SERVICING & STORMWATER MANAGEMENT REPORT 476 WILBROD STREET, OTTAWA



Building Perspective Prepared by Arca-Verde Architecture Inc.

Project No.: CCO-22-3130

City File No.: D07-12-22-0051

Prepared for:

Sleepwell Property Management 423 Bronson Avenue Ottawa, Ontario K1R 6J5

Prepared by:

McIntosh Perry Consulting Engineers Ltd. 115 Walgreen Road Carp, ON K0A 1L0

April 27th, 2022

Revised December 7th, 2022

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1.0 PROJECT DESCRIPTION

1.1 Purpose

McIntosh Perry (MP) has been retained by Sleepwell Property Management to prepare this Servicing and Stormwater Management Report in support of the Site Plan Control for the proposed development located at 476 Wilbrod Street within the City of Ottawa.

The main purpose of this report is to present a servicing and stormwater management design for the development in accordance with the recommendations and guidelines provided by the City of Ottawa (City), the Rideau Valley Conservation Authority (RVCA), and the Ministry of the Environment, Conservation and Parks (MECP). This report will address the water, sanitary, and storm sewer servicing for the development, ensuring that existing and available services will adequately service the proposed development.

This report should be read in conjunction with the following drawings:

- CCO-22-3130, C101 Grading, Drainage, Erosion, Sediment Control, and Servicing Plan
- CCO-22-3130, PRE Pre-Development Drainage Area Plan (Appendix E)
- CCO-22-3130, POST Post-Development Drainage Area Plan (Appendix F)

1.2 Site Description



Figure 1: Site Map

The subject property, herein referred to as the site, is located at 476 Wilbrod Street within the Rideau-Vanier Ward. The site covers approximately 0.09 ha and is located along Wilbrod Street between Cobourg Street and Charlotte Street. The site is zoned for Residential Fifth Density (R5B H (18)). See Site Location Plan in Appendix A for more details.

1.3 Proposed Development and Statistics

The proposed development consists of a 3-storey residential building addition to the existing residential building. Additionally, the development proposes a second and third storey addition to the existing 1-storey addition in the rear. The proposed development will contain 20 residential units. Refer to Site Plan prepared by Arca-Verde Architecture Inc in Appendix B for further details.

1.4 Existing Conditions and Infrastructures

The site currently contains a 2 ^{1/2}-storey detached building with a 1-storey rear addition.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal rights-of-way(s):

Wilbrod Street

- 305 mm diameter DI watermain,
- o 250 mm diameter PVC sanitary sewer, and
- o A 900 mm diameter Concrete storm sewer tributary to the Rideau River approximately 0.27 km downstream.

1.5 Approvals

The proposed development is subject to the City of Ottawa site plan control process. Site plan control requires the City to review, provided concurrence and approve the engineering design package. Permits to construct can be requested once the City has issued a site plan agreement.

An Environmental Compliance Approval (*ECA*) through the Ministry of Environment, Conservation and Parks (*MECP*) is not anticipated for the development since the site is located on one parcel of land, does not propose industrial usage, and is not located within a combined sewershed. As a result, the stormwater management system meets the exemption requirements under O.Reg 525/90.

2.0 BACKROUND STUDIES, STANDARDS, AND REFERENCES

2.1 Background Reports / Reference Information

As-built drawings of existing services, provided by the City of Ottawa Information centre, within the vicinity of the proposed site were reviewed in order to identify infrastructure available to service the proposed development.

A topographic survey (22-3223) of the site was completed by Mcintosh Perry Surveying Inc. and dated March 1st, 2022.

The Site Plan (A000) was prepared by Arca-Verde Architecture Inc. and dated February 10th, 2022 (*Site Plan*).

2.2 Applicable Guidelines and Standards

City of Ottawa:

- ◆ Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (Ottawa Sewer Guidelines)
 - Technical Bulletin ISTB-2014-01 City of Ottawa, February 2014. (ISTB-2014-01)
 - Technical Bulletin PIEDTB-2016-01 City of Ottawa, September 2016. (PIEDTB-2016-01)
 - Technical Bulletin ISTB-2018-01 City of Ottawa, January 2018. (ISTB-2018-01)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (ISTB-2018-03)
 - Technical Bulletin ISTB-2019-01 City of Ottawa, January 2019. (ISTB-2019-01)
 - Technical Bulletin ISTB-2019-02 City of Ottawa, February 2019. (ISTB-2019-02)
- Ottawa Design Guidelines Water Distribution City of Ottawa, July 2010. (Ottawa Water Guidelines)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 2014. (ISDTB-2014-02)
 - Technical Bulletin ISTB-2018-02 City of Ottawa, March 2018. (ISTB-2018-02)

Ministry of Environment, Conservation and Parks:

- ◆ Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (MECP Stormwater Design Manual)
- ◆ Design Guidelines for Sewage Works, Ministry of the Environment, 2008. (MECP Sewer Design Guidelines)

Other:

♦ Water Supply for Public Fire Protection, Fire Underwriters Survey, 2020. (FUS Guidelines)

3.0 PRE-CONSULTATION SUMMARY

A pre-consultation meeting was held with City staff on December 21st, 2021, regarding the proposed site servicing. Specific design parameters to be incorporated within this design include the following:

- > Pre-development and post-development flows shall be calculated using a time of concentration (Tc) no less than 10 minutes.
- ➤ Control 5 through 100-year post-development flows to the 5-year pre-development flows with a combined C value to a maximum of 0.50.

4.0 WATERMAIN

4.1 Existing Watermain

There is an existing 305 mm diameter DI watermain within Wilbrod Street. The site is located within the 1W pressure zone, as per the Water Distribution System mapping included in *Appendix C*. The existing building is serviced via 5/8" O.D. copper water service connected to the 305 mm diameter watermain within Wilbrod Street. There are two municipal hydrants on Wilbrod Street and two municipal hydrants on Charlotte Street available to service the proposed development. For hydrant locations, see the Hydrant Coverage Figure included in *Appendix C*.

4.2 Proposed Watermain

It is proposed to service the proposed building addition through the existing building. A mechanical consultant will need to review and confirm whether the existing service can provide sufficient pressure throughout the building addition or if upgrades to the existing building are required to accommodate the addition.

Table 1, below, summarizes the water supply design criteria obtained from the *Ottawa Water Guidelines* and utilized for the water analysis.

Site Area 0.09 ha

Residential 280 L/day/person

Residential Apartment – 1 Bedroom 1.4 person/unit

Max Day Peaking Factor - Residential 9.5 x avg. day

Peak Hour Peaking Factor - Residential 14.3 x avg. day

Table 1: Water Supply Design Criteria

The water analysis results have been summarized in *Table 2*, below. The fire flow demand accounted for both the existing floor area and the proposed area.

Table 2: Summary of Estimated Water Demand

Design Parameter	Total Flow (L/s) Proposed
Average Daily Demand	0.09
Max Day Demand	0.86
Max Day Demand + FUS (200 L/s)	200.86
Peak Hour Demand	1.30

The Fire Underwriters Survey 2020 (FUS) method was utilized to estimate the required fire flow for the site. Fire flow requirements were calculated per City of Ottawa Technical Bulletin *ISTB-2018-02*. The following parameters were assumed:

- Type of construction Wood Frame Construction
- Occupancy Type Limited Combustible
- Sprinkler Protection Non-Sprinklered System

The results of the calculations yielded a required fire flow of 12,000 L/min. The detailed calculations for the FUS can be found in Appendix C.

The City provided the estimated water pressures at both for the average day scenario, peak hour scenario and the max day plus fire flow scenario for the demands indicated by the correspondence in *Appendix C*. The resulting pressures for the boundary conditions results are shown in *Table 3*, below.

Table 3: Boundary	Conditions Results

Scenario	Proposed Demands (L/S)	Connection 1 HGL (m H₂O)*/kPa		
Average Day Demand	0.09	47.2 / 463.0		
Maximum Daily + Fire Flow Demand	200.86	38.2 / 374.7		
Peak Hourly Demand	1.30	37.9 / 371.8		
*Adjusted for an estimated ground elevation of 58.75m above the connection point for connection.				

The normal operating pressure range is anticipated to be 372 kPa to 463 kPa and will not be less than 275 kPa (40 psi) or exceed 689 kPa (100 psi). The proposed watermains will meet the minimum required 20 psi (140 kPa) from the *Ottawa Water Guidelines* at the ground level under maximum day demand and fire flow conditions.

To confirm the adequacy of fire flow to protect the proposed development, public fire hydrants within 150 m of the proposed building were analysed per City of Ottawa *ISTB 2018-02* Appendix I Table 1. Based on City guidelines (*ISTB-2018-02*), the existing hydrants can provide adequate fire protection to the proposed development. The results are summarized in *Table 4*, below.

Table 4: Fire Protection Confirmation

Building	Fire Flow Demand	Fire Hydrant(s)	Fire Hydrant(s)	Combined Fire
	(L/min.)	within 75m	within 150m	Flow (L/min.)
476 Wilbrod Street	12,000 (FUS)	0	4 public	15,200

5.0 SANITARY DESIGN

5.1 Existing Sanitary Sewer

There is an existing 250 mm diameter sanitary sewer within Wilbrod Street, fronting the subject site. The subject site currently contributes wastewater to the Wilbrod Street sewer system.

The existing building is serviced via a 100-125mm diameter ABS/Transite/PVC service. CCTV inspection of the existing sanitary service was completed by Drain-All on August 25, 2022. The footage revealed fine roots at two joints. Based on the information provided by the CCTV inspection, the existing sanitary service is adequate for reuse but will first require cleaning.

5.2 Proposed Sanitary Sewer

It is proposed to service the proposed building addition through the existing building. A mechanical consultant will need to review and confirm whether upgrades to the existing building are required to accommodate the addition.

Table 5, below, summarizes the wastewater design criteria identified by the *Ottawa Sewer Guidelines*.

Design ParameterValueSite Area0.09 haResidential280 L/person/day1 Bedroom Apartment1.4 persons/unitResidential Peaking Factor3.69Extraneous Flow Allowance0.33 L/s/ha

Table 5: Sanitary Design Criteria

Table 6, below, summarizes the estimated wastewater flow from the proposed development. Refer to *Appendix D* for detailed calculations.

Table 6: Summary of Estimated Sanitary Flow

Design Parameter	Total Flow (L/s)
Total Estimated Average Dry Weather Flow	0.10
Total Estimated Peak Dry Weather Flow	0.35
Total Estimated Peak Wet Weather Flow	0.37

The full flowing capacity of a 100 mm diameter sanitary service at an assumed slope of 1.0% is estimated to be 5.39 L/s. Based on *Table 6*, above, the existing sanitary service is assumed to be adequately sized for the proposed development.

6.0 STORM SEWER DESIGN

6.1 Existing Storm Sewers

Stormwater runoff from the site is currently tributary to the Rideau River within the Lower Rideau River sub-watershed. There is an existing 900 mm diameter storm sewer within Wilbrod Street that is available to service the site. The existing sewer is tributary to the Rideau River approximately 0.27 km downstream (outlet ID OUT04451). Based on coordination with the developer, there are no sump pump systems currently installed within the existing building.

6.2 Proposed Storm Sewers

A new 250 mm diameter storm service is proposed to be extended from the 900 mm storm sewer within Wilbrod Street to the rear of the site. Refer to drawing *C101* for a detailed servicing layout.

Runoff from the parking area and rear yard (B1) will be collected and conveyed to a catch basin in the parking area. The catch basin will contain a 74mm plug style ICD.

Runoff from the side yard (B2) will be unrestricted and maintain existing drainage patterns.

A new foundation drainage system is proposed for the building addition, complete with a sump pump which will outlet to surface. Detailed design for the foundation drainage system will be completed by others In support of the building permit application.

See CCO-22-3130 - *POST* include in *Appendix F* of this report for more details. The Stormwater Management design for the subject property will be outlined in *Section 7.0* of this report.

7.0 PROPOSED STORMWATER MANAGEMENT

7.1 Design Criteria and Methodology

The following design criteria have been employed in developing the stormwater management design for the site as directed by the City and the RVCA:

Quality Control

• Quality controls are not required for the site based on correspondence with the RVCA included in *Appendix B*.

Quantity Control

- Any storm events greater than 5-year, up to 100-year, and including the 100-year storm event must be detained on site.
- Based on coordination with City Staff, runoff from area B1 is permitted up to 15.0 L/s.

7.2 Runoff Calculations

Runoff calculations presented in this report are derived using the Rational Method, given as:

Q = 2.78CIA (L/s)

Where: C = Runoff coefficient

= Rainfall intensity in mm/hr (City of Ottawa IDF curves)

A = Drainage area in hectares

It is recognized that the Rational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any SWM facility sized using this method is expected to function as intended. The following coefficients were used to develop an average C for each area:

Roofs/Concrete/Asphalt	0.90
Undeveloped and Grass	0.20

As per the *City of Ottawa - Sewer Design Guidelines*, the 5-year balanced 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

7.3 Pre-Development Drainage

It has been assumed that the site contains no stormwater management controls for flow attenuation. Runoff from the existing roof is directed towards the side yard. Surface runoff flows overland towards Wilbrod Street and Charlotte Street. The estimated pre-development peak flows for the 5 and 100-year events within the development area are summarized below in *Table 7*. See CCO-22-3130 - *PRE* in *Appendix E* and *Appendix G* for calculations.

Drainage Area (ha) C C (5-Year) (100-Year) 5-Year 100-Year

0.66

7.14

13.85

0.58

Table 7: Pre-Development Runoff Summary

7.4 Post-Development Drainage

Α1

0.04

Based on the correspondence with City staff included in *Appendix B*, the development will be required to restrict area B1 to a maximum release rate of 15.0 L/s for the 5- through 100-year events. Please see summary in *Table 8*, below.

Table 8: Required Restricted Flow for Area B1

Drainage Area	Area (ha)	C (5-Year)	Q (L/s) 5-Year (Max)
B1	0.04	0.65	15.0

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See CCO-22-3130 - *POST* in *Appendix F* of this report for more details. A summary of the post-development runoff calculations can be found below.

Table 9: Post-Development Runoff Summary

Drainage Area	Area		estricted w (L/s)		tricted w (L/s)	100-year Storage	100-year Storage
Alea	(ha)	5-Year	100-Year	5-Year	100-Year	Required (m ³)	Available (m³)
B1	0.04	6.75	13.02	6.75	13.02		
B2	0.01	1.66	3.16	1.66	3.16		
Total	0.04	8.41	16.19	8.41	16.18	0.0	0.0

Runoff from Area B1 will be collected by a catchbasin and a rear yard landscaping catchbasin. The proposed catchbasin will include a 74mm plug style ICD. Area B1 will have a maximum release rate of 13.02 L/s for the 5- through 100-year events. Due to area B1 having an uncontrolled release rate less than 15.0 L/s, the ICD will not restrict runoff for the 5- through 100-year storm events. In the event of flows exceeding 15.0 L/s, water will pond in the parking area to an elevation of 71.69 before spilling towards the Charlotte Street right-of-way. The ICD has been designed to have a maximum release rate of 15.0 L/s at a design head corresponding to a water elevation of 71.69.

Runoff from area B2 will be unrestricted and maintain existing drainage patterns.

Foundation drainage for the building addition is proposed to outlet to surface. Detailed design of the foundation drainage system to be provided by others.

8.0 EROSION AND SEDIMENT CONTROL

8.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all-natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Silt fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catch basins and filter fabric is to be placed under the grates of all existing catch basins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures are to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the *Site Grading, Drainage and* Sediment & *Erosion Control Plan* for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

8.2 Permanent Measures

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.

9.0 SUMMARY

- A 2nd and 3rd-storey addition to the existing building and 3-storey building addition is proposed to be constructed at 476 Wilbrod Street. The *Site Plan* proposes a total of 20 residential units, with street access from Charlotte Street.
- The FUS method estimated fire flow indicated that 12,000 L/min is required for the proposed development.
- The development is estimated to have a peak wet weather flow of 0.37 L/s.
- Based on coordination with City Staff, the development will be required to attenuate postdevelopment flows for Area B1 to a maximum of 15.0 L/s.
- Quality controls are not required for the development based on correspondence with the RVCA.

10.0 RECOMMENDATION

Based on the information presented in this report, we recommend that City of Ottawa approve this Servicing and Stormwater Management report in support of the proposed development at 476 Wilbrod Street.

This report is respectfully being submitted for approval.

Regards,

McIntosh Perry Consulting Engineers Ltd.



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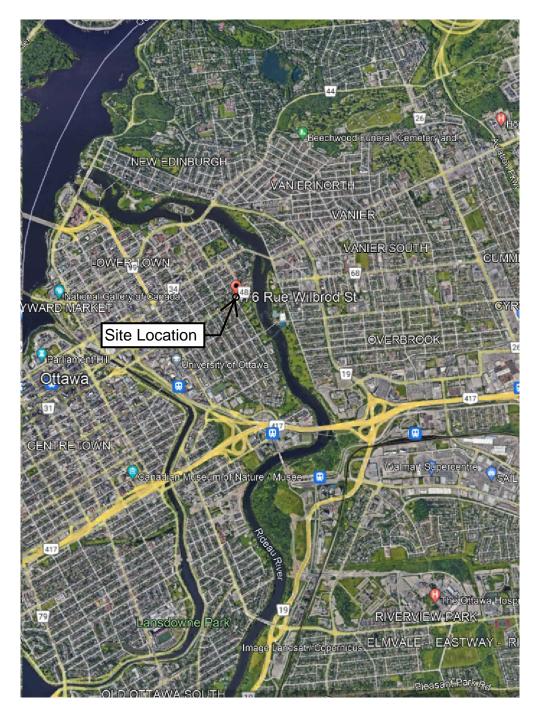
11.0 STATEMENT OF LIMITATIONS

This report was produced for the exclusive use of <u>Sleepwell Property Management</u>. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Parks and Climate Change, City of Ottawa and local approval agencies. McIntosh Perry reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by McIntosh Perry and site visits were performed, no field verification/measures of any information were conducted.

Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. McIntosh Perry accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, McIntosh Perry should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.

APPENDIX A KEY PLAN



CLIENT:						
SLEEPWELL PROPER	TY MANAGE	MENT				
PROJECT: 476 WILBROD STREET						
TITLE: SITE LOC	SITE LOCATION					
MCINTOSH PERRY 115 Walgreen Road, RR3, Carp, ON KOA 1LO	PROJECT: CCO-22-3130	FIGURE:				
Tel: 613-836-2184 Fax: 613-836-3742 www.mcintoshperry.com	DATE: APR. 27, 2022	SCALE: N.T.S				

APPENDIX B BACKGROUND DOCUMENTS

Pre-Application Consultation Meeting Notes

Property Address: 476 Wilbroad Avenue December 21, 2021

Attendees:

City of Ottawa:

Jessica Button (File Lead), Christopher Moise (Urban Design), Adrian van Wyk (Heritage), Mohamed Fawzi (Engineering), Craig Hamilton (Committee of Adjustment), Wally Dubyk (Transportation)

Applicant Team: Collin Hopkins (Sleepwell), Michael Pexito

Community: John Verbaas (Action Sandi Hill)

Meeting notes:

Opening & attendee introduction

- Introduction of meeting attendees
- Overview of proposal:
 - Proposal would be subject to a Zoning By-law Amendment (Minor) / Minor Variance, Site Plan Control, Complex Process.
 - Additional 8 1-bedroom rental units to be located in a rear addition. The
 existing rooming house is proposed to be renovated into 12 1-bedroom
 apartments. 20 units total. 5 new parking spaces are also planned.

Preliminary comments and questions from staff and agencies, including follow-up actions:

Planning (Jessica Button)

- Application required
 - Zoning By-law Amendment / Minor Variance
 - Site Plan Control Complex (Manager Approval, Public Consultation)
 - Property is designated as Low Profile Residential. See the Sandy Hill Secondary Plan for additional policy direction.
 - Review and appropriately address the policies in the City's new Official Plan which may be in force and effect at the time of application. Note that the Secondary Plan Polices are included in the Sandy Hill Character Area of the Central and East Secondary Plan. The property is designated Local Neighbourhood with a maximum height of 4 storeys.
 - The following City policies or guidelines are applicable to the site:
 - Tree Protection By-law
 - Waste Management Guidelines
 - The site includes the following zones:

- R5B H(18)
- Mature Neighbourhood Overlay (N/A in R5)
- Heritage Overlay
- After 20m form the front lot line, a 6.0m interior side yard setback is required. Consider tree protection, width of pathway access, amenity space, and addition permissions in the rear yard minimum setback.
- Refer to amenity area requirements of the ZBL and demonstrate complaince.
- Refer to section 111 Bicycle Parking and demonstrate complaince. Staff strongly encourage 1 space per unit of sheltered or interior bicycle parking.
- Review provisions of Section 65, Permitted Encroachments into yards, and consider the location of the deck, setback, design and usability of space. Also consider tree protection in this location.
- Refer to Section 60 Heritage Overlay and demonstrate compliance
- Floor plans will be required
- Provide a zoning table and demonstrate complaince or highlight relief requested
- Provide building height of existing and proposed building.
- Garbage storage and loading shall be interior to the building, located away from existing residential uses.
- Confirm easement / access for parking
- Dimension concrete walkway. This appears narrow especially adjacent to parking
- A tree protection plan will be required
- Cash in lieu of parkland dedication will be required

Urban Design (Christopher Moise)

- This proposal does not run along one of the City's Design Priority Areas and does not need to attend the City's UDRP for design review. Staff will be responsible for evaluating the proposal and providing design direction;
- We thank the applicant for providing detailed plans and rendering images at the pre-consultation which greatly assist in our understanding of the project and review of the proposal. We generally appreciate the intent to keep the scale and material choices in line with the existing building and believe a reasonable approach has been taken in locating additional massing on the site;
- We do however, recommend clarity be provided on the site plan drawing in two areas:
 - How is the rear yard amenity space being changed/improved?
 Please illustrate what is existing and what is new;
 - How are cars intended to access the five indicated parking spaces?
 Please illustrate how vehicles are intended to reach the parking;
- Bike storage: We recommend a suitable ratio of protected bike parking be provided on the site, either in the building or in the rear yard in a suitable location:

- Garbage storage: We recommend protected garbage storage be provided on the site, either in the building or in the rear yard in a suitable location;
- Landscaping: We recommend providing a landscaping plan that illustrates landscaping approach on the site;
- Trees: We recommend opportunities for new trees be illustrated on the landscape plan;
- We recommend accessibility standards be checked as providing suitable access will change the design presented;
- A scoped Design Brief is a required submittal (and separate from any UDRP submission) for all Site Plan/Re-zoning applications and can be combined with the Planning Rationale. Please see the Design Brief Terms of Reference provided (note the renderings provided at the preconsultation meeting are sufficient).
- This is an exciting project in an area full of potential. We look forward to helping you achieve its goals with the highest level of design resolution. We are happy to assist and answer any questions regarding the above. Good luck.

Engineering (Mohamed Fawzi) Water Boundary Conditions:

Will be provided at request of consultant. Requests must include the location of the service and the expected loads required by the proposed development. Please provide the following and <u>submit Fire Flow Calculation Sheet</u> per FUS method with the request:

- Location of service
- Type of development and amount of required fire flow (per FUS method <u>include</u> <u>FUS calculation sheet with request</u>)
- Average Daily Demand (I/s)
- Maximum Hourly Demand (I/s)
- Maximum Daily Demand (I/s)
- Water Supply Redundancy Fire Flow:
 Applicant to ensure that a second service with an inline valve chamber be provided where the average daily demand exceeds 50 m³ / day (0.5787 l/s per day)

Water services larger than 19 mm require a Water Data Card. Please complete card and submit.

Stormwater Management:

Coefficient (C) of runoff determined **as per existing conditions** but in no case more than 0.5.

• TC = To be calculated, minimum 10 minutes

- Any storm events greater than 5 year, up to 100 year, and including 100-year storm event must be detained on site.
- Foundation drains are to be independently connected to sewer main unless being pumped with appropriate back up power, sufficient sized pump and back flow prevention.
- Roof drains are to be connected downstream of any incorporated ICD within the SWM system.

Stormwater Management Criteria (Quality Control)

Include a section in the SWM report concerning quality control requirements. It is the consultant's responsibility to check with the relevant Conservation Authority for quality control issues and include this information in the SWM report.

Noise and Vibration Study:

Noise Study required due to Major Collector Road (Charlotte Street).

Phase I and Phase II ESA:

- Phase I ESA is required; Phase II ESA may be required depending on the results of the Phase I ESA. Phase I ESA must include an EcoLog ERIS Report.
- Phase I ESA and Phase II ESAs must conform to clause 4.8.4 of the Official Plan that requires that development applications conform to Ontario Regulation 153/04.

Required Studies

- Stormwater Management Report
- Servicing Brief
- Geotechnical Study
- Phase I ESA
- Phase II ESA (depends on outcome of Phase I)
- Noise Study

Required Plans

- Site Servicing Plan
- Grade Control and Drainage Plan
- Erosion and Sediment Control Plan (Can be combined with grading plan)
- The Servicing Study Guidelines for Development Applications are available at the following address: https://ottawa.ca/en/city-hall/planning-and-development-application-review-process/development-application-review-process/development-application-study-guidelines-development-applications
- 2. Servicing and site works shall be in accordance with the following documents:
 - ⇒ Ottawa Sewer Design Guidelines (October 2012)

- ⇒ Ottawa Design Guidelines Water Distribution (2010)
- ⇒ Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
- ⇒ City of Ottawa Environmental Noise Control Guidelines (January, 2016)
- ⇒ City of Ottawa Park and Pathway Development Manual (2012)
- ⇒ City of Ottawa Accessibility Design Standards (2012)
- ⇒ Ottawa Standard Tender Documents (latest version)
- ⇒ Ontario Provincial Standards for Roads & Public Works (2013)
- 3. Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at lnformationCentre@ottawa.ca or by phone at (613) 580-2424 x.44455).
- 4. Any proposed work in utility easements requires written consent of easement owner.
- 5. If the existing services are proposed to be reused a CCTV Report confirming that the existing services are in adequate condition will be required.

Transportation (Wally Dubyk)

- Wilbrod Street is classified as a local road. There are no additional protected ROW limits identified in the OP.
- The development site proposes 8 bedroom units & 12 bedroom apartments and 5 parking spaces. This development would not generate sufficient traffic to warrant a TIA report. Submission of TDM checklists is encouraged even if a TIA study report is not considered to be required.
- The purchaser, tenant or sub-lessee acknowledges the unit being rented/sold is not provided with any on-site parking and should a tenant/purchaser have a vehicle for which they wish to have parking that alternative and lawful arrangements will need to be made to accommodate their parking need at an alternative location. The Purchaser/Tenant also acknowledges that the availability and regulations governing on-street parking vary; that access to on-street parking, including through residential on-street parking permits issued by the City cannot be guaranteed now or in the future; and that a purchaser, tenant or sub-lessee intending to rely on on-street parking for their vehicle or vehicles does so at their own risk.
- Please keep in mind that on street parking is not a viable option for tenants. Ensure that potential tenants are aware that there is no provision for parking.

- Permanent structures such as curbing, stairs, retaining walls, and underground parking foundation also bicycle parking racks are not to extend into the City's right-of-way limits.
- The Owner shall be required to enter into maintenance and liability agreement for all pavers, plant and landscaping material placed in the City right-of-way and the Owner shall assume all maintenance and replacement responsibilities in perpetuity.
- Bicycle parking spaces are required as per Section 111 of the Ottawa Comprehensive Zoning By-law. Bicycle parking spaces should be located in safe

Heritage (Adrian van Wyk)

- This property is listed on the City's Heritage Register. A Heritage Permit is not required to make alterations to the property; however, should the applicant wish to demolish the existing building, the City must be provided with at least 60 days' written notice of the applicant's intention to demolish.
- The property is categorized as a Category 3 property within the Sandy Hill Cultural Heritage Character Area. Section 5.2.11 of the Sandy Hill Cultural Heritage Character Area Guidelines should be consulted as it relates to additions to Category 3 properties:
 - "5.2.11 Additions
 - "1. Additions to historic buildings should be sympathetic to the existing building, subordinate to, and distinguishable from the original. Falsifying a past architectural style in a new addition is strongly discouraged.
 - "2. Additions should generally be located in the rear yard.
 - "3. Where not located in the rear yard, additions should be consistent with the streetscape with respect to setback, height, scale, and massing.
 - "4. New additions should respect the existing wall to window ratio and proportion of the existing building."
- The Heritage Planning Branch has no concerns with the proposal at this time.

Forestry (Mark Richardson)

TCR requirements:

- 1. a Tree Conservation Report (TCR) must be supplied for review along with the suite of other plans/reports required by the City
 - a. an approved TCR is a requirement of Site Plan approval.
 - b. The TCR may be combined with the LP provided all information is supplied
- 2. As of January 1 2021, any removal of privately-owned trees 10cm or larger in diameter, or publicly (City) owned trees of any diameter requires a tree permit issued under the Tree Protection Bylaw (Bylaw 2020 340); the permit will be based on an approved TCR and made available at or near plan approval.
- 3. The Planning Forester from Planning and Growth Management as well as foresters from Forestry Services will review the submitted TCR
 - a. If tree removal is required, both municipal and privately-owned trees will be addressed in a single permit issued through the Planning Forester
 - b. Compensation may be required for city owned trees if so, it will need to be paid prior to the release of the tree permit
- 4. the TCR must list all trees on site, as well as off-site trees if the CRZ extends into the developed area, by species, diameter and health condition
- 5. please identify trees by ownership private onsite, private on adjoining site, city owned, co-owned (trees on a property line)
- 6. the TCR must list all trees on adjacent sites if they have a critical root zone that extends onto the development site
- 7. If trees are to be removed, the TCR must clearly show where they are, and document the reason they cannot be retained
- 8. All retained trees must be shown and all retained trees within the area impacted by the development process must be protected as per City guidelines available at Tree Protection Specification or by searching Ottawa.ca
 - a. the location of tree protection fencing must be shown on a plan
 - b. show the critical root zone of the retained trees
 - c. if excavation will occur within the critical root zone, please show the limits of excavation
- the City encourages the retention of healthy trees; if possible, please seek opportunities for retention of trees that will contribute to the design/function of the site.
- 10. For more information on the process or help with tree retention options, contact Mark Richardson mark.richardson@ottawa.ca or on City of Ottawa

LP tree planting requirements:

For additional information on the following please contact tracy.smith@Ottawa.ca

Minimum Setbacks

- Maintain 1.5m from sidewalk or MUP/cycle track.
- Maintain 2.5m from curb
- Coniferous species require a minimum 4.5m setback from curb, sidewalk or MUP/cycle track/pathway.
- Maintain 7.5m between large growing trees, and 4m between small growing trees. Park or open space planting should consider 10m spacing.
- Adhere to Ottawa Hydro's planting guidelines (species and setbacks)
 when planting around overhead primary conductors.

Tree specifications

- Minimum stock size: 50mm tree caliper for deciduous, 200cm height for coniferous.
- Maximize the use of large deciduous species wherever possible to maximize future canopy coverage
- Tree planting on city property shall be in accordance with the City of Ottawa's Tree Planting Specification; and include watering and warranty as described in the specification (can be provided by Forestry Services).
- Plant native trees whenever possible
- No root barriers, dead-man anchor systems, or planters are permitted.
- No tree stakes unless necessary (and only 1 on the prevailing winds side of the tree)

Hard surface planting

- Curb style planter is highly recommended
- No grates are to be used and if guards are required, City of Ottawa standard (which can be provided) shall be used.
- Trees are to be planted at grade

Soil Volume

Please ensure adequate soil volumes are met:

Tree Type/Size	Single Tree Soil Volume (m3)	Multiple Tree Soil Volume (m3/tree)
Ornamental	15	9
Columnar	15	9
Small	20	12
Medium	25	15

Large	30	18			
Conifer	25	15			

Please note that these soil volumes are not applicable in cases with Sensitive Marine Clay.

Sensitive Marine Clay

 Please follow the City's 2017 Tree Planting in Sensitive Marine Clay guidelines

Committee of Adjustment (Craig Hamilton)

- Variances have been accurately identified and Staff do not have particular concerns at this time
- Please contact Committee of Adjustment Coordinators directly for process, fees and timelines relating to the Minor Variance at <u>CofA@ottawa.ca</u>
- Please do not hesitate to contact me at <u>Craig.hamilton@ottawa.ca</u> should you
 wish to review submission form correctness and plans prior to formally
 applying for minor variance.

City Surveyor

- The determination of property boundaries, minimum setbacks and other regulatory constraints are a critical component of development. An Ontario Land Surveyor (O.L.S.) needs to be consulted at the outset of a project to ensure properties are properly defined and can be used as the geospatial framework for the development.
- Topographic details may also be required for a project and should be either carried out by the O.L.S. that has provided the Legal Survey or done in consultation with the O.L.S. to ensure that the project is integrated to the appropriate control network.

Questions regarding the above requirements can be directed to the City's Surveyor, Bill Harper, at Bill.Harper@ottawa.ca

Community Association (John Verbaas)

- Bicycle parking should be provided for each unit. Secure bicycle parking should be provided.
- Garbage needs to be planed for and accommodated property.

Next steps

 City Staff encourage applicant to discuss the proposal with Councillor, community groups and neighbours

 City staff to send follow-up email confirming submission requirements 								

476 WILBROD APARTMENTS

Site Plan Approval Drawing List					
Sheet Number	Sheet Name				
SA-00	COVER PAGE				
SA-01	RATIONALE SUMMARY				
SA-02	SITE PLAN				
SA-03	SITE & GRADING PLAN				
SA-04	EXISTING ELEVATION - NORTH & SOUTH				
SA-05	EXISTING ELEVATION - EAST				
SA-06	EXISTING ELEVATION - WEST				
SA-07	ELEVATION - NORTH & SOUTH				
SA-08	ELEVATION - EAST				
SA-09	ELEVATION - WEST				
SA-10	COLOUR RENDERING - BEFORE				
SA-11	COLOUR RENDERINGS - AFTER				
SA-12	BASEMENT PLAN				
SA-13	GROUND FLOOR PLAN				
SA-14	SECOND FLOOR PLAN				
SA-15	THIRD FLOOR PLAN				
SA-16	LANDSCAPE PLAN				
SA-17	GARBAGE COLLECTION PLAN				
SA-18	GARAGE ENCLOSURE ELEVATIONS				
SA-19	ZONING CHART				



WILBROD APARTMENTS

SITE PLAN APPROVAL COVER PAGE

SCALE:

FEBRUARY 10TH, 2022



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THE DESIGN INTENT OF THIS NEWLY PROPOSED REDEVELOPMENT IS TO COMPLEMENT THE ORIGINAL DESIGN OF THE EXISTING BUILDING WHILE ADDING SPACE AND REVITALIZING THE INTERIOR. THE NEWLY PROPOSED ADDITION ADDS ~100M^2 TO EACH OF THE EXISTING THREE STOREYS TO CREATE 20 NEW APARTMENTS. THE FORM OF THE ADDITION IS TO MAINTAIN A SIMILAR PRESENCE TO THE EXISTING BUILDING BY; HAVING SIMILAR BRICK COURSING & COLOUR, SIMILAR SIDING FINISH & COLOUR, SAME ROOF SLOPE, SAME ROOF HEIGHT, AND SIMILAR STYLE DORMERS.

CHANGES TO THE EXTERIOR WERE ONLY MADE AS A REQUIREMENT FOR DESIGN VIABILITY AND CODE COMPLIANCE.

WILBROD APARTMENTS

SITE PLAN APPROVAL RATIONALE SUMMARY

SCALE:

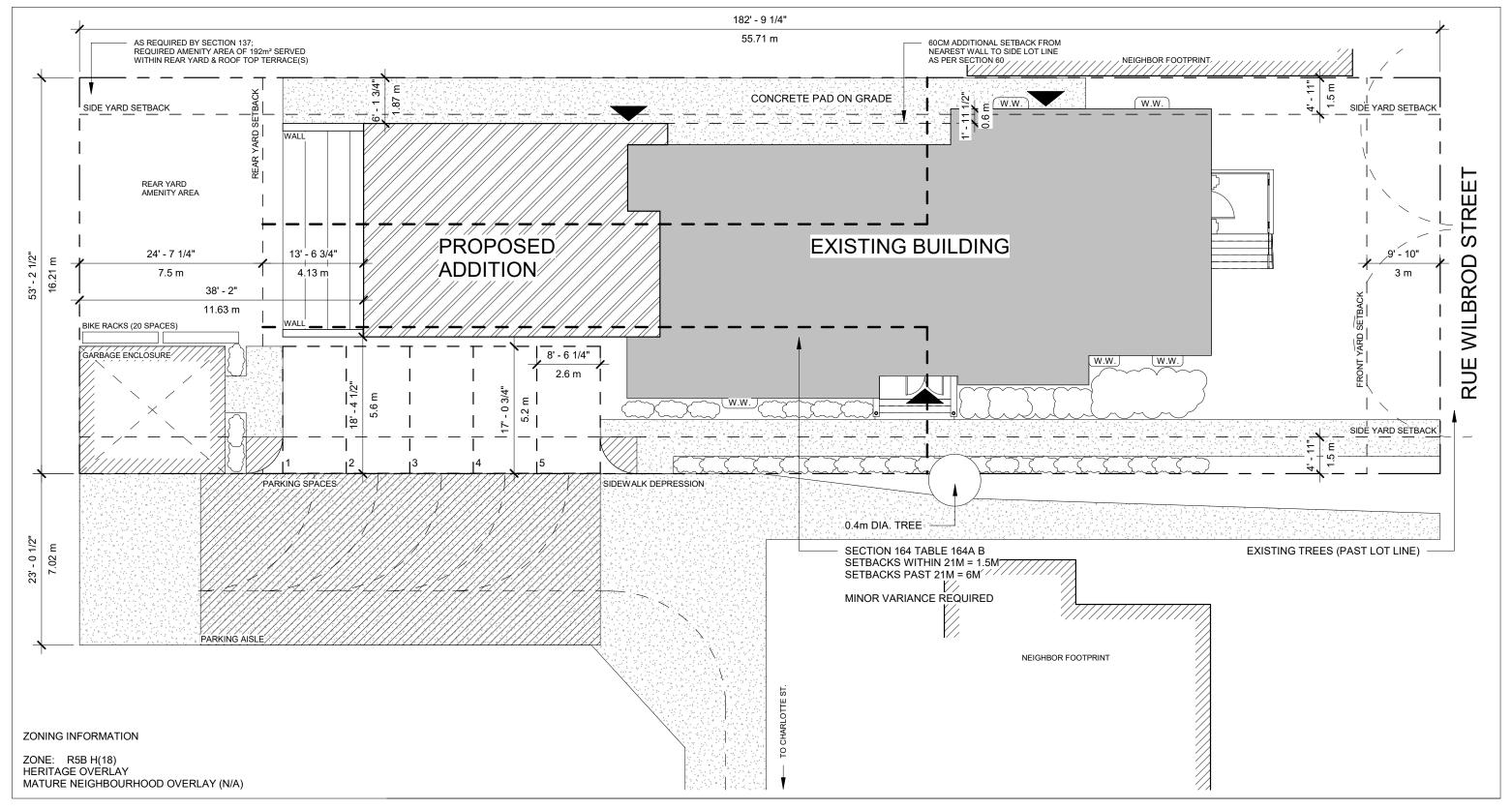
FEBRUARY 10TH, 2022



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WILBROD APARTMENTS

SITE PLAN APPROVAL SITE PLAN

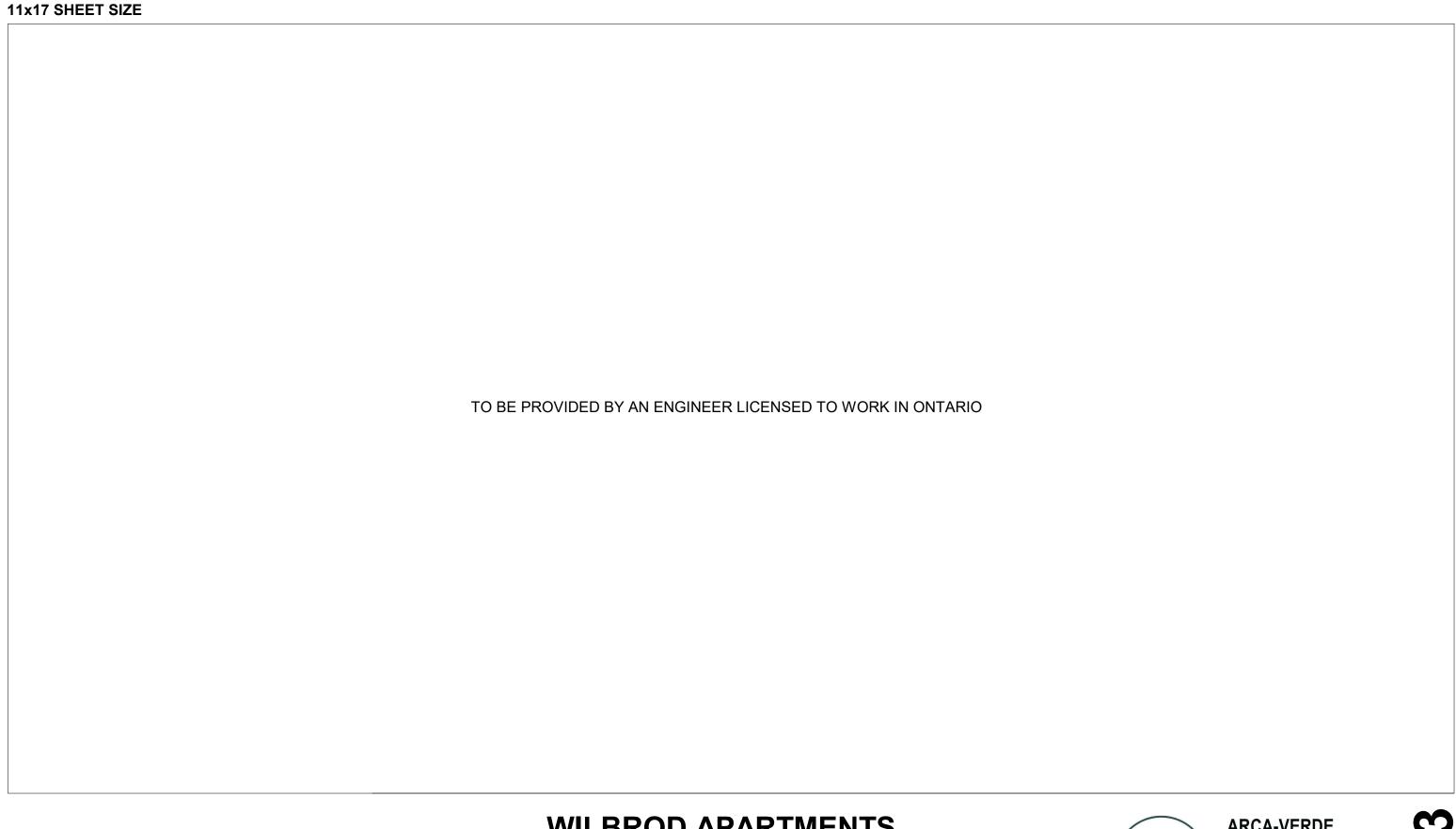
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WILBROD APARTMENTS

SITE PLAN APPROVAL SITE & GRADING PLAN

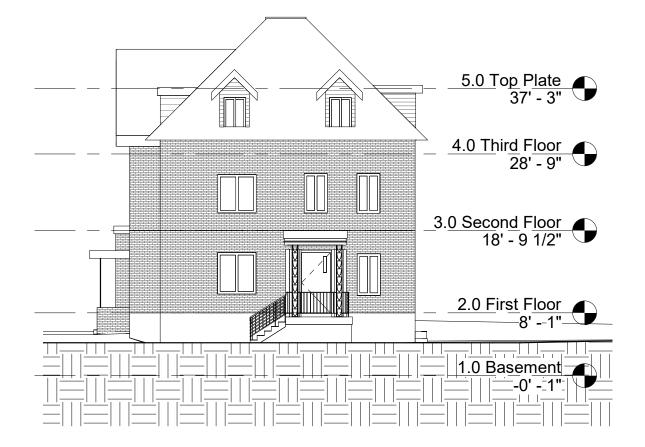
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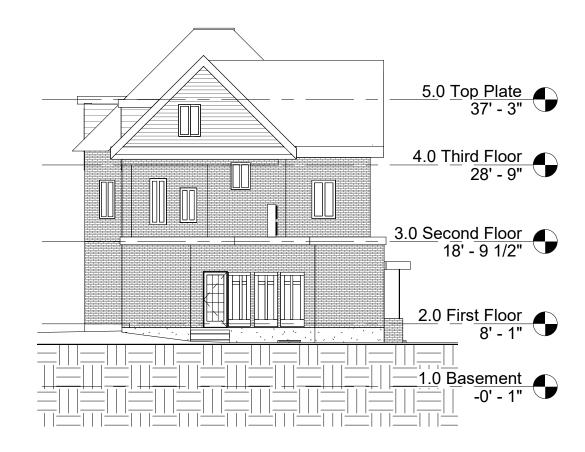
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EXISTING BUILDING HEIGHT: 12.0m NEW ADDITION HEIGHT: 12.0m

WILBROD APARTMENTS

SITE PLAN APPROVAL
EXISTING ELEVATION - NORTH & SOUTH

SCALE: 1:150

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WILBROD APARTMENTS

SITE PLAN APPROVAL EXISTING ELEVATION - EAST

SCALE: 1:150

FEBRUARY 10TH, 2022



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WILBROD APARTMENTS

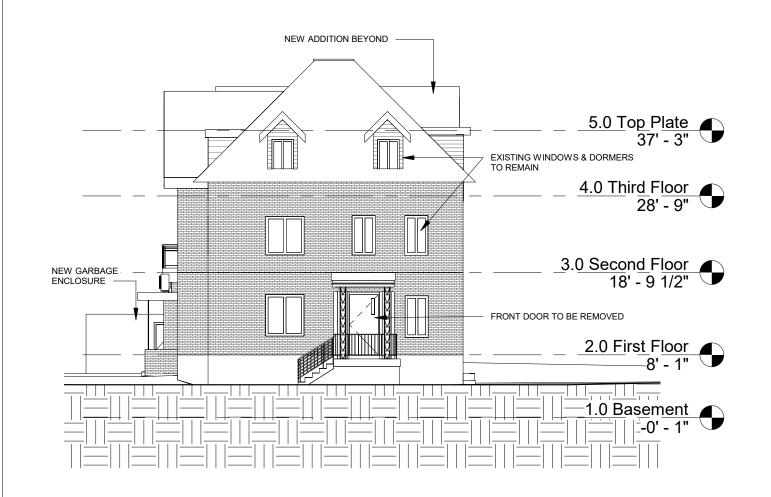
SITE PLAN APPROVAL EXISTING ELEVATION - WEST

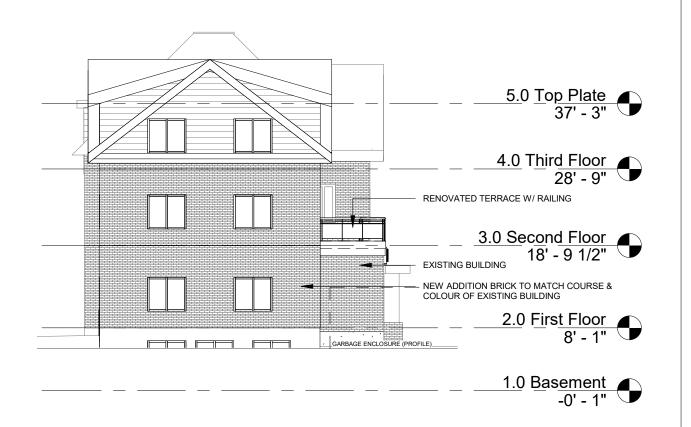
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WILBROD APARTMENTS

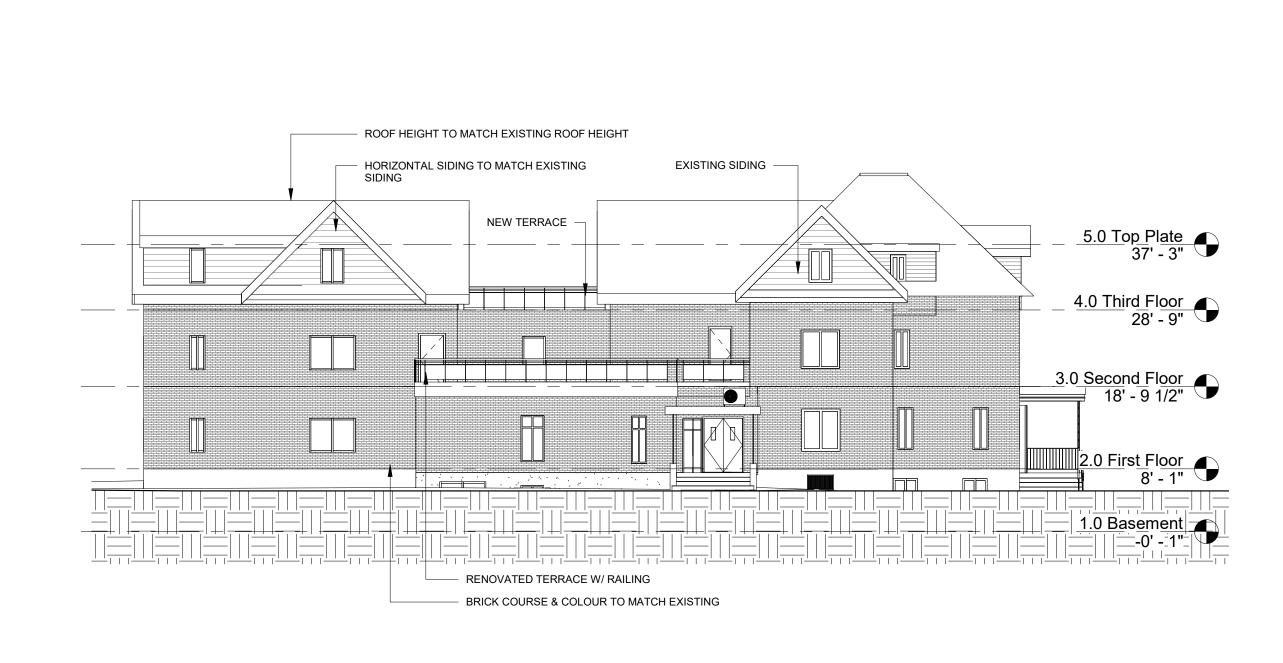
SITE PLAN APPROVAL ELEVATION - NORTH & SOUTH

SCALE: 1:150

FEBRUARY 10TH, 2022



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WILBROD APARTMENTS

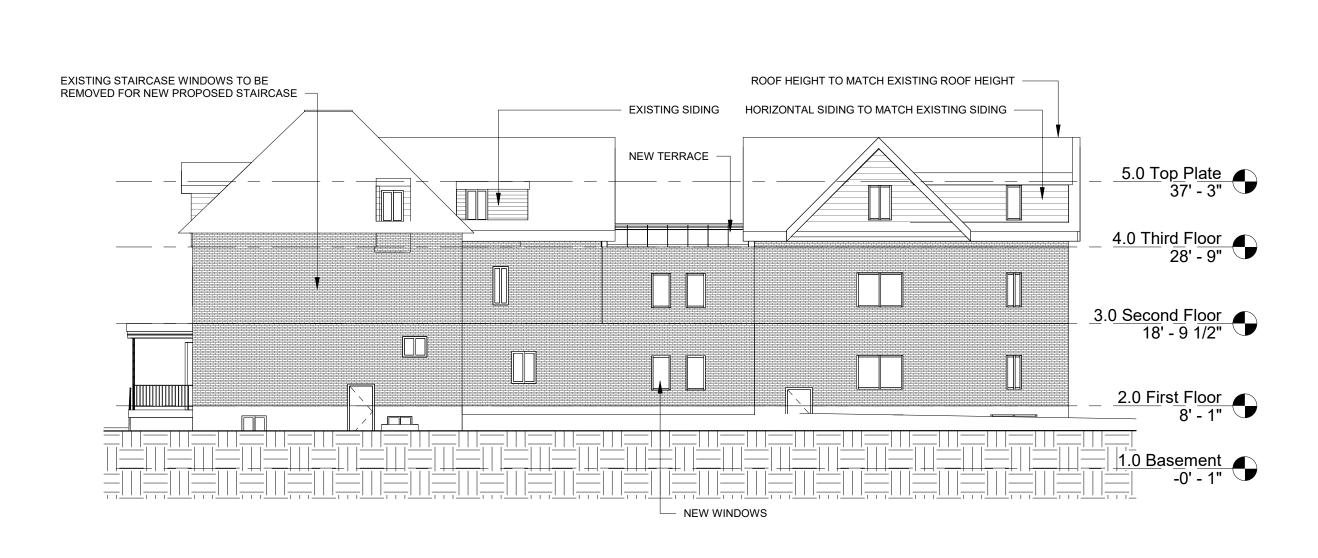
SITE PLAN APPROVAL ELEVATION - EAST

SCALE: 1:150

FEBRUARY 10TH, 2022



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WILBROD APARTMENTS

SITE PLAN APPROVAL ELEVATION - WEST

SCALE: 1:150

FEBRUARY 10TH, 2022



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SITE PLAN APPROVAL
COLOUR RENDERING - BEFORE

SCALE:

FEBRUARY 10TH, 2022



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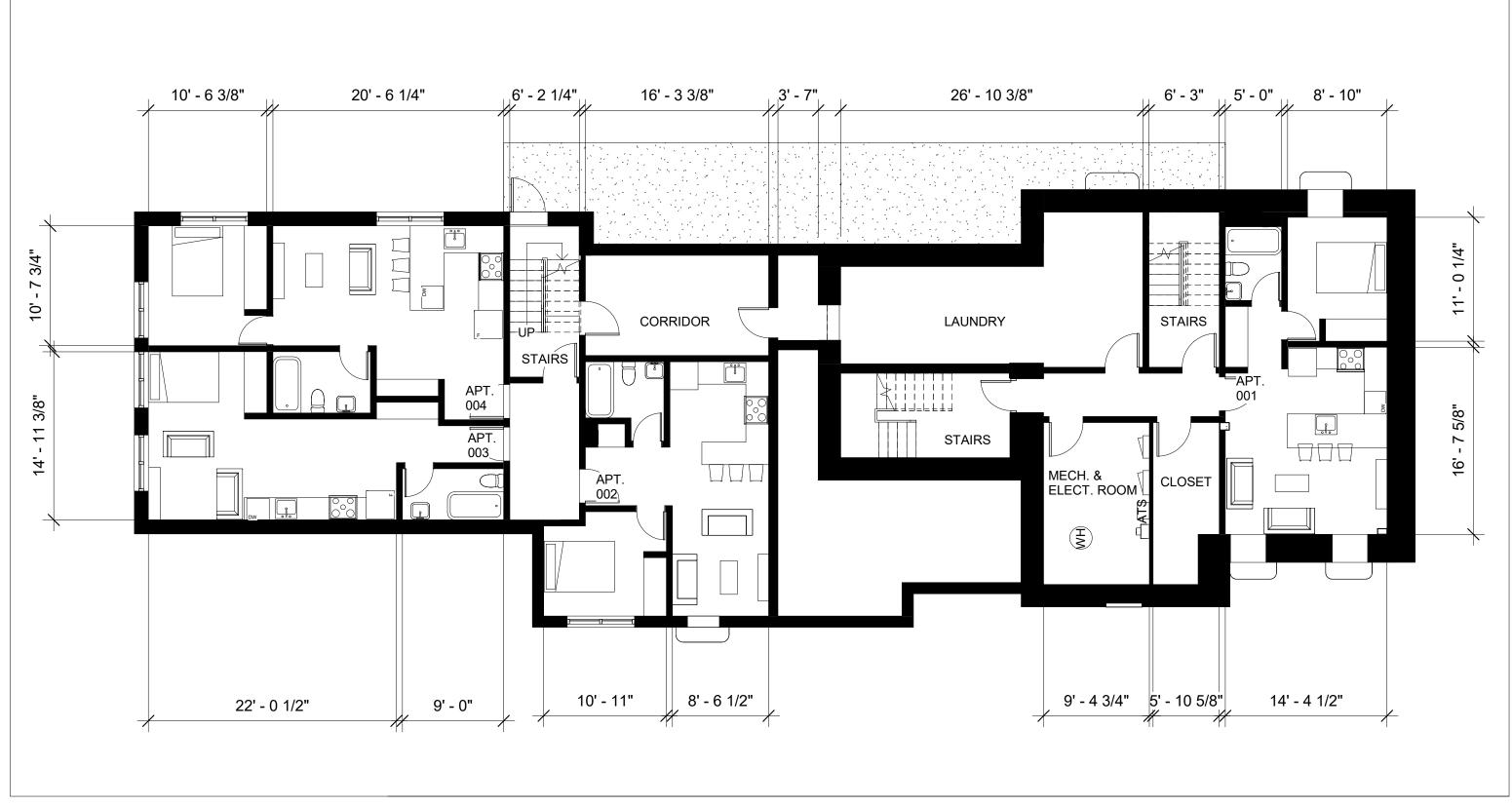
SITE PLAN APPROVAL
COLOUR RENDERINGS - AFTER

SCALE:

FEBRUARY 10TH, 2022



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SITE PLAN APPROVAL BASEMENT PLAN

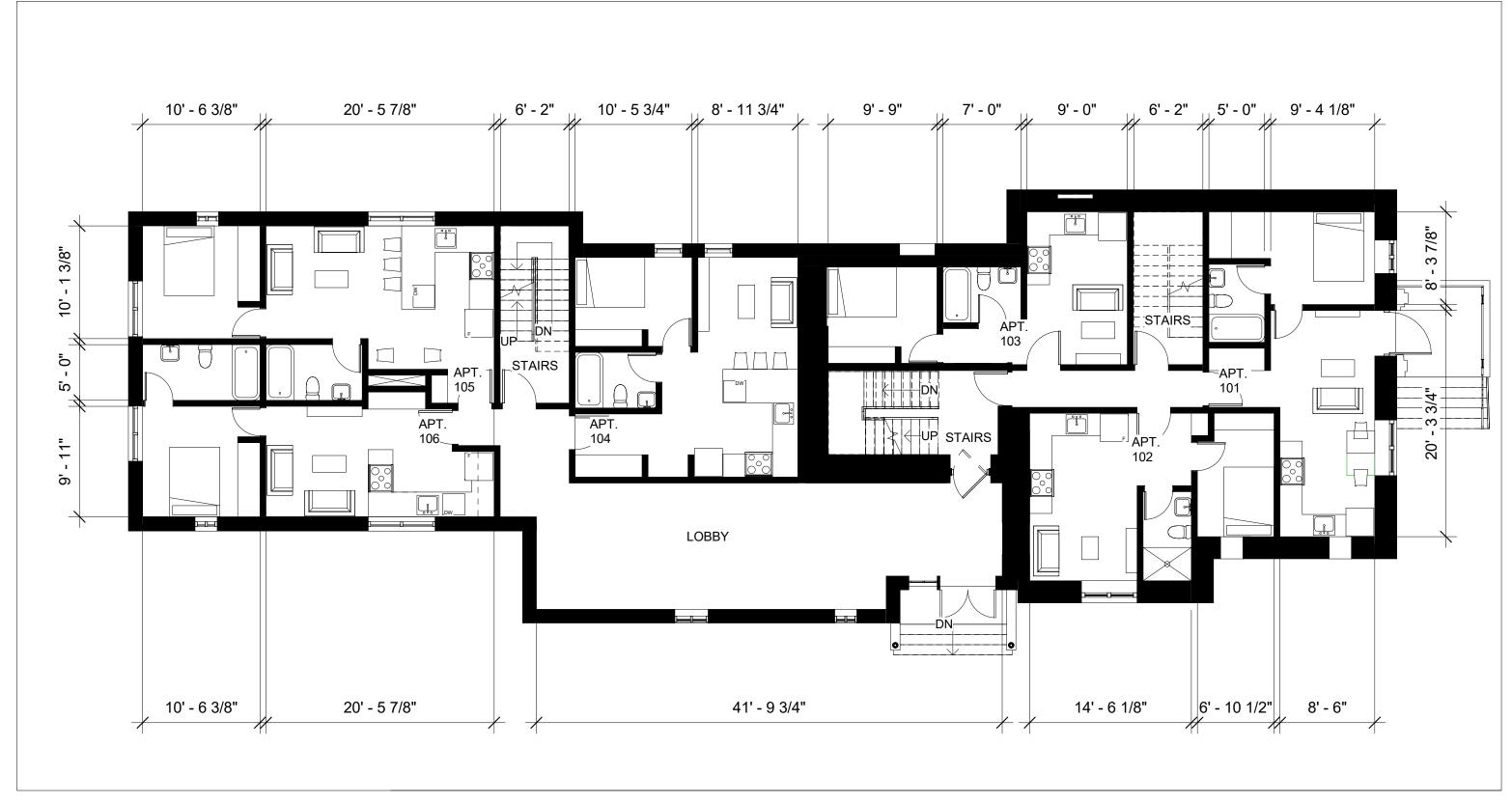
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SITE PLAN APPROVAL GROUND FLOOR PLAN

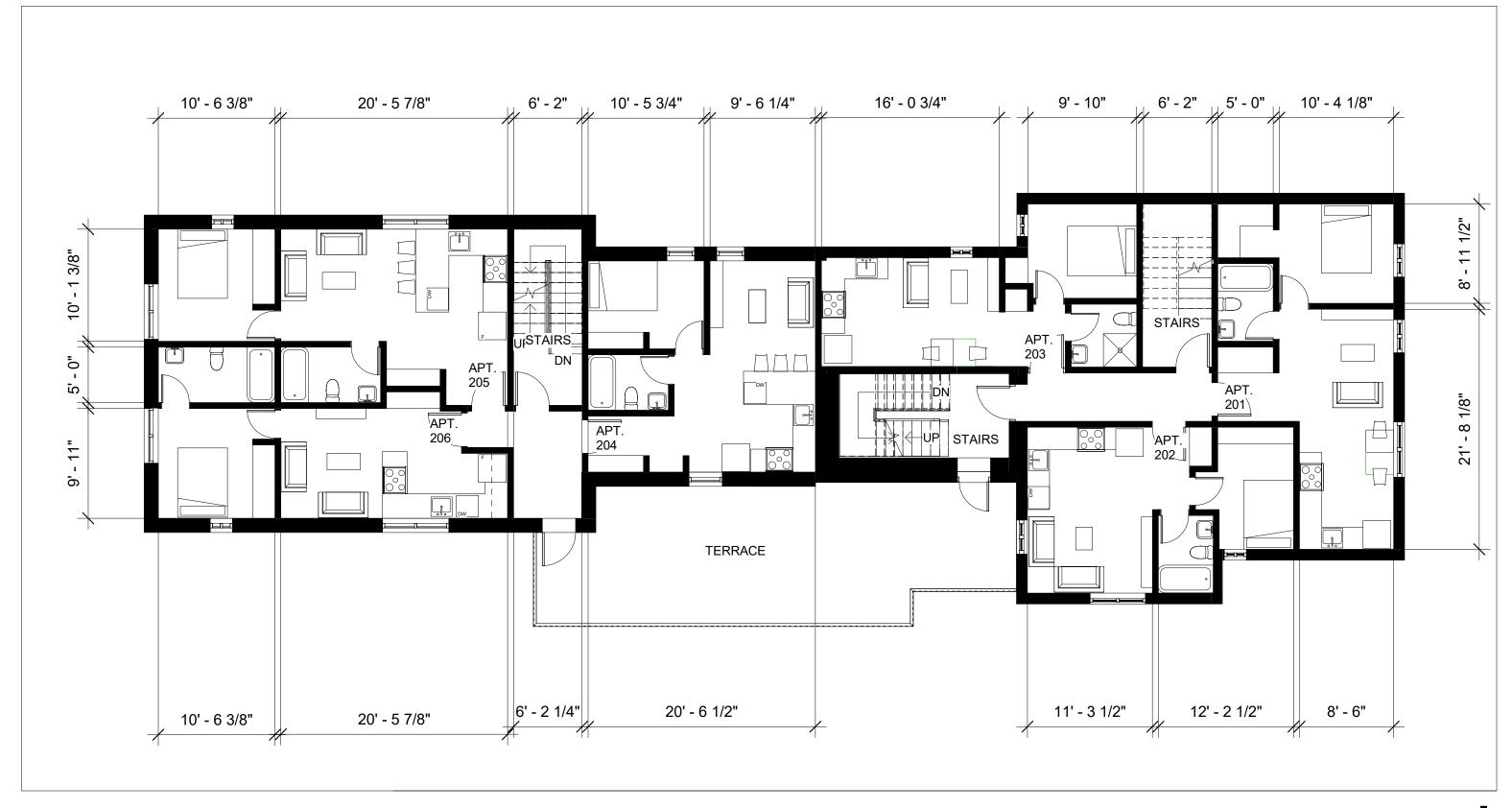
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SITE PLAN APPROVAL SECOND FLOOR PLAN

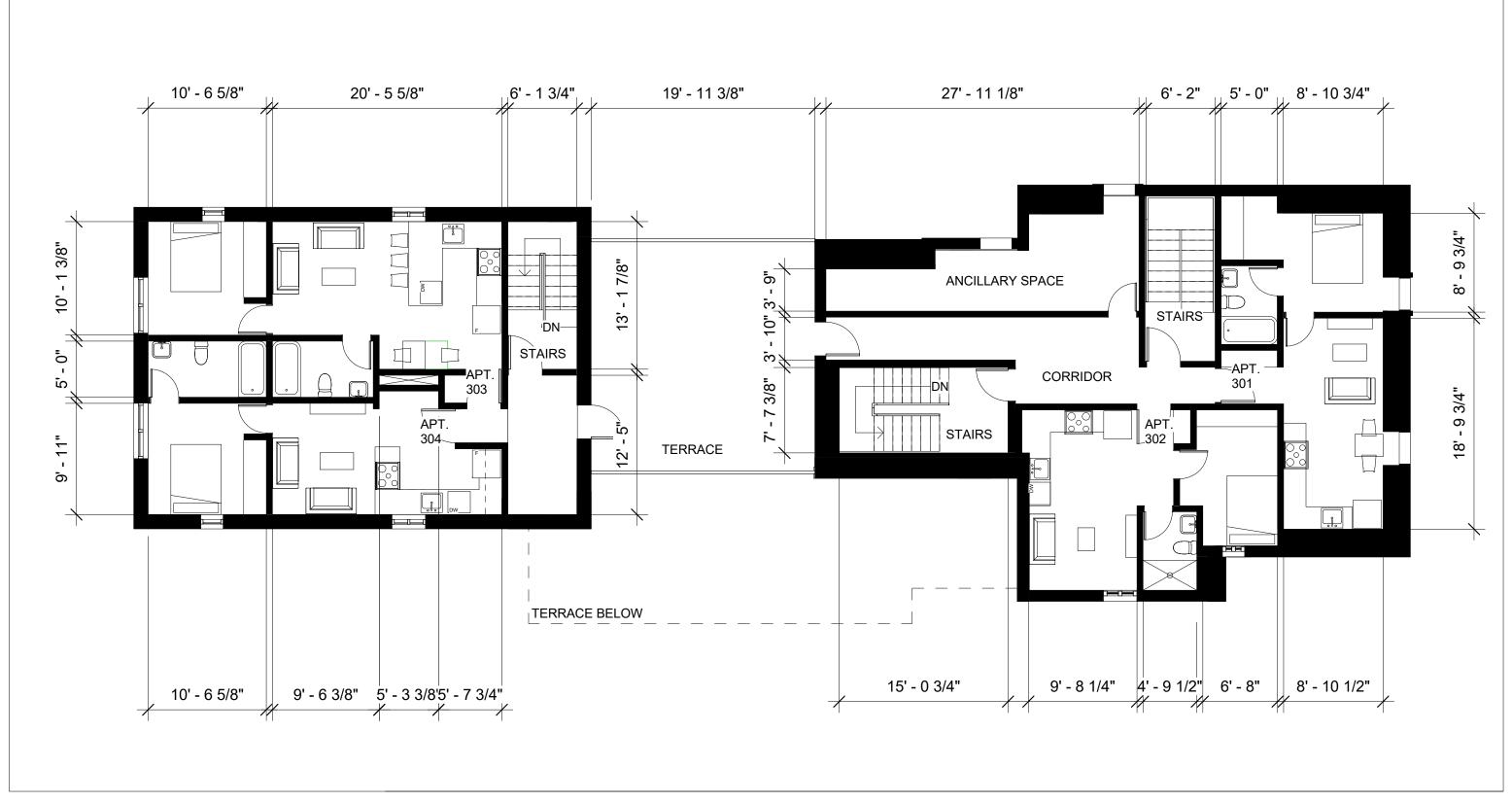
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SITE PLAN APPROVAL THIRD FLOOR PLAN

SCALE: 1:100

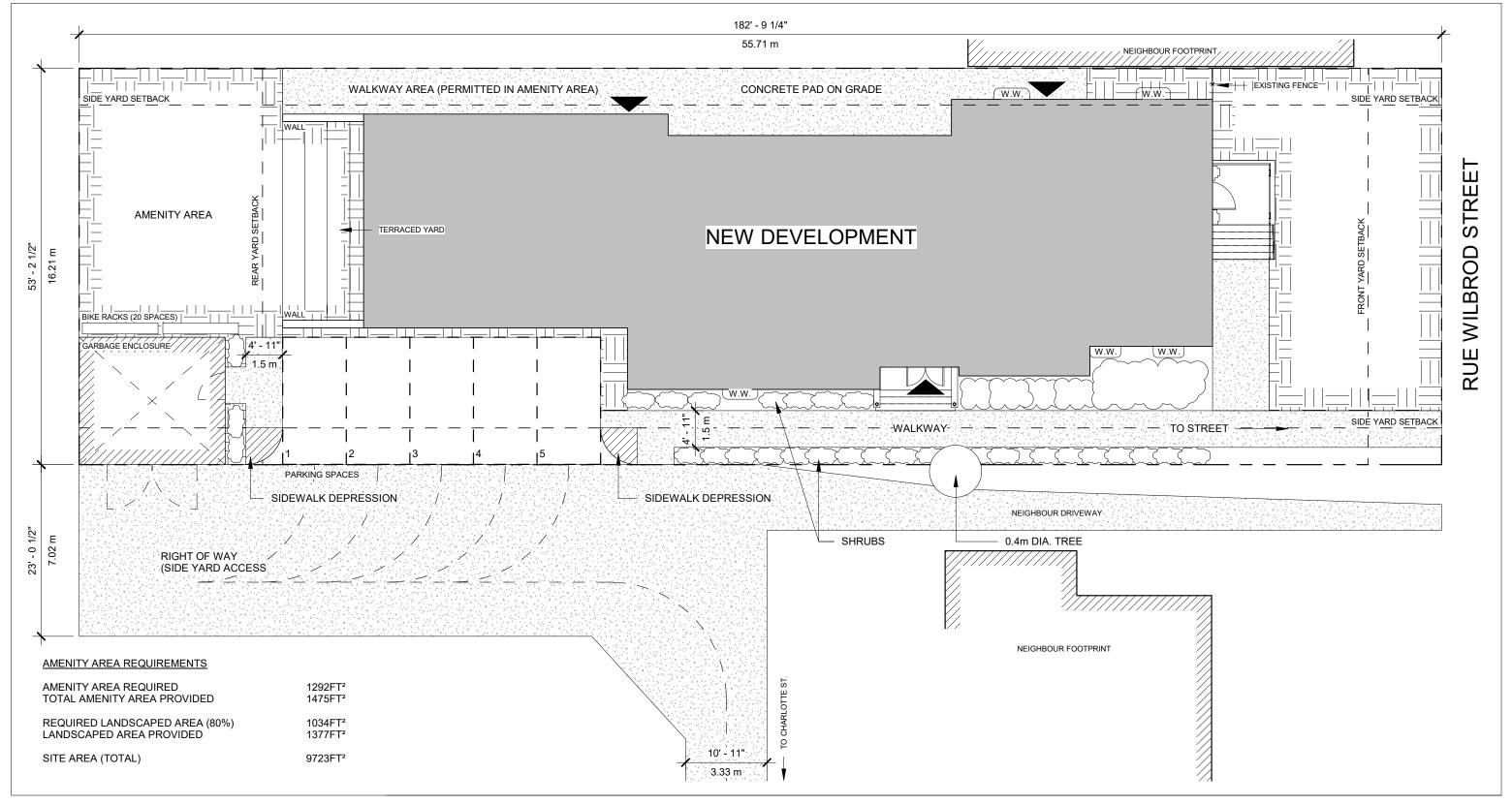
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WILBROD APARTMENTS

SITE PLAN APPROVAL LANDSCAPE PLAN

SCALE: 1:150

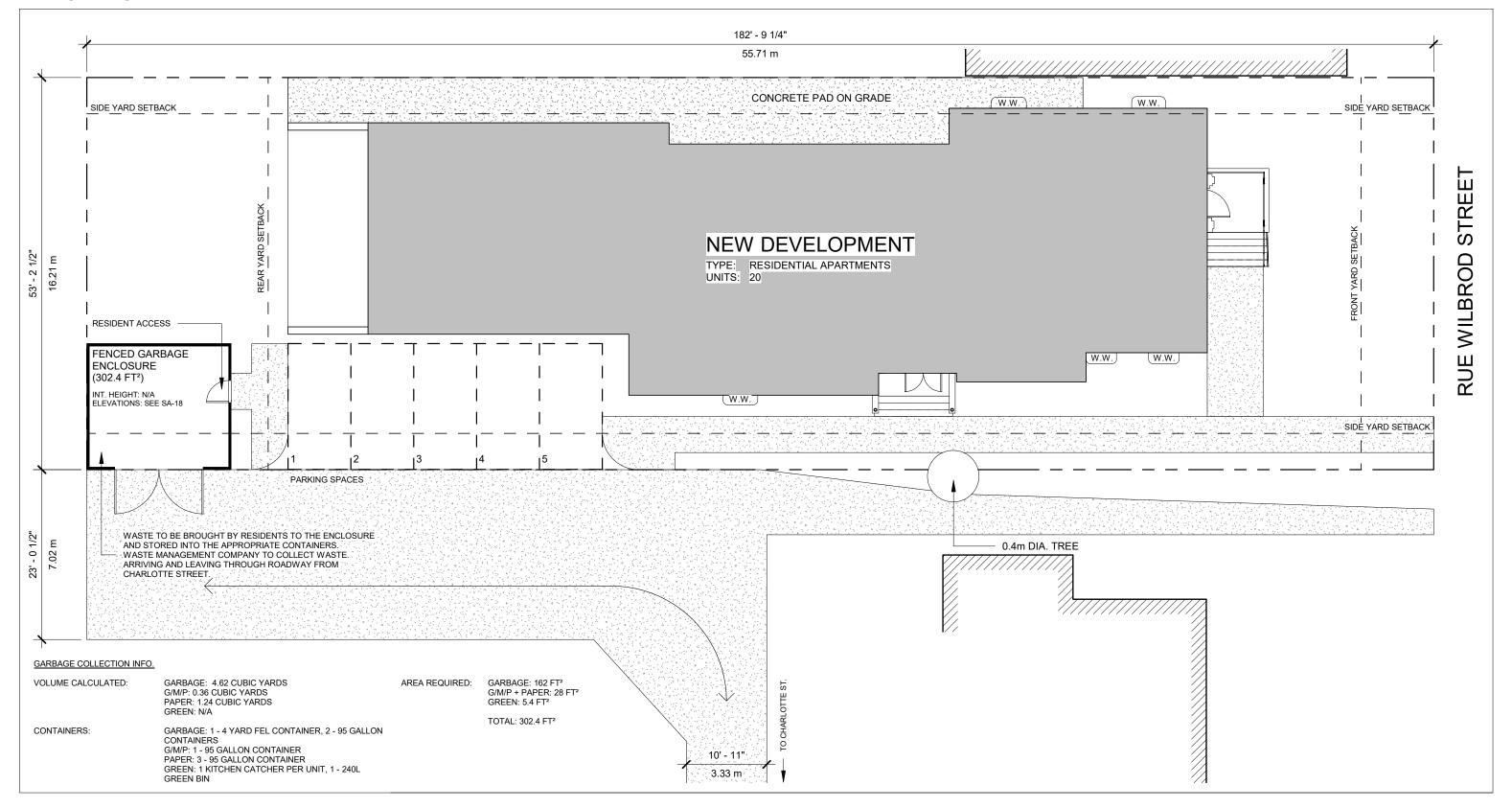
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WILBROD APARTMENTS

SITE PLAN APPROVAL GARBAGE COLLECTION PLAN

SCALE: 1:150

FEBRUARY 10TH, 2022

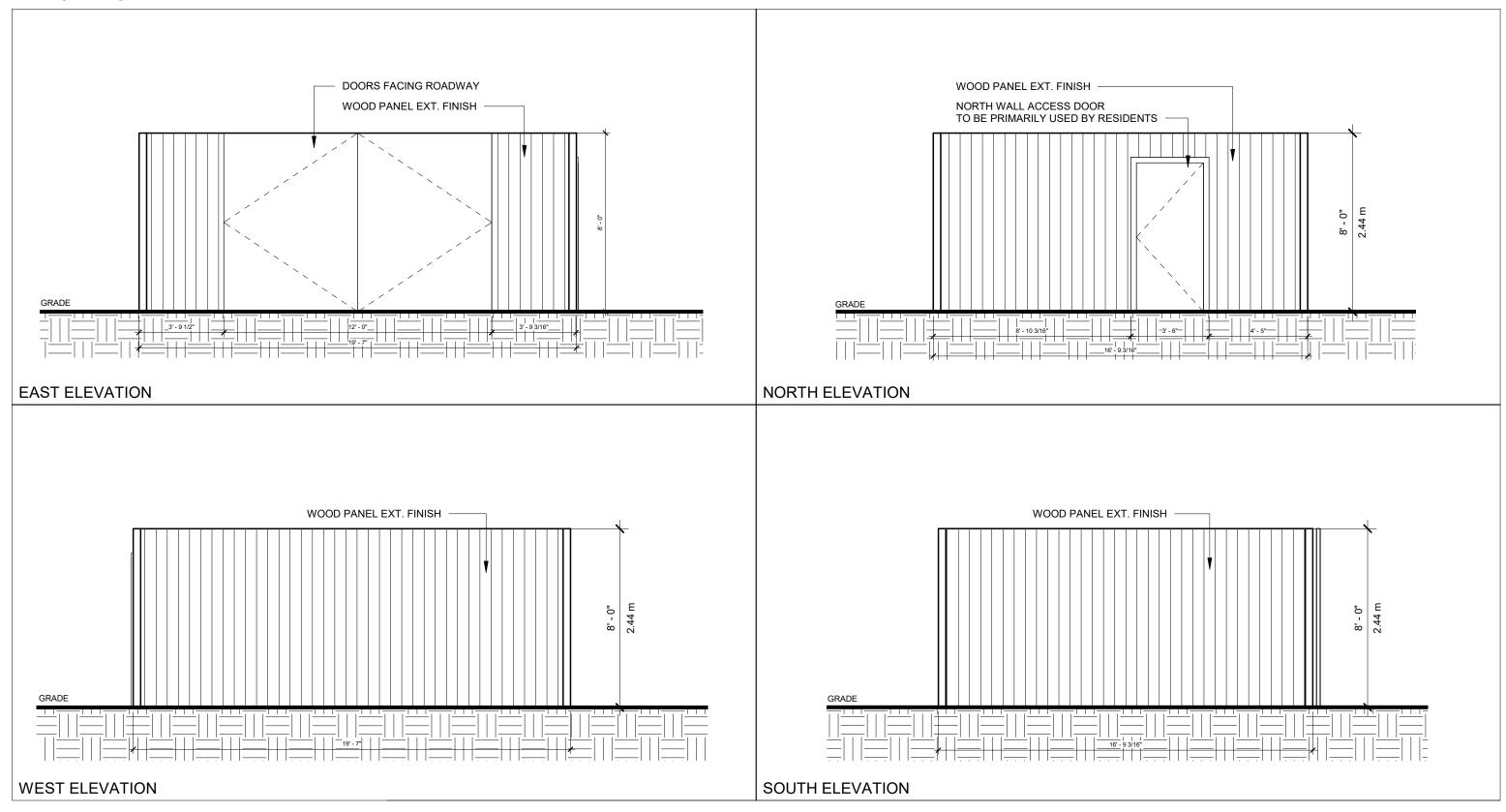


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WILBROD APARTMENTS

SITE PLAN APPROVAL
GARAGE ENCLOSURE ELEVATIONS

SCALE: 1:50

FEBRUARY 10TH, 2022



ARCA-VERDE ARCHITECTURE INC.

ZONING COMPLIANCE CHART - 476 WILBROD ADDITION

CLAUSE	NAME/ TOPIC	REQUIREMENTS	RESPONSE
137	AMENITY AREA	- 15m ² PER UNIT, UP TO 8 UNITS - PLUS 6m ² PER UNIT IN EXCESS OF 8 (REQUIREMENTS FIRST 8 UNITS) WITHIN REAR YARD AT GRADE - 80% SOFT LANDSCAPED	NO ACTION SPACE IS ACCOMMODATED FOR WITHIN REAR YARD AND ROOF TOP TERRACES
111	BIKE STORAGE	- 1 BIKE STORAGE AREA PER UNIT - LOCATED WITHIN REAR YARD OR INSIDE BUILDING	20 BIKE PARKING/ STORAGE AREAS ACCOMMODATED WITHIN REAR YARD
107	PARKING CLEARANCE	FOR PARKING ARRANGED 71-90°; MINIMUM 6.7m AISLE REQUIRED	NO ACTION EXISTING AISLE EXCEEDS MINIMUM
109 (3)	PARKING LOCATION	PARKING SPACE CANNOT BE LOCATED WITHIN FRONT YARD AND CORNER SIDE	NO ACTION PROPOSED PARKING WITHIN REAR AND SIDE YARD
65	PERMITTED PROJECTIONS	 CHIMNEY, CHIMNEY BOX, FIREPLACE EAVES, EAVES-TROPHS, GUTTERS ORNAMENTAL ELEMENTS CANOPIES AND AWNINGS FIRE ESCAPES, OPEN STAIRWAYS ETC. COVERED OR UNCOVERED BALCONY BAY WINDOWS AIR CONDITIONER CONDENSER CARPORTS (BEFORE 2008-10-08) 	1. NO CHIMNEYS, ETC. IN PROPOSAL 2. EAVES-TROPH IS >0.6m TO A LOT LINE 3. NO ORNAMENTAL ELEMENTS IN PROPOSAL 4. ALL PROJECTIONS ARE WITHIN SITE SETBACKS 5. EXTERIOR STAIRS ARE LOCATED WITHIN INTERIOR SIDE YARD, NO LIMIT 6. ROOF TERRACES DO NOT PROJECT INTO SETBACKS 7. NO BAY WINDOWS IN PROPOSAL 8. NO EXTERIOR A/C UNITS 9. TBD
60	HERITAGE OVERLAY	FOR ADDITIONS; 1. HEIGHT OF WALLS AND ROOF SLOPES MATCH 2. LOCATED ENTIRELY WITHIN REAR YARD 3. ADDITIONAL SETBACK OF 60cm FROM THE NEAREST WALL TO SIDE LOT LINES	1. ADDITION MATCHES EXISTING WALL HEIGHTS AND ROOF SLOPES 2. ADDITION IS LOCATED WITHIN REAR YARD 3. ADDITION DIMENSIONS ADJUSTED TO MEET REQUIREMENT
164	SIDE SETBACKS	SETBACK WITHIN 21m = 1.5m SETBACK PAST 21m = 6.0m	MINOR VARIANCE REQUIRED

WILBROD APARTMENTS

SITE PLAN APPROVAL ZONING CHART

SCALE:

FEBRUARY 10TH, 2022



ARCA-VERDE ARCHITECTURE INC.

Francis Valenti

From: Eric Lalande <eric.lalande@rvca.ca>

Sent: April 27, 2022 3:29 PM

To: Francis Valenti

Subject: RE: 22-3130 - 476 Wilbrod Street - Quality Control Requirement

Hi Francis,

After reviewing the Site Plan, the RVCA confirms no additional on-site water quality protection is required based on plan proposed.

Thank you,

Eric Lalande, MCIP, RPP

Planner, RVCA 613-692-3571 x1137

From: Francis Valenti < F. Valenti@McIntoshPerry.com >

Sent: Wednesday, April 27, 2022 11:22 AM **To:** Eric Lalande <eric.lalande@rvca.ca>

Subject: RE: 22-3130 - 476 Wilbrod Street - Quality Control Requirement

Hi Eric,

Please find attached the site plan for your review.

Regards,

Francis Valenti, EIT

Engineering Intern, Land Development T. 613.714.6895 | **C.** 613.808.2123

F.Valenti@McIntoshPerry.com | www.mcintoshperry.com

McINTOSH PERRY

Turning Possibilities Into Reality

From: Eric Lalande <eric.lalande@rvca.ca>

Sent: April 27, 2022 11:14 AM

To: Francis Valenti < F. Valenti@McIntoshPerry.com >

Subject: RE: 22-3130 - 476 Wilbrod Street - Quality Control Requirement

Hi Francis,

Do you have a site plan available?

Thanks,

Eric Lalande, MCIP, RPP

Planner, RVCA

613-692-3571 x1137

From: Francis Valenti < F. Valenti@McIntoshPerry.com >

Sent: Wednesday, April 27, 2022 9:39 AM **To:** Eric Lalande < <u>eric.lalande@rvca.ca</u>>

Cc: Robert Freel < r.freel@mcintoshperry.com>

Subject: 22-3130 - 476 Wilbrod Street - Quality Control Requirement

Good Morning,

We wanted to touch base with you regarding the proposed development at 476 Wilbrod Street.

The property covers approximately 0.09 ha, and currently contains a 2 ½-storey residential building with a 1-storey rear addition. The proposed development includes a 3-storey rear addition. Five asphalt parking spots are proposed, however three of the five spots will be replacing existing asphalt. Additional asphalt removals in the existing rear parking area will result in a net reduction of 65.9 m² of drive aisle/parking asphalt within the property. Surface runoff from the parking area will be collected and conveyed to a catch basin with a sump. Surface runoff from the roof and rear yard amenity area will be directed towards a grass swale to promote infiltration. Runoff collected on site will be directed to the 900mm concrete storm sewer within Wilbrod street, where, as seen in the above figure, it will travel approximately 0.27km downstream to Outlet#04451 at the Rideau River.

Due to the overall reduction in asphalt drive aisle and parking areas, it is requested that specific quality controls not be required for the development. Can you please review and confirm?

Thanks,

Francis Valenti, EIT

Engineering Intern, Land Development
T. 613.714.6895 | C. 613.808.2123
F.Valenti@McIntoshPerry.com | www.mcintoshperry.com

McINTOSH PERRY

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Platinum member

Francis Valenti

From: Norm Emond <nemond@cwwcanada.com>

Sent: August 2, 2022 11:23 AM **To:** Alison Gosling; Paul Leroux

Subject: RE: Maximum Lengths for Cleaning CB Leads

Follow Up Flag: Follow up Flag Status: Flagged

Good Morning Alison,

We can flush a CB line up to 100m in a straight line 4" and up. Pumping the CB, we can go approximately 60m. If you have any questions, please call me.

Thanks,

Norm Emond

Project Manager



Clean Water Works Inc. www.cwwcanada.com







Direct: 613-745-9536

Toll Free: 1-866-695-0155 Ext. 249

Cell.: 613-913-9412

Ottawa (Head Office)

1800, Bantree Street Ottawa (Ontario) K1B 5L6 Tel.: 613-745-2444

Fax: 613-745-9994

Montréal

2700, Sabourin Street Montréal (Québec) H4S 1M2

Tel.: 514-738-2666 Fax: 514-738-9762

From: Alison Gosling <a.gosling@mcintoshperry.com>

Sent: August 2, 2022 10:09 AM

To: Paul Leroux <paull@cwwcanada.com>; Norm Emond <nemond@cwwcanada.com>

Subject: Maximum Lengths for Cleaning CB Leads

Good morning Paul and Norm,

I hope you had a great long weekend.

We are hoping you could help us out with a question regarding catchbasin cleaning. What is the maximum a CB lead can be for CWW to be able to clean the service?

Let me know if you have any questions.

Thanks in advance,

Alison Gosling, P.Eng.

Project Engineer, Land Development

T. 613.714.4629

a.gosling@mcintoshperry.com | www.mcintoshperry.com

McINTOSH PERRY

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File Nos.: D07-12-22-0051

October 18, 2022

Colin Hopkins
Property Manager
Sleepwell Property Management
423 Bronson Avenue, Ottawa, ON K1R 6J5
colin@sleepwellmanagement.com

Subject: Site Plan Control Application

476 Wilbrod Street 2nd Review Comments

Please find below the consolidated comments from the review of the above-noted application.

Planning (Kimberley Baldwin)

- 1. In General Notes on the site plan, please add the legal description or PIN of the property, derived from the property survey.
 - a. Also add the Instrument number of the driveway easement on the plan.
- 2. Rear yard terrace The detail on the elevation plans shows the terrace as a steps behind the building, whereas the site and landscape plans show soft landscaping immediately behind the building. Please revise and align all plans.
- 3. Shed Please confirm if the shed doors are legally permitted to open onto the shared ROW.
- 4. Bicycle parking
 - a. Provide dimensions of the bicycle parking in accordance with Section 111 of the Zoning By-law
 - b. The bicycle parking and area leading to those spaces is soft landscaping. The parking and path leading to the bicycle spaces should be a hard surface, and ideally covered. Please review the layout and functionality of the bicycle parking. Revise both the site and landscape plans.
- 5. Percentage of soft landscaping in the rear yard If revisions are made to the amount of soft landscaping as per comments 2-4 above, please revise the zoning chart on the site plan to reflect the new percentages
- 6. Identify the one visitor parking space with a 'V' on the site plan



7. Elevations

- a. All elevations must be metric. Please revise.
- b. Please show the elevation plans in metric
- c. Will lighting be provided above doors? Please show on the plan
- d. Ensure the elevation plans match the information shown on the site and landscape plans (eg. rear yard terrace).

Infrastructure (Mohammed Fawzi)

Phase One Environmental Site Assessment, prepared McIntosh Perry Consulting Engineers Ltd., dated April 8, 2022

- 8. HLUI response must be provided.
- The City's Environmental Remediation Unit has indicated that completion of a Phase Two ESA is required based on the presence of a PCA. Please provide a Phase Two ESA.

Servicing & Stormwater Management Report, prepared by McIntosh Perry Consulting Engineers Ltd., dated September 12, 2022

- 10. Section 4.2 of the report must indicate the size of the existing water service to be reused and whether it is adequately sized for the development.
- 11. Section 5.1 should clearly state that the condition of the sanitary pipe has been verified from the CCTV Report and footage and based on the report the condition of the pipe is adequate for reuse. Moreover, please provide a copy of the report (can be provided as an appendix in the report) as well as the footage (can be sent to me directly).
- 12. Section 6.2 should clearly state that the condition of the foundation lateral has been verified from the CCTV Report and footage and based on the report the condition of the pipe is adequate for reuse. Please provide a copy of the report (can be provided as an appendix in the report) as well as the footage (can be sent to me directly). Please also confirm that the existing foundation drainage system is not connected to the sanitary sewer. If so, the weeping tile of the proposed addition cannot connect to the existing foundation drainage system.
- 13. After further internal review with the City's Stormwater Management Group, due to the outlet system being the Rideau River and not the Sandy Hill Combined System and given that it is a partial site redevelopment, the proposed ICD can be removed and replaced with a 15L/s ICD (likely removing the need for storage system). Please note that this exception is allowed on a case by case basis and does not set a precedent for future development applications.



Forestry Comments

- 14. A tree permit from the Planning Forester is needed prior to any tree removal associated with the development.
- 15. Diameter, health status, and ownership are required for all on site trees as well as any trees on adjoining sites that have a critical root zone extending onto the development site.
 - a. If a co-owned tree needs to be removed, permission from the adjoining landowner must be obtained prior to TCR approval and release of the tree permit.
- 16. For retained trees where there will be excavation within the critical root zone, provide:
 - b. Show limits of excavation
 - c. The tree at 488 Wilbrod may be impacted by the new sidewalk installation note that the City may request that a Registered Professional Forester (RPF) confirm the stability and survivability of any retained tree if the planning forester deems that the information supplied is not sufficient.

Contact Mark.Richardson@ottawa.ca for additional information.

4. For the next submission

- The next submission should address <u>all and each</u> of the comments or issues, to ensure the effectiveness and consistency of the next review.
- A cover letter must be included that states how each comment was addressed in the resubmission. Please co-ordinate the numbering of each resubmission comment, or issue, with the above noted comment number.
- Please include a PDF copy of revised drawings and revised reports.
- All addenda or revisions to any studies or plans must be accompanied by a PDF copy.

We are happy to meet you to discuss comments and resolve issues. Please contact me at your earliest convenience to confirm the meeting date, time, format and location.

Should there be any other questions, please do not hesitate to contact me.

Sincerely,

Kimberley Baldwin, MCIP RPP

Planner II

cc. Mohammed Fawzi, Infrastructure Approvals Project Manager Mark Richardson, Planning Forester



SLEEPWELL PROPERTY **MANAGEMENT**

SANITARY LATERAL CCTV **INSPECTIONS**

JOB LOCATION:

476 WILBROD STREET OTTAWA, ONTARIO JOB DATE:

Wednesday, August 25, 2021















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PROJECT SUMMARY

The following is the result from the camera inspections of the sanitarysewer lateral pipe located as shown in the map of this report at the premises of 476 wilbrod Street Ottawa, Ontario.

CLEANING

The lines were not cleaned prior to the CCTV condition assessment.

PLR

Each inspected pipe segment has a unique identification ID. This unique ID is the Pipe Line Reference ID or PLR. All the IDs are labeled in the map.

REPORT NAVIGATION

The PLR is used to navigate this report online. It's a link to the video inspection and video file. You can jump from the Summary to the actual inspection details by clicking the PLR link. The report also has a table of contents that are also link to each section of the report.

DVD/USB DRIVE

The DVD or USB drive contains the digital report and videos.

STRUCTURAL AND OPERATIONAL DEFECTS

The following table#1 below describes the structural and operational defects of each individual sanitary pipe segment inspected identified by its unique ID or PipeLine Reference ID. The comments depict any additional information about each segment inspected.















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Table#1 below describes the structural and operational defects of each individual sanitary pipe segment inspected identified by its unique ID or PipeLine Reference ID.

•	•		
PIPELINE REF. ID (PLR)	STRUCTURAL DEFECTS	OPERATIONAL DEFETCS	COMMENTS
SAN1	None	RF	Roots fine at the joints were ovsered in the line at the trasite pipe segment. The Inspection ended at the connection to the city main line at 18.2 meters from the top of the BWV. Note that the pipe starts at 100 mm ABS and changes to transite and pipe size to 125 mm. The changes material one more time to PVC.















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Drain-All

Insp. Date	8/25/2021	Sewer Type	SANITARY
Client	SLEEPWELL PROPERTY MANAGEMENT	Pipe Diameter	100/125 mm
Job Contact	COLIN HOPKIN	Pipe Material	ABS/Transite/PVC
Work order	17166	Camera Direction	With Flow
Upstream MH	Back Water Valve (BWV)	ID/Video name(s)	SAN1.mp4
Downstream MH	OUTSIDE CITY MAIN LINE	DVD#/USB#	1
	476 WILBROD STREET OTTAWA, ONTARIO	Job Number	1
Job Address	THE THE TENT OF TH	Operator	James Friesen
Location Details	THE POINT OF ENTRY: The Back Water Valve (BWV) is	located inside the house's b	asement.

Comments

Roots fine at the joints were ovsered in the line at the trasite pipe segment. The Inspection ended at the connection to the city main line at 18.2 meters from the top of the BWV. Note that the pipe starts at 100 mm ABS and changes to transite and pipe size to 125 mm. The changes material one more time to PVC.

0.0 Line	art of inspection e deviates down				FROM	TO	
0.0	e deviates down]			Start at access point Back Water Valve (BWV)
0.4 Wat	c deviates down			0.4			Drop into the BWV.
0.4 Wat	iter Level	0		İ			
0.5 Line	e Deviates Left			İ			
1.6 Line	e Deviates Right			İ			
9.8 Line	e deviates down			0.6			Drop into the line.
10.5 Mat	terial Change			İ			Transite
10.5 Dim	nesion of Sewer Changes		125	İ			
12.3 Roc	ots Fine			Ì	11	4	At joint
13.0 Roc	ots Fine				9	4	At joint
13.2 Mat	terial Change			İ			PVC
14.0 Line	e Deviates Right			j			
14.5 Line	e Deviates Left			j			
15.6 Line	e Deviates Right						















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Drain-A

Insp. Date	8/25/2021				Sewer T	ype	SANITARY
Client	SLEEPWELL PRO	PERTY MANA	GEMENT		Pipe Dia	ameter	100/125 mm
Job Contact	COLIN HOPKIN			Pipe Ma	iterial	ABS/Transite/PVC	
Work order	17166	Ī			Camera	Direction	With Flow
Upstream MH	Back Water Valve (Back Water Valve (BWV)				o name(s)	SAN1.mp4
Downstream MH	OUTSIDE CITY MAI				DVD#/U	SB#	1
	476 WILBROD STREET OTTAWA, ONTARIO				Job Nu	mber	1
Job Address	476 WILBROD STR	EET OTTAWA, O	NTARIO		Operato	or	James Friesen
Location Details	THE POINT OF ENT	RY:The Back Wa	ater Valve (E	BWV) is loc	ated inside	e the house's b	pasement.
Comments	connection to the o	ity main line at	18.2 meters	from the t	top of the I	BWV. Note tha	nspection ended at the it the pipe starts at 100 one more time to PVC.
DISTANCE CODI	E DESCRIPTION	% SIZE (mm)	LENGTH (m)	CLOCK FROM	CLOCK TO	REMARKS	
18.2 Finish S	urvey					End at OUTS	IDE CITY MAIN LINE.















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To the table of content



PROJECT NAME

476 WILBROD STREET- SANITARY SEWER LATERAL CCTV INSPECTION

VIDEO NAME SAN1

Figure#1: Start of inspection at access point Back Water Valve (BWV)

Figure#2: A view to the 0% water level and left deviation at 0.5 meters from the top of Back Water Valve (BWV).

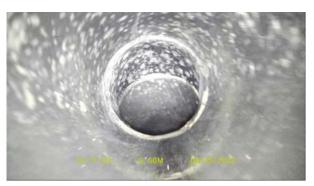




Figure#3: A view to the right line deviation at 1.4 meters from the top of Back Water Valve (BWV).

Figure#4: A view to the line deviation downwards at 9.6 meters from the top of Back Water Valve (BWV).



















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To the table of content



PROJECT NAME VIDEO NAME

476 WILBROD STREET- SANITARY SEWER LATERAL CCTV INSPECTION

SAN1

Figure#5: A view to the materia and diameter change to Transite and 125 mm respectivelly at 10.5 meters from the top of Back Water Valve (BWV).

Figure#6: A view to the roots fine from 11 to 4 o'clock at 11.9 meters from the top of Back Water Valve (BWV).





Figure#7: A view to the roots fine from 9 to 4 o'clock at 13 meters from the top of Back Water Valve (BWV).

Figure#8: A view to the material change to PVC at 13.2 meters from the top of Back Water Valve (BWV).



















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PROJECT NAME

VIDEO NAME

476 WILBROD STREET- SANITARY SEWER LATERAL CCTV INSPECTION

SAN1

Figure#9: A view to the right line deviation at 13.6 meters from the top of Back Water Valve (BWV).

Figure#10: A view to the left line deviation at 14.5 meters from the top of Back Water Valve (BWV).





Figure#11: A view to the right line deviation at 15.8 meters from the top of Back Water Valve (BWV).

Figure#12: A view to the end point OUTSIDE CITY MAIN LINE at 18.1 meters from the top of Back Water Valve (BWV).



















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							WORK ORD	ER #:	
	Drain-	All SEW				SHEET	•		
			5	KETCH NOT TO S	CALE		TARGET: □ Yes	□ No	
	WELL MANAGEN	MENT	□ PSLL		e (mm/dd/yyyy	-			
Municip			□ CONFLICT	Contractor Assigned: DRAIN-ALL					
Address		□ Yes □ □		Operator Name: Contractor Tel: 613-739-1070					
	ed With:	1165	NO	Contractor Email: locates@drainall.com					
Method		out 🗆 Outside	Cleanout	Status: Complete Partial Basement: Yes No					
	□ Lateral from	Main □ Toilet			Incomplete	□ Septic	□ Unknowr	1	
Remain	ing Lateral Distance	e:		Full Latera	l Distance:				
Depth at Building Depth at Property	Line: N			476		W.BI	All measurements must b in metric (m) to one dec		
Depth at				2.3m-	N.BL	2.5m	<i>u</i> – V		
		<u> </u>	PLR: SAN1	2.1m-	>				
Depth at Sewer M	lain: —S.AE	WILBROD ST		2.0m 1.6	>\	-SANM	.0m ↑ 16.0m		
Additio	nal Remarks:								
	BUILDING LINE	-BL-	GAS MAIN		-GM-	TRANSFORM	ER		
LEGEND	FENCE LINE	-FL-	GAS SERVICE		-GS-	WELL		⊗	
垣	FACE OF CURB	-FC-	SANITARY MAIN	VI	-SANM-	POLE	1	 	
<u>8</u>	ASPHALT EDGE RETAINING WALL	-AE- -RW-	SANITARY LATERA STORM MAIN	AL.	-SANL- -STRM-	CATCH BASIN MANHOLE	I	СВ	
	DRIVEWAY	-DW-	STORM LATERAL		-STRIVI-	HYDRANT		MH	
	SIDEWALK	-SW-	NORTH, SOUTH, E	AST, WEST	-N-S-E-W-	VALVE		×	

WITHIN 1M (AS MEASURED HORIZONTALLY FROM THE FIELD MARKINGS) HAND DIG TO AVOID DAMAGING THE UNDERGROUND PLANT. IF YOU DAMAGE THE PLANT YOU MAY BE HELD LIABLE. CONTACT THE UTILITY OWNER IMMEDIATELY. DEPTH VARIES AND MUST BE VERIFIED BY HAND DIGGING OR VACUUM EXCAVATION. MEASUREMENTS ARE TAKEN USING WHEEL, UNLESS OTHERWISE STATED. ALL UNDERGROUND PLANT OUTSIDE LOCATED AREA ARE SHOWN FOR REFERENCE ONLY. IF PAINT MARKS ARE NOT VISIBLE, PLEASE CALL FOR REMARK.



Observations Problem

Structural Condition - Brickwork

DB – Displaced Bricks

DI – Dropped Invert

MB - Missing Bricks

Structural Condition – Mortar Missing

MM - Mortar missing medium

MS - Mortar missing slight

MT – Mortar missing total

Structural Condition – Surface damage

SSL - Spalling large

SSM - Spalling medium

SSS - Spalling slight

SWL – Wear large

SWM – Wear medium

SWS - Wear slight

Structural Condition – Joint Displaced

JDL – Joint Displaced Large

JDM – Joint Displaced Medium

Structural Condition - Cracked

CC – Crack Circumferential

CL - Crack Longitudinal

CM – **Crack multiple**

Structural Condition - Open Joint

OJL - Open Joint Large

OJM – Open Joint Medium

 $\underline{Structural\ Condition-Fractured}$

FC – Fracture Circumferential

FL – Fracture Longitudinal

FM – Fracture Multiple

<u>Structural Condition – Broken</u>

(pipe sewers)

B-Broken

H - Hole

Structural Condition – Deformed

D – **Deformed Sewer**

Structural Condition – Collapsed

X – Collapsed

Service Defects – Roots

RF – Roots fine

RFJ – Roots fine at joint

RM - Roots mass

RMJ – roots mass at joint

RT – Roots tap

RJ – Roots tap at joint

<u>Service Defects – Line</u>

LD - Line deviates down

LL - Line deviates left

LR – Line deviates right

LU – Line deviates up

Service Defects – Infiltration

ID – Infiltration dripper

IDJ – Infiltration dripper at joint

IG – Infiltration gusher

IGJ – Infiltration gusher at joint

IR – Infiltration runner

IRJ – Infiltration runner at joint

IS – Infiltration seeper

ISJ – Infiltration seeper at joint

Service Defects – Encrustation

EH – Encrustation heavy

EHJ - Encrustation heavy at joint

EL – Encrustation light

ELJ – Encrustation light at joint

EM – Encrustation medium

EMJ – Encrustation medium at ioint

Service Defects – Debris

DE - Debris

DEG – **Debris** grease

DES – Debris silt

Service Defects – Obstruction

OB - Debris grease



















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Info

Construction Features – Connection

CN – Connection

CNI - Connection intruding

CX - Connection defective

CXI – Connection defective intruding

<u>Construction Features – Junction</u>

JN - Junction

JX – Junction Defective

Construction Features - Lining defect

LN – Lining defect

<u>Construction Features – Major branch</u>

BR – Branch major

Construction Features – Manhole/node

MH – Manhole/Node

Miscellaneous Features

CU – Camera underwater DC – Dimension of sewers changes

GO – General Observation

GP - General Photograph

LC - Lining Change

MC – Material change

PC – Pipe length change

SC – Shape change

V – Vermin (rats and/or mice)

Other Codes

Inspections

CID – Continue inspection

downstream

CIU – Continue inspection

upstream

FH – Finish Survey

SA – Survey abandoned

ST – Start of Survey

Reasons & Purpose

A- Structural or service condition

defects

B-Infiltration

C- Assessment of complete remedial or renovation works

D- Pre-adoption

E- Pre-acceptance

F- Sample survey to determine

asset condition

G- Associated with future capital

scheme including drainage area

planning

H- Resurvey for any reason

X- Other

Z- Not known

Surface Type & Location

A- Main road (urban)

B- Main road (suburban/rural)

C- Light road

D- Footpath or verge (within

highway boundary)

E- Fields (farmland and public

open space)

F- Gardens (within private

property)

G-Woodland

X- Difficult access (motorway, railway, watercourse, inside building)

Weather

1- Dry

2- Heavy Rain

3- Light Rain

4- Showers

5- Snow





















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Pipe Type

AC – Asbestos Cement

AC - Alkathene

AK - Alkathene

BR - Brick

CC - Concrete box culvert

CI - Cast Iron

CO - Concrete

CSB – Concrete segments (bolted)

CSU - Concrete segments

(unbolted)

DI - Ductile Iron

GRC – Glass reinforced cement

GRP – Glass reinforced plastic

MAC - Masonry (in regular

courses)

MAR - Masonry (randomly

coursed)

PE - Polyethylene

PF - Pitch fibre

PP - Polypropylene

PSC – Plastic/steel composite

PVC - Polyvinyl chloride

RPM - Reinforced plastic matrix

SI – Spun (grey) iron

ST - Steel

VC – Vitrified clay

XXX - Other

ZZZ – Not known

Pipe Shape

A- Arched (with flat bottom)

B-Barrel

C- Circular

E- Egg shaped

H- Horseshoe

O- Oval

R- Rectangular

S- Square

T- Trapezoidal

U- U-shaped with flat top

X- Other

Use of Sewer

D- Combined

F- Foul

S- Surface water

T- Trade effluent

W- Watercourse (culverted)

X- Other

Z- Not known

Lining Method

BL – Bitumen

CL - Cement

CPP – Cured in place

IS – Soft inversion type liner

PL – Plastic

RL - Resin

XXX - Other

ZZ - Not known

Pre-Cleaning

N- No pre-cleaning

Y- Pre-cleaning was carried out

Z-Not known

Continuous defects

Sn (where n=1,2,3,...etc) Start of

continuous defect

Fn (where n=1,2,3,...etc) End of

continuous defect















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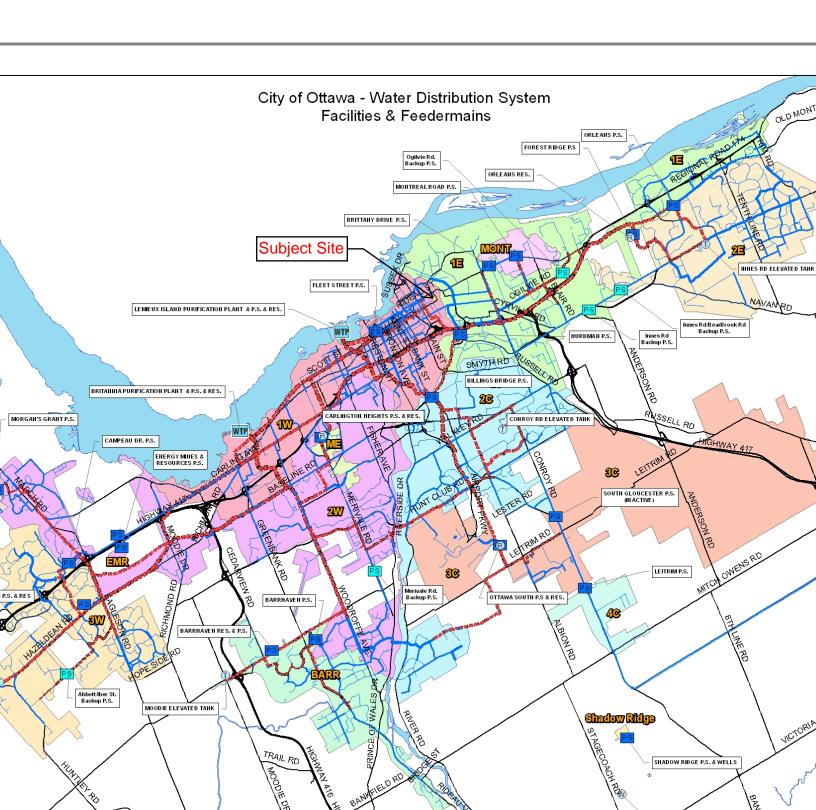


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								WORK ORDI	ER #:	
	D	rain-	All SEW	ER LATE	ERAL LO		SHEET	WO3008	9	
				3	KEICH NOT TO 30	ALL		TARGET: ✓ Yes	□ No	
SLEEP	WELL	MANAGEN	ЛENT	✓ PSLL	Locate Dat	e (mm/dd/yyyy): 08/25/2022			
Municip	-				Contractor	Assigned: DF	RAIN-ALL			
Address	s: 476 W	/ILBROD ST.			Operator N	lame: JAMES	S FRIESEN			
Branch			⊐ Yes 🗷 N	0	Contractor Tel: 613-739-1070					
Branche					Contractor Email: locates@drainall.com					
Method		nside Clean		Cleanout	Status: ☑ (□ Partial	Basement: ✓ Yes		
		ateral from				ncomplete	□ Septic	□ Unknown		
Remain	ing Late	ral Distanc	e: 0.0m		Full Latera	Distance: 16.	.5m	All magazinem anta milat ha	المعادة المعادة	
Depth at Building 2.65n	Line:	⊗ N			476	ē	W.BL	All measurements must be in metric (m) to one deci		
Depth at Property	*									
3.23n						N.BL		,		
Depth at Road Edg	ge:				2.3m		2.5m			
2.25m	n :					SANL				
Depth at Sewer M 2.78m	lain:				2.1m -		9.0m			
	-	-S.AE	 		2.0m					
			WILBROD ST.		1.6	m > \	-SANM	0m <u>-</u> ↑ 16.0m		
Additio	nal Rem	arks:								
	BUILDI	NG LINE	-BL- (GAS MAIN		-GM-	TRANSFORME	ΕR		
LEGEND	FENCE			GAS SERVICE		-GS-	WELL		⊗	
	FACE O			SANITARY MAIN		-SANM-	POLE		ŏ	
Ģ		T EDGE		SANITARY LATERA	AL	-SANL-	CATCH BASIN		СВ	
"		ING WALL		STORM MAIN		-STRM-	MANHOLE		MH	
	DRIVEV SIDEWA			STORM LATERAL NORTH, SOUTH, E	EAST, WEST	-STRL- -N-S-E-W-	HYDRANT VALVE		XXIIO &	

APPENDIX C WATERMAIN CALCULATIONS

476 Wilbrod Pressure Map Figure



CCO-22-3130 - 476 Wilbrod - Water Demands

 Project:
 476 Wilbrod

 Project No.:
 CCO-22-3130

 Designed By:
 FV

 Checked By:
 AG

 Date:
 August 23, 2022

 Site Area:
 0.09 gross ha

Residential NUMBER OF UNITS UNIT RATE

1 Bedroom Apartment 20 units 1.4 persons/unit

Total Population 28 persons

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	
Residential	280	L/c/d	
Industrial - Light	35,000	L/gross ha/d	
Industrial - Heavy	55,000	L/gross ha/d	
Shopping Centres	2,500	L/(1000m² /d	
Hospital	900	L/(bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/(Space/d)	
Motels	150	L/(bed-space/d)	
Hotels	225	L/(bed-space/d)	
Tourist Commercial	28,000	L/gross ha/d	
Other Commercial	28,000	L/gross ha/d	
	Residential	0.09	L/s
AVERAGE DAILY DEMAND	Commercial/Industrial/		
	Institutional	0.00	L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	A	MOUNT	UNITS
Residential	9.5	x avg. day	L/c/d
Industrial	1.5	x avg. day	L/gross ha/d
Commercial	1.5	x avg. day	L/gross ha/d
Institutional	1.5	x avg. day	L/gross ha/d
	Residential	0.86	L/s
MAXIMUM DAILY DEMAND	Commercial/Industrial/		
	Institutional	0.00	L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	A	MOUNT	UNITS
Residential	14.3	x avg. day	L/c/d
Industrial	1.8	x max. day	L/gross ha/d
Commercial	1.8	x max. day	L/gross ha/d
Institutional	1.8	x max. day	L/gross ha/d
	Residential		L/s
MAXIMUM HOUR DEMAND	Commercial/Industrial/		
	Institutional	0.00	L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT

CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.09	L/s
MAXIMUM DAILY DEMAND	0.86	L/s
MAXIMUM HOUR DEMAND	1.30	L/s

CCO-22-3130 - 476 Wilbrod - OBC Fire Calculations

476 Wilbrod Project: Project No.: CCO-22-3130 Designed By: FV Checked By: AG Date: August 23, 2022

Ontario 2006 Building Code Compendium (Div. B - Part 3)

Water Supply for Fire-Fighting - Apartment Building

Building is classified as Group: **C - Residential Occupancies** (from table 3.2.2.55)

Building is of noncombustible construction or of heavy timber construction conforming to Article 3.1.4.6. Floor assemblies are fire separations but with no fire-resistance rating. Roof assemblies, mezzanines, loadbearing walls, columns and arches do not have a

From Div. B A-3.2.5.7. of the Ontario Building Code - 3. Building On-Site Water Supply:

(a) Q = K x V x Stot

Q = minimum supply of water in litres

K = water supply coefficient from Table 1

V = total building volume in cubic metres

Stot = total of spatial coefficient values from the property line exposures on all sides as obtained from the formula:

Stot = 1.0 + [Sside1+Sside2+Sside3+...etc.]

К	16	(from Table 1 pg A-31) (Worst case occupancy {E / F2} 'K' value used)		
V	2,916	(Total building volume in m³.)		
Stot	2.0	(From figure 1 pg A-32)		
Q =	93,325.12	L		

From Table 2: Required Minimum Water Supply Flow Rate (L/s)

if Q < 108,000 L 2700 L/min 713 gpm

9.24 m

3.07 m

Snorth

Seast

Ssouth

Swest

From Figure 1 (A-32) 0.1

0.5

0.0

CCO-22-3130 - 476 Wilbrod - Fire Underwriters Survey

 Project:
 476 Wilbrod

 Project No.:
 CCO-22-3130

 Designed By:
 FV

 Checked By:
 AG

 Date:
 August 23, 2022

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.: City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

 $F = 220 \times C \times VA$ Where: F = Required fire flow in liters per minute

 $\boldsymbol{\mathsf{C}}$ = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in

the building being considered.

Construction Type Wood Frame

C 1.5 A 978.2 m²

Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area) 978.2 m²

Calculated Fire Flow 10,321.2 L/min 10,000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:

Limited Combustible -15%

Fire Flow 8,500.0 L/min

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Non-Sprinklered 0%

Reduction 0.0 L/min

D. INCREASE FOR EXPOSURE (No Rounding)

	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor		
Exposure 1	Over 30 m	Wood frame	N/A	N/A	N/A	0%	
Exposure 2	3.1 to 10	Wood frame	17	1	17.0	15%	
Exposure 3	10.1 to 20	Wood frame	6.5	1	6.5	10%	
Exposure 4	0 to 3	Fire Resistive - Non Combustible (Unprotected Openings)	15.9	9	143.1	15%	
					% Increase*	40%	

Increase* 3,400.0 L/min

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow	11,900.0 L/min
Fire Flow Required**	12,000.0 L/min

^{*}In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

^{**}In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

CCO-22-3130 - 476 Wilbrod - Boundary Condition Unit Conversion

 Project:
 476 Wilbrod

 Project No.:
 CCO-22-3130

 Designed By:
 FV

 Checked By:
 AG

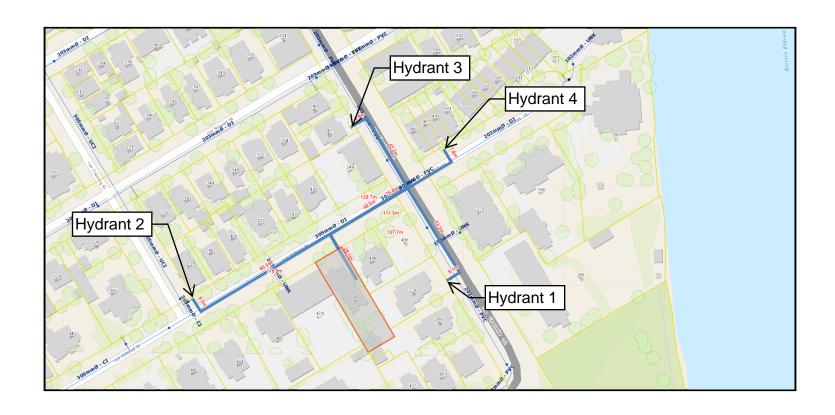
 Date:
 August 23, 2022

Boundary Conditions Unit Conversion

WILBROD STREET

Scenario	Height (m)	Elevation (m)	m H₂O	PSI	kPa
Avg. DD	115.4	68.2	47.2	67.2	463.0
Fire Flow (45 L/s or 2,700 L/min)	109.1	68.2	40.9	58.2	401.2
Fire Flow (200 L/s or 12,000 L/min)	106.4	68.2	38.2	54.4	374.7
Peak Hour	106.1	68.2	37.9	53.9	371.8

476 Wilbrod Hydrant Coverage Figure



APPENDIX D SANITARY CALCULATIONS

CCO-22-3130 - 476 Wilbrod - Sanitary Demands

476 Wilbrod Project: Project No.: CCO-22-3130 Designed By: FV Checked By: RF Date: Aug-22

Site Area 0.09

Gross ha 1.40 Persons per unit 1 Bedrom Apartment 20 **Total Population** 28 Persons **Amenity Space** 137.00 m²

DESIGN PARAMETERS

Institutional/Commercial Peaking 1.5

Residential Peaking Factor 3.69 * Using Harmon Formula = 1+(14/(4+P^0.5))*0.8

where P = population in thousands, Harmon's Correction Factor = 0.8

Mannings coefficient (n) 0.013

Demand (per capita) 280 L/day Infiltration allowance 0.33 L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.00
Wet	0.03
Total	0.03

AVERAGE DAILY DEMAND

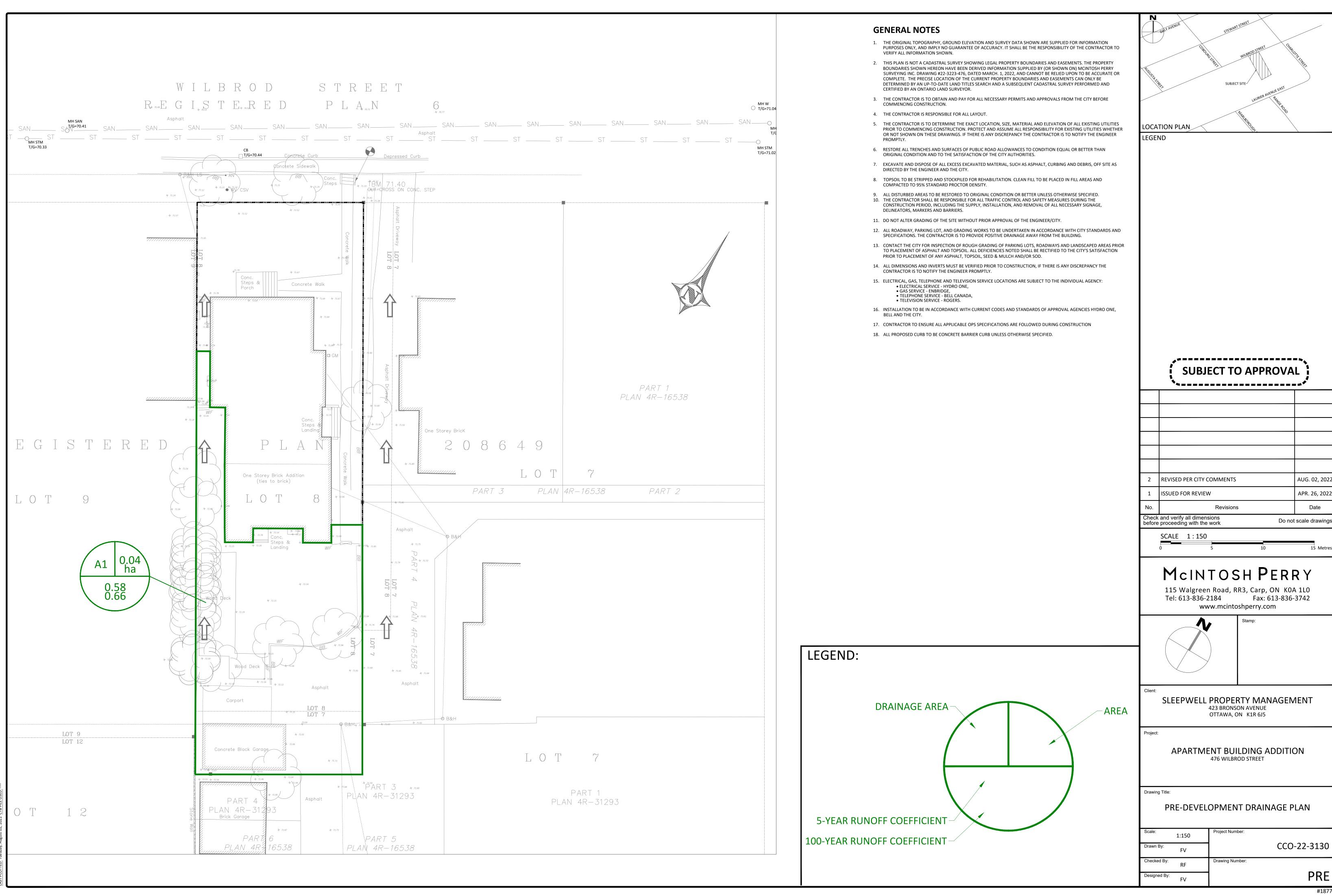
DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	28	0.09
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m² /d)	137.00	0.004
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.09	L/s	
PEAK RESIDENTIAL FLOW	0.33	L/s	
AVERAGE ICI FLOW	0.00	L/s	
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.01	L/s	
PEAK INDUSTRIAL FLOW	0.00	L/s	
TOTAL PEAK ICI FLOW	0.01	L/s	

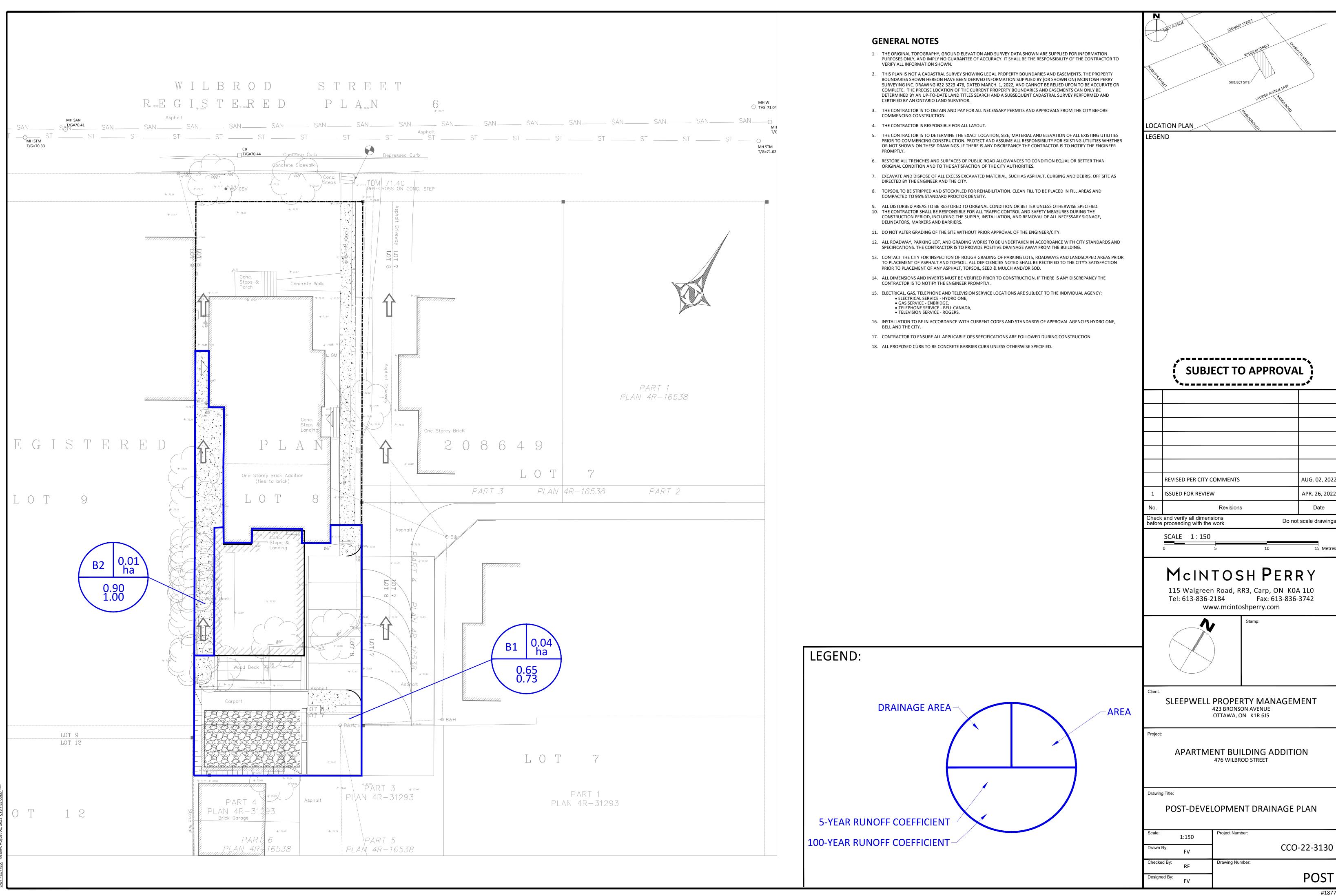
TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.10	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	0.35	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	0.37	L/s

APPENDIX E PRE-DEVELOPMENT DRAINAGE PLAN



APPENDIX F POST-DEVELOPMENT DRAINAGE PLAN



APPENDIX G STORMWATER MANAGEMENT CALCULATIONS

CCO-22-3130 - 476 Wilbrod

1 of 4

Tc (min)	Intensity (mm/hr)				
	5-Year	100-Year			
20	70.3	120.0			
10	104.2	178.6			

C-Values						
Impervious	0.90					
Gravel	0.60					
Pervious	0.20					

Pre-Development Runoff Coefficient

Drainage	Impervious	Gravel	Pervious Area	Average C	Average C
Area	Area (m²)	(m²)	(m²)	(5-year)	(100-year)
A1	231	0	193.73	0.58	0.66

Pre-Development Runoff Calculations

Drainage	Area	С	С	Tc	Q (L/s)	
A1	0.0424	0.58	0.66	10	7.14	13.85
Total	0.0424				7.14	13.85

Post-Development Runoff Coefficient

Drainage Area	Impervious Area (m²)	Gravel (m²)	Pervious Area (m²)	Average C (5-year)	Average C (100-year)
B1	230	0	131.07	0.65	0.73
B2	64	0	0	0.90	1.00

Post-Development Runoff Calculations

Drainage Area		C		Tc	Q		
Area	(ha)	5-Year	100-Year	(min)	5-Year	100-Year	
B1	0.0361	0.65	0.73	10	6.75	13.02	7
B2	0.0064	0.90	1.00	10	1.66	3.16	Unrestric
Total	0.0424				8.41	16.19	

cted

Required Restricted Flow for Area B1

Drainage	Area	С	Tc	Q (L/s)
Area	(ha)	5-Year	(min)	5-Year
A1	0.0424	0.50	10	15.00

Post-Development Restricted Runoff Calculations

Drainage Area	Unrestricted Flow (L/S)		Restricted Flow (L/S)		Storage Re	equired (m³)	Storage Pro	ovided (m³)
Alea	5-year	100-Year	5-Year	100-Year	5-Year	100-Year	5-Year	100-Year
B1	6.75	13.02	6.75	13.02				
B2	1.66	3.16	1.66	3.16				
Total	8.41	16.19	8.41	16.19	0.00	0.00	0.00	0.00

CCO-22-3130 - 476 Wilbrod

Storage Requirements for Area B1	2 of 4

		_			_		
5-1	Yea	r Si	t∩r	m	Fv	(en	t

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
10	104.2	6.75	15.00	-8.25	0.00

Maximum Storage Required 5-year = 0 m³

100-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
10	178.6	13.03	15.00	-1.97	0.00

Maximum Storage Required 100-year = 0 m³

100-Year + 20% Climate Change Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
10	214.3	15.63	15.00	0.63	0.38
20	144.0	10.50	15.00	-4.50	0.00

Maximum Storage Required = 0.4 m³

5-Year Storm Event Storage Summary

Location	T/G	INV. (out)	Storage Available (m³) =	0.0
CB1	71.66	69.91	Storage Required (m³) =	0.0

100-Year Storm Event Storage Summary

Location	T/G	INV. (out)	Storage Available (m³) = 0.0
CB1	71.66	69.91	Storage Required (m³) = 0.0

100-Year + 20% Climate Change Event

Location	T/G	INIV (out)	Depth (m)	Head (m)	Water Elev. (m)
		iivv. (Out)	. ,	rieau (III)	` '
CB1	71.66	69.91	1.74	1.74	71.69

Storage Available (m³) =	0.1
Storage Required (m ³) =	0.4

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For Orifice Flow, C= 3 of 4 0.60 For Weir Flow, C= 1.84

Orifice 1 Orifice 2 Weir 1 Weir 2 69.91 invert elevation Χ Χ center of crest elevation 69.95 Χ orifice width / weir length 74 mm Χ Χ weir height orifice area (m²) 0.004 Χ Χ

Flevation Discharge Table - Storm Pouting

			Elevati	on Discharge 1	Гable - Storm	Routing				
Elevation	Orif	fice 1	Ori	fice 2		ir 1	We	eir 2	Total	
Elevation	H [m]	Q [m ³ /s]	H [m]	Q [m ³ /s]	H [m]	Q [m ³ /s]	H [m]	$Q[m^3/s]$	Q [L/s]	Ì
71.40	1.45	0.014	Х	Х			Х	Х	14	Ī
71.41	1.46	0.01	Х	Х			Х	Х	14	Ī
71.42	1.47	0.01	Х	Х			Х	Х	14	Ī
71.43	1.48	0.01	Х	Х			Х	Х	14	Ī
71.44	1.49	0.01	Х	Х			Х	Х	14	Ī
71.45	1.50	0.01	Х	Х			Х	Х	14	
71.46	1.51	0.01	Х	Х			Х	Х	14	
71.47	1.52	0.01	Х	Х			Х	Х	14	
71.48	1.53	0.01	Х	Х			Х	Х	14	
71.49	1.54	0.01	Х	Х			Х	Х	14	
71.50	1.55	0.01	Х	Х			Х	Х	14	
71.51	1.56	0.01	Х	Х			Х	Х	14	
71.52	1.57	0.01	Х	Х			Х	Х	14	
71.53	1.58	0.01	Х	Х			Х	Х	14	
71.54	1.59	0.01	Х	Х			Х	Х	14	
71.55	1.60	0.01	Х	Х			Х	Х	14.39	
71.56	1.61	0.01	Х	Х			Х	Х	14.43	
71.57	1.62	0.01	Х	Х			Х	Х	14.48	
71.58	1.63	0.01	Х	Х			Х	Х	14.52	
71.59	1.64	0.01	Х	Х			Х	Х	14.56	
71.60	1.65	0.01	Х	Х			Х	Х	14.61	
71.61	1.66	0.01	Х	Х			Х	Х	14.65	
71.62	1.67	0.01	Х	Х			Х	Х	14.70	
71.63	1.68	0.01	Х	Х			Х	Х	14.74	
71.64	1.69	0.01	Х	Х			Х	Х	14.78	
71.65	1.70	0.01	Х	Х			Х	Х	14.83	
71.66	1.71	0.01	Х	Х			Х	Х	14.87	
71.67	1.72	0.01	Х	Х			Х	Х	14.92	
71.68	1.73	0.01	Х	Х			Х	Х	14.96	
71.69	1.74	0.02	Х	Х			Х	Х	15.00	100Y+20%
71.70	1.75	0.02	Х	Х			Х	Х	15.04	
71.71	1.76	0.02	Х	Х			Х	Х	15.09]
71.72	1.77	0.02	Х	Х			Х	Х	15.13	
71.73	1.78	0.02	Х	Х			Х	Х	15.17	
71.74	1.79	0.02	Х	Х			Х	Х	15.22]
71.75	1.80	0.02	Х	Х			Х	Х	15.26	
71.76	1.81	0.02	Х	Х			Х	Х	15	
71.77	1.82	0.02	Х	Х			Х	Х	15	

- Notes: 1. For Orifice Flow, User is to Input an Elevation Higher than Crown of Orifice.
 - 2. Orifice Equation: $Q = cA(2gh)^{1/2}$
 - 3. Weir Equation: Q = CLH^{3/2}
 - 4. These Computations Do Not Account for Submergence Effects Within the Pond Riser.
 - 5. H for orifice equations is depth of water above the centroide of the orifice.
 - 6. H for weir equations is depth of water above the weir crest.

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Time of Concentration Pre-Development

Drainage Area	Sheet Flow	Slope of	Tc (min)	Tc (min)
ID	Distance (m)	Land (%)	(5-Year)	(100-Year)
A1	58	1.79	7	5

Therefore, a Tc of 10 can be used

Tc= (3.26(1.1-c)L^0.5/S^0.33)

c = Balanced Runoff Coefficient L = Length of drainage area S = Average slope of watershed APPENDIX H
CITY OF OTTAWA DESIGN CHECKLIST

City of Ottawa

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

Criteria	Location (if applicable)
Executive Summary (for larger reports only).	N/A
Date and revision number of the report.	On Cover
 Location map and plan showing municipal address, boundary, and layout of proposed development. 	Appendix A
$\ \square$ Plan showing the site and location of all existing services.	Site Servicing Plan (C102)
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual.	1.1 Purpose1.2 Site Description
watershed plans that provide context to which individual developments must adhere.	·
	6.0 Stormwater Management
 Summary of pre-consultation meetings with City and other approval agencies. 	Appendix B
 Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, 	1.1 Purpose
Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and	1.2 Site Description
develop a defendable design criteria.	6.0 Stormwater Management
\square Statement of objectives and servicing criteria.	3.0 Pre-Consultation Summary



☐ Identification of existing and proposed infrastructure available in the immediate area.	N/A
☐ Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Site Grading Plan (C101)
☐ Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Site Grading Plan (C101)
 Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts. 	N/A
☐ Proposed phasing of the development, if applicable.	N/A
Reference to geotechnical studies and recommendations concerning servicing.	Section 2.0 Background Studies, Standards and References
 All preliminary and formal site plan submissions should have the following information: Metric scale North arrow (including construction North) Key plan Name and contact information of applicant and property owner Property limits including bearings and dimensions Existing and proposed structures and parking areas Easements, road widening and rights-of-way Adjacent street names 	Site Grading Plan (C101)

4.2 Development Servicing Report: Water

Criteria	Location (if applicable)
☐ Confirm consistency with Master Servicing Study, if available	N/A
 Availability of public infrastructure to service proposed development 	N/A
☐ Identification of system constraints	N/A
☐ Identify boundary conditions	Appendix C
☐ Confirmation of adequate domestic supply and pressure	N/A
 Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. 	Appendix C
 Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves. 	N/A
 Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design 	N/A
☐ Address reliability requirements such as appropriate location of shut-off valves	N/A
☐ Check on the necessity of a pressure zone boundary modification.	N/A
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Appendix C, Section 4.2

 Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions. 	Site Servicing Plan (C101)
 Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation. 	N/A
☐ Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Appendix C
 Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference. 	N/A

4.3 Development Servicing Report: Wastewater

Criteria	Location (if applicable)
☐ Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	N/A
Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
☐ Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 5.2 Proposed Sanitary Sewer

☐ Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Section 5.3 Proposed Sanitary Design
☐ Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	N/A
 Description of proposed sewer network including sewers, pumping stations, and forcemains. 	Section 5.2 Proposed Sanitary Sewer
☐ Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
 Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development. 	N/A
☐ Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
☐ Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
☐ Special considerations such as contamination, corrosive environment etc.	N/A

4.4 Development Servicing Report: Stormwater Checklist

Criteria	Location (if applicable)
 Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property) 	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
☐ Analysis of available capacity in existing public infrastructure.	N/A
 A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern. 	Pre & Post-Development Plans
☐ Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5-year event (dependent on the receiving sewer design) to 100-year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
☐ Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Set-back from private sewage disposal systems.	N/A
☐ Watercourse and hazard lands setbacks.	N/A
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
☐ Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
☐ Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5-year return period) and major events (1:100-year return period).	Appendix G

☐ Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Site Grading Plan
Calculate pre-and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 7.0 Proposed Stormwater Management Appendix G
Any proposed diversion of drainage catchment areas from one outlet to another.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
 Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities. 	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
☐ If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/A
☐ Identification of potential impacts to receiving watercourses	N/A
Identification of municipal drains and related approval requirements.	N/A
 Descriptions of how the conveyance and storage capacity will be achieved for the development. 	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
100-year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Grading Plan (C101)
☐ Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A

 Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors. 	Section 8.0 Sediment & Erosion Control
☐ Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
 Identification of fill constraints related to floodplain and geotechnical investigation. 	N/A

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

Criteria	Location (if applicable)
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/A
 Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. 	N/A
☐ Changes to Municipal Drains.	N/A
☐ Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A

4.6 Conclusion Checklist

Criteria	Location (if applicable)
☐ Clearly stated conclusions and recommendations	Section 9.0 Summary
	Section 10.0 Recommendations
☐ Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	All are stamped
☐ All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	All are stamped