Site Servicing & Stormwater Management Report

Building Expansion 64 Cleopatra Drive

City of Ottawa

Old Project No. 15031-1 New Project No. 19028-1

Prepared for:

2336925 Ontario Inc. 64 Cleopatra Drive Ottawa, Ontario K2G 0B4

June 26, 2015 Rev. June 19, 2020 Rev. June 7, 2021



• Site Servicing and Grading Plan

• Erosion and Sediment Control Plan

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(Dwg. 003-19028-SG1)

(Dwg. 004-19028-SC1)

1.0 INTRODUCTION

The subject site is located on the west side of Cleopatra Drive (#64), at the west end of Caesar Avenue in the City of Ottawa. The site currently has an existing two (2) storey industrial/office building and an asphalt parking lot. The existing building is to remain and shall be expanded with a proposed one (1) storey addition of approximately 4,185sq.ft. The existing asphalt parking lot (i.e. along the east and south sides of the existing building) will be reconfigured; therefore, increasing the landscaped area which in return reduces the imperviousness of the site. The north side of the existing building (i.e. gravel parking lot / storage area), will be removed to accommodate the new building addition. The overall site has an area of 0.28ha, however only 0.066ha is being redeveloped.

The existing building is serviced from a 10 inch (250mm) diameter sanitary sewer along Cleopatra Drive. As for water, the site is currently serviced by an on-site well. Approximately half the site drains overland to the existing hydro easement along the west side of the site, where as the other half of the site drains overland easterly ultimately draining to catchbasins along Cleopatra Drive.

A site investigation was undertaken to verify the existing site conditions and to establish the impact the new building expansion would have on the site. The City of Ottawa was contacted to verify existing civil infrastructure services along Cleopatra Drive and information on the servicing requirements for this project.

2.0 WATER SERVICE

As mentioned above, the building is currently serviced by an on-site water well. The intent is to decommission the existing water well and connect to the existing 12 inch (300mm) diameter watermain along Cleopatra Drive. Using the City of Ottawa guideline of 35 cu.m/ha.d for light industrial use, the anticipated average daily demand for the site has been calculated at 9.8cu.m/day or 0.116L/s. Therefore, the anticipated peak hourly rate shall be in the magnitude of 0.21L/s, which is minimal. No fire sprinklers are anticipated at this time.

The anticipated fire flow (based on the Fire Underwriters Survey) was calculated to be 7,000 L/min or **116.7** L/s. A detailed calculation can be seen in Appendix B.

The following are boundary conditions provided by the City (i.e. Justin Armstrong on May 30, 2019 – correspondence attached in Appendix B):

Minimum HGL = 126.5m Maximum HGL = 134.0m Max Day + Fire Flow = 126.0m Based on a ground elevation of 90.15m:

Minimum HGL = 51.7 psi Maximum HGL = 62.4 psi Max Day + Fire Flow = 51.0 psi

Ainley has reviewed the results of the City of Ottawa hydraulic analysis and find that they meet the requirements set out by the ODG for water distribution, as seen below:

- Normal operating pressure ranges between 50 psi and 80 psi under a condition of maximum daily flow.
- Under maximum hourly demand conditions, the pressures are not less than 40 psi.
- During periods of maximum day and fire flow demand, the residual pressure at any point in the distribution system shall not be less than 20 psi.
- The maximum pressure at any point in the distribution system in occupied areas outside of the public right-of-way shall not exceed 80 psi.
- The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 100 psi.

With regards to the existing well, we note that the UTM coordinates from the well record don't seem to line up correctly with the property; however, does state that the well is in fact on the property (please see attached Well Record in Appendix C). Decommissioning must be done by a licensed well contractor in accordance with Wells Regulation (R.R.O 1990 Regulation 903) as per Ontario Water Resource Act.

3.0 SANITARY SEWER SERVICE

As mentioned above, the site is currently serviced from a 10 inch (250mm) diameter sanitary sewer along Cleopatra Drive. Under City of Ottawa policy, secondary sanitary service connections to a property are not generally permitted. The intent is to provide a new sanitary service (decommissioning and/or removing the old service), and branching off a secondary line (on-site) towards the new building addition at a proposed Sanitary MH. The secondary line to the new building addition is intended to service the floor drains only, therefore no additional flows are expected.

4.0 DRAINAGE & STORM WATER SYSTEM

The stormwater management facility for this development has been designed to attenuate the release of stormwater runoff from the redeveloped area of the site to a rate not greater than the 5 year pre-development runoff rate of 9.5 l/s, (i.e. see correspondence from the City of Ottawa in Appendix A confirming the quantity control criteria for the site).

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We note that in pre-development conditions, the entire proposed redeveloped area drains towards the west side of the site (i.e. not towards the municipal right of way). Due to the existing topography, a small section of the proposed redeveloped area (drainage area A-3) will drain uncontrolled to the west side of the site. As well, a small section of the proposed redeveloped area (drainage area A-2) will drain uncontrolled to the east towards Cleopatra Drive. Therefore, over controlling drainage area A-1 (proposed building and grassed area to the north) will be required.

This has been achieved by installing an inlet control device (i.e. Vertical Hydrovex Valve – see Appendix C) in the ditch inlet catchbasin and by providing the appropriate ponding volume within the swale/ditch system, (refer to Detention Area Volume Calculations in Appendix C). We note that the emergency overland flow high point will be set at 0.18m below the finished floor elevation. Flows from A1 will be directed to the ditch storage via eavestrough. Due to the nature of the site (industrial area, no basement, overland flow not to right of way), there is limited opportunity for property damage to occur should this overland flow fail to convey the flow from this small area. Nonetheless, this overland flow highpoint elevation meets the requirements of the current City of Ottawa OSDG for the elevations of overland flow high points compared to finished floors.

Since the proposed redeveloped area is primarily building (which typically doesn't require stormwater quality treatment – considered clean water); that the proposed reconfiguration of the existing parking lot does not change the existing drainage pattern and increases the landscaped area which in return reduces the imperviousness of the site; and, that the existing industrial subdivision drains to an existing stormwater management pond/facility, we note that no stormwater quality requirements are required for this site (i.e. see correspondence from the RVCA in Appendix C).

Also, based on our pre-consultation with Ministry of the Environment (i.e. Charlie Primeau – Water Compliance Supervisor) on June 5, 2019 and subsequent correspondence with the City of Ottawa, an Industrial ECA (direct submission) will be required for this project. Correspondence from the MOE, City of Ottawa and a copy of the existing C of A for the downstream storm water pond received by the City of Ottawa has been included in the report for reference in Appendix C.

4.1 5 YEAR PRE-DEVELOPMENT FLOW

 $Q = R \times A \times I \times N$

Redeveloped Site Area A = 0.066 hectares Runoff Coefficient R = 0.50 (gravel) Time of Concentration Tc = 10 min

SITE SERVICING & STORMWATER MANAGEMENT REPORT BUILDING EXPANSION – 64 CLEOPATRA DRIVE

Rainfall Intensity (5yr) I = 104.19 mm/hr

5 year Pre-Dev. Flow: $Q = 0.50 \times 0.066 \times 104.19 \times 2.778$

 $Q = 9.5 \, l/s$

4.2 100 YEAR POST-DEVELOPMENT FLOW (AREA A1)

 $Q = R \times A \times I \times N$

Redeveloped Site Area A = 0.054 hectares

Runoff Coefficient $R = (0.014 \times 0.20) + (0.039 \times 0.90)$

0.053

R = 0.72

Time of Concentration Tc = 10 min

Rainfall Intensity (100yr) I = 178.56 mm/hr

100 year Post-Dev. Flow: $Q = 0.72 \times 0.053 \times 178.56 \times 2.778$

Q = 18.9 l/s

4.3 100 YEAR POST-DEVELOPMENT FLOW (AREA A2)

 $Q = R \times A \times I \times N$

Redeveloped Site Area A = 0.005 hectares

Runoff Coefficient $R = (0.003 \times 0.20) + (0.002 \times 0.90)$

0.005

R = 0.43

Time of Concentration Tc = 10 min

Rainfall Intensity (100yr) I = 178.56 mm/hr

100 year Post-Dev. Flow: $Q = 0.43 \times 0.005 \times 178.56 \times 2.778$

O = 1.1 l/s

4.4 100 YEAR POST-DEVELOPMENT FLOW (AREA A3)

 $Q = R \times A \times I \times N$

Redeveloped Site Area A = 0.008 hectares Runoff Coefficient R = 0.90 (asphalt)

Time of Concentration Tc = 10 min

Rainfall Intensity (100yr) I = 178.56 mm/hr

100 year Post-Dev. Flow: $Q = 0.90 \times 0.008 \times 178.56 \times 2.778$

 $Q = 3.6 \, l/s$

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4.5 STORAGE REQUIREMENTS

As noted previously, the site has been designed to limit the rate of runoff (for the redeveloped portion of the site) to the 5 year pre-development release rate of 9.5 l/s for rainfall events up to and including the 100 year post-development event.

Over controlling Area A1 to 4.8 l/s [9.5 l/s (pre-dev.) - 1.2 l/s (Area A2) - 3.4 (area A3)] ensures the 5 year pre-development release rate of 9.5 l/s is achieved.

Storage volume requirements were determined by applying the 5-year and 100-year rainfall intensity values at 10-minute intervals until a peak storage volume was attained.

Return	Time	Intensity	Flow	Controlled	Net Runoff To	Storage Req'd
Period	(min)	(mm/hr)	Q (L/s)	Release	Be Stored (L/s)	m3
	10	104.19	11.00	4.8	6.2	3.7
5 Year	20	70.25	7.42	4.8	2.6	3.1
	30	53.93	5.69	4.8	0.9	1.6
	40	44.18	4.67	4.8	-0.1	-0.3
	50	37.65	3.98	4.8	-0.8	-2.5
	10	178.56	18.85	4.8	14.0	8.4
100 Year	20	119.95	12.66	4.8	7.9	9.4
	30	91.87	9.70	4.8	4.9	8.8
	40	75.15	7.93	4.8	3.1	7.5
	50	63.95	6.75	4.8	1.9	5.8

Therefore, the resulting 100-year release rate from the redeveloped portion of the site is less/equal to the allowable release rate of 9.5 l/s by providing 9.4 cu.m of storage within the swale/ditch system on the north side of the proposed building expansion.

5.0 EROSION AND SEDIMENT CONTROL

Erosion and sediment control measures shall be implemented during construction to minimize the migration of sediments from the proposed construction. To accomplish this task, items such as silt fences, and geo-textile membranes shall be installed to capture sediment before it leaves the construction areas. In addition, all stockpiles shall be covered and located away from waterways and exposed areas and shall be vegetated as soon as possible. During construction, all erosion control features shall be maintained and repaired as necessary and adjacent roadways kept free of debris and sediment as required. Drawing 19028 - SC1 shows detailed requirements for erosion and sediment control.

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6.0 CONCLUSION

- 1. The intent is to decommission the existing water well and connect to the existing 300mm diameter watermain along Cleopatra Drive. The anticipated peak hourly rate has been calculated at 0.21 L/s. The fire flow water demand has been calculated at 116.7 L/s. No fire sprinklers are anticipated at this time.
- 2. No increase in sanitary flows is anticipated.
- 3. The redeveloped portion of the site has been designed to limit the rate of runoff to the 5 year pre-development release rate for rainfall events up to and including the 100 year post-development event.
- 4. Erosion and sediment control has been incorporated into the contract drawings.
- 5. An Industrial ECA (direction submission) will be required for this project and will be completed / submitted separately.

We trust that this report meets all of your requirements. Should you have any questions or require further clarification, please do not hesitate to contact our office.

Sincerely,

Prepared by:

Reviewed by:

Ainley Graham and Associates Ltd.

Ainley Graham and Associates Ltd.



<u>Limited Licensee</u>

Name: J.W.XU

Number: 100171806 Category: CIVIL: see limitation

Limitations:

This licence is subject to the limitations as detailed

on the certificate.

Association of Professional Engineers of Ontario

Jiawu Xu, LEL, C.E.T. Project Manager Guy Ste-Croix, LEL, C.E.T.,PMP Branch Manager

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APPENDIX A

- Table 1: Storm Water Management Summary Sheet
- Table 2 4: Storage Tables
- City of Ottawa Correspondence re. stromwater quantity requirements.

AINLEY Project: 19028-64 CLEOPATRA DRIVE

Location: 64 CLEOPATRA DRIVE
Client: City Of Ottawa

Table 1. Stormwater Management Summary Sheet

												Head on			Head
Sub	Sub	C =	C =	C =	Composite	Outlet	Controlled	Top of	Ponding	Invert or	Pipe dia	Orifice	Diameter	Hydrovex	on Hydrovex
Area	Area	0.2	0.6	0.9	'C'	Location	Release	Grate	Depth	Pan Elev.	(if plug type)	(if plug)	of Orifice	Model	
I.D.	(ha)						(L/s)	(m)	(m)	(m)	(mm)	(m)	(mm)		
A1	0.053	0.014	0.000	0.039	0.72	DICB 1	4.8	89.53	0.62	88.10	250	1.93	41	75VHV-1	2.05
A2	0.006	0.004	0.000	0.002	0.43	FREE FLOW	1.2								
A3	0.008	0.000	0.000	0.008	0.90	FREE FLOW	3.4								

Table 2-Storage Requirements for AREA 1

Area 0.05 hectares

Runoff Coefficient = 0.72 post development

Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Controlled Release	Net Runoff To Be Stored (L/s)	Storage Req'd m3
	10	104.19	11.00	4.8	6.2	3.7
5 Year	20	70.25	7.42	4.8	2.6	3.1
	30	53.93	5.69	4.8	0.9	1.6
	40	44.18	4.67	4.8	-0.1	-0.3
	50	37.65	3.98	4.8	-0.8	-2.5
	10	178.56	18.85	4.8	14.0	8.4
100 Year	20	119.95	12.66	4.8	7.9	9.4
	30	91.87	9.70	4.8	4.9	8.8
	40	75.15	7.93	4.8	3.1	7.5
	50	63.95	6.75	4.8	1.9	5.8

Table 3-Storage Requirements for AREA 2

Area 0.01 hectares

Runoff Coefficient = 0.43 post development

Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Controlled Release	Net Runoff To Be Stored (L/s)	Storage Req'd m3
	10	104.19	0.68	1.2	-0.5	-0.3
5 Year	20	70.25	0.46	1.2	-0.7	-0.9
	30	53.93	0.35	1.2	-0.8	-1.5
	40	44.18	0.29	1.2	-0.9	-2.2
	50	37.65	0.25	1.2	-1.0	-2.9
	10	178.56	1.17	1.2	0.0	0.0
100 Year	20	119.95	0.79	1.2	-0.4	-0.5
	30	91.87	0.60	1.2	-0.6	-1.1
	40	75.15	0.49	1.2	-0.7	-1.7
	50	63.95	0.42	1.2	-0.8	-2.3

Area 0.01 hectares

Runoff Coefficient = 0.90 post development

Return Period	Time (min)	Intensity (mm/hr)	Flow Q (L/s)	Controlled Release	Net Runoff To Be Stored (L/s)	Storage Req'd m3
	10	104.19	1.98	3.4	-1.4	-0.9
5 Year	20	70.25	1.34	3.4	-2.1	-2.5
	30	53.93	1.03	3.4	-2.4	-4.3
	40	44.18	0.84	3.4	-2.6	-6.1
	50	37.65	0.72	3.4	-2.7	-8.1
	10	178.56	3.40	3.4	0.0	0.0
100 Year	20	119.95	2.28	3.4	-1.1	-1.3
	30	91.87	1.75	3.4	-1.7	-3.0
	40	75.15	1.43	3.4	-2.0	-4.7
	50	63.95	1.22	3.4	-2.2	-6.6





From: Kuruvilla, Santhosh < Santhosh.Kuruvilla@ottawa.ca>

Sent: February 22, 2021 12:01 PM

To: Guy Ste-Croix

Cc: mdblakely_jon@bellnet.ca

Subject: FW: D07-12-16-0029 - 64 Cleopatra Drive - 2nd Review Comments
Attachments: D07-12-16-0029 - Cleopatra Drive - 2nd Review Comments.pdf

Good morning Guy,

I just want to let you know that the quantity control criteria that you used in your report for this site is the correct one (i.e. 5-year flow based on the lesser of C=0.5 or existing). This is related to comment #19 in the attached comment.

If you have any questions, please let me know.

Thanks,

Santhosh

From: Gorni, Colette <colette.gorni@ottawa.ca>

Sent: February 19, 2021 4:56 PM To: mdblakely_jon@bellnet.ca

Cc: Kuruvilla, Santhosh <Santhosh.Kuruvilla@ottawa.ca>; Gorni, Colette <colette.gorni@ottawa.ca>

Subject: D07-12-16-0029 - 64 Cleopatra Drive - 2nd Review Comments

Good afternoon Jonathan,

Please find attached the City's comments on the engineering portion of the second submission of the Site Plan Control application for 64 Cleopatra Drive.

Kind regards,

Colette Gorni

Planner I | Urbaniste I
Development Review West | Services d'examen demandes d'aménagements Ouest
Planning, Infrastucture and Economic Development Department
City of Ottawa | Ville d'Ottawa
613-580-2424, ext./poste 21239

Colette.Gorni@ottawa.ca

During this period of uncertainty surrounding COVID-19, we are following best practices recommended to minimize the risk of exposure, while ensuring that service to our clients remains as uninterrupted as possible. For the most part I am working from home and will respond to emails at my earliest opportunity.

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APPENDIX B

- FUS Calculations
- Boundary Conditions incl. Correspondence



MEMORANDUM

Ainley Graham & Associates Limited
2724 Fenton Road, Gloucester, ON K1T 3T7
Tel: (613) 822-1052 • Fax: (613) 822-1573

Email: ottawa@ainleygroup.com

Justin Armstrong Copies to: Guy Ste-Croix

Branch Manager Ainley Group

From: Paul Le Blanc

City of Ottawa

EIT

To:

Ainley Group Date: 02/05/2019

Ref: 64 Cleopatra Drive (D07-12-16-0029) File: 19028-1

Attached: N/A

It is understood that FUS calculations are required to determine the boundary conditions

64 Cleopatra Fire Flow

Step	Parameter	Value	Note
Α	Type of Construction	Ordinary	Assumed Steel construction with
	C ((: :	C 10	combustible interior components
	Coefficient related to	C = 1.0	
	the type of		
	construction		
В	Ground Floor Area	N/A	Considering that some floors are of
			differing areas (mezzanine, attic storage
			of ex building), only the total floor area
			is given, as below
С	Height in Storeys	3 storeys in existing building,	
		1 storey plus Mezzanine in	
		proposed building	
	Total floor area:	A = 924.4 m2	
D	Fire flow:	$F = 220 \text{ C A}^{0.5}$	Flow rounded to the nearest 1000
		$= 220 \times 1.0 \times (924.4)^{0.5}$	L/min.
		= 6 688.87 L/min	
		= 7 000 L/min	
E	Occupancy class	Combustible	Given that this building will be used as
	Occupancy Charge	0%	a mechanical garage, and this use is not
	Occupancy increase or	7 000 L/min x 0% =	listed as either low or high hazard
	decrease	0 L/min	occupancy,

	Fire Flow	F = 11 000 L/min + - 0 L/min F = 7 000 L/min	No rounding applied.
F	Sprinkler Protection	Not Included	It is understood that this building will
	Sprinkler credit	0%	not be equipped with a sprinkler
	Sprinkler FF reduction	7 000 x 0% = 0 L/min	system.
G	West side exposure:	Exposing wall: Ordinary type	No exposed wall within 45 meters of
		construction	the proposed structure
		Distance: over 45 m	
	West side exposure	0%	
	charge:		
	North side exposure:	Exposing wall: Ordinary type	No exposed wall within 45 meters of
		construction	the proposed structure
		Distance: over 45 m	
	North side exposure	0%	
	charge:		
	East side exposure:	Exposing wall: Ordinary type	Exposed wall of Ordinary construction
		construction	
		Distance: 36.9 m	
	East side exposure	5%	
	charge:		
	South Side Exposure:	Exposing wall: Ordinary type	No exposed wall within 45 meters of
		construction	the proposed structure
		Distance: over 45 m	
	South side exposure	0%	
	charge:		
	Total exposure charge:	=0%+0%+5%+0%	The total exposure charge is below the
		= 5%	maximum value of 75%.
	Increase for exposures:	= 7 000 L/min x 5%	
		= 350 L/min	
Н	Fire flow	F = 7 000 L/min + 350 L/min	Flow rounded to nearest 1000 L/min.
		F = 7 350 L/min	
		= 7 000 L/min	
		= 116.7 L/s	

Thus, to conclude;

• Fire Flow for the proposed building (considering the existing building and proposed building as a single fire area) will be **116.7** L/s.

Prepared by:

AINLEY GRAHAM & ASSOCIATES LIMITED

Paul Le Blanc, EIT Engineer in Training

Ainley Group





From: Armstrong, Justin < justin.armstrong@ottawa.ca>

Sent: May 30, 2019 11:34 AM

To: 'Paul Le Blanc'

Cc: Guy Ste-Croix; xu@ainleygroup.com

Subject: RE: D07-12-16-0029 64 Cleopatra Drive - Water Boundary card request

Attachments: 64 Cleopatra May 2019.pdf

Hi Paul,

Please see boundary conditions below as well as assumed connection point in attached PDF.

The following are boundary conditions, HGL, for hydraulic analysis at 64 Cleopatra (zone 2C) assumed to be connected the 305mm on Cleopatra (see attached PDF for location).

Minimum HGL = 126.5m

Maximum HGL = 134.0m

MaxDay + FireFlow (117 L/s) = 126.0m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Regards,

Justin Armstrong, E.I.T.

Engineering Intern

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - West Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2400 ext./poste 21746, justin.armstrong@ottawa.ca

From: Paul Le Blanc < leblanc@ainleygroup.com>

Sent: May 28, 2019 3:08 PM

To: Armstrong, Justin < justin.armstrong@ottawa.ca>

Cc: 'Guy Ste-Croix' <stecroix@ainleygroup.com>; xu@ainleygroup.com

Subject: RE: D07-12-16-0029 64 Cleopatra Drive - Water Boundary card request





Cheers,

Paul Le Blanc, EIT Engineering Intern



Ainley Graham & Associates Limited 2724 Fenton Road

Ottawa, Ontario, K1T 3T7 Tel: (613) 822-1052 ext. 229

Fax: (613) 822-1573 Cell: (613) 222-4346 leblanc@ainleygroup.com

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From: Armstrong, Justin [mailto:justin.armstrong@ottawa.ca]

Sent: May-28-19 2:16 PM

To: 'Paul Le Blanc'

Cc: Guy Ste-Croix; xu@ainleygroup.com

Subject: RE: D07-12-16-0029 64 Cleopatra Drive - Water Boundary card request

Hi Paul,

I circulated your request to our boundary conditions group on Friday. I believe they have 10 business days to provide boundary conditions. They will sometimes provide them sooner depending on how backlogged they are. I will keep you informed.

Justin

Justin Armstrong, E.I.T.

Engineering Intern

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - West Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2400 ext./poste 21746, justin.armstrong@ottawa.ca

From: Paul Le Blanc < leblanc@ainleygroup.com>

Sent: May 28, 2019 12:37 PM

To: Armstrong, Justin < justin.armstrong@ottawa.ca>



Ste-Croix <stecroix@ainleygroup.com>; xu@ainleygroup.com

st: FW: D07-12-16-0029 64 Cleopatra Drive - Water Boundary card request



Hello Justin;

Just following up on the below boundary condition request. Please let me know if you have any questions.

Cheers,

Paul Le Blanc, EIT Engineering Intern



Ainley Graham & Associates Limited 2724 Fenton Road Ottawa, Ontario, K1T 3T7 Tel: (613) 822-1052 ext. 229

Fax: (613) 822-1573 Cell: (613) 222-4346 leblanc@ainleygroup.com

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From: Paul Le Blanc [mailto:leblanc@ainleygroup.com]

Sent: May-15-19 12:10 PM To: 'Armstrong, Justin'

Cc: 'Guy Ste-Croix'; 'mary.dickinson@ottawa.ca'; 'mark.young@ottawa.ca'

Subject: RE: D07-12-16-0029 64 Cleopatra Drive - Water Boundary card request

Hi Justin;

Thank you for responding to our request. I have included the information requested below;

 An image of the site indicating the location of the site's proposed water connection to the City main.

See attached servicing drawing snapshot

- Average daily demand 0.116 L/s
- Maximum daily demand 0.175 L/s
- Peak hour demand 0.210 L/s
- Required fire flow based on the FUS method (see attached Technical Bulletin ISTB-2018-02 which outlines the FUS method in detail)

116.67 L/s - See attached memorandum for calculations

Cheers,







Ainley Graham & Associates Limited 2724 Fenton Road

Ottawa, Ontario, K1T 3T7 Tel: (613) 822-1052 ext. 229

Fax: (613) 822-1573 Cell: (613) 222-4346 leblanc@ainleygroup.com

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From: Armstrong, Justin [mailto:justin.armstrong@ottawa.ca]

Sent: May-07-19 8:36 AM

To: Oram, Cody; 'Paul Le Blanc'; Young, Mark

Cc: Guy Ste-Croix

Subject: RE: D07-12-16-0029 64 Cleopatra Drive - Water Boundary card request

Hi Paul,

In order to have boundary conditions generated for the site, please provide the following:

- An image of the site indicating the location of the site's proposed water connection to the City main.
- Average daily demand
- Maximum daily demand
- Peak hour demand
- Required fire flow based on the FUS method (see attached Technical Bulletin ISTB-2018-02 which outlines the FUS method in detail)

Regards,

Justin Armstrong, E.I.T.

Engineering Intern

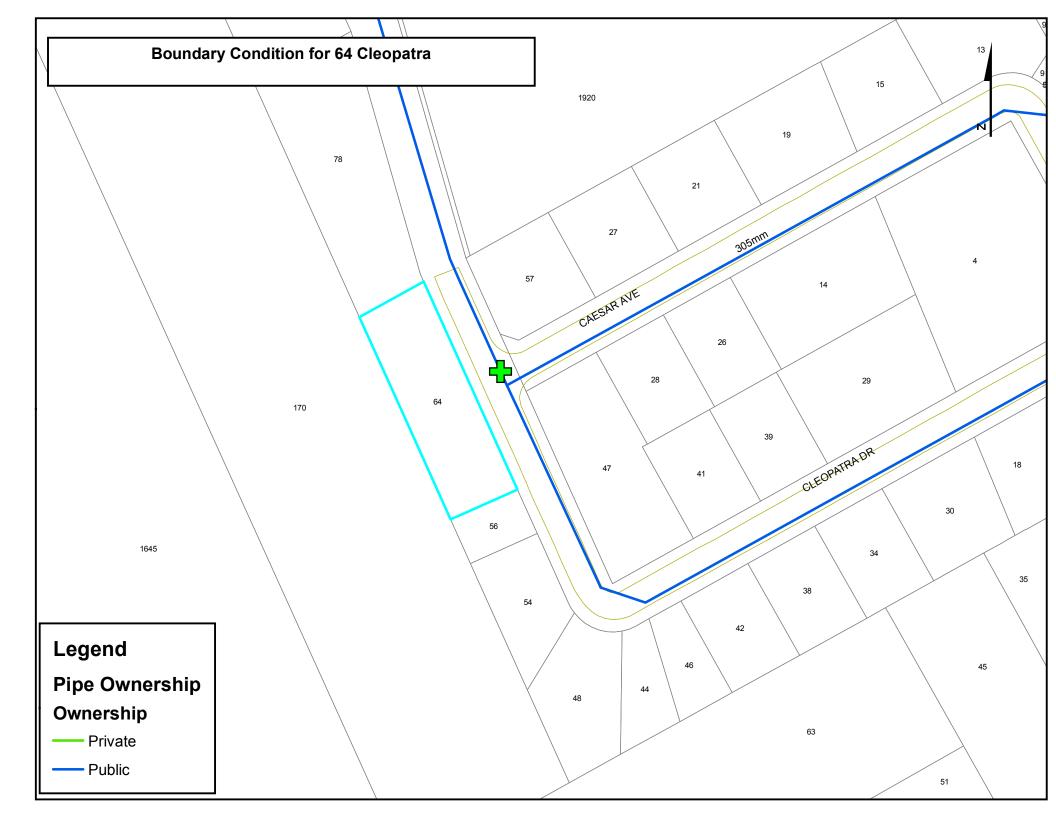
Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - West Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2400 ext./poste 21746, justin.armstrong@ottawa.ca



APPENDIX C

- Existing Well Record
- RVCA correspondence re. stormwater quality requirements
 City of Ottawa correspondence re. ECA

- MOE correspondence re. ECA
 Existing C of A Downstream Pond
 Hydrovex Sizing Form
 Proposed Detention Area Volume Calculations



31G5b

WATER RESOURCES
DIVISION

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(Signature of Licensèd Drilling or Boring Contractor)						
Form 7 15M-60-4138					Undang.	
OWRC COPY				6 323 416		





From: Jamie Batchelor < jamie.batchelor@rvca.ca>

Sent: May 31, 2021 4:05 PM

To: Guy Ste-Croix; Eric Lalande

Subject: RE: 64 Cleopatra Drive

Good Afternoon Guy,

The only thing I would add is that no additional water quality control is required because the repaved area is within the same footprint, with no new infrastructure proposed and is currently sheet drained to an existing grassed area at the rear of the property which currently provides some water quality treatment.

Jamie Batchelor, MCIP, RPP Planner, ext. 1191 Jamie.batchelor@rvca.ca



3889 Rideau Valley Drive PO Box 599, Manotick ON K4M 1A5 T 613-692-3571 | 1-800-267-3504 F 613-692-0831 | www.rvca.ca

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From: Guy Ste-Croix <stecroix@ainleygroup.com>

Sent: Monday, May 17, 2021 4:34 PM

To: Jamie Batchelor < jamie.batchelor@rvca.ca>; Eric Lalande < eric.lalande@rvca.ca>

Subject: RE: 64 Cleopatra Drive

Hi Jamie,

Per our meeting earlier today, we summarize the following:

- The building addition / expansion area at 64 Cleopatra Drive in Ottawa is considered 'clean water' (i.e. primarily building / roof and grass area); therefore, no stormwater quality is required.
- With regards to the reconfiguration of the existing parking lot, we note that a small section will be repaved and
 other sections of asphalt will be removed all together. The proposed reconfiguration of the existing parking lot
 does not change the existing drainage pattern and increases the landscaped area which in return reduces the
 imperviousness of the site; therefore, no stormwater quality is required.

We ask that you please acknowledge receipt and confirmation of the above noted statements, so this can be included in our SWM report.

Regards,





Minley

Ainley Graham & Associates Limited 2724 Fenton Road Ottawa, Ontario, K1T 3T7 Tel: (613) 822-1052 ext. 225

Fax: (613) 822-1573 Cell: (613) 858-8943 stecroix@ainleygroup.com

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From: Guy Ste-Croix

Sent: May 13, 2021 2:26 PM

To: Jamie Batchelor < jamie.batchelor@rvca.ca>; Eric Lalande < eric.lalande@rvca.ca>

Subject: RE: 64 Cleopatra Drive

Hi Jamie – yes, I'm available Monday May 17th at 10am.

Guy Ste-Croix, LEL, C.E.T., PMP Branch Manager



Ainley Graham & Associates Limited 2724 Fenton Road Ottawa, Ontario, K1T 3T7

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From: Jamie Batchelor [mailto:jamie.batchelor@rvca.ca]

Sent: May 13, 2021 2:01 PM

To: Guy Ste-Croix <stecroix@ainleygroup.com>; Eric Lalande <eric.lalande@rvca.ca>

Subject: RE: 64 Cleopatra Drive

Hi Guy,

I'm wondering if a call may be good to go over the proposal. Are you available on Monday May 17th at 10 am for a Teams call?





3889 Rideau Valley Drive PO Box 599, Manotick ON K4M 1A5 T 613-692-3571 | 1-800-267-3504 F 613-692-0831 | www.rvca.ca

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From: Guy Ste-Croix <stecroix@ainleygroup.com>

Sent: Wednesday, April 28, 2021 8:55 AM To: Eric Lalande < eric.lalande@rvca.ca >

Cc: Jamie Batchelor < jamie.batchelor@rvca.ca>

Subject: 64 Cleopatra Drive

Hi Eric,

We are working on a building expansion at 64 Cleopatra Drive in Ottawa. The site currently has an existing two (2) storey industrial/office building and an asphalt parking lot. The existing building is to remain and shall be expanded with a proposed one (1) storey addition of approximately 4,200sq.ft. The existing asphalt parking lot (i.e. along the east and south sides of the existing building) will be reconfigured; therefore, increasing the landscaped area which in return reduces the imperviousness of the site. The north side of the existing building (i.e. gravel parking lot / storage area), will be removed to accommodate the new building addition. The overall site has an area of 0.28ha, however only 0.066ha is being redeveloped. The redeveloped portion of the site will be controlled to the 5-year pre-development level. Since the proposed redeveloped area is primarily building (which typically doesn't require storm water quality treatment) and that the existing industrial subdivision drains to an existing stormwater management pond/facility, we note that it's our interpretation that no stormwater quality requirements will be required for this site. That being said, with regards to water quality control, the City of Ottawa has requested: "Please confirm what the Rideau Valley Conservation Authority (RVCA) has identified as the quality control criteria for this site." We ask that you please confirm by email so we can include RVCA correspondence in our report as requested by the City.

Regards,

Guy Ste-Croix, LEL, C.E.T., PMP Branch Manager

\linley

Ainley Graham & Associates Limited 2724 Fenton Road Ottawa, Ontario, K1T 3T7

Tel: (613) 822-1052 ext. 225

Fax: (613) 822-1573 Cell: (613) 858-8943 stecroix@ainleygroup.com

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From: Kuruvilla, Santhosh <Santhosh.Kuruvilla@ottawa.ca>

Sent: May 27, 2021 1:12 PM

To: Guy Ste-Croix

Subject: RE: D07-12-16-0029 - 64 Cleopatra Drive

Attachments: FW: 64 Cleopatra Drive, D07-12-16-0029, Zoning-IG, - Construction of an additional

building to an existing site

Hi Guy,

You will have to apply for an Industrial ECA and it will be a direct submission. Please see attached email for the reason.

Thanks.

Santhosh

From: Kuruvilla, Santhosh Sent: May 14, 2021 12:26 PM

To: Guy Ste-Croix <stecroix@ainleygroup.com> Subject: RE: D07-12-16-0029 - 64 Cleopatra Drive

Hi Guy,

I forwarded your information to our senior engineer and she requested the following additional information from you.

Hi Santhosh,

Please ask the applicant to answer each of these questions. Charles will send the request to Aziz once this is provided.

- 1) Applicants name and address:
- 2) The site name and location:
- 3) Who will own the sewer works:
- 4) Description of the proposed sewage works:
- 5) Confirmation that outside of the fact the proposed sewage works will remain privately owned, the works meet all the requirements of items 1) and 2) of schedule A of the City's ToR agreement:

Thanks,





'From: Guy Ste-Croix < stecroix@ainleygroup.com >

Sent: May 13, 2021 2:25 PM

To: Kuruvilla, Santhosh < <u>Santhosh.Kuruvilla@ottawa.ca</u>>

Subject: D07-12-16-0029 - 64 Cleopatra Drive

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Hi Santhosh,

Further to our discussion re. an ECA possibly not being required for the building expansion project at 64 Cleopatra Drive within the City of Ottawa, we provide the following description of what the building will be used for: The building will be operated by a Civil contractor and will be used for storage space (with a small amount of office space) and garage for heavy Civil equipment. We note that the redeveloped area is primarily roof which typically doesn't require stormwater quality treatment. Also, the building will not have a storm sewer connection. The building (Area A1 – see below) will be directed to the ditch storage area via eavestroughing. A catchbasin with an inlet control device will be installed in the grassed / ditched area to control post-development flows to pre-development levels. Therefore, it appears (based on our interpretation) that the expected stormwater effluent from the proposed building expansion area is non-industrial.





From: Primeau, Charlie (MECP) < Charlie.Primeau@ontario.ca>

Sent: June 5, 2019 7:07 AM

To: Paul Le Blanc Cc: Guy Ste-Croix

Subject: RE: D07-12-16-0029 64 Cleopatra Drive - 170 Cleopatra Pond ECA

Good morning Paul,

Thanks for providing a copy of the SWM facilty. That old CofA is pretty simple with no final effluent monitoring.

Based on my review, an ECA would be required for the above noted development.

Let me know if you required any further assistance.

Charlie Primeau
Water Compliance Supervisor
Ottawa District Office and Corwall Area Office
Ministry of the Environment, Conservation and Parks
2430 Don Reid Drive, Ottawa ON K1H 1E1

Tel: (613) 521 3450 or 1 800 860-2195

Cell: (613) 277-3727

From: Paul Le Blanc < leblanc@ainleygroup.com >

Sent: June 4, 2019 4:13 PM

To: Primeau, Charlie (MECP) < Charlie. Primeau@ontario.ca>

Cc: Guy Ste-Croix <stecroix@ainleygroup.com>

Subject: FW: D07-12-16-0029 64 Cleopatra Drive - 170 Cleopatra Pond ECA

Hi Charlie;

Thanks for your call this past Friday; I appreciate the assistance in keeping this project moving.

I've attached the existing C of A for the downstream storm water pond received from the City of Ottawa; please let me know if this is sufficient for the purpose of identifying the pond and any associated requirements regarding our project. The address is also in the attached email.

I've also confirmed that the building is operated by a civil contractor, and that it will be used for storage space and garage for heavy equipment.

As discussed, I'm also clarifying that the receiving storm water on-site storage area will only be receiving water from the proposed roof and the grass area adjacent to the proposed addition.

Please let me know if you require anything further to complete our pre-consultation meeting.

Cheers,

Paul Le Blanc, EIT Engineering Intern





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Fax: (613) 822-1573 Cell: (613) 222-4346 leblanc@ainleygroup.com

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From: Armstrong, Justin [mailto:justin.armstrong@ottawa.ca]

Sent: June-04-19 4:01 PM

To: 'Paul Le Blanc'

Subject: D07-12-16-0029 64 Cleopatra Drive - 170 Cleopatra Pond ECA

Hi Paul,

See attached for the C of A we have on file for the storm pond at 170 Cleopatra.

Regards,

Justin Armstrong, E.I.T.

Engineering Intern

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - West Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2400 ext./poste 21746, justin.armstrong@ottawa.ca

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Certificate No. . .

3-1153-32-836

RECEIVED L07-03-COFA (Vol. 1)

Certificate of Approval (Sewage)

JAN 20 1987 (SWF-1127)

GORE & STORRIE LTD. . CITY OF NEPEAN Whereas OTTAWA

XX

has applied in accordance with Section 24 of the Ontario Water Resources Act for approval of:-

Stormwater retention pond to be constructed in the Merivale Area (Lot 27, Concession 1) of the City of Nepean consisting of a fenced impoundment with a total surface area and effective storage volume of approximately 1.0 ha and 1.3 ha - m respectively at the 35.366 m water elevation; including outlet control structure with 200 mm diameter orifice and high level overflow weir at elevation 85.366 m, and inlet sewer and inlet structure; all in accordance with the plans and specifications prepared by Gore and Storrie Limited, Consulting Engineers, at a total estimated cost, including engineering and contingencies, of ONE MILLION ONE HUNDRED FIFTY EIGHT THOUSAND DOLLARS (\$1,158,000.00).

> THIS IS A TRUE COPY OF THE ORIGINAL CERTIFICATE MARLED

(SIGNED)

Now therefore this is to certify that after due enquiry the said proposed works have been approved under Section 24 of the Ontario Water Resources Act.

DATED AT TORONTO this

13th

day of

January

1983

Attn:-Mr. D.E. Hobbs, Clerk, City of Nepean

cc:-Mr. A.C. Bellinger, Comm. of Works

-Mr. R.E. Moore, MOE SE, Reg. Dir.

-Gore and Storrie Limited

Attn:-Mr. D.W. Smith, P. Eng.

11.11116



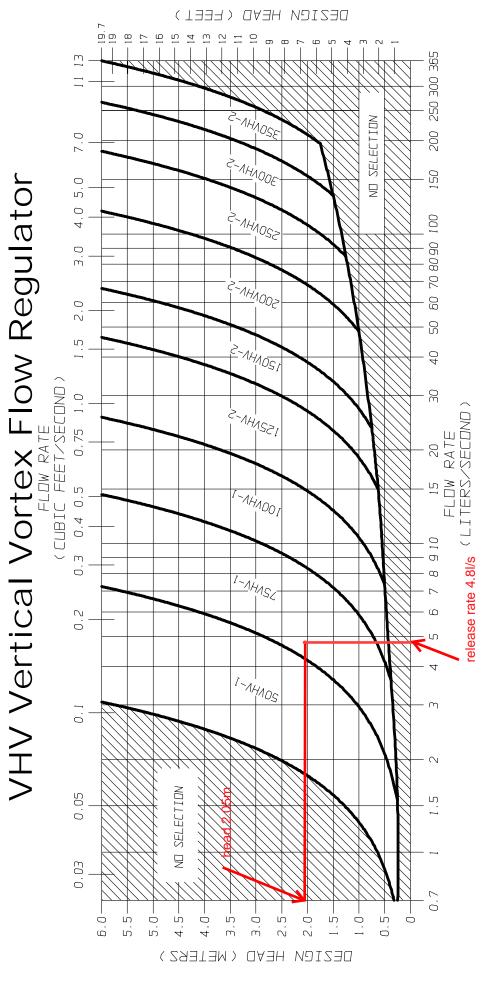


FIGURE 3 - VHV

JOHN MEUNIER

AINLEY Project: 19028-1 Location: 64 Cleopatra Drive

Proposed Detention Area (Pond) Volume Calculations

Pond #	Length	Average top width	Average bottom width	Side slope	Upstream depth	Downstream depth	Average Depth	Section area	Volume
	m	m	m		m	m	m	sq.m	cu.m
A1	24.50	2.23	0.00	2.00	0.12	0.57	0.35	0.38	9.42

APPENDIX D

•	Removal and Adjustment	(Dwg. 001-19028-REM1)
•	Storm Drainage Area Plan	(Dwg. 002-19028-STM1)
•	Site Servicing and Grading Plan	(Dwg. 003-19028-SG1)
•	Erosion and Sediment Control Plan	(Dwg. 004-19028-SC1)

