EDWARD J. CUHACI AND ASSOCIATES ARCHITECTS INC.

ÉCOLE ÉLÉMENTAIRE CATHOLIQUE AVALON III
TENTH LINE ROAD, OTTAWA, ON

SERVICING AND STORMWATER MANAGEMENT REPORT

JUNE 2, 2023

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SITE PLAN APPLICATION

PROJECT NO.: 221-12984-00
DATE: JUNE 2023

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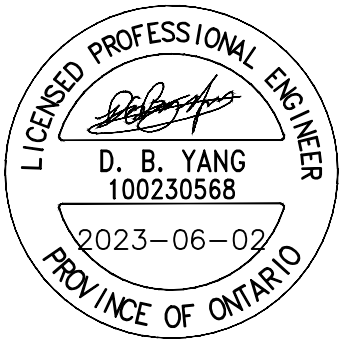

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1 GENERAL

1.1 EXECUTIVE SUMMARY

WSP was retained by Edward J. Cuhaci and Associates Architects Inc. to provide servicing, grading and stormwater management design services for the proposed new Avalon III Ecole Catholic Elementarie School on a 1.747 ha site located at the southwest corner of Tenth Line Road and Sweetvalley Drive, in the proposed Summerside South Phase 1 subdivision development within the Mer Bleue Community in Orleans Ottawa. The construction of services and base course asphalt is complete on Sweetvalley Drive, on which the school property will front. All services for the school site will be available from Sweetvalley Drive. The subjected development is bounded by the Phase 1 of the subdivision development to the north, McKinnon's Creek channel block to the west and Existing residential block to the south. The future McKinnon's Creek channel block will be designed by the subdivision developer in a later date. This report outlines findings and calculations pertaining to the servicing of the proposed building with a gross building area of 2,308.8 square metres.

The proposed school building is a two storey school building with gross floor area of 2,862.8 square metre and maximum building height will not be higher than 18 metres which is located at the northeast corner of the subjected site, southwest corner of the Tenth Line Road and Sweetvalley Drive intersection. To the south of the proposed school building, there will be future portable classroom. These portable classrooms will be removed when the addition to the school is needed. East of the school, it's Tenth Line Road. Tenth Line Road will be widened in the future. Southwest of the school, there will be playground and practise football/soccer field. West of the school, there will be parking spots for the staff and visitors. Since the proposed school and portable classrooms are located close to the Sweetvalley Drive and Tenth Line Road R.O.W. These streets will be used as the fire route to service the school building and portable classrooms area.

There will be three future additional parking spots to the west of the proposed parking areas will be constructed at a later time. The current grading and servicing design have been provided to allow for the future site plan changes with minimal changes to grading and servicing modifications only within the areas that will be impacted by the future development.

The surrounding neighbourhood is being developed by Mattamy Home Mer Bleue 2 Limited. with David Schaefer Engineering Ltd providing engineering design services. Information regarding the proposed municipal services was provided by DSEL, as described in Design Brief – Summerside South – Phase 1, 2464 Tenth Line Road, Project: 15-766, Revised June 24, 2019. Excerpts from the Design Brief are provided in Appendix A of this report.

Currently the land proposed for the building abuts the collector road Sweetvalley Drive which is located to the north of the subject site. The natural topography of the property in the vicinity of the collector road slopes from both east and west towards Pewee Place. Currently the land is vacant and half grass covered and half abandoned storage warehouse. The total study area was considered to be 1.747 ha in size. It is part of lot 5, concession 11, geographic Township of Cumberland in City of Ottawa. Based on the topographic survey, portion of the site is sloping from the northeast corner to the southwest corner and will be draining toward McKinnon's Creek Channel block. The east portion of the site is slopping toward Tenth Line Road. The south portion of the site is slopping toward the existing residential block to the south. The existing piped stormwater system within Summerside South phase 1 subdivision development conveys drainage to expanded Avalon West SWM Facility then discharges to the McKinnon's Creek.

As per the Summerside South Phase 1 Design Brief by DSEL, the following criteria apply: runoff from all storm events up to and including the 1:100 year event must be restricted to a calculated rate based on an imperviousness ratio of 0.50, 2 year simulated flow of 567 l/s. The subject site must provide sufficient storage to accommodate runoff from the 1:100 year event. Stormwater quality control is not required for this site. Design of a drainage and stormwater management system in this development must be prepared in accordance with the following documents:

- Sewer Design Guidelines, City of Ottawa, October 2012;
- Stormwater Management Planning and Design Manual, Ministry of the Environment, March 2003; and
- Stormwater Management Facility Design Guidelines, City of Ottawa, April 2012

This report was prepared utilizing servicing design criteria obtained from the City of Ottawa and outlines the design for water, sanitary wastewater, and stormwater facilities, including stormwater management.

The format of this report matches that of the servicing study checklist found in Section 4 of the City of Ottawa's Servicing Study Guidelines for Development Applications, November 2009.

The following municipal services are available at the north property line as recorded from drawings received from IBI Group: Sweetvalley Drive:

- 900 mm storm sewer 1800 mm storm sewer, 200mm sanitary sewer and 300mm watermain.

It is proposed that:

- On-site stormwater management systems, employing surface storage and roof storage will be provided to attenuate flow rates leaving the school site. Existing drainage patterns, previously established controlled flow rates and storm sewers will be maintained.
- The subject site maintains emergency overland flow routes for flows deriving from storm events in excess of the maximum design event to the proposed municipal rights-of-way at the southern boundaries of the subdivision development as depicted in DSEL Drawing 34, which attached to Appendix A for reference.

1.2 DATE AND REVISION NUMBER

This version of the report is the initial issue, dated June 2, 2023.

1.3 LOCATION MAP AND PLAN

The proposed institutional development is located at 700 Cope Drive, Stittsville, Ontario at the location shown in Figure 1-1 below.

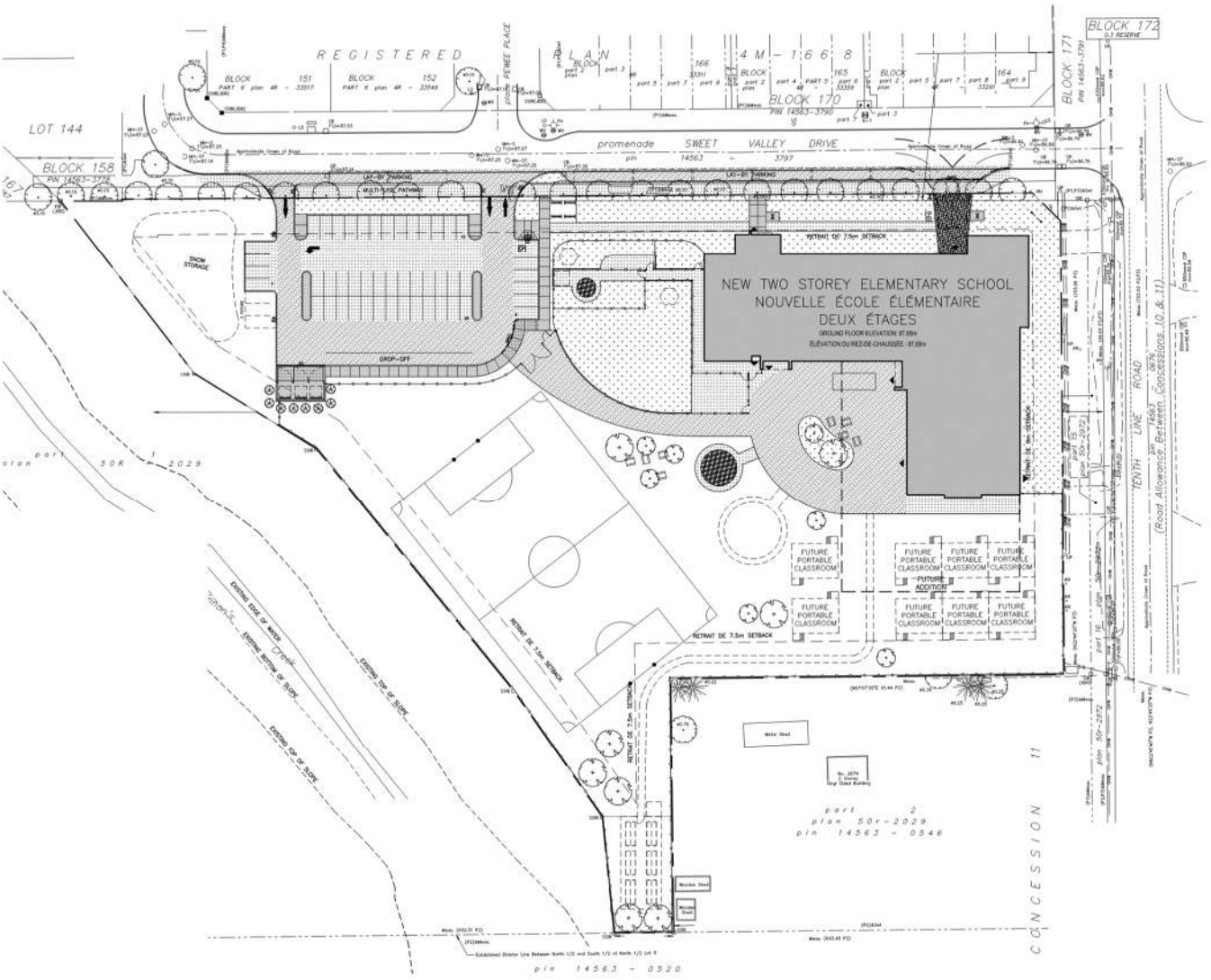


Figure 1-1 Site Location

1.4 ADHERENCE TO ZONING AND RELATED REQUIREMENTS

The proposed property use will be in conformance with zoning and related requirements prior to approval and construction and is understood to be in conformance with current zoning.

1.5 PRE-CONSULTATION MEETINGS

A pre-consultation meeting was held with the City of Ottawa on August 23, 2022. Notes from this meeting are provided in Appendix A.

1.6 HIGHER LEVEL STUDIES

The review for servicing has been undertaken in conformance with, and utilizing information from, the following documents:

- Ottawa Sewer Design Guidelines, Second Edition, Document SDG002, October 2012, City of Ottawa including:
    - Technical Bulletin ISDTB-2012-4 (20 June 2012)
    - Technical Bulletin ISDTB-2014-01 (05 February 2014)
    - Technical Bulletin PIEDTB-2016-01 (September 6, 2018)
    - Technical Bulletin ISDTB-2018-01 (21 March 2018)
    - Technical Bulletin ISDTB-2018-04 (27 June 2018)
- Ottawa Design Guidelines – Water Distribution, July 2010 (WDG001), including:
    - Technical Bulletin ISDTB-2014-02 (May 27, 2014)
    - Technical Bulletin ISTB-2018-02 (21 March 2018)
- Stormwater Management Planning and Design Manual, Ontario Ministry of the Environment and Climate Change, March 2003 (SMPDM).
- Design Brief – Summerside South -Phase 1 2464 Tenth Line Road, DSEL, Project 15-766, Revised June 24, 2019. (Includes water, sanitary and storm servicing.)
- Stormwater Management Report for Summerside South Phase 1, JFSA, Project 1102-13, Revised June 2019.
- Design Guidelines for Drinking-Water Systems, Ontario Ministry of the Environment and Climate Change, 2008 (GDWS).
- Fire Underwriters Survey, Water Supply for Public Fire Protection (FUS), 2020.

1.7 STATEMENT OF OBJECTIVES AND SERVICING CRITERIA

The objective of the site servicing is to meet the requirements for the proposed modification of the site while adhering to the stipulations of the applicable higher-level studies and City of Ottawa servicing design guidelines.

1.8 AVAILABLE EXISTING AND PROPOSED INFRASTRUCTURE

Existing storm sewer and watermain stubs have been provided from Sweetvalley Drive to the north of frontage of the site. The sanitary sewer can also be extended from the existing 200mm diameter sanitary along Sweetvalley Drive to the proposed Elementary School next to the existing storm stub. The storm sewer will be connected to the existing 900 mm stub, and flows from south to north. Water and storm sewer stubs have already been provided to the property boundary during the time of construction of Sweetvalley Drive. The works provided by the subdivision developer have already included the water valve and box at the property line, and all work within the right of way, excluding the driveway entrances, water service will be routed to the water entry room from the existing stub. Ultimately, the storm flows from Sweetvalley Drive (servicing the school site) to the Pewee Place storm sewer are intended to be directed to a permanent Avalon West SWM Facility that will provide quality and quantity treatment for Summerside South Phase 1 subdivision, and including the school site. Quality control is not required on the school site, but quantity control is required to restrict the discharge for all events up to a 100 year event to the 2 year flow rate provided by DSEL.

Site access for vehicles will be provided from Sweetvalley Drive. The driveways being provided are two-way entrances at the centre north boundary.

1.9 ENVIRONMENTALLY SIGNIFICANT AREAS, WATERCOURSES AND MUNICIPAL DRAINS

The proposed development site is surrounded by residential and commercial lands. The west boundary is abutting the McKinnon's Creek Channel Block. Existing Runoff from the site is currently draining overland toward McKinnon's Creek. However, post development runoff toward the creek will be reduced significantly.

1.10 CONCEPT LEVEL MASTER GRADING PLAN

The existing and proposed grading are shown on Drawings C02 - Grading Plan. Existing grading was identified in a topographic survey and is noted in the background of Drawings C02. The proposed grading will be reviewed by the geotechnical engineer. The geotechnical investigation was completed in March 07, 2023 by Exp Service Inc. The site topographic survey, provides evidence of direction of overland flow of the site. Minor grade changes will be made to grades at the development perimeter for the proposed bus drop off lay-by and entrances location.
Grading will employ terraced slopes of 3H:1V to provide transitions from the new work areas to existing grades. No changes will be made to grades at the property perimeter other than the north boundary.

1.11 DEVELOPMENT PHASING

The proposed development includes future portable classrooms and building addition. The impervious area associated with the future development has been taken into account in the stormwater management calculations. The future hard surfaces take up a bit of the green space than the current condition, and therefore were conservatively used in the calculation of runoff.

1.12 GEOTECHNICAL SUTDY

A geotechnical investigation report has been prepared by Exp Services Inc. (Project OTT-22017859-A0, March 07, 2023), and its recommendations has been taken into account in developing the engineering specifications.

2 WATER DISTRIBUTION

2.1 CONSISTENCY WITH MASTER SERVICING STUDY AND AVAILABILITY OF PUBLIC INFRASTRUCTURE

The site is bounded to the Summerside South Subdivision Phase 1. SSS Phase 1 is located within Zone 2E of the City's water distribution system, which is fed by two booster pumping stations and the Innes Road elevated storage tank at Belcourt Boulevard, providing balancing, fire, and emergency storage. There is an existing 305mm diameter municipal watermain along Sweetvalley Drive providing water to the property. The new elementary school will be protected with a supervised automatic fire protection sprinkler system and will require a 203mm diameter water service. The fire department connection is located at the north side of the building at the main entrance fronting Sweetvalley Drive. It is 45m away from the existing municipal FH on Sweetvalley Drive. No changes are required to the existing City water distribution system to allow servicing for this property. A single 203mm water service and an isolation valve in between will be made to the existing 203mm diameter stub at the north boundary from Sweetvalley Drive for the proposed development site. The 203mm diameter private watermain services connecting the existing 305mm municipal watermain will provide redundancy for the school building. Water can be supplied from both side of Sweetvalley Drive and Pewee Place. The 203mm dia. water service will be extended to the building mechanical room.

2.2 SYSTEM CONSTRAINTS AND BOUNDARY CONDITIONS

Boundary conditions have been provided by the City of Ottawa at the connection at Sweetvally Drive. The fire flow of 133.3 l/s (8,000 l/min) was estimated for the proposed school with using the FUS calculation method and is included in Appendix B.

Table 2-1 Boundary Condition

BOUNDARY CONDITIONS AT COPE DRIVE		
SCENARIO	Head (m)	Pressure (psi)
Basic Day (MAX HGL)	130.3	62.9
Peak Hour (MIN HGL)	126.0	56.9
Max Day + Fire Flow (ICI)	126.3	57.3

2.3 CONFIRMATION OF ADEQUATE DOMESTIC SUPPLY AND PRESSURE

Water demands are based on Table 4.2 of the Ottawa Design Guidelines – Water Distribution. As previously noted, the development is considered as institutional development, consisting of classroom, gymnasium and kitchen. A water demand calculation sheet is included in Appendix B, and the total water demands are summarized as follows:

	WSP
Average Day	0.57 l/s
Maximum Day	0.86 l/s
Peak Hour	1.55 l/s

The 2010 City of Ottawa Water Distribution Guidelines stated that the preferred practice for design of a new distribution system is to have normal operating pressures range between 345 kPa (50 psi) and 552 kPa (80 psi) under maximum daily flow conditions. Other pressure criteria identified in the guidelines are as follows:

Minimum Pressure	Minimum system pressure under peak hour demand conditions shall not be less than 276 kPa (40 psi)
Fire Flow	During the period of maximum day demand, the system pressure shall not be less than 140 kPa (20 psi) during a fire flow event.
Maximum Pressure	Maximum pressure at any point the distribution system shall not exceed 689 kPa (100 psi). In accordance with the Ontario Building/Plumbing Code, the maximum pressure should not exceed 552 kPa (80 psi). Pressure reduction controls may be required for buildings where it is not possible/feasible to maintain the system pressure below 552 kPa.

Water pressure at municipal connections check:

Min. HGL @ Connection 1 – Pavement elevation = 126.0m – 87.08m = 38.92m = 381.58 kPa

Water pressure at building connection (at average day) check:

Max. HGL @ Connection 1 – Finished floor elevation = 130.3m – 87.85 = 53.04m = 146.19 kPa

Water pressure at building connection (at max. hour demand) check:

Min. HGL @ Connection 1 – Finished floor elevation = 126.0m-87.85m = 38.15m = 374.03 kPa

Water pressure at building connection (at max. day + fire demand):

(Max Day + Fire) HGL @ Connection 1 - Finished floor elevation = 126.3m-87.85m = 38.45m = 376.97 kPa

The minimum water pressure inside the building at the connection is determined with the minimum HGL condition, resulting in a pressure of 374.03 kPa which exceed the minimum requirement of 276 kPa per the guidelines.

2.4 CONFIRMATION OF ADEQUATE FIRE FLOW PROTECTION

The fire flow rate has been calculated using the Fire Underwriters Survey (FUS) method. The method takes into account the type of building construction, the building occupancy, the use of sprinklers and the exposures to adjacent structures. Assuming fire resistive construction and a fully supervised sprinkler system, a fire flow demand of 8,000 l/min for the new high school. The fire flow rate of 6,000 l/min (100 l/s) is calculated for the future portable classrooms. Copy of the FUS calculations are included in Appendix D.

The demand of 8,000 l/min can be delivered through two fire hydrants. The existing two public hydrants are located at the northside of Sweetvalley Drive, one of them is within 45 m to the building Siamese, and is rated at 5,700 l/min., the other one is within 75 m to the building and is rated at 5,700 l/min. The two hydrants have a combined total of 11,400 l/min.

The demand of 6,000 l/min from the future portable classrooms can also be met through the combination of two fire hydrants from Sweetvalley Drive, they are within 100m to the future portable classrooms, and are rated at 3,800 l/min each. The combined total of 7,600 l/min. And there will be future fire hydrants along Tenth Line Road to the east when the watermain system, expansion is completed.

The proposed building on site will be serviced by a single 203 mm service off the existing 203 mm watermain extended from the Sweetvalley Drive and Pewee Place intersection. The service will run into the water entry room. The proposed building will be fully sprinklered and fire protection will be provided with the fire department Siamese connection within 45 m of the existing municipal fire hydrant at Sweetvalley Drive. The Siamese connection is located on the north side of the building.

The boundary condition for Maximum Day and Fire Flow results in a pressure of 376.97 kPa at the ground floor level. In the guidelines, a minimum residual pressure of 140 kPa must be maintained in the distribution system for a fire flow and maximum day event. As a pressure of 376.97 kPa is achieved, the fire flow requirement is exceeded.

2.5 CHECK OF HIGH PRESSURE

High pressure is not a concern. The maximum water pressure inside the building at the connection is determined with the maximum HGL condition, resulting in a pressure of 381.58 kPa which is less than the 552 kPa threshold in the guideline in which pressure control is required. Based on this result, pressure control is not required for this building.

2.6 PHASING CONSTRAINTS

No development phasing has been detailed for the site. The site plan does indicate possible future development of additional parking lots. The projected occupancy load has been taken into account in the fire demand and water demand calculations. No phasing constraints exist.

2.7 RELIABILITY REQUIREMENTS

One shut off valve is provided for the private watermain at the study boundary from Sweetvalley Drive. Water can be supplied from both sides of Sweetvalley Drive and Pewee Place, west, east and north can be isolated.

2.8 CAPABILITY OF MAJOR INFRASTRUCTURE TO SUPPLY SUFFICIENT WATER

The existing infrastructure for the Sumerside South Phase 1 Subdivision is capable of meeting the domestic demand based on City requirements and fire demand as determined by FUS requirements for the proposed residential units.

2.9 DESCRIPTION OF PROPOSED WATER DISTRIBUTION NETWORK

A 203 mm private water service is proposed to be extended into the proposed elementary school from the existing stub. The 203 mm private water service will be split inside the building, one branch will be connected to the water meter, the other branch will be connected to the fire suppression system. No private hydrant is required for this site.

2.10 OFF-SITE REQUIREMENTS

No off-site improvements to watermains, feedermains, pumping stations, or other water infrastructure are required to maintain existing conditions and service the adjacent buildings, other than the connection of the new private watermain to the City watermain in the south frontage of the site.

2.11 CALCULATION OF WATER DEMANDS

Water demands were calculated as described in Sections 2.3 and 2.4 above and is also attached in Appendix B.

2.12 MODEL SCHEMATIC

The water works consist single building service, a model schematic is not required for this development.

2.13 WATER SUPPLY CONCLUSION

The proposed school will be serviced internally by 203 mm water service, which will be connected to the existing 203 mm watermain stub from Sweetvally Drive. A detailed hydraulic calculation has been completed above to confirm that the proposed water network can deliver all domestic and fire flows as per the Ministry of the Environment, City of Ottawa and Fire Underwriters criteria.

The proposed water supply design conforms to all relevant City guidelines and policies.

3 WASTEWATER DISPOSAL

3.1 DESIGN CRITERIA

In accordance with the City of Ottawa's Sewer Design Guidelines, the following design criteria have been utilized in order to predict wastewater flows generated by the subject site and complete the sewer design;

- Minimum Velocity 0.6 m/s
- Maximum Velocity 3.0 m/s
- Manning Roughness Coefficient 0.013
- Total est. hectares institutional use 1.747
- Average sanitary flow for institutional use 28,000 L/Ha/day
- Commercial/Institutional Peaking Factor 1.5
- Infiltration Allowance (Total) 0.33 L/Ha/s
- Minimum Sewer Slopes – 200 mm diameter 0.32%

The area of 1.747 ha represents the lot area of the new building and immediate surrounding area to the sides of the new building. This is the sanitary collection area that is being considered to contribute to the new 200mm sanitary service extending from the existing 200mm sanitary sewer from Sweetvalley Drive and Pewee Place intersection to the new building.

3.2 CONSISTENCY WITH MASTER SERVICING STUDY

The outlet for the sanitary service from the proposed building is the 200 mm diameter municipal sewer at Sweetvalley Drive and Pewee Place intersection. The Ottawa Sewer Design Guidelines provide estimates of sewage flows based on institutional development.

The criteria to determine anticipated actual peak flow based on site used as described in Ottawa Sewer Design Guidelines Appendix 4-A are as follows.

- Institutional 28000 L/Ha/day = 0.324 L/Ha/s
- Peak flow = (0.324 L/Ha/s x 1.765 ha x 1.5 peaking factor) + 0.33 l/Ha/s x 1.765 ha = 1.01 L/s

The on-site sanitary sewer network has been designed in accordance with 5.35 L/s as described above.

3.3 DESCRIPTION OF EXISTING SANITARY SEWER

The existing sanitary outlet for Summerside South Phase 1 is the Tenth Line Road Pump Station (TLPS), which in turn outlets by forcemain to the Esprit Drive Collector. The TLPS is located above the north east corner of SSS Phase 1, at Tenth Line Road.

3.4 VERIFICATION OF AVAILABLE CAPACITY IN DOWNSTREAM SEWER

The capacity of the downstream 250 mm diameter sewer to existing sanitary manhole 110A has 34% left capacity base on DSEL sanitary sewer design sheet, which is adequate for the flow assumptions from the proposed site as noted above. The servicing pipe capacity is capable to handle the estimated peak sanitary flow rate of 1.01 L/s for the proposed development site. Please refer to sanitary sewer design sheet in Appendix C.

3.5 CALCULATIONS FOR NEW SANITARY SEWER

The 200 mm diameter sanitary service from the sanitary manhole 100 to the building will have a slope of 1.0 %, and a capacity of 32.80 l/s, with a velocity of 1.04 m/s. The 200 mm diameter sanitary service from the sanitary manhole 100 to sanitary manhole 101 have a slope of 0.50%, and a capacity of 23.19 l/s with a velocity of 0.74 m/s. And it will be the same from sanitary manhole to existing sanitary manhole 402A on Sweetvalley Drive. The servicing pipe capacity exceeds the estimated peak sanitary flow rate of 1.01 L/s for the proposed development site.

3.6 DESCRIPTION OF PROPOSED SEWER NETWORK

The proposed sanitary sewer network on site will consist of a 200 mm diameter building service, and two new 1200 mm diameter manholes.

4 SITE STORM SERVICING

4.1 EXISTING CONDITION

The subject site is located within the McKinnon's Creek Watershed and is subject to regulations of the South Nation Conservation (SNC).

The site is currently undeveloped, consisting of grass covered partially agricultural lands and storage warehouse with McKinnon's Creek to the west of the property. The site is sloping from north to south and slightly below the grade of Sweetvalley Drive.

The existing Avalon West SWM Facility, originally designed to service Avalon West Neighbourhood 5, north of Summerside Lands, was revised to accommodate SSW Phases 1, 2 and 3. An expansion of the existing Avalon West SWM Facility is proposed to service SSS Phase 1 and the subject site.

4.2 ANALYSIS OF AVAILABLE CAPACITY IN PUBLIC INFRASTRUCTURE

Using the Rational Method, with coefficient of 0.25 for pervious areas, 0.90 for asphalt and concrete pavement, and 1.0 for building roof, and a 10-minute time of concentration, results in an estimated 2-year flow of 186.51 l/s from this area. The receiving 900 mm diameter storm sewer has been designed with the capacity to accept 567 l/s from the school site. Capacity in the minor system is not a concern.

4.3 DRAINAGE DRAWING

Drawing C03 shows the detail site sewer network. Drawings C02 provides proposed grading and drainage, and include existing grading information. Drawing C05 and C06 provides a pre and post-construction drainage sub-area plan. Site sub-area information is also provided on the storm sewer design sheet attached in Appendix C.

4.4 WATER QUANTITY CONTROL OBJECTIVE

The water quantity objective for the site is to limit the flow release to 186.51 l/s. Excess flows above this limit for the school site up to those generated by the 100 year storm event from drainage on the school site are temporarily stored on site.

No provision is required on the school's site to accommodate any flow from the adjacent lands. All flows exceeding the defined minor system capacity and on-site storage capability will enter the major system, with overflow to the City right of way, on the north boundaries of the site.

The maximum overland runoff spill elevation for this site is 87.25, and a 180 mm dia. circular plate ICDs are proposed to be used on the outlet inside CBMH108 to restrict the flow rate leaving the site to 119.27 l/s at 3.11 m head, based on the 100 year elevation of 87.20. In theory, the runoff water will be detained on site up to the 100-yr rainfall event, and for those scenarios exceeding 100-yr rainfall event, the runoff water will be discharged offsite once all the available storage areas have reached their maximum capacities. The school site can provide a total of 163.69m³ of surface storage volume, but the required storage for 100-yr will be only 124.26 m³. The ponded water will not reach the max spill elevation under 100 year and lesser events. The site has more storage capacity than required as a result of the grading design. This will allow extra detention of water on the site during extreme events, and will reduce stress on the downstream stormwater management pond. If rain falls at a rate higher than the soccer field soil can absorb, then there will be surface ponding at the designated locations shown on the drawings. If the soccer field and landscaped areas allow for infiltration, the available surface storage volume will be further increased. In theory, the use of lower runoff coefficients for landscaped surfaces already accounts for a certain degree of absorption in these areas.

4.5 WATER QUALITY CONTROL OBJECTIVE

The site is not required to achieve water quality objectives. Water quality objectives are achieved through downstream works as noted in the DSEL Design Brief.

4.6 DESIGN CRITERIA

The stormwater system was designed following the principles of dual drainage, making accommodation for both major and minor flow.

Some of the key criteria include the following:

• Design Storm (minor system)	1:2 year return (Ottawa)
• Rational Method Sewer Sizing	
• Initial Time of Concentration	10 minutes
• Runoff Coefficients	
Landscaped Areas	C = 0.25
Playground Mulch Areas	C = 0.40
Gravel Areas	C = 0.75
Asphalt/Concrete	C = 0.90
Traditional Roof	C = 1.00
• Pipe Velocities	0.80 m/s to 6.0 m/s
• Minimum Pipe Size	250 mm diameter (200 mm CB Leads and service pipes)

4.7 PROPOSED MINOR SYSTEM

The detailed design for this site will maintain the existing storm sewer network to Sweetvalley Drive and Pewee Place intersection of the development site. The drainage system consists of a series of manholes, catchbasins and storm sewers leading to the outlet manhole STMH110 at the north of the site. All drainage areas on the site are collected in the site piped drainage system.

It is also customary for larger buildings to be provided with piped storm services for roof drainage. There are no downspouts proposed. Separate outlet pipes are provided for foundation drains and roof drains, and therefore roof drainage will not negatively impact the foundation. The storm services are connected to the storm sewer downstream of CBMH108 which is downstream of the controlled flow point, ensuring an unobstructed flow for these areas.

Using the above noted criteria, the existing on-site storm sewers were sized accordingly. A detailed storm sewer design sheet and the associated post development storm sewer drainage area plan are included in Appendix C.

4.8 STORMWATER MANAGEMENT

The subject site will be limited to a release rate of 186.51 l/s, this will be achieved through the inlet control devices at the downstream of CBMH108.

Flows generated that are in excess of the site's allowable release rate will be stored on site in surface storage areas or by the use of roof top storage and gradually released into the minor system so as not to exceed the site's allocation.

The maximum surface retention depth of the developed areas will be limited to 200mm during a 1:100 year event. Maximum ponding levels are 250mm prior to spill over. The maximum ponding elevation is 87.25m, which is well below the building ground floor level of 87.85m.

No surface ponding will occur during a 2 year event, and only minimal ponding will occur during a 5 year event.

Overland flow routes will be provided in the grading to permit emergency overland flow from the site. The overflow routes will eliminate any increase in ponding depth for events exceeding 100 years.

At certain locations within the site, the opportunity to store runoff is limited due to grading constraints and building geometry. These locations are located at the perimeter of the site where it is necessary to tie into public boulevards and existing property line elevation, and it is not always feasible to capture or store stormwater runoff.

The site grading and ponding has been designed to control water generated during the 1:100-year event, with no overflow leaving the site at this control level. Please refer to the SWM Calculations in Appendix C.

4.9 INLET CONTROLS

As noted in previous sections, the maximum allowable release rate for the 1.747 Ha site is 186.51 L/s.
As noted in Section 4.8, small portion of the site will be left to discharge to the right of way and existing property line at an uncontrolled rate.

$Q_{(uncontrolled)}$	= 2.78 x C x I_{100yr} x A	where:
C	= 0.31 (Weighted average post-development C)	
I_{100yr}	= Intensity of 100-year storm event (mm/hr)	
	= 1735.688/((Tc+6.014)^(0.82)); where T_c = 10 minutes	
A	= Area = 0.236 Ha	

Therefore, the uncontrolled release to the right of way can be determined as:

= 43.30 L/s

The maximum allowable release rate from the remainder of the site can then be determined as:

$Q_{(max\ allowable)} = Q_{(total\ allowable)} - Q_{(uncontrolled)}$

= 186.51 L/s – 43.30 L/s

= **143.21 L/s**

Based on the flow allowance at the outlet location, CBMH108, inlet control devices (ICD) were chosen in the design. The design of the inlet control device is unique to the associated drainage areas and is determined based on a number of factors, including hydraulic head and allowable release rate. The inlet control device will be designed according to the manufacturer's design charts. The restrictions will cause the on-site catchbasins and manholes to surcharge, generating surface ponding in the parking and landscaped areas. Ponding locations and elevations are summarized on the drainage areas plan C06.

4.10 ON-SITE DETENTION

Any excess storm water up to the 100-year event is to be stored on-site in order to not surcharge the downstream municipal storm sewer system. Detention will be provided in parking and landscape areas and building rooftops, where feasible. As previously noted, the volume of storage is dependent on the characteristics of each individual drainage area. It should be

noted that greater than 0.30 m of vertical separation has been provided from all maximum ponding elevations to lowest building openings.

The following Table summarizes the on site storage requirements during the 1:100-year events.

Table 4-1: On-Site Storage Requirements

Total Area (Ha)	Location	Controlled/ Uncontrolled	Runoff Coefficient		Outlet Location	Total Storage Provided (m^3)	100-Year Controlled	
			2 & 5 Year	100 Year			Restricted Flow (L/s)	Required Storage (m^3)
1.297	Surface	Controlled	0.44	0.51	CBMH108	163.69	119.27	124.69
0.231	Building Roof	Controlled	1.00	1.00	STMH110	80.00	17.64	74.44
0.236	R.O.W./Property	Uncontrolled	0.31	0.37			43.30	0
TOTAL						**243.69**	**180.21**	**198.17**

In all instances the required storage is met with surface ponds which retain the stormwater and discharge at the restricted flow rate to the sewer system. Refer to the grading plan for storage information.

The following Table summarizes the inlet control devices to be utilized on the site. ICD pre-set flow curves can be found in Appendix C.

Table 4-2: ICD Type

Structure ID	PROPOSED ICD			
	100-YR Head	Flow (L/s)	Type	OUTLET DIA.
CBMH108	3.11	119.27	180 mm Dia. Circular ICD	450 mm Dia. CONC.

As demonstrated above, the site uses new inlet control device to restrict the 100 year storm event to the criteria approved by the City of Ottawa. Restricted stormwater will be contained onsite by utilizing surface ponding storage. In the 100 year event, there will be no overflow off-site from restricted areas.

The sum of restrictions on the site is 180.21 L/s, which is less than the maximum allowable release of 186.51 L/s noted in Section 4.9.

4.11 WATERCOURSES

The major and minor system flows will be conveyed through the internal network, outletting to Sweetvalley Drive, and ultimately outlet to the Expanded Avalon SWM Pond Facility, where they are treated for an Enhanced Level of Protection (80% TSS removal) prior to release to McKinnon's Creek.

4.12 PRE AND POST DEVELOPMENT PEAK FLOW RATES

Pre and post development peak flow rates for the impacted areas of the site have been noted in storm sewer design sheet.

4.13 IMPACTS TO RECEIVING WATERCOURSES

No significant negative impact is anticipated to downstream receiving watercourses due to proposed quantity and quality control measures, the separation of the site from the eventual receiving watercourse as a result of discharge through City owned sewers, and the expanded Avalon SWM Pond Facility.

5 SEDIMENT AND EROSION CONTROL

5.1 GENERAL

During construction, existing storm sewer system can be exposed to sediment loadings. A number of construction techniques designed to reduce unnecessary construction sediment loadings will be used including the following.

- Silt sack will remain on open surface structures such as manholes and catchbasins until these structures are commissioned and put into use.
- Installation of silt fence, where applicable, around the perimeter of the proposed work area.
- The installation of straw bales within existing drainage features surrounds the site.
- Bulkhead barriers will be installed in the outlet pipes.

During construction of the services, any trench dewatering using pumps will be fitted with a "filter sock." Thus, any pumped groundwater will be filtered prior to release to the existing surface runoff. The contractor will inspect and maintain the filter sock as needed including sediment removal and disposal.

All catchbasins, and to a lesser degree, manholes, convey surface water to sewers. Consequently, until the surrounding surface has been completed, these structures will be covered to prevent sediment from entering the minor storm sewer system. These measures will stay in place and be maintained during construction and build-out until it is appropriate to remove them.

During construction of any development both imported and native soils are placed in stockpiles. Mitigative measures and proper management to prevent these materials entering the sewer system are needed.

During construction of the deeper watermains and sewers, imported granular bedding materials are temporarily stockpiled on site. These materials are however quickly used up and generally placed before any catchbasins are installed.

Refer to the Erosion and Sedimentation Control Plan C04 provided in Appendix E.

6 APPROVAL AND PERMIT REQUIREMENTS

6.1 GENERAL

The proposed development is subject to site plan approval and building permit approval.

No approvals related to municipal drains are required.

No permits or approvals are anticipated to be required from the Ontario Ministry of Transportation, National Capital Commission, Parks Canada, Public Works and Government Services Canada, or any other provincial or federal regulatory agency.

7 CONCLUSION CHECKLIST

7.1 CONCLUSIONS AND RECOMMENDATIONS

WSP was retained by Edward J. Cuhaci and Associates Architects Inc. to provide this Servicing and SWM report in support of the Site Plan Application for the subject site Avalon III French Catholic Elementary School and planned two storey school building therein. The services investigated were water supply, wastewater servicing, and stormwater conveyance.

The water demand was calculated as 1.55 L/s peak hour domestic demand and 133 L/s max day plus fire flow. Per coordination with the City for the supply watermain boundary conditions, a 203mm watermain stub from Sweetvalley Drive, it was confirmed the existing system has sufficient capacity to supply the domestic and fire demands within system pressure limits.

The sanitary sewer demand was calculated as 0.86 L/s peak demand. A downstream capacity check has been completed; the downstream system has sufficient capacity to receive the proposed demand.

The site will be required by the city to limit the discharge rate of the stormwater to the pre-development 2yr storm rate, storing the stormwater up to the post-development 100yr storm. Estimates of the runoff rates lead to an approximate maximum site discharge rate of 180.21 L/s, with a required storage for approximately 198.17 m^3.

Therefore, it is confirmed the existing infrastructure is sufficient to support the proposed development. It should be noted that all demand calculations are estimates based on conceptual architectural plans and are subject to change during the design phase.

7.2 COMMENTS RECEIVED FROM REVIEW AGENCIES

This is the first submission, no city comments.

APPENDIX

A

- PRE-CONSULTATION MEETING NOTES
- DSEL DRAWINGS FOR SSS PHASE 1
- ARCHITECTURAL SITE PLAN
- TOPO SURVEY

From: Murshid, Shoma
Sent: September 06, 2022 10:50 AM
To: Zofia Jurewicz <zofiaj@cuhaci.com>; Paquette Planning Associates Ltd. <paquetteplanning@sympatico.ca>
Cc: Rasool, Rubina <Rubina.Rasool@ottawa.ca>; Giampa, Mike <Mike.Giampa@ottawa.ca>; McAlpine, Anissa <anissa.mcalpine@ottawa.ca>; Ippersiel, Matthew <Matthew.Ippersiel@ottawa.ca>; Richardson, Mark <Mark.Richardson@ottawa.ca>; James Holland <jholland@nation.on.ca>; Rehman, Sami <Sami.Rehman@ottawa.ca>; Luc Poulin <Poulil@ecolecatholique.ca>; Denis Chabot <chabod@ecolecatholique.ca>; Sclauzero, Cass <cass.sclauzero@ottawa.ca>
Subject: 2666 Tenth Line Road - CECCE Elementary School on Claridge Lands in Mer Bleue Expansion Urban Expansion Area 10

Good morning Dan and Zofia,

Thank you for meeting with us on August 23, 2022 to review your concept plan (2 attachments) for a one-storey elementary school for CECCE.

This proposal triggers a Zoning By-law Amendment (Major) and a New, Complex Site Plan Control development review applications.

If you do end up submitting both development applications concurrently, there will be a 10% deduction in the planning fee component for both applications.

The **Zoning By-law Amendment** category being triggered is Major and is public-consultation based. The submission fee for this application is $22,472.80 + an initial Conservation Authority Fee of $400.00. For the Zoning By-law Amendment to be deemed complete at the time of submission, a complete application form, fees and the following plans, studies and documentation will be required (all in PDF format):

Concept Plan, showing proposed uses and landscaping and/or Site Plan

Planning Rationale, including Design Statement

Survey Plan

Topographical Survey Plan

Elevations

Geotechnical Report

Servicing & Stormwater Management Reports

Phase 1 ESA (Phase 2 ESA if required)
Tree Conservation Report
EIS
Transportation Impact Assessment

Noise Study

For the **Site Plan Control application**, the category being triggered is 'Complex (Manager Approval, Public Consultation) and the submission fee for this is $49,964.88 + Initial Engineering Design Review and Inspection Fee (based on a sliding scale for the value of the Infrastructure and Landscaping) and an initial Conservation Authority Fee of $1,065.00. For the Site Plan Control application to be deemed complete at the time of submission, a complete application form, fees and the following plans and studies will be required (all in PDF format):

Site Plan

Landscape Plan/Tree Conservation Report (can be combined)

Site Servicing Plan

Survey Plan

Topographical Survey Plan

Planning Rationale, including design statement

Erosion and Sediment Control Plan (can be combined with the Grade Control and Drainage Plan

Stormwater Management Report (can be combined with the Site Servicing Report)

Grade Control and Drainage Plan

Site Servicing Report

Stormwater Management Report

Geotechnical Report

Phase 1 ESA (Phase 2 if required)

TIA

Noise Study

EIS

Floor Plans

Elevations

City's General Urban Planning Comments:

There is a Landowner's Agreement and Cost-Sharing Agreement in place, "Area 10 Funding Agreement & CSA". The trustee is Soloway Wright's Ursula Melinz. The landowners within this agreement must provide a clearance letter for this zoning by-law amendment application and site plan control application prior to their approvals.

Zoning By-law Amendments

If a complete application is received by no later than the day before the new Official Plan is adopted (October 27, 2021), it will be processed on the basis of existing Official Plan policy provided it is consistent with the 2020 Provincial Policy Statement.

For complete applications received after the day before the new Official Plan is adopted on October 27, 2021), but before Ministry approval of the Official Plan, any reports going forward to Committee and Council under this circumstance must be evaluated against the

existing Official Plan and must also include an evaluation of the application against the Council approved new Official Plan (and the new Secondary Plan, where applicable). In the period between Council approval of the New OP and the Minister's approval of the New OP, City staff will apply whichever provision, as between the Current and New OP, is more restrictive.

Zoning By-law amendments that conform to the new Official Plan but not the current Official Plan

Council can pass the by-law after the new Official Plan is adopted but it only comes into force if the relevant policies authorizing it are approved by the Minister. Pursuant to the Planning Act, section 24, subsections (2) and (2.1) Council may pass a by-law that does not conform with the official plan but will conform to the new Official Plan once it comes into effect. If the new Official Plan does not come into effect the by-law has no force and effect.

Please note there is an approved Mer Bleue Urban Expansion Area 10 Community Design Plan (CDP). The Mer Bleue Urban Expansion Area 10 Community Design Plan (CDP) has been prepared by the Mer Bleue Land Owners Group (MBLOG), in collaboration with the City of Ottawa. The CDP is intended to demonstrate how development of the Mer Bleue Urban Expansion Area 10 (MBUEA) will achieve the requirements of the Official Plan. The CDP also provides a planning framework for the implementation of Official Plan policy through the subsequent development approvals process and will therefore be used as a guide for the preparation and review of future applications for development. N.B. There is also an EMP and MSS for this same area.

City Urban Design Comments:

- PRUD Staff support the decision to highlight the corner of the site with a prominent architectural feature.
- Explore the possibility of eliminating the need for the small parking lot on Sweet Valley Drive. This would reduce the amount of paving along the public frontage and would free up more space for landscaping adjacent to the main entrance.

- Please line the two public frontages with trees.
- Ensure that sidewalks are continuous and uninterrupted across vehicular apertures.
- If possible, please narrow the widths of the vehicular apertures and reduce the turning radii as much and possible. As designed, they may encourage higher speeds.
- If possible, please look to move the bicycle parking (4) closer to an entrance.
- Include a bicycle parking rack near the main entrance on Sweet Valley Drive.
- Please continue to study what the best approach for the interface between the schoolyard and McKinnon's Creek would be. Should the edge be fenced or not? Please be mindful of linking in to the community active transportation network and impacts on adjacencies to the soccer field.
- The full-size soccer field appears very tight where it is located. Consider that part of the game is played outside the boundaries (corner kicks, throw-ins), balls are often kicked out of bounds, and there is space needed for team benches and spectators. Whether fenced or not, consider a landscape buffer where the property abuts the creek to prevent balls from rolling away.
- Consider a formal pathway connecting to the gardens. Otherwise a goat trail will likely form, cutting across the soccer field.
- PRUD staff support the on-street laybys from an urban design perspective.
- An Urban Design Brief is required as a part of your submission. This may be combined with your Planning Rationale report. Please refer to the attached Urban Design Brief Terms of Reference to inform the content of the brief.
- Please reference any design direction in the CDP in the brief and demonstrate how the proposal conforms to its policies.
- This application is not subject to review by the Urban Design Review Panel.

City Engineering Comments:

As mentioned, the applicant may be required to pay into the N5 Pond separately as there are no SWM DC charges. Gary Baker has confirmed the site is not subject to SWM DC charges.

Otherwise, please see attached for engineering comments.

Note, there is a moratorium on newly paved roads. Check attached engineering comments for further information.

City Transportation and Noise Comments:

*A 0.5 metre conveyance from the northern perimeter of the lot line is required in order to add it to the already conveyed 2.5 metre MUP land obtained directly north from Mattamy.

A TIA is warranted- please proceed to scoping.

The application will not be deemed complete until the submission of the draft step 2-4, including the functional draft RMA package (if applicable) and/or monitoring report (if applicable).

Although a full review of the TIA Strategy report (Step 4) is not required prior to an application, it is strongly recommended.

Synchro files are required at Step 4.

ROW protection on Tenth Line Road is 37.5 m.

A Noise Impact Study is required

Clear throat requirements as per TAC guidelines.

Please note that all new applications (pre-consultation meetings dated after March 3, 2021) must use the NEW TRANS Trip Generation Manual when forecasting site generated trips using this manual.

The TRANS committee (a joint transportation planning committee serving the National Capital region) finalized a new manual early in March 2021. The document will be available in French and English on the TRANS website http://www.ncr-trans-rcn.ca/surveys/2009-trip-generation.

The new manual has simplified the conversion from vehicle trips to person trips and then trips by modal share. The City has also developed a spreadsheet that will apply the factors of location and building type to quickly provide the existing trip numbers by mode share. This spreadsheet has been attached.

*Latest construction plans for ROW of Tenth Line Road and ROW of Sweet Valley Drive - Please contact Bill Harper, Program Manager (SAM) / City Surveyor at bill.harper@ottawa.ca, or call 613-580-2424, ext. 21083.

A layby on Sweet Valley is possible but I'll need a design (RMA report, design submission, municipal consent). A layby on Tenth Line will not be supported.

A MUP should be at least 3m will be required along Sweet Valley Drive.

City Forestry Comments:

Planning Forester - TCR requirements:

1. A Tree Conservation Report (TCR) must be supplied for review along with the suite of other plans/reports required by the City

    a. an approved TCR is a requirement of Site Plan approval.

    b. The TCR may be combined with the EIS provided all information is supplied

2. Any removal of privately-owned trees 10cm or larger in diameter, or city-owned trees of any diameter requires a tree permit issued under the Tree Protection Bylaw (Bylaw 2020 – 340); the permit will be based on an approved TCR and made available at or near plan approval.

3. The TCR must contain 2 separate plans:

    a. Plan/Map 1 - show existing conditions with tree cover information

    b. Plan/Map 2 - show proposed development with tree cover information

    c. Please ensure retained trees are shown on the landscape plan

4. the TCR must list all trees on site, as well as off-site trees if the CRZ extends into the developed area, by species, diameter and health condition

5. please identify trees by ownership – private onsite, private on adjoining site, city owned, co-owned (trees on a property line)

6. If trees are to be removed, the TCR must clearly show where they are, and document the reason they cannot be retained

7. All retained trees must be shown, and all retained trees within the area impacted by the development process must be protected as per City guidelines available at Tree Protection Specification or by searching Ottawa.ca

    a. the location of tree protection fencing must be shown on the plan

    b. show the critical root zone of the retained trees

8. the City encourages the retention of healthy trees; if possible, please seek opportunities for retention of trees that will contribute to the design/function of the site.

9. For more information on the process or help with tree retention options, contact Mark Richardson mark.richardson@ottawa.ca or on City of Ottawa

LP tree planting requirements:

For additional information on the following please contact tracy.smith@Ottawa.ca

Minimum Setbacks

- Maintain 1.5m from sidewalk or MUP/cycle track or water service laterals.
- Maintain 2.5m from curb
- Coniferous species require a minimum 4.5m setback from curb, sidewalk or MUP/cycle track/pathway.
- Maintain 7.5m between large growing trees, and 4m between small growing trees. Park or open space planting should consider 10m spacing, except where otherwise approved in naturalization / afforestation areas. Adhere to Ottawa

Hydro's planting guidelines (species and setbacks) when planting around overhead primary conductors.

Tree specifications

- Minimum stock size: 50mm tree caliper for deciduous, 200cm height for coniferous.
- Maximize the use of large deciduous species wherever possible to maximize future canopy coverage
- Tree planting on city property shall be in accordance with the City of Ottawa's Tree Planting Specification; and include watering and warranty as described in the specification (can be provided by Forestry Services).
- Plant native trees whenever possible
- No root barriers, dead-man anchor systems, or planters are permitted.
- No tree stakes unless necessary (and only 1 on the prevailing winds side of the tree)

Hard surface planting

- Curb style planter is highly recommended
- No grates are to be used and if guards are required, City of Ottawa standard (which can be provided) shall be used.
- Trees are to be planted at grade

Soil Volume

- Please document on the LP that adequate soil volumes can be met:

Tree Type/Size	Single Tree Soil Volume (m3)	Multiple Tree Soil Volume (m3/tree)
Ornamental	15	9
Columnar	15	9
Small	20	12
Medium	25	15
Large	30	18
Conifer	25	15

Please note that these soil volumes are not applicable in cases with Sensitive Marine Clay.

Sensitive Marine Clay

- Please follow the City's 2017 Tree Planting in Sensitive Marine Clay guidelines

Tree Canopy Cover

- The landscape plan shall show how the proposed tree planting will replace and increase canopy cover on the site over time, to support the City's 40% urban forest canopy cover target.

- At a site level, efforts shall be made to provide as much canopy cover as possible, through tree planting and tree retention, with an aim of 40% canopy cover at 40 years, as appropriate.

- Indicate on the plan the projected future canopy cover at 40 years for the site.

City Environmental Policy Comments (these comments have not been updated since October 2021):

The proposed development should confirm the servicing requirements and development approvals established as part of the subdivision approval and the master servicing study to the north.

If the proposal is permitted to proceed (as part of the subdivision to the north), then an EIS will be required for either a zoning amendment or a site plan control application. This stretch of McKinnon's creek is identified as part of the NHS, as per OP Schedule L.

The EIS will address the following items:

-explore the hazard lands and floodplain required for that stretch of the McKinnon's Creek.

-draw recommendations from the EMP (Mer Bleue Urban Expansion Study Area – Environmental Management Plan, Morrison-Hershfield Ltd, Dec2017)

-draw recommendations from the CDP (Mer Bleue Expansion Area – Community Design Plan, IBI Group, Jun 2017)

-provide recommendations for revegetation and enhancements to the riparian areas along McKinnon's Creek

-potential significant habitat for threatened or endangered species

-provide recommendations to increase energy and water efficiency based on landscaping and layout, as per OP 4.9

-if there is substantial glass proposed on the design, recommend drawing design elements from the City's bird-safe design guidelines (Sept 2020)

I would encourage the applicant to consult with South Nation Conservation Authority to determine if any permits or approvals are required under their regulations.

I would also recommend consulting with the engineer's report for the municipal drain because there are concerns about how the stormwater management for this area will impact and potentially contribute to flooding downstream.

City Parks Comments:

- The applicant and land owners should be aware that parkland dedication will continue to be tracked through the development application process. Parkland dedication requirements remain unchanged from that detailed in the Community Design Plan.
- Please note, a school is exempted from parkland dedication as per our Parkland dedication by-law (as approved by Council Aug 31, 2022) "where the school provides for the students' outdoor recreational needs on-site at the time of development and maintains sufficient outdoor recreational space on-site at the time of redevelopment"
- The Demonstration Plan in the Secondary plan, and the CDP show the Ecole Catholique being co-located with a neighbourhood park, south of Wall Road. It is unknow at this time, if there would be further opportunities to co-locate a school with the park block south of Wall Road. Please keep Park staff informed as development applications proceed in order for staff to comment on the location of the neighbourhood park block south of Wall Road.
- The opportunity to co-location school blocks and park blocks should continue to be sought by development applications within the Mer Bleue community expansion.

South Nation Conservation Authority Comments:

Here are my comments for the August 23rd meeting concerning the French Catholic school proposal. They should be read in conjunction with our previous comments from October 2021 (FOUND below this section).

Natural Heritage

- The Environmental Management Plan (EMP) indicates (8.1.2) that : For the protection of the common aquatic habitat observed in McKinnon's Creek, a setback consisting of the greater of 15 m from the top-of-slope or 30 m from the normal high water mark in the urban area (which may be refined through further study during preparation and review of the draft plan of subdivision), as recommended in the Official Plan, Section 4.7.3.2 and 4.7.3.6, and is identified on Figure 4.4.
- It is our understanding that this area will become a separate parcel (ie., separated from the school parcel) and placed in a restrictive Zone, as done for the subdivision to the north of this property. It is our understanding that the setback will include a Mixed Use Path (MUP) but that access to the creek will be limited to allow the riparian buffer to function.
- A landscaping plan for the full McKinnon's Creek corridor is required by the EMP; however, should this development proceed prior to the completion of this plan, a landscaping plan that meets the objectives of the EMP will be required for this property.
- An Environmental Impact Assessment is required for development adjacent to fish habitat. In addition, a headwater feature has been identified (Drain 14) along the north property boundary. The management recommendations for the headwater feature (outlined as an appendix to the EMP) should be addressed in the EIS. The Conservation Partners will provide a review of the EIS.
- The Conservation Partners support the development of a resource (similar to a Homeowner's Guide) that outlines the ecological significance, restoration and enhancement works and best management practices for the McKinnon's Creek Corridor. This could help to use raise awareness amongst school studies/staff in the future. The Conservation Partners can provide similar resources and background studies and can assist in the review of the resource.

Stormwater Management

- The Conservation Partners do not object to an additional outlet to McKinnon's Creek in place of directing stormwater into existing infrastructure and ultimately to the existing stormwater pond if it can be shown to not have negative impacts on flooding and erosion, upstream and downstream of the outlet.
- Should the option for a new outlet be pursued, it may necessitate a revision to the McKinnon's Creek 100-year floodplain study, which will require review and approval from South Nation Conservation. The applicant may submit a scaled site plan and request a preliminary review to assess whether the change in land use differs from the SNC model, requiring further analyses.

- Should the option for a new outlet be pursued, the applicant will be responsible for stormwater treatment of runoff quality and quantity. The design must demonstrate a 80% TSS removal. The quantity must meet City of Ottawa requirements. The design package should include at a minimum, a report demonstrating how the quality/quantity targets will be achieved, a grading and drainage plan, and a sediment and erosion control plan. The Conservation Partners will provide a technical review.

Conservation Authority Regulation 170/06

- Any interference with a watercourse/headwater feature, including an outlet to McKinnon's Creek, will require a permit and restrictions may apply.
- There is a 100-year floodplain contained within the banks of McKinnon's Creek. The elevation of the floodplain at the north end of the property is 84.38 meters above sea level. Any development within or 15m adjacent to this elevation will require a permit and restrictions may apply. It is anticipated that this area will fall within the McKinnon's Creek Corridor and a restrictive Zone.

SNCA – October 2021 follow-up notes:

- The development should implement the direction approved though the Council-approved Master Servicing Study (MSS) and Environmental Impact Statement, prepared for the Mer Bleue Expansion Lands.

Environmental

- The EMP Section 8.1.1 requires a Planting Plan at the subdivision stage to enhance the woody vegetation cover in McKinnon's Creek corridor where needed. Section 8.1.1 (final point, pg98) also indicates that a detailed design of the McKinnon's Creek will be undertaken as a single integrated design from the Avalon South pond outlet to the downstream extent of the proposed lowering just upstream of Navan Road. The planting plan for the subject property should be integrated with the detail design for the corridor.
- For the protection of aquatic habitat in McKinnon's Creek, the EMP Section 8.1.2 recommends a setback consisting of 15m from the top-of-slope or 30m from the normal high water mark, as identified on Figure 4.4 of the EMP. This setback should be clearly delineated on all plans.
- Figure 3-6 and Table 3.3 identify the drainage features along the north boundary of the parcel as Drain 14, and provides a management recommendation of 'mitigation'. The

feature should be discussed within the Environmental Impact statement, including how the management recommendation will be implemented.
- An Environmental Impact Statement and Landscaping Plan are recommended for the subject property to demonstrate how the recommendations of the EMP will be satisfied.

Stormwater Management

- Should stormwater be directed towards the Neighbourhood 5 stormwater pond, it must be demonstrated that the pond has capacity.
- The stormwater design should include at a minimum, a report demonstrating how water quality and quantity treatment standards will be achieved, a grading and drainage plan, and a sediment and erosion control plan.
- Note that when stormwater outlets to approved municipal infrastructure, the Conservation Partners do not undertake a technical review; however, we request to be included in the circulation of the stormwater design to confirm.
- Any modifications to the stormwater pond, including an alteration to the outlet or a change in outflows, will require a technical review by South Nation Conservation.
- Any changes to the outflow may require a revision to the McKinnon's Creek 100-year floodplain analysis, along with a technical review of the revision by South Nation Conservation.
- Likewise, should drainage be directed towards McKinnon's Creek directly via uncontrolled flow, a revision to the McKinnon's Creek 100-year analysis and a technical review by South Nation Conservation may be necessary.
- Any drainage from the subject site must demonstrate that there is legal and sufficient outlet for the additional flows. A Municipal Drain petition is currently underway to designate McKinnon's Creek a municipal drain.

Conservation Authority Regulations

- Any interference with a watercourse, including a headwater drainage feature (Drain 14, noted above) and an alteration to a stormwater outlet, may require a permit under O. Reg. 170/06 and restrictions may apply.

Further items to consider for both site plan control and zoning amendment are:

- Bird-safe safety design guidelines are now in effect.

https://ottawa.ca/en/city-hall/public-engagement/projects/bird-friendly-design-guidelines

https://ottawa.ca/en/city-hall/public-engagement/projects/bird-friendly-design-guidelines#bird-friendly-design-guidelines

- Consider the reduction of energy and water demands within your development proposal through lot layout and landscaping, as outlined in the OP Section 4.9.

- Plant locally appropriate native species along the southern and western boundaries of the property and along the parking lots. This will offer shaded parking spots and reduce the urban heat island effect.

- Staff would caution a reduction in the setbacks abutting the public realm. It still needs to be demonstrated that street tree planting **of canopy shade trees** can be accommodated through the site and particularly and along the public RoWs (Sweetvalley Drive and Tenth Line Road).

Minimum Drawing and File Requirements - All Plans:

Plans are to be submitted on standard **A1 size** (594mm x 841mm) sheets, utilizing an appropriate Metric scale (1:200, 1:250, 1:300, 1:400, or 1:500).

With all submitted hard copies provide **individual** PDF of the DWGs and for reports please provide one PDF file of each report. **All PDF documents are to be unlocked and flattened.**

Closing comments:

In order to sever the lands, please seek a pre-consultation with a Committee of Adjustment Planner, Cass Sclauzero at cass.slauzero@ottawa.ca or at 613-580-2424-27597.

Best wishes,

Shoma Murshid, MCIP, RPP

(she/ her/ elle)
File Lead, Planner II

Responsable de dossier, urbaniste II
City of Ottawa/ Ville d'Ottawa
Development Review (Suburban Services, East)/ Examen des projets d'aménagement (Services suburbains Est)
Planning, Real Estate and Economic Development Department / Direction générale de la planification, des biens immobiliers et du développement économique

110 Laurier Avenue West, 4th Floor, Ottawa ON K1P 1J1/ 110, avenue Laurier Ouest, 4e étage, Ottawa (Ontario) K1P 1J1
Mail Code/ Code de courrier : 01-14
Tel/ Tél: (613) 580-2424 ext. 15430
Fax/ Téléc. : (613) 580-4751

e-mail/ courriel : shoma.murshid@ottawa.ca

www.ottawa.ca

TOPOGRAPHIC PLAN OF SURVEY OF

PART OF LOT 5
CONCESSION 11
Geographic Township of Cumberland
CITY OF OTTAWA

Surveyed by Annis, O'Sullivan, Vollebekk Ltd.

Scale 1 : 400

16 12 8 4 0 8 16 Metres

Metric

DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

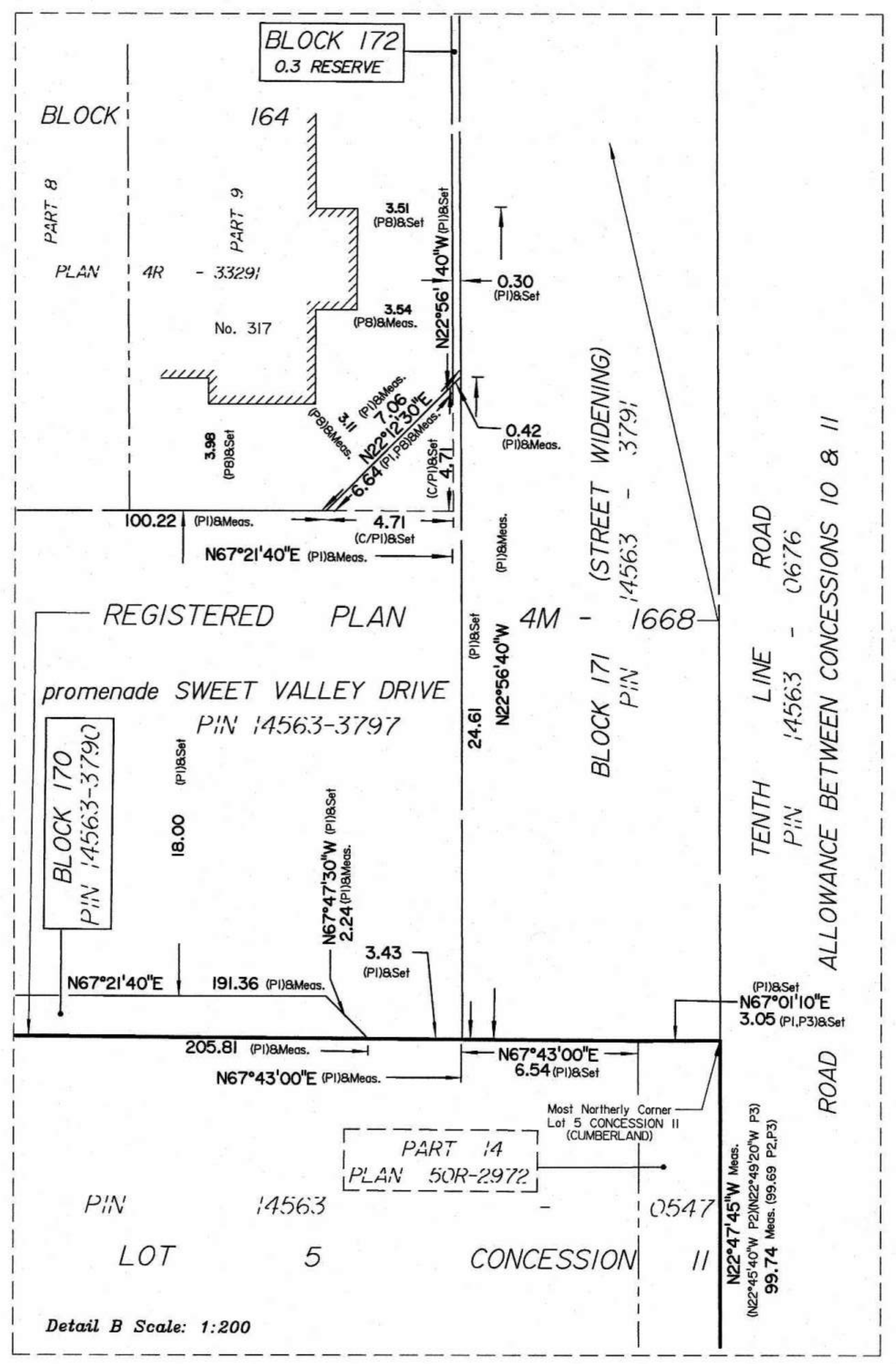
Surveyor's Certificate

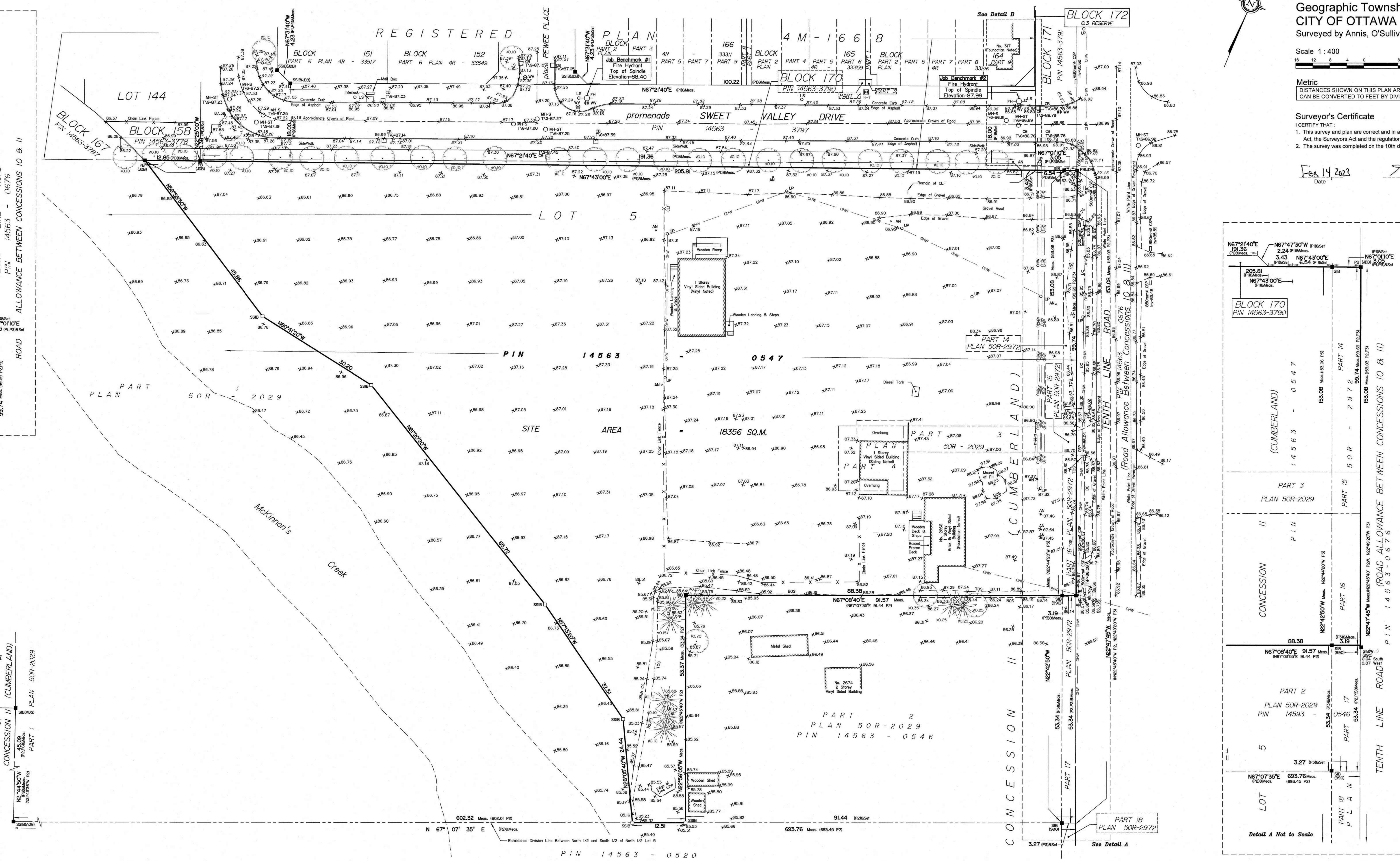
I CERTIFY THAT :

1. This survey and plan are correct and in accordance with the Surveys Act, the Surveyors Act and the regulations made under them.
2. The survey was completed on the 10th day of February, 2023.

Feb 14, 2023
Date

T. Hartwick
Ontario Land Surveyor





Notes & Legend

	Denotes	
—□—	"	Survey Monument Planted
—■—	"	Survey Monument Found
SIB	"	Standard Iron Bar
PB	"	Plastic Bar
Meas.	"	Measured
C/	"	Calculation per
(AOG)	"	Annis, O'Sullivan, Vollebekk Ltd.
(P1)	"	Registered Plan 4M-1668
(P2)	"	Plan 50R-2029
(P3)	"	Plan 50R-2972
(P4)	"	Plan 4R-33517
(P5)	"	Plan 4R-33311
(P6)	"	Plan 4R-32601
(P7)	"	Plan 4R-31218
(P8)	"	Plan 4R-33291
	"	Deciduous Tree
	"	Coniferous Tree
FH	"	Fire Hydrant
WV	"	Water Valve
MH-ST	"	Maintenance Hole (Storm Sewer)
MH-S	"	Maintenance Hole (Sanitary)
OHW	"	Overhead Wires
CB	"	Catch Basin
TB	"	Unidentified Terminal Box
CLF	"	Chain Link Fence
TOS	"	Top of Slope
BOS	"	Bottom of Slope
CSP	"	Corrugated Steel Pipe
DC	"	Ditch Centre
T/G	"	Top of Grate
Inv	"	Invert
	"	Gate
UP	"	Utility Pole
AN	"	Anchor
Ø	"	Diameter
+ 65.00	"	Location of Elevations
+ 65.00	"	Top of Concrete Curb Elevation
———	"	Property Line

Bearings are grid, derived from Can-Net 2016 Real Time Network GPS observations, referenced to Specified Control Points 01919680184 and 019198434761, MTM Zone 9 (76°30' West Longitude) NAD-83 (original).

For bearing comparisons, a rotation of 0°42'10" counter-clockwise was applied to bearings on plan (P2).

Topographic data was collected under Winter Conditions. Snow cover and ice preclude determining location and elevation of some topographical data that is otherwise visible.

ELEVATION NOTES

1. Elevations shown are geodetic and are referred to the CGVD28 geodetic datum.
2. It is the responsibility of the user of this information to verify that the job benchmark has not been altered or disturbed and that it's relative elevation and description agrees with the information shown on this drawing.

UTILITY NOTES

1. This drawing cannot be accepted as acknowledging all of the utilities and it will be the responsibility of the user to contact the respective utility authorities for confirmation.
2. Only visible surface utilities were located.
3. A field location of underground plant by the pertinent utility authority is mandatory before any work involving breaking ground, probing, excavating etc.

ASSOCIATION OF ONTARIO LAND SURVEYORS
PLAN SUBMISSION FORM
V-44962

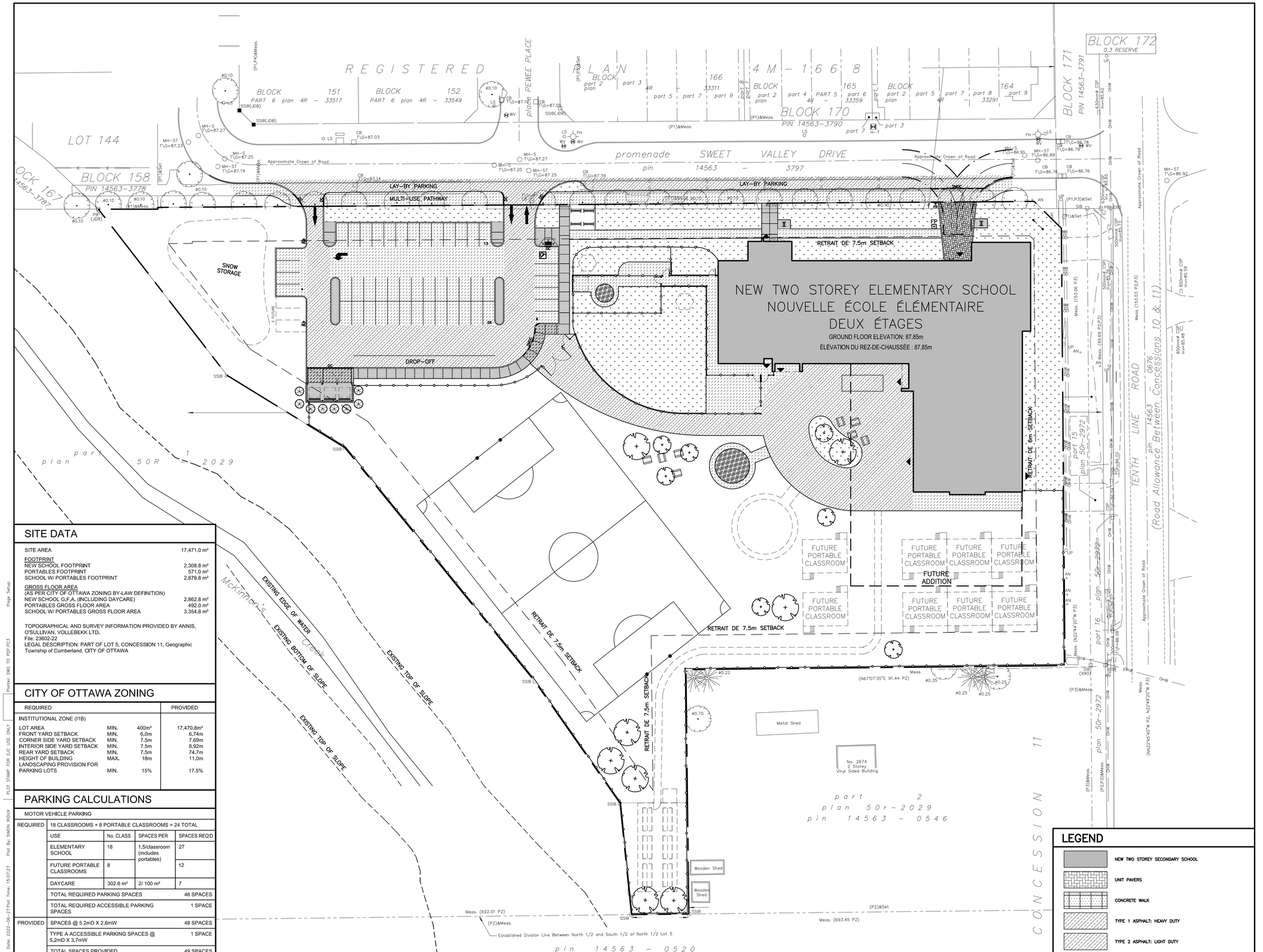
THIS PLAN IS NOT VALID UNLESS IT IS AN EMBOSSED ORIGINAL COPY ISSUED BY THE SURVEYOR In accordance with Regulation 1026, Section 29 (3).

ANNIS, O'SULLIVAN, VOLLEBEKK LTD.
14 Concourse Gate, Suite 500
Nepean, Ont. K2E 7S6
Phone: (613) 727-0850 / Fax: (613) 727-1079
Email: Nepean@aovltd.com

Ontario Land Surveyors

Job No. 23602-22 ECCE PtL t5 C11 CU T D2 | FL

Y:\2022\23602-22 Ecoles Catholiques Centre-Est_ Beside 2680 Tenth Line_TOPO_MA\Drawings\23602-22 ECCE PtL5 C11 CU T D2.dwg



SITE DATA

SITE AREA	17,471.0 m²
FOOTPRINT	
NEW SCHOOL FOOTPRINT	2,308.8 m²
PORTABLES FOOTPRINT	571.0 m²
SCHOOL W/ PORTABLES FOOTPRINT	2,879.8 m²
GROSS FLOOR AREA	
(AS PER CITY OF OTTAWA ZONING BY-LAW DEFINITION)	
NEW SCHOOL G.F.A. (INCLUDING DAYCARE)	2,862.8 m²
PORTABLES GROSS FLOOR AREA	492.0 m²
SCHOOL W/ PORTABLES GROSS FLOOR AREA	3,354.8 m²

TOPOGRAPHICAL AND SURVEY INFORMATION PROVIDED BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD.
File: 23602-22
LEGAL DESCRIPTION: PART OF LOT 5, CONCESSION 11, Geographic Township of Cumberland, CITY OF OTTAWA

CITY OF OTTAWA ZONING

REQUIRED			PROVIDED
INSTITUTIONAL ZONE (I1B)			
LOT AREA	MIN.	400m²	17,470.8m²
FRONT YARD SETBACK	MIN.	6.0m	6.74m
CORNER SIDE YARD SETBACK	MIN.	7.5m	7.69m
INTERIOR SIDE YARD SETBACK	MIN.	7.5m	8.92m
REAR YARD SETBACK	MIN.	7.5m	74.7m
HEIGHT OF BUILDING	MAX.	18m	11.0m
LANDSCAPING PROVISION FOR PARKING LOTS	MIN.	15%	17.5%

PARKING CALCULATIONS

MOTOR VEHICLE PARKING

REQUIRED: 18 CLASSROOMS + 8 PORTABLE CLASSROOMS = 24 TOTAL

USE	No. CLASS	SPACES PER	SPACES REQ'D
ELEMENTARY SCHOOL	18	1.5/classroom (includes portables)	27
FUTURE PORTABLE CLASSROOMS	8		12
DAYCARE	302.6 m²	2/ 100 m²	7
TOTAL REQUIRED PARKING SPACES			46 SPACES
TOTAL REQUIRED ACCESSIBLE PARKING SPACES			1 SPACE

PROVIDED

SPACES @ 5.2mD X 2.6mW	48 SPACES
TYPE A ACCESSIBLE PARKING SPACES @ 5.2mD X 3.7mW	1 SPACE
TOTAL SPACES PROVIDED	49 SPACES

BICYCLE PARKING

REQUIRED

USE	GROSS AREA	SPACES PER	SPACES REQ'D
SCHOOL	2,560.2 m²	1 per 100 m² of gross floor area	26 SPACES
DAYCARE	302.6 m²	1 per 250 m² of gross floor area	2 SPACES
PORTABLES	492 m²	1 per 100 m² of gross floor area	5 SPACES
TOTAL REQUIRED BICYCLE PARKING SPACES			33 SPACES

PROVIDED: TOTAL BICYCLE SPACES PROVIDED — 40 SPACES

LEGEND

- NEW TWO STOREY SECONDARY SCHOOL
- UNIT PAVERS
- CONCRETE WALK
- TYPE 1 ASPHALT: HEAVY DUTY
- TYPE 2 ASPHALT: LIGHT DUTY
- NEW SEED AND TOPSOIL (REFER TO LANDSCAPE DRAWING)
- NEW SOD AND TOPSOIL (REFER TO LANDSCAPE DRAWING)
- MULCH (REFER TO LANDSCAPE DRAWING)
- ENGINEERED WOOD FIBER

1 A001 SITE PLAN
1:400

Plot Date: 2022-06-27 Plot Time: 15:07:27 Plot By: SIMON RIDUK Plotter: DWG To PDF.PC3 PLOT STAMP FOR FOR USE ONLY Page Setup:

Écoles catholiques Centre-Est

ISSUE NO.	REV. NO.	DATE YYYY/MM/DD	ISSUE
0	0	2023/05/16	ISSUED FOR INFORMATION

TRUE NORTH | PLAN NORTH

EDWARD J. CUHACI & ASSOCIATES ARCHITECTS Inc.
171 Slater St, Suite 100, Ottawa, Ontario, K1P 5H7
Fax: (613) 236-1944 Telephone: (613) 236-7135 E-mail: info@cuhaci.com

PROJECT TITLE/TITRE DU PROJET
ÉCOLE ÉLÉMENTAIRE CATHOLIQUE AVALON III
TENTH LINE ROAD
OTTAWA, ON

CONSEIL DES ÉCOLES CATHOLIQUES DU CENTRE-EST
4000, RUE LABELLE, OTTAWA, ON K1J 1A1

DRAWING TITLE/TITRE DU DESSIN
SITE PLAN

SCALE / ÉCHELLE	1:400	PROJ. No 2215	ISSUE No 0 / REV. No 0
DRAWN BY / DESSINÉ PAR	S.P.	DRAWING/DESSIN	A001
CHECKED BY / VÉRIFIÉ PAR	S.R./Z.O.J.		
DATE	SEPTEMBER 2022	FICHIER ACAD FILE: 2215-A001.dwg	

PAVEMENT DESIGN

40mm SUPERPAVE 12.5 ASPHALTIC CONCRETE
50mm SUPERPAVE 19.0 ASPHALTIC CONCRETE
50mm SUPERPAVE 19.0 ASPHALTIC CONCRETE
150mm GRANULAR "A" CRUSHED STONE
600mm GRANULAR "B" TYPE II

PERMISSION REQUIRED FOR WORK ON ADJACENT LANDS

NOTE RE: THRUST BLOCK BEDDING
TO INCREASE THE VERTICAL ALLOWABLE BEARING CAPACITY TO 100 KPa OR GREATER, A MINIMUM 400mm GRANULAR 'A' OR OPSS GRANULAR 'B' TYPE II COMPACTED TO 95% SPMDD SHOULD BE INSTALLED AS THE BEDDING MATERIAL.

ANY DISTURBED AREA DURING CONSTRUCTION TO BE RESTORED TO THE ORIGINAL CONDITION OR BETTER TO THE SATISFACTION OF THE AUTHORITIES HAVING JURISDICTION

CONTRACTOR TO VERIFY THE PRECISE LOCATIONS AND INVERT ELEVATIONS OF Ex. UNDERGROUND SERVICES AND Ex. UTILITIES PRIOR TO STARTING CONSTRUCTION

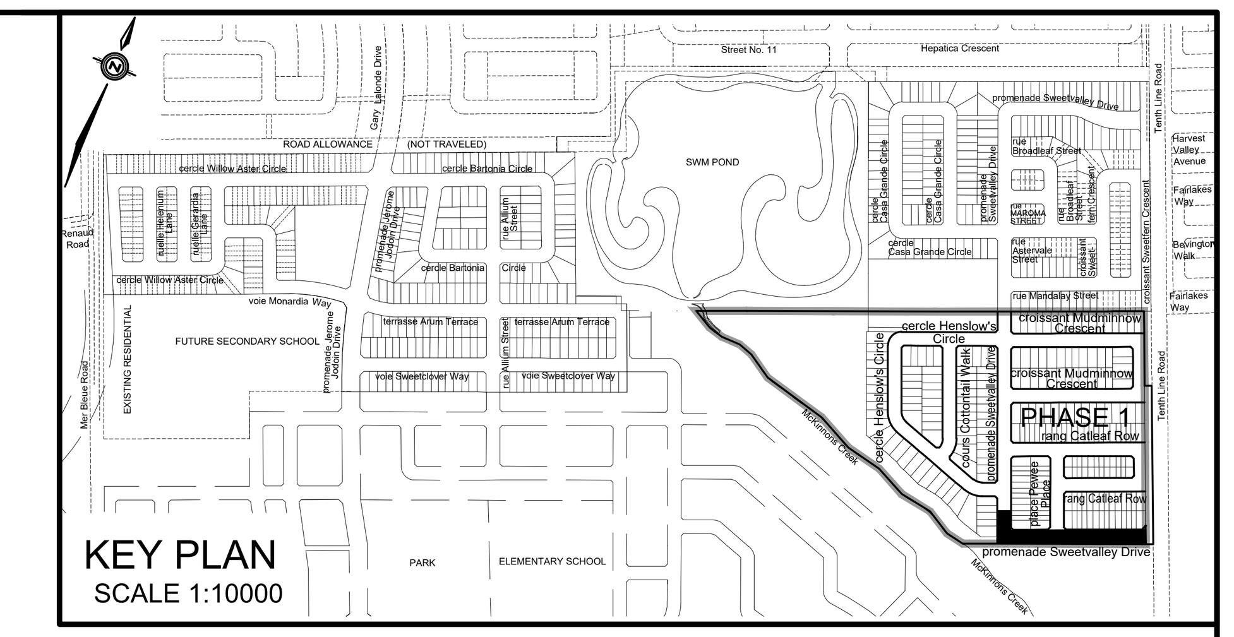
NOTE RE: TEST PIT/BOREHOLE EXCAVATIONS
ANY DISTURBED MATERIAL ENCOUNTERED BELOW THE SUBGRADE LEVEL WITHIN A BUILDING FOOTPRINT TO BE SUB-EXCAVATED AND BACKFILLED WITH COMPACTED ENGINEERED FILL AS PER GEOTECHNICAL ENGINEERS RECOMMENDATION.

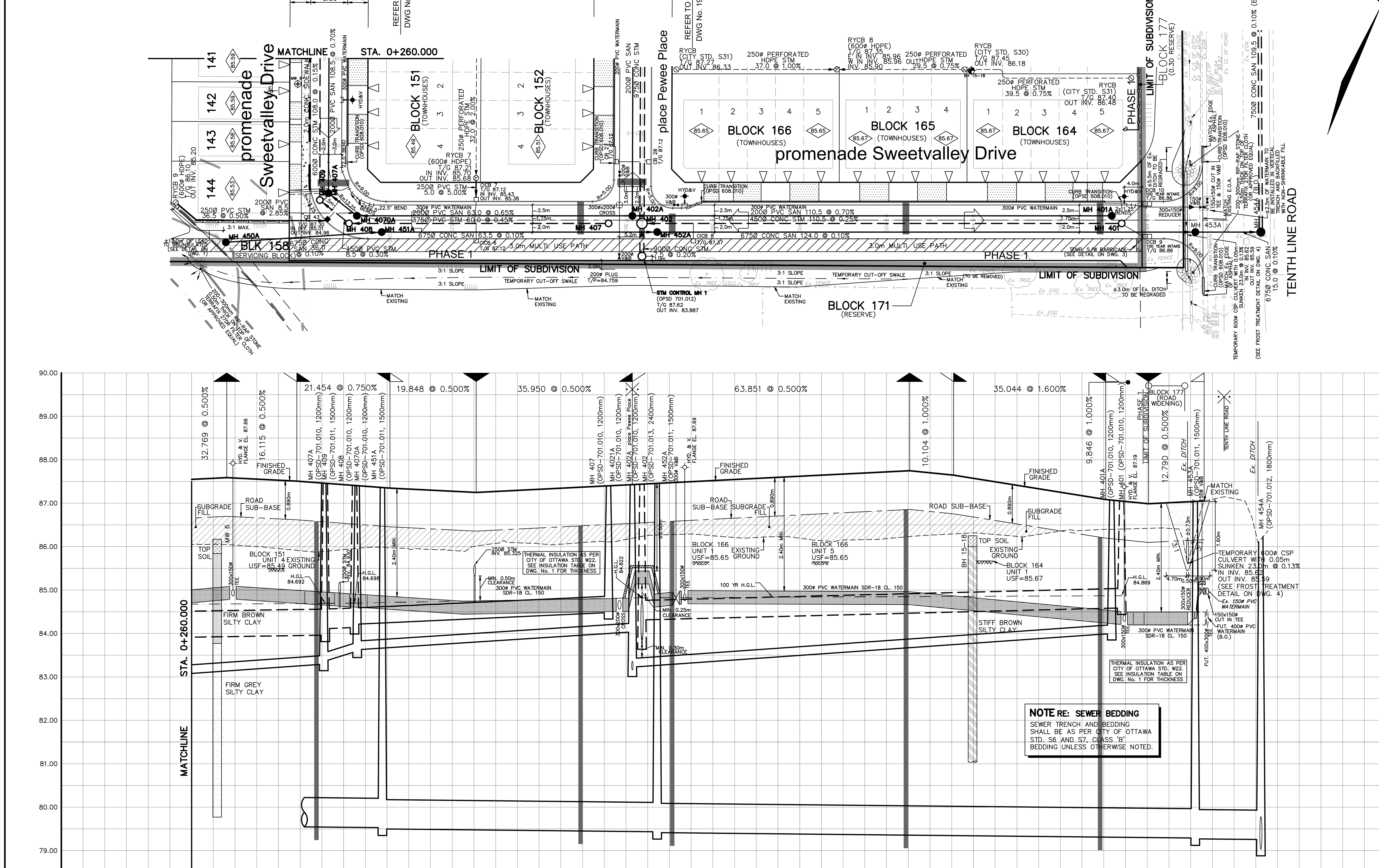
NOTE RE: WATERMAIN / WATER SERVICE
1. INSULATION REQUIRED FOR WATERMAIN / WATER SERVICE WHERE THE SEPARATION BETWEEN WATERMAIN / WATER SERVICE AND OTHER SERVICES AND STRUCTURES IS LESS THAN 1.2m AND THE COVER IS LESS THAN 2.4m. REFER TO CITY STD. W23 FOR DETAIL.
2. FOR SERVICE INSTALLATION AT SEWER CROSSING, REFER TO CITY STD. W38 FOR DETAIL

NOTE
FOR WATERMAIN CROSSING BELOW AND ABOVE SEWERS, REFER TO CITY STD. W25 AND W25.2, RESPECTIVELY, WHERE APPLICABLE

NOTE
FOR WATERMAIN STUBS, 2.4m MIN. COVER TO BE PROVIDED

NOTE: ICD
FOR ICD APPLICATION, REFER TO DRAWINGS No. 6 TO 9, 36 & 37 FOR DETAIL.





PROPOSED GRADES	87.547, 87.588, 87.528, 87.507, 87.461, 87.458, 87.408, 87.403, 87.389, 87.358, 87.348, 87.275, 87.247, 87.320, 87.420, 87.427, 87.425, 87.436, 87.454, 87.520, 87.620, 87.720, 87.746, 87.645, 87.572, 87.252, 87.085, 87.071, 87.048, 86.989, 87.040, 87.050, 87.071
TOP OF WATERMAIN	85.006, 85.091 HYD., 85.022, 84.983 22.5° HOR. BEND, 84.938 22.5° HOR. BEND, 84.915 22.5° HOR. BEND, 84.891 84.888 22.5° HOR. BEND, 84.760, 84.742, 84.775, 84.809 300x200ø CROSS, 84.809, 84.809, 85.439 2x22.5° VER. BEND, 85.439, 84.906 2x22.5° VER. BEND, 84.961 300ø V&B, 84.985 HYD., 84.982, 84.974, 84.966, 84.964, 84.824, 84.637, 84.500 HYD., 84.496 11.25° HOR. BEND, 84.496 11.25° HOR. BEND, 84.496 300x150ø REDUCER, 84.496 2x45° VER. BEND, 85.050 150ø V&B, 85.050, 85.050 150x150 TAPPING SLEEVE
STORM INVERT	600Ø CONC STM 108.0 @ 0.15% 65-D; 450Ø PVC STM 8.5 @ 0.30% SDR-35; NW 83.961; E 84.111; W 84.137; NE 84.212; 375Ø PVC STM 60.0 @ 0.45% SDR-35; SW 84.482; SE 83.862; NW 83.787; NE 84.312; 450Ø CONC STM 110.5 @ 0.25% 65-D; SW 84.588
SANITARY INVERT	200Ø PVC SAN 108.5 @ 0.70% SDR-35; 200Ø PVC SAN 8.5 @ 2.85% SDR-35; NW 83.293; SE 83.353; NW 83.595; E 83.625; 200Ø PVC SAN 63.0 @ 0.65% SDR-35; W 84.036; NW 83.157; NE 83.225; 200Ø PVC SAN 110.5 @ 0.70% SDR-35; SW 83.999
SANITARY TRUNK INVERT	675Ø CONC SAN 36.0 @ 0.10% 140-D; SW 79.528; NE 79.498; 675Ø CONC SAN 63.5 @ 0.10% 140-D; SW 79.434; NE 79.414; 675Ø CONC SAN 124.0 @ 0.10% 140-D; SW 79.290; NE 79.270; 675Ø CONC SAN 15.0 @ 0.10% 140-D; SW 79.255; NW 79.030 (BULKHEAD)
CENTERLINE CHAINAGE	0+260.000, 0+268.098, 0+269.513, 0+280.000, BC 0+283.911, 0+284.213, 0+286.044, 0+290.420, 0+290.782, 0+292.893, 0+296.379, 0+297.510, 0+298.114, 0+300.000, 0+300.412, EC 0+303.924, 0+304.776, 0+305.667, 0+320.000, 0+322.656, 0+325.515, 0+340.000, PI 0+358.500, 0+360.000, 0+360.549, 0+361.070, 0+361.468, 0+363.684, 0+365.299, 0+366.586, 0+367.177, 0+371.516, 0+373.516, 0+380.000, 0+400.000, 0+420.000, 0+425.000, 0+425.316, 0+435.420, 0+440.000, 0+460.000, 0+470.464, 0+471.834, 0+474.124, 0+474.716, 0+476.814, 0+479.282, 0+480.000, 0+480.310, PI 0+490.056, 0+488.351, 0+491.366, 0+491.920, 0+491.064, 0+492.623, 0+493.100, 0+493.823, PI 0+498.351, 0+500.000, 0+506.160

LEGEND

CROSS
45° BEND
LATERAL
HYDRANT, VALVE & VB
TEE
VALVE & VC
VALVE & VB
22.5° BEND
11.25° BEND
REDUCER
CAP
SANITARY MAINTENANCE HOLE
CAP
STREET CATCHBASIN & LEAD
STORM MAINTENANCE HOLE
CURB INLET CATCHBASIN & LEAD
CATCHBASIN/ MAINTENANCE HOLE
INTERCONNECTED CATCH BASIN & LEADS
CAP
SINGLE SERVICE LOCATION (ST, SAN & WM)
SINGLE SERVICE LOCATION (ST, SAN & WM)
TEE CATCHBASIN
PERFORATED PIPE
ELBOW CATCHBASIN
2 YEAR WATER LINE (WL)
DESIGNED TOP OF VALLEY (TOV)
15m OFFSET FROM TOV
30m OFFSET FROM 2 YEAR WL
FLOOD PLAIN
15m OFFSET FROM FLOOD PLAIN

HYDRO TRANSFORMER
STREET LIGHT STANDARD
DITCH AND CULVERT
CONCRETE SIDEWALK
CURB & DEPRESSED CURB
ASPHALT SIDEWALK
1.5m CHAIN LINK FENCE (UNLESS OTHERWISE NOTED)
3.0m NOISE BARRIER
WOOD PRIVACY BARRIER
1.5m POST AND RAIL FENCE
PHASING LIMITS
PROPERTY BOUNDARY
BOREHOLE (BH), TEST PIT (TP), AUGER HOLE (AH), MONITORING WELL LOCATION, CONCEPTUAL WELL LOCATION — AS PER GEOTECHNICAL REPORT
TOP OF FOUNDATION ELEVATION
FINISHED FLOOR ELEVATION
UNDERSIDE OF FOOTING ELEVATION
NUMBER OF RISERS
UNITS REQUIRING PRESSURE REDUCING VALVES
WALKOUT UNITS
SLAB ON GRADE
OVERLAND FLOW DIRECTION
EXTERNAL OVERLAND FLOW DIRECTION
OTHER PHASES
LIMIT OF HAZARD LANDS / LIMIT OF DEVELOPMENT

TOPOGRAP
TOPOGRAPHIC INF
PROJECT No. 17-
CITY OF OTTAWA

LEGAL INF
CALCULATED M-F
PROJECT No. 17-
3rd SUBMISSI

Will Curry
WILL CURRY
PROJECT MANAGER
PLANNING, INFRASTRUCTURE & ECONOMIC DEVEVELOPMENT DEPARTMENT, CITY OF OTTAWA

BENCH MAR
ELEVATIONS SHOWN
BENCH MARK No.
TOWNSHIP: CUMBER
CONCRETE MONUMENT ON THE EAST SIDE OF MER BLEUE ROAD, 0.15KM NORTH OF ROAD INTERSECTION WITH NAVAN ROAD.

APPROVED
By currywi at 8:25 am, Aug 20, 2019

No.	DATE	BY	DESCRIPTION
4.	19-08-07	W.L.	REVISED PER CITY COMMENTS
3.	19-07-05	W.L.	REVISED PER CITY COMMENTS
2.	19-06-21	W.L.	2nd SUBMISSION
1.	18-11-15	W.L.	1st SUBMISSION

CITY OF OTTAWA

PROJECT No. 15-766

LICENSED PROFESSIONAL ENGINEER W. LIU 100167932 19-08-07 PROVINCE OF ONTARIO

PLAN AND PROFILE OF
promenade Sweetvalley Drive
(STA. 0+260.000 TO STA. 0+493.100)
© DSEL

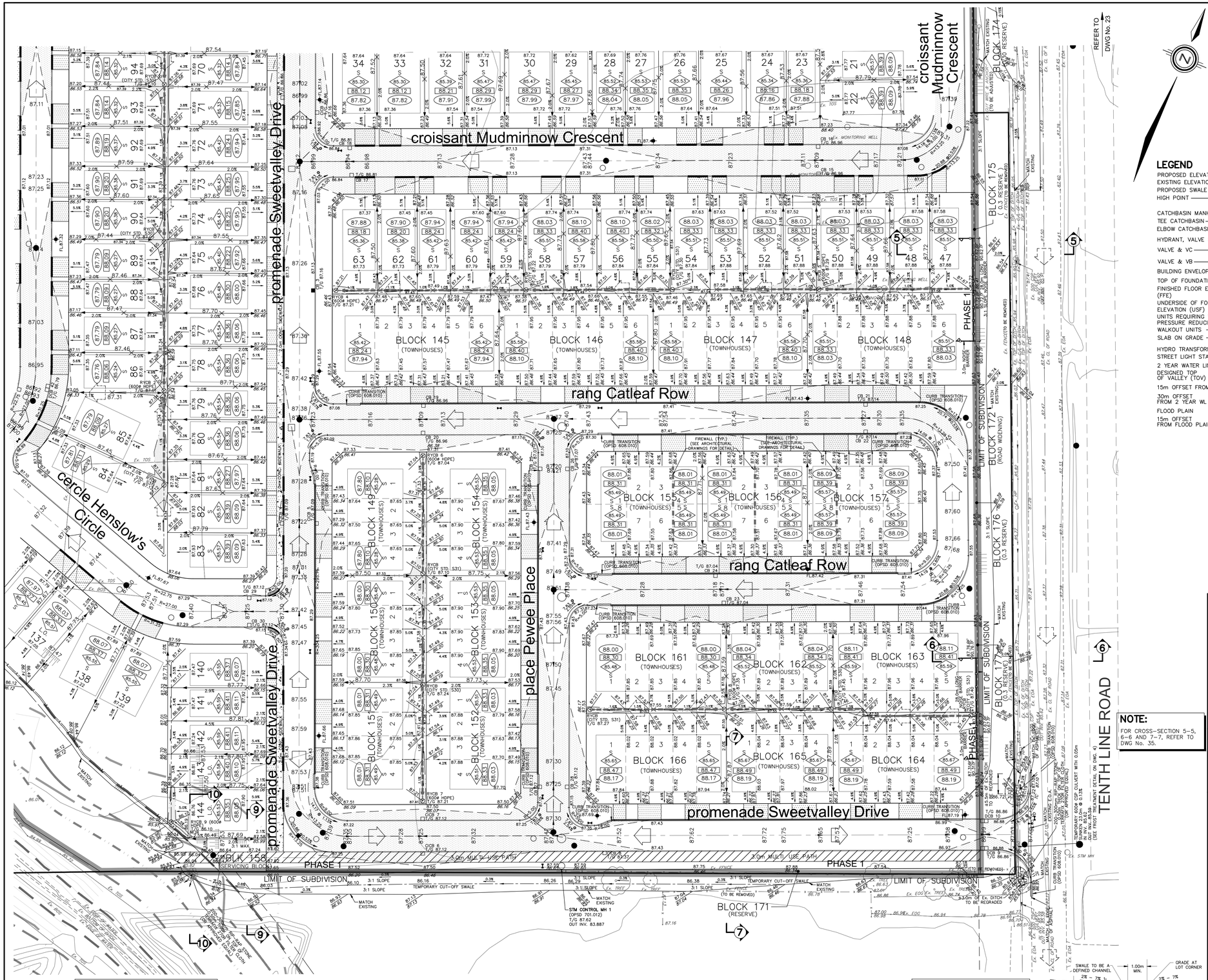
MATTAMY (MER BLEUE 2) LIMITED

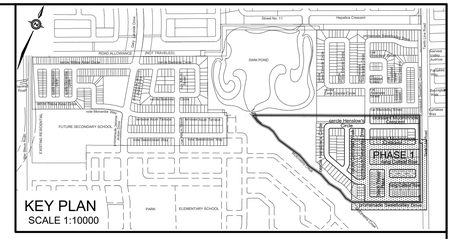
SUMMERSIDE SOUTH
PHASE 1

DSEL david schaeffer engineering ltd
120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
www.DSEL.ca

DRAWN BY: V.W./S.L.	CHECKED BY: P.P./C.M.	DRAWING NO.	SHEET NO.
DESIGNED BY: P.P./W.L.	CHECKED BY: K.M.		11
SCALE: H=1:500/V=1:50	DATE: NOVEMBER 2018		

CITY FILE No. D07-16-17-0028 CITY PLAN No. 17857





LEGEND

PROPOSED ELEVATION	103.45	OVERLAND FLOW DIRECTION	
EXISTING ELEVATION	102.23		
PROPOSED SWALE GRADE	1.5%	EXTERNAL OVERLAND FLOW DIRECTION	
HIGH POINT	102.16		
CATCHBASIN MANHOLE		REAR YARD OVERLAND FLOW DIRECTION	
TEE CATCHBASIN			
ELBOW CATCHBASIN			
HYDRANT, VALVE & VB		RETAINING WALL AND ELEVATIONS	T/W=103.10 B/W=102.44
VALVE & VC		1.5m CHAIN LINK FENCE (UNLESS OTHERWISE NOTED)	
VALVE & VB		1.5m POST AND RAIL FENCE	
BUILDING ENVELOPE		3.0m NOISE BARRIER (SEE LANDSCAPE DWG. FOR DETAILS)	87.78TF 84.78BF
TOP OF FOUNDATION (TOF)	94.70	PHASING LIMITS	
FINISHED FLOOR ELEVATION (FFE)	95.00	PROPERTY BOUNDARY	
UNDERSIDE OF FOOTING ELEVATION (USF)	92.00	3:1 TERRACING MAXIMUM SLOPE	100.20 99.20
UNITS REQUIRING WATER PRESSURE REDUCING VALVES	PRV		
WALKOUT UNITS	W.O.	PONDING AREA WITH SPILLWAY ELEVATION (MAXIMUM 0.30m)	89.64
SLAB ON GRADE	SOG	250ø PVC PERFORATED PIPE (REFER TO CITY STD S29 FOR REAR YARD TRENCH AND PIPE DETAILS ONLY) (SUBDRAIN APPLIED FOR SLOPE LESS THAN 1.5%)	
HYDRO TRANSFORMER			
STREET LIGHT STANDARD	LS	2 HOUR FIREWALL (SEE ARCHITECTURAL DRAWINGS FOR DETAIL)	
2 YEAR WATER LINE (WL)			
DESIGNED TOP OF VALLEY (TOV)			
15m OFFSET FROM TOV			
30m OFFSET FROM 2 YEAR WL			
FLOOD PLAIN			
15m OFFSET FROM FLOOD PLAIN			

Will Curry

WILL CURRY
PROJECT MANAGER
PLANNING, INFRASTRUCTURE & ECONOMIC DEVELOPMENT DEPARTMENT, CITY OF OTTAWA
GRADING & DRAINAGE APPROVED

TOPOGRAPHIC INFORMATION
TOPOGRAPHIC INFORMATION PROVIDED BY J.
PROJECT No. 17-10-025-01, SURVEY DAT
CITY OF OTTAWA 2K MAPPING, RECEIVED ON

APPROVED
By currywi at 8:41 am, Aug 20, 2019

LEGAL INFORMATION
CALCULATED M-PLAN PROVIDED BY J.D. BARNES LIMITED,
PROJECT No. 17-10-025-00 MPT, RECEIVED ON JUNE 04, 2019.

3rd SUBMISSION 19-08-07

BENCH MARK No. 01919680229 ELEVATION = 86.120 m
ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM AND ARE DERIVED FROM THE NCC BENCH MARK No. 01919680229 HAVING A PUBLISHED ELEVATION OF 86.120 METRES.
TOWNSHIP: CUMBERLAND
CONCRETE MONUMENT ON THE EAST SIDE OF MER BLEUE ROAD, 0.15KM NORTH OF ROAD INTERSECTION WITH NAVAN ROAD.

No.	DATE	BY	DESCRIPTION
4.	19-08-07	W.L.	REVISED PER CITY COMMENTS
3.	19-07-05	W.L.	REVISED PER CITY COMMENTS
2.	19-06-21	W.L.	2nd SUBMISSION
1.	18-11-15	W.L.	1st SUBMISSION

NOTE:
FOR CROSS-SECTION 5-5, 6-6 AND 7-7, REFER TO DWG No. 35.

Ottawa CITY OF OTTAWA

PROJECT No. 15-766

LICENSED PROFESSIONAL ENGINEER W. LIU 100167932 19-08-07 PROVINCE OF ONTARIO

GRADING PLAN

© DSEL

MATTAMY (MER BLEUE 2) LIMITED

SUMMERSIDE SOUTH PHASE 1

DSEL david schaeffer engineering ltd
120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
www.DSEL.ca

DRAWN BY:	V.W./S.L.	CHECKED BY:	P.P./C.M.	DRAWING NO.	SHEET NO.
DESIGNED BY:	P.P./W.L.	CHECKED BY:	K.M.		25
SCALE:	1:500	DATE:	NOVEMBER 2018		

NOTE RE: TEST PIT/BOREHOLE EXCAVATIONS
ANY DISTURBED MATERIAL ENCOUNTERED BELOW THE SUBGRADE LEVEL WITHIN A BUILDING FOOTPRINT TO BE SUB-EXCAVATED AND BACKFILLED WITH COMPACTED ENGINEERED FILL AS PER GEOTECHNICAL ENGINEERS RECOMMENDATION.

NOTE RE: PERFORATED PIPE
PERFORATED PIPE IS REQUIRED FOR SWALE SLOPE LESS THAN 1.5% REFER TO CITY STD. S29, S30 FOR REAR YARD TRENCH AND PIPE DETAIL ONLY

ANY DISTURBED AREA DURING CONSTRUCTION TO BE RESTORED TO THE ORIGINAL CONDITION OR BETTER TO THE SATISFACTION OF THE AUTHORITIES HAVING JURISDICTION

CONTRACTOR TO VERIFY THE PRECISE LOCATIONS AND INVERT ELEVATIONS OF Ex. UNDERGROUND SERVICES AND Ex. UTILITIES PRIOR TO STARTING CONSTRUCTION

PERMISSION REQUIRED FOR WORK ON ADJACENT LANDS

NOTE:
ALL EXISTING BUILDING, FENCE, CULVERT AND SPRINKLERS WITHIN LOTS, BLOCKS AND PROPOSED ROW TO BE REMOVED, WHERE APPLICABLE

NOTE:
ALL SWALES SHALL BE 0.15m DEEP WITH 3:1 SIDE SLOPES UNLESS OTHERWISE INDICATED

NOTE:
A GEOTECHNICAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO IS TO INSPECT ALL SUBGRADE SURFACES FOR FOOTING AND PAVEMENT STRUCTURES PRIOR TO CONSTRUCTION

TYPICAL GRADE CONTROL SWALE
N.T.S.

CITY FILE No. D07-16-17-0028 CITY PLAN No. 17857

APPENDIX

B

- FIRE UNDERWRITERS SURVEY – FIRE FLOW CALCULATION FOR BUILDING
- FIRE UNDERWRITERS SURVEY – FIRE FLOW CALCULATION FOR PORTABLE CLASSROOM
- WATER DEMAND CALCULATION
- BOUNDARY CONDITION

Fire Flow Design Sheet (FUS)

2666 Tenth Line Road, City of Ottawa, Ontario

City of Ottawa

WSP Project No. 221-12984-00

Date: 16-Mar-23

Proposed Avalon III (2-Storey school block)Elementary school
Fire Flow Requirements Based on Fire Underwriters Survey (FUS) 2020

1. An estimate of the Fire Flow required for a given fire area may be estimated by: $F = 220\ C\ w\ A$

F = required fire flow in litres per minute
C = coefficient related to the type of construction
- 1.5 for **Type V** Wood Frame Construction
- 0.8 for **Type IV-A** Mass Timber Construction
- 0.9 for **Type IV-B** Mass Timber Construction
- 1.0 for **Type IV-C** Mass Timber Construction
- 1.5 for **Type IV-D** Mass Timber Construction
- 1.0 for **Type III** Ordinary Construction
- 0.8 for **Type II** Noncombustible Construction
- 0.6 for **Type I** Fire resistive Construction

A =2-b) The single largest Floor Area plus 25% of each of the two immediately adjoining floors

A = 2914 m^2
C = 0.8
F = 9500.3 L/min

rounded off to 10,000 L/min (min value of 2000 L/min)

2. The value obtained in 1. may be reduced by as much as 25% for occupancies having a low contents fire hazard.

Non-combustible	-25%
Limited Combustible	-15%
Combustible	0%
Free Burning	15%
Rapid Burning	25%

Reduction due to low occupancy hazard -15% x 10,000 = 8,500 L/min

3. The value obtained in 2. may be reduced by as much as 50% for buildings equipped with automatic sprinkler protection.

Adequate Sprinkler confirms to NFPA13	-30%
Water supply common for sprinklers & fire hoses	-10%
Fully supervised system	-10%
No Automatic Sprinkler System	0%

Reduction due to Sprinkler System -40% x 8,500 = -3,400 L/min

4. The value obtained in 2. is increased for structures exposed within 45 metres by the fire area under consideration.

Separation	Charge
0 to 3 m	25%
3.1 to 10 m	20%
10.1 to 20 m	15%
20.1 to 30 m	10%
30.1 to 45 m	0%

Side 1	30	10%	north side
Side 2	75	0%	east side
Side 3	7.5	20%	south side
Side 4	99	0%	west side
		30%	(Total shall not exceed 75%)

Increase due to separation 30% x 8,500 = 2,550 L/min

5. The flow requirement is the value obtained in 2., minus the reduction in 3., plus the addition in 4.

The fire flow requirement is 8,000 L/min (Rounded to nearest 1000 L/min)
or **133 L/sec**
or 2,113 gpm (us)
or 1,760 gpm (uk)

Based on method described in:
"Water Supply for Public Fire Protection - A Guide to Recommended Practice", 2020
by Fire Underwriters Survey

Fire Flow Design Sheet (FUS)
2666 Tenth Line Road, City of Ottawa, Ontario
City of Ottawa
WSP Project No. 221-12984-00
Date: 16-Mar-23

Proposed Avalon III (Portable Building)
Elementary school
Fire Flow Requirements Based on Fire Underwriters Survey (FUS) 2020

1. An estimate of the Fire Flow required for a given fire area may be estimated by: $F = 220\ C\ w\ A$

F = required fire flow in litres per minute
C = coefficient related to the type of construction
- 1.5 for **Type V** Wood Frame Construction
- 0.8 for **Type IV-A** Mass Timber Construction
- 0.9 for **Type IV-B** Mass Timber Construction
- 1.0 for **Type IV-C** Mass Timber Construction
- 1.5 for **Type IV-D** Mass Timber Construction
- 1.0 for **Type III** Ordinary Construction
- 0.8 for **Type II** Noncombustible Construction
- 0.6 for **Type I** Fire resistive Construction

A =2-b) The single largest Floor Area plus 25% of each of the two immediately adjoining floors

A = 214 m^2
C = 1.5
F = 4828.7 L/min

rounded off to 5,000 L/min (min value of 2000 L/min)

2. The value obtained in 1. may be reduced by as much as 25% for occupancies having a low contents fire hazard.

Non-combustible	-25%
Limited Combustible	-15%
Combustible	0%
Free Burning	15%
Rapid Burning	25%

Reduction due to low occupancy hazard -15% x 5,000 = 4,250 L/min

3. The value obtained in 2. may be reduced by as much as 50% for buildings equipped with automatic sprinkler protection.

Adequate Sprinkler confirms to NFPA13	-30%
Water supply common for sprinklers & fire hoses	-10%
Fully supervised system	-10%
No Automatic Sprinkler System	0%

Reduction due to Sprinkler System -10% x 4,250 = -425 L/min

4. The value obtained in 2. is increased for structures exposed within 45 metres by the fire area under consideration.

Separation	Charge
0 to 3 m	25%
3.1 to 10 m	20%
10.1 to 20 m	15%
20.1 to 30 m	10%
30.1 to 45 m	0%

Side 1	6	20%	north side
Side 2	75	0%	east side
Side 3	3.6	20%	south side
Side 4	99	0%	west side
		40%	(Total shall not exceed 75%)

Increase due to separation 40% x 4,250 = 1,700 L/min

5. The flow requirement is the value obtained in 2., minus the reduction in 3., plus the addition in 4.

The fire flow requirement is 6,000 L/min (Rounded to nearest 1000 L/min)
or **100 L/sec**
or 1,585 gpm (us)
or 1,320 gpm (uk)

Based on method described in:
"Water Supply for Public Fire Protection - A Guide to Recommended Practice", 2020
by Fire Underwriters Survey

Water Demand Calculation Sheet

Project:	**Avalon III Elementary school**	**Date:**	**2023-03-16**
Location:	**2666 Tenth Line Road, City of Ottawa, ON**	**Design:**	**N.N.**
WSP Project No.	**221-12984-00**	**Checked:**	**D.B.Y**
		Page:	**1 of 1**

Proposed Buildings	Residential				Non-Residential			Average Daily			Maximum Daily			Maximum Hourly			Fire
	Units			Beds	Industrial	Institutional	Commercial	Demand (l/s)			Demand (l/s)			Demand (l/s)			Demand
	SF	APT	ST		(ha)	(ha)	(ha)	Res.	Non-Res.	Total	Res.	Non-Res.	Total	Res.	Non-Res.	Total	(l/min)
Proposed Two Storey Elementary Schoool Bldg.						1.77			0.57	0.57		0.86	0.86		1.55	1.55	8,000

Population Densities	
Single Family	3.4 person/unit
Semi-Detached	2.7 person/unit
Duplex	2.3 person/unit
Townhome (Row)	2.7 person/unit
Bachelor Apartment	1.4 person/unit
1 Bedroom Apartment	1.4 person/unit
2 Bedroom Apartment	2.1 person/unit
3 Bedroom Apartment	3.1 person/unit
4 Bedroom Apartment	4.1 person/unit
Avg. Apartment	1.8 person/unit

Average Daily Demand	
Residential	280 l/cap/day
Industrial	35000 l/ha/day
Institutional	28000 l/ha/day
Commercial	28000 l/ha/day

Maximum Daily Demand	
Residential	2.5 x avg. day
Industrial	1.5 x avg. day
Institutional	1.5 x avg. day
Commercial	1.5 x avg. day

Maximum Hourly Demand	
Residential	2.2 x max. day
Industrial	1.8 x max. day
Institutional	1.8 x max. day
Commercial	1.8 x max. day

Boundary Conditions
2666 Tenth Line Rd

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	34	0.57
Maximum Daily Demand	52	0.86
Peak Hour	93	1.55
Fire Flow Demand #1	8,000	133.33

Location

Results

Connection 1 – Sweetvalley Dr.

Demand Scenario	Head (m)	Pressure[1] (psi)
Maximum HGL	130.3	62.9
Peak Hour	126.0	56.9
Max Day plus Fire 1	126.3	57.3

[1] Ground Elevation = 86.1 m

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the

water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Yang, Winston

From: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Sent: March 22, 2023 8:04 AM
To: Yang, Winston; Murshid, Shoma
Cc: Zofia Jurewicz; Nwanise, Nwanise
Subject: RE: Boundary condition request for 2666 Tenth Line Rd - CECCE Elementary School on Claridge Lands in Mer Bleue Expansion Urban Expansion Area 10
Attachments: 2666 Tenth Line Rd_Boundary_Condition(17March2023).docx

Hello Winston,

Please find attached the water boundary conditions,

Best,

Rubina

Rubina Rasool
Project Manager
Planning, Infrastructure and Economic Development Department
Development Review – East Branch
City of Ottawa
110 Laurier Avenue West Ottawa, ON K1P 1J1
rubina.rasool@ottawa.ca

From: Yang, Winston <Winston.Yang@wsp.com>
Sent: March 17, 2023 12:49 AM
To: Murshid, Shoma <Shoma.Murshid@ottawa.ca>; Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Cc: Zofia Jurewicz <zofiaj@cuhaci.com>; Nwanise, Nwanise <Nwanise.Nwanise@wsp.com>; Baird, Natasha <Natasha.Baird@ottawa.ca>
Subject: Re: Boundary condition request for 2666 Tenth Line Rd - CECCE Elementary School on Claridge Lands in Mer Bleue Expansion Urban Expansion Area 10
Importance: High

Hi Shoma,

As per the pre-consultation meeting direction, here is the water supply boundary condition request for the proposed Avalon III Elementary School Development at 2666 Tenth Line Road in Orleans.
The site is proposed to be serviced from the existing 203mm diameter watermain stub from Sweetvalley Drive.

The proposed 2- storey elementary school block has the highest fire flow demand on the site and has been adopted as the worst case scenario. This building will be equipped with an automatic fire protection sprinkler system that complies with NFPA 13. There are two existing public fire hydrants on Sweetvalley Drive next to the subject site, both of which are within 45m of the building.

The domestic water demands were calculated using the City of Ottawa's Water Design Guidelines while fire demands were calculated using FUS 2020.
The results are summarized below:

Proposed Building	Average Daily Demand (l/s)	Maximum Daily Demand (l/s)	Maximum Hourly Demand (l/s)	Fire Demand (l/min)
Elementary School	0.57	0.86	1.55	8000

I have attached the Water demand, FUS calculation spreadsheet and Map showing conceptual water service connection for your review.

Thank you,

Winston Ding Bang Yang, P.Eng., PMP
Senior Civil Engineer
Land Development and Municipal Engineering - Ottawa

T+ 1 613-690-0538
M+ 1 647-628-8108

WSP Canada Inc.
2611 Queensview Drive Suite 300

Ottawa, Ontario
K2B 8K2

wsp.com

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APPENDIX

C

- SANITARY SEWER DESIGN SHEET
- EXISTING SANITARY SEWER DESIGN SHEET BY DSEL

SANITARY SEWER DESIGN SHEET
New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00
Date: June 02, 2023

LOCATION			RESIDENTIAL AREA AND POPULATION												INDUSTRIAL				COMMERCIAL		INSTITUTIONAL		I+C+I	INFILTRATION			TOTAL	PIPE					
LOCATION	FROM M.H.	TO M.H.	INDV AREA (ha)	ACCU AREA (ha)	NUMBER OF UNITS						POPULATION		PEAK FACT.	PEAK FLOW (l/s)	GROSS AREA (ha)	DEVEL. AREA (ha)	ACCU. AREA (ha)	PEAK FACTOR	INDIV AREA (ha)	ACCU. AREA (ha)	INDIV AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	INDIV AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	**TOTAL FLOW (l/s)**	LENGTH (m)	DIA. (mm)	SLOPE (%)	CAP. (FULL) (l/s)	VEL. (FULL) (m/s)	AVAIL. CAP. (%)
					SINGLES	SEMIS	AVG TOWNS	STACKED TOWNS	2-BED APT.	3-BED APT.	INDIV POP.	ACCU POP.																					
CONTRIBUTION From place Pewee Place, Pipe 404A - 406 A																																	
CONTRIBUTION From place Pewee Place, Pipe 404A - 406 A				1.530								142		0.00										1.530	2.21							
	EX.406A	*EX.408A*	0.270	2.480			8.00				22	240	3.49	2.72										0.270	2.48	0.82	**3.54**	*70.00*	*200*	*0.35*	19.40	0.62	**81.78%**
promenade Sweetvalley Drive				2.480								240													2.48							
To promenade Sweetvalley Drive, Pipe 408A - 413A																																	
	EX.401A	*EX.402A*	0.490	0.490			14.00				38	38	3.67	0.45										0.490	0.49	0.16	**0.61**	*110.50*	*200*	*0.70*	27.44	0.87	**97.77%**
Contribution From Proposed school to EX. SANMH402A																																	
	School	SANMH100																			1.75	1.75	0.85	1.747	0.46	0.15	**1.00**	9.70	200	1.00	32.80	1.04	**96.95%**
	SANMH100	SANMH101																				1.75	0.85	0.000	0.46	0.15	**1.00**	65.50	200	0.50	23.19	0.74	**95.68%**
	SANMH101	*EX.402A*																				1.75	0.85	0.000	0.46	0.15	**1.00**	11.70	200	1.50	40.17	1.28	**97.51%**
	EX.402A	*EX.4070A*	0.110	0.110								0										1.75	0.85	0.110	0.57	0.19	**1.04**	*63.00*	*200*	*0.65*	26.44	0.84	**96.08%**
	EX.4070A	*EX.407A*	0.040	0.150								0										1.75	0.85	0.040	0.61	0.20	**1.05**	*8.50*	*200*	*2.85*	55.37	1.76	**98.10%**
			0.340	0.490			12.00				33	33										1.75	0.85	0.340	0.95								
	EX.407A	*EX.408A*	0.410	0.900	9						31	64	3.63	0.75								1.75	0.85	0.410	1.36	0.45	**2.05**	*108.50*	*200*	*0.70*	27.44	0.87	**92.52%**
Contribution From rang Catleaf Row, Pipe 406A - 408A			2.480	2.480								240										1.75	0.85	2.480	3.84							
	EX.408A	*EX.413A*	0.310	3.690	7						24	328	3.45	3.67								1.75	0.85	0.310	4.15	1.37	**5.89**	*71.00*	*200*	*0.35*	19.40	0.62	**69.67%**
Contribution From croissant Mudminnow Crescent, Pipe 412A - 413A			1.120	1.120								96										1.75	0.85	1.120	5.27							
	EX.413A	*EX.426A*	0.230	5.040	4						14	438	3.40	4.83								1.75	0.85	0.230	5.50	1.82	**7.49**	*74.50*	*200*	*0.35*	19.40	0.62	**61.38%**
Contribution From croissant Mudminnow Crescent, Pipe 415A - 426A			9.830	9.830								681										1.75	0.85	9.830	15.33							
Contribution From cercle Henslow's Circle, Pipe 425A - 426A			3.060	3.060								211										1.75	0.85	3.060	18.39							
	EX.426A	*EX.110A*	0.06	17.99								1330	3.17	13.68								1.75	0.85	0.060	18.45	6.09	**20.62**	*39.50*	*250*	*0.25*	29.73	0.61	**30.67%**

DESIGN PARAMETERS

RESIDENTIAL AVG. DAILY FLOW =	280	COMMERCIAL PEAK FACTOR =	1.5	(WHEN AREA > 20%)	PEAK POPULATION FLOW, (l/s) =	P*q*M/86400	UNIT TYPE / PERSONS/UNIT
COMMERCIAL AVG. DAILY FLOW =	28,000		1.0	(WHEN AREA < 20%)	PEAK EXTRANEOUS FLOW, (l/s) =	I*Ac	SINGLES 3.4
	0.324				RESIDENTIAL PEAKING FACTOR, M =	1+(14/(4+P^0.5))*K	SEMI-DETACHED 2.7
INSTITUTIONAL AVG. DAILY FLOW =	28,000	INSTITUTIONAL PEAK FACTOR =	1.5	(WHEN AREA > 20%)	Ac = CUMULATIVE AREA (ha)		TOWNHOMES 2.7
	0.324		1.0	(WHEN AREA < 20%)	P = POPULATION (THOUSANDS)		WALK UP TOWNS 1.8
LIGHT INDUSTRIAL FLOW =	35,000						2-BED APT. UNIT 2.1
	0.405	RESIDENTIAL CORRECTION FACTOR, K =	0.80		SEWER CAPACITY, Qcap (l/s) =	1/N S^(1/2) R^(2/3) Ac	3-BED APT. UNIT 3.1
HEAVY INDUSTRIAL FLOW =	55,000	MANNING N =	0.013		(MANNING'S EQUATION)		
	0.637	PEAK EXTRANEOUS FLOW, I (l/s/ha) =	0.33				

DESIGNED:	**NO.**	**REVISION**	**DATE**
M.S.	1.	City Submission No.1	2023-06-02
CHECKED:			
D.B.Y			
PROJECT:			
Avalon III Elementary School			
Institutional Development			
LOCATION:			
Ottawa, Ontario			
PAGE NO:	**FILE & DWG. REFERENCE:**		
1 of 1	C03		

AVALON WEST
(NEIGHBOURHOOD 5)
SWM FACILITY
REFER TO
IBI GROUP
PROJECT No. 30113

SUMMERSIDE WEST PHASE I
REFER TO
DAVID SCHAEFFER ENGINEERING LIMITED
PROJECT No. 12-609

AVALON - STAGE 11
(REG. PLAN 4M-1352)

TENTH LINE

TENTH LINE ROAD

McKinnon's Creek

SWM POND
PERMANENT W/L 83.15m
100 YEAR W/L 84.54m

cercle Henslow's Circle

croissant Mudminnow Crescent

cours Cottontail Walk

promenade Sweetvalley Drive

rang Catleaf Row

place Newer Place

rue Mandalay Street

BLOCK 163 (PARK)

BLOCK 159 (0.3m SERVICE BLOCK/ACCESS ROAD & OVERLAND FLOW ROUTE)

BLOCK 160 (PARK)

BLOCK 168 (SWM POND)

BLOCK 169 (0.3m SERVICE BLOCK/ACCESS ROAD & OVERLAND FLOW ROUTE)

BLOCK 171 (RESERVE)

BLOCK 172 (0.3m RESERVE)

BLOCK 173 (0.3m RESERVE)

BLOCK 175 (0.3m RESERVE)

BLOCK 176 (0.3m RESERVE)

150A- 15A
0.08 INF.

150A- 15A
1.19

TOTAL Q=5.52 L/S
FROM PARK

150A- 15A
0.47

421A- 422A
0.07 4

426A- 110A
0.06 INF.

422A- 425A
0.26 17

425A- 426A
0.27 17

420A- 421A
0.60 51

419A- 420A
0.20 11

418A- 419A
0.32 17

424A- 425A
0.50 41

423A- 424A
0.38 28

417A- 418A
0.36 21

416A- 417A
0.10 4

413A- 426A
0.23 14

408A- 413A
0.31 24

407A- 408A
0.41 31

11A- 150A
0.03 INF.

110A- 11A
0.06 INF.

10A - 11A
0.43 38

9A - 10A
0.03 3

6A - 9A
0.34 44

9A - 10A
0.34 31

1A - 7A
0.24 33

8A - 9A
0.18 17

7A - 8A
0.05 3

409A- 414A
0.06 4

415A- 426A
0.34 28

414A- 415A
0.80 68

410A- 409A
0.23 17

412A- 413A
0.57 48

411A- 412A
0.55 48

411A- 410A
0.06 4

406A- 408A
0.27 22

405A- 406A
0.68 76

404A- 406A
0.16 17

407A- 408A
0.34 33

402A- 404A
0.27 22

403A- 404A
0.61 71

401A- 402A
0.49 38

4070A 407A
0.04 INF.

402A- 4070A
0.11 INF.

3.32 366

A=8.34Ha
67 POP=560
RESIDENTIAL

A=0.92Ha
INFILTRATION

A=47.51Ha
POP=3558
RESIDENTIAL

A=3.83Ha
COMMERCIAL

A=2.54Ha
INSTITUTIONAL

A=8.35Ha
PARK

Q = 147.37 L/S
FROM PUMP STATION
VIA FORCEMAIN

KEY PLAN
SCALE 1:10000

PHASE 1

LEGEND

SANITARY DRAINAGE BOUNDARY
SANITARY SUB-DRAINAGE BOUNDARY
SANITARY DRAINAGE BOUNDARY (OTHER PHASES)
UPSTREAM MH TO DOWNSTREAM MH — 43A - 44A
AREA IN HECTARES — 0.78
POPULATION — 61
UPSTREAM MH TO DOWNSTREAM MH
AREA IN OTHER PHASES IN HECTARES — 43A-44A 0.78
POPULATION — 61
EXTERNAL AREA IN HECTARES — A=18.99Ha
EXTERNAL POPULATION — 65 POP=1242
DENSITY (PERSONS/HECTARE)
EXTERNAL LAND USE — RESIDENTIAL
MAINTENANCE HOLE — 200mmø SAN — MH202A
CAP

TOPOGRAPHIC INFORMATION

TOPOGRAPHIC INFORMATION PROVIDED BY J.D. BARNES LIMITED, PROJECT No. 17-10-025-01, SURVEY DATED AUGUST 01, 2018. CITY OF OTTAWA 2K MAPPING, RECEIVED ON MARCH 04, 2013.

LEGAL INFORMATION

CALCULATED M-PLAN PROVIDED BY J.D. BARNES LIMITED, PROJECT No. 17-10-025-00 MP1, RECEIVED ON JUNE 04, 2019.

2nd SUBMISSION 19-06-21

NOT FOR CONSTRUCTION

BENCH MARK No. 01919680229 ELEVATION = 86.120 m

ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM AND ARE DERIVED FROM THE NCC BENCH MARK No. 01919680229 HAVING A PUBLISHED ELEVATION OF 86.120 METRES.

TOWNSHIP: CUMBERLAND

CONCRETE MONUMENT ON THE EAST SIDE OF MER BLEUE ROAD, 0.15KM NORTH OF ROAD INTERSECTION WITH NAVAN ROAD.

No.	DATE	BY	DESCRIPTION
2.	19-06-21	W.L.	2nd SUBMISSION
1.	18-11-15	W.L.	1st SUBMISSION

CITY OF OTTAWA

PROJECT No. 15-766

LICENSED PROFESSIONAL ENGINEER
W. LIU
100167932
19-06-21
PROVINCE OF ONTARIO

SANITARY DRAINAGE PLAN

MATTAMY (MER BLEUE 2) LIMITED

SUMMERSIDE SOUTH
PHASE 1

DSEL
david schaeffer engineering ltd
120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
www.DSEL.ca

DRAWN BY:	V.W./S.L.	CHECKED BY:	P.P./C.M.	DRAWING NO.	SHEET NO.
DESIGNED BY:	P.P./W.L	CHECKED BY:	K.M.		33
SCALE:	1:1000	DATE:	NOVEMBER 2018		

CITY FILE No. D07-16-17-0028 CITY PLAN No. 17857

SANITARY SEWER CALCULATION SHEET

Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION									COMM		INSTIT		PARK				I+C+I+P	INFILTRATION				PIPE						
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	UNITS Singles	UNITS Townhouse	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	ICI Ratio	ICI Peaking Factor	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL. (FULL.) (m/s)	VEL. (ACT.) (m/s)
Contribution From place Pewee Place, Pipe 404A - 406A								1.53	142												1.53	2.21									
	406A	408A	0.27	8		8	22	2.48	240	3.49	2.72										0.27	2.48	0.82	3.54	70.0	200	0.35	19.40	0.18	0.62	0.47
To promenade Sweetvalley Drive, Pipe 408A - 413A								2.48	240													2.48									
promenade Sweetvalley Drive																															
	401A	402A	0.49	14		14	38	0.49	38	3.67	0.45										0.49	0.49	0.16	0.61	110.5	200	0.70	27.44	0.02	0.87	0.35
To place Pewee Place, Pipe 402A - 404A								0.49	38													0.49									
	402A	4070A	0.11					0.11	0												0.11	0.11	0.04	0.04	63.0	200	0.65	26.44	0.00	0.84	0.14
	4070A	407A	0.04					0.15	0												0.04	0.15	0.05	0.05	8.5	200	2.85	55.37	0.00	1.76	0.25
			0.34	12		12	33	0.49	33												0.34	0.49									
	407A	408A	0.41	9	9		31	0.90	64	3.63	0.75										0.41	0.90	0.30	1.05	108.5	200	0.70	27.44	0.04	0.87	0.41
Contribution From rang Catleaf Row, Pipe 406A - 408A								2.48	240												2.48	3.38									
	408A	413A	0.31	7	7		24	3.69	328	3.45	3.67										0.31	3.69	1.22	4.88	71.0	200	0.35	19.40	0.25	0.62	0.51
Contribution From croissant Mudminnow Crescent, Pipe 412A - 413A								1.12	96												1.12	4.81									
	413A	426A	0.23	4	4		14	5.04	438	3.40	4.83										0.23	5.04	1.66	6.49	74.5	200	0.35	19.40	0.33	0.62	0.55
Contribution From croissant Mudminnow Crescent, Pipe 415A - 426A								9.83	681												9.83	14.87									
Contribution From cercle Henslow's Circle, Pipe 425A - 426A								3.06	211												3.06	17.93									
	426A	*Ex. 110A*	0.06					17.99	1330	3.17	13.68										0.06	17.99	5.94	19.61	39.5	250	0.25	29.73	0.66	0.61	0.65

Add a row for the proposed school site here

Copy these runs to our sanitary sewer design sheet template

LICENSED PROFESSIONAL ENGINEER
P.E. MORENO PICKART
100215995
PROVINCE OF ONTARIO

DESIGN PARAMETERS

Park Flow =	9300	L/ha/da
Average Daily Flow =	280	l/p/day
Comm/Inst Flow =	28000	L/ha/da
Industrial Flow =	35000	L/ha/da
Max Res. Peak Factor =	4.00	
Park Peak Factor =	1.50	

Industrial Peak Factor = as per MOE Graph

Extraneous Flow =	0.330	L/s/ha	
Minimum Velocity =	0.600	m/s	
Manning's n = (Conc)	0.013	(Pvc)	0.013
Townhouse coeff=	2.7		
Single house coeff=	3.4		

Designed: C.M.K.

Checked: P.P.

Dwg. Reference: Sanitary Drainage Plan, Dwg. No. 34

PROJECT: **SUMMERSIDE SOUTH PHASE 1**

LOCATION: **City of Ottawa**

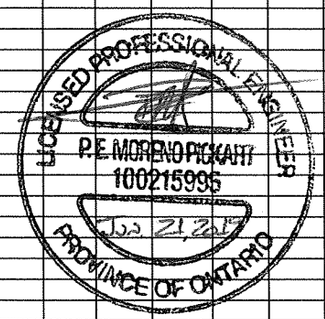
File Ref: 15-766 | Date: **June, 2019** | Sheet No. 2 of 2

766_SAN_2_Updated.xlsx

SANITARY SEWER CALCULATION SHEET

Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION									COMM		INSTIT		PARK				I+C+I+P	INFILTRATION				PIPE						
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	UNITS Singles	UNITS Townhouse	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	ICI Ratio	ICI Peaking Factor	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL. (FULL) (m/s)	VEL. (ACT.) (m/s)
cours Cottontail Walk																															
	423A	424A	0.38	8	8		28	0.38	28	3.69	0.33										0.38	0.38	0.13	0.46	42.0	200	0.65	26.44	0.02	0.84	0.32
	424A	425A	0.50	12	12		41	0.88	69	3.63	0.81										0.50	0.88	0.29	1.10	98.5	200	0.35	19.40	0.06	0.62	0.33
To cercle Henslow's Circle, Pipe 425A - 426A								0.88	69													0.88									
cercle Henslow's Circle																															
	416A	417A	0.10	1	1		4	0.10	4	3.76	0.05										0.10	0.10	0.03	0.08	17.5	200	1.30	37.40	0.00	1.19	0.22
	417A	418A	0.36	6	6		21	0.46	25	3.69	0.30										0.36	0.46	0.15	0.45	49.0	200	0.35	19.40	0.02	0.62	0.26
	418A	419A	0.32	5	5		17	0.78	42	3.66	0.50										0.32	0.78	0.26	0.76	72.0	200	0.35	19.40	0.04	0.62	0.29
	419A	420A	0.20	3	3		11	0.98	53	3.65	0.63										0.20	0.98	0.32	0.95	13.5	200	1.55	40.83	0.02	1.30	0.54
	420A	421A	0.60	15	15		51	1.58	104	3.59	1.21										0.60	1.58	0.52	1.73	92.5	200	0.35	19.40	0.09	0.62	0.38
	421A	422A	0.07	1	1		4	1.65	108	3.59	1.26										0.07	1.65	0.54	1.80	12.0	200	0.35	19.40	0.09	0.62	0.39
	422A	425A	0.26	5	5		17	1.91	125	3.57	1.45										0.26	1.91	0.63	2.08	63.5	200	0.35	19.40	0.11	0.62	0.40
Contribution From cours Cottontail Walk, Pipe 424A - 425A								0.88	69												0.88	2.79									
	425A	426A	0.27	5	5		17	3.06	211	3.51	2.40										0.27	3.06	1.01	3.41	72.5	200	0.35	19.40	0.18	0.62	0.46
To promenade Sweetvalley Drive, Pipe 426A - 110A								3.06	211													3.06									
croissant Mudminnow Crescent																															
	411A	412A	0.55	14	14		48	0.55	48	3.65	0.57										0.55	0.55	0.18	0.75	85.5	200	0.65	26.44	0.03	0.84	0.37
	412A	413A	0.57	14	14		48	1.12	96	3.60	1.12										0.57	1.12	0.37	1.49	84.0	200	0.35	19.40	0.08	0.62	0.36
To promenade Sweetvalley Drive, Pipe 413A - 426A								1.12	96													1.12									
	411A	410A	0.06	1	1		4	0.06	4	3.76	0.05										0.06	0.06	0.02	0.07	13.0	200	0.65	26.44	0.003	0.84	0.17
	410A	409A	0.23	5	5		17	0.29	21	3.70	0.25										0.23	0.29	0.10	0.35	56.5	200	0.35	19.40	0.02	0.62	0.23
Contribution From External			8.34	1	1		560	8.63	581												8.34	8.63									
	409A	414A	0.06	1	1		4	8.69	585	3.35	6.35										0.06	8.69	2.87	9.22	10.5	200	0.35	19.40	0.48	0.62	0.61
	414A	415A	0.80	20	20		68	9.49	653	3.33	7.05										0.80	9.49	3.13	10.18	120.0	200	0.35	19.40	0.52	0.62	0.62
	415A	426A	0.34	8	8		28	9.83	681	3.32	7.33										0.34	9.83	3.24	10.57	50.5	200	0.35	19.40	0.54	0.62	0.63
To promenade Sweetvalley Drive, Pipe 426A - 110A								9.83	681													9.83									
place Pewee Place																															
Contribution From promenade Sweetvalley Drive, Pipe 401A - 402A								0.49	38												0.49	0.49									
	402A	404A	0.27	8		8	22	0.76	60	3.64	0.71										0.27	0.76	0.25	0.96	66.5	200	0.35	19.40	0.05	0.62	0.32
Contribution From rang Catleaf Row, Pipe 403A - 404A								0.61	71												0.61	1.37									
	404A	406A	0.16	4		4	11	1.53	142	3.56	1.64										0.16	1.53	0.50	2.14	47.0	200	0.35	19.40	0.11	0.62	0.40
To rang Catleaf Row, Pipe 406A - 408A								1.53	142													1.53									
rang Catleaf Row																															
	403A	404A	0.61	26		26	71	0.61	71	3.63	0.83										0.61	0.61	0.20	1.04	114.5	200	0.80	29.34	0.04	0.93	0.43
To place Pewee Place, Pipe 404A - 406A								0.61	71													0.61									
	405A	406A	0.68	28		28	76	0.68	76	3.62	0.89										0.68	0.68	0.22	1.12	113.5	200	0.65	26.44	0.04	0.84	0.42



DESIGN PARAMETERS

Park Flow =	9300	L/ha/da
Average Daily Flow =	280	l/p/day
Comm/inst Flow =	28000	L/ha/da
Industrial Flow =	35000	L/ha/da
Max Res. Peak Factor =	4.00	
Park Peak Factor =	1.50	

Industrial Peak Factor = as per MOE Graph
Extraneous Flow = 0.330 L/s/ha
Minimum Velocity = 0.600 m/s
Manning's n = (Conc) 0.013 (Pvc) 0.013
Townhouse coeff= 2.7
Single house coeff= 3.4

Designed: C.M.K.
Checked: P.P.
Dwg. Reference: Sanitary Drainage Plan, Dwg. No. 34

PROJECT: **SUMMERSIDE SOUTH PHASE 1**
LOCATION: **City of Ottawa**
File Ref: 15-766
Date: **June, 2019**
Sheet No. 1 of 2

766_SAN_2_Updated.xlsx

SANITARY SEWER CALCULATION SHEET

Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION									COMM		INSTIT		PARK				I+C+I+P	INFILTRATION				PIPE						
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	UNITS Singles	UNITS Townhouse	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	ICI Ratio	ICI Peaking Factor	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL. (FULL.) (m/s)	VEL. (ACT.) (m/s)
Contribution From place Pewee Place, Pipe 404A - 406A								1.53	142												1.53	2.21									
	406A	408A	0.27	8		8	22	2.48	240	3.49	2.72										0.27	2.48	0.82	3.54	70.0	200	0.35	19.40	0.18	0.62	0.47
To promenade Sweetvalley Drive, Pipe 408A - 413A								2.48	240													2.48									
promenade Sweetvalley Drive																															
	401A	402A	0.49	14		14	38	0.49	38	3.67	0.45										0.49	0.49	0.16	0.61	110.5	200	0.70	27.44	0.02	0.87	0.35
To place Pewee Place, Pipe 402A - 404A								0.49	38													0.49									
	402A	4070A	0.11					0.11	0												0.11	0.11	0.04	0.04	63.0	200	0.65	26.44	0.00	0.84	0.14
	4070A	407A	0.04					0.15	0												0.04	0.15	0.05	0.05	8.5	200	2.85	55.37	0.00	1.76	0.25
			0.34	12		12	33	0.49	33												0.34	0.49									
	407A	408A	0.41	9	9		31	0.90	64	3.63	0.75										0.41	0.90	0.30	1.05	108.5	200	0.70	27.44	0.04	0.87	0.41
Contribution From rang Catleaf Row, Pipe 406A - 408A								2.48	240												2.48	3.38									
	408A	413A	0.31	7	7		24	3.69	328	3.45	3.67										0.31	3.69	1.22	4.88	71.0	200	0.35	19.40	0.25	0.62	0.51
Contribution From croissant Mudminnow Crescent, Pipe 412A - 413A								1.12	96												1.12	4.81									
	413A	426A	0.23	4	4		14	5.04	438	3.40	4.83										0.23	5.04	1.66	6.49	74.5	200	0.35	19.40	0.33	0.62	0.55
Contribution From croissant Mudminnow Crescent, Pipe 415A - 426A								9.83	681												9.83	14.87									
Contribution From cercle Henslow's Circle, Pipe 425A - 426A								3.06	211												3.06	17.93									
	426A	*Ex. 110A*	0.06					17.99	1330	3.17	13.68										0.06	17.99	5.94	19.61	39.5	250	0.25	29.73	0.66	0.61	0.65

DESIGN PARAMETERS

Park Flow =	9300	L/ha/da
Average Daily Flow =	280	l/p/day
Comm/Inst Flow =	28000	L/ha/da
Industrial Flow =	35000	L/ha/da
Max Res. Peak Factor =	4.00	
Park Peak Factor =	1.50	

Industrial Peak Factor = as per MOE Graph

Extraneous Flow =	0.330	L/s/ha	
Minimum Velocity =	0.600	m/s	
Manning's n = (Conc)	0.013	(Pvc)	0.013
Townhouse coeff=	2.7		
Single house coeff=	3.4		

Designed: C.M.K.

Checked: P.P.

Dwg. Reference: Sanitary Drainage Plan, Dwg. No. 34

PROJECT: **SUMMERSIDE SOUTH PHASE 1**

LOCATION: **City of Ottawa**

File Ref: 15-766 | Date: **June, 2019** | Sheet No. 2 of 2

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SANITARY SEWER CALCULATION SHEET

Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION								COMM		INSTIT		PARK				I+C+I+P	INFILTRATION				PIPE						
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	UNITS Singles	UNITS Townhouse	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	ICI Ratio	ICI Peaking Factor	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL. (FULL) (m/s)	VEL. (ACT.) (m/s)
Sanitary Trunk																														
Contribution From External								47.51	3558				3.83		2.54		8.35				62.23	62.23								
								0.92													0.92	63.15								
	450A	451A						48.43	3558	2.90	33.47		3.83		2.54		8.35	0.10	1.00	3.41	0.00	63.15	20.84	57.72	36.0	675	0.10	265.82	0.22	0.74	0.59
	451A	452A						48.43	3558	2.90	33.47		3.83		2.54		8.35	0.10	1.00	3.41	0.00	63.15	20.84	57.72	63.5	675	0.10	265.82	0.22	0.74	0.59
	452A	453A						48.43	3558	2.90	33.47		3.83		2.54		8.35	0.10	1.00	3.41	0.00	63.15	20.84	57.72	124.0	675	0.10	265.82	0.22	0.74	0.59
	453A	454A						48.43	3558	2.90	33.47		3.83		2.54		8.35	0.10	1.00	3.41	0.00	63.15	20.84	57.72	15.0	675	0.10	265.82	0.22	0.74	0.59
To TENTH LINE ROAD, Pipe 454A - 455A								48.43	3558				3.83		2.54		8.35					63.15									
TENTH LINE ROAD																														
Contribution From promenade Sweetvalley Drive, Sanitary Trunk Pipe 453A - 454A (B.O.)								48.43	3558				3.83		2.54		8.35				63.15	63.15		147.37	(Flow from Pump Station Via Forcemain)					
	454A (B.O)	455A (B.O.)	3.32				366	51.75	3924	2.87	36.53		3.83		2.54		8.35	0.10	1.00	3.41	3.32	66.47	21.94	209.25	109.5	750	0.10	352.05	0.59	0.80	0.83

LICENSED PROFESSIONAL ENGINEER
P.E. MORENO PICKART
100215995
June 21, 2019
PROVINCE OF ONTARIO

DESIGN PARAMETERS

Park Flow =	9300	L/ha/da
Average Daily Flow =	280	l/p/day
Comm/Inst Flow =	28000	L/ha/da
Industrial Flow =	35000	L/ha/da
Max Res. Peak Factor =	4.00	
Park Peak Factor =	1.50	

Industrial Peak Factor = as per MOE Graph
Extraneous Flow = 0.330 L/s/ha
Minimum Velocity = 0.600 m/s
Manning's n = (Conc) 0.013 (Pvc) 0.013
Townhouse coeff= 2.7
Single house coeff= 3.4

Designed: C.M.K.
Checked: P.P.
Dwg. Reference: Sanitary Drainage Plan, Dwg. No. 34

PROJECT: **SUMMERSIDE SOUTH PHASE 1**
LOCATION: **City of Ottawa**
File Ref: 15-766
Date: **June, 2019**
Sheet No. 1 of 1

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APPENDIX

D

- STORM SEWER DESIGN SHEET
- EXISTING SEWER DESIGN SHEET BY DSEL
- DWG C05 – PRE-DEVELOPMENT DRAINAGE PLAN
- DWG C06 – POST-DEVELOPMENT DRAINAGE PLAN
- STORMWATER MANAGEMENT CALCULATIONS

STORM SEWER DESIGN SHEET
New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00
Date: JUNE 02, 2023

LOCATION				AREA (Ha)						RATIONAL DESIGN FLOW												PROPSOED SEWER DATA								
LOCATION	AREA ID	FROM	TO	C= 0.25	C= 0.50	C= 0.70	C= 0.80	C= 0.90	C= 1.00	IND 2.78AC	CUM 2.78 AC	INLET (min)	TOTAL (min)	i (2) (mm/hr)	i (5) (mm/hr)	i (100) (mm/hr)	BLDG FLOW (L/s)	2yr PEAK FLOW (L/s)	5yr PEAK FLOW (L/s)	100yr PEAK FLOW (L/s)	ICD FIXED FLOW (L/s)	DESIGN FLOW (L/s)	MODIFIED DESIGN FLOW (L/s)	MATERIAL PIPE	SIZE (mm)	SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	VELOCITY (m/s)	TIME IN PIPE	AVAIL CAP (2yr) (L/s)	AVAIL CAP (2yr) (%)
To Sweetvalley Drive																															
SCHOOL SITE	S101	DICB01	CBMH100	0.081						0.056	0.056	20.00	20.23	52.03	70.25	119.95		2.93				2.93		PVC DR-35	200.0	1.00	14.40	32.83	1.04	0.23	29.90	91.08%
	S102	CBMH100	STMH102	0.012				0.048		0.128	0.185	20.23	20.62	51.66	69.75	119.09		9.54				9.54		PVC DR-35	300.0	0.35	19.10	57.27	0.81	0.39	47.72	83.33%
	S111	CB03	CBMH111	0.142						0.099	0.099	20.00	20.26	52.03	70.25	119.95		5.13				5.13		PVC DR-35	250.0	1.00	19.00	59.53	1.21	0.26	54.39	91.37%
	S110	CBMH111	CBMH101	0.640						0.445	0.445	20.26	20.60	51.61	69.68	118.97		22.96				22.96		PVC DR-35	250.0	0.50	17.65	42.09	0.86	0.34	19.13	45.46%
	S104	CBMH101	STMH102					0.045		0.113	0.557	20.60	20.78	51.08	68.95	117.71		28.47				28.47		PVC DR-35	300.0	0.35	8.50	57.27	0.81	0.18	28.80	50.28%
	S103	CB02	STMH102					0.047		0.118	0.118	10.00	10.14	76.81	104.19	178.56		9.03				9.03		PVC DR-35	200.0	1.00	8.50	32.83	1.04	0.14	23.80	72.49%
		STMH102	CBMH103							0.000	0.860	20.78	21.18	50.81	68.58	117.08		43.68				43.68		PVC DR-35	300.0	0.35	19.40	57.27	0.81	0.40	13.58	23.72%
	S105	CBMH103	CBMH108					0.058		0.145	1.005	21.18	21.34	50.21	67.76	115.67		50.45				50.45		PVC DR-35	300.0	0.35	7.85	57.27	0.81	0.16	6.82	11.90%
	S112	CB04	CBMH108	0.212						0.147	0.147	20.00	20.41	52.03	70.25	119.95		7.67				7.67		PVC DR-35	200.0	1.00	25.70	32.83	1.04	0.41	25.17	76.65%
	S117	CB05	CBMH104		0.072					0.100	0.100	10.00	10.32	76.81	104.19	178.56		7.69				7.69		PVC DR-35	200.0	1.00	20.30	32.83	1.04	0.32	25.14	76.59%
	S116	CBMH104	CBMH105	0.068				0.002		0.052	0.152	10.32	10.85	75.58	102.51	175.65		11.51				11.51		PVC DR-35	300.0	0.35	25.35	57.27	0.81	0.52	45.75	79.89%
	S115	CBMH105	CBMH106		0.091			0.006		0.142	0.294	10.85	11.54	73.70	99.93	171.18		21.66				21.66		PVC DR-35	300.0	0.35	33.70	57.27	0.81	0.69	35.61	62.18%
	S114	CB06	CBMH106	0.008				0.093		0.238	0.238	10.00	10.40	76.81	104.19	178.56		18.30				18.30		PVC DR-35	200.0	1.00	25.30	32.83	1.04	0.40	14.53	44.27%
	S113	CBMH106	CBMH107	0.060				0.035		0.129	0.661	11.54	12.50	71.36	96.71	165.61		47.19				47.19		PVC DR-35	375.0	0.26	46.40	89.49	0.81	0.96	42.30	47.27%
	S107, S108	CB07	CBMH107	0.056				0.017		0.081	0.081	20.00	20.20	52.03	70.25	119.95		4.24				4.24		PVC DR-35	200.0	1.00	12.25	32.83	1.04	0.20	28.59	87.09%
	S106	CBMH107	CBMH108	0.065				0.018		0.090	0.833	20.20	20.59	51.72	69.82	119.22		43.08				43.08		PVC DR-35	375.0	0.26	19.10	89.49	0.81	0.39	46.41	51.86%
		CBMH108	STMH110							0.000	1.985	21.34	21.37	49.97	67.44	115.11		99.20				99.20		PVC DR-35	450.0	0.30	1.50	156.32	0.98	0.03	57.12	36.54%
		BLDG	STMH109						0.231	0.642	0.642	10.00	10.02	76.81	104.19	178.56		49.32				49.32		PVC DR-35	300.0	1.00	1.85	96.80	1.37	0.02	47.48	49.05%
		STMH109	STMH110							0.000	0.642	10.02	10.81	76.72	104.07	178.35		49.27				49.27		PVC DR-35	375.0	0.26	38.05	89.49	0.81	0.78	40.22	44.95%
		STMH110	*EX. STM CONTROL MH1*							0.000	2.627	21.37	21.62	49.93	67.39	115.02		131.19				131.19		PVC DR-35	450.0	0.30	15.00	156.32	0.98	0.25	25.13	16.07%
SWEETVALLEY DRIVE		*EX. STM CONTROL MH1*	*EX.STMH402A*							0.000	2.627	21.62	21.72	49.56	66.88	114.15		130.22				130.22		CONC	*900.0*	*0.20*	*7.50*	810.41	1.27	0.10	680.20	83.93%

Definition:
Q=2.78CiA, where:
Q = Peak Flow in Litres per Second (L/s)
A = Area in Hectares (Ha)
i = Rainfall Intensity in millimeters per hour (mm/hr)
i = 732.951/(TC+6.199)^0.810 — 2 Year
i = 1174.184/(TC+6.014)^0.816 — 5 Year
i = 1735.688/(TC+6.014)^0.820 — 100 Year

Notes:
1. Mannings coefficient (n) = 0.013

Time-of-Concentration in the Swale
FAA Equation: t (min) = 3.258 [(1.1 - C) L^0.5 / S^.33]
Where: Longest Watercourse Length, L (m). S (%)
Runoff Coef.C = Impervious

No.	L (m)	S %	Tc (min)
			#DIV/0!

Designed: M.S.
Checked: D.Y.
Dwg. Reference: C06

No.	Revision	Date
1.	City Submission No. 1	2023-06-02

File Reference: 221-12984-00
Date: 2023-06-02
Sheet No: 1 of 1

CLIENT:

Écoles catholiques Centre-Est

CLIENT REF. #:

ARCHITECT:

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OTTAWA ONTARIO CANADA K2B 8K2
TEL.: 1-613-829-2800 | FAX: 1-613-829-8299 | WWW.WSPGROUP.COM

PROJECT:

ÉCOLE ÉLÉMENTAIRE CATHOLIQUE AVALON III
TENTH LINE ROAD
OTTAWA, ON

CONSEIL DES ÉCOLES CATHOLIQUES DU CENTRE-EST
4000, RUE LABELLE, OTTAWA, ON K1J 1A1

KEY PLAN:

SEAL:

LICENSED PROFESSIONAL ENGINEER
D. B. YANG
100230568
2023-06-02
PROVINCE OF ONTARIO

NOT VALID UNLESS SIGNED AND DATED

DISCLAIMER: COPYRIGHT:

BEARING NOTE

BEARING ARE GRID, DERIVED FROM THE CAN-NET REAL TIME NETWORK GPS OBSERVATIONS, REFERENCED TO SPECIFIED CONTROL POINTS 01919680184 AND 019198434761, MTM ZONE 9 (76°30' WEST LONGITUDE) NAD83 (ORIGINAL)
BENCHMARK #1 N:5033845.03 E:384791.31 Z:88.40
BENCHMARK #2 N:5033883.86 E:384885.06 Z:87.99

ISSUED FOR - REVISION:

IS	RE	DATE	DESCRIPTION
1		2023-06-02	ISSUED FOR SPA

PROJECT NO: 221-12984-00	DATE: 2023-06-01
ORIGINAL SCALE: 1:300	IF THIS BAR IS NOT 25mm LONG, ADJUST YOUR PLOTTING SCALE.
DESIGNED BY: D.Y./M.S.	
DRAWN BY: S.T-R.	
CHECKED BY: D.Y.	25mm

DISCIPLINE: CIVIL

TITLE:

PRE-DEVELOPMENT DRAINAGE AREA PLAN

SHEET NUMBER: C05

SHEET #: OF

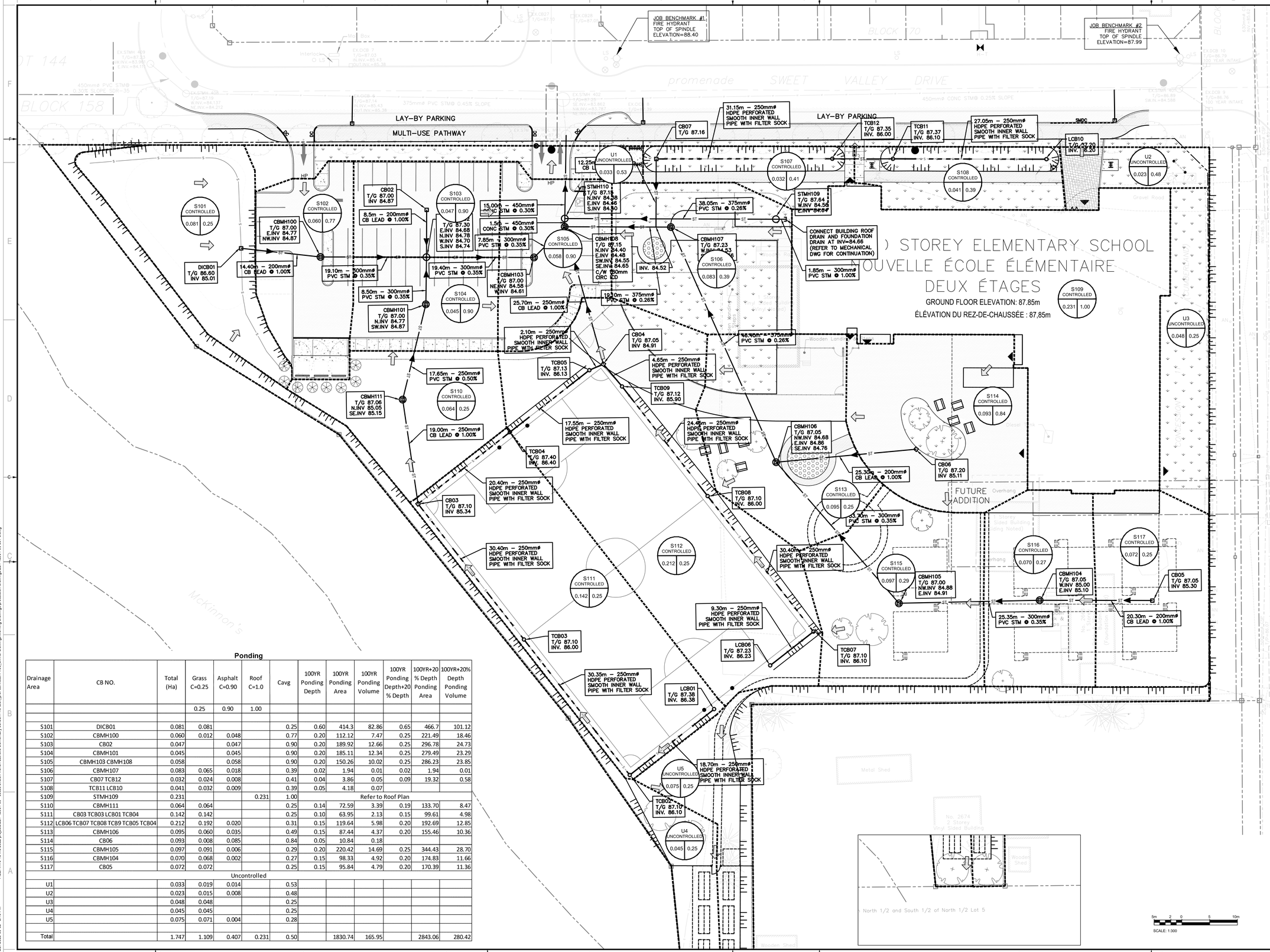
ISSUE: ISSUED FOR SPA

REV #: 0

DATE OF: 2023-06-02

FILE: v:\10-13 me 2022 projects\221-12984-00 - cecce avalon french catholic elementary school\14.0 tech_prof services\14.00_civil\3. drawings\working drawings\production drawings\221-12984-00 c01-6.dwg

CTB: wsp_kingston_24x36.ctb
PLOTTED: 2023-06-02 - 2:39 PM



Ponding

Drainage Area	CB NO.	Total (Ha)	Grass C=0.25	Asphalt C=0.90	Roof C=1.0	Cavg	100YR Ponding Depth	100YR Ponding Area	100YR Ponding Volume	100YR Ponding Depth+20 % Depth	100YR+20 % Depth Ponding Area	100YR+20% Depth Ponding Volume
			0.25	0.90	1.00							
S101	DICB01	0.081	0.081			0.25	0.60	414.3	82.86	0.65	466.7	101.12
S102	CBMH100	0.060	0.012	0.048		0.77	0.20	112.12	7.47	0.25	221.49	18.46
S103	CB02	0.047		0.047		0.90	0.20	189.92	12.66	0.25	296.78	24.73
S104	CBMH101	0.045		0.045		0.90	0.20	185.11	12.34	0.25	279.49	23.29
S105	CBMH103 CBMH108	0.058		0.058		0.90	0.20	150.26	10.02	0.25	286.23	23.85
S106	CBMH107	0.083	0.065	0.018		0.39	0.02	1.94	0.01	0.02	1.94	0.01
S107	CB07 TCB12	0.032	0.024	0.008		0.41	0.04	3.86	0.05	0.09	19.32	0.58
S108	TCB11 LCB10	0.041	0.032	0.009		0.39	0.05	4.18	0.07			
S109	STMH109	0.231			0.231	1.00	Refer to Roof Plan					
S110	CBMH111	0.064	0.064			0.25	0.14	72.59	3.39	0.19	133.70	8.47
S111	CB03 TCB03 LCB01 TCB04	0.142	0.142			0.25	0.10	63.95	2.13	0.15	99.61	4.98
S112	LCB06 TCB07 TCB08 TCB9 TCB05 TCB04	0.212	0.192	0.020		0.31	0.15	119.64	5.98	0.20	192.69	12.85
S113	CBMH106	0.095	0.060	0.035		0.49	0.15	87.44	4.37	0.20	155.46	10.36
S114	CB06	0.093	0.008	0.085		0.84	0.05	10.84	0.18			
S115	CBMH105	0.097	0.091	0.006		0.29	0.20	220.42	14.69	0.25	344.43	28.70
S116	CBMH104	0.070	0.068	0.002		0.27	0.15	98.33	4.92	0.20	174.83	11.66
S117	CB05	0.072	0.072			0.25	0.15	95.84	4.79	0.20	170.39	11.36
Uncontrolled												
U1		0.033	0.019	0.014		0.53						
U2		0.023	0.015	0.008		0.48						
U3		0.048	0.048			0.25						
U4		0.045	0.045			0.25						
U5		0.075	0.071	0.004		0.28						
Total		1.747	1.109	0.407	0.231	0.50		1830.74	165.95		2843.06	280.42

CLIENT:
Écoles catholiques Centre-Est
CLIENT REF. #:
ARCHITECT:
EDWARD J CUHACI & ASSOCIATES ARCHITECTS Inc.
171 Slater St, Suite 100, Ottawa, Ontario, K1P 5H7
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300-2611 QUEENSVIEW DRIVE
OTTAWA ONTARIO CANADA K2B 8K2
TEL.: 1-613-829-2800 | FAX: 1-613-829-8299 | WWW.WSPGROUP.COM

PROJECT:
ÉCOLE ÉLÉMENTAIRE CATHOLIQUE AVALON III
TENTH LINE ROAD
OTTAWA, ON

CONSEIL DES ÉCOLES CATHOLIQUES DU CENTRE-EST
4000, RUE LABELLE, OTTAWA, ON K1J 1A1

KEY PLAN:

SEAL:
LICENSED PROFESSIONAL ENGINEER
D. B. YANG
100230568
2023-06-02
PROVINCE OF ONTARIO

NOT VALID UNLESS SIGNED AND DATED

BEARING NOTE:
BEARING ARE GRID, DERIVED FROM THE CAN-NET REAL TIME NETWORK GPS OBSERVATIONS, REFERENCED TO SPECIFIED CONTROL POINTS 01919680184 AND 01919643761. MTM ZONE 9 (76° 30' WEST LONGITUDE) NAD83 (ORIGINAL)
BENCHMARK #1 N 5033845.03 E 364791.31 Z 88.40
BENCHMARK #2 N 5033883.86 E 364855.06 Z 87.99

ISSUED FOR - REVISION:

IS	RE.	DATE	DESCRIPTION
1		2023-06-02	ISSUED FOR SPA

PROJECT NO.: 221-12984-00
DATE: 2023-06-01
ORIGINAL SCALE: 1:300
IF THIS BAR IS NOT 25mm LONG, ADJUST YOUR PLOTTING SCALE
DESIGNED BY: D.Y./M.S.
DRAWN BY: S.T-R.
CHECKED BY: D.Y.
25mm
DISCIPLINE: CIVIL
TITLE: STORM DRAINAGE AREA PLAN
SHEET NUMBER: C06
SHEET #: OF
ISSUE: ISSUED FOR SPA
REV #: 0
DATE OF: 2023-06-02

CTB: wsp_kingston_24v36.ctb
PLOTTED: 2023-06-02 - 2:40 PM
FILE: v:\10-13 ma 2022 projects\221-12984-00 - cocce avalon french catholic elementary school\14 0 tech_prof services\14.60_civil\3. drawings\working drawings\production drawings\221-12984-00-c06.dwg

New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00

Stormwater Management Summary

Drainage Area I.D.	Location	Sub Area (ha)	Avg. Composite 'C' 5 yr	Avg. Composite 'C' 100 yr	Outlet Location	5 Year Uncontrolled/ Controlled Release (L/s)	5 year Storage Required (m^3)	100 Year Uncontrolled/ Controlled Release (L/s)	100 year Storage Required (m^3)	Total Storage Provided (m^3)
					Total Allowable Release Rate			**186.51**		
CONTROLLED										
S1 - S18	CB01	1.297	0.44	0.51	Sweet Valley Drive	118.30	27.82	119.27	124.69	163.69
S6	STMH01	0.231	1.00	1.00	Sweet Valley Drive	17.64	32.97	17.64	73.48	80.00
UNCONTROLLED										
UC (S5,S6 S15)		0.236	0.310	0.37		21.20		43.30		
					Maximum Release Rate (WSP, 2023)			**180.21**		
Total		1.764				157.14	60.79	180.21	198.17	243.69

New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00

Table 1a - Allowable Release Rate (Pre-Development)

Runoff Coefficient Equation

$C = (A_{hard} \times 0.9 + A_{soft} \times 0.25)/A_{tot}$

2 Year Event

	C	Intensity	Area
5 Year	0.50	76.81	1.747
2.78CIA=	186.51		
	186.51	L/s	

*Use a 10.00 minute time of concentration for 5 year

Design Parameters (DSEL & JFSA, June 2019)

Area ID	Area (HA)	MH	D/S Segment	IMP Ratio	Storage Use (m^3)	2 Year Simulated Flow (L/s)
402 - 404	3.32	CTRL MH1	402	0.80	447.13	567

Note: *Assumed ponding volume.
Assumes that on-site storage will be provided up to the 100 year 3 hour Chicago event

Equations:

Flow Equation

Q = 2.78 x C x I x A

Where:

C is the runoff coefficient

I is the intensity of rainfall, City of Ottawa IDF

Rainfall Intensity = $732.951/(T+6.199)^{\wedge -0.810}$ T= time in minutes

A is the total drainage area

New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00

TABLE 2 - Uncontrolled Flow

Post Dev run-off Coefficient "C"

			2 & 5 Year Event		100 Year Event	
Area	**Surface**	**Ha**	**"C"**	**C_{avg}**	**"C"+25%**	***C_{avg}**
Total	Asphalt	0.026	0.90	**0.31**	0.99	**0.37**
0.236	Roof	0.000	1.00		1.00	
	Grass	0.198	0.25		0.31	

Runoff Coefficient Equation

$C = (A_{hard} \times 0.9 + A_{soft} \times 0.2)/A_{tot}$

$*C = (A_{hard} \times 1.0 + A_{soft} \times 0.25)/A_{tot}$

*Runoff coefficients increased by 25% up to a maximum value of 0.99 for the 100-Year event

Post Dev Free Flow

2 Year Event

Pre Dev.	**C**	**Intensity**	**Area**
5 Year	0.31	104.19	0.236
2.78CIA=	21.19		
21.20	L/S		

**Use a 10 minute time of concentration for 5 year

100 Year Event

Pre Dev.	**C**	**Intensity**	**Area**
100 Year	0.37	178.56	0.236
2.78CIA=	43.35		
43.30	L/S		

**Use a 10 minute time of concentration for 100 year

Equations:

Flow Equation

Q = 2.78 x C x I x A

Where:

C is the runoff coefficient

I is the intensity of rainfall, City of Ottawa IDF

A is the total drainage area

New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00

TABLE 3 - Storage Required for New Avalon III ES

Maximum Allowable Release Rate to Ex. CTRL MH1:
186.51 l/s

Post Dev run-off Coefficient "C"

			2 & 5 Year Event		100 Year Event	
Area	Surface	Ha	"C"	C_{avg}	"C" x 1.25	$C_{100\ avg}$
Total	Asphalt	0.381	0.90	**0.44**	0.99	**0.51**
1.292	Roof		1.00		1.00	
	Grass	0.911	0.25		0.31	

*Areas are approximate based on Architectural site plan and Storm Draiange Area Plan

Runoff Coefficient Equation
$C = (A_{hard} \times 0.9 + A_{soft} \times 0.2)/A_{tot}$
$*C = (A_{hard} \times 1.0 + A_{soft} \times 0.25)/A_{tot}$

*Runoff coefficients increased by 25% up to a maximum value of 0.99 for the 100-Year event

QUANTITY STORAGE REQUIREMENTS - 5 Year

1.292 = Area(ha)
0.44 = C
186.5 l/s = max allowable release rate

Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Controlled Runoff (L/s)	Net Runoff To Be Stored (L/s)	Storage Req'd m^3	Storage Avail m^3
	10	**104.19**	**164.66**	**118.30**	**46.36**	**27.82**	**163.69**
	20	70.25	111.02	118.30	-7.28	-8.74	163.69
	30	53.93	85.23	118.30	-33.08	-59.54	163.69
5 YEAR	40	44.18	69.83	118.30	-48.48	-116.34	163.69
	50	37.65	59.51	118.30	-58.80	-176.39	163.69
	60	32.94	52.06	118.30	-66.24	-238.46	163.69

Orifice #1 Sizing
CBMH108

Event	Flow (L/s)	Head (m)	ORIFICE AREA(m^2)	SQUARE (1-side mm)	CIRC (mmØ)
5 Year	118.30	3.06	0.025	160	180
100 Year	119.27	3.11	0.025	160	180

Orifice Control Sizing
$Q = 0.6 \times A \times (2gh)1/2$
Where:
Q is the release rate in m^3/s
A is the orifice area in m^2
g is the acceleration due to gravity, $9.81 m/s^2$
h is the head of water above the orifice centre in m
d is the diameter of the orifice in m

QUANTITY STORAGE REQUIREMENTS - 100 Year

1.292 = Area(ha)
0.51 = *C
186.5 l/s = max allowable release rate

Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Controlled Runoff (L/s)	Net Runoff To Be Stored (L/s)	Storage Req'd m^3	Storage Avail m^3
	10	**178.56**	**327.08**	**119.27**	**207.82**	**124.69**	**163.69**
	20	119.95	219.72	119.27	100.46	120.55	163.69
100 YEAR	30	91.87	168.28	119.27	49.02	88.23	163.69
	40	75.15	137.65	119.27	18.39	44.12	163.69
	50	63.95	117.15	119.27	-2.11	-6.34	163.69
	60	55.89	102.39	119.27	-16.88	-60.76	163.69
	70	49.79	91.20	119.27	-28.06	-117.86	163.69

Orifice Invert =	84.000	m
Ponding Elevation =	87.200	m
Top of CB Elevation =	87.150	m

Note: Orifice #1 is located on the downstream invert of CBMH108

Equations:

Flow Equation
$Q = 2.78 \times C \times I \times A$
Where:
C is the runoff coefficient
I is the intensity of rainfall, City of Ottawa IDF
A is the total drainage area

New Orleans Catholic ES
Avalon III - Institutional Development
Ottawa, ON
Project: 221-12984-00

TABLE 4 - Proposed Roof Drains

Roof Drains Release Rate

Total Roof Area =	0.231	Ha
Total Roof Ponding Area =	0.185	Ha
Ponding Depth =	0.07 ~ 0.15	m
The flow rate through each Roof Drain will be =	5 ~ 25.0	gpm
	0.32 ~ 1.58	L/s
Estimated Number of Roof Drains =	14.00	
Estimated Total flow rate =	17.64	

TABLE 1. Adjustable Accutrol Flow Rate Settings

Weir Opening Exposed	1"	2"	3"	4"	5"	6"
	Flow Rate (gallons per minute)					
Fully Exposed	5	10	15	20	25	30
3/4	5	10	13.75	17.5	21.25	25
1/2	5	10	12.5	15	17.5	20
1/4	5	10	11.25	12.5	13.75	15
Closed	5	5	5	5	5	5

Post Dev run-off Coefficient "C"

			2 & 5 Year Event		100 Year Event	
Area	Surface	Ha	"C"	C_{avg}	"C" x 1.25	$C_{100\ avg}$
Total	Asphalt		0.90	**1.00**	0.99	**1.00**
0.231	Roof	0.231	1.00		1.00	
	Grass		0.25		0.31	

*Areas are approximate based on Architectural site plan

Runoff Coefficient Equation
$C = (A_{hard} \times 0.9 + A_{soft} \times 0.2)/A_{tot}$
$*C = (A_{hard} \times 1.0 + A_{soft} \times 0.25)/A_{tot}$

*Runoff coefficients increased by 25% up to a maximum value of 0.99 for the 100-Year event

QUANTITY STORAGE REQUIREMENTS - 5 Year

0.231 = Area(ha)
1.00 = C

Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Allowable Runoff (L/s)	Net Runoff To Be Stored (L/s)	Storage Req'd (m^3)	Storage Available* (m^3)
	10	104.19	66.91	17.64	49.27	29.56	80.00
	20	**70.25**	**45.11**	**17.64**	**27.47**	**32.97**	**80.00**
5 YEAR	30	53.93	34.63	17.64	16.99	30.58	80.00
	40	44.18	28.37	17.64	10.73	25.76	80.00
	50	37.65	24.18	17.64	6.54	19.62	80.00

257.340 40
345 54.1752

QUANTITY STORAGE REQUIREMENTS - 100 Year

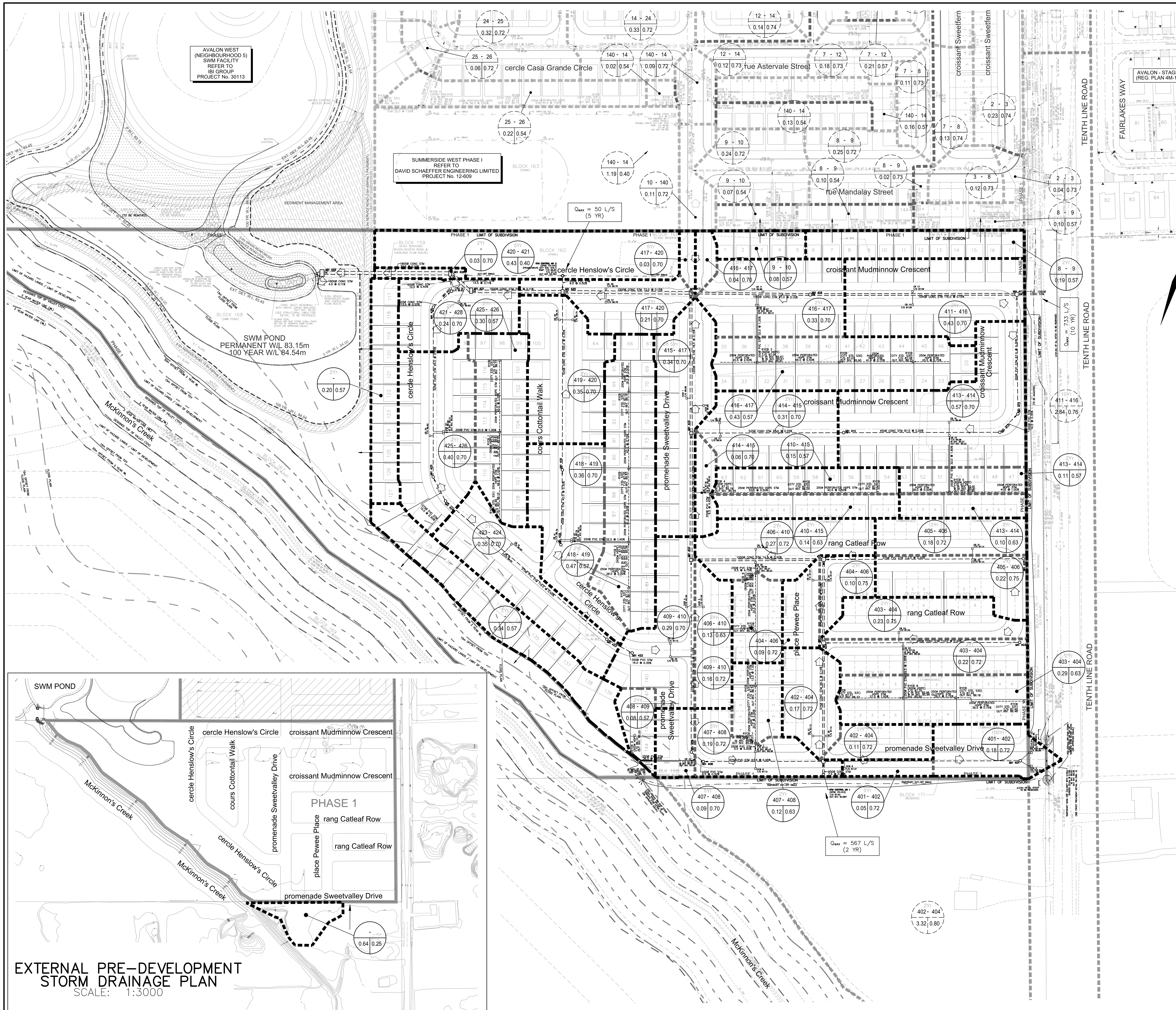
0.231 = Area(ha)
1.00 = *C

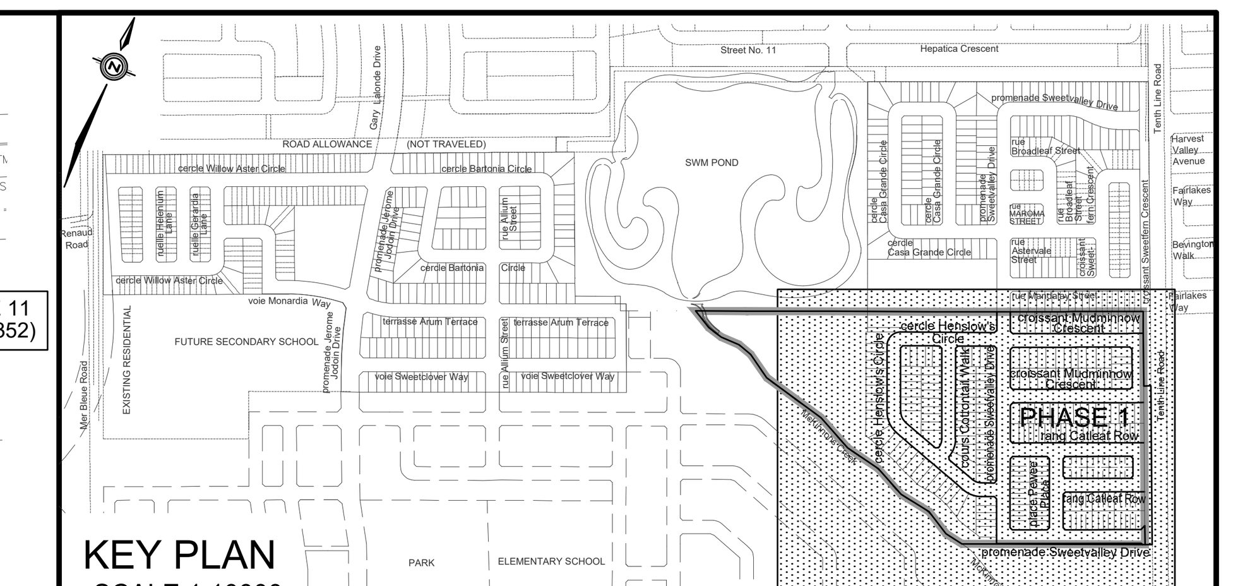
Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Allowable Runoff (L/s)	Net Runoff To Be Stored (L/s)	Storage Req'd (m^3)	Storage Available (m^3)
	10	178.56	114.67	17.64	97.03	58.22	80.00
	20	119.95	77.03	17.64	59.39	71.27	80.00
100 YEAR	**30**	**91.87**	**59.00**	**17.64**	**41.36**	**74.44**	**80.00**
	40	75.15	48.26	17.64	30.62	73.48	80.00
	50	63.95	41.07	17.64	23.43	70.29	80.00
	60	55.89	35.89	17.64	18.25	65.72	80.00
	70	49.79	31.97	17.64	14.33	60.20	80.00

*Storage available is calculated using roof ponding area mulitplied by the maximum ponding depth, and divided by 3 for a conical pond.

Equations:

Flow Equation
Q = 2.78 x C x I x A
Where:
C is the runoff coefficient
I is the intensity of rainfall, City of Ottawa IDF
A is the total drainage area





KEY PLAN
SCALE 1:10000

LEGEND

STORM DRAINAGE BOUNDARY	
SUB-DRAINAGE BOUNDARY	
STORM FREQUENCY	2YI
UPSTREAM MH TO DOWNSTREAM MH	43 - 44
AREA IN HECTARES	0.37
RUNOFF COEFFICIENT	0.51
EXTERNAL 2.78AC =	2.78AC=14.40
EXTERNAL TIME OF CONCENTRATION	TC=14.5 MIN
EXTERNAL BLENDED RUNOFF COEFFICIENT	C=0.70
UPSTREAM MH TO DOWNSTREAM MH	21-23
AREA IN OTHER PHASES IN HECTARES	0.28
RUNOFF COEFFICIENT	0.78
STREET CATCHBASIN & LEAD / MAINTENANCE HOLE	MH202
CURB INLET CATCHBASIN & LEAD / CATCHBASIN/ MAINTENANCE HOLE	CBMH201
INTERCONNECTED CATCH BASIN & LEADS / CAP	
OVERLAND FLOW DIRECTION	
EXTERNAL OVERLAND FLOW DIRECTION	

TOPOGRAPHIC INFORMATION

TOPOGRAPHIC INFORMATION PROVIDED BY J.D. BARNES LIMITED, PROJECT No. 17-10-025-01, SURVEY DATED AUGUST 01, 2018. CITY OF OTTAWA 2K MAPPING, RECEIVED ON MARCH 04, 2013.

LEGAL INFORMATION

CALCULATED M-PLAN PROVIDED BY J.D. BARNES LIMITED, PROJECT No. 17-10-025-00 MP1, RECEIVED ON JUNE 04, 2019.

2nd SUBMISSION 19-06-21

NOT FOR CONSTRUCTION

BENCH MARK No. 01919680229 ELEVATION = 86.120 m

ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM AND ARE DERIVED FROM THE NCC BENCH MARK No. 01919680229 HAVING A PUBLISHED ELEVATION OF 86.120 METRES.

TOWNSHIP: CUMBERLAND

CONCRETE MONUMENT ON THE EAST SIDE OF MER BLEUE ROAD, 0.15KM NORTH OF ROAD INTERSECTION WITH NAVAN ROAD.

No.	DATE	BY	DESCRIPTION
2.	19-06-21	W.L.	2nd SUBMISSION
1.	18-11-15	W.L.	1st SUBMISSION

Ottawa CITY OF OTTAWA

LICENSED PROFESSIONAL ENGINEER W. LIU 100167932 19-06-21 PROVINCE OF ONTARIO

PROJECT No. 15-766

STORM DRAINAGE PLAN

© DSEL

MATTAMY (MER BLEUE 2) LIMITED

SUMMERSIDE SOUTH PHASE 1

DSEL david schaeffer engineering ltd

120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
www.DSEL.ca

DRAWN BY:	V.W./S.L.	CHECKED BY:	P.P./C.M.	DRAWING NO.	SHEET NO.
DESIGNED BY:	P.P./W.L.	CHECKED BY:	K.M.		34
SCALE:	1:1000	DATE:	NOVEMBER 2018		

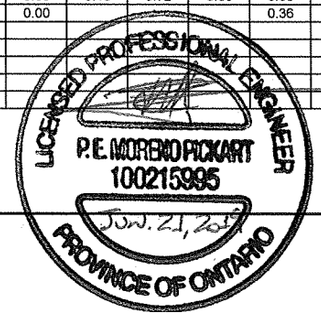
CITY FILE No. D07-16-17-0028 CITY PLAN No. 17857

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
Collector Roads Return Frequency = 5 years
Manning 0.013 Arterial Roads Return Frequency = 10 years

Ottawa

LOCATION			AREA (Ha) 2 YEAR				AREA (Ha) 5 YEAR				AREA (Ha) 10 YEAR				AREA (Ha) 100 YEAR				FLOW						SEWER DATA							
			AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO
Location	From Node	To Node	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)		(%)	(m)	(l/s)	(m/s)	FLOW (min.)	Q/Q full
cours Cottontail Walk																																
	Ctrl MH 2	420			0.00	0.00	0.43	0.40	0.48	0.48			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	50	300	300	PVC	0.50	9.0	68	0.97	0.16	0.73
To cercle Henslow's Circle, Pipe 420 - 421						0.00				0.48				0.00				0.00	10.16														
			0.36	0.70	0.70	0.70			0.00	0.00			0.00	0.00			0.00	0.00														
	418	419	0.47	0.57	0.74	1.45			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	111	450	450	CONC	0.30	42.0	156	0.98	0.71	0.71
	419	420	0.35	0.70	0.68	2.13			0.00	0.00			0.00	0.00			0.00	0.00	10.71	74.17	100.57	117.88	172.29	158	525	525	CONC	0.35	100.0	254	1.18	1.42	0.62
To cercle Henslow's Circle, Pipe 420 - 421						2.13				0.00				0.00				0.00	12.13														
croissant Mudminnow Crescent																																
			0.10	0.63	0.18	0.18			0.00	0.00			0.00	0.00			0.00	0.00														
			0.11	0.57	0.17	0.35			0.00	0.00			0.00	0.00			0.00	0.00														
	413	414	0.57	0.70	1.11	1.46			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	112	450	450	CONC	0.25	87.5	143	0.90	1.63	0.79
					0.00	1.46	0.06	0.70	0.12	0.12			0.00	0.00			0.00	0.00														
	414	415	0.31	0.70	0.60	2.06			0.00	0.12			0.00	0.00			0.00	0.00	11.63	71.08	96.32	112.87	164.94	158	525	525	CONC	0.25	85.5	215	0.99	1.43	0.73
To promenade Sweetvalley Drive, Pipe 415 - 417						2.06				0.12				0.00				0.00	13.06														
	413	412			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	300	300	PVC	0.35	14.5	57	0.81	0.30	0.00
	412	411			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.30	75.68	102.64	120.31	175.87	0	300	300	PVC	0.35	67.0	57	0.81	1.38	0.00
					0.00	0.00			0.00	0.00	2.84	0.76	6.00	6.00			0.00	0.00														
	411	416	0.43	0.70	0.84	0.84			0.00	0.00			0.00	6.00			0.00	0.00	11.68	70.91	96.09	112.60	164.55	735	1050	1050	CONC	0.15	116.0	1058	1.22	1.58	0.69
					0.00	0.84	0.04	0.70	0.08	0.08			0.00	6.00			0.00	0.00														
			0.33	0.70	0.64	1.48			0.00	0.08			0.00	6.00			0.00	0.00														
	416	417	0.43	0.57	0.68	2.16			0.00	0.08			0.00	6.00			0.00	0.00	13.26	66.20	89.63	105.00	153.38	780	1050	1050	CONC	0.15	67.0	1058	1.22	0.91	0.74
To cercle Henslow's Circle, Pipe 417 - 420						2.16				0.08				6.00				0.00	14.18														
place Pewee Place																																
	Ctrl MH 1	402	3.32	0.80	7.38	7.38			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	567	900	900	CONC	0.20	7.5	810	1.27	0.10	0.70
Contribution From promenade Sweetvalley Drive, Pipe 401 - 402						0.00				0.10				0.00				0.36	12.05														
					0.00	7.38	0.11	0.72	0.22	0.32			0.00	0.00			0.00	0.36														
	402	404	0.17	0.72	0.34	7.72			0.00	0.32			0.00	0.00			0.00	0.36	12.05	69.72	94.46	110.68	161.73	627	975	975	CONC	0.15	66.5	868	1.16	0.95	0.72
Contribution From rang Catleaf Row, Pipe 403 - 404						1.43				0.00				0.00				0.00	11.87														
			0.09	0.72	0.18	9.33			0.00	0.32			0.00	0.00			0.00	0.36														
	404	406	0.10	0.75	0.21	9.54			0.00	0.32			0.00	0.00			0.00	0.36	13.01	66.91	90.60	106.14	155.05	723	975	975	CONC	0.20	47.0	1002	1.34	0.58	0.72
To rang Catleaf Row, Pipe 406 - 410						9.54				0.32				0.00				0.36	13.59														
rang Catleaf Row																																
			0.22	0.72	0.44	0.44			0.00	0.00			0.00	0.00			0.00	0.00														
			0.23	0.75	0.48	0.92			0.00	0.00			0.00	0.00			0.00	0.00														
	403	404	0.29	0.63	0.51	1.43			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	110	450	450	CONC	0.30	110.0	156	0.98	1.87	0.70
To place Pewee Place, Pipe 404 - 406						1.43				0.00				0.00				0.00	11.87														
			0.18	0.72	0.36	0.36			0.00	0.00			0.00	0.00			0.00	0.00														
	405	406	0.22	0.75	0.46	0.82			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	63	375	375	PVC	0.30	112.0	96	0.87	2.15	0.66
Contribution From place Pewee Place, Pipe 404 - 406						9.54				0.32				0.00				0.36	13.59														
			0.13	0.63	0.23	10.59			0.00	0.32			0.00	0.00			0.00	0.36														
	406	410	0.27	0.72	0.54	11.13			0.00	0.32			0.00	0.00			0.00	0.36	13.59	65.30	88.40	103.55	151.26	809	1050	1050	CONC	0.20	74.5	1221	1.41	0.88	0.66
To promenade Sweetvalley Drive, Pipe 410 - 415						11.13				0.32				0.00				0.36	14.47														
promenade Sweetvalley Drive																																
					0.00	0.00	0.05	0.72	0.10	0.10			0.00	0.00			0.00	0.00														
	401	402			0.00	0.00			0.00	0.10			0.00	0.00	0.18	0.72	0.36	0.36	10.00	76.81	104.19	122.14	178.56	75	450	450	CONC	0.25	110.5	143	0.90	2.05	0.52
To place Pewee Place, Pipe 402 - 404						0.00				0.10				0.00				0.36	12.05														



Definitions:
Q = 2.78 AIR, where
Q = Peak Flow in Litres per second (L/s)
A = Areas in hectares (ha)
I = Rainfall Intensity (mm/h)
R = Runoff Coefficient

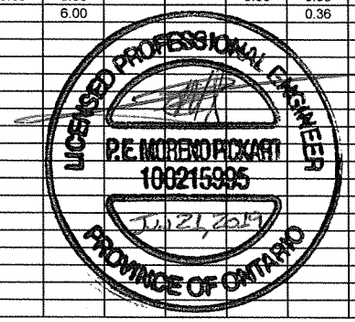
Notes:
1) Ottawa Rainfall-Intensity Curve
2) Min. Velocity = 0.80 m/s

Designed: C.M.K.	PROJECT: SUMMERSIDE SOUTH PHASE 1		
Checked: P.P	LOCATION: City of Ottawa		
Dwg. Reference: Storm Drainage Plan, Dwg. No. 35	File Ref: 15-766	Date: Jun 2019	Sheet No. SHEET 1 OF 2

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
Collector Roads Return Frequency = 5 years
Manning 0.013 Arterial Roads Return Frequency = 10 years

	LOCATION		AREA (Ha) 2 YEAR				AREA (Ha) 5 YEAR				AREA (Ha) 10 YEAR				AREA (Ha) 100 YEAR				FLOW						SEWER DATA								
			AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO
Location	From Node	To Node	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)		(%)	(m)	(l/s)	(m/s)	FLOW (min.)	Q/Q full
					0.00	0.00	0.09	0.70	0.18	0.18			0.00	0.00			0.00	0.00															
			0.12	0.63	0.21	0.21			0.00	0.18			0.00	0.00			0.00	0.00															
	407	408			0.00	0.21	0.19	0.72	0.38	0.56			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	74	375	375	PVC	0.45	67.5	118	1.06	1.06	0.63
	408	409	0.08	0.57	0.13	0.34			0.00	0.56			0.00	0.00			0.00	0.00	11.06	72.97	98.93	115.94	169.45	80	450	450	CONC	0.30	8.5	156	0.98	0.14	0.51
					0.00	0.34			0.00	0.56			0.00	0.00			0.00	0.00															
					0.00	0.34	0.16	0.72	0.32	0.88			0.00	0.00			0.00	0.00															
	409	410			0.00	0.34	0.29	0.70	0.56	1.44			0.00	0.00			0.00	0.00	11.20	72.48	98.25	115.14	168.28	166	600	600	CONC	0.15	108.0	238	0.84	2.14	0.70
Contribution From rang Catleaf Row, Pipe 406 - 410						11.13				0.32				0.00				0.36	14.47														
			0.14	0.63	0.25	11.71			0.00	1.76			0.00	0.00			0.00	0.36															
	410	415	0.15	0.57	0.24	11.95			0.00	1.76			0.00	0.00			0.00	0.36	14.47	63.04	85.30	99.90	145.91	956	1200	1200	CONC	0.15	71.0	1510	1.34	0.89	0.63
Contribution From croissant Mudminnow Crescent, Pipe 414 - 415						2.06				0.12				0.00				0.00	13.06														
	415	417			0.00	14.01	0.34	0.70	0.66	2.54			0.00	0.00			0.00	0.36	15.36	60.93	82.42	96.51	140.93	1114	1200	1200	CONC	0.15	78.5	1510	1.34	0.98	0.74
To cercle Henslow's Circle, Pipe 417 - 420						14.01				2.54				0.00				0.36	16.34														
cercle Henslow's Circle																																	
	422	423			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	300	300	PVC	0.35	16.0	57	0.81	0.33	0.00
	423	424	0.35	0.70	0.68	0.68			0.00	0.00			0.00	0.00			0.00	0.00	10.33	75.56	102.48	120.13	175.60	51	375	375	PVC	0.50	119.0	124	1.12	1.77	0.42
	424	425			0.00	0.68			0.00	0.00			0.00	0.00			0.00	0.00	12.10	69.60	94.29	110.48	161.43	47	375	375	PVC	0.30	15.0	96	0.87	0.29	0.49
			0.30	0.57	0.48	1.16			0.00	0.00			0.00	0.00			0.00	0.00															
	425	426	0.40	0.70	0.78	1.93			0.00	0.00			0.00	0.00			0.00	0.00	12.38	68.72	93.09	109.07	159.35	133	525	525	CONC	0.20	95.5	192	0.89	1.79	0.69
	426	427			0.00	1.93			0.00	0.00			0.00	0.00			0.00	0.00	14.18	63.78	86.32	101.10	147.66	123	525	525	CONC	0.20	13.5	192	0.89	0.25	0.64
To Servicing Block, Pipe 427 - 428						1.93				0.00				0.00				0.00	14.43														
Contribution From promenade Sweetvalley Drive, Pipe 415 - 417						14.01				2.54				0.00				0.36	16.34														
Contribution From croissant Mudminnow Crescent, Pipe 416 - 417						2.16				0.08				6.00				0.00	14.18														
					0.00	16.17	0.03	0.70	0.06	2.67			0.00	6.00			0.00	0.36															
	417	420	0.21	0.70	0.41	16.58			0.00	2.67			0.00	6.00			0.00	0.36	16.34	58.78	79.47	93.04	135.84	1794	1500	1500	CONC	0.15	72.0	2738	1.55	0.77	0.66
Contribution From cours Cottontail Walk, Pipe 419 - 420						2.13				0.00				0.00				0.00	12.13														
Contribution From Park, Pipe Ctrl MH 2 - 420						0.00				0.48				0.00				0.00	10.16														
	420	421			0.00	18.70			0.00	3.15			0.00	6.00			0.00	0.36	17.11	57.19	77.30	90.49	132.10	1904	1500	1500	CONC	0.11	51.5	2344	1.33	0.65	0.81
	421	428	0.24	0.70	0.47	19.17			0.00	3.15			0.00	6.00			0.00	0.36	17.76	55.94	75.59	88.48	129.14	1888	1500	1500	CONC	0.11	14.5	2344	1.33	0.18	0.81
To Servicing Block, Pipe 428 - 429						19.17				3.15				6.00				0.36	17.94														
Servicing Block																																	
Contribution From cercle Henslow's Circle, Pipe 426 - 427						1.93				0.00				0.00				0.00	14.43														
	427	428			0.00	1.93			0.00	0.00			0.00	0.00			0.00	0.00	14.43	63.15	85.45	100.08	146.16	122	600	600	CONC	0.40	10.0	388	1.37	0.12	0.31
Contribution From cercle Henslow's Circle, Pipe 421 - 428						19.17				3.15				6.00				0.36	17.94														
	428	429			0.00	21.11			0.00	3.15			0.00	6.00			0.00	0.36	17.94	55.60	75.12	87.93	128.34	1984	1800	1800	CONC	0.11	73.0	3812	1.50	0.81	0.52
	429	Headwall			0.00	21.11			0.00	3.15			0.00	6.00			0.00	0.36	18.75	54.13	73.11	85.57	124.88	1931	1800	1800	CONC	0.11	4.5	3812	1.50	0.05	0.51
To SWM Pond						21.11				3.15				6.00				0.36	18.80														



Definitions:
Q = 2.78 AIR, where
Q = Peak Flow in Litres per second (L/s)
A = Areas in hectares (ha)
I = Rainfall Intensity (mm/h)
R = Runoff Coefficient

Notes:
1) Ottawa Rainfall-Intensity Curve
2) Min. Velocity = 0.80 m/s

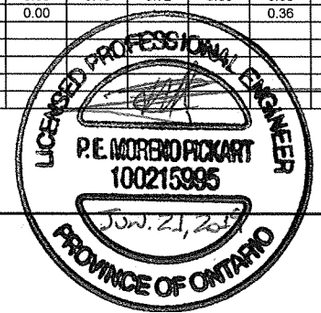
Designed: C.M.K.	PROJECT: SUMMERSIDE SOUTH PHASE 1		
Checked: P.P	LOCATION: City of Ottawa		
Dwg. Reference: Storm Drainage Plan, Dwg. No. 35	File Ref: 15-766	Date: Jun 2019	Sheet No. SHEET 2 OF 2

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
Collector Roads Return Frequency = 5 years
Manning 0.013 Arterial Roads Return Frequency = 10 years

Ottawa

LOCATION			AREA (Ha) 2 YEAR				AREA (Ha) 5 YEAR				AREA (Ha) 10 YEAR				AREA (Ha) 100 YEAR				FLOW						SEWER DATA							
			AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO
Location	From Node	To Node	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)		(%)	(m)	(l/s)	(m/s)	FLOW (min.)	Q/Q full
cours Cottontail Walk																																
	Ctrl MH 2	420			0.00	0.00	0.43	0.40	0.48	0.48			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	50	300	300	PVC	0.50	9.0	68	0.97	0.16	0.73
To cercle Henslow's Circle, Pipe 420 - 421						0.00				0.48				0.00				0.00	10.16														
			0.36	0.70	0.70	0.70			0.00	0.00			0.00	0.00			0.00	0.00														
	418	419	0.47	0.57	0.74	1.45			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	111	450	450	CONC	0.30	42.0	156	0.98	0.71	0.71
	419	420	0.35	0.70	0.68	2.13			0.00	0.00			0.00	0.00			0.00	0.00	10.71	74.17	100.57	117.88	172.29	158	525	525	CONC	0.35	100.0	254	1.18	1.42	0.62
To cercle Henslow's Circle, Pipe 420 - 421						2.13				0.00				0.00				0.00	12.13														
croissant Mudminnow Crescent																																
			0.10	0.63	0.18	0.18			0.00	0.00			0.00	0.00			0.00	0.00														
			0.11	0.57	0.17	0.35			0.00	0.00			0.00	0.00			0.00	0.00														
	413	414	0.57	0.70	1.11	1.46			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	112	450	450	CONC	0.25	87.5	143	0.90	1.63	0.79
					0.00	1.46	0.06	0.70	0.12	0.12			0.00	0.00			0.00	0.00														
	414	415	0.31	0.70	0.60	2.06			0.00	0.12			0.00	0.00			0.00	0.00	11.63	71.08	96.32	112.87	164.94	158	525	525	CONC	0.25	85.5	215	0.99	1.43	0.73
To promenade Sweetvalley Drive, Pipe 415 - 417						2.06				0.12				0.00				0.00	13.06														
	413	412			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	300	300	PVC	0.35	14.5	57	0.81	0.30	0.00
	412	411			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.30	75.68	102.64	120.31	175.87	0	300	300	PVC	0.35	67.0	57	0.81	1.38	0.00
					0.00	0.00			0.00	0.00	2.84	0.76	6.00	6.00			0.00	0.00														
	411	416	0.43	0.70	0.84	0.84			0.00	0.00			0.00	6.00			0.00	0.00	11.68	70.91	96.09	112.60	164.55	735	1050	1050	CONC	0.15	116.0	1058	1.22	1.58	0.69
					0.00	0.84	0.04	0.70	0.08	0.08			0.00	6.00			0.00	0.00														
			0.33	0.70	0.64	1.48			0.00	0.08			0.00	6.00			0.00	0.00														
	416	417	0.43	0.57	0.68	2.16			0.00	0.08			0.00	6.00			0.00	0.00	13.26	66.20	89.63	105.00	153.38	780	1050	1050	CONC	0.15	67.0	1058	1.22	0.91	0.74
To cercle Henslow's Circle, Pipe 417 - 420						2.16				0.08				6.00				0.00	14.18														
place Pewee Place																																
	Ctrl MH 1	402	3.32	0.80	7.38	7.38			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	567	900	900	CONC	0.20	7.5	810	1.27	0.10	0.70
Contribution From promenade Sweetvalley Drive, Pipe 401 - 402						0.00				0.10				0.00				0.36	12.05														
					0.00	7.38	0.11	0.72	0.22	0.32			0.00	0.00			0.00	0.36														
	402	404	0.17	0.72	0.34	7.72			0.00	0.32			0.00	0.00			0.00	0.36	12.05	69.72	94.46	110.68	161.73	627	975	975	CONC	0.15	66.5	868	1.16	0.95	0.72
Contribution From rang Catleaf Row, Pipe 403 - 404						1.43				0.00				0.00				0.00	11.87														
			0.09	0.72	0.18	9.33			0.00	0.32			0.00	0.00			0.00	0.36														
	404	406	0.10	0.75	0.21	9.54			0.00	0.32			0.00	0.00			0.00	0.36	13.01	66.91	90.60	106.14	155.05	723	975	975	CONC	0.20	47.0	1002	1.34	0.58	0.72
To rang Catleaf Row, Pipe 406 - 410						9.54				0.32				0.00				0.36	13.59														
rang Catleaf Row																																
			0.22	0.72	0.44	0.44			0.00	0.00			0.00	0.00			0.00	0.00														
			0.23	0.75	0.48	0.92			0.00	0.00			0.00	0.00			0.00	0.00														
	403	404	0.29	0.63	0.51	1.43			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	110	450	450	CONC	0.30	110.0	156	0.98	1.87	0.70
To place Pewee Place, Pipe 404 - 406						1.43				0.00				0.00				0.00	11.87														
			0.18	0.72	0.36	0.36			0.00	0.00			0.00	0.00			0.00	0.00														
	405	406	0.22	0.75	0.46	0.82			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	63	375	375	PVC	0.30	112.0	96	0.87	2.15	0.66
Contribution From place Pewee Place, Pipe 404 - 406						9.54				0.32				0.00				0.36	13.59														
			0.13	0.63	0.23	10.59			0.00	0.32			0.00	0.00			0.00	0.36														
	406	410	0.27	0.72	0.54	11.13			0.00	0.32			0.00	0.00			0.00	0.36	13.59	65.30	88.40	103.55	151.26	809	1050	1050	CONC	0.20	74.5	1221	1.41	0.88	0.66
To promenade Sweetvalley Drive, Pipe 410 - 415						11.13				0.32				0.00				0.36	14.47														
promenade Sweetvalley Drive																																
					0.00	0.00	0.05	0.72	0.10	0.10			0.00	0.00			0.00	0.00														
	401	402			0.00	0.00			0.00	0.10			0.00	0.00	0.18	0.72	0.36	0.36	10.00	76.81	104.19	122.14	178.56	75	450	450	CONC	0.25	110.5	143	0.90	2.05	0.52
To place Pewee Place, Pipe 402 - 404						0.00				0.10				0.00				0.36	12.05														



Definitions:
Q = 2.78 AIR, where
Q = Peak Flow in Litres per second (L/s)
A = Areas in hectares (ha)
I = Rainfall Intensity (mm/h)
R = Runoff Coefficient

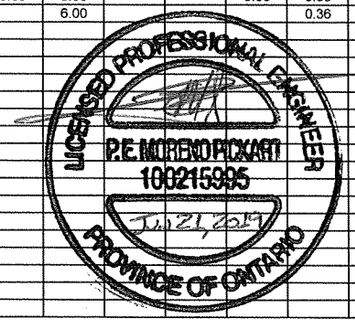
Notes:
1) Ottawa Rainfall-Intensity Curve
2) Min. Velocity = 0.80 m/s

Designed: C.M.K.	PROJECT: SUMMERSIDE SOUTH PHASE 1		
Checked: P.P	LOCATION: City of Ottawa		
Dwg. Reference: Storm Drainage Plan, Dwg. No. 35	File Ref: 15-766	Date: Jun 2019	Sheet No. SHEET 1 OF 2

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
Collector Roads Return Frequency = 5 years
Manning 0.013 Arterial Roads Return Frequency = 10 years

	LOCATION		AREA (Ha) 2 YEAR				AREA (Ha) 5 YEAR				AREA (Ha) 10 YEAR				AREA (Ha) 100 YEAR				FLOW						SEWER DATA								
			AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	AREA	R	Indiv.	Accum.	Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO
Location	From Node	To Node	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	(Ha)		2.78 AC	2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)		(%)	(m)	(l/s)	(m/s)	FLOW (min.)	Q/Q full
					0.00	0.00	0.09	0.70	0.18	0.18			0.00	0.00			0.00	0.00															
			0.12	0.63	0.21	0.21			0.00	0.18			0.00	0.00			0.00	0.00															
	407	408			0.00	0.21	0.19	0.72	0.38	0.56			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	74	375	375	PVC	0.45	67.5	118	1.06	1.06	0.63
	408	409	0.08	0.57	0.13	0.34			0.00	0.56			0.00	0.00			0.00	0.00	11.06	72.97	98.93	115.94	169.45	80	450	450	CONC	0.30	8.5	156	0.98	0.14	0.51
					0.00	0.34			0.00	0.56			0.00	0.00			0.00	0.00															
					0.00	0.34	0.16	0.72	0.32	0.88			0.00	0.00			0.00	0.00															
	409	410			0.00	0.34	0.29	0.70	0.56	1.44			0.00	0.00			0.00	0.00	11.20	72.48	98.25	115.14	168.28	166	600	600	CONC	0.15	108.0	238	0.84	2.14	0.70
Contribution From rang Catleaf Row, Pipe 406 - 410						11.13				0.32				0.00				0.36	14.47														
			0.14	0.63	0.25	11.71			0.00	1.76			0.00	0.00			0.00	0.36															
	410	415	0.15	0.57	0.24	11.95			0.00	1.76			0.00	0.00			0.00	0.36	14.47	63.04	85.30	99.90	145.91	956	1200	1200	CONC	0.15	71.0	1510	1.34	0.89	0.63
Contribution From croissant Mudminnow Crescent, Pipe 414 - 415						2.06				0.12				0.00				0.00	13.06														
	415	417			0.00	14.01	0.34	0.70	0.66	2.54			0.00	0.00			0.00	0.36	15.36	60.93	82.42	96.51	140.93	1114	1200	1200	CONC	0.15	78.5	1510	1.34	0.98	0.74
To cercle Henslow's Circle, Pipe 417 - 420						14.01				2.54				0.00				0.36	16.34														
cercle Henslow's Circle																																	
	422	423			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	300	300	PVC	0.35	16.0	57	0.81	0.33	0.00
	423	424	0.35	0.70	0.68	0.68			0.00	0.00			0.00	0.00			0.00	0.00	10.33	75.56	102.48	120.13	175.60	51	375	375	PVC	0.50	119.0	124	1.12	1.77	0.42
	424	425			0.00	0.68			0.00	0.00			0.00	0.00			0.00	0.00	12.10	69.60	94.29	110.48	161.43	47	375	375	PVC	0.30	15.0	96	0.87	0.29	0.49
			0.30	0.57	0.48	1.16			0.00	0.00			0.00	0.00			0.00	0.00															
	425	426	0.40	0.70	0.78	1.93			0.00	0.00			0.00	0.00			0.00	0.00	12.38	68.72	93.09	109.07	159.35	133	525	525	CONC	0.20	95.5	192	0.89	1.79	0.69
	426	427			0.00	1.93			0.00	0.00			0.00	0.00			0.00	0.00	14.18	63.78	86.32	101.10	147.66	123	525	525	CONC	0.20	13.5	192	0.89	0.25	0.64
To Servicing Block, Pipe 427 - 428						1.93				0.00				0.00				0.00	14.43														
Contribution From promenade Sweetvalley Drive, Pipe 415 - 417						14.01				2.54				0.00				0.36	16.34														
Contribution From croissant Mudminnow Crescent, Pipe 416 - 417						2.16				0.08				6.00				0.00	14.18														
					0.00	16.17	0.03	0.70	0.06	2.67			0.00	6.00			0.00	0.36															
	417	420	0.21	0.70	0.41	16.58			0.00	2.67			0.00	6.00			0.00	0.36	16.34	58.78	79.47	93.04	135.84	1794	1500	1500	CONC	0.15	72.0	2738	1.55	0.77	0.66
Contribution From cours Cottontail Walk, Pipe 419 - 420						2.13				0.00				0.00				0.00	12.13														
Contribution From Park, Pipe Ctrl MH 2 - 420						0.00				0.48				0.00				0.00	10.16														
	420	421			0.00	18.70			0.00	3.15			0.00	6.00			0.00	0.36	17.11	57.19	77.30	90.49	132.10	1904	1500	1500	CONC	0.11	51.5	2344	1.33	0.65	0.81
	421	428	0.24	0.70	0.47	19.17			0.00	3.15			0.00	6.00			0.00	0.36	17.76	55.94	75.59	88.48	129.14	1888	1500	1500	CONC	0.11	14.5	2344	1.33	0.18	0.81
To Servicing Block, Pipe 428 - 429						19.17				3.15				6.00				0.36	17.94														
Servicing Block																																	
Contribution From cercle Henslow's Circle, Pipe 426 - 427						1.93				0.00				0.00				0.00	14.43														
	427	428			0.00	1.93			0.00	0.00			0.00	0.00			0.00	0.00	14.43	63.15	85.45	100.08	146.16	122	600	600	CONC	0.40	10.0	388	1.37	0.12	0.31
Contribution From cercle Henslow's Circle, Pipe 421 - 428						19.17				3.15				6.00				0.36	17.94														
	428	429			0.00	21.11			0.00	3.15			0.00	6.00			0.00	0.36	17.94	55.60	75.12	87.93	128.34	1984	1800	1800	CONC	0.11	73.0	3812	1.50	0.81	0.52
	429	Headwall			0.00	21.11			0.00	3.15			0.00	6.00			0.00	0.36	18.75	54.13	73.11	85.57	124.88	1931	1800	1800	CONC	0.11	4.5	3812	1.50	0.05	0.51
To SWM Pond						21.11				3.15				6.00				0.36	18.80														



Definitions:
Q = 2.78 AIR, where
Q = Peak Flow in Litres per second (L/s)
A = Areas in hectares (ha)
I = Rainfall Intensity (mm/h)
R = Runoff Coefficient

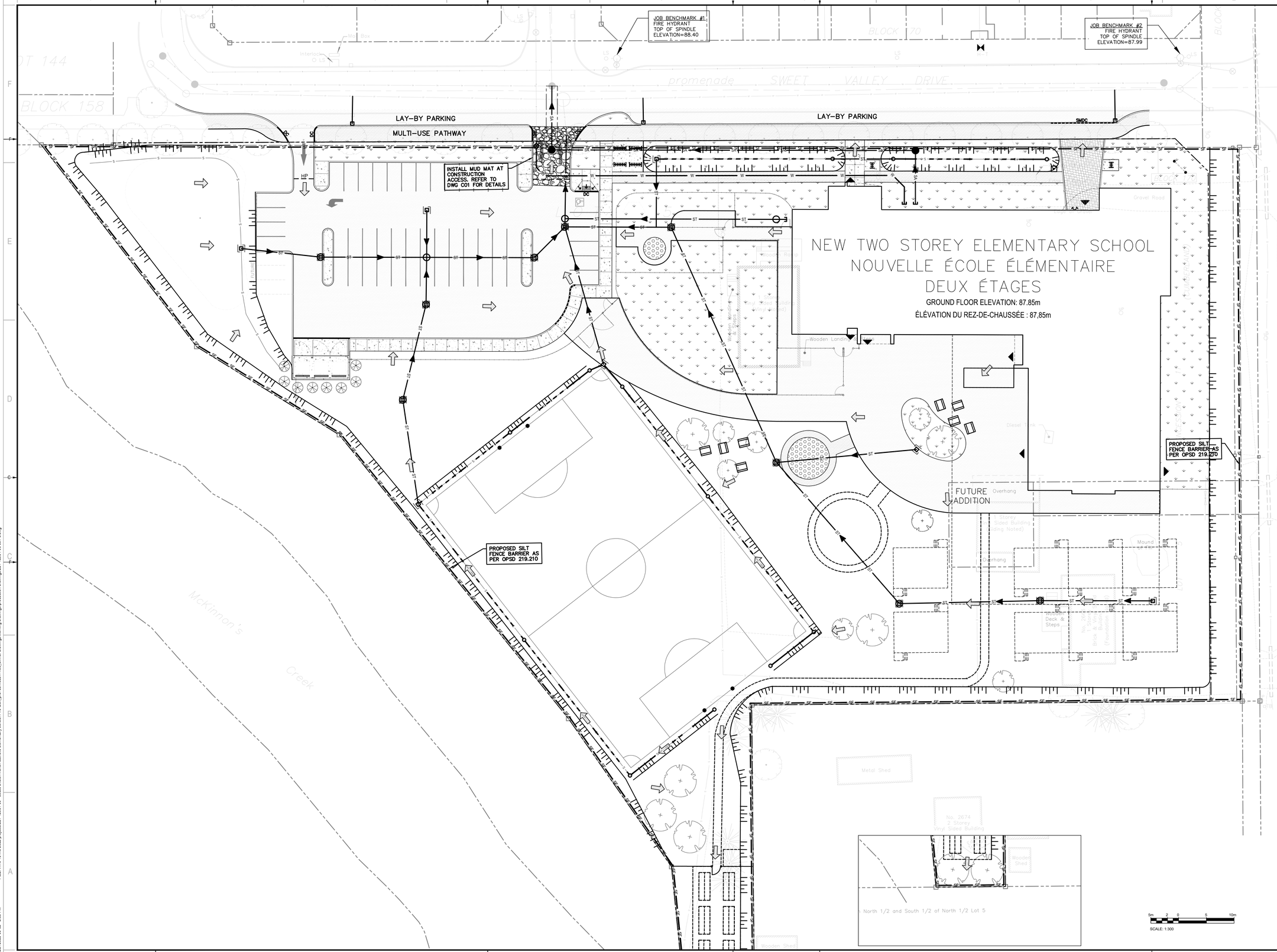
Notes:
1) Ottawa Rainfall-Intensity Curve
2) Min. Velocity = 0.80 m/s

Designed: C.M.K.	PROJECT: SUMMERSIDE SOUTH PHASE 1		
Checked: P.P	LOCATION: City of Ottawa		
Dwg. Reference: Storm Drainage Plan, Dwg. No. 35	File Ref: 15-766	Date: Jun 2019	Sheet No. SHEET 2 OF 2

APPENDIX

E

- DWG C04 – EROSION AND SEDIMENT CONTROL PLAN



CLIENT:

Écoles catholiques Centre-Est

CLIENT REF. #:

ARCHITECT:

EDWARD J CUHACI & ASSOCIATES ARCHITECTS Inc.
171 Slater St, Suite 100, Ottawa, Ontario, K1P 5H7
Fax: (613) 236-1944 Telephone: (613) 236-7135 E-mail: info@cuhaci.com

wsp
300-2611 QUEENSVIEW DRIVE
OTTAWA ONTARIO CANADA K2B 8K2
TEL.: 1-613-829-2800 | FAX: 1-613-829-8299 | WWW.WSPGROUP.COM

PROJECT:

ÉCOLE ÉLÉMENTAIRE CATHOLIQUE AVALON III

TENTH LINE ROAD
OTTAWA, ON

CONSEIL DES ÉCOLES CATHOLIQUES DU CENTRE-EST
4000, RUE LABELLE, OTTAWA, ON K1J 1A1

KEY PLAN:

SEAL:

LICENSED PROFESSIONAL ENGINEER
D. B. YANG
100230568
2023-06-02
PROVINCE OF ONTARIO

NOT VALID UNLESS SIGNED AND DATED

BEARING NOTE:

BEARING ARE GRID, DERIVED FROM THE CAN-NET REAL TIME NETWORK GPS OBSERVATIONS, REFERENCED TO SPECIFIED CONTROL POINTS 01919680184 AND 01919843476?, MTM ZONE 9 (76°30' WEST LONGITUDE) NAD83 (ORIGINAL)
BENCHMARK #1 N:5033845.03 E:384791.31 Z:88.40
BENCHMARK #2 N:5033883.86 E:384895.06 Z:87.99

ISSUED FOR - REVISION:

IS	RE	DATE	DESCRIPTION
1		2023-06-02	ISSUED FOR SPA

PROJECT NO.: 221-12984-00
DATE: 2023-06-01
ORIGINAL SCALE: 1:300
DESIGNED BY: D.Y./M.S.
DRAWN BY: S.T-R.
CHECKED BY: D.Y.

IF THIS BAR IS NOT 25mm LONG, ADJUST YOUR PLOTTING SCALE.
25mm

DISCIPLINE: CIVIL

TITLE:

EROSION AND SEDIMENT CONTROL PLAN

SHEET NUMBER: C04
SHEET #: OF

ISSUE: ISSUED FOR SPA
DATE OF: 2023-06-02
REV #: 0

CTB: wsp_kingston_24x36.ctb
PLOTTED: 2023-06-02 - 2:39 PM
FILE: v:\10-13 ms 2022 projects\221-12984-00 - cscce avalon french catholic elementary school\14 0 tech_prof services\14.60_civil\3. drawing\working drawings\production drawings\221-12984-00 c01-6.dwg

APPENDIX

F

- SUBMISSION CHECK LIST

4.1 General Content

☒ Executive Summary (for larger reports only).

Comments: Refer to Servicing Report Section 1.1

☒ Date and revision number of the report.

Comments: *Refer to front page of the Report*

☒ Location map and plan showing municipal address, boundary, and layout of proposed development.

Comments: *Refer to Figure 1.1 Site Location for Location Map and Plan*

☒ Plan showing the site and location of all existing services.

Comments: *Refer to drawing C03*

☒ Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.

Comments: *Refer to Architectural Site Plan*

☒ Summary of Pre-consultation Meetings with City and other approval agencies.

Comments: Refer to Appendix A for Pre-Consultation Meeting Notes

☒ Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defendable design criteria.

Comments: *N/A*

☒ Statement of objectives and servicing criteria.

Comments: Refer to Servicing Report Section 1.7

☒ Identification of existing and proposed infrastructure available in the immediate area.

Comments: *Refer to drawing C03*

☒ Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).

Comments: *N/A*

☒ Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.

Comments: *Refer to drawing C02*

☒ Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.

Comments: *N/A*

☒ Proposed phasing of the development, if applicable.

Comments: *N/A*

☒ Reference to geotechnical studies and recommendations concerning servicing.

Comments: *N/A*

☒ All preliminary and formal site plan submissions should have the following information:

- ☒ Metric scale
- ☒ North arrow (including construction North)
- ☒ Key plan
- ☒ Name and contact information of applicant and property owner
- ☒ Property limits including bearings and dimensions
- ☒ Existing and proposed structures and parking areas
- ☒ Easements, road widening and rights-of-way
- ☒ Adjacent street names

Comments: *Refer to drawing C02 to C06*

4.2 Development Servicing Report: Water

☒ Confirm consistency with Master Servicing Study, if available

Comments: *Refer to Servicing Report Section 2.1*

☒ Availability of public infrastructure to service proposed development

Comments: Refer to Servicing Report Section 2.1

☒ Identification of system constraints

Comments: N/A

☒ Identify boundary conditions

Comments: *Refer to Servicing Report Section 2.2*

☒ Confirmation of adequate domestic supply and pressure

Comments: *Refer to Servicing Report Section 2.3*

☒ Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.

Comments: *Refer to Servicing Report Section 2.4*

☒ Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.

Comments: *Refer to Servicing Report Section 2.5*

☒ Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design

Comments: *Refer to Servicing Report Section 2.6*

☒ Address reliability requirements such as appropriate location of shut-off valves

Comments: *Refer to Servicing Report Section 2.7*

☒ Check on the necessity of a pressure zone boundary modification.

Comments: *Refer to Servicing Report Section 2.8*

☒ Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range

Comments: *Refer to Servicing Report Section 2.9*

☒ Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.

Comments: *Refer to Servicing Report Section 2.10*

☒ Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.

Comments: *Refer to Servicing Report Section 2.11*

☒ Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.

Comments: *Refer to Servicing Report Section 2.12*

☒ Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.

Comments: *Refer to Servicing Report Section 2.13*

4.3 Development Servicing Report: Wastewater

☒ Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).

Comments: *Refer to Servicing Report Section 3.1*

☒ Confirm consistency with Master Servicing Study and/or justifications for deviations.

Comments: *Refer to Servicing Report Section 3.2*

☒ Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.

Comments: *Refer to Servicing Report Section 3.3*

☒ Description of existing sanitary sewer available for discharge of wastewater from proposed development.

Comments: *Refer to Servicing Report Section 3.4*

☒ Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)

Comments: *Refer to Servicing Report Section 3.5*

☒ Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.

Comments: Refer to Servicing Report Section 3.9 and 3.11

☒ Special considerations such as contamination, corrosive environment etc.

Comments: *Refer to Servicing Report Section 3.8*

4.4 Development Servicing Report: Stormwater

☒ Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)

Comments: *Refer to Servicing Report Section 4.1*

☒ Analysis of available capacity in existing public infrastructure.

Comments: *Refer to Servicing Report Section 4.2*

☒ A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.

Comments: *Refer to drawing C02*

☒ Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.

Comments: *Refer to Servicing Report Section 4.4*

☒ Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.

Comments: *Refer to Servicing Report Section 4.5*

☒ Description of the stormwater management concept with facility locations and descriptions with references and supporting information.

Comments: *Refer to Servicing Report Section 4.6-4.10*

☒ Set-back from private sewage disposal systems.

Comments: *N/A*

☒ Watercourse and hazard lands setbacks.

Comments: N/A

☒ Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.

Comments: N/A

☒ Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.

Comments: *N/A*

☒ Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).

Comments: *Refer to Servicing Report Section 4.6-4.10 and Appendix D*

☒ Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.

Comments: *Refer to Servicing Report Section 4.11*

☒ Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.

Comments: *Refer to Servicing Report Section 4.12*

☒ Any proposed diversion of drainage catchment areas from one outlet to another.

Comments: *Refer to Servicing Report Section 4.13*

☒ Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.

Comments: Refer to Servicing Report Section 4.6-4.10, Appendix D and drawing C03

☒ If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.

Comments: Refer to Servicing Report Section 4.6-4.10 and Appendix D

☒ Identification of potential impacts to receiving watercourses

Comments: *Refer to Servicing Report Section 4.15*

☒ Identification of municipal drains and related approval requirements.

Comments: *Refer to Servicing Report Section 4.16*

☒ Descriptions of how the conveyance and storage capacity will be achieved for the development.

Comments: *Refer to Servicing Report Section 4.17*

☒ 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.

Comments: Refer to drawings C02 and C03

☒ Inclusion of hydraulic analysis including hydraulic grade line elevations.

Comments: *Refer to Servicing Report Section 4.18*

☒ Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.

Comments: *Refer to Servicing Report Section 5.0*

☒ Identification of floodplains - proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.

Comments: *N/A*

☒ Identification of fill constraints related to floodplain and geotechnical investigation.

Comments: *N/A*

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

☐ Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.

Comments: Not applicable.

☐ Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.

Comments: Not applicable.

☐ Changes to Municipal Drains.

Comments: *Not applicable.*

☐ Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)

Comments: *Not applicable.*

4.6 Conclusion Checklist

☒ Clearly stated conclusions and recommendations

Comments:

☐ Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.

Comments: *Further comments to be added following site plan application review.*

☒ All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario

Comments: