

SUBJECT

City Gates Phase 2 - Interim Parking Design
Arcadis Engineering Review

TO

Andrew Kent
Senior Director, Developments
Killam REIT
Halifax, NS

DATE

22/08/2025

DEPARTMENT

Land Engineering

PROJECT NUMBER

38729-5.2.2.1

COPIES TO

Matt Anderson Petitpas
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NAME

Terry Brule
Principal – Practice Lead
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Introduction

This technical memorandum has been prepared for Killam REIT and provides a review of the civil engineering aspects supporting the proposed interim surface parking area design as part of Phase 2 of the City Gates mixed-use site plan. The memorandum is based on the City Park Redevelopment Phase 2 Design Brief, prepared by IBI Group and dated December 2018. This document can be made available if needed. However, for reference, a copy of the Phase 2 drawing (38729-C-101 Site Servicing) is included in **Appendix A** to illustrate the proposed adjustments for this phase.

Figure 1 shows the location site plan for the interim parking within Phase 2, for which the proponent is seeking approvals. This interim parking is bordered by the future Phase 3 Block 2 to the north, the previously completed Phase 2 Block 5 to the east, and Vantage Point Private to the west and south. The plan consists of constructing 64 additional parking spaces within the existing landscaped areas until the proposed future residential building in Block 3 is constructed.

Figure 1: Site Location



This memorandum will outline the impacts on stormwater management and site grading for the proposed parking addition. It should be noted due to the limited extents of construction required to support the proposed works, there will be no impacts on Sanitary collection and disposal or water supply.

Stormwater Management

The proposed storm outlet for the subject site is shown on the City Park Development Phase 2 drawing C-103 Site Servicing and is included in **Appendix B**. In the proposed interim parking design, it is proposed to connect to the existing catchbasin leads and extend to the center of proposed parking lots to maximize ponding capacities while maintaining the same release rate that was previously designed (2.0 L/s). As shown in the Stormwater Management Modified Rational Method calculations, the 2 revised ponding areas provide sufficient surface storage for the 100yr storm event. Due to the release rate of the existing inlet control devices located at the catchbasins to be utilized, there will be minor surface ponding during the 5yr storm event. A copy of these SWM calculations have been provided in **Appendix B**.

A storm sewer design sheet reflecting the revised drainage areas, runoff coefficients, and associated storm pipes, which have been highlighted for reference, is included in **Appendix B**. It should be noted that the storm pipe running from MH202 to EXMH124 remains slightly over capacity, primarily due to its dimensions and slope, as well as the increased runoff coefficients from the additional parking spaces. However, any resulting minor surcharging will not be problematic, as this storm sewer segment exclusively handles parking lot surface drainage and is not connected to any buildings. At most, this could result in temporary detention upstream within the storm pipes, associated structures, and surface areas. Importantly, all other pipes, both upstream and downstream, have sufficient capacity to convey the flows without any surcharging. A copy of the Phase 2 Interim Parking updated Storm Drainage Area plan (38729-C-502 Storm Drainage Area Plan) has been included in **Appendix B**, as well as a copy of the Phase 2 Storm Drainage Area plan (38729-C-501 Storm Drainage Area Plan) to show the extents of the drainage design changes.

Site Grading

The proposed grading for the interim parking, as shown in drawing 38729-202 Grading Plan, can be found in **Appendix C**. A review of the Phase 2 site grading plan (drawing C-201 Grading Plan) confirms that the proposed grading aligns with the existing conditions as shown on the Phase 2 plan and follows standard design concepts to ensure proper drainage and integration with surrounding areas. A copy of the Phase 2 site grading plan (C-201) is also included in **Appendix C**.

Conclusion

In summary, apart from some minor surcharging, the proposed stormwater management and servicing maintain the existing level of service prescribed in the Design Brief prepared by IBI Group, dated December 2018, for the City Parks Development Phase 2. Additionally, the proposed site grading integrates with the surrounding existing grades while ensuring sufficient stormwater surface detention and compliance with the City's design guidelines. We therefore confident that the servicing design shown on the City Parks Interim Parking engineering plans should be satisfactory to the City of Ottawa.

We trust our conclusions are satisfactory for your purposes. We are, of course, available to review and discuss the information contained within this document.

Yours truly,

Arcadis Professional Services (Canada) Inc.



Terry Brule, P.Eng.



Appendix A



Killam
PROPERTIES INC

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HOBIN
ARCHITECTURE

Project Title

**2280 CITY PARK
DEVELOPMENT**

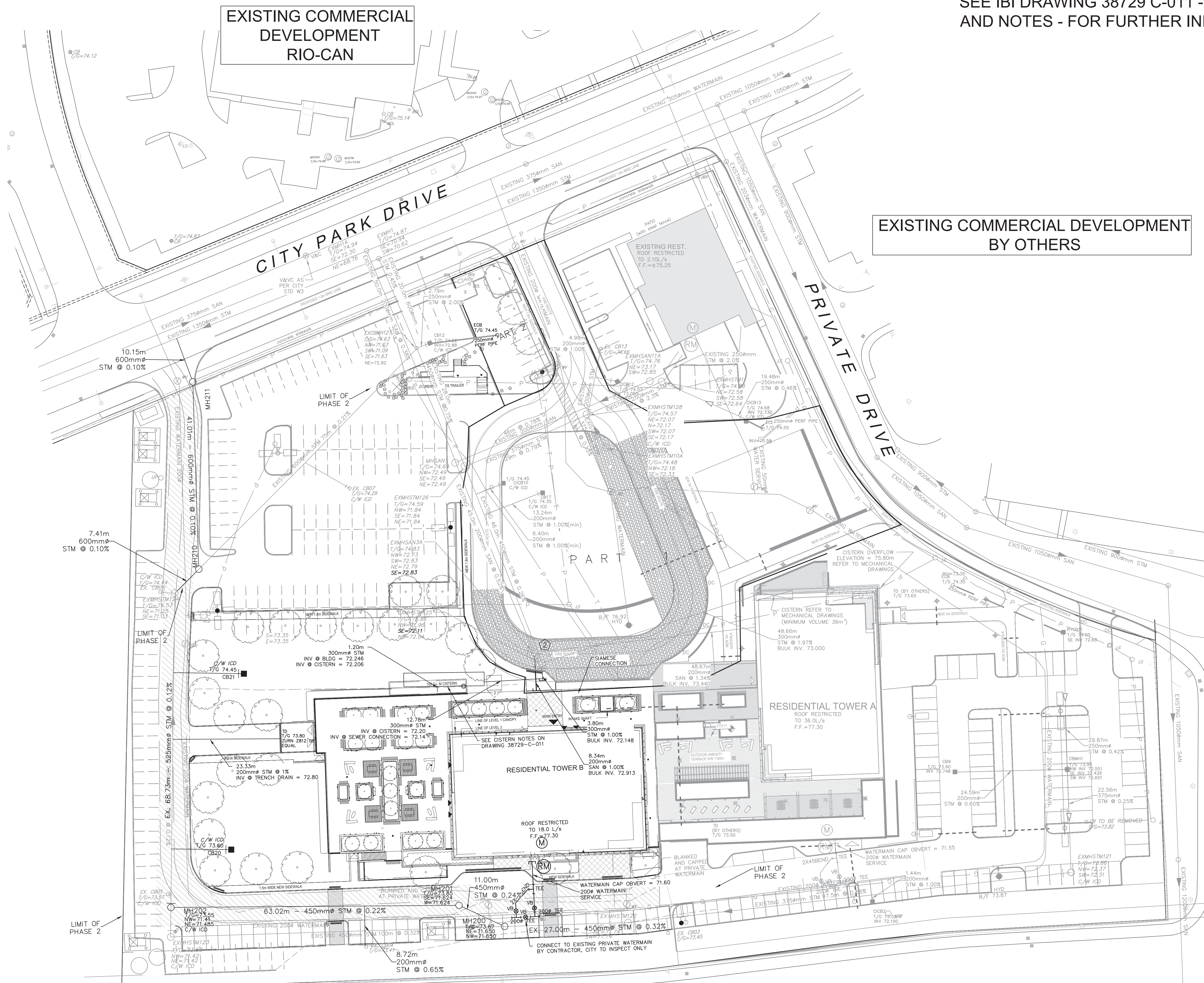
PHASE 2



Design	Date
J.E.B.	AUG. 2018

Project No.	Drawing No.
38729	C-101

D07-12-18-0122




Appendix B



This drawing has been prepared solely for the intended use, thus any reproduction or distribution for any purpose other than authorized by Arcadis is forbidden. Written dimensions shall have precedence over scaled dimensions. Contractors shall verify and be responsible for all dimensions and conditions on the job, and Arcadis shall be informed of any variations from the dimensions and conditions shown on the drawing. Shop drawings shall be submitted to Arcadis for general conformance before proceeding with fabrication.

Arcadis Professional Services (Canada) Inc.
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PROJECT
KILLAM - CITY PARK PHASE 2
INTERIM PARKING

200 FRONTIER PATH PVT.

PROJECT NO:
38729

DRAWN BY:
D.D. / E.H.

PROJECT M
T.R.B.

T.R.D.
SHEET TITLE
REV.

GENERAL SERVICING PLAN

SHEET NUMBER

C-103

ISSUE

1

SCALE CHECK	..	File Location: J:\38729-SilverCity\ECAS.9 Drawings\59civil\current\Parking\Sheets\C-103 SERVICING PLAN.dwg	Last Saved: August 22, 2025, by dored5754	Plotted: August 22, 2025 6:48:53 AM by Dore, Denis
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HECK		
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IBI GROUP
333 PRESTON STREET
OTTAWA, ON
K1S 5N4

PHASE 2 - INTERIM PARKING

PROJECT: Killam REIT City Gates
DATE: 2025-08-20
FILE: 38729 - 5.7
REV #: 6 - 2025-08-20
DESIGNED BY: MAP
CHECKED BY: TB

STORMWATER MANAGEMENT

Formulas and Descriptions

i_{2yr} = 1:2 year Intensity = $732.951 / (T_c + 6.199)^{0.810}$
 i_{5yr} = 1:5 year Intensity = $998.071 / (T_c + 6.053)^{0.814}$
 i_{100yr} = 1:100 year Intensity = $1735.688 / (T_c + 6.014)^{0.820}$
 T_c = Time of Concentration (min)
 C = Average Runoff Coefficient
 A = Area (Ha)
 Q = Flow = $2.78CiA$ (L/s)

Maximum Allowable Release Rate

Restricted Flowrate (based TOD 5y @ $C=0.5$)

C =	0.5	*as per proposed Storm Sewer Design sheet
T_c =	17.27 min	
i_{100yr} =	76.87 mm/hr	
A_{TOTAL} =	2.86 Ha	
Q_{TOTAL} =		305.61 L/s
Q_{TOTAL} =		305.61 L/s

Uncontrolled Release ($Q_{uncontrolled} = 2.78 \cdot C \cdot i_{100yr} \cdot A_{uncontrolled}$)

$C =$	0.3
$T_c =$	10 min
$i_{100yr} =$	178.56 mm/hr
$A_{uncontrolled} =$	0.26 Ha
<hr/>	
$Q_{uncontrolled} =$	38.72 L/s

Maximum Allowable Release Rate ($Q_{max\ allowable} = Q_{restricted} - Q_{uncontrolled}$)

$Q_{max\ allowable}$ =	266.89 L/s
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MODIFIED RATIONAL METHOD (100-Year & 5-Year)

Drainage Area		1	(1 & 8 with weighted average C)		
Area (Ha)		0.380			
C =		0.79	Restricted Flow Q_r (L/s)= 4.00		
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78xCi_{100yr}A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
183	23.59	19.74	4.00	15.74	172.80
185	23.39	19.57	4.00	15.57	172.81
186	23.29	19.48	4.00	15.48	172.81
187	23.19	19.40	4.00	15.40	172.81
189	22.99	19.24	4.00	15.24	172.80

Drainage Area		1				
Area (Ha)		0.380				
C =	0.70	Restricted Flow Q_r (L/s)=			4.00	
5-Year Ponding						
T_c Variable (min)	i_{syrr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{syrr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume $5yr$ (m^3)	
95	23.31	17.23	4.00	13.23	75.43	
97	22.94	16.96	4.00	12.96	75.43	
98	22.76	16.83	4.00	12.83	75.43	
99	22.58	16.70	4.00	12.70	75.43	
101	22.24	16.44	4.00	12.44	75.41	

Storage (m ³)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	172.81	175.10	3.90	0.00

Storage (m ³)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	75.43	175.10	3.90	0.00

IN-LINE STORAGE (Structure)

IN-LINE STORAGE (Structure)			IN-LINE STORAGE (Structure)		
0.6m X 0.6m CB			1.2mDia CBMH's		
0.36 m3/m	Height (m)	Storage (m3)	1.13 m3/m	Height (m)	Storage (m3)
RYCB11	1.12	0.40	CBMH1	1.17	1.33
RYCB12	1.02	0.37	Total:		1.33
CB9	0.85	0.31			
Total:		1.08			

overflows to: 2,3,4

IN-LINE STORAGE (Pipe)

IN-LINE STORAGE (Pipe)			
Pipe storage			
Structure to Structure	Length (m)	Dia (m)	Storage (m3)
RYCB12 - RYCB11	23.29	0.20	0.73
RYCB11 - CBMH1	28.43	0.25	1.40
CB9 - CBMH1	24.59	0.20	0.77
CBMH1 - EXMHSTM121	22.56	0.38	2.49
Total:			5.39

overflows to: 2,3,4

Drainage Area		2,3,4			
Area (Ha)	0.310				
C =	1.00	Restricted Flow Q_r (L/s)= 25.00			
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_o = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_o - Q_r$ (L/s)	Volume 100yr (m^3)
27	98.66	85.02	25.00	60.02	97.24
29	94.01	81.02	25.00	56.02	97.48
30	91.87	79.17	25.00	54.17	97.51
31	89.83	77.41	25.00	52.41	97.49
33	86.03	74.14	25.00	49.14	97.31

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	97.51	81.48	15.75	0.28

IN-LINE STORAGE (Structure)			IN-LINE STORAGE (Structure)		
0.6m X 0.6m CB			1.2mDia CBMH's		
0.36 m3/m	Height (m)	Storage (m3)	1.13 m3/m	Height (m)	Storage (m3)
CICB2	1.35	0.49	EXMHSTM121	1.37	1.55
EXCB03	1.10	0.40	EXMHSTM122	1.85	2.09
EXCB04	0.85	0.31			
Total:		1.19	Total:		3.64

overflows to: offsite

Drainage Area		5			
Area (Ha)	0.130	Restricted Flow Q_r (L/s)= 27.00			
C =	1.00				
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	$Q_p = 2.78xCi_{100yr}A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
9	188.25	68.04	27.00	41.04	22.16
11	169.91	61.40	27.00	34.40	22.71
12	162.13	58.59	27.00	31.59	22.75
13	155.11	56.06	27.00	29.06	22.66
15	142.89	51.64	27.00	24.64	22.18

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
2.74	25.49	26.06	0	0.00

overflows to: offsite

Drainage Area		5A			
Area (Ha)		0.090			
C =		0.88			
		Restricted Flow Q_r (L/s)= 2.00			
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	$Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
88	41.83	9.16	2.00	7.16	37.79
90	41.11	9.00	2.00	7.00	37.80
91	40.76	8.92	2.00	6.92	37.81
92	40.42	8.85	2.00	6.85	37.81
94	39.76	8.70	2.00	6.70	37.81

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
	37.81	40.66	0	0.00

Drainage Area		5B			
Area (Ha)		0.110			
C =		0.84			
		Restricted Flow Q_r (L/s)= 2.00			
100-year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	$Q_p = 2.78xCi_{100yr}A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
104	36.77	9.42	2.00	7.42	46.28
106	36.23	9.28	2.00	7.28	46.29
107	35.97	9.21	2.00	7.21	46.30
108	35.71	9.15	2.00	7.15	46.30
110	35.20	9.02	2.00	7.02	46.30

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
	46.30	52.44	0	0.00

Drainage Area		2,3,4			
Area (Ha)		0.310			
C =		0.90		Restricted Flow Q _r (L/s)= 25.00	
5-Year Ponding					
T _c Variable (min)	i _{5yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{5yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
13	90.63	70.29	25.00	45.29	35.33
15	83.56	64.81	25.00	39.81	35.83
16	80.46	62.41	25.00	37.41	35.91
17	77.61	60.19	25.00	35.19	35.90
19	72.53	56.25	25.00	31.25	35.63

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	35.91	81.48	15.75	0.00

IN-LINE STORAGE (Pipe)			
Pipe storage			
Structure to Structure	Length (m)	Dia (m)	Storage (m3)
MHSTM121 - MHSTM122	97.50	0.38	10.77
MHSTM122 - MHSTM123	100.00	0.45	15.90
Total:			26.67

overflows to: offsite

Drainage Area		5			
Area (Ha)		0.130			
C =	0.86	Restricted Flow Q_r (L/s)= 27.00			
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	$Peak\ Flow$ $Q_p = 2.78 \times C i_{5yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 5yr (m^3)
2	182.69	56.78	27.00	29.78	3.57
4	152.51	47.40	27.00	20.40	4.90
5	141.18	43.88	27.00	16.88	5.06
6	131.57	40.89	27.00	13.89	5.00
8	116.11	36.09	27.00	9.09	4.36

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	5.06	26.06	0	0.00

overflows to: offsite

Drainage Area		5A			
Area (Ha)		0.090			
C =	0.70	Restricted Flow Q_r (L/s)= 2.00			
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	Q_p Peak Flow $Q_p = 2.78 \times C i_{5yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 5yr (m^3)
40	44.18	7.74	2.00	5.74	13.77
42	42.68	7.48	2.00	5.48	13.80
43	41.97	7.35	2.00	5.35	13.81
44	41.29	7.23	2.00	5.23	13.81
46	39.99	7.00	2.00	5.00	13.81

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	13.81	40.66	0	0.00

Drainage Area		5B			
Area (Ha)		0.110			
C =	0.67	Restricted Flow Q_r (L/s)= 2.00			
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	Peak Flow $Q_p=2.78xCi_{5yr}A$ (L/s)	Q_r (L/s)	Q_p-Q_r (L/s)	Volume 5yr (m^3)
48	38.78	7.95	2.00	5.95	17.12
50	37.65	7.71	2.00	5.71	17.14
51	37.12	7.60	2.00	5.60	17.15
52	36.59	7.50	2.00	5.50	17.15
54	35.60	7.29	2.00	5.29	17.15

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	17.15	52.44	0	0.00

Drainage Area		6			
Area (Ha)	0.070				
C =	1.00	Restricted Flow Q_r (L/s)= 22.00			
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_o = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
4	262.41	51.06	22.00	29.06	6.98
6	226.01	43.98	22.00	21.98	7.91
7	211.67	41.19	22.00	19.19	8.06
8	199.20	38.76	22.00	16.76	8.05
10	178.56	34.75	22.00	12.75	7.65

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	8.06	5.32	0	2.74

overflows to: 5

Drainage Area		17, 19 & 7			
Area (Ha)	0.580				
C =	0.83	Restricted Flow Q _r (L/s)= 30.00			
100-Year Ponding					
T _c Variable (min)	i _{100yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{100yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 100yr (m ³)
36	80.96	108.24	30.00	78.24	169.01
37	79.42	106.18	30.00	76.18	169.11
38	77.93	104.19	30.00	74.19	169.16
39	76.51	102.29	30.00	72.29	169.16
41	73.83	98.71	30.00	68.71	169.03

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
8.80	177.96	212.44	0.00	0.00

overflow from: 20, 19

overflows to: 6

Drainage Area		20			
Area (Ha)		0.090			
C =		1.00			
		Restricted Flow Q_r (L/s)=	19.80 Existing		
100-Year Ponding					
T_c		Peark Flow			
Variable	i_{100yr}	$Q_p = 2.78xCi_{100yr}A$	Q_r	$Q_p - Q_r$	Volume
(min)	(mm/hour)	(L/s)	(L/s)	(L/s)	100yr
8	199.20	49.84	19.80	30.04	14.42
10	178.56	44.68	19.80	24.88	14.93
11	169.91	42.51	19.80	22.71	14.99
12	162.13	40.57	19.80	20.77	14.95
14	148.72	37.21	19.80	17.41	14.62

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	14.99	7.20	0.00	7.79

overflows to: 7

Drainage Area		12, 13				
Area (Ha)		0.200				
C =		1.00	Restricted Flow Q_r (L/s)=		33.00	
100-Year Ponding						
T_c Variable (min)	i_{100yr} (mm/hour)	Peark Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	
12	162.13	90.15	33.00	57.15	41.15	
14	148.72	82.69	33.00	49.69	41.74	
15	142.89	79.45	33.00	46.45	41.80	
16	137.55	76.48	33.00	43.48	41.74	
18	128.08	71.21	33.00	38.21	41.27	

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
3.08	44.89	43.88	0.00	1.01

overflow from: 10.00

overflows to: 19

Drainage Area		10			
Area (Ha)		0.040			
C =	1.00	Restricted Flow Q_r (L/s)= 18.00			
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
2	315.00	35.03	18.00	17.03	2.04
4	262.41	29.18	18.00	11.18	2.68
5	242.70	26.99	18.00	8.99	2.70
6	226.01	25.13	18.00	7.13	2.57
8	199.20	22.15	18.00	4.15	1.99

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
5.02	7.71	3.63	1.00	3.08

overflow from: 9.00

overflows to: 12

Drainage Area		6			
Area (Ha)	0.070				
C =	0.84	Restricted Flow Q _r (L/s)= 22.00			
5-Year Ponding					
T _c Variable (min)	i _{5yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{5yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
1	203.51	33.27	22.00	11.27	0.68
2	182.69	29.86	22.00	7.86	0.94
3	166.09	27.15	22.00	5.15	0.93
5	141.18	23.08	22.00	1.08	0.32

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	0.94	5.32	0	0.00

overflows to: 5

Drainage Area		17, 19 & 7			
Area (Ha)		0.580			
C =		0.66	Restricted Flow Q _r (L/s)=		30.00
5-Year Ponding					
T _c Variable (min)	i _{sy} (mm/hour)	Peak Flow Q _p = 2.78xCi _{sy} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
16	80.46	86.06	30.00	56.06	53.81
18	74.97	80.19	30.00	50.19	54.20
19	72.53	77.57	30.00	47.57	54.23
20	70.25	75.14	30.00	45.14	54.17
22	66.15	70.75	30.00	40.75	53.79

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	54.23	212.44	0	0.00

overflows to: 6

Drainage Area		20			
Area (Ha)		0.090			
C =	0.90	Restricted Flow Q _r (L/s)= 19.80			
5-Year Ponding					
T _c Variable (min)	i _{5yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{5yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
2	182.69	41.14	19.80	21.34	2.56
4	152.51	34.34	19.80	14.54	3.49
5	141.18	31.79	19.80	11.99	3.60
6	131.57	29.63	19.80	9.83	3.54
8	116.11	26.15	19.80	6.35	3.05

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	3.60	7.20	0	0.00

overflows to: 7

Drainage Area		12, 13			
Area (Ha)		0.200			
C =	0.90	Restricted Flow Q _r (L/s)= 33.00			
5-Year Ponding					
T _c Variable (min)	i _{5yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{5yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
4	152.51	76.32	33.00	43.32	10.40
6	131.57	65.84	33.00	32.84	11.82
7	123.30	61.70	33.00	28.70	12.05
8	116.11	58.10	33.00	25.10	12.05
10	104.19	52.14	33.00	19.14	11.48

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	12.05	43.88	0	0.00

overflows to: 19

Drainage Area		10			
Area (Ha)		0.040			
C =	0.30	Restricted Flow Q_r (L/s)=		18.00	
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{5yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 5yr (m^3)
-5	956.98	31.92	18.00	13.92	-4.18
-3	402.34	13.42	18.00	-4.58	0.82
-2	319.47	10.66	18.00	-7.34	0.88
-1	266.98	8.91	18.00	-9.09	0.55
1	203.51	6.79	18.00	-11.21	-0.67

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	0.88	3.63	1	0.00

overflows to: 12

Drainage Area		9			
Area (Ha)	0.080				
C =	1.00	Restricted Flow Q_r (L/s)= 28.00			
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
3	286.05	63.62	28.00	35.62	6.41
5	242.70	53.98	28.00	25.98	7.79
6	226.01	50.26	28.00	22.26	8.02
7	211.67	47.07	28.00	19.07	8.01
9	188.25	41.87	28.00	13.87	7.49

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	8.02	3.00	0.00	5.02

overflows to: 10

Drainage Area		9			
Area (Ha)	0.080	Restricted Flow Q_r (L/s)= 28.00			
C =	0.30				
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{5yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 5yr (m^3)
-5	956.98	63.85	28.00	35.85	-10.75
-3	402.34	26.84	28.00	-1.16	0.21
-2	319.47	21.32	28.00	-6.68	0.80
-1	266.98	17.81	28.00	-10.19	0.61
1	203.51	13.58	28.00	-14.42	-0.87

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	0.80	3.00	0	0.00

overflows to: 10

Drainage Area		PHASE 2			
Area (Ha)	0.250	Restricted Flow Q_r (L/s)= 18.00			
C =	1.00				
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
30	91.87	63.85	18.00	45.85	82.53
32	87.89	61.08	18.00	43.08	82.71
33	86.03	59.79	18.00	41.79	82.75
34	84.27	58.57	18.00	40.57	82.75
36	80.96	56.27	18.00	38.27	82.66

Storage (m^3)				
Overflow	Required	Cistern	Sub-surface	Balance
0.00	82.75	120.00	0.00	0.00

Drainage Area		PHASE 2			
Area (Ha)	0.250	Restricted Flow Q_r (L/s)= 18.00			
C =	0.90				
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{5yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 5yr (m^3)
15	83.56	52.27	18.00	34.27	30.84
17	77.61	48.54	18.00	30.54	31.15
18	74.97	46.89	18.00	28.89	31.21
19	72.53	45.36	18.00	27.36	31.20
21	68.13	42.62	18.00	24.62	31.02

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	31.21	120.00	0	0.00

Drainage Area		PHASE 2 Cistern sizing only			
Area (Ha)		0.250			
C =		1.00		Restricted Flow Q_r (L/s)= 9.00	
100-year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
62	54.54	37.91	9.00	28.91	107.54
64	53.26	37.02	9.00	28.02	107.59
65	52.65	36.59	9.00	27.59	107.60
66	52.05	36.17	9.00	27.17	107.60
68	50.89	35.37	9.00	26.37	107.58

Storage (m^3)				
Overflow	Required	Cistern	Sub-surface	Balance
0.00	107.60	120.00	0.00	0.00

Drainage Area		PHASE 2			
Area (Ha)		0.250			
C =		0.90		Restricted Flow Q_r (L/s)= 9.00	
5-Year Ponding					
T_c Variable (min)	i_{5yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{5yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 5yr (m^3)
33	50.53	31.61	9.00	22.61	44.76
35	48.52	30.35	9.00	21.35	44.83
36	47.58	29.76	9.00	20.76	44.84
37	46.67	29.20	9.00	20.20	44.83
39	44.98	28.14	9.00	19.14	44.78

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	44.84	120.00	0	0.00

Drainage Area		11			
Area (Ha)	0.050				
C =	1.00	Restricted Flow Q_r (L/s)= 2.10			
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78Ci_{100yr}A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
53	61.28	8.52	2.10	6.42	20.41
54	60.44	8.40	2.10	6.30	20.41
55	59.62	8.29	2.10	6.19	20.42
56	58.83	8.18	2.10	6.08	20.42
57	58.07	8.07	2.10	5.97	20.42

Storage (m^3)				
Overflow	Required	Roof	Sub-surface	Balance
0.00	20.42	22.00	0.00	0.00

Drainage Area		11			
Area (Ha)		0.050		Restricted Flow Q _r (L/s)= 2.10	
C =		0.90			
5-Year Ponding					
T _c Variable (min)	i _{5yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{5yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
28	56.49	7.07	2.10	4.97	8.34
30	53.93	6.75	2.10	4.65	8.36
31	52.74	6.60	2.10	4.50	8.37
32	51.61	6.46	2.10	4.36	8.36
34	49.50	6.19	2.10	4.09	8.35

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	8.37	22.00	0	0.00

Drainage Area 14 (TWR A)					
Area (Ha)	0.210	Restricted Flow Q_r (L/s)= 36.00			
C =	1.00				
100-Year Ponding					
T_c Variable (min)	i_{100yr} (mm/hour)	$Peak\ Flow$ $Q_p = 2.78xCi_{100yr}A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)
12	162.13	94.65	36.00	58.65	42.23
13	155.11	90.55	36.00	54.55	42.55
14	148.72	86.82	36.00	50.82	42.69
15	142.89	83.42	36.00	47.42	42.68
16	137.55	80.30	36.00	44.30	42.53

Storage (m^3)				
Overflow	Required	Cistern	Sub-surface	Balance
0.00	42.69	48.00	0.00	0.00

Drainage Area 14 (TWR A)					
Area (Ha)	0.210				
C =	0.90				
Restricted Flow Q _r (L/s)= 36.00					
5-Year Ponding					
T _c Variable (min)	i _{5yr} (mm/hour)	Peak Flow Q _p =2.78xCi _{5yr} A (L/s)	Q _r (L/s)	Q _p -Q _r (L/s)	Volume 5yr (m ³)
4	152.51	80.13	36.00	44.13	10.59
6	131.57	69.13	36.00	33.13	11.93
7	123.30	64.79	36.00	28.79	12.09
8	116.11	61.01	36.00	25.01	12.00
10	104.19	54.75	36.00	18.75	11.25

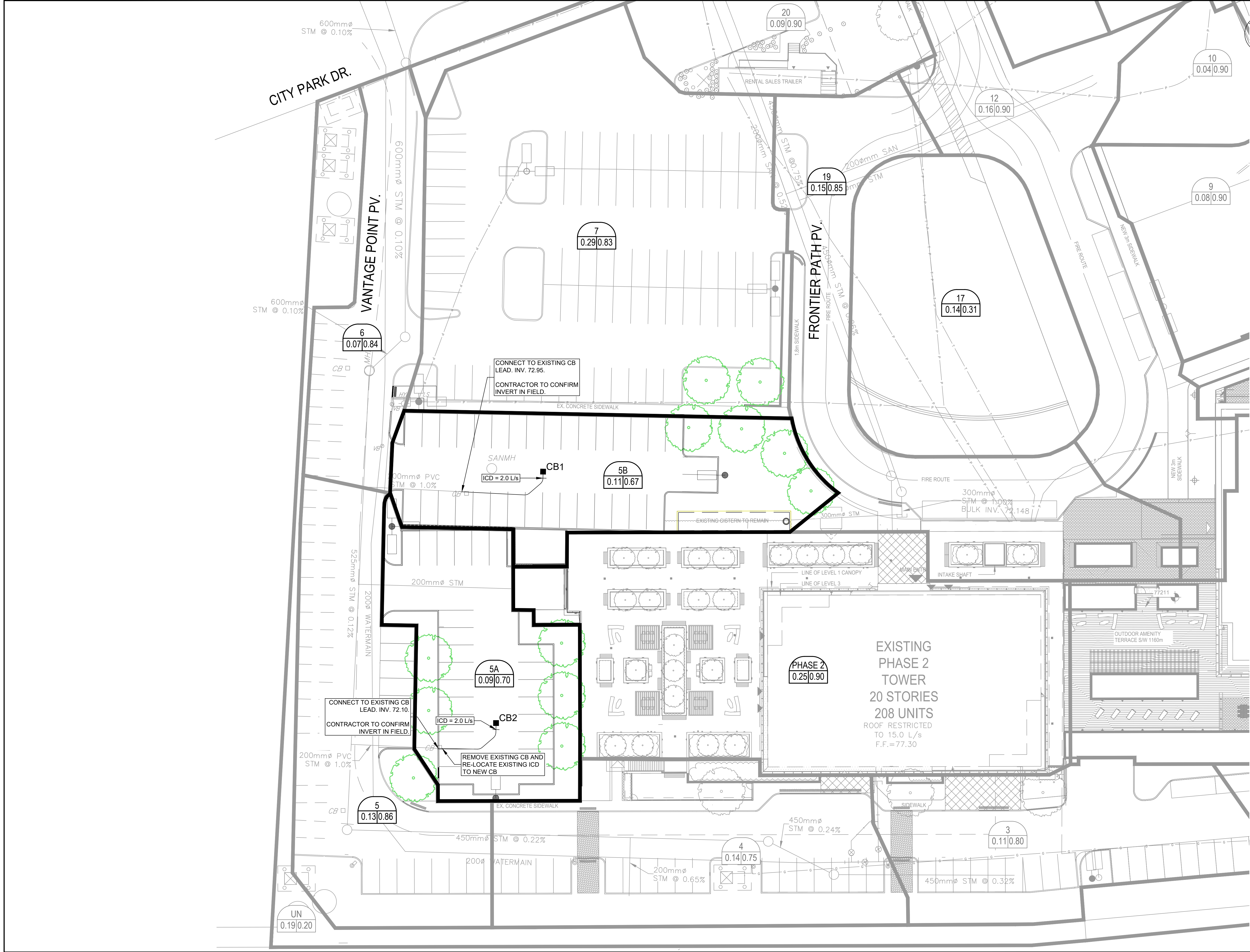
Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	12.09	48.00	0	0.00



City Park Drive Rio-Can Redevelopment
City of Ottawa
RioCan Management Inc.

**PHASE 2
INTERIM PARKING**

Definitions: Q = 2.78CIA, where: Q = Peak Flow in Litres per Second (L/s) A = Area in Hectares (Ha) i = Rainfall intensity in millimeters per hour (mm/hr) [i = 998.071 / (TC+6.053)*0.814] 5 YEAR [i = 1174.184 / (TC+6.014)*0.816] 10 YEAR [i = 1735.688 / (TC+6.014)*0.820] 100 YEAR	Notes: 1. Mannings coefficient (n) = 0.013 2. Existing pipe diameters taken from OMM design brief dated February 1999 3. Existing pipe lengths and slopes taken from field survey data by Stantec and IBI Group	Designed:	Designed: MAP				
		Checked:	Checked: TRB	No.	Revision		
				1.	City submission No. 1		
				2.	City submission No. 3		
				3.	As-Built		
		Dwg. Reference:	38729-500	4.	Interim Parking SPRA No. 1		
				File Reference:		Date:	Sheet No:
				38729.5.7.1		2025-08-21	1 of 1



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No.	DESCRIPTION	DATE
1	ISSUED FOR SPRA	2025-08-21

CONSULTANTS

1:250

SEAL

PRIME CONSULTANT

333 Preston Street - Suite 500
Ottawa ON K1S 5N4 Canada
tel 613 225 1311
www.arcadis.com

PROJECT

KILLAM - CITY PARK PHASE 2
INTERIM PARKING

200 FRONTIER PATH PVT.

PROJECT NO:

38729

DRAWN BY:

D.D. / E.H.

CHECKED BY:

M.P.

PROJECT MGR:

T.R.B.

APPROVED BY:

T.R.B.

SHEET TITLE

STORM DRAINAGE AREA PLAN

SHEET NUMBER

C-502

ISSUE

1



Killam
PROPERTIES INC

Hobin Architecture Incorporated

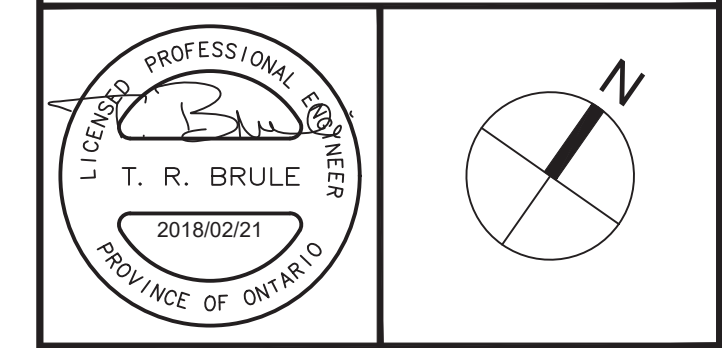
63 Farnham Street
Ottawa, Ontario
Canada K1S 3K7

T: 613 238 7200
F: 613 235 2056
E: info@hobin.com

hobin
ARCHITECTURE

Project Title

2280 CITY PARK
DEVELOPMENT
PHASE 2

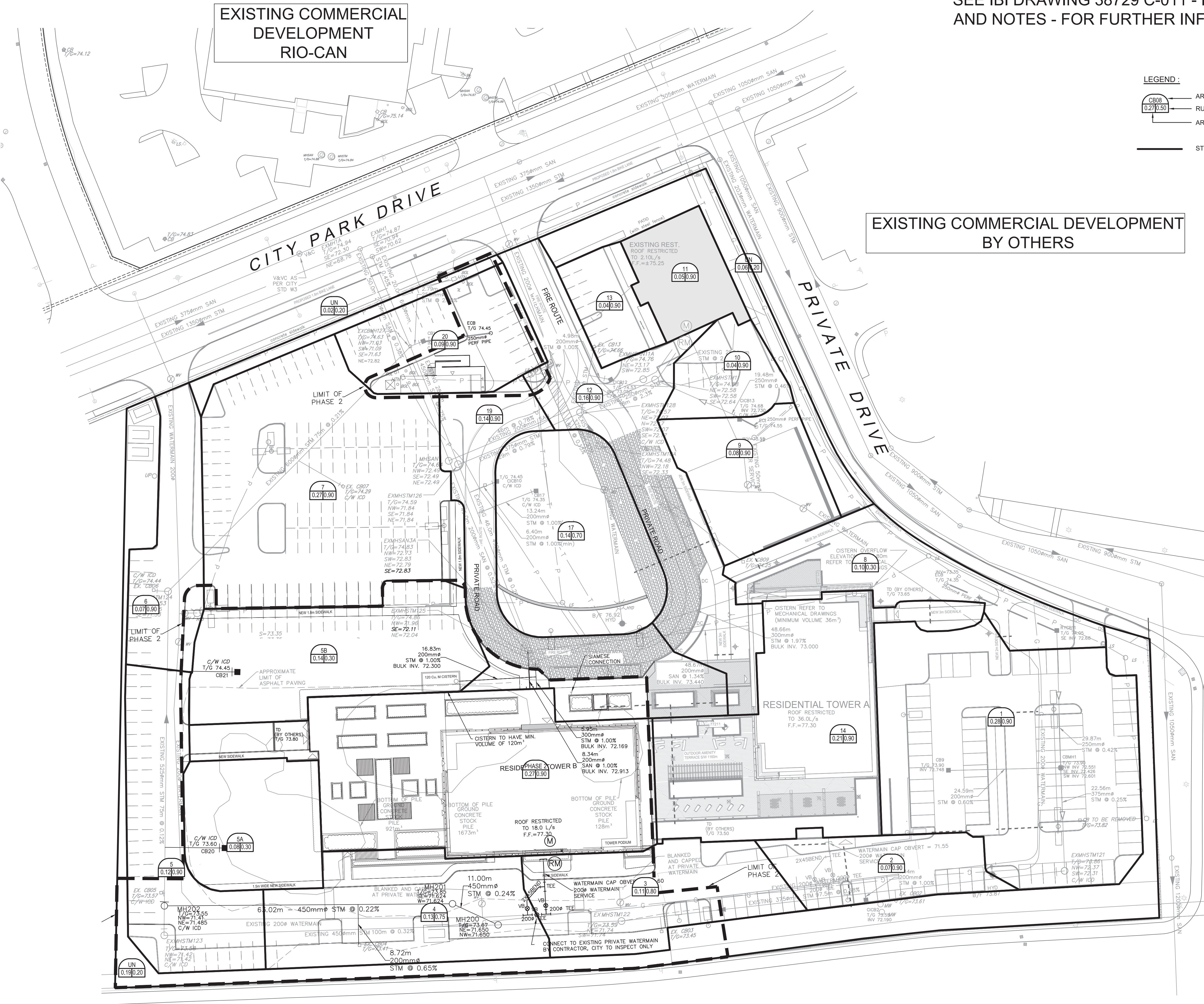


STORM DRAINAGE
AREA PLAN

Scale	1 : 400
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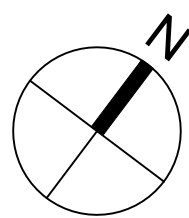
Design J.E.B.	Date AUG. 2018
Drawn E.H.	Checked T.R.B.
Project No. 38729	Drawing No. C-501

D07-12-18-0122



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Appendix C



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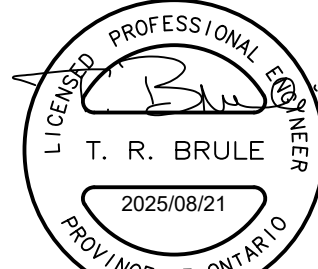
ISSUES		
No.	DESCRIPTION	DATE
1	ISSUED FOR SPRA	2025-08-21



CONSULTANTS



SEAL



PRIME CONSULTANT



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Ottawa ON K1S 5N4 Canada
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PROJECT
KILLAM - CITY PARK PHASE 2
INTERIM PARKING

200 FRONTIER PATH PVT.

PROJECT NO:
38729

DRAWN BY:
D.D. / E.H.

CHECKED BY:
M.P.

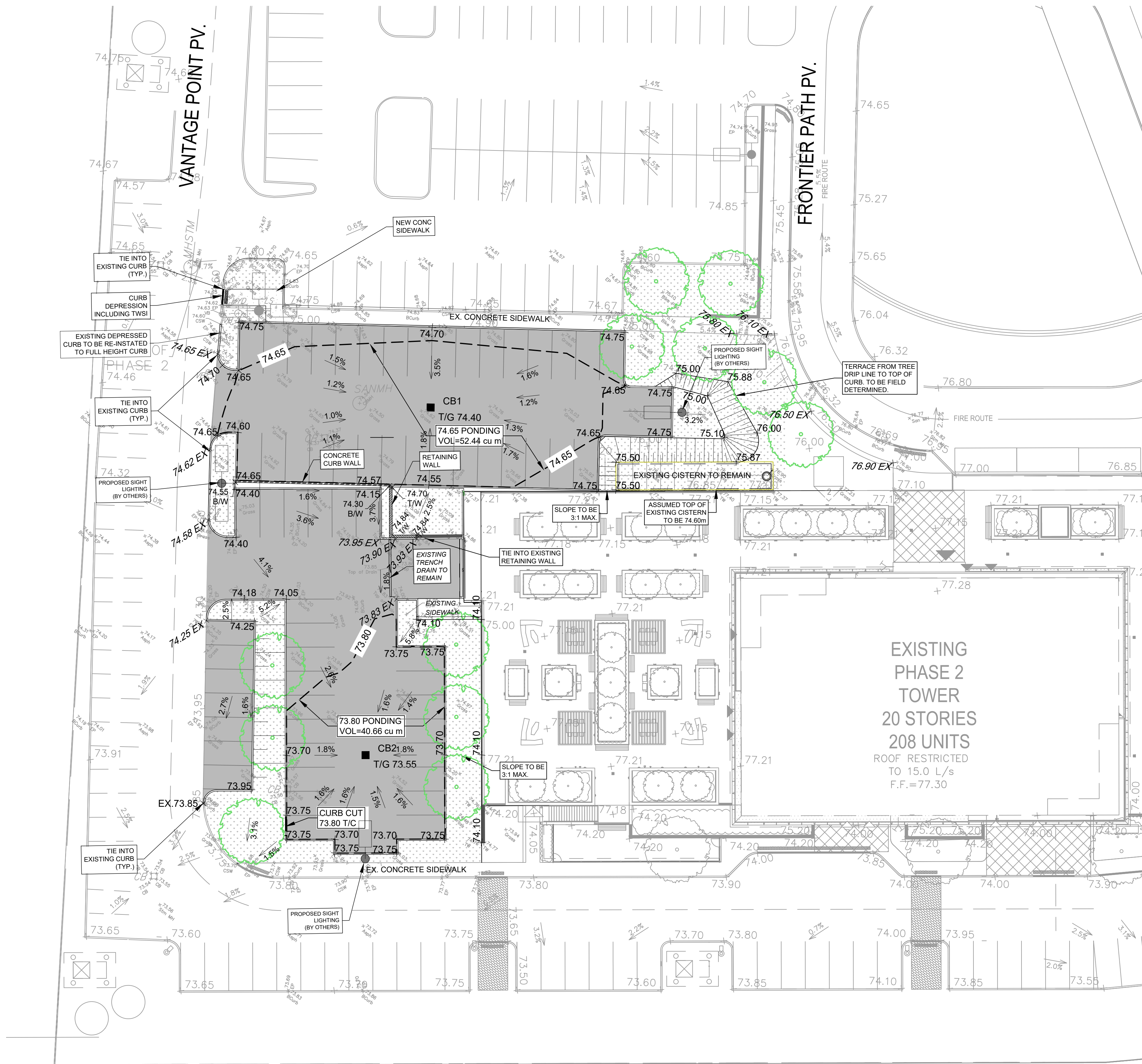
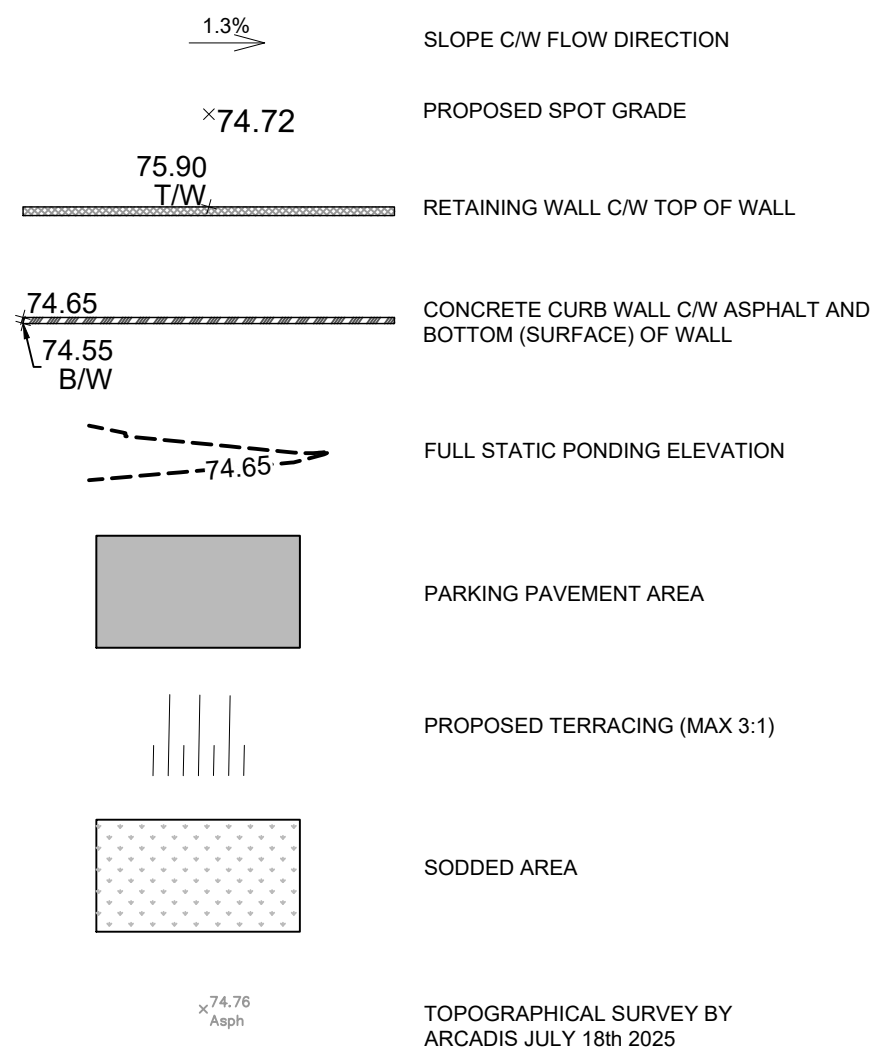
PROJECT MGR:
T.R.B.

APPROVED BY:
T.R.B.

SHEET TITLE
GRADING PLAN

SHEET NUMBER C-202	ISSUE 1
-------------------------------------	--------------------------

GRADING LEGEND

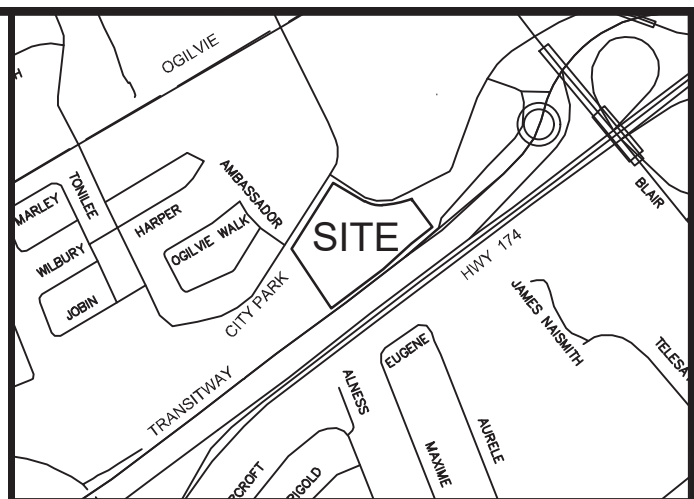


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D07-xx-xx-xxxx

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SEE IBI DRAWING 38729 C-011 - DETAILS
AND NOTES - FOR FURTHER INFORMATION

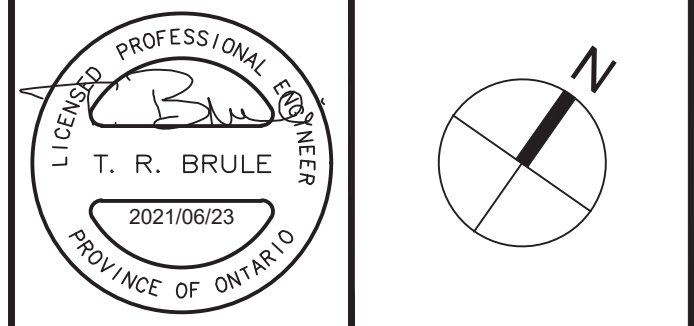


No.	REVISIONS	By	Date
14			
13			
12	REVISED PHASE 2 GRADING	TRB	21:06:23
11	UPDATED ASPHALT AREAS	TRB	20:11:12
10	ISSUED FOR CONSTRUCTION	TRB	19:12:10
9	RE-ISSUED FOR CONSTRUCTION	TRB	19:09:24
8	RE-ISSUED FOR CONSTRUCTION	TRB	19:03:29
7	ISSUED FOR CONSTRUCTION	TRB	19:02:21
6	REVISED PER CITY COMMENTS	TRB	18:12:12
5	REVISED PH. 2 EXISTING GROUND GRADES	TRB	18:12:03
4	ISSUED FOR TENDER	TRB	18:11:22
3	REVISED PER CITY COMMENTS	TRB	18:11:09
2	ISSUED FOR 66% REVIEW	TRB	18:10:31
1	ISSUED TO CITY	TRB	18:08:08



IBI IBI GROUP
400 - 303 Preston Street
Ottawa ON K1S 5N4 Canada
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ibigroup.com

Project Title
**2280 CITY PARK DEVELOPMENT
PHASE 2**



Drawing Title
SITE GRADING

Scale
1 : 400

Design	J.E.B.	Date	AUG. 2018
Drawn	E.H.	Checked	T.R.B.
Project No.	38729	Drawing No.	C-201

ALL ASPHALT AND
CURB WORKS
WITHIN THE LIMIT
OF PHASE 2 TO BE
NEW AND/OR
RE-BUILT

