SERVICEABILITY REPORT REPORT No. R-821-157A

T.L. MAK ENGINEERING CONSULTANTS LTD.

JUNE 2023

REFERENCE FILE NUMBER 821-157

### Introduction

The developer of this site is proposing to redevelop the existing residential lot described as Lot 3 Registered Plan 267570 City of Ottawa by constructing a three (3) storey residential apartment building consisting of eighteen (18) units, including two (2) 2-bedroom units, fourteen (14) 1-bedroom units and two (2) studio/bachelor units.

The municipal address of the property is referenced as 1240 Carling Avenue and it is located in the City Ward (16 - River). The site is situated on the south side of Carling Avenue, west of Viscount Avenue and east of Merivale Road. See site plan and legal survey plan in **Appendix A** for details.

The area of this property is ±0.045 hectares. In addition to the (3)-storey residential building, the other development features will comprise of concrete walkway access to the front entrance off Carling Avenue, a barrier free entrance facing Merivale Road, including an amenity area located in the rear yard, as well as a vehicle parking space at grade, soft landscaping is also proposed along the front (northside) of the building with additional landscaped areas throughout the site, etc., to meet the City of Ottawa's site plan requirements.

A site geotechnical report was prepared for this site by the owner's soils engineer, GEMTEC entitled Geotechnical Investigation – Proposed Residential Development (Project No. 100382.003 dated February 10, 2023) for this proposed development property.

This serviceability report will provide the City of Ottawa with our serviceability brief to address the proposed servicing scheme for this site.

### **Existing Site Conditions and Servicing**

This property is currently occupied by a 1½-storey vinyl-stucco, stone siding residential building. The existing house currently has its own asphalt driveway off of Carling Avenue for vehicle access and parking. Most of the existing site is currently permeable surface covered and consisting of grass/landscaped areas with the remaining areas being roof area, asphalt laneway, porches, decks and a shed. For additional details of the site's pre-development conditions, refer to the coloured Google Image (2023) and aerial photography from (GeoOttawa 2021) in Appendix B.

The topography of the land is found to be gently sloping from south to north or (back to front) with an average slope of  $\pm 2.5\%$ .

The existing house water services and sanitary lateral currently servicing the existing dwelling on 1240 Carling Avenue will be removed. The existing water services shall be blanked at the

main and the existing house laterals shall be capped at the front property line as part of the requirements for re-development of this site.

As for the availability of underground municipal services, there are existing municipal services along Carling Avenue in front of this property consisting of a 375mm diameter storm sewer, a 225mm diameter sanitary sewer and a 150mm diameter watermain for development of this property. Refer to the City of Ottawa Carling Avenue and Merivale Road UCC drawing and plan and profile drawings included in **Appendix C** for details.

Because the site is located within a separated sewer shed, therefore, the approval exemption under Ontario Regulations 525/98 would apply since storm water discharges from this site will outlet flow into a storm sewer. Thus, an Environmental Compliance Approval (ECA) application will not be required to be submitted to the Ministry.

### **Proposed Residential Apartment Building Site**

Vehicle access to this site will not be required for development of this property. A concrete surface parking space is proposed mainly for vehicle access for deliveries to the residential entrance at the front of the building. Concrete walkways are proposed along the north and east side of the new building for pedestrian access to front and side entrances.

### A. Water Supply

The proposed building located within Pressure Zone 2W at 1240 Carling Avenue is a 3-storey residential building. The building contains eighteen (18) residential units, namely two (2) studios/bachelor, fourteen (14) 1-bedroom units, and two (2) 2-bedroom units. Three (3) 1-bedroom units will be constructed at basement level. Each floor covers an area of approximately 256 m², for a total gross floor area of approximately 1,027 m², including the basement level. The building will be serviced by the 305 mm dia. watermain along Merivale Road or the 152 mm dia. watermain along Carling Avenue.

The ground elevation on the property is approximately 79.5 m, as obtained from geoOttawa elevation contours (Figure 1 in Appendix D), and the Site Survey Plan provided to Stantec (see attached Site Survey Plan in Appendix D).

### **Demand Projections**

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines. For residential units, a consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 2.5. Peak hour (PKHR) demands were calculated by multiplying MXDY by a factor of 2.2. Persons per unit (PPU) for each unit were estimated based on the City of

Ottawa's Water Design Guidelines. **Table 1** shows the estimated domestic demands of the proposed building.

Table 1: Estimated Domestic Demand

Unit Type	Unit Count	PPU	Consumption	AVDY		MXDY		PKHR	
				L/d	L/s	L/d	L/s	L/d	L/s
Apartment, Bachelor	2	1.4	280	784	0.01	1,960	0.02	4,312	0.05
Apartment, 1-Bedroom	14	1.4	280	5,488	0.06	13,720	0.16	30,184	0.35
Apartment, 2-Bedroom	2	2.1	280	1,176	0.01	2,940	0.03	6,468	0.07
Total	18			7,448	0.09	18,620	0.22	40,964	0.47

The fire flow requirement was determined following the Fire Underwriter Survey (FUS) method. For this analysis, the building was classified as wood frame construction, with building contents that are limited in combustibility. It is understood that the building will not have a sprinkler system. The resulting total required fire flow (RFF) is 14,000 L/min (233 L/s) for a duration of 3 hours. Details are provided in the attached FUS Fire Flow Calculations in Appendix D. The proposed Site Plan attached in Appendix D was used to determine distances from the proposed building to the property lines. Figure 2 in Appendix D provides separation distances from adjacent buildings.

The City has previously indicated that the City's Fire Marshall and various City departments were reviewing fire flow requirements for low- and mid-rise buildings. As per the City of Ottawa's Water Design Guidelines, the FUS method is to be used for fire flow requirements affecting watermain sizing; with regards to fire protection on private property and not requiring new watermains, these are covered by the Ontario Building Code (OBC).

As such, the fire flow requirement was also determined following the OBC's Office of the Fire Marshal (OFM) method. The proposed building will be of wood frame construction, where floors are fire separations, but without fire-resistance ratings. The resulting total required fire flow (RFF) for a non-sprinklered building is 5,400 L/min (90 L/s) for a minimum duration of 40 min. Details are provided in the attached **OFM Fire Flow Calculations** in **Appendix D**. **Figure 3** in **Appendix D** provides separation distances between the property line to the street centreline.

In summary, the estimated water demands for the proposed building are as follows:

- AVDY = 7,448 L/d (0.09 L/s)
- MXDY = 18,620 L/d (0.22 L/s);
- PKHR = 40,964 L/d (0.47 L/s); and,
- Fire Flow (FUS) = 14,000 L/min (233 L/s);
- Fire Flow (OFM) = 5,400 L/min (90 L/s).

### **Boundary Conditions**

The hydraulic gradeline (HGL) boundary conditions for 1240 Carling Avenue, as presented in **Table 2**, were provided by the City on June 1, 2023 (see attached **Water Boundary Conditions Email** in **Appendix D**).

**Table 2: Boundary Conditions** 

Demand Scenario	Head (m)		
Minimum HGL <sup>1</sup> (Peak Hour)	124.4		
Maximum HGL <sup>1</sup> (Average Day)	132.8		
Maximum Day + Fire Flow <sup>2</sup> (FUS – 14,000 L/min) – Connection at the 305 mm dia. watermain along Merivale Road	102.9		

<sup>&</sup>lt;sup>1</sup> Resulting boundary condition valid for either connection to the existing network (i.e., watermain along Merivale Road or Carling Avenue).

### **Hydraulic Analysis**

### Peak Hour & Average Day

During average day demands, the resulting maximum hydraulic gradeline of 132.8 m corresponds to a maximum pressure of 558 kPa (81 psi). This value is slightly above the maximum pressure objective of 552 kPa (80 psi). As per the City guidelines, pressures exceeding 552 kPa (80 psi) require reduction mitigation, which could be achieved with a pressure reducing valve along the service line to the proposed building.

During peak hour demands, the resulting minimum hydraulic gradeline of 124.4 m corresponds to a peak hour pressure of 475 kPa (69 psi). This value is above the minimum pressure objective of 276 kPa (40 psi) for residential buildings up to two storeys. From a servicing perspective, the peak hour pressure exceeds this objective and is therefore considered acceptable. Given that this building consists of a total of 3 storeys, further consideration will be needed to service the higher floors. Adding 5 psi per floor above two stories (i.e., 1 additional floor), a minimum pressure of 310 kPa (45 psi) would be required for the third floor. The peak hour pressure calculated is equal to this objective and is therefore considered acceptable.

Supporting hydraulic calculations are attached in Appendix D.

### Maximum Day + Fire Flow

At the proposed connection to the 305 mm dia. watermain along Merivale Road, a maximum day plus fire flow (14,000 L/min as per FUS) hydraulic gradeline of 102.9 m corresponds to a

<sup>&</sup>lt;sup>2</sup> The City noted that a fire flow of 9,300 L/min (155 L/s) at 20 psi residual pressure is available for the connection at the 152 mm dia. watermain along Carling Avenue.

residual pressure of 265 kPa (38 psi) at this location, which is above the minimal residual pressure requirement of 140 kPa (20 psi).

Along the proposed connection on Carling Avenue (152 mm dia. watermain), the available fire flow is 9,300 L/min (155 L/s) at 20 psi residual. This is less than the required fire flow obtained from the FUS Guidelines. However, the available fire flow is greater than the RFF obtained from the OFM method (90 L/s or 5,400 L/min). As the City previously indicated that the Ontario Building Code (OFM method) covers the servicing of buildings which do not require new watermains, the connection along Carling Avenue is also deemed acceptable.

Based on Table 1 of Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02 and a desktop review (i.e., Google Street View) to confirm hydrant class, four (4) hydrants are located in the vicinity of the proposed building. Two (2) Class AA hydrants are within 75 m, both with a capacity contribution of up to 5,700 L/min. Two (2) others Class AA hydrants are within 150 m from the site, both with a capacity contribution of up to 3,800 L/min. The combined hydrant flow coverage for 1240 Carling Avenue is therefore 19,000 L/min, which is above the RFF obtained from both the FUS (14,000 L/min), and the OFM (5,400 L/min) methods.

The hydrant coverage is illustrated in **Figure 4** attached in **Appendix D**. A breakdown of the hydrant coverage is summarized in **Table 3**.

Table 3: Minimum Fire Hydrant Coverage

Building			Minimum					
	Calculated Fire Flow Demand	Hydrant	Withi	in 75 m	Between 15	Combined Hydrant		
	(L/min)	Class	Quantity	Contrib. to RFF	Quantity	Contrib. to	Flow Coverage (L/min)	
1240 Carling Avenue	14,000 (FUS); 5,400 (OFM)	AA	2	5,700	2	3,800		
		Α					19,000	
		В						
		С				2012		

In conclusion, based on the boundary condition provided, the 305 mm dia. watermain on Merivale Road or the 152 mm dia. watermain on Carling Avenue provides adequate fire flow capacity as per the Ontario Building Code's Office of the Fire Marshal (OFM) method for the proposed development at 1240 Carling Avenue. The 305 mm dia. watermain on Merivale Road also provides sufficient flow to meet the Fire Underwriter Survey (FUS) required fire flow.

Resulting pressures during anticipated demand flows meet the pressure objectives during peak demand conditions, as per the City of Ottawa's Drinking Water Design Guidelines. However, during average day demand conditions, the expected maximum pressure is slightly above the

maximum pressure objective of 552 kPa (80 psi). Thus, an individual pressure reducing valve is required along the service line to the proposed building.

### B. Sanitary Flow

The peak sanitary flow for the proposed eighteen (18) units, which comprise of fourteen (14) 1-bedroom units, two (2) 2-bedroom units and two (2) studio/bachelor units, is estimated at Q = 0.37 L/s with an infiltration rate of 0.02 L/s. Please refer to **Appendix E** regarding sanitary flow calculations. This flow will enter the existing 225mm diameter sanitary sewer on Carling Avenue via a proposed new 150mm diameter PVC sanitary service lateral from the three (3)-storey residential apartment building.

The existing peak sanitary flow of this site with a single detached dwelling unit is Q = 0.06 L/s with an infiltration rate of 0.02 L/s. The net increase in flow from this proposed development is 0.31 L/s.

Waste water from the existing Carling Avenue 225mm dia. sanitary sewer located in front of this site then in turn outlets further west and north along Merivale Road and into the existing 1050mm dia. Merivale Road collector sanitary sewer. Waste water flow from this collector sewer then flows further north into the existing 1200mm dia. sewer at Faraday Street.

### C. Storm Flow

The storm-water outlet for the proposed development property will be the existing 375mm diameter storm sewer located on Carling Avenue. Stormwater attenuation on site will be accomplished by means of rooftop storage with controlled roof drains that regulate flow off site.

Two (2) roof drains are proposed for this apartment building that will restrict maximum flow to a rate of 1.27 L/s (0.95 L/s + 0.32 L/s) under a head of 150mm and into the Carling Avenue storm sewer. The calculated net allowable controlled release rate from this site is estimated at 4.07 L/s under the 2 year pre-development event.

At this development site, for storm events up to the 100-year event the maximum post development flow draining off-site is the controlled roof top flow plus the 100-year uncontrolled flow from the site draining to the front which totals to 4.77 L/s (1.27 L/s + 3.5 L/s) and is 0.70 L/s greater than the allowable flow of 4.07 L/s.

Based on the residential site plan from the owner's architect, the average post-development runoff coefficient is estimated at C = 0.75 and A = 0.045 hectares.

An estimation of the 2-year pre-development flow condition was carried out using the criteria accepted by the City of Ottawa. If post-development C valve exceeds the lesser of the  $C_{pre} = 0.42$  or  $C_{allow} = 0.5$  (max) then SWM is required. So from our calculations, the  $C_{pre} = 0.42$  value will be used at  $t_c = 10$  minutes for pre-development allowable flow calculation off-site.

The pre-development flow rate calculated into a storm sewer for this residential area is the lesser of the two (2)-Year storm event where  $C_{\text{allow}} = 0.5$  (max.) runoff value and  $t_c = 10$  minutes or the average  $C_{\text{pre}}$  value which is 0.42 using  $t_c = 10$  minutes. Because the site  $C_{\text{post}} = 0.75$  and  $C_{\text{pre}} = 0.42$  then SWM measures are required.

Therefore, based on our calculation, on-site retention is required for this proposed development site, because the site post-development C value of 0.75 is greater than the  $C_{pre} = 0.42$ .

The storage volume for the two (2)-year and up to the 100-year storm event will be stored by means of flat rooftop on the top of the third floor of the apartment building. Also refer to the site storm drainage report (Report No. R-821-157) for further details.

To control the two (2)-Year storm-water release rate off-site from roof top to a rate of 1.27 L/s, a site storage volume of approximately 3.56  $m^3$  minimum is required during the two (2)-year event.

During the two (2)-year storm event for the flat rooftop storage, the ponding depth of rooftop area 1 is estimated at 100 mm at the drain and 0mm at the roof perimeter, assuming a 1.3% minimum roof pitch to the drain. The ponding depth of roof top area 2 is estimated at 110mm at the drain and 0mm at the roof perimeter assuming a 1.3% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 2.34 m³ and the rooftop storage available at Roof Area 2 is 2.68 m³, for a total of 5.02 m³, which is greater than the required volume of 3.56 m³.

During the 100-year storm event for the flat rooftop storage, the ponding depth of Roof Area 1 and 2 is estimated at 150 mm at the drain and 0mm at the roof perimeter, assuming a 1.3% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is  $7.92 \text{ m}^3$  and the rooftop storage available at Roof Area 2 is  $6.75 \text{ m}^3$ , for a total of  $14.67 \text{ m}^3$ , which is greater than the required volume of  $12.86 \text{ m}^3$ .

Therefore, by means of flat building rooftop storage and grading the site to the proposed grades as shown on the Proposed Grading and Servicing Plan and Proposed Rooftop Stormwater Management Plan Dwg. 821-157 G-1 and 821-157 SWM-1 respectively, the desirable two (2)-year storm and 100-year storm event detention volume of 5.02 m<sup>3</sup> and

14.67 m<sup>3</sup> respectively will be available on site. Refer to Storm Drainage Report (Report No. R-821-157) in **Appendix D** for detailed calculations of available storage volumes.

At this development site, for storm events up to the 100-year event the maximum post development flow draining off-site is the controlled roof top flow plus the 100-year uncontrolled flow from the site draining to the front of the lot totals to 4.77 L/s (1.27 L/s + 3.50 L/s) which is approximately 0.70 L/s greater than the allowable flow of 4.07 L/s.

In comparing the pre-development flow of the current site conditions to the post development flow and with the SWM regulated flow plus uncontrolled flow from the proposed site under the post development conditions at the 2-Year event = 2.59 L/s and the 100-Year event = 4.77 L/s whereupon the post development flow events are less than the current 2-Year event and less than the current 100-Year event pre-development flow estimate for the site at 2-Year  $_{\text{Pre}} = 4.07 \text{ L/s}$  and 100-Year  $_{\text{Pre}} = 10.77 \text{ L/s}$ . Therefore with this proposed development, stormwater flow off-site is considered improved from that of the existing conditions.

The building weeping tile drainage will outlet via its separate 150mm diameter PVC storm lateral. The roof drains will be outletted also via a separate 150mm PVC storm lateral in which both laterals will be connected to the existing Carling Avenue 375mm dia. storm sewer. The City of Ottawa recommends that pressurized drain pipe material be used in the building for the roof drain leader pipe in the event of surcharging in the City storm sewer system. Refer to the proposed site grading and servicing plan Dwg. 821-157 for details.

### **Erosion and Sediment Control**

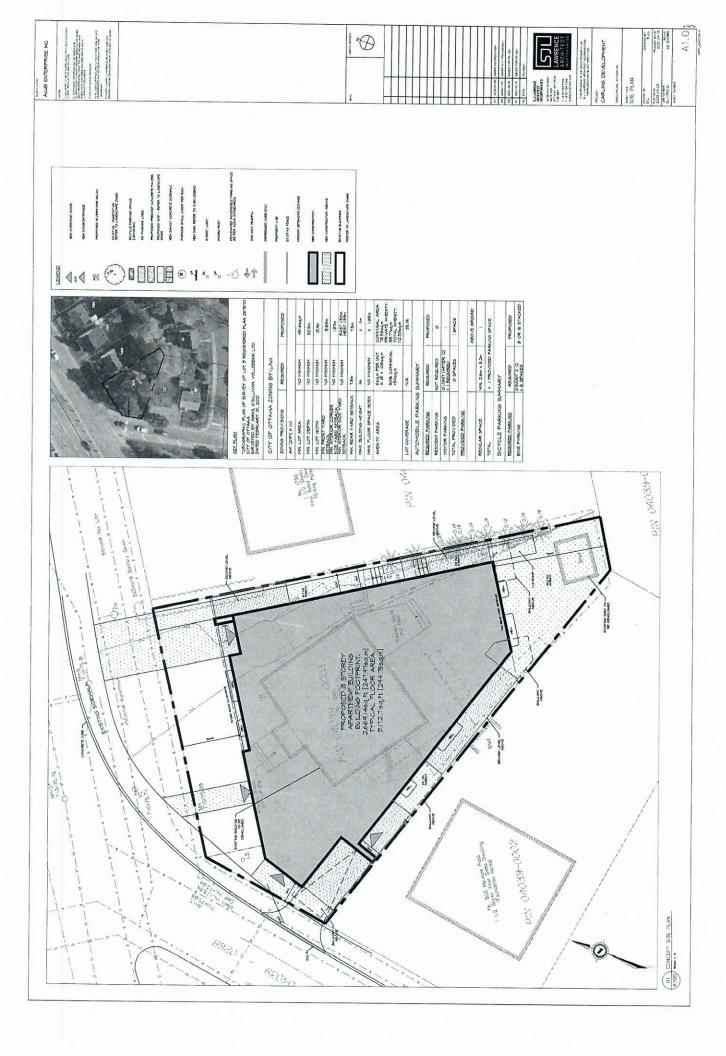
The contractor shall implement Best Management Practices to provide for protection of the receiving storm sewer during construction activities. These practices are required to ensure no sediment and/or associated pollutants are released to the receiving watercourse. These practices include installation of a "siltsack" catch basin sediment control device or equal in catch basins as recommended by manufacturer on-site and off-site within the Merivale Road Carling Avenue road right of way adjacent to this property. Siltsack shall be inspected every 2 to 3 weeks and after major storm. The deposits will be disposed of as per the requirements of the contract. See Dwg. #821-157 ESC-1 for details.

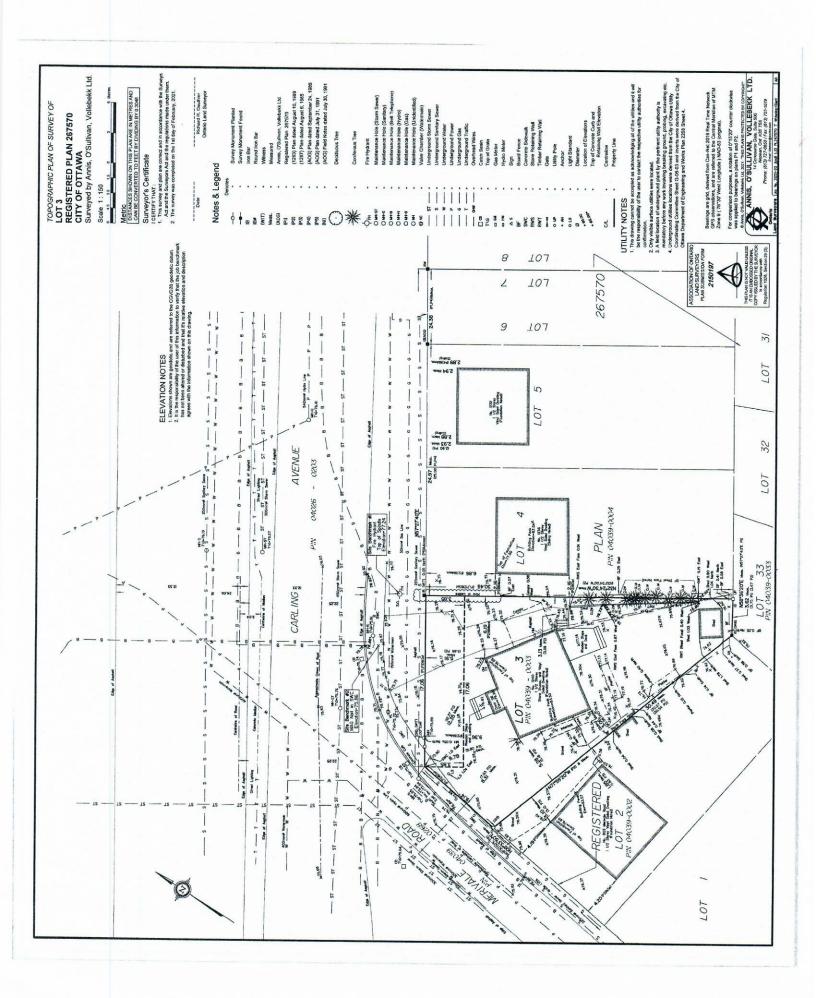
Refer to **Appendix F** for the summary of the Development Servicing Study Checklist that is applicable to this development.

PREPARED BY T.L. MAK ENGINEERING CONSULTANTS LTD.

TONY L. MAK, P.ENG

### APPENDIX A SITE PLAN AND LEGAL SURVEY PLAN



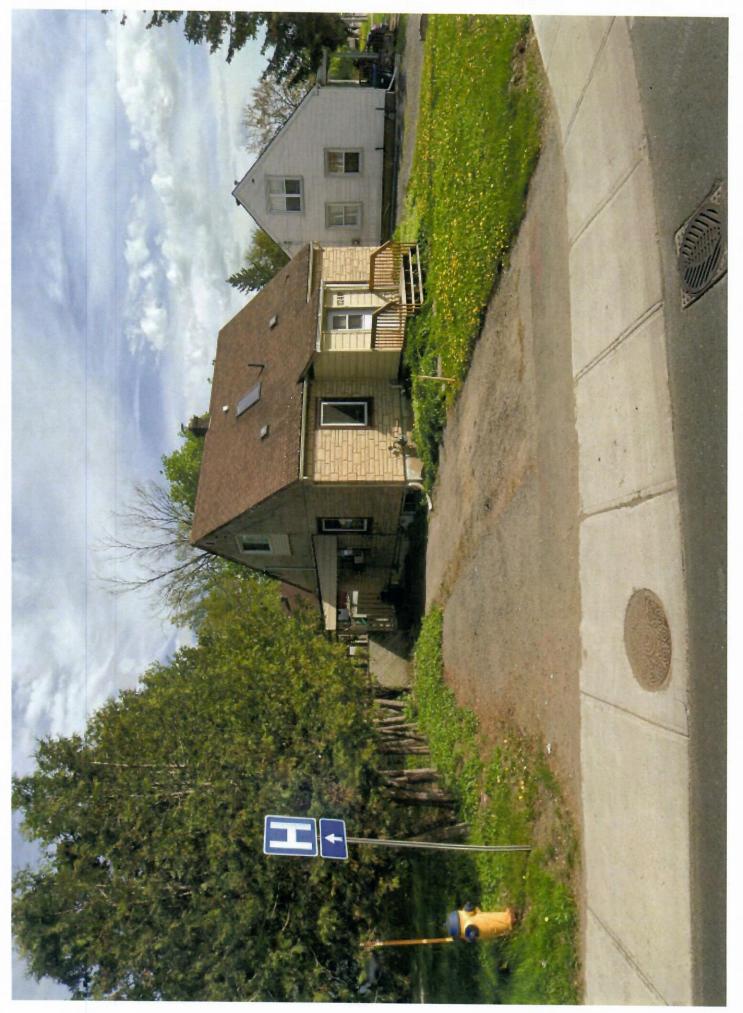


## APPENDIX B SITE PRE-DEVELOPMENT CONDITION GOOGLE IMAGE (2023) AND

**AERIAL PHOTOGRAPHY 2021 (GEOOTTAWA)** 







APPENDIX C

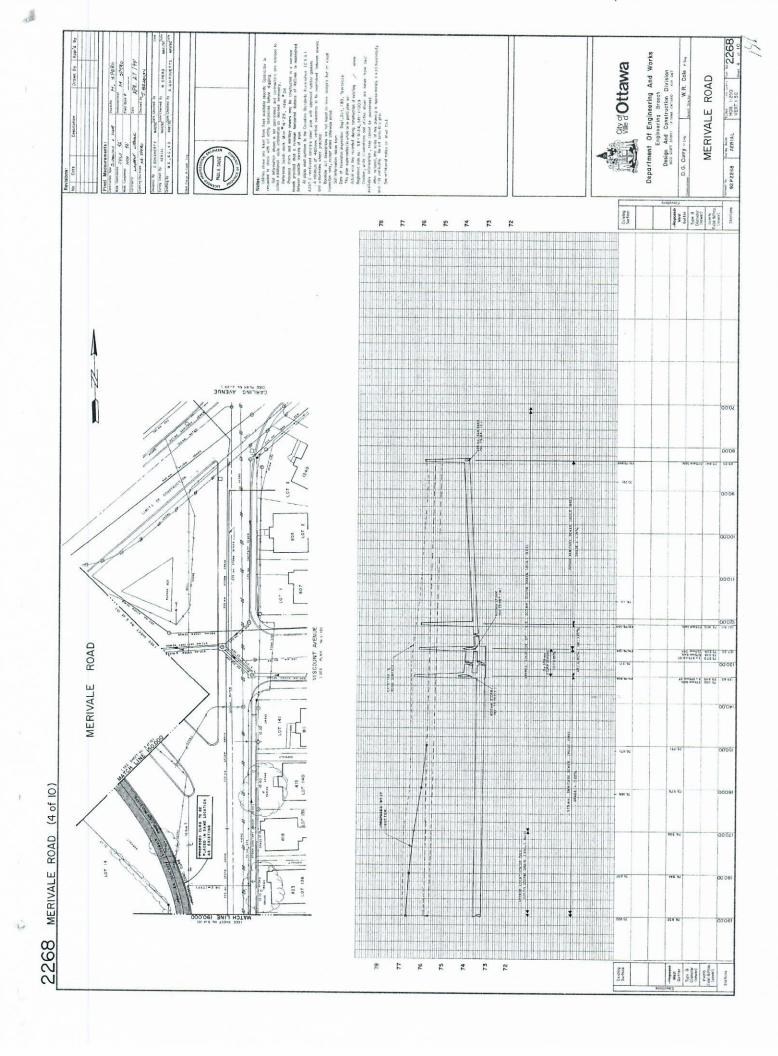
CARLING AVENUE AND MERIVALE ROAD

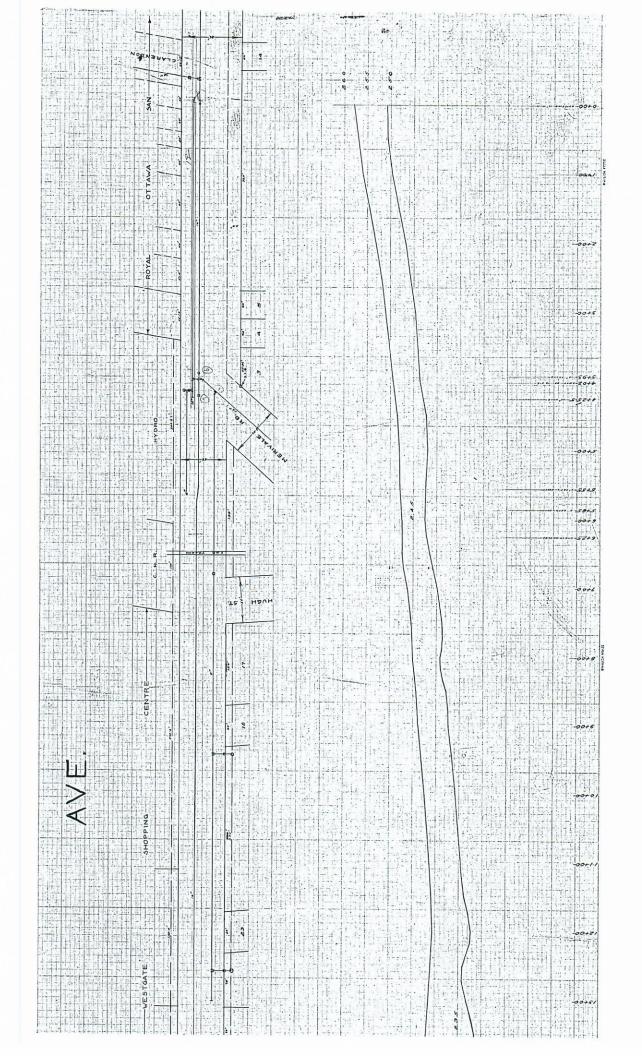
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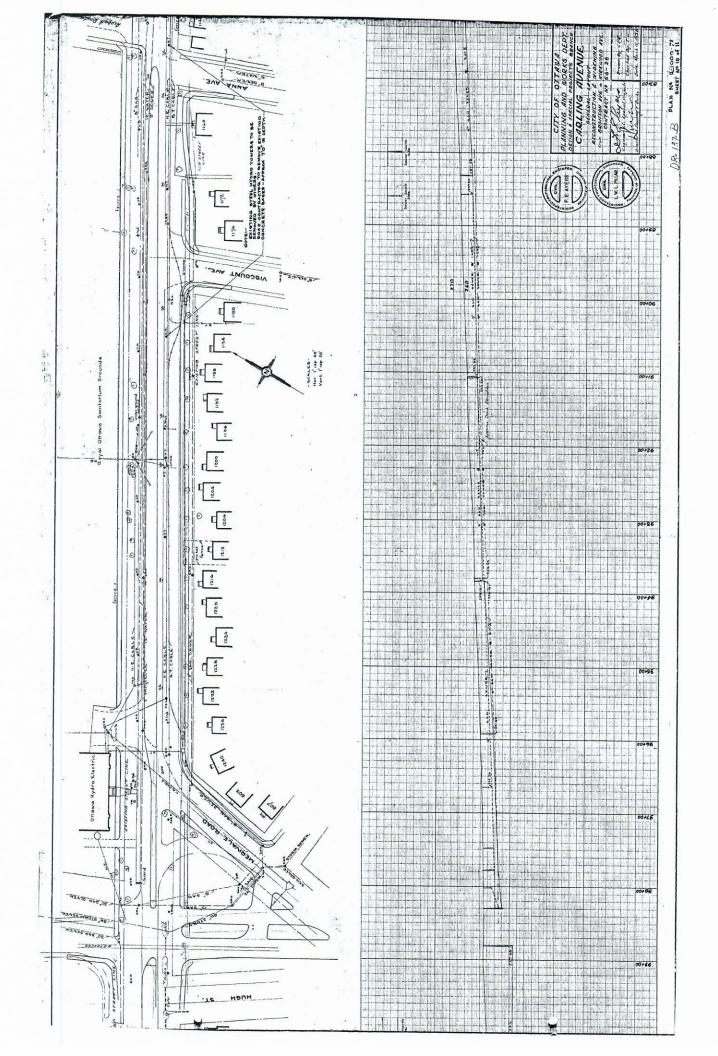
PLAN AND PROFILE

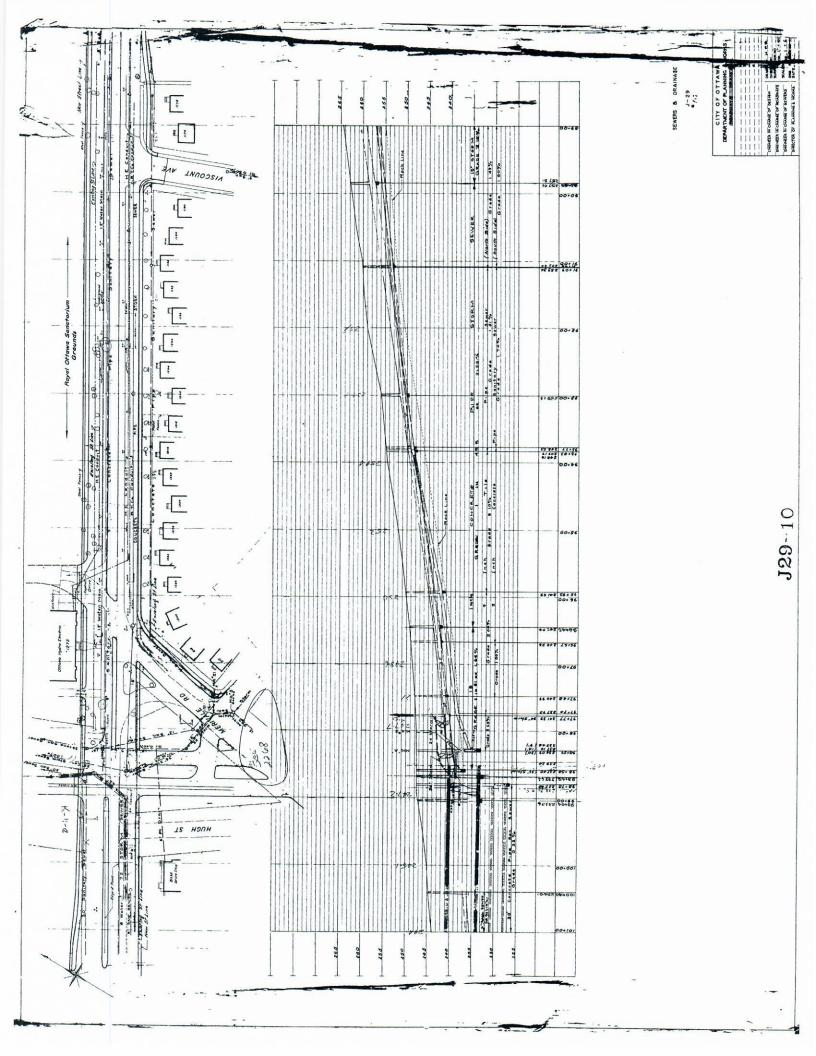
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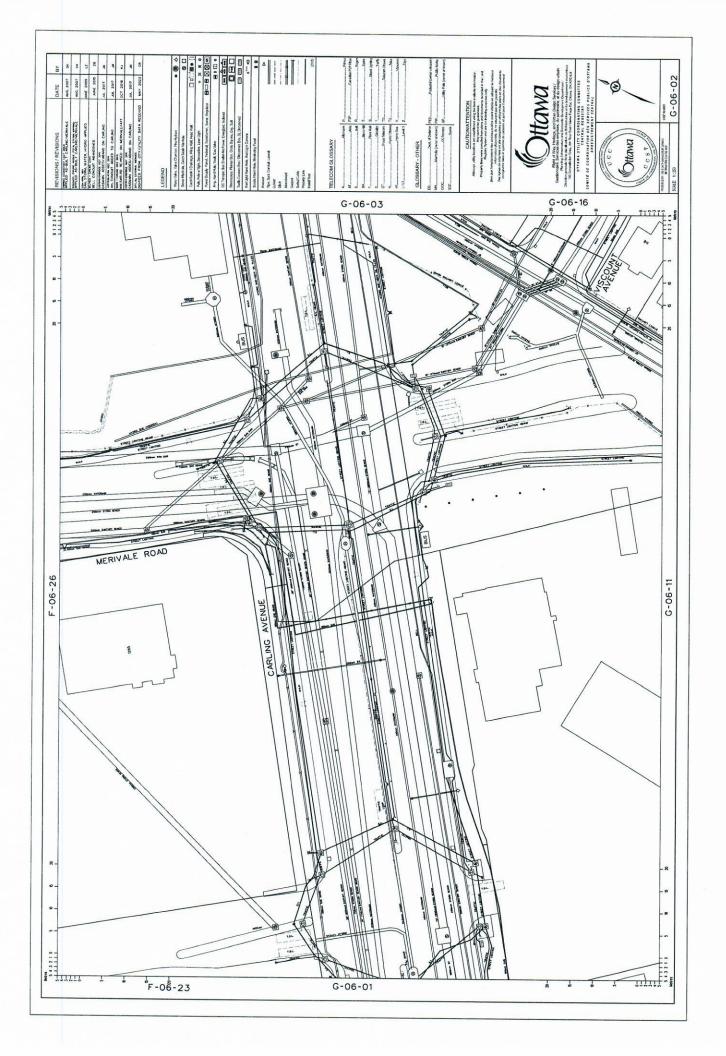
UCC DRAWINGS

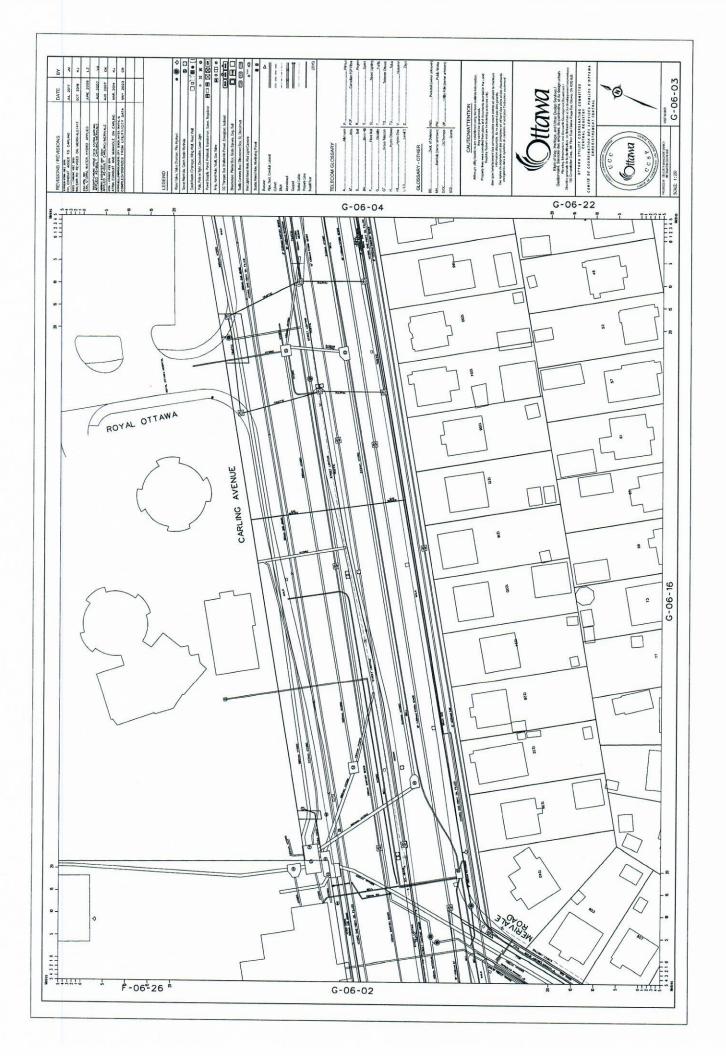












### PROPOSED THREE STOREY RESIDENTIAL APARTMENT BUILDING SITE

**LOT 3 (R-PLAN 287570)** 

### **1240 CARLING AVENUE**

### **CITY OF OTTAWA**

### APPENDIX D

- SITE PLAN AND ARCHITECTURAL DRAWINGS
- SITE SURVEY PLAN
- ELEVATION CONTOURS (FIGURE 1)
- FUS FIRE FLOW CALCULATIONS
- OFM FIRE FLOW CALCULATIONS
- FUS EXPOSURE DISTANCES (FIGURE 2)
- OFM EXPOSURE DISTANCES (FIGURE 3)
- WATER BOUNDARY CONDITIONS
- SUPPORTING HYDRAULIC CALCULATIONS
- HYDRANT SPACING (FIGURE 4)

<b>ATTACHMENT</b>	1	SITE	PLAN	AND	ACHITECT	URAL	<b>DRAWINGS</b>
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# CARLING DEVELOPMENT 1240 CARLING, OTTAWA ON. ISSUED FOR COORDINATION 2023-05-04 4:40:54 PM

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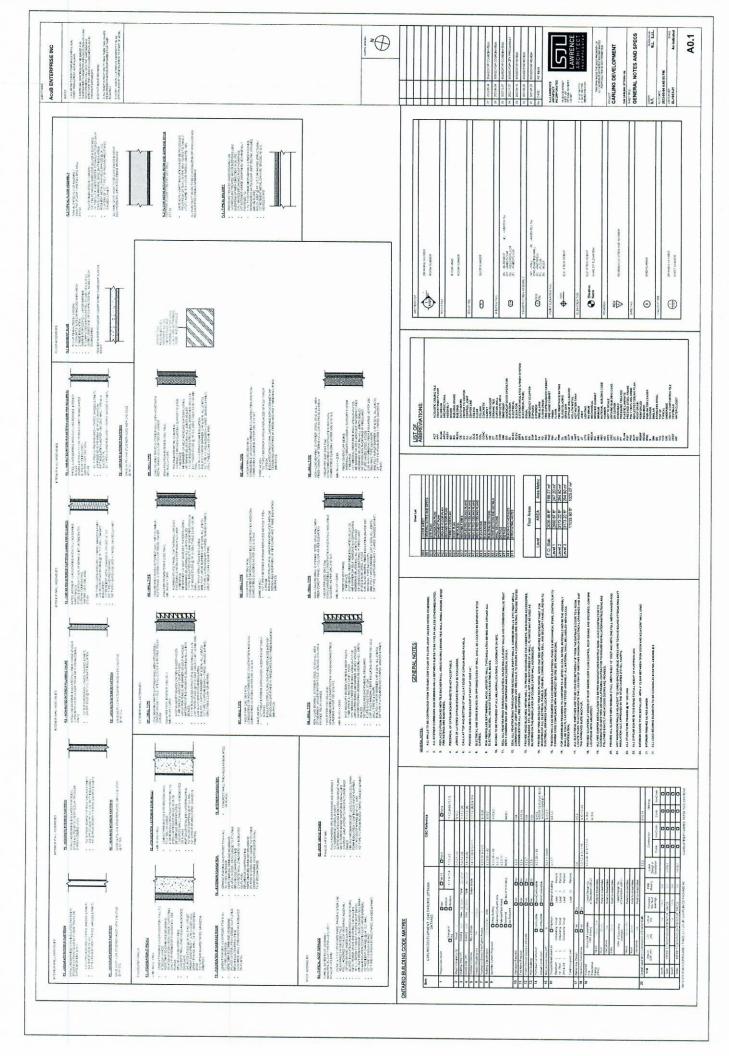
STRUCTURAL DRAWINGS

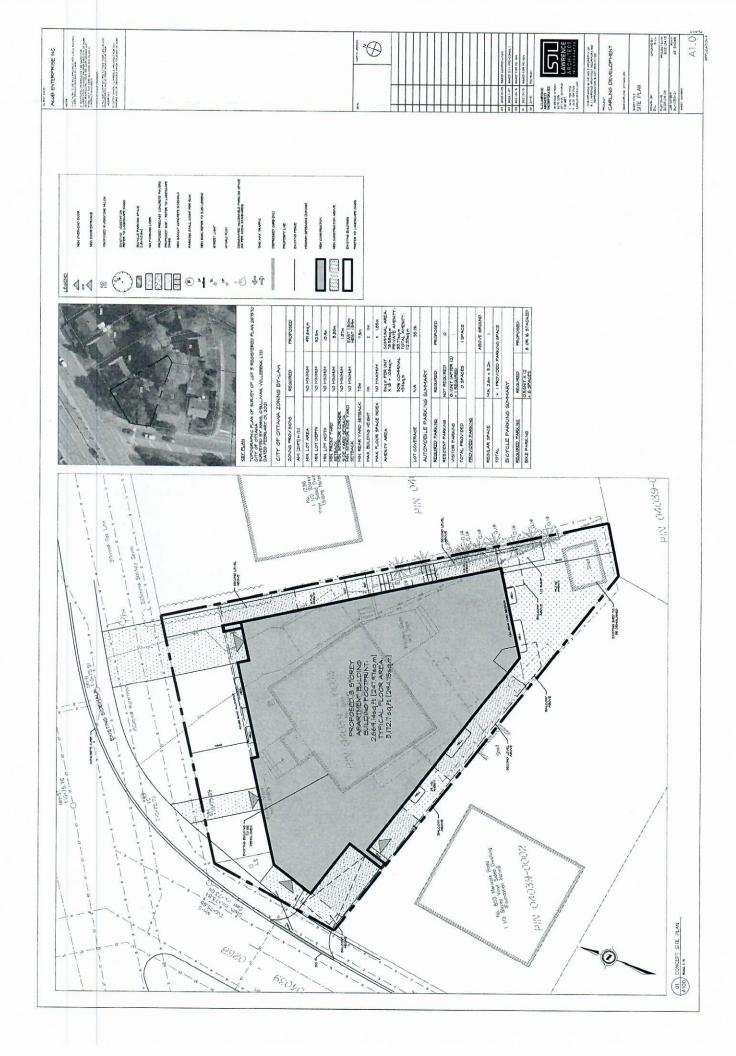


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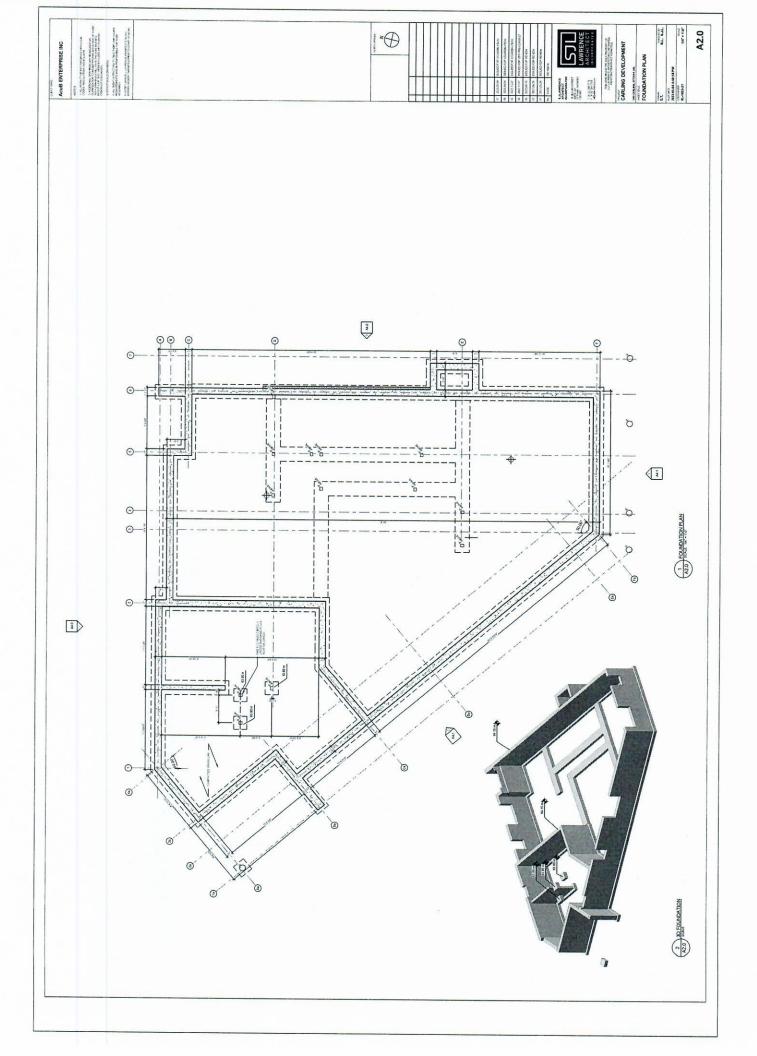
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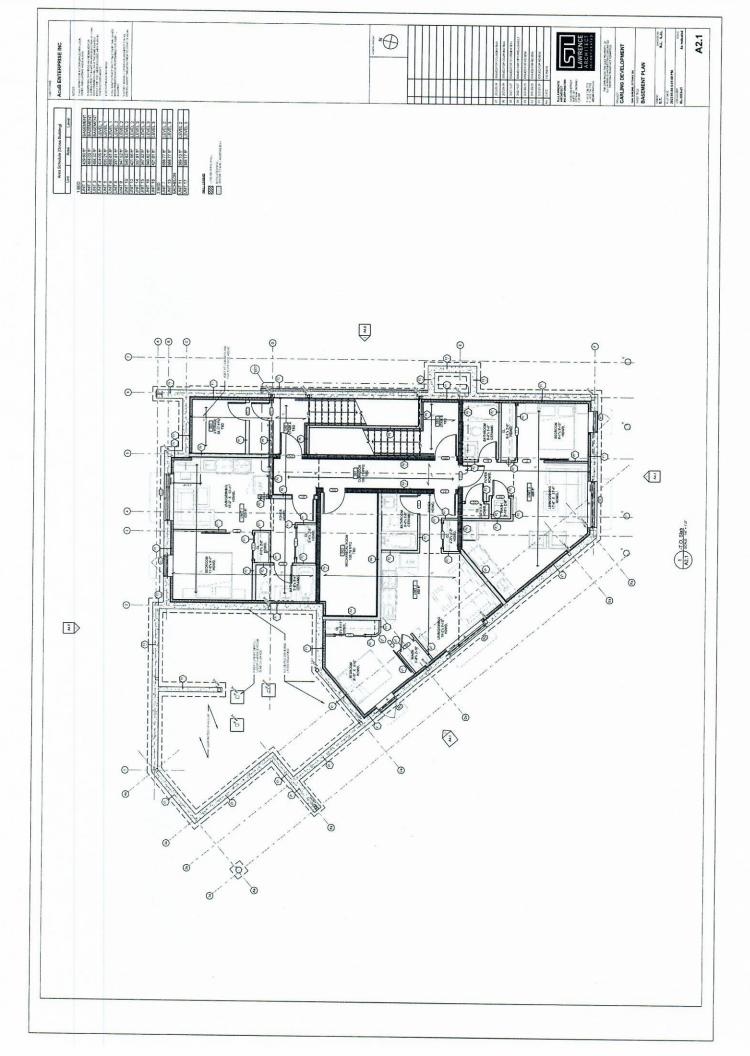
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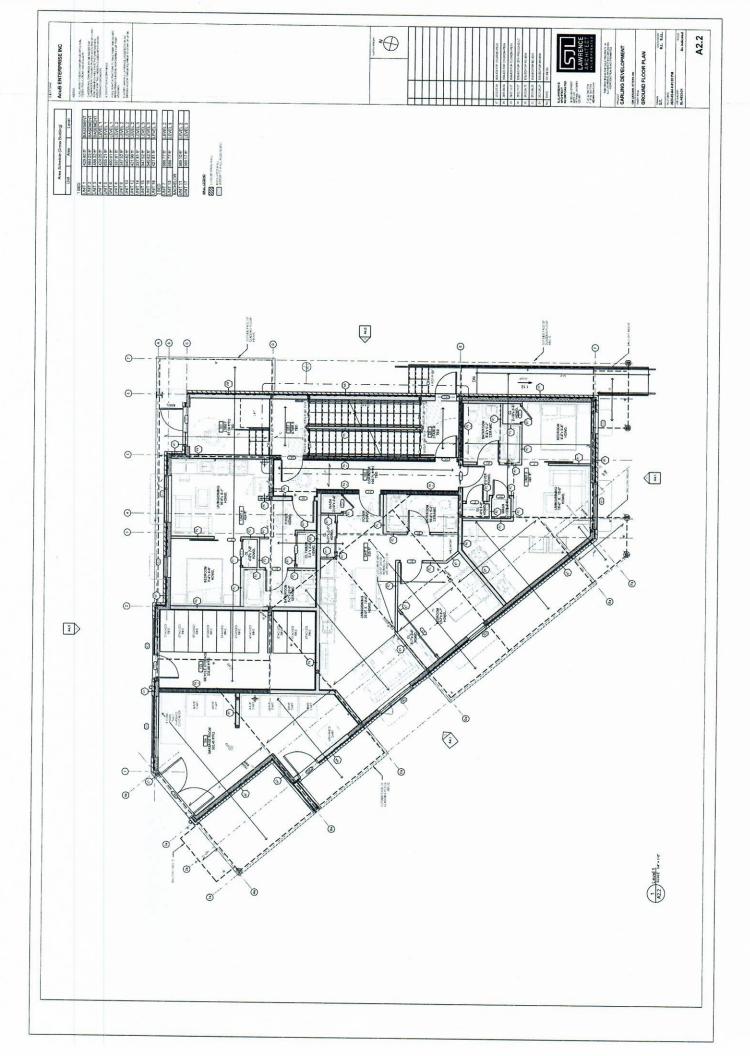


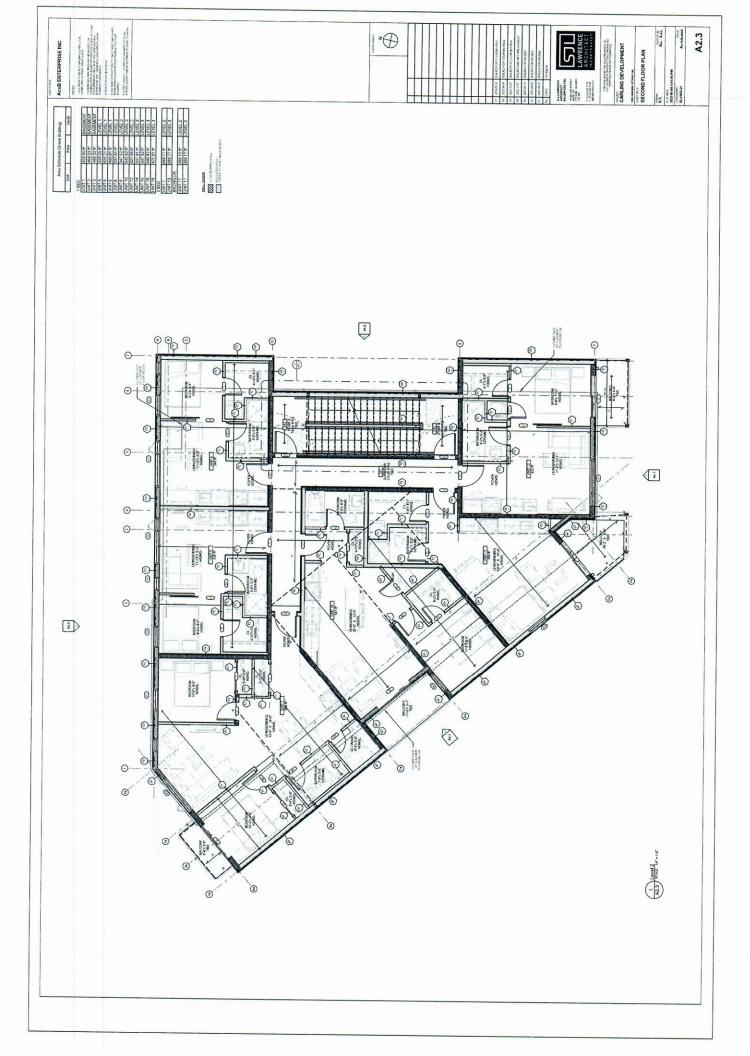


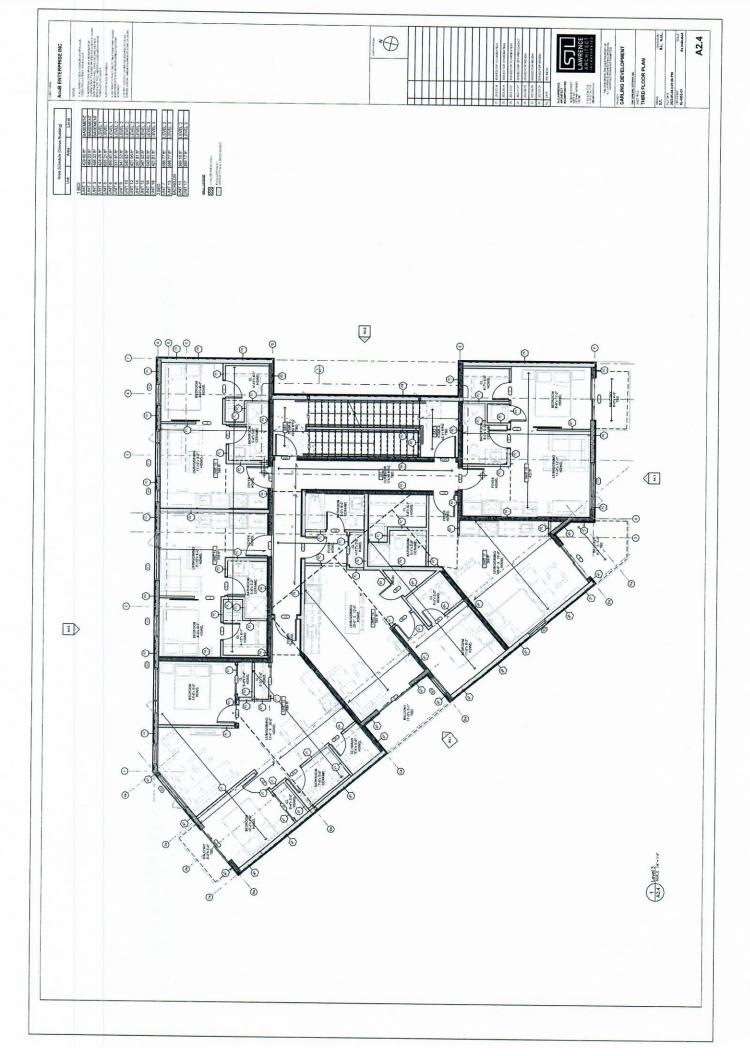
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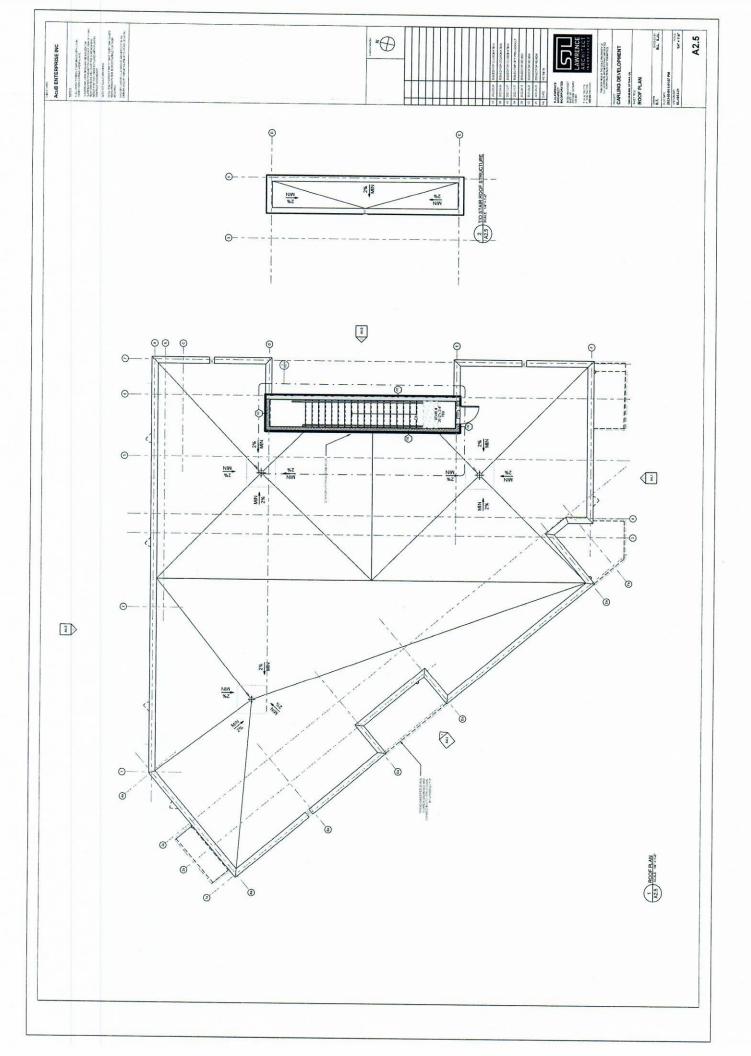


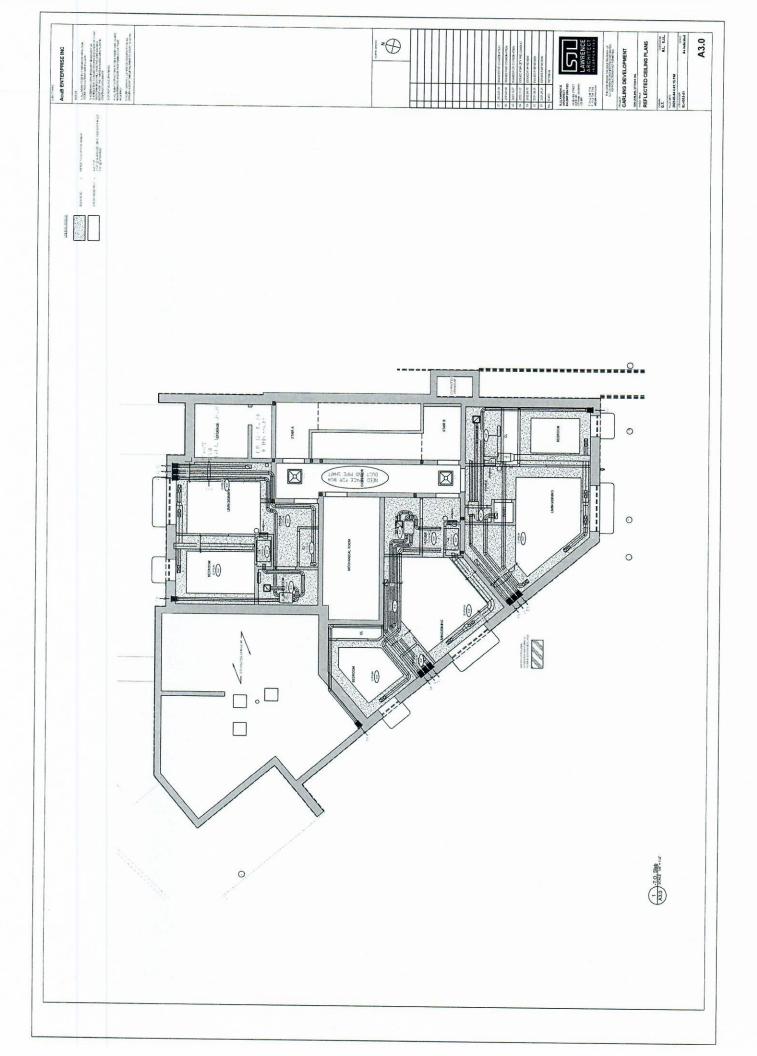


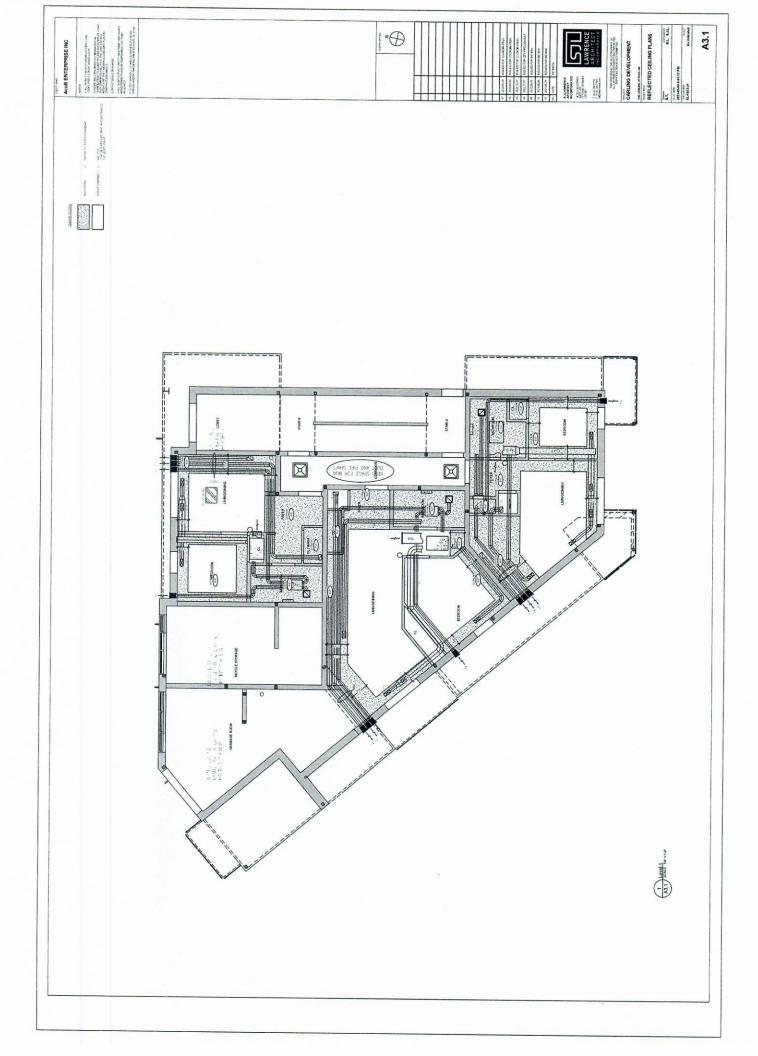


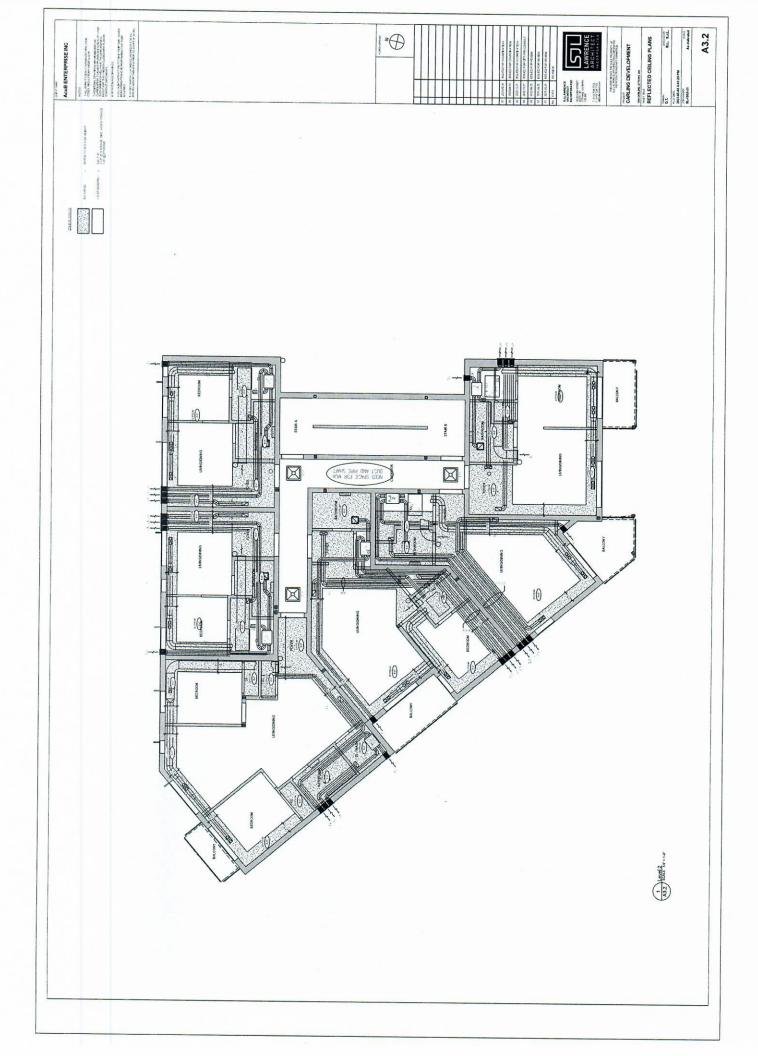


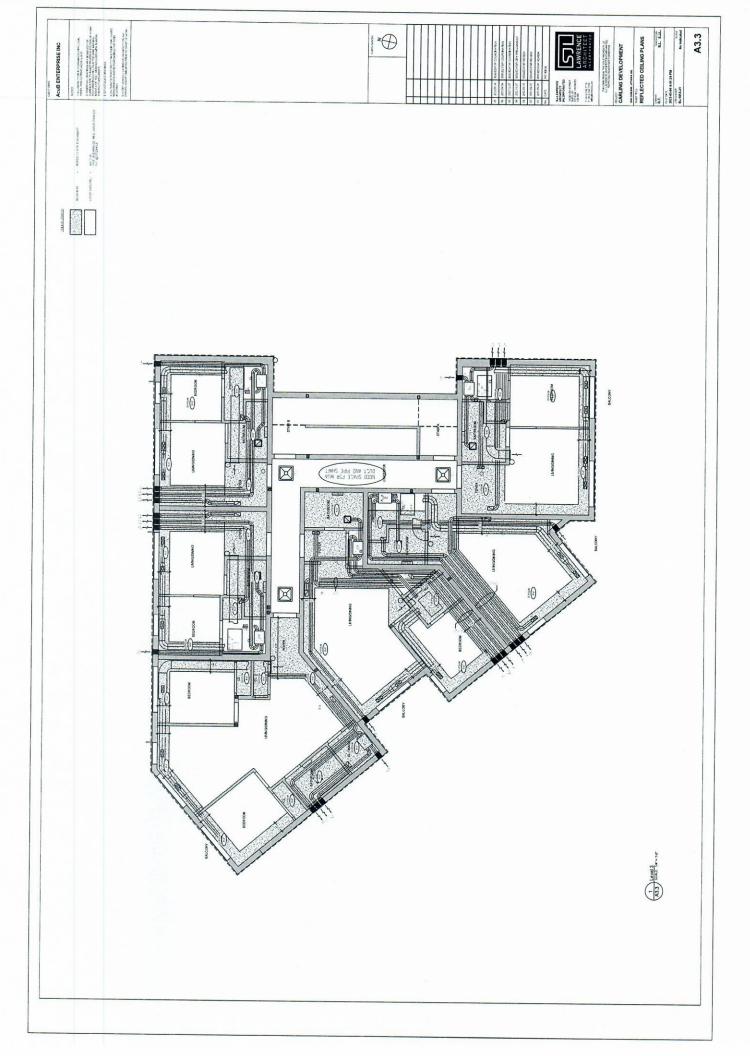




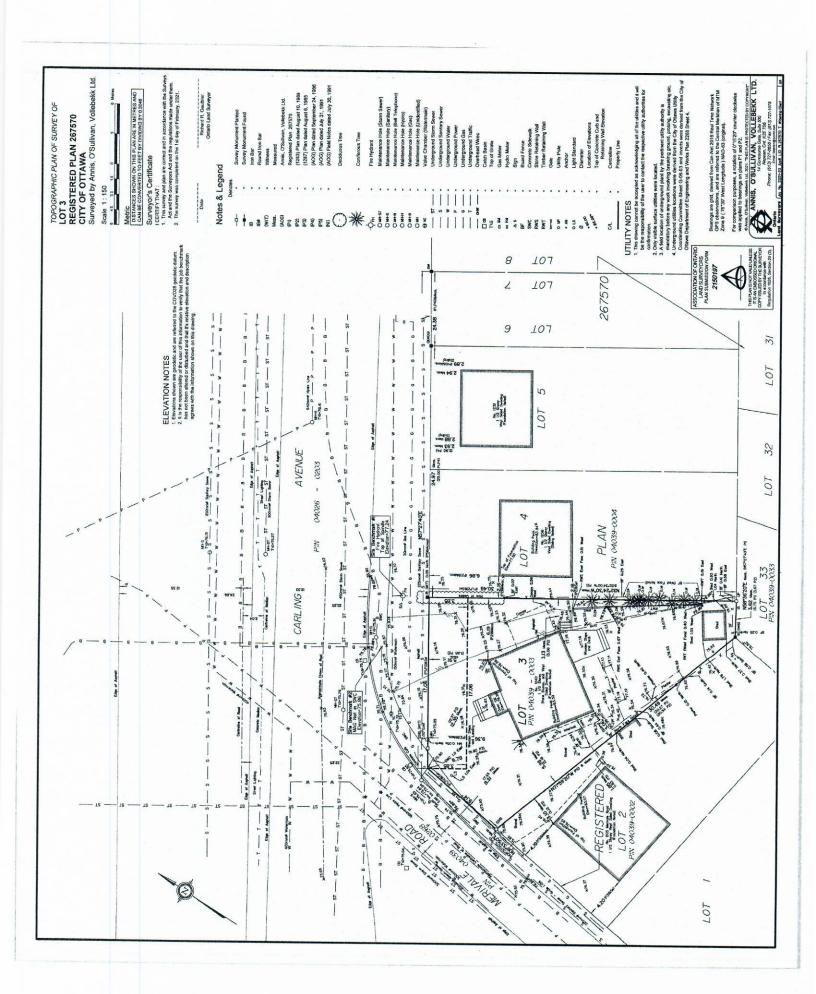




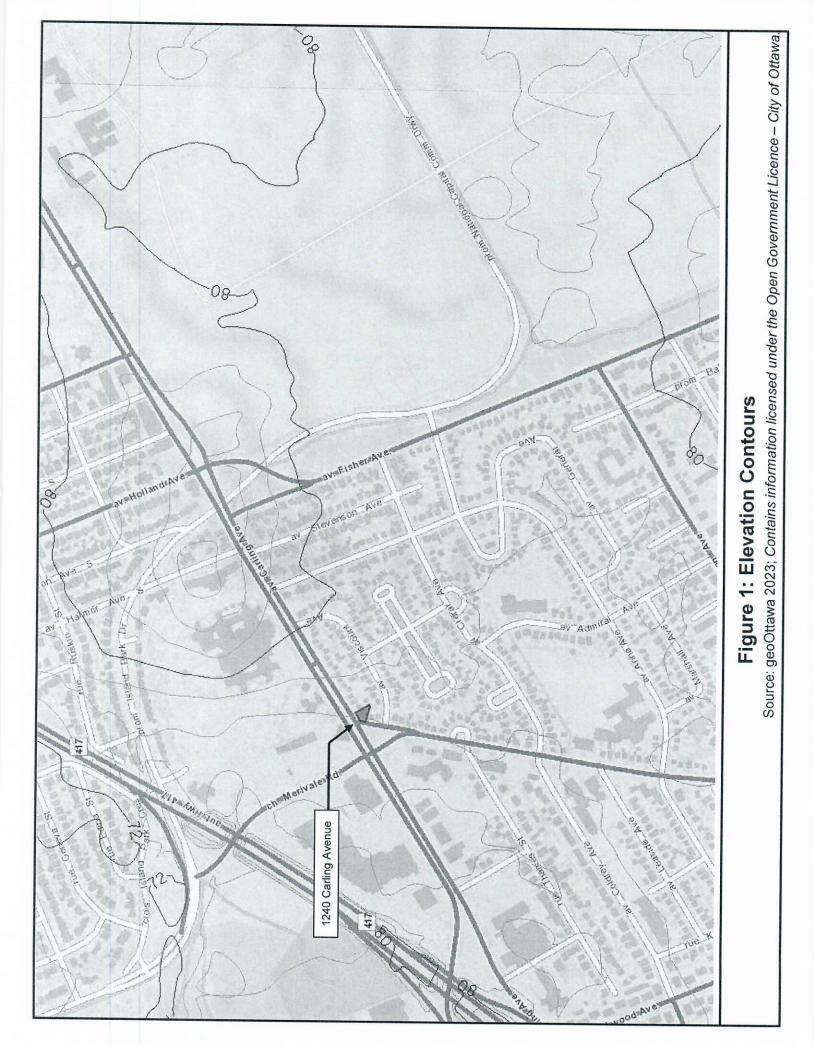




**ATTACHMENT 2: SITE SURVEY PLAN** 



**ATTACHMENT 3: FIGURE 1 – ELEVATION CONTOURS** 



**ATTACHMENT 4: FUS FIRE FLOW CALCULATIONS** 



### FUS Fire Flow Calculation - Long Method

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 2020

Stantec Project #: 163401084 Project Name: 1240 Carling Avenue Date: May 8, 2023

Fire Flow Calculation #: 1 Building Type/Description/Name: Residential

Data inputted by: Hamidreza Mohabbat
Data reviewed by: Alexandre Mineault-Guitard M.A.Sc., ing., P.Eng.

Detached units.

Notes: Wood frame; 2 storeys more than 50% above grade. Gross floor area of 1,500 to 2,000 sqf. Not sprinklered.

Building setbacks per site plan (dated 2022/05/31).

Step	7 3 3 3 3 3 3 3 4		derwriters Survey Determination						
otop	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fir Flow (L/min)	
		Framing Material							
	Choose Frame Used for Construction of Unit		Type V - Wood Frame	1.5					
1			Type IV-A - Mass Timber	0.8	8				
			Type IV-B - Mass Timber	0.9					
		Coefficient related to	Type IV-C - Mass Timber	1	Type V - Wood Frame	4.5			
		type of construction (C)	Type IV-D - Mass Timber	1.5	Type v - vvood Frame	1.5	m		
			Type III - Ordinary construction	1					
		p. 15	Type II - Non-combustible construction	0.8					
			Type I - Fire resistive construction	0.6					
	Choose Type of			Floor Space Are	a				
2	Housing (if TH, Enter Number of		Single Family	0					
	Units Per TH Block)	Type of Housing	Townhouse - indicate # of units	0	Other (Comm, Ind, Apt	18	Units		
			Other (Comm, Ind, Apt etc.)	18	etc.)				
2.2	# of Storeys	Number of Floors/S	Storeys in the Unit (do not include basemen	t if 50% below grade):	3	3	Storeys		
3	Enter Ground Floor	Average Floor A	rea (A) based on total floor area of all floor	s for one unit /non-fire	279				
3	Area of One Unit			resistive construction):		279	Area in		
3.1	Obtain Total Effective Building	Total Effective Buildin	ng Area (# of Storeys x # of Units (if single f		Square Metres (m2)	837	Square Metres (m²)		
4	Area Obtain Required Fire Flow without		Required Fire Flow (without reducti	ons or increases per F				40.000	
	Reductions		Round to	nearest 1,000 L/min				10,000	
5	Apply Factors Affecting Burning		Reductions/Increa	ses Due to Factor	rs Affecting Burning			-	
	Choose		Non-combustible		Trincoling Danning				
		Occupancy Content	Limited combustible	-0.25 -0.15					
5.1	Combustibility of	Hazard Reduction or	Combustible		Limited combunithis	0.45			
	<b>Building Contents</b>	Surcharge	Free burning	0.15	Limited combustible	-0.15	N/A	8,500	
			Tree barriing	0.15		Ų.			
				Rapid burning	0.25				
			Rapid burning  Adequate Sprinkler conforms to NERA13	0.25	white the second second second second				
Т		Sprinkler Reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
5.2		Sprinkler Reduction  Water Supply Credit		-0.3 0	Water supply is not	0			
5.2	22 C 10 C		Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and	-0.3 0		- VAN	N/A N/A	0	
5.2	Due to Presence of	Water Supply Credit Sprinkler Supervision	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised	-0.3 0 -0.1	Water supply is not standard or N/A Sprinkler not fully	0	N/A	0	
5.2	Due to Presence of	Water Supply Credit	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A	-0.3 0 -0.1 0 -0.1	Water supply is not standard or N/A	- VAN			
5.2	Due to Presence of	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised	-0.3 0 -0.1 0 -0.1	Water supply is not standard or N/A Sprinkler not fully supervised or N/A	0	N/A N/A	0	
5.2	Due to Presence of	Water Supply Credit Sprinkler Supervision Credit	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A	-0.3 0 -0.1 0 -0.1	Water supply is not standard or N/A Sprinkler not fully	0	N/A	0	
78 3000	Due to Presence of Sprinklers  Choose Presence of Sprinklers for	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform	-0.3 0 -0.1 0 -0.1 0 s to NFPA13	Water supply is not standard or N/A Sprinkler not fully supervised or N/A None for exposures Water supply is not	0	N/A N/A N/A	0	
78 3000	Due to Presence of Sprinklers  Choose Presence of C	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and	-0.3 0 -0.1 0 -0.1 0 s to NFPA13	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures	0	N/A N/A	0	
5.2	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept, hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures	-0.3 0 -0.1 0 -0.1 0 -0.1 0 s to NFPA13	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully	0	N/A N/A N/A	0	
78 3000	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex	-0.3 0 -0.1 0 -0.1 0 -0.1 0 s to NFPA13 fire dept. hose line of	Water supply is not standard or N/A Sprinkler not fully supervised or N/A None for exposures Water supply is not standard or N/A for exposures	0	N/A N/A N/A	0	
78 3000	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully supervised or N/A for ex Sprinkler not fully supervised or N/A for ex Front Yard (North Front)	-0.3 0 -0.1 0 -0.1 0 s to NFPA13 fire dept. hose line of posures ervised posures 30.1m or greater	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for	0	N/A N/A N/A	0	
5.3	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m  Choose Separation Distance Returned	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision Exposure Distance	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully super Sprinkler not fully supervised or N/A for ex Front Yard (North Front) Right Side (East Front)	-0.3 0 -0.1 0 -0.1 0 -0.1 0 s to NFPA13 fire dept. hose line of posures	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for exposures	0	N/A N/A N/A N/A	0	
55.3	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m  Choose Separation Distance Returned	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully supervised or N/A for ex Sprinkler not fully supervised or N/A for ex Front Yard (North Front) Right Side (East Front) Rear Yard (South Front)	-0.3 0 -0.1 0 -0.1 0 s to NFPA13  fire dept. hose line of posures ervised posures 3.1 to 10.0m 3.1 to 10.0m	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for exposures  0	0	N/A N/A N/A	0	
55.3	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m  Choose Separation Distance Between	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision Exposure Distance	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully supervised or N/A for ex Sprinkler not fully supervised or N/A for ex Front Yard (North Front) Right Side (East Front) Rear Yard (South Front) Left Side (West Front)	-0.3 0 -0.1 0 -0.1 0 s to NFPA13  fire dept. hose line of posures envised posures 30.1m or greater 3.1 to 10.0m 3.1 to 10.0m 0 to 3.0m	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for exposures  0 0.2 0.2 0.25	0 0 0.65	N/A N/A N/A N/A m	0	
5.3	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m  Choose Separation Distance Between Units	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision Exposure Distance	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully supervised or N/A for ex Sprinkler not fully supervised or N/A for ex Front Yard (North Front) Right Side (East Front) Rear Yard (South Front)	-0.3 0 -0.1 0 -0.1 0 s to NFPA13  fire dept. hose line of posures envised posures 30.1m or greater 3.1 to 10.0m 3.1 to 10.0m 0 to 3.0m	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for exposures  0 0.2 0.2 0.25	0 0 0.65	N/A N/A N/A N/A m	0	
5.3	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m  Choose Separation Distance Between Units  Obtain Required	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision Exposure Distance	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully supervised or N/A for ex Sprinkler not fully supervised or N/A for ex Front Yard (North Front) Right Side (East Front) Rear Yard (South Front) Left Side (West Front)	-0.3 0 -0.1 0 -0.1 0 s to NFPA13  fire dept. hose line of posures envised posures 30.1m or greater 3.1 to 10.0m 3.1 to 10.0m 0 to 3.0m	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for exposures  0 0.2 0.2 0.25	0 0 0 0.65	N/A N/A N/A N/A m its applied:	0 0 0 5,525	
5.3	Due to Presence of Sprinklers  Choose Presence of Sprinklers for Exposures within 30m  Choose Separation Distance Between Units	Water Supply Credit Sprinkler Supervision Credit Sprinkler Conforms to NFPA13 Water Supply Sprinkler Supervision Exposure Distance	Adequate Sprinkler conforms to NFPA13 None Water supply is standard for sprinkler and fire dept. hose line Water supply is not standard or N/A Sprinkler system is fully supervised Sprinkler not fully supervised or N/A Adequate sprinkler for exposures conform None for exposures Water supply is standard for sprinkler and exposures Water supply is not standard or N/A for ex Sprinkler system of exposures is fully supervised or N/A for ex Sprinkler not fully supervised or N/A for ex Front Yard (North Front) Right Side (East Front) Rear Yard (South Front) Left Side (West Front)	-0.3 0 -0.1 0 -0.1 0 s to NFPA13  fire dept. hose line of posures envised posures 30.1m or greater 3.1 to 10.0m 3.1 to 10.0m 0 to 3.0m	Water supply is not standard or N/A  Sprinkler not fully supervised or N/A  None for exposures  Water supply is not standard or N/A for exposures  Sprinkler not fully supervised or N/A for exposures  0 0.2 0.2 0.2 1,000 L/min, with m	0 0 0 0 0.65 ax/min lim	N/A N/A N/A N/A m its applied:	0 0 0 5,525	

**ATTACHMENT 5: OFM FIRE FLOW CALCULATIONS** 

### Fire Flow Calculations as per the Ontario Building Code (OBC)



### **OFM Fire Flow Calculation**

Calculations based on Fire Protection Water Supply Guideline for Part 3 in the Ontario Building Code by the Office of the Fire Marshal (OFM 1999)

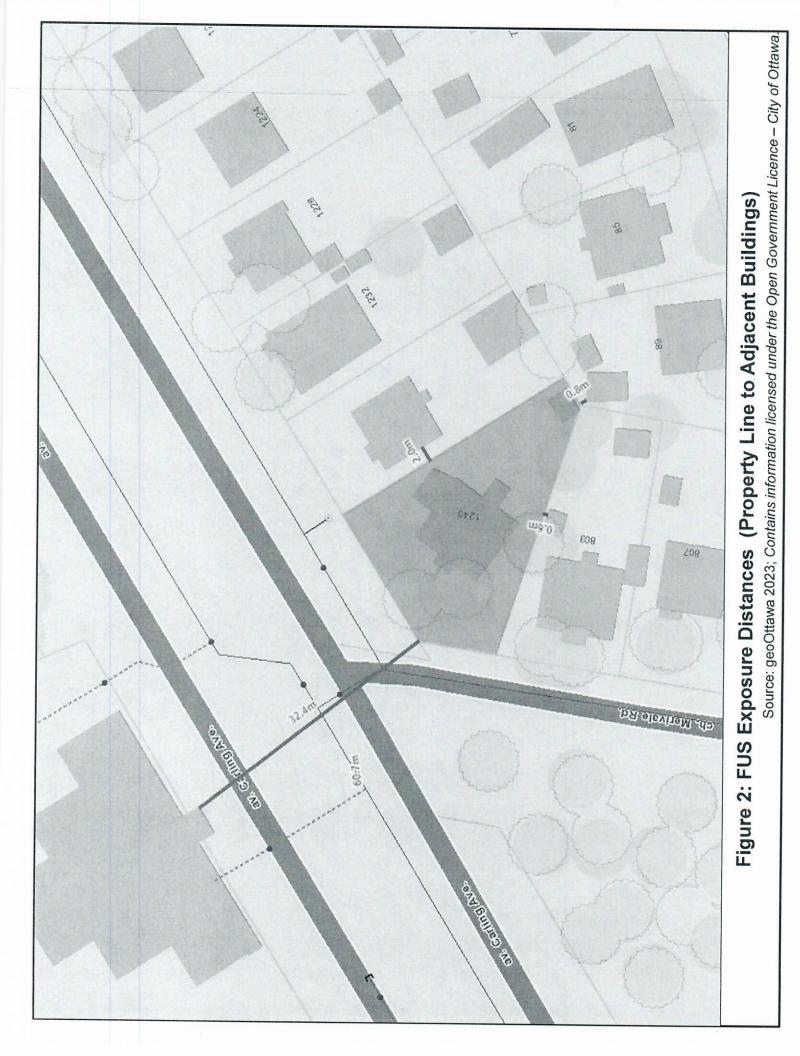
Stantec Project #: 163401084

Project Name: 1240 Carling Avenue
Date: June 5, 2023
Data inputted by: Alexandre Mineault-Guitard M.A.Sc., ing., P.Eng.
Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

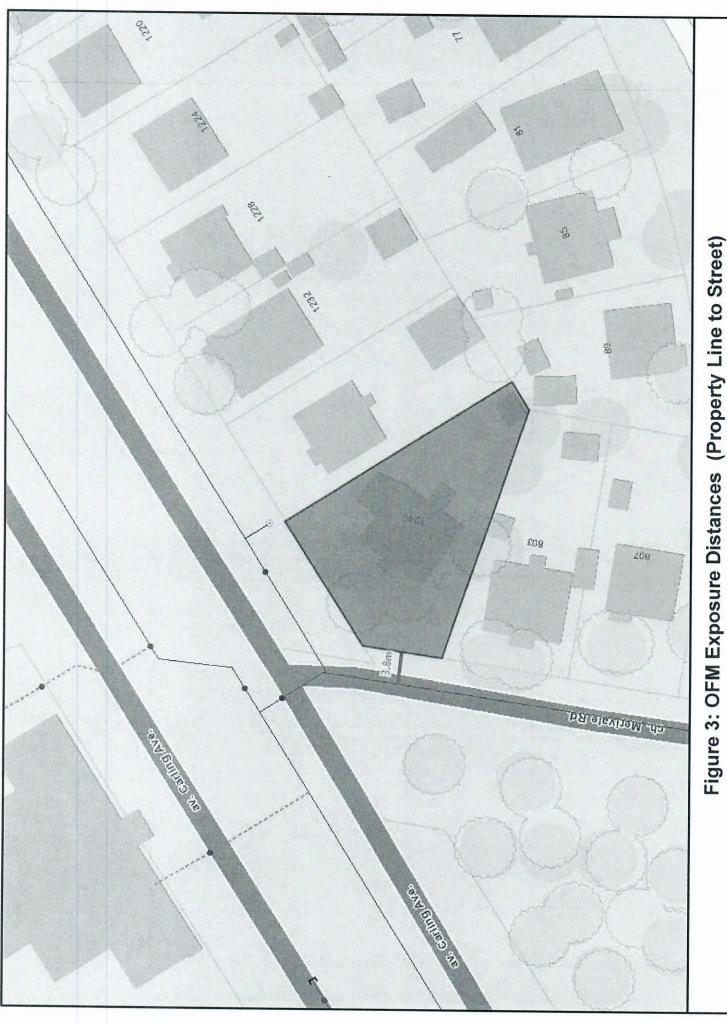
Fire Flow Calculation #: 2
Building Type/Description/Name: Residential

			he Fire Marshal Determination of Required	Fire Protection	Water Supp	ly	
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit
1			General Building De	A TOTAL CONTRACTOR OF THE PARTY			
1.1	Enter Number of Storeys		Number of Floors/Storeys in the Ur	4	4	Storeys	
	Choose Type of		Single Family	1 0			Citaroye
1.2	Housing (if TH, Enter	Type of	Townhouse - indicate # of units	0	Other (Comm,	40	11-7-
	Number of Units Per TH Block)	Housing	Other (Comm, Ind, Apt etc.)	18	Ind, Apt etc.)	18	Units
1,3	Choose Presence of			0 111 0			
3511	Sprinklers Choose Presence of			Sprinklers?	None	None	N/A
1.4	Firewalls		Fi	irewall separations?	None	None	N/A
1.5	Choose Presence of			Stand-pipe system?	None	None	N/A
2	Stand-Pipe System		Determining Water Supply 0		Notic	None	N/A
				Construction			
			Non-combustible construction + fire separations + fire-	Construction			
			resistance ratings in accordance with Section 3.2.2 of OBC	Type I			
	Choose Type of		Non-combustible construction + fire separations + no				
2.1	Construction	Type of	fire-resistance rating	Type II	Type IV	N/A	N/A
		Construction	Combustible construction + fire separations + fire- resistance ratings in accordance with Section 3.2.2 of	Type III	Type IV	l INIO	IVA
			OBC	туре п			
			Combustible construction + fire separations + no fire- resistance rating	Type IV			
				Classification			
			A-2, B-1, B-2, B-3, C, D	23			
2.2	Choose Classification	Occupancy	A-4, F-3	28		A-2, B-1, B-2,	
		Classification (OBC)	A-1, A-3 E, F-2	32 39		B-3, C, D	N/A
		()	F-1	53			
2.3	Water Supply		Water Supply Coefficient K			23	NUA
3	Coefficient (K)		Determining Building Vo	23	N/A		
				Space Area			
3.1	Enter Ground Floor		11001	Space Area	257		
	Area of One Unit		Avera	ige Floor Area (A):	Square Metres	257	Area in Square Mete (m <sup>2</sup> )
			Build	ing Height	(m2)		(1117)
			Dullu	ing neight	01.1		
3.2	Building Height (h)				61.4		
3.2	Building Height (h)			Bottom Elevation :	61.4 Meters (m)	45.0	
3.2	Building Height (h)			Bottom Elevation :	Meters (m) 76.6	15.2	Height in Meters (m
					Meters (m)	15.2	
3.3	Building Height (h)  Building Volume (V)		Building Volume V = A * h		Meters (m) 76.6	15.2 3,910	Volume in Meters Cu
			Determining Spatial Coef	Top Elevation :	Meters (m) 76.6		
3.3			Determining Spatial Coef	Top Elevation : -	Meters (m) 76.6 Meters (m)		Volume in Meters Cu
3.3			Determining Spatial Coef North Side Property Line to Street Centreline (Street Facing)	Top Elevation : -	Meters (m) 76.6		Volume in Meters Cu
3.3	Building Volume (V)	Exposure	Determining Spatial Coef	Top Elevation : -  ### Top Elevation : -  ### 1.2	Meters (m) 76.6 Meters (m)		Volume in Meters Cu
3.3	Building Volume (V)  Choose Exposure	Exposure Distance from	Determining Spatial Coef  North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing)	Top Elevation : -	Meters (m) 76.6 Meters (m)		Volume in Meters Cu
3.3	Building Volume (V)  Choose Exposure Distances from	Distance from Building to	Determining Spatial Coef  North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance  East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2	Meters (m) 76.6 Meters (m) 0.50	3,910	Volume in Meters Cu (m³)
3.3	Building Volume (V)  Choose Exposure	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2 1.2 1.2	Meters (m) 76.6 Meters (m) 0.50		Volume in Meters Cu (m³)
3.3	Building Volume (V)  Choose Exposure Distances from Building to Property	Distance from Building to	Determining Spatial Coef  North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2	Meters (m) 76.6 Meters (m) 0.50	3,910	Volume in Meters Cu (m³)
3.3	Building Volume (V)  Choose Exposure Distances from Building to Property	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2 0 1.2 1.2 1.2 1.2	Meters (m) 76.6 Meters (m) 0.50 0.50 0.50	3,910	Volume in Meters Cu (m³)
3.3	Building Volume (V)  Choose Exposure Distances from Building to Property	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2 1.2 1.2 1.2 0 1.2 1.2 0 1.2 1.2 0 1.2	Meters (m) 76.6 Meters (m) 0.50	3,910	Volume in Meters Cu (m³)
3.3 4	Building Volume (V)  Choose Exposure Distances from Building to Property	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2 1.2 1.2 0 1.2 1.2 1.2 1.2 1.2 1.2 1.2 1.2 1.2 1.2	Meters (m) 76.6 Meters (m) 0.50 0.50 0.50	3,910	Volume in Meters Cu (m³)
3.3 4	Choose Exposure Distances from Building to Property Line	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance Total Spatial Coefficient Stot = 1 + ∑ Stantonics	Top Elevation :-  ### 1.2	Meters (m) 76.6 Meters (m) 0.50 0.50 0.50	3,910	Volume in Meters Cu (m³)
3.3 4 4.1	Choose Exposure Distances from Building to Property Line	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance Total Spatial Coefficient State = 1 + \(\sumeq\) S  Determining Required Minimum Supply of	Top Elevation :-  fficient S  1.2 3.8 5.0 1.2 0 1.2 1.2 1.2 1.2 0 1.2 5.x 0 Top Elevation :-	Meters (m) 76.6 Meters (m)  0.50  0.50  0.50  0.50	2.00	Volume in Meters Cu (m³)  Distance in Meters (r
3.3 4 4.1 4.2 5	Building Volume (V)  Choose Exposure Distances from Building to Property Line  Total Spatial Coefficient  Obtain Required Fire	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance Total Spatial Coefficient Stot = 1 + ∑ S  Determining Required Minimum Supply of Water, rounded to	Top Elevation :-  ### 1.2	0.50 0.50 0.50 0.50 0.50 0.50 0.50	2.00	Volume in Meters Cu (m³)  Distance in Meters (n
3.3 4 4.1	Building Volume (V)  Choose Exposure Distances from Building to Property Line  Total Spatial Coefficient	Distance from Building to Property Line	North Side Property Line to Street Centreline (Street Facing) Total Exposure Distance East Side Property Line to Street Centreline (Street Facing) Total Exposure Distance South Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance West Side Property Line to Street Centreline (Street Facing) Total Exposure Distance Total Spatial Coefficient State = 1 + \(\sumeq\) S  Determining Required Minimum Supply of	Top Elevation : -  fficient S  1.2 3.8 5.0 1.2 0 1.2 1.2 1.2 1.2 0 1.2 5. 0 1.2 0 1.2 1.2 0 1.2 1.2 0 1.2 1.2 0 1.2 1.2 0 1.2 0 1.2 1.2 0	0.50  0.50  0.50  0.50  0.50  0.40  0.50  0.50  0.50  0.50  0.50	2.00	Distance in Meters (n N/A L L/min

ATTACHMENT 6: FIGURE 2 – FUS EXPOSURE DISTANCES



**ATTACHMENT 7 : FIGURE 3 – OFM EXPOSURE DISTANCES** 



Source: geoOttawa 2023; Contains information licensed under the Open Government Licence – City of Ottawa.

**ATTACHMENT 8: WATER BOUNDARY CONDITIONS** 

### Mineault-Guitard, Alexandre

From:

TL MaK <tlmakecl@bellnet.ca>

Sent:

Monday, June 5, 2023 12:19 PM Mineault-Guitard, Alexandre

To: Cc:

Alemany, Kevin

Subject:

RE: 1240 Carling Avenue - Water Boudary Conditions Request

**Attachments:** 

1240 Carling Avenue May 2023.pdf

Hi Alex.

Attached please find the City's water boundary conditions for your calculations received on June 1, 2023.

Thank You,

Tony Mak

T.L. Mak Engineering Consultants Ltd. 1455 Youville Drive, Suite 218 Ottawa, ON. K1C 6Z7 Tel. 613-837-5516 | Fax: 613-837-5277

E-mail: tlmakecl@bellnet.ca

From: Cassidy, Tyler [mailto:tyler.cassidy@ottawa.ca]

Sent: June 1, 2023 3:00 PM

To: TL MaK

Cc: 'Kayla Blakely'; 'Robert Haslett'

Subject: RE: 1240 Carling Avenue - Water Boudary Conditions Request

Hi Tony,

Please see the boundary conditions below for the proposed development at 1240 Carling Avenue. Note that there are constraints on fire flow availability in the area.

The following are boundary conditions, HGL, for hydraulic analysis at 1240 Carling Avenue (zone 2W2C) with assumed to be connected to the 152 mm watermain on Carlin Avenue OR the 305 mm on Merivale Road (see attached PDF for location).

### **Both Connections:**

Minimum HGL: 124.4 m Maximum HGL: 132.8 m

Fire Flow:

Max Day + Fire Flow (233.3 L/s): 102.9 m (Merivale Connection)

Available fire flow at 20 psi: 155 L/s, assuming ground elevation of 75.9 m (Carling Connection)

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Tyler Cassidy, P.Eng

Infrastructure Project Manager,

Planning, Real Estate and Economic Development Department / Direction générale de la planification, des biens immobiliers et du développement économique - South Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2424 ext./poste 12977, Tyler.Cassidy@ottawa.ca

From: TL MaK <tlmakecl@bellnet.ca>

Sent: June 01, 2023 1:44 PM

To: Cassidy, Tyler <tyler.cassidy@ottawa.ca>

Cc: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>; 'Kayla Blakely' <k.blakely@novatech-eng.com>; 'Robert Haslett'

<rob@haslettconstruction.com>

Subject: RE: 1240 Carling Avenue - Water Boudary Conditions Request

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Hi Tyler,

We are following-up on our water boundary request of May 8, 2023, could you please update us in regards to the status of this request. Sorry for the bother, we've been waiting for over 3 weeks from the City on this request as we are trying to complete our servicing report for next week.

Please forward us this information ASAP so that we can proceed with our work on this report. Thank you for your prompt attention to this matter.

Let us know if you have any questions or comments.

Regards,

Tony Mak

T.L. Mak Engineering Consultants Ltd. 1455 Youville Drive, Suite 218 Ottawa, ON. K1C 6Z7 Tel. 613-837-5516 | Fax: 613-837-5277

E-mail: tlmakecl@bellnet.ca

From: Cassidy, Tyler [mailto:tyler.cassidy@ottawa.ca]

**Sent:** May 8, 2023 2:54 PM

To: TL MaK

Cc: Fawzi, Mohammed

Subject: RE: 1240 Carling Avenue - Water Boudary Conditions Request

Hi Tony,

I've submitted your boundary condition request to our Water Resources group. Please allow up to 10 business days for results to be provided.

Thank you,

Tyler Cassidy, P.Eng

Infrastructure Project Manager,

Planning, Real Estate and Économic Development Department / Direction générale de la planification, des biens immobiliers et du développement économique - South Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2424 ext./poste 12977, Tyler.Cassidy@ottawa.ca

From: TL MaK < tlmakecl@bellnet.ca>

Sent: May 08, 2023 1:34 PM

To: Cassidy, Tyler < tyler.cassidy@ottawa.ca>

Cc: Fawzi, Mohammed < mohammed.fawzi@ottawa.ca >

Subject: 1240 Carling Avenue - Water Boudary Conditions Request

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Hi Tyler,

Regarding this site, we are requesting for water boundary conditions from the City of Ottawa to be provided for our hydraulic analysis. The particulars are as follows:

The proposed building located within Pressure Zone 2W at 1240 Carling Avenue is a 3-storey residential building. The building contains eighteen (18) residential units, namely two (2) studios/bachelor, fourteen (14) 1-bedroom units, and two (2) 2-bedroom units. Three (3) 1-bedroom units will be constructed at basement level. Each floor covers an area of approximately 256 m², for a total gross floor area of approximately 1,027 m², including the residential units located at basement level. The building is serviced by the 305mm watermain along Merivale Road or the 152mm watermain along Carling Avenue.

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines. For residential units, a consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 2.5. Peak hour (PKHR) demands were calculated by multiplying MXDY by a factor of 2.2. Persons per unit (PPU) for each unit were estimated based on the City of Ottawa's Water Design Guidelines. Table 1 shows the estimated domestic demands of the proposed building.

**Table 1: Estimated Domestic Demand** 

Unit Type	Unit	PPU	Consumption	AV	'DY	MX	DY	PKI	НR
	Count	PPU	Consumption	L/d	L/s	L/d	L/s	L/d	L/s
Apartment, Bachelor	2	1.4	280	784	0.01	1,960	0.02	4,312	0.05

Total	18			7,448	0.09	18,620	0.22	40,964	0.47
Apartment, 2-Bedroom	2	2.1	280	1,176	0.01	2,940	0.03	6,468	0.07
Apartment, 1-Bedroom	14	1.4	280	5,488	0.06	13,720	0.16	30,184	0.35

The fire flow requirement was determined following the Fire Underwriter Survey (FUS) method and is provided in the attached worksheet. For this analysis, the building was classified as wood frame construction, with building contents that are limited in combustibility. It is understood that the building will not have a sprinkler system. The resulting total required fire flow (RFF) is 14,000 L/min (233 L/s) for a duration of 3 hours.

### In summary:

- AVDY = 7,448 L/d (0.09 L/s);
- MXDY = 18,620 L/d (0.22 L/s);
- PKHR = 40,964 L/d (0.47 L/s); and,
- Fire Flow = 14,000 L/min (233 L/s).

The City is requested to provide boundary conditions for the Average Day, Maximum Day, Peak Hour and Fire Flow conditions indicated above.

Thank you for your prompt attention to this matter. Please forward the boundary conditions as soon as possible.

Have a good day.

Regards,

Tony Mak

T.L. Mak Engineering Consultants Ltd. 1455 Youville Drive, Suite 218 Ottawa, ON. K1C 6Z7 Tel. 613-837-5516 | Fax: 613-837-5277 E-mail: tlmakecl@bellnet.ca

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ATTACHMENT 9 : SUPPORTING H	HYDRAULIC CALCULATIONS



### **Supporting Hydraulic Calculations**

Stantec Project #: 163401084

Project Name: 1240 Carling Avenue

Date: June 5, 2023

Data inputted by: Alexandre Mineault-Guitard M.A.Sc., ing., P.Eng.

Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

### Boundary Conditions provided by the City:

Scenario 1: Peak Hour (Min HGL): 124.4 m;

Scenario 2: Average Day (Max HGL): 132.8 m; and

Scenario 3: Maximum Day plus Fire Flow: 102.9 m.

### **Sample Calculations**

HGL(m) = hp + hz

where: hp = Pressure Head (m); and hz = Elevation Head (m), estimated from topography.

For Scenario 1, we have:

HGL(m) = 124.4 and hz (m) = 75.9.

Rearranging Equation 1, we can calculate the Pressure Head (hp) as follow:

(1)

hp (m) = HGL - hz $\therefore hp = 124.4 - 75.9 \text{ m} = 48.5 \text{ m}.$ 

To convert from Pressure Head (m) to a pressure value (kPa), the following equation can be used:

P (kPa) = (p \* g \* hp) / 1000 (2)

where:  $\rho$  = density of water = 1000 kg/m<sup>3</sup>; and g = gravitational acceleration = 9.81 m/s<sup>2</sup>.

Using Equation 2, we can calculate the Pressure Head (hp) as follow:

P (kPa) = (1000 \* 9.81 \* 48.5) / 1000 ∴ P = 475 kPa.

Considering that 1 kPa = 0.145 psi, the pressure under Scenario 1 is equal to:

P = 69 psi.

Applying the same procedures, the pressures under Scenario 2 and Scenario 3 are calculated as follows: Scenario 2: P = 81 psi; and Scenario 3: P = 38 psi.

### To summarize:

Scenario 1: Minimum Pressure under Peak Hour Demand: 475 kPa (69 psi)

Scenario 2: Maximum Pressure under Average Day Demand: 558 kPa (81 psi)

Scenario 3: Minimum Pressure under Maximum Day + Fire Flow Demand: 265 kPa (38 psi)



### **Supporting Hydraulic Calculations**

Stantec Project #: 163401084

Project Name: 1240 Carling Avenue

Date: June 5, 2023

Data inputted by: Alexandre Mineault-Guitard M.A.Sc., ing., P.Eng.

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∴ P = 475 kPa.

Considering that 1 kPa = 0.145 psi, the pressure under Scenario 1 is equal to:

P = 69 psi

Applying the same procedures, the pressures under Scenario 2 and Scenario 3 are calculated as follows: Scenario 2: P = 81 psi; and Scenario 3: P = 38 psi.

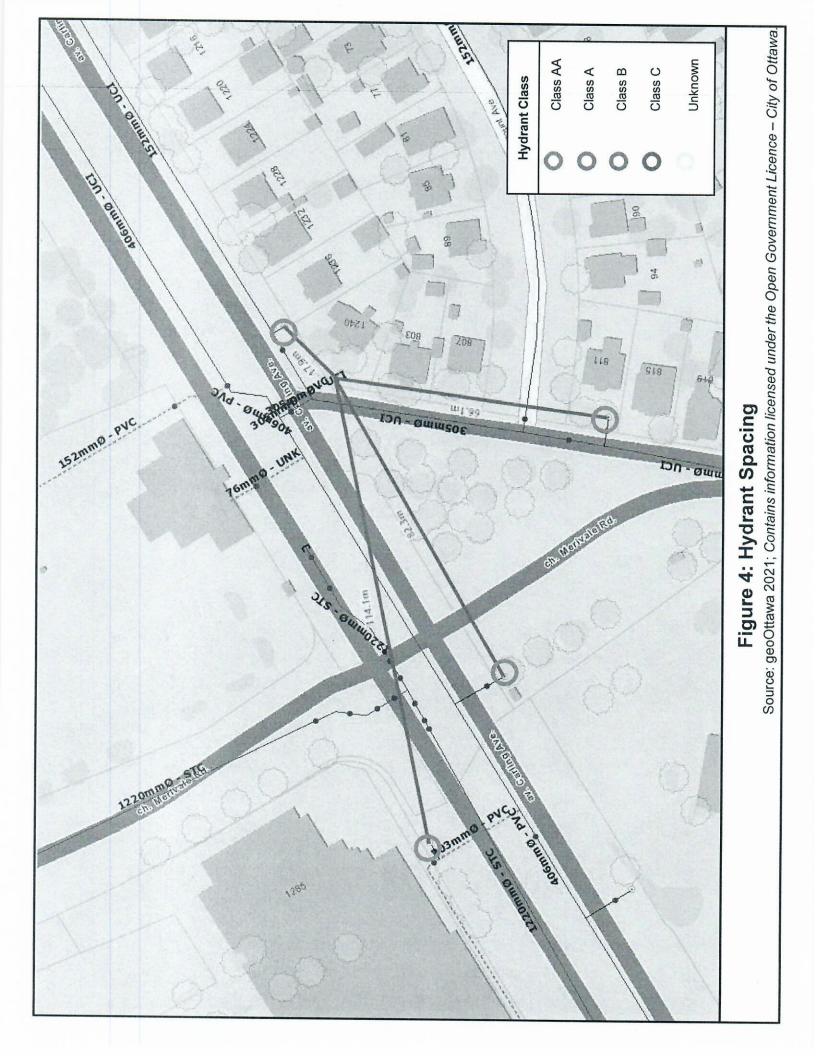
### To summarize:

Scenario 1: Minimum Pressure under Peak Hour Demand: 475 kPa (69 psi)

Scenario 2: Maximum Pressure under Average Day Demand: 558 kPa (81 psi)

Scenario 3: Minimum Pressure under Maximum Day + Fire Flow Demand: 265 kPa (38 psi)

**ATTACHMENT 10: FIGURE 4 - HYDRANT SPACING** 



# PROPOSED THREE STOREY RESIDENTIAL APARTMENT BUILDING SITE LOT 3 (R-PLAN 287570) 1240 CARLING AVENUE CITY OF OTTAWA

### APPENDIX E SANITARY SEWER DESIGN SHEET SHEET No. 1 OF 1

	population in 1000's = area in hectares	SEWER GapacityFull flow Actual (L/s) velocity velocity  n=\triangle (m/s) O(d)	1.12	MENT LOIL
	P popu	SEWER Capacity (L/s) n=0013	6 6 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	AVENUE APPATIME OTTAW
	(L/s) (L/s) (s) where A	PROPOSED Type Grade of %	Co. J. Co	5000
	$M = 1 + \frac{14}{4 + \sqrt{p}}$ where P $Q(p) = \frac{PqM}{86.4}$ (L/s) $\frac{PqM}{86.4}$ (L/s) where A $Q(1) = 1\Lambda$ (L/s) $+ Q(1)$ (L/s)	Type of pipe	No.	202
	Q(p)		<u> </u>	HOCAL WREE SITE-
SHEET		Longth (m)		12/ 056b
	Par John	Peak design flow O(d) (L/s)	#26.0	PROJECT 12 PROJECT 12 PUNLDINA
DESIGN	1-2-1 ppu	Peak ixtraneous flow Q(i) (L/s)	6.0.0	200
	1122	Pop. 110w 83 Q(p) (1./5)	742.0	TLM 762
SEWER	RESIDENTIAL 2 BEDROOM 1 BEDROOM	Peaking factor M	#	DESIGN CHECKED DATE JANE
	RES -	3	240.0	PA G B
TARY		CUMULATIVE Area A	20.6	在51
SANI		DUAL Area A hectares	540.0	
S	ap, d)	NDIVIDUAL Area Pop. hectar	26.6	TILE# 827
	average daily per capita flow (22/1/cap, d) = unit of peak extraneous flow (22/1/ha. s) = peaking factor (p) = peak population flow (L/s) (i) = peak extraneous flow (L/s) (d) = peak design flow	10	5AN 5250 5AN 5250	
	aily per capita flow (2 ak extraneous flow (2 population flow (1/s) extraneous flow (1/s) design flow	FROM	SITE	
77	q = average daily per capita flow (2) = unit of peak extraneous flow (2) M=peaking factor Q (p) = peak population flow (1/s) Q (i) = peak extraneous flow (1/s) Q (d) = peak design flow	STREET	CARLING AVENNE AVENNE AVENNE TOWY L	

## PROPOSED THREE STOREY RESIDENTIAL APARTMENT BUILDING SITE LOT 3 (R-PLAN 287570) 1240 CARLING AVENUE CITY OF OTTAWA

### APPENDIX F DEVELOPMENT SERVICING STUDY CHECKLIST SUMMARY





### Servicing study guidelines for development applications

### 4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

### 4.1 General Content

	Executive Summary (for larger reports only).
×	Date and revision number of the report.
×	Location map and plan showing municipal address, boundary, and layout of proposed development.
×	Plan showing the site and location of all existing services.
	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.
	Summary of Pre-consultation Meetings with City and other approval agencies.
	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defendable design criteria.
×	Statement of objectives and servicing criteria.
×	Identification of existing and proposed infrastructure available in the immediate area.
	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).
	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fil constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.
	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.
	Proposed phasing of the development, if applicable

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- ☑ Reference to geotechnical studies and recommendations concerning servicing.
- All preliminary and formal site plan submissions should have the following information:
  - Metric scale
  - · North arrow (including construction North)
  - Key plan
  - Name and contact information of applicant and property owner
  - ${\scriptstyle \circ}$  Property limits including bearings and dimensions
  - Existing and proposed structures and parking areas
  - · Easements, road widening and rights-of-way
  - Adjacent street names

### 4.2 Development Servicing Report: Water

Confirm consistency with Master Servicing Study, if available
Availability of public infrastructure to service proposed development
Identification of system constraints
Identify boundary conditions
Confirmation of adequate domestic supply and pressure
Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.
Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design
Address reliability requirements such as appropriate location of shut-off valves
Check on the necessity of a pressure zone boundary modification.
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range





×	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.
	Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.
×	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.
×	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.
	4.3 Development Servicing Report: Wastewater
×	Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).
	Confirm consistency with Master Servicing Study and/or justifications for deviations.
	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.
×	Description of existing sanitary sewer available for discharge of wastewater from proposed development.
	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)
×	Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.
	Description of proposed sewer network including sewers, pumping stations, and forcemains.
	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).
	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.
	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.
	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.
	Special considerations such as contamination, corrosive environment etc.





### 4.4 Development Servicing Report: Stormwater Checklist

	drain, right-of-way, watercourse, or private property)
	Analysis of available capacity in existing public infrastructure.
×	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.
×	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.
	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.
×	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.
	Set-back from private sewage disposal systems.
	Watercourse and hazard lands setbacks.
	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.
	Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.
×	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).
	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.
×	Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.
	Any proposed diversion of drainage catchment areas from one outlet to another.
×	Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.
	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.
	Identification of potential impacts to receiving watercourses
	Identification of municipal drains and related approval requirements.
×	Descriptions of how the conveyance and storage capacity will be achieved for the development.
×	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBF) and overall grading





	Inclusion of hydraulic analysis including hydraulic grade line elevations.
	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.
	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.
	Identification of fill constraints related to floodplain and geotechnical investigation.
	4.5 Approval and Permit Requirements: Checklist
	The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:
	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lake and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.
	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.
	Changes to Municipal Drains.
	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)
	4.6 Conclusion Checklist
×	Clearly stated conclusions and recommendations
	Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.
×	All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario