

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS 273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



Project No.: CCO-22-1643-01

Prepared for:

Trevor Watkins
1500 Thomas Argue Road
Carp, Ontario
K0A 1L0

Prepared by:

McIntosh Perry Consulting Engineers Ltd.
115 Walgreen Road
Carp, Ontario
K0A 1L0

February 3, 2023

EXECUTIVE SUMMARY

McIntosh Perry ('MP') was retained by Trevor Watkins ('the Client') to conduct a Hydrogeological Assessment and Terrain Analysis in support of a proposed storage facility development for the property located at 273-275 Russ Bradley Road (previously known as 1500 Thomas Argue Road) in Carp, Ontario. It is our understanding that this hydrogeological assessment and terrain analysis is needed based on a requirement from the City of Ottawa as a condition for a privately serviced development, as part of the site plan application process.

Based on documentation provided, the Site is located immediately south of Russ Bradley Road and approximately 70 metres southwest from Carp Road. It is our understanding that the client is looking to develop a private storage facility, which includes twelve (12) self storage buildings, a small office area, and washroom facilities. The area in which this private storage facility will be placed is approximately 2.4 hectares (ha.) in size.

This report has been prepared using data collected from a drilled test well at 273-275 Russ Bradley Road (Test Well 1, TW1) by McIntosh Perry staff on September 13, 2022. It is our understanding that this well (TW1) is the well that will be utilized to service the proposed development. Therefore, the hydrogeological data gathered during the pumping test and subsequent analyses are deemed to be wholly representative of hydrogeological conditions at the Site and future groundwater to be utilized by occupants.

Ground surface at the Site gently slopes throughout the Site towards the south. Site elevation ranges from approximately 111 – 114 metres above sea level (m asl). Surface drainage is interpreted to reflect surface topography and is likely controlled via areas of permeable ground surface. An unnamed creek runs along the south border of the Site, flowing southwest. Surface water and shallow groundwater in the vicinity of the Site likely flows toward this creek.

Test Well 1 was pumped for a duration of 420 minutes and was sampled twice during this time. The pumping rate changed throughout the pumping test in order to adequately reflect a stabilized quantity of water being pumped from the well. The pumping rate at the start of the test was 60 L/min, which was maintained for approximately 35 minutes, at which time the pumping rate was changed to 53.3 L/min for an additional 157 minutes. The pumping rate was changed again 192 minutes after the start of the test to 48 L/min – this rate was maintained for three minutes and was then changed as water levels were not stabilizing. The pumping rate was changed to 42 L/min 195 minutes after the start of the test and remained at that rate until the pump was shut off. The cumulative weighted average pumping rate was 47.8 L/min for the duration of the test, which is considered sufficient to supply future development of a private commercial development.

Water quality results indicate that the bedrock aquifer provides good quality water, which may be considered generally suitable for human consumption. All analytical results were compared to the Ontario Drinking Water Standards, Objectives, and Guidelines (ODWS). Based on the analytical results from TW1 on September 13, 2022, the following exceedances were noted:

- Hardness (OG: 100 mg/L): TW1-1 (271 mg/L) and TW1-2 (265 mg/L)
- Sulphide: (AO: 0.05 mg/L): TW1-1 (3.14 mg/L) and TW1-2 (3.36 mg/L)
- Turbidity: (AO: 5 NTU): TW1-1 (34.8 NTU)
- Aluminum: (AO: 0.1 mg/L): TW1-1 (0.68 mg/L) and TW1-2 (0.14 mg/L)
- Iron (AO: 0.3 mg/L): TW1-1 (0.82 mg/L); and
- The health warning limit for sodium (20 mg/L) was exceeded in sample TW1-1 (22.7 mg/L) and TW1-2 (24.1 mg/L)

All aforementioned exceedances are of an aesthetic or operational nature, are not health related, and can be readily treated if so desired.

On-site overburden in the area of the proposed severance is listed by the Ontario Geological Survey (OGS) as coarse-textured glaciomarine deposits of sand, gravel, minor silt and clay. This is supported by the MECP WWIS records, which indicate mainly sand, clay, and gravel overburden for wells listed within 500 m of the Site. On-site bedrock is generally characterized as limestone, dolostone, shale, arkose, and sandstone of the Simcoe Group of the Shadow Lake Formation (OGS, 2021), which is supported by a majority of well records in the area that list the bedrock as either "shale" or "limestone". The average depth to bedrock is approximately 34 m below ground surface (bgs) for listed wells within 500 m of the Site.

MECP Procedure D-5-4 (Technical Guideline for Individual On-Site Sewage Systems: Water Quality Impact Risk Assessment) outlines the provision for predictive assessment of attenuation. This predictive assessment utilizes a 3-step process where each "step" must be assessed if the previous "step" has not been met. In reference to this Site, Step 1 details whether lot size has sufficient spatial area for the natural attenuation of nitrate-nitrogen. This step has been met as the average lot size will be greater than 1.0 ha with no lot being less than 0.8 ha. Step 2 outlines system isolation consideration, which has additionally been met as there is sufficient spatial area on the retained lot. The thickness of overburden warrants the Site as not hydrogeologically sensitive. As Step 1 and 2 have been met, contaminant attenuation considerations and calculations were not completed for this report.

Based on the analyses performed for this hydrogeological assessment, McIntosh Perry is of the opinion that the aquifer for which the test well intersects can adequately supply water for the proposed private development on-Site.

TABLE OF CONTENTS

| | |
|--|----|
| EXECUTIVE SUMMARY | I |
| 1.0 INTRODUCTION | 1 |
| 1.1 Consultation | 1 |
| 2.0 BACKGROUND | 2 |
| 2.1 Site Setting | 2 |
| 2.2 Neighbouring Properties and Land Uses | 2 |
| 2.3 Hydrology | 2 |
| 2.4 Geology and Hydrogeology | 3 |
| 2.4.1 Recharge and Discharge Areas | 3 |
| 2.4.2 Potential Sources of Contamination | 3 |
| 2.4.3 Water Well Record Review | 3 |
| 2.4.4 Hydro-Fracking | 4 |
| 3.0 METHODOLOGY – HYDROGEOLOGICAL ASSESSMENT | 5 |
| 4.0 RESULTS | 7 |
| 4.1 Static Conditions | 7 |
| 4.2 Pumping Test – TW1 | 7 |
| 4.2.1 Well Yield | 8 |
| 4.2.2 Transmissivity | 8 |
| 4.2.3 Long Term Yield | 8 |
| 4.2.4 Water Quality | 10 |
| 4.2.5 Water Treatment | 11 |
| 4.1 Long-term Groundwater Monitoring | 12 |
| 5.0 TERRAIN ANALYSIS | 12 |
| 5.1 Preamble | 12 |
| 5.2 General Site Evaluation | 12 |
| 5.2.1 Overburden Depth | 12 |
| 5.2.2 Overburden Characterization | 13 |

| | | |
|-------|--|----|
| 5.2.3 | Soil Classification for Private Sanitary Servicing..... | 13 |
| 5.2.4 | Bedrock | 14 |
| 5.3 | <i>Predictive Assessment - Contaminant Attenuation</i> | 14 |
| 5.3.1 | Contaminant Attenuation | 14 |
| 6.0 | CONCLUSIONS AND RECOMMENDATIONS | 15 |
| 6.1 | <i>Conclusions</i> | 15 |
| 6.1.2 | Terrain Evaluation..... | 16 |
| 6.2 | <i>Recommendations</i> | 16 |
| 6.2.1 | Well Construction | 16 |
| 6.2.2 | Well Yields..... | 16 |
| 6.2.3 | Water Quality Treatment..... | 17 |
| 6.2.4 | Wastewater Treatment..... | 17 |
| 7.0 | LIMITATIONS | 18 |
| 8.0 | CLOSURE | 19 |
| 9.0 | REFERENCES | 20 |

TABLES

| | |
|---------|--|
| Table 1 | Summary of Field Parameters (appended) |
| Table 2 | Summary of Laboratory Data (appended) |
| Table 3 | Summary of Test Holes (in report) |

FIGURES

| | |
|----------|--|
| Figure 1 | Site Location |
| Figure 2 | Site Layout |
| Figure 3 | MECP Water Well Information System Summary |
| Figure 4 | Test Hole Layout |

APPENDICES

| | |
|------------|--|
| Appendix A | MECP Water Well Information System Data |
| Appendix B | Well Record |
| Appendix C | Equipment Calibration Certificate |
| Appendix D | Water Level Data and Pumping Test Analysis |
| Appendix E | Laboratory Certificates of Analysis |

| | |
|------------|---|
| Appendix F | Transmissivity, Farvolden and Moell Method Calculations, Langelier Saturation Index (LSI) and Ryznar Stability Index (RSI) Calculations |
| Appendix G | Infiltration Work Summary for 273-275 Russ Bradley Road |
| Appendix H | Grain Size Analysis |

1.0 INTRODUCTION

McIntosh Perry ('MP') was retained by Trevor Watkins ('the Client') to conduct a Hydrogeological Assessment and Terrain Analysis in support of a proposed storage facility development for the property located at 273-275 Russ Bradley Road ('the Site', previously known as 1500 Thomas Argue Road) in Carp, Ontario. It is our understanding that the client is looking to develop a private storage facility, which includes twelve (12) self storage buildings. The area in which this private storage facility will be placed is approximately 2.4 hectares (ha.) in size.

The Site location is shown on Figure 1 – Site Location, and an outline of the Site showing the neighbouring properties and the proposed area of future development is presented on Figure 2 – Site Layout.

This report has been prepared using data collected by McIntosh Perry staff on September 13, 2022, from a drilled test well (A342436) located at 273-275 Russ Bradley Road. Hydrogeological data from this well are considered representative of the subject Site, as this well will be utilized for servicing the proposed development.

This Hydrogeological Evaluation addresses the following:

- Well Record search and evaluation;
- Background hydrogeological evaluation;
- Oversight of a 420-minute pumping test at 273-275 Russ Bradley Road;
- Water level and flow monitoring, field water quality analyses;
- Sampling and analysis – includes 2 sample analyzed for the 'Subdivision Supply Suite' of parameters (including trace metals and volatiles);
- Summary of infiltration data throughout portions of the Site, completed as part of the infiltration assessment of subsurface materials; and
- Data Evaluation and Report.

1.1 Consultation

McIntosh Perry conducted a pre-consultation with a representative from the City of Ottawa via phone call on August 19, 2022. Michel Kearney, P.Geo. from the City of Ottawa provided information on what would be required for this Hydrogeological Report and Terrain Analysis, including the following:

- The Hydrogeological Report prepared for the Site must follow the guidelines stipulated in Procedure D-5-5 (Private Wells: Water Supply Assessment);
- It was communicated that a 6-hour pumping test would be acceptable if the drilled test well shows sufficient water quality and quantity (proper rate and recovery);
- Volatile organic carbons (VOCs) and metals are to be included in the subdivision package for the water quality analysis; and
- The terrain analysis needs to provide an impact of the septic system at the property line.

2.0 BACKGROUND

2.1 Site Setting

The Site is located in the community of Carp, located in West-Carleton-March Ward in the City of Ottawa, Ontario (Figure 1). The Site is currently unoccupied, vacant land. The Site is unused, with the Carp Airport located in close proximity (west) of the Site. Due to the proximity of the airport, the Site is designated as "Air Transportation Facility Zone" per the City of Ottawa Zoning By-Law No. 2008-250.

At the time of investigation, on-site conditions consisted primarily of a grassed and forested area. Based on a review of aerial photos and field observations, it appears that the Site has never been contemporarily developed.

2.2 Neighbouring Properties and Land Uses

The Site is located south of the intersection of Carp Road and Russ Bradley Road. The Site is within a rural land use area, and is surrounded by Carp Road to the north/northeast, William Mooney Road to the South, the Carp Airport to the west. Land-use on all sides from the Site include mainly commercial and industrial properties.

The proposed total area of the Site, which includes the private storage facility, consists of an approximate 2.4-hectare portion of land. While MECP Water Well Information System (WWIS) records for the area do not provide the detailed locations of most wells, all properties developed in proximity to the Site are assumed to be privately serviced with wells and on-site sewage systems.

Figure 3 – MECP Wells Record Summary, presents the MECP Well Tag numbers and approximate well locations, where available, for wells within approximately 500 m of the Site. Well Records within 500 m of the Site are included in Appendix A.

2.3 Hydrology

Topography was reviewed on the Atlas of Canada Toporama website. Site elevation is approximately 111 – 114 metres above sea level (m asl). Ground surface at the Site is generally gently sloped throughout the site towards the south, towards an unnamed creek which flows west.

Surface drainage is interpreted to reflect surface topography and is likely controlled via areas of permeable ground surface. An unnamed creek runs south of the Site, flowing towards the west. Shallow groundwater in the vicinity of the Site likely flows toward this creek. The closest permanent water body is Carp River, located approximately 1.6 km north of the Site, at its closest point. On a regional scale, surface water is likely to flow to the west/northwest towards Carp River, eventually flowing into Lac des Chats.

It is noted that during the fieldwork completed for the Hydrogeological Assessment, the Site appeared to be poorly drained; standing water was present in several areas of the Site, which was not consistent with the City of Ottawa's designation of the property as a high-infiltration area. This was further supported by data

indicating low infiltration rates, as measured by *in-situ* testing with a Guelph Permeameter across portions of the Site (as summarized below).

2.4 Geology and Hydrogeology

On-site overburden in the area of the proposed severance is listed by the Ontario Geological Survey (OGS) as coarse-textured glaciomarine deposits of sand, gravel, minor silt and clay. This is supported by the MECP WWIS records, which indicate mainly sand, clay, and gravel overburden for wells listed within 500 m of the Site. Refer to Section 5.0 for a more detailed discussion regarding surficial geology.

On-site bedrock is generally characterized as limestone, dolostone, shale, arkose, and sandstone of the Simcoe Group of the Shadow Lake Formation (OGS, 2021), which is supported by a majority of well records in the area that list the bedrock as either “shale” or “limestone”.

Based on surrounding topography, shallow groundwater is interpreted to have a west/northwest component.

2.4.1 Recharge and Discharge Areas

Based on a review of topographic data, geological maps, and Site visits, the property slopes slightly upwards to the south. Shallow groundwater and surface water likely flow towards the west/northwest.

Based on previously noted permeability testing, infiltration is relatively low across the Site. This appears to be partly due to high water levels within the overburden soils (see Appendix G), as well as relatively low hydraulic conductivities in the shallow soil. Site observations made in June and August 2022, prior to the pumping test, indicate that the property and development area is highly saturated, with many areas of stagnant standing water. The wooded area on-Site in particular appears to be a local topographic low point.

2.4.2 Potential Sources of Contamination

A windshield survey of the surrounding area was conducted in combination with a Site walkthrough and review of maps and zoning information. The Site is located in a predominantly rural commercial area. This does not appear to pose any significant source of contamination to the proposed severance. No obvious potentially contaminating activities (e.g., fuel outlets, improperly maintained bulk fuel storage, salt storage, manure piles, livestock yards, etc.) were observed in the vicinity of the Site at the time of inspection.

The Site and surrounding properties are not connected to municipal services. As such, there are likely private on-site sewage systems at nearby residences.

2.4.3 Water Well Record Review

The MECP's WWIS database indicated twenty-two (22) water wells that are located within 500 m of the centre of the Site. Five (5) of these records have no information available or are listed as abandoned wells. Nine (9) wells are listed for domestic or commercial water supply purposes, four (4) are listed as observation

wells, and one (1) well is listed as a test/monitoring well. The MECP WWIS records are shown on Figure 3, and data are summarized in Appendix A.

Wells were completed in varying subsurface materials including clay, sand, gravel, limestone, and shale, ranging in final depths of 0.3 – 48.7 m below ground surface (bgs). The average depth to bedrock was reported to be 34 m bgs. Driller-reported static groundwater levels ranged from 1.1 – 15.2 m bgs.

Driller-reported well yields ranged from 11.4 – 94.7 L/min.

For the on-Site well (TW1), the well was completed primarily in clay (mixed with gravel) from ground surface to 41.5 m bgs), followed by limestone at 41.5 m to 152 m bgs. The driller-reported static groundwater level was 60 m bgs, and the well yield (prior to hydro-fracking), was listed at 18.9 L/min.

2.4.4 Hydro-Fracking

Prior to the pumping test administered on September 13th, 2022, Test Well 1 (TW1) was hydro-fracked by a licensed well driller (Ontario Water Well Fracturing Ltd.) to increase yield. Hydro-fracking (or hydro-fracturing) is a process whereby water is injected into the well at a high pressure to create small fractures within the bedrock material in order to facilitate greater infiltration of groundwater into the well itself. Hydro-fracking was performed on this well as there were previously identified issues with regards to water supply and production.

3.0 METHODOLOGY – HYDROGEOLOGICAL ASSESSMENT

McIntosh Perry conducted a hydrogeological investigation at the Site to assess the feasibility of servicing the proposed development. The work generally followed the guidance of MECP Procedure D-5-5: Technical Guideline for Private Wells: Water Supply Assessment.

McIntosh Perry tested the drilled test well located at 273-275 Russ Bradley Road (Test Well 1, TW1 – A342436), which is representative of the hydrogeological conditions across the proposed development. The well record is saved in Appendix B, appended to this report.

A 420-minute pumping test was conducted at TW1 by McIntosh Perry staff on September 13, 2022. Based on correspondence received from the City of Ottawa (dated August 19, 2022), it was expressed that a 6-hour pumping test would be sufficient if the well indicated sufficient water quantity and quality. Based on conditions encountered at the time of the pumping test (involving changing the pumping rate to allow groundwater to stabilize), a 420-minute (7 hour) pumping test was completed.

Groundwater was pumped directly from TW1 using a pump provided and installed by Air Rock Drilling. The pumped water was directed away from the test well and was allowed to flow overland across the Site.

During the testing period, water levels in the well were measured using an electronic water level tape. Water quality (pH, temperature, conductivity, oxygen reduction potential, turbidity, dissolved oxygen, and total dissolved solids) was also monitored and recorded in the field during the test using calibrated instruments (general parameters -Horiba U-52; Turbidity - LaMotte 2020). The LaMotte 2020 turbidity meter is calibrated monthly by McIntosh Perry staff following manufacturers instructions. The calibration certificate for the Horiba U-52 completed by the rental company (Maxim) is included in Appendix C. Additional visual water quality observations were observed including colour, clarity/turbidity, odour, and effervescence, as seen in Table 1 appended to this report. Groundwater chemistry had stabilized prior to collecting samples of the well water.

One sample (TW1_1) was collected for laboratory analysis, taken 180 minutes after the start of the pumping test. An additional sample (TW1_2) was collected for laboratory analysis 415 minutes after the start of the test. These sample were analyzed for the full suite of subdivision supply parameters, including metals, microbial, and VOCs.

At the time both samples were collected from TW1, residual chlorine readings indicated a value of 0.0 mg/L using a Hach DR900 colorimeter; the Hach DR900 was zero standardized prior to collecting samples. All groundwater samples were collected unfiltered and unchlorinated, directly into clean bottles supplied by the analytical laboratories (Paracel Laboratories Ltd., Ottawa, ON). The samples were kept on ice and delivered directly to Paracel under strict chain of custody procedures. All of the samples were received by the laboratory within 24 hours of collection.

Paracel is fully accredited by the Standards Council of Canada/Canadian Association for Laboratory Accreditation (SCC/CALA) and has accreditation for Ontario Safe Drinking Water Act (OSDWA) testing.

During the pumping test, water level monitoring consisted of manual readings with an electronic water level tape. Drawdown was measured in the pumped well and measurements were made until at least 95% recovery were achieved, or 24 hours had passed (whichever came first). A data logger was not used as part of this assessment due to concerns with down-hole entanglement.

Drawdown and recovery data from the pumping tests were plotted and analyzed using the Cooper-Jacob solution. The hydraulic conductivity (K , m/s) and transmissivity (T , m²/d) and long-term yield (Farvolden and Moell Method) of the aquifer were estimated.

Storativity could not be assessed properly without the use of an additional observation well, which was not available at the time of the test.

It is noted that in addition to the pumping test completed, McIntosh Perry completed an infiltration assessment across the Site to determine the general infiltration rates of subsurface materials. Based on this assessment completed in October 2022, permeability across the Site was low given the excess of saturated soils encountered. Two infiltration studies were conducted on-Site. In June of 2022, the advancement of three (3) test locations was completed, two (2) of which were outside of the proposed infiltration infrastructure area. Results of this program indicated low infiltration rates which ranged from 3.4×10^{-6} to 4.9×10^{-7} m/s. In October, additional infiltration testing was completed on-Site where the proposed infiltration infrastructure would be placed. Three (3) test locations were advanced, and low infiltration rates were again found, with rates ranging from 1.74×10^{-8} to 6.4×10^{-6} m/s. Appendix G provides additional information on the June 2022 infiltration program.

4.0 RESULTS

A drawdown curve and tabular data from the pumping test conducted at the Site are available in Appendix D. A summary of groundwater quality data and the official Laboratory Certificates of Analysis are available in Table 2 and Appendix E, respectively.

4.1 Static Conditions

Prior to the initiation of pumping, water levels were measured in the well. The static groundwater level was recorded at 9.87 m below top of casing (btoc) at the time of the pumping test ($t=0$). Assigning an arbitrary site benchmark of 100.00 m (local) to the top of the casing, the static water elevation in the well was 90.13 m above datum (ad). According to the MECP Well Record for TW1 (A342436) the proposed pump depth was recommended to be 91.4 m bgs – the depth used at the time of the pumping test was 85.3 m bgs. The pumping depth used during the test corresponded to an available water column of approximately 75.5 m.

Standing water or evidence of groundwater discharge was not observed at the test well location at the time of the pumping test.

4.2 Pumping Test – TW1

The pumping test was conducted at TW1 (273-275 Russ Bradley Road) was performed under the supervision of McIntosh Perry on September 13, 2022. Water was pumped directly from the test well using equipment provided by Air Rock. The water discharge was directed away from the test well and was allowed to flow overland across the Site, away from the well. At the time of the pumping test, the weather was approximately 20°C and cloudy.

At 7:40 AM, the pump was turned on and the flow rate adjusted to approximately 60 L/min. This pumping rate was maintained for approximately 35 minutes, at which time the pumping rate was changed to 53.3 L/min for an additional 157 minutes. The pumping rate was changed again 192 minutes after the start of the test to 48 L/min – this rate was maintained for three minutes and was then changed as water levels were not stabilizing. The pumping rate was changed to 42 L/min 195 minutes after the start of the test and remained at that rate until the pump was shut off (420 minutes after the start of the test).

The stepwise reductions in the pumping rate described above were performed as water levels were not stabilizing. The higher pump rate that was originally used at the start of the pumping test was reduced in order to achieve a more sustainable pumping rate which could be maintained for the remainder of the test. All pumping rates used were greater than the minimum daily water demand of approximately 13.7 L/min.

The groundwater level ranged between 9.87 – 53.95 m btoc, with a maximum drawdown of 44.08 m observed. At the end of the test, approximately 38.7 m of the available water column remained. Following pump shutoff (420 minutes), the water level was recorded at 11.7 m btoc (88.24 m ad) within 50 minutes, representing approximately 97% recovery.

All water level measurement data are presented in Table 2, appended to this report.

4.2.1 Well Yield

The pumping test undertaken by McIntosh Perry provides a reasonable indication of the yield of the Test Well. During this test, over 20,000 L of water was pumped from the well. Given that the typical volume (daily flow) required for an individual employee per eight hour work shift is 75 L, the 20,000 L pumped would be sufficient for over 250 employees. It is anticipated that no more than two employees will staff the operations per eight hour shift.

4.2.2 Transmissivity

The transmissivity for TW1 was calculated following the Cooper-Jacob method. The calculations for Transmissivity are presented in Appendix F. Transmissivity was calculated using the following equation:

$$T = \frac{2.3 Q}{4 \pi \Delta s}$$

Where:

- T is the transmissivity (m²/day)
- Q is the pumping rate during the pumping test (L/min); and,
- Δs is the differential for residual drawdown for one log cycle (m)

Using drawdown and recovery data, a transmissivity during the drawdown period was 0.6 m²/day, and a transmissivity during the recovery phase was calculated at 0.5 m²/d using the Cooper-Jacob method.

Assuming an aquifer thickness of 109.12 m (as approximated by the interval between the bottom of the casing and the bottom of the well), the screened formation of TW1 was calculated to have an average hydraulic conductivity of 9.35 x 10⁻⁸ m/s.

Storativity (S) could not be calculated as no observation wells were available for measurement at the time of the pumping test.

A summary of the well and hydrogeological properties determined during the testing work at the Site are presented in Appendix D. The calculations for Transmissivity are presented in Appendix F.

4.2.3 Long Term Yield

The theoretical long-term safe yield was calculated using both the Farvolden and Moell methods. Drawdown data were used, as they are likely more representative of aquifer conditions (see above Section 4.2.2).

It is important to note that the safe yield may be less than the values calculated below, due to well-field interference that may be present in the final lot/supply well configuration.

Farvolden Equation

The long-term yield (Q₂₀) was calculated using the following Farvolden equation:

$$Q_{20} = 0.68 T H_a S_f$$

Where:

- Q_{20} is the twenty-year safe yield;
- T is the transmissivity;
- H_a is the available water column height (above the pump); and
- S_f is a safety factor (0.7).

Based on the Farvolden Method, calculations indicate that a twenty-year safe yield is on the order of 14 L/min. This means that TW1 could theoretically sustain continuous pumping for 20 years at this rate.

Moell Method

The Moell Method was also used to calculate the theoretical long-term safe yield for the pumping well. The long-term yield (Q_{20}) was calculated using the following Moell equation:

$$(Q_{20}) = (Q H_a S_f) / (s100 + 5 \Delta s)$$

Where:

- Q_{20} is the twenty-year safe yield (m^3/day);
- H_a is the available water column height (m);
- S_f is a safety factor (0.7);
- $s100$ is the drawdown at 100 minutes (semi-log long-term graph);
- Δs is the change in hydraulic head over one log cycle (drawdown vs. log time, see Appendix D);
and
- Q is the pumping rate during the pumping test (L/min).

Using the Moell Method, calculations indicate that a twenty-year safe yield for the well is on the order of 12 L/min.

The twenty-year (long-term) safe yield calculations described above for this supply well ranged from 12-14 L/min. These calculations are inherently conservative, as the pump will likely cycle on and off over a shorter period of time. The peak hourly flow rates will likely be less than the calculated values above. Further, the 7-hour pumping test conducted indicates sustainable flow rates which are considered to be sufficient to support the proposed development. Therefore, McIntosh Perry is of the opinion that the aquifer is capable of supplying water at a flow rate greater than the minimum of 13.7 L/min (as outlined in Procedure D-5-5), as well as the per-person requirements of 450 L/day, for the proposed private storage facility.

The calculations for the Farvolden and Moell method are presented in Appendix F.

4.2.4 Water Quality

Laboratory Certificates of Analysis for on-site groundwater testing are presented in Appendix E. A summary of field and laboratory results from the Test Well is presented in Tables 1A, 1B, and 2. Two samples were taken during the 420-minute pumping test of TW1 on September 13, 2022. The first sample (TW1-1) was taken 180 minutes after the start of the test, and the second sample (TW1-2) was taken 415 minutes after the start of the test. Both samples were taken directly from the pump discharge hose into laboratory supplied containers.

Prior to collection of the groundwater samples, the residual chlorine (total and free chlorine) reading using the Hach DR900 colorimeter was 0 mg/L after 164 minutes and 360 minutes after pumping. Prior to usage, the Hach DR900 was calibrated according to the manufacturer's printed instructions.

All analytical results were compared to the Ontario Drinking Water Standards, Objectives, and Guidelines (ODWS).

Based on the analytical results from TW1 on September 13, 2022, the following exceedances were noted:

- Hardness (OG: 100 mg/L): TW1-1 (271 mg/L) and TW1-2 (265 mg/L)
- Sulphide: (AO: 0.05 mg/L): TW1-1 (3.14 mg/L) and TW1-2 (3.36 mg/L)
- Turbidity: (AO: 5 NTU): TW1-1 (34.8 NTU)
- Aluminum: (AO: 0.1 mg/L): TW1-1 (0.68 mg/L) and TW1-2 (0.14 mg/L)
- Iron (AO: 0.3 mg/L): TW1-1 (0.82 mg/L); and
- The health warning limit for sodium (20 mg/L) was exceeded in sample TW1-1 (22.7 mg/L) and TW1-2 (24.1 mg/L)

No health-related maximum acceptable concentration (MAC) were exceeded.

The bacteria were all non-detectable (0 cts/100 mL for E-coli, Fecal Coliforms, and Total Coliforms), in the sample that was collected at TW1.

Field-reported turbidity was considerably lower than laboratory-reported turbidity throughout the pumping test. While turbidity dropped to acceptable levels throughout the test, elevated turbidity is likely a result of the hydrofracking process, and should improve with continued well development. With further well development (pumping and use of the well), any fine grained material is agitated, causing it to become suspended and then removed during pumping. Thus, with continued use of the well, turbidity values are expected to decrease.

The Langelier Saturation Index (LSI) and Ryznar Stability Index (RSI) were calculated for TW1 (Appendix F). These results indicate that there is potential for scale to form on pipes, and that any calcium carbonate formation is not likely to form a protective corrosion inhibitor film (LSI=0.34 , RSI=7,2).

4.2.5 Water Treatment

A review of the analytical data collected for the groundwater sample revealed exceedances of the well of Aesthetic Objectives (AO) or Operational Guidelines (OG). No MACs (health related) were exceeded. While the analysis of groundwater did not reveal any health-related issues, treatment can be utilized to make the water more palatable, if so desired. All parameters which exceeded AO and OG can be treated to improve water quality. In addition, aesthetic parameters such as total dissolved solids and iron are expected to either improve with continued development and use or can be readily treated.

After review of the analytical results, the following methodologies for treatment are recommended:

| | |
|------------|---|
| Turbidity: | Carbon filtration, greensand filtration, reverse osmosis |
| Salts: | Reverse osmosis |
| Hardness: | Ion exchange, reverse osmosis |
| Iron: | Reverse osmosis, greensand filter |
| Sulphide: | Adsorption, aeration, chlorination, greensand filtration, oxidation |
| Aluminum: | Distillation, reverse osmosis |

Filtration is a treatment method that can be used to address the above noted exceedances for turbidity, iron, and sulphide. Several filtration methods exist and offer adequate treatment for issues related to well water treatment. The use of granulated activated carbon filters or greensand, for example, constitute two methods of filtration.

Coagulation is a chemical water treatment process. It involves the use of a material which precipitates into water and causes fine particles to agglomerate into larger particles, which can then be removed via settling and/or filtration.

Distillation is a treatment process in which water is converted into a vapor state, then cooled, condensed, and collected. It is done to remove solids and other impurities from the water.

Reverse osmosis is a treatment process in which dissolved ions are removed from water using a difference in pressure through a semi-permeable membrane. This membrane will filter water and prevent certain undesirable dissolved materials from passing through.

Oxidation/aeration involves the injection of oxygen into the well water, whereby granular media (such as manganese-oxide) is used and allows for the adsorption of iron and manganese.

Ion exchange (often seen in the form of water softeners) is a treatment which can remove ferrous iron from the well water.

Chlorination involves the introduction of chlorine into the well. Chlorine will allow for disinfection and decrease the quantity of sulphide and other undesirable parameters.

Further development of the well is recommended. This will lower turbidity, hardness, iron, and aluminum

concentrations. As indicated in Section 4.2.4 above, with continued development (pumping and use of the well), fine-grained materials are agitated and become dissolved, which are then removed from the well during further development.

4.1 Long-term Groundwater Monitoring

As infiltration throughout the subsurface materials on the Site appears to be low (see Appendix G for the infiltration memo conducted in October 2022), additional information regarding shallow groundwater is needed in the proposed development area. McIntosh Perry has installed a shallow groundwater monitoring well (BH22-2) to assist in characterizing the shallow groundwater regime in proximity to proposed stormwater management infrastructure. This well is in addition to an existing on-site shallow groundwater monitoring well (BH21-1) installed as part of McIntosh Perry's geotechnical scope of work.

Monitoring well BH22-2, installed within the proposed infiltration gallery area, was completed on December 6, 2022. This well was installed by Strata Drilling Group using a Geoprobe to a maximum depth of 15 ft (4.5 m) bgs. It is noted that during the well installation, the saturated soils continued to slough into the open hole, causing a slight upwelling of the well casing/pipe. Immediately after the monitoring well was installed, geodetic elevations of the ground surface of the borehole and monument casing was obtained, as well as geodetic elevations of nearby supply wells.

Further groundwater level readings at BH22-2 will continue at the Site throughout the upcoming winter (2022) and spring (2023) months. Water level data will be primarily used for stormwater management purposes.

5.0 TERRAIN ANALYSIS

5.1 Preamble

A series of four (4) test holes were advanced by McIntosh Perry staff at various locations throughout the proposed septic area on November 24th, 2022 (see Figure 4). The test hole locations were advanced using a hand auger and shovel, completed to characterize subsurface materials, the depth of overburden, depth to shallow groundwater, and to permit the collection of overburden soil samples for characterization. It is noted that holes were only advanced to a maximum depth of 2.0 m bgs as required for the purposes of assessment for the future septic location.

5.2 General Site Evaluation

5.2.1 Overburden Depth

Overburden across the Site was found to be relatively thick, having an average thickness of 1.2 m. The test hole locations are outlined on Figure 4. It is important to note that moist to saturated conditions were observed within each test hole advanced within the proposed septic location.

Although overburden based on the terrain analysis alone was estimated to be a maximum of 2 m thick (see TP-2, below), overburden thickness was additionally surmised based on information from the subsurface materials encountered during the infiltration assessment, as well as an overview of well records from the Site.

In addition, based on the geotechnical investigation completed for the Site in which three (3) boreholes were advanced, bedrock was not encountered. Overburden depth was found to be at least 6.7 m bgs.

5.2.2 Overburden Characterization

The soil and groundwater conditions logged in the test holes are presented in Table 3 below. The test hole summaries indicate the subsurface conditions at the specific test hole locations only; subsurface conditions at other locations outside of the investigated area could differ from those encountered within the investigated area.

| Table 3: Summary of Test Holes | | | |
|--------------------------------|-----------------|--------------------|--|
| Test Pit ID | Total Depth (m) | Depth to Water (m) | Soil Characteristics |
| TP-1 | 1.0 | 1.0 | Grey/brown silty sand, trace clay, loose, moist to wet |
| TP-2 | 2.0 | 1.5 – 2.0 | Grey/brown silty sand, trace clay, loose, moist to wet |
| TP-3 | 1.0 | 0.7 – 0.8 | Grey/brown silty sand, trace clay, loose, moist to wet |
| TP-4 | 1.1 | 1.0 | Grey/brown silty sand, trace clay, loose, moist to wet |

The soil descriptions in this report are based on commonly accepted classification and identification employed in engineering practice. It is noted that no bedrock was encountered during the digging of test holes. In addition, no bedrock was encountered during the geotechnical investigation. McIntosh Perry employed judgement in the classification and description of soil and may not be exact but are accurate to what is common in current engineering practice. The grain size analysis is included in Appendix H.

5.2.3 Soil Classification for Private Sanitary Servicing

Comparison of the soil classification for the Unified Soil Classification as provided in the Ministry of Municipal Affairs and Housing (MMAH) Supplementary Standard SB-6: Time and Soil Descriptions, reveals that the main native soil underlying the upper topsoil appears to be within the following soil group:

- SM: Silty sands, sand-silt mixtures

- According to Table 2 of SB-6, the SM group of soils has a coefficient of permeability (K) of 10^{-5} to 10^{-3} with a percolation time (T) of 8 to 20 min/cm. This soil type has a medium to low permeability and is deemed acceptable as the native receiving soil for proposed Class 4 sewage systems.

Based on the encountered overburden, it is recommended that the topsoil layer be stripped where the septic system is proposed for construction. The thickness of native overburden has been determined through an overview of well records from the Site, subsurface conditions and depths of overburden encountered during the infiltration assessment, as well as the observation of soil thicknesses encountered during the terrain assessment portion. Given the general thickness of native overburden suitable for septic disposal bed construction, partial or fully raised septic beds may be required due to the shallow depth to the overburden groundwater, to meet the Ontario Building Code (OBC) requirement of 0.9 m separation between bedrock or shallow groundwater and the underside of the disposal bed pipe.

5.2.4 Bedrock

As previously discussed, on-site bedrock is generally characterized as limestone, dolostone, shale, arkose, and sandstone of the Simcoe Group of the Shadow Lake Formation (OGS, 2021). No bedrock was encountered on-Site during the test hole advancements, nor the geotechnical investigation,

5.3 Predictive Assessment - Contaminant Attenuation

5.3.1 Contaminant Attenuation

The MECP Procedure D-5-4 (Technical Guideline for Individual On-site Sewage Systems: Water Quality Impact Risk Assessment) outlines the provision for predictive assessment of attenuation. The predictive assessment is a 3-step process where each subsequent “step” must be assessed if the previous “step” is not met. The 3-step process is as follows:

- Step 1 – Lot Size Consideration
- Step 2 – System Isolation Consideration
- Step 3 – Contaminant Attenuation Considerations

The following outlines the results of the sewage system impact assessment undertaken by McIntosh Perry.

Step 1 - Lot Size Consideration

The area of the property is roughly 5.8 ha.

Based on the lot area noted above, Step 1 of the 3-Step process is satisfied; the proposed lot size is greater than 1 hectare, with no lot less than 0.8 hectares.

There is sufficient spatial area for the natural attenuation of nitrate-nitrogen at acceptable concentrations based on MECP Procedure D-5-4. Based on well records and observations made in the field during the terrain

assessment, overburden thickness is on average greater than 2.0 m. In addition, the overburden thickness was found to be at least 6.7 m bgs as observed during the geotechnical investigation.

Step 2 - System Isolation Consideration

As previously outlined, the proposed lot sizes are greater than 1.0 ha., therefore System Isolation Considerations are not applicable to the proposed development. If it can be demonstrated that the sewage system effluent is hydrogeologically isolated from the existing or potential drinking water supply aquifer, then the risk to groundwater is considered to be low. The system isolation review needs to account for lands that extend up to 500 metres from the Site.

Based on a review of available geological information and mapping, and in conjunction with site observations made during the Terrain Analysis and infiltration assessment, the overburden depth on-site has a thickness of approximately 34 m bgs and consists of primarily fine-grained material (silty sand with clay). Groundwater was found at depths ranging from 0.7 to 1.5 m bgs at TP-3 and TP-2, respectively, as seen in Table 3 above. Due to the thickness of overburden, and the depth at which the supply aquifer is found (25 to 48 m bgs) the Site is not considered to be hydrogeologically sensitive. Step 1 and Step 2 have been met, and therefore contaminant attenuation considerations (as outlined in Step 3) are not necessary for this report.

6.0 CONCLUSIONS AND RECOMMENDATIONS

6.1 Conclusions

6.1.1.1 Well Yield

McIntosh Perry conducted a 420-minute pumping test at an average pumping rate of approximately 47.8 L/min.

During the pumping test, greater than 20,000 litres of groundwater was pumped from the well. Total drawdown resulting from the 420-minute pumping test was 44.08 m. Within 50 minutes following the cessation of pumping, water level recovery for the well was recorded approximately 97%.

Calculations for long term yield ranged from 12 L/min (Moell) to 14 L/min (Farvolden). These calculations are inherently conservative, as the pump will likely cycle on and off over a shorter period of time. The peak hourly flow rates will likely be less than the calculated values above. Further, the 7-hour pumping test conducted indicates sustainable flow rates which are considered to be sufficient to support the proposed development. Therefore, McIntosh Perry is of the opinion that the aquifer is capable of supplying water at a flow rate greater than the minimum of 13.7 L/min (as outlined in Procedure D-5-5) for the proposed private storage facility.

6.1.1.2 Water Quality and Treatment

All analytical results were compared to the Ontario Drinking Water Standards, Objectives, and Guidelines (ODWS). Based on the analytical results from the groundwater sampled from the on-Site well on September 13, 2022, the following exceedances were noted:

- Hardness (OG: 100 mg/L): TW1-1 (271 mg/L) and TW1-2 (265 mg/L)
- Sulphide: (AO: 0.05 mg/L): TW1-1 (3.14 mg/L) and TW1-2 (3.36 mg/L)
- Turbidity: (AO: 5 NTU): TW1-1 (34.8 NTU)
- Aluminum: (AO: 0.1 mg/L): TW1-1 (0.68 mg/L) and TW1-2 (0.14 mg/L)
- Iron (AO: 0.3 mg/L): TW1-1 (0.82 mg/L); and
- The health warning limit for sodium (20 mg/L) was exceeded in sample TW1-1 (22.7 mg/L) and TW1-2 (24.1 mg/L)

No health-related maximum acceptable concentrations (MAC) were exceeded. All AO and OG exceedances are considered treatable, if so desired.

6.1.2 Terrain Evaluation

Soil materials encountered during the terrain assessment consisting of fine, loose, moist to wet silty sand. It was shown that thickness of soils extends to 6.7 m bgs, based only on the depth at which holes were dug for the purposes of subsurface characterization for the septic assessment, as well as subsurface conditions encountered during the infiltration assessment.

Based on the soils encountered during the terrain assessment and review of subsurface materials from the well records, as well as the proposed size of the lot (severed) it has been determined that there is sufficient spatial area for the natural attenuation of nitrate-nitrogen at acceptable concentrations based on MECP Procedure D-5-4. Due to the thickness of overburden, the Site is not considered to be hydrogeologically sensitive.

6.2 Recommendations

6.2.1 Well Construction

- Referencing the Well Record for the Site well (A342436), it has been determined that the on-Site supply well meets the requirements under O.Reg. 903.

6.2.2 Well Yields

- Calculations for long term well yield indicate that the aquifer currently utilized can support the proposed development.

6.2.3 Water Quality Treatment

- Further development of the well prior to connection to a structure is recommended. This will likely lower turbidity, hardness, and metals concentrations;
- If water softening is desired, the use of potassium salts (i.e., KCl) is recommended. With the use of conventional water softeners, it is important to note that sodium concentrations will be elevated;
- Aesthetic parameters such as total dissolved solids and iron are expected to either improve with continued development and use or can be readily treated, if so desired. Iron can be treated through cation exchange, greensand filtration, or oxidation with filtration through proprietary filter media or chlorination followed by sand or multimedia filtration, depending on the iron concentrations; and
- It is recommended that the Client notify the local Medical Officer of Health as the sodium concentration exceeds the health-related warning limit.

6.2.4 Wastewater Treatment

- The overburden for the site is comprised of silty sand to sandy silt mixtures (SM) which have a low to medium permeability and is acceptable for construction of septic systems per the Ontario Building Code (OBC);
- The depth to bedrock or perched groundwater may be less than 2.0 m; the construction of raised or partially raised disposal beds is potentially required; and
- Construction of septic system will require conformance to the OBC for all aspects including setback distances from residences and wells.

7.0 LIMITATIONS

This report has been prepared and the work referred to in this report has been undertaken by McIntosh Perry Consulting Engineers Ltd. for the applicants and the regulatory authority. It is intended for the sole and exclusive use of the applicants, their affiliated companies and partners and their respective insurers, agents, employees, advisors, and reviewers. The report may not be relied upon by any other person or entity without the express written consent (Reliance Letter) of McIntosh Perry Consulting Engineers Ltd.

Any use which a third party makes of this report, or any reliance on decisions made based on it, without a reliance letter are the responsibility of such third parties. McIntosh Perry Consulting Engineers Ltd. accept no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

The investigation undertaken by McIntosh Perry Consulting Engineers Ltd. with respect to this report and any conclusions or recommendations made in this report reflect McIntosh Perry Consulting Engineers Ltd. judgment based on the Site conditions observed at the time of the site inspection on the date(s) set out in this report and on information available at the time of the preparation of this report.

This report has been prepared for specific application to this Site and it is based, in part, upon visual observation of the Site, subsurface investigation at discrete locations and depths, and specific analysis of specific chemical parameters and materials during a specific time interval, all as described in this report. Unless otherwise stated, the findings cannot be extended to previous or future Site conditions, portions of the Site which were unavailable for direct investigation, subsurface locations which were not investigated directly, or chemical parameters, materials or analysis which were not addressed. Substances other than those addressed by the investigation described in this report may exist within the Site, substances addressed by the investigation may exist in areas of the Site not investigated and concentrations of substances addressed which are different than those reported may exist in areas other than the locations from which samples were taken.

If site conditions or applicable standards change or if any additional information becomes available at a future date, modifications to the findings, conclusions and recommendations in this report may be necessary.

8.0 CLOSURE

We trust that this information is satisfactory for your present requirements. Should you have any questions or require additional information, please do not hesitate to contact the undersigned.

Respectfully submitted,
McIntosh Perry Consulting Engineers Ltd.



Rebecca Leduc, M.Sc.
Environmental Scientist
(343) 764-2080
r.leduc@mcintoshperry.com



Jordan Bowman, P.Geo., P.Biol. (AB)
Manager, Geo-Environmental
(613) 714-4602
j.bowman@mcintoshperry.com



Fraser Armstrong, P.Eng.
Sr. Project Engineer
(343) 344-2663
f.armstrong@mcintoshperry.com

Ref.: U:\Ottawa\01 Project - Proposals\2022 Jobs\CCO\CCO-22-1643-01 Trevor Watkins SPC 1500 Thomas Argue\Hydro
G\Report\CCO-22-1643-01 Russ Bradley_Hydrogeological Assessment_03Feb2023.docx

9.0 REFERENCES

OGS Earth, 2021. Ontario Ministry of Northern Development, Mines and Forestry, - Ontario Geological Survey Earth – for Google Earth. Overburden classification data for Eastern Ontario.

OGS Earth, 2021. Ontario Ministry of Northern Development, Mines and Forestry, - Ontario Geological Survey Earth – for Google Earth. Bedrock classification data for Eastern Ontario.

MECP, 1995. Procedure D-5-4 Technical Guideline for Individual On-site Sewage Systems: Water Quality Impact Risk Assessment.

MECP, 1996. Procedure D-5-5 Technical Guideline for Private Wells: Water Supply Assessment.

MECP (Ontario Ministry of the Environment, Conservation and Parks). 2018. MECP Water Well Record Database (queried September 2022).

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN
ANALYSIS
273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



TABLES

Table 1
Summary of Field Water Quality Parameters
273-275 Russ Bradley Road (TW1)

| Pumping Test at: | | TW1 | | Date: | | 13-Sep-22 | | | |
|--------------------|-----------------|------|----------------------|------------------|----------------------------|-----------|--------|--------------|-------------------|
| Time Elapsed (min) | Turbidity (NTU) | pH | Conductivity (us/cm) | Temperature (°C) | Dissolved Oxygen (DO) mg/L | TDS (ppm) | Odour | Effervesence | Flow Rate (L/min) |
| 8 | 15.4 | 7.55 | 596 | 10.31 | 2.41 | 381 | Sulfur | N/A | 60 |
| 11 | 37.1 | 7.94 | 563 | 9.26 | 1.13 | 359 | Sulfur | N/A | 60 |
| 15 | 30.5 | 8.03 | 540 | 9.02 | 0.79 | 349 | Sulfur | N/A | 60 |
| 19 | 28.4 | 7.89 | 546 | 8.41 | 0.6 | 349 | Sulfur | N/A | 60 |
| 24 | 29.4 | 7.5 | 550 | 8.41 | 0.37 | 352 | Sulfur | N/A | 60 |
| 30 | 32.7 | 7.47 | 555 | 8.43 | 0.55 | 355 | Sulfur | N/A | 60 |
| 42 | 38.8 | 7.93 | 563 | 8.48 | 1.91 | 360 | Sulfur | N/A | 53.3 |
| 57 | 52.3 | 7.67 | 565 | 8.4 | 0.3 | 362 | Sulfur | N/A | 53.3 |
| 68 | 56.6 | 7.56 | 560 | 8.4 | 0.5 | 358 | Sulfur | N/A | 53.3 |
| 79 | 65.1 | 7.52 | 562 | 8.4 | 1.0 | 360 | Sulfur | N/A | 53.3 |
| 94 | 66.9 | 7.53 | 562 | 8.4 | 1.1 | 360 | Sulfur | N/A | 53.3 |
| 108 | 62.6 | 7.56 | 562 | 8.41 | 0.89 | 360 | Sulfur | N/A | 53.3 |
| 128 | 53.8 | 7.58 | 563 | 8.41 | 0.7 | 360 | Sulfur | N/A | 53.3 |
| 143 | 50.7 | 7.63 | 563 | 8.44 | - | 360 | Sulfur | N/A | 53.3 |
| 170 | 46.1 | 7.66 | 562 | 8.45 | 0.84 | 360 | Sulfur | N/A | 53.3 |
| 180 | 44.8 | 7.55 | 563 | 8.47 | 0.98 | 360 | Sulfur | N/A | 48 |
| 218 | 41.3 | 7.45 | 568 | 8.68 | 0.98 | 364 | Sulfur | N/A | 42 |
| 228 | 38.4 | 7.38 | 570 | 8.6 | 0.32 | 365 | Sulfur | N/A | 42 |
| 266 | 17.7 | 7.48 | 573 | 8.57 | 0.93 | 367 | Sulfur | N/A | 42 |
| 285 | 18 | 7.55 | 584 | 8.54 | 0.36 | 374 | Sulfur | N/A | 42 |
| 300 | 12.8 | 7.6 | 584 | 8.51 | 0.37 | 374 | Sulfur | N/A | 42 |
| 334 | 12.4 | 7.7 | 583 | 8.51 | 0.37 | 373 | Sulfur | N/A | 42 |
| 355 | 10.1 | 7.7 | 577 | 8.52 | - | 369 | Sulfur | N/A | 42 |
| 368 | 7.8 | 7.73 | 574 | 8.56 | 1 | 368 | Sulfur | N/A | 42 |
| 376 | 7.3 | 7.75 | 572 | 8.56 | - | 366 | Sulfur | N/A | 42 |
| 413 | 6.6 | 7.87 | 572 | 8.64 | 0.33 | 0.366 | Sulfur | N/A | 42 |
| Notes: | | | | | | | | | |

(us/cm) Microsiemens per centimetre
(°C) Degrees celsius
mg/L Milligrams per litre
L/min Litres per minute
N/A Not Analyzed

Table 2
Summary of Laboratory Water Quality Results
273-275 Russ Bradley Road

| Sample ID | Units | MDL | ODWSOG | Limit Type | TW1-1 | TW1-2 |
|-----------------------------------|------------|-------|----------------------------|------------|------------|------------|
| Sample Date | | | | | 13-Sep-22 | 13-Sep-22 |
| Location | | | | | | |
| Parameter: | | | | | | |
| <i>Microbiological Parameters</i> | | | | | | |
| E. Coli | CFU/100 mL | 1 | 0 CFU/100 mL (0 CFU/100mL) | MAC | ND (1) | ND (1) |
| Fecal Coliforms | CFU/100 mL | 1 | - | - | ND (1) | ND (1) |
| Total Coliforms | CFU/100 mL | 1 | 0 CFU/100 mL (0 CFU/100mL) | MAC | ND (1) | ND (1) |
| <i>General Inorganics</i> | | | | | | |
| Alkalinity (as CaCO3) | mg/L | 5 | 500 mg/L | OG | 187 | 186 |
| Ammonia as N (N-NH3) | mg/L | 0.01 | - | - | 0.11 | 0.07 |
| Dissolved Organic Carbon (DOC) | mg/L | 0.5 | 5 mg/L | AO | 1.4 | 1.5 |
| Colour | TCU | 2 | 5 TCU | AO | 4 | ND (2) |
| Conductivity | uS/cm | 5 | - | - | 499 | 509 |
| Hardness | mg/L | 0.824 | 100 mg/L | OG | 271 | 265 |
| pH | pH Units | 0.1 | - | - | 7.9 | 7.9 |
| Phenols | mg/L | 0.001 | - | - | ND (0.001) | ND (0.001) |
| Total Dissolved Solids | mg/L | 10 | 500 mg/L | AO | 278 | 270 |
| Sulphide (S2) | mg/L | 0.02 | 0.05 mg/L | AO | 3.14 | 3.36 |
| Tannin & Lignin | mg/L | 0.1 | 0.05 mg/L | AO | ND (0.1) | ND (0.1) |
| Total Kjeldahl Nitrogen | mg/L | 0.1 | - | - | 0.1 | ND (0.1) |
| Turbidity | NTU | 0.1 | 5 NTU | AO | 34.8 | 3.3 |
| <i>Anions</i> | | | | | | |
| Chloride (Cl) | mg/L | 1 | 250 mg/L | AO | 22.5 | 25.6 |
| Fluoride (F) | mg/L | 0.1 | 1.5 mg/L | MAC | 0.8 | 1.4 |
| Nitrate as N (N-NO3) | mg/L | 0.1 | 10 mg/L | MAC | ND (0.1) | ND (0.1) |
| Nitrite as N (N-NO2) | mg/L | 0.05 | 1 mg/L | MAC | ND (0.05) | ND (0.05) |
| Phosphate as P | mg/L | 0.2 | - | - | ND (0.2) | 0.3 |
| Sulphate (SO4) | mg/L | 1 | 500 mg/L | AO | 35.1 | 34.3 |
| <i>Metals</i> | | | | | | |
| Mercury | ug/L | 0.1 | 0.001 mg/L (1 ug/L) | MAC | ND (0.1) | ND (0.1) |
| Aluminum | ug/L | 1 | 0.1 mg/L (100 ug/L) | AO | 680 | 140 |
| Antimony | ug/L | 0.5 | 0.006 mg/L (6 ug/L) | MAC | ND (0.5) | ND (0.5) |
| Arsenic | ug/L | 1 | 0.01 mg/L (10 ug/L) | MAC | ND (1) | ND (1) |
| Barium | ug/L | 1 | 2 mg/L (2000 ug/L) | MAC | 295 | 277 |
| Beryllium | ug/L | 0.5 | - | - | ND (0.5) | ND (0.5) |
| Boron | ug/L | 10 | 5 mg/L (5000 ug/L) | MAC | 89 | 94 |
| Cadmium | ug/L | 0.1 | 0.007 mg/L (7 ug/L) | MAC | ND (0.1) | ND (0.1) |
| Calcium | ug/L | 100 | - | - | 72,200 | 71,600 |
| Chromium | ug/L | 1 | 0.05 mg/L (50 ug/L) | MAC | 2 | ND (1) |
| Cobalt | ug/L | 0.5 | - | - | ND (0.5) | ND (0.5) |
| Copper | ug/L | 0.5 | 1 mg/L (1000 ug/L) | AO | ND (0.5) | ND (0.5) |
| Iron | ug/L | 100 | 0.3 mg/L (300 ug/L) | AO | 820 | 139 |
| Lead | ug/L | 0.1 | 0.005 mg/L (5 ug/L) | MAC | 0.2 | ND (0.1) |
| Magnesium | ug/L | 200 | - | - | 22,100 | 21,000 |
| Manganese | ug/L | 5 | 0.05 mg/L (50 ug/L) | AO | 19 | 6 |
| Molybdenum | ug/L | 0.5 | - | - | ND (0.5) | ND (0.5) |
| Nickel | ug/L | 1 | - | - | ND (1) | ND (1) |
| Potassium | ug/L | 100 | - | - | 5,330 | 4,940 |
| Selenium | ug/L | 1 | 0.05 mg/L (50 ug/L) | MAC | ND (1) | ND (1) |
| Silver | ug/L | 0.1 | - | - | ND (0.1) | ND (0.1) |
| Sodium | ug/L | 200 | 20 mg/L (20,000 ug/L) | AO | 22,700 | 24,100 |
| Strontium | ug/L | 10 | 7 mg/L (7000 ug/L) | MAC | 3120 | 3290 |
| Thallium | ug/L | 0.1 | - | - | ND (0.1) | ND (0.1) |
| Tin | ug/L | 5 | - | - | ND (5) | ND (5) |
| Titanium | ug/L | 5 | - | - | 77 | 15 |
| Tungsten | ug/L | 10 | - | - | ND (10) | ND (10) |
| Uranium | ug/L | 0.1 | 0.02 mg/L (20 ug/L) | MAC | 0.1 | ND (0.1) |
| Vanadium | ug/L | 0.5 | - | - | 2.5 | ND (0.5) |
| Zinc | ug/L | 5 | 5 mg/L (5000 ug/L) | AO | 9 | ND (5) |
| <i>Volatiles</i> | | | | | | |
| Acetone | ug/L | 5.0 | - | - | N/A | ND (5.0) |
| Benzene | ug/L | 0.5 | 0.001 mg/L (1 ug/L) | MAC | N/A | ND (0.5) |
| Bromodichloromethane | ug/L | 0.5 | - | - | N/A | ND (0.5) |
| Bromoform | ug/L | 0.5 | - | - | N/A | ND (0.5) |
| Bromomethane | ug/L | 0.5 | - | - | N/A | ND (0.5) |
| Carbon Tetrachloride | ug/L | 0.2 | 0.002 mg/L (2 ug/L) | MAC | N/A | ND (0.2) |
| Chlorobenzene | ug/L | 0.5 | 0.08 mg/L (80 ug/L) | MAC | N/A | ND (0.5) |

Table 2
Summary of Laboratory Water Quality Results
273-275 Russ Bradley Road

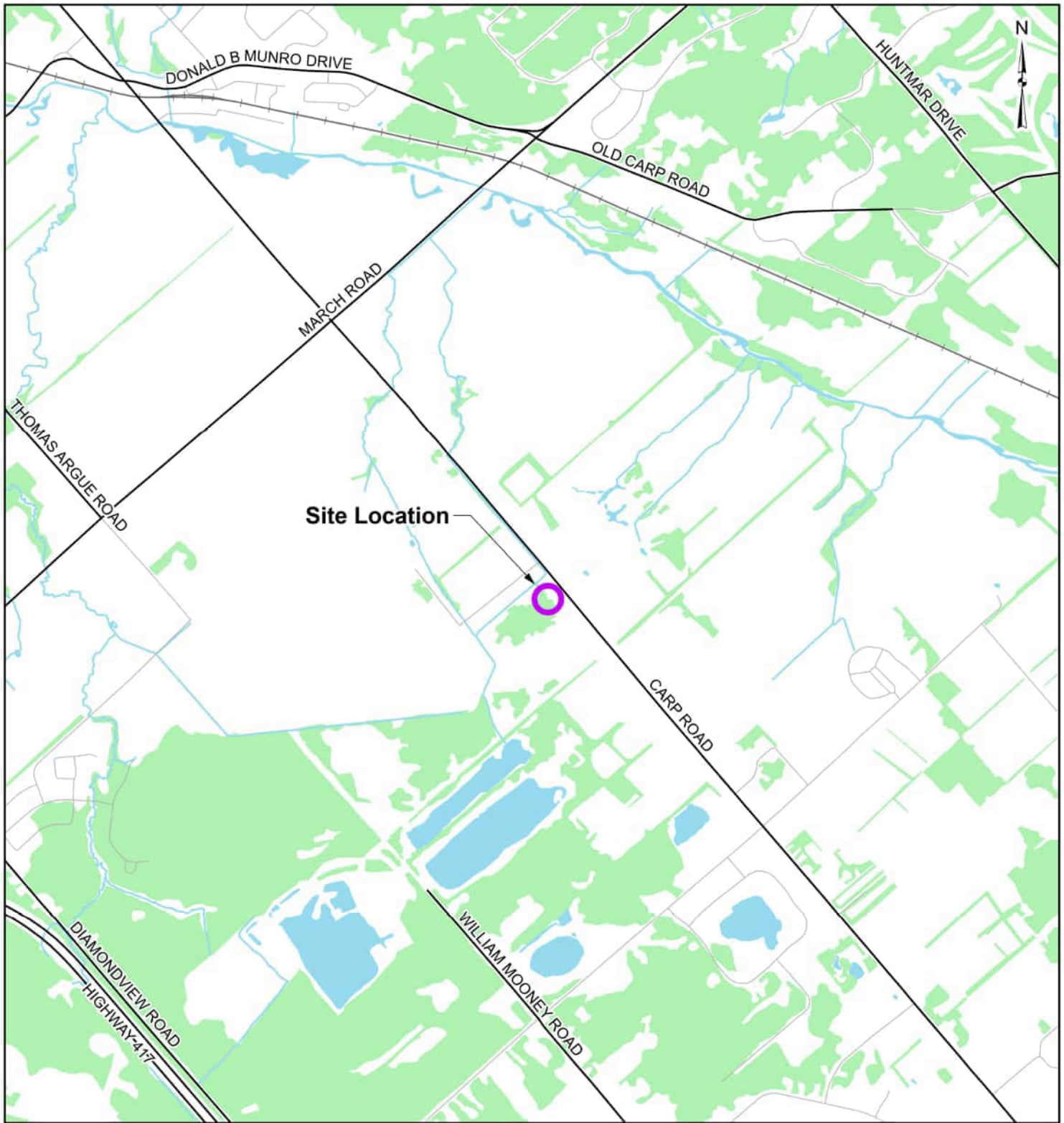
| Sample ID | Units | MDL | ODWSOG | Limit Type | TW1-1 | TW1-2 |
|---|-------|------|----------------------|------------|-----------|-----------|
| Sample Date | | | | | 13-Sep-22 | 13-Sep-22 |
| Location | | | | | | |
| Parameter: | | | | | | |
| Chloroethane | ug/L | 1.0 | | | N/A | ND (1.0) |
| Chloroform | ug/L | 0.5 | | | N/A | ND (0.5) |
| Chloromethane | ug/L | 3.0 | | | N/A | ND (3.0) |
| Dibromochloromethane | ug/L | 0.5 | | | N/A | ND (0.5) |
| Dichlorodifluoromethane | ug/L | 1.0 | | | N/A | ND (1.0) |
| Ethylene dibromide (dibromoethane, 1,2) | ug/L | 0.2 | | | N/A | ND (0.2) |
| 1,2-Dichlorobenzene | ug/L | 0.5 | 0.2 mg/L (200 ug/L) | MAC | N/A | ND (0.5) |
| 1,3-Dichlorobenzene | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,4-Dichlorobenzene | ug/L | 0.5 | 0.005 mg/L (5 ug/L) | MAC | N/A | ND (0.5) |
| 1,1-Dichloroethane | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,2-Dichloroethane | ug/L | 0.5 | 0.005 mg/L (5 ug/L) | MAC | N/A | ND (0.5) |
| 1,1-Dichloroethylene | ug/L | 0.5 | 0.014 mg/L (14 ug/L) | MAC | N/A | ND (0.5) |
| cis-1,2-Dichloroethylene | ug/L | 0.5 | | | N/A | ND (0.5) |
| trans-1,2-Dichloroethylene | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,2-Dichloroethylene, total | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,2-Dichloropropane | ug/L | 0.5 | | | N/A | ND (0.5) |
| cis-1,3-Dichloropropylene | ug/L | 0.5 | | | N/A | ND (0.5) |
| trans-1,3-Dichloropropylene | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,3-Dichloropropene, total | ug/L | 0.5 | | | N/A | ND (0.5) |
| Ethylbenzene | ug/L | 0.5 | 0.14 mg/L (140 ug/L) | MAC | N/A | ND (0.5) |
| Hexane | ug/L | 1.0 | | | N/A | ND (1.0) |
| Methyl Ethyl Ketone (2-Butanone) | ug/L | 5.0 | | | N/A | ND (5.0) |
| Methyl Butyl Ketone (2-Hexanone) | ug/L | 10.0 | | | N/A | ND (10.0) |
| Methyl Isobutyl Ketone | ug/L | 5.0 | | | N/A | ND (5.0) |
| Methyl tert-butyl ether | ug/L | 2.0 | | | N/A | ND (2.0) |
| Methylene Chloride | ug/L | 5.0 | 0.05 mg/L (50 ug/L) | MAC | N/A | ND (5.0) |
| Styrene | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,1,1,2-Tetrachloroethane | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,1,2,2-Tetrachloroethane | ug/L | 0.5 | | | N/A | ND (0.5) |
| Tetrachloroethylene | ug/L | 0.5 | 0.01 mg/L (10 ug/L) | MAC | N/A | ND (0.5) |
| Toluene | ug/L | 0.5 | 0.06 mg/L (60 ug/L) | MAC | N/A | ND (0.5) |
| 1,1,1-Trichloroethane | ug/L | 0.5 | | | N/A | ND (0.5) |
| 1,1,2-Trichloroethane | ug/L | 0.5 | | | N/A | ND (0.5) |
| Trichloroethylene | ug/L | 0.5 | 0.005 mg/L (5 ug/L) | MAC | N/A | ND (0.5) |
| Trichlorofluoromethane | ug/L | 1.0 | | | N/A | ND (1.0) |
| 1,3,5-Trimethylbenzene | ug/L | 0.5 | | | N/A | ND (0.5) |
| Vinyl Chloride | ug/L | 0.5 | 0.001 mg/L (1 ug/L) | MAC | N/A | ND (0.5) |
| m/p-Xylene | ug/L | 0.5 | | | N/A | ND (0.5) |
| o-Xylene | ug/L | 0.5 | | | N/A | ND (0.5) |
| Xylenes, total | ug/L | 0.5 | 0.09 mg/L (90 ug/L) | MAC | N/A | ND (0.5) |

| | |
|------------|--|
| Notes: | |
| 1050 | Exceeds Ontario Drinking Water Standards, Objectives, and Guidelines |
| 21 | Exceeds health warning limit for sodium (20 mg/L) |
| MDL | Method Detection Limit |
| ODWSOG | Ontario Drinking Water Standards, Objectives, and Guidelines (MOECC, 2003 rev. 2006; PIBs 4449e01) |
| AO | Aesthetic Objective |
| MAC | Maximum Allowable Concentration (Health-Related Parameter) |
| OG | Operational Guideline |
| ug/L | Micrograms per litre |
| mg/L | Milligrams per litre |
| TCU | True Colour Units |
| uS/cm | Microsems per centimeter |
| NTU | Nephelometric Turbidity Units |
| CFU/100 mL | Colony-forming units (bacteria) per 100 mL |








HYDROGEOLOGICAL ASSESSMENT AND TERRAIN
ANALYSIS
273-275 RUSS BRADLEY ROAD, CARP, ONTARIO

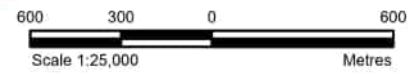


FIGURES



LEGEND

-  Site Location
-  Local Road
-  Major Road
-  Railroad
-  Watercourse
-  Waterbody
-  Wooded Area



REFERENCE

GIS data provided by the Ontario Ministry of Northern Development, Mines, Natural Resources and Forestry, 2022.



| | | | |
|----------------------------|----------------|---|----------|
| CLIENT: | | TREVOR WATKINS | |
| PROJECT: | | HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS | |
| TITLE: | | SITE LOCATION | |
| PROJECT NO: CCO-22-1643-01 | | FIGURE: | 1 |
| Date | Sep., 27, 2022 | | |
| GIS | AH | | |
| Checked By | RL | | |

McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A1L0
 Tel: 613-436-2184 Fax: 613-436-3742
 www.mcintoshperry.com

C:\Users\james.mclintosh\Documents\Projects\2022\CCO\CCO-22-1643-Trevor Watkins - EIS - 1600-Thornton-Arquivalenced\Environmental\CCO-22-1643 - Hydrogeological Assessment and Terrain Analysis.mxd



LEGEND

-  Borehole Locations
-  Site Boundary

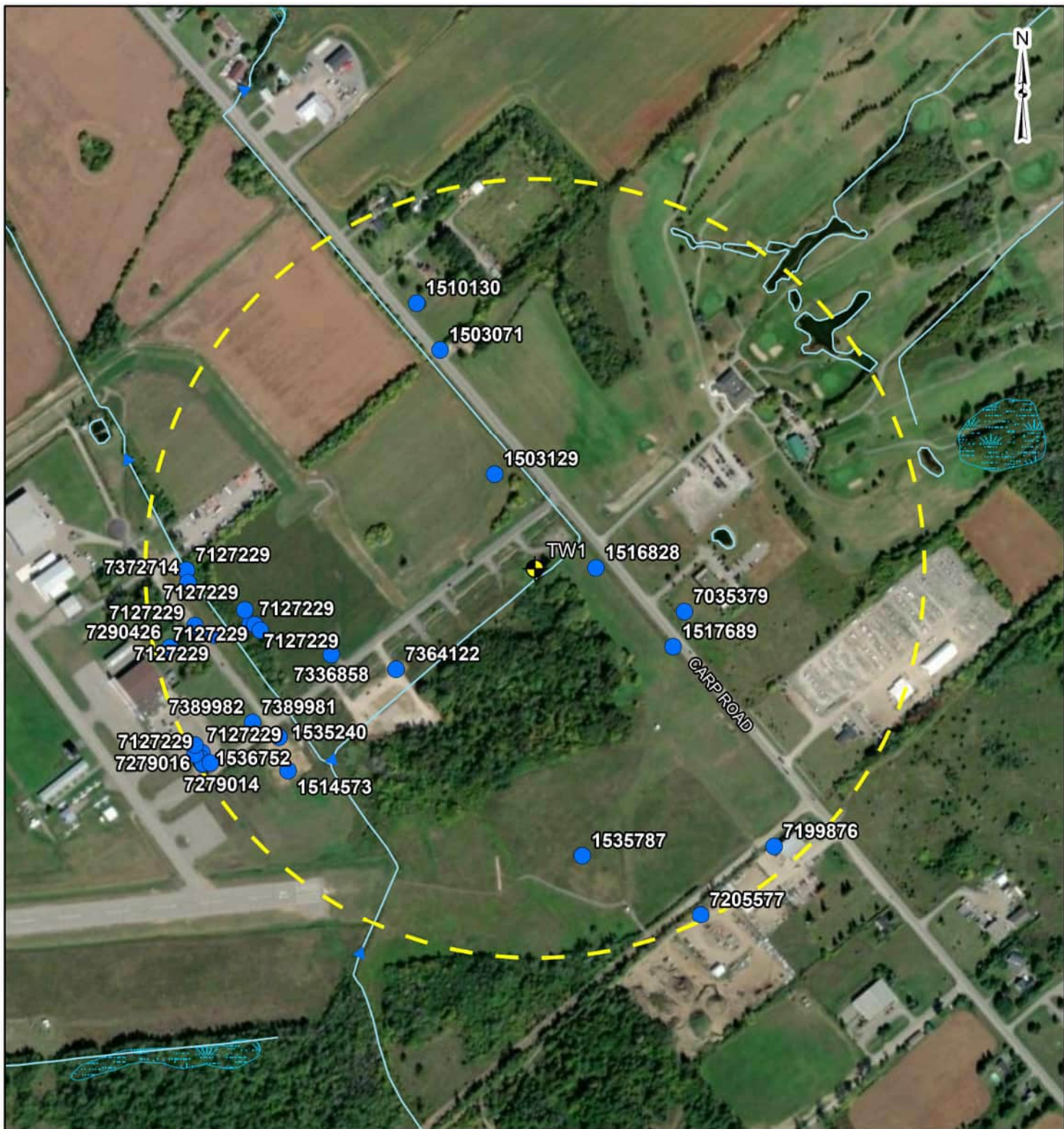
REFERENCE

GIS data provided by the Ontario Ministry of Northern Development, Mines, Natural Resources and Forestry, 2022.



| | | | |
|----------------------------|----------------|--|--|
| CLIENT: | | TREVOR WATKINS | |
| PROJECT: | | HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS | |
| TITLE: | | SITE LAYOUT | |
| PROJECT NO: CCO-22-1643-01 | | FIGURE: | |
| Date | Sep., 27, 2022 | 2 | |
| GIS | AH | | |
| Checked By | RL | | |

McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A1L0
 Tel: 613-838-2184 Fax: 613-838-3742
 www.mcintoshperry.com



LEGEND

-  Borehole Locations
-  MECP Well Location
-  500m Buffer
-  Watercourse
-  Unevaluated Wetland
-  Waterbody



REFERENCE

GIS data provided by the Ontario Ministry of Northern Development, Mines, Natural Resources and Forestry, 2022.

| | | | |
|----------------------------|----------------|--|--|
| CLIENT: | | TREVOR WATKINS | |
| PROJECT: | | HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS | |
| TITLE: | | MECP WATER WELL INFORMATION SYSTEM | |
| PROJECT NO: CCO-22-1643-01 | | FIGURE: | |
| Date | Sep., 27, 2022 | 3 | |
| GIS | AH | | |
| Checked By | RL | | |

McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A1L0
 Tel: 613-839-2184 Fax: 613-839-3742
 www.mcintoshperry.com

C:\Users\hamed\Documents\Projects\2022\CCO\CCO-22-1643 Trevor Watkins - EIS - 1600_Threats_AirQualityandEnvironmental\CCO-22-1643_MECPWellRecords.mxd



LEGEND

 Site Boundary



REFERENCE

GIS data provided by the Ontario Ministry of Northern Development, Mines, Natural Resources and Forestry, 2022.

| | | | |
|----------------------------|----------------|---|--|
| CLIENT: | | TREVOR WATKINS | |
| PROJECT: | | HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS | |
| TITLE: | | TEST HOLE LOCATIONS | |
| PROJECT NO: CCO-22-1643-01 | | FIGURE: | |
| Date | Nov., 28, 2022 | 4 | |
| GIS | AH | | |
| Checked By | RL | | |

McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A1L0
 Tel: 613-838-2184 Fax: 613-838-3742
 www.mcintoshperry.com

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS

273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX A: MECP WATER WELL INFORMATION SYSTEM DATA

| WELL ID | COMPLETION DATE | WELL DEPTH (ft) | STATIC WATER LEVEL (ft) | DEPTH TO BEDROCK (ft) | BORING HEAD SIZE (in) | FINAL STATUS | DOMESTIC USE | USE | FAHRENHEIT | NOVEMBER | PH | PUMP TEST | PUMPING RATE | FLOWING RATE | RECORD RATE | WATER STATE AT THE TEST | PUMP AND INCH | PUMPING DURATION (hr) |
|---------|-----------------|-----------------|-------------------------|-----------------------|-----------------------|------------------|--------------|-------------|------------|----------|-----------|-----------|--------------|--------------|-------------|-------------------------|---------------|-----------------------|
| 156019 | 14-Jun-58 | 41 | 15.1 | 41.1 | 100211 | Water Supply | Domestic | Impingement | 42006.5 | 50195.1 | 10573.64 | 50.58 BOP | 10 GPM | 30 GPM | | Cloudy | PUMP | 1h |
| 156020 | 14-Jun-58 | 47 | 8.5 | 46.3 | 100212 | Water Supply | Domestic | | 42006.5 | 50195.1 | 10573.64 | 26.45 ft | 3 GPM | 3 GPM | CLEAR | PUMP | 2h | |
| 156021 | 17-Jul-58 | 41 | 9.9 | 39.9 | 100213 | Water Supply | Domestic | | 42006.5 | 50195.1 | 10573.64 | 32.46 ft | 20 GPM | 20 GPM | CLEAR | PUMP | 1h | |
| 156022 | 13-Feb-75 | 53.3 | 37.5 | 15.8 | 100214 | Water Supply | Domestic | | 42045.5 | 50191.62 | 105815.18 | 18.36 BOP | 20 GPM | 20 GPM | CLOUDY | BAUER | 2h | |
| 156023 | 01-Jul-71 | 44.2 | 12.3 | 19.7 | 100215 | Water Supply | Domestic | | 42006.5 | 50195.1 | 10573.64 | 45.56 ft | 20 GPM | 20 GPM | CLEAR | PUMP | 1h | |
| 156024 | 11-Nov-81 | 24.1 | 46.5 | 24.1 | 100216 | Water Supply | Domestic | | 42006.5 | 50195.1 | 105811.18 | 200.00 ft | 140 GPM | 140 GPM | CLEAR | PUMP | 1h | |
| 156025 | 26-Sep-05 | 47.9 | 47.9 | 36.7 | 1112200 | Observation Well | Not Used | | 42044 | 501920 | 1112200 | | | | | | PUMP | 1h |
| 156026 | 20-Sep-05 | 27.4 | 5.9 | 0 | 1112120 | Water Supply | Municipal | PUBLIC | 42061.3 | 50195.1 | 11121181 | 5.91 m | 480 LPM | 480 LPM | CLEAR | PUMP | 1h | |
| 156027 | 10-Jul-05 | 3.7 | 0 | 0 | 1148184 | Observation Well | Not Used | | 42026 | 501972 | 1148172 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 1118201 | 0 | Monitoring | | | 42094 | 501964 | 1118201 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42025 | 501965 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42023 | 501978 | 10020434 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42087 | 501964 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42093 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42036 | 501949 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501936 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 42016 | 501948 | 10020465 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42038 | 501958 | 10020702 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020465 | Test Hole | Monitoring | | | 41927 | 501965 | 10020706 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | 10020434 | Test Hole | Monitoring | | | 42040 | 501942 | 10020464 | | | | | | | |
| 712229 | 15-Jun-09 | 0 | 0 | | | | | | | | | | | | | | | |

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS

273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX B: WELL RECORD (TW1)

CERTIFICATE OF WELL COMPLIANCE



I (**Jeremy Hanna**) **AIR ROCK DRILLING CO. LTD.** - DO HEREBY CERTIFY

that I am licensed to drill water wells in the Province of Ontario, and that I have supervised the drilling of the water well on the property of :

OWNER: 2852569 ONTARIO INC (Trevor Watkins)

Location: *273-275 Russ Bradley Road, Carp

LOT: Block 15 - 16 CON: PLAN # 4R-1511 S/L # X

Ottawa-Carleton / Geographical Township of MARCH

I CERTIFY FURTHER that, I am aware of the well drilling requirements, the guidelines, recommendations and regulations of the Ministry of the Environment governing well installations in the Province of Ontario, and the standards specified in any subdivision agreement and hydrogeological report applicable to this site and City Standards.

AND DO HEREBY CERTIFY THAT the said well has been drilled, cased, grouted (cement or bentonite) as applicable and constructed in strict conformity with the standards required.

Signed this 5TH Day of JULY, 2022

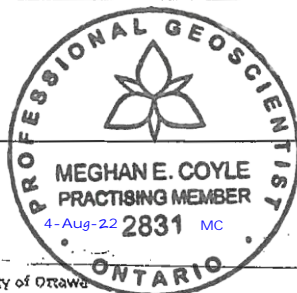
Jeremy Hanna (T3632)

Air Rock Drilling Co. Ltd. (C-7681)

The Engineer on behalf of the Landowner set out above, Certifies that he/she has inspected the well and it was constructed in accordance with the specifications in O.Reg 903, this report and the Hydrogeological Report with regards to casing length and grouting requirements.

Signed this 4th day of August, 2022

(Engineer)



202464
TAR A342436



Measurements recorded in: Metric Imperial

Well Owner's Information

First Name: _____ Last Name/Organization: **2852569 Ontario Inc. C/O Trevor Watkins** E-mail Address: _____ Well Constructed by Well Owner

Mailing Address (Street Number/Name): **971 Melrose Road** Municipality: **Shannonville** Province: **ON** Postal Code: **K0K 3A0** Telephone No. (inc. area code): _____

Well Location

Address of Well Location (Street Number/Name): **273-275 Russ Bradley Road** Township: **March** Lot: **Block 15-16** Concession: **X**

County/District/Municipality: **Ottawa Carleton** City/Town/Village: **Carp** Province: **Ontario** Postal Code: _____

UTM Coordinates Zone: **18** Easting: **420743** Northing: **5019433** Municipal Plan and Sublot Number: **4R-1511**

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

| General Colour | Most Common Material | Other Materials | General Description | Depth (m) |
|----------------|----------------------|-----------------|---------------------|-----------|
| | | | | From To |
| Blue | Clay | Mixed w/ Gravel | | 0 136 |
| Grey & Black | Limestone | | | 136 200 |
| Grey & Black | Limestone | | | 200 500 |

Annular Space

| Depth Set at (m) | Type of Sealant Used (Material and Type) | Volume Placed (m³) |
|------------------|--|--------------------|
| 142' - 132' | Neat cement | 12.48 |
| 132' - 0' | Bentonite slurry | 29.4 |

Results of Well Yield Testing

| Time (min) | Draw Down | | Recovery | |
|--------------|-----------------|-----------------|------------|-----------------|
| | Water Level (m) | Water Level (m) | Time (min) | Water Level (m) |
| Static Level | 331' | | | 150' |
| 1 | 36.5 | | 1 | 142 |
| 2 | 39.7 | | 2 | 140 |
| 3 | 42.8 | | 3 | 138 |
| 4 | 45.6 | | 4 | 137 |
| 5 | 48.4 | | 5 | 135 |
| 10 | 63.2 | | 10 | 126 |
| 15 | 77.3 | | 15 | 119 |
| 20 | 85.9 | | 20 | 114 |
| 25 | 95.2 | | 25 | 108 |
| 30 | 104 | | 30 | 102 |
| 40 | 122 | | 40 | 92.7 |
| 50 | 135 | | 50 | 84.5 |
| 60 | 150 | | 60 | 75.1 |

After test of well yield, water was:
 Clear and sand free
 Other, specify: **Not tested**

If pumping discontinued, give reason: **X**

Pump intake set at (m): **300**

Pumping rate (l/min/GPM): **5 US**

Duration of pumping: **1 hrs + 0 min**

Final water level end of pumping (m): **150**

If flowing give rate (l/min/GPM): **X**

Recommended pump depth (m): **400**

Recommended pump rate (l/min/GPM): **5 GPM**

Well production (l/min/GPM): **5**

Disturbed? Yes No

Method of Construction: Cable Tool Rotary (Conventional) Rotary (Reverse) Driving Air Percussion Other, specify: **Surged X3**

Well Use: Domestic Livestock Industrial Other, specify: _____

Construction Record - Casing

| Inside Diameter (cm) | Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel) | Wall Thickness (cm) | Depth (m) |
|----------------------|--|---------------------|-----------|
| | | | From To |
| 6 1/4" | Steel | .188" | +2' 142' |
| 6" | Open Hole | | 142' 500' |

Status of Well: Water Supply Replacement Well Test Hole Recharge Well Dewatering Well Observation and/or Monitoring Hole Alteration (Construction) Abandoned, Insufficient Supply Abandoned, Poor Water Quality Abandoned, other, specify: _____ Other, specify: _____

Construction Record - Screen

| Outside Diameter (cm) | Material (Plastic, Galvanized, Steel) | Slot No. | Depth (m) |
|-----------------------|---------------------------------------|----------|-----------|
| | | | From To |
| | | | |

Water Details

| Water found at Depth (m) | Kind of Water: <input type="checkbox"/> Fresh <input checked="" type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify: _____ |
|--------------------------|--|
| 200' | |
| | |
| | |

Hole Diameter

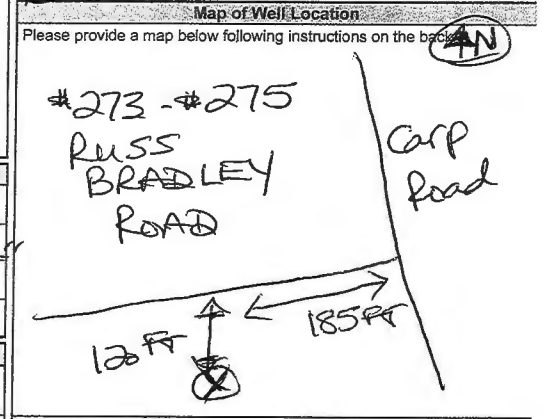
| Depth (m) | Diameter (cm) |
|-----------|---------------|
| From To | |
| 0' 142' | 9 3/4" |
| 142' 500' | 6" |

Well Contractor and Well Technician Information

Business Name of Well Contractor: **Air Rock Drilling Co. Ltd.** Well Contractor's Licence No.: **C17881**

Business Address (Street Number/Name): **8850 Franktown Road** Municipality: **Richmond**

Province: **ON** Postal Code: **N0A 2Z0** Business E-mail Address: **air-rock@sympatico.ca**



Business Name of Well Technician: **Hanna, Jeremy** Date Submitted: **2022-07-31**

Well Contractor's Licence No.: **13892** Signature of Technician and/or Contractor: _____

Comments: **1 HP 5 GPM SET AT 400 FEET**

Well owner's information package delivered: Yes No

Date Package Delivered: **2022-07-11**

Ministry Use Only: Audit No. **2379045**

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS

273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX C: CERTIFICATE OF CALIBRATION



CERTIFICATE OF CALIBRATION

The HORIBA Instrument listed below has been inspected and calibrated following the Manufacturer's specifications and methods.

Instrument Model: **HORIBA U-52** Serial Number: **R86W200F** Calibration Date: **September 12, 2022**

| <u>2-POINT pH</u> | <u>CONDUCTIVITY</u> | <u>TURBIDITY</u> | <u>DISSOLVED OXYGEN</u> | <u>OXIDIZATION-REDUCTION POTENTIAL</u> | <u>TEMPERATURE</u> |
|---|-------------------------------------|------------------------------------|--|--|--|
| 4.00 pH, 7.00 pH | 4.49mS/cm ZERO CHECKED | 0 & 100 NTU | 9.09 mg/L @ 20 DegC SODIUM SULFITE ZERO | 240mV | Fisher Scientific s/n 210412377 exp: May 18/2023 |
| AutoCal 4.00 pH Solution LOT # 2GE898 | AutoCal Solution LOT # 2GE898 | AutoCal Solution LOT# 2GE898 | Oakton Zero Solution LOT # 754262 | Hanna ORP LOT # 5768 | |
| Expiry Date: May 31, 2023 | Expiry Date: May 31, 2023 | Expiry Date: May 31, 2023 | Expiry Date: May 1, 2023 | Expiry Date: October 1, 2025 | |
| pH 7.00 LOT # 1GF003 | @25 DegC LOT # 1GF256 | Turb. 100 NTU LOT # A2018 | | | |
| Expiry Date: June 1, 2023 | Expiry Date: May 31, 2023 | Expiry Date: February 28, 2024 | | | |

The calibration standard used is considered to be a certified standard and is traceable to the National Institute of Standards and Technology (NIST). Certificate of Analysis is available upon request.

The instrument indicated above is now certified to be operating within the Manufacturer's specifications. This does not eliminate the requirement for regular maintenance and pre-use sensor response checks in order to ensure continued complete and accurate operating condition.

Certified By: Jeff Loney

Maxim Environmental and Safety Inc.

sales@maximenvironmental.com
www.maximenvironmental.com



Head Office:
9 - 170 Ambassador Dr., Mississauga, ON L5T 2H9
(905)670-1304 | Toll Free (888)285-2324

Ottawa Office:
9 - 148 Colonnade Rd., Ottawa, ON K2E 7R4
(613)224-4747 | Toll Free (888)285-2324

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN
ANALYSIS
273-275 RUSS BRADLEY ROAD, CARP, ONTARIO

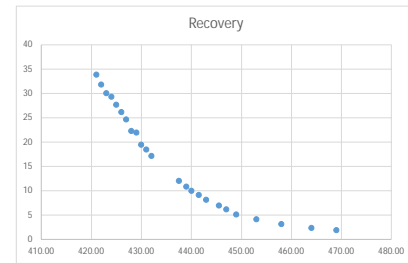
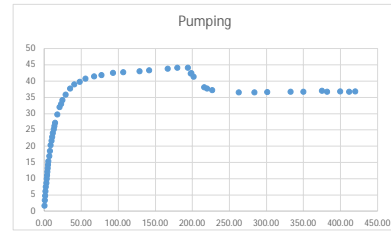


APPENDIX D: WATER LEVEL DATA AND PUMPING TEST ANALYSES

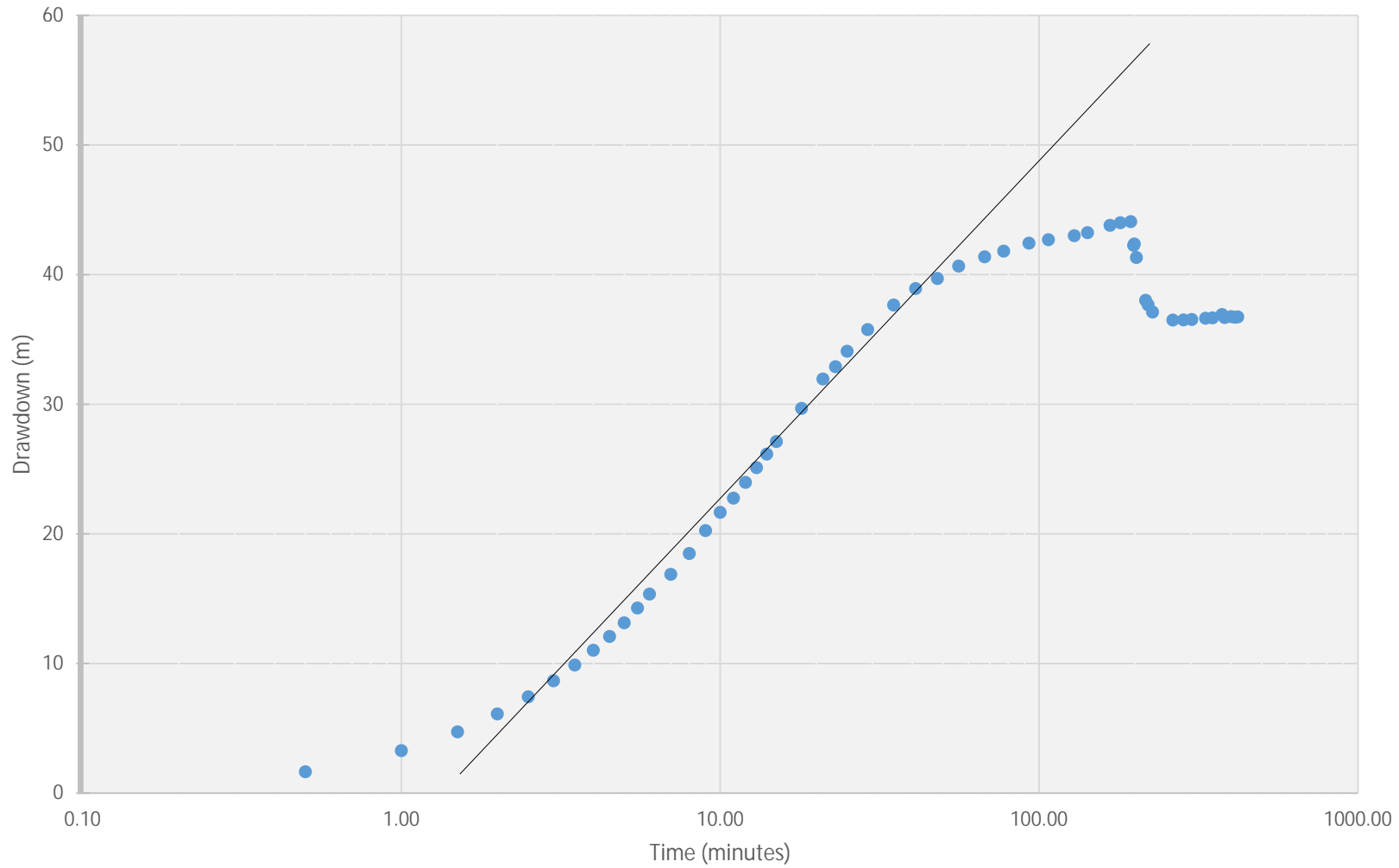
Summary of Water Level Data
Pumping Test - TW1 September 13, 2022

TOC Elevation (assumed) 100 m AD (Above Datum)
 Static Water Level 9.87 m BTOC
 Static Water Elevation 90.13 m AD (Above Datum)
 95% Recovery 12.07405 m BTOC
 87.92595 m AD (Above Datum)
 Well depth 152.4 m BTOC 500 FT
 Pump Depth 85.34 m BTOC 280 FT

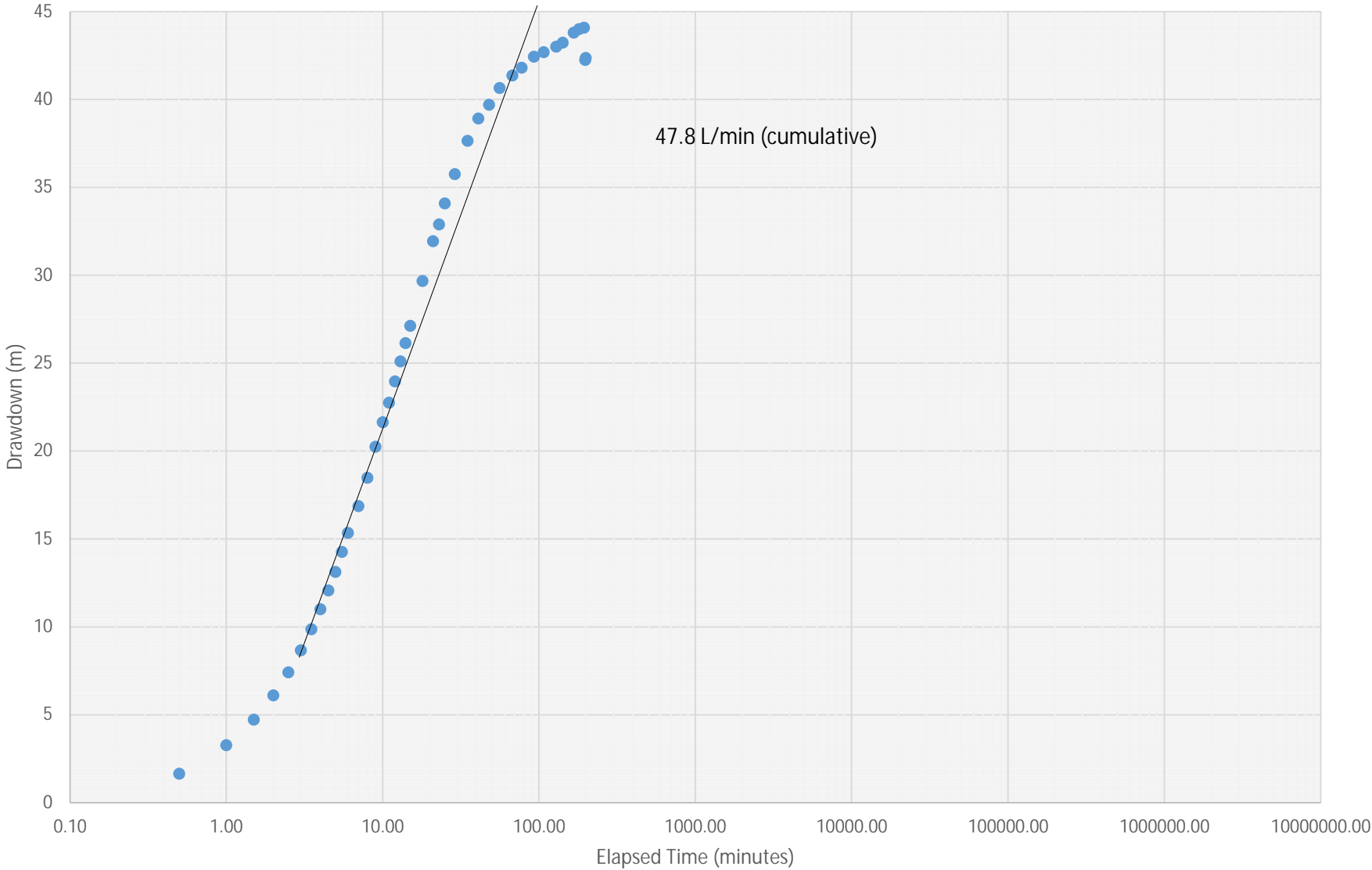
| Elapsed Time (minutes) | Water Level (m BTOC) | Elapsed Time after pump shut off (min) | T/t' | Water Level (m Datum) | Drawdown (m) | Water Column Remaining (m) | % Utilization | Notes |
|------------------------|----------------------|--|---------|-----------------------|--------------|----------------------------|---------------|------------------------|
| 0.00 | 9.870 | | | 90.13 | 0 | 75.47 | 0.0% | Pump on at 7:40am |
| 0.50 | 11.515 | | | 88.485 | 1.645 | 73.825 | 1.1% | 60 LPM until 35 min |
| 1.00 | 13.140 | | | 86.86 | 3.27 | 72.2 | 2.1% | 53.3 LPM until 192 min |
| 1.50 | 14.590 | | | 85.41 | 4.72 | 70.75 | 3.1% | 48 LPM until 195 min |
| 2.00 | 15.978 | | | 84.022 | 6.108 | 69.362 | 4.0% | 42 LPM until 420 min |
| 2.50 | 17.293 | | | 82.707 | 7.423 | 68.047 | 4.9% | |
| 3.00 | 18.535 | | | 81.465 | 8.665 | 66.805 | 5.7% | |
| 3.50 | 19.740 | | | 80.26 | 9.87 | 65.6 | 6.5% | |
| 4.00 | 20.880 | | | 79.12 | 11.01 | 64.46 | 7.2% | |
| 4.50 | 21.945 | | | 78.055 | 12.075 | 63.395 | 7.9% | |
| 5.00 | 23.000 | | | 77 | 13.13 | 62.34 | 8.6% | |
| 5.50 | 24.140 | | | 75.86 | 14.27 | 61.2 | 9.4% | |
| 6.00 | 25.218 | | | 74.782 | 15.348 | 60.122 | 10.1% | |
| 7.00 | 26.740 | | | 73.26 | 16.87 | 58.6 | 11.1% | |
| 8.00 | 28.345 | | | 71.655 | 18.475 | 56.995 | 12.1% | |
| 9.00 | 30.120 | | | 69.88 | 20.25 | 55.22 | 13.3% | |
| 10.00 | 31.518 | | | 68.482 | 21.648 | 53.822 | 14.2% | |
| 11.00 | 32.615 | | | 67.385 | 22.745 | 52.725 | 14.9% | |
| 12.00 | 33.833 | | | 66.167 | 23.963 | 51.507 | 15.7% | |
| 13.00 | 34.970 | | | 65.03 | 25.1 | 50.37 | 16.5% | |
| 14.00 | 36.015 | | | 63.985 | 26.145 | 49.325 | 17.2% | |
| 15.00 | 36.990 | | | 63.01 | 27.12 | 48.35 | 17.8% | |
| 18.00 | 39.540 | | | 60.46 | 29.67 | 45.8 | 19.5% | |
| 21.00 | 41.810 | | | 58.19 | 31.94 | 43.53 | 21.0% | |
| 23.00 | 42.760 | | | 57.24 | 32.89 | 42.58 | 21.6% | |
| 25.00 | 43.950 | | | 56.05 | 34.08 | 41.39 | 22.4% | |
| 29.00 | 45.620 | | | 54.38 | 35.75 | 39.72 | 23.5% | |
| 35.00 | 47.520 | | | 52.48 | 37.65 | 37.82 | 24.7% | |
| 41.00 | 48.785 | | | 51.215 | 38.915 | 36.555 | 25.5% | |
| 48.00 | 49.565 | | | 50.435 | 39.695 | 35.775 | 26.0% | |
| 56.00 | 50.525 | | | 49.475 | 40.655 | 34.815 | 26.7% | |
| 67.50 | 51.236 | | | 48.764 | 41.366 | 34.104 | 27.1% | |
| 77.50 | 51.673 | | | 48.327 | 41.803 | 33.667 | 27.4% | |
| 93.00 | 52.295 | | | 47.705 | 42.425 | 33.045 | 27.8% | |
| 107.00 | 52.560 | | | 47.44 | 42.69 | 32.78 | 28.0% | |
| 129.00 | 52.863 | | | 47.137 | 42.993 | 32.477 | 28.2% | |
| 142.00 | 53.097 | | | 46.903 | 43.227 | 32.243 | 28.4% | |
| 167.00 | 53.661 | | | 46.339 | 43.791 | 31.679 | 28.7% | |
| 180.00 | 53.865 | | | 46.135 | 43.995 | 31.475 | 28.9% | |
| 194.00 | 53.951 | | | 46.049 | 44.081 | 31.389 | 28.9% | |
| 198.00 | 52.115 | | | 47.885 | 42.245 | 33.225 | 27.7% | |
| 199.00 | 52.225 | | | 47.775 | 42.355 | 33.115 | 27.8% | |
| 202.00 | 51.187 | | | 48.813 | 41.317 | 34.153 | 27.1% | |
| 216.00 | 47.885 | | | 52.115 | 38.015 | 37.455 | 24.9% | |
| 220.00 | 47.525 | | | 52.475 | 37.655 | 37.815 | 24.7% | |
| 227.00 | 46.980 | | | 53.02 | 37.11 | 38.36 | 24.4% | |
| 263.00 | 46.360 | | | 53.64 | 36.49 | 38.98 | 23.9% | |
| 284.00 | 46.367 | | | 53.633 | 36.497 | 38.973 | 23.9% | |
| 301.50 | 46.405 | | | 53.595 | 36.535 | 38.935 | 24.0% | |
| 333.00 | 46.494 | | | 53.506 | 36.624 | 38.846 | 24.0% | |
| 350.00 | 46.530 | | | 53.47 | 36.66 | 38.81 | 24.1% | |
| 375.00 | 46.780 | | | 53.22 | 36.91 | 38.56 | 24.2% | |
| 382.00 | 46.550 | | | 53.45 | 36.68 | 38.79 | 24.1% | |
| 400.00 | 46.621 | | | 53.379 | 36.751 | 38.719 | 24.1% | |
| 412.00 | 46.568 | | | 53.432 | 36.698 | 38.772 | 24.1% | |
| 420.00 | 46.599 | | | 53.401 | 36.729 | 38.741 | 24.1% | Pump off at 2:40pm |
| 421.00 | 43.720 | 1.0 | 421.000 | 56.28 | 33.85 | 41.62 | 22.2% | |
| 422.00 | 41.700 | 2.000 | 211.000 | 58.3 | 31.83 | 43.64 | 20.9% | |
| 423.00 | 39.914 | 3.000 | 141.000 | 60.085 | 30.044 | 45.626 | 19.7% | |
| 424.00 | 39.190 | 4.000 | 106.000 | 60.81 | 29.32 | 46.15 | 19.2% | |
| 425.00 | 37.570 | 5.000 | 85.000 | 62.43 | 27.7 | 47.77 | 18.2% | |
| 426.00 | 36.024 | 6.000 | 71.000 | 63.976 | 26.154 | 49.316 | 17.2% | |
| 427.00 | 34.540 | 7.000 | 61.000 | 65.46 | 24.67 | 50.8 | 16.2% | |
| 428.00 | 32.153 | 8.000 | 53.500 | 67.847 | 22.283 | 53.187 | 14.6% | |
| 429.00 | 31.800 | 9.000 | 47.667 | 68.2 | 21.93 | 53.54 | 14.4% | |
| 430.00 | 29.346 | 10.000 | 43.000 | 70.654 | 19.476 | 55.994 | 12.8% | |
| 431.00 | 28.335 | 11.000 | 39.182 | 71.665 | 18.465 | 57.005 | 12.1% | |
| 432.00 | 27.000 | 12.000 | 36.000 | 73 | 17.13 | 58.34 | 11.2% | |
| 437.50 | 21.880 | 17.000 | 25.735 | 78.12 | 12.01 | 63.46 | 7.9% | |
| 439.00 | 20.710 | 19.000 | 23.105 | 79.29 | 10.84 | 64.63 | 7.1% | |
| 440.00 | 19.872 | 20 | 22.000 | 80.128 | 10.002 | 65.468 | 6.6% | |
| 441.50 | 19.000 | 21.5 | 20.535 | 81 | 9.13 | 66.34 | 6.0% | |
| 443.00 | 18.000 | 23 | 19.261 | 82 | 8.13 | 67.34 | 5.3% | |
| 445.50 | 16.785 | 25.5 | 17.471 | 83.215 | 6.915 | 68.555 | 4.5% | |
| 447.00 | 16.000 | 27 | 16.556 | 84 | 6.13 | 69.34 | 4.0% | |
| 449.00 | 15.000 | 29 | 15.483 | 85 | 5.13 | 70.34 | 3.4% | |
| 453.00 | 14.000 | 33 | 13.727 | 86 | 4.13 | 71.34 | 2.7% | |
| 458 | 13.000 | 38 | 12.053 | 87 | 3.13 | 72.34 | 2.1% | |
| 464 | 12.200 | 44 | 10.545 | 87.8 | 2.33 | 73.14 | 1.5% | |
| 469 | 11.765 | 49 | 9.571 | 88.235 | 1.895 | 73.575 | 1.2% | |



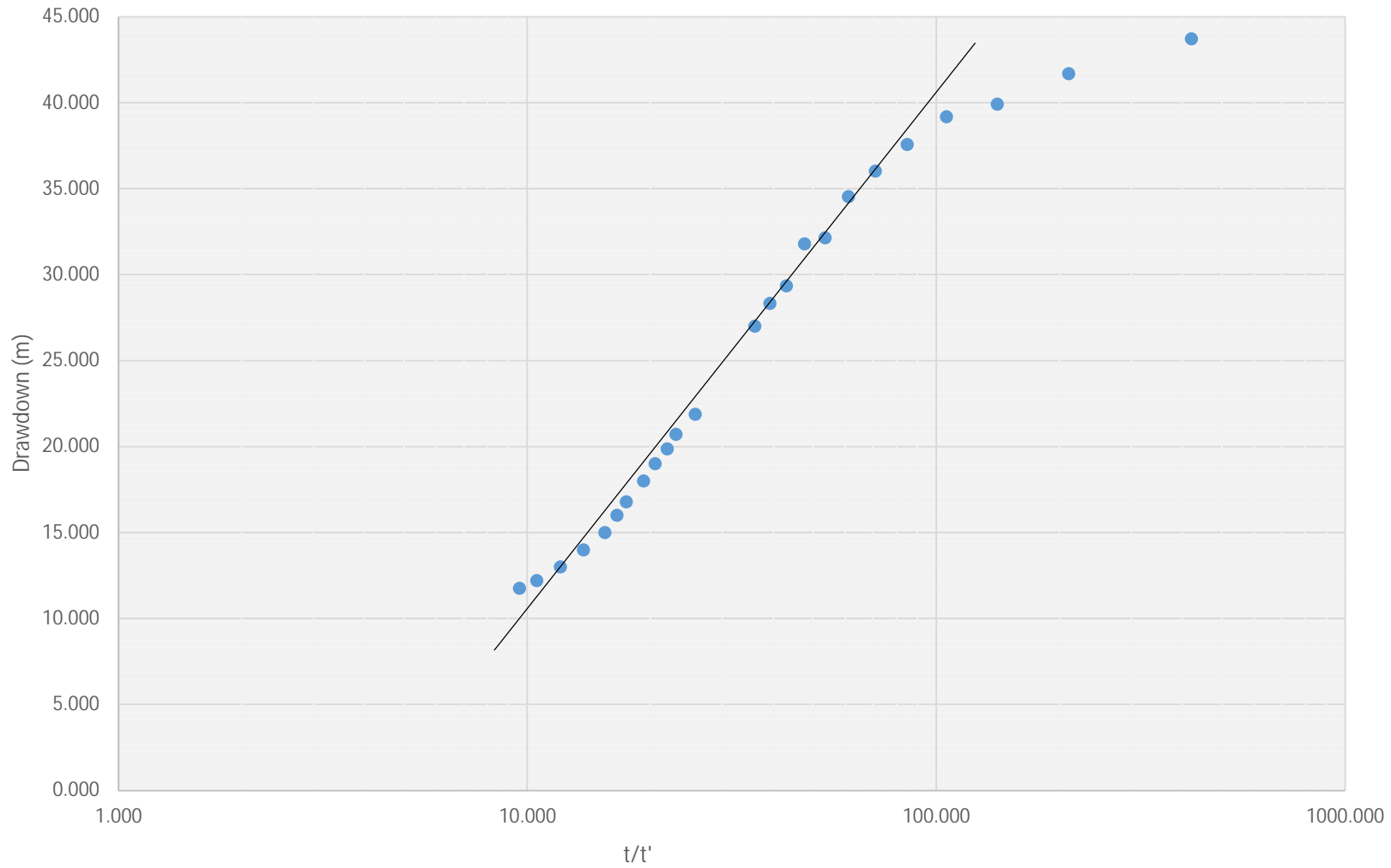
Drawdown vs Log Time
Pumping Test (Drawdown), Sep 13, 2022
TW1 - 273/275 Russ Bradley



Drawdown vs Log Time
Pumping Test (Long-Term), Sep 13, 2022
TW1 - 273/275 Russ Bradley



Drawdown vs Log Time
Pumping Test (Recovery), Sep 13, 2022
TW1 - 273/275 Russ Bradley



HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS

273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX E: LABORATORY CERTIFICATES OF ANALYSIS

Certificate of Analysis

McIntosh Perry Consulting Eng. (Carp)

115 Walgreen Rd.
Carp, ON K0A 1L0
Attn: Monica Black

Client PO: Russ Bradley
Project: 22-1643-01
Custody: 67014

Report Date: 22-Sep-2022
Order Date: 13-Sep-2022

Order #: 2238202

This Certificate of Analysis contains analytical data applicable to the following samples as submitted :

| Paracel ID | Client ID |
|------------|-----------|
| 2238202-01 | TW1-1 |
| 2238202-02 | TW1-2 |

Approved By:



Dale Robertson, BSc
Laboratory Director

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Analysis Summary Table

| Analysis | Method Reference/Description | Extraction Date | Analysis Date |
|-----------------------------|--|-----------------|---------------|
| Alkalinity, total to pH 4.5 | EPA 310.1 - Titration to pH 4.5 | 14-Sep-22 | 14-Sep-22 |
| Ammonia, as N | EPA 351.2 - Auto Colour | 14-Sep-22 | 14-Sep-22 |
| Anions | EPA 300.1 - IC | 21-Sep-22 | 21-Sep-22 |
| Colour | SM2120 - Spectrophotometric | 14-Sep-22 | 14-Sep-22 |
| Conductivity | EPA 9050A- probe @25 °C | 14-Sep-22 | 14-Sep-22 |
| Dissolved Organic Carbon | MOE E3247B - Combustion IR, filtration | 14-Sep-22 | 14-Sep-22 |
| E. coli | MOE E3407 | 14-Sep-22 | 14-Sep-22 |
| Fecal Coliform | SM 9222D | 14-Sep-22 | 14-Sep-22 |
| Mercury by CVAA | EPA 245.2 - Cold Vapour AA | 14-Sep-22 | 14-Sep-22 |
| Metals, ICP-MS | EPA 200.8 - ICP-MS | 15-Sep-22 | 15-Sep-22 |
| pH | EPA 150.1 - pH probe @25 °C | 14-Sep-22 | 14-Sep-22 |
| Phenolics | EPA 420.2 - Auto Colour, 4AAP | 14-Sep-22 | 14-Sep-22 |
| Hardness | Hardness as CaCO ₃ | 15-Sep-22 | 15-Sep-22 |
| Sulphide | SM 4500SE - Colourimetric | 15-Sep-22 | 15-Sep-22 |
| Tannin/Lignin | SM 5550B - Colourimetric | 15-Sep-22 | 16-Sep-22 |
| Total Coliform | MOE E3407 | 14-Sep-22 | 14-Sep-22 |
| Total Dissolved Solids | SM 2540C - gravimetric, filtration | 14-Sep-22 | 15-Sep-22 |
| Total Kjeldahl Nitrogen | EPA 351.2 - Auto Colour, digestion | 15-Sep-22 | 16-Sep-22 |
| Turbidity | SM 2130B - Turbidity meter | 15-Sep-22 | 15-Sep-22 |
| VOCs by P&T GC-MS | EPA 624 - P&T GC-MS | 15-Sep-22 | 15-Sep-22 |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

| | | | | |
|---------------------|-----------------|-----------------|---|---|
| Client ID: | TW1-1 | TW1-2 | - | - |
| Sample Date: | 13-Sep-22 10:40 | 13-Sep-22 14:35 | - | - |
| Sample ID: | 2238202-01 | 2238202-02 | - | - |
| MDL/Units | Water | Water | - | - |

Microbiological Parameters

| | | | | | |
|-----------------|-------------|--------|--------|---|---|
| E. coli | 1 CFU/100mL | ND [1] | ND [1] | - | - |
| Fecal Coliforms | 1 CFU/100mL | ND | ND | - | - |
| Total Coliforms | 1 CFU/100mL | ND [1] | ND [1] | - | - |

General Inorganics

| | | | | | |
|--------------------------|--------------|--------|--------|---|---|
| Alkalinity, total | 5 mg/L | 187 | 186 | - | - |
| Ammonia as N | 0.01 mg/L | 0.11 | 0.07 | - | - |
| Dissolved Organic Carbon | 0.5 mg/L | 1.4 | 1.5 | - | - |
| Colour | 2 TCU | 4 | <2 | - | - |
| Conductivity | 5 uS/cm | 499 | 509 | - | - |
| Hardness | 0.824 mg/L | 271 | 265 | - | - |
| pH | 0.1 pH Units | 7.9 | 7.9 | - | - |
| Phenolics | 0.001 mg/L | <0.001 | <0.001 | - | - |
| Total Dissolved Solids | 10 mg/L | 278 | 270 | - | - |
| Sulphide | 0.02 mg/L | 3.14 | 3.36 | - | - |
| Tannin & Lignin | 0.1 mg/L | <0.1 | <0.1 | - | - |
| Total Kjeldahl Nitrogen | 0.1 mg/L | 0.1 | <0.1 | - | - |
| Turbidity | 0.1 NTU | 34.8 | 3.3 | - | - |

Anions

| | | | | | |
|----------------|-----------|-------|-------|---|---|
| Chloride | 1.0 mg/L | 22.5 | 25.6 | - | - |
| Fluoride | 0.1 mg/L | 0.8 | 1.4 | - | - |
| Nitrate as N | 0.1 mg/L | <0.1 | <0.1 | - | - |
| Nitrite as N | 0.05 mg/L | <0.05 | <0.05 | - | - |
| Phosphate as P | 0.2 mg/L | <0.2 | 0.3 | - | - |
| Sulphate | 1.0 mg/L | 35.1 | 34.3 | - | - |

Metals

| | | | | | |
|-----------|----------|-------|-------|---|---|
| Mercury | 0.1 ug/L | <0.1 | <0.1 | - | - |
| Aluminum | 1 ug/L | 680 | 140 | - | - |
| Antimony | 0.5 ug/L | <0.5 | <0.5 | - | - |
| Arsenic | 1 ug/L | <1 | <1 | - | - |
| Barium | 1 ug/L | 295 | 277 | - | - |
| Beryllium | 0.5 ug/L | <0.5 | <0.5 | - | - |
| Boron | 10 ug/L | 89 | 94 | - | - |
| Cadmium | 0.1 ug/L | <0.1 | <0.1 | - | - |
| Calcium | 100 ug/L | 72200 | 71600 | - | - |
| Chromium | 1 ug/L | 2 | <1 | - | - |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

| | Client ID: | TW1-1 | TW1-2 | - | - |
|------------|--------------|-----------------|-----------------|---|---|
| | Sample Date: | 13-Sep-22 10:40 | 13-Sep-22 14:35 | - | - |
| | Sample ID: | 2238202-01 | 2238202-02 | - | - |
| | MDL/Units | Water | Water | - | - |
| Cobalt | 0.5 ug/L | <0.5 | <0.5 | - | - |
| Copper | 0.5 ug/L | <0.5 | <0.5 | - | - |
| Iron | 100 ug/L | 820 | 139 | - | - |
| Lead | 0.1 ug/L | 0.2 | <0.1 | - | - |
| Magnesium | 200 ug/L | 22100 | 21000 | - | - |
| Manganese | 5 ug/L | 19 | 6 | - | - |
| Molybdenum | 0.5 ug/L | <0.5 | <0.5 | - | - |
| Nickel | 1 ug/L | <1 | <1 | - | - |
| Potassium | 100 ug/L | 5330 | 4940 | - | - |
| Selenium | 1 ug/L | <1 | <1 | - | - |
| Silver | 0.1 ug/L | <0.1 | <0.1 | - | - |
| Sodium | 200 ug/L | 22700 | 24100 | - | - |
| Strontium | 10 ug/L | 3120 | 3290 | - | - |
| Thallium | 0.1 ug/L | <0.1 | <0.1 | - | - |
| Tin | 5 ug/L | <5 | <5 | - | - |
| Titanium | 5 ug/L | 77 | 15 | - | - |
| Tungsten | 10 ug/L | <10 | <10 | - | - |
| Uranium | 0.1 ug/L | 0.1 | <0.1 | - | - |
| Vanadium | 0.5 ug/L | 2.5 | <0.5 | - | - |
| Zinc | 5 ug/L | 9 | <5 | - | - |

| Volatiles | | | | | |
|-------------------------|----------|---|------|---|---|
| Acetone | 5.0 ug/L | - | <5.0 | - | - |
| Benzene | 0.5 ug/L | - | <0.5 | - | - |
| Bromodichloromethane | 0.5 ug/L | - | <0.5 | - | - |
| Bromoform | 0.5 ug/L | - | <0.5 | - | - |
| Bromomethane | 0.5 ug/L | - | <0.5 | - | - |
| Carbon Tetrachloride | 0.2 ug/L | - | <0.2 | - | - |
| Chlorobenzene | 0.5 ug/L | - | <0.5 | - | - |
| Chloroethane | 1.0 ug/L | - | <1.0 | - | - |
| Chloroform | 0.5 ug/L | - | <0.5 | - | - |
| Chloromethane | 3.0 ug/L | - | <3.0 | - | - |
| Dibromochloromethane | 0.5 ug/L | - | <0.5 | - | - |
| Dichlorodifluoromethane | 1.0 ug/L | - | <1.0 | - | - |
| 1,2-Dibromoethane | 0.2 ug/L | - | <0.2 | - | - |
| 1,2-Dichlorobenzene | 0.5 ug/L | - | <0.5 | - | - |
| 1,3-Dichlorobenzene | 0.5 ug/L | - | <0.5 | - | - |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

| | Client ID: | TW1-1 | TW1-2 | - | - |
|----------------------------------|--------------|-----------------|-----------------|---|---|
| | Sample Date: | 13-Sep-22 10:40 | 13-Sep-22 14:35 | - | - |
| | Sample ID: | 2238202-01 | 2238202-02 | - | - |
| | MDL/Units | Water | Water | - | - |
| 1,4-Dichlorobenzene | 0.5 ug/L | - | <0.5 | - | - |
| 1,1-Dichloroethane | 0.5 ug/L | - | <0.5 | - | - |
| 1,2-Dichloroethane | 0.5 ug/L | - | <0.5 | - | - |
| 1,1-Dichloroethylene | 0.5 ug/L | - | <0.5 | - | - |
| cis-1,2-Dichloroethylene | 0.5 ug/L | - | <0.5 | - | - |
| trans-1,2-Dichloroethylene | 0.5 ug/L | - | <0.5 | - | - |
| 1,2-Dichloroethylene, total | 0.5 ug/L | - | <0.5 | - | - |
| 1,2-Dichloropropane | 0.5 ug/L | - | <0.5 | - | - |
| cis-1,3-Dichloropropylene | 0.5 ug/L | - | <0.5 | - | - |
| trans-1,3-Dichloropropylene | 0.5 ug/L | - | <0.5 | - | - |
| 1,3-Dichloropropene, total | 0.5 ug/L | - | <0.5 | - | - |
| Ethylbenzene | 0.5 ug/L | - | <0.5 | - | - |
| Hexane | 1.0 ug/L | - | <1.0 | - | - |
| Methyl Ethyl Ketone (2-Butanone) | 5.0 ug/L | - | <5.0 | - | - |
| Methyl Butyl Ketone (2-Hexanone) | 10.0 ug/L | - | <10.0 | - | - |
| Methyl Isobutyl Ketone | 5.0 ug/L | - | <5.0 | - | - |
| Methyl tert-butyl ether | 2.0 ug/L | - | <2.0 | - | - |
| Methylene Chloride | 5.0 ug/L | - | <5.0 | - | - |
| Styrene | 0.5 ug/L | - | <0.5 | - | - |
| 1,1,1,2-Tetrachloroethane | 0.5 ug/L | - | <0.5 | - | - |
| 1,1,1,2,2-Tetrachloroethane | 0.5 ug/L | - | <0.5 | - | - |
| Tetrachloroethylene | 0.5 ug/L | - | <0.5 | - | - |
| Toluene | 0.5 ug/L | - | <0.5 | - | - |
| 1,1,1-Trichloroethane | 0.5 ug/L | - | <0.5 | - | - |
| 1,1,2-Trichloroethane | 0.5 ug/L | - | <0.5 | - | - |
| Trichloroethylene | 0.5 ug/L | - | <0.5 | - | - |
| Trichlorofluoromethane | 1.0 ug/L | - | <1.0 | - | - |
| 1,3,5-Trimethylbenzene | 0.5 ug/L | - | <0.5 | - | - |
| Vinyl chloride | 0.5 ug/L | - | <0.5 | - | - |
| m,p-Xylenes | 0.5 ug/L | - | <0.5 | - | - |
| o-Xylene | 0.5 ug/L | - | <0.5 | - | - |
| Xylenes, total | 0.5 ug/L | - | <0.5 | - | - |
| 4-Bromofluorobenzene | Surrogate | - | 115% | - | - |
| Dibromofluoromethane | Surrogate | - | 93.0% | - | - |
| Toluene-d8 | Surrogate | - | 107% | - | - |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Method Quality Control: Blank

| Analyte | Result | Reporting Limit | Units | Source Result | %REC | %REC Limit | RPD | RPD Limit | Notes |
|-----------------------------------|--------|-----------------|-----------|---------------|------|------------|-----|-----------|-------|
| Anions | | | | | | | | | |
| Chloride | ND | 1.0 | mg/L | | | | | | |
| Fluoride | ND | 0.1 | mg/L | | | | | | |
| Nitrate as N | ND | 0.1 | mg/L | | | | | | |
| Nitrite as N | ND | 0.05 | mg/L | | | | | | |
| Phosphate as P | ND | 0.2 | mg/L | | | | | | |
| Sulphate | ND | 1.0 | mg/L | | | | | | |
| General Inorganics | | | | | | | | | |
| Alkalinity, total | ND | 5 | mg/L | | | | | | |
| Ammonia as N | ND | 0.01 | mg/L | | | | | | |
| Dissolved Organic Carbon | ND | 0.5 | mg/L | | | | | | |
| Colour | ND | 2 | TCU | | | | | | |
| Conductivity | ND | 5 | uS/cm | | | | | | |
| Phenolics | ND | 0.001 | mg/L | | | | | | |
| Total Dissolved Solids | ND | 10 | mg/L | | | | | | |
| Sulphide | ND | 0.02 | mg/L | | | | | | |
| Tannin & Lignin | ND | 0.1 | mg/L | | | | | | |
| Total Kjeldahl Nitrogen | ND | 0.1 | mg/L | | | | | | |
| Turbidity | ND | 0.1 | NTU | | | | | | |
| Metals | | | | | | | | | |
| Mercury | ND | 0.1 | ug/L | | | | | | |
| Aluminum | ND | 1 | ug/L | | | | | | |
| Antimony | ND | 0.5 | ug/L | | | | | | |
| Arsenic | ND | 1 | ug/L | | | | | | |
| Barium | ND | 1 | ug/L | | | | | | |
| Beryllium | ND | 0.5 | ug/L | | | | | | |
| Boron | ND | 10 | ug/L | | | | | | |
| Cadmium | ND | 0.1 | ug/L | | | | | | |
| Calcium | ND | 100 | ug/L | | | | | | |
| Chromium | ND | 1 | ug/L | | | | | | |
| Cobalt | ND | 0.5 | ug/L | | | | | | |
| Copper | ND | 0.5 | ug/L | | | | | | |
| Iron | ND | 100 | ug/L | | | | | | |
| Lead | ND | 0.1 | ug/L | | | | | | |
| Magnesium | ND | 200 | ug/L | | | | | | |
| Manganese | ND | 5 | ug/L | | | | | | |
| Molybdenum | ND | 0.5 | ug/L | | | | | | |
| Nickel | ND | 1 | ug/L | | | | | | |
| Potassium | ND | 100 | ug/L | | | | | | |
| Selenium | ND | 1 | ug/L | | | | | | |
| Silver | ND | 0.1 | ug/L | | | | | | |
| Sodium | ND | 200 | ug/L | | | | | | |
| Strontium | ND | 10 | ug/L | | | | | | |
| Thallium | ND | 0.1 | ug/L | | | | | | |
| Tin | ND | 5 | ug/L | | | | | | |
| Titanium | ND | 5 | ug/L | | | | | | |
| Tungsten | ND | 10 | ug/L | | | | | | |
| Uranium | ND | 0.1 | ug/L | | | | | | |
| Vanadium | ND | 0.5 | ug/L | | | | | | |
| Zinc | ND | 5 | ug/L | | | | | | |
| Microbiological Parameters | | | | | | | | | |
| E. coli | ND | 1 | CFU/100mL | | | | | | |
| Fecal Coliforms | ND | 1 | CFU/100mL | | | | | | |
| Total Coliforms | ND | 1 | CFU/100mL | | | | | | |
| Volatiles | | | | | | | | | |
| Acetone | ND | 5.0 | ug/L | | | | | | |
| Benzene | ND | 0.5 | ug/L | | | | | | |
| Bromodichloromethane | ND | 0.5 | ug/L | | | | | | |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Method Quality Control: Blank

| Analyte | Result | Reporting Limit | Units | Source Result | %REC | %REC Limit | RPD | RPD Limit | Notes |
|----------------------------------|--------|-----------------|-------|---------------|------|------------|-----|-----------|-------|
| Bromoform | ND | 0.5 | ug/L | | | | | | |
| Bromomethane | ND | 0.5 | ug/L | | | | | | |
| Carbon Tetrachloride | ND | 0.2 | ug/L | | | | | | |
| Chlorobenzene | ND | 0.5 | ug/L | | | | | | |
| Chloroethane | ND | 1.0 | ug/L | | | | | | |
| Chloroform | ND | 0.5 | ug/L | | | | | | |
| Chloromethane | ND | 3.0 | ug/L | | | | | | |
| Dibromochloromethane | ND | 0.5 | ug/L | | | | | | |
| Dichlorodifluoromethane | ND | 1.0 | ug/L | | | | | | |
| 1,2-Dibromoethane | ND | 0.2 | ug/L | | | | | | |
| 1,2-Dichlorobenzene | ND | 0.5 | ug/L | | | | | | |
| 1,3-Dichlorobenzene | ND | 0.5 | ug/L | | | | | | |
| 1,4-Dichlorobenzene | ND | 0.5 | ug/L | | | | | | |
| 1,1-Dichloroethane | ND | 0.5 | ug/L | | | | | | |
| 1,2-Dichloroethane | ND | 0.5 | ug/L | | | | | | |
| 1,1-Dichloroethylene | ND | 0.5 | ug/L | | | | | | |
| cis-1,2-Dichloroethylene | ND | 0.5 | ug/L | | | | | | |
| trans-1,2-Dichloroethylene | ND | 0.5 | ug/L | | | | | | |
| 1,2-Dichloroethylene, total | ND | 0.5 | ug/L | | | | | | |
| 1,2-Dichloropropane | ND | 0.5 | ug/L | | | | | | |
| cis-1,3-Dichloropropylene | ND | 0.5 | ug/L | | | | | | |
| trans-1,3-Dichloropropylene | ND | 0.5 | ug/L | | | | | | |
| 1,3-Dichloropropene, total | ND | 0.5 | ug/L | | | | | | |
| Ethylbenzene | ND | 0.5 | ug/L | | | | | | |
| Hexane | ND | 1.0 | ug/L | | | | | | |
| Methyl Ethyl Ketone (2-Butanone) | ND | 5.0 | ug/L | | | | | | |
| Methyl Butyl Ketone (2-Hexanone) | ND | 10.0 | ug/L | | | | | | |
| Methyl Isobutyl Ketone | ND | 5.0 | ug/L | | | | | | |
| Methyl tert-butyl ether | ND | 2.0 | ug/L | | | | | | |
| Methylene Chloride | ND | 5.0 | ug/L | | | | | | |
| Styrene | ND | 0.5 | ug/L | | | | | | |
| 1,1,1,2-Tetrachloroethane | ND | 0.5 | ug/L | | | | | | |
| 1,1,2,2-Tetrachloroethane | ND | 0.5 | ug/L | | | | | | |
| Tetrachloroethylene | ND | 0.5 | ug/L | | | | | | |
| Toluene | ND | 0.5 | ug/L | | | | | | |
| 1,1,1-Trichloroethane | ND | 0.5 | ug/L | | | | | | |
| 1,1,2-Trichloroethane | ND | 0.5 | ug/L | | | | | | |
| Trichloroethylene | ND | 0.5 | ug/L | | | | | | |
| Trichlorofluoromethane | ND | 1.0 | ug/L | | | | | | |
| 1,3,5-Trimethylbenzene | ND | 0.5 | ug/L | | | | | | |
| Vinyl chloride | ND | 0.5 | ug/L | | | | | | |
| m,p-Xylenes | ND | 0.5 | ug/L | | | | | | |
| o-Xylene | ND | 0.5 | ug/L | | | | | | |
| Xylenes, total | ND | 0.5 | ug/L | | | | | | |
| Surrogate: 4-Bromofluorobenzene | 88.0 | | ug/L | | 110 | 50-140 | | | |
| Surrogate: Dibromofluoromethane | 73.4 | | ug/L | | 91.8 | 50-140 | | | |
| Surrogate: Toluene-d8 | 84.7 | | ug/L | | 106 | 50-140 | | | |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Method Quality Control: Duplicate

| Analyte | Result | Reporting Limit | Units | Source Result | %REC | %REC Limit | RPD | RPD Limit | Notes |
|-----------------------------------|--------|-----------------|-----------|---------------|------|------------|------|-----------|-------|
| Anions | | | | | | | | | |
| Chloride | 22.0 | 1.0 | mg/L | 22.5 | | | 2.3 | 10 | |
| Fluoride | 0.87 | 0.1 | mg/L | 0.81 | | | 6.6 | 10 | |
| Nitrate as N | ND | 0.1 | mg/L | ND | | | NC | 10 | |
| Nitrite as N | ND | 0.05 | mg/L | ND | | | NC | 10 | |
| Phosphate as P | 0.28 | 0.2 | mg/L | ND | | | NC | 10 | |
| Sulphate | 35.0 | 1.0 | mg/L | 35.1 | | | 0.5 | 10 | |
| General Inorganics | | | | | | | | | |
| Alkalinity, total | 312 | 5 | mg/L | 319 | | | 2.1 | 14 | |
| Ammonia as N | 0.096 | 0.01 | mg/L | 0.098 | | | 2.1 | 18 | |
| Dissolved Organic Carbon | 3.3 | 0.5 | mg/L | 3.8 | | | 14.7 | 37 | |
| Colour | ND | 2 | TCU | 2 | | | NC | 12 | |
| Conductivity | 35300 | 5 | uS/cm | 36000 | | | 1.9 | 5 | |
| pH | 7.6 | 0.1 | pH Units | 7.5 | | | 1.1 | 3.3 | |
| Phenolics | ND | 0.001 | mg/L | ND | | | NC | 10 | |
| Total Dissolved Solids | 280 | 10 | mg/L | 278 | | | 0.7 | 10 | |
| Sulphide | 0.02 | 0.02 | mg/L | 0.02 | | | 9.1 | 10 | |
| Tannin & Lignin | ND | 0.1 | mg/L | ND | | | NC | 11 | |
| Total Kjeldahl Nitrogen | 0.37 | 0.1 | mg/L | 0.31 | | | 15.7 | 16 | |
| Turbidity | 34.0 | 0.1 | NTU | 34.8 | | | 2.3 | 10 | |
| Metals | | | | | | | | | |
| Mercury | ND | 0.1 | ug/L | ND | | | NC | 20 | |
| Aluminum | 15.5 | 1 | ug/L | 12.3 | | | 22.8 | 20 | QR-05 |
| Antimony | 0.51 | 0.5 | ug/L | 0.72 | | | NC | 20 | |
| Arsenic | 2.6 | 1 | ug/L | 2.5 | | | 4.6 | 20 | |
| Barium | 127 | 1 | ug/L | 126 | | | 0.5 | 20 | |
| Beryllium | ND | 0.5 | ug/L | ND | | | NC | 20 | |
| Boron | 107 | 10 | ug/L | 108 | | | 0.3 | 20 | |
| Cadmium | ND | 0.1 | ug/L | ND | | | NC | 20 | |
| Calcium | 146000 | 100 | ug/L | 147000 | | | 1.3 | 20 | |
| Chromium | ND | 1 | ug/L | ND | | | NC | 20 | |
| Cobalt | 1.79 | 0.5 | ug/L | 1.81 | | | 1.2 | 20 | |
| Copper | 1.29 | 0.5 | ug/L | 1.38 | | | 6.6 | 20 | |
| Iron | 275 | 100 | ug/L | 275 | | | 0.2 | 20 | |
| Lead | 1.74 | 0.1 | ug/L | 1.73 | | | 0.6 | 20 | |
| Magnesium | 27200 | 200 | ug/L | 27500 | | | 1.0 | 20 | |
| Manganese | 159 | 5 | ug/L | 159 | | | 0.1 | 20 | |
| Molybdenum | 8.17 | 0.5 | ug/L | 8.33 | | | 1.9 | 20 | |
| Nickel | 3.3 | 1 | ug/L | 3.3 | | | 1.6 | 20 | |
| Potassium | 6030 | 100 | ug/L | 6000 | | | 0.6 | 20 | |
| Selenium | ND | 1 | ug/L | ND | | | NC | 20 | |
| Silver | ND | 0.1 | ug/L | ND | | | NC | 20 | |
| Sodium | 112000 | 200 | ug/L | 113000 | | | 0.3 | 20 | |
| Strontium | 2130 | 10 | ug/L | 2120 | | | 0.5 | 20 | |
| Thallium | 0.15 | 0.1 | ug/L | 0.15 | | | 3.8 | 20 | |
| Tin | ND | 5 | ug/L | ND | | | NC | 20 | |
| Titanium | ND | 5 | ug/L | ND | | | NC | 20 | |
| Tungsten | ND | 10 | ug/L | ND | | | NC | 20 | |
| Uranium | 1.6 | 0.1 | ug/L | 1.6 | | | 1.0 | 20 | |
| Vanadium | 0.53 | 0.5 | ug/L | 0.54 | | | 2.1 | 20 | |
| Zinc | ND | 5 | ug/L | 5 | | | NC | 20 | |
| Microbiological Parameters | | | | | | | | | |
| E. coli | ND | 1 | CFU/100mL | ND | | | NC | 30 | BAC14 |
| Fecal Coliforms | ND | 1 | CFU/100mL | ND | | | NC | 30 | |
| Total Coliforms | ND | 1 | CFU/100mL | ND | | | NC | 30 | BAC14 |
| Volatiles | | | | | | | | | |
| Acetone | ND | 5.0 | ug/L | ND | | | NC | 30 | |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Method Quality Control: Duplicate

| Analyte | Result | Reporting Limit | Units | Source Result | %REC | %REC Limit | RPD | RPD Limit | Notes |
|----------------------------------|--------|-----------------|-------|---------------|------|------------|------|-----------|-------|
| Benzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Bromodichloromethane | 4.16 | 0.5 | ug/L | 4.03 | | | 3.2 | 30 | |
| Bromoform | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Bromomethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Carbon Tetrachloride | ND | 0.2 | ug/L | ND | | | NC | 30 | |
| Chlorobenzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Chloroethane | ND | 1.0 | ug/L | ND | | | NC | 30 | |
| Chloroform | 8.50 | 0.5 | ug/L | 9.99 | | | 16.1 | 30 | |
| Chloromethane | ND | 3.0 | ug/L | ND | | | NC | 30 | |
| Dibromochloromethane | 4.18 | 0.5 | ug/L | 3.88 | | | 7.4 | 30 | |
| Dichlorodifluoromethane | ND | 1.0 | ug/L | ND | | | NC | 30 | |
| 1,2-Dibromoethane | ND | 0.2 | ug/L | ND | | | NC | 30 | |
| 1,2-Dichlorobenzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,3-Dichlorobenzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,4-Dichlorobenzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,1-Dichloroethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,2-Dichloroethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,1-Dichloroethylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| cis-1,2-Dichloroethylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| trans-1,2-Dichloroethylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,2-Dichloropropane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| cis-1,3-Dichloropropylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| trans-1,3-Dichloropropylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Ethylbenzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Hexane | ND | 1.0 | ug/L | ND | | | NC | 30 | |
| Methyl Ethyl Ketone (2-Butanone) | ND | 5.0 | ug/L | ND | | | NC | 30 | |
| Methyl Butyl Ketone (2-Hexanone) | ND | 10.0 | ug/L | ND | | | NC | 30 | |
| Methyl Isobutyl Ketone | ND | 5.0 | ug/L | ND | | | NC | 30 | |
| Methyl tert-butyl ether | ND | 2.0 | ug/L | ND | | | NC | 30 | |
| Methylene Chloride | ND | 5.0 | ug/L | ND | | | NC | 30 | |
| Styrene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,1,1,2-Tetrachloroethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,1,2,2-Tetrachloroethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Tetrachloroethylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Toluene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,1,1-Trichloroethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| 1,1,2-Trichloroethane | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Trichloroethylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Trichlorofluoromethane | ND | 1.0 | ug/L | ND | | | NC | 30 | |
| 1,3,5-Trimethylbenzene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Vinyl chloride | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| m,p-Xylenes | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| o-Xylene | ND | 0.5 | ug/L | ND | | | NC | 30 | |
| Surrogate: 4-Bromofluorobenzene | 90.4 | | ug/L | | 113 | 50-140 | | | |
| Surrogate: Dibromofluoromethane | 71.9 | | ug/L | | 89.8 | 50-140 | | | |
| Surrogate: Toluene-d8 | 85.7 | | ug/L | | 107 | 50-140 | | | |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Method Quality Control: Spike

| Analyte | Result | Reporting Limit | Units | Source Result | %REC | %REC Limit | RPD | RPD Limit | Notes |
|---------------------------|--------|-----------------|-------|---------------|------|------------|-----|-----------|-------|
| Anions | | | | | | | | | |
| Chloride | 31.9 | 1.0 | mg/L | 22.5 | 94.2 | 77-123 | | | |
| Fluoride | 1.85 | 0.1 | mg/L | 0.81 | 104 | 79-121 | | | |
| Nitrate as N | 1.17 | 0.1 | mg/L | ND | 117 | 79-120 | | | |
| Nitrite as N | 1.06 | 0.05 | mg/L | ND | 106 | 84-117 | | | |
| Phosphate as P | 5.19 | 0.2 | mg/L | ND | 104 | 59-141 | | | |
| Sulphate | 45.2 | 1.0 | mg/L | 35.1 | 100 | 74-126 | | | |
| General Inorganics | | | | | | | | | |
| Ammonia as N | 0.338 | 0.01 | mg/L | 0.098 | 96.0 | 81-124 | | | |
| Dissolved Organic Carbon | 12.3 | 0.5 | mg/L | 3.8 | 85.1 | 60-133 | | | |
| Phenolics | 0.028 | 0.001 | mg/L | ND | 110 | 67-133 | | | |
| Total Dissolved Solids | 90.0 | 10 | mg/L | ND | 90.0 | 75-125 | | | |
| Sulphide | 0.48 | 0.02 | mg/L | 0.02 | 90.8 | 79-115 | | | |
| Tannin & Lignin | 1.1 | 0.1 | mg/L | ND | 108 | 71-113 | | | |
| Total Kjeldahl Nitrogen | 2.38 | 0.1 | mg/L | 0.31 | 103 | 81-126 | | | |
| Metals | | | | | | | | | |
| Mercury | 2.74 | 0.1 | ug/L | ND | 91.2 | 70-130 | | | |
| Aluminum | 55.9 | 1 | ug/L | 12.3 | 87.2 | 80-120 | | | |
| Arsenic | 52.2 | 1 | ug/L | 2.5 | 99.5 | 80-120 | | | |
| Barium | 167 | 1 | ug/L | 126 | 82.0 | 80-120 | | | |
| Beryllium | 41.7 | 0.5 | ug/L | ND | 83.4 | 80-120 | | | |
| Boron | 57 | 10 | ug/L | 16 | 81.9 | 80-120 | | | |
| Cadmium | 40.2 | 0.1 | ug/L | ND | 80.4 | 80-120 | | | |
| Calcium | 133000 | 100 | ug/L | 124000 | 87.7 | 80-120 | | | |
| Chromium | 53.0 | 1 | ug/L | ND | 105 | 80-120 | | | |
| Cobalt | 50.5 | 0.5 | ug/L | 1.81 | 97.4 | 80-120 | | | |
| Copper | 45.3 | 0.5 | ug/L | 1.38 | 87.9 | 80-120 | | | |
| Iron | 2690 | 100 | ug/L | 275 | 96.6 | 80-120 | | | |
| Lead | 43.6 | 0.1 | ug/L | 1.73 | 83.7 | 80-120 | | | |
| Magnesium | 37700 | 200 | ug/L | 27900 | 97.4 | 80-120 | | | |
| Manganese | 363 | 5 | ug/L | 313 | 99.2 | 80-120 | | | |
| Molybdenum | 53.0 | 0.5 | ug/L | 8.33 | 89.4 | 80-120 | | | |
| Nickel | 49.9 | 1 | ug/L | 3.3 | 93.0 | 80-120 | | | |
| Potassium | 16600 | 100 | ug/L | 6000 | 106 | 80-120 | | | |
| Selenium | 45.9 | 1 | ug/L | ND | 91.3 | 80-120 | | | |
| Silver | 49.3 | 0.1 | ug/L | ND | 98.7 | 80-120 | | | |
| Sodium | 27700 | 200 | ug/L | 17900 | 98.0 | 80-120 | | | |
| Strontium | 334 | 10 | ug/L | 291 | 85.6 | 80-120 | | | |
| Thallium | 42.9 | 0.1 | ug/L | 0.15 | 85.5 | 80-120 | | | |
| Tin | 45.9 | 5 | ug/L | ND | 91.2 | 80-120 | | | |
| Titanium | 57.9 | 5 | ug/L | ND | 114 | 80-120 | | | |
| Tungsten | 51.3 | 10 | ug/L | ND | 91.2 | 80-120 | | | |
| Uranium | 48.0 | 0.1 | ug/L | 1.6 | 92.8 | 80-120 | | | |
| Vanadium | 54.3 | 0.5 | ug/L | 0.54 | 108 | 80-120 | | | |
| Zinc | 46 | 5 | ug/L | ND | 91.1 | 80-120 | | | |
| Volatiles | | | | | | | | | |
| Acetone | 71.9 | 5.0 | ug/L | ND | 71.9 | 50-140 | | | |
| Benzene | 35.6 | 0.5 | ug/L | ND | 89.1 | 60-130 | | | |
| Bromodichloromethane | 35.5 | 0.5 | ug/L | ND | 88.8 | 60-130 | | | |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Method Quality Control: Spike

| Analyte | Result | Reporting Limit | Units | Source Result | %REC | %REC Limit | RPD | RPD Limit | Notes |
|----------------------------------|--------|-----------------|-------|---------------|------|------------|-----|-----------|-------|
| Bromoform | 41.5 | 0.5 | ug/L | ND | 104 | 60-130 | | | |
| Bromomethane | 41.3 | 0.5 | ug/L | ND | 103 | 50-140 | | | |
| Carbon Tetrachloride | 33.0 | 0.2 | ug/L | ND | 82.6 | 60-130 | | | |
| Chlorobenzene | 42.2 | 0.5 | ug/L | ND | 105 | 60-130 | | | |
| Chloroethane | 34.9 | 1.0 | ug/L | ND | 87.4 | 50-140 | | | |
| Chloroform | 40.5 | 0.5 | ug/L | ND | 101 | 60-130 | | | |
| Chloromethane | 35.8 | 3.0 | ug/L | ND | 89.4 | 50-140 | | | |
| Dibromochloromethane | 40.2 | 0.5 | ug/L | ND | 100 | 60-130 | | | |
| Dichlorodifluoromethane | 37.4 | 1.0 | ug/L | ND | 93.6 | 50-140 | | | |
| 1,2-Dibromoethane | 39.2 | 0.2 | ug/L | ND | 98.0 | 60-130 | | | |
| 1,2-Dichlorobenzene | 42.1 | 0.5 | ug/L | ND | 105 | 60-130 | | | |
| 1,3-Dichlorobenzene | 40.4 | 0.5 | ug/L | ND | 101 | 60-130 | | | |
| 1,4-Dichlorobenzene | 40.2 | 0.5 | ug/L | ND | 101 | 60-130 | | | |
| 1,1-Dichloroethane | 36.2 | 0.5 | ug/L | ND | 90.6 | 60-130 | | | |
| 1,2-Dichloroethane | 37.0 | 0.5 | ug/L | ND | 92.5 | 60-130 | | | |
| 1,1-Dichloroethylene | 32.2 | 0.5 | ug/L | ND | 80.5 | 60-130 | | | |
| cis-1,2-Dichloroethylene | 40.0 | 0.5 | ug/L | ND | 100 | 60-130 | | | |
| trans-1,2-Dichloroethylene | 36.1 | 0.5 | ug/L | ND | 90.2 | 60-130 | | | |
| 1,2-Dichloropropane | 35.6 | 0.5 | ug/L | ND | 89.0 | 60-130 | | | |
| cis-1,3-Dichloropropylene | 38.2 | 0.5 | ug/L | ND | 95.5 | 60-130 | | | |
| trans-1,3-Dichloropropylene | 42.6 | 0.5 | ug/L | ND | 106 | 60-130 | | | |
| Ethylbenzene | 37.2 | 0.5 | ug/L | ND | 93.0 | 60-130 | | | |
| Hexane | 44.1 | 1.0 | ug/L | ND | 110 | 60-130 | | | |
| Methyl Ethyl Ketone (2-Butanone) | 97.9 | 5.0 | ug/L | ND | 97.9 | 50-140 | | | |
| Methyl Butyl Ketone (2-Hexanone) | 66.6 | 10.0 | ug/L | ND | 66.6 | 50-140 | | | |
| Methyl Isobutyl Ketone | 97.3 | 5.0 | ug/L | ND | 97.3 | 50-140 | | | |
| Methyl tert-butyl ether | 66.1 | 2.0 | ug/L | ND | 66.1 | 50-140 | | | |
| Methylene Chloride | 35.6 | 5.0 | ug/L | ND | 88.9 | 60-130 | | | |
| Styrene | 37.1 | 0.5 | ug/L | ND | 92.7 | 60-130 | | | |
| 1,1,1,2-Tetrachloroethane | 35.5 | 0.5 | ug/L | ND | 88.7 | 60-130 | | | |
| 1,1,2,2-Tetrachloroethane | 33.8 | 0.5 | ug/L | ND | 84.6 | 60-130 | | | |
| Tetrachloroethylene | 38.6 | 0.5 | ug/L | ND | 96.6 | 60-130 | | | |
| Toluene | 38.1 | 0.5 | ug/L | ND | 95.3 | 60-130 | | | |
| 1,1,1-Trichloroethane | 31.1 | 0.5 | ug/L | ND | 77.8 | 60-130 | | | |
| 1,1,2-Trichloroethane | 40.8 | 0.5 | ug/L | ND | 102 | 60-130 | | | |
| Trichloroethylene | 30.0 | 0.5 | ug/L | ND | 75.0 | 60-130 | | | |
| Trichlorofluoromethane | 39.0 | 1.0 | ug/L | ND | 97.6 | 60-130 | | | |
| 1,3,5-Trimethylbenzene | 37.7 | 0.5 | ug/L | ND | 94.3 | 60-130 | | | |
| Vinyl chloride | 40.0 | 0.5 | ug/L | ND | 99.9 | 50-140 | | | |
| m,p-Xylenes | 77.9 | 0.5 | ug/L | ND | 97.4 | 60-130 | | | |
| o-Xylene | 37.4 | 0.5 | ug/L | ND | 93.4 | 60-130 | | | |
| Surrogate: 4-Bromofluorobenzene | 89.3 | | ug/L | | 112 | 50-140 | | | |
| Surrogate: Dibromofluoromethane | 69.4 | | ug/L | | 86.7 | 50-140 | | | |
| Surrogate: Toluene-d8 | 70.8 | | ug/L | | 88.5 | 50-140 | | | |

Certificate of Analysis

Report Date: 22-Sep-2022

Client: McIntosh Perry Consulting Eng. (Carp)

Order Date: 13-Sep-2022

Client PO: Russ Bradley

Project Description: 22-1643-01

Qualifier Notes:

Login Qualifiers :

Container and COC sample IDs don't match - Nutrients and metals bottles read TW1-1; chain of custody reads TW1-2.

Applies to samples: TW1-2

Sample Qualifiers :

1 : A2C - Background counts greater than 200

QC Qualifiers :

BAC14 A2C - Background counts greater than 200

QR-05 Duplicate RPDs higher than normally accepted. Remaining batch QA\QC was acceptable. May be sample effect.

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable

ND: Not Detected

MDL: Method Detection Limit

Source Result: Data used as source for matrix and duplicate samples

%REC: Percent recovery.

RPD: Relative percent difference.

NC: Not Calculated



| | |
|---|--|
| Parcel Order Number (Lab Use Only) 2238202 | Chain Of Custody (Lab Use Only) No 67014 |
|---|--|

| | | |
|--|---|--|
| Client Name: McIntosh Perry | Project Ref: Russ Bradley | Page <u>1</u> of <u>1</u> |
| Contact Name: Monica Black | Quote #: 22-334 | Turnaround Time <input type="checkbox"/> 1 day <input type="checkbox"/> 3 day <input type="checkbox"/> 2 day <input checked="" type="checkbox"/> Regular |
| Address: 115 Walgreen Road, Carp ON KOA 1LO | PO #: 22-1643-01 | |
| Telephone: 613 227 6953 | E-mail: m.black@mcintoshperry.com j.bowman@mcintoshperry.com | |
| Date Required: _____ | | |

| <input type="checkbox"/> REG 153/04 <input type="checkbox"/> REG 406/19 | Other Regulation | Matrix Type: S (Soil/Sed.) GW (Ground Water) SW (Surface Water) SS (Storm/Sanitary Sewer) P (Paint) A (Air) O (Other) | | Required Analysis | | | | | |
|--|--|---|-----------------|-------------------|--------------|--------------------------------|------------|----------------------------------|----------|
| <input type="checkbox"/> Table 1 <input type="checkbox"/> Res/Park <input type="checkbox"/> Med/Fine | <input type="checkbox"/> REG 558 <input type="checkbox"/> PWQO | Matrix | Air Volume | # of Containers | Sample Taken | sub div package, less bacteria | EC, FC, TC | Trace metals including strontium | VOCs |
| <input type="checkbox"/> Table 2 <input type="checkbox"/> Ind/Comm <input type="checkbox"/> Coarse | <input type="checkbox"/> CCME <input type="checkbox"/> MISA | | | | | | | | |
| <input type="checkbox"/> Table 3 <input type="checkbox"/> Agri/Other | <input type="checkbox"/> SU - Sani <input type="checkbox"/> SU - Storm | | | | | | | | |
| <input type="checkbox"/> Table _____ | Mun: _____ | | | | | | | | |
| For RSC: <input type="checkbox"/> Yes <input type="checkbox"/> No | <input checked="" type="checkbox"/> Other: ODWSOG | | | | | | | | |
| Sample ID/Location Name | | Date | Time | | | | | | |
| 1 | TW1-1 | 13-09-22 | 10:40 AM | 9 | GW | X | X | X | |
| 2 | TW1-2 | 13-09-22 | 2:35 PM | 11 | GW | X | X | X | X |
| 3 | | | | | | | | | |
| 4 | | | | | | | | | |
| 5 | | | | | | | | | |
| 6 | | | | | | | | | |
| 7 | | | | | | | | | |
| 8 | | | | | | | | | |
| 9 | | | | | | | | | |
| 10 | | | | | | | | | |

Comments: **these are not drinking water samples -> they are raw, untreated groundwater samples**

Method of Delivery: **Walk in**

| | | |
|--|--|---|
| Relinquished By (Sign): [Signature] | Received By (Sign): [Signature] | Verified By: [Signature] |
| Relinquished By (Print): Monica Black | Date/Time: 13-09-22 17:00 | Date/Time: Sept 14, 22 8:00 |
| Date/Time: 13-09-22 4:08pm | Temperature: 13.3 °C | pH Verified: <input type="checkbox"/> By: _____ |

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN
ANALYSIS
273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX F: TRANSMISSIVITY, FARVOLDEN, AND MOELL
CALCULATIONS, AND LANGELIER SATURATION INDEX (LSI) AND
RYZNAR STABILITY INDEX (RSI) CALCULATIONS

Cooper-Jacob Analysis: Calculations

TW1 - 273-275 Russ Bradley Road

Pumping Rate

47.79 l/min

Transmissivity

TW1 (Drawdown)

T = 2.3 Q / 4π ΔS
 T = 2.3 (68.83 m³/day)/4π (20 m)
 T = 0.6 m²/day
 7.29093E-06 m²/s

Q = 47.79 L/min
 Q = ((47.79 L/min)/(1000L))*(60 min)(24 hour)
 68.8 m³/day

Note:

Q uses a weighted average from the pumping rates used within the pumping test.

Cumulative Average

| | | | |
|------------|-----|--------|---------|
| 60 L/min | 35 | 2100 | |
| 53.3 L/min | 157 | 8368.1 | |
| 48 L/min | 3 | 144 | 20062.1 |
| 42 L/min | 225 | 9450 | 47.7669 |

TW1 (Recovery)

T = 2.3 Q / 4π ΔS
 T = 2.3 (68.8 m³/day)/4π (25 m)
 T = 0.5 m²/day
 5.83274E-06 m²/s

Q = 47.79 L/min
 Q = ((47.79 L/min)/(1000L))*(60 min)(24 hour)
 68.8 m³/day

Total 20062.1
 Cum. Average 47.7669 L/min

Δs = 25 m

Farvolden

Q20 = 0.68 T Ha Sf

Ha = the available water column height (m)
 Sf = safety factor
 T = Transmissivity (m²/day)

Average

T = 0.5 m²/day
 T = 0.6 m²/day
 Safety Factor 0.7

TW1 (drawdown)

Q20 = 0.68 (1.12 m²/day)(75.47 m)(0.5)

Q20 = 20 m³/day
 Q20 = 19758 L/day
 Q20 = 14 L/min

Rec'd Pump Settin 85.34 m
 static WL 9.87 m
 Ha (avail. head) = 75.47 m

Moell

Q20 = (Q Ha Sf) / (s100 + 5 Δs)

Q = the pumping rate (m³/day)
 Ha = the available water column height (m)
 Sf = safety factor
 s100 = the drawdown at 100 minutes (semi-log long-term graph)
 Δs = the change in hydraulic head over one log cycle (drawdown vs. log time)

TW1 (drawdown)

Q20 = ((47.8 m³/day)(75.47 m)(0.7))/(43 m + 5(11.2 m))
 Q20 = 18 m³/day
 Q20 = 17655 L/day
 Q20 = 12 L/min

Q = 47.8 m³/day
 Ha = 75.47 m
 Safety Factor 0.7
 s100 43
 Δs 20

Hydraulic Conductivity

b = aquifer thickness b = 109.12 m end of casing = 142 ft 43.28 m
 T = transmissivity end of hole = 500 ft 152.4 m
 K = hydraulic conductivity

K = T/b

K = 6.68E-08 m/s Drawdown 9.35E-08
 5.35E-08 m/s Recovery

Comments: Aquifer thickness of 109.12 m corresponds to the interval between the bottom of the casing and the bottom of the well (casing to 43.28 m BGS, WL at 9.87 mBTOC and end of hole at 152.4)

Langelier Saturation Index (LSI)

If LSI is negative: No potential to scale, the water will dissolve CaCO₃

If LSI is positive: Scale can form and CaCO₃ precipitation may occur

If LSI is close to zero: Borderline scale potential. Water quality or changes in temperature, or evaporation could change the index.

The LSI is probably the most widely used indicator of cooling water scale potential. It is purely an equilibrium index and deals only with the thermodynamic driving force for calcium carbonate scale formation and growth.

$$LSI = pH - pH_s$$

Where:

pH is the measured water pH

pH_s is the pH at saturation in calcite or calcium carbonate and is defined as:

$$pH_s = (9.3 + A + B) - (C + D)$$

Where:

$$A = (\text{Log}_{10} [\text{TDS}] - 1) / 10$$

$$B = -13.12 \times \text{Log}_{10} (^{\circ}\text{C} + 273) + 34.55$$

$$C = \text{Log}_{10} [\text{Ca}^{2+} \text{ as CaCO}_3] - 0.4$$

$$D = \text{Log}_{10} [\text{alkalinity as CaCO}_3]$$

| Test Well 1 | | | | |
|-------------|------|--|---|----------|
| pH | 7.9 | | A | 0.143136 |
| TDS | 270 | | B | 2.411629 |
| Hardness | 265 | | C | 2.023246 |
| Alkalinity | 186 | | D | 2.269513 |
| Temp. | 8.56 | | | |
| pHs = | | | | 7.562007 |
| LSI = | | | | 0.337993 |
| RSI = | | | | 7.224013 |

Ryznar Stability Index (RSI)

$$RSI = 2(pH_s) - pH$$

Where:

pH is the measured water pH

pH_s is the pH at saturation in calcite or calcium carbonate

The empirical correlation of the Ryznar stability index can be summarized as follows:

RSI << 6 the scale tendency increases as the index decreases

RSI >> 7 the calcium carbonate formation probably does not lead to a protective corrosion inhibitor film

RSI >> 8 mild steel corrosion becomes an increasing problem.

Project No.: CCO-22-1643-01

HYDROGEOLOGICAL ASSESSMENT AND TERRAIN
ANALYSIS
273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX G: INFILTRATION WORK SUMMARY FOR 273-275 RUSS
BRADLEY ROAD

September 7, 2022

To Whom it May Concern:

Re: Infiltration Work Summary for 273-275 Russ Bradley, Ottawa, ON

McIntosh Perry staff completed infiltration testing on June 7, 2022, at the locations shown on Figure 1, below. A Guelph Permeameter (a constant head permeameter used to measure in-situ vertical hydraulic conductivities of soil) was set up in three separate locations (TP1, TP2, TP3) for a total of three double-head infiltration tests. Additional tests were attempted (TP4, TP5) however a majority of the proposed development area was saturated and deemed unsuitable for testing. Test locations were selected based on the permeability of soils and subsequent capacity to complete the infiltration testing. In this case, only one (1) hole (TP1) was completed within the proposed infiltration area, while the remaining (TP2, TP3) were completed in close proximity to Carp Road – outside of the desired infiltration area. Holes were advanced using a hand auger.



Figure 1 – Guelph Permeameter Test Locations

Where possible, each infiltration test consisted of at least a 5-7 cm head test, based on the level of saturation and presence of water in each hole where testing was attempted. Water was added to the Guelph Permeameter reservoir and allowed to infiltrate into the soil at the specified head pressure. Changes in reservoir water level (h) were recorded at regular intervals and normalized for change in time (t). Each test was considered complete when dh/dt (change in head / change in time) reached a steady-state for at least three consecutive measurements.

Appendix C.2 of the Toronto Region Conservation Authority's (TRCA) Stormwater Management Criteria (August 2012) provides guidance on the calculation of infiltration rates using field saturated hydraulic conductivity (K_{fs}). The recommended calculation is as follows:

$$K_{fs} = (6 \times 10^{-11}) (I^{3.7363})$$

Where:

- K_{fs} is the field saturated hydraulic conductivity (in cm/s), as measured by a Guelph Permeameter, double-ring infiltrometer, single-ring infiltrometer, or other accepted method
- I is the infiltration rate (in mm/hr)

Based on the above calculation, the estimated soil infiltration rate (I) from the data collected at TP1, TP2, and TP3 is shown in the table below.

Table 1: Infiltration Rates

| Borehole ID | K_{fs} cm/s | Corrected I* (mm/hr) |
|-------------|-----------------------|-------------------------|
| TP1 | 3.4×10^{-6} | 5.35 |
| TP2 | 1.02×10^{-7} | 2.09 |
| TP3 | 4.9×10^{-7} | 3.18 |

**Includes a safety factor calculated per TRCA guidance*

As shown, the highest infiltration rate was observed in TP1 at a depth of approximately 0.05 m bgs (approximately 113.85 m asl). The lowest infiltration rate was observed in TP2 at a depth of approximately 0.15 m bgs (approximately 113.75 m asl). These values are generally consistent with the observed stratigraphy, in that fine-grain materials will typically have lower hydraulic conductivity rates.

It is noted that the field infiltration testing for TP2 and TP3 were conducted outside of the proposed infiltration infrastructure area, and are situated in close proximity to Card Road. Based on Site observations from June 7 and August 30, 2022, the property and development area appear to be highly saturated, with many areas of stagnant standing water. In particular, the wooded area of the Site appears to be a local topographic low point, and is not considered suitable for infiltration trenches or galleries in its current state. In several instances where infiltration tests were attempted, excess water logging caused no movement in the water column. As noted above, additional testing was done in subsequent holes TP2 and TP3 within an area closer to Carp Road.

TP2 and TP3 were advanced to depths of approximately 0.15 and 0.2 m bgs, respectively. The soil stratigraphy was consistent at all depths, consisting of wet, silty sand with trace clay.

It is McIntosh Perry's opinion that at this particular Site, alternate options for stormwater management should be investigated. It is expected that any infiltrative solutions to stormwater management will be hampered by high groundwater levels, standing water, and fine-grained overburden that is not conducive to infiltration.

Sincerely,

Original Signed

Jordan Bowman, P.Geo., P.Biol.
Manager (Geo-Environmental)
(613) 714-4602
j.bowman@mcintoshperry.com

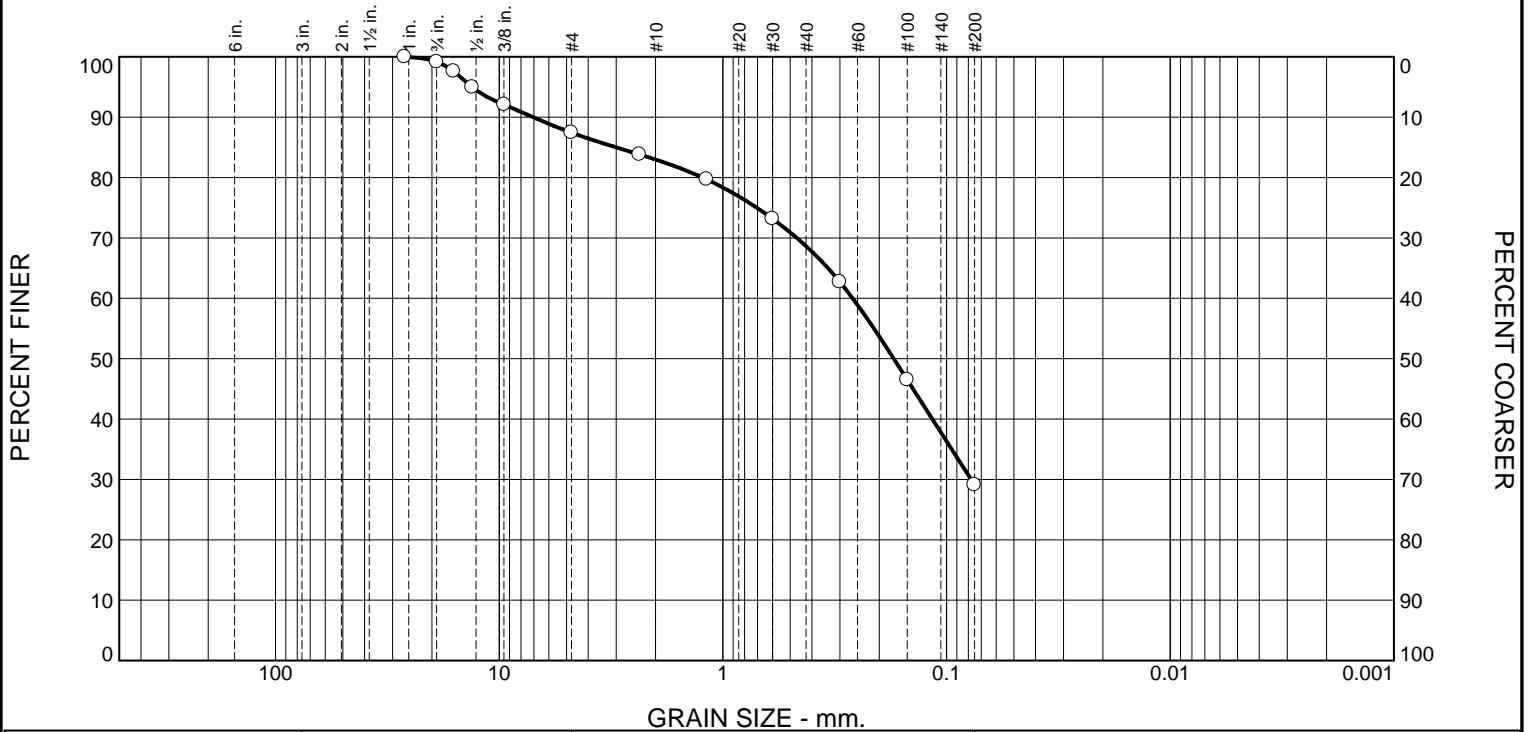
HYDROGEOLOGICAL ASSESSMENT AND TERRAIN ANALYSIS

273-275 RUSS BRADLEY ROAD, CARP, ONTARIO



APPENDIX H: GRAIN SIZE ANALYSIS

Particle Size Distribution Report



| % +75mm | % Gravel | | % Sand | | | % Fines | |
|---------|----------|------|--------|--------|------|---------|------|
| | Coarse | Fine | Coarse | Medium | Fine | Silt | Clay |
| 0.0 | 0.8 | 11.8 | 4.4 | 14.3 | 39.6 | 29.1 | |

| TEST RESULTS | | | |
|--------------|---------------|------------------|----------------|
| Opening Size | Percent Finer | Spec.* (Percent) | Pass? (X=Fail) |
| 26.5mm | 100.0 | | |
| 19.0mm | 99.2 | | |
| 16.0mm | 97.6 | | |
| 13.2mm | 95.0 | | |
| 9.5mm | 92.1 | | |
| 4.75mm | 87.4 | | |
| 2.36mm | 83.8 | | |
| 1.18mm | 79.7 | | |
| 0.600mm | 73.2 | | |
| 0.300mm | 62.7 | | |
| 0.150mm | 46.5 | | |
| 0.075mm | 29.1 | | |

* (no specification provided)

Material Description

Silty/Clayey Sand some fine Gravel

Atterberg Limits (ASTM D 4318)

PL= _____ LL= _____ PI= _____

Classification

USCS (D 2487)= _____ AASHTO (M 145)= _____

Coefficients

D₉₀= 7.0505 D₈₅= 2.9939 D₆₀= 0.2630
D₅₀= 0.1723 D₃₀= 0.0777 D₁₅= _____
D₁₀= _____ C_u= _____ C_c= _____

Remarks

F.M.=1.75

Date Received: Nov 24,2022 Date Tested: Nov 28,2022

Tested By: R.C

Checked By: J.Hopwood-Jones

Title: Lab Manager

Location: TP SS-2 Sample Number: SS-2 Depth: 25cm-1m

Date Sampled: Nov 24,2022

McINTOSH PERRY

Client: Trever Watkins
Project: 273&275 Russ Bradley Rd.

Project No: CCO-221643-01

Figure

GRAIN SIZE DISTRIBUTION TEST DATA

2022-11-29

Client: Trever Watkins

Project: 273&275 Russ Bradley Rd.

Project Number: CCO-221643-01

Location: TP SS-2

Depth: 25cm-1m

Sample Number: SS-2

Material Description: Silty/Clayey Sand some fine Gravel

Sample Date: Nov 24,2022

Date Received: Nov 24,2022

Tested By: R.C

Test Date: Nov 28,2022

Checked By: J.Hopwood-Jones

Title: Lab Manager

Sieve Test Data

| Dry Sample and Tare (grams) | Tare (grams) | Cumulative Pan Tare Weight (grams) | Sieve Opening Size | Cumulative Weight Retained (grams) | Percent Finer | Percent Retained |
|-----------------------------|--------------|------------------------------------|--------------------|------------------------------------|---------------|------------------|
| 926.14 | 0.00 | 0.00 | 26.5mm | 0.00 | 100.0 | 0.0 |
| | | | 19.0mm | 7.84 | 99.2 | 0.8 |
| | | | 16.0mm | 22.03 | 97.6 | 2.4 |
| | | | 13.2mm | 46.69 | 95.0 | 5.0 |
| | | | 9.5mm | 73.48 | 92.1 | 7.9 |
| | | | 4.75mm | 116.36 | 87.4 | 12.6 |
| | | | 2.36mm | 149.73 | 83.8 | 16.2 |
| | | | 1.18mm | 188.07 | 79.7 | 20.3 |
| | | | 0.600mm | 248.42 | 73.2 | 26.8 |
| | | | 0.300mm | 345.28 | 62.7 | 37.3 |
| | | | 0.150mm | 495.39 | 46.5 | 53.5 |
| | | | 0.075mm | 656.59 | 29.1 | 70.9 |

Fractional Components

| Cobbles | Gravel | | | Sand | | | | Fines | | |
|---------|--------|------|-------|--------|--------|------|-------|-------|------|-------|
| | Coarse | Fine | Total | Coarse | Medium | Fine | Total | Silt | Clay | Total |
| 0.0 | 0.8 | 11.8 | 12.6 | 4.4 | 14.3 | 39.6 | 58.3 | | | 29.1 |

| D ₅ | D ₁₀ | D ₁₅ | D ₂₀ | D ₃₀ | D ₄₀ | D ₅₀ | D ₆₀ | D ₈₀ | D ₈₅ | D ₉₀ | D ₉₅ |
|----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|
| | | | | 0.0777 | 0.1158 | 0.1723 | 0.2630 | 1.2299 | 2.9939 | 7.0505 | 13.2424 |

| |
|-------------------------|
| Fineness Modulus |
| 1.75 |