

Z.V. Holdings Corporation

Preliminary Geotechnical Investigation

1881/1883 Merivale and Adjacent Lot, Ottawa



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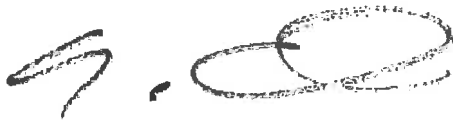
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Acronyms and Abbreviations

Arcadis	Arcadis Canada Inc.
AST	Aboveground Storage Tank
BVL	Bureau Veritas Laboratories
COPE	Construction, Occupancy, Protection, Exposure
CSA	Canadian Standards Association
ESA	Environmental Site Assessment
FIP	Fire Insurance Plan
HASP	Health and Safety Plan
LDPE	Low-density polyethylene
masl	metres above sea level
mald	metres above local datum
mbgs	metres below ground surface
MECP	Ontario Ministry of the Environment, Conservation and Parks
PCA	Potentially Contaminating Activity
PHC	Petroleum hydrocarbons
PVC	Polyvinyl chloride
QA/QC	Quality assurance/quality control
RDL	Reportable detection limit
RPD	Relative percent difference
SCS	Site Condition Standards
SLS	Serviceability Limit State
SPMDD	Standard Proctor Maximum Dry Density
TOC	Top of Casing
ULS	Ultimate Limit State
VOCs	Volatile Organic Compound

Executive Summary

Arcadis Canada Inc. (Arcadis) was retained by Z.V. Holdings Corporation to conduct a Preliminary Geotechnical Investigation in conjunction with Phase One and Two Environmental Site Assessment (ESA) updates of the properties at 1881 and 1883 Merivale Road and Adjacent Lot, Ottawa, ON (the Site). The Preliminary Geotechnical Assessment was required to develop preliminary designs and evaluate alternatives for the proposed new construction. The proposed new construction includes two raised, one-storey, office/warehouse space buildings; Building A of 3540m² and Building B of 3070m², across a total site area of 14,113m² under zoning designation IG – General Industrial zone.

It was understood that the objectives of the Preliminary Geotechnical Investigation were to determine the subsoil and groundwater conditions at the property by means of advancing boreholes and taking soil samples for geotechnical testing. The objective of the investigation was to then provide geotechnical recommendations for design of the proposed development, including construction considerations which may affect the design process.

Arcadis carried out the borehole drilling program on 15 and 16 September 2022, with a return visit to the site to measure groundwater levels on 11 October 2022. The investigation consisted of the advancement of boreholes at nine locations to a typical depth of 5m below existing ground surface (mbgs). Three of the boreholes were completed as piezometers to measure static groundwater levels. One borehole (BH22-10) was advanced using a dynamic cone to a maximum depth of 10.9 metres below surface (mbgs). Selected soil samples obtained were analyzed for grain size distribution and corrosivity parameters.

Generally, the subsurface profile encountered at borehole locations consists of a layer of sand fill at surface underlain by native fine sand with silt/silty sand. The surface fill layer along Jamie Avenue consisted of gravel fill while a medium to coarse sand (often characterized as topsoil when organics were noted) was encountered at other locations. No bedrock was encountered at any borehole locations.

SPT values in the overburden at a depth of 1.5 to 2.5mbgs ranged from 9 to 28 per 0.3m with an average SPT of 16 over the nine boreholes reviewed (BH22-2 to BGH22-10); indicative of a compact relative density. Borehole BH22-10 was advanced beyond 5m depth using a dynamic cone to an eventual depth of 10.9mbgs. SPT values from the cone were 15 at 5.6mbgs and increased with depth. No indications of voids or any very soft overburden materials were observed.

Soil samples submitted for analytical testing to assess the corrosion potential for exposed ferrous metals and the potential of sulphate attacks against subsurface concrete structures found no concerns related to potentially aggressive/ corrosive environment and GU cement was considered suitable for use.

Geotechnical recommendations were provided for slab on grade as well as strip or spread shallow footing construction.

It is also recommended that a professional engineering firm be retained during construction to perform a) observation of and verification of all bearing surfaces prior to the placement of foundation concrete; b) sampling and testing of the concrete and fill materials used; c) periodic observation of the condition of unsupported excavation side slopes in excess of 3m in height, if applicable; d) observation of all subgrades prior to backfilling; e) field density tests to determine the level of compaction achieved; f) sampling and testing of the bituminous concrete including mix design reviews.

1 Introduction

Arcadis Canada (Arcadis) was commissioned by the Z.V. Holdings Corporation to conduct a Preliminary Geotechnical Investigation for the properties located at 1881-1883 Merivale Road, in the City of Ottawa, Ontario (the Site, refer to Figure 1 attached to this report). It is understood that two raised, one-storey warehouse buildings are proposed for the Site. The objectives of the investigation were to provide preliminary geotechnical information to guide design and potential construction of the proposed development.

The scope of work for the preliminary geotechnical investigation included:

- Completion of a field investigation consisting of nine borehole advanced across the Site;
- Water level measurements taken from installed monitoring wells;
- Specified laboratory index testing;
- Geotechnical engineering analyses; and
- Preparation of a preliminary geotechnical engineering report.

The following report has been prepared specifically and solely for the aforementioned project which is described herein. It contains our preliminary findings and geotechnical recommendations pertaining to the design and construction of the subject development as they are understood at the time of writing this report. We note that the recommendations provided in this report are intended solely for the preliminary planning of this development. Geotechnical recommendations may change with proposed design changes. Further investigation will be required before detailed geotechnical parameters can be provided.

This report does not address environmental concerns associated with the Site. Please reference the Arcadis Phase One and Two ESA reports completed for these properties, as provided under separate cover.

1.1 Site Description

Address #1: 1881 and 1883 Merivale Road, Ottawa, Ontario

Location: The site is surrounded by mixed residential and commercial properties. It is located on the east side of Merivale Road and south side of Jamie Avenue in the City of Ottawa, Ontario. Refer to **Figure 1** following the text.

Latitude and Longitude: 45° 19' 57.8244" N, 75° 43' 19.7508" W (centroid)

Zoning: IG- General Industrial Use

Site Area: 14,113 m²

The subject site is currently occupied by an undeveloped grass field with a graveled school bus parking lot adjacent to the Jamie street property access. Grass covered areas and mature bushes/trees are found on the periphery of the property. The existing ground surface at the site is relatively flat with a gentle downslope towards the west.

1.2 Physical Setting

The subject property is situated in an urban area zoned for institutional land use. The subject area is relatively flat and generally level with surrounding grades having an approximate elevation of 91masl. Based on site observations and topographic maps, both 1881 and 1883 Merivale properties slope from east to west towards Merivale Road. Based on site observations and topographic maps both 1881 and 1883 Merivale slope from east to west towards Merivale Road. Generally, 1883 Merivale Road is at a higher elevation than 1881 Merivale Road, which is at a slightly higher elevation than 6 and 12 Jamie Avenue. Site drainage was not observed, no ditches were observed on either site. The site surface stormwater would be expected to flow from east to west towards Merivale Road. The regional topography is relatively flat. A depiction of the site is shown on **Figure 2**.

1.3 Local Geology and Hydrogeology

Bedrock geology mapping for the Site indicates that local bedrock is described as the Rockcliffe Formation comprising shale with lenses of sandstone (Geological Survey of Canada Map 1058A Generalized Bedrock Geology Ottawa, Ontario and Quebec).

The surficial geology mapping referenced describes surface soils as: Deltaic and Estuarine deposits, medium to fine grained sand, in some place fossiliferous; lying outside abandoned channels; most common deposit is a combined strip delta-sand plain that developed as fluvial water levels fell (Geological Survey of Canada Map 1056A Surficial Geology Ottawa, Ontario and Quebec).

Borehole logs and figures from surrounding properties were also referenced for surficial soil geology from records returned in the ERIS Ecolog search. According to the data contained in logs for wells advanced in close proximity to the property, the soils were generally characterized as sand and/or medium sand from surface to a depth ranging from 10.67 to 15.2 metres below ground surface (mbgs) before encountering bedrock.

Regional shallow groundwater may be directed westwards towards surficial water bodies in the Pinhey Forest but it is expected that deep groundwater flow may be in a northeasterly direction towards the Rideau River, based on a review of local topographical features and drainage patterns. Immediate area groundwater flow may be influenced by local features such as the presence of utilities and site facilities.

1.4 Proposed Development

The specific study area is located at 1881-1883 Merivale Road in Ottawa, Ontario. The proposed new construction includes two raised, one-storey buildings; Building A of 3540m² area and Building B of 3070m² area office/warehouse space, across a total site area of 14, 113m² under zoning designation IG – General Industrial zone.

1.5 Previous Reporting

Arcadis completed previous reporting for this subject property, which included the following reports: *Phase I and II Environmental Site Assessment, Arcadis – dated 17 December 2019*.

2 Scope of Work

The scope of work for the preliminary geotechnical investigation, conducted concurrently with the Phase One and Two ESA Updates for the Site, included the following:

- i. Development of a Site Specific Health and Safety Plan governing all site activities;
- ii. Performance of Utility OneCall clearances as well as borehole location clearances performed by a private utility locator;
- iii. Advancement of nine boreholes to depths ranging from 5.18 to 10.67mbgs (refer to Figure 2 for locations);
- iv. Installation of monitoring wells in three of the advanced boreholes;
- v. Selection of soil samples obtained for geotechnical analyses and submission to an accredited laboratory for testing;
- vi. Return to site and measurement of water levels, where possible;
- vii. Survey of all boreholes and monitoring wells to a temporary, local benchmark; and
- viii. Preparation of a preliminary geotechnical report summarizing the results of the onsite investigation and providing basic geotechnical recommendations to inform the proposed design and construction.

3 Method of Investigation

3.1 General

The field work for this investigation was carried out on 15 and 16 September (with a subsequent site visit on 11 October) under the supervision of Mr. Lennart DeGroot, B.Sc. of Arcadis. Nine boreholes were drilled on the subject property at the locations shown on **Figure 2**. Borehole locations were selected to correspond with the proposed building footprints as shown on the preliminary site layout drawings provided to Arcadis.

The Site area and all test locations were cleared for buried utilities prior to the start of the field investigation program. Ontario OneCall was contacted to determine the location of public utilities, and USL-1 was contracted to clear private utilities.

Standard field procedures are summarized in the following sections.

3.2 Borehole Drilling

Nine boreholes (BH22-2 through -10) were advanced on 15 and 16 September 2022 as shown on **Figure 2**. Borehole depths ranged from 5.18mbgs to 10.67mbgs. Descriptions of the soil stratigraphy encountered are presented on the borehole logs included as **Appendix A**.

The boreholes were advanced using a truck-mounted CME-55 auger drill rig (with hollow stem augers) operated by a two-person crew from Downing Estate Drilling. All fieldwork was conducted under the full-time supervision of Arcadis personnel, under the direction of a senior engineer. The proposed borehole at location BH22-1 was not able to be advanced due to safety concerns associated with heavy traffic at that location. Boreholes were backfilled with bentonite clay chips at the end of site operations, with the exception of three boreholes (BH22-2, -4, and -6) which were outfitted as monitoring wells for purposes of evaluating water table elevation measurement and groundwater sampling.

3.1.2 Soil Sampling and In-Situ Testing

Soil samples were collected from the boreholes using a 51mm diameter, 0.6m long split-spoon (SS) sampler on a continuous basis to 3mbgs, then at intervals of 1.6m thereafter. Standard penetration testing (SPT) was performed, and “N” values were recorded at the time of sample collection to assess soil density conditions.

All soil samples were visually inspected and initially classified on site. The split-spoon samples were placed in sealed plastic bags or jars and logged in the field for soil type, moisture content, colour, structure, and visual evidence of potential contamination, then transferred to the Arcadis laboratory for further evaluation. Borehole logs were prepared on the basis of sample and drilling process observations in the field describing the encountered strata and are presented in **Appendix A**. Site photographs are presented in **Appendix B**. Samples were selected and submitted to ALS in Ottawa, Ontario, for the selected geotechnical testing.

All samples will be stored at the laboratory for a period of one month after the issue date of this report. They will then be discarded unless we are otherwise directed. All excess soil cuttings were used as backfill when reinstating boreholes.

3.3 Groundwater Monitoring Well Installation and Groundwater Elevation Measurement

Three boreholes were finished as groundwater monitoring wells – BH22-2, -4, and -6 – to permit monitoring of the groundwater levels at Site. The monitoring wells comprised 50mm diameter Schedule 40 PVC Triloc riser pipes with a 3.05m long No. 10 slot intake zone (well screen). Silica sand was placed around the piping to a height of at least 300mm above the top of the well screen as filter pack. The remaining annular space was filled with a bentonite clay seal. A protective aluminum flushmount casing was then cemented in place at the top of the well.

In accordance with O.Reg. 903, well records were submitted to the MECP for the monitoring wells installed at the Site. The well tag and well record was submitted by the subcontracted licenced well drillers (Downing) who performed the installation.

A dedicated WaTerra inertial pump was installed in each monitoring well. The well was developed by hand-pumping the WaTerra sampler to ensure that at least three well volumes of water were removed (or until the well ran dry) to reduce the potential effects of foreign material introduced through drilling and to maximize the responsiveness of the surrounding geological materials.

Groundwater table monitoring was completed on 11 October 2022 at all wells within the same time period to ensure that the results are representative of conditions across the Site. Any unusual weather conditions and modifying features were noted on the log.

3.4 Field Survey

The test holes and monitoring wells installed on Site were located and surveyed in the field by Arcadis personnel during the initial fieldwork. Elevations were surveyed using a TopCon laser-level unit to a local datum, where the top of the concrete NE corner of the residential building used as the Diver's Wearhouse was assigned an elevation of 100.00m. The borehole location northing and easting coordinates were determined using a handheld GPS. The borehole locations are presented on **Figure 2** and the ground surface elevations are shown on each borehole log (**Appendix A**).

4 Geotechnical Laboratory Testing Program

Geotechnical laboratory testing was carried out on representative samples recovered from the boreholes, to effectively classify the soil strata observed in the field. This program included:

- Natural moisture content on all recovered samples where feasible;
- Grain size (sieve and hydrometer) testing on two samples;
- Coarse/fine material analyses on four samples; and
- Corrosivity suite testing on three samples.

The results of the testing program have been summarized in tabular format following the text of this report. Samples subjected to geotechnical testing have been identified on the borehole logs presented as **Appendix A**. Where applicable, the results of the index testing have been included on the borehole logs.

Additional soil and groundwater analytical results are reported in the corresponding Phase II ESA report prepared by Arcadis, presented under a separate cover. Environmental results and potential liabilities are not discussed in this report, unless where specifically noted.

The laboratory certificates of analyses are presented in **Appendix C**.

5 Subsurface Conditions

Generally, the subsurface profile encountered at borehole locations consists of a layer of gravel fill or topsoil underlain by native silty sand/sandy silt units. grading to light brown fine sand with depth. The surface fill layer along Jamie Avenue consisted of gravel fill while a medium to coarse sandy topsoil was encountered at other locations. No bedrock was encountered at any borehole location.

Reference should be made to the borehole logs in **Appendix A** for specific details of the soil profiles encountered at each borehole location.

5.1 Fill Soils

Fill soils were encountered at each borehole location onsite.

5.1.1 Gravel/Sand Fill

Gravel or sand fill (parking lot surfacing, at some locations) was encountered at borehole locations BH22-2, -3, -5 and -8 at surface and extended to depths ranging from 0.3 to 1.2mbgs. This unit was generally described as light grey and dry, with some to trace sand. The natural water content was measured at 0-8% - moisture content was not measured at each location if the sample was considered too dry. The SPT values obtained in this strata ranged from 5-18 blows per 300mm, indicating a loose to compact (medium dense) soil.

5.1.2 Topsoil

Topsoil was encountered at borehole locations that were not in areas used as parking space; BH22-4, -6, -7, -9, and -10. This unit was encountered at surface and extended to depths ranging from 0.3 to 0.6mbgs. This stratum was generally described as moist and brown to black organic material and medium to coarse sand, with or without organic lenses, trace roots and grass debris. The natural water content was measured at 10-16%. SPT values in this unit ranged from 2-4, indicating a loose to very loose soil.

5.2 Native Soils

5.2.1 Silty Sand/Silt with Sand

Silty sand/sand with silt was encountered at each borehole location, underlying the surficial fill units. This unit was general described as dry to moist, light to dark brown or grey fine sand with silt. Some lenses of coarser sand material were noted, as well as mottled texture in BH22-9. The natural water content was measured at 2-19%, with an average of 8%. The SPT values in this stratum ranged from 9-33, indicating a loose to dense soil.

5.2.2 Sand

Native sand was encountered in BH22-7 at a depth of 2.4mbgs and continuing to borehole termination at 5.18mbgs. This stratum was described as moist, light brown to light grey fine to coarse sand with some silt. The natural water

content was measured at 5%. SPT values in this unit ranged from 23-38, indicating a compact (medium dense) to dense soil.

5.3 Bedrock

The bedrock surface was not encountered in any of the borehole locations advanced. The cone penetration test performed at BH22-10 was terminated at a depth of 10.67mbgs.

5.4 Groundwater

Groundwater levels were measured at the monitoring wells in the borehole locations on 11 October 2022. The measured groundwater levels in monitoring wells are presented in **Table 2** following the text of this report – of the three wells, only one was observed to have collected liquid. Borehole logs indicated the presence of a wet soil at depths ranging from 2.5 to 4.5mbgs. Capillary action in the silty native soils may tend to wick moisture upwards. Based on these observations, the long-term shallow groundwater table is anticipated to vary seasonally between 3.5 to 5.5m depth below grade. It is anticipated that groundwater elevations may vary between the monitoring time and the time of construction.

The groundwater levels measured in 2019 are noted on **Figure 3**. These are not necessarily considered representative of current conditions but may be used as a reference when discussing construction methodology.

6 Geotechnical Discussion and Recommendations

It is understood that the site is intended to be developed as warehouse space, with two raised, single-storey warehouse buildings occupying the proposed footprints as shown on **Figure 2** at the rear of this report. Further geotechnical engineering analyses and/or investigation work may be required if the design changes beyond what has been proposed.

From a geotechnical perspective, the subject site condition is satisfactory for the construction of the proposed two buildings. It is recommended that the proposed building be founded on conventional shallow foundations placed over competent native silty sands. The geotechnical recommendations provided herein to assist preliminary foundation and building design are general in nature as limited details are available regarding the proposed structures. The recommendations should be reviewed by Arcadis prior to final design and construction to assess their applicability to the proposed structure. Further engineering, analyses and investigation work may be required once the final building parameters and configuration is known.

It is assumed that the proposed buildings would be no more than one raised storey in height. On the basis of the results of the field investigation program carried out during this study, the following geotechnical recommendations are provided.

6.1 Foundation Considerations

6.1.1 Shallow Foundations

The subsurface conditions encountered at the site are considered suitable for support for the proposed warehouse buildings on spread or strip footings, provided that they can be designed using the bearing resistance values provided below. Due to the presence of competent soils at shallow depths, deep foundations involving piles or caissons is not considered necessary or cost-effective. All existing fill, topsoil, organics, humus, reworked fill and any other deleterious material should be excavated and removed and the spread or strip footings founded on the underlying competent native silty sands/sands.

The maximum bearing resistance for spread or strip foundations up to 1.5m in width founded on the undisturbed native silty sands may be designed using a net allowable serviceability limit states (SLS) bearing capacity of 100kPa and a factored bearing resistance value at ultimate limit states (ULS) of 200 kPa. The maximum bearing resistance for pad footings up to 4m in width may be taken as the same.

Exterior wall support structures placed on undisturbed compact silty sand can be designed using a bearing resistance value SLS of 100kPa and a factored bearing resistance at ULS of 200kPa. All founding surfaces must be proof rolled by adequately sized compaction equipment making several passes under dry conditions and above freezing temperatures.

A geotechnical resistance factor of 0.5 was applied to the reported bearing resistance values at ULS.

In the proposed warehouse building footprint areas, the surface of undisturbed native silty fine sand stratum is located 0.6 to 1.8 m below grade. Total and differential settlements of properly designed and installed foundations are estimated to not exceed 25mm.

Proof-rolling and geotechnical inspection is required to ensure that founding surfaces are of acceptable undisturbed, native soils prior to placing crushed stone, engineered fill or concrete.

6.1.2 Slabs-on-Grade

It is anticipated that slab-on-grade floor construction may be required for the main floor of the buildings. The surficial topsoil/humus layer is considered unsuitable for support of building floor slabs due to its compressible nature and should be excavated and removed. Any underlying reworked overburden is considered adequate for support of building floor slabs. The underlying native silty sands, if exposed during regrading of the site, are also considered suitable for slab-on-grade floor support. Exposed surfaces of the reworked or undisturbed sandy soil should be proof rolled to identify soft spots, which should be repaired through excavation and backfilled with OPSS Granular B fill material compacted to not less than 95% SPMDD.

Any building floor slabs should typically be constructed to be independent of building foundation walls, or any other part of the structure founded on different soils/foundations to minimize differential settlement.

A minimum 150 mm-thick layer of compacted, free-draining granular or crushed stone material should be placed between the subgrade and the building floor slab to provide proper sub-slab drainage, moisture migration and support. If reworked overburden or native fill options are used, given the variable subgrade soil gradation potentially present it is recommended that a non-woven geotextile layer (Terrafix 270R or equivalent) be placed to separate crushed stone from the subgrade.

Proof-rolling and geotechnical inspection is required to ensure that founding surfaces are of acceptable undisturbed, native soils prior to placing crushed stone, engineered fill or concrete.

6.1.3 Frost Protection

All exterior foundations should be provided with a minimum of 1.5m soil cover, or equivalent, to provide frost protection. Frost protection should also be provided for any slabs exposed to the elements. The silty or fine sand stratum is considered to be frost susceptible. Due to its freezing potential, the silty or fine sand material is not recommended as backfill to exterior building walls.

Trench excavations and pavement construction are difficult activities to complete during freezing conditions without introducing frost in the sub-grade or in the excavation walls and bottoms. Precautions should be taken if such activities are to be carried out during freezing conditions.

6.2 Site Grading and Preparation

6.2.1 Recommendations for Soil Removal

Asphalt, topsoil, and deleterious fill, such as material containing high content of organic materials or construction remnants, should be stripped entirely from under the proposed building footprint and other settlement sensitive structures (e.g. pavement structures). All overburden at the subject property within the proposed building footprints are expected to be removed. Further geotechnical analyses on the stratum will be required if the fill is to be considered for construction use (founding surface, backfill, etc.) on site.

6.2.2 Engineered and Native Fill

Fill used for grading beneath the proposed building should consist of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. This material should be tested and approved prior to delivery to the site. The fill should be placed in lifts no greater than 300mm thick and compacted using suitable compaction equipment for the lift thickness. Load-bearing fill soils placed beneath the building footings and paved areas should be compacted to at least 98% of the material's standard Proctor maximum dry density (SPMDD).

Topsoil and humus excavated throughout the site may be stockpiled for future use on site. Reworked native soils excavated during the course of foundation installation are considered to be frost susceptible and as such are not recommended for use as load-bearing material. The material may be reused on site as general upfill as required for berms or landscaping.

Non-specified existing fill, along with site-excavated soil, can be used as general landscaping fill where settlement of the ground surface is of minor concern. This material should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids. If this material is to be used to build up the subgrade level for areas to be paved, it should be compacted in thin lifts to at least 98% of the material's SPMDD (this will require Proctor testing).

Non-specified existing fill and site excavated soils are not suitable for use as backfill against foundation walls unless used in conjunction with a composite drainage membrane. Further geotechnical testing and analyses on this material to confirm consistency and suitability is required before it is approved for construction use such as this.

6.2.1 Excess Soils

The removal of any excess soil from the site should follow the requirements of O.Reg. 406/19- ***On-Site and Excess Soil Management***.

6.3 Seismic Considerations

6.3.1 Seismic Hazard

The Ottawa area falls within the Western Quebec Seismic Zone (WQSZ), according to the Geological Survey of Canada. Based on a review of Ontario Geological Survey maps (map 431A), the project Site is not underlain by any known faults. Under the 2015 Ontario Building Code, a seismic hazard with a 2% probability of exceedance in 50 years has been retained for design of the building structure. The design earthquake magnitude retained for this event is 6.1, and represents the mean magnitude of the de-aggregation of the PGA seismic hazard for Ottawa.

6.3.2 Liquefaction Assessment

Liquefaction is a seismically induced phenomenon that can cause soil densification and excess pore pressures which then can lead to potentially large surface settlements and sudden temporary losses in bearing strength. These then can cause lateral spreading and catastrophic soil failures (or flow slides) which are often observed

alongside rivers or shorelines. The shallow overburden present at the subject site is not considered to be potentially liquifiable.

6.3.3 Seismic Classification

At this preliminary stage, the site class for seismic site response can be taken as **Class D** (stiff soil) for the foundations bearing on soil profile materials with an average N_{60} between 15 to 50.

Seismic classifications should be verified during the subsequent detailed geotechnical investigation(s) using field MASW/ESPAC and seismic refraction methods.

6.4 Pavement Recommendations

Founding soils for pavements structure must be proof-rolled and inspected by qualified personnel prior to pavement structure construction. Where required at the subject site, the recommended pavement structures for parking areas and access lanes are shown below:

Table 6-1: Recommended Pavement Structure-Car Only Parking Areas

Thickness (mm)	Material Description
50	Wear Course: HL-3 or Superpave 12.5 Asphaltic Concrete
150	Base: OPSS Granular A Crushed Stone base
300	Subbase: OPSS Granular B Type II
Subgrade: Either fill, competent in-situ soil or OPSS Granular B Type I or II material placed over competent in-situ soil or fil.	

Table 6-2: Recommended Pavement Structure- Access Lanes and Heavy Truck Parking/Loading Areas

Thickness (mm)	Material Description
40	Wear Course: HL-3 or Superpave 12.5 Asphaltic Concrete
50	Binder Course: HL-8 or Superpave 19.0 Asphaltic Concrete
150	Base: OPSS Granular A Crushed Stone base
450	Subbase: OPSS Granular B Type II
Subgrade: Either fill, in-situ soil or OPSS Granular B Type I or II material placed over in-situ soil or fil.	

Minimum Performance Graded (PG) 58-34 asphalt cement is recommended for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with compacted OPSS Granular B Type II material. Weak subgrade conditions may be

experienced over service trench fill materials. This may require the use of a geotextile, such as Terrafix 270R or equivalent, thicker subbase or other measures that can be recommended at the time of construction as part of the field observation program.

The pavement granular base and subbase should be placed in maximum 300mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment.

7 Design and Construction Precautions

7.1 Temporary Excavations

Temporary excavations are expected to be shallow and must conform to the stipulations made in O.Reg. 213/91 promulgated under the Occupational Health and Safety Act. Most soils that will be encountered in temporary excavations are anticipated to be Type 3, as defined under the Regulation. Therefore, open cut side slopes would need to be cut back at an inclination of no steeper than 1 horizontal to 1 vertical (1H:1V). For slopes which are unsupported in the longer term, and might experience free-thaw cycles, flatter side slope inclinations could be required. It is not anticipated that excavations would extend below the groundwater table, but any soils below such would be considered Type 4 and require 3H:1V slopes

7.2 Foundation Drainage and Backfill

Based on the amount of silt present in native soils on site, it is recommended that a perimeter foundation drainage system be provided for the proposed structure. The system should consist of a 100 mm diameter perforated corrugated plastic pipe, surrounded on all sides by 100mm of 19mm clear crushed stone which is placed at the footing level around the exterior perimeter of the structure. The perimeter drainage pipe system should direct water to a suitable outlet.

7.3 Pipe Bedding and Backfill

Bedding and backfill materials should be in accordance with the most recent Material Specifications & Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Services Branch of the City of Ottawa.

A minimum of 150mm of OPSS Granular A should be placed for bedding for sewer or water pipes when placed on soil subgrade. If the bedding is placed on bedrock, the thickness of the bedding should be increased to 300 mm for sewer pipes. The bedding should extend to the spring line of the pipe. The material should be placed in a maximum 300mm thick loose lifts and compacted to a minimum of 95% of its SPMDD.

The cover material, which should consist of OPSS Granular A, should extend from the springline of the pipe to at least 300mm above the obvert of the pipe. The material should be placed in a maximum 300 mm thick loose lifts and compacted to a minimum of 95% of its SPMDD.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (to about 1.8 m below finished grade) should match the soils exposed at the trench walls to minimize differential frost heaving. The trench backfill should be placed in a maximum 300 mm thick loose lifts and compacted to a minimum of 95% of the material's SPMDD.

If required, frost depth protection can be provided to duct banks or similar using an overlay of Styrofoam SM insulation. Insulation overlay design and backfill parameters can be provided by Arcadis once embedment depths have been confirmed.

7.4 Groundwater Control

6.4.1 Groundwater Control for Building Construction

Based on our observations, it is anticipated that groundwater infiltration into the excavations should be negligible given use of shallow spread or strip footings under summer conditions. Pumping from open sumps should be sufficient to control the groundwater influx through the sides of shallow excavations under summer conditions.

The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium. Discharged water should be subject to filtering before discharge. A municipal permit will be required if impounded water is pumped into the sewer. Any sewer discharges should be conducted to meet City of Ottawa sewer discharge bylaw standards.

The finished exterior surface grades of the proposed structure should be sloped away from the building to prevent surface ponding and infiltration immediately adjacent to the building exterior walls. Backfill adjacent to all exterior walls should comprise compacted, free-draining granular materials (OPSS Granular B or equivalent).

6.4.2 Permit to Take Water/ EASR

It is unlikely that a Ministry of the Environment, Conservation and Parks (MECP) permit to take water (PTTW) is required for this site (typically required if more than 400,000 L/day of ground and/or surface water is to be pumped during the construction phase). For typical ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for the completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O. Reg. 63/16.

Neither a PTTW or an EASR is expected to be required for this site given the shallow nature of the proposed foundation footings and the inferred depth of water table across the site.

7.5 Winter Construction

The subsoil fill conditions at this site mostly consist of frost susceptible materials. In presence of water and freezing conditions, ice could form within the soil mass. Heaving upon freezing and settlement upon thawing could occur. Precautions should be taken if winter construction is considered for this project.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the use of straw, propane heaters, tarpaulins or other suitable means. In this regard, the base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Any trench excavations should be carried out in a manner that will avoid the introduction of frozen materials into the trenches. As well, pavement construction is difficult during winter. The subgrade consists of frost susceptible soil which will experience total and differential frost heaving as the work takes place. In addition, the introduction of frost, snow or ice into the pavement materials, which is difficult to avoid, could adversely affect the performance

of the pavement structure. Additional information and recommendations can be provided during the design and construction project phases if requested.

7.6 Corrosion Potential

Three soil samples were submitted for corrosivity testing, from BH22-3, -5, and -6. The results of analytical corrosivity testing on soil are summarized in Table 4 following the text of this report.

The laboratory results on soil indicate that the sulphate content was non-detect in samples submitted, indicating a non-corrosive environment. As the threshold for chloride content requiring amended concrete is 0.2%, while the maximum concentration observed was at 41µg/g (or 0.0041%), which is acceptable. The neutral pH levels (from 7.5 to 6.6) of the three samples analyzed indicate that this is not a contributing factor in creating a corrosive environment for exposed ferrous metals at this site.

Based on the National Corrugated Steel Pipe Association, a low soil resistivity relates to increased potential corrosion activity and is governed by the content of electrolytes, which consist of moisture, minerals and dissolved salts which can vary throughout the seasons. Typically, the lower the resistivity, the higher will be the soil corrosivity. Corrosive soil environments occur with a resistivity between 30 and 50 Ohm-m while even lower values are highly corrosive. Based on the soil samples tested with a resulting minimum resistivity of 4950 Ohm-cm (or 49.5 Ohm-m), the corrosion rating for the subject property soil classifies it essentially a non-corrosive environment.

Uncontaminated, high quality concrete normally provides excellent corrosion protection for reinforcing steel. The high pH environment of the concrete (greater than 12.5) keeps the reinforcing steel in a non-active corrosion state. Intrusion of chlorides into the concrete through contact with chloride-contaminated soil, water or marine atmosphere, however, may lead to corrosion of the embedded reinforcing steel. Sulphate attack can cause extensive cracking, expansion and loss of bonds between the cement paste and aggregates. Type GU Portland cement should therefore be suitable for use in concrete at this site based on the results as reported by the laboratory.

8 Future Recommendations

8.1 Detailed Geotechnical Investigation

The geotechnical recommendations provided herein are preliminary in nature. It is recommended that a detailed geotechnical investigation be performed once the new building design has been finalized and once proposed footing alignments and depths are known. Recommendations for future geotechnical work on site and in the laboratory to support the building design process should include, but are not limited to:

- Further geotechnical testing (additional grain size analyses, Proctor testing) of any onsite fill/ soil materials proposed for reuse ;
- A soil management plan be developed with the aim of handling soils suitable as native fill, as well as excess soils generated during the construction process, in an efficient and cost-effective manner; and
- Confirmatory boreholes to confirm soil types, distributions and depths to bedrock at defined intervals across the proposed building footprint and especially at proposed corners, anticipated footing locations, etc.

8.2 Geotechnical Consultation During Design Process

The preliminary geotechnical recommendations provided herein to assist foundation and building design are general in nature as specifics of the structure have yet to be determined. These recommendations should be reviewed by Arcadis prior to final design and construction to assess their applicability to the proposed structure. Site-specific foundation design recommendations will be required for components of the proposed structure.

8.3 Geotechnical Supervision During Construction

Development of the subject site will require movement of a variety of soil types and potentially specialized foundation installations. It is recommended that a qualified geotechnical engineer be retained to inspect and approve the subgrade prior to placement of utility lines, watermain thrust blocks, pavement structures, building floor slab/foundations or to supervise the installation of foundations. Geotechnical supervision should also be provided to ensure that engineered fill placed beneath floor slabs, roadways and parking areas is properly compacted and that any weak soil layers are properly removed. Geotechnical inspection of the bearing conditions for the proposed foundation system should also be carried out.

Geotechnical site supervision and review is required during future construction activities. It is recommended that the following material testing and observation program be performed by a licensed geotechnical engineering consultant during construction operations:

- Observation of all bearing surfaces prior to the placement of concrete/crushed stone/engineered fill;
- Sampling and testing of the concrete and fill materials used;
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3m in height, if applicable;
- Observation of all subgrades prior to backfilling;

- Field density tests to determine the level of compaction achieved; and
- Sampling and testing of the bituminous concrete including mix design reviews.

A report confirming that these construction works have been conducted in general accordance with geotechnical recommendations would then be issued following the completion of a satisfactory material testing and observation program by the geotechnical consultant. It is recommended that all footing excavations be inspected by competent geotechnical personnel to ensure that a proper bearing surface has been attained and that foundation designs are suited to site conditions.

8.4 Existing Wells

The groundwater wells installed at this site will require decommissioning at the time of construction in accordance with O.Reg. 461/19 protocols.

9 Closure

The field work and reporting for this investigation was carried out by Mr. Lennart de Groot, B.Sc. and Mr. Justin Cameron, B.Sc., working under the direction and final review of Mr. Troy Austrins, P.Eng., PMP and Mr. Ryan Janzen, P. Eng.

We trust that the contents of this report are sufficient for your present purposes. If you have any questions, please call.

Respectfully submitted,

Arcadis Canada Inc.



Ryan Janzen, P.Eng.
Project Engineer



Troy Austrins, P.Eng., PMP
Team Lead

10 Statement of Limitations

This report, prepared for the Z.V. Holdings Corporation, does not provide certification or warranty, expressed or implied, that the investigation conducted by Arcadis uncovered all potential geotechnical constraints at the site. The conclusions and recommendations presented in this geotechnical investigation report are based on the information determined at the borehole locations. The information contained within this report in no way reflects the environmental aspect of the site or soil, unless specifically reported upon. Subsurface and groundwater conditions between and beyond the test locations may differ from those encountered at the specific locations tested, and conditions may be encountered during construction which were not detected and could not be anticipated at the time of the site investigation. It is recommended that Arcadis be retained during construction to confirm that the subsurface conditions throughout the subject property do not differ materially from those conditions encountered at the test locations. The benchmark and ground surface elevations in this report were used to establish relative elevation differences between the test locations and should not be used for other purposes, such as grading, excavating, planning, development, etc.

The design recommendations provided in this report are applicable only to the project described in the text and then only if constructed substantially in accordance with the details stated in this report. Since all details of the design may not have been available at the time this report was prepared, it is recommended that a qualified engineering consultant be retained during future stages of the design process to verify that the design is consistent with the recommendations of this report, and that the assumptions made in the analyses contained in this report are still valid. The need for additional subsurface investigation work and laboratory testing should be reviewed by the retained qualified engineering consultant during the course of the detailed design work.

The comments given in this report on potential construction problems and possible methods of construction are intended only for the guidance of the designer. The number of boreholes/ groundwater wells may not be sufficient to determine all of the factors that may affect construction methods and costs (e.g., the thickness of surficial topsoil and fill layers can vary markedly and unpredictably). Contractors bidding on the project or undertaking the construction should, therefore, make their own interpretations of the factual information in this report and draw their own conclusions as to how the subsurface conditions may affect their bid or work.

Furthermore, this report was prepared by Arcadis for Z.V. Holdings Corporation. The material in it reflects the best judgement of Arcadis based on the information available at the time of preparation, Sept./Oct. 2022. Changes to soil and/or groundwater quality in the areas investigated can occur following the date of testing. Any use which a third party makes of the report, or reliance on, or decisions to be based on it, is the responsibility of such third parties. Arcadis accepts no liability, whether in negligence, contract or arising on any other basis for damages or from indemnification arising from decisions or actions by others based on this report. Please note that the recommendations provided in this report are intended solely for the preliminary planning of this development. Further geotechnical investigation will be required before detailed geotechnical parameters can be established.

Tables

Table 1
Elevations Summary

Borehole Number	Co-ordinates		Ground Surface Elevation (local)	Borehole Depth (mbgs)	Borehole Base Elevation (local)	Depth to Well Screen (mbgs)	Well Screen Elevation (local)	Depth to bedrock (mbgs)	Bedrock Elevation (local)
	N	E							
BH22-2	45°20.011'	075°43.315'	98.98	5.18	93.80	2.13	96.85	--	--
BH22-3	45°19.988'	075°43.303'	98.96	5.18	93.78	--	--	--	--
BH22-4	45°19.976'	075°43.332'	98.97	5.18	93.79	2.13	96.84	--	--
BH22-5	45°19.996'	075°43.338'	98.81	5.18	93.63	--	--	--	--
BH22-6	45°19.968'	075°43.324'	99.04	5.61	93.43	2.56	96.48	--	--
BH22-7	45°19.947'	075°43.306'	99.86	5.18	94.68	--	--	--	--
BH22-8	45°19.934'	075°43.335'	99.48	5.18	94.30	--	--	--	--
BH22-9	45°19.957'	075°43.351'	99.15	5.18	93.97	--	--	--	--
BH22-10	45°19.952'	075°43.306'	99.86	10.67	89.19	--	--	10.67	89.19

- Notes:
- All screen intervals are 3.05m. Elevation given is the top of the screen.
 - Bedrock was not proved during this investigation, depth given is the inferred bedrock surface.
 - A local datum was used, with the catchbasin present at the northwestern corner given as 100.00m elevation.

Table 2
Groundwater Levels

Borehole / MW Number	Ground Surface Elevation (m)	Depth to Water (m) 2022.10.11	Water Elevation (m) 2022.10.11
BH22-2	98.98	4.85	94.13
BH22-4	98.97	dry	--
BH22-6	98.04	dry	--

Notes: - Water levels were measured using an oil-water interface probe.

Table 3
Grain Size Analyses Results

Borehole Number	Sample	Gravel	Sand	Silt	Clay	Classification
BH22-3	2	0%	67%	30%	3%	Silty SAND, fine grained.
BH22-4	2	0%	69%	27%	4%	Silty SAND, fine grained.
BH22-5	2	0%	47%	46%	6%	SAND-SILT, trace clay.
BH22-6	2	0%	78%	18%	4%	SAND, fine-grained, some silt.
BH22-7	5	0%	84%	15%	1%	SAND, fine-grained, some silt.
BH22-10	2	0%	38%	56%	6%	SAND-SILT, trace clay.

Notes:

- Grain size analyses were performed by ALS.
- Laboratory certificates are provided in the report appendices.

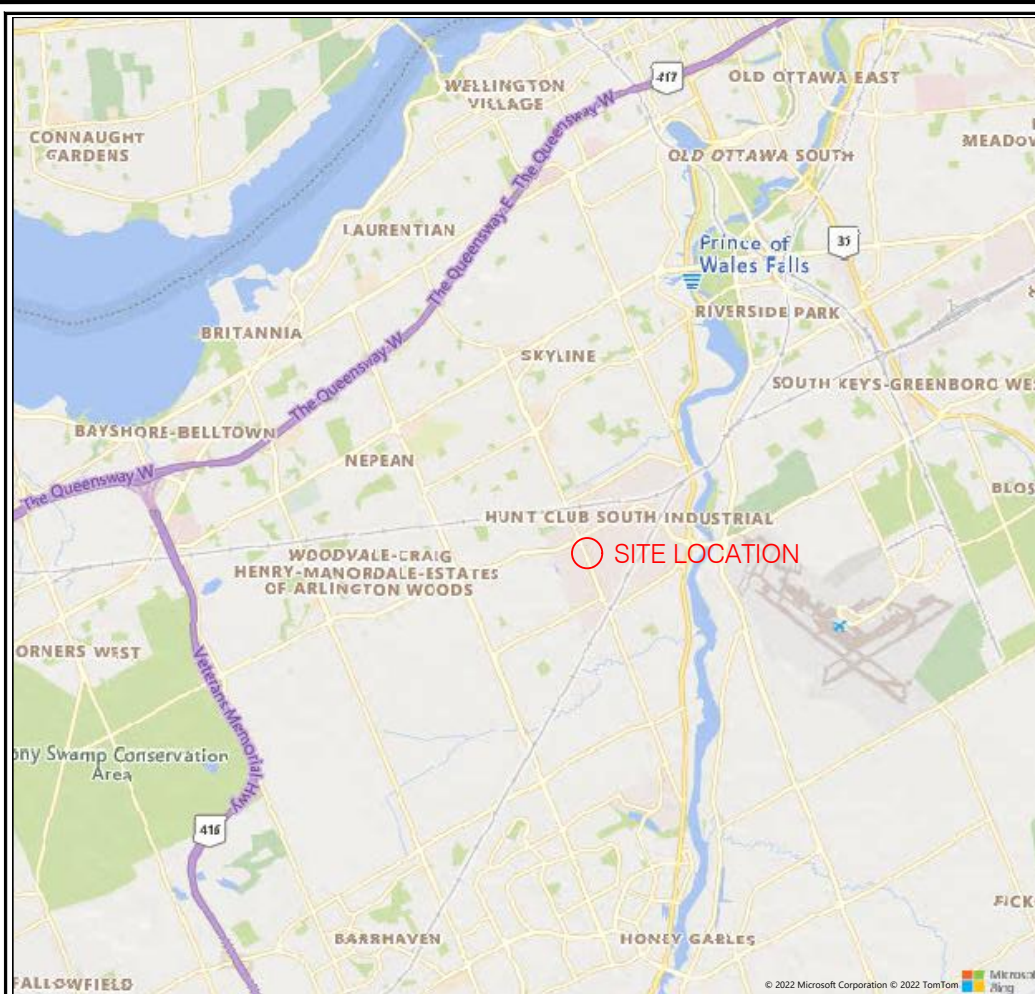
Table 4
Results of Corrosivity Suite Analyses



Borehole Number	Sample	Depth (mbgs)	Sulphide (µg/g)	Chloride (20:1) (µg/g)	Sulphate (20:1) (µg/g)	pH (pH units)	Electrical Conductivity (µS/cm)	Resistivity (ohm.cm)	Redox Potential (mV)
BH22-3	2	1.80	<20	30	<20	7.54	202	4950	486
BH22-5	3	1.80	<20	41	<20	7.52	194	5150	457
BH22-6	3	1.80	<20	<5	<20	6.63	40.2	24900	409

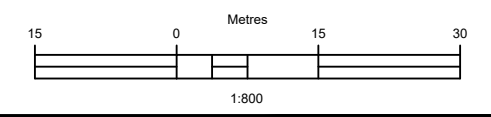
- Notes:
- Chloride and Sulphate were determined on the extract obtained from the 20:1 leaching procedure.
 - All tests were performed by ALS, a CALA accredited laboratory.
 - Laboratory certificates are provided in the report appendices.

Figures

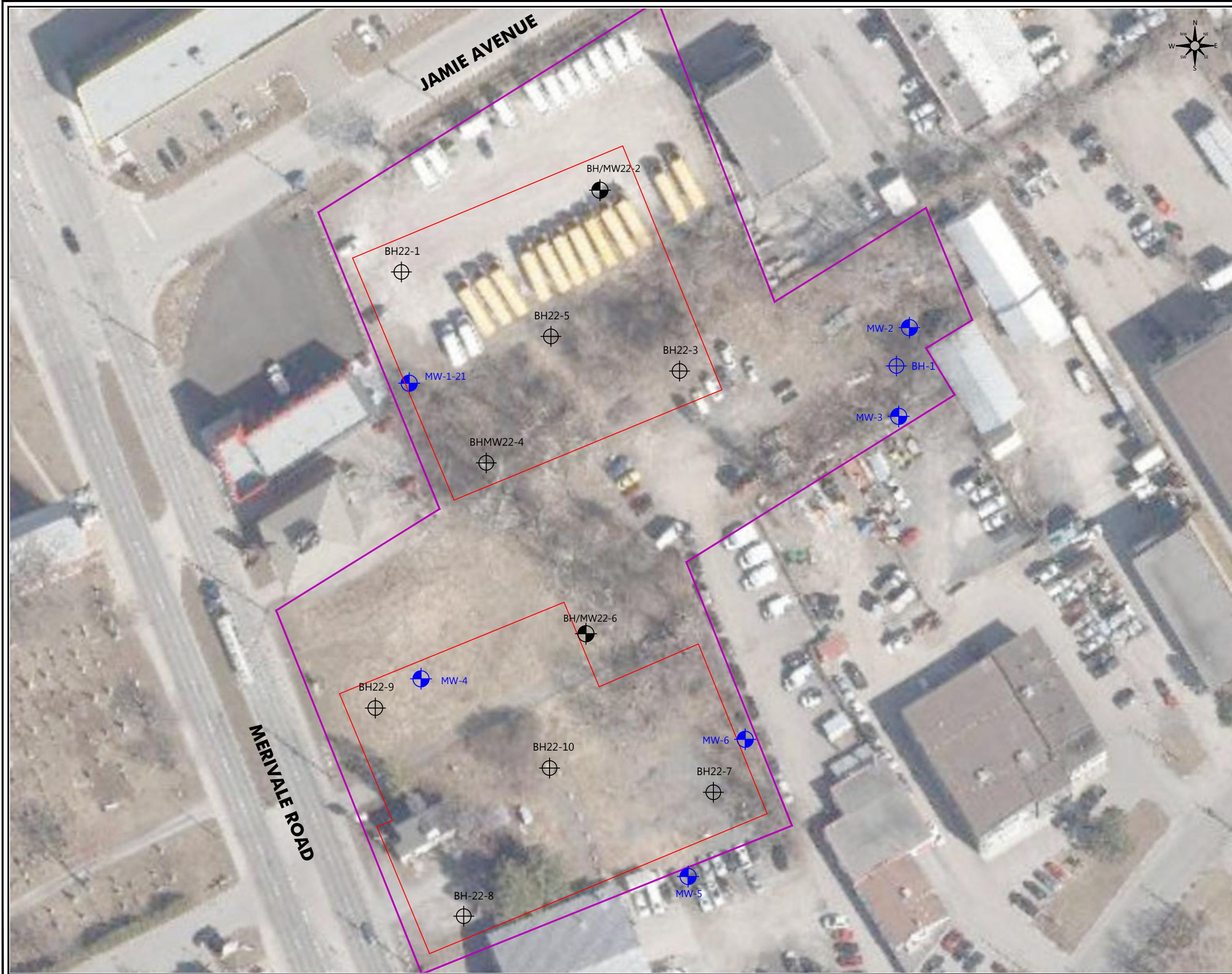


LEGEND

- SITE BOUNDARY
- LOT LINES
- PROPOSED BUILDING FOOTPRINT

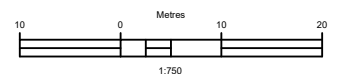


Title:	KEY PLAN
Project:	GEOTECHNICAL INVESTIGATION 1881 & 1883 MERIVALE RD and 6 & 12 JAMIE AVENUE OTTAWA, ONTARIO
Client:	Z.V.HOLDINGS CORP.
Date:	October 2022
ARCADIS	
FIGURE 1	

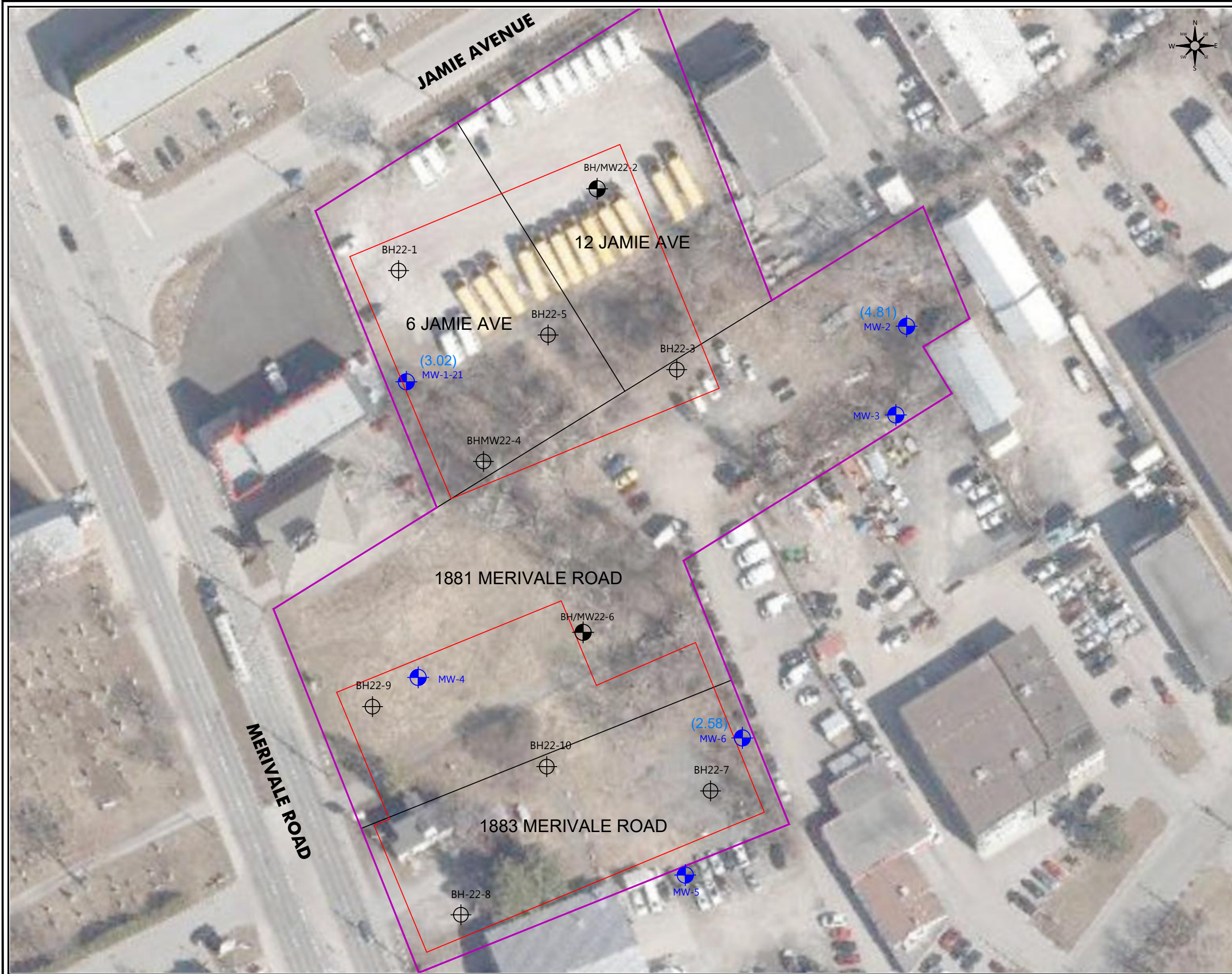


LEGEND





- SITE BOUNDARY
- LOT LINES
- BOREHOLE LOCATION
- MONITORING WELL LOCATION
- BOREHOLE/MONITORING WELL LOCATION (ARCADIS, 2022)
- BOREHOLE LOCATION (ARCADIS, 2022)

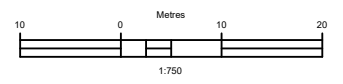



Title:	BOREHOLE LOCATION PLAN
Project:	GEOTECHNICAL INVESTIGATION 1881 & 1883 MERIVALE RD and 6 & 12 JAMIE AVENUE OTTAWA, ONTARIO
Client:	Z.V.HOLDINGS CORP.
Date:	October 2022
FIGURE 2	



LEGEND

-  SITE BOUNDARY
-  LOT LINES
-  BOREHOLE LOCATION
-  MONITORING WELL LOCATION
- (4.81) GROUNDWATER TABLE DEPTH (OCTOBER 2019)



Title: GROUNDWATER TABLE DEPTHS (OCTOBER 2019)	
Project: GEOTECHNICAL INVESTIGATION 1881 & 1883 MERIVALE RD and 6 & 12 JAMIE AVENUE OTTAWA, ONTARIO	
Client: Z.V.HOLDINGS CORP.	
	Date: October 2022
FIGURE 3	

Appendix A

Borehole Logs

Logo








Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-2 (MW22-2)

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-15 COMPLETED 22-9-16
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 98.98 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING 4.85 m / Elev 94.13 m

GEOTECH BH COLUMNS 30127480 1881 MERIVAL GEOTECH BH RVJ (1).GPJ GINT STD CANADA LAB.GDT 22-11-22

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		WELL GRADED GRAVEL, SANDY, light gray, angular, fine to coarse grained, dry, very dense (GRAVEL FILL)										
1		SILTY SAND, light brown, fine grained, moist, loose to medium dense, trace gravel	SS 1	50	35-55-25-12 (80)							
2		Sand becomes fine to coarse	SS 2	75	8-8-9-10 (17)			8				
3		Colour becomes brown to grey	SS 3	67	4-5-4-50 (9)			3				
4			SS 4	92	5-5-4-11 (9)			3				
5		Becomes wet	SS 5	83	6-8-9-9 (17)			8				
5			SS 6	50	3-7-9-12 (16)			6				

Bottom of borehole at 5.20 meters.

Appendix A

Borehole Logs

Logo








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BORING NUMBER BH22-2 (MW22-2)

CLIENT ZV Holdings
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 DATE STARTED 22-9-15 COMPLETED 22-9-16
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 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 98.98 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING 4.85 m / Elev 94.13 m

GEOTECH BH COLUMNS 30127480 1881 MERIVAL GEOTECH BH RVJ (1).GPJ GINT STD CANADA LAB.GDT 22-11-22

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		WELL GRADED GRAVEL, SANDY, light gray, angular, fine to coarse grained, dry, very dense (GRAVEL FILL)										
1		SILTY SAND, light brown, fine grained, moist, loose to medium dense, trace gravel	SS 1	50	35-55-25-12 (80)							
2		Sand becomes fine to coarse	SS 2	75	8-8-9-10 (17)			8				
3		Colour becomes brown to grey	SS 3	67	4-5-4-50 (9)			3				
4			SS 4	92	5-5-4-11 (9)			3				
5		Becomes wet	SS 5	83	6-8-9-9 (17)			8				
5			SS 6	50	3-7-9-12 (16)			6				

Bottom of borehole at 5.20 meters.

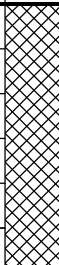


Logo

Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-3

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-16 COMPLETED 22-9-16
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 98.96 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING ---

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
1		WELL GRADED GRAVEL, SANDY, light gray, angular, fine to coarse grained, dry, loose to medium dense some dark brown topsoil (GRAVEL FILL)	SS 0	75	18-12-6-2 (18)							
			SS 1	75	2-2-5-3 (7)			6				
2		SILTY SAND, light brown, fine grained, moist, medium dense, trace gravel	SS 2	75	6-8-10-10 (18)			10				33
		Colour becomes light grey	SS 3	75	4-7-8-6 (15)			8				
			SS 4	75	4-8-8-10 (16)			14				
5		Becomes wet	SS 5	75	6-9-6-7 (15)							

Bottom of borehole at 5.20 meters.

GEOTECH BH COLUMNS 30127480 1881 MERIVALE GEOTECH BH RVJ (1) GPJ GINT STD CANADA LAB GDT 22-11-22

Logo

Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-4 (MW22-4)

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-15 COMPLETED 22-9-15
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 98.97 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING ---

GEOTECH BH COLUMNS 30127480 1881 MERIVAL GEOTECH BH RVJ (1) GPJ GINT STD CANADA LAB GDT 22-11-22

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		ORGANIC SOIL WITH SAND, dark brown, medium to coarse grained, moist, very loose, some organics, and roots (TOPSOIL)	SS 0	50	1-1-1-1 (2)			10				
1		SILTY SAND, light brown, fine grained, moist, medium dense, trace gravel	SS 1	58	5-11-13-16 (24)			5				31
2			SS 2	25	5-11-11-11 (22)			5				
		Becomes wet at 2.5mbs	SS 3	5000	4-7-9-8 (16)			3				
3			SS 4	92	6-8-7-7 (15)			14				
4												
5		Becomes light grey, wet and fine to medium grained	SS 5	83	4-12-13-16 (25)							

Bottom of borehole at 5.20 meters.

Logo






Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-5

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-16 COMPLETED 22-9-16
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 98.81 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING ---

GEOTECH BH COLUMNS 30127480 1881 MERIVAL GEOTECH BH RVJ (1) GPJ GINT STD CANADA LAB GDT 22-11-22

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		WELL GRADED GRAVEL, SANDY, light gray, angular, fine to coarse grained, dry, medium dense (GRAVEL FILL)	SS 1	50	18-4-7-7 (11)							
1		SILT WITH SAND, dark brown to black, well graded, fine to coarse grained, moist, loose to medium dense, some organics, trace clay pockets of organic material	SS 2	67	7-7-7-7 (14)			14				52
		SILTY SAND, light gray to brown, fine grained, moist, medium dense, trace gravel	SS 3	50	5-5-5-5 (10)			17				
2		SILTY SAND, light gray to brown, fine grained, moist, medium dense, trace gravel	SS 4	83	6-12-14-10 (26)			5				
		Colour becomes only grey	SS 5	92	7-10-13-14 (23)			4				
		Becomes wet at 4.5mbgs	SS 6	75	7-12-14-14 (26)			5				

Bottom of borehole at 5.20 meters.

Logo

Arcadis Canada Inc.
1050 Morrison Drive, Suite 201
Ottawa, ON
Telephone: 613-721-0555

BORING NUMBER BH22-6 (MW22-6)

CLIENT ZV Holdings
PROJECT NUMBER 30127480
DATE STARTED 22-9-15 COMPLETED 22-9-15
DRILLING CONTRACTOR Downing Estate Drilling
DRILLING METHOD HSA
LOGGED BY LDeGroot CHECKED BY RJanzen
NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
GROUND ELEVATION 99.04 m HOLE SIZE 15cm
GROUND WATER LEVELS:
AT TIME OF DRILLING ---
AT END OF DRILLING ---
AFTER DRILLING ---

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		ORGANIC SOIL WITH SAND, light brown, well graded, fine to coarse grained, moist, very loose, some organics, trace roots and grass debris (TOPSOIL)	SS 1	50	1-1-2-2 (3)			12				
1		POORLY GRADED SAND, light gray, fine grained, moist, loose to medium dense, some silt	SS 2	75	6-10-12-11 (22)			5				
2		Becoming wet at 2.8mbgs	SS 3	100	4-8-9-8 (17)			6				22
			SS 4	67	3-5-5-5 (10)			11				
3			SS 5	67	3-3-7-6 (10)			19				
4												
5			SS 6	83	6-15-18-21 (33)							

Bottom of borehole at 5.61 meters.

GEOTECH BH COLUMNS 30127480 1881 MERIVAL GEOTECH BH RVJ (1).GPJ GINT STD CANADA LAB.GDT 22-11-22

Logo

Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-7

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-15 COMPLETED 22-9-15
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 99.86 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING ---

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		ORGANIC SOIL WITH SAND, light brown to black, well graded, fine to coarse grained, moist, loose, some organics, trace gravel (TOPSOIL)	SS 1	50	1-2-2-1 (4)			11				
1		POORLY GRADED SAND, light brown, fine grained, moist, medium dense, some silt	SS 2	75	4-7-10-8 (17)			6				
			SS 3	92	3-7-8-9 (15)			6				
2			SS 4	83	4-8-10-13 (18)			5				
		WELL GRADED SAND, light brown, fine to coarse grained, moist, medium dense to dense, some silt	SS 5	83	7-12-11-10 (23)			5				16
3												
4												
		Colour becomes light grey	SS 6	67	4-17-21-16 (38)							
5												

Bottom of borehole at 5.20 meters.

GEOTECH BH COLUMNS 30127480 1881 MERIVALE GEOTECH BH RVJ (1) GPJ GINT STD CANADA LAB GDT 22-11-22

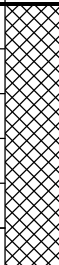


Logo

Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-8

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-16 COMPLETED 22-9-16
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 99.48 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING ---

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
1		WELL GRADED GRAVEL, SANDY, light gray, angular, fine to coarse grained, dry, loose (GRAVEL FILL)	SS 1	33	11-3-1-2 (4)							
		WELL GRADED SAND, brown, fine to coarse grained, loose (SAND FILL)	SS 2	92	5-3-2-4 (5)			8				
2		SILTY SAND, brown, medium to coarse grained, moist, medium dense	SS 3	83	3-9-11-11 (20)			7				
		Sand becomes fine, colour becomes grey	SS 4	75	4-8-9-11 (17)			4				
			SS 5	83	5-11-11-10 (22)			9				
5		Colour becomes dark grey	SS 6	83	6-12-13-9 (25)							

Bottom of borehole at 5.20 meters.

GEOTECH BH COLUMNS 30127480 1881 MERIVALE GEOTECH BH RVJ (1) GPJ GINT STD CANADA LAB GDT 22-11-22

Logo

Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-9

CLIENT ZV Holdings
 PROJECT NUMBER 30127480
 DATE STARTED 22-9-16 COMPLETED 22-9-16
 DRILLING CONTRACTOR Downing Estate Drilling
 DRILLING METHOD HSA
 LOGGED BY LDeGroot CHECKED BY RJanzen
 NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
 PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
 GROUND ELEVATION 99.15 m HOLE SIZE 15cm
 GROUND WATER LEVELS:
 AT TIME OF DRILLING ---
 AT END OF DRILLING ---
 AFTER DRILLING ---

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		ORGANIC SOIL WITH SAND, light brown to black, well graded, fine to coarse grained, moist, very loose, some organics, trace gravel (TOPSOIL)	SS 1	83	1-1-2-1 (3)			16				
1		SILTY SAND, dark brown to light gray, fine grained, moist, medium dense	SS 2	92	4-9-12-11 (21)			8				
		Colour become grey	SS 3	92	6-10-15-13 (25)			9				
2		Coarse sand lenses	SS 4	100	5-9-10-9 (19)			8				
3			SS 5	100	4-10-12-15 (22)			6				
4												
5		Sand becomes fine to coarse	SS 6	75	5-8-11-10 (19)							

Bottom of borehole at 5.20 meters.

GEOTECH BH COLUMNS 30127480 1881 MERIVALE GEOTECH BH RVJ (1) GPJ GINT STD CANADA LAB GDT 22-11-22

Logo

Arcadis Canada Inc.
 1050 Morrison Drive, Suite 201
 Ottawa, ON
 Telephone: 613-721-0555

BORING NUMBER BH22-10

CLIENT ZV Holdings
PROJECT NUMBER 30127480
DATE STARTED 22-9-16 **COMPLETED** 22-9-16
DRILLING CONTRACTOR Downing Estate Drilling
DRILLING METHOD HSA
LOGGED BY LDeGroot **CHECKED BY** RJanzen
NOTES _____

PROJECT NAME 1881-1883 Merivale Geotech
PROJECT LOCATION 1881 Merivale Road, Ottawa, ON
GROUND ELEVATION 99.86 m **HOLE SIZE** 15cm
GROUND WATER LEVELS:
AT TIME OF DRILLING ---
AT END OF DRILLING ---
AFTER DRILLING ---

DEPTH (m)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	DRY UNIT WT. (Mg/m ³)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		ORGANIC SOIL WITH SAND, dark brown to black, well graded, fine to coarse grained, moist, very loose, some organics, trace roots (TOPSOIL)	SS 0	33	1-1-1-1 (2)							
		SILT WITH SAND, light gray, fine grained, moist, loose to medium dense	SS 1	67	1-1-8-14 (9)			14				
		Some mottling visible	SS 2	83	5-10-9-10 (19)			12				
2			SS 3	50	9-9-9-11 (18)			15				62
			SS 4	67	5-13-12-12 (25)			2				
4			Trace gravel, mottled texture	SS 5	75	4-7-15-15 (22)						
		SPT cone test begins at 5.18mbgs.	SPT		7-8-18-18 (26)							
6			SPT		17-17-17-18 (34)							
			SPT		18-19-23-23 (42)							
			SPT		22-23-22-23 (45)							
8			SPT		14-15-10-9 (25)							
			SPT		14-13-17-18 (30)							
			SPT		22-23-28-29 (51)							
10			SPT		26-26-27-28 (53)							
		Borehole ends.	SPT		27-27-18-18 (45)							

Bottom of borehole at 10.67 meters.

GEOTECH BH COLUMNS: 30127480 1881 MERIVALE GEOTECH BH_RVJ (1).GPJ GINT STD CANADA LAB.GDT 23-2-1

Appendix B

Photo Log

Project Photographs

Preliminary Geotechnical Investigation – 1881/1883 Merivale and Adjacent Lot, Ottawa, ON



Photo: 1

Date: 16 September 2022

Description:

View of the drilling of borehole location MW22-6 at the northeast portion of the 6/12 Jamie Avenue bus parking lot. Looking northeast.



Photo: 2

Date: 16 September 2022

Description:

View of the drilling of borehole location BH22-5 at the southwest portion the 6/12 Jamie Avenue bus parking lot. Looking northeast.

Project Photographs

Preliminary Geotechnical Investigation – 1881/1883 Merivale and Adjacent Lot, Ottawa, ON



Photo: 3

Date: 16 September 2022

Description:

View of the drilling of borehole location BH22-8 at the southwest portion of 1881 Merivale Road, looking northeast. The scuba divers' warehouse is visible.



Photo: 4

Date: 16 September 2022

Description:

View of the drilling of borehole location MW22-4 at the northern portion of 1881 Merivale Road, looking north.

Project Photographs

Preliminary Geotechnical Investigation – 1881/1883 Merivale and Adjacent Lot, Ottawa, ON



Photo: 5

Date: 16 September 2022

Description:

View of a split spoon containing the typical light brown fine sand observed at most areas on the property.

Appendix C

Laboratory Certificates of Analysis



CERTIFICATE OF ANALYSIS

<p>Work Order : WT2214822</p> <p>Amendment : 1</p> <p>Client : Arcadis Canada Inc.</p> <p>Contact : Lennart DeGroot</p> <p>Address : 1050 Morrison Drive Suite 201 Ottawa ON Canada K2H 1L1</p> <p>Telephone : 613 721 0555</p> <p>Project : 30127480</p> <p>PO : ----</p> <p>C-O-C number : ----</p> <p>Sampler : ----</p> <p>Site : ----</p> <p>Quote number : Waterloo 2022 Price List</p> <p>No. of samples received : 42</p> <p>No. of samples analysed : 42</p>	<p>Page : 1 of 10</p> <p>Laboratory : Waterloo - Environmental</p> <p>Account Manager : Emily Smith</p> <p>Address : 60 Northland Road, Unit 1 Waterloo ON Canada N2V 2B8</p> <p>Telephone : +1 519 886 6910</p> <p>Date Samples Received : 19-Sep-2022 14:55</p> <p>Date Analysis Commenced : 21-Sep-2022</p> <p>Issue Date : 31-Oct-2022 10:15</p>
--	--

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QC Interpretive report to assist with Quality Review and Sample Receipt Notification (SRN).

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is conducted in accordance with US FDA 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Laboratory Department</i>
Amanda Ganouri-Lumsden	Department Manager - Microbiology and Prep	Centralized Prep, Waterloo, Ontario
Hedy Lai	Team Leader - Inorganics	Inorganics, Saskatoon, Saskatchewan
Hedy Lai	Team Leader - Inorganics	Sask Soils, Saskatoon, Saskatchewan
Jon Fisher	Department Manager - Inorganics	Inorganics, Waterloo, Ontario
Joseph Scharbach		Centralized Prep, Waterloo, Ontario
Niral Patel		Centralized Prep, Waterloo, Ontario



General Comments

The analytical methods used by ALS are developed using internationally recognized reference methods (where available), such as those published by US EPA, APHA Standard Methods, ASTM, ISO, Environment Canada, BC MOE, and Ontario MOE. Refer to the ALS Quality Control Interpretive report (QCI) for applicable references and methodology summaries. Reference methods may incorporate modifications to improve performance.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

Please refer to Quality Control Interpretive report (QCI) for information regarding Holding Time compliance.

Key : CAS Number: Chemical Abstracts Services number is a unique identifier assigned to discrete substances
 LOR: Limit of Reporting (detection limit).

Unit	Description
-	No Unit
%	percent
µS/cm	Microsiemens per centimetre
mg/kg	milligrams per kilogram
mV	millivolts
ohm cm	ohm centimetre (resistivity)
pH units	pH units

<: less than.

>: greater than.

Surrogate: An analyte that is similar in behavior to target analyte(s), but that does not occur naturally in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED on SRN or QCI Report, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

Sample Comments

Sample	Client Id	Comment
WT2214822-007	BH22-3-2	Sample(s) XXX: Limited sample was available for PSA (100g minimum is standard). Measurement Uncertainty for PSA results may be higher than usual.
WT2214822-020	MW22-4-2	Sample(s) XXX: Limited sample was available for PSA (100g minimum is standard). Measurement Uncertainty for PSA results may be higher than usual.
WT2214822-025	BH22-5-2	Sample(s) XXX: Limited sample was available for PSA (100g minimum is standard). Measurement Uncertainty for PSA results may be higher than usual.
WT2214822-030	MW22-6-2	Sample(s) XXX: Limited sample was available for PSA (100g minimum is standard). Measurement Uncertainty for PSA results may be higher than usual.
WT2214822-038	BH22-7-5	Sample(s) XXX: Limited sample was available for PSA (100g minimum is standard). Measurement Uncertainty for PSA results may be higher than usual.



WT2214822-040

BH22-10-2

Sample(s) XXX: Limited sample was available for PSA (100g minimum is standard). Measurement Uncertainty for PSA results may be higher than usual.



Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID				
(Matrix: Soil/Solid)					MW22-2-1	MW22-2-2	MW22-2-3	MW22-2-4	MW22-2-5
Client sampling date / time					16-Sep-2022 09:00	16-Sep-2022 09:00	16-Sep-2022 09:00	16-Sep-2022 09:00	16-Sep-2022 09:00
Analyte	CAS Number	Method	LOR	Unit	WT2214822-001	WT2214822-002	WT2214822-003	WT2214822-004	WT2214822-005
					Result	Result	Result	Result	Result
Physical Tests									
moisture	----	E144	0.25	%	8.08	3.28	3.22	8.18	6.09

Please refer to the General Comments section for an explanation of any qualifiers detected.

Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID				
(Matrix: Soil/Solid)					BH22-3-1	BH22-3-2	BH22-3-3	BH22-3-4	BH22-8-1
Client sampling date / time					16-Sep-2022 12:00	16-Sep-2022 12:00	16-Sep-2022 12:00	16-Sep-2022 12:00	16-Sep-2022 14:00
Analyte	CAS Number	Method	LOR	Unit	WT2214822-006	WT2214822-007	WT2214822-008	WT2214822-009	WT2214822-010
					Result	Result	Result	Result	Result
Physical Tests									
conductivity (1:2 leachate)	----	E100-L	5.00	µS/cm	----	202	----	----	----
moisture	----	E144	0.25	%	5.84	9.93	7.95	13.5	8.06
oxidation-reduction potential [ORP]	----	E125	0.10	mV	----	486	----	----	----
pH (1:2 soil:CaCl2-aq)	----	E108A	0.10	pH units	----	7.54	----	----	----
resistivity	----	EC100R	100	ohm cm	----	4950	----	----	----
Particle Size									
passing (9.5 mm)	----	E181	1.0	%	----	100	----	----	----
passing (4.75 mm)	----	E181	1.0	%	----	100	----	----	----
passing (19 mm)	----	E181	1.0	%	----	100	----	----	----
passing (25.4 mm)	----	E181	1.0	%	----	100	----	----	----
passing (38.1 mm)	----	E181	1.0	%	----	100	----	----	----
passing (50.8 mm)	----	E181	1.0	%	----	100	----	----	----
passing (76.2 mm)	----	E181	1.0	%	----	100	----	----	----
passing (1.0 mm)	----	E182	1.0	%	----	100	----	----	----
passing (0.841 mm)	----	E182	1.0	%	----	100	----	----	----
passing (0.50 mm)	----	E182	1.0	%	----	99.8	----	----	----
passing (0.420 mm)	----	E182	1.0	%	----	99.7	----	----	----



Analytical Results

Sub-Matrix: Soil/Solid

Client sample ID

(Matrix: Soil/Solid)

					BH22-3-1	BH22-3-2	BH22-3-3	BH22-3-4	BH22-8-1
Client sampling date / time					16-Sep-2022 12:00	16-Sep-2022 12:00	16-Sep-2022 12:00	16-Sep-2022 12:00	16-Sep-2022 14:00
Analyte	CAS Number	Method	LOR	Unit	WT2214822-006	WT2214822-007	WT2214822-008	WT2214822-009	WT2214822-010
					Result	Result	Result	Result	Result
Particle Size									
passing (0.250 mm)	----	E182	1.0	%	----	95.9	----	----	----
passing (0.149 mm)	----	E182	1.0	%	----	75.6	----	----	----
passing (0.125 mm)	----	E182	1.0	%	----	61.8	----	----	----
passing (0.075 mm)	----	E182	1.0	%	----	33.1	----	----	----
passing (0.063 mm)	----	E182	1.0	%	----	22.7	----	----	----
passing (0.05 mm)	----	E182	1.0	%	----	13.4	----	----	----
passing (0.0312 mm)	----	E183	1.0	%	----	7.8	----	----	----
passing (0.020 mm)	----	E183	1.0	%	----	4.5	----	----	----
passing (0.005 mm)	----	E183	1.0	%	----	3.4	----	----	----
passing (0.004 mm)	----	E183	1.0	%	----	3.0	----	----	----
passing (0.002 mm)	----	E183	1.0	%	----	2.2	----	----	----
grain size curve	----	E185	-	-	----	See	----	----	----
passing (2.0 mm)	----	E181	1.0	%	----	Attached 100	----	----	----
Inorganic Parameters									
sulfides, acid volatile	----	E396-L	0.20	mg/kg	----	<0.20	----	----	----
Leachable Anions & Nutrients									
chloride, soluble ion content	16887-00-6	E236.Cl	5.0	mg/kg	----	29.8	----	----	----
sulfate, soluble ion content	14808-79-8	E236.SO4	20	mg/kg	----	<20	----	----	----

Please refer to the General Comments section for an explanation of any qualifiers detected.



Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID				
(Matrix: Soil/Solid)					BH22-8-2	BH22-8-3	BH22-8-4	BH22-9-1	BH22-9-2
Client sampling date / time					16-Sep-2022 14:00	16-Sep-2022 14:00	16-Sep-2022 14:00	16-Sep-2022 13:00	16-Sep-2022 13:00
Analyte	CAS Number	Method	LOR	Unit	WT2214822-011	WT2214822-012	WT2214822-013	WT2214822-014	WT2214822-015
					Result	Result	Result	Result	Result
Physical Tests									
moisture	----	E144	0.25	%	7.19	4.09	8.75	15.9	7.82

Please refer to the General Comments section for an explanation of any qualifiers detected.

Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID				
(Matrix: Soil/Solid)					BH22-9-3	BH22-9-4	BH22-9-5	MW22-4-1	MW22-4-2
Client sampling date / time					16-Sep-2022 13:00	16-Sep-2022 13:00	16-Sep-2022 13:00	15-Sep-2022 15:45	15-Sep-2022 15:45
Analyte	CAS Number	Method	LOR	Unit	WT2214822-016	WT2214822-017	WT2214822-018	WT2214822-019	WT2214822-020
					Result	Result	Result	Result	Result
Physical Tests									
moisture	----	E144	0.25	%	9.34	7.60	6.48	10.1	4.59
Particle Size									
sand (>0.075mm)	----	E178	1.0	%	----	----	----	----	69.3
finer (<0.075mm)	----	E178	1.0	%	----	----	----	----	30.6
texture class	----	E178	-	-	----	----	----	----	Coarse

Please refer to the General Comments section for an explanation of any qualifiers detected.



Analytical Results

Sub-Matrix: Soil/Solid (Matrix: Soil/Solid)					Client sample ID	MW22-4-3	MW22-4-4	MW22-4-5	BH22-5-1	BH22-5-2
Client sampling date / time					15-Sep-2022 15:45	15-Sep-2022 15:45	15-Sep-2022 15:45	16-Sep-2022 08:50	16-Sep-2022 08:50	
Analyte	CAS Number	Method	LOR	Unit	WT2214822-021	WT2214822-022	WT2214822-023	WT2214822-024	WT2214822-025	
					Result	Result	Result	Result	Result	
Physical Tests										
moisture	---	E144	0.25	%	5.44	2.88	13.8	14.3	16.9	
Particle Size										
passing (9.5 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (4.75 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (19 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (25.4 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (38.1 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (50.8 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (76.2 mm)	---	E181	1.0	%	---	---	---	---	100	
passing (1.0 mm)	---	E182	1.0	%	---	---	---	---	94.3	
passing (0.841 mm)	---	E182	1.0	%	---	---	---	---	93.2	
passing (0.50 mm)	---	E182	1.0	%	---	---	---	---	80.7	
passing (0.420 mm)	---	E182	1.0	%	---	---	---	---	77.9	
passing (0.250 mm)	---	E182	1.0	%	---	---	---	---	74.3	
passing (0.149 mm)	---	E182	1.0	%	---	---	---	---	66.6	
passing (0.125 mm)	---	E182	1.0	%	---	---	---	---	61.9	
passing (0.075 mm)	---	E182	1.0	%	---	---	---	---	52.2	
passing (0.063 mm)	---	E182	1.0	%	---	---	---	---	43.4	
passing (0.05 mm)	---	E182	1.0	%	---	---	---	---	33.8	
passing (0.0312 mm)	---	E183	1.0	%	---	---	---	---	21.5	
passing (0.020 mm)	---	E183	1.0	%	---	---	---	---	14.9	
passing (0.005 mm)	---	E183	1.0	%	---	---	---	---	6.4	
passing (0.004 mm)	---	E183	1.0	%	---	---	---	---	5.7	
passing (0.002 mm)	---	E183	1.0	%	---	---	---	---	4.8	
grain size curve	---	E185	-	-	---	---	---	---	See Attached	
passing (2.0 mm)	---	E181	1.0	%	---	---	---	---	99.4	

Please refer to the General Comments section for an explanation of any qualifiers detected.



Analytical Results

Sub-Matrix: Soil/Solid

Client sample ID

(Matrix: Soil/Solid)

					BH22-5-3	BH22-5-4	BH22-5-5	MW22-6-1	MW22-6-2
Client sampling date / time					16-Sep-2022 08:50	16-Sep-2022 08:50	16-Sep-2022 08:50	15-Sep-2022 11:15	15-Sep-2022 11:15
Analyte	CAS Number	Method	LOR	Unit	WT2214822-026	WT2214822-027	WT2214822-028	WT2214822-029	WT2214822-030
					Result	Result	Result	Result	Result
Physical Tests									
conductivity (1:2 leachate)	----	E100-L	5.00	µS/cm	194	----	----	----	----
moisture	----	E144	0.25	%	4.82	4.42	5.22	11.9	5.34
oxidation-reduction potential [ORP]	----	E125	0.10	mV	457	----	----	----	----
pH (1:2 soil:CaCl2-aq)	----	E108A	0.10	pH units	7.52	----	----	----	----
resistivity	----	EC100R	100	ohm cm	5150	----	----	----	----
Particle Size									
sand (>0.075mm)	----	E178	1.0	%	----	----	----	----	78.5
finer (<0.075mm)	----	E178	1.0	%	----	----	----	----	21.5
texture class	----	E178	-	-	----	----	----	----	Coarse
Inorganic Parameters									
sulfides, acid volatile	----	E396-L	0.20	mg/kg	<0.20	----	----	----	----
Leachable Anions & Nutrients									
chloride, soluble ion content	16887-00-6	E236.Cl	5.0	mg/kg	40.5	----	----	----	----
sulfate, soluble ion content	14808-79-8	E236.SO4	20	mg/kg	<20	----	----	----	----

Please refer to the General Comments section for an explanation of any qualifiers detected.



Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID				
(Matrix: Soil/Solid)					MW22-6-3	MW22-6-4	MW22-6-5	BH22-7-1	BH22-7-2
Client sampling date / time					15-Sep-2022 11:15	15-Sep-2022 11:15	15-Sep-2022 11:15	15-Sep-2022 13:00	15-Sep-2022 13:00
Analyte	CAS Number	Method	LOR	Unit	WT2214822-031	WT2214822-032	WT2214822-033	WT2214822-034	WT2214822-035
					Result	Result	Result	Result	Result
Physical Tests									
conductivity (1:2 leachate)	----	E100-L	5.00	µS/cm	40.2	----	----	----	----
moisture	----	E144	0.25	%	5.90	10.6	19.4	10.6	5.59
oxidation-reduction potential [ORP]	----	E125	0.10	mV	409	----	----	----	----
pH (1:2 soil:CaCl2-aq)	----	E108A	0.10	pH units	6.63	----	----	----	----
resistivity	----	EC100R	100	ohm cm	24900	----	----	----	----
Inorganic Parameters									
sulfides, acid volatile	----	E396-L	0.20	mg/kg	<0.20	----	----	----	----
Leachable Anions & Nutrients									
chloride, soluble ion content	16887-00-6	E236.Cl	5.0	mg/kg	<5.0	----	----	----	----
sulfate, soluble ion content	14808-79-8	E236.SO4	20	mg/kg	<20	----	----	----	----

Please refer to the General Comments section for an explanation of any qualifiers detected.

Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID				
(Matrix: Soil/Solid)					BH22-7-3	BH22-7-4	BH22-7-5	BH22-10-1	BH22-10-2
Client sampling date / time					15-Sep-2022 13:00	15-Sep-2022 13:00	15-Sep-2022 13:00	15-Sep-2022 14:10	15-Sep-2022 14:10
Analyte	CAS Number	Method	LOR	Unit	WT2214822-036	WT2214822-037	WT2214822-038	WT2214822-039	WT2214822-040
					Result	Result	Result	Result	Result
Physical Tests									
moisture	----	E144	0.25	%	6.03	4.98	4.63	14.4	12.0
Particle Size									
sand (>0.075mm)	----	E178	1.0	%	----	----	83.9	----	38.1
finer (<0.075mm)	----	E178	1.0	%	----	----	16.1	----	61.9
texture class	----	E178	-	-	----	----	Coarse	----	Fine

Please refer to the General Comments section for an explanation of any qualifiers detected.



Analytical Results

Sub-Matrix: Soil/Solid					Client sample ID		BH22-10-3	BH22-10-4	----	----	----
(Matrix: Soil/Solid)					Client sampling date / time		15-Sep-2022 14:10	15-Sep-2022 14:10	----	----	----
Analyte	CAS Number	Method	LOR	Unit	WT2214822-041	WT2214822-042	-----	-----	-----	-----	-----
Physical Tests					Result	Result	----	----	----	----	----
moisture	----	E144	0.25	%	14.6	1.99	----	----	----	----	----

Please refer to the General Comments section for an explanation of any qualifiers detected.



QUALITY CONTROL INTERPRETIVE REPORT

<p>Work Order : WT2214822</p> <p>Amendment : 1</p> <p>Client : Arcadis Canada Inc.</p> <p>Contact : Lennart DeGroot</p> <p>Address : 1050 Morrison Drive Suite 201 Ottawa ON Canada K2H 1L1</p> <p>Telephone : 613 721 0555</p> <p>Project : 30127480</p> <p>PO : ----</p> <p>C-O-C number : ----</p> <p>Sampler : ----</p> <p>Site : ----</p> <p>Quote number : Waterloo 2022 Price List</p> <p>No. of samples received : 42</p> <p>No. of samples analysed : 42</p>	<p>Page : 1 of 15</p> <p>Laboratory : Waterloo - Environmental</p> <p>Account Manager : Emily Smith</p> <p>Address : 60 Northland Road, Unit 1 Waterloo, Ontario Canada N2V 2B8</p> <p>Telephone : +1 519 886 6910</p> <p>Date Samples Received : 19-Sep-2022 14:55</p> <p>Issue Date : 31-Oct-2022 10:15</p>
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This report is automatically generated by the ALS LIMS (Laboratory Information Management System) through evaluation of Quality Control (QC) results and other QA parameters associated with this submission, and is intended to facilitate rapid data validation by auditors or reviewers. The report highlights any exceptions and outliers to ALS Data Quality Objectives, provides holding time details and exceptions, summarizes QC sample frequencies, and lists applicable methodology references and summaries.

Key

- Anonymous: Refers to samples which are not part of this work order, but which formed part of the QC process lot.
- CAS Number: Chemical Abstracts Service number is a unique identifier assigned to discrete substances.
- DQO: Data Quality Objective.
- LOR: Limit of Reporting (detection limit).
- RPD: Relative Percent Difference.

Workorder Comments

Holding times are displayed as "----" if no guidance exists from CCME, Canadian provinces, or broadly recognized international references.

Summary of Outliers

Outliers : Quality Control Samples

- No Method Blank value outliers occur.
- No Duplicate outliers occur.
- No Laboratory Control Sample (LCS) outliers occur
- No Test sample Surrogate recovery outliers exist.

Outliers: Reference Material (RM) Samples

- No Reference Material (RM) Sample outliers occur.

Outliers : Analysis Holding Time Compliance (Breaches)

- No Analysis Holding Time Outliers exist.

Outliers : Frequency of Quality Control Samples

- No Quality Control Sample Frequency Outliers occur.



Analysis Holding Time Compliance

This report summarizes extraction / preparation and analysis times and compares each with ALS recommended holding times, which are selected to meet known provincial and /or federal requirements. In the absence of regulatory hold times, ALS establishes recommendations based on guidelines published by organizations such as CCME, US EPA, APHA Standard Methods, ASTM, or Environment Canada (where available). Dates and holding times reported below represent the first dates of extraction or analysis. If subsequent tests or dilutions exceeded holding times, qualifiers are added (refer to COA).

If samples are identified below as having been analyzed or extracted outside of recommended holding times, measurement uncertainties may be increased, and this should be taken into consideration when interpreting results.

Where actual sampling date is not provided on the chain of custody, the date of receipt with time at 00:00 is used for calculation purposes.

Where only the sample date without time is provided on the chain of custody, the sampling date at 00:00 is used for calculation purposes.

Matrix: Soil/Solid

Evaluation: * = Holding time exceedance ; ✓ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis				
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval	
				Rec	Actual			Rec	Actual		
Inorganic Parameters : Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)											
Glass soil jar/Teflon lined cap BH22-3-2	E396-L	16-Sep-2022	21-Sep-2022	14 days	5 days	✓	21-Sep-2022	7 days	0 days	✓	
Inorganic Parameters : Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)											
Glass soil jar/Teflon lined cap BH22-5-3	E396-L	16-Sep-2022	21-Sep-2022	14 days	5 days	✓	21-Sep-2022	7 days	0 days	✓	
Inorganic Parameters : Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)											
Glass soil jar/Teflon lined cap MW22-6-3	E396-L	15-Sep-2022	21-Sep-2022	14 days	6 days	✓	21-Sep-2022	7 days	0 days	✓	
Leachable Anions & Nutrients : Water Extractable Chloride by IC											
Glass soil jar/Teflon lined cap BH22-3-2	E236.Cl	16-Sep-2022	26-Sep-2022	30 days	10 days	✓	28-Sep-2022	28 days	2 days	✓	
Leachable Anions & Nutrients : Water Extractable Chloride by IC											
Glass soil jar/Teflon lined cap BH22-5-3	E236.Cl	16-Sep-2022	26-Sep-2022	30 days	10 days	✓	28-Sep-2022	28 days	2 days	✓	
Leachable Anions & Nutrients : Water Extractable Chloride by IC											
Glass soil jar/Teflon lined cap MW22-6-3	E236.Cl	15-Sep-2022	26-Sep-2022	30 days	11 days	✓	28-Sep-2022	28 days	2 days	✓	
Leachable Anions & Nutrients : Water Extractable Sulfate by IC											
Glass soil jar/Teflon lined cap BH22-3-2	E236.SO4	16-Sep-2022	26-Sep-2022	30 days	10 days	✓	28-Sep-2022	28 days	2 days	✓	



Matrix: Soil/Solid

Evaluation: * = Holding time exceedance ; ✓ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis				
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval	
				Rec	Actual			Rec	Actual		
Leachable Anions & Nutrients : Water Extractable Sulfate by IC											
Glass soil jar/Teflon lined cap BH22-5-3	E236.S04	16-Sep-2022	26-Sep-2022	30 days	10 days	✓	28-Sep-2022	28 days	2 days	✓	
Leachable Anions & Nutrients : Water Extractable Sulfate by IC											
Glass soil jar/Teflon lined cap MW22-6-3	E236.S04	15-Sep-2022	26-Sep-2022	30 days	11 days	✓	28-Sep-2022	28 days	2 days	✓	
Particle Size : CCME fine/coarse Particle Size Analysis by wet sieve											
Glass soil jar/Teflon lined cap BH22-10-2	E178	15-Sep-2022	----	----	----		30-Sep-2022	180 days	15 days	✓	
Particle Size : CCME fine/coarse Particle Size Analysis by wet sieve											
Glass soil jar/Teflon lined cap BH22-7-5	E178	15-Sep-2022	----	----	----		30-Sep-2022	180 days	15 days	✓	
Particle Size : CCME fine/coarse Particle Size Analysis by wet sieve											
Glass soil jar/Teflon lined cap MW22-4-2	E178	15-Sep-2022	----	----	----		30-Sep-2022	180 days	15 days	✓	
Particle Size : CCME fine/coarse Particle Size Analysis by wet sieve											
Glass soil jar/Teflon lined cap MW22-6-2	E178	15-Sep-2022	----	----	----		30-Sep-2022	180 days	15 days	✓	
Particle Size : Grain Size Report (Attachment) Hydrometer/Sieve Method											
Glass soil jar/Teflon lined cap BH22-3-2	E185	16-Sep-2022	----	----	----		29-Sep-2022	----	----		
Particle Size : Grain Size Report (Attachment) Hydrometer/Sieve Method											
Glass soil jar/Teflon lined cap BH22-5-2	E185	16-Sep-2022	----	----	----		29-Sep-2022	----	----		
Particle Size : Particle Size Analysis - Hydrometer											
Glass soil jar/Teflon lined cap BH22-3-2	E183	16-Sep-2022	23-Sep-2022	----	----		23-Sep-2022	365 days	7 days	✓	



Matrix: Soil/Solid

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis				
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval	
				Rec	Actual			Rec	Actual		
Particle Size : Particle Size Analysis - Hydrometer											
Glass soil jar/Teflon lined cap BH22-5-2	E183	16-Sep-2022	23-Sep-2022	----	----		23-Sep-2022	365 days	7 days	✔	
Particle Size : Particle Size Analysis - Sieve <2mm											
Glass soil jar/Teflon lined cap BH22-3-2	E182	16-Sep-2022	23-Sep-2022	----	----		23-Sep-2022	365 days	7 days	✔	
Particle Size : Particle Size Analysis - Sieve <2mm											
Glass soil jar/Teflon lined cap BH22-5-2	E182	16-Sep-2022	23-Sep-2022	----	----		23-Sep-2022	365 days	7 days	✔	
Particle Size : Particle Size Analysis - Sieve >2mm											
Glass soil jar/Teflon lined cap BH22-3-2	E181	16-Sep-2022	23-Sep-2022	----	----		23-Sep-2022	365 days	7 days	✔	
Particle Size : Particle Size Analysis - Sieve >2mm											
Glass soil jar/Teflon lined cap BH22-5-2	E181	16-Sep-2022	23-Sep-2022	----	----		23-Sep-2022	365 days	7 days	✔	
Physical Tests : Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)											
Glass soil jar/Teflon lined cap BH22-3-2	E100-L	16-Sep-2022	28-Sep-2022	----	----		28-Sep-2022	30 days	12 days	✔	
Physical Tests : Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)											
Glass soil jar/Teflon lined cap BH22-5-3	E100-L	16-Sep-2022	28-Sep-2022	----	----		28-Sep-2022	30 days	12 days	✔	
Physical Tests : Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)											
Glass soil jar/Teflon lined cap MW22-6-3	E100-L	15-Sep-2022	28-Sep-2022	----	----		28-Sep-2022	30 days	13 days	✔	
Physical Tests : Moisture Content by Gravimetry											
Glass soil jar/Teflon lined cap BH22-10-1	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----		



Matrix: Soil/Solid

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis			
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval
				Rec	Actual			Rec	Actual	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-10-2	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-10-3	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-10-4	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-3-1	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-3-2	E144	16-Sep-2022	----	----	----		21-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-3-3	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-3-4	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-5-1	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-5-2	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	



Matrix: Soil/Solid

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis			
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval
				Rec	Actual			Rec	Actual	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-5-3	E144	16-Sep-2022	----	----	----		21-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-5-4	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-5-5	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-7-1	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-7-2	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-7-3	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-7-4	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-7-5	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-8-1	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	



Matrix: Soil/Solid

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis			
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval
				Rec	Actual			Rec	Actual	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-8-2	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-8-3	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-8-4	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-9-1	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-9-2	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-9-3	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-9-4	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap BH22-9-5	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-2-1	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	



Matrix: Soil/Solid

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis			
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval
				Rec	Actual			Rec	Actual	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-2-2	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-2-3	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-2-4	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-2-5	E144	16-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-4-1	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-4-2	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-4-3	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-4-4	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	
Physical Tests : Moisture Content by Gravimetry										
Glass soil jar/Teflon lined cap MW22-4-5	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----	



Matrix: Soil/Solid

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis				
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval	
				Rec	Actual			Rec	Actual		
Physical Tests : Moisture Content by Gravimetry											
Glass soil jar/Teflon lined cap MW22-6-1	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----		
Physical Tests : Moisture Content by Gravimetry											
Glass soil jar/Teflon lined cap MW22-6-2	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----		
Physical Tests : Moisture Content by Gravimetry											
Glass soil jar/Teflon lined cap MW22-6-3	E144	15-Sep-2022	----	----	----		21-Sep-2022	----	----		
Physical Tests : Moisture Content by Gravimetry											
Glass soil jar/Teflon lined cap MW22-6-4	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----		
Physical Tests : Moisture Content by Gravimetry											
Glass soil jar/Teflon lined cap MW22-6-5	E144	15-Sep-2022	----	----	----		22-Sep-2022	----	----		
Physical Tests : ORP by Electrode											
Glass soil jar/Teflon lined cap BH22-3-2	E125	16-Sep-2022	22-Sep-2022	----	----		22-Sep-2022	180 days	6 days	✔	
Physical Tests : ORP by Electrode											
Glass soil jar/Teflon lined cap BH22-5-3	E125	16-Sep-2022	22-Sep-2022	----	----		22-Sep-2022	180 days	6 days	✔	
Physical Tests : ORP by Electrode											
Glass soil jar/Teflon lined cap MW22-6-3	E125	15-Sep-2022	22-Sep-2022	----	----		22-Sep-2022	180 days	7 days	✔	
Physical Tests : pH by Meter (1:2 Soil:0.01M CaCl2 Extraction) - As Received											
Glass soil jar/Teflon lined cap BH22-3-2	E108A	16-Sep-2022	22-Sep-2022	----	----		22-Sep-2022	30 days	6 days	✔	



Matrix: **Soil/Solid**

Evaluation: ✖ = Holding time exceedance ; ✔ = Within Holding Time

Analyte Group Container / Client Sample ID(s)	Method	Sampling Date	Extraction / Preparation				Analysis			
			Preparation Date	Holding Times		Eval	Analysis Date	Holding Times		Eval
				Rec	Actual			Rec	Actual	
Physical Tests : pH by Meter (1:2 Soil:0.01M CaCl2 Extraction) - As Received										
Glass soil jar/Teflon lined cap BH22-5-3	E108A	16-Sep-2022	22-Sep-2022	----	----		22-Sep-2022	30 days	6 days	✔
Physical Tests : pH by Meter (1:2 Soil:0.01M CaCl2 Extraction) - As Received										
Glass soil jar/Teflon lined cap MW22-6-3	E108A	15-Sep-2022	22-Sep-2022	----	----		22-Sep-2022	30 days	7 days	✔

Legend & Qualifier Definitions

Rec. HT: ALS recommended hold time (see units).



Quality Control Parameter Frequency Compliance

The following report summarizes the frequency of laboratory QC samples analyzed within the analytical batches (QC lots) in which the submitted samples were processed. The actual frequency should be greater than or equal to the expected frequency.

Matrix: **Soil/Solid**

Evaluation: ✖ = QC frequency outside specification; ✔ = QC frequency within specification.

Quality Control Sample Type	Method	QC Lot #	Count		Frequency (%)		
			QC	Regular	Actual	Expected	Evaluation
Analytical Methods							
Laboratory Duplicates (DUP)							
Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)	E396-L	657907	1	4	25.0	4.7	✔
CCME fine/coarse Particle Size Analysis by wet sieve	E178	674236	1	8	12.5	5.0	✔
Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)	E100-L	659587	1	19	5.2	5.0	✔
Moisture Content by Gravimetry	E144	659122	4	70	5.7	5.0	✔
ORP by Electrode	E125	659317	1	3	33.3	5.0	✔
Particle Size Analysis - Hydrometer	E183	663154	1	5	20.0	5.0	✔
Particle Size Analysis - Sieve <2mm	E182	663153	1	5	20.0	5.0	✔
pH by Meter (1:2 Soil:0.01M CaCl2 Extraction) - As Received	E108A	659219	1	20	5.0	5.0	✔
Water Extractable Chloride by IC	E236.Cl	659594	1	3	33.3	5.0	✔
Water Extractable Sulfate by IC	E236.SO4	659593	1	3	33.3	5.0	✔
Laboratory Control Samples (LCS)							
Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)	E396-L	657907	1	4	25.0	4.7	✔
CCME fine/coarse Particle Size Analysis by wet sieve	E178	674236	1	8	12.5	5.0	✔
Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)	E100-L	659587	2	19	10.5	10.0	✔
Moisture Content by Gravimetry	E144	659122	4	70	5.7	5.0	✔
ORP by Electrode	E125	659317	1	3	33.3	5.0	✔
Particle Size Analysis - Hydrometer	E183	663154	1	5	20.0	5.0	✔
Particle Size Analysis - Sieve <2mm	E182	663153	1	5	20.0	5.0	✔
Particle Size Analysis - Sieve >2mm	E181	663152	1	5	20.0	5.0	✔
pH by Meter (1:2 Soil:0.01M CaCl2 Extraction) - As Received	E108A	659219	1	20	5.0	5.0	✔
Water Extractable Chloride by IC	E236.Cl	659594	2	3	66.6	10.0	✔
Water Extractable Sulfate by IC	E236.SO4	659593	2	3	66.6	10.0	✔
Method Blanks (MB)							
Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)	E396-L	657907	1	4	25.0	4.7	✔
Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)	E100-L	659587	1	19	5.2	5.0	✔
Moisture Content by Gravimetry	E144	659122	4	70	5.7	5.0	✔
Water Extractable Chloride by IC	E236.Cl	659594	1	3	33.3	5.0	✔
Water Extractable Sulfate by IC	E236.SO4	659593	1	3	33.3	5.0	✔



Methodology References and Summaries

The analytical methods used by ALS are developed using internationally recognized reference methods (where available), such as those published by US EPA, APHA Standard Methods, ASTM, ISO, Environment Canada, BC MOE, and Ontario MOE. Reference methods may incorporate modifications to improve performance (indicated by "mod").

Analytical Methods	Method / Lab	Matrix	Method Reference	Method Descriptions
Conductivity in Soil (1:2 Soil:Water Extraction) (Low Level)	E100-L Waterloo - Environmental	Soil/Solid	CSSS Ch. 15 (mod)/APHA 2510 (mod)	Conductivity, also known as Electrical Conductivity (EC) or Specific Conductance, is measured by immersion of a conductivity cell with platinum electrodes into a soil sample that has been added in a defined ratio of soil to deionized water, then shaken well and allowed to settle. Conductance is measured in the fluid that is observed in the upper layer.
pH by Meter (1:2 Soil:0.01M CaCl ₂ Extraction) - As Received	E108A Waterloo - Environmental	Soil/Solid	MOEE E3137A	pH is determined by potentiometric measurement with a pH electrode, and is conducted at ambient laboratory temperature (normally 20 ± 5°C) and is carried out in accordance with procedures described in the Analytical Protocol (prescriptive method). A minimum 10g portion of the sample, as received, is extracted with 20mL of 0.01M calcium chloride solution by shaking for at least 30 minutes. The aqueous layer is separated from the soil by centrifuging, settling, or decanting and then analyzed using a pH meter and electrode.
ORP by Electrode	E125 Waterloo - Environmental	Soil/Solid	APHA 2580 (mod)	Oxidation Reduction Potential (ORP) is reported as the oxidation-reduction potential of the platinum metal-reference electrode employed in the analysis, measured in mV.
Moisture Content by Gravimetry	E144 Waterloo - Environmental	Soil/Solid	CCME PHC in Soil - Tier 1	Moisture is measured gravimetrically by drying the sample at 105°C. Moisture content is calculated as the weight loss (due to water) divided by the wet weight of the sample, expressed as a percentage.
CCME fine/coarse Particle Size Analysis by wet sieve	E178 Saskatoon - Environmental	Soil/Solid	CCME Vol 4 Analytical Methods	An air-dried sample is reduced to < 2 mm size and mixed with a dispersing agent (sodium hexametaphosphate). The sample is washed through a 200 mesh (0.075 mm) sieve. The retained mass of sample is used to determine % sand fraction. If the percentage of sand is >50%, the soil is considered to be coarse textured soil. If the percentage of sand is <50%, the soil is considered to be fine textured.
Particle Size Analysis - Sieve >2mm	E181 Saskatoon - Environmental	Soil/Solid	ASTM D6913-17 (mod)	Soil samples are disaggregated and sieved through a 2mm sieve. Material retained on the sieve is then further sieved through a series of sieves. The amount passing through the sieves is measured gravimetrically.
Particle Size Analysis - Sieve <2mm	E182 Saskatoon - Environmental	Soil/Solid	ASTM D6913-17 (mod)	Soil samples are disaggregated and sieved through a 2mm sieve. Material passed through the sieve is then further disaggregated using calgon solution and passed through a series of sieves. The amount passing through the sieves is measured gravimetrically.
Particle Size Analysis - Hydrometer	E183 Saskatoon - Environmental	Soil/Solid	ASTM D7928-21 (mod)	Soil material is separated from coarse material (>2mm). A specimen is then disaggregated through mixing with Calgon solution. The material is then suspended in solution wherein regular hydrometer readings are taken at specific time intervals. The principles of Stokes' Law are applied to determine the amount of material remaining in solution as well as the maximum particle size remaining in solution at the specified time.



Analytical Methods	Method / Lab	Matrix	Method Reference	Method Descriptions
Grain Size Report (Attachment) Hydrometer/Sieve Method	E185 Saskatoon - Environmental	Soil/Solid	ASTM D6913/D7928	A grain size curve is a graphical representation of the particle sizing of a sample representing the percent passing against the effective particle size.
Water Extractable Chloride by IC	E236.Cl Waterloo - Environmental	Soil/Solid	EPA 300.1	Inorganic anions are analyzed by Ion Chromatography with conductivity and/or UV detection using a soil sample that has been added in a defined ratio of soil to deionized water, then shaken well and allowed to settle. Anions are measured in the fluid that is observed in the upper layer.
Water Extractable Sulfate by IC	E236.SO4 Waterloo - Environmental	Soil/Solid	EPA 300.1	Inorganic anions are analyzed by Ion Chromatography with conductivity and/or UV detection using a soil sample that has been added in a defined ratio of soil to deionized water, then shaken well and allowed to settle. Anions are measured in the fluid that is observed in the upper layer.
Acid Volatile Sulfide in Soil by Colourimetry (0.2 mg/kg)	E396-L Waterloo - Environmental	Soil/Solid	APHA 4500S2J	This analysis is carried out in accordance with the method described in APHA 4500 S2-J. After extraction the Acid Volatile Sulphide is determined colourimetrically.
Resistivity Calculation for Soil Using E100-L	EC100R Waterloo - Environmental	Soil/Solid	APHA 2510 B	Soil Resistivity (calculated) is determined as the inverse of the conductivity of a 2:1 water:soil leachate (dry weight). This method is intended as a rapid approximation for Soil Resistivity. Where high accuracy results are required, direct measurement of Soil Resistivity by the Wenner Four-Electrode Method (ASTM G57) is recommended.

Preparation Methods	Method / Lab	Matrix	Method Reference	Method Descriptions
Leach 1:2 Soil:Water for pH/EC	EP108 Waterloo - Environmental	Soil/Solid	BC WLAP METHOD: PH, ELECTROMETRIC, SOIL	The procedure involves mixing the dried (at <60°C) and sieved (No. 10 / 2mm) sample with deionized/distilled water at a 1:2 ratio of sediment to water.
Leach 1:2 Soil : 0.01CaCl2 - As Received for pH	EP108A Waterloo - Environmental	Soil/Solid	MOEE E3137A	A minimum 10g portion of the sample, as received, is extracted with 20mL of 0.01M calcium chloride solution by shaking for at least 30 minutes. The aqueous layer is separated from the soil by centrifuging, settling or decanting and then analyzed using a pH meter and electrode.
Preparation of ORP by Electrode	EP125 Waterloo - Environmental	Soil/Solid	APHA 2580 (mod)	Field-moist sample is extracted in a 1:2 ratio with DI water and then analyzed by ORP meter.
Anions Leach 1:10 Soil:Water (Dry)	EP236 Waterloo - Environmental	Soil/Solid	EPA 300.1	5 grams of dried soil is mixed with 50 grams of distilled water for a minimum of 30 minutes. The extract is filtered and analyzed by ion chromatography.
Distillation for Acid Volatile Sulfide in Soil	EP396-L Waterloo - Environmental	Soil/Solid	APHA 4500S2J	Acid Volatile Sulfide is determined by colourimetric measurement on a sediment sample that has been treated with hydrochloric acid within a purge and trap system, where the evolved hydrogen sulfide gas is carried into a basic solution by argon gas for analysis.

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Project : 30127480



<i>Preparation Methods</i>	<i>Method / Lab</i>	<i>Matrix</i>	<i>Method Reference</i>	<i>Method Descriptions</i>
Dry and Grind in Soil/Solid <60°C	EPP442 Waterloo - Environmental	Soil/Solid	Soil Sampling and Methods of Analysis, Carter 2008	After removal of any coarse fragments and reservation of wet subsamples a portion of homogenized sample is set in a tray and dried at less than 60°C until dry. The sample is then particle size reduced with an automated crusher or mortar and pestle, typically to <2 mm. Further size reduction may be needed for particular tests.

QUALITY CONTROL REPORT

Work Order	: WT2214822	Page	: 1 of 7
Amendment	: 1		
Client	: Arcadis Canada Inc.	Laboratory	: Waterloo - Environmental
Contact	: Lennart DeGroot	Account Manager	: Emily Smith
Address	: 1050 Morrison Drive Suite 201 Ottawa ON Canada K2H 1L1	Address	: 60 Northland Road, Unit 1 Waterloo, Ontario Canada N2V 2B8
Telephone	:	Telephone	: +1 519 886 6910
Project	: 30127480	Date Samples Received	: 19-Sep-2022 14:55
PO	: ----	Date Analysis Commenced	: 21-Sep-2022
C-O-C number	: ----	Issue Date	: 31-Oct-2022 10:15
Sampler	: ---- 613 721 0555		
Site	: ----		
Quote number	: Waterloo 2022 Price List		
No. of samples received	: 42		
No. of samples analysed	: 42		

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Quality Control Report contains the following information:

- Laboratory Duplicate (DUP) Report; Relative Percent Difference (RPD) and Data Quality Objectives
- Reference Material (RM) Report; Recovery and Data Quality Objectives
- Method Blank (MB) Report; Recovery and Data Quality Objectives
- Laboratory Control Sample (LCS) Report; Recovery and Data Quality Objectives

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is conducted in accordance with US FDA 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Laboratory Department</i>
Amanda Ganouri-Lumsden	Department Manager - Microbiology and Prep	Waterloo Centralized Prep, Waterloo, Ontario
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Client : Arcadis Canada Inc.
Project : 30127480



General Comments

The ALS Quality Control (QC) report is optionally provided to ALS clients upon request. ALS test methods include comprehensive QC checks with every analysis to ensure our high standards of quality are met. Each QC result has a known or expected target value, which is compared against predetermined Data Quality Objectives (DQOs) to provide confidence in the accuracy of associated test results. This report contains detailed results for all QC results applicable to this sample submission. Please refer to the ALS Quality Control Interpretation report (QCI) for applicable method references and methodology summaries.

Key :

Anonymous = Refers to samples which are not part of this work order, but which formed part of the QC process lot.

CAS Number = Chemical Abstracts Service number is a unique identifier assigned to discrete substances.

DQO = Data Quality Objective.

LOR = Limit of Reporting (detection limit).

RPD = Relative Percent Difference

= Indicates a QC result that did not meet the ALS DQO.

Workorder Comments

Holding times are displayed as "---" if no guidance exists from CCME, Canadian provinces, or broadly recognized international references.



Laboratory Duplicate (DUP) Report

A Laboratory Duplicate (DUP) is a randomly selected intralaboratory replicate sample. Laboratory Duplicates provide information regarding method precision and sample heterogeneity. ALS DQOs for Laboratory Duplicates are expressed as test-specific limits for Relative Percent Difference (RPD), or as an absolute difference limit of 2 times the LOR for low concentration duplicates within ~ 4-10 times the LOR (cut-off is test-specific).

Sub-Matrix: Soil/Solid

					Laboratory Duplicate (DUP) Report						
Laboratory sample ID	Client sample ID	Analyte	CAS Number	Method	LOR	Unit	Original Result	Duplicate Result	RPD(%) or Difference	Duplicate Limits	Qualifier
Physical Tests (QC Lot: 659122)											
WT2214822-001	MW22-2-1	moisture	----	E144	0.25	%	8.08	7.88	2.55%	20%	----
Physical Tests (QC Lot: 659123)											
WT2214822-010	BH22-8-1	moisture	----	E144	0.25	%	8.06	8.07	0.109%	20%	----
Physical Tests (QC Lot: 659219)											
TY2201744-003	Anonymous	pH (1:2 soil:CaCl2-aq)	----	E108A	0.10	pH units	7.56	7.69	1.70%	5%	----
Physical Tests (QC Lot: 659220)											
WT2214822-032	MW22-6-4	moisture	----	E144	0.25	%	10.6	10.2	3.74%	20%	----
Physical Tests (QC Lot: 659317)											
WT2214822-007	BH22-3-2	oxidation-reduction potential [ORP]	----	E125	0.10	mV	486	472	2.92%	25%	----
Physical Tests (QC Lot: 659587)											
WT2214860-003	Anonymous	conductivity (1:2 leachate)	----	E100-L	5.00	µS/cm	0.492 mS/cm	510	3.59%	20%	----
Physical Tests (QC Lot: 660485)											
WT2214804-001	Anonymous	moisture	----	E144	0.25	%	78.4	78.1	0.386%	20%	----
Particle Size (QC Lot: 663153)											
WT2214849-001	Anonymous	passing (0.05 mm)	----	E182	1.0	%	84.1	83.3	0.977%	15%	----
		passing (0.063 mm)	----	E182	1.0	%	90.4	90.1	0.353%	15%	----
		passing (0.075 mm)	----	E182	1.0	%	96.3	96.4	0.146%	15%	----
		passing (0.125 mm)	----	E182	1.0	%	96.9	97.0	0.137%	15%	----
		passing (0.149 mm)	----	E182	1.0	%	97.2	97.3	0.133%	15%	----
		passing (0.250 mm)	----	E182	1.0	%	97.6	97.7	0.0742%	15%	----
		passing (0.420 mm)	----	E182	1.0	%	98.0	98.0	0.0182%	15%	----
		passing (0.50 mm)	----	E182	1.0	%	98.1	98.1	0.00625%	15%	----
		passing (0.841 mm)	----	E182	1.0	%	98.5	98.4	0.0404%	15%	----
passing (1.0 mm)	----	E182	1.0	%	98.5	98.5	0.0361%	15%	----		
Particle Size (QC Lot: 663154)											
WT2214849-001	Anonymous	passing (0.002 mm)	----	E183	1.0	%	27.4	28.4	3.70%	20%	----
		passing (0.004 mm)	----	E183	1.0	%	45.0	47.2	4.84%	20%	----
		passing (0.005 mm)	----	E183	1.0	%	51.9	53.8	3.66%	20%	----
		passing (0.020 mm)	----	E183	1.0	%	75.0	74.1	1.19%	20%	----
		passing (0.0312 mm)	----	E183	1.0	%	77.6	76.6	1.35%	20%	----



Sub-Matrix: Soil/Solid					Laboratory Duplicate (DUP) Report						
Laboratory sample ID	Client sample ID	Analyte	CAS Number	Method	LOR	Unit	Original Result	Duplicate Result	RPD(%) or Difference	Duplicate Limits	Qualifier
Particle Size (QC Lot: 674236)											
SK2205375-027	Anonymous	sand (>0.075mm)	----	E178	1.0	%	<1.0	<1.0	0	Diff <2x LOR	----
Inorganic Parameters (QC Lot: 657907)											
WT2214267-003	Anonymous	sulfides, acid volatile	----	E396-L	0.20	mg/kg	0.24	0.33	0.09	Diff <2x LOR	----
Leachable Anions & Nutrients (QC Lot: 659593)											
WT2214822-007	BH22-3-2	sulfate, soluble ion content	14808-79-8	E236.SO4	20	mg/kg	<20	<20	0	Diff <2x LOR	----
Leachable Anions & Nutrients (QC Lot: 659594)											
WT2214822-007	BH22-3-2	chloride, soluble ion content	16887-00-6	E236.Cl	5.0	mg/kg	29.8	28.8	1.0	Diff <2x LOR	----

Method Blank (MB) Report

A Method Blank is an analyte-free matrix that undergoes sample processing identical to that carried out for test samples. Method Blank results are used to monitor and control for potential contamination from the laboratory environment and reagents. For most tests, the DQO for Method Blanks is for the result to be < LOR.

Sub-Matrix: Soil/Solid

Analyte	CAS Number	Method	LOR	Unit	Result	Qualifier
Physical Tests (QCLot: 659122)						
moisture	----	E144	0.25	%	<0.25	----
Physical Tests (QCLot: 659123)						
moisture	----	E144	0.25	%	<0.25	----
Physical Tests (QCLot: 659220)						
moisture	----	E144	0.25	%	<0.25	----
Physical Tests (QCLot: 659587)						
conductivity (1:2 leachate)	----	E100-L	5	µS/cm	<5.00	----
Physical Tests (QCLot: 660485)						
moisture	----	E144	0.25	%	<0.25	----
Inorganic Parameters (QCLot: 657907)						
sulfides, acid volatile	----	E396-L	0.2	mg/kg	<0.20	----
Leachable Anions & Nutrients (QCLot: 659593)						
sulfate, soluble ion content	14808-79-8	E236.SO4	20	mg/kg	<20	----
Leachable Anions & Nutrients (QCLot: 659594)						
chloride, soluble ion content	16887-00-6	E236.Cl	5	mg/kg	<5.0	----



Laboratory Control Sample (LCS) Report

A Laboratory Control Sample (LCS) is an analyte-free matrix that has been fortified (spiked) with test analytes at known concentration and processed in an identical manner to test samples. LCS results are expressed as percent recovery, and are used to monitor and control test method accuracy and precision, independent of test sample matrix.

Sub-Matrix: Soil/Solid

					Laboratory Control Sample (LCS) Report				
					Spike	Recovery (%)	Recovery Limits (%)		
Analyte	CAS Number	Method	LOR	Unit	Concentration	LCS	Low	High	Qualifier
Physical Tests (QCLot: 659122)									
moisture	----	E144	0.25	%	50 %	100	90.0	110	----
Physical Tests (QCLot: 659123)									
moisture	----	E144	0.25	%	50 %	100	90.0	110	----
Physical Tests (QCLot: 659219)									
pH (1:2 soil:CaCl2-aq)	----	E108A	----	pH units	7 pH units	101	98.0	102	----
Physical Tests (QCLot: 659220)									
moisture	----	E144	0.25	%	50 %	101	90.0	110	----
Physical Tests (QCLot: 659587)									
conductivity (1:2 leachate)	----	E100-L	5	µS/cm	1409 µS/cm	105	90.0	110	----
Physical Tests (QCLot: 660485)									
moisture	----	E144	0.25	%	50 %	101	90.0	110	----
Inorganic Parameters (QCLot: 657907)									
sulfides, acid volatile	----	E396-L	0.2	mg/kg	2.536 mg/kg	99.4	70.0	130	----
Leachable Anions & Nutrients (QCLot: 659593)									
sulfate, soluble ion content	14808-79-8	E236.SO4	20	mg/kg	5000 mg/kg	102	70.0	130	----
Leachable Anions & Nutrients (QCLot: 659594)									
chloride, soluble ion content	16887-00-6	E236.Cl	5	mg/kg	5000 mg/kg	101	80.0	120	----



Reference Material (RM) Report

A Reference Material (RM) is a homogenous material with known and well-established analyte concentrations. RMs are processed in an identical manner to test samples, and are used to monitor and control the accuracy and precision of a test method for a typical sample matrix. RM results are expressed as percent recovery of the target analyte concentration. RM targets may be certified target concentrations provided by the RM supplier, or may be ALS long-term mean values (for empirical test methods).

Sub-Matrix:

Laboratory sample ID	Reference Material ID	Analyte	CAS Number	Method	Reference Material (RM) Report				
					RM Target Concentration	Recovery (%) RM	Recovery Limits (%)		Qualifier
							Low	High	
Physical Tests (QCLot: 659317)									
	RM	oxidation-reduction potential [ORP]	----	E125	475 mV	102	80.0	120	----
Physical Tests (QCLot: 659587)									
	RM	conductivity (1:2 leachate)	----	E100-L	1031.5 µS/cm	111	70.0	130	----
Particle Size (QCLot: 663152)									
	RM	passing (19 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (2.0 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (25.4 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (38.1 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (4.75 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (50.8 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (76.2 mm)	----	E181	100 %	100	90.0	110	----
	RM	passing (9.5 mm)	----	E181	100 %	100	90.0	110	----
Particle Size (QCLot: 663153)									
	RM	passing (0.05 mm)	----	E182	49.81 %	99.2	90.0	110	----
	RM	passing (0.063 mm)	----	E182	54.27 %	98.6	90.8	109	----
	RM	passing (0.075 mm)	----	E182	58.38 %	98.2	91.4	109	----
	RM	passing (0.125 mm)	----	E182	68.06 %	99.3	92.7	107	----
	RM	passing (0.149 mm)	----	E182	72.71 %	99.7	93.1	107	----
	RM	passing (0.250 mm)	----	E182	85.38 %	99.2	94.1	106	----
	RM	passing (0.420 mm)	----	E182	92.78 %	99.7	94.6	105	----
	RM	passing (0.50 mm)	----	E182	93.78 %	99.7	94.7	105	----
	RM	passing (0.841 mm)	----	E182	97.34 %	99.8	94.9	105	----
	RM	passing (1.0 mm)	----	E182	97.77 %	99.8	94.9	105	----
Particle Size (QCLot: 663154)									
	RM	passing (0.002 mm)	----	E183	21.14 %	89.0	76.0	124	----
	RM	passing (0.004 mm)	----	E183	24.64 %	93.7	80.0	120	----
	RM	passing (0.005 mm)	----	E183	25.91 %	96.3	82.0	118	----
	RM	passing (0.020 mm)	----	E183	37.12 %	96.9	87.0	113	----



Sub-Matrix:

Laboratory sample ID	Reference Material ID	Analyte	CAS Number	Method	Reference Material (RM) Report				
					RM Target Concentration	Recovery (%) RM	Recovery Limits (%)		Qualifier
							Low	High	
Particle Size (QCLot: 663154) - continued									
	RM	passing (0.0312 mm)	----	E183	42.58 %	98.7	88.0	112	----
Particle Size (QCLot: 674236)									
	RM	sand (>0.075mm)	----	E178	42.85 %	93.6	88.0	112	----
Leachable Anions & Nutrients (QCLot: 659593)									
	RM	sulfate, soluble ion content	14808-79-8	E236.SO4	217 mg/kg	107	60.0	140	----
Leachable Anions & Nutrients (QCLot: 659594)									
	RM	chloride, soluble ion content	16887-00-6	E236.Cl	673 mg/kg	99.4	70.0	130	----

ALS Laboratory Group

819-58th Street, Saskatoon, SK

Client Name: WT2214822007

Project Number:

Client Sample ID BH22-3-2

Lab Sample ID WT2214822007

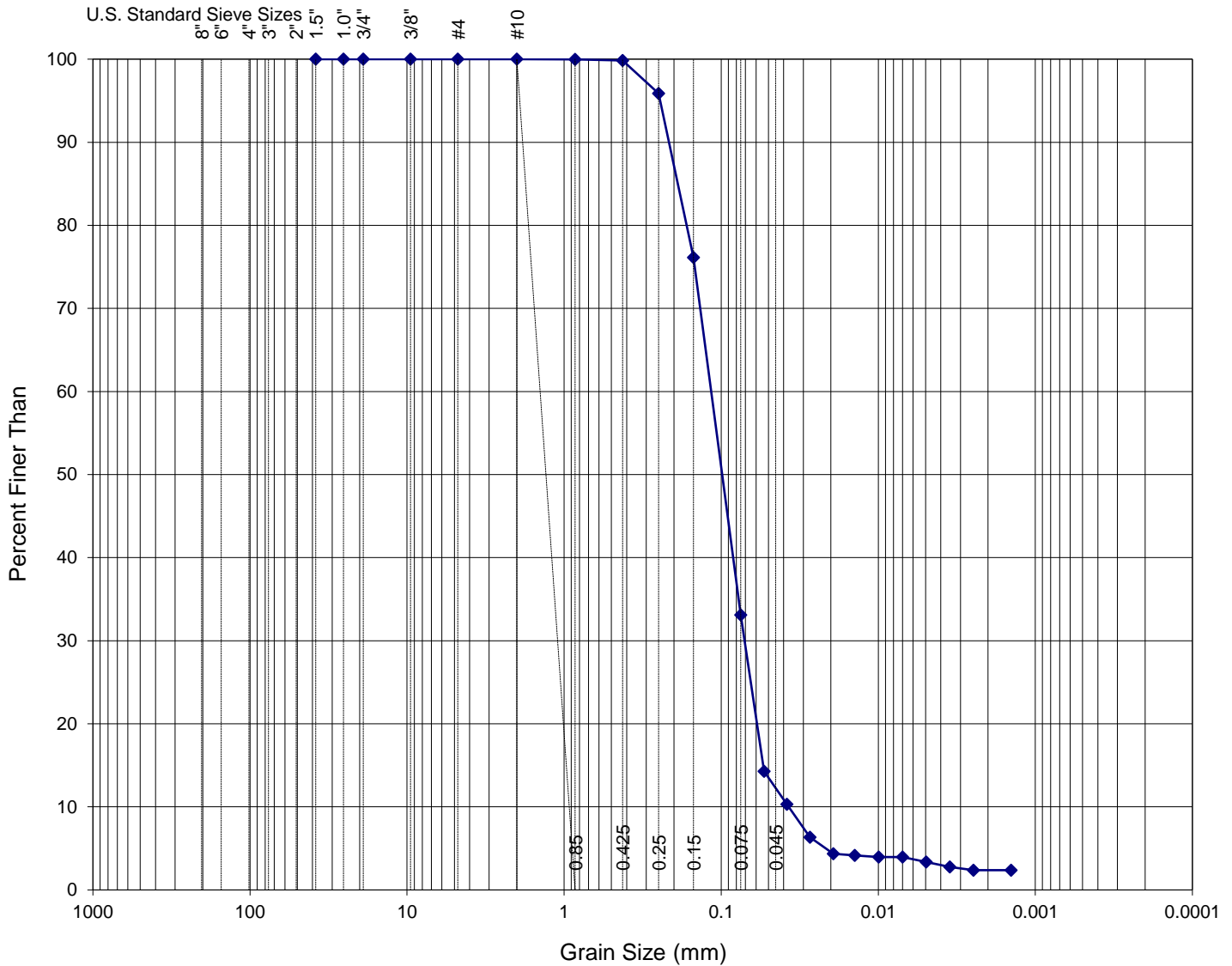
Date Sample Received 00-Jan-00

Test Completion Date: 28-Sep-22

Analyst: SIH

PARTICLE SIZE DISTRIBUTION CURVE

BOULDERS	COBBLES	GRAVEL		SAND SIZES			SILT	CLAY
		COARSE	FINE	COARSE	MEDIUM	FINE		



METHOD DESCRIPTION

Method Reference: ASTM D6913 & D7928

Dispersion method: Mechanical

Dispersion period: 1 minute cm/s

DESCRIPTION OF SAND AND GRAVEL PARTICLES

Shape: Angular

Hardness: Hard

SUMMARY OF RESULTS

GRAIN SIZE	WT %	DIA. RANGE (mm)
% GRAVEL :	<1	> 4.75
% COARSE SAND :	<1	2.0 - 4.75
% MEDIUM SAND :	<1	0.425 - 2.0
% FINE SAND :	66.73	0.075 - 0.425
% SILT :	29.71	0.075 - 0.005
% CLAY :	3.39	< 0.005

ALS Laboratory Group

819-58th Street, Saskatoon, SK

PARTICLE SIZE DISTRIBUTION CURVE

Client Name: WT2214822020

Project Number:

Client Sample ID MW22-4-2

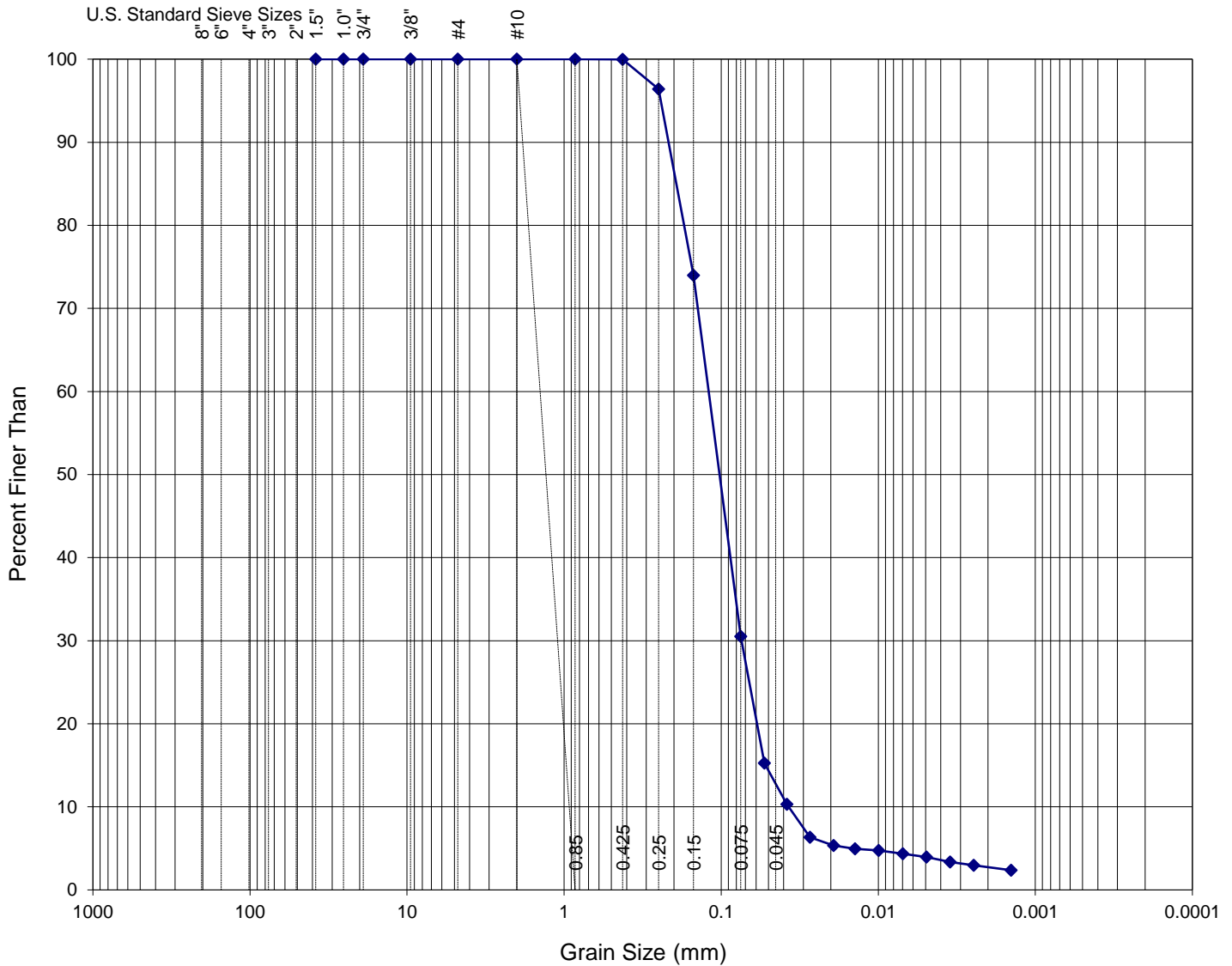
Lab Sample ID WT2214822020

Date Sample Received 00-Jan-00

Test Completion Date: 28-Sep-22

Analyst: SIH

BOULDERS	COBBLES	GRAVEL		SAND SIZES			SILT	CLAY
		COARSE	FINE	COARSE	MEDIUM	FINE		



METHOD DESCRIPTION

Method Reference: ASTM D6913 & D7928

Dispersion method: Mechanical

Dispersion period: 1 minute cm/s

DESCRIPTION OF SAND AND GRAVEL PARTICLES

Shape: Angular

Hardness: Hard

SUMMARY OF RESULTS

GRAIN SIZE	WT %	DIA. RANGE (mm)
% GRAVEL :	<1	> 4.75
% COARSE SAND :	<1	2.0 - 4.75
% MEDIUM SAND :	<1	0.425 - 2.0
% FINE SAND :	69.44	0.075 - 0.425
% SILT :	26.55	0.075 - 0.005
% CLAY :	3.98	< 0.005

ALS Laboratory Group

819-58th Street, Saskatoon, SK

PARTICLE SIZE DISTRIBUTION CURVE

Client Name: WT2214822025

Project Number:

Client Sample ID BH22-5-2

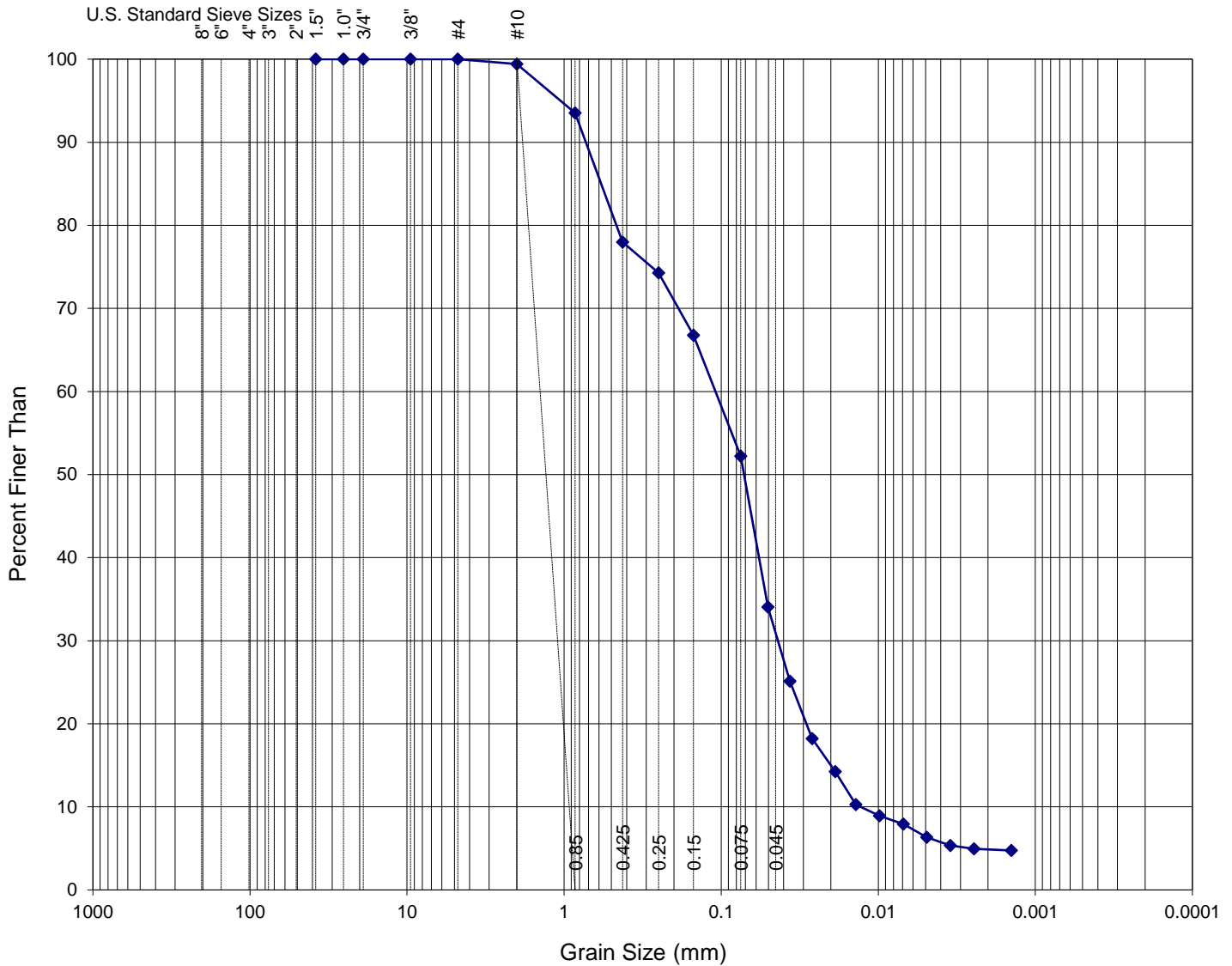
Lab Sample ID WT2214822025

Date Sample Received 00-Jan-00

Test Completion Date: 28-Sep-22

Analyst: SIH

BOULDERS	COBBLES	GRAVEL		SAND SIZES			SILT	CLAY
		COARSE	FINE	COARSE	MEDIUM	FINE		



METHOD DESCRIPTION

Method Reference: ASTM D6913 & D7928

Dispersion method: Mechanical

Dispersion period: 1 minute cm/s

DESCRIPTION OF SAND AND GRAVEL PARTICLES

Shape: Angular

Hardness: Hard

SUMMARY OF RESULTS

GRAIN SIZE	WT %	DIA. RANGE (mm)
% GRAVEL :	<1	> 4.75
% COARSE SAND :	<1	2.0 - 4.75
% MEDIUM SAND :	21.40	0.425 - 2.0
% FINE SAND :	25.78	0.075 - 0.425
% SILT :	45.80	0.075 - 0.005
% CLAY :	6.42	< 0.005

ALS Laboratory Group

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Client Name: WT2214822030

Project Number:

Client Sample ID MW22-6-2

Lab Sample ID WT2214822030

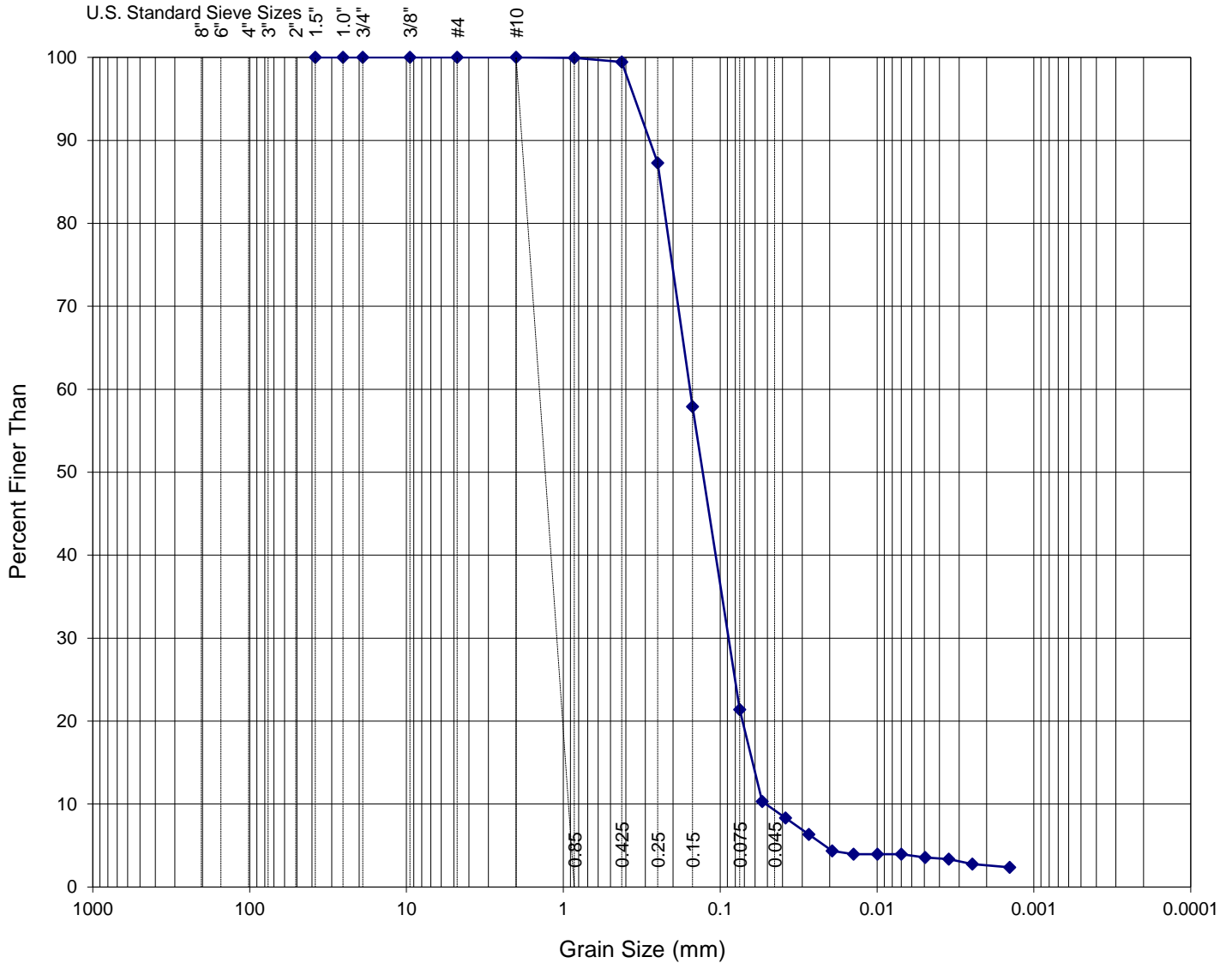
Date Sample Received 00-Jan-00

Test Completion Date: 28-Sep-22

Analyst: SIH

PARTICLE SIZE DISTRIBUTION CURVE

BOULDERS	COBBLES	GRAVEL		SAND SIZES			SILT	CLAY
		COARSE	FINE	COARSE	MEDIUM	FINE		



METHOD DESCRIPTION

Method Reference: ASTM D6913 & D7928

Dispersion method: Mechanical

Dispersion period: 1 minute cm/s

SUMMARY OF RESULTS

GRAIN SIZE	WT %	DIA. RANGE (mm)
% GRAVEL :	<1	> 4.75
% COARSE SAND :	<1	2.0 - 4.75
% MEDIUM SAND :	<1	0.425 - 2.0
% FINE SAND :	78.05	0.075 - 0.425
% SILT :	17.81	0.075 - 0.005
% CLAY :	3.58	< 0.005

DESCRIPTION OF SAND AND GRAVEL PARTICLES

Shape: Angular

Hardness: Hard

ALS Laboratory Group

819-58th Street, Saskatoon, SK

PARTICLE SIZE DISTRIBUTION CURVE

Client Name: WT2214822038

Project Number:

Client Sample ID BH22-7-5

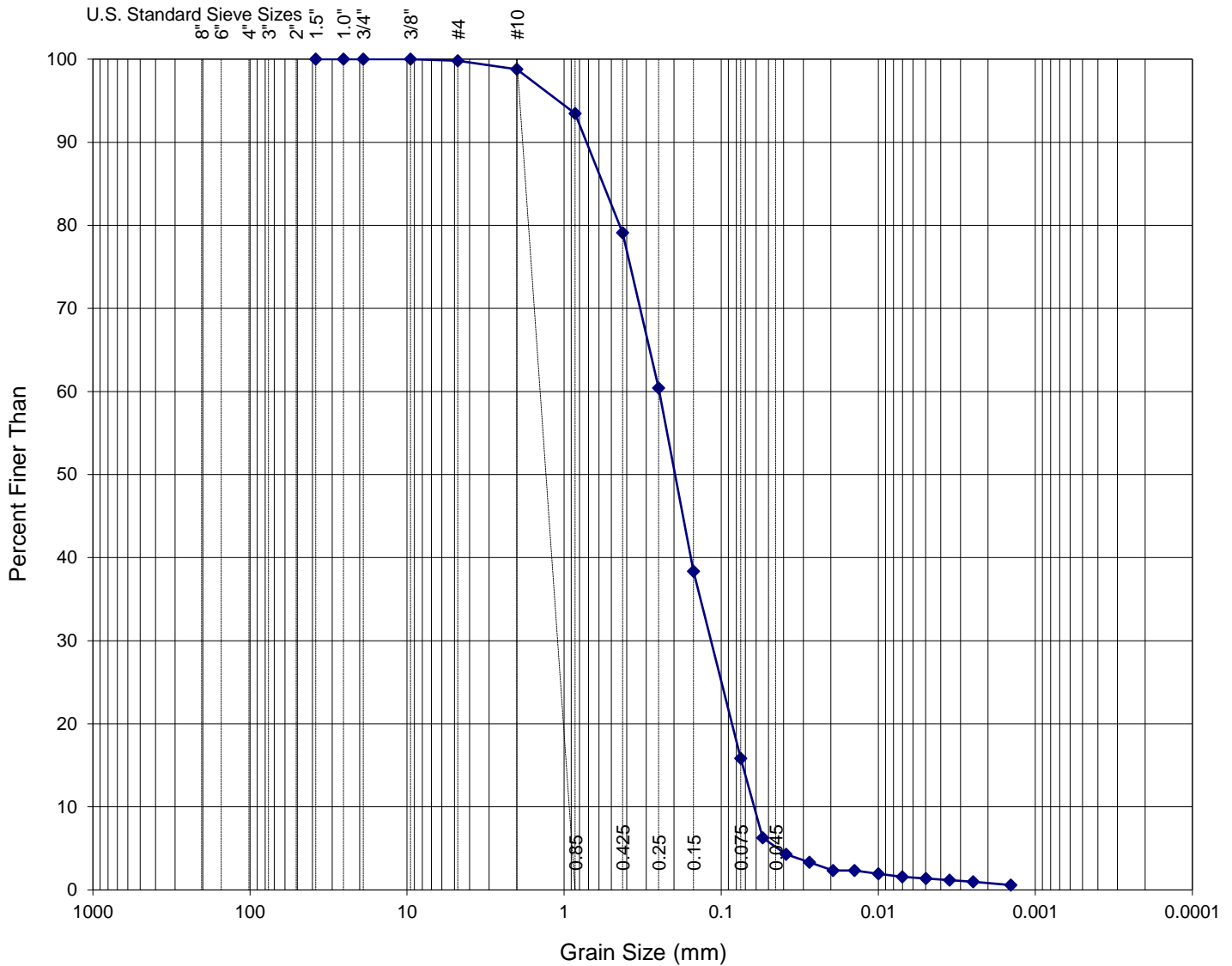
Lab Sample ID WT2214822038

Date Sample Received 00-Jan-00

Test Completion Date: 28-Sep-22

Analyst: SIH

BOULDERS	COBBLES	GRAVEL		SAND SIZES			SILT	CLAY
		COARSE	FINE	COARSE	MEDIUM	FINE		



METHOD DESCRIPTION

Method Reference: ASTM D6913 & D7928

Dispersion method: Mechanical

Dispersion period: 1 minute cm/s

DESCRIPTION OF SAND AND GRAVEL PARTICLES

Shape: Angular

Hardness: Hard

SUMMARY OF RESULTS

GRAIN SIZE	WT %	DIA. RANGE (mm)
% GRAVEL :	<1	> 4.75
% COARSE SAND :	1.02	2.0 - 4.75
% MEDIUM SAND :	19.69	0.425 - 2.0
% FINE SAND :	63.24	0.075 - 0.425
% SILT :	14.47	0.075 - 0.005
% CLAY :	1.37	< 0.005

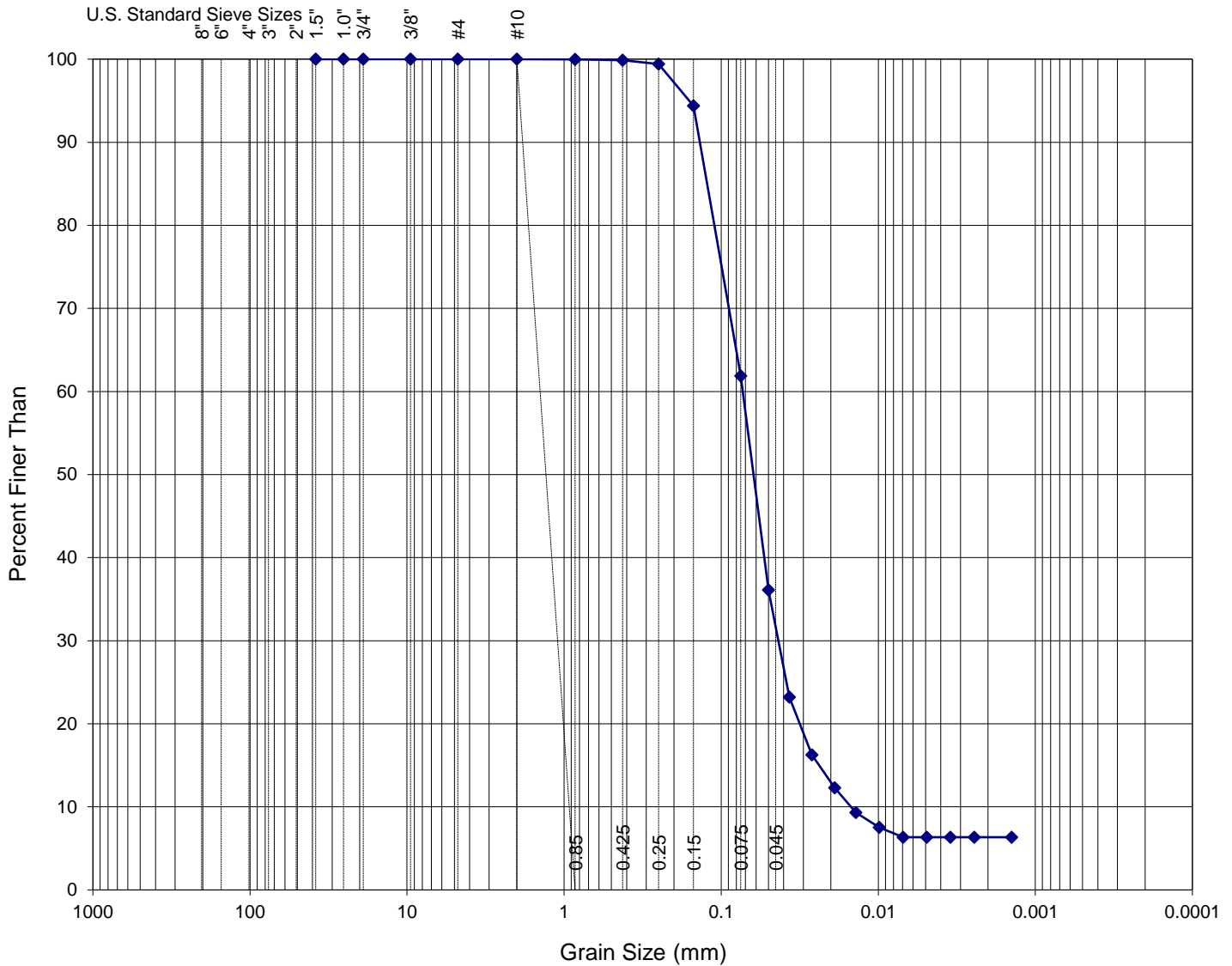
ALS Laboratory Group

819-58th Street, Saskatoon, SK

PARTICLE SIZE DISTRIBUTION CURVE

Client Name: WT2214822040
 Project Number:
 Client Sample ID BH22-10-2
 Lab Sample ID WT2214822040
 Date Sample Received 00-Jan-00
 Test Completion Date: 28-Sep-22
 Analyst: SIH

BOULDERS	COBBLES	GRAVEL		SAND SIZES			SILT	CLAY
		COARSE	FINE	COARSE	MEDIUM	FINE		



METHOD DESCRIPTION

Method Reference: ASTM D6913 & D7928

Dispersion method: Mechanical

Dispersion period: 1 minute cm/s

DESCRIPTION OF SAND AND GRAVEL PARTICLES

Shape: Angular

Hardness: Hard

SUMMARY OF RESULTS

GRAIN SIZE	WT %	DIA. RANGE (mm)
% GRAVEL :	<1	> 4.75
% COARSE SAND :	<1	2.0 - 4.75
% MEDIUM SAND :	<1	0.425 - 2.0
% FINE SAND :	38.00	0.075 - 0.425
% SILT :	55.52	0.075 - 0.005
% CLAY :	6.35	< 0.005

Chain of Custody (COC) / Analytical Request Form

COC Number: 17 - Page 1 of 5

Affix ALS barcode label here (lab use only)

Canada Toll Free: 1 800 668 9878



Report To Contact and company name below will appear on the final report. Company: ARCADIS Canada Inc. Contact: Lennart de Groot Phone: 613-809-2379 Company address below will appear on the final report. Street: 1050 Morrison Drive City/Province: Ottawa, ON Postal Code: K2H 8K7		Report Format / Distribution Select Report Format: <input checked="" type="checkbox"/> PDF <input checked="" type="checkbox"/> EXCEL <input checked="" type="checkbox"/> EDD (DIGITAL) Quality Control (QC) Report with Report <input type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> Compare Results to Criteria on Report - provide details below, if box checked Select Distribution: <input checked="" type="checkbox"/> EMAIL <input type="checkbox"/> MAIL <input type="checkbox"/> FAX Email 1 or Fax: Lennart.deGroot@arcadis.com Email 2 Email 3	
Invoice To: Same as Report To <input type="checkbox"/> YES <input type="checkbox"/> NO Copy of Invoice with Report <input type="checkbox"/> YES <input type="checkbox"/> NO Company: AccountsPayable.canada@arcadis.com Contact: AccountsPayable.canada@arcadis.com Project Information ALS Account # / Quote #: Q88485 (2022 SOA) Job #: 30127480 PO / AFE: LSD:		Invoice Distribution Select Invoice Distribution: <input checked="" type="checkbox"/> EMAIL <input type="checkbox"/> MAIL <input type="checkbox"/> FAX Email 1 or Fax: Lennart.deGroot@arcadis.com Email 2: AccountsPayable.canada@arcadis.com Oil and Gas Required Fields (client use) AFE/Coast Center: PO# Major/Minor Code: Routing Code: Requisitioner: Location:	
ALS Lab Work Order # (lab use only): Sample Identification and/or Coordinates (This description will appear on the report)		ALS Contact: Emily Smith Sampler:	
MW22-2-1 MW22-2-2 MW22-2-3 MW22-2-4 MW22-2-5 BH22-3-1 BH22-3-2 BH22-3-3 BH22-3-4	Date (dd-mm-yy) 16-09-2022 16-09-2022 16-09-2022 16-09-2022 16-09-2022 16-09-2022 16-09-2022 16-09-2022	Time (hh:mm) 9:00 9:00 9:00 9:00 9:00 12:00 12:00 12:00 12:00	Sample Type Soil Soil Soil Soil Soil Soil Soil Soil Soil
Drinking Water (DW) Samples (client use) <input type="checkbox"/> YES <input type="checkbox"/> NO Are samples taken from a Regulated DW System? <input type="checkbox"/> YES <input type="checkbox"/> NO		Special Instructions / Specify Criteria to add on report by clicking on the drop-down list below (electronic COC only)	
SHIPMENT RELEASE (client use) Released by: Lennart de Groot Date: 19-09-2022		INITIAL SHIPMENT RECEPTION (lab use only) Received by: <i>Colin F</i> Date: 9/19/22 WHITE - LABORATORY COPY YELLOW - CLIENT COPY	
REFERENCE TO BACK PAGE FOR ALS LOCATIONS AND SAMPLING INFORMATION Failure to complete all portions of this form may delay analysis. Please fill in this form LEGIBLY. By the use of this form the user acknowledges and agrees with the Terms and Conditions as specified on the back page of the white - report copy. 1. If any water samples are taken from a Regulated Drinking Water (DW) System, please submit using an Authorized DW COC form.		FINAL SHIPMENT RECEPTION (lab use only) Received by: <i>AS</i> Date: 20 Sep 22 Time: 14:55 Time: 14:55	

SUSPECTED HAZARD (see Special Instructions)

SAMPLES ON HOLD

Environmental Division
 Waterloo
 Work Order Reference
WT2214822

Telephone: +1 519 886 8910

Analysis Request

Indicate Filtered (F), Preserved (P) or Filled and Preserved (FP) below

Moisture content	Hydrometer Analysis	Full Grain Size Plot	Corrosivity

NUMBER OF CONTAINERS

1	R			
1	R			
1	R			
1	R			
1	R			
1	R			
3	R	R	R	
1	R			
1	R			

Sample Condition as Received (lab use only)

Frozen SIF Observations Yes No
 Ice Packs Ice Cubes Custody seal intact Yes No
 Cooling Initiated

INITIAL COOLER TEMPERATURES °C: 14.9
 FINAL COOLER TEMPERATURES °C: 10.5

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NOV 2019 FRONT

Chain of Custody (COC) / Analytical Request Form

COC Number: 17 -

Page 2 of 5

Affix ALS barcode label here (lab use only)

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Report To Contact and company name below will appear on the final report

Company: ARCADIS Cañada Inc.
 Contact: Lennart de Groot
 Phone: 613-809-2379
 Street: 1050 Morrison Drive
 City/Province: Ottawa, ON
 Postal Code: K2H 8K7

Report Format / Distribution
 Select Report Format: PDF EXCEL EDD (DIGITAL)
 Quality Control (QC) Report with Report YES NO
 Compare Results to Criteria on Report - provide details below if box checked
 Select Distribution: EMAIL MAIL FAX
 Email 1 or Fax: Lennart.deGroot@arcadis.com
 Email 2
 Email 3

Invoice To Same as Report To YES NO
 Copy of invoice with Report YES NO
 Accounts Payable: canada@arcadis.com
Project Information
 ALS Account # / Quote #: Q88485 (2022 SOA)
 Job #: 30127480
 PO / AFE:
 LSD:

Invoice Distribution
 Select Invoice Distribution: EMAIL MAIL FAX
 Email 1 or Fax: Lennart.deGroot@arcadis.com
 Email 2: AccountsPayable.canada@arcadis.com
Oil and Gas Required Fields (client use)
 AFE/Coat Center: PO#
 Major/Minor Code: Routing Code:
 Requisitioner:
 Location:

ALS Lab Work Order # (lab use only):	ALS Contact:	Emily Smith	Sampler:	Date (dd-mm-yy)	Time (hh:mm)	Sample Type
BH22-8-1				16-09-2022	14:00	Soil
BH22-8-2				16-09-2022	14:00	Soil
BH22-8-3				16-09-2022	14:00	Soil
BH22-8-4				16-09-2022	14:00	Soil
BH22-9-1				16-09-2022	13:00	Soil
BH22-9-2				16-09-2022	13:00	Soil
BH22-9-3				16-09-2022	13:00	Soil
BH22-9-4				16-09-2022	13:00	Soil
BH22-9-5				16-09-2022	13:00	Soil

Special Instructions / Specify Criteria to add on report by clicking on the drop-down list below (electronic COC only)

Drinking Water (DW) Samples (client use)
 YES NO
 Are samples taken from a Regulated DW System?
 YES NO
 Are samples for human consumption/ use?
 YES NO

SHIPMENT RELEASE (client use)
 Released by: Lennart de Groot
 Date: 19-09-2022

INITIAL SHIPMENT RECEPTION (lab use only)
 Received by: Guita F
 Date: 9/19/22
 Time: 14:55

FINAL SHIPMENT RECEPTION (lab use only)
 Received by: AP
 Date: 20-SEP-22
 Time: 10:00

NUMBER OF CONTAINERS

Moisture content	Hydrometer Analysis	Full Grain Size Plot	Corrosivity
1 R			
1 R			
1 R			
1 R			
1 R			
1 R			
1 R			
1 R			
1 R			
1 R			
1 R			

ANALYSIS REQUEST
 Indicate Filtered (F), Preserved (P) or Filtered and Preserved (FIP) below

SAMPLES ON HOLD
 SUSPECTED HAZARD (see Special Instructions)

SELECT SERVICE LEVEL BELOW - CONTACT YOUR AM TO CONFIRM ALL E&P TAT'S (SURCHARGES MAY APPLY)
 Regular [R] Standard TAT if received by 3 pm - business days - no surcharges apply
 4 day [P4-20%]
 3 day [P3-25%]
 2 day [P2-50%]
 1 Business day [E - 100%]
 Same Day, Weekend or Statutory holiday [E2 - 200%]
 (Laboratory opening fees may apply)

Date and Time Required for all E&P TAT's: dd-mm-yy hh:mm
 For tests that can not be performed according to the service level selected, you will be contacted.

EMERGENCY
 4 day [P4-20%]
 3 day [P3-25%]
 2 day [P2-50%]
 1 Business day [E - 100%]
 Same Day, Weekend or Statutory holiday [E2 - 200%]
 (Laboratory opening fees may apply)

SAMPLE CONDITION AS RECEIVED (lab use only)
 Frozen SIF Observations Yes No
 Ice Packs Ice Cubes Custody seal intact Yes No
 Cooling initiated

INITIAL COOLER TEMPERATURES °C: 14.9
 FINAL COOLER TEMPERATURES °C: 10.2

REFER TO BACK PAGE FOR ALS LOCATIONS AND SAMPLING INFORMATION
 Failure to complete all portions of this form may delay analysis. Please fill in this form LEGIBLY. By the use of this form the user acknowledges and agrees with the Terms and Conditions as specified on the back page of the white - report copy.
 1. If any water samples are taken from a Regulated Drinking Water (DW) System, please submit using an Authorized DW COC form.

Chain of Custody (COC) / Analytical Request Form

COC Number: 17 - Page 3 of 5

Affix ALS barcode label here (lab use only)

Canada Toll Free: 1 800 668 9878



Report To Contact and company name below will appear on the final report. Company: ARCADIS Canada Inc. Contact: Lennart de Groot Phone: 613-809-2379 Company address below will appear on the final report. Street: 1050 Morrison Drive City/Province: Ottawa, ON Postal Code: K2H 8K7		Report Format / Distribution Select Report Format: <input checked="" type="checkbox"/> PDF <input checked="" type="checkbox"/> EXCEL <input checked="" type="checkbox"/> EDD (DIGITAL) Quality Control (QC) Report with Report <input type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> Compare Results to Criteria on Report - provide details below if box checked Select Distribution: <input checked="" type="checkbox"/> EMAIL <input type="checkbox"/> MAIL <input type="checkbox"/> FAX Email 1 or Fax: Lennart.deGroot@arcadis.com Email 2 Email 3	
Invoice To: Same as Report To <input type="checkbox"/> YES <input type="checkbox"/> NO Copy of Invoice with Report <input type="checkbox"/> YES <input type="checkbox"/> NO Company: Accounts Payable: canada@arcadis.com Contact: Accounts Payable: canada@arcadis.com Project Information ALS Account # / Quote #: Q88485 (2022 SOA) Job #: 30127480 PO / AFE: LSD:		Invoice Distribution Select Invoice Distribution: <input checked="" type="checkbox"/> EMAIL <input type="checkbox"/> MAIL <input type="checkbox"/> FAX Email 1 or Fax: Lennart.deGroot@arcadis.com Email 2: Accounts Payable: canada@arcadis.com Oil and Gas Required Fields (client use) AFE/Coast Center: PO# Major/Minor Code: Routing Code: Requisitioner: Location:	
ALS Lab Work Order # (lab use only): Sample Identification and/or Coordinates (This description will appear on the report)		ALS Contact: Emily Smith Sampler:	
ALS Sample # (lab use only)	Date (dd-mm-yy)	Time (hh:mm)	Sample Type
MW22-4-1	15-09-2022	15:45	Soil
MW22-4-2	15-09-2022	15:45	Soil
MW22-4-3	15-09-2022	15:45	Soil
MW22-4-4	15-09-2022	15:45	Soil
MW22-4-5	15-09-2022	15:45	Soil
BH22-5-1	16-09-2022	8:50	Soil
BH22-5-2	16-09-2022	8:50	Soil
BH22-5-3	16-09-2022	8:50	Soil
BH22-5-4	16-09-2022	8:50	Soil
BH22-5-5	16-09-2022	8:50	Soil

Special Instructions / Specify Criteria to add on report by clicking on the drop-down list below (electronic COC only)	
Drinking Water (DW) Samples (client use) Are samples taken from a Regulated DW System? <input type="checkbox"/> YES <input type="checkbox"/> NO	SHIPMENT RELEASE (client use) Date: 19-09-2022
Are samples for human consumption/ use? <input type="checkbox"/> YES <input type="checkbox"/> NO	INITIAL SHIPMENT RECEPTION (lab use only) Received by: [Signature] Date: 9/19/22 Time: 14:55

Select Service Level Below - Contact your AM to confirm all E&P TATs (surcharges may apply) Regular [R] <input checked="" type="checkbox"/> Standard TAT if received by 3 pm - business days - no surcharges apply 1 Business day [E - 100%] Same Day, Weekend or Statutory holiday [E2 - 200%] (Laboratory opening fees may apply)	
Date and Time Required for all E&P TATs: dd-mm-yy hh:mm	Analysis Request For tests that can not be performed according to the service level selected, you will be contacted.

NUMBER OF CONTAINERS 1 R 2 R 1 R 1 R 1 R 1 R 2 R 1 R 1 R 1 R	Moisture content Hydrimeter Analysis Fill Grain Size Plot Corrosivity	Indicate Filtered (F), Preserved (P) or Filtered and Preserved (F/P) below	SUSPECTED HAZARD (see Special Instructions)
--	--	--	---

SAMPLE CONDITION AS RECEIVED (lab use only) Frozen <input type="checkbox"/> Ice Packs <input checked="" type="checkbox"/> Ice Cubes <input type="checkbox"/> SIF Observations Yes <input type="checkbox"/> No <input type="checkbox"/> Custody seal intact Yes <input type="checkbox"/> No <input type="checkbox"/> Cooling initiated <input type="checkbox"/>	INITIAL COOLER TEMPERATURES °C 14.9 FINAL COOLER TEMPERATURES °C 10.2
Received by: [Signature] Date: 20 Sept 22 Time: 19:00	FINAL SHIPMENT RECEPTION (lab use only)

REFER TO BACK PAGE FOR ALS LOCATIONS AND SAMPLING INFORMATION
 Failure to complete all portions of this form may delay analysis. Please fill in this form LEGIBLY. By the use of this form the user acknowledges and agrees with the Terms and Conditions as specified on the back page of the white - report copy.
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Chain of Custody (COC) / Analytical Request Form

COC Number: 17 -

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www.alslablab.com

<p>Report To</p> <p>Contact and company name below will appear on the final report</p> <p>Company: ARCADIS Canada Inc. Contact: Lennart de Groot Phone: 613-809-2379 Company address below will appear on the final report Street: 1050 Morrison Drive City/Province: Ottawa, ON Postal Code: K2H 8K7</p>	<p>Report Format / Distribution</p> <p>Select Report Format: <input type="checkbox"/> PDF <input type="checkbox"/> EXCEL <input type="checkbox"/> EDD (DIGITAL) Quality Control (QC) Report with Report <input type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> Compare Results to Criteria on Report - provide details below if box checked Select Distribution: <input type="checkbox"/> EMAIL <input type="checkbox"/> MAIL <input type="checkbox"/> FAX Email 1 or Fax Lennart.deGroot@arcadis.com Email 2 Email 3</p>	<p>Select Service Level Below - Contact your AM to confirm all E&P TATs (surcharges may apply)</p> <p>Regular [R] <input type="checkbox"/> Standard TAT if received by 3 pm - business days - no surcharges apply 4 day [P4-20%] <input type="checkbox"/> 1 Business day [E - 100%] 3 day [P3-25%] <input type="checkbox"/> Same Day, Weekend or Statutory holiday [E2 -200%] 2 day [P2-50%] <input type="checkbox"/> (Laboratory opening fees may apply)]</p> <p>Date and Time Required for all E&P TATs: dd-mm-yy hh:mm For tests that can not be performed according to the service level selected, you will be contacted.</p>	<p>Invoice To</p> <p>Same as Report To <input type="checkbox"/> YES <input type="checkbox"/> NO Copy of Invoice with Report <input type="checkbox"/> YES <input type="checkbox"/> NO Company: AccountPayable.canada@arcadis.com Contact: AccountPayable.canada@arcadis.com</p> <p>Project Information</p> <p>ALS Account # / Quote #: Q88485 (2022 SOA) Job #: 30127480 PO / AFE: LSD:</p>																																																																		
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Sample Condition AS RECEIVED (lab use only) Frozen <input type="checkbox"/> SIF Observations Yes <input type="checkbox"/> No <input type="checkbox"/> Ice Packs <input checked="" type="checkbox"/> Ice Cubes <input type="checkbox"/> Custody seal intact Yes <input type="checkbox"/> No <input type="checkbox"/> Cooling Initiated <input type="checkbox"/>		SUSPECTED HAZARD (see Special Instructions) SAMPLES ON HOLD	

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