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Site Servicing Report

Mixed-Use Site Plan (Block 15)
2983, 3053 and 3079 Navan Road & 2690 Pagé Road



Site Servicing Report

2983, 3053 and 3079 Navan Road & 2690 Pagé Road

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1.0 INTRODUCTION

1.1 General

In 2023, J.L. Richards & Associates Limited (JLR) was retained by 12714001 Canada Inc. (the Owner) to prepare the detailed design of municipal infrastructure for Site Plan Approval (SPA) of the Mixed-Use Site Plan (Block 15). This Site Servicing Report (SSR) presents the servicing constraints and strategies for water, wastewater, stormwater servicing, and stormwater management in accordance with the City of Ottawa Design Guidelines, the associated technical bulletins and relevant design excerpts.

1.2 Site Description

The Mixed-Use Site Plan (Block 15) is located within the City of Ottawa's Official Plan boundary and consists of a 0.54 ha parcel bounded by Brian Coburn to the northwest and the proposed East Ridge Orleans Subdivision in the remaining directions (refer to the East Ridge Orleans Subdivision Site Servicing Report prepared by J.L. Richards and Associates, dated September 22, 2023). The legal description of the subject property can be found in the Draft Plan of Subdivision attached to Appendix A.

A topographical survey was completed by Stantec Inc. in August 2023 (Appendix A). The survey indicates that a portion of Block 15 adjacent to Brian Coburn conveys overland flow to existing ditch inlet catch basins along Brian Coburn. The remaining existing ground surface generally slopes in a southeasterly direction towards the future subdivision lands.

1.3 Proposed Development

The proposed development will consist of two (2) mid-rise residential condominiums with underground parking and one (1) storey of commercial space. One (1) of the condominium buildings consists of 47 units, and the second (2) consists of 36 units. Mixed-Use Site Plan (Block 15) has a total of 83 units. The Concept Plan for the Mixed-Use Site Plan (Block 15) is attached to Appendix A.

1.4 Proposed Connections to Existing Infrastructure

Block 15 is proposed to be serviced by the infrastructure that is part of the East Ridge Orleans Subdivision. One sanitary, storm and water service will extend to the parking garage footprint from the subdivision as shown in the servicing drawings.

1.5 Consultation and Permits

A pre-consultation meeting was held on September 13, 2023, to discuss the planning process, design criteria, and servicing constraints. A copy of the pre-consultation meeting notes and the site servicing checklist has been provided in Appendix B.

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2.0 WATER SERVICING

2.1 Water Supply Design Criteria

A Hydraulic Network Analysis (HNA) was completed as part of the detailed design for the East Ridge Orleans subdivision to confirm that the proposed watermains could provide adequate supply while complying with both the Ottawa Design Guidelines for Water Distribution (July 2010) and Technical Bulletins ISDTB-2014-02, ISTB-2018-02 and ISTB-2021-03. These documents are herein referred to as the Design Guidelines and TB-2014-02, TB-2018-02, and TB-2021-03, respectively.

The HNA completed as part of the East Ridge Orleans Subdivision design included water demands for the Mixed Used Site Plan (Block 15). The HNA has since been updated to reflect the proposed water service lateral for Block 15 but is based on the same demands and the boundary conditions used in the original East Ridge Orleans Subdivision HNA (refer to Appendix C for a copy of City correspondence for boundary conditions).

Section 4.2.2 of the Design Guidelines states the following criteria for development additions to the public water distribution system:

- Under maximum hourly demand conditions (peak hour), the residual pressures shall not be less than 276 kPa (40 psi);
- During periods of maximum day and fire flow demand, the residual pressure at any point in the distribution system shall not be less than 140 kPa (20 psi);
- In accordance with the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi);
- The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi); and
- Feedermains, which have been provided primarily for the purpose of redundancy, shall meet, at a minimum, the basic day plus fire flow demand.

2.2 Domestic Water Demands

The water demands presented in this section are based on the site layout and unit count shown in the Site Plan (Appendix A) and commercial flows. Domestic water demands were calculated for 83 apartment units with an average density of 1.8 persons per unit, giving a total population of 149 people. Commercial flows were calculated for a commercial area of 0.09 ha.

The residential and commercial consumption rates for average day, maximum day, and peak hour demand were set in accordance with Table 4-2 of the Design Guidelines. Table 1 summarizes the water consumption rates and peaking factors used in the HNA.

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Table 1: Water Demands

Demand Scenario	Residential Water Consumption or Peaking Factor	Commercial Water Consumption or Peaking Factor	Total Demands (L/s)
Average Day Demand	280 L/c/d	28,000 L/ha/d	0.52
Maximum Day Demand	2.5 x Avg Day	1.5 x Avg Day	1.26
Peak Hour Demand	2.2 x Max Day	1.8 x Max Day	2.74

2.3 Fire Flow Requirements

The City has specified that the Fire Underwriters Survey (FUS) method shall be used for any public or private site where new fire hydrants are being designed. Specifically, the required fire flow (RFF) for each structure was calculated in accordance with TB-2018-02.

The required fire flow for the Mixed-Use Site Plan (Block 15) was calculated as 200 L/s for Building C and 217 L/s for Building D. Refer to Appendix C for the detailed RFF calculations for the critical fire area.

Both buildings within Block 15 will be equipped with a fully supervised automatic sprinkler designed and installed in accordance with NFPA 13. In the analysis, a sprinkler flow of 1500 L/min (25 L/s) was applied to each building as recommended by the Owner's Mechanical Engineer (refer to Appendix C).

2.4 Proposed Water Servicing, Boundary Conditions and Water Model

2.4.1 Proposed Water Servicing

Water will be supplied to the Mixed-Use Site Plan (Block 15) by a 150 mm diameter water service that will connect to the 200 mm diameter watermain loop within the East Ridge Orleans subdivision. Fire protection will be provided by each building's sprinkler system and the proposed hydrants within the East Ridge Orleans Subdivision. The Siamese connection for each building is shown on the Servicing Drawings and is located no more than 45 m away from a hydrant as required by the OBC (refer to hydrant coverage markup in Appendix C).

2.4.2 Boundary Conditions

Hydraulic boundary conditions were provided by the City at the two proposed connection locations for the East Ridge Orleans subdivision (Connection 1 and Connection 2). Table 2 summarizes the hydraulic boundary conditions received (refer to Appendix C for a copy of the City correspondence).

The boundary condition for 233 L/s was used in this analysis since this is above the maximum required fire flow for the site (refer to Section 2.3).

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Table 2: Hydraulic Boundary Conditions

Demand Scenarios	Connection 1 Head (m)	Connection 2 Head (m)
Maximum HGL	130.7	130.7
Peak Hour	127.0	126.8
Max Day plus Fire Flow #3 13,980 L/min (233.00 L/s)	124.9	122.7

2.4.3 Water Model

A hydraulic water model within the WaterCAD® software platform was used to carry out the HNA (refer to the overall schematics presented in Appendix C). The water demands from Table 1 and the boundary conditions from Table 2 were input into the model for each demand scenario. Table 3 summarizes the watermain diameters and roughness coefficients used in the model, based on Sections 4.2.12 and 4.3.5 of the Design Guidelines.

Table 3: Watermain Internal Diameters and C-Factors

Nominal Diameter	Inside Diameter	C-Factor
150 mm	155 mm	100
200 mm	204 mm	110
300 mm	297 mm	120

2.5 Simulation Results

The HNA was carried out under steady-state peak hour, maximum day plus fire flow, and maximum pressure conditions to confirm that the proposed water servicing can meet the design criteria outlined in Section 2.1.

2.5.1 Peak Hour

The simulation results found the minimum pressure at the site (node J-23 in the model) during the peak hour condition to be 406 kPa (58.9 psi) (refer to Appendix C), which exceeds the minimum pressure criterion of 276 kPa (40 psi) per the Design Guidelines.

2.5.2 Maximum Day Plus Fire Flow

Fire water supply will be provided by the fully automatic sprinkler system and the hydrants within the East Ridge Orleans subdivision. To ensure adequate fire protection, the maximum day demand shown in Table 1 was analyzed simultaneously with the fire flow requirements. As mentioned in Section 2.3, a sprinkler system flow of 1500 L/min (25 L/s)

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was assumed for Block 15 and it was assumed that only one (1) building would require fire flow at once.

Once the maximum day demands, the sprinkler system demands, and the 14,000 L/min (233 L/s) boundary condition provided by the City (refer to Table 2) were input in the model, the fire flow simulation was carried out by allowing WaterCAD® to calculate the available fire flow that can be drawn from a hydrant without allowing any part of the system to experience pressures less than 140 kPa (20 psi).

From the simulation results, the system is expected to deliver a minimum of 14,000 L/min (233 L/s) to the site through the contribution of the sprinkler system and the nearby hydrants. Attached to Appendix C is a map of the hydrant coverage for Block 15 which confirms that both buildings within the block meet the RFF of 233 L/s through the aggregate sum of hydrant flows. Based on Appendix I of TB-2018-02 hydrants within 75 m of a building can provide 95 L/s of fire flow and hydrants within 75 to 150 m of a building can provide 63 L/s of fire flow.

2.5.3 Maximum Pressure

Based on a zero (0 L/s) demand condition, the simulation results found the maximum pressure at the site to be 444 kPa (64.40 psi). This value is below the maximum pressure constraint of 552 kPa (80 psi), therefore pressure reducing valves (PRVs) are not anticipated to be required.

2.6 Summary and Conclusions

Based on the water simulation results, the proposed development can be serviced by the proposed 150 mm water service lateral connected to the 200 mm watermain loop within the East Ridge Orleans Subdivision. Furthermore, adequate fire water supply can be achieved with the proposed hydrants within the East Ridge Orleans Subdivision.

3.0 WASTEWATER SERVICING

3.1 Design Criteria

The sanitary sewer system within the Mixed-Use Site Plan (Block 15) is designed in accordance with the Ottawa Sewer Design Guidelines and subsequent technical bulletins. The design parameters are applied under two scenarios as per ISTB Technical Bulletin 2018-01. The key design parameters have been summarized in the table below.

Table 4: Wastewater Key Design Parameters

Design Parameter	Design Value
Commercial Average Flow	28,000 L/gross ha/Day
Residential Average Flow	280 L/Cap/Day
Residential Peaking Factor	Harmon's Formula
Commercial Peaking Factor	1.5
Harmon's Correction Factor (K)	0.8
Infiltration Allowance	0.33 L/s/ha

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Design Parameter	Design Value
Manning's Roughness Coefficient (n)	0.013
Allowable Slopes	Varies (Refer to Section 6.1.2.2 of ODSG)
Allowable Velocities	0.6 m/s – 3.0 m/s
Allowable Freeboard	-

3.2 Proposed Sanitary Servicing and Design Flows

Wastewater generated from Block 15 will be conveyed via a proposed 200 mm diameter sanitary service lateral, which will then discharge into the East Ridge Orleans Subdivision. Based on the design criteria above and the site constraints, a total design peak flow of 1.95 L/s is calculated for the development. The table below summarizes the peak sanitary flow for the site plan. A detailed design sheet can be found in Appendix E.

Table 5: Sanitary Design Flow Summary

Area	Site Area	Unit Count	Unit Density	Pop.	Harmon's Peaking Factor	Res. Peak Flow	Comm. Peak Flow	Infilt. Flow	Total Flow
Block 15	0.54 ha	83	1.8 ppu	149 persons	3.55	1.72 L/s	0.05 L/s	0.18 L/s	1.95 L/s

A 200 mm diameter sanitary service lateral is expected to have sufficient capacity to convey the wastewater flows for the site. The flows from this block were already incorporated in the design of the sanitary sewer for the subdivision (refer to the East Ridge Orleans Subdivision Site Servicing Report prepared by J.L. Richards and Associates, dated February 16, 2024). As shown in this report, the subdivision will consist of 200 mm diameter pipes which will have, at minimum, 20.24 L/s of capacity. Furthermore, the overall subdivision inclusive of the Site Plan Blocks 14, 15 and 17 is expected to generate 12.98 L/s of wastewater flows which is less than the minimum expected capacity of the pipes by 35%. Therefore, it is anticipated that the sanitary sewer system downstream will have sufficient capacity for the 1.95 L/s of wastewater flows generated by Block 15.

Furthermore, it has been confirmed by the Mechanical Engineer on file that a 200mm diameter service will be sufficient to convey the flows generated by the mechanical fixtures (refer to Appendix E).

3.3 Summary and Conclusions

Wastewater servicing for Block 15 will be designed in accordance with the City of Ottawa Sewer Design Guidelines, the associated technical bulletins, and various background documents as highlighted throughout this section. Wastewater will be conveyed via a proposed 200 mm diameter sanitary service lateral which will then discharge into the East Ridge Orleans Subdivision.

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4.0 STORM SERVICING AND STORMWATER MANAGEMENT

4.1 Design Criteria

Storm and stormwater management servicing for the Mixed-Use Site Plan (Block 15) was developed in accordance with the City of Ottawa 2012 Sewer Design Guidelines (OSDG) and the more recent Technical Bulletin PIEDTB-2016-01 (September 6, 2016). These two documents are herein referred to as the Design Guidelines in this section. A summary of the key storm and stormwater management criteria follows:

- Control minor system flows to the allowable release rates of 44 L/s as identified in Table 5-4 Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2024);
- The runoff coefficients (C-factors) to be calculated based on the ratio of pervious and impervious surfaces depicted on proposed site plans;
- Minimum roadway slope of 0.1% from crest-to-crest for overland flow route;
- Minimum rear yard slope in the absence of perforated pipe system of 1.5% along with swale side slopes of 3 horizontal to 1 vertical;
- Maximum parking ponding depth of 350 mm (static and dynamic) as per the Design Guidelines and maximum depth of surface flow to be 300 mm;
- Minimum vertical clearance of 0.15 m between the spill elevation on the street and the finished grade (garage elevation);
- Minimum vertical clearance of 0.30 m between the surface spill elevation and the ground elevation at the building in the rear yards;
- Major system flows, up to and including the 1:100-year design storm event, are contained within the site using the parking lot area and surface drainage.
- Quality control will be accommodated by Pond #3 to meet an MECP Enhanced Level of Protection (80% TSS removal) as identified in Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2024).
- Provide measures to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

4.2 Proposed Stormwater Management Approach

In order to achieve the allowable release rates, the stormwater management of the site will include:

- Rooftop control with rooftop storage and released at a cumulative controlled release rate of 2 L/s for each building; per WATTS RD-200 detail for Small Area Roof Drain (Appendix D) and as recommended by the landowner's mechanical engineer.
- Surface storage within the parking lots in greater than a 1:2-year event with captured flows conveyed to the building plumbing system and internal cistern;
- Ramp runoff captured and conveyed to the building plumbing system and internal cistern;
- Storage in the building cistern with flows pumped at a controlled rate to the minor system with a backwater valve at the connection; and,
- Controlled release of the flows captured in the ditch drainage system for the surrounding landscaped areas.

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Foundation drains will be connected to the internal building plumbing system therefore no HGL analysis is required.

4.3 Proposed Minor System Servicing

4.3.1 Runoff Coefficient

A minor system connection will be provided from the building envelope to the minor system within the subdivision. The connection will be sized based on the 1:2-year release from the site. The runoff coefficient is based on the ratio of impervious surfaces and grassed or landscaped areas. The breakdown between pervious and impervious surfaces is shown on the figure entitled Overall Site Imperviousness in Appendix D.

4.4 Stormwater Management Modelling Approach

4.4.1 Dual Drainage Model

The analysis of both major and minor drainage systems was carried out to demonstrate their compliance with respect to the design criteria described in Section 4.1. The performance of the major overland system and minor storm sewer system was analyzed with PCSWMM. This software is a dynamic model which allows both hydrologic and hydraulic components to be simulated in the same platform and also allows the simulation of the interaction between the major and minor systems. The PCSWMM software platform was used to:

- Generate the surface runoff hydrograph for each sub-area under various recurrences.
- Subdivide each inflow hydrograph into its minor and major system components based on the proposed inlet capture rates and roadway sag storage.
- Assess cascading, if any, and carry out dynamic routing of storm flows to determine flow depths along the roadways. As previously stated, the maximum major overland flow depths within the parking lot areas are to be limited to 350 mm or less, as per Technical Bulletin PIEDTB-2016-01.

PCSWMM was set-up to evaluate the proposed servicing as detailed on Drawing C01 and C02. Subcatchments were delineated for the structure roof areas, parking lot low points and landscaped low points. Model schematics are prepared in Appendix D.

4.4.2 Boundary Conditions

Boundary conditions are taken from the downstream subdivision model issued as part of the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2024).

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The downstream 1:100-year HGL at the connection to the Subdivision at MH514 is identified as 81.07 m which is lower than any minor system within the site and therefore there is no backflow from the boundary condition and no impacts on HGL.

The rear-yard outlet connection into the subdivision connects directly into the pipe from STM STUB 16 to EXST MH514. The downstream 1:100-year HGL at STM STUB 16 is 81.28 m which is below the catch basin invert of 83.28 m at CB112.

4.4.3 Hydrological Modelling Parameters

The following parameters were used in the hydrologic component of PCSWMM:

- **Areas and Imperviousness:** Catchment ID and drainage areas used by PCSWMM match those shown on either Drawing DST or Figure 1 (Appendix D).
- **Catchment Width:** The catchment width is estimated at the width of overland sheet flow based on the grading of the catchment and slope direction.
- **Manning's Roughness Coefficient:** Manning's Roughness Coefficients of 0.013 and 0.25 were used for the impervious and pervious surfaces, respectively.
- **Horton Infiltration parameters:** City of Ottawa OSDG Horton Infiltration Parameters have been used in the modelling.
- **Initial Abstraction:** Initial abstraction of 4.67 mm and 1.57 mm was used for the pervious and impervious surfaces respectively, consistent with the OSDG.

4.4.4 Simulation of Storm Distributions

To assess peak flow rates and peak volume storage requirements the 3-hour Chicago storm has been simulated for the site for the 1:2-year event and 1:100-year event and the 24-hour SCS storms for the 1:100-year event.

4.5 Simulation Results

4.5.1 Low Point Ponding Analysis

Ponding depths in the low points in the parking area and landscaped areas are shown in Table 6.

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Table 6: Catchbasin Ponding Depths

Low Point ID	Top of Grate (m)	Maximum Static Depth (mm)	3-hour Chicago 1:2 year Depth (mm)	3-hour Chicago 1:100 year Depth (mm)	24-hour SCS 1:100 year Depth (mm)
1	85.30	200	10*	30	30
2	85.25	200	20*	50	40
3	85.20	150	20*	30	30
4	85.15	150	20*	40	30
5	85.22	180	0*	10	10
6	85.30	300	0	0	0
7	85.20	180	0	0	0
8	85.15	100	0	0	0

Those values marked with * are greater than 0 mm in the 2-year event due to the model setup. In order to represent the Zurn inlet control device, the orifice has been set at the top of grate elevation as the control is at the surface. Therefore, the model requires a head buildup over the structure for simulation of flows.

The simulation results compiled in Table 6 shows that:

- No ponding nor dynamic flow will occur in the 1:2-year event;
- Maximum ponding depth of 50 mm during the 1:100-year event; and,
- There is no spill from the site in the 1:100-year event.

4.5.2 Roof Drainage

The roof drainage system is to be designed by the Mechanical Engineer to achieve a flow rate of 2 L/s from each roof structure. The maximum depth of ponding required, assuming storage across 66% of the rooftop area, is 80 mm.

4.5.3 Parking Lot Drainage

The parking lot drainage system connects to the building cistern and is controlled by the ZURN_Z150F-6NH (detail provided in Appendix D) to allow the 1:2-year event to drain and for the surface to provide storage during greater events up to the 1:100 year. Capture rates are shown in **Error! Reference source not found..**

Table 7: Parking Area ICD Capture Analysis

Low Point	ICD Type	1:2 year Rational Flow (l/s)	1:100 year 3-hour Chicago Flow (l/s)	1:100 year 3-hour Chicago Head (m)
1	ZURN_Z150F-6NH	3	20	30
2		11	36	50

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Low Point	ICD Type	1:2 year Rational Flow (l/s)	1:100 year 3-hour Chicago Flow (l/s)	1:100 year 3-hour Chicago Head (m)
3		7	20	30
4		10	27	40
5		1	6	10

The table shows that the parking lot ICDs capture the 1:2-year design flow.

4.5.4 Landscaped Drainage

The landscaped drainage consists of typical rear yard system of swales and perforated pipes. At the downstream end the Vortex ICD 95 provides flow control to 10 L/s in the 1:100-year event with a head of 1.63m. The release is greater than the rational method 1:2-year design flow of 4 L/s for the combined upstream catchments. The Vortex ICD sizing ensures that there is no spill from Low Point 8 in the 1:100 year event.

4.5.5 Building Release Rates

To maintain overall release rate from the site at the allowable flow rate of 44 L/s, the building drainage system must be controlled to 29 L/s. Based on a pumped rate of 29 L/s a cistern size of 49 m³ is required in the building basement.

4.6 Summary and Conclusions

The stormwater servicing achieves a release rate from the site to the minor system limited to the allowable release rate and contains up to the 1:100 year on site.

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5.0 Erosion and Sedimentation Control

Erosion and sediment control measures, as outlined in the Ontario Ministry of Natural Resources (MNR) Guidelines on Erosion and Sediment Control for Urban Construction Sites, will be implemented to trap sediment on site. The following erosion and sediment control measures can be implemented during construction as shown on the Erosion and Sediment Control Plan (Drawing ESC):

- Supply and installation of a silt fence barrier, as per OPSD 219.110.
- Supply and installation of siltsack or sentinel CB inserts between the frame and cover of catch basins and maintenance holes adjacent to the project area during construction, to prevent sediment from entering the sewer system.
- Stockpiling of material during construction is to be located along flat areas away from drainage paths. For material placed on sloped areas, stockpiles are to be enclosed with a silt fence to protect watercourses.
- All catch basins are to be equipped with sumps, inspected frequently, and cleaned as required.
- Temporary ICD to be placed blocking part of the sanitary sewer pipe in the connecting maintenance holes to eliminate construction debris from entering the existing sanitary sewer system. The ICDs are to be removed after the proposed sanitary sewers have been fully cleaned.

The proposed removal and reinstatement measures as well as the erosion control measures shall conform to the following documents:

- "Guidelines on Erosion and Sediment Control for Urban Construction Sites" published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs, and Transportation & Communication, Association of Construction Authorities of Ontario and Urban Development Institute, Ontario, May 1987.
- "MTO Drainage Manual", Chapter F: "Erosion of Materials and Sediment Control", Ministry of Transportation & Communications, 1985.
- "Erosion and Sediment Control" Training Manual by Ministry of Environment, Spring 1998.
- Applicable Regulations and Guidelines of the Ministry of Natural Resources.

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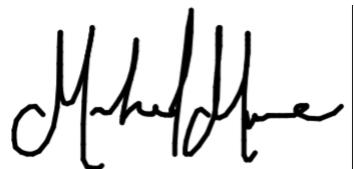
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Appendix A

Concept Plan, Draft Plan of
Subdivision and Topographical
Survey

NAVAN ROAD
DEVELOPMENT2983, NAVAN Road, Orleans,
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OTTAWA, ON K2C 3G4

KEY PLAN



DO NOT USE FOR CONSTRUCTION

DATE
2024-07-17DESIGNED
P.POMERLEAUDRAWN
P.POMERLEAUPROJECT No
20054CHECKED
P.MARTIN

SHEET TITLE

GLOBAL SITE PLAN

SHEET No

A100

IT IS THE RESPONSIBILITY OF THE APPROPRIATE CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS ON THE SITE AND TO REPORT ALL ERRORS AND/OR OMISSIONS TO THE ARCHITECT. ALL CONTRACTORS MAINTAIN THEIR OWN PAPERWORK COPIED AND DRAWN TO SCALE DRAWINGS.

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LEGEND - SITE PLAN

LOT LINE	
SETBACKS	
ELECTRICAL LINE	
EASEMENTS	
0000 GEO	ÉLEVATION GÉOÉSIQUE
EXISTING BUILDING	
DEMOLISHED BUILDING	
EXISTING TREE	DEMOLISHED TREE
NEW TREE	
NEW PLANT	
GRASS	
PEA GRAVEL	
SIDEWALK	ASPHALT
RADIANT ZONE	LANDSCAPED AREA

ARCHAUTICAL

(416) 651-8954

INFO@PMAARCHITECTES.COM

3070, CHEMIN DES QUATRE-BOURGEOIS

QUEBEC (QC) G1W 2K4

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L'Atelier

Architectes

53 BOUL. SAINT-RAYMOND,

GATINEAU, QC J8Y 1R8

CIVIL ENGINEERS / PLANNER

J.L. Richards

ENGINEERS-ARCHITECTS-PLANNERS

1565 CARLING AVENUE, SUITE 700,

OTTAWA, ON K1Z 8R1

MECHANICAL & ELECTRICAL ENGINEERS

Q|M&E

ENGINEERING

9 GURDWARA ROAD, UNIT 200,

OTTAWA, ON K2E 7X6

SURVEYOR

Stantec

1331 CLYDE AVENUE, SUITE 400,

OTTAWA, ON K2C 3C4

ARCHITECT SEAL


REVISIONS	
2	FOR CITY REVIEW
1	FOR CITY REVIEW
NO	DESCRIPTION
	DATE

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DATE 2024-07-17	DESIGNED P.POMERLEAU
DRAWN P.POMERLEAU	
PROJECT No 20054	CHECKED P.MARTIN
	SHEET TITLE
SITE PLAN - BLOCK 15	
SHEET No	A101





Site Servicing Report
2983, 3053 and 3079 Navan Road & 2690 Pagé Road

Appendix B

Pre Consultation Meeting Notes
and Site Servicing Report
Checklist



File No.: PC2023-0226

Carmine Zayoun
12714001 Canada Inc (Zayoun Group)
Via email: carmine@zayoungroup.com

Subject: Pre-Consultation: Meeting Feedback
Proposed Site Plan Application – 2983 Navan Road
Four residential and two mixed-use buildings – PC2023-0226

Please find below information regarding next steps as well as consolidated comments from the above-noted pre-consultation meeting held on September 13, 2023.

Pre-Consultation Preliminary Assessment

1 <input type="checkbox"/>	2 <input type="checkbox"/>	3 <input type="checkbox"/>	4 <input checked="" type="checkbox"/>	5 <input type="checkbox"/>
----------------------------	----------------------------	----------------------------	---------------------------------------	----------------------------

One (1) indicates that considerable major revisions are required while five (5) suggests that the proposal appears to meet the City's key land use policies and guidelines. This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.

Next Steps

1. A review of the proposal and materials submitted for the above-noted pre-consultation has been undertaken. Please proceed to complete a Phase 2 / Phase 3 Pre-consultation Application Form and submit it together with the necessary studies and/or plans to planningcirculations@ottawa.ca.
2. In your subsequent pre-consultation submission, please ensure that all comments or issues detailed herein are addressed. A detailed cover letter stating how each issue has been addressed must be included with the submission materials. Please coordinate the numbering of your responses within the cover letter with the comment number(s) herein.
3. Please note, if your development proposal changes significantly in scope, design, or density before the Phase 3 pre-consultation, you may be required to complete or repeat the Phase 2 pre-consultation process.

Supporting Information and Material Requirements

1. The attached **Study and Plan Identification List** outlines the information and material that has been identified, during this phase of pre-consultation, as either required (R) or advised (A) as part of a future complete application submission.
 - a. The required plans and studies must meet the City's Terms of Reference (ToR) and/or Guidelines, as available on Ottawa.ca. These ToR and Guidelines outline



the specific requirements that must be met for each plan or study to be deemed adequate.

Consultation with Technical Agencies

1. You are encouraged to consult with technical agencies early in the development process and throughout the development of your project concept. A list of technical agencies and their contact information is enclosed.

Planning

Comments:

1. In the Official Plan the subject site is designated as Neighbourhood is modified with the Evolving Neighbourhood overlay. Brian Coburn Boulevard is also designated as a Minor Corridor. The property is further identified as Low-density residential in the EUC – Phases 1 Community Design Plans (CDP). The subject lands are currently zoned GM[2546]H(14.5) General Mixed-Use, Exception and DR Development Reserve.
2. Committee of Adjustment

No variances have been identified at this point. I would be supportive of reducing the parking requirement for the residential units if it will result in addition landscaping and tree cover. Staff will set up a meeting with a Committee of Adjustment Plan if any required.

3. Design guidelines

The following guidelines are meant as a starting place to help guide the design. I realize that they are specifically related to your project. [Urban Design Guidelines for Low-rise Infill Housing](#)

4. Landscape requirements

All required yards are to be landscaped with parking located between the buildings. Green spaces and tree canopy needs to be maximized.

5. Parking requirements

Parking should comply with Sections 100, 101, 106, 109 and 111

Vehicle and bicycle parking should be situated with easily access while minimizing pavement.

6. Easements

Will there be any easements required?



7. Commercial units should be orientated to the streets as much as possible.
8. Provide locations of signage and ensure that space is made available for tree planting

Urban Design

9. This proposal does not run along or does not meet the threshold in one of the City's Design Priority Areas and need not attend the City's UDRP. Staff will be responsible for evaluating the Urban Design Brief and providing design direction.
10. An Urban Design Brief is a required submittal Re-zoning applications. The Urban Design Brief should be structured by generally following the headings highlighted under Section 3 – Contents of these Terms of Reference. Please see the Urban Design Brief Terms of Reference provided.
11. We recommend further detail be provided about the low-rise apartments to better understand their relationship to the surrounding buildings and properties.
12. If this site is located outside the greenbelt, a shadow analysis will be required.
13. We recommend the low-rise buildings fronting City streets consider grade related units accessed from the street to further 'fit-in' with the surrounding low-rise residential community where feasible.
14. We recommend additional landscaping detail be provided around the low-rise apartments to better understand how the buildings relate to their context.
15. We recommend additional detail be provided of the front yards of the townhouse units facing City streets to better understand how they will integrate with the public realm.
16. We recommend tree planting in front yards facing public right-of-way.
17. When a wind and/or shadow studies are required please refer to the Terms of Reference for the wind analysis and shadow analysis to conduct the studies and evaluate the impacts.
18. Note. The Urban Design Brief submittal should have a section which addresses these pre-consultation comments.



This is an exciting project in an area full of potential. We look forward to helping you achieve its goals with the highest level of design resolution. We are happy to assist and answer any questions regarding the above. Feel free to contact the Urban Design Planner, Christopher Moise, at Christopher.Moise@ottawa.ca, for follow-up questions.

Engineering

Comments:

19. The Stormwater Management Criteria, for the subject site, is to be based on the following:

- a. The 5-yr storm event using the IDF information derived from the Meteorological Services of Canada rainfall data, taken from the MacDonald Cartier Airport, collected 1966 to 1997.
- b. For separated sewer system built pre-1970 the design of the storm sewers are based on a 2 year storm.
- c. The pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less (§ 8.3.7.3).
- d. A calculated time of concentration (Cannot be less than 10 minutes).
- e. Flows to the storm sewer in excess of the 5-year storm release rate, up to and including the 100-year storm event, must be detained on site.
- f. For a combined sewer system the maximum C= 0.4 or the pre-development C value, whichever is less. In the absence of other information the allowable release rate shall be based on a 2 year storm event.

20. Deep Services (Storm, Sanitary & Water Supply)

- a. Provide existing servicing information and the recommended location for the proposed connections. Services should ideally be grouped in a common trench to minimize the number of road cuts.
- b. Connections to trunk sewers and easement sewers are typically not permitted.
- c. Provide information on the monitoring manhole requirements – should be located in an accessible location on private property near the property line (ie. Not in a parking area).
- d. Review provision of a high-level sewer.

- e. Sewer connections to be made above the springline of the sewermain as per:
 - i. Std Dwg S11.1 for flexible main sewers – connections made using approved tee or wye fittings.
 - ii. Std Dwg S11 (For rigid main sewers) – lateral must be less than 50% the diameter of the sewermain,
 - iii. Std Dwg S11.2 (for rigid main sewers using bell end insert method) – for larger diameter laterals where manufactured inserts are not available; lateral must be less than 50% the diameter of the sewermain,
 - iv. Connections to manholes permitted when the connection is to rigid main sewers where the lateral exceeds 50% the diameter of the sewermain. – Connect obvert to obvert with the outlet pipe unless pipes are a similar size.
 - v. No submerged outlet connections.

21. Water Boundary condition requests must include the location of the service (map or plan with connection location(s) indicated) and the expected loads required by the proposed development, including calculations. Please provide the following information:

*no BCR will be requested for res site
plans, no watermains built, navan
connection already provided as part
of subdivision*

- a. Location of service
- b. Type of development and the amount of fire flow required (as per FUS).
- c. Average daily demand: ____ l/s.
- d. Maximum daily demand: ____ l/s.
- e. Maximum hourly daily demand: ____ l/s.

22. An MECP Environmental Compliance Approval [Industrial Sewage Works or Municipal/Private Sewage Works] will be required for the proposed development. Please contact the Ministry of the Environment, Conservation and Parks, Ottawa District Office to arrange a pre-submission consultation:

- a. Charlie Primeau at (613) 521-3450, ext. 251 or Charlie.Primeau@ontario.ca
- b. Emily Diamond at (613) 521-3450, ext. 238 or Emily.Diamond@ontario.ca

General Comments:

23. Review of the Phase 3 submission for this application will not occur until the detailed design of the subdivision that it is within (D07-16-21-0027) is approved.
24. At the stage of site plan approval, a condition will be imposed detailing that a commencement work notification will not be issued until the subdivision's infrastructure is in-service.

25. Engineering Studies:

26. All engineering studies (detailed in the Study and Plan Identification List form) are to follow the to be approved draft plan of subdivision D07-16-21-0027

Feel free to contact Derek Unrau, Infrastructure Project Manager, for follow-up questions.

Noise

Comments:

27. Noise report is required to identify and mitigate traffic noise from Brian Coburn Boulevard and Navan Road

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at Mike.Giampa@ottawa.ca , for follow-up questions.

Transportation

Comments:

- a. A full TIA is not required as this site is covered under the recent subdivision TIA.
- b. A memo including the pertinent subdivision trips is sufficient.
- c. The right of way protection on Brian Coburn and Navan Roads is 40m and 37.5m, respectively

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at Mike.Giampa@ottawa.ca , for follow-up questions.

Planning Forestry

Comments:



28. A Tree Conservation Report and Landscape Plan must be submitted with both SPC applications
29. A permit is required prior to any tree removal on site. The tree permit will be released upon site plan approval. Please contact the File Lead or the Planning Forester, Hayley Murray (hayley.murray@ottawa.ca) for information on obtaining the tree permit.
30. If marine clay soils are present, setbacks on City properties must adhere to the 2017 SMC guidelines (attached). The Geotechnical report must address the implications of these soils, if present, on tree planting in relation to private land.
31. If underground parking is planned, a design must be provided for the site to support tree planting
32. We expect a very strong landscape plan to re-establish canopy cover across the properties. Tree planting and protecting existing urban forest canopy is imperative to reach the City's target of 40% canopy cover.

Feel free to contact Hayley Murray, Planning Forester, for follow-up questions.

Parkland

Comments:

33. Parkland contributions were made through the Subdivision process.

Feel free to contact Jessica Button, Parks Planner, for follow-up questions

Conservation Authority

Comments:

34. The Rideau Valley Conservation authority will be commenting on this application

Feel free to contact RVCA, for follow-up questions.

Other

35. The High Performance Development Standard (HPDS) is a collection of voluntary and required standards that raise the performance of new building projects to achieve sustainable and resilient design. The HPDS was passed by Council on April 13, 2022.



- a. At this time, the HPDS is not in effect and Council has referred the 2023 HPDS Update Report back to staff with direction to bring forward an updated report to Committee with recommendations for revised phasing timelines, resource requirements and associated amendments to the Site Plan Control By-law by no later than Q1 2024.
- b. Please refer to the HPDS information attached and ottawa.ca/HPDS for more information.

Submission Requirements and Fees

1. Outlines the application type/subtype required and the associated fees
 - a. Additional information regarding fees related to planning applications can be found [here](#).
2. The attached **Study and Plan Identification List** outlines the information and material that has been identified as either required (R) or advised (A) as part of a future complete application submission.
 - a. The required plans and studies must meet the City's Terms of Reference (ToR) and/or Guidelines, as available on [Ottawa.ca](#). These ToR and Guidelines outline the specific requirements that must be met for each plan or study to be deemed adequate.
3. All of the above comments or issues should be addressed to ensure the effectiveness of the application submission review.

Should there be any questions, please do not hesitate to contact myself or the contact identified for the above areas / disciplines.

Yours Truly,
Steve Belan

cc.

Tim Chadder
Raad Akrawi
Madelen Fellows
Karla Ferrey
Tatyana Roumie
Christopher Moise
Derek Unrau
Mike Giampa
Haley Murray
Jessica Button

12714001 Canada Inc – Mixed Use Site Plan (Block 15)
2983, 3053 and 3079 Navan Road & 2690 Pagé Road

SITE SERVICING REPORT CHECKLIST

REFERENCED STUDIES AND REPORTS	REFERENCE
Site Servicing Report for 12714001 Canada Inc, Mixed Used Site Plan (Block 15) 2983, 3053 and 3079 Navan Road & 2690 Pagé Road (J.L. Richards & Associates Limited, December 8, 2023)	Site Servicing Report

4.1	GENERAL CONTENT	REFERENCE
<input type="checkbox"/>	Executive Summary (for larger reports only).	N/A
<input checked="" type="checkbox"/>	Date and revision number of the report.	Site Servicing Report
<input checked="" type="checkbox"/>	Location map and plan showing municipal address, boundary, and layout of proposed development.	Site Servicing Report (Appendix A) All Drawings
<input checked="" type="checkbox"/>	Plan showing the site and location of all existing services.	Servicing Plan
<input checked="" type="checkbox"/>	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Site Servicing Report
<input checked="" type="checkbox"/>	Summary of Pre-consultation Meetings with City and other approval agencies.	Site Servicing Report (Appendix 'A')
<input checked="" type="checkbox"/>	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defendable design criteria.	Reference made to Stantec 2005 EUC ISSU
<input checked="" type="checkbox"/>	Statement of objectives and servicing criteria.	Site Servicing Report
<input checked="" type="checkbox"/>	Identification of existing and proposed infrastructure available in the immediate area.	Site Servicing Report Servicing Plan
<input type="checkbox"/>	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	N/A

<input checked="" type="checkbox"/>	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Grading Plan
<input type="checkbox"/>	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
<input type="checkbox"/>	Proposed phasing of the development, if applicable.	N/A
<input checked="" type="checkbox"/>	Reference to geotechnical studies and recommendations concerning servicing.	Site Servicing Report and Drawings
<input checked="" type="checkbox"/>	All preliminary and formal site plan submissions should have the following information: <ul style="list-style-type: none"> ▪ Metric scale ▪ North arrow (including construction North) ▪ Key plan ▪ Name and contact information of applicant and property owner ▪ Property limits, including bearings and dimensions ▪ Existing and proposed structures and parking areas ▪ Easements, road widening and rights-of-way ▪ Adjacent street names 	All Drawings

4.2	SITE SERVICING REPORT: WATER	REFERENCE
<input type="checkbox"/>	Confirm consistency with Master Servicing Study, if available.	N/A
<input checked="" type="checkbox"/>	Availability of public infrastructure to service proposed development.	Site Servicing Report (Section 2.0) Servicing Plan
<input checked="" type="checkbox"/>	Identification of system constraints.	Site Servicing Report (Section 2.0) Servicing Plan
<input checked="" type="checkbox"/>	Identify boundary conditions.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Confirmation of adequate domestic supply and pressure.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	Site Servicing Report (Section 2.0)

<input type="checkbox"/>	Definition of phasing constraints. Hydraulic modelling is required to confirm servicing for all defined phases of the project, including the ultimate design.	N/A
<input checked="" type="checkbox"/>	Address reliability requirements, such as appropriate location of shutoff valves.	Site Servicing Report (Section 2.0)
<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification.	N/A
<input checked="" type="checkbox"/>	Reference to water supply analysis to show that major infrastructure can deliver sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants), including special metering provisions.	Site Servicing Report (Section 2.0) Servicing Plan
<input type="checkbox"/>	Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input checked="" type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	Site Servicing Report (Section 2.0)

4.3	SITE SERVICING REPORT: WASTEWATER	REFERENCE
<input checked="" type="checkbox"/>	Summary of proposed design criteria (Note: Wet weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Site Servicing Report (Section 3.0,
<input type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	Stantec 2005 EUC ISSU
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the Guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input checked="" type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Site Servicing Report (Section 3.0) Servicing Plan

<input checked="" type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable.)	Site Servicing Report (Section 3.0)
<input checked="" type="checkbox"/>	Calculations related to dry weather and wet weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Site Servicing Report (Section 3.0)
<input checked="" type="checkbox"/>	Description of proposed sewer network, including sewers, pumping stations and forcemains.	Site Servicing Report (Section 3.0) Servicing Plan
<input type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/>	Special considerations, such as contamination, corrosive environment, etc.	N/A

4.4	SITE SERVICING REPORT: STORMWATER	REFERENCE
<input checked="" type="checkbox"/>	Description of drainage outlets and downstream constraints, including legality of outlets (i.e., municipal drain, right-of-way, watercourse, or private property).	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Servicing, Grading and Drainage Plans
<input checked="" type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Site Servicing Report (Section 4.0)

<input checked="" type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
<input type="checkbox"/>	Setback from private sewage disposal systems.	N/A
<input type="checkbox"/>	Watercourse and hazard lands setbacks.	N/A
<input checked="" type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Site Servicing Report (Appendix 'A')
<input type="checkbox"/>	Confirm consistency with subwatershed and Master Servicing Study, if applicable study exists.	Stantec 2005 EUC ISSU
<input checked="" type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:2 year return period) and major events (1:100 year return period).	Site Servicing Report (Section 4.0)
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	N/A
<input checked="" type="checkbox"/>	Calculate pre- and post-development peak flow rates, including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Proposed minor and major systems, including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Servicing, Grading and Drainage Plans
<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Quantity control proposed per Site Servicing Report (Section 4.0)
<input type="checkbox"/>	Identification of potential impacts to receiving watercourses.	N/A
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A
<input checked="" type="checkbox"/>	Description of how the conveyance and storage capacity will be achieved for the development.	Site Servicing Report (Section 4.0)

<input checked="" type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
<input checked="" type="checkbox"/>	Inclusion of hydraulic analysis, including hydraulic grade line elevations.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Site Servicing Report (Section 5.0) Servicing Plan
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5 APPROVAL AND PERMIT REQUIREMENTS	REFERENCE
The Site Servicing Report shall provide a list of applicable permits and regulatory approvals necessary for the proposed development, as well as the relevant issues affecting such approval. The approval and permitting shall include but not be limited to the following:	
<input type="checkbox"/>	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams, as defined in the Act.
<input type="checkbox"/>	Application for Environmental Compliance Approval (ECA) under the Ontario Water Resources Act.
<input type="checkbox"/>	Changes to Municipal Drains.
<input type="checkbox"/>	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation, etc.).

4.6 CONCLUSION CHECKLIST	REFERENCE
<input checked="" type="checkbox"/>	Clearly stated conclusions and recommendations.
<input checked="" type="checkbox"/>	Comments received from review agencies, including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.

<input checked="" type="checkbox"/>	All draft and final reports shall be signed and stamped by a Professional Engineer registered in Ontario.	Site Servicing Report All Drawings

Site Servicing Report
2983, 3053 and 3079 Navan Road & 2690 Pagé Road

Appendix C

Water Servicing

WATERMAIN DEMAND CALCULATION SHEET

PROJECT : NAVAN ROAD DEVELOPMENT PROJECT
LOCATION : CITY OF OTTAWA
DEVELOPER : 12714001 Canada Inc.

NODE	RESIDENTIAL			NON-RESIDENTIAL			AVERAGE DAILY DEMAND (l/s)			MAXIMUM DAILY DEMAND (l/s)			PEAK HOUR DEMAND (l/s)		
	UNITS		POP'N	COMM		(ha.)	Res.	Non-res.	Total	Res.	Non-res.	Total	Res.	Non-res.	Total
	Townhouses (TH)	Condo Units (CU)		0.09	0.48		0.48	0.03	0.52	1.21	0.05	1.26	2.66	0.08	2.74
J-23	0	83	149												
TOTALS	0	83	149				0.09	0.48	0.52	1.21	0.05	1.26	2.66	0.08	2.74

ASSUMPTIONS					
RESIDENTIAL DENSITIES		AVG. DAILY DEMAND		MAX. HOURLY DEMAND	
- Townhouse (TH)	<u>2.7</u> p / p / u	- Residential	<u>280</u> l / cap / day	- Residential	<u>1,540</u> l / cap / day
- Condo Units (CU)	<u>1.8</u> p / p / u	- Institutional	<u>28,000</u> l / ha / day	- Institutional	<u>75,600</u> l / ha / day
		- Commercial	<u>28,000</u> l / ha / day	- Commercial	<u>75,600</u> l / ha / day
		MAX. DAILY DEMAND			
		- Residential	<u>700</u> l / cap / day		
		- Institutional	<u>42,000</u> l / ha / day		
		- Commercial	<u>42,000</u> l / ha / day		

FUS Fire Flow Calculations

NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building
(JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Wood Frame	Building C (4 Story Mixed Use Condominium Building)
	Coefficient (C)	1.5	
B	Ground Floor Area	929	m ²
C	Height in storeys	4	storeys
	Total Floor Area	3716	m ²
D	Fire Flow Formula	$F=220C\sqrt{A}$	
	Fire Flow	20116	L/min
	Rounded Fire Flow	20000	L/min
			Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Limited Combustible	Residential buildings have a limited combustible occupancy.
	Occupancy Charge	-15%	
	Occupancy Increase or Decrease	-3000	
	Fire Flow	17000	L/min
F	Sprinkler Protection	Automatic Fully Supervised	
	Sprinkler Credit	-50%	
	Decrease for Sprinkler	-8500	L/min
G	North Side Exposure		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	35.7	m
	Height of Exposed Wall:	4	storeys
	Length-Height Factor	142.9	m-storeys
	Separation Distance	26	m
	North Side Exposure Charge	10%	
	East Side Exposure		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	50	m
	East Side Exposure Charge	0%	
	South Side Exposure		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	35.9	m
	Height of Exposed Wall:	4	storeys
	Length-Height Factor	143.5	m-storeys
	Separation Distance	26.89	m
	South Side Exposure Charge	10%	
	West Side Exposure		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	50	m
	West Side Exposure Charge	0%	
	Total Exposure Charge	20%	The total exposure charge is below the maximum value of 75%.
	Increase for Exposures	3400	L/min
H	Fire Flow	11900	L/min
	Rounded Fire Flow	12000	L/min
			Flow rounded to nearest 1000 L/min.
City Cap	Required Fire Flow (RFF)	12000	L/min
		200	L/s

Fire Underwriters Survey (FUS) Fire Flow Calculations
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

FUS Fire Flow Calculations

NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building
(JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Wood Frame	Building D (4 Story Mixed Use Condominium Building)
	Coefficient (C)	1.5	
B	Ground Floor Area	929	m ²
C	Height in storeys	4	storeys
	Total Floor Area	3716	m ²
D	Fire Flow Formula	$F=220C\sqrt{A}$	
	Fire Flow	20116	L/min
	Rounded Fire Flow	20000	L/min
			Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Limited Combustible	Residential buildings have a limited combustible occupancy.
	Occupancy Charge	-15%	
	Occupancy Increase or Decrease	-3000	
	Fire Flow	17000	L/min
F	Sprinkler Protection	Automatic Fully Supervised	
	Sprinkler Credit	-50%	
	Decrease for Sprinkler	-8500	L/min
G	<i>North Side Exposure</i>		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	35.9	m
	Height of Exposed Wall:	4	storeys
	Length-Height Factor	143.5	m-storeys
	Separation Distance	26.89	m
	North Side Exposure Charge	10%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	7.8	m
	Height of Exposed Wall:	2	storeys
	Length-Height Factor	15.5	m-storeys
	Separation Distance	11.5	m
	East Side Exposure Charge	12%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Non-combustible	
	Length of Exposed Wall:	23.9	m
	Height of Exposed Wall:	1	storeys
	Length-Height Factor	23.9	m-storeys
	Separation Distance	32.28	m
	South Side Exposure Charge	5%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Wood Frame	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	50	m
	West Side Exposure Charge	0%	
	Total Exposure Charge	27%	The total exposure charge is below the maximum value of 75%.
	Increase for Exposures	4590	L/min
H	Fire Flow	13090	L/min
	Rounded Fire Flow	13000	L/min
			Flow rounded to nearest 1000 L/min.
City Cap	Required Fire Flow (RFF)	13000	L/min
		217	L/s

Fire Underwriters Survey (FUS) Fire Flow Calculations
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

2983, NAVAN Road, Orleans,
ON K1C 7G4789 BOUL. SAINT-JOSEPH, SUITE 100
GATINEAU, QC J8Y 4B8

OWNER

ARCHITECTURAL

PIN

PMA ARCHITECTES

(416) 651-8954

INFO@PMAARCHITECTES.COM

3070, CHEMIN DES QUATRE-BOURGEOIS

QUEBEC (QC) G1W 2K4

PMAARCHITECTES.COM

53 BOUL. SAINT-RAYMOND,
GATINEAU, QC J8Y 1R8

ENGINEERS / PLANNER

1565 CARLING AVENUE, SUITE 700,
OTTAWA, ON K1Z 8R1

SURVEYOR

1331 CLYDE AVENUE, SUITE 400,
OTTAWA, ON K2C 3G4

KEY PLAN

ARCHITECT SEAL

LOTS AREAS			
LOT NUMBER	AREAS (M2)	LOT NUMBER	AREAS (M2)
B01-1	394	B06-4	154
B01-2	184	B06-5	163
B01-3	184	B06-6	154
B01-4	189	B06-7	369
B01-5	189	B07	2,002
B01-6	184	B08-1	525
B01-7	184	B08-2	174
B01-8	299	B08-3	184
B02-1	281	B08-4	174
B02-2	176	B08-5	184
B02-3	184	B08-6	174
B02-4	184	B08-7	234
B02-5	174	B09-1	234
B02-6	233	B09-2	174
B03-1	250	B09-3	184
B03-2	182	B09-4	184
B03-3	182	B09-5	174
B03-4	182	B09-6	234
B03-5	182	B10-1	234
B03-6	182	B10-2	174
B03-7	250	B10-3	184
B04-1	233	B10-4	184
B04-2	174	B10-5	174
B04-3	184	B10-6	487
B04-4	174	B11-1	748
B04-5	184	B11-2	286
B04-6	174	B11-3	265
B04-7	278	B11-4	246
B05-1	368	B11-5	242
B05-2	154	B11-6	242
B05-3	163	B11-7	321
B05-4	163	B12	1,232
B05-5	154	B13	240
B05-6	206	B14	5,728
B06-1	206	B15	5,399
B06-2	154	B16	7,811
B06-3	163	B17	5,312

SITE PLAN LEGEND	
EXISTING BUILDING	LOT LINE
NEW BUILDING	SETBACKS
NEW BUILDING WITH COMMERCIAL SPACE AT-GRADE	NEW TREE
GRASS	FIREWALL
ASPHALT	SIDEWALK

SITE INFORMATION & DEVELOPMENT STATISTICS

LOTS	PIN	04756 - 0303
		04756 - 0315
		04756 - 0316
		04756 - 1337
ZONING	GM	(2546) H(14.5)
SITE AREA	TOTAL SITE AREA:	~53,441.14 m ² (5.34ha)
	TOTAL DEVELOPABLE AREA:	~45,956.28 m ² (4.59ha)
	NET SITE AREA:	~38,956.28 m ² (3.89ha)
UNITS	TOWNHOUSES:	67 UNITS
BLOCK 01:	1 X RESIDENTIAL APARTMENT BUILDING	48 UNITS
	1 X MIXED USE BUILDING	36 UNITS
	RESIDENTIAL:	~929 m ²
BLOCK 02:	1 X RESIDENTIAL APARTMENT BUILDING	47 UNITS
	1 X MIXED USE BUILDING	36 UNITS
	RESIDENTIAL:	~929 m ²
BLOCK 03:	2 X RESIDENTIAL APARTMENT BUILDING	96 UNITS
	TOTAL NUMBER OF UNITS:	330 UNITS
	TOTAL COMMERCIAL SPACES:	~1,858 m ²
	REQUIRED	PROVIDED
MAXIMUM DENSITY	NO MAX.	84.8 units/net ha
MINIMUM LOT WIDTH	NO MIN.	5.8 m
MINIMUM LOT AREA	NO MIN.	174 m ²
MAXIMUM BUILDING HEIGHT	14.5 m	14.5 m
SETBACKS		
MINIMUM FRONT YARD:	3 m	3 m
MINIMUM CORNER SIDE YARD:	3 m	3 m
MINIMUM INTERIOR SIDE YARD:		
NON-RESIDENTIAL OR MIXED-USE:	5 m	5 m
LOW-RISE RESIDENTIAL :	1.2 m	1.2 m
MID-RISE RESIDENTIAL :	3 m	3 m
MINIMUM REAR YARD:		
ABUTTING A STREET:	3 m	3 m
FROM A RESIDENTIAL ZONE:	7.5 m	7.5 m
FOR A RESIDENTIAL BUILDING:	7.5 m	7.5 m
PARKING RATES		
R9 - TOWNHOUSES:	1 p/unit = 67	67 GARAGES
VISITOR:	0	67 DRIVE AISLES
BLOCK 14:		
R12 - APPARTMENTS	1.2 p/unit = 101	101 (UNDERGROUND)
VISITOR:	0.2 p/unit = 17	17 (UNDERGROUND)
N79 - RETAIL STORE:	3.4 p/100 m ² GFA = 32	32 (EXTERIOR)
		TOTAL: 150
BLOCK 15:		
R12 - APPARTMENTS	1.2 p/unit = 100	100 (UNDERGROUND)
VISITOR:	0.2 p/unit = 17	17 (UNDERGROUND)
N79 - RETAIL STORE:	3.4 p/100 m ² GFA = 32	32 (EXTERIOR)
		TOTAL: 150
BLOCK 18:		
R12 - APPARTMENTS	1.2 p/unit = 116	145 (UNDERGROUND)
VISITOR:	0.2 p/unit = 17	17 (8 EXT. + 12 UND.)
		TOTAL: 162
GROSS FLOOR AREA		
TOWNHOUSE A:		267 m ²
TOWNHOUSE B:		239 m ²
TOWNHOUSE C:		232 m ²
TOWNHOUSE C (CORNER UNIT):		236 m ²
TOWNHOUSE D:		225 m ²
TOTAL MODEL 01 (BBBBBB)		1,968 m ²
TOTAL MODEL 02 (BBBBBA)		1,729 m ²
TOTAL MODEL 03 (ABBBBA)		1,490 m ²
TOTAL MODEL 04 (CDCDCDC)		1,611 m ²
TOTAL MODEL 05 (CDCDC)		1,386 m ²
MIXED USE BUILDING (TOTAL OF 2 BUILDINGS):	TOTAL: 4,130 m ²	
RESIDENTIAL:	3,201 m ²	
COMMERCIAL:	929 m ²	
RESIDENTIAL APARTMENT BUILDING (TOTAL OF 4 BUILDINGS):	TOTAL: 4,130 m ²	
RESIDENTIAL:	4,130 m ²	
NOTE		
1. ASSUMES TYPICAL RESIDENTIAL FLOOR HEIGHT OF 3m.		
2. THE BASE PLAN (LOT LINES, EXISTING ROADS AND SURROUNDING AREAS) IS BASED ON THE TOPOGRAPHICAL PLAN OF SURVEY, SURVEYED STANTEC GEOMATICS LTD.		
3. DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048.		
IT IS THE RESPONSIBILITY OF THE APPROPRIATE CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS ON THE SITE AND TO REPORT ALL ERRORS AND/OR OMISSIONS TO THE ARCHITECT. ALL CONTRACTORS MUST FOLLOW ALL PERTINENT CODES AND LAWS. DO NOT SCALE DRAWINGS.		
THIS DOCUMENT AND ITS CONTENTS IS COPYRIGHTED. ANY REPRODUCTION IS PROHIBITED UNLESS GRANTED BY THE ARCHITECT.		
DO NOT USE FOR CONSTRUCTION		
DATE	2023-11-29	DESIGNED
DRAWN	PP	
PROJECT No	20054	CHECKED
	PM	
SHEET TITLE		
SITE PLAN		
SHEET No		

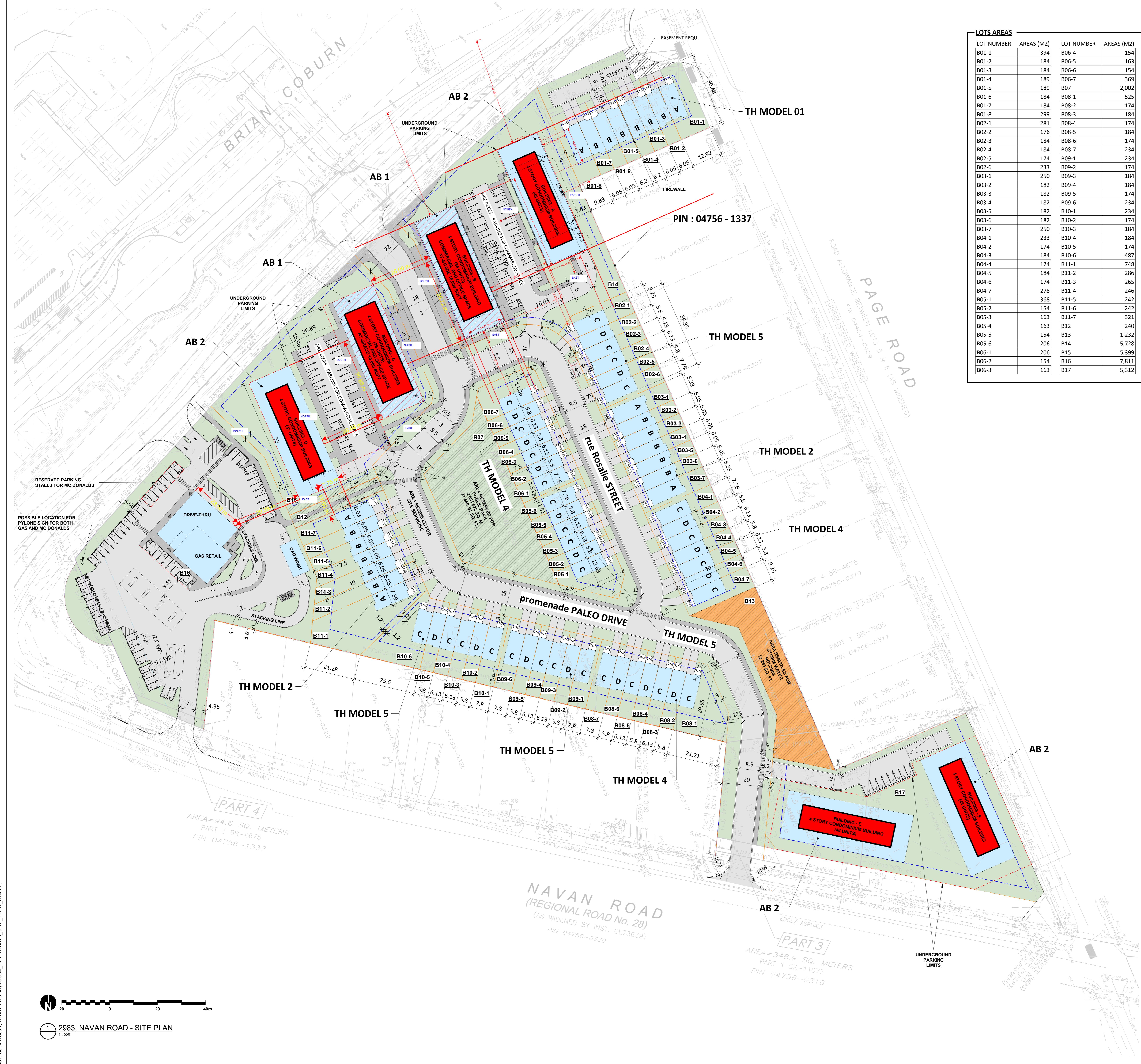


Table 1. Maximum flow to be considered from a given hydrant

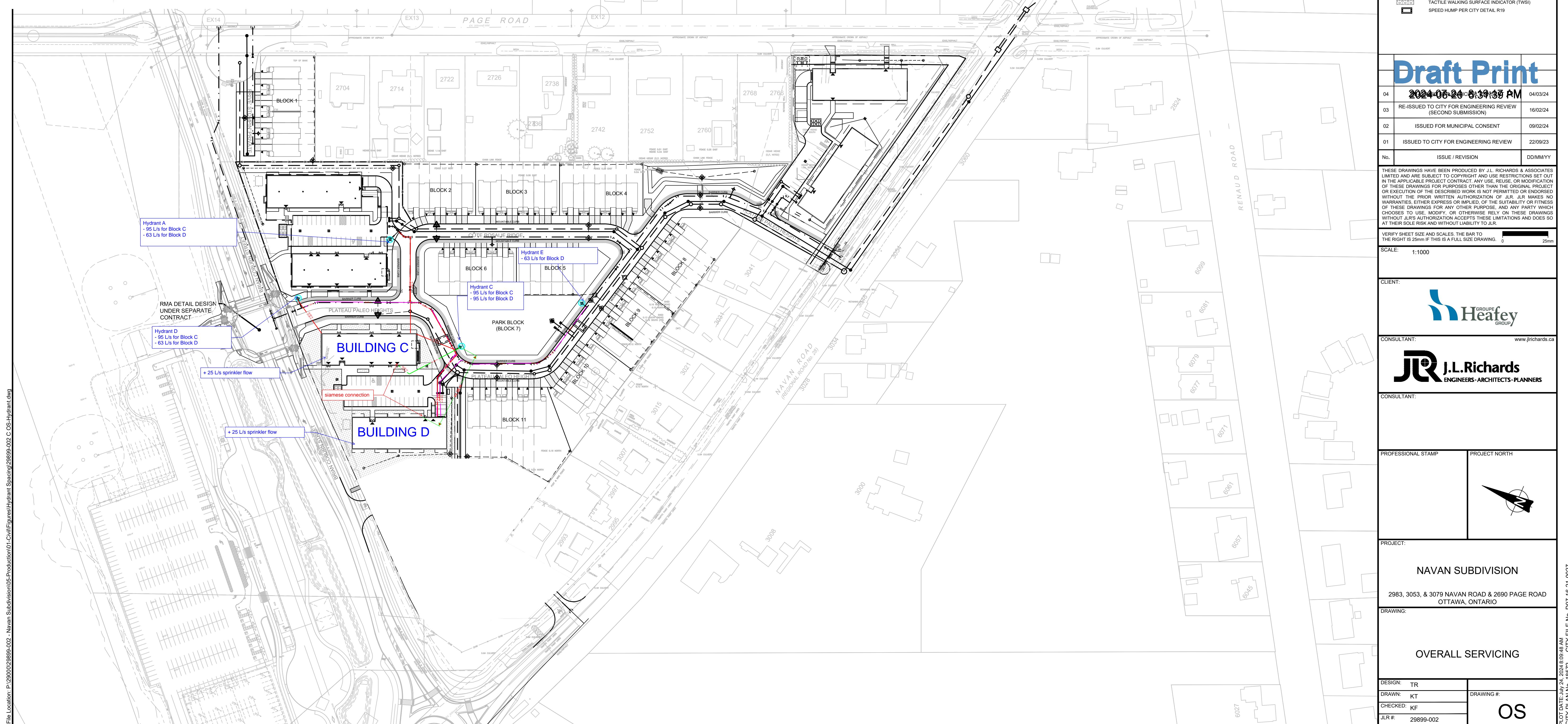
Hydrant Class	Distance to asset/structure/building (m) ^a	Contribution to required fire flow (L/min) ^b
AA	≤ 75	5,700
	> 75 and ≤ 150	3,800
A	≤ 75	3,800
	> 75 and ≤ 150	2,850
B	≤ 75	1,900
	> 75 and ≤ 150	1,500
C	≤ 75	800
	> 75 and ≤ 150	800

^a Distance of contributing hydrant from the structure, measured in accordance with NFPA 1 (Appendix A).

^b Maximum flow contribution to be considered for a given asset/structure/building, at a residual pressure of 20 psi, measured at the location of the main, at ground level.

Block 15

Required Fire Flow Building C = 200 L/s (Required Fire Flow Building D = 233 L/s)



William Rugamba

From: William Rugamba
Sent: July 22, 2024 8:18 AM
To: William Rugamba
Subject: FW: Re-confirmation of Mechanical Items for Servicing Report

William Rugamba, M.Eng., B.A.Sc., EIT
Civil Engineering Graduate
Ottawa, ON
Work: [343-804-4374](tel:343-804-4374)

From: Sarith Lopez <slopez@qmeengineering.com>
Sent: Wednesday, July 17, 2024 8:07 PM
To: Mahad Musse <mmusse@jlrichards.ca>; Chuck Clark <CWC@qmeengineering.com>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>
Subject: RE: Re-confirmation of Mechanical Items for Servicing Report

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Hi Mahad,

All items below confirmed

Regards

Sarith López
Project Manager

9 Gurdwara Road, Unit 200
Ottawa, ON K2E 7X6
T: 613-366-4763 ext. 129
slopez@qmeengineering.com



From: Mahad Musse <mmusse@jlrichards.ca>
Sent: Tuesday, July 16, 2024 3:08 PM
To: Sarith Lopez <slopez@qmeengineering.com>; Chuck Clark <CWC@qmeengineering.com>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>
Subject: Re-confirmation of Mechanical Items for Servicing Report

Hi Sarith/Chuck,

Thank you for the information below and in our meetings. I understand all these items have been discussed before but we need to submit something as part of our report. Can you just re-confirm the following questions below and then we will attach your confirmation to the Report.

1. Please confirm that the sanitary service size of 200mm diameter for the Site Plan Blocks (Block 14, 15 and 17) is preferred by the mechanical engineer on file;
2. Please confirm that a sprinkler flow of 25 L/s can be assumed for the Site Plan Blocks (Block 14, 15 and 17) at this stage.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca



William Rugamba

From: William Rugamba
Sent: July 15, 2024 4:00 PM
To: William Rugamba
Subject: FW: Navan Subdivision - Boundary Condition Request
Attachments: NavanSubdivision_Boundary Condition(4july2024).docx

William Rugamba, M.Eng., B.A.Sc., EIT

Civil Engineering Graduate

Ottawa, ON

Work: [343-804-4374](tel:343-804-4374)

From: Polyak, Alex <alex.polyak@ottawa.ca>
Sent: Monday, July 15, 2024 10:12 AM
To: Mahad Musse <mmusse@jlrichards.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Good morning Mahad,

Please find the boundary conditions attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., C.E.T., P.Eng. 

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Development and Building Services Department (PDBS) | Direction générale des services de la planification, de l'aménagement et du bâtiment (DGSPAB)

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca



From: Mahad Musse <mmusse@jlrichards.ca>
Sent: July 12, 2024 1:31 PM
To: Polyak, Alex <alex.polyak@ottawa.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Hi Alex,

Just wondering if you have a status update for the boundary conditions for Navan.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4



Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca



From: Mahad Musse <mmusse@jlrichards.ca>

Sent: Wednesday, July 3, 2024 11:02 AM

To: Polyak, Alex <alex.polyak@ottawa.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

Good morning Alex,

As we discussed last week our Client is looking into the option of converting the row townhouse units into duplex units (townhouse units with apartments in the basement). As a result, this will increase the total demand on the site and we will therefore require new water boundary conditions. We'd like to note that the footprint of the blocks will not change and neither will their layout or any of the offsets.

As a summary:

- Domestic demands were calculated based on a daily consumption rate of 280 L/cap/day with peaking factors consistent with City of Ottawa Guidelines
- Required Fire Flow (RFF) was calculated in accordance to the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection and the City of Ottawa FUS protocol (Bulletin ISDTB-2014-02 & Bulletin ISDTB-2018-02), which considers material, expose distance & height. We have attached the calculation spreadsheet and the figure.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.74 L/s

Maximum Day Demand: 10.53 L/s

Peak Hour Demand: 18.17 L/s

Required Fire Flow (per FUS): 6,000 L/min (100 L/s)

Required Fire Flow (per FUS): 10,000 L/min (167 L/s)

Required Fire Flow (per FUS): 14,000 L/min (233 L/s)

Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

For your reference, the previous boundary condition received from the City is attached and below is the email chain.

If you have any questions or comments please let us know.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca

From: Polyak, Alex <alex.polyak@ottawa.ca>
Sent: Thursday, August 17, 2023 3:01 PM
To: William Rugamba <wrugamba@jlrichards.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Hello William,

Sorry that I missed your call, I was in a meeting. The boundary conditions are attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., P.Eng

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca



From: William Rugamba <wrugamba@jlrichards.ca>
Sent: August 15, 2023 9:26 AM
To: Polyak, Alex <alex.polyak@ottawa.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Good morning Alex,

Just wanted to follow up on the status of this boundary request. Please let me know if you need anything else from us.

Thanks,
William

William Rugamba, M.Eng.
Civil Engineering Intern

J.L. Richards & Associates Limited
1000-343 Preston Street, Ottawa, ON K1S 1N4
Direct: 343-804-4374



From: Tatyana Roumie
Sent: Tuesday, July 25, 2023 3:53 PM
To: 'alex.polyak@ottawa.ca' <alex.polyak@ottawa.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; carmine <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>
Subject: Navan Subdivision - Boundary Condition Request

Hello Alex.

To support our upcoming detailed design for the site, we are requesting updated boundary conditions for the 3079 Navan Road Development.

As a brief history, we received boundary conditions from the City in July 2021 (attached, but with incorrect connection locations) and again in April 2022 (also attached) in support of the functional servicing design. We understand from the April 2022 boundary conditions that the maximum available fire flow for the site is 250 L/s.

We are currently requesting updated boundary conditions for this site as we are commencing the detailed servicing design and this request will accommodate the recent site plan changes and proposed connection points. This request is also applicable to the upcoming site plan designs which will be submitted as separate applications.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.44 L/s

Maximum Day Demand: 9.77 L/s

Peak Hour Demand: 16.50 L/s

Required Fire Flow (per FUS): 6,000 L/min (100 L/s)

Required Fire Flow (per FUS): 10,000 L/min (167 L/s)

Required Fire Flow (per FUS): 14,000 L/min (233 L/s)

Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

Thanks,
Tatyana

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Boundary Conditions Navan Subdivision

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	404	6.74
Maximum Daily Demand	632	10.53
Peak Hour	1,090	18.17
Fire Flow Demand #1	6,000	100.00
Fire Flow Demand #2	10,000	166.67
Fire Flow Demand #3	14,000	233.33
Fire Flow Demand #4	15,000	250.00

Location



Results

Connection 1 - Page Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	64.0
Peak Hour	127.0	58.6
Max Day plus Fire Flow #1	128.2	60.4
Max Day plus Fire Flow #2	126.8	58.3
Max Day plus Fire Flow #3	124.9	55.7
Max Day plus Fire Flow #4	124.4	55.0

¹ Ground Elevation = 85.7 m

Connection 2 - Navan Road East

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	71.4
Peak Hour	126.8	65.9
Max Day plus Fire Flow #1	127.7	67.1
Max Day plus Fire Flow #2	125.5	64.1
Max Day plus Fire Flow #3	122.7	60.1
Max Day plus Fire Flow #4	121.9	58.9

¹ Ground Elevation = 80.5 m

Connection 3 - Navan Road West

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	69.3
Peak Hour	126.8	63.8
Max Day plus Fire Flow #1	127.3	64.5
Max Day plus Fire Flow #2	124.6	60.6
Max Day plus Fire Flow #3	120.9	55.3
Max Day plus Fire Flow #4	119.8	53.8

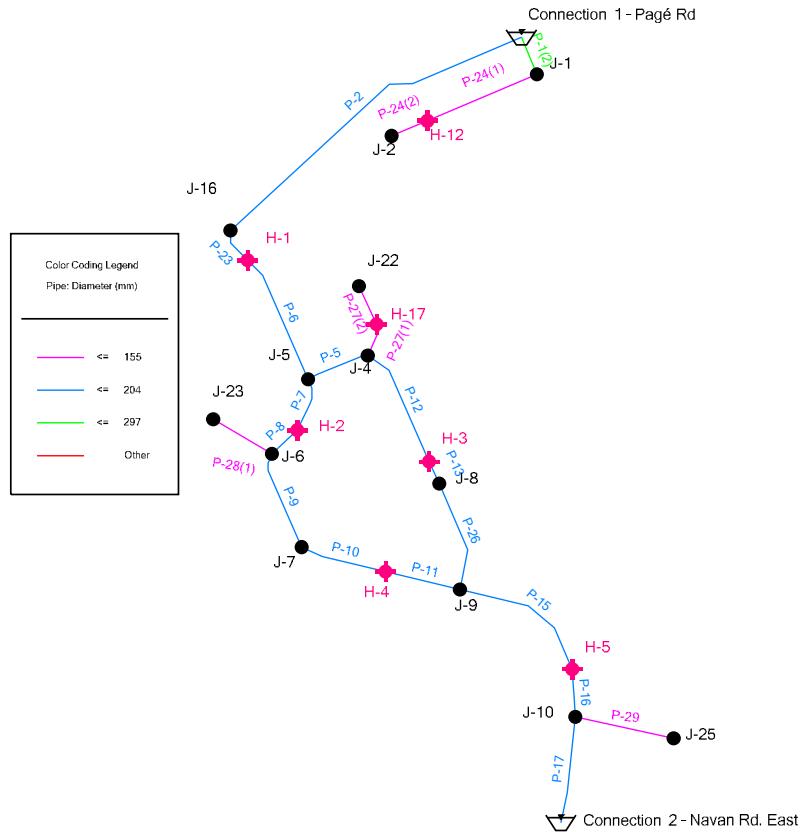
¹ Ground Elevation = 81.9 m

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of water mains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Mixed-Use Site Plan (Block 15)

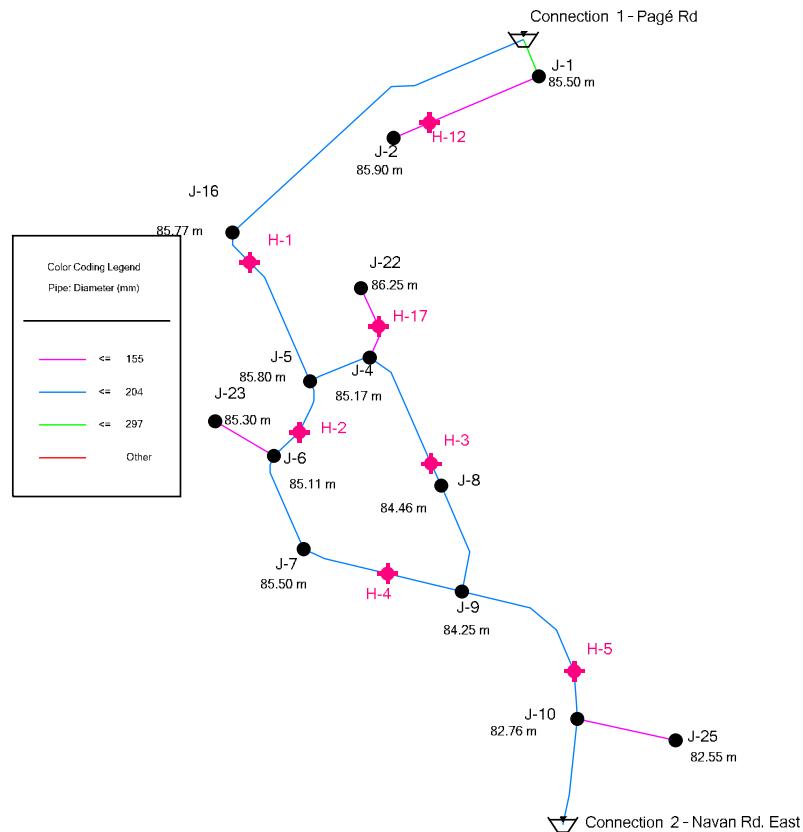
Model Schematic



Mixed-Use Site Plan (Block 15)

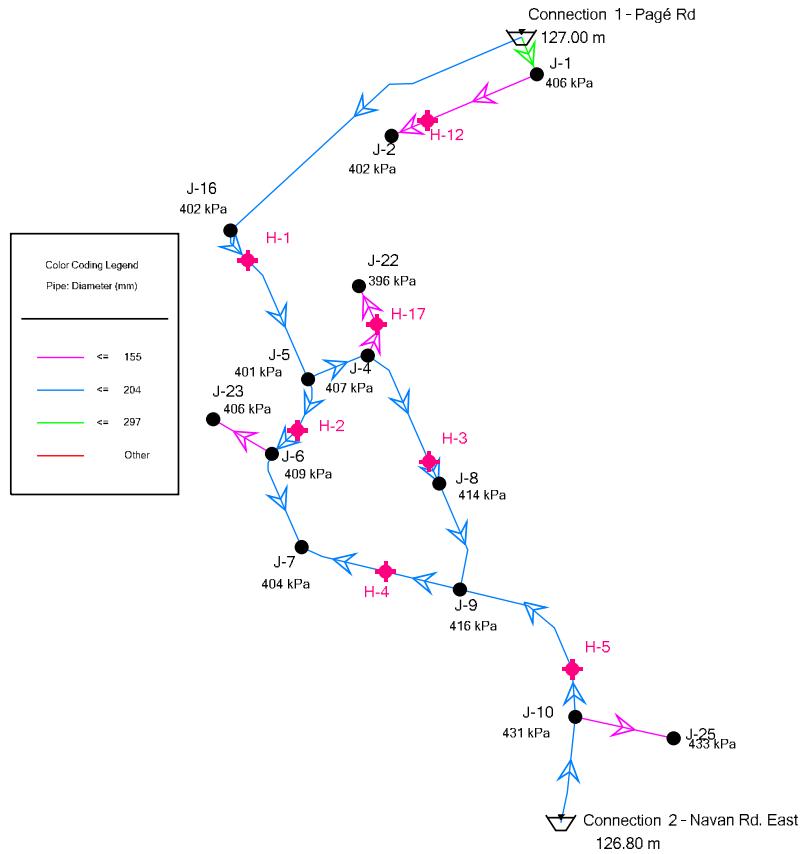
Model Schematic

Elevation Model



Mixed-Use Site Plan (Block 15)

Peak Hour Demand



Mixed-Use Site Plan (Block 15)

Peak Hour Demand

Junction Table

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-22	86.25	2.78	126.76	396
J-5	85.80	0.00	126.78	401
J-16	85.77	0.00	126.85	402
J-2	85.90	0.58	127.00	402
J-7	85.50	4.95	126.77	404
J-23	85.30	2.74	126.76	406
J-1	85.50	0.00	127.00	406
J-4	85.17	0.00	126.78	407
J-6	85.03	0.00	126.77	409
J-8	84.46	1.90	126.77	414
J-9	84.25	1.46	126.77	416
J-10	82.76	0.00	126.78	431
J-25	82.55	3.08	126.76	433

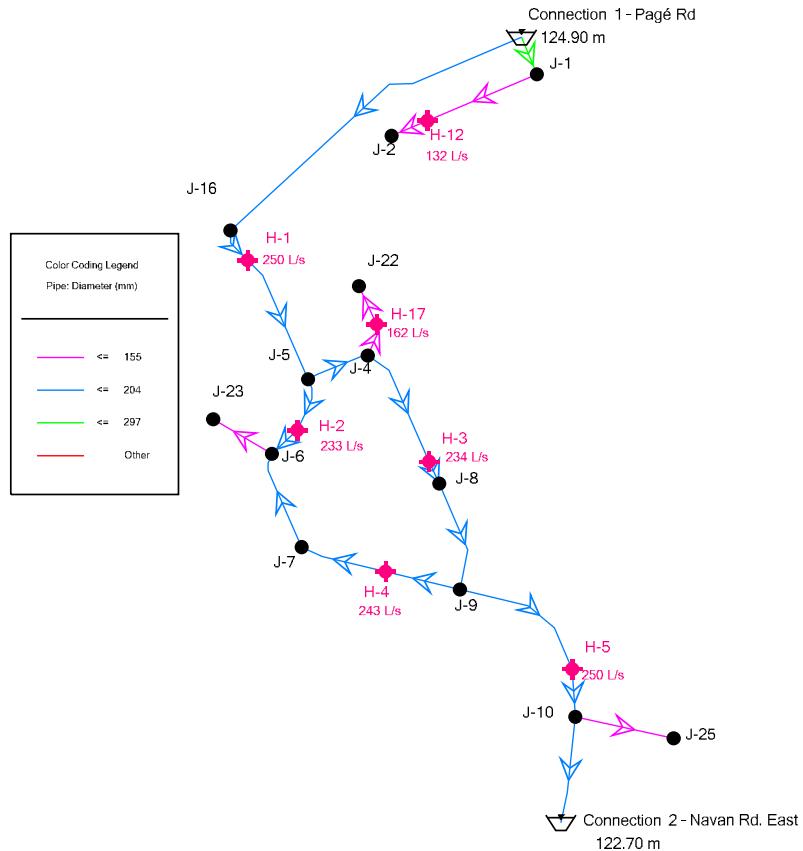
Mixed-Use Site Plan (Block 15)

Peak Hour Demand

Pipe Table

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
49	P-2	168	204	PVC	110.0	10	0.32
52	P-5	31	204	PVC	110.0	-5	0.15
82	P-6	64	204	PVC	110.0	-10	0.32
85	P-7	26	204	PVC	110.0	5	0.17
86	P-8	16	204	PVC	110.0	5	0.17
54	P-9	48	204	PVC	110.0	3	0.08
91	P-10	41	204	PVC	110.0	-2	0.07
92	P-11	36	204	PVC	110.0	-2	0.07
104	P-12	59	204	PVC	110.0	2	0.07
105	P-13	11	204	PVC	110.0	2	0.07
108	P-15	70	204	PVC	110.0	-3	0.10
109	P-16	23	204	PVC	110.0	-3	0.10
97	P-17	50	204	PVC	110.0	-6	0.20
110	P-23	17	204	PVC	110.0	10	0.32
129	P-24(1)	56	155	PVC	100.0	1	0.03
130	P-24(2)	18	155	PVC	100.0	1	0.03
123	P-26	53	204	PVC	110.0	0	0.01
154	P-27(1)	16	155	PVC	100.0	3	0.15
155	P-27(2)	20	155	PVC	100.0	3	0.15
171	P-28(1)	32	155	PVC	100.0	3	0.15
183	P-29	47	155	PVC	100.0	3	0.16

Mixed-Use Site Plan (Block 15) Max Day + Fire Flow Requirement

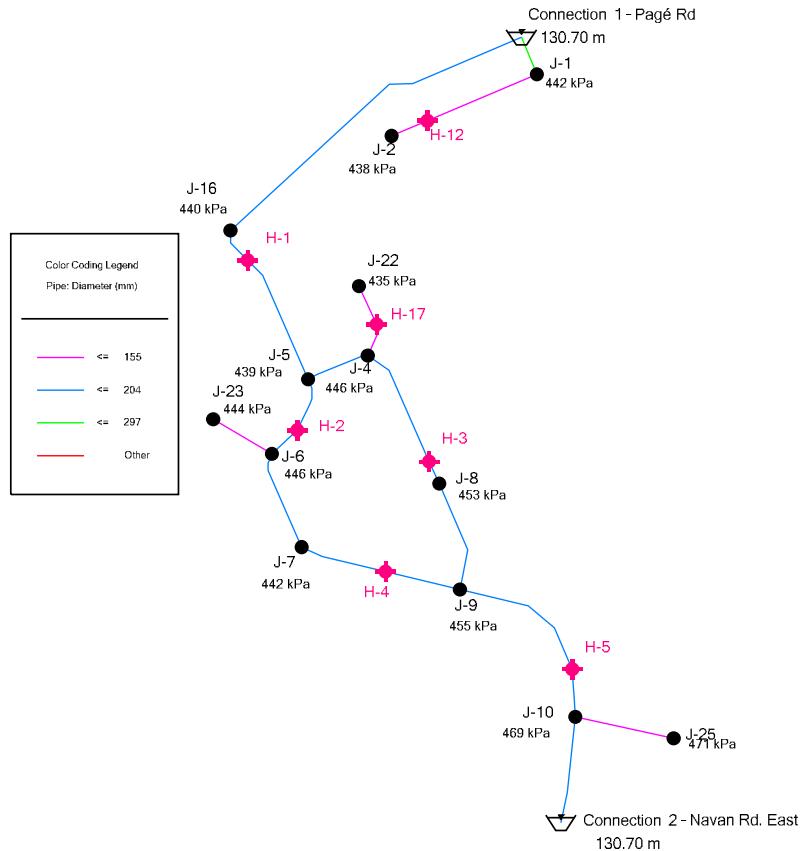


Mixed-Use Site Plan (Block 15)
Max Day + Fire Flow Requirement
Hydrant Table

Label	Satisfies Fire Flow Constraints?	Fire Flow (Available) (L/s)	Flow (Total Available) (L/s)	Pressure (Residual Lower Limit) (kPa)	Pressure (Calculated Residual) (kPa)	Pressure (Calculated System Lower Limit) (kPa)	Junction w/ Minimum Pressure (System)
H-1	True	250	250	140	152	178	J-16
H-2	True	233	233	140	140	142	J-23
H-4	True	243	243	140	140	142	J-7
H-3	True	234	234	140	140	158	J-8
H-5	True	250	250	140	256	247	J-23
H-12	True	132	132	140	142	140	J-2
H-17	True	162	162	140	143	140	J-22

Mixed-Use Site Plan (Block 15)

Maximum Pressure Analysis



Mixed-Use Site Plan (Block 15)

Maximum Pressure Analysis

Junction Table

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-22	86.25	0	130.70	435
J-2	85.90	0	130.70	438
J-5	85.80	0	130.70	439
J-16	85.77	0	130.70	440
J-7	85.50	0	130.70	442
J-1	85.50	0	130.70	442
J-23	85.30	0	130.70	444
J-4	85.17	0	130.70	446
J-6	85.11	0	130.70	446
J-8	84.46	0	130.70	453
J-9	84.25	0	130.70	455
J-10	82.76	0	130.70	469
J-25	82.55	0	130.70	471

Mixed-Use Site Plan (Block 15)
Maximum Pressure Analysis

Pipe Table

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
114	P-1(2)	19	297	PVC	120.0	0	0.00
49	P-2	168	204	PVC	110.0	0	0.00
52	P-5	31	204	PVC	110.0	0	0.00
82	P-6	64	204	PVC	110.0	0	0.00
85	P-7	26	204	PVC	110.0	0	0.00
86	P-8	16	204	PVC	110.0	0	0.00
54	P-9	48	204	PVC	110.0	0	0.00
91	P-10	41	204	PVC	110.0	0	0.00
92	P-11	36	204	PVC	110.0	0	0.00
104	P-12	59	204	PVC	110.0	0	0.00
105	P-13	11	204	PVC	110.0	0	0.00
108	P-15	70	204	PVC	110.0	0	0.00
109	P-16	23	204	PVC	110.0	0	0.00
97	P-17	50	204	PVC	110.0	0	0.00
110	P-23	17	204	PVC	110.0	0	0.00
129	P-24(1)	56	155	PVC	100.0	0	0.00
130	P-24(2)	18	155	PVC	100.0	0	0.00
123	P-26	53	204	PVC	110.0	0	0.00
154	P-27(1)	16	155	PVC	100.0	0	0.00
155	P-27(2)	20	155	PVC	100.0	0	0.00
171	P-28(1)	32	155	PVC	100.0	0	0.00
183	P-29	47	155	PVC	100.0	0	0.00

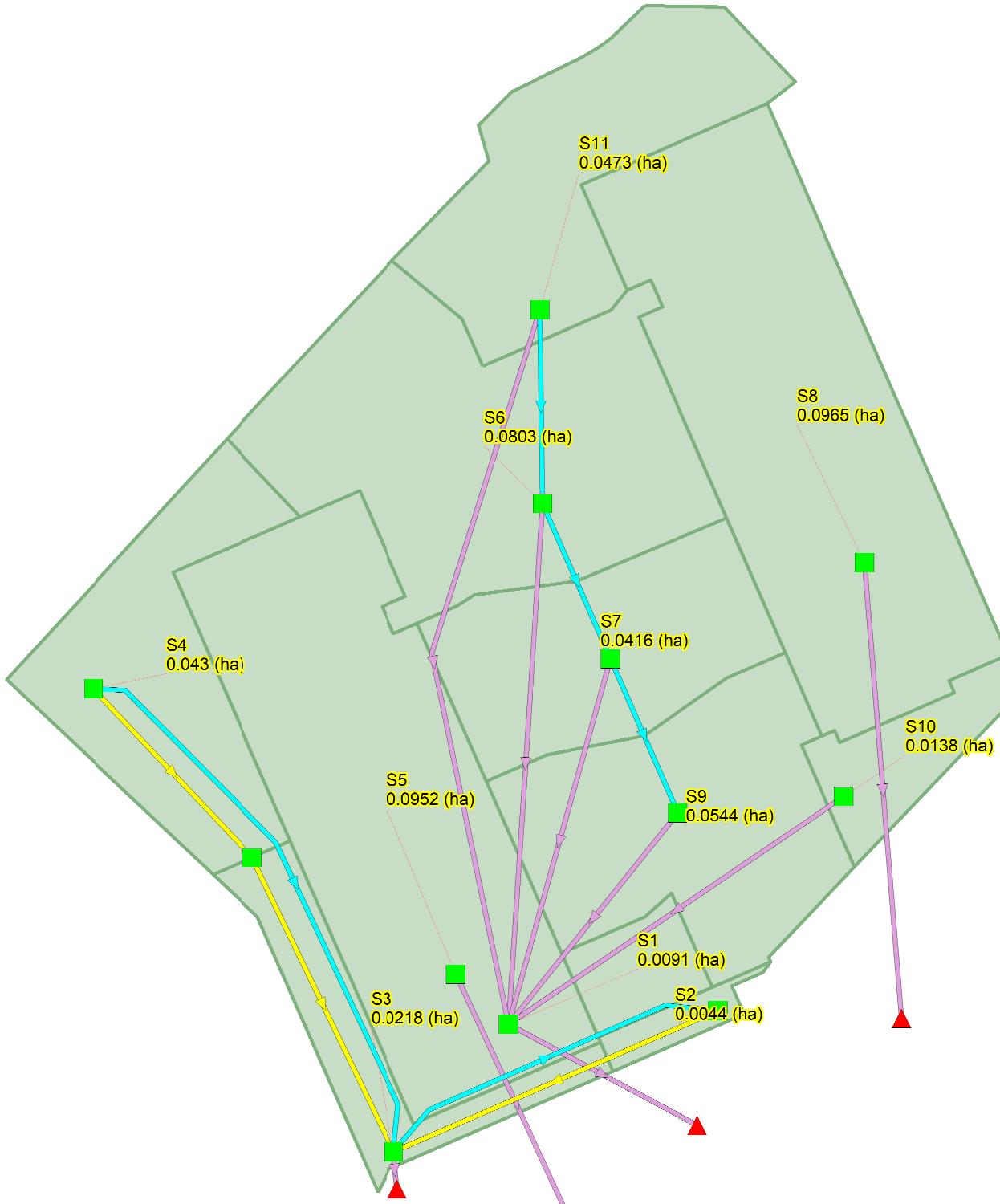
Site Servicing Report
2983, 3053 and 3079 Navan Road & 2690 Pagé Road

Appendix D

Stormwater Management

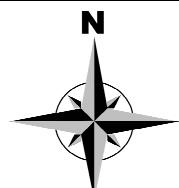
PIPE REACH		Peak Flow Estimation												Upstream Geometry												Downstream Geometry															
LOCATION	From MH	To MH	C-Factor (1.2)	0.20	0.50	Total Area (ha)	Add. Area from Upstream Runs	Cur. Total	Inlet Time (min.)	In Pipe Flow (m³/min.)	Total Time	2.1 Year Peak Flow (m³/min.)	2.1 Year Peak Flow (m³/min.)	1-3 Year Peak Intensity (hrs)	1-3 Year Peak Flow (m³/min.)	ICD Flow ⁽¹⁾ EX STM STB 16	65' Pipe Controlled Flow ⁽²⁾ (ft)	ICD Flow ⁽³⁾ CB 112	Rooftop Drains (ft)	Stormflow ⁽⁴⁾ (ft)	Total Peak Flow (m³/min.)	Type	Nominal Dia. (in.)	Actual Dia. (mm)	Slope	Length (m)	Q Full (m³/s)	Residual Capacity ⁽⁵⁾ (m³)	% Full	TG From	Obvert	Invert	Cover	TG To	Obvert	Invert	Cover	Q/I Ratio	Actual Flow (m³/min.)	Flow Rate (m³/min.)	Flow Rate (ft³/sec.)
EAST ORLEANS RIDGE SUBDIVISION / GAS BAR SITE PLAN (BLOCK 16)	UPSTREAM GAS STATION ⁽⁶⁾	EXST MH014	0.082	0.087	0.50	1.94	5.11	18.00	0.83	16.93	0.05	2.29	2.34	9.61	180.11	68.00	47.30	10.00	126.00	CONCRETE	420	531.00	0.03%	50.20	224.33	1.00	44.22	80%	80.30	61.40	80.95	3.00	34.93	81.35	0.80	0.00	204.30	1.00	1.00		
BLOCK 16	BLOCK 15	ST MH015	0.088	0.341	0.42	0.43	10.00	0.04	16.94	0.00	0.00	79.81	99.24	0.00	180.11	68.00	47.30	10.00	126.00	PVC	300	304.80	0.03%	50.20	142.67	1.00	73.42	40%	80.85	61.40	81.18	4.00	35.75	81.08	0.80	0.00	204.87	1.00	1.00		
EAST ORLEANS RIDGE SUBDIVISION	ST MH015	EXST MH014	0.088	0.341	0.42	0.43	10.00	0.04	16.94	0.00	0.00	79.84	99.24	0.00	180.11	68.00	47.30	10.00	126.00	CONCRETE	450	457.20	0.03%	10.00	148.72	0.91	79.83	40%	80.75	61.30	80.93	3.00	34.90	81.25	0.80	0.00	204.00	1.00	1.00		
EAST ORLEANS RIDGE SUBDIVISION	UPSTREAM SUBDIVISION ⁽⁶⁾	EXST MH014	0.082	0.087	0.50	1.94	5.11	18.00	0.83	16.93	0.05	0.00	0.00	0.00	121.07	0.00	12.00	0.00	12.00	PVC	250	250.00	1.00%	14.00	145.75	0.00	12.00	0.00	12.00	54.47	54.47	30%	0.00	0.00	0.00	0.00	Refer to Note 9	Refer to Note 9			
EAST ORLEANS RIDGE SUBDIVISION	EXST MH014	EXST MH012	0.27	1.366	16.82	1.90	11.98	0.05	0.06	4.10	73.75	0.00	0.00	0.00	124.00	0.00	12.00	0.00	12.00	CONCRETE	900	900.00	0.03%	42.00	370.96	1.00	76.47	80%	80.30	61.40	80.95	3.00	34.93	81.35	0.80	0.00	204.30	1.00	1.00		
Design Parameters (Per OSDO)																																									
Minoring Coefficient = 0.11																																									
C-Factor = 0.20 + 0.50 * (0.11 + 0.0001 * 8.0001 * 8.00)																																									
Note: Tc is the time of concentration in minutes																																									
Drainage Areas Breakdown		Total Site Area												Notes on Plug Flows												Notes on Pipe Flow and Pipe Sizes															
Controlled Area Within Site Property Line		0.04 ha												(1) Flow rate controlled downstream of EX STM STB 16 as part of the Gas Bar (Block 16)												(1) Peak flow rates are equal to the allowable release rate for Block 15 (43 L/s), gas station controlled flow (55.00 L/s), roadway controlled flow (23 L/s and 20 L/s), near yard controlled flow (12 L/s)															
Existing Areas - Outlets to Subdivision		0.407 ha												(2) ICD Flow Rate at CB112												(2) ICD Flow Rates are conservatively stated for 1.2 Year Peak Flow Rate															
Total Captured Area		0.504 ha												(3) ICD Flow Rate at CB112												(3) ICD Flow Rates are based on rooftop and roof drains for Buildings C and D															
Uncaptured Areas - Outlets to Subdivision		0.04 ha												(4) Stormflow Rates for rooftops and roof drains for Buildings C and D												(4) Stormflow Rates															
Additional Areas from Upstream Runs		1.40 ha												(5) Catcher Outflow Rate												(5) Catcher Outflow Rate															
Total Area from MH014 to MH012 on Subdivision Design Sheet		1.40 ha												Notes on Pipe Flow and Pipe Sizes												Notes on Pipe Flow and Pipe Sizes															
This area is a portion of the total site area (0.54 ha)														This area is a portion of the total site area (0.54 ha)													This area is a portion of the total site area (0.54 ha)														
This area is a portion of the total upstream runs (1.40 ha)														This area is the sum of the Block 15 captured areas (0.50 ha) and the upstream areas (1.40 ha)													This area is the sum of the Block 15 captured areas (0.50 ha) and the upstream areas (1.40 ha)														

REAR YARD CATCH BASIN TABLE



Legend

- ▲ Outfalls
- Storages
- Conduits
- Weirs
- Outlets
- Subcatchments



150 m

PROJECT:

NAVAN RESIDENTIAL AND COMMERCIAL SITE PLAN - BLOCK 15 Ottawa, ON

DRAWING:

Overall System Model Schematic

J.L.Richards
ENGINEERS · ARCHITECTS · PLANNERS

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DESIGN:
ML

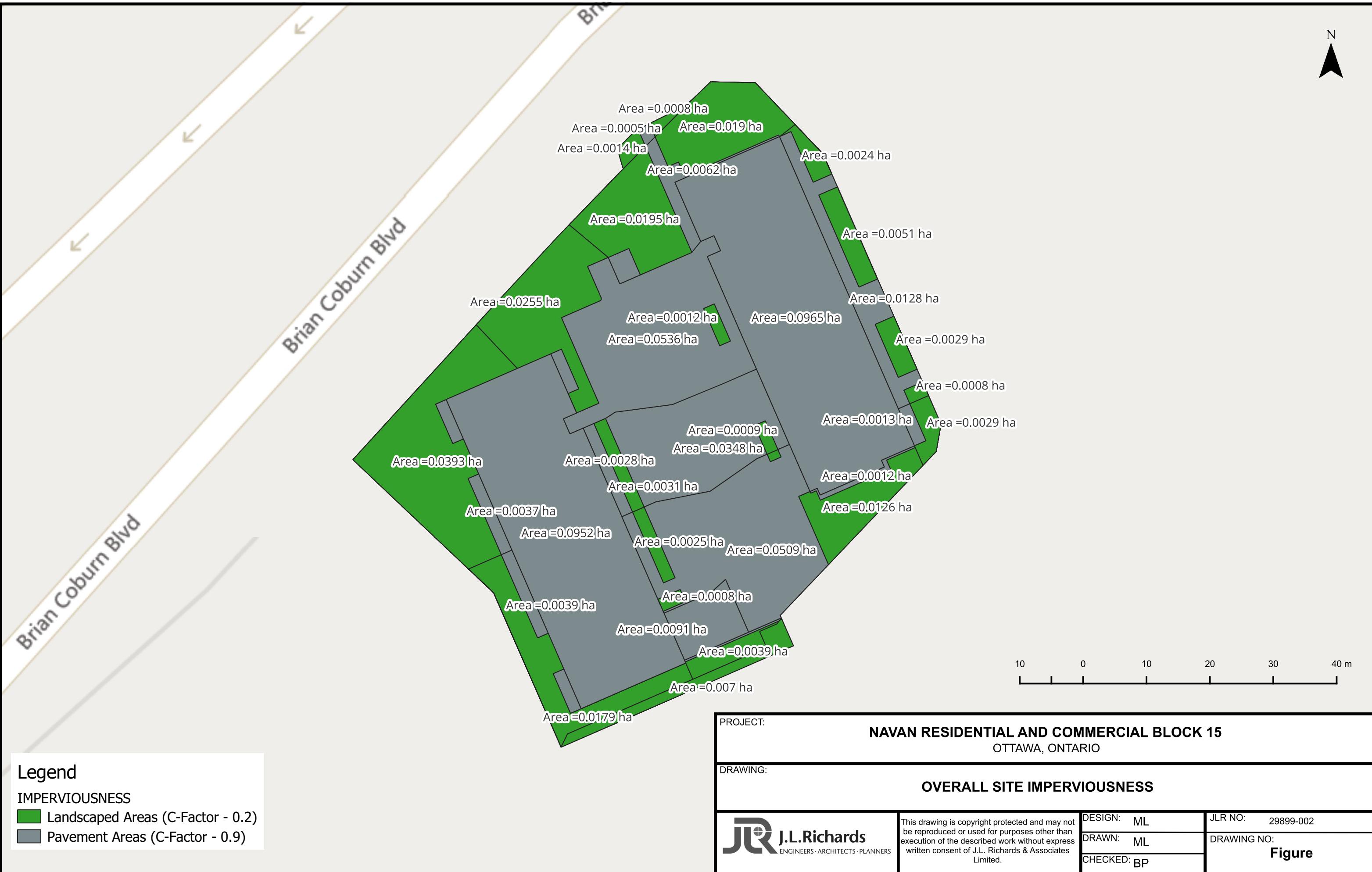
JLR NO.:
29899-002

DRAWN:
ML

DRAWING NO.:

Figure 1

CHECKED:
BP



Post-Development 3-hour Chicago 1:2 year Event

Post-Development 3-hour Chicago 1:2 year Event

```
[TITLE]
;;Project Title/Notes

[OPTIONS]
;;Option      Value
FLOW_UNITS    CMS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE    01/01/2000
START_TIME    00:00:00
REPORT_START_DATE 01/01/2000
REPORT_START_TIME 00:00:00
END_DATE     01/01/2000
END_TIME     06:00:00
SWEEP_START   01/01
SWEEP_END    12/31
DRY_DAYS     0
REPORT_STEP   00:01:00
WET_STEP      00:01:00
DRY_STEP      00:01:00
ROUTING_STEP  5
RULE_STEP    00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQNATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA   0
MAX_TRIALS     8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL   5
LAT_FLOW_TOL   5
MINIMUM_STEP   0.5
THREADS       12

[EVAPORATION]
;;Data Source Parameters
CONSTANT      0.0
DRY_ONLY      NO

[RAINGAGES]
;;Name Format Interval SCF Source
;;----- -----
Rainfall     INTENSITY 0:10 1.0  TIMESERIES 3CHI002

[SUBCATCHMENTS]
;;Name Rain Gage Outlet Area %Imperv Width %Slope
CurLen SnowPack
;;----- -----
S1 Rainfall St_UnGrd 0.0991 100 6.1 15 0
S10 Rainfall CB118 0.0138 8.79 28.998 1 0
S11 Rainfall CB114 0.0473 18.805 70.001 2 0
S2 Rainfall CB113 0.0044 0.002 4.3 1.9 0
S3 Rainfall CB112 0.0214 17.802 75 1 0
S4 Rainfall CB110 0.043 8.51 52.001 3.5 0
S5 Rainfall Root_1 0.0852 99.998 104.996 1 0
S6 Rainfall CB115 0.0033 60.775 45 2.5 0
S7 Rainfall CB116 0.0416 90.416 28.601 1 0
S8 Rainfall Roof_2 0.0965 100 105.005 1.7 0
S9 Rainfall CB117 0.0544 93.568 50 2.4 0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo
PctRouted
;;----- -----
S1 0.013 0.25 1.57 4.67 0 OUTLET
S10 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S11 0.013 0.25 1.57 4.67 0 OUTLET
S2 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S3 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S4 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S5 0.013 0.25 1.57 4.67 0 OUTLET
S6 0.013 0.25 1.57 4.67 0 OUTLET
S7 0.013 0.25 1.57 4.67 0 OUTLET
S8 0.013 0.25 1.57 4.67 0 OUTLET
S9 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]
;;Subcatchment Param1 Param2 Param3 Param4 Param5
;;----- -----
S1 76.2 13.2 4.14 7 0
S10 76.2 13.2 4.14 7 0
S11 76.2 13.2 4.14 7 0
S2 76.2 13.2 4.14 7 0
S3 76.2 13.2 4.14 7 0
S4 76.2 13.2 4.14 7 0
S5 76.2 13.2 4.14 7 0
S6 76.2 13.2 4.14 7 0
S7 76.2 13.2 4.14 7 0
S8 76.2 13.2 4.14 7 0
S9 76.2 13.2 4.14 7 0

[OUTFALLS]
;;Name Elevation Type Stage Data Gated Route To
;;----- -----
OF4 80.628 FIXED 81.11 NO
OF7 80.628 FIXED 81.11 NO
STUB15 80.628 FIXED 81.11 NO
STUB16 80.646 FIXED 81.28 NO

[STORAGE]
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params
SurDepth Fvap Psi Ksat IMD
;;----- -----
CB110 83.75 1.9 0 TABULAR CB110 0
CB111 83.59 2.01 0 TABULAR CB111 0
CB112 83.28 2.12 0 TABULAR CB112 0
CB113 83.55 1.7 0 TABULAR CB113 0
CB114 85.3 0.25 0 TABULAR CB114 0
CB115 85.25 0.31 0 TABULAR CB115 0
CB116 85.2 0.4 0 TABULAR CB116 0
CB117 85.15 0.15 0 TABULAR CB117 0
CB118 85.22 0.18 0 TABULAR CB118 0
Roof_1 96.35 0.15 0 FUNCTIONAL 0 0 635 0
Roof_2 96.35 0.15 0 FUNCTIONAL 0 0 643 0
St_UnGrd 81.8 1 0 FUNCTIONAL 0 0 49 0

[CONDUITS]
;;Name From Node To Node Length Roughness InOffset
OutOffset InitFlow MaxFlow
;;----- -----

```

Post-Development 3-hour Chicago 1:2 year Event

MHF_IPEX_TYPE_D	0.9	0.0461	Vortex_ICD_55	0.6	0.0021
MHF_IPEX_TYPE_D	1	0.0485	Vortex_ICD_55	0.7	0.0023
MHF_IPEX_TYPE_D	1.2	0.0532	Vortex_ICD_55	0.8	0.0024
MHF_IPEX_TYPE_D	1.4	0.0574	Vortex_ICD_55	0.9	0.0026
MHF_IPEX_TYPE_D	1.6	0.0614	Vortex_ICD_55	1	0.0027
MHF_IPEX_TYPE_D	1.8	0.0651	Vortex_ICD_55	1.2	0.003
MHF_IPEX_TYPE_D	2	0.0687	Vortex_ICD_55	1.4	0.0032
MHF_IPEX_TYPE_D	2.5	0.0768	Vortex_ICD_55	1.6	0.0033
MHF_IPEX_TYPE_D	3	0.0841	Vortex_ICD_55	1.8	0.0036
;Tempest Rating Curve for MHF IPEX TYPE E, No grate allowance			Vortex_ICD_55	2	0.0038
MHF_IPEX_TYPE_E	Rating	0	Vortex_ICD_55	2.5	0.0043
MHF_IPEX_TYPE_E	0.1	0.0205	Vortex_ICD_55	3	0.0047
MHF_IPEX_TYPE_E	0.2	0.0289	;Tempest Rating Curve for Vortex ICD 60, No grate allowance		
MHF_IPEX_TYPE_E	0.3	0.0355	Vortex_ICD_60	Rating	0
MHF_IPEX_TYPE_E	0.4	0.0409	Vortex_ICD_60	0.1	0.0011
MHF_IPEX_TYPE_E	0.5	0.0458	Vortex_ICD_60	0.2	0.0015
MHF_IPEX_TYPE_E	0.6	0.0501	Vortex_ICD_60	0.3	0.0018
MHF_IPEX_TYPE_E	0.7	0.0542	Vortex_ICD_60	0.4	0.0021
MHF_IPEX_TYPE_E	0.8	0.0579	Vortex_ICD_60	0.5	0.0023
MHF_IPEX_TYPE_E	0.9	0.0614	Vortex_ICD_60	0.6	0.0025
MHF_IPEX_TYPE_E	1	0.0677	Vortex_ICD_60	0.7	0.0027
MHF_IPEX_TYPE_E	1.2	0.0709	Vortex_ICD_60	0.8	0.0029
MHF_IPEX_TYPE_E	1.4	0.0766	Vortex_ICD_60	0.9	0.0031
MHF_IPEX_TYPE_E	1.6	0.0819	Vortex_ICD_60	1	0.0032
MHF_IPEX_TYPE_E	1.8	0.0868	Vortex_ICD_60	1.2	0.0036
MHF_IPEX_TYPE_E	2	0.0915	Vortex_ICD_60	1.4	0.0038
MHF_IPEX_TYPE_E	2.5	0.1023	Vortex_ICD_60	1.6	0.0041
MHF_IPEX_TYPE_E	3	0.1121	Vortex_ICD_60	1.8	0.0045
O_Roof1	Rating	0	Vortex_ICD_60	2	0.0046
O_Roof1	0.001	0.002	Vortex_ICD_60	2.5	0.0051
O_Roof1	0.15	0.002	Vortex_ICD_60	3	0.0056
O_Roof2	Rating	0	;Tempest Rating Curve for Vortex ICD 65, No grate allowance		
O_Roof2	0.001	0.0025	Vortex_ICD_65	Rating	0
O_Roof2	0.15	0.0025	Vortex_ICD_65	0.1	0.0012
O_St_UnGrd	Rating	0	Vortex_ICD_65	0.2	0.0016
O_St_UnGrd	0.01	0.029	Vortex_ICD_65	0.3	0.002
O_St_UnGrd	1	0.029	Vortex_ICD_65	0.4	0.0023
;Tempest Rating Curve for Vortex ICD 100, No grate allowance			Vortex_ICD_65	0.5	0.0025
Vortex_ICD_100	Rating	0	Vortex_ICD_65	0.6	0.0028
Vortex_ICD_100	0.1	0.0028	Vortex_ICD_65	0.7	0.003
Vortex_ICD_100	0.2	0.004	Vortex_ICD_65	0.8	0.0032
Vortex_ICD_100	0.3	0.0049	Vortex_ICD_65	0.9	0.0034
Vortex_ICD_100	0.4	0.0056	Vortex_ICD_65	1	0.0036
Vortex_ICD_100	0.5	0.0063	Vortex_ICD_65	1.2	0.004
Vortex_ICD_100	0.6	0.0069	Vortex_ICD_65	1.4	0.0043
Vortex_ICD_100	0.7	0.0075	Vortex_ICD_65	1.6	0.0046
Vortex_ICD_100	0.8	0.008	Vortex_ICD_65	1.8	0.0049
Vortex_ICD_100	0.9	0.0085	Vortex_ICD_65	2	0.0051
Vortex_ICD_100	1	0.0089	Vortex_ICD_65	2.5	0.0057
Vortex_ICD_100	1.2	0.0098	Vortex_ICD_65	3	0.0063
Vortex_ICD_100	1.4	0.0106	;Tempest Rating Curve for Vortex ICD 70, No grate allowance		
Vortex_ICD_100	1.6	0.0113	Vortex_ICD_70	Rating	0
Vortex_ICD_100	1.8	0.012	Vortex_ICD_70	0.1	0.0015
Vortex_ICD_100	2	0.0126	Vortex_ICD_70	0.2	0.0019
Vortex_ICD_100	2.5	0.0141	Vortex_ICD_70	0.3	0.0023
Vortex_ICD_100	3	0.0155	Vortex_ICD_70	0.4	0.0027
;Tempest Rating Curve for Vortex ICD 105, No grate allowance			Vortex_ICD_70	0.5	0.003
Vortex_ICD_105	Rating	0	Vortex_ICD_70	0.6	0.0033
Vortex_ICD_105	0.1	0.0031	Vortex_ICD_70	0.7	0.0036
Vortex_ICD_105	0.2	0.0044	Vortex_ICD_70	0.8	0.0038
Vortex_ICD_105	0.3	0.0054	Vortex_ICD_70	0.9	0.0041
Vortex_ICD_105	0.4	0.0062	Vortex_ICD_70	1	0.0043
Vortex_ICD_105	0.5	0.0069	Vortex_ICD_70	1.2	0.0047
Vortex_ICD_105	0.6	0.0076	Vortex_ICD_70	1.4	0.0051
Vortex_ICD_105	0.7	0.0082	Vortex_ICD_70	1.6	0.0055
Vortex_ICD_105	0.8	0.0088	Vortex_ICD_70	1.8	0.0058
Vortex_ICD_105	0.9	0.0093	Vortex_ICD_70	2	0.0061
Vortex_ICD_105	1	0.0098	Vortex_ICD_70	2.5	0.0068
Vortex_ICD_105	1.2	0.0107	Vortex_ICD_70	3	0.0075
Vortex_ICD_105	1.4	0.0116	;Tempest Rating Curve for Vortex ICD 75, No grate allowance		
Vortex_ICD_105	1.6	0.0124	Vortex_ICD_75	Rating	0
Vortex_ICD_105	1.8	0.0131	Vortex_ICD_75	0.1	0.0016
Vortex_ICD_105	2	0.0139	Vortex_ICD_75	0.2	0.0022
Vortex_ICD_105	2.5	0.0155	Vortex_ICD_75	0.3	0.0027
Vortex_ICD_105	3	0.017	Vortex_ICD_75	0.4	0.0032
;Tempest Rating Curve for Vortex ICD 40, No grate allowance			Vortex_ICD_75	0.5	0.0035
Vortex_ICD_40	Rating	0	Vortex_ICD_75	0.6	0.0039
Vortex_ICD_40	0.1	0.0004	Vortex_ICD_75	0.7	0.0042
Vortex_ICD_40	0.2	0.0006	Vortex_ICD_75	0.8	0.0045
Vortex_ICD_40	0.3	0.0007	Vortex_ICD_75	0.9	0.0048
Vortex_ICD_40	0.4	0.0009	Vortex_ICD_75	1	0.005
Vortex_ICD_40	0.5	0.001	Vortex_ICD_75	1.2	0.0055
Vortex_ICD_40	0.6	0.001	Vortex_ICD_75	1.4	0.0059
Vortex_ICD_40	0.7	0.0011	Vortex_ICD_75	1.6	0.0063
Vortex_ICD_40	0.8	0.0012	Vortex_ICD_75	1.8	0.0067
Vortex_ICD_40	0.9	0.0013	Vortex_ICD_75	2	0.0071
Vortex_ICD_40	1	0.0014	Vortex_ICD_75	2.5	0.0079
Vortex_ICD_40	1.2	0.0015	Vortex_ICD_75	3	0.0087
Vortex_ICD_40	1.4	0.0016	;Tempest Rating Curve for Vortex ICD 80, No grate allowance		
Vortex_ICD_40	1.6	0.0017	Vortex_ICD_80	Rating	0
Vortex_ICD_40	1.8	0.0018	Vortex_ICD_80	0.1	0.0018
Vortex_ICD_40	2	0.0019	Vortex_ICD_80	0.2	0.0026
Vortex_ICD_40	2.5	0.0022	Vortex_ICD_80	0.3	0.0031
Vortex_ICD_40	3	0.0024	Vortex_ICD_80	0.4	0.0036
;Tempest Rating Curve for Vortex ICD 45, No grate allowance			Vortex_ICD_80	0.5	0.0041
Vortex_ICD_45	Rating	0	Vortex_ICD_80	0.6	0.0044
Vortex_ICD_45	0.1	0	Vortex_ICD_80	0.7	0.0048
Vortex_ICD_45	0.2	0.0006	Vortex_ICD_80	0.8	0.0051
Vortex_ICD_45	0.3	0.0007	Vortex_ICD_80	0.9	0.0054
Vortex_ICD_45	0.4	0.0009	Vortex_ICD_80	1	0.0057
Vortex_ICD_45	0.5	0.001	Vortex_ICD_80	1.2	0.0063
Vortex_ICD_45	0.6	0.0011	Vortex_ICD_80	1.4	0.0068
Vortex_ICD_45	0.7	0.0013	Vortex_ICD_80	1.6	0.0072
Vortex_ICD_45	0.8	0.0014	Vortex_ICD_80	1.8	0.0077
Vortex_ICD_45	0.9	0.0015	Vortex_ICD_80	2	0.0081
Vortex_ICD_45	1	0.0016	Vortex_ICD_80	2.5	0.0089
Vortex_ICD_45	1.2	0.002	Vortex_ICD_80	3	0.0099
Vortex_ICD_45	1.4	0.0021	;Tempest Rating Curve for Vortex ICD 85, No grate allowance		
Vortex_ICD_45	1.6	0.0023	Vortex_ICD_85	Rating	0
Vortex_ICD_45	1.8	0.0024	Vortex_ICD_85	0.1	0.002
Vortex_ICD_45	2	0.0026	Vortex_ICD_85	0.2	0.0029
Vortex_ICD_45	2.5	0.0029	Vortex_ICD_85	0.3	0.0035
Vortex_ICD_45	3	0.0031	Vortex_ICD_85	0.4	0.0041
;Tempest Rating Curve for Vortex ICD 50, No grate allowance			Vortex_ICD_85	0.5	0.0045
Vortex_ICD_50	Rating	0	Vortex_ICD_85	0.6	0.005
Vortex_ICD_50	0.1	0.0007	Vortex_ICD_85	0.7	0.0054
Vortex_ICD_50	0.2	0.001	Vortex_ICD_85	0.8	0.0057
Vortex_ICD_50	0.3	0.0012	Vortex_ICD_85	0.9	0.0061
Vortex_ICD_50	0.4	0.0014	Vortex_ICD_85	1	0.0064
Vortex_ICD_50	0.5	0.0016	Vortex_ICD_85	1.2	0.0067
Vortex_ICD_50	0.6	0.0018	Vortex_ICD_85	1.4	0.0076
Vortex_ICD_50	0.7	0.0019	Vortex_ICD_85	1.6	0.0077
Vortex_ICD_50	0.8	0.0021	Vortex_ICD_85	1.8	0.0086
Vortex_ICD_50	0.9	0.0023	Vortex_ICD_85	2	0.0091
Vortex_ICD_50	1	0.0025	Vortex_ICD_85	2.5	0.0101
Vortex_ICD_50	1.2	0.0027	Vortex_ICD_85	3	0.0111
Vortex_ICD_50	1.4	0.0029	;Tempest Rating Curve for Vortex ICD 90, No grate allowance		
Vortex_ICD_50	1.6	0.003	Vortex_ICD_90	Rating	0
Vortex_ICD_50	1.8	0.003	Vortex_ICD_90	0.1	0.0022
Vortex_ICD_50	2	0.0032	Vortex_ICD_90	0.2	0.0032
Vortex_ICD_50	2.5	0.0036	Vortex_ICD_90	0.3	0.0039
Vortex_ICD_50	3	0.0039	Vortex_ICD_90	0.4	0.0045
;Tempest Rating Curve for Vortex ICD 55, No grate allowance			Vortex_ICD_90	0.5	0.0051
Vortex_ICD_55	Rating	0	Vortex_ICD_90	0.6	0.0055
Vortex_ICD_55	0.1	0.0009	Vortex_ICD_90	0.7	0.006
Vortex_ICD_55	0.2	0.0012	Vortex_ICD_90	0.8	0.0064
Vortex_ICD_55	0.3	0.0015	Vortex_ICD_90	0.9	0.0068
Vortex_ICD_55	0.4	0.0017	Vortex_ICD_90	1	0.0072
Vortex_ICD_55	0.5	0.0019	Vortex_ICD_90	1.2	0.0079
Vortex_ICD_55	0.6	0.0021	Vortex_ICD_90	1.4	0.0085
Vortex_ICD_55	0.7	0.0023	Vortex_ICD_90	1.6	0.0091
Vortex_ICD_55	0.8	0.0025	Vortex_ICD_90	1.8	0.0096

Post-Development 3-hour Chicago 1:2 year Event

Vortex_ICD_90 2 0.0102
Vortex_ICD_90 2.5 0.0114
Vortex_ICD_90 3 0.0125

;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex_ICD_95 Rating 0 0
Vortex_ICD_95 0.1 0.0026
Vortex_ICD_95 0.2 0.0046
Vortex_ICD_95 0.3 0.0044
Vortex_ICD_95 0.4 0.0051
Vortex_ICD_95 0.5 0.0057
Vortex_ICD_95 0.6 0.0062
Vortex_ICD_95 0.7 0.0067
Vortex_ICD_95 0.8 0.0071
Vortex_ICD_95 0.9 0.0076
Vortex_ICD_95 1 0.008
Vortex_ICD_95 1.2 0.0087
Vortex_ICD_95 1.4 0.0094
Vortex_ICD_95 1.6 0.0101
Vortex_ICD_95 1.8 0.0107
Vortex_ICD_95 2 0.0113
Vortex_ICD_95 2.5 0.0126
Vortex_ICD_95 3 0.0138

;From Zurn Manual RD178
ZURN_Z150F-6NH Rating 0 0
ZURN_Z150F-6NH 0.0127 0.00503838308477861
ZURN_Z150F-6NH 0.0257 0.013242013231177
ZURN_Z150F-6NH 0.0384 0.0261203653888
ZURN_Z150F-6NH 0.0508 0.0357128365763805
ZURN_Z150F-6NH 0.0765 0.0425019726111045
ZURN_Z150F-6NH 0.1024 0.0433196215564931

	Storage	0	0.073
CB110	0	0.073	
CB110	1.55	0.073	
CB110	1.85	120.85	
CB110	1.9	120.85	
CB111	Storage	0	0.073
CB111	2.01	0.073	
CB112	Storage	0	0.36
CB112	1.92	0.36	
CB112	2.1	19.13	
CB112	2.12	19.13	
CB113	Storage	0	0.073
CB113	1.6	0.073	
CB113	1.7	3.62	
CB114	Storage	0	0.36
CB114	0.2	19.2	
CB114	0.25	19.2	
CB115	Storage	0	0.36
CB115	0.2	155.53	
CB115	0.31	155.53	
CB116	Storage	0	0.36
CB116	0.15	73.19	
CB116	0.4	73.19	
CB117	Storage	0	0.36
CB117	0.15	122.26	
CB118	Storage	0	0.36
CB118	0.18	37.7	

[TIMESERIES]
;;Name Date Time Value

;;Rainfall (mm/hr)

	01/01/2000 00:00:00	2.491
3CHI002	01/01/2000 00:10:00	2.966
3CHI002	01/01/2000 00:20:00	3.696
3CHI002	01/01/2000 00:30:00	4.976
3CHI002	01/01/2000 00:40:00	7.828
3CHI002	01/01/2000 00:50:00	19.966
3CHI002	01/01/2000 01:00:00	76.005
3CHI002	01/01/2000 01:10:00	22.777
3CHI002	01/01/2000 01:20:00	11.852
3CHI002	01/01/2000 01:30:00	8.025
3CHI002	01/01/2000 01:40:00	6.096
3CHI002	01/01/2000 01:50:00	4.938
3CHI002	01/01/2000 02:00:00	4.165
3CHI002	01/01/2000 02:10:00	3.132
3CHI002	01/01/2000 02:20:00	3.197
3CHI002	01/01/2000 02:30:00	2.873
3CHI002	01/01/2000 02:40:00	2.613
3CHI002	01/01/2000 02:50:00	2.4
3CHI002	01/01/2000 03:00:00	0

;;Rainfall (mm/hr)

	01/01/2000 00:00:00	5.339
3CHI100	01/01/2000 00:10:00	6.376
3CHI100	01/01/2000 00:20:00	7.977
3CHI100	01/01/2000 00:30:00	10.797
3CHI100	01/01/2000 00:40:00	17.136
3CHI100	01/01/2000 00:50:00	45.128
3CHI100	01/01/2000 01:00:00	178.907
3CHI100	01/01/2000 01:10:00	51.056
3CHI100	01/01/2000 01:20:00	26.163
3CHI100	01/01/2000 01:30:00	17.571
3CHI100	01/01/2000 01:40:00	13.277
3CHI100	01/01/2000 01:50:00	10.712
3CHI100	01/01/2000 02:00:00	9.008
3CHI100	01/01/2000 02:10:00	7.793
3CHI100	01/01/2000 02:20:00	6.883
3CHI100	01/01/2000 02:30:00	6.174
3CHI100	01/01/2000 02:40:00	5.607
3CHI100	01/01/2000 02:50:00	5.142
3CHI100	01/01/2000 03:00:00	0

;Sum of Infiltrated flow from above the garage slab calculated from Infiltration graphs for S11 multiplied by the total pervious area above the garage.

InfiltratedInflow 01/01/2000 00:01:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:02:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:03:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:04:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:05:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:06:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:07:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:08:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:09:00 3.711007E-05
InfiltratedInflow 01/01/2000 00:10:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:11:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:12:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:13:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:14:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:15:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:16:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:17:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:18:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:19:00 4.418645E-05
InfiltratedInflow 01/01/2000 00:20:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:21:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:22:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:23:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:24:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:25:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:26:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:27:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:28:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:29:00 5.506175E-05
InfiltratedInflow 01/01/2000 00:30:00 7.413075E-05
InfiltratedInflow 01/01/2000 00:31:00 7.810222E-05
InfiltratedInflow 01/01/2000 00:32:00 8.760457E-05
InfiltratedInflow 01/01/2000 00:33:00 9.052508E-05
InfiltratedInflow 01/01/2000 00:34:00 9.114623E-05

InfiltratedInflow 01/01/2000 00:35:00 9.126959E-05
InfiltratedInflow 01/01/2000 00:36:00 9.129378E-05
InfiltratedInflow 01/01/2000 00:37:00 9.12985E-05
InfiltratedInflow 01/01/2000 00:38:00 9.129943E-05
InfiltratedInflow 01/01/2000 00:39:00 9.12996E-05
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InfiltratedInflow 01/01/2000 00:41:00 0.0001420377
InfiltratedInflow 01/01/2000 00:42:00 0.0001435931
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InfiltratedInflow 01/01/2000 00:48:00 0.0001436281
InfiltratedInflow 01/01/2000 00:49:00 0.0001436281
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InfiltratedInflow 01/01/2000 00:54:00 0.0003663276
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InfiltratedInflow 01/01/2000 00:58:00 0.0003663362
InfiltratedInflow 01/01/2000 00:59:00 0.0003663362
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InfiltratedInflow 01/01/2000 01:03:00 0.0006335554
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InfiltratedInflow 01/01/2000 01:10:00 0.000446003
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InfiltratedInflow 01/01/2000 01:23:00 0.0003040496
InfiltratedInflow 01/01/2000 01:24:00 0.0003016868
InfiltratedInflow 01/01/2000 01:25:00 0.0002920555
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InfiltratedInflow 01/01/2000 01:43:00 0.0002236687
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InfiltratedInflow 01/01/2000 01:45:00 0.0002218153
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InfiltratedInflow 01/01/2000 01:47:00 0.000217152
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Post-Development 3-hour Chicago 1:2 year Event

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[REPORT]
 ;;Reporting Options
 INPUT YES
 CONTROLS NO
 SUBCATCHMENTS ALL
 NODES ALL
 LINKS ALL

[TAGS]
 Subcatch S1 Ramp
 Subcatch S5 Building
 Subcatch S6 ParkingLot
 Subcatch S7 ParkingLot
 Subcatch S8 Building
 Subcatch S9 ParkingLot
 Node St_UnGrd Underground_Storage

[MAP]
 DIMENSIONS 381280.91645 5032948.93205 381379.86255 5033068.41295
 UNITS Meters

[COORDINATES]
 ;;Node X-Coord Y-Coord
 ;;-----
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 OF7 381346.502 5032963.916
 STUB15 381335.776 5032954.363
 STUB16 381319.979 5032951.277
 CB116 381299.102 5033002.143
 CB111 381307.131 5032987.174
 CB112 381319.643 5032961.536
 CB113 381348.344 5032974.064
 CB114 381332.566 5033036.058
 CB115 381332.827 5033118.906
 CB116 381316.815 5033091.889
 CB117 381340.774 5032991.599
 CB118 381359.473 5032992.996
 Roof_1 381325.164 5032977.273
 Roof_2 381361.335 5033013.684
 St_UnGrd 381329.806 5032972.84

[VERTICES]
 ;;Link X-Coord Y-Coord
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 W3 381322.829 5032965.271
 W3 381343.656 5032974.514
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 W5 381309.233 5032988.935
 W5 381320.071 5032965.768
 OL6 381323.041 5033005.693

[POLYGONS]
 ;;Subcatchment X-Coord Y-Coord
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 S1 381344.215 5032984.464
 S10 381360.442 5032986.777
 S10 381355.738 5032997.554
 S10 381357.523 5032998.333
 S10 381358.707 5032999.877
 S10 381359.142 5032997.774
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 S10 381374.019 5033005.534
 S10 381375.365 5033002.452
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 S2 381337.733 5032971.705
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 S4 381304.879 5033024.646
 S4 381311.368 5033017.747
 S4 381321.435 5032964.05
 S4 381337.931 5032971.251
 S4 381339.051 5032968.685
 S4 381319.267 5032960.048
 S4 381318.559 5032961.111
 S4 381330.659 5032986.085
 S4 381325.414 5033003.349
 S4 381304.879 5033024.646
 S4 381311.368 5033017.747
 S4 381321.435 5032964.05
 S4 381337.931 5032971.251
 S4 381339.051 5032968.685
 S4 381319.267 5032960.048
 S4 381318.559 5032961.111
 S4 381330.659 5032986.085
 S4 381325.414 503300

Post-Development 3-hour Chicago 1:2 year Event

S5 381316.631 5033020.045
S5 381320.706 5033010.711
S5 381318.612 5033009.797
S5 381319.652 5033007.414
S5 381321.746 5033008.328
S5 381327.851 5032994.342
S5 381334.493 5032959.128
S5 381337.733 5032971.025
S5 381321.435 5032964.05
S5 381310.51 5032989.076
S5 381300.135 5033012.843
S5 381311.368 5033017.747
S5 381316.631 5033000.045
S6 381321.401 5033010.804
S6 381339.894 5033036.045
S6 381340.298 5033037.792
S6 381342.392 5033038.706
S6 381343.432 5033036.323
S6 381341.338 5033035.409
S6 381349.093 5033017.644
S6 381336.616 5033012.046
S6 381335.821 5033012.047
S6 381326.803 5033010.853
S6 381325.27 5033009.866
S6 381321.746 5033008.328
S6 381319.652 5033007.414
S6 381318.612 5033009.797
S6 381316.631 5033010.804
S6 381316.631 5033020.045
S6 381311.368 5033017.747
S6 381304.879 5033024.646
S6 381307.751 5033027.787
S6 381319.536 5033040.417
S6 381325.697 5033035.259
S6 381331.51 5033010.804
S7 381354.292 5033005.735
S7 381349.118 5033003.477
S7 381341.795 5032998.364
S7 381333.208 5032996.68
S7 381327.851 5032994.342
S7 381321.746 5033008.328
S7 381326.27 5033000.045
S7 381326.803 5033010.853
S7 381335.821 5033012.047
S7 381349.093 5033017.644
S7 381354.292 5033005.735
S8 381336.223 5033047.127
S8 381352.72 5033054.328
S8 381341.019 5033004.414
S8 381368.8 5033003.286
S8 381369.264 5033002.193
S8 381359.142 5032997.774
S8 381358.678 5032998.837
S8 381357.523 5032998.333
S8 381355.797 5033002.286
S8 381352.338 5033005.409
S8 381342.332 5033036.433
S8 381342.392 5033038.706
S8 381340.298 5033037.792
S8 381336.223 5033047.127
S9 381360.442 5032986.777
S9 381352.823 5032958.774
S9 381332.406 5032978.297
S9 381342.946 5032978.233
S9 381352.688 5032978.233
S9 381347.856 5032976.123
S9 381344.215 5032984.464
S9 381341.866 5032982.347
S9 381334.493 5032979.128
S9 381332.851 5032996.432
S9 381332.498 5032996.432
S9 381341.795 5032998.364
S9 381349.118 5033003.477
S9 381354.292 5033005.735
S9 381357.523 5032998.333
S9 381355.738 5032997.554
S9 381360.442 5032986.777

Storage Node X-Coord Y-Coord

[SYMBOLS]
~~**;Gage X-Coord Y-Coord**~~

Roughness

Link Summary

Name	From Node	To Node	Type	Length	%Slope
C1	CB111	CB112	CONDUIT	28.9	0.8647
0.0130	CB113	CB112	CONDUIT	31.3	0.6067
0.0130	CB110	CB111	CONDUIT	20.5	0.7815
W1	CB115	CB116	WEIR		
W2	CB116	CB117	WEIR		
W3	CB112	CB113	WEIR		
W4	CB114	CB115	WEIR		
W5	CB110	CB112	WEIR		
C2	CB112	STUB16	OUTLET		
OL1	CB115	St_UnGrd	OUTLET		
OL2	CB116	St_UnGrd	OUTLET		
OL3	CB117	St_UnGrd	OUTLET		
OL4	Roof_1	STUB15	OUTLET		
OL5	Roof_2	OF4	OUTLET		
OL6	CB114	St_UnGrd	OUTLET		
OL7	St_UnGrd	OF7	OUTLET		
OL8	CB118	St_UnGrd	OUTLET		

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.15	0.02	0.04	0.15	1	0.01
C3	CIRCULAR	0.15	0.02	0.04	0.15	1	0.01
C5	CIRCULAR	0.15	0.02	0.04	0.15	1	0.01

Analysis Options

Flow Units CMS

Process Models:

- Rainfall/Runoff YES
- RDI 2 NO
- Snowmelt NO
- Groundwater NO
- Flow Routing LFS
- Box Culvert Allowed NO
- Water Quality NO
- Infiltration Method HORTON
- Flow Routing Method DYNWAVE
- Surcharge Method EXTRAN
- Starting Date 01/01/2000 00:00:00
- Ending Date 01/01/2000 06:00:00
- Interception Days 00:01:00
- Report Time Step 00:01:00
- Wet Time Step 00:01:00
- Dry Time Step 00:01:00
- Routing Time Step 5.00 sec
- Variable Time Step YES
- Maximum Trials 8
- Number of Threads 1
- Head Tolerance 0.001500 m

Runoff Quantity Continuity Volume hectare-m Depth mm

Runoff	Quantity	Continuity	Volume	Depth
Total Precipitation	0.016		31.879	
Evaporation Loss	0.000		31.000	
Infiltration Loss	0.005		9.717	
Surface Runoff	0.011		21.078	
Final Storage	0.001		1.117	
Continuity Error (%)	-0.103			

Post-Development 3-hour Chicago 1:2 year Event

 Flow Routing Continuity Volume Volume
 ***** hectare-m 10^6 ltr

 Dry Weather Inflow 0.000 0.000
 Wet Weather Inflow 0.011 0.107
 Groundwater Inflow 0.000 0.000
 RDW Inflow 0.000 0.000
 External Inflow 0.000 0.02
 External Outflow 0.011 0.109
 Flooding Loss 0.000 0.000
 Evaporation Loss 0.000 0.000
 Exfiltration Loss 0.000 0.000
 Initial Stored Volume 0.000 0.000
 Final Stored Volume 0.000 0.000
 Continuity Error (%) 0.000

 Highest Continuity Errors

 Node CB113 (18.32%)

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 Link OL7 (7)

 Most Frequent Nonconverging Nodes

 Convergence obtained at all time steps.

 Routing Time Step Summary

 Minimum Time Step : 4.50 sec
 Average Time Step : 5.00 sec
 Maximum Time Step : 5.00 sec
 % of Time in Steady State : 0.00
 Average Iterations per Step : 2.00
 % of Steps Not Converging : 0.00
 Time Step Frequency :
 5.000 - 1.155 sec 100.00 %
 3.155 - 1.991 sec 0.00 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

 Total Total Peak Runoff Total Total Total Imperv Perv
 Runoff Runoff Precip Runon Evap Infil Runoff Runoff
 mm 10^6 ltr CMS mm mm mm mm mm mm

 S1 0.00 0.00 0.952 31.88 0.00 0.00 0.00 30.36 0.00
 S10 0.00 0.00 0.038 31.88 0.00 0.00 30.55 2.67 1.20
 S12 0.00 0.00 0.038 31.88 0.00 0.00 28.60 5.71 3.00
 S14 0.00 0.00 0.034 31.88 0.00 0.00 31.75 0.00 0.13
 S2 0.00 0.00 0.004 31.88 0.00 0.00 28.56 5.40 3.07
 S3 0.00 0.00 0.096 31.88 0.00 0.00 30.56 2.58 1.20
 S4 0.00 0.00 0.038 31.88 0.00 0.00 30.35 0.00 0.00
 S5 0.00 0.00 0.052 31.88 0.00 0.00 30.35 0.00 0.00
 S6 0.03 0.02 0.638 31.88 0.00 0.00 10.52 20.27 0.07
 S7 0.02 0.01 0.638 31.88 0.00 0.00 3.01 27.44 0.05
 S8 0.01 0.01 0.862 31.88 0.00 0.00 0.00 30.35 0.00
 S9 0.03 0.02 0.052 31.88 0.00 0.00 0.00 30.35 0.00
 S10 0.02 0.01 0.892 31.88 0.00 0.00 2.01 28.40 0.04

 Node Depth Summary

 Node Type Average Depth Maximum Depth Maximum HGL Time of Max Occurrence Reported Max Depth
 Node Type Meters Meters Meters days hr:min Meters

 OF4 OUTFALL 0.48 0.48 81.11 0 00:00 0.48
 OF7 OUTFALL 0.48 0.48 81.11 0 00:00 0.48
 STUB15 OUTFALL 0.48 0.48 81.11 0 00:00 0.48
 STUB16 OUTFALL 0.63 0.63 81.28 0 00:00 0.63
 CB110 STORAGE 0.00 0.03 83.78 0 01:10 0.03
 CB111 STORAGE 0.00 0.03 83.62 0 01:11 0.03
 CB112 STORAGE 0.00 0.03 83.55 0 01:14 0.03
 CB113 STORAGE 0.00 0.01 83.31 0 01:10 0.01
 CB114 STORAGE 0.00 0.02 85.21 0 01:10 0.02
 CB115 STORAGE 0.00 0.02 85.22 0 01:10 0.02
 CB116 STORAGE 0.00 0.02 85.17 0 01:10 0.02
 CB117 STORAGE 0.00 0.02 85.22 0 01:10 0.00
 CB118 STORAGE 0.00 0.00 85.22 0 01:10 0.00
 Roof_1 STORAGE 0.01 0.03 96.38 0 01:40 0.03
 Roof_2 STORAGE 0.01 0.03 96.38 0 01:40 0.03
 St_UnGrd STORAGE 0.00 0.06 81.86 0 01:10 0.06

 Node Inflow Summary

 Flow Maximum Lateral Maximum Total Lateral Time of Max Inflow Total Inflow
 Balance Inflow Inflow Occurrence Volume Volume
 Error Node Type CMS CMS days hr:min 10^6 ltr 10^6 ltr
 Percent

 OF4 OUTFALL 0.000 0.002 0 00:51 0 0.0293
 0.000 0.029 0 01:02 0 0.0495
 0.000 0.000 0.002 0 00:51 0 0.0289
 0.000 0.000 0.002 0 01:11 0 0.000515
 0.000 0.001 0.001 0 01:10 0 0.000515
 CB111 STORAGE 0.658 0.000 0.001 0 01:10 0 0.000515
 CB112 STORAGE 0.004 0.002 0.002 0 01:10 0 0.000669 0.00119
 CB113 STORAGE 1.070 0.000 0.000 0 01:10 5.84e-06 5.84e-06
 CB114 STORAGE 0.000 0.003 0.003 0 01:10 0.00142 0.00142
 CB115 STORAGE -0.013 0.012 0.012 0 01:10 0.0163 0.0163
 CB116 STORAGE -0.017 0.008 0.008 0 01:10 0.0114 0.0114
 CB117 STORAGE -0.013 0.011 0.011 0 01:10 0.0155 0.0155
 CB118 STORAGE 0.006 0.000 0.000 0 01:10 0.000166 0.000166
 CB119 STORAGE -0.006 0.020 0.020 0 01:10 0.0289 0.0289
 CB120 STORAGE -0.002 0.021 0.021 0 01:10 0.0293 0.0293
 St_UnGrd STORAGE -0.002 0.003 0.037 0 01:10 0.00465 0.0495
 0.005

 Node Surcharge Summary

 No nodes were surcharged.

 Node Flooding Summary

 No nodes were flooded.

 Storage Volume Summary

 Maximum Average Avg Evap Exfil Maximum Max Time of Max
 Outflow Volume Pcnt Pcnt Pcnt Volume Pcnt Occurrence
 Storage Unit 1000 m³ Full Loss Loss 1000 m³ Full days hr:min
 CMS

 CB110 0.000 0.0 0.0 0.0 0.000 0.0 0 01:10
 CB111 0.000 0.1 0.0 0.0 0.000 1.4 0 01:11
 CB112 0.000 0.0 0.0 0.0 0.000 1.1 0 01:11
 CB113 0.000 0.0 0.0 0.0 0.000 0.1 0 01:14
 CB114 0.000 0.0 0.0 0.0 0.000 0.2 0 01:10
 CB115 0.000 0.0 0.0 0.0 0.000 0.7 0 01:10
 CB116 0.000 0.0 0.0 0.0 0.000 0.3 0 01:10
 CB117 0.000 0.1 0.0 0.0 0.000 2.3 0 01:10
 CB118 0.000 0.0 0.0 0.0 0.000 0.0 0 01:10
 Roof_1 0.007 6.9 0.0 0.0 0.017 17.6 0 01:40
 Roof_2 0.007 7.1 0.0 0.0 0.017 17.7 0 01:40
 St_UnGrd 0.000 0.1 0.0 0.0 0.003 5.6 0 01:10
 0.029

 Outfall Loading Summary

 Outfall Node Flow Freq Avg Flow CMS Max Flow CMS Total Volume 10^6 ltr
 Outfall Node Pcnt CMS

 OF4 74.27 0.002 0.002 0.029
 OF7 58.74 0.004 0.029 0.049
 STUB15 73.34 0.002 0.002 0.029
 STUB16 6.60 0.001 0.002 0.001
 System 53.23 0.008 0.035 0.109

 Link Flow Summary

 Link Type Maximum |Flow| CMS Time of Max Occurrence |Veloc| m/sec Maximum |Flow| CMS Max/Full Flow Max/Full Depth
 Link Type

 C1 CONDUIT 0.001 0 01:11 0.47 0.08 0.19
 C3 CONDUIT 0.000 0 01:12 0.07 0.00 0.03
 C5 CONDUIT 0.001 0 01:10 0.61 0.10 0.19
 W1 WEIR 0.000 0 00:00 0.00 0.00 0.00
 W2 WEIR 0.000 0 00:00 0.00 0.00 0.00
 W3 WEIR 0.000 0 00:00 0.00 0.00 0.00
 W4 WEIR 0.000 0 00:00 0.00 0.00 0.00
 W5 WEIR 0.000 0 00:00 0.00 0.00 0.00
 C2 DUMMY 0.000 0 01:11 0.00 0.00 0.00
 OL1 DUMMY 0.012 0 01:10 0.00 0.00 0.00
 OL2 DUMMY 0.008 0 01:10 0.00 0.00 0.00
 OL3 DUMMY 0.011 0 01:10 0.00 0.00 0.00
 OL4 DUMMY 0.002 0 00:51 0.00 0.00 0.00
 OL5 DUMMY 0.002 0 00:51 0.00 0.00 0.00
 OL6 DUMMY 0.003 0 01:10 0.00 0.00 0.00
 OL7 DUMMY 0.029 0 01:02 0.00 0.00 0.00
 OL8 DUMMY 0.000 0 01:10 0.00 0.00 0.00

 Flow Classification Summary

 Adjusted /Actual Length Up Dry Down Dry Sub Crit Sup Crit Up Crit Down Crit Norm Crit Inlet Crit
 Conduit Dry Dry

 C1 1.00 0.19 0.00 0.00 0.00 0.00 0.81 0.00 0.00
 C3 1.00 0.19 0.00 0.01 0.00 0.00 0.80 0.00 0.00
 C5 1.00 0.19 0.00 0.00 0.80 0.01 0.00 0.00 0.80 0.00

 Conduit Surcharge Summary

 No conduits were surcharged.
 Analysis begun on: Thu Oct 10 10:09:49 2024
 Analysis ended on: Thu Oct 10 10:09:49 2024
 Total elapsed time: < 1 sec

Post-Development 3-hour Chicago 1:100 year Event

Post-Development 3-hour Chicago 1:100-year Event

```
[TITLE]
;;Project Title/Notes

[OPTIONS]
;;Option      Value
FLOW_UNITS    CMS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE    01/01/2000
START_TIME    00:00:00
REPORT_START_DATE 01/01/2000
REPORT_START_TIME 00:00:00
END_DATE     01/01/2000
END_TIME     06:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      -
REPORT_STEP   00:01:00
WET_STEP      00:01:00
DRY_STEP      00:01:00
ROUTING_STEP  5
RULE_STEP     00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQNATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA   0
MAX_TRIALS     8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL   5
LAT_FLOW_TOL   5
MINIMUM_STEP   0.5
THREADS       12

[EVAPORATION]
;;Data Source Parameters
;;-
CONSTANT      0.0
DRY_ONLY      NO

[RAINGAGES]
;;Name Format Interval SCF Source
;;-
Rainfall     INTENSITY 0:10 1.0 TIMESERIES 3CHI100

[SUBCATCHMENTS]
;;Name Rain Gage Outlet Area %Imperv Width %Slope
CurLen SnowPack
;;-
S1 Rainfall St_UnGrd 0.0991 100 6.1 15 0
S10 Rainfall CB118 0.0138 8.79 28.998 1 0
S11 Rainfall CB114 0.0473 18.805 70.001 2 0
S2 Rainfall CB113 0.0044 0.002 4.3 1.9 0
S3 Rainfall CB112 0.0218 17.802 75 1 0
S4 Rainfall CB110 0.043 8.51 52.001 3.5 0
S5 Rainfall Roof_1 0.0852 99.998 104.996 1 0
S6 Rainfall CB115 0.0033 60.775 45 2.5 0
S7 Rainfall CB116 0.0416 90.416 28.601 7 0
S8 Rainfall Roof_2 0.0965 100 105.005 1 0
S9 Rainfall CB117 0.0544 93.568 50 2.4 0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo
PctRouted
;;-
S1 0.013 0.25 1.57 4.67 0 OUTLET
S10 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S11 0.013 0.25 1.57 4.67 0 OUTLET
S3 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S4 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S5 0.013 0.25 1.57 4.67 0 OUTLET
S6 0.013 0.25 1.57 4.67 0 OUTLET
S7 0.013 0.25 1.57 4.67 0 OUTLET
S8 0.013 0.25 1.57 4.67 0 OUTLET
S9 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]
;;Subcatchment Param1 Param2 Param3 Param4 Param5
;;-
S1 76.2 13.2 4.14 7 0
S10 76.2 13.2 4.14 7 0
S11 76.2 13.2 4.14 7 0
S2 76.2 13.2 4.14 7 0
S3 76.2 13.2 4.14 7 0
S4 76.2 13.2 4.14 7 0
S5 76.2 13.2 4.14 7 0
S6 76.2 13.2 4.14 7 0
S7 76.2 13.2 4.14 7 0
S8 76.2 13.2 4.14 7 0
S9 76.2 13.2 4.14 7 0

[OUTFALLS]
;;Name Elevation Type Stage Data Gated Route To
;;-
OF4 80.628 FIXED 81.11 NO
OF7 80.628 FIXED 81.11 NO
STUB15 80.628 FIXED 81.11 NO
STUB16 80.646 FIXED 81.28 NO

[STORAGE]
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params
SurDepth Fvap Psi Ksat IMD
;;-
CB110 83.75 1.9 0 TABULAR CB110 0
CB111 83.59 2.01 0 TABULAR CB111 0
CB112 83.28 2.12 0 TABULAR CB112 0
CB113 83.55 1.7 0 TABULAR CB113 0
CB114 85.3 0.25 0 TABULAR CB114 0
CB115 85.25 0.31 0 TABULAR CB115 0
CB116 85.2 0.4 0 TABULAR CB116 0
CB117 85.15 0.15 0 TABULAR CB117 0
CB118 85.22 0.18 0 TABULAR CB118 0
Roof_1 96.35 0.15 0 FUNCTIONAL 0 0 635 0
Roof_2 96.35 0.15 0 FUNCTIONAL 0 0 643 0
St_UnGrd 81.8 1 0 FUNCTIONAL 0 0 49 0

[CONDUITS]
;;Name From Node To Node Length Roughness InOffset
OutOffset InitFlow MaxFlow
;;-

```

C1 CB111 CB112 28.913 0.013 83.59 83.34
C3 CB113 CB112 31.317 0.013 83.55 83.36
C5 CB110 CB111 20.475 0.013 83.75 83.59

[WEIRS]
;;Name From Node To Node Type CrestHt Qcoeff Gated
EndCon EndCoeff Surcharge RoadWidth RoadSurf Coeff. Curve
;;-
W1 CB115 CB116 TRANSVERSE 85.45 1.84 NO
W2 CB116 CB117 TRANSVERSE 85.35 1.84 NO
W3 CB112 CB113 TRANSVERSE 85.38 1.84 NO
W4 CB114 CB115 TRANSVERSE 85.5 1.84 NO
W5 CB110 CB112 TRANSVERSE 85.6 1.84 NO

[OUTLETS]
;;Name From Node To Node Offset Type
QTable/Qcoeff Qexpon Gated
;;-
C2 Vortex_ICD_95 CB112 NO STUB16 83.28 TABULAR/HEAD
OL1 CB115 St_UnGrd 85.25 TABULAR/HEAD ZURN_Z150F-
OL2 CB116 St_UnGrd 85.2 TABULAR/HEAD ZURN_Z150F-
OL3 CB117 St_UnGrd 85.15 TABULAR/HEAD ZURN_Z150F-
OL4 Roof_1 STUB15 96.35 TABULAR/HEAD O_Roof1
OL5 Roof_2 OF4 96.35 TABULAR/HEAD O_Roof1
OL6 CB114 St_UnGrd 85.3 TABULAR/HEAD ZURN_Z150F-
OL7 St_UnGrd OF7 81.8 TABULAR/HEAD O_St_UnGrd
OL8 CB118 St_UnGrd 85.22 TABULAR/HEAD ZURN_Z150F-

[XSECTIONS]
;;Link Culvert Shape Geom1 Geom2 Geom3 Geom4 Barrels
;;-
C1 CIRCULAR 0.15 0 0 0 1
C3 CIRCULAR 0.15 0 0 0 1
C5 CIRCULAR 0.15 0 0 0 1
W1 RECT_OPEN 0.11 0 0 0 0
W2 RECT_OPEN 0.05 0 0 0 0
W3 RECT_OPEN 0.02 0 0 0 0
W4 RECT_OPEN 0.05 7 0 0 0
W5 RECT_OPEN 0.05 3.487 0 0 0

[LOSSES]
;;Link Kentry Kexit Kavg Flap Gate Seepage
;;-
;[INFLOWS]
;;Node Constituent Time Series Type Mfactor Sfactor Baseline
Pattern
;;-
St_UnGrd FLOW InfiltratedInflow FLOW 1.0 1 0

[CURVES]
;;Name Type X-Value Y-Value
;;-
;Tempest Rating Curve for MHF IPEX TYPE A, No grate allowance
MHF_IPEX_TYPE_A Rating 0 0
MHF_IPEX_TYPE_A 0.1 0.0057
MHF_IPEX_TYPE_A 0.2 0.0081
MHF_IPEX_TYPE_A 0.3 0.0099
MHF_IPEX_TYPE_A 0.4 0.0114
MHF_IPEX_TYPE_A 0.5 0.0128
MHF_IPEX_TYPE_A 0.6 0.014
MHF_IPEX_TYPE_A 0.7 0.0151
MHF_IPEX_TYPE_A 0.8 0.0162
MHF_IPEX_TYPE_A 0.9 0.0172
MHF_IPEX_TYPE_A 1 0.0181
MHF_IPEX_TYPE_A 1.2 0.0198
MHF_IPEX_TYPE_A 1.4 0.0214
MHF_IPEX_TYPE_A 1.6 0.0229
MHF_IPEX_TYPE_A 1.8 0.0243
MHF_IPEX_TYPE_A 2 0.0256
MHF_IPEX_TYPE_A 2.5 0.0286
MHF_IPEX_TYPE_A 3 0.0313

;Tempest Rating Curve for MHF IPEX TYPE B, No grate allowance
MHF_IPEX_TYPE_B Rating 0 0
MHF_IPEX_TYPE_B 0.1 0.0081
MHF_IPEX_TYPE_B 0.2 0.0115
MHF_IPEX_TYPE_B 0.3 0.0141
MHF_IPEX_TYPE_B 0.4 0.0162
MHF_IPEX_TYPE_B 0.5 0.0182
MHF_IPEX_TYPE_B 0.6 0.0199
MHF_IPEX_TYPE_B 0.7 0.0215
MHF_IPEX_TYPE_B 0.8 0.023
MHF_IPEX_TYPE_B 0.9 0.0244
MHF_IPEX_TYPE_B 1 0.0257
MHF_IPEX_TYPE_B 1.2 0.0281
MHF_IPEX_TYPE_B 1.4 0.0304
MHF_IPEX_TYPE_B 1.6 0.0325
MHF_IPEX_TYPE_B 1.8 0.0344
MHF_IPEX_TYPE_B 2 0.0363
MHF_IPEX_TYPE_B 2.5 0.0406
MHF_IPEX_TYPE_B 3 0.0445

;Tempest Rating Curve for MHF IPEX TYPE C, No grate allowance
MHF_IPEX_TYPE_C Rating 0 0
MHF_IPEX_TYPE_C 0.1 0.0106
MHF_IPEX_TYPE_C 0.2 0.015
MHF_IPEX_TYPE_C 0.3 0.0183
MHF_IPEX_TYPE_C 0.4 0.0212
MHF_IPEX_TYPE_C 0.5 0.0237
MHF_IPEX_TYPE_C 0.6 0.0259
MHF_IPEX_TYPE_C 0.7 0.028
MHF_IPEX_TYPE_C 0.8 0.0299
MHF_IPEX_TYPE_C 0.9 0.0317
MHF_IPEX_TYPE_C 1 0.0335
MHF_IPEX_TYPE_C 1.2 0.0356
MHF_IPEX_TYPE_C 1.4 0.0396
MHF_IPEX_TYPE_C 1.6 0.0423
MHF_IPEX_TYPE_C 1.8 0.0449
MHF_IPEX_TYPE_C 2 0.0473
MHF_IPEX_TYPE_C 2.5 0.0529
MHF_IPEX_TYPE_C 3 0.0579

;Tempest Rating Curve for MHF IPEX TYPE D, No grate allowance
MHF_IPEX_TYPE_D Rating 0 0
MHF_IPEX_TYPE_D 0.1 0.0154
MHF_IPEX_TYPE_D 0.2 0.0217
MHF_IPEX_TYPE_D 0.3 0.0266
MHF_IPEX_TYPE_D 0.4 0.0307
MHF_IPEX_TYPE_D 0.5 0.0343
MHF_IPEX_TYPE_D 0.6 0.0376
MHF_IPEX_TYPE_D 0.7 0.0406
MHF_IPEX_TYPE_D 0.8 0.0434
MHF_IPEX_TYPE_D 0.9 0.0461
MHF_IPEX_TYPE_D 1 0.0485
MHF_IPEX_TYPE_D 1.2 0.0532
MHF_IPEX_TYPE_D 1.4 0.0574

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MHF_IPEX_TYPE_D    1.6    0.0614
MHF_IPEX_TYPE_D    1.8    0.0651
MHF_IPEX_TYPE_D    2     0.0687
MHF_IPEX_TYPE_D    2.5    0.0768
MHF_IPEX_TYPE_D    3     0.0841

;Tempest Rating Curve for MHF IPEX TYPE E, No grate allowance
MHF_IPEX_TYPE_E    Rating 0
MHF_IPEX_TYPE_E    0.1    0.0205
MHF_IPEX_TYPE_E    0.2    0.0289
MHF_IPEX_TYPE_E    0.3    0.0355
MHF_IPEX_TYPE_E    0.4    0.0409
MHF_IPEX_TYPE_E    0.5    0.0458
MHF_IPEX_TYPE_E    0.6    0.0501
MHF_IPEX_TYPE_E    0.7    0.0542
MHF_IPEX_TYPE_E    0.8    0.0579
MHF_IPEX_TYPE_E    0.9    0.0614
MHF_IPEX_TYPE_E    1     0.0647
MHF_IPEX_TYPE_E    1.2    0.0709
MHF_IPEX_TYPE_E    1.4    0.0766
MHF_IPEX_TYPE_E    1.6    0.0819
MHF_IPEX_TYPE_E    1.8    0.0868
MHF_IPEX_TYPE_E    2     0.0915
MHF_IPEX_TYPE_E    2.5    0.1023
MHF_IPEX_TYPE_E    3     0.1121

O_Roof1   Rating 0
O_Roof1   0.001  0.002
O_Roof1   0.15   0.002

O_Roof2   Rating 0
O_Roof2   0.001  0.0025
O_Roof2   0.15   0.0025

O_St_UnGrd Rating 0
O_St_UnGrd 0.01  0.029
O_St_UnGrd 1     0.029

;Tempest Rating Curve for Vortex ICD 100, No grate allowance
Vortex_ICD_100   Rating 0
Vortex_ICD_100   0.1    0.0028
Vortex_ICD_100   0.2    0.004
Vortex_ICD_100   0.3    0.0059
Vortex_ICD_100   0.4    0.0066
Vortex_ICD_100   0.5    0.0063
Vortex_ICD_100   0.6    0.0069
Vortex_ICD_100   0.7    0.0075
Vortex_ICD_100   0.8    0.008
Vortex_ICD_100   0.9    0.0085
Vortex_ICD_100   1     0.0099
Vortex_ICD_100   1.2    0.0098
Vortex_ICD_100   1.4    0.0106
Vortex_ICD_100   1.6    0.0113
Vortex_ICD_100   1.8    0.012
Vortex_ICD_100   2     0.0126
Vortex_ICD_100   2.5    0.0141
Vortex_ICD_100   3     0.0155

;Tempest Rating Curve for Vortex ICD 105, No grate allowance
Vortex_ICD_105   Rating 0
Vortex_ICD_105   0.1    0.0031
Vortex_ICD_105   0.2    0.0044
Vortex_ICD_105   0.3    0.0054
Vortex_ICD_105   0.4    0.0062
Vortex_ICD_105   0.5    0.0069
Vortex_ICD_105   0.6    0.0076
Vortex_ICD_105   0.7    0.0082
Vortex_ICD_105   0.8    0.0088
Vortex_ICD_105   0.9    0.0093
Vortex_ICD_105   1     0.0098
Vortex_ICD_105   1.2    0.0107
Vortex_ICD_105   1.4    0.0115
Vortex_ICD_105   1.6    0.0124
Vortex_ICD_105   1.8    0.0131
Vortex_ICD_105   2     0.0139
Vortex_ICD_105   2.5    0.0155
Vortex_ICD_105   3     0.017

;Tempest Rating Curve for Vortex ICD 40, No grate allowance
Vortex_ICD_40    Rating 0
Vortex_ICD_40    0.1    0.0004
Vortex_ICD_40    0.2    0.0006
Vortex_ICD_40    0.3    0.0007
Vortex_ICD_40    0.4    0.0009
Vortex_ICD_40    0.5    0.001
Vortex_ICD_40    0.6    0.001
Vortex_ICD_40    0.7    0.0011
Vortex_ICD_40    0.8    0.0012
Vortex_ICD_40    0.9    0.0013
Vortex_ICD_40    1     0.0014
Vortex_ICD_40    1.2    0.0015
Vortex_ICD_40    1.4    0.0016
Vortex_ICD_40    1.6    0.0017
Vortex_ICD_40    1.8    0.0018
Vortex_ICD_40    2     0.0019
Vortex_ICD_40    2.5    0.0022
Vortex_ICD_40    3     0.0024

;Tempest Rating Curve for Vortex ICD 45, No grate allowance
Vortex_ICD_45    Rating 0
Vortex_ICD_45    0.1    0.0006
Vortex_ICD_45    0.2    0.0008
Vortex_ICD_45    0.3    0.001
Vortex_ICD_45    0.4    0.0011
Vortex_ICD_45    0.5    0.0013
Vortex_ICD_45    0.6    0.0014
Vortex_ICD_45    0.7    0.0015
Vortex_ICD_45    0.8    0.0016
Vortex_ICD_45    0.9    0.0017
Vortex_ICD_45    1     0.0018
Vortex_ICD_45    1.2    0.002
Vortex_ICD_45    1.4    0.0021
Vortex_ICD_45    1.6    0.0023
Vortex_ICD_45    1.8    0.0024
Vortex_ICD_45    2     0.0026
Vortex_ICD_45    2.5    0.0029
Vortex_ICD_45    3     0.0031

;Tempest Rating Curve for Vortex ICD 50, No grate allowance
Vortex_ICD_50    Rating 0
Vortex_ICD_50    0.1    0.0007
Vortex_ICD_50    0.2    0.001
Vortex_ICD_50    0.3    0.0012
Vortex_ICD_50    0.4    0.0014
Vortex_ICD_50    0.5    0.0016
Vortex_ICD_50    0.6    0.0018
Vortex_ICD_50    0.7    0.0019
Vortex_ICD_50    0.8    0.002
Vortex_ICD_50    0.9    0.0021
Vortex_ICD_50    1     0.0023
Vortex_ICD_50    1.2    0.0025
Vortex_ICD_50    1.4    0.0027
Vortex_ICD_50    1.6    0.0029
Vortex_ICD_50    1.8    0.003
Vortex_ICD_50    2     0.0032
Vortex_ICD_50    2.5    0.0036
Vortex_ICD_50    3     0.0039

;Tempest Rating Curve for Vortex ICD 55, No grate allowance
Vortex_ICD_55    Rating 0
Vortex_ICD_55    0.1    0.0009
Vortex_ICD_55    0.2    0.0012
Vortex_ICD_55    0.3    0.0015
Vortex_ICD_55    0.4    0.0017
Vortex_ICD_55    0.5    0.0019
Vortex_ICD_55    0.6    0.0021
Vortex_ICD_55    0.7    0.0023
Vortex_ICD_55    0.8    0.0024
Vortex_ICD_55    0.9    0.0026

Vortex_ICD_55    Rating 0
Vortex_ICD_55    1     0.0027
Vortex_ICD_55    1.2    0.003
Vortex_ICD_55    1.4    0.0032
Vortex_ICD_55    1.6    0.0034
Vortex_ICD_55    1.8    0.0036
Vortex_ICD_55    2     0.0038
Vortex_ICD_55    2.5    0.0043
Vortex_ICD_55    3     0.0047

;Tempest Rating Curve for Vortex ICD 60, No grate allowance
Vortex_ICD_60    Rating 0
Vortex_ICD_60    0.1    0.0011
Vortex_ICD_60    0.2    0.0015
Vortex_ICD_60    0.3    0.0018
Vortex_ICD_60    0.4    0.002
Vortex_ICD_60    0.5    0.0023
Vortex_ICD_60    0.6    0.0025
Vortex_ICD_60    0.7    0.0027
Vortex_ICD_60    0.8    0.0029
Vortex_ICD_60    0.9    0.0031
Vortex_ICD_60    1     0.0032
Vortex_ICD_60    1.2    0.0035
Vortex_ICD_60    1.4    0.0036
Vortex_ICD_60    1.6    0.0038
Vortex_ICD_60    1.8    0.0041
Vortex_ICD_60    2     0.0043
Vortex_ICD_60    2.5    0.0046
Vortex_ICD_60    3     0.0056

;Tempest Rating Curve for Vortex ICD 65, No grate allowance
Vortex_ICD_65    Rating 0
Vortex_ICD_65    0.1    0.0012
Vortex_ICD_65    0.2    0.0016
Vortex_ICD_65    0.3    0.002
Vortex_ICD_65    0.4    0.0023
Vortex_ICD_65    0.5    0.0025
Vortex_ICD_65    0.6    0.0028
Vortex_ICD_65    0.7    0.003
Vortex_ICD_65    0.8    0.0032
Vortex_ICD_65    0.9    0.0034
Vortex_ICD_65    1     0.0036
Vortex_ICD_65    1.2    0.004
Vortex_ICD_65    1.4    0.0043
Vortex_ICD_65    1.6    0.0046
Vortex_ICD_65    1.8    0.0049
Vortex_ICD_65    2     0.0051
Vortex_ICD_65    2.5    0.0057
Vortex_ICD_65    3     0.0063

;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex_ICD_70    Rating 0
Vortex_ICD_70    0.1    0.0013
Vortex_ICD_70    0.2    0.0019
Vortex_ICD_70    0.3    0.0023
Vortex_ICD_70    0.4    0.0027
Vortex_ICD_70    0.5    0.003
Vortex_ICD_70    0.6    0.0033
Vortex_ICD_70    0.7    0.0036
Vortex_ICD_70    0.8    0.0038
Vortex_ICD_70    0.9    0.0041
Vortex_ICD_70    1     0.0043
Vortex_ICD_70    1.2    0.0047
Vortex_ICD_70    1.4    0.0051
Vortex_ICD_70    1.6    0.0055
Vortex_ICD_70    1.8    0.0058
Vortex_ICD_70    2     0.0061
Vortex_ICD_70    2.5    0.0068
Vortex_ICD_70    3     0.0075

;Tempest Rating Curve for Vortex ICD 75, No grate allowance
Vortex_ICD_75    Rating 0
Vortex_ICD_75    0.1    0.0016
Vortex_ICD_75    0.2    0.0022
Vortex_ICD_75    0.3    0.0027
Vortex_ICD_75    0.4    0.0032
Vortex_ICD_75    0.5    0.0035
Vortex_ICD_75    0.6    0.0039
Vortex_ICD_75    0.7    0.0042
Vortex_ICD_75    0.8    0.0045
Vortex_ICD_75    0.9    0.0048
Vortex_ICD_75    1     0.005
Vortex_ICD_75    1.2    0.0055
Vortex_ICD_75    1.4    0.0059
Vortex_ICD_75    1.6    0.0063
Vortex_ICD_75    1.8    0.0067
Vortex_ICD_75    2     0.0071
Vortex_ICD_75    2.5    0.0079
Vortex_ICD_75    3     0.0087

;Tempest Rating Curve for Vortex ICD 80, No grate allowance
Vortex_ICD_80    Rating 0
Vortex_ICD_80    0.1    0.0018
Vortex_ICD_80    0.2    0.0026
Vortex_ICD_80    0.3    0.0031
Vortex_ICD_80    0.4    0.0036
Vortex_ICD_80    0.5    0.004
Vortex_ICD_80    0.6    0.0044
Vortex_ICD_80    0.7    0.0048
Vortex_ICD_80    0.8    0.0051
Vortex_ICD_80    0.9    0.0054
Vortex_ICD_80    1     0.0057
Vortex_ICD_80    1.2    0.0063
Vortex_ICD_80    1.4    0.0068
Vortex_ICD_80    1.6    0.0072
Vortex_ICD_80    1.8    0.0077
Vortex_ICD_80    2     0.0081
Vortex_ICD_80    2.5    0.0089
Vortex_ICD_80    3     0.0099

;Tempest Rating Curve for Vortex ICD 85, No grate allowance
Vortex_ICD_85    Rating 0
Vortex_ICD_85    0.1    0.002
Vortex_ICD_85    0.2    0.0029
Vortex_ICD_85    0.3    0.0035
Vortex_ICD_85    0.4    0.0041
Vortex_ICD_85    0.5    0.0045
Vortex_ICD_85    0.6    0.005
Vortex_ICD_85    0.7    0.0054
Vortex_ICD_85    0.8    0.0057
Vortex_ICD_85    0.9    0.0061
Vortex_ICD_85    1     0.0064
Vortex_ICD_85    1.2    0.007
Vortex_ICD_85    1.4    0.0076
Vortex_ICD_85    1.6    0.0081
Vortex_ICD_85    1.8    0.0086
Vortex_ICD_85    2     0.0091
Vortex_ICD_85    2.5    0.0101
Vortex_ICD_85    3     0.0111

;Tempest Rating Curve for Vortex ICD 90, No grate allowance
Vortex_ICD_90    Rating 0
Vortex_ICD_90    0.1    0.0022
Vortex_ICD_90    0.2    0.0032
Vortex_ICD_90    0.3    0.0039
Vortex_ICD_90    0.4    0.0045
Vortex_ICD_90    0.5    0.0051
Vortex_ICD_90    0.6    0.0055
Vortex_ICD_90    0.7    0.006
Vortex_ICD_90    0.8    0.0064
Vortex_ICD_90    0.9    0.0068
Vortex_ICD_90    1     0.0072
Vortex_ICD_90    1.2    0.0079
Vortex_ICD_90    1.4    0.0085
Vortex_ICD_90    1.6    0.0091
Vortex_ICD_90    1.8    0.0096
Vortex_ICD_90    2     0.0102
Vortex_ICD_90    2.5    0.0114
Vortex_ICD_90    3     0.0125

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;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex ICD_95 Rating 0 0
Vortex ICD_95 0.1 0.0026
Vortex ICD_95 0.2 0.0036
Vortex ICD_95 0.3 0.0044
Vortex ICD_95 0.4 0.0051
Vortex ICD_95 0.5 0.0057
Vortex ICD_95 0.6 0.0062
Vortex ICD_95 0.7 0.0067
Vortex ICD_95 0.8 0.0071
Vortex ICD_95 0.9 0.0076
Vortex ICD_95 1 0.008
Vortex ICD_95 1.2 0.0087
Vortex ICD_95 1.4 0.0094
Vortex ICD_95 1.6 0.0101
Vortex ICD_95 1.8 0.0107
Vortex ICD_95 2 0.0113
Vortex ICD_95 2.5 0.0126
Vortex ICD_95 3 0.0138

;From Zurn Manual RD178
ZURN_Z150F-6NH Rating 0 0
ZURN_Z150F-6NH 0.0127 0.00503838308477861
ZURN_Z150F-6NH 0.0257 0.0132420013231177
ZURN_Z150F-6NH 0.0384 0.0234992054543888
ZURN_Z150F-6NH 0.0508 0.0357128365761305
ZURN_Z150F-6NH 0.0765 0.042501972611045
ZURN_Z150F-6NH 0.1024 0.0433196215564931

CB110	Storage	0	0.073
CB110		1.55	0.073
CB110		1.85	120.85
CB110		1.9	120.85
CB111	Storage	0	0.073
CB111		2.01	0.073
CB112	Storage	0	0.36
CB112		1.92	0.36
CB112		2.1	19.13
CB112		2.12	19.13
CB113	Storage	0	0.073
CB113		1.6	0.073
CB113		1.7	3.62
CB114	Storage	0	0.36
CB114		0.2	19.2
CB114		0.25	19.2
CB115	Storage	0	0.36
CB115		0.2	155.53
CB115		0.31	155.53
CB116	Storage	0	0.36
CB116		0.15	73.19
CB116		0.4	73.19
CB117	Storage	0	0.36
CB117		0.15	122.26
CB118	Storage	0	0.36
CB118		0.18	37.7

[TIMESERIES]
;Name Date Time Value
;-----
;Rainfall (mm/hr)
3CH1100 01/01/2000 00:00:00 5.339
3CH1100 01/01/2000 00:10:00 6.376
3CH1100 01/01/2000 00:20:00 7.977
3CH1100 01/01/2000 00:30:00 10.797
3CH1100 01/01/2000 00:40:00 17.136
3CH1100 01/01/2000 00:50:00 45.128
3CH1100 01/01/2000 01:00:00 178.107
3CH1100 01/01/2000 01:10:00 51.056
3CH1100 01/01/2000 01:20:00 26.163
3CH1100 01/01/2000 01:30:00 17.571
3CH1100 01/01/2000 01:40:00 13.77
3CH1100 01/01/2000 01:50:00 10.712
3CH1100 01/01/2000 02:00:00 9.008
3CH1100 01/01/2000 02:10:00 7.793
3CH1100 01/01/2000 02:20:00 6.883
3CH1100 01/01/2000 02:30:00 6.174
3CH1100 01/01/2000 02:40:00 5.607
3CH1100 01/01/2000 02:50:00 5.142
3CH1100 01/01/2000 03:00:00 0

;Sum of Infiltrated flow from above the garage slab calculated from Infiltration graphs for S11 multiplied by the total pervious area above the garage.
InfiltratedInflow 01/01/2000 00:01:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:02:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:03:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:04:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:05:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:06:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:07:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:08:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:09:00 7.95386E-05
InfiltratedInflow 01/01/2000 00:10:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:11:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:12:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:13:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:14:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:15:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:16:00 9.498747E-05
InfiltratedInflow 01/01/2000 00:17:00 0.0001139877
InfiltratedInflow 01/01/2000 00:18:00 0.0001139877
InfiltratedInflow 01/01/2000 00:19:00 0.0001164632
InfiltratedInflow 01/01/2000 00:20:00 0.0001407495
InfiltratedInflow 01/01/2000 00:21:00 0.0001455192
InfiltratedInflow 01/01/2000 00:22:00 0.0001462368
InfiltratedInflow 01/01/2000 00:23:00 0.0001463445
InfiltratedInflow 01/01/2000 00:24:00 0.0001463593
InfiltratedInflow 01/01/2000 00:25:00 0.0001463616
InfiltratedInflow 01/01/2000 00:26:00 0.0001463619
InfiltratedInflow 01/01/2000 00:27:00 0.000146362
InfiltratedInflow 01/01/2000 00:28:00 0.000146362
InfiltratedInflow 01/01/2000 00:29:00 0.000146362
InfiltratedInflow 01/01/2000 00:30:00 0.000146362
InfiltratedInflow 01/01/2000 00:31:00 0.000146362
InfiltratedInflow 01/01/2000 00:32:00 0.000146362
InfiltratedInflow 01/01/2000 00:33:00 0.000146362
InfiltratedInflow 01/01/2000 00:34:00 0.000146362
InfiltratedInflow 01/01/2000 00:35:00 0.000146362
InfiltratedInflow 01/01/2000 00:36:00 0.000146362
InfiltratedInflow 01/01/2000 00:37:00 0.000146362
InfiltratedInflow 01/01/2000 00:38:00 0.000146362
InfiltratedInflow 01/01/2000 00:39:00 0.000146362
InfiltratedInflow 01/01/2000 00:40:00 0.0002925396
InfiltratedInflow 01/01/2000 00:41:00 0.0003126802
InfiltratedInflow 01/01/2000 00:42:00 0.0003142753
InfiltratedInflow 01/01/2000 00:43:00 0.0003143887
InfiltratedInflow 01/01/2000 00:44:00 0.0003144075
InfiltratedInflow 01/01/2000 00:45:00 0.0003144117
InfiltratedInflow 01/01/2000 00:46:00 0.0003144112
InfiltratedInflow 01/01/2000 00:47:00 0.0003144113
InfiltratedInflow 01/01/2000 00:48:00 0.0003144113
InfiltratedInflow 01/01/2000 00:49:00 0.0003144113
InfiltratedInflow 01/01/2000 00:50:00 0.000670688
InfiltratedInflow 01/01/2000 00:51:00 0.0008389822
InfiltratedInflow 01/01/2000 00:52:00 0.00095337
InfiltratedInflow 01/01/2000 00:53:00 0.000582052
InfiltratedInflow 01/01/2000 00:54:00 0.0005563559
InfiltratedInflow 01/01/2000 00:55:00 0.000532373
InfiltratedInflow 01/01/2000 00:56:00 0.0005099892
InfiltratedInflow 01/01/2000 00:57:00 0.0004890978
InfiltratedInflow 01/01/2000 00:58:00 0.0004695993
InfiltratedInflow 01/01/2000 00:59:00 0.0004514008

InfiltratedInflow	Date	Time	Value
InfiltratedInflow	01/01/2000	01:00:00	0.0004344156
InfiltratedInflow	01/01/2000	01:01:00	0.0004185629
InfiltratedInflow	01/01/2000	01:02:00	0.0004037672
InfiltratedInflow	01/01/2000	01:03:00	0.0003895979
InfiltratedInflow	01/01/2000	01:04:00	0.0003770694
InfiltratedInflow	01/01/2000	01:05:00	0.0003650402
InfiltratedInflow	01/01/2000	01:06:00	0.0003538181
InfiltratedInflow	01/01/2000	01:07:00	0.000342544
InfiltratedInflow	01/01/2000	01:08:00	0.0003335544
InfiltratedInflow	01/01/2000	01:09:00	0.0003244265
InfiltratedInflow	01/01/2000	01:10:00	0.0003159071
InfiltratedInflow	01/01/2000	01:11:00	0.0003079558
InfiltratedInflow	01/01/2000	01:12:00	0.0003055346
InfiltratedInflow	01/01/2000	01:13:00	0.0002956082
InfiltratedInflow	01/01/2000	01:14:00	0.0002871436
InfiltratedInflow	01/01/2000	01:15:00	0.000281101
InfiltratedInflow	01/01/2000	01:16:00	0.0002754788
InfiltratedInflow	01/01/2000	01:17:00	0.0002702229
InfiltratedInflow	01/01/2000	01:18:00	0.0002653175
InfiltratedInflow	01/01/2000	01:19:00	0.0002607392
InfiltratedInflow	01/01/2000	01:20:00	0.0002564661
InfiltratedInflow	01/01/2000	01:21:00	0.0002521779
InfiltratedInflow	01/01/2000	01:22:00	0.0002487556
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InfiltratedInflow	01/01/2000	01:28:00	0.0002281515
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InfiltratedInflow	01/01/2000	01:45:00	0.0002073069
InfiltratedInflow	01/01/2000	01:46:00	0.0002065963
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InfiltratedInflow	01/01/2000	02:04:00	0.0001995219
InfiltratedInflow	01/01/2000	02:05:00	0.0001993304
InfiltratedInflow	01/01/2000	02:06:00	0.0001991516
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InfiltratedInflow	01/01/2000	02:08:00	0.000198829
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InfiltratedInflow	01/01/2000	02:46:00	0.0001968075
InfiltratedInflow	01/01/2000	02:47:00	0.0001967969
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InfiltratedInflow	01/01/2000	02:50:00	0.0001967693
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InfiltratedInflow	01/01/2000	02:52:00	

Post-Development 3-hour Chicago 1:100 year Event

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[REPORT]
 ;;Reporting Options
 INPUTS YES
 CONTROLS NO
 SUBCATCHMENTS ALL
 NODES ALL
 LINKS ALL

[TAGS]
 Subcatch S1 Ramp
 Subcatch S5 Building
 Subcatch S6 ParkingLot
 Subcatch S7 Building
 Subcatch S8 Building
 Subcatch S9 ParkingLot
 Node St_UnGrd Underground_Storage

[MAP]
 DIMENSIONS 381280.91645 5032948.93205 381379.86255 5033068.41295
 UNITS Meters

[COORDINATES]
 ;;Node X-Coord Y-Coord

;-----
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 OF7 381346.502 5032963.916
 STUB15 381335.776 5032954.363
 STUB16 381319.979 5032958.277
 CB110 381293.102 5033002.513
 CB111 381307.131 5032987.6
 CB112 381319.643 5032961.536
 CB113 381328.444 5032962.444
 CB114 381332.566 5033036.058
 CB115 381332.827 5033018.906
 CB116 381338.815 5033005.189
 CB117 381344.774 5032991.539
 CB118 381359.473 5032992.996
 Root_1 381325.164 5032911.273
 Roof_2 381361.355 5033113.684
 St_UnGrd 381329.806 5032972.84

[VERTICES]
 ;;Link X-Coord Y-Coord

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 W3 381322.829 5032965.277
 W3 381325.66 5032974.14
 W5 381295.919 5033002.386
 W5 381309.233 5032988.935
 W5 381320.071 5032965.768
 OL6 381323.041 5033005.693

[POLYGONS]
 ;;Subcatchment X-Coord Y-Coord

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 S1 381347.856 5032976.123
 S1 381337.733 5032971.705
 S1 381334.493 5032979.128
 S1 381341.864 5032982.351
 S1 381342.215 5032986.444
 S10 381364.422 5032996.577
 S10 381355.738 5032997.554
 S10 381357.523 5032998.333
 S10 381358.678 5032998.837
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 S10 381369.264 5033002.193
 S10 381368.8 5033005.256
 S10 381369.119 5033005.254
 S10 381375.365 5033002.452
 S10 381360.442 5032986.777
 S10 381341.862 5033062.982
 S11 381348.903 5033062.839
 S11 381355.186 5033056.237
 S11 381352.72 5033054.328
 S11 381352.23 5033054.177
 S11 381340.298 5033037.792
 S11 381338.894 5033036.045
 S11 381327.521 5033031.08
 S11 381325.697 5033035.259
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 S11 381318.66 5033049.243
 S11 381320.155 5033050.329
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 S11 381335.192 5033057.864
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 S11 381336.581 5033058.572
 S11 381337.055 5033058.848
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 S2 381352.652 5032978.217
 S2 381352.769 5032978.266
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 S5 381318.612 5033009.797
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 S6 381343.736 5033015.306
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Post-Development 3-hour Chicago 1:100 year Event

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S9 381347.856 5032976.123
S9 381343.215 5032981.464
S9 381343.565 5032982.177
S9 381334.493 5032979.128
S9 381327.851 5032994.342
S9 381333.208 5032996.68
S9 381341.795 5032998.364
S9 381349.118 5033003.477
S9 381354.292 5032987.517
S9 381336.243 5032998.333
S9 381355.738 5032997.554
S9 381360.442 5032986.777

;;Storage Node X-Coord Y-Coord
;----
;[SYMBOLS]
;;Gage X-Coord Y-Coord
;----

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Element Count

Number of rain gages 1
Number of subcatchments 11
Number of nodes 16
Number of links 17
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name Data Source Data Type Recording Interval
Rainfall 3CHI100 INTENSITY 10 min.

Subcatchment Summary

Name Area Width %Imperv %Slope Rain Gage Outlet
St_UnGrd 0.01 6.10 100.00 15.0000 Rainfall
S1 0.01 29.00 8.79 1.0000 Rainfall CB118
S11 0.05 70.00 18.00 2.0000 Rainfall CB111
S2 0.00 4.30 0.00 1.9000 Rainfall CB113
S3 0.02 75.00 17.80 1.0000 Rainfall CB112
S4 0.04 52.00 8.51 3.5000 Rainfall CB110
S5 0.10 105.00 100.00 1.0000 Rainfall Roof_1
S6 0.08 45.00 66.78 2.5000 Rainfall CB115
S7 0.04 28.60 90.42 2.7000 Rainfall CB116
S8 0.10 105.00 100.00 1.0000 Rainfall Roof_2
S9 0.05 50.00 93.37 2.4000 Rainfall CB117

Node Summary

Name Type Invert Elev. Max. Depth Ponded Area External Inflow
OF4 OUTFALL 80.63 0.00 0.0
OF7 OUTFALL 80.63 0.00 0.0
STUB15 OUTFALL 80.63 0.00 0.0
STUB16 OUTFALL 80.65 0.00 0.0
CB110 STORAGE 83.75 1.00 0.0
CB111 STORAGE 83.59 2.01 0.0
CB112 STORAGE 83.28 2.12 0.0
CB113 STORAGE 83.55 1.70 0.0
CB114 STORAGE 85.30 0.25 0.0
CB115 STORAGE 85.25 0.31 0.0
CB116 STORAGE 85.20 0.40 0.0
CB117 STORAGE 85.15 0.15 0.0
CB118 STORAGE 85.22 0.18 0.0
Roof_1 STORAGE 96.35 0.15 0.0
Roof_2 STORAGE 96.35 0.15 0.0
St_UnGrd STORAGE 81.80 1.00 0.0 Yes

Link Summary

Name From Node To Node Type Length %Slope
Roughness

C1 CB111 CB112 CONDUIT 28.9 0.8647
0.0130 CB113 CB112 CONDUIT 31.3 0.6067
0.0130 C5 CB110 CB111 CONDUIT 20.5 0.7815
0.0130 W1 CB115 CB116 WEIR
W2 CB116 CB117 WEIR
W3 CB112 CB113 WEIR
W4 CB114 CB115 WEIR
W5 CB110 CB112 WEIR
C2 CB112 STUB16 OUTLET
OL1 CB115 St_UnGrd OUTLET
OL2 CB116 St_UnGrd OUTLET
OL3 CB117 St_UnGrd OUTLET
OL4 Roof_1 STUB15 OUTLET
OL5 Roof_2 OF4 OUTLET
OL6 CB114 St_UnGrd OUTLET
OL7 St_UnGrd OF7 OUTLET
OL8 CB118 St_UnGrd OUTLET

Cross Section Summary

Conduit Shape Full Depth Full Area Hyd. Rad. Max. Width No. of Barrels Full Flow
C1 CIRCULAR 0.15 0.02 0.04 0.15 1 0.01
C3 CIRCULAR 0.15 0.02 0.04 0.15 1 0.01
C5 CIRCULAR 0.15 0.02 0.04 0.15 1 0.01

Analysis Options

Flow Units CMS
Process Models:
Rainfall/Runoff YES
RDI NO
Snowmelt NO
Groundwater NO
Flow Routing LFS
Flow Routing Allowed NO
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 01/01/2000 00:00:00
Ending Date 01/01/2000 06:00:00
Interpretation Days 00:01:00
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 5.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001500 m

Runoff Quantity Continuity Volume Depth

Runoff Quantity Continuity hectare-m mm

Total Precipitation 0.036 71.708
Evaporation Loss 0.000 0.000
Infiltration Loss 0.007 13.013
Surface Runoff 0.029 57.684
Final Storage 0.001 1.117
Continuity Error (%) -0.149

Post-Development 3-hour Chicago 1:100 year Event

Flow Routing Continuity		Volume hectare-m	Volume 10^6 ltr		CB114	STORAGE	0.020	0.020	0 01:10	0.0161	0.0161
Dry Weather Inflow		0.000	0.000		-0.000	STORAGE	0.037	0.037	0 01:10	0.0452	0.0452
Wet Weather Inflow		0.029	0.293		-0.005	STORAGE	0.020	0.020	0 01:10	0.0276	0.0276
Groundwater Inflow		0.000	0.000		-0.008	STORAGE	0.027	0.027	0 01:10	0.0368	0.0368
RDW Inflow		0.000	0.000		-0.006	STORAGE	0.006	0.006	0 01:10	0.00424	0.00424
External Inflow		0.024	0.241		-0.001	Roof_1	0.047	0.047	0 01:10	0.0669	0.0669
External Outflow		0.000	0.000		0.003	Roof_2	0.048	0.048	0 01:10	0.0678	0.0678
Flooding Loss		0.000	0.000		0.000	St_UndGrd	0.005	0.005	0 01:10	0.00886	0.139
Evaporation Loss		0.000	0.000								
Exfiltration Loss		0.000	0.000								
Initial Stored Volume		0.000	0.000								
Final Stored Volume		0.005	0.054								
Continuity Error (%)		-0.013									

Time-Step Critical Elements											
None											

Highest Flow Instability Indexes											
Link OL7 (6)											

Most Frequent Nonconverging Nodes											
Convergence obtained at all time steps.											

Routing Time Step Summary											
Minimum Time Step : 0.29 sec											
Average Time Step : 5.00 sec											
Maximum Time Step : 5.00 sec											
% of Time in Steady State : 0.00											
Average Iterations per Step : 2.00											
% of Steps Not Converging : 0.00											
Time Step Frequencies :											
5.000	-	3.155	sec	99.98 %							
3.155	-	1.991	sec	0.00 %							
1.991	-	1.256	sec	0.02 %							
1.256	-	0.792	sec	0.00 %							
0.792	-	0.500	sec	0.00 %							

Subcatchment Runoff Summary											
Total	Total	Peak	Total	Total	Total	Imperv	Perv				
Runoff	Runoff	Runoff	Runoff	Runon	Evap	Infil	Runoff				
mm	10^6 ltr	mm	mm	mm	mm	mm	mm				
Subcatchment	Coeff										
mm	CMS										
S1		71.71	0.00	0.00	0.00	70.25	0.00				
70.25	0.01	0.00	0.980								
S10		71.71	0.00	0.00	40.91	6.17	30.75				
30.75	0.00	0.01	0.429								
S11		71.71	0.00	0.00	37.49	13.19	34.01				
34.01	0.02	0.02	0.474								
S2		71.71	0.00	0.00	44.16	0.00	27.60				
27.60	0.00	0.00	0.385								
S3		71.71	0.00	0.00	37.68	12.49	33.87				
33.87	0.01	0.01	0.472								
S4		71.71	0.00	0.00	40.96	5.97	30.70				
30.70	0.01	0.02	0.448								
S5		71.71	0.00	0.00	20.74	0.00	0.00				
70.24	0.07	0.05	0.980								
S6		71.71	0.00	0.00	14.50	46.91	9.35				
56.26	0.05	0.04	0.785								
S7		71.71	0.00	0.00	4.14	63.51	2.75				
66.26	0.03	0.02	0.924								
S8		71.71	0.00	0.00	70.24	0.00	0.00				
70.24	0.07	0.05	0.980								
S9		71.71	0.00	0.00	2.78	65.73	1.86				
67.59	0.04	0.03	0.943								

Node Depth Summary											
Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters					
OF4	OUTFALL	0.48	0.48	81.11	0 00:00	0.48					
OF7	OUTFALL	0.48	0.48	81.11	0 00:00	0.48					
STUB15	OUTFALL	0.48	0.48	81.11	0 00:00	0.48					
STUB16	OUTFALL	0.63	0.63	81.28	0 00:00	0.63					
CB110	STORAGE	0.08	1.25	85.00	0 01:13	1.25					
CB111	STORAGE	0.09	1.37	84.96	0 01:14	1.37					
CB112	STORAGE	0.13	1.86	84.91	0 01:15	1.63					
CB113	STORAGE	0.19	1.86	84.91	0 01:15	1.36					
CB114	STORAGE	0.00	0.03	85.33	0 01:10	0.03					
CB115	STORAGE	0.00	0.05	85.30	0 01:10	0.05					
CB116	STORAGE	0.00	0.03	85.23	0 01:10	0.03					
CB117	STORAGE	0.00	0.04	85.19	0 01:10	0.04					
CB118	STORAGE	0.00	0.01	85.23	0 01:10	0.01					
Roof_1	STORAGE	0.05	0.08	96.43	0 02:20	0.08					
Roof_2	STORAGE	0.05	0.08	96.43	0 02:21	0.08					
St_UndGrd	STORAGE	0.10	0.98	82.78	0 01:20	0.98					

Node Inflow Summary											
Flow		Maximum Lateral	Maximum Total	Lateral	Time of Max Inflow	Total Inflow					
Balance		Inflow	Inflow	Occurrence	Volume	Volume					
Error	Type	CMS	CMS	days hr:min	10^6 ltr	10^6 ltr					
Node	Type										
Percent											
OF4	OUTFALL	0.000	0.002	0 00:32	0	0.0402					
0.000	OUTFALL	0.000	0.029	0 01:00	0	0.139					
0.000	OUTFALL	0.000	0.002	0 00:32	0	0.0402					
STUB15	OUTFALL	0.000	0.010	0 01:15	0	0.0218					
STUB16	OUTFALL	0.000	0.018	0 01:10	0.0132	0.0132					
-0.017	STORAGE	0.018	0.018	0 01:10	0.0132	0.0132					
CB111	STORAGE	0.000	0.014	0 01:10	0	0.0132					
-0.319	STORAGE	0.010	0.018	0 01:10	0.00739	0.0229					
0.020	STORAGE	0.002	0.007	0 01:05	0.00121	0.00227					

Conduit Surcharge Summary											
Conduit		Adjusted Actual Length		Dry	Dry	Dry	Fraction of Time in Flow	Class	Adjusted Actual Length		
C1		1.00		0.17	0.00	0.00	0.14	0.00	0.00	0.69	0.02
C3		1.00		0.17	0.00	0.00	0.14	0.00	0.00	0.69	0.02
C5		1.00		0.17	0.00	0.00	0.82	0.01	0.00	0.00	0.72

Flow Classification Summary											
Conduit		Both Ends	Hours	Upstream	Dnstream		Above Normal Flow	Capacity	Both Ends	Hours	Hours
C1			0.61	0.61	0.74		0.01	0.01		0.01	0.01
C3			0.62	0.62	0.73		0.01	0.01		0.01	0.01
C5			0.55	0.55	0.61		0.01	0.01		0.01	0.01

Analysis begun on:	Thu Oct 10 10:10:11 2024										
Analysis ended on:	Thu Oct 10 10:10:12 2024										
Total elapsed time:	00:00:01										

Post-Development 24-hour SCS 1:100-year Event

[TITLE]
;Project Title/Notes

[OPTIONS]
;Option Value
FLOW_UNITS CMS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE 01/01/2000
START_TIME 00:00:00
REPORT_START_DATE 01/01/2000
REPORT_START_TIME 00:00:00
END_DATE 01/02/2000
END_TIME 00:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 5
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 12

[EVAPORATION]
;Data Source Parameters

CONSTANT 0.0
DRY_ONLY NO

[RAINGAGES]
;Name Format Interval SCF Source
Rainfall INTENSITY 0:15 1.0 TIMESERIES 24SCS100

[SUBCATCHMENTS]
;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen
SnowPack ;

S1 Rainfall St_UnGrd 0.0091 100 6.1 15 0
S10 Rainfall CB18 0.0138 8.79 28.998 1 0
S11 Rainfall CB14 0.0473 18.805 70.001 2 0
S2 Rainfall CB13 0.0474 0.002 4.3 1.9 0
S3 Rainfall CB12 0.0218 17.802 75 1 0
S4 Rainfall CB10 0.043 8.5 52.001 3.5 0
S5 Rainfall Roof_1 0.0952 99.998 104.996 1 0
S6 Rainfall CB15 0.0803 66.775 45.001 2.5 0
S7 Rainfall CB16 0.0416 90.416 28.601 2.7 0
S8 Rainfall Roof_2 0.0965 100 105.005 1 0
S9 Rainfall CB17 0.0544 93.568 50 2.4 0

[LOSSES]
;Link Kentry Kexit Kavg Flap Gate Seepage

C1 CIRCULAR 0.15 0 0 0 0 1
C3 CIRCULAR 0.15 0 0 0 0 1
C5 CIRCULAR 0.15 0 0 0 0 1

W1 RECT_OPEN 0.11 17.1 0 0 0 0
W2 RECT_OPEN 0.25 17.1 0 0 0 0
W3 RECT_OPEN 0.02 4.58 0 0 0 0
W4 RECT_OPEN 0.05 7 0 0 0 0
W5 RECT_OPEN 0.05 3.487 0 0 0 0

[INFLows]
;Node Constituent Time Series Type Mfactor Sfactor Baseline Pattern

St_UnGrd FLOW InfiltatedInflow FLOW 1.0 1 0

[CURVES]
;Name Type X-Value Y-Value

MHF_IPEX_TYPE_A Rating Curve for MHF IPEX TYPE A, No grate allowance
MHF_IPEX_TYPE_A Rating 0.1 0.0057
MHF_IPEX_TYPE_A Rating 0.2 0.0081
MHF_IPEX_TYPE_A Rating 0.3 0.0099
MHF_IPEX_TYPE_A Rating 0.4 0.0114
MHF_IPEX_TYPE_A Rating 0.5 0.0128
MHF_IPEX_TYPE_A Rating 0.6 0.014
MHF_IPEX_TYPE_A Rating 0.7 0.0151
MHF_IPEX_TYPE_A Rating 0.8 0.0162
MHF_IPEX_TYPE_A Rating 0.9 0.0172
MHF_IPEX_TYPE_A Rating 1 0.0181
MHF_IPEX_TYPE_A Rating 1.2 0.0198
MHF_IPEX_TYPE_A Rating 1.4 0.0214
MHF_IPEX_TYPE_A Rating 1.6 0.0229
MHF_IPEX_TYPE_A Rating 1.8 0.0243
MHF_IPEX_TYPE_A Rating 2 0.0256
MHF_IPEX_TYPE_A Rating 2.5 0.0286
MHF_IPEX_TYPE_A Rating 3 0.0313

[SUBAREAS]
;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted

S1 0.013 0.25 1.57 4.67 0 OUTLET
S10 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S11 0.013 0.25 1.57 4.67 0 OUTLET
S2 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S3 0.013 0.25 1.57 4.67 0 PREVIOUS 100
S4 0.013 0.25 1.57 4.67 0 OUTLET
S5 0.013 0.25 1.57 4.67 0 OUTLET
S6 0.013 0.25 1.57 4.67 0 OUTLET
S7 0.013 0.25 1.57 4.67 0 OUTLET
S8 0.013 0.25 1.57 4.67 0 OUTLET
S9 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]
;Subcatchment Param1 Param2 Param3 Param4 Param5

S1 76.2 13.2 4.14 7 0
S10 76.2 13.2 4.14 7 0
S11 76.2 13.2 4.14 7 0
S2 76.2 13.2 4.14 7 0
S3 76.2 13.2 4.14 7 0
S4 76.2 13.2 4.14 7 0
S5 76.2 13.2 4.14 7 0
S6 76.2 13.2 4.14 7 0
S7 76.2 13.2 4.14 7 0
S8 76.2 13.2 4.14 7 0
S9 76.2 13.2 4.14 7 0

[OUTFALLS]
;Name Elevation Type Stage Data Gated Route To

O54 80.628 FIXED 81.11 NO
OFT 80.628 FIXED 81.11 NO
STUB15 80.628 FIXED 81.11 NO
STUB16 80.646 FIXED 81.28 NO

[STORAGE]
;Name Elev. MaxDepth InitDepth Shape Curve Name/Params

CB110 83.75 1.9 0 TABULAR CB110 0
CB111 83.59 2.01 0 TABULAR CB111 0
CB112 83.28 2.12 0 TABULAR CB112 0
CB113 83.55 1.7 0 TABULAR CB113 0
CB114 85.3 0.25 0 TABULAR CB114 0
CB115 85.25 0.31 0 TABULAR CB115 0
CB116 85.2 0.4 0 TABULAR CB116 0
CB117 85.15 0.15 0 TABULAR CB117 0
CB118 85.22 0.18 0 TABULAR CB118 0

[CONDUTTS]
;Name From Node To Node Length Roughness InOffset OutOffset

InitFlow MaxFlow

;;-----
C1 0 0.08 CB111 CB112 28.913 0.013 83.59 83.34
C3 0 0.04 CB113 CB112 31.317 0.013 83.55 83.36
C5 0 0.04 CB110 CB111 20.475 0.013 83.75 83.59

[WEIRS]
;Name EndCon EndCoeff From Node Surcharge To Node RoadWidth RoadSurf Type Coeff Curve Qcoeff Gated

W1 0 CB115 CB116 TRANSVERSE 85.45 1.84 NO
W2 0 CB116 CB117 TRANSVERSE 85.35 1.84 NO
W3 0 CB112 CB113 TRANSVERSE 85.38 1.84 NO
W4 0 CB114 CB115 TRANSVERSE 85.5 1.84 NO
W5 0 CB110 CB112 TRANSVERSE 85.6 1.84 NO

[OUTLETS]
;Name Qexpon Gated From Node To Node Offset Type QTable/Qcoeff

C2 NO CB112 STUB16 83.28 TABULAR/HEAD Vortex_ICD_95
OL1 NO CB115 St_UnGrd 85.25 TABULAR/HEAD ZURN_Z150F-6NH
OL2 NO CB116 St_UnGrd 85.2 TABULAR/HEAD ZURN_Z150F-6NH
OL3 NO CB117 St_UnGrd 85.15 TABULAR/HEAD ZURN_Z150F-6NH
OL4 NO Roof_1 STUB15 96.35 TABULAR/HEAD O_Roof1
OL5 NO Roof_2 OF4 96.35 TABULAR/HEAD O_Roof1
OL6 NO CB114 St_UnGrd 85.3 TABULAR/HEAD ZURN_Z150F-6NH
OL7 NO St_UnGrd OF7 81.8 TABULAR/HEAD O_St_UnGrd
OL8 NO CB118 St_UnGrd 85.22 TABULAR/HEAD ZURN_Z150F-6NH

[SECTIONS]
;Link Culvert Shape Geom1 Geom2 Geom3 Geom4 Barrels

C1 CIRCULAR 0.15 0 0 0 0 1
C3 CIRCULAR 0.15 0 0 0 0 1
C5 CIRCULAR 0.15 0 0 0 0 1

W1 RECT_OPEN 0.11 17.1 0 0 0 0
W2 RECT_OPEN 0.25 17.1 0 0 0 0
W3 RECT_OPEN 0.02 4.58 0 0 0 0
W4 RECT_OPEN 0.05 7 0 0 0 0
W5 RECT_OPEN 0.05 3.487 0 0 0 0

[LOSSES]
;Link Kentry Kexit Kavg Flap Gate Seepage

St_UnGrd FLOW InfiltatedInflow FLOW 1.0 1 0

[CURVES]
;Name Type X-Value Y-Value

MHF_IPEX_TYPE_B Rating Curve for MHF IPEX TYPE B, No grate allowance
MHF_IPEX_TYPE_B Rating 0 0
MHF_IPEX_TYPE_B Rating 0.1 0.0081
MHF_IPEX_TYPE_B Rating 0.2 0.0115
MHF_IPEX_TYPE_B Rating 0.3 0.0141
MHF_IPEX_TYPE_B Rating 0.4 0.0162
MHF_IPEX_TYPE_B Rating 0.5 0.0182
MHF_IPEX_TYPE_B Rating 0.6 0.0199
MHF_IPEX_TYPE_B Rating 0.7 0.0215
MHF_IPEX_TYPE_B Rating 0.8 0.023
MHF_IPEX_TYPE_B Rating 0.9 0.0244
MHF_IPEX_TYPE_B Rating 1 0.0257
MHF_IPEX_TYPE_B Rating 1.2 0.0281
MHF_IPEX_TYPE_B Rating 1.4 0.0304
MHF_IPEX_TYPE_B Rating 1.6 0.0325
MHF_IPEX_TYPE_B Rating 1.8 0.0344
MHF_IPEX_TYPE_B Rating 2 0.0363
MHF_IPEX_TYPE_B Rating 2.5 0.0406
MHF_IPEX_TYPE_B Rating 3 0.0445

[CURVES]
;Name Type X-Value Y-Value

MHF_IPEX_TYPE_C Rating Curve for MHF IPEX TYPE C, No grate allowance
MHF_IPEX_TYPE_C Rating 0 0
MHF_IPEX_TYPE_C Rating 0.1 0.0106
MHF_IPEX_TYPE_C Rating 0.2 0.0115
MHF_IPEX_TYPE_C Rating 0.3 0.0133
MHF_IPEX_TYPE_C Rating 0.4 0.0212
MHF_IPEX_TYPE_C Rating 0.5 0.0237
MHF_IPEX_TYPE_C Rating 0.6 0.0259
MHF_IPEX_TYPE_C Rating 0.7 0.028
MHF_IPEX_TYPE_C Rating 0.8 0.0299
MHF_IPEX_TYPE_C Rating 0.9 0.0317
MHF_IPEX_TYPE_C Rating 1 0.0335
MHF_IPEX_TYPE_C Rating 1.2 0.0366
MHF_IPEX_TYPE_C Rating 1.4 0.0396
MHF_IPEX_TYPE_C Rating 1.6 0.0423
MHF_IPEX_TYPE_C Rating 1.8 0.0449
MHF_IPEX_TYPE_C Rating 2 0.0473
MHF_IPEX_TYPE_C Rating 2.5 0.0529
MHF_IPEX_TYPE_C Rating 3 0.0579

[CURVES]
;Name Type X-Value Y-Value

MHF_IPEX_TYPE_D Rating Curve for MHF IPEX TYPE D, No grate allowance
MHF_IPEX_TYPE_D Rating 0 0
MHF_IPEX_TYPE_D Rating 0.1 0.0154
MHF_IPEX_TYPE_D Rating 0.2 0.0217
MHF_IPEX_TYPE_D Rating 0.3 0.0266
MHF_IPEX_TYPE_D Rating 0.4 0.0307
MHF_IPEX_TYPE_D Rating 0.5 0.0343
MHF_IPEX_TYPE_D Rating 0.6 0.0376
MHF_IPEX_TYPE_D Rating 0.7 0.0406
MHF_IPEX_TYPE_D Rating 0.8 0.0434
MHF_IPEX_TYPE_D Rating 0.9 0.0461
MHF_IPEX_TYPE_D Rating 1 0.0485
MHF_IPEX_TYPE_D Rating 1.2 0.0532

Post-Development 24-hour SCS 1:100 year Event

October 2024

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MHF_IPEX_TYPE_D 1.4 0.0574
MHF_IPEX_TYPE_D 1.6 0.0614
MHF_IPEX_TYPE_D 1.8 0.0651
MHF_IPEX_TYPE_D 2 0.0687
MHF_IPEX_TYPE_D 2.5 0.0768
MHF_IPEX_TYPE_D 3 0.0841

;Tempest Rating Curve for MHF IPEX TYPE E, No grate allowance
MHF_IPEX_TYPE_E Rating 0 0
MHF_IPEX_TYPE_E 0.1 0.0205
MHF_IPEX_TYPE_E 0.2 0.0289
MHF_IPEX_TYPE_E 0.3 0.0355
MHF_IPEX_TYPE_E 0.4 0.0409
MHF_IPEX_TYPE_E 0.5 0.0458
MHF_IPEX_TYPE_E 0.6 0.0501
MHF_IPEX_TYPE_E 0.7 0.0542
MHF_IPEX_TYPE_E 0.8 0.0579
MHF_IPEX_TYPE_E 0.9 0.0614
MHF_IPEX_TYPE_E 1 0.0647
MHF_IPEX_TYPE_E 1.2 0.0709
MHF_IPEX_TYPE_E 1.4 0.0766
MHF_IPEX_TYPE_E 1.6 0.0819
MHF_IPEX_TYPE_E 1.8 0.0868
MHF_IPEX_TYPE_E 2 0.0915
MHF_IPEX_TYPE_E 2.5 0.1023
MHF_IPEX_TYPE_E 3 0.1121

O_Roof1 Rating 0 0
O_Roof1 0.001 0.002
O_Roof1 0.15 0.002

O_Roof2 Rating 0 0
O_Roof2 0.001 0.0025
O_Roof2 0.15 0.0025

O_St_UnGrd Rating 0 0
O_St_UnGrd 0.01 0.029
O_St_UnGrd 1 0.029

;Tempest Rating Curve for Vortex ICD 100, No grate allowance
Vortex_ICD_100 Rating 0 0
Vortex_ICD_100 0.1 0.0028
Vortex_ICD_100 0.2 0.004
Vortex_ICD_100 0.3 0.0049
Vortex_ICD_100 0.4 0.0056
Vortex_ICD_100 0.5 0.0063
Vortex_ICD_100 0.6 0.0069
Vortex_ICD_100 0.7 0.0075
Vortex_ICD_100 0.8 0.008
Vortex_ICD_100 0.9 0.0085
Vortex_ICD_100 1 0.0089
Vortex_ICD_100 1.2 0.0098
Vortex_ICD_100 1.4 0.0106
Vortex_ICD_100 1.6 0.0113
Vortex_ICD_100 1.8 0.012
Vortex_ICD_100 2 0.0126
Vortex_ICD_100 2.5 0.0141
Vortex_ICD_100 3 0.0155

;Tempest Rating Curve for Vortex ICD 105, No grate allowance
Vortex_ICD_105 Rating 0 0
Vortex_ICD_105 0.1 0.0031
Vortex_ICD_105 0.2 0.0044
Vortex_ICD_105 0.3 0.0054
Vortex_ICD_105 0.4 0.0062
Vortex_ICD_105 0.5 0.0069
Vortex_ICD_105 0.6 0.0076
Vortex_ICD_105 0.7 0.0082
Vortex_ICD_105 0.8 0.0088
Vortex_ICD_105 0.9 0.0093
Vortex_ICD_105 1 0.0098
Vortex_ICD_105 1.2 0.0107
Vortex_ICD_105 1.4 0.0116
Vortex_ICD_105 1.6 0.0124
Vortex_ICD_105 1.8 0.0131
Vortex_ICD_105 2 0.0139
Vortex_ICD_105 2.5 0.0155
Vortex_ICD_105 3 0.017

;Tempest Rating Curve for Vortex ICD 40, No grate allowance
Vortex_ICD_40 Rating 0 0
Vortex_ICD_40 0.1 0.0004
Vortex_ICD_40 0.2 0.0005
Vortex_ICD_40 0.3 0.0007
Vortex_ICD_40 0.4 0.0009
Vortex_ICD_40 0.5 0.001
Vortex_ICD_40 0.6 0.001
Vortex_ICD_40 0.7 0.0011
Vortex_ICD_40 0.8 0.0012
Vortex_ICD_40 0.9 0.0013
Vortex_ICD_40 1 0.0014
Vortex_ICD_40 1.2 0.0015
Vortex_ICD_40 1.4 0.0016
Vortex_ICD_40 1.6 0.0017
Vortex_ICD_40 1.8 0.0018
Vortex_ICD_40 2 0.0019
Vortex_ICD_40 2.5 0.0022
Vortex_ICD_40 3 0.0024

;Tempest Rating Curve for Vortex ICD 45, No grate allowance
Vortex_ICD_45 Rating 0 0
Vortex_ICD_45 0.1 0.0006
Vortex_ICD_45 0.2 0.0008
Vortex_ICD_45 0.3 0.001
Vortex_ICD_45 0.4 0.0011
Vortex_ICD_45 0.5 0.0013
Vortex_ICD_45 0.6 0.0014
Vortex_ICD_45 0.7 0.0015
Vortex_ICD_45 0.8 0.0016
Vortex_ICD_45 0.9 0.0017
Vortex_ICD_45 1 0.0018
Vortex_ICD_45 1.2 0.002
Vortex_ICD_45 1.4 0.0021
Vortex_ICD_45 1.6 0.0023
Vortex_ICD_45 1.8 0.0024
Vortex_ICD_45 2 0.0026
Vortex_ICD_45 2.5 0.0029
Vortex_ICD_45 3 0.0031

;Tempest Rating Curve for Vortex ICD 50, No grate allowance
Vortex_ICD_50 Rating 0 0
Vortex_ICD_50 0.1 0.0007
Vortex_ICD_50 0.2 0.001
Vortex_ICD_50 0.3 0.0012
Vortex_ICD_50 0.4 0.0014
Vortex_ICD_50 0.5 0.0016
Vortex_ICD_50 0.6 0.0018
Vortex_ICD_50 0.7 0.0019
Vortex_ICD_50 0.8 0.002
Vortex_ICD_50 0.9 0.0021
Vortex_ICD_50 1 0.0023
Vortex_ICD_50 1.2 0.0025
Vortex_ICD_50 1.4 0.0027
Vortex_ICD_50 1.6 0.0029
Vortex_ICD_50 1.8 0.003
Vortex_ICD_50 2 0.0032
Vortex_ICD_50 2.5 0.0036
Vortex_ICD_50 3 0.0039

;Tempest Rating Curve for Vortex ICD 55, No grate allowance
Vortex_ICD_55 Rating 0 0
Vortex_ICD_55 0.1 0.0009
Vortex_ICD_55 0.2 0.0012
Vortex_ICD_55 0.3 0.0015
Vortex_ICD_55 0.4 0.0017
Vortex_ICD_55 0.5 0.0019
Vortex_ICD_55 0.6 0.0021
Vortex_ICD_55 0.7 0.0023
Vortex_ICD_55 0.8 0.0024

Vortex_ICD_55 0.9 0.0025
Vortex_ICD_55 1 0.0027
Vortex_ICD_55 1.2 0.0029
Vortex_ICD_55 1.4 0.0031
Vortex_ICD_55 1.6 0.0034
Vortex_ICD_55 1.8 0.0036
Vortex_ICD_55 2 0.0038
Vortex_ICD_55 2.5 0.0043
Vortex_ICD_55 3 0.0047

;Tempest Rating Curve for Vortex ICD 60, No grate allowance
Vortex_ICD_60 Rating 0 0
Vortex_ICD_60 0.1 0.0011
Vortex_ICD_60 0.2 0.0015
Vortex_ICD_60 0.3 0.0018
Vortex_ICD_60 0.4 0.0021
Vortex_ICD_60 0.5 0.0023
Vortex_ICD_60 0.6 0.0025
Vortex_ICD_60 0.7 0.0027
Vortex_ICD_60 0.8 0.0029
Vortex_ICD_60 0.9 0.0031
Vortex_ICD_60 1 0.0032
Vortex_ICD_60 1.2 0.0036
Vortex_ICD_60 1.4 0.0038
Vortex_ICD_60 1.6 0.0041
Vortex_ICD_60 1.8 0.0043
Vortex_ICD_60 2 0.0046
Vortex_ICD_60 2.5 0.0051
Vortex_ICD_60 3 0.0056

;Tempest Rating Curve for Vortex ICD 65, No grate allowance
Vortex_ICD_65 Rating 0 0
Vortex_ICD_65 0.1 0.0012
Vortex_ICD_65 0.2 0.0016
Vortex_ICD_65 0.3 0.002
Vortex_ICD_65 0.4 0.0023
Vortex_ICD_65 0.5 0.0025
Vortex_ICD_65 0.6 0.0028
Vortex_ICD_65 0.7 0.003
Vortex_ICD_65 0.8 0.0032
Vortex_ICD_65 0.9 0.0034
Vortex_ICD_65 1 0.0036
Vortex_ICD_65 1.2 0.004
Vortex_ICD_65 1.4 0.0043
Vortex_ICD_65 1.6 0.0046
Vortex_ICD_65 1.8 0.0049
Vortex_ICD_65 2 0.0051
Vortex_ICD_65 2.5 0.0057
Vortex_ICD_65 3 0.0063

;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex_ICD_70 Rating 0 0
Vortex_ICD_70 0.1 0.0013
Vortex_ICD_70 0.2 0.0019
Vortex_ICD_70 0.3 0.0023
Vortex_ICD_70 0.4 0.0027
Vortex_ICD_70 0.5 0.003
Vortex_ICD_70 0.6 0.0033
Vortex_ICD_70 0.7 0.0036
Vortex_ICD_70 0.8 0.0038
Vortex_ICD_70 0.9 0.0041
Vortex_ICD_70 1 0.0043
Vortex_ICD_70 1.2 0.0047
Vortex_ICD_70 1.4 0.0051
Vortex_ICD_70 1.6 0.0055
Vortex_ICD_70 1.8 0.0058
Vortex_ICD_70 2 0.0061
Vortex_ICD_70 2.5 0.0068
Vortex_ICD_70 3 0.0075

;Tempest Rating Curve for Vortex ICD 75, No grate allowance
Vortex_ICD_75 Rating 0 0
Vortex_ICD_75 0.1 0.0016
Vortex_ICD_75 0.2 0.0022
Vortex_ICD_75 0.3 0.0027
Vortex_ICD_75 0.4 0.0032
Vortex_ICD_75 0.5 0.0035
Vortex_ICD_75 0.6 0.0039
Vortex_ICD_75 0.7 0.0042
Vortex_ICD_75 0.8 0.0045
Vortex_ICD_75 0.9 0.0048
Vortex_ICD_75 1 0.005
Vortex_ICD_75 1.2 0.0055
Vortex_ICD_75 1.4 0.0059
Vortex_ICD_75 1.6 0.0063
Vortex_ICD_75 1.8 0.0067
Vortex_ICD_75 2 0.0071
Vortex_ICD_75 2.5 0.0079
Vortex_ICD_75 3 0.0087

;Tempest Rating Curve for Vortex ICD 80, No grate allowance
Vortex_ICD_80 Rating 0 0
Vortex_ICD_80 0.1 0.0018
Vortex_ICD_80 0.2 0.0026
Vortex_ICD_80 0.3 0.0031
Vortex_ICD_80 0.4 0.0036
Vortex_ICD_80 0.5 0.004
Vortex_ICD_80 0.6 0.0044
Vortex_ICD_80 0.7 0.0048
Vortex_ICD_80 0.8 0.0051
Vortex_ICD_80 0.9 0.0054
Vortex_ICD_80 1 0.0057
Vortex_ICD_80 1.2 0.0063
Vortex_ICD_80 1.4 0.0068
Vortex_ICD_80 1.6 0.0072
Vortex_ICD_80 1.8 0.0077
Vortex_ICD_80 2 0.0081
Vortex_ICD_80 2.5 0.009
Vortex_ICD_80 3 0.0099

;Tempest Rating Curve for Vortex ICD 85, No grate allowance
Vortex_ICD_85 Rating 0 0
Vortex_ICD_85 0.1 0.002
Vortex_ICD_85 0.2 0.0029
Vortex_ICD_85 0.3 0.0035
Vortex_ICD_85 0.4 0.0041
Vortex_ICD_85 0.5 0.0045
Vortex_ICD_85 0.6 0.005
Vortex_ICD_85 0.7 0.0054
Vortex_ICD_85 0.8 0.0057
Vortex_ICD_85 0.9 0.0061
Vortex_ICD_85 1 0.0064
Vortex_ICD_85 1.2 0.007
Vortex_ICD_85 1.4 0.0076
Vortex_ICD_85 1.6 0.0081
Vortex_ICD_85 1.8 0.0086
Vortex_ICD_85 2 0.0091
Vortex_ICD_85 2.5 0.0101
Vortex_ICD_85 3 0.0111

;Tempest Rating Curve for Vortex ICD 90, No grate allowance
Vortex_ICD_90 Rating 0 0
Vortex_ICD_90 0.1 0.0022
Vortex_ICD_90 0.2 0.0032
Vortex_ICD_90 0.3 0.0039
Vortex_ICD_90 0.4 0.0045
Vortex_ICD_90 0.5 0.0051
Vortex_ICD_90 0.6 0.0055
Vortex_ICD_90 0.7 0.006
Vortex_ICD_90 0.8 0.0064
Vortex_ICD_90 0.9 0.0068
Vortex_ICD_90 1 0.0072
Vortex_ICD_90 1.2 0.0079
Vortex_ICD_90 1.4 0.0085
Vortex_ICD_90 1.6 0.0091
Vortex_ICD_90 1.8 0.0096
Vortex_ICD_90 2 0.0102
Vortex_ICD_90 2.5 0.0114
Vortex_ICD_90 3 0.0125

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Post-Development 24-hour SCS 1:100 year Event

October 2024

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;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex_ICD_95 Rating 0
Vortex_ICD_95 0.1 0.0026
Vortex_ICD_95 0.2 0.0036
Vortex_ICD_95 0.3 0.0044
Vortex_ICD_95 0.4 0.0051
Vortex_ICD_95 0.5 0.007
Vortex_ICD_95 0.6 0.0062
Vortex_ICD_95 0.7 0.0067
Vortex_ICD_95 0.8 0.0071
Vortex_ICD_95 0.9 0.0076
Vortex_ICD_95 1 0.008
Vortex_ICD_95 1.2 0.0087
Vortex_ICD_95 1.4 0.0094
Vortex_ICD_95 1.6 0.0101
Vortex_ICD_95 1.8 0.0107
Vortex_ICD_95 2 0.0113
Vortex_ICD_95 2.5 0.0126
Vortex_ICD_95 3 0.0138

;From Zurn Manual Rd178
ZURN_Z150F-6NH Rating 0
ZURN_Z150F-6NH 0.0127 0.00503838308477861
ZURN_Z150F-6NH 0.0257 0.0132420013231177
ZURN_Z150F-6NH 0.0384 0.0234992054543888
ZURN_Z150F-6NH 0.0508 0.0357128365761305
ZURN_Z150F-6NH 0.0765 0.042501972611045
ZURN_Z150F-6NH 0.1024 0.0433196215564931

CB110 Storage 0 0.073
CB110 1.55 0.073
CB110 1.85 120.85
CB110 1.9 120.85

CB111 Storage 0 0.073
CB111 2.01 0.073

CB112 Storage 0 0.36
CB112 1.92 0.36
CB112 2.1 19.13
CB112 2.12 19.13

CB113 Storage 0 0.073
CB113 1.6 0.073
CB113 1.7 3.62

CB114 Storage 0 0.36
CB114 0.2 19.2
CB114 0.25 19.2

CB115 Storage 0 0.36
CB115 0.2 155.53
CB115 0.31 155.53

CB116 Storage 0 0.36
CB116 0.15 73.19
CB116 0.4 73.19

CB117 Storage 0 0.36
CB117 0.15 122.26

CB118 Storage 0 0.36
CB118 0.18 37.7

[TIMESERIES]
;Name Date Time Value
;-----

;Rainfall (mm/hr)
24SCS100 01/01/2000 00:00:00 1.548
24SCS100 01/01/2000 00:15:00 1.548
24SCS100 01/01/2000 00:30:00 1.548
24SCS100 01/01/2000 00:45:00 1.548
24SCS100 01/01/2000 01:00:00 0.7224
24SCS100 01/01/2000 01:15:00 0.7224
24SCS100 01/01/2000 01:30:00 0.7224
24SCS100 01/01/2000 01:45:00 0.7224
24SCS100 01/01/2000 02:00:00 1.3416
24SCS100 01/01/2000 02:15:00 1.3416
24SCS100 01/01/2000 02:30:00 1.3416
24SCS100 01/01/2000 02:45:00 1.3416
24SCS100 01/01/2000 03:00:00 1.3416
24SCS100 01/01/2000 03:15:00 1.3416
24SCS100 01/01/2000 03:30:00 1.3416
24SCS100 01/01/2000 03:45:00 1.3416
24SCS100 01/01/2000 04:00:00 1.7544
24SCS100 01/01/2000 04:15:00 1.7544
24SCS100 01/01/2000 04:30:00 1.7544
24SCS100 01/01/2000 04:45:00 1.7544
24SCS100 01/01/2000 05:00:00 1.548
24SCS100 01/01/2000 05:15:00 1.548
24SCS100 01/01/2000 05:30:00 1.548
24SCS100 01/01/2000 05:45:00 1.064
24SCS100 01/01/2000 06:00:00 2.064
24SCS100 01/01/2000 06:15:00 2.064
24SCS100 01/01/2000 06:30:00 2.064
24SCS100 01/01/2000 06:45:00 2.064
24SCS100 01/01/2000 07:00:00 2.064
24SCS100 01/01/2000 07:15:00 2.064
24SCS100 01/01/2000 07:30:00 2.064
24SCS100 01/01/2000 07:45:00 2.064
24SCS100 01/01/2000 08:00:00 2.7864
24SCS100 01/01/2000 08:15:00 2.7864
24SCS100 01/01/2000 08:30:00 2.7864
24SCS100 01/01/2000 08:45:00 2.7864
24SCS100 01/01/2000 09:00:00 3.3024
24SCS100 01/01/2000 09:15:00 3.3024
24SCS100 01/01/2000 09:30:00 3.7152
24SCS100 01/01/2000 09:45:00 3.7152
24SCS100 01/01/2000 10:00:00 4.7472
24SCS100 01/01/2000 10:15:00 4.7472
24SCS100 01/01/2000 10:30:00 6.3984
24SCS100 01/01/2000 10:45:00 6.594
24SCS100 01/01/2000 11:00:00 9.9072
24SCS100 01/01/2000 11:15:00 9.9072
24SCS100 01/01/2000 11:30:00 42.9312
24SCS100 01/01/2000 11:45:00 113.9328
24SCS100 01/01/2000 12:00:00 14.8608
24SCS100 01/01/2000 12:15:00 14.8608
24SCS100 01/01/2000 12:30:00 7.6368
24SCS100 01/01/2000 12:45:00 7.6368
24SCS100 01/01/2000 13:00:00 5.7228
24SCS100 01/01/2000 13:15:00 5.5728
24SCS100 01/01/2000 13:30:00 4.3344
24SCS100 01/01/2000 13:45:00 4.3344
24SCS100 01/01/2000 14:00:00 3.3024
24SCS100 01/01/2000 14:15:00 3.3024
24SCS100 01/01/2000 14:30:00 3.3024
24SCS100 01/01/2000 14:45:00 3.3024
24SCS100 01/01/2000 15:00:00 2.8896
24SCS100 01/01/2000 15:15:00 2.8896
24SCS100 01/01/2000 15:30:00 2.8896
24SCS100 01/01/2000 15:45:00 2.8896
24SCS100 01/01/2000 16:00:00 2.2704
24SCS100 01/01/2000 16:15:00 2.2704
24SCS100 01/01/2000 16:30:00 2.2704
24SCS100 01/01/2000 16:45:00 2.2704
24SCS100 01/01/2000 17:00:00 2.3736
24SCS100 01/01/2000 17:15:00 2.3736
24SCS100 01/01/2000 17:30:00 2.3736
24SCS100 01/01/2000 17:45:00 2.3736
24SCS100 01/01/2000 18:00:00 1.548
24SCS100 01/01/2000 18:15:00 1.548
24SCS100 01/01/2000 18:30:00 1.548
24SCS100 01/01/2000 18:45:00 1.548
24SCS100 01/01/2000 19:00:00 1.2384
24SCS100 01/01/2000 19:15:00 1.2384
24SCS100 01/01/2000 19:30:00 1.2384
24SCS100 01/01/2000 19:45:00 1.2384

24SCS100 01/01/2000 20:00:00 1.7544
24SCS100 01/01/2000 20:15:00 1.7544
24SCS100 01/01/2000 20:30:00 1.7544
24SCS100 01/01/2000 20:45:00 1.7544
24SCS100 01/01/2000 21:00:00 1.1352
24SCS100 01/01/2000 21:15:00 1.1352
24SCS100 01/01/2000 21:30:00 1.1352
24SCS100 01/01/2000 21:45:00 1.1352
24SCS100 01/01/2000 22:00:00 1.032
24SCS100 01/01/2000 22:15:00 1.032
24SCS100 01/01/2000 22:30:00 1.032
24SCS100 01/01/2000 22:45:00 1.032
24SCS100 01/01/2000 23:00:00 1.032
24SCS100 01/01/2000 23:15:00 1.032
24SCS100 01/01/2000 23:30:00 1.032
24SCS100 01/01/2000 23:45:00 1.032
24SCS100 01/02/2000 00:00:00 0

;Rainfall (mm/hr)
3CH1100 01/01/2000 00:00:00 5.339
3CH1100 01/01/2000 00:15:00 6.376
3CH1100 01/01/2000 00:30:00 7.977
3CH1100 01/01/2000 00:45:00 10.797
3CH1100 01/01/2000 01:00:00 17.136
3CH1100 01/01/2000 01:15:00 51.056
3CH1100 01/01/2000 01:30:00 26.163
3CH1100 01/01/2000 01:45:00 13.571
3CH1100 01/01/2000 02:00:00 13.77
3CH1100 01/01/2000 02:15:00 10.712
3CH1100 01/01/2000 02:30:00 9.008
3CH1100 01/01/2000 02:45:00 7.793
3CH1100 01/01/2000 03:00:00 6.883
3CH1100 01/01/2000 03:15:00 6.174
3CH1100 01/01/2000 03:30:00 5.607
3CH1100 01/01/2000 03:45:00 5.142
3CH1100 01/01/2000 03:00:00 0

;Sum of Infiltrated flow from above the garage slab calculated from Infiltration graphs for S1 multiplied by the total pervious area above the garage.
InfiltratedInflow 01/01/2000 00:00:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:05:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:10:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:15:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:20:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:25:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:30:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:35:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:40:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:45:00 2.306158E-05
InfiltratedInflow 01/01/2000 00:50:00 2.306158E-05
InfiltratedInflow 01/01/2000 01:00:00 178.107
InfiltratedInflow 01/01/2000 01:10:00 51.056
InfiltratedInflow 01/01/2000 01:20:00 26.163
InfiltratedInflow 01/01/2000 01:30:00 13.571
InfiltratedInflow 01/01/2000 01:40:00 13.77
InfiltratedInflow 01/01/2000 02:00:00 10.712
InfiltratedInflow 01/01/2000 02:10:00 9.008
InfiltratedInflow 01/01/2000 02:20:00 7.793
InfiltratedInflow 01/01/2000 02:30:00 6.883
InfiltratedInflow 01/01/2000 02:40:00 6.174
InfiltratedInflow 01/01/2000 02:50:00 5.607
InfiltratedInflow 01/01/2000 03:00:00 5.142
InfiltratedInflow 01/01/2000 03:00:00 0

;Too many data points (1440 in total).
[REPORT]
;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]
Subcatch S1 Ramp
Subcatch S5 Building
Subcatch S6 ParkingLot
Subcatch S7 ParkingLot
Subcatch S8 Building
Subcatch S9 ParkingLot
Node St_UnGrd Underground_Storage

[MAP]
DIMENSIONS 381280.91645 5032948.93205 381379.86255 5033068.41295
UNITS Meters

[COORDINATES]
;Node X-Coord Y-Coord
;-----
OP4 381364.593 5032973.357
OP7 381346.502 5032963.916
STUB15 381335.776 5032954.363
STUB16 381319.979 5032958.277
CB110 381293.102 5033002.513
CB111 381278.111 5032971.6
CB112 381219.543 5032961.536
CB113 381348.344 5032974.064
CB114 381332.566 5033036.058
CB115 381332.827 5033018.906
CB116 381338.815 5033005.189
CB117 381344.774 5032991.539
CB118 381353.473 5032992.996
Roof_1 381361.164 5033013.273
Roof_2 381361.335 5033013.684
St_UnGrd 381329.806 5032972.84

[VERTICES]
;Link X-Coord Y-Coord
;-----
W2 381322.829 5032965.271
W3 381343.656 5032974.514
W5 381295.919 5033002.386
W6 381309.233 5032988.935
W5 381320.071 5032965.768
OL6 381323.041 5033005.693

[POLYGONS]
;Subcatchment X-Coord Y-Coord
;-----
S1 381384.215 5032984.464
S1 381347.856 5032976.123
S1 381337.733 5032971.705
S1 381341.438 5032971.028
S1 381341.864 5032982.351
S1 381344.215 5032984.464
S10 381360.442 5032986.777
S10 381355.738 5032997.554
S10 381357.523 5032998.333
S10 381358.678 5032998.837
S10 381359.142 5032977.774
S10 381360.264 5033000.253
S10 381368.1 5033003.256
S10 381374.019 5033005.534
S10 381375.365 5033002.452
S10 381360.442 5032986.777
S10 381361.482 5033062.982
S11 381341.862 5033062.839
S11 381348.903 5033062.839
S11 381351.166 5033062.837
S11 381352.72 5033054.328
S11 381336.223 5033047.127
S11 381340.298 5033037.792
S11 381338.894 5033036.045
S11 381327.521 5033031.08
S11 381338.697 5033035.259
S11 381341.066 5033040.17
S11 381328.068 5033049.243
S11 381327.155 5033052.4
S11 381330.052 5033055.329
S11 381335.192 5033057.864
S11 381336.099 5033058.311
S11 381336.581 5033058.572
S11 381337.055 5033059.168
S11 381337.621 5033059.137
S11 381337.977 5033059.441
S11 381338.424 5033059.758
S11 381338.861 5033060.089
S11 381341.862 5033062.982
S2 381339.051 5032968.685
S2 381337.931 5032971.251


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S2 381337.733 5032971.705
S2 381347.856 5032976.123
S2 381352.652 5032978.217
S2 381352.769 5032978.266
S2 381352.887 5032978.313
S2 381353.006 5032978.356
S2 381352.331 5032977.428
S2 381352.51 5032977.428
S2 381350.662 5032973.753
S2 381339.051 5032968.685
S3 381318.259 5032957.972
S3 381307.625 5032982.332
S3 381303.659 5032986.085
S3 381310.51 5032889.076
S3 381311.335 5032989.076
S3 381337.931 5032971.251
S3 381339.051 5032968.685
S3 381319.267 5032960.048
S3 381318.259 5032957.972
S4 381303.659 5032986.085
S4 381285.414 5033003.349
S4 381311.368 5032989.076
S4 381311.368 5033017.747
S4 381300.135 5033012.843
S4 381310.51 5032989.076
S4 381303.659 5032986.085
S5 381316.631 5033020.045
S5 381320.706 5033010.711
S5 381311.62 5033007.477
S5 381319.552 5033007.414
S5 381321.746 5033008.328
S5 381327.851 5032994.342
S5 381334.493 5032979.128
S5 381337.733 5032971.705
S5 381337.931 5032971.251
S5 381311.368 5032989.076
S5 381310.51 5032989.076
S5 381300.135 5033012.843
S5 381311.368 5033017.747
S5 381316.631 5033020.045
S6 381327.521 5033031.08
S6 381338.894 5033036.045
S6 381324.298 5033036.752
S6 381342.122 5033008.328
S6 381343.432 5033036.323
S6 381341.338 5033035.409
S6 381349.093 5033017.644
S6 381343.736 5033015.306
S6 381335.821 5033012.047
S6 381325.803 5033009.33
S6 381325.57 5033009.866
S6 381321.746 5033008.328
S6 381319.652 5033007.414
S6 381318.612 5033009.797
S6 381320.706 5033010.711
S6 381316.631 5033020.045
S6 381311.368 5033017.747
S6 381304.779 5033024.146
S6 381307.751 5033027.787
S6 381319.536 5033040.417
S6 381325.697 5033035.259
S6 381327.521 5033031.08
S7 381354.292 5033035.735
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S7 381333.208 5032996.68
S7 381327.851 5032994.342
S7 381321.746 5033008.328
S7 381325.27 5033009.866
S7 381326.803 5033010.853
S7 381330.811 5033017.77
S7 381349.393 5033017.644
S7 381354.292 5033005.735
S8 381336.223 5033047.127
S8 381352.72 5033054.328
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S8 381368.8 5033003.256
S8 381363.264 5033032.193
S8 381349.442 5033091.744
S8 381358.678 5032998.837
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S8 381355.797 5033002.286
S8 381341.338 5033035.409
S8 381343.432 5033036.323
S8 381340.392 5033039.466
S8 381340.598 5033037.792
S8 381336.223 5033047.127
S9 381360.442 5032986.777
S9 381352.823 5032978.774
S9 381353.006 5032978.356
S9 381352.846 5032978.297
S9 381352.688 5032996.233
S9 381352.556 5032978.535
S9 381344.215 5032984.464
S9 381341.866 5032982.347
S9 381334.493 5032979.128
S9 381327.851 5032994.342
S9 381333.208 5032996.68
S9 381349.118 5033003.777
S9 381354.292 5033005.735
S9 381357.523 5032998.333
S9 381355.738 5032997.554
S9 381360.442 5032986.777

;;Storage Node X-Coord Y-Coord

[SYMBOLS]
;;Gage X-Coord Y-Coord

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Element Count

Number of rain gages 1
Number of subcatchments 11
Number of nodes 16
Number of links 17
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
Rainfall	24SCS100	INTENSITY	15 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
S1	0.01	6.10	100.00	15.0000	Rainfall	St_UnGrd
S10	0.01	29.00	8.79	1.0000	Rainfall	CB118
S11	0.05	70.00	18.80	2.0000	Rainfall	CB114
S2	0.00	4.30	100.00	1.9000	Rainfall	CB113
S3	0.02	75.00	17.50	1.0000	Rainfall	CB112
S4	0.04	50.00	8.51	3.5000	Rainfall	CB110
S5	0.10	105.00	100.00	1.0000	Rainfall	Roof_1
S6	0.08	45.00	66.78	2.5000	Rainfall	CB115
S7	0.04	28.60	90.42	2.7000	Rainfall	CB116
S8	0.10	105.00	100.00	1.0000	Rainfall	Roof_2
S9	0.05	50.00	93.57	2.4000	Rainfall	CB117

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF4	OUTFALL	80.63	0.00	0.0	
OF7	OUTFALL	80.63	0.00	0.0	
STUB15	OUTFALL	80.63	0.00	0.0	
STUB16	OUTFALL	80.65	0.00	0.0	
CB110	STORAGE	83.75	1.90	0.0	
CB111	STORAGE	83.59	2.01	0.0	
CB112	STORAGE	83.28	2.12	0.0	
CB113	STORAGE	83.45	1.70	0.0	
CB114	STORAGE	85.30	0.25	0.0	
CB115	STORAGE	85.25	0.31	0.0	
CB116	STORAGE	85.20	0.40	0.0	
CB117	STORAGE	85.15	0.15	0.0	
CB118	STORAGE	85.22	0.18	0.0	
Roof_1	STORAGE	96.35	0.15	0.0	
Roof_2	STORAGE	96.35	0.15	0.0	
St_UnGrd	STORAGE	81.60	1.00	Yes	

Link Summary

Name	From Node	To Node	Type	Length	%Slope
Roughness					

-

C1	CB111	CB112	CONDUIT	28.9	0.8647
0.0130	CB113	CB112	CONDUIT	31.3	0.6067
0.0130	CB110	CB111	CONDUIT	20.5	0.7815
0.0130	W1	CB115	CB116	WEIR	
	CB116	CB117	WEIR		
	W2	CB112	CB113	WEIR	
	W3	CB114	CB115	WEIR	
	W4	CB110	CB112	WEIR	
	W5	CB112	STUB16	OUTLET	
	C2	CB111	St_UnGrd	OUTLET	
	O1	CB115	St_UnGrd	OUTLET	
	OL2	CB116	St_UnGrd	OUTLET	
	OL3	CB117	St_UnGrd	OUTLET	
	OL4	Roof_1	STUB15	OUTLET	
	OL5	Roof_2	OF4	OUTLET	
	OL6	CB114	St_UnGrd	OUTLET	
	OL7	St_UnGrd	OF7	OUTLET	
	OL8	CB118	St_UnGrd	OUTLET	

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.15	0.02	0.04	0.15	1	0.01
C3	CIRCULAR	0.15	0.02	0.04	0.15	1	0.01
C5	CIRCULAR	0.15	0.02	0.04	0.15	1	0.01

Analysis Options

Flow Units CMS
Process Models:
Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed NO
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 01/01/2000 00:00:00
Ending Date 01/02/2000 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet/Dry Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 5.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001500 m

Runoff Quantity Continuity hectare-m

Total Precipitation	0.052	103.200
Evaporation Loss	0.000	0.000
Infiltration Loss	0.012	23.698
Surface Runoff	0.040	78.367
Final Storage	0.001	1.200
Continuity Error (%)	-0.062	

Flow Routing Continuity hectare-m

Volume	Volume	Volume
10^-6 ltr	hectare-m	mm

Post-Development 24-hour SCS 1:100 year Event

October 2024

Dry Weather Inflow	0.000	0.000	CB115	STORAGE	0.024	0.024	0	12:00	0.0619	0.0619	-
Wet Weather Inflow	0.040	0.398	CB116	STORAGE	0.013	0.013	0	12:00	0.0393	0.0393	-
Groundwater Inflow	0.000	0.000	CB117	STORAGE	0.017	0.017	0	12:00	0.0527	0.0527	-
RDII Inflow	0.000	0.000	0.001								
External Inflow	0.000	0.005	CB118	STORAGE	0.004	0.004	0	12:00	0.00407	0.00407	-
External Outflow	0.000	0.402	Roof_1	STORAGE	0.030	0.030	0	11:58	0.0967	0.0967	-
Flooding Loss	0.000	0.000	Roof_2	STORAGE	0.031	0.031	0	12:00	0.098	0.098	-
Evaporation Loss	0.000	0.000	0.000								
Exfiltration Loss	0.000	0.000	St_UnGrd	STORAGE	0.003	0.075	0	12:00	0.014	0.187	0.001
Initial Stored Volume	0.000	0.000									
Final Stored Volume	0.000	0.000									
Continuity Error (%)	-0.008										

Time-Step Critical Elements											

None											

Highest Flow Instability Indexes											

All links are stable.											

Most Frequent Nonconverging Nodes											

Convergence obtained at all time steps.											

Routing Time Step Summary											

Minimum Time Step	0.34 sec		Average Time Step	5.00 sec		Maximum Time Step	5.00 sec		Maximum	Average	Evap
% of Time in Steady State	0.00		Average Iterations per Step	2.00		% of Steps Not Converging	0.00		Volume	Pcnt	Exfil
Time Step Frequencies									1000 m³	Full	Maximum
5.000 - 1.500 sec	100.00 %		3.155 - 1.991 sec	0.00 %		1.991 - 1.256 sec	0.00 %		CB110	0.000	0.0
1.256 - 0.792 sec	0.00 %		0.792 - 0.500 sec	0.00 %		0.500 - 0.250 sec	0.00 %		CB111	0.000	1.0
0.500 - 0.250 sec			0.250 - 0.125 sec	0.00 %		0.125 - 0.062 sec	0.00 %		CB112	0.000	0.4
0.062 - 0.031 sec			0.031 - 0.016 sec	0.00 %		0.016 - 0.008 sec	0.00 %		CB113	0.000	0.5
0.008 - 0.004 sec			0.004 - 0.002 sec	0.00 %		0.002 - 0.001 sec	0.00 %		CB114	0.000	0.0
0.001 - 0.000 sec			0.000 - 0.000 sec	0.00 %					CB115	0.000	0.0

Subcatchment Runoff Summary											

Total Runoff	Total Runoff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv	Perv				
Runoff Subcatchment mm	Runoff mm	Runoff Coeff	Runon mm	Runon mm	Runoff mm	Runoff mm	Runoff mm				
mm 10^6 ltr	CMS										
S1	103.20	0.00	0.00	0.00	101.64	0.00					
101.64	0.01	0.00	0.985								
S10	103.20	0.00	0.00	73.59	8.93	29.50					
29.50	0.00	0.00	0.28								
S11	103.20	0.00	0.00	71.51	19.11	31.43					
31.43	0.01	0.01	0.305								
S2	103.20	0.00	0.00	75.86	0.00	27.37					
27.37	0.00	0.00	0.065								
S3	103.20	0.00	0.00	71.52	18.09	31.44					
31.44	0.01	0.01	0.305								
S4	103.20	0.00	0.00	73.61	8.65	29.49					
29.49	0.01	0.01	0.286								
S5	103.20	0.00	0.00	25.01	67.84	9.29					
101.57	0.10	0.03	0.884								
77.13	0.06	0.02	0.347								
S7	103.20	0.00	0.00	7.17	91.85	2.73					
94.58	0.04	0.01	0.916								
S8	103.20	0.00	0.00	0.00	101.57	0.00					
101.57	0.10	0.03	0.984								
S9	103.20	0.00	0.00	4.81	95.07	1.84					
96.90	0.05	0.02	0.939								

Node Depth Summary											

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters					
OF4	OUTFALL	0.48	0.48	81.11	0 00:00	0.48					
OF7	OUTFALL	0.48	0.48	81.11	0 00:00	0.48					
STUB15	OUTFALL	0.48	0.48	81.11	0 00:00	0.48					
STUB16	OUTFALL	0.63	0.63	81.28	0 00:00	0.63					
CB110	STORAGE	0.02	1.27	85.02	0 12:01	1.27					
CB111	STORAGE	0.02	1.39	84.98	0 12:02	1.39					
CB112	STORAGE	0.03	1.64	84.92	0 12:02	1.64					
CB113	STORAGE	0.02	1.37	84.92	0 12:02	1.37					
CB114	STORAGE	0.00	0.03	85.33	0 12:00	0.03					
CB115	STORAGE	0.00	0.03	85.29	0 12:00	0.04					
CB116	STORAGE	0.00	0.03	85.23	0 12:00	0.03					
CB117	STORAGE	0.00	0.03	85.18	0 12:00	0.03					
CB118	STORAGE	0.00	0.01	85.23	0 12:00	0.01					
Roof_1	STORAGE	0.01	0.06	96.41	0 13:00	0.06					
Roof_2	STORAGE	0.02	0.06	96.41	0 13:00	0.06					
St_UnGrd	STORAGE	0.01	0.78	82.58	0 12:02	0.78					

Node Inflow Summary											

Flow		Maximum Lateral	Maximum Total	Lateral Inflow	Time of Max Inflow	Total Inflow					
Balance		Lateral Inflow	Total Inflow	Inflow Occurrence	Volume	Volume					
Error Node Percent	Type	CMS	CMS	days hr:min	10^6 ltr	10^6 ltr					
OF4	OUTFALL	0.000	0.002	0 11:04	0	0.0979					
0.007	OUTFALL	0.000	0.029	0 11:45	0	0.187					
0.000	STUB15	0.000	0.002	0 11:04	0	0.0966					
0.000	STUB16	0.000	0.010	0 12:02	0	0.0208					
0.021	CB110	STORAGE	0.012	0.012	0 12:00	0.0127	0.0127	-			
0.022	CB111	STORAGE	0.000	0.010	0 12:00	0	0.0127				
0.148	CB112	STORAGE	0.006	0.014	0 12:00	0.00685	0.0216	-			
0.026	CB113	STORAGE	0.001	0.005	0 11:49	0.00012	0.0021				
0.001	CB114	STORAGE	0.014	0.014	0 12:00	0.0149	0.0149	-			

Conduit Surcharge Summary											

Conduit	Type	Adjusted Length	/Actual Length	Dry	Dry	Up	Down	Sub	Fraction of Time in Flow	Class	
C1	CONDUIT	0.008	0	12:00	0	0.50	0.55	1.00	0.00	0.01	0.00
C3	CONDUIT	0.004	0	11:49	0	0.28	0.35	1.00	0.00	0.01	0.00
C5	CONDUIT	0.010	0	11:49	0	0.82	0.76	1.00	0.00	0.01	0.00
W1	WEIR	0.000	0	00:00	0	0.00	0.00	0.00	0.00	0.00	0.00
W2	WEIR	0.000	0	00:00	0	0.00	0.00	0.00	0.00	0.00	0.00
W3	WEIR	0.000	0	00:00	0	0.00	0.00	0.00	0.00	0.00	0.00
W4	WEIR	0.000	0	00:00	0	0.00	0.00	0.00	0.00	0.00	0.00
W5	WEIR	0.000	0	00:00	0	0.00	0.00	0.00	0.00	0.00	0.00
C2	DUMMY	0.024	0	12:02	0	0.00	0.00	0.00	0.00	0.00	0.00
OL1	DUMMY	0.024	0	12:00	0	0.00	0.00	0.00	0.00	0.00	0.00
OL2	DUMMY	0.013	0	12:00	0	0.00	0.00	0.00	0.00	0.00	0.00
OL3	DUMMY	0.017	0	12:00	0	0.00	0.00	0.00	0.00	0.00	0.00

Engineering Specification

Job Name _____

Contractor _____

Job Location _____

Approval _____

Engineer _____

Contractor's P.O. No. _____

Approval _____

Representative _____

Tag _____

RD-200

Small Area Roof Drain

Specification

Watts RD-200 epoxy coated cast iron roof drain with flashing clamp with integral gravel stop, self-locking polyethylene dome (standard), and no hub (standard) outlet.

Pipe Sizing	
Suffix	Description
2	2"(51) Pipe Size
3	3"(76) Pipe Size
4	4"(102) Pipe Size (NH Only)



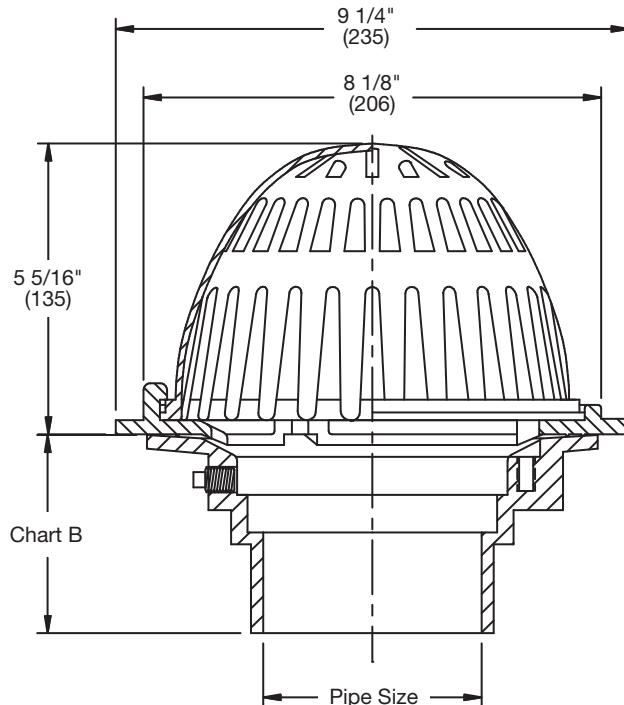
Outlet Type	
Suffix	Description
NH	No Hub (MJ)
P	Push On
T	Threaded
X	Inside Caulk

Options	
Suffix	Description
-13	Galvanized Body & Flashing Clamp
-B	Sump Receiver
-D	Underdeck Clamp
-F	Deck Flange/Adj. Extension
-GSS	Stainless Steel Ballast Guard
-K	Ductile Iron Dome
-K13	Galvanized Dome
-K80	Aluminum Dome
-K81	Rough Bronze Dome
-K83	SS Mesh Covered Dome
-L	Vandal Proof Dome
-R	2" External Water Dam
-SO	Side Outlet
-W	Adjustable Internal Water Dam

**Deck Opening 6 1/2"(165)
with Sump Receiver 8"(203)**

Optional Body Material	
Suffix	Description
-60	PVC Body w/Socket Outlet
-61	ABS Body w/Socket Outlet

Free Area	
Sq. In.	
35	



Pipe Size	Std. No Hub	P Push On	T Female Thread	X Inside Caulk	Chart B	
					60/61 PVC/ ABS	
2"(51)	3 5/8"(92)	4 1/4"(108)	4 1/4"(108)	4 1/2"(108)	4"(102)	
3"(76)	3 5/8"(92)	4 1/4"(108)	4 1/4"(108)	4 1/2"(108)	4"(102)	
4"(102)	3 5/8"(92)	4 1/4"(108)	4 1/4"(108)	4 1/2"(108)	4"(102)	

NOTICE

The information contained herein is not intended to replace the full product installation and safety information available or the experience of a trained product installer. You are required to thoroughly read all installation instructions and product safety information before beginning the installation of this product.

Watts product specifications in U.S. customary units and metric are approximate and are provided for reference only. For precise measurements, please contact Watts Technical Service. Watts reserves the right to change or modify product design, construction, specifications, or materials without prior notice and without incurring any obligation to make such changes and modifications on Watts products previously or subsequently sold.

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Adjustable Accutrol Weir
Tag: _____

**Adjustable Flow Control
for Roof Drains**

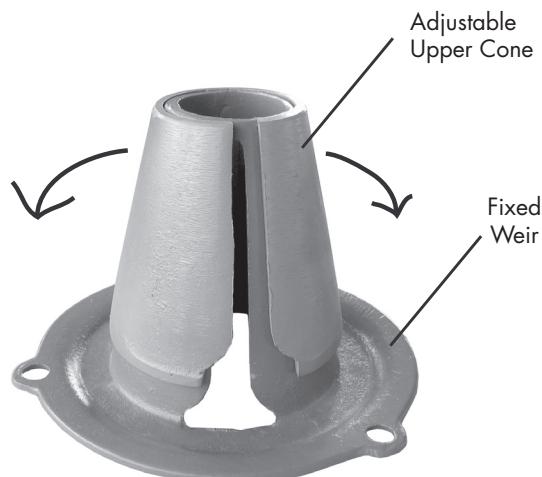
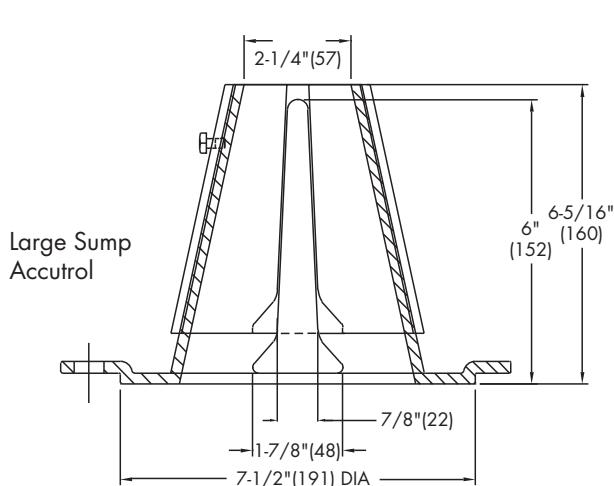
ADJUSTABLE ACCUTROL (for Large Sump Roof Drains only)

For more flexibility in controlling flow with heads deeper than 2", Watts Drainage offers the Adjustable Accutrol. The Adjustable Accutrol Weir is designed with a single parabolic opening that can be covered to restrict flow above 2" of head to less than 5 gpm per inch, up to 6" of head. To adjust the flow rate for depths over 2" of head, set the slot in the adjustable upper cone according to the flow rate required. Refer to Table 1 below.
Note: Flow rates are directly proportional to the amount of weir opening that is exposed.

EXAMPLE:

For example, if the adjustable upper cone is set to cover 1/2 of the weir opening, flow rates above 2" of head will be restricted to 2-1/2 gpm per inch of head.

Therefore, at 3" of head, the flow rate through the Accutrol Weir that has 1/2 the slot exposed will be:
[5 gpm (per inch of head) x 2 inches of head] + 2-1/2 gpm (for the third inch of head) = 12-1/2 gpm.



1/2 Weir Opening Exposed Shown Above

TABLE 1. Adjustable Accutrol Flow Rate Settings

Weir Opening Exposed	1"	2"	3"	4"	5"	6"
	Flow Rate (gallons per minute)					
Fully Exposed	5	10	15	20	25	30
3/4	5	10	13.75	17.5	21.25	25
1/2	5	10	12.5	15	17.5	20
1/4	5	10	11.25	12.5	13.75	15
Closed	5	5	5	5	5	5

Job Name _____

Contractor _____

Job Location _____

Contractor's P.O. No. _____

Engineer _____

Representative _____

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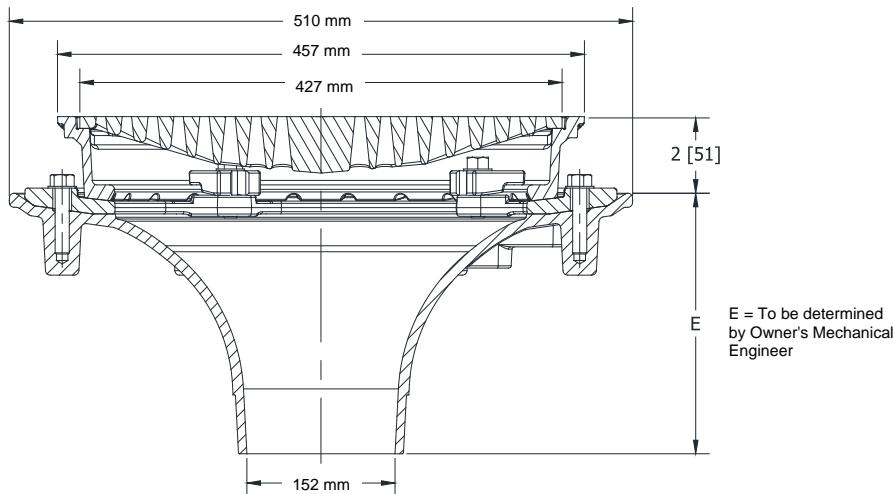
Z150F

FLOFORCE™ HIGH PERFORMANCE PROMENADE DECK
DRAIN WITH ROTATABLE FRAME AND HEEL-PROOF GRATE

SPECIFICATION SHEET

TAG

Design and Dimensional Data (inches and [mm]) are Subject to Manufacturing Tolerances and Change Without Notice



4 [102] No-Hub Illustrated

A Outlet Size In. [mm]	B Body Diameter In [mm]	C Frame Size In [mm]	D Grate Size In [mm]	Weight lbs [kg]	Grate Open Area Sq. In. [cm ²]
2,3,4 [51,76,102]	16-9/16 [421]	14 [356]	12-13/16 [325]	66 [30]	44 [284]
6,8 [152, 203]	20-3/32 [510]	18 [457]	16-13/16 [427]	94 [43]	70 [452]

ENGINEERING SPECIFICATION: ZURN Z150F

FLOFORCE™ High efficient flow performing roof drain for promenade deck roof drain applications. Drain incorporates a smooth funnel-shaped interior surface, providing a seamless transition to outlet connection and eliminating internal obstructions within the body. Complete with Dura-Coated cast iron body with membrane flashing clamp, rotatable square promenade frame, seepage openings, securing clamps, and heavy-duty ductile iron heel-proof grate.

OPTIONS (Check/specify appropriate options)**PIPE SIZE**

2, 3, 4 [51, 76, 102]

6, 8 [152, 203]

2, 3, 4 [51, 76, 102]

6 [152]

2, 3, 4 [51, 76, 102]

6, 8 [152, 203]

3, 4 [76, 102]

6 [152]

(Specify size/type) **OUTLET**

NH No-Hub

6-15/16 [176]

NH No-Hub

7-7/16 [189]

NL Neo-Loc

7-3/8 [187]

NL Neo-Loc

8-1/32 [204]

IP Threaded

5-15/16, 6-3/16, 6-5/16 [151, 157, 160]

IP Threaded

6-11/16, 6-3/4 [170, 171]

IC Inside Caulk

5-13/16 [148]

IC Inside Caulk

6-1/16 [154]

PREFIXES Z D.C.C.I. Body and Frame with Ductile Iron Grate* ZN D.C.C.I. Body and Frame with Ductile Iron Grate and Polished Nickel Bronze Veneer Finish**SUFFIXES** -AR Acid Resistant Epoxy Coated -R Roof Sump Receiver -C Underdeck Clamp -SC Secondary Clamp Collar -DP Top-Set® Deck Plate (Replaces both the -C and -R) -TC Neo-Loc Test Cap Gasket -DR Top-Set® Drain Riser (2, 3, 4 [51, 76, 102] NL Bottom Outlet Only) -E Static Extension 1 [25] thru 4 [102] (Specify Ht.) -VP Vandal-Proof Secured Top -EA Adjustable Extension Assembly -Y Type 304 [CF8] SS Sediment Bucket 2-1/8 [54] thru 3-1/2 [89] -85 Type 304 [CF8] Stainless Steel Perforated Extension -G Galvanized Cast Iron -89 2 [51] High Overflow Dam and Low-Profile Pedestal -PD Low-Profile Pedestal Paver Dome Paver Dome

* Regularly furnished unless otherwise specified

WARNING: Cancer and Reproductive Harm - www.P65Warnings.ca.gov**ADVERTENCIA:** Cáncer y daño reproductivo - www.P65Warnings.ca.gov**AVERTISSEMENT:** Cancer et effets néfastes sur la reproduction - www.P65Warnings.ca.gov

Site Servicing Report

2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario

Appendix E

Sanitary Servicing

JLR NO. 29899-002 (NAVAN BLOCK 15)																																					
Street Name	Pipe Reach		Commercial/Institutional							Infiltration			Peak Design Flow L/s	Pipe Data							Upstream Geometry				Downstream Geometry				Self Cleansing Velocities								
	From	To	Apartments	Pop.	Cum. Pop.	Peaking Factor	Residential Flow (L/s)	Area (ha)	Cum. Area (ha)	Peaking Factor	Inst. Flow (L/s)	Area (ha)	Cum. Area (ha)	Peak Extr. Flow L/s	Dia	Type	Actual Diameter	Slope	Q Full (L/s)	V Full	Length	Residual Capacity	% Full	TG From	Obvert	Invert	Cover	TG TO	Drop	Obvert	Invert	Cover	Q/Qf Ratio	Flow Depth (mm)	Actual Velocity ⁽⁷⁾ (m/s)	Flow Depth Dia. Ratio (d/D)	
BLOCK 15	BLOCK 15	14	83	149	149	3.55	1.72	0.09	0.09	1.50	0.05	0.57	0.57	0.19	1.95	200	Circular	203.20	1.50%	41.91	1.29	6.10	39.95	5%	85.850	82.467	82.264	3.383	85.840	0.060	82.376	82.173	3.464	0.05	29.67	0.66	0.15
EAST ORLEANS RIDGE SUBDIVISION		14	13		149	3.55	1.72		0.09	1.50	0.05		0.57	0.19	1.95	200	Circular	203.20	1.50%	41.91	1.29	13.50	39.95	5%	85.840	82.316	82.113	3.524	84.940		82.113	81.910	2.827	0.05	29.67	0.66	0.15

William Rugamba

From: William Rugamba
Sent: July 22, 2024 8:18 AM
To: William Rugamba
Subject: FW: Re-confirmation of Mechanical Items for Servicing Report

William Rugamba, M.Eng., B.A.Sc., EIT
Civil Engineering Graduate
Ottawa, ON
Work: [343-804-4374](tel:343-804-4374)

From: Sarith Lopez <slopez@qmeengineering.com>
Sent: Wednesday, July 17, 2024 8:07 PM
To: Mahad Musse <mmusse@jlrichards.ca>; Chuck Clark <CWC@qmeengineering.com>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>
Subject: RE: Re-confirmation of Mechanical Items for Servicing Report

[CAUTION] This email originated from outside JLR. Do not click links or open attachments unless you recognize the sender and know the content is safe. Do not forward suspicious emails, if you are unsure, please send a separate message to Helpdesk.

Hi Mahad,

All items below confirmed

Regards

Sarith López
Project Manager

9 Gurdwara Road, Unit 200
Ottawa, ON K2E 7X6
T: 613-366-4763 ext. 129
slopez@qmeengineering.com



From: Mahad Musse <mmusse@jlrichards.ca>
Sent: Tuesday, July 16, 2024 3:08 PM
To: Sarith Lopez <slopez@qmeengineering.com>; Chuck Clark <CWC@qmeengineering.com>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>
Subject: Re-confirmation of Mechanical Items for Servicing Report

Hi Sarith/Chuck,

Thank you for the information below and in our meetings. I understand all these items have been discussed before but we need to submit something as part of our report. Can you just re-confirm the following questions below and then we will attach your confirmation to the Report.

1. Please confirm that the sanitary service size of 200mm diameter for the Site Plan Blocks (Block 14, 15 and 17) is preferred by the mechanical engineer on file;
2. Please confirm that a sprinkler flow of 25 L/s can be assumed for the Site Plan Blocks (Block 14, 15 and 17) at this stage.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

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