



**Site Servicing and
Stormwater Management Report
1158 Old Second Line Road
Ottawa, Ontario**

Type of Document:
Zoning By-law Amendment

Client:
SLK Limited Partnership

Developer:
Theberge Homes

Project Number:
OTT-00245003-A0

Prepared By: Zhidong Pan, M.Eng., P.Eng.
Reviewed By: Bruce Thomas, P.Eng.

EXP Services Inc.
100-2650 Queensview Drive
Ottawa, ON K2B 8H6

Date Submitted:
June 14, 2023
Rev1 – November 14, 2023

Site Servicing and Stormwater Management Report 1158 Old Second Line Road, Ottawa, Ontario

Type of Document:
Zoning By-law Amendment

Client:
SLK Limited Partnership

Developer:
Theberge Homes

Project Number:
OTT-00245003-A1

Prepared By:
EXP Services Inc.
100-2650 Queensview Drive
Ottawa, ON K2B 8H6
Canada
T: 613 688-1899
F: 613 225-7337
www.exp.com



Zhidong Pan, P.Eng., M.Eng.
Senior Water Resources Engineer
Infrastructure Services

Bruce Thomas, P.Eng.
Senior Project Manager
Infrastructure Services

Date Submitted:
November 14, 2023

Legal Notification

This report was prepared by EXP Services Inc. for the account of **SLK Limited Partnership**.

Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. EXP Services Inc. accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this project.

Table of Contents

1	Introduction	1
1.1	Site Description and Proposed Development	1
1.2	Background Documents	1
1.3	Existing Infrastructure	2
1.4	Consultation and Permits	3
2	Geotechnical Considerations	3
3	Watermain Servicing	4
3.1	Methodology	4
3.2	Design Criteria	4
3.3	Water Demands	4
3.4	Fire Flow Requirements	5
3.5	Boundary Conditions	6
3.6	Modelling Results	7
3.7	Hydrant Spacing	8
3.8	Water Age Analysis	9
4	Sanitary Sewer Design	10
4.1	Design Criteria	10
4.2	Proposed Sanitary Servicing	10
4.3	Downstream Sanitary Sewer System	11
5	Stormwater Management	13
5.1	Design Criteria	13
5.1.1	Minor System Design Criteria	13
5.1.2	Major System Design Criteria	13
5.2	Runoff Coefficients	13
5.3	Calculation of Allowable Release Rate	14
5.4	Pre-Development Conditions	14
5.5	Proposed Stormwater System	15
5.6	Design Methodology	15
5.7	Storm Sewer Design	16
5.8	Hydrology	17
5.8.1	Catchment Parameters	17
5.8.2	Storage Node Parameters	19
5.8.3	ADS Underground Storage Chamber	19
5.8.4	Outlet Node Parameters	20
5.9	Dual Drainage Modelling Methodology	20
5.9.1	Model Development	20
5.9.2	Storm Events Modelled	22
5.9.3	Inlet Control at Ponding Conditions (Road Sag)	22

5.10	Modelling Results	23
5.11	Hydraulics.....	23
5.11.1	Hydraulic Grade Line Analysis.....	23
5.12	Culvert Sizing	26
6	Erosion and Sediment Control	30
7	Conclusions.....	31

List of Figures

Figure 1 – Subcatchments Used for Sizing Entrance Culverts	26
Figure 2 – 50-year and 100-year HGL within East Ditch Along Second Line Road.....	28
Figure 3 –100-year HGL within East Ditch Using Raised Boundary Condition (0.3m above Obvert).....	29
Figure A1: Site Location Plan	A
Figure A3: Water Model Layout, Boundary Condition #1	A
Figure A4: Water Model Layout, Boundary Condition #2	A
Figure A5: Offsite Sanitary Drainage – Morgan’s Grant Phase 12.....	A

List of Tables

Table 3-1: Summary of Calculated Fire Flow for Each Block.....	6
Table 3-2: Boundary Conditions Provided by City of Ottawa	7
Table 3-3: Summary of Results for Peak Hour (Boundary Location #2)	7
Table 3-4: Summary Results for Maximum Day Plus Fire Flow (Boundary Location #2)	8
Table 3-5: Summary of Fire Flow Requirements Based on Hydrant Spacing.....	8
Table 3-6: Water Age Results	9
Table 5-1: Summary of Pre-Development Peak Flows from Proposed Site	15
Table 5-2: General Subcatchment Parameters	18
Table 5-3: Post-Development Subcatchment Parameters	19
Table 5-4: Storage Node Parameters.....	19
Table 5-5: Underground Chamber Stage-Storage Data.....	20
Table 5-7: Peak Flows at Outfalls.....	23
Table 5-8: Maximum Ponding Depths	23
Table 5-6: Peak Flows in East Ditch Along Old Second Line Road.....	27
Table B1: Water Demand Chart	B
Table B2: Summary of Required Fire Flows (RFFs)	B
Table B3: to B27 Calculation of Fire Flow Requirements for Buildings.....	B
Table D1: Sanitary Design Sheet	D
Table E1: 2-year Storm Sewer Calculation Sheet.....	E
Table E2: 100-year HGL Storm Sewer Calculation Sheet	E
Table E3: Average Runoff Coefficients (Pre-Development).....	E
Table E4: Pre-Development Runoff Calculations	E
Table E5: Allowable Runoff Calculations.....	E
Table E6: Average Runoff Coefficients (Post-Development)	E
Table E7: Summary of Post Development Runoff (Uncontrolled and Controlled)	E
Table E9: Storage Volumes for 2-year, 5-Year and 100-Year Storms (MRM).....	E

*EXP Services Inc.
SLK Limited Partnership
1158 Old Second Line Road
OTT-00245003-A1
November 14, 2023*

List of Appendices

- Appendix A – Figures
- Appendix B – Water Tables
- Appendix C – Water Distribution Modelling Results
- Appendix D – Sanitary Design Sheet
- Appendix E – Stormwater Design Sheets
- Appendix F – Stormwater Modelling Results
- Appendix G – Correspondence
- Appendix H – Manufacturer Information
- Appendix I – Background Information
- Appendix J – Drawings

1 Introduction

1.1 Site Description and Proposed Development

SLK Limited Partnership retained EXP Services Inc. (EXP) to undertake a site servicing and stormwater management study in support of the development applications at 1158 Old Second Line Road in the City of Ottawa. The property is situated on Old Second Line Road, 270m south of Old Carp Road as shown on Figure A1 in Appendix A.

The existing property consists of two (2) parcels. The northern parcel (PIN 045261418) consists of Parts 1 & 2 on Plan 4R-26462, whereas the southern parcel (PIN 045260207) consists of Parts 1 & 2 on Plan 5R-1175 and Part 1 on Plan 5R-8154. The two parcels combine for a total of 1.229 hectares, of which, a 0.029-hectare portion along Old Second Line Road will be reserved for a 3.0m road widening. The total site area being developed will be 1.20 hectares.

The development is comprised of one hundred (100) 3.5 storey stacked units. The 1.20-hectare development being proposed by SLK Limited Partnership will consist of eight (8) 3.5 storey stacked unit blocks ranging from 10 to 14 units each, a private roadway with adjacent parking stalls and a shared amenity space. The proposed site is bounded to the south and north by Phases 11 & 12D of the Morgan's Grant Development respectively, to the west by Old Second Line Road, and east by City of Ottawa owned land which is subject to an easement in favor of Hydro One.

A private roadway is proposed with one connection onto Old Second Line Road. All utilities will be located within the common roadway block. Sanitary and storm sewers and water infrastructure will require an 11m easement extending north from the site to Goward Drive and a 6m easement southerly to Whernside Terrace is required for a second watermain connection.

This report will discuss the adequacy of the adjacent municipal storm sewers, sanitary sewers and watermains to convey the storm runoff, sewage flows and provide the water demands that will result from the proposed development. It will identify any sanitary, storm or watermain servicing requirements, and provide a design brief for submission, along with the engineering drawings, for City of Ottawa approval.

1.2 Background Documents

Various design guidelines were referred to in preparing the current report including:

- Sewer Design Guidelines, Second Edition, Document SDG002, October 2012, City of Ottawa including:
- Technical Bulletin ISDTB-2012-4 (20 June 2012)
- Technical Bulletin ISDTB-2014-01 (05 February 2014)
- Technical Bulletin PIEDTB-2016-01 (September 6, 2016)
- Technical Bulletin ISDTB-2018-01 (21 March 2018)
- Technical Bulletin ISDTB-2018-04 (27 June 2018)
- Ottawa Design Guidelines – Water Distribution, July 2010 (WDG001), including:
- Technical Bulletin ISDTB-2014-02 (27 May 2014)
- Technical Bulletin ISTB-2018-02 (21 March 2018)

- Technical Bulletin ISTB-2021-03 (18th August 2021)
- Ontario Ministry of Transportation (MTO) Drainage Manual, 1995-1997
- Stormwater Management Planning and Design Manual, Ontario Ministry of the Environment and Climate Change, March 2003 (SMPDM)
- Design Guidelines for Drinking-Water Systems, Ontario Ministry of the Environment and Climate Change, 2008 (GDWS)
- Fire Underwriters Survey, Water Supply for Public Fire Protection (FUS), 2020
- Ontario Building Code 2012, Ministry of Municipal Affairs and Housing.

As the proposed site is within the Morgan's Grant Development, various Master Servicing and Stormwater Management Reports were reviewed in preparation of this report. The following reports, which were provided by City staff, are identified below:

- Master Servicing Study for the Morgan's; Grant Subdivision. J.L. Richards & Associates Limited, February 2001. City Report No: R-2168.
- Morgan's Grant Subdivision, Phase 12D Stormwater Management Report, J.L. Richards & Associates Limited, Sept 22, 2005. City Report No: R-1591-B.
- Morgan's Grant, Phase 12D Subdivision, Stormwater Site Management Plan, J.L. Richards & Associates Limited, August 2005. City Report No: R-1591-A.

The first document above provides the sanitary and storm sewer designs for Phases 1 – 9 of Morgan's Grant along with lands west of the Hydro Corridor and east of Old Second Line Road. This Master Servicing Study also makes an allowance for sanitary flows from Phases 12A-12D, which includes the proposed 1.2-hectare property at 1158 Old Second Line Road.

The second and third document noted above, provide stormwater design information specifically for the latest Phase of Morgan's Grant (Phase 12D), however the documents include background information for all Phase 12 stages (12A – 12D), since the entire Phase 12 area is serviced by a downstream stormwater management facility.

Additional information on the sanitary, storm and water system designs taken from each noted report, is provided in subsequent sections of this report.

1.3 Existing Infrastructure

The current 1.2-hectare site contains a single-family home that is serviced by a groundwater well and a septic tank and tile field bed. The septic tank and tile field is located between the building and Old Second Line Road, and a drilled well is located behind the home. The site is almost entirely sloped towards the hydro corridor; however, with a small percentage of the site sloped to Old Second Line Road. Runoff to Old Second Line Road is collected and conveyed in the existing roadside ditch.

There are no available municipal services located within Old Second Line Road (except for a 300mm sewage forcemain servicing Carp). As the site topography slopes easterly to the Hydro corridor with an almost ± 4 m grade change, the services will be required to connect to the municipal sanitary, storm and water infrastructure within Goward Drive. In addition, a second watermain connection within Whernside

Terrace is necessary. Additional information on the water supply requirements is provided later in this report.

An 11.0m wide easement from the site to Goward Drive will be required for the proposed 200mm sanitary sewer, 450mm storm sewer and 200mm watermain. The second 6.0-metre-wide easement extending south towards Whernside Terrace will be necessary for a 200mm watermain. These easements are in accordance with 3.3.1.2 of the City of Ottawa Sewer Design Guidelines, and 3.3.1.2 of the City of Ottawa Water Distribution Design Guidelines.

1.4 Consultation and Permits

Consultation meetings were held between SLK Limited Partnership and the City of Ottawa prior to design commencement. These meetings outlined the submission requirements and provided information to assist with the development proposal.

The storm and sanitary sewers will require Environmental Compliance Approvals (ECA's), filed through a direct submission with the MECP. The following summarizes the anticipated Environment Compliance Approvals (ECA's) required by the Ministry of Conservation and Parks (MECP), formerly the Ministry of the Environment and Climate Change (MOECC):

- Municipal and Private Sewage Works for **Sanitary and Storm Sewers**.
- Municipal and Private Sewage Works for the establishment of the **Stormwater Management Works** (SWM) which will include the onsite flow control methods and associated stormwater detention.

Prior to completion of the ECA application, City signoff on the infrastructure design will be necessary.

The proposed site is located within the Mississippi Valley Conservation Authority (MVCA) jurisdiction, therefore signoff from the MVCA will be required prior to subdivision and ECA approval. As the proposed site is located within the catchment area tributary to the Morgan's grant SWM Facility (City SWMF-1227), no additional onsite quality control requirements are expected.

2 Geotechnical Considerations

A geotechnical investigation was completed by EXP Services Inc, on April 12, 2018, and was prepared to establish the subsurface and groundwater conditions onsite, and to provide and discuss excavation, dewatering, and backfilling requirements. It also provides grade-raise, pavement and foundation design requirements.

In general, the site is treed and contains 150mm to 300mm of topsoil overlaid with sandy silt and silty sand. Below the ground surface, rock refusal depth varied between 0.3m to 1.7m, based on eleven (11) test pits and boreholes.

A maximum grade raise requirement of 2.0m was established for the site.

An additional geotechnical investigation was completed on January 2020, following the tree clearing to establish additional rock elevations with the site. This information has been added to the engineering drawings. The additional test pit investigation was completed to provide additional information on the depth to rock. An Additional Test Pit Investigation letter dated March 20, 2020, was prepared for the additional

thirty-four (34) test pits excavated onsite. A June 9, 2023, letter confirmed these two reports remain valid for the proposed development.

3 Watermain Servicing

3.1 Methodology

The water distribution system proposed for this development is designed in accordance with the City of Ottawa Design Guidelines (July 2010). The following steps indicate the basic methodology that was used in the hydraulic analysis:

- A water distribution model was created by adding junction nodes at intersections and creating watermains between the junctions.
- For each junction node the water demand was determined based on the number of contributing homes and the corresponding population.
- The water consumption rates were calculated for the maximum day and maximum hour conditions.
- Hydraulic boundary conditions were set from the information obtained from the City of Ottawa.
- The required fire flow was determined, and
- The proposed water distribution model was simulated in and the results compared with the City of Ottawa criteria.

3.2 Design Criteria

A summary of design parameters used in the water distribution model were taken from Section 4.0 of the City's Guidelines, and are as follows:

- Population Density (2-bedroom apartment) 2.1 person/unit
- Average daily water consumption (Residential) 280 L/cap/day
- Maximum Day Factor (2.5 x Avg. Day)
- Maximum Hour Factor (2.2 x Max. Day)
- C factor
 - 150 mm 100
 - 200 mm & 250 mm 110
- Minimum Allowable Pressure 275 kPa (40 psi)
- Maximum Allowable Pressure 552 kPa (80 psi)
- Minimum Static Pressure (Under Fire Flow Conditions) 140 kPa (20 psi)

3.3 Water Demands

The domestic water demands are estimated below, utilizing parameters from the SDG002 and the GDWS. The following summarizes the parameters used.

Population:

- 100 - 2 Bedroom Apartment x 2.1 person/unit = 210 persons
- Average daily water consumption = 280 L/person/day
- Maximum Day Factor = 2.5 x Avg. Day
- Maximum Hour Factor = 2.2 x Max. Day

The average, maximum day and peak hour domestic (residential) demands for the building are as follows:

- Average Day = 280 x 210 / 86,400 = 0.68 L/sec
- Maximum Day = 2.5 x 0.68 = 1.70 L/sec
- Peak Hour = 2.2 x 1.70 = 3.74 L/sec

Detailed calculations of the domestic water demands are provided in Table B1 of Appendix B.

3.4 Fire Flow Requirements

Water for fire protection will be available utilizing the proposed fire hydrants located along the proposed private roadway. The required fire flows for the proposed site were calculated based on typical values as established by the Fire Underwriters Survey 2020 (FUS). The fire flow requirements were calculated for all blocks. It was determined the required fire flows range from 100 L/sec (6,000 L/min) to 117 L/sec (7,000 L/min).

The following equation from the Fire Underwriters document “Water Supply for Public Fire Protection”, 2020, was used for calculation of the on-site supply rates required to be supplied by the hydrants:

$$F = 200 * C * \sqrt{A}$$

where

- F = Required Fire flow in Litres per minute
- C = Coefficient related to type of Construction
- A = Total Floor Area in square metres

A reduction for low hazard occupancy of -15% for residential dwellings, and an increase for fire area exposure ranging from +26% (min) to +60% (max) was used. Below is a sample calculation of the fire flow requirements for Block 8 (the most critical) residential building.

Required Fire Flow Calculation for Block 8

- Type of Construction = Wood Frame
- Coeff Related to Construction = 1.5
- Ground Floor Area = 174 m²
- Number of Floors = 3

- Fire Flow Requirement, FF = 200 * 1.5 * \sqrt{A}
 = 200 * 1.5 * $\sqrt{174 * 3}$
 = 7,540 L/min or 8,000 L/min (rounded)

Occupancy Class	= Limited Combustible
Occupancy Charge	= -15%
Fire Flow Requirement, FF	= 8,000 *-15% = -1,200 L/min = 6,800 L/min
Sprinkler Protection Credit	= 0%
Charges Due to Exposures	= sum for all sides = 4% + 0% + 0% + 4% = 8%
Required Fire Flow (RFF)	= 6,800 L/min + 544 L/min = 7,344 L/min = 7,000 L/min (rounded to closest 1,000) = 117 L/sec

The following table summarizes the required fire flows for each residential townhome block.

Table 3-1: Summary of Calculated Fire Flow for Each Block

Apartment Block #	Calculated Fire Flow (L/sec)
Block 1	100
Block 2	117
Block 3	100
Block 4	100
Block 5	100
Block 6	100
Block 7	100
Block 8	117

The fire flow requirements for all proposed buildings range from **100 L/sec (6,000 L/min)** to **117 L/sec (7,000 L/min)** based on the FUS. Please refer to tables in Appendix B for detailed calculations using the FUS.

3.5 Boundary Conditions

The development site is in Morgan's Grant pressure zone. Boundary conditions were provided for modelling purposes. Bentley OpenFlows WaterGEMS (CONNECT Edition Update 3) modelling software was used to simulate pressures and flows under maximum day plus fire flow and peak hour conditions.

Boundary conditions were obtained from City of Ottawa personnel for hydraulic modeling. Boundary conditions were used for the connection points at either Connection Location # 1 on Goward Drive (J-105) or Connection Location # 2 (J-170) on Whernside Terrace. Refer to Appendix I for the boundary system information provided by City of Ottawa staff.

Table 3-2: Boundary Conditions Provided by City of Ottawa

Condition	HGL in metres (psi) at Location #1 on Goward Drive (Ground Elev: 101.4m)	HGL at Location #2 on Whernside Terrace (Ground Elev: 102.2m)
Max HGL	150.2m (69.3 psi)	150.2m (68.2 psi)
Peak Hour	140.9m (56.1 psi)	140.9m (55.1 psi)
Max Day plus FF (at 7,000 L/min)	121.2 m (28.2 psi)	128.9 m (38.0 psi)

3.6 Modelling Results

The results of the modelling under the peak hourly condition based on the boundary condition at Location #1, is summarized in Table 3-3 below. Results for both locations #1 and # 2 are included in Appendix D.

Table 3-3: Summary of Results for Peak Hour (Boundary Location #2)

Label	Demand (L/s)	Elevation (m)	Hydraulic Grade (m)	Pressure (psi)
J-100	0.00	100.76	140.87	57
J-105	0.42	101.19	140.87	56
J-106	0.48	101.19	140.87	56
J-110	0.00	101.70	140.86	56
J-115	0.52	102.60	140.86	54
J-120	1.05	103.00	140.86	54
J-125	0.37	103.40	140.86	53
J-130	0.00	103.90	140.86	52
J-135	0.45	104.20	140.86	52
J-140	0.00	103.50	140.86	53
J-145	0.45	102.60	140.86	54
J-150	0.00	102.60	140.86	54
J-155	0.37	102.50	140.86	54
J-160	0.00	102.60	140.86	54
J-165	0.00	101.70	140.87	56
J-170	0.00	101.19	140.87	55
J-175	0.00	101.19	140.87	56
J-180	0.52	103.50	140.51	53
J-185	0.00	101.19	140.87	56

Table 3-4: Summary Results for Maximum Day Plus Fire Flow (Boundary Location #2)

Hydrant Label	Fire Flow (Needed), (L/sec)	Modeled Flow (L/sec)
J-125 (FH-1)	117.0	120.0
J-135 (FH-2)	100.0	117.0
J-145 (FH-3)	100.0	121.5
J-155 (FH-4)	117.0	124.6

The modeled minimum and maximum working pressures anticipated within the development are 52 psi and 56 psi under peak hour conditions. And modeled available fire flows under the maximum day demand conditions range from 117.0 L/sec to 124.6 L/sec.

3.7 Hydrant Spacing

A review of the hydrant spacing was completed to ensure compliance with Appendix I of Technical Bulletin ISTB-2018-02. All hydrants within 150 m were reviewed to assess the total possible contribution of flow. For each hydrant the distance to the proposed building was determined to arrive at the contribution of fire flow from each. All hydrants are expected to be of Class AA as per Section 5.1 of Appendix I.

Table 3-5 below summarizes all fire hydrants within a 150 m distance from each of the residential blocks. For each hydrant the distance measured along a fire route or roadway was used and its contribution to the required fire flow. A detailed table showing the distances and fire flow from each hydrant to each block can be found in Table B11 of Appendix B.

Table 3-5: Summary of Fire Flow Requirements Based on Hydrant Spacing

Hydrant #	Fire Flow Contribution (L/min)							
	Block 1	Block 2	Block 3	Block 4	Block 5	Block 6	Block 7	Block 8
FH-1	5,700	5,700	5,700	5,700	5,700	3,800	3,800	5,700
FH-2	5,700	5,700	3,800	3,800	3,800	3,800	3,800	3,800
FH-3	3,800	3,800	3,800	3,800	3,800	5,700	5,700	3,800
FH-4	3,800	3,800	3,800	5,700	3,800	5,700	5,700	5,700
Total (L/min)	19,000	19,000	19,000	19,000	19,000	19,000	19,000	19,000
FUS RFF in L/min or (L/sec)	6,000 (100)	7,000 (117)	6,000 (100)	6,000 (100)	6,000 (100)	6,000 (100)	6,000 (100)	7,000 (117)
Meets FF Requirement (Yes/No)	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes

From this table the total available contribution of flow from hydrants which are in proximity to each townhome block was estimated at 19,000 L/min. These values exceed the required fire flows for each residential block as identified in Appendix I of Technical Bulletin ISTB-2018-02.

3.8 Water Age Analysis

A review of the age of the water within the proposed system was completed to ensure that an appropriate size of watermain was selected, which was not unnecessarily oversized. The maximum residence time was estimated based on volume of water within the private system between the connection point on Goward Drive and the property boundary of the proposed site. It was assumed that the most conservative approach was to estimate the total volume of water within the water system assuming a dead-ended system. This analysis assumed the entire site at 1158 Old Second Line Road would be fed from the connection point at Goward Drive. This is a conservative approach, as in reality water would be fed from both connection (Goward and Whernside Terrace). The following summarizes the watermain lengths, and volumes used in this analysis:

Total length of 200mm watermains: 508 m
 Total length of 50mm watermain services: 9 m
 Total length of 25mm watermain services: 675 m (50 services at 13.5m avg. length each)

Volume of water within all watermains/services: 16.308 m³ or 16,308 litres

Using the demands in Table B1, the time required for full exhaustion of the 16.3 m³ of water was calculated based on the demands noted in Table B1. In addition, the minimum night demand of 0.068 L/sec was calculated using MOECC Table 3.3 with a minimum peaking factor of 0.10. The following water age estimates are provided in Table 3-6 below.

Table 3-6: Water Age Results

Demand Condition	Demand (L/sec)	Time Required for Full Water Volume Turnover (hours)
Minimum Night	0.068	66.6
Average Day	0.68	6.7
Maximum Day	1.70	2.7
Peak Hour	3.74	1.2

Although a time of 44.4 hours (was calculated based on a minimum demand of 0.068 L/sec), it should be noted that this demand rate would apply only during an 8-hour nighttime period. After the 8-hour nighttime period, an average rate of 0.68 L/sec would apply during the 16-hour daytime. Based on this, the time required for the full exhaustion of 10.9 m³, would approximately 8.0+5.9 = 13.9 hours.

Similarly, there are 15 existing single-family homes on Goward Drive that are located west of the proposed connection point. For this 200mm diameter watermain, the estimated volume is 3.9 m³ based on 10.0m long 19mm diameter services and 123m of 200mm watermain. The time required for the full exhaustion of 3.9 m³, at a minimum night and average day demands of 0.017 and 0.17 L/sec respectively would be approximately 8.0+5.6 = 13.6 hours.

Therefore, the age of the water within the proposed development is expected to be similar to that of the adjacent existing subdivision.

4 Sanitary Sewer Design

4.1 Design Criteria

The sewage flows were calculated using City of Ottawa design criteria as follows:

- Unit Density (2-bedroom apartment) = 2.1 person/unit
- Average Residential Flow Allowance = 280 L/person/day
- Peaking Factor (Harmon Formula) = $1 + 14 / (4 + (P/1000)^{0.5}) * K$
- Correction Factor, K = 0.8
- Full Flow Velocity = 0.60 m/sec to 3.0 m/sec
- Extraneous Flow Allowance = 0.33 L/ha/sec

4.2 Proposed Sanitary Servicing

The sanitary sewer system is designed based on a population flow, and an area-based infiltration allowance. Using the above noted design criteria for the sanitary sewers, the sewage flows were calculated as follows:

Population:

- No of Units: = 100
- Unit Type: = 2-bedroom apartment
- Unit Density = 2.1 person/unit
- 100-2-bedroom apartment x 2.1 person/unit = 210 persons

Sanitary Flow

- Average Residential Flow Allowance = 280 L/person/day
- Correction Factor, K = 0.8
- Peak Factor = $1 + 14 / (4 + (126.9/1000)^{0.5}) * K$ = 3.51
- Avg. Domestic Flow = $210 \times 280 \text{ L/person/day} \times (1/86,400 \text{ sec/day})$ = 0.68 L/sec
- Peak Domestic Flow = $0.68 \text{ L/sec} \times 3.51$ = 2.39 L/sec

Extraneous Flows

- Extraneous Flow Allowance = 0.33 L/ha/sec
- Q Infiltration = $0.33 \text{ L/ha/sec} \times 1.229 \text{ ha}$ = 0.41 L/sec

Total Sewage Flow

- Total Sanitary Flow = $2.39 + 0.40$ = **2.79 L/sec**

The estimated peak sanitary flow rate from the proposed property is **2.79 L/sec** based on City of Ottawa Design Guidelines.

The permitted flow velocities within the sanitary sewer system range between 0.60 m/sec and 3.0 m/sec under full-flow conditions. All new sanitary sewers within the proposed site development will be 200mm in diameter therefore a sewer slope of 0.32% is necessary to meet the minimum velocity requirement of 0.60 m/sec. Similarly, the maximum permitted slope of a 200mm sanitary sewer would be 8.1% to meet the maximum 3.0 m/sec full-flow velocity.

A sanitary sewer design sheet was prepared to confirm the sanitary sewer pipe diameters and full-flow velocities. The selected pipe slopes range from between 0.40% and 2.43%, having full flow velocities in the range of 0.66 m/sec to 1.62 m/sec. The capacities of the sanitary sewers would therefore be between 21.1 L/sec and 51.9 L/sec.

4.3 Downstream Sanitary Sewer System

The proposed sanitary sewer within the development site will discharge to a 200mm sanitary sewer on Goward Drive. This sanitary sewer was installed during the development of Phase 12D of Morgan's Grant Subdivision. The development at 1158 Old Second Line Road falls within Phase 12 of this subdivision.

A review of the sanitary sewer design provided in the 2003 Master Servicing Study, indicated that the original sanitary drainage area and sewage parameters for Phase 12 were based on the following:

Original Morgan's Grant Phase 12 Sanitary Design

Area	= 30.9 ha
Residential Density (Towns & Singles)	= 4.0 person/unit
Population	= 1896 persons
Average Residential Flow Allowance	= 350 L/person/day
Institution Flow Allowance	= 50,000 L/ha/fay
Residential Peaking Factor	= Harmon Formula
Institutional Peaking Factor	= 1.5

In Appendix B of the 2003 Master Servicing Study a sanitary sewer design sheet identifies a total peak flow from Phase 12 of **38.9 L/sec**. The sanitary sewer design sheet from the 2003 MSS is provided in Appendix I, with the specific rows highlighted.

To confirm adequate capacity is available in the downstream system a review of the as-constructed conditions was completed and the peak sewage rates were re-calculated based on current City Guidelines.

Figure A5 in Appendix A illustrates Phase 12 area of Morgan's Grant. It consists of residential, institutional and open space uses. Using the City of Ottawa's urban building GIS layer, it was determined that Phase 12 contains *244 single family*, *112 townhomes*, and one school. The entire area is 30.9 hectares and is made up of 2.90 hectares of institutional, 2.74 hectares of open space, with the remaining 25.26 hectares being residential / municipal roadways.

The sewage flows for Morgan's Grant Phase 12, based on current City Guidelines were re-calculated as follows:

2-bedroom Apartment Unit (Proposed Development)	= 100
Single Family Homes	= 244

Townhomes	= 112
Unit Density (2-bedroom apartment)	= 2.1 person/unit
Unit Density (Single Family Homes)	= 3.4 person/unit
Unit Density (Townhomes)	= 2.7 person/unit
100 - 2 bedroom apartment x 2.1 person/unit	= 210 persons
244 - Single Units x 3.4 person/unit	= 830 persons
112 Townhomes x 2.7 person/unit	= 302 persons
Residential Population = 210 + 830 + 302	= 1342 persons

Residential Sewage Flow

Residential Flow Allowance	= 280 L/person/day
Correction Factor, K	= 0.8
Peak Factor = $1 + (14 / (4 + (P/1000)^{0.5})) * K$	
Peak Factor = $1 + (14 / (4 + (1342/1000)^{0.5})) * 0.8$	
Peak Factor = $1 + (2.79) * 0.8$	= 3.17
Avg. Domestic Flow = $1342 \times 280 \text{ L/person/day} \times (1/86,400 \text{ sec/day})$	= 4.35 L/sec
Peak Domestic Flow = $4.35 \text{ L/sec} \times 3.17$	= 13.79 L/sec

Institutional Sewage Flow

Institutional Flow Allowance	= 28,000 L/day/ha
Institutional Peaking Factor	= 1.5
Peak Institutional Flow = $28,000 \times 2.9 \times (1/86,400 \text{ sec/day}) \times 1.5$	= 1.41 L/sec

Extraneous Flows

Total Area	= 30.9 hectares
Extraneous Flow Allowance	= 0.33 L/ha/sec
Extraneous Flows = (0.33×30.9)	= 10.20 L/sec

Total Sewage Flow

Total Sanitary Flow = $13.79 + 1.41 + 10.20$	= 25.40 L/sec
--	----------------------

The re-calculated peak sewage flows under fully developed conditions for the Phase 12 Morgan's Grant subdivision is calculated to be 25.40 L/sec, which includes 2.79 L/sec sewage flow from the proposed 100-unit (2-bedroom apartment) development at 1158 Old Second Line Road. It should be noted that the original design was completed based on a higher average wastewater flow allowance. The City of Ottawa's residential flow allowance is now 280 L/person/day as per Technical Bulletin ISTB-2018-01. Therefore, the existing infrastructure is conservatively designed in accordance with today's standard guidelines. It can be concluded that the existing sanitary sewer infrastructure in Morgan's Grants Phase 12 subdivision will be adequate to service the additional peak sanitary flows from the 1158 Old Second Line Road development.

5 Stormwater Management

5.1 Design Criteria

The stormwater system was designed in conformance with the latest version of the City of Ottawa Design Guidelines (October 2012). Section 5 “Storm and Combined Sewer Design”, and Section 8 “Stormwater Management” from the design manual were referenced.

5.1.1 Minor System Design Criteria

The proposed storm sewer system includes two sections which are separated by the proposed underground storage chambers. The primary storm sewer section which is upstream of the storage chamber has been designed and sized to accommodate the peak runoffs for the 100-year design storm. The secondary storm sewer section which is downstream of the storage chamber has been designed and sized to accommodate the restricted discharge from the storage chamber plus all foundation drains from the building blocks.

The peak discharge from the development site to the existing storm sewer on Goward Drive is limited to 100 L/sec, based on the capture rate established for this site as per the Stormwater Site Management Plan for Morgan’s Grant Phase 12D.

- The storm sewer within the Morgan’s Grant Subdivision were designed as a minor (pipe) and major drainage (overland) system, or a dual drainage concept. The minor system was designed to convey runoff based on the 5-year storm under free-flow conditions. Inlet control devices (ICD’s) are used within the Morgan’s Grant Subdivision to limit the capture rate to the minor system.
- Hydraulic Grade Line (HGL) Analysis within the Morgan’s Grant Subdivision was prepared based on the 100-year City of Ottawa IDF parameters. The HGL analysis was based on 100-year captured flows.

5.1.2 Major System Design Criteria

- Rear yard ponding is permitted as per City of Ottawa Guidelines, up to a maximum of 300mm in depth, however the volume cannot be accounted for.
- The maximum permitted 100-year ponding depth on the private streets is 350mm.
- The product of the depth of flow x velocity must be less than 0.6 m/sec under the 100-year storm as per City Guidelines.
- Overland Flow is permitted to be discharged to the Hydro corridor, with not more than 126 m³ of runoff from the proposed site, as per the Morgan’s Grant, Phase 12D SWM report.
- The major system (roadway) has been designed to convey surface runoff easterly to the Hydro One corridor.
- A minimum of 150mm of vertical clearance must be provided between the spill elevation on the street and the ground elevation at the building.

5.2 Runoff Coefficients

Average runoff coefficients for all catchments were calculated using PCSWMM’s area weighting routine. This modelling software has a GIS engine which allows for catchment (or polygon) definition including attributes. The runoff coefficients for all catchments were area weighted to derive at average runoff

coefficients based on hard surfaces (concrete or asphalt) having an imperviousness of 95%, soft surfaces (landscaping surfaces) having a percent imperviousness of 5%. The conversion from an imperviousness percent to a runoff coefficient was taken as $C = (IMP \cdot 0.70) / 100 + 0.20$, with the imperviousness (IMP) as a percentage.

The average runoff coefficient for the overall site area under post-development conditions was calculated as 0.63, whereas the pre-development average runoff coefficient was less than 0.10. Runoff coefficients for individual catchment ranged from 0.24 to 0.80.

5.3 Calculation of Allowable Release Rate

To control runoff from the site it will be necessary to limit post-development flows to the allowable capture based on previous Morgan’s Grant, Phase 12D design.

The allowable release rate from the site was set just below the design peak flow rate for the minor system. From the original storm design sheet, the storm sewer was sized based on a 5-year level of service with a runoff coefficient of 0.50 and a time of concentration of 20 minutes. The following parameters will be used to determine the allowable release rates from the proposed site to the existing sewer on Goward Drive, using the Rational Formula.

$$Q_{ALL} = 2.78 C I A$$

where:

- Q_{ALL} = Peak Discharge (L/sec)
- C = Runoff Coefficient ($C=0.50$)
- I = Average Rainfall Intensity for return period (70.25 mm/hr)
 $= 732.951 / (T_c + 6.199)^{0.810}$ (5-year)
- T_c = Time of concentration (20 mins)
- A = Drainage Area (1.20 hectares)

$$Q_{ALL} = 2.78 * C * I * A$$

$$Q_{ALL} = 2.78 * 0.50 * 70.25 * 1.20$$

$$Q_{ALL} = 117.2 \text{ L/sec}$$

The peak design flow, based on the 5-year storm, was estimated at 117.2 L/sec. This peak storm flow was taken from the third row of the original storm design sheet for the Morgan’s Grant Phase 12D, and is attached for reference in Appendix I.

Although the storm sewer system was based on this peak flow, 100 L/sec (or 5 inlets at 20 L/sec/inlet) was used as the minor system capture rate. Since the captured rate of 100 L/sec was used in the Hydraulic Grade Line Analysis for the downstream storm sewers in Morgan’s Grant, the allowable discharge rate to the storm sewer system from the site was limited to 100 L/sec. Runoff in excess of the 100 L/sec capture rate will be detained onsite within underground stormwater storage chambers or will overflow and be stored within the Hydro corridor.

5.4 Pre-Development Conditions

The proposed site is 1.2 hectares in area and is currently undeveloped, except for a single residential home. This home will be demolished for re-development of the site. The topography of the site is generally in an easterly direction, however a small area along Old Second Line flows westerly towards this roadway. A

pre-development drainage plan for the site was prepared using PCSWMM. The watershed delineation routine was used to establish the catchment areas, based on the Digital Raster Acquisition Project of Eastern Ontario (DRAPE) 2m x 2m digital terrain models (DTM). The DTM images were loaded into PCSWMM, and the watershed delineation tool was used to establish overland flow routes and catchments. Engineering drawing C08 illustrates the pre-development catchment for the property, along with the catchment tributary to the culvert at Goward Drive. This catchment was generated to allow for sizing of new culverts under the proposed roadways, and to confirm the allowable discharge rates to Old Second Line Road. The pre-development runoff coefficient for the site was determined to be 0.23.

Using a time of concentration of 20 minutes, an average 2 & 5 -year runoff coefficient of 0.23, and an average 100-year runoff coefficient of 0.29 (increased by 25%) the pre-development release rates from the site were estimated at 40.1 L/sec, 54.2 L/sec and 115.6 L/sec for the 2-year, 5-year and 100-year storms respectively using the Rational Method. Based on drawing C08 the estimated pre-development flows to each outlet (either Old Second Line Road or Hydro Corridor) are summarized in Table 5-1 below. Runoff rates based on the Rational Method and PCSWMM compare well.

Table 5-1: Summary of Pre-Development Peak Flows from Proposed Site

Outlet Location	100-year Pre-Development Peak Flow (L/sec)	
	Rational Method	3hr Chicago Storm (PCSWMM)
To Old Second Line Road	9.3	22.9
To Hydro Corridor	106.3	81.8
Totals	115.6	104.7

5.5 Proposed Stormwater System

Due to the re-development of the site the overall post development runoff coefficient will increase over existing conditions. The increase in runoff is due to an increase in imperviousness levels (additional hard surfaces, roof areas and hard landscaping). The post-development average runoff coefficient for the site was calculated as 0.67, based on an average runoff coefficient of 0.20 for grassed areas and 0.90 for hard surfaces.

The proposed development will consist of eight (8) blocks of stacked townhomes totaling 100 units. The proposed storm sewer includes two sections. The primary storm sewer section, which is upstream of the proposed underground storage chamber, will collect runoff from the restricted onsite drainage catchment and discharge it to the underground storage chamber. The secondary storm sewer section, which is downstream to the proposed underground storage chamber, will receive the restricted discharge from the storage chamber and foundation drains from each building block, and will be directed easterly and then outlet northerly to the existing 450mm and 525mm storm sewers on Goward Drive. The roadways will be sloped easterly with low points being located at the proposed garbage enclosure. Overland flow from the roadways will be directed to this retention area and will spill out to the Hydro corridor under the major storm event beyond the 100-year storm. Engineering drawing C09 illustrates the storm drainage system and associated catchments.

5.6 Design Methodology

The methodology for the design stormwater management portion of the storm sewer system is as follows:

- Place appropriate number and the location of inlets to ensure depth of flow meets City guidelines.

- Meet allowable discharge rate to the Goward Drive Storm sewer of not more than 100 L/sec.
- Ensure no more than 126 m³ of stormwater volume discharges to the Hydro corridor under the 100-year storm event.
- Ensure that resultant 100-year hydraulic grade line does not raise closer than 300mm from the underside of the proposed building footing (USFs).
- Develop a dynamic hydrologic/hydraulic model that provides peak flow hydrographs and water surface profiles which routes runoff from catchments to the outlet.

The site is designed using a dual drainage stormwater model. Dual drainage systems consist of two separate and distinct networks, being a) the minor (or storm sewer) system and b) the major (or street) system. Storm sewer inlets intercept runoff from catchments and links are created between the major system and the minor system.

For this analysis all minor and major system components were included in the PCSWMM model, including inlet control devices (ICDs) in catchbasins, inlet control for catchbasins in flow-by conditions, and storage for catch basins in ponding conditions.

Rating curves were developed for ICD's based on manufactures specifications, for surface and curb-inlet type catchbasins, and for surface ponding areas.

5.7 Storm Sewer Design

Average runoff coefficients were calculated for all drainage areas for sizing of the storm sewers. Post-development drainage areas are illustrated on drawing C09. Average runoff coefficients were calculated for each catchment and inlet times of 10 minutes were used as per City of Ottawa Guidelines.

A minimum 250mm diameter storm sewer is proposed for the main line storm sewer capturing surface runoff. The primary storm sewer sections were sized for the 100-year peak flow and verified in PCSWMM modeling analysis. The secondary storm sewer sections were sized for the estimated foundation drain flows, which was assumed 0.9 L/s for each building block, plus the restricted discharge from the underground storage chamber. All new storm sewers were sized for the 2-year peak flow. Design sheets for the 2-year sizing of the storm sewer system are included in Appendix E.

5.8 Hydrology

PCSWMM was used to create a dual drainage hydrologic/hydraulic model of the storm sewer system. The model accounts for both the minor system (storm sewer) and the major system (roads / swale). Catchbasins were modelled in either a flow-by condition or in a ponding condition. For catchbasins in flow-by conditions inlet capture curves were developed based on the type of curbs used (mountable curb in this case), and the inlet type (surface inlet catchbasins). Ponding areas were modelled as storage nodes with surface ponding represented by area-depth curves above the inlet control devices (ICDs) located at the outlet pipe invert. Calculations of runoff was completed based on the PCSWMM's EPA SWM 5 engine. Catchment parameters were taken from City of Ottawa's SDG002 Design parameters. The following design parameters and assumptions are noted as follows:

- Infiltration losses based on Horton Equation as per City of Ottawa SDG002.
- Impervious and pervious depression storage as per City of Ottawa SDG002.
- 5-year, 3-hour Chicago storm used to review minor system design based on Rational Method.
- 100-year, 3-hour Chicago storm used assess impact of major event and determine peak flows and depth of runoff.
- Runoff coefficient for all subcatchments were determined using area weighting routine and based on actual hard and soft surface areas. Runoff coefficients were calculated from the impervious levels using the relationship $C = (IMP \times 0.7) + 0.20$.
- Subcatchment areas were derived tributary to each surface inlet (catchbasin).
- Subcatchment widths are determined using PCSWMM's SET FLOW LENGTH / WIDTH routine. A Flow-Path layer was created in PCSWMM and flow paths were created for each subcatchment. The software averages the flow path lengths to calculate the subcatchment widths. The width is equal to the subcatchment area divided by the overland flow path length.
- Inflows from all catchbasins were restricted with inlet control devices (ICDs) necessary to ensure allowable capture rate of not more than 100 L/sec was maintained.

5.8.1 Catchment Parameters

Drawing C09 illustrates the post-development storm drainage system. Flow path lengths for each subcatchment were determined based on the average overland flow path length, with the catchment width being the area/length. All subcatchment slopes were set at an average of 4% for front and rear yards. The following summarizes the general subcatchment parameters used:

Table 5-2: General Subcatchment Parameters

Parameter	PCSWMM Parameter	Value
Infiltration Loss Method		Horton
Maximum Infiltration Rate	Max. Infil. Rate	76 mm/hr
Minimum Infiltration Rate	Min. Infil. Rate	13.2 mm/hr
Decay Constant (1/hr)	Decay Constant	4.14
Manning N (Impervious)	N Impev	0.013
Manning N (Pervious)	N Perv	0.25
Depression Storage – Pervious Surfaces	Dstore Imperv	1.57 mm
Depression Storage – Impervious Surfaces	Dstore Perv	4.67 mm
Zero Percent Impervious	Zero Imper	25%
Subcatchment Slopes	Slope	2% - 4% front yards & back yards

Figure 1 and Table 5.3 below presents the individual subcatchment parameters that were developed and used in the PCSWMM model.

Figure 1 - Post Development Catchments



Table 5-3: Post-Development Subcatchment Parameters

Name	Area (ha)	Flow Length (m)	Width (m)	Slope (%)	Imperv. (%)	CAVG	Outlet
U01	0.0403	108.8	3.7	3.8	18.6	0.33	Outfall Second Line
U02	0.0966	123.8	7.8	4	5.7	0.24	CB01
U03	0.0701	89.9	7.8	5.8	5.7	0.24	Outfall Hydro Corridor
A01	0.2586	80.3	32.2	3.2	94.3	0.86	CB03
A02	0.3651	74.9	48.7	3.5	88.6	0.82	CB02
A03	0.1826	110.7	16.5	1.8	100	0.90	CB08
A04	0.1263	47.5	26.6	3.7	25.7	0.38	CB06
A05	0.0612	153	4	2.6	5.7	0.24	CB10

5.8.2 Storage Node Parameters

All catchbasins will be equipped with (ICDs) to ensure that captured flows do not exceed the allowable rate at the outfall, and as a result ponding will occur. All catchbasins were established as storage nodes. All storage nodes use a stage-volume relationship which was assigned based on the maximum depth and area of ponding. For catchbasins in flow-by conditions no storage was assigned above the inlet lid elevation. Rating curves at ponding locations are provided in Table E8 of Appendix E.

Table 5-4: Storage Node Parameters

Name	Invert (m)	Rim* (m)	Depth (m)	Storage Curve	Coeff	Exp	Constant (m ²)	Curve Name
CB01	99.8	101.35	1.55	TABULAR	1000	0	0	CB01_NO_PONDING
CB02	101.09	102.55	1.46	TABULAR	1000	0	0	CB02Ponding
CB03	101.45	103.5	2.05	TABULAR	1000	0	0	CB03Ponding
CB06	101.5	102.78	1.28	TABULAR	1000	0	0	CB06_PONDING
CB08	101.16	102.6	1.44	TABULAR	1000	0	0	CB08_PONDING
CB10	101.4	102.55	1.15	TABULAR	1000	0	0	CB10_PONDING
CB13	99.7	101.25	1.88	TABULAR	1000	0	0	CB13_PONDING
Outfall_Hydro_Corridor	100.8	101	0.2	TABULAR	1000	0	0	PARK-STORAGE

5.8.3 ADS Underground Storage Chamber

An underground storage chamber system is proposed at the middle part of the development site, south of building Block 8, and will collect and detain stormwater runoff conveyed from the primary storm sewers. The size of the underground chambers is large enough to contain the 100-year, 100-year plus 20%, and Historical storm events. The maximum volume of the underground chambers is 361.8 m³ at its maximum spill elevation of 101.86 m, and 509.3 m³ at the top of granular layer elevation of 102.44 m. during the 100-year storm event, the maximum modeled storage volume is ±290 m³ at a depth of 0.884 m, and during the 100-year plus 20% (stress test), the storage would be ±362 m³ at a depth of 1.10 m.

The following summarizes the underground chamber parameters used, with the volumes noted to only include the area above the pond bottom.

- Chamber model: ADS MC-3500
- Number of Chambers: 56
- Number of end of caps: 10

- Voids in the stones (porosity): 40%
- Amount of stone above chambers: 305 mm
- Elevation and area at top of chamber (spill to manhole) = 313.81 m² @ 101.86m
- Amount of stone below chambers: 229 mm
- Elevation and area at base of granular / stone = 239.88 m² @ 100.76m

Table 5-5: Underground Chamber Stage-Storage Data

Depth (m)	Incr Depth (m)	Elevation (m)	Area (m ²)	Incr Vol (m ³)	Cumul Vol (m ³)	Desc
1.676	0.306	102.44	239.88	73.1	509.28	Top of above stone
1.37	0.26	102.13	242.06	120.13	436.18	Top of chamber
1.10	0.871	101.86	313.81	316.05	361.8	Spill Elev
0.229	0.229	100.99	239.88	45.75	54.84	Top of base stone
0.00	0.00	100.76	239.88	6.09	6.09	Bottom Elev

The design information sheet and the modeled stage-storage curve for the underground chambers are included in Appendix E.

The underground storage chambers will not be designed for quality treatment of runoff. Runoff from the storm sewer system will be discharged to the downstream stormwater facility within Phase 12 of Morgan's Grant. The facility was designed for a normal level of water protection (70% TSS).

A copy of certificate of Approval for this stormwater facility is provided in Appendix I for reference. Additionally, the Operation and Maintenance Manual for this stormwater facility was provided by the City of Ottawa, which indicated that water quality sampling performed in 2008, 2009 and 2010, resulting the average TSS removal efficiencies of 91%, 87% and 75 % respectively.

5.8.4 Outlet Node Parameters

The proposed catchbasins and stormwater manholes will not be equipped with no inlet control devices as the design purpose of the primary storm sewer system is to accommodate the peak runoff from the 100-year storm event. There is one exception to this, which is a rear yard catchbasin (CB01) located near the outlet sewer easement, in which the catchbasin (CB01) is equipped with an IPEX Tempest inlet control device (ICD-IPEX-LMF80) which restricts a maximum flow of 3.84 L/s under the 100-year storm event.

A 215 mm diameter orifice plate will be installed in the stormwater manhole (206A) which is downstream of the underground storage chamber, to ensure that the peak discharge rate entering the Goward Drive storm sewer is less than the allowable rate of 100 L/s.

5.9 Dual Drainage Modelling Methodology

5.9.1 Model Development

The subcatchment (or storm drainage areas) were developed in Autodesk CIVIL 3D and imported into PCSWMM. PCSWMM was then used to generate impervious levels for each subcatchment with the area-weighting command. Storm sewers and manholes were imported from CIVIL 3D as GIS shape files and the node and conduit elevations and sizes were inputted based on the preliminary sizing completed with

the Rational Method analysis. Connections between the catchbasin nodes and the sewer main were converted to OUTLETS to represent the ICDs. Once all the minor system components were inputted, the major system was defined connecting inlets.

The major system was represented as irregular conduits based on a half-street cross-section. The transect editor in PCSWMM was used to establish this transect, which was applied to the majority of the major system. In addition, swale and roadway spill irregular transects were used to represent the overland flows. In flow-by conditions all subcatchments were linked to major system nodes place just upstream (u/s) of the catchbasin storage nodes. Between the u/s node and the catchbasins were represented by a PCSWMM OUTLET. These outlets were established with rating curves to represent the approach-flow and depth, and the inlet capture rate. Additional information on the rating curves under flow-by and ponding conditions is provided in proceeding sections of this report.

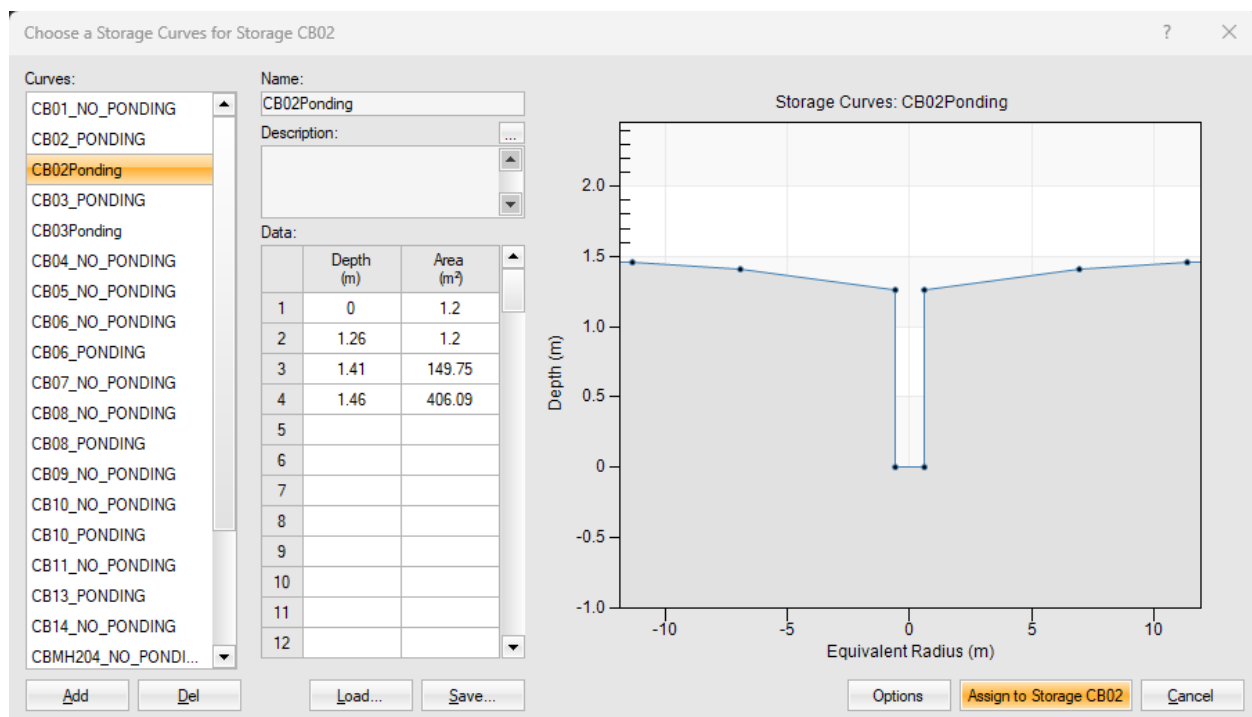
Figure 2 – Model Schematic Showing Minor and Major System Components



Figure 2 above presents a portion of the PCSWMM model which demonstrates the object connectivity. The subcatchment are illustrates as white polygons, with their area number, area in hectares and percent imperviousness labelled. The yellow lines and circles represent the storm sewer system and manholes, with red dashed lines representing the OUTLET links (or ICDs). The dashed blue lines represent the major

system and irregular channels. Catch basins are shown as red squares and looking closely you can see two OUTLETS connecting the CBs to the storm sewer and the major system nodes. Downstream of each CB represent the ICD, whereas upstream of the CB storage nodes the OUTLET represents the inlet capacity. At ponding locations, the storage nodes were defined based on the depth to the ICD. The storage rating curves at each catchbasin was modeled similar to the illustration in Figure 3 below.

Figure 3 – Representation of Rating Curves for Modelling of Storage at Ponding Locations



5.9.2 Storm Events Modelled

A total of seven (7) storm events were modelled, which includes the following:

- 3-hour 2-year Chicago storm (timestep 10 mins)
- 3-hour 5-year Chicago storm (timestep 10 mins)
- 3-hour 100-year Chicago storm (timestep 10 mins)
- 3-hour 100-year + 20% Chicago storm (timestep 10 mins)
- Historical storms occurring July 1, 1979, Aug 4, 1988, August 08, 1996

5.9.3 Inlet Control at Ponding Conditions (Road Sag)

The ponding capture curves are used when an inlet is located in a ponding area. The inlet capacities of the surface inlet catchbasins was derived from Appendix 7-A.9. The design chart provides the capture rates (Qc) of the inlets at various ponding depths. Rating curves for these surface type inlets are based on a standard catchbasin cover openings as per OPSD 400.01 & 400.03.

5.10 Modelling Results

The following summarizes the results of various storm events to ensure the design criteria is met. This includes: 1) total discharge rate to Goward Drive storm sewer is less than 100 L/sec, 2) Total overland flow discharge to Old Second Line Road is less than pre-development levels, and 3) Maximum permitted overflow volume to Hydro Corridor cannot exceed 126 m³. 4) Maximum ponding depth of 300mm.

The results of the PCSWMM model runs are provided in proceeding sections. Table 5-7 below summarizes the peak flows at the storm sewer outfall on Goward Street (City storm manhole no: MHST48926), and to the Old Second Line Road right-of-way.

Table 5-7: Peak Flows at Outfalls

Storm Event	Outfall_Second_Line - Max. Flow (L/sec)	Outfall_Goward_Storm - Max. Flow (L/sec)	Overflow to Hydro Corridor (m ³)
Chicago_3h_2yr	1.63	57.04	0
Chicago_3h_5yr	2.78	69.81	2.1
Chicago_3h_100yr	7.61	97.16	23
Chicago_3h_100yr + 20%	10.52	107.70	38.3
Historic_Jul1-79	6.35	103.00	35.5
Historic_Aug4-88	8.75	96.74	22.5
Historic_Aug8-96	3.73	78.45	10.8

The maximum ponding depths at the catchbasins under the 100-year storm event are presented in Table 5-8.

Table 5-8: Maximum Ponding Depths

Catchbasins	Max Ponding Depth (m) under 100-year
CB01	0.05
CB02	0.14
CB03	0.10
CB06	0.08
CB08	0.07
CB10	0.04
CB13	0.01

5.11 Hydraulics

5.11.1 Hydraulic Grade Line Analysis

A hydraulic grade line (HGL) analysis was completed to confirm the 100-year water surface profile will be at least 300mm below the proposed underside of footing elevations of the units. In addition, the HGL of the 100-year plus 20% (rare event) was completed to ensure that the water surface profile was below the building footings.

The downstream boundary condition within the Goward Drive storm sewer was taken from the "Morgan's Grant – Stage 12" Master Design Sheet, which is included in Appendix I. An additional boundary condition was provided by the City of Ottawa, which was to be used to review that the HGL was below the USFs of the proposed building using a higher boundary condition. This boundary condition was approximately 1.0m

higher than the boundary condition from the Master Design Sheet noted above. The City, based on their own review, estimates the actual HGL to be the higher values due to downstream storm sewer issues. The following summarizes the boundary conditions used.

- 100-yr HGL elevation at STMH 907 (connecting manhole) = 98.22m
- 100-yr + 20% HGL elevation at STMH 907 (connecting manhole) = 99.26m

A steady-state HGL analysis was completed based on free-flow conditions. Free-flow conditions would be a more conservative approach to ensure that the 100-year levels due to surcharging would still have the appropriate clearance to the underside of footing elevations. The steady state analysis was based on the 100-year HGL boundary condition.

Based on this analysis, we can confirm the maximum 100-yr HGL meet the City's clearance requirements. Using captured flows rather than free-flow conditions would result in the HGL being within the storm sewer pipe and not raise above the obvert of the storm sewer system

The HGLs was plotted from PCSWMM, based on 1) the fixed boundary condition of 98.22m for the 100-year storm, and 2) the fixed boundary condition of 99.26m for the 100-year + 20% storm.

A profile from storm manhole 200 through to the outfall on Goward Street is shown below in Figure 4 and Figure 5. It is shown that during the 100-yr event the maximum water surface elevations will remain below the underside of footing (USFs) with at least 300mm clearance, and for the 100-year Plus 20%, the HGL is below the USFs.

Figure 4 – 100-yr HGL Profile West Antelope Private (Boundary HGL 98.22m)

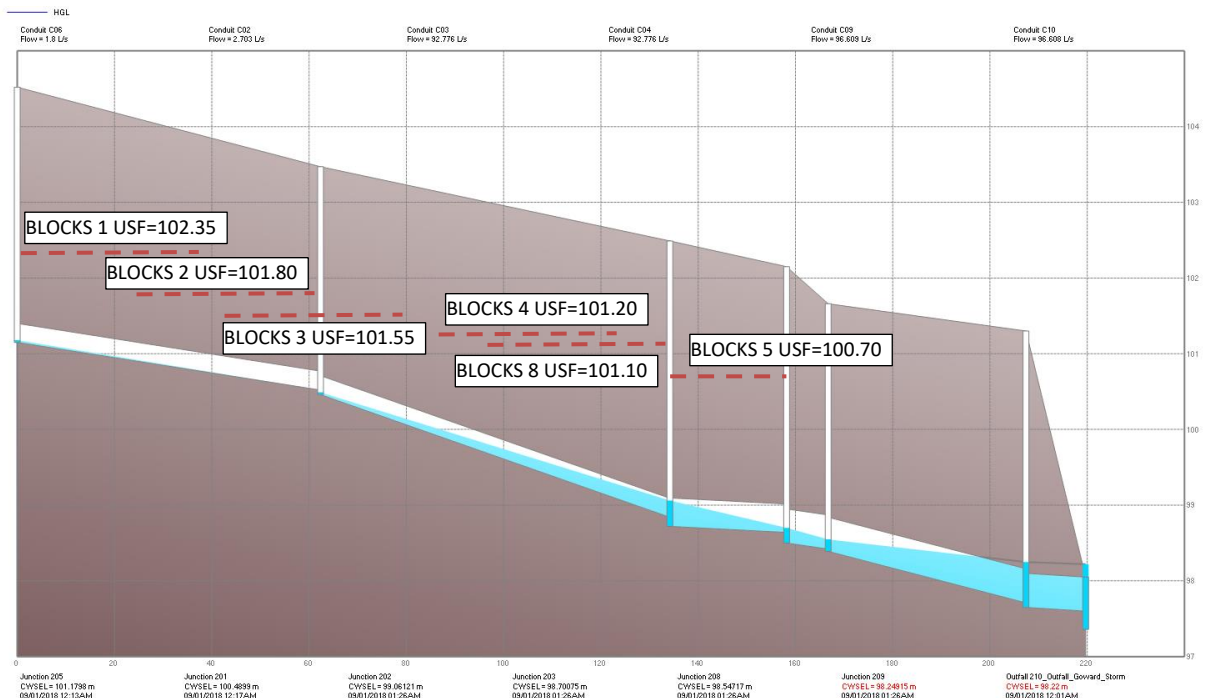


Figure 5 – 100-yr HGL Profile East Antelope Private (Boundary HGL 98.22m)

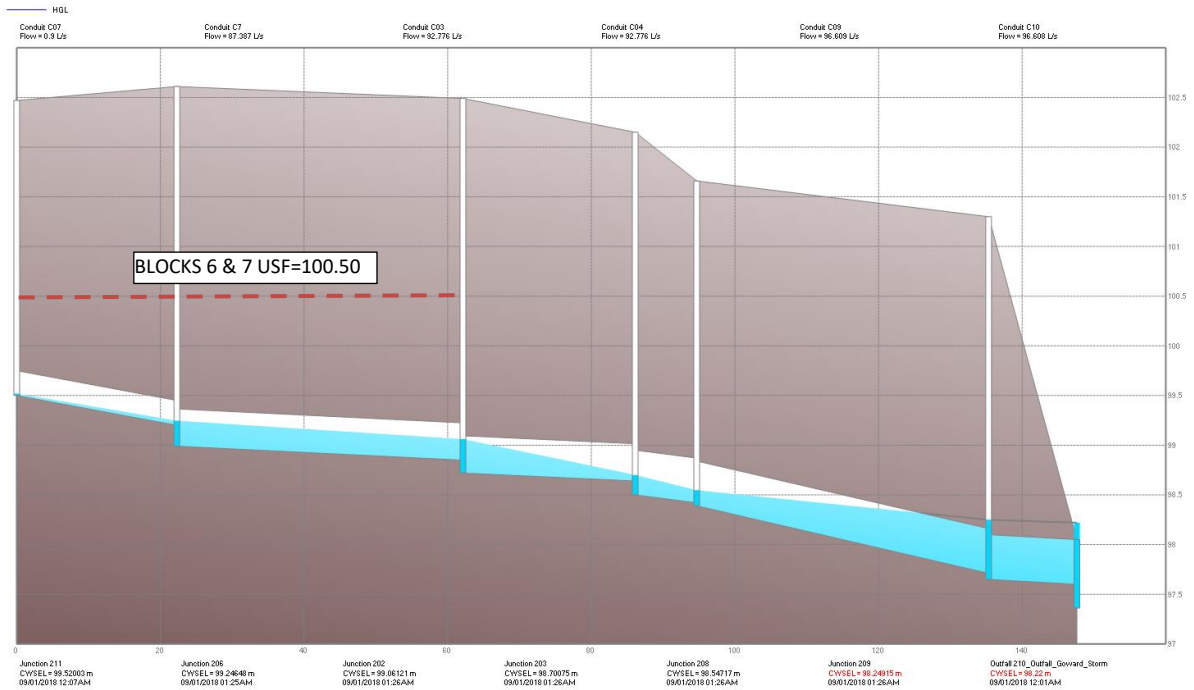


Figure 6 – 100-yr + 20% HGL Profile West Antelope Private (Boundary HGL 99.26m)

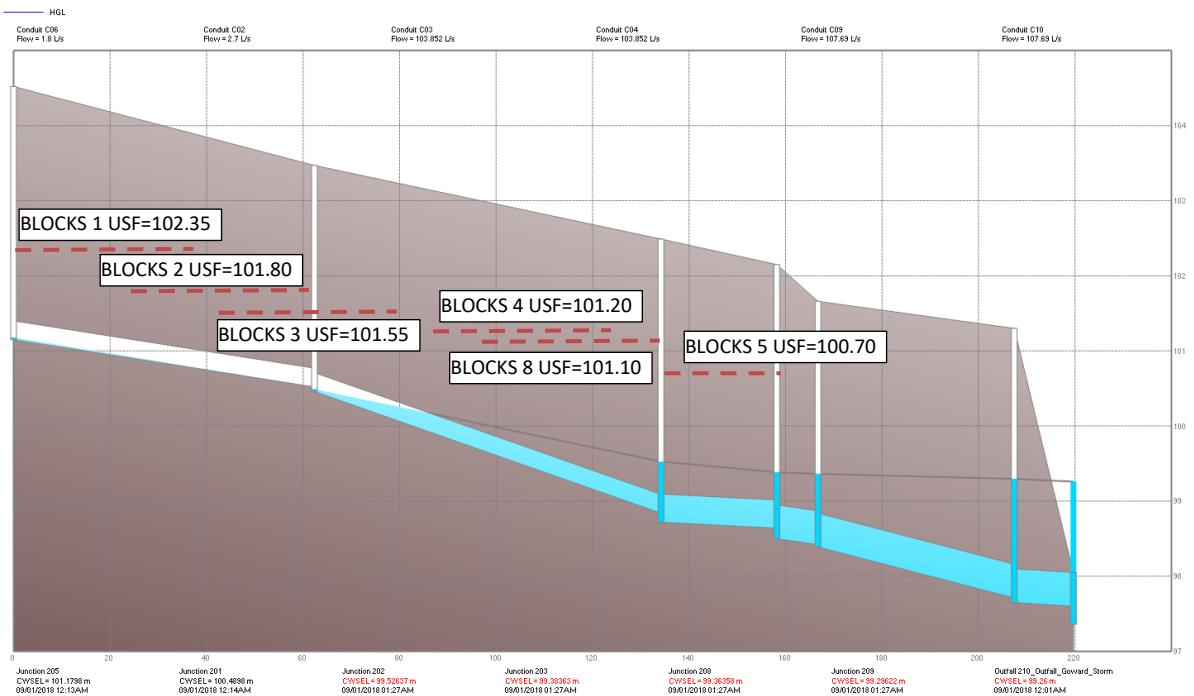
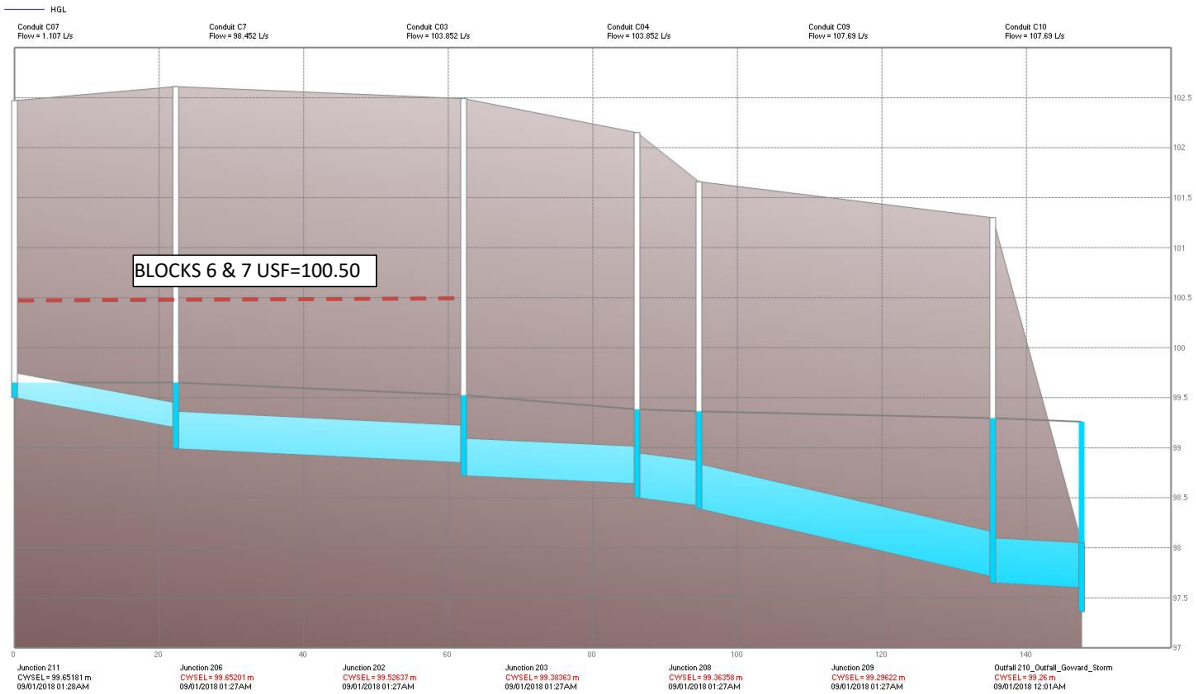


Figure 7 – 100-yr + 20% HGL Profile East Antelope Private (Boundary HGL 99.26m)



5.12 Culvert Sizing

The entrance culvert on Old Second Line Road was designed in accordance with Section 6.4.2 of the City of Ottawa’s Design Guidelines, which states culverts shall be a minimum diameter of 500mm. For the culverts crossing these roadways, any culvert greater than 6 metres in length shall be designed for the 50-year return period for local urban roadways. The culvert is 21 metres in length; therefore, the following summarizes the culvert design requirements:

Peak flows in the ditches under a various storm events were calculated using the pre-development subcatchments that were derived in PPCSWMM and adjusted for post-development conditions. Figure 6 below shows the drainage areas tributary to the proposed culverts and the existing culvert crossing Goward street.

Figure 8 – Subcatchments Used for Sizing Entrance Culverts



Upstream of the proposed culverts under Antelope Private, a ditch inlet structure (City # IN58746) is located on the east side of Old Second Line Road approximately 135 metres south of Antelope Private (south leg). This DICB is equipped with an inlet control device (197mm DIA orifice) with a 300mm outlet pipe discharging to Whernside Terrace storm sewer. The existing ditch south of the subject property is sloped southerly back to the DICB, therefore surface ponding will occur within the ditch. A PCSWMM model was completed to derive peak flows within the east ditch between Klondike Road and Goward Street. Peak flows for the 2yr through 100yr, along with the 100-yr plus 20% and stress test events were run. Existing culvert elevations were taken from documents noted in the Section 1.2. The following summarizes the existing culvert data used.

Culvert Crossing at Goward Street

- U/S Invert = 101.32 m
- D/S Invert = 101.10 m
- Pipe Type = CSP
- Pipe Dia = 400mm

The flowing summarizes the peak flows at three critical locations: 1) Upstream end of proposed 500mm culvert #1 - North leg of Antelope Private, 2) Upstream end of proposed 500mm Culvert #2 - South leg of Antelope Private, and 3) Upstream end of existing 400mm Culvert #3 – crossing Goward Street.

Table 5-6: Peak Flows in East Ditch Along Old Second Line Road.

Storm Event	¹ Peak Flows at Upstream End of Proposed Culvert at Antelope Private – South Leg (L/sec)	² Peak Flows at Upstream End of Proposed Culvert at Antelope Private – North Leg (L/sec)	³ Peak Flows at Downstream End of Proposed Culvert at Goward Street (L/sec)
Chicago 3h 2yr	6.2	18.6	33.2
Chicago 3h 5yr	10.5	31.7	59.9
Chicago 3h 10yr	12.7	38.4	74.6
Chicago 3h 25yr	29.6	46.7	91.8
Chicago 3h 50yr	117.3	112.6	119.6
Chicago 3h 100yr	149.8	169.0	180.5
Chicago 3h 100yr + 20%	224.5	266.8	298.2
Historic Jul1-79	43.9	58.6	76.6
Historic Aug4-88	105.6	66.4	103.1
Historic Aug8-96	12.9	38.6	73.7
1 - Peak flows at Junction J5 of Conduit C1 2 - Peak flows at Junction J3 of Conduit C2 3 - Peak Flows at Junction J1 of Conduit C3			

The 50-year peak flows at proposed Culvert #1, proposed Culvert #2, and existing Culvert #3 were estimated at 117.3 L/sec, 112.6 L/sec and 119.6 L/sec respectively.

Each culvert will convey the 50-year peak flow based on an assumed free boundary condition outfall. Figure below illustrates the HGL within the ditch. One can see that the 50-year and 100-year HGL's are slightly lower than the Historical storm events, however all conveyed within the culverts.

Figure 9 – 50-year and 100-year HGL within East Ditch Along Old Second Line Road.

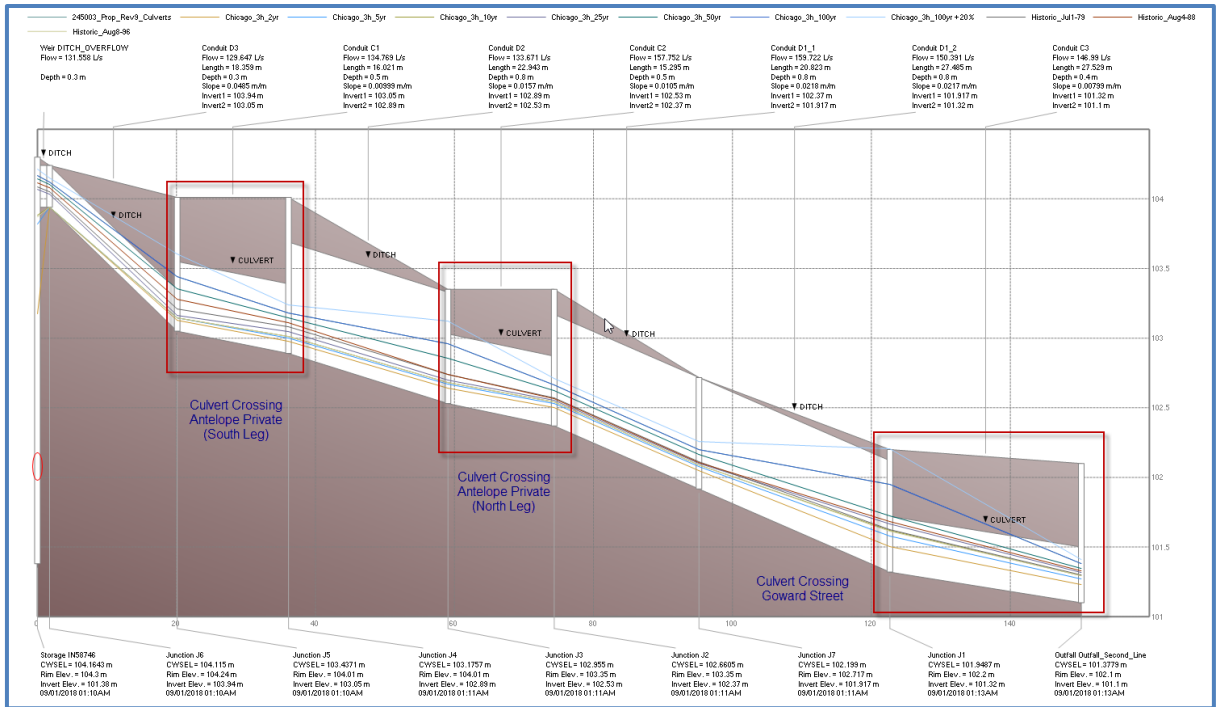
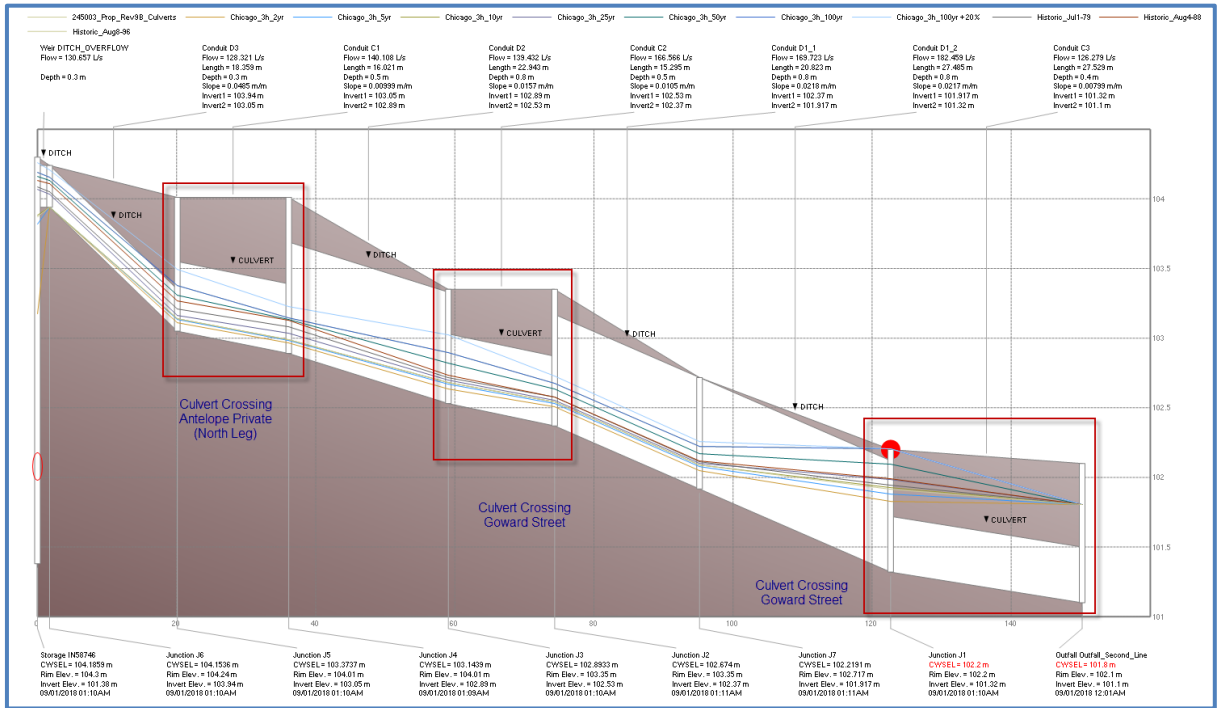


Figure 10 –100-year HGL within East Ditch Using Raised Boundary Condition (0.3m above Obvert)



A secondary review of 100-year HGL within ditch using a fixed outfall of 0.30m above the top of the existing culvert (101.8m) culvert was completed, and illustrated in Figure . Even with culvert at Goward Street modelled having a tailwater condition 0.3m above the top of the outlet pipe, the new proposed culverts will have capacity to convey the peak flows. It should be noted that during the stress test event, the anticipated WSEL will be just at the ground surface (for free outlet conditions) and in a spill condition on the upstream end of the culvert crossing Goward Street. This is a typical condition for culverts sized for storm events less than the 100yr plus 20%.

Due to the minimal cover and shallow depth of the existing ditch, the 500mm culverts were selected. It should be noted that the existing culvert crossing the current residential home on the property is only 400mm in diameter and the downstream culvert crossing Goward Street is also a 400mm diameter.

6 Erosion and Sediment Control

During all construction activities, erosion and sedimentation shall be controlled by the following techniques:

- extent of exposed soils shall be limited at any given time,
- exposed areas shall be re-vegetated as soon as possible,
- filter cloth shall be installed between frame and cover of all new catch basins and catch basin manholes,
- filter cloth shall be installed between frame and cover of the existing catch basins and catch basin manholes as identified on the site grading and erosion control plan,
- light duty silt fencing will be used to control runoff around the construction area. Silt fencing locations are identified on the site grading and erosion control plan.
- visual inspection shall be completed daily on sediment control barriers and any damage repaired immediately. Care will be taken to prevent damage during construction operations,
- In some cases barriers may be removed temporarily to accommodate the construction operations. The affected barriers will be reinstated at night when construction is completed,
- Sediment control devices will be cleaned of accumulated silt as required. The deposits will be disposed of as per the requirements of the contract,
- during the course of construction, if the engineer believes that additional prevention methods are required to control erosion and sedimentation, the contractor will install additional silt fences or other methods as required to the satisfaction of the engineer, and
- Construction and maintenance requirements for erosion and sediment controls are to comply with Ontario Provincial Standard Specification (OPSS) OPSS 805, and City of Ottawa specifications.

7 Conclusions

The proposed 1.2-hectare development by Theberge Homes is comprised of one hundred (100), 3.5 storey stacked units. The following summarizes the servicing and stormwater requirements for the site:

- The allowable capture rate from the proposed site was based on the minor system capture rate established as part of the Morgan's Grant Subdivision Phase 12D which was set at 100 L/sec. This capture rate was set just below the 5-year rate for the 1.2-hectare site using a time of concentration of 20 minutes and a runoff coefficient of 0.50, which was calculated at 117.2 L/sec.
- The 100-year pre-development peak flow rate based on the Rational Method was estimated at 9.3 L/sec and 106.3 L/sec to the Old Second Line ditch and the Hydro Corridor respectively. Dynamic modelling of pre-development flows resulted in peak flows of 22.9 L/sec & 81.8 L/sec for the same storm events.
- A 215 mm diameter orifice plate will be installed at the stormwater manhole downstream of the underground chambers to control runoff to the allowable discharge rate of 86.13 L/s under the 100-year storm. An IPEX Tempest inlet control device (ICD-IPEX-LMF80) will be installed at CB01 to restrict a maximum flow of 3.84 L/s under the 100-year storm event. The peak discharge to the exiting storm sewer on Goward Street (City storm manhole no: MHST48926) is 97.2 L/s, which includes 7.2 L/s foundation flows.
- Based on the dynamic modelling, the total minor system capture rate to the Goward Drive Storm sewer is 57.0 L/sec, 69.8 L/sec and 97.2 L/sec for the 2-year, 5-year and 100-year storm events.
- Underground chambers for stormwater will be used to detain runoff. The underground chambers will be used for quantity control of runoff only and have a volume of 509.3 m³ at a depth of 1.67 metres. The 100-year elevation & volume was calculated at 101.655m and 293.4 m³, whereas the 100-year +20% depth and volume are 101.892 m and 370.5 m³.
- All units have an underside of footing elevation a minimum of 0.30 metres above the storm sewer hydraulic grade line. An overland flow route is provided for the major storm event.
- The proposed development has an estimated peak sewage flow of 2.79 L/sec based on City of Ottawa Guidelines. A new 200mm sewer will be installed with a minimum slope of 0.40% having a full flow capacity of 21.6 L/sec, and full flow velocity of 0.67 m/sec. The sanitary sewer will be connected into the existing municipal sanitary sewer on Goward Drive. A review of the downstream capacity in the sanitary sewers in Morgan's Grant indicate adequate capacity is available.
- A hydraulic water model was developed to determine the pressures available under peak hour and maximum day plus fire flow conditions. Two boundary conditions were provided by City staff for modelling. Two connections to the existing city water distribution system are necessary as there will be more than 50 residential units.
- The calculated minimum and maximum working pressures anticipated within the development is between 52 psi and 56.0 psi under peak hourly conditions. Fire walls are proposed to divide the blocks. The maximum estimated fire flow requirement based on the FUS was calculated at 117 L/sec. townhome block. And modeled available fire flows under the maximum day demand conditions range from 117.0 L/sec to 124.6 L/sec.
- An overland flow route is provided for the major storm event.

Appendix A – Figures

Figure A1: Site Location Plan


Figure A3: Water Model Layout, Boundary Condition #1

Figure A4: Water Model Layout, Boundary Condition #2

Figure A5: Offsite Sanitary Drainage – Morgan’s Grant Phase 12



1158 OLD SECOND
LINE ROAD

exp Services Inc. 100-2650 Queensview Drive Ottawa, ON K2B 8H6 www.exp.com		DESIGN JLF	1158 OLD SECOND LINE ROAD THEBERGE HOMES	SCALE 1:10000
		DRAWN SAB		SKETCH NO
		DATE JAN 2020	SITE LOCATION PLAN	FIG A1
		FILE NO 245003		



BOUNDARY CONDITION #1 ON
GOWARD DRIVE (GROUND
ELEVATION: 101.4 m)

exp Services Inc.
100-2650 Queensview Drive
Ottawa, ON K2B 8H6
www.exp.com



DESIGN	ZP
DRAWN	ZP
DATE	OCT 2023
FILE NO	245003

1158 OLD SECOND LINE RD
THEBERGE HOMES
WATER MODEL LAYOUT
BOUNDARY CONDITION #1

SCALE
1:1250
SKETCH NO
FIG A3



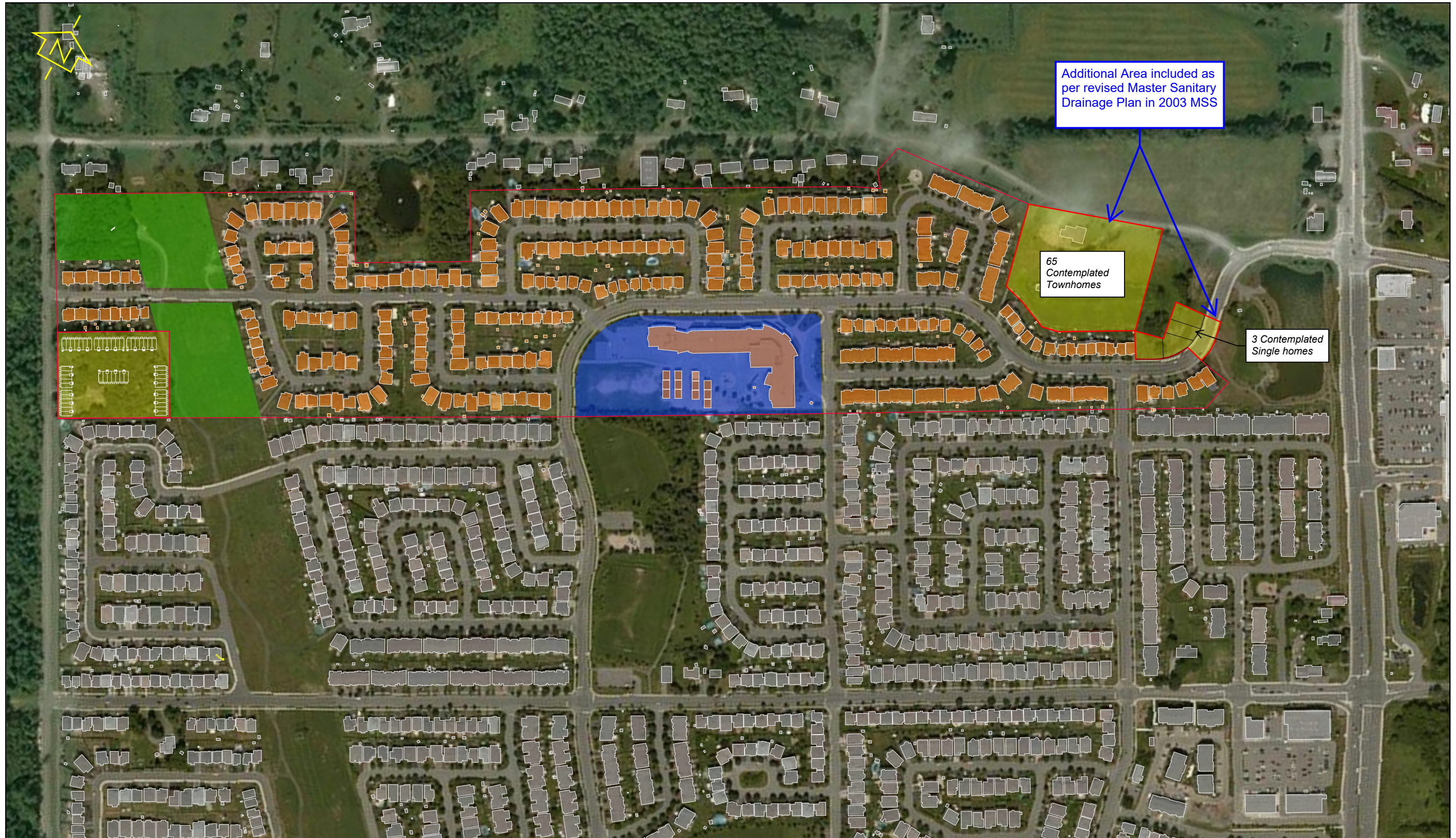
exp Services Inc.
 100-2650 Queensview Drive
 Ottawa, ON K2B 8H6
 www.exp.com



DESIGN	ZP
DRAWN	ZP
DATE	Oct 2023
FILE NO	245003

1158 OLD SECOND LINE RD
 THEBERGE HOMES
 WATER MODEL LAYOUT
 BOUNDARY CONDITION #2

SCALE	1:1250
SKETCH NO	FIG A4



Additional Area included as per revised Master Sanitary Drainage Plan in 2003 MSS

65 Contemplated Townhomes

3 Contemplated Single homes

DESIGN	JLF
DRAWN	SAB
DATE	JAN 2020
FILE NO	245003

1158 OLD SECOND LINE ROAD
 THEBERGE HOMES
 OFFSITE SANITARY DRIANAGE
 MORGAN'S GRANT PHASE 12

Appendix B – Water Tables

Table B1: Water Demand Chart

Table B2: Summary of Required Fire Flows (RFFs)

Table B3: to B27 Calculation of Fire Flow Requirements for Buildings

**TABLE B1
WATER DEMAND CHART**

Location: 1158 Old Second Line		Population Densities															
Project No: OTT-00245003		Single Family	3.4 person/unit														
Designed by: Z. Pan		Semi-Detached	2.7 person/unit														
Checked By: B. Thomas		Duplex	2.3 person/unit														
Date Revised: June 08, 2023		Townhome (Row)	2.7 person/unit														
		Bachelor Apartment	1.4 person/unit														
		1 Bedroom Apartment	1.4 person/unit														
		2 Bedroom Apartment	2.1 person/unit														
		3 Bedroom Apartment	3.1 person/unit														
		Avg. Apartment	1.8 person/unit														
Water Consumption																	
Residential = <u>280</u> L/cap/day																	
Junction	No. of Units									Total Persons (pop)	Average Demand (L/day)	Demands in (L/sec)					
	Singles/Semis/Towns				Apartments							Maximum Demand (L/day)	Peak Hourly Demand (L/day)	Avg Day (L/s)	Max Day (L/s)	Max Hour (L/s)	
	Single Family	Semi-Detached	Duplex	Townhome	Bachelor	1 Bedroom	2 Bedroom	4 Bedroom	Avg Apt.			2.50 x Avg Day	2.20 x Max Day				
Proposed Buildings																	
J-115						14				29.4	8232	20,580	45,276	0.10	0.24	0.52	
J-120							28			58.8	16464	41,160	90,552	0.19	0.48	1.05	
J-125							10			21.0	5880	14,700	32,340	0.07	0.17	0.37	
J-135						12				25.2	7056	17,640	38,808	0.08	0.20	0.45	
J-145						12				25.2	7056	17,640	38,808	0.08	0.20	0.45	
J-155							10			21.0	5880	14,700	32,340	0.07	0.17	0.37	
J-180						14				29.4	8232	20,580	45,276	0.10	0.24	0.52	
Subtotal						100				210.0	58,800	147,000	323,400	0.68	1.70	3.74	
Existing Homes																	
J-105	7									23.8	6664	16,660	36,652	0.08	0.19	0.42	
J-106	8									27.2	7616	19,040	41,888	0.09	0.22	0.48	
Subtotal										51.0	14280	35,700	78,540	0.17	0.41	0.91	
Totals =	15									261.0	73,080	182,700	401,940	0.85	2.11	4.65	

Summary

SUMMARY OF REQUIRED FIREFLOWS (RFFs)

Building #	Fire Flow, F (L/min)	² Type of Constr. Coeff, C	³ Reduction Due to Occupancy (%)	⁴ Reduction Due to Sprinklers (%)	⁵ Total Increase due to Exposures (%)	⁶ Required Fire Flow in	
						(L/min)	(L/sec)
BLOCK 1. Two (2) Northern units of 6 unit block. 3-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-25%	6,000	100
BLOCK 1. Two (2) Central units of 6 unit block. 3-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	0%	5,000	83
BLOCK 1. Two (2) Southern units of 6 unit block. 3-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-4%	5,000	83
BLOCK 2. Two (2) Southern units of 5 unit block. 3-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-22%	6,000	100
BLOCK 2. Two (2) Central units of 5 unit block. 3-storeys. (2 Firewall added), NO sprinklers.	8,000	1.5	-15%	0%	-2%	7,000	117
BLOCK 2. One (1) Northern unit of 5 unit block. 3-storeys. (2 Firewall added), NO sprinklers.	4,000	1.5	-15%	0%	-14%	4,000	67
BLOCK 3. One (1) Western unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	4,000	1.5	-15%	0%	-10%	4,000	67
BLOCK 3. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-11%	6,000	100
BLOCK 3. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-11%	6,000	100
BLOCK 3. Two (2) Eastern unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-24%	6,000	100
BLOCK 4. Two (2) Western units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-24%	6,000	100
BLOCK 4. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-2%	5,000	83
BLOCK 4. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-2%	5,000	83
BLOCK 4. One (1) Eastern unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	4,000	1.5	-15%	0%	-25%	4,000	67
BLOCK 5. One (1) Western unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	4,000	1.5	-15%	0%	-22%	4,000	67
BLOCK 5. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	0%	5,000	83
BLOCK 5. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	0%	5,000	83
BLOCK 5. Two (2) Eastern unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-10%	6,000	100
BLOCK 6. Two (2) Central units of 5 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-4%	5,000	83
BLOCK 6. One (1) Northern unit of 5 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.	4,000	1.5	-15%	0%	-10%	4,000	67
BLOCK 6. Two (2) Southern units of 5 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-22%	6,000	100
BLOCK 7. Two (2) Northern units of 6 unit block. 3.5-storeys. (1 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-22%	6,000	100
BLOCK 7. Two (2) Northern units of 6 unit block. 3.5-storeys. (1 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	0%	5,000	83
BLOCK 7. Two (2) Southern units of 6 unit block. 3.5-storeys. (1 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-11%	6,000	100
BLOCK 8. Three (3) Western unit of 7 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.	8,000	1.5	-15%	0%	-8%	7,000	117
BLOCK 8. Two (2) Central unit of 7 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-2%	5,000	83
BLOCK 8. Two (2) Eastern units of 7 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.	6,000	1.5	-15%	0%	-6%	5,000	83

Notes

1 - If basements are included (<50% below grade) then denoted as +.

2 - Types of constructions: 0.8 for non-combustible, 1.0 for ordinary construction, 1.5 for wood frame construction.

3 - Reductions due to Occupancy are -25% for non-combustible or -15% for limited combustible.

4 - Reductions due to Sprinkler Systems

5 - Increase due to exposures were calculated based on FUS 2020.

6 - Required Fire Flows are rounded to nearest 1,000 L/min.

Min = 67
Max = 117

BLOCK 1-north
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: **BLOCK 1. Two (2) Northern units of 6 unit block. 3-storeys. (2 Firewall added), NO sprinklers.**

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	348.0 m ²	
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,156
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	3	1	0 to 3	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	FW	0	Firewall						0%	25%	1,275
	East	50	6	>45m					0%			
	West	50	6	>45m					0%			
									0%			
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
	Total Required Fire Flow, L/s =										100	

Exposure Charges for Exposing Walls of Wood Frame Constructon (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 1-central
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: BLOCK 1. Two (2) Central units of 6 unit block. 3-storeys. (2 Firewall added), NO sprinklers.



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	348.0 m ²	
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,156
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input						Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)	
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible						-15%	-900	5,100	
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers						0%	0	5,100		
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%	0%	0
	South	FW	0	Firewall						0%		
	East	50	6	>45m						0%		
	West	50	6	>45m						0%		
Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =												
Total Required Fire Flow, L/s =											83	

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 1-south
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: **BLOCK 1. Two (2) Southern units of 6 unit block. 3-storeys. (2 Firewall added), NO sprinklers.**



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	348.0 m ²	
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,156
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%		
	South	22	4	20.1 to 30	Type V	29	2	58	4C	4%	4%	204
	East	50	6	>45m						0%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 2-south
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: **BLOCK 2. Two (2) Southern units of 5 unit block. 3-storeys. (2 Firewall added), NO sprinklers.**



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	351.6 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,188
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%			No Sprinkler	Min =	0			0%	0	5,100
	No Sprinkler	0%				Max =	0					
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%			Not Standard Water Supply or Unavailable					0%	0	5,100
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%			Not Fully Supervised					0%	0	5,100
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers		-25%	Reduction due to Community Sprinklers							0%	0	5,100
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	FW	0	Firewall						0%		
	South	3	1	0 to 3	Type V	12.6	3.5	44.1	1C	22%	22%	1,122
	East	30	4	20.1 to 30	Type V	4	3.5	14	4A	0%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
	Total Required Fire Flow, L/s =										100	

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 2-central
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: **BLOCK 2. Two (2) Central units of 5 unit block. 3-storeys. (2 Firewall added), NO sprinklers.**

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	527.4 m ²	
	Floor 3 and above		176	100%	176			
	Floor 2		176	100%	176			
	Floor 1 (Main Level)		176	100%	176			
	Basement (At least 50% below grade, not included)		176	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							7,579
Fire Flow (F)	Rounded to nearest 1,000							8,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-1,200	6,800
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	6,800	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	6,800	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	6,800	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	6,800	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%	2%	136
	South	FW	0	Firewall						0%		
	East	30	4	20.1 to 30	Type V	9.0	3.5	31.5	4B	2%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										7,000	
											Total Required Fire Flow, L/s =	117

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 2-north
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: **BLOCK 2. One (1) Northern unit of 5 unit block. 3-storeys. (2 Firewall added), NO sprinklers.**



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	175.8 m ²	
	Floor 3 and above		59	100%	59			
	Floor 2		59	100%	59			
	Floor 1 (Main Level)		59	100%	59			
	Basement (At least 50% below grade, not included)		59	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							4,375
Fire Flow (F)	Rounded to nearest 1,000							4,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-600	3,400
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	3,400	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	3,400	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	3,400	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	3,400	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	18.3	3	10.1 to 20	Type V	12.6	3.5	44.1	3C	12%	14%	476
	South	FW	0	Firewall						0%		
	East	30	4	20.1 to 30	Type V	8.1	3.5	28.35	4B	2%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										4,000	
											Total Required Fire Flow, L/s =	67

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 3-west
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: **BLOCK 3. One (1) Western unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.**

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	174.9 m ²		
	Floor 3 and above		58	100%	58			
	Floor 2		58	100%	58			
	Floor 1 (Main Level)		58	100%	58			
	Basement (At least 50% below grade, not included)		58	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						4,364	
Fire Flow (F)	Rounded to nearest 1,000						4,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-600	3,400
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	3,400	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	3,400	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	3,400	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	3,400	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	18.3	3	10.1 to 20	Type V	4.8	2	9.6	4A	0%	10%	340
	East	FW	0	Firewall			3.5	16.8	3A	10%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										4,000	
											Total Required Fire Flow, L/s =	67

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 3-central-1

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 3. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	349.7 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,171
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%			No Sprinkler		Min = 0			0%	0	5,100
	No Sprinkler	0%					Max = 0					
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%			Not Standard Water Supply or Unavailable					0%	0	5,100
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%			Not Fully Supervised					0%	0	5,100
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers		-25%	Reduction due to Community Sprinklers							0%	0	5,100
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	9.8	2	19.6	4A	0%		
	South	18.3	3	10.1 to 20	Type V	9.8	3.5	34.3	3B	11%	11%	561
	East	FW	0	Firewall						0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
											Total Required Fire Flow, L/s =	100

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V Wood Frame
 Type IV-III (U) Mass Timber or Ordinary with Unprotected Openings
 Type IV-III (P) Mass Timber or Ordinary with Protected Openings
 Type II-I (U) Noncombustible or Fire Resistive with Unprotected Openings
 Type II-I (P) Noncombustible or Fire Resistive with Protected Openings
 Firewall Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 3-central-2

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 3. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	349.7 m ²		
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,171	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	19.2	3	10.1 to 20	Type V	9.8	2	19.6	4A	0%	11%	561
	East	FW	0	Firewall						0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
											Total Required Fire Flow, L/s =	100

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V Wood Frame
 Type IV-III (U) Mass Timber or Ordinary with Unprotected Openings
 Type IV-III (P) Mass Timber or Ordinary with Protected Openings
 Type II-I (U) Noncombustible or Fire Resistive with Unprotected Openings
 Type II-I (P) Noncombustible or Fire Resistive with Protected Openings
 Firewall Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 3-east

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 3. Two (2) Eastern unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	349.7 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,171
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)	
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100	
	Limited Combustible	-15%											
	Combustible	0%											
	Free Burning	15%											
	Rapid Burning	25%											
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100		
	No Sprinkler	0%		Max =	0								
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100		
	Not Standard Water Supply or Unavailable	0%											
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100		
	Not Fully Supervised or N/A	0%											
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100		
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	9.8	2	19.6	4A	0%	24%	1,224	
	South	23.5	4	20.1 to 30	Type V	9.8	3.5	34.3	4B	2%			
	East	3	1	0 to 3	Type V	12.2	3.5	42.7	1C	22%			
	West	FW	0	Firewall						0%			
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	Total Required Fire Flow, L/s =	100

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 4-west
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 4. Two (2) Western units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	349.7 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,171
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	21.0	4	20.1 to 30	Type V	9.5	2	19	4A	0%	24%	1,224
	South	23.8	4	20.1 to 30	Type V	9.5	3.5	33.25	4B	2%		
	East	FW	0	Firewall						0%		
	West	3	1	0 to 3	Type V	12.2	3.5	42.7	1C	22%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min = 6,000											
Total Required Fire Flow, L/s = 100												

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 4-central-1

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 4. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	349.7 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,171
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%			No Sprinkler	Min =	0			0%	0	5,100
	No Sprinkler	0%				Max =	0					
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%			Not Standard Water Supply or Unavailable					0%	0	5,100
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%			Not Fully Supervised					0%	0	5,100
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers		-25%	Reduction due to Community Sprinklers							0%	0	5,100
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	23.8	4	20.1 to 30	Type V	9.5	2	19	4A	0%	2%	102
	East	FW	0	Firewall						0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
	Total Required Fire Flow, L/s =										83	

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V Wood Frame
 Type IV-III (U) Mass Timber or Ordinary with Unprotected Openings
 Type IV-III (P) Mass Timber or Ordinary with Protected Openings
 Type II-I (U) Noncombustible or Fire Resistive with Unprotected Openings
 Type II-I (P) Noncombustible or Fire Resistive with Protected Openings
 Firewall Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 4-central-2

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 4. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	349.7 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,171
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	21.0	4	20.1 to 30	Type V	9.5	2	19	4A	0%	2%	102
	South	23.8	4	20.1 to 30	Type V	9.5	3.5	33.25	4B	2%		
	East	FW	0	Firewall						0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V Wood Frame
 Type IV-III (U) Mass Timber or Ordinary with Unprotected Openings
 Type IV-III (P) Mass Timber or Ordinary with Protected Openings
 Type II-I (U) Noncombustible or Fire Resistive with Unprotected Openings
 Type II-I (P) Noncombustible or Fire Resistive with Protected Openings
 Firewall Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 4-east

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: BLOCK 4. One (1) Eastern unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	174.9 m ²	
	Floor 3 and above		58	100%	58			
	Floor 2		58	100%	58			
	Floor 1 (Main Level)		58	100%	58			
	Basement (At least 50% below grade, not included)		58	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							4,364
Fire Flow (F)	Rounded to nearest 1,000							4,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)	
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-600	3,400	
	Limited Combustible	-15%											
	Combustible	0%											
	Free Burning	15%											
	Rapid Burning	25%											
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	3,400		
	No Sprinkler	0%		Max =	0								
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	3,400		
	Not Standard Water Supply or Unavailable	0%											
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	3,400		
	Not Fully Supervised or N/A	0%											
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	3,400		
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)		
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	25%	850	4,250
	South	23.8	4	20.1 to 30	Type V	4.8	2	9.6	4A	0%			
	East	3	1	0 to 3	Type V	4.8	3.5	16.8	4A	0%			
	West	FW	0	Firewall	Type V	29	3.5	101.5	1F	25%			
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										4,000	Total Required Fire Flow, L/s =	67

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 5-west
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 5. One (1) Western unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	174.9 m ²		
	Floor 3 and above		58	100%	58			
	Floor 2		58	100%	58			
	Floor 1 (Main Level)		58	100%	58			
	Basement (At least 50% below grade, not included)		58	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						4,364	
Fire Flow (F)	Rounded to nearest 1,000						4,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-600	3,400
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	3,400	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	3,400	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	3,400	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	3,400	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	21.0	4	20.1 to 30	Type V	4.8	2	9.6	4A	0%	22%	748
	South	23.8	4	20.1 to 30	Type V	4.8	3.5	16.8	4A	0%		
	East	FW	0	Firewall						0%		
	West	3	1	0 to 3	Type V	12.2	3.5	42.7	1C	22%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min = 4,000											
Total Required Fire Flow, L/s = 67												

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 5-central-1

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 5. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	349.7 m ²		
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,171	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	0%	0
	South	50	6	>45m	Type V	23.8	3.5	83.3	6	0%		
	East	FW	0	Firewall						0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V Wood Frame
 Type IV-III (U) Mass Timber or Ordinary with Unprotected Openings
 Type IV-III (P) Mass Timber or Ordinary with Protected Openings
 Type II-I (U) Noncombustible or Fire Resistive with Unprotected Openings
 Type II-I (P) Noncombustible or Fire Resistive with Protected Openings
 Firewall Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 5-central-2

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 5. Two (2) Central units of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	349.7 m ²	
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,171
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	50	6	>45m						0%	0	
	East	FW	0	Firewall					0%			
	West	FW	0	Firewall					0%			
Total Required Fire Flow, Rounded to the Nearest 1,000 L/min = 5,000												
Obtain Required Fire Flow	Total Required Fire Flow, L/s = 83											

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 5-east

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 5. Two (2) Eastern unit of 7 unit block. 3.5-storeys. (3 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	349.7 m ²		
	Floor 3 and above		117	100%	117			
	Floor 2		117	100%	117			
	Floor 1 (Main Level)		117	100%	117			
	Basement (At least 50% below grade, not included)		117	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,171	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	21.0	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	18.2	3	10.1 to 20	Type V	4.8	3.5	16.8	3A	10%	10%	
	East	50	6	>45m						0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
											Total Required Fire Flow, L/s =	100

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 6-north
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: BLOCK 6. One (1) Northern unit of 5 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	174.0 m ²	
	Floor 3 and above		58	100%	58			
	Floor 2		58	100%	58			
	Floor 1 (Main Level)		58	100%	58			
	Basement (At least 50% below grade, not included)		58	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							4,353
Fire Flow (F)	Rounded to nearest 1,000							4,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-600	3,400
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	3,400	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	3,400	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	3,400	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	3,400	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	18.3	3	10.1 to 20	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	FW	0	Firewall						10%	340	3,740
	East	50	6	>45m						0%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										4,000	
											Total Required Fire Flow, L/s =	67

Exposure Charges for Exposing Walls of Wood Frame Constructon (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 6-central
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: **BLOCK 6. Two (2) Central units of 5 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.**

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	348.0 m ²		
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,156	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%	4%	204
	South	FW	0	Firewall						0%		
	East	50	6	>45m						0%		
	West	30	4	20.1 to 30	Type V	12.4	3.5	43.4	4C	4%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Constructon (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 6-south
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: **BLOCK 6. Two (2) Southern units of 5 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.**



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	348.0 m ²	
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,156
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%		
	South	3	1	0 to 3	Type V	12.3	3.5	43.05	1C	22%	22%	1,122
	East	50	6	>45m						0%		
	West	30	4	20.1 to 30	Type V	2.4	3.5	8.4	4A	0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
Total Required Fire Flow, L/s =												100

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 7-north
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: **BLOCK 7. Two (2) Northern units of 6 unit block. 3.5-storeys. (1 Firewall added), NO sprinklers.**



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	348.0 m ²		
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,156	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	3	1	0 to 3	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	FW	0	Firewall						22%	1,122	
	East	50	6	>45m						0%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
											Total Required Fire Flow, L/s =	100

Exposure Charges for Exposing Walls of Wood Frame Constructon (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 7-central
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: **BLOCK 7. Two (2) Central units of 6 unit block. 3.5-storeys. (1 Firewall added), NO sprinklers.**

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	348.0 m ²		
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,156	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%	0%	5,100
	South	FW	0	Firewall						0%		
	East	50	6	>45m						0%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 7-south
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020

Building No: **BLOCK 7. Two (2) Southern units of 6 unit block. 3.5-storeys. (1 Firewall added), NO sprinklers.**



An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	348.0 m ²		
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,156	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	FW	0	Firewall						0%		
	South	12	3	10.1 to 20	Type V	12.3	2	24.6	3B	11%	11%	561
	East	50	6	>45m						0%		
	West	50	6	>45m						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										6,000	
											Total Required Fire Flow, L/s =	100

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 8-west
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 8. Three (3) Western unit of 7 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	522.0 m ²		
	Floor 3 and above		174	100%	174			
	Floor 2		174	100%	174			
	Floor 1 (Main Level)		174	100%	174			
	Basement (At least 50% below grade, not included)		174	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						7,540	
Fire Flow (F)	Rounded to nearest 1,000						8,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-1,200	6,800
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	6,800	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	6,800	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	6,800	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%		Reduction due to Community Sprinklers							0%	0	6,800
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)	Total Charge (%)	Total Exposure Charge (L/min)
	North	23.8	4	20.1 to 30	Type V	14.2	4	56.8	4C	4%	8%	544
	South	48	6	>45m	Type V	14.2	2	28.4	6	0%		
	East	FW	0	Firewall						0%		
	West	29.9	4	20.1 to 30	Type V	12.2	3.5	42.7	4C	4%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min = 7,000											
Total Required Fire Flow, L/s = 117												

Exposure Charges for Exposing Walls of Wood Frame Construcion (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 8-central
FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: **BLOCK 8. Two (2) Central unit of 7 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.**

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where: F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	Comment	348.0 m ²	
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)							6,156
Fire Flow (F)	Rounded to nearest 1,000							6,000

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	23.8	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	50	6	>45m	Type V	9.6	3.5	33.6	4B	2%	2%	102
	East	FW	0	Firewall			2	19.2	6	0%		
	West	FW	0	Firewall						0%		
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

BLOCK 8-east

FIRE FLOW REQUIREMENTS BASED ON FIRE UNDERWRITERS SURVEY(FUS) 2020



Building No: BLOCK 8. Two (2) Eastern units of 7 unit block. 3.5-storeys. (2 Firewall added), NO sprinklers.

An estimate of the Fire Flow required for a given fire area may be estimated by:

$$F = 220 * C * \text{SQRT}(A)$$

where:

F = required fire flow in litres per minute
 A = total floor area in m² (including all storeys, but excluding basements at least 50% below grade)
 C = coefficient related to the type of construction

Task	Options	Multiplier	Input				Value Used	Fire Flow Total (L/min)
Choose Building Frame (C)	Wood Frame	1.5	Wood Frame				1.5	
	Ordinary Construction	1						
	Non-combustible Construction	0.8						
	Fire Resistive Construction	0.6						
Input Building Floor Areas (A)			Area	% Used	Area Used	348.0 m ²		
	Floor 3 and above		116	100%	116			
	Floor 2		116	100%	116			
	Floor 1 (Main Level)		116	100%	116			
	Basement (At least 50% below grade, not included)		116	0%	0			
Fire Flow (F)	F = 220 * C * SQRT(A)						6,156	
Fire Flow (F)	Rounded to nearest 1,000						6,000	

Reductions/Increases Due to Factors Effecting Burning

Task	Options	Multiplier	Input							Value Used	Fire Flow Change (L/min)	Fire Flow Total (L/min)
Choose Combustibility of Building Contents	Non-combustible	-25%	Limited Combustible							-15%	-900	5,100
	Limited Combustible	-15%										
	Combustible	0%										
	Free Burning	15%										
	Rapid Burning	25%										
Choose Reduction Due to Sprinkler System	Adequate Sprinkler Conforms to NFPA13	-30%	No Sprinkler	Min =	0				0%	0	5,100	
	No Sprinkler	0%		Max =	0							
	Standard Water Supply for Fire Department Hose Line and for Sprinkler System	-10%	Not Standard Water Supply or Unavailable						0%	0	5,100	
	Not Standard Water Supply or Unavailable	0%										
	Fully Supervised Sprinkler System	-10%	Not Fully Supervised						0%	0	5,100	
	Not Fully Supervised or N/A	0%										
Reduction for Community Sprinklers	-25%	Reduction due to Community Sprinklers							0%	0	5,100	
Choose Structure Exposure Distance	Exposures	Separation Dist (m)	Cond	Separation Conditon	Exposed Wall type	Exposed Wall Length				Total Charge (%)	Total Exposure Charge (L/min)	
	North	23.8	4	20.1 to 30	Type V	Length (m)	No of Storeys	Length-Height Factor	Sub-Condition	Charge (%)		
	South	50	6	>45m	Type V	9.6	4	38.4	4B	2%	6%	
	East	30	4	20.1 to 30	Type V	9.6	2	19.2	6	0%		
	West	30	4	20.1 to 30	Type V	12.4	4	49.6	4C	4%		
West	FW	0	Firewall						0%			
Obtain Required Fire Flow	Total Required Fire Flow, Rounded to the Nearest 1,000 L/min =										5,000	
											Total Required Fire Flow, L/s =	83

Exposure Charges for Exposing Walls of Wood Frame Construction (from Table G5)

Type V	Wood Frame
Type IV-III (U)	Mass Timber or Ordinary with Unprotected Openings
Type IV-III (P)	Mass Timber or Ordinary with Protected Openings
Type II-I (U)	Noncombustible or Fire Resistive with Unprotected Openings
Type II-I (P)	Noncombustible or Fire Resistive with Protected Openings
Firewall	Firewall

Conditions for Separation

Separation Dist	Condition
0m to 3m	1
3.1m to 10m	2
10.1m to 20m	3
20.1m to 30m	4
> 30.1m	5

LOOKUP CHART - don't erase

Separation Distance (m)	Condition	Separation (metres)		Length-height Factor of exposed wall of adjacent structure (m.storey)		Sub-Condition	Charge				
							Construction of exposed wall of adjacent structure				
							Wood Frame Type V	Mass Timber or Ordinary with Unprotected Openings Type IV-III (U)	Mass Timber or Ordinary with Protected Openings Type IV-III (P)	Noncombustible or Fire Resistive with Unprotected Openings Type II-I (U)	Noncombustible or Fire Resistive with Protected Openings Type II-I (P)
0 to 3	1	0m to 3.0m	0 to 20	1A	20%	15%	5%	10%	0%		
			21 to 40	1B	21%	16%	6%	11%	1%		
			41 to 60	1C	22%	17%	7%	12%	2%		
			61 to 80	1D	23%	18%	8%	13%	3%		
			81 to 100	1E	24%	19%	9%	14%	4%		
			101 to 500	1F	25%	20%	10%	15%	5%		
3.1 to 10	2	3m to 10.0m	0 to 20	2A	15%	10%	3%	6%	0%		
			21 to 40	2B	16%	11%	4%	7%	0%		
			41 to 60	2C	17%	12%	5%	8%	1%		
			61 to 80	2D	18%	13%	6%	9%	2%		
			81 to 100	2E	19%	14%	7%	10%	3%		
			101 to 500	2F	20%	15%	8%	11%	4%		
10.1 to 20	3	10m to 20.0m	0 to 20	3A	10%	5%	0%	3%	0%		
			21 to 40	3B	11%	6%	1%	4%	0%		
			41 to 60	3C	12%	7%	2%	5%	0%		
			61 to 80	3D	13%	8%	3%	6%	1%		
			81 to 100	3E	14%	9%	4%	7%	2%		
			101 to 500	3F	15%	10%	5%	8%	3%		
20.1 to 30	4	20m to 30m to	0 to 20	4A	0%	0%	0%	0%	0%		
			21 to 40	4B	2%	1%	0%	0%	0%		
			41 to 60	4C	4%	2%	0%	1%	0%		
			61 to 80	4D	6%	3%	1%	2%	0%		
			81 to 100	4E	8%	4%	2%	3%	0%		
			101 to 500	4F	10%	5%	3%	4%	0%		
> 30.0	5	30.1m to up	0 to 500	6	0%	0%	0%	0%	0%		

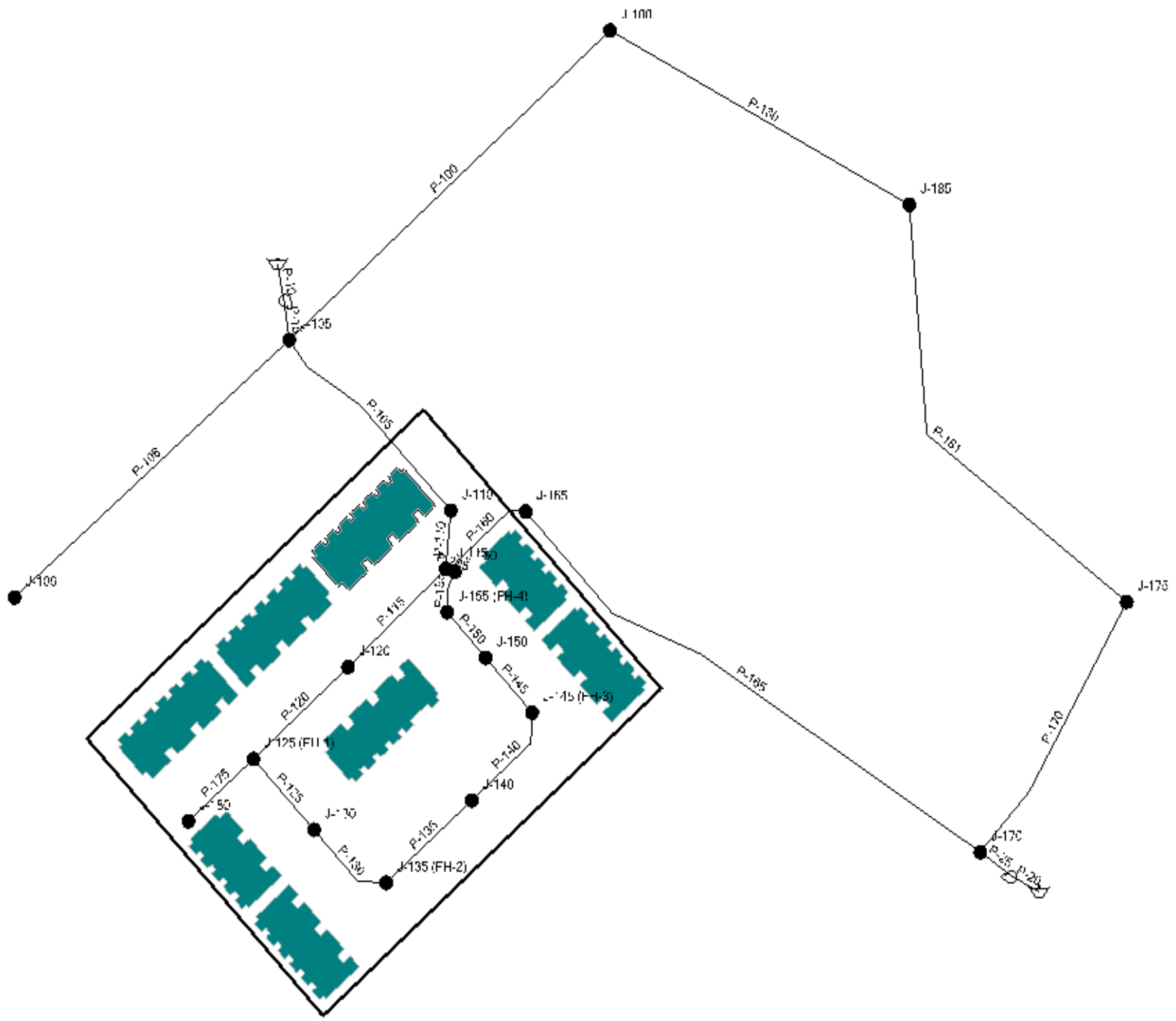
Appendix C – Water Distribution Modelling Results

Boundary Condition 1 Result Tables

- Peak Hour Scenario
 - Junction Table
 - Pipe Table
 - Reservoir Table
- Max Day Plus Fireflow Scenario
 - Junction Table
 - Pipe Table
 - Fire Flow Report
 - Reservoir Table

Boundary Condition 2 Result Tables

- Peak Hour Scenario
 - Junction Table
 - Pipe Table
 - Reservoir Table
- Max Day Plus Fireflow Scenario
 - Junction Table
 - Pipe Table
 - Fire Flow Report
 - Reservoir Table



Hydraulic Model Inventory: WaterGEMS Model_1158 Old Second Line_2023-09-26_New Boundary.wtg

Title
 Engineer
 Company
 Date 6/6/2023
 Notes

Scenario Summary

ID	1
Label	Base
Notes	
Active Topology	Base Active Topology
Physical	Base Physical
Demand	Base Demand
Initial Settings	Base Initial Settings
Operational	Base Operational
Age	Base Age
Constituent	Base Constituent
Trace	Base Trace
Fire Flow	Base Fire Flow
Energy Cost	Base Energy Cost
Transient	Base Transient
Pressure Dependent Demand	Base Pressure Dependent Demand
Failure History	Base Failure History
SCADA	Base SCADA
User Data Extensions	Base User Data Extensions
Steady State/EPS Solver Calculation Options	Base Calculation Options
Transient Solver Calculation Options	Base Calculation Options

Network Inventory

Pipes	24	<None>	1
Laterals	0	-Constant Speed - Four- Quadrant Characteristics	2
Junctions	19	-Constant Speed - Pump Definition	0
Hydrants	0	-Shut Down After Time Delay	0
Tanks	0	-Variable Speed/Torque	0
Reservoirs	2	-Pump Start - Variable Speed/Torque	0
Customer Meters	0	Pump Stations	0
Taps	0	Variable Speed Pump Batteries	0
SCADA Elements	0	PRV's	0
Pumps	2	PSV's	0
-Constant Power	0	PBV's	0
-Custom Extended	0	FCV's	0
-Design Point (1 Point)	1	TCV's	0
-Multiple Point	0	GPV's	0
-Standard (3 Point)	0	Isolation Valves	0
-Standard Extended	0	Spot Elevations	0

Hydraulic Model Inventory: WaterGEMS Model_1158 Old Second Line_2023-09-26_New Boundary.wtg

Transient Network Inventory			
Turbines	0	Rupture Disks	0
Periodic Head-Flows	0	Discharges to Atmosphere	0
Air Valves	0	Orifices Between Pipes	0
Hydropneumatic Tanks	0	Valves With Linear Area Change	0
Surge Valves	0	Surge Tanks	0
Check Valves	0		
Pressure Pipes Inventory			
100.0 (mm)	24 m	350.0 (mm)	222 m
203.0 (mm)	706 m	1,000.0 (mm)	8 m
282.9 (mm)	76 m	All Diameters	1,035 m

FlexTable: Junction Table

Label	Elevation (m)	Unit Demand Collection
J-100	100.76	<Collection: 0 items>
J-105	101.40	<Collection: 1 items>
J-106	101.19	<Collection: 1 items>
J-110	101.70	<Collection: 0 items>
J-115	102.60	<Collection: 1 items>
J-120	103.00	<Collection: 1 items>
J-125 (FH-1)	103.40	<Collection: 1 items>
J-130	103.90	<Collection: 0 items>
J-135 (FH-2)	104.20	<Collection: 1 items>
J-140	103.50	<Collection: 0 items>
J-145 (FH-3)	102.60	<Collection: 1 items>
J-150	102.60	<Collection: 0 items>
J-155 (FH-4)	102.50	<Collection: 1 items>
J-160	102.60	<Collection: 0 items>
J-165	101.70	<Collection: 0 items>
J-170	102.20	<Collection: 0 items>
J-175	101.19	<Collection: 0 items>
J-180	103.50	<Collection: 1 items>
J-185	101.19	<Collection: 0 items>

FlexTable: Pipe Table

Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen-Williams C	Has User Defined Length?	Length (User Defined) (m)
P-175	24	J-125 (FH-1)	J-180	50.0	PVC	100.0	False	0
P-100	118	J-100	J-105	203.0	PVC	110.0	False	0
P-105	63	J-105	J-110	203.0	PVC	110.0	False	0
P-110	15	J-110	J-115	203.0	PVC	110.0	False	0
P-115	37	J-115	J-120	203.0	PVC	110.0	False	0
P-120	35	J-120	J-125 (FH-1)	203.0	PVC	110.0	False	0
P-125	25	J-125 (FH-1)	J-130	203.0	PVC	110.0	False	0
P-130	26	J-130	J-135 (FH-2)	203.0	PVC	110.0	False	0
P-135	31	J-135 (FH-2)	J-140	203.0	PVC	110.0	False	0
P-140	30	J-140	J-145 (FH-3)	203.0	PVC	110.0	False	0
P-145	19	J-145 (FH-3)	J-150	203.0	PVC	110.0	False	0
P-150	16	J-150	J-155 (FH-4)	203.0	PVC	110.0	False	0
P-155	11	J-155 (FH-4)	J-160	203.0	PVC	110.0	False	0
P-160	26	J-160	J-165	203.0	PVC	110.0	False	0
P-165	153	J-165	J-170	203.0	PVC	110.0	False	0
P-205	3	J-115	J-160	203.0	PVC	110.0	False	0
P-106	100	J-105	J-106	203.0	PVC	110.0	False	0
P-170	76	J-170	J-175	282.9	PVC	110.0	False	0
P-180	93	J-100	J-185	350.0	PVC	130.0	False	0
P-181	128	J-185	J-175	350.0	PVC	130.0	False	0
P-10	10	R-1	PMP-1	1,000.0	Ductile Iron	130.0	True	2
P-20	9	R-2	PMP-2	1,000.0	Ductile Iron	130.0	True	2
P-15	10	PMP-1	J-105	1,000.0	Ductile Iron	130.0	True	2
P-25	10	PMP-2	J-170	1,000.0	Ductile Iron	130.0	True	2

FlexTable: Junction Table

Boundary Connection
#1 - PHD Scenario

Label	Elevation (m)	Demand (L/s)	Pressure (psi)
J-100	100.76	0.00	57
J-105	101.40	0.42	56
J-106	101.19	0.48	56
J-110	101.70	0.00	56
J-115	102.60	0.52	54
J-120	103.00	1.05	54
J-125 (FH-1)	103.40	0.37	53
J-130	103.90	0.00	52
J-135 (FH-2)	104.20	0.45	52
J-140	103.50	0.00	53
J-145 (FH-3)	102.60	0.45	54
J-150	102.60	0.00	54
J-155 (FH-4)	102.50	0.37	54
J-160	102.60	0.00	54
J-165	101.70	0.00	56
J-170	102.20	0.00	55
J-175	101.19	0.00	56
J-180	103.50	0.52	53
J-185	101.19	0.00	56

FlexTable: Junction Table

Label	Elevation (m)	Demand (L/s)	Fire Flow (Available) (L/s)
J-100	100.76	0.00	(N/A)
J-105	101.40	0.19	(N/A)
J-106	101.19	0.22	(N/A)
J-110	101.70	0.00	(N/A)
J-115	102.60	0.24	107.29
J-120	103.00	0.48	103.03
J-125 (FH-1)	103.40	0.17	100.10
J-130	103.90	0.00	97.95
J-135 (FH-2)	104.20	0.20	96.16
J-140	103.50	0.00	98.88
J-145 (FH-3)	102.60	0.20	101.56
J-150	102.60	0.00	103.50
J-155 (FH-4)	102.50	0.17	105.40
J-160	102.60	0.00	107.06
J-165	101.70	0.00	(N/A)
J-170	102.20	0.00	(N/A)
J-175	101.19	0.00	(N/A)
J-180	103.50	0.24	(N/A)
J-185	101.19	0.00	(N/A)

Boundary Connection
#1 - Fire Flow Analysis

FlexTable: Junction Table

Label	Elevation (m)	Demand (L/s)	Pressure (psi)
J-100	100.76	0.00	57
J-105	101.40	0.42	56
J-106	101.19	0.48	56
J-110	101.70	0.00	56
J-115	102.60	0.52	54
J-120	103.00	1.05	54
J-125 (FH-1)	103.40	0.37	53
J-130	103.90	0.00	52
J-135 (FH-2)	104.20	0.45	52
J-140	103.50	0.00	53
J-145 (FH-3)	102.60	0.45	54
J-150	102.60	0.00	54
J-155 (FH-4)	102.50	0.37	54
J-160	102.60	0.00	54
J-165	101.70	0.00	56
J-170	102.20	0.00	55
J-175	101.19	0.00	56
J-180	103.50	0.52	53
J-185	101.19	0.00	56

Boundary Connection
#2 - PHD Scenario

FlexTable: Junction Table

Label	Elevation (m)	Demand (L/s)	Fire Flow (Available) (L/s)
J-100	100.76	0.00	(N/A)
J-105	101.40	0.19	(N/A)
J-106	101.19	0.22	(N/A)
J-110	101.70	0.00	(N/A)
J-115	102.60	0.24	125.88
J-120	103.00	0.48	122.64
J-125 (FH-1)	103.40	0.17	120.02
J-130	103.90	0.00	118.12
J-135 (FH-2)	104.20	0.20	117.04
J-140	103.50	0.00	119.31
J-145 (FH-3)	102.60	0.20	121.53
J-150	102.60	0.00	123.09
J-155 (FH-4)	102.50	0.17	124.60
J-160	102.60	0.00	125.95
J-165	101.70	0.00	(N/A)
J-170	102.20	0.00	(N/A)
J-175	101.19	0.00	(N/A)
J-180	103.50	0.24	(N/A)
J-185	101.19	0.00	(N/A)

Boundary Connection
#2 - Fire Flow Analysis

Appendix D – Sanitary Design Sheet

Table D1: Sanitary Design Sheet

**TABLE D1
SANITARY SEWER CALCULATION SHEET**

LOCATION				RESIDENTIAL AREAS AND POPULATIONS											COMMERCIAL			INFILTRATION			SEWER DATA									
Street	U/S MH	D/S MH	Area Number	Area (ha)	NUMBER OF UNITS						POPULATION		Peak Factor	Peak Flow (L/sec)	AREA (ha)		Peak Flow (L/sec)	AREA (ha)		INFILT FLOW (L/s)	TOTAL FLOW (L/s)	Nom Dia (mm)	Actual Dia (mm)	Slope (%)	Length (m)	Capacity (L/sec)	Q/Q _{CAP} (%)	Full Velocity (m/s)	V/V _{FULL}	V _{ACT} (m/s)
					Singles	Semis	Towns	Batch or 1-Bed Apt.	2-Bed Apt.	3-Bed Apt.	Total Units	INDIV			ACCU	INDIV		ACCU												
Antelope Priv	SANMH105	SANMH101	1	0.2323					22		22	46.2	46.2	3.66	0.55			0.2323	0.2323	0.08	0.62	200	203.2	1.05	57.01	35.06	0.02	1.07	0.30	0.32
Antelope Priv	SANMH100	SANMH101	2	0.0941					8		8	16.8	16.8	3.71	0.20			0.0941	0.0941	0.03	0.23	200	203.2	2.09	21.32	49.47	0.00	1.51	0.30	0.45
Antelope Priv	SANMH101	SANMH102	3	0.3970					42		42	88.2	151.2	3.55	1.74			0.3970	0.7234	0.24	1.98	200	203.2	2.23	71.84	51.10	0.04	1.56	0.46	0.72
Antelope Priv	SANMH106	SANMH102	4	0.4072					22		22	46.2	46.2	3.66	0.55			0.4072	0.4072	0.13	0.68	200	203.2	1.09	57.00	35.72	0.02	1.09	0.30	0.33
Antelope Priv	SANMH102	SANMH103	5	0.0982					6		6	12.6	210	3.51	2.39			0.0982	1.2288	0.41	2.79	200	203.2	0.90	25.63	32.46	0.09	0.99	0.61	0.60
Antelope Priv	SANMH103	SANMH108										210	3.51	2.39				1.2288	0.41	2.79	200	203.2	0.92	9.75	32.82	0.09	1.00	0.61	0.61	
	SANMH108	SANMH109										210	3.51	2.39				1.2288	0.41	2.79	200	203.2	1.68	40.46	44.35	0.06	1.35	0.55	0.74	
	SANMH109	EXMH										210	3.51	2.39				1.2288	0.41	2.79	200	203.2	1.25	11.20	38.26	0.07	1.17	0.59	0.69	
				1.229							100	100	210.0				1.229	294.21												

Residential Avg. Daily Flow, q (L/p/day) =	280	Commercial Peak Factor =	1.5 (when area >20%)	Peak Population Flow, (L/sec) =	Persons/Unit	Designed:	Project:
Commercial Avg. Daily Flow (L/gross ha/day) =	28,000		1.0 (when area <20%)	Peak Extraneous Flow, (L/sec) =	3.4	A. Salem, P.Eng.	1158 Second Line
or L/gross ha/sec =	0.324	Institutional Peak Factor =	1.5 (when area >20%)	Residential Peaking Factor, M =	2.7	Checked:	Location:
Institutional Avg. Daily Flow (L/day/ha) =	28,000		1.0 (when area <20%)	A _c = Cumulative Area (hectares)	2.7	B. Thomas, P.Eng.	Ottawa, Ontario
or L/gross ha/day =	0.324	Residential Correction Factor, K =	0.80	P = Population (thousands)	1.4	File Reference:	Page No:
Light Industrial Flow (L/gross ha/day) =	35,000	Manning N =	0.013	Sewer Capacity, Q _{cap} (L/sec) =	2.1	245003 Sanitary Design Sheet, Nov	1 of 1
or L/gross ha/sec =	0.40509	Peak extraneous flow, I (L/s/ha) =	0.33 (Total I/I)	(Manning's Equation)	3.1	14 2023.xlsx	
Light Industrial Flow (L/gross ha/day) =	55,000				3.8		
or L/gross ha/sec =	0.637						

Appendix E – Stormwater Design Sheets

Table E1: 2-year Storm Sewer Calculation Sheet

Table E2: 100-year HGL Storm Sewer Calculation Sheet

Table E3: Average Runoff Coefficients (Pre-Development)

Table E4: Pre-Development Runoff Calculations

Table E5: Allowable Runoff Calculations

Table E6: Average Runoff Coefficients (Post-Development)

Table E7: Summary of Post Development Runoff (Uncontrolled and Controlled)

Table E9: Storage Volumes for 2-year, 5-Year and 100-Year Storms (MRM)

TABLE E1: 2-YEAR STORM SEWER CALCULATION SHEET



Return Period Storm = **2-year** (2-year, 5-year, 100-year)
 Default Inlet Time= 10 (minutes)
 Manning Coefficient = 0.013 (dimensionless)

From Node	To Node	AREA INFO				FLOW (UNRESTRICTED)							INDIV CAP FLOW (L/s)	CUMUL CAP FLOW (L/s)	SEWER DATA											
		Area No.	Area (ha)	Σ Area (ha)	Average R	Indiv. 2.78*A*R	Accum. 2.78*A*R	Tc (mins)	I (mm/h)	Indiv. Flow	Return Period	Q (L/s)			Dia (mm) Actual	Dia (mm) Nominal	Type	Slope (%)	Length (m)	Capacity, Q _{CAP} (L/sec)	Velocity (m/s)		Time in Pipe, Tt (min)	Hydraulic Ratios		
																			Vf	Va		Q/Q _{CAP}	Va/Vf			
CBMH04	CBMH05	A1	0.2590	0.2590	0.86	0.619	0.6192	10.00	76.81	47.56	2-year	47.6	47.56	47.56	381.0	375	CONC	0.53	56.85	133.16	1.16	0.81	1.17	0.36	0.70	
CBMH05	STMMH07	A2	0.3650	0.6240	0.82	0.832	0.8321	10.00	76.81	63.91	2-year	63.9	63.91	63.91	457.0	450	CONC	0.52	30.68	214.23	1.29	0.90	0.57	0.30	0.70	
CB10	CBMH08	A5	0.0610	0.0610	0.24	0.041	0.0407	10.00	76.81	3.13	2-year	3.1	3.13	3.13	254.0	250	PVC	1.39	17.23	73.22	1.43	0.44	0.65	0.04	0.31	
CBMH08	STMMH07	A3	0.1820	0.2430	0.90	0.455	0.4961	10.00	76.81	34.97	2-year	38.1	38.10	38.10	381.0	375	CONC	2.74	9.49	302.77	2.63	1.50	0.11	0.13	0.57	
STMMH07	CHAMBERS			0.8670			1.3281	10.00	76.81		2-year	102.0	102.01	102.01	610.0	600	CONC	1.57	6.37	804.02	2.72	1.55	0.07	0.13	0.57	
CB06	CHAMBERS	A4	0.1260	0.1260	0.38	0.133	0.133	10.00	76.81	10.22	2-year	10.2	10.2	10.22	254.0	250	PVC	1.84	5.00	84.15	1.64	0.94	0.09	0.12	0.57	
STMMH205	STMMH201							10.00	76.81		2-year	1.8	1.8	1.80	254.0	250	PVC	1.02	62.35	62.66	1.22	0.38	2.74	0.03	0.31	
STMMH200	STMMH201							10.00	76.81		2-year	0.45	0.45	2.25	254.0	250	PVC	1.84	24.10	84.15	1.64	0.51	0.79	0.03	0.31	
STMMH201	STMMH202							10.00	76.81		2-year	2.25	2.25	4.50	254.0	250	PVC	2.26	71.84	93.26	1.82	0.56	2.12	0.02	0.31	
STMMH211	STMMH206							10.00	76.81		2-year	0.90	0.9	0.90	254.0	250	PVC	1.12	22.29	65.66	1.28	0.40	0.93	0.01	0.31	
CHAMBERS	STMMH206A			0.9930				10.00	76.81		2-year	49.82	49.8	49.82	305.0	300	PVC	1.94	1.55	140.76	1.91	1.33	0.02	0.35	0.70	
STMMH206A	STMMH206			0.9930				10.00	76.81		2-year			49.82	305.0	300	PVC	1.53	11.10	125.00	1.69	1.20	0.15	0.40	0.71	
STMMH206	STMMH202			0.9930				10.00	76.81		2-year	0.90	0.9	51.62	381.0	375	CONC	0.40	39.84	115.68	1.00	0.71	0.94	0.45	0.71	
STMMH202	STMMH203			0.9930				10.00	76.81		2-year			51.62	457.0	450	CONC	0.33	25.02	170.66	1.03	0.72	0.58	0.30	0.70	
STMMH203	STMMH208			0.9930				10.00	76.81		2-year			51.62	457.0	450	CONC	0.98	8.56	294.10	1.77	1.12	0.13	0.18	0.63	
STMMH208	STMMH209	U2	0.0970	1.0900	0.24	0.065	0.065	10.00	76.81	4.97	2-year	5.0	1.14	52.76	457.0	450	CONC	1.67	40.65	383.92	2.32	1.34	0.50	0.14	0.58	
STMMH209	STMMH210			1.0900				10.00	76.81		2-year			52.76	457.0	450	CONC	0.40	12.28	187.89	1.13	0.79	0.26	0.28	0.70	
TOTALS =			1.0900			2.145																				

Definitions:

Q = 2.78*AIR, where
 Q = Peak Flow in Litres per second (L/s)
 A = Watershed Area (hectares)
 I = Rainfall Intensity (mm/h)
 R = Runoff Coefficients (dimensionless)

Ottawa Rainfall Intensity Values from Sewer Design Guidelines, SDG002

	a	b	c
2-year	732.951	6.199	0.810
5-year	998.071	6.053	0.814
100-year	1735.688	6.014	0.820

Designed:	Project:	
Zhidong Pan, P.Eng.	THEBERGE HOMES	
Checked:	Location:	
B. Thomas, P.Eng.	1158 OLD SECOND LINE ROAD	
Dwg Reference:	File Ref:	Sheet No:
Drawing C09	245003 Storm Design Sheets_2023-11-0.xlsx	1 of 1

TABLE E2: 100-YEAR STORM SEWER CALCULATION SHEET



Return Period Storm = **100-year** (2-year, 5-year, 100-year)
 Default Inlet Time= 10 (minutes)
 Manning Coefficient = 0.013 (dimensionless)

From Node	To Node	AREA INFO				FLOW (UNRESTRICTED)								INDIV CAP FLOW (L/s)	CUMUL CAP FLOW (L/s)	SEWER DATA									
		Area No.	Area (ha)	Σ Area (ha)	Average R	Indiv. 2.78*A*R	Accum. 2.78*A*R	Tc (mins)	I (mm/h)	Indiv. Flow	Return Period	Q (L/s)	Dia (mm) Actual			Dia (mm) Nominal	Type	Slope (%)	Length (m)	Capacity, Q _{CAP} (L/sec)	Velocity (m/s)		Time in Pipe, Tt (min)	Hydraulic Ratios	
																			Vf	Va		Q/Q _{CAP}	Va/Vf		
CBMH04	CBMH05	A1	0.2590	0.2590	0.86	0.619	0.6192	10.00	178.56	110.57	100-year	110.6	110.57	110.57	381.0	375	CONC	0.53	56.85	133.16	1.16	1.16	0.82	0.83	1.00
CBMH05	STMMH07	A2	0.3650	0.6240	0.82	0.832	0.8321	10.00	178.56	148.57	100-year	148.6	148.57	148.57	457.0	450	CONC	0.52	30.68	214.23	1.29	1.23	0.42	0.69	0.95
CB10	CBMH08	A5	0.0610	0.0610	0.24	0.041	0.0407	10.00	178.56	7.27	100-year	7.3	7.27	7.27	254.0	250	PVC	1.39	17.23	73.22	1.43	0.76	0.38	0.10	0.53
CBMH08	STMMH07	A3	0.1820	0.2430	0.90	0.455	0.4961	10.00	178.56	81.31	100-year	88.6	88.58	88.58	381.0	375	CONC	2.74	9.49	302.77	2.63	1.84	0.09	0.29	0.70
STMMH07	CHAMBERS			0.8670			1.3281	10.00	178.56		100-year	237.1	237.15	237.15	610.0	600	CONC	1.57	6.37	804.02	2.72	1.90	0.06	0.29	0.70
CB06	CHAMBERS	A4	0.1260	0.1260	0.38	0.133	0.133	10.00	178.56	23.77	100-year	23.8	23.8	23.77	254.0	250	PVC	1.84	5.00	84.15	1.64	1.15	0.07	0.28	0.70
STMMH205	STMMH201							10.00	178.56		100-year	1.8	1.8	1.80	254.0	250	PVC	1.02	62.35	62.66	1.22	0.38	2.74	0.03	0.31
STMMH200	STMMH201							10.00	178.56		100-year	0.45	0.45	2.25	254.0	250	PVC	1.84	24.10	84.15	1.64	0.51	0.79	0.03	0.31
STMMH201	STMMH202							10.00	178.56		100-year	2.25	2.25	4.50	254.0	250	PVC	2.26	71.84	93.26	1.82	0.56	2.12	0.02	0.31
STMMH211	STMMH206							10.00	178.56		100-year	0.90	0.9	0.90	254.0	250	PVC	1.12	22.29	65.66	1.28	0.40	0.93	0.01	0.31
CHAMBERS	STMMH206A			0.9930				10.00	178.56		100-year	86.68	86.7	86.68	305.0	300	PVC	1.94	1.55	140.76	1.91	1.73	0.01	0.62	0.91
STMMH206A	STMMH206			0.9930				10.00	178.56		100-year			86.68	305.0	300	PVC	1.53	11.10	125.00	1.69	1.61	0.12	0.69	0.95
STMMH206	STMMH202			0.9930				10.00	178.56		100-year	0.90	0.9	88.48	381.0	375	CONC	0.40	39.84	115.68	1.00	0.98	0.67	0.76	0.98
STMMH202	STMMH203			0.9930				10.00	178.56		100-year			88.48	457.0	450	CONC	0.33	25.02	170.66	1.03	0.73	0.57	0.52	0.71
STMMH203	STMMH208			0.9930				10.00	178.56		100-year			88.48	457.0	450	CONC	0.98	8.56	294.10	1.77	1.24	0.11	0.30	0.70
STMMH208	STMMH209	U2	0.0970	1.0900	0.24	0.065	0.065	10.00	178.56	11.56	100-year	11.6	3.84	92.32	457.0	450	CONC	1.67	40.65	383.92	2.32	1.55	0.44	0.24	0.67
STMMH209	STMMH210			1.0900				10.00	178.56		100-year			92.32	457.0	450	CONC	0.40	12.28	187.89	1.13	0.80	0.26	0.49	0.71
TOTALS =		1.0900				2.145																			

Definitions:
 Q = 2.78*AIR, where
 Q = Peak Flow in Litres per second (L/s)
 A = Watershed Area (hectares)
 I = Rainfall Intensity (mm/h)
 R = Runoff Coefficients (dimensionless)

Ottawa Rainfall Intensity Values from Sewer Design Guidelines, SDG002

	a	b	c
2-year	732.951	6.199	0.810
5-year	998.071	6.053	0.814
100-year	1735.688	6.014	0.820

Designed:	Project:	
Zhidong Pan, P.Eng.	THEBERGE HOMES	
Checked:	Location:	
B. Thomas, P.Eng.	1158 OLD SECOND LINE ROAD	
Dwg Reference:	File Ref:	Sheet No:
Drawing C09	245003 Storm Design Sheets_2023-11-0.xlsx	1 of 1



Chamber Model -	MC-3500
Units -	Metric
Number of Chambers -	56
Number of End Caps -	10
Voids in the stone (porosity) -	40 %
Base of Stone Elevation -	100.76 m
Amount of Stone Above Chambers -	305 mm
Amount of Stone Below Chambers -	229 mm

600 sq.meters Min. Area - 272.91 sq.meters

Include Perimeter Stone in Calculations

Click for Stage Area Data

Click to Invert Stage Area Data

[Click Here for Imperial](#)

StormTech MC-3500 Cumulative Storage Volumes

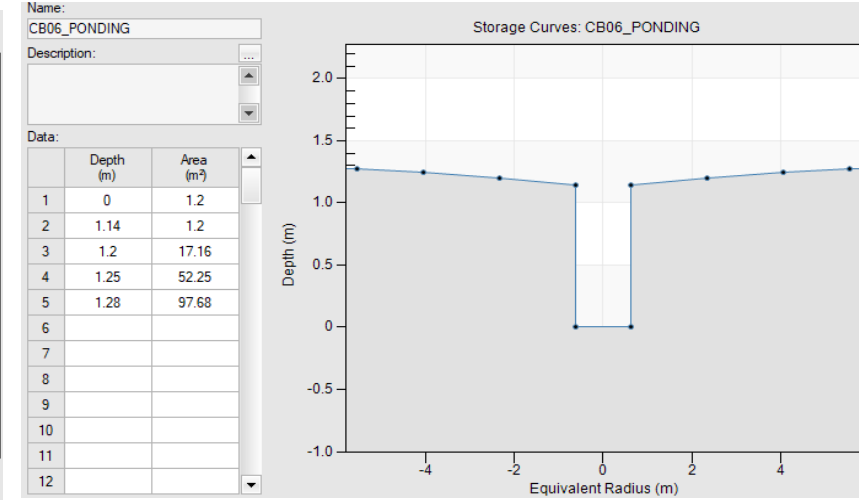
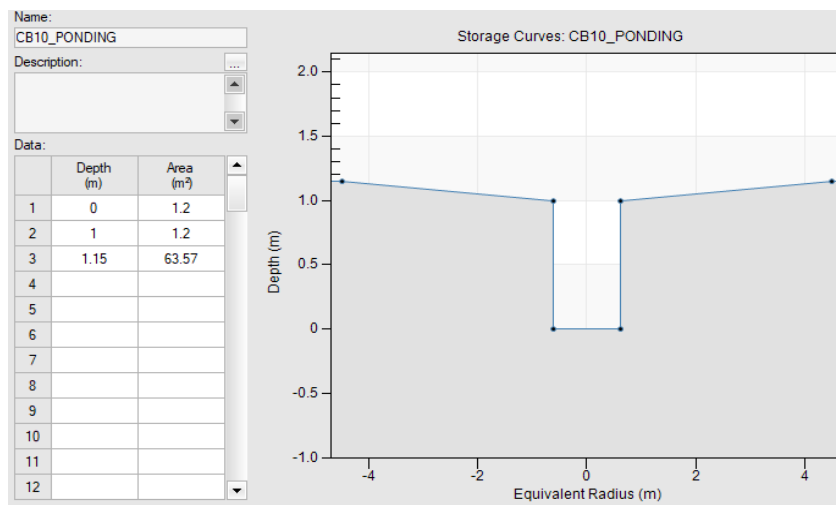
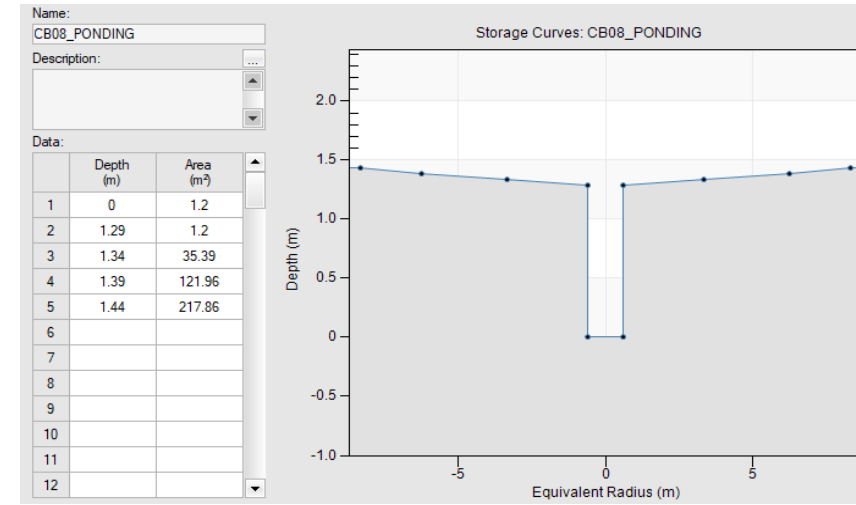
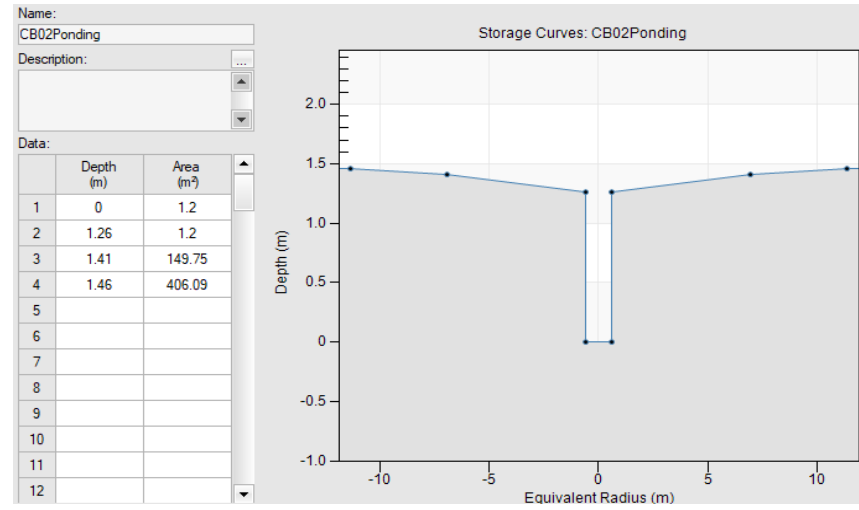
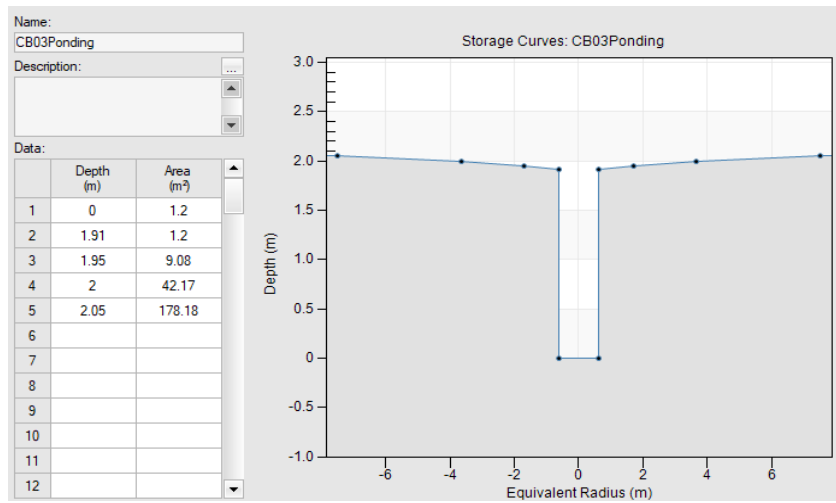
Height of System (mm)	Incremental Single Chamber (cubic meters)	Incremental Single End Cap (cubic meters)	Incremental Chambers (cubic meters)	Incremental End Cap (cubic meters)	Incremental Stone (cubic meters)	Incremental Ch. EC and Stone (cubic meters)	Cumulative System (cubic meters)	Elevation (meters)
1676	0.00	0.00	0.00	0.00	6.093	6.09	509.28	102.44
1651	0.00	0.00	0.00	0.00	6.093	6.09	503.19	102.41
1626	0.00	0.00	0.00	0.00	6.093	6.09	497.10	102.39
1600	0.00	0.00	0.00	0.00	6.093	6.09	491.01	102.36
1575	0.00	0.00	0.00	0.00	6.093	6.09	484.91	102.33
1549	0.00	0.00	0.00	0.00	6.093	6.09	478.82	102.31
1524	0.00	0.00	0.00	0.00	6.093	6.09	472.73	102.28
1499	0.00	0.00	0.00	0.00	6.093	6.09	466.63	102.26
1473	0.00	0.00	0.00	0.00	6.093	6.09	460.54	102.23
1448	0.00	0.00	0.00	0.00	6.093	6.09	454.45	102.21
1422	0.00	0.00	0.00	0.00	6.093	6.09	448.36	102.18
1397	0.00	0.00	0.00	0.00	6.093	6.09	442.26	102.16
1372	0.00	0.00	0.09	0.00	6.056	6.15	436.17	102.13
1346	0.01	0.00	0.31	0.01	5.967	6.28	430.02	102.11
1321	0.01	0.00	0.47	0.01	5.902	6.38	423.74	102.08
1295	0.01	0.00	0.64	0.01	5.831	6.49	417.36	102.06
1270	0.02	0.00	1.09	0.02	5.649	6.76	410.87	102.03
1245	0.03	0.00	1.63	0.02	5.431	7.09	404.12	102.00
1219	0.04	0.00	1.98	0.03	5.288	7.30	397.03	101.98
1194	0.04	0.00	2.26	0.04	5.176	7.47	389.73	101.95
1168	0.04	0.00	2.49	0.04	5.079	7.61	382.26	101.93
1143	0.05	0.00	2.71	0.05	4.992	7.74	374.65	101.90
1118	0.05	0.01	2.90	0.05	4.913	7.86	366.90	101.88
1092	0.05	0.01	3.07	0.06	4.841	7.97	359.04	101.85
1067	0.06	0.01	3.24	0.06	4.774	8.07	351.07	101.83
1041	0.06	0.01	3.39	0.07	4.712	8.16	343.00	101.80
1016	0.06	0.01	3.53	0.07	4.654	8.25	334.83	101.78
991	0.07	0.01	3.66	0.08	4.600	8.33	326.58	101.75
965	0.07	0.01	3.78	0.08	4.549	8.41	318.25	101.73
940	0.07	0.01	3.90	0.08	4.500	8.48	309.84	101.70
914	0.07	0.01	4.01	0.09	4.454	8.55	301.36	101.67
889	0.07	0.01	4.11	0.09	4.411	8.62	292.81	101.65
864	0.08	0.01	4.21	0.09	4.370	8.68	284.19	101.62
838	0.08	0.01	4.31	0.10	4.331	8.74	275.51	101.60
813	0.08	0.01	4.39	0.10	4.294	8.79	266.78	101.57
787	0.08	0.01	4.48	0.11	4.259	8.84	257.99	101.55
762	0.08	0.01	4.56	0.11	4.225	8.89	249.14	101.52
737	0.08	0.01	4.64	0.11	4.193	8.94	240.25	101.50
711	0.08	0.01	4.71	0.12	4.163	8.99	231.31	101.47
686	0.09	0.01	4.78	0.12	4.135	9.03	222.32	101.45
660	0.09	0.01	4.84	0.12	4.108	9.07	213.29	101.42
635	0.09	0.01	4.91	0.12	4.080	9.11	204.22	101.40
610	0.09	0.01	4.96	0.13	4.056	9.15	195.11	101.37
584	0.09	0.01	5.02	0.13	4.033	9.18	185.96	101.34
559	0.09	0.01	5.07	0.13	4.010	9.22	176.78	101.32
533	0.09	0.01	5.12	0.14	3.989	9.25	167.56	101.29
508	0.09	0.01	5.17	0.14	3.969	9.28	158.31	101.27
483	0.09	0.01	5.22	0.14	3.950	9.31	149.03	101.24
457	0.09	0.01	5.26	0.14	3.931	9.34	139.72	101.22
432	0.09	0.01	5.30	0.15	3.913	9.36	130.39	101.19
406	0.10	0.01	5.34	0.15	3.897	9.39	121.02	101.17
381	0.10	0.01	5.38	0.15	3.881	9.41	111.64	101.14
356	0.10	0.02	5.41	0.15	3.866	9.43	102.23	101.12
330	0.10	0.02	5.45	0.15	3.851	9.46	92.79	101.09
305	0.10	0.02	5.48	0.16	3.837	9.48	83.34	101.06
279	0.10	0.02	5.52	0.16	3.823	9.50	73.86	101.04
254	0.10	0.02	5.56	0.17	3.802	9.53	64.37	101.01
229	0.00	0.00	0.00	0.00	6.093	6.09	54.84	100.99
203	0.00	0.00	0.00	0.00	6.093	6.09	48.74	100.96
178	0.00	0.00	0.00	0.00	6.093	6.09	42.65	100.94
152	0.00	0.00	0.00	0.00	6.093	6.09	36.56	100.91
127	0.00	0.00	0.00	0.00	6.093	6.09	30.46	100.89
102	0.00	0.00	0.00	0.00	6.093	6.09	24.37	100.86
76	0.00	0.00	0.00	0.00	6.093	6.09	18.28	100.84
51	0.00	0.00	0.00	0.00	6.093	6.09	12.19	100.81
25	0.00	0.00	0.00	0.00	6.093	6.09	6.09	100.79

Stage Area Data

Depth (meter)	Elevation (meter)	Area (m ²)	Area (hectare)
1.68	102.44	239.88	0.0240
1.65	102.41	239.88	0.0240
1.63	102.39	239.88	0.0240
1.60	102.36	239.88	0.0240
1.57	102.33	239.88	0.0240
1.55	102.31	239.88	0.0240
1.52	102.28	239.88	0.0240
1.50	102.26	239.88	0.0240
1.47	102.23	239.88	0.0240
1.45	102.21	239.88	0.0240
1.42	102.18	239.88	0.0240
1.40	102.16	239.88	0.0240
1.37	102.13	242.06	0.0242
1.35	102.11	247.31	0.0247
1.32	102.08	251.14	0.0251
1.30	102.06	255.34	0.0255
1.27	102.03	266.07	0.0266
1.24	102.00	278.99	0.0279
1.22	101.98	287.40	0.0287
1.19	101.95	294.00	0.0294
1.17	101.93	299.77	0.0300
1.14	101.90	304.92	0.0305
1.12	101.88	309.59	0.0310
1.09	101.85	313.81	0.0314
1.07	101.83	317.79	0.0318
1.04	101.80	321.41	0.0321
1.02	101.78	324.87	0.0325
0.99	101.75	328.07	0.0328
0.97	101.73	331.08	0.0331
0.94	101.70	333.96	0.0334
0.91	101.67	336.64	0.0337
0.89	101.65	339.19	0.0339
0.86	101.62	341.61	0.0342
0.84	101.60	343.91	0.0344
0.81	101.57	346.10	0.0346
0.79	101.55	348.18	0.0348
0.76	101.52	350.16	0.0350
0.74	101.50	352.06	0.0352
0.71	101.47	353.85	0.0354
0.69	101.45	355.52	0.0356
0.66	101.42	357.12	0.0357
0.63	101.40	358.73	0.0359
0.61	101.37	360.16	0.0360
0.58	101.34	361.54	0.0362
0.56	101.32	362.88	0.0363
0.53	101.29	364.12	0.0364
0.51	101.27	365.32	0.0365
0.48	101.24	366.46	0.0366
0.46	101.22	367.55	0.0368
0.43	101.19	368.59	0.0369
0.41	101.17	369.56	0.0370
0.38	101.14	370.50	0.0371
0.36	101.12	371.37	0.0371
0.33	101.09	372.26	0.0372
0.30	101.06	373.08	0.0373
0.28	101.04	373.90	0.0374
0.25	101.01	375.16	0.0375
0.23	100.99	375.88	0.0375
0.20	100.96	376.64	0.0375
0.18	100.94	377.38	0.0375
0.15	100.91	378.08	0.0375
0.13	100.89	378.75	0.0375
0.10	100.86	379.38	0.0375
0.08	100.84	379.98	0.0375
0.05	100.81	380.55	0.0375
0.03	100.79	381.08	0.0375

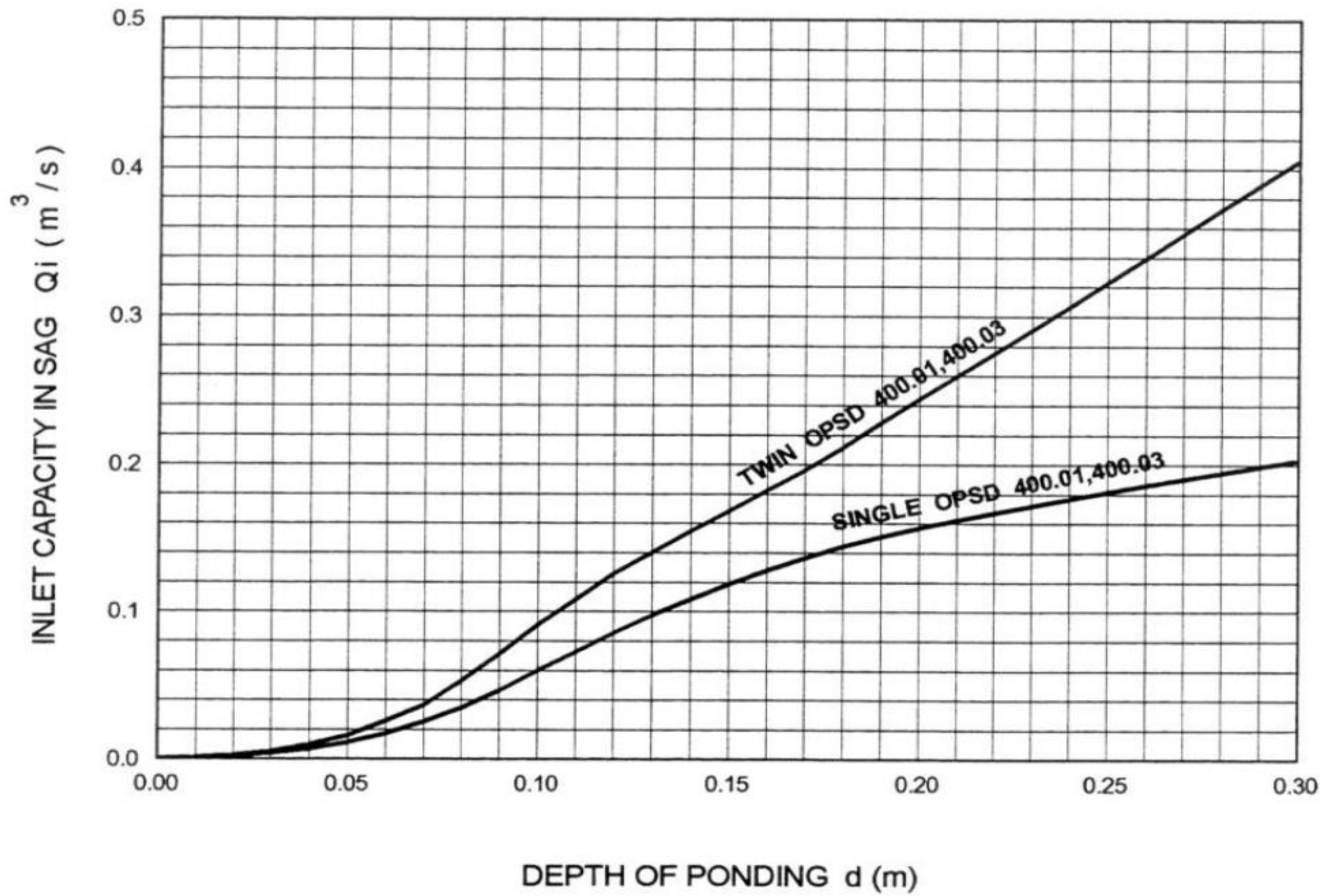
E4 - Catchbasin Ponding Area-Depth Stage Curves

CB	CB03	CBMH04	CB02	CBMH05	CBMH08	CB10	CB06	
RIM (m)	103.43	103.36	102.43	102.35	102.45	102.4	102.64	
INV (m)		101.45		101.09	101.16	101.4	101.5	
CB Depth (m)		1.91		1.26	1.29	1	1.14	
Modeled Depth (m)		2.05		1.46	1.44	1.15	1.28	
Spill elevation (m)		103.5		102.55	102.6	102.55	102.78	
Ponding Elevation (m)	Ponding depth (m)	Area (m2)	Ponding depth (m)	Area (m2)	Ponding depth (m)	Area (m2)	Ponding depth (m)	Area (m2)
101.45	0	1.2	101.09	0	1.2	101.5	0	1.2
103.36	1.91	1.2	102.35	1.26	1.2	102.64	1.14	1.2
103.4	1.95	9.08	102.5	1.41	149.75	102.7	1.2	17.16
103.45	2	42.17	102.55	1.46	406.09	102.75	1.25	52.25
103.5	2.05	178.18			102.6	1.44	217.86	97.68



E5 - Sag Inlet Capacity Curves

DEPTH OF PONDING (m)	INLET CAPACITY IN SAG (L/s)	DEPTH OF PONDING (m)	INLET CAPACITY IN SAG (L/s)	DEPTH OF PONDING (m)	INLET CAPACITY IN SAG (L/s)
0	0	0	0	0	0
0.022	1	0.022	2	0.022	2
0.032	3	0.032	6	0.039	8
0.043	7	0.043	14	0.052	16
0.06	16	0.06	32	0.067	31
0.072	26	0.072	52	0.074	41
0.081	34	0.081	68	0.091	70
0.097	55	0.097	110	0.102	92
0.115	77	0.115	154	0.116	118
0.128	93	0.128	186	0.135	145
0.151	116	0.151	232	0.163	183
0.161	127	0.161	254	0.181	210
0.183	143	0.183	286	0.219	269
0.201	154	0.201	308	0.25	319
0.22	164	0.22	328	0.277	363
0.243	174	0.243	348	0.294	390
0.261	183	0.261	366	0.301	402
0.281	191	0.281	382		
0.3	198	0.3	396		



Appendix F

SWM Modelling Results

00245003_Old Second Line_PCSWMM_Post-Development_2023-10-27
Input File (Details)
Output File (Status)

245003_Prop_Cond_Rev9_Culverts
Input File (Details)
Output File (Status)

[TITLE]
 DUAL DRAINAGE MODEL (Boundary HGL = 98.22)
 REVISED DUE TO CITY REVIEW MEMO OFF AUG 28, 2023
 By Zhidong Pan
 October 30, 2023

[OPTIONS]
 ;;Options Value
 ;;-----
 FLOW_UNITS LPS
 INFILTRATION HORTON
 FLOW_ROUTING DYNWAVE
 LINK_OFFSETS ELEVATION
 MIN_SLOPE 0
 ALLOW_PONDING YES
 SKIP_STEADY_STATE NO

START_DATE 09/01/2018
 START_TIME 00:00:00
 REPORT_START_DATE 09/01/2018
 REPORT_START_TIME 00:00:00
 END_DATE 09/01/2018
 END_TIME 09:00:00
 SWEEP_START 01/01
 SWEEP_END 12/31
 DRY_DAYS 0
 REPORT_STEP 00:01:00
 WET_STEP 00:01:00
 DRY_STEP 00:01:00
 ROUTING_STEP 1

INERTIAL_DAMPING PARTIAL
 NORMAL_FLOW_LIMITED BOTH
 FORCE_MAIN_EQUATION H-W
 VARIABLE_STEP 0.75
 LENGTHENING_STEP 1
 MIN_SURFAREA 0
 MAX_TRIALS 8
 HEAD_TOLERANCE 0.0015
 SYS_FLOW_TOL 5
 LAT_FLOW_TOL 5
 MINIMUM_STEP 1
 THREADS 4

[EVAPORATION]
 ;;Type Parameters
 ;;-----
 CONSTANT 0.0
 DRY_ONLY NO

[RAINGAGES]
 ;; Rain Time Snow Data
 ;;Name Type Intrvl Catch Source
 ;;-----
 Chicago_3h_100yr INTENSITY 0:10 1.0 TIMESERIES Chicago_3h_100yr
 Chicago_3h_100yr_+20% INTENSITY 0:10 1.0 TIMESERIES Chicago_3h_100yr_+20%
 Chicago_3h_2yr INTENSITY 0:10 1.0 TIMESERIES Chicago_3h_2yr

```

Chicago_3h_5yr      INTENSITY 0:10    1.0    TIMESERIES Chicago_3h_5yr
Historic_1979-07-01 INTENSITY 0:05    1.0    TIMESERIES Historic_1979_July_01
Historic_1988-08-04 INTENSITY 0:05    1.0    TIMESERIES
Historic_1988_August_04
Historic_1996-08-08 INTENSITY 0:05    1.0    TIMESERIES
Historic_1996_August_08

```

[SUBCATCHMENTS]

```

;;
Curb      Snow
;;Name    Raingage      Outlet      Total      Pcnt.      Pcnt.
Length    Pack
;;-----
-----
A01      Chicago_3h_100yr CB03      0.2586    94.3      32.2      3.2
0
A02      Chicago_3h_100yr CB02      0.3651    88.6      48.7      3.5
0
A03      Chicago_3h_100yr CB08      0.1826    100       16.5      1.8
0
A04      Chicago_3h_100yr CB06      0.1263    25.7      26.6      3.7
0
A05      Chicago_3h_100yr CB10      0.0612    5.7       4         2.6
0
U01      Chicago_3h_100yr Outfall_Second_Line 0.0403    18.6      3.7      3.8
0
U02      Chicago_3h_100yr CB01      0.0966    5.7       7.8      4
0
U03      Chicago_3h_100yr CB13      0.0701    5.7       7.8      5.8
0

```

[SUBAREAS]

```

;;Subcatchment  N-Imperv  N-Perv  S-Imperv  S-Perv  PctZero  RouteTo
PctRouted
;;-----
-----
A01      0.013    0.25    1.57      4.67    25       OUTLET
A02      0.013    0.25    1.57      4.67    25       OUTLET
A03      0.013    0.25    1.57      4.67    25       OUTLET
A04      0.013    0.25    1.57      4.67    25       OUTLET
A05      0.013    0.25    1.57      4.67    25       OUTLET
U01      0.013    0.25    1.57      4.67    25       OUTLET
U02      0.013    0.25    1.57      4.67    25       OUTLET
U03      0.013    0.25    1.57      4.67    25       OUTLET

```

[INFILTRATION]

```

;;Subcatchment  MaxRate  MinRate  Decay  DryTime  MaxInfil
;;-----
-----
A01      76.2     13.2     4.14   7        0
A02      76.2     13.2     4.14   7        0
A03      76.2     13.2     4.14   7        0
A04      76.2     13.2     4.14   7        0
A05      76.2     13.2     4.14   7        0
U01      76.2     13.2     4.14   7        0
U02      76.2     13.2     4.14   7        0
U03      76.2     13.2     4.14   7        0

```

[JUNCTIONS]

```

;;      Invert      Max.      Init.      Surcharge  Poned

```


;;Name	Elev.	Depth	Depth	Depth	Area
200	100.96	2.58	0	0	0
201	100.46	3.01	0	0	0
202	98.72	3.77	0	0	0
203	98.5	3.65	0	0	0
205	101.15	3.37	0	0	0
206	98.99	3.62	0	0	0
206A	100.72	2.09	0	0	0
208	98.39	3.27	0	0	0
209	97.65	3.65	0	0	0
211	99.5	2.97	0	0	0
CBMH04	101.45	2.05	0	0	0
CBMH05	101.09	1.46	0	0	0
CBMH08	101.16	1.54	0	0	0
STMH07	100.86	1.72	0	0	0

[OUTFALLS]

;;	Invert	Outfall	Stage/Table	Tide	
;;Name	Elev.	Type	Time Series	Gate	Route To
210_Outfall_Goward_Storm	97.36	FIXED	98.22	NO	
Outfall_Second_Line	102.76	FREE		NO	

[STORAGE]

;;	Invert	Max.	Init.	Storage	Curve
Evap.	Elev.	Depth	Depth	Curve	Params
;;Name	Infiltration parameters				
Frac.					
CB01	99.8	1.55	0	TABULAR	CB01_NO_PONDING
0					
CB02	101.09	1.46	0	TABULAR	CB02Ponding
0					
CB03	101.45	2.05	0	TABULAR	CB03Ponding
0					
CB06	101.5	1.28	0	TABULAR	CB06_PONDING
0					
CB08	101.16	1.44	0	TABULAR	CB08_PONDING
0					
CB10	101.4	1.15	0	TABULAR	CB10_PONDING
0					
CB13	99.7	1.55	0	TABULAR	CB13_PONDING
0					
MC-3500	100.76	1.88	0	TABULAR	MC-3500_56_Chamber
0					
Outfall_Hydro_Corridor	100.8	0.2	0	TABULAR	PARK-STORAGE
0					

[CONDUITS]

;;	Inlet	Outlet	Manning	Inlet
Outlet	Init.	Max.		
;;Name	Node	Node	Length	N
Offset	Flow	Flow		Offset
C01	200	201	24.1	0.013
100.52	0	0		100.96

C02		201		202	71.84	0.013	100.46
98.84	0		0				
C03		202		203	23.99	0.013	98.72
98.64	0		0				
C04		203		208	8.56	0.013	98.5
98.42	0		0				
C06		205		201	62.35	0.013	101.15
100.52	0		0				
C07		211		206	22.29	0.013	99.5
99.2	0		0				
C09		208		209	40.65	0.013	98.39
97.71	0		0				
C10		209		210_Outfall_Goward_Storm	12.284	0.013	97.65
97.6	0		0				
C13		CBMH08		STMH07	12.94	0.013	101.16
100.9	0		0				
C15		206A		206	11.1	0.013	100.72
100.55	0		0				
C16		STMH07		MC-3500	6.37	0.013	100.86
100.76	0		0				
;Swale							
C51		CB13		Outfall_Hydro_Corridor	86.248	0.013	101.1
100.8	0		0				
;Swale							
C52		CB01		Outfall_Hydro_Corridor	37.415	0.013	101.2
100.83	0		0				
C7		206		202	39.839	0.013	98.99
98.85	0		0				
C8		CBMH04		CBMH05	56.85	0.013	101.45
101.15	0		0				
C9		CBMH05		STMH07	30.68	0.013	101.09
100.9	0		0				

[ORIFICES]

;;	Inlet	Outlet	Orifice	Crest	Disch.
;;Name	Node	Node	Type	Height	Coeff.
Gate Time					
;	-----	-----	-----	-----	-----
C17	MC-3500	206A	SIDE	100.76	0.62
NO	0				

[OUTLETS]

;;	Inlet	Outlet	Outflow	Outlet
Qcoeff/	Node	Flap	Height	Type
;;Name	Node	Node		
QTable	Qexpon	Gate		
;	-----	-----	-----	-----
CB01-ICD	CB01	208	99.8	TABULAR/DEPTH
IPEX-LMF60				ICD-
		NO		
CB02-ICD	CB02	CBMH05	102.26	TABULAR/DEPTH
SignleCB_OP	400.01	NO		
CB03-ICD	CB03	CBMH04	103.36	TABULAR/DEPTH
DoubleCB_OP	400.01	NO		
CB06-ICD	CB06	MC-3500	102.64	TABULAR/DEPTH
SignleCB_OP	400.01	NO		
CB08-ICD	CB08	CBMH08	102.45	TABULAR/DEPTH
DoubleCB_OP	400.01	NO		

CB10-CID CB10 CBMH08 102.4 TABULAR/DEPTH
 SignleCB_OPSD400.01 NO

[XSECTIONS]

```
;;Link                            Shape                            Geom1                            Geom2                            Geom3                            Geom4
Barrels
;-----
```

Link	Shape	Geom1	Geom2	Geom3	Geom4	Barrels
C01	CIRCULAR	0.25	0	0	0	1
C02	CIRCULAR	0.25	0	0	0	1
C03	CIRCULAR	0.375	0	0	0	1
C04	CIRCULAR	0.45	0	0	0	1
C06	CIRCULAR	0.25	0	0	0	1
C07	CIRCULAR	0.25	0	0	0	1
C09	CIRCULAR	0.45	0	0	0	1
C10	CIRCULAR	0.45	0	0	0	1
C13	CIRCULAR	0.375	0	0	0	1
C15	CIRCULAR	0.3	0	0	0	1
C16	CIRCULAR	0.6	0	0	0	1
C51	RECT_OPEN	0.15	5	0	0	1
C52	RECT_OPEN	0.15	5	0	0	1
C7	CIRCULAR	0.375	0	0	0	1
C8	CIRCULAR	0.375	0	0	0	1
C9	CIRCULAR	0.45	0	0	0	1
C17	CIRCULAR	0.215	0	0	0	1

[TRANSECTS]

```
;;Transect Data in HEC-2 format
;
;Inlet channnels to Dry Pond
;Bottom width 3.0m
;Max 0.15m depth
;3:1 side slopes
NC 0.25    0.25    0.25
X1 Dry_pond_inlet    5            0.0    0.0    0.0    0.0    0.0    0.0
0.0
GR 0.3    -2        0           -1.5    0        0        0        1.5    0.3
2
;
;Outlet Channel from Dry Pond
;Emergency Overlflow
;Bottom width of 2.0m
;Max 0.30m depth
;3:1 side slopes
NC 0.25    0.25    0.25
X1 Dry_Pond_Outlet    5            0.0    0.0    0.0    0.0    0.0    0.0
0.0
GR 0.3    -1.9    0           -1        0        0        0        1        0.3
1.9
;
;Half street, width = 3.35m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope =
0.03m/m, bank-height = 0.20m.
NC 0.02    0.02    0.013
X1 Half_3.35m_Street_Barrier_Curb    4            0.0    3.35    0.0    0.0    0.0
0.0    0.0
GR 0.1    0        0           3.35    0.15    3.35    0.2    5.02
;
;Half Street Section
```

```

;Half Road Width = 3.2m (asphalt)
;Road width 3.35m (gutte to gutter)
;Mountable Curb, SC1.3
;Curb Height = 0.085m
;cross-slope = 0.03m/m
NC 0.020 0.020 0.013
X1 Half_3.35m_Street_Mountable 6 0.0 3.2 0.0 0.0 0.0
0.0 0.0
GR 0.102 0 0.006 3.2 0 3.35 0.036 3.368 0.088
3.55
GR 0.165 6.1
;
NC 0.01 0.01 0.01
X1 Half_3.35m_Street_Parking 3 0.0 0.0 0.0 0.0 0.0
0.0 0.0
GR 0.096 0 0 3.2 0.096 6.4
;
;Rear Yard Swales
;Max Depth = 0.3m
;Total width 2m
NC 0.01 0.01 0.01
X1 Rear_Swale 3 0.0 0.0 0.0 0.0 0.0
0.0
GR 0.3 -1 0 0 0.3 1
;
;Spill at Street 1/3 and 1/4
;3.35m at 3% and 2.94m at 3.4%
;Max depth 0.10m
NC 0.013 0.013 0.013
X1 Road-Spill-1 3 0.0 0.0 0.0 0.0 0.0
0.0
GR 0.1 0 0 3.35 0.1 6.29
;
;Spill at street intersection between
;CB3 and CB4
;3.2m at 3% and 3.12m at 3.2%
;Max depth 0.10m
NC 0.013 0.013 0.013
X1 Road-Spill-CB3-CB4 3 0 0.0 0.0 0.0 0.0
0.0
GR 0.1 0 0 3.2 0.1 6.32
;
NC 0.01 0.01 0.01
X1 Spill_CB08-CB07 3 0.0 0.0 0.0 0.0 0.0
0.0
GR 0.1 -3.4 0 0 0.1 8.77
;
;Spill over CL road and CB10 to cb11
NC 0.013 0.013 0.013
X1 Spill_CB10-CB11 4 0.0 0.0 0.0 0.0 0.0
0.0
GR 0.177 -6.299 0 0 0.027 5.4 0.177 5.425
;
NC 0.01 0.01 0.01
X1 Spill_CB6-CB7 4 0.0 0.0 0.0 0.0 0.0
0.0
GR 0.185 -10.1 0 0 0.035 3.5 0.185 3.55
;

```

```

;Major Sytem Flow in Hydro Corridor
;Assumed as sheet flow
;Max channel width 6m
;Max depth of 100mm

```

```

NC 0.01    0.01    0.01
X1 Spill_in_Hydro_Corridor 5      0.0    0.0    0.0    0.0    0.0
0.0      0.0
GR 0.15    -2.5    0      -2.5    0      0      0      2.5    0.15
2.5

```

[LOSSES]

```

;;Link          Inlet      Outlet      Average      Flap Gate      SeepageRate
;;-----
C03             0          1.3         0             NO              0
C04             0          0.4         0             NO              0
C09             0          0.4         0             NO              0
C10             0          0.8         0             NO              0

```

[INFLOWS]

```

;;
Baseline Baseline          Param  Units  Scale
;;Node          Parameter      Time Series  Type    Factor  Factor  Value
Pattern
;;-----
-----
200             FLOW          ""           FLOW    1.0    1      0.45
201             FLOW          ""           FLOW    1.0    1      0.45
202             FLOW          ""           FLOW    1.0    1      2.7
205             FLOW          ""           FLOW    1.0    1      1.8
206             FLOW          ""           FLOW    1.0    1      0.9
211             FLOW          ""           FLOW    1.0    1      0.9

```

[CURVES]

```

;;Name          Type      X-Value  Y-Value
;;-----
;Inlet Capacity for Standard CB
;3% Crossfall
;2% Gutter Grade
CB-IC          Rating    0         0
CB-IC          Rating    0.009     5
CB-IC          Rating    0.017     10
CB-IC          Rating    0.053     33
CB-IC          Rating    0.068     45
CB-IC          Rating    0.074     50
CB-IC          Rating    0.079     54
CB-IC          Rating    0.088     61
CB-IC          Rating    0.096     61

```

```

;CICB as per S22

```

```

;3% Crossfall
;2% Gutter Grade
CICB-IC        Rating    0         0
CICB-IC        Rating    0.022     3
CICB-IC        Rating    0.028     5
CICB-IC        Rating    0.052     12
CICB-IC        Rating    0.067     16
CICB-IC        Rating    0.073     18
CICB-IC        Rating    0.078     20

```

CICB-IC	0.087	23
CICB-IC	0.094	26
DoubleCB_OPSP400.01 Rating	0	0
DoubleCB_OPSP400.01	0.022	2
DoubleCB_OPSP400.01	0.032	6
DoubleCB_OPSP400.01	0.043	14
DoubleCB_OPSP400.01	0.06	32
DoubleCB_OPSP400.01	0.072	52
DoubleCB_OPSP400.01	0.081	68
DoubleCB_OPSP400.01	0.097	110
DoubleCB_OPSP400.01	0.115	154
DoubleCB_OPSP400.01	0.128	186
DoubleCB_OPSP400.01	0.151	232
DoubleCB_OPSP400.01	0.161	254
DoubleCB_OPSP400.01	0.183	286
DoubleCB_OPSP400.01	0.201	308
DoubleCB_OPSP400.01	0.22	328
DoubleCB_OPSP400.01	0.243	348
DoubleCB_OPSP400.01	0.261	366
DoubleCB_OPSP400.01	0.281	382
DoubleCB_OPSP400.01	0.3	396
ICD-IPEX-LMF100 Rating	0	0
ICD-IPEX-LMF100	0.2	3.944
ICD-IPEX-LMF100	0.4	5.578
ICD-IPEX-LMF100	0.6	6.831
ICD-IPEX-LMF100	0.8	7.888
ICD-IPEX-LMF100	1	8.819
ICD-IPEX-LMF100	1.2	9.661
ICD-IPEX-LMF100	1.4	10.435
ICD-IPEX-LMF100	1.6	11.155
ICD-IPEX-LMF100	1.8	11.832
ICD-IPEX-LMF100	2	12.472
ICD-IPEX-LMF100	2.2	13.081
ICD-IPEX-LMF100	2.4	13.662
ICD-IPEX-LMF100	2.6	14.22
ICD-IPEX-LMF100	2.8	14.757
ICD-IPEX-LMF100	3	15.275
ICD-IPEX-LMF100	3.2	15.776
ICD-IPEX-LMF100	3.4	16.261
ICD-IPEX-LMF100	3.6	16.733
ICD-IPEX-LMF100	3.8	17.191
ICD-IPEX-LMF-105 Rating	0	0
ICD-IPEX-LMF-105	0.1	3.1
ICD-IPEX-LMF-105	0.2	4.38
ICD-IPEX-LMF-105	0.3	5.36
ICD-IPEX-LMF-105	0.4	6.19
ICD-IPEX-LMF-105	0.5	6.92
ICD-IPEX-LMF-105	0.6	7.58
ICD-IPEX-LMF-105	0.7	8.19
ICD-IPEX-LMF-105	0.8	8.76
ICD-IPEX-LMF-105	0.9	9.29
ICD-IPEX-LMF-105	1	9.79
ICD-IPEX-LMF-105	1.2	10.72
ICD-IPEX-LMF-105	1.4	11.58

ICD-IPEX-LMF-105		1.6	12.38
ICD-IPEX-LMF-105		1.8	13.14
ICD-IPEX-LMF-105		2	13.85
ICD-IPEX-LMF-105		2.5	15.48
ICD-IPEX-LMF-105		3	16.96

ICD-IPEX-LMF60	Rating	0	0
ICD-IPEX-LMF60		0.1	1.02
ICD-IPEX-LMF60		0.2	1.45
ICD-IPEX-LMF60		0.3	1.77
ICD-IPEX-LMF60		0.4	2.05
ICD-IPEX-LMF60		0.5	2.29
ICD-IPEX-LMF60		0.6	2.51
ICD-IPEX-LMF60		0.7	2.71
ICD-IPEX-LMF60		0.8	2.9
ICD-IPEX-LMF60		0.9	3.07
ICD-IPEX-LMF60		1	3.24
ICD-IPEX-LMF60		1.2	3.55
ICD-IPEX-LMF60		1.4	3.83
ICD-IPEX-LMF60		1.6	4.1
ICD-IPEX-LMF60		1.8	4.35
ICD-IPEX-LMF60		2	4.58
ICD-IPEX-LMF60		2.5	5.12
ICD-IPEX-LMF60		3	5.61

ICD-IPEX-LMF70	Rating	0	0
ICD-IPEX-LMF70		0.1	1.36
ICD-IPEX-LMF70		0.2	1.92
ICD-IPEX-LMF70		0.3	2.35
ICD-IPEX-LMF70		0.4	2.72
ICD-IPEX-LMF70		0.5	3.04
ICD-IPEX-LMF70		0.6	3.33
ICD-IPEX-LMF70		0.7	3.59
ICD-IPEX-LMF70		0.8	3.84
ICD-IPEX-LMF70		0.9	4.07
ICD-IPEX-LMF70		1	4.29
ICD-IPEX-LMF70		1.2	4.7
ICD-IPEX-LMF70		1.4	5.08
ICD-IPEX-LMF70		1.6	5.43
ICD-IPEX-LMF70		1.8	5.76
ICD-IPEX-LMF70		2	6.07
ICD-IPEX-LMF70		2.5	6.79
ICD-IPEX-LMF70		3	7.44

ICD-IPEX-LMF75	Rating	0	0
ICD-IPEX-LMF75		0.1	1.58
ICD-IPEX-LMF75		0.2	2.24
ICD-IPEX-LMF75		0.3	2.74
ICD-IPEX-LMF75		0.4	3.16
ICD-IPEX-LMF75		0.5	3.54
ICD-IPEX-LMF75		0.6	3.88
ICD-IPEX-LMF75		0.7	4.19
ICD-IPEX-LMF75		0.8	4.47
ICD-IPEX-LMF75		0.9	4.75
ICD-IPEX-LMF75		1	5
ICD-IPEX-LMF75		1.2	5.48
ICD-IPEX-LMF75		1.4	5.92

ICD-IPEX-LMF75	1.6	6.33
ICD-IPEX-LMF75	1.8	6.71
ICD-IPEX-LMF75	2	7.08
ICD-IPEX-LMF75	2.5	7.91
ICD-IPEX-LMF75	3	8.67

;IPEX-LMF-80

;MAX FLOW 6 L/SEC AT 1.3m

ICD-IPEX-LMF80	Rating	0	0
ICD-IPEX-LMF80		0.2	2.544
ICD-IPEX-LMF80		0.4	3.597
ICD-IPEX-LMF80		0.6	4.406
ICD-IPEX-LMF80		0.8	5.087
ICD-IPEX-LMF80		1	5.688
ICD-IPEX-LMF80		1.2	6.23
ICD-IPEX-LMF80		1.4	6.73
ICD-IPEX-LMF80		1.6	7.194
ICD-IPEX-LMF80		1.8	7.631
ICD-IPEX-LMF80		2	8.044
ICD-IPEX-LMF80		2.2	8.436
ICD-IPEX-LMF80		2.4	8.811
ICD-IPEX-LMF80		2.6	9.171
ICD-IPEX-LMF80		2.8	9.517
ICD-IPEX-LMF80		3	9.851
ICD-IPEX-LMF80		3.2	10.174
ICD-IPEX-LMF80		3.4	10.487
ICD-IPEX-LMF80		3.6	10.792
ICD-IPEX-LMF80		3.8	11.087

ICD-IPEX-LMF85	Rating	0	0
ICD-IPEX-LMF85		0.1	2.03
ICD-IPEX-LMF85		0.2	2.87
ICD-IPEX-LMF85		0.3	3.51
ICD-IPEX-LMF85		0.4	4.05
ICD-IPEX-LMF85		0.5	4.53
ICD-IPEX-LMF85		0.6	4.96
ICD-IPEX-LMF85		0.7	5.36
ICD-IPEX-LMF85		0.8	5.73
ICD-IPEX-LMF85		0.9	6.08
ICD-IPEX-LMF85		1	6.41
ICD-IPEX-LMF85		1.2	7.02
ICD-IPEX-LMF85		1.4	7.58
ICD-IPEX-LMF85		1.6	8.11
ICD-IPEX-LMF85		1.8	8.6
ICD-IPEX-LMF85		2	9.06
ICD-IPEX-LMF85		2.5	10.13
ICD-IPEX-LMF85		3	11.1

ICD-IPEX-LMF90	Rating	0	0
ICD-IPEX-LMF90		0.1	2.27
ICD-IPEX-LMF90		0.2	3.21
ICD-IPEX-LMF90		0.3	3.93
ICD-IPEX-LMF90		0.4	4.54
ICD-IPEX-LMF90		0.5	5.07
ICD-IPEX-LMF90		0.6	5.56
ICD-IPEX-LMF90		0.7	6
ICD-IPEX-LMF90		0.8	6.41

ICD-IPEX-LMF90	0.9	6.8
ICD-IPEX-LMF90	1	7.17
ICD-IPEX-LMF90	1.2	7.86
ICD-IPEX-LMF90	1.4	8.49
ICD-IPEX-LMF90	1.6	9.07
ICD-IPEX-LMF90	1.8	9.62
ICD-IPEX-LMF90	2	10.14
ICD-IPEX-LMF90	2.5	11.34
ICD-IPEX-LMF90	3	12.42

ICD-IPEX-LMF95	Rating	0	0
ICD-IPEX-LMF95		0.1	2.52
ICD-IPEX-LMF95		0.2	3.57
ICD-IPEX-LMF95		0.3	4.37
ICD-IPEX-LMF95		0.4	5.05
ICD-IPEX-LMF95		0.5	5.64
ICD-IPEX-LMF95		0.6	6.18
ICD-IPEX-LMF95		0.7	6.68
ICD-IPEX-LMF95		0.8	7.14
ICD-IPEX-LMF95		0.9	7.57
ICD-IPEX-LMF95		1	7.98
ICD-IPEX-LMF95		1.2	8.74
ICD-IPEX-LMF95		1.4	9.44
ICD-IPEX-LMF95		1.6	10.1
ICD-IPEX-LMF95		1.8	10.71
ICD-IPEX-LMF95		2	11.29
ICD-IPEX-LMF95		2.5	12.62
ICD-IPEX-LMF95		3	13.82

;iPEX tYPE A
;20 l/SEC @ 1.4

ICD-IPEX-TYPEA	Rating	0	0
ICD-IPEX-TYPEA		0.2	8.088
ICD-IPEX-TYPEA		0.4	11.438
ICD-IPEX-TYPEA		0.6	14.008
ICD-IPEX-TYPEA		0.8	16.175
ICD-IPEX-TYPEA		1	18.085
ICD-IPEX-TYPEA		1.2	19.811
ICD-IPEX-TYPEA		1.4	21.398
ICD-IPEX-TYPEA		1.6	22.875
ICD-IPEX-TYPEA		1.8	24.263
ICD-IPEX-TYPEA		2	25.575
ICD-IPEX-TYPEA		2.2	26.824
ICD-IPEX-TYPEA		2.4	28.017
ICD-IPEX-TYPEA		2.6	29.16
ICD-IPEX-TYPEA		2.8	30.261
ICD-IPEX-TYPEA		3	31.323
ICD-IPEX-TYPEA		3.2	32.351
ICD-IPEX-TYPEA		3.4	33.346
ICD-IPEX-TYPEA		3.6	34.313
ICD-IPEX-TYPEA		3.8	35.253

;MHF Type C

ICD-IPEX-TYPE-C	Rating	0	0
ICD-IPEX-TYPE-C		0.2	14.965
ICD-IPEX-TYPE-C		0.4	21.163
ICD-IPEX-TYPE-C		0.6	25.92

ICD-IPEX-TYPE-C	0.8	29.93
ICD-IPEX-TYPE-C	1	33.462
ICD-IPEX-TYPE-C	1.2	36.656
ICD-IPEX-TYPE-C	1.4	39.593
ICD-IPEX-TYPE-C	1.6	42.327
ICD-IPEX-TYPE-C	1.8	44.895
ICD-IPEX-TYPE-C	2	47.323
ICD-IPEX-TYPE-C	2.2	49.633
ICD-IPEX-TYPE-C	2.4	51.84
ICD-IPEX-TYPE-C	2.6	53.957
ICD-IPEX-TYPE-C	2.8	55.993
ICD-IPEX-TYPE-C	3	57.959
ICD-IPEX-TYPE-C	3.2	59.859
ICD-IPEX-TYPE-C	3.4	61.702
ICD-IPEX-TYPE-C	3.6	63.49
ICD-IPEX-TYPE-C	3.8	65.23

;IPEX TYPE D

;49 L/sec AT 1m

ICD-IPEX-TYPED	Rating	0	0
ICD-IPEX-TYPED		0.2	21.883
ICD-IPEX-TYPED		0.4	30.948
ICD-IPEX-TYPED		0.6	37.903
ICD-IPEX-TYPED		0.8	43.766
ICD-IPEX-TYPED		1	48.932
ICD-IPEX-TYPED		1.2	53.603
ICD-IPEX-TYPED		1.4	57.898
ICD-IPEX-TYPED		1.6	61.895
ICD-IPEX-TYPED		1.8	65.65
ICD-IPEX-TYPED		2	69.201
ICD-IPEX-TYPED		2.2	72.578
ICD-IPEX-TYPED		2.4	75.806
ICD-IPEX-TYPED		2.6	78.901
ICD-IPEX-TYPED		2.8	81.879
ICD-IPEX-TYPED		3	84.753
ICD-IPEX-TYPED		3.2	87.533
ICD-IPEX-TYPED		3.4	90.227
ICD-IPEX-TYPED		3.6	92.843
ICD-IPEX-TYPED		3.8	95.387

;IPEX MHF Type F

ICD-IPEX-TYPE-F	Rating	0	0
ICD-IPEX-TYPE-F		0.2	28.951
ICD-IPEX-TYPE-F		0.4	40.943
ICD-IPEX-TYPE-F		0.6	50.145
ICD-IPEX-TYPE-F		0.8	57.902
ICD-IPEX-TYPE-F		1	64.737
ICD-IPEX-TYPE-F		1.2	70.916
ICD-IPEX-TYPE-F		1.4	76.598
ICD-IPEX-TYPE-F		1.6	81.886
ICD-IPEX-TYPE-F		1.8	86.854
ICD-IPEX-TYPE-F		2	91.552
ICD-IPEX-TYPE-F		2.2	96.02
ICD-IPEX-TYPE-F		2.4	100.29
ICD-IPEX-TYPE-F		2.6	104.385
ICD-IPEX-TYPE-F		2.8	108.326
ICD-IPEX-TYPE-F		3	112.128

ICD-IPEX-TYPE-F	3.2	115.805
ICD-IPEX-TYPE-F	3.4	119.369
ICD-IPEX-TYPE-F	3.6	122.83
ICD-IPEX-TYPE-F	3.8	126.195

SignleCB_OPSP400.01 Rating	0	0
SignleCB_OPSP400.01	0.022	1
SignleCB_OPSP400.01	0.032	3
SignleCB_OPSP400.01	0.043	7
SignleCB_OPSP400.01	0.06	16
SignleCB_OPSP400.01	0.072	26
SignleCB_OPSP400.01	0.081	34
SignleCB_OPSP400.01	0.097	55
SignleCB_OPSP400.01	0.115	77
SignleCB_OPSP400.01	0.128	93
SignleCB_OPSP400.01	0.151	116
SignleCB_OPSP400.01	0.161	127
SignleCB_OPSP400.01	0.183	143
SignleCB_OPSP400.01	0.201	154
SignleCB_OPSP400.01	0.22	164
SignleCB_OPSP400.01	0.243	174
SignleCB_OPSP400.01	0.261	183
SignleCB_OPSP400.01	0.281	191
SignleCB_OPSP400.01	0.3	198

;CB IN FLOW-BY CONDITION

CB01_NO_PONDING Storage	0	0.36
CB01_NO_PONDING	1.4	0.36
CB01_NO_PONDING	1.4001	0
CB01_NO_PONDING	1.55	0

;CB IN PONDING CONDITION

CB02_PONDING Storage	0	0
CB02_PONDING	1.4	0.36
CB02_PONDING	1.5	30
CB02_PONDING	1.58	55.9
CB02_PONDING	1.5801	0
CB02_PONDING	1.65	0

CB02Ponding Storage	0	1.2
CB02Ponding	1.26	1.2
CB02Ponding	1.41	149.75
CB02Ponding	1.46	406.09

;CB IN PONDING CONDITION

CB03_PONDING Storage	0	0
CB03_PONDING	1.4	0.36
CB03_PONDING	1.43	3
CB03_PONDING	1.47	16.8
CB03_PONDING	1.4701	0
CB03_PONDING	1.58	0

CB03Ponding Storage	0	1.2
CB03Ponding	1.91	1.2
CB03Ponding	1.95	9.08
CB03Ponding	2	42.17
CB03Ponding	2.05	178.18

```

;CB IN FLOW-BY CONDITION
CB04_NO_PONDING Storage 0 0
CB04_NO_PONDING 1.4 0.36
CB04_NO_PONDING 1.4001 0
CB04_NO_PONDING 1.4002 0
CB04_NO_PONDING 1.5501 0

;CB IN FLOW-BY CONDITION
CB05_NO_PONDING Storage 0 0
CB05_NO_PONDING 1.4 0.36
CB05_NO_PONDING 1.4001 0
CB05_NO_PONDING 1.4002 0
CB05_NO_PONDING 1.5501 0

;CB IN FLOW-BY CONDITION
CB06_NO_PONDING Storage 0 0
CB06_NO_PONDING 1.4 0.36
CB06_NO_PONDING 1.4001 0
CB06_NO_PONDING 1.4002 0
CB06_NO_PONDING 1.5501 0

CB06_PONDING Storage 0 1.2
CB06_PONDING 1.14 1.2
CB06_PONDING 1.2 17.16
CB06_PONDING 1.25 52.25
CB06_PONDING 1.28 97.68

;NCB IN FLOW-BY CONDITION
CB07_NO_PONDING Storage 0 0
CB07_NO_PONDING 1.4 0.36
CB07_NO_PONDING 1.4001 0
CB07_NO_PONDING 1.4002 0
CB07_NO_PONDING 1.7001 0

;CB IN FLOW-BY CONDITION
CB08_NO_PONDING Storage 0 0
CB08_NO_PONDING 1.4 0.36
CB08_NO_PONDING 1.4001 0
CB08_NO_PONDING 1.4002 0
CB08_NO_PONDING 1.5501 0

CB08_PONDING Storage 0 1.2
CB08_PONDING 1.29 1.2
CB08_PONDING 1.34 35.39
CB08_PONDING 1.39 121.96
CB08_PONDING 1.44 217.86

;CB IN FLOW-BY CONDITION
CB09_NO_PONDING Storage 0 0
CB09_NO_PONDING 1.4 0.36
CB09_NO_PONDING 1.4001 0
CB09_NO_PONDING 1.4002 0
CB09_NO_PONDING 1.5501 0

;CB IN FLOW-BY CONDITION
CB10_NO_PONDING Storage 0 0
CB10_NO_PONDING 1.8 0.36

```

CB10_NO_PONDING		1.8001	0
CB10_NO_PONDING		1.8002	0
CB10_NO_PONDING		1.9501	0
CB10_PONDING	Storage	0	1.2
CB10_PONDING		1	1.2
CB10_PONDING		1.15	63.57
;CB IN FLOW-BY CONDITION			
CB11_NO_PONDING	Storage	0	0
CB11_NO_PONDING		1.4	0.36
CB11_NO_PONDING		1.4001	0
CB11_NO_PONDING		1.4002	0
CB11_NO_PONDING		1.7001	0
CB11_NO_PONDING		2.5	0
;CB IN PONDING CONDITION			
CB13_PONDING	Storage	0	0
CB13_PONDING		1.4	0.36
CB13_PONDING		1.55	4.3
CB13_PONDING		1.59	8.2
CB13_PONDING		1.5901	0
CB13_PONDING		1.7	0
;CB IN FLOW-BY CONDITION			
CB14_NO_PONDING	Storage	0	0
CB14_NO_PONDING		1.4	0.36
CB14_NO_PONDING		1.4001	0
CB14_NO_PONDING		1.4002	0
CB14_NO_PONDING		1.5501	0
;CBMH204			
;IN FLOW-BY CONDITION			
CBMH204_NO_PONDING	Storage	0	0
CBMH204_NO_PONDING		2	1.131
CBMH204_NO_PONDING		2.0001	0
CBMH204_NO_PONDING		2.0002	0
CBMH204_NO_PONDING		2.1501	0
;DRY POND REVISED			
DRY-POND	Storage	0	0
DRY-POND		0.0001	0.36
DRY-POND		0.4	0.36
DRY-POND		0.4001	60.36
DRY-POND		1.4	60.36
DRY-POND		1.4001	0.36
DRY-POND		1.6	0.36
DRY-POND		1.6001	104.27
DRY-POND		2.2	104.27
DRY-POND		2.202	137.7
DRY-POND		2.45	185.6
DRY-POND		2.8	253.3
DRY-POND		2.85	264.2
DRY-POND		2.8501	0
;dry pond			
DRY-POND-OBSOLETE	Storage	0	0
DRY-POND-OBSOLETE		0.9	0.36

DRY-POND-OBSOLETE	1.4	45.01
DRY-POND-OBSOLETE	1.6	112.88
DRY-POND-OBSOLETE	2.2	161.13
DRY-POND-OBSOLETE	2.8	354.63
DRY-POND-OBSOLETE	2.85	4500.52
DRY-POND-OBSOLETE	2.8501	0

;ADS Underground Storage - 100 chambers

MC-3500_100_Chambers Storage	0	239.879
MC-3500_100_Chambers	0.051	239.879
MC-3500_100_Chambers	0.076	239.879
MC-3500_100_Chambers	0.102	239.879
MC-3500_100_Chambers	0.127	239.879
MC-3500_100_Chambers	0.152	239.879
MC-3500_100_Chambers	0.178	239.879
MC-3500_100_Chambers	0.203	239.879
MC-3500_100_Chambers	0.229	239.879
MC-3500_100_Chambers	0.254	478.317
MC-3500_100_Chambers	0.279	476.286
MC-3500_100_Chambers	0.305	474.843
MC-3500_100_Chambers	0.33	473.417
MC-3500_100_Chambers	0.356	471.87
MC-3500_100_Chambers	0.381	470.349
MC-3500_100_Chambers	0.406	468.702
MC-3500_100_Chambers	0.432	467.012
MC-3500_100_Chambers	0.457	465.208
MC-3500_100_Chambers	0.483	463.298
MC-3500_100_Chambers	0.508	461.31
MC-3500_100_Chambers	0.533	459.223
MC-3500_100_Chambers	0.559	457.041
MC-3500_100_Chambers	0.584	454.716
MC-3500_100_Chambers	0.61	452.3
MC-3500_100_Chambers	0.635	449.804
MC-3500_100_Chambers	0.66	446.988
MC-3500_100_Chambers	0.686	444.185
MC-3500_100_Chambers	0.711	441.264
MC-3500_100_Chambers	0.737	438.126
MC-3500_100_Chambers	0.762	434.792
MC-3500_100_Chambers	0.787	431.313
MC-3500_100_Chambers	0.813	427.661
MC-3500_100_Chambers	0.838	423.816
MC-3500_100_Chambers	0.864	419.782
MC-3500_100_Chambers	0.889	415.525
MC-3500_100_Chambers	0.914	411.05
MC-3500_100_Chambers	0.94	406.336
MC-3500_100_Chambers	0.965	401.271
MC-3500_100_Chambers	0.991	395.959
MC-3500_100_Chambers	1.016	390.335
MC-3500_100_Chambers	1.041	384.242
MC-3500_100_Chambers	1.067	377.852
MC-3500_100_Chambers	1.092	370.84
MC-3500_100_Chambers	1.118	363.404
MC-3500_100_Chambers	1.143	355.161
MC-3500_100_Chambers	1.168	346.074
MC-3500_100_Chambers	1.194	335.859
MC-3500_100_Chambers	1.219	324.178
MC-3500_100_Chambers	1.245	309.253

MC-3500_100_Chambers	1.27	286.297
MC-3500_100_Chambers	1.295	267.224
MC-3500_100_Chambers	1.321	259.794
MC-3500_100_Chambers	1.346	253.023
MC-3500_100_Chambers	1.372	243.765
MC-3500_100_Chambers	1.397	239.879
MC-3500_100_Chambers	1.422	239.879
MC-3500_100_Chambers	1.448	239.879
MC-3500_100_Chambers	1.473	239.879
MC-3500_100_Chambers	1.499	239.879
MC-3500_100_Chambers	1.524	239.879
MC-3500_100_Chambers	1.549	239.879
MC-3500_100_Chambers	1.575	239.879
MC-3500_100_Chambers	1.6	239.879
MC-3500_100_Chambers	1.626	239.879
MC-3500_100_Chambers	1.651	239.879
MC-3500_100_Chambers	1.676	239.879
MC-3500_100_Chambers	1.677	0
MC-3500_100_Chambers	1.88	0

;ADS Underground Storage - 56 chambers

MC-3500_56_Chamber Storage	0	239.879
MC-3500_56_Chamber	0.051	239.879
MC-3500_56_Chamber	0.076	239.879
MC-3500_56_Chamber	0.102	239.879
MC-3500_56_Chamber	0.127	239.879
MC-3500_56_Chamber	0.152	239.879
MC-3500_56_Chamber	0.178	239.879
MC-3500_56_Chamber	0.203	239.879
MC-3500_56_Chamber	0.229	239.879
MC-3500_56_Chamber	0.254	375.156
MC-3500_56_Chamber	0.279	373.901
MC-3500_56_Chamber	0.305	373.076
MC-3500_56_Chamber	0.33	372.259
MC-3500_56_Chamber	0.356	371.373
MC-3500_56_Chamber	0.381	370.501
MC-3500_56_Chamber	0.406	369.557
MC-3500_56_Chamber	0.432	368.588
MC-3500_56_Chamber	0.457	367.554
MC-3500_56_Chamber	0.483	366.46
MC-3500_56_Chamber	0.508	365.32
MC-3500_56_Chamber	0.533	364.125
MC-3500_56_Chamber	0.559	362.875
MC-3500_56_Chamber	0.584	361.544
MC-3500_56_Chamber	0.61	360.162
MC-3500_56_Chamber	0.635	358.733
MC-3500_56_Chamber	0.66	357.125
MC-3500_56_Chamber	0.686	355.523
MC-3500_56_Chamber	0.711	353.854
MC-3500_56_Chamber	0.737	352.063
MC-3500_56_Chamber	0.762	350.161
MC-3500_56_Chamber	0.787	348.178
MC-3500_56_Chamber	0.813	346.097
MC-3500_56_Chamber	0.838	343.905
MC-3500_56_Chamber	0.864	341.609
MC-3500_56_Chamber	0.889	339.186
MC-3500_56_Chamber	0.914	336.641

MC-3500_56_Chamber	0.94	333.96
MC-3500_56_Chamber	0.965	331.083
MC-3500_56_Chamber	0.991	328.066
MC-3500_56_Chamber	1.016	324.872
MC-3500_56_Chamber	1.041	321.414
MC-3500_56_Chamber	1.067	317.786
MC-3500_56_Chamber	1.092	313.807
MC-3500_56_Chamber	1.118	309.588
MC-3500_56_Chamber	1.143	304.917
MC-3500_56_Chamber	1.168	299.773
MC-3500_56_Chamber	1.194	294
MC-3500_56_Chamber	1.219	287.402
MC-3500_56_Chamber	1.245	278.988
MC-3500_56_Chamber	1.27	266.072
MC-3500_56_Chamber	1.295	255.344
MC-3500_56_Chamber	1.321	251.142
MC-3500_56_Chamber	1.346	247.31
MC-3500_56_Chamber	1.372	242.055
MC-3500_56_Chamber	1.397	239.879
MC-3500_56_Chamber	1.422	239.879
MC-3500_56_Chamber	1.448	239.879
MC-3500_56_Chamber	1.473	239.879
MC-3500_56_Chamber	1.499	239.879
MC-3500_56_Chamber	1.524	239.879
MC-3500_56_Chamber	1.549	239.879
MC-3500_56_Chamber	1.575	239.879
MC-3500_56_Chamber	1.6	239.879
MC-3500_56_Chamber	1.626	239.879
MC-3500_56_Chamber	1.651	239.879
MC-3500_56_Chamber	1.676	239.879
MC-3500_56_Chamber	1.677	0
MC-3500_56_Chamber	1.88	0

;SURFACE PONDING

PARK-STORAGE	Storage	0	3200
PARK-STORAGE		0.1	3790

[TIMESERIES]

;Name	Date	Time	Value
;	-----	-----	-----
;Rainfall (mm/hr)			
Chicago_3h_100yr	09/01/2018	00:00:00	5.33
Chicago_3h_100yr	09/01/2018	00:10:00	6.365
Chicago_3h_100yr	09/01/2018	00:20:00	7.963
Chicago_3h_100yr	09/01/2018	00:30:00	10.778
Chicago_3h_100yr	09/01/2018	00:40:00	17.106
Chicago_3h_100yr	09/01/2018	00:50:00	44.599
Chicago_3h_100yr	09/01/2018	01:00:00	178.25
Chicago_3h_100yr	09/01/2018	01:10:00	50.968
Chicago_3h_100yr	09/01/2018	01:20:00	26.118
Chicago_3h_100yr	09/01/2018	01:30:00	17.541
Chicago_3h_100yr	09/01/2018	01:40:00	13.254
Chicago_3h_100yr	09/01/2018	01:50:00	10.693
Chicago_3h_100yr	09/01/2018	02:00:00	8.992
Chicago_3h_100yr	09/01/2018	02:10:00	7.78
Chicago_3h_100yr	09/01/2018	02:20:00	6.871
Chicago_3h_100yr	09/01/2018	02:30:00	6.164

Chicago_3h_100yr	09/01/2018	02:40:00	5.597
Chicago_3h_100yr	09/01/2018	02:50:00	5.133
Chicago_3h_100yr	09/01/2018	03:00:00	0

;Rainfall (mm/hr)

Chicago_3h_100yr_+20%	09/01/2018	0:00:00	6.396
Chicago_3h_100yr_+20%	09/01/2018	0:20:00	9.5556
Chicago_3h_100yr_+20%	09/01/2018	0:30:00	12.9336
Chicago_3h_100yr_+20%	09/01/2018	0:40:00	20.5272
Chicago_3h_100yr_+20%	09/01/2018	0:50:00	53.5188
Chicago_3h_100yr_+20%	09/01/2018	1:00:00	213.9
Chicago_3h_100yr_+20%	09/01/2018	1:10:00	61.1616
Chicago_3h_100yr_+20%	09/01/2018	1:20:00	31.3416
Chicago_3h_100yr_+20%	09/01/2018	1:30:00	21.0492
Chicago_3h_100yr_+20%	09/01/2018	1:40:00	15.9048
Chicago_3h_100yr_+20%	09/01/2018	1:50:00	12.8316
Chicago_3h_100yr_+20%	09/01/2018	2:00:00	10.7904
Chicago_3h_100yr_+20%	09/01/2018	2:10:00	9.336
Chicago_3h_100yr_+20%	09/01/2018	2:20:00	8.2452
Chicago_3h_100yr_+20%	09/01/2018	2:30:00	7.396801
Chicago_3h_100yr_+20%	09/01/2018	2:40:00	6.716401
Chicago_3h_100yr_+20%	09/01/2018	2:50:00	6.1596
Chicago_3h_100yr_+20%	09/01/2018	3:00:00	0

;Rainfall (mm/hr)

Chicago_3h_2yr	09/01/2018	00:00:00	2.491
Chicago_3h_2yr	09/01/2018	00:10:00	2.966
Chicago_3h_2yr	09/01/2018	00:20:00	3.696
Chicago_3h_2yr	09/01/2018	00:30:00	4.976
Chicago_3h_2yr	09/01/2018	00:40:00	7.828
Chicago_3h_2yr	09/01/2018	00:50:00	19.966
Chicago_3h_2yr	09/01/2018	01:00:00	76.805
Chicago_3h_2yr	09/01/2018	01:10:00	22.777
Chicago_3h_2yr	09/01/2018	01:20:00	11.852
Chicago_3h_2yr	09/01/2018	01:30:00	8.025
Chicago_3h_2yr	09/01/2018	01:40:00	6.096
Chicago_3h_2yr	09/01/2018	01:50:00	4.938
Chicago_3h_2yr	09/01/2018	02:00:00	4.165
Chicago_3h_2yr	09/01/2018	02:10:00	3.613
Chicago_3h_2yr	09/01/2018	02:20:00	3.197
Chicago_3h_2yr	09/01/2018	02:30:00	2.873
Chicago_3h_2yr	09/01/2018	02:40:00	2.613
Chicago_3h_2yr	09/01/2018	02:50:00	2.4
Chicago_3h_2yr	09/01/2018	03:00:00	0

;Rainfall (mm/hr)

Chicago_3h_5yr	09/01/2018	00:00:00	3.256
Chicago_3h_5yr	09/01/2018	00:10:00	3.881
Chicago_3h_5yr	09/01/2018	00:20:00	4.844
Chicago_3h_5yr	09/01/2018	00:30:00	6.532
Chicago_3h_5yr	09/01/2018	00:40:00	10.308
Chicago_3h_5yr	09/01/2018	00:50:00	26.529
Chicago_3h_5yr	09/01/2018	01:00:00	104.193
Chicago_3h_5yr	09/01/2018	01:10:00	30.286
Chicago_3h_5yr	09/01/2018	01:20:00	15.655
Chicago_3h_5yr	09/01/2018	01:30:00	10.568
Chicago_3h_5yr	09/01/2018	01:40:00	8.013

Chicago_3h_5yr	09/01/2018	01:50:00	6.482
Chicago_3h_5yr	09/01/2018	02:00:00	5.462
Chicago_3h_5yr	09/01/2018	02:10:00	4.733
Chicago_3h_5yr	09/01/2018	02:20:00	4.186
Chicago_3h_5yr	09/01/2018	02:30:00	3.76
Chicago_3h_5yr	09/01/2018	02:40:00	3.418
Chicago_3h_5yr	09/01/2018	02:50:00	3.137
Chicago_3h_5yr	09/01/2018	03:00:00	0

Historic_1979_July_01		0:00:00	0
Historic_1979_July_01		0:05:00	2.3
Historic_1979_July_01		0:10:00	2.3
Historic_1979_July_01		0:15:00	8.89
Historic_1979_July_01		0:20:00	8.89
Historic_1979_July_01		0:25:00	8.89
Historic_1979_July_01		0:30:00	8.89
Historic_1979_July_01		0:35:00	38.1
Historic_1979_July_01		0:40:00	38.1
Historic_1979_July_01		0:45:00	38.1
Historic_1979_July_01		0:50:00	38.1
Historic_1979_July_01		0:55:00	38.1
Historic_1979_July_01		1:00:00	38.1
Historic_1979_July_01		1:05:00	38.1
Historic_1979_July_01		1:10:00	50.8
Historic_1979_July_01		1:15:00	50.8
Historic_1979_July_01		1:20:00	76.2
Historic_1979_July_01		1:25:00	106.7
Historic_1979_July_01		1:30:00	106.7
Historic_1979_July_01		1:35:00	71.1
Historic_1979_July_01		1:40:00	71.1
Historic_1979_July_01		1:45:00	30.5
Historic_1979_July_01		1:50:00	30.5
Historic_1979_July_01		1:55:00	30.5
Historic_1979_July_01		2:00:00	30.5
Historic_1979_July_01		2:05:00	3.8
Historic_1979_July_01		2:10:00	3.8
Historic_1979_July_01		2:15:00	3.8
Historic_1979_July_01		2:20:00	3.8
Historic_1979_July_01		2:25:00	3.8
Historic_1979_July_01		2:30:00	3.8
Historic_1979_July_01		2:35:00	3.8
Historic_1979_July_01		2:40:00	3.8
Historic_1979_July_01		2:45:00	3.8
Historic_1979_July_01		2:50:00	3.8
Historic_1979_July_01		2:55:00	3.8
Historic_1979_July_01		3:00:00	3.8

Historic_1988_August_04		0:00:00	0
Historic_1988_August_04		0:05:00	0.1
Historic_1988_August_04		0:10:00	0.1
Historic_1988_August_04		0:15:00	0
Historic_1988_August_04		0:20:00	3.7
Historic_1988_August_04		0:25:00	6.2
Historic_1988_August_04		0:30:00	101.5
Historic_1988_August_04		0:35:00	15.5
Historic_1988_August_04		0:40:00	29.3
Historic_1988_August_04		0:45:00	19.8

Historic_1988_August_04	0:50:00	1.5
Historic_1988_August_04	0:55:00	1.7
Historic_1988_August_04	1:00:00	5.4
Historic_1988_August_04	1:05:00	24.6
Historic_1988_August_04	1:10:00	26.5
Historic_1988_August_04	1:15:00	34.9
Historic_1988_August_04	1:20:00	10.2
Historic_1988_August_04	1:25:00	27.1
Historic_1988_August_04	1:30:00	104.4
Historic_1988_August_04	1:35:00	27.5
Historic_1988_August_04	1:40:00	62.5
Historic_1988_August_04	1:45:00	31.8
Historic_1988_August_04	1:50:00	79.8
Historic_1988_August_04	1:55:00	67.5
Historic_1988_August_04	2:00:00	156.2
Historic_1988_August_04	2:05:00	5.1
Historic_1988_August_04	2:10:00	0.2
Historic_1988_August_04	2:15:00	0.2
Historic_1988_August_04	2:20:00	0.2
Historic_1988_August_04	2:25:00	0.2
Historic_1988_August_04	2:30:00	0.2
Historic_1988_August_04	2:35:00	0.2
Historic_1988_August_04	2:40:00	0.2
Historic_1988_August_04	2:45:00	0.2
Historic_1988_August_04	2:50:00	0.2
Historic_1988_August_04	2:55:00	0.2
Historic_1988_August_04	3:00:00	12.8
Historic_1988_August_04	3:05:00	14
Historic_1988_August_04	3:10:00	22.2
Historic_1988_August_04	3:15:00	21.8
Historic_1988_August_04	3:20:00	1.4
Historic_1988_August_04	3:25:00	0.2
Historic_1988_August_04	3:30:00	0.2
Historic_1988_August_04	3:35:00	0.2
Historic_1988_August_04	3:40:00	0.2
Historic_1988_August_04	3:45:00	0.2
Historic_1988_August_04	3:50:00	0.2
Historic_1988_August_04	3:55:00	0.2
Historic_1988_August_04	4:00:00	0.2
Historic_1988_August_04	4:05:00	0.2
Historic_1988_August_04	4:10:00	0.2
Historic_1988_August_04	4:15:00	0.2
Historic_1988_August_04	4:20:00	0.2
Historic_1988_August_04	4:25:00	0.2
Historic_1988_August_04	4:30:00	0.2
Historic_1988_August_04	4:35:00	0.2
Historic_1988_August_04	4:40:00	0.2
Historic_1988_August_04	4:45:00	0.2
Historic_1988_August_04	4:50:00	0.2
Historic_1988_August_04	4:55:00	0.2
Historic_1988_August_04	5:00:00	2.9
Historic_1988_August_04	5:05:00	7.8
Historic_1988_August_04	5:10:00	10
Historic_1988_August_04	5:15:00	6.3
Historic_1988_August_04	5:20:00	5.1
Historic_1988_August_04	5:25:00	9.8
Historic_1988_August_04	5:30:00	2.6

Historic_1988_August_04	5:35:00	1.7
Historic_1988_August_04	5:40:00	0
Historic_1988_August_04	5:45:00	0
Historic_1996_August_08	0:00:00	4
Historic_1996_August_08	0:05:00	11.9
Historic_1996_August_08	0:10:00	26.5
Historic_1996_August_08	0:15:00	13.3
Historic_1996_August_08	0:20:00	0
Historic_1996_August_08	0:25:00	2.7
Historic_1996_August_08	0:30:00	0
Historic_1996_August_08	0:35:00	8
Historic_1996_August_08	0:40:00	18.6
Historic_1996_August_08	0:45:00	10.6
Historic_1996_August_08	0:50:00	21.2
Historic_1996_August_08	0:55:00	2.7
Historic_1996_August_08	1:00:00	2.7
Historic_1996_August_08	1:05:00	15.9
Historic_1996_August_08	1:10:00	66.3
Historic_1996_August_08	1:15:00	55.7
Historic_1996_August_08	1:20:00	122
Historic_1996_August_08	1:25:00	88.9
Historic_1996_August_08	1:30:00	9.3
Historic_1996_August_08	1:35:00	8
Historic_1996_August_08	1:40:00	4
Historic_1996_August_08	1:45:00	0
Historic_1996_August_08	1:50:00	2.7
Historic_1996_August_08	1:55:00	0
Historic_1996_August_08	2:00:00	0
Historic_1996_August_08	2:05:00	0
Historic_1996_August_08	2:10:00	5.3
Historic_1996_August_08	2:15:00	0
Historic_1996_August_08	2:20:00	0
Historic_1996_August_08	2:25:00	0
Historic_1996_August_08	2:30:00	0
Historic_1996_August_08	2:35:00	0
Historic_1996_August_08	2:40:00	0
Historic_1996_August_08	2:45:00	4
Historic_1996_August_08	2:50:00	53.1
Historic_1996_August_08	2:55:00	69
Historic_1996_August_08	3:00:00	63.7
Historic_1996_August_08	3:05:00	58.4
Historic_1996_August_08	3:10:00	47.8
Historic_1996_August_08	3:15:00	15.9
Historic_1996_August_08	3:20:00	13.3
Historic_1996_August_08	3:25:00	8
Historic_1996_August_08	3:30:00	5.3
Historic_1996_August_08	3:35:00	6.6
Historic_1996_August_08	3:40:00	2.7
Historic_1996_August_08	3:45:00	4
Historic_1996_August_08	3:50:00	2.7
Historic_1996_August_08	3:55:00	4
Historic_1996_August_08	4:00:00	2.7
Historic_1996_August_08	4:05:00	5.3
Historic_1996_August_08	4:10:00	4
Historic_1996_August_08	4:15:00	2.7
Historic_1996_August_08	4:20:00	4

Historic_1996_August_08	4:25:00	2.7
Historic_1996_August_08	4:30:00	1.3
Historic_1996_August_08	4:35:00	1.3
Historic_1996_August_08	4:40:00	0
Historic_1996_August_08	4:45:00	0
Historic_1996_August_08	4:50:00	0
Historic_1996_August_08	4:55:00	0
Historic_1996_August_08	5:00:00	2.7
Historic_1996_August_08	5:05:00	0
Historic_1996_August_08	5:10:00	0
Historic_1996_August_08	5:15:00	0
Historic_1996_August_08	5:20:00	0
Historic_1996_August_08	5:25:00	0
Historic_1996_August_08	5:30:00	0
Historic_1996_August_08	5:35:00	0
Historic_1996_August_08	5:40:00	1.3
Historic_1996_August_08	5:45:00	0

[REPORT]

```
;;Reporting Options
INPUT      YES
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

Node	200	STMH
Node	201	STMH
Node	202	STMH
Node	203	STMH
Node	205	STMH
Node	206	STMH
Node	206A	STMH
Node	208	STMH
Node	209	STMH
Node	211	STMH
Node	CBMH04	STMH
Node	CBMH05	STMH
Node	CBMH08	STMH
Node	STMH07	STMH
Node	CB01	CB
Node	CB02	CB
Node	CB03	CB
Node	CB06	CB
Node	CB08	CB
Node	CB10	CB
Node	CB13	CB
Node	Outfall_Hydro_Corridor	SURFACE
Link	C01	Minor_System
Link	C02	Minor_System
Link	C03	Minor_System
Link	C04	Minor_System
Link	C06	Minor_System
Link	C07	Minor_System
Link	C09	Minor_System
Link	C10	Minor_System

Link	C51	Major_System
Link	C52	Major_System
Link	C17	ORIFICE
Link	CB01-ICD	ICD
Link	CB02-ICD	ICD
Link	CB03-ICD	ICD
Link	CB06-ICD	ICD
Link	CB08-ICD	ICD
Link	CB10-CID	ICD

[MAP]

DIMENSIONS	347912.9211	5023308.49455	348086.0369	5023505.27245
UNITS	Meters			

[COORDINATES]

;;Node	X-Coord	Y-Coord
;;-----	-----	-----
200	347942.247	5023368.119
201	347959.628	5023383.746
202	348013.761	5023432.657
203	348021.014	5023455.522
205	348002.635	5023337.033
206	348040.329	5023402.974
206A	348032.225	5023395.805
208	348015.365	5023462.287
209	347981.529	5023484.819
211	348055.509	5023385.792
CBMH04	347970.499	5023384.798
CBMH05	348012.318	5023422.434
CBMH08	348040.958	5023395.339
STMH07	348033.942	5023399.596
210_Outfall_Goward_Storm	347977.136	5023496.328
Outfall_Second_Line	347946.349	5023351.695
CB01	348005.487	5023463.31
CB02	348007.658	5023428.22
CB03	347960.122	5023385.842
CB06	348021.516	5023396.893
CB08	348041.011	5023394.566
CB10	348053.11	5023380.972
CB13	348075.395	5023401.357
MC-3500	348030.168	5023396.172
Outfall_Hydro_Corridor	348037.798	5023471.547

[VERTICES]

;;Link	X-Coord	Y-Coord
;;-----	-----	-----
C51	348078.168	5023413.519
C51	348077.723	5023424.274
C51	348070.199	5023437.801
C51	348065.43	5023446.903
C51	348049.983	5023460.005
C51	348046.567	5023464.093
C51	348045.703	5023465.677
C51	348042.829	5023468.099
C51	348040.522	5023469.531
C52	348006.07	5023463.803
C52	348012.636	5023472.817

C52	348031.808	5023473.441
C52	348034.005	5023472.384
CB01-ICD	348007.761	5023466.423
CB01-ICD	348008.441	5023466.47
CB01-ICD	348014.279	5023462.554
CB03-ICD	347966.348	5023382.995
CB06-ICD	348025.369	5023395.092
CB08-ICD	348040.844	5023394.943
CB10-CID	348052.912	5023382.206
CB10-CID	348042.276	5023394.925

[POLYGONS]

;;Subcatchment	X-Coord	Y-Coord
;;-----	-----	-----
A01	347976.883	5023380.004
A01	347995.801	5023359.302
A01	347998.299	5023352.206
A01	347993.527	5023337.069
A01	347995.104	5023330.144
A01	347986.121	5023321.762
A01	347967.036	5023342.801
A01	347976.06	5023351.084
A01	347974.051	5023353.234
A01	347964.921	5023345.092
A01	347948.898	5023362.661
A01	347947.672	5023365.765
A01	347935.864	5023379.057
A01	347929.391	5023386.192
A01	347953.905	5023408.475
A01	347962.147	5023399.429
A01	347969.308	5023393.515
A01	347973.123	5023388.99
A01	347976.883	5023380.004
A02	347976.883	5023380.004
A02	347973.123	5023388.99
A02	347969.308	5023393.515
A02	347962.147	5023399.429
A02	347964.348	5023401.484
A02	347956.12	5023410.502
A02	347980.594	5023432.771
A02	347989.781	5023422.718
A02	347992.018	5023424.703
A02	347982.803	5023434.799
A02	348007.372	5023457.01
A02	348015.495	5023447.999
A02	348017.706	5023450.012
A02	348018.795	5023448.725
A02	348020.214	5023448.098
A02	348028.1	5023439.849
A02	348035.92	5023441.763
A02	348051.875	5023424.145
A02	348042.92	5023415.873
A02	348043.872	5023414.763
A02	348027.454	5023400.538
A02	348016.754	5023400.024
A02	348012.687	5023404.571
A02	347988.184	5023382.344

A02	347983.972	5023386.653
A02	347976.883	5023380.004
A03	348003.795	5023345.61
A03	347993.597	5023336.76
A03	347993.527	5023337.069
A03	347998.299	5023352.206
A03	347995.801	5023359.302
A03	348002.699	5023363.516
A03	348007.402	5023367.753
A03	348010.675	5023366.683
A03	348033.361	5023387.396
A03	348027.454	5023400.538
A03	348043.872	5023414.763
A03	348044.863	5023413.633
A03	348053.959	5023421.871
A03	348066.734	5023407.813
A03	348057.956	5023399.877
A03	348057.552	5023394.462
A03	348047.834	5023385.583
A03	348051.352	5023381.481
A03	348007.622	5023341.571
A03	348003.795	5023345.61
A04	347988.184	5023382.344
A04	348012.687	5023404.571
A04	348016.754	5023400.024
A04	348027.454	5023400.538
A04	348033.361	5023387.396
A04	348010.675	5023366.683
A04	348007.402	5023367.753
A04	348002.699	5023363.516
A04	347995.801	5023359.302
A04	347976.883	5023380.004
A04	347983.972	5023386.653
A04	347988.184	5023382.344
A05	348057.956	5023399.877
A05	348066.734	5023407.813
A05	348073.067	5023400.844
A05	348063.998	5023392.612
A05	348065.838	5023390.53
A05	347987.972	5023319.761
A05	347986.121	5023321.762
A05	347995.104	5023330.144
A05	347993.597	5023336.76
A05	348003.795	5023345.61
A05	348007.622	5023341.571
A05	348051.352	5023381.481
A05	348047.834	5023385.583
A05	348057.552	5023394.462
A05	348057.956	5023399.877
U01	347920.79	5023388.49
U01	347922.904	5023390.463
U01	347927.988	5023384.952
U01	347929.391	5023386.192
U01	347935.864	5023379.057
U01	347947.672	5023365.765
U01	347948.898	5023362.661
U01	347964.921	5023345.092

U01	347974.051	5023353.234
U01	347976.06	5023351.084
U01	347967.036	5023342.801
U01	347986.121	5023321.762
U01	347987.972	5023319.761
U01	347985.373	5023317.439
U01	347920.79	5023388.49
U02	348012.594	5023472.69
U02	348007.372	5023457.01
U02	347982.803	5023434.799
U02	347992.018	5023424.703
U02	347989.781	5023422.718
U02	347980.594	5023432.771
U02	347956.12	5023410.502
U02	347964.348	5023401.484
U02	347962.147	5023399.429
U02	347953.905	5023408.475
U02	347929.391	5023386.192
U02	347927.988	5023384.952
U02	347922.904	5023390.463
U02	348012.594	5023472.69
U03	348077.652	5023401.231
U03	348065.838	5023390.53
U03	348063.998	5023392.612
U03	348073.067	5023400.844
U03	348053.959	5023421.871
U03	348044.863	5023413.633
U03	348043.872	5023414.763
U03	348042.92	5023415.873
U03	348051.875	5023424.145
U03	348035.92	5023441.763
U03	348028.1	5023439.849
U03	348020.214	5023448.098
U03	348018.795	5023448.725
U03	348017.706	5023450.012
U03	348015.495	5023447.999
U03	348007.372	5023457.01
U03	348012.594	5023472.69
U03	348077.652	5023401.231

[SYMBOLS]

```
;;Gage          X-Coord      Y-Coord
;;-----
```

[PROFILES]

```
;;Name          Links
;;-----
"Node 200 to Node 210_Outfall_Goward_Storm" C01 C02 C03 C04 C09
"Node 200 to Node 210_Outfall_Goward_Storm" C10
"Node 205 to Node 210_Outfall_Goward_Storm" C06 C02 C03 C04 C09
"Node 205 to Node 210_Outfall_Goward_Storm" C10
"Node 204 to Node 210_Outfall_Goward_Storm" C05 C07 C03 C04 C09
"Node 204 to Node 210_Outfall_Goward_Storm" C10
"Node CBE02 to Node 210_Outfall_Goward_Storm" CB10-ICD CB14-ICD C08 C07 C03
"Node CBE02 to Node 210_Outfall_Goward_Storm" C04 C09 C10
"Node CB02 to Node 210_Outfall_Goward_Storm" C13 CB05-ICD C02 C03 C04
"Node CB02 to Node 210_Outfall_Goward_Storm" C09 C10
```

"Node CB03 to Node Outfall_Hydro_Corridor" C59 C58 C56 POND-IN1 C61
"Node CBMH05-MAJ to Node Outfall_Hydro_Corridor" C55 POND-IN1 C61
"Node CB12-MAJ to Node Outfall_Hydro_Corridor" C53 POND-IN1 C61
"Node CB09-MAJ to Node Outfall_Hydro_Corridor" C57 POND-IN2 C61
"Node CB04-DS to Node 203" C58 CB08-IC CB08-ICD C03
"Node CB07 to Node 210_Outfall_Goward_Storm" CB07-ICD C07 C03 C04 C09
"Node CB07 to Node 210_Outfall_Goward_Storm" C10
"Node CB01 to Node 210_Outfall_Goward_Storm" CB01-ICD C09 C10

[TITLE]
 OFFSITE CULVERT ANALYSIS (free outfall)
 By J Fitzpatrick
 Sept 17, 2020

[OPTIONS]
 ;;Options Value
 ;;-----
 FLOW_UNITS LPS
 INFILTRATION HORTON
 FLOW_ROUTING DYNWAVE
 LINK_OFFSETS ELEVATION
 MIN_SLOPE 0
 ALLOW_PONDING YES
 SKIP_STEADY_STATE NO

START_DATE 09/01/2018
 START_TIME 00:00:00
 REPORT_START_DATE 09/01/2018
 REPORT_START_TIME 00:00:00
 END_DATE 09/01/2018
 END_TIME 09:00:00
 SWEEP_START 01/01
 SWEEP_END 12/31
 DRY_DAYS 0
 REPORT_STEP 00:01:00
 WET_STEP 00:01:00
 DRY_STEP 00:01:00
 ROUTING_STEP 5
 RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
 NORMAL_FLOW_LIMITED BOTH
 FORCE_MAIN_EQUATION H-W
 VARIABLE_STEP 0.75
 LENGTHENING_STEP 1
 MIN_SURFAREA 0
 MAX_TRIALS 8
 HEAD_TOLERANCE 0.0015
 SYS_FLOW_TOL 5
 LAT_FLOW_TOL 5
 MINIMUM_STEP 1
 THREADS 4

[EVAPORATION]
 ;;Type Parameters
 ;;-----
 CONSTANT 0.0
 DRY_ONLY NO

[RAINGAGES]
 ;; Name Rain Time Snow Data
 ;; Type Intrvl Catch Source
 ;;-----
 Chicago_3h_100yr INTENSITY 0:10 1.0 TIMESERIES Chicago_3h_100yr
 Chicago_3h_2yr INTENSITY 0:10 1.0 TIMESERIES Chicago_3h_2yr
 Chicago_3h_5yr INTENSITY 0:10 1.0 TIMESERIES Chicago_3h_5yr

[SUBCATCHMENTS]
 ;; Name Raingage Outlet Total Area Pcnt. Pcnt. Curb Snow
 ;; Area Imperv Width Slope Length Pack
 ;;-----
 S01 Chicago_3h_100yr J3 0.0332 70.7 31.8 4 0
 S02 Chicago_3h_100yr 210_Outfall_Goward_Storm 1.1509 61.3 104.6 3 0
 S16 Chicago_3h_100yr J1 0.0087 45.1 24.9 4 0
 S17 Chicago_3h_100yr J5 0.0077 41 17.8 4 0
 S20 Chicago_3h_100yr IN58746 0.9373 43.7 85.2 1 0
 S21 Chicago_3h_100yr J5 0.0338 64.5 42.46 33 0
 S22 Chicago_3h_100yr J3 0.0488 68 42.46 33 0
 S23 Chicago_3h_100yr J1 0.1175 53.6 42.46 33 0

[SUBAREAS]
 ;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted
 ;;-----
 S01 0.013 0.25 1.57 4.67 25 OUTLET
 S02 0.013 0.25 1.57 4.67 25 OUTLET
 S16 0.013 0.25 1.57 4.67 0 OUTLET
 S17 0.013 0.25 1.57 4.67 0 OUTLET
 S20 0.013 0.25 1.57 4.67 0 OUTLET
 S21 0.013 0.25 1.57 4.67 0 OUTLET
 S22 0.013 0.25 1.57 4.67 0 OUTLET
 S23 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]
 ;;Subcatchment MaxRate MinRate Decay DryTime MaxInfil
 ;;-----
 S01 76.2 13.2 4.14 7 0
 S02 76.2 13.2 4.14 7 0
 S16 76.2 13.2 4.14 7 0
 S17 76.2 13.2 4.14 7 0
 S20 76.2 13.2 4.14 7 0
 S21 76.2 13.2 4.14 7 0
 S22 76.2 13.2 4.14 7 0
 S23 76.2 13.2 4.14 7 0

[JUNCTIONS]
 ;; Invert Max. Init. Surcharge Ponded
 ;; Name Elev. Depth Depth Area
 ;;-----
 J1 101.32 0.88 0 0 0
 J2 102.37 0.98 0 0 0
 J3 102.53 0.82 0 0 0
 J4 102.89 1.12 0 0 0
 J5 103.05 0.96 0 0 0
 J6 103.94 0.3 0 0 0
 J7 101.917 0.733 0 0 0

[OUTFALLS]
 ;; Invert Outfall Stage/Table Tide
 ;; Name Elev. Type Time Series Gate Route To
 ;;-----
 210_Outfall_Goward_Storm 97.5 FIXED 98.22 NO
 Outfall_Second_Line 101.1 FREE NO
 Outfall_Whernside 100.56 FREE NO

[STORAGE]
 ;; Invert Max. Init. Storage Curve Evap.
 ;; Name Elev. Depth Depth Curve Params Area Frac.
 ;;-----
 IN58746 101.38 2.92 0.6 TABULAR IN58746_PONDING 0 0

[CONDUITS]
 ;; Inlet Outlet
 ;; Name Node Node Length Manning Inlet Outlet Init.
 ;; Offset Offset Flow
 ;;-----
 C1 J5 J4 16.021 0.025 103.05 102.89 0
 C2 J3 J2 15.295 0.025 102.53 102.37 0
 C3 J1 Outfall_Second_Line 27.529 0.025 101.32 101.1 0
 D1_1 J2 J7 20.823 0.05 102.37 101.917 0
 D1_2 J7 J1 27.485 0.05 101.917 101.32 0
 D2 J4 J3 22.943 0.05 102.89 102.53 0
 D3 J6 J5 18.359 0.05 103.94 103.05 0

[ORIFICES]
 ;; Inlet Outlet Orifice Crest Disch. Flap Open/Close
 ;; Name Node Node Type Height Coeff. Gate Time
 ;;-----
 C6 IN58746 Outfall_Whernside SIDE 101.98 0.61 NO 0

[WEIRS]
 ;; Inlet Outlet Weir Crest Disch. Flap End
 ;; Name Node Node Type Height Coeff. Gate Con.
 ;;-----

```

;-----
DITCH_OVERFLOW IN58746 J6 V-NOTCH 103.94 1.35 NO 0

[XSECTIONS]
;;Link Shape Geom1 Geom2 Geom3 Geom4 Barrels
;-----
C1 CIRCULAR 0.5 0 0 0 1 6
C2 CIRCULAR 0.5 0 0 0 1 6
C3 CIRCULAR 0.4 0 0 0 1 6
D1_1 TRIANGULAR 0.8 4 0 0 1
D1_2 TRIANGULAR 0.8 4 0 0 1
D2 TRIANGULAR 0.8 4 0 0 1
D3 TRIANGULAR 0.3 3 0 0 1
C6 CIRCULAR 0.197 0 0 0 0
DITCH_OVERFLOW TRIANGULAR 0.3 3 0.2 0.2

[TRANSECTS]
;;Transect Data in HEC-2 format
;
;Inlet channels to Dry Pond
;Bottom width 2.0m
;Max 0.15m depth
;3:1 side slopes
NC 0.25 0.25 0.25
X1 Dry_pond_inlet 5 0.0 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.3 -1.6 0 -1 0 0 0 1 0.3 1.6
;
;Outlet Channel from Dry Pond
;Emergency Overflow
;Bottom width of 2.0m
;Max 0.30m depth
;3:1 side slopes
NC 0.25 0.25 0.25
X1 Dry_Pond_Outlet 5 0.0 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.3 -1.9 0 -1 0 0 0 1 0.3 1.9
;
;Half street, width = 3.35m, curb = 0.15m, cross-slope = 0.03m/m, bank-slope = 0.03m/m, bank-height =
NC 0.02 0.02 0.013
X1 Half_3.35m_Street_Barrier_Curb 4 0.0 3.35 0.0 0.0 0.0 0.0 0.0
GR 0.1 0 0 3.35 0.15 3.35 0.2 5.02
;
;Half Street Section
;Half Road Width = 3.2m (asphalt)
;Road width 3.35m (gutte to gutter)
;Mountable Curb, SCL1.3
;Curb Height = 0.085m
;cross-slope = 0.03m/m
NC 0.020 0.020 0.013
X1 Half_3.35m_Street_Mountable 6 0.0 3.2 0.0 0.0 0.0 0.0
GR 0.102 0 0.006 3.2 0 3.35 0.036 3.368 0.088 3.55
GR 0.165 6.1
;
NC 0.01 0.01 0.01
X1 Half_3.35m_Street_Parking 3 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.096 0 0 3.2 0.096 6.4
;
;Rear Yard Swales
;Max Depth = 0.3m
;Total width 2m
NC 0.01 0.01 0.01
X1 Rear_Swale 3 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.3 -1 0 0 0.3 1
;
;Spill at Street 1/3 and 1/4
;3.35m at 3% and 2.94m at 3.4%
;Max depth 0.10m
NC 0.013 0.013 0.013
X1 Road-Spill-1 3 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.1 0 0 3.35 0.1 6.29
;
;Spill at street intersection between

```

```

;CB3 and CB4
;3.2m at 3% and 3.12m at 3.2%
;Max depth 0.10m
NC 0.013 0.013 0.013
X1 Road-Spill-CB3-CB4 3 0 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.1 0 0 3.2 0.1 6.32
;
NC 0.01 0.01 0.01
X1 Spill_CB08-CB07 3 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.1 -3.4 0 0 0.1 8.77
;
;Spill over CL road and CB10 to cb11
NC 0.013 0.013 0.013
X1 Spill_CB10-CB11 4 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.177 -6.299 0 0 0.027 5.4 0.177 5.425
;
NC 0.01 0.01 0.01
X1 Spill_CB6-CB7 4 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.185 -10.1 0 0 0.035 3.5 0.185 3.55
;
;Major Sytem Flow in Hydro Corridor
;Assumed as sheet flow
;Max channel width 6m
;Max depth of 100mm
NC 0.01 0.01 0.01
X1 Spill_in_Hydro_Corridor 5 0.0 0.0 0.0 0.0 0.0 0.0
GR 0.15 -2.5 0 -2.5 0 0 0 0.15 2.5

[LOSSES]
;;Link Inlet Outlet Average Flap Gate SeepageRate
;-----
C1 0.5 0 0 NO 0
C2 0.5 0 0 NO 0
C3 0.5 0 0 NO 0

[CURVES]
;;Name Type X-Value Y-Value
;-----
;PONDING CONDITION
IN58746_PONDING Storage 0 0
IN58746_PONDING 2.37 0.36
IN58746_PONDING 2.56 280
IN58746_PONDING 2.561 0

[TIMESERIES]
;;Name Date Time Value
;-----
;Rainfall (mm/hr)
Chicago_3h_100yr 09/01/2018 00:00:00 5.33
Chicago_3h_100yr 09/01/2018 00:10:00 6.365
Chicago_3h_100yr 09/01/2018 00:20:00 7.963
Chicago_3h_100yr 09/01/2018 00:30:00 10.778
Chicago_3h_100yr 09/01/2018 00:40:00 17.106
Chicago_3h_100yr 09/01/2018 00:50:00 44.599
Chicago_3h_100yr 09/01/2018 01:00:00 178.25
Chicago_3h_100yr 09/01/2018 01:10:00 50.968
Chicago_3h_100yr 09/01/2018 01:20:00 26.118
Chicago_3h_100yr 09/01/2018 01:30:00 17.541
Chicago_3h_100yr 09/01/2018 01:40:00 13.254
Chicago_3h_100yr 09/01/2018 01:50:00 10.693
Chicago_3h_100yr 09/01/2018 02:00:00 8.992
Chicago_3h_100yr 09/01/2018 02:10:00 7.78
Chicago_3h_100yr 09/01/2018 02:20:00 6.871
Chicago_3h_100yr 09/01/2018 02:30:00 6.164
Chicago_3h_100yr 09/01/2018 02:40:00 5.597
Chicago_3h_100yr 09/01/2018 02:50:00 5.133
Chicago_3h_100yr 09/01/2018 03:00:00 0
;Rainfall (mm/yr)
Chicago_3h_2yr 09/01/2018 00:00:00 2.491
Chicago_3h_2yr 09/01/2018 00:10:00 2.966

```

```

Chicago_3h_2yr 09/01/2018 00:20:00 3.696
Chicago_3h_2yr 09/01/2018 00:30:00 4.976
Chicago_3h_2yr 09/01/2018 00:40:00 7.828
Chicago_3h_2yr 09/01/2018 00:50:00 19.966
Chicago_3h_2yr 09/01/2018 01:00:00 76.805
Chicago_3h_2yr 09/01/2018 01:10:00 22.777
Chicago_3h_2yr 09/01/2018 01:20:00 11.852
Chicago_3h_2yr 09/01/2018 01:30:00 8.025
Chicago_3h_2yr 09/01/2018 01:40:00 6.096
Chicago_3h_2yr 09/01/2018 01:50:00 4.938
Chicago_3h_2yr 09/01/2018 02:00:00 4.165
Chicago_3h_2yr 09/01/2018 02:10:00 3.613
Chicago_3h_2yr 09/01/2018 02:20:00 3.197
Chicago_3h_2yr 09/01/2018 02:30:00 2.873
Chicago_3h_2yr 09/01/2018 02:40:00 2.613
Chicago_3h_2yr 09/01/2018 02:50:00 2.4
Chicago_3h_2yr 09/01/2018 03:00:00 0

```

;Rainfall (mm/hr)

```

Chicago_3h_5yr 09/01/2018 00:00:00 3.256
Chicago_3h_5yr 09/01/2018 00:10:00 3.881
Chicago_3h_5yr 09/01/2018 00:20:00 4.844
Chicago_3h_5yr 09/01/2018 00:30:00 6.532
Chicago_3h_5yr 09/01/2018 00:40:00 10.308
Chicago_3h_5yr 09/01/2018 00:50:00 26.529
Chicago_3h_5yr 09/01/2018 01:00:00 104.193
Chicago_3h_5yr 09/01/2018 01:10:00 30.286
Chicago_3h_5yr 09/01/2018 01:20:00 15.655
Chicago_3h_5yr 09/01/2018 01:30:00 10.568
Chicago_3h_5yr 09/01/2018 01:40:00 8.013
Chicago_3h_5yr 09/01/2018 01:50:00 6.482
Chicago_3h_5yr 09/01/2018 02:00:00 5.462
Chicago_3h_5yr 09/01/2018 02:10:00 4.733
Chicago_3h_5yr 09/01/2018 02:20:00 4.186
Chicago_3h_5yr 09/01/2018 02:30:00 3.76
Chicago_3h_5yr 09/01/2018 02:40:00 3.418
Chicago_3h_5yr 09/01/2018 02:50:00 3.137
Chicago_3h_5yr 09/01/2018 03:00:00 0

```

[REPORT]

```

;;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

```

[TAGS]

```

Subcatch S01 INCL
Subcatch S02 NOT_INCL
Subcatch S16 INCL
Subcatch S17 INCL
Subcatch S20 INCL
Subcatch S21 INCL
Subcatch S22 INCL
Subcatch S23 INCL
Link C1 CULVERT
Link C2 CULVERT
Link C3 CULVERT
Link D1_1 DITCH
Link D1_2 DITCH
Link D2 DITCH
Link D3 DITCH
Link C6 CB_LEAD
Link DITCH_OVERFLOW DITCH

```

[MAP]

```

DIMENSIONS 347865.830158754 5023053.25361629 348218.600449102 5023517.42628871
UNITS Meters

```

[COORDINATES]

```

;;Node X-Coord Y-Coord
;;-----
J1 347896.589 5023403.646
J2 347930.647 5023369.382
J3 347940.885 5023358.011
J4 347955.913 5023340.668
J5 347966.526 5023328.659
J6 347979.136 5023315.374
J7 347915.966 5023384.166
210_Outfall_Goward_Storm 347977.136 5023496.328
Outfall_Second_Line 347881.865 5023421.298
Outfall_Whernside 348084.105 5023260.217
IN58746 348052.125 5023235.108

```

[VERTICES]

```

;;Link X-Coord Y-Coord
;;-----

```

[POLYGONS]

```

;;Subcatchment X-Coord Y-Coord
;;-----
S01 347955.691 5023367.554
S01 347967.672 5023354.462
S01 347965.128 5023346.077
S01 347964.647 5023344.753
S01 347965.883 5023343.248
S01 347968.506 5023341.283
S01 347965.8 5023338.788
S01 347963.426 5023341.39
S01 347942.433 5023364.56
S01 347940.082 5023367.153
S01 347943.522 5023370.277
S01 347947.034 5023366.408
S01 347955.691 5023367.554
S02 347978.269 5023330.265
S02 347973.455 5023335.568
S02 347970.63 5023339.204
S02 347968.506 5023341.283
S02 347965.883 5023343.248
S02 347964.647 5023344.753
S02 347965.128 5023346.077
S02 347967.672 5023354.462
S02 347955.691 5023367.554
S02 347947.034 5023366.408
S02 347943.522 5023370.277
S02 347937.784 5023376.6
S02 347937.535 5023376.374
S02 347935.975 5023378.092
S02 347935.366 5023377.539
S02 347932.935 5023380.193
S02 347928.485 5023385.457
S02 347925.711 5023382.939
S02 347920.747 5023388.487
S02 348012.622 5023472.739
S02 348022.687 5023461.647
S02 348067.661 5023412.03
S02 348077.551 5023401.287
S02 347985.263 5023317.376
S02 347983.366 5023319.465
S02 347980.178 5023322.95
S02 347982.773 5023325.305
S02 347978.269 5023330.265
S16 347932.935 5023380.193
S16 347935.366 5023377.539
S16 347935.975 5023378.092
S16 347937.535 5023376.374
S16 347937.784 5023376.6
S16 347943.522 5023370.277
S16 347940.082 5023367.153
S16 347937.732 5023369.747
S16 347930.373 5023377.866

```

S16	347925.711	5023382.939
S16	347928.485	5023385.457
S16	347932.935	5023380.193
S17	347978.269	5023330.265
S17	347978.269	5023330.265
S17	347982.773	5023325.305
S17	347980.178	5023322.95
S17	347968.127	5023336.211
S17	347965.8	5023338.788
S17	347968.506	5023341.283
S17	347970.63	5023339.204
S17	347973.455	5023335.568
S17	347978.269	5023330.265
S20	347974.07	5023310.322
S20	347983.366	5023319.465
S20	347985.263	5023317.386
S20	347989.539	5023321.265
S20	347994.086	5023314.503
S20	348012.211	5023308.311
S20	348019.836	5023301.75
S20	348020.668	5023292.449
S20	348023.129	5023286.805
S20	348028.728	5023282.271
S20	348031.542	5023276.979
S20	348045.162	5023262.228
S20	348060.297	5023251.023
S20	348063.899	5023240.431
S20	348071.486	5023234.534
S20	348094.512	5023228.492
S20	348101.072	5023216.814
S20	348119.485	5023208.882
S20	348123.803	5023201.041
S20	348130.641	5023197.746
S20	348135.828	5023190.774
S20	348144.064	5023176.719
S20	348148.814	5023173.363
S20	348157.027	5023163.337
S20	348161.171	5023155.322
S20	348188.517	5023146.191
S20	348185.69	5023135.754
S20	348186.714	5023134.034
S20	348191.418	5023130.854
S20	348200.193	5023129.496
S20	348202.565	5023125.793
S20	348199.3	5023109.063
S20	348189.019	5023095.291
S20	348192.11	5023088.99
S20	348193.711	5023078.857
S20	348188.216	5023074.352
S20	348004.632	5023278.44
S20	347980.206	5023303.411
S20	347974.07	5023310.322
S21	347956.192	5023330.348
S21	347965.8	5023338.788
S21	347968.127	5023336.22
S21	347980.178	5023322.959
S21	347983.366	5023319.465
S21	347974.07	5023310.322
S21	347966.143	5023319.677
S21	347957.781	5023328.59
S21	347956.192	5023330.348
S22	347930.553	5023358.311
S22	347940.107	5023367.135
S22	347942.433	5023364.57
S22	347963.426	5023341.4
S22	347965.8	5023338.788
S22	347956.192	5023330.348
S22	347950.102	5023337.084
S22	347942.778	5023345.227
S22	347930.553	5023358.311
S23	347940.107	5023367.135

S23	347930.553	5023358.311
S23	347928.17	5023360.862
S23	347914.253	5023375.912
S23	347901.106	5023389.827
S23	347891.696	5023400.635
S23	347883.773	5023409.251
S23	347887.448	5023412.409
S23	347892.178	5023415.982
S23	347914.065	5023410.787
S23	347917.36	5023408.003
S23	347923.92	5023396.325
S23	347925.757	5023393.1
S23	347920.747	5023388.496
S23	347925.711	5023382.948
S23	347930.373	5023377.875
S23	347937.732	5023369.756
S23	347940.082	5023367.163
S23	347940.107	5023367.135

[SYMBOLS]

```
;;Gage          X-Coord      Y-Coord
;;-----
```

[PROFILES]

```
;;Name          Links
;;-----
"Node IN58746 to Node Outfall_Second_Line" DITCH_OVERFLOW D3 C1 D2 C2
"Node IN58746 to Node Outfall_Second_Line" D1_1 D1_2 C3
"Node J6 to Node Outfall_Second_Line" D3 C1 D2 C2 D1_1
"Node J6 to Node Outfall_Second_Line" D1_2 C3
```

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DUAL DRAINAGE MODEL (Boundary HGL = 98.22)
 REVISED DUE TO CITY REVIEW MEMO OFF AUG 28, 2023
 By Zhidong Pan

 Element Count

Number of rain gages 7
 Number of subcatchments ... 8
 Number of nodes 25
 Number of links 23
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
Chicago_3h_100yr	Chicago_3h_100yr	INTENSITY	10 min.
Chicago_3h_100yr_+20%	Chicago_3h_100yr_+20%	INTENSITY	10 min.
Chicago_3h_2yr	Chicago_3h_2yr	INTENSITY	10 min.
Chicago_3h_5yr	Chicago_3h_5yr	INTENSITY	10 min.
Historic_1979-07-01	Historic_1979_July_01	INTENSITY	5 min.
Historic_1988-08-04	Historic_1988_August_04	INTENSITY	5 min.
Historic_1996-08-08	Historic_1996_August_08	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage
Outlet					
A01	0.26	32.20	94.30	3.2000	Chicago_3h_100yr
CB03					
A02	0.37	48.70	88.60	3.5000	Chicago_3h_100yr
CB02					
A03	0.18	16.50	100.00	1.8000	Chicago_3h_100yr
CB08					
A04	0.13	26.60	25.70	3.7000	Chicago_3h_100yr
CB06					
A05	0.06	4.00	5.70	2.6000	Chicago_3h_100yr
CB10					
U01	0.04	3.70	18.60	3.8000	Chicago_3h_100yr
Outfall_Second_Line					
U02	0.10	7.80	5.70	4.0000	Chicago_3h_100yr
CB01					
U03	0.07	7.80	5.70	5.8000	Chicago_3h_100yr
CB13					

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
200	JUNCTION	100.96	2.58	0.0	Yes
201	JUNCTION	100.46	3.01	0.0	Yes
202	JUNCTION	98.72	3.77	0.0	Yes
203	JUNCTION	98.50	3.65	0.0	
205	JUNCTION	101.15	3.37	0.0	Yes
206	JUNCTION	98.99	3.62	0.0	Yes
206A	JUNCTION	100.72	2.09	0.0	
208	JUNCTION	98.39	3.27	0.0	
209	JUNCTION	97.65	3.65	0.0	
211	JUNCTION	99.50	2.97	0.0	Yes
CBMH04	JUNCTION	101.45	2.05	0.0	
CBMH05	JUNCTION	101.09	1.46	0.0	
CBMH08	JUNCTION	101.16	1.54	0.0	
STMH07	JUNCTION	100.86	1.72	0.0	
210_Outfall_Goward_Storm	OUTFALL	97.36	0.69	0.0	
Outfall_Second_Line	OUTFALL	102.76	0.00	0.0	
CB01	STORAGE	99.80	1.55	0.0	
CB02	STORAGE	101.09	1.46	0.0	
CB03	STORAGE	101.45	2.05	0.0	
CB06	STORAGE	101.50	1.28	0.0	
CB08	STORAGE	101.16	1.44	0.0	
CB10	STORAGE	101.40	1.15	0.0	
CB13	STORAGE	99.70	1.55	0.0	
MC-3500	STORAGE	100.76	1.88	0.0	
Outfall_Hydro_Corridor	STORAGE	100.80	0.20	0.0	

Link Summary

Name	Slope	Roughness	From Node	To Node	Type	Length	%
C01	1.8260	0.0130	200	201	CONDUIT	24.1	
C02	2.2556	0.0130	201	202	CONDUIT	71.8	
C03	0.3335	0.0130	202	203	CONDUIT	24.0	
C04	0.9346	0.0130	203	208	CONDUIT	8.6	
C06	1.0105	0.0130	205	201	CONDUIT	62.4	
C07	1.3460	0.0130	211	206	CONDUIT	22.3	
C09	1.6731	0.0130	208	209	CONDUIT	40.7	
C10	0.4070	0.0130	209	210_Outfall_Goward_Storm	CONDUIT		12.3

C13		CBMH08	STMH07	CONDUIT	12.9
2.0097	0.0130				
C15		206A	206	CONDUIT	11.1
1.5317	0.0130				
C16		STMH07	MC-3500	CONDUIT	6.4
1.5701	0.0130				
C51		CB13	Outfall_Hydro_Corridor	CONDUIT	86.2
0.3478	0.0130				
C52		CB01	Outfall_Hydro_Corridor	CONDUIT	37.4
0.9890	0.0130				
C7		206	202	CONDUIT	39.8
0.3514	0.0130				
C8		CBMH04	CBMH05	CONDUIT	56.9
0.5277	0.0130				
C9		CBMH05	STMH07	CONDUIT	30.7
0.6193	0.0130				
C17		MC-3500	206A	ORIFICE	
CB01-ICD		CB01	208	OUTLET	
CB02-ICD		CB02	CBMH05	OUTLET	
CB03-ICD		CB03	CBMH04	OUTLET	
CB06-ICD		CB06	MC-3500	OUTLET	
CB08-ICD		CB08	CBMH08	OUTLET	
CB10-CID		CB10	CBMH08	OUTLET	

Cross Section Summary

Full Conduit Flow	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels

C01	CIRCULAR	0.25	0.05	0.06	0.25	1
80.36						
C02	CIRCULAR	0.25	0.05	0.06	0.25	1
89.32						
C03	CIRCULAR	0.38	0.11	0.09	0.38	1
101.25						
C04	CIRCULAR	0.45	0.16	0.11	0.45	1
275.65						
C06	CIRCULAR	0.25	0.05	0.06	0.25	1
59.78						
C07	CIRCULAR	0.25	0.05	0.06	0.25	1
69.00						
C09	CIRCULAR	0.45	0.16	0.11	0.45	1
368.80						
C10	CIRCULAR	0.45	0.16	0.11	0.45	1
181.91						
C13	CIRCULAR	0.38	0.11	0.09	0.38	1
248.57						
C15	CIRCULAR	0.30	0.07	0.07	0.30	1
119.69						
C16	CIRCULAR	0.60	0.28	0.15	0.60	1
769.41						
C51	RECT_OPEN	0.15	0.75	0.14	5.00	1
924.03						
C52	RECT_OPEN	0.15	0.75	0.14	5.00	1
1558.08						

C7	CIRCULAR	0.38	0.11	0.09	0.38	1
103.94						
C8	CIRCULAR	0.38	0.11	0.09	0.38	1
127.37						
C9	CIRCULAR	0.45	0.16	0.11	0.45	1
224.38						

Transect Summary

Transect Dry_pond_inlet

Area:

0.0172	0.0345	0.0519	0.0695	0.0871
0.1049	0.1228	0.1408	0.1589	0.1771
0.1955	0.2139	0.2325	0.2512	0.2700
0.2889	0.3079	0.3271	0.3463	0.3657
0.3852	0.4048	0.4245	0.4443	0.4643
0.4843	0.5045	0.5248	0.5452	0.5657
0.5863	0.6071	0.6279	0.6489	0.6700
0.6912	0.7125	0.7339	0.7555	0.7771
0.7989	0.8208	0.8428	0.8649	0.8871
0.9095	0.9319	0.9545	0.9772	1.0000

Hrad:

0.0237	0.0472	0.0705	0.0936	0.1165
0.1392	0.1617	0.1841	0.2063	0.2283
0.2501	0.2718	0.2933	0.3146	0.3358
0.3568	0.3777	0.3985	0.4191	0.4395
0.4599	0.4800	0.5001	0.5200	0.5398
0.5595	0.5791	0.5985	0.6178	0.6370
0.6561	0.6751	0.6940	0.7128	0.7314
0.7500	0.7684	0.7868	0.8051	0.8232
0.8413	0.8593	0.8772	0.8950	0.9127
0.9303	0.9479	0.9653	0.9827	1.0000

Width:

0.7550	0.7600	0.7650	0.7700	0.7750
0.7800	0.7850	0.7900	0.7950	0.8000
0.8050	0.8100	0.8150	0.8200	0.8250
0.8300	0.8350	0.8400	0.8450	0.8500
0.8550	0.8600	0.8650	0.8700	0.8750
0.8800	0.8850	0.8900	0.8950	0.9000
0.9050	0.9100	0.9150	0.9200	0.9250
0.9300	0.9350	0.9400	0.9450	0.9500
0.9550	0.9600	0.9650	0.9700	0.9750
0.9800	0.9850	0.9900	0.9950	1.0000

Transect Dry_Pond_Outlet

Area:

0.0139	0.0281	0.0425	0.0572	0.0721
0.0872	0.1026	0.1183	0.1342	0.1503
0.1667	0.1834	0.2003	0.2174	0.2348
0.2525	0.2704	0.2885	0.3069	0.3255
0.3444	0.3635	0.3829	0.4025	0.4224
0.4425	0.4629	0.4835	0.5044	0.5255
0.5469	0.5685	0.5904	0.6125	0.6348

	0.6574	0.6803	0.7034	0.7267	0.7503
	0.7742	0.7983	0.8226	0.8472	0.8721
	0.8972	0.9225	0.9481	0.9739	1.0000
Hrad:					
	0.0266	0.0527	0.0784	0.1035	0.1283
	0.1526	0.1766	0.2001	0.2234	0.2463
	0.2688	0.2911	0.3131	0.3348	0.3562
	0.3774	0.3984	0.4191	0.4396	0.4598
	0.4799	0.4998	0.5195	0.5390	0.5583
	0.5775	0.5965	0.6153	0.6340	0.6526
	0.6710	0.6893	0.7075	0.7255	0.7434
	0.7612	0.7789	0.7965	0.8139	0.8313
	0.8486	0.8657	0.8828	0.8998	0.9167
	0.9335	0.9503	0.9669	0.9835	1.0000
Width:					
	0.5358	0.5453	0.5547	0.5642	0.5737
	0.5832	0.5926	0.6021	0.6116	0.6211
	0.6305	0.6400	0.6495	0.6589	0.6684
	0.6779	0.6874	0.6968	0.7063	0.7158
	0.7253	0.7347	0.7442	0.7537	0.7632
	0.7726	0.7821	0.7916	0.8011	0.8105
	0.8200	0.8295	0.8389	0.8484	0.8579
	0.8674	0.8768	0.8863	0.8958	0.9053
	0.9147	0.9242	0.9337	0.9432	0.9526
	0.9621	0.9716	0.9811	0.9905	1.0000
Transect Half_3.35m_Street_Barrier_Curb					
Area:					
	0.0005	0.0020	0.0044	0.0079	0.0123
	0.0177	0.0241	0.0315	0.0399	0.0492
	0.0596	0.0709	0.0832	0.0965	0.1108
	0.1261	0.1423	0.1595	0.1778	0.1970
	0.2172	0.2383	0.2605	0.2836	0.3078
	0.3324	0.3570	0.3816	0.4062	0.4309
	0.4555	0.4801	0.5047	0.5294	0.5540
	0.5786	0.6032	0.6280	0.6536	0.6801
	0.7077	0.7363	0.7658	0.7963	0.8278
	0.8603	0.8937	0.9282	0.9636	1.0000
Hrad:					
	0.0153	0.0306	0.0459	0.0611	0.0764
	0.0917	0.1070	0.1223	0.1376	0.1529
	0.1682	0.1834	0.1987	0.2140	0.2293
	0.2446	0.2599	0.2752	0.2905	0.3057
	0.3210	0.3363	0.3516	0.3669	0.3822
	0.4118	0.4413	0.4706	0.4998	0.5289
	0.5579	0.5867	0.6154	0.6439	0.6723
	0.7006	0.7288	0.7570	0.7842	0.8098
	0.8339	0.8567	0.8781	0.8985	0.9177
	0.9359	0.9531	0.9695	0.9851	1.0000
Width:					
	0.0267	0.0534	0.0801	0.1068	0.1335
	0.1602	0.1869	0.2135	0.2402	0.2669
	0.2936	0.3203	0.3470	0.3737	0.4004
	0.4271	0.4538	0.4805	0.5072	0.5339
	0.5606	0.5873	0.6139	0.6406	0.6673
	0.6673	0.6673	0.6673	0.6673	0.6673
	0.6673	0.6673	0.6673	0.6673	0.6673

0.6673	0.6673	0.6806	0.7073	0.7339
0.7605	0.7871	0.8137	0.8403	0.8669
0.8935	0.9202	0.9468	0.9734	1.0000

Transect Half_3.35m_Street_Mountable

Area:

0.0003	0.0011	0.0026	0.0049	0.0079
0.0116	0.0161	0.0213	0.0272	0.0339
0.0413	0.0495	0.0585	0.0683	0.0789
0.0904	0.1026	0.1156	0.1294	0.1440
0.1595	0.1757	0.1928	0.2106	0.2292
0.2487	0.2690	0.2906	0.3136	0.3381
0.3641	0.3910	0.4187	0.4471	0.4763
0.5061	0.5367	0.5680	0.6000	0.6328
0.6662	0.7004	0.7354	0.7710	0.8074
0.8444	0.8822	0.9208	0.9600	1.0000

Hrad:

0.0104	0.0224	0.0387	0.0553	0.0729
0.0909	0.1094	0.1281	0.1470	0.1660
0.1849	0.2024	0.2202	0.2383	0.2566
0.2751	0.2937	0.3125	0.3313	0.3501
0.3691	0.3881	0.4071	0.4261	0.4452
0.4643	0.4793	0.4889	0.5008	0.5139
0.5289	0.5586	0.5878	0.6165	0.6446
0.6722	0.6990	0.7253	0.7510	0.7761
0.8007	0.8247	0.8482	0.8712	0.8937
0.9158	0.9375	0.9587	0.9795	1.0000

Width:

0.0138	0.0284	0.0467	0.0650	0.0833
0.1016	0.1199	0.1382	0.1565	0.1748
0.1933	0.2132	0.2331	0.2531	0.2730
0.2929	0.3128	0.3328	0.3527	0.3726
0.3925	0.4125	0.4324	0.4523	0.4723
0.4922	0.5174	0.5534	0.5893	0.6253
0.6596	0.6775	0.6954	0.7133	0.7313
0.7492	0.7671	0.7850	0.8029	0.8208
0.8388	0.8567	0.8746	0.8925	0.9104
0.9283	0.9463	0.9642	0.9821	1.0000

Transect Half_3.35m_Street_Parking

Area:

0.0004	0.0016	0.0036	0.0064	0.0100
0.0144	0.0196	0.0256	0.0324	0.0400
0.0484	0.0576	0.0676	0.0784	0.0900
0.1024	0.1156	0.1296	0.1444	0.1600
0.1764	0.1936	0.2116	0.2304	0.2500
0.2704	0.2916	0.3136	0.3364	0.3600
0.3844	0.4096	0.4356	0.4624	0.4900
0.5184	0.5476	0.5776	0.6084	0.6400
0.6724	0.7056	0.7396	0.7744	0.8100
0.8464	0.8836	0.9216	0.9604	1.0000

Hrad:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000

	0.5200	0.5400	0.5600	0.5800	0.6000
	0.6200	0.6400	0.6600	0.6800	0.7000
	0.7200	0.7400	0.7600	0.7800	0.8000
	0.8200	0.8400	0.8600	0.8800	0.9000
	0.9200	0.9400	0.9600	0.9800	1.0000
Width:					
	0.0200	0.0400	0.0600	0.0800	0.1000
	0.1200	0.1400	0.1600	0.1800	0.2000
	0.2200	0.2400	0.2600	0.2800	0.3000
	0.3200	0.3400	0.3600	0.3800	0.4000
	0.4200	0.4400	0.4600	0.4800	0.5000
	0.5200	0.5400	0.5600	0.5800	0.6000
	0.6200	0.6400	0.6600	0.6800	0.7000
	0.7200	0.7400	0.7600	0.7800	0.8000
	0.8200	0.8400	0.8600	0.8800	0.9000
	0.9200	0.9400	0.9600	0.9800	1.0000

Transect Rear_Swale
Area:

	0.0004	0.0016	0.0036	0.0064	0.0100
	0.0144	0.0196	0.0256	0.0324	0.0400
	0.0484	0.0576	0.0676	0.0784	0.0900
	0.1024	0.1156	0.1296	0.1444	0.1600
	0.1764	0.1936	0.2116	0.2304	0.2500
	0.2704	0.2916	0.3136	0.3364	0.3600
	0.3844	0.4096	0.4356	0.4624	0.4900
	0.5184	0.5476	0.5776	0.6084	0.6400
	0.6724	0.7056	0.7396	0.7744	0.8100
	0.8464	0.8836	0.9216	0.9604	1.0000
Hrad:					
	0.0200	0.0400	0.0600	0.0800	0.1000
	0.1200	0.1400	0.1600	0.1800	0.2000
	0.2200	0.2400	0.2600	0.2800	0.3000
	0.3200	0.3400	0.3600	0.3800	0.4000
	0.4200	0.4400	0.4600	0.4800	0.5000
	0.5200	0.5400	0.5600	0.5800	0.6000
	0.6200	0.6400	0.6600	0.6800	0.7000
	0.7200	0.7400	0.7600	0.7800	0.8000
	0.8200	0.8400	0.8600	0.8800	0.9000
	0.9200	0.9400	0.9600	0.9800	1.0000
Width:					
	0.0200	0.0400	0.0600	0.0800	0.1000
	0.1200	0.1400	0.1600	0.1800	0.2000
	0.2200	0.2400	0.2600	0.2800	0.3000
	0.3200	0.3400	0.3600	0.3800	0.4000
	0.4200	0.4400	0.4600	0.4800	0.5000
	0.5200	0.5400	0.5600	0.5800	0.6000
	0.6200	0.6400	0.6600	0.6800	0.7000
	0.7200	0.7400	0.7600	0.7800	0.8000
	0.8200	0.8400	0.8600	0.8800	0.9000
	0.9200	0.9400	0.9600	0.9800	1.0000

Transect Road-Spill-1
Area:

	0.0004	0.0016	0.0036	0.0064	0.0100
	0.0144	0.0196	0.0256	0.0324	0.0400
	0.0484	0.0576	0.0676	0.0784	0.0900

0.1024	0.1156	0.1296	0.1444	0.1600
0.1764	0.1936	0.2116	0.2304	0.2500
0.2704	0.2916	0.3136	0.3364	0.3600
0.3844	0.4096	0.4356	0.4624	0.4900
0.5184	0.5476	0.5776	0.6084	0.6400
0.6724	0.7056	0.7396	0.7744	0.8100
0.8464	0.8836	0.9216	0.9604	1.0000

Hrad:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Width:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Transect Road-Spill-CB3-CB4

Area:

0.0004	0.0016	0.0036	0.0064	0.0100
0.0144	0.0196	0.0256	0.0324	0.0400
0.0484	0.0576	0.0676	0.0784	0.0900
0.1024	0.1156	0.1296	0.1444	0.1600
0.1764	0.1936	0.2116	0.2304	0.2500
0.2704	0.2916	0.3136	0.3364	0.3600
0.3844	0.4096	0.4356	0.4624	0.4900
0.5184	0.5476	0.5776	0.6084	0.6400
0.6724	0.7056	0.7396	0.7744	0.8100
0.8464	0.8836	0.9216	0.9604	1.0000

Hrad:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Width:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000

0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Transect Spill_CB08-CB07

Area:

0.0004	0.0016	0.0036	0.0064	0.0100
0.0144	0.0196	0.0256	0.0324	0.0400
0.0484	0.0576	0.0676	0.0784	0.0900
0.1024	0.1156	0.1296	0.1444	0.1600
0.1764	0.1936	0.2116	0.2304	0.2500
0.2704	0.2916	0.3136	0.3364	0.3600
0.3844	0.4096	0.4356	0.4624	0.4900
0.5184	0.5476	0.5776	0.6084	0.6400
0.6724	0.7056	0.7396	0.7744	0.8100
0.8464	0.8836	0.9216	0.9604	1.0000

Hrad:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Width:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Transect Spill_CB10-CB11

Area:

0.0010	0.0041	0.0092	0.0164	0.0256
0.0368	0.0502	0.0654	0.0813	0.0975
0.1140	0.1308	0.1479	0.1653	0.1831
0.2012	0.2195	0.2382	0.2572	0.2765
0.2961	0.3160	0.3363	0.3568	0.3777
0.3988	0.4203	0.4421	0.4642	0.4866
0.5093	0.5323	0.5557	0.5793	0.6033
0.6276	0.6522	0.6770	0.7023	0.7278
0.7536	0.7797	0.8062	0.8329	0.8600
0.8874	0.9151	0.9431	0.9714	1.0000

Hrad:

0.0145	0.0291	0.0436	0.0582	0.0727
--------	--------	--------	--------	--------

0.0873	0.1018	0.1209	0.1473	0.1732
0.1987	0.2238	0.2484	0.2727	0.2966
0.3202	0.3435	0.3664	0.3891	0.4115
0.4336	0.4555	0.4771	0.4986	0.5197
0.5407	0.5615	0.5821	0.6026	0.6228
0.6429	0.6628	0.6826	0.7022	0.7217
0.7410	0.7602	0.7793	0.7983	0.8171
0.8359	0.8545	0.8730	0.8914	0.9097
0.9280	0.9461	0.9642	0.9821	1.0000

Width:

0.0711	0.1423	0.2134	0.2845	0.3557
0.4268	0.4979	0.5466	0.5574	0.5682
0.5790	0.5898	0.6006	0.6114	0.6221
0.6329	0.6437	0.6545	0.6653	0.6761
0.6869	0.6977	0.7085	0.7193	0.7301
0.7409	0.7517	0.7625	0.7733	0.7841
0.7949	0.8057	0.8165	0.8273	0.8381
0.8489	0.8597	0.8705	0.8812	0.8920
0.9028	0.9136	0.9244	0.9352	0.9460
0.9568	0.9676	0.9784	0.9892	1.0000

Transect Spill_CB6-CB7

Area:

0.0007	0.0028	0.0062	0.0111	0.0174
0.0250	0.0340	0.0444	0.0562	0.0693
0.0829	0.0971	0.1117	0.1268	0.1425
0.1586	0.1752	0.1923	0.2099	0.2280
0.2466	0.2656	0.2852	0.3053	0.3258
0.3469	0.3684	0.3904	0.4130	0.4360
0.4595	0.4835	0.5080	0.5330	0.5585
0.5845	0.6109	0.6379	0.6654	0.6933
0.7218	0.7507	0.7801	0.8101	0.8405
0.8714	0.9028	0.9347	0.9671	1.0000

Hrad:

0.0167	0.0334	0.0501	0.0668	0.0835
0.1002	0.1169	0.1336	0.1503	0.1726
0.1992	0.2251	0.2503	0.2750	0.2992
0.3229	0.3461	0.3690	0.3915	0.4137
0.4356	0.4572	0.4786	0.4997	0.5206
0.5413	0.5618	0.5821	0.6022	0.6222
0.6421	0.6618	0.6814	0.7008	0.7202
0.7394	0.7585	0.7776	0.7965	0.8153
0.8341	0.8528	0.8714	0.8900	0.9085
0.9269	0.9453	0.9636	0.9818	1.0000

Width:

0.0419	0.0838	0.1257	0.1676	0.2095
0.2514	0.2933	0.3352	0.3771	0.4044
0.4193	0.4342	0.4491	0.4640	0.4789
0.4938	0.5087	0.5236	0.5384	0.5533
0.5682	0.5831	0.5980	0.6129	0.6278
0.6427	0.6576	0.6724	0.6873	0.7022
0.7171	0.7320	0.7469	0.7618	0.7767
0.7916	0.8064	0.8213	0.8362	0.8511
0.8660	0.8809	0.8958	0.9107	0.9256
0.9404	0.9553	0.9702	0.9851	1.0000

Transect Spill_in_Hydro_Corridor

Area:

0.0200	0.0400	0.0600	0.0800	0.1000
0.1200	0.1400	0.1600	0.1800	0.2000
0.2200	0.2400	0.2600	0.2800	0.3000
0.3200	0.3400	0.3600	0.3800	0.4000
0.4200	0.4400	0.4600	0.4800	0.5000
0.5200	0.5400	0.5600	0.5800	0.6000
0.6200	0.6400	0.6600	0.6800	0.7000
0.7200	0.7400	0.7600	0.7800	0.8000
0.8200	0.8400	0.8600	0.8800	0.9000
0.9200	0.9400	0.9600	0.9800	1.0000

Hrad:

0.0212	0.0423	0.0634	0.0844	0.1054
0.1263	0.1472	0.1680	0.1888	0.2095
0.2302	0.2508	0.2714	0.2919	0.3124
0.3328	0.3532	0.3735	0.3938	0.4141
0.4343	0.4544	0.4745	0.4946	0.5146
0.5345	0.5544	0.5743	0.5941	0.6139
0.6336	0.6533	0.6730	0.6925	0.7121
0.7316	0.7511	0.7705	0.7898	0.8092
0.8284	0.8477	0.8669	0.8860	0.9051
0.9242	0.9432	0.9622	0.9811	1.0000

Width:

1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000
1.0000	1.0000	1.0000	1.0000	1.0000

NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Starting Date 09/01/2018 00:00:00
Ending Date 09/01/2018 09:00:00

Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 1.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 4
 Head Tolerance 0.001500 m

	Volume hectare-m	Depth mm
Runoff Quantity Continuity		
Total Precipitation	0.086	71.584
Evaporation Loss	0.000	0.000
Infiltration Loss	0.019	15.739
Surface Runoff	0.066	55.099
Final Storage	0.001	0.793
Continuity Error (%)	-0.066	

	Volume hectare-m	Volume 10 ⁶ ltr
Flow Routing Continuity		
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.066	0.662
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.023	0.233
External Outflow	0.085	0.854
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.001
Final Stored Volume	0.004	0.040
Continuity Error (%)	0.200	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 1.00 sec
 Average Time Step : 1.00 sec
 Maximum Time Step : 1.00 sec

Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.01

 Subcatchment Runoff Summary

Total Runoff Subcatchment 10^6 ltr	Peak Runoff Runoff LPS	Runoff Coeff	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	
A01	0.18	126.79	0.951	71.58	0.00	0.00	2.48	68.05
A02	0.24	176.49	0.917	71.58	0.00	0.00	4.99	65.61
A03	0.13	89.29	0.984	71.58	0.00	0.00	0.00	70.43
A04	0.05	37.60	0.524	71.58	0.00	0.00	33.82	37.51
A05	0.01	5.57	0.322	71.58	0.00	0.00	48.45	23.08
U01	0.01	7.62	0.449	71.58	0.00	0.00	39.28	32.12
U02	0.02	11.55	0.350	71.58	0.00	0.00	46.49	25.05
U03	0.02	11.63	0.376	71.58	0.00	0.00	44.62	26.92

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
200	JUNCTION	0.01	0.01	100.97	0 00:26	0.01
201	JUNCTION	0.03	0.03	100.49	0 00:44	0.03
202	JUNCTION	0.14	0.34	99.06	0 01:26	0.34
203	JUNCTION	0.09	0.20	98.70	0 01:26	0.20
205	JUNCTION	0.03	0.03	101.18	0 00:43	0.03
206	JUNCTION	0.09	0.26	99.25	0 01:25	0.26
206A	JUNCTION	0.06	0.19	100.91	0 01:24	0.19
208	JUNCTION	0.07	0.16	98.55	0 01:26	0.16
209	JUNCTION	0.57	2.32	99.97	0 00:00	0.60
211	JUNCTION	0.02	0.02	99.52	0 02:20	0.02
CBMH04	JUNCTION	0.03	0.54	101.99	0 01:10	0.52
CBMH05	JUNCTION	0.08	0.67	101.76	0 01:11	0.66

CBMH08	JUNCTION	0.06	0.50	101.66	0	01:24	0.50
STMH07	JUNCTION	0.13	0.80	101.66	0	01:24	0.80
210_Outfall_Goward_Storm	OUTFALL	0.86	0.86	98.22	0	00:00	0.86
Outfall_Second_Line	OUTFALL	0.00	0.00	102.76	0	00:00	0.00
CB01	STORAGE	0.16	1.40	101.20	0	01:17	1.40
CB02	STORAGE	1.16	1.40	102.49	0	01:10	1.39
CB03	STORAGE	1.85	2.01	103.46	0	01:10	2.01
CB06	STORAGE	1.08	1.22	102.72	0	01:10	1.22
CB08	STORAGE	1.24	1.38	102.54	0	01:10	1.38
CB10	STORAGE	0.90	1.04	102.44	0	01:10	1.04
CB13	STORAGE	1.25	1.41	101.11	0	01:15	1.41
MC-3500	STORAGE	0.17	0.89	101.65	0	01:24	0.89
Outfall_Hydro_Corridor	STORAGE	0.01	0.01	100.81	0	08:50	0.01

Node Inflow Summary

Total Inflow Volume		Flow Balance Error	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence	Lateral Inflow Volume 10^6 ltr
ltr	Percent			LPS	LPS	days hr:min	10^6
200	0.0146	0.262	JUNCTION	0.45	0.45	0 00:00	0.0146
201	0.0872	0.307	JUNCTION	0.45	2.70	0 00:43	0.0146
202	0.829	0.054	JUNCTION	2.70	93.34	0 01:25	0.0875
203	0.829	0.018	JUNCTION	0.00	93.32	0 01:26	0
205	0.0583	0.402	JUNCTION	1.80	1.80	0 00:00	0.0583
206	0.655	0.037	JUNCTION	0.90	87.94	0 01:24	0.0292
206A	0.597	0.000	JUNCTION	0.00	86.14	0 01:24	0
208	0.846	0.039	JUNCTION	0.00	97.16	0 01:26	0
209	0.851	0.586	JUNCTION	0.00	277.70	0 00:00	0
211	0.0292	0.214	JUNCTION	0.90	0.90	0 00:00	0.0292
CBMH04	0.174	0.421	JUNCTION	0.00	126.62	0 01:10	0
CBMH05	0.411	0.387	JUNCTION	0.00	285.60	0 01:09	0
CBMH08	0.14	0.005	JUNCTION	0.00	93.85	0 01:10	0

STMH07	JUNCTION	0.00	368.98	0	01:08	0
0.566	-0.428					
210_Outfall_Goward_Storm	OUTFALL	0.00	277.70	0	00:00	0
0.851	0.000					
Outfall_Second_Line	OUTFALL	7.62	7.62	0	01:10	0.0129
0.0129	0.000					
CB01	STORAGE	11.55	11.55	0	01:10	0.0242
0.0242	-0.587					
CB02	STORAGE	176.49	176.49	0	01:10	0.24
0.24	-0.000					
CB03	STORAGE	126.79	126.79	0	01:10	0.176
0.176	0.000					
CB06	STORAGE	37.60	37.60	0	01:10	0.0474
0.0474	-0.000					
CB08	STORAGE	89.29	89.29	0	01:10	0.129
0.129	0.000					
CB10	STORAGE	5.57	5.57	0	01:10	0.0141
0.0141	0.002					
CB13	STORAGE	11.63	11.63	0	01:10	0.0189
0.0189	7.047					
MC-3500	STORAGE	0.00	395.99	0	01:08	0
0.614	-0.013					
Outfall_Hydro_Corridor	STORAGE	0.00	14.33	0	01:16	0
0.0245	6.705					

Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
209	JUNCTION	8.99	1.807	1.333
STMH07	JUNCTION	0.81	0.195	0.925

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

of Max Occurrence	Maximum Outflow Storage Unit	Average Volume	Avg Full	Evap Loss	Exfil Loss	Maximum Volume	Max Full	Time days
hr:min	LPS	1000 m3	Full	Loss	Loss	1000 m3	Full	days

CB01		0.000	12	0	0	0.001	100	0
01:09	9.26							
CB02		0.002	6	0	0	0.011	40	0
01:10	166.25							
CB03		0.002	25	0	0	0.005	50	0
01:10	126.62							
CB06		0.001	23	0	0	0.002	42	0
01:10	36.52							
CB08		0.002	11	0	0	0.005	35	0
01:10	88.81							
CB10		0.001	18	0	0	0.002	25	0
01:10	5.16							
CB13		0.000	37	0	0	0.000	43	0
01:15	8.91							
MC-3500		0.051	10	0	0	0.293	58	0
01:24	96.96							
Outfall_Hydro_Corridor		0.019	3	0	0	0.023	3	
0 08:50	0.00							

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
210_Outfall_Goward_Storm	100.00	26.27	277.70	0.851
Outfall_Second_Line	32.16	1.24	7.62	0.013
System	66.08	27.51	277.70	0.864

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C01	CONDUIT	0.45	0 00:26	0.44	0.01	0.05
C02	CONDUIT	2.71	0 00:56	0.82	0.03	0.51
C03	CONDUIT	93.32	0 01:26	1.04	0.92	0.76
C04	CONDUIT	93.32	0 01:26	1.45	0.34	0.42
C06	CONDUIT	1.80	0 00:43	0.55	0.03	0.12
C07	CONDUIT	0.90	0 01:10	0.49	0.01	0.14
C09	CONDUIT	97.16	0 01:26	0.85	0.26	0.68
C10	CONDUIT	277.70	0 00:00	2.03	1.53	1.00
C13	CONDUIT	91.34	0 01:08	1.15	0.37	1.00
C15	CONDUIT	86.14	0 01:24	1.84	0.72	0.63
C16	CONDUIT	363.79	0 01:08	1.98	0.47	1.00
C51	CONDUIT	8.91	0 01:16	0.30	0.01	0.04

C52	CONDUIT	5.43	0	01:17	0.22	0.00	0.03
C7	CONDUIT	87.94	0	01:25	1.19	0.85	0.63
C8	CONDUIT	121.61	0	01:09	1.18	0.95	1.00
C9	CONDUIT	278.78	0	01:08	1.82	1.24	1.00
C17	ORIFICE	86.14	0	01:24			1.00
CB01-ICD	DUMMY	3.84	0	01:17			
CB02-ICD	DUMMY	166.25	0	01:10			
CB03-ICD	DUMMY	126.62	0	01:10			
CB06-ICD	DUMMY	36.52	0	01:10			
CB08-ICD	DUMMY	88.81	0	01:10			
CB10-CID	DUMMY	5.16	0	01:10			

Flow Classification Summary

Inlet Conduit Ctrl	Adjusted /Actual Length	----- Fraction of Time in Flow Class -----							
		Dry	Dry	Dry	Crit	Crit	Crit	Crit	Ltd
C01 0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00
C02 0.00	1.00	0.00	0.00	0.00	0.26	0.02	0.00	0.72	0.28
C03 0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00
C04 0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00
C06 0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00
C07 0.00	1.00	0.00	0.00	0.00	0.09	0.01	0.00	0.90	0.10
C09 0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00
C10 0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
C13 0.00	1.00	0.05	0.01	0.00	0.26	0.03	0.00	0.65	0.12
C15 0.00	1.00	0.04	0.00	0.00	0.00	0.00	0.00	0.96	0.00
C16 0.00	1.00	0.04	0.00	0.00	0.88	0.07	0.00	0.00	0.60
C51 0.00	1.00	0.12	0.28	0.00	0.57	0.03	0.00	0.00	0.81
C52 0.00	1.00	0.92	0.00	0.00	0.00	0.00	0.00	0.08	0.00
C7 0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00
C8 0.00	1.00	0.05	0.00	0.00	0.18	0.00	0.00	0.77	0.09
C9 0.00	1.00	0.04	0.00	0.00	0.25	0.00	0.00	0.71	0.07

 Conduit Surcharge Summary

Conduit	----- Both Ends	Hours Full Upstream	----- Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
C09	0.01	0.01	8.99	0.01	0.01
C10	8.99	8.99	9.00	0.01	0.01
C13	0.62	0.62	1.27	0.01	0.01
C16	0.81	0.81	1.05	0.01	0.01
C8	0.06	0.06	0.75	0.01	0.01
C9	0.66	0.70	1.08	0.14	0.05

Analysis begun on: Tue Oct 31 12:44:05 2023
 Analysis ended on: Tue Oct 31 12:44:06 2023
 Total elapsed time: 00:00:01

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.013)

OFFSITE CULVERT ANALYSIS (free outfall)
 By J Fitzpatrick
 Sept 17, 2020

WARNING 02: maximum depth increased for Node J7

 Element Count

Number of rain gages 3
 Number of subcatchments ... 8
 Number of nodes 11
 Number of links 9
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
Chicago_3h_100yr	Chicago_3h_100yr	INTENSITY	10 min.
Chicago_3h_2yr	Chicago_3h_2yr	INTENSITY	10 min.
Chicago_3h_5yr	Chicago_3h_5yr	INTENSITY	10 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
S01	0.03	31.80	70.70	4.0000	Chicago_3h_100yr	J3
S02	1.15	104.60	61.30	3.0000	Chicago_3h_100yr	210_Outfall_Goward
S16	0.01	24.90	45.10	4.0000	Chicago_3h_100yr	J1
S17	0.01	17.80	41.00	4.0000	Chicago_3h_100yr	J5
S20	0.94	85.20	43.70	1.0000	Chicago_3h_100yr	IN58746
S21	0.03	42.46	64.50	33.0000	Chicago_3h_100yr	J5
S22	0.05	42.46	68.00	33.0000	Chicago_3h_100yr	J3
S23	0.12	42.46	53.60	33.0000	Chicago_3h_100yr	J1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	101.32	0.88	0.0	
J2	JUNCTION	102.37	0.98	0.0	
J3	JUNCTION	102.53	0.82	0.0	
J4	JUNCTION	102.89	1.12	0.0	
J5	JUNCTION	103.05	0.96	0.0	
J6	JUNCTION	103.94	0.30	0.0	
J7	JUNCTION	101.92	0.80	0.0	
210_Outfall_Goward_Storm	OUTFALL	97.50	0.00	0.0	
Outfall_Second_Line	OUTFALL	101.10	0.40	0.0	
Outfall_Whernside	OUTFALL	100.56	0.00	0.0	
IN58746	STORAGE	101.38	2.92	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope Roughness
------	-----------	---------	------	--------	------------------

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.50	0.20	0.12	0.50	1	196.24
C2	CIRCULAR	0.50	0.20	0.12	0.50	1	200.84
C3	CIRCULAR	0.40	0.13	0.10	0.40	1	96.82
D1_1	TRIANGULAR	0.80	1.60	0.37	4.00	1	2439.08
D1_2	TRIANGULAR	0.80	1.60	0.37	4.00	1	2437.18
D2	TRIANGULAR	0.80	1.60	0.37	4.00	1	2071.33
D3	TRIANGULAR	0.30	0.45	0.15	3.00	1	552.52

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.50	0.20	0.12	0.50	1	196.24
C2	CIRCULAR	0.50	0.20	0.12	0.50	1	200.84
C3	CIRCULAR	0.40	0.13	0.10	0.40	1	96.82
D1_1	TRIANGULAR	0.80	1.60	0.37	4.00	1	2439.08
D1_2	TRIANGULAR	0.80	1.60	0.37	4.00	1	2437.18
D2	TRIANGULAR	0.80	1.60	0.37	4.00	1	2071.33
D3	TRIANGULAR	0.30	0.45	0.15	3.00	1	552.52

 Transect Summary

Transect Dry_pond_inlet

Area:	0.0155	0.0311	0.0470	0.0630	0.0792
	0.0956	0.1122	0.1290	0.1459	0.1631
	0.1804	0.1979	0.2156	0.2335	0.2515
	0.2698	0.2882	0.3068	0.3256	0.3446
	0.3638	0.3831	0.4027	0.4224	0.4423
	0.4624	0.4827	0.5031	0.5238	0.5446
	0.5656	0.5868	0.6082	0.6298	0.6515
	0.6735	0.6956	0.7179	0.7404	0.7631
	0.7859	0.8090	0.8322	0.8556	0.8792
	0.9030	0.9270	0.9511	0.9755	1.0000
Hrad:	0.0255	0.0507	0.0755	0.0999	0.1241
	0.1479	0.1714	0.1946	0.2176	0.2402
	0.2627	0.2848	0.3067	0.3284	0.3499
	0.3711	0.3921	0.4129	0.4336	0.4540
	0.4742	0.4943	0.5141	0.5339	0.5534
	0.5728	0.5920	0.6111	0.6300	0.6488
	0.6675	0.6860	0.7044	0.7226	0.7408
	0.7588	0.7767	0.7945	0.8121	0.8297
	0.8472	0.8645	0.8818	0.8989	0.9160
	0.9330	0.9499	0.9667	0.9834	1.0000
Width:	0.6325	0.6400	0.6475	0.6550	0.6625
	0.6700	0.6775	0.6850	0.6925	0.7000
	0.7075	0.7150	0.7225	0.7300	0.7375
	0.7450	0.7525	0.7600	0.7675	0.7750
	0.7825	0.7900	0.7975	0.8050	0.8125
	0.8200	0.8275	0.8350	0.8425	0.8500
	0.8575	0.8650	0.8725	0.8800	0.8875
	0.8950	0.9025	0.9100	0.9175	0.9250
	0.9325	0.9400	0.9475	0.9550	0.9625
	0.9700	0.9775	0.9850	0.9925	1.0000

Transect Dry_Pond_Outlet

Area:	0.0139	0.0281	0.0425	0.0572	0.0721
	0.0872	0.1026	0.1183	0.1342	0.1503
	0.1667	0.1834	0.2003	0.2174	0.2348

Hrad: 0.2525 0.2704 0.2885 0.3069 0.3255
0.3444 0.3635 0.3829 0.4025 0.4224
0.4425 0.4629 0.4835 0.5044 0.5255
0.5469 0.5685 0.5904 0.6125 0.6348
0.6574 0.6803 0.7034 0.7267 0.7503
0.7742 0.7983 0.8226 0.8472 0.8721
0.8972 0.9225 0.9481 0.9739 1.0000

Hrad: 0.0266 0.0527 0.0784 0.1035 0.1283
0.1526 0.1766 0.2001 0.2234 0.2463
0.2688 0.2911 0.3131 0.3348 0.3562
0.3774 0.3984 0.4191 0.4396 0.4598
0.4799 0.4998 0.5195 0.5390 0.5583
0.5775 0.5965 0.6153 0.6340 0.6526
0.6710 0.6893 0.7075 0.7255 0.7434
0.7612 0.7789 0.7965 0.8139 0.8313
0.8486 0.8657 0.8828 0.8998 0.9167
0.9335 0.9503 0.9669 0.9835 1.0000

Width: 0.5358 0.5453 0.5547 0.5642 0.5737
0.5832 0.5926 0.6021 0.6116 0.6211
0.6305 0.6400 0.6495 0.6589 0.6684
0.6779 0.6874 0.6968 0.7063 0.7158
0.7253 0.7347 0.7442 0.7537 0.7632
0.7726 0.7821 0.7916 0.8011 0.8105
0.8200 0.8295 0.8389 0.8484 0.8579
0.8674 0.8768 0.8863 0.8958 0.9053
0.9147 0.9242 0.9337 0.9432 0.9526
0.9621 0.9716 0.9811 0.9905 1.0000

Transect Half_3.35m_Street_Barrier_Curb
Area:

0.0005 0.0020 0.0044 0.0079 0.0123
0.0177 0.0241 0.0315 0.0399 0.0492
0.0596 0.0709 0.0832 0.0965 0.1108
0.1261 0.1423 0.1595 0.1778 0.1970
0.2172 0.2383 0.2605 0.2836 0.3078
0.3324 0.3570 0.3816 0.4062 0.4309
0.4555 0.4801 0.5047 0.5294 0.5540
0.5786 0.6032 0.6280 0.6536 0.6801
0.7077 0.7363 0.7658 0.7963 0.8278
0.8603 0.8937 0.9282 0.9636 1.0000

Hrad: 0.0153 0.0306 0.0459 0.0611 0.0764
0.0917 0.1070 0.1223 0.1376 0.1529
0.1682 0.1834 0.1987 0.2140 0.2293
0.2446 0.2599 0.2752 0.2905 0.3057
0.3210 0.3363 0.3516 0.3669 0.3822
0.4118 0.4413 0.4706 0.4998 0.5289
0.5579 0.5867 0.6154 0.6439 0.6723
0.7006 0.7288 0.7570 0.7842 0.8098
0.8339 0.8567 0.8781 0.8985 0.9177
0.9359 0.9531 0.9695 0.9851 1.0000

Width: 0.0267 0.0534 0.0801 0.1068 0.1335
0.1602 0.1869 0.2135 0.2402 0.2669
0.2936 0.3203 0.3470 0.3737 0.4004
0.4271 0.4538 0.4805 0.5072 0.5339
0.5606 0.5873 0.6139 0.6406 0.6673
0.6673 0.6673 0.6673 0.6673 0.6673
0.6673 0.6673 0.6673 0.6673 0.6673
0.6673 0.6673 0.6806 0.7073 0.7339
0.7605 0.7871 0.8137 0.8403 0.8669
0.8935 0.9202 0.9468 0.9734 1.0000

Transect Half_3.35m_Street_Mountable
Area:

0.0003 0.0011 0.0026 0.0049 0.0079
0.0116 0.0161 0.0213 0.0272 0.0339
0.0413 0.0495 0.0585 0.0683 0.0789
0.0904 0.1026 0.1156 0.1294 0.1440

Hrad: 0.1595 0.1757 0.1928 0.2106 0.2292
0.2487 0.2690 0.2906 0.3136 0.3381
0.3641 0.3910 0.4187 0.4471 0.4763
0.5061 0.5367 0.5680 0.6000 0.6328
0.6662 0.7004 0.7354 0.7710 0.8074
0.8444 0.8822 0.9208 0.9600 1.0000

Hrad: 0.0104 0.0224 0.0387 0.0553 0.0729
0.0909 0.1094 0.1281 0.1470 0.1660
0.1849 0.2024 0.2202 0.2383 0.2566
0.2751 0.2937 0.3125 0.3313 0.3501
0.3691 0.3881 0.4071 0.4261 0.4452
0.4643 0.4793 0.4889 0.5008 0.5139
0.5289 0.5586 0.5878 0.6165 0.6446
0.6722 0.6990 0.7253 0.7510 0.7761
0.8007 0.8247 0.8482 0.8712 0.8937
0.9158 0.9375 0.9587 0.9795 1.0000

Width: 0.0138 0.0284 0.0467 0.0650 0.0833
0.1016 0.1199 0.1382 0.1565 0.1748
0.1933 0.2132 0.2331 0.2531 0.2730
0.2929 0.3128 0.3328 0.3527 0.3726
0.3925 0.4125 0.4324 0.4523 0.4723
0.4922 0.5174 0.5534 0.5893 0.6253
0.6596 0.6775 0.6954 0.7133 0.7313
0.7492 0.7671 0.7850 0.8029 0.8208
0.8388 0.8567 0.8746 0.8925 0.9104
0.9283 0.9463 0.9642 0.9821 1.0000

Transect Half_3.35m_Street_Parking
Area:

0.0004 0.0016 0.0036 0.0064 0.0100
0.0144 0.0196 0.0256 0.0324 0.0400
0.0484 0.0576 0.0676 0.0784 0.0900
0.1024 0.1156 0.1296 0.1444 0.1600
0.1764 0.1936 0.2116 0.2304 0.2500
0.2704 0.2916 0.3136 0.3364 0.3600
0.3844 0.4096 0.4356 0.4624 0.4900
0.5184 0.5476 0.5776 0.6084 0.6400
0.6724 0.7056 0.7396 0.7744 0.8100
0.8464 0.8836 0.9216 0.9604 1.0000

Hrad: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Width: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Transect Rear_Swale
Area:

0.0004 0.0016 0.0036 0.0064 0.0100
0.0144 0.0196 0.0256 0.0324 0.0400
0.0484 0.0576 0.0676 0.0784 0.0900
0.1024 0.1156 0.1296 0.1444 0.1600
0.1764 0.1936 0.2116 0.2304 0.2500

Hrad: 0.2704 0.2916 0.3136 0.3364 0.3600
0.3844 0.4096 0.4356 0.4624 0.4900
0.5184 0.5476 0.5776 0.6084 0.6400
0.6724 0.7056 0.7396 0.7744 0.8100
0.8464 0.8836 0.9216 0.9604 1.0000

0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Width: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Transect Road-Spill-1

Area: 0.0004 0.0016 0.0036 0.0064 0.0100
0.0144 0.0196 0.0256 0.0324 0.0400
0.0484 0.0576 0.0676 0.0784 0.0900
0.1024 0.1156 0.1296 0.1444 0.1600
0.1764 0.1936 0.2116 0.2304 0.2500
0.2704 0.2916 0.3136 0.3364 0.3600
0.3844 0.4096 0.4356 0.4624 0.4900
0.5184 0.5476 0.5776 0.6084 0.6400
0.6724 0.7056 0.7396 0.7744 0.8100
0.8464 0.8836 0.9216 0.9604 1.0000

Hrad: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Width: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Transect Road-Spill-CB3-CB4

Area: 0.0004 0.0016 0.0036 0.0064 0.0100
0.0144 0.0196 0.0256 0.0324 0.0400
0.0484 0.0576 0.0676 0.0784 0.0900
0.1024 0.1156 0.1296 0.1444 0.1600
0.1764 0.1936 0.2116 0.2304 0.2500
0.2704 0.2916 0.3136 0.3364 0.3600

Hrad: 0.3844 0.4096 0.4356 0.4624 0.4900
0.5184 0.5476 0.5776 0.6084 0.6400
0.6724 0.7056 0.7396 0.7744 0.8100
0.8464 0.8836 0.9216 0.9604 1.0000

0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Width: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Transect Spill_CB08-CB07

Area: 0.0004 0.0016 0.0036 0.0064 0.0100
0.0144 0.0196 0.0256 0.0324 0.0400
0.0484 0.0576 0.0676 0.0784 0.0900
0.1024 0.1156 0.1296 0.1444 0.1600
0.1764 0.1936 0.2116 0.2304 0.2500
0.2704 0.2916 0.3136 0.3364 0.3600
0.3844 0.4096 0.4356 0.4624 0.4900
0.5184 0.5476 0.5776 0.6084 0.6400
0.6724 0.7056 0.7396 0.7744 0.8100
0.8464 0.8836 0.9216 0.9604 1.0000

Hrad: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Width: 0.0200 0.0400 0.0600 0.0800 0.1000
0.1200 0.1400 0.1600 0.1800 0.2000
0.2200 0.2400 0.2600 0.2800 0.3000
0.3200 0.3400 0.3600 0.3800 0.4000
0.4200 0.4400 0.4600 0.4800 0.5000
0.5200 0.5400 0.5600 0.5800 0.6000
0.6200 0.6400 0.6600 0.6800 0.7000
0.7200 0.7400 0.7600 0.7800 0.8000
0.8200 0.8400 0.8600 0.8800 0.9000
0.9200 0.9400 0.9600 0.9800 1.0000

Transect Spill_CB10-CB11

Area: 0.0010 0.0041 0.0092 0.0164 0.0256
0.0368 0.0502 0.0654 0.0813 0.0975
0.1140 0.1308 0.1479 0.1653 0.1831
0.2012 0.2195 0.2382 0.2572 0.2765
0.2961 0.3160 0.3363 0.3568 0.3777
0.3988 0.4203 0.4421 0.4642 0.4866
0.5093 0.5323 0.5557 0.5793 0.6033


```

*****
Volume      Volume
Flow Routing Continuity  hectare-m    10^6 ltr
-----
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.115      1.146
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.115      1.146
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.030

```

```

*****
Time-Step Critical Elements
*****
None

```

```

*****
Highest Flow Instability Indexes
*****
Link C2 (3)
Link C3 (2)
Link D1_1 (2)

```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      : 2.82 sec
Average Time Step      : 4.99 sec
Maximum Time Step      : 5.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 2.03
Percent Not Converging  : 0.02

```

```

*****
Subcatchment Runoff Summary
*****

```

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imperv Runoff mm	Perv Runoff mm	Total Runoff mm
S01	71.58	0.00	0.00	12.69	49.86	8.33	58.19
S02	71.58	0.00	0.00	17.89	43.19	9.82	53.01
S16	71.58	0.00	0.00	23.72	31.59	15.68	47.28
S17	71.58	0.00	0.00	25.52	28.72	16.81	45.53
S20	71.58	0.00	0.00	27.90	30.62	12.40	43.02
S21	71.58	0.00	0.00	15.31	45.18	10.20	55.38
S22	71.58	0.00	0.00	13.81	47.65	9.17	56.82
S23	71.58	0.00	0.00	20.14	37.58	13.13	50.71

```

*****
Node Depth Summary
*****

```

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.04	0.63	101.95	0 01:12	0.63

```

J2      JUNCTION      0.03    0.29   102.66    0 01:10    0.29
J3      JUNCTION      0.02    0.43   102.96    0 01:10    0.42
J4      JUNCTION      0.02    0.29   103.18    0 01:10    0.29
J5      JUNCTION      0.01    0.39   103.44    0 01:10    0.39
J6      JUNCTION      0.00    0.18   104.12    0 01:10    0.18
J7      JUNCTION      0.03    0.29   102.20    0 01:11    0.28
210_Outfall_Goward_Storm  OUTFALL      0.00    0.00   97.50    0 00:00    0.00
Outfall_Second_Line  OUTFALL      0.02    0.28   101.38    0 01:12    0.28
Outfall_Whernside    OUTFALL      0.00    0.00   100.56    0 00:00    0.00
IN58746              STORAGE      0.76    2.78   104.16    0 01:10    2.78

```

```

*****
Node Inflow Summary
*****

```

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	58.28	180.54	0 01:11	0.0637	0.166	0.191
J2	JUNCTION	0.00	158.59	0 01:10	0	0.102	-0.137
J3	JUNCTION	38.94	169.04	0 01:10	0.047	0.102	0.111
J4	JUNCTION	0.00	139.09	0 01:10	0	0.0548	-0.097
J5	JUNCTION	19.48	149.77	0 01:10	0.0222	0.0552	0.776
J6	JUNCTION	0.00	133.31	0 01:10	0	0.0331	-0.050
J7	JUNCTION	0.00	159.77	0 01:11	0	0.102	-0.135
210_Outfall_Goward_Storm	OUTFALL	433.48	433.48	0 01:10	0.61	0.61	0.000
Outfall_Second_Line	OUTFALL	0.00	147.00	0 01:12	0	0.165	0.000
Outfall_Whernside	OUTFALL	0.00	119.16	0 01:10	0	0.37	0.000
IN58746	STORAGE	250.98	250.98	0 01:10	0.403	0.403	-0.040

```

*****
Node Surge Summary
*****

```

No nodes were surcharged.

```

*****
Node Flooding Summary
*****

```

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate LPS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Depth Meters
IN58746	9.00	0.00	0 00:00	0.000	-0.135

```

*****
Storage Volume Summary
*****

```

Storage Unit	Average Volume 1000 m3	Avg Full Pcnt	Evap Loss Pcnt	Exfil Loss Pcnt	Maximum Volume 1000 m3	Max Full Pcnt	Time of Max Occurrence days hr:min	Maximum Outflow LPS
IN58746	-0.068	0	0	0	0.000	0	0 00:00	252.47

```

*****
Outfall Loading Summary
*****

```

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
210_Outfall_Goward_Storm	58.57	32.86	433.48	0.610
Outfall_Second_Line	36.16	14.59	147.00	0.165
Outfall_Whernside	52.88	21.87	119.16	0.370
System	49.20	69.33	119.16	1.146

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	139.09	0 01:10	0.97	0.71	0.68
C2	CONDUIT	158.59	0 01:10	1.05	0.79	0.72
C3	CONDUIT	147.00	0 01:12	1.29	1.52	0.85
D1_1	CONDUIT	159.77	0 01:11	0.78	0.07	0.36
D1_2	CONDUIT	155.68	0 01:11	0.35	0.06	0.56
D2	CONDUIT	137.46	0 01:10	0.47	0.07	0.45
D3	CONDUIT	130.77	0 01:10	0.74	0.24	0.79
C6	ORIFICE	119.16	0 01:10			1.00
DITCH_OVERFLOW	WEIR	133.31	0 01:10			0.75

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class									
		Up Dry	Down Dry	Sub Dry	Crit Dry	Up Sup	Down Sup	Norm Crit	Inlet Ltd	Ctrl	
C1	1.00	0.03	0.57	0.00	0.40	0.00	0.00	0.00	0.00	0.96	
C2	1.00	0.00	0.26	0.00	0.74	0.00	0.00	0.00	0.00	0.98	
C3	1.00	0.02	0.00	0.00	0.98	0.00	0.00	0.00	0.00	0.23	
D1_1	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.87	
D1_2	1.00	0.01	0.00	0.00	0.99	0.00	0.00	0.00	0.35	0.00	
D2	1.00	0.00	0.03	0.00	0.97	0.00	0.00	0.00	0.30	0.00	
D3	1.00	0.03	0.09	0.00	0.88	0.00	0.00	0.00	0.88	0.00	

Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full Normal Flow	Hours Capacity Limited
	Both Ends	Upstream	Dnstream		
C3	0.01	0.13	0.01	0.18	0.01
D3	0.01	0.01	0.06	0.01	0.01

Analysis begun on: Fri Sep 25 13:34:43 2020
Analysis ended on: Fri Sep 25 13:34:43 2020
Total elapsed time: < 1 sec

Appendix G – Correspondence

Correspondence from City of Ottawa – Hydraulic Boundary Conditions

Boundary Conditions 1158 Old Second Line

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	51	0.85
Maximum Daily Demand	127	2.11
Peak Hour	279	4.65
Fire Flow Demand #1	7,000	116.67

Location



Results

Connection 1 – Goward Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	150.2	69.3
Peak Hour	140.9	56.1
Max Day plus Fire Flow #1	121.2	28.2

¹ Ground Elevation = 101.4 m

Connection 2 – Whernside Terr.

Demand Scenario	Head (m)	Pressure¹ (psi)
Maximum HGL	150.2	68.2
Peak Hour	140.9	55.1
Max Day plus Fire Flow #1	128.9	38.0

¹ Ground Elevation = 102.2 m

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.



COMMUNICATION

To: Samantha Gatchene | City Development Department
City of Ottawa
Date: November 06th, 2023
By: Email **From:** Brandon Lawrence

Project: Old Second Line Development
1158 Old Second Line Rd, Kanata, Ottawa, ON
Project No. SL-1086-22
Subject: Response to SPC Comments No.1

Dear Samantha Gatchene,
In response to your comments dated August 28th, 2023, please find below the additional information for the comments you requested.

SPC Comments: Site Servicing and Stormwater Management Report 158 Old Second Line Road,

87. Include a section in the report discussing the proposed firewalls for the buildings. Provide in the report explanation as to why they're required and confirm that the firewalls will be properly constructed (fire resistance rating not less than 2 hours) and properly protected in accordance with Building Code. Include letter from the architect confirming this in the appendices if necessary. Note that the City will be taking securities for the proposed firewalls at the time of approval. [SJL] The firewalls required by the fire flow calculations provided by EXP will be designed and constructed in accordance with OBC. To achieve the 2-hour required FRR, the firewalls will be constructed per assembly UL U342, or an approved equivalent.

We trust this is satisfactory,
S. J. LAWRENCE ARCHITECT INCORPORATED

Shawn J. Lawrence, OAA, NSAA, MRAIC

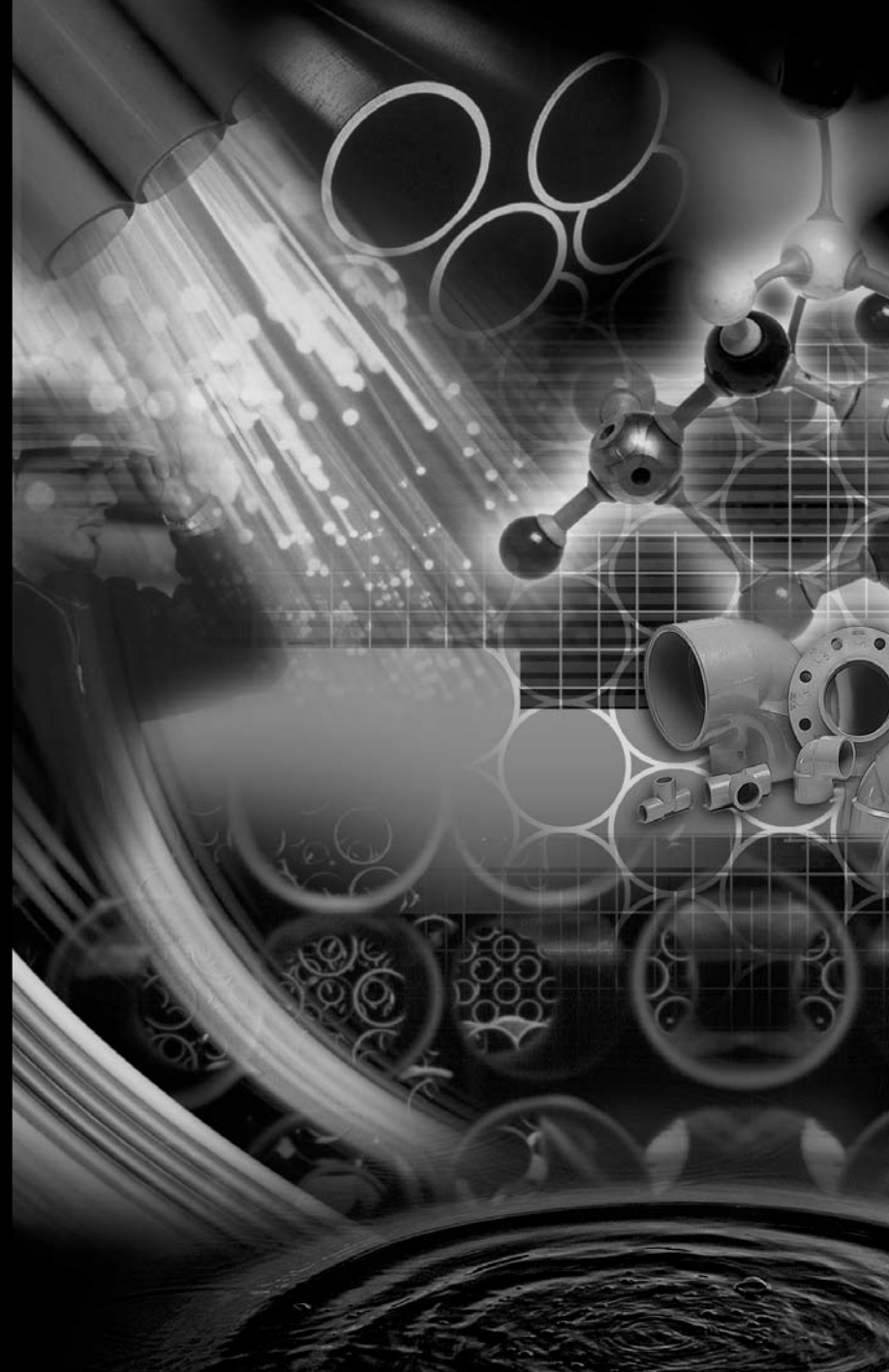


Appendix H – Manufacturer Information

Tempest Inlet Control Device

Volume III: TEMPEST™ INLET CONTROL DEVICES

Municipal Technical
Manual Series



SECOND EDITION

LMF (Low to Medium Flow) ICD

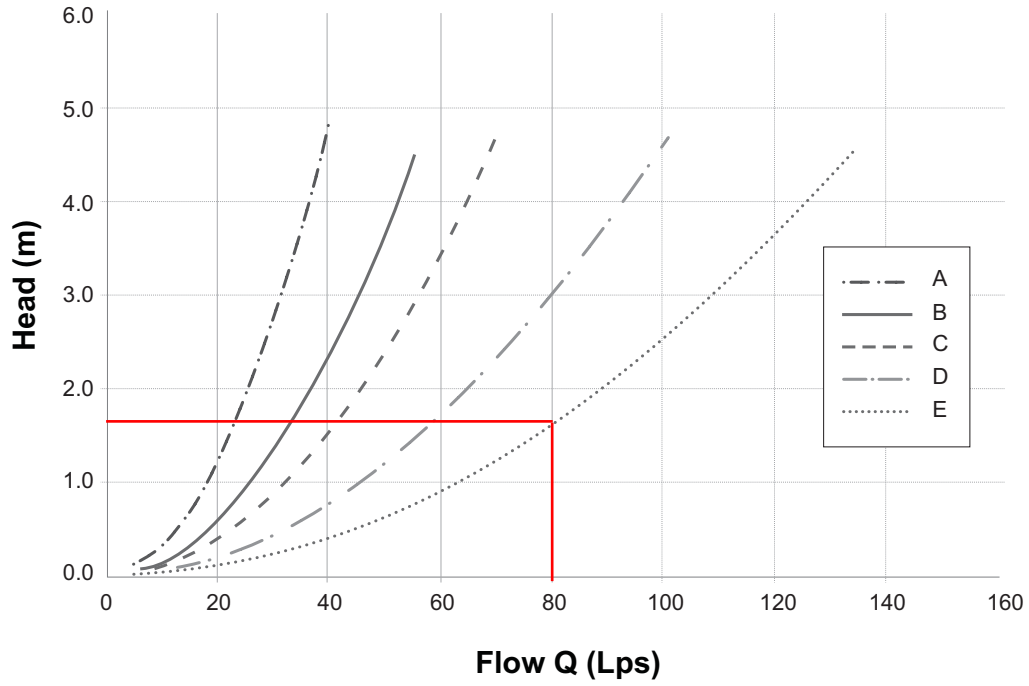
HF (High Flow) ICD

MHF (Medium to High Flow) ICD



IPEX

Chart 3: HF & MHF Preset Flow Curves



TEMPEST
 HF & MHF ICD

Appendix I – Background Information

- **Master Design Sheet (Hydraulic Grade Line Analysis).** From Stormwater Management Report, Morgan Grant, Phase 12D (Report R-1591.A). 1 page.
- **5-year Storm Design Sheet.** From Stormwater Management Report, Morgan Grant, Phase 12D (Report R-1591.A). 2 pages.
- **Stormwater Storage / Overland Balance Table.** From Stormwater Management Report, Morgan Grant, Phase 12D (Report R-1591.B). 1 page.
- **Storm Drainage Plan, Morgan's Grant Phase 12D.** From Stormwater Management Report, Morgan Grant, Phase 12D (Report R-1591.B). 1 page.
- **Storm Drainage Plan, Morgan's Grant Phase 12D.** 1 page.
- **Sanitary Drainage Plan, Morgan's Grant Phase 12D.** 1 page.
- **Morgan's Grant Master Sanitary Flows.** From Master Servicing Study for the Morgan's Grant Subdivision (Report R-2287). 9 pages.
- **Master Drainage Plan (Sanitary).** From Master Servicing Study for the Morgan's Grant Subdivision (Report R-2287). 1 page.
- **Pages 3, 4, 5 from Master Servicing Study for the Morgan's Grant Subdivision (Report R-2287).** 3 pages.
- **ECA for Storm and Sanitary Sewers No: 1005-6J6K7W-14**
- **ECA for SWM Facility. No: 9327-54JRK4-14**
- **Morgan's Grant Phase 12D, Goward Drive – Plan & Profile, Drawing # 17732-15.** 1 page.

100 YEAR IDF CURVE

STREET	MANHOLE NUMBER	AREAS WITH UNRESTRICTED FLOWS (ha)	1:100 YR PEAK FLOW GENERATION												SEWER DATA								UPSTREAM								HEAD LOSS CALCULATION										COVER																				
			0.2	0.38	0.4	0.45	0.5	0.55	0.6	0.7	0.95	2.78AR	Time min	Intensity mm/hr	Peak Flow L/s	ICD Type	CUMM @ 20.0 ft	ICD Flow (l/s)	External Flow (l/s)	CUMM Est. Flow (l/s)	1:100 Yr Peak Flow (l/s)	Q ₁₀₀ Coeff	Dia (mm)	Slope %	Q Full (l/s)	V Full (m/s)	V Partial (m/s)	Depth of flow (m)	V surch (m/s)	Length (m)	Flow Time (min)	Pi Center Line	Obvert	Obvert Drop	Invert	Cover	Pi Center Line	Obvert	Obvert Drop	Invert	Cover	I	L/D	K	FRICT	V2/2g (m)	Straight	Expansion	Bend	Major Loss (m)	Minor Loss (m)	Total Losses (m)	Applied Losses (m)	1:100 YR HGL ELEVATIONS	COVER ELEVATIONS	Upstr.	Downs.	Upstr.	Downs.		
KLONDIKE RD	667A	667								0.00	14.07	28.92	84.81	1192.95			35	700		212.00	912.00	0.95	750	0.68	957.73	2.10	2.41	0.587	2.00	63.00	0.50	103.200	98.758		98.008	4.442	101.950	98.330		97.580	3.620	0.023	82.677	1.905	0.266	0.044				0.564	0.044	0.608	0.000	98.758	98.330	1.742	0.920	4.442	3.620		
KLONDIKE RD	667	666	0.220	0.240	0.030					0.63	14.70	28.92	84.81	1246.23			2	37	740		212.00	912.00	0.58	750	1.98	1834.25	3.58	3.80	0.411	2.09	100.00	0.47	101.950	98.330		97.580	3.620	99.550	96.350		95.600	3.200	0.023	131.234	3.024	0.734	0.110				2.221	0.110	2.331	0.000	98.330	96.350	0.920	0.500	3.620	3.200	
KLONDIKE RD	666	665	0.160	0.160						0.40	15.10	29.39	83.83	1265.48			1	38	760		212.00	912.00	0.42	750	4.00	2322.30	5.09	4.87	0.343	2.13	97.50	0.32	101.950	98.330		97.580	3.620	95.000	91.850		91.000	3.150	0.023	127.953	2.948	0.734	0.182				3.568	0.182	3.749	0.000	95.750	91.850	1.100	0.450	3.800	3.150	
PIEKOFF CR	660	661	0.730							0.81	21.80	30.00	110.77	89.92			3	3	60		0.00	60.00	0.40	300	2.20	149.60	2.05	1.93	0.134	0.82	101.60	0.83	100.500	97.441		97.141	3.059	98.500	95.207		94.907	3.393	0.031	333.233	10.425	0.034	0.005				0.259	0.005	0.264	0.005	97.441	95.207	0.359	0.563	3.059	3.393	
PIEKOFF CR	661	663	0.530	0.070						0.69	1.50	20.83	107.57	161.18			3	6	120		0.00	120.00	0.56	300	4.50	214.03	2.93	3.06	0.162	1.64	78.80	0.45	98.500	95.207		94.907	3.393	94.900	91.660		91.360	3.240	0.031	258.530	8.085	0.138		0.138	0.141		1.115	0.279	1.394	1.394	95.207	91.664	0.503	0.506	3.293	3.206	
WALLESEND AVE	655	663	0.120	0.320						0.58	0.58	20.00	110.77	64.05			1	1	20		0.00	20.00	0.26	300	0.60	78.14	1.07	0.91	0.104	0.27	73.40	1.14	95.250	92.104		91.804	3.146	94.900	91.664		91.364	3.236	0.031	240.814	7.531	0.004	0.001				0.029	0.004	0.033	0.033	92.104	91.664	0.446	0.506	3.146	3.206	
WALLESEND AVE	663	664								0.00	2.08	21.27	105.92	219.97					7	140		0.00	140.00	0.61	450	0.60	230.39	1.40	1.46	0.256	0.85	32.40	0.38	94.900	91.664		91.244	3.206	94.600	91.500		91.050	3.100	0.027	70.866	1.936	0.037	0.006				0.072	0.006	0.077	0.077	91.664	91.500	0.506	0.400	3.206	3.100
WALLESEND AVE	664	665	0.350	0.160						0.61	2.68	21.66	104.56	281.08			1	8	160		0.00	160.00	0.69	450	0.60	230.39	1.40	1.50	0.279	0.97	48.30	0.57	94.600	91.500		91.050	3.100	95.000	91.210		90.760	3.790	0.027	105.643	2.886	0.048		0.048	0.049		0.072	0.098	0.170	0.077	91.500	91.252	0.400	1.048	3.100	3.748	
KLONDIKE RD	665	665A	0.160							0.22	18.01	29.71	83.18	1497.73			1	47	940		212.00	1152.00	0.93	750	3.48	2166.59	4.75	4.93	0.389	2.53	60.70	0.21	95.000	91.252		90.502	3.748	91.800	89.140		88.360	2.600	0.023	79.659	1.836	1.239	0.186	1.239			2.274	1.424	3.698	0.000	91.252	89.140	1.048	0.500	3.748	2.600	
KLONDIKE RD	665A	648								0.00	18.01	29.92	82.75	1490.00			0	47	940		212.00	1152.00	0.56	900	0.40	1194.45	1.82	2.09	0.713	1.75	22.50	0.21	91.800	88.690		87.790	3.110	91.800	88.600		87.700	3.200	0.022	24.006	0.504	0.222	0.033	0.222			0.119	0.256	0.374	0.000	88.600	88.600	0.410	0.500	3.110	3.200	
KLONDIKE RD	648	647	0.890	0.260	0.160					1.01	40.25	27.90	87.03	3502.76			4	111	2220		212.00	2432.00	0.95	1200	0.40	2572.39	2.20	2.51	0.939	2.08	85.00	0.64	91.800	88.606		87.406	3.194	91.400	88.266		87.066	3.134	0.020	89.718	1.374	0.321	0.048				0.441	0.048	0.489	0.000	88.606	88.266	0.484	0.434	3.194	3.134	
WIMBLETON ST	548A	548								0.00	0.00	20.00	110.77	0.00			0	0	0		0.00	0.00	0.00	300	0.40	63.80	0.87	0.00	0.000	0.00	14.00	0.27	91.673	88.908		88.608	2.765	91.500	88.852		88.552	2.648	0.031	45.932	1.437	0.000					0.000	0.000	0.000	0.000	88.908	88.852	0.065	-0.052	2.765	2.648	
WIMBLETON ST	548	547	0.720							1.00	1.00	20.27	109.71	109.80			3	3	60		0.00	60.00	0.52	375	0.40	115.68	1.01	1.03	0.194	0.53	55.50	0.91	91.500	88.852		88.477	2.648	91.400	88.630		88.255	2.770	0.029	145.869	4.229	0.054					0.232	0.000	0.232	0.000	88.852	88.630	0.070	0.070	2.648	2.770	
WIMBLETON ST	547	546	0.290							0.32	1.32	21.18	106.27	146.33			3	3	60		0.00	60.00	0.52	375	0.40	115.68	1.01	1.03	0.194	0.53	55.50	0.91	91.400	88.630		88.215	2.810	91.400	88.554		88.179	2.876	0.029	23.022	0.386	0.054		0.054	0.004		0.227	0.004	0.232	0.000	88.630	88.554	0.110	0.176	2.810	2.876	
WIMBLETON ST	546	545	0.530							0.74	2.08	21.33	105.73	217.52			3	6	120		0.00	120.00	0.40	450	0.40	188.11	1.15	1.22	0.261	0.73	73.00	0.26	91.400	88.554		88.104	2.876	91.470	88.262		87.812	3.208	0.027	159.868	4.262	0.076	0.011				0.330	0.011	0.341	0.000	88.554	88.262	0.278	0.503	3.208	2.876	
WIMBLETON ST	545	547	0.220							0.31	2.36	22.39	102.08	241.21			1	7	140		0.00	140.00	0.74	450	0.40	188.11	1.15	1.24	0.293	0.85	42.00	0.61	91.470	88.222		87.772	3.248	91.400	88.054		87.804	3.346	0.027	91.864	2.510	0.078		0.078	0.080		0.197	0.158	0.355	0.000	88.222	88.054	0.548	0.646	3.248	3.346	
KLONDIKE RD	647	646	0.810							0.88	43.29	28.55	85.81	3705.83			3	121	2420		212.00	2632.00	0.96	1200	0.45	2726.43	2.34	2.68	0.951	2.25	99.00	0.71	91.500	88.266		87.066	3.334	91.650	87.821		86.621	3.830	0.020	81.201	1.600	0.367	0.055				0.588	0.055	0.643	0.000	88.266	87.821	0.834	1.129	3.334	3.830	
KLONDIKE RD	646	645	0.920							1.02	44.31	29.25	84.11	3726.91			3	124	2480		212.00	2692.00	0.85	1200	0.60	3150.52	2.70	3.02	0.866	2.31	98.00	0.61	91.650	88.266		86.620	3.830	90.400	87.232		86.032	3.168	0.020	80.381	1.584	0.466		0.476	0.738		0.738	0.476	1.214	0.000	87.820	87.232	1.130	1.468	3.830	3.168	
KLONDIKE RD	645	644	0.130							0.14	44.45	29.86	82.87	3684.03			1	110	2200		212.00	2412.00	0.84	1200	0.50	2869.68	2.46	2.75	0.853	2.07	70.30	0.48	90.400	87.170		85.970	3.230	92.000	86.820		85.620	5.180	0.020	57.661	1.136	0.387	0.058				0.440	0.058	0.498	0.000	87.170	86.820	0.530	2.480	3.230	5.180	
BRECHIN CR	695	695	0.420							0.47	40.47	20.00	110.77	51.73			2	2	40		0.00	40.00	0.40	300	1.00	100.88	1.38	1.33	0.131	0.55	46.70	0.59	92.850	89.936		89.636	2.914	92.505	89.449		89.149	3.056	0.031	159.777	4.997	0.060		0.060	0.025		0.460	0.115	0.565	0.000	89.936	89.449	0.214	0.356	3.056	3.164	
BRECHIN CR	695	695	0.340							0.36	0.85	20.59	108.47	91.67			1	3	60		0.00	60.00	0.39	375	0.70	153.03	1.24	1.28	0.164	0.53	67.60	1.09	92.505	89.449		89.074	3.056	92.000	88.936		88.461	3.184	0.029	229.921	6.675	0.083		0.083	0.085		0.554	0.168	0.722	0.000	89.449	88.936	0.356	0.464	3.		



CITY OF OTTAWA
MINTO DEVELOPMENTS INC.
MORGAN'S GRANT SUBDIVISION - PHASE 12D
JLR NO. 17732

STORM SEWER DESIGN SHEET
 Rev. No. 0: MOE Submission for Phase 12D - May 11/ 2005
 Rev. No. 1: City Comments for Phase 12D - July 11/ 2005
 Rev. No. 2: City Comments for Phase 12D - August 11/ 2006
 Rev. No. 3: Issued with Phase 12D SWM Report - August 24/ 2007
 Designed by: J.B.
 Checked by: L.J./G.F.

DESIGN PARAMETERS

Manning's Coefficient, $n =$	0.013
IDF CURVE =	5 year

STREET	M.H. #		AREAS FOR "R" in (ha)							PEAK FLOW COMPUTATION					SEWER							UPSTREAM				DOWNSTREAM				COMMENTS			
	FROM	TO	0.2	0.3	0.4	0.45	0.5	0.6	0.7	2.78AR	2.78AR (CUM.)	TIME (min.)	INTENS. (mm/hr)	PEAK FL. (L/s)	DIA. (mm)	SLOPE (%)	CAPAC. (L/s)	VEL. (m/s)	LENGTH (m)	FL.TIME (min.)	RESIDUAL CAP. (L/s)	Pr. Center Line	Obvert Drop	Obvert	Invert	Cover	Pr. Center Line	Obvert	Invert		Cover		
BLK 248	Hydro Ea.	906	0.720		0.118					0.53	0.53	26.00	59.35	31.54	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	Flow controlled to 30 L/s
GOWARD DRIVE	908	907			0.702		1.216			2.47	2.47	20.00	70.25	173.58	450	1.05	304.76	1.86	112.00	1.01	131.18	102.20		99.400	98.943	2.80	101.34	98.224	97.767	3.12	PHASE 12D		
	907	906			0.203					0.23	2.70	21.01	68.12	213.69	525	0.40	283.74	1.27	114.87	1.51	70.05	101.34	0.027	98.197	97.664	3.14	100.70	97.738	97.204	2.96	+ Fixed flowrate from Blks 247/248 (30 L/s)		
ISHPATINA CRESCENT	804	801								0.00	0.00	20.00	70.25	0.00	300	0.40	63.80	0.87	17.43	0.33	63.80	100.96		98.060	97.755	2.90	100.96	97.990	97.685	2.97	PHASE 12D		
	801	906								0.00	0.00	20.33	69.53	0.00	300	0.40	63.80	0.87	59.50	1.13	63.80	100.96	0.040	97.950	97.645	3.01	100.70	97.712	97.407	2.99	PHASE 12D		
													21.47																				
BLK 246	705	704	1.157		0.191					0.86	0.86	30.00	53.93	46.14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	Flow controlled to 50 L/s	
FINLAYSON CRESCENT	705	704								0.00	0.00	20.00	70.25	50.00	300	1.20	110.50	1.51	8.98	0.10	60.50	101.47		98.670	98.365	2.80	101.33	98.562	98.257	2.77	+ Fixed flowrate from Blk 246 (50 L/s)		
	704	701			0.360					0.40	0.40	20.10	70.03	78.04	300	1.20	110.50	1.51	41.34	0.45	32.46	101.33	0.040	98.522	98.217	2.81	100.77	98.026	97.721	2.74	PHASE 12D		
	701	700			0.180					0.20	0.60	20.55	69.06	91.47	375	0.40	115.67	1.01	40.92	0.67	24.21	100.77		98.026	97.645	2.74	100.62	97.862	97.481	2.76	PHASE 12D		
	700	906								0.00	0.60	21.23	67.67	90.63	375	0.40	115.67	1.01	12.73	0.21	25.04	100.62	0.150	97.712	97.331	2.91	100.70	97.662	97.281	3.04	PHASE 12D		
GOWARD DRIVE	906	905			0.837				0.93	4.23	22.51	65.18	355.56	600	0.45	429.67	1.47	67.57	0.76	74.12	100.70		97.662	97.052	3.04	100.66	97.357	96.748	3.30	PHASE 12D			
ISHPATINA CRESCENT	804	803	0.391		0.823					1.13	1.13	20.00	70.25	79.56	375	0.30	100.18	0.88	68.89	1.31	20.61	100.96		98.160	97.779	2.80	100.87	97.953	97.572	2.92	PHASE 12D		
	803	802								0.00	1.13	21.31	67.51	76.46	375	0.30	100.18	0.88	11.88	0.23	23.72	100.87	0.040	97.913	97.532	2.96	100.86	97.878	97.497	2.98	PHASE 12D		
	802	905			0.049					0.05	1.19	21.53	67.06	79.60	375	0.30	100.18	0.88	67.47	1.28	20.57	100.86	0.040	97.838	97.457	3.02	100.66	97.635	97.254	3.02	PHASE 12D		
GOWARD DRIVE	905	904			0.415				0.46	5.88	23.28	63.79	454.85	600	0.70	535.90	1.84	48.61	0.44	81.04	100.66	0.041	97.316	96.707	3.34	100.91	96.976	96.367	3.93	PHASE 12D			
FINLAYSON CRESCENT	705	703								0.00	0.00	20.00	70.25	0.00	300	0.35	59.68	0.82	77.42	1.58	59.68	101.47		98.670	98.365	2.80	101.33	98.399	98.094	2.93	PHASE 12D		
	703	702								0.00	0.00	21.58	66.97	0.00	300	0.35	59.68	0.82	12.38	0.25	59.68	101.33	0.040	98.359	98.054	2.97	101.23	98.316	98.011	2.91	PHASE 12D		
	702	904			0.548					0.61	0.61	21.83	66.47	40.51	300	0.45	67.67	0.93	86.27	1.55	27.16	101.23	0.040	98.276	97.971	2.95	100.91	97.887	97.583	3.02	PHASE 12D		
													23.38																				
GOWARD DRIVE	904	903			0.552					0.61	7.10	23.72	63.02	527.41	600	1.20	701.66	2.40	77.00	0.53	174.24	100.91	0.963	96.013	95.404	4.90	99.50	95.089	94.480	4.41	PHASE 12C		
	903	902								0.00	7.10	24.25	62.11	520.99	600	1.73	842.47	2.89	40.00	0.23	321.49	99.50	0.100	94.990	94.380	4.51	97.59	94.298	93.688	3.29	PHASE 12C		
	902	901			0.355					0.39	7.49	24.48	61.73	542.64	600	1.84	868.37	2.98	40.00	0.22	325.73	97.59	0.600	93.698	93.088	3.89	95.75	92.962	92.353	2.79	PHASE 12C		
	901	900			0.417					0.46	7.96	24.71	61.37	568.35	675	0.85	807.97	2.19	72.50	0.55	239.62	95.75	0.465	92.497	91.812	3.25	94.95	91.882	91.196	3.07	PHASE 12C		
	900	114								0.00	7.96	25.26	60.48	561.32	750	1.09	1212.49	2.66	36.90	0.23	651.16	94.95	0.293	91.589	90.827	3.36	94.69	91.187	90.425	3.50	PHASE 12C		
TESKIWA CRESCENT	903	602			0.496					0.55	0.55	20.00	70.25	38.75	300	0.50	71.33	0.98	86.90	1.48	32.58	99.50		96.575	96.270	2.93	98.94	96.141	95.836	2.80	PHASE 12C		
	602	601			0.233					0.26	0.81	21.48	67.16	54.44	300	0.75	87.36	1.20	12.30	0.17	32.92	98.94	0.040	96.101	95.796	2.84	98.79	96.008	95.703	2.78	PHASE 12C		
	601	600			0.339					0.38	1.19	21.65	66.82	79.36	300	2.65	164.21	2.25	69.00	0.51	84.85	98.79	0.040	95.968	95.663	2.82	96.93	94.140	93.835	2.79	PHASE 12C		
	600	116								0.00	1.19	22.16	65.83	78.19	300	3.23	181.29	2.48	64.50	0.43	103.11	96.93	0.266	93.874	93.569	3.06	95.50	91.790	91.486	3.71	PHASE 12C		
	HALTON TERRACE	116	115			0.321					0.36	1.54	22.60	65.02	100.43	525	0.70	375.35	1.68	41.10	0.41	274.92	95.50	0.279	91.531	90.998	3.97	95.30	91.244	90.710	4.06	PHASE 12A	
115		114			0.594					0.66	2.21	23.00	64.28	141.74	525	0.70	375.35	1.68	40.80	0.40	233.61	95.30	0.040	91.204	90.670	4.10	94.65	90.918	90.385	3.73	PHASE 12A		
HALTON TERRACE	114	113			0.478					0.53	10.69	25.49	60.12	722.98	825	0.60	1159.90	2.10	46.10	0.37	436.92	94.65	0.040	90.878	90.040	3.77	94.05	90.601	89.763	3.45	PHASE 12A		
	113	112A			0.460					0.51	11.21	25.86	59.56	747.44	825	0.65	1207.26	2.19	33.00	0.25	459.82	94.05	0.030	90.571	89.733	3.48	94.10	90.357	89.519	3.74	PHASE 12A		
	112A	112								1.61	12.82	26.11	59.18	838.48	825	0.65	1207.26	2.19	31.00	0.24	368.79	94.10		90.357	89.519	3.74	94.25	90.155	89.317	4.09	+School Flow (2.78xAC = 1.61) from CCL		
DGE MOORE CRESCENT	901	504	0.513		0.249					0.56	0.56	20.00	70.25	39.49	300	0.50	71.33	0.98	69.30	1.18	31.84	95.75		93.150	92.845	2.60	95.61	92.804	92.499	2.81	PHASE 12C		
	504	503								0.00	0.56	21.18	67.76	38.09	300	1.00	100.87	1.38	12.30	0.15	62.78	95.61	0.040	92.764	92.459	2.85	95.51	92.641	92.336	2.87	PHASE 12C		
	503	502	0.408		0.599					0.89	1.46	21.33	67.46	98.16	375	0.85	168.62	1.48	98.00	1.10	70.46	95.51		92.641	92.260	2.87	94.60	91.808	91.427	2.79	PHASE 12C		
	502	501	0.282		0.987					1.25	2.71	22.43	65.32	176.99	525	0.35	265.41	1.19	93.00	1.30	88.42	94.60		91.808	91.274	2.79	94.44	91.482	90.949	2.96	PHASE 12C		

STREET	M.H. #		AREAS FOR "R" in (ha)							PEAK FLOW COMPUTATION					SEWER						UPSTREAM				DOWNSTREAM				COMMENTS		
	FROM	TO	0.2	0.3	0.4	0.45	0.5	0.6	0.7	2.78AR	2.78AR (CUM.)	TIME (min.)	INTENS. (mm/hr)	PEAK FL. (L/s)	DIA. (mm)	SLOPE (%)	CAPAC. (L/s)	VEL. (m/s)	LENGTH (m)	FL.TIME (min.)	RESIDUAL CAP. (L/s)	Pr. Center Line	Obvert Drop	Obvert	Invert	Cover	Pr. Center Line	Obvert		Invert	Cover
MUSKEGO CRESCENT	402	111			0.222					0.25	0.25	20.00	70.25	17.34	300	0.87	94.09	1.29	78.77	1.02	76.75	92.06		89.473	89.168	2.59	92.51	88.788	88.483	3.72	PHASE 12B
HALTON TERRACE	111 110	110 109			0.200 0.579					2.46 0.64	19.08 19.72	26.58 26.99	58.48 57.90	1195.63 1221.74	825 825	1.20 1.20	1640.35 1640.35	2.97 2.97	72.40 81.90	0.41 0.46	444.72 418.61	92.51 91.00	0.017 0.560	88.771 87.342	87.933 86.504	3.74 3.66	91.00 90.10	87.902 86.359	87.064 85.521	3.10 3.74	+School Flow (2.78xAC = 2.24) from CCL PHASE 12A
MUSKEGO CRESCENT	402 401 400	401 400 303			0.236 0.427 0.976					0.26 0.66 1.19	0.26 0.92 2.12	20.00 20.19 20.78	70.25 69.84 68.59	18.44 64.46 145.19	300 300 375	0.80 2.30 1.74	90.22 153.08 241.26	1.24 2.10 2.12	13.84 74.30 70.02	0.19 0.59 0.55	71.79 88.62 96.07	92.06 91.96 90.24		89.179 89.028 87.317	88.874 88.723 86.936	2.88 2.93 2.92	91.96 90.24 88.84	89.068 87.317 86.099	88.763 87.012 85.718	2.89 2.92 2.74	PHASE 12B PHASE 12B PHASE 12B
DUNOLLIE CRESCENT	304	303						0.154		0.21	0.21	20.00	70.25	15.04	300	0.30	55.25	0.76	11.22	0.25	40.21	88.65		86.134	85.829	2.52	88.84	86.100	85.796	2.74	PHASE 12B
DUNOLLIE CRESCENT	303	109			0.240					0.27	2.60	21.33	67.46	175.25	525	0.36	269.18	1.20	85.61	1.18	93.93	88.84		86.099	85.565	2.74	90.10	85.791	85.257	4.31	PHASE 12B
HALTON TERRACE	109	108			0.130	0.460	0.147			0.92	23.24	27.45	57.25	1410.62	825	1.20	1640.35	2.97	66.80	0.37	229.73	90.10		85.791	84.953	4.31	88.53	84.990	84.151	3.54	PHASE 12A
DUNOLLIE CRESCENT	302A 302 301 300	302 301 300 108	0.216					0.085 0.716 0.288		0.24 1.00 0.40 0.00	0.24 1.23 1.63 1.63	20.00 20.26 21.48 21.64	70.25 69.69 67.17 66.84	16.74 85.96 109.74 109.21	300 375 375 450	0.50 0.35 0.40 0.20	71.33 108.20 115.67 133.01	0.98 0.95 1.01 0.81	15.18 69.40 9.99 90.70	0.26 1.22 0.16 1.87	54.59 22.24 5.94 23.80	88.53 88.45 88.20 88.27		85.387 85.311 85.028 84.988	85.082 84.930 84.647 84.531	3.14 3.14 3.17 3.28	88.45 88.20 88.27 88.53	85.311 85.068 84.988 84.807	85.006 84.687 84.607 84.350	3.14 3.13 3.28 3.72	PHASE 12B PHASE 12B PHASE 12B PHASE 12B
HALTON TERRACE	108 107	107 106			0.500					0.63 0.00	25.50 25.50	27.82 28.07	56.74 56.40	1526.82 1518.28	1050 1050	0.45 0.45	1910.95 1910.95	2.14 2.14	31.70 43.10	0.25 0.34	384.13 392.67	88.53 88.75		84.807 84.624	83.740 83.557	3.72 4.13	88.75 88.05	84.664 84.430	83.597 83.363	4.09 3.62	PHASE 12A PHASE 12A
McBRIEN STREET	203 202 201 200	202 201 200 106			0.130			0.690		0.14 0.96 0.00 0.00	0.14 1.10 1.10 1.10	20.00 20.96 21.80 21.94	70.25 68.21 66.53 66.25	10.16 75.28 73.43 73.12	300 375 375 375	1.52 0.85 0.85 1.75	124.37 168.62 168.62 241.95	1.70 1.48 1.48 2.12	98.50 74.40 12.70 20.90	0.96 0.84 0.14 0.16	114.21 93.35 95.20 168.83	90.71 89.09 88.60 88.35		87.706 86.209 85.546 85.408	87.401 85.828 85.165 85.027	3.00 2.88 3.05 2.94	89.09 88.60 88.35 87.92	86.209 85.576 85.438 85.043	85.904 85.195 85.057 84.662	2.88 3.02 2.91 2.88	PHASE 12A PHASE 12A PHASE 12A PHASE 12A
HALTON TERRACE	106 105	105 Ex. 101			0.447 0.312	0.652	0.084			0.56 1.54	27.16 28.70	28.40 28.69	55.95 55.57	1599.90 1675.06	1050 1200	0.55 0.40	2112.63 2572.29	2.36 2.20	41.00 88.70	0.29 0.67	512.74 897.23	88.05 87.05	0.040 0.215	84.390 83.950	83.323 82.730	3.66 3.10	87.25 87.10	84.165 83.595	83.098 82.376	3.09 3.51	PHASE 12A PHASE 12A

$\Sigma = 8.885ha$

Total area = 24.834ha

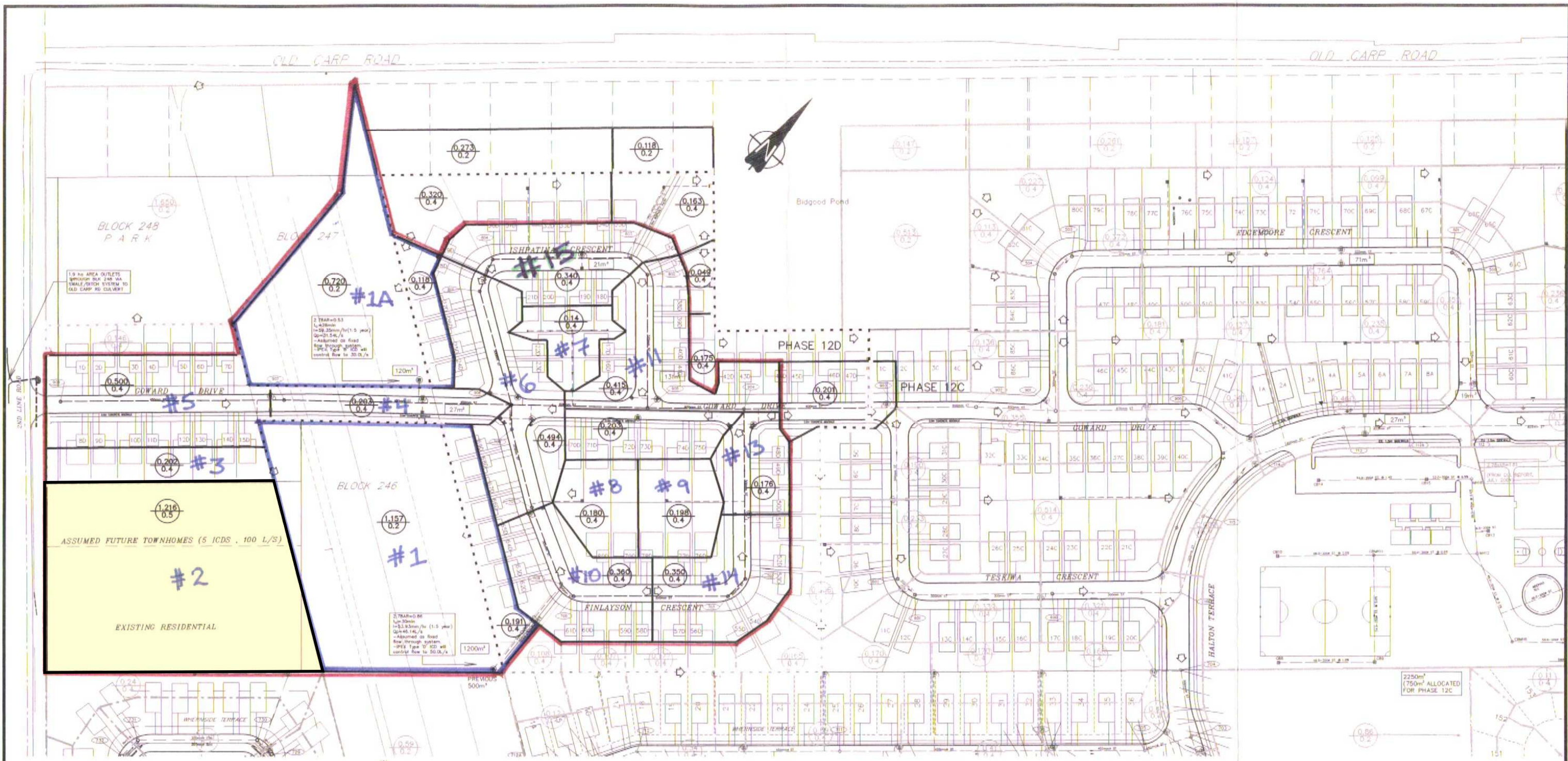
CITY OF OTTAWA
 MORGAN'S GRANT PHASE 12D SUBDIVISION
 MINTO DEVELOPMENTS INC.

Designed by: J.B.
 Checked by: G.F.
 Date: August 2005

JLR Project No. 17732

STORMWATER STORAGE / OVERFLOW BALANCE TABLE

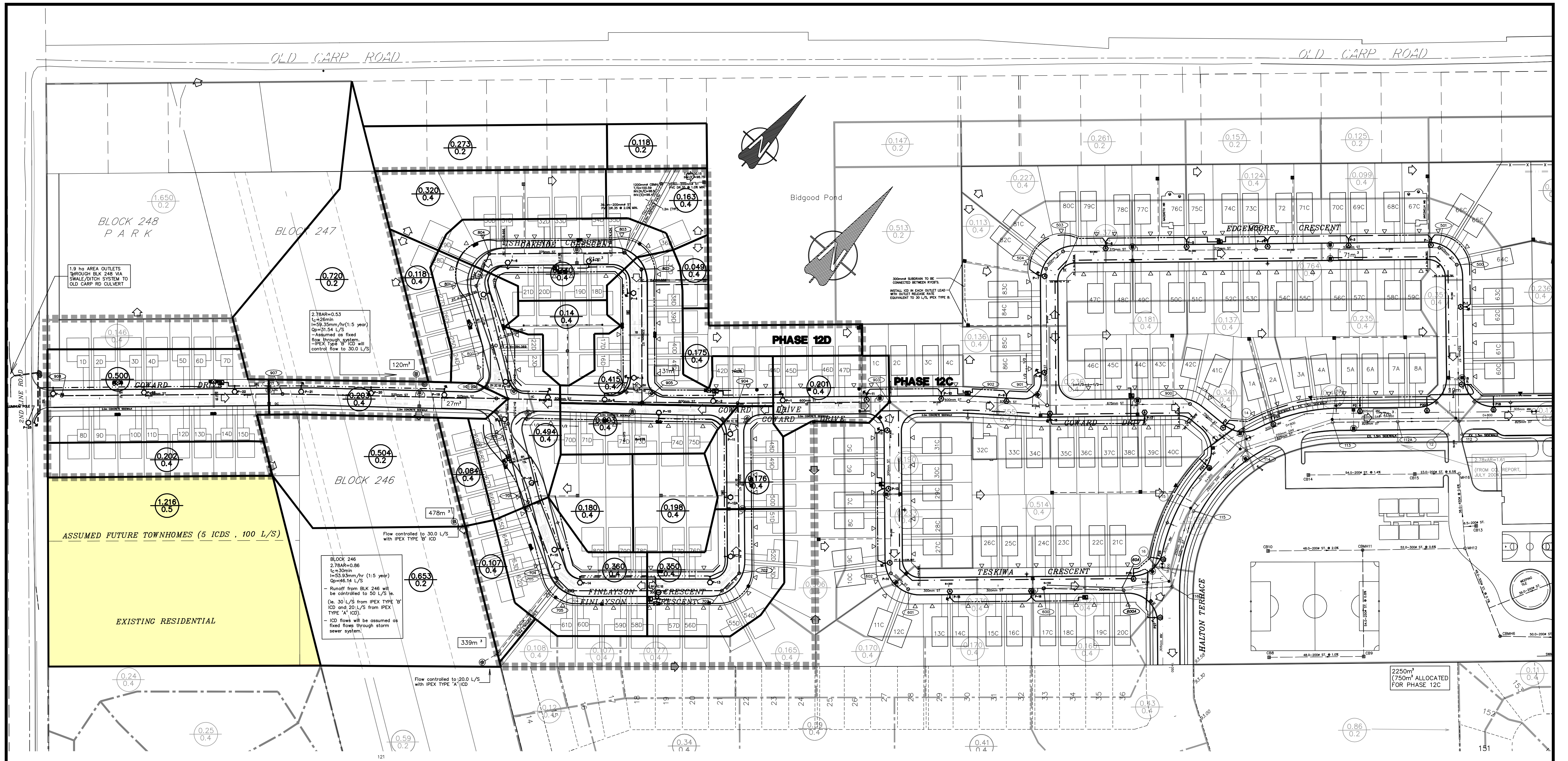
DRAINAGE AREA				INLET FLOW				STORAGE (m ³)			OVERFLOW	SURPLUS	
CATCHMENT	AREA #	"C" FACTOR	AREA (Ha)	INLETS (l/s)		Unrest. RYCBs	Equiv. Flow	REQUIRED	LOCAL + OVERFLOW (m ³)	PROVIDED	(m ³)	TO	STORAGE m ³
				LOCAL (m ³)	(m ³)			AREA #					
ISHPATINA	#15	0.400	0.340	1	0	0	20	29.51	29.51	20.60	8.91	#11	
FINLAYSON	#14	0.400	0.350	1	0	0	20	30.72	30.72	0.00	30.72	#13	
FINLAYSON (at GOWARD)	#13	0.400	0.176	1	0	0	20	11.88	42.61	0.00	42.61	#11	
GOWARD	#11	0.400	0.618	2	0	0	40	51.73	103.26	131.30	-28.04	#6	28.04
FINLAYSON	#10	0.400	0.360	1	0	0	20	31.95	31.95	0.00	31.95	#6	
RY (73, 74, 75, 76, 77, 78)	#9	0.400	0.198	0	0	34	34	10.61	10.61	0.00	10.61	#8	
RY (70, 71, 72, 79, 80)	#8	0.400	0.180	0	0	34	34	8.90	19.51	0.00	19.51	#6	
RY (16-23)	#7	0.400	0.140	0	0	62	62	0.00	0.00	0.00	0.00	#6	
GOWARD (at FINLAYSON/ISHPATINA)	#6	0.400	0.494	2	0	0	40	37.97	89.43	0.00	89.43	#4	
GOWARD	#5	0.400	0.500	1	0	0	20	50.26	50.26	0.00	50.26	#4	
GOWARD	#4	0.400	0.203	1	0	0	20	14.47	154.16	27.08	127.08	#1	
FUTURE TOWNHOUSES	#2	0.500	1.216	5	0	0	100	126.46	126.46	0.00	126.46	#1	
RY(8-15)	#3	0.400	0.202	0	0	34	34	10.99	10.99	0.00	10.99	#1	
BLK 246 and RY of units 62-69	#1	0.228	1.348	2.5	0	0	50	64.19	328.72	1213.00	-884.28	-	884.28



- LEGEND**
- EXISTING CATCH BASIN
 - PROPOSED CATCH BASIN
 - INTERCONNECTED ROADWAY CB C/W ONE 19.8L/S IPEX TYPE 'A' ICD OR CITY APPROVED EQUIVALENT
 - ⊕ CATCH BASIN WITH INDIVIDUAL 19.8 L/S IPEX TYPE 'A' ICD OR CITY APPROVED EQUIVALENT
 - PROPOSED HYDRANT
 - EXISTING HYDRANT
 - PHASING LIMITS
 - EXISTING STORM SEWER & MANHOLE
 - PROPOSED STORM SEWER & MANHOLE
 - PROPOSED CATCH BASIN & LEAD
 - 8 LOT NUMBER
 - ◇ PROPOSED OVERLAND DRAINAGE
 - DRAINAGE AREA LIMITS
 - ⊙ EXISTING AREA IN HECTARES
 - ⊙ 'R' RUNOFF COEFFICIENT
 - ⊙ PROPOSED AREA IN HECTARES
 - ⊙ 'R' RUNOFF COEFFICIENT
 - 1300m³ PONDING VOLUME

MAJOR OVERLAND FLOW IS ROUTED TO HYDRO EASEMENT

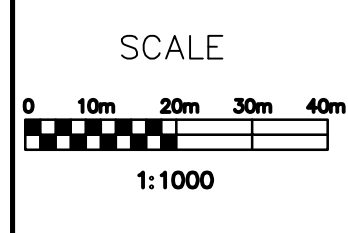
		SCALE		<p>J.L. Richards & Associates Limited 864 Lady Ellen Place Ottawa, ON Canada K1Z 5M2 Tel: 613 728 3571 Fax: 613 728 6012</p>	<p>DESIGN: J.B.</p> <p>CHECKED: L.J.</p> <p>DRAWN: A.M.</p> <p>CHECKED:</p> <p>APPROVED:</p>		<p>MORGAN'S GRANT PHASE 12D STORM DRAINAGE PLAN</p>		JOB No.
3	26/08/05	ISSUED WITH 12D SWM REPORT	L.J.						17732
2	11/07/05	REVISED PER CITY COMMENTS	L.J.	DATED: APRIL 2005					
1	11/05/05	ISSUED FOR MOE APPROVAL	L.J.	DWG No.					
								D-ST 7	



- LEGEND**
- EXISTING CATCH BASIN
 - PROPOSED CATCH BASIN
 - INTERCONNECTED ROADWAY CB C/W ONE 19.8 L/S IPEX TYPE 'A' ICD OR CITY APPROVED EQUIVALENT
 - CATCH BASIN WITH INDIVIDUAL 19.8 L/S IPEX TYPE 'A' ICD OR CITY APPROVED EQUIVALENT
 - PROPOSED HYDRANT
 - EXISTING HYDRANT
 - PHASING LIMITS
 - EXISTING STORM SEWER & MANHOLE
 - PROPOSED STORM SEWER & MANHOLE
 - PROPOSED CATCH BASIN & LEAD
 - LOT NUMBER
 - PROPOSED OVERLAND DRAINAGE
 - DRAINAGE AREA LIMITS
 - EXISTING AREA IN HECTARES 'R' RUNOFF COEFFICIENT
 - PROPOSED AREA IN HECTARES 'R' RUNOFF COEFFICIENT
 - 1300m³ PONDING VOLUME

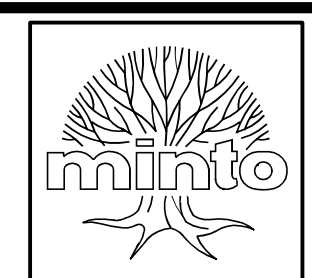
RECORD DRAWING
 PRODUCED FROM INFORMATION PROVIDED BY FIELD INSPECTOR DATE: MAY 03, 2013
J.L. RICHARDS & ASSOCIATES LIMITED

6	03/05/13	RECORD DRAWING	L.J.
4	18/10/05	REVISED PER HYDRO ONE COMMENTS	L.J.
3	26/08/05	ISSUED WITH 12D SWM REPORT	L.J.
2	11/07/05	REVISED PER CITY COMMENTS	L.J.
1	11/05/05	ISSUED FOR MOE APPROVAL	L.J.



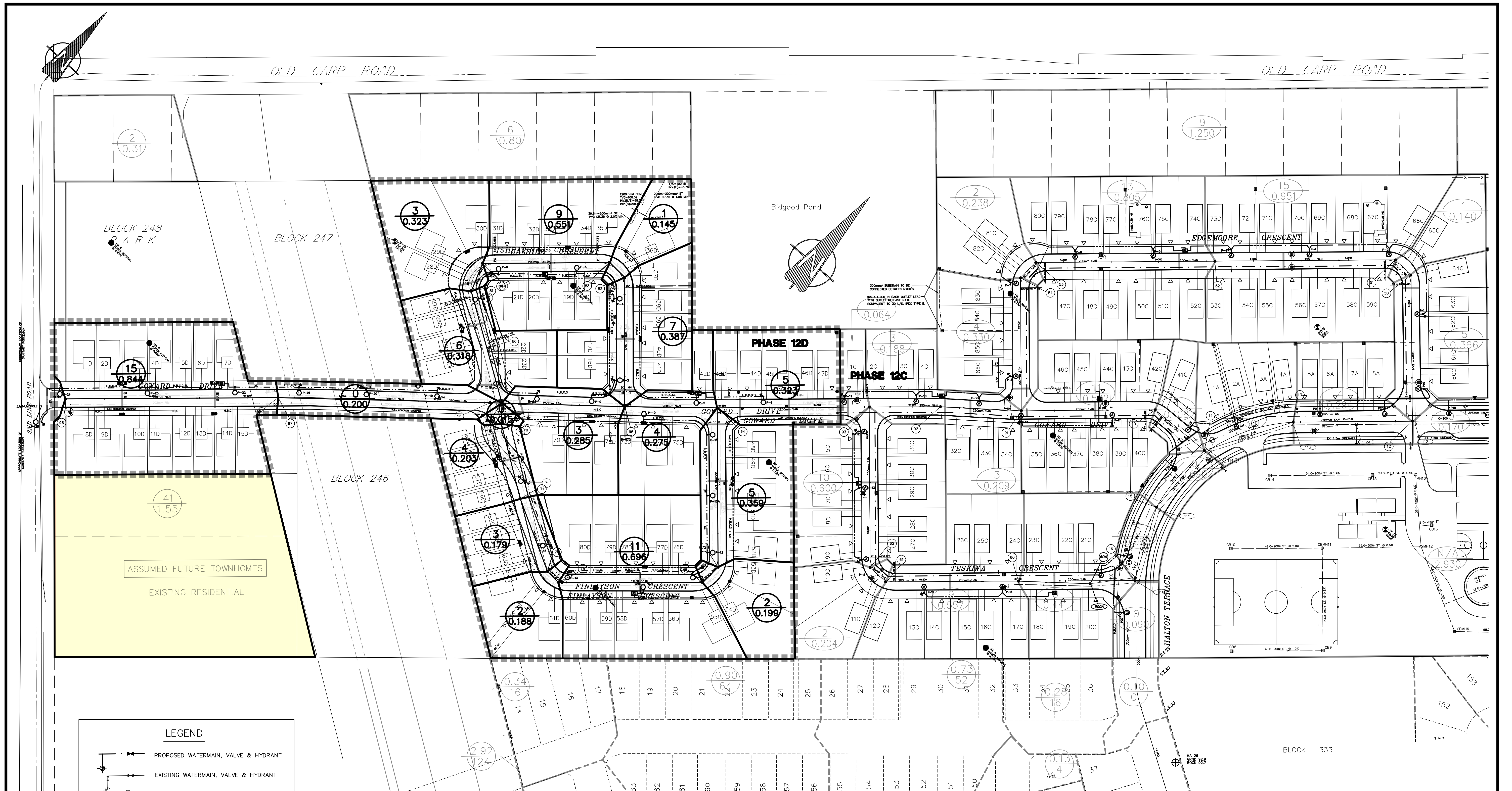
J.L. Richards & Associates Limited
 864 Lady Ellen Place
 Ottawa, ON Canada K1Z 5M2
 Tel: 613 728 3571
 Fax: 613 728 6012

DESIGN	J.B.
CHECKED	L.J.
DRAWN	A.M.
CHECKED	
APPROVED	



MORGAN'S GRANT
 PHASE 12D
 STORM DRAINAGE PLAN

JOB. No.	17732
DATED	APRIL 2005
DWG. No.	D-ST 7

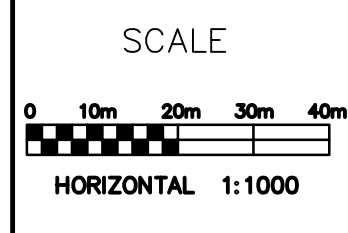


LEGEND

- PROPOSED WATERMAIN, VALVE & HYDRANT
- EXISTING WATERMAIN, VALVE & HYDRANT
- EXISTING SANITARY SEWER & MANHOLE
- PROPOSED SANITARY SEWER & MANHOLE
- 45D LOT NUMBER
- EXISTING DRAINAGE BOUNDARY
- EXISTING NUMBER OF UNITS
- EXISTING AREA IN HECTARES
- PHASE 12D LIMITS
- DRAINAGE BOUNDARY
- NUMBER OF UNITS
- AREA IN HECTARES

RECORD DRAWING
 PRODUCED FROM INFORMATION PROVIDED BY FIELD INSPECTOR
 date: MAY 03, 2013
 J.L. RICHARDS & ASSOCIATES LIMITED

3	03/05/13	RECORD DRAWING	L.J.
2	11/07/05	BUILDING FOOTPRINT REVISED	L.J.
1	11/05/05	ISSUED FOR MOE APPROVAL	L.J.



J.L. Richards & Associates Limited
 864 Lady Ellen Place
 Ottawa, ON Canada
 K1Z 5M2
 Tel: 613 728 3571
 Fax: 613 728 6012

DESIGN	J.B.
CHECKED	L.J.
DRAWN	M.B.
CHECKED	
APPROVED	



MORGAN'S GRANT
 PHASE 12D
 SANITARY DRAINAGE PLAN

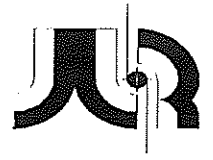
JOB. No.	17732
DATED	AUGUST 2004
DWG. No.	D-SAN 7

N:\17732\PHASE 12D\17732-C-D-SAN 7 132.dwg

Extracted pages from
Morgan's Grant MSS 2003

APPENDIX 'G'

SANITARY SEWER DESIGN SHEET (2003 MSS)



J.L. Richards & Associates Limited
 Consulting Engineers, Architects & Planners

CITY OF OTTAWA
MINTO DEVELOPMENT INC.
MORGAN'S GRANT SUBDIVISION - PHASE 10A & 10B
JLR NO. 17730

SANITARY SEWER DESIGN SHEET
 Revised September 16, 2003

Designed by: J.B.
 Checked by: L.J.

DESIGN PARAMETERS

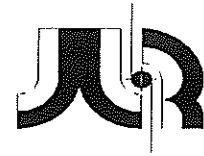
I = 0.280	l/s/ha	q (res) = 350	l/cap/day
Singles = 4.0	pers / unit	q (com) = 50,000	l/ha/day
Townhouses = 4.0	pers / unit	q (inst) = 50,000	l/ha/day

STREET	M.H. #		NO. of UNITS	RESIDENTIAL				NON-RESIDENTIAL				INFIL. FLOW l/s	PEAK FLOW l/s	SEWER DATA					UPSTREAM		DOWNSTREAM		COMMENTS				
	FROM	TO		INDIVIDUAL POPUL. people	INDIVIDUAL AREA ha	CUMMULATIVE POPUL. people	CUMMULATIVE AREA ha	Peaking Factor	POPUL. FLOW l/s	AREA ha	CUMM. AREA ha			Peaking Factor	NON-RES. FLOW (l/s)	DIA. mm	Slope %	CAPAC. l/s	VEL. m/s	LENGTH m	RESIDUAL CAP. (l/s)	Obvert Drop		Obvert	Invert	Obvert	Invert
	HALTON TERRACE	152		151	20	0.51	20	0.51	4.00	0.32	0.00			0.00	1.50	0.00	0.14	0.47	200	3.00	59.26	1.83		68.60	58.79		

DESIGN PARAMETERS

I = 0.280 l/s/ha	q (res) = 350 l/cap/day
Singles = 4.0 pers / unit	q (com) = 50,000 l/ha/day
Townhouses = 4.0 pers / unit	q (inst) = 50,000 l/ha/day

STREET	M.H. #		NO. of UNITS	RESIDENTIAL				Peaking Factor	POPUL. FLOW l/s	NON-RESIDENTIAL				INFIL. FLOW l/s	PEAK FLOW l/s	SEWER DATA						UPSTREAM		DOWNSTREAM		COMMENTS		
	FROM	TO		POPUL. people	AREA ha	POPUL. people	AREA ha			AREA ha	CUMM. AREA ha	Peaking Factor	NON-RES. FLOW (l/s)			DIA. mm	Slope %	CAPAC. l/s	VEL. m/s	LENGTH m	RESIDUAL CAP. (l/s)	Obvert Drop	Obvert	Invert	Obvert		Invert	
WHERNSIDE TERRACE	235	234		24	0.37	24	0.37	4.00	0.39	0.00	0.00	1.50	0.00	0.10	0.49	200	1.65	43.95	1.36	72.70	43.46			102.74	102.70	101.70	101.50	Phase 11
COLNE WAY	234	239		32	0.51	56	0.88	4.00	0.91	0.00	0.00	1.50	0.00	0.25	1.15	200	1.57	42.87	1.32	72.60	41.72			101.54	101.50	100.56	100.36	Phase 11
	239	240		28	0.42	84	1.30	4.00	1.36	0.00	0.00	1.50	0.00	0.36	1.73	200	2.50	54.10	1.67	55.10	52.37			100.32	100.27	99.09	98.89	Phase 11
	240	241		0	0.02	84	1.32	4.00	1.36	0.00	0.00	1.50	0.00	0.37	1.73	200	0.60	26.50	0.82	8.90	24.77			98.84	98.82	98.96	98.76	Phase 11
COLNE WAY	241	238		0	0.11	84	1.43	4.00	1.36	0.00	0.00	1.50	0.00	0.40	1.76	250	0.40	39.18	0.77	76.40	37.42			98.77	98.73	98.68	98.43	Phase 11
	238	242		0	0.15	168	2.80	4.00	2.72	0.00	0.00	1.50	0.00	0.78	3.51	250	0.40	39.23	0.77	77.30	35.73			98.44	98.40	98.34	98.09	Phase 11
KLONDIKE RD.	244	243		242	3.87	242	3.87	4.00	3.92	0.00	0.00	1.50	0.00	1.08	5.00	200	2.39	52.90	1.63	99.60	47.90			103.35	103.30	101.12	100.92	Phase 11
	243	242		39	0.85	281	4.72	4.00	4.55	0.00	0.00	1.50	0.00	1.32	5.87	200	1.00	34.21	1.06	104.70	28.34			100.83	100.80	99.95	99.75	Phase 11
	242	167A		0	0.18	449	7.70	4.00	7.27	0.00	0.00	1.50	0.00	2.16	9.43	250	0.50	43.86	0.87	36.60	34.44			97.73	97.69	97.76	97.51	Phase 11
STREET NO. 1	26	27		20	0.35	20	0.35	4.00	0.32	0.00	0.00	1.50	0.00	0.10	0.42	250	1.00	62.03	1.22	104.70	61.61			104.475	104.221	103.428	103.174	Phase 10B
	27	28		28	0.49	48	0.84	4.00	0.78	0.00	0.00	1.50	0.00	0.24	1.01	250	1.00	62.03	1.22	104.70	61.02			103.43	103.174	102.381	102.127	Phase 10B
	28	29		8		56	0.84	4.00	0.91	0.00	0.00	1.50	0.00	0.24	1.14	250	1.00	62.03	1.22	104.70	60.89			102.38	102.127	101.334	101.080	Phase 10B
	29	30		40	0.60	96	1.44	4.00	1.56	0.00	0.00	1.50	0.00	0.40	1.96	250	1.00	62.03	1.22	104.70	60.07			101.33	101.080	100.287	100.033	Phase 10B
	30	31		44	0.57	140	2.01	4.00	2.27	0.00	0.00	1.50	0.00	0.56	2.83	250	1.00	62.03	1.22	104.70	59.20			100.29	100.033	99.240	98.986	Phase 10B
	31	32				140	2.01	4.00	2.27	0.00	0.00	1.50	0.00	0.56	2.83	250	1.00	62.03	1.22	104.70	59.20			99.24	98.986	98.193	97.939	Phase 10B
STREET NO. 1	27	35		48	0.91	48	0.91	4.00	0.78	0.00	0.00	1.50	0.00	0.25	1.03	250	1.00	62.03	1.22	104.70	61.00			103.43	103.174	102.381	102.127	Phase 10B
	35	34		44	0.91	92	1.82	4.00	1.49	0.00	0.00	1.50	0.00	0.51	2.00	250	1.00	62.03	1.22	104.70	60.03			102.38	102.127	101.334	101.080	Phase 10B
	34	33				92	1.82	4.00	1.49	0.00	0.00	1.50	0.00	0.51	2.00	250	1.00	62.03	1.22	104.70	60.03			101.33	101.080	100.287	100.033	Phase 10B
	33	32		16		108	1.82	4.00	1.75	0.00	0.00	1.50	0.00	0.51	2.26	250	1.00	62.03	1.22	104.70	59.77			100.29	100.033	99.240	98.986	Phase 10B
HYDRO EASEMENT	32	36			1.80	248	5.63	4.00	4.02	0.00	0.00	1.50	0.00	1.58	5.59	250	1.00	62.03	1.22	104.70	56.44			98.19	97.939	97.146	96.892	Phase 10B
	36	37				248	5.63	4.00	4.02	0.00	0.00	1.50	0.00	1.58	5.59	250	1.00	62.03	1.22	104.70	56.44			97.15	96.892	96.099	95.845	Phase 10B
	37	167A				248	5.63	4.00	4.02	0.00	0.00	1.50	0.00	1.58	5.59	250	1.00	62.03	1.22	104.70	56.44			96.10	95.845	95.052	94.798	Phase 10B
KLONDIKE RD.	167A	167			697	13.33	3.90	11.00	0.00	0.00	1.50	0.00	3.73	14.73	250	2.20	92.01	1.82	100.00					95.05	94.798	92.852	92.598	Phase 10B
KLONDIKE RD.	167	166		92	0.84	789	14.17	3.86	12.35	0.00	0.00	1.50	0.00	3.97	16.32	250	2.20	92.01	1.82	100.00	75.69			97.580	97.330	95.380	95.130	PREVIOUSLY SUBMITTED FOR C. OF A.
	166	165		88	0.80	877	14.97	3.84	13.63	0.00	0.00	1.50	0.00	4.19	17.82	250	4.00	124.07	2.45	97.50	106.25			94.780	94.530	90.880	90.630	PREVIOUSLY SUBMITTED FOR C. OF A.
PIEKOFF CR.	160	161		48	0.69	48	0.69	4.00	0.78	0.00	0.00	1.50	0.00	0.19	0.97	200	2.20	50.72	1.56	102.10	49.75			96.844	96.644	94.600	94.400	PREVIOUSLY SUBMITTED FOR C. OF A.
	161	163		20	0.34	68	1.03	4.00	1.10	0.00	0.00	1.50	0.00	0.29	1.39	200	4.50	72.58	2.24	79.10	71.19			94.600	94.400	91.041	90.841	PREVIOUSLY SUBMITTED FOR C. OF A.
WALLSEND AVE.	155	163		80	0.54	80	0.54	4.00	1.30	0.00	0.00	1.50	0.00	0.15	1.45	200	0.60	26.50	0.82	70.70	25.05			91.504	91.304	91.080	90.880	PREVIOUSLY SUBMITTED FOR C. OF A.
WALLSEND AVE.	163	164		16	0.15	164	1.72	4.00	2.66	0.00	0.00	1.50	0.00	0.48	3.14	200	0.60	26.50	0.82	34.00	23.36			90.984	90.784	90.780	90.580	PREVIOUSLY SUBMITTED FOR C. OF A.
	164	165		16	0.18	180	1.90	4.00	2.92	0.00	0.00	1.50	0.00	0.53	3.45	200	0.60	26.50	0.82	46.70	23.05			90.780	90.580	90.500	90.300	PREVIOUSLY SUBMITTED FOR C. OF A.
KLONDIKE RD.	165	165A		40	0.35	1097	17.22	3.77	16.77	0.00	0.00	1.50	0.00	4.82	21.59	250	4.30	128.63	2.54	59.30	107.04			90.280	90.030	87.730	87.480	PREVIOUSLY SUBMITTED FOR C. OF A.
	165A	148		0	0.00	1097	17.22	3.77	16.77	0.00	0.00	1.50	0.00	4.82	21.59	250	0.40	39.23	0.77	21.00	17.64			87.134	86.884	87.050	86.800	PREVIOUSLY SUBMITTED FOR C. OF A.
KLONDIKE RD.	148	147		43	0.28	2268	33.18	3.54	32.55	0.00	0.00	1.50	0.00	9.29	41.84	300	0.40	63.80	0.87	85.3	21.96			87.016	86.716	86.675	86.375	PHASE 5
WIMBLEDON ST.	48A	48		12	0.13	12	0.13	4.00	0.19	0.00	0.00	1.50	0.00	0.04	0.23	200	1.00	34.21	1.06	15.0	33.98			88.213	88.013	88.063	87.863	PHASE 5
	48	47		28	0.38	40	0.51	4.00	0.65	0.00	0.00	1.50	0.00	0.14	0.79	200	0.60	26.50	0.82	53.3	25.71			88.033	87.833	87.713	87.513	PHASE 5
	47	46		12	0.04	52	0.55	4.00	0.84	0.00	0.00	1.50	0.00	0.15	1.00	200	0.60	26.50	0.82	9.1	25.50			87.683	87.483	87.628	87.428	PHASE 5
	46	45		80	0.61	132	1.16	4.00	2.14	0.00	0.00	1.50	0.00	0.32	2.46	200	0.60	26.50	0.82	71.9	24.04			87.598	87.398	87.167	86.967	PHASE 5
	45	147		16	0.17	148	1.33	4.00	2.40	0.00	0.00	1.50	0.00	0.37	2.77	200	0.60	26.50	0.82	41.0	23.73			87.167	86.967	86.921	86.721	PHASE 5
KLONDIKE RD.	147	146		24	0.54	2440	35.05	3.52	34.76	0.00	0.00	1.50	0.00	9.81	44.58	300	0.40	63.80	0.87	98.5	19.22			86.675	86.375	86.281	85.981	PHASE 5
	146	145		44	0.52	2484	35.57	3.51	35.33	0.00	0.00	1.50	0.00	9.96	45.29	300	0.60	78.14	1.07	96.8	32.85			86.281	85.981	85.700	85.400	PHASE 5



J.L. Richards & Associates Limited
Consulting Engineers, Architects & Planners

CITY OF OTTAWA

SANITARY SEWER DESIGN SHEET
Revised September 16, 2003

MINTO DEVELOPMENT INC.
MORGAN'S GRANT SUBDIVISION - PHASE 10A & 10B
JLR NO. 17730

Designed by: J.B.
Checked by: L.J.

DESIGN PARAMETERS

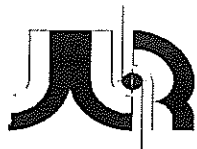
l = 0.280 l/s/ha	q (res) = 350 l/cap/day
Singles = 4.0 pers / unit	q (com) = 50,000 l/ha/day
Townhouses = 4.0 pers / unit	q (inst) = 50,000 l/ha/day

STREET	M.H. #		NO. of UNITS	RESIDENTIAL				NON-RESIDENTIAL				SEWER DATA					UPSTREAM		DOWNSTREAM		COMMENTS							
	FROM	TO		INDIVIDUAL	CUMMULATIVE	Peaking Factor	POPUL.	AREA	CUMM.	Peaking Factor	NON-RES.	INFIL.	PEAK	DIA.	Slope %	CAPAC.	VEL.	LENGTH	RESIDUAL CAP.	Obvert		Obvert	Invert	Obvert	Invert			
				POPUL. people	AREA ha		POPUL. people		AREA ha		RES. FLOW l/s									AREA ha		RES. FLOW (l/s)		FLOW l/s	FLOW l/s	Invert	Invert	
BRECHIN ST.	203	198		20	0.33	20	0.33	4.00	0.32	0.00	0.00	1.50	0.00	0.09	0.42	200	1.00	34.21	1.06	71.7	33.80			89.570	89.370	88.853	88.653	PHASE 7
EALING ST.	198	197		32	0.34	60	0.81	4.00	0.97	0.00	0.00	1.50	0.00	0.23	1.20	200	1.00	34.21	1.06	64.00	33.01			88.802	88.602	88.162	87.962	PHASE 7
	197	192		24	0.27	84	1.08	4.00	1.36	0.00	0.00	1.50	0.00	0.30	1.66	200	1.00	34.21	1.06	50.7	32.55			88.162	87.962	87.655	87.455	PHASE 7
PENRITH ST.	192	193		12	0.19	284	3.96	4.00	4.60	0.00	0.00	1.50	0.00	1.11	5.71	200	0.60	26.50	0.82	34.90	20.79			86.369	86.169	86.160	85.960	PHASE 7
	193	194		8	0.15	292	4.11	4.00	4.73	0.00	0.00	1.50	0.00	1.15	5.88	200	0.60	26.50	0.82	35.1	20.62			86.129	85.929	85.918	85.718	PHASE 7
BRECHIN ST.	196	195		16	0.45	16	0.45	4.00	0.26	0.00	0.00	1.50	0.00	0.13	0.39	200	1.00	34.21	1.06	47.5	33.83			89.346	89.146	88.871	88.671	PHASE 7
	195	194		48	0.67	64	1.12	4.00	1.04	0.00	0.00	1.50	0.00	0.31	1.35	200	0.70	28.62	0.88	85.2	27.27			88.801	88.601	88.205	88.005	PHASE 7
PENRITH ST.	194	145		20	0.28	376	5.51	4.00	6.09	0.00	0.00	1.50	0.00	1.54	7.64	250	0.40	39.18	0.77	73.30	31.55			85.942	85.692	85.650	85.400	PHASE 7
LAXFORD DR.	145	27		44	0.59	2904	41.67	3.45	40.64	0.00	0.00	1.50	0.00	11.67	52.30	300	0.40	63.80	0.87	103.60	11.49			85.672	85.372	85.258	84.958	PHASE 5
TYNE CRT.	41	40		56	0.97	56	0.97	4.00	0.91	0.00	0.00	1.50	0.00	0.27	1.18	200	1.00	34.21	1.06	50.00	33.03			87.800	87.600	87.300	87.100	PHASE 5
	40	27		16	0.29	72	1.26	4.00	1.17	0.00	0.00	1.50	0.00	0.35	1.52	200	1.00	34.21	1.06	72.00	32.69			86.626	86.426	85.906	85.706	PHASE 5
LAXFORD DR.	27	Stub		16	1.88	2992	44.81	3.44	41.74	0.00	0.00	1.50	0.00	12.55	54.28	300	0.44	67.28	0.92	34.80	13.00			85.258	84.958	85.103	84.803	PHASE 5
	26	26		0	0.00	2992	44.81	3.44	41.74	0.00	0.00	1.50	0.00	12.55	54.28	300	0.44	67.28	0.92	29.90	13.00			85.103	84.803	84.970	84.670	PHASE 5
	26	25		0	0.05	2992	44.86	3.44	41.74	0.00	0.00	1.50	0.00	12.56	54.30	300	0.43	66.40	0.91	12.00	12.11			84.905	84.605	84.853	84.553	PHASE 5
	25	24		8	1.99	3000	46.85	3.44	41.83	0.00	0.00	1.50	0.00	13.12	54.95	300	0.41	64.50	0.88	20.30	9.55			84.805	84.505	84.722	84.422	PHASE 5
REDCAR CR.	33	34		40	0.58	40	0.58	4.00	0.65	0.00	0.00	1.50	0.00	0.16	0.81	200	0.60	26.50	0.82	79.3	25.69			86.846	86.646	86.370	86.170	PHASE 5
	34	24		0	0.08	40	0.66	4.00	0.65	0.00	0.00	1.50	0.00	0.18	0.83	200	0.60	26.50	0.82	25.0	25.67			85.570	85.370	85.420	85.220	PHASE 5
LAXFORD DR.	24	22		36	0.60	3076	48.11	3.43	42.78	0.00	0.00	1.50	0.00	13.47	56.25	300	0.40	63.80	0.87	95.1	7.55			84.720	84.420	84.340	84.040	PHASE 5
KEIGHLIY CIRC.	13	14		4	0.05	4	0.05	4.00	0.06	0.00	0.00	1.50	0.00	0.01	0.08	200	0.60	26.50	0.82	11.5	26.42			88.021	87.821	87.952	87.752	PHASE 5
	14	15		24	0.42	28	0.47	4.00	0.45	0.00	0.00	1.50	0.00	0.13	0.59	200	0.60	26.50	0.82	40.0	25.92			87.922	87.722	87.682	87.482	PHASE 5
	15	16		20	0.42	48	0.89	4.00	0.78	0.00	0.00	1.50	0.00	0.25	1.03	200	0.72	29.03	0.90	58.0	28.00			87.625	87.425	87.207	87.007	PHASE 5
LARK LANE	16	22		0	0.00	48	0.89	4.00	0.78	0.00	0.00	1.50	0.00	0.25	1.03	200	4.25	70.53	2.17	79.0	69.51			87.567	87.367	84.210	84.010	PHASE 5
LAXFORD DR.	22	21		12	0.31	3136	49.31	3.43	43.52	0.00	0.00	1.50	0.00	13.81	57.33	300	0.40	63.80	0.87	50.1	6.47			84.300	84.000	84.100	83.800	PHASE 5
REDCAR CR.	33	32		60	0.66	60	0.66	4.00	0.97	0.00	0.00	1.50	0.00	0.18	1.16	200	1.46	41.35	1.28	97.5	40.20			86.624	86.424	85.200	85.000	PHASE 5
	32	31		8	0.06	68	0.72	4.00	1.10	0.00	0.00	1.50	0.00	0.20	1.30	200	1.46	41.34	1.27	10.9	40.04			84.819	84.619	84.660	84.460	PHASE 5
	31	30		28	0.55	96	1.27	4.00	1.56	0.00	0.00	1.50	0.00	0.36	1.91	200	0.58	26.06	0.80	73.4	24.14			84.626	84.426	84.200	84.000	PHASE 5
	30	21		4	0.13	100	1.40	4.00	1.62	0.00	0.00	1.50	0.00	0.39	2.01	200	0.68	28.25	0.87	28.7	26.24			84.196	83.996	84.000	83.800	PHASE 5
LAXFORD DR.	21	20		44	0.57	3280	51.28	3.41	45.30	0.00	0.00	1.50	0.00	14.36	59.66	300	0.81	90.79	1.24	87.2	31.13			83.916	83.616	83.210	82.910	PHASE 5
	20	138		0	0.00	3280	51.28	3.41	45.30	0.00	0.00	1.50	0.00	14.36	59.66	300	0.60	78.14	1.07	21.3	18.48			83.178	82.878	83.050	82.750	PHASE 5
KLONDIKE RD NORTH	144A	144		32	0.45	32	0.45	4.00	0.52	0.00	0.00	1.50	0.00	0.13	0.64	200	0.57	25.76	0.79	73.4	25.12			86.640	86.440	86.224	86.024	PHASE 5
GOULBOURN RD (PHASE 5B)	144	143		68	0.77	100	1.22	4.00	1.62	0.00	0.00	1.50	0.00	0.34	1.96	200	1.22	37.73	1.16	100.0	35.77			86.224	86.024	85.008	84.808	PHASE 5
	143	142A		40	0.42	140	1.64	4.00	2.27	0.00	0.00	1.50	0.00	0.46	2.73	200	1.83	46.24	1.43	82.0	43.52			85.008	84.808	83.510	83.310	PHASE 5
	142A	142		24	0.37	164	2.01	4.00	2.66	0.00	0.00	1.50	0.00	0.56	3.22	200	1.75	45.30	1.40	69.0	42.08			83.510	83.310	82.300	82.100	PHASE 5
WOLISTON CRES.	177	176		52	0.74	52	0.74	4.00	0.84	0.00	0.00	1.50	0.00	0.21	1.05	200	1.00	34.21	1.06	93.9	33.16			87.785	87.585	86.846	86.646	PHASE 6
	176	175		8	0.21	60	0.95	4.00	0.97	0.00	0.00	1.50	0.00	0.27	1.24	200	1.00	34.21	1.06	11.0	32.98			86.796	86.596	86.686	86.486	PHASE 6
	175	174		40	0.65	100	1.60	4.00	1.62	0.00	0.00	1.50	0.00	0.45	2.07	200	1.00	34.21	1.06	89.80	32.14			86.636	86.436	85.738	85.538	PHASE 6
	174	173		36	0.28	136	1.88	4.00	2.20	0.00	0.00	1.50	0.00	0.53	2.73	200	1.04	34.96	1.08	65.80	32.23			85.738	85.538	85.051	84.851	PHASE 6
PALTON ST.	189	173		24	0.26	24	0.26	4.00	0.39	0.00	0.00	1.50	0.00	0.07	0.46	200	1.05	35.06	1.08	78.50	34.60			85.925	85.725	85.101	84.901	PHASE 6
WOLISTON CRES. FLAMBOROUGH WAY	173	172		52	0.47	212	2.61	4.00	3.44	0.00	0.00	1.50	0.00	0.73	4.17	200	0.83	31.17	0.96	84.20	27.00			85.051	84.851	84.352	84.152	PHASE 6
	172	171		92	0.77	304	3.38	4.00	4.93	0.00	0.00	1.50	0.00	0.95	5.87	250	0.60	47.86	0.94	107.00	41.99			83.722	83.472	83.085	82.835	

DESIGN PARAMETERS

i = 0.280	l/s/ha	q (res) = 350	l/cap/day
Singles = 4.0	pers / unit	q (com) = 50,000	l/ha/day
Townhouses = 4.0	pers / unit	q (inst) = 50,000	l/ha/day

STREET	M.H. #		NO. of UNITS	RESIDENTIAL				Peaking Factor	POPUL. FLOW l/s	NON-RESIDENTIAL				INFIL. FLOW l/s	PEAK FLOW l/s	SEWER DATA					UPSTREAM		DOWNSTREAM		COMMENTS		
	FROM	TO		POPUL. people	AREA ha	CUMMULATIVE POPUL. people	CUMMULATIVE AREA ha			CUMM. AREA ha	Peaking Factor	NON-RES. FLOW (l/s)	DIA. mm			Slope %	CAPAC. l/s	VEL. m/s	LENGTH m	RESIDUAL CAP. (l/s)	Obvert Drop	Obvert	Invert	Obvert		Invert	
PALTON ST.	188A	189		40	0.54	72	1.12	4.00	1.17	0.00	0.00	1.50	0.00	0.31	1.48	200	1.08	35.56	1.10	77.10	34.08		86.091	85.891	85.260	85.060	PHASE 6
	189	185		36	0.34	108	1.46	4.00	1.75	0.00	0.00	1.50	0.00	0.41	2.16	200	0.60	26.50	0.82	55.00	24.34		85.200	85.000	84.870	84.670	PHASE 6
	185	184		24	0.35	132	1.81	4.00	2.14	0.00	0.00	1.50	0.00	0.51	2.65	200	0.60	26.50	0.82	61.80	23.86		84.870	84.670	84.500	84.300	PHASE 6
WOLISTON CRES.	184	170		48	0.54	324	4.58	4.00	5.25	0.00	0.00	1.50	0.00	1.28	6.53	200	1.71	44.74	1.38	84.20	38.21		84.516	84.316	83.076	82.876	PHASE 6
FLAMBOROUGH WAY	170	142		44	0.41	756	9.05	3.88	11.87	0.00	0.00	1.50	0.00	2.53	14.40	250	0.37	37.88	0.75	85.00	23.48		82.600	82.350	82.283	82.033	PHASE 6 (as-built info. added)
GOULBOURN RD (PHASE 5B)	142	141		12	0.22	932	11.28	3.82	14.42	0.00	0.00	1.50	0.00	3.16	17.58	250	0.31	34.68	0.68	64.0	17.10		82.283	82.033	82.083	81.833	PHASE 5 (as-built info. Added)
	141	140		4	0.14	936	11.42	3.82	14.48	0.00	0.00	1.50	0.00	3.20	17.68	250	0.27	31.99	0.63	28.2	14.32		82.032	81.782	81.957	81.707	PHASE 5 (as-built info. Added)
KEIGHLEY CIRC.	13	12		36	0.57	36	0.57	4.00	0.58	0.00	0.00	1.50	0.00	0.16	0.74	200	2.00	48.38	1.49	70.0	47.64		88.000	87.800	86.600	86.400	PHASE 5
	12	11		36	0.51	72	1.08	4.00	1.17	0.00	0.00	1.50	0.00	0.30	1.47	200	2.00	48.38	1.49	70.0	46.92		86.600	86.400	85.200	85.000	PHASE 5
KEIGHLEY CIRC.	16	17		44	0.70	44	0.70	4.00	0.71	0.00	0.00	1.50	0.00	0.20	0.91	200	2.00	48.38	1.49	79.0	47.48		86.580	86.380	86.000	85.800	PHASE 5
	17	18		4	0.10	48	0.80	4.00	0.78	0.00	0.00	1.50	0.00	0.22	1.00	200	1.00	34.21	1.06	12.5	33.21		85.955	85.755	85.830	85.630	PHASE 5
	18	19		12	0.20	60	1.00	4.00	0.97	0.00	0.00	1.50	0.00	0.28	1.25	200	1.00	34.21	1.06	35.0	32.96		85.800	85.600	85.450	85.250	PHASE 5
	19	11		0	0.15	60	1.15	4.00	0.97	0.00	0.00	1.50	0.00	0.32	1.29	200	0.60	26.50	0.82	38.0	25.21		85.428	85.228	85.200	85.000	PHASE 5
KEIGHLEY CIRC.	11	10		28	0.35	160	2.58	4.00	2.59	0.00	0.00	1.50	0.00	0.72	3.31	200	3.80	66.69	2.06	56.0	63.38		85.128	84.928	83.000	82.800	PHASE 5
	10	140		0	0.00	160	2.58	4.00	2.59	0.00	0.00	1.50	0.00	0.72	3.31	200	2.00	48.38	1.49	34.0	45.07		82.555	82.355	81.875	81.675	PHASE 5
GOULBOURN RD (PHASE 5B)	140	139		24	0.46	1120	14.46	3.77	17.09	0.00	0.00	1.50	0.00	4.05	21.14	250	0.31	34.53	0.68	81.0	13.39		81.942	81.692	81.691	81.441	PHASE 5 (as-built info. Added)
	139	138		48	0.59	1168	15.05	3.76	17.77	0.00	0.00	1.50	0.00	4.21	21.98	250	0.46	42.13	0.83	87.8	20.15		81.690	81.440	81.285	81.035	PHASE 5 (as-built info. Added)
GOULBOURN RD	138	137		4	0.35	4452	66.68	3.29	59.36	0.00	0.00	1.50	0.00	18.67	78.03	375	0.49	128.22	1.12	70.4	50.19		81.340	80.965	80.994	80.619	PHASE 5 (as-built info. Added)
GOULBOURN RD	135	136		48	0.66	48	0.66	4.00	0.78	0.00	0.00	1.50	0.00	0.18	0.96	200	1.39	40.26	1.24	72.2	39.30		84.200	84.000	83.200	83.000	PHASE 5
	136	137		20	0.39	68	1.05	4.00	1.10	0.00	0.00	1.50	0.00	0.29	1.40	200	0.71	28.83	0.89	70.4	27.43		83.200	83.000	82.700	82.500	PHASE 5
OAKHAM RIDGE	137	135				4520	67.73	3.29	60.16	0.00	0.00	1.50	0.00	18.96	79.12	375	0.40	115.49	1.01	46.7	36.37		80.872	80.497	80.686	80.311	EXISTING SEWER (as-built info. Added)
	135	134		24	0.40	4544	68.13	3.28	60.44	0.00	0.00	1.50	0.00	19.08	79.51	375	0.39	114.65	1.01	59.8	35.14		80.670	80.295	80.435	80.060	EXISTING SEWER (as-built info. Added)
	134	133		4	0.11	4548	68.24	3.28	60.48	0.00	0.00	1.50	0.00	19.11	79.59	375	0.70	152.55	1.34	5.75	72.96		80.425	80.050	80.385	80.010	EXISTING SEWER (as-built info. Added)
	133	132		28	0.25	4576	68.49	3.28	60.81	0.00	0.00	1.50	0.00	19.18	79.99	375	0.37	111.68	0.98	43.5	31.69		80.327	79.952	80.165	79.790	EXISTING SEWER (as-built info. Added)
	132	130		12	0.23	4588	68.72	3.28	60.95	0.00	0.00	1.50	0.00	19.24	80.19	375	0.36	109.88	0.96	44.1	29.69		80.137	79.762	79.978	79.603	EXISTING SEWER (as-built info. Added)
	131	130		52	0.73	52	0.73	4.00	0.84	0.00	0.00	1.50	0.00	0.20	1.05	200	2.52	54.34	1.68	110.0	53.30		82.850	82.650	80.075	79.875	EXISTING SEWER
Phases 1, 2, 3, 4w, 5, 6, 7, 8, 9 based on 4 pers/unit (Point B)	130	128		36	0.51	4676	69.96	3.27	61.98	0.00	0.00	1.50	0.00	19.59	81.56	450	0.15	115.42	0.70	97.6	33.86		80.047	79.597	79.900	79.450	EXISTING SEWER (as-built info. Added)
	128	4		32	0.54	4708	70.50	3.27	62.35	0.00	0.00	1.50	0.00	19.74	82.09	450	0.09	88.00	0.54	86.8	5.92		79.870	79.420	79.794	79.344	EXISTING SEWER (as-built info. Added)
Street No. 2	26	14		10	0.37	40	0.37	4.00	0.65	0.00	0.00	1.50	0.00	0.10	0.75	250	2.87	105.09	2.07	83.4	104.34		104.476	104.221	102.081	101.827	Stage 10A
	14	13		28	0.83	152	1.20	4.00	2.46	0.00	0.00	1.50	0.00	0.34	2.80	250	2.48	97.75	1.93	104.8	94.95	0.08	102.001	101.747	99.399	99.145	Stage 10A
	13	12		4	0.19	168	1.39	4.00	2.72	0.00	0.00	1.50	0.00	0.39	3.11	250	2.95	106.55	2.10	9.7	103.43	0.05	99.349	99.095	99.062	98.808	Stage 10A
	12	11		10	0.38	208	1.77	4.00	3.37	0.00	0.00	1.50	0.00	0.50	3.87	250	2.25	93.03	1.84	78.5	89.16	0.05	99.012	98.758	97.247	96.993	Stage 10A
Street No. 7	26	25		6	0.33	24	0.33	4.00	0.39	0.00	0.00	1.50	0.00	0.09	0.48	250	1.50	75.97	1.50	51.0	75.49		105.010	104.756	104.245	103.991	Stage 10A
	25	24		10	0.66	64	0.99	4.00	1.04	0.00	0.00	1.50	0.00	0.28	1.31	250	1.38	72.77	1.44	79.0	71.45	0.03	104.215	103.961	103.128	102.874	Stage 10A
	24	23		3	0.26	76	1.25	4.00	1.23	0.00	0.00	1.50	0.00	0.35	1.58	250	2.67	101.36	2.00	9.0	99.78	0.10	103.028	102.774	102.788	102.534	Stage 10A
	23	22		4	0.32	92	1.57	4.00	1.49	0.00	0.00	1.50	0.00	0.44	1.93	250	1.40	73.50	1.45	78.0	71.57	0.08	102.708	102.454	101.613	101.359	Stage 10A
Street No. 3	14	15		10	0.40	40	0.40	4.00	0.65	0.00	0.00	1.50	0.00	0.11	0.76	250	0.70	51.90	1.02	60.0	51.14		102.454	102.206	102.040	101.786	Stage 10A
	15	19		19	0.50	116	0.90	4.00	1.88	0.00	0.00	1.50	0.00	0.25	2.13	250	0.63	49.35	0.97	76.0	47.22	0.03	102.010	101.756	101.529	101.275	Stage 10A
Street No. 3			7	28	0.23	236	2.70	4.00	3.82	0.00	0.00	1.50	0.00	0.76	4.58	250	2.27	93.38	1.84	84.0	88.80	0.07	101.459	101.205	99.555	99.301	Stage 10A
Street No. 4	20	19		15	0.52	60	0.52	4.00	0.97	0.00	0.00	1.50	0.00	0.15	1.12	250	0.81	55.97	1.10	104.2	54.85		98.866	98.612	98.102	97.848	Stage 10A
	19	18		4	0.24	76	0.76	4.00	1.23	0.00	0.00	1.50	0.00	0.21	1.44	250	0.61	48.45	0.96	14.4	47.01	0					



J.L. Richards & Associates Limited
Consulting Engineers, Architects & Planners

CITY OF OTTAWA

SANITARY SEWER DESIGN SHEET
Revised September 16, 2003

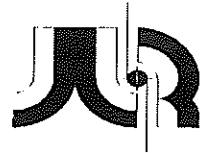
MINTO DEVELOPMENT INC.
MORGAN'S GRANT SUBDIVISION - PHASE 10A & 10B
JLR NO. 17730

Designed by: J.B.
Checked by: L.J.

DESIGN PARAMETERS

I = 0.280 l/s/ha	q (res) = 350 l/cap/day
Singles = 4.0 pers / unit	q (com) = 50,000 l/ha/day
Townhouses = 4.0 pers / unit	q (inst) = 50,000 l/ha/day

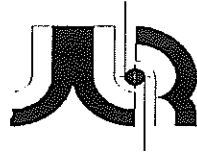
STREET	M.H. #		NO. of UNITS	RESIDENTIAL		CUMMULATIVE		Peaking Factor	POPUL. FLOW l/s	AREA ha	NON-RESIDENTIAL		INFIL. FLOW l/s	PEAK FLOW l/s	SEWER DATA					UPSTREAM		DOWNSTREAM		COMMENTS				
	FROM	TO		POPUL. people	AREA ha	POPUL. people	AREA ha				NON-RES. FLOW (l/s)	DIA. mm			Slope %	CAPAC. l/s	VEL. m/s	LENGTH m	RESIDUAL CAP. (l/s)	Obvert Drop	Obvert	Invert	Obvert		Invert			
Brady Avenue	7	7	11	44	0.61	384	4.42	4.00	6.22	0.00	0.00	1.50	0.00	1.24	7.46	250	1.83	83.94	1.66	85.2	76.48	0.07	99.485	99.231	97.925	97.671	Stage 10A	
Street No. 6	7	6	12	48	0.44	868	9.72	3.84	13.50	0.00	0.00	1.50	0.00	2.72	16.22	300	0.22	47.31	0.65	69.8	31.09	0.07	92.798	92.493	92.644	92.339	Stage 10A	
	6	5	26	104	0.79	972	10.51	3.81	14.99	0.00	0.00	1.50	0.00	2.94	17.94	300	0.22	47.31	0.65	101.4	29.38	0.03	92.614	92.309	92.391	92.086	Stage 10A	
	5	4	23	92	0.68	1064	11.19	3.78	16.30	0.00	0.00	1.50	0.00	3.13	19.44	300	0.22	47.31	0.65	84.5	27.88	0.03	92.361	92.056	92.175	91.870	Stage 10A	
	4	3	7	28	0.36	1092	11.55	3.78	16.70	0.00	0.00	1.50	0.00	3.23	19.93	300	0.22	47.31	0.65	95.4	27.38	0.03	92.145	91.840	91.935	91.631	Stage 10A	
	3	2	6	24	0.34	1116	11.89	3.77	17.04	0.00	0.00	1.50	0.00	3.33	20.37	300	0.22	47.31	0.65	61.5	26.95	0.03	91.905	91.601	91.770	91.465	Stage 10A	
Terry Fox Drive	2	50				1116	11.89	3.77	17.04	0.00	0.00	1.50	0.00	3.33	20.37	300	0.85	92.89	1.27	100.0	72.52	0.08	91.690	91.385	90.842	90.537	Stage 10A	
	50	51				1116	11.89	3.77	17.04	0.00	0.00	1.50	0.00	3.33	20.37	300	0.70	84.40	1.16	100.0	64.03	0.03	90.812	90.507	90.112	89.807	Stage 10A	
	51	52				1116	11.89	3.77	17.04	0.00	0.00	1.50	0.00	3.33	20.37	300	0.74	86.83	1.19	61.7	66.47	0.03	90.082	89.777	89.625	89.320	Stage 10A	
Carp Forcemain (58 L/s) Condo Site		62													58.00													Stage 10A
	62	62	215	860	4.80	860	4.80	3.84	13.38	0.00	0.00	1.50	0.00	1.34	14.73													Stage 10A
	62	63				860	4.80	3.84	13.38	0.00	0.00	1.50	0.00	1.34	72.73								89.405	89.100	88.621	88.316	Stage 10A	
Terry Fox Drive Sta 1936-2016		63				860	4.80	3.84	13.38	0.00	0.00	1.50	0.00	1.34	72.73	300	0.70	84.40	1.16	112.0	11.67			88.621	88.316	88.531	88.226	Stage 10A
	63	52				860	4.80	3.84	13.38	0.00	0.00	1.50	0.00	1.34	72.73	300	0.75	87.36	1.20	12.0	14.63							Stage 10A
Terry Fox Drive Sta 2016-2092	52	53				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	0.50	129.20	1.13	80.0	37.79			88.531	88.150	88.132	87.751	Stage 10A
Sta 2016-2092	53	54				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	0.50	129.33	1.13	75.0	37.92	0.03	88.102	87.721	87.727	87.346	Stage 10A	
Sta 2092-2182	54	55				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	1.13	194.42	1.71	70.0	103.01	0.03	87.697	87.316	86.906	86.525	Stage 10A	
Sta 2182-2246	55	56				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	0.50	129.33	1.13	80.0	37.92	0.03	86.876	86.495	86.476	86.095	Stage 10A	
Sta 2246-2286	56	57				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	0.70	153.02	1.34	100.0	61.61	0.03	86.446	86.065	85.746	85.365	Stage 10A	
Sta 2286-2366	57	58				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	0.55	135.64	1.19	100.0	44.23	0.03	85.716	85.335	85.166	84.785	Stage 10A	
Sta 2366-2470	58	59				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	0.53	132.65	1.16	95.0	41.24	0.03	85.136	84.755	84.636	84.255	Stage 10A	
	59	60				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	1.79	244.97	2.15	90.0	153.56	0.03	84.606	84.225	82.991	82.610	Stage 10A	
Sta 2470-2563	60	EX				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	91.41	375	1.64	234.51	2.06	90.5	143.10	0.03	82.961	82.580	81.473	81.092	Stage 10A	
Minto Lands West of Hydro Easement based on 4 pers/unit (Point A)		EX				1976	16.69	3.59	28.74	0.00	0.00	1.50	0.00	4.67	33.41													
Total for Morgan's Grant Subdivision north of Terry Fox Drive (4 pers/unit) to Hines Rd San. sewer (Points A & B)	36					9608	135.79	2.97	115.67	0.00	0.00	1.50	0.00	38.02	153.69													
Total for Morgan's Grant Subdivision north of Terry Fox Drive (3.05 pers/unit) to Hines Rd San. sewer (Points A & B)	36					7326	135.79	3.09	91.63	0.00	0.00	1.50	0.00	38.02	129.65													
Street No. 9	98	97	55	220	2.06	220	2.06	4.00	3.56	0.00	0.00	1.50	0.00	0.58	4.14	250	0.70	51.90	1.02	110.00	47.76			98.800	98.546	98.030	97.776	Phase 12
	97	98	0	0	0.19	220	2.25	4.00	3.56	0.00	0.00	1.50	0.00	0.63	4.19	250	0.80	55.48	1.09	113.00	51.29	0.030	98.000	97.746	97.096	96.842	Phase 12	
Street No. 8	84	81	2	8	0.22	8	0.22	4.00	0.13	0.00	0.00	1.50	0.00	0.06	0.19	250	0.50	43.86	0.87	14.00	43.67			98.270	98.016	98.200	97.946	Phase 12
	81	80	4	16	0.25	24	0.47	4.00	0.39	0.00	0.00	1.50	0.00	0.13	0.52	250	0.50	43.86	0.87	39.20	43.34	0.050	98.150	97.896	97.954	97.700	Phase 12	
	80	80	2	8	0.14	32	0.61	4.00	0.52	0.00	0.00	1.50	0.00	0.17	0.69	250	3.45	115.22	2.27	29.20	114.53	0.271	97.683	97.429		96.422	Phase 12	
Street No. 7	75	71	4	16	0.27	16	0.27	4.00	0.26	0.00	0.00	1.50	0.00	0.08	0.33	250	0.65	50.01	0.99	48.30	49.68			98.410	98.186	98.126	97.872	Phase 12
	71	70	1	4	0.14	20	0.41	4.00	0.32	0.00	0.00	1.50	0.00	0.11	0.44	250	0.65	50.01	0.99	32.70	49.57	0.050	98.076	97.822	97.864	97.610	Phase 12	
	70	70	0	0	0.02	20	0.43	4.00	0.32	0.00	0.00	1.50	0.00	0.12	0.44	250	0.65	50.01	0.99	15.40	49.57	0.030	97.834	97.580	97.733	97.479	Phase 12	
Street No. 9		95	3	12	0.29	284	3.58	4.00	4.60	0.00	0.00	1.50	0.00	1.00	5.60	250	0.45	41.61	0.82	71.00	36.01	0.130	96.546	96.292	96.226	95.972	Phase 12	
Street No. 8	84	83	10	40	0.56	40	0.56	4.00	0.65	0.00	0.00	1.50	0.00	0.16	0.80	250	0.75	53.72	1.06	67.40	52.92			98.270	98.016	97.765	97.511	Phase 12
	83	82	1	4	0.13	44	0.69	4.00	0.71	0.00	0.00	1.50	0.00	0.19	0.91	250	0.75	53.72	1.06	11.50	52.82	0.060	97.705	97.451	97.618	97.364	Phase 12	
	82	95	7	28	0.41	72	1.10	4.00	1.17	0.00	0.00	1.50	0.00	0.31	1.47	250	1.95	86.62	1.71	72.10	85.15	0.080	97.538	97.284		95.878	Phase 12	
Street No. 9	95	94	4	16	0.28	372	4.96	4.00	6.03	0.00	0.00	1.50	0.00	1.39	7.42	250	0.60	48.05	0.95	48.20	40.63	0.070	96.062	95.808	95.773	95.519	Phase 12	
Street No. 7	75	74	1	4	0.10	4	0.10	4.00	0.06	0.00	0.00	1.50	0.00	0.03	0.09	250	0.40	39.23	0.77	74.00	39.14			98.410	98.186	98.144	97.890	Phase 12
	74	73	11	44	0.71	48	0.81	4.00	0.78	0.00	0.00	1.50	0.00	0.23	1.00	250	1.35	72.08	1.42	11.50	71.07	0.050	98.094	97.840	97.939	97.685	Phase 12	
	73	72	2	8	0.18	56	0.99	4.00	0.91	0.00	0.00	1.50																



DESIGN PARAMETERS

$I = 0.280$ l/s/ha	$q \text{ (res)} = 350$ l/cap/day
Singles = 4.0 pers / unit	$q \text{ (com)} = 50,000$ l/ha/day
Townhouses = 4.0 pers / unit	$q \text{ (instl)} = 50,000$ l/ha/day

STREET	M.H. #		NO. of UNITS	RESIDENTIAL				Peaking Factor	POPUL. FLOW l/s	AREA ha	NON-RESIDENTIAL				INFIL. FLOW l/s	PEAK FLOW l/s	SEWER DATA						UPSTREAM		DOWNSTREAM		COMMENTS		
	FROM	TO		POPUL. people	AREA ha	POPUL. people	AREA ha				AREA ha	Peaking Factor	NON-RES. FLOW (l/s)	DIA. mm			Slope %	CAPAC. l/s	VEL. m/s	LENGTH m	RESIDUAL CAP. (l/s)	Obvert Drop	Obvert	Invert	Obvert	Invert			
	93	92	4	16	0.27	492	7.05	3.98	7.93	0.00	0.00	1.50	0.00	1.97	9.90	250	3.60	117.70	2.32	58.40	107.80	0.194	94.649	94.395	92.547	92.293		Phase 12	
	92	91	2	8	0.13	500	7.18	3.97	8.05	0.00	0.00	1.50	0.00	2.01	10.06	250	3.35	113.54	2.24	21.70	103.48	0.500	92.047	91.793	91.320	91.066		Phase 12	
	91	90	8	32	0.54	532	7.72	3.96	8.53	0.00	0.00	1.50	0.00	2.16	10.70	250	0.75	53.72	1.06	70.40	43.03	0.080	91.240	90.986	90.712	90.458		Phase 12	
	90	89	2	8	0.17	540	7.89	3.96	8.66	0.00	0.00	1.50	0.00	2.21	10.86	250	1.05	63.57	1.25	33.60	52.70	0.060	90.652	90.398	90.299	90.045		Phase 12	
Street No. 6	93	62	10	40	0.60	40	0.60	4.00	0.65	0.00	0.00	1.50	0.00	0.17	0.82	250	1.75	82.06	1.62	84.50	61.25		96.100	95.846	94.621	94.367		Phase 12	
	62	61	2	8	0.20	48	0.80	4.00	0.78	0.00	0.00	1.50	0.00	0.22	1.00	250	1.40	73.40	1.45	11.40	72.40	0.040	94.581	94.327	94.422	94.168		Phase 12	
	61	60	9	36	0.56	84	1.36	4.00	1.36	0.00	0.00	1.50	0.00	0.38	1.74	250	1.75	82.06	1.62	70.00	80.32	0.040	94.382	94.128	93.157	92.903		Phase 12	
	60	16	6	24	0.44	108	1.80	4.00	1.75	0.00	0.00	1.50	0.00	0.50	2.25	250	3.15	110.10	2.17	61.10	107.84	0.030	93.127	92.873	91.202	90.948		Phase 12	
Street No. 1	16	15	0	0	0.18	108	1.98	4.00	1.75	0.00	0.00	1.50	0.00	0.55	2.30	250	0.70	51.90	1.02	41.10	49.60	0.500	90.702	90.448	90.414	90.160		Phase 12	
	15	14	0	0	0.09	108	2.07	4.00	1.75	0.00	0.00	1.50	0.00	0.58	2.33	250	0.75	53.72	1.06	39.10	51.39	0.040	90.374	90.120	90.081	89.827		Phase 12	
Street No. 1	13	12	3	12	0.27	660	10.23	3.91	10.45	0.00	0.00	1.50	0.00	2.86	13.32	250	0.61	48.45	0.96	49.10	35.13	0.340	89.741	89.487	89.442	89.188		Phase 12	
	13	12	4	16	0.34	676	10.57	3.90	10.69	0.00	0.00	1.50	0.00	2.96	13.65	250	0.66	50.40	0.99	62.70	36.75	0.030	89.412	89.158	88.998	88.744		Phase 12	
Street No. 5	54A	54	4	16	0.33	16	0.33	4.00	0.26	0.00	0.00	1.50	0.00	0.09	0.35	250	0.45	41.61	0.82	69.90	41.26		92.100	91.846	91.785	91.531		Phase 12	
	54	53	2	8	0.24	24	0.57	4.00	0.39	0.00	0.00	1.50	0.00	0.16	0.55	250	1.50	75.97	1.50	11.50	75.43	0.040	91.745	91.491	91.573	91.319		Phase 12	
	53	52	13	52	0.78	76	1.35	4.00	1.23	0.00	0.00	1.50	0.00	0.38	1.61	250	0.40	39.23	0.77	94.20	37.62	0.160	91.413	91.159	91.036	90.782		Phase 12	
	52	51	15	60	0.92	136	2.27	4.00	2.20	0.00	0.00	1.50	0.00	0.64	2.84	250	0.40	39.23	0.77	95.00	36.39	0.090	90.946	90.692	90.566	90.312		Phase 12	
	51	50	1	4	0.14	140	2.41	4.00	2.27	0.00	0.00	1.50	0.00	0.67	2.94	250	0.40	39.23	0.77	11.50	36.29	0.030	90.536	90.282	90.490	90.236		Phase 12	
	50	12	5	20	0.36	160	2.77	4.00	2.59	0.00	0.00	1.50	0.00	0.78	3.37	250	1.20	67.95	1.34	76.90	64.59	0.040	90.450	90.196	89.527	89.273		Phase 12	
Street No. 1	12	11	0	0	0.17	836	13.51	3.85	13.03	0.00	0.00	1.50	0.00	3.78	16.82	250	0.95	60.46	1.19	79.70	43.65	0.030	89.968	88.714	88.711	87.957		Phase 12	
Street No. 4	42	11	5	20	0.36	20	0.36	4.00	0.32	0.00	0.00	1.50	0.00	0.10	0.42	250	0.99	61.72	1.22	76.70	61.30		88.860	88.626	88.121	87.867		Phase 12	
Street No. 1	11	10	4	16	0.35	872	14.22	3.84	13.56	0.00	0.00	1.50	0.00	3.98	17.54	250	1.26	69.63	1.37	69.40	52.09	0.492	87.629	87.375	86.754	86.500		Phase 12	
	10	9	10	40	0.65	912	14.87	3.83	14.13	0.00	0.00	1.50	0.00	4.16	18.30	250	1.47	75.21	1.48	84.90	56.91	0.572	86.182	85.928	84.611	84.680		Phase 12	
Street No. 4	42	41	1	4	0.14	4	0.14	4.00	0.06	0.00	0.00	1.50	0.00	0.04	0.10	250	1.00	62.03	1.22	11.40	61.93		88.860	88.626	88.766	88.512		Phase 12	
	41	40	11	44	0.71	48	0.85	4.00	0.78	0.00	0.00	1.50	0.00	0.24	1.02	250	1.70	80.88	1.60	72.70	79.87	0.050	88.716	88.462	87.480	87.226		Phase 12	
	40	30	8	32	0.50	80	1.35	4.00	1.30	0.00	0.00	1.50	0.00	0.38	1.67	250	2.90	105.64	2.08	73.30	103.96	0.040	87.440	87.186	85.314	85.060		Phase 12	
Street No. 3	33	32A	1	4	0.33	4	0.33	4.00	0.06	0.00	0.00	1.50	0.00	0.09	0.16	250	1.00	62.03	1.22						0.254	0.000		Phase 12	
	32A	31A	0	0	0.00	4	0.33	4.00	0.06	0.00	0.00	1.50	0.00	0.09	0.16	250	1.00	62.03	1.22						0.254	0.000		Phase 12	
	31A	30	0	0	0.00	4	0.33	4.00	0.06	0.00	0.00	1.50	0.00	0.09	0.16	250	1.00	62.03	1.22						0.254	0.000		Phase 12	
Old Carp Road	-	30	24	96	3.29	96	3.29	4.00	1.56	0.00	0.00	1.50	0.00	0.92	2.48	250	0.40	39.23	0.77	95.00	36.76								Phase 12
Street No. 3	30	29	6	24	0.27	204	5.24	4.00	3.31	0.00	0.00	1.50	0.00	1.47	4.77	250	0.40	39.23	0.77	85.40	34.46	0.070	85.244	84.990	84.903	84.649		Phase 12	
Street No. 1	29	28	12	48	0.50	1164	20.61	3.76	17.71	0.00	0.00	1.50	0.00	5.77	23.48	250	1.24	69.08	1.36	64.30	45.59	0.248	84.655	84.401	83.857	83.603		Phase 12	
Street No. 3	33	32	10	40	0.33	40	0.33	4.00	0.65	0.00	0.00	1.50	0.00	0.09	0.74	250	1.05	63.57	1.25	57.90	62.82		85.100	84.846	84.492	84.238		Phase 12	
	32	31	4	16	0.22	56	0.55	4.00	0.91	0.00	0.00	1.50	0.00	0.15	1.06	250	0.70	51.90	1.02	10.90	50.84	0.030	84.462	84.208	84.386	84.132		Phase 12	
	31	30	17	68	0.56	124	1.11	4.00	2.01	0.00	0.00	1.50	0.00	0.31	2.32	250	0.40	39.23	0.77	86.10	36.91	0.160	84.226	83.972	83.881	83.627		Phase 12	
Street No. 1	28	27	1	4	0.13	1292	21.85	3.73	19.50	0.00	0.00	1.50	0.00	6.12	25.62	250	0.40	39.23	0.77	35.50	13.62	0.416	83.111	83.187	83.299	83.045		Phase 12	
	27	26	2	8	0.27	1300	22.12	3.72	19.61	0.00	0.00	1.50	0.00	6.19	25.80	250	0.45	41.61	0.82	40.10	15.81	0.040	83.259	83.005	83.079	82.825		Phase 12	
Street No. 2	23	22	24	96	3.85	96	3.85	4.00	1.56	2.93	2.93	1.50	2.54	1.08	5.18	250	1.52	76.48	1.51	98.50	71.30		87.083	86.829	85.586	85.332	Future School Flow (2.93ha x 50,000 L/ha/d)		
	22	21	18	72	0.59	168	4.44	4.00	2.72	0.00	2.93	1.50	2.54	1.24	6.51	250	0.85	57.19	1.13	74.90	50.68	0.076	85.510	85.256	84.873	84.619		Phase 12	
	21	20	2	8	0.10	176	4.54	4.00	2.85	0.00	2.93	1.50	2.54	1.27	6.67	250	4.50	131.59	2.60	13.30	124.93	0.030	84.843	84.589	84.245	83.991		Phase 12	
	20	19	2																										



J.L. Richards & Associates Limited
Consulting Engineers, Architects & Planners

CITY OF OTTAWA
MINTO DEVELOPMENT INC.
MORGAN'S GRANT SUBDIVISION - PHASE 10A & 10B
JLR NO. 17730

SANITARY SEWER DESIGN SHEET
Revised September 16, 2003

Designed by: J.B.
Checked by: L.J.

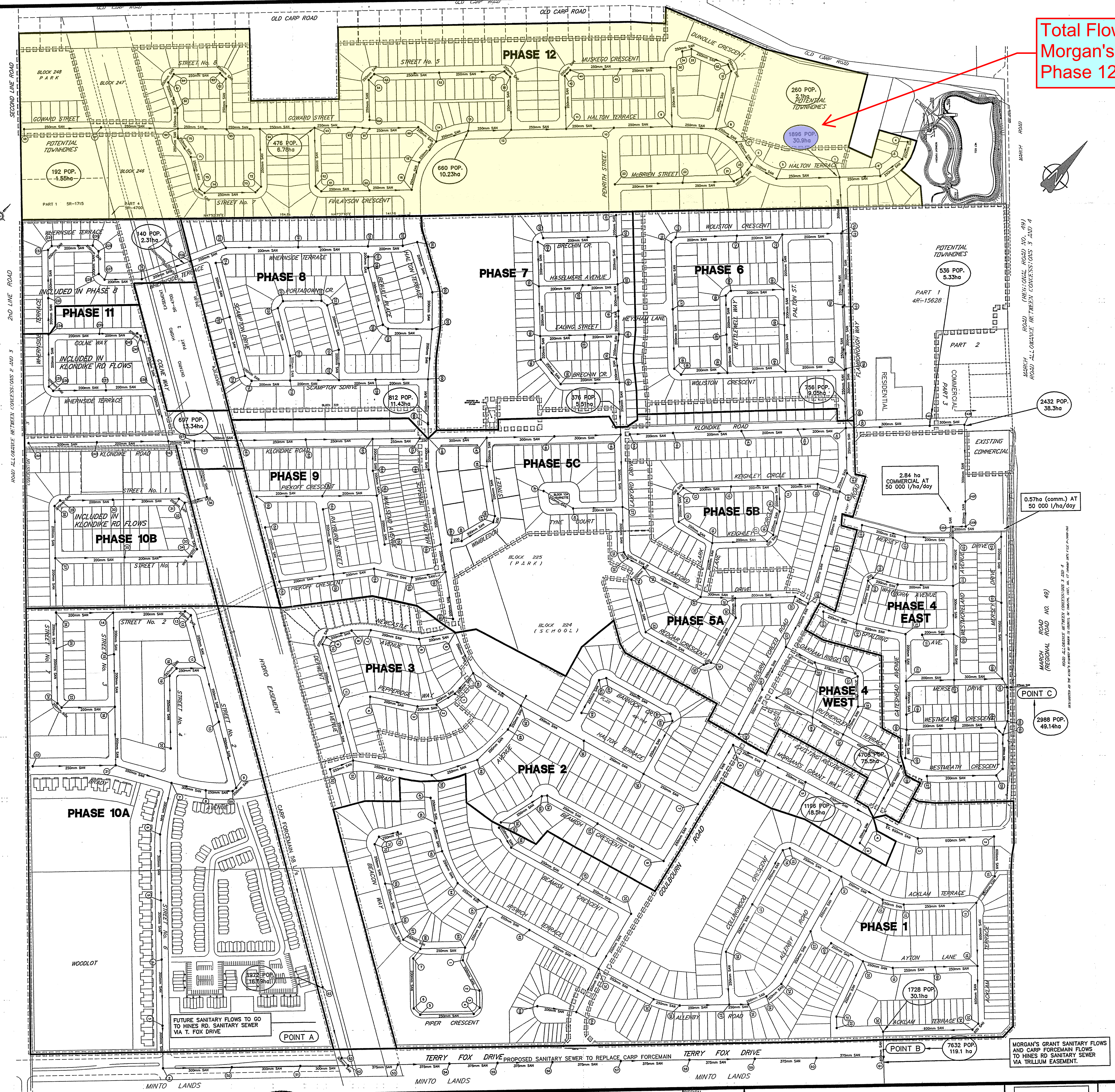
DESIGN PARAMETERS

Table with design parameters: I = 0.280 l/s/ha, q(res) = 350 l/cap/day, Singles = 4.0 pers/unit, q(com) = 50,000 l/ha/day, Townhouses = 4.0 pers/unit, q(inst) = 50,000 l/ha/day

Peak Sewage Flow from Phase 12 = 38.87 L/s

Main data table with columns: STREET, M.H. #, NO. of UNITS, INDIVIDUAL (POPUL. people, AREA ha), CUMMULATIVE (POPUL. people, AREA ha), Peaking Factor, POPUL. FLOW l/s, AREA ha, CUMM. AREA ha, Peaking Factor, NON-RES. FLOW (l/s), INFIL. FLOW l/s, PEAK FLOW l/s, SEWER DATA (DIA. mm, Slope %, CAPAC. l/s, VEL. m/s, LENGTH m, RESIDUAL CAP. (l/s)), UPSTREAM (Obvert Drop, Obvert, Invert), DOWNSTREAM (Obvert, Invert), COMMENTS

Total Flow from Morgan's Grant Phase 12



1	17/09/03	REVISED PHASE 10 AND 12	L.J.
---	----------	-------------------------	------

SCALE
25m 50m 75m 100m
HORIZONTAL 1:3000

J.L. Richards & Associates Limited
Consulting Engineers, Architect & Planners
OTTAWA, KINGSTON, SUDBURY, CANADA.

DESIGN: L.J.
DRAWN: M.Z.
CHECKED:
APPROVED:



MORGAN'S GRANT SUBDIVISION
MASTER DRAINAGE PLAN (SANITARY)

DATED	SEPT. 2003
DWG. No.	17730-SA

3.0 SANITARY SEWAGE

3.1 Existing Sanitary Systems

Sanitary sewage generated from the Morgan's Grant Subdivision is conveyed to the following two sanitary sewer outlets:

- .1 a 375 mm diameter sewer flowing easterly across March Road approximately 200 m north of Morgan's Grant Way, which eventually outlets into the East March Trunk Sewer; and
- .2 a 600 mm diameter sanitary sewer crossing Terry Fox Drive approximately 200 m west of March Road, which outlets to the Hines Road sanitary sewer.

The 375 mm diameter sewer collects sanitary sewage from approximately 65 ha, of which Morgan's Grant accounts for approximately 29 ha. This outlet collects sewage for most of Morgan's Grant Phase 4, the commercial area located north of Morgan's Grant Phase 4, and approximately 32 ha of land located north of Morgan's Grant Phase 6 (i.e. Phase 12).

The 600 mm diameter sewer collects sanitary sewage for approximately 125 ha. This outlet collects sewage from all areas included in Morgan's Grant Phases 1, 2 and 3, the westerly portion of Morgan's Grant Phase 4, all areas included in Morgan's Grant Phases 5, 6, 7, 8 and 9 and some of the lands west of the hydro easement adjacent to Klondike Road (i.e. Phase 10B and Phase 11). This 600 mm diameter sanitary sewer will also collect sewage from the remainder of the Morgan's Grant lands, west of the hydro easement, via a future sanitary sewer down Terry Fox Drive (Phase 10A).

3.2 Sanitary Flows

The design of local sanitary sewers is summarized in the following table (peaking factors for each are shown in parentheses):

Land Use	Sanitary Flow Contribution		
	L/cap/day	L/ha/day	L/s/ha
Residential	350 (Harmon)		
Commercial		50,000 (1.5)	
Institutional		50,000 (1.5)	
Infiltration			0.28 (1.0)

The Harmon peaking factor was calculated for each pipe reach to determine the sanitary peak flows in residential development areas. This peaking factor provides an increased peaking factor for smaller urban areas over larger developments. The following formula is used to derive the Harmon peaking factor:

$$\text{Harmon} = 1 + \frac{14}{(4 + P^{1/2})}$$

A 1.5 peaking factor was utilized for land uses other than residential areas (i.e. institutional, commercial, etc.). Sanitary flows estimated with the above information were calculated on the conservative assumption that sanitary peak flows occurred simultaneously.

For purposes of designing flows in local sanitary sewers within the Morgan's Grant Plan of Subdivision, the standard of four persons per unit was used. This results in flows of 34.74 L/s, 128.27 L/s and 60.77 L/s at Points A, B and C on the enclosed Master Drainage Plan (see also enclosed Sanitary Sewer Design Sheet).

Flows from Point A will be combined with the Village of Carp forcemain flows and conveyed via a replacement sanitary sewer down Terry Fox Drive to Point B, where flows then travel south through the Trillium easement to the upper end of the Hines Road sewer.

It is proposed that the existing forcemain be replaced with a gravity sewer between Points A and B, and sized to convey the Morgan's Grant Stage 10A lands and ultimate Village of Carp flows of 58 L/s. Rock has been removed to install the forcemain along Terry Fox Drive. There is an economic benefit to replacing the existing forcemain with a gravity sewer rather than removing additional rock in a second trench. The City will also benefit by minimizing the number of utilities it will be responsible for maintaining in this section of the right-of-way.

A temporary outlet is proposed for the forcemain flows to an extension of the sanitary sewer on Klondike Road near the hydro easement prior to the Terry Fox Drive pipe replacement.

At Terry Fox Drive, the flows from the Morgan's Grant Subdivision are based on 3.05 persons per unit, for consistency with the City of Ottawa (former Region of Ottawa-Carleton) Wastewater Master Plan, which results in a flow of 130.53 L/s. The City of Ottawa (former Region of Ottawa-Carleton) has advised that, at that point, allowable sanitary flows were as follows:

Morgan's Grant Subdivision	136 L/s	
Village of Carp Forcemain	<u>58 L/s</u>	
	194 L/s	(i.e. 600 mm diameter sanitary at 0.1% has a capacity of 194 L/s)

Sanitary flow from Point C leaving the Subdivision, results in projected flows of 60.77 L/s (see enclosed Sanitary Sewer Design Sheet).

The capacity of this sewer crossing under March Road at point C is 96.02 L/s (i.e. 375 mm sanitary at 0.30%).

3.3 Summary

The proposed sanitary sewer servicing scheme has been developed to accommodate all of the lands within the boundaries of Morgan's Grant Subdivision, as well as the recently acquired Phase 12 lands, and the Sanitary Sewer Design Sheets demonstrate that all sanitary sewer flows are within the allocations provided by the City of Ottawa (i.e. City of Kanata, Region of Ottawa-Carleton).



Ministry
of the
Environment

Ministère
de
l'Environnement

CERTIFICATE OF APPROVAL
MUNICIPAL AND PRIVATE SEWAGE WORKS
NUMBER 4208-6J7J5T
Issue Date: November 17, 2005

Minto Developments Inc.
427 Laurier Avenue West, No. 300
Ottawa, Ontario
K1R 7Y2

Site Location: Morgan's Grant Subdivision Stage 12D
Part of Lots 11 and 12, Concession 3
Ottawa City, Ontario

You have applied in accordance with Section 53 of the Ontario Water Resources Act for approval of:

storm and sanitary sewers to be constructed in the City of Ottawa on Ishpatina Crescent, Goward Drive, and Finlayson Crescent, all in accordance with the application from Minto Developments Inc., dated May 11th, 2005, including final plans and specifications prepared by J.L. Richards & Associates Limited

In accordance with Section 100 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, as amended, you may by written notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 101 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, provides that the Notice requiring the hearing shall state:

1. The portions of the approval or each term or condition in the approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The Certificate of Approval number;
6. The date of the Certificate of Approval;
7. The name of the Director;
8. The municipality within which the works are located;

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
2300 Yonge St., 12th Floor
P.O. Box 2382
Toronto, Ontario
M4P 1E4

AND

The Director
Section 53, *Ontario Water Resources Act*
Ministry of the Environment
2 St. Clair Avenue West, Floor 12A
Toronto, Ontario
M4V 1L5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 314-4600, Fax: (416) 314-4506 or www.ert.gov.on.ca**

The above noted sewage works are approved under Section 53 of the Ontario Water Resources Act.

CONTENT COPY OF ORIGINAL

DATED AT TORONTO this 17th day of November, 2005

Aziz Ahmed, P.Eng.
Director
Section 53, *Ontario Water Resources Act*

EC/
c: District Manager, MOE Ottawa
Robert L. Phillips, C.E.T., Program Manager,
Infrastructure Approvals West, City of Ottawa
Lee Jablonski, P.Eng., J.L. Richards & Associates Limited



Ministry
of the
Environment

Ministère
de
l'Environnement

CERTIFICATE OF APPROVAL
MUNICIPAL AND PRIVATE SEWAGE WORKS
NUMBER 8692-54QSUG

Minto Developments Inc.
427 Laurier Avenue West, Suite 300
Ottawa, Ontario
K1R 7Y2

Site Location: Morgan's Grant
Part of Lot 11, Concession 3
Ottawa City,

You have applied in accordance with Section 53 of the Ontario Water Resources Act for approval of:

Stormwater management facility to be located in the southern quadrant of the intersection Old Carp Road and March Road in the City of Ottawa as follows:

- a 5.7 metre, 1500 millimetre diameter inlet sewer discharging into the first chamber of a splitter box;
- a splitter box divided into two(2) chambers, containing: a weir in the first chamber, directing the runoff to the sediment forebay; a weir in the second chamber with its crest 0.5 m above the crest elevation in the first chamber, directing runoff to the wet cell via an overflow pipe; a spillway with its invert elevation elevation 0.8 m higher than the crest elevation in the first chamber, directing runoff to the wet cell;
- a sediment forebay with an average depth of 1.3 metres, an average width of 24 metres and a length of 82 metres discharging treated runoff to the wet cell via a weir with the crest at the same level as the weir crest in the first chamber;
- a wet cell consisting of a permanent pool volume of 10,250 cubic metres and an active storage of 13,000 cubic metres and an outlet structure containing a weir with crest elevation 3 metres lower than the weir crest in the first chamber of the splitter box; discharging treated runoff to an existing municipal drain via an approximately 150 metres, 1650 millimetre diameter outlet storm sewer running along March Road;

all in accordance with the application from Minto Developments Inc. dated August 14 2001, including design brief, final plans, specifications and other supporting documents prepared by Cumming Cockburn Limited

For the purpose of this Certificate of Approval and the terms and conditions specified below, the following definitions apply:

"certificate" means this entire certificate of approval document, issued in accordance with Section 53 of the *Ontario Water Resources Act*, and includes any schedules;

"Director" means any Ministry employee appointed by the Minister pursuant to section 5 of the *Ontario Water Resources Act*;

"District Manager" means the District Manager of the Ottawa District Office of the Ministry;

"Environmental Appeal Board" means the Environmental Review Tribunal established pursuant to the Environmental Review Tribunal Act;

"Ministry" means the Ontario Ministry of the Environment;

"Owner" means Minto Developments Inc. and includes its successors and assignees;

"works" means the sewage works described in the Owner's application, this certificate and in the supporting documentation referred to herein, to the extent approved by this certificate;

"grab sample" means an individual representative sample of sewage collected in accordance with Section 3.1.1 of the Ministry's publication entitled "Protocol for the Sampling and Analysis of Industrial/Municipal Waste Water", dated January 1999, and as amended from time to time;

You are hereby notified that this approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL CONDITION

(a) Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the works in accordance with the description given in this Certificate, the application for approval of the works and the submitted supporting documents and plans and specifications as listed in this Certificate.

(b) Where there is a conflict between a provision of any submitted document referred to in this Certificate and the Conditions of this Certificate, the Conditions in this Certificate shall take precedence, and where there is a conflict between the listed submitted documents, the document bearing the most recent date shall prevail.

2. EFFLUENT MONITORING AND RECORDING

The Owner shall, establish and carry out, upon commencement of operation of the sewage works, the following monitoring program:

(a) In a given calendar year, at least five rainfall events shall be selected during the period from the beginning of May to the end of September of that year and for each event, composite samples shall be constituted from three (3) grab samples of the storm run off at the inlet to the pond before it discharges to the sediment forebay and four (4) grab samples of the effluent leaving the pond at the outlet structure at approximately 1,2,4,6 and 8 hours from the start of each rainfall event and, the composite samples shall be analyzed for the Total Suspended Solids

(b) The sampling and analyses required in subsection (1) shall be performed in accordance with the Ministry's publication "Protocol for the Sampling and Analysis of Industrial/Municipal Wastewater" January 1999 and as amended; or as described in the American Public Health Association's publication " Standard Methods for Examination of Water and Wastewater", 20th Edition, 1998 and as amended;

(c) Pursuant to subsections (1) and (2) the owner shall prepare and submit in writing a monitoring report to the District Manager by the 31st day of October immediately following the monitoring period and which shall include, as a minimum, results of the water quality monitoring program, the stage curve for the outlet weir - validated in the course of the monitoring period - the hyetographs and outlet hydrographs for the storms associated with the said water quality analyses and, an assessment of the facility's performance;

(d) The monitoring program described in subsections (1), (2) and (3) shall begin when approximately 80 % of the land mass tributary to the sewage works have been developed. After its inception, the said monitoring program shall last for a period of no less than three (3) consecutive years.

3. The Owner shall make all necessary investigations, take all necessary steps and obtain all necessary approvals so as to ensure that the physical structure, siting and operations of the stormwater works do not constitute a safety or health hazard to the general public.

4. The Owner shall ensure that sediment and excessive decaying vegetation are removed from the above noted stormwater management system at such a frequency as to prevent the excessive buildup and potential overflow of sediment and/or decaying vegetation into the receiving watercourse.

The reasons for the imposition of these terms and conditions are as follows:

1. Condition 1 is imposed to ensure that the works are built and operated in the manner in which they were described for review and upon which approval was granted. This condition is also included to emphasize the precedence of Conditions in the Certificate and the practice that the Certificate is based on the most current document, if several conflicting documents are submitted for review.

CONTENT COPY OF ORIGINAL

2. Condition No. 2 is included to ensure that the information relating to the operation of the sewage works is made available to Shirley's Brook.

3. Condition 3 is imposed because it is not in the public interest for the Director to approve facilities which, by reason of potential health and safety hazards do not generally comply with legal standards or approval requirements falling outside the purview of this Ministry.

4. Condition 4 is included as regular removal of sediment and excessive decaying vegetation from this approved stormwater management system are required to mitigate the impact of sediment and/or decaying vegetation on the downstream receiving watercourse. It is also required to ensure that adequate storage is maintained in the stormwater management facilities at all times as required by the design.

In accordance with Section 100 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, as amended, you may by written notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 101 of the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, provides that the Notice requiring the hearing shall state:

1. The portions of the approval or each term or condition in the approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The Certificate of Approval number;
6. The date of the Certificate of Approval;
7. The name of the Director;
8. The municipality within which the works are located;

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
2300 Yonge St., 12th Floor
P.O. Box 2382
Toronto, Ontario
M4P 1E4

AND

The Director
Section 53, *Ontario Water Resources Act*
Ministry of the Environment
2 St. Clair Avenue West, Floor 12A
Toronto, Ontario
M4V 1L5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 314-4600, Fax: (416) 314-4506 or www.ert.gov.on.ca**

The above noted sewage works are approved under Section 53 of the Ontario Water Resources Act.

DATED AT TORONTO this 21st day of December, 2001

Mohamed Dhalla, P.Eng.
Director
Section 53, *Ontario Water Resources Act*

SK/
c: District Manager, MOE Ottawa
Peter Spal, Cuming Cockburn Limited

LEGEND

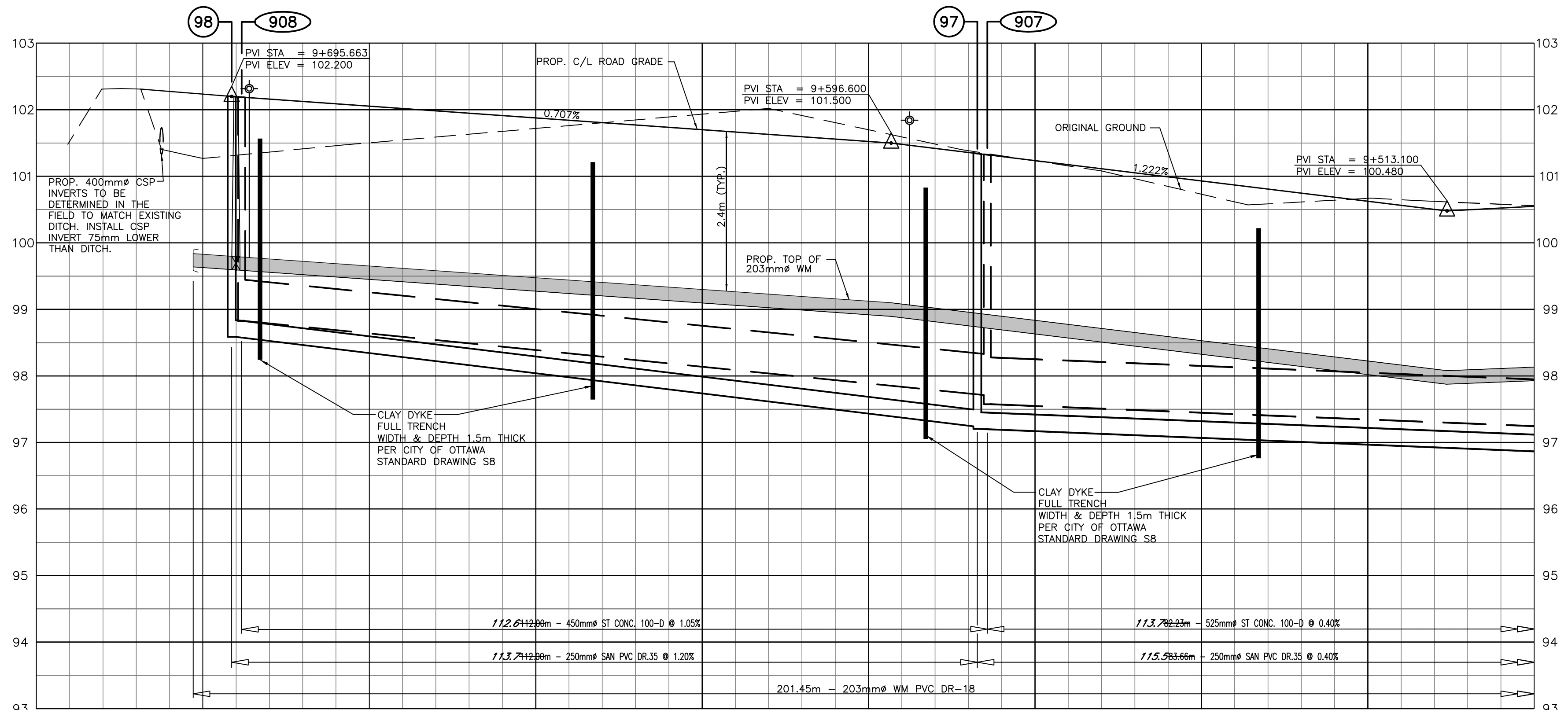
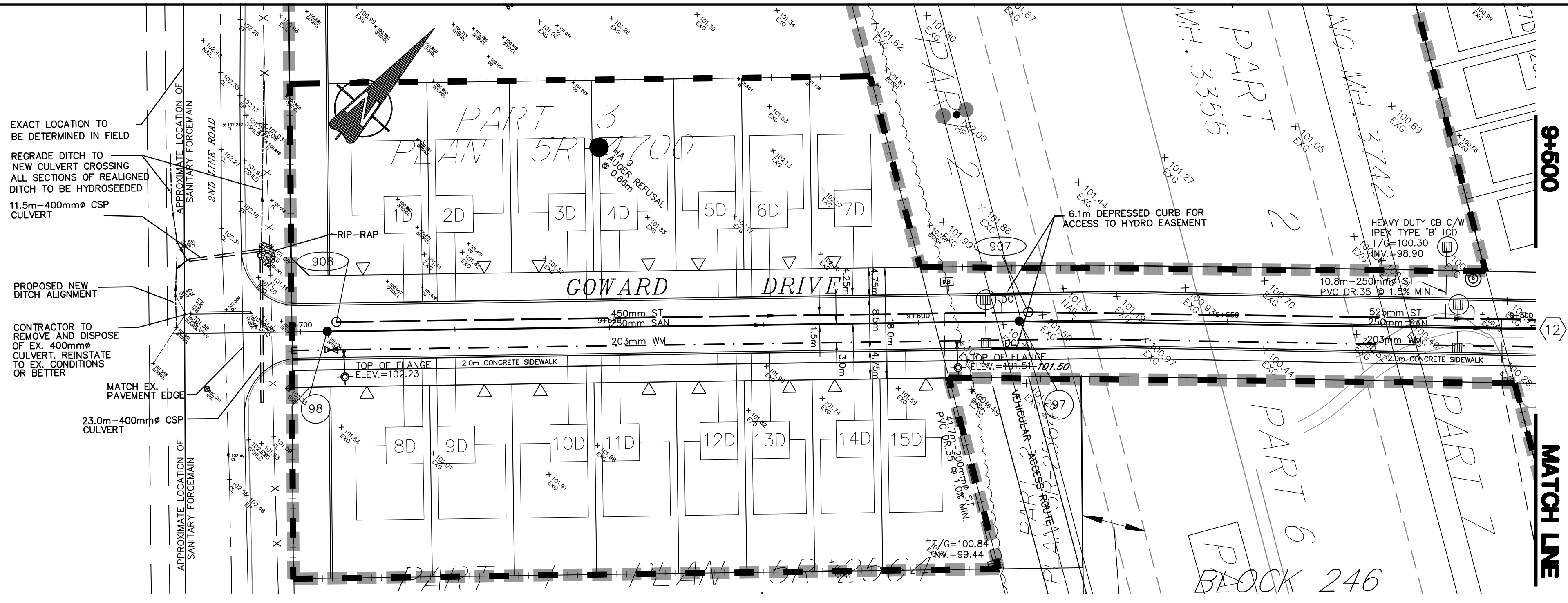
- EXISTING CATCH BASIN
- PROPOSED CATCH BASIN
- INTERCONNECTED ROADWAY CB C/W ONE 19.8 L/S IPEX TYPE 'A' ICD OR CITY APPROVED EQUIVALENT
- CATCH BASIN WITH INDIVIDUAL 19.8 L/S IPEX TYPE 'A' ICD OR CITY APPROVED EQUIVALENT
- PROPOSED CATCH BASIN / MANHOLE
- PROPOSED CATCH BASIN FOR LANDSCAPED AREAS
- PROPOSED WATERMAIN, VALVE & HYDRANT
- EXISTING WATERMAIN, VALVE & HYDRANT
- WATERMAIN VALVE AND VALVE CHAMBER
- WATERMAIN VALVE AND VALVE BOX
- EXISTING SANITARY SEWER & MANHOLE
- EXISTING STORM SEWER & MANHOLE
- PROPOSED SANITARY SEWER & MANHOLE
- PROPOSED STORM SEWER & MANHOLE
- PROPOSED CATCH BASIN & LEAD
- 30C LOT NUMBER
- 2.0m CONC. CURB AND SIDEWALK
- DEPRESSED CURB
- SILT FENCE (OPSD 249.110)
- PHASING LIMIT
- DRAWING NUMBER
- TEST PIT SHOWING ELEVATION OF ROCK
- HAND AUGER HOLE LOCATION SHOWING DEPTH FROM EXISTING GROUND SURFACE TO REFUSAL (INFERRED DEPTH TO BEDROCK)

ALL MANHOLES TO BE 1200mm ϕ UNLESS SHOWN OTHERWISE

RECORD DRAWING
 PROVIDED BY FIELD INSPECTOR
 date: MAY 03, 2013
J.L. RICHARDS & ASSOCIATES LIMITED

NOTES:

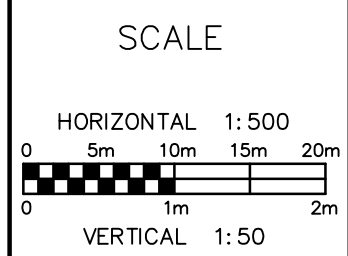
1. INSTALL CLAY DYKES AT 50m INTERVALS BEGINNING AT THE LOW END OF THE SEWERS AND PROCEEDING UPSTREAM FOR EACH OF THE THREE STREETS SERVICED. EACH CLAY DYKE SHALL BE CONSTRUCTED OF CLAY GEOTECHNICAL MATERIAL AND PLACED THE FULL HEIGHT AND WIDTH OF THE TRENCH AND BE 1.5m THICK.



DESIGN PROFILE ELEVATIONS	W.M. TOP ELEVATIONS	STORM SEWER INV. ELEVATION	SANITARY SEWER INV. ELEVATION	C.L. ROADWAY STATION	DESIGN PROFILE ELEVATIONS	W.M. TOP ELEVATIONS	STORM SEWER INV. ELEVATION	SANITARY SEWER INV. ELEVATION	C.L. ROADWAY STATION
				9+725					9+500
				9+701.450					
				9+695.663					
				9+695.030					
				9+684.165					
				9+683.030					
				9+675					
				9+650					
				9+625					
				9+600					
				9+583.840					
				9+582.160					
				9+575					
				9+550					
				9+525					
				9+500					

W:\77232\DWG\180773 6 12 13.dwg

	6	02/05/13	RECORD DRAWING	L.J.
	5	24/11/05	ISSUED FOR CONSTRUCTION	L.J.
	4	18/10/05	REVISED PER HYDRO ONE COMMENTS	L.J.
	3	11/07/05	REVISED PER CITY COMMENTS	L.J.
	2	11/05/05	ISSUED FOR MOE APPROVAL	L.J.
	1	15/04/05	ISSUED TO BLASTING CONTRACTOR	L.J.



J.L. Richards & Associates Limited
 864 Lady Ellen Place
 Ottawa, ON Canada
 K1Z 5M2
 Tel: 613 728 3571
 Fax: 613 728 6012

DESIGN	J.B.
CHECKED	L.J.
DRAWN	M.B.
CHECKED	
APPROVED	



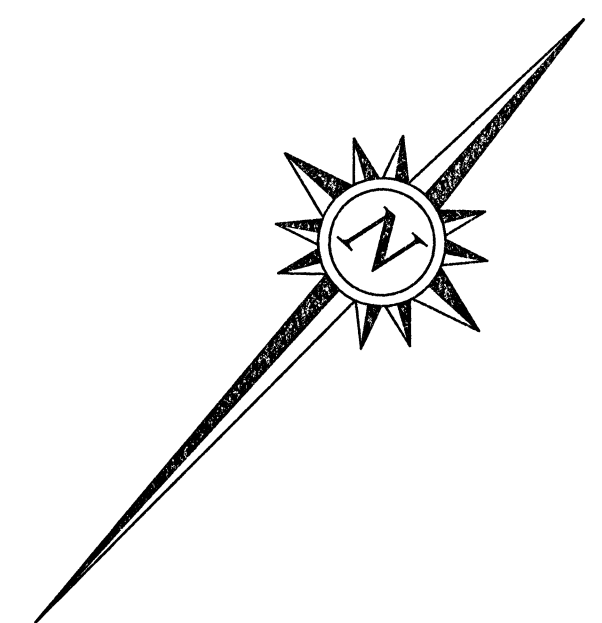
MORGAN'S GRANT
 PHASE 12D
 GOWARD DRIVE
 PLAN & PROFILE
 FROM STA. 9+500.00 TO STA. 9+695.66

DATED: APRIL 2005
 DWG. No. 17732-13

Appendix J – Drawings

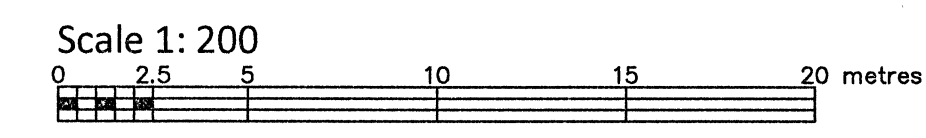
Site Plan Drawing

Survey Plan



PART OF LOT 11
CONCESSION 3
GEOGRAPHIC TOWNSHIP OF MARCH
CITY OF OTTAWA

FARLEY, SMITH & DENIS SURVEYING LTD., 2018



Metric Note
Distances and coordinates on this plan are in metres and can be converted to feet by dividing by 0.3048.

Distance Note
Distances shown on this plan are ground distances and can be converted to grid distances by multiplying by the combined scale factor of 0.99991.

Bearing Note
Bearings are grid and are referred to the easterly limit of Second Line Road having a bearing of N 42° 13' 50" W as shown on Registered Plan 4M-1309 and are referred to the Central Meridian of MTM Zone 9 (76°30' West Longitude) NAD-83 (Original).

Elevation Notes
1. Elevations shown are geoidic and are referred to Geoidic Datum CGVD-1978.
2. It is the responsibility of the user of this information to verify that the job benchmark has not been altered or disturbed and that its relative elevation and description agrees with the information shown on this drawing.

Utility Notes
1. The drawing cannot be accepted as acknowledging all of the utilities and it will be the responsibility of the user to contact the respective utility authorities for confirmation.
2. Only visible surface utilities were located.
3. Underground utility data derived from City of Ottawa utility sheet reference A-8-25.
4. Sanitary and storm sewer grades were derived from field measurement.
5. A field location of underground plans by the permittee utility authority is mandatory before any work involving breaking ground, probing, excavating, etc.

Note
Trees located within 5m corridor of subject boundary only.

Notes & Legend

—	Denotes	Survey Monument Planted
—	Denotes	Survey Monument Found
SIB	Standard Iron Bar	
SIB	Short Standard Iron Bar	
IB	Iron Bar	
(W)	Witness	
M	Measured	
(P1)	Plan 4M-2642	
(P2)	Registered Plan 4M-1309	
(P3)	Registered Plan 4M-1345	
(P4)	Plan 4M-0110	
(P5)	Plan SR-1715	
—	Denotes	Maintenance Hole (Storm)
—	Denotes	Maintenance Hole (Sanitary)
—	Denotes	Underground Storm Sewer
—	Denotes	Underground Sanitary Sewer
—	Denotes	Underground Water
—	Denotes	Underground (Electrical)
—	Denotes	Overhead Wires
—	Denotes	Utility Pole
—	Denotes	Anchor
—	Denotes	Ditch Inlet
—	Denotes	Corrugated Steel Pipe
—	Denotes	Inv.
—	Denotes	Fire Hydrant
—	Denotes	Top of Pipe
—	Denotes	Top of Gate
—	Denotes	Top of Wall
—	Denotes	Borehole
—	Denotes	Diameter
—	Denotes	Chain Link Fence
—	Denotes	Post and Wire Fence
—	Denotes	Board Fence
—	Denotes	Sound Barrier Fence
—	Denotes	Irregular Stone Retaining Wall
—	Denotes	Wood Retaining Wall
—	Denotes	Rail Fence
—	Denotes	Centreline
—	Denotes	North Face
—	Denotes	Cedar Hedge
—	Denotes	Top of Wall
—	Denotes	U/Side
—	Denotes	Underside of Eave
—	Denotes	Edge of Gravel
—	Denotes	Top of Foundation
—	Denotes	Top of Foundation
—	Denotes	Elev.
—	Denotes	Location of Elevations
—	Denotes	Top of Concrete Curb/Elevation
—	Denotes	Property Line
—	Denotes	Deciduous Tree
—	Denotes	Coniferous Tree

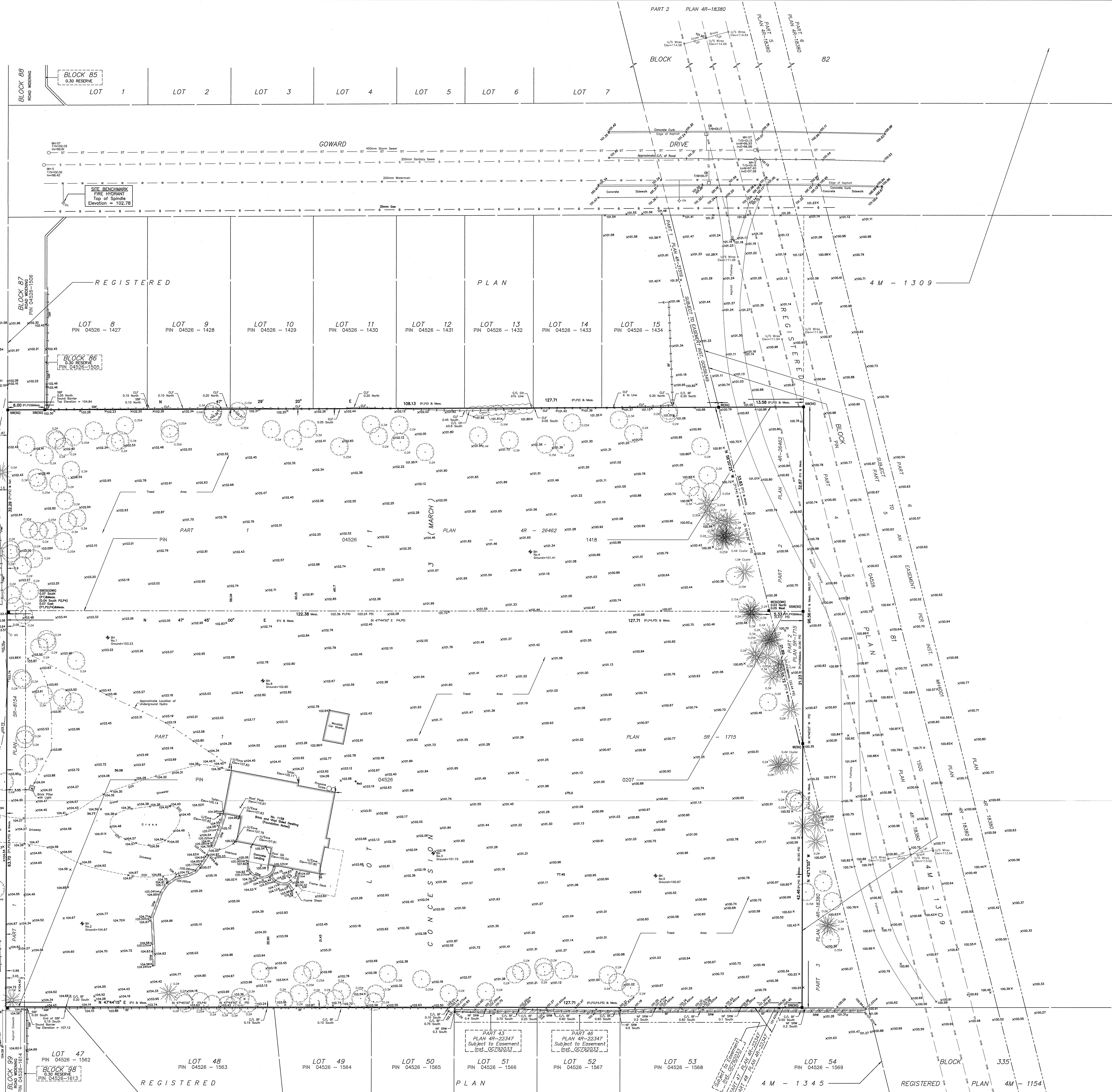
Surveyor's Certificate
I certify that:
1. This survey and plan are correct and in accordance with the Survey Act, the Surveyors Act and the Regulations made under them.
2. The survey was completed on the 28th day of February, 2018.

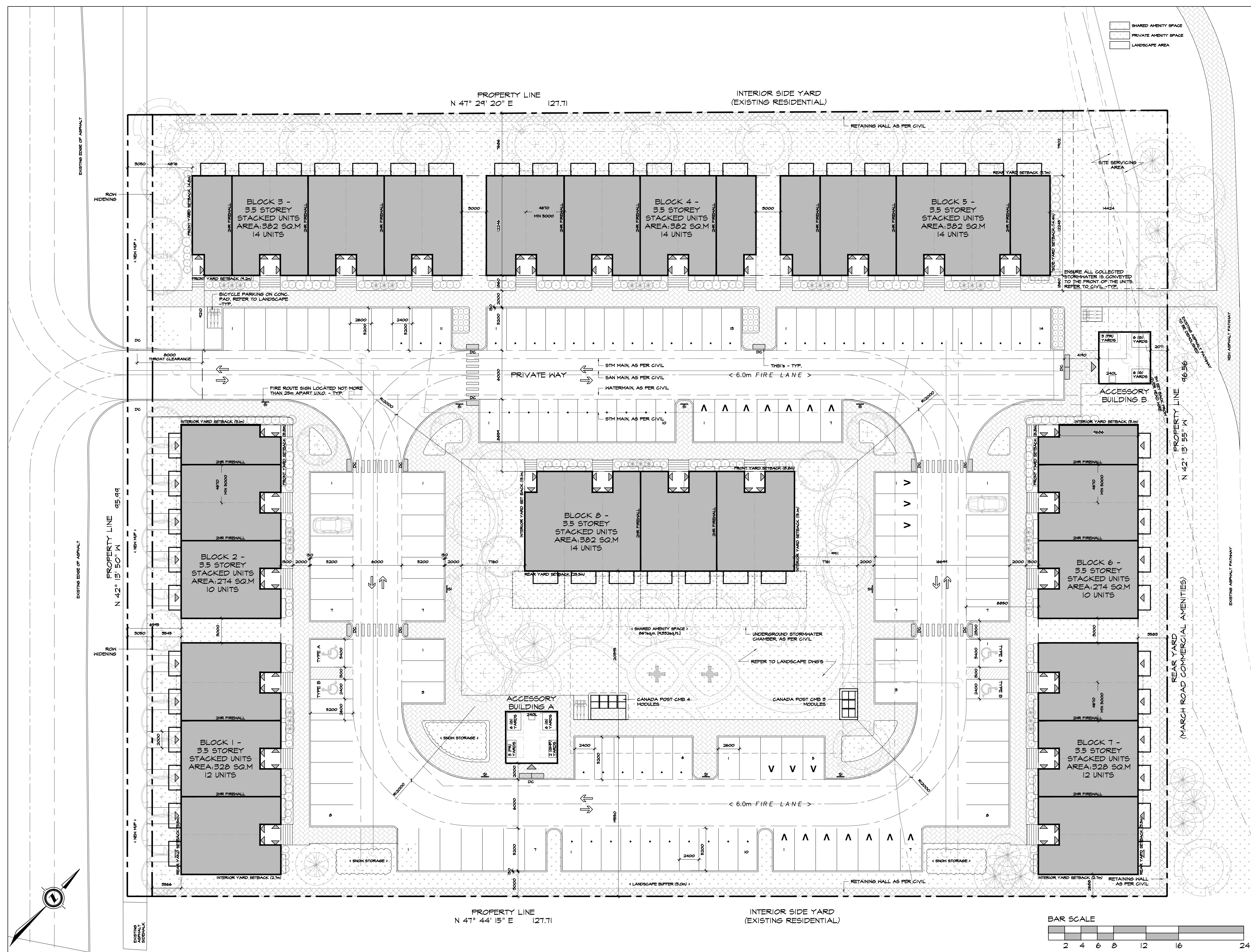
[Signature]
Farley, Smith & Denis
Ontario Land Surveyor

TOPOGRAPHIC DATA WAS COLLECTED UNDER WINTER CONDITIONS, SNOW COVER AND ICE. PRECISE DETERMINING LOCATION AND ELEVATION OF SOME TOPOGRAPHICAL DATA THAT IS OTHERWISE VISIBLE.

FARLEY, SMITH & DENIS SURVEYING LTD.
ONTARIO LAND SURVEYORS
CANADA LAND SURVEYORS

190 COLONADE ROAD, OTTAWA, ONTARIO K2E 7Y5
TEL: (613) 727-8226 FAX: (613) 727-8226





01 PROPOSED SITE PLAN
A1.0 SCALE: 1:250

- LEGEND**
- NEW OVERHEAD DOOR
 - NEW DOOR / ENTRANCE
 - BICYCLE PARKING SPACE (1.8Mx0.6M)
 - NO PARKING LINES
 - PARKING STALL COUNT PER ROW
 - NEW SIGN, REFER TO SIGN LEGEND
 - FIRE ROUTE SIGN
 - STREET LIGHT
 - DESIGNATED ACCESSIBLE PARKING SPACE AS PER AODA STANDARDS
 - VISITOR PARKING
 - TWO WAY TRAFFIC
 - DEPRESSED CURB (DC)
 - TACTILE WALKING SURFACE INDICATORS (TWSIs)
 - PROPERTY LINE
 - PROPOSED FENCE
 - MINIMUM SETBACKS (ZONING)
 - NEW CONSTRUCTION
 - EXISTING BUILDINGS
 - BUILDING MOUNTED LIGHTS REFER TO ELECTRICAL DWGS

BUILDING AREAS (NOT INCLUDING BASEMENT)		
	BUILDING FOOT PRINT (SQ.M)	GROSS AREA (SQ.M)
BLOCK 1	328m ²	984m ²
BLOCK 2	274m ²	822m ²
BLOCK 3	382m ²	1146m ²
BLOCK 4	382m ²	1146m ²
BLOCK 5	382m ²	1146m ²
BLOCK 6	274m ²	822m ²
BLOCK 7	328m ²	984m ²
BLOCK 8	382m ²	1146m ²
TOTAL	2350m ²	7050m ²

CITY OF OTTAWA ZONING BY-LAW

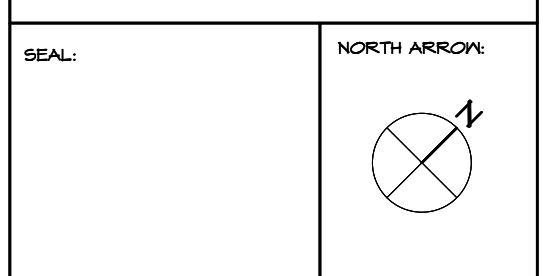
ZONING MECHANISM (R32 - PUD)	PROVISION	PROVIDED
MINIMUM LOT WIDTH	18.0m	96.0m
MINIMUM LOT AREA	1,400m ²	12,294.4m ²
MAXIMUM BUILDING HEIGHT	AS PER DWELLING TYPE, DWELLING TYPE NOT PERMITTED	11.31m
MINIMUM FRONT YARD SETBACK	3.0m	3.5m
MINIMUM REAR YARD SETBACK	6.0m	3.5m
MINIMUM INTERIOR YARD SETBACK	6.0m	2.6m
MINIMUM REQUIRED VEHICLE PARKING SPACES (AREA C OF SCHEDULE 1A)	RESIDENTIAL: 1.2 PER DWELLING UNIT VISITOR: 0.2 PER DWELLING UNIT TOTAL	120 20 140
PROVIDED PARKING	REGULAR SPACES ACCESSIBLE SPACE (TYPE A) ACCESSIBLE SPACE (TYPE B) COMPACT SPACES TOTAL	80 (20 VISITORS) 2 2 56 140
BICYCLE PARKING REQUIRED	5% OF 140 SPACES = 7 SPACES	9 BICYCLE PARKING SPACES
MINIMUM DRIVEWAY WIDTH	PARKING LOT: 6.0m	6.0m
MINIMUM AISLE WIDTH	PARKING LOT: 6.0m	6.0m
MINIMUM PARKING SPACE DIMENSIONS	LENGTH: 5.2m WIDTH: 2.6m	LENGTH: 5.2m WIDTH: 2.6m
MINIMUM WIDTH OF LANDSCAPED AREA AROUND A PARKING LOT	UP TO 40% OF REQUIRED PARKING SPACES MAY BE 4.6m x 2.4m	40% = 56 SPACES
MINIMUM WIDTH OF LANDSCAPED AREA WITHIN A PARKING LOT	ABUTTING A STREET: 3.0m NOT ABUTTING A STREET: 1.5m	3.0m 2.6m
MINIMUM REQUIRED LANDSCAPED AREA WITHIN A PARKING LOT	15%	>15% = 1535.7m ²
PROVIDED SHARED AMENITY SPACE	50% OF AMENITY AREA (100 UNITS x 6m ²) = 600m ² x 50% = 300m ²	867m ²
PROVIDED PRIVATE AMENITY SPACE	N/A	1122m ²
PROVIDED TOTAL LANDSCAPE AREA		3916m ²
OUTDOOR REFUSE COLLECTION	MIN. SETBACK FROM A PUBLIC STREET: 9.0m MIN. SETBACK FROM ANY LOT LINE: 3.0m SCREENING MIN. HEIGHT: 2.0m	>9.0m >3.0m 2.0m
MINIMUM WIDTH OF PRIVATE WAY	6.0m	6.0m
MINIMUM SETBACK FOR ANY WALL OF A RESIDENTIAL USE BUILDING TO A PRIVATE WAY	NOTWITHSTANDING ANY FRONT YARD SETBACK REQUIREMENT ASSOCIATED WITH ANY ZONE OR SUBZONE, THE MINIMUM SETBACK FOR ANY WALL OF A RESIDENTIAL USE BUILDING TO A PRIVATE WAY IS 1.6m	3.5m
MINIMUM SEPARATION AREA BETWEEN BUILDINGS WITHIN A PLANNED UNIT DEVELOPMENT	3m	3m
GARBAGE	0.231 CUBIC YARDS PER UNIT: 100 x 0.231 = 23.1	24 YARDS
RECYCLING (MORE THAN 18 UNITS)	0.016 CUBIC YARDS PER UNIT FOR FEL GLASS/METAL/PLASTIC: 100 x 0.016 = 1.6	2 YARDS
	0.062 CUBIC YARDS PER UNIT FOR FEL FIBRE CONTAINERS: 100 x 0.062 = 6.2	6 YARDS
ORGANICS	"KITCHEN CATCHER" CONTAINER FOR EACH DWELLING UNIT AND ONE COMMUNAL 240L GREEN CONTAINER PER 50 UNITS	2: 240L CONTAINERS

CLIENT NAME: THE BERGE HOMES

NOTES:
1) ALL WORK TO BE IN COMPLIANCE WITH LOCAL BUILDING CODES, REGULATIONS AND BY-LAWS.
2) ADDITIONAL DRAWINGS MAY BE ISSUED FOR CLARIFICATION TO ASSIST PROPER EXECUTION OF WORK. SUCH DRAWINGS WILL HAVE THE SAME MEANINGS AND INTENT AS IF THEY WERE INCLUDED WITH PLANS IN CONTRACT DOCUMENTS.
3) DO NOT SCALE DRAWINGS.
4) ALL SUB-CONTRACTORS TO TAKE THEIR OWN ON-SITE MEASUREMENTS AND BE RESPONSIBLE FOR THEIR ACCURACY.
5) NOTIFY SHAWN J. LAWRENCE ARCHITECT FOR ANY ERRORS AND/OR OMISSIONS PRIOR TO START OF WORK.

UNIT COUNT

STACKED TOWNS: 100 UNITS



NO.	DATE	REVISION
14	2023.11.04	ISSUED FOR COORDINATION
13	2023.11.01	ISSUED FOR COORDINATION
12	2023.04.12	ISSUED FOR REVIEW
11	2023.08.21	ISSUED FOR REVIEW
10	2023.07.13	ISSUED FOR COORDINATION
04	2023.06.13	ISSUED FOR SPG
08	2023.03.20	RE-ISSUED FOR PRE-CONSULT
07	2023.03.14	ISSUED FOR REVIEW
06	2023.02.22	RE-ISSUED FOR PRE-CONSULT
05	2023.02.14	ISSUED FOR REVIEW
04	2023.01.14	ISSUED FOR REVIEW
03	2022.11.22	ISSUED FOR PRE-CONSULT
02	2022.11.18	ISSUED FOR REVIEW
01	2022.11.04	ISSUED FOR REVIEW



KEY PLAN

TOPOGRAPHIC PLAN OF SURVEY OF PART OF LOT 11, CONCESSION 3, GEOGRAPHIC TOWNSHIP OF MARCH, CITY OF OTTAWA

SURVEYED BY FARLEY, SMITH & DENIS SURVEYING LTD.

S.J. LAWRENCE ARCHITECT INCORPORATED
19 DEAKIN STREET
SUITE 209
OTTAWA, ONTARIO
K2E 6B7
T: (613) 784-1110
F: (613) 784-1103
s.j.lawrence@sjla.com

THIS DRAWING IS THE SOLE PROPERTY OF S.J. LAWRENCE ARCHITECT INCORPORATED. REPRODUCTION IS NOT PERMITTED.

PROJECT:
OLD SECOND LINE DEVELOPMENT
1150 OLD SECOND LINE ROAD, OTTAWA, ON

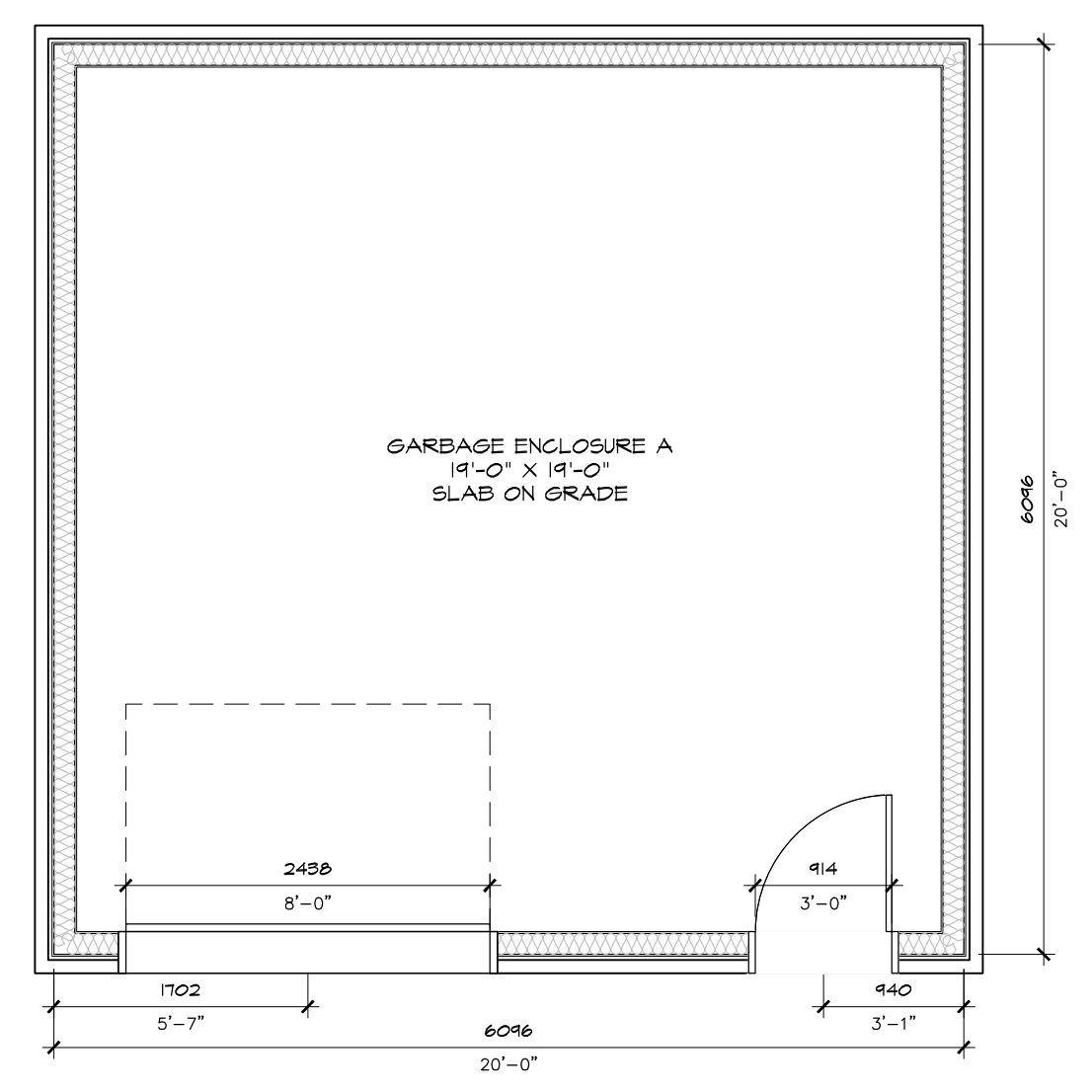
SHEET TITLE:
SITE PLAN

DRAWN BY: [Name] CHECKED BY: [Name]
DATE: 2023.11.04 DATE: [Date]

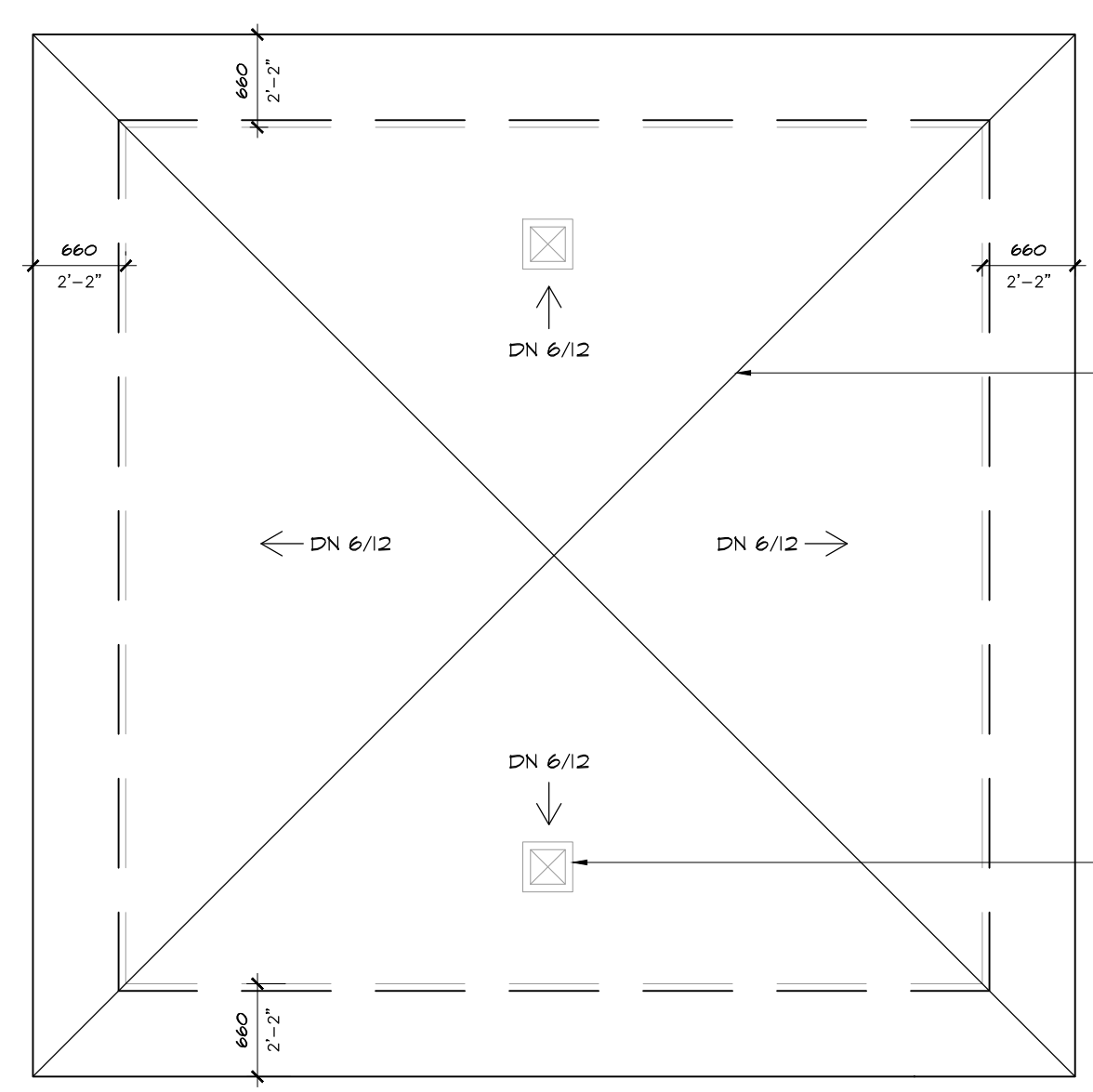
JOB NUMBER: SL-1086-22 SCALE: 1:250
SHEET NUMBER: [Number]

NOTES:
 1) ALL WORK TO BE IN COMPLIANCE WITH LOCAL BUILDING CODES, REGULATIONS AND BY-LAWS.
 2) ADDITIONAL DRAWINGS MAY BE ISSUED FOR CLARIFICATION TO ASSIST PROPER EXECUTION OF WORK. SUCH DRAWINGS WILL HAVE THE SAME MEANINGS AND INTENT AS IF THEY WERE INCLUDED WITH PLANS IN CONTRACT DOCUMENTS.
 3) DO NOT SCALE DRAWINGS.
 4) ALL SUB-CONTRACTORS TO TAKE THEIR OWN ON-SITE MEASUREMENTS AND BE RESPONSIBLE FOR THEIR ACCURACY.
 5) NOTIFY SHAWN J. LAWRENCE ARCHITECT FOR ANY ERRORS AND/OR OMISSIONS PRIOR TO START OF WORK.

KEY NOTES:
 1. WALL SCENCE
 2. SIDING
 3. PRECAST CONCRETE SILL
 4. STONE VENEER



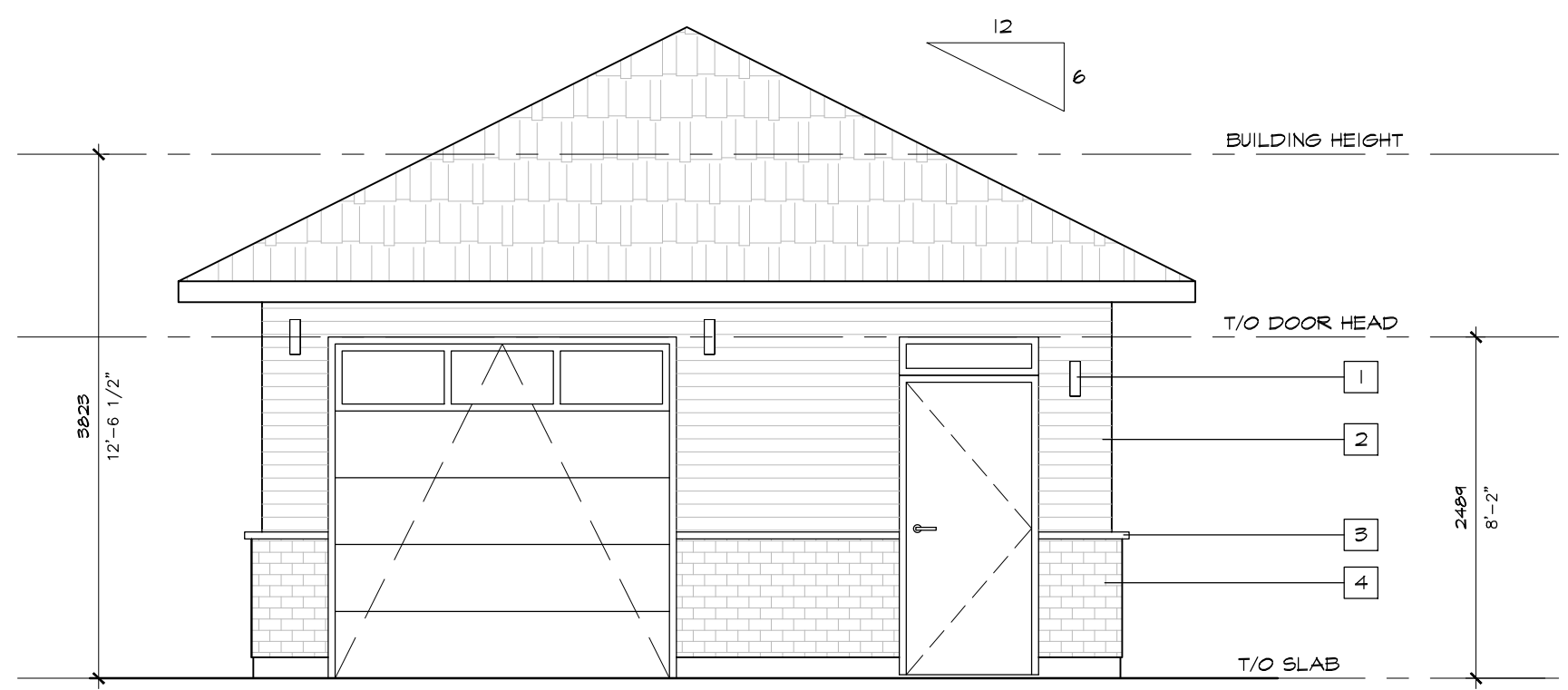
01 ACCESSORY BUILDING A FLOOR PLAN
 A1.1 SCALE: 1/50



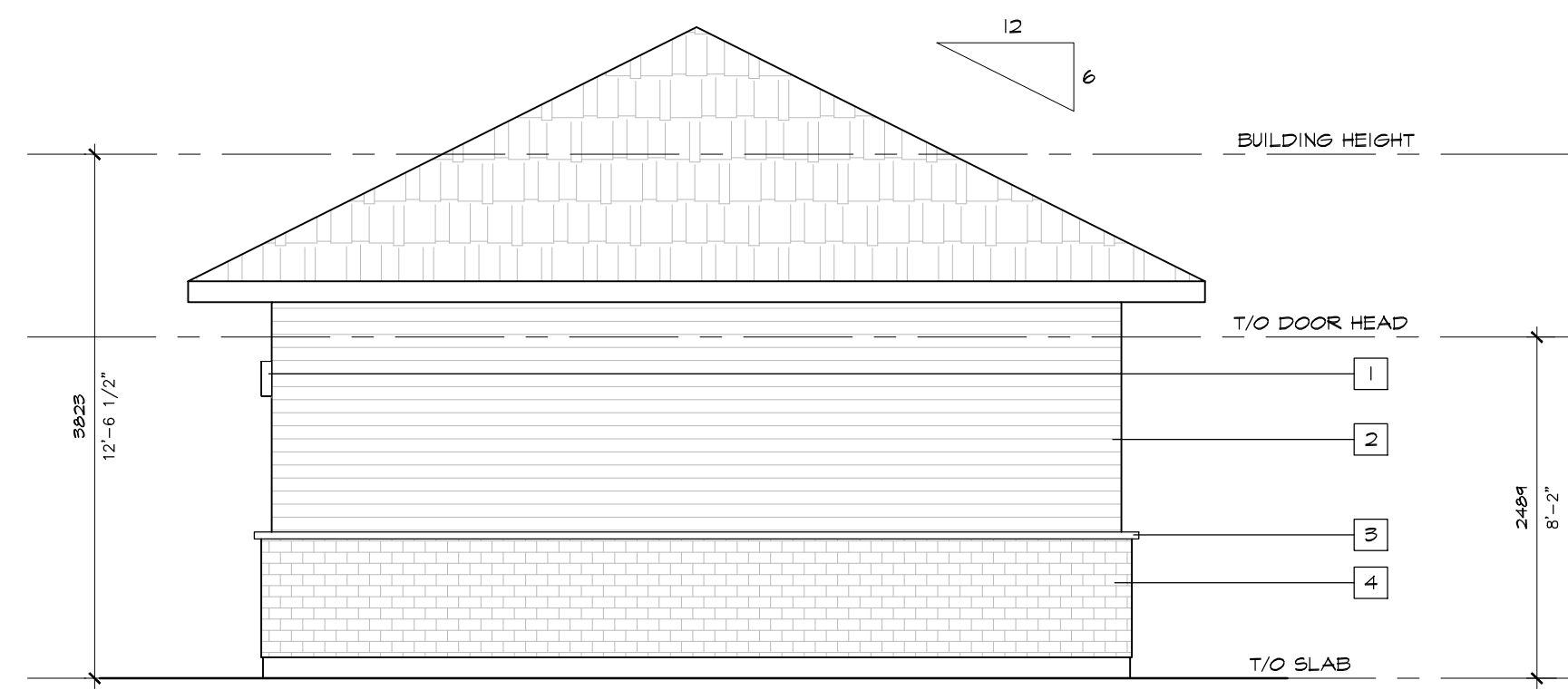
PROVIDE 24" PRE-FINISHED METAL FLASHING (COLOUR TO MATCH SHINGLES) & 48" ICE & WATER SHIELD FOR ALL ROOF VALLEYS & ROOF TO WALL INTERSECTIONS. ROOF FLASHING TO COMPLY WITH 9.26 O.B.C. 2012 -TYP.

PROVIDE MIN. 1/500TH CLEAR VENT AREA OF THE INSULATED CEILING AREA. MIN. 25% VENTING AREA THROUGH ROOF VENTS & MIN. 25% VENTING AREA AT SOFFIT. UNIFORM DISTRIBUTION ON OPPOSITE SIDES OF THE BUILDING - TYP.

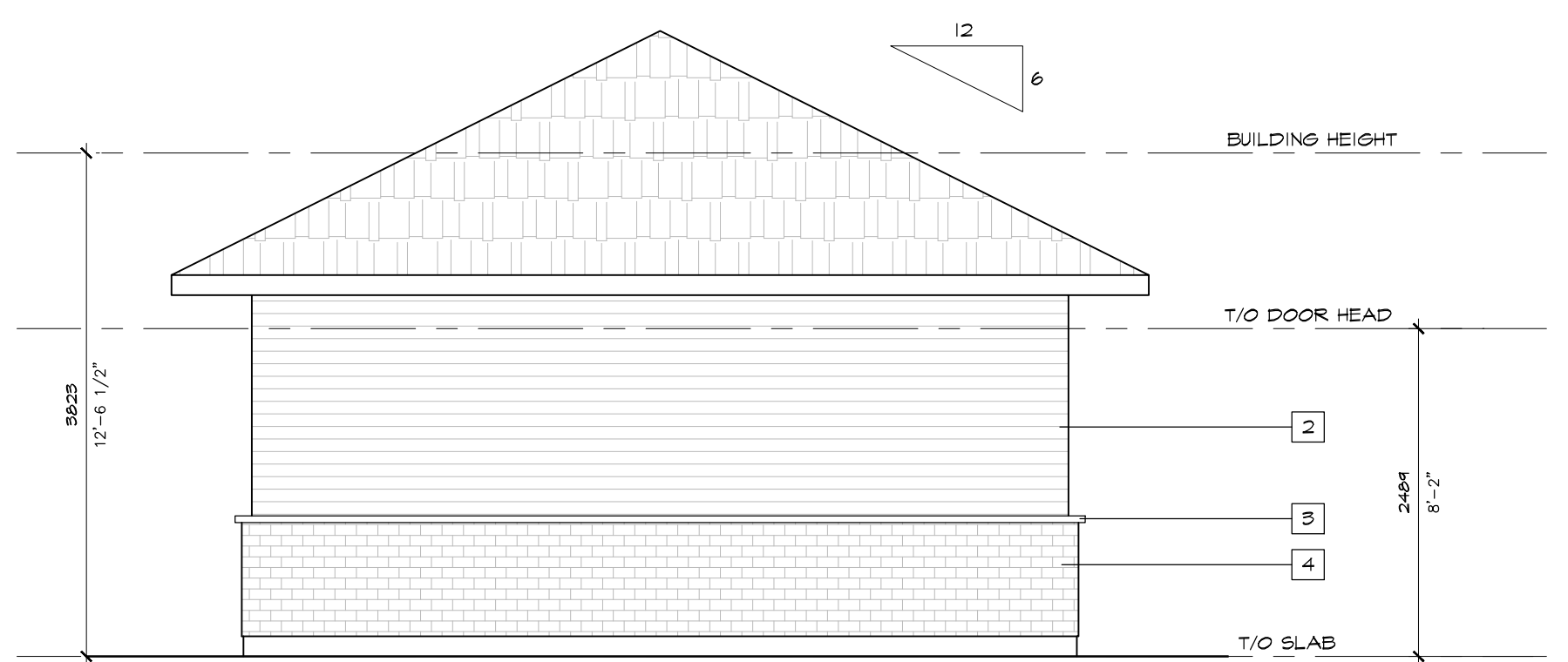
02 ACCESSORY BUILDING A ROOF PLAN
 A1.1 SCALE: 1/50



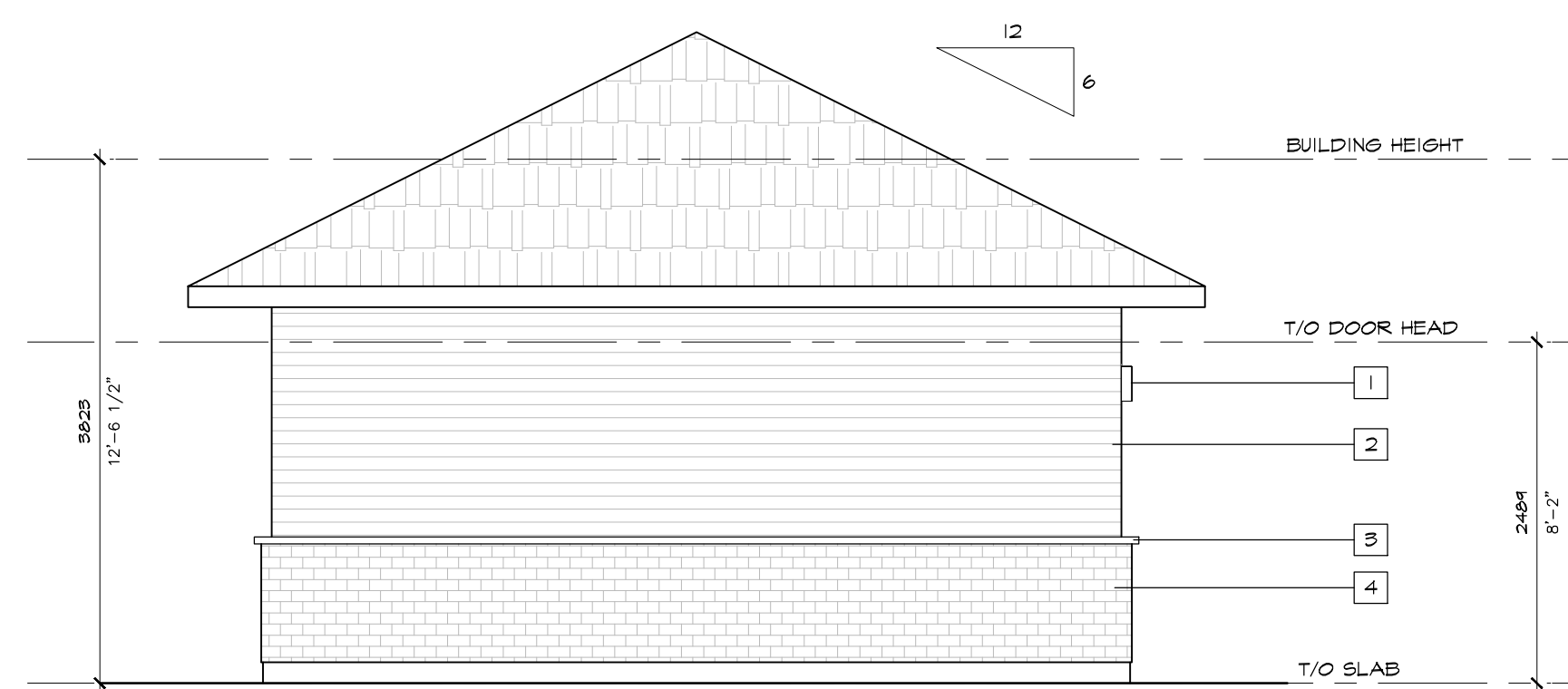
03 ACCESSORY BUILDING A FRONT ELEVATION
 A1.1 SCALE: 1/250



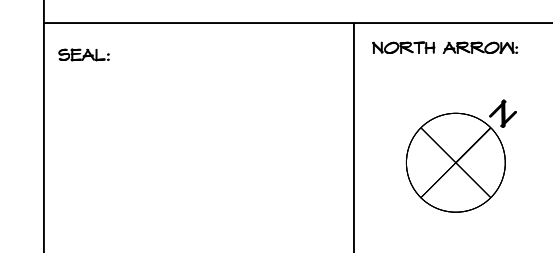
04 ACCESSORY BUILDING A RIGHT ELEVATION
 A1.1 SCALE: 1/250



05 ACCESSORY BUILDING A REAR ELEVATION
 A1.1 SCALE: 1/250



06 ACCESSORY BUILDING A LEFT ELEVATION
 A1.1 SCALE: 1/250



NO.	DATE	REVISION
14	2023.11.04	ISSUED FOR COORDINATION
13	2023.11.01	ISSUED FOR COORDINATION
12	2023.04.12	ISSUED FOR REVIEW
11	2023.08.21	ISSUED FOR REVIEW
10	2023.01.13	ISSUED FOR COORDINATION
09	2023.06.13	ISSUED FOR SPG
08	2023.03.20	RE-ISSUED FOR PRE-CONSULT
07	2023.03.14	ISSUED FOR REVIEW
06	2023.02.22	RE-ISSUED FOR PRE-CONSULT
05	2023.02.14	ISSUED FOR REVIEW
04	2023.01.19	ISSUED FOR REVIEW
03	2022.11.22	ISSUED FOR PRE-CONSULT
02	2022.11.18	ISSUED FOR REVIEW
01	2022.11.09	ISSUED FOR REVIEW

S.J. LAWRENCE ARCHITECT INCORPORATED
 19 DEAKIN STREET
 SUITE 209
 OTTAWA, ONTARIO
 K2E 6B1
 T: (613) 794-1100 F: (613) 794-1103
 sjl@sjlarchitect.com



THIS DRAWING IS THE SOLE PROPERTY OF S.J. LAWRENCE ARCHITECT INCORPORATED. REPRODUCTION IS NOT PERMITTED.

PROJECT:
OLD SECOND LINE DEVELOPMENT
 1156 OLD SECOND LINE ROAD, OTTAWA, ON

SHEET TITLE:
ACCESSORY BUILDING A

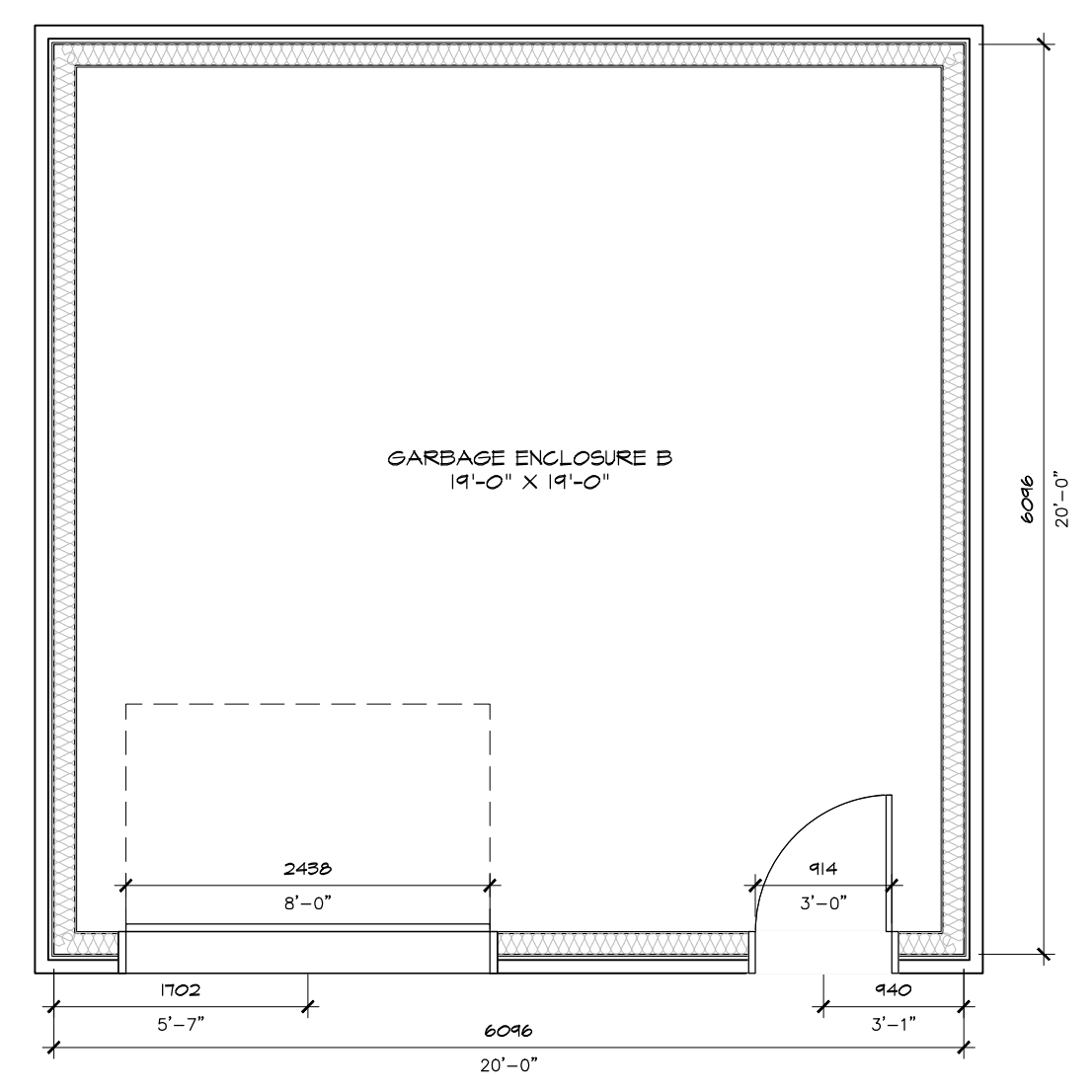
DRAWN BY: S.J.L. CHECKED BY: S.J.L.
 DATE: 2023.11.04 PROJECT DATE: DATE

JOB NUMBER: SL-1086-22 SCALE: AS NOTED

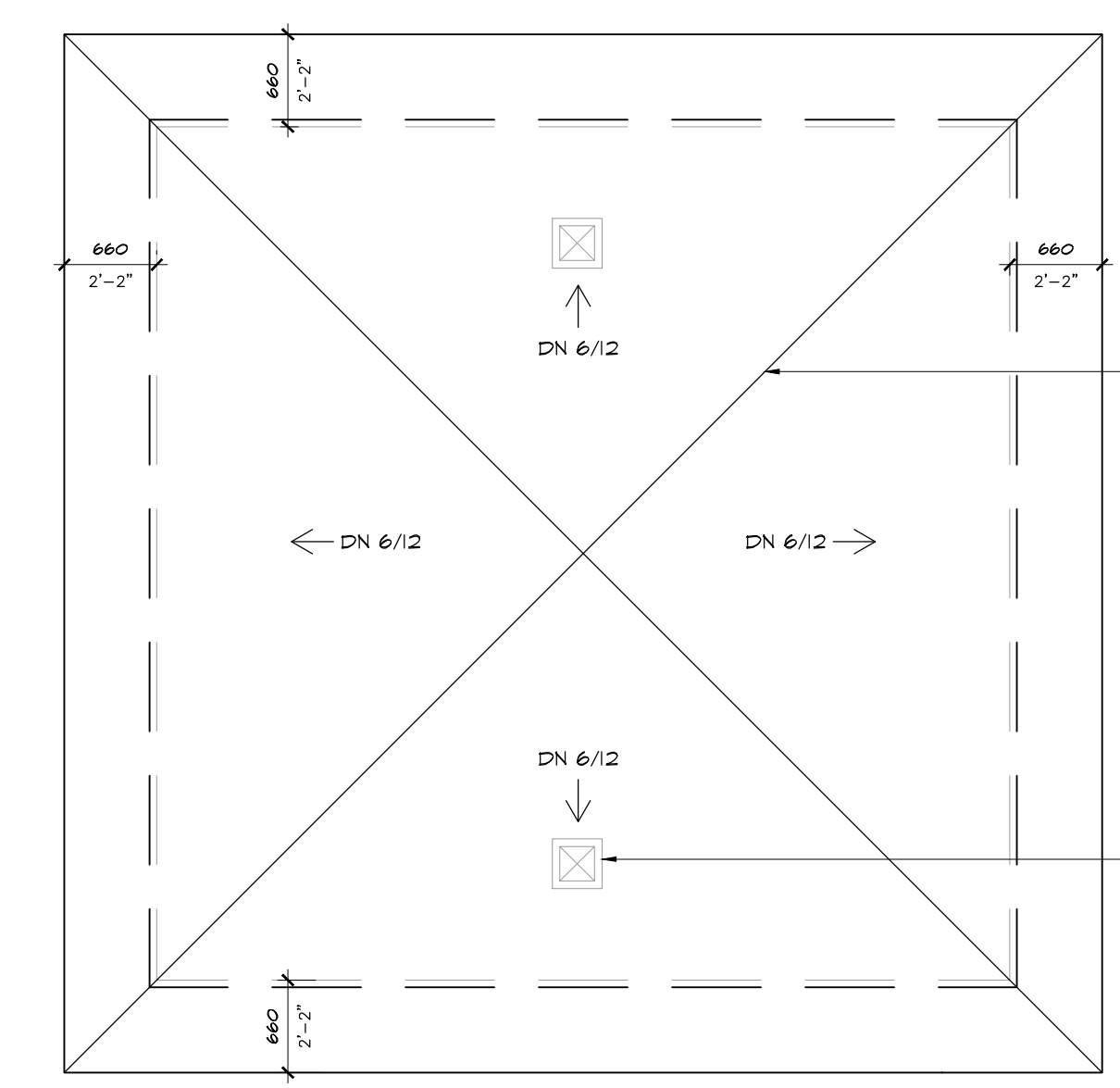
SHEET NUMBER:
A1.1

NOTES:
 1) ALL WORK TO BE IN COMPLIANCE WITH LOCAL BUILDING CODES, REGULATIONS AND BY-LAWS.
 2) ADDITIONAL DRAWINGS MAY BE ISSUED FOR CLARIFICATION TO ASSIST PROPER EXECUTION OF WORK. SUCH DRAWINGS WILL HAVE THE SAME MEANINGS AND INTENT AS IF THEY WERE INCLUDED WITH PLANS IN CONTRACT DOCUMENTS.
 3) DO NOT SCALE DRAWINGS.
 4) ALL SUB-CONTRACTORS TO TAKE THEIR OWN ON-SITE MEASUREMENTS AND BE RESPONSIBLE FOR THEIR ACCURACY.
 5) NOTIFY SHAWN J. LAWRENCE ARCHITECT FOR ANY ERRORS AND/OR OMISSIONS PRIOR TO START OF WORK.

KEY NOTES:
 1. WALL SCENCE
 2. SIDING
 3. PRECAST CONCRETE SILL
 4. STONE VENEER



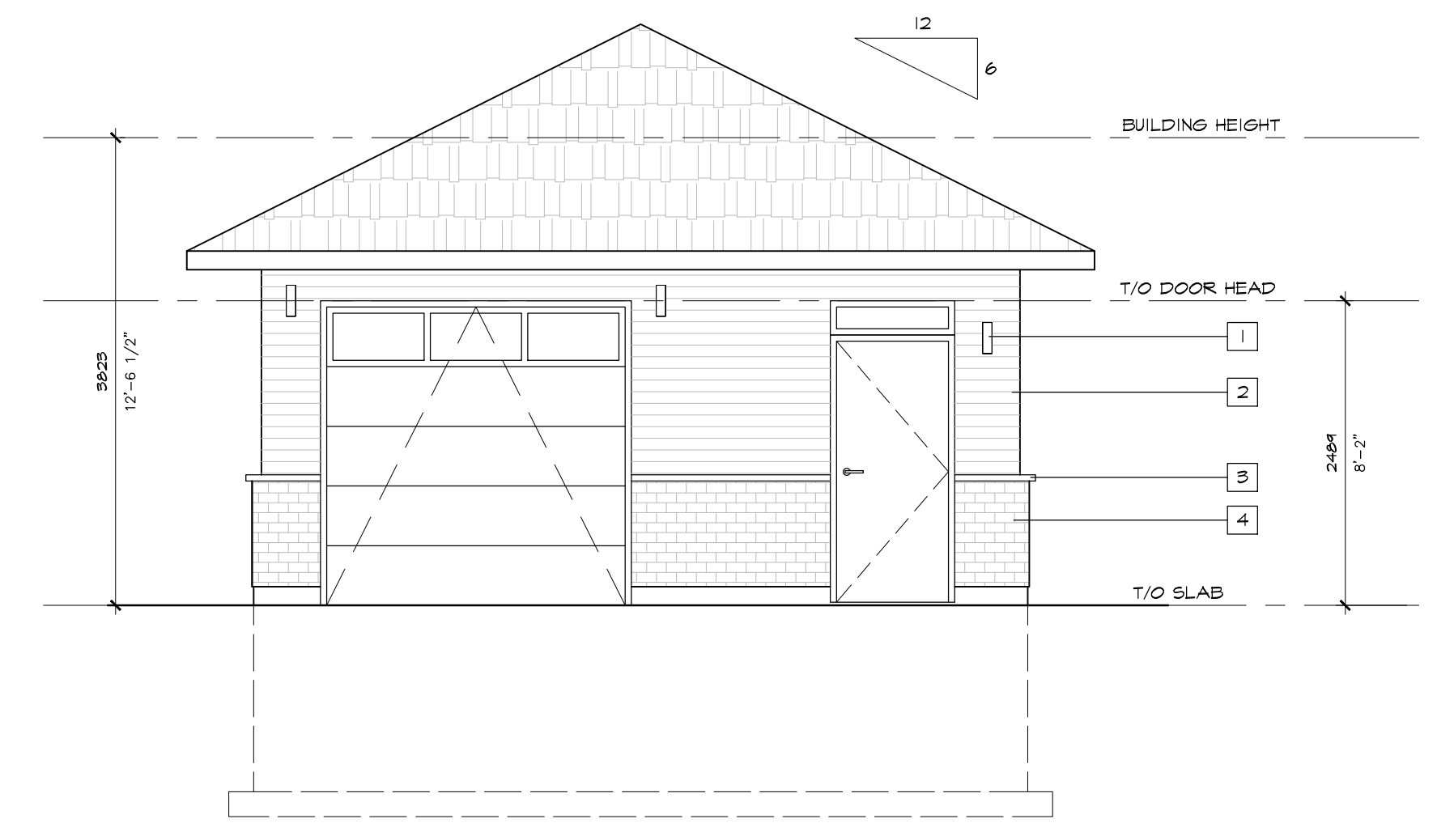
01 ACCESSORY BUILDING B FLOOR PLAN
 A1.2 SCALE: 1/50



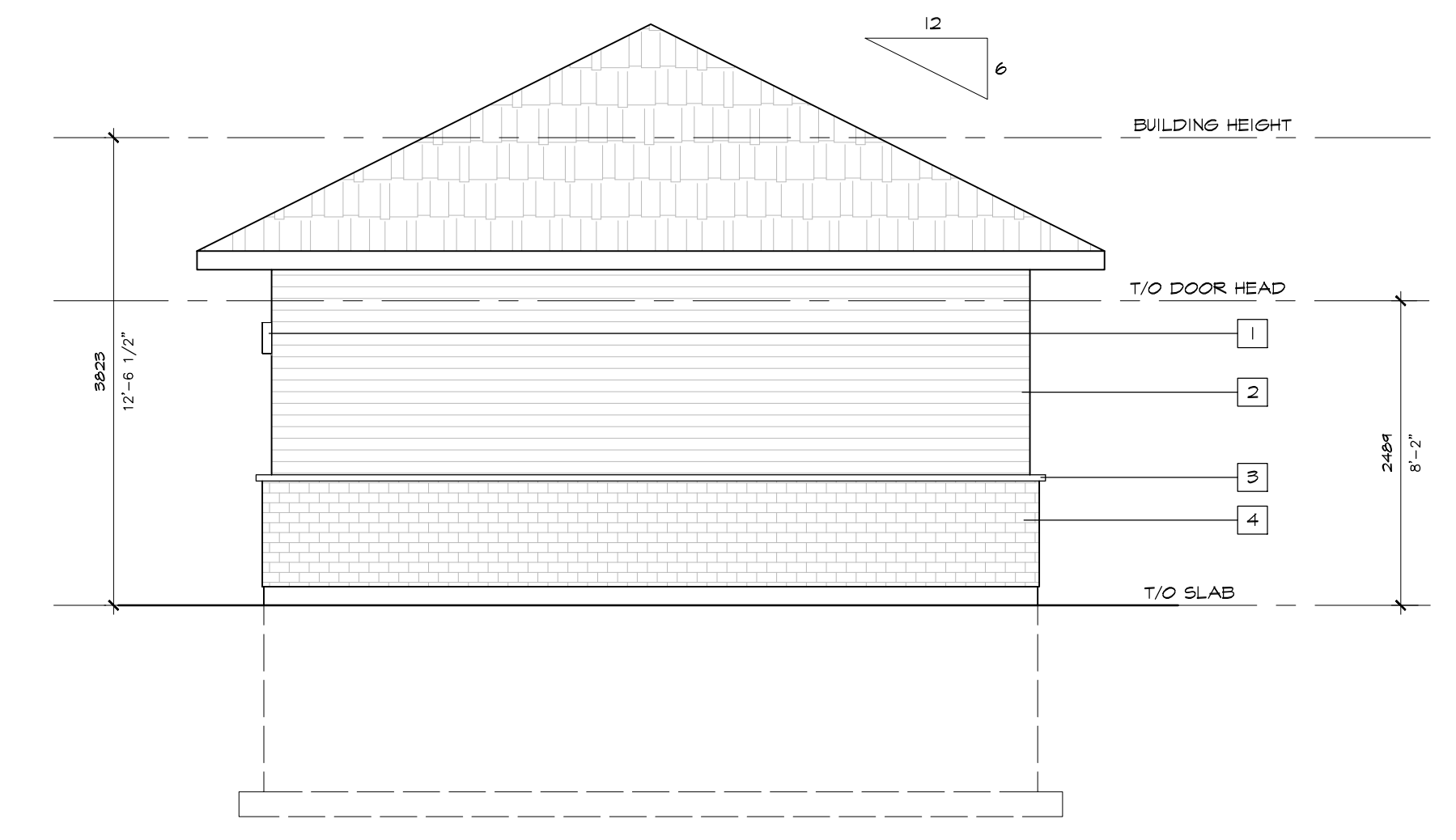
PROVIDE 24" PRE-FINISHED METAL FLASHING (COLOUR TO MATCH SHINGLES) & 48" ICE & WATER SHIELD FOR ALL ROOF VALLEYS & ROOF TO WALL INTERSECTIONS. ROOF FLASHING TO COMPLY WITH 9.26 O.B.C. 2012 -TYP.

PROVIDE MIN. 1/800TH CLEAR VENT AREA OF THE INSULATED CEILING AREA. MIN. 25% VENTING AREA THROUGH ROOF VENTS & MIN. 25% VENTING AREA AT SOFFIT. UNIFORM DISTRIBUTION ON OPPOSITE SIDES OF THE BUILDING - TYP.

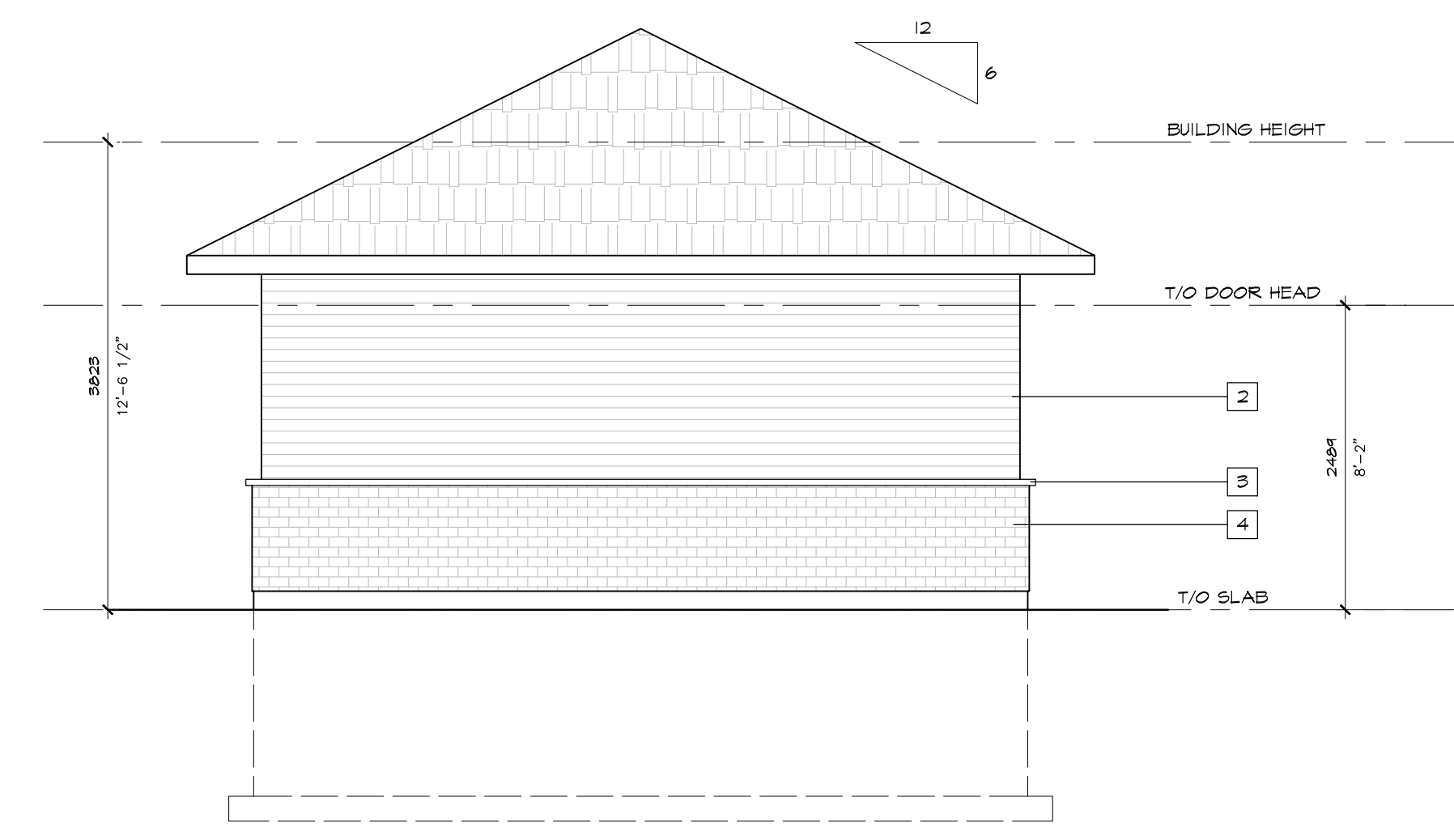
02 ACCESSORY BUILDING B ROOF PLAN
 A1.2 SCALE: 1/50



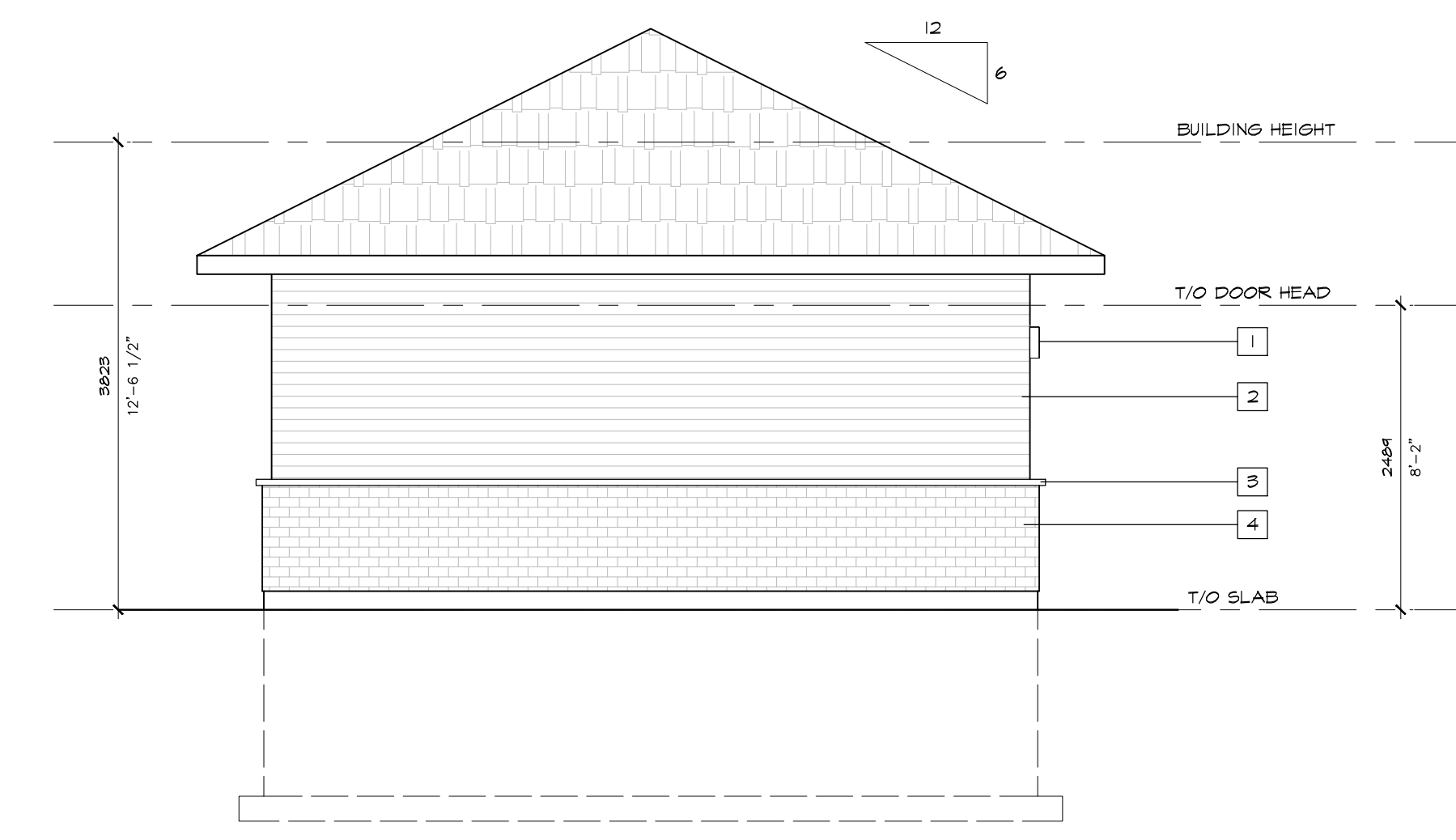
03 ACCESSORY BUILDING B FRONT ELEVATION
 A1.2 SCALE: 1/250



04 ACCESSORY BUILDING B RIGHT ELEVATION
 A1.2 SCALE: 1/250



05 ACCESSORY BUILDING B REAR ELEVATION
 A1.2 SCALE: 1/250



06 ACCESSORY BUILDING B LEFT ELEVATION
 A1.2 SCALE: 1/250

SEAL: _____ NORTH ARROW:

No.	DATE	REVISION
14	2023.11.04	ISSUED FOR COORDINATION
13	2023.11.01	ISSUED FOR COORDINATION
12	2023.04.12	ISSUED FOR REVIEW
11	2023.08.21	ISSUED FOR REVIEW
10	2023.01.13	ISSUED FOR COORDINATION
09	2023.06.13	ISSUED FOR SPG
08	2023.03.20	RE-ISSUED FOR PRE-CONSULT
07	2023.03.14	ISSUED FOR REVIEW
06	2023.02.22	RE-ISSUED FOR PRE-CONSULT
05	2023.02.14	ISSUED FOR REVIEW
04	2023.01.19	ISSUED FOR REVIEW
03	2022.11.22	ISSUED FOR PRE-CONSULT
02	2022.11.18	ISSUED FOR REVIEW
01	2022.11.09	ISSUED FOR REVIEW

S.J. LAWRENCE ARCHITECT INCORPORATED
 19 DEAKIN STREET
 SUITE 209
 OTTAWA, ONTARIO
 K2E 0B1
 T: (613) 794-1100 F: (613) 794-1103
 sjl@sjlarchitect.com



THIS DRAWING IS THE SOLE PROPERTY OF S.J. LAWRENCE ARCHITECT INCORPORATED. REPRODUCTION IS NOT PERMITTED.

PROJECT:
OLD SECOND LINE DEVELOPMENT
 1150 OLD SECOND LINE ROAD, OTTAWA, ON

SHEET TITLE:
ACCESSORY BUILDING B

DRAWN BY: _____ CHECKED BY: _____
 D.T. B.L. S.J.L.

PLOT DATE: 2023.11.04 PROJECT DATE: _____
 DATE DATE

JOB NUMBER: SL-1086-22 SCALE: AS NOTED

SHEET NUMBER: **A1.2**