

## Assessment of Adequacy of Servicing Report – Rev2

2545 9th Line Road Metcalfe, Ontario

ASB Greenworld Ltd February 2, 2024

→ The Power of Commitment



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#### 1. Introduction

### 1.1 Purpose of this Report

GHD Limited (GHD) was retained by ASB Greenworld Limited (ASB or "the Client") to complete this Assessment of Adequacy of Servicing Report in support of ASB's acquisition and future occupation of the property located at 2545 9<sup>th</sup> Line Road, in Metcalfe, Ontario (Site or Property). This report is required as part of ASB's Zoning By-law Amendment (ZBLA) application being submitted to the City of Ottawa.

It is understood that ASB initially proposes to use the operational portion of the Site for storage and distribution of garden products. Additional planning and studies may be required based on future development and increased operations to be implemented by ASB, as applicable.

This report presents the various services available at the Site including but not limited to water supply, stormwater management, and septic systems. It is noted that no municipal services are currently provided at the Site. This report summarizes the details from Site inspections and studies/calculations for the services provided at the Site.

#### 1.2 Scope and Limitations

This report: has been prepared by GHD for ASB Greenworld Ltd and may only be used and relied on by ASB Greenworld Ltd for the purpose agreed between GHD and ASB Greenworld Ltd.

GHD otherwise disclaims responsibility to any person other than ASB Greenworld Ltd arising in connection with this report. GHD also excludes implied warranties and conditions, to the extent legally permissible.

The services undertaken by GHD in connection with preparing this report were limited to those specifically detailed in the report and are subject to the scope limitations set out in the report.

The opinions, conclusions and any recommendations in this report are based on conditions encountered and information reviewed at the date of preparation of the report. GHD has no responsibility or obligation to update this report to account for events or changes occurring subsequent to the date that the report was prepared.

The opinions, conclusions and any recommendations in this report are based on assumptions made by GHD described in this report. GHD disclaims liability arising from any of the assumptions being incorrect.

#### **Accessibility of documents**

If this report is required to be accessible in any other format, this can be provided by GHD upon request and at an additional cost if necessary.

## 2. Site Description

#### 2.1 Location

The Site has the municipal address of 2545 9th Line Road and is located roughly 500 metres (m) north of the intersection of Victoria Street and 9th Line Road Street in Metcalfe, Ontario which is within the City of Ottawa limits. The Site fronts onto 9th Line Road on the west side and is surrounded by agricultural and forest lands on all sides. The operational portion of the Site covers an area of approximately 14.3 hectares (ha) and is currently developed with several buildings and warehouses, and asphalt and gravel parking areas. The remainder of the Property is surrounded by agricultural fields and forested areas, for a total property area of approximately 40.1 ha.

A Site Location Map and a Site Plan are provided on Figure 1 and Figure 2, respectively.

#### 2.2 Site Characteristics

The Site is relatively flat with local topography sloping radially outward from the central developed area. Mapping indicates topographic relief is on the order of 10 m across the Site. Based on a review of historical aerial imagery, the built portion of the Site has been present at the Site since prior to 1976 with the majority of the current buildings being developed at that time. Buildings consist of an office building on the west side, vacant mushroom buildings, along the west and several operational buildings in the central part of the Site. Approximately 15 percent impervious cover consists of parking areas, driveway areas, and buildings, with the remainder of the Site consisting of approximately 85 percent pervious cover made up of crop lands, forests, lawns and vegetated fields.

Surface water from the Site is drained by an intermittent tributary to the North Castor River. The north branch of the tributary originates west of 9th line and flows through the north part of the Site, then south and leaves the property at the east part of the property. The south branch of the tributary appears to originate near the south part of the Site and flows east where it confluences with the north branch within an unevaluated wetland prior to flowing north along the east part of the Site.

Along 9th Line Road stormwater generally sheet flows over very gently sloped lawns towards 9th Line Road and is drained by road side vegetated swales north/south to the tributaries. Similarly on the north side stormwater sheetflows over lawn areas to the north branch of the tributary. Along the east side stormwater drains via sheetflow and then via some vegetated swales to the north tributary. Along the south side stormwater sheetflows over lawn areas to forest areas eventually to the south branch of the tributary.

## 3. Services Capacity

### 3.1 Water Supply

As part of GHD's Hydrogeological Assessment Report (dated December 8, 2022; also submitted with ZBLA application), GHD observed three drilled water supply wells on the Site to be used for ASB operations. Two drilled wells were located within well pits to the north of the office building (TW-1 and M-1; 0.15 m diameter wells – To supply water to office and former mushroom building) and one drilled well was located above grade within a pump house near the central storage building (TW-2; 0.15 m diameter well – To supply water to other auxiliary buildings and fire water supply). All three (3) wells were fitted with well seals to facilitate venting, piping and electrical. The location of the three water supply wells is presented on **Figure 2**. A fourth water supply well was located to the east of the Donut Factory building on the northern portion of the Site but was not assessed as part of this report.

Well records were not provided to GHD for TW-1, TW-2 or M-1 and GHD was not able to definitively match any of the MECP database well records to the Site wells. The publicly available MECP well records indicate construction dates in the 1970s, prior to Ontario Regulation 903. The well records indicate that the wells installed and mapped to the Site were constructed with 0.15 m diameter steel casings that were installed to depths of 6 m or greater. Ontario Regulation 903 requires that the well owner must maintain the well at all times after the completion of the well's structural stage in a way that prevents surface water and other foreign materials from entering the well. Based on our observations it is recommended that the wells to be utilized in the future be fitted with pit-less adapters and the casing extended above the ground surface and a vermin proof cap installed. Future changes or alternations to the wells will need to be completed in accordance with Ontario Regulation 903. Wells that are not being used or maintained for future use should be abandoned in accordance with Ontario Regulation 903.

It is understood that ASBs current proposed water usage will be for the office building (kitchen and bathrooms) and warehouse/storage buildings with no processing on-Site that would require water usage. Water usage would therefore be related to general cleaning, washroom or kitchen purposes. Staffing is proposed to consist of 2-5 employees to start with a potential of up to 10-15 employees. In reference to Section 8 of the Ontario Building Code, subsection 8.2.1.3. – Sewage System Design Flows, the water usage for a warehouse with 15 staff, three loading bays and 260 square metres (m²) of office space would be on the order of 2,550 litres per day (L/day). Designs flows

are conservative in nature with actual daily usage typically two to three times less. **Table 3.1** provides the calculations that were utilized to estimate the maximum daily water usage of the proposed development.

Table 3.1 Water Usage

Volume (L)	Establishment	Quantity	Water Usage (L)
75	Office Building: per each 9.3 m <sup>2</sup> of floor space	260 m <sup>2</sup> /9.3 m <sup>2</sup> ≈ 28	2,100
150	Warehouse: Per loading bay	3	450
		TOTAL:	2,550

Based on the results of the hydrogeological assessment (GHD, December 2022), the pumped water wells (TW-1 and TW-2) had sufficient water capacity as follows:

- TW-1 | After 6 hours of pumping at a rate of approximately 26.5 litres per minute (L/min), the maximum drawdown was about 2.4 m over the course of the testing with about 37.0 m of available drawdown remaining above the bottom of the well. Approximately 6 percent of the available drawdown was used during the pumping test. Recovery measurements were collected manually for 60 minutes after pumping ceased, with the water level recovering approximately 65 percent in 1 hour and approximately 80 percent in 24 hours. The estimated transmissivity of the pumped water well was 33.6 m²/day based on the drawdown and 12.0 m²/day based on the recovery period and represents a moderate transmissivity. The specific capacity for this well is calculated to be 11.1 L/min/m based upon the pumping test completed.
- TW-2 | After 6 hours of pumping at a rated of approximately 26.5 L/min, the maximum drawdown was about 0.5 m over the course of the testing with about 88.3 m of available drawdown remaining above the bottom of the well. Approximately 0.5 percent of the available drawdown was used during the pumping test. Recovery measurements were collected manually for 60 minutes after pumping ceased, with the water level recovering approximately 76 percent in 1 hour and fully recovered 100 percent in 4 hours and 50 minutes. The estimated transmissivity for TW-2 was 83.9 m²/day based on the drawdown and 186.5 m²/day based on the recovery period and represents a high transmissivity. The specific capacity for this well is calculated to be 52.9 L/min/m based upon the pumping test completed.

TW-2 is to remain to provide water to buildings and fire water supply. TW-1, M-1 and any other wells not to be used should be abandoned in accordance with Ontario Regulation 903.

### 3.2 Fire Water Supply

The Site is serviced with an approximately 200 cubic metre (m³) (200,000 L) concrete basin inside the Fire Water Building located in the centre of the Site (refer to Figure 2). Water for the basin is supplied from water supply well TW-2, with an assumed pumping capacity of 75 L/min. Assuming a typical fire truck can spray approximately 950 to 3,785 L/min, the estimated time before the basin would empty is approximately 3.8 to 0.9 hours. Refer to **Appendix C** for applicable Fire Water Estimate Calculations.

Reference is made to the OFM – TG-03-1999: Fire Protection Water Supply Guideline for Part 3 of the Ontario Building Code. A one-storey building not exceeding  $600 \text{m}^3$  requires a minimum flow of 1,800L/min (476gpm) as per Table 2 minimum water supply flow rates. The minimum fire protection water supply is required to provide the minimum flow rate for a duration of 30 minutes. Based on the fire water estimate calculation provided in Appendix C the tank can supply 1,800L/min for 1.8 hrs.

### 3.3 Septic System

The Site is serviced with existing traditional septic tanks/pump chambers and subsurface disposal beds. It is understood that two septic systems are located on the Site, with one located to the north of the office building and the second located east/southeast of the former mushroom building. The septic systems were inspected by a licensed contractor, Green Valley Environmental (GVE), with their findings presented in a letter report dated October 25, 2022

(refer to **Appendix A**), with applicable notes referenced below. GHD also contacted the Ottawa Septic System Office (OSSO) which provided septic records for the Site (**refer to Appendix B**).

#### Office Building Septic System

It is understood that a licenced septic contractor will design and install a replacement system. The GVE report indicates that a larger septic tank and septic bed may be required. The septic system design for the system should be provided to the Ottawa Septic System Office (OSSO) to support the rezoning of the property and should demonstrate that suitable location for the septic system can be found.

The design should take into account a number of possible constraints:

- a. setbacks to existing wells as noted in the report, and
- b. shallow overburden/thin soils which may be potentially hydrogeological sensitive, and
- c. setbacks to any watercourse crossing the property (to the north and east of existing septic bed)

#### Former Mushroom Building Septic System

The second septic system is dedicated to the former Mushroom Building and consists of two septic tanks to the east of the building (connected to two separate bathrooms) and a septic bed to the southeast of the building. The septic tanks had broken concrete lids and inside walls, and the partition walls were rotted. ASB is not intending to use the Former Mushroom Building and associated bathrooms at this time, and upon future demolition of the building these septic tanks should also be decommissioned. The OSSO should be contacted to initiate the decommission process for the mushroom building septic system. Septic system needs for future development will be planned/approved as required.

#### 3.4 Stormwater

The management of stormwater under current conditions is described in Section 2.2 above. The majority of the site drainage flows over large tracts of pervious lawns/agricultural lands and swales prior to entering intermittent tributaries that flow through the Site. Stormwater drainage patterns and amounts are anticipated to have remained similar to present conditions for several decades. As the majority of the Site is pervious and drainage patterns and stormwater quantity will remain the same as it has for decades and the receiving watercourses flow through several low-lying areas, we anticipate no concerns with stormwater servicing capacities.

Water quality is anticipated to be of good quality as the majority of the operations will involve inside storage of materials. The minor amounts of road wear from trucks and loading equipment is anticipated to be managed with flow over existing lawns and vegetated swales, which will remove the majority of any particulates in the stormwater.

## 4. Environmental Compliance Approvals

Current ECAs associated with the Site include Amended Certificate of Approval (C of A) No. 6569-6DRHP2 for Industrial Sewage Works issued to Continental Mushroom Corporation in 2005, which is for a wastewater recovery and re-use system including a series of ponds and pumps for the former mushroom operations. A copy of the ECA is presented in Appendix D. ASB is in the process of having this document transferred under their company name, and will comply with the terms and conditions listed regarding operations & maintenance, sludge handling, and record keeping. Although limited to no use of the current wastewater system are required or planned at this time, ASB will not operate this wastewater system till ECA transfer is approved and system has been fully maintained pending ASB's purchase of the Property.

ASB will be sending future applications for ECA amendment(s) to the MECP for review and approval for any changes to Industrial Sewage Works at the Site, including list of equipment/systems to be decommissioned and any new equipment/processes to be installed as part of current proposed Phase 1 and 2 activities (compost transfer facility operations). In consultation with GHD, ASB is also evaluating possible requirements for an ECA specific to compost transfer facility operations. Any ECA application to the MECP will include applicable site condition studies, details on equipment and processes to be installed/decommissioned, and applicable design & operation plans.

## 5. Signatures

We trust this report meets your immediate needs. Should any questions arise regarding any aspect of our report, please contact our office.

All of Which is Respectfully Submitted,

**GHD** 

Steve Gagne, H.S.Bc.

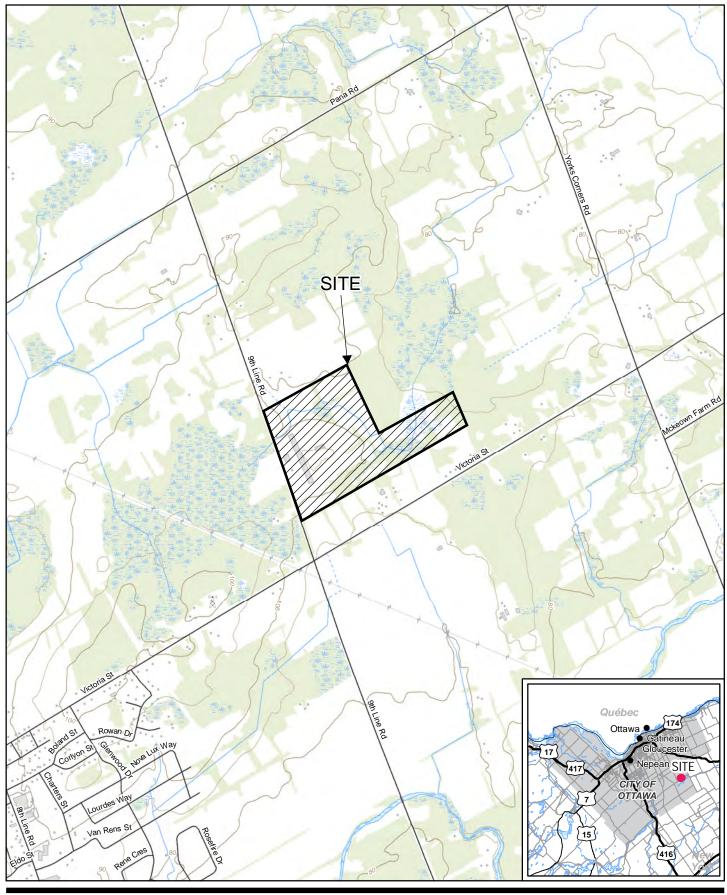
Associate, Project Director

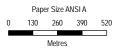
bogh R Oradon

Joseph Drader, P.Eng.



# Figures





Map Projection: Transverse Mercator Horizontal Datum: North American 1983 Grid: NAD 1983 UTM Zone 18N

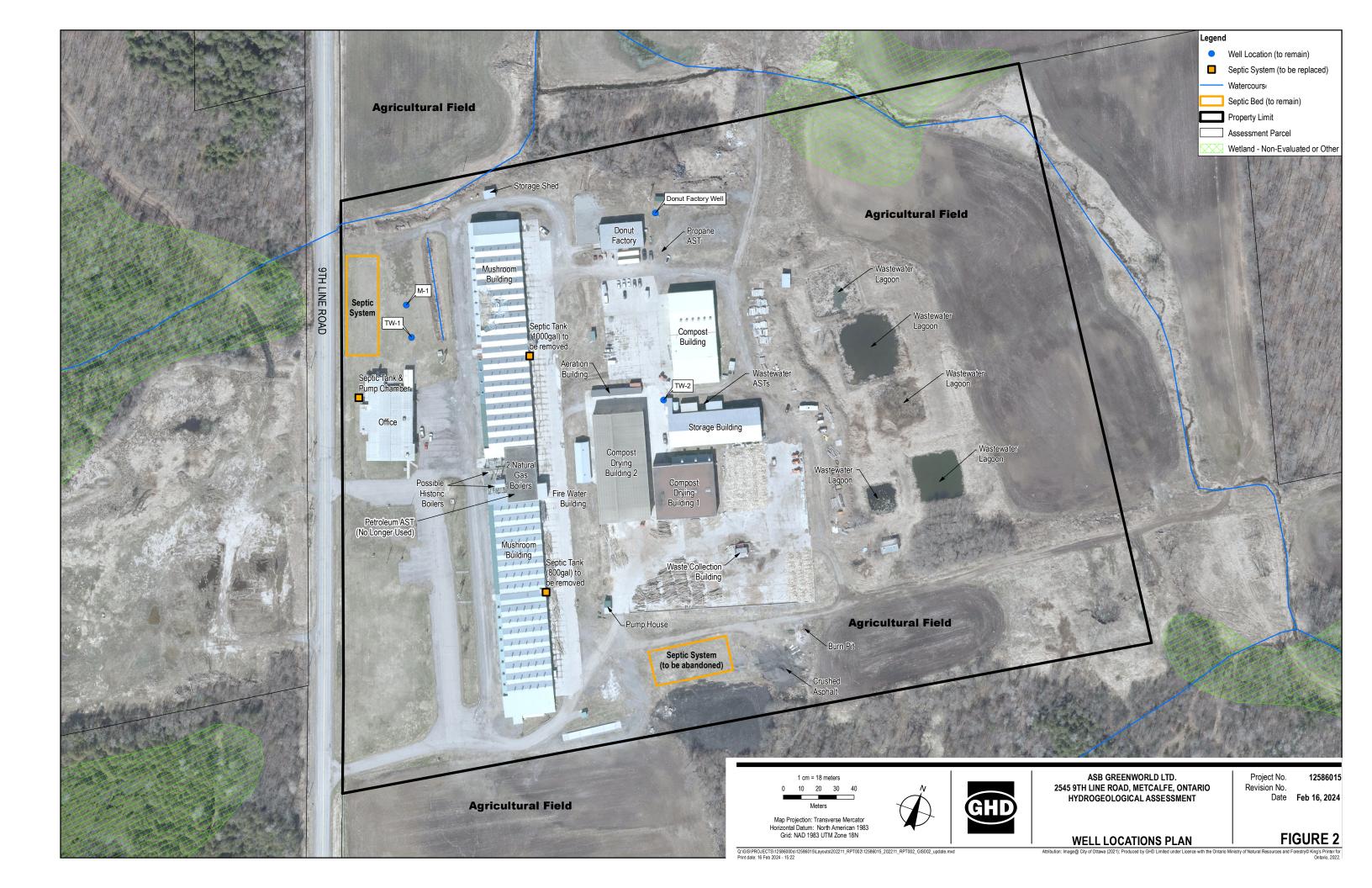


ASB GREENWORD LTD. 2545 9TH LINE ROAD, METCALFE, ONTARIO

Project No. 12586015 Revision No.

Date **Dec 15**, 2022

FIGURE 1



## Appendices

# Appendix A

**Green Valley Environmental Septic Inspection** 



October 25, 2022

Re: Septic System Inspection Report

Property: 2545 9th Line Rd, Metcalfe, ON K0A 2P0

Dear Joseph Draper,

Further to your request, this firm has carried out septic tank pump-out and an evaluation of the existing sewage systems servicing the Office building and the Mushroom building. The purpose of these work has been to carry out a field investigation to determine the current condition of the sewage systems, to visually inspect the disposal field area, pump chamber, septic tanks and to report on any unsafe conditions and/or signs of any systems failing.

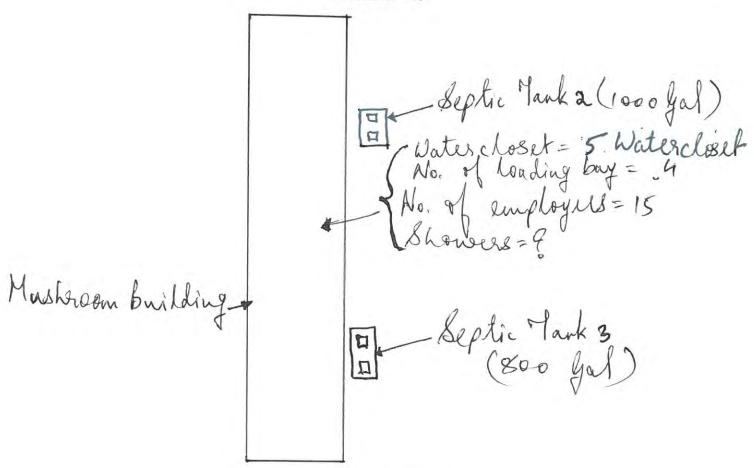
Attached is a report on each of the systems servicing the Office building and Mushroom building. The systems are identified as per attached site layout labeled by GHD. Summary of findings as follows:

Office Building Septic System: Here, septic system consists of septic bed with 4 runs of 30m each, septic tank is 800gal (3600L) and pump chamber (200 gal) with pump. Four test holes were dug on the septic bed and as per our findings, there was no bio-mat build up found and no standing water found which means the septic bed condition is good. Septic tank was pumped-out at the time of inspection and it was filled with gravels on the bottom. Septic tank needs a repair and needs risers/lids. Pump chamber connection to the septic bed is unknown and needs a new pump if it is connected to the septic bed. The existing septic system is not to the current building code as the daily design flow of the Office Building (assuming 15 employees and 1200 sq. m. of floor space) is 9675 L/day and it will need an 11,500 L septic tank along with the treatment unit, pump chamber with pump, distribution box and a shallow buried trench bed with 12 runs of 28.34m each (assuming the soil type is Clay) to meet the building code requirements. See sketch #1 on the inspection report and images of existing septic system attached to it.

Mushroom Building: Here, septic system consists of two septic tanks (1000gal and 800gal) and a septic bed. Septic tanks were pumped-out at the time of inspection. Septic tanks had broken concrete lids, inside walls and partition walls are rotted. It is recommended to have the septic tanks replaced with new ones and meet the current building code requirements. Septic bed was not inspected as per the instructions provided at the time of inspection. The daily design flow of the Mushroom Building (assuming 5 water closet and 4 loading bays) is 5350 L/day and it will need a 6000 L (or two 3600L) septic tank/s along with the treatment unit, pump chamber with pump, distribution box and a shallow buried trench bed with 7 runs of 26.16m each (assuming the soil type is Clay) to meet the building code requirements.

Regards,
Davis Patel
Qualified Septic System Designer
(BCIN: 119685)

SKETCH - 2

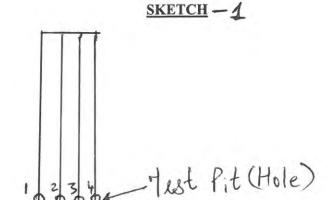


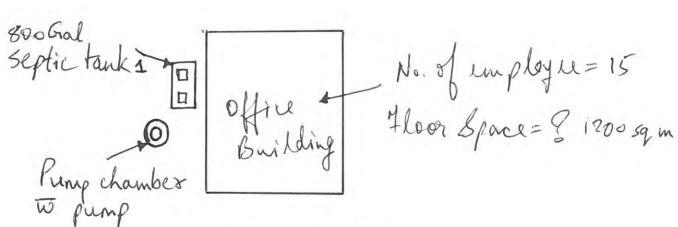
#### TESTHOLE DESCRIPTION

\* Septic Yanks 1 fz: Not good condition.

\* Septic tanks are notted from inside and broken lids. Needs Replacement.







TESTHOLE DESCRIPTION

- \* 1,2,3,4 Test holes looks good. No bio-mat found
- \* Septic Tank needs repair on the inside and new risers and lids required.
- \* Pump Chambers not in good condition.
  Unknown connection of the pump chamber/pump
  to the septic Bed.



















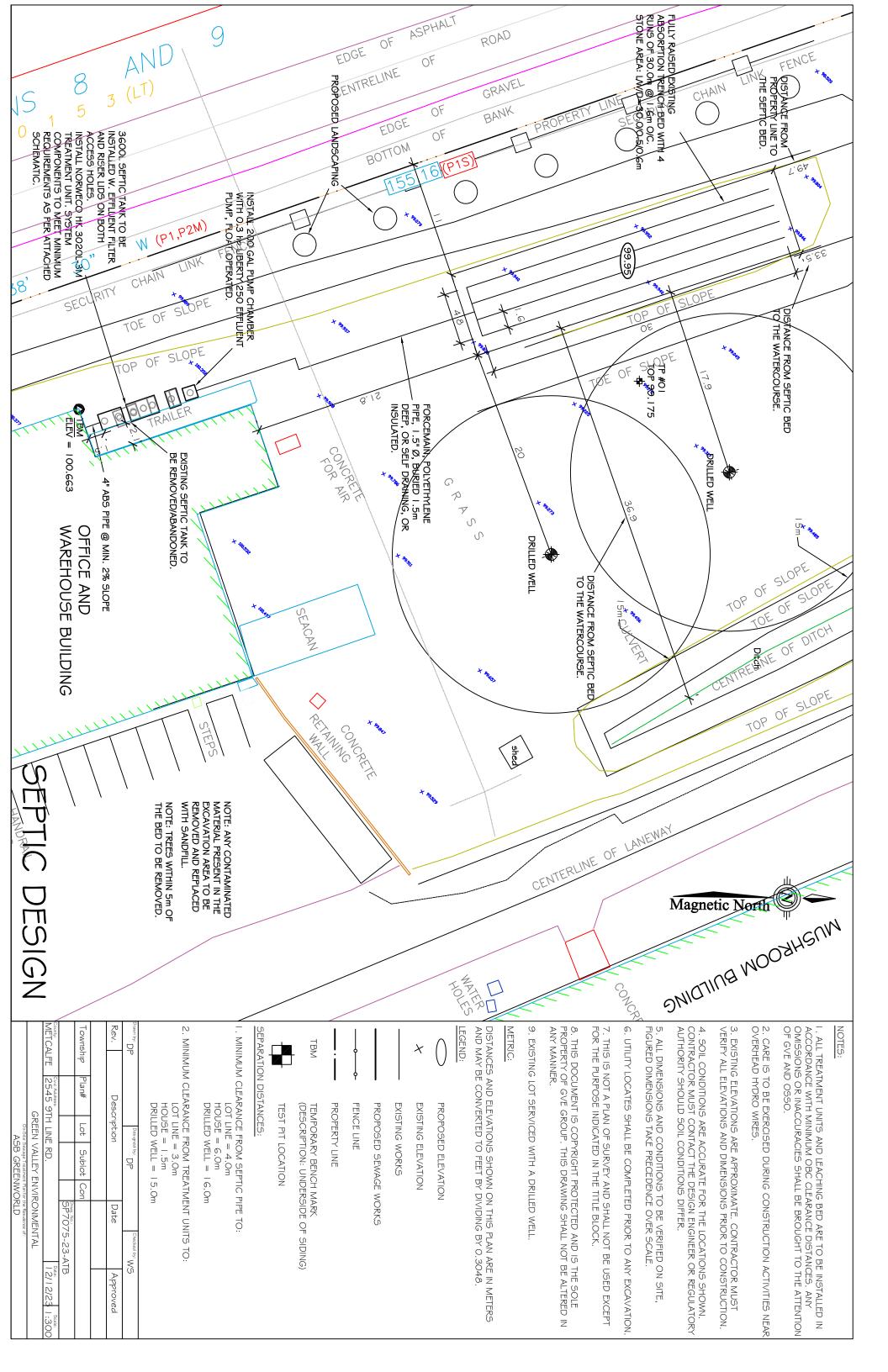


ASB Greenworld	
2545 9th Line Rd.	

	Warehouse	
0 Watercloset	950 Per Washroom	0 L/day
3 Loading Bay	150 Per Loading Bay	450 L/day
0 Floor Drains	125 Per FD(not pressurized)	0 L/day
Sub-Total		450 L/day

	Office	
15 Employees	75 Per Employee	1125 L/day
260m²	75 Per 9.3m²	2100 L/day
Governing flow		2100 L/day

Total	2550 L/day
lotai	2550 L/uay



# Appendix B

**Ottawa Septic System Office Documents** 

## File Search Reply - Match Found

Information per applicant

To

Steve Gagne

Date:

December 14, 2022

Email:

steve.gagne@ghd.com

Phone:

705-768-6350

From:

Ottawa Septic System Office

Phone:

613.692.3571 - Press "4" for the Septic office

Email:

septic@rvca.ca

Follow up Inquiries Please Reference:

FS-22-169

Archive file(s) 95-310

Civic Address:

2545 9th Line Road

Former Township:

Osgoode

**Property Owner Last Name:** 

12586015

Lot 19&20 Con: 9

Part:

Plan:

5R3469

Septic system designed per the Real estate feature listing attached records for: obtained via the internet: Bedrooms

**Bathrooms** 

Square M

1 Winor 7 Sonts

#### Attachment(s):

- As-Built Drawings
- Permit
- Use Permit (Certificate of Completion)

The foregoing information is given for your convenience only. Supplementary requests are necessary for conformity with other legislation such as flood plain or shoreline works. It should be clearly understood that you must satisfy yourself as to whether the premises and the existing or proposed use thereof is or would be in conformity with all applicable regulations. For further information please contact the Ottawa Septic System Office staff at the number listed above. Thank you for contacting the Ottawa Septic System Office.

Part 8 Inspector: Alex Dekleine

#### Permit List

Permit

Application Number	Appl Date	Application Type	Permit Number			Issued Date	St#	Street
010624	000000	Construction	OS010624	N	N	20-OCT-1999	2545 9T	
010675	000000	Construction	OS010675	N	N	20-DEC-1999	2545 9T	
011121	01-JAN-2001	Construction	05011121	N	N	03JAN-2001	2545 9T	
95-307	02-NOV-1995	Sewage System	95-307	N	N	000000	2545 9T	
95-310	24-MAR-1995	Sewage System	95-310	N	N	26JUN-1996	2545 9T	
A04-004845	20-MAY-2004	Construction	0404913	N	N	15JUN-2004	2545 9T	
A04-005712	08JUN-2004	Construction	0407447	N	N	24-AUG-2004	2545 9T	
A08-001197	11-MAR-2008	Construction	0802107	N	N	17-APR-2008	2545 9T	
ARC21-3792	000000	Road Cut	RC213697	N	N	04-OCT-2021	2545 9T	
FS-18-235	13-NOV-2018	Sewage System	FS-18-235	N	N	000000	2545 9T	H LINE
FS-22-169	12-DEC-2022	Sewage System	FS-22-169	N	N	000000	2545 9T	
RC041759	000 -0000	Road Cut	RC041759	N	N	28-FEB-2008	2545 9T	

Municipal Address - 2545 9TH LINE RD - Osgoode - RURAL AREA

Property Address First Name

2545 9TH LINE RD LAND MAN INC CON 9 PT LOTS 19 & 20 RP:5R-3469 PART 2

2545 9TH LINE RD HAY MAN INC
SEE OWNER SEE OWNER

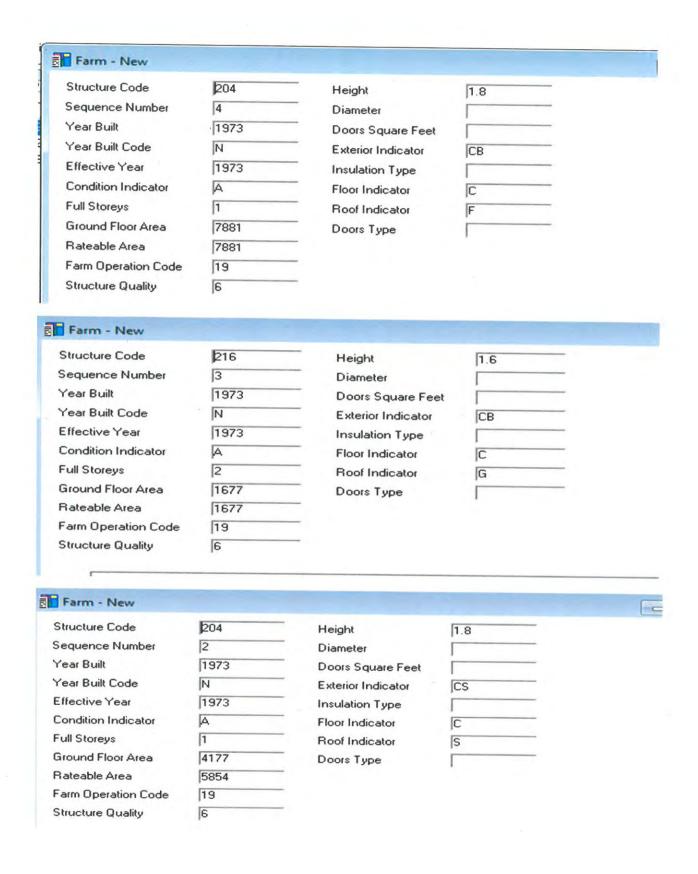
CON 9 PT LOTS 19 & 20 RP;5R-3469 PART 2

Assessment R	oll Number	0614700	05518901 Pr	evious Roll Number 08	0100005518901	
Property / Ten	nant Address	2545 9TH	I LINE RD			
Legal Descript	ion	CON 9 P	T LOTS 19 & 20 RP;5R-3469 PART	2		
Unit School St	upport	P	Mailing Address	8719 VICTORIA ST	METCALF	E ON KC
Business Scho	ool Support		Homogeneous Neighbourhood	666	Unit Class	
Property Code		230	Equalization		Realty Tax Class	
Municipal War	d	20	Create Date	20001020	Business Tax Class	
Municipal Poll		10	Number of Stuctures	9	Business Percent	
Municipal Poll	Suffix	1	Names Per Roll Number	2	Tenant Tax Liability	
Mill Rate			Subordinates Per Roll Number	2	Partnership Code	
Assessor Neig	hbourhood		Property Class		Publicly Traded	
School Code			Change Date - Subordinate		Prime/Subordinate	0000
_			Change Date - Primary	20220811		
Message Text						
Name		LAND MA	AN INC			

#### 

Farm - New				
tructure Code	203	Height	1.8	
equence Number	6	Diameter		
'ear Built	1975	Doors Square Feet		
'ear Built Code	N	Exterior Indicator	CS	
Effective Year	1975	Insulation Type		
Condition Indicator	A	Floor Indicator	C	
Full Storeys	1	Roof Indicator	G	
Ground Floor Area	37654	Doors Type		
Rateable Area	37654	-		
Farm Operation Code	19	-		
Structure Quality	5	_		
Farm - New				
Structure Code	203	Height	1.6	
Sequence Number	5	Diameter		
Year Built	2000	Doors Square Feet		
Year Built Code	E	Exterior Indicator	CS	
Effective Year	2000	Insulation Type		
			•	

Structure Code	203	Height	1.6
Sequence Number	5	Diameter	
Year Built	2000	Doors Square Feet	
Year Built Code	E	Exterior Indicator	CS
Effective Year	2000	Insulation Type	
Condition Indicator	A	Floor Indicator	C
Full Storeys	1	Roof Indicator	G
Ground Floor Area	9600	Doors Type	
Rateable Area	9600		
Farm Operation Code	19		
Structure Quality	5	_	



tructure Code	509	Building Height	1.6
Sequence Number	1	Basement Finished Area	
Construction Character		Heat Type Indicator	NO
Quality Indicator	6.5	- Air Conditioning Indicator	N
Shape Indicator	A	Effective Year	1986
Year Built	1986	Part Storeys	0
Build Year Code	N	Ground Floor Area	0
Condition Indicator	A	Unit Number	
Full Storeys	1	Basement Area	
Total Area	10560		
Estimated Character Quality		5	

Structure Code	204	Height	1.4
Sequence Number	7	Diameter	
Year Built	1975	Doors Square Feet	
Year Built Code	N	Exterior Indicator	CS
Effective Year	1975	Insulation Type	
Condition Indicator	Α	Floor Indicator	C
Full Storeys	1	Roof Indicator	G
Ground Floor Area	6970	Doors Type	
Rateable Area	6970	7	
Farm Operation Code	19	_	
Structure Quality	6		

En Farm - New			
Structure Code	229	Height	1.6
Sequence Number	8	Diameter	
Year Built	2005	Doors Square Feet	
Year Built Code	N	Exterior Indicator	PC
Effective Year	2005	Insulation Type	
Condition Indicator	A	Floor Indicator	C
Full Storeys		Roof Indicator	S
Ground Floor Area	12800	Doors Type	
Rateable Area			
Farm Operation Code	19		
Structure Quality	2		

Commercial - New			
Structure Code	508	Building Height	2.6
Sequence Number	9	Basement Finished Area	
Construction Character		Heat Type Indicator	NO
Quality Indicator	4	Air Conditioning Indicator	N
Shape Indicator	A	Effective Year	2005
Year Built	2005	Part Storeys	0
Build Year Code	N	Ground Floor Area	0
Condition Indicator	A	Unit Number	
Full Storeys		Basement Area	
Total Area	18000		
Estimated Character Quality			

Contravendons - Could I	ior generate description
Seq Number	1
Compliance Date	21-DEC-2012
Reference & Section	18 B.C. A.
Test and Sample Required	Provide letter from structural engineer confirming that settlement of soils supporting structure is within tolerable limits allowed by 2006 ontario building code and that settlement will not be detrimental to building structure. (Refer to soils consultant letter by Golder Associates - project 03-1120-0204 dated Feb. 25, 2009 regarding supporting soils)

Amended Compliance Date 1 00 - -0000

Amended Compliance Date 2 00 - -0000

Amended Compliance Date 3 00 - -0000

Date Complied 00 - -0000

## Municipal Address - 2545 9TH LINE RD - Osgoode - RURAL AREA

Application Number	Application Type	Date	Brief Description
PC2022-0142	Pre-App Consultation	25-MAY-2022	Proposal to re-use the existing facilities, previously used for a mushroom growing operation, to
PC2018-0107	Pre-App Consultation	18-APR-2018	Production of cannabis fresh, fried and oil, packaging of cannabis, storing cannabis products
05Jan-005	Compliance	04-JAN-2005	
D06-03-22-0121	Historical Land Use Inv.	07-JUL-2022	HLUI



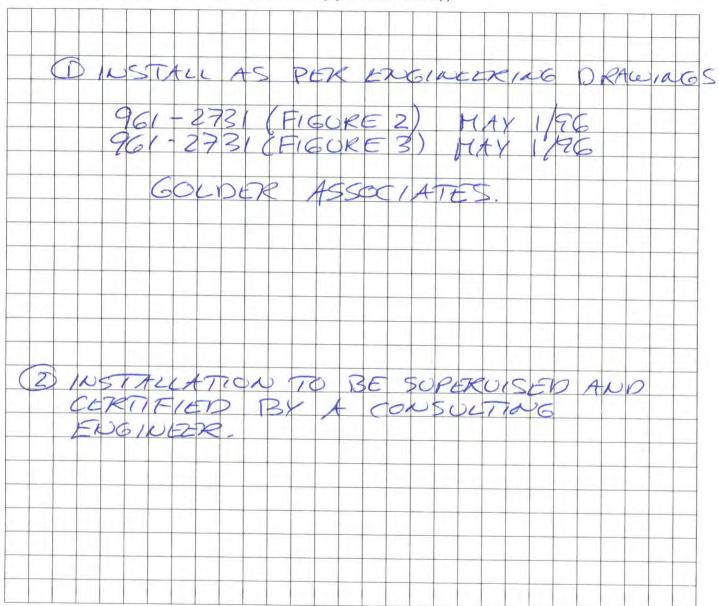
## Application Form And Certificate Of Approval For A Class 2 – 6 Sewage System

Applica	tion No	821	1942	. 9	310
Fee Re	peipt No	44	0	1/	<u> </u>
Date Re	ceived	Λ.	nar.	22/9	<u></u>

Personal information contained on this form is collected under the authority of the Environmental Protection Act, Part VIII. It is used to facilitate the issuance of a Certificate of Approval as prescribed in Section 77 of the Act. Questions should be directed to the Ministry's District Office in your area.

suarice of a Certificate of Approval as prescribed in Section 77 of the Act	ct. Questions should	d be directed to the Mi	nistry's District Office	in vour area.
1. Name and mailing address (number, street, city, town, etc.) of owner (1) NENTAL MUSHROUM COAP (19)	2. Name and	address (number, stree		
2545 974 LINE KD	TO B	E DET	E RIMAINIE	- 17
METCALFE, ONT KOA & PO				
Tel. no. (613) -821 (41)		Tel. no.	( ) –	
Alternate Tel. no. (613 - 22) 1262				
	Security of Contract	FARMOR	FICE BL	171
(construct,install, alter, extend, enlarge)	ge system to serve			
Region/County/District Ward, Township, Town	Lot No.   Co		e family dwelling, motel, etc	
Property OTTAWA OSGOODE TU	UP 19+20	nc.No. Sub Lot No.	Plan No.	Area of lot (m²)  100 Acre
State number of People rolled People rolled Virinals Washbasins Showers & bathtubs	6. Water supply	Dug or bored well	Drilled well	Municipal
Total fixture 48.5 Assessment Roll No. 060 000055 [[90]]  Attach completed sketch on Page 2. List other attachments.	or Existing	Other		
Attach completed sketch on Page 2. List other attachments.				
COLDER REPORT 96+	273/			
Relationship to severance (if applicable)  9. Directions to lot (Highway)	vay No., secondary roads	signs to follow, etc.)	- 11 00 1	17
Lot approval pending	4 / / /	6 EM21	0 ~ 1317	64
Severance Application No.	Ut NO,	NOATH O	NggL	INE
TOA OF	A 1-			MIKUAN
POR OUT	Con Con	ga- CONT		MUSIARa
l certify that the above information is complete and	correct and th	at, if approved, th	e work will confo	rm with
Provincial requirements for sewage systems and lo	ocal Municipal E	By-laws.		
Name and address of agent (if agent is completing this form) — number, street, city, to	town, etc.	Signature of owner of agent	(if agent is completing this f	orm)
HOR CONTINENTAL MUG	HILDOM.	011	/ //-	
RY de algora (Como Salle	1. A	1986	· ~ 2001	4
1704	Cathorna	Dates	Caro-	
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. Inspector's Report		V		
pection time and date	Sub	-surface conditions encou	ntered	
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P.M. SONE 5 ,1	19 76	- 0 -	REFER TO	5
ather Representing owner Leaching be	oed design criteria	0.25	ENGINEER	2106
SUNNY Depth to rock	ck Design H.W.T.	0.50	KEPORI:	
	_	0.75	GOLDER A	SOCIATES
Length of distribution pipe Working capacity of se	m. m. septic/holding tank	1.00	FILE: 961	-273/
guirements 126 EVICTIA	75 Day of 1500 XX 10	1.25	TRINE 1	99C
metres ZATSTAGE	litres	1,50	301-2	., 0
Conditions of approval and reasons (e.g. fill, grading, d	drainage improve	ments, design sewad	ge flows)	
Reasons where proposal not acceptable (add additional			go 110110)	
SEE APPENDIX "	/ / /	ed)		
JUL IFFENDIX	9/			
	6			
	***************************************			

- 12. LOT DIAGRAM AND SEWAGE SYSTEM PLAN: Draw to scale indicating north point and showing:
  - (a) Location of sewage system components (eg. tanks, leaching bed). Locate and show horizontal distances from system to adjacent existing or proposed buildings, water supplies (including neightbours), existing on-site sewage systems, driveways, property lines, lakes, rivers, water courses, swimming pools.
  - (b) Lot dimensions, topographic features (e.g. swamps, steep slopes) near system.
  - (c) If any part of proposal conforms to specific standard drawing, give reference number(s).



# A Certificate of Approval for this application is refused for the reasons given in Section 11 Page 1 Inspected and Recommended by Refused Director CERTIFICATE OF APPROVAL Application approved and this Certificate of Approval under Section 77 of the Environmental Proctection Act is hereby issued for the proposal outlined on Pages 1 and 2 of the application and its attachments as amended by the requirements and conditions of Section 11 provided that the sewage system shall be completed and a Use Permit issued within 12 months of the issue hereof or such extended period as the Director on application allows. DO NOT OPERATE THE SYSTEM UNTIL A USE PERMIT IS ISSUED. Inspected and Recommended by Issued Date Director Date Director Under Section 139 of the Environmental Protection Act, an applicant may appeal a decision by writing to the Director and to the Environmental Appeal Board, 112 St. Clair Avenue West, Suite 502, Torofilo, Ontario, M4V 1N3 within 15 days of receipt of the decision.

1040 (02/94) Page 2 of 2

### APPENDIX "C"

•	install as per lot d	iagram page 2 and typ	ical drawing "	<u>rr</u>					
•	appendix "G" must be completed and returned prior to the installation inspection								
• .	appendix "B" (filte to the installation	er medium) and weight inspection	bills must be comp	leted and returned prior					
	Refer to Pumping	requirements here att	ached						
•	Refer to Holding	tank (Class 5) requirer	nents here attache	ed					
•	Trees within(Silver Maples, V	metres of the lead Villows: 8 metres min.	ching bed must be → Others: 6	removed metres min.)					
•	Inlet and outlet o	of the septic tank must	be sealed proper	ly to ensure a watertight					
	Permit: 1. The leach 2. The man Certificate 3. Three (3) provided. distribution 4. The four	ing bed and septic tank tles (if required) must e of Approval must be holes, from an outside The openings must e	t must be covered to be in place and met. e corner to anothe expose the paper must be staked.	o the issuance of a Use and all conditions of the r (in a diagonal) must be or gravel on top of the					
	Drawing No.	Date	Compa	any/Consultant					
96	1-2731(F162) 1-2731 (F163)		GOLDER	ASSOCIATES ASSOCIATES					
	the system is ins	talled as per Ontario Ricate of Approval N°).	ulting engineer wit egulation 358 and	the Certification that the Certificate of Approval					
	Date		, 1	Part VIII					

### APPENDIX "G" PRIVATE SEWAGE DISPOSAL SYSTEM INSTALLER'S AS BUILT REPORT

NOTE: The Use Permit Inspection Permit #: 82 (19 \(\xi\)20	1 ( ) ( )		uthority	receiv	es this report completed in full.		
Installed By:	Appl	licant's Name:					
	.6. 7. 8.	Date of Installation:  Pack of this sheet as per the example below.  6. mantle extension(s)  7. pump chamber & distribution box (if applicable)  8. elevation of tile obvert (top of tile) at 4 outside corne  9. original grade elevation reference on Certificate of A (3 areas outside of fill mantle area to be shown)					
drilled/dug well	POOL 8	• 9	8 6		1 1		
1	4 7	5	8	• 9	3		
Septic Tank Volume: Septic Tank manufactured by:	2.37	of Tile:		— Rı	ins of Metres each		
Estimated "T" time of imported fill  The following measured horizontal  septic tank & tile bed to any septic tank and tile bed to s  tile bed to any structure (inc tile bed to property lines. If s  tile bed and septic tank to s	distances must be show y well within 35 metres structure that is being ser cluding pools, driveways) greater than 15 m. show	viced ) if less than 10 r > 15 m.	m.				
NOTE: Before submitting this report  1) The well is installed; 2) The immediate area surror silt); 3) The permeable, backfill refertablish the mantle (if reconstruction).	ounding the tile bed or filte	er bed is free of	excavate	ed imp	ermeable material (i.e.) clay,		
I hereby certify that the septic tank and the Certificate of Approval No.	system as described in		nstalled	in acc	ordance with the Regulations		
		Property Owner	's Signatu	re			
Installer's Licence #		Installer's Signa	ture				

### Ottawa-Carleton Septic System Office Bureau des systèmes septiques d'Ottawa-Carleton

### Installation Report • Rapport d'installation

Section A Class 4 & 4 F.M.	Septic tank/holding tank : \( \begin{align*} \begin	Sketch: (if not installed as per C. of A.)
Section B Leaching Bed	Type: CLASS 7  Height: OF Header: □-level  Runs: Length: Thickness: ○ C  Paper: □ yes □ no Slopes:  Ends capped: □ yes □ no Interconnect □  Pipes: Diameter □ 3 inch □ 4 inch  Make: PUC - ROYAL	Structure(s): House:  Lot Lines: Wells*: Control  Watercourses: Tree:  Between Trenches: Mantles: metres in direction(s  thickness: Elevations: (if required)
Section C Class 6 only	Audible & visual failure warning alarm installed  yes  no	□ Proprietary aerobic sewage treatment plant:
Section D Sections A, B & C	pump chamber pump present floats installed electrical wiring alarm	forced main: check valve frost protection installed joints sealed properly other:
Section E Section A, B & C	Distribution Box  sealed joints level frost protection  baffle or other compacted base - number of outlets:	Diagram:
Section F Class 5 only	Audible & visual failure warning alarm installed  yes no	□ prefabricated □ poured on-site
Section G Class 2 & 3 only	Side wall Construction:  Cover Construction:	
nspected b	□ Not Passed (see remarks)	

### Ottawa-Carleton Septic System Approvals Bureau des systèmes septiques d'Ottawa-Carleton

Final Grading Inspection  OURCASI
pplicants Name: CONTINENTIAL HUSHROOM  pplicants #: 82(19:20-9)310 Date: NUU   196 Time: 2:50 PM  resent on site: BRIAN STRATON & DAN MORRIS  aspector: TERRY K. DAUIDSON Date:
Is the finished elevation at the correct elevation relative to the reference grade (refer to Bench March on Certificate of Approval)?
Depth of cover measured from the top of the crushed stone layer. $ X_{1} = \underbrace{42}_{cm} \text{ cm}  X_{2} = \underbrace{3}_{cm} \text{ cm}  X_{3} = \underbrace{40}_{cm} \text{ cm}  \text{Photograph taken:}                                    $
Description of type of earth cover measured from the top of the distribution pipes.
Is the top of the bed shaped to shed water?  ves  no
Is the side slope visible? ☐ yes ☐ no
Length of Mantle:
L <sub>1</sub> = m
Does the depth of mantle (D) exceed .25m? □ yes □ no
Description of mantle material:
If required, was frost protection placed over the i) septic tank
Is all drainage directed away from the tile bed?
Was a photograph of the complete system taken? □ yes □ no
Inspection accepted ☐ yes ☐ no
For re-inspection, call 692-0160 or 1-800-459-5975.
Comments:
SEAL OUTLET PIPE  L2  \[ \alpha_1 \]



### USE PERMIT FOR CLASS 4, 5, 6 SEWAGE SYSTEMS

APPLICATION NO. 82/19520)310

		STATE OF THE PARTY		
NSPECTION DETAILS	Zi5Opm	NOU 1/96	WEATHER OUERCA	ST
REPRESENTING:	THE OWNER		THE INSTALLER  DAN MC	ORRIS
	nk of working capa			
MAKE AND MODEL, IF PREFABRICATED TA	ANK EX	151106	aw en villa	
in runs and fed b	(type and product)	mm(millimetres) diameter uct description e.g. manufa		ch pipe is made) laid
Location     a) System components ins     b) If located other than	talled as shown on ap in (a) use space be	oplication supporting Certific clow for sketch and dimens ning bed including orientatio	ons from permanent points	of reference sufficient
3. The following work remain	The second secon			
☐ Backfill System and Con  Stabilize All Sloped Sur			Shed Run-off and Divert Water	
The second section of the section of the second section of the section of the second section of the secti		USE PERMIT	NOVE THE PERSON BUY OF	iluipee re
is hereby issued to (Owne	CONTIN	ENTA MUSH		and operation of the
	tion number in acco	ordance with the application	ed persuant to the Certificaten and Certificate of Approvation Vard/Township/Municipality Sub-Lot No.	
INSPECTED AND RECOMME	ended by	PERMIT ISSUED BY	DATE ISSU	T 4/98

Section 76(a) of the Act provides that no change can be made to any building(s) or structure(s) in connection with which this sewage system is used, if the operation or effectiveness of the sewage system will or is likely to be affected by the change, unless a new Certificate of Approval is obtained.

Section 139 of the Act provides that an applicant for a permit may appeal a decision to refuse to issue a permit. Written notice of appeal must be forwarded to the Director (who refused to issue the permit) and to the Environmental Appeal Board, 112 St. Clair Avenue West, Suite 502, Toronto, Ontario M4V 1N3 within 15 days of receipt of a permit.

WARNING: UNDER NO CIRCUMSTANCES SHOULD A HOMEOWNER ENTER A SEPTIC TANK. NOXIOUS GASES WHICH ARE HEAVIER THAN AIR REMAIN IN THE TANK AFTER THE TOP IS REMOVED, AND HAVE CAUSED DEATH BOTH TO THE ORIGINAL VICTIM AND TO THOSE WHO ATTEMPT TO RESCUE HIM FROM THE TANK.

### Ottawa-Carleton Septic System Office Bureau des systèmes septiques d'Ottawa-Carleton







December 4, 1995 File: P190

Continental Mushroom Corp. (1989) Ltd. c/o Mr. Lyle Whitham, General Manager 2545 9th Line Road Metcalfe, Ontario K0A 2P0

Re: Lot 19 & 20, Concession 9

Township of Osgoode

Dear Sir,

Thank you for your fax dated on December 4<sup>th</sup> past, providing information of the soils type and percolation rates. With this information, we have now completed our review of your applications for three (3) Certificates of Approval.

By our calculations, the daily flow for which the sewage systems should be designed (as per the MANUAL OF POLICY, PROCEDURES AND GUIDELINES FOR PRIVATE SEWAGE DISPOSAL SYSTEMS) exceed 4 500 litres/day for two (2) of the three beds (office excluded).

Accordingly the systems are considered to be a non-standard systems and are to be designed and installed according to the M.O.E.E. requirements described in the attached information.

In order to process the Certificates of Approval, we will require that a consulting engineer's report be submitted to demonstrate how the requirements for a non-standard system have been met. We realize that these requirements will further delay the repairs of the malfunctioning systems, but we are obliged, in our capacity as agents for the Ministry of Environment and Energy, to ensure that these requirements are met, so that the systems will function properly without negative impact on the Environment.

The estimated daily average flow for the office will not exceed 4 500 litres therefore it is not considered a non-standard system. Although the proposed size of the replacement system is inadequate and must be re-evaluated. The Ottawa-Carleton Septic System Office is an Approval Agency, not a design consultant, therefore it is the responsibility of the proponent to demonstrate that the system design meets all the requirements of the Act, the Regulations and the Design Manual.

If you have any questions, please contact Denis Longpré or the undersigned. Thank you in advance for your patience and cooperation.

Yours truly,

Terry K. Davidson, P.Eng. Director Part VIII Environmental Protection Act

TKD/djl

### Golder Associates Ltd.

1796 Courtwood Crescent Ottawa, Ontorio, Canada K2C 2B5 Telephone (613) 224-5864 Fax (613) 224-9928



REPORT ON

TERRAIN AND HYDROGEOLOGICAL ASSESSMENT

PROPOSED REPLACEMENT SEPTIC SEWAGE DISPOSAL SYSTEMS

CONTINENTAL MUSHROOM CORP. (1989) LTD

METCALFE, ONTARIO

Submitted to:

Continental Mushroom Corp. (1989) Ltd. 2545 9th Line Road Metcalfe, Ontario K0A 2P0

### DISTRIBUTION:

4 copies - Continental Mushroom Corp. (1989) Ltd.

2 copies - Golder Associates Ltd.

May 1996 961-2731

### Golder Associates Ltd.

1796 Courtwood Crescent Ottawa, Ontario, Canada K2C 2B5 Telephone (613) 224-5864 Fax (613) 224-9928



May 14, 1996

961-2731

Continental Mushroom Corp. (1989) Ltd. 2545 9th Line Road Metcalfe, Ontario K0A 2P0

Attention: Mr. L. Whitham

General Manager

RE:

TERRAIN AND HYDROGEOLOGICAL ASSESSMENT PROPOSED REPLACEMENT SEPTIC SEWAGE SYSTEMS CONTINENTAL MUSHROOM CORP. (1989) LTD.

METCALFE, ONTARIO

Dear Sirs

This letter reports the results of a terrain and hydrogeological investigation carried out at the above site near Metcalfe, Ontario. The purpose of this investigation was to determine the general soil and groundwater conditions in the area of the two proposed septic tile fields and based on an interpretation of the factual information obtained, to provide a design for the two proposed septic systems. Also, the hydrogeological aspects of one of the proposed septic systems was to address the Ontario Ministry of the Environmental and Energy (MOEE) Reasonable Use Criteria for groundwater.

### DESCRIPTION OF PROJECT AND SITE

Continental Mushroom operates a mushroom growing facility just east of the Town of Metcalfe on 9th Line road (see Key Plan, Figure 1). This facility is near the Town of Metcalfe, however the town has no communal water or wastewater servicing, and for this reason, the only practical option for Continental Mushroom is to utilize bedrock wells and septic systems for its on-site water

supply and wastewater handling requirements. Evaluations are, however, presently underway to assess the provision of communal servicing for the town of Metcalfe.

The septic systems presently include three separate tanks and fields servicing two buildings, namely the main office shipping building and production houses building as shown on Figure 2, Site Plan. The location of five bedrock wells is also presented on the Site Plan. Several wells logs for bedrock wells on the Continental Mushroom property from Ontario Ministry of the Environment and Energy data files is presented in Attachment A.

The three septic field systems have failed to varying degree and are to be replaced with two new septic tile bed systems, one for the main office/shipping building in the same general location as the present field and the second to replace the two septic tile bed systems servicing the production houses buildings. The two fields for the production houses are proposed to be combined into one larger tile bed system more at the back of the property (approximately 100 metres southeast of the production houses buildings) in order to be further away from areas of high traffic and activity, shallow bedrock and water supply wells.

Based on available geological information and the results of a previous subsurface investigation for on-site building foundations, it is expected that the site is underlain by an extensive deposit of native silty sand glacial till overlain by fill materials. Geology maps of the area indicate the bedrock underlying the site consists of dolostone of the Oxford formation.

### **PROCEDURE**

The field work for this investigation was carried out on April 18, 1996, at which time 13 test pits were put down within the two areas proposed for the replacement septic system leaching beds using a backhoe supplied and operated by the owner. Test pits TP-1 to TP-5 were excavated near the main office/shipping building with the remaining (TP-6 to TP-13) were excavated back of the production house building as located on the Site Plan, Figure 2. The test pits were advanced to depths of 0.90 to 1.4 metres near the main office/shipping building and 0.3 to 1.8 metres at the back of the property. The soil types encountered in the test pits were classified based on visual and tactile examination of the materials exposed in the walls of the test pits. The groundwater conditions were observed in the test pits during the short period of time that the test pits were left

### **Golder Associates**

open at the time of the field work. The field work was supervised by a member of our engineering staff who directed the test pitting operation and logged the subsurface conditions at the test pits. A description of the subsurface conditions encountered in each of the test pits put down during this investigation is given in the Record of Test Pits, Table 1, following the text of this report. The approximate locations of the test pits are shown on the Site Plan, Figure 2.

The ground surface elevations within each of the two areas of the proposed septic system development were determined by Golder Associates Ltd. The elevations of the main office shipping building were referenced to a temporary benchmark (TBM) described as the northwest corner of main office/shipping building foundation. The elevations for the proposed, combined, septic field systems to the southeast of the main production house building was based on a temporary benchmark (100.00) at the southeast corner of the production house building. The temporary benchmarks were assigned an elevation of 100.00 metres as referenced to local datum. The ground surface elevations within the area of the site proposed for the septic system were also determined by Golder Associates Ltd. with reference to the temporary benchmark. A contour plan of the two proposed septic development areas is shown on Figure 2.

### SUBSURFACE CONDITIONS

A detailed description of the subsurface conditions encountered in the test pits is given on the Record of Test Pits, Table 1. The test pit logs indicate the subsurface conditions at the specific test locations only. Boundaries between zones on the logs are often not distinct, but rather are transitional and have been interpreted. The following is a summarized account of the subsurface conditions at the site for each of the two proposed septic tile bed replacement areas:

### Main Office/Shipping Building

The results of the test pits indicate that the area of the proposed septic system leaching bed (TP-1 to TP-5) are underlain by about a 0.3 to 1.4 metre thickness of silty sand till over dolostone bedrock. The water well records for the existing wells at the site (see Figure 2 and Attachment A) indicate that the wells vary from approximately 30 to 90 metres depth with water bearing zones no shallower than 27.5 metres. A review of a surficial geology map of the site area, available well records for existing water wells in the area of the site as well as the results of previous test pits put

down at proposed septic system location indicates that the silty sand and glacial till deposits are continuous in this area.

The on-site drill logs for the existing wells at the site indicate that the silty sand glacial deposits are underlain by limestone/dolostone and possibly sandstone at depth.

The results of observations within TP-1 to TP-5 indicate that the groundwater level in the area of the proposed septic leaching bed for the main office/shipping building is at a depth of about 1 metre below the existing ground surface with a general flow to the north. For the production houses building, the overburden is thicker to the south with the topography grading to the southeast. The groundwater levels follow the topography with groundwater flow toward the bushed area to the southeast.

### PROPOSED EXPANSION OF SEPTIC SEWAGE DISPOSAL SYSTEM

### General

This section of the report provides engineering guidelines concerning the geotechnical and hydrogeological aspects of the project, based on our interpretation of the existing test hole data and present project requirements. Contractors bidding on or undertaking the works should examine the factual results of the investigation, satisfy themselves as to the adequacy of the information for construction and make their own interpretation of the factual data as it affects their proposed construction techniques, safety, schedule and equipment capabilities.

The professional services retained for this project include only the geotechnical and hydrogeological aspects of the subsurface conditions at the site. The presence or implications of possible surface and/or subsurface contamination resulting from previous activities or uses of the site and/or resulting from the introduction onto the site of materials from off site sources are outside the terms of reference for this project and have not been investigated or addressed.

### **Design Considerations**

The results of the test pits put down within each of the two proposed septic field development areas indicate very similar soil types based on the field descriptions and on the grain-size distribution of Figure 4. The hydraulic conductivity for these two soils is essentially the same based on estimates from Hazen ( $D^2_{10} = k$  cm/sec) and Sherand ( $0.36D^2_{15} = k$  cm/sec) and is in the order of 1 x 10<sup>-4</sup> centimetres per second. This hydraulic conductivity is the equivalent of a "T" time of approximately 12 minutes per centimetre.

### Main Office/Shipping Building

The concept for the proposed septic leaching bed is to develop a new, fully raised field while utilizing the present septic tank. The old septic field and piping will be totally removed with the waste soil being hauled to the back of the property while the plastic piping will be recycled or landfilled.

Once the old septic bed has been removed, the base of the excavation, namely the native glacial till, will be scarified to ensure a good hydraulie connection between the septic bed materials and native soils. The fully raised bed will consist of silty sand with an in place, long term percolation rate of approximately 10 minutes per centimetre.

It is understood that the proposed leaching bed will serve approximately 32 employees. The maximum volume of septic effluent expected to be handled by the septic system is estimated at 2400 litres per day. Details of the septic effluent volume calculations are given in Attachment B. The classification of this waste is sewage of domestic origin, toilet waste and water sink waste. Based on a design percolation rate of 10 minutes per centimetre for the compacted silty sand fill for the leaching bed, a minimum total leaching bed tile length of 120 metres is required. Further design and construction details are provided in the attached Figures 2 and 4. These figures show the location of the septic tank, the leaching bed and mantle layout, and pertinent site features within the proposed leaching bed area.

### Production House Building

The concept for the sanitary wastes from the production house building is to develop one new field away from traffic and building run-off areas to replace the two poorly operating systems, each servicing half of the production house building. The present septic tanks are proposed to be incorporated in the overall design, however, one new pumping chamber near the most southerly septic tank is proposed to distribute the wastewater flows, alternatively to each half of the proposed field.

The design flow for the proposed septic field system is based on measured water consumption of approximately 2600 litres per day (December 1995 to March 1996) for 70 employees in half of the production houses building and this figure was doubled to accommodate the 70 employees in the other, identical half of the structure. Based on a "T" time of 10 minutes per centimetre, the septic field requires 260 metres of tile pipe with 300 metres incorporated into the design.

The design calculations for the pump chambers and septic field system are outlined in Attachment B while the design drawings and layout are presented in Figures 2, 3 and 4, respectively.

### Reasonable Use of Groundwater Considerations (Production House Building only)

In terms of the potential off-site impact of the septic waste, MOEE Guideline B-7 (MOEE, 1994b) addresses the level of off-site contaminant impact on groundwater considered acceptable by the MOEE and defines the level of impact on groundwater beyond which some form of migration measure(s) would be warranted

Under MOEE Guideline B-7, a change in the quality of groundwater on adjacent properties will only be acceptable if the quality is not degraded in excess of 50 percent of the difference between background concentrations and established water quality criteria for aesthetic related parameters, and 25 percent of the difference between background conditions and established water quality criteria for health related parameters.

### **Golder Associates**

To obtain a general indication as to the potential impact of septic effluent on the properties adjoining the Continental Mushroom site, a nitrate dilution model was utilized. The water balance method was used to estimate dilution and effects using a net potential infiltration of 220 millimetres per year for the Metcalfe area. A daily effluent loading of 5200 litres per day for the septic system was assumed. The nitrate dilution calculation is provided in Attachment C.

With regard to treatment and dispersal of effluent from the leaching beds, a maximum nitrate concentration of approximately 0.8 milligrams per litre was defined by calculating the theoretical area required to reduce the concentration of nitrate in the effluent from an assumed 40 milligrams per litre (mg/L) (as N) to 2.5 mg/L (as N) or lower at the property boundary by dilution as a result of the infiltration of meteoric water only. The site area of 40 hectares is nearly three times larger than the theoretical minimum area requirements determined using the nitrate dilution model. The nitrate dilution model does not include any nitrate loss from nutrient uptake or denitrification. Therefore, it is concluded that the impact of the proposed development on groundwater at the property lines would be acceptable.

Also the presence of the thicker silty sand glacial deposits to the south will act as a barrier to significant downward migration of the effluent to the underlying bedrock aquifer. The silty sand till is indicated to extend well out beyond the area of the proposed leaching bed. Accordingly, the effluent plume from the septic system will be quite isolated from and is therefore not a significant potential contaminant source to the local water supply. This conclusion is further verified by the present septic systems which have had no measurable impact to the several on site water wells. Consequently, moving the field to greater separation distances would essentially eliminate the potential for well water impact. Furthermore, the water well records for on-site wells (Attachment A) indicate at least 27 metres of lowly permeable bedrock to the shallowest water bearing seams in any of the wells.

### CONSTRUCTION CONSIDERATIONS

Construction of the leaching bed and mantle should be carried out using equipment which will not over compact the granular materials and render them relatively impermeable. In this regard, it is suggested that only light, track mounted equipment be used.

### **Golder Associates**

In order to ensure that the guidelines in this report have been interpreted as intended by the owner, it is suggested that the owner and/or his contractor contact the geotechnical engineer prior to starting construction to discuss his proposed methodology. It is also considered important that the materials proposed for use for the septic system be approved by the geotechnical engineer before use and that the construction of the leaching bed be inspected by the geotechnical personnel throughout construction.

### ADDITIONAL CONSIDERATIONS

This report and the attached Figures 2 and 4 showing details of the design of the proposed septic systems have been prepared for the sole use of the owner. It is understood that the owner, Continental Mushroom Corp. (1989) Ltd. will be constructing the proposed septic system using a local contractor. It is recommended that Golder Associates Ltd. review the proposed construction design with the designated contractor and that a practical field monitoring program be developed for quality control during construction.

We trust this report provides sufficient information for your purposes. If you have any questions concerning this report, please contact our office.

Yours truly,

GOLDER ASSOCIATES LTD.

R.D. Sinclair, P.Eng. Senior Environmental Engineer

RDS:do

Attachments

Table X
Figures 1 to 4
Attachments A to C

TABLE X
RECORD OF TEST PITS

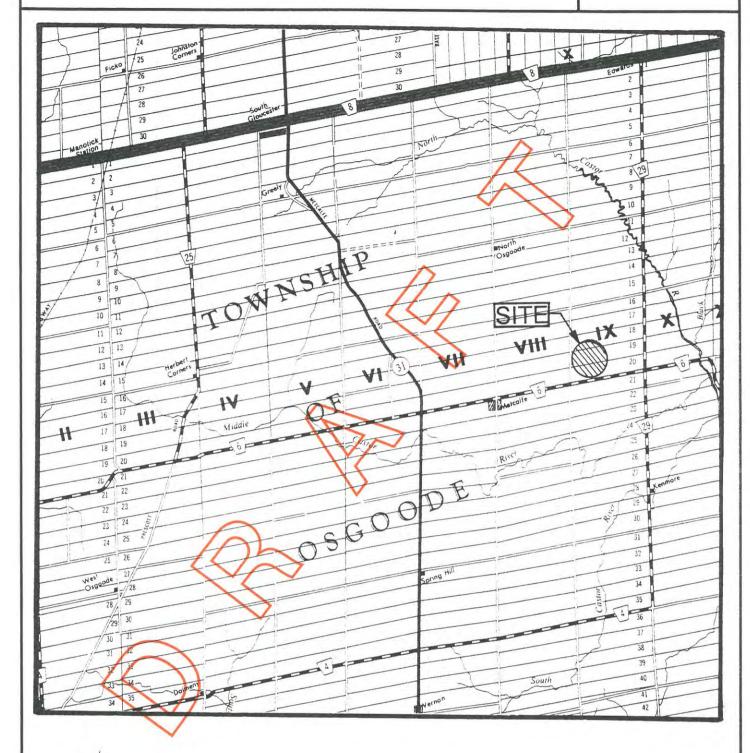
Test Pit	Depth	Soil
Number	(metres)	Description
	14464 C42	//
TP1	0.00 - 0.12	TOPSOIK
	0.12 - 0.91	Dark brown SANDY SILT with CLAY and cobbles and
		GRAVEL (fill)
	0.91 - 1.22	Brown SANDY SILT and
		GRAVEL
	1.22	End of test pit
		Bucket refused at 1.22 m -
		bedrock appeared competent,
		therefore BEDROCK assumed
		at depth of 1.22 m.
	_	Water at 1.22 m below ground
	100	surface. Sample was taken.
TP2	0.00 - 0.09	TOPSOIL
	0.09 0.88	Red brown SILTY SAND with
		cobbles and GRAVEL (till)
	0.88	End of test pit
(()		Refusal at 0.88 m in nesting
1,00		boulders.
	$\triangleright$	Test pit dry upon completion
		of excavating.
TP3	0.00 - 0.09	TOPSOIL
	0.09 - 1.07	Brown SILTY SAND with
		cobbles and small boulders
	1.07	End of test pit
		Nesting of boulders at 1.07 m
		Less silty from 0.76 m to
		bottom of test pit
		Refusal occurred at 1.07m in
		fractured bedrock

Test Pit	Depth	Soil
Number	(metres)	Description
TP4	0.00 - 0.09	TOPSOIL
	0.09 - 1.16	Red brown SILTY SAND with GRAVEL, cobbles and boulders (glacial till)
	1.16 - 1.43	Grey-brown and red-brown SANDY SILT to GRAVEL
	1.43	End of test pit
		Water encountered in test pit
	= /	Uneven suface at bottom of
		test pit - probably boulders
TP5	0.00 - 0.08	TOPSOIL
	0.08 - 0.52	Fine brown SAND, some GRAVEL
	0.52 - 0.61	Fine grey SAND (fill)
	0.61 - 1.31	Brown SILTY SAND, cobbles, boulders (till)
	1.31	End of test pit (BEDROCK)
		Test pit dry upon completion of excavating
TP6	0.00 - 0.12	TOPSOIL
<	0.12 - 0.88	Grey-brown fine-medium SAND with GRAVEL and cobbles (glacial till)
	0.88	End of test pit (BEDROCK)
		Surface runoff, water table no determined.

Test Pit	Depth	Soil
Number	(metres)	Description
TP7	0.00 - 0.08	Organic material
	0.08 - 0.30	Grey-brown fine-medium SAND with GRAVEL and
	0.30	cobbles (glacial till) End of test pit
	0.50	Lind of test pit
		Bedrock is composed of badly
		fractured, generally flat lying limestone or dolomite.
TP8	0.09	BEDROCK encountered
TP9	0.00 - 0.30	TOPSOIL - very organic
117	0.30 - 1.22	Grey-brown fine-medium
		SAND with GRAVEL,
	2.44	cobbles and boulders
	1.22	End of test pit (BEDROCK)
		Test pit dry upon completion of excavation
TP10	0.00 - 1.07	BACKFILL (boulder-sized)
		mixed with GLACIAL TILL
	1.07 - 1.83	Fine red-brown SILTY SAND with GRAVEL, cobbles, occasional boulder
	1.83	End of test pit

Test Pit	Depth	Soil
Number	(metres)	Description
TP11	0.00 - 0.09	TOPSOIL - very organic
	0.09 - 1.34	Fine red-brown SILTY SAND with GRAVEL, cobbles and occasional boulder (till)
	1.34	End of test pit
		Water encountered at bottom of test pit.
TP12	0.00 - 0.06	TOPSOIL
	0.06 - 1.46	Fine red-brown SILTY SAND with GRAVEL, cobbles and occasional boulder (dry till)
	1.46	End of test pit
TP13	0.00 - 0.15	TOPSOIL - very organic
	0.15 - 1.77	Fine-medium grey-brown SAND with GRAVEL, cobbles and some boulders (till)
	1,77	End of test pit (BEDROCK)

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SCALE: 1:100,000

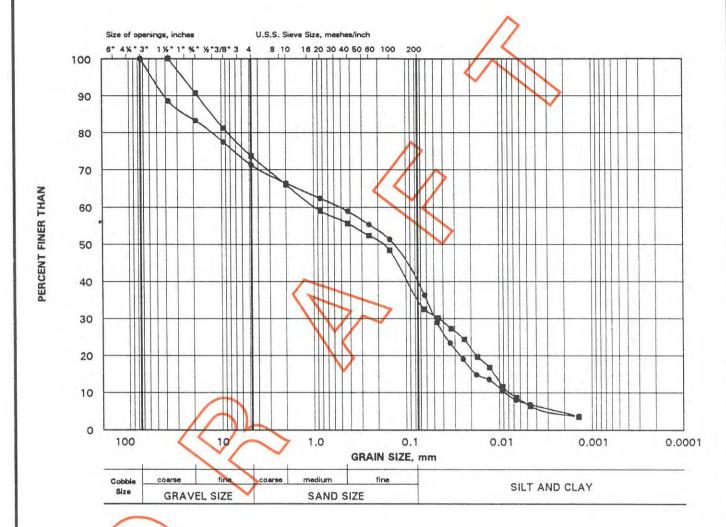
SPECIAL NOTE
THIS DRAWING IS TO BE READ IN CONJUNCTION
WITH ACCOMPANYING REPORT

Date MAY 03 1996
Project 961-2731

Golder Associates

Drawn K.M Chkd. BS





YMBOL TEST	SAMPLE
• 2	Composit
<b>=</b> 11	Composit

Project 961-2730

**Golder Associates** 

May 1996 961-2731

ATTACHMENT A

WATER WELL LOGS

CONTINENTAL MUSHROOM CORP. (1989) LTD. METCALFE, ONTARIO

LMSN	LMSN	SAND	0800		GREY	0020 PCKD LMSN		9000	GREY		LMSN MCRD				0020 HARD	1	9050					0287
BRWN	BLCK	BRWN	STNS		0051	PCKD STNS RED	9600	LMSN	0600		LMSN		0138	0051	LMSN		SUNS					SUNS
0600	0030	00040	LMSN	0120	LMSN	CLAY CLAY	SOFT		SHLE		GREY	0100	LMSN	SONDS	GREY	1	LMSN	0075	0250		7117	LMSN
MSN	LMSN	CLAY	BRWN	ROCK		GREY			RED		5000	SHLE	GREY	GREY	0800	<		LMSN	SUNS		MON	SREY
0012 GREY LMSN 0090 BRWN	0001 BLCK 1 SNDS 0198	BLUE	0038	BLCK	9000	00004 0031 GRVI	BLUE	FILL	LIMITED	LMSN 0180	SNDY	LMSN	STNS 0004 (	0000	MUSHROOK AND BLDR ERY HARD	MUSHROOF	SAND OROGA		NENTAL MUSHROOM	MUSHROOM	ES CDEV IMEN 0117	MUSHROOM AND 0003 (
MOHAUPT H GREY LMSN 0	SAND GREY	EWEG S TPSL BRWN			ROBERTS RON BRWN TPSL GRVL IMSN 0150	CLAY CLAY CLAY	SOFT SCOTTE	SAND	GREY LMSN UIBY LARCH HOMES LIMITED GREY SAND GRVL 0048	0003		WITHINALL L		MCDONALD A H GREY CLAY BLDR	CONTINENTAL BRWN CLAY S GREY LMSN V	0065 CONTINENTA	CONTINENTAL MUSHROOM	BLDR 0003 LMSN 0090 VANDERTILAART W BLDR CLAY 0010 ROCK	GRVI 0004 IMSN 0220		CELMS CHARLES	BRWN 1PSL UDCZ GRET LMSN 0117 CONTINENTAL MUSHROOM BRWN TPSL SAND 0003 GREY LMSN SNDS
1/00 DO	1/00 DO	1/00 DO	1/00 DO	1/00 DO	1/00 DO	2/00 DO	1/30 D0	2/00 DO	1/00 DO	1/00 DO	1/00 no	1/00 00/1	1/00 DO	1/00/00/1	1/00 CO	10/00 CO IN	1/00 IN	1/00 DO	1/00 IN		1/10 DO	10/00 IR
22	50	9	15	80	ι,	80	89	25	5	9	54	120	7	15	30	18 1	15	01	52		4	7 1
30	20	58	15	80	100	50	70	75	120	9	96	7011	130	15	30	120	85	57	230		100	120
20	10	FLW	10		20	^	FLW	FLW	05	78	38	10	28	10	30	21	35	13	22		FLW	18
106	195	78	7.8	120	120	95	3	90	291	175	110		130	20	45	297	85	22	250	DRY	115	280
A.	FR	FR	FR	FR	FR	2	) # J	FR	F	FR	FR	FR	FR	FR	FR	FR	FR	FR	FR		FR	FR
Z.	9	5	9	2	9	0	Sal	9	9	9	9	9	22	12	89	9	9	9	9	9	5	9
1558	1558	1517	1517	1517	1558	1558	1517	3658	1558	1836	1558	1836	1505	1517	1558	1505	1836	1802	1836	1836	1517	1505
280 05/70	245 04/74	240 10/72	250 05/81	300 08/70	90 08/72	842 12/76	305 06/72	245 06/73	245 05/73	305 10/72	308 11/82	300 09/72	310 06/62	262 09/71	300 08/78	302 09/72	300 05/74	290 01/61	300 10/74	300 10/74		294 09/72
463445 2 5013460	464940 2 5013640	465160 2 5013960	465199 2		5012736	465200 2 5013870		465375 2 5013145	465424 2 5013047	464020 3		464135 3			464500 3 5010580		5010585				5.1	5011538 464505 2 5010455
15- 40 10665 501	15- 4	12264 50	15- 4	1 4	15039 50 15039 50	15781 50	L	15- 4 15- 4 13800 50	13232 50	12274 50	1 10	15- 4	, и		15- 4 16652 50	15- 4	12298 50	m "	1 4	- 1		13261 50 15- 4 12297 50
12	13	13	13	14	14	14	15	15	15	16	16	17	17	17	19	19	19	0	19	19	0	20
6	6.	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6
CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON	CON

## OTTAWA-CARLETON 15

OWNER/LOG/SCREEN

WATER TEST TIME HR/MN TEST RATE GPM PUMP LVL FEET CSG KIND WATER STATE EASTING ELEY DIA OF FOUND LYL NORTHING FEET DATE DRILLER INS WATER FEET FEET MELL LOT MUNICIPALITY CONCESSION

DEPTHS IN FEET TO WHICH FORMATIONS EXTEND

SHLE 0060 BRWN HPAN BLDR 0026 BRWN GRYL SAND 0032 MCINTYRE J TPSL STNS 0008 HPAN 0037 BLCK LMSN 0054 GREY LMSN 0262 GREY HPAN 0014 BRWN LMSN GREY LMSN 0260 1900 0024 LMSN 0055 TPSL CLAY MSND 0005 GREY LMSN RED 0900 HARRISON E GRVL 0006 RED HPAN 0012 RED HOODS H BLUE CLAY 0025 GREY LMSN 0097 GREY CLAY HPAN 0050 GRVL 0055 0043 LMSN 0084 GREY LMSN SUNS 0003 LMSN 0160 0084 CLAY TPSL 0010 LMSN 0051 GREY VANNOORT DWIGHT BRWN CLAY TPSL 0003 GREY HPAN 0008 GREY BLDR GRYL 0019 LMSN SAND MAYMANN CARL R HPAN 0008 LMSN 0050 0165 GRW FPSL 0002 LMSN 0046 CONT MUSHRM CORP DILLA BOUGH D W BRWN CLAY GRYL GREY GRVL 0002 PRDR 0100 LMSN CLAY 0020 BLDR BRWN TPSL 0002 POAPET E GREY HPAN 0030 MYSTEANSKI JOHN BLCK TPSL 0007 BRWN LMSN 0065 BLUE LMSN 0100 GREY SNDS 0284 MSND BLDR 0016 ZANBELT JOHN VANDAM D T KINGSBURY KINGSBURY FROLICH R SNDS 0270 HAARSMA J ROSS HUGH MARION R BOWMAN G DUBORD J YANON G HARBER 9600 DO 00 DO CO DO 00 00 DO 00 00 DO ST DO 00 ST 00 00 DO DO 00 00 2/00 DO DO ST 2/00 1/20 1/00 /30 2/00 1/00 /30 1/00 /30 1/10 2/00 1/00 2/00 1/00 2/00 2/00 1/00 1/00 2/00 1/00 2/00 9 M 2 10 15 4 M 14 20 20 2 10 40 15 20 2 15 100 20 95 30 19 16 25 20 06 16 11 25 15 21 40 30 17 28 12 10 25 8 6 15 22 20 30 22 18 25 20 10 14 64 80 80 100 150 30 121 267 85 85 282 55 63 43 50 06 28 45 21 21 50 58 90 54 92 30 156 82 54 FR SU 2 2 4 9 2 4 9 9 4 3113 1526 1517 1517 1526 3601 3658 3658 1517 1802 1503 2308 3504 1836 3504 3644 1802 3504 01/68 09/90 07/63 02/73 260 06/63 10/75 260 06/53 255 12/59 11/72 280 12/55 280 04/63 287 05/70 07/73 06/73 91/10 10/59 06/53 255 11/67 280 10/74 10/72 03/61 62/90 295 275 275 285 285 262 295 285 260 255 250 (CONTINUED...) 464799 467060 465198 5009140 15- 464508 2279 5010410 464780 464719 464890 5010290 465230 465560 465570 465590 006595 465126 466900 466070 5008170 066995 5010196 5010500 466220 5011080 465999 466325 5009810 465420 5008530 466980 5009140 5009100 5010027 5010799 5010830 5009820 5009770 5009210 5008707 5008137 13795 15-7665 10586 13804 14341 15-15-9853 7667 8994 7669 15-13266 15-7671 15-15102 15-15-7672 15-7675 15-14107 9991 15-15-15-15-12090 15-15-15-20 56 OSGOODE TOWNSHIP 20 20 20 21 21 21 22 22 22 23 24 52 25 56 26 26 20 21 21 27 6 6 CON 12 1



DESIGN CALCULATIONS SEPTIC SEWAGE DISPOSAL SYSTEMS CONTINENTAL MUSHROOM CORP. (1989) LTD.

- A) MAIN OFFICE SHIPPING BUILDING SYSTEM
  B) PRODUCTION HOUSE, BUILDING SYSTEM



### ATTACHMENT B

### **ESTIMATED SEWAGE FLOWS**

### A) MAIN OFFICE/SHIPPING BUILDING

Staff - 35 Persons

### **Summary of Operations**

7 days per week - 1 shift (8 hours)



### Flow Calculation

Based on 75 litres per staff person per day

 $32 \times 75 = 2,400$  litres

Total = 2,400 litres per day



### Leaching Bed

Capacity required, 2,400 litres per day (Q)

T time selected = 10 minutes per centimetre

Total length of tile L = QT = 2,400 (10) = 120 metres 200

Length of tile provided in design = 120 metres

The tile field is to consist of one tile field containing 120 metres of perforated, 100 millimetre diameter tile

### Septic Tank

User tank presently in place. Gravity flow system.

### ATTACHMENT B (continued)

### B) PRODUCTION HOUSE BUILDING

 $2 \times 70 = 140$  employees

### **Summary of Operations**

7 days per week, one shift day (8 hours)

### Flow Calculation

Based on measured water consumption

Measured flow (half of building) = 2,600 litres per day (each half of building has identical operation)

Total = 5,200 litres per day

### Leaching Bed

Capacity required, 5,200 litres per day (Q)

T time selected = 6 minutes per centimetre

Total length of tile L =  $\underline{QT} = 5,200 (10) = 260$  metres 200 = 200

Length of tile provided in design = 300 metres

The tile field is to consist of two tile fields, each containing 150 metres of 100 millimetre diameter perforated tile.

### ATTACHMENT B (continued)

### Septic Tank

Using two tanks presently in place

### **Pumping Station**

The effluent will be pumped to each of the leaching beds by a double pumping system complete with pumps, floats and alarm. The pumps will alternate with each cycle, thereby, dosing each half of the bed on successive pumping cycles.

The quantity of effluent discharge from the dosing chamber shall not be less than 3/4 of the total interior volume of the distribution pipe in each of the septic tile fields. The distribution pipe will have a 100 millimetre diameter.

- Length of distribution pipe = 150 metres in each the field
- Volume of distribution pipe =  $\frac{\pi D^2}{4} \times 100 = 1.18 \text{ m}^3 = 1178 \text{ litres}$
- Minimum dosing volume = 0.75 × 1178 = 885 litres

Therefore, a minimum of 885 litres must be pumped from the dosing chamber with each pumping cycle. Dosing volume selected, 900 litres

### **Pumps**

Pumps will discharge effluent from the chamber for not more than 20 minutes per cycle. Fifteen minutes per pump cycle selected. The pumps will alternate between pumping cycles.

Pump flow rate = 60 litres per minute (14 Imperial gallons per minute)

### ATTACHMENT B (continued)

### Forcemain

Criteria:

Minimum velocity 0.8 metres/second

Maximum velocity 2.5 metres/second

Design:

Q = VA

Where Q = Flow volume

V = Flow velocity

A = Area of pipe

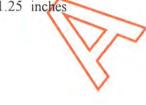
Flow velocity selected, 1.5 metres/second

$$A = Q = \frac{0.001 \text{ m}^3/\text{second}}{1.5 \text{ m/second}} = 0.00067 \text{ m}^2$$

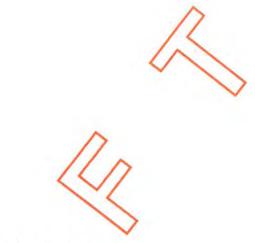
$$A = \frac{\pi}{4}D^2 = 0.0067 \text{ m}^2 \text{ where } D = \text{pipe diameter}$$

$$D^2 = \frac{0.0067 \times 4}{\pi} = 0.029 \text{ metres}$$

D = 29 millimetres or 1.25 inches



May 1996 961-2731



### ATTACHMENT C

NITRATE DILUTION CALCULATIONS SEPTIC SYSTEM DESIGN CONTINENTAL MUSHROOM CORP. (1989) LTD. METCALFE, ONTARIO



### ATTACHMENT C

### Nitrate Dilution Calculation

$$NO_{3 (Boundary mg/L)} = \frac{TotalMassof Nitrate}{TotalWaterVolume} = \frac{Background \& SepticWaste}{Infitlration \& SepticWaste}$$

Mass of Nitrate-Nitrogen

Background Nitrate = 0 mg/L (assumed)

Septic Waste =  $40 \text{ mg/L} \times 5200 \text{ l/day} \times 365 \text{ day/year} = 7.6 \times 10^7 \text{ mg/L/year}$ 

### Liquid Volumes

Net Potential Infiltration = 0.22 metres/year (for area)

Total Infiltration = 40 hectares x 0.22 metres/year = 88,000,000 litres per year

Septic Flow = 5200 litres per day x 365 days per year = 1,900,000 litres per year

$$NO_{3(Boundary)} = \frac{7.6x10^7 mg / year}{9x10^7 l / vear} = 0.84 mg / L$$

Require approximately 14 hectares to provide dilution to 2.5 mg/L or approximately 20 hectares if the main office/shipping building is included.

# Appendix C Fire Water Estimate Calculations

### **Appendix C: Fire Water Estimate Calculations**

Calculations by: Catherine Dang Checked by: Dan Roth, P.Eng.

Project: ASB Greenworld-Metcalfe Site

Project Number: 12586015

Date: 4-Oct-22



Tank Volume (m³)	200
Tank Volume (gal)	52834.4
Well Pumping Rate (gpm)	20

### Time to fill tank (hr) 44.0 (tank volume / well pumping rate)/60

### Scenario 1 (250 gpm)

Qnet = 250 gpm - 20 gpm	230 gpm
Time to run out of water (hr) = (volume of tank / Qnet)/60	3.8

### Scenario 2 (500 gpm)

Qnet = 500 gpm - 20 gpm	480 gpm
Time to run out of water (hr)	1.8
volume of tank / Qnet)/60	

### Scenario 3 (1000 gpm)

Qnet = 1000 gpm - 20 gpm	980 gpm
Time to run out of water (hr) =	0.0
(values a of tople / Opat)/CO	0.9

(volume of tank / Qnet)/60

### Appendix D

**Site Environmental Compliance Approvals** 



Ministry of the Environment Ministère de l'Environnement AMENDED CERTIFICATE OF APPROVAL INDUSTRIAL SEWAGE WORKS NUMBER 6569-6DRHP2

Continental Mushroom Corporation (1989) Ltd.

P.O. Box 520 Metcalfe, Ontario

K0A 2P0

Site Location: Continental Mushroom Corporation (1989) Ltd.

2545 9th Line Road (Lots 19 & 20, Concession 9)

Ottawa City

You have applied in accordance with Section 53 of the Ontario Water Resources Act for approval of:

wastewater recovery and re-use system to eliminate the discharge of storm run-off and the process wastewater generated from the facility at the above location, as follows:

### PROPOSED WORKS

- one (1) pond with an approximate volumetric capacity of 500 cubic metres (100,000 imperial gallons) with approximate dimensions of 23 square metres by 1 metre deep, to provide additional storage capacity for the prevention of overflow from the works to the North Castor River;

### **EXISTING WORKS**

- four (4) submersible pumps each rated at 5.0 litres/second at 7.2 of TDH to elevate wastewater from collection sumps at the mushroom growing building and packaging area to a gravity line which flows to an in-ground tank (Tank A);
- one (1) in-ground concrete tank (Tank A) with a volumetric capacity of 22,500 litres complete with a submersible pump rated at 5.0 litres/second at 7.2 metres of TDH for transfer of the wastewater to an adjacent tank (Tank B);
- one (1) in-ground concrete tank (Tank B) with a volumetric capacity of 45,000 litres complete with a submersible pump rated at 6.2 litres/second at 6.8 metres of TDH for return of screened wastewater either to the Mushroom Growing Building or to discharge to a catch basin/manhole;
- one (1) catch basin/manhole (MHS1) for collection of run-off from areas east of Mushroom Growing Building, west of Tunnel Building, west of Compost Production Wharf and the wastewater flow from Tank B prior to discharge by gravity to Tank D;
- one (1) in-ground concrete tank (Tank D) with a volumetric capacity of 22,500 litres and equipped with a pump rated at 4.5 litres/second at 5.0 metres of TDH for collection of washwater from the Tunnel Building, Machine Storage Building and catch basin/manhole (MHS1) with discharge to a screen (Kason Model K40-1-53 vibrating screen) for removal of suspended solids and then to Pond # 1;
- one (1) waterproof lined recovery pond (Pond # 1) with a volumetric capacity of 450,000 litres and equipped with aeration nozzles to minimize anaerobic condition, associated odour problem and to prevent freezing of the wastewater during winter months;
- one (1) pump and piping system for transfer of wastewater to Tank B and to the two (2) above ground storage tanks (45,000 litres and 54,000 litres) located adjacent to Machine Storage Building for re-use of the wastewater;
- one (1) overflow detention pond (Pond # 2) with a volumetric capacity of 675,000 litres for storage of any excess

wastewater discharged from the Pond # 1 as backup;

- one (1) emergency pond (Pond # 3) with a volumetric capacity of 414,000 litres for storage of wastewater, if any, discharged from Pond # 2 as backup;
- one (1) twinned pair tanks (Tank F) with a total volumetric capacity of 180,000 litres for collection of run-off from Compost Production Wharf via one (1) 13,500 litre capacity tank and one (1) screen (Kason Cross Flow Sieve);
- one (1) pump rated at 10.0 litres/second at 6.0 metres of TDH for transfer of run-off from the Tank F to the Compost Production Wharf for use in compost production;
- one (1) emergency overflow pond (Pond # 4) with a volumetric capacity of 166,500 litres for storage of run-off in the event of overflow from Tank F; and
- all other appurtenances essential for proper operation of the aforementioned sewage works;

all in accordance with the following documents:

- 1. The Application for Approval of Industrial Sewage Works dated November 23, 2001 and the associated documents submitted by the Manager, Continental Mushroom Corporation (1989) Ltd., Metcalfe, Ontario.
- 2. The Application for Approval of Industrial Sewage Works dated April 4, 2005 submitted by Lyle Whitham, the Manager, Continental Mushroom Corporation (1989) Ltd., Metcalfe, Ontario and all supporting information.

For the purpose of this Certificate of Approval and the terms and conditions specified below, the following definitions apply:

"Certificate" means this entire certificate of approval document, issued in accordance with Section 53 of the Ontario Water Resources Act, and includes any schedules;

"*Director*" means any *Ministry* employee appointed by the Minister pursuant to section 5 of the <u>Ontario Water Resources Act</u>;

"District Manager" means the District Manager of the Ottawa District Office of the Ministry;

"Ministry" means the Ontario Ministry of the Environment;

"Owner" means Continental Mushroom Corporation (1989) Ltd. and includes its successors and assignees;

"Works" means the sewage works described in the Owner's application, this Certificate and in the supporting documentation referred to herein, to the extent approved by this Certificate.

You are hereby notified that this approval is issued to you subject to the terms and conditions outlined below:

### TERMS AND CONDITIONS

### 1. GENERAL PROVISIONS

- (1) Except as otherwise provided by these Conditions, the *Owner* shall design, build, install, operate and maintain the *Works* in accordance with the description given in this *Certificate*, the application for approval of the works and the submitted supporting documents and plans and specifications as listed in this *Certificate*.
- (2) Where there is a conflict between a provision of any submitted document referred to in this *Certificate* and the Conditions of this *Certificate*, the Conditions in this *Certificate* shall take precedence, and where there is a conflict between the listed submitted documents, the document bearing the most recent date shall prevail.
- (3) Where there is a conflict between the listed submitted documents, and the application, the application shall take

precedence unless it is clear that the purpose of the document was to amend the application.

### 2. EXPIRY OF APPROVAL

The approval issued by this *Certificate* will cease to apply to those parts of the *Works* which have not been constructed within five (5) years of the date of this *Certificate*.

### 3. CHANGE OF OWNER

The *Owner* shall notify the *District Manager* and the *Director*, in writing, of any of the following changes within thirty (30) days of the change occurring:

- (a) change of Owner;
- (b) change of address of the Owner;
- (c) change of partners where the *Owner* is or at any time becomes a partnership, and a copy of the most recent declaration filed under the <u>Business Names Act</u>, R.S.O. 1990, c.B17 shall be included in the notification to the *District Manager*; and
- (d) change of name of the corporation where the *Owner* is or at any time becomes a corporation, and a copy of the most current information filed under the <u>Corporations Information Act</u>, R.S.O. 1990, c. C39 shall be included in the notification to the *District Manager*.

### 4. OPERATION AND MAINTENANCE.

- (1) The *Owner* shall ensure that the design minimum liquid retention volume(s) is maintained at all times.
- (2) The *Owner* shall inspect the *Works* at least once a year and, if necessary, clean and maintain the *Works* to prevent the excessive buildup of sediments, oil/grit, and/or vegetation.
- (3) The *Owner* shall maintain a logbook to record the results of these inspections and any cleaning and maintenance operations undertaken, and shall keep the logbook at the owner's offices for inspection by the *Ministry*. The logbook shall include the following:
  - (a) the name of the Works;
  - (b) the date and results of each inspection, maintenance and cleaning, including an estimate of the quantity of any materials removed; and
  - (c) the date of each spill within the catchment area, including follow-up actions / remedial measures undertaken.

### 5. SLUDGE HANDLING

The owner shall handle and dispose of the sludge generated from the *Works* in accordance with Part V of the Environmental Protection Act.

### 6. <u>RECORD KEEPING</u>

The *Owner* shall retain for a minimum of five (5) years from the date of their creation, all records and information related to or resulting from the operation and maintenance activities required by this *Certificate*.

*The reasons for the imposition of these terms and conditions are as follows:* 

- 1. Condition 1 is imposed to ensure that the *Works* are built and operated in the manner in which they were described for review and upon which approval was granted. This condition is also included to emphasize the precedence of Conditions in the *Certificate* and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
- 2. Condition 2 is included to ensure that, when the *Works* are constructed, the *Works* will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment..
- 3. Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to approved works and to ensure that subsequent owners of the works are made aware of the certificate and continue to operate the works in compliance with it.
- 4. Condition 4 is included to require that the *Works* be properly operated and maintained such that the environment is protected.
- 5. Condition No. 5 is included to ensure that sludge generated from the works is disposed of in an acceptable manner.
- 6. Condition 6 is included to require that all records are retained for a sufficient time period to adequately evaluate the long-term operation and maintenance of the *Works*.

### This Certificate of Approval revokes and replaces Certificate(s) of Approval No. 9511-55QLME issued on February 21, 2002.

In accordance with Section 100 of the <u>Ontario Water Resources Act</u>, R.S.O. 1990, Chapter 0.40, as amended, you may by written notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 101 of the <u>Ontario Water Resources Act</u>, R.S.O. 1990, Chapter 0.40, provides that the Notice requiring the hearing shall state:

- 1. The portions of the approval or each term or condition in the approval in respect of which the hearing is required, and;
- 2. The grounds on which you intend to rely at the hearing in relation to <u>each</u> portion appealed.

The Notice should also include:

- 3. The name of the appellant;
- 4. The address of the appellant;
- 5. The Certificate of Approval number;
- 6. The date of the Certificate of Approval;
- 7. The name of the Director;
- 8. The municipality within which the works are located;

*And the Notice should be signed and dated by the appellant.* 

This Notice must be served upon:

The Secretary\* Environmental Review Tribunal 2300 Yonge St., 12th Floor P.O. Box 2382 Toronto, Ontario M4P 1E4 <u>AND</u>

The Director Section 53, *Ontario Water Resources Act* Ministry of the Environment 2 St. Clair Avenue West, Floor 12A Toronto, Ontario M4V 1L5

The above noted sewage works are approved under Section 53 of the Ontario Water Resources Act.

DATED AT TORONTO this 4th day of July, 2005

Mohamed Dhalla, P.Eng. Director Section 53, *Ontario Water Resources Act* 

KD/ c: District Manager, MOE Ottawa. Robert Stiles, NOTRA Inc.



→ The Power of Commitment