OLENDER VETERINARIAN CLINIC 4 CAMPBELL REID COURT OTTAWA. ONTARIO

TRAFFIC ASSESSMENT	
August 12, 2022	
Prepared for:	
Dr. Olender	
735 Traffic Assessment_2.doc	

D. J. Halpenny & Associates Ltd.

CONSULTING TRANSPORTATION ENGINEERS
P.O. Box 774, Manotick, ON K4M 1A7 - Tel (613) 692-8662 - David@DJHalpenny.com

OLENDER VETERINARIAN CLINIC 4 CAMPBELL REID COURT OTTAWA, ONTARIO

TRAFFIC ASSESSMENT

1. INTRODUCTION

The Olender Veterinarian Clinic proposes a new clinic located at the northeast corner of the intersection of March Road and Dunrobin Road. The property has a municipal address of 4 Campbell Reid Court and is approximately 7,917 m² in size. The property is zoned "RU" (Rural Countryside Zone) which will support the development. The site is currently occupied by one single-family home which has been demolished due to fire damage, and replaced by another single-family home with access onto Campbell Reid Court. The lot has a 15.7 m frontage onto Campbell Reid Court which is sufficient for the existing single-family home. The clinic will be located behind the house, but the lack of frontage means that Campbell Reid Court cannot accommodate both the clinic access and single-family home. It is proposed that the clinic utilize the existing access to Dunrobin Road which would provide a more visible access and eliminate patrons of the clinic from traveling along a local street (Campbell Reid Court).

This Traffic Assessment study will examine the possibility of providing a site access onto Dunrobin Road. The analysis will further determine the geometry of the access and turning movements, and the impact the access may have on the operation of the adjacent roads and intersections.

2. ROADS AND INTERSECTIONS

CAMPBELL REID COURT

Campbell Reid Court is a two lane local street with a rural cross section, gravel shoulders and no sidewalks. The street is 375 m in length which connects to Cameron Harvey Drive at the north end and a cul-de-sac at the south. The intersection of Campbell Reid/Cameron Harvey is located 20 m from Dunrobin Road (centre to centre). The speed limit is unposted along Campbell Reid Court.

DUNROBIN ROAD

Dunrobin Road (Ottawa Road 9) is a two lane arterial road with 2 m paved shoulders. Dunrobin Road is identified in the Transportation Master Plan (TMP) as a Spine Route in the Cycling Network - Primary Rural. The speed limit is posted at 60 km./h. past the site, and increases to 80 km./h. approximately 1.8 km north of the site.

MARCH ROAD

March Road (Ottawa Road 49) is identified in the TMP as a two lane arterial road and a Spine Route in the Cycling Network. The roadway has gravel shoulders which change to paved shoulders in the vicinity of the Dunrobin/March intersection. The posted speed limit is 80 km./h.

DUNROBIN/MARCH INTERSECTION

The intersection of Dunrobin Road and March Road is a "T" intersection where March Road forms the eastbound and westbound approaches, and Dunrobin Road the southbound approach. The southbound Dunrobin Road approach has an exclusive left turn lane with 70 m of vehicular storage and a 90 m taper. The intersection is controlled by three phase traffic signals. The intersection has the following lane configuration:

Southbound Dunrobin Approach One exclusive left turn lane (70 m storage)

One shared left/right turn lane

Eastbound March Approach One exclusive left turn lane (100 m storage)

One through lane

Westbound March Approach One through lane

One exclusive right turn lane (100 m storage)

Below is an aerial photograph of the Dunrobin/March intersection.

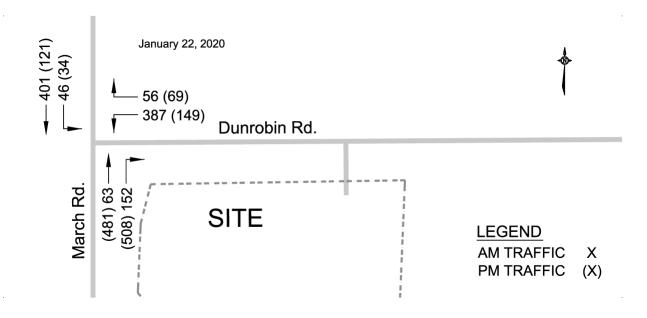
DUNROBIN ROAD AND MARCH ROAD INTERSECTION



Traffic counts taken on January 22, 2020 were obtained from the City of Ottawa. The peak AM hour count is provided in the Appendix as Exhibit 1 and the peak PM hour

count as Exhibit 2. Figure 1 shows the 2020 peak AM and PM hour traffic at the Dunrobin/March intersection taken pre-COVID-19.

FIGURE 1 - 2020 PEAK AM AND PM HOUR TRAFFIC COUNTS



FUTURE ROAD PROJECTS

The TMP has identified the widening of March Road from a two lane road to a four lane road under the "Road Network - 2031 Network Concept". The future road widening would provide additional vehicle capacity to growth areas in north Kanata.

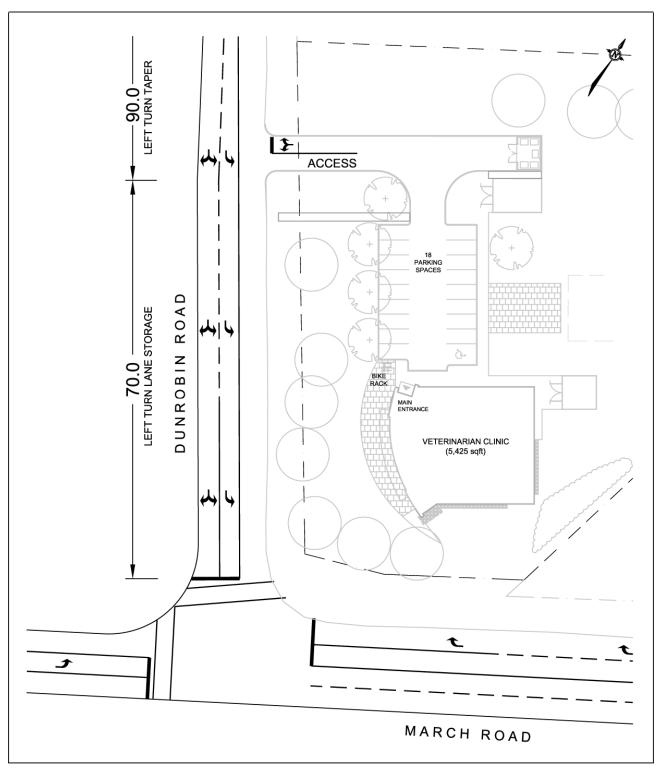
3. OLENDER VETERINARY CLINIC

The proposed Olender clinic will be a one storey building located at the northeast corner of the intersection of Dunrobin Road and March Road. The clinic will be replacing the existing clinic at 591 March Road in Kanata.

The clinic is proposed to have a gross floor area of approximately 5,425 ft² (504.0 m²) and one access point onto Dunrobin Road. The site access would be located at approximately the same location as the existing access at a distance of 90 m from the Dunrobin/March intersection (centreline to centreline). The site will provide 18 parking spaces including one barrier free space in the parking lot for use by staff and patrons.

The site will have one access point onto Dunrobin Road. The access for the clinic will be 6.0 m in width with one lane entering and one lane exiting. The exiting lane will have a shared left/right turning movement onto Dunrobin Road. The access will be located 70 m north of the stop bar at the southbound approach to the Dunrobin/March intersection. Figure 2 shows a site plan and lane arrangement for the site.

FIGURE 2 - CONCEPTUAL SITE PLAN



3.1 Site Generated Trips

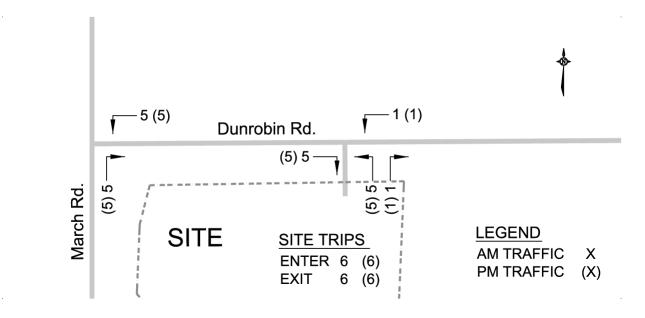
The veterinary clinic will be open to the public between the hours of 8:00 AM to 8:00 PM Tuesday to Thursday, and from 8:00 AM to 5:00 PM Monday and Friday. The clinic will also be open on Saturday.

A survey was taken at the existing clinic during the week of March 15-20, 2021. The survey shown below recorded patrons arriving for veterinary visits, and to pick up medication and pet food.

	Mon. N	March 15 (8a	m -5pm)	Tues. N	1arch 16 (8a	m -8pm)	Wed. N	1arch 17 (8a	m -8pm)	Thurs. N	March 18 (8a	am -8pm)	Fri. Ma	arch 19 (8an	n -5pm)	Sat. M	arch 20 (9ar	n -4pm)	Weekly
Hours	Appt.	Med P/U	Food P/U	Appt.	Med P/U	Food P/U	Appt.	Med P/U	Food P/U	Appt.	Med P/U	Food P/U	Appt.	Med P/U	Food P/U	Appt.	Med P/U	Food P/U	Total
8am - 9am		1		2		1	4			4			2		1				15
9am - 10am	555		I	1		275			1	1	1	2	4	1	1	2	perc		14
10am - 11am	1	1	I			1			3	1	000	175-75	2	70.000	1000	2	1		12
11am - 12pm	2	1	I	1	2	1		1				1	1			1	1	1	12
12pm - 1pm	1	1	1	1	37		2					1	1	2	1	1		1	13
1pm - 2pm	2	1	2	4	2	1	2		1	3		3	1	3	2250	2	1	1000	28
2pm - 3pm	1	1	1	2			4	1	1	2	2		4	2	1	2		1	25
3pm - 4pm	1	1	2	3	1	2	1	120		3	2	1	1	0.000	1			2	21
4pm - 5pm		2	1		1		==	2	I	0.0	9	2	2	1	1		1	1000	12
5pm - 6pm				1	1	1	2	3	1	2	1	2							14
6pm - 7pm				1		2				1	1								5
7pm - 8pm				1							1								2
Daily TOTAL	8	9	7	17	7	9	15	7	7	17	8	12	18	9	6	10	2	5	173

The survey shows that during the period between 8:00 AM and 9:00 AM Monday to Friday there was an average of 3 patrons entering the site, and between 3:00 PM and 4:00 PM there was an average of 4 patrons entering the site. The analysis has assumed an average of 4 patrons per hour, and applied a 1.5 peaking factor producing a total of 6 patrons entering and 6 patrons exiting during both the weekday peak AM and PM hours. The client has further said that approximately 75 percent of their patrons are from the Kanata area. The expected peak AM and PM hour site generated trips are shown Figure 3.

FIGURE 3 - PEAK HOUR SITE GENERATED TRIPS



3.2 Traffic Analysis

The analysis of the site access and the Dunrobin/March intersection will use the *Highway Capacity Software, Version 7.9.5*, which uses the capacity analysis procedure as documented in the *Highway Capacity Manual (HCM) 2010 and HCM 6th Edition*.

For signalized intersections (Dunrobin/March intersection), the operation or level of service of an intersection is determined from the volume to capacity ratio (v/c) for each lane movement as documented by the City of Ottawa in the *Transportation Impact Assessment Guidelines (2017)*. The following relates the level of service with the volume to capacity ratio at each lane movement.

LEVEL OF SERVICE	VOLUME TO CAPACITY RATIO
Level of Service A Level of Service B Level of Service C	0 to 0.60 0.61 to 0.70 0.71 to 0.80
Level of Service D Level of Service E Level of Service F	0.81 to 0.90 0.91 to 1.00 > 1.00

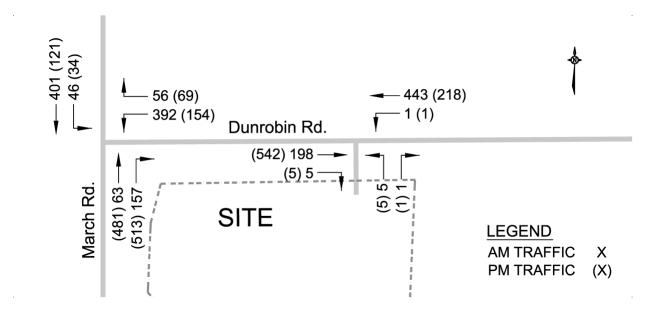
For unsignalized intersections (Access/Dunrobin intersection), the level of service of each lane movement and approach is determined as a function of the average control delay of vehicles at the approach. The following relates the level of service of each lane movement with the expected control delay at the approach.

LEVEL OF SERVICE	AVERAGE CONTRO	OL DELAY
Level of Service A Level of Service B Level of Service C Level of Service D Level of Service E Level of Service F	0-10 sec./vehicle >10-15 sec./vehicle >15-25 sec./vehicle >25-35 sec./vehicle >35-50 sec./vehicle >50 sec./vehicle	Little or No Delay Short Traffic Delays Average Traffic Delays Long Traffic Delays Very Long Traffic Delays Extreme Delays – Demand Exceeds Capacity

The expected length of queue at the critical lane movements for an unsignalized two-way stop controlled intersection was determined by the calculation of the 95th percentile queue at the lane approach. The 95th percentile queue length is the calculated 95th greatest queue length out of 100 occurrences at a movement during a 15-minute peak period. The 95th percentile queue length is a function of the capacity of a movement and the total expected traffic, with the calculated value determining the magnitude of the queue by representing the queue length as fractions of vehicles.

The traffic analysis was conducted at both the site access onto Dunrobin Road and the Dunrobin/March signalized intersection. The analysis utilized the 2020 traffic counts taken at the Dunrobin/March intersection. The existing 2020 counts of Figure 1 were added to the expected site generated trips of Figure 3 to produce the total traffic which is shown in Figure 4. The result is the expected traffic at the site access and Dunrobin/March intersection (Figure 4) using the pre-COVID-19 traffic.

FIGURE 4 - TOTAL PEAK AM AND PM HOUR TRAFFIC



DUNROBIN/MARCH INTERSECTION

The intersection of Dunrobin Road and March Road is a traffic signal controlled intersection with Dunrobin Road forming the southbound approach. The operational analysis has utilized a 98 second signal cycle for the two phase cycle in the peak AM hour, and 114 second cycle for the three phase cycle during the peak PM hour. The traffic signal timing plan is provided as Exhibit 3 in the Appendix.

The operational analysis used the 2020 traffic volumes and included the trips from the proposed site. The analysis determined that the intersection functioned at an acceptable level of service (LOS) for all lane movements, with the intersection functioning at an overall LOS "A" during both the weekday peak AM and PM hour. Table 1 summarizes the operation of the Dunrobin/March intersection with the analysis work sheet for the total traffic provided as Exhibit 4 for the peak AM hour and Exhibit 5 for the peak PM hour.

The analysis examined the length of vehicular queuing (50th percentile) at all approaches to the Dunrobin/March intersection. The critical time period for queuing at the southbound Dunrobin Road approach is during the weekday peak AM hour. The analysis determined that the vehicular queue at the southbound left turn movement was 120.6 ft (36.8 m) and at the shared left/right turn movement 133.5 ft (40.7 m). The conceptual site plan of Figure 2 shows that the site access is located 70 m north of the stop bar at the southbound approach. The peak AM hour queuing would result in only a minor impact on the operation of the site access and would not extend past the proposed site access as calculated in the 50th percentile queuing analysis.

TABLE 1
DUNROBIN/MARCH INTERSECTION – LOS & v/c Ratio

INTERSECTION APPROACH		DAY PEAK AM R 2020 Total	WEEKDAY PEAK PM HOUR 2020 Total				
AFFROACH	LOS	v/c Ratio	LOS	v/c Ratio			
EB Left - March	А	0.068	Α	0.075			
EB Through - March	Α	0.465	Α	0.116			
WB Through - March	Α	0.075	В	0.653			
WB Right - March	Α	0.137	Α	0.598			
SB Left - Dunrobin	А	0.450	Α	0.274			
SB Left/Through - Dunrobin	Α	0.354	Α	0.275			
Total Intersection	Α	0.252	Α	0.336			

ACCESS/DUNROBIN INTERSECTION

The proposed site access will be located onto Dunrobin Road approximately 90 m (centreline to centreline) from the Dunrobin/March intersection. The site access will be located at the same location as the existing access to the site which is shown in the photograph below.

SITE ACCESS AND DUNROBIN ROAD INTERSECTION



The southbound approach to the Dunrobin/March intersection comprises of a shared left/right turn lane and an exclusive left turn lane with 70 m of vehicular storage and a 90 m taper. The site access intersection will be located at approximately the transition from the southbound left turn taper to full width storage lane. The photograph shows the location of the existing/proposed access, which is also shown in the conceptual site plan of Figure 2.

The intersection will be a two-way stop control "T" intersection with a stop sign at the westbound access approach. The access will be 6.0 m in width with one lane entering and one lane exiting. The westbound exiting lane will contain a shared left/right turn lane. The intersection has the following lane configuration:

Northbound Dunrobin Approach One shared through/right lane

Southbound Dunrobin Approach One exclusive left turn lane (90 m taper)

One through lane

Westbound Site Access Approach One shared left/right turn lane (stop sign)

The operational analysis determined that during both the peak AM and PM hours the southbound March Road left/through lane would function at a LOS "A" and the site access approach at a LOS "B", with the intersection operating at an overall level of service LOS "B". Table 2 summarizes the operation of the intersection with the analysis work sheets provided as Exhibit 6 and Exhibit 7.

TABLE 2
ACCESS/DUNROBIN INTERSECTION – LOS & Control Delay

INTERSECTION APPROACH		DAY PEAK AM R 2020 Total	WEEKDAY PEAK PM HOUR 2020 Total				
ALLINOAGII	LOS	Delay (sec.)	LOS	Delay (sec.)			
WB Right - Site Access	В	11.4	В	11.4			
SB Left/Through - Dunrobin	Α	7.6	Α	8.6			
Total Intersection	В		В				

The operational analysis determined that during the peak AM and PM hour the southbound left turn movement would experience a 95th percentile queue of 0.0 vehicles. The southbound Dunrobin Road through movement traffic would be able to pass any left turning queued vehicles using the southbound left/right turn lane. The westbound site access approach would experience a 95th percentile queue of 0.0 vehicles during the peak AM hour and 0.1 vehicles during the peak PM hour.

SUMMARY

A Site Plan Application is being prepared for the Olender Veterinarian Clinic which will be located on a parcel of land at the northeast corner of the intersection of Dunrobin Road and March Road. The property has a municipal address of 4 Campbell Reid Court and currently is occupied by one single-family home. The width of the municipal road frontage onto Campbell Reid Court is 15.7 m which would not provide sufficient width for the single-family home and an access road to the clinic. It is proposed that the access to the clinic be located at the same location as the existing property access onto Dunrobin Road at 90 m (centreline to centreline) from the Dunrobin/March intersection.

The following is a summary of the findings of the assessment:

- Trip counts of patrons of the clinic obtained from the client determined that during the weekday peak AM and PM hour the clinic would expect 6 patrons who would be both entering and exiting the site.
- 2) The access would be located at the transition point between the 70 m storage lane and 90 m taper for the southbound Dunrobin Road approach to the Dunrobin/March intersection (See Figure 2).
- 3) The access will be 6 m in width with one lane entering and one lane exiting. It is proposed that the access provide full turning movements entering and exiting.
- 4) Preliminary comments from City of Ottawa staff in an email dated April 9, 2021 stated that staff has no immediate concerns with a full movement access to the site. A copy of the email is provided as Exhibit 8 in the Appendix.
- 5) An operational analysis determined that the proposed access and traffic would have a minor impact on the adjacent roads triggering no requirement for roadway modifications. The access is proposed to provide full turning movements.
- 6) Vehicular queuing (50th percentile) at the southbound Dunrobin Road approach to the Dunrobin/March intersection during the weekday peak AM hour was determined to be 40.7 m. The queue would not extend past the site access or have an impact on the operation of the access or traffic along Dunrobin Road.

Prepared by:

David & Wals

David J. Halpenny, M. Eng., P. Eng.



APPENDIX

TRAFFIC COUNTS TRAFFIC SIGNAL TIMING PLAN **INTERSECTION ANALYSIS WORK SHEETS** PRELIMINARY COMMENTS FROM CITY OF OTTAWA STAFF

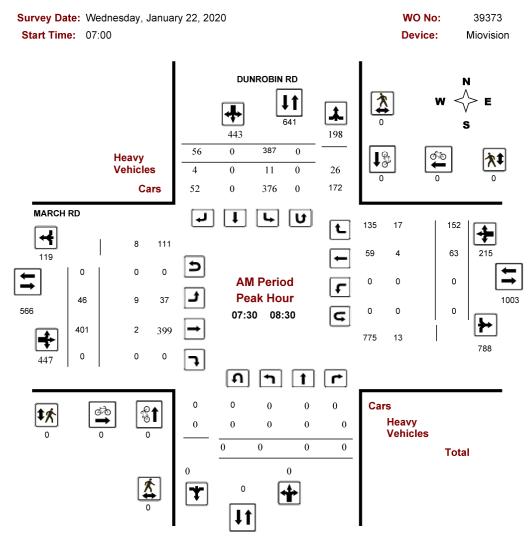
EXHIBIT 1 2020 PEAK AM HOUR TRAFFIC COUNTS - DUNROBIN/MARCH INTERSECTION



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

DUNROBIN RD @ MARCH RD



Comments 5472188 - WED JAN 22, 2020 - 8HRS - LORETTA

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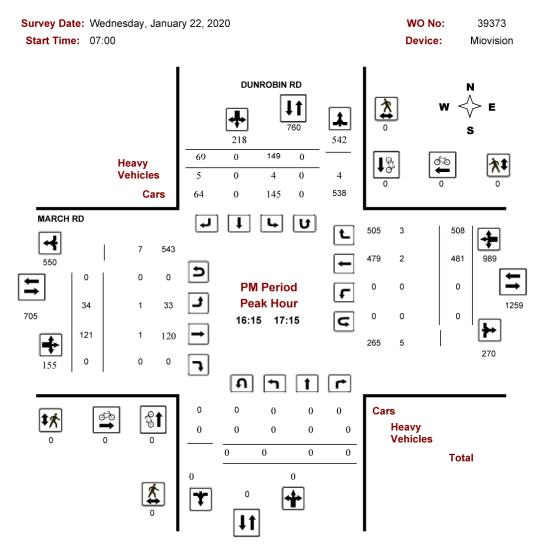
EXHIBIT 2 2020 PEAK PM HOUR TRAFFIC COUNTS - DUNROBIN/MARCH INTERSECTION



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

DUNROBIN RD @ MARCH RD



Comments 5472188 - WED JAN 22, 2020 - 8HRS - LORETTA

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EXHIBIT 3 TRAFFIC SIGNAL TIMING PLAN - DUNROBIN/MARCH INTERSECTION

Traffic Signal Timing

City of Ottawa, Transportation Services Department

Traffic Signal Operations Unit

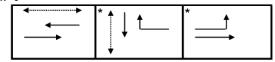
Intersection:	Main:	March	Side	e:	Dunrobin
Controller:	MS-3200		TS	D:	5645
Author:	Matthew	Anderson	Da	te:	17-Mar-2021

Existing Timing Plans[†]

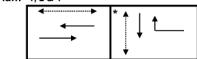
Plan **Ped Minimum Time** AM Peak Off Peak PM Peak Walk DW A+R Night 3 2 4 Cycle Free Free Free Free Offset min=56.3 EB Thru min=56.3 min=41.3 min=41.3 4.6+1.7 min=56.3 min=56.3 WB Thru min=41.3 13 4.6+1.7 min=41.3 max=36.3 max=26.3 SB Thru max=41.3 max=31.3 3.7+2.6 14 WBRT max=31.3 3.7+2.6 max=41.3 max=36.3 max=26.3 EBLT max=21.3 4.6+1.7

Phasing Sequence[‡]





Plan: 1,2 & 4



Schedule

Weekday

Time	Plan
0:10	4
6:00	1
9:30	2
14:20	3
18:00	2
22:00	4

Weekend

Time	Plan
0:10	4
10:00	2
21:00	4

Notes

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

Cost is \$59.96 (\$53.06 + HST)

EXHIBIT 4 2020 PEAK AM HOUR TRAFFIC ANALYSIS - DUNROBIN/MARCH INTERSECTION

		HCS	7 Sig	nalize	d Int	ersec	tion F	Resul	ts Sur	nmary	,				
General Inforn	nation							!	Intersect	ion Info	ormati	on	_ 6	∳ [14 L
Agency								\rightarrow	Duration,		0.250)		**	
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Urban Street		Dunrobin Road		Analys	sis Year	2020			Analysis	Period	1> 7:	00	7		
Intersection		Dunrobin/March		File Na	ame	2020	AM.xus	3							
Project Descrip	tion	Dr. Olender Vet Clir	nic											14141	7
Demand Inform	nation				EB		\top	WE	3	$\overline{}$	NB		\top	SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v				46	401			63	157				392	0	56
				- 12			•		100						
Signal Informa	ation					2115	\top	\top	\neg	\top					1
Cycle, s	98.0	Reference Phase	2	1	<u> -</u> 3 ⁴	1543							4		Φ
Offset, s	0	Reference Point	End	<u> </u>								1	2	3	4
Uncoordinated	No	Simult, Gap E/W	On	Green		35.0	0.0	0.0	0.0	0.0			—		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow Red	1.7	3.7 2.6	0.0	0.0	0.0	0.0		5	6	7	
1 Sice Widde	1 ixeu	Official Cap 19/5	Oll	rteu	1.7	2.0	10.0	0.0	10.0	10.0					
Times Deculée				EDI	_	CDT	\A/D		WDT	NIDI	_	NDT	CDI		CDT
Timer Results				EBI	-	EBT	WB	_	WBT	NBL	-	NBT	SBI	-	SBT
Assigned Phas	e			_	_	2	_	-	6		_		-	-	4
Case Number				_	_	6.0	_	_	7.0		_		-	_	10.0
Phase Duration	1, S					56.7		_	56.7		_		_		41.3
Change Period						6.3			6.3						6.3
Max Allow Hea	dway (<i>l</i>	<i>MAH</i>), s				0.0			0.0						3.2
Queue Clearan	ice Time	e (g s), s													14.3
Green Extension	n Time	(g e), s				0.0			0.0						1.0
Phase Call Pro	bability														1.00
Max Out Proba	bility														0.00
Movement Gro	oup Res	sults			EB		_	WB			NB			SB	
Approach Move				L	T	R	T.	Т	R	L	T	R	L	T	R
Assigned Move				5	2	1	<u> </u>	6	16			1	7	4	14
		· \ //-		50	436		-	68	105			-	277	210	14
Adjusted Flow		,.		_	_		_						-	_	
		ow Rate (s), veh/h/l	n	1301	1786		-	1730				-	1674	1614	
Queue Service		<u> </u>		1.9	15.0		_	1.9	3.6			-	12.3	11.8	
		e Time (<i>g c</i>), s		3.9	15.0		_	1.9	3.6			-	12.3	11.8	
Green Ratio (g				0.52	0.52			0.52	0.52				0.37	0.37	
Capacity (c), v	/eh/h			730	937			907	769				615	593	
Volume-to-Cap	acity Ra	atio (X)		0.068	0.465			0.075	0.137				0.450	0.354	
Back of Queue	(Q), ft	/In (50 th percentile)		15	154			19.3	31.2				120.6	133.5	
Back of Queue	(Q), v	eh/ln (50 th percenti	le)	0.6	6.1			0.7	1.2				4.7	5.1	
Queue Storage	Ratio (RQ) (50 th percent	tile)	0.04	0.00			0.00	0.08				0.56	0.00	
Uniform Delay	(d 1), s	/veh		12.5	14.7			11.5	11.9				23.5	33.0	
Incremental De	lay (d 2), s/veh		0.2	1.7			0.2	0.4				0.2	0.1	
Initial Queue D				0.0	0.0			0.0	0.0				0.0	0.0	
Control Delay (,.		12.7	16.3			11.7	12.3				23.7	33.1	
Level of Service				В	В			В	В				C	C	
Approach Dela				15.9		В	12.1	_	В	0.0			27.8	_	С
Intersection De	•			15.8).4		D	0.0			C 27.6	,	0
	, 5, 40														
Multimodal Re	sults				EB			WB			NB			SB	
Pedestrian LOS	S Score	/LOS		1.38	3	Α	1.90		В	2.04		В	1.96	3	В
Bicycle LOS So	oro / 1 (ne		1.29		Α	0.77	7	Α				1.29		Α

EXHIBIT 5 2020 PEAK PM HOUR TRAFFIC ANALYSIS - DUNROBIN/MARCH INTERSECTION

		HCS	7 Sig	nalize	d Int	ersec	tion F	Resu	lts Sur	nmary	,				
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Agency								\rightarrow	Duration,		0.250				
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Jurisdiction		City of Ottawa		Time F		1 00111	PM Hou	_	PHF		0.92		*		-
Urban Street		Dunrobin Road		Analys	is Yea	r 2020			Analysis	Period	1> 7:	00	7		
Intersection		Dunrobin/March		File Na	ame	2020_	PM.xus	3							
Project Descrip	tion	Dr. Olender Vet Clir	nic										1	14144	* 1
	- 11						_) A #		_			_		
Demand Inform					EB			WE	_		NB			SB	
Approach Move				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), v	eh/h		_	34	121	_	-	48	1 513	_	_	_	154	0	69
Signal Informa	tion					_		_		_					7
Cycle, s	114.0	Reference Phase	2	1	E3	1.2							я		小
Offset, s	0	Reference Point	End									1	2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Green		50.1	30.0	0.0	0.0	0.0		_	~		
Force Mode				Yellow	-	4.6 1.7	3.7 2.6	0.0		0.0		^		7	0
Force wode	Fixed	Simult. Gap N/S	On	Red	1.7	1.7	2.0	0.0	0.0	0.0		٥	ь	/	0
Timer Results				EBI	_	EBT	WB	1	WBT	NBL	_	NBT	SBI		SBT
				5	-	2	VVD	-	6	INDL	-	NDI	361	-	4
Assigned Phase Case Number	e			1.0	_	4.0	_	_	7.3		_		_	_	10.0
				21.3	-	77.7	_	-	56.4		-		-	-	36.3
Phase Duration		.) . 0			_	6.3	_		6.3				_	_	6.3
Change Period		, .		6.3			-	-					-	-	
		,·		3.1		0.0	_		0.0				-	_	3.3 9.4
Queue Clearan				3.0	-	0.0	-	-	0.0				-	-	
Green Extension		(<i>g</i> _e), s		0.0	-	0.0	_	-	0.0		_		-	_	0.4
Phase Call Prol				1.00	-		_	-			-		-	_	1.00
Max Out Proba	DIIITY			0.00	,		_	-			-		_	_	0.00
Movement Gro	oup Res	sults			EB			WB			NB			SB	
Approach Move				L	T	R		Т	R	L	T	R	L	T	R
Assigned Move				5	2	1	<u> </u>	6	16			1,	7	4	14
Adjusted Flow F) veh/h		37	132		_	523	405				126	117	
		ow Rate (s), veh/h/l	n	1674	1786			1786	_				1688	1561	
Queue Service		, ,	"	1.0	3.3		_	26.0	_				6.7	7.4	
Cycle Queue C		- , .		1.0	3.3			26.0	_				6.7	7.4	
Green Ratio (g		C 11110 (g c), 3		0.61	0.64		_	0.45	_				0.27	0.27	
Capacity (c), v				490	1134			801	678				459	424	
Volume-to-Capa		atio (X)		0.075				0.653	_				0.274		
		/In (50 th percentile)		9.1	31.9			_	3 218.7				69	74.2	
		eh/ln (50 th percenti		0.4	1.3			11.5	_				2.7	2.9	
		RQ) (50 th percent		0.4	0.00			0.00	_				0.32	0.00	
Uniform Delay		, , , ,	()	12.6	8.4			24.5	-				32.6	37.3	
Incremental De				0.0	0.2			4.1	3.9				0.1	0.1	
Initial Queue De	, ,	,.		0.0	0.0			0.0	0.0				0.0	0.0	
Control Delay (,		12.7	8.6			28.7	27.6				32.8	37.4	
Level of Service				B	A			C C	C C				C	D D	
Approach Delay				9.5	_	Α	28.3	_	С	0.0			35.0	_	D
Intersection De				9.5			7.1		U	0.0			C		D
Multimodal Re					EB			WB			NB			SB	
Pedestrian LOS				1.36	\rightarrow	Α	1.9	-	В	2.16		В	1.96	-	В
Bicycle LOS So	ore / LO	OS		0.77	'	Α	2.02	2	В				0.89	9	Α

EXHIBIT 6 PEAK AM HOUR TRAFFIC ANALYSIS - SITE ACCESS/DUNROBIN INTERSECTION

			-	I WO			,	ntrol	veh	ort_						
General Information							Site	Inforn	natio	n						
Analyst	Т						Inters	ection			Site A	ccess/D	unrobin	Road		
Agency/Co.							Juriso	liction			City c	of Ottawa	a			
Date Performed	4/12/	2021					East/\	West Stre	eet		Site A	ccess				
Analysis Year	2020						North	/South S	Street		Dunro	obin Roa	ıd			
Time Analyzed	Peak	AM Hou	ır				Peak	Hour Fac	ctor		0.92					
Intersection Orientation	North	-South					Analy	sis Time	Period (hrs)	0.25					
Project Description	Olend	der Vete	rnary Cli	nic												
Lanes																
				* *Y↑ * ∩		† † † † r Street: Nor		4 + A + + t								
Vehicle Volumes and Ad	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	10	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	2	0
Configuration							LR					TR		LT	T	
													$\overline{}$	$\overline{}$		
Volume (veh/h)	_					5		1			198	5		1	443	
Volume (veh/h) Percent Heavy Vehicles (%)						5		0			198	5		0	443	
											198	5			443	
Percent Heavy Vehicles (%)						0)				198	5			443	
Percent Heavy Vehicles (%) Proportion Time Blocked						0)				198	5			443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%)				Undi	vided	0)				198	5			443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized	eadwa	ys		Undi	vided	0)				198	5			443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage	eadwa	ys		Undi	vided	0					198	5			443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H	eadwa	ys		Undi	vided	0		0			198	5		0	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec)	eadwa	ys		Undi	vided	7.5)	6.2			198	5		4.1	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec)	eadwa	ys		Undi	vided	7.5		6.2			198	5		4.1 4.10	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec)			ervice		vided	7.5 6.80 3.5		6.2 6.20 3.3			198	5		4.1 4.10 2.2	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec)			ervice		vided	7.5 6.80 3.5	7	6.2 6.20 3.3			198	5		4.1 4.10 2.2	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an			ervice		vided	7.5 6.80 3.5		6.2 6.20 3.3			198	5		4.1 4.10 2.2 2.20	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h)			ervice		vided	7.5 6.80 3.5	7	6.2 6.20 3.3			198	5		4.1 4.10 2.2 2.20	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h)			ervice		ivided	7.5 6.80 3.5	7 567	6.2 6.20 3.3			198	5		4.1 4.10 2.2 2.20	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio			ervice		vided	7.5 6.80 3.5	7 567 0.01	6.2 6.20 3.3			198	5		4.1 4.10 2.2 2.20 1 1360 0.00	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio 95% Queue Length, Q ₉₅ (veh)			ervice		vided	7.5 6.80 3.5	7 567 0.01	6.2 6.20 3.3			198	5		4.1 4.10 2.2 2.20 1 1360 0.00	443	
Percent Heavy Vehicles (%) Proportion Time Blocked Percent Grade (%) Right Turn Channelized Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio 95% Queue Length, Q95 (veh) Control Delay (s/veh)			ervice		vided	7.5 6.80 3.5	7 567 0.01 0.0 11.4 B	6.2 6.20 3.3			198	5		1 1360 0.00 0.00 7.6 A	.00	

EXHIBIT 7 PEAK PM HOUR TRAFFIC ANALYSIS - SITE ACCESS/DUNROBIN INTERSECTION

								TILIOI	Rep								
General Information							Site Information										
Analyst	\top						Intersection				Site Access/Dunrobin Road						
Agency/Co.					Jurisdiction				City of Ottawa								
Date Performed	4/12/	4/12/2021					East/West Street				Site Access						
Analysis Year	2020						North/South Street				Dunrobin Road						
Time Analyzed	Peak PM Hour						Peak Hour Factor				0.92						
Intersection Orientation	North-South						Analysis Time Period (hrs)				0.25						
Project Description	Olender Veternary Clinic																
Lanes																	
				4 *\ \ \ \ \ \		ナ ナキア r Street: Nor		7 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4									
Vehicle Volumes and Ad	justme	nts															
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	10	1	2	3	4U	4	5	6	
Number of Lanes	_	0	0	0		0	1	0	0	0	1	0	0	0	2	0	
Configuration	_						LR					TR		LT	T		
Volume (veh/h)						5		1			542	5		1	218		
Percent Heavy Vehicles (%)	_					0		0						0			
Proportion Time Blocked																	
Percent Grade (%)					0												
Right Turn Channelized																	
	+																
Median Type Storage				Undi	vided												
Median Type Storage	eadwa	ys		Undi	vided												
	eadwa	ys		Undi	vided	7.5		6.2						4.1			
Median Type Storage Critical and Follow-up H	eadwa	ys		Undi	vided	7.5 6.80		6.2						4.1			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec)	eadwa	ys		Undi	vided	_											
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec)	eadwa	ys		Undi	vided	6.80		6.20						4.10			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec)			ervice		vided	6.80 3.5		6.20 3.3						4.10 2.2			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec)			ervice		vided	6.80 3.5	7	6.20 3.3						4.10 2.2			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an			ervice		vided	6.80 3.5	7 388	6.20 3.3						4.10 2.2 2.20			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an			ervice		vided	6.80 3.5		6.20 3.3						4.10 2.2 2.20			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h)			ervice		vided	6.80 3.5	388	6.20 3.3						4.10 2.2 2.20 1 992			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio			ervice		vided	6.80 3.5	388	6.20 3.3						4.10 2.2 2.20 1 992 0.00			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, an Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio 95% Queue Length, Q95 (veh)			ervice		vided	6.80 3.5	388 0.02 0.1	6.20 3.3						4.10 2.2 2.20 1 992 0.00 0.0			
Median Type Storage Critical and Follow-up H Base Critical Headway (sec) Critical Headway (sec) Base Follow-Up Headway (sec) Follow-Up Headway (sec) Delay, Queue Length, and Flow Rate, v (veh/h) Capacity, c (veh/h) y/c Ratio 95% Queue Length, Q95 (veh) Control Delay (s/veh)			ervice		vided	6.80 3.5	388 0.02 0.1 14.4 B	6.20 3.3						4.10 2.2 2.20 1 992 0.00 0.0 8.6 A	0.0		

EXHIBIT 8
PRELIMINARY CITY OF OTTAWA STAFF COMMENTS

RE: Olender Veterinary Clinic

Subject: RE: Olender Veterinary Clinic

From: "McMahon, Patrick" <patrick.mcmahon@ottawa.ca>

Date: 4/9/2021, 1:57 PM

To: David J Halpenny <david@djhalpenny.com>

Good afternoon Dave,

At this stage, there are no immediate concerns with the provision of a full movement access at this location. The access should be located as far north as possible, and typical review of sight lines and access parameters will be completed once a concept/site plan has been presented.

Thank you, Pat

Patrick McMahon
Project Manager, Infrastructure Approvals | GPRJ Approbation des demandes d'infrastructure
Development Review Branch | Dir Examen des projets d'aménagement
City of Ottawa | Ville d'Ottawa
Tel |Tél.: 613-580- 2424 ext. | poste 23298
web | Site Web: www.ottawa.ca

----Original Message----

From: David J Halpenny <a href

Sent: March 17, 2021 8:39 AM

To: Giampa, Mike Mike.Giampa@ottawa.ca
Subject: Olender Veterinary Clinic

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Good Morning Mike,

I am doing some preliminary traffic work for a veterinary clinic located at the northeast corner of the intersection of Dunrobin Road and March Road. The municipal address of the site is 4 Campbell Reid Court.

I believe you would be the Project Manager, Infrastructure Approvals contact.

My client would like to have an access onto Dunrobin Road. The access would be located approximately where the existing access is, approximately 75 m to 80 m from the southbound Dunrobin Rd. stop bar. The access would be located beyond the left turn storage lane.

The clinic would generate about 4 patients an hour. We would like to provide a full movement access onto Dunrobin Road. Do you have any comments.

Dave

-David Halpenny, M. Eng., P. Eng.
D. J. Halpenny & Associates Ltd.
Phone: (613) 692-8662
Email: David@DJHalpenny.com

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