018
02280> | C= .819
02281> used in: INTENSITY = A / (t + B)^C
02282>
02283> Duration of storm = 3.00 hrs
02284> Storm time step = 10.00 min
02285> Time to peak ratio = .33
02286>
02287> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
02288> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
02289>
02290>
02291>
02292>
02293>
02294>
02295>

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02296> .17 4.934 | 1.00 144.693 | 1.83 9.014 | 2.67 4.701
02297> .33 6.152 | 1.17 43.904 | 2.00 7.571 | 2.83 4.310
02298> .50 8.282 | 1.33 22.224 | 2.17 6.544 | 3.00 3.983
02299> .67 13.006 | 1.50 14.852 | 2.33 5.776 |
02300> .83 33.041 | 1.67 11.192 | 2.50 5.179 |
02301>
02302>
02303> 001:0003-----
02304>
02305> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
02306> | ICASEd= 1 (read and print data)
02307> | FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE -----
02308> | PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----0
02309> Horton's infiltration equation parameters:
02310> [F= 50.00 mm/hr] [F= 7.50 mm/hr] [DCA= 2.00 /hr] [F= .00 mm]
02311> Parameters for PERVIOUS surfaces in STANDHYD:
02312> [IAPER= 4.67 mm] [LGP=40.00 m] [MNP=.250]
02313> Parameters for IMPERVIOUS surfaces in STANDHYD:
02314> [IAMP= 1.57 mm] [CL=1.50] [MNI=.035]
02315> Parameters used in NASHYD:
02316> [Ia= 4.67 mm] [N= 3.00]
02317>
02318> 001:0004-----
02319>
02320> ***** ORGAWORLD FILE *****
02321>
02322> * SUB-AREA No.1
02323>
02324> | CALIB STANDHYD | Area (ha)= 2.07
02325> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
02326>
02327>
02328> IMPERVIOUS PERVIOUS (i)
02329> Surface Area (ha)= 1.74 .33
02330> Dep. Storage (mm)= 1.57 4.67
02331> Average Slope (%)= .52 1.00
02332> Length (m)= 204.72 20.00
02333> Mannings n = .030 .250
02334>
02335> Max.eff.inten.(mm/hr)= 144.69 47.07
02336> over (min) 7.50 15.00
02337> Storage Coeff. (min)= 6.81 (ii) 14.56 (ii)
02338> Unit Hyd. Tpeak (min)= 7.50 15.00
02339> Unit Hyd. peak (cms)= .16 .08
02340>
02341> PEAK FLOW (cms)= .52 .03 *TOTALS*
02342> TIME TO PEAK (hrs)= 1.04 1.21 1.532 (iii)
02343> RUNOFF VOLUME (mm)= 56.66 25.35 1.042
02344> TOTAL RAINFALL (mm)= 58.23 58.23 51.647
02345> RUNOFF COEFFICIENT = .97 .44 58.226
02346>
02347> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02348> CN* = 81.0 Ia = Dep. Storage (Above)
02349> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02350> THAN THE STORAGE COEFFICIENT.
02351> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02352>
02353> 001:0005-----
02354>
02355> * SUB-AREA No.2
02356>
02357> | CALIB STANDHYD | Area (ha)= 1.54
02358> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
02359>
02360>
02361> IMPERVIOUS PERVIOUS (i)
02362> Surface Area (ha)= 1.42 .12
02363> Dep. Storage (mm)= 1.57 4.67
02364> Average Slope (%)= .50 1.00
02365> Length (m)= 244.34 5.00
02366> Mannings n = .030 .030
02367>
02368> Max.eff.inten.(mm/hr)= 144.69 65.19
02369> over (min) 7.50 7.50
02370> Storage Coeff. (min)= 7.66 (ii) 8.49 (ii)
02371> Unit Hyd. Tpeak (min)= 7.50 7.50
02372> Unit Hyd. peak (cms)= .15 .14
02373>
02374> PEAK FLOW (cms)= .40 .01 *TOTALS*
02375> TIME TO PEAK (hrs)= 1.04 1.08 1.042
02376> RUNOFF VOLUME (mm)= 56.66 25.35 54.152
02377> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02378> RUNOFF COEFFICIENT = .97 .44 .930
02379>
02380> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02381> CN* = 81.0 Ia = Dep. Storage (Above)
02382> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02383> THAN THE STORAGE COEFFICIENT.
02384> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02385>
02386> 001:0006-----
02387>
02388> * SUB-AREA No.3
02389>
02390> | CALIB STANDHYD | Area (ha)= 1.40
02391> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02392>
02393>
02394> IMPERVIOUS PERVIOUS (i)
02395> Surface Area (ha)= 1.36 .04
02396> Dep. Storage (mm)= 1.57 4.67
02397> Average Slope (%)= .51 1.00
02398> Length (m)= 225.63 5.00
02399> Mannings n = .030 .030
02400>
02401> Max.eff.inten.(mm/hr)= 144.69 65.19
02402> over (min) 7.50 7.50
02403> Storage Coeff. (min)= 7.26 (ii) 8.09 (ii)
02404> Unit Hyd. Tpeak (min)= 7.50 7.50
02405> Unit Hyd. peak (cms)= .15 .14
02406>
02407> PEAK FLOW (cms)= .40 .00 *TOTALS*
02408> TIME TO PEAK (hrs)= 1.04 1.08 1.400 (iii)
02409> RUNOFF VOLUME (mm)= 56.66 25.35 55.717
02410> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
02411> RUNOFF COEFFICIENT = .97 .44 .957
02412>
02413> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02414> CN* = 81.0 Ia = Dep. Storage (Above)
02415> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02416> THAN THE STORAGE COEFFICIENT.
02417> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02418>
02419> 001:0007-----
02420>
02421> | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02422> | ID1 01:010 | (ha) (cms) (hrs) (mm) (cms)
02423> | ID2 02:020 | 2.07 .532 1.04 51.65 .000
02424> | SUM 04:040 | 1.54 .418 1.04 54.15 .000
02425>
02426> SUM 04:040 3.61 .950 1.04 52.72 .000
02427>
02428> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02429>
02430>

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02431> 001:0008-----

02432> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02433> | ID1 03:030 | (ha) (cms) (hrs) (mm) (mm) (cms)

02434> | +ID2 04:040 | 1.40 .400 1.04 55.72 .000

02435> | SUM 05:050 | 3.61 .950 1.04 52.72 .000

02436> | | 5.01 1.350 1.04 53.55 .000

02437> | | | | | | |

02438> | | | | | | |

02439> | | | | | | |

02440> | | | | | | |

02441> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02442> 001:0009-----

02443> * SUB-AREA No.4

02444> | CALIB STANDHYD | Area (ha)= .89

02445> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02446> | IMPERVIOUS PERVIOUS (i)

02447> | Surface Area (ha)= .86 .03

02448> | Dep. Storage (mm)= 1.57 4.67

02449> | Average Slope (%)= .93 .70

02450> | Length (m)= 164.82 40.00

02451> | Mannings n = .030 .250

02452> | Max. eff. Inten. (mm/hr)= 144.69 44.12

02453> | over (min)= 5.00 17.50

02454> | Storage Coeff. (min)= 5.02 (ii) 18.44 (ii)

02455> | Unit Hyd. Tpeak (min)= 5.00 17.50

02456> | Unit Hyd. peak (cms)= .22 .06

02457> | | | | |

02458> | PEAK FLOW (cms)= .30 .00 *TOTALS*

02459> | TIME TO PEAK (hrs)= 1.00 1.25 (iii)

02460> | RUNOFF VOLUME (mm)= 56.66 25.35

02461> | TOTAL RAINFALL (mm)= 58.23 58.23

02462> | RUNOFF COEFFICIENT = .97 .44 .957

02463> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

02464> | CN* = 81.0 Ia = Dep. Storage (Above)

02465> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

02466> | THAN THE STORAGE COEFFICIENT.

02467> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02468> 001:0010-----

02469> * SUB-AREA No.5

02470> | CALIB STANDHYD | Area (ha)= 2.66

02471> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02472> | IMPERVIOUS PERVIOUS (i)

02473> | Surface Area (ha)= 2.58 .08

02474> | Dep. Storage (mm)= 1.57 4.67

02475> | Average Slope (%)= .61 1.50

02476> | Length (m)= 207.25 20.00

02477> | Mannings n = .030 .250

02478> | Max. eff. Inten. (mm/hr)= 144.69 51.33

02479> | over (min)= 7.50 12.50

02480> | Storage Coeff. (min)= 6.54 (ii) 13.16 (ii)

02481> | Unit Hyd. Tpeak (min)= 7.50 12.50

02482> | Unit Hyd. peak (cms)= .16 .09

02483> | | | | |

02484> | PEAK FLOW (cms)= .78 .01 *TOTALS*

02485> | TIME TO PEAK (hrs)= 1.04 1.17 1.042

02486> | RUNOFF VOLUME (mm)= 56.66 25.35

02487> | TOTAL RAINFALL (mm)= 58.23 58.23

02488> | RUNOFF COEFFICIENT = .97 .44 .957

02489> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

02490> | CN* = 81.0 Ia = Dep. Storage (Above)

02491> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

02492> | THAN THE STORAGE COEFFICIENT.

02493> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02494> 001:0011-----

02495> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02496> | ID1 06:060 | (ha) (cms) (hrs) (mm) (mm) (cms)

02497> | +ID2 07:070 | .89 .296 1.00 55.72 .000

02498> | SUM 08:080 | 2.66 .783 1.04 55.72 .000

02499> | | 3.55 1.060 1.04 55.72 .000

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02566> 001:0015-----

02567> | ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

02568> | 01:HIP01 DT= 2.50 | (ha) (cms) (hrs) (mm) (mm) (cms)

02569> | ID1 10:POND | 8.56 .189 2.06 54.45 .000

02570> | +ID2 01:HIP01 | 19.90 2.548 1.17 45.44 .000

02571> | SUM 02:HIP02 | 28.46 2.622 1.17 48.15 .000

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02701> CALIB STANDHYD | Area (ha)= 12.20
 02702> 07:HIP07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00

02703> IMPERVIOUS PERVIOUS (i)
 02704> Surface Area (ha)= 8.66 3.54
 02705> Dep. Storage (mm)= 1.57 4.67
 02706> Average Slope (%)= .70 1.50
 02707> Length (m)= 210.00 100.00
 02708> Mannings n = .030 .250

02709> Max. eff. Inten. (mm/hr)= 144.69 101.36
 02710> over (min)= 7.50 20.00
 02711> Storage Coeff. (min)= 6.32 (ii) 19.58 (ii)
 02712> Unit Hyd. Tpeak (min)= 7.50 20.00
 02713> Unit Hyd. peak (cms)= .17 .06

02714> PEAK FLOW (cms)= 1.86 .59 *TOTALS*
 02715> TIME TO PEAK (hrs)= 1.04 1.29 1.042
 02716> RUNOFF VOLUME (mm)= 56.66 34.22 45.437
 02717> TOTAL RAINFALL (mm)= 58.23 58.23 58.226
 02718> RUNOFF COEFFICIENT = .97 .59 .780

02719> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 02720> CN* = 81.0 Ia = Dep. Storage (Above)
 02721> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 02722> THAN THE STORAGE COEFFICIENT.
 02723> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02724> 001:0021-----
 02725> *SUB-AREA No.5

02726> DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
 02727> 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
 02728> U.H. Tp (hrs)= .170

02729> Unit Hyd. Qpeak (cms)= .899
 02730> PEAK FLOW (cms)= .459 (i)
 02731> TIME TO PEAK (hrs)= 1.167
 02732> RUNOFF VOLUME (mm)= 29.155
 02733> TOTAL RAINFALL (mm)= 58.226
 02734> RUNOFF COEFFICIENT = .501

02735> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02736> 001:0022-----

02737> ADD HYD (HIP08) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
 02738> (ha) (cms) (hrs) (mm) (cms)
 02739> ID1 06:HIP06 61.06 6.741 1.17 46.70 .000
 02740> +ID2 07:HIP07 12.20 2.109 1.04 45.44 .000
 02741> +ID3 08:Pond-B 4.00 .459 1.17 29.15 .000
 02742> SUM 09:HIP08 77.26 8.998 1.13 45.59 .000

02743> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02744> 001:0023-----

02745> ROUTE RESERVOIR | Requested routing time step = 1.0 min.
 02746> IN-09: (HIP08) |
 02747> OUT-10: (HIP-PO) |

02748> ===== OUTFLOW STORAGE TABLE =====
 02749> OUTFLOW STORAGE OUTFLOW STORAGE
 02750> (cms) (ha.m.) (cms) (ha.m.)
 02751> .000 .0000E+00 .724 .2210E+01
 02752> .048 .5740E-01 .1937 .2501E+01
 02753> .054 .2434E+00 1.262 .2798E+01
 02754> .059 .5834E+00 1.404 .3101E+01
 02755> .062 .8400E+00 1.532 .3410E+01
 02756> .064 .1102E+01 1.650 .3724E+01
 02757> .147 .1370E+01 2.409 .4048E+01
 02758> .280 .1644E+01 3.689 .4370E+01
 02759> .472 .1924E+01 .000 .0000E+00

02760> ROUTING RESULTS AREA OPEAK TPEAK R.V.
 02761> (ha) (cms) (hrs) (mm)
 02762> INFLOW>09: (HIP08) 77.26 8.998 1.125 45.591
 02763> OUTFLOW>10: (HIP-PO) 77.26 1.004 3.083 45.591

02764> PEAK FLOW REDUCTION [Qout/Qin] (%)= 11.160
 02765> TIME SHIFT OF PEAK FLOW (min)= 117.50
 02766> MAXIMUM STORAGE USED (ha.m.)= .2562E+01

02767> 001:0024-----

02768> *SUB-AREA No. 6
 02769> DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
 02770> 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
 02771> U.H. Tp (hrs)= .800

02772> Unit Hyd. Qpeak (cms)= .129
 02773> PEAK FLOW (cms)= .079 (i)
 02774> TIME TO PEAK (hrs)= 2.000
 02775> RUNOFF VOLUME (mm)= 21.442
 02776> TOTAL RAINFALL (mm)= 58.226
 02777> RUNOFF COEFFICIENT = .368

02778> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02779> 001:0025-----

02780> ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
 02781> (ha) (cms) (hrs) (mm) (cms)
 02782> ID1 10:HIP-PO 77.26 1.004 3.08 45.59 .000
 02783> +ID2 01:A3 2.70 .079 2.00 21.44 .000
 02784> SUM 02:Ultima 79.96 1.051 3.01 44.78 .000

02785> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

02786> 001:0026-----

02787> ***** CALCULATION OF 3HR - 1:50 YEAR STORM EVENT *****
 02788> *
 02789> *****

02790> START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
 02791> Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\

02792> TZERO = .00 hrs on 0
 02793> METOUT= 2 (output = METRIC)
 02794> NRUN = 001
 02795> NSTORM= 0

02796> 001:0002-----

02797> CHICAGO STORM | IDF curve parameters: A=1569.580

02836> | Total= 64.81 mm | B= 6.014
 02837> C= 820
 02838> used in: INTENSITY = A / (t + B)^C

02839> Duration of storm = 3.00 hrs
 02840> Storm time step = 10.00 min
 02841> Time to peak ratio = .33

02842> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
 02843> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
 02844> .17 5.467 | 1.00 161.471 | 1.83 10.000 | 2.67 5.209
 02845> .33 6.820 | 1.17 48.876 | 2.00 8.397 | 2.83 4.774
 02846> .50 9.187 | 1.33 24.704 | 2.17 7.256 | 3.00 4.412
 02847> .67 14.441 | 1.50 16.495 | 2.33 6.403 |
 02848> .83 36.764 | 1.67 12.422 | 2.50 5.740 |

02849> *****
 02850> 001:0003-----

02851> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORCA.VAL
 02852> ICASRev = 1 (read and print data)
 02853> FileTitle= ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ---
 02854> PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 ----

02855> Horton's infiltration equation parameters:
 02856> [P= 50.00 mm/hr] [Fc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
 02857> Parameters for PERVIOUS surfaces in STANDHYD:
 02858> [Iaper= 4.67 mm] [LGP=40.00 mm] [MNP= .250]
 02859> Parameters for IMPERVIOUS surfaces in STANDHYD:
 02860> [Iaimp= 1.57 mm] [CLI= 1.50] [MNI= .035]
 02861> Parameters used in NASHYD:
 02862> [Ia= 4.67 mm] [N= 3.00]

02863> 001:0004-----

02864> ***** ORGAWORLD FILE *****
 02865> * SUB-AREA No.1

02866> CALIB STANDHYD | Area (ha)= 2.07
 02867> 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

02868> IMPERVIOUS PERVIOUS (i)
 02869> Surface Area (ha)= 1.74 .33
 02870> Dep. Storage (mm)= 1.57 4.67
 02871> Average Slope (%)= .52 1.00
 02872> Length (m)= 204.72 20.00
 02873> Mannings n = .030 .250

02874> Max. eff. Inten. (mm/hr)= 161.47 62.27
 02875> over (min)= 7.50 12.50
 02876> Storage Coeff. (min)= 6.51 (ii) 13.44 (ii)
 02877> Unit Hyd. Tpeak (min)= 7.50 12.50
 02878> Unit Hyd. peak (cms)= .16 .09

02879> PEAK FLOW (cms)= .59 .03 *TOTALS*
 02880> TIME TO PEAK (hrs)= 1.04 1.17 1.609 (iii)
 02881> RUNOFF VOLUME (mm)= 63.24 30.21 57.952
 02882> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
 02883> RUNOFF COEFFICIENT = .98 .47 .894

02884> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 02885> CN* = 81.0 Ia = Dep. Storage (Above)
 02886> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 02887> THAN THE STORAGE COEFFICIENT.
 02888> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02889> 001:0005-----

02890> *SUB-AREA No.2

02891> CALIB STANDHYD | Area (ha)= 1.54
 02892> 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00

02893> IMPERVIOUS PERVIOUS (i)
 02894> Surface Area (ha)= 1.42 .12
 02895> Dep. Storage (mm)= 1.57 4.67
 02896> Average Slope (%)= .50 1.00
 02897> Length (m)= 244.34 5.00
 02898> Mannings n = .030 .030

02899> Max. eff. Inten. (mm/hr)= 161.47 78.73
 02900> over (min)= 7.50 7.50
 02901> Storage Coeff. (min)= 7.33 (ii) 8.10 (ii)
 02902> Unit Hyd. Tpeak (min)= 7.50 7.50
 02903> Unit Hyd. peak (cms)= .15 .14

02904> PEAK FLOW (cms)= .46 .02 *TOTALS*
 02905> TIME TO PEAK (hrs)= 1.04 1.08 1.042
 02906> RUNOFF VOLUME (mm)= 63.24 30.21 60.594
 02907> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
 02908> RUNOFF COEFFICIENT = .98 .47 .935

02909> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 02910> CN* = 81.0 Ia = Dep. Storage (Above)
 02911> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 02912> THAN THE STORAGE COEFFICIENT.
 02913> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02914> 001:0006-----

02915> *SUB-AREA No.3

02916> CALIB STANDHYD | Area (ha)= 1.40
 02917> 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02918> IMPERVIOUS PERVIOUS (i)
 02919> Surface Area (ha)= 1.36 .04
 02920> Dep. Storage (mm)= 1.57 4.67
 02921> Average Slope (%)= .51 1.00
 02922> Length (m)= 225.63 5.00
 02923> Mannings n = .030 .030

02924> Max. eff. Inten. (mm/hr)= 161.47 78.73
 02925> over (min)= 7.50 7.50
 02926> Storage Coeff. (min)= 6.95 (ii) 7.72 (ii)
 02927> Unit Hyd. Tpeak (min)= 7.50 7.50
 02928> Unit Hyd. peak (cms)= .16 .15

02929> PEAK FLOW (cms)= .45 .01 *TOTALS*
 02930> TIME TO PEAK (hrs)= 1.04 1.08 1.042
 02931> RUNOFF VOLUME (mm)= 63.24 30.21 62.245
 02932> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
 02933> RUNOFF COEFFICIENT = .98 .47 .960

02934> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 02935> CN* = 81.0 Ia = Dep. Storage (Above)
 02936> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 02937> THAN THE STORAGE COEFFICIENT.
 02938> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02939> 001:0007-----

02940> *SUB-AREA No.4

02941> CALIB STANDHYD | Area (ha)= 1.40
 02942> 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00

02943> IMPERVIOUS PERVIOUS (i)
 02944> Surface Area (ha)= 1.36 .04
 02945> Dep. Storage (mm)= 1.57 4.67
 02946> Average Slope (%)= .51 1.00
 02947> Length (m)= 225.63 5.00
 02948> Mannings n = .030 .030

02949> Max. eff. Inten. (mm/hr)= 161.47 78.73
 02950> over (min)= 7.50 7.50
 02951> Storage Coeff. (min)= 6.95 (ii) 7.72 (ii)
 02952> Unit Hyd. Tpeak (min)= 7.50 7.50
 02953> Unit Hyd. peak (cms)= .16 .15

02954> PEAK FLOW (cms)= .45 .01 *TOTALS*
 02955> TIME TO PEAK (hrs)= 1.04 1.08 1.042
 02956> RUNOFF VOLUME (mm)= 63.24 30.21 62.245
 02957> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
 02958> RUNOFF COEFFICIENT = .98 .47 .960

02959> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 02960> CN* = 81.0 Ia = Dep. Storage (Above)
 02961> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 02962> THAN THE STORAGE COEFFICIENT.
 02963> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

02964> 001:0007-----

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02971 |
02972 | ADD HYD (040 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02973 | ID1 03:010 (ha) (cms) (hrs) (mm) (mm) (cms)
02974 | +ID2 02:020 1.54 .475 1.04 60.59 .000
02975 |
02976 | SUM 04:040 3.61 1.084 1.04 59.08 .000
02977 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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02981 |
02982 | 001:0008 |
02983 |
02984 | ADD HYD (050 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02985 | ID1 03:030 (ha) (cms) (hrs) (mm) (mm) (cms)
02986 | +ID2 04:040 3.61 1.084 1.04 59.08 .000
02987 |
02988 | SUM 05:050 5.01 1.538 1.04 59.96 .000
02989 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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02991 |
02992 | 001:0009 |
02993 |
02994 | * SUB-AREA No.4
02995 | CALIB STANDHYD | Area (ha)= .89
02996 | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
02997 |

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03001 | IMPERVIOUS PERVIOUS (i)
03002 | Surface Area (ha)= .86 .03
03003 | Dep. Storage (mm)= 1.57 4.67
03004 | Average Slope (ft)= .93 .70
03005 | Length (m)= 164.82 42.00
03006 | Mannings n = .030 .250
03007 |
03008 | Max.eff.Inten.(mm/hr)= 161.47 53.28
03009 | over (min)= 5.00 17.50
03010 | Storage Coeff. (min)= 4.80 (ii) 17.24 (ii)
03011 | Unit Hyd. Tpeak (min)= 5.00 17.50
03012 | Unit Hyd. peak (cms)= .23 .07
03013 |
03014 | PEAK FLOW (cms)= .33 .00 *TOTALS*
03015 | TIME TO PEAK (hrs)= 1.00 1.25 (iii)
03016 | RUNOFF VOLUME (mm)= 63.24 30.21 62.245
03017 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03018 | RUNOFF COEFFICIENT = .98 .47 .960
03019 |
03020 | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03021 | CN* = 81.0 Ia = Dep. Storage (Above)
03022 | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03023 | THAN THE STORAGE COEFFICIENT.
03024 | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03025 |

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03027 | 001:0010 |
03028 |
03029 | * SUB-AREA No.5
03030 | CALIB STANDHYD | Area (ha)= 2.66
03031 | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
03032 |

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03033 | IMPERVIOUS PERVIOUS (i)
03034 | Surface Area (ha)= 2.58 .08
03035 | Dep. Storage (mm)= 1.57 4.67
03036 | Average Slope (ft)= .61 1.50
03037 | Length (m)= 207.25 20.00
03038 | Mannings n = .030 .250
03039 |
03040 | Max.eff.Inten.(mm/hr)= 161.47 62.27
03041 | over (min)= 7.50 12.50
03042 | Storage Coeff. (min)= 6.26 (ii) 12.39 (ii)
03043 | Unit Hyd. Tpeak (min)= 7.50 12.50
03044 | Unit Hyd. peak (cms)= .17 .09
03045 |
03046 | PEAK FLOW (cms)= .88 .01 *TOTALS*
03047 | TIME TO PEAK (hrs)= 1.04 1.17 1.042
03048 | RUNOFF VOLUME (mm)= 63.24 30.21 62.245
03049 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03050 | RUNOFF COEFFICIENT = .98 .47 .960
03051 |

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03052 | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03053 | CN* = 81.0 Ia = Dep. Storage (Above)
03054 | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03055 | THAN THE STORAGE COEFFICIENT.
03056 | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03057 |

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03059 |
03060 | 001:0011 |
03061 |
03062 | ADD HYD (080 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03063 | ID1 06:060 (ha) (cms) (hrs) (mm) (mm) (cms)
03064 | +ID2 07:070 2.66 .886 1.04 62.25 .000
03065 |
03066 | SUM 08:080 3.55 1.197 1.04 62.25 .000
03067 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03071 |
03072 | 001:0012 |
03073 |
03074 | ADD HYD (090 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03075 | ID1 05:050 (ha) (cms) (hrs) (mm) (mm) (cms)
03076 | +ID2 08:080 3.55 1.197 1.04 62.25 .000
03077 |
03078 | SUM 09:090 8.56 2.735 1.04 60.91 .000
03079 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03081 |
03082 | 001:0013 |
03083 |
03084 | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03085 | IN>09: (090 ) |
03086 | OUT<10: (POND ) |

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03087 |
03088 |
03089 |
03090 |
03091 |
03092 |
03093 |
03094 |
03095 |
03096 |
03097 |
03098 |
03099 |

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03100 | ROUTING RESULTS AREA QPEAK TPEAK R.V.
03101 | (ha) (cms) (hrs) (mm)
03102 | INFLOW >09: (090 ) 8.56 2.735 1.042 60.910
03103 | OUTFLOW <10: (POND ) 8.56 .233 1.944 60.908
03104 |
03105 | PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.503

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TIME SHIFT OF PEAK FLOW (min)= 54.17
MAXIMUM STORAGE USED (ha.m.)= .3967E+00

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03106 |
03107 |
03108 |
03109 |
03110 | 001:0014 |
03111 |
03112 | * Remaining Hawthorne Industrial Park *
03113 |
03114 |
03115 | * SUB-AREA No.1
03116 |
03117 | CALIB STANDHYD | Area (ha)= 19.90
03118 | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03119 |

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03120 | IMPERVIOUS PERVIOUS (i)
03121 | Surface Area (ha)= 14.13 5.77
03122 | Dep. Storage (mm)= 1.57 4.67
03123 | Average Slope (ft)= .60 1.50
03124 | Length (m)= 580.00 100.00
03125 | Mannings n = .030 .250
03126 |
03127 | Max.eff.Inten.(mm/hr)= 138.95 102.13
03128 | over (min)= 12.50 25.00
03129 | Storage Coeff. (min)= 12.38 (ii) 25.60 (ii)
03130 | Unit Hyd. Tpeak (min)= 12.50 25.00
03131 | Unit Hyd. peak (cms)= .09 .04
03132 |
03133 | PEAK FLOW (cms)= 2.46 .95 *TOTALS*
03134 | TIME TO PEAK (hrs)= 1.13 1.38 3.001 (iii)
03135 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03136 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03137 | RUNOFF COEFFICIENT = .98 .62 .796
03138 |

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03139 | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03140 | CN* = 81.0 Ia = Dep. Storage (Above)
03141 | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03142 | THAN THE STORAGE COEFFICIENT.
03143 | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03144 |

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03145 | 001:0015 |
03146 |
03147 |
03148 | ADD HYD (HIP02 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03149 | ID1 10:POND (ha) (cms) (hrs) (mm) (mm) (cms)
03150 | +ID2 01:HIP01 8.56 .233 1.94 60.91 .000
03151 |
03152 | SUM 02:HIP02 28.46 3.092 1.17 54.37 .000
03153 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03154 |
03155 | 001:0016 |
03156 |
03157 | * SUB-AREA No.2
03158 | CALIB STANDHYD | Area (ha)= 17.00
03159 | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03160 |

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03161 | IMPERVIOUS PERVIOUS (i)
03162 | Surface Area (ha)= 12.07 4.93
03163 | Dep. Storage (mm)= 1.57 4.67
03164 | Average Slope (ft)= .65 1.50
03165 | Length (m)= 450.00 100.00
03166 | Mannings n = .030 .250
03167 |
03168 | Max.eff.Inten.(mm/hr)= 161.47 109.61
03169 | over (min)= 10.00 22.50
03170 | Storage Coeff. (min)= 9.77 (ii) 22.63 (ii)
03171 | Unit Hyd. Tpeak (min)= 10.00 22.50
03172 | Unit Hyd. peak (cms)= .11 .05
03173 |
03174 | PEAK FLOW (cms)= 2.38 .88 *TOTALS*
03175 | TIME TO PEAK (hrs)= 1.08 1.33 2.819 (iii)
03176 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03177 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03178 | RUNOFF COEFFICIENT = .98 .62 .796
03179 |

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03180 | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03181 | CN* = 81.0 Ia = Dep. Storage (Above)
03182 | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03183 | THAN THE STORAGE COEFFICIENT.
03184 | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03185 |

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03186 | 001:0017 |
03187 |
03188 | * SUB-AREA No.3
03189 | CALIB STANDHYD | Area (ha)= 15.60
03190 | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
03191 |

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03192 | IMPERVIOUS PERVIOUS (i)
03193 | Surface Area (ha)= 11.08 4.52
03194 | Dep. Storage (mm)= 1.57 4.67
03195 | Average Slope (ft)= .50 1.50
03196 | Length (m)= 600.00 100.00
03197 | Mannings n = .030 .250
03198 |
03199 | Max.eff.Inten.(mm/hr)= 138.95 96.02
03200 | over (min)= 12.50 27.50
03201 | Storage Coeff. (min)= 13.34 (ii) 26.90 (ii)
03202 | Unit Hyd. Tpeak (min)= 12.50 27.50
03203 | Unit Hyd. peak (cms)= .09 .04
03204 |
03205 | PEAK FLOW (cms)= 1.86 .72 *TOTALS*
03206 | TIME TO PEAK (hrs)= 1.13 1.42 2.237 (iii)
03207 | RUNOFF VOLUME (mm)= 63.24 39.90 51.566
03208 | TOTAL RAINFALL (mm)= 64.81 64.81 64.806
03209 | RUNOFF COEFFICIENT = .98 .62 .796
03210 |

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03211 | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03212 | CN* = 81.0 Ia = Dep. Storage (Above)
03213 | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03214 | THAN THE STORAGE COEFFICIENT.
03215 | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03216 |

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03217 | 001:0018 |
03218 |
03219 | ADD HYD (HIP05 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03220 | ID1 03:HIP03 (ha) (cms) (hrs) (mm) (mm) (cms)
03221 | +ID2 04:HIP04 15.60 2.237 1.17 51.57 .000
03222 |
03223 | SUM 05:HIP05 32.60 5.019 1.13 51.57 .000
03224 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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03225 |
03226 | 001:0019 |
03227 |
03228 | ADD HYD (HIP06 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03229 | ID1 05:HIP05 (ha) (cms) (hrs) (mm) (mm) (cms)
03230 | SUM 05:HIP05 32.60 5.019 1.13 51.57 .000
03231 |
03232 |
03233 |
03234 |
03235 |

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03241> +ID2 02:HIF02 28.46 3.092 1.17 54.37 .000
 03242> SUM 06:HIF06 61.06 8.054 1.13 52.87 .000
 03243>
 03244>
 03245> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03246>
 03247>

03248> 001:0020-----
 03249> *
 03250> * SUB-AREA No. 4
 03251>

03252> | CALIB STANDHYD | Area (ha)= 12.20
 03253> | 07:HIF07 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 03254>
 03255>

03256> IMPERVIOUS PERVIOUS (i)
 03257> Surface Area (ha)= 8.66 3.54
 03258> Dep. Storage (mm)= 1.57 4.67
 03259> Average Slope (%)= .70 1.50
 03260> Length (m)= 210.00 100.00
 03261> Mannings n = .030 .250

03262> Max. eff. Inten. (mm/hr)= 161.47 126.32
 03263> over (min)= 5.00 17.50
 03264> Storage Coeff. (min)= 6.05 (ii) 18.19 (ii)
 03265> Unit Hyd. Tpeak (min)= 5.00 17.50
 03266> Unit Hyd. peak (cms)= .20 .06

03267> *TOTALS*
 03268> PEAK FLOW (cms)= 2.19 .73 2.470 (iii)
 03269> TIME TO PEAK (hrs)= 1.00 1.25 1.042
 03270> RUNOFF VOLUME (mm)= 63.24 39.90 51.566
 03271> TOTAL RAINFALL (mm)= 64.81 64.81 64.806
 03272> RUNOFF COEFFICIENT = .98 .62 .796

03273> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03274> CN* = 81.0 Ia = Dep. Storage (Above)
 03275> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03276> THAN THE STORAGE COEFFICIENT.
 03277> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03278>
 03279>

03280> 001:0021-----
 03281> *
 03282> * SUB-AREA No. 5
 03283>

03284> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
 03285> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
 03286> U.H. Tp(hrs)= .170
 03287>
 03288>

03289> Unit Hyd Qpeak (cms)= .899
 03290>
 03291> PEAK FLOW (cms)= .551 (i)
 03292> TIME TO PEAK (hrs)= 1.125
 03293> RUNOFF VOLUME (mm)= 34.455
 03294> TOTAL RAINFALL (mm)= 64.806
 03295> RUNOFF COEFFICIENT = .532

03296> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03297>
 03298>
 03299>
 03300> 001:0022-----

03301> | ADD HYD (HIF08) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
 03302> (ha) (cms) (hrs) (mm) (cms)
 03303> ID1 06:HIF06 61.06 8.054 1.13 52.87 .000
 03304> +ID2 07:HIF07 12.20 2.470 1.04 51.57 .000
 03305> +ID3 08:Pond-B 4.00 .551 1.13 34.45 .000
 03306> SUM 09:HIF08 77.26 10.570 1.13 51.71 .000
 03307>
 03308>
 03309>
 03310> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03311>
 03312>

03313> 001:0023-----
 03314> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
 03315> | IN>09: (HIF08) |
 03316> | OUT<10: (HIF-PO) |

03317> ===== OUTFLOW STORAGE TABLE =====
 03318> OUTFLOW STORAGE | OUTFLOW STORAGE
 03319> (cms) (ha.m.) | (cms) (ha.m.)
 03320> .000 .0000E+00 | .724 .2210E+01
 03321> .048 .5740E-01 | .937 .2501E+01
 03322> .054 .2434E+00 | 1.262 .2798E+01
 03323> .059 .5834E+00 | 1.404 .3101E+01
 03324> .062 .8400E+00 | 1.532 .3410E+01
 03325> .064 .1102E+01 | 1.650 .3724E+01
 03326> .147 .1370E+01 | 2.409 .4044E+01
 03327> .280 .1644E+01 | 3.689 .4370E+01
 03328> .472 .1924E+01 | .000 .0000E+00

03329> ROUTING RESULTS AREA OPEAK TPEAK R.V.
 03330> (ha) (cms) (hrs) (mm)
 03331> INFLOW >09: (HIF08) 77.26 10.570 1.125 51.714
 03332> OUTFLOW<10: (HIF-PO) 77.26 1.280 2.917 51.714

03333> PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.106
 03334> TIME SHIFT OF PEAK FLOW (min) = 107.50
 03335> MAXIMUM STORAGE USED (ha.m.) = .2836E+01
 03336>
 03337>
 03338>

03339> 001:0024-----
 03340> *
 03341> * SUB-AREA No. 6
 03342>

03343> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
 03344> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
 03345> U.H. Tp(hrs)= .800
 03346>
 03347>

03348> Unit Hyd Qpeak (cms)= .129
 03349>
 03350> PEAK FLOW (cms)= .096 (i)
 03351> TIME TO PEAK (hrs)= 1.958
 03352> RUNOFF VOLUME (mm)= 25.767
 03353> TOTAL RAINFALL (mm)= 64.806
 03354> RUNOFF COEFFICIENT = .398

03355> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03356>
 03357>
 03358>
 03359> 001:0025-----

03360> | ADD HYD (Ultima) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
 03361> (ha) (cms) (hrs) (mm) (cms)
 03362> ID1 10:HIF-PO 77.26 1.280 2.92 51.71 .000
 03363> +ID2 01:A3 2.70 .096 1.96 25.77 .000
 03364> SUM 02:Ultima 79.96 1.348 2.63 50.84 .000
 03365>
 03366>
 03367>

03368> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03369>
 03370>
 03371> 001:0026-----

03372> *****
 03373> * CALCULATION OF 3HR - 1:100 YEAR STORM EVENT *
 03374> *****
 03375>

03376> | START | Project dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
 03377> Rainfall dir.: V:\20983.DU\ENG\FINALS-1\SWHYM-1\
 03378> TEERO = .00 hrs on 0
 03379> METOUT= 2 (output = METRIC)
 03380> NRUM = 001
 03381> NSTORM= 0
 03382>
 03383> 001:0002-----

03384> | CHICAGO STORM | IDF curve parameters: A=1735.688
 03385> | Total= 71.66 mm | B= 6.014
 03386> C= .820
 03387> used in: INTENSITY = A / (t + B)^C
 03388>
 03389>

03390> Duration of storm = 3.00 hrs
 03391> Storm time step = 10.00 min
 03392> Time to peak ratio = .33
 03393>

03394> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
 03395> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
 03396> .17 6.04E-01 | 1.00 178.559 | 1.83 11.059 | 1.27 5.700
 03397> .33 7.542 | 1.17 54.049 | 2.00 9.285 | 2.83 5.280
 03398> .50 10.159 | 1.33 27.319 | 2.17 8.024 | 3.00 4.879
 03399> .67 15.969 | 1.50 18.240 | 2.33 7.080 |
 03400> .83 40.655 | 1.67 13.737 | 2.50 6.347 |

03401>
 03402> 001:0003-----
 03403>
 03404> | DEFAULT VALUES | Filename: V:\20983.DU\ENG\FINALS-1\SWHYM-1\ORGA.VAL
 03405> | CASEADV = (read and print data)
 03406> FileTitle= ----- ENTER YOUR COMMENTS ON THIS LINE AND THE NEXT ONE ----
 03407> ----- PARAMETER VALUES MUST BE ENTERED AFTER COLUMN 60 -----
 03408>
 03409> Horton's infiltration equation parameters:
 03410> [Pw= 50.00 mm/hr] [Pc= 7.50 mm/hr] [DCAY= 2.00 /hr] [P= .00 mm]
 03411> Parameters for PERVIOUS surfaces in STANDHYD:
 03412> [IAPER= 4.67 mm] [LGP=40.00 m] [MNP= .250]
 03413> Parameters for IMPERVIOUS surfaces in STANDHYD:
 03414> [IAlmp= 1.57 mm] [CLI= 1.50] [MNI= .035]
 03415> Parameters used in NASHYD:
 03416> [Ia= 4.67 mm] [N= 3.00]
 03417>

03418> 001:0004-----
 03419> *****
 03420> ORGAWORLD FILE *
 03421> *****
 03422> *

03423> * SUB-AREA No. 1
 03424>
 03425> | CALIB STANDHYD | Area (ha)= 2.07
 03426> | 01:010 DT= 2.50 | Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
 03427>
 03428>

03429> IMPERVIOUS PERVIOUS (i)
 03430> Surface Area (ha)= 1.74 .33
 03431> Dep. Storage (mm)= 1.57 4.67
 03432> Average Slope (%)= .52 1.00
 03433> Length (m)= 204.72 20.00
 03434> Mannings n = .030 .250

03435> Max. eff. Inten. (mm/hr)= 178.56 74.05
 03436> over (min)= 7.50 12.50
 03437> Storage Coeff. (min)= 6.26 (ii) 12.72 (ii)
 03438> Unit Hyd. Tpeak (min)= 7.50 12.50
 03439> Unit Hyd. peak (cms)= .17 .09

03440> *TOTALS*
 03441> PEAK FLOW (cms)= .66 .04
 03442> TIME TO PEAK (hrs)= 1.04 1.17 1.042
 03443> RUNOFF VOLUME (mm)= 70.09 35.46 64.553
 03444> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03445> RUNOFF COEFFICIENT = .98 .49 .901

03446> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03447> CN* = 81.0 Ia = Dep. Storage (Above)
 03448> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03449> THAN THE STORAGE COEFFICIENT.
 03450> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03451>
 03452>

03453> 001:0005-----
 03454> *
 03455> * SUB-AREA No. 2
 03456>

03457> | CALIB STANDHYD | Area (ha)= 1.54
 03458> | 02:020 DT= 2.50 | Total Imp(%)= 92.00 Dir. Conn.(%)= 92.00
 03459>
 03460>

03461> IMPERVIOUS PERVIOUS (i)
 03462> Surface Area (ha)= 1.42 .12
 03463> Dep. Storage (mm)= 1.57 4.67
 03464> Average Slope (%)= .50 1.00
 03465> Length (m)= 244.34 5.00
 03466> Mannings n = .030 .030

03467> Max. eff. Inten. (mm/hr)= 178.56 93.23
 03468> over (min)= 7.50 7.50
 03469> Storage Coeff. (min)= 7.04 (ii) 7.76 (ii)
 03470> Unit Hyd. Tpeak (min)= 7.50 7.50
 03471> Unit Hyd. peak (cms)= .16 .15

03472> *TOTALS*
 03473> PEAK FLOW (cms)= .51 .02 .534 (iii)
 03474> TIME TO PEAK (hrs)= 1.04 1.08 1.042
 03475> RUNOFF VOLUME (mm)= 70.09 35.46 67.324
 03476> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03477> RUNOFF COEFFICIENT = .98 .49 .939

03478> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03479> CN* = 81.0 Ia = Dep. Storage (Above)
 03480> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03481> THAN THE STORAGE COEFFICIENT.
 03482> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03483>
 03484>

03485> 001:0006-----
 03486> *
 03487> * SUB-AREA No. 3
 03488>

03489> | CALIB STANDHYD | Area (ha)= 1.40
 03490> | 03:030 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
 03491>
 03492>

03493> IMPERVIOUS PERVIOUS (i)
 03494> Surface Area (ha)= 1.36 .04
 03495> Dep. Storage (mm)= 1.57 4.67
 03496> Average Slope (%)= .51 1.00
 03497> Length (m)= 225.63 5.00
 03498> Mannings n = .030 .030

03499> Max. eff. Inten. (mm/hr)= 178.56 93.23
 03500> over (min)= 7.50 7.50
 03501> Storage Coeff. (min)= 6.67 (ii) 7.39 (ii)
 03502> Unit Hyd. Tpeak (min)= 7.50 7.50
 03503> Unit Hyd. peak (cms)= .16 .15

03504> *TOTALS*
 03505> PEAK FLOW (cms)= .50 .01 .509 (iii)
 03506> TIME TO PEAK (hrs)= 1.04 1.08 1.042
 03507> RUNOFF VOLUME (mm)= 70.09 35.46 69.056
 03508> TOTAL RAINFALL (mm)= 71.66 71.66 71.665

03511> RUNOFF COEFFICIENT = .98 .49 .964
 03512>
 03513> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03514> CN* = 81.0 Ia = Dep. Storage (Above)
 03515> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03516> THAN THE STORAGE COEFFICIENT.
 03517> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03518>
 03519>
 03520> 001:0007-----
 03521>
 03522> | ADD HYD (040) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 03523> | | (ha) (cms) (hrs) (mm) (cms)
 03524> | ID1 01:010 2.07 .685 1.04 64.55 .000
 03525> | +ID2 02:020 1.54 .534 1.04 67.32 .000
 03526> |-----
 03527> | SUM 04:040 3.61 1.220 1.04 65.74 .000
 03528>
 03529> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03530>
 03531>
 03532> 001:0008-----
 03533>
 03534> | ADD HYD (050) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 03535> | | (ha) (cms) (hrs) (mm) (cms)
 03536> | ID1 03:030 1.40 .509 1.04 69.06 .000
 03537> | +ID2 04:040 3.61 1.220 1.04 65.74 .000
 03538> |-----
 03539> | SUM 05:050 5.01 1.729 1.04 66.66 .000
 03540>
 03541> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03542>
 03543>
 03544> 001:0009-----
 03545> * SUB-AREA No.4
 03546> | CALIB STANDHYD | Area (ha)= .89
 03547> | 06:060 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
 03548> |-----
 03549> | IMPERVIOUS PERVIOUS (i)
 03550> | Surface Area (ha)= .86 .03
 03551> | Dep. Storage (mm)= 1.57 4.67
 03552> | Average Slope (%)= .93 1.50
 03553> | Length (m)= 164.82 40.00
 03554> | Mannings n = .030 .250
 03555> |-----
 03556> | Max. eff. Inten. (mm/hr)= 178.56 67.61
 03557> | over (min)= 5.00 15.00
 03558> | Storage Coeff. (min)= 4.62 (ii) 15.92 (ii)
 03559> | Unit Hyd. Tpeak (min)= 5.00 15.00
 03560> | Unit Hyd. peak (cms)= .24 .07
 03561> |-----
 03562> | PEAK FLOW (cms)= .37 .00 *TOTALS*
 03563> | TIME TO PEAK (hrs)= 1.00 1.21 1.000
 03564> | RUNOFF VOLUME (mm)= 70.09 35.46 69.056
 03565> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03566> | RUNOFF COEFFICIENT = .98 .49 .964
 03567>
 03568> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03569> CN* = 81.0 Ia = Dep. Storage (Above)
 03570> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03571> THAN THE STORAGE COEFFICIENT.
 03572> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03573>
 03574>
 03575>
 03576>
 03577> 001:0010-----
 03578> * SUB-AREA No.5
 03579> | CALIB STANDHYD | Area (ha)= 2.66
 03580> | 07:070 DT= 2.50 | Total Imp(%)= 97.00 Dir. Conn.(%)= 97.00
 03581> |-----
 03582> | IMPERVIOUS PERVIOUS (i)
 03583> | Surface Area (ha)= 2.58 .08
 03584> | Dep. Storage (mm)= 1.57 4.67
 03585> | Average Slope (%)= .61 1.50
 03586> | Length (m)= 207.25 20.00
 03587> | Mannings n = .030 .250
 03588> |-----
 03589> | Max. eff. Inten. (mm/hr)= 178.56 74.05
 03590> | over (min)= 5.00 12.50
 03591> | Storage Coeff. (min)= 6.01 (ii) 11.73 (ii)
 03592> | Unit Hyd. Tpeak (min)= 5.00 12.50
 03593> | Unit Hyd. peak (cms)= .20 .09
 03594> |-----
 03595> | PEAK FLOW (cms)= 1.03 .01 *TOTALS*
 03596> | TIME TO PEAK (hrs)= 1.00 1.17 1.000
 03597> | RUNOFF VOLUME (mm)= 70.09 35.46 69.056
 03598> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03599> | RUNOFF COEFFICIENT = .98 .49 .964
 03600>
 03601> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03602> CN* = 81.0 Ia = Dep. Storage (Above)
 03603> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03604> THAN THE STORAGE COEFFICIENT.
 03605> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03606>
 03607>
 03608>
 03609>
 03610> 001:0011-----
 03611>
 03612> | ADD HYD (080) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 03613> | | (ha) (cms) (hrs) (mm) (cms)
 03614> | ID1 06:060 2.89 .374 1.00 69.06 .000
 03615> | +ID2 07:070 2.66 1.034 1.00 69.06 .000
 03616> |-----
 03617> | SUM 08:080 3.55 1.408 1.00 69.06 .000
 03618>
 03619> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03620>
 03621>
 03622> 001:0012-----
 03623>
 03624> | ADD HYD (090) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 03625> | | (ha) (cms) (hrs) (mm) (cms)
 03626> | ID1 05:050 5.01 1.729 1.04 66.66 .000
 03627> | +ID2 08:080 3.55 1.408 1.00 69.06 .000
 03628> |-----
 03629> | SUM 09:090 8.56 3.067 1.04 67.66 .000
 03630>
 03631> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03632>
 03633>
 03634> 001:0013-----
 03635>
 03636> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
 03637> | IN>09: (090) |
 03638> | OUT<10: (POND) |
 03639> |-----
 03640> | OUTFLOW STORAGE | OUTFLOW STORAGE
 03641> | (cms) (ha.m.) | (cms) (ha.m.)
 03642> | .000 .0000E+00 | .593 .6281E+00
 03643> | .008 .6500E-01 | .654 .6631E+00
 03644> | .017 .1311E+00 | .797 .7391E+00
 03645> | .093 .2831E+00 | .950 .8274E+00
 03646> | .233 .3971E+00 | 1.304 .9157E+00

03646> .337 .4731E+00 | 1.880 .1004E+01
 03647> .465 .5491E+00 | 2.577 .1092E+01
 03648> .531 .5871E+00 | .000 .0000E+00
 03649>
 03650> ROUTING RESULTS AREA QPEAK TPEAK R.V.
 03651> (ha) (cms) (hrs) (mm)
 03652> INFLOW >09: (090) 8.56 3.067 1.042 67.655
 03653> OUTFLOW<10: (POND) 8.56 .283 1.861 67.653
 03654>
 03655> PEAK FLOW REDUCTION [Out/In] (%) = 9.214
 03656> TIME SHIFT OF PEAK FLOW (min) = 49.17
 03657> MAXIMUM STORAGE USED (ha.m.) = .4333E+00
 03658>
 03659>
 03660> 001:0014-----
 03661> *****
 03662> * Remaining Hawthorne Industrial Park *
 03663> *****
 03664> * SUB-AREA No.1
 03665> | CALIB STANDHYD | Area (ha)= 19.90
 03666> | 01:HIP01 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 03667> |-----
 03668> | IMPERVIOUS PERVIOUS (i)
 03669> | Surface Area (ha)= 14.13 5.77
 03670> | Dep. Storage (mm)= 1.57 4.67
 03671> | Average Slope (%)= .60 1.50
 03672> | Length (m)= 580.00 100.00
 03673> | Mannings n = .030 .250
 03674> |-----
 03675> | Max. eff. Inten. (mm/hr)= 153.66 117.89
 03676> | over (min)= 12.50 25.00
 03677> | Storage Coeff. (min)= 11.89 (ii) 24.37 (ii)
 03678> | Unit Hyd. Tpeak (min)= 12.50 25.00
 03679> | Unit Hyd. peak (cms)= .09 .05
 03680> |-----
 03681> | PEAK FLOW (cms)= 2.77 1.13 *TOTALS*
 03682> | TIME TO PEAK (hrs)= 1.13 1.38 3.419 (iii)
 03683> | RUNOFF VOLUME (mm)= 70.09 45.94 58.015
 03684> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03685> | RUNOFF COEFFICIENT = .98 .64 .810
 03686>
 03687> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03688> CN* = 81.0 Ia = Dep. Storage (Above)
 03689> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03690> THAN THE STORAGE COEFFICIENT.
 03691> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03692>
 03693>
 03694>
 03695> 001:0015-----
 03696>
 03697> | ADD HYD (HIP02) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 03698> | | (ha) (cms) (hrs) (mm) (cms)
 03699> | ID1 10:POND 8.56 .283 1.86 67.65 .000
 03700> | +ID2 01:HIP01 19.90 3.419 1.17 58.02 .000
 03701> |-----
 03702> | SUM 02:HIP02 28.46 3.554 1.17 60.91 .000
 03703>
 03704> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 03705>
 03706>
 03707>
 03708> 001:0016-----
 03709> * SUB-AREA No.2
 03710> | CALIB STANDHYD | Area (ha)= 17.00
 03711> | 03:HIP03 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 03712> |-----
 03713> | IMPERVIOUS PERVIOUS (i)
 03714> | Surface Area (ha)= 12.07 4.93
 03715> | Dep. Storage (mm)= 1.57 4.67
 03716> | Average Slope (%)= .65 1.50
 03717> | Length (m)= 450.00 100.00
 03718> | Mannings n = .030 .250
 03719> |-----
 03720> | Max. eff. Inten. (mm/hr)= 178.56 126.60
 03721> | over (min)= 10.00 22.50
 03722> | Storage Coeff. (min)= 9.39 (ii) 21.52 (ii)
 03723> | Unit Hyd. Tpeak (min)= 10.00 22.50
 03724> | Unit Hyd. peak (cms)= .12 .05
 03725> |-----
 03726> | PEAK FLOW (cms)= 2.68 1.05 *TOTALS*
 03727> | TIME TO PEAK (hrs)= 1.08 1.33 3.203 (iii)
 03728> | RUNOFF VOLUME (mm)= 70.09 45.94 58.015
 03729> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03730> | RUNOFF COEFFICIENT = .98 .64 .810
 03731>
 03732> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03733> CN* = 81.0 Ia = Dep. Storage (Above)
 03734> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03735> THAN THE STORAGE COEFFICIENT.
 03736> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03737>
 03738>
 03739>
 03740>
 03741> 001:0017-----
 03742> * SUB-AREA No.3
 03743> | CALIB STANDHYD | Area (ha)= 15.60
 03744> | 04:HIP04 DT= 2.50 | Total Imp(%)= 71.00 Dir. Conn.(%)= 50.00
 03745> |-----
 03746> | IMPERVIOUS PERVIOUS (i)
 03747> | Surface Area (ha)= 11.08 4.52
 03748> | Dep. Storage (mm)= 1.57 4.67
 03749> | Average Slope (%)= .50 1.50
 03750> | Length (m)= 600.00 100.00
 03751> | Mannings n = .030 .250
 03752> |-----
 03753> | Max. eff. Inten. (mm/hr)= 153.66 117.89
 03754> | over (min)= 12.50 25.00
 03755> | Storage Coeff. (min)= 12.82 (ii) 25.30 (ii)
 03756> | Unit Hyd. Tpeak (min)= 12.50 25.00
 03757> | Unit Hyd. peak (cms)= .09 .04
 03758> |-----
 03759> | PEAK FLOW (cms)= 2.10 .87 *TOTALS*
 03760> | TIME TO PEAK (hrs)= 1.13 1.38 3.203 (iii)
 03761> | RUNOFF VOLUME (mm)= 70.09 45.94 58.015
 03762> | TOTAL RAINFALL (mm)= 71.66 71.66 71.665
 03763> | RUNOFF COEFFICIENT = .98 .64 .810
 03764>
 03765> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 03766> CN* = 81.0 Ia = Dep. Storage (Above)
 03767> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 03768> THAN THE STORAGE COEFFICIENT.
 03769> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 03770>
 03771>
 03772>
 03773>
 03774> 001:0018-----
 03775> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 03776> | | (ha) (cms) (hrs) (mm) (cms)
 03777> | ID1 03:HIP03 17.00 3.203 1.13 58.02 .000
 03778> | +ID2 04:HIP04 15.60 2.612 1.17 58.02 .000
 03779> |-----
 03780> | SUM 05:HIP05 32.60 5.815 1.17 58.02 .000

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03781> SUM 05:HIP05 32.60 5.767 1.13 58.02 .000
03782>
03783> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03784>
03785>
03786> 001:0019-----
03787>
03788> | ADD HYD (HIP05) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03789> | (ha) (cms) (hrs) (mm) (cms)
03790> ID1 05:HIP05 32.60 5.767 1.13 58.02 .000
03791> +ID2 02:HIP02 28.46 3.554 1.17 60.91 .000
03792>
03793> SUM 06:HIP06 61.06 9.239 1.13 59.36 .000
03794>
03795> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03796>
03797>
03798> 001:0020-----
03799> *
03800> * SUB-AREA No. 4
03801>
03802> | CALIB STANDHYD | Area (ha)= 12.20
03803> | 07:HIP07 DT= 2.50 | Total Imp (%)= 71.00 Dir. Conn. (%)= 50.00
03804>
03805> IMPERVIOUS PERVIOUS (i)
03806> Surface Area (ha)= 8.66 3.54
03807> Dep. Storage (mm)= 1.57 4.67
03808> Average Slope (%)= .70 1.50
03809> Length (m)= 210.00 100.00
03810> Mannings n = .030 .250
03811>
03812> Max. eff. Inten. (mm/hr)= 178.56 146.17
03813> over (min)= 5.00 17.50
03814> Storage Coeff. (min)= 5.81 (ii) 17.27 (ii)
03815> Unit Hyd. Tpeak (min)= 5.00 17.50
03816> Unit Hyd. peak (cms)= .20 .07
03817>
03818> PEAK FLOW (cms)= 2.46 .87 *TOTALS*
03819> TIME TO PEAK (hrs)= 1.00 1.25 2.793 (iii)
03820> RUNOFF VOLUME (mm)= 70.09 45.94 58.015
03821> TOTAL RAINFALL (mm)= 71.66 71.66 71.665
03822> RUNOFF COEFFICIENT = .98 .64 .810
03823>
03824> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03825> CN* = 81.0 Ia = Dep. Storage (Above)
03826> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03827> THAN THE STORAGE COEFFICIENT.
03828> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03829>
03830>
03831> 001:0021-----
03832> *
03833> * SUB-AREA No. 5
03834>
03835> | DESIGN NASHYD | Area (ha)= 4.00 Curve Number (CN)=85.00
03836> | 08:Pond-B DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03837> | U.H. Tp (hrs)= .170
03838>
03839> Unit Hyd Qpeak (cms)= .899
03840>
03841> PEAK FLOW (cms)= .649 (i)
03842> TIME TO PEAK (hrs)= 1.125
03843> RUNOFF VOLUME (mm)= 40.139
03844> TOTAL RAINFALL (mm)= 71.665
03845> RUNOFF COEFFICIENT = .560
03846>
03847> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03848>
03849>
03850> 001:0022-----
03851>
03852> | ADD HYD (HIP08) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03853> | (ha) (cms) (hrs) (mm) (cms)
03854> ID1 06:HIP06 61.06 9.239 1.13 59.36 .000
03855> +ID2 07:HIP07 12.20 2.793 1.04 58.02 .000
03856> +ID3 08:Pond-B 4.00 .649 1.13 40.14 .000
03857>
03858> SUM 09:HIP08 77.26 12.109 1.13 58.16 .000
03859>
03860> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03861>
03862>
03863> 001:0023-----
03864>
03865> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03866> | IN>09: (HIP08) |
03867> | OUT<10: (HIP-PO) |
03868>
03869>
03870> ===== OUTFLOW STORAGE TABLE =====
03871> OUTFLOW STORAGE | OUTFLOW STORAGE
03872> (cms) (ha.m.) | (cms) (ha.m.)
03873> .000 .0000E+00 | .724 .2210E+01
03874> .048 .5740E-01 | .937 .2501E+01
03875> .054 .2434E+00 | 1.262 .2798E+01
03876> .059 .5834E+00 | 1.404 .3101E+01
03877> .062 .8400E+00 | 1.532 .3410E+01
03878> .064 .1102E+01 | 1.650 .3724E+01
03879> .147 .1370E+01 | 2.409 .4044E+01
03880> .280 .1644E+01 | 3.689 .4370E+01
03881> .472 .1924E+01 | .000 .0000E+00
03882>
03883> ROUTING RESULTS AREA QPEAK TPEAK R.V.
03884> (ha) (cms) (hrs) (mm)
03885> INFLOW >09: (HIP08) 77.26 12.109 1.125 58.156
03886> OUTFLOW <10: (HIP-PO) 77.26 1.432 2.889 58.156
03887>
03888> PEAK FLOW REDUCTION [Qout/Qin] (%)= 11.826
03889> TIME SHIFT OF PEAK FLOW (min)= 105.83
03890> MAXIMUM STORAGE USED (ha.m.)= .3168E+01
03891>
03892> 001:0024-----
03893> *
03894> * SUB-AREA No. 6
03895>
03896> | DESIGN NASHYD | Area (ha)= 2.70 Curve Number (CN)=76.00
03897> | 01:A3 DT= 2.50 | Ia (mm)= 4.670 # of Linear Res. (N)= 3.00
03898> | U.H. Tp (hrs)= .800
03899>
03900> Unit Hyd Qpeak (cms)= .129
03901>
03902> PEAK FLOW (cms)= .114 (i)
03903> TIME TO PEAK (hrs)= 1.958
03904> RUNOFF VOLUME (mm)= 30.490
03905> TOTAL RAINFALL (mm)= 71.665
03906> RUNOFF COEFFICIENT = .425
03907>
03908> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03909>
03910>
03911> 001:0025-----
03912>
03913> | ADD HYD (Ultima) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
03914> | (ha) (cms) (hrs) (mm) (cms)
03915> ID1 10:HIP-PO 77.26 1.432 2.89 58.16 .000
03916> +ID2 01:A3 2.70 .114 1.96 30.49 .000
03917>

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03916> SUM 02:Ultima 79.96 1.515 2.57 57.22 .000
03917>
03918> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03919>
03920>
03921> 001:0026-----
03922> FINISH
03923>
03924> *****
03925> WARNINGS / ERRORS / NOTES
03926>
03927> Simulation ended on 2009-05-15 at 08:45:24
03928>
03929>
03930>

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Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:10 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 0.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA								CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT						FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)				
			Area at C of		SUM(A)	SUM(A*C)	TOTAL A*C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D _{10yr} m	D _{max} m	SS X:1	SLOPE %	Q _{10yr} l/s	Q _{100yr} l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL	OUTLET CONTROL				HW 1:10 (m)			
	0.70 (ha)	0.90 (ha)																															
NORTHERN CATCHMENT AREA																																	
WEST SIDE SAPPERS RIDGE	2	3	1.86	0.18	2.04	1.46	1.46	4.07	4.07	15.00	97.85	398.2	0.00	0.42	1.20	3.00	0.50	424.2	6973.0	0.80	136.80							2.84	92.50	91.82			
WEST SIDE SAPPERS RIDGE	3	4	1.89	0.14	2.03	1.45	2.92	4.04	8.11	17.84	88.22	715.4	0.00	0.51	1.20	3.00	0.80	904.2	8856.1	1.16	111.00							1.60	91.82	90.93			
WEST SIDE SAPPERS RIDGE	4	5	1.76	0.15	1.91	1.36	4.28	3.79	11.90	19.44	83.68	995.9	0.00	0.58	1.20	3.00	0.51	1011.3	7029.1	1.00	112.85							1.88	90.93	90.36			
WEST SIDE SAPPERS RIDGE	5	6	2.43	0.11	2.54	1.80	6.08	5.00	16.90	21.32	78.96	1334.4	0.00	0.65	1.20	3.00	0.62	1513.4	7762.6	1.19	82.79							1.16	90.36	89.85			
										22.47																							
NORTH ENTRANCE TO SOMME STREET	8	6		0.03	0.03	0.03	0.03	0.08	0.08	15.00	97.85	7.3	0.00	0.20	1.20	3.00	1.30	94.9	11276.7	0.79	10.00							0.21	89.98	89.85			
										15.21																							
CULVERT CROSSING	6	14		0.00	0.00	0.00	6.11	0.00	16.97	22.47	76.34	1295.8					0.50				20.00	2	----	1.15 x 0.82	NO	YES	0.75	0.38	89.85	89.75			
										22.85																							
NORTH PORTION SOMME STREET	13	14	0.85	0.03	0.88	0.62	0.62	1.73	1.73	15.00	97.85	169.2	0.00	0.30	1.20	3.00	2.30	372.0	14999.4	1.38	10.00							0.12	89.98	89.75			
										15.12																							
NORTH PORTION SOMME STREET	14	15	2.93	0.24	3.17	2.27	8.99	6.30	25.00	22.85	75.52	1888.2	0.00	0.74	1.20	3.00	0.50	1926.6	6992.8	1.17	184.04							2.62	89.75	88.83			
NORTH PORTION SOMME STREET	15	16	2.08	0.18	2.26	1.62	10.61	4.50	29.50	25.47	70.36	2075.4	0.00	0.77	1.20	3.00	0.57	2291.4	7480.8	1.29	145.08							1.88	88.83	88.00			
NORTH PORTION SOMME STREET	16	18	2.34	0.23	2.57	1.85	12.46	5.13	34.63	27.35	67.11	2323.9	0.00	0.80	1.20	3.00	0.51	2399.6	7074.8	1.25	185.66							2.48	88.00	87.05			
NORTH PORTION SOMME STREET	18	19	0.00	0.05	0.05	0.05	12.50	0.13	34.75	29.82	63.30	2199.9	0.00	0.76	1.20	3.00	0.72	2476.8	8372.8	1.43	41.86							0.49	87.05	86.75			
										30.31																							
EAST SIDE SAPPERS RIDGE	9	10	1.74	0.19	1.93	1.39	1.39	3.86	3.86	15.00	97.85	378.0	0.00	0.41	1.20	3.00	0.50	399.2	6996.6	0.79	147.87							3.11	92.40	91.66			
EAST SIDE SAPPERS RIDGE	10	11	1.49	0.14	1.63	1.17	2.56	3.25	7.11	18.11	87.42	622.0	0.00	0.49	1.20	3.00	0.66	735.9	8019.2	1.02	111.04							1.81	91.66	90.93			
EAST SIDE SAPPERS RIDGE	11	12	0.73	0.14	0.87	0.64	3.20	1.77	8.88	19.92	82.40	732.0	0.00	0.52	1.20	3.00	0.55	785.5	7304.8	0.97	104.49							1.80	90.93	90.36			
EAST SIDE SAPPERS RIDGE	12	7	0.18	0.09	0.27	0.21	3.40	0.58	9.46	21.72	78.02	738.2	0.00	0.49	1.20	3.00	0.81	818.5	8919.0	1.14	72.55							1.06	90.36	89.77			
NORTH PORTION SOMME STREET	7	20	1.07	0.23	1.30	0.96	4.36	2.66	12.12	22.79	75.66	916.9	0.00	0.57	1.20	3.00	0.50	956.8	6966.1	0.98	177.39							3.01	89.77	88.89			
NORTH PORTION SOMME STREET	20	21	2.27	0.19	2.46	1.76	6.12	4.89	17.01	25.80	69.76	1186.8	0.00	0.62	1.20	3.00	0.50	1200.1	6981.9	1.04	147.49							2.36	88.89	88.16			
NORTH PORTION SOMME STREET	21	22	3.43	0.30	3.73	2.67	8.79	7.43	24.44	28.16	65.80	1608.1	0.00	0.70	1.20	3.00	0.56	1759.0	7404.4	1.20	232.84							3.24	88.16	86.85			
										31.40																							
SOUTHERN CATCHMENT AREA																																	
SOUTH PORTION SOMME STREET	23A	23B	0.00	0.25	0.25	0.23	0.23	0.63	0.63	15.00	97.85	61.2	0.00	0.20	1.20	3.00	0.64	66.3	7883.5	0.55	181.00							5.46	93.65	92.50			
CULVERT CROSSING	23B	23C		0.00	0.00	0.00	0.23	0.00	0.63	20.46	81.05	50.7					0.42				24.00	1	500	----	NO	YES	0.33	1.55	92.50	92.40			
SOUTH PORTION SOMME STREET	23C	24A	0.00	0.17	0.17	0.15	0.38	0.43	1.05	22.00	77.38	81.3	0.00	0.22	1.20	3.00	0.82	97.0	8946.1	0.67	110.00							2.74	92.40	91.50			
CULVERT CROSSING	24A	24B		0.00	0.00	0.00	0.38	0.00	1.05	24.75	71.70	75.3					0.42				24.00	1	500	----	NO	YES	0.34	1.04	91.50	91.40			
SOUTH PORTION SOMME STREET	24B	24C	0.00	0.21	0.21	0.19	0.57	0.53	1.58	25.79	69.78	110.0	0.00	0.25	1.20	3.00	0.70	126.0	8258.2	0.67	142.00							3.52	91.40	90.41			
ORGAWORLD - SITE	U/S	24C	1:10 year peak flow = 132 L/s, see Table 4 of Orgaworld Stormwater Site Management Plan, Sept. 2008										132.0																				
SOUTH PORTION SOMME STREET	24C	25	3.70	0.32	4.02	2.88	3.44	8.00	9.58	29.31	64.05	745.3	0.00	0.52	1.20	3.00	0.54	783.8	7289.5	0.97	244.84							4.22	90.41	89.08			
SOUTH PORTION SOMME STREET	25	26	2.63	0.12	2.75	1.95	5.39	5.42	14.99	33.53	58.41	1007.7	0.00	0.58	1.20	3.00	0.51	1013.1	7041.5	1.00	90.75							1.51	89.08	88.62			
SOUTH PORTION SOMME STREET	26	27A	3.15	0.20	3.35	2.39	7.78	6.63	21.63	35.04	56.65	1357.2	0.00	0.62	1.20	3.00	0.65	1370.0	7970.4	1.19	157.06							2.20	88.62	87.60			
SOUTH PORTION SOMME STREET	27A	27B	0.00	0.03	0.03	0.03	7.81	0.08	21.70	37.24	54.29	1310.1	0.00	0.61	1.20	3.00	0.65	1312.4	7973.8	1.18	20.00							0.28	87.60	87.47			
CULVERT CROSSING	27B	27C		0.00	0.00	0.00	7.81	0.00	21.70	37.53	54.00	1303.8					0.73				15.00	1	----	1.39 X 0.97	YES	NO	0.87	0.20	87.47	87.36			
CORNER OF POND	27C	19	0.00	0.11	0.11	0.10	7.88	0.28	21.98	37.73	53.79	1314.2	0.00	0.65	1.20	3.00	0.71	1622.9	8324.0	1.28	72.00							0.94	87.36	86.85			
										38.67																							

Hawthorne Industrial Park

OPEN DITCH/CULVERT DESIGN SHEET

City of Ottawa

Prepared by: M. Buchanan, E.I.T.

JLR 20983

February 2009 (Revised April 2009)

Checked by: G. Forget, P.Eng.

1:100 year Ottawa International Airport IDF Curve

Increase Runoff Coefficient by 25.0%

DETAILS	NODES		DRAINAGE AREA					PEAK FLOW GENERATION					OPEN DITCH/SWALE DATA						CULVERTS SIZED UNDER 1:10 YEAR STORM EVENT					FLOW TIME (min)	U/S Inv (m)	D/S Inv (m)		
			Area at C of		SUM(A)	SUM(A*1.25*C) 25% increase in C factor	TOTAL A°C	2.78AR	2.78AR CUM	TIME min.	INTENS. mm/hr	PEAK FL. l/s	BW m	D m	SS X:1	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	No. of Barrels	DIA (mm)	B x D (m)	INLET CONTROL				OUTLET CONTROL	
	0.70 (ha)	0.90 (ha)																										
NORTHERN CATCHMENT AREA																												
WEST SIDE SAPPERS RIDGE	2	3	1.86	0.18	2.04	1.81	1.81	5.02	5.02	15.00	142.89	718.0		0.00	1.20	3.00	0.50	6973.0	1.61	136.80					1.41	92.50	91.82	
WEST SIDE SAPPERS RIDGE	3	4	1.89	0.14	2.03	1.80	3.61	5.00	10.02	16.41	135.47	1357.9		0.00	1.20	3.00	0.80	8856.1	2.05	111.00					0.90	91.82	90.93	
WEST SIDE SAPPERS RIDGE	4	5	1.76	0.15	1.91	1.69	5.29	4.69	14.71	17.31	131.16	1929.7		0.00	1.20	3.00	0.51	7029.1	1.63	112.85					1.16	90.93	90.36	
WEST SIDE SAPPERS RIDGE	5	6	2.43	0.11	2.54	2.23	7.53	6.21	20.92	18.47	126.06	2637.5		0.00	1.20	3.00	0.62	7762.6	1.80	82.79					0.77	90.36	89.85	
										19.24																		
NORTH ENTRANCE TO SOMME STREET	8	6		0.03	0.03	0.03	0.03	0.08	0.08	15.00	142.89	11.9		0.00	1.20	3.00	1.30	11276.7	2.61	10.00					0.06	89.98	89.85	
										15.06																		
CULVERT CROSSING	6	14		0.00	0.00	0.00	7.56	0.00	21.01	19.24	122.91	2581.8					0.50			20.00	2	----	1.15 x 0.82	NO	YES	0.19	89.85	89.75
										19.43																		
NORTH PORTION SOMME STREET	13	14	0.85	0.03	0.88	0.77	0.77	2.15	2.15	15.00	142.89	307.4		0.00	1.20	3.00	2.30	14999.4	3.47	10.00					0.05	89.98	89.75	
										15.05																		
NORTH PORTION SOMME STREET	14	15	2.93	0.24	3.17	2.80	11.13	7.79	30.95	19.43	122.15	3780.5		0.00	1.20	3.00	0.50	6992.8	1.62	184.04					1.89	89.75	88.83	
NORTH PORTION SOMME STREET	15	16	2.08	0.18	2.26	2.00	13.13	5.56	36.51	21.32	115.16	4204.4		0.00	1.20	3.00	0.57	7480.8	1.73	145.08					1.40	88.83	88.00	
NORTH PORTION SOMME STREET	16	18	2.34	0.23	2.57	2.28	15.41	6.33	42.84	22.72	110.55	4736.0		0.00	1.20	3.00	0.51	7074.8	1.64	185.66					1.89	88.00	87.05	
NORTH PORTION SOMME STREET	18	19	0.00	0.05	0.05	0.05	15.46	0.14	42.98	24.61	104.93	4509.7		0.00	1.20	3.00	0.72	8372.8	1.94	41.86					0.36	87.05	86.75	
										24.97																		
EAST SIDE SAPPERS RIDGE	9	10	1.74	0.19	1.93	1.71	1.71	4.76	4.76	15.00	142.89	680.4		0.00	1.20	3.00	0.50	6996.6	1.62	147.87					1.52	92.40	91.66	
EAST SIDE SAPPERS RIDGE	10	11	1.49	0.14	1.63	1.44	3.16	4.02	8.78	16.52	134.93	1184.3		0.00	1.20	3.00	0.66	8019.2	1.86	111.04					1.00	91.66	90.93	
EAST SIDE SAPPERS RIDGE	11	12	0.73	0.14	0.87	0.78	3.94	2.16	10.94	17.52	130.23	1424.7		0.00	1.20	3.00	0.55	7304.8	1.69	104.49					1.03	90.93	90.36	
EAST SIDE SAPPERS RIDGE	12	7	0.18	0.09	0.27	0.25	4.18	0.69	11.63	18.55	125.73	1462.2		0.00	1.20	3.00	0.81	8919.0	2.06	72.55					0.59	90.36	89.77	
NORTH PORTION SOMME STREET	7	20	1.07	0.23	1.30	1.17	5.35	3.24	14.87	19.13	123.33	1834.1		0.00	1.20	3.00	0.50	6966.1	1.61	177.39					1.83	89.77	88.89	
NORTH PORTION SOMME STREET	20	21	2.27	0.19	2.46	2.18	7.53	6.05	20.92	20.97	116.41	2435.6		0.00	1.20	3.00	0.50	6981.9	1.62	147.49					1.52	88.89	88.16	
NORTH PORTION SOMME STREET	21	22	3.43	0.30	3.73	3.30	10.83	9.18	30.10	22.49	111.29	3350.0		0.00	1.20	3.00	0.56	7404.4	1.71	232.84					2.26	88.16	86.85	
										24.75																		
SOUTHERN CATCHMENT AREA																												
SOUTH PORTION SOMME STREET	23A	23B	0.00	0.25	0.25	0.25	0.25	0.70	0.70	15.00	142.89	99.3		0.00	1.20	3.00	0.64	7883.5	1.82	181.00					1.65	93.65	92.50	
CULVERT CROSSING	23B	23C		0.00	0.00	0.00	0.25	0.00	0.70	16.65	134.29	93.3					0.42			24.00	1	500	----	NO	YES	0.84	92.50	92.40
SOUTH PORTION SOMME STREET	23C	24A	0.00	0.17	0.17	0.17	0.42	0.47	1.17	17.49	130.34	152.2		0.00	1.20	3.00	0.82	8946.1	2.07	110.00					0.89	92.40	91.50	
CULVERT CROSSING	24A	24B		0.00	0.00	0.00	0.42	0.00	1.17	18.38	126.45	147.6					0.42			24.00	1	500	----	NO	YES	0.53	91.50	91.40
SOUTH PORTION SOMME STREET	24B	24C	0.00	0.21	0.21	0.21	0.63	0.58	1.75	18.91	124.24	217.6		0.00	1.20	3.00	0.70	8258.2	1.91	142.00					1.24	91.40	90.41	
ORGAWORLD - SITE	U/S	24C	1:100 year peak flow = 283 l/s, see Table 4 of Orgaworld Stormwater Site Management Plan, Sept. 2008										283.0															
SOUTH PORTION SOMME STREET	24C	25	3.70	0.32	4.02	3.56	4.19	9.89	11.64	20.15	119.40	1672.8		0.00	1.20	3.00	0.54	7289.5	1.69	244.84					2.42	90.41	89.08	
SOUTH PORTION SOMME STREET	25	26	2.63	0.12	2.75	2.42	6.61	6.73	18.37	22.57	111.05	2323.0		0.00	1.20	3.00	0.51	7041.5	1.63	90.75					0.93	89.08	88.62	
SOUTH PORTION SOMME STREET	26	27A	3.15	0.20	3.35	2.96	9.57	8.22	26.59	23.49	108.17	3159.5		0.00	1.20	3.00	0.65	7970.4	1.84	157.06					1.42	88.62	87.60	
SOUTH PORTION SOMME STREET	27A	27B	0.00	0.03	0.03	0.03	9.60	0.08	26.67	24.91	104.09	3059.5		0.00	1.20	3.00	0.65	7973.8	1.85	20.00					0.18	87.60	87.47	
CULVERT CROSSING	27B	27C		0.00	0.00	0.00	9.60	0.00	26.67	25.09	103.59	3046.2					0.73			15.00	1	----	1.39 X 0.97	YES	NO	0.09	87.47	87.36
CORNER OF POND	27C	19	0.00	0.11	0.11	0.11	9.71	0.31	26.98	25.18	103.36	3071.7		0.00	1.20	3.00	0.71	8324.0	1.93	72.00					0.62	87.36	86.85	
										25.80																		

4.0 Observations

4.1 Surface Conditions

The central portion of the subject site consists of an engineered fill pad which has undergone a successful deep dynamic compaction program by others in June 2022. The remainder of the subject site is generally undeveloped and grass covered, with a slope and watercourse near the northern boundary of the site. Fill piles were also noted within the northern limits of the site, and a gravel surfaced access road bisects the southeast corner of the property.

Based on available aerial photographs, the subject site was densely forested as recently as 1991. By 1999, tree clearing and earthwork operations were ongoing within the subject site and overall business park. Aerial photographs taken between 1999 and 2017 indicate that the subject site was used for stockpiling fill and that numerous temporary access roads had bisected various areas throughout the site. By 2022, it was observed that the central portion of the subject site had been prepared for a ground improvement program. Reference should be made to Figure 2 – Aerial Photograph – 1991, Figure 3 – Aerial Photograph – 1999 and Figure 4 – Aerial Photograph – 2022.

The site is bordered to the north by Rideau Road, to the east by vacant properties located within the overall business park, and to the south and west by Somme Street. The ground surface across the subject site is generally level at approximate geodetic elevation 90 to 91 m, but slopes downward near the northern property boundary to about geodetic elevation 85 m.

4.2 Subsurface Profile

Generally, the subsurface profile encountered at the test hole locations consists of topsoil underlain by an approximate 2.3 to 10 m thickness of fill material which is further underlain by a layer of clayey silt to silty clay and/or silty sand. Generally, the fill material was observed to consist of a silty sand and/or silty clay with varying amounts of organics, gravel, asphalt, concrete and bedrock fragments.

A layer of compact to dense, brown silty sand to sandy silt with gravel, cobbles and boulders was noted underlying the fill material. The silty sand to sandy silt was noted to be grey in colour by approximate depths ranging from 5.8 to 7.6 m below ground surface, and extended to maximum depths ranging from 6.9 to 11.9 m.

Coring through boulders within the silty sand to sandy silt deposit was completed by others at boreholes BH 1 and BH 3 at approximate depths of 7.3 and 10.0 m, respectively, below the ground surface.

A layer of brown silty sand with gravel was noted underlying the fill material and silty clay to clayey silt at borehole BH 1 and extended to an approximate depth of 8.4 m below the existing ground surface.

Bedrock

Practical refusal to augering was encountered at approximate depths ranging from 5.3 to 12.0 m below ground surface. Practical refusal to the DCPT was noted at a depth of about 5.9 m at DCPT 5 by others. The bedrock was cored by others at boreholes BH 1-21, BH 2-21 and BH 1 through BH3, and was noted to consist of poor to excellent quality, grey limestone with interbedded sandstone. The bedrock was cored to a maximum depth of 18.9 m below the existing ground surface.

Uniaxial compressive strength (UCS) testing was carried out on a total of 2 bedrock samples by others. The test results indicate that the UCS of the bedrock samples were 139 and 125 MPa for the samples tested at borehole BH 2-21.

Reference should be made to the Soil Profile and Test Data Sheets in Appendix 1 for details of the soil and bedrock profile encountered at each borehole location.

Grain Size Distribution and Hydrometer Testing

A total of 11 grain size analyses and hydrometer tests were completed by others. The results are summarized in Table 1 on the following page and are presented in Appendix 1.

Table 1 – Summary of Grain Size Distribution Analysis					
Test Hole	Sample	Gravel (%)	Sand (%)	Silt (%)	Clay (%)
BH1-21	SS2B*	17	60	19	4
BH1-21	SS13	16	32	36	16
BH2-21	SS12	20	38	33	9
BH2-21	SS18	6	29	42	23
BH3-21	SS8	19	49	26	6
BH5-21	SS3*	25	38	29	8
BH5-21	SS7	10	38	41	11
BH1	SS3*	51	43	6	1
BH2	SS4*	1	2	36	61
BH2	SS7*	25	38	29	8
BH3	SS10	8	47	37	8
Note: * – Soil sample taken within the fill material.					

Atterberg Limit Tests

A total of 6 soil samples were submitted for Atterberg Limits testing by others. The test results indicate that the selected soil samples were generally classified as an Inorganic Clay of Low Plasticity (CL) to an Inorganic Silt of Low Plasticity (ML). These classifications are in accordance with the Unified Soil Classification System. The results are summarized in Table 2 below.

Table 2 – Summary of Atterberg Limits Results						
Borehole	Sample	Depth (m)	LL (%)	PL (%)	PI (%)	Classification
BH 1-21	SS13	9.1 – 9.8	26	18	8	CL-ML
BH 2-21	SS12	8.4 – 9.0	25	17	8	CL-ML
BH 2-21	SS18	13.0 – 13.6	28	14	14	CL
BH 3-21	SS8	5.3 – 5.9	17	13	4	CL-ML
BH 5-21	SS7	4.6 – 5.2	20	13	7	CL-ML
BH 2	SS4*	2.3 – 3.0	69	21	48	CH
Notes: * – Soil sample taken within the fill material; LL: Liquid Limit; PL: Plastic Limit; PI: Plasticity Index; CH: Inorganic Clay of High Plasticity; CL: Inorganic Clay of Low Plasticity; ML: Inorganic Silt of Low Plasticity						

4.3 Groundwater

Groundwater levels were measured in the monitoring wells and standpipe piezometers following completion of the respective drilling programs. The measured groundwater levels are presented on the Soil Profile and Test Data sheets in Appendix 1, and in Table 1 below:

Table 3 – Summary of Groundwater Levels				
Borehole Number	Ground Surface Elevation (m)	Measured Groundwater Level		Dated Recorded
		Depth (m)	Elevation (m)	
BH 1-25	91.31	3.44	87.87	June 5, 2025
BH 2-25	91.06	3.01	88.05	June 5, 2025
BH 3B-25	90.80	3.31	87.49	June 5, 2025
BH 4-25	92.18	7.54	84.64	June 5, 2025
BH 1	91.80	2.92	88.88	May 28, 2019
		3.09	88.71	November 10, 2020
BH 2	92.38	3.82	88.56	May 28, 2019
		3.17	89.21	November 10, 2020
BH1 (GHD)	90.21*	3.99	86.22	August 18, 2020
		2.95	87.26	June 3, 2022
MW7-08	93.81*	3.68	90.13	July 17, 2008
	92.85	2.95	89.90	November 10, 2020
B5-1	90.48*	7.63	82.85	-
Note: Ground surface elevations at borehole location are referenced to a geodetic datum. * – Ground Surface elevations surveyed by others				

However, it should be noted that groundwater levels are subject to seasonal fluctuations. Therefore, the groundwater levels could vary at the time of construction.

5.0 Discussion

5.1 Geotechnical Assessment

From a geotechnical perspective, the subject site is suitable for the proposed development. Foundation support for the proposed buildings is recommended to consist of conventional spread footings bearing on the existing fill material which has undergone a successful ground improvement program, as approved by others. Documentation of the ground improvement program completed to-date is provided in Appendix 3.

However, the ground improvement program was completed a few years ago for a different building configuration, and the ground improvement contractor should also review that the existing ground improvement is suitable for the loading from the currently proposed buildings. Further, if the existing ground improvement does not encompass the entirety of the currently proposed buildings, then additional ground improvement will be required.

Due to the presence of silty clay deposit at this site, the proposed development will be subjected to grade raise restrictions. Our permissible grade raise recommendations are discussed in Section 5.3.

The current site plan shows the proposed development extending near the northern property boundary. This will require the construction of a retaining wall or mechanically stabilized earth (MSE) wall to stabilize and shift the existing slope. This is discussed further in Section 6.8.

The above and other considerations are discussed in the following paragraphs.

5.2 Site Grading and Preparation

Stripping Depth

Topsoil and any fill containing significant amounts of deleterious or organic materials should be stripped from under the proposed buildings and other settlement sensitive structures.

However, it is anticipated that the existing fill underlying the proposed asphalt paved areas, free of deleterious material and significant amounts of organics, can be left in place. In this case, it is recommended that the existing fill layer be proof-rolled with several passes of a vibratory drum roller, under dry conditions and above freezing temperatures, and which is approved by Paterson personnel at the

time of construction. Any poor performing areas noted during the proof-rolling operation should be removed and replaced with an approved fill.

Engineered Fill Placement

Engineered fill used for grading beneath the proposed buildings, where required, should consist of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. This material should be tested and approved prior to delivery to the site. The fill should be placed in lifts no greater than 300 mm thick and compacted using suitable compaction equipment for the lift thickness. Fill placed beneath the building and paved areas should be compacted to at least 98% of the material's standard Proctor maximum dry density (SPMDD).

Non-specified existing fill, along with site-excavated soil, can be used as general landscaping fill where settlement of the ground surface is of minor concern. This material should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids. If this material is to be used to build up the subgrade level for areas to be paved, it should be compacted in thin lifts to at least 95% of the material's SPMDD.

Ground Improvement

As noted above in Section 5.1, a ground improvement program was previously completed at this site for a different building configuration. Therefore, the ground improvement contractor should review that the existing ground improvement is suitable for the loading from the currently proposed buildings. Further, if the existing ground improvement does not encompass the entirety of the currently proposed buildings, then additional ground improvement will be required.

Ground improvement programs are based on the level of density and the depth of improved soil required.

The existing ground improvement at the site was completed by deep dynamic compaction, which uses a heavy weight dropped from a crane, and which is effective for compacting loose soils to significant depths.

Ground improvement programs are completed by design-build contractors, many of which have proprietary systems. The ground improvement contractor should determine the appropriate method for additional ground improvement, based on the configuration and loading of the currently proposed development.

The ground improvement program should also account for loose fill soils below the groundwater table, and should sufficiently densify them such that they are not susceptible to the effects of liquefaction.

A detailed design submittal should then be provided for Paterson's review and comment prior to the commencement of additional ground improvement operations.

Vibration Considerations

Construction operations could be the cause of vibrations, and possibly, sources of nuisance to the community. Therefore, means to reduce the vibration levels as much as possible should be incorporated in the construction operations to maintain a cooperative environment with the residents.

The following construction equipment could be the source of vibrations: ground improvement, hoe ram, compactor, dozer, crane, truck traffic, etc. Vibrations, whether caused by the ground improvement or by other construction operations, could be the cause or the source of detrimental vibrations at the nearby buildings and structures. Therefore, it is recommended that all vibrations be limited.

Two parameters determine the permissible vibrations, the maximum peak particle velocity and the frequency. For low frequency vibrations, the maximum allowable peak particle velocity is less than that for high frequency vibrations. As a guideline, the peak particle velocity should be less than 15 mm/s between frequencies of 4 to 12 Hz, and 50 mm/s above a frequency of 40 Hz (interpolate between 12 and 40 Hz).

It should be noted that these guidelines are for current construction standards. These guidelines are above perceptible human level and, in some cases, could be very disturbing to some people. It is recommended that a pre-construction survey be completed to minimize the risks of claims during or following the construction of the proposed buildings.

5.3 Foundation Design

Bearing Resistance Values

Footings placed on an existing fill subgrade which has undergone a ground improvement program, approved by others, can be designed using a bearing resistance value at serviceability limit states (SLS) of **200 kPa** and a factored

bearing resistance value at ultimate limit states (ULS) of **275 kPa**, in accordance with the recommendations provided by the ground improvement specialists.

Footings bearing on an approved fill subgrade and designed using the bearing resistance values provided herein will be subjected to potential post-construction total and differential settlements of 25 and 19 mm, respectively, in accordance with the recommendations provided by the ground improvement specialists.

Lateral Support

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Above the groundwater level, adequate lateral support is provided to the in-situ bearing medium soils when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V passes only through in situ soil.

Permissible Grade Raise Restrictions

Due to the presence of the silty clay soils observed within portions of this site, a permissible grade raise restriction is recommended for the proposed development at the subject site. Based on the testing results, a permissible grade raise restriction of **2.0 m** above the existing ground surface is recommended.

If higher than permissible grade raises are required, preloading with or without a surcharge, lightweight fill, and/or other measures should be investigated to reduce the risks of unacceptable long-term post construction total and differential settlements.

5.4 Design for Earthquakes

The site class for seismic site response can be taken as **Class X_D**. A higher seismic site class (such as Class X_C) may be achievable following completion of the ground improvement program, however, a site-specific shear wave velocity test would be required to accurately determine the higher seismic site classification for foundation design of the proposed buildings, as defined in Table 4.1.8.4.A of the Ontario Building Code (OBC) 2024.

If required, Paterson can conduct the site-specific shear wave velocity test. Following completion of the ground improvement program, the soils underlying the subject site will not be susceptible to liquefaction.

5.5 Slab on Grade Construction

With the removal of all topsoil and fill, containing significant amounts of deleterious or organic materials, the existing fill material, which has undergone a successful ground improvement program, approved by others, is considered to be an acceptable subgrade surface on which to commence backfilling for slab on grade construction.

It is recommended that the upper 200 mm sub-floor fill consists of OPSS Granular A crushed stone. All backfill materials within the footprint of the proposed buildings should be placed in a maximum of 300 mm thick loose layers and compacted to at least 98% of the material's SPMDD.

5.6 Pavement Design

For design purposes, the pavement structures presented in the following tables are recommended for the design of car only parking areas, access lanes, heavy truck parking and loading areas.

Table 4 – Recommended Pavement Structure – Car Only Parking Areas	
Thickness (mm)	Material Description
50	Wear Course – HL-3 or Superpave 12.5 Asphaltic Concrete
150	BASE – OPSS Granular A Crushed Stone
300	SUBBASE – OPSS Granular B Type II
SUBGRADE – Either fill, in situ soil or OPSS Granular B Type I or II material placed over in situ soil or fill.	

Table 5 – Recommended Pavement Structure – Access Lanes and Heavy Truck Parking/Loading Areas	
Thickness (mm)	Material Description
40	Wear Course – HL-3 or Superpave 12.5 Asphaltic Concrete
50	Binder Course – HL-8 or Superpave 19.0 Asphaltic Concrete
150	BASE – OPSS Granular A Crushed Stone
450	SUBBASE – OPSS Granular B Type II
SUBGRADE – Either fill, in situ soil or OPSS Granular B Type I or II material placed over in situ soil or fill.	

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type II material.

The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 99% of the material's SPMDD using suitable vibratory equipment.

5.7 SWM Pond Design

As noted above, it is understood that a SWM Pond will be constructed at the subject site. From a geotechnical perspective, the construction of the proposed SWM Pond is considered suitable for this site. However, it is recommended that Paterson complete a review of the detailed design, once available, as details such as pond bottom elevation, proposed side slopes, permanent pond elevation, 5-year and 100-year water elevations, were not known at the time of preparation of this report.

Generally, the subsoil conditions at the borehole locations throughout the proposed SWM Pond generally consist of a silty sand to silty clay fill underlain by glacial till and bedrock. Accordingly, a pond liner is recommended which may consist of one of the following:

- ☐ A 500 mm thick layer of workable, brown silty clay which is compacted with several passes of a sheepsfoot roller, and placed directly over the existing fill, or a
- ☐ 150 mm thick layer of 19 mm clear crushed stone placed over the fill subgrade, followed by a scrim-reinforced geosynthetic clay liner (such as the Bentofix Thermal Lock SRNWL Series Geosynthetic Clay Liner, or approved equivalent) which is capped with a 300 mm thick layer of OPSS Granular A material.

The long-term performance of the proposed SWM Pond will depend on the stability of its excavation side slopes. From a geotechnical perspective, the sidewall slopes should be constructed at 3H:1V or shallower.

6.0 Design and Construction Precautions

6.1 Foundation Drainage and Backfill

Backfill against the exterior sides of the foundation walls should consist of free draining, non-frost susceptible granular materials. Imported granular materials, such as clean sand or OPSS Granular B Type I granular material, can be used for this purpose.

6.2 Protection of Footings Against Frost Action

Perimeter foundations of heated structures are required to be insulated against the deleterious effects of frost action. A minimum of 1.5 m of soil cover, or a minimum of 0.6 m of soil cover in conjunction with adequate foundation insulation, should be provided.

Exterior unheated foundations, such as those for isolated exterior piers, are more prone to deleterious movement associated with frost action than the exterior walls of the heated structure and require additional protection, such as soil cover of 2.1 m, or an equivalent combination of soil cover and foundation insulation.

6.3 Excavation Side Slopes

The temporary excavation side slopes anticipated should either be excavated to acceptable slopes or retained by shoring systems from the beginning of the excavation until the structure is backfilled. It is expected that sufficient room will be available for the greater part of the excavation to be undertaken by open-cut methods (i.e. unsupported excavations).

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be cut back at 1H:1V or flatter. The flatter slope is required for excavation below groundwater level. The subsurface soil is considered to be mainly a Type 2 and 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should maintain safe working distance from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress.

It is recommended that a trench box be used at all times to protect personnel working in trenches with steep or vertical sides. It is expected that services will be installed by “cut and cover” methods and excavations will not be left open for extended periods of time.

6.4 Pipe Bedding and Backfill

Bedding and backfill materials should be in accordance with the most recent Material Specifications and Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Services Branch of the City of Ottawa.

A minimum of 150 mm of OPSS Granular A should be placed for bedding for sewer or water pipes when placed on a soil subgrade. The bedding should extend to the spring line of the pipe. Cover material, from the spring line to a minimum of 300 mm above the obvert of the pipe, should consist of OPSS Granular A (concrete or PSM PVC pipes) or sand (concrete pipe). The bedding and cover materials should be placed in maximum 225 mm thick lifts and compacted to 98% of the SPMDD.

It should generally be possible to re-use the moist (not wet) site-generated fill above the cover material if the excavation and filling operations are carried out in dry weather conditions.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (about 1.8 m below finished grade) should match the soils exposed at the trench walls to minimize differential frost heaving. The trench backfill should be placed in maximum 300 mm thick loose lifts and compacted to a minimum of 95% of the material's SPMDD. All cobbles larger than 200 mm in their longest direction should be segregated from re-use as trench backfill.

6.5 Groundwater Control

Based on our observations, it is anticipated that groundwater infiltration into the excavations should be low to moderate and controllable using open sumps. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

Groundwater Control for Building Construction

A temporary Ministry of Environment, Conservation and Parks (MECP) permit to take water (PTTW) may be required if more than 400,000 L/day of ground and/or surface water are to be pumped during the construction phase. At least 4 to

5 months should be allowed for completion of the application and issuance of the permit by the MECP.

For typical ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Persons as stipulated under O.Reg. 63/16.

Impacts to Neighbouring Properties

The proposed buildings will have slabs-on-grade, therefore, the excavations for these buildings are not expected to extend below the groundwater level. Accordingly, dewatering is not anticipated, and impacts to adjacent properties as a result of dewatering or excavation is not anticipated.

6.6 Winter Construction

Precautions must be taken if winter construction is considered for this project. The subsoil conditions at this site consist of frost susceptible materials. In the presence of water and freezing conditions, ice could form within the soil mass. Heaving and settlement upon thawing could occur.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the use of straw, propane heaters and tarpaulins or other suitable means. In this regard, the base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Trench excavations and pavement construction are also difficult activities to complete during freezing conditions without introducing frost in the subgrade or in the excavation walls and bottoms. Precautions should be taken if such activities are to be carried out during freezing conditions. Additional information could be provided, if required.

6.7 Corrosion Potential and Sulphate

The results of analytical testing on the soil samples retrieved by others show that the sulphate content is less than 0.1%. These results are indicative that Type 10 Portland cement (normal cement) would be appropriate for this site. The chloride

content and the pH of the samples indicate that they are not significant factors in creating a corrosive environment for exposed ferrous metals at this site, whereas the resistivity is indicative of an aggressive corrosive environment.

6.8 Slope Stability Assessment

The slope conditions at the northern limits of the site were reviewed by Paterson field personnel during the recent and previous geotechnical investigations. Two (2) slope cross-sections (Sections A-A' and B-B') were studied, which are considered to be the worst-case scenarios. The cross-section locations are presented on Drawing PG7567-1 – Test Hole Location Plan in Appendix 2.

The existing slope, which extends down to a ditch, was generally observed to be vegetated with trees. No signs of active or historic erosion were observed along the slope face.

Paterson has performed 2 sets of analyses, one where this slope is unreinforced and in its current configuration, and another where it is built out near the property line and reinforced using a retaining wall or mechanically stabilized earth (MSE) wall.

Slope Stability Analysis

The analyses of the slope stability were carried out using SLIDE, a computer program which permits a two-dimensional slope stability analysis using several methods including the Bishop's method and Morgenstern-Price method, which are widely used and accepted analysis methods. The program calculates a factor of safety, which represents the ratio of the forces resisting failure to those favouring failure. Theoretically, a factor of safety of 1.0 represents a condition where the slope is stable.

However, due to intrinsic limitations of the calculation methods and the variability of the subsurface soil and groundwater conditions, a factor of safety greater than 1.0 is generally required for the failure risk to be considered acceptable. A minimum factor of safety of 1.5 is generally recommended for conditions where the slope failure would comprise permanent structures.

An analysis considering seismic loading was also completed. A horizontal acceleration of 0.2035 g was considered for the sections under seismic loading conditions. A factor of safety of 1.1 is considered to be satisfactory for stability analyses including seismic loading.

The cross-sections were analyzed taking into consideration the existing conditions observed during our site visit, and our review of the available soils information and topographic mapping. Subsoil conditions at the cross-sections were inferred based on nearby boreholes and general knowledge of the area's geology.

The effective and total soil strength parameters used for the analyses were chosen based on the subsoil information recovered during the previous investigations. The effective strength soil parameters used for the static and seismic analyses are presented in Table 6 below.

Table 6 – Total Effective and Total Soil and Material Parameters			
Soil Layer	Unit Weight (kN/m ³)	Friction Angle (degrees)	Cohesion (kPa)
Dynamic Compacted Granular Fill	21	35	0
Engineered Fill	21	36	0
Existing Fill	20	34	0
Grey Silty Sand	20	35	0
Glacial Till	20	35	1
Bedrock	22	-	1,000

Static Loading Analysis

Paterson performed 2 sets of analyses for the slope, one where the slope is unreinforced, and another where it is reinforced using an MSE wall. The results of the static analyses for the unreinforced and reinforced slopes at Sections A-A' and B-B' are shown on Figures 2A, 2C, 3A, and 3C in Appendix 2. For the proposed unreinforced conditions, the factor of safety was found to be 0.34 and 0.447 at Sections A-A' and B-B', respectively, while the factor of safety of the reinforced slope exceeded 1.5 at both cross-section locations.

Based on these results, the proposed reinforced slope is considered to be stable under static loading, while the unreinforced slope requires a Stable Slope Allowance to achieve a factor of safety of 1.5 under static conditions.

Seismic Loading Analysis

An analysis considering seismic loading for proposed conditions was also completed.

The results of the seismic analyses for the unreinforced and reinforced slopes at Sections A-A' and B-B' are shown on Figures 2B, 2D, 3B, and 3D in Appendix 2. For the proposed unreinforced conditions, the factor of safety was found to be 0.386 and 0.254 at Sections A-A' and B-B', respectively, while the factor of safety of the reinforced slope exceeded 1.1 at both cross-section locations.

Based on these results, the proposed reinforced slope is considered to be stable under seismic loading, while the unreinforced slope requires a Stable Slope Allowance to achieve a factor of safety of 1.1 under seismic conditions.

Geotechnical Setback – Limit of Hazard Lands

Under the existing slope conditions, Stable Slope Allowances of 11.1 and 25.7 m are required at cross-sections A-A' and B-B' in order to achieve the appropriate factors of safety under static and seismic conditions. In addition to the Stable Slope Allowances, Toe Erosion and Erosion Access Allowances of 2 m and 6 m, respectively, are required from the top of slope. **Therefore, Limit of Hazard Lands setbacks of 19.1 and 33.7 m are required at cross-sections A-A' and B-B', this is shown on the attached Drawing PG7567-1 – Test Hole Location Plan in Appendix 2.**

However, for the reinforced case where the slope is built out and supported with an MSE wall or retaining wall, sufficient factors of safety are present under static and seismic conditions such that Stable Slope Allowance setbacks are not required. Further, erosion protection will be incorporated into the MSE wall design such that a Toe Erosion Allowance is not required for the reinforced case. Also, since the slope will have suitable factors of safety and will have erosion protection, an Erosion Access Allowance will not be required for future repair of the slope. **Accordingly, no Limit of Hazard Lands setback is required for the reinforced case where the slope is built out and supported with an MSE wall.**

The design of the MSE wall can be prepared during the detailed design stage of the project. The MSE wall design will need to be stamped by a Professional Engineer licensed in the province of Ontario, and will need to be reviewed by Paterson to confirm that it is in accordance with the geotechnical design recommendations provided in this report.

7.0 Recommendations

It is a requirement for the foundation data provided herein to be applicable that the following material testing, and observation program be performed by the geotechnical consultant.

- Observation of all bearing surfaces prior to the placement of concrete.
- Sampling and testing of the concrete and fill materials.
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- Observation of all subgrades prior to backfilling.
- Field density tests to determine the level of compaction achieved.
- Sampling and testing of the bituminous concrete including mix design reviews.

All excess soils, with the exception of engineered crushed stone fill, generated by construction activities that will be transported on-site or off-site should be handled as per *Ontario Regulation 406/19: On-Site and Excess Soil Management*.

A report confirming that these works have been conducted in general accordance with our recommendations could be issued upon request, following the completion of a satisfactory material testing and observation program by Paterson



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