

Cambium Reference: 17281-002

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Table of Contents

1.0	Introduction	1
1.1	Site Description	1
2.0	Physical Setting	3
2.1	Topography and Drainage	
2.2	Physiography	3
2.3	Overburden Geology	4
2.4	Bedrock Geology	4
2.5	Vulnerable and Regulated Areas	4
3.0	Subsurface Investigation	е
3.1	Borehole Logs	6
3.2	Physical Laboratory Testing	g
4.0	Groundwater Investigation	10
4.1	Well Inventory Survey	10
4.1.1	MECP Well Records Assessment	10
4.1.2	Door-to-Door Well Survey	11
4.2	Groundwater Quality	12
4.3	Single Well Hydraulic Tests (SWHTs)	14
5.0	Dewatering Assessment	16
5.1	Excavation Design Parameters	16
5.2	Estimated Dewatering Rate – Construction Phase	16
5.3	Estimated Dewatering Rate – Operational Phase	18
5.4	Assessment of Required Regulatory Permits or Registration	19
6.0	Water Balance Assessment	21
6.1	Water Budget and Total Water Surplus	22
6.2	Annual Infiltration and Runoff	23
6.3	Pre-Development Water Balance	24



November 11, 2024

6.4	Post-Development Water Balance	24
6.5	Water Balance Comparison	25
6.6	Required Infiltration from Roof Runoff	25
6.7	Water Balance Assessment Summary	26
7.0	Conceptual Wastewater Design	27
7.1	Concept Design Details	27
7.1.1	Septic Tank	27
7.1.2	Treatment Unit	27
7.1.3	Leaching Bed	28
8.0	Conclusions and Recommendations	29
9.0	Closing	32
10.0	References	33
11.0	Standard Limitations	35



Cambium Reference: 17281-002 November 11, 2024

List of Embedded Tables

Table 1	Summary of Measured Water Levels	8
Table 2	Monitoring Well Construction Details	
Table 3	Grain Size Distribution Analysis Results	g
Table 4	MECP Water Well Information Summary	10
Table 5	Summary of Results Exceeding PWQO Criteria	12
Table 6	Summary of Results Exceeding Storm Sewer By-law Criteria	13
Table 7	Summary of Results Exceeding Sanitary Sewer By-law Criteria	13
Table 8	Hydraulic Conductivity Estimates derived via SWHTs	15
Table 9	Calculated Construction Dewatering Rates	17
Table 10	Calculated Permanent Dewatering Rate	19
Table 11	Summary of Pre- and Post-Development Areas	22
Table 12	Determination of Infiltration Factor	23
Table 13	Pre-Development Water Balance	24
Table 14	Post-Development Water Balance	25
Table 15	Water Balance Comparison	25
Table 16	Requirement of Infiltration from Roof Runoff	26

List of Appended Figures

Figure 1	Site Location Plan
Figure 2	Site and Borehole Location Plan
Figure 3	MECP Well Records Within 500m
Figure 4	Pre-Development Plan
Figure 5	Post-Development Plan
Figure 6	Conceptual Sewage System Design

Cambium Inc. Page iii



November 11, 2024

List of Appendices

Appendix A Property and Land Information

Appendix B Borehole Logs

Appendix C Grain Size Analysis Results

Appendix D Well Inventory Survey Results

Appendix E Groundwater Quality Lab Results

Appendix F Single Well Hydraulic Test Results

Appendix G Dewatering Calculations

Appendix H Water Balance Calculations



November 11, 2024

1.0 Introduction

Cambium Inc. (Cambium) was retained by Cassidy EW Construction Consultant Ltd. (the Client) to complete a hydrogeological assessment and terrain analysis for the proposed redevelopment of the land located at 1386 and 1394 Greely Lane, Ottawa, Ontario (the Site).

The purpose of the field work and testing was to obtain information on the general subsurface and groundwater conditions at the Site by means of groundwater monitoring well measurements, as well as field and laboratory tests. This report addresses the hydrogeological aspects of the subsurface conditions at the Site. Cambium has also completed a Geotechnical Investigation (Cambium, 2023a) and a Phase Two Environmental Site Assessment (Cambium, 2023b) prior to the hydrogeological assessment and relevant details of these investigations have been incorporated into this report. Detailed information from the Geotechnical Investigation and the Phase Two Environmental Site Assessment were provided under separate cover.

This report provides the results of the hydrogeological assessment and should be read in conjunction with the "Standard Limitations" in Section 11.0, which forms an integral part of this document. The reader's attention is specifically drawn to this information, as it is essential for the proper use and interpretation of this report. The data, interpretations, and recommendations contained in this report pertain to a specific project as described in the report and are not applicable to any other project or site location. If the project is modified in concept, location, or elevation, or if the project is not initiated within eighteen months of the date of the report, Cambium should be given an opportunity to confirm that the recommendations in this report are still valid.

1.1 Site Description

The Site is an irregularly shaped 0.47 ha (1.15 acres) property that is developed for commercial use. It contains a single-storey commercial car wash building, two temporary seacan storage units, and an additional single storey metal storage building adjacent to the commercial building. A driveway connects to the adjacent Greely Lane at two locations on the



November 11, 2024

north side of the site. The remainder of the property is landscaped, with the southern portion of the Site predominantly occupied by a septic bed raised at a higher elevation than the grade. The Site is bound by Greely Lane to the east, Parkway Road to the south, and commercial/light industrial use to the north and west.

Based on discussions with the Client and preliminary site sketches provided to Cambium, it understood that the proposed plan is to construct one 1,110 m² (12,000 ft²) building for light industrial use which will be divided in three 370 m² (4,000 ft²) units with two loading bays, two washrooms, and an estimated 5 employees for each unit. The building will be constructed slab-on-grade with perimeter foundations that will extend to below the local frost penetration depths. The development will include at grade parking and driveways to access delivery doors at the backs of each building.

The proposed finished floor elevations (FFE) have not yet been determined; however, it is anticipated that the grades of the Site will not differ significantly from the current grades of the property, exclusive of the raised septic bed on the southern property. The grade there will be lowered as a result of removal of the septic bed.

The regional location of the Site is identified on Figure 1, the property and surrounding areas are outlined on Figure 2, and a Site plan is included in Appendix A.



November 11, 2024

2.0 Physical Setting

2.1 Topography and Drainage

Based on regional topographic maps the Site area is relatively flat with a gentle slope to the east-southeast towards the North Castor River. The Site has a raised septic bed located in the southern portion of the property with a topographic high of approximately 100 meters above sea level (masl).

The Site is located within the Castor River quaternary watershed and the North Castor River is located approximately 250 m south-southeast of the Site. North Castor River subsequently flows eastward into South Nation River, which is a tributary to Ottawa River.

Regionally, surface elevation decreases to the east toward Ottawa River. It is assumed that local drainage will follow the local surficial topography and flow towards the south-southeast ultimately discharging into the North Castor River. Based on the location of the nearest water bodies and topographic relief, the inferred that the regional groundwater flow direction is easterly.

2.2 Physiography

The Site is located in the physiographic region known as the Russell and Prescott Sand Plains (Chapman & Putnam, 1984). The Russell and Prescott Sand Plains region covers and area of approximately 1,490 km² extending from Ottawa to Hawkesbury. The Sand Plains are a relatively flat region with a clay valley located to the south, which was formed as a delta by the Ottawa River and tributaries of the Champlain Sea. The sand deposits have a thickness of 5 m to 10 m in the northern region of the plains and thin towards the clay plains of the south. The sand plains consist of coarser grained sands to the north grading into fine sand to silt in the south. The region is underlain by stratified red and grey clays (Appendix A).



November 11, 2024

2.3 Overburden Geology

According to Miscellaneous Release – Data 128 from the Ontario Geological Survey (2010) the predominant overburden of the Site consists of coarse-textured glaciomarine deposits (sand, gravel, minor silt and clay) (Appendix A).

2.4 Bedrock Geology

According to Miscellaneous Release – Data 219 from the Ontario Geological Survey (2007), the bedrock in the area of the Site consists of the Beekmantown Group. The Beekmantown Group consists of two formations: the March and Oxford Formations. The bedrock of the Site consists of the Oxford Formation and is described as dolostone, minor shale and sandstone (Appendix A).

2.5 Vulnerable and Regulated Areas

The Site is situated within the South Nation Source Protection Area, under jurisdiction of the South Nation Conservation Authority, as per the Source Water Protection Information Atlas (SPIA) from the Ministry of the Environment, Conservation and Parks (MECP) (2024a). The Site is within the following areas:

- Intake Protection Zone 3 (IPZ-3) with a vulnerability score of 7
- Significant Ground Water Recharge Area (SGRA) with a vulnerability score of N/A
- Highly Vulnerable Aquifer (HVA) with a vulnerability score of 6

IPZs are areas surrounding water courses and lakes which have surface water intakes for water supply. There is potential that contaminants spilled within IPZs may reach intakes more quickly than the ability to take appropriate action to shut down the intake should a spill occur. IPZ-3s are defined as event-based areas only. They are areas that can contribute contaminants under an extreme event (e.g., high winds or heavy rain) at a concentration that would result in deterioration of untreated source water. Best management practices should be used to minimize the potential for the release of chemicals to the environment during future operations at the Site.



November 11, 2024

SGRAs are landscape surfaces which allow a high volume of water to infiltrate into the ground. A recharge area is classified as significant if the recharge rate for a particular area is greater than the average watershed recharge rate by 15% or more and the area has a hydrological connection to a surface water body or to an aquifer that is a source of groundwater for a drinking water system (Ministry of the Environment, Conservation and Parks, 2021). SGRAs are delineated using models which consider topography, surficial soil, land cover and climate. The SGRA in the vicinity of the Site does not have a vulnerability score associated with it. Efforts should be made to maintain the Site pre-development water balance as much as practicable following redevelopment. Water balance information is presented in Section 6.0.

HVAs are aquifers that are more sensitive to contamination as a result of the proximity to surface (shallow aquifers). By default, all HVA's have a vulnerability score of 6. Best management practices should be used to minimize the potential for the release of chemicals to the subsurface environment during future operations at the Site.

A review of the Natural Heritage System database from the Ministry of Natural Resources and Forestry (2024) indicates the Site is not located within any Areas of Natural and Scientific Interest.

The Site does not fall under a regulated area, as per the South Nation Conservation Authority or O.Reg. 41/24.

The source protection, natural heritage, and conservation area mapping is attached in Appendix A.



November 11, 2024

3.0 Subsurface Investigation

Cambium staff completed a borehole investigation at the Site on March 7th to 8th, 2023, to assess subsurface conditions. A total of nine boreholes, designated as BH101-23 through BH109-23, were advanced at the Site to depths ranging from approximately 3.7 to 6.7 meters below ground surface (mbgs). Test pit locations are shown in Figure 3 and test pit logs are included in Appendix B.

3.1 Borehole Logs

Subsurface conditions generally consist of surficial deposits of pavements or topsoil overlying a relatively thin deposit of fill overlying native deposits of clays and silts.

A summary of general lithological details obtained from the investigation is presented below.

Topsoil

Topsoil was encountered from the surface of all boreholes with the exception BH101-23 and BH108-23. The thickness of the topsoil ranges from 0.10 to 0.91 m.

Asphaltic Concrete

Asphaltic concrete was encountered from the surface of BH101-23 and BH108-23 that were advanced in the existing paved areas. The thickness of the asphalt measures 0.08 and 0.05 m in BH101-23 and BH108-23, respectively.

Base Material

Pavement base material was encountered underlying the asphaltic concrete. The base material is composed of brown gravelly sand with some silt. The thickness of the material measures 380 and 560 mm in BH101-23 and BH108-23, respectively.

Fill Material

Fill material other than the pavement structure was encountered at all borehole locations. The fill material varies slightly in composition between borehole locations but is predominantly composed of silty sandy. The material ranges from trace gravel to gravelly, and trace clay was



Cambium Reference: 17281-002 November 11, 2024

noted in BH105-23 and BH107-23. Roots were noted within the fill material in BH102-23. The fill material varies in colour between brown and grey depending on location.

The thickness of the fill material ranges from 0.1 to 1.4 m and extends to depth ranging from 0.3 to 1.5 mbgs.

Clayey Silt

Native deposits of grey, sandy, clayey silt were encountered underlying the fill material at all borehole locations at depths ranging from 0.3 to 1.5 mbgs. A notable decrease in clay content was observed in BH103-23 and BH104-23 at a depth of 2.3 mbgs as the material transitions to the non-cohesive underlying deposits.

Boreholes BH108-23 and BH109-23 terminated within the clayey silt deposits at depths of 1.5 mbgs. The deposit was fully penetrated at all other borehole locations. The thickness of the deposits at these locations ranges from 0.9 to 2.3 m, and the deposits extend to depths ranging from 2.3 to 3.2 mbgs.

Silty Sand

A native deposit of grey silty sand was observed in BH101-23 underlying the clayey silt deposit at a depth of 2.6 mbgs. The deposit measures 0.5 m in thickness and extends to a depth of 3.1 mbgs. A seam similar in composition was noted in BH104-23 at a depth of 3.1 mbgs. The seam measured 0.10 m.

Silt

Native deposits of silt were encountered underlying the clayey silt and silty sand in boreholes BH101-23 through BH107-23. The deposit is grey in colour and contains some sand to sandy and trace clay.

The silt deposits were encountered at depths ranging from 2.3 to 3.2 mbgs. Where encountered, all boreholes terminated within the silt at depths ranging from 3.7 to 6.7 mbgs.



November 11, 2024

Groundwater

Groundwater was observed at all borehole locations during drilling. Unstabilized groundwater level measurements were recorded upon completion of drilling and monitoring wells were installed in three locations (BH105-23, BH106-23, and BH107-23) to enable further characterization. A subsequent monitoring event was completed as part of Phase II ESA work, as well as during hydraulic testing detailed later in this report (Section 4.2). As demonstrated in Table 1, there is significant variability in groundwater levels, which is expected within shallow unconfined aquifers.

Table 1 Summary of Measured Water Levels

Borehole ID	Water Level (mbgs)				
Dolellole ID	Post-drilling	March 15, 2023	April 19, 2024		
BH101-23	1.1	-	-		
BH102-23	1.5	-	-		
BH103-23	0.9	-	-		
BH104-23	0.6	-	-		
BH105-23	2.0	1.3	0.62		
BH106-23	1.5	0.9	0.30		
BH107-23	1.8	1.1	0.36		
BH108-23	0.8	-	-		
BH109-23	1.1	-	-		

Further well construction details for the three monitoring wells are provided in Table 2.

Table 2 Monitoring Well Construction Details

	Surface		Well Casing	Screen	Details
Well ID	Elevation (masl)	Well Depth (mbgs)	Stick-up (mags¹)	Top of Screen (mbgs)	Bottom of Screen (mbgs)
BH105-23	100.65	3.06	0.92	0.62	3.06
BH106-23	100.38	2.75	1.00	0.31	2.75
BH107-23	99.86	3.05	0.75	0.61	3.05

¹ meters above ground surface



November 11, 2024

All monitoring wells with water were developed after installation. Development involved purging ten well volumes of groundwater or three times dry from the wells by hand pumping with Waterra tubing and a foot valve.

3.2 Physical Laboratory Testing

Physical laboratory testing, including grain size distribution analysis, was completed on four soil samples to confirm textural classification identified during field logging and obtain percolation rate estimates. Analysis results are based on the Unified Soil Classification System (USCS) scale. A summary of results is provided in Table 3. Complete laboratory analysis reports are provided in Appendix C.

Table 3 Grain Size Distribution Analysis Results

Sample Location	Depth (mbgs)	Description	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	T-time (min/cm)
BH101- 23 SS3	1.5 to 2.1	Sandy Clayey Silt	0	22	57	21	40
BH101- 23 SS6	3.8 to 4.4	Silt some Sand trace Clay	0	19	77	4	20
BH104- 23 SS4	2.3 to 2.9	Sandy Silt some Clay	0	25	57	18	35
BH104- 23 SS6	3.8 to 4.4	Sandy Silt trace Clay	0	22	74	4	20

November 11, 2024

4.0 Groundwater Investigation

The results obtained for the water supply assessment are discussed in the following subsections.

4.1 Well Inventory Survey

4.1.1 MECP Well Records Assessment

Cambium accessed the MECP Water Well Information System (WWIS) to review water well records within 500 m of the Site (Ministry of the Environment, Conservation and Parks, 2024b). A total of 73 records were identified, 64 of which describe wells installed into bedrock and 9 installed into overburden. The records identified two monitoring/test wells, two abandoned wells, three recharge well and the remaining wells were either water supply wells or unknown use. The locations of wells records identified within 500 m of the Site are illustrated in Figure 3. A summary of water well information, including total depth, static water level, and recommended pumping rate, is presented in Table 4. Further details are provided Appendix D.

One well with well record ID 7448964 is identified to be present at the Site by the WWIS. No details are provided on the record, however.

Table 4 MECP Water Well Information Summary

		Depth (mbgs)	Depth Water Found (mbgs)	Static Water Level (mbgs)	Recommended Pumping Rate (L/min)
Bedrock	Minimum	10.67	9.75	1.00	18.00
Wells	Maximum	101.50	100.58	15.00	182.00
Count = 64	Average	32.18	27.79	4.41	55.90
Overburden	Minimum	4.88	13.11	4.00	23.00
Wells	Maximum	50.00	16.76	5.00	46.00
Count = 9	Average	15.90	14.66	4.23	38.26

A summary of other information outlined in the well records is provided below:



November 11, 2024

- The general lithology described by the well records is a sequence of overburden overlying limestone which is subsequently underlain by sandstone.
- The overburden is described as predominantly sand which is overlain by a clay layer in some locations. Gravel is also present at depth at some wells.
- The average contact depth between overburden materials and limestone bedrock is 16.5 mbgs (4.0 to 63.4 mbgs).
- Water supply in the area surrounding the Site is primarily derived from the bedrock aquifer.
 Based on the high static water level recorded compared to the depth that water was found, it is inferred that the bedrock aquifer is at least partially confined.
- The bedrock aquifer is productive, with a geometric mean recommended pumping rate of approximately 56 L/min for bedrock wells.

4.1.2 Door-to-Door Well Survey

A door-to-door survey of all accessible properties within 500 m of the property was conducted by Cambium staff on April 22nd, 2024, to confirm details in the public record and to identify any wells not included in the MECP records assessment. Due to the commercial and industrial development of the surrounding area, a number of properties were not accessible to the general public. Five properties were visited, and in-person interviews were conducted with available office workers regarding the condition and details of their water supply well(s), including the method of construction, water level, pump intake, well, and water level depths, water use, and general water quality and well yield.

If the property was accessible but a representative was not available, a letter was left in the mailbox with a pre-paid return envelope. The letter explained the nature of the proposed project and the survey and provided direct contact information for Cambium's project manager.

Details and responses from the well use survey are provided in Appendix D. Generally, workers indicated that the water supply for the surrounding area is not good quality due to hardness and suspect iron and sulphur.



November 11, 2024

4.2 Groundwater Quality

Groundwater quality samples were collected BH106-24 during hydraulic testing activities on April 19, 2024.

Samples were submitted for analysis of general organic and inorganic chemistry to Caduceon Environmental Laboratories in Ottawa, which is accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA). Samples were stored at a temperature between 0 °C and 10 °C prior to and during transport.

Water quality results were compared against Provincial Water Quality Objectives (PWQO) and City of Ottawa Sewer Discharge Bylaw 2003-514 guidelines. Certificates of Analysis for the samples are included in Appendix E. A summary of parameters exceeding the PWQO and Sewer By-law criteria is provided in Table 5, Table 6, and Table 7.

Table 5 Summary of Results Exceeding PWQO Criteria

		DWOO	BH10	06-23
Parameter	Units	PWQO Criteria	2024/04/22 (Total)	2024/08/10 (Dissolved)
Phosphorus	ug/L	10	8,720	<10
Arsenic	ug/L	5	27.5	1.0
Cadmium	ug/L	0.1	1.12	0.211
Cobalt	ug/L	0.9	103	1.1
Copper	ug/L	5	301	5.4
Lead	ug/L	1	76.8	0.08
Thallium	ug/L	0.3	1.82	<0.05
Uranium	ug/L	5	11.4	4.68
Vanadium	ug/L	6	327	0.3
Benzo[a]anthracene	ug/L	0.0004	<0.05*	-
Benzo(g,h,i)perylene	ug/L	0.00002	<0.05*	-
Butyl Benzyl Phthalate	ug/L	0.2	<1*	-
Chrysene	ug/L	0.0001	<0.05*	-
Dibenzo(a,h)anthracene	ug/L	0.002	<0.05*	-
Fluoranthene	ug/L	0.0008	<0.05*	-



November 11, 2024

		PWQO	BH106-23		
Parameter	Units	Criteria	2024/04/22 (Total)	2024/08/10 (Dissolved)	
Phenanthrene	ug/L	0.03	<0.05*	-	
Formaldehyde	ug/L	0.8	<8*	-	
Nonylphenols	ug/L	0.04	<1*	-	

Bolded numbers indicate exceedance with respect to applicable guideline value

Table 6 Summary of Results Exceeding Storm Sewer By-law Criteria

		Storm Sewer	BH106-23		
Parameter	Units	Criteria	2024/04/22 (Total)	2024/08/10 (Filtered/Dissolved)	
Total Suspended Solids	mg/L	15	9,480	<3	
Phosphorus	mg/L	0.4	8.72	<0.01	
Arsenic	mg/L	0.02	0.0275	0.001	
Chromium	mg/L	0.08	0.249	<0.0011	
Copper	mg/L	0.04	0.301	0.054	

Bolded numbers indicate exceedance with respect to applicable guideline value

Table 7 Summary of Results Exceeding Sanitary Sewer By-law Criteria

		Sanitary	BH106-23		
Parameter	Units	Sewer Criteria	2024/04/22 (Total)	2024/08/10 (Filtered/Dissolved)	
Total Suspended Solids	mg/L	350	9,480	<3	

Bolded numbers indicate exceedance with respect to applicable guideline value

Based on the results of the chemical analysis, the following comments on groundwater quality are made.

 Both the unfiltered and filtered samples had numerous parameters measured at concentrations in excess of PWQO criteria. Treatment of excavation water would be required prior to discharge to off-site surface receiving environments.

^{*} Laboratory Reporting Limit exceeds PWQO value



November 11, 2024

• The method detection limit concentrations for many total metals and semi-volatile organics were greater than some of the PWQO criteria for these parameters. This is a limitation of laboratory analysis and is not confirmation that the guideline value was exceeded.

- Total suspended solids (TSS), phosphorus, arsenic, chromium, and copper concentrations
 were above City of Ottawa Storm Sewer Discharge guidelines in the unfiltered sample. The
 filtered sample had concentrations less than guideline values for all parameters, indicating
 that filtration is a suitable treatment method to enable discharge to this receptor.
- The filtered water quality sample had concentrations less than City of Ottawa Sanitary
 Sewer Discharge guideline values for all parameters, indicating that filtration is a suitable treatment method to enable discharge to this receptor.
- It is recommended that a water quality sample of treated water be submitted for laboratory analysis prior to discharge during construction activities to confirm the treatment system adequately reduces elevated parameters to acceptable concentrations.

4.3 Single Well Hydraulic Tests (SWHTs)

Cambium staff visited the Site on April 19th, 2024, to perform in-situ single well hydraulic tests (SWHTs) on select monitoring wells.

Rising head tests were conducted in each well by inducing an instantaneous change in head (water level) in the monitoring wells. Water level changes were achieved by introducing/removing a solid slug.

Water level recovery was monitored using a Solinst Levelogger pressure transducer data logger, with manual measurements collected simultaneously at regular intervals.

The hydraulic conductivity of the geological formations adjacent to the screened portion of each well was estimated via the AquiferTest Pro software using the Hvorslev method (Hvorslev, 1951). A summary of results is presented in Table 8. Detailed analytical reports are provided in Appendix F.



Cambium Reference: 17281-002 November 11, 2024

Table 8 Hydraulic Conductivity Estimates derived via SWHTs

Monitoring	Screened	Hydraulic Conductivity, K (m/s)				
Well	Lithology	Test 1	Test 2	Test 3	Geometric Mean	
BH105-24	Silty sand to Sandy clayey silt	6.4 x 10 ⁻⁹	3.4 x 10 ⁻⁹	-	4.6 x 10 ⁻⁹	
BH106-24	Sandy clayey silt	2.2 x 10 ⁻⁷	1.9 x 10 ⁻⁷	2.1 x10 ⁻⁷	2.1 x 10 ⁻⁷	
BH107-24	Sandy clayey silt to silt	1.9 x 10 ⁻⁹	-	-	1.9 x 10 ⁻⁹	
Geometric Mean					1.2 x 10 ⁻⁸	

Estimated hydraulic conductivities for the tested wells screened within the silty clay unit ranged between 1.9 x10⁻⁹ and 2.2 x10⁻⁷ m/s, with an overall geometric mean value of 1.2 x10⁻⁸ m/s. These values are consistent with published values for the tested materials (unconsolidated silt) (Freeze & Cherry, 1979).



Cambium Reference: 17281-002

November 11, 2024

5.0 **Dewatering Assessment**

The requirements for construction dewatering generally depend on the Site's soil and groundwater conditions including soil type, soil permeability or hydraulic conductivity, local groundwater levels, and the design of the proposed works, such as the foundation/basement elevation or pipe invert level, as well as the size of proposed structure/excavation. The following subsections detail the specific excavation parameters and anticipated dewatering rates for the Site.

5.1 Excavation Design Parameters

It is understood that the footprint of the proposed slab-on-grade building will be approximately 1,110 m².

For construction purposes, it is assumed that excavation for footings will occur along a linear perimeter with dimensions of 23 m by 55 m. It is further assumed that during footing emplacement, groundwater will be temporarily lowered to a minimum of 1 m below the frost line to ensure dry conditions during footing construction, to a total depth of 2.5 mbgs.

For permanent operations, due to the high-water levels at the Site, permanent dewatering will be required to ensure water levels beneath the building remain below the frost line level (approximately 1.5 mbgs) throughout the year. A maximum water level of 0.30 mbgs was measured in BH106-23 on April 19, 2024.

5.2 Estimated Dewatering Rate – Construction Phase

An estimated dewatering rate for the construction phase of the proposed development was calculated a modified Dupuit-Forchheimer equation developed for linear excavations according to Powers, Corwin, Schmall, & Kaeck (2007):

$$Q = \frac{\pi K(H^2 - h^2)}{\ln(R_0/r_s)} + 2\left[\frac{xK(H^2 - h^2)}{2L}\right]$$

Cambium Reference: 17281-002 November 11, 2024

Where:

 $Q = dewatering rate (m^3/s)$

K = hydraulic conductivity (m/s)

H = initial hydraulic head in aquifer (m)

 $h = target \ hydraulic \ head \ (initial \ hydraulic \ head - target \ drawdown) \ (m)$

 $R_0 = distance to radial source (from excavation center)$

 $r_s = equivalent single well radius = width of trench/2 (m)$

x = unit length of trench (m)

 $L = distance to line source (from excavation center) = R_0/2 (m)$

A summary of calculated dewatering rates for per 50 m linear excavation, given a target depth to water of 2.5 mbgs, is provided in Table 9. Detailed calculations are provided in Appendix G.

Table 9 Calculated Construction Dewatering Rates

	Hydraulic Conductivity (K)	Radius of Influence (from excavation edge)	Dewatering Rate (Q)	
	m/s	m	m³/day	L/s
Minimum	1.9 x10 ⁻⁹	0.3	0.14	0.002
Maximum	2.1 x10 ⁻⁷	3.0	4.70	0.05
Geometric Mean	1.2 x10 ⁻⁸	0.7	0.65	0.01

Using the hydraulic conductivity estimates presented in Table 9, the estimated radius of influence from the edge of the excavation ranges from 0.3 to 3.0 m (average 0.7 m). The estimated dewatering rate ranges from 0.14 m³/day (140 L/day, or 0.002 L/s) to 4.70 m³/day (4,700 L/day, or 0.05 L/s), with a geometric mean average value of 0.65 m³/day (650 L/day, or 0.01 L/s).

Applying a safety factor of 2 to account for uncertainty resulting from heterogeneity of subsurface materials and other unknown factors, the estimated dewatering rate for 50 m sections of footing excavation ranges from 0.28 m³/day (280 L/day, or 0.004 L/s) to 9.4 m³/day

Cambium Reference: 17281-002 November 11, 2024

November 11, 2024

(9,400 L/day, or 0.10 L/s), with a geometric mean average value of 1.30 m³/day (1,300 L/day, or 0.02 L/s).

It is noted that the above equation is designed to represent steady state pumping conditions. In general, at the beginning of the pumping, the pumping rate required to lower Site water levels to acceptable levels may be greater than the rate estimated for steady state conditions as incoming water replaces the volume of excavated soils. Additionally, the above equation does not account for any precipitation that may occur during the construction process.

5.3 Estimated Dewatering Rate - Operational Phase

An estimated dewatering rate for the operational phase of the proposed development was calculated using a modified Dupuit-Forchheimer equation (Powers, Corwin, Schmall, & Kaeck, 2007). Calculations for a square dewatering area with an equivalent radius were employed.

$$Q = \frac{\pi K (H^2 - h^2)}{\ln(R_0 / r_{\rm s})}$$

Where:

 $Q = dewatering \ rate \ (m^3/s)$

K = hydraulic conductivity (m/s)

H = initial hydraulic head in aquifer (m)

 $h = target \ hydraulic \ head \ (initial \ hydraulic \ head - target \ drawdown) \ (m)$

 $R_0 = zone \ of \ influence \ (from \ excavation \ center) = 3000 (H - h) \sqrt{K} \ (m)$

 $r_s = equivalent single well radius$

For square excavations, the equivalent radius (r_s) can be determined as the radius of a circle with the same area as the excavation, or with the same perimeter as the excavation.

Here, the equivalent area method was used such that

$$r_s = \sqrt{\frac{excavation\ area}{\pi}}$$

A summary of calculated dewatering rates for per 50 m linear excavation, given a target depth to water of 2.5 mbgs, is provided in Table 10. Detailed calculations are provided in Appendix G



Cambium Reference: 17281-002 November 11, 2024

Table 10 Calculated Permanent Dewatering Rate

	Hydraulic Conductivity (K)	Radius of Influence (from excavation edge)	Dewatering Rate (Q)	
	m/s	m	m³/day	L/s
Minimum	1.9 x10 ⁻⁹	0.2	0.4	0.005
Maximum	2.1 x10 ⁻⁷	1.6	4.6	0.05
Geometric Mean	1.2 x10 ⁻⁸	0.4	1.1	0.01

Using the hydraulic conductivity estimates presented in Table 10, the estimated radius of influence from the edge of the building footprint ranges from 0.2 to 1.6 m (average 0.4 m). The estimated dewatering rate ranges from 0.4 m³/day (400 L/day, or 0.005 L/s) to 4.6 m³/day (4,600 L/day, or 0.05 L/s), with a geometric mean average value of 1.1 m³/day (1,100 L/day, or 0.01 L/s).

Applying a safety factor of 2 to account for uncertainty resulting from heterogeneity of subsurface materials and other unknown factors, the estimated permanent dewatering rate for the building footprint ranges from 0.8 m³/day (800 L/day, or 0.01 L/s) to 9.2 m³/day (9,200 L/day, or 0.10 L/s), with a geometric mean average value of 2.2 m³/day (2,200 L/day, or 0.02 L/s).

It is noted that the above calculations are an approximation only, which can be further refined based on results observed during the construction phase of the proposed development.

Cambium recommends reassessment of dewatering rates once construction nears the completion stage.

5.4 Assessment of Required Regulatory Permits or Registration

Any construction dewatering or other water taking in Ontario is governed by The Ontario Water Resources Act (Ontario Regulation 387/04 and/or Ontario Regulation 63/16) and/or the Environmental Protection Act (Registrations under Part II.2).



November 11, 2024

Where construction dewatering is required in amounts in excess of 400,000 L/day, a Permit to Take Water (PTTW) must be obtained. For temporary construction dewatering (six months or less) greater than 50,000 L/day but less than 400,000 L/day, registration through Environmental Activity and Sector Registry (EASR) is required. For dewatering rates less than 50,000 L/day, neither a PTTW or EASR registration is required.

As the maximum estimated dewatering rate for both construction activities and long-term building operation is less than 9,500 L/day, neither PTTW nor EASR registration will be required for the proposed development.



November 11, 2024

6.0 Water Balance Assessment

A water balance assessment was completed to determine the potential change in groundwater recharge that could occur due to the proposed development. Generally, any property can be categorized into three broad types of areas: paved, roof, and landscape/vegetated. Currently, the Site is undeveloped with all land landscape/vegetated. In the post-development scenario, the amount of paved and roof areas at the Site will usually increase and the amount of landscape/vegetated area will decrease. This has the potential to impact the amount of water that infiltrates into the ground and is available to replenish natural ground- and surface-water systems, which must be considered as part of the development process.

To compare the difference in infiltration that may result from the proposed development, a water balance calculation was completed to determine the amount of surplus water that is currently generated at the Site. Site characteristics such as surficial soil type, topography, and the amount of pervious and impervious areas were then used to estimate the volume of water infiltrating at the Site. Calculations were completed for both pre-and post-development scenarios, so that a comparison could be made to identify potential changes in infiltration as well as mitigation measures which could be employed to reduce development impacts.

Figure 5 presents the post-development plans of the proposed development. As a detailed breakdown of landscape and building details are yet to be determined, the paved, roof, and landscape areas for the developed lots were calculated based on an assumption that each surface type comprises 10%, 50%, and 40% of the total developed lot area, respectively. Table 11 provides a summary of statistics for the total areas for each type of surface at the Site for both pre- and post-development scenarios. Further discussion of each component completed for the water balance assessment is provided in the following subsections.

Cambium Reference: 17281-002 November 11, 2024

Table 11 Summary of Pre- and Post-Development Areas

Type of Land Coverage	Pre-Development Areas (m²)	Post-Development Areas (m²)	
Paved Area	811	2,246	
Roof Area	365	1,261	
Landscape/Vegetated Area	3,502	1,171	
Total (m ²)	4,678	4,678	

6.1 Water Budget and Total Water Surplus

Based on the Thornthwaite and Mather methodology (1957), the water balance is an accounting of water in the hydrologic cycle. Precipitation (P) falls as rain and snow. It can run off towards lakes and streams (R), infiltrate to the groundwater table (I), or evaporate from the ground or be used for transpiration by vegetation (ET). When long-term average values of P, R, I, and ET are used, there is minimal or no net change to groundwater storage (Δ S).

The annual water budget can be expressed as:

$$P = R + I + ET + \Delta S$$

Where:

P = Precipitation (mm/yr)

R = Run-off (mm/yr)

I = Infiltration (mm/yr)

ET = Evapotranspiration (mm/yr)

 ΔS = Change in soil water storage (mm/yr)

Total water surplus is defined as the difference between precipitation and evapotranspiration. It is the amount of water per unit area that can either infiltrate into on-site soils or be directed off-site as runoff. An assumption for the calculation of water surplus is that changes in soil water storage are negligible over the course of a year. It is also assumed that the catchment area for

Cambium Reference: 17281-002 November 11, 2024

the water balance described above is completely contained within Site boundaries (i.e. the model does not account for catchment areas that extend off-site).

An annual water budget for the Site was calculated using the thirty-year climate normal data (1981-2010) provided by Environment Canada for the Ottawa MacDonald-Cartier International Airport (Climate ID 6106000), located approximately 114 km north (Environment Canada, 2024). A detailed table outlining the calculations is provided in Appendix H. In summary, the average annual precipitation and evapotranspiration at the Site is estimated to be 944 mm/yr and 547 mm/yr, respectively. Therefore, the water surplus at the Site is estimated to be 397 mm/yr.

6.2 Annual Infiltration and Runoff

To determine the amount of water infiltrated into on-site soils annually, the total volume of water available is multiplied by an infiltration factor (IF). The total volume of water available is obtained by multiplying the water surplus value determined from the water balance described above by the total permeable landscape area at the Site. The infiltration factor, which ranges from 0 to 1, is estimated based on topography, soils and cover as per the Stormwater Management Planning and Design Manual (Ministry of the Environment, 2003). As outlined in Table 12, the infiltration factor at the Site was assigned a value of 0.6.

Table 12 Determination of Infiltration Factor

Factor	Value
Topography	Flat land, avg. slope < 0.6 m/km = 0.3
Soil	Silty Loam = 0.2
Cover	Cultivated Land = 0.1
Infiltration Factor (IF)	0.6

The annual volume of water that infiltrates at the site is calculated as follows:

 $I(m^3/yr) = Water Surplus(m/yr) * Total landscape area(m^2/yr) * Infiltration Factor$

The annual infiltration at the Site is expected to vary based on a number of factors (i.e. actual precipitation, variation in soil composition, soil compaction, etc.).



November 11, 2024

The annual runoff that occurs at the Site varies between permeable and impermeable surfaces. On permeable landscape surfaces, the runoff is calculated as the difference between total precipitation and annual infiltration. On impermeable surfaces where there is no infiltration, the runoff is calculated as 90% of precipitation, with the remaining 10% of precipitation lost directly to evaporation.

Annual infiltration and runoff volumes were calculated for the Site for both pre- and postdevelopment scenarios. Details of the calculations are provided in Appendix H. A discussion of the water balance used to calculate the infiltration and runoff volumes for each scenario is provided in Section 6.3 and Section 6.4.

6.3 Pre-Development Water Balance

The water balance for existing conditions at the Site is summarized in Table 13. The predevelopment infiltration rate and runoff rate was calculated to be 834 m³/yr and 1,555 m³/yr, respectively.

Table 13 Pre-Development Water Balance

Land Use		Area (m²)	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	Run- off (m³)
Impervious Areas	Paved Area	811	766	77	-	689
	Roof Area	365	345	34	-	310
Pervious Areas	Landscape Area	3,502	3,306	1,916	834	556
Total		4,678	4,416	2,027	834	1,555

6.4 Post-Development Water Balance

A comparison of water balances for the pre-development and post-development scenarios is summarized in Table 15. There is a net infiltration deficit of approximately 555 m³/yr, compared to the pre-development infiltration. The run-off rate upon development of the Site is projected to increase by 1,610 m³/yr.

Cambium Reference: 17281-002 November 11, 2024

Table 14 Post-Development Water Balance

<u> </u>						
Land Use		Area (m²)	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	Run- off (m³)
Impervious Areas	Paved Area	2,246	2,120	212	-	1,908
	Roof Area	1,261	1,190	119	-	1,071
Pervious Areas	Landscape Area	1,171	1,105	641	279	186
Total		4,678	4,416	972	279	3,166

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

6.5 Water Balance Comparison

A comparison of water balances for the pre-development and post-development scenarios is summarized in Table 15. There is a net infiltration deficit of approximately 555 m³/yr, compared to the pre-development infiltration. The run-off rate upon development of the Site is projected to increase by 1,610 m³/yr.

Table 15 Water Balance Comparison

	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	Run-off (m³)
Pre-Development	4,416	2,027	834	1,555
Post-Development	4,416	972	279	3,166
Change in Volume	-	-1,055	-555	1,610
Change in %	-	-52	-67	104

6.6 Required Infiltration from Roof Runoff

To compensate for the post-development infiltration deficit, a portion of roof run-off water can be captured and directed towards infiltration. As the infiltration deficit volume is 555 m³/yr and the total roof run-off volume is projected to be 1,071 m³/yr, the percentage of roof run-off that is required to be redirected to maintain pre-development infiltration volumes is 52%. These details are summarized in Table 16.



Cambium Reference: 17281-002 November 11, 2024

Table 16 Requirement of Infiltration from Roof Runoff

Volume of Pre-Development Infiltration (m³/yr)	834
Volume of Post-Development Infiltration (m³/yr)	279
Deficit from Pre to Post Development Infiltration (m³/yr)	555
Percentage of Roof Runoff required to match the pre-development infiltration (%)	52

6.7 Water Balance Assessment Summary

Based on the calculations detailed in the preceding subsections, a summary of the water balance assessment is as follows:

- Impervious post-development area (roof and pavement) is projected to increase by approximately 2,331 m² when compared to pre-development conditions.
- Without implementing any mitigation measures, it is estimated that the reduction of pervious surfaces at the Site will create a net deficit in infiltration of approximately 555 m³/yr.
- To regain the lost volume of water infiltrated, a diversion of approximately 52% of roof runoff would be required to maintain pre-development water balance conditions (assuming 100% of diverted water is infiltrated).
- Implementation of Low Impact Development measures would enhance the Site's ability to
 infiltrate diverted roof run-off water into pervious areas. Due to the high groundwater levels
 however, a civil design engineer should be involved in designing any suitable infiltration
 measures across the Site.

November 11, 2024

7.0 Conceptual Wastewater Design

Part 8 of the Ontario Building Code (OBC) details the design, construction, operation, and maintenance of sewage systems. A conceptual peak sewage design flow was calculated following a review of OBC Table 8.2.1.3.B is summarized as follows:

- Warehouse: 150 L/day/loading bay x 4 loading bays = 600 L/day
- Factory: 75 L/employee per 8 hr shift x 16 person occupancy = 1,200 L/day
 - Total sewage design flow = 1,800 L/day

7.1 Concept Design Details

A daily sewage design flow volume of 1,800 L/day is calculated for the proposed light industrial building.

7.1.1 Septic Tank

All fixtures within the proposed building should be directed to the inlet of the septic tank. Based on the design flow for non-residential occupancy, the proposed septic tank capacity was calculated as follows in accordance with section 8.2.2.3. of the OBC:

A single two compartment septic tank with capacity of 6,000 L would be suitable to achieve the minimal capacity requirements.

7.1.2 Treatment Unit

It is understood the client wishes to use a Premier Tech Aqua Ecoflo advanced treatment unit. The Ecoflo biofilter model proposed is the ECOFLO STB-650B rated for flows up to 2,500 L/day.



November 11, 2024

7.1.3 Leaching Bed

Following the subsurface investigation, native soils were observed to be similar, consisting of a surficial layer of topsoil and silty sand fill to depths ranging from 0.3 to 1.0 mbgs overlying sandy clayey silt and sandy silt. Groundwater was encountered between 0.6 and 2.0 mbgs across all boreholes. Soil sample results are summarized in Section 3.2 above and have estimated percolation rates between 20 and 40 min/cm.

Considering the available land constraints and using a conservative estimated percolation rate of 40 min/cm, a partially raised Type A area bed has been conceptually designed below using the following information and calculations:

- Design flow (Q) = 1,800 L/day
- Native Soil T-time (T) = 40 min/cm
- Configuration: partially raised
- Stone area = Q/75 when Q < 3,000 L/day = 1,800/75 = 24 m²
 - Proposed concept design: 5.6 m x 4.5 m = 25.2 m²
- Mantle area (imported sand fill) = $QT/400 = 1,800 \times 40 / 400 = 180 \text{ m}^2$
 - Proposed concept design: 21.6 m x 8.5 m = 183.6 m²

Based on the filter bed mantle requirement, the total bed footprint would be approximately 21.6 m by 8.5 m, as shown on Figure 6.

The Type A Area Bed will likely require to be raised above original grade. Assuming a raised height of 1.0 m, setback distances shown on Figure 6 were increased accordingly.

The area of the Site appears to provide adequate space for the installation of an on-site sewage system and appears to meet the required setback distances outlined in OBC Tables 8.2.1.6.A and 8.2.1.6.B. However, this should be considered and evaluated during the detailed sewage system design stage. The Site conditions appear feasible to install an on-site sewage system.



November 11, 2024

8.0 Conclusions and Recommendations

Cambium was retained by the Client to complete a hydrogeological assessment for proposed redevelopment of the land located at 1386 and 1394 Greely Lane, Ottawa, Ontario. Development plans include construction of one 1,110 m² (12,000 ft²) slab-on grade building which will be divided in three 370 m² (4,000 ft²) light industrial use units.

The subsurface investigation completed at the site indicates the lithology is comprised primarily of surficial deposits of pavements or topsoil overlying a relatively thin deposit of fill overlying native deposits of clays and silts. T-times estimated from laboratory analysis of soil samples collected from the native deposits range from 20 to 40 min/cm.

Monitoring wells installed in three locations (BH105-23, BH106-23, and BH107-23) indicate water levels vary across the site and fluctuate seasonally. A minimum water level of 1.3 mbgs was measured in BH105-23 on March 15, 2023, and a maximum water level of 0.30 mbgs was measured in BH106-23 on April 19, 2024. Hydraulic testing (rising head slug tests) provided hydraulic conductivity estimates for the shallow aquifer ranging from 1.9 x10⁻⁹ to 2.2 x10⁻⁷ m/s with a geometric mean estimate of 1.2 x10⁻⁸ m²/s.

Water Quality Analysis

Analysis of water quality samples from BH106-23 identified a number of parameters with concentrations exceeding PWQO criteria in both unfiltered and filtered samples. All parameters had concentrations below City of Ottawa storm and sanitary sewer discharge guidelines, indicating that filtration is a suitable treatment method to enable discharge to these receptors. Should on-site treatment and discharge to surface (i.e. drainage ditch) be the preferred option for dewatering, it is recommended that a water quality sample of treated water be submitted for laboratory analysis prior to discharge during construction activities to confirm the treatment system adequately reduces elevated parameters to acceptable concentrations.

Dewatering Assessment

Due to the high groundwater levels at the Site, dewatering during both the construction phase and permanent building operation will be required. During construction, it is estimated than an



November 11, 2024

average dewatering rate of 1.30 m³/day (1,300 L/day, or 0.02 L/s) will be needed to achieve dry conditions per 50 m section of footing excavation. This rate represents steady state pumping conditions and higher volumes may be required to lower Site water levels to acceptable levels during the initial stage of pumping. Additionally, the estimate does not account for any precipitation that may occur during the construction process.

For permanent operations, it is estimated that an estimated average dewatering rate of 2.2 m³/day (2,200 L/day, or 0.02 L/s) will be required to ensure water levels beneath the building remain below the frost line level (approximately 1.5 mbgs) throughout the year. It is recommended that dewatering rates be reassessed however, once building construction nears the completion stage.

The maximum estimated dewatering rate for both construction activities and long-term building operation are less than 9,500 L/day. As such, neither a PTTW nor an EASR registration will be required for the proposed development.

Water Balance

It is projected that impervious post-development area (roof and pavement) will increase by approximately 2,331 m² when compared to pre-development conditions, which will create a net deficit in infiltration to groundwater of approximately 555 m³/yr if no mitigation measures are implanted.

To regain the lost volume of water infiltrated, a diversion of approximately 52% of roof run-off would be required to maintain pre-development water balance conditions (assuming 100% of diverted water is infiltrated).

Implementation of Low Impact Development measures would enhance the Site's ability to infiltrate diverted roof run-off water into pervious areas. Due to the high groundwater levels however, a civil design engineer should be involved in designing any suitable infiltration measures across the Site.



November 11, 2024

Conceptual Wastewater Design

A daily sewage design flow volume of 1,800 L/day was calculated for the proposed light industrial building. Given the site lithology and estimated T-times, a total septic bed footprint of approximately 21.6 m by 8.5 m, with a 6,000 L septic tank and ECOFLO advanced treatment unit, will be required. The bed will be at least partially raised due to Site conditions, with the specific height to be determined during the final building design.

Overall, the Site conditions appear feasible to install an on-site sewage system, and there is adequate space for the installation which appears to meet the required OBC setback distances. However, this should be considered and evaluated during the detailed sewage system design stage.



Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario Cassidy EW Construction Consultant Ltd.

Cambium Reference: 17281-002

November 11, 2024

9.0 Closing

We trust that the information in this submission meets your current requirements. If you have any questions regarding the contents of this report, please contact the undersigned.

Respectfully submitted,

Cambium Inc.

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Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario Cassidy EW Construction Consultant Ltd.

Cambium Reference: 17281-002

November 11, 2024

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Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario Cassidy EW Construction Consultant Ltd.

Cambium Reference: 17281-002

November 11, 2024

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Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
November 11, 2024

11.0 Standard Limitations

Limited Warranty

In performing work on behalf of a client, Cambium relies on its client to provide instructions on the scope of its retainer and, on that basis, Cambium determines the precise nature of the work to be performed. Cambium undertakes all work in accordance with applicable accepted industry practices and standards. Unless required under local laws, other than as expressly stated herein, no other warranties or conditions, either expressed or implied, are made regarding the services, work or reports provided.

Reliance on Materials and Information

The findings and results presented in reports prepared by Cambium are based on the materials and information provided by the client to Cambium and on the facts, conditions and circumstances encountered by Cambium during the performance of the work requested by the client. In formulating its findings and results into a report, Cambium assumes that the information and materials provided by the client or obtained by Cambium from the client or otherwise are factual, accurate and represent a true depiction of the circumstances that exist. Cambium relies on its client to inform Cambium if there are changes to any such information and materials. Cambium does not review, analyze or attempt to verify the accuracy or completeness of the information or materials provided, or circumstances encountered, other than in accordance with applicable accepted industry practice. Cambium will not be responsible for matters arising from incomplete, incorrect or misleading information or from facts or circumstances that are not fully disclosed to or that are concealed from Cambium during the provision of services, work or reports.

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When preparing reports, Cambium considers applicable legislation, regulations, governmental guidelines and policies to the extent they are within its knowledge, but Cambium is not qualified to advise with respect to legal matters. The presentation of information regarding applicable legislation, regulations, governmental guidelines and policies is for information only and is not intended to and should not be interpreted as constituting a legal opinion concerning the work completed or conditions outlined in a report. All legal matters should be reviewed and considered by an appropriately qualified legal practitioner.

Site Assessments

A site assessment is created using data and information collected during the investigation of a site and based on conditions encountered at the time and particular locations at which fieldwork is conducted. The information, sample results and data collected represent the conditions only at the specific times at which and at those specific locations from which the information, samples and data were obtained and the information, sample results and data may vary at other locations and times. To the extent that Cambium's work or report considers any locations or times other than those from which information, sample results and data was specifically received, the work or report is based on a reasonable extrapolation from such information, sample results and data but the actual conditions encountered may vary from those extrapolations.

Only conditions at the site and locations chosen for study by the client are evaluated; no adjacent or other properties are evaluated unless specifically requested by the client. Any physical or other aspects of the site chosen for study by the client, or any other matter not specifically addressed in a report prepared by Cambium, are beyond the scope of the work performed by Cambium and such matters have not been investigated or addressed.

Reliance

Cambium's services, work and reports may be relied on by the client and its corporate directors and officers, employees, and professional advisors. Cambium is not responsible for the use of its work or reports by any other party, or for the reliance on, or for any decision which is made by any party using the services or work performed by or a report prepared by Cambium without Cambium's express written consent. Any party that relies on services or work performed by Cambium or a report prepared by Cambium without Cambium's express written consent, does so at its own risk. No report of Cambium may be disclosed or referred to in any public document without Cambium's express prior written consent. Cambium specifically disclaims any liability or responsibility to any such party for any loss, damage, expense, fine, penalty or other such thing which may arise or result from the use of any information, recommendation or other matter arising from the services, work or reports provided by Cambium.

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Potential liability to the client arising out of the report is limited to the amount of Cambium's professional liability insurance coverage. Cambium shall only be liable for direct damages to the extent caused by Cambium's negligence and/or breach of contract. Cambium shall not be liable for consequential damages.

Personal Liability

The client expressly agrees that Cambium employees shall have no personal liability to the client with respect to a claim, whether in contract, tort and/or other cause of action in law. Furthermore, the client agrees that it will bring no proceedings nor take any action in any court of law against Cambium employees in their personal capacity.

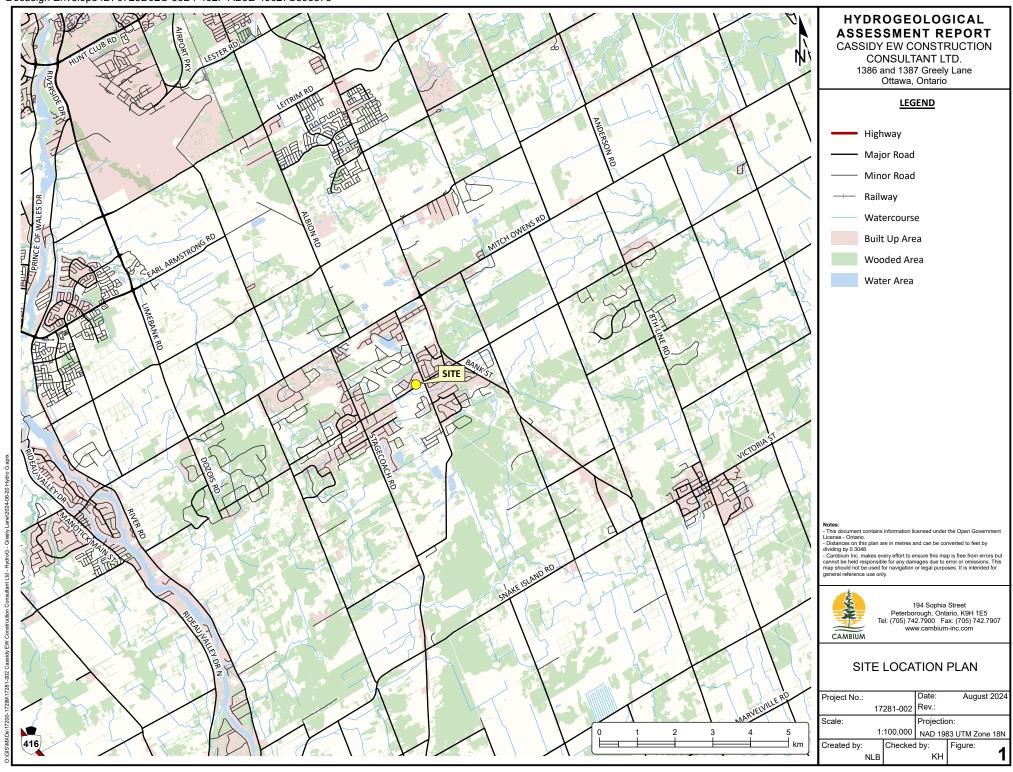


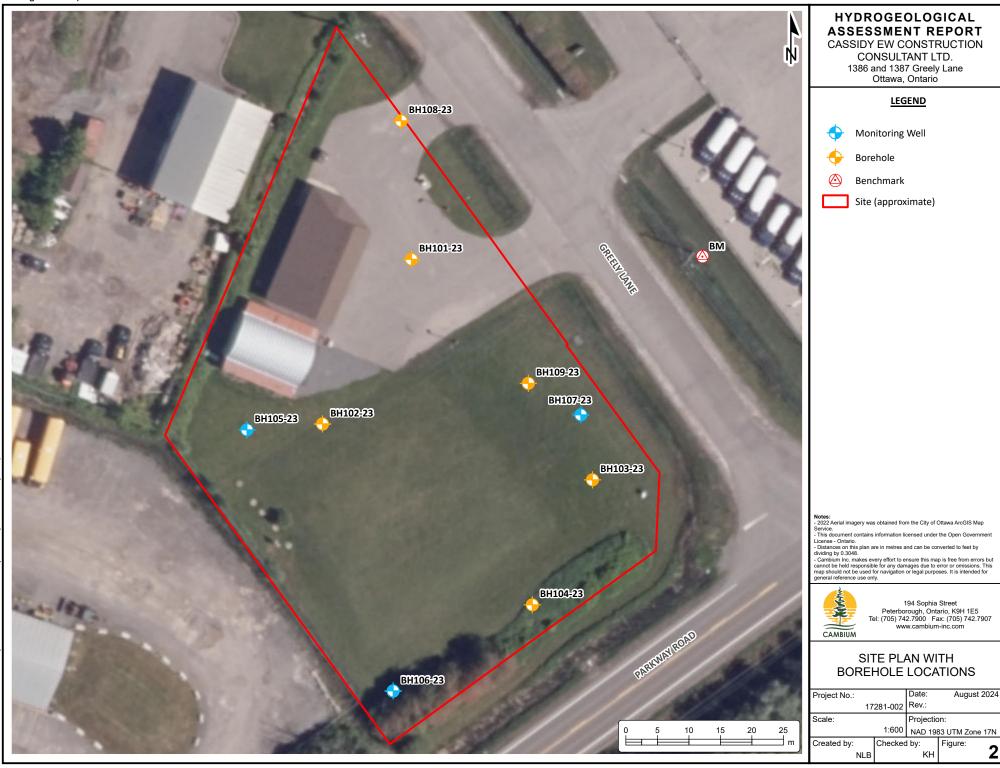
Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario Cassidy EW Construction Consultant Ltd.

Cambium Reference: 17281-002

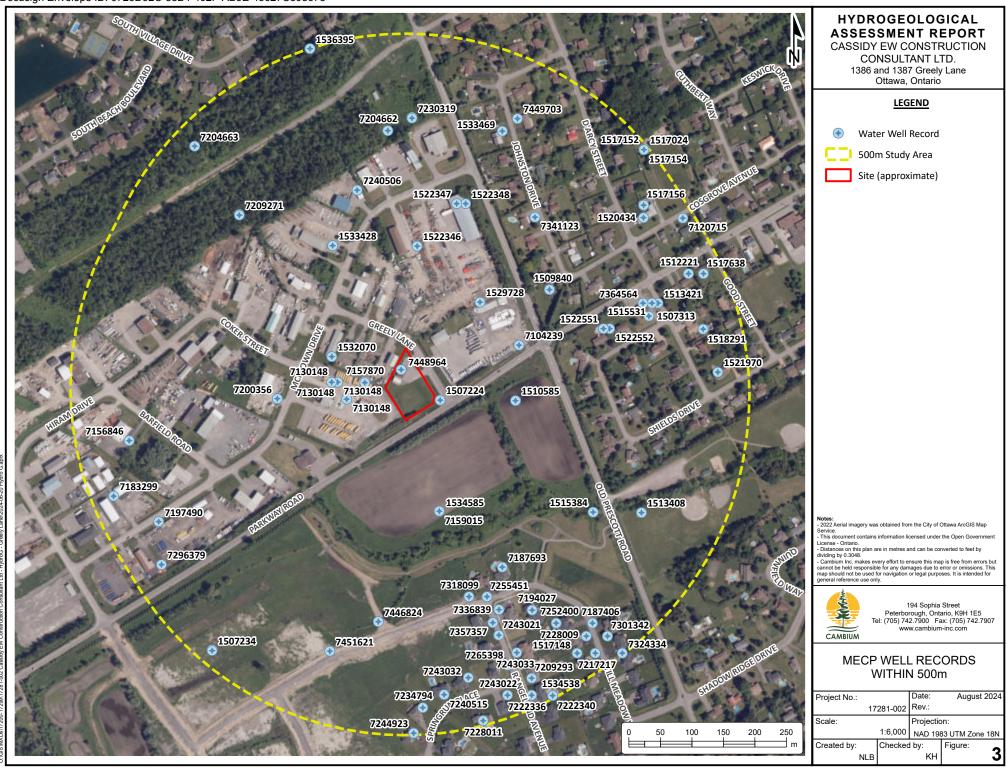
November 11, 2024

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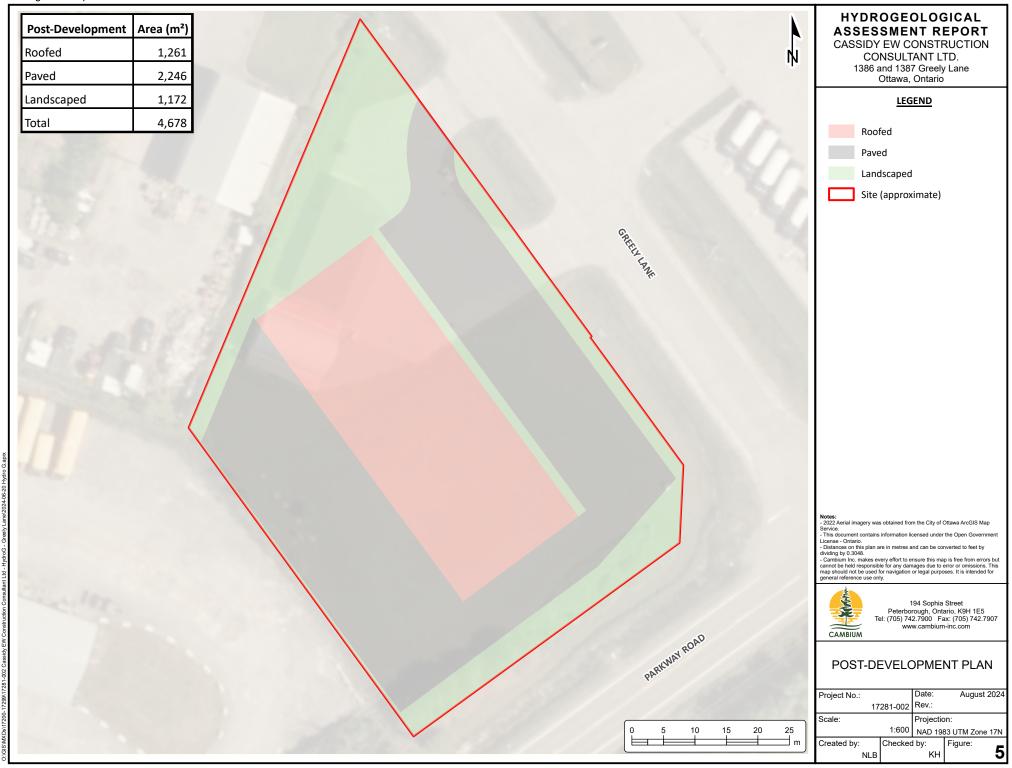


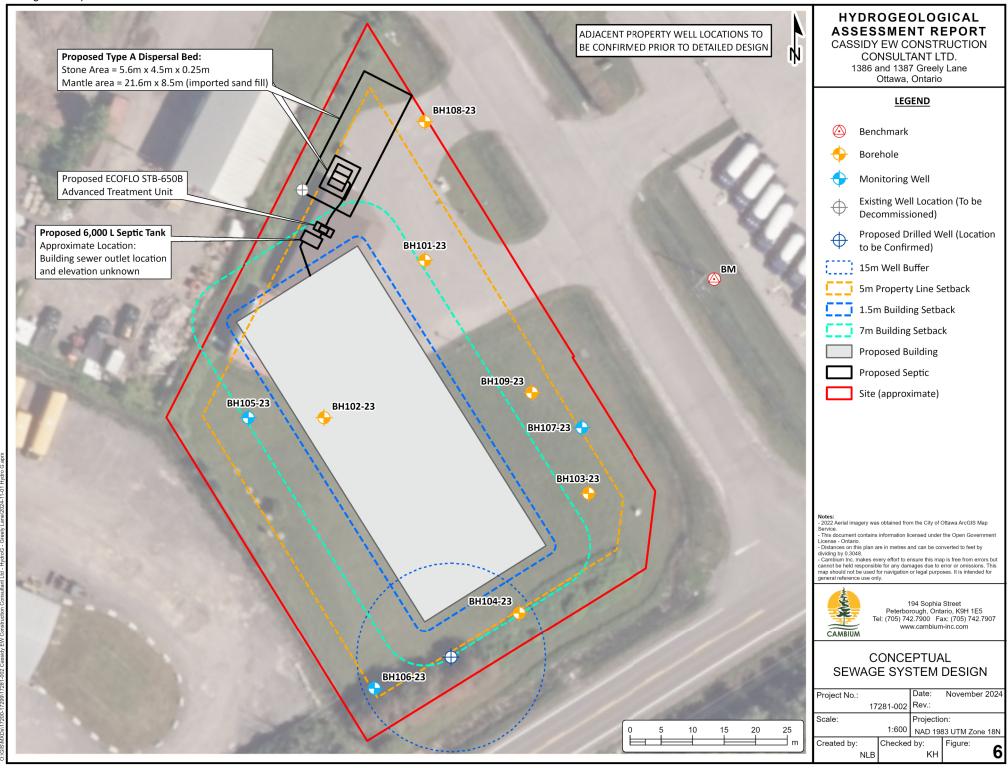
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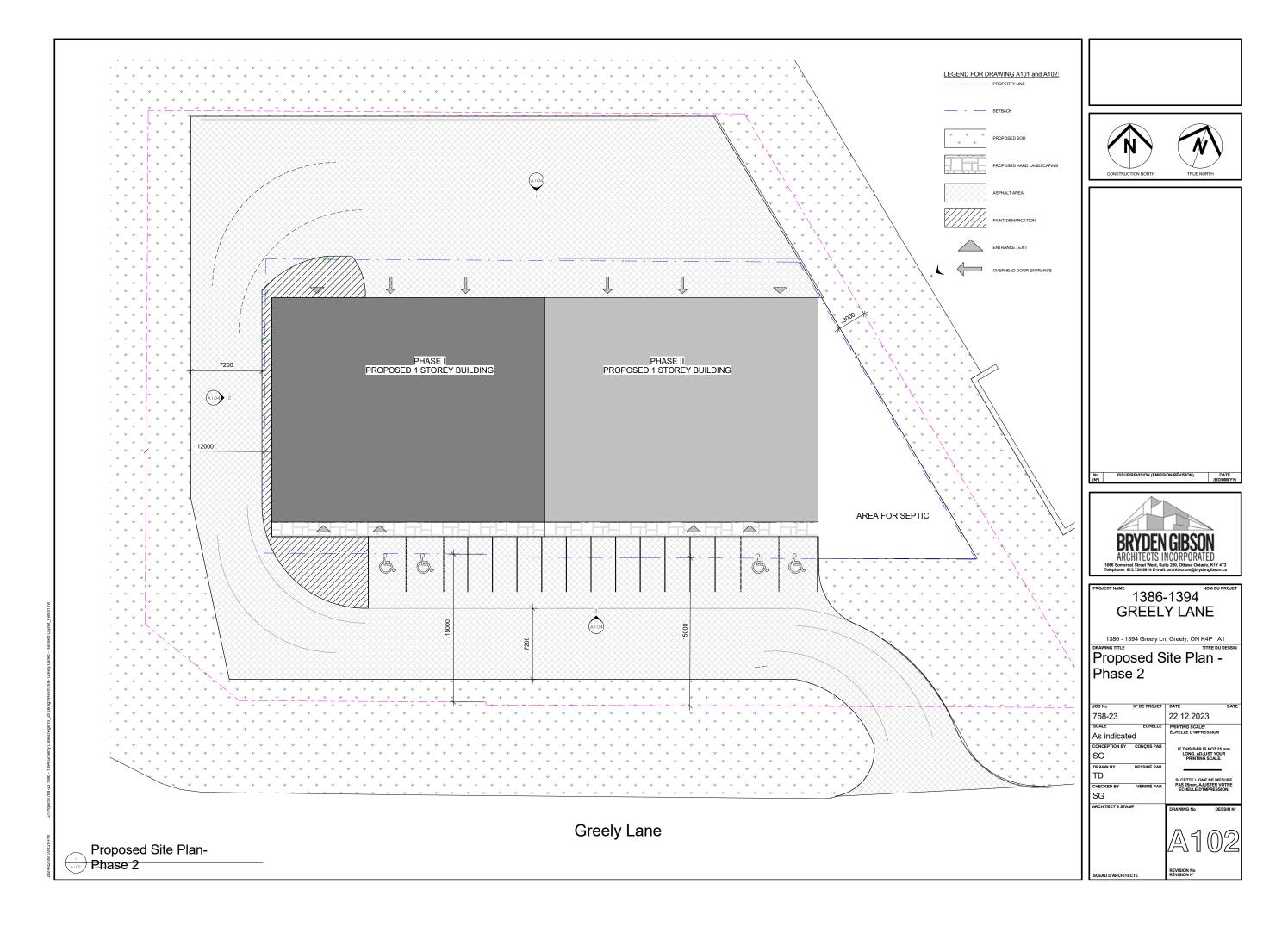






Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
November 11, 2024

	Appendix A
Property and	Land Information



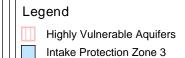






Source Protection Information Atlas Map





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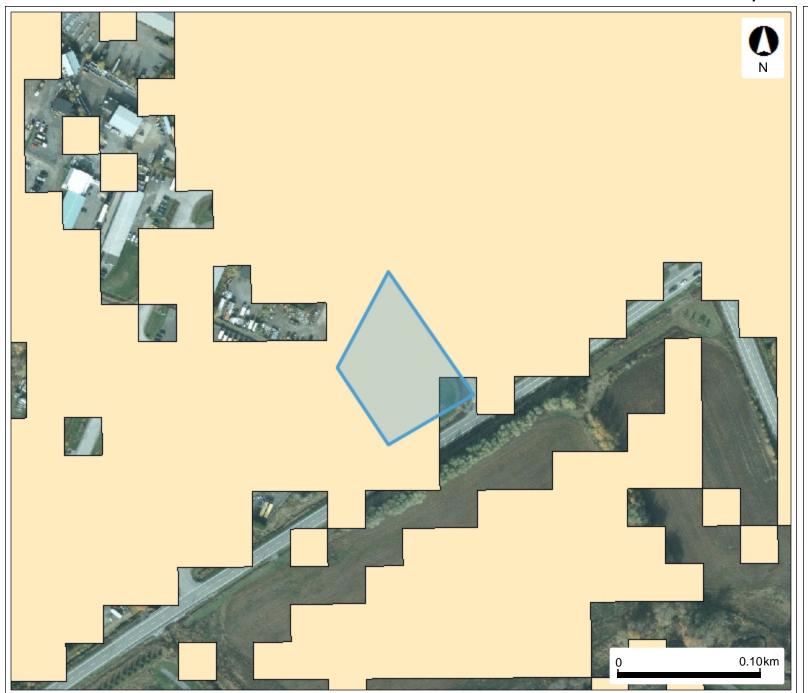


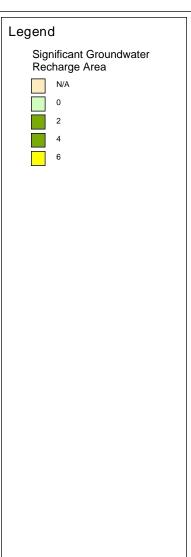
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Map Created: 5/27/2024

Map Center: 45.25878 N, -75.57137 W

Source Protection Information Atlas - SGRA Map





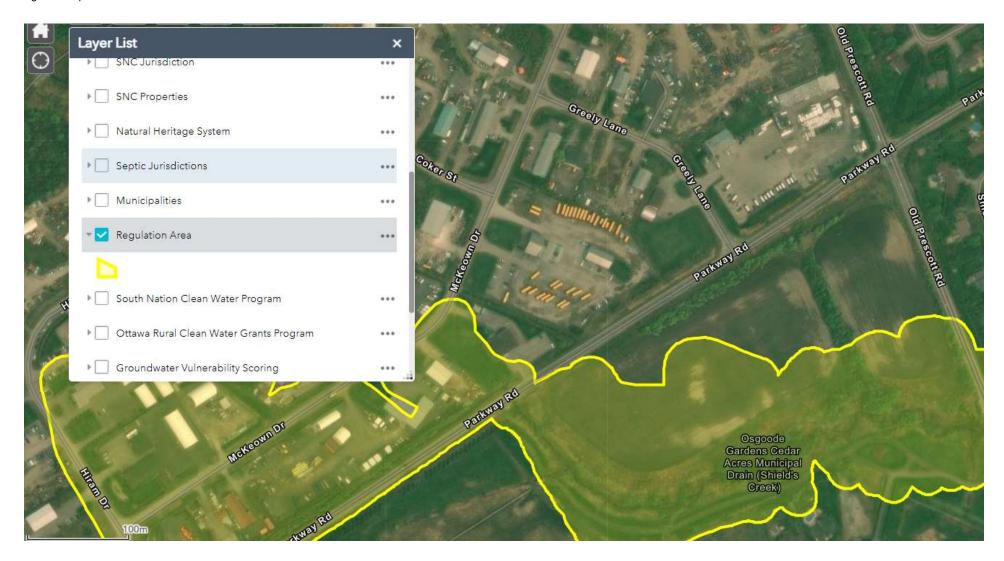
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Map Created: 5/27/2024

Map Center: 45.25878 N, -75.57137 W





Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
November 11, 2024

Appendix	В
Borehole Loc	zs



Client:Cassidy EW ConstructionProject Name:1386 & 1394 Greely LaneLog of Borehole:BH101-23

Contractor:OGS Inc.Method:Track Mounted Hollow Stem AugerPage:1 of 1Location:Ottawa, ONElevation:100.75 mRELDate Completed:March 8, 2023

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011868 **E**: 455169

	SUE	BSURFACE PROFILE				SAMP	LE			
Elevation (m) Depth	Lithology	Description Elevation Depth	Number	Туре	% Recovery	SPT (N)	% Moisture 25 50 75	Shear Strength Cu, kPa nat V tem V. 20 40 60 80 SPT (N)	Well Installation	Log Notes
		,					· · · · · · · · · · · · · · · · · · ·			
100.80		ASPHALT: 75 mm 100.67								
100.2 + 0.5		FILL: (SM) GRAVELLY SAND: 0.08 brown, moist, some silt [base material] 100.29	1A	SS	100	75	10%	75		
+ 0.5		FILL: (SM) SILTY SAND: grey, moist, gravelly	1B	SS			12.7%			
99.8 — 1		99.68	2A	SS	83	7	0.5%	7		
+		(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm	2B	SS	83		18.8%			
99.2 + 1.5			3	SS	75	4	18.8%	•4		1.5m: ATT SS3: 19.8%LL 12.5%PL
98.8—2										
98.2 + 2.5		98.16	4A	SS			21%	9		
+ 2.0		(SM) SILTY SAND: grey, wet, trace clay	4B	SS	67	9	19.5%			
97.8—3		97.7								
97.2 - 3.5		(ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay	5	SS	63	15	17.1%	• 15		
+		-becomes moist, dense								
96.8 4			6	SS	67	46	13.3%	•46		
96.2 + 4.5		-becomes very dense					-			
95.8—5			7	ss	88	88	e ^{14%}	8	3	
+							-			
95.2 + 5.5		-becomes wet, compact	8	SS	67	20	18%	20		
94.8—6		94.65								
+		Borehole terminated @ 6.1m due to target depth achieved.								
94.2 + 6.5										Borehole caved at 2.1
93.8 7										mbgs. Groundwater encountered at 1.1 mbgs following completion.
+										
93.8	1	1						GRAINSIZE S	AMPLE GRAVEL SAN SS1B 20 53 SS3 0 22 SS6 0 19	D SILT CLAY 27 57 21 77 4
1m = 24 units										



Client: Cassidy EW Construction Project Name: 1386 & 1394 Greely Lane Log of Borehole: BH102-23

Contractor:OGS Inc.Method:Track Mounted Hollow Stem AugerPage:1 of 1Location:Ottawa, ONElevation:100.46 mRELDate Completed:March 8, 2023

Project No.: 17281-001 - B **UTM:** 18 T **N:** 5011843 **E:** 455153

The scription and the second and the	` '	Vell Ilation Log Notes
FILL: (SM) SILTY SAND: brown, wet, compact, gravelly, with roots FILL: (SM) SILTY SAND: brown, wet, compact, gravelly, with roots 99.49 2A SS 99.49 2B SS 92 4 19.5% 98.5 2 -becomes soft 98 - 2.5 97.5 - 3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay -becomes very dense 96.5 - 4 6 SS 71 63		
FILL: (SM) SILTY SAND: brown, wet, compact, gravelly, with roots FILL: (SM) SILTY SAND: brown, wet, compact, gravelly, with roots 18 SS 63 11 13.6% 18 SS 63 11 13.6% 19 SS 63 11 13.6% 10 SS 63 11 13		
100 - 0.5 Section Pick Sandy		
99.5 1 (ML) sandy CLAYEY SILT: grey, cohesive, w-PL, firm 2B SS 92 4 19.5% 98.5 2 -becomes soft 4 SS 67 3 18.5% 3 97.5 3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay -becomes very dense 6 SS 71 63 13.4%		
99.5 1 (ML) sandy CLAYEY SILT: grey, cohesive, w-PL, firm 2B SS 92 4 19.5% 4 98.5 2 -becomes soft 4 SS 67 3 18.5% 3 3 97.5 3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay -becomes very dense 6 SS 71 63 13.4%		
99 - 1.5 98.5 - 2 98 - 2.5 97.5 - 3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay 96.5 - 4 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay -becomes very dense 28		
98.5—2 98.5—2 -becomes soft 4 SS 67 3 18.5% 97.41 97.5—3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay -becomes very dense 6 SS 71 63 13.4%		
-becomes soft 98 - 2.5 97.5 - 3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay -becomes very dense 6 SS 71 63 13.4%		
98 - 2.5 97.5 - 3 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay 96.5 - 4 - becomes very dense 4 SS 67 3 18.5% 97.41 5 SS 42 18 15.3% 18.6%		
97 - 3.5 (ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay 5 SS 42 18 15.3% -becomes very dense 6 SS 71 63		
97 - 3.5 96.5 - 4 (ML) SiL1: grey, non-conesive, wet, compact, some sand, trace clay 5		
96.5 4 6 SS 71 63 13.4%		
6 SS 71 63 •		
96 - 4.5	63	
7 SS 79 69 13.0%	69	
95.5 - 5		
†		
95 - 5.5 8 SS 75 56 • 14.4%	56	
94.5 6 94.36		
Borehole terminated @ 6.1m due to target depth achieved.		
+		Borehole caved at 4.0 mbgs. Groundwater measured at 1.5 mbgs
93.5 7		following completion.
93.5		1
· ·		
1m = 24 units	RAINSIZE <u>[SAMPLE I GR</u>	AVEL SAND SILT CLAY

Contractor: OGS Inc.

Location: Ottawa, ON



Client: Cassidy EW Construction Project Name: 1386 & 1394 Greely Lane

Method: Track Mounted Hollow Stem Auger

Elevation: 100.45 mREL

Project No.: 17281-001 - B **UTM:** 18 T **N:** 5011831 **E:** 455195

Page: 1 of 1

Date Completed: March 8, 2023

BH103-23

Log of Borehole:

	SUB	SURFACE PROFILE				SAMP	LE			
Elevation (m) Depth	Lithology	Description Elevation Depth	Number	Туре	% Recovery	SPT (N)	% Moisture 25 50 75	Shear Strength Cu, kPa nat V rem V. \$\vert{\text{em} V. \text{\$\subseteq}}\$ 20 40 60 80 SPT (N) 20 40 60 80	Well Installation	Log Notes
100.4 — 0				1	1		39.1%			
	West West	TOPSOIL: 300 mm 100.15	1A	SS	67	2		2		
100 + 0.5		FILL: (SM) SILTY SAND: grey, wet, trace gravel	1B	ss			22%			
		99.46	2A	ss			22.4%			
99.4 1		(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm	2B	ss	79	4	19.3%	• 1		
99 + 1.5		-becomes stiff								
98.4—2			3	ss	88	8	16.7%	• ⁸		
		-decrease in clay content, becomes CL-ML								
98 + 2.5			4	SS	92	10	15.6%	1 0		
97.4—3		97.4								
97 - 3.5		(ML) sandy SILT: grey, non-cohesive, wet, compact, trace clay	5	SS	88	17	14.3%	•17		
96.4 4			6	SS	79	15	14.1%	1 5		
96 + 4.5		-becomes dense	7	SS	71	39	13.6%	39		
+							1			
95 + 5.5			8	SS	71	47	13.7%	47 •		
94.4—6	ШЦ	94.35								
94 + 6.5		Borehole terminated @ 6.1m due to target depth achieved.								
										Borehole caved at 4.9 mbgs. Groundwater
93.4 7										measured at 0.9 mbgs following completion.
93.5										
								GRAINSIZE SA DISTRIBUTION	MPLE GRAVEL SAN	D SILT CLAY
1m = 24 units										

Logged By: FI

Input By: BV

Peterborough, Barrie, Oshawa, Kingston, Ottawa



Client:Cassidy EW ConstructionProject Name:1386 & 1394 Greely LaneLog of Borehole:BH104-23

Contractor:OGS Inc.Method:Track Mounted Hollow Stem AugerPage:1 of 1Location:Ottawa, ONElevation:100.52 mRELDate Completed:March 8, 2023

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011812 **E**: 455184

	SUB	SURFACE PROFILE				SAMP	LE			
Elevation (m) Depth	Lithology	Description Elevation Depth	Number	Туре	% Recovery	SPT (N)	% Moisture 25 50 75	Shear Strength Cu, kPa nat V frem V 6 20 40 60 80 SPT (N) 20 40 60 80	Well Installation	Log Notes
100.50	VISITIVED.	TOPSOIL: 125 mm 100.39	1A 1B	SS			18.5			
100 + 0.5		FILL: (SM) SILTY SAND: 0.13 brown, wet, very loose	1C	SS	42	3	31.3%	3		
							20.7%			
99.5—1	1444	99.55	2A	SS		ļ.,		4		
		(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm	2B	ss	75	4	20%			
99 + 1.5			3	SS	79	4	18.2%	• 4		
98.5—2		-decrease in clay content,								
98 + 2.5		becomes CL-ML, soft				_	18%	3		2.3m: ATT SS4: 18.5%LL 13.1%PL
			4	SS	100	3				
97.5—3		-100 mm silty sand seam 97.29					1			
97 + 3.5		(ML) sandy SILT: grey, non-cohesive, wet, compact, trace clay	5	SS	83	10	16.3%	•10		
96.5 4			_				14.3%	26		
+			6	SS	75	26				
96 + 4.5										
95.5—5			7	ss	83	28	13.9%	28		
							-			
95 + 5.5		-becomes dense					1			
+			8	ss	79	39	13.9%	39		
94.5—6		94.42					+			
94 + 6.5		Borehole terminated @ 6.1m due to target depth achieved.								
+										Borehole caved at 4.6 mbgs. Groundwater measured at 0.6 mbgs
93.5 7										following completion.
93.6	•			•	-	•	, , , ,	GRAINSIZE S/ DISTRIBUTION	AMPLE GRAVEL SAN SS4 0 25 SS6 0 22	D SILT CLAY 57 18 74 4
1m = 24 units										



Client:Cassidy EW ConstructionProject Name:1386 & 1394 Greely LaneLog of Borehole:BH105-23

Contractor:OGS Inc.Method:Track Mounted Hollow Stem AugerPage:1 of 1Location:Ottawa, ONElevation:100.65 mRELDate Completed:March 8, 2023

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011843 **E**: 455141

	SUB	SSURFACE PROFILE				SAMP	LE			
Elevation (m) Depth	Lithology	Description Elevation Depth	Number	Туре	% Recovery	SPT (N)	% Moisture 25 50 75	Shear Strength Cu, kPa nat V tem V. 20 40 60 80 SPT (N) 20 40 60 80	Well Installation	Log Notes
400.0							77.00		_	
100.60		TOPSOIL: 150 mm 100.5	1A	SS			37.1%		Cap Bentonite Plug	
100.2 + 0.5		FILL: (SM) SILTY SAND: 0.15 brown, wet, loose, some gravel, trace clay	1B	ss	67	7	19.3%	• 7	Riser	
+		-becomes grey, decrease in silt	2A	SS			12.1%			
99.6 — 1		content	20	33	63	11	15.4%	• 11	*	
+			2B	SS						
99.2 + 1.5		99.13								
98.6—2		(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm	3	SS	92	4	19.9%	• 4	Sand Bock Screen	
							200			
98.2 + 2.5		98.21	4A	SS			20%	5		
30.2 - 2.3		(ML) SILT: grey, non-cohesive, wet, loose, some sand, trace clay	4B	ss	63	5	18.1%	• 3		
97.6—3		-becomes compact					1		Сар	
97.2 + 3.5		96.99	5	SS	50	16	16.1%	1 6		
96.6—4		Borehole terminated @ 3.7m due to target depth achieved.								
										Groundwater
96.2 + 4.5										measured at 2.0 mbgs following completion.
95.6—5										
33.0 7 3										
95.2 + 5.5										
†										
94.6 6										
†										
94.2 + 6.5										
+										
93.6 7										
93.7								CDAINGIZE TO	AMADIE LODAVEL L. CO.	
								GRAINSIZE <u>[S</u> DISTRIBUTION	AMPLETGRAVELT SAN	ID SILT CLAY
1m = 24 units										



Client: Cassidy EW Construction Project Name: 1386 & 1394 Greely Lane

Method: Track Mounted Hollow Stem Auger

Log of Borehole: BH106-23 Page: 1 of 1

Contractor: OGS Inc.
Location: Ottawa, ON

Elevation: 100.38 mASL

Date Completed: March 7, 2023

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011800 **E**: 455161

99.4 — 1 1		SUB	SURFACE PROFILE				SAMP	LE			
99.9 - 0.5 Fall Cid St X Samura 10 55 54 3 3 3 3 3 3 3 3 3	Elevation (m) Depth	Lithology		Number	Туре	% Recovery	SPT (N)		20 40 60 80 SPT (N)		Log Notes
99.9 - 0.5 Fall Cid St X Samura 10 55 54 3 3 3 3 3 3 3 3 3								F. F.			
99.9 - 0.5 Filt. (SM) sit. Y SANT SANT	00.4 \top 0	West West		1A	SS			19.4%			
99.4 1 MIL) sandy CLAYEY SLT: 0.86 2B SS 75 3 is	99.9 + 0.5		brown, wet, very loose, trace			54	3	31.1%	9 3	Rigg	
99.4 1 MIL) sandy CLAYEY SLT: 0.86 2B SS 75 3 is	+		99.52	24	92			22.9%			
98.4 - 2 97.9 - 2.5 97.4 - 3 (ML) sandy SILT: grey, non-cohesive, well, compact, trace clay 98.7 - 4 Borehole terminated @ 3.7m ^{3.66} due to target depth achieved. 98.9 - 4.5 99.9 - 4.5 99.9 - 6.5 99.9 - 6.5 99.9 - 6.5 99.9 - 6.5	99 4 1			20	33	ļ			3		
98.4 - 2 97.9 - 2.5 97.4 - 3 (ML) sandy SILT: grey, non-cohesive, well, compact, trace clay 98.7 - 4 Borehole terminated @ 3.7m ^{3.66} due to target depth achieved. 98.9 - 4.5 99.9 - 4.5 99.9 - 6.5 99.9 - 6.5 99.9 - 6.5 99.9 - 6.5	7		grey, cohesive, w>PL, soft	2B	ss	75	3	19%			
98.4 - 2 97.9 - 2.5 97.4 - 3 96.9 - 3.5 MI) sandy SUT: grey, non-collesive, wet, compact, trace clay March Marc	98.9 + 1.5		-becomes firm					18.3%	5	Sand Back	
96.9 - 3.5	98.4—2			3	SS	100	5	1			
96.9 - 3.5	97.9 + 2.5			4	SS	92	6				
96.9 - 3.5	974 3		97.33								
96.4—4 due to target depth achieved. 95.9—4.5 94.9—5.5 94.4—6 93.9—6.5 93.4—7	+		(ML) sandy SILT: grey, non-cohesive, wet, compact, trace clay	5	ss	75	15	15.8%	● 15	i∴ s es- ∨u p	
95.9 4.5 95.4 5 94.4 6 93.9 6.5 93.4 7	96.4 4		Borenole terminated @ 3.7m								Groundwater
94.9 - 5.5 94.4 - 6 93.9 - 6.5 93.4 - 7 93.4	95.9 + 4.5										measured at 1.5 mbg
94.4 6 93.9 6.5 93.4 7 93.4 7	95.4 - 5										
93.9 - 6.5 93.4 - 7 93.4 - 7	94.9 - 5.5										
93.4 7 93.4 GRAINSIZE [SAMPLEI GRAVELI SAND SILT CLAY]	94.4—6										
93.4 GRAINSIZE [SAMPLE] GRAVEL SAND SILT CLAY	93.9 - 6.5										
GRAINSIZE <u>ISAMPLETGRAVELT SAND T SILT T CLAY T</u>	93.4 - 7										
	93.4								GPAINICIZE IC.	AMDIELGDAVELL CAN	ID I SHT I CLAY
										AIVIPLETURAVELT SAN	ND 1 SILI I CLAY

Logged By: FI

Input By: BV

Peterborough, Barrie, Oshawa, Kingston, Ottawa



Client: Cassidy EW Construction Project Name: 1386 & 1394 Greely Lane Log of Borehole: BH107-23

Contractor:OGS Inc.Method:Track Mounted Hollow Stem AugerPage:1 of 1Location:Ottawa, ONElevation:99.86 mRELDate Completed:March 8, 2023

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011845 **E**: 455203

Section Description Desc		SUB	SURFACE PROFILE				SAMP	LE			
99.4 - 0.5 99.4 - 0.5 99.4 - 0.5 99.4 - 1.5 98.9 - 1 98.4 - 1.5 98.9 - 2 97.4 - 2.5 98.9 - 3 98.9 - 4 98.9 - 5 98.9 - 3 98.9 - 6 99.9 - 3 99.9	Elevation (m) Depth	Lithology		Number	Туре	% Recovery	SPT (N)	1	20 40 60 80 SPT (N)	Well	Log Notes
98.9 — 1 98.4 — 1.5 97.9 — 2 97.4 — 2.5 96.9 — 3 96.4 — 3.5 96.4 — 3.5 96.4 — 4.5 93.9 — 6 93.4 — 6.5 93.9 — 6 93.4 — 6.5 92.9 — 7			FILL: (SM) SILTY SAND: 0.08 brown, wet, trace clay 99.56	1B	SS	79	6	24%	6	Bentonite Plug Riser	
3 SS 100 7 15-44 0 7 19-45 1979. 97.4 - 2.5 96.9 - 3 96.4 - 3.5 96.9 - 4 95.4 - 4.5 94.9 - 5 94.4 - 5.5 93.9 - 6 93.9 - 6 93.4 - 6.5 92.9 - 7	98.9 — 1		grey, cohesive, w>PL, stiff	2	SS	79	9	15.9%			
97.4	-		-becomes firm	3	SS	100	7	15.4%	• 7	Safeen	
96.9 — 3 96.4 — 3.5 Borehole terminated @ 3.7m 3.66 due to target depth achieved. 95.4 — 4.5 94.9 — 5 93.9 — 6 93.4 — 6.5 92.9 — 7 92.9	97.4 + 2.5 +		(ML) sandy SILT: grey, non-cohesive, wet, compact,	4	ss	75	17		•17		
95.9 — 4 95.4 — 4.5 94.9 — 5 93.9 — 6 93.4 — 6.5 92.9 — 7 92.9	+			5	ss	63	16	14.8%	1 6	Сар	
95.4 4.5 94.9 5 94.4 5.5 93.9 6 93.4 6.5 92.9 7 92.9	95.9 4		Borehole terminated @ 3.7m 3.66								
94.4 - 5.5 93.9 - 6 93.4 - 6.5 92.9 - 7 92.9 - GRAINSIZE [SAMPLE] GRAVEL SAND SILT CLAY											
93.4 - 6.5 92.9 - 7 92.9 - GRAINSIZE [SAMPLE] GRAVEL SAND SILT CLAY]											
92.9 — 7 92.9 — GRAINSIZE [SAMPLE GRAVEL SAND SILT CLAY]											
GRAINSIZE <u>[SAMPLE] GRAVELT SAND T SILT T CLAY</u>											
DISTRIBUTION	92.9								GRAINSIZE S	AMPLETGRAVELT SAN	ID SILT CLAY



Client: Cassidy EW Construction Project Name: 1386 & 1394 Greely Lane Log of Borehole:

Contractor: OGS Inc. Method: Track Mounted Hollow Stem Auger

Location: Ottawa, ON **Elevation:** 100.8 mREL

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011890 **E**: 455169

	SUE	SSURFACE PROFILE				SAMP	LE			
Elevation (m) Depth	Lithology	Description Elevation Depth	Number	Туре	% Recovery	SPT (N)	% Moisture 25 50 75	Shear Strength Cu, kPa nat v fem v 6 20 40 60 80 SPT (N) 20 40 60 80	Well Installation	Log Notes
100.0			-							
100.8 0		ASPHALT: 50 mm 100.75								
100.3 + 0.5		FILL: (SM) GRAVELLY SAND, brown, wet, some silt [base material] 100.19	1A	ss	100	64	14.2% 31.5%	64		
+		FILL: (SM) SAND and SILT: grey, wet	1B	SS			-			
99.8 — 1		99.73	2A	SS			16.6%			
00.0		(ML) sandy CLAYEY SILT: grey, non-cohesive, w>PL, firm 99.28	2B	ss	67	3	20.1%	3		
99.3 + 1.5		Borehole terminated @ 1.5m due to target depth achieved.					-			
98.8 - 2						<u> </u> 				Borehole remained
98.3 + 2.5										open. Groundwater measured at 0.8 mbgs following completion.
+										
97.8—3										
97.3 - 3.5										
96.8 4										
96.3 + 4.5										
95.8 - 5										
95.3 + 5.5										
94.8 6										
94.3 + 6.5										
93.8 7										
03.0										
93.8	•		•	•		•		GRAINSIZE SA DISTRIBUTION	AMPLE GRAVEL SAN SS1B 0 63	D SILT CLAY
1m = 24 units										

BH108-23

March 8, 2023

1 of 1

Page:

Date Completed:



Client:Cassidy EW ConstructionProject Name:1386 & 1394 Greely LaneLog of Borehole:BH109-23

Contractor:OGS Inc.Method:Track Mounted Hollow Stem AugerPage:1 of 1Location:Ottawa, ONElevation:100.34 mRELDate Completed:March 7, 2023

Project No.: 17281-001 - B **UTM**: 18 T **N**: 5011847 **E**: 455186

	SUB	SURFACE PROFILE				SAMPI	LE			
Elevation (m) Depth	Lithology	Description Elevation Depth	Number	Туре	% Recovery	SPT (N)	% Moisture 25 50 75	Shear Strength Cu, kPa	Well Installation	Log Notes
100.3 — 0		TOPSOIL: 915 mm	1	ss	25	2	44.3%	2		
99.3 — 1		99.43 FILL: (SM) SILTY SAND: grey, wet 99.32/ (ML) sandy CLAYEY SILT: grey, non-cohesive, w>PL, soft 98.82	2A 2B	SS	83	3	28.9%	3		
98.8 + 1.5 I		Borehole terminated @ 1.5m due to target depth achieved.								Borehole remained open. Groundwater measured at 1.1 mbgs
97.8 + 2.5 + 97.3 - 3										following completion.
96.8 + 3.5										
95.8 - 4.5										
95.3—5 - 94.8 - 5.5										
94.3—6										
93.8 + 6.5 + 93.3 - 7										
93.4								GRAINSIZE 5/2 DISTRIBUTION	AMPLETGRAVELT SAN	D SILT CLAY



Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario Cassidy EW Construction Consultant Ltd.

Cambium Reference: 17281-002

November 11, 2024

		A	pp	endi	x C
Grain	Size	Analys	is	Res	ults





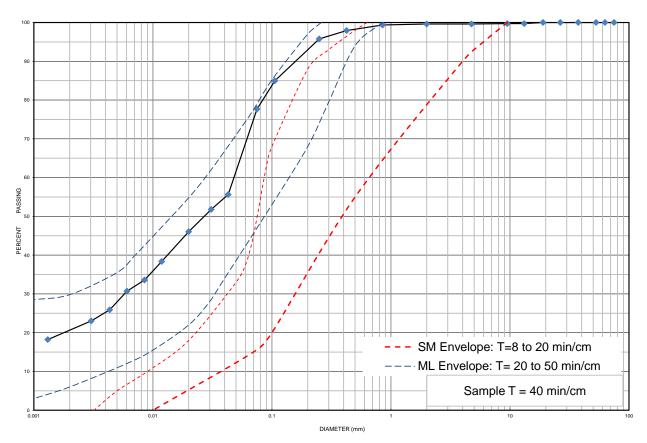
Project Number: 17281-002 Client: Cassidy E.W. Construction Consultant Ltd.

Project Name: Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa

Sample Date: March 7-8, 2023 Sampled By: Farhan Imtiaz - Cambium Inc.

Location: BH 101-23 SS 3 **Depth:** 1.5 m to 2.1 m **Lab Sample No:** S-23-0475

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	М				
CLAV & SILT (=0.075 mm)	SAND (<4.	75 mm to 0.075 mm)		GRAVE	L (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE		



	MIT SOIL CLASSIFICATION SYSTEM									
CLAY	CLAY SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS		
CLAT			SAND	GRAVEL	GRAVEL		BOOLDERS			

Borehole No.	Sample No.	Depth	Gravel	Sand		Silt	Clay	Moisture
BH 101-23	SS 3	1.5 m to 2.1 m	0	22		57	21	18.8
	Description	Classification	D ₆₀	D ₃₀		D ₁₀	Cu	C _c
S	andy Clayey Silt	ML	0.0480	0.0058	3	-	-	-

Additional information availabe upon request





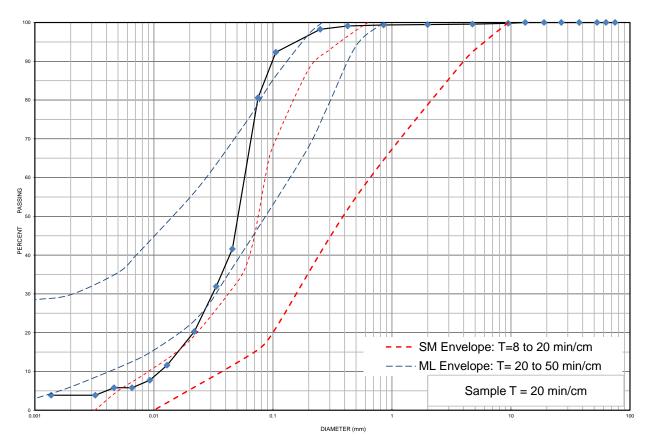
Project Number: 17281-002 Client: Cassidy E.W. Construction Consultant Ltd.

Project Name: Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa

Sample Date: March 7-8, 2023 Sampled By: Farhan Imtiaz - Cambium Inc.

Location: BH 101-23 SS 6 **Depth:** 3.8 m to 4.4 m **Lab Sample No:** S-23-0476

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	М				
CLAV 8 CHT (-0.075 mm)	SAND (<4.	75 mm to 0.075 mm)		GRAVE	_ (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	GRAVEL (>4.75 mm) FINE COARSI	COARSE		



	MIT SOIL CLASSIFICATION SYSTEM									
CLAY	CLAY SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS		
CLAT			SAND			GRAVEL		BOOLDERS		

Borehole No.	Sample No.	Depth	Gravel	Sand		Silt	Clay	Moisture
BH 101-23	SS 6	3.8 m to 4.4 m	0	19		77	4	13.3
	Description	Classification	D ₆₀	D ₃₀		D ₁₀	Cu	C _c
Silt so	ome Sand trace Clay	ML	0.057	0.032	2	0.012	4.75	1.50

Additional information availabe upon request





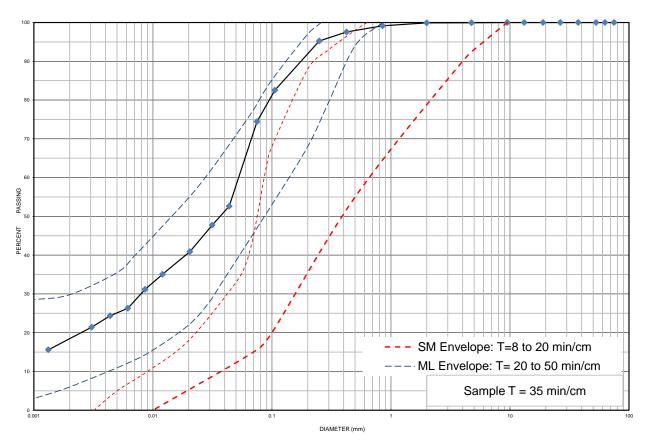
Project Number: 17281-002 Client: Cassidy E.W. Construction Consultant Ltd.

Project Name: Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa

Sample Date: March 7-8, 2023 Sampled By: Farhan Imtiaz - Cambium Inc.

Location: BH 104-23 SS 4 **Depth:** 2.3 m to 2.9 m **Lab Sample No:** S-23-0477

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	М				
CLAV 9 CH T (-0.075 mm)	SAND (<4.	75 mm to 0.075 mm)		GRAVE	_ (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	GRAVEL (>4.75 mm) FINE COARSE			



	MIT SOIL CLASSIFICATION SYSTEM									
CLAY	CLAY SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS		
CLAT			SAND	GRAVEL	GRAVEL		BOOLDERS			

Borehole No.	Sample No.	Depth	Gravel	Sand		Silt	Clay	Moisture
BH 104-23	SS 4	2.3 m to 2.9 m	0	25		57	18	18.0
	Description	Classification	D ₆₀	D ₃₀		D ₁₀	Cu	C _c
Sai	ndy Silt some Clay	ML	0.053	0.008	3	-	-	-

Additional information availabe upon request





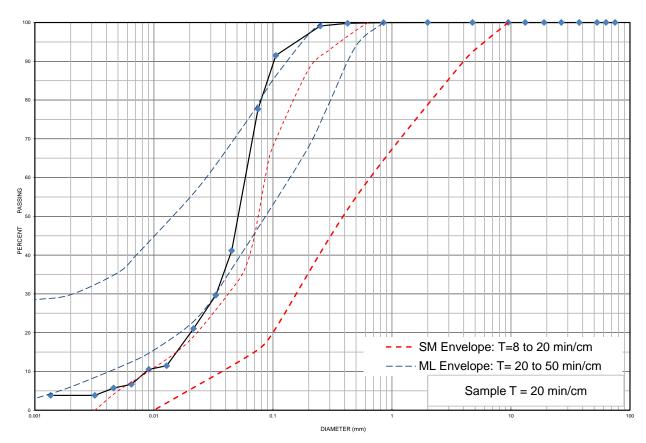
Project Number: 17281-002 Client: Cassidy E.W. Construction Consultant Ltd.

Project Name: Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa

Sample Date: March 7-8, 2023 Sampled By: Farhan Imtiaz - Cambium Inc.

Location: BH 104-23 SS 6 **Depth:** 3.8 m to 4.4 m **Lab Sample No:** S-23-0478

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	М				
CLAV 9 CH T (-0.075 mm)	SAND (<4.	75 mm to 0.075 mm)		GRAVE	_ (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	GRAVEL (>4.75 mm) FINE COARSE			



	MIT SOIL CLASSIFICATION SYSTEM									
CLAY SILT	SH T	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS		
CLAY	AY SILT		SAND			GRAVEL		BOOLDERS		

Borehole No.	Sample No.		Depth		Gravel 0		Sand 22		Silt		Clay	Moisture 14.3	
BH 104-23	SS 6	3.8 m to 4.4 m							74		4		
	Description		Classification		D ₆₀		D ₃₀		D ₁₀		Cu	C _c	
Sandy Silt trace Clay		ML		0.0590		0.0340		0.0087		6.78	2.25		

Additional information availabe upon request



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Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
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		Appendix					
Well	Inventory	y Survey	/ Results				

Water Well Records Summary Report

Produced by Cambium Inc. using MOECP Water Well Information System (WWIS)

All units in meters unless otherwise specified



Well ID: 1507224 Construction Date: 1965-09-22	Easting: 45 Northing: 5	UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m						
	Well Depth: 20.7 Well Diameter (cm): 15.2 Water First Found: 16.8 Static Level: 6 Layer: Driller's Description: 1 MEDIUM SAND		Water Kind Final Status Primary Water Use: Top: Bottom:		FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate Pumping Duration (h:m):	23 : 23 0:30	
			0	4.57				
	2	LIMESTONE	4.57	20.7				
Well ID: 1507232 Construction Date: 1964-07-06	Easting: 45 Northing: 5		UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m					
	Well Depth: 20.4 Well Diameter (cm): 5.08 Water First Found: 20.4 Static Level: 2		Water Kind Final Status Primary Water Use:		FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	32 23 2:0	
	Layer: Dr	iller's Description:	Тор:	Bottom:				
	1	MEDIUM SAND	0	5.49				
	2	BOULDERS	5.49	14.0				
	3	LIMESTONE	14.0	20.4				
Well ID: 1507234 Construction Date: 1964-07-06	Easting: 45		UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m					
	Well Depth: 20.7 Well Diameter (cm): 5.08 Water First Found: 20.7 Static Level: 1		Water Kind Final Status Primary Water Use:		FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	45 23 2:0	
	Layer: Dr	iller's Description:	Тор:	Bottom:				
	1	MEDIUM SAND	0	5.49				
	2	BOULDERS	5.49	14.3				
	3	LIMESTONE	14.3	20.7				
Well ID: 1507313 Construction Date: 1966-12-06	Easting: 45			UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m				
	Well Depth: 18.3 Well Diameter (cm): 12.7 Water First Found: 15.2 Static Level: 6		Water Kind Final Status Primary Water Use:		FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	27 23 1:0	
	Layer: Dr	iller's Description:	Тор:	Bottom:				
	1	GRAVEL	0	5.49				

LIMESTONE

7.62

10.7

Well ID: 1509840 Easting: 455391 UTM Zone 18 Construction Date: 1968-08-21 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m **Water Kind FRESH** Pump Rate (LPM): Well Depth: 12.8 **Final Status Recommended Pump Rate: 23** Well Diameter (cm): 10.2 Water Supply Primary Water Use: Domestic **Pumping Duration (h:m):** Water First Found: 12.8 0:30 **Static Level:** Laver: Driller's Description: Top: **Bottom:** TOPSOIL 0 0.91 1 2 **HARDPAN** 0.91 3.96 3 LIMESTONE 3.96 12.8 Well ID: 1510585 **Easting:** 455331 UTM Zone 18 Construction Date: 1970-05-28 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m **Water Kind FRESH** Pump Rate (LPM): 45 Well Depth: 32.9 **Final Status** Water Supply **Recommended Pump Rate: 36** Well Diameter (cm): 15.2 Primary Water Use: Domestic Pumping Duration (h:m): **Water First Found:** 32 1:0 Static Level: 5 Layer: Driller's Description: Top: **Bottom:** 1 **TOPSOIL** 0 1.52 2 **GRAVEL** 1.52 5.18 3 LIMESTONE 32.9 5 18 Well ID: 1512221 Easting: 455604 UTM Zone 18 Construction Date: 1973-01-12 Northing: 5E+06 Positional Accuracy: margin of error: 300 m - 1 km **Water Kind FRESH** Pump Rate (LPM): 91 Well Depth: 14.6 **Final Status** Water Supply **Recommended Pump Rate: 23** Well Diameter (cm): 15.2 Primary Water Use: Domestic Pumping Duration (h:m): Water First Found: 14.0 Static Level: 4 Layer: **Driller's Description:** Top: **Bottom:** 1 SAND 0 2.74 2 SAND 2.74 12.2 3 LIMESTONE 12.2 14.6 Well ID: 1513408 **Easting:** 455523 UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100 m Construction Date: 1973-09-10 Northing: 5E+06 **Water Kind FRESH** Pump Rate (LPM): 36 Well Depth: 10.7 **Final Status Recommended Pump Rate: 23** Well Diameter (cm): 12.7 Water Supply **Water First Found:** Primary Water Use: Domestic Pumping Duration (h:m): 1:57 9.75 **Static Level:** Layer: Driller's Description: Top: **Bottom: HARDPAN** 1 0 7.62

Well ID: 1513421 **Easting:** 455556 UTM Zone 18 Construction Date: 1973-09-26 Northing: 5E+06 Positional Accuracy: margin of error: 300 m - 1 km **Water Kind FRESH** Pump Rate (LPM): Well Depth: 13.1 Well Diameter (cm): 12.7 **Final Status** Water Supply **Recommended Pump Rate: 45** Water First Found: Primary Water Use: Domestic Pumping Duration (h:m): 1:10 13 1 Static Level: Laver: Driller's Description: Top: **Bottom:** 1 HARDPAN 0 13.1 Well ID: 1515384 UTM Zone 18 Easting: 455451 Construction Date: 1976-06-19 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m Well Depth: 38.1 **Water Kind** Not stated Pump Rate (LPM): **Final Status** Water Supply **Recommended Pump Rate: 18** Well Diameter (cm): Water First Found: 12.8 Primary Water Use: Domestic Pumping Duration (h:m): **Static Level:** Layer: Driller's Description: Top: **Bottom:** 1 SAND 0 5.79 1 SAND 5.79 0 1 **SAND** 0 5.79 1 **SAND** n 5.79 2 LIMESTONE 38.1 5.79 2 LIMESTONE 5.79 38.1 2 LIMESTONE 38.1 5.79 2 LIMESTONE 5.79 38.1 Well ID: 1515531 **Easting:** 455551 UTM Zone 18 Construction Date: 1976-08-13 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind FRESH** Pump Rate (LPM): 91 Well Depth: 16.8 **Final Status Recommended Pump Rate: 68** Well Diameter (cm): 15.2 Water Supply Primary Water Use: Municipal Pumping Duration (h:m): Water First Found: 1:30 16.1 Static Level: Layer: **Driller's Description:** Top: **Bottom:** 0 1 **GRAVEL** 8.23 2 **HARDPAN** 8.23 15.2 3 **SANDSTONE** 15 2 15.5 **UNKNOWN TYPE** 15.5 16.8 Well ID: 1517024 Easting: 455530 UTM Zone 18 Construction Date: 1979-07-09 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind** Pump Rate (LPM): 91 Well Depth: **FRESH** 15.5 Well Diameter (cm): 15.2 **Final Status Recommended Pump Rate: 55** Water Supply **Water First Found:** Primary Water Use: Domestic Pumping Duration (h:m): 14.6 **Static Level:** 6 **Driller's Description:** Laver: Top: **Bottom: HARDPAN** 1 0 4.88 2 **SAND** 4.88 13.7 3 **GRAVEL** 13.7 14.3 4 LIMESTONE 14.3 15.5

Well ID: 1517148 Easting: 455430 UTM Zone 18 Construction Date: 1979-10-05 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind FRESH** Pump Rate (LPM): Well Depth: 16.8 Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate: 45** Water First Found: 13.7 Primary Water Use: Livestock Pumping Duration (h:m): 1:30 Static Level: Laver: Driller's Description: Top: **Bottom:** 1 HARDPAN 0 11.6 2 SAND 13.7 11.6 3 LIMESTONE 13.7 16.8 Well ID: 1517152 Easting: 455530 UTM Zone 18 Construction Date: 1979-10-05 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind FRESH** Pump Rate (LPM): 114 Well Depth: 15.5 **Recommended Pump Rate: 68** Well Diameter (cm): 15.2 **Final Status** Water Supply **Water First Found:** 14.9 Primary Water Use: Domestic Pumping Duration (h:m): 1:30 Static Level: 5 Layer: Driller's Description: **Bottom:** Top: 1 **SAND** 0 10.7 2 **HARDPAN** 10.7 12.2 3 LIMESTONE 15.5 12 2 Well ID: 1517154 Easting: 455530 UTM Zone 18 Construction Date: 1979-10-05 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind FRESH** Pump Rate (LPM): 82 Well Depth: 16.2 **Final Status** Water Supply **Recommended Pump Rate: 45** Well Diameter (cm): 15.2 Primary Water Use: Domestic Pumping Duration (h:m): Water First Found: 14.9 Static Level: 6 **Driller's Description:** Top: **Bottom:** Layer: 0 13.1 1 SAND 2 LIMESTONE 13.1 16.1 Well ID: 1517156 Easting: 455530 UTM Zone 18 Construction Date: 1979-10-05 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind** Pump Rate (LPM): 82 Well Depth: **FRESH** 15.2 **Final Status Recommended Pump Rate: 36** Well Diameter (cm): 15.2 Water Supply **Water First Found:** 14.3 Primary Water Use: Domestic Pumping Duration (h:m): Static Level: Layer: Driller's Description: Top: **Bottom:** 1 **SAND** 0 12.5 2 LIMESTONE 12.5 15.2 Well ID: 1517638 Easting: 455630 UTM Zone 18 Construction Date: 1981-09-08 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m **Water Kind** Pump Rate (LPM): **FRESH** 136 Well Depth: 12.5 **Final Status Recommended Pump Rate: 23** Well Diameter (cm): 15.2 Water Supply Primary Water Use: Domestic Pumping Duration (h:m): **Water First Found:** 12.2 Static Level: **Bottom:** Layer: Driller's Description: Top:

Water First Found: 19.2 Primary Water Use: Domestic Pumping Duration (h:m): **Static Level:** 2 **Driller's Description:** Laver: Top: **Bottom:** 0 1 **SAND** 1.22 2 SAND 1.22 6.1 3 **HARDPAN** 6.1 15.2 SAND 4 15.2 16.8

E LINASCTONS

Layer: Driller's Description:

LIMESTONE 16.8 19.8

Well ID: 1518698 Easting: 455530 UTM Zone 18 Construction Date: 1983-11-24 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m Well Depth: 22.9 **Water Kind FRESH** Pump Rate (LPM): 45 **Final Status Recommended Pump Rate: 23** Well Diameter (cm): 15.2 Water Supply Primary Water Use: Domestic Pumping Duration (h:m): Water First Found: 20.4 Static Level: Layer: **Driller's Description:** Top: **Bottom:** 1 SAND 0 2.44 2 SAND 2.44 11.6 3 SAND 11.6 14.6 **HARDPAN** 18.3 4 14 6 5 LIMESTONE 18.3 22.9 Well ID: 1520434 Easting: 455527 UTM Zone 18 Construction Date: 1986-02-20 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m Well Depth: 195 **Water Kind FRESH** Pump Rate (LPM): 68 **Final Status** Water Supply **Recommended Pump Rate: 68** Well Diameter (cm): 15.2 Primary Water Use: Domestic **Pumping Duration (h:m):** Water First Found: 15.9 Static Level: Layer: Driller's Description: Top: **Bottom: GRAVEL** 0 1 1.83 1 **GRAVEL** 0 1.83 2 CLAY 1.83 7.32 2 CLAY 1.83 7.32 3 7.32 CLAY 13.4 3 CLAY 7.32 13.4 LIMESTONE 4 13.4 19.5 4 LIMESTONE 13.4 19.5 Well ID: 1522346 Easting: 455172 UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m Construction Date: 1988-06-21 Northing: 5E+06 **Water Kind** 91 **FRESH** Pump Rate (LPM): Well Depth: 38.4 **Recommended Pump Rate: 91** Well Diameter (cm): 15.2 **Final Status** Water Supply Primary Water Use: Industrial Pumping Duration (h:m): Water First Found: 29 Static Level: Layer: Driller's Description: Top: **Bottom:** 1 **SAND** 0 2.44 2 **SAND** 2.44 17.1 3 LIMESTONE 38.4 17.1 Well ID: 1522347 Easting: 455239 UTM Zone 18 Construction Date: 1988-06-21 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m **Water Kind FRESH** Pump Rate (LPM): Well Depth: 18.9 **Final Status** Recommended Pump Rate: 2E+ Well Diameter (cm): 15.2 Recharge Well Primary Water Use: Cooling And A **Water First Found:** 18.3 Pumping Duration (h:m): Static Level: 3

Top:

Bottom:

usign Envelope ID: 0723D92C-88E	1	SAND	0	2.74			
	2	SAND	2.74	17.4			
	3	LIMESTONE	17.4	18.9			
Well ID: 1522348 Construction Date: 1988-06-21	_	455254 g: 5E+06	UTM Zone Positional	_	margin of error :	100 m - 300 m	
		imeter (cm): 15.2 irst Found: 18.3	Water Kin Final Statu Primary W	ıs	FRESH Recharge Well Cooling And A	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	182 2E+ 1:0
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	SAND	0	2.74			
	2	SAND	2.74	17.4			
	3	LIMESTONE	17.4	18.9			
Well ID: 1522551 Construction Date: 1988-08-18	_	455474 g: 5E+06	UTM Zone Positional		margin of error :	100 m - 300 m	
		imeter (cm): 15.2 irst Found: 15.9	Water Kin Final Statu Primary W	ıs	FRESH Recharge Well Cooling And A	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 45 0:45
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	SAND	0	2.74			
	1	SAND	0	2.74			
	2	TILL	2.74	10.7			
	2	TILL	2.74	10.7			
	3	GRAVEL	10.7	14.6			
	3	GRAVEL	10.7	14.6			
	4	LIMESTONE	14.6	19.8			
	4	LIMESTONE	14.6	19.8			
Well ID: 1522552 Construction Date: 1988-08-18	_	455484 g: 5E+06	UTM Zone Positional		margin of error :	100 m - 300 m	
		imeter (cm): 15.2 irst Found: 17.1	Water Kin Final Statu Primary W	ıs	FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 45 0:45
	Layer:	Driller's Description: SAND	Top: 0	Bottom: 2.44			
	1	SAND	0	2.44			
	2	TILL	2.44	9.75			
	2	TILL	2.44	9.75			
	3	GRAVEL	9.75	14.6			
	3	GRAVEL	9.75	14.6			
	4	LIMESTONE	14.6	19.8			
	4	ENVIESTORE					

Well ID: 1529728 **Easting: 455273** UTM Zone 18 Construction Date: 1997-12-22 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m **Water Kind** Not stated Pump Rate (LPM): 227 Well Depth: 23.2 Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate: 23** Primary Water Use: Domestic Water First Found: 17.1 Pumping Duration (h:m): Static Level: Laver: Driller's Description: Top: **Bottom:** TOPSOIL 1 0 1.22 2 CLAY 1.22 2.74 3 CLAY 2.74 10.4 4 SAND 10.4 15.5 5 LIMESTONE 15.5 18.9 6 LIMESTONE 18.9 23.2 Well ID: 1532070 **Easting:** 455043 UTM Zone 18 Construction Date: 2001-07-17 Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m Well Depth: 18.3 **Water Kind** Not stated Pump Rate (LPM): 45 Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate: 45** Primary Water Use: Commerical Pumping Duration (h:m): Water First Found: 16.8 Static Level: Layer: Driller's Description: Top: **Bottom:** 1 SAND 0 1.52 1 SAND 0 1.52 2 CLAY 1.52 11.9 2 CLAY 1.52 11.9 3 **COARSE GRAVEL** 18.3 11.9 3 **COARSE GRAVEL** 11.9 18.3 Well ID: 1533428 Easting: 455042 UTM Zone 18 Construction Date: 2002-12-17 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m Well Depth: 68 **Water Kind** Not stated Pump Rate (LPM): 45 **Final Status** Water Supply **Recommended Pump Rate: 23** Well Diameter (cm): 15.2 Water First Found: 65.8 Primary Water Use: Domestic Pumping Duration (h:m): Static Level: 11 **Driller's Description:** Layer: Top: **Bottom:** 1 **TOPSOIL** 0 1.22 1 **TOPSOIL** 0 1.22 2 **SAND** 1.22 3.66 2 **SAND** 1.22 3.66 3 CLAY 3.66 9.14 3 CLAY 3.66 9.14 4 **SAND** 9.14 17.7 4 **SAND** 9.14 17.7 **LIMESTONE** 5 17.7 48.8 5 LIMESTONE 48.8 17.7 6 **SANDSTONE** 48.8 68

SANDSTONE 6

2

LIMESTONE

12.2

45.7

48.8 68

Well ID: 1533469 **Easting:** 455311 UTM Zone 18 Construction Date: 2002-12-23 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m Well Depth: 102 **Water Kind** Not stated Pump Rate (LPM): 41 **Final Status Water Supply Recommended Pump Rate: 41** Well Diameter (cm): 20.3 Water First Found: 101 Primary Water Use: Domestic Pumping Duration (h:m): Static Level: 15 Layer: **Driller's Description:** Top: **Bottom:** 1 SAND 0 18.9 1 SAND 0 18.9 2 LIMESTONE 18.9 57.3 2 LIMESTONE 18.9 57.3 3 LIMESTONE 57.3 69.2 3 LIMESTONE 57.3 69.2 4 **SANDSTONE** 69.2 102 4 **SANDSTONE** 69.2 102 Well ID: 1534585 **Easting:** 455214 UTM Zone 18 Construction Date: 2004-03-31 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m **Water Kind** Pump Rate (LPM): 84 Not stated Well Depth: 41.8 Well Diameter (cm): **Final Status** Test Hole **Recommended Pump Rate: 36** Primary Water Use: Not Used Pumping Duration (h:m): Water First Found: 41.1 Static Level: **Driller's Description:** Bottom: Layer: Top: 1 CLAY 0 10.1 0 1 CLAY 10.1 SANDSTONE 2 10.1 15.2 2 **SANDSTONE** 10.1 15.2 3 LIMESTONE 15.2 41.8 3 LIMESTONE 15.2 41.8 **Easting:** 454797 Well ID: 1536286 UTM Zone 18 Construction Date: 2006-04-12 Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m **Water Kind** Pump Rate (LPM): Well Depth: 91 45.7 **Final Status Recommended Pump Rate: 91** Well Diameter (cm): Water Supply Primary Water Use: Domestic Pumping Duration (h:m): **Water First Found:** 43.2 **Static Level:** 10 Layer: Driller's Description: **Bottom:** Top: 1 SAND 0 12.2 1 SAND 0 12.2 2 LIMESTONE 12.2 45.7

Well ID: 1536661 Easting: 454807 UTM Zone 18 Construction Date: 2006-09-07 Positional Accuracy: margin of error: 10 - 30 m Northing: 5E+06 **Water Kind** Pump Rate (LPM): Well Depth: 25 Well Diameter (cm): **Final Status** Water Supply **Recommended Pump Rate: 91** Primary Water Use: Domestic Water First Found: Pumping Duration (h:m): 16.8 Static Level: Laver: Driller's Description: Top: **Bottom:** 1 SAND 0 5.18 1 SAND 0 5.18 1 SAND 0 5.18 1 SAND 0 5.18 2 CLAY 5.18 11 2 CLAY 5.18 11 2 CLAY 5.18 11 2 CLAY 5.18 11 3 LIMESTONE 11 25 3 LIMESTONE 11 25 3 LIMESTONE 11 25 3 LIMESTONE 11 25 UTM Zone 18 Well ID: 1536715 **Easting:** 454725 Construction Date: 2006-10-11 Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m **Water Kind** Pump Rate (LPM): 91 Well Depth: 56.7 **Final Status** Water Supply **Recommended Pump Rate: 91** Well Diameter (cm): Water First Found: 54.3 Primary Water Use: Domestic Pumping Duration (h:m): Static Level: 10 **Driller's Description: Bottom:** Layer: Top: 1 CLAY 0 2.74 1 CLAY 0 2.74 2 SAND 2.74 13.1 2 SAND 2.74 13.1 3 LIMESTONE 46.0 13.1 3 LIMESTONE 13.1 46.0 4 **SANDSTONE** 46.0 56.7 **SANDSTONE** 4 46.0 56.7 Well ID: 7040754 **Easting: 454738** UTM Zone 18 Construction Date: 2007-02-12 Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m Well Depth: **Water Kind** Pump Rate (LPM): 91 48.8 **Final Status** Water Supply **Recommended Pump Rate: 91** Well Diameter (cm): Primary Water Use: Domestic Pumping Duration (h:m): **Water First Found:** 19.8 **Static Level:** 10 Top: **Driller's Description: Bottom:** Layer: 1 **SAND** 0 12.5 1 **SAND** 0 12.5 SAND 0 12.5 1

ocusign Envelope ID: 0723D92C-88E	34-462F-AB5I 1	E-45627C898378 SAND	0	12.5			
	2	LIMESTONE	12.5	45.7			
	2	LIMESTONE	12.5	45.7			
	2	LIMESTONE	12.5	45.7			
	2	LIMESTONE	12.5	45.7			
	3	SANDSTONE	45.7	48.8			
	3	SANDSTONE	45.7	48.8			
	3	SANDSTONE	45.7	48.8			
	3	SANDSTONE	45.7	48.8			
Well ID: 7048698 Construction Date: 2007-08-29	Easting: 4 Northing:		UTM Zone		margin of error : 1	0 - 30 m	
	Well Depti Well Diam Water Firs Static Leve	eter (cm): st Found: 45.7	Water Kin Final Statu Primary W	ıs	Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 91 1:0
	Layer: D	Oriller's Description:	Тор:	Bottom:			
	1	SAND	0	12.2			
	1	SAND	0	12.2			
	1	SAND	0	12.2			
	1	SAND	0	12.2			
	2	LIMESTONE	12.2	43			
	2	LIMESTONE	12.2	43			
	2	LIMESTONE	12.2	43			
	2	LIMESTONE	12.2	43			
	3	SANDSTONE	43	48.8			
	3	SANDSTONE	43	48.8			
	3	SANDSTONE	43	48.8			
	3	SANDSTONE	43	48.8			
Well ID: 7104239 Construction Date: 2008-04-28	Easting: 4 Northing:		UTM Zone Positional		margin of error : 1	0 - 30 m	
	Well Depti Well Diam Water Firs Static Leve	eter (cm): st Found:	Water Kin Final Statu Primary W	ıs	Abandoned-Ot	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	
	Layer: D	Oriller's Description:	Top: 0	Bottom: 18.9			
Well ID: 7120715 Construction Date: 2009-03-19	Easting: 4 Northing:		UTM Zone		margin of error : 3	0 m - 100 m	
	Well Depti Well Diam Water Firs Static Leve	eter (cm): st Found:	Water Kin Final Statu Primary W	ıs		Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	82 46 1:
		Oriller's Description:	Тор:	Bottom:			

Well ID: 7130148

Construction Date: 2009-09-22

Easting: 455051 UTM Zone 18

Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m

Well Depth: 4.88 Well Diameter (cm): 5.2 Water First Found: Static Level:

Water Kind Final Status Monitoring an Primary Water Use: Monitoring an Pump Rate (LPM): **Recommended Pump Rate:** Pumping Duration (h:m):

Layer:	Driller's Description:	Top:	Bottom:
1	GRAVEL	0	0.61
1	GRAVEL	0	0.61
1	GRAVEL	0	0.61
1	GRAVEL	0	0.61
2	SAND	0.61	1.5
2	SAND	0.61	1.5
2	SAND	0.61	1.5
2	SAND	0.61	1.5
3	CLAY	1.5	2.74
3	CLAY	1.5	2.74
3	CLAY	1.5	2.74
3	CLAY	1.5	2.74
4	SILT	2.74	4.88
4	SILT	2.74	4.88
4	SILT	2.74	4.88
4	SILT	2.74	4.88

Well ID: 7156846

Construction Date: 2010-12-29

Easting: 454720 UTM Zone 18

Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m

Well Depth: 36.6 Well Diameter (cm): 15.2 Water First Found: 19.8

Static Level: 1

Water Kind	Untested
Final Status	Water Supply
Primary Water Use:	Domestic

Pump Rate (LPM): 91 **Recommended Pump Rate: 91** Pumping Duration (h:m):

Layer:	Driller's Description:	Тор:	Bottom:
1	SAND	0	8.53
1	SAND	0	8.53
1	SAND	0	8.53
2	SAND	8.53	16.5
2	SAND	8.53	16.5
2	SAND	8.53	16.5
3	LIMESTONE	16.5	36.6
3	LIMESTONE	16.5	36.6
3	LIMESTONE	16.5	36.6

4

LIMESTONE

LIMESTONE

12.8

30.2

30.2

32

Well ID: 7157870 Easting: 455093 UTM Zone 18 Construction Date: 2011-01-17 Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m **Water Kind** Untested Pump Rate (LPM): Well Depth: 54.9 **Final Status Recommended Pump Rate: 91** Well Diameter (cm): 15.2 Water Supply Primary Water Use: Domestic Pumping Duration (h:m): Water First Found: 53.0 Static Level: Laver: Driller's Description: Top: **Bottom:** SAND 0 17.1 1 1 SAND 0 17.1 1 SAND 0 17.1 1 SAND 0 17.1 SAND 0 17.1 1 SAND 0 17.1 1 SAND 0 17.1 1 1 SAND 0 17.1 2 LIMESTONE 17.1 54.9 Well ID: 7159015 UTM Zone 18 Easting: 455214 Construction Date: 2011-02-10 Northing: 5E+06 Positional Accuracy: margin of error: 10 - 30 m **Water Kind** Pump Rate (LPM): Well Depth: Well Diameter (cm): **Final Status** Abandoned-Ot **Recommended Pump Rate: Primary Water Use:** Pumping Duration (h:m): Water First Found: Static Level: Layer: Driller's Description: Top: **Bottom:** Well ID: 7183294 UTM Zone 18 **Easting:** 455487 Construction Date: 2012-06-29 Northing: 5E+06 Positional Accuracy: margin of error: 100 m - 300 m Well Depth: **Water Kind** Untested Pump Rate (LPM): 91 32 **Final Status Recommended Pump Rate: 91** Well Diameter (cm): 15.2 Water Supply **Water First Found:** 30.2 Primary Water Use: Domestic Pumping Duration (h:m): **Static Level:** 4 Layer: Driller's Description: Top: **Bottom:** 1 CLAY 0 1.83 2 SAND 1.83 12.8

Well ID: 7183299

Construction Date: 2012-06-29

Easting: 454693 UTM Zone 18

Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m

Water Kind Untested Pump Rate (LPM): Well Depth: 61.3 **Final Status Recommended Pump Rate: 55** Well Diameter (cm): 15.1 Water Supply Water First Found: Primary Water Use: Domestic Pumping Duration (h:m): 56.4

Static Level:

Static Le	evel: 5		
Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	1.52
1	SAND	0	1.52
1	SAND	0	1.52
1	SAND	0	1.52
2	CLAY	1.52	6.40
2	CLAY	1.52	6.40
2	CLAY	1.52	6.40
2	CLAY	1.52	6.40
3	SAND	6.40	18.3
3	SAND	6.40	18.3
3	SAND	6.40	18.3
3	SAND	6.40	18.3
4	LIMESTONE	18.3	34.8
4	LIMESTONE	18.3	34.8
4	LIMESTONE	18.3	34.8
4	LIMESTONE	18.3	34.8
5	SANDSTONE	34.8	54.6
5	SANDSTONE	34.8	54.6
5	SANDSTONE	34.8	54.6
5	SANDSTONE	34.8	54.6
6	SANDSTONE	54.6	56.4
6	SANDSTONE	54.6	56.4
6	SANDSTONE	54.6	56.4
6	SANDSTONE	54.6	56.4
7	SANDSTONE	56.4	61.3
7	SANDSTONE	56.4	61.3
7	SANDSTONE	56.4	61.3
7	SANDSTONE	56.4	61.3

Well ID: 7187406

Construction Date: 2012-09-20

Easting: 455459 UTM Zone 18

Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m

Water Kind Pump Rate (LPM): Well Depth: Untested 29.9 **Final Status** Water Supply **Recommended Pump Rate: 46** Well Diameter (cm): 15.9 Primary Water Use: Domestic Pumping Duration (h:m): Water First Found:

Layer: Driller's Description: Top:

Static Level:

Bottom: TOPSOIL 1 0 2.74

82

55

cusign Envelope ID: 0723D92C-88E	34-462F-AB5 1	E-45627C898378 TOPSOIL	0	2.74		
	2	CLAY	2.74	4.87		
	2	CLAY	2.74	4.87		
	3	SAND	4.87	9.14		
	3	SAND	4.87	9.14		
	4	GRAVEL	9.14	11.3		
	4	GRAVEL	9.14	11.3		
	5	LIMESTONE	11.3	29.9		
	5	LIMESTONE	11.3	29.9		
Well ID: 7187693 Construction Date: 2012-09-22	Easting: 4		UTM Zone Positional		margin of error :	30 m - 100 m
	Well Dept Well Diam Water Firs Static Leve	neter (cm): 15.9 st Found: 24.7	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): 91 Recommended Pump Rate: 91 Pumping Duration (h:m): 1:
	Layer: [Oriller's Description:	Тор:	Bottom:		
	1	SAND	0	11.6		
	1	SAND	0	11.6		
	2	LIMESTONE	11.6	24.7		
	2	LIMESTONE	11.6	24.7		
	3	LIMESTONE	24.7	27.4		
	3	LIMESTONE	24.7	27.4		
Well ID: 7194027 Construction Date: 2012-12-21	Easting: 4 Northing:		UTM Zone Positional		margin of error :	30 m - 100 m
	Well Dept Well Diam Water Firs Static Lev	neter (cm): 15.4 st Found: 33.2	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): 91 Recommended Pump Rate: 91 Pumping Duration (h:m): 1:0
	Layer: [Oriller's Description:	Тор:	Bottom:		
	1	SAND	0	15.2		
	1	SAND	0	15.2		
	1	SAND	0	15.2		
		_				
	1	SAND	0	15.2		
	1 2		0 15.2	15.2 33.2		
		SAND				
	2	SAND LIMESTONE	15.2	33.2		
	2	SAND LIMESTONE LIMESTONE	15.2 15.2	33.2 33.2		
	2 2 2	SAND LIMESTONE LIMESTONE LIMESTONE	15.2 15.2 15.2	33.2 33.2 33.2		
	2 2 2 2	SAND LIMESTONE LIMESTONE LIMESTONE LIMESTONE	15.2 15.2 15.2 15.2	33.2 33.2 33.2 33.2		
	2 2 2 2 3	SAND LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	15.2 15.2 15.2 15.2 33.2	33.2 33.2 33.2 33.2 52.4		
	2 2 2 2 3 3	SAND LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	15.2 15.2 15.2 15.2 33.2 33.2	33.2 33.2 33.2 33.2 52.4 52.4		
	2 2 2 2 3 3 3	SAND LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	15.2 15.2 15.2 15.2 33.2 33.2	33.2 33.2 33.2 33.2 52.4 52.4		

usign Envelope ID: 0723D92C-88E	84-462F-AE 4	35E-45627C898378 SANDSTONE	52.4	58.5			
	4	SANDSTONE	52.4	58.5			
	5	SANDSTONE	58.5	61			
	5	SANDSTONE	58.5	61			
	5	SANDSTONE	58.5	61			
	5	SANDSTONE	58.5	61			
Well ID: 7197490 Construction Date: 2013-02-19	_	: 454766 og: 5E+06	UTM Zone Positional		margin of error :	30 m - 100 m	
		ameter (cm): 14.9 First Found: 36.3	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 91 1:
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	SAND	0	17.4			
	1	SAND	0	17.4			
	1	SAND	0	17.4			
	1	SAND	0	17.4			
	2	LIMESTONE	17.4	36.3			
	2	LIMESTONE	17.4	36.3			
	2	LIMESTONE	17.4	36.3			
	2	LIMESTONE	17.4	36.3			
	3	LIMESTONE	36.3	37.5			
	3	LIMESTONE	36.3	37.5			
	3	LIMESTONE	36.3	37.5			
	3	LIMESTONE	36.3	37.5			
	4	LIMESTONE	37.5	42.7			
	4	LIMESTONE	37.5	42.7			
	4	LIMESTONE	37.5	42.7			
	4	LIMESTONE	37.5	42.7			
Well ID: 7200356 Construction Date: 2013-04-15	_	: 454958 ng: 5E+06	UTM Zone Positional		margin of error :	100 m - 300 m	
		ameter (cm): 14.9 First Found: 46.9	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 91 1:0
	Layer:		Тор:	Bottom:			
	1	SAND	0	13.7			
	1	SAND	0	13.7			
	1	SAND	0	13.7			
	1	SAND	0	13.7			
	2	LIMESTONE	13.7	42.1			
	2	LIMESTONE LIMESTONE	13.7 13.7	42.1 42.1			

	Well Dept Well Dian Water Fir Static Lev	neter (cm): 15.6 st Found: 48.2	Water Kind Final Statu Primary W	ıs	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	55 55 1:0
Well ID: 7204663 Construction Date: 2013-07-16	Easting: 4		UTM Zone Positional		margin of error :	30 m - 100 m	
	8	SANDSTONE	89	91.4			
	8	SANDSTONE	89	91.4			
	7	SANDSTONE	49.1	89			
	7	SANDSTONE	49.1	89			
	6	LIMESTONE	41.5	49.1			
	6	LIMESTONE	41.5	49.1			
	5	SANDSTONE	38.1	41.5			
	5	SANDSTONE	38.1	41.5			
	4	LIMESTONE	18.3	38.1			
	3 4	LIMESTONE	18.3	38.1			
	3	SAND SAND	10.4 10.4	18.3 18.3			
	2	SAND	3.35	10.4			
	2	SAND	3.35	10.4			
	1	SAND	0	3.35			
	1	SAND	0	3.35			
	Layer: I	Driller's Description	і: Тор:	Bottom:			
	Well Dept Well Diam Water Firs Static Lev	neter (cm): 15.6 st Found: 89	Water Kind Final Statu Primary W	ıs	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	55 55 1:0
Construction Date: 2013-07-16	Northing:	5E+06			margin of error :		
Well ID: 7204662	Easting: 4	455133	UTM Zone	18			
	5	SANDSTONE	55.5	61			
	5	SANDSTONE	55.5	61			
	5	SANDSTONE	55.5	61			
	5	SANDSTONE	55.5	61			
	4	SANDSTONE	46.9	55.5			
	4	SANDSTONE SANDSTONE	46.9 46.9	55.5 55.5			
	4	SANDSTONE	46.9	55.5			
	3	SANDSTONE	42.1	46.9			
	3	SANDSTONE	42.1	46.9			
	3	SANDSTONE	42.1	46.9			
	3	SANDSTONE	42.1	46.9			
	2	LIMESTONE	13.7	42.1			
cusign Envelope ID. 0723D32C-001	34-40ZI -AD3						

Top:

Bottom:

Layer: Driller's Description:

Docusign Envelope ID: 0723D92C-88B4-462F-AB5E-45627C898378

	Well Depth: Well Diame Water First Static Level:	ter (cm): 15.6 Found: 21.0	Water Kind Final Status Primary Wa	5	Untested Water Supply Domestic	Pump Rate (LPM): 91 Recommended Pump Rate: 91 Pumping Duration (h:m): 1:
Well ID: 7209271 Construction Date: 2013-10-10	Easting: 45 Northing: 5		UTM Zone Positional A		margin of error :	30 m - 100 m
	7	LIMESTONE	57.6	61		
	7	LIMESTONE	57.6	61		
	7	LIMESTONE	57.6	61		
	7	LIMESTONE	57.6	61		
	6	LIMESTONE	48.2	57.6		
	6	LIMESTONE	48.2	57.6		
	6	LIMESTONE	48.2	57.6		
	6	LIMESTONE	48.2	57.6		
	5	LIMESTONE	40.2	48.2		
	5	LIMESTONE	40.2	48.2		
	5	LIMESTONE	40.2	48.2		
	5	LIMESTONE	40.2	48.2		
	4	LIMESTONE	14.3	40.2		
	4	LIMESTONE	14.3	40.2		
	4	LIMESTONE	14.3	40.2		
	3	SAND LIMESTONE	11.6 14.3	14.3 40.2		
	3	SAND	11.6	14.3		
	3	SAND	11.6	14.3		
	3	SAND	11.6	14.3		
	2	SILT	4.27	11.6		
	2	SILT	4.27	11.6		
	2	SILT	4.27	11.6		
	2	SILT	4.27	11.6		
	1	SAND	0	4.27		
	1	SAND	0	4.27		
	1	SAND	0	4.27		
	1	SAND	0	4.27		

Layer: Driller's Description: Top: **Bottom:** 0 1 SAND 6.1 1 SAND 0 6.1 1 SAND 0 6.1 1 SAND 0 6.1 SAND 0 1 6.1 SAND 0 1 6.1 2 SAND 6.1 14.6

	Well Depth: Well Diame Water First Static Level:	ter (cm): 15.6 Found: 21.0	Water Kind Final Status Primary Wa	5	Untested Water Supply Domestic	Pump Rate (LPM): 91 Recommended Pump Rate: 91 Pumping Duration (h:m): 1:
Well ID: 7209271 Construction Date: 2013-10-10	Easting: 45 Northing: 5		UTM Zone Positional A		margin of error :	30 m - 100 m
	7	LIMESTONE	57.6	61		
	7	LIMESTONE	57.6	61		
	7	LIMESTONE	57.6	61		
	7	LIMESTONE	57.6	61		
	6	LIMESTONE	48.2	57.6		
	6	LIMESTONE	48.2	57.6		
	6	LIMESTONE	48.2	57.6		
	6	LIMESTONE	48.2	57.6		
	5	LIMESTONE	40.2	48.2		
	5	LIMESTONE	40.2	48.2		
	5	LIMESTONE	40.2	48.2		
	5	LIMESTONE	40.2	48.2		
	4	LIMESTONE	14.3	40.2		
	4	LIMESTONE	14.3	40.2		
	4	LIMESTONE	14.3	40.2		
	3	SAND LIMESTONE	11.6 14.3	14.3 40.2		
	3	SAND	11.6	14.3		
	3	SAND	11.6	14.3		
	3	SAND	11.6	14.3		
	2	SILT	4.27	11.6		
	2	SILT	4.27	11.6		
	2	SILT	4.27	11.6		
	2	SILT	4.27	11.6		
	1	SAND	0	4.27		
	1	SAND	0	4.27		
	1	SAND	0	4.27		
	1	SAND	0	4.27		

Layer: Driller's Description: Top: **Bottom:** 0 1 SAND 6.1 1 SAND 0 6.1 1 SAND 0 6.1 1 SAND 0 6.1 SAND 0 1 6.1 SAND 0 1 6.1 2 SAND 6.1 14.6

Oocusign Envelope ID: 0723D92C-88B4-462F-A		<i>.</i>	4-0	
2	SAND	6.1	14.6	
2	SAND	6.1	14.6	
2	SAND	6.1	14.6	
2	SAND	6.1	14.6	
2	SAND	6.1	14.6	
3	LIMESTONE	14.6	21.0	
3	LIMESTONE	14.6	21.0	
3	LIMESTONE	14.6	21.0	
3	LIMESTONE	14.6	21.0	
3	LIMESTONE	14.6	21.0	
3	LIMESTONE	14.6	21.0	
4	LIMESTONE	21.0	42.4	
4	LIMESTONE	21.0	42.4	
4	LIMESTONE	21.0	42.4	
4	LIMESTONE	21.0	42.4	
4	LIMESTONE	21.0	42.4	
4	LIMESTONE	21.0	42.4	
5	SANDSTONE	42.4	44.8	
5	SANDSTONE	42.4	44.8	
5	SANDSTONE	42.4	44.8	
5	SANDSTONE	42.4	44.8	
5	SANDSTONE	42.4	44.8	
5	SANDSTONE	42.4	44.8	
6	SANDSTONE	44.8	51.8	
6	SANDSTONE	44.8	51.8	
6	SANDSTONE	44.8	51.8	
6	SANDSTONE	44.8	51.8	
6	SANDSTONE	44.8	51.8	
6	SANDSTONE	44.8	51.8	
7	SANDSTONE	51.8	54.9	
7	SANDSTONE	51.8	54.9	
7	SANDSTONE	51.8	54.9	
7	SANDSTONE	51.8	54.9	
7	SANDSTONE	51.8	54.9	
7	SANDSTONE	51.8	54.9	

Well ID: 7217217 **Construction Date:** 2014-03-03

Easting: 455459 **UTM Zone** 18

Northing: 5E+06 Positional Accuracy: margin of error : 30 m - 100 m

Well Depth:32.3Water KindUntestedPump Rate (LPM):91Well Diameter (cm):15.2Final StatusWater SupplyRecommended Pump Rate:91Water First Found:30.2Primary Water Use:DomesticPumping Duration (h:m):1:Static Level:6

Layer: Driller's Description: Top: Bottom:

		Well Depth: Well Diameter Water First Fo Static Level:		61 15.1 26.2 8	Water Kin Final Statu Primary W	IS	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate Pumping Duration (h:m):	91 : 91 1:
Well ID: Construct	7228009 tion Date: 2014-09-22	Easting: 4554 Northing: 5E+			UTM Zone Positional	_	margin of error :	30 m - 100 m	
		3	LIMES	STONE	30.2	32.3			
		3	LIMES	STONE	30.2	32.3			
		2	LIMES	STONE	14.3	30.2			
		2	LIMES	STONE	14.3	30.2			
		1	SA	ND	0	14.3			
Docusign Env	elope ID: 0723D92C-88B	1		90376 ND	0	14.3			

evel: 8	•	
Driller's Description:	Тор:	Bottom:
SAND	0	15.2
LIMESTONE	15.2	26.2
LIMESTONE	26.2	40.8
LIMESTONE	40.8	54.9
SANDSTONE	54.9	59.1
	Driller's Description: SAND SAND SAND SAND SAND SAND SAND LIMESTONE SANDSTONE SANDSTONE SANDSTONE	Driller's Description: Top: SAND 0 LIMESTONE 15.2 LIMESTONE 15.2 LIMESTONE 26.2 LIMESTONE 26.2 LIMESTONE 26.2 LIMESTONE 40.8 LIMESTONE 40.8

cusign Envelope ID: 0723D92C-88E	34-462F-AB 5	35E-45627C898378 SANDSTONE	54.9	59.1			
	6	SANDSTONE	59.1	61			
	6	SANDSTONE	59.1	61			
	6	SANDSTONE	59.1	61			
	6	SANDSTONE	59.1	61			
	6	SANDSTONE	59.1	61			
	6	SANDSTONE	59.1	61			
Well ID: 7230319 Construction Date: 2014-10-29	_	455162 g: 5E+06	UTM Zone Positional		margin of error :	30 m - 100 m	
		meter (cm): 15.2 irst Found: 88.7	Water Kind Final Statu Primary W		Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	55 55 1:
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	SAND	0	9.14			
	1	SAND	0	9.14			
	2	GRAVEL	9.14	17.7			
	2	GRAVEL	9.14	17.7			
	3	LIMESTONE	17.7	43			
	3	LIMESTONE	17.7	43			
	4	LIMESTONE	43	48.2			
	4	LIMESTONE	43	48.2			
	5	SANDSTONE	48.2	88.7			
	5	SANDSTONE	48.2	88.7			
	6	SANDSTONE	88.7	90.5			
	6	SANDSTONE	88.7	90.5			
	_	455080 g: 5E+06	UTM Zone Positional		margin of error :	30 m - 100 m	
	Northing Well Dep Well Dia	g: 5E+06 oth: 61 meter (cm): 15.1 irst Found: 41.8	Positional Water Kind Final Statu	Accuracy: d	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36 36 1:0
	Northing Well Deg Well Dia Water Fi Static Le Layer:	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description:	Positional Water Kind Final Statu Primary W	Accuracy: d us Jater Use: Bottom:	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Dep Well Dia Water Fi Static Le	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4	Positional Water Kind Final Statu Primary W	Accuracy: d us /ater Use:	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Deg Well Dia Water Fi Static Le Layer:	g: 5E+06 oth: 61 meter (cm): 15.1 irst Found: 41.8 ovel: 4 Driller's Description: CLAY CLAY	Positional Water Kind Final Statu Primary W Top:	Accuracy: d us /ater Use: Bottom: 16.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Dep Well Dia Water Fi Static Le Layer: 1 1	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY	Positional Water Kine Final Statu Primary W Top: 0 0 0	d us /ater Use: Bottom: 16.8 16.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Deg Well Dia Water Fi Static Le Layer: 1 1 1	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY CLAY	Positional Water Kind Final Statu Primary W Top: 0 0 0 0	Accuracy: d is /ater Use: Bottom: 16.8 16.8 16.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Deg Well Dia Water Fi Static Le Layer: 1 1 2	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY CLAY CLAY LIMESTONE	Positional Water Kind Final Statu Primary W Top: 0 0 0 16.8	Accuracy: d us /ater Use: Bottom: 16.8 16.8 16.8 41.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Deg Well Dia Water Fi Static Le Layer: 1 1 2 2	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY CLAY CLAY LIMESTONE LIMESTONE	Positional Water Kine Final Statu Primary W Top: 0 0 0 0 16.8 16.8	Accuracy: d is /ater Use: Bottom: 16.8 16.8 16.8 41.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Der Well Dia Water Fi Static Le Layer: 1 1 2 2 2	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY CLAY CLAY LIMESTONE	Positional Water Kine Final Statu Primary W Top: 0 0 0 0 16.8 16.8	Accuracy: d us /ater Use: Bottom: 16.8 16.8 16.8 41.8 41.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
	Northing Well Deg Well Dia Water Fi Static Le Layer: 1 1 2 2 2 2	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE LIMESTONE	Positional Water Kine Final Statu Primary W Top: 0 0 0 0 16.8 16.8 16.8 16.8	Accuracy: d is /ater Use: Bottom: 16.8 16.8 16.8 41.8 41.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36
Well ID: 7240506 Construction Date: 2015-04-24	Northing Well Der Well Dia Water Fi Static Le Layer: 1 1 2 2 2	g: 5E+06 pth: 61 meter (cm): 15.1 irst Found: 41.8 evel: 4 Driller's Description: CLAY CLAY CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE	Positional Water Kine Final Statu Primary W Top: 0 0 0 0 16.8 16.8	Accuracy: d us /ater Use: Bottom: 16.8 16.8 16.8 41.8 41.8	Untested Water Supply	Pump Rate (LPM): Recommended Pump Rate:	36

0 m				
Rate (LPM): mended Pump Rate: ng Duration (h:m):	91 : 91 1:			
UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100 m				
	68 : 68 : 10			
0 m Rate (LPM): mended Pump Rate: ng Duration (h:m):				
Rate (LPM): mended Pump Rate:				
Rate (LPM): mended Pump Rate:				
Ra m				

cusign Envelope ID: 0723D92C-88I	34-462F-AB5I 1	E-45627C898378 CLAY	0	15.9			
	2	LIMESTONE	15.9	26.2			
	2	LIMESTONE	15.9	26.2			
	2	LIMESTONE	15.9	26.2			
	2	LIMESTONE	15.9	26.2			
	3	LIMESTONE	26.2	46.9			
	3	LIMESTONE	26.2	46.9			
	3	LIMESTONE	26.2	46.9			
	3	LIMESTONE	26.2	46.9			
	4	LIMESTONE	46.9	48.8			
	4	LIMESTONE	46.9	48.8			
	4	LIMESTONE	46.9	48.8			
	4	LIMESTONE	46.9	48.8			
	•						
Well ID: 7243033 Construction Date: 2015-06-15	Easting: 4 Northing:		UTM Zone Positional		margin of error : 3	30 m - 100 m	
	Well Depti Well Diam Water Firs Static Leve	eter (cm): 15.2 t Found: 57.9	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): 91 Recommended Pump Rate: 91 Pumping Duration (h:m): 1:	
	Layer: D	riller's Description:	Тор:	Bottom:			
	1	CLAY	0	14.6			
	1	CLAY CLAY	0	14.6 14.6			
	1	CLAY	0	14.6			
	1	CLAY CLAY	0	14.6 14.6			
	1 1 1	CLAY CLAY CLAY	0 0 0	14.6 14.6 14.6			
	1 1 1 2	CLAY CLAY CLAY LIMESTONE	0 0 0 14.6	14.6 14.6 14.6 48.8			
	1 1 1 2 2	CLAY CLAY CLAY LIMESTONE LIMESTONE	0 0 0 14.6 14.6	14.6 14.6 14.6 48.8 48.8			
	1 1 1 2 2 2	CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE	0 0 0 14.6 14.6 14.6	14.6 14.6 14.6 48.8 48.8			
	1 1 1 2 2 2 2	CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE LIMESTONE	0 0 14.6 14.6 14.6	14.6 14.6 14.6 48.8 48.8 48.8			
	1 1 2 2 2 2 2 3	CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	0 0 14.6 14.6 14.6 14.6 48.8	14.6 14.6 14.6 48.8 48.8 48.8 57.9			
	1 1 2 2 2 2 2 3 3	CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	0 0 14.6 14.6 14.6 14.6 48.8 48.8	14.6 14.6 14.6 48.8 48.8 48.8 57.9			
	1 1 2 2 2 2 2 3 3 3	CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	0 0 14.6 14.6 14.6 14.6 48.8 48.8	14.6 14.6 14.6 48.8 48.8 48.8 57.9 57.9			
	1 1 2 2 2 2 2 3 3 3 3	CLAY CLAY CLAY LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE LIMESTONE	0 0 14.6 14.6 14.6 14.6 48.8 48.8 48.8	14.6 14.6 14.6 48.8 48.8 48.8 57.9 57.9 57.9			
	1 1 2 2 2 2 3 3 3 3 4	CLAY CLAY CLAY CLAY LIMESTONE	0 0 14.6 14.6 14.6 48.8 48.8 48.8 57.9	14.6 14.6 14.6 48.8 48.8 48.8 57.9 57.9 57.9 57.9			
	1 1 2 2 2 2 2 3 3 3 3 4 4	CLAY CLAY CLAY LIMESTONE	0 0 14.6 14.6 14.6 48.8 48.8 48.8 57.9	14.6 14.6 14.6 48.8 48.8 48.8 57.9 57.9 57.9 57.9 63.4 63.4			

5

5

5

LIMESTONE

LIMESTONE

LIMESTONE

LIMESTONE

63.4

63.4

63.4

63.4

65.5

65.5

65.5

65.5

Well ID: 7252399

Construction Date: 2015-11-17

Easting: 455519 UTM Zone 18

Positional Accuracy: margin of error: 30 m - 100 m Northing: 5E+06

Water Kind Untested Pump Rate (LPM): Well Depth: 25 **Final Status Recommended Pump Rate: 91** Well Diameter (cm): 15.2 Water Supply Primary Water Use: Domestic Pumping Duration (h:m): Water First Found: 17.7 Static Level:

Laver: Driller's Description: Top: **Bottom:** SAND 0 9.14 1 1 SAND 0 9.14 1 **SAND** 0 9.14

1 SAND 0 9.14 2 LIMESTONE 9.14 17.7 LIMESTONE 2 9.14 17.7 2 LIMESTONE 9.14 17.7 2 LIMESTONE 9.14 17.7 3 LIMESTONE 17.7 22.9 3 22.9 LIMESTONE 17.7 3 LIMESTONE 22.9 17.7 3 17.7 22.9

LIMESTONE 4 LIMESTONE 22.9 25 4 LIMESTONE 22.9 25 LIMESTONE 4 22.9 25

Static Level:

4 LIMESTONE 22.9 25

Easting: 455399 UTM Zone 18 Well ID: 7252400 Construction Date: 2015-11-17 Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m

2

Water Kind Untested Pump Rate (LPM): 91 Well Depth: 48.8 Well Diameter (cm): 15.9 **Final Status** Water Supply **Recommended Pump Rate: 91** Primary Water Use: Domestic Pumping Duration (h:m): Water First Found: 44.2

Layer: **Driller's Description:** Top: **Bottom:** 1 SAND 0 8.84 1 SAND 0 8.84 1 SAND 0 8.84 SAND 8.84 1 0 2 LIMESTONE 44.2 8.84 2 LIMESTONE 8.84 44.2 2 LIMESTONE 8.84 44.2 2 LIMESTONE 8.84 44.2 LIMESTONE 46.9 3 44.2 3 LIMESTONE 44.2 46.9 3 LIMESTONE 46.9 44.2 3 LIMESTONE 44.2 46.9 4 LIMESTONE 46.9 48.8

4 LIMESTONE 46.9 48.8 4 LIMESTONE 46.9 48.8 4 LIMESTONE 46.9 48.8

Well ID: 7255451

Construction Date: 2016-01-06

Easting: 455289 **UTM Zone** 18

Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m

Well Depth: 64.0
Well Diameter (cm): 15.6
Water First Found: 62.5
Static Level: 8

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1:0

Static Le	evei: 8		
Layer:	Driller's Description:	Тор:	Bottom:
1	CLAY	0	15.9
1	CLAY	0	15.9
1	CLAY	0	15.9
1	CLAY	0	15.9
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
3	LIMESTONE	48.8	49.1
3	LIMESTONE	48.8	49.1
3	LIMESTONE	48.8	49.1
3	LIMESTONE	48.8	49.1
4	LIMESTONE	49.1	62.5
4	LIMESTONE	49.1	62.5
4	LIMESTONE	49.1	62.5
4	LIMESTONE	49.1	62.5
5	LIMESTONE	62.5	64.0
5	LIMESTONE	62.5	64.0
5	LIMESTONE	62.5	64.0
5	LIMESTONE	62.5	64.0

Well ID: 7265398

Construction Date: 2016-06-21

Easting: 455315 **UTM Zone** 18

Northing: 5E+06 Positional Accuracy: margin of error: 30 m - 100 m

Well Depth: 73.2 Well Diameter (cm): 15.9 Water First Found: 70.7 Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1:0

Static Level: 8

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9

	1	CLAY	0	3.05		
	1	CLAY	0	3.05		
	1	CLAY	0	3.05		
	-	Oriller's Description:	-	Bottom:		
	Well Dept Well Diam Water Firs Static Leve	neter (cm): 15.9 st Found: 64.9	Water Kin Final Statu Primary W	ıs	Untested Water Supply Domestic	Pump Rate (LPM): 91 Recommended Pump Rate: 91 Pumping Duration (h:m): 1:0
Well ID: 7296379 Construction Date: 2017-10-03	Easting: 4 Northing:			Accuracy:	margin of error :	
	6	SANDSTONE	70.7	73.2		
	6	SANDSTONE	70.7	73.2		
	6	SANDSTONE	70.7	73.2		
	6	SANDSTONE	70.7	73.2		
	6	SANDSTONE	70.7	73.2		
	6	SANDSTONE	70.7	73.2		
	5	SANDSTONE	59.1	70.7		
	5	SANDSTONE	59.1	70.7		
	5	SANDSTONE	59.1	70.7		
	5	SANDSTONE	59.1	70.7		
	5	SANDSTONE	59.1	70.7		
	5	SANDSTONE	59.1	70.7		
	4	SANDSTONE	55.5	59.1		
	4	SANDSTONE	55.5	59.1		
	4	SANDSTONE	55.5	59.1		
	4	SANDSTONE	55.5	59.1		
	4	SANDSTONE	55.5	59.1		
	4	SANDSTONE	55.5	59.1		
	3	SANDSTONE	48.8	55.5		
	3	SANDSTONE	48.8	55.5		
	3	SANDSTONE	48.8	55.5		
	3	SANDSTONE	48.8	55.5		
	3	SANDSTONE	48.8	55.5		
	3	SANDSTONE	48.8	55.5		
	2 2	LIMESTONE LIMESTONE	15.9 15.9	48.8 48.8		
	2	LIMESTONE	15.9	48.8		
	2	LIMESTONE	15.9	48.8		
	2	LIMESTONE	15.9	48.8		
	2	LIMESTONE	15.9	48.8		
usign Envelope ID. 0123D32C-00L	34-4021 -AD3	L- 4 3021 0080310				

GRAVEL

3.05

17.7

Docusign Envelope ID: 0723D92C-88B4-462F-AB5E-45627C898378

cusign Envelope ID: 0723D92C-88E	34-462F-AB5E- 2	45627C898378 GRAVEL	3.05	17.7			
	2	GRAVEL	3.05	17.7			
	2	GRAVEL	3.05	17.7			
	3	LIMESTONE	17.7	46.0			
	3	LIMESTONE	17.7	46.0			
	3	LIMESTONE	17.7	46.0			
	3	LIMESTONE	17.7	46.0			
	4	SANDSTONE	46.0	63.7			
	4	SANDSTONE	46.0	63.7			
	4	SANDSTONE	46.0	63.7			
	4	SANDSTONE	46.0	63.7			
	5	SANDSTONE	63.7	64.9			
	5	SANDSTONE	63.7	64.9			
	5	SANDSTONE	63.7	64.9			
	5	SANDSTONE	63.7	64.9			
	6	SANDSTONE	64.9	67.1			
	6	SANDSTONE	64.9	67.1			
	6	SANDSTONE	64.9	67.1			
	6	SANDSTONE	64.9	67.1			
Well ID: 7301342 Construction Date: 2017-12-14	Easting: 455482 Northing: 5E+06		UTM Zone Positional		margin of error :	30 m - 100 m	
	Well Depth: Well Diamet Water First Static Level:	er (cm): 15.9 Found: 35.4	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 91 1:0
	Layer: Dri	ller's Description:	Тор:	Bottom:			
	1	SAND	0	10.1			
	1	SAND	0	10.1			
	1	SAND	0	10.1			
	1	SAND	0	10.1			
	2	LIMESTONE	10.1	36.6			
	2	LIMESTONE	10.1	36.6			
	2	LIMESTONE	10.1	36.6			
	2	LIMESTONE	10.1	36.6			
Well ID: 7318099 Construction Date: 2018-09-10	Easting: 455258 Northing: 5E+06			UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100 m		30 m - 100 m	
	_	E+06	. oomona.				
	Northing: 5 Well Depth:	61 ter (cm): 15.2 Found: 57	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 91 1:
	Northing: 5 Well Depth: Well Diamet Water First Static Level: Layer: Dri	61 ter (cm): 15.2 Found: 57 8 Iller's Description:	Water Kind Final Statu Primary W Top:	s ater Use: Bottom:	Water Supply Domestic	Recommended Pump Rate:	91
	Northing: 5 Well Depth: Well Diamet Water First Static Level:	61 ser (cm): 15.2 Found: 57 8	Water Kind Final Statu Primary W	s ater Use:	Water Supply Domestic	Recommended Pump Rate:	91

Docusign Envelope ID: 0723D92C-88B4-462F-AB5E-45627C898378

Docusign Envelope ID: 0723D92C-88B							
	2	SAND	4.27	15.2			
	2	SAND	4.27	15.2			
	3	LIMESTONE	15.2	41.8			
	3	LIMESTONE	15.2	41.8			
	4	SANDSTONE	41.8	45.7			
	4	SANDSTONE	41.8	45.7			
	5	SANDSTONE	45.7	57			
	5	SANDSTONE	45.7	57			
	6	SANDSTONE	57	61			
	6	SANDSTONE	57	61			
Well ID: 7324334 Construction Date: 2018-12-11	_	455498 g: 5E+06	UTM Zone Positional		margin of error : 3	30 m - 100 m	
		imeter (cm): 15.9 irst Found: 15.2	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	46 46 1:
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	SAND	0	7.31			
	1	SAND	0	7.31			
	2	SAND	7.31	11.3			
	2	SAND	7.31	11.3			
	3	LIMESTONE	11.3	15.2			
	3	LIMESTONE	11.3	15.2			
	4	LIMESTONE	15.2	18.3			
	4	LIMESTONE	15.2	18.3			
Well ID: 7336839 Construction Date: 2019-07-10	_	455307 g: 5E+06	UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100			30 m - 100 m	
		imeter (cm): 15.9 irst Found: 22	Water Kind Final Statu Primary W	s	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	91 91 1:0
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	SAND	0	12.5			
	1	SAND	0	12.5			
	2	LIMESTONE	12.5	22			
	2	LIMESTONE	12.5	22			

LIMESTONE

22

25

Well ID: 7341123

Construction Date: 2019-09-06

Easting: 455360

Northing: 5E+06

UTM Zone 18

Positional Accuracy: margin of error: 30 m - 100 m

Water Kind Well Depth:

Final Status Water Supply Pump Rate (LPM):

Recommended Pump Rate:

Water First Found:

Well Diameter (cm):

Primary Water Use:

Pumping Duration (h:m):

Static Level:

Layer: Driller's Description:

Top: **Bottom:**

Well ID: 7357357

Construction Date: 2020-04-28

Easting: 455292 Northing: 5E+06 UTM Zone 18

Positional Accuracy: margin of error: 30 m - 100 m

Well Depth: 24.7 Well Diameter (cm): 15.6 Water First Found:

Water Kind Final Status 22.9

Untested Water Supply Primary Water Use: Domestic

Pump Rate (LPM): **Recommended Pump Rate: 91** Pumping Duration (h:m):

Static Level:

Layer: Driller's Description: Top: 1 CLAY 1 CLAY

LIMESTONE

LIMESTONE

15.2 24.7

Bottom:

15.2

15.2

24.7

Well ID: 7364564

Construction Date: 2020-08-13

Easting: 455536

2

2

Northing: 5E+06

UTM Zone 18

0

0

15.2

Positional Accuracy: margin of error: 30 m - 100 m

Well Depth:

Well Diameter (cm): **Water First Found:**

Water Kind Final Status

Top:

Primary Water Use:

Pump Rate (LPM):

Recommended Pump Rate: Pumping Duration (h:m):

Static Level:

Layer: Driller's Description:

Bottom:



Hydrogeological Assessment Cassidy EW Construction Consultant Ltd.

Ref. No.: 17281-002

2024-04-29

Well Use Survey Summary Report

Number	Street	Spoke to employee	Participated in program	In-person survey results	Paper survey results
1368	Greely Lane	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Survey not returned
1375	Greely Lane	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Returned survey indicates the well is approx 40ft deep and installe 31 years ago. No water quality or quanity issues are noted.
1380	Greely Lane	No	No	Left letter in mailbox	Returned survey indicates water is obtained from a well constructed in approx. 1989, and is used for shower, septic, and vehicle washing. A sulfur smell is noted for water quality.
6906	McKeown Drive	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Survey not returned
6876	McKeown Drive	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Survey not returned



Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
November 11, 2024

	App	endix E
Groundwater Qua	ality Lab	Results





Final Report

C.O.C.: G 107579 REPORT No: 24-010898 - Rev. 1

Report To:

Cambium Environmental - Kingston

625 Fortune Crescent

#1

Kingston, ON K7P 0L5

Attention: Kyle Horner

CADUCEON Environmental Laboratories

2378 Holly Lane

Ottawa, ON K1V 7P1

DATE RECEIVED: 2024-Apr-22 CUSTOMER PROJECT: 17280-002

2024-Jul-30 P.O. NUMBER:

DATE REPORTED: 2024-Jul-30 SAMPLE MATRIX: Ground Water

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
Anions (Liquid)	1	OTTAWA	PCURIEL	2024-Apr-24	A-IC-01	SM 4110B
BOD5 (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	BOD-001	SM 5210B
Cond/pH/Alk Auto (Liquid)	1	OTTAWA	SBOUDREAU	2024-Apr-22	COND-02/PH-02/A	SM 2510B/4500H/
					LK-02	2320B
Cyanide Total (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-23	CN-001	SM 4500-CN-E
Formaldehyde (Subcontracted)	1	TESTMARK	SISLAM	2024-Apr-26		Subcontracted
Ion Balance (Calc.)	1	OTTAWA	ASCHNEIDER		CP-028	MECP E3196
Chromium VI (Liquid)	1	OTTAWA	STAILLON	2024-Apr-25	D-CRVI-01	MECP E3056
ICP/MS Total (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Apr-24	D-ICPMS-01	EPA 6020
ICP/OES Total (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-29	D-ICP-01	SM 3120B
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-24	D-ICP-01	SM 3120B
Mercury (Liquid)	1	OTTAWA	TBENNETT	2024-Apr-24	D-HG-02	SM 3112B
NDMA Liquid (Subcontract)	1	SGS_LAKEFIELD	SISLAM	2024-May-30		Subcontracted
Ammonia (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	NH3-001	SM 4500NH3
Nonylphenols (Subcontracted)	1	SGS_LAKEFIELD	SISLAM	2024-Apr-30		Subcontracted
OC Pesticides (Liquid)	1	KINGSTON	CSUMMERHAYS	2024-Apr-23	PESTCL-001	EPA 8081
Oil & Grease (Liquid)	1	KINGSTON	MLANE	2024-Apr-25	O&G-001	SM 5520
Phenols (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-25	PHEN-01	MECP E3179
Sulphide (Liquid)	1	KINGSTON	EHINCH	2024-Apr-23	H2S-001	SM 4500-S2
SVOC - Semi-Volatiles (Liquid)	1	KINGSTON	EASIEDU	2024-Apr-24	NAB-W-001	EPA 8270D
TP & TKN (Liquid)	1	KINGSTON	KDIBBITS	2024-Apr-29	TPTKN-001	MECP E3516.2
TSS (Liquid)	1	KINGSTON	MCLOSS	2024-Apr-23	TSS-001	SM 2540D
Turbidity (Liquid)	1	OTTAWA	STAILLON	2024-Apr-23	A-TURB-01	SM 2130B
VOC-Volatiles Full (Water)	1	RICHMOND_HILL	FLENA	2024-Apr-24	C-VOC-02	EPA 8260

R.L. = Reporting Limit

NC = Not Calculated

Test methods may be modified from specified reference method unless indicated by an *

Final Report REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D. Date Collected	24-010898-1 2024-Apr-19
Parameter	Units	R.L.	Limits		-
Alkalinity(CaCO3) to pH4.5	mg/L	5			283
рН @25°C	pH units	-	11.0, 9.0	SAN, STORM	7.85
Turbidity	NTU	0.1			7070
Fluoride	mg/L	0.1	10	SAN	<0.1
Sulphate	mg/L	1	1500	SAN	84
BOD5	mg/L	3	300, 25.0	SAN, STORM	3
Total Suspended Solids	mg/L	3	350, 15.0	SAN, STORM	9480
Phosphorus (Total)	mg/L	0.01	10, 0.4	SAN, STORM	8.72
Total Kjeldahl Nitrogen	mg/L	0.1	100	SAN	6.3
Ammonia (N)-Total (NH3+NH4)	mg/L	0.05			0.15
Ammonia (N)-unionized	mg/L	0.01			<0.01
Sulphide	mg/L	0.01	2	SAN	0.01
Cyanide (Total)	mg/L	0.005	2, 0.02	SAN, STORM	<0.005
Phenolics	mg/L	0.001	1, 0.008	SAN, STORM	<0.001
Hardness (as CaCO3)	mg/L	0.02			368
Aluminum	mg/L	0.01			0.07
Barium	mg/L	0.001			0.165
Calcium	mg/L	0.02			105
Iron	mg/L	0.005			0.020
Magnesium	mg/L	0.02			25.6
Tungsten	mg/L	0.01			<0.01

Final Report REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D. Date Collected	24-010898-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19 -
Zinc	mg/L	0.005			<0.005
Zirconium	mg/L	0.003			<0.003
Hardness (as CaCO3)	mg/L	-			789
Aluminum (Total)	mg/L	0.01	50	SAN	0.03
Bismuth (Total)	mg/L	0.02	5	SAN	<0.02
Boron (Total)	mg/L	0.005	25	SAN	0.028
Cadmium (Total)	mg/L	0.005	0.02, 0.008	SAN, STORM	<0.005
Calcium (Total)	mg/L	0.02			97.7
Chromium (Total)	mg/L	0.002	5, 0.08	SAN, STORM	<0.002
Cobalt (Total)	mg/L	0.005	5	SAN	<0.005
Copper (Total)	mg/L	0.002	3, 0.04	SAN, STORM	0.008
Iron (Total)	mg/L	0.005			<0.005
Lead (Total)	mg/L	0.02	5, 0.12	SAN, STORM	<0.02
Magnesium (Total)	mg/L	0.02			27.3
Manganese (Total)	mg/L	0.001	0.05, 5	STORM, SAN	0.003
Molybdenum (Total)	mg/L	0.01	5	SAN	<0.01
Nickel (Total)	mg/L	0.01	3, 0.08	SAN, STORM	<0.01
Silver (Total)	mg/L	0.005	5, 0.12	SAN, STORM	<0.005
Tin (Total)	mg/L	0.05	5	SAN	<0.05
Titanium (Total)	mg/L	0.005	5	SAN	<0.005
Tungsten (Total)	mg/L	0.01			<0.01

Final Report REPORT No: 24-010898 - Rev. 1

				Client I.D.	ВН106
				Sample I.D. Date Collected	24-010898-1 2024-Apr-19
Parameter	Units	R.L.	Limits		-
Vanadium (Total)	mg/L	0.005	5	SAN	<0.005
Zinc (Total)	mg/L	0.005	3, 0.04	SAN, STORM	<0.005
Zirconium (Total)	mg/L	0.003			<0.003
Antimony (Total)	mg/L	0.0001	5	SAN	0.0007
Arsenic (Total)	mg/L	0.0001	0.02, 1	STORM, SAN	0.0275
Beryllium (Total)	mg/L	0.0001			0.0032
Cadmium (Total)	mg/L	0.000015	0.008	STORM	0.00112
Chromium (Total)	mg/L	0.001	0.08	STORM	0.249
Cobalt (Total)	mg/L	0.0001			0.103
Copper (Total)	mg/L	0.0001	0.04	STORM	0.301
Lead (Total)	mg/L	0.00002	0.12	STORM	0.0768
Molybdenum (Total)	mg/L	0.0001			0.0076
Nickel (Total)	mg/L	0.0002	0.08	STORM	0.189
Selenium (Total)	mg/L	0.001	0.02, 5	STORM, SAN	<0.001
Silver (Total)	mg/L	0.0001	0.12	STORM	0.0011
Thallium (Total)	mg/L	0.00005			0.00182
Uranium (Total)	mg/L	0.00005			0.0114
Vanadium (Total)	mg/L	0.0001			0.327
Chromium (VI)	mg/L	0.01			<0.01
Mercury	mg/L	0.00002	0.001, 0.0004	SAN, STORM	<0.00002
Anion Sum	meq/L	-			16.6

Final Report

REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
Permuta	11.9.	ъ.	12	Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits		-
Cation Sum	meq/L	-			15.3
% Difference	%	-			4.03
lon Ratio	-	-			1.08
Sodium Adsorption Ratio	-	-			4.28
TDS (Ion Sum Calc)	mg/L	1			893
TDS(calc.)/EC(actual)	-	-			0.540
Conductivity Calc	µmho/cm	-			1590
Conductivity Calc / Conductivity	-	-			0.959
Langelier Index(25°C)	-	-			0.800
Saturation pH (25°C)	-	-			7.05
pH (Client Data)	pH units	-			6.97
Temperature (Client Data)	°C	-			9.9

Final Report REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19 -
Benzene	mg/L	0.0005	0.01, 0.002	SAN, STORM	<0.0005
Bromodichloromethane	mg/L	0.002	0.35	SAN	<0.002
Bromoform	mg/L	0.005	0.63	SAN	<0.005
Bromomethane	mg/L	0.0005	0.11	SAN	<0.0005
Carbon Tetrachloride	mg/L	0.0002	0.057	SAN	<0.0002
Chlorobenzene	mg/L	0.0005	0.057	SAN	<0.0005
Chloroethane	mg/L	0.003	0.27	SAN	<0.003
Chloroform	mg/L	0.001	0.08, 0.002	SAN, STORM	<0.001
Chloromethane (Methyl Chloride)	mg/L	0.002	0.19	SAN	<0.002
Dibromochloromethane	mg/L	0.002	0.057	SAN	<0.002
Ethylene Dibromide	mg/L	0.0002	0.028	SAN	<0.0002
Dichlorobenzene,1,2-	mg/L	0.0005	0.088, 0.0056	SAN, STORM	<0.0005
Dichlorobenzene,1,3-	mg/L	0.0005	0.036	SAN	<0.0005
Dichlorobenzene,1,4-	mg/L	0.0005	0.017, 0.0068	SAN, STORM	<0.0005
Dichloroethane,1,1-	mg/L	0.0005	0.2	SAN	<0.0005
Dichloroethane,1,2-	mg/L	0.0005	0.21	SAN	0.0007
Dichloroethylene,1,1-	mg/L	0.0005	0.04	SAN	<0.0005
Dichloroethylene,1,2-cis-	mg/L	0.0005	0.2, 0.0056	SAN, STORM	<0.0005
Dichloroethylene,1,2-trans-	mg/L	0.0005	0.2	SAN	<0.0005
Dichloropropane,1,2-	mg/L	0.0005	0.85	SAN	<0.0005
Dichloropropene,1,3-cis-	mg/L	0.0005	0.07	SAN	<0.0005

Final Report REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
Damanatan	11-24-	D.I	Limite	Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits		-
Dichloropropene,1,3-trans-	mg/L	0.0005	0.07, 0.0056	SAN, STORM	<0.0005
Ethylbenzene	mg/L	0.0005	0.057, 0.002	SAN, STORM	<0.0005
Dichloromethane (Methylene Chloride)	mg/L	0.005	0.211, 0.0052	SAN, STORM	<0.005
Styrene	mg/L	0.0005	0.04	SAN	<0.0005
Tetrachloroethane,1,1,2,2-	mg/L	0.0005	0.04, 0.017	SAN, STORM	<0.0005
Tetrachloroethylene	mg/L	0.0005	0.05, 0.0044	SAN, STORM	<0.0005
Toluene	mg/L	0.0005	0.08, 0.002	SAN, STORM	<0.0005
Trichloroethane,1,1,1-	mg/L	0.0005	0.054	SAN	<0.0005
Trichloroethane,1,1,2-	mg/L	0.0005	0.8	SAN	<0.0005
Trichloroethylene	mg/L	0.0005	0.054, 0.0076	SAN, STORM	<0.0005
Trichlorofluoromethane (Freon 11)	mg/L	0.005	0.02	SAN	<0.005
Trimethylbenzene,1,3,5-	mg/L	0.0001	0.003	SAN	<0.0001
Vinyl Chloride	mg/L	0.0002	0.4	SAN	<0.0002
Xylene, m,p-	μg/L	1			<1
Xylene, m,p,o-	mg/L	0.0011	0.32, 0.0044	SAN, STORM	<0.0011
Xylene, o-	μg/L	0.5			<0.5
Oil & Grease (Total)	mg/L	1.0			1.7
Oil and Grease (Mineral)	mg/L	1.0	15	SAN	<1.0
Oil and Grease (Anim/Veg)	mg/L	1.0	150	SAN	1.4

Final Report REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
P	11.26	ъ.	1116.	Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits		-
Acenaphthene	μg/L	0.05			<0.05
Acenaphthylene	μg/L	0.05			<0.05
Anthracene	μg/L	0.05			<0.05
Benzo[a]anthracene	μg/L	0.05			<0.05
Benzo(a)pyrene	μg/L	0.01			<0.01
Benzo(b)fluoranthene	μg/L	0.05			<0.05
Benzo(b+k)fluoranthene	μg/L	0.1			<0.1
Benzo(g,h,i)perylene	μg/L	0.05			<0.05
Benzo(k)fluoranthene	µg/L	0.05			<0.05
Butyl Benzyl Phthalate	mg/L	0.001	0.017	SAN	<0.001
Bis(2-Chloroethoxy)methane	mg/L	0.002	0.036	SAN	<0.002
Bis(2-ethylhexyl) Phthalate	mg/L	0.005	0.28	SAN	<0.005
Chrysene	μg/L	0.05			<0.05
Dibenzo(a,h)anthracene	μg/L	0.05			<0.05
Di-n-Butyl Phthalate	mg/L	0.0010	0.057	SAN	<0.0010
Dichlorophenol,2,4-	mg/L	0.0002	0.044	SAN	<0.0002
Diethyl Phthalate	mg/L	0.0010	0.2	SAN	<0.0010
Di-n-Octyl Phthalate	mg/L	0.0010	0.03	SAN	<0.0010
Fluoranthene	mg/L	0.00005	0.059	SAN	<0.00005
Fluorene	μg/L	0.05			<0.05
Indeno(1,2,3,-cd)Pyrene	μg/L	0.05			<0.05

Final Report

REPORT No: 24-010898 - Rev. 1

			Client I.D.	BH106
			Sample I.D.	24-010898-1
			Date Collected	2024-Apr-19
Units	R.L.	Limits		-
mg/L	0.002	0.05	SAN	<0.002
mg/L	0.00005	0.032	SAN	<0.00005
μg/L	1			<1
mg/L	0.00005	0.022	SAN	<0.00005
mg/L	0.00005	0.059, 0.064	SAN, STORM	<0.00005
µg/L	0.05			<0.05
µg/L	0.05			<0.05
mg/L	0.0001	0.015, 0.006	SAN, STORM	<0.0001
			Client I.D.	BH106
			Sample I.D.	24-010898-1
			Date Collected	2024-Apr-19
Units	R.L.	Limits		-
mg/L	0.00001	0.00004	STORM	<0.00001
	mg/L mg/L mg/L mg/L mg/L pg/L pg/L pg/L Mg/L	mg/L 0.002 mg/L 0.00005 μg/L 1 mg/L 0.00005 mg/L 0.00005 μg/L 0.05 μg/L 0.05 mg/L 0.0001	mg/L 0.002 0.05 mg/L 0.00005 0.032 μg/L 1 mg/L 0.00005 0.022 mg/L 0.059, 0.064 μg/L 0.05 μg/L 0.05 mg/L 0.001 0.015, 0.006	Units R.L. Limits mg/L 0.002 0.05 SAN mg/L 0.00005 0.032 SAN μg/L 1 SAN SAN mg/L 0.00005 0.022 SAN mg/L 0.00005 0.059, 0.064 SAN, STORM μg/L 0.05 SAN, STORM mg/L 0.0001 0.015, 0.006 SAN, STORM Client I.D. Sample I.D. Date Collected Units R.L. Limits

Final Report

REPORT No: 24-010898 - Rev. 1

Subcontracted Analyses				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits		-
Formaldehyde	mg/L	-	0.3	SAN	<0.008
Nitrosodimethylamine (NDMA)	mg/L	-	0.4	SAN	<0.0004
Nonylphenol Monoethoxylate	mg/L	-			<0.01
Nonylphenol Diethoxylate	mg/L	-			<0.01
Nonylphenols	mg/L	-	0.0025, 0.001	SAN, STORM	<0.001
Nonylphenol Ethoxylates	mg/L	-	0.025, 0.01	SAN, STORM	<0.01

Revised to include additional dissolved metals at clients request

SAN: Sanitary Sewer By Law STORM: Storm Sewer By Law

Summary of Exceedances								
Sanitary Sewer By Law								
BH106	Found Value	Limit						
Total Suspended Solids	9480	350						
Storm Sewer By Law								
BH106	Found Value	Limit						
Total Suspended Solids	9480	15.0						
Phosphorus (Total)	8.72	0.4						
Arsenic (Total)	0.0275	0.02						
Chromium (Total)	0.249	0.08						
Copper (Total)	0.301	0.04						
Nickel (Total)	0.189	0.08						

[:] City of Ottawa

2024-Aug-07

Ground Water



CERTIFICATE OF ANALYSIS



C A D U C E N'IRON MENTAL LABORATOR ES

Client committed. Quality assured. Canadian owned.

C.O.C.: G 107579 REPORT No: 24-010898 - Rev. 2

Report To:

Cambium Environmental - Kingston

625 Fortune Crescent

#1

Kingston, ON K7P 0L5

Attention: Kyle Horner

DATE REPORTED:

SAMPLE MATRIX:

CADUCEON Environmental Laboratories

2378 Holly Lane

Ottawa, ON K1V 7P1

DATE RECEIVED: 2024-Apr-22 CUSTOMER PROJECT: 17280-002

RICHMOND HILL

P.O. NUMBER:

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
Anions (Liquid)	1	OTTAWA	PCURIEL	2024-Apr-24	A-IC-01	SM 4110B
BOD5 (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	BOD-001	SM 5210B
Cond/pH/Alk Auto (Liquid)	1	OTTAWA	SBOUDREAU	2024-Apr-22	COND-02/PH-02/A	SM 2510B/4500H/
					LK-02	2320B
Cyanide Total (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-23	CN-001	SM 4500-CN-E
Formaldehyde (Subcontracted)	1	TESTMARK	SISLAM	2024-Apr-26		Subcontracted
Ion Balance (Calc.)	1	OTTAWA	ASCHNEIDER		CP-028	MECP E3196
Chromium VI (Liquid)	1	OTTAWA	STAILLON	2024-Apr-25	D-CRVI-01	MECP E3056
ICP/MS Total (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Apr-24	D-ICPMS-01	EPA 6020
ICP/OES Total (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-29	D-ICP-01	SM 3120B
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-24	D-ICP-01	SM 3120B
Mercury (Liquid)	1	OTTAWA	TBENNETT	2024-Apr-24	D-HG-02	SM 3112B
NDMA Liquid (Subcontract)	1	SGS_LAKEFIELD	SISLAM	2024-May-30		Subcontracted
Ammonia (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	NH3-001	SM 4500NH3
Nonylphenols (Subcontracted)	1	SGS_LAKEFIELD	SISLAM	2024-Apr-30		Subcontracted
OC Pesticides (Liquid)	1	KINGSTON	CSUMMERHAYS	2024-Apr-23	PESTCL-001	EPA 8081
Oil & Grease (Liquid)	1	KINGSTON	MLANE	2024-Apr-25	O&G-001	SM 5520
Phenols (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-25	PHEN-01	MECP E3179
Sulphide (Liquid)	1	KINGSTON	EHINCH	2024-Apr-23	H2S-001	SM 4500-S2
SVOC - Semi-Volatiles (Liquid)	1	KINGSTON	EASIEDU	2024-Apr-24	NAB-W-001	EPA 8270D
TP & TKN (Liquid)	1	KINGSTON	KDIBBITS	2024-Apr-29	TPTKN-001	MECP E3516.2
TSS (Liquid)	1	KINGSTON	MCLOSS	2024-Apr-23	TSS-001	SM 2540D
Turbidity (Liquid)	1	OTTAWA	STAILLON	2024-Apr-23	A-TURB-01	SM 2130B

FLENA

2024-Apr-24

R.L. = Reporting Limit

VOC-Volatiles Full (Water)

NC = Not Calculated

Test methods may be modified from specified reference method unless indicated by an $\,^{\star}$

Michelle Dubien
Data Specialist

C-VOC-02

EPA 8260

Final Report REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D. Date Collected	24-010898-1 2024-Apr-19
Parameter	Units	R.L.	Limits		-
Alkalinity(CaCO3) to pH4.5	mg/L	5			283
pH @25°C	pH units	-	8.5	PWQO	7.85
Turbidity	NTU	0.1			7070
Fluoride	mg/L	0.1			<0.1
Sulphate	mg/L	1			84
BOD5	mg/L	3			3
Total Suspended Solids	mg/L	3			9480
Phosphorus (Total)	μg/L	10	10	INTERIM	8720
Total Kjeldahl Nitrogen	mg/L	0.1			6.3
Ammonia (N)-Total (NH3+NH4)	mg/L	0.05			0.15
Ammonia (N)-unionized	μg/L	10.0	20	PWQO	<10.0
Sulphide	mg/L	0.01			0.01
Cyanide (Total)	mg/L	0.005			<0.005
Phenolics	μg/L	1	1	PWQO	<1
Hardness (as CaCO3)	mg/L as CaCO3	0			368
Aluminum	μg/L	10	75	INTERIM	70
Barium	μg/L	1			165
Calcium	μg/L	20			105000
Iron	μg/L	5	300	PWQO	20
Magnesium	μg/L	20			25600
Tungsten	μg/L	10			<10

Final Report REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19 -
Zinc	μg/L	5	30	PWQO	<5
Zirconium	μg/L	3			<3
Hardness (as CaCO3)	mg/L as CaCO3	-			789
Aluminum (Total)	μg/L	10			30
Bismuth (Total)	μg/L	20			<20
Boron (Total)	μg/L	5	200	INTERIM	28
Cadmium (Total)	μg/L	5	0.1, 0.2	INTERIM, PWQO	<5
Calcium (Total)	μg/L	20			97700
Chromium (Total)	μg/L	2			<2
Cobalt (Total)	μg/L	5	0.9, 0.0	INTERIM, PWQO	<5
Copper (Total)	μg/L	2	5, 0.0	INTERIM, PWQO	8
Iron (Total)	μg/L	5	300	PWQO	<5
Lead (Total)	μg/L	20	1, 0.0	INTERIM, PWQO	<20
Magnesium (Total)	μg/L	20			27300
Manganese (Total)	μg/L	1			3
Molybdenum (Total)	μg/L	10	40, 0.0	INTERIM, PWQO	<10
Nickel (Total)	μg/L	10	25	PWQO	<10
Silver (Total)	μg/L	5	0.1	PWQO	<5
Tin (Total)	μg/L	50			<50
Titanium (Total)	μg/L	5			<5
Tungsten (Total)	μg/L	10	30	INTERIM	<10

Final Report REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19 -
Vanadium (Total)	μg/L	5			<5
Zinc (Total)	μg/L	5	20, 30	INTERIM, PWQO	<5
Zirconium (Total)	μg/L	3	4	INTERIM	<3
Antimony (Total)	μg/L	0.1	20	INTERIM	0.7
Arsenic (Total)	μg/L	0.1	5, 5	INTERIM, PWQO	27.5
Beryllium (Total)	μg/L	0.1	11	PWQO	3.2
Cadmium (Total)	µg/L	0.015	0.1, 0.2	INTERIM, PWQO	1.12
Chromium (Total)	μg/L	1			249
Cobalt (Total)	μg/L	0.1	0.9	INTERIM	103
Copper (Total)	μg/L	0.1	5	INTERIM	301
Lead (Total)	μg/L	0.02	1, 5	INTERIM, PWQO	76.8
Molybdenum (Total)	μg/L	0.1	40	INTERIM	7.6
Nickel (Total)	μg/L	0.2	25	PWQO	189
Selenium (Total)	μg/L	1	100	PWQO	<1
Silver (Total)	μg/L	0.1	0.1	PWQO	1.1
Thallium (Total)	μg/L	0.05	0.3, 0.3	INTERIM, PWQO	1.82
Uranium (Total)	μg/L	0.05	5	INTERIM	11.4
Vanadium (Total)	μg/L	0.1	6	INTERIM	327
Chromium (VI)	μg/L	10	1	PWQO	<10
Mercury	μg/L	0.02	0.2	PWQO	<0.02
Anion Sum	meq/L	-			16.6

Final Report

REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits	I	-
Cation Sum	meq/L	-			15.3
% Difference	%	-			4.03
Ion Ratio	-	-			1.08
Sodium Adsorption Ratio	-	-			4.28
TDS (Ion Sum Calc)	mg/L	1			893
TDS(calc.)/EC(actual)	-	-			0.540
Conductivity Calc	µmho/cm	-			1590
Conductivity Calc / Conductivity	-	-			0.959
Langelier Index(25°C)	-	-			0.800
Saturation pH (25°C)	-	-			7.05
pH (Client Data)	pH units	-			6.97
Temperature (Client Data)	°C	-			9.9

Final Report REPORT No: 24-010898 - Rev. 2

				Client I.D. Sample I.D.	BH106 24-010898-1
				Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits		-
Benzene	μg/L	0.5	100	INTERIM	<0.5
Bromodichloromethane	μg/L	2	200	INTERIM	<2
Bromoform	μg/L	5	60	INTERIM	<5
Bromomethane	μg/L	0.5	0.9	INTERIM	<0.5
Carbon Tetrachloride	μg/L	0.2			<0.2
Chlorobenzene	μg/L	0.5	15	PWQO	<0.5
Chloroethane	μg/L	3			<3
Chloroform	μg/L	1			<1
Chloromethane (Methyl Chloride)	μg/L	2	700	INTERIM	<2
Dibromochloromethane	μg/L	2	40	INTERIM	<2
Ethylene Dibromide	μg/L	0.2	5, 5	INTERIM, PWQO	<0.2
Dichlorobenzene,1,2-	μg/L	0.5	2.5	PWQO	<0.5
Dichlorobenzene,1,3-	μg/L	0.5	2.5	PWQO	<0.5
Dichlorobenzene,1,4-	μg/L	0.5	4	PWQO	<0.5
Dichloroethane,1,1-	μg/L	0.5	200	INTERIM	<0.5
Dichloroethane,1,2-	μg/L	0.5	100	INTERIM	0.7
Dichloroethylene,1,1-	μg/L	0.5	40	INTERIM	<0.5
Dichloroethylene,1,2-cis-	μg/L	0.5	200	INTERIM	<0.5
Dichloroethylene,1,2-trans-	μg/L	0.5	200	INTERIM	<0.5
Dichloropropane,1,2-	μg/L	0.5	0.7	INTERIM	<0.5
Dichloropropene,1,3-cis-	μg/L	0.5			<0.5

Final Report REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D. Date Collected	24-010898-1 2024-Apr-19
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19
Dichloropropene,1,3-trans-	μg/L	0.5	7	INTERIM	<0.5
Ethylbenzene	μg/L	0.5	8	INTERIM	<0.5
Dichloromethane (Methylene Chloride)	μg/L	5	100	INTERIM	<5
Styrene	μg/L	0.5	4	INTERIM	<0.5
Tetrachloroethane,1,1,2,2-	μg/L	0.5	70	INTERIM	<0.5
Tetrachloroethylene	μg/L	0.5	50	INTERIM	<0.5
Toluene	μg/L	0.5	0.8, 0.8	INTERIM, PWQO	<0.5
Trichloroethane,1,1,1-	μg/L	0.5	10	INTERIM	<0.5
Trichloroethane,1,1,2-	μg/L	0.5	800	INTERIM	<0.5
Trichloroethylene	μg/L	0.5	20	INTERIM	<0.5
Trichlorofluoromethane (Freon 11)	μg/L	5			<5
Trimethylbenzene,1,3,5-	μg/L	0.1	3	INTERIM	<0.1
Vinyl Chloride	μg/L	0.2	600	INTERIM	<0.2
Xylene, m,p-	μg/L	1			<1
Xylene, m,p,o-	μg/L	1.1			<1.1
Xylene, o-	μg/L	0.5	40	INTERIM	<0.5
Oil & Grease (Total)	mg/L	1.0			1.7
Oil and Grease (Mineral)	mg/L	1.0			<1.0
Oil and Grease (Anim/Veg)	mg/L	1.0			1.4

Final Report REPORT No: 24-010898 - Rev. 2

				Client I.D. Sample I.D.	BH106 24-010898-1
				Date Collected	2024-Apr-19
Parameter	Units	R.L.	Limits		
Acenaphthene	μg/L	0.05			<0.05
Acenaphthylene	μg/L	0.05			<0.05
Anthracene	μg/L	0.05	0.0008	PWQO	<0.05
Benzo[a]anthracene	μg/L	0.05	0.0004	INTERIM	<0.05
Benzo(a)pyrene	μg/L	0.01			<0.01
Benzo(b)fluoranthene	μg/L	0.05			<0.05
Benzo(b+k)fluoranthene	μg/L	0.1			<0.1
Benzo(g,h,i)perylene	μg/L	0.05	0.00002	INTERIM	<0.05
Benzo(k)fluoranthene	μg/L	0.05			<0.05
Butyl Benzyl Phthalate	μg/L	1	0.2	INTERIM	<1
Bis(2-Chloroethoxy)methane	μg/L	2			<2
Bis(2-ethylhexyl) Phthalate	μg/L	5			<5
Chrysene	μg/L	0.05	0.0001	INTERIM	<0.05
Dibenzo(a,h)anthracene	μg/L	0.05	0.002	INTERIM	<0.05
Di-n-Butyl Phthalate	μg/L	1	4	PWQO	<1
Dichlorophenol,2,4-	μg/L	0.2	0.2	PWQO	<0.2
Diethyl Phthalate	μg/L	1			<1
Di-n-Octyl Phthalate	μg/L	1	0.6	PWQO	<1
Fluoranthene	μg/L	0.05	0.0008	INTERIM	<0.05
Fluorene	μg/L	0.05	0.2	INTERIM	<0.05
Indeno(1,2,3,-cd)Pyrene	μg/L	0.05			<0.05

Final Report

REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19 -
Indole	μg/L	2			<2
Methylnaphthalene,1-	μg/L	0.05	2	INTERIM	<0.05
Methylnaphthalene,2-(1-)	μg/L	1			<1
Methylnaphthalene,2-	μg/L	0.05	2	INTERIM	<0.05
Naphthalene	μg/L	0.05	7	INTERIM	<0.05
Phenanthrene	μg/L	0.05	0.03	INTERIM	<0.05
Pyrene	μg/L	0.05			<0.05
Total PAH	μg/L	0.1			<0.1
				Client I.D.	BH106
				Sample I.D.	24-010898-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Apr-19 -
Hexachlorobenzene	μg/L	0.01			<0.01

Final Report

REPORT No: 24-010898 - Rev. 2

Subcontracted Analyses				Client I.D.	BH106
Parameter	Units	R.L.	Limits	Sample I.D. Date Collected	24-010898-1 2024-Apr-19 -
Formaldehyde	μg/L	-	0.8	INTERIM	<8
Nitrosodimethylamine (NDMA)	μg/L	-	15	INTERIM	<0.4
Nonylphenol Monoethoxylate	μg/L	-			<10
Nonylphenol Diethoxylate	μg/L	-			<10
Nonylphenols	μg/L	-	0.04	INTERIM	<1
Nonylphenol Ethoxylates	μg/L	-			<10

Revised to change guideline to PWQO

: PWQO Limits INTERIM: Interim PWQO PWQO: PWQO

Final Report REPORT No: 24-010898 - Rev. 2

Litter of the DMOO		
Interim PWQO BH106	Found Value	Limit
Phosphorus (Total)	8720	10
Cadmium (Total)	<5	0.1
Cobalt (Total)	<5	0.1
Copper (Total)	8	5
Lead (Total)	<20	1
Arsenic (Total)	27.5	5
Cadmium (Total)	1.12	0.1
	1.12	0.1
Connec (Total)		
Copper (Total)	301	5
Lead (Total)	76.8	1
Thallium (Total)	1.82	0.3
Uranium (Total)	11.4	5
Vanadium (Total)	327	6
Benzo[a]anthracene	<0.05	0.0004
Benzo(g,h,i)perylene	<0.05	0.00002
Butyl Benzyl Phthalate	<1	0.2
Chrysene	<0.05	0.0001
Dibenzo(a,h)anthracene	<0.05	0.002
Fluoranthene	<0.05	0.0008
Phenanthrene	<0.05	0.03
Formaldehyde	<8	0.8
Nonylphenols	<1	0.04
PWQO	'	
BH106	Found Value	Limit
Cadmium (Total)	<5	0.2
Silver (Total)	<5	0.1
Arsenic (Total)	27.5	5
Cadmium (Total)	1.12	0.2
Lead (Total)	76.8	5
Nickel (Total)	189	25
Silver (Total)	1.1	0.1
Thallium (Total)	1.82	0.3
Chromium (VI)	<10	1
Anthracene	<0.05	0.0008
Di-n-Octyl Phthalate	<1	0.6

Docusign Envelope ID: 0723D92C-88B4-462F-AB5E-45627C898378

CADUCEON Environmental Laboratories Certificate of Analysis

Final Report

REPORT No: 24-010898 - Rev. 2



CERTIFICATE OF ANALYSIS

17281-002

Final Report

C.O.C.: G 106721 REPORT No: 24-024417 - Rev. 0

Report To:

Cambium Environmental - Kingston

31 Hyperion Crt

Suite 102

Kingston, ON K7K 7G3

CADUCEON Environmental Laboratories

285 Dalton Ave

Kingston, ON K7K 6Z1

CUSTOMER PROJECT:

Attention: Natasha Augustine

DATE RECEIVED: 2024-Aug-10

DATE REPORTED: 2024-Aug-16 P.O. NUMBER:

SAMPLE MATRIX: Ground Water

Analyses Qty Site Analyzed Authorized Date Analyzed Lab Method Reference Method ICP/MS (Liquid) **OTTAWA** AOZKAYMAK 2024-Aug-13 D-ICPMS-01 EPA 200.8 1 ICP/OES (Liquid) 1 **OTTAWA APRUDYVUS** 2024-Aug-14 D-ICP-01 SM 3120B TSS (Liquid) KINGSTON **DCASSIDY** 2024-Aug-15 TSS-001 SM 2540D 1

R.L. = Reporting Limit

NC = Not Calculated

Test methods may be modified from specified reference method unless indicated by an $\,^\star$

				Client I.D.	BH106
Parameter	Units	R.L.	Limits	Sample I.D. Date Collected	24-024417-1 2024-Aug-08 -
Total Suspended Solids	mg/L	3			<3
Hardness (as CaCO3)	mg/L as CaCO3	0			380
Aluminum	μg/L	10	75	INTERIM	20
Boron	μg/L	5	200	INTERIM	62
Calcium	μg/L	20			107000
Iron	μg/L	5	300	PWQO	334
Magnesium	μg/L	20			27400
Tungsten	μg/L	10			<10
Zinc	μg/L	5	30	PWQO	<5
Zirconium	μg/L	3			<3

Final Report

REPORT No: 24-024417 - Rev. 0

				Client I.D.	BH106
				Sample I.D.	24-024417-1
				Date Collected	2024-Aug-08
Parameter	Units	R.L.	Limits		-
Antimony	μg/L	0.1	20, 5	INTERIM, PWQO	0.3
Arsenic	μg/L	0.1	5, 0.0	INTERIM, PWQO	1.0
Beryllium	μg/L	0.1	0.0, 11	INTERIM, PWQO	<0.1
Cadmium	μg/L	0.015	0.1, 0.2	INTERIM, PWQO	0.211
Chromium	μg/L	1.0			<1.0
Cobalt	μg/L	0.1			1.1
Copper	μg/L	0.1	5	INTERIM	5.4
Lead	μg/L	0.02	1, 5	INTERIM, PWQO	0.08
Molybdenum	μg/L	0.1	40	INTERIM	5.0
Nickel	μg/L	0.2	25	PWQO	3.8
Selenium	μg/L	1.00	100	PWQO	<1.00
Silver	μg/L	0.1	0.1	PWQO	<0.1
Thallium	μg/L	0.05	0.3, 0.3	INTERIM, PWQO	<0.05
Uranium	μg/L	0.05	5	INTERIM	4.68
Vanadium	μg/L	0.1	6	INTERIM	0.3

: PWQO Limits INTERIM: Interim PWQO PWQO: PWQO

Final Report

REPORT No: 24-024417 - Rev. 0

Summary of Exceedances							
Interim PWQO							
BH106	Fo	und Value	Limit				
Cadmium		0.211	0.1				
Copper		5.4	5				
PWQO							
BH106	Fo	und Value	Limit				
Iron		334	300				
Cadmium		0.211	0.2				



CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G 106721 REPORT No: 24-024417 - Rev. 2

Report To:

Cambium Environmental - Kingston

31 Hyperion Crt

Suite 102

Kingston, ON K7K 7G3

DATE REPORTED:

CADUCEON Environmental Laboratories

285 Dalton Ave

Kingston, ON K7K 6Z1

Attention: Natasha Augustine

DATE RECEIVED: 2024-Aug-10 CUSTOMER PROJECT: 17281-002

2024-Sep-05 P.O. NUMBER:

SAMPLE MATRIX: Ground Water

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
ICP/MS (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Aug-13	D-ICPMS-01	EPA 200.8
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Aug-14	D-ICP-01	SM 3120B
TSS (Liquid)	1	KINGSTON	DCASSIDY	2024-Aug-15	TSS-001	SM 2540D

R.L. = Reporting Limit

NC = Not Calculated

Test methods may be modified from specified reference method unless indicated by an $\,^{\star}$

				Client I.D.	BH106
Parameter	Units	R.L.	Limits	Sample I.D. Date Collected	24-024417-1 2024-Aug-08
Total Suspended Solids	mg/L	3	350, 15.0	SAN, STORM	<3
Hardness (as CaCO3)	mg/L as CaCO3	0.02			380
Aluminum	mg/L	0.01	50	SAN	0.02
Boron	mg/L	0.005	25	SAN	0.062
Calcium	mg/L	0.02			107
Iron	mg/L	0.005			0.334
Magnesium	mg/L	0.02			27.4
Phosphorus	mg/L	0.1			<0.1
Tungsten	mg/L	0.01			<0.01
Zinc	mg/L	0.005	3, 0.04	SAN, STORM	<0.005

Steve Garrett
Director of Laboratory Services

Final Report REPORT No: 24-024417 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-024417-1
Parameter	Units	R.L.	Limits	Date Collected	2024-Aug-08
Zirconium	mg/L	0.003	Limio		<0.003
Antimony	mg/L	0.0001	5	SAN	0.0003
Arsenic	mg/L	0.0001	1, 0.02	SAN, STORM	0.0010
Beryllium	mg/L	0.0001			<0.0001
Cadmium	mg/L	0.000015	0.02, 0.008	SAN, STORM	0.000211
Chromium	mg/L	0.001	5, 0.08	SAN, STORM	<0.001
Cobalt	mg/L	0.0001	5	SAN	0.0011
Copper	mg/L	0.0001	3, 0.04	SAN, STORM	0.0054
Lead	mg/L	0.00002	5, 0.12	SAN, STORM	0.00008
Molybdenum	mg/L	0.0001	5	SAN	0.0050
Nickel	mg/L	0.0002	3, 0.08	SAN, STORM	0.0038
Selenium	mg/L	0.001	5, 0.02	SAN, STORM	<0.001
Silver	mg/L	0.0001	5, 0.12	SAN, STORM	<0.0001
Thallium	mg/L	0.00005			<0.00005
Uranium	mg/L	0.00005			0.00468
Vanadium	mg/L	0.0001	5	SAN	0.0003

Revised to add Phosphorous result by ICP

: City of Ottawa SAN: Sanitary Sewer By Law STORM: Storm Sewer By Law

Steve Garrett
Director of Laboratory Services



CERTIFICATE OF ANALYSIS

Final Report

REPORT No: 24-027621 - Rev. 0 C.O.C.: G 112298

Report To:

Cambium Environmental - Kingston

31 Hyperion Crt

Suite 102

Kingston, ON K7K 7G3

CADUCEON Environmental Laboratories

285 Dalton Ave

Kingston, ON K7K 6Z1

Attention: Natasha Augustine

2024-Sep-06 CUSTOMER PROJECT: 17281-002 DATE RECEIVED:

2024-Sep-10 DATE REPORTED: P.O. NUMBER:

Ground Water SAMPLE MATRIX:

Analyses Qty Site Analyzed Authorized Date Analyzed Lab Method Reference Method TP & TKN (Liquid) KINGSTON YLIEN 2024-Sep-10 TPTKN-001 MECP E3516.2

R.L. = Reporting Limit

NC = Not Calculated

Test methods may be modified from specified reference method unless indicated by an *

		Parameter	Phosphorus (Total)
		Units	mg/L
		R.L.	0.01
Client I.D.	Sample I.D.	Date Collected	-
BH106	24-027621-1	2024-Sep-05	<0.01

Steve Garrett Director of Laboratory Services



Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
November 11, 2024

				App	endi	κ F
Single	Well	Hydra	aulic	Test	Resu	Its



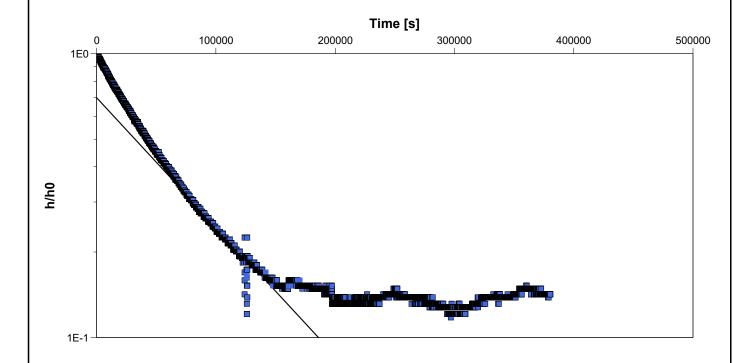
Project: Hydrogeological Assessment

Number: 17281-002

Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely LaneSlug Test: BH105 - Slug Test 1Test Well: BH105-23Test Conducted by: MCTest Date: 4/19/2024Analysis Performed by: NAHvorslevAnalysis Date: 7/11/2024

Aquifer Thickness: 2.62 m



Observation Well	Hydraulic Conductivity	
	[m/s]	
BH105-23	6.35 × 10 ⁻⁹	



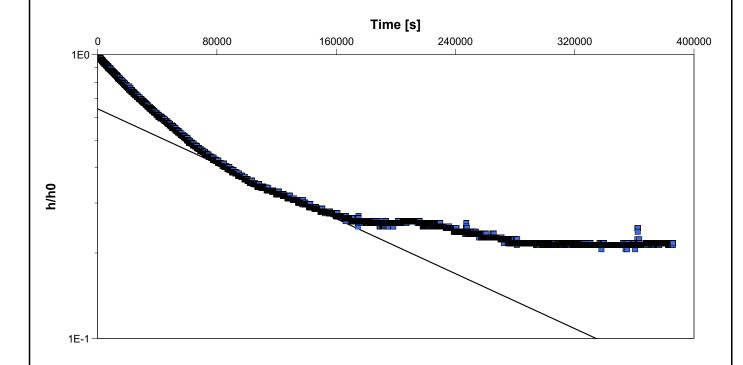
Project: Hydrogeological Assessment

Number: 17281-002

Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH105 - Slug Test 2	Test Well: BH105-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024

Aquifer Thickness: 2.62 m



Observation Well	Hydraulic Conductivity	
	[m/s]	
BH105-23	3.38 × 10 ⁻⁹	



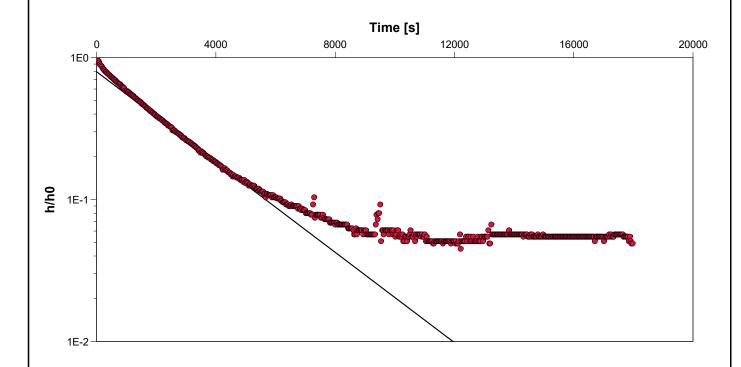
Project: Hydrogeological Assessment

Number: 17281-002

Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH106 - Slug Test 1	Test Well: BH106-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024

Aquifer Thickness: 2.46 m



Calculation	usina	Hvorslev

Observation Well	Hydraulic Conductivity	
	[m/s]	
BH106-23	2.22 × 10 ⁻⁷	



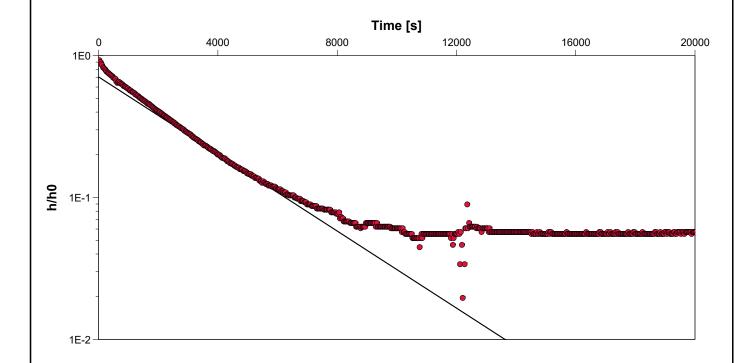
Project: Hydrogeological Assessment

Number: 17281-002

Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH106 - Slug Test 2	Test Well: BH106-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024

Aquifer Thickness: 2.46 m



Observation Well	Hydraulic Conductivity	
	[m/s]	
BH106-23	1.90 × 10 ⁻⁷	



Project: Hydrogeological Assessment

Number: 17281-002

Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane

Slug Test: BH106 - Slug Test 3

Test Well: BH106-23

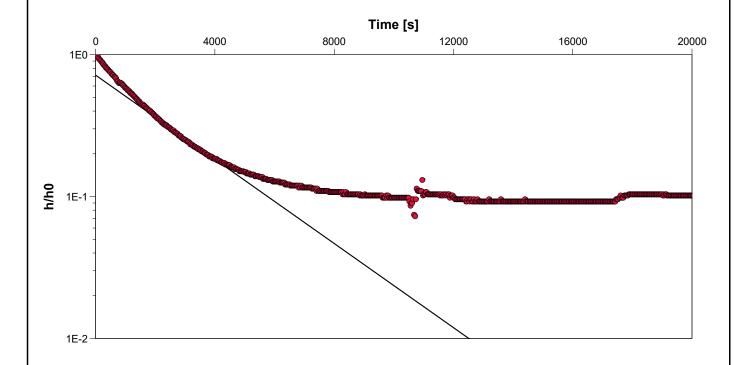
Test Date: 4/19/2024

Analysis Performed by: NA

Hvorslev

Analysis Date: 7/11/2024

Aquifer Thickness: 2.46 m



ſ	Observation Well	Hydraulic Conductivity	
		[m/s]	
Ī	BH106-23	2.07 × 10 ⁻⁷	



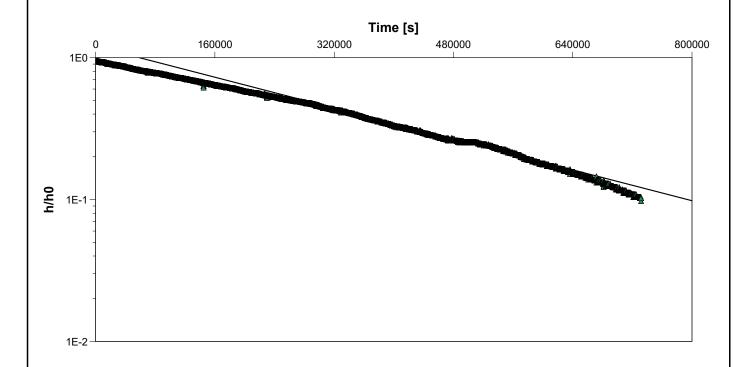
Project: Hydrogeological Assessment

Number: 17281-002

Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH107 - Slug Test 1	Test Well: BH107-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024

Aquifer Thickness: 2.89 m



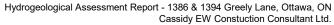
Calculation	usina	Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH107-23	1.90 × 10 ⁻⁹	



Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
Cassidy EW Construction Consultant Ltd.
Cambium Reference: 17281-002
November 11, 2024

	Appendix G
Dewatering	Calculations



Cambium Ref. No.: 17281-002



DEWATERING CALCULATIONS - CONSTRUCTION PHASE

Modified Dupuit-Forchheimer Equation: unconfined flow into a linear excavation. Calculations assume no flow boundary at aquifer base

Excavation Area		Initial Depth to Groundwater	Target Depth to Groundwater	Base of	Unit Length of Trench (a)	Width of Trench (b)	Hydraulic Conductivity (K)	Drawdown (s)	R	r _w = b/2	R _o	In(R _o /r _w)	L = R _o /2	н	h = H-s	\mathbf{Q}_{ends}	Q _{trench}		Q _{total}	
		mbgs	mbgs	mbgs	m	m	m/s	m	m	m	m	-	m	m	m	m³/s	m ³ /s	m³/s	L/s	L/d
Elongated Trench @ 50 m Increments	Minimum K	0.30	2.50	3.60	50	2	1.90E-09	2.20	0.29	1.00	1.29	0.25	0.64	3.30	1.10	0.000000	0.000001	0.000002	0.002	143
	Maximum K	0.30	2.50	3.60	50	2	2.06E-07	2.20	2.99	1.00	3.99	1.39	2.00	3.30	1.10	0.000005	0.000050	0.000054	0.05	4,702
	Geometric mean K	0.30	2.50	3.60	50	2	1.22E-08	2.20	0.73	1.00	1.73	0.55	0.86	3.30	1.10	0.000001	0.000007	0.000008	0.01	648

s = target drawdown (initial - target depth to groundwater) (m)

R_o = radius of influence of construction dewatering/pumping, from center of excavation (m)

L = distance to line source (m)

r_s = equivalent single well radius (m)

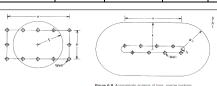
H = Initial hydraulic head in aquifer (m)

h = hydraulic head at radius of well (m)

Q = construction dewatering rate (m³/s)

*For base of aquifer, use target depth to groundwater plus 50% of target drawdown (s), unless specific geological conditions dictate otherwise.

For practical use, R is presented as zone of influence for reporting purposes, with the distance defined from edge of excavation.





Source: Powers, J. Patrick, et al. "Construction dewatering and groundwater control." (2007)

$$Q = \frac{\pi K (H^2 - h^2)}{\ln R_0 / r_s} + 2 \left[\frac{x K (H^2 - h^2)}{2L} \right]$$
 (6.10b)

x = unit length of trench

R = 3000*s*sqrt(K)

Source: Kyrieleis, W. and Sichardt, W. "Grundwasserabsenkung bei

Fundierungsarbeiten" Springer, Berlin, 1930

 $R_0 = R$, if $R >> r_s$ (R >> rs when $R/r_s > 100$)

else, $R_o = R + r_s$

Source: Cashman and Preene. "Groundwater Lowering in Construction." (2013)



Hydrogeological Assessment Report - 1386 & 1394 Greely Lane, Ottawa, ON Cassidy EW Constuction Consultant Ltd.

Cambium Ref. No.: 17281-002

DEWATERING CALCULATIONS - OPERATIONAL PHASE

Modified Dupuit-Forchheimer Equation: unconfined flow into a rectangular excavation. Calculations assume no flow boundary at aquifer base

Excavation Area		to	Target Depth to Groundwater	Depth to Base of Aquifer*	Excavation Length (a)	Excavation Width (b)	Hydraulic Conductivity (K)	Drawdown (s)	R	r _w = √(ab/π)	R _o	In(R _o /r _w)	Н	h _w = H-s		Q _{total}	
		mbgs	mbgs	mbgs	m	m	m/s	m	m	m	m	-	m	m	m ³ /s	L/s	L/d
Rectangular excavation with dimensions axb	Minimum K	0.30	1.50	3.60	23	55	1.9E-09	1.20	0.16	20.07	20.22	0.01	3.30	2.10	0.000005	0.005	429
	Maximum K	0.30	1.50	3.60	23	55	2.1E-07	1.20	1.63	20.07	21.70	0.08	3.30	2.10	0.000054	0.05	4,628
Geor	netric mean K	0.30	1.50	3.60	23	55	1.2E-08	1.20	0.40	20.07	20.46	0.02	3.30	2.10	0.000013	0.01	1,093

s = target drawdown (initial - target depth to groundwater) (m)

 R_{o} = radius of influence of construction dewatering/pumping, from center of excavation (m)

r_s = equivalent single well radius (m)

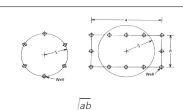
H = Initial hydraulic head in aquifer (m)

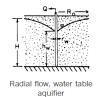
h = hydraulic head at radius of well (m)

Q = construction dewatering rate (m^3/s)

*For base of aquifer, use target depth to groundwater plus 50% of target drawdown (s), unless specific geological conditions dictate otherwise.

For practical use, R is presented as zone of influence for reporting purposes, with the distance defined from edge of excavation.







(from Table 6.1, pg 67)

*Use $r_w = r_s$ for rectangular excavations

R = 3000*s*sqrt(K)

Source: Kyrieleis, W. and Sichardt, W.
"Grundwasserabsenkung bei Fundierungsarbeiten"
Springer, Berlin, 1930

 R_o = R, if $R >> r_s$ (R >> rs when $R/r_s > 100$) else, R_o = $R + r_s$

Source: Cashman and Preene. "Groundwater Lowering in Construction." (2013)

Source: Powers, J. Patrick, et al. "Construction dewatering and groundwater control." (2007)



Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario
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November 11, 2024

	Appendix	Н
Water Balance	Calculation	16



Water Balance Calculations 1386 and 1394 Greely Lane, Ottawa, ON

	Т	HORNTI	-IWAITE	-TYPE M	ONTHLY	/ WATER-	BALAN	CE MOD	EL				
mo	dified fro	om Ding	man 20:	15: Box 6	5-8 (pg 2	.99) using	ET mod	del of Ho	mon (1	963)			
		Ir	nput Dat	:a	Computed Values								
											Surplus	397	mm/yr
Weather Station Location:	Grachy	ON				atitude:	4E 2	degree					, 7.
Weather Station Location.	Greely,	ON				.atituue:	45.5	uegree					
	20.6	42.6	4.5	40.0	10.0	22.4	24.0	42.4	2.6	0.0	40.5	22.0	
Solar Declination (degree)		-12.6	-1.5	10.0	19.0	23.1	21.0				-18.5	-23.0	
DayLength (hr)*	9.0	10.3	11.8	13.4	14.7	15.4	15.0	13.9	12.4	10.8	9.4	8.6	
Available Water St	torage C	apacity	0.21	m/m	Roc	ot Depth	460	mm	S	OILmax	96.6	mm	
MONTHLY WATER BALANCE DATA Temperatures in C, water-balance terms in mm.													
Month:	J	F	M	A	M	J	J	Α	S	0	N	D	Year
		=====	191		=====							=====	
TEA 4 DED 4 TUDE (T)	10.2		2.2					10.0					
TEMPERATURE (T)	-10.3	-8.1	-2.3	6.3	13.3	18.5	21.0		15.0		1.5	-6.2	044
PRECIPITATION (P)	65.4	54.3	64.4	74.5	80.3	92.8	91.9		90.1	86.1	81.9	76.4	944
RAIN	25.0	18.7	31.1	63.0	80.1	92.8	91.9		90.1	82.2	64.5	33.5	758
snow	40	36	33	12	0	0	0	0	0	4	17	43	185
MELT FACTOR (F)	0.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.25	0.00	
PACK	96	132	165	0	0	0	0	0	0	0	13	56	
MELT	0	0	0	177	0	0	0	0	0	4	4	0	185
INPUT (W)	25	19	31	240	80	93	92	86	90	86	69	34	944
POTENTIAL ET (PET)	0	0	0	41	73	101	118	101	65	38	21	0	557
NET INPUT (ΔW)	25	19	31	199	8	-8	-26	-16	25	48	48	34	
SOIL MOISTURE (SOIL)	97	97	97	97	97	89	68	58	82	97	97	97	
ΔSOIL	0	0	0	0	0	-8	-21	-10	25	14	0	0	0
ET	0	0	0	41	73	100	113	96	65	38	21	0	547
SURPLUS=W-ET-DSOIL	25	19	31	199	8	0	0	0	0	34	48	34	397
Notes:													
Precipitation, Rain, Temperature, and L	atituda ar	e innutter	l naramet	arc									
SOILmax = available water storage cap			paramet										
m = month													
D = Day length (hrs) =2*cos ⁻¹ (-tan(Latiti	ude)*tan([Declination	n))/0.2618	[calculation	on is in rad	dians]							
SNOW _m = P _m -RAIN _m													
$F_m = 0 \text{ if } T_m \le 0^{\circ}\text{C}; F_m = 0.167*T_m \text{ if } 0^{\circ}\text{C}$	<t<sub>m<6°C; F</t<sub>	= 1 if T_	>=6°C										
$PACK_{m} = (1-F_{m})*(SNOW_{m}+PACK_{m-1})$													
MELT = F _m *(SNOW _m +PACK _{m-1})													
$W_m = RAIN_m + MELT_m$.													
PET = 0 if T_m <0; otherwise PET = 2.98*0	0.611*exp	(17.3*T _m /	(T _m +237)),	/(T _m +237.2	2)*Numbe	r of days in	month [H	lamon ET i	model (19	63)]			
$\Delta W_m = W_m - PET_m$	•	<u> </u>											
SOIL = $min\{[\Delta W_m + SOIL_{m-1}], SOILmax\}, in$	f ΔWm>0;	otherwise	SOIL = SC	OIL _{m-1} * exp	(ΔW/SOIL	max)							
Δ SOIL = SOIL _{m-1} -SOIL _m ET = PET if W _m > PET; otherwise, ET=W	-45011												
LI – PEI II W _m > PEI; Otherwise, EI=W	m-∆3OIL												

1 Climate Information

3 Pre-Development Property Statistics

4 Post-Development Property Statistics

Total Paved Area

Total Landscape Area

Total Roof Area

Total Paved Area

Total Roof Area

Total Landscape Area

Total

Total



Pre- and Post-Development Water Balance Calculations 1386 and 1394 Greely Lane, Ottawa, ON

944 mm/yr Precipitation 547 mm/yr **Actual Evapotranspiration** Water Surplus 397 mm/yr 2 Infiltration Rates Table 2 Approach - Infiltration factors Topography: Flat 0.3 Soil Type: Silty Loam 0.2 Cover: Cultivated land 0.1 **Total Infiltration Factor** 0.6 **238** mm/yr Infiltration (Water Surplus * Infiltration Factor) Run-off (Water Surplus - Infiltration) **159** mm/yr Table 3 Approach - Typical Recharge Rates Coarse Sand and Gravel >250 mm/yr Fine to medium sand 200-250 mm/yr Silty sand to sandy silt 150-200 mm/yr Silt 125-150 mm/yr Clayey Silt 100- 125 mm/yr Clay <100 mm/yr Site development area is underlain predominantly by Silty Sand to Clayey Silt Based on the above, the recharge rate is typically 125-150 mm/yr

 m^2

811

365

3,502

4,678

 m^2

2,246

1,261

1,171

4,678

ha

0.08

0.04

0.35

0.47

ha

0.22

0.13

0.12

0.47

5	Pre-Deve	lonment	Water	Ralance

Land	Use	Area (m²)	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	KUII-UII /m3\					
Imamorraious Aroos	Paved Area	811	766	77	-	689					
Impervious Areas	Roof Area	365	345	34	-	310					
Pervious Areas	Landscape Area	3,502	3,306	1,916	834	556					
	Totals	4,678	4,416	2,027	834	1,555					
Assuming no infiltration occi	Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.										

6 Post-Development Water Balance

Land Use		Area (m²)	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	Kun-on
						/ma3\
Ilmpervious Areas	Paved Area	2,246	2,120	212	-	1,908
	Roof Area	1,261	1,190	119	-	1,071
Pervious Areas	Landscape Area	1,171	1,105	641	279	186
	Totals	4,678	4,416	972	279	3,166

7 Comparision of Pre- and Post -Development

	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	Kun-011 /m³\
Pre-Development	4,416	2,027	834	1,555
Post-Development	4,416	972	279	3,166
Change in Volume	•	- 1,055	- 555	1,610
Change in %	-	- 52	- 67	104

8 Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration (m³/yr)	
Volume of Post-Development Infiltration (m³/yr)	279
Deficit from Pre to Post Development Infiltration (m³/yr)	
Percentage of Roof Runoff required to match the pre-development infiltration (%)	