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## Site Servicing Report

# 2983 Navan Road – Block 16 Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash



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**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

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## **1.0 INTRODUCTION**

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### **1.1 General**

In 2023, J.L. Richards & Associates Limited (JLR) was retained by 12714001 Canada Inc. (the Owner) to prepare the detailed design of municipal infrastructure for Site Plan Approval (SPA) of their parcel located at 2983 Navan Road. The mixed-use site known as “Block 16 - Gas Station, Commercial Building, Drive-Thru Restaurant and Car Wash” will be referred to herein as Block 16. This Site Servicing Report (SSR) presents the servicing constraints and strategies for water, wastewater, stormwater servicing, and stormwater management in accordance with the City of Ottawa Design Guidelines, the associated technical bulletins and relevant design excerpts. This SSR also includes strategies for implementing erosion and sedimentation control measures throughout the construction phase of the project.

### **1.2 Site Description**

The Block 16 is located within the City of Ottawa’s Official Plan boundary and consists of a 0.77 ha parcel bounded by Navan Road to the south and existing residential properties to the east, the proposed East Ridge Orleans Subdivision and Future Mixed-Use Block 15 to the north, and Brian Coburn Boulevard to the west. The legal description of the subject property can be found in the Draft Plan of Subdivision attached to Appendix A.

A topographical survey was completed by Stantec Inc. in August 2023 (Appendix A). The survey indicates that the existing ground surface generally slopes downwards in a southeasterly direction towards Navan Road.

### **1.3 Proposed Development**

The proposed commercial development will consist of a gas station including a car wash, a commercial retail space along with a Drive-Thru Restaurant. The Concept Plan for the Block 16 is attached to Appendix A.

### **1.4 Proposed Connections to Existing Infrastructure**

The proposed site plan will be serviced via the future East Ridge Orleans Subdivision and via existing infrastructure on Navan Road as follows and as shown on the servicing drawings:

#### **Watermain**

- Connection to the proposed existing 204 mm diameter watermain along Navan Road

#### **Sanitary**

- Connection to the future East Ridge Orleans Subdivision. A 200mm sanitary sewer stub will be dropped to service Block 16 as part of this future subdivision.

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#### **Storm**

- Connection to the future East Ridge Orleans Subdivision. A 450mm storm sewer stub will be dropped to service Block 16 as part of this future subdivision.

#### **1.5 Consultation and Permits**

An initial pre-consultation meeting was held on July 6, 2022, followed by a Phase 2 Pre-consultation help on September 13, 2023, each to discuss the proposed site plan, the planning approval process requirements, provide clarifications on design criteria, and high level discussion on servicing constraints. A copy of the pre-consultation meeting notes and the site servicing checklist has been provided in Appendix B.

## **2.0 WATER SERVICING**

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#### **2.1 Water Supply Design Criteria**

A Hydraulic Network Analysis (HNA) was completed as part of the detailed design for the East Ridge Orleans subdivision to confirm that the proposed watermains could provide adequate supply while complying with both the Ottawa Design Guidelines for Water Distribution (July 2010) and Technical Bulletins ISDTB-2014-02, ISTB-2018-02 and ISTB-2021-03. These documents are herein referred to as the Design Guidelines and TB-2014-02, TB-2018-02, and TB-2021-03, respectively.

The HNA completed as part of the East Ridge Orleans Subdivision design included water demands for the Commercial Site Plan (Block 16). The HNA has since been updated to reflect the proposed water service lateral for Block 16 but is based on the same demands and the boundary conditions used in the original East Ridge Orleans Subdivision HNA (refer to Appendix C for a copy of City correspondence for boundary conditions).

Section 4.2.2 of the Design Guidelines states the following criteria for development additions to the public water distribution system:

- Under maximum hourly demand conditions (peak hour), the residual pressures shall not be less than 276 kPa (40 psi);
- During periods of maximum day and fire flow demand, the residual pressure at any point in the distribution system shall not be less than 140 kPa (20 psi);
- In accordance with the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi);
- The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi); and
- Feedermains, which have been provided primarily for the purpose of redundancy, shall meet, at a minimum, the basic day plus fire flow demand.

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## **2.2 Domestic Water Demands**

The estimated commercial water demands presented in this section are based on the site layout proposed in the Site Plan (Appendix A). A plug flow of 3.60 L/s was added to each demand scenario to consider the demands required for the car wash (refer to Appendix D for a confirmation letter from the mechanical engineer). Table 1 summarizes the water demands projected for this site.

**Table 1: Water Demands**

Demand Scenario	Commercial Water Consumption or Peaking Factor	Commercial Water Demands (L/s)	Car Wash Demands (L/s)	Total Demands (L/s)
Average Day Demand	28,000 L/ha/d	0.25	3.60 L/s	3.85
Maximum Day Demand	1.5 x Avg Day	0.37	3.60 L/s	3.97
Peak Hour Demand	1.8 x Max Day	0.67	3.60 L/s	4.27

## **2.3 Fire Flow Requirements**

The City has specified that the Fire Underwriters Survey (FUS) method shall be used for any public or private site where new fire hydrants are being designed. Specifically, the required fire flow (RFF) for each structure was calculated in accordance with TB-2018-02.

The required fire flow for the Commercial Site Plan (Block 16) was calculated to be 83 L/s. Refer to Appendix C for the detailed RFF calculations for the critical fire area.

## **2.4 Proposed Water Servicing, Boundary Conditions and Water Model**

### **2.4.1 Proposed Water Servicing**

Water will be supplied to the Commercial Site Plan (Block 16) by a 150 mm diameter water service that will connect to the existing 305 mm watermain on Navan Road, located east of the intersection between Navan Road and Brian Coburn Blvd. Fire protection will be provided by a new proposed hydrant within the site. As shown in the servicing plan, the car wash is serviced directly by the gas station. Design of the service will be verified by Owner's mechanical engineer however, as noted in Section 2.2, the demand from the car wash has been included in this HNA.

Watermain roughness coefficients were determined using friction factors presented in Section 4.2.12 of the Design Guidelines and the internal pipe diameters were modelled based on Section 4.3.5 of the design Guidelines.

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**2.4.2 Boundary Conditions**

Hydraulic boundary conditions were provided by the City at the proposed connection location listed in Section 2.4.1 above. Table 2 summarizes the hydraulic boundary conditions received by the City (refer to Appendix C for a copy of the City correspondence). The boundary condition for maximum day plus fire flow corresponds to a required fire flow of 100 L/s. It is noted that the fire flow demand for the City boundary condition is more conservative than the calculated fire flow requirement of 5000 L/min (83 L/s).

**Table 2: Hydraulic Boundary Conditions**

Demand Scenarios	Connection 3 Head (m)
Maximum HGL	130.7
Peak Hour	126.8
Max Day plus Fire Flow 6,000 L/min (100.00 L/s)	127.3

**2.4.3 Water Model**

A hydraulic water model within the WaterCAD® software platform was used to carry out the HNA (refer to the overall schematics presented in Appendix C). The water demands from Table 1 and the boundary conditions from Table 2 were input into the model for each demand scenario. Table 3 summarizes the watermain diameters and roughness coefficients used in the model, based on Sections 4.2.12 and 4.3.5 of the Design Guidelines.

**Table 3: Watermain Internal Diameters and C-Factors**

Nominal Diameter	Inside Diameter	C-Factor
150 mm	155 mm	100
200 mm	204 mm	110
300 mm	297 mm	120

**2.5 Simulation Results**

The HNA was carried out under steady-state peak hour, maximum day plus fire flow, and maximum pressure conditions to confirm that the proposed water servicing can meet the design criteria outlined in Section 2.1.

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#### **2.5.1 Peak Hour**

The simulation results found the minimum pressure at the site during the peak hour condition to be 399 kPa (57.9psi) (refer to Appendix C), which exceeds the minimum pressure criterion of 276 kPa (40 psi) per the Design Guidelines.

#### **2.5.2 Maximum Day Plus Fire Flow**

Fire water supply will be provided by a proposed hydrant off the 150 mm diameter water service for Block 16. Hydrant spacing was carried out in accordance with the Design Guidelines.

To ensure adequate fire protection, the maximum day demand shown in Table 1 was analyzed simultaneously with the fire flow requirements. The fire flow simulation was carried out by allowing WaterCAD® to calculate the maximum fire flow that can be drawn from the hydrant without allowing any part of the system to experience pressures less than 140 kPa (20 psi). Using the 6,000 L/min (100 L/s) boundary condition provided by the City (refer to Table 2), the system is expected to deliver a minimum of 6,000 L/min (100 L/s) within the site. Per Appendix I of TB-2018-02, adequate water supply can be provided by the hydrant to the proposed site.

#### **2.5.3 Maximum Pressure**

Based on a zero (0 L/s) demand condition, the simulation results found the maximum pressure at the site to be 437 kPa (63.4). This value is below the maximum pressure constraint of 552 kPa (80 psi), therefore pressure reducing valves (PRVs) are not anticipated to be required.

## **2.6 Summary and Conclusions**

Based on the water simulation results, the proposed development can be serviced by the proposed 150 mm water service lateral connected to the 305 mm diameter watermain on Navan Road. Furthermore, adequate fire water supply can be achieved with the proposed hydrant off the 150 mm water lateral servicing Block 16.

## **3.0 WASTEWATER SERVICING**

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### **3.1 Design Criteria**

The sanitary sewer system within the Block 16 is designed in accordance with the Ottawa Sewer Design Guidelines and subsequent technical bulletins. The design parameters are applied under two scenarios as per ISTB Technical Bulletin 2018-01. The key design parameters have been summarized in Table 4.

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**Table 4: Wastewater Key Design Parameters**

<b>Design Parameter</b>	<b>Design Value</b>
Commercial Average Flow	28,000 L/gross ha/Day
Residential Average Flow	280 L/Cap/Day
Residential Peaking Factor	Harmon's Formula
Commercial Peaking Factor	1.5
Harmon's Correction Factor (K)	0.8
Infiltration Allowance	0.33 L/s/ha
Manning's Roughness Coefficient (n)	0.013
Allowable Slopes	Varies (Refer to Section 6.1.2.2 of ODSG)
Allowable Velocities	0.6 m/s – 3.0 m/s
Allowable Freeboard	-

### 3.2 Proposed Sanitary Servicing and Design Flows

Wastewater generated from the Block 16 will be conveyed via a proposed 200 mm diameter sanitary sewer system. Wastewater will then discharge into the East Ridge Orleans Subdivision via a 200mm sanitary sewer stub proposed as part of the subdivision as shown on the Servicing Plan.

Wastewater flows from the proposed development are presented in the Block 16 Sanitary Design Sheet (refer to Appendix D). Based on the design criteria presented in Table 3-1 the total design peak flow of 4.23 L/s is calculated for the development which is based on the site area of 0.77ha. Table 5 summarizes the results from the sanitary design sheet.

**Table 5: Sanitary Design Flow Summary**

<b>Commercial Type</b>	<b>Site Area</b>	<b>Average Flow</b>	<b>Com. Peak Flow</b>	<b>Infilt. Flow</b>	<b>Total Flow</b>
Commercial Flows	0.77 ha	28,000 L/gross ha/Day	0.374 L/s	0.254 L/s	0.628 L/s
Car Wash Flows	-	-	-	-	3.6 L/s
Total Wastewater Flows – Block 16					4.23 L/s

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The flows from Block 16 (4.23 L/s) were incorporated in the detailed design of the sanitary sewer within the future East Ridge Orleans Subdivision thus there is sufficient downstream capacity. It is proposed to adopt the sanitary servicing strategy described in this section.

### **3.3 Summary and Conclusions**

Wastewater servicing for Block 16 will be designed in accordance with the City of Ottawa Sewer Design Guidelines, the associated technical bulletins, and various background documents as highlighted throughout this section. Wastewater generated from the Block 16 will be conveyed via a proposed 200 mm diameter sanitary sewer outletting to the East Ridge Orleans Subdivision to the north of the site. It is recommended that this wastewater servicing plan be implemented to provide adequate sanitary servicing for the proposed development.

## **4.0 STORM SERVICING AND STORMWATER MANAGEMENT**

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### **4.1 Design Criteria**

Storm and stormwater management servicing for the Gas Bar Site Plan (Block 16) was developed in accordance with the City of Ottawa 2012 Sewer Design Guidelines (OSDG) and the more recent Technical Bulletin PIEDTB-2016-01 (September 6, 2016). These two documents are herein referred to as the Design Guidelines in this section. A summary of the key storm and stormwater management criteria follows:

- Control minor system flows to the allowable release rates of 68 L/s as identified in Table 5-4 Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2024);
- The runoff coefficients (C-factors) to be calculated based on the ratio of pervious and impervious surfaces depicted on proposed site plans;
- Minimum roadway slope of 0.1% from crest-to-crest for overland flow route;
- Maximum parking ponding depth of 350 mm (static and dynamic) as per the Design Guidelines and maximum depth of surface flow to be 300 mm;
- Minimum vertical clearance of 0.15 m between the spill elevation on the street and the finished grade;
- Major system flows, up to and including the 1:100-year design storm event, are contained within the site.
- Quality control will be accommodated by Pond #3 to meet an MECP Enhanced Level of Protection (80% TSS removal) as identified in Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023).
- Ponding in landscaped areas to enhance groundwater recharge in accordance with the City of Ottawa Urban Design Guidelines for Gas Stations, Guideline 30.
- Provide measures to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

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#### **4.2 Proposed Stormwater Management Approach**

In order to achieve the allowable release rates, the stormwater management of the site will include:

- Surface storage within the site in greater than a 1:2-year event with captured flows conveyed to the minor system;
- Controlled release of the flows captured in the minor system for the entire site using Inlet Control Devices (ICDs).
- Flows stored in oversized storage pipes underground. Flows will accumulate in the storage pipes and be released from the site via an OGS and orifice control into the minor system on Paleo Drive.

#### **4.3 Proposed Minor System Servicing**

Internal to the gas station site, two minor system sewer runs will be provided on either side of the main building structure. Both of these sewer runs will be oversized and will connect into a single manhole upstream of the connection upstream of the easement stub, upstream of Paleo Drive. The connection manhole will have a 127mm diameter orifice plate on the downstream outlet sewer to control to the allowable release rate, 68 L/s, as defined in the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023). The upsized storm sewers will be 525mm diameter.

Downstream of the orifice an Stormceptor EFO4 model, or equivalent, will provide 80% TSS removal as well as capture of oils and spills.

The runoff coefficient is based on the ratio of impervious surfaces and areas. A design sheet for sizing of the sewers to confirm capacity for the 1:2-year rational method flow is provided in Appendix E.

The gas station roof structure is uncontrolled and drains directly to the minor system.

There are no basements and therefore no HGL constraints in the system.

#### **4.4 Stormwater Management Modelling Approach**

##### **4.4.1 Dual Drainage Model**

The analysis of both major and minor drainage systems was carried out to demonstrate their compliance with respect to the design criteria described in Section 4.6. The performance of the major overland system and minor storm sewer system was analyzed with PCSWMM. This software is a dynamic model which allows both hydrologic and hydraulic components to be simulated in the same platform and also allows the simulation of the interaction between the major and minor systems. The PCSWMM software platform was used to:

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- Generate the surface runoff hydrograph for each sub-area under various recurrences.
- Subdivide each inflow hydrograph into its minor and major system components based on the proposed inlet capture rates and roadway sag storage.
- Assess cascading, if any, and carry out dynamic routing of storm flows to determine flow depths along the roadways. As previously stated, the maximum major overland flow depths within the parking lot areas are to be limited to 350 mm or less, as per Technical Bulletin PIEDTB-2016-01.

PCSWMM was set-up to evaluate the proposed servicing as detailed on Drawing C01 and C02. Subcatchments were delineated for the structure roof areas, parking lot low points and landscaped low points. Model schematics are prepared in Appendix F.

#### **4.4.2 Boundary Conditions**

Boundary conditions are taken from the downstream subdivision model issued as part of the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023).

The downstream 1:100-year HGL at the connection to the Subdivision at MH518A is identified as 81.28 m. The boundary condition in the model was set at this elevation as a constant during the storm simulations.

## **4.5 Modelling Parameters**

### **4.5.1 Hydrological Modelling Parameters**

The following parameters were used in the hydrologic component of PCSWMM:

- **Areas and Imperviousness:** Catchment ID and drainage areas used by PCSWMM match those shown on either Drawing DST or Figure E-1 (Appendix E1). Sealed and roof areas are set at 100% impervious and other grassed or landscaped areas are pervious.
- **Catchment Width:** The catchment width is estimated at the width of overland sheet flow based on the grading of the catchment and slope direction.
- **Manning's Roughness Coefficient:** Manning's Roughness Coefficients of 0.013 and 0.25 were used for the impervious and pervious surfaces, respectively.
- **Horton Infiltration parameters:** City of Ottawa OSDG Horton Infiltration Parameters have been used in the modelling.

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- **Initial Abstraction:** Initial abstraction of 4.67 mm and 1.57 mm was used for the pervious and impervious surfaces respectively, consistent with the OSDG.

#### **4.5.2 Simulation of Storm Distributions**

To assess peak flow rates and peak volume storage requirements the 3-hour Chicago storm has been simulated for the site for the 1:2-year event and 1:100-year event and the 24-hour SCS storms for the 1:100-year event.

## **4.6 Simulation Results**

### **4.6.1 Low Point Ponding Analysis**

Ponding depths in the low points in the parking area and landscaped areas are shown in Table 6.

**Table 6: Catchbasin Ponding Depths**

<b>Low Point ID</b>	<b>Top of Grate (m)</b>	<b>Maximum Static Depth (mm)</b>	<b>3-hour Chicago 1:2 year Depth (mm)</b>	<b>3-hour Chicago 1:100 year Depth (mm)</b>	<b>24-hour SCS 1:100 year Depth (mm)</b>
1	85.25	190/300	0	260	230
2A	85.35	250	0	0	0
2B	85.40	220	0	0	0
3	85.35	300	0	60	30
4	84.75	150	0	100	0
5	85.35	150	0	100	80
6	85.37	80	0	100	90
7	84.55	300	0	300	160
8	85.04	270	0	290	250
9	84.80	300	0	200	130
10	85.18	240	0	150	90
11	85.12	300	0	210	170
12	85.45	150	0	90	70
13	85.50	150	0	160	150

The simulation results compiled in Table 6 shows that:

- No ponding nor dynamic flow will occur in the 1:2-year event;

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- Maximum ponding depth of 300 mm during the 1:100-year event; and,
- There is no spill from the site in the 1:100-year event.

#### **4.6.2 Site Release Rate**

Table 8 below shows the release rates from the site via the 127mm diameter orifice plate. All release rates are below the 68 L/s allowable release rate.

**Table 7: Release Rates**

	<b>3-hour Chicago 1:2 year Release Rate (L/s)</b>	<b>3-hour Chicago 1:100 year Release Rate (L/s)</b>	<b>24-hour SCS 1:100 year Release Rate (L/s)</b>
Flow at MH 514	47	66	65

#### **4.7 Water Quality**

An OGS unit is proposed for the site to provide site specific water quality to 80% TSS removal and capture of oils and spills. The sizing details for the unit are contained in Appendix F. The unit sized for the site is an Stormceptor EFO4, or equivalent.

#### **4.8 Summary and Conclusions**

The stormwater servicing achieves a release rate from the site to the minor system limited to the allowable release rate and contains up to the 1:100 year on site.

## **5.0 Erosion and Sediment Control**

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Erosion and sediment control measures, as outlined in the Ontario Ministry of Natural Resources (MNR) Guidelines on Erosion and Sediment Control for Urban Construction Sites, will be implemented to trap sediment on site. The following erosion and sediment control measures can be implemented during construction as shown on the Erosion and Sediment Control Plan (Drawing ESC):

- Supply and installation of a silt fence barrier, as per OPSD 219.110.
- Supply and installation of siltsack or sentinel CB inserts between the frame and cover of catch basins and maintenance holes adjacent to the project area during construction, to prevent sediment from entering the sewer system.
- Stockpiling of material during construction is to be located along flat areas away from drainage paths. For material placed on sloped areas, stockpiles are to be enclosed with a silt fence to protect watercourses.

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- All catch basins are to be equipped with sumps, inspected frequently, and cleaned as required.
- Temporary ICDs are to be placed blocking part of the sewer pipe in the connecting storm maintenance holes to eliminate construction debris from entering the existing storm sewer system. The ICDs are to be removed after the proposed storm sewers have been fully cleaned.
- A mud mat is to be built at each of the site entranceways to prevent the transport of sediment onto paved surfaces. The mud mat shall be:
  - Minimum of 20 m in length for the full width of the entrance way (10 m wide minimum).
  - Minimum of 400 mm thick underlain with a geotextile (or graded aggregate filter); and
  - Constructed with 50 mm diameter clear stone for the first 10 m (extending from the paved street) and the remainder of the length with 150 mm diameter clear stone.

The proposed removal and reinstatement measures as well as the erosion control measures shall conform to the following documents:

- “Guidelines on Erosion and Sediment Control for Urban Construction Sites” published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs, and Transportation & Communication, Association of Construction Authorities of Ontario and Urban Development Institute, Ontario, May 1987.
- “MTO Drainage Manual”, Chapter F: “Erosion of Materials and Sediment Control”, Ministry of Transportation & Communications, 1985.
- “Erosion and Sediment Control” Training Manual by Ministry of Environment, Spring 1998.
- Applicable Regulations and Guidelines of the Ministry of Natural Resources.

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## **6.0 CONCLUSIONS**

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Block 16 will be serviced as follows:

- Water servicing will be provided by connection to the proposed watermain along the Navan Road.
- Wastewater servicing will be provided by a connection to the future East Ridge Orleans Subdivision
- Storm servicing will be provided by a connection to the future East Ridge Orleans Subdivision
- Flows exceeding the allowable peak flow for Block 16, will be held on-site, using a combination of both, above ground and underground storage.

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Bobby Pettigrew, P. Eng.  
Senior Water Resource Engineer

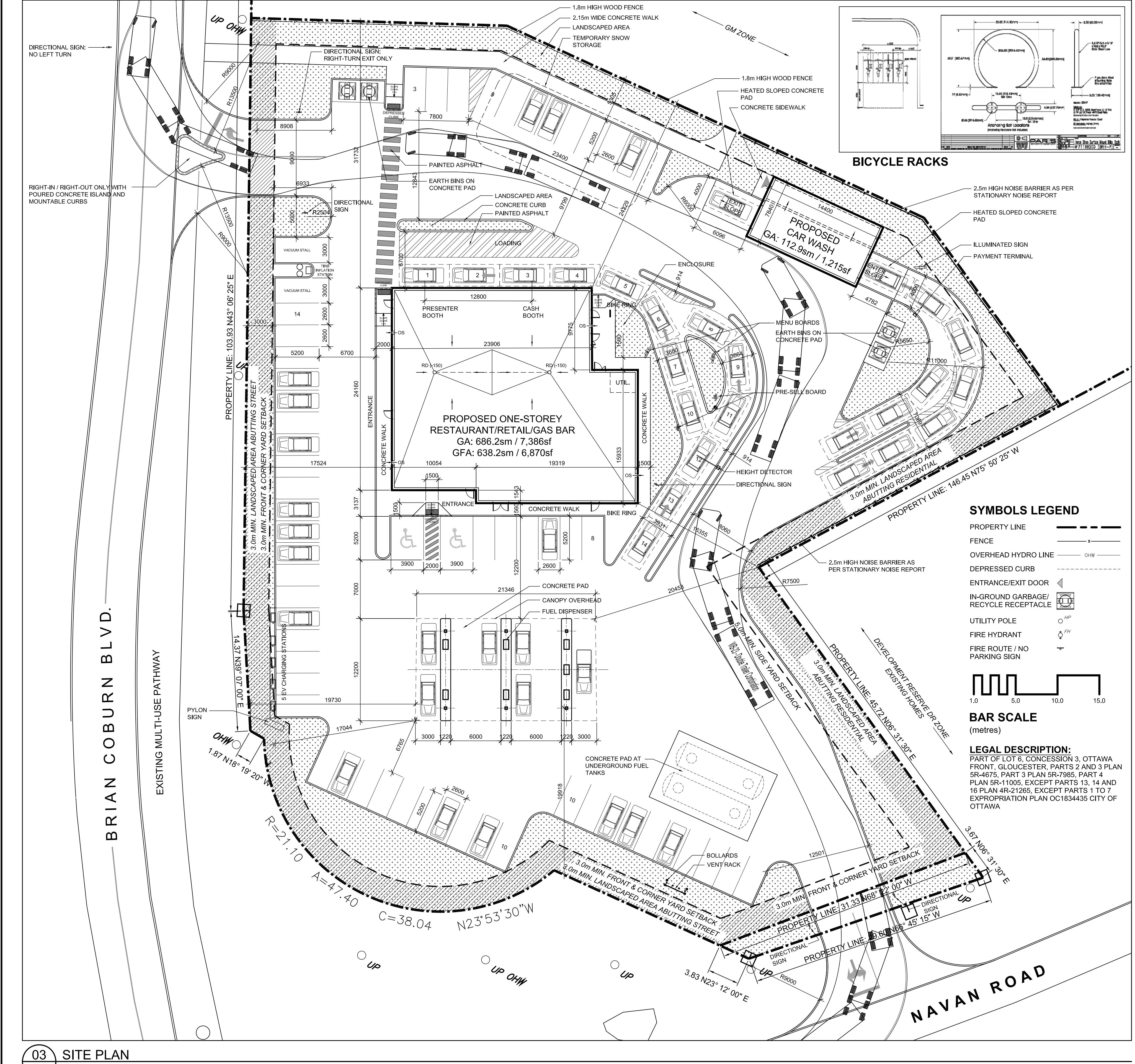
**Site Servicing Report**  
**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

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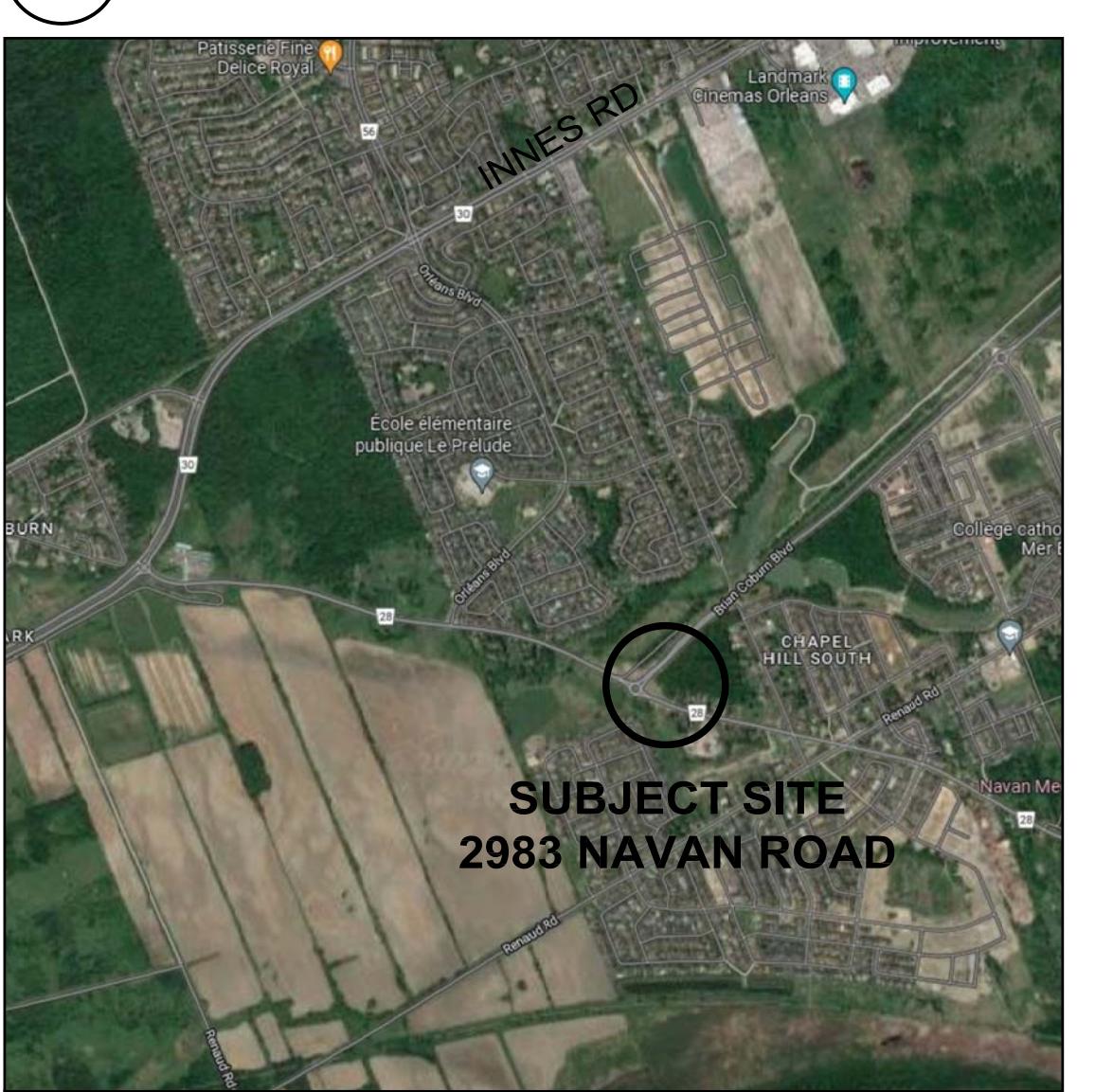
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## **Appendix A**

Concept Plan, Draft Plan of  
Subdivision and Topographical  
Survey



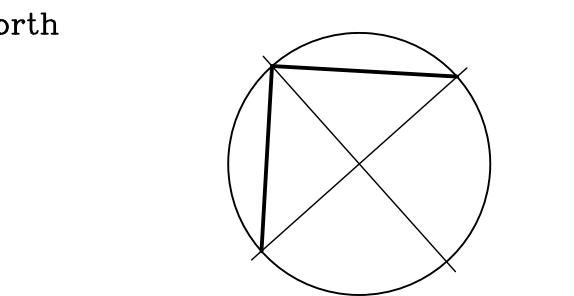
02 SITE & BUILDING DATA and ZONING REVIEW			
SP-A01			SCALE: NTS
08	IW	SITE PLAN APPLICATION	08 DEC 2023
09	IW	REVISED FOR SPA	01 MAR 2024
10	IW	REVISED FOR SPA	14 AUG 2024



**OWNER:**  
1274001 CANADA INC.  
100-768 Boulevard St Joseph  
Gatineau, QC J8Y 4B8

**PLANNING, CIVIL & TRAFFIC CONSULTANT:**  
J.L.RICHARDS & ASSOCIATES LTD.  
1000-343 Preston Street  
Ottawa, ON K1Z 1N4

**LANDSCAPE ARCHITECT:**  
JAMES B. LENNOX & ASSOCIATES INC.  
3332 Carling Avenue  
Ottawa, ON K2H 5A8



Revisions			
No.	By	Description	Date
08	IW	SITE PLAN APPLICATION	08 DEC 2023
09	IW	REVISED FOR SPA	01 MAR 2024
10	IW	REVISED FOR SPA	14 AUG 2024

## NEW RESTAURANT, CONVENIENCE STORE & GAS BAR


2130 BRIAN COBURN BLVD.


Drawing  
**SITE PLAN**

Scale	Stamp
AS SHOWN	
Drawn	
AK / KE	
Checked	
Project No.	Drawing No.
22-127	SP-A01
Date	12 MAY 2022
PLAN NO. _____	

ONTARIO ASSOCIATION OF ARCHITECTS  
IAN EDWARD WILLSON LICENCE 7456

DO7-16-21-0027





**Site Servicing Report**  
**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

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## **Appendix B**

Pre Consultation Meeting Notes  
and Site Servicing Report  
Checklist



File No.: PC2023-0227

Carmine Zayoun  
12714001 Canada Inc (Zayoun Group)  
Via email: carmine@zayoungroup.com

**Subject:** Pre-Consultation: Meeting Feedback  
Proposed Site Plan Application – 2983 Navan Road ‘  
Gas Station and Commercial Building – PC2023-0227

Please find below information regarding next steps as well as consolidated comments from the above-noted pre-consultation meeting held on September 13, 2023.

### **Pre-Consultation Preliminary Assessment**

1 <input type="checkbox"/>	2 <input type="checkbox"/>	3 <input type="checkbox"/>	4 <input checked="" type="checkbox"/>	5 <input type="checkbox"/>
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One (1) indicates that considerable major revisions are required while five (5) suggests that the proposal appears to meet the City's key land use policies and guidelines. This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.

### **Next Steps**

1. A review of the proposal and materials submitted for the above-noted pre-consultation has been undertaken. Please proceed to complete a Phase 2 / Phase 3 Pre-consultation Application Form and submit it together with the necessary studies and/or plans to [planningcirculations@ottawa.ca](mailto:planningcirculations@ottawa.ca).
2. In your subsequent pre-consultation submission, please ensure that all comments or issues detailed herein are addressed. A detailed cover letter stating how each issue has been addressed must be included with the submission materials. Please coordinate the numbering of your responses within the cover letter with the comment number(s) herein.
3. Please note, if your development proposal changes significantly in scope, design, or density before the Phase 3 pre-consultation, you may be required to complete or repeat the Phase 2 pre-consultation process.

### **Supporting Information and Material Requirements**

1. The attached **Study and Plan Identification List** outlines the information and material that has been identified, during this phase of pre-consultation, as either required (R) or advised (A) as part of a future complete application submission.
  - a. The required plans and studies must meet the City's Terms of Reference (ToR) and/or Guidelines, as available on [Ottawa.ca](http://Ottawa.ca). These ToR and Guidelines outline



the specific requirements that must be met for each plan or study to be deemed adequate.

### **Consultation with Technical Agencies**

1. You are encouraged to consult with technical agencies early in the development process and throughout the development of your project concept. A list of technical agencies and their contact information is enclosed.

### **Planning**

Comments:

1. In the Official Plan the subject site is designated as Neighbourhood is modified with the Evolving Neighbourhood overlay. Brian Coburn Boulevard is also designated as a Minor Corridor. The property is further identified as Low-density residential in the EUC – Phases 1 Community Design Plans (CDP). The subject lands are currently zoned GM[2546]H(14.5) General Mixed-Use, Exception.
2. Committee of Adjustment

No variances have been identified at this point. Staff will set up a meeting with a Committee of Adjustment Plan if any required.

3. Design guidelines

Urban Design Guidelines for Drive-Through Facilities

Urban Design Guidelines for Gas Stations

4. Landscape requirements

Landscape buffers will need to comply with Section 110 of the Zoning By-law and are consistent with the design guidelines.

The turning radius encroached into the landscape buffers on the demonstration plan. Ensure all landscaping is protected by barrier curbs

5. Parking requirements

Parking should comply with Sections 100, 101, 106, 109 and 111

Vehicle and bicycle parking should be situated with easily access while minimizing pavement. There seems to be a lot of pavement in front of this building.

6. Easements



Are there service easements required over the pedestrian walkway connecting this property with the subdivision?

7. Confirm that the location of the Car Wash will not become a noise problem for future residents of the townhouses behind it. Noise study will need to identify any issues and provide remediation measures.
8. Provide locations of signage and ensure that space is made available for tree planting

### **Urban Design**

9. Relevant guidelines – The City's Urban Design Guidelines for Gas Stations and Urban Design Guidelines for Drive-Through Facilities are both applicable to this site. The applicant should ensure their submission meets the direction of these guidelines.
10. Design Brief - A Design Brief is required. Please refer to the attached Terms of Reference for details.
11. Public realm – Please refer to the attached PDF for Urban Design comments related to the location of sidewalks, walkways and pedestrian movements on the site
12. Landscaping - Extensive tree and shrub planting is needed on this site, in particular to soften the interface with the existing and future residential and to enhance the ROW.

Feel free to contact the Urban Design Planner, Christopher Moise, at [Christopher.Moise@ottawa.ca](mailto:Christopher.Moise@ottawa.ca), for follow-up questions

### **Engineering**

Comments:

13. General Comments:
  - a. Review of the Phase 3 submission for this application will not occur until the detailed design of the subdivision that it is within (D07-16-21-0027) is approved.
  - b. At the stage of site plan approval, a condition will be imposed detailing that a commence work notification will not be issued until the subdivision's infrastructure is in-service.
14. Engineering Studies:
  - a. All engineering studies (detailed in the Study and Plan Identification List form) are to follow the to be approved draft plan of subdivision D07-16-21-0027.



- b. An interceptor pit is required with the provision of the car wash.
15. An MECP Environmental Compliance Approval **Industrial Sewage Works** will be required for the proposed development. Please contact the Ministry of the Environment, Conservation and Parks, Ottawa District Office to arrange a pre-submission consultation:

- b. Emily Diamond at (613) 521-3450, ext. 238 or  
[Emily.Diamond@ontario.ca](mailto:Emily.Diamond@ontario.ca)

**Note:** this site does not meet the City's requirements for ToR. To have the ECA application reviewed under ToR, a request will need to be sent to Charles Warnock ([charles.warnock@ottawa.ca](mailto:charles.warnock@ottawa.ca)).

Feel free to contact Reed Adams ([reed.adams@ottawa.ca](mailto:reed.adams@ottawa.ca)), Infrastructure Project Manager, for follow-up questions.

### **Noise**

Comments:

16. A stationary noise report for the car wash is required because of the adjacent residential.

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at [Mike.Giampa@ottawa.ca](mailto:Mike.Giampa@ottawa.ca) , for follow-up questions.

### **Transportation**

Comments:

17. A full TIA is not required as this site is covered under the recent subdivision TIA.
18. A memo including the pertinent subdivision trips is sufficient.
19. The right of way protection on Brian Coburn and Navan Roads is 40m and 37.5m, respectively

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at [Mike.Giampa@ottawa.ca](mailto:Mike.Giampa@ottawa.ca) , for follow-up questions

### **Planning Forestry**

Comments:

20. A Tree Conservation Report and Landscape Plan must be submitted with both SPC applications



21. A permit is required prior to any tree removal on site. The tree permit will be released upon site plan approval. Please contact the File Lead or the Planning Forester, Hayley Murray ([hayley.murray@ottawa.ca](mailto:hayley.murray@ottawa.ca)) for information on obtaining the tree permit.
22. If marine clay soils are present, setbacks on City properties must adhere to the 2017 SMC guidelines (attached). The Geotechnical report must address the implications of these soils, if present, on tree planting in relation to private land.
23. If underground parking is planned, a design must be provided for the site to support tree planting
24. We expect a very strong landscape plan to re-establish canopy cover across the properties. Tree planting and protecting existing urban forest canopy is imperative to reach the City's target of 40% canopy cover.

Feel free to contact Hayley Murray, Planning Forester, for follow-up questions.

### **Parkland**

Comments:

25. Parkland contributions were made through the Subdivision process.

Feel free to contact Jessica Button, Parks Planner, for follow-up questions.

### **Conservation Authority**

Comments:

26. The Rideau Valley Conservation authority will be commenting on this application

Feel free to contact RVCA, for follow-up questions.

### **Other**

27. The High Performance Development Standard (HPDS) is a collection of voluntary and required standards that raise the performance of new building projects to achieve sustainable and resilient design. The HPDS was passed by Council on April 13, 2022.
  - a. At this time, the HPDS is not in effect and Council has referred the 2023 HPDS Update Report back to staff with direction to bring forward an updated report to Committee with recommendations for revised phasing timelines, resource requirements and associated amendments to the Site Plan Control By-law by no later than Q1 2024.



- b. Please refer to the HPDS information attached and ottawa.ca/HPDS for more information.

### **Submission Requirements and Fees**

1. Outlines the application type/subtype required and the associated fees
  - a. Additional information regarding fees related to planning applications can be found [here](#).
2. The attached **Study and Plan Identification List** outlines the information and material that has been identified as either required (R) or advised (A) as part of a future complete application submission.
  - a. The required plans and studies must meet the City's Terms of Reference (ToR) and/or Guidelines, as available on [Ottawa.ca](#). These ToR and Guidelines outline the specific requirements that must be met for each plan or study to be deemed adequate.
3. All of the above comments or issues should be addressed to ensure the effectiveness of the application submission review.

Should there be any questions, please do not hesitate to contact myself or the contact identified for the above areas / disciplines.

Yours Truly,  
Steve Belan

cc.

Tim Chadder  
Raad Akrawi  
Madelen Fellows  
Karla Ferrey  
Tatyana Roumie  
Christopher Moise  
Adam Reed  
Mike Giampa  
Haley Murray  
Jessica Button

**12714001 Canada Inc – Block 16 – Gas Bar, Commercial Building/Drive-Through Restaurant and Car Wash**

**2983 Navan Road**

**SITE SERVICING REPORT CHECKLIST**

<b>REFERENCED STUDIES AND REPORTS</b>	<b>REFERENCE</b>
Site Servicing Report for 12714001 Canada Inc, Block 16 – Gas Bar, Commercial Building/Drive-Through Restaurant and Car Wash, 2983 Navan Road Road (J.L. Richards & Associates Limited, December 8, 2023)	<a href="#">Site Servicing Report</a>

<b>4.1</b>	<b>GENERAL CONTENT</b>	<b>REFERENCE</b>
<input type="checkbox"/>	Executive Summary (for larger reports only).	<a href="#">N/A</a>
<input checked="" type="checkbox"/>	Date and revision number of the report.	<a href="#">Site Servicing Report</a>
<input checked="" type="checkbox"/>	Location map and plan showing municipal address, boundary, and layout of proposed development.	<a href="#">Site Servicing Report (Appendix A) All Drawings</a>
<input checked="" type="checkbox"/>	Plan showing the site and location of all existing services.	<a href="#">Servicing Plan</a>
<input checked="" type="checkbox"/>	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	<a href="#">Site Servicing Report</a>
<input checked="" type="checkbox"/>	Summary of Pre-consultation Meetings with City and other approval agencies.	<a href="#">Site Servicing Report (Appendix 'A')</a>
<input checked="" type="checkbox"/>	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defendable design criteria.	<a href="#">Reference made to Stantec 2005 EUC ISSU</a>
<input checked="" type="checkbox"/>	Statement of objectives and servicing criteria.	<a href="#">Site Servicing Report</a>
<input checked="" type="checkbox"/>	Identification of existing and proposed infrastructure available in the immediate area.	<a href="#">Site Servicing Report Servicing Plan</a>
<input type="checkbox"/>	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	<a href="#">N/A</a>

<input checked="" type="checkbox"/>	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	<a href="#">Grading Plan</a>
<input type="checkbox"/>	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	<a href="#">N/A</a>
<input type="checkbox"/>	Proposed phasing of the development, if applicable.	<a href="#">N/A</a>
<input checked="" type="checkbox"/>	Reference to geotechnical studies and recommendations concerning servicing.	<a href="#">Site Servicing Report and Drawings</a>
<input checked="" type="checkbox"/>	All preliminary and formal site plan submissions should have the following information: <ul style="list-style-type: none"> <li>▪ Metric scale</li> <li>▪ North arrow (including construction North)</li> <li>▪ Key plan</li> <li>▪ Name and contact information of applicant and property owner</li> <li>▪ Property limits, including bearings and dimensions</li> <li>▪ Existing and proposed structures and parking areas</li> <li>▪ Easements, road widening and rights-of-way</li> <li>▪ Adjacent street names</li> </ul>	<a href="#">All Drawings</a>

<b>4.2</b>	<b>SITE SERVICING REPORT: WATER</b>	<b>REFERENCE</b>
<input type="checkbox"/>	Confirm consistency with Master Servicing Study, if available.	<a href="#">N/A</a>
<input checked="" type="checkbox"/>	Availability of public infrastructure to service proposed development.	<a href="#">Site Servicing Report (Section 2.0) Servicing Plan</a>
<input checked="" type="checkbox"/>	Identification of system constraints.	<a href="#">Site Servicing Report (Section 2.0) Servicing Plan</a>
<input checked="" type="checkbox"/>	Identify boundary conditions.	<a href="#">Site Servicing Report (Section 2.0)</a>
<input checked="" type="checkbox"/>	Confirmation of adequate domestic supply and pressure.	<a href="#">Site Servicing Report (Section 2.0)</a>
<input checked="" type="checkbox"/>	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	<a href="#">Site Servicing Report (Section 2.0)</a>
<input checked="" type="checkbox"/>	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	<a href="#">Site Servicing Report (Section 2.0)</a>

<input type="checkbox"/>	Definition of phasing constraints. Hydraulic modelling is required to confirm servicing for all defined phases of the project, including the ultimate design.	N/A
<input checked="" type="checkbox"/>	Address reliability requirements, such as appropriate location of shutoff valves.	Site Servicing Report (Section 2.0)
<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification.	N/A
<input checked="" type="checkbox"/>	Reference to water supply analysis to show that major infrastructure can deliver sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants), including special metering provisions.	Site Servicing Report (Section 2.0) Servicing Plan
<input type="checkbox"/>	Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input checked="" type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	Site Servicing Report (Section 2.0)

4.3	SITE SERVICING REPORT: WASTEWATER	REFERENCE
<input checked="" type="checkbox"/>	Summary of proposed design criteria (Note: Wet weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Site Servicing Report (Section 3.0,
<input type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	Stantec 2005 EUC ISSU
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the Guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input checked="" type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Site Servicing Report (Section 3.0) Servicing Plan

<input checked="" type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable.)	Site Servicing Report (Section 3.0)
<input checked="" type="checkbox"/>	Calculations related to dry weather and wet weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Site Servicing Report (Section 3.0)
<input checked="" type="checkbox"/>	Description of proposed sewer network, including sewers, pumping stations and forcemains.	Site Servicing Report (Section 3.0) Servicing Plan
<input type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/>	Special considerations, such as contamination, corrosive environment, etc.	N/A

4.4	SITE SERVICING REPORT: STORMWATER	REFERENCE
<input checked="" type="checkbox"/>	Description of drainage outlets and downstream constraints, including legality of outlets (i.e., municipal drain, right-of-way, watercourse, or private property).	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Servicing, Grading and Drainage Plans
<input checked="" type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Site Servicing Report (Section 4.0)

<input checked="" type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Site Servicing Report (Section 4.0)  Servicing, Grading and Drainage Plans
<input type="checkbox"/>	Setback from private sewage disposal systems.	N/A
<input type="checkbox"/>	Watercourse and hazard lands setbacks.	N/A
<input checked="" type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Site Servicing Report (Appendix 'A')
<input type="checkbox"/>	Confirm consistency with subwatershed and Master Servicing Study, if applicable study exists.	Stantec 2005 EUC ISSU
<input checked="" type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:2 year return period) and major events (1:100 year return period).	Site Servicing Report (Section 4.0)
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	N/A
<input checked="" type="checkbox"/>	Calculate pre- and post-development peak flow rates, including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Proposed minor and major systems, including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Servicing, Grading and Drainage Plans
<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Quantity control proposed per Site Servicing Report (Section 4.0)
<input type="checkbox"/>	Identification of potential impacts to receiving watercourses.	N/A
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A
<input checked="" type="checkbox"/>	Description of how the conveyance and storage capacity will be achieved for the development.	Site Servicing Report (Section 4.0)

<input checked="" type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
<input checked="" type="checkbox"/>	Inclusion of hydraulic analysis, including hydraulic grade line elevations.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Site Servicing Report (Section 5.0) Servicing Plan
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

<b>4.5 APPROVAL AND PERMIT REQUIREMENTS</b>		<b>REFERENCE</b>
The Site Servicing Report shall provide a list of applicable permits and regulatory approvals necessary for the proposed development, as well as the relevant issues affecting such approval. The approval and permitting shall include but not be limited to the following:		
<input type="checkbox"/>	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams, as defined in the Act.	N/A
<input type="checkbox"/>	Application for Environmental Compliance Approval (ECA) under the Ontario Water Resources Act.	As part of future submission
<input type="checkbox"/>	Changes to Municipal Drains.	N/A
<input type="checkbox"/>	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation, etc.).	N/A

<b>4.6 CONCLUSION CHECKLIST</b>		<b>REFERENCE</b>
<input checked="" type="checkbox"/>	Clearly stated conclusions and recommendations.	Site Servicing Report
<input checked="" type="checkbox"/>	Comments received from review agencies, including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	Not yet applicable

<input checked="" type="checkbox"/>	All draft and final reports shall be signed and stamped by a Professional Engineer registered in Ontario.	<a href="#">Site Servicing Report</a> <a href="#">All Drawings</a>

**Site Servicing Report**  
**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

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## **Appendix C**

Water Servicing

WATERMAIN DEMAND CALCULATION SHEET
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**PROJECT :** NAVAN ROAD DEVELOPMENT PROJECT - GAS BAR  
**LOCATION :** CITY OF OTTAWA  
**DEVELOPER :** 12714001 Canada Inc.

NODE	RESIDENTIAL		NON-RESIDENTIAL COMM (ha.)	AVERAGE DAILY DEMAND (l/s)				MAXIMUM DAILY DEMAND (l/s)				PEAK HOUR DEMAND (l/s)				
	UNITS			Res.	Non-res.	Plug flow	Total	Res.	Non-res.	Plug flow	Total	Res.	Non-res.	Plug flow	Total	
	Townhouses (TH)	Condo Units (CU)														
J-14	0	0	0	0.77	0.00	0.25	3.60	3.85	0.00	0.37	3.60	3.97	0.00	0.67	3.60	4.27
TOTALS	0	0	0	0.77	0.00	0.25	3.60	3.85	0.00	0.37	3.60	3.97	0.00	0.67	3.60	4.27

ASSUMPTIONS									
<b>RESIDENTIAL DENSITIES</b>			<b>AVG. DAILY DEMAND</b>			<b>MAX. HOURLY DEMAND</b>			
- Townhouse (TH)	<u>2.7</u>	p / p / u	- Residential		<u>280</u> l / cap / day	- Residential			<u>1,540</u> l / cap / day
- Condo Units (CU)	<u>1.8</u>	p / p / u	- Institutional		<u>28,000</u> l / ha / day	- Institutional			<u>75,600</u> l / ha / day
		p / p / u	- Commercial		<u>28,000</u> l / ha / day	- Commercial			<u>75,600</u> l / ha / day
			<b>MAX. DAILY DEMAND</b>						
		p / p / u	- Residential		<u>700</u> l / cap / day				
			- Institutional		<u>42,000</u> l / ha / day				
			- Commercial		<u>42,000</u> l / ha / day				

**FUS Fire Flow Calculations**  
**NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building**  
**(JLR 29899-002)**

Step	Parameter	Value	Note
A	Type of Construction	Non-combustible	
	Coefficient (C)	0.8	
B	Ground Floor Area	686	m <sup>2</sup>
			Commercial area consisting of a Gas Retail and Drive Thru
C	Height in storeys	1	storeys
	Total Floor Area	686	m <sup>2</sup>
D	Fire Flow Formula	$F=220C\sqrt{A}$	
	Fire Flow	4610	L/min
	Rounded Fire Flow	5000	L/min
			Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Combustible	
	Occupancy Charge	0%	
	Occupancy Increase or Decrease	0	
	Fire Flow	5000	L/min
F	Sprinkler Protection	None	
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0	L/min
G	North Side Exposure		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	4 Storey Condo Unit
	Length of Exposed Wall:	32.2	m
	Height of Exposed Wall:	4	storeys
	Length-Height Factor	128.6	m-storeys
	Separation Distance	38.96	m
	North Side Exposure Charge	5%	
	East Side Exposure		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	46	m
	East Side Exposure Charge	0%	
	South Side Exposure		
	Exposing Wall:	Non-combustible	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	46	m
	South Side Exposure Charge	0%	Over 45 m to next structure
	West Side Exposure		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	46	m
	West Side Exposure Charge	0%	Over 200 m to next structure
	Total Exposure Charge	5%	The total exposure charge is below the maximum value of 75%.
	Increase for Exposures	250	L/min
H	Fire Flow	5250	L/min
	Rounded Fire Flow	5000	L/min
			Flow rounded to nearest 1000 L/min.
<b>City Cap</b>	<b>Required Fire Flow (RFF)</b>	<b>5000</b>	<b>L/min</b>
		83	L/s

Fire Underwriters Survey (FUS) Fire Flow Calculations  
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

## FUS Fire Flow Calculations

**NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building**  
(JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Non-combustible	
	Coefficient (C)	0.8	
B	Ground Floor Area	107	m <sup>2</sup>
C	Height in storeys	1	storeys
	Total Floor Area	107	m <sup>2</sup>
D	Fire Flow Formula	$F=220C\sqrt{A}$	
	Fire Flow	1821	L/min
	Rounded Fire Flow	2000	L/min
			Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Combustible	
	Occupancy Charge	0%	
	Occupancy Increase or Decrease	0	
	Fire Flow	2000	L/min
F	Sprinkler Protection	None	
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0	L/min
G	<i>North Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Wood Frame	Townhomes
	Length of Exposed Wall:	14.4	m
	Height of Exposed Wall:	2	storeys
	Length-Height Factor	28.9	m-storeys
	Separation Distance	18.81	m
	North Side Exposure Charge	12%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	46	m
	East Side Exposure Charge	0%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Non-combustible	Gas Retail/Drive Thru
	Length of Exposed Wall:	5.4	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	32.22	m
	South Side Exposure Charge	5%	Over 45 m to next structure
	<i>West Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0	m
	Height of Exposed Wall:	0	storeys
	Length-Height Factor	0.0	m-storeys
	Separation Distance	46	m
	West Side Exposure Charge	0%	Over 200 m to next structure
	Total Exposure Charge	17%	The total exposure charge is below the maximum value of 75%.
	Increase for Exposures	340	L/min
H	Fire Flow	2340	L/min
	Rounded Fire Flow	2000	L/min
	<b>City Cap (RFF)</b>	<b>2000</b>	<b>L/min</b>
		33	L/s

Fire Underwriters Survey (FUS) Fire Flow Calculations  
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

## **William Rugamba**

---

**From:** William Rugamba  
**Sent:** July 15, 2024 4:00 PM  
**To:** William Rugamba  
**Subject:** FW: Navan Subdivision - Boundary Condition Request  
**Attachments:** NavanSubdivision\_Boundary Condition(4july2024).docx

**William Rugamba**, M.Eng., B.A.Sc., EIT

Civil Engineering Graduate

Ottawa, ON

Work: [343-804-4374](tel:343-804-4374)

---

**From:** Polyak, Alex <[alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)>  
**Sent:** Monday, July 15, 2024 10:12 AM  
**To:** Mahad Musse <[mmusse@jlrichards.ca](mailto:mmusse@jlrichards.ca)>  
**Cc:** Karla Ferrey <[kferrey@jlrichards.ca](mailto:kferrey@jlrichards.ca)>; Raad Akrawi <[rakrawi@groupeheafey.com](mailto:rakrawi@groupeheafey.com)>; Carmine Zayoun <[carmine@zayoungroup.com](mailto:carmine@zayoungroup.com)>; Armstrong, Justin <[justin.armstrong@ottawa.ca](mailto:justin.armstrong@ottawa.ca)>; Tatyana Roumie <[troumie@jlrichards.ca](mailto:troumie@jlrichards.ca)>  
**Subject:** RE: Navan Subdivision - Boundary Condition Request

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Good morning Mahad,

Please find the boundary conditions attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., C.E.T., P.Eng. 

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.  
Planning, Development and Building Services Department (PDBS) | Direction générale des services de la planification, de l'aménagement et du bâtiment (DGSPAB)

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Email: [alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)  
Cell : 613-857-4380  
[www.Ottawa.ca](http://www.Ottawa.ca)



---

**From:** Mahad Musse <[mmusse@jlrichards.ca](mailto:mmusse@jlrichards.ca)>  
**Sent:** July 12, 2024 1:31 PM  
**To:** Polyak, Alex <[alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)>  
**Cc:** Karla Ferrey <[kferrey@jlrichards.ca](mailto:kferrey@jlrichards.ca)>; Raad Akrawi <[rakrawi@groupeheafey.com](mailto:rakrawi@groupeheafey.com)>; Carmine Zayoun <[carmine@zayoungroup.com](mailto:carmine@zayoungroup.com)>; Armstrong, Justin <[justin.armstrong@ottawa.ca](mailto:justin.armstrong@ottawa.ca)>; Tatyana Roumie <[troumie@jlrichards.ca](mailto:troumie@jlrichards.ca)>  
**Subject:** RE: Navan Subdivision - Boundary Condition Request

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Hi Alex,

Just wondering if you have a status update for the boundary conditions for Navan.

Thanks  
Mahad



**Mahad Musse**, B.Eng., EIT  
Civil Engineering Graduate

1000-343 Preston Street  
Ottawa, ON, K1S 1N4



Work: [343-633-1501](tel:343-633-1501)  
[mmusse@jlrichards.ca](mailto:mmusse@jlrichards.ca)



---

**From:** Mahad Musse <[mmusse@jlrichards.ca](mailto:mmusse@jlrichards.ca)>

**Sent:** Wednesday, July 3, 2024 11:02 AM

**To:** Polyak, Alex <[alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)>

**Cc:** Karla Ferrey <[kferrey@jlrichards.ca](mailto:kferrey@jlrichards.ca)>; Raad Akrawi <[rakrawi@groupeheafey.com](mailto:rakrawi@groupeheafey.com)>; Carmine Zayoun <[carmine@zayoungroup.com](mailto:carmine@zayoungroup.com)>; Armstrong, Justin <[justin.armstrong@ottawa.ca](mailto:justin.armstrong@ottawa.ca)>; Tatyana Roumie <[troumie@jlrichards.ca](mailto:troumie@jlrichards.ca)>

**Subject:** RE: Navan Subdivision - Boundary Condition Request

Good morning Alex,

As we discussed last week our Client is looking into the option of converting the row townhouse units into duplex units (townhouse units with apartments in the basement). As a result, this will increase the total demand on the site and we will therefore require new water boundary conditions. We'd like to note that the footprint of the blocks will not change and neither will their layout or any of the offsets.

As a summary:

- Domestic demands were calculated based on a daily consumption rate of 280 L/cap/day with peaking factors consistent with City of Ottawa Guidelines
- Required Fire Flow (RFF) was calculated in accordance to the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection and the City of Ottawa FUS protocol (Bulletin ISDTB-2014-02 & Bulletin ISDTB-2018-02), which considers material, expose distance & height. We have attached the calculation spreadsheet and the figure.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.74 L/s

Maximum Day Demand: 10.53 L/s

Peak Hour Demand: 18.17 L/s

Required Fire Flow (per FUS): 6,000 L/min (100 L/s)

Required Fire Flow (per FUS): 10,000 L/min (167 L/s)

Required Fire Flow (per FUS): 14,000 L/min (233 L/s)

Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

For your reference, the previous boundary condition received from the City is attached and below is the email chain.

If you have any questions or comments please let us know.

Thanks  
Mahad



**Mahad Musse**, B.Eng., EIT  
Civil Engineering Graduate

1000-343 Preston Street  
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)  
[mmusse@jlrichards.ca](mailto:mmusse@jlrichards.ca)

---

**From:** Polyak, Alex <[alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)>  
**Sent:** Thursday, August 17, 2023 3:01 PM  
**To:** William Rugamba <[wrugamba@jlrichards.ca](mailto:wrugamba@jlrichards.ca)>  
**Cc:** Karla Ferrey <[kferrey@jlrichards.ca](mailto:kferrey@jlrichards.ca)>; Raad Akrawi <[rakrawi@groupeheafey.com](mailto:rakrawi@groupeheafey.com)>; Carmine Zayoun <[carmine@zayoungroup.com](mailto:carmine@zayoungroup.com)>; Shahira Jalal <[sjalal@jlrichards.ca](mailto:sjalal@jlrichards.ca)>  
**Subject:** RE: Navan Subdivision - Boundary Condition Request

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Hello William,

Sorry that I missed your call, I was in a meeting. The boundary conditions are attached.

Regards,

---

**Oleksandr (Alex) Polyak, B.Eng., P.Eng**

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.  
Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

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[www.Ottawa.ca](http://www.Ottawa.ca)



---

**From:** William Rugamba <[wrugamba@jlrichards.ca](mailto:wrugamba@jlrichards.ca)>  
**Sent:** August 15, 2023 9:26 AM  
**To:** Polyak, Alex <[alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)>  
**Cc:** Karla Ferrey <[kferrey@jlrichards.ca](mailto:kferrey@jlrichards.ca)>; Raad Akrawi <[rakrawi@groupeheafey.com](mailto:rakrawi@groupeheafey.com)>; Carmine Zayoun <[carmine@zayoungroup.com](mailto:carmine@zayoungroup.com)>; Shahira Jalal <[sjalal@jlrichards.ca](mailto:sjalal@jlrichards.ca)>  
**Subject:** RE: Navan Subdivision - Boundary Condition Request

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Good morning Alex,

Just wanted to follow up on the status of this boundary request. Please let me know if you need anything else from us.

Thanks,  
William

**William Rugamba, M.Eng.**  
Civil Engineering Intern

J.L. Richards & Associates Limited  
1000-343 Preston Street, Ottawa, ON K1S 1N4  
Direct: 343-804-4374



Platinum  
member

---

**From:** Tatyana Roumie  
**Sent:** Tuesday, July 25, 2023 3:53 PM  
**To:** 'alex.polyak@ottawa.ca' <[alex.polyak@ottawa.ca](mailto:alex.polyak@ottawa.ca)>  
**Cc:** Karla Ferrey <[kferrey@jlrichards.ca](mailto:kferrey@jlrichards.ca)>; Raad Akrawi <[rakrawi@groupeheafey.com](mailto:rakrawi@groupeheafey.com)>; carmine <[carmine@zayoungroup.com](mailto:carmine@zayoungroup.com)>; Shahira Jalal <[sjalal@jlrichards.ca](mailto:sjalal@jlrichards.ca)>  
**Subject:** Navan Subdivision - Boundary Condition Request

Hello Alex.

To support our upcoming detailed design for the site, we are requesting updated boundary conditions for the 3079 Navan Road Development.

As a brief history, we received boundary conditions from the City in July 2021 (attached, but with incorrect connection locations) and again in April 2022 (also attached) in support of the functional servicing design. We understand from the April 2022 boundary conditions that the maximum available fire flow for the site is 250 L/s.

We are currently requesting updated boundary conditions for this site as we are commencing the detailed servicing design and this request will accommodate the recent site plan changes and proposed connection points. This request is also applicable to the upcoming site plan designs which will be submitted as separate applications.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

**Average Day Demand: 6.44 L/s**

**Maximum Day Demand: 9.77 L/s**

**Peak Hour Demand: 16.50 L/s**

**Required Fire Flow (per FUS): 6,000 L/min (100 L/s)**

**Required Fire Flow (per FUS): 10,000 L/min (167 L/s)**

**Required Fire Flow (per FUS): 14,000 L/min (233 L/s)**

**Required Fire Flow (per FUS): 15,000 L/min (250 L/s)**

Thanks,  
Tatyana

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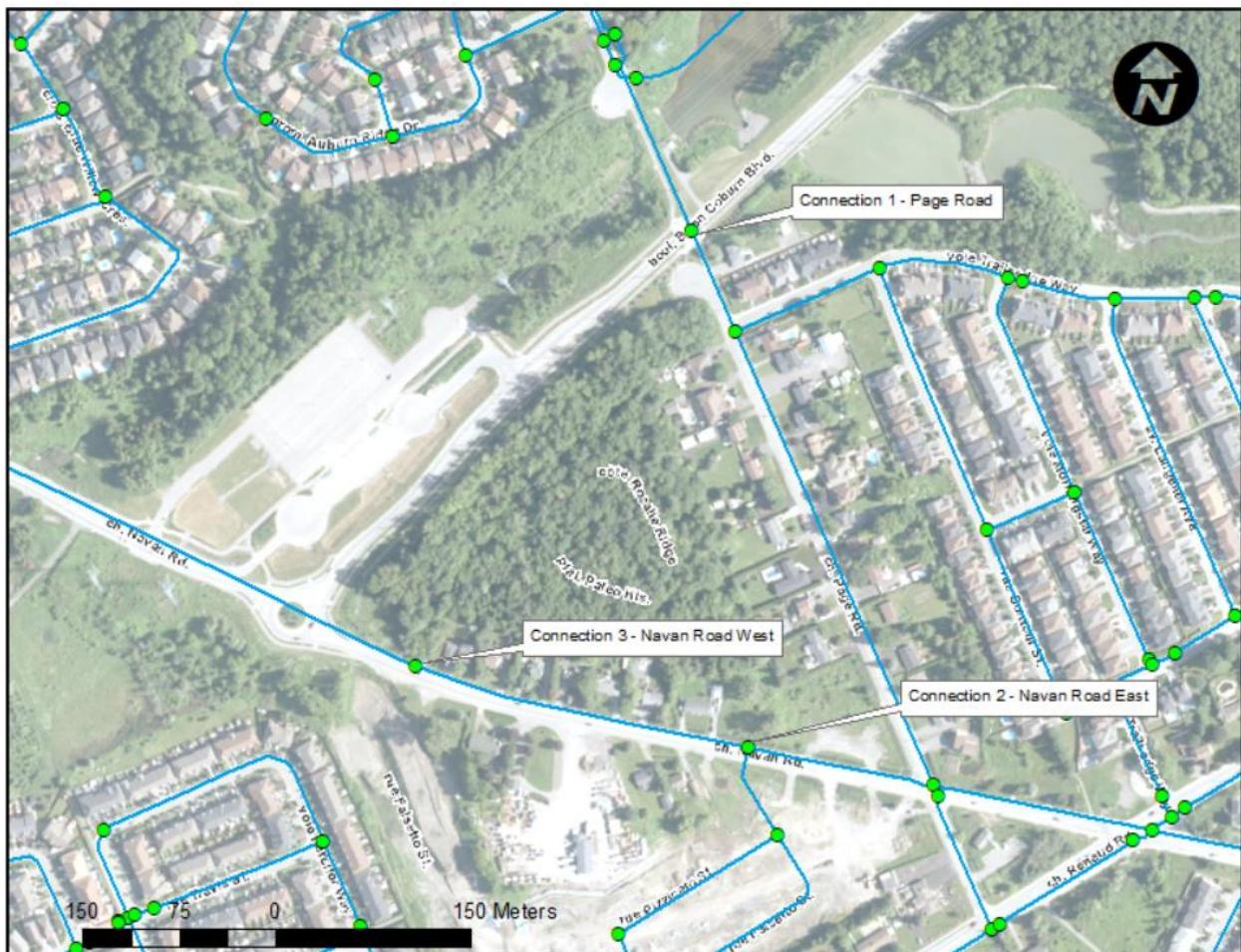
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## Boundary Conditions Navan Subdivision

### Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	404	6.74
Maximum Daily Demand	632	10.53
Peak Hour	1,090	18.17
Fire Flow Demand #1	6,000	100.00
Fire Flow Demand #2	10,000	166.67
Fire Flow Demand #3	14,000	233.33
Fire Flow Demand #4	15,000	250.00

### Location



## **Results**

### **Connection 1 - Page Road**

Demand Scenario	Head (m)	Pressure <sup>1</sup> (psi)
Maximum HGL	130.7	64.0
Peak Hour	127.0	58.6
Max Day plus Fire Flow #1	128.2	60.4
Max Day plus Fire Flow #2	126.8	58.3
Max Day plus Fire Flow #3	124.9	55.7
Max Day plus Fire Flow #4	124.4	55.0

<sup>1</sup> Ground Elevation = 85.7 m

### **Connection 2 - Navan Road East**

Demand Scenario	Head (m)	Pressure <sup>1</sup> (psi)
Maximum HGL	130.7	71.4
Peak Hour	126.8	65.9
Max Day plus Fire Flow #1	127.7	67.1
Max Day plus Fire Flow #2	125.5	64.1
Max Day plus Fire Flow #3	122.7	60.1
Max Day plus Fire Flow #4	121.9	58.9

<sup>1</sup> Ground Elevation = 80.5 m

### **Connection 3 - Navan Road West**

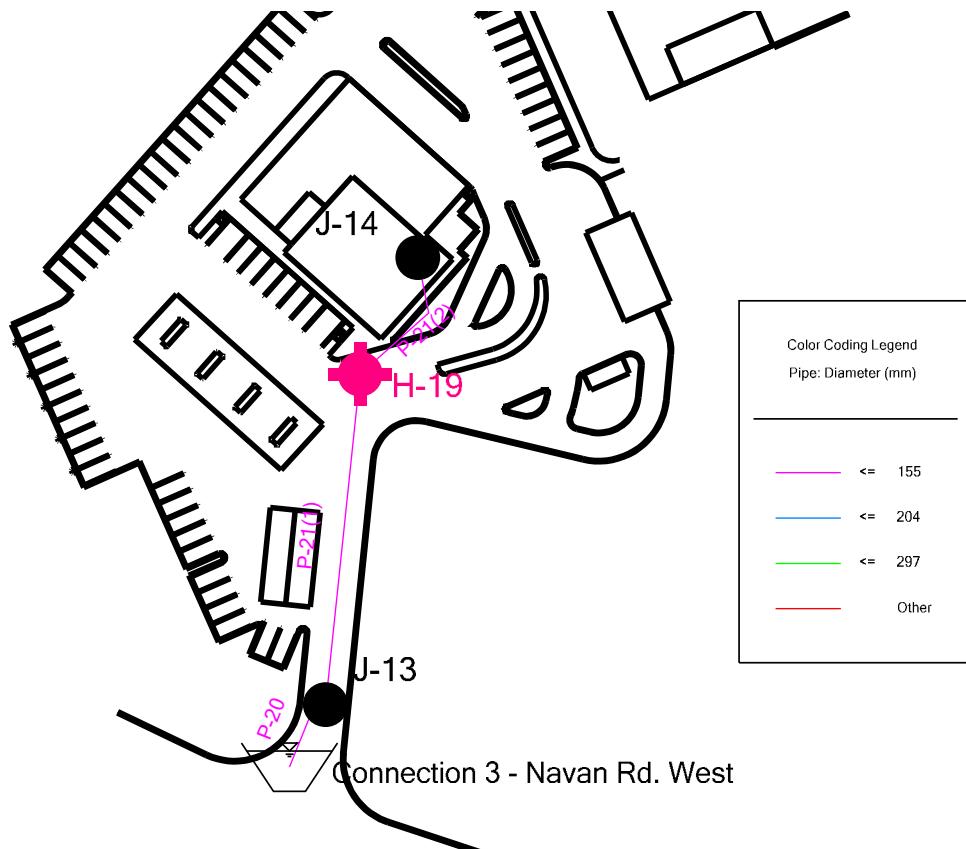
Demand Scenario	Head (m)	Pressure <sup>1</sup> (psi)
Maximum HGL	130.7	69.3
Peak Hour	126.8	63.8
Max Day plus Fire Flow #1	127.3	64.5
Max Day plus Fire Flow #2	124.6	60.6
Max Day plus Fire Flow #3	120.9	55.3
Max Day plus Fire Flow #4	119.8	53.8

<sup>1</sup> Ground Elevation = 81.9 m

## **Disclaimer**

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of water mains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

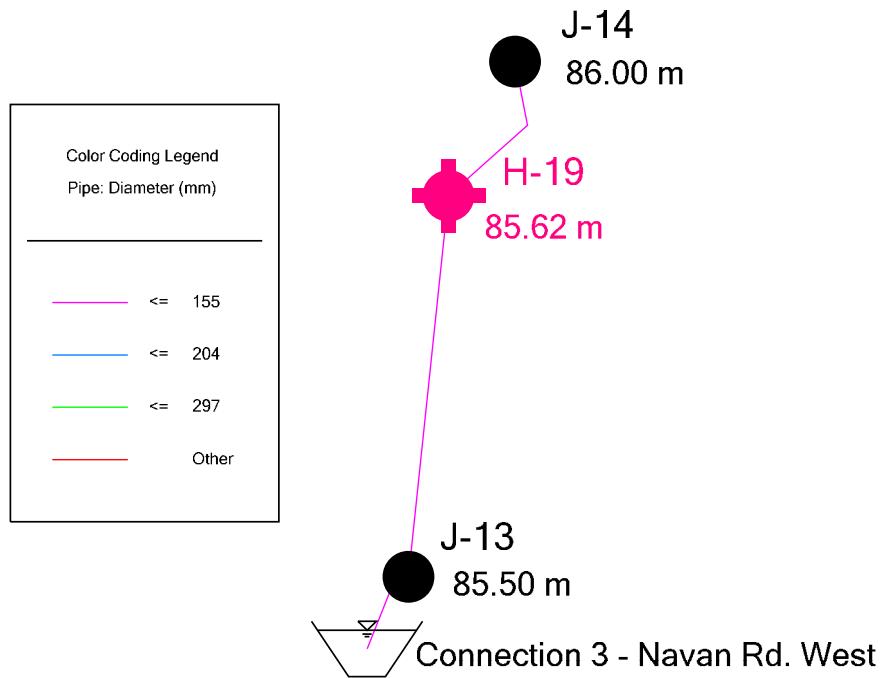
**Gas Bar, Commercial Building, and Car Wash (Block 16)**  
**Model Schematic**



**Gas Bar, Commercial Building, and Car Wash (Block 16)**

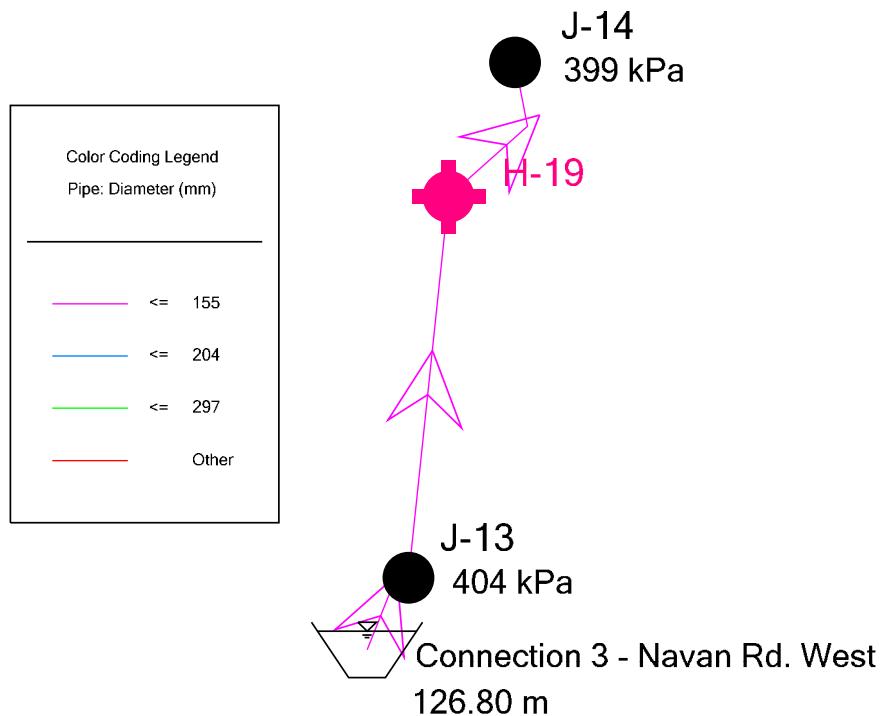
**Model Schematic**

**Elevation Model**



## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Peak Hour Demand**



## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Peak Hour Demand**

#### **Junction Table**

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-14	86.00	4.27	126.74	399
J-13	85.50	0.00	126.79	404

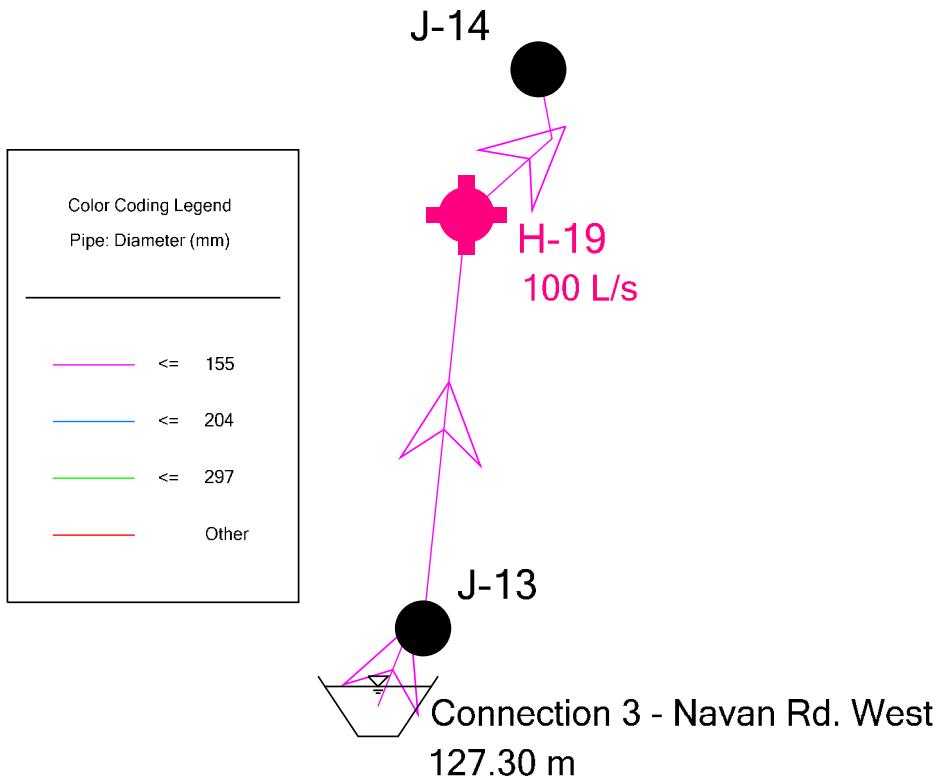
## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Peak Hour Demand**

#### **Pipe Table**

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
61	P-20	11	155	PVC	100.0	4.27	0.23
175	P-21(1)	49	155	PVC	100.0	4.27	0.23
176	P-21(2)	22	155	PVC	100.0	4.27	0.23

**Gas Bar, Commercial Building, and Car Wash (Block 16)**  
**Max Day + Fire Flow Requirement**



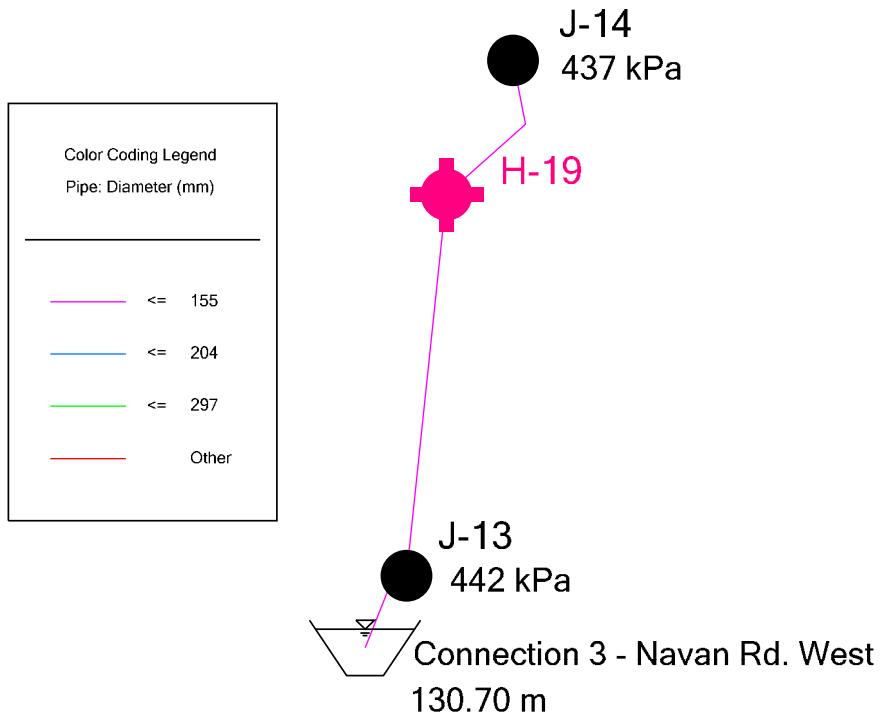
## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Max Day + Fire Flow Requirement**

Label	Satisfies Fire Flow Constraints?	Fire Flow (Available) (L/s)	Flow (Total Available) (L/s)	Pressure (Residual Lower Limit) (kPa)	Pressure (Calculated Residual) (kPa)	Pressure (Calculated System Lower Limit) (kPa)	Junction w/ Minimum Pressure (System)
H-19	True	100	100	140	224	238	J-14

## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Maximum Pressure Analysis**



## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Maximum Pressure Analysis**

#### **Junction Table**

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-14	86.00	0	130.70	437
J-13	85.50	0	130.70	442

## **Gas Bar, Commercial Building, and Car Wash (Block 16)**

### **Maximum Pressure Analysis**

**Pipe Table**

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
61	P-20	11	155	PVC	100.0	0	0.00
175	P-21(1)	49	155	PVC	100.0	0	0.00
176	P-21(2)	22	155	PVC	100.0	0	0.00

**Site Servicing Report**  
**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

---

---

## **Appendix D**

Sanitary Design Sheet

Street Name	MH No.		Residential							Commercial/Institutional					Infiltration			Peak Design Flow L/s	Pipe Data							Upstream Geometry				Downstream Geometry							
	From	To	Multiples	Apartments	Area (ha)	Pop.	Cum. Pop.	Cum. Area (ha)	Peaking Factor	Residential Flow (L/s)	Area (ha)	Cum. Area (ha)	Peaking Factor	Inst. Flow (L/s)	Plug Flow (L/s)	Area (ha)	Cum. Area (ha)	Peak Extr. Flow L/s	Dia	Type	Actual Diameter	Slope	Q Full (L/s)	V Full	Length	Residual Capacity	% Full	TG From	Obvert	Invert	Cover	TG TO	Drop	Obvert	Invert	Cover	
GAS STATION	GAS BAR	22			0.00	0	0	0.00	3.80	0.00	0.77	0.77	1.50	0.37		0.77	0.77	0.25	0.63	200	Circular	203.20	1.00%	34.22	1.06	27.26	33.59	2%	86.000	83.369	83.165	2.631	85.590	0.510	83.096	82.893	2.494
GAS STATION	CAR WASH	22			0.00	0	0	0.00	3.80	0.00		0.00	1.50	0.00	3.60	0.00	0.00	0.00	3.60	200	Circular	203.20	1.00%	34.22	1.06	5.91	30.62	11%	85.770	82.705	82.502	3.065	85.590	0.060	82.646	82.443	2.944
GAS STATION TO EXISTING STUB	22	STUB 16			0.00	0	0	0.00	3.80	0.00		0.77	1.50	0.37	3.60	0.00	0.77	0.25	4.23	200	Circular	203.20	0.65%	27.59	0.85	2.02	23.36	15%	85.590	82.586	82.383	3.004	85.350		82.573	82.370	2.777
EXISTING STUB TO PALEO DRIVE	STUB 16	21			0.04	0	0	0.04	3.80	0.00	0.77	1.50	0.37	3.60	0.04	0.81	0.27	4.24	200	Circular	203.20	0.65%	27.59	0.85	38.75	23.34	15%	85.350	82.573	82.370	2.777	85.041		82.321	82.118	2.720	
			0	0	0.00	0																															

Design Parameters	
Single Family Population	3.4 Cap/Unit
Semi-Detached/Townhouse Population	2.7 Cap/Unit
Apartments Population	1.8 Cap/Unit
Residential Flows	0.30 L/Cap/Day
Infiltration Flows	0.33 L/s/ha
Correction Factor	0.8
Commercial Peak Factor	-
Institutional/Commercial Average Flow	28000 L/gross half d
Manning Coefficient	0.013





Friday, August 16, 2024

J. L. Richards  
1000-343 Preston St.  
Ottawa, Ontario, K1S 1N4

Attn: W. Rugamba

Re: 2983 Navan Road, Ontario - Gas Bar-Commercial-QM&E Project # 23-047  
Confirmations

Dear William,

This letter serves as confirmation of items you sent by e-mail August 12, 2024.

1. Maximum domestic water flow rate is 57 gpm (3.60 L/s).
2. That quantity was arrived at by referring to data provided by the client's car wash specialist.
3. The design of the oil interceptor has been completed, it will be located entirely within the walls of the car wash building.

Please do not hesitate to contact us should you have any questions or comments.

Yours truly,

A handwritten signature in black ink that reads "C.W. Clark".

C. W. Clark, P.Eng  
QM&E Engineering  
CC: by you to whom it may concern

**Site Servicing Report**  
**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

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## **Appendix E**

Storm Design Sheet

## JLR NO. 29899-002 (NAVAN BLOCK 14)

PIPE REACH			Peak Flow Estimation												Sewer Data						Upstream Geometry				Downstream Geometry				Self Cleansing Velocities									
LOCATION	From MH	To MH	C-Factor (1:2)		Total Area (ha)	Cum. Total Area (ha)	Inlet Time (min.)	In Pipe Flow Time (min)	1:2 Year Storm (RATIONAL METHOD)			Plug Flows	Total Peak Flow <sup>(6)</sup> (L/s)	Type	Nominal Dia. (mm)	Actual Dia. (mm)	Slope	Length (m)	Q Full (m/s)	V Full (m/s)	Residual Capacity <sup>(6)</sup> (L/s)	% Full	TG From	Obvert	Invert	Cover	TG To	Drop	Obvert	Invert	Cover	Q/Q Ratio	Flow Depth (mm)	Actual Velocity <sup>(7)</sup> (m/s)	Flow Depth to Dia. Ratio (d/D)			
			0.20	0.90					2.78AR	Cum. 2.78AR	1:2 Year Intensity (mm/hr)																											
GAS STATION	MH520	MH519	0.055	0.265	0.32	0.32	10.00	0.75	10.75	0.69	0.69	76.81	53.33		53.33	CONCRETE	525	533.40	0.40%	57.15	284.46	1.27	231.14	19%	85.374	83.320	82.787	2.05	85.709	0.060	83.090	82.557	2.62	0.19	156.29	0.98	0.29	
GAS STATION	MH519	MH518			0.00	0.32	10.75	0.38	11.12	0.00	0.69	74.05	51.41		51.41	CONCRETE	525	533.40	0.50%	31.97	317.25	1.42	265.84	16%	85.709	83.030	82.497	2.68	85.468	1.052	82.871	82.337	2.60	0.16	145.08	1.04	0.27	
GAS STATION	MH522	MH521	0.066	0.168	0.23	0.23	10.00	0.53	10.53	0.46	0.46	76.81	35.15		35.15	CONCRETE	525	533.40	0.50%	45.09	317.25	1.42	282.10	11%	85.505	83.016	82.482	2.49	85.630		82.790	82.257	2.84	0.11	119.48	0.93	0.22	
GAS STATION	MH521	MH518	0.030	0.149	0.18	0.41	10.53	0.38	10.91	0.39	0.85	74.83	63.33		63.33	CONCRETE	525	533.40	0.35%	27.21	265.81	1.19	202.48	24%	85.630	82.790	82.257	2.84	85.468	0.876	82.695	82.161	2.77	0.24	177.09	0.98	0.33	
GAS STATION	MH518	MH518A	0.021	0.016	0.04	0.77	11.12	0.29	11.41	0.05	1.59	72.74	115.83		66.14	66.14	CONCRETE	450	457.20	0.25%	15.62	148.72	0.91	32.89	78%	85.468	81.819	81.361	3.65	85.591	0.300	81.780	81.322	3.81	0.44	213.51	0.88	0.47
EAST ORLEANS RIDGE SUBDIVISION	EXST MH518A	EXST MH514 <sup>(8)</sup>			Refer to Note 8	1.07	11.41	0.83	12.24	2.29	2.29	71.78	164.59		Refer to Note 8	CONCRETE	525	533.40	0.25%	50.21	224.33	1.00	69.74	73%	85.400	81.480	80.946	3.92	84.620		81.354	80.821	3.27		Refer to Note 8			

Design Parameters (Per OSDG)  
Manning's Coefficient = 0.013  
1:2 Year Intensity =  $732.951 / (T_c + 6.199)^{0.810}$

Note: Tc is the time of concentration in minutes

Drainage Areas Breakdown  
Total Site Area: 0.77 ha  
Controlled Area Within Site Property Line: 0.77 ha →  
Existing Navan Rd Rear-Yard Area Captured Within Site: 0.05 ha  
Block 15 Rear-Yard area captured in 518A-514: 0.07 ha  
Subdivision Block 11 Rear-Yard Area in 518A-514: 0.14  
Existing Navan Rd Rear-Yard Area Captured in 518A-514: 0.04  
Total Captured Areas: 1.07  
Uncontrolled Area - Outlet to Subdivision: 0.00 ha →

Notes on Plug Flows  
(1) Orifice Flow Rate from Block 16  
Notes on Peak Flow and Pipe Sizing  
(5) Peak flows are lower than the allowable release rate for Block 16 (68 L/s)  
(6) Pipes are conservatively sized for 1:2 Year Peak Flow Rate  
(7) Actual Velocities based on actual peak flows from Note 5  
(8) Details from Existing Sewers Downstream of EXST MH518A (referred to as STUB 16) can be found within East Orleans Ridge Subdivision Design Sheet. This line is only used to carry over the downstream values for time of concentration and 1:2 year peak flow

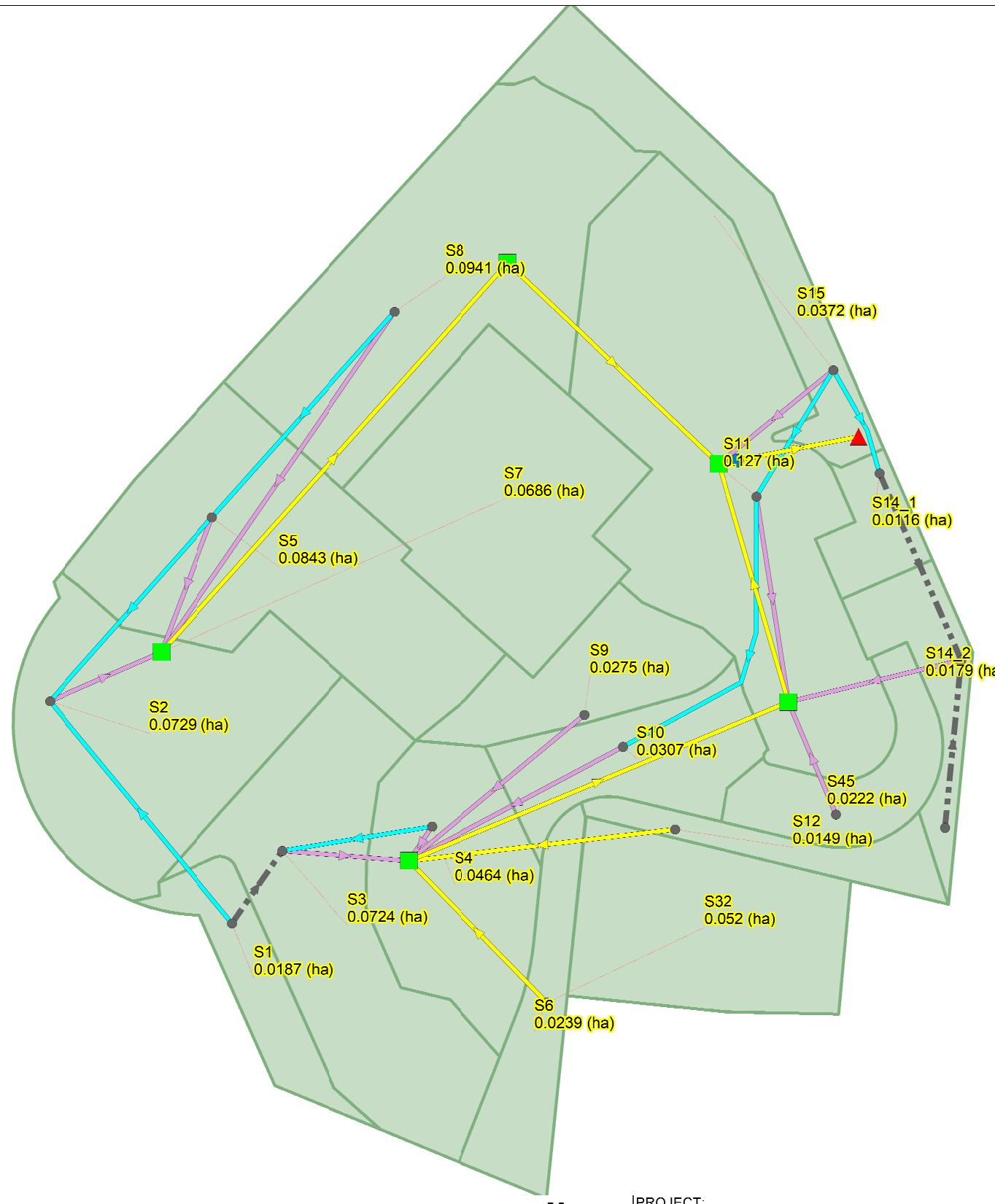
**Site Servicing Report**  
**2983 Navan Road – Block 16**  
**Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash**

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## **Appendix F**

Stormwater Management



### Legend

Junctions
Outfalls
Storage
Manholes
Manholes
Conduits
Storm Sewers
Storm Sewers
Weirs
Outlets
Subcatchments



150 m

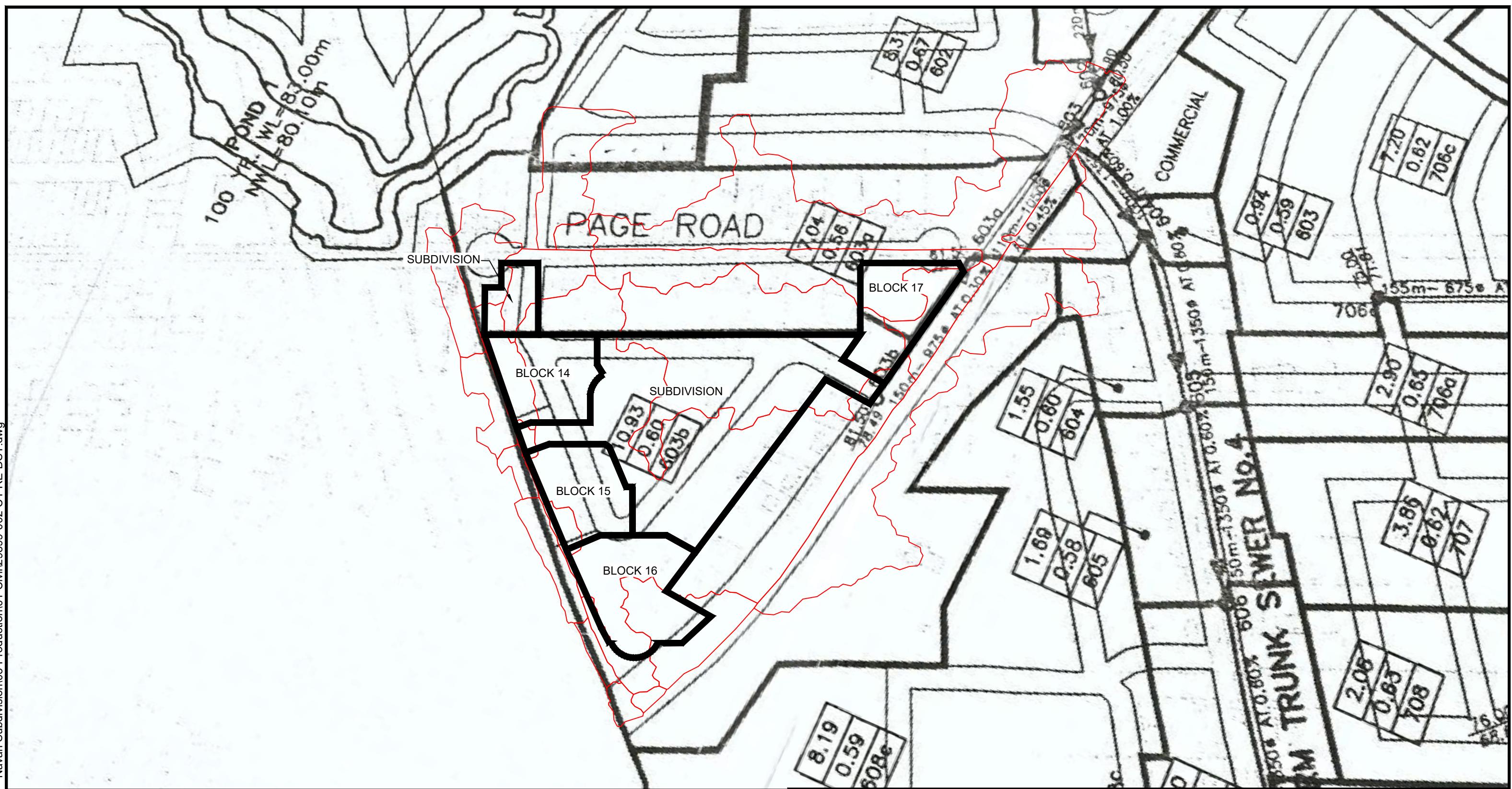
**2983 Navan Road - Block 16**  
Ottawa, ON

### Overall System Model Schematic



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Richards & Associates Limited.

DESIGN:	ML	JLR NO.:	29899-002
DRAWN:	ML	DRAWING NO.:	
CHECKED:	BP		Figure 3



LEGEND:

— PRE-DEVELOPMENT DRAINAGE AREAS

■ NAVAN SITE PLAN AND SUBDIVISION BOUNDARY

NOTE:  
UNDERLYING CATCHMENT DELINEATION FROM GLOUCESTER EUC INFRASTRUCTURE  
SERVICING UPDATE 2005 WHICH INFORMED THE ALLOWABLE RELEASE RATES FROM  
THE SITE AND NO PRE-DEVELOPMENT MODELLING WAS REQUIRED

PROJECT:

2983, 3053 AND 3079 NAVAN RD & 2690 PAGE RD

DRAWING:

PRE-DEVELOPMENT DRAINAGE PLAN



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J.L. Richards & Associates Limited.

DESIGN:	BP
DRAWN:	KC
CHECKED:	BP
JLR #:	29899-002

FIGURE

## Post-Development 3-hour Chicago 1:2 year Event

[TITLE]  
;;Project Title/Notes

[OPTIONS]  
;;Option Value  
FLOW\_UNITS LPS  
INFILTRATION HORTON  
FLOW\_MODELING DYNWAVE  
LINK\_OFFSETS ELEVATION  
MIN\_SLOPE 0  
ALLOW\_PONDING NO  
SKIP\_STEADY\_STATE NO

START\_DATE 01/01/2000  
START\_TIME 00:00:00  
REPORT\_START\_DATE 01/01/2000  
REPORT\_START\_TIME 00:00:00  
END\_DATE 01/01/2000  
END\_TIME 06:00:00  
SWEEP\_START 01/01  
SWEEP\_END 12/31  
DRY\_DAYS 0  
REPORT\_STEP 00:01:00  
WET\_STEP 00:01:00  
DRY\_STEP 00:01:00  
ROUTING\_STEP 1  
RULE\_STEP 00:00:00

INERTIAL\_DAMPING PARTIAL  
NORMAL\_FLOW\_LIMITED BOTH  
FORCE\_MAIN\_EQNATION H-W  
VARIABLE\_STEP 0.75  
LENGTHENING\_STEP 0  
MIN\_SURFAREA 0  
MAX\_TRIALS 8  
HEAD\_TOLERANCE 0.0015  
SYS\_FLOW\_TOL 5  
LAT\_FLOW\_TOL 5  
MINIMUM\_STEP 0.5  
THREADS 12

[EVAPORATION]  
;;Data Source Parameters  
;;  
CONSTANT 0.0  
DRY\_ONLY NO

[RAINGAGES]  
;;Name Format Interval SCF Source  
;;  
3CHI002 INTENSITY 0:10 1.0 TIMESERIES 3CHI002  
3CHI100 INTENSITY 0:10 1.0 TIMESERIES 3CHI100

[SUBCATCHMENTS]  
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen [CONDUITS]  
;;  
SnowPack 3CHI002 CB208 0.0187 0 29.61 2.2 0 ;Name InitFlow From Node To Node Length Roughness InOffset OutOffset  
;;  
S1 3CHI002 CB204 0.0307 99.617 55.32 2.2 0 C2 CB208 CB207 9.666 0.013 83.3 83.2  
S10 3CHI002 CB202 0.127 90.325 65.66 2.2 0 0 0 CB212A CB212 18.475 0.013 83.62 83.25  
S11 3CHI002 CB213 0.0149 0.369 6.44 2.2 0 C4 0 CB211 CB212 22.346 0.013 83.7 83.25  
S14\_1 3CHI002 CB211 0.0116 52.676 22.78 2.2 0 0 0 CB211 CB212 22.346 0.013 83.7 83.25  
S14\_2 3CHI002 CB212 0.0119 31.654 58.12 2.2 0 C5 0 CB211 CB212 22.346 0.013 83.7 83.25  
S15 3CHI002 CB209 0.0379 42.124 11.27 2.2 0 0 0 CB213 CB213 29.34 0.013 83.15 82.757  
S2 3CHI002 CB209 0.0729 62.837 33.61 2.2 0 0 0 CB213 CB213 29.34 0.013 83.15 82.757  
S3 3CHI002 CB207 0.0724 96.692 45.79 2.2 0 0 0 CB214 CB214 21.65 0.013 82.95 82.757  
S32 3CHI002 CB214 0.052 14.286 29.7 2.2 0 0 0 CB214 CB214 21.65 0.013 82.95 82.757  
S4 3CHI002 CB206 0.0464 99.975 24.67 2.2 0 0 0 PVC Pipes  
S45 3CHI002 CB203 0.0222 99.962 14.32 2.2 0 Pipe\_(104)\_P-STM 518\_Orifice 518A\_P-STM 15.623 0.013 81.361 81.322  
S5 3CHI002 CB200 0.0443 89.589 82.96 2.2 0 Pipe\_(104)\_P-STM 518\_Orifice 518A\_P-STM 15.623 0.013 81.361 81.322  
S6 3CHI002 CB204 0.0239 0 46.5 2.2 0 0 0 PVC Pipes  
S7 3CHI002 520\_P-STM 0.0686 100 25.95 2.2 0 Pipe\_(110)\_P-STM 521\_P-STM 518\_P-STM 27.209 0.013 82.257 82.161  
S8 3CHI002 CB201 0.0941 80.117 35.06 2.2 0 0 0 PVC Pipes  
S9 3CHI002 CB205 0.0275 76.83 19.08 2.2 0 0 0 Pipe\_(111)\_P-STM 522\_P-STM 521\_P-STM 45.094 0.013 82.482 82.257

[SUBAREAS]  
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted [ORIFICES]  
;;  
S1 0.013 0.25 1.57 4.67 0 OUTLET ;Name CloseTime From Node To Node Type Offset Qcoeff Gated  
S10 0.013 0.25 1.57 4.67 0 OUTLET ;  
S11 0.013 0.25 1.57 4.67 0 OUTLET ;  
S12 0.013 0.25 1.57 4.67 0 OUTLET ;  
S14\_1 0.013 0.25 1.57 4.67 0 PERVIOUS 100 ;  
S14\_2 0.013 0.25 1.57 4.67 0 PERVIOUS 100 ;  
S15 0.013 0.25 1.57 4.67 0 PERVIOUS 100 ;  
S2 0.013 0.25 1.57 4.67 0 OUTLET ;  
S3 0.013 0.25 1.57 4.67 0 OUTLET ;  
S32 0.013 0.25 1.57 4.67 0 OUTLET ;  
S4 0.013 0.25 1.57 4.67 0 OUTLET ;  
S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100 ;  
S5 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100 ;  
S6 0.013 0.25 1.57 4.67 0 OUTLET ;  
S7 0.013 0.25 1.57 4.67 0 OUTLET ;  
S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100 ;  
S9 0.013 0.25 1.57 4.67 0 OUTLET ;  
[WEIRS]  
;;EndCon EndCoeff From Node To Node RoadWidth RoadSurf Type CrestHt Qcoeff Gated  
;;  
[INFILTRATION]  
;;Subcatchment Param1 Param2 Param3 Param4 Param5  
;;  
S1 76.2 13.2 4.14 7 0 W1 CB200 CB209 TRANSVERSE 85.31 1.84 NO  
S10 76.2 13.2 4.14 7 0 W2 CB210B CB211 TRANSVERSE 85.6 1.84 NO  
S11 76.2 13.2 4.14 7 0 W3 CB210B CB202 TRANSVERSE 85.45 1.84 NO  
S12 76.2 13.2 4.14 7 0 W4 CB202 CB204 TRANSVERSE 85.55 1.84 NO  
S14\_1 76.2 13.2 4.14 7 0 W5 CB20 CB207 TRANSVERSE 85.45 1.84 NO  
S14\_2 76.2 13.2 4.14 7 0 W6 CB201 CB200 TRANSVERSE 85.65 1.84 NO  
S15 76.2 13.2 4.14 7 0 W7 CB200 CB209 TRANSVERSE 85.42 1.84 NO  
S2 76.2 13.2 4.14 7 0 [OUTLETS]  
;;Name Qexpon Gated From Node To Node Offset Type QTable/Qcoeff  
;;  
S3 76.2 13.2 4.14 7 0 CB200 CB200 520\_P-STM 83.18 TABULAR/HEAD IPEX\_Type\_A  
S32 76.2 13.2 4.14 7 0 CB201 CB201 520\_P-STM 83.09 TABULAR/HEAD Vortex\_ICD\_100  
[OUTFALLS]  
;;Name Elevation Type Stage Data Gated Route To  
;;  
;Cylindrical Structure Slab Top Circular Frame SI CB202 CB202 521\_P-STM 82.76 TABULAR/HEAD IPEX\_Type\_A  
518A\_P-STM 80.646 FIXED 81.2 NO CB203 CB203 521\_P-STM 82.75 TABULAR/HEAD Vortex\_ICD\_70  
CB204 CB204 522\_P-STM 82.75 TABULAR/HEAD Vortex\_ICD\_65  
[STORAGE]  
;;Name Elev. MaxDepth InitDepth SurDepth Aponded  
;;  
;Cylindrical Structure Slab Top Circular Frame SI CB205 CB205 522\_P-STM 82.85 TABULAR/HEAD Vortex\_ICD\_65  
518\_P-STM 81.061 4.406 0.139 FUNCTIONAL 0 0 1.13 0 CB206 CB206 522\_P-STM 82.77 TABULAR/HEAD Vortex\_ICD\_70  
CB207 CB207 522\_P-STM 83.15 TABULAR/HEAD Vortex\_ICD\_100  
;Cylindrical Structure Slab Top Circular Frame SI CB209 CB209 520\_P-STM 83.2 TABULAR/HEAD IPEX\_Type\_A  
519\_P-STM 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0 CB210B CB210B 518\_P-STM 82.65 TABULAR/HEAD Vortex\_ICD\_65  
CB212 CB212 521\_P-STM 83.2 TABULAR/HEAD Vortex\_ICD\_100  
;Cylindrical Structure Slab Top Circular Frame SI NO  
520\_P-STM 82.487 2.888 0 FUNCTIONAL 0 0 1.13 0 NO NO  
[XSECTIONS]

;Link Shape Geom1 Geom2 Geom3 Geom4 Barrels Vortex\_ICD\_100 1 8.9  
 Culvert ;----- Vortex\_ICD\_100 1.2 9.8  
 C2 CIRCULAR 0.25 0 0 0 1 Vortex\_ICD\_100 1.4 10.6  
 C4 CIRCULAR 0.25 0 0 0 1 Vortex\_ICD\_100 1.6 11.3  
 C5 CIRCULAR 0.25 0 0 0 1 Vortex\_ICD\_100 1.8 12  
 CB213 CIRCULAR 0.25 0 0 0 1 Vortex\_ICD\_100 2 12.6  
 CB214 CIRCULAR 0.25 0 0 0 1 Vortex\_ICD\_100 2.5 14.1  
 Pipe\_-(104)-(P-Stm)-2 CIRCULAR 0.45 0 0 0 1 Vortex\_ICD\_100 3 15.5  
 Pipe\_-(110)-(P-Stm)-CIRCULAR 0.525 0 0 0 1 ,Tempest Rating Curve for Vortex ICD 105, No grate allowance  
 Pipe\_-(111)-(P-Stm)-CIRCULAR 0.525 0 0 0 1 Vortex\_ICD\_105 Rating 0 0  
 Pipe\_-(71)-(P-Stm) CIRCULAR 0.525 0 0 0 1 Vortex\_ICD\_105 0.1 3.1  
 Pipe\_-(72)-(P-Stm) CIRCULAR 0.525 0 0 0 1 Vortex\_ICD\_105 0.2 4.4  
 OR1 CIRCULAR 0.25 0 0 0 1 Vortex\_ICD\_105 0.3 5.4  
 WL RECT\_OPEN 0.03 1.5 0 0 1 Vortex\_ICD\_105 0.4 6.2  
 W2 RECT\_OPEN 0.05 2.59 0 0 1 Vortex\_ICD\_105 0.5 6.9  
 W3 RECT\_OPEN 0.2 1.34 0 0 1 Vortex\_ICD\_105 0.6 7.6  
 W4 RECT\_OPEN 0.1 4.9 0 0 1 Vortex\_ICD\_105 0.7 8.2  
 W5 RECT\_OPEN 0.05 4.725 0 0 1 Vortex\_ICD\_105 0.8 8.8  
 W6 RECT\_OPEN 0.1 6 0 0 1 Vortex\_ICD\_105 0.9 9.3  
 W7 RECT\_OPEN 0.08 10.5 0 0 1 Vortex\_ICD\_105 1 9.8  
 [LOSSES] ;Link Kentry Kexit Kavg Flap Gate Seepage  
 ;-----  
 [CURVES] ;Name Type X-Value Y-Value ;Tempest Rating Curve for Vortex ICD 40, No grate allowance  
 ;----- Vortex\_ICD\_40 Rating 0 0  
 CB210-OUT85.5 Rating 0 0 Vortex\_ICD\_40 0.1 0.4  
 CB210-OUT85.5 85.5 0.04 Vortex\_ICD\_40 0.2 0.6  
 CB210-OUT85.5 100 0.04 Vortex\_ICD\_40 0.3 0.7  
 CB210-OUT-85.6 Rating 0 0 Vortex\_ICD\_40 0.4 0.9  
 CB210-OUT-85.6 85.6 0.08 Vortex\_ICD\_40 0.5 1  
 CB210-OUT-85.6 100 0.08 Vortex\_ICD\_40 0.6 1.1  
 ;IPEX Type A ICD Rating Curve Vortex\_ICD\_40 0.7 1.1  
 IPEX\_Type\_A Rating 0 0 Vortex\_ICD\_40 0.8 1.2  
 IPEX\_Type\_A 0.1 5.7 Vortex\_ICD\_40 0.9 1.3  
 IPEX\_Type\_A 0.2 8.1 Vortex\_ICD\_40 1 1.4  
 IPEX\_Type\_A 0.3 9.9 Vortex\_ICD\_40 1.2 1.5  
 IPEX\_Type\_A 0.4 11.4 Vortex\_ICD\_40 1.4 1.6  
 IPEX\_Type\_A 0.5 12.8 Vortex\_ICD\_40 1.6 1.7  
 IPEX\_Type\_A 0.6 14 Vortex\_ICD\_40 1.8 1.8  
 IPEX\_Type\_A 0.7 15.1 Vortex\_ICD\_40 2 1.9  
 IPEX\_Type\_A 0.8 16.2 Vortex\_ICD\_40 2.5 2.2  
 IPEX\_Type\_A 0.9 17.2 Vortex\_ICD\_40 3 2.4  
 IPEX\_Type\_A 1 18.1 ;Tempest Rating Curve for Vortex ICD 45, No grate allowance  
 IPEX\_Type\_A 1.2 19.8 Vortex\_ICD\_45 Rating 0 0  
 IPEX\_Type\_A 1.4 21.4 Vortex\_ICD\_45 0.1 0.6  
 IPEX\_Type\_A 1.6 22.9 Vortex\_ICD\_45 0.2 0.8  
 IPEX\_Type\_A 1.8 24.3 Vortex\_ICD\_45 0.3 1  
 IPEX\_Type\_A 2 25.6 Vortex\_ICD\_45 0.4 1.1  
 IPEX\_Type\_A 2.5 28.6 Vortex\_ICD\_45 0.5 1.3  
 IPEX\_Type\_A 3 31.3 Vortex\_ICD\_45 0.6 1.4  
 IPEX\_Type\_A 1 25.7 Vortex\_ICD\_45 0.7 1.5  
 IPEX\_Type\_B Rating 0 0 Vortex\_ICD\_45 0.8 1.6  
 IPEX\_Type\_B 0.1 8.1 Vortex\_ICD\_45 0.9 1.7  
 IPEX\_Type\_B 0.2 11.5 Vortex\_ICD\_45 1 1.8  
 IPEX\_Type\_B 0.3 14.1 Vortex\_ICD\_45 1.2 2  
 IPEX\_Type\_B 0.4 16.2 Vortex\_ICD\_45 1.4 2.1  
 IPEX\_Type\_B 0.5 18.2 Vortex\_ICD\_45 1.6 2.3  
 IPEX\_Type\_B 0.6 19.9 Vortex\_ICD\_45 1.8 2.4  
 IPEX\_Type\_B 0.7 21.5 Vortex\_ICD\_45 2 2.6  
 IPEX\_Type\_B 0.8 23 Vortex\_ICD\_45 2.5 2.9  
 IPEX\_Type\_B 0.9 24.4 Vortex\_ICD\_45 3 3.1  
 IPEX\_Type\_B 1 25.7 ;Tempest Rating Curve for Vortex ICD 50, No grate allowance  
 IPEX\_Type\_B 1.2 28.1 Vortex\_ICD\_50 Rating 0 0  
 IPEX\_Type\_B 1.4 30.4 Vortex\_ICD\_50 0.1 0.7  
 IPEX\_Type\_B 1.6 32.5 Vortex\_ICD\_50 0.2 1  
 IPEX\_Type\_B 1.8 34.4 Vortex\_ICD\_50 0.3 1.2  
 IPEX\_Type\_B 2 36.3 Vortex\_ICD\_50 0.4 1.4  
 IPEX\_Type\_B 2.5 40.6 Vortex\_ICD\_50 0.5 1.6  
 IPEX\_Type\_B 3 44.5 Vortex\_ICD\_50 0.6 1.8  
 IPEX\_Type\_B 1 33.5 Vortex\_ICD\_50 0.7 1.9  
 IPEX\_Type\_B 1.2 36.6 Vortex\_ICD\_50 0.8 2  
 IPEX\_Type\_B 1.4 39.6 Vortex\_ICD\_50 0.9 2.1  
 IPEX\_Type\_B 1.6 42.3 Vortex\_ICD\_50 1 2.3  
 IPEX\_Type\_B 1.8 44.9 Vortex\_ICD\_50 1.2 2.5  
 IPEX\_Type\_B 2 47.3 Vortex\_ICD\_50 1.4 2.7  
 IPEX\_Type\_B 2.5 52.9 Vortex\_ICD\_50 1.6 2.9  
 IPEX\_Type\_B 3 57.9 Vortex\_ICD\_50 1.8 3  
 IPEX\_Type\_C Rating 0 0 ;Tempest Rating Curve for Vortex ICD 55, No grate allowance  
 IPEX\_Type\_C 0 0 Vortex\_ICD\_55 Rating 0 0  
 IPEX\_Type\_C 0.1 10.6 Vortex\_ICD\_55 0.1 0.9  
 IPEX\_Type\_C 0.2 15 Vortex\_ICD\_55 0.2 1.2  
 IPEX\_Type\_C 0.3 18.3 Vortex\_ICD\_55 0.3 1.5  
 IPEX\_Type\_C 0.4 21.2 Vortex\_ICD\_55 0.4 1.7  
 IPEX\_Type\_C 0.5 23.7 Vortex\_ICD\_55 0.5 1.9  
 IPEX\_Type\_C 0.6 25.9 Vortex\_ICD\_55 0.6 2.1  
 IPEX\_Type\_C 0.7 28 Vortex\_ICD\_55 0.7 2.3  
 IPEX\_Type\_C 0.8 29.9 Vortex\_ICD\_55 0.8 2.4  
 IPEX\_Type\_C 0.9 31.7 Vortex\_ICD\_55 0.9 2.6  
 IPEX\_Type\_C 1 33.5 Vortex\_ICD\_55 1 2.7  
 IPEX\_Type\_C 1.2 36.6 Vortex\_ICD\_55 1.2 3  
 IPEX\_Type\_C 1.4 39.6 Vortex\_ICD\_55 1.4 3.2  
 IPEX\_Type\_C 1.6 42.3 Vortex\_ICD\_55 1.6 3.4  
 IPEX\_Type\_C 1.8 44.9 Vortex\_ICD\_55 1.8 3.6  
 IPEX\_Type\_C 2 47.3 Vortex\_ICD\_55 2 3.8  
 IPEX\_Type\_C 2.5 52.9 Vortex\_ICD\_55 2.5 4.3  
 IPEX\_Type\_C 3 57.9 Vortex\_ICD\_55 3 4.7  
 IPEX\_Type\_AA ICD Rating Curve ;Tempest Rating Curve for Vortex ICD 60, No grate allowance  
 IPEX\_Type\_D Rating 0 0 Vortex\_ICD\_60 Rating 0 0  
 IPEX\_Type\_D 0 0 Vortex\_ICD\_60 0.1 1.1  
 IPEX\_Type\_D 0.1 15.4 Vortex\_ICD\_60 0.2 1.5  
 IPEX\_Type\_D 0.2 21.7 Vortex\_ICD\_60 0.3 1.9  
 IPEX\_Type\_D 0.3 26.6 Vortex\_ICD\_60 0.4 2.1  
 IPEX\_Type\_D 0.4 30.7 Vortex\_ICD\_60 0.5 2.3  
 IPEX\_Type\_D 0.5 34.3 Vortex\_ICD\_60 0.6 2.5  
 IPEX\_Type\_D 0.6 37.6 Vortex\_ICD\_60 0.7 2.7  
 IPEX\_Type\_D 0.7 40.6 Vortex\_ICD\_60 0.8 2.9  
 IPEX\_Type\_D 0.8 43.4 Vortex\_ICD\_60 0.9 3.1  
 IPEX\_Type\_D 0.9 46.1 Vortex\_ICD\_60 1 3.2  
 IPEX\_Type\_D 1 48.5 Vortex\_ICD\_60 1.2 3.6  
 IPEX\_Type\_D 1.2 53.2 Vortex\_ICD\_60 1.4 3.8  
 IPEX\_Type\_D 1.4 57.4 Vortex\_ICD\_60 1.6 4.1  
 IPEX\_Type\_D 1.6 61.4 Vortex\_ICD\_60 1.8 4.3  
 IPEX\_Type\_D 1.8 65.1 Vortex\_ICD\_60 2 4.6  
 IPEX\_Type\_D 2 68.7 Vortex\_ICD\_60 2.5 5.1  
 IPEX\_Type\_D 2.5 76.8 Vortex\_ICD\_60 3 5.6  
 IPEX\_Type\_D 3 84.1 ;Tempest Rating Curve for Vortex ICD 65, No grate allowance  
 IPEX\_Type\_E Rating 0 0 Vortex\_ICD\_65 Rating 0 0  
 IPEX\_Type\_E 0 0 Vortex\_ICD\_65 0.1 1.2  
 IPEX\_Type\_E 0.1 20.5 Vortex\_ICD\_65 0.2 1.6  
 IPEX\_Type\_E 0.2 28.9 Vortex\_ICD\_65 0.3 2  
 IPEX\_Type\_E 0.3 35.5 Vortex\_ICD\_65 0.4 2.3  
 IPEX\_Type\_E 0.4 40.9 Vortex\_ICD\_65 0.5 2.5  
 IPEX\_Type\_E 0.5 45.8 Vortex\_ICD\_65 0.6 2.8  
 IPEX\_Type\_E 0.6 50.1 Vortex\_ICD\_65 0.7 3  
 IPEX\_Type\_E 0.7 54.2 Vortex\_ICD\_65 0.8 3.2  
 IPEX\_Type\_E 0.8 57.9 Vortex\_ICD\_65 0.9 3.4  
 IPEX\_Type\_E 0.9 61.4 Vortex\_ICD\_65 1 3.6  
 IPEX\_Type\_E 1 64.7 Vortex\_ICD\_65 1.2 3.8  
 IPEX\_Type\_E 1.2 70.9 Vortex\_ICD\_65 1.4 4.1  
 IPEX\_Type\_E 1.4 76.6 Vortex\_ICD\_65 1.6 4.3  
 IPEX\_Type\_E 1.6 81.9 Vortex\_ICD\_65 1.8 4.6  
 IPEX\_Type\_E 1.8 86.8 Vortex\_ICD\_65 2 4.9  
 IPEX\_Type\_E 2 91.5 Vortex\_ICD\_65 2.5 5.1  
 IPEX\_Type\_E 2.5 102.3 Vortex\_ICD\_65 3 5.7  
 IPEX\_Type\_E 3 112.1 ;Tempest Rating Curve for Vortex ICD 100, No grate allowance  
 Vortex\_ICD\_100 Rating 0 0 Vortex\_ICD\_100 Rating 0 0  
 Vortex\_ICD\_100 0 0 Vortex\_ICD\_100 0.1 0.9  
 Vortex\_ICD\_100 0.1 2.8 Vortex\_ICD\_100 0.2 1.6  
 Vortex\_ICD\_100 0.2 4 Vortex\_ICD\_100 0.3 2  
 Vortex\_ICD\_100 0.3 4.9 Vortex\_ICD\_100 0.4 2.3  
 Vortex\_ICD\_100 0.4 5.6 Vortex\_ICD\_100 0.5 2.5  
 Vortex\_ICD\_100 0.5 6.3 Vortex\_ICD\_100 0.6 2.8  
 Vortex\_ICD\_100 0.6 6.9 Vortex\_ICD\_100 0.7 3  
 Vortex\_ICD\_100 0.7 7.5 Vortex\_ICD\_100 0.8 3.2  
 Vortex\_ICD\_100 0.8 8 Vortex\_ICD\_100 0.9 3.4  
 Vortex\_ICD\_100 0.9 8.5 Vortex\_ICD\_100 1 3.6

# Post-Development 3-hour Chicago 1:2 year Event

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;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex_ICD_70 Rating 0 0
Vortex_ICD_70 0.1 1.3
Vortex_ICD_70 0.2 1.9
Vortex_ICD_70 0.3 2.3
Vortex_ICD_70 0.4 2.7
Vortex_ICD_70 0.5 3
Vortex_ICD_70 0.6 3.3
Vortex_ICD_70 0.7 3.6
Vortex_ICD_70 0.8 3.8
Vortex_ICD_70 0.9 4.1
Vortex_ICD_70 1 4.3
Vortex_ICD_70 1.2 4.7
Vortex_ICD_70 1.4 5.1
Vortex_ICD_70 1.6 5.5
Vortex_ICD_70 1.8 5.8
Vortex_ICD_70 2 6.1
Vortex_ICD_70 2.5 6.8
Vortex_ICD_70 3 7.5

;Tempest Rating Curve for Vortex ICD 75, No grate allowance
Vortex_ICD_75 Rating 0
Vortex_ICD_75 0.1 1.6
Vortex_ICD_75 0.2 2.2
Vortex_ICD_75 0.3 2.7
Vortex_ICD_75 0.4 3.2
Vortex_ICD_75 0.5 3.5
Vortex_ICD_75 0.6 3.9
Vortex_ICD_75 0.7 4.2
Vortex_ICD_75 0.8 4.5
Vortex_ICD_75 0.9 4.8
Vortex_ICD_75 1 5
Vortex_ICD_75 1.2 5.5
Vortex_ICD_75 1.4 5.9
Vortex_ICD_75 1.6 6.3
Vortex_ICD_75 1.8 6.7
Vortex_ICD_75 2 7.1
Vortex_ICD_75 2.5 7.9
Vortex_ICD_75 3 8.7

;Tempest Rating Curve for Vortex ICD 80, No grate allowance
Vortex_ICD_80 Rating 0
Vortex_ICD_80 0.1 1.8
Vortex_ICD_80 0.2 2.6
Vortex_ICD_80 0.3 3.1
Vortex_ICD_80 0.4 3.6
Vortex_ICD_80 0.5 4
Vortex_ICD_80 0.6 4.4
Vortex_ICD_80 0.7 4.8
Vortex_ICD_80 0.8 5.1
Vortex_ICD_80 0.9 5.4
Vortex_ICD_80 1 5.7
Vortex_ICD_80 1.2 6.3
Vortex_ICD_80 1.4 6.8
Vortex_ICD_80 1.6 7.2
Vortex_ICD_80 1.8 7.7
Vortex_ICD_80 2 8.1
Vortex_ICD_80 2.5 9
Vortex_ICD_80 3 9.9

;Tempest Rating Curve for Vortex ICD 85, No grate allowance
Vortex_ICD_85 Rating 0
Vortex_ICD_85 0.1 2
Vortex_ICD_85 0.2 2.9
Vortex_ICD_85 0.3 3.5
Vortex_ICD_85 0.4 4.1
Vortex_ICD_85 0.5 4.5
Vortex_ICD_85 0.6 5
Vortex_ICD_85 0.7 5.4
Vortex_ICD_85 0.8 5.7
Vortex_ICD_85 0.9 6.1
Vortex_ICD_85 1 6.4
Vortex_ICD_85 1.2 7
Vortex_ICD_85 1.4 7.6
Vortex_ICD_85 1.6 8.1
Vortex_ICD_85 1.8 8.6
Vortex_ICD_85 2 9.1
Vortex_ICD_85 2.5 10.1
Vortex_ICD_85 3 11.1

;Tempest Rating Curve for Vortex ICD 90, No grate allowance
Vortex_ICD_90 Rating 0
Vortex_ICD_90 0.1 2.2
Vortex_ICD_90 0.2 3.2
Vortex_ICD_90 0.3 3.9
Vortex_ICD_90 0.4 4.5
Vortex_ICD_90 0.5 5.1
Vortex_ICD_90 0.6 5.5
Vortex_ICD_90 0.7 6
Vortex_ICD_90 0.8 6.4
Vortex_ICD_90 0.9 6.8
Vortex_ICD_90 1 7.2
Vortex_ICD_90 1.2 7.9
Vortex_ICD_90 1.4 8.5
Vortex_ICD_90 1.6 9.1
Vortex_ICD_90 1.8 9.6
Vortex_ICD_90 2 10.2
Vortex_ICD_90 2.5 11.4
Vortex_ICD_90 3 12.5

;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex_ICD_95 Rating 0
Vortex_ICD_95 0.1 2.6
Vortex_ICD_95 0.2 3.6
Vortex_ICD_95 0.3 4.4
Vortex_ICD_95 0.4 5.1
Vortex_ICD_95 0.5 5.7
Vortex_ICD_95 0.6 6.2
Vortex_ICD_95 0.7 6.7
Vortex_ICD_95 0.8 7.1
Vortex_ICD_95 0.9 7.6
Vortex_ICD_95 1 8
Vortex_ICD_95 1.2 8.7
Vortex_ICD_95 1.4 9.4
Vortex_ICD_95 1.6 10.1
Vortex_ICD_95 1.8 10.7
Vortex_ICD_95 2 11.3
Vortex_ICD_95 2.5 12.6
Vortex_ICD_95 3 13.8

CB200 Storage 0 0.36
CB200 2 0.36
CB200 2.24 210.73
CB200 2.32 210.73

CB201 Storage 0 0.36
CB201 2.41 0.36
CB201 2.56 169.64
CB201 2.66 169.64

CB202 Storage 0 0.36
CB202 2.6 0.36
CB202 2.79 283.64
CB202 2.89 283.64

CB203 Storage 0 0.36
CB203 2.6 0.36
CB203 2.9 178.09

CB204 Storage 0 0.36
CB204 2.6 0.36
CB204 2.75 146.08

CB205 Storage 0 0.36
CB205 2.6 0.36
CB205 2.75 86.52

CB206 Storage 0 0.36
CB206 2.6 0.36

[TIMESERIES]
;Name Date Time Value
-----
;Rainfall (mm/hr)
3CHI002 01/01/2000 00:00:00 2,491
3CHI002 01/01/2000 00:10:00 2,966
3CHI002 01/01/2000 00:20:00 3,696
3CHI002 01/01/2000 00:30:00 4,976
3CHI002 01/01/2000 00:40:00 7,828
3CHI002 01/01/2000 00:50:00 19,366
3CHI002 01/01/2000 01:00:00 76,805
3CHI002 01/01/2000 01:10:00 22,777
3CHI002 01/01/2000 01:20:00 11,852
3CHI002 01/01/2000 01:30:00 8,025
3CHI002 01/01/2000 01:40:00 6,096
3CHI002 01/01/2000 01:50:00 4,938
3CHI002 01/01/2000 02:00:00 4,165
3CHI002 01/01/2000 02:10:00 3,113
3CHI002 01/01/2000 02:20:00 3,197
3CHI002 01/01/2000 02:30:00 2,873
3CHI002 01/01/2000 02:40:00 2,613
3CHI002 01/01/2000 02:50:00 2.4
3CHI002 01/01/2000 03:00:00 0

,Rainfall (mm/hr)
3CHI100 01/01/2000 00:00:00 5,339
3CHI100 01/01/2000 00:10:00 6,376
3CHI100 01/01/2000 00:20:00 7,977
3CHI100 01/01/2000 00:30:00 10,797
3CHI100 01/01/2000 00:40:00 17,136
3CHI100 01/01/2000 00:50:00 45,128
3CHI100 01/01/2000 01:00:00 178,107
3CHI100 01/01/2000 01:10:00 51,056
3CHI100 01/01/2000 01:20:00 26,163
3CHI100 01/01/2000 01:30:00 17,571
3CHI100 01/01/2000 01:40:00 13,277
3CHI100 01/01/2000 01:50:00 10,712
3CHI100 01/01/2000 02:00:00 9,008
3CHI100 01/01/2000 02:10:00 7,753
3CHI100 01/01/2000 02:20:00 6,883
3CHI100 01/01/2000 02:30:00 6,174
3CHI100 01/01/2000 02:40:00 5,607
3CHI100 01/01/2000 02:50:00 5,142
3CHI100 01/01/2000 03:00:00 0

[REPORT]
;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]
Node 518_Orifice RY_Manhole
Node CB200 RY_Manhole
Node CB201 RY_Manhole
Node CB202 RY_Manhole
Node CB203 RY_Manhole
Node CB204 RY_Manhole
Node CB205 RY_Manhole
Node CB206 RY_Manhole
Node CB207 RY_Manhole
Node CB208 RY_Manhole
Node CB209 RY_Manhole
Node CB210B RY_Manhole
Node CB211 RY_Manhole
Node CB212 RY_Manhole
Node CB212A RY_Manhole
Node CB213 RY_Manhole
Node CB214 RY_Manhole
Link C2 RY_Sewer
Link C4 RY_Sewer
Link C5 RY_Sewer

[MAP]
DIMENSIONS 381219.0294 5032865.6782 381334.8286 5033009.9278
UNITS Meters

[COORDINATES]
;Node X-Coord Y-Coord
-----
518_Orifice 381303.7 5032953.188
518A_(P-Stm) 381317.029 5032955.797
518_(P-Stm) 381301.695 5032952.807
519_(P-Stm) 381278.558 5032974.866
520_(P-Stm) 381240.562 5032932.182
521_(P-Stm) 381267.744 5032909.775
522_(P-Stm) 381267.739 5032909.252
CB200 381246.119 5032946.933
CB201 381266.142 5032969.504
CB202 381305.816 5032949.229
CB203 381314.519 5032914.342
CB204 381291.187 5032821.79
CB205 381267.739 5032909.262
CB206 381270.233 5032913.017
CB207 381253.811 5032910.356
CB208 381248.314 5032902.406
CB209 381228.288 5032926.78
CB210B 381314.246 5032963.095
CB211 381319.351 5032951.771
CB212 381324.283 5032946.238
CB212A 381326.077 5032912.9
CB213 381296.875 5032912.718
CB214 381282.831 5032893.727

[VERTICES]
;Link X-Coord Y-Coord
-----
```









# Post-Development 3-hour Chicago 1:2 year Event

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S9      381299.199    5032927.907
S9      381298.873    5032927.758
S9      381298.542    5032927.621
S9      381298.206    5032927.496
S9      381297.866    5032927.382
S9      381287.164    5032924.395
S9      381276.098    5032921.781
S9      381275.769    5032921.545
S9      381273.896    5032926.903
S9      381277.469    5032930.913
S9      381278.399    5032931.986
S9      381281.347    5032929.355
S9      381290.74     5032939.886

;;Storage Node X-Coord Y-Coord
;;-----[SYMBOLS]
;:Gage X-Coord Y-Coord
;;-----[SYMBOLS]

***** EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4) *****
***** Element Count *****
Number of rain gages ..... 2
Number of subcatchments .... 17
Number of nodes ..... 23
Number of links ..... 29
Number of pollutants ..... 0
Number of land uses ..... 0

***** Rainage Summary *****
***** Subcatchment Summary *****
***** Node Summary *****
***** Link Summary *****
***** Cross Section Summary *****

-----[Tables]


| Name    | Data Source | Data Type | Recording Interval |
|---------|-------------|-----------|--------------------|
| 3CHI002 | 3CHI002     | INTENSITY | 10 min.            |
| 3CHI100 | 3CHI100     | INTENSITY | 10 min.            |



| Name  | Area | Width  | %Imperv | %Slope | Rain Gage | Outlet      |
|-------|------|--------|---------|--------|-----------|-------------|
| S1    | 0.02 | 29.61  | 0.00    | 2.2000 | 3CHI002   | CB208       |
| S10   | 0.03 | 55.32  | 99.62   | 2.2000 | 3CHI002   | CB204       |
| S11   | 0.13 | 65.66  | 90.33   | 2.2000 | 3CHI002   | CB202       |
| S12   | 0.01 | 6.44   | 0.07    | 2.2000 | 3CHI002   | CB203       |
| S14_1 | 0.01 | 24.78  | 52.68   | 2.2000 | 3CHI002   | CB211       |
| S14_2 | 0.02 | 58.12  | 31.65   | 2.2000 | 3CHI002   | CB212       |
| S15   | 0.04 | 108.74 | 42.75   | 2.2000 | 3CHI002   | CB210B      |
| S2    | 0.07 | 33.61  | 62.94   | 2.2000 | 3CHI002   | CB209       |
| S3    | 0.07 | 45.79  | 96.69   | 2.2000 | 3CHI002   | CB207       |
| S32   | 0.05 | 29.70  | 14.29   | 2.2000 | 3CHI002   | CB214       |
| S4    | 0.05 | 27.67  | 99.47   | 2.2000 | 3CHI002   | CB205       |
| S45   | 0.02 | 44.22  | 99.96   | 2.2000 | 3CHI002   | CB203       |
| S5    | 0.08 | 82.96  | 89.59   | 2.2000 | 3CHI002   | CB200       |
| S6    | 0.02 | 40.30  | 0.00    | 2.2000 | 3CHI002   | CB214       |
| S7    | 0.07 | 25.95  | 100.00  | 2.2000 | 3CHI002   | 520_(P-Stm) |
| S8    | 0.09 | 35.06  | 80.12   | 2.2000 | 3CHI002   | CB201       |
| S9    | 0.03 | 19.08  | 76.83   | 2.2000 | 3CHI002   | CB205       |



| Name         | Type     | Invert Elev. | Max. Depth | Ponded Area | External Inflow |
|--------------|----------|--------------|------------|-------------|-----------------|
| 518_Orifice  | JUNCTION | 81.36        | 4.11       | 0.0         |                 |
| 518A_(P-Stm) | OUTFALL  | 80.65        | 1.13       | 0.0         |                 |
| 518_(P-Stm)  | STORAGE  | 81.06        | 4.41       | 0.0         |                 |
| 519_(P-Stm)  | STORAGE  | 82.20        | 3.51       | 0.0         |                 |
| 520_(P-Stm)  | STORAGE  | 82.49        | 2.89       | 0.0         |                 |
| 521_(P-Stm)  | STORAGE  | 81.96        | 3.67       | 0.0         |                 |
| 522_(P-Stm)  | STORAGE  | 82.18        | 3.32       | 0.0         |                 |
| CB200        | STORAGE  | 83.18        | 2.32       | 0.0         |                 |
| CB201        | STORAGE  | 83.09        | 2.66       | 0.0         |                 |
| CB202        | STORAGE  | 82.76        | 2.89       | 0.0         |                 |
| CB203        | STORAGE  | 82.75        | 2.90       | 0.0         |                 |
| CB204        | STORAGE  | 82.75        | 2.75       | 0.0         |                 |
| CB205        | STORAGE  | 82.85        | 2.75       | 0.0         |                 |
| CB206        | STORAGE  | 82.77        | 2.73       | 0.0         |                 |
| CB207        | STORAGE  | 83.15        | 2.30       | 0.0         |                 |
| CB208        | STORAGE  | 83.30        | 2.04       | 0.0         |                 |
| CB209        | STORAGE  | 83.20        | 1.90       | 0.0         |                 |
| CB210B       | STORAGE  | 82.65        | 3.00       | 0.0         |                 |
| CB211        | STORAGE  | 83.70        | 1.90       | 0.0         |                 |
| CB212        | STORAGE  | 83.20        | 2.42       | 0.0         |                 |
| CB212A       | STORAGE  | 83.62        | 2.00       | 0.0         |                 |
| CB213        | STORAGE  | 83.15        | 1.75       | 0.0         |                 |
| CB214        | STORAGE  | 82.95        | 1.90       | 0.0         |                 |



| Name                               | From Node                        | To Node     | Type    | Length | %Slope |
|------------------------------------|----------------------------------|-------------|---------|--------|--------|
| C2                                 | CB208                            | CB207       | CONDUIT | 9.7    | 1.0346 |
| 0.0130                             | CB212A                           | CB212       | CONDUIT | 18.5   | 2.0031 |
| C4                                 | CB211                            | CB212       | CONDUIT | 22.3   | 2.0142 |
| 0.0130                             | CB213                            | 522_(P-Stm) | CONDUIT | 29.3   | 1.3396 |
| C5                                 | CB211                            | CB212       | CONDUIT | 21.6   | 0.8915 |
| CB213                              | CB213                            | 522_(P-Stm) | CONDUIT | 29.3   | 1.3396 |
| 0.0130                             | CB214                            | 522_(P-Stm) | CONDUIT | 21.6   | 0.8915 |
| Pipe_-(104)_-(P-Stm)_2 518_Orifice | 518A_(P-Stm)                     | CONDUIT     | 15.6    | 0.2496 |        |
| 0.0130                             | Pipe_-(110)_-(P-Stm) 521_(P-Stm) | 518_(P-Stm) | CONDUIT | 27.2   | 0.3528 |
| 0.0130                             | Pipe_-(111)_-(P-Stm) 522_(P-Stm) | 521_(P-Stm) | CONDUIT | 45.1   | 0.4990 |
| 0.0130                             | Pipe_-(71)_-(P-Stm) 520_(P-Stm)  | 519_(P-Stm) | CONDUIT | 57.1   | 0.4025 |
| 0.0130                             | Pipe_-(72)_-(P-Stm) 519_(P-Stm)  | 518_(P-Stm) | CONDUIT | 32.0   | 0.5005 |
| 0.0130                             | OR1                              | 518_(P-Stm) | ORIFICE |        |        |
| W1                                 | CB208                            | CB209       | WEIR    |        |        |
| W2                                 | CB209                            | CB211       | WEIR    |        |        |
| W3                                 | CB210B                           | CB202       | WEIR    |        |        |
| W4                                 | CB202                            | CB204       | WEIR    |        |        |
| W5                                 | CB206                            | CB207       | WEIR    |        |        |
| W6                                 | CB201                            | CB200       | WEIR    |        |        |
| W7                                 | CB200                            | CB209       | WEIR    |        |        |
| CB200                              | CB200                            | 520_(P-Stm) | OUTLET  |        |        |
| CB201                              | CB201                            | 520_(P-Stm) | OUTLET  |        |        |
| CB202                              | CB202                            | 521_(P-Stm) | OUTLET  |        |        |
| CB203                              | CB203                            | 521_(P-Stm) | OUTLET  |        |        |
| CB204                              | CB204                            | 522_(P-Stm) | OUTLET  |        |        |
| CB205                              | CB205                            | 522_(P-Stm) | OUTLET  |        |        |
| CB206                              | CB206                            | 522_(P-Stm) | OUTLET  |        |        |
| CB207                              | CB207                            | 522_(P-Stm) | OUTLET  |        |        |
| CB209                              | CB209                            | 520_(P-Stm) | OUTLET  |        |        |
| CB210B                             | CB210B                           | 518_(P-Stm) | OUTLET  |        |        |
| CB212                              | CB212                            | 521_(P-Stm) | OUTLET  |        |        |



| Conduit                         | Shape    | Full Depth | Full Area | Hyd. Rad. | Max. Width | No. of Barrels | Full Flow |
|---------------------------------|----------|------------|-----------|-----------|------------|----------------|-----------|
| C2                              | CIRCULAR | 0.25       | 0.05      | 0.06      | 0.25       | 1              | 60.49     |
| C4                              | CIRCULAR | 0.25       | 0.05      | 0.06      | 0.25       | 1              | 84.17     |
| C5                              | CIRCULAR | 0.25       | 0.05      | 0.06      | 0.25       | 1              | 84.40     |
| CB213                           | CIRCULAR | 0.25       | 0.05      | 0.06      | 0.25       | 1              | 68.83     |
| CB214                           | CIRCULAR | 0.25       | 0.05      | 0.06      | 0.25       | 1              | 56.15     |
| Pipe_-(104)_-(P-Stm)_2 CIRCULAR |          | 0.45       | 0.16      | 0.11      | 0.45       | 1              |           |
| 142.46                          |          |            |           |           |            |                |           |
| Pipe_-(110)_-(P-Stm) CIRCULAR   |          | 0.53       | 0.22      | 0.13      | 0.53       | 1              | 255.47    |
| Pipe_-(111)_-(P-Stm) CIRCULAR   |          | 0.53       | 0.22      | 0.13      | 0.53       | 1              | 303.80    |
| Pipe_-(71)_-(P-Stm) CIRCULAR    |          | 0.53       | 0.22      | 0.13      | 0.53       | 1              | 272.85    |
| Pipe_-(72)_-(P-Stm) CIRCULAR    |          | 0.53       | 0.22      | 0.13      | 0.53       | 1              | 304.28    |


```

# Post-Development 3-hour Chicago 1:2 year Event

July 2024

## Analysis Options

Flow Units ..... LPS  
 Process Models:  
 Rainfall/Runoff ..... YES  
 RDII ..... NO  
 Snowmelt ..... NO  
 Groundwater ..... NO  
 Flow Routing ..... YES  
 Ponding Allowed ..... NO  
 Water Quality ..... NO  
 Infiltration Method ..... HORTON  
 Flow Routing Method ..... HYUNWAE  
 Surcharge Method ..... EXTRAN  
 Starting Date ..... 01/01/2000 00:00:00  
 Ending Date ..... 01/01/2000 06:00:00  
 Antecedent Dry Days ..... 0.0  
 Report Time Step ..... 00:01:00  
 Wet Time Step ..... 00:01:00  
 Dry Time Step ..... 00:01:00  
 Routing Time Step ..... 1.00 sec  
 Variable Time Step ..... YES  
 Maximum Trials ..... 8  
 Number of Threads ..... 1  
 Head Tolerance ..... 0.001500 m

Runoff Quantity Continuity	Volume	Depth
	hectare-m	mm
Total Precipitation	0.026	31.880
Evaporation Loss	0.000	0.000
Infiltration Loss	0.007	8.863
Surface Runoff	0.018	21.891
Final Storage	0.001	1.158
Continuity Error (%)	-0.103	

Flow Routing Continuity	Volume	Volume
	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.018	0.180
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.018	0.178
Flood Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.002
Continuity Error (%)	0.021	

Highest Continuity Errors  
 Node 520\_(P-Stm) (1.34%)

Time-Step Critical Elements  
 None

Highest Flow Instability Indexes  
 All links are stable.

Most Frequent Nonconverging Nodes  
 Convergence obtained at all time steps.

Routing Time Step Summary				
Minimum Time Step	: 0.50 sec	Average Time Step	: 1.00 sec	
Maximum Time Step	: 1.00 sec	% of Time in Steady State	: 0.00	
Average Iterations per Step	: 2.00	% of Steps Not Converging	: 0.00	
Time Step Frequencies	1.000 - 0.899 sec	100.00 %	0.971 - 0.758 sec	0.00 %
	0.758 - 0.660 sec	0.00 %	0.660 - 0.574 sec	0.00 %
	0.574 - 0.500 sec	0.00 %		

Subcatchment Runoff Summary

Total Runoff	Total Runoff	Peak Runoff	Total Runon	Total Evap	Total Infil	Imperv	Perv
Runoff Subcatchment	Runoff Subcatchment	Runoff Coeff	Runon mm	Evap mm	Infil mm	Runoff mm	Runoff mm
			10^6 ltr	LPS			
S1		31.88	0.00	0.00	31.69	0.00	0.20
0.20	0.00	0.20	0.006				
S10		31.88	0.00	0.00	0.12	30.24	0.01
30.25	0.01	6.54	0.949				
S11		31.88	0.00	0.00	3.05	27.41	0.04
27.45	0.03	24.86	0.861				
S12		31.88	0.00	0.00	31.69	0.11	0.07
0.18	0.00	0.06	0.006				
S14_1		2.04	0.346				
11.03	0.00			0.00	20.09	15.99	11.03
S14_2		31.88	0.00	0.00	25.38	9.60	6.06
6.06	0.00	2.56	0.190				
S15		31.88	0.00	0.00	22.71	12.97	8.57
8.57	0.00	6.09	0.269				
S2		31.88	0.00	0.00	11.76	19.10	0.06
19.16	0.01	10.02	0.311				
S3		31.88	0.00	0.00	1.03	29.34	0.02
29.36	0.02	15.12	0.921				
S32		31.88	0.00	0.00	29.66	4.34	1.14
2.01	0.00	1.28	0.063				
S4		31.88	0.00	0.00	0.01	30.33	0.00
30.33	0.01	9.90	0.341				
S45		31.88	0.00	0.00	0.01	30.33	0.00
30.33	0.01	4.74	0.951				
S5		31.88	0.00	0.00	3.27	27.25	0.06
27.25	0.02	16.19	0.855				
S6		31.88	0.00	0.00	31.68	0.00	0.21
0.21	0.00	0.27	0.007				
S7		31.88	0.00	0.00	0.00	30.33	0.00
30.33	0.02	14.63	0.311				
S8		31.88	0.00	0.00	6.30	24.35	0.05
24.35	0.02	16.11	0.764				
S9		31.88	0.00	0.00	7.32	23.32	0.07
23.39	0.01	4.63	0.734				

# Post-Development 3-hour Chicago 1:2 year Event

July 2024

CB207	0.000	0.1	0.0	0.0	0.001	1.9	0	01:10
13.08								
CB208	0.000	0.0	0.0	0.0	0.000	0.9	0	01:10
2.80								
CB209	0.000	0.0	0.0	0.0	0.000	0.2	0	01:10
9.88								
CB210B	0.000	0.0	0.0	0.0	0.000	0.5	0	01:12
2.80								
CB211	0.000	0.0	0.0	0.0	0.000	0.1	0	01:10
2.03								
CB212	0.000	0.0	0.0	0.0	0.000	0.8	0	01:11
3.35								
CB212A	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00
0.00								
CB213	0.000	0.0	0.0	0.0	0.000	0.3	0	01:19
1.47								
CB214	0.000	0.0	0.0	0.0	0.000	0.3	0	01:16
8.94								

## \*\*\*\*\* Outfall Loading Summary \*\*\*\*\*

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
518A (P-Stm)	62.73	13.16	46.70	0.178
System	62.73	13.16	46.70	0.178

## \*\*\*\*\* Link Flow Summary \*\*\*\*\*

Link	Type	Maximum  Flow  LPS	Time of Max Occurrence days hr:min	Maximum  Veloc  m/sec	Max/Full Flow	Max/Full Depth
C2	CONDUIT	3.99	0 01:02	0.10	0.07	1.00
C4	CONDUIT	0.00	0 00:00	0.00	0.00	0.19
C5	CONDUIT	2.03	0 01:10	0.67	0.02	0.24
CB213	CONDUIT	1.47	0 01:19	0.11	0.02	0.56
CB214	CONDUIT	8.94	0 01:11	0.33	0.16	0.97
Pipe_(104)_-_(P-Stm)_-2	CONDUIT	46.70	0 01:18	0.94	0.33	0.35
Pipe_(110)_-_(P-Stm)_-2	CONDUIT	57.49	0 01:07	0.72	0.08	1.00
Pipe_(111)_-_(P-Stm)	CONDUIT	31.95	0 00:11	0.50	0.10	1.00
Pipe_(71)_-_(P-Stm)	CONDUIT	54.18	0 01:10	0.99	0.20	0.87
Pipe_(72)_-_(P-Stm)	CONDUIT	50.98	0 01:06	0.97	0.17	1.00
OR1	ORIFICE	46.73	0 01:19			1.00
W1	WEIR	0.00	0 00:00			0.00
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	0.00	0 00:00			0.00
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	0.00	0 00:00			0.00
W6	WEIR	0.00	0 00:00			0.00
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	16.08	0 01:10			
CB201	DUMMY	13.84	0 01:10			
CB202	DUMMY	23.39	0 01:08			
CB203	DUMMY	2.44	0 01:08			
CB204	DUMMY	4.37	0 01:10			
CB205	DUMMY	3.33	0 01:08			
CB206	DUMMY	6.50	0 01:10			
CB207	DUMMY	11.33	0 01:10			
CB209	DUMMY	9.88	0 01:10			
CB210B	DUMMY	2.89	0 01:11			
CB212	DUMMY	3.55	0 01:11			

## \*\*\*\*\* Flow Classification Summary \*\*\*\*\*

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
	Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Norm Crit	Inlet Ltd	Inlet Ctrl
C2	1.00	0.12	0.01	0.00	0.13	0.01	0.00	0.72	0.04
C4	1.00	0.03	0.01	0.00	0.02	0.00	0.00	0.01	0.00
C5	1.00	0.17	0.00	0.00	0.02	0.00	0.00	0.80	0.03
CB213	1.00	0.09	0.00	0.00	0.09	0.00	0.00	0.82	0.08
CB214	1.00	0.09	0.00	0.00	0.09	0.00	0.00	0.82	0.03
Pipe_(104)_-_(P-Stm)_-2	1.00	0.11	0.01	0.00	0.00	0.00	0.00	0.89	0.00
Pipe_(110)_-_(P-Stm)	1.00	0.11	0.00	0.00	0.15	0.00	0.00	0.75	0.00
Pipe_(111)_-_(P-Stm)	1.00	0.10	0.00	0.00	0.89	0.00	0.00	0.00	0.77
Pipe_(71)_-_(P-Stm)	1.00	0.10	0.00	0.00	0.11	0.00	0.00	0.79	0.04
Pipe_(72)_-_(P-Stm)	1.00	0.11	0.00	0.00	0.13	0.00	0.00	0.75	0.01

## \*\*\*\*\* Conduit Surcharge Summary \*\*\*\*\*

Conduit	Hours Both Ends	Hours Upstream	Hours Dnstream	Above Full Normal Flow	Hours Capacity Limited
C2	0.35	0.35	0.38	0.01	0.01
CB213	0.04	0.01	0.32	0.01	0.01
CB214	0.01	0.01	0.02	0.01	0.01
Pipe_(110)_-_(P-Stm)	0.53	0.53	0.62	0.01	0.01
Pipe_(111)_-_(P-Stm)	0.32	0.32	0.53	0.01	0.01
Pipe_(71)_-_(P-Stm)	0.01	0.01	0.23	0.01	0.01
Pipe_(72)_-_(P-Stm)	0.30	0.30	0.46	0.01	0.01

Analysis begun on: Thu Aug 29 09:25:38 2024  
 Analysis ended on: Thu Aug 29 09:25:38 2024  
 Total elapsed time: < 1 sec

## Post-Development 3-hour Chicago 1:100 year Event

[TITLE]  
;;Project Title/Notes

[OPTIONS]  
;;Option Value  
FLOW\_UNITS LPS  
INFILTRATION HORTON  
FLOW\_ROUTING DYNWAVE  
LINK\_OFSETS ELEVATION  
MIN\_SLOPE 0  
ALLOW\_PONDING NO  
SKIP\_STEADY\_STATE NO

START\_DATE 01/01/2000  
START\_TIME 00:00:00  
REPORT\_START\_DATE 01/01/2000  
REPORT\_START\_TIME 00:00:00  
END\_DATE 01/01/2000  
END\_TIME 06:00:00  
SWEEP\_START 00:01  
SWEEP\_END 12/31  
DRY\_DAYS 0  
REPORT\_STEP 00:01:00  
WET\_STEP 00:01:00  
DRY\_STEP 00:01:00  
ROUTING\_STEP 00:00:00  
RULE\_STEP 00:00:00

INERTIAL\_DAMPING PARTIAL  
NORMAL\_FLOW\_LIMITED BOTH  
FORCE\_MAIN\_EQUATION H-W  
VARIABLE\_STEP 0.75  
LENGEHEMING\_STEP 0  
MIN\_SURFACE 0  
MAX\_TRIALS 8  
HEAD\_TOLERANCE 0.0015  
SYS\_FLOW\_TOL 5  
LAT\_FLOW\_TOL 5  
MINIMUM\_STEP 0.5  
THREADS 12

[EVAPORATION]  
;;Data Source Parameters  
;;-----  
CONSTANT 0.0  
DRY\_ONLY NO

[RAINGAGES]  
;;Name Format Interval SCF Source  
;;-----  
3CHI100 INTENSITY 0:10 1.0 TIMESERIES 3CHI100

[SUBCATCHMENTS]  
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen  
;;-----  
S1 3CHI100 CB208 0.0187 0 29.61 2.2 0  
S10 3CHI100 CB204 0.0307 99.617 55.32 2.2 0  
S11 3CHI100 CB202 0.127 90.325 65.66 2.2 0  
S12 3CHI100 CB213 0.0149 0.369 6.44 2.2 0  
S14\_1 3CHI100 CB211 0.0165 52.656 22.56 2.2 0  
S14\_2 3CHI100 CB212 0.0179 31.654 58.12 2.2 0  
S15 3CHI100 CB210B 0.0372 42.751 108.74 2.2 0  
S2 3CHI100 CB209 0.0729 62.937 33.61 2.2 0  
S3 3CHI100 CB207 0.0724 96.692 45.79 2.2 0  
S32 3CHI100 CB214 0.052 14.286 29.7 2.2 0  
S4 3CHI100 CB206 0.0464 99.975 24.67 2.2 0  
S45 3CHI100 CB203 0.0222 99.962 14.32 2.2 0  
S5 3CHI100 CB200 0.0833 99.589 82.96 2.2 0  
S6 3CHI100 CB214 0.0239 0 40.1 2.2 0  
S7 3CHI100 520\_(P-Stm) 0.0686 100 25.95 2.2 0  
S8 3CHI100 CB201 0.0941 80.117 35.06 2.2 0  
S9 3CHI100 CB205 0.0275 76.83 19.08 2.2 0

[SUBAREAS]  
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted  
;;-----  
S1 0.013 0.25 1.57 4.67 0 OUTLET  
S10 0.013 0.25 1.57 4.67 0 OUTLET  
S11 0.013 0.25 1.57 4.67 0 OUTLET  
S12 0.013 0.25 1.57 4.67 0 OUTLET  
S14\_1 0.013 0.25 1.57 4.67 0 PREVIOUS 100  
S14\_2 0.013 0.25 1.57 4.67 0 PREVIOUS 100  
S15 0.013 0.25 1.57 4.67 0 PREVIOUS 100  
S2 0.013 0.25 1.57 4.67 0 OUTLET  
S3 0.013 0.25 1.57 4.67 0 OUTLET  
S32 0.013 0.25 1.57 4.67 0 PREVIOUS 80  
S4 0.013 0.25 1.57 4.67 0 OUTLET  
S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100  
S5 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100  
S6 0.013 0.25 1.57 4.67 0 OUTLET  
S7 0.013 0.25 1.57 4.67 0 OUTLET  
S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100  
S9 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]  
;;Subcatchment Param1 Param2 Param3 Param4 Param5  
;;-----  
S1 76.2 13.2 4.14 7 0  
S10 76.2 13.2 4.14 7 0  
S11 76.2 13.2 4.14 7 0  
S12 76.2 13.2 4.14 7 0  
S14\_1 76.2 13.2 4.14 7 0  
S14\_2 76.2 13.2 4.14 7 0  
S15 76.2 13.2 4.14 7 0  
S2 76.2 13.2 4.14 7 0  
S3 76.2 13.2 4.14 7 0  
S32 76.2 13.2 4.14 7 0  
S4 76.2 13.2 4.14 7 0  
S45 76.2 13.2 4.14 7 0  
S5 76.2 13.2 4.14 7 0  
S6 76.2 13.2 4.14 7 0  
S7 76.2 13.2 4.14 7 0  
S8 76.2 13.2 4.14 7 0  
S9 76.2 13.2 4.14 7 0

[JUNCTIONS]  
;;Name Elevation MaxDepth InitDepth SurDepth Aponded  
;;-----  
518\_Orifice 81.361 4.106 0 0 0

[OUTFALLS]  
;;Name Elevation Type Stage Data Gated Route To  
;;-----  
;Cylindrical Structure Slab Top Circular Frame SI  
518\_(P-Stm) 80.646 FIXED 81.28 NO

[STORAGE]  
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params  
;;-----  
;Cylindrical Structure Slab Top Circular Frame SI  
518\_(P-Stm) 81.061 4.406 0.219 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI  
519\_(P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI

520\_(P-Stm) 82.487 2.888 0 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI  
521\_(P-Stm) 81.957 3.673 0 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI  
522\_(P-Stm) 82.182 3.323 0 FUNCTIONAL 0 0 1.13 0  
0 ;85.1800000001676  
CB200 83.18 2.32 0 TABULAR CB200  
0 ;85.5000000001435  
CB201 83.09 2.66 0 TABULAR CB201  
0 ;85.36  
CB202 82.76 2.89 0 TABULAR CB202  
0 ;85.350000000931  
CB203 82.75 2.9 0 TABULAR CB203  
0 ;85.350000000822731  
CB204 82.75 2.75 0 TABULAR CB204  
0 ;85.450000000823662  
CB205 82.85 2.75 0 TABULAR CB205  
0 ;85.48  
CB206 82.77 2.73 0 TABULAR CB206  
0 ;85.05  
CB207 83.15 2.3 0 TABULAR CB207  
0 ;84.90000008021652  
CB208 83.3 2.04 0 TABULAR CB208  
0 ;84.8  
CB209 83.2 1.9 0 TABULAR CB209  
0 ;85.25  
CB210B 82.65 3 0 TABULAR CB210B  
0 ;85.35  
CB211 83.7 1.9 0 TABULAR CB211  
0 ;85.4  
CB212 83.2 2.42 0 TABULAR CB212  
0 ;85.5000000000554  
CB212A 83.62 2 0 TABULAR CB212A  
0 ;84.75  
CB213 83.15 1.75 0 TABULAR CB213  
0 ;84.55  
CB214 82.95 1.9 0 TABULAR CB214  
0

[CONDUITS]  
;;Name From Node To Node Length Roughness InOffset OutOffset  
;;-----  
C2 0 CB208 CB207 9.666 0.013 83.3 83.2  
0 ;C4 0 CB212A CB212 18.475 0.013 83.62 83.25  
0 ;C5 0 CB211 CB212 22.346 0.013 83.7 83.25  
0 ;CB213 522\_(P-Stm) 29.34 0.013 83.15 82.757  
0 ;CB214 522\_(P-Stm) 21.65 0.013 82.95 82.757  
0 ;PVC Pipes  
Pipe\_(110)\_P-Stm 518\_(P-Stm) 518\_(P-Stm) 15.623 0.013 81.361 81.322  
0 ;PVC Pipes  
Pipe\_(110)\_P-Stm 521\_(P-Stm) 518\_(P-Stm) 27.209 0.013 82.257 82.161  
0 ;PVC Pipes  
Pipe\_(111)\_P-Stm 522\_(P-Stm) 521\_(P-Stm) 45.094 0.013 82.482 82.257  
0 ;Concrete Pipes 100-D  
Pipe\_(71)\_P-Stm 520\_(P-Stm) 519\_(P-Stm) 57.146 0.013 82.787 82.557  
0 ;Concrete Pipes 100-D  
Pipe\_(72)\_P-Stm 519\_(P-Stm) 518\_(P-Stm) 31.967 0.013 82.497 82.337  
0 ;ORIFICES  
;;Name From Node To Node Type Offset Qcoeff Gated  
;;-----  
OR1 0 518\_(P-Stm) 518\_Orifice SIDE 81.361 0.65 NO  
0 ;WEIRS  
;;Name EndCon EndCoeff From Node Surcharge To Node RoadWidth Type Coeff. Curve CrestHt Qcoeff Gated  
;;-----  
W1 0 0 CB208 CB209 TRANSVERSE 85.31 1.84 NO  
0 ;W2 0 0 CB212 CB211 TRANSVERSE 85.6 1.84 NO  
0 ;W3 0 0 CB210B CB202 TRANSVERSE 85.45 1.84 NO  
0 ;W4 0 0 CB202 CB204 TRANSVERSE 85.55 1.84 NO  
0 ;W5 0 0 CB206 CB207 TRANSVERSE 85.45 1.84 NO  
0 ;W6 0 0 CB201 CB200 TRANSVERSE 85.65 1.84 NO  
0 ;W7 0 0 CB200 CB209 TRANSVERSE 85.42 1.84 NO  
0 ;OUTLETS  
;;Name From Node To Node Offset Type QTable/Qcoeff  
;;-----  
CB200 CB200 520\_(P-Stm) 83.18 TABULAR/HEAD IPEX\_Type\_A  
NO ;Critical  
CB201 CB201 520\_(P-Stm) 83.09 TABULAR/HEAD Vortex\_ICD\_100  
NO ;Critical  
CB202 CB202 521\_(P-Stm) 82.76 TABULAR/HEAD IPEX\_Type\_A  
NO CB203 CB203 521\_(P-Stm) 82.75 TABULAR/HEAD Vortex\_ICD\_70  
NO CB204 CB204 522\_(P-Stm) 82.75 TABULAR/HEAD Vortex\_ICD\_65  
NO CB205 CB205 522\_(P-Stm) 82.85 TABULAR/HEAD Vortex\_ICD\_65  
NO CB206 CB206 522\_(P-Stm) 82.77 TABULAR/HEAD Vortex\_ICD\_70  
NO CB207 CB207 522\_(P-Stm) 83.15 TABULAR/HEAD Vortex\_ICD\_100  
NO CB209 CB209 520\_(P-Stm) 83.2 TABULAR/HEAD IPEX\_Type\_A

## Post-Development 3-hour Chicago 1:100-year Event

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CB210B   CB210B      518_(P-Stm)    82.65     TABULAR/HEAD    Vortex_ICD_65      Vortex_ICD_100  0.4      5.6
NO          NO           521_(P-Stm)    83.2      TABULAR/HEAD    Vortex_ICD_100  0.5      6.3
CB212   CB212      521_(P-Stm)    83.2      TABULAR/HEAD    Vortex_ICD_100  0.6      6.9
NO          NO           521_(P-Stm)    83.2      TABULAR/HEAD    Vortex_ICD_100  0.7      7.5
Vortex_ICD_100  0.8      8
Vortex_ICD_100  0.9      8.5
Vortex_ICD_100  1        8.9
Vortex_ICD_100  1.2      9.8
Vortex_ICD_100  1.4      10.6
Vortex_ICD_100  1.6      11.3
Vortex_ICD_100  1.8      12
Vortex_ICD_100  2        12.6
Vortex_ICD_100  2.5      14.1
Vortex_ICD_100  3        15.5
;[XSECTIONS]
;;Link       Shape     Geom1      Geom2      Geom3      Geom4      Barrels
;;-----
C2      CIRCULAR  0.25      0        0        0        1
C4      CIRCULAR  0.25      0        0        0        1
C5      CIRCULAR  0.25      0        0        0        1
CB213   CIRCULAR  0.25      0        0        0        1
CB214   CIRCULAR  0.25      0        0        0        1
Pipe_-(104)_-(P-Stm)_2_CIRCULAR 0.45      0        0        0        0        1
Pipe_-(110)_-(P-Stm)_CIRCULAR 0.525     0        0        0        0        1
Pipe_-(111)_-(P-Stm)_CIRCULAR 0.525     0        0        0        0        1
Pipe_-(71)_-(P-Stm)_CIRCULAR 0.525     0        0        0        0        1
Pipe_-(72)_-(P-Stm)_CIRCULAR 0.525     0        0        0        0        1
OR1     CIRCULAR  0.127     0        0        0        0
W1      RECT_OPEN 0.13      0        0        0        1.5
W2      RECT_OPEN 0.05      0        0        0        2.59
W3      RECT_OPEN 0.2       0        0        0        1.34
W4      RECT_OPEN 0.1       0        0        0        4.9
W5      RECT_OPEN 0.05      0        0        0        4.725
W6      RECT_OPEN 0.1       0        0        0        6
W7      RECT_OPEN 0.08      0        0        0        10.5
;[LOSSES]
;;Link       Kentry     Kexit      Kavg      Flap      Gate      Seepage
;;-----
;[CURVES]
;;Name       Type     X-Value      Y-Value
;;-----
CB210-OUT85.5 Rating      0         0
CB210-OUT85.5 Rating      85.5     0.04
CB210-OUT85.5 Rating      100      0.04
CB210-OUT-85.6 Rating      0         0
CB210-OUT-85.6 Rating      85.6     0.08
CB210-OUT-85.6 Rating      100      0.08
;IPEX Type A ICD Rating Curve
IPEX_Type_A Rating      0         0
IPEX_Type_A Rating      0.1      5.7
IPEX_Type_A Rating      0.2      8.1
IPEX_Type_A Rating      0.3      9.9
IPEX_Type_A Rating      0.4      11.4
IPEX_Type_A Rating      0.5      12.8
IPEX_Type_A Rating      0.6      14
IPEX_Type_A Rating      0.7      15.1
IPEX_Type_A Rating      0.8      16.2
IPEX_Type_A Rating      0.9      17.2
IPEX_Type_A Rating      1        18.1
IPEX_Type_A Rating      1.2      19.8
IPEX_Type_A Rating      1.4      21.4
IPEX_Type_A Rating      1.6      22.9
IPEX_Type_A Rating      1.8      24.3
IPEX_Type_A Rating      2        25.6
IPEX_Type_A Rating      2.5      28.6
IPEX_Type_A Rating      3        31.3
;IPEX Type B ICD Rating Curve
IPEX_Type_B Rating      0         0
IPEX_Type_B Rating      0.1      8.1
IPEX_Type_B Rating      0.2      11.5
IPEX_Type_B Rating      0.3      14.1
IPEX_Type_B Rating      0.4      16.2
IPEX_Type_B Rating      0.5      18.2
IPEX_Type_B Rating      0.6      19.9
IPEX_Type_B Rating      0.7      21.5
IPEX_Type_B Rating      0.8      23
IPEX_Type_B Rating      0.9      24.4
IPEX_Type_B Rating      1        25.7
IPEX_Type_B Rating      1.2      28.1
IPEX_Type_B Rating      1.4      30.4
IPEX_Type_B Rating      1.6      32.5
IPEX_Type_B Rating      1.8      34.4
IPEX_Type_B Rating      2        36.3
IPEX_Type_B Rating      2.5      40.6
IPEX_Type_B Rating      3        44.5
;IPEX Type C ICD Rating Curve
IPEX_Type_C Rating      0         0
IPEX_Type_C Rating      0.1      10.6
IPEX_Type_C Rating      0.2      15
IPEX_Type_C Rating      0.3      18.3
IPEX_Type_C Rating      0.4      21.2
IPEX_Type_C Rating      0.5      23.7
IPEX_Type_C Rating      0.6      25.9
IPEX_Type_C Rating      0.7      28
IPEX_Type_C Rating      0.8      29.9
IPEX_Type_C Rating      0.9      31.7
IPEX_Type_C Rating      1        33.5
IPEX_Type_C Rating      1.2      36.6
IPEX_Type_C Rating      1.4      39.6
IPEX_Type_C Rating      1.6      42.3
IPEX_Type_C Rating      1.8      44.9
IPEX_Type_C Rating      2        47.3
IPEX_Type_C Rating      2.5      52.9
IPEX_Type_C Rating      3        57.9
;IPEX Type AA ICD Rating Curve
IPEX_Type_D Rating      0         0
IPEX_Type_D Rating      0.1      15.4
IPEX_Type_D Rating      0.2      21.7
IPEX_Type_D Rating      0.3      26.6
IPEX_Type_D Rating      0.4      30.7
IPEX_Type_D Rating      0.5      34.3
IPEX_Type_D Rating      0.6      37.6
IPEX_Type_D Rating      0.7      40.6
IPEX_Type_D Rating      0.8      43.4
IPEX_Type_D Rating      0.9      46.1
IPEX_Type_D Rating      1        48.5
IPEX_Type_D Rating      1.2      53.2
IPEX_Type_D Rating      1.4      57.4
IPEX_Type_D Rating      1.6      61.4
IPEX_Type_D Rating      1.8      65.1
IPEX_Type_D Rating      2        68.7
IPEX_Type_D Rating      2.5      76.8
IPEX_Type_D Rating      3        84.1
;IPEX Type E ICD Rating Curve
IPEX_Type_E Rating      0         0
IPEX_Type_E Rating      0.1      20.5
IPEX_Type_E Rating      0.2      23.9
IPEX_Type_E Rating      0.3      35.5
IPEX_Type_E Rating      0.4      40.9
IPEX_Type_E Rating      0.5      45.8
IPEX_Type_E Rating      0.6      50.1
IPEX_Type_E Rating      0.7      54.2
IPEX_Type_E Rating      0.8      57.9
IPEX_Type_E Rating      0.9      61.4
IPEX_Type_E Rating      1        64.7
IPEX_Type_E Rating      1.2      70.9
IPEX_Type_E Rating      1.4      76.6
IPEX_Type_E Rating      1.6      81.9
IPEX_Type_E Rating      1.8      86.8
IPEX_Type_E Rating      2        91.5
IPEX_Type_E Rating      2.5      102.3
IPEX_Type_E Rating      3        112.1
;Tempest Rating Curve for Vortex ICD 100, No grate allowance
Vortex_ICD_100 Rating      0         0
Vortex_ICD_100 Rating      0.1      2.8
Vortex_ICD_100 Rating      0.2      4
Vortex_ICD_100 Rating      0.3      4.9
Vortex_ICD_100 Rating      0.4      5.6
Vortex_ICD_100 Rating      0.5      6.3
Vortex_ICD_100 Rating      0.6      6.9
Vortex_ICD_100 Rating      0.7      7.5
Vortex_ICD_100 Rating      0.8      8
Vortex_ICD_100 Rating      0.9      8.5
Vortex_ICD_100 Rating      1        8.9
Vortex_ICD_100 Rating      1.2      9.8
Vortex_ICD_100 Rating      1.4      10.6
Vortex_ICD_100 Rating      1.6      11.3
Vortex_ICD_100 Rating      1.8      12
Vortex_ICD_100 Rating      2        12.6
Vortex_ICD_100 Rating      2.5      14.1
Vortex_ICD_100 Rating      3        15.5
;Tempest Rating Curve for Vortex ICD 105, No grate allowance
Vortex_ICD_105 Rating      0         0
Vortex_ICD_105 Rating      0.1      3.1
Vortex_ICD_105 Rating      0.2      4.4
Vortex_ICD_105 Rating      0.3      5.4
Vortex_ICD_105 Rating      0.4      6.2
Vortex_ICD_105 Rating      0.5      6.9
Vortex_ICD_105 Rating      0.6      7.6
Vortex_ICD_105 Rating      0.7      8.2
Vortex_ICD_105 Rating      0.8      8.8
Vortex_ICD_105 Rating      0.9      9.3
Vortex_ICD_105 Rating      1        9.8
Vortex_ICD_105 Rating      1.2      10.7
Vortex_ICD_105 Rating      1.4      11.6
Vortex_ICD_105 Rating      1.6      12.4
Vortex_ICD_105 Rating      1.8      13.1
Vortex_ICD_105 Rating      2        13.9
Vortex_ICD_105 Rating      2.5      15.5
Vortex_ICD_105 Rating      3        17
;Tempest Rating Curve for Vortex ICD 40, No grate allowance
Vortex_ICD_40 Rating      0         0
Vortex_ICD_40 Rating      0.1      0.4
Vortex_ICD_40 Rating      0.2      0.6
Vortex_ICD_40 Rating      0.3      0.7
Vortex_ICD_40 Rating      0.4      0.9
Vortex_ICD_40 Rating      0.5      1
Vortex_ICD_40 Rating      0.6      1
Vortex_ICD_40 Rating      0.7      1.1
Vortex_ICD_40 Rating      0.8      1.2
Vortex_ICD_40 Rating      0.9      1.3
Vortex_ICD_40 Rating      1        1.4
Vortex_ICD_40 Rating      1.2      1.5
Vortex_ICD_40 Rating      1.4      1.6
Vortex_ICD_40 Rating      1.6      1.7
Vortex_ICD_40 Rating      1.8      1.8
Vortex_ICD_40 Rating      2        1.9
Vortex_ICD_40 Rating      2.5      2.2
Vortex_ICD_40 Rating      3        2.4
;Tempest Rating Curve for Vortex ICD 45, No grate allowance
Vortex_ICD_45 Rating      0         0
Vortex_ICD_45 Rating      0.1      0.6
Vortex_ICD_45 Rating      0.2      0.8
Vortex_ICD_45 Rating      0.3      1
Vortex_ICD_45 Rating      0.4      1.1
Vortex_ICD_45 Rating      0.5      1.3
Vortex_ICD_45 Rating      0.6      1.4
Vortex_ICD_45 Rating      0.7      1.5
Vortex_ICD_45 Rating      0.8      1.6
Vortex_ICD_45 Rating      0.9      1.7
Vortex_ICD_45 Rating      1        1.8
Vortex_ICD_45 Rating      1.2      2
Vortex_ICD_45 Rating      1.4      2.1
Vortex_ICD_45 Rating      1.6      2.3
Vortex_ICD_45 Rating      1.8      2.4
Vortex_ICD_45 Rating      2        2.6
Vortex_ICD_45 Rating      2.5      2.9
Vortex_ICD_45 Rating      3        3.1
;Tempest Rating Curve for Vortex ICD 50, No grate allowance
Vortex_ICD_50 Rating      0         0
Vortex_ICD_50 Rating      0.1      0.7
Vortex_ICD_50 Rating      0.2      1
Vortex_ICD_50 Rating      0.3      1.2
Vortex_ICD_50 Rating      0.4      1.4
Vortex_ICD_50 Rating      0.5      1.6
Vortex_ICD_50 Rating      0.6      1.8
Vortex_ICD_50 Rating      0.7      1.9
Vortex_ICD_50 Rating      0.8      2
Vortex_ICD_50 Rating      0.9      2.1
Vortex_ICD_50 Rating      1        2.3
Vortex_ICD_50 Rating      1.2      2.5
Vortex_ICD_50 Rating      1.4      2.7
Vortex_ICD_50 Rating      1.6      2.9
Vortex_ICD_50 Rating      1.8      3
Vortex_ICD_50 Rating      2        3.2
Vortex_ICD_50 Rating      2.5      3.6
Vortex_ICD_50 Rating      3        3.9
;Tempest Rating Curve for Vortex ICD 55, No grate allowance
Vortex_ICD_55 Rating      0         0
Vortex_ICD_55 Rating      0.1      0.9
Vortex_ICD_55 Rating      0.2      1
Vortex_ICD_55 Rating      0.3      1.2
Vortex_ICD_55 Rating      0.4      1.4
Vortex_ICD_55 Rating      0.5      1.6
Vortex_ICD_55 Rating      0.6      1.8
Vortex_ICD_55 Rating      0.7      1.9
Vortex_ICD_55 Rating      0.8      2
Vortex_ICD_55 Rating      0.9      2.1
Vortex_ICD_55 Rating      1        2.3
Vortex_ICD_55 Rating      1.2      2.5
Vortex_ICD_55 Rating      1.4      2.7
Vortex_ICD_55 Rating      1.6      2.9
Vortex_ICD_55 Rating      1.8      3
Vortex_ICD_55 Rating      2        3.2
Vortex_ICD_55 Rating      2.5      3.6
Vortex_ICD_55 Rating      3        3.9
;Tempest Rating Curve for Vortex ICD 60, No grate allowance
Vortex_ICD_60 Rating      0         0
Vortex_ICD_60 Rating      0.1      1.1
Vortex_ICD_60 Rating      0.2      1.5
Vortex_ICD_60 Rating      0.3      1.8
Vortex_ICD_60 Rating      0.4      2.1
Vortex_ICD_60 Rating      0.5      2.3
Vortex_ICD_60 Rating      0.6      2.5
Vortex_ICD_60 Rating      0.7      2.7
Vortex_ICD_60 Rating      0.8      2.9
Vortex_ICD_60 Rating      0.9      3.1
Vortex_ICD_60 Rating      1        3.2
Vortex_ICD_60 Rating      1.2      3.5
Vortex_ICD_60 Rating      1.4      3.8
Vortex_ICD_60 Rating      1.6      4.1
Vortex_ICD_60 Rating      1.8      4.3
Vortex_ICD_60 Rating      2        4.6
Vortex_ICD_60 Rating      2.5      5.1
Vortex_ICD_60 Rating      3        5.6
;Tempest Rating Curve for Vortex ICD 65, No grate allowance
Vortex_ICD_65 Rating      0         0
Vortex_ICD_65 Rating      0.1      1.2
Vortex_ICD_65 Rating      0.2      1.6
Vortex_ICD_65 Rating      0.3      2
Vortex_ICD_65 Rating      0.4      2.3
Vortex_ICD_65 Rating      0.5      2.5
Vortex_ICD_65 Rating      0.6      2.8
Vortex_ICD_65 Rating      0.7      3
Vortex_ICD_65 Rating      0.8      3.2
Vortex_ICD_65 Rating      0.9      3.4
Vortex_ICD_65 Rating      1        3.6
Vortex_ICD_65 Rating      1.2      3.9
Vortex_ICD_65 Rating      1.4      4.3
Vortex_ICD_65 Rating      1.6      4.6
Vortex_ICD_65 Rating      1.8      4.9
Vortex_ICD_65 Rating      2        5.1
Vortex_ICD_65 Rating      2.5      5.6
;Tempest Rating Curve for Vortex ICD 100, No grate allowance
Vortex_ICD_100 Rating      0         0
Vortex_ICD_100 Rating      0.1      2.8
Vortex_ICD_100 Rating      0.2      4
Vortex_ICD_100 Rating      0.3      4.9
Vortex_ICD_100 Rating      0.4      5.6
Vortex_ICD_100 Rating      0.5      6.3
Vortex_ICD_100 Rating      0.6      6.9
Vortex_ICD_100 Rating      0.7      7.5
Vortex_ICD_100 Rating      0.8      8
Vortex_ICD_100 Rating      0.9      8.5
Vortex_ICD_100 Rating      1        8.9
Vortex_ICD_100 Rating      1.2      9.8
Vortex_ICD_100 Rating      1.4      10.6
Vortex_ICD_100 Rating      1.6      11.3
Vortex_ICD_100 Rating      1.8      12
Vortex_ICD_100 Rating      2        12.6
Vortex_ICD_100 Rating      2.5      14.1
Vortex_ICD_100 Rating      3        15.5

```

# Post-Development 3-hour Chicago 1:100-year Event

July 2024

```

Vortex_ICD_65 1.6 4.6 CB205 Storage 0 0.36
Vortex_ICD_65 1.8 4.9 CB205 2.6 0.36
Vortex_ICD_65 2 5.1 CB205 2.75 86.52
Vortex_ICD_65 2.5 5.7 CB206 Storage 0 0.36
Vortex_ICD_65 3 6.3 CB206 2.6 0.36
;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex_ICD_70 Rating 0 CB206 2.68 61.65
Vortex_ICD_70 0.1 1.3 CB206 2.73 61.65
Vortex_ICD_70 0.2 1.9 CB207 Storage 0 0.36
Vortex_ICD_70 0.3 2.3 CB207 1.97 0.36
Vortex_ICD_70 0.4 2.7 CB207 2.3 182.21
Vortex_ICD_70 0.5 3 CB208 Storage 0 0.073
Vortex_ICD_70 0.6 3.3 CB208 1.74 0.073
Vortex_ICD_70 0.7 3.6 CB208 2.01 67.88
Vortex_ICD_70 0.8 3.8 CB208 2.04 67.88
Vortex_ICD_70 0.9 4.1 CB209 Storage 0 0.073
Vortex_ICD_70 1 4.3 CB209 1.6 0.073
Vortex_ICD_70 1.2 4.7 CB209 1.9 84.87
Vortex_ICD_70 1.4 5.1 CB210B Storage 0 0.073
Vortex_ICD_70 1.6 5.5 CB210B 2.6 0.073
Vortex_ICD_70 1.8 5.8 CB210B 2.9 60.53
Vortex_ICD_70 2 6.1 CB210B 3 60.53
Vortex_ICD_70 2.5 6.8 CB212A Storage 0 0.073
Vortex_ICD_70 3 7.5 CB212A 2 0.073
;Tempest Rating Curve for Vortex ICD 75, No grate allowance
Vortex_ICD_75 Rating 0 CB211 Storage 0 0.073
Vortex_ICD_75 0.1 1.6 CB211 1.65 0.073
Vortex_ICD_75 0.2 2.2 CB211 1.9 21.99
Vortex_ICD_75 0.3 2.7 CB212 Storage 0 0.36
Vortex_ICD_75 0.4 3.2 CB212 2.2 0.36
Vortex_ICD_75 0.5 3.5 CB212 2.42 55.12
Vortex_ICD_75 0.6 3.9 CB212A Storage 0 0.073
Vortex_ICD_75 0.7 4.2 CB212A 2 0.073
Vortex_ICD_75 0.8 4.5 CB213 Storage 0 0.073
Vortex_ICD_75 0.9 4.8 CB213 1.6 0.073
Vortex_ICD_75 1 5 CB213 1.75 9.05
Vortex_ICD_75 1.2 5.5 CB214 Storage 0 0.073
Vortex_ICD_75 1.4 5.9 CB214 1.6 0.073
Vortex_ICD_75 1.6 6.3 CB214 1.9 34.75
Vortex_ICD_75 1.8 6.7 CB214 2 0.073
Vortex_ICD_75 2 7.1 CB214 2.6 0.073
Vortex_ICD_75 2.5 7.9 CB214 3 0.073
Vortex_ICD_75 3 8.7 CB214 3.5 0.073
;Tempest Rating Curve for Vortex ICD 80, No grate allowance
Vortex_ICD_80 Rating 0 0 [TIMESERIES]
Vortex_ICD_80 0.1 1.8 ;Name Date Time Value
Vortex_ICD_80 0.2 2.6 ;-----
Vortex_ICD_80 0.3 3.1 ;-----
Vortex_ICD_80 0.4 3.6 ;-----
Vortex_ICD_80 0.5 4 CB214 [REPORT]
Vortex_ICD_80 0.6 4.4 ;-----: Reporting Options
Vortex_ICD_80 0.7 4.8 INPUT YES
Vortex_ICD_80 0.8 5.1 CONTROLS NO
Vortex_ICD_80 0.9 5.4 SUBCATCHMENTS ALL
Vortex_ICD_80 1 5.7 NODES ALL
Vortex_ICD_80 1.2 6.3 LINKS ALL
Vortex_ICD_80 1.4 6.8
Vortex_ICD_80 1.6 7.2
Vortex_ICD_80 1.8 7.7
Vortex_ICD_80 2 8.1
Vortex_ICD_80 2.5 9
Vortex_ICD_80 3 9.9
;Tempest Rating Curve for Vortex ICD 85, No grate allowance
Vortex_ICD_85 Rating 0 0 [TAGS]
Vortex_ICD_85 0 0 Node 518_Orifice RY_Manhole
Vortex_ICD_85 0.1 2 Node CB200 RY_Manhole
Vortex_ICD_85 0.2 2.9 Node CB201 RY_Manhole
Vortex_ICD_85 0.3 3.5 Node CB202 RY_Manhole
Vortex_ICD_85 0.4 4.1 Node CB203 RY_Manhole
Vortex_ICD_85 0.5 4.5 Node CB204 RY_Manhole
Vortex_ICD_85 0.6 5 Node CB205 RY_Manhole
Vortex_ICD_85 0.7 5.4 Node CB206 RY_Manhole
Vortex_ICD_85 0.8 5.7 Node CB207 RY_Manhole
Vortex_ICD_85 0.9 6.1 Node CB208 RY_Manhole
Vortex_ICD_85 1 6.4 Node CB209 RY_Manhole
Vortex_ICD_85 1.2 7 Node CB210B RY_Manhole
Vortex_ICD_85 1.4 7.6 Node CB211 RY_Manhole
Vortex_ICD_85 1.6 8.1 Node CB212 RY_Manhole
Vortex_ICD_85 1.8 8.6 Node CB212A RY_Manhole
Vortex_ICD_85 2 9.1 Node CB213 RY_Manhole
Vortex_ICD_85 2.5 10.1 Node CB214 RY_Manhole
Vortex_ICD_85 3 11.1 Node C2 RY_Sewer
Vortex_ICD_90 Rating 0 0 Link C4 RY_Sewer
Vortex_ICD_90 0 0 Link C5 RY_Sewer
;Tempest Rating Curve for Vortex ICD 90, No grate allowance
Vortex_ICD_90 Rating 0 0 [MAP]
Vortex_ICD_90 0.1 2.2 DIMENSIONS 381219.0294 5032865.6782 381334.8286 5033009.9278
Vortex_ICD_90 0.2 3.2 UNITS Meters
Vortex_ICD_90 0.3 3.9
Vortex_ICD_90 0.4 4.5
Vortex_ICD_90 0.5 5.1
Vortex_ICD_90 0.6 5.5
Vortex_ICD_90 0.7 6
Vortex_ICD_90 0.8 6.4
Vortex_ICD_90 0.9 6.8
Vortex_ICD_90 1 7.2
Vortex_ICD_90 1.2 7.9
Vortex_ICD_90 1.4 8.5
Vortex_ICD_90 1.6 9.1
Vortex_ICD_90 1.8 9.6
Vortex_ICD_90 2 10.2
Vortex_ICD_90 2.5 11.4
Vortex_ICD_90 3 12.5
;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex_ICD_95 Rating 0 0 [COORDINATES]
Vortex_ICD_95 0 0 ;Node X-Coord Y-Coord
Vortex_ICD_95 0 0 518_Orifice 381303.7 5032953.188
Vortex_ICD_95 0.1 2.6 518A_(P-Stm) 381317.029 5032955.797
Vortex_ICD_95 0.2 3.6 518_(P-Stm) 381301.695 5032952.807
Vortex_ICD_95 0.3 4.4 519_(P-Stm) 381278.558 5032974.866
Vortex_ICD_95 0.4 5.1 520_(P-Stm) 381240.562 5032832.182
Vortex_ICD_95 0.5 5.7 521_(P-Stm) 381212.324 5032830.56
Vortex_ICD_95 0.6 6.2 522_(P-Stm) 381267.539 5032909.252
Vortex_ICD_95 0.7 6.7 CB200 381246.119 5032946.933
Vortex_ICD_95 0.8 7.1 CB201 381266.142 5032969.504
Vortex_ICD_95 0.9 7.6 CB202 381305.816 5032949.229
Vortex_ICD_95 1 8 CB203 381314.519 5032914.342
Vortex_ICD_95 1.2 8.7 CB204 381291.187 5032921.779
Vortex_ICD_95 1.4 9.4 CB205 381286.939 5032925.282
Vortex_ICD_95 1.6 10.1 CB206 381253.233 5032910.17
Vortex_ICD_95 1.8 10.7 CB207 381253.811 5032910.356
Vortex_ICD_95 2 11.3 CB208 381248.314 5032902.406
Vortex_ICD_95 2.5 12.6 CB209 381228.288 5032926.78
CB210B 381314.246 5032963.095
Vortex_ICD_95 3 13.8 CB211 381319.351 5032951.771
CB200 Storage 0 0.36 CB212 381328.283 5032931.288
CB200 2 0.36 CB212A 381295.87 5032912.5
CB200 2.24 210.73 CB213 381296.875 5032911.718
CB200 2.32 210.73 CB214 381282.831 5032893.727
CB201 Storage 0 0.36 [VERTICES]
CB201 2.41 0.36 ;Link X-Coord Y-Coord
CB201 2.56 169.64 W2 381318.013 5032956.452
CB201 2.66 169.64 W4 381305.808 5032934.267
CB202 Storage 0 0.36 W4 381304.151 5032928.843
CB202 2.6 0.36
CB202 2.79 283.64
CB202 2.89 283.64
CB203 Storage 0 0.36 [POLYGONS]
CB203 2.6 0.36 ;Subcatchment X-Coord Y-Coord
CB203 2.9 178.09 S1 381237.464 5032804.825
CB204 Storage 0 0.36 S1 381240.329 5032805.467
CB204 2.6 0.36 S1 381240.224 5032906.884
CB204 2.75 146.08 S1 381246.264 5032909.575
S1 381246.323 5032909.6
S1 381246.383 5032909.624
S1 381246.444 5032909.645
S1 381246.506 5032909.663

```









```
;;
[SYMBOLS]
;Gage X-Coord Y-Coord
;;
-----
```

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

\*\*\*\*\*
Element Count
\*\*\*\*\*
Number of rain gages ..... 1
Number of subcatchments .... 17
Number of nodes ..... 23
Number of links ..... 29
Number of pollutants ..... 0
Number of land uses ..... 0

\*\*\*\*\*
Raingage Summary
\*\*\*\*\*
Name Data Source Data Type Recording Interval
-----

3CHI100 3CHI100 INTENSITY 10 min.

\*\*\*\*\*
Subcatchment Summary
\*\*\*\*\*
Name Area Width %Imperv %Slope Rain Gage Outlet
-----

S1 0.02 29.61 0.00 2,2000 3CHI100 CB208  
S10 0.03 55.32 99.62 2,2000 3CHI100 CB204  
S11 0.13 65.66 90.33 2,2000 3CHI100 CB202  
S12 0.01 6.44 0.37 2,2000 3CHI100 CB213  
S14\_1 0.01 22.48 52.68 2,2000 3CHI100 CB211  
S14\_2 0.02 68.12 31.65 2,2000 3CHI100 CB212  
S15 0.04 108.74 42.75 2,2000 3CHI100 CB210B  
S2 0.07 33.61 62.94 2,2000 3CHI100 CB209  
S3 0.07 24.67 95.97 2,2000 3CHI100 CB207  
S32 0.05 29.70 14.29 2,2000 3CHI100 CB214  
S4 0.05 11.32 99.56 2,2000 3CHI100 CB206  
S45 0.02 12.32 99.59 2,2000 3CHI100 CB203  
S5 0.08 22.16 89.59 2,2000 3CHI100 CB200  
S6 0.02 40.30 0.00 2,2000 3CHI100 CB214  
S7 0.07 25.95 100.00 2,2000 3CHI100 520\_(P-  
Stm)  
S8 0.09 35.06 80.12 2,2000 3CHI100 CB201  
S9 0.03 19.08 76.83 2,2000 3CHI100 CB205

\*\*\*\*\*
Node Summary
\*\*\*\*\*
Name Type Invert Elev. Max. Depth Ponded Area External Inflow
-----

518\_Orifice JUNCTION 81.36 4.11 0.0  
518A\_(P-Stm) OUTFALL 80.65 1.13 0.0  
518\_(P-Stm) STORAGE 81.06 4.41 0.0  
519\_(P-Stm) STORAGE 82.20 3.51 0.0  
520\_(P-Stm) STORAGE 82.49 2.89 0.0  
521\_(P-Stm) STORAGE 81.96 3.67 0.0  
522\_(P-Stm) STORAGE 82.10 3.32 0.0  
CB200 STORAGE 83.18 2.32 0.0  
CB201 STORAGE 83.09 2.66 0.0  
CB202 STORAGE 82.76 2.89 0.0  
CB203 STORAGE 82.75 2.90 0.0  
CB204 STORAGE 82.75 2.75 0.0  
CB205 STORAGE 82.85 2.75 0.0  
CB206 STORAGE 82.77 2.73 0.0  
CB207 STORAGE 83.15 2.30 0.0  
CB208 STORAGE 83.30 2.04 0.0  
CB209 STORAGE 83.20 1.90 0.0  
CB210B STORAGE 82.65 3.00 0.0  
CB211 STORAGE 83.70 1.90 0.0  
CB212 STORAGE 83.20 2.42 0.0  
CB212A STORAGE 83.62 2.00 0.0  
CB213 STORAGE 83.15 1.75 0.0  
CB214 STORAGE 82.95 1.90 0.0

\*\*\*\*\*
Link Summary
\*\*\*\*\*
Name From Node To Node Type Length %Slope
-----

Roughness

-

C2 CB208 CB207 CONDUIT 9.7 1.0346  
0.0130 0.0130 C4 CB212A CB212 CONDUIT 18.5 2.0031  
0.0130 C5 CB211 CB212 CONDUIT 22.3 2.0142  
0.0130 CB213 CB213 522\_(P-Stm) CONDUIT 29.3 1.3396  
0.0130 CB214 CB214 522\_(P-Stm) CONDUIT 21.6 0.8915  
0.0130 Pipe\_-(104)\_ (P-Stm)\_2 518\_Orifice 518A\_(P-Stm) CONDUIT 15.6 0.2496  
0.0130 Pipe\_-(110)\_ (P-Stm) 521\_(P-Stm) 518\_(P-Stm) CONDUIT 27.2 0.3528  
0.0130 Pipe\_-(111)\_ (P-Stm) 522\_(P-Stm) 521\_(P-Stm) CONDUIT 45.1 0.4990  
0.0130 Pipe\_-(71)\_ (P-Stm) 520\_(P-Stm) 519\_(P-Stm) CONDUIT 57.1 0.4025  
0.0130 Pipe\_-(72)\_ (P-Stm) 519\_(P-Stm) 518\_(P-Stm) CONDUIT 32.0 0.5005  
0.0130 OR1 518\_(P-Stm) 518\_Orifice ORIFICE  
W1 CB208 CB209 WEIR  
W2 CB210B CB211 WEIR  
W3 CB201 CB202 WEIR  
W4 CB202 CB204 WEIR  
W5 CB206 CB207 WEIR  
W6 CB201 CB200 WEIR  
W7 CB200 CB209 WEIR  
CB200 CB200 520\_(P-Stm) OUTLET  
CB201 CB201 520\_(P-Stm) OUTLET  
CB202 521\_(P-Stm) OUTLET  
CB203 521\_(P-Stm) OUTLET  
CB204 CB204 522\_(P-Stm) OUTLET  
CB205 CB205 522\_(P-Stm) OUTLET  
CB206 CB206 522\_(P-Stm) OUTLET  
CB207 CB207 522\_(P-Stm) OUTLET  
CB209 CB209 520\_(P-Stm) OUTLET  
CB210B CB210B 518\_(P-Stm) OUTLET  
CB212 CB212 521\_(P-Stm) OUTLET

\*\*\*\*\*
Cross Section Summary
\*\*\*\*\*
Conduit Shape Full Depth Full Area Hyd. Rad. Max. Width No. of Barrels Full Flow
-----

C2 CIRCULAR 0.25 0.05 0.06 0.25 1 60.49  
C4 CIRCULAR 0.25 0.05 0.06 0.25 1 84.17  
C5 CIRCULAR 0.25 0.05 0.06 0.25 1 84.40  
CB213 CIRCULAR 0.25 0.05 0.06 0.25 1 68.83  
CB214 CIRCULAR 0.25 0.05 0.06 0.25 1 56.15  
14\_Pipe\_-(104)\_ (P-Stm)\_2 CIRCULAR 0.45 0.16 0.11 0.45 1  
14\_Pipe\_-(46)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 255.47  
Pipe\_-(110)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 303.80  
Pipe\_-(111)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 272.85  
Pipe\_-(71)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 304.28





## Post-Development 24-hour SCS 1:100-year Event

[TITLE]  
;;Project Title/Notes

[OPTIONS]  
;;Option Value  
FLOW\_UNITS LPS  
INFILTRATION HORTON  
FLOW\_ROUTING DYNWAVE  
LINK\_OFSETS ELEVATION  
MIN\_SLOPE 0  
ALLOW\_PONDING NO  
SKIP\_STEADY\_STATE NO

START\_DATE 01/01/2000  
START\_TIME 00:00:00  
REPORT\_START\_DATE 01/01/2000  
REPORT\_START\_TIME 00:00:00  
END\_DATE 01/02/2000  
END\_TIME 00:00:00  
SWEEP\_START 01/01  
SWEEP\_END 12/31  
DRY\_DAYS 0  
REPORT\_STEP 00:01:00  
WET\_STEP 00:01:00  
DRY\_STEP 00:01:00  
ROUTING\_STEP RULE\_STEP 00:00:00

INERTIAL\_DAMPING PARTIAL  
NORMAL\_FLOW\_LIMITED BOTH  
FORCE\_MAIN\_EQUATION H-W  
VARIABLE\_STEP 0.75  
LENGHEADING\_STEP 0  
MIN\_SURFACE 0  
MAX\_TRIALS 8  
HEAD\_TOLERANCE 0.0015  
SYS\_FLOW\_TOL 5  
LAT\_FLOW\_TOL 5  
MINIMUM\_STEP 0.5  
THREADS 12

[EVAPORATION]  
;;Data Source Parameters  
;;  
CONSTANT 0.0  
DRY\_ONLY NO

[RAINGAGES]  
;;Name Format Interval SCF Source  
24SCS100 INTENSITY 0:15 1.0 TIMESERIES 24SCS100  
3CH100 INTENSITY 0:10 1.0 TIMESERIES 3CH100

[SUBCATCHMENTS]  
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen  
SnowPack ;;;

S1 24SCS100 CB208 0.0187 0 29.61 2.2 0  
S10 24SCS100 CB204 0.0307 99.617 55.32 2.2 0  
S11 24SCS100 CB202 0.127 90.325 65.66 2.2 0  
S12 24SCS100 CB213 0.0119 0.369 6.44 2.2 0  
S14\_1 24SCS100 CB211 0.0116 52.166 22.78 2.2 0  
S14\_2 24SCS100 CB212 0.0179 31.654 58.12 2.2 0  
S15 24SCS100 CB210B 0.0372 42.751 108.74 2.2 0  
S2 24SCS100 CB209 0.0729 62.937 33.61 2.2 0  
S3 24SCS100 CB207 0.0724 96.692 45.79 2.2 0  
S32 24SCS100 CB214 0.052 14.286 29.1 2.2 0  
S4 24SCS100 CB205 0.0164 99.975 24.67 2.2 0  
S45 24SCS100 CB203 0.022 99.965 14.32 2.2 0  
S5 24SCS100 CB200 0.0843 89.589 82.96 2.2 0  
S6 24SCS100 CB214 0.0239 0 40.3 2.2 0  
S7 24SCS100 520\_(P-Stm) 0.0684 100 25.95 2.2 0  
S8 24SCS100 CB201 0.0941 80.117 35.06 2.2 0  
S9 24SCS100 CB205 0.0275 76.83 19.08 2.2 0

[SUBAREAS]  
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted  
;;  
S1 0.013 0.25 1.57 4.67 0 OUTLET  
S10 0.013 0.25 1.57 4.67 0 OUTLET  
S11 0.013 0.25 1.57 4.67 0 OUTLET  
S12 0.013 0.25 1.57 4.67 0 OUTLET  
S14\_1 0.013 0.25 1.57 4.67 0 PREVIOUS 100  
S14\_2 0.013 0.25 1.57 4.67 0 PREVIOUS 100  
S15 0.013 0.25 1.57 4.67 0 PREVIOUS 100  
S2 0.013 0.25 1.57 4.67 0 OUTLET  
S3 0.013 0.25 1.57 4.67 0 OUTLET  
S32 0.013 0.25 1.57 4.67 0 PREVIOUS 80  
S4 0.013 0.25 1.57 4.67 0 OUTLET  
S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100  
S5 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100  
S6 0.013 0.25 1.57 4.67 0 OUTLET  
S7 0.013 0.25 1.57 4.67 0 OUTLET  
S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100  
S9 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]  
;;Subcatchment Param1 Param2 Param3 Param4 Param5  
;;  
S1 76.2 13.2 4.14 7 0  
S10 76.2 13.2 4.14 7 0  
S11 76.2 13.2 4.14 7 0  
S12 76.2 13.2 4.14 7 0  
S14\_1 76.2 13.2 4.14 7 0  
S14\_2 76.2 13.2 4.14 7 0  
S15 76.2 13.2 4.14 7 0  
S2 76.2 13.2 4.14 7 0  
S3 76.2 13.2 4.14 7 0  
S32 76.2 13.2 4.14 7 0  
S4 76.2 13.2 4.14 7 0  
S45 76.2 13.2 4.14 7 0  
S5 76.2 13.2 4.14 7 0  
S6 76.2 13.2 4.14 7 0  
S7 76.2 13.2 4.14 7 0  
S8 76.2 13.2 4.14 7 0  
S9 76.2 13.2 4.14 7 0

[JUNCTIONS]  
;;Name Elevation MaxDepth InitDepth SurDepth Apended  
;;  
S18\_Orifice 81.361 4.106 0 0 0

[OUTFALLS]  
;;Name Elevation Type Stage Data Gated Route To  
;;  
518A\_(P-Stm) 80.646 FIXED 81.27 NO

[STORAGE]  
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params  
;;  
SurDepth\_Fevap 81.061 4.406 0.209 FUNCTIONAL 0 0 1.13 0

;Cylindrical Structure Slab Top Circular Frame SI  
518\_(P-Stm) 81.061 4.406 0.209 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI  
519\_(P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI

520\_(P-Stm) 82.487 2.888 0 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI  
521\_(P-Stm) 81.957 3.673 0 FUNCTIONAL 0 0 1.13 0  
;Cylindrical Structure Slab Top Circular Frame SI  
522\_(P-Stm) 82.182 3.323 0 FUNCTIONAL 0 0 1.13 0  
;85.1800000001676  
CB200 83.18 2.32 0 TABULAR CB200 0  
;85.5000000001435  
CB201 83.09 2.66 0 TABULAR CB201 0  
;85.36  
CB202 82.76 2.89 0 TABULAR CB202 0  
;85.350000000931  
CB203 82.75 2.9 0 TABULAR CB203 0  
;85.350008022731  
CB204 82.75 2.75 0 TABULAR CB204 0  
;85.450008023662  
CB205 82.85 2.75 0 TABULAR CB205 0  
;85.48  
CB206 82.77 2.73 0 TABULAR CB206 0  
;85.05  
CB207 83.15 2.3 0 TABULAR CB207 0  
;84.900008021652  
CB208 83.3 2.04 0 TABULAR CB208 0  
;84.8  
CB209 83.2 1.9 0 TABULAR CB209 0  
;85.25  
CB210B 82.65 3 0 TABULAR CB210B 0  
;85.35  
CB211 83.7 1.9 0 TABULAR CB211 0  
;85.4  
CB212 83.2 2.42 0 TABULAR CB212 0  
;85.500000000554  
CB212A 83.62 2 0 TABULAR CB212A 0  
;84.75  
CB213 83.15 1.75 0 TABULAR CB213 0  
;84.55  
CB214 82.95 1.9 0 TABULAR CB214 0

[CONDUTS]  
;;Name From Node To Node Length Roughness InOffset OutOffset  
InitFlow MaxFlow  
;;  
C2 CB208 CB207 9.666 0.013 83.3 83.2  
C4 CB212A CB212 18.475 0.013 83.62 83.25  
C5 CB211 CB212 22.346 0.013 83.7 83.25  
CB213 522\_(P-Stm) 29.34 0.013 83.15 82.757  
CB214 522\_(P-Stm) 21.65 0.013 82.95 82.757  
;PVC Pipes  
Pipe\_(104)\_ (P-Stm) 2 518\_Orifice 518A\_(P-Stm) 15.623 0.013 81.361 81.322  
;PVC Pipes  
Pipe\_(110)\_ (P-Stm) 521\_(P-Stm) 518\_(P-Stm) 27.209 0.013 82.257 82.161  
;PVC Pipes  
Pipe\_(111)\_ (P-Stm) 522\_(P-Stm) 521\_(P-Stm) 45.094 0.013 82.482 82.257  
;Concrete Pipes 100-D  
Pipe\_(71)\_ (P-Stm) 520\_(P-Stm) 519\_(P-Stm) 57.146 0.013 82.787 82.557  
;Concrete Pipes 100-D  
Pipe\_(72)\_ (P-Stm) 519\_(P-Stm) 518\_(P-Stm) 31.967 0.013 82.497 82.337  
;ORIFICES  
;;Name From Node To Node Type Offset Qcoeff Gated  
CloseTime  
OR1 518\_(P-Stm) 518\_Orifice SIDE 81.361 0.65 NO

[WEIRS]  
;;Name From Node To Node Type CrestHt Qcoeff Gated  
EndCon EndCoeff Surcharge RoadWidth RoadSurf Coeff. Curve  
;;  
W1 CB208 CB209 TRANSVERSE 85.31 1.84 NO  
W2 CB210B CB211 TRANSVERSE 85.6 1.84 NO  
W3 CB210B CB202 TRANSVERSE 85.45 1.84 NO  
W4 CB202 CB204 TRANSVERSE 85.55 1.84 NO  
W5 CB206 CB207 TRANSVERSE 85.45 1.84 NO  
W6 CB201 CB200 TRANSVERSE 85.65 1.84 NO  
W7 CB202 CB209 TRANSVERSE 85.42 1.84 NO  
;OUTLETS  
;;Name From Node To Node Offset Type QTable/Qcoeff  
Qexpon Gated  
;;  
CB200 CB200 520\_(P-Stm) 83.18 TABULAR/HEAD IPEX\_Type\_A  
;Critical  
CB201 CB201 520\_(P-Stm) 83.09 TABULAR/HEAD Vortex\_ICD\_100  
;Critical  
CB202 CB202 521\_(P-Stm) 82.76 TABULAR/HEAD IPEX\_Type\_A  
CB203 CB203 521\_(P-Stm) 82.75 TABULAR/HEAD Vortex\_ICD\_70  
CB204 CB204 522\_(P-Stm) 82.75 TABULAR/HEAD Vortex\_ICD\_65  
CB205 CB205 522\_(P-Stm) 82.85 TABULAR/HEAD Vortex\_ICD\_65  
CB206 CB206 522\_(P-Stm) 82.77 TABULAR/HEAD Vortex\_ICD\_70  
CB207 CB207 522\_(P-Stm) 83.15 TABULAR/HEAD Vortex\_ICD\_100  
CB209 CB209 520\_(P-Stm) 83.2 TABULAR/HEAD IPEX\_Type\_A  
CB210B CB210B 518\_(P-Stm) 82.65 TABULAR/HEAD Vortex\_ICD\_65  
CB212 CB212 521\_(P-Stm) 83.2 TABULAR/HEAD Vortex\_ICD\_100  
;Link Culvert  
Shape Geom1 Geom2 Geom3 Geom4 Barrels

```

;-----
;----- C2      CIRCULAR  0.25      0       0       0       1
;----- C4      CIRCULAR  0.25      0       0       0       1
;----- C5      CIRCULAR  0.25      0       0       0       1
;----- CB213   CIRCULAR  0.25      0       0       0       1
;----- CB214   CIRCULAR  0.25      0       0       0       1
;----- Pipe_--_(104)_(P-Stm)_CIRCULAR 0.45 0       0       0       1
;----- Pipe_--_(110)_(P-Stm)_CIRCULAR 0.525 0       0       0       1
;----- Pipe_--_(111)_(P-Stm)_CIRCULAR 0.525 0       0       0       1
;----- Pipe_--_(71)_(P-Stm)_CIRCULAR 0.525 0       0       0       1
;----- Pipe_--_(72)_(P-Stm)_CIRCULAR 0.525 0       0       0       1
;----- OR1     CIRCULAR  0.127     0       0       0       1
;----- W1      RECT_OPEN  0.03      1.5      0       0       1
;----- W2      RECT_OPEN  0.05      2.59     0       0       1
;----- W3      RECT_OPEN  0.2       3.34     0       0       1
;----- W4      RECT_OPEN  0.1       4.9      0       0       1
;----- W5      RECT_OPEN  0.05      4.725    0       0       1
;----- W6      RECT_OPEN  0.1       6       0       0       1
;----- W7      RECT_OPEN  0.08      10.5     0       0       1
;----- [LOSSES]
;----- Link   Kentry   Kexit   Kavg   Flap   Gate   Seepage
;----- ;----- [CURVES]
;----- ;Name   Type   X-Value Y-Value
;----- ;----- CB210-OUT85.5 Rating 0       0.04
;----- CB210-OUT85.5 Rating 85.5    0.04
;----- CB210-OUT85.5 Rating 100     0.04
;----- CB210-OUT-85.6 Rating 0       0
;----- CB210-OUT-85.6 Rating 85.6    0.08
;----- CB210-OUT-85.6 Rating 100     0.08
;----- ;IPEX Type A ICD Rating Curve
;----- IPEX_Type_A Rating 0       0
;----- IPEX_Type_A Rating 0.1      5.7
;----- IPEX_Type_A Rating 0.2      8.1
;----- IPEX_Type_A Rating 0.3      9.9
;----- IPEX_Type_A Rating 0.4      11.4
;----- IPEX_Type_A Rating 0.5      12.8
;----- IPEX_Type_A Rating 0.6      14
;----- IPEX_Type_A Rating 0.7      15.1
;----- IPEX_Type_A Rating 0.8      16.2
;----- IPEX_Type_A Rating 0.9      17.2
;----- IPEX_Type_A Rating 1       18.1
;----- IPEX_Type_A Rating 1.2      19.8
;----- IPEX_Type_A Rating 1.4      21.4
;----- IPEX_Type_A Rating 1.6      22.9
;----- IPEX_Type_A Rating 1.8      24.3
;----- IPEX_Type_A Rating 2       25.6
;----- IPEX_Type_A Rating 2.5      28.6
;----- IPEX_Type_A Rating 3       31.3
;----- ;IPEX Type B ICD Rating Curve
;----- IPEX_Type_B Rating 0       0
;----- IPEX_Type_B Rating 0.1      8.1
;----- IPEX_Type_B Rating 0.2      11.5
;----- IPEX_Type_B Rating 0.3      14.1
;----- IPEX_Type_B Rating 0.4      16.2
;----- IPEX_Type_B Rating 0.5      18.2
;----- IPEX_Type_B Rating 0.6      19.9
;----- IPEX_Type_B Rating 0.7      21.5
;----- IPEX_Type_B Rating 0.8      23
;----- IPEX_Type_B Rating 0.9      24.4
;----- IPEX_Type_B Rating 1       25.7
;----- IPEX_Type_B Rating 1.2      28.1
;----- IPEX_Type_B Rating 1.4      30.4
;----- IPEX_Type_B Rating 1.6      32.5
;----- IPEX_Type_B Rating 1.8      34.4
;----- IPEX_Type_B Rating 2       36.3
;----- IPEX_Type_B Rating 2.5      40.6
;----- IPEX_Type_B Rating 3       44.5
;----- ;IPEX Type C ICD Rating Curve
;----- IPEX_Type_C Rating 0       0
;----- IPEX_Type_C Rating 0.1      10.6
;----- IPEX_Type_C Rating 0.2      15
;----- IPEX_Type_C Rating 0.3      18.3
;----- IPEX_Type_C Rating 0.4      21.2
;----- IPEX_Type_C Rating 0.5      23.7
;----- IPEX_Type_C Rating 0.6      25.9
;----- IPEX_Type_C Rating 0.7      28
;----- IPEX_Type_C Rating 0.8      29.9
;----- IPEX_Type_C Rating 0.9      31.7
;----- IPEX_Type_C Rating 1       33.5
;----- IPEX_Type_C Rating 1.2      36.6
;----- IPEX_Type_C Rating 1.4      39.6
;----- IPEX_Type_C Rating 1.6      42.3
;----- IPEX_Type_C Rating 1.8      44.9
;----- IPEX_Type_C Rating 2       47.3
;----- IPEX_Type_C Rating 2.5      52.9
;----- IPEX_Type_C Rating 3       57.9
;----- ;IPEX Type AA ICD Rating Curve
;----- IPEX_Type_D Rating 0       0
;----- IPEX_Type_D Rating 0.1      15.4
;----- IPEX_Type_D Rating 0.2      21.7
;----- IPEX_Type_D Rating 0.3      26.6
;----- IPEX_Type_D Rating 0.4      30.7
;----- IPEX_Type_D Rating 0.5      34.3
;----- IPEX_Type_D Rating 0.6      37.6
;----- IPEX_Type_D Rating 0.7      40.6
;----- IPEX_Type_D Rating 0.8      43.4
;----- IPEX_Type_D Rating 0.9      46.1
;----- IPEX_Type_D Rating 1       48.5
;----- IPEX_Type_D Rating 1.2      53.2
;----- IPEX_Type_D Rating 1.4      57.4
;----- IPEX_Type_D Rating 1.6      61.4
;----- IPEX_Type_D Rating 1.8      65.1
;----- IPEX_Type_D Rating 2       68.7
;----- IPEX_Type_D Rating 2.5      76.8
;----- IPEX_Type_D Rating 3       84.1
;----- ;IPEX Type E ICD Rating Curve
;----- IPEX_Type_E Rating 0       0
;----- IPEX_Type_E Rating 0.1      20.5
;----- IPEX_Type_E Rating 0.2      28.9
;----- IPEX_Type_E Rating 0.3      35.5
;----- IPEX_Type_E Rating 0.4      40.9
;----- IPEX_Type_E Rating 0.5      45.8
;----- IPEX_Type_E Rating 0.6      50.1
;----- IPEX_Type_E Rating 0.7      54.2
;----- IPEX_Type_E Rating 0.8      57.9
;----- IPEX_Type_E Rating 0.9      61.4
;----- IPEX_Type_E Rating 1       64.7
;----- IPEX_Type_E Rating 1.2      70.9
;----- IPEX_Type_E Rating 1.4      76.6
;----- IPEX_Type_E Rating 1.6      81.9
;----- IPEX_Type_E Rating 1.8      86.8
;----- IPEX_Type_E Rating 2       91.5
;----- IPEX_Type_E Rating 2.5      102.3
;----- IPEX_Type_E Rating 3       112.1
;----- ;Tempest Rating Curve for Vortex ICD 100, No grate allowance
;----- Vortex_ICD_100 Rating 0       0
;----- Vortex_ICD_100 Rating 0.1      2.8
;----- Vortex_ICD_100 Rating 0.2      4
;----- Vortex_ICD_100 Rating 0.3      4.9
;----- Vortex_ICD_100 Rating 0.4      5.6
;----- Vortex_ICD_100 Rating 0.5      6.3
;----- Vortex_ICD_100 Rating 0.6      6.9
;----- Vortex_ICD_100 Rating 0.7      7.5
;----- Vortex_ICD_100 Rating 0.8      8
;----- Vortex_ICD_100 Rating 0.9      8.5
;----- Vortex_ICD_100 Rating 1       8.9
;----- Vortex_ICD_100 Rating 1.2      9.8
;----- Vortex_ICD_100 Rating 1.4      10.6
;----- Vortex_ICD_100 Rating 1.6      11.3
;----- Vortex_ICD_100 Rating 1.8      12
;----- Vortex_ICD_100 Rating 2       12.6
;----- Vortex_ICD_100 Rating 2.5      14.1
;----- Vortex_ICD_100 Rating 3       15.5
;----- ;Tempest Rating Curve for Vortex ICD 105, No grate allowance
;----- Vortex_ICD_105 Rating 0       0
;----- Vortex_ICD_105 Rating 0.1      0.1
;----- Vortex_ICD_105 Rating 0.2      0.2
;----- Vortex_ICD_105 Rating 0.3      0.3
;----- Vortex_ICD_105 Rating 0.4      0.4
;----- Vortex_ICD_105 Rating 0.5      0.5
;----- Vortex_ICD_105 Rating 0.6      0.6
;----- Vortex_ICD_105 Rating 0.7      0.7
;----- Vortex_ICD_105 Rating 0.8      0.8
;----- Vortex_ICD_105 Rating 0.9      0.9
;----- Vortex_ICD_105 Rating 1       0.9
;----- Vortex_ICD_105 Rating 1.2      1.2
;----- Vortex_ICD_105 Rating 1.4      1.4
;----- Vortex_ICD_105 Rating 1.6      1.6
;----- Vortex_ICD_105 Rating 1.8      1.8
;----- Vortex_ICD_105 Rating 2       1.8
;----- Vortex_ICD_105 Rating 2.5      2.5
;----- Vortex_ICD_105 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 40, No grate allowance
;----- Vortex_ICD_40 Rating 0       0
;----- Vortex_ICD_40 Rating 0.1      0.1
;----- Vortex_ICD_40 Rating 0.2      0.2
;----- Vortex_ICD_40 Rating 0.3      0.3
;----- Vortex_ICD_40 Rating 0.4      0.4
;----- Vortex_ICD_40 Rating 0.5      0.5
;----- Vortex_ICD_40 Rating 0.6      0.6
;----- Vortex_ICD_40 Rating 0.7      0.7
;----- Vortex_ICD_40 Rating 0.8      0.8
;----- Vortex_ICD_40 Rating 0.9      0.9
;----- Vortex_ICD_40 Rating 1       0.9
;----- Vortex_ICD_40 Rating 1.2      1.2
;----- Vortex_ICD_40 Rating 1.4      1.4
;----- Vortex_ICD_40 Rating 1.6      1.6
;----- Vortex_ICD_40 Rating 1.8      1.8
;----- Vortex_ICD_40 Rating 2       1.8
;----- Vortex_ICD_40 Rating 2.5      2.5
;----- Vortex_ICD_40 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 45, No grate allowance
;----- Vortex_ICD_45 Rating 0       0
;----- Vortex_ICD_45 Rating 0.1      0.1
;----- Vortex_ICD_45 Rating 0.2      0.2
;----- Vortex_ICD_45 Rating 0.3      0.3
;----- Vortex_ICD_45 Rating 0.4      0.4
;----- Vortex_ICD_45 Rating 0.5      0.5
;----- Vortex_ICD_45 Rating 0.6      0.6
;----- Vortex_ICD_45 Rating 0.7      0.7
;----- Vortex_ICD_45 Rating 0.8      0.8
;----- Vortex_ICD_45 Rating 0.9      0.9
;----- Vortex_ICD_45 Rating 1       0.9
;----- Vortex_ICD_45 Rating 1.2      1.2
;----- Vortex_ICD_45 Rating 1.4      1.4
;----- Vortex_ICD_45 Rating 1.6      1.6
;----- Vortex_ICD_45 Rating 1.8      1.8
;----- Vortex_ICD_45 Rating 2       1.8
;----- Vortex_ICD_45 Rating 2.5      2.5
;----- Vortex_ICD_45 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 50, No grate allowance
;----- Vortex_ICD_50 Rating 0       0
;----- Vortex_ICD_50 Rating 0.1      0.1
;----- Vortex_ICD_50 Rating 0.2      0.2
;----- Vortex_ICD_50 Rating 0.3      0.3
;----- Vortex_ICD_50 Rating 0.4      0.4
;----- Vortex_ICD_50 Rating 0.5      0.5
;----- Vortex_ICD_50 Rating 0.6      0.6
;----- Vortex_ICD_50 Rating 0.7      0.7
;----- Vortex_ICD_50 Rating 0.8      0.8
;----- Vortex_ICD_50 Rating 0.9      0.9
;----- Vortex_ICD_50 Rating 1       0.9
;----- Vortex_ICD_50 Rating 1.2      1.2
;----- Vortex_ICD_50 Rating 1.4      1.4
;----- Vortex_ICD_50 Rating 1.6      1.6
;----- Vortex_ICD_50 Rating 1.8      1.8
;----- Vortex_ICD_50 Rating 2       1.8
;----- Vortex_ICD_50 Rating 2.5      2.5
;----- Vortex_ICD_50 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 55, No grate allowance
;----- Vortex_ICD_55 Rating 0       0
;----- Vortex_ICD_55 Rating 0.1      0.1
;----- Vortex_ICD_55 Rating 0.2      0.2
;----- Vortex_ICD_55 Rating 0.3      0.3
;----- Vortex_ICD_55 Rating 0.4      0.4
;----- Vortex_ICD_55 Rating 0.5      0.5
;----- Vortex_ICD_55 Rating 0.6      0.6
;----- Vortex_ICD_55 Rating 0.7      0.7
;----- Vortex_ICD_55 Rating 0.8      0.8
;----- Vortex_ICD_55 Rating 0.9      0.9
;----- Vortex_ICD_55 Rating 1       0.9
;----- Vortex_ICD_55 Rating 1.2      1.2
;----- Vortex_ICD_55 Rating 1.4      1.4
;----- Vortex_ICD_55 Rating 1.6      1.6
;----- Vortex_ICD_55 Rating 1.8      1.8
;----- Vortex_ICD_55 Rating 2       1.8
;----- Vortex_ICD_55 Rating 2.5      2.5
;----- Vortex_ICD_55 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 60, No grate allowance
;----- Vortex_ICD_60 Rating 0       0
;----- Vortex_ICD_60 Rating 0.1      0.1
;----- Vortex_ICD_60 Rating 0.2      0.2
;----- Vortex_ICD_60 Rating 0.3      0.3
;----- Vortex_ICD_60 Rating 0.4      0.4
;----- Vortex_ICD_60 Rating 0.5      0.5
;----- Vortex_ICD_60 Rating 0.6      0.6
;----- Vortex_ICD_60 Rating 0.7      0.7
;----- Vortex_ICD_60 Rating 0.8      0.8
;----- Vortex_ICD_60 Rating 0.9      0.9
;----- Vortex_ICD_60 Rating 1       0.9
;----- Vortex_ICD_60 Rating 1.2      1.2
;----- Vortex_ICD_60 Rating 1.4      1.4
;----- Vortex_ICD_60 Rating 1.6      1.6
;----- Vortex_ICD_60 Rating 1.8      1.8
;----- Vortex_ICD_60 Rating 2       1.8
;----- Vortex_ICD_60 Rating 2.5      2.5
;----- Vortex_ICD_60 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 65, No grate allowance
;----- Vortex_ICD_65 Rating 0       0
;----- Vortex_ICD_65 Rating 0.1      0.1
;----- Vortex_ICD_65 Rating 0.2      0.2
;----- Vortex_ICD_65 Rating 0.3      0.3
;----- Vortex_ICD_65 Rating 0.4      0.4
;----- Vortex_ICD_65 Rating 0.5      0.5
;----- Vortex_ICD_65 Rating 0.6      0.6
;----- Vortex_ICD_65 Rating 0.7      0.7
;----- Vortex_ICD_65 Rating 0.8      0.8
;----- Vortex_ICD_65 Rating 0.9      0.9
;----- Vortex_ICD_65 Rating 1       0.9
;----- Vortex_ICD_65 Rating 1.2      1.2
;----- Vortex_ICD_65 Rating 1.4      1.4
;----- Vortex_ICD_65 Rating 1.6      1.6
;----- Vortex_ICD_65 Rating 1.8      1.8
;----- Vortex_ICD_65 Rating 2       1.8
;----- Vortex_ICD_65 Rating 2.5      2.5
;----- Vortex_ICD_65 Rating 3       2.4
;----- ;Tempest Rating Curve for Vortex ICD 70, No grate allowance
;----- Vortex_ICD_70 Rating 0       0

```











EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

\*\*\*\*\*  
Element Count  
\*\*\*\*\*  
Number of rain gages ..... 2  
Number of subcatchments ... 17  
Number of nodes ..... 23  
Number of links ..... 29  
Number of pollutants ..... 0  
Number of land uses ..... 0

\*\*\*\*\*  
Raingage Summary  
\*\*\*\*\*  
Name Data Source Data Type Recording Interval  
24SCS100 24SCS100 INTENSITY 15 min.  
3CHI100 3CHI100 INTENSITY 10 min.

\*\*\*\*\*  
Subcatchment Summary  
\*\*\*\*\*  
Name Area Width %Imperv %Slope Rain Gage Outlet  
-----  
S1 0.02 29.61 0.00 2.2000 24SCS100 CB208  
S10 0.03 55.32 99.62 2.2000 24SCS100 CB204  
S11 0.13 65.66 90.33 2.2000 24SCS100 CB202  
S12 0.01 65.44 0.77 2.2000 24SCS100 CB203  
S14\_1 0.01 24.78 52.68 2.2000 24SCS100 CB211  
S14\_2 0.02 58.12 31.65 2.2000 24SCS100 CB212  
S15 0.04 108.74 42.75 2.2000 24SCS100 CB210B  
S2 0.07 33.61 62.94 2.2000 24SCS100 CB209  
S3 0.07 45.79 96.69 2.2000 24SCS100 CB207  
S22 0.05 29.70 14.29 2.2000 24SCS100 CB214  
S4 0.05 24.67 99.77 2.2000 24SCS100 CB205  
S45 0.02 44.22 99.96 2.2000 24SCS100 CB203  
S5 0.08 82.96 89.59 2.2000 24SCS100 CB200  
S6 0.02 40.30 0.00 2.2000 24SCS100 CB214  
S7 0.07 25.95 100.00 2.2000 24SCS100 520\_(P-  
Stm)  
S8 0.09 35.06 80.12 2.2000 24SCS100 CB201  
S9 0.03 19.08 76.83 2.2000 24SCS100 CB205

\*\*\*\*\*  
Node Summary  
\*\*\*\*\*  
Name Type Invert Elev. Max. Depth Ponded Area External Inflow  
-----  
518\_Orifice JUNCTION 81.36 4.11 0.0  
518A\_(P-Stm) OUTFALL 80.65 1.13 0.0  
518\_(P-Stm) STORAGE 81.06 4.41 0.0  
519\_(P-Stm) STORAGE 82.20 3.51 0.0  
520\_(P-Stm) STORAGE 82.49 2.89 0.0  
521\_(P-Stm) STORAGE 81.96 3.67 0.0  
522\_(P-Stm) STORAGE 82.18 3.32 0.0  
CB200 STORAGE 83.18 2.32 0.0  
CB201 STORAGE 83.09 2.66 0.0  
CB202 STORAGE 82.76 2.89 0.0  
CB203 STORAGE 82.75 2.90 0.0  
CB204 STORAGE 82.75 2.75 0.0  
CB205 STORAGE 82.85 2.75 0.0  
CB206 STORAGE 82.77 2.73 0.0  
CB207 STORAGE 83.15 2.30 0.0  
CB208 STORAGE 83.30 2.04 0.0  
CB209 STORAGE 83.20 1.90 0.0  
CB210B STORAGE 82.65 3.00 0.0  
CB211 STORAGE 83.70 1.90 0.0  
CB212A STORAGE 83.62 2.42 0.0  
CB213 STORAGE 83.15 1.75 0.0  
CB214 STORAGE 82.95 1.90 0.0

\*\*\*\*\*  
Link Summary  
\*\*\*\*\*  
Name From Node To Node Type Length %Slope  
Roughness  
-----  
C2 CB208 CB207 CONDUIT 9.7 1.0346  
C4 CB212A CB212 CONDUIT 18.5 2.0031  
C5 CB211 CB212 CONDUIT 22.3 2.0142  
CB213 CB213 522\_(P-Stm) CONDUIT 29.3 1.3396  
CB214 CB214 522\_(P-Stm) CONDUIT 21.6 0.8915  
Pipe\_-(104)\_ (P-Stm)\_2 518\_Orifice 518A\_(P-Stm) CONDUIT 15.6 0.2496  
Pipe\_-(110)\_ (P-Stm) 521\_(P-Stm) 518\_(P-Stm) CONDUIT 27.2 0.3528  
Pipe\_-(111)\_ (P-Stm) 522\_(P-Stm) 521\_(P-Stm) CONDUIT 45.1 0.4990  
Pipe\_-(71)\_ (P-Stm) 520\_(P-Stm) 519\_(P-Stm) CONDUIT 57.1 0.4025  
Pipe\_-(72)\_ (P-Stm) 519\_(P-Stm) 518\_(P-Stm) CONDUIT 32.0 0.5005  
0.0130 OR1 518\_(P-Stm) 518\_Orifice ORIFICE  
W1 CB208 CB209 WEIR  
W2 CB201 CB201 WEIR  
W3 CB210B CB202 WEIR  
W4 CB202 CB204 WEIR  
W5 CB206 CB207 WEIR  
W6 CB201 CB200 WEIR  
W7 CB200 CB209 WEIR  
CB200 CB200 520\_(P-Stm) OUTLET  
CB201 CB201 520\_(P-Stm) OUTLET  
CB202 CB202 521\_(P-Stm) OUTLET  
CB203 CB203 521\_(P-Stm) OUTLET  
CB204 CB204 522\_(P-Stm) OUTLET  
CB205 CB205 522\_(P-Stm) OUTLET  
CB206 CB206 522\_(P-Stm) OUTLET  
CB207 CB207 522\_(P-Stm) OUTLET  
CB209 CB209 520\_(P-Stm) OUTLET  
CB210B CB210B 518\_(P-Stm) OUTLET  
CB212 CB212 521\_(P-Stm) OUTLET

\*\*\*\*\*  
Cross Section Summary  
\*\*\*\*\*  
Conduit Shape Full Depth Full Area Hyd. Rad. Max. Width No. of Barrels Full Flow  
-----  
C2 CIRCULAR 0.25 0.05 0.06 0.25 1 60.49  
C4 CIRCULAR 0.25 0.05 0.06 0.25 1 84.17  
C5 CIRCULAR 0.25 0.05 0.06 0.25 1 84.40  
CB213 CIRCULAR 0.25 0.05 0.06 0.25 1 68.83  
CB214 CIRCULAR 0.25 0.05 0.06 0.25 1 56.15  
Pipe\_-(104)\_ (P-Stm)\_2 CIRCULAR 0.45 0.16 0.11 0.45 1  
142.46 Pipe\_-(110)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 255.47  
Pipe\_-(111)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 303.80  
Pipe\_-(71)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 272.85  
Pipe\_-(72)\_ (P-Stm) CIRCULAR 0.53 0.22 0.13 0.53 1 304.28



# Post-Development 24-hour SCS 1:100-year Event

August 2024

CB210B	0.000	0.6	0.0	0.0	0.005	35.5	0	12:01
6.26								
CB211	0.000	0.1	0.0	0.0	0.000	2.6	0	12:12
3.47								
CB212	0.000	0.3	0.0	0.0	0.001	8.0	0	12:12
5.94								
CB212A	0.000	1.7	0.0	0.0	0.000	55.3	0	12:12
1.36								
CB213	0.000	0.5	0.0	0.0	0.000	14.2	0	12:09
8.24								
CB214	0.000	0.3	0.0	0.0	0.002	30.3	0	12:08
18.95								

## Outfall Loading Summary

Outfall Node	Flow Freq Pct	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
518A (P-Stm)	93.05	8.15	64.71	0.655
System	93.05	8.15	64.71	0.655

## Link Flow Summary

Link	Type	Maximum  Flow  LPS	Time of Max Occurrence days hr:min	Maximum [Veloc] m/sec	Max/ Full Flow	Max/ Full Depth
C2	CONDUIT	12.10	0 11:49	0.25	0.20	1.00
C4	CONDUIT	4.74	0 11:56	0.11	0.06	1.00
C5	CONDUIT	3.89	0 11:55	0.60	0.05	1.00
CB213	CONDUIT	16.59	0 11:50	0.39	0.24	1.00
CB214	CONDUIT	18.95	0 11:59	0.59	0.34	1.00
Pipe_-(104)_-(P-Stm)	CONDUIT	64.71	0 11:49:09	1.04	0.45	1.41
Pipe_-(110)_-(P-Stm)	CONDUIT	52.47	0 11:49:09	0.73	0.21	1.00
Pipe_-(111)_-(P-Stm)	CONDUIT	34.44	0 11:49	0.48	0.11	1.00
Pipe_-(71)_-(P-Stm)	CONDUIT	66.91	0 11:51	0.86	0.25	1.00
Pipe_-(72)_-(P-Stm)	CONDUIT	43.59	0 11:50	0.88	0.14	1.00
OR1	ORIFICE	64.71	0 12:09			1.00
W1	WEIR	0.00	0 00:00			0.00
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	9.31	0 11:59			0.14
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	10.18	0 12:00			0.22
W6	WEIR	0.00	0 00:00			0.00
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	23.50	0 11:51			
CB201	DUMMY	1.93	0 11:49			
CB202	DUMMY	27.75	0 11:48			
CB203	DUMMY	4.32	0 11:59			
CB204	DUMMY	4.98	0 11:52			
CB205	DUMMY	4.51	0 11:55			
CB206	DUMMY	6.50	0 11:49			
CB207	DUMMY	12.51	0 11:49			
CB209	DUMMY	18.18	0 11:54			
CB210B	DUMMY	4.87	0 11:52			
CB212	DUMMY	4.68	0 11:50			

## Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Dry	Dry	Up	Down	Sub	Up	Up	Down
C2	1.00	0.46	0.02	0.00	0.07	0.00	0.00	0.45	0.00
C4	1.00	0.48	0.01	0.00	0.05	0.00	0.00	0.45	0.01
C5	1.00	0.48	0.00	0.00	0.06	0.00	0.00	0.46	0.02
CB213	1.00	0.05	0.00	0.00	0.07	0.00	0.00	0.88	0.01
CB214	1.00	0.04	0.00	0.00	0.07	0.00	0.00	0.89	0.01
Pipe_-(104)_-(P-Stm)_2	1.00	0.07	0.00	0.00	0.00	0.00	0.00	0.93	0.00
Pipe_-(110)_-(P-Stm)	1.00	0.06	0.00	0.00	0.08	0.00	0.00	0.85	0.00
Pipe_-(111)_-(P-Stm)	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.86
Pipe_-(71)_-(P-Stm)	1.00	0.06	0.00	0.00	0.07	0.00	0.00	0.87	0.01
Pipe_-(72)_-(P-Stm)	1.00	0.07	0.00	0.00	0.08	0.00	0.00	0.85	0.01

## Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full			Hours Capacity Limited	
	Both Ends	Upstream	Dnstream	Normal	Flow	Limited		
C2	1.48	1.48	1.50	0.01			0.00	
C4	0.95	0.95	1.00	0.01			0.01	
C5	0.89	0.89	1.22	0.01			0.01	
CB213	1.19	1.19	1.44	0.01			0.01	
CB214	1.31	1.31	1.44	0.01			0.01	
Pipe_-(110)_-(P-Stm)	1.62	1.62	1.73	0.01			0.01	
Pipe_-(111)_-(P-Stm)	1.44	1.44	1.62	0.01			0.01	
Pipe_-(71)_-(P-Stm)	1.24	1.24	1.38	0.01			0.01	
Pipe_-(72)_-(P-Stm)	1.43	1.43	1.55	0.01			0.01	

Analysis begun on: Thu Aug 29 09:25:39 2024  
 Analysis ended on: Thu Aug 29 09:25:41 2024  
 Total elapsed time: 00:00:02

# Stormceptor® EF Sizing Report

## Imbrium® Systems

### ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

08/28/2024

Province:	Ontario
City:	Ottawa
Nearest Rainfall Station:	OTTAWA CDA RCS
Climate Station Id:	6105978
Years of Rainfall Data:	20
Site Name:	
Drainage Area (ha):	0.82
% Imperviousness:	73.64

Runoff Coefficient 'c': 0.74

Project Name:	Navan Gas Bar
Project Number:	29899-002
Designer Name:	Bobby Pettigrew
Designer Company:	J.L. Richards & Associates Ltd
Designer Email:	bpettigrew@jlrichards.ca
Designer Phone:	343-804-5381
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	19.63
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	66.00
Peak Conveyance (maximum) Flow Rate (L/s):	66.00
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	657
Estimated Average Annual Sediment Volume (L/yr):	534

### Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EFO4	80
EFO6	90
EFO8	95
EFO10	97
EFO12	99

Recommended Stormceptor EFO Model: **EFO4**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **80**

Water Quality Runoff Volume Capture (%): **> 90**

**Stormceptor® EF Sizing Report****THIRD-PARTY TESTING AND VERIFICATION**

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators and performance has been third-party verified in accordance with the ISO 14034 Environmental Technology Verification (ETV) protocol.

**PERFORMANCE**

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

**PARTICLE SIZE DISTRIBUTION (PSD)**

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size ( $\mu\text{m}$ )	Percent Less Than	Particle Size Fraction ( $\mu\text{m}$ )	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

## Stormceptor® EF Sizing Report

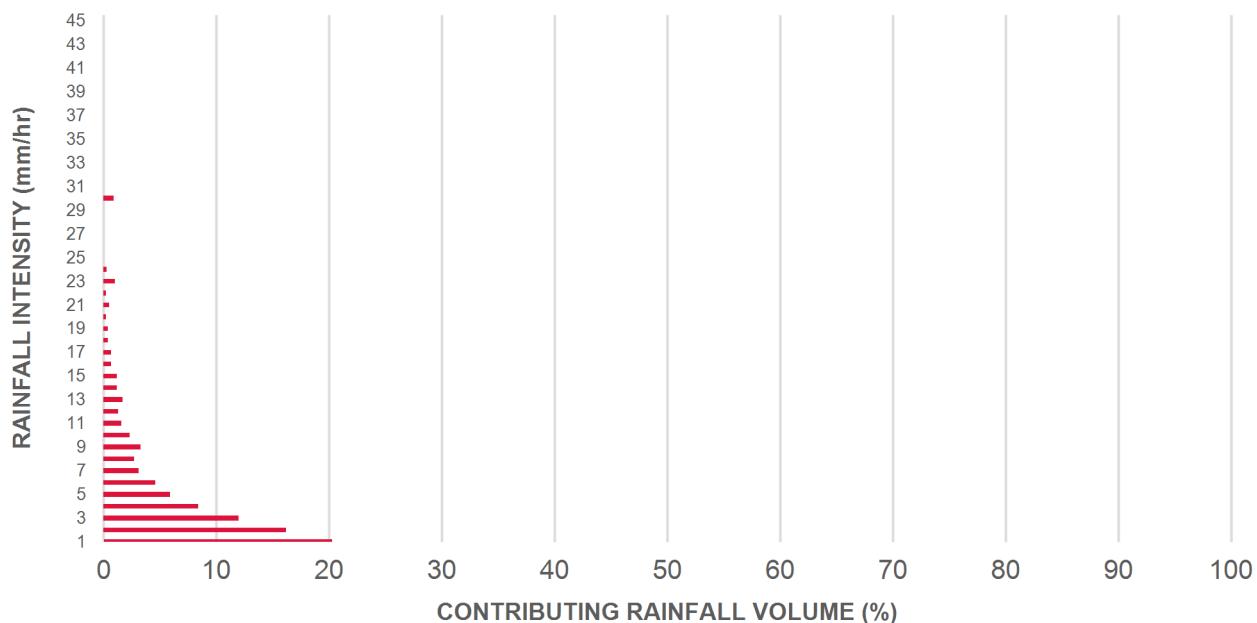
### Upstream Flow Controlled Results

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.6	8.6	0.85	51.0	42.0	100	8.6	8.6
1.00	20.3	29.0	1.69	101.0	85.0	98	20.0	28.6
2.00	16.2	45.2	3.38	203.0	169.0	87	14.1	42.7
3.00	12.0	57.2	5.07	304.0	254.0	81	9.7	52.4
4.00	8.4	65.6	6.76	406.0	338.0	77	6.5	58.9
5.00	5.9	71.6	8.46	507.0	423.0	73	4.3	63.3
6.00	4.6	76.2	10.15	609.0	507.0	69	3.2	66.5
7.00	3.1	79.3	11.84	710.0	592.0	65	2.0	68.5
8.00	2.7	82.0	13.53	812.0	676.0	64	1.8	70.2
9.00	3.3	85.3	15.22	913.0	761.0	63	2.1	72.3
10.00	2.3	87.6	16.91	1015.0	846.0	63	1.4	73.8
11.00	1.6	89.2	18.60	1116.0	930.0	62	1.0	74.8
12.00	1.3	90.5	20.29	1218.0	1015.0	61	0.8	75.6
13.00	1.7	92.2	21.98	1319.0	1099.0	59	1.0	76.6
14.00	1.2	93.5	23.68	1421.0	1184.0	57	0.7	77.3
15.00	1.2	94.6	25.37	1522.0	1268.0	56	0.6	77.9
16.00	0.7	95.3	27.06	1623.0	1353.0	53	0.4	78.3
17.00	0.7	96.1	28.75	1725.0	1437.0	51	0.4	78.7
18.00	0.4	96.5	30.44	1826.0	1522.0	48	0.2	78.9
19.00	0.4	96.9	32.13	1928.0	1607.0	46	0.2	79.1
20.00	0.2	97.1	33.82	2029.0	1691.0	43	0.1	79.2
21.00	0.5	97.5	35.51	2131.0	1776.0	41	0.2	79.4
22.00	0.2	97.8	37.20	2232.0	1860.0	39	0.1	79.4
23.00	1.0	98.8	38.90	2334.0	1945.0	38	0.4	79.8
24.00	0.3	99.1	40.59	2435.0	2029.0	36	0.1	79.9
25.00	0.9	100.0	42.28	2537.0	2114.0	35	0.3	80.3
30.00	0.9	100.9	50.73	3044.0	2537.0	29	0.3	80.5
35.00	-0.9	100.0	59.19	3551.0	2959.0	25	N/A	80.3
40.00	0.0	100.0	66.00	3960.0	3300.0	22	0.0	80.3
45.00	0.0	100.0	66.00	3960.0	3300.0	22	0.0	80.3
<b>Estimated Net Annual Sediment (TSS) Load Reduction =</b>								<b>80 %</b>

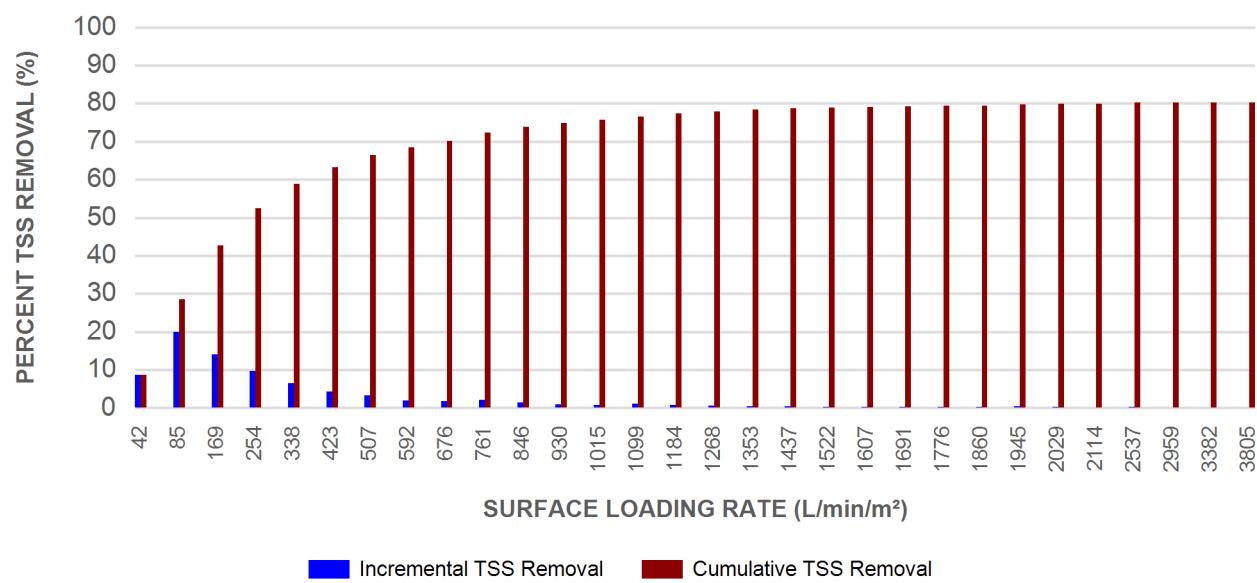
Climate Station ID: 6105978 Years of Rainfall Data: 20

## Stormceptor® EF Sizing Report

### RAINFALL DATA FROM OTTAWA CDA RCS RAINFALL STATION



### INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



## Stormceptor® EF Sizing Report

### Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

### SCOUR PREVENTION AND ONLINE CONFIGURATION

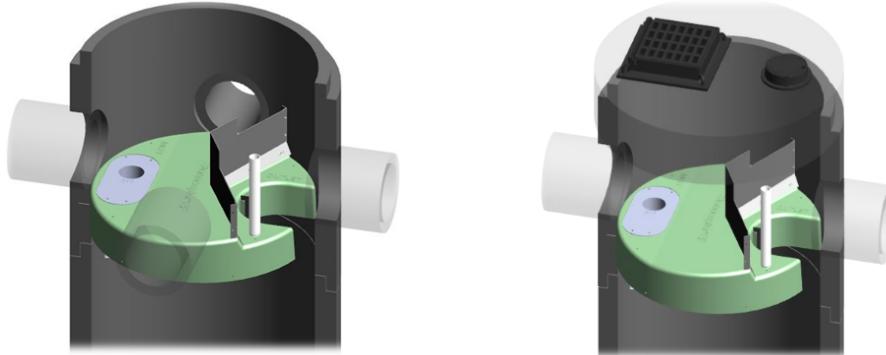
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

### DESIGN FLEXIBILITY

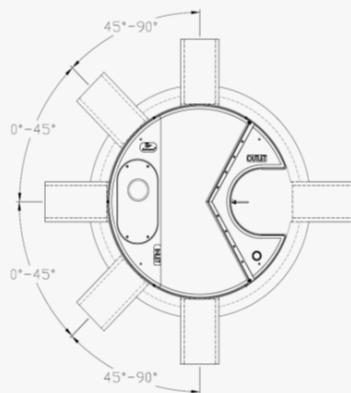
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

### OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



## Stormceptor® EF Sizing Report



### INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

### HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

### Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

\*Increased sump depth may be added to increase sediment storage capacity

\*\* Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³ )

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

### STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

### STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

**Stormceptor® EF Sizing Report****STANDARD PERFORMANCE SPECIFICATION FOR  
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE****PART 1 – GENERAL****1.1 WORK INCLUDED**

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

**1.2 REFERENCE STANDARDS & PROCEDURES**

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

**1.3 SUBMITTALS**

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

**PART 2 – PRODUCTS****2.1 OGS POLLUTANT STORAGE**

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m <sup>3</sup> sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m <sup>3</sup> sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m <sup>3</sup> sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m <sup>3</sup> sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m <sup>3</sup> sediment / 2,476 L oil

**PART 3 – PERFORMANCE & DESIGN****3.1 GENERAL**

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



**Stormceptor® EF Sizing Report**

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

### **3.2 SIZING METHODOLOGY**

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m<sup>2</sup> to 1400 L/min/m<sup>2</sup>, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m<sup>2</sup> and 1400 L/min/m<sup>2</sup> shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m<sup>2</sup> shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m<sup>2</sup>. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m<sup>2</sup>.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m<sup>2</sup> shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m<sup>2</sup>, and shall be calculated using a simple proportioning formula, with 1400 L/min/m<sup>2</sup> in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m<sup>2</sup>.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

### **3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING**

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m<sup>2</sup>.

### **3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING**

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to



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assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m<sup>2</sup> to 2600 L/min/m<sup>2</sup>) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.





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