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Residential Development 1040 Somerset Street West

Stormwater Management Report



Prepared for: Claridge Homes

RESIDENTIAL DEVELOPMENT 1040 SOMERSET STREET WEST OTTAWA, ONTARIO

STORMWATER MANAGEMENT REPORT

Prepared by:

NOVATECH ENGINEERING CONSULTANTS LTD. 240 Michael Cowpland Dr. - Suite 200 Ottawa, Ontario K2M 1P6

> File No.: 112191 Report Reference No.: R-2013-004 Prepared: April 02, 2013 Revised: April 14,2022

April 14, 2022

City of Ottawa
Development Review Central
Planning, Infrastructure and Economic Development
110 Laurier Avenue West
Ottawa, ON
K1P1J1

Attention: Mr. Jean-Charles Renaud

Reference: Residential Development

1040 Somerset Street West Stormwater Management Report

Our File No.: 112191

Enclosed herein is the Stormwater Management Report for the proposed Residential development at 1040 Somerset Street West, located in the southeast quadrant of the Breezehill Avenue North / Somerset Street West intersection. This report is submitted in support of the site plan application for the site and presents a stormwater management plan for the re-development of the site.

Trusting this report is adequate for your purposes. Should you have any questions, or require additional information, please contact us.

Yours truly,

NOVATECH ENGINEERING CONSULTANTS LTD.

Greg MacDonald, P.Eng

5 Milarel

Director, Land Development and Public Sector Infrastructure

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112191-GP General Plan of Services

112191-GR Grading and Erosion Control Plan 112191-ESC Erosion and Sediment Control Plan

112191-SWM Stormwater Management Plan

1.0 INTRODUCTION

Novatech has been retained to prepare a Stormwater management Report for the proposed residential development located at 1040 Somerset Street West within the City of Ottawa. The purpose of this report is to support the site plan application for the subject development. The property is located in the southeast quadrant of the Breezehill Avenue North / Somerset Street West intersection in the City of Ottawa, as shown in **Figure 1a – Aerial Photo** and **Figure 1b – Key Plan**.

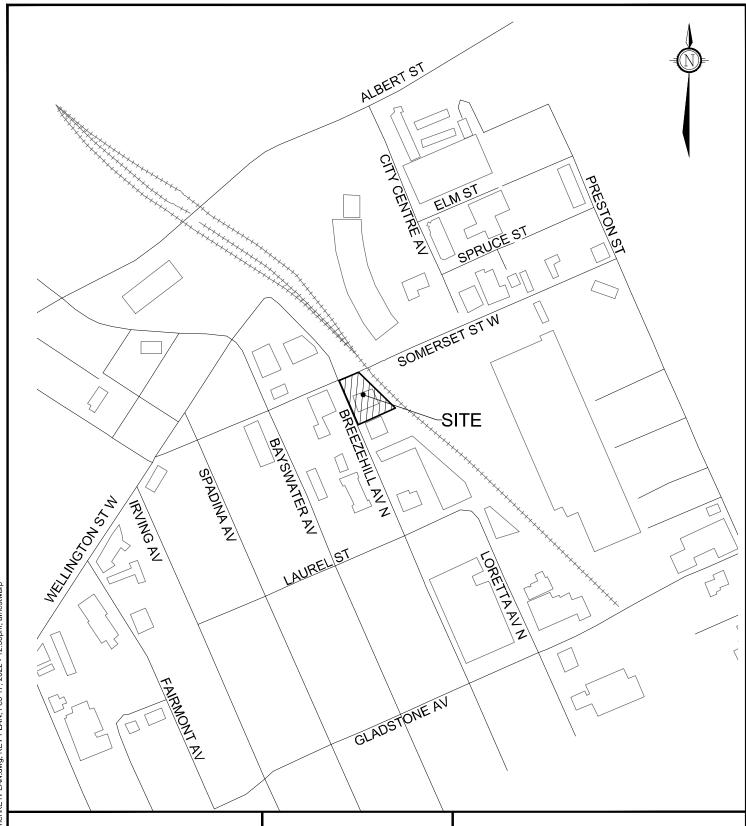
Figure 1a: Aerial Photo of Subject Site



Photo courtesy of www.bing.com/maps

The subject site is approximately 0.135 ha in area and is bound by Somerset Street West to the north, the O-Train transit corridor to the east; a meditation center and an auto repair shop (53 Breezehill Avenue North) to the south; and Breezehill Avenue N. to the west. The existing property is currently occupied by a one storey building with commercial uses.

The proposed re-development will consist of a 30-storey tower with 262 condominium units to be constructed in one phase. The building will include $105m^2$ of commercial floor space, located on the ground floor, and a total of 145 underground parking spaces will be provided on 7 levels of underground parking. **Refer to Figure 2 – Site Plan for details**. Access to the proposed site will a single two-way vehicular ramp access to the underground parking garage which will connect to Breezehill Avenue N. A copy of the topographical survey which shows the property outline is included in the back of this report. **Refer to Figure 3 – Existing Conditions**.



NOV/\TECH

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CITY OF OTTAWA 1040 SOMERSET STREET WEST

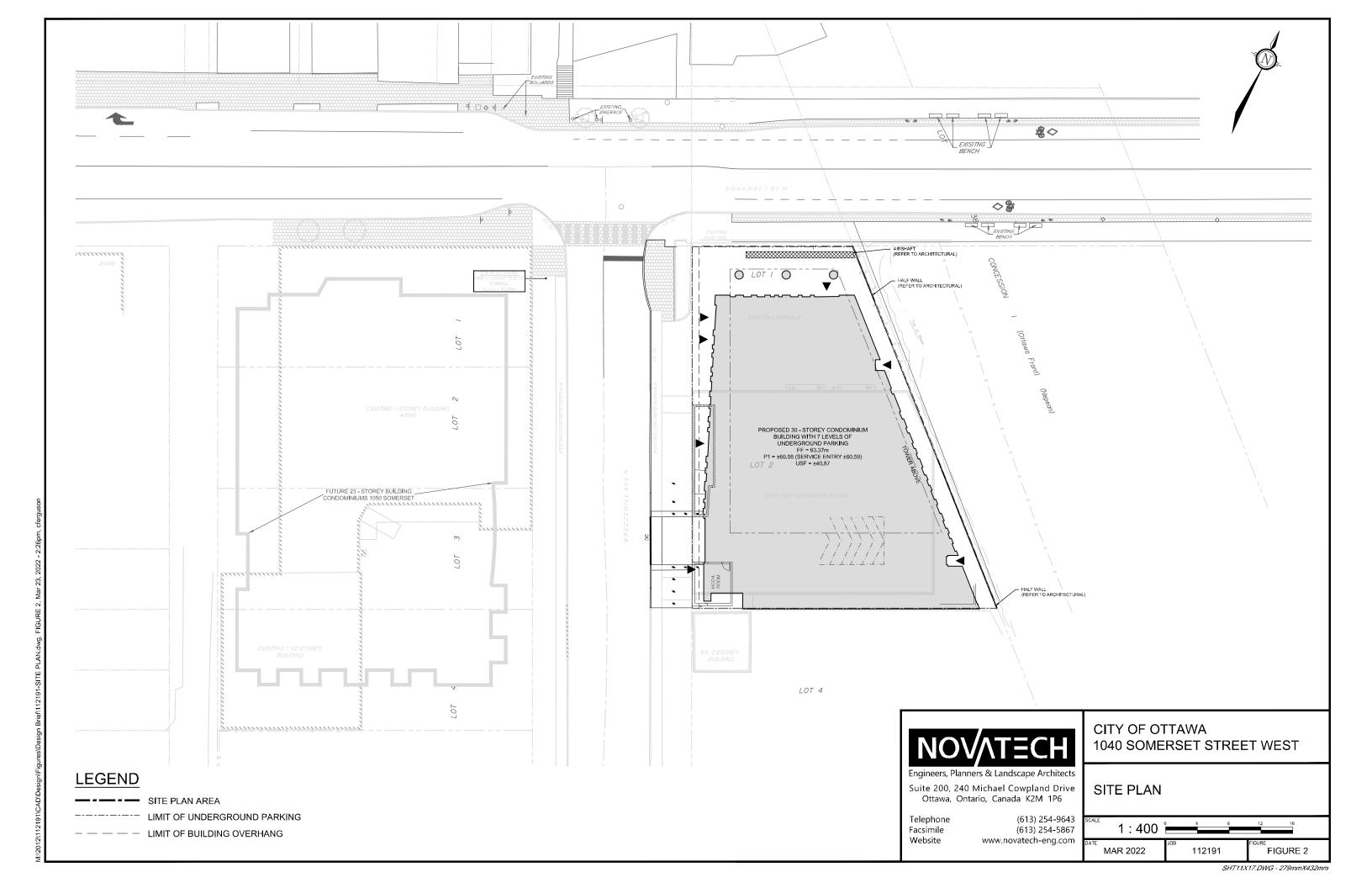
KEY PLAN

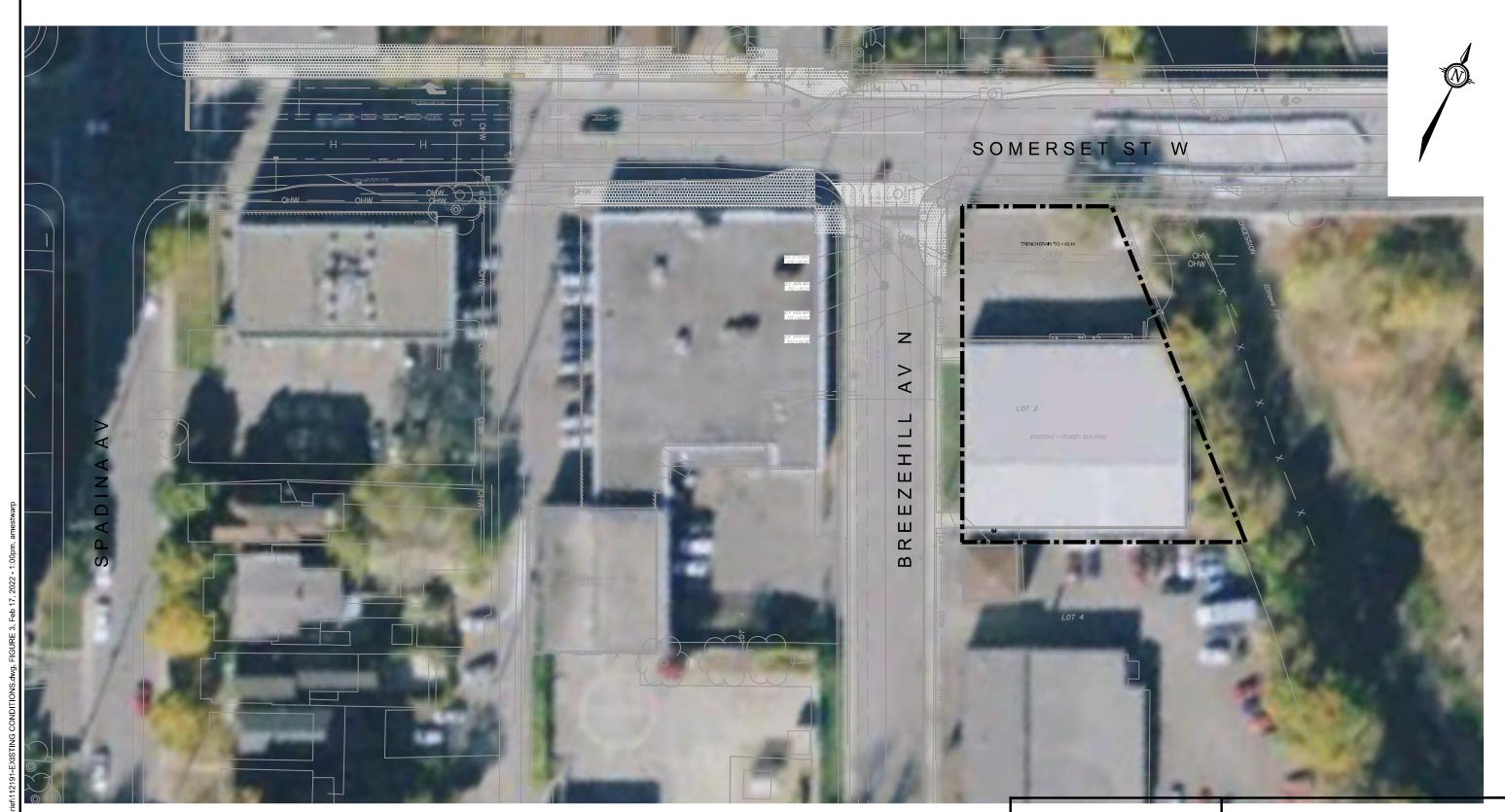
N.T.S

TE MAR 2022 JOB 112191

SHT8X11.DWG - 216mmx279mm

FIGURE 1b





LEGEND

SITE PLAN AREA

NOVATECH

Engineers, Planners & Landscape Architects Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6

Telephone Facsimile Website (613) 254-9643 (613) 254-5867 www.novatech-eng.com CITY OF OTTAWA 1040 SOMERSET STREET WEST

EXISTING CONDITIONS PLAN

1:500 5 10 15 20 MAR 2022 JOB 112191 FIGURE 3

SHT11X17.DWG - 279mmX432mm

2.0 CRITERIA

Through correspondence with the City of Ottawa, the following criteria have been adopted to control post-development stormwater discharge from the site:

- ➤ Control proposed development flows, up to and including the 100-year storm event, to a 5-year allowable release rate calculated using a runoff coefficient (C) of 0.50 and a time of concentration (T_c) of 20 minutes;
- ➤ Determine size and location of proposed drainage system components and identify any modifications required to existing infrastructure to meet SWM targets;
- Provide source controls which are in conformity with the City of Ottawa requirements, where possible;
- ➤ Limit ponding to 0.15 m for all rooftop storage areas and 0.30 m for all parking storage areas; and;
- ➤ Provide guidelines to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

The approach to the stormwater management design is to determine the allowable release rate for the site, calculate the uncontrolled flow, and ensure that the remaining flow, in combination with the uncontrolled flow, does not exceed the allowable release rate. All proposed development runoff in excess of the allowable release rate, will be attenuated on-site prior to being released into the storm sewer on Somerset Street West and Breezehill Avenue North.

3.0 EXISTING CONDITIONS

3.1 The Site

Under existing conditions, the site in question is currently occupied by a one storey building with commercial uses, including an auto care shop, a charity organization and an art studio, as illustrated in **Figure 3**. Stormwater flows from the site are currently conveyed to the existing storm sewer system within Breezehill Avenue North via overland flows, as well as the abutting Ottawa O-Train land.

As part of this development, all stormwater will be controlled on site and discharged via a 250 mm dia. storm service from the proposed condominium development that will connect to the existing 1350 mm dia. storm sewer on Breezehill Avenue North. The proposed storm service connection to the building will be equipped with a backwater valve.

3.2 Allowable Release Rate

The city requires that on-site stormwater management be implemented to control post-development stormwater discharge for the 100-year storm event to the 5-year storm, a time of concentration (T_c) of 20 minutes and a runoff coefficient (C) of 0.50. The allowable release rate for the proposed 0.135 ha development was calculated using the Rational Method to be 13.2 L/s. Calculations are included within **Appendix A**, for reference.

4.0 PROPOSED DEVELOPMENT STORMWATER MANAGEMENT DESIGN

Stormwater runoff flow from the site will be a combination of uncontrolled direct runoff and controlled flow. Stormwater management will be achieved with an internal cistern within the P1 mezzanine parking level. The site will be graded such that flows exceeding the 100-year storm event will be conveyed overland to Breezehill Avenue North, and the neighboring railway property as per existing conditions.

4.1 Stormwater Management Modeling

The performance of the proposed stormwater management system was evaluated using a dual-drainage model created in PCSWMM. The PCSWMM model simulates the storage and routing of flows through the proposed storm drainage network. The results of the analysis were used to:

- Calculate the storm sewer hydraulic grade line and ponding elevations for the 5-year, and 100-year storm events.
- Determine the allowable release rates from each drainage area and size the required inlet control devices (ICD's).
- Calculate the modelled runoff from the controlled portions and uncontrolled portions of the site under post-development conditions.

The design storms used in the hydrologic analysis model include the 6-hour and 4-hour Chicago distributions and the 12-hour and 24-hour SCS Type II distribution for the 2-year, 5-year and 100-years storm events. IDF data was taken from the *City of Ottawa Sewer Design Guidelines* (OSDG) (October 2012). The 6-hour Chicago storm distribution was found to generate the highest peak flows and the model results from this distribution are documented in the following tables. The model schematic, system parameters and output files are provided in **Appendix B**.

4.2 Drainage Areas

The development will consist of multiple drainage areas that are highlighted on the Stormwater Management Plan (112191-SWM) enclosed in the back of this report. The following is a detailed description of how the flows from each area will be managed.

Table 4.1 Drainage Area Descriptions

| Drainage Area No. | Total area (ha) | Runoff Coefficient - C | Description |
|-------------------|-----------------|------------------------|----------------------|
| D-01 | 0.006 | 0.90 | Uncontrolled |
| D-02 | 0.002 | 0.90 | Uncontrolled |
| R-01 | 0.126 | 0.90 | Controlled – Cistern |
| R-02 | 0.001 | 0.90 | Controlled – Cistern |
| Total = | 0.135 | | |

Drainage Areas D-01, and D-02 are the surficial uncontrolled runoff at the ground elevation that will flow overland towards Breezehill Avenue North, and the transit corridor, respectively. Drainage Areas R-01, and R-02 are rooftop areas which will drain into the cistern within the P1 mezzanine parking level and will ultimately discharge to the existing storm system within the Breezehill Avenue North right-of-way.

4.3 Uncontrolled Development Flows

The uncontrolled development flows from Areas D-01, and D-02 were calculated using PCSWM and results are summarized in Table 4.2. The model schematics and results are contained in **Appendix B**.

Table 4.2 Proposed Development Uncontrolled Flows Summary

| Area ID | 5-Year Flows (L/s) | 100-Year Flows (L/s) | 100-Year +20% Flows (L/s) |
|------------|-----------------------|-------------------------|------------------------------|
| D-01 | 1.73 | 2.98 | 3.57 |
| D-02 | 0.58 | 0.99 | 1.19 |
| Total | 2.31 | 3.97 | 4.76 |

4.4 Controlled Development Flows

The roof area will outlet to the 1350mm diameter storm sewer system in Breezehill Avenue N. The post-development flow from this sub-catchment area will be attenuated using an inlet control device (ICD) installed within the proposed building cistern within the P1 parking level.

Table 4.4 summarizes the post-development design flow from this sub-catchment area as well as the ICD specifications, the anticipated ponding elevations, storage volumes required and storage volume provided for the 2-year, 5-year and the 100-year design events.

Table 4.4: Area R-01 & R-02 Design Flow and ICD Table

| | Sub-Catchment Area R-01 & R-02 | | | | | |
|--------------|--------------------------------|----------------------|--------------------------|-------------------------------|------------------------------|--|
| Design Event | ICD Type | Design Flow (L/s) | Ponding Elevation (m) | Storage Vol. Required (m³) | Max Storage Provided (m³) | |
| 2-Year | | 5.30 L/s | 61.30 m | 21.0 m³ | | |
| 5-Year | LMF 85 | 6.34 L/s | 61.59 m | 29.0 m³ | 60.27 m³ | |
| 100-Year | 100-Year | | 62.43 m | 53.0 m³ | | |

Table 4.5 below compares the post-development design flows for the 5-year and 100-year design events to the target allowable release rates.

Table 4.5 Stormwater Management Post-Development Flow Summary

| Drainage Area ID | Drainage Area (ha) | Runoff Coefficient "C" | 5-Year Flows (L/s) | 100-Year Flows (L/s) | |
|------------------------|-----------------------|------------------------------|-----------------------|-------------------------|--|
| D-01 | 0.006 | 0.90 | 1.73 | 2.98 | |
| D-02 | D-02 0.002 0.90 | | 0.58 | 0.99 | |
| Direct Run | off Sub Total | 2.31 | 3.97 | | |
| R-01 | 0.126 | 0.90 | 6 24 | 0.74 | |
| R-02 | 0.001 | 0.90 | 6.34 | 8.74 | |
| Controlled | Flows Sub Tot | 6.34 | 8.74 | | |
| Total Post | Development F | 8.65 | 12.71 | | |
| Total Allow | able Release F | 13.2 | 13.2 | | |

As indicated in the **Table 4.5** above, the 5-year and 100-year post-development flows will be less than the target allowable release rate for the site. Refer to **Appendix B** for PCSSWMM model schematics and results, and Figures. Refer to the Grading Plan (112191-GR) and the Stormwater Management Plan for more details.

4.5 Major Overland Drainage

A major overland flow route will be provided for storms greater than the 100-year storm event. Stormwater will be directed to the Breezehill Avenue N. and the neighboring railway block as per the existing conditions. The major overland system is shown on the Grading Plan (drawing 112191-GR).

5.0 EROSION AND SEDIMENT CONTROL MEASURES

Temporary erosion and sediment control measures will be implemented on-site during construction in accordance with the Best Management Practices for Erosion and Sediment Control. This includes the following temporary measures:

- Filter socks (catchbasin inserts) will be placed in existing and proposed catchbasins and catchbasin manholes, and will remain in place until vegetation has been established and construction is completed;
- Silt fencing will be placed along the surrounding construction limits;
- Mud mats will be installed at the site entrances;
- Strawbale or rock check dams will be installed in swales and ditches;
- The contractor will be required to perform regular street sweeping and cleaning as required, to suppress dust and to provide safe and clean roadways adjacent to the construction site;

Erosion and sediment control measures should be inspected daily and after every rain event to determine maintenance, repair or replacement requirements. Sediments or granulars that enter site sewers shall be removed immediately by the contractor. These measures will be implemented prior to the commencement of construction and maintained in good order until vegetation has been established. Refer to the Erosion and Sediment Control Plan (drawing 112191-ESC) for additional information.

6.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the findings of this report, a stormwater management scheme has been identified that will achieve the allowable release rate required by the City. Therefore, the following conclusions are made:

- Stormwater discharge from the site will be controlled to the allowable 5-year pre-development storm event rate of 13.2 L/s by an ICD and storage within the proposed cistern within the P1 parking level for both the 5-year and 100-year post-development storm events;
- The site will be graded such that flows in excess of the 100-year storm event will be conveyed overland to Breezehill Avenue North, and the existing rail corridor.
- Sediment and erosion control measures will be implemented during construction.

7.0 CLOSURE

This report has been prepared in accordance with the requirements for site plan submission and is hereby submitted for approval.

NOVATECH ENGINEERING CONSULTANTS LTD.

Prepared by:

Anthony Mestwarp, P.Eng. Project Engineer

Reviewed by:



Greg MacDonald, P.Eng.
Director, Land Development
and Public Sector Infrastructure

Stormwater Design Prepared by:

Vahid Mehdipour, M.Sc.

Vahid Mehdipour

Water Resources

APPENDIX A IDF CURVES, RATIONAL METHOD, RUNOFF

RATIONAL METHOD

The Rational Method was used to determine both the allowable runoff as well as the proposed development runoff for the site. The equation is as follows:

Q=2.78 CIA

Where:

Q is the runoff in L/s
C is the weighted runoff coefficient*
I is the rainfall intensity in mm/hr**
A is the area in hectares

*The weighted runoff coefficient is determined for each of the catchment areas as follows:

$$C = \frac{(A_{perv} \times C_{perv}) + (A_{imp} \times C_{imp})}{A_{tot}}$$

Where:

A_{perv} is the pervious area in hectares

C_{perv} is the pervious area runoff coefficient (Cperv=0.20)

A_{imp} is the impervious area in hectares

C_{imp} is the impervious area runoff coefficient (Cimp=0.90)

A_{tot} is the catchment area (Aperv + Aimp) in hectares

ALLOWABLE RELEASE RATE AS SPECIFIED BY THE CITY

The allowable release rate was calculated for the proposed 0.1345 ha re-developed site, using a runoff coefficient (C) of 0.50 and a time of concentration (T_c) of 20 minutes, as specified by the City of Ottawa.

Drainage Area (A) = 0.1345 ha Runoff Coefficient (C) = 0.50 Intensity (I5) = 70.25 mm/hr

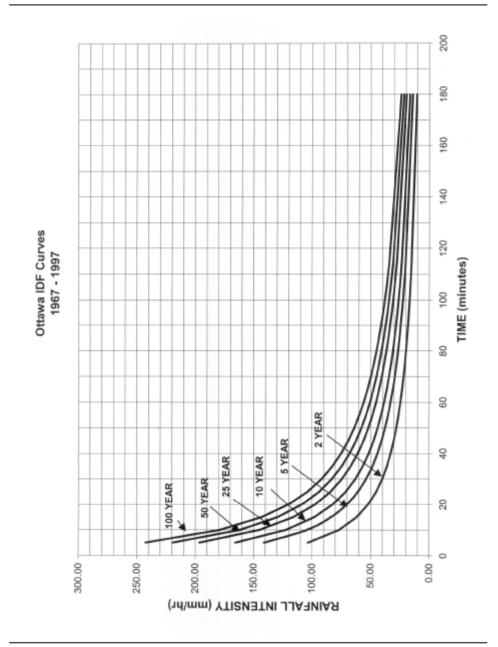
Q5= 2.78 CIA Q5= 2.78 x 0.50 x 70.25 mm/hr x 0.1345 ha Q5= 13.13 L/s

^{**} The rainfall intensity is taken from the City of Ottawa IDF Curves with a time of concentration of 20 min (refer to attached IDF Curves) as specified by the City of Ottawa.

Ottawa Sewer Design Guidelines

APPENDIX 5-A

OTTAWA INTENSITY DURATION FREQUENCY (IDF) CURVE



City of Ottawa Appendix 5-A.1 November 2004

^{*} IDF CURVE FROM OTTAWA SEWER DESIGN GUIDELINES - NOV 2004

DATE PREPARED: MARCH 2022

C is the runoff coefficient

PROJECT #: 112191
PROJECT NAME: 1040 SOMERSET STREET WEST
LOCATION: 1040 SOMERSET STREET WEST, CITY OF OTTAWA



TABLE 1A: Allowable Runoff Coefficient "C"

| Area | "C" |
|-------|------|
| Total | 0.50 |
| 0.135 | 0.50 |

TABLE 1B: Allowable Flows

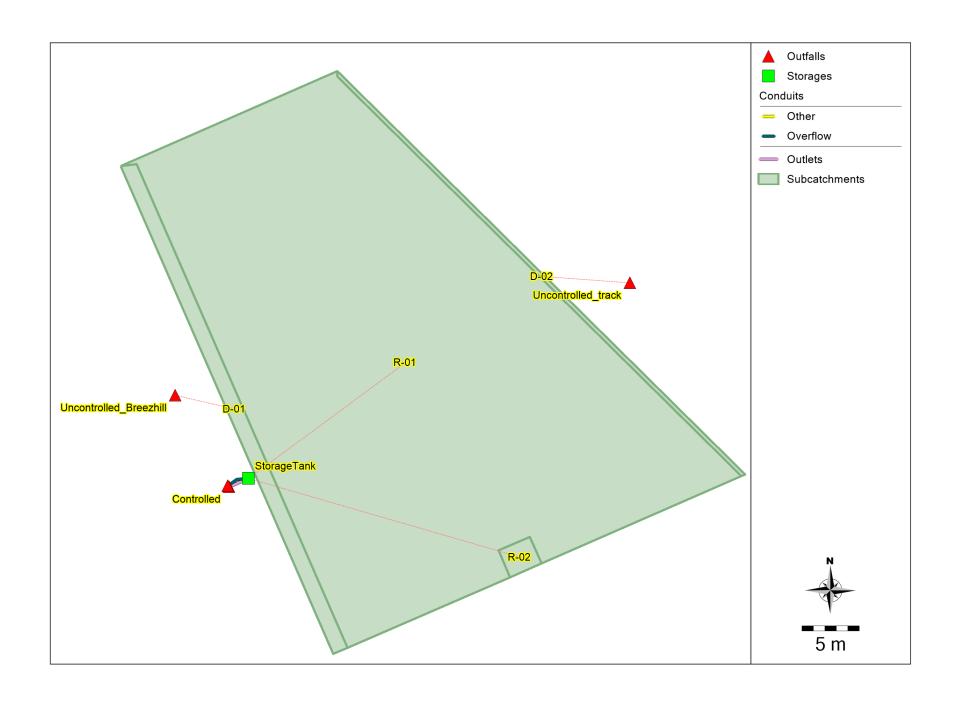
| Outlet Options | Area (ha) | "C" | Tc (min) | Q _{5 Year} (L/s) | Q _{ALLOW} (L/s) |
|--------------------|--------------|------|----------|------------------------------|-----------------------------|
| Cambridge Street N | 0.135 | 0.50 | 20 | 13.2 | 13.2 |

Time of Concentration Tc= 20 Equations: min Intensity (2 Year Event) 52.03 mm/hr Flow Equation Intensity (5 Year Event) I₅= 70.25 mm/hr $Q = 2.78 \times C \times I \times A$ Where: Intensity (100 Year Event) I₁₀₀= 119.95 mm/hr

100 year Intensity = 1735.688 / (Time in min + 6.014) $^{0.820}$

I is the rainfall intensity, City of Ottawa IDF 5 year Intensity = $998.071 / (Time in min + 6.053)^{0.814}$ A is the total drainage area

APPENDIX B SWM CALCULATIONS



Refer to the attached PCSWMM output file (112191_20220414.pcz) For model details.

APPENDIX C CORRESPONDENCE

From: Wu, John < John.Wu@ottawa.ca>
Sent: Wednesday, March 9, 2022 8:12 AM

To: Anthony Mestwarp <a.mestwarp@novatech-eng.com>

Subject: RE: 112191 - Storm Boundary Conditions Request - 1040 Somerset Street

The 100 year HGL as per our flood risk model is 58.02 m. The 5 year is within the pipe at 56.31.

John

From: Anthony Mestwarp <<u>a.mestwarp@novatech-eng.com</u>>

Sent: March 3, 2022 10:12 AM
To: Wu, John < John. Wu@ottawa.ca >

Cc: Vahid Mehdipour <v.mehdipour@novatech-eng.com>

Subject: RE: 112191 - Storm Boundary Conditions Request - 1040 Somerset Street

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Can you please let me know the approximate timing to provide the Hydraulic Grade line elevations within the existing stm sewer for the 100-yr, and 5-yr storm events.

Thanks,

Anthony Mestwarp, P.Eng., Project Engineer | Land Development Engineering

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext. 216 | Fax: 613.254.5867 The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Wu, John < <u>John.Wu@ottawa.ca</u>>
Sent: Monday, February 28, 2022 12:03 PM

To: Anthony Mestwarp < a.mestwarp@novatech-eng.com>

Subject: RE: 112191 - Storm Boundary Conditions Request - 1040 Somerset Street

Yes, I can confirm that.

From: Anthony Mestwarp <<u>a.mestwarp@novatech-eng.com</u>>

Sent: February 28, 2022 11:57 AM
To: Wu, John < John. Wu@ottawa.ca>

Cc: Vahid Mehdipour <<u>v.mehdipour@novatech-eng.com</u>>; Curtis Ferguson <<u>c.ferguson@novatech-</u>

eng.com>

Subject: 112191 - Storm Boundary Conditions Request - 1040 Somerset Street

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Can you please confirm the HGL within the 1350mm storm within the Breezehill Avenue N. right of way fronting the 1040 Somerset Street W. site.

Based on the SWM site criteria:

- Coefficient of 0.50
- Time of concentration of 20 min
- 5-year stm

The maximum run-off from the site will be restricted to 13.2L/s.

The approximate storm service connection location is noted on the attached image for reference.

Please let me know if you require any further information.

Thanks,

Anthony Mestwarp, P.Eng., Project Engineer | Land Development Engineering

NOVATECH Engineers, Planners & Landscape Architects

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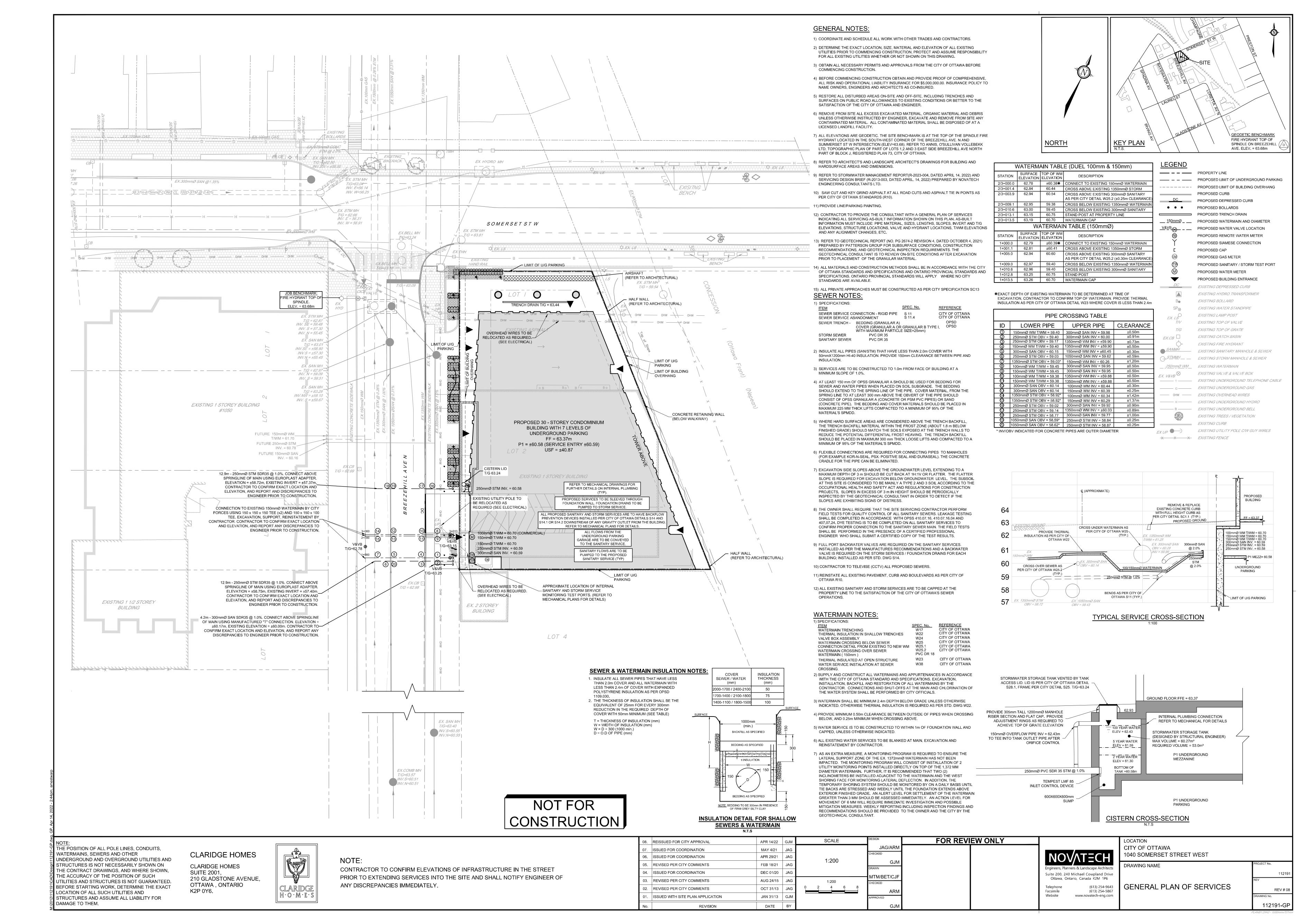
ATTACHED DRAWINGS

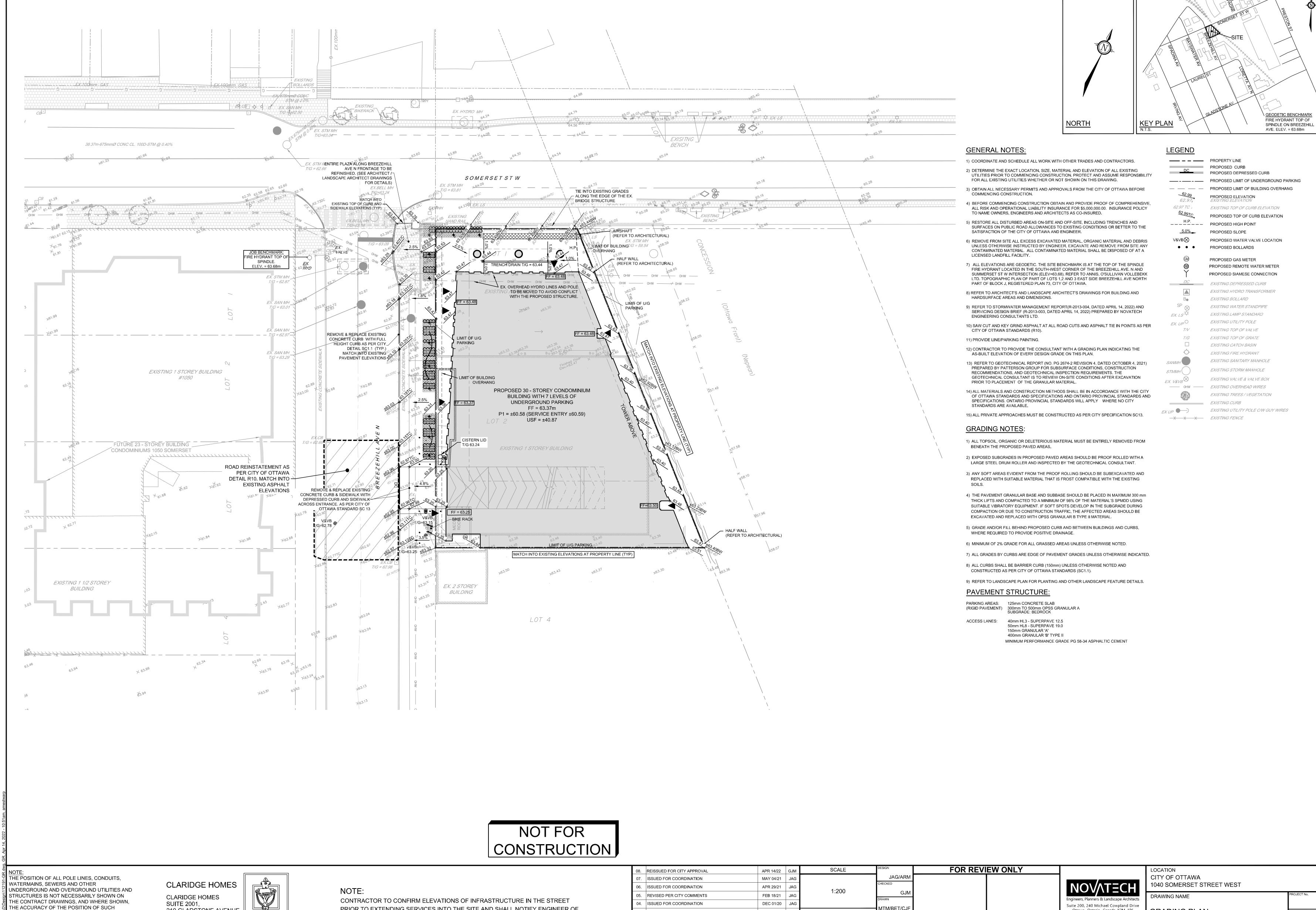
112191-GP GENERAL PLAN OF SERVICES

112191-GR GRADING PLAN

112191-ESC Erosion and Sediment Control Plan

112191-SWM Stormwater Management Plan





UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

210 GLADSTONE AVENUE OTTAWA , ONTARIO K2P 0Y6.

CLARIDGE $H \cdot O \cdot M \cdot E \cdot S$

PRIOR TO EXTENDING SERVICES INTO THE SITE AND SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IMMEDIATELY.

MTM/BET/CJF 3. REVISED PER CITY COMMENTS AUG 24/15 JA 1:200 2. REVISED PER CITY COMMENTS OCT 31/13 JA ARM ISSUED WITH SITE PLAN APPLICATION JAN 31/13 GJW DATE BY REVISION

Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6 Telephone (613) 254-9643 Facsimile (613) 254-5867

www.novatech-eng.com

Website

GRADING PLAN

REV # 08 112191-GR

