

4 Campbell Reid Court, Dunrobin  
Site Servicing and Stormwater Management Report



Project # CW-05-21

Prepared for:

Dr. Andrzej Olender

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Ottawa, ON, K2W 1B6

By:

*Arch-Nova Design Inc.*

September 2022

Updated April 2023

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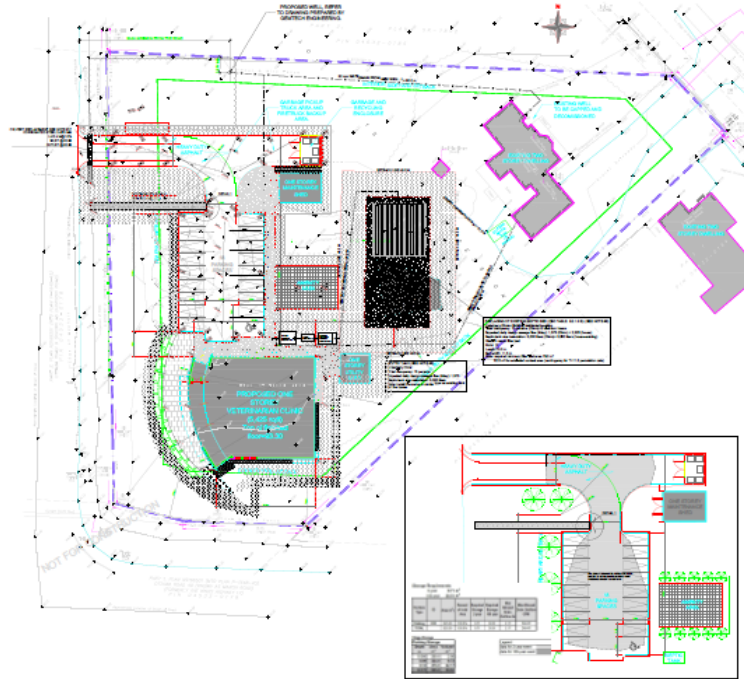
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## 1. Introduction

The subject property is located at 4 Campbell Reid Court, Ottawa. The proposed work comprises of a veterinary clinic building, a parking and amenity area and storage sheds.



**4 Campbell Reid Court, Ottawa: Site Plan**

Currently the property is used as a residential with a house located along the north edge of the property. The property is surrounded by Dunrobin Road on its west side, March Road on south and Cameron Harvey Road on its north edge. The east side borders with another residential property. Between the property and Dunrobin and March Road an existing drain runs toward south and south-east. Its purpose is to drain water from properties along Dunrobin Road. The grade of the property raises from 93.10 m on east corner to 93.49 m on its west side at the entrance to the site.

The drain's depth varies from 0.3m to 0.5 m.

"Hydrogeological Investigation and Terrain Assessment"<sup>1</sup> report provided information on existing and proposed water well and septic systems on site. In general, the report suggested drilling a new well and installation of a new septic system for the for the clinic. Existing well is susceptible to increased chlorides and it is not recommended for

<sup>1</sup> GEMTEC: "Hydrogeological Investigation and Terrain Assessment" July 2022

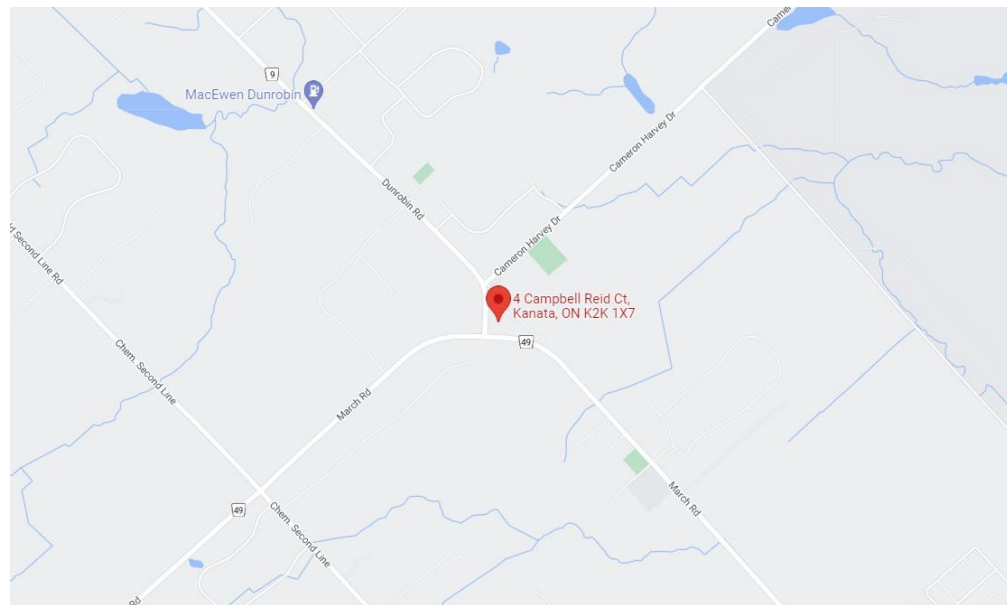
use in the clinic. A newly drilled well was tested and the flow and yield are recorded. A complete geo data as well as terrain analysis are provided in GEMTEC's report.

The new well was tested for 8 hours and samples were taken to the laboratory for: bacteriological, chemical, physical (hardness), metals, chlorides and total dissolved solids.

The water level was monitored for 30 days and fluctuation recorded was between 1.3-1.8 m below casing.

All tested materials had elevated results however, after repeated pumping some of readings decreased to acceptable levels. This trend of normalizing of all components is expected in the future.

In conclusion the report assessed the new well (TW22-1) as capable of pumping 20 m<sup>3</sup> of water daily and to supply both: the clinic and residential building if required.



4 Campbell Reid Court, Ottawa: Location

## 2. Public Services Capacity

This section of the report will analyze existing municipal services and the potential impact of the proposed building at 4 Campbell Reid Court on the existing service capacity.

### 2.1 Water Supply

The entire site is supplied by a private well. The capacity of newly drilled well is capable of pumping 20 m<sup>3</sup> of water daily and to supply both: the clinic and residential building.



The existing well is deemed as unsuitable for domestic water supply as the level of chlorides and nitrates is above acceptable. The well will be abandoned and capped.

The new well is proposed to be connected to the clinic by 32 mm pipe as well as a second branch 32 mm diameter to the residential building. It was recommended to have a hydrocell pumping system inside the clinic building as well as a small tank so the well pump is not running all the time. The house should have the same hydrocell system too.

Fire protection in the area is provided from Fire Station #45 at 640 Cameron Harvey Drive. Distance from the fire station to 4 Campbell Reid is 580 m.

Calculated fire demand is 3,000 l/min or 360 m<sup>3</sup> of water for duration of 2 hours.

Following table is showing required flow for establishment like a veterinary clinic and required pressure for operation of the hydrocell pumps inside the building. It will be a responsibility of the owner and contractor to choose and install the pumping system and a reservoir, if required. Also, the pressure in the plumbing system will be determined by a mechanical engineer based on the equipment requirements.

**Water Supply Design Criteria**

Design Parameter	Value
Residential Average Apartment	1.8 P/unit
Residential Average Daily Demand	280 L/d/P
Residential Maximum Daily Demand	9.5 x Average Daily *
Residential Maximum Hourly	1.5 x Maximum Daily *
Other Occupancy OBC Table 8.2.1.3 B: Vet Clinic	275/practitioner, 75/employee (8 hour shift)
Commercial Maximum Daily Demand	1.5 x Average Daily
Commercial Maximum Hourly	1.8 x Maximum Daily
Minimum Watermain Size	-----
Minimum Depth of Cover	2.4m from top of watermain to finished grade
During Peak Hourly Demand operating pressure must remain within	275kPa and 552kPa (40-80 psi; 28-56m)
* Residential Max. Daily and Max. Hourly peaking factors per MOE Guidelines for Drinking-Water Systems Table 3-3 for 0 to 500 persons.	

**Table 1: Water Supply Design Criteria**



**Fire Station #45: location and distance**

## **2.2 Sanitary Sewer**

Sanitary sewer service will be provided on site by a septic system. The new development is characterized as a medical practice (veterinary clinic) with total of 3 practitioners and 10 personnel for duration of one shift of 8 hours. Ontario Building Code (OBC) "Table 8.2.1.3.B - Other Occupancies" is used for sizing the onsite sanitary system: for practitioners 275 l/day and for employees per 8-hour shift 75 l/day.

Infiltration and inflow of 0.33 l/s/ha was considered for the calculation however, the property drains toward drain channels around and away from the proposed location of sewer laterals and septic tank so the infiltration rate into the sewer laterals and the septic tank is assessed as very small to insignificant.

In addition to the statement above it is anticipated that the filter bed will be fully raised construction so no impact from ground water is expected.

The estimated outflow for the new building is **0.02 l/sec** (Average Dry Weather Flow).

Detailed calculation of pre and post development flow is presented in Appendix A.

## **2.3 Septic System**

As the area is not serviced by municipal sewage, an onsite septic system is proposed. Based on the outflow calculation of 1,575 l/day, required primary septic tank capacity is calculated to be 5,000 liters.

Existing residential building is already serviced by a septic system and fully raised filter bed. This system also has a capacity of 5,000 liters for the septic tank.

A recommendation is to install a 10,000 liters tertiary system to service both, the clinic and the residential building. The existing septic system for the residential building will be disconnected and cleaned from the property upon the new system is built. similar model for the veterinary clinic for the reason of standardized maintenance and operation.

The proposed filter bed will be Class 4 raised filter bed and it has 10 (ten) runs. Total inflow to the filter bed from the clinic and the house is calculated to be 3,900 l/day. Percolation rate is estimated to 11.5 min/cm with loading rates of 10 l/m<sup>2</sup>/day. Minimum required filter bed loading area is calculated to 78.0 m<sup>2</sup> however, the bed is sized 110 m<sup>2</sup> with extended contact area of 52.76 m<sup>2</sup>. The reason for oversizing is in potential lower rate of loading.

The design will be submitted to the Ottawa Septic System Office (OSSO) upon the acceptance of conceptual design provided in this report.

As stated above, it is recommended to construct the same type of septic and filter bed as the existing one. As-built plans of existing system are presented in Appendix C .

## **2.4 Site Stormwater Services**

Current site and the rest of surface of the lot at 4 Campbell Reid Court represent a typical rural site with a residential building on north side and forested and open space toward south. In this area the existing sewage filter bed is located. All stormwater runoff is under uncontrolled condition. Along the property's west side and Dunrobin road an existing drain runs in direction north-south. Current entrance from Dunrobin to the site will be retained. A culvert is installed at the location. It appears as a concrete pipe. As a new access driveway will be wider new culverts are proposed: twin 375 mm corrugated steel pipes, 10.4 m long. There are no other significant physical features of the area such as depressions, wet land or surface retention.

The proposed development will have large impervious areas such as roofs, parking's and driveways. Modified rational method was used to assess an excess runoff and onsite storage volume. An assumption was that the predevelopment runoff coefficient is C=0.25. The ground water level is relatively low so the infiltration is good with minimum or no surface runoff.

For the post development analysis, the sub-catchments are delineated. The uncontrolled runoff area is measured 0.3974 ha with weighted factor of 0.38 (landscape, building, amenity areas, sheds).

Surface Type	ID	Area (ha)	Percent of total Area	C	A X C (ha)
Landscape	A3	0.2297	57.8%	0.25	0.057
Amenity Area	A4	0.1172	29.5%	0.40	0.047
Building	A5	0.0505	12.7%	0.90	0.045
<b>TOTAL</b>		0.3974	100.0%		0.150
<b>Weighted C =</b>					0.38

The controlled area is measured 0.08452 ha with weighted runoff factor of 0.9 (parking).

Surface Type	ID	Area (ha)	Percent of total Area	C	A X C (ha)
Parking	A1	0.0757	89.6%	0.90	0.068
Shed 1	A2	0.0051	6.0%	0.90	0.005
Shed 2	A6	0.0037	4.4%	0.90	0.003
<b>TOTAL</b>		0.08452	4.4%		0.076
<b>Weighted C =</b>					0.90

The parking area will be graded to create a storage for excess of water. Total controlled runoff is calculated to be 3.77 l/sec.

#### Predevelopment Runoff:

##### Uncontrolled Runoff

2-year	36.01	l/sec
100-year	104.67	l/sec

##### Controlled Runoff:

2-year	0.00	l/sec
100-year	0.00	l/sec

#### Postdevelopment Runoff:

##### Uncontrolled Runoff

2-year	32.24	l/sec
100-year	93.71	l/sec

##### Controlled Runoff:

2-year	16.24	l/sec
100-year	41.96	l/sec

#### Controlled allowable runoff

##### Controlled Runoff:

<b>2-year</b>	<b>3.77</b>	<b>l/sec</b>
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100-year storage volume required to be stored on site and released under predevelopment 2-year runoff is:

**Stage-Storage**

<b>Parking Storage</b>		
<b>Depth</b>	<b>Area</b>	<b>Volume</b>
m	m <sup>2</sup>	m <sup>3</sup>
0.030	250.0	2.50
<b>0.081</b>	<b>324.0</b>	<b>8.75</b>
<b>0.14</b>	<b>475.0</b>	<b>22.17</b>
<b>0.194</b>	<b>525.0</b>	<b>33.95</b>

The 100-year predevelopment runoff is 104.67 l/sec. The post development 100-year uncontrolled runoff is 93.71 l/sec and with the controlled outflow of 3.77 l/sec it will make total of 97.48 l/sec which is less than the predevelopment 100-year runoff.

Difference in elevations between proposed parking lot and the existing drain is only 0.5 m so any standard inlet control device cannot be installed. For such a reason a curb installed 100 mm pipe with opening of 60% was designed. It releases water into a swale 1.45 m wide and 23.0 m long at 2.1% slope, connecting the parking and the ditch. The bottom of the swale is proposed to be covered with a large river rock material. It will also serve capture small particles before water reaches the ditch.

Top of the curb is set at 0.175 m above the bottom of inlet. It will serve as the storage depth control as well as an emergency overflow point.

## **2.5 Site Grading**

The site grading is designed to create a storage for the stormwater on the parking area and to evacuate water from the access areas to the facility. Slopes toward existing drain and the site access driveway maintained and will not change general pattern of runoff or cause erosion.

Area south-west from existing filter bed is reserved for the extension and the grading will match the existing grading of the filter bed.

## **3. Conclusion and Recommendation**

### **3.1 Water Supply**

The site is serviced from existing well which is deemed as not suitable based on the quality of water. A new well was drilled and it has sufficient capacity to service both: the residential house and the new veterinary clinic.

### 3.2 Sanitary Sewer

The site (existing house) is serviced by a septic system however a new filter bed is proposed and will serve both: the house and the clinic. A new septic tank for the clinic and the existing house with capacity of 10,000 liters will be installed beside the clinic's building and connected by forcemain the filter bed. The filter bed consists of 10 runs and with extended contact area.

The design will be submitted to the Ottawa Septic System Office (OSSO) upon the acceptance of conceptual design provided in this report.

### 3.3 Stormwater

The 100-year predevelopment runoff is 104.67 l/sec. The post development 100-year uncontrolled runoff is 93.71 l/sec and with the controlled outflow of 3.77 l/sec it will make total of 97.48 l/sec which is less than the predevelopment 100-year runoff.

Difference in elevations between proposed parking lot and the existing drain is only 0.5 m so any standard inlet control device cannot be installed. For such a reason a curb installed 100 mm pipe with opening of 60% was designed. It releases water into a swale 1.45 m wide and 23.0 m long at 2.1% slope, connecting the parking and the ditch. The bottom of the swale is proposed to be covered with a large river rock material. It will also serve capture small particles before water reaches the ditch.

Existing driveway/access to the site will remain in function but widened and with asphalt surface. As a new access driveway will be wider new culverts are proposed: twin 375 mm corrugated steel pipes, 10.4 m long. The existing ditch will remain unchanged as it appears operational. New swale connecting the parking and the ditch will function to direct water from the parking to the ditch as well as to dissipate water to settle particles (quality control).

Prepared by:

Zoran Mrdja, P.Eng., FEC

September, 2022  
Updated: April 2023



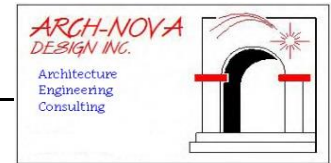
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## Appendix A: Calculations







**PRE-DEVELOPMENT (CONTROLLED RUNOFF)**

The pre-development time of concentration is **10** minutes

where:

$$I_2 = 732.951 / (Tc + 6.199)^{0.810}$$

$$I_2 = \mathbf{76.8 \text{ mm/hr}}$$

$$I_{100} = 1735.688 / (Tc + 6.014)^{0.820}$$

$$I_{100} = \mathbf{178.6 \text{ mm/hr}}$$

Surface Type	ID	Area (ha)	Percent of total Area	C	A X C (ha)
Site	A1	0.00000	0.0%	0.25	0.000
<b>TOTAL</b>		0.0000	0.0%		0.000
<b>Weighted C =</b>					0.00

$$Q_{2pre} = (2.78) \cdot (C) \cdot (I_2) \cdot (A)$$

$$Q_{2pre} = 2.78 \times 0.00 \times 76.8 \times 0.0000$$

$$Q_{2pre} = \mathbf{0.00 \text{ L/s}}$$

$$Q_{100pre} = (2.78) \cdot (C) \cdot (I_{100}) \cdot (A)$$

$$Q_{100pre} = 2.78 \times 0.00 \times 178.6 \times 0.0000$$

$$Q_{100pre} = \mathbf{0.00 \text{ L/s}}$$

C=0.6 used for predevelopment calculation (City of Ottawa requirement)

**POST-DEVELOPMENT (CONTROLLED RUNOFF)**

The post-development time of concentration is **10** minutes

where:

$$I_2 = 732.951 / (Tc + 6.199)^{0.810}$$

$$I_2 = \mathbf{76.8 \text{ mm/hr}}$$

$$I_{100} = 1735.688 / (Tc + 6.014)^{0.820}$$

$$I_{100} = \mathbf{178.6 \text{ mm/hr}}$$

Surface Type	ID	Area (ha)	Percent of total Area	C	A X C (ha)
Parking	A1	0.0757	89.6%	0.90	0.068
Shed 1	A2	0.0051	6.0%	0.90	0.005
Shed 2	A6	0.0037	4.4%	0.90	0.003
<b>TOTAL</b>		0.08452	4.4%		0.076
<b>Weighted C =</b>					0.90

$$Q_{2post} = (2.78) \cdot (C) \cdot (I_2) \cdot (A)$$

$$Q_{2post} = 2.78 \times 0.90 \times 76.8 \times 0.0845$$

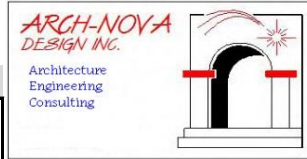
$$Q_{2post} = \mathbf{16.24 \text{ L/s}}$$

$$Q_{100post} = (2.78) \cdot (C) \cdot (I_{100}) \cdot (A)$$

$$Q_{100post} = 2.78 \times 1.00 \times 178.6 \times 0.0845$$

$$Q_{100post} = \mathbf{41.96 \text{ L/s}}$$

## ALLOWABLE RUNOFF



### Predevelopment Runoff:

#### Uncontrolled Runoff

2-year	36.01	l/sec
100-year	104.67	l/sec

#### Controlled Runoff:

2-year	0.00	l/sec
100-year	0.00	l/sec

### Postdevelopment Runoff:

#### Uncontrolled Runoff

2-year	32.24	l/sec
100-year	93.71	l/sec

#### Controlled Runoff:

2-year	16.24	l/sec
100-year	41.96	l/sec

### Controlled allowable runoff

#### Controlled Runoff:

<b>2-year</b>	<b>3.77</b>	<b>l/sec</b>
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Comment:

### Storage Volumes (2-Year Storm)

Project: 384 Frank St.

T<sub>c</sub> = 10 (mins)  
 C<sub>AVG</sub> = 0.90 (dimensionless)  
 Area = 0.0845 (hectares)  
 Storm = 2 (year)  
 Release Rate = 3.77 (L/sec)  
 Time Interval = 10 (mins)

Duration (min)	Rainfall Intensity (mm/hr)	Peak Flow (L/sec)	Release Rate (L/sec)	Storage Rate (L/sec)	Storage (m <sup>3</sup> )
1	148	3.1	3.77		
11	73	15.5	3.77	11.70	7.72
21	50	10.7	3.77	6.91	8.70
31	39	8.3	3.77	4.51	8.40
41	32	6.8	3.77	3.06	7.53
51	28	5.8	3.77	2.08	6.36
61	24	5.1	3.77	1.36	4.98
71	22	4.6	3.77	0.82	3.48
81	20	4.2	3.77	0.39	1.88
91	18	3.8	3.77	0.04	0.20
101	17	3.5	3.77	-0.25	-1.54
111	15	3.3	3.77	-0.50	-3.32
121	14	3.1	3.77	-0.71	-5.15
131	14	2.9	3.77	-0.89	-7.00
141	13	2.7	3.77	-1.05	-8.88
151	12	2.6	3.77	-1.19	-10.79
161	12	2.5	3.77	-1.32	-12.72
171	11	2.3	3.77	-1.43	-14.66
181	11	2.2	3.77	-1.53	-16.63
191	10	2.1	3.77	-1.62	-18.60
201	10	2.1	3.77	-1.71	-20.59
211	9	2.0	3.77	-1.78	-22.60
221	9	1.9	3.77	-1.86	-24.61
231	9	1.8	3.77	-1.92	-26.63
241	8	1.8	3.77	-1.98	-28.66
251	8	1.7	3.77	-2.04	-30.70
261	8	1.7	3.77	-2.09	-32.75
271	7.7	1.6	3.77	-2.14	-34.80

#### Notes

- 1) For a storm duration that is less than the time of concentration the peak flow is equal to the product of 2.78CIA and the ratio of the storm duration to the time of concentration.
- 2) Rainfall Intensity, I = 732.951 / (T<sub>c</sub> + 6.199)<sup>0.810</sup> (2 year, City of Ottawa)
- 3) Peak Flow = Duration/T<sub>c</sub> x 2.78 x C x I x A (Duration < T<sub>c</sub>)
- 4) Peak Flow = 2.78 x C x I x A (Duration > T<sub>c</sub>)
- 5) Storage = Duration x Storage Rate

### Storage Volumes (100-Year Storm)

T<sub>c</sub> = 10 (mins)  
 C<sub>AVG</sub> = 1.00 (dimensionless)  
 Area = 0.0845 (hectares)  
 Storm = 100 (year)  
 Release Rate = 3.77 (L/sec)  
 Time Interval = 10 (mins)

Duration (min)	Rainfall Intensity (mm/hr)	Peak Flow (L/sec)	Release Rate (L/sec)	Storage Rate (L/sec)	Storage (m <sup>3</sup> )
1	351	8.3	3.77		
11	170	39.9	3.77	36.15	23.86
21	116	27.3	3.77	23.56	29.68
31	90	21.1	3.77	17.34	32.25
41	74	17.3	3.77	13.58	33.41
51	63	14.8	3.77	11.04	33.79
61	55	13.0	3.77	9.20	33.69
71	49	11.6	3.77	7.81	33.25
81	45	10.5	3.77	6.70	32.58
91	41	9.6	3.77	5.81	31.72
101	38	8.8	3.77	5.07	30.72
111	35	8.2	3.77	4.44	29.60
121	33	7.7	3.77	3.91	28.39
131	31	7.2	3.77	3.45	27.10
141	29	6.8	3.77	3.04	25.74
151	27	6.5	3.77	2.69	24.33
161	26	6.1	3.77	2.37	22.86
171	25	5.8	3.77	2.08	21.35
181	24	5.6	3.77	1.82	19.80
191	23	5.4	3.77	1.59	18.21
201	22	5.1	3.77	1.38	16.60
211	21	4.9	3.77	1.18	14.95
221	20	4.8	3.77	1.00	13.28
231	20	4.6	3.77	0.84	11.58
241	19	4.5	3.77	0.68	9.87
251	18	4.3	3.77	0.54	8.13
261	18	4.2	3.77	0.41	6.38
271	17	4.1	3.77	0.28	4.60

#### Notes

- 1) For a storm duration that is less than the time of concentration the peak flow is equal to the product of 2.78CIA and the ratio of the storm duration to the time of concentration.
- 2) Rainfall Intensity, I = 1735.688 / (T<sub>c</sub> + 6.014)<sup>0.820</sup> (100 year, City of Ottawa)
- 3) Peak Flow = Duration/T<sub>c</sub> x 2.78 x C x I x A (Duration < T<sub>c</sub>)
- 4) Peak Flow = 2.78 x C x I x A (Duration > T<sub>c</sub>)
- 5) Storage = Duration x Storage Rate



**Storage Requirements**

2-year **8.70 m<sup>3</sup>**  
 100-year **33.79 m<sup>3</sup>**

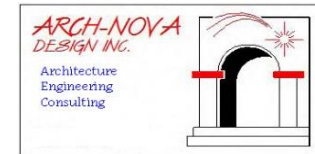
Surface Type	ID	Area (m <sup>2</sup> )	Percent of total Area	Required Storage 2 year	Required Storage 100 year	Max Allowed Drain Outflow l/s	Max Allowed Drain Outflow GPM
Parking	ICD	525.00	100.0%	8.70	33.79	3.77	59.73
<b>TOTAL</b>		525.00	100.0%	8.70	33.79	3.77	59.73

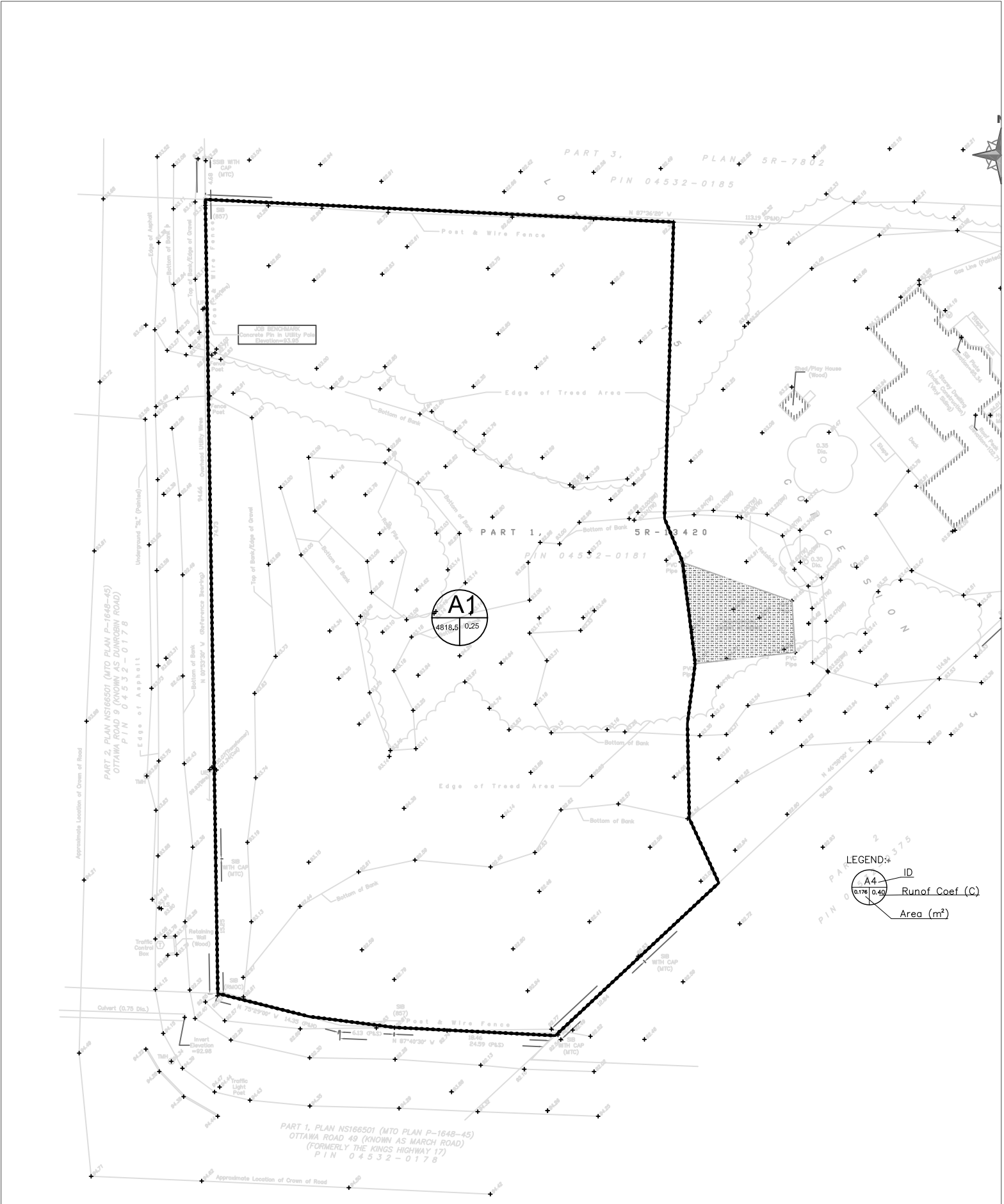
**Stage-Storage**

<b>Parking Storage</b>		
Depth m	Area m <sup>2</sup>	Volume m <sup>3</sup>
0.030	250.0	2.50
<b>0.081</b>	<b>324.0</b>	<b>8.75</b>
<b>0.14</b>	<b>475.0</b>	<b>22.17</b>
<b>0.194</b>	<b>525.0</b>	<b>33.95</b>

Legend:	
data for 2-year event	
data for 100-year event	

Notes:

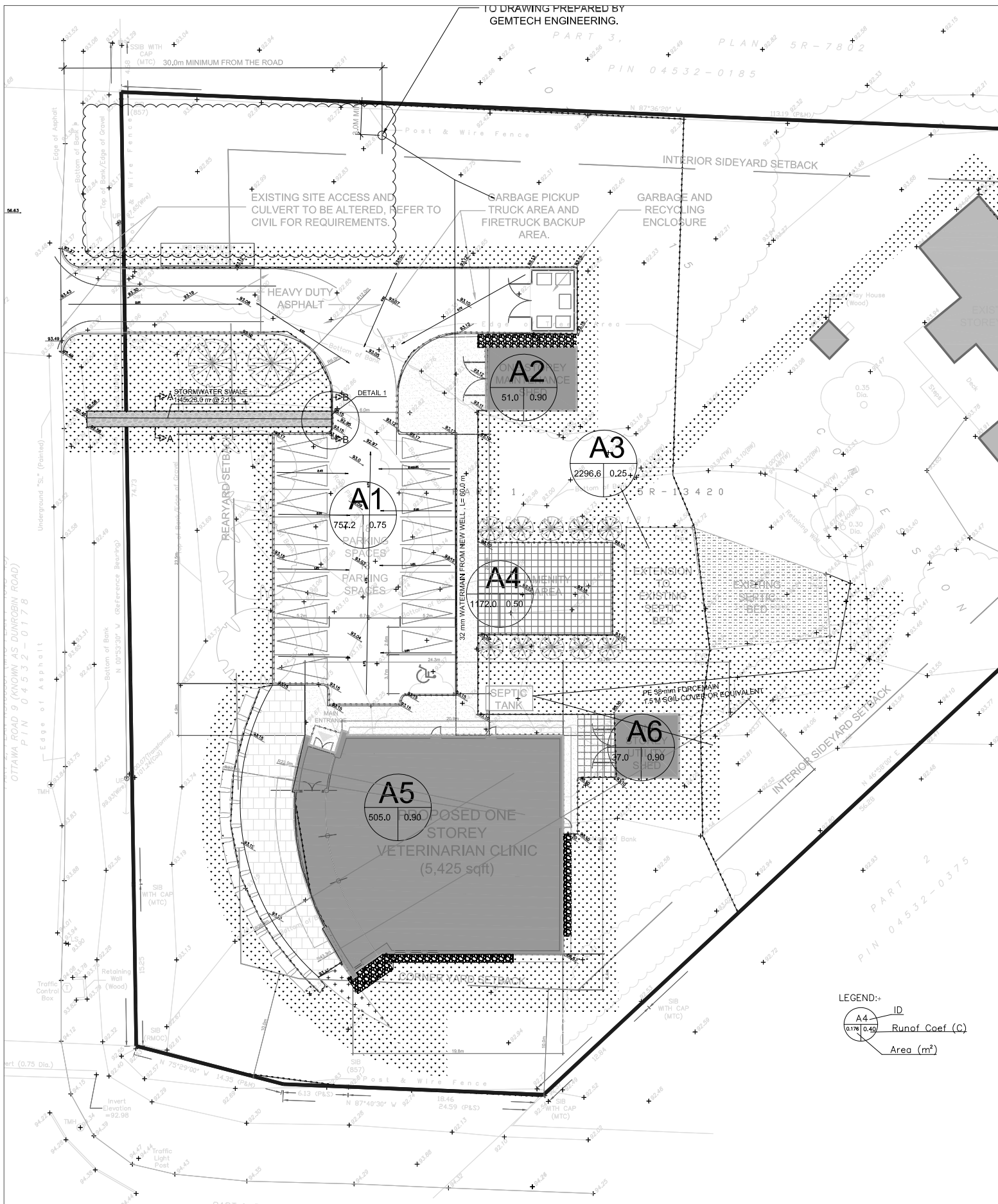




**4 Campbell Reid Crt., Donrobin  
 SWM PREDEVELOPMENT**

**ARCH-NOVA Design Inc.**

45 Banner Road NEPEAN ON K2H 8X5  
 613-702-3403 contact@archnova.ca





**DESIGN CALCULATIONS  
FOR CLASS 2, 4 & 5 ON-SITE SEWAGE SYSTEM**

Owner: Dr. Olander	Designer: Mrdja	2022-09-23
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**STEP 1**

**DAILY SEWAGE FLOW** (Based on Hydraulic Loads for Fixtures, Floor Area, and Number of Bedrooms)

Plumbing Fixture Description	Existing Number of Fixtures	Proposed Number of Fixtures	Total x Fixture Units Value = Number of Fixture Units		
Bathroom group (toilet, sink, bathtub)	2		2	6	12
Toilet (alone)	1		1	4	4
Washbasin	2		2	1.5	3
Bathtub or Shower	1		1	1.5	1.5
Kitchen Sink(s)	1		1	1.5	1.5
Bar Sink			0	1.5	0
Dishwasher	1		1	1.5	1.5
Washing Machine	1		1	1.5	1.5
Bidet			0	1	0
Laundry Tub	1		1	1.5	1.5
Other:			0		0
<b>Total Fixture Units</b>					<b>26.5</b>
<b>Proposed:</b>	220	m <sup>2</sup>	2368.1 ft <sup>2</sup>		
<b>Existing:</b>	195	m <sup>2</sup>	2099 ft <sup>2</sup>		
<b>Total Finished Floor Area Excluding Area of Finished Basement</b>					
0.00 m <sup>2</sup>					
0.00 ft <sup>2</sup>					
(Multiply m <sup>2</sup> x 10.764 = ft <sup>2</sup> )					

From the chart below, please calculate the expected daily sewage flow for your proposed building, and mark the total in the space provided. For non-residential occupancies see Table 8.2.1.3 B O.B.C.

Residential Occupancy	Existing	Q in Litres	Calculations
1 Bedroom		750	0
2 Bedrooms		1100	0
3 Bedrooms		1600	0
4 Bedrooms	1	2000	2000
5 Bedrooms		2500	0
<b>Additional Flow for:</b>			0
Each Bedroom over 5		500	0
Floor Space for each 10m <sup>2</sup> over 200 m <sup>2</sup> up to 400 m <sup>2</sup>	0	100	0
Floor Space for each 10m <sup>2</sup> over 400 m <sup>2</sup> up to 600 m <sup>2</sup>		75	0
Floor Space for each 10m <sup>2</sup> over 600 m <sup>2</sup> <b>OR*</b>		50	0
Each fixture unit over 20 fixture units total	6.5	50	325
<b>Total</b>			<b>2325</b>

*\*NOTE: Where you need to do multiple calculations, signified by the "OR" in the table, do the calculation for daily sewage flow based on bedrooms and floor space first, then fixture units, and use the larger of the two calculations.*

Other Occupancy (Table 8.2.1.3 (B))			
Establishment Type: Veterinary Clinic	Occupant Load	Volume (Liters)	Calculations
Per practitioner	3	275	825
Per employee per 8 hours shift	10	75	750
Per stall, kennel or cage if floor drain connected		75	0
<b>Total</b>			<b>1575</b>

**EXPECTED DAILY DESIGN SEWAGE FLOW (Q) 3,900.00** Liters

**(Use Q for the following calculations)**

## STEP 2

### PROPERTY SOIL PROFILE AND PERCOLATION RATE (T) DESCRIPTION

Soil Type	Coarse Gravel, no fines	Gravel, some small rocks	Gravel, sand mix, some fines	Sand, fairly uniform, some fines	Sandy, Loam mix	Silty, Loam, almost clay	Clay, smears well, rolls into ribbon
T-time Min/cm	0 to 1	1 to 5	5 to 10	10 to 15	15 to 25	25 to 50	> 50

**ON-SITE PROFILE** (Subtract useable depth of Soil from 1.5m (5') for depth of imported fill)

Soil Depth Meters	Percolation Rate T	Soil Type (see above)	Depth of Rock/Impervious Soil/Groundwater Table $\checkmark$	Topsoil to be removed:
0.2				Depth <input type="text" value=""/> m <input type="text" value="0"/> (ft)
0.4				<b>Usable Existing Soil:</b>
0.6	11.5	sand	1	Depth <input type="text" value="0.25"/> m <input type="text" value="0.825"/> (ft)
0.8				<b>Imported Fill:</b>
1.0				Depth <input type="text" value="0.75"/> m <input type="text" value="2.475"/> (ft)
1.2				Percolation Rate (T) <input type="text" value="10"/> min/cm
1.4				<b>Excavation of Existing Soil:</b>
1.6				Depth <input type="text" value=""/> m <input type="text" value="0"/> (ft)

#### CONTACT AREA CALCULATION

If you do not have a minimum of 250 mm (10") of useable soil on the property, you will need to import the mantle, or contact area. Choose T range, divide Q by Loading Rate for T.

Percolation Time (T) of Soil (min/cm)	Loading Rates (L/m <sup>2</sup> /day)	Q
1 < T ≤ 20	10	3900
20 < T ≤ 35	8	
35 < T ≤ 50	6	
T > 50	4	
Column 1	2	

Contact Area  Daily Sewage Flow (Q) ÷ Loading Rate =  m<sup>2</sup>

### STEP 3

**A) SEPTIC TANK SIZE CALCULATION** To calculate the minimum capacity of your septic tank, use the following formulas. Minimum tank size is 3600 Litres.

Residential:	Q =	2325	2 x Q =	4650	Litres	Tank Size:	5,000.00
Other Occupants:	Q =	1,575.00	3 x Q =	4,725.00	Litres	Tank Size:	5,000.00

**B) LEACHING BED LENGTH CALCULATION** (Divide meters by 0.305 to convert to feet)

Length (m) = (QxT)/200	224.25	(m)	735.25	ft
------------------------	--------	-----	--------	----

#### DESCRIPTION

Number of Runs:	10	D - Box	Y	N	Header	Y	N
Distance between runs	1.60						
Run length	22.43						
Leaching Bed Width	33.00						
Leaching Bed Area	872.03						

#### FILTER BED

Where you may not have sufficient area on your property to install a leaching bed, you may install a filter bed for your distribution system.

**FILTER BED CALCULATION** (Multiply m<sup>2</sup> x 10.764 to convert T to ft<sup>2</sup>)

If your daily sewage flow is less than 3000 litres per day, perform calculation 1), or if your daily sewage flow exceeds 3000 litres per day, perform calculation 2).

<b>1) Filter Bed Surface Area = Q ÷ 75 expressed as m<sup>2</sup></b>			
Q	3,900.00	/	75 = 52.00 m <sup>2</sup>
<b>2) Filter Bed Surface Area = Q ÷ 50 expressed as m<sup>2</sup></b>			
Q	3,900.00	/	50 = 78.00 m <sup>2</sup>

Where Percolation Rate T < 11.5 then Q/75 = Area of filter bed 52 m<sup>2</sup> or 559.7 ft<sup>2</sup>

The total square area is calculated by measuring the length, and multiplying it against the width. In most instances, the filter bed is constructed long and narrow, as opposed to a square. This helps the bed "breathe," as more oxygen can penetrate the filter bed from the sides, and from above.

Filter Bed Loading Area	78.00	m <sup>2</sup>	Length m	11	Width m	7.09
	839.59	ft <sup>2</sup>	Length ft	36.30	Width ft	23.40

#### EXTENDED CONTACT AREA

Where Percolation Rate T > 11.5 then (Q x T)/850 = Extended Contact Area 52.76 m<sup>2</sup> or 570.0 m<sup>2</sup>

Filter Bed Loading Area	52.76	m <sup>2</sup>	Length m	11	Width m	4.80
	567.96	ft <sup>2</sup>	Length ft	36.30	Width ft	15.83

**Water Supply Design Criteria**

<b>Design Parameter</b>	<b>Value</b>
Residential Average Apartment	1.8 P/unit
Residential Average Daily Demand	280 L/d/P
Residential Maximum Daily Demand	9.5 x Average Daily *
Residential Maximum Hourly	1.5 x Maximum Daily *
Other Occupancy OBC Table 8.2.1.3 B: Vet Clinic	275/practitioner, 75/employee (8 hour shift)
Commercial Maximum Daily Demand	1.5 x Average Daily
Commercial Maximum Hourly	1.8 x Maximum Daily
Minimum Watermain Size	150mm diameter
Minimum Depth of Cover	2.4m from top of watermain to finished grade
must remain within	275kPa and 552kPa (40-80 psi; 28-56m)
During fire flow operating pressure must not drop below	140kPa (20 psi; 14 m)
* Residential Max. Daily and Max. Hourly peaking factors per MOE Guidelines for Drinking-Water Systems Table 3-3 for 0 to 500 persons.	

**Water Demand and Boundary Conditions****Proposed Conditions**

<b>Design Parameter</b>	<b>Anticipated Demand<sup>1</sup> (L/min)</b>	<b>Boundary Condition<sup>2</sup> (m)</b>
Average Daily Demand	1.09	
Max Day + Fire Flow	3,001.64	
Peak Hour	2.95	
<sup>1</sup> Water demand calculation per Water Supply Guidelines. See Appendix B for detailed calculations. <sup>2</sup> Boundary conditions supplied by the City of Ottawa. See Appendix B for correspondence with the City.		

**Domestic Demand**

Type of Housing	Per / Unit	Units	Pop
Single Family	3.4	0	0
Semi-detached	2.7		0
Townhouse	2.7		0
Apartment			0
Bachelor	1.4		0
1 Bedroom	1.4		0
2 Bedroom	2.1		0
3 Bedroom	3.1		0
4 Bedroom	4.2	0	0

	Pop	Avg. Daily		Max Day		Peak Hour	
		m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min
<b>Total Domestic Demand</b>	0	0.00	0.00	0.00	0.00	0.00	0.00

**Institutional / Commercial / Industrial Demand**

Property Type	Unit Rate		Units	Avg. Daily		Max Day		Peak Hour	
				m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min
Commercial floor space	2.5	L/m <sup>2</sup> /d							
Office	75.0	L/9.3m <sup>2</sup> /d							
Veterinary Clinic	275.0	L/practitioner/d	3	0.83	0.57	1.24	0.86	2.23	1.55
Veterinary Clinic	75.0	L/employee/d	10	0.75	0.52	1.13	0.78	2.03	1.41
Industrial -Heavy	55,000.0	L/gross ha/d							
<b>Total I/C/I Demand</b>				1.58	1.09	2.36	1.64	4.25	2.95

<b>Total Demand</b>	1.58	1.09	2.36	1.64	4.25	2.95
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\* Estimated number of seats at 1seat per 9.3m<sup>2</sup>

**Water Demand and Boundary Conditions****Proposed Conditions****Water Supply Design Criteria**

Design Parameter	Value
Residential Average Apartment	1.8 P/unit
Residential Average Daily Demand	280 L/d/P
Residential Maximum Daily Demand	2.5 x Average Daily *
Residential Maximum Hourly	2.2 x Maximum Daily *
Commercial Demand	2.5 L / m <sup>2</sup> /d
Commercial Maximum Daily Demand	1.5 x Average Daily
Commercial Maximum Hourly	1.8 x Maximum Daily
Minimum Watermain Size	150mm diameter
Minimum Depth of Cover	2.4m from top of watermain to finished grade
must remain within	275kPa and 552kPa
During fire flow operating pressure must not drop below	140kPa
* Residential Max. Daily and Max. Hourly peaking factors per MOE Guidelines for Drinking-Water Systems Table 3-3 for 0 to 500 persons.	

Design Parameter	Anticipated Demand <sup>1</sup> (L/min)	Boundary Condition <sup>2</sup> (kPa)
Average Daily Demand	1.09	
Max Day + Fire Flow	3,002.95	
Peak Hour	2.95	
<sup>1</sup> Water demand calculation per Water Supply Guidelines. See Appendix B for detailed calculations. <sup>2</sup> Boundary conditions supplied by the City of Ottawa. See Appendix B for correspondence with the City.		

**Wastewater Design Criteria**

Design Parameter	Value
Residential Average Apartment	1.8 P/unit
Average Daily Demand	280 L/cap/day
Peaking Factor	Harmon's Peaking Factor. Max 4.0, Min 2.0
Correction Factor (City of Ottawa Tech.Bulletin ISTB-2018-0	0.8
Commercial Space	28,000 L/ha/day
Infiltration and Inflow Allowance	0.28L/s/ha
Sanitary sewers are to be sized employing the Manning's Equation	$Q = (1/n)AR^{2/3}S^{1/2}$
Minimum Sewer Size	200mm diameter
Minimum Manning's 'n'	0.013
Minimum Depth of Cover	2.5m from crown of sewer to grade
Minimum Full Flowing Velocity	0.6m/s
Maximum Full Flowing Velocity	3.0m/s
<i>Extracted from Sections 4 and 6 of the City of Ottawa Sewer Design Guidelines, November 2012.</i>	



## Sanitary Sewer Post Development Outflow

<b>Site Area</b>	<b>0.4 ha</b>
<b>Extraneous Flow Allowances</b>	
<b>Infiltration / Inflow</b>	<b>0.132 L/s</b>

**Domestic Contributions**

Unit Type	Unit Rate	Units	Pop
Single Family	3.4	0	0
Semi-detached and duplex	2.7		0
Duplex	2.3		0
Townhouse	2.7		0
Apartment			
Bachelor	1.4		0
1 Bedroom	1.4	0	0
2 Bedroom	2.1	0	0
3 Bedroom	3.1	0	0
4 Bedroom	4.2	0	0
<b>Total Population</b>			<b>0</b>
<b>Average Domestic Flow</b>			<b>0.00 L/s</b>
<b>Peaking Factor</b>			<b>3.80</b>
<b>Peak Domestic Flow</b>			<b>0.00 L/s</b>

**Institutional / Commercial / Industrial Contributions**

Property Type	Unit Rate	No. of Units	Avg Wastewater (L/s)
Commercial	28,000 L/gross ha/d	0.006215	0.00
Veterinary Clinic	275 l/unit/day	3	0.01
Veterinary Clinic	75 l/person/day	10	0.01
Institutional	28,000 L/gross ha/d	0	0.00
Industrial - Light	35,000 L/gross ha/d	0	0.00
Industrial - Heavy	55,000 L/gross ha/d	0	0.00
<b>Average I/C/I Flow</b>			<b>0.020</b>
<b>Peak Institutional / Commercial Flow*</b>			<b>0.030</b>
<b>Peak Industrial Flow**</b>			<b>0.000</b>
<b>Peak I/C/I Flow</b>			<b>0.030</b>

<b>Total Estimated Average Dry Weather Flow Rate</b>	<b>0.020</b>
<b>Total Estimated Peak Dry Weather Flow Rate</b>	<b>0.030</b>
<b>Total Estimated Peak Wet Weather Flow Rate</b>	<b>0.162</b>

FUS Fire Flow Calculations

Project: 4 Campbell Reid Court

Calculations Based on 1999 Publication "Water Supply for Public Protection Fire Protection " by Fire Underwriters' Survey (FUS)

Fire Flow Calculation #: 1

Date August 19, 2022 Building Type/Description/Name: Veterinary Clinic

Data input by: Zoran Mrdja, P.Eng.

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method									
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Framing Material							
		Coefficient related to type of construction (C)	Wood Frame	1.50	Fire resistive construction (>2 hrs)	0.60			
			Ordinary construction	1.00					
			Non-combustible construction	0.80					
			Fire resistive construction (< 2 hrs)	0.70					
Fire resistive construction (> 2 hrs)	0.60								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Floor Space Area							
		Type of Housing	Single Family	1	Other (Comm, ind)	1	Units		
			Townhouse - indicate # of units	1					
			Other (Comm, Ind, etc.)	1					
2.2	# of Storeys	Number of Floors/ Storeys in the Unit (do not include basement):					Storeys		
3	Enter Ground Floor Area of One Unit	Enter Ground Floor Area (A) of One Unit Only :							
		Measurement Units	Square Feet (ft <sup>2</sup> )	5425.0	area	504	Area in Square Meters (m <sup>2</sup> )		
			Square Metres (m <sup>2</sup> )	504	dist				
			Hectares (ha)	0					
4	Obtain Required Fire Flow without Reductions	Required Fire Flow( without reductions or increases per FUS) ( $F = 220 * C * \sqrt{A}$ ) Round to nearest 1000L/min						2,963	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	0.25	Non-combustible	-0.25	N/A	-741	
			Limited combustible	-0.15					
			Combustible	0.00					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Complete Automatic Sprinkler Protection	-0.3	None	0.00	N/A	0	
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	North Side	30.1-45 m	0.05	0.20	m	593	
			East Side	30.1-45 m	0.05				
			South Side	30.1-45 m	0.05				
			West Side	30.1-45 m	0.05				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1000 L/min, with max/min limits applied:						3,000	
		Total Required Fire Flow (above) in L/s:						50	
		Required Duration of Fire Flow (hrs)						2.00	
		Required Volume of Fire Flow (m <sup>3</sup> )						360	

Note: The most current FUS document should be referenced before design to ensure that the above figures are consistent with the intent of the Guideline

Legend	
	Drop down menu - choose option, or enter value.
	No information, No input required.

**Note:**  
The most current FUS document should be referenced before design to ensure that the above figures are consistent with the intent of the Guideline.

## Appendix B: Correspondence

Pre-application Consultation PC2019-0171

4 Campbell Crt

Meeting Notes

July 18, 2019

Attendees

Drs Olenders, owners

Dr Firestone, with owner

Tim Moore, agent

Sami Rehman, Environmental Planner

Kevin Hall, Project Manager

Michel Kearney, Hydrogeologist

Neeti Paudel, Transportation

Cheryl McWilliams, File Lead

Proposal:

- New 6000 sq ft veterinary hospital, combining their March and Dunrobin facilities
- Keep existing house (No basement renovation for house proposed now)
- Proposed new septic for vet building
- Access vet building off of Dunrobin Road

Transportation

*In the case that the access is proposed on Dunrobin Road;*

Please follow the Transportation Impact Assessment (TIA) Guidelines:

- Submit TIA Step 2 Scoping, a full TIA will be required.
- Please note that the application will not be deemed complete until it meets the necessary TIA components (Draft Steps 1-4 of the TIA). If roadway modifications are required, draft RMA package will have to be submitted with Step 4 of the TIA.

*In the case that the access is proposed on Campbell Reid Court – TIA will not be required.*

Right of way protection for March Road is 34m. Ensure this is protected.

Right of way to be protected for Dunrobin Road is 30m.

When submitting site plans:

- Show all details of the road abutting the site including the opposite curb. Pavement markings, accesses and sidewalks must be included.
- Turning templates are required for all accesses and internal movements. Ensure the turning templates show the largest vehicle turning in to and out of the site at all accesses.

- Curb radii at the access should be reduced as much as possible. Ensure the measurements are shown in the site plan.

Noise Impact Study is required for the following:

- Road. *Dunrobin Road and March Road are both classified as arterial roads.* Corner triangles (Dunrobin Road and March Road) as per OP Annex 1 - Road Classification and Rights-of-Way at the following locations on the final plan will be required: Arterial Road to Arterial Road: 10 metre x 10 metres

### Environment

- Part of the natural heritage system overlay
- Species at risk needs to be considered and woodlot linkages
- Requires Tree Conservation Report and Environmental Impact Statement
- The subject property is part of the Natural Heritage System (NHS) and as per Official Plan policies (Section 2.4.2 and 4.7.8), the proposed development requires an Environmental Impact Statement (EIS). This part of the NHS is one of the natural landscape linkages. The EIS must demonstrate that the proposed development will have no negative impacts to the natural landscape linkage and its associated ecological functions. Furthermore, the EIS should examine potential significant habitat for Species at Risk on the subject property (OP 4.7.4).
- Further details of the EIS requirements can be found in OP Section 4.7.8 and the EIS guidelines: [https://documents.ottawa.ca/sites/default/files/documents/eis\\_guidelines2015\\_en.pdf](https://documents.ottawa.ca/sites/default/files/documents/eis_guidelines2015_en.pdf)
- The proposed development will also require a Tree Conservation Report (TCR), as per OP Section 4.7.2. Further details of the requirements for the TCR can be found in the TCR guidelines:
- <https://ottawa.ca/en/residents/water-and-environment/trees-and-community-forests/protection#tree-conservation-report-guidelines>
- To avoid duplications, the TCR can be combined with the EIS.

### Engineering

- Stormwater management is required to demonstrate post to pre run off
- Contact Matt Craig at Mississippi Valley Conservation Authority for stormwater quality requirements
- Road side ditch should be the legal outlet – NEED TO CON.
- Require a geotechnical report
- Require no light spillage onto adjacent properties
- Show any on-site snow storage
- Geotech report will be required.
- Hydro-Geological report required
- Lighting certificate confirming the site light meets our standards
- Stormwater report. The CA should be contacted to confirm the quality requirements. They need to confirm they have a legal outlet for the runoff from this site.
- Consult what the onsite fire requirements will be.
- Access from Dunrobin and possible use of an access on to Campbell Reid.

### Fire

- May need storage tanks – comments provided by Fire Services
- Will need fire route shown on the site plan

#### Private Servicing (hydrogeological report required)

- Proposal would have separate well and septic for each use
- Concerns with separation distances
- Soils are thin (less than one metre)
- Septic capacity if more than 10000 l per day for the lot then needs Ministry of Environment Conservation and Parks approval (house typically 2-3000)

#### Planning

- Need to buffer the site and especially parking from March and Dunrobin roads, Best done through maintaining trees and undergrowth along those two roads. In particular along March and at the north west corner along Dunrobin Road.
- Looking for an enhanced building façade with any visibility to the roads. Looking to have the materials and colours for the most part 'fit' with a rural context. Some visibility from the roads for the building and signage would be appropriate.
- We will be looking for some screening off of Campbell Court abutting the property owner to the south to ensure buffering between any commercial use, access or parking and loading form that residential property.
- Access off of Campbell court may be best for safety
- The application would be a Site Plan – Rural based – Standard one.

## Appendix C: Guidelines, Existing Reports, Studies and References, Plans



The following studies were utilized in the preparation of this report:

- **Ottawa Sewer Design Guidelines,**  
City of Ottawa, SDG002, October 2012, amended 2019 & 2020. (City Standards)
  - Technical Bulletin ISTB-2018-01, City of Ottawa, March 21, 2018.
  - Technical Bulletin ISTB-2018-03, City of Ottawa, March 21, 2018.
- **Ottawa Design Guidelines – Water Distribution**  
City of Ottawa, July 2010., (Water Supply Guidelines)
  - Technical Bulletin ISD-2010-2, City of Ottawa, December 15, 2010.
  - Technical Bulletin ISDTB-2014-02, City of Ottawa, May 27, 2014.
  - Technical Bulletin ISDTB-2018-02, City of Ottawa, March 21, 2018.
- **Design Guidelines for Sewage Works,**  
Ministry of the Environment, 2008., (MOE Design Guidelines)
- **Stormwater Planning and Design Manual,**  
Ministry of the Environment, March 2003.,(SWMP Design Manual)
- **Ontario Building Code Compendium**  
Ministry of Municipal Affairs and Housing Building Development Branch, January 1, 2010 Update.(OBC)
- **Water Supply for Public Fire Protection**  
Fire Underwriters Survey, 1999., (FUS)
- **NFPA 13 – Standard for the Installation of Sprinkler Systems**  
National Fire Protection Association, 2016., (NFPA Standards)







### Installation Report • Rapport d'installation

Applicant: Simpson Legal Description: Lot: 15 Conc.: 3 S.Lot: \_\_\_\_\_ R.Plan: \_\_\_\_\_  
 File #: 07-005 Present on site: \_\_\_\_\_ Inspector: TJH  
 Date: Nov 5/07 Time: 3:05 pm Weather: overcast  
 Civic Address: 4 Campbell Road Court

**Scarification Inspection and/or Clay Seal**  
 Time: 2:15 pm Date: Aug 28, 2007 Approved:  yes  no  
 Weather: Sun, Warm On-site: \_\_\_\_\_ Inspector: G. Wilson  
 Length: 28.5m 29m Width: 15 m Elevations: Bed Area: 99.11 masl  
 Comments: \_\_\_\_\_ Mantle: 98.53 masl

**Section A Tank**  
 Septic tank/holding tank size: \_\_\_\_\_ (L) Filter make and model: \_\_\_\_\_  
 Make and model: EXISTING Inlet: \_\_\_\_\_ Outlet: \_\_\_\_\_  
 plastic  concrete  fiberglass  on-site Lids: \_\_\_\_\_ Baffles: \_\_\_\_\_  
 prefabricated

**Section B Treatment Unit**  
 Make: \_\_\_\_\_ Connections: \_\_\_\_\_  
 Model: \_\_\_\_\_ Electrical: \_\_\_\_\_

**Section C Leaching Bed**  
 Location: REAR YARD Distances: \_\_\_\_\_  
 Type: TRENCH Structure(s): \_\_\_\_\_ House: 78m  
 Height: OK Lot Lines: \_\_\_\_\_ Wells\*: 1) \_\_\_\_\_  
 Header:  level End Pipe:  level Watercourses: \_\_\_\_\_ 2) \_\_\_\_\_  
 Runs: 8 Length: 11m Tree(s): \_\_\_\_\_  
 Gravel Size: 3/4" Thickness: OK  
 Fall on Runs:  yes  no Between Trenches: 1.6m  
 Ends capped:  yes  no Interconnect  Mantles: 15 metres in 1 direction(s)  
 Geotextile:  yes  no Paper:  yes  no thickness: 0.25  
 Pipes: Diameter  3 in.  4 in.  chamber syst. Other: \_\_\_\_\_  
 Make: ROYAL Elevations: (if required) \_\_\_\_\_  
 Extended base: \_\_\_\_\_ Header: \_\_\_\_\_ Ends: \_\_\_\_\_  
 Paperwork for F.M.:  grain size and C.U.  Weigh Bills **Area Bed:** \_\_\_\_\_  
**Shallow Buried Trench:** Runs: \_\_\_\_\_ Length: \_\_\_\_\_ Stone: Length: \_\_\_\_\_ Width: \_\_\_\_\_  
 Pipe:  1"  1.5" Chamber: \_\_\_\_\_ Sand: Length: \_\_\_\_\_ Width: \_\_\_\_\_

**Section D Pump Chamber**  
 pump chamber  pump present  forced main:  check valve  
 floats installed  electrical wiring  frost protection installed  
 alarm:  inside  outside  other: \_\_\_\_\_  
 joints sealed properly

**Section E Distribution Box**  
 sealed joints  baffle or other  
 level  compacted base  
 frost protection number of outlets: \_\_\_\_\_  
 Diagram:

\*affected neighbouring wells

Picture(s) taken  Not approved, for re-inspection, call 692-0160 or 1-800-459-5975. Please ensure that ALL noted deficiencies have been rectified prior to calling for a re-inspection.

Approved in Full

Preliminary On-Site Approval (additional paperwork required, etc.)

Remarks: PUMP CHAMBER & DISTRIBUTION SYSTEM TO BE INSTALLED



R.V.C.A. RECEIVED

JAN - 3 2007



Ottawa Septic System Office Bureau des systèmes septiques d'Ottawa

## Application for a Permit to Construct or Demolish#

This form is authorized under the Building Code Sentence 2.4.1.1A.(2).

07-005

## For use by Principal Authority

REQUIRED FOR ALL INQUIRIES

Application number:	Permit number (if different):
Date received:	Rqll number:

Application submitted to: Rideau Valley Conservation Authority

(Name of municipality, upper-tier municipality, board of health or conservation authority)

INV# 00059-2007

<b>A. Project information</b>			
Building number, street name 4 Campbell Reid Court		Unit number	Lot/con. 15/3
Municipality Ottawa (Kanata)	Postal code K2K 1X7	Plan number/other description Part 1, Plan 5R-13420	
Project value est. \$		Area of work (m <sup>2</sup> )	
<b>B. Applicant</b> Applicant is: <input type="checkbox"/> Owner or <input checked="" type="checkbox"/> Authorized agent of owner			
Last name	First name	Corporation or partnership Paterson Group Inc.	
Street address 28 Concourse Gate		Unit number 1	Lot/con.
Municipality Ottawa (Nepean)	Postal code K2E 7T7	Province Ontario	E-mail avanschie@patersongroup.ca
Telephone number (613) 226-7381	Fax (613) 226-6344	Cell number ( )	
<b>C. Owner (if different from applicant)</b>			
Last name Simpson	First name Geoff	Corporation or partnership	
Street address 4 Campbell Reid Court		Unit number	Lot/con.
Municipality Ottawa (Kanata)	Postal code K2K 1X7	Province Ontario	E-mail
Telephone number (613) 592-0692	Fax ( )	Cell number (613) 853-3358	
<b>D. Builder (optional)</b>			
Last name	First name	Corporation or partnership (if applicable)	
Street address		Unit number	Lot/con.
Municipality	Postal code	Province	E-mail
Telephone number ( )	Fax ( )	Cell number ( )	
<b>E. Purpose of application</b>			
<input type="checkbox"/> New construction <input type="checkbox"/> Addition to an existing building <input checked="" type="checkbox"/> Alteration/repair <input type="checkbox"/> Demolition <input type="checkbox"/> Conditional Permit			
Proposed use of building		Current use of building Residential	
Description of proposed work			
Install a Class 4 Sewage System			
DIRECTIONS - MARCH ROAD TO DUNROBIN ROAD TO CAMPBELL REID COURT			
<b>F. Tarion Warranty Corporation (Ontario New Home Warranty Program)</b>			
i. Is proposed construction for a new home as defined in the Ontario New Home Warranties Plan Act? If no, go to section G.		<input type="checkbox"/> Yes	<input type="checkbox"/> No
ii. Is registration required under the Ontario New Home Warranties Plan Act?		<input type="checkbox"/> Yes	<input type="checkbox"/> No
iii. If yes to (ii) provide registration number(s): _____			

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**G. Attachments**

- i. Attach documents establishing compliance with applicable law as set out in Article 1.1.3.3.
- ii. Attach Schedule 1 for each individual who reviews and takes responsibility for design activities.
- iii. Attach Schedule 2 where application is to construct on-site, install or repair a sewage system.
- iv. Attach types and quantities of plans and specifications for the proposed construction or demolition that are prescribed by the by-law, resolution, or regulation of the municipality, upper-tier municipality, board of health or conservation authority to which this application is made.

07-005

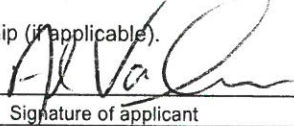
**H. Declaration of applicant**

I Albert Van Schie - Paterson Group Inc. certify that:  
(print name)

- 1. The information contained in this application, attached schedules, attached plans and specifications, and other attached documentation is true to the best of my knowledge.
- 2. I have authority to bind the corporation or partnership (if applicable).

January 2, 2007

Date



Signature of applicant

Personal information contained in this form and schedules is collected under the authority of subsection 8(1.1) of the *Building Code Act, 1992*, and will be used in the administration and enforcement of the *Building Code Act, 1992*. Questions about the collection of personal information may be addressed to: a) the Chief Building Official of the municipality or upper-tier municipality to which this application is being made, or, b) the inspector having the powers and duties of a chief building official in relation to sewage systems or plumbing for an upper-tier municipality, board of health or conservation authority to whom this application is made, or, c) Director, Building and Development Branch, Ministry of Municipal Affairs and Housing 777 Bay St., 2nd Floor. Toronto, M5G 2E5 (416) 585-6666.



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**Schedule 1: Designer Information**

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

<b>A. Project Information</b>			
Building number, street name 4 Campbell Reid Court		Unit no.	Lot/con. 15/3
Municipality Ottawa (Kanata)	Postal code K2K 1X7	Plan number/ other description Part 1, Plan 5R-13420	
<b>B. Individual who reviews and takes responsibility for design activities</b>			
Name Albert Van Schie		Firm Paterson Group Inc.	
Street address 28 Concourse Gate		Unit no. 1	Lot/con.
Municipality Ottawa, Nepean	Postal code K2E 7T7	Province Ontario	E-mail avanschie@patersongroup.ca
Telephone number (613) 226-7381	Fax number (613) 226-6344	Cell number ( )	
<b>C. Design activities undertaken by individual identified in Section B. [Building Code Table 2.20.2.1]</b>			
<input type="checkbox"/> House	<input type="checkbox"/> HVAC – House	<input type="checkbox"/> Building Structural	
<input type="checkbox"/> Small Buildings	<input type="checkbox"/> Building Services	<input type="checkbox"/> Plumbing – House	
<input type="checkbox"/> Large Buildings	<input type="checkbox"/> Detection, Lighting and Power	<input type="checkbox"/> Plumbing – All Buildings	
<input type="checkbox"/> Complex Buildings	<input type="checkbox"/> Fire Protection	<input checked="" type="checkbox"/> On-site Sewage Systems	
Description of designer's work Sewage System Design			
<b>D. Declaration of Designer</b>			
I <u>Albert Van Schie - Paterson Group Inc.</u> declare that (choose one as appropriate): (print name)			
<input checked="" type="checkbox"/> I review and take responsibility for the design work on behalf of a firm registered under subsection 2.17.4. of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>24387</u> Firm BCIN: <u>29346</u>			
<input type="checkbox"/> I review and take responsibility for the design work and am qualified in the appropriate category as an "other designer" under subsection 2.17.5. of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____			
<input type="checkbox"/> The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____			
I certify that:			
1. The information contained in this schedule is true to the best of my knowledge.			
2. I have authority to bind the corporation or partnership (if applicable).			
<u>January 2, 2007</u> Date	 Signature of Designer		

\*For the purposes of this form, "individual" means the "person" referred to in Clause 2.17.4.7.(1)(d), Article 2.17.5.1. and all other persons who are exempt from qualification under Subsections 2.17.4. and 2.17.5.

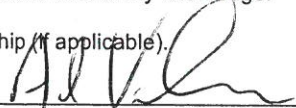
NOTE:

1. Firm and Individual BCIN numbers are not required for building permit applications submitted prior to January 1, 2006
2. Schedule 1 does not need to be completed by architects, or holders of a Certificate of Practice or a Temporary License under the *Architects Act*.

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 REFER TO:

O.S.S.O. PERMIT #  
 07-005  
 REQUIRED INQUIRIES

**Schedule 2: Sewage System Installer Information**

<b>A. Project Information</b>			
Building number, street name 4 Campbell Reid Court		Unit number	Lot/con. 15/3
Municipality Ottawa (Kanata)	Postal code K2K 1X7	Plan number/ other description Part 1, Plan 5R-13420	
<b>B. Sewage system installer</b>			
Is the installer of the sewage system engaged in the business of constructing on-site, installing, repairing, servicing, cleaning or emptying sewage systems, in accordance with Building Code Article 2.18.1.1?			
<input type="checkbox"/> Yes (Continue to Section C)		<input type="checkbox"/> No (Continue to Section E)	
<input checked="" type="checkbox"/> Installer unknown at time of application (Continue to Section E)			
<b>C. Registered installer information (where answer to B is "Yes")</b>			
Name		BCIN	
Street address		Unit number	Lot/con.
Municipality	Postal code	Province	E-mail
Telephone number ( )	Fax ( )	Cell number ( )	
<b>D. Qualified supervisor information (where answer to section B is "Yes")</b>			
Name of qualified supervisor(s)		Building Code Identification Number (BCIN)	
<b>E. Declaration of Applicant:</b>			
I, <u>Albert Van Schie - Paterson Group Inc.</u> declare that: (print name)			
<input checked="" type="checkbox"/> I am the applicant for the permit to construct the sewage system. If the installer is unknown at time of application, I shall submit a new Schedule 2 prior to construction when the installer is known;			
OR			
<input type="checkbox"/> I am the holder of the permit to construct the sewage system, and am submitting a new Schedule 2 now that the installer is known.			
I certify that:			
1. The information contained in this schedule is true to the best of my knowledge.			
2. I have authority to bind the corporation or partnership (if applicable).			
<u>January 2, 2007</u> Date		 Signature of applicant	



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 REFER TO: **Schedule 4**

**Proposed Services**

Do Not Complete  
 Permit No.                       
 Revision No.                       
 Date 07-005  
 O.S.S.O. PERMIT #  
 REQUIRED FOR ALL INQUIRIES

1. Engineered

- Yes
- No

2. Water supply

- Proposed
- Existing

3. Type of work proposed

- New Installation
- Replacement
- Alteration

4. Type of Well

- Dug/bored/Sandpoint well
- Drilled well
- Municipal
- Other

5. Residential Sewage Design Flow Info.

Bedrooms 4  
 House (floor area) 220 m<sup>2</sup>  
 People                       
 Total Fixture Units 15.5 (Schedule 8)  
 Residential Flow 2200 L/day

6. Sewage Design Flow for Other Occupancies

Design Flow                      L/day  
 Detailed sewage flow calculations:  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

7. Type of System

- Treatment Unit \_\_\_\_\_
- Class 2 – Leaching Pit
- Class 3 – Cesspool
- Class 4 – Shallow Buried Trench

---

- Class 4 – Trench
  - Fully raised
  - Partially raised
  - In-ground
- Class 4 – Filter Media
  - Fully raised
  - Partially raised
  - In-ground

- Class 4 – Area Bed
  - Fully raised
  - Partially raised
  - In-ground
- Class 4 – Aerobic with Trench
  - Fully raised
  - Partially raised
  - In-ground
- Class 4 – Aerobic with Filter Media
  - Fully raised
  - Partially raised
  - In-ground
- Class 5 – Holding Tank





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 Schedule 5  
 Sewage System Details

Do Not Complete  
 Permit No. \_\_\_\_\_  
 Revision No. 07-005  
 Date \_\_\_\_\_  
 U.S.S.O. PERMIT #  
 REQUIRED FOR ALL INQUIRIES

Type of System Class 4 - Trench (Schedule 4)

Septic/Holding Tank Existing L

Septic Tank Effluent Filter Yes

Treatment Unit - Make & Model \_\_\_\_\_

Number of Units \_\_\_\_\_

Refer to Typical Drawing PH0547-1&2

Mantle Information:

Native or imported = 15m in W direction(s)

Slope subgrade 2% (min) % slope

Westerly direction(s)

Site to be Scarified (If in clay) YES / NO

Clay Seal Required (If in bedrock) YES / NO

**Trench**

Distribution Pipe Length 88 m

Loading Area 423 m<sup>2</sup>

Type of Chamber \_\_\_\_\_

Length of Chamber \_\_\_\_\_ m

**Area Bed**

Stone \_\_\_\_\_ m<sup>2</sup>

Sand \_\_\_\_\_ m<sup>2</sup>

Pipe \_\_\_\_\_ m

**Shallow Buried Trench**

Pipe Length \_\_\_\_\_ m

**Filter Media Bed**

Stone \_\_\_\_\_ m<sup>2</sup>

Extended Base \_\_\_\_\_ m<sup>2</sup>

Pipe \_\_\_\_\_ m

Weight of Filter Media \_\_\_\_\_ Kg

Loading Area \_\_\_\_\_ m<sup>2</sup>

Note: Alarm required for all pumping systems

Construction Notes: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_



Ottawa Septic System Office Bureau des systèmes septiques d'Ottawa

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REFER TO:

Do Not Complete  
Permit No. 0550 PERMIT #  
Revision No. 07-005  
Date 07-005  
REQUIRED FOR ALL INQUIRIES

**Schedule 6**  
**Soil and Water Table Information**  
**(Minimum depth of test pit: 2 metres)**

Name of Applicant/Agent: <u>Paterson Group Inc.</u> Date: <u>November 24, 2006</u> Time: _____ Applicant/Agent Signature: _____ <u>AS</u>	Inspector: <u>JKD</u> Date: <u>JAN 10/07</u> Time: _____ Inspector Signature: _____
---	---

EG (.....)	Soil Description	T	EG (.....)	Soil Description	T
_____ _____ 1.0 m _____ _____ 1.5 m _____ _____ 2.0 m _____	REFER TO PATERSON GROUP DRAWING PH0547-2	_____ _____ _____ _____ _____ _____	_____ .5m _____ _____ 1.0 m _____ _____ 1.5 m _____ _____ 2.0 m _____	AS PER ENGINEER	_____ _____ _____ _____ _____ _____

EG (.....)	Soil Description	T	EG (.....)	Soil Description	T
_____ .5m _____ _____ 1.0 m _____ _____ 1.5 m _____ _____ 2.0 m _____	_____ _____ _____ _____ _____ _____	_____ _____ _____ _____ _____ _____	_____ .5m _____ _____ 1.0 m _____ _____ 1.5 m _____ _____ 2.0 m _____	_____ _____ _____ _____ _____ _____	_____ _____ _____ _____ _____ _____

**LEGEND**  
 BR = Bedrock                      HGWT = High ground water table                      EG = Existing grade  
 GWT = Ground water table                      M = metres                      T = percolation rate





Ottawa Septic System Office Bureau des systèmes septiques d'Ottawa

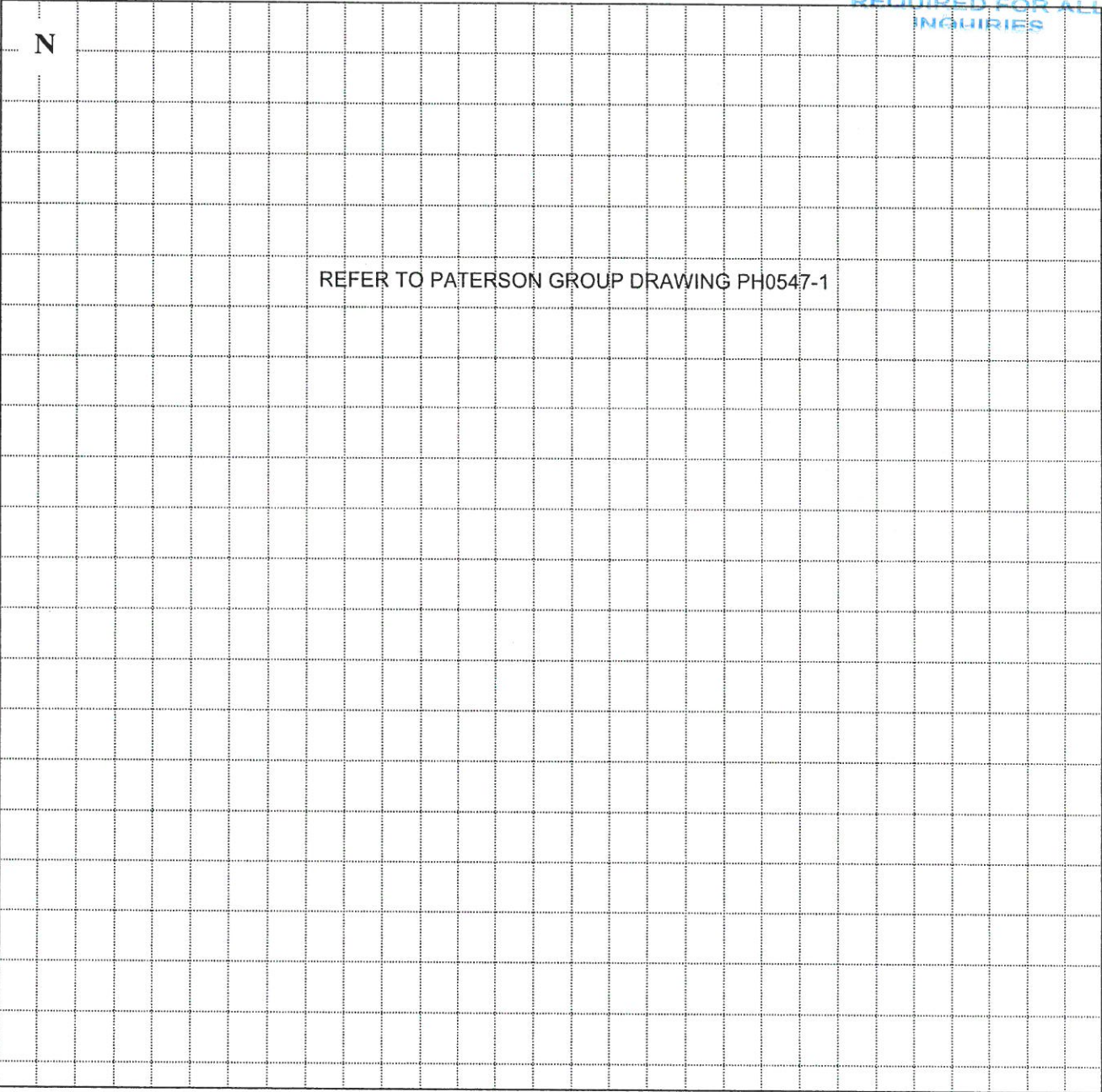
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Permit No. \_\_\_\_\_  
Revision No. 0550.PERMIT#  
Date 07-005

Scale: 1Block = \_\_\_\_\_

### Schedule 7 Layout Section

REQUIRED FOR ALL  
INQUIRIES



Dug Well  Drilled Well  Neighbouring Homes  Benchmark --- Tile Drainage  Property Line

Elevations (metric only)		Min. of 5 elevations in proposed system area (in X pattern)	
B.M. <u>100.00</u> m		X <sub>1</sub> _____	X <sub>2</sub> _____
B.M Description <u>Top of Deck @ Rear Left Corner of</u> <u>House</u>		X <sub>3</sub> _____	X <sub>4</sub> _____
Exact Location _____		X <sub>5</sub> _____	X <sub>6</sub> (toe) _____
		X <sub>7</sub> _____	X <sub>8</sub> _____



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**Schedule 8  
Fixture unit count**

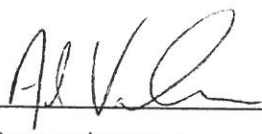
Do Not Complete  
Permit No O.S.S.O. PERMIT #  
Revision No \_\_\_\_\_  
Date 07-005  
REQUIRED FOR ALL INQUIRIES

Fixtures	# Existing + # Proposed X unit count = Fixture Count					
<b>Bathroom</b>						
Bathroom group (toilet, sink and tub or shower) with flush tank	2	+		X	6	= 12.0
Bathtub with/without overhead shower		+		X	1.5	=
Shower stall		+		X	1.5	=
Wash basin (1_ inch trap)		+		X	1.5	=
Watercloset (toilet) tank operated		+		X	4	=
Bidet		+		X	1	=
<b>Kitchen</b>						
Dishwasher	1	+		X	0.5	= 0.5
Sink with/without garbage grinder(s), domestic and other small type single, double or 2 single with a common trap	1	+		X	1.5	= 1.5
<b>Other</b>						
Domestic washing machine	1	+		X	1.5	= 1.5
Combination sink and laundry tray single or double (Installed on 1_ trap)		+		X	1.5	=

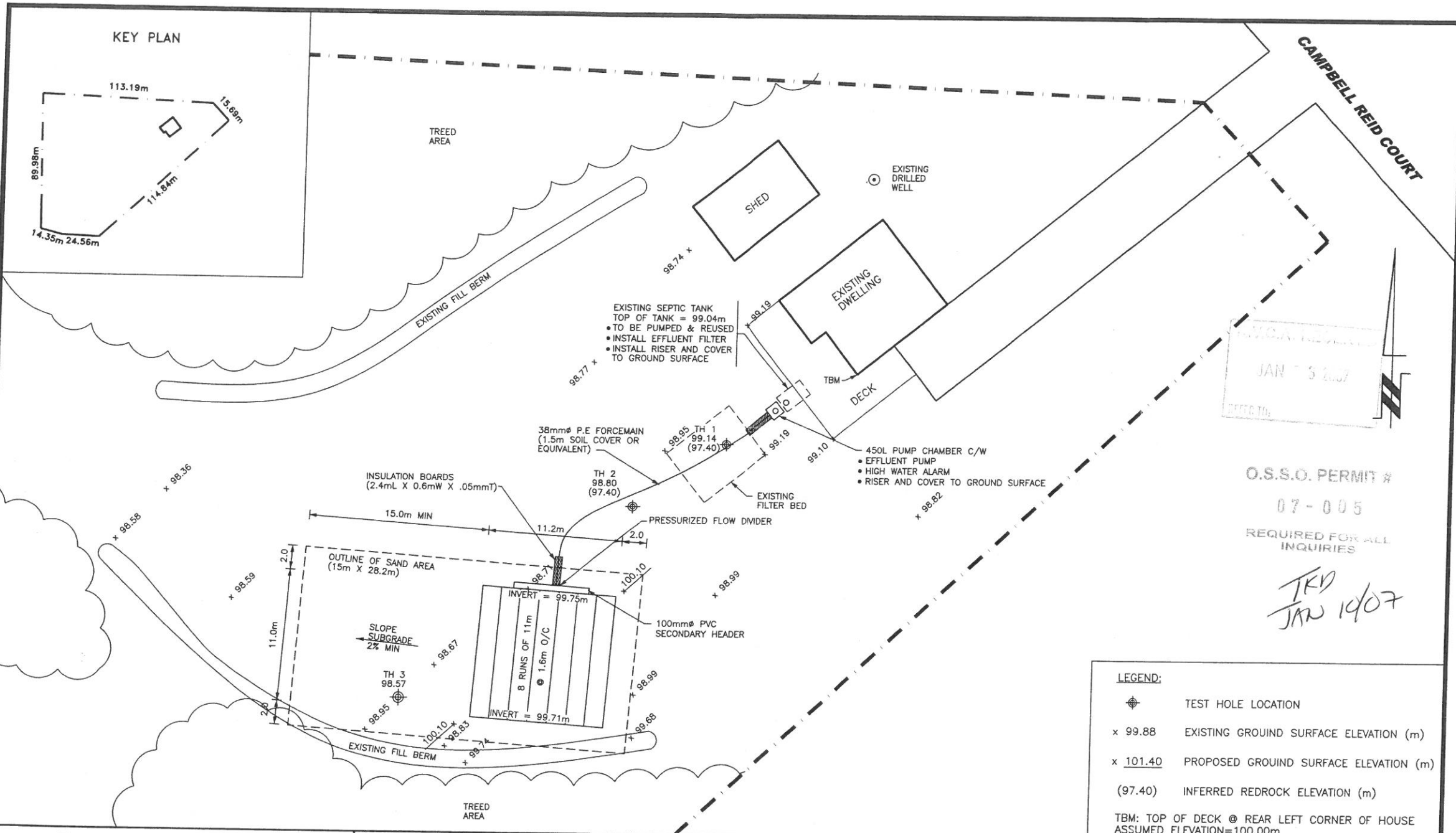
**Total: 15.5**

Insert the TOTAL in section 5 of Schedule 4 (0.Reb.403/97 Table 7.4.9.3)

1. Sump pumps and floor drains are not to be connected to the sewage system. Connection of such fixtures to a sewage system may lead to a hydraulic failure of the said system. The above mentioned fixtures should be discharged separately to an approved Class 2 (leaching pit) sewage system.
2. Where laundry waste is not more than 20% of the total daily design sanitary sewage flow, it may discharge to a sewage system (Part 8, OBC, 8.1.3.1(2)).

  
Agent/Owner signature

January 2, 2007  
Date



JAN 13 2007

O.S.S.O. PERMIT #  
07-005  
REQUIRED FOR ALL  
INQUIRIES

TRD  
JAN 14 07

**LEGEND:**

- ⊕ TEST HOLE LOCATION
- x 99.88 EXISTING GROUND SURFACE ELEVATION (m)
- x 101.40 PROPOSED GROUND SURFACE ELEVATION (m)
- (97.40) INFERRED REDROCK ELEVATION (m)

TBM: TOP OF DECK ⊙ REAR LEFT CORNER OF HOUSE  
ASSUMED ELEVATION=100.00m

**patersongroup**  
consulting engineers  
28 Concourse Gate, Unit 1, Ottawa, Ontario K2E 7T7

Scale: 1:300  
Des.: JB  
Dwn.: MH  
Chkd.: AVS

MR. GEOFF SIMPSON  
PROPOSED SEWAGE SYSTEM REPLACEMENT  
4 CAMPBELL REID COURT  
OTTAWA (KANATA), ONTARIO

SEWAGE SYSTEM  
LAYOUT PLAN

Dwg. No. PH0547-1  
Report No.: PH0547-1  
Date: 11/2006



**CLASS 4 ABSORPTION TRENCH STYLE BED**

**NOTES:**

1) ESTIMATE OF DAILY SEWAGE FLOW (Q)

- NO. OF BEDROOMS = 4
- LIVING AREA OF DWELLING = 220 m<sup>2</sup>
- FIXTURE COUNT = 15.5

DESIGN SEWAGE FLOW = 2200 L/DAY

2) SUBSURFACE CONDITIONS

SOILS INFORMATION GATHERED BY PATERSON GROUP INC.

TH 2, ELEV. 98.80m	TH 3, ELEV. 98.57m
0-0.10 TOPSOIL	0-0.10 TOPSOIL
0.10-1.36 SILTY CLAY	0.10-0.50 ROCK FILL INTERMIXED WITH SILT, SAND & CLAY
1.36 REFUSAL ON INFERRED BEDROCK	0.50 REFUSAL ON ROCK FILL
- EFFLUENT @ 1.00m	- DRY UPON COMPLETION

3) DISTRIBUTION PIPING NETWORK

- LENGTH OF PIPE REQUIRED =  $QT/200 = 2200(8)/200 = 88m$
- USE 8 RUNS OF 11m EACH RUN.
- TOTAL LENGTH OF PIPE PROVIDED = 88 LINEAR METRES

4) HYDRAULIC LOADING RATE

- TOTAL BED AREA = 423 m<sup>2</sup>
- HYDRAULIC LOADING RATE = 5.2 L/DAY/m<sup>2</sup>

5) SEPTIC TANK

- PUMP & RE-USE EXISTING SEPTIC TANK
- AN EFFLUENT FILTER SHALL BE INSTALLED ON THE OUTLET PIPE IN THE EXISTING SEPTIC TANK.
- INSTALL RISERS AND COVER TO PROVIDE ACCESS FROM GROUND SURFACE OVER FILTER.

6) PUMP CHAMBER

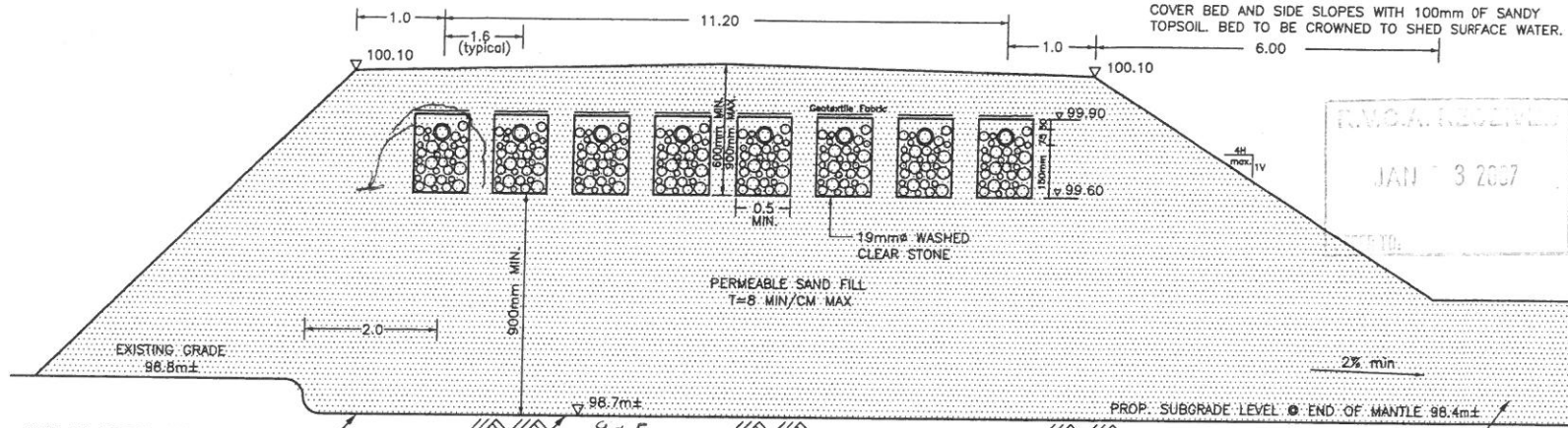
- INSTALL A 450L MINIMUM PUMP CHAMBER IN SERIES AND DOWNSTREAM FROM THE TREATMENT UNIT.
- AN EFFLUENT PUMP (MYERS ME40 OR EQUIVALENT) AND A HIGH WATER ALARM SHALL BE INSTALLED IN THE PUMP CHAMBER.
- THE RECOMMENDED PUMP DUTY CYCLE IS 290L.
- A 3mmØ DRAIN HOLE SHALL BE INSTALLED IN THE BOTTOM OF THE FORCEMAIN IN THE PUMP CHAMBER.
- RISERS WITH A COVER SHALL BE INSTALLED OVER THE CHAMBER TO PROVIDED ACCESS FROM THE GROUND SURFACE.

7) PRESSURIZED FLOW DIVIDER/FORCEMAIN

- A 38mmØ (NOMINAL) FORCEMAIN SHALL BE USED TO CARRY THE EFFLUENT FROM THE PUMP CHAMBER TO THE LEACHING BED.
- THE FORCEMAIN SHALL BE PROVIDED WITH 1.5m SOIL COVER OR EQUIVALENT.
- THE FORCEMAIN SHALL BE OVERLAIN WITH 50mm T x 600mm W INSULATION BOARDS AT THE PIPE CONNECTIONS.
- THE FORCEMAIN SHALL DISCHARGE INTO A PRESSURIZED FLOW DIVIDER (PREMIER TECH PSF-200) WHICH SHALL BE INSTALLED INTO A SECONDARY HEADER PIPE.
- THE FLOW DIVIDING TEE SHALL BE INSTALLED LEVEL ON A BED OF COMPACTED SAND.

8) LEACHING BED

- A FULLY-RAISED FILL BASED ABSORPTION TRENCH STYLE LEACHING BED TO BE INSTALLED.
- A MINIMUM THICKNESS OF 0.9m OF IMPORTED SAND FILL, HAVING A PERCOLATION RATE OF NOT GREATER 8 min./cm, SHALL BE PROVIDED BELOW THE BASE OF THE TRENCHES.
- THE SUBGRADE SURFACE FOR THE BED AND MANTLE AREAS SHOULD BE PREPARED BY REMOVAL OF ALL EXISTING TOPSOIL AND SUBEXCAVATING TO AT LEAST ELEV. 98.7m±.
- THE SPECIFIED CONTACT LEVEL, ELEV. 98.7m±, SHALL BE ESTABLISHED USING PERMEABLE SAND FILL, WHERE REQUIRED.



STRIP ALL TOPSOIL AND SUBEXCAVATE TO AT LEAST ELEV. 98.7m±, WITHIN THE LIMITS OF THE SAND AREA

ESTABLISH SPECIFIED CONTACT LEVEL (ELEV. 98.90m) USING PERMEABLE SAND FILL, WHERE REQUIRED

**PROFILE**  
NOT TO SCALE

- THE DISTRIBUTION PIPES SHOULD CONSIST OF 75mmØ PERFORATED SEPTIC PIPE WHICH SHOULD BE EMBEDDED IN A 300mm THICK BY 500mm (MIN.) WIDE LAYER OF 19mmØ WASHED CLEAR STONE.
- THE DISTRIBUTION PIPES SHOULD BE INSTALLED WITH A UNIFORM DOWNWARD SLOPE FROM ELEV. 99.75m AT THE HEADER TO ELEV. 99.71m AT THE FOOTER.
- THE ENDS OF EACH RUN SHALL BE INTERCONNECTED WITH A FOOTER PIPE.
- THE CLEAR STONE LAYER SHOULD BE COVERED WITH AN APPROVED GEOTEXTILE FABRIC.
- THE SURFACE OF THE BED SHOULD BE COVERED WITH PERMEABLE SAND FOLLOWED BY APPROXIMATELY 100mm OF SANDY TOPSOIL. THE BED AREA SHOULD BE VEGETATED.
- THE TOTAL THICKNESS OF THE COVER OVER THE BASE OF THE TRENCHES SHOULD BE WITHIN THE RANGE OF 0.6m TO 0.9m.
- THE SIDES OF THE BED SHOULD BE SLOPED IN THE RANGE OF 4H:1V OR SHALLOWER.

9) MINIMUM CLEARANCE FROM LEACHING BED

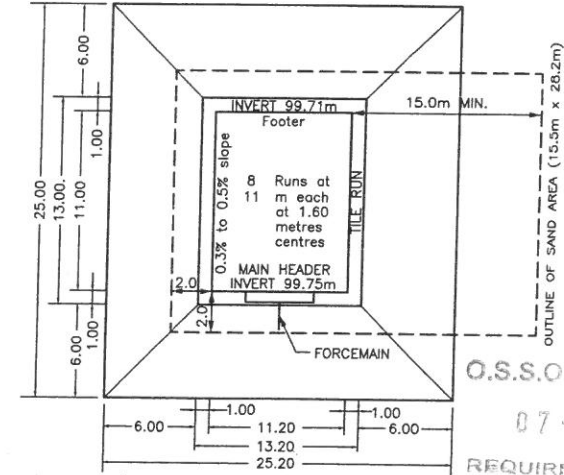
- 6.0m FROM ANY PROPERTY LINE.
- 8.0m FROM ANY STRUCTURE
- 18.0m FROM ANY DRILLED WELL.
- 5.0m FROM ANY TREES UNLESS OTHERWISE APPROVED.
- 5.0m FROM DRIVEWAY

10) MINIMUM CLEARANCE FROM TANK(S)

- 1.5m FROM ANY STRUCTURE
- 15.0m FROM ANY DRILLED WELL
- 3.0m FROM ANY PROPERTY LINE

11) OTHER PERTINENT DETAILS

- POOLS AND ALL APPURTENANCES, SUCH AS FENCES, PATIOS, ETC. SHOULD BE LOCATED A MINIMUM OF 5m FROM THE DISTRIBUTION SYSTEM OF LEACHING BED.
- THE BACKWASH WATERS FROM ANY TREATMENT UNIT, SUCH AS A WATER SOFTENER, SHOULD NOT DISCHARGE INTO THE SEPTIC TANK SYSTEM.
- THE CONTRACTOR IS RESPONSIBLE TO LOCATE AND PROTECT ALL EXISTING UNDERGROUND SERVICES.
- THE FIRM OF PATERSON GROUP INC. HAS PROVIDED DESIGN SERVICES ONLY FOR THE SUBJECT SEWAGE SYSTEM. THE DESIGN HAS BEEN CARRIED OUT IN ACCORDANCE WITH OUR INTERPRETATION OF PART 8 OF THE ONTARIO BUILDING CODE, 1997. INSPECTIONS BY THE CONSULTANT DURING THE INSTALLATION OF THE SYSTEM IS A REQUIREMENT OF SOME REGULATING AUTHORITIES AND IS STRONGLY RECOMMENDED BY THIS FIRM.



**PLAN VIEW**  
NOT TO SCALE

O.S.S.O. PERMIT #  
07-005  
REQUIRED FOR ALL INQUIRIES

TFO  
JAN 19/07

REVISED  
JAN 3 2007

**paterson group**  
consulting engineers  
28 Concourse Gate, Unit 1, Ottawa, Ontario K2E 7T7

Scale:	N.T.S.
Des.:	JB
Dwn:	MH
Chkd:	AVS

MR. GEOFF SIMPSON  
PROPOSED SEWAGE SYSTEM REPLACEMENT  
4 CAMPBELL REID COURT  
OTTAWA (KANATA), ONTARIO

SEWAGE SYSTEM  
DETAILS & NOTES

Dwg. No.	PH0547-2
Report No.:	PH0547-1
Date:	12/2006



R.V.C.A. RECEIVED  
NOV 02 2007  
REFER TO:

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Scale:  $\square = 1$  metre

SSO PERMIT #  
SEPTIC PERMIT NO.  
07-005

REQUIRED FOR ALL  
INQUIRIES

4 CAMPBELL RIED COURT

WELL  
11.2m

EXISTING  
HOUSE

Tank

Pump chamber

8 ROUNDS OF  
11 M EARTH AT  
160 M

15m  
11m  
11m

Slope  
SUBGRADE

