SERVICING & STORMWATER MANAGEMENT REPORT MIXED USE DEVELOPMENT – 3030 ST JOSEPH BLVD



Project No.: CCO-24-0142

City File No.: D07-12-23-0090

Prepared for:

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Prepared by:

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1.0 PROJECT DESCRIPTION

1.1 Purpose

Egis (formerly McIntosh Perry) has been retained by Theberge Developments Ltd. to prepare this Servicing and Stormwater Management Report in support of the Site Plan Control process for the proposed mixed-use development, located at 3030 St Joseph Boulevard within the City of Ottawa.

The main purpose of this report is to present a servicing design for the development in accordance with the recommendations and guidelines provided by the City of Ottawa (City), the Rideau Valley Conservation Authority (RVCA), and the Ministry of the Environment, Conservation and Parks (MECP). This report will address the water, sanitary and storm sewer servicing for the development, ensuring that existing and available services will adequately service the proposed development.

This report should be read in conjunction with the following drawings:

- CCO-24-0142, C101 Site Grading and Drainage Plan,
- CCO-24-0142, C102 Site Servicing Plan,
- CCO-24-0142, PRE Pre-Development Drainage Area Plan (Appendix E), and
- CCO-24-0142, POST Post-Development Drainage Area Plan (Appendix F)

1.2 Site Description



Figure 1: Site Map

The subject property, herein referred to as the site, is located 3030 St Joseph Boulevard within the Orleans East-Cumberland ward. It is described as Part 1 Plan of Part of Lots 1,2,3 and 4, Registered



Plan 17, City of Ottawa. The land in question covers approximately *0.26 ha* and is located between St Joseph Boulevard and Duford Drive. See the Site Location Plan in *Appendix A* for more details.

1.3 Proposed Development and Statistics

The proposed development consists of an 18-storey mixed-use residential building. Site access will be provided via a drive aisle extending from St Joseph Boulevard with underground parking provided for residents and visitors. Refer to the Site Plan prepared by RLA Architecture included in *Appendix B* for details.

1.4 Existing Conditions and Infrastructure

The existing site is currently undeveloped and is not serviced by any sanitary or water services. There is an existing 254mm diameter cast iron and 305mm diameter PVC municipal watermain which traverse the site from Duford to St. Joseph. There is also an existing 450mm diameter concrete municipal sanitary which traverses the site from Duford to St. Joseph. Storm servicing for the site is provided via municipal catch basins within Duford Drive and St Joseph Boulevard.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal rights-of-way(s):

- St Joseph Boulevard
 - o 305 mm diameter cast iron watermain,
 - o 305 mm diameter PVC watermain,
 - o 406 mm diameter ductile irom watermain
 - o 610 mm C01 Watermain
 - 250 mm diameter and 450 mm diameter concrete sanitary sewer tributary to the Cumberland Collector, and a
 - o 675-750 mm diameter concrete storm sewer tributary to Taylor Creek approximately 0.4 km downstream.
- Duford Drive
 - o 254mm diameter cast iron watermain,
 - o 450 mm concrete sanitary sewer tributary to the Cumberland Collector, and a
 - 600 mm diameter concrete storm sewer tributary to Taylor Creek approximately
 0.50 km downstream.



1.5 Approvals

The proposed development is subject to the City of Ottawa site plan control approval process. Site plan control requires the City to review, provided concurrence and approve the engineering design package. Permits to construct can be requested once the City has issued a site plan agreement.

An Environmental Compliance Approval (ECA) through the Ministry of Environment, Conservation and Parks (MECP) is not anticipated to be required for the development since the development is contained within a single parcel of land, is not within a combined sewershed, and does not propose industrial sewage. As a result, the stormwater management system meets the exemption requirements under O.Reg 525/90.

2.0 BACKROUND STUDIES

Background studies that have been completed for the proposed site include City of Ottawa as-built drawings, and a topographical survey.

As-built drawings of existing services within the vicinity of the proposed site were reviewed in order to determine accurate servicing and stormwater management schemes for the site.

A topographic survey of the site was completed by Annis, O'Sullivan, Vollebekk Ltd.

2.1 Applicable Guidelines and Standards

City of Ottawa:

- Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (Ottawa Sewer Guidelines)
 - Technical Bulletin ISTB-2014-01 City of Ottawa, February 2014. (ISTB-2014-01)
 - Technical Bulletin PIEDTB-2016-01 City of Ottawa, September 2016. (PIEDTB-2016-01)
 - Technical Bulletin ISTB-2018-01 City of Ottawa, January 2018. (ISTB-2018-01)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (ISTB-2018-03)
 - Technical Bulletin ISTB-2019-01 City of Ottawa, January 2019. (ISTB-2019-01)
 - Technical Bulletin ISTB-2019-02 City of Ottawa, February 2019. (ISTB-2019-02)
- Ottawa Design Guidelines Water Distribution City of Ottawa, July 2010. (Ottawa Water Guidelines)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 2014. (ISDTB-2014-02)
 - Technical Bulletin ISTB-2018-02 City of Ottawa, March 2018. (ISTB-2018-02)

Ministry of Environment, Conservation and Parks:

• Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (MECP Stormwater Design Manual)



• Design Guidelines for Sewage Works, Ministry of the Environment, 2008. (*MECP Sewer Design Guidelines*)

Other:

• Water Supply for Public Fire Protection, Fire Underwriters Survey, 2020. (FUS Guidelines)



3.0 PRE-CONSULTATION SUMMARY

A pre-consultation meeting was conducted on March 23, 2023 regarding the proposed site. Specific design parameters to be incorporated within this design include the following:

- Post-development 100-year storm to be restricted to pre-development 5-year storm.
- The allowable release rate is to be computed using the lesser of C=0.5 or existing.
- Quality control as per RVCA requirements

The notes from the City of Ottawa can be found in Appendix B.



4.0 WATERMAIN

4.1 Existing Watermain

The site is located within the 2E pressure zone, as per the Water Distribution System Mapping included in *Appendix C*. There are existing, 406mm ductile iron and 610mm C01 watermains within St Joseph Boulevard. There is an existing 254 cast iron - 305mm PVC watermain which extends through the site into Duford Drive. There are three public hydrants located on St Joseph Boulevard available to provide fire flow to the development.

There are currently no existing private water services within the site.

4.2 Watermain Relocation

The existing 254 cast iron – 305mm PVC watermain currently located within the site is to be relocated around the site prior to the construction of the development. The proposed watermain will be extend around the site within the Duford Drive ROW before connecting back to the existing watermain within St. Joseph Boulevard. Refer to plans *C102, C202* and *C203* for detailed servicing layouts.

4.3 Proposed Watermain

Dual 150mm diameter PVC water services are proposed to service the development complete with water valves located at the property line. The water services are proposed to be serviced by the existing 406 mm diameter ductile iron watermain within St Joseph Boulevard. The services are designed to have a minimum of 2.4m cover. Refer to drawing *C102* for a detailed servicing layout.

The Fire Underwriters Survey 2020 (FUS) method was utilized to determine the required fire flow for the site. The 'C' factor (type of construction) for the FUS calculation was determined to be 0.8 (non-combustible construction). The total effective floor area ('A' value) for the FUS calculation was determined to be 3,309 m^2 . The results of the calculations yielded a required fire flow of 5,000 *L/min*. The detailed calculations for the FUS and can be found in *Appendix C*.

The water demands for the proposed building have been calculated to adhere to the *Ottawa Design Guidelines – Water Distribution* manual and can be found in *Appendix C*. The results have been summarized below. In accordance with Section 4.3.1 of the guidelines, service areas with a basic day demand greater than 50 m³/day require a dual connection to the municipal system. The basic day demand for the development is estimated to be $103 \text{ m}^3/\text{day}$, therefore a dual connection is required.



Table 1: Water Demands

Site Area	0.26ha
Residential	280 L/c/day
Commercial	28,000 L/ha/day
Average Day Demand (L/s)	1.08
Maximum Daily Demand (L/s)	3.60
Peak Hourly Demand (L/s)	5.41
FUS Fire Flow Requirement (L/s)	83.33

To confirm the adequacy of fire flow to protect the proposed development, public fire hydrants within 150 m of the proposed building were reviewed per City of Ottawa *ISTB 2018-02* Appendix I Table 1. Based on City guidelines (*ISTB-2018-02*), the existing hydrants can provide adequate fire coverage to the proposed development. The results are summarized below.

Table 2: Fire Protection Confirmation

Building	Fire Flow Demand	Fire Hydrant(s)	Fire Hydrant(s)	Combined Fire
	(L/min.)	within 75m*	within 150m*	Flow (L/min.)
Proposed Building	5,000 (FUS)	1	2	13,000

*Fire hydrants within 75 metres contribute 5,700 L/min to fire flow and fire hydrants within 150 meters contribute 3,800 L/min to fire flow, respectively, per ISTB-2018-02.

The City provided the estimated water pressures at both for the average day scenario, peak hour scenario and the max day plus fire flow scenario for the demands indicated by the correspondence in *Appendix C*. The resulting pressures for the boundary conditions results are shown in *Table 3*, below.



CC	$^{\circ}$	10	112
CC	U-2	4-0	142

Table 3: Boundar	y Condition Results

Scenario	Proposed Demands (L/s)	Connection 1 HGL (m H ₂ O)*/kPa	
Average Day Demand	1.08	44.7 / 438.5	
Maximum Daily + Fire Flow Demand	86.93	41.8 / 410.1	
Peak Hourly Demand	5.41	40.0 / 392.4	
*Adjusted for an estimated ground elevation of 69.4m above the connection point.			

The normal operating pressure range is anticipated to be 392 kPa to 439 kPa and will not be less than 275 kPa (40 psi) or exceed 689 kPa (100 psi). The proposed watermains will meet the minimum required 20 psi (140 kPa) from the Ottawa Water Guidelines at the ground level under maximum day demand and fire flow conditions. A pressure reducing valve is not anticipated to be required since the pressures do not exceed 552 kPa (80 psi) in the average day scenario.



5.0 SANITARY DESIGN

5.1 Existing Sanitary Sewer

There is an existing 250 mm diameter and 450 mm diameter Concrete sanitary sewer within St Joseph Boulevard, tributary to the Cumberland Collector. There is an existing 450mm concrete sanitary that sewer extends through the site from Duford Drive. Refer to the City of Ottawa Trunk Sewer Map figure available in *Appendix D*.

5.2 Sanitary Sewer Realignment

The existing 450mm diameter concrete sanitary sewer, which is routed through the site, is to be realigned from MHSA353869 to MHSA4436, to allow for the construction of the proposed building.

As shown on plan *C102* a maintenance structure (*MH2A*) is proposed to tie into the existing sewer just downstream of MHSA35869. The proposed alignment will route the sewer outside of the property within the Duford Drive before tying back into the existing 450mm diameter sewer at *MH2E* just downstream of existing MHSA4436. Refer to plan *C102*.

The contractor will be required to submit a formal construction phasing and flow management plan to both Egis and the City inspector for approval prior to commencement of construction.

5.2.1 Pipe Capacity

The approximate capacity of the existing 450mm sewer downstream of MHSA4436 is *119 L/s*. As demonstrated on the *Sanitary Sewer Design Relocation Sheet* in *Appendix D* the capacity of the realigned 450mm diameter sewer network from *MH2A* to *MH2E* is *495 L/s*. Therefore, capacity of the realigned network exceeds that of the existing immediate downstream network. Due to the complexity of the further downstream network, the City will need to advise of any downstream constraints.

5.3 Proposed Sanitary Sewer

A new 200 mm diameter gravity sanitary service is proposed to be connected to the existing 250 mm diameter sanitary sewer within St Joseph Boulevard. The sanitary service will be complete with a maintenance hole (MH1A) which will be installed at the property line as per the *City of Ottawa – Sewer Design Guidelines*.

The proposed development consists of a mixed-use residential building. The peak design flows for the proposed buildings were calculated using criteria from the Ottawa Sewer Guidelines and are summarized in *Table 4*, below.



Design Parameter	Value
Site Area	0.26 ha
Residential Demand	280 L/c/d
Residential Peaking Factor	3.45
Commercial/Amenity	2,800 L/(1000m² /d)
Institutional Peaking Factor	1.0
Extraneous Flow Allowance	0.33 L/s/ha

Table 4:	Sanitary	Design	Criteria
	Sanntary	Design	ontena

Based on the unit occupancy statistics provided by the architect, the proposed site development will generate a flow of *4.29 L/s. Table 5* below, summarizes the estimated wastewater flow from the proposed development. Refer to *Appendix D for* detailed calculations.

Table 5: Summary of Estimated Sa	anitary Flow
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Design Parameter	Total Flow (L/s)
Total Estimated Average Dry Weather Flow	1.17
Total Estimated Peak Dry Weather Flow	3.80
Total Estimated Peak Wet Weather Flow	3.88

5.4 Proposed Sanitary Capacity

The proposed sanitary network has been designed to attain a minimum full flow target velocity (cleansing velocity) of 0.6 m/s and a full flow velocity of not more than 3.0 m/s. The capacity of the proposed 200 mm diameter sanitary service with a slope of 1.00% is *34.22 L/s* based on sanitary sewer design calculations available in *Appendix D*.

Due to the complexity of the downstream network, the City will need to advise of any downstream constraints.



6.0 STORM SEWER DESIGN

6.1 Existing Storm Sewers

Water runoff from the site is currently tributary to Taylor Creek within the Ottawa River East Subwatershed. Storm servicing for the site is currently provided via municipal catchbasins within St Joseph Boulevard and Duford Drive.

There is an existing 600mm diameter concrete sewer within Duford Drive. There is also a 675mm diameter concrete sewer within St Joseph Boulevard. Drainage collected from the existing storm sewers is tributary to Taylor Creek approximately 0.5 km downstream and ultimately the Ottawa River.

6.2 Proposed Storm Sewers

A new 250mm diameter storm service is proposed to be extended from the existing 600mm diameter storm sewer within Duford Drive. The sewer system will provide attenuation for the roof area and at grade walkway area east of the building by an internal cistern complete with a flow attenuation device to be specified by the mechanical engineer.

Runoff from the landscaped area south of the building will be directed to a catchbasin complete with a Tempest LMF40 ICD to provide attenuation. Attenuated flow will discharge to the existing 600mm diameter storm sewer via a 250mm diameter catchbasin lead.

See drawing *C102* for a detailed servicing layout and *CCO-23-1150 - POST* included in *Appendix 'F'* of this report for more details. The Stormwater Management design for the subject property will be outlined in *Section 7.0* of this report.



7.0 PROPOSED STORMWATER MANAGEMENT

7.1 Design Criteria and Methodology

Stormwater management for the proposed site will be maintained through two methods. The first will store and control runoff collected from the roof of the proposed building and the at-grade walkway area within the site. The second will control stormwater via a subsurface storage tank and will collect runoff from the at-grade area to the south of the site. Flow will be directed to a catch basin complete with an ICD for attenuation before discharging to the existing 600mm diameter concrete storm sewer within Duford Drive.

In summary, the following design criteria have been employed in developing the stormwater management design for the site as directed by the RVCA and City:

Quality Control

• Due to the site proximity to the outlet runoff must be designed to achieve an 80% total suspended solids removal (*enhanced* level).

Quantity Control

- Pre-development time of concentration (TC) shall be calculated and be no less than 10minutes. Post development runoff shall be calculated with a TC of 10 minutes.
- Control site post-development flows to the 5-year pre-development flows with a calculated C value no greater than 0.50.

7.2 Runoff Calculations

Runoff calculations presented in this report are derived using the Rational Method, given as:

Q = 2.78CIA (L/s)					
Where	С	= Runoff coefficient			
	I	= Rainfall intensity in mm/hr (City of Ottawa IDF curves)			
	А	= Drainage area in hectares			

It is recognized that the Rational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any SWM facility sized using this method is expected to function as intended.



Roofs/Concrete/Asphalt	0.90
Gravel	0.60
Undeveloped and Grass	0.20

The following coefficients were used to develop an average C for each area:

As per the *City of Ottawa - Sewer Design Guidelines*, the average 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

7.3 Pre-Development Drainage

It has been assumed that the site contains no stormwater management controls for flow attenuation. The estimated pre-development peak flows for the 5, and 100-year events are summarized below in *Table 6*. See CCO-24-0142 - *PRE* in *Appendix E* and *Appendix G* for calculations.

Table 6: Pre-Development Runoff Summary

Drainage Area	Area (ha)	Q (L/s)	
		5-Year	100-Year
A1	0.264	15.31	32.79
Total	0.264	15.31	32.79

See Appendix E for an existing conditions drainage plan and Appendix G for calculations.

7.4 Post-Development Drainage

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See CCO-24-0142 - *POST* in *Appendix F* of this report for more details. A summary of the Post-Development Runoff Calculations can be found below.

Drainage Area	Area (ha)	5-Year Peak Flow (L/s)	100-Year Peak Flow (L/s)	100-Year Storage Requirement (m ³)	100-Year Storage Available (m³)
B1	0.179	1.26	2.40	102.3	102.3
B2	0.057	1.10	1.30	5.7	13.0
B3	0.028	6.04	11.58		
Total	0.264	8.40	15.28	107.92	115.26



Runoff for Area B1 will be collected by roof drains (uncontrolled) and a trench drain (*TD1*) before being conveyed to the internal cistern. The *102.3* m^3 internal cistern is anticipated to direct stormwater to the outlet at a maximum flow rate of *1.26* L/s and *2.40* L/s for the 5 and 100-year storms, respectively. Flows in excess of the 100-year storm will need to be directed towards Duford Street via a cistern overflow. A flow attenuation device is to be specified by the Mechanical Engineer.

Runoff for Area B2 will be collected by catch basins (*CB1 & LCB2*) complete with a Tempest LMF 40 ICD which will provide attenuation. Runoff storage for area B2 will be provided by a Triton S-29B storage tank (or approved equivalent). Refer to *Appendix G* for detailed storage tank drawings. Runoff will be restricted to maximum flow rate of 1.10 L/s and 1.30 L/ for the 5 and 100-year storms, respectively, with up to 13.0 m^3 of tank storage.

Runoff for Area B3 will be directed to the St Joseph Boulevard right of way without attenuation and will be compensated for in areas with attenuation.

7.5 Quality Control

The following methods will be utilized to provide quality controls for the site:

- Quality controls for Area B1 will be provided via the cistern in a settling pit. No surface parking or drive aisles are proposed within this area. Runoff from at grade areas will combine with clean roof drainage before discharging to the city sewer. Cistern details are to be provided by the mechanical engineer.
- Area B2 will collect runoff from landscaped areas and therefore drainage is considered clean.



8.0 EROSION AND SEDIMENT CONTROL

8.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Silt fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catchbasins and filter fabric is to be placed under the grates of all existing catchbasins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures is to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the *Site Grading, Drainage and Sediment & Erosion Control Plan* for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

8.2 Permanent Measures

Rip-rap will be placed at all locations that have the potential for concentrated flow. It is crucial that the Contractor ensure that the geotextile is keyed in properly to ensure runoff does not undermine the rip rapped area. Additional rip rap is to be placed at erosion prone locations as identified by the Contractor / Contract Administrator / City or Conservation Authority.

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the



site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.



9.0 SUMMARY

- A new mixed-use residential building is proposed to be constructed at 3030 St Joseph Boulevard.
- Dual 150 mm diameter water services are proposed to be connected to the existing 406 mm diameter watermain within St Joseph Boulevard.
- A new 200 mm diameter sanitary service complete with a maintenance hole at the property line is proposed to service the development, extending from the existing 250 mm diameter sanitary sewer within St Joseph Boulevard.
- A new 250mm storm service for rooftop, surface, and foundation drainage is proposed to service the development. The storm service will connect to the 600 mm diameter storm sewer within Duford Drive.
- A new 200 mm storm service is proposed to convey drainage from the landscaped south of the building to the existing 600 mm diameter storm sewer within Duford Drive.
- Storage for the 5- through 100-year storm events will be provided through an internal cistern and a sub-surface storage system.
- Quality control is proposed to be provided via the cistern settling pit.



10.0 RECOMMENDATION

Based on the information presented in this report, we recommend that City of Ottawa approve this Servicing and Stormwater Management Report in support of the proposed mixed-use residential development at 3030 St Joseph Boulevard.

This report is respectfully being submitted for approval.

Regards,

Egis Canada Ltd.



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11.0 STATEMENT OF LIMITATIONS

This report was produced for the exclusive use of Theberge Developments Ltd. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Conservation and Parks, City of Ottawa and local approval agencies. Egis reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by Egis and site visits were performed, no field verification/measures of any information were conducted.

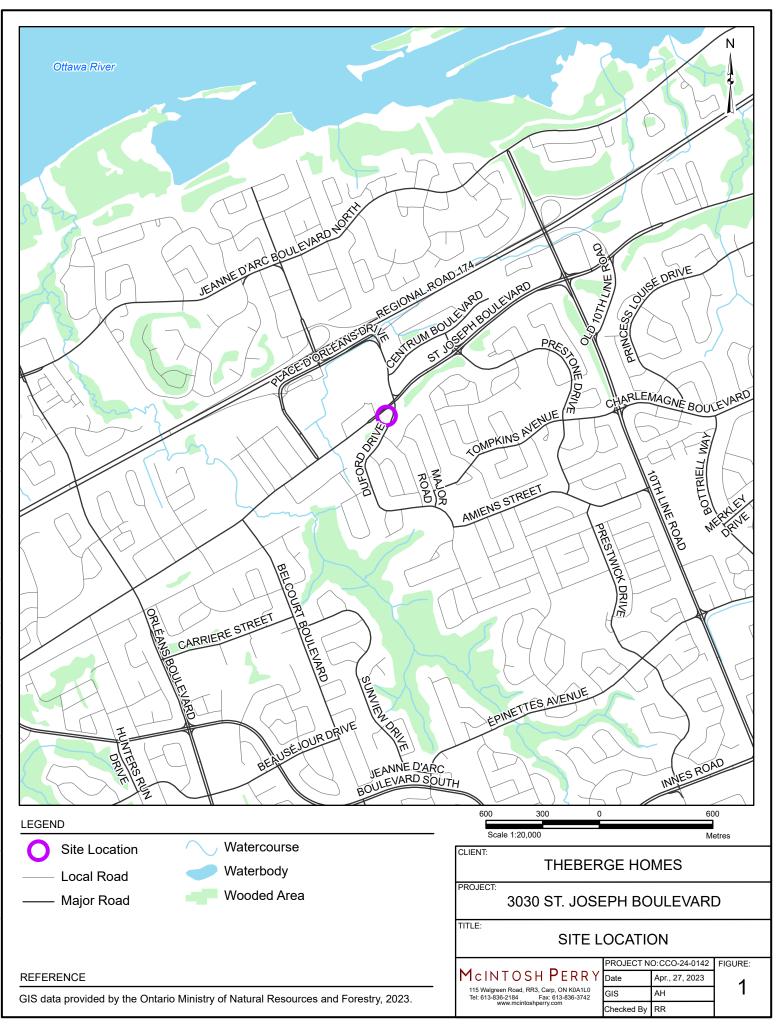
Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. Egis accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

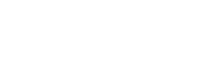
The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, Egis should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.



APPENDIX A KEY PLAN









APPENDIX B BACKGROUND DOCUMENTS From: Belan, Steve <<u>Steve.Belan@ottawa.ca</u>> Sent: Thursday, May 4, 2023 4:00 PM To: Scott Alain <<u>alain@fotenn.com</u>>; Tamara Nahal <<u>nahal@fotenn.com</u>> Cc: Belan, Steve <<u>Steve.Belan@ottawa.ca</u>>; Jeremy Silburt <<u>jeremy@thebergehomes.com</u>>; Joey Theberge <<u>joeytheberge@thebergehomes.com</u>>; Giles, Peter <<u>peter.giles1@ottawa.ca</u>>; Hassan, Selma <<u>Selma.Hassan@ottawa.ca</u>>; Polyak, Alex <<u>alex.polyak@ottawa.ca</u>>; McMahon, Patrick <<u>patrick.mcmahon@ottawa.ca</u>>; Wildman, Geraldine <<u>Geraldine.Wildman@ottawa.ca</u>> Subject: FW: Pre-con Follow-up - 3030 St Joseph - Zoning, OPA and Site Plan

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Scott and Tamara,

Please refer to the below and/or attached notes regarding the Pre-Application Consultation (pre-con) Meeting held on Wednesday March 22, 2023 for the property at 1887 St. Joseph Blvd for a Rezoning Application in anticipation of selling the property to a developer to construct building complex consisting mid and modest high-rise residential buildings with a private street and POP. I have also attached the required Plans & Study List for application submission.

Below or attached are staff's preliminary comments based on the information available at the time of pre-con meeting:

Planning

- Policies and provisions
 - The New Official Plan has designated the subject property as Hub, Evolving Neighbourhood and on a Major Collector and is in close proximity with the Place de Orleans Light Rail station. The secondary plan has recently been approved and been appealed and set out a similar development scenario for the site.
- The Official Plan Amendment will address policy concerns related to the ambiguity within the Official Plan pertaining to this site and the proposed development.
- Zoning will be specific to the proposed development on this site and a schedule will be employee to implement the building's terracing. A (-h) Hold Provision will be placed on the site until there is an approved Site Plan and an agreement to relocate the existing sewer which crosses the property.
- o Community benefits vs. the Section 37
 - The previous Zoning By-law Amendment required a Section 37 agreement be executed..
 - "Community Benefits Charge", <u>By-law No. 2022-307</u>, of 4% of the land value. This charge will be required for ALL buildings that are 5 or more storeys and 10 or more units and will be required at the time of building permit. Questions regarding this change can be directed to <u>Ranbir.Singh@ottawa.ca</u>.
 - Two ways to approach this: the previous Section 37 agreement can be amended to include the up-lift of the 2 additional storeys, with an exemption from paying the CBC. Or, we remove the current agreement and charge the entire CBC towards the development as assessed at the time of site plan approval and paid at the time of the Building permit.
- I have concerns that these application will stir up a similar response within the community as the original application made.
- For Zoning By-law Amendments the Applicant must now provide a proposed strategy for public consultation as directed by Bill 73

<u>Urban Design</u>

UDRP, Design Guidelines, Design Brief and Analyses

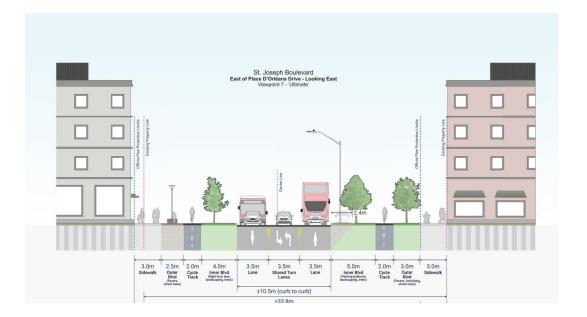
- 1. The site is subject to UDRP review. Given the question of height has been essentially determined via the SP and Zoning, the applicant can likely expect Panel attention focused on the building design, relationship to adjacent uses and to the public realm.
- 2. The City's Urban Design Guidelines for Highrise Buildings are applicable to the site. Given the UDRP review, the applicant should pay particular attention to these guidelines and be able to indicate how the design responds to these.
- 3. A Design Brief is required. A Terms of Reference for the Design Brief is attached; all elements highlighted in yellow must be addressed in written and / or graphic format in the Design Brief. Given the sloped site, the Design Brief needs to include a number of clear site sections that illustrate how the building is designed in consideration of the topography.
- 4. Shadow and Wind Analysis are required. The Terms of Reference for these are attached.
- 5. The Site and Landscape Plans need to clearly show:
 - a. The footprint of the underground parking
 - b. The location of all street lights, hydro poles and wires

Public Realm

6. The site is subject to the Orleans Corridor Study. This Study includes cross-sections for the evolution of the St. Joseph Blvd. ROW; these are shown in Annex B. The cross-section applicable to this site is shown below.

The proposed building setback makes it difficult to meet the vision for the ROW in the short to medium term. If the future sidewalk is built as part of this proposal, there is no soft surface area for street tree planting, until the ultimate ROW design is implemented. This is show on the first slide of the attached PDF "3030 St. Joseph frontage design". However, if the future sidewalk location is delayed and instead trees are planted, these trees will all have to be ripped out when the ultimate ROW is built.

The second slid of the PDF "3030 St. Joseph frontage design" illustrates a proposal that would see a slight deviation from the ROW design, but that meets the spirit of the design and allows street trees to be planted now that would remain in place through the ultimate ROW design. It would create a present and future condition that would enhance both the public realm and frontage of the building.



- 7. At the corner of St. Joseph and Duford, as well as going up the hill, the ROW design is predominantly hard surface and includes stairs and retaining walls. Urban Design has concerns with this. To help achieve OP goals policies related to the urban tree canopy and urban heat island, the area between the sidewalk and the property line should be soft surface and include street trees (planted 8 10m o/c depending on species).
- 8. As the applicant designs the amenity area / POPS on their site, special attention needs to be paid to ensuring clear site lines of all spaces so that no CEPTED concerns are raised.
- 9. Any elements that remain in the ROW (planter boxes, stairs, ramps, retaining walls etc.) will require a permanent encroachment agreement with the City. The encroachment agreement has required annual fees included on the Tax roll. The Site Plan Control agreement must include a condition related to the requirement for the encroachment agreement.
- 10. Similarly, a Maintenance and Liability Agreement (MLA) will also be required, in perpetuity, for any stairs, retaining walls, pavers etc. that remain in the ROW. The property owner will be responsible for repairs, snow clearing etc. The agreement must be registered on title. The Site Plan Control agreement must include a condition related to the requirement for the MLA.

Feel free to contact the Urban Design Planner, Selma Hassan, at <u>selma.hassan@ottawa.caa</u>, for follow-up questions.

Engineering

Site Plan Requirements

Required:

Water Boundary condition requests must include the location of the service and the expected loads required by the proposed development. Please provide the following information:

- Location of service connections (MAP provided)
- Type of development and the amount of fire flow required (as per FUS).

- Average daily demand: ____ l/s.
- Maximum daily demand: ____l/s.
- Maximum hourly daily demand: ____ l/s.

Submission Documents:

- Site Plan
- Topographical Plan of Survey Plan with a published Bench Mark
- Removals Plan
- Grading & Drainage Plan
- General Plan of Services
- Erosion & Sediment Control Plan
- Design Brief and Stormwater Management Report
- Geotechnical Report including a slope stability analysis

Design Criteria - Civil Engineer to contact me directly

- Suggest to prepare a concept servicing plan and share it with the City for comment prior to a detailed submission.
- o Stormwater Management requires runoff detention on site:
 - Post 100 year to Pre 5 year
- \circ The allowable release rate is to be computed using the lesser of C=0.5 or existing.
- Consider pedestrian Accessibilities at max 5%.
- o Quality control as per Rideau Valley Conservation Authority Requirements.
- Permissible ponding of 350mm for 100-year. No spilling to adjacent sites.
- At 100-year ponding elevation you must spill to the ROW.
- o 100-year Spill elevation must be 300mm lower than any building opening or ramp.

Minimum Drawing and File Requirements- All Plans

- Plans are to be submitted on standard **A1 size** (594mm x 841mm) sheets, utilizing an appropriate Metric scale (1:200, 1:250, 1:300, 1:400, or 1:500).
- With all submitted hard copies provide individual PDF of the DWGs and for reports please provide one PDF file of the reports. All PDF documents are to be unlocked and flattened.

Feel free to contact the Infrastructure Project Manager, Alex Polyak, at <u>Alex.Polyak@ottawa.ca</u>, for follow-up questions.

Transportation

- Follow Traffic Impact Assessment Guidelines
 - Ensure that the screening form is signed and stamped.
 - Proceed with the scoping report. Communication between the consultant and City is required at every step of the process.
 - The application cannot be deemed complete without the submission of the TIA Strategy. It is preferred to complete this step prior to application to allow time for potential RMA.
 - Ensure that the site triangle at Duford and St. Joseph is at least 5mx5m.
- Noise Impact Studies required for the following at Site Plan:
 - Road (proximity to St Joseph/Duford); and,

- Stationary (if there will be any exposed mechanical equipment due to the proximity to neighbouring noise sensitive land uses).
- The clear throat requirement for 100-200 apartment units accessing an arterial is 25m.
- On site plan:
 - Show all details of the roads abutting the site up to and including the opposite curb; include such items as pavement markings, accesses and/or sidewalks.
 - Turning templates will be required for all accesses showing the largest vehicle to access the site; required for internal movements and at all access (entering and exiting and going in both directions).
 - Show all curb radii measurements; ensure that all curb radii are reduced as much as possible.
 - The aisle width requirement is 6.7m.
 - Sidewalk is to be continuous across access as per City Specification 7.1.
- The site is within 600m of future Place d'Orleans LRT Station therefore TOD measures would apply. To achieve target mode shares within TOD zones, we highly recommend developments to provide as many TDM measures as possible.
- As the site proposed is residential, AODA legislation applies for all areas accessible to the public (i.e. outdoor pathways, parking, etc.).
- St. Joseph and Duford have protected rights of way of 37.5m and 24m, respectively according to the Official Plan. A widening might be required along the St. Joseph frontage, clearly show the ultimate property line as shown in the Orleans Corridor Secondary Plan, it appears that a 1m widening is required. Tree planting as shown on the concept plan would be temporary until the ultimate cross-section is constructed.
- While the location of the St. Joseph access is preferred as far from the intersection of St Joseph and Duford, as shown, it may require a median extension to prevent westbound and northbound left turns. This would be at the applicant's cost. Explore the possibility of providing access from Duford.
- Consider locating the bicycle parking at grade, ensure that the number of bicycle parking spaces are shown on the site plan.

Feel free to contact the Transportation Project Manager, Patrick McMahon, at [patrick.mcmahon@ottawa.ca], for follow-up questions.

Parkland

A new Parkland Dedication By-law, <u>By-law No. 2022-280</u>, was approved by Council on August 31, 2022. The By-law increases the required parkland conveyance for mid-rise and high-rise residential development and includes one-year transition policies for in-stream development and building permit applications or those that met the requirements for completeness by September 1, 2022.

City Surveyor

- The determination of property boundaries, minimum setbacks and other regulatory constraints are a critical component of development. An Ontario Land Surveyor (O.L.S.) needs to be consulted at the outset of a project to ensure properties are properly defined and can be used as the geospatial framework for the development.
- Topographic details may also be required for a project and should be either carried out by the O.L.S. that has provided the Legal Survey or done in consultation with the O.L.S. to ensure that the project is integrated to the appropriate control network.

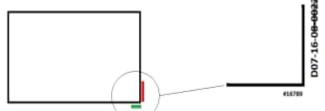
Questions regarding the above requirements can be directed to the City's Surveyor, Bill Harper, at <u>Bill.Harper@ottawa.ca</u>

Waste Services

- New multi-unit residential development, defined as containing six (6) or more units, intending to receive City waste collection services will be required, as of June 1, 2022, to participate in the City's Green Bin program in accordance with Council's approval of the <u>multi-residential waste</u> <u>diversion strategy</u>. The development must include adequate facilities for the proper storage of allocated garbage, recycling, and green bin containers and such facilities built in accordance with the approved site design. Questions regarding this change and requirements can be directed to <u>Andre.Laplante@ottawa.ca</u>.
- o A project of this size will be on private services and

<u>Other</u>

- Contact the Conservation Authority (RVCA) Office for their requirements
- Plans are to be standard A1 size (594 mm x 841 mm) or Arch D size (609.6 mm x 914.4 mm) sheets, dimensioned in metric and utilizing an appropriate Metric scale (1:200, 1:250, 1:300, 1:400 or 1:500).
- o All PDF submitted documents are to be unlocked, flattened and not saved as a portfolio file. .
- Please use the standard border (below) A0.1 Place on all plans; DWG # and D07 # as per sample



Use Bold Black text: Your Numbers are as per the colours listed here. DWG XXXXX (place number on the bottom right) D07 Number D07-12-23-xxxx.

- For information/question related to Development Charge, please contact AJ Mohmmand, Development Information Officer, Suburban East at <u>DIOCentrum@ottawa.ca</u> or 613-580-2424, ext. 29674
- For sites containing one or more buildings with a total GFA greater than 2000 square metres with more than 75 units
 - A Waste Reduction Workplan Summary is required for the construction project as required by O.Reg. 102/94, being "Waste Audits and Waste Reduction Work Plans" made under the Environmental Protection Act, RSO 1990, c E.19, as amended.
- You are encouraged to contact the Ward Councillor, Councillor Luloff, about the proposal.

Please refer to the links to <u>Guide to preparing studies and plans</u> and <u>fees</u> for further information. Additional information is available related to <u>building permits</u>, <u>development charges</u>, and the <u>Accessibility Design</u> <u>Standards</u>. Be aware that other fees and permits may be required, outside of the development review process. You may obtain background drawings by contacting <u>geoinformation@ottawa.ca</u>.

It is anticipated that, as a result of the *More Homes for Everyone Act, 2022*, for applications for site plan approval and zoning by-law amendments, new processes in respect of pre-application consultation will be put in place. The new processes are anticipated to require a multiple phase pre-application consultation approach before an application will be deemed complete. Applicants who have not filed a complete application by the effective date may be required to undertake further pre-application consultation(s) consistent with the provincial changes. The by-laws to be amended include By-law 2009-320, the Pre-Consultation By-law, By-law 2022-239, the planning fees by-law and By-law 2022-254, the Information and Materials for Planning Application By-law.

These pre-con comments are valid for one year. If you submit a development application(s) after this time, you may be required to meet for another pre-consultation meeting and/or the submission requirements may

change. You are as well encouraged to contact us for a follow-up meeting if the plan/concept will be further refined.

Please do not hesitate to contact me if you have any questions.

Regards,

Steve Belan, MCIP, RPP

Planner Planning Services, Development Review Services Planning, Corporate Real Estate and Economic Development department (PRED) City of Ottawa / Ville d'Ottawa 110 Laurier Avenue West, 4th Floor / 110, avenue Laurier Ouest, 4e étage Ottawa, ON K1P 1J1 Telephone / tél.: 613-580-2424 ext./poste 27591 E-mail / courriel: <u>Steve.Belan@ottawa.ca</u>

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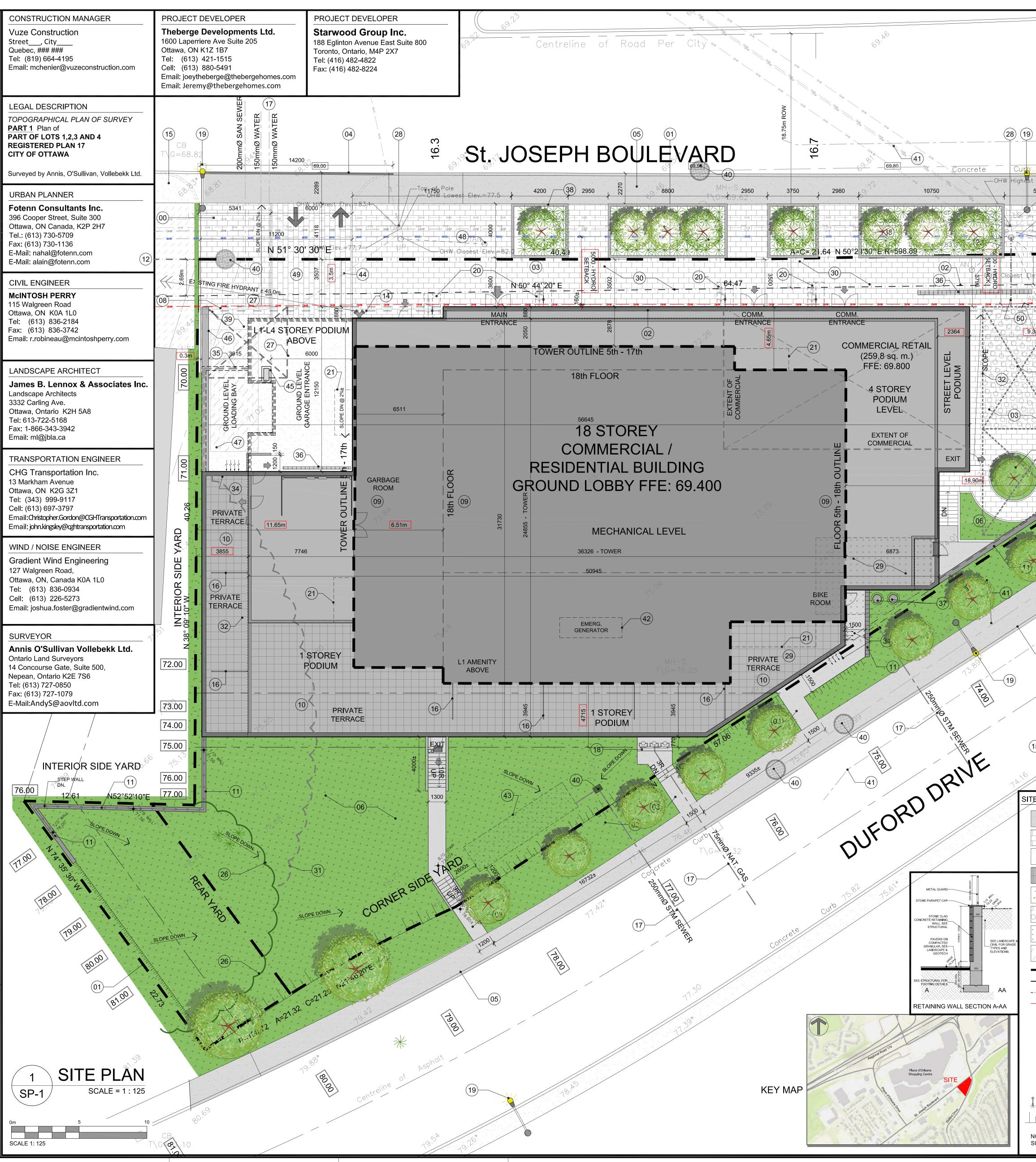
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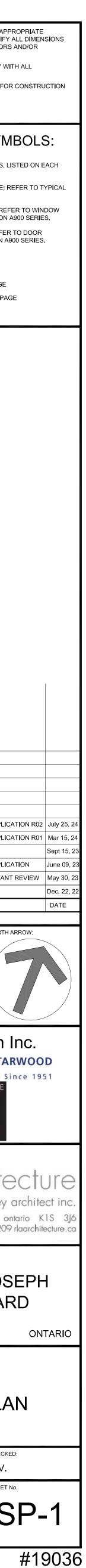
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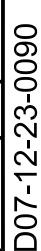


PLOT SCALE: 1:1

F:\2022\2222 - Duford Dr_3030 St Joseph Blvd\01 Design Development\01 Site Plan\2222 3030 St Joseph SP-1 2024 07 22.dwg

-1	PROJECT IN	FORMATION	IT IS THE RESPONSIBILITY OF THE APPF CONTRACTOR TO CHECK AND VERIFY A
6 ^{9.61}	Zoning By-law 2008-250 Consolidation AM3[2705] S438	SITE AREA 0.264 ha. 2,643.97 sq. (28,459) sq.	ft. OMISSIONS TO THE ARCHITECT.
	ZONING BUILDING HEIGHT	REQUIREDPROVIDE16 STOREY'S / 52.4m18 STOREY'S / 59.5	—
	ALLOWABLE PROJECTION - AMENITY LEVEL GRADE (GEODETIC ELEVATION - ASL)	0.0m 4.0 72.40m ASL 72.40m ASL	UNTIL SIGNED BY THE ARCHITECT.
	DENSITY - MAXIMUM FLOOR SPACE INDEX FRONT YARD SETBACK - AS PER SCHEDULE S438		.1 COPYRIGHT RESERVED.
	CORNER YARD SETBACK (Saint Joseph / Duford Avenue) INTERIOR SIDE YARD SETBACK	3.5m / 0.0m 3.5m / 0.3 0.0m 0.3	NOTATION SYM
OHM	AMENITY AREA - TOTAL PER UNIT - 6.0m ² AMENITY AREA - 50% COMMUNAL PER UNIT - 3.0m ²	1,212m ² 1,635r 606m ² 785r	n ² (00) INDICATES DRAWING NOTES, LIS
	RESIDENTIAL PARKING (AREA 'Z' - MIN. 0 / MAX. 1.75 PER UNIT) VISITOR PARKING - 0.1 PER UNIT AFTER 12 UNITS	MIN. 0 / MAX. 354 14	
	COMMERCIAL PARKING (AREA 'Z') BICYCLE PARKING - RESIDENTIAL - 0.5 PER UNIT	NOT REQUIRED	0 INDICATES WINDOW TYPE; REFE
$\frac{6}{CB}$	BICYCLE PARKING - COMMERCIAL - 1 PER 250m ² GFA AISLE & DRIVEWAY MINIMUM / MAXIMUM WIDTH	2 6.0m / 6.7m 6.0m / 6.7	8 INDICATES DOOR TYPE; REFER
t Elev.=84.3 5200 OHW, Elev.=82.1	ST. JOSEPH BOULEVARD FRONTAGE MINIMUM GROUND FLOOP		
5200 OHW Elev.= 82.1	=69.94 10. ¹ 10. ⁸	PROJECT STATISTICS	
OHW Etex = 77.4	онw онw онw онw онw онw они	GROSS BUILDING	DETAIL REFERENCE PAGE
	N OHW CHW OHW OHW OHW OHW	(GFA DEFINED PER SECTION 54, CITY OF OTTAWA'S BYLAWS DEFINITION, "GROSS FLOOR AREA") PARKING LEVELS (3 TYPICAL) N	Ά
		GROUND FLOOR 259.8 sq. (2,796) sq. (2,796) sq.	
		452.4 sq. 1st FLOOR (4,870) sq.	
	10	2nd FLOOR 1,042.5 sq. (11,221) sq. (11,221) sq.	
		3th & 4th FLOOR 2 x 1,042.5 sq. m. 2,085.9 sq. fl. 2 x (11,221) sq. ft. (22,442) sq. fl.	
	2283	5th to 17th FLOOR 13 x 714.0 sq. m. 13 x (7,685) sq. ft. 9,282.0 sq. (99,905) sq.	ft.
		18th FLOOR 552.0 sq. (5,941) sq.	
		MECHANICAL LEVEL N	_
		TOTAL AREA 13,674.6 sq. (147,192) sq.	
		UNIT STATISTICS	
		STUDIO UNIT	0
		1 BEDROOM + DEN UNIT	2 11
		2 BEDROOM UNIT 3 BEDROOM UNIT	50 4
	71.00	TOTAL 20	77
		COMMERCIAL RETAIL 190.0 sq. (2.250) sq. (2	
	(40)	(2,050) sq.	ι ι .
	41)		_
		REQUIRED by ZONING BY-LAW RESIDENCE - AREA 'Z' NONE REQUIRED	0
			20
		COMMERCIAL RETAIL (RETAIL FOOD, BANK & CONVENIENCE STORE) - AREA 'Z' NONE REQUIRED	0
122 4		TOTAL 2	20
	 PROPERTY LINE PROPOSED ROAD WIDENING 	PROVIDED	
T\G=72.15	3 HARD SURFACE PAVING, SEE LANDSCAPE PLAN	RESIDENCE - 0.6 PER UNIT 12	
(41)	4 CONTINUOUS DEPRESSED SIDEWALK THROUGH DRIVE	VISITOR - 0.1 PER UNIT (AFTER 12) 2 TOTAL 14	14
13.18	BARRIER CURB / SIDEWALK	LOCATION	6 ISSUED FOR OPA, ZA & SPC APPLICA
13. 3.8	 (6) SOFT LANDSCAPING, SEE LANDSCAPE PLAN (7) 6.0m WIDE GARAGE ENTRY RAMP WITH TRENCH DRAIN 	P3 LEVEL UNDER GROUND PARKING	5 ISSUED FOR OPA, ZA & SPC APPLICA
	8 OUTLINE OF BELOW GRADE PARKING GARAGE		$\begin{array}{c c} 4 \\ \hline 4 \\ \hline 3 \hline$
05)	9 OUTLINE OF UPPER FLOORS	GROUND FLOOR PARKING TOTAL	3 2 ISSUED FOR OWNER / CONSULTANT
	11 RETAINING WALL WITH GUARD RAIL AS REQUIRED WHERE ADJACENT GRADE IS >600mm BELOW T/O WALL		
12.0	EXISTING FIRE HYDRANT	BICYCLE PARKING	REVISIONS: — ARCHITECT SEAL: NORTH A
	13 AT GRADE VENTILATION GRILL (INTAKE / EXHAUST) (14) SIAMESE CONNECTION		BRIO ASSOC
	15 EXISTING CITY SIDEWALK	RESIDENCE- 0.5 PER UNIT (206 UNITS)10COMMERCIAL RETAIL- 1.0 PER 250m² OF G.F.A.	ARCHITECT SEAL: NORTH A
	16 PRIVACY SCREEN (17) PROPOSED SERVICES	TOTAL 10	15 ROBERICKI LAHEY
	(17) PROPOSED SERVICES(18) GAS EQUIPMENT LOCATION	PROVIDED	SEAL DATE: STAMP DATE
	(19) EXISTING STREET LIGHT / TRAFFIC POLE	EXTERIOR INDOOR ON L1 FLOOR 22	8 CLIENT:
	(20) BICYCLE PARKING SPACE WITH RACK	TOTAL 23	_I SL JUSEPH I
TE PLAN SYMBOLS	22 150mm WIDE BARRIER CURB - SEE CIVIL FOR HEIGHTS		Investment Builders Sin
CONCRETE SIDEWALK / WALK	 RE-INSTATE EXISTING BOARD FENCE 150mm WIDE BARRIER CURB 		
	25 PROPOSED TREES, SEE LANDSCAPE FOR DETAILS	PAVED SURFACE = 43.7 sq. m. 1.6 BUILDING FOOTPRINT = 1,618.9 sq. m. 61.2	
	26) EXISTING TREE TO REMAIN, PROTECT AS REQUIRED (27) WATER ENTRY IN P1 PARKING GARAGE	LANDSCAPE OPEN SPACE = 981.4 sq. m. 37.2 TOTAL = 2,643.97 sq. m. 100.0	HOMES
DRIVING SURFACE	28 HYDRO POLE WITH O/H WIRES		ARCHITECT:
UNIT PAVERS AT PRIVATE TERRACE	29 CISTERN AT GROUND FLOOR	AMENITY SPACE	rla/archite
SOFT LANDSCAPING	 (30) POSSIBLE COMMERCIAL PATIO (31) EXISTING TREE TO BE REMOVED 	AT GRADE COMMUNAL = 150.0 sq. GRND FLOOR AMENITY ROOM COMMUNAL = 175.0 sq. 101 FLOOR AMENITY ROOM COMMUNAL = 260.0 sq.	". roderick lahev o
AREA OF MAIN ELECTRICAL ROOM BELOW	32 LINE OF MAIN ELECTRICAL ROOM BELOW	1st FLOOR AMENITY ROOM COMMUNAL = 360.0 sq. 1 1st FLOOR TERRACE COMMUNAL = 0.0 sq. 1	n. 56 beech street, ottawa, on
	 (33) LINE OF SECONDARY ELECTRICAL ROOM BELOW (34) SIDE-WALL VENTILATION GRILL (INTAKE / EXHAUST) 	ROOF INTERIOR COMMUNAL =140.0 sq. 1ROOF TOP COMMUNAL =145.0 sq. 1	n
	35 LINE OF L1 PODIUM TERRACE ABOVE	PRIVATE TERRACE = 885.0 sq. 1 PRIVATE BALCONIES = 395.0 sq. 1	
AREA OF TEMP. LOADING FOR DELIVERIES	 36) TRENCH DRAIN, SEE CIVIL & MECHANICAL (37) ACCESS HATCHES FOR CISTERN @ L1 LEVEL. 	TOTAL = 2,250.0 sq. TOTAL COMMUNAL = 885.0 sq.	
	37 ASSESS HATCHEST OR CONTENT OF CHARGE FILE VEL. 38 150mm H. CURB FOR TREE PLANTER. SEE LANDSCAPE	REQUIRED - 6.0M ² PER UNIT (207) = 1,242.0 sq. REQUIRED COMMUNAL @ 50% = 621.0 sq.	
PROPOSED ROAD WIDENING PROPOSED HYDRO DUCT BANK LINES	AREA OF TEMPORARY LOADING FOR DELIVERIES (3.5m X 9.0m)		
LINE OF 5.0M HYDRO SETBACK	 (40) CATCH BASIN / MANHOLE - SEE CIVIL (41) RE-ROUTED EXISTING SERVICES - SEE CIVIL 	SOLID WASTE(207 UNITGARBAGE - COMPACTED- 0.11 PER UNIT11 YARE	
	APPROXIMATE LOCATION OF EMERGENCY GENERATOR	RECYCLING GMP- 0.018 PER UNIT4 YARERECYCLING FIBER- 0.038 PER UNIT8 YARE	os
ENTRANCE / EXIT DOOR	(43) LINE OF STORAGE CHAMBER BELOW. SEE CIVIL	COMPOST - 240L PER 50 UNITS	4
FH FIRE HYDRANT	$(43) \qquad \text{DRAWINGS FOR DETAILS}$		SITE PLA
• EXISTING TREES	 RELOCATED GROUND FLOOR COLUMN 		
SIAMESE CONNECTION	(46) 1200mm H. PRIVACY WALL		DRAWN: CHECKED
	 47 ***RESERVED*** 48 EXISTING GUY-WIRE TO BE RETAINED AND CONNECTED 		R.V. R.V.
♣ ★ AND STALE - SEE CIVIL & LANDSCAPE			SCALE: SHEET NO
NOTE: SEE LANDSCAPE FOR ALL	 (49) PROVIDE CONTRASTING COLOUR AND PATTERN FOR DRIVEWAY @ ENTRANCE. SEE LANDSCAPE FOR DETAILS (50) LINE OF HYDRO CONNECTION TO PRIMARY ELECTRICAL 		1:125 PROJECT NO.
SURFACE MATERIAL AND PATTERN	50 LINE OF HYDRO CONNECTION TO PRIMARY ELECTRICAL ROOM BELOW. SEE ELECTRICAL DRAWINGS		2222
1 Site Plan\2222 3030 St Joseph SP-1 2024	07 22.dwg		· · · ·











CCO-24-0142 - 3030 St Joseph Boulevard - Water Demands

Project:	3030 St Joseph Boulevard		
Project No.:	CCO-24-0142		
Designed By:	RRR		
Checked By:	CJM		
Date:	July 23, 2024		
Site Area:	0.26 gros	s ha	
Residential	NUMBER OF UNITS	UNIT RATE	
Single Family	hom	es 3.4	persons/unit
Semi-detached	hom	es 2.7	persons/unit
Townhouse	hom	es 2.7	persons/unit
Bachelor Apartment	<mark>0</mark> units	5 1.4	persons/unit
1 Bedroom Apartment	153 units	5 1.4	persons/unit
2 Bedroom Apartment	50 units	5 2.1	persons/unit
3 Bedroom Apartment	4 units	3.1	persons/unit
Average Apartment	unit	5 1.8	persons/unit
Total Population	332 pers	ons	
<u>Commercial</u>	190 m2		
<u>Industrial - Light</u>	m2		
Industrial - Heavy	m2		

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	
Residential	280	L/c/d	
Industrial - Light	35,000	L/gross ha/d	
Industrial - Heavy	55,000	L/gross ha/d	
Shopping Centres	2,500	L/(1000m² /d	
Hospital	900	L/(bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/(Space/d)	
Motels	150	L/(bed-space/d)	
Hotels	225	L/(bed-space/d)	
Tourist Commercial	28,000	L/gross ha/d	
Other Commercial	28,000	L/gross ha/d	
	Residential	1.08	L/s
AVERAGE DAILY DEMAND	Commercial/Industrial/		
	Institutional	0.01	L/s



MAXIMUM DAILY DEMAND

DEMAND TYPE	ļ A	AMOUNT	UNITS
Residential	3.34	x avg. day	L/c/d
Industrial	1.5	x avg. day	L/gross ha/d
Commercial	1.5	x avg. day	L/gross ha/d
Institutional	1.5	x avg. day	L/gross ha/d
	Residential	3.59	L/s
MAXIMUM DAILY DEMAND	Commercial/Industrial/		
	Institutional	0.01	L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	ļ	AMOUNT	UNITS
Residential	5.01	x avg. day	L/c/d
Industrial	1.8	x max. day	L/gross ha/d
Commercial	1.8	x max. day	L/gross ha/d
Institutional	1.8	x max. day	L/gross ha/d
	Residential	5.39	L/s
MAXIMUM HOUR DEMAND	Commercial/Industrial/		
	Institutional	0.02	L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	1.08	L/s
MAXIMUM DAILY DEMAND	3.60	L/s
MAXIMUM HOUR DEMAND	5.41	L/s



CCO-24-0142 - 3030 St Joseph Boulevard - Fire Underwriters Survey

Project:	3030 St Joseph Boulevard
Project No.:	CCO-24-0142
Designed By:	RRR
Checked By:	CJM
Date:	July 23, 2024

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.: City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE	REQUIREMENT (Rounded	to the nearest 1000 L	/min)				
F	= 220 x C x vA Where:		re flow in liters per minute				
			related to the type of construction.				
			loor area in square meters (including	all storey's, but excluding ba	sements at lea	ast 50 percent bel	low grade) in
		the building be	eing considered.				
	Construction	n Type Non-Combust	ble Construction				
		С	0.8		A 13,31	7.3 m ²	
					A	19.4 m ²	Desta da di Ventinal On enime
			Total Floor Area (per the 2020	FUS Page 20 - Total Effective	Area) 3,30	19.4 m ⁻	Protected Vertical Openings
Ca	alculated Fire Flow					4.9 L/min	
					10,00	0.0 L/min	
B REDU	ICTION FOR OCCUPANCY T	YPF (No Rounding)					
	JCTION FOR OCCUPANCY T						
	JCTION FOR OCCUPANCY T rom Page 24 of the Fire Unc Non-Combu	lerwriters Survey:		-25%			
	rom Page 24 of the Fire Uno	lerwriters Survey:		-25%			
Fr	rom Page 24 of the Fire Uno	lerwriters Survey:		-25%	7,50	0.0 L/min	
Fr Fi	rom Page 24 of the Fire Und Non-Combi	lerwriters Survey: ustible		-25%	7,50	10.0 L/min	
Fr Fi	rom Page 24 of the Fire Und Non-Combu ire Flow	derwriters Survey: Istible PE (No Rounding)		-25%	7,50	10.0 L/min	
Fr Fi	rom Page 24 of the Fire Und Non-Combi re Flow JCTION FOR SPRINKLER TYP	derwriters Survey: Istible PE (No Rounding)			7,50	0.0 L/min	
Fr Fi C. REDU	rom Page 24 of the Fire Und Non-Combi re Flow JCTION FOR SPRINKLER TYP	derwriters Survey: Istible PE (No Rounding)				0.0 L/min 0.0 L/min	
Fr Fi C. REDU Re	rom Page 24 of the Fire Und Non-Combi ire Flow JCTION FOR SPRINKLER TYF Fully Supervised Sprin eduction	derwriters Survey: ustible PE (No Rounding) klered					
Fr Fi C. REDU Re	rom Page 24 of the Fire Und Non-Combi ire Flow JCTION FOR SPRINKLER TYF Fully Supervised Sprin	derwriters Survey: ustible PE (No Rounding) klered		-50%	-3,75	0.0 L/min	
Fr Fi C. REDU Re	rom Page 24 of the Fire Und Non-Combi ire Flow JCTION FOR SPRINKLER TYF Fully Supervised Sprin eduction	derwriters Survey: ustible PE (No Rounding) klered ounding)	Cons.of Exposed Wall		-3,75 ed Heigt		nt
Fr C. REDU Re D. INCRI	rom Page 24 of the Fire Und Non-Combi Ire Flow JCTION FOR SPRINKLER TYf Fully Supervised Sprin eduction EASE FOR EXPOSURE (No R	derwriters Survey: ustible PE (No Rounding) klered ounding)	Cons.of Exposed Wall dinary - Mass Timber (Unprotected)	-50% Length Expose	-3,75 ed Heigt	0.0 L/min ht Length-Heigh	nt 0%
Fr Fi C. REDU Re	rom Page 24 of the Fire Und Non-Combi ICTION FOR SPRINKLER TYP Fully Supervised Sprin eduction EASE FOR EXPOSURE (No R Separation Distance (m	derwriters Survey: ustible PE (No Rounding) klered ounding))		-50% Length Expose Adjacent Wall (-3,75 ed Heigt (m) (Storio	<mark>0.0 L/min</mark> ht Length-Heigh es) Factor	
Fr C. REDU C. INCRI D. INCRI	rom Page 24 of the Fire Und Non-Combi ire Flow JCTION FOR SPRINKLER TYF Fully Supervised Sprin eduction EASE FOR EXPOSURE (No R Separation Distance (m Over 30 m	derwriters Survey: ustible PE (No Rounding) klered ounding)) Orn Orn Orn	dinary - Mass Timber (Unprotected)	-50% Length Expose Adjacent Wall (55	-3,75 ed Heigt (m) (Storia 2	0.0 L/min ht Length-Heigh es) Factor 110.0	0%

Increase*

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow Fire Flow Required**

 * In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

 ** In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

1,275.0 L/min

5,025.0 L/min 5,000.0 L/min



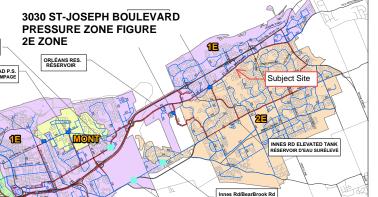
CCO-24-0142 - 3030 St Joseph Boulevard - Boundary Condition Unit Conversion

Project:	3030 St Joseph Boulevard
Project No.:	CCO-24-0142
Designed By:	RRR
Checked By:	CJM
Date:	July 23, 2024

Boundary Conditions Unit Conversion

St. Joseph Boulevard

Scenario	Height (m)	Elevation (m)	m H ₂ O	PSI	kPa
Avg. DD	114.1	69.4	44.7	63.6	438.5
Fire Flow (83 L/s or 5,000 L/min)	111.2	69.4	41.8	59.5	410.1
Peak Hour	109.4	69.4	40.0	56.9	392.4
Ground Elevation = 69.4m					



APPENDIX D SANITARY CALCULATIONS





CCO-24-0142 - 3030 St Joseph - Sanitary Demands

Project:	3030 St Joseph			
Project No.:	CCO-24-0142			-
Designed By:	R.R.R.			-
Checked By:	C.J.M.			-
Date:	July 23, 2024			-
				-
Site Area	0.26	Gross ha		_
1 Bedroom	153	1.40	Persons per unit	_
2 Bedroom	50	2.10	Persons per unit	-
3 Bedroom	4	3.10	Persons per unit	
Bachelor	0	1.40	Persons per unit	-
				-
Total Population	332	Persons	_	
Commercial Area	190.00	m ²		
Amenity Space	2250.00	m ²	_	
DESIGN PARAMETERS				
Institutional/Commercial Peaking Facto	1	*Check technical bulleting (Eith	her use 1.0 or 1.5)	
Residential Peaking Factor	3.45	* Using Harmon Formula = 1+		
0			ands, Harmon's Correction Factor	- = 0.8
Mannings coefficient (n)	0.013			
Demand (per capita)	280	L/day		
Infiltration allowance	0.33	L/s/Ha		
EXTRANEOUS FLOW ALLOWANCES			_	
	Infiltration / Inflow	Flow (L/s)		
	Dry	0.01		
	Wet	0.07	1	
	Total	0.09		
AVERAGE DAILY DEMAND				
DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	332	1.08
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)	2440.00	0.08
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0



AVERAGE RESIDENTIAL FLOW	1.08	L/s	
PEAK RESIDENTIAL FLOW	3.71	L/s	
AVERAGE ICI FLOW	0.08	L/s	
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.08	L/s	
PEAK INDUSTRIAL FLOW	0.00	L/s	
TOTAL PEAK ICI FLOW	0.08	L/s	

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	1.17	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	3.80	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	3.88	L/s

** PEAK INDUSTRIAL FLOW PER CITY OF OTTAWA SEWER DESIGN GUIDELINES APPENDIX 4B

SANITARY SEWER DESIGN SHEET

 PROJECT:
 CCO-24-0142

 LOCATION:
 3030 St. Joseph

	LOCA	ATION						RESIDENTIA	L							ICI AREAS				INFILTR	RATION ALLC	OWANCE	FLOW				SEWER DAT	ΓA		
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
					UNI	T TYPES		AREA	POPU	LATION		PEAK			ARE	A (ha)			PEAK	ARE	A (ha)	FLOW	DESIGN	CAPACITY	LENGTH	DIA	SLOPE	VELOCITY	AVAI	ILABLE
STREET	AREA I	D FROM MH	TO MH	BAC/1-	2-BED	3-BED		(ha)	IND	CUM	PEAK FACTOR	FLOW		JTIONAL		IERCIAL		STRIAL	FLOW	IND	CUM	(L/s)	FLOW	(L/s)	(m)	(mm)	(%)	(full) (m/s)		PACITY
		IVIH	IVIH	BED	-						FACTUR	(L/s)	IND	CUM	IND	CUM	IND	CUM	(L/S)				(L/S)					(m/s)	L/s	(%)
St Joseph Boulevard		BLDG	MH1A	153	50	4		0.26	332.0	332.0	3.45	3.71	1	0.00	0.24	0.24		0.00	0.08	0.26	0.26	0.09	3.88	34.22	3.00	200	1.00	1.055	30.34	88.67
		MH1A	EX. Sewer						0.0	332.0	3.45	3.71		0.00		0.24		0.00	0.08	0.00	0.26	0.09	3.88	34.22	16.85	200	1.00	1.055	30.34	88.67
Design Parameters:				Notes:							Designed:		RRR			No.					Revision							Date		
					ngs coefficie			0.013																						
Residential		ICI Areas		2. Deman	nd (per capita	a):	280	0 L/day																						
BAC/1-																														
BED 1.4 p/p/u			Peak Factor	3. Infiltra	tion allowan	ce:	0.33	3 L/s/Ha			Checked:		AM																	
2-BED 2.1 p/p/u	INST	28,000 L/Ha/day	1	4. Resider	ntial Peaking	Factor:																								
3-BED 3.1 p/p/u	COM	28,000 L/Ha/day	1		Harmon F	ormula = 1+(14/(4+P^0.5	6)*0.8)																						
Other 60 p/p/Ha	IND	35,000 L/Ha/day	MOE Chart		where P =	population i	n thousands	;			Project No	.:	CCO-24-01	42																
																												Sheet No:		
																												1 of 1		



SANITARY SEWER RELOCATION DESIGN SHEET

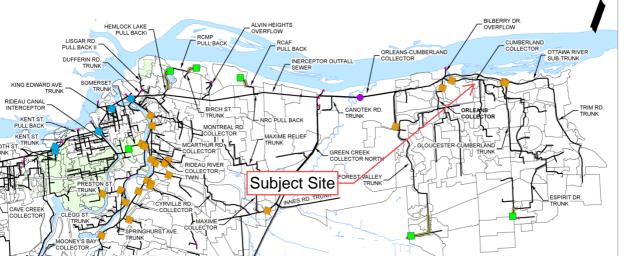
PROJECT:	
LOCATION:	

CCO-24-0142 3030 St. Joseph

	LOCATION						R	RESIDENTIA	L					ICI A	REAS			INFILTRA	TION ALLC	WANCE	FLOW				SEWER DAT	A		
1	2	3	4	5	6	7	8	9	10 11	12	13	14	15	16	17	18 19	20	21	22	23	24	25	26	27	28	29	30	31
					UNIT	TYPES		AREA	POPULATION		PEAK			AREA (ha)			PEAK	AREA	(ha)	FLOW	DESIGN	CAPACITY	LENGTH	DIA	SLOPE	VELOCITY	AVAI	
STREET	AREA ID	FROM	TO	BAC/1-	2-BED	3-BED		(ha)	IND CUM	PEAK	FLOW		JTIONAL	COMMERCI		INDUSTRIAL	FLOW	IND	CUM	(L/s)	FLOW	(L/s)	(m)	(mm)	(%)	(full)	CAPA	ACITY
		MH	MH	BED	Z-DED	J-DED		(114)		FACTOR	(L/s)	IND	CUM	IND C	UM	IND CUM	(L/s)	IND	COM	(E7.3)	(L/s)	(L/ 3)	(11)	((()))	(70)	(m/s)	L/s	(%)
St Joseph Blvd. Existing Downstream Capacity		MHSA4436	MHS20921																			118.97	12.02	450	0.16	0.725		
		MH2A	MH2B																			495.03	6.00	450	2.77	3.015		\square
—		MH2B	MH2C																			786.93	35.49	450	7.00	4.793		<u> </u>
		MH2C	MH2D																			767.59	21.77	450	6.66	4.675		
Proposed Alignment Duford Dr. to St Joseph Blvd.		MH2D	MH2E (TIE INTO EX. SEWER)																			458.86	39.54	450	2.38	2.795		
Design Parameters: Residential		ICI Areas			gs coefficien d (per capita)			0.013 L/day		Designed:		RRR		1	lo.				Revision			•				Date		
BAC/1- BED 1.4 p/p/u 2-BED 2.1 p/p/u		00 L/Ha/day	Peak Factor 1	4. Residen	itial Peaking	Factor:		L/s/Ha		Checked:		AM																
3-BED 3.1 p/p/u Other 60 p/p/Ha		00 L/Ha/day 00 L/Ha/day	I MOE Chart			rmula = 1+(1- population in		U.Ø)		Project No.	:	CCO-24-014	42															
ote: Capacity of propsed alignme etween MHSA4436 to MHSA2092	ent exceeds immediat 21. Due to the comple	te existing downsti exity of the downst	ream capcity			· · · ·				-,																Sheet No:		
e City will need to advise od any	downstream constra	aints.		1						1																1 of 1		



3030 St-Joseph Sanitary Trunk Figure





APPENDIX E PRE-DEVELOPMENT DRAINAGE PLAN



THE ORIGINAL TOPOGRAPHY, GROUND ELEVATION AND SURVEY DATA SHOWN ARE SUPPLIED FOR INFORMATION PURPOSES ONLY, AND IMPLY NO GUARANTEE OF ACCURACY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL

BOUNDARIES SHOWN HEREON HAVE BEEN DERIVED FROM INFORMATION SUPPLIED BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD. AND CANNOT BE RELIED UPON TO BE ACCURATE OR COMPLETE. THE PRECISE LOCATION OF THE CURRENT PROPERTY BOUNDARIES AND EASEMENTS CAN ONLY BE DETERMINED BY AN UP-TO-DATE LAND TITLES SEARCH AND A SUBSEQUENT CADASTRAL SURVEY PERFORMED AND CERTIFIED BY AN ONTARIO LAND SURVEYOR. THE BEARINGS DISPLAYED ARE ASTRONOMIC, DERIVED FROM PART OF THE EASTERLY LIMIT OF PLAN 50R-4499, SHOWN TO BE N 18°10'10"E. REFER TO THE

2. THIS PLAN IS NOT A CADASTRAL SURVEY SHOWING LEGAL PROPERTY BOUNDARIES AND EASEMENTS. THE PROPERTY

THE CONTRACTOR IS TO DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR LOCATION PLAN TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME ALL RESPONSIBILITY FOR EXISTING UTILITIES WHETHER OR NOT SHOWN ON THESE DRAWINGS. IF THERE IS ANY DISCREPANCY THE CONTRACTOR IS TO NOTIFY THE ENGINEER PROMPTLY. LEGEND 6. RESTORE ALL TRENCHES AND SURFACES OF PUBLIC ROAD ALLOWANCES TO CONDITION EQUAL OR BETTER THAN ORIGINAL BARRIER CURB & CURB DEPRESSION PROPOSED CONCRETE $\bigcirc^{\mathsf{MH}\#}$ STORM MANHOLE 8. TOPSOIL TO BE STRIPPED AND STOCKPILED FOR REHABILITATION. CLEAN FILL TO BE PLACED IN FILL AREAS AND COMPACTED TO CATCHBASIN, CURB INLET OR DITCH INLET SANITARY MANHOLE 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TRAFFIC CONTROL AND SAFETY MEASURES DURING THE CONSTRUCTION PERIOD, INCLUDING THE SUPPLY, INSTALLATION, AND REMOVAL OF ALL NECESSARY SIGNAGE, DELINEATORS, MARKERS AND – – – PROPERTY LINE EX. WATER VALVE/CHAMBER 12. ALL ROADWAY, PARKING LOT, AND GRADING WORKS TO BE UNDERTAKEN IN ACCORDANCE WITH CITY STANDARDS AND PROP. VALVE PER CITY W24 -0-13. CONTACT THE CITY FOR INSPECTION OF ROUGH GRADING OF PARKING LOTS, ROADWAYS AND LANDSCAPED AREAS PRIOR TO FIRE HYDRANT PLACEMENT OF ASPHALT AND TOPSOIL. ALL DEFICIENCIES NOTED SHALL BE RECTIFIED TO THE CITY'S SATISFACTION PRIOR TO PROPOSED WALL 14. ALL DIMENSIONS AND INVERTS MUST BE VERIFIED PRIOR TO CONSTRUCTION, IF THERE IS ANY DISCREPANCY THE CONTRACTOR PROPOSED SIAMESE CONNECTION PROPOSED WATER METER AND REMOTE METER M 🕅 PROPOSED CONCRETE 16. INSTALLATION TO BE IN ACCORDANCE WITH CURRENT CODES AND STANDARDS OF APPROVAL AGENCIES HYDRO ONE, BELL AND SIDEWALK PROP AND EX. REDUCER P U/G HYDRO DUCT PER ELEC. _____ ST_____ EX. STM SEWER 19. THIS PLAN MUST BE READ IN CONJUNCTION WITH THE GEOTECHNICAL INVESTIGATION BY PATERSON GROUP, REPORT NO. _____ SAN_____ EX. SAN SEWER 20. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTION PHASING AND LAYOUT. A PHASING PLAN SHALL BE ESTABLISHED FOR THE REALIGNMENT OF THE PROPOSED WATER AND SANITARY SERVICES TO ENSURE CONTINUAL SERVICE FOR THE OFF-SITE 3 ISSUED FOR REVIEW 2 ISSUED FOR SITE PLAN CONTROL ISSUED FOR SITE PLAN CONTROL Revisions Check and verify all dimensions before proceeding with the work SCALE 1:200

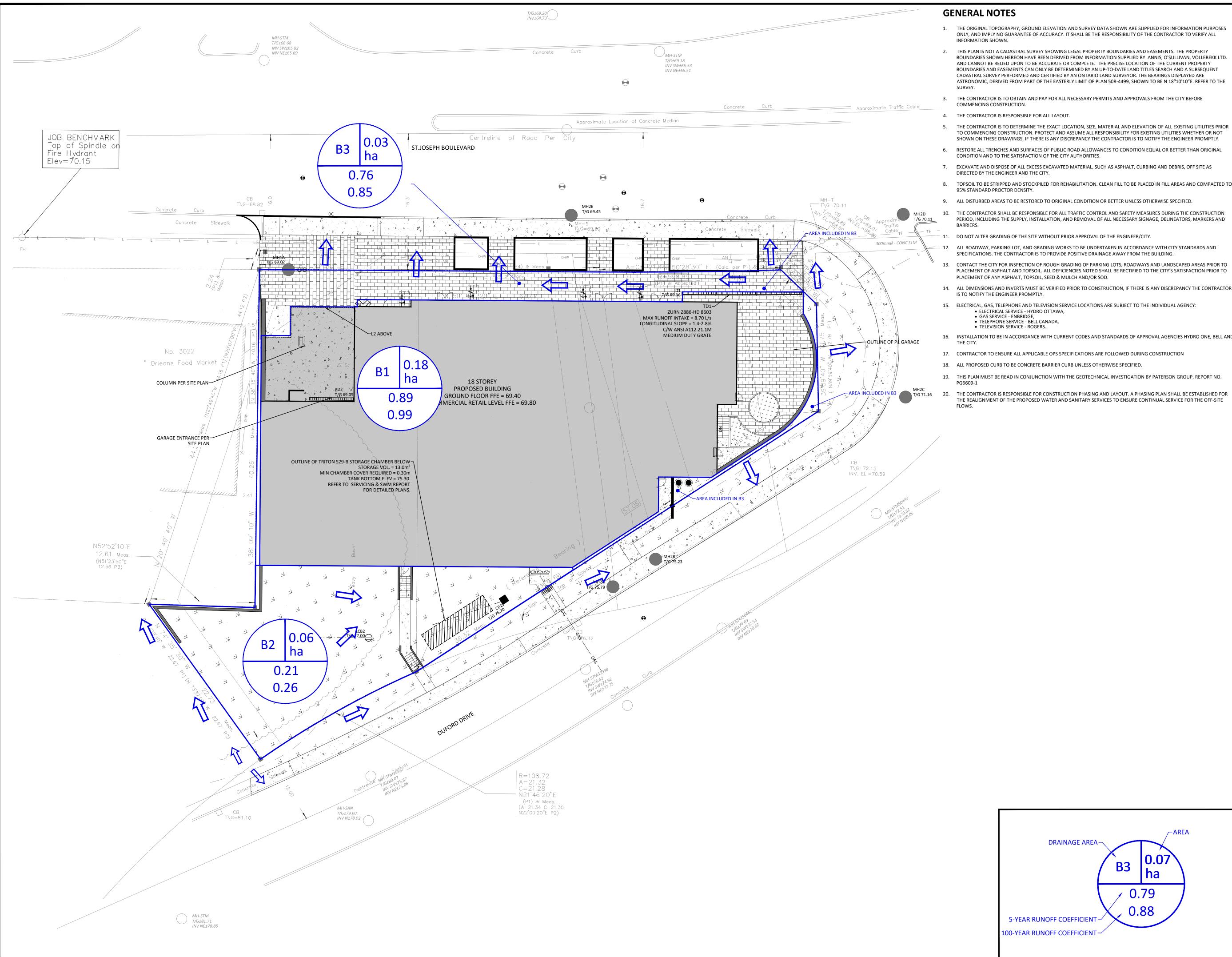
SLOPING AT 3:1 PROPOSED ELEVATION EXISTING ELEVATION ×95.94 93.20 $x_{(5)}^{95.94}$ SWALE ELEVATION TOP/BOTTOM WALL FACE ELEVATIONS ×^{T/w100.50} B/w90.50 PROPOSED EMERGENCY OVERLAND FLOW ROUT ⇒ POST-DEVELOPMENT DRAINAGE DIRECTIONS HEAVY DUTY SILT FENCE BARRIER PER OPSD 219.130 BUILDING ENTRANCE PROPOSED GRASS , , , , , — · — · — · — CENTRELINE OF SWALE OPOSED ROADCUT A REINSTATEMENT PER CIT CB SILT SACK PER C101 DETAIL \otimes 1 SERVICE/SEWER CROSSING EX. UG STREET LIGHT ----- TF ----- EX. TRAFFIC LINE _____ W____ EX. WATERMAIN — GAS — EX. GAS ____OHW___EX. OHW GAS CAR EX. TRAFFIC CABLE — BELL — EX. BELL FX. ROGERS _#==================== SUBJECT TO APPROVAL [™]♥===============≠≠[≠]¹ JUL 31, 2024 MAR 04, 2024 JUNE 02, 2023 Date Do not scale drawings 20 Metres 115 Walgreen Road, R.R.3 Carp, ON K0A 1L0 Tel: 613-836-2184 Fax: 613-836-3742 www.egis-group.com Client: THEBERGE DEVELOPMENTS LTD 1600 LAPERRIERE AVE OTTAWA, ON K1Z 8P5 Project: 3030 ST. JOSEPH BOULEVARD OTTAWA ON Drawing Title: PRE-DEVELOPMENT DRAINAGE PLAN 0 Project Number: Scale: 1:200 CCO-24-0142 Drawn By: RRR Checked By: Drawing Number: PRE Ő Designed By: RRR

1903

-AREA DRAINAGE AREA-0.07 **B3** ha 0.79 0.88 5-YEAR RUNOFF COEFFICIENT-**100-YEAR RUNOFF COEFFICIENT-**



APPENDIX F POST-DEVELOPMENT DRAINAGE PLAN



THE ORIGINAL TOPOGRAPHY, GROUND ELEVATION AND SURVEY DATA SHOWN ARE SUPPLIED FOR INFORMATION PURPOSES ONLY, AND IMPLY NO GUARANTEE OF ACCURACY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL

BOUNDARIES SHOWN HEREON HAVE BEEN DERIVED FROM INFORMATION SUPPLIED BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD. AND CANNOT BE RELIED UPON TO BE ACCURATE OR COMPLETE. THE PRECISE LOCATION OF THE CURRENT PROPERTY BOUNDARIES AND EASEMENTS CAN ONLY BE DETERMINED BY AN UP-TO-DATE LAND TITLES SEARCH AND A SUBSEQUENT CADASTRAL SURVEY PERFORMED AND CERTIFIED BY AN ONTARIO LAND SURVEYOR. THE BEARINGS DISPLAYED ARE ASTRONOMIC, DERIVED FROM PART OF THE EASTERLY LIMIT OF PLAN 50R-4499, SHOWN TO BE N 18°10'10"E. REFER TO THE

2. THIS PLAN IS NOT A CADASTRAL SURVEY SHOWING LEGAL PROPERTY BOUNDARIES AND EASEMENTS. THE PROPERTY

LOCATION PLAN LEGEND BARRIER CURB & CURB DEPRESSION SLOPING AT 3:1 PROPOSED CONCRETE PROPOSED ELEVATION EXISTING ELEVATION ×95.94 93.20 O^{MH#} $x_{(5)}^{95.94}$ SWALE ELEVATION STORM MANHOLE TOP/BOTTOM WALL FACE ELEVATIONS ×^{T/w100.50} B/w90.50 CATCHBASIN, CURB INLET OR DITCH INLET PROPOSED EMERGENCY OVERLAND FLOW ROUTE ⇒ SANITARY MANHOLE POST-DEVELOPMENT DRAINAGE DIRECTIONS — — — PROPERTY LINE HEAVY DUTY SILT FENCE EX. WATER BARRIER PER OPSD 219.130 VALVE/CHAMBER PROP. VALVE PER CITY W24 BUILDING ENTRANCE PROPOSED GRASS , , , , , FIRF HYDRANT PROPOSED WALL — · — · — · — CENTRELINE OF SWALE PROPOSED SIAMESE CONNECTION OPOSED ROADCUT EINSTATEMENT PER CIT PROPOSED WATER METER AND REMOTE METER M 🕅 CB SILT SACK PER C101 DETAIL \otimes PROPOSED CONCRETE SIDEWALK (1)SERVICE/SEWER CROSSING PROP AND EX. REDUCER EX. UG STREET LIGHT ----- TF ----- EX. TRAFFIC LINE P U/G HYDRO DUCT PER ELEC. EX. WATERMAIN _____ ST_____ EX. STM SEWER — GAS — EX. GAS _____ SAN_____ EX. SAN SEWER ____OHW___EX. OHW GAS CAR EX. TRAFFIC CABLE — BELL — EX. BELL FX. ROGERS ▲★============== SUBJECT TO APPROVAL [™]♥===============≠≠[≠]¹ 3 ISSUED FOR REVIEW JUL 31, 2024 MAR 04, 2024 2 ISSUED FOR SITE PLAN CONTROL JUNE 02, 2023 ISSUED FOR SITE PLAN CONTROL Date Revisions Check and verify all dimensions Do not scale drawings before proceeding with the work SCALE 1:200 20 Metres 115 Walgreen Road, R.R.3 Carp, ON K0A 1L0 Tel: 613-836-2184 Fax: 613-836-3742 www.egis-group.com Client: THEBERGE DEVELOPMENTS LTD 1600 LAPERRIERE AVE OTTAWA, ON K1Z 8P5 Project: 3030 ST. JOSEPH BOULEVARD OTTAWA ON Drawing Title: POST-DEVELOPMENT DRAINAGE PLAN \bigcirc Project Number: Scale: 1:200 CCO-24-0142 Drawn By: RRR Checked By: Drawing Number: AM

-AREA DRAINAGE AREA-0.07 **B3** ha 0.79 0.88 5-YEAR RUNOFF COEFFICIENT-**100-YEAR RUNOFF COEFFICIENT-**

Designed By:

RRR

Õ

APPENDIX G STORMWATER MANAGEMENT CALCULATIONS





CO-24-0142 - 3030 St.Joseph Boulevard - SWM Calculations

Tc		nsity n/hr)		
(min)	5-Year	100-Year		
10	104.2	178.6	PRE-DEVELOPMENT	Imperviou
10	104.2	178.6	POST-DEVELOPMENT	Gravel

C-Values				
Impervious	0.90			
Gravel	0.60			
Pervious	0.20			

Pre-Development Runoff Coefficient

Drainage	Impervious	Gravel	Pervious Area	Average C	Average C
Area	Area (m ²)	(m²)	(m²)	(5-year)	(100-year)
A1	0	0	2,643	0.20	0.25

Pre-Development Runoff Calculations

Drainage	Area	C	C	Тс	Q (L/s)
Area	(ha)	5-Year	100-Year	(min)	5-Year	100-Year
A1	0.26	0.20	0.25	10	15.31	32.79
Total	0.26				15.31	32.79

Post-Development Runoff Coefficient

Drainage Area	Impervious Area (m²)	Gravel (m²)	Pervious Area (m²)	Average C (5-year)	Average C (100-year)
B1	1,781	0	18	0.89	0.99
B2	67	0	501	0.28	0.34
B3	219	0	56	0.76	0.85

Post-Development Runoff Calculations

Drainage	Area	C	С	Тс	Q (L/s)	
Area	(ha)	5-Year	100-Year	(min)	5-Year	100-Year	
B1	0.180	0.89	0.99	10	46.54	88.64	Restricted
B2	0.057	0.28	0.34	10	4.65	9.54	Restricted
B3	0.028	0.76	0.85	10	6.04	11.58	Unrestricted
Total	0.26				57.22	109.75	

Required Restricted Flow

Drainage	Area	С	Тс	Q (L/s)
Area	(ha)	5-Year	(min)	5-Year
A1	0.26	0.20	10	15.31

Post-Development Restricted Runoff Calculations

Drainage Area	Unrestricted Flow (L/S)		Restricted Flow (L/S)		Storage Re	quired (m ³)	Storage Pro	ovided (m ³)
Alea	5-year	100-Year	5-Year	100-Year	5-Year	100-Year	5-Year	100-Year
B1	46.54	88.64	1.26	2.40	55.0	102.3	102.3	102.3
B2	4.65	9.54	1.10	1.30	2.4	6.5	13.0	13.0
B3	6.04	11.58	6.04	11.58				
Total	57.22	109.75	8.40	15.28	57.48	108.79	115.26	115.26

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m³

CO-24-0142 - 3030 St.Joseph Boulevard - SWM Calculations

Storage Requirements for Area B1									
5-Year Storm	5-Year Storm Event								
Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)				
200	13.0	5.81	1.26	4.55	54.56				
210	12.6	5.63	1.26	4.37	55.04				
220	12.1	5.40	1.26	4.14	54.71				
230	11.7	5.23	1.26	3.97	54.73				
240	11.3	5.05	1.26	3.79	54.54				

Maximum Storage Required 5-year =

100-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
190	22.9	11.37	2.40	8.97	102.24
200	22.0	10.92	2.40	8.52	102.26
210	21.1	10.47	2.40	8.07	101.74
220	20.4	10.13	2.40	7.73	102.00
230	19.7	9.78	2.40	7.38	101.84
240	19.0	9.43	2.40	7.03	101.26
250	18.4	9.13	2.40	6.73	101.01
260	17.8	8.84	2.40	6.44	100.41
270	17.3	8.59	2.40	6.19	100.25
280	16.8	8.34	2.40	5.94	99.79
Ν	Aavimum Sto	100	m ³		

Maximum Storage Required 100-year = 102 m³

5-Year Storm Event Storage Summary

	Storage Available (m ³) = 102.3	,
	Storage Required (m ³) = 55.0	
~		_

100-Year Storm Event Storage Summary

Storage Available (m ³) = 102.3	*
Storage Required (m ³) = 102.3	

*Available Storage from internal cistern

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CO-24-0142 - 3030 St.Joseph Boulevard - SWM Calculations

Storage Requirements for Area B2 5-Year Storm Event							
Tc (min)	l (mm/hr)	Runoff (L/s) B2	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)		
5	141.2	6.30	1.10	5.20	1.56		
10	104.2	4.65	1.10	3.55	2.13		
15	83.6	3.73	1.10	2.63	2.37		
20	70.3	3.13	1.10	2.03	2.44		
25	60.9	2.72	1.10	1.62	2.42		

Maximum Storage Required 5-year =

100-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B2	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
5	242.7	12.96	1.30	11.66	3.50
15	142.9	7.63	1.30	6.33	5.70
25	103.8	5.54	1.30	4.24	6.37
35	82.6	4.41	1.30	3.11	6.54
45	69.1	3.69	1.30	2.39	6.46
55	59.6	3.18	1.30	1.88	6.22
65	52.6	2.81	1.30	1.51	5.89
75	47.3	2.53	1.30	1.23	5.52
85	43.0	2.30	1.30	1.00	5.08
95	39.4	2.10	1.30	0.80	4.59
Ν	Aaximum Sto	rage Require	d 100-year =	6.5	m ³

Storage Available in Triton S-29B Storage Tank* (m3)=

 $13 (m^3)$

m³

Refer to manufacturer's specification sheet in Appendix G for full storage tank details/specificaitons.

5-Year Storm Event Storage Summary

2	Storage Available (m ³) = 13.0
	Storage Required (m ³) = 2.4

100-Year Storm Event Storage Summary

Storage Available (m ³) = 13.0
Storage Required (m ³) = 6.5

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CO-24-0142 - 3030 St.Joseph Boulevard - SWM Calculations

Time of Concentration Pre-Development									
Drainage Area	Sheet Flow	Slope of	Tc (min)	Tc (min)					
ID	Distance (m)	Land (%)	(5-Year)	(100-Year)					
A1	44	10.00	9	9					

Therefore, a Tc of 10 can be used

4 of 4

Tc= (3.26(1.1-c)L^0.5/S^0.33)

c = Balanced Runoff Coefficient

L = Length of drainage area

S = Average slope of watershed

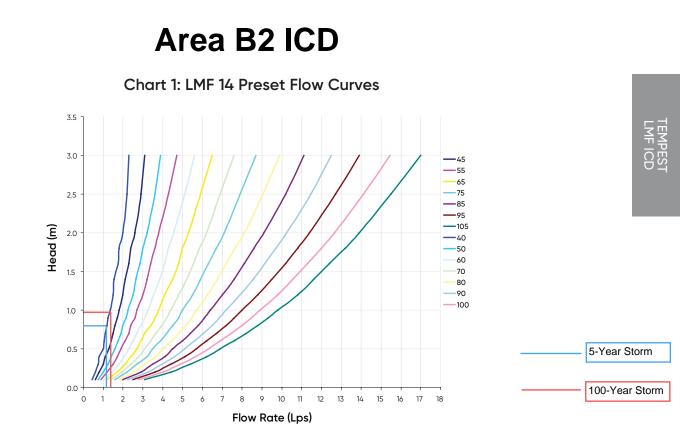
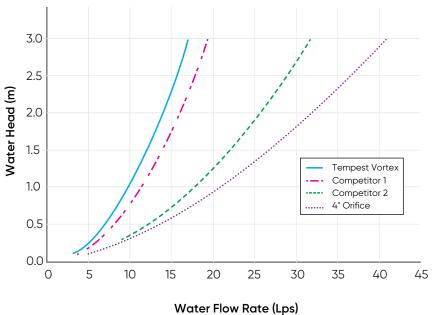


Chart 2: LMF Flow vs. ICD Alternatives



STORM SEWER DESIGN SHEET

 PROJECT:
 CCO-24-0142

 LOCATION:
 3030 St. Joseph

	LOCA	ATION		C	CONTRIBUTING AREA (ha) RATIONAL DESIGN FLOW									RATIONAL DESIGN FLOW									SEWER DATA				
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
STREET	AREA ID	FROM MH	TO MH	C-VALUE	AREA	INDIV AC	CUMUL AC	INLET (min)	TIME IN PIPE	TOTAL (min)	i (5) (mm/hr)	i (10) (mm/hr)	i (100) (mm/hr)	5yr PEAK FLOW (L/s)		100yr PEAK FLOW (L/s)	FIXED FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA	PIPE SIZE (mm) W) H	SLOPE (%)	VELOCITY (m/s)	AVAIL C. (L/s)	AP (5yr) (%)
								()		()	((,	((_,)	()				(/	((2, 3)	()
Duford Drive	B1	BLDG	EX. 600mm STM Sewer	0.89	0.18	0.16	0.16	10.00	0.38	10.38	104.19	122.14	178.56	46.54	54.56	79.75		46.54	62.04	27.57	250			1.00	1.224	15.50	24.98%
	B2	LCB2	CB1	0.28	0.06	0.02	0.02	10.00	0.09	10.09	104.19	122.14	178.56	4.65	5.45	7.96		7.96	62.04	6.27	250			1.00	1.224	54.08	87.17%
		CB1	EX. 600mm STM Sewer			-	0.02	10.09	0.16	10.25	103.74	121.61	177.78	4.63	5.42	7.93		7.93	62.04	11.87	250			1.00	1.224	54.11	87.22%
Definitions:				Notes:				Designed:					No.					Revision							Date		
Q = 2.78CiA, where:				1. Mannings coef	ficient (n) =		0.013	-					1.														
Q = Peak Flow in Litres	per Second (L/s)			_				RRR																			
A = Area in Hectares (ha								Checked:																			
i = Rainfall intensity in																											
[i = 998.071 / (TC+6.0		5 YEAR						AM																			
[i = 1174.184 / (TC+6		10 YEAR						Project No.:																			
[i = 1735.688 / (TC+6	.014)^0.820]	100 YEAR																ite:							Sheet No:		
				1				CCO-24-0142									2024	.07.25							1 of 1		





APPENDIX H CITY OF OTTAWA DESIGN CHECKLIST

Parameters

Units: Metric

Storage Volume: 12 Cu m

Chamber Selection: S-29B

Header Row Position: Left

Fill Over Embedment Stone: 300 mm

Controlled By: width 3.00 m

Embedment Stone mm:

Over: 150 Under: 150 Porosity: 0.4

Min 150mm over and under

Double Stacked

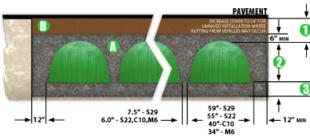
Double Stacked?: No

Stone Between:

Note: After making an input change you must hit calculate to update the Field Diagram and Project Results.

* The image generation will not save if using MicroSoft Edge

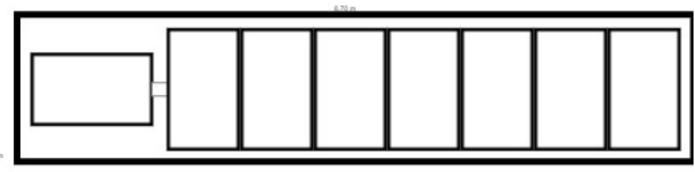
Project Results

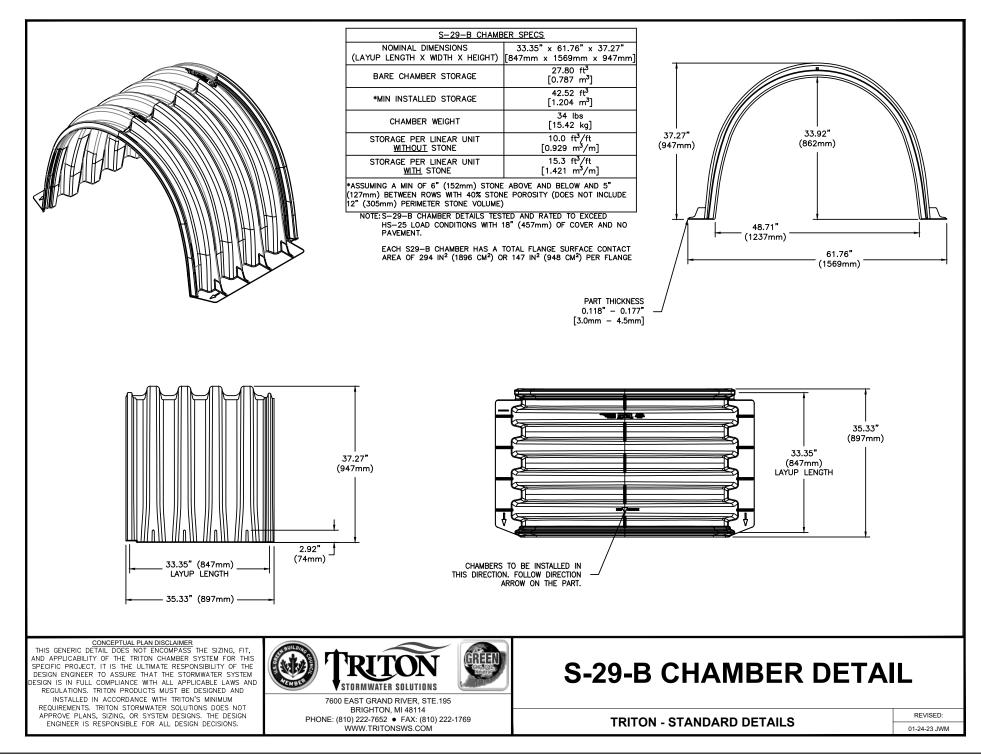


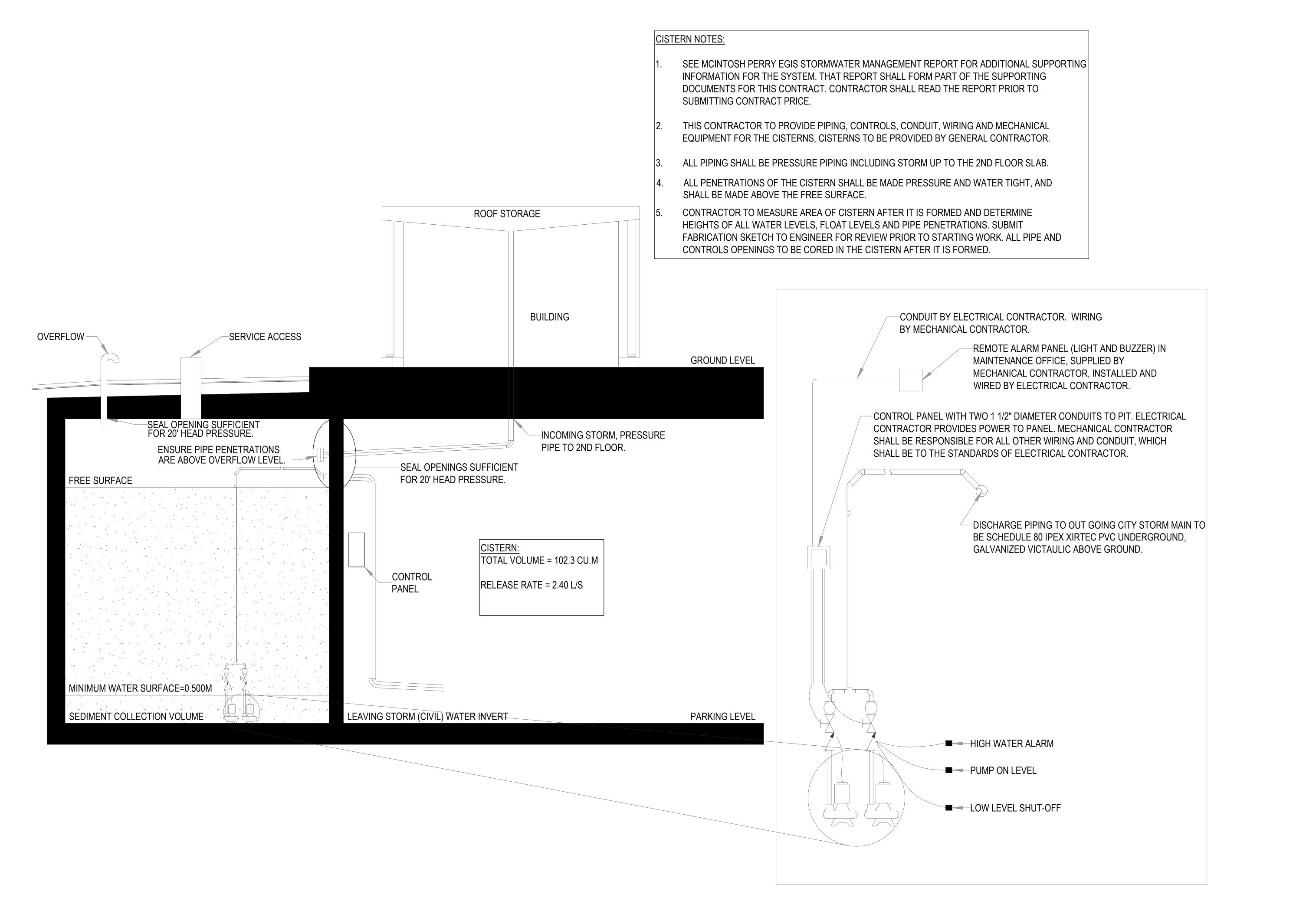
Total Cover Over Chambers: 301 mm
Peight Of Chamber: 947 mm
Embedment Stone Under Chambers: 151 mm
Volume of Embedment Stone Required: 17 Cu. m
Olume of Fill Material Required: 6 Cu. m
Total Storage Provided: 14 Cu. m
Type Of Chambers: S-29B
Of Chambers Required: 8
Of End Caps Required: 4
Required Bed Size: 19 Sq. m
Volume of Excavation: 24 Cu. m
* Area of Filter Fabric: 47 Sq. m
of Chambers Long: 7
of rows: 1
Actual Trench Length: 8.70 m

Actual Trench Width: 2.18 m

* Filter Fabric quantity for Fabric on Top and Sides of System Only, does not include overlap









SCALE: N.T.S.

City of Ottawa

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

Criteria	Location (if applicable)
Executive Summary (for larger reports only).	N/A
Date and revision number of the report.	On Cover
Location map and plan showing municipal address, boundary, and layout of proposed development.	Appendix A
□ Plan showing the site and location of all existing services.	Site Servicing Plan (C102)
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and	1.1 Purpose
watershed plans that provide context to which individual developments must adhere.	1.2 Site Description
	6.0 Stormwater Management
Summary of pre-consultation meetings with City and other approval agencies.	Appendix B
Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments,	1.1 Purpose
Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and	1.2 Site Description
develop a defendable design criteria.	6.0 Stormwater Management
□ Statement of objectives and servicing criteria.	3.0 Pre-Consultation Summary



 Identification of existing and proposed infrastructure available in the immediate area. 	N/A
Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Site Grading Plan (C101)
Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Site Grading Plan (C101)
Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
Proposed phasing of the development, if applicable.	N/A
Reference to geotechnical studies and recommendations concerning servicing.	Section 2.0 Background Studies, Standards and References
 All preliminary and formal site plan submissions should have the following information: Metric scale North arrow (including construction North) Key plan Name and contact information of applicant and property owner Property limits including bearings and dimensions Existing and proposed structures and parking areas Easements, road widening and rights-of-way Adjacent street names 	Site Grading Plan (C101)



4.2 Development Servicing Report: Water

Criteria	Location (if applicable)
Confirm consistency with Master Servicing Study, if available	N/A
 Availability of public infrastructure to service proposed development 	N/A
Identification of system constraints	N/A
Identify boundary conditions	Appendix C
Confirmation of adequate domestic supply and pressure	N/A
 Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. 	Appendix C
Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	N/A
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A
Address reliability requirements such as appropriate location of shut-off valves	N/A
Check on the necessity of a pressure zone boundary modification.	N/A
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Appendix C, Section 4.2



Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Site Servicing Plan (C101)
Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Appendix C
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N/A

4.3 Development Servicing Report: Wastewater

Criteria	Location (if applicable)
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	N/A
Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
 Description of existing sanitary sewer available for discharge of wastewater from proposed development. 	Section 5.2 Proposed Sanitary Sewer



 Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable) 	Section 5.3 Proposed Sanitary Design
Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	N/A
Description of proposed sewer network including sewers, pumping stations, and forcemains.	Section 5.2 Proposed Sanitary Sewer
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
Special considerations such as contamination, corrosive environment etc.	N/A



4.4 Development Servicing Report: Stormwater Checklist

Criteria	Location (if applicable)
 Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property) 	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
□ Analysis of available capacity in existing public infrastructure.	N/A
A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Pre & Post-Development Plans
□ Water quantity control objective (e.g. controlling post- development peak flows to pre-development level for storm events ranging from the 2 or 5-year event (dependent on the receiving sewer design) to 100-year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Set-back from private sewage disposal systems.	N/A
□ Watercourse and hazard lands setbacks.	N/A
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
 Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5-year return period) and major events (1:100-year return period). 	Appendix G



Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Site Grading Plan
Calculate pre-and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 7.0 Proposed Stormwater Management Appendix G
Any proposed diversion of drainage catchment areas from one outlet to another.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post- development flows up to and including the 100-year return period storm event.	N/A
□ Identification of potential impacts to receiving watercourses	N/A
Identification of municipal drains and related approval requirements.	N/A
 Descriptions of how the conveyance and storage capacity will be achieved for the development. 	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
100-year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Grading Plan (C101)
Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A



 Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors. 	Section 8.0 Sediment & Erosion Control
Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

Criteria	Location (if applicable)
 Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act. 	N/A
 Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. 	N/A
Changes to Municipal Drains.	N/A
 Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) 	N/A



4.6 Conclusion Checklist

Criteria	Location (if applicable)
Clearly stated conclusions and recommendations	Section 9.0 Summary
	Section 10.0 Recommendations
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	All are stamped
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	All are stamped

