

Stormwater Management Report and Servicing Brief

Proposed 3-Storey Low-Rise Apartment 1435/1455 Morisset Avenue Ottawa, Ontario

Prepared for:

Firm Capital 163 Cartwright Avenue Toronto, ON M6A 1V5

Attention: Mr. Eddy Boudiwan

LRL File No.: 200572

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1 INTRODUCTION AND SITE DESCRIPTION

LRL Associates Ltd. was retained by Firm Capital to complete a Stormwater Management Analysis and Servicing Brief for a proposed three (3) storey residential development located at 1435/1455 Morisset Avenue in Ottawa, Ontario. The property is legally described as Lot 230-238, Registered Plan 327, City of Ottawa, refer to Survey included in Appendix F. The location of the proposed development can be viewed in Figure 1 below.



Figure 1: Arial View of Proposed Development

The proposed new apartment building will have a footprint of 578 m² and consist of 31 units (16x1bedroom and 15x2-bedroom apartments). The site will also encompass a paved parking area in the eastern portion of the lot. The proposed development will have one vehicular entrance with a depressed sidewalk on Morisset Avenue and one pedestrian main entrance via a pathway to Morisset Ave.

This report has been prepared in consideration of the terms and conditions noted above and with the civil drawings prepared for the proposed new development. Should there be any changes in the design features, which may relate to the stormwater management and servicing design considerations, LRL Associates Ltd. should be advised to review the report recommendations.

2 EXISTING SITE AND DRAINAGE DESCRIPTION

The portion of land to be developed has a rectangular shape with a frontage of approximately 61 m along Morisset Avenue and a depth of approximately 30.7 m. With these dimensions, the property has a surface area of approximately 0.19 ha. The property is surrounded with residential buildings to the west and north, parking lot in the east, and Morisset Ave in the south. Currently, the proposed site is a parking lot which is being used by the apartment building in the east of the site. The existing parking lot will be demolished prior to development.

Along the west face of the existing site is an impervious asphalt pavement driveway with a retaining wall. Along the north side of the existing site is a small landscape area separated by a

fence of the existing residential buildings in the north. Overland stormwater from the existing parking lot generally flow uncontrolled towards the low-lying area in the north and in the eastern portion of the parking lot which will eventually drains onto Morisset Ave.

3 SCOPE OF WORK

As per applicable guidelines, the scope of work includes the following:

Stormwater management

- Calculate the allowable stormwater release rate.
- Calculate the anticipated post-development stormwater release rates.
- Demonstrate how the target quantity objectives will be achieved.

Water services

- Calculate the expected water supply demand at average and peak conditions.
- Calculate the required fire flow as per the Fire Underwriters Survey (FUS) method.
- Confirm the adequacy of water supply and pressure during peak flow and fire flow.
- Describe the proposed water distribution network and connection to the existing system.

Sanitary services

- Describe the existing sanitary sewers available to receive wastewater from the building.
- Calculate peak flow rates from the development.
- Describe the proposed sanitary sewer system.
- Review impact of increased sanitary flow on downstream sanitary sewer.

4 REGULATORY APPROVALS

An MECP Environmental Compliance Approval is not expected to be required for installation of the proposed storm and sanitary sewers within the site. A Permit to Take Water is not anticipated to be required for pumping requirements for sewer installation. The Rideau Valley Conservation Authority will need to be consulted in order to obtain municipal approval for site development. No other approval requirements from other regulatory agencies are anticipated.

5 STORMWATER MANAGEMENT

5.1 Existing Stormwater Infrastructure

The topography of the site in pre-development conditions was reviewed to determine the direction of flow from overland runoff. In pre-development conditions, majority of the stormwater appears to flow uncontrolled overland towards Morisset Ave. The balance, at the rear of the property, appears to flow uncontrolled overland into the neighbouring property along the north property border. Refer to Appendix B for pre- and post-development watershed information.

As previously discussed, the south end of the subject property is bordered by Morisset Avenue. Hence, the proposed storm service connection will be located at Morisset Ave. At this location, a dedicated 375 mm diameter storm sewer, flowing east, is available on the north side of the street for a potential connection.

5.2 Design Criteria

The stormwater management criteria for this development are based on pre-consultation with City of Ottawa officials, the City of Ottawa Sewer Design Guidelines, 2012 (City Standards), as well as the Ministry of the Environment's Stormwater Management Planning and Design Manual, 2003 (SWMPD Manual).

5.2.1 Water Quality

Based on correspondence with Rideau Valley Conservation Authority (RVCA), the distance to the downstream outlet is greater than 2 km, therefore, the stormwater runoff from the site does not require any additional on-site water quality controls. Refer to Appendix B for the RVCA's opinion on water quality controls.

5.2.2 Water Quantity

All storm events up to and including the 100-year event will be controlled to the 2-year predevelopment level. The sites major overland flow route has been designed to ensure that storm events beyond the 100-year design storm can be safely conveyed overland towards the Morisset Ave Right of Way (ROW). The minor system (storm sewers) within the site are sized to convey the 2-year storm event flow from the site to the municipal storm sewer on Morisset Ave.

Based on the pre-development catchment area calculations, the pre-development weighted runoff was found to be C=0.9. Detailed calculations can be found in Appendix B.

Though post-development conditions are not introducing a large increase in pervious surfaces, quantity control will still have to be implemented. The allowable release rate is calculated using the maximum runoff coefficient of C=0.5, as per the City of Ottawa requirement. The 100-year and 2-year post development flows will be controlled to the 2-year allowable flow rate of the site. Events greater than the 100-year storm are permitted to flow overland to the ROW.

5.3 Method of Analysis

The Modified Rational Method has been used to calculate the runoff rate from the site, and to quantify the detention storage required for quantity control of the development. Refer to Appendix B for allowable release rate as well as storage calculations.

5.4 Allowable Release Rate

The maximum allowable release rate was calculated using the Rational Method for the 2-year pre-development runoff value. Runoff from post-development conditions must be controlled to the 2-year pre-development level using a maximum of C=0.5 for the runoff coefficient, for both minor and major storms (2-year up to 100-year storms), using a time of concentration not less than 10 minutes.

The Rational Method runoff coefficients (C) for each catchment have been calculated based on appropriately assigned coefficients weighted by area of land cover within the drainage area. A summary of catchment areas with calculated C values is included in Appendix B.

Below is the allowable release rate calculation (2-year storm, C=0.5)

 $Q = 2.78CIA = 2.78 \times 0.5 \times 76.80 \times 0.185 = 19.70 L/s$

i.e. the allowable release rate for this site is 19.70 L/s.

5.5 Proposed Stormwater Quantity Controls

The proposed stormwater management quantity control for this development will be accomplished using a flow restrictor in the storm sewer, as well as roof drains restricting the flow leaving the rooftop. Ponding required as a result of quantity control will be accomplished through a combination of rooftop storage and parking lot surface storage.

The proposed site storm sewer and stormwater management system are shown on Site Servicing Plan C-401 (Appendix E) and detailed calculations including the design sheet can be found in Appendix B.

The existing site is delineated by catchment EWS-01 (see drawing C701 in Appendix E) which currently drains uncontrolled off the site towards Morisset Ave ROW and towards the rear of the property.

The site has been analyzed and post-development watersheds have been allocated, see drawing C701 in Appendix E. A small portion of the exterior landscape boundary (along south property line) and ramps (at main entrance and along west property line) will drain off the property uncontrolled. Watersheds WS-05 & WS-07, consisting of mostly grass area and pavers will flow uncontrolled towards Morisset Ave ROW. Watersheds WS-06 & WS-08 consisting of ramp at main entrance and door exit area in the east, will be captured by trench drains and connected with STM sewer downstream of ICD, thus flows uncontrolled. Likewise, stormwater from watershed WS-09 will be captured by a trench drain and conveyed uncontrolled via a storm sewer to the existing sewer on Morisset Ave.

Grading proposed will provide positive overland drainage to the proposed storm water management systems.

Stormwater captured on the roof (WS-04) will be controlled by the roof drains, and conveyed to the storm sewer network, downstream of the ICD.

All overland stormwater captured using catch basins and trench drains will ultimately be conveyed, via underground storm sewers, to the City storm sewer running along Morisset Ave. Table 1 summarizes the drainage areas, calculations can be found in Appendix B.

Drainage Area Name	Area (ha)	Weighted Runoff Coefficient	100-year Weighted Runoff Coefficient (25% increase)
WS-01 (controlled)	0.015	0.32	0.40
WS-02 (controlled)	0.049	0.82	1.00
WS-03 (controlled)	0.042	0.88	1.00
WS-04 (controlled)	0.057	0.90	1.00
WS-05 (uncontrolled)	0.005	0.49	0.61
WS-06 (uncontrolled)	0.003	0.90	1.00
WS-07 (uncontrolled)	0.004	0.20	0.25
WS-08 (uncontrolled)	0.000	0.90	1.00
WS-09 (uncontrolled)	0.010	0.69	0.86
Total	0.185	0.79	0.99

Table 1: Post-Development Drainage Areas

Tables 2 summarizes the release rates and storage volumes required to meet the allowable release rate for 100-, 5- and 2-year flows.

Description	Area	Release Rate (L/s)		Storage Required (m ³)			Storage Provided (m ³)	
	(ha)	100 Yr	5 Yr	2 Yr	100 Yr	5 Yr	2 Yr	
WS-01, WS-02, WS-03 (Controlled)	0.106	9.02	9.02	9.02	26.63	9.30	4.37	29.34
WS-04 (Controlled)	0.057	2.53	2.53	2.53	23.39	9.05	6.04	26.82
Uncontrolled Area	0.022	8.15	3.81	2.81	N/A	N/A	N/A	N/A
Total	0.185	19.70	15.36	14.36	50.02	18.35	10.41	56.16

 Table 2: Stormwater Release Rate & Storage Volume Summary

The project runoff exceeding the allowable release rate will be stored on-site via surficial ponding and the building rooftop. The 100-year maximum ponding elevation and depths can be found on drawing "C601 – Stormwater Management Plan" of Appendix E.

5.5.1 Proposed Stormwater Quantity Controls

To throttle the 100-year storm flows, the stormwater will be controlled at CBMH03 using an Inlet Control Device (ICD), Hydrovex Vortex Flow Regulator 100VHV-1 (or approved equivalent). This ICD will control the stormwater runoff quantity during the storm events greater than 2-year. The site is graded to have a high-water level (HWL) of 97.82 m while providing storage as required with a maximum controlled release rate of 9.02 L/s (H=1.98 m) from the parking lot portion of the site. The storage created by this ICD can be seen on drawing C601 (Appendix E). Greater details on select ICD can be found in Appendix B.

5.5.2 Rooftop Storage & Release Rates

Rooftop detention of stormwater will be achieved through outlet control with the use of four (4) proposed roof drains. The flow through these drains is dependent on the height of water above the drains (Head) and the type & setting of the drain. The rooftop has been assumed to be low sloping, providing four separate ponding areas, each with a single roof drain restricting the discharge rate to 0.63 L/s per roof drain (at maximum head, during the 100-year storm event). This results in a total release rate of 2.53 L/s from the roof. Each roof control device is appropriately selected to provide a flow rate of 0.63 L/s at a maximum flow depth of 0.15 m.

Stormwater flow from the roof is proposed to be controlled via four Watts roof drains (or approved equivalent). This Watts drains is appropriately selected with an adjustable flow control set to closed which will allow a maximum discharge of 0.63 L/s at a maximum flow depth of 0.15 m. Refer to Appendix B for additional detail on rooftop release rate and storage calculations and drawing C601 (Appendix E) for the extent of roof top storage.

6 WATER SUPPLY AND FIRE PROTECTION

6.1 Existing Water Supply Services and Fire Hydrant Coverage

The subject property is located to the north of an existing 150 mm dia. water main running in the east-west direction on the south side of Morisset Avenue.

There are three (3) existing fire hydrant (FH) along Morisset Ave in proximity to the proposed site: one (1) within 76 m and two (2) additional within 305 m. A figure summarizing the locations of each FH can be seen in Appendix C.

6.2 Water Supply Demand and Fire Flow

According to the City of Ottawa Design Guidelines, the average daily water consumption rate for residential developments is 350 L/c/d.

The interior layout and architectural floor plans (Appendix F) have been reviewed, and it was determined that the building will house sixteen (16) 1-bedroom apartments and fifteen (15) 2-bedroom apartments. Based on the City of Ottawa Design guidelines for population projection,

this translates to approximately 53.9 residents. Table 3 summarizes the population count of the proposed development as interpreted using table 4.1 of the City of Ottawa Design Guidelines-Water Distribution, 2010.

Proposed Unit type	Persons Per Unit	Number of Units	Population
1-Bedroom	1.4	16	22.4
2-Bedroom	2.1	15	31.5
	Total	31	53.9

Table	3:	Residential	Po	pulation	Estimate
Table	υ.	Residential	10	pulation	Lotinate

The water supply requirements for the residential units in the proposed building are calculated using the following formula:

$$Q = (q \times P \times M)$$

Where,

q = Average water consumption (L/capita/day)

P = Design population (capita)

M = Peak factor

Calculated domestic water demands are summarized below, see Appendix C for calculation details.

- Average day demand = 0.22 L/s
- Maximum daily demand = 1.63 L/s
- Peak hour demand = 18.27 L/s

The fire flow requirements were estimated using the method prescribed by the Fire Underwriters' Survey (FUS). This method is based on the floor area of the building to be protected, the type and combustibility of the structural frame and the separation distances with adjoining building units.

Table 4 summarize the input parameters used for the FUS calculations. A minimum fire flow demand of 75 L/s was calculated, refer to Appendix C for the fire flow calculation sheets.

P t	Frame used	Combustibility of Contents	Presence	Separation Distance			
Parameter	for Building		of Sprinklers	North	East	South	West
Value according to	Ordinary	Limited	Yes	20.1-	>45	20.1-	3.1-10
FUS options	Construction	Combustible	165	30 m	m	30 m	m
Surcharge/reduction from base flow	1	-0.15%	-0.50%	10%	0%	10%	20%

 Table 4: Input Parameters for Fire Flow Calculations

The boundary condition received from the City specified available fire flow of 85 L/s at 20 psi. Therefore, to respect the boundary condition, following revisions have been made since the boundary condition request

- Type of construction Ordinary (previously wood frame)
- Fully supervised automatic sprinkler (previously no sprinkler)
- 2h fire-resistance assembly to compartmentalize the building into two (see the confirmation letter provided by Architect in Appendix C)

With the inclusion of above modification, it is our professional opinion that the revised fire flow demand will not exceed available fire flow.

6.3 Water Supply Servicing Design

Considering the presence of sprinkler system and a minimum recommended size, the proposed building will be serviced by a new 150 mm dia. water service which. The proposed service will provide water for both domestic and fire protection water demand (i.e. sprinkler system). The proposed service will be connected to the existing watermain on Morisset Ave to the south-east corner of the building. Refer to LRL drawing C401 for the layout of the proposed water services. Table 5 below summarizes the design criteria which have been respected during the design of the water service connections at this development.

Design Parameter	Value
Minimum cover	2.4 m
Desired pressure range under maximum daily flow condition	50 and 80 psi
Minimum pressure under peak hourly flow condition	40 psi
Minimum pressure under the maximum day plus fire flow condition	20 psi

Table 5: Water Supply Design Criteria

The boundary conditions provided by the City at the Morisset Avenue connection expressed as the level of hydraulic grade line (HGL) are summarized in Table 6.

Water Pressure at Morisset Ave				
	Press	ure		
	HGL (m)			
Minimum	Minimum 143.7		69.0	
Maximum	158.2	618.03	89.6	

Table 6. Boundary Conditions

The maximum pressure is estimated to be more than 80 psi, therefore a pressure check at completion of construction is required to determine if a pressure reducing valve is required as the residual pressure is not to exceed 80 psi.

6.4 Available Fire Flows

Table 7 shows location of existing fire hydrants from the proposed building. Based on Table 18.5.4.3 (Maximum Fire Flow Hydrant Capacity) of City of Ottawa Technical Bulletin ISTB-2018-02, the combined available fire flow of 11,356 L/min exceeds the required fire flow demand of 4500 L/min.

Building	Fire Flow Demand (L/min)	Fire Hydrants a Ave - Distance t	-	Available Combined Fire Flow (L/min)
		≤ 76	≥ 152 and ≤ 305	
Proposed 3- Storey Low- Rise Building	4500	1	2	(1x5678) + (2x2839) = 11,356

7 SANITARY SERVICE

7.1 Existing Sanitary Sewer Services

Existing infrastructure surrounding the proposed development were reviewed. It was determined that there is an existing 225 mm dia. sanitary sewer running east along Morisset Avenue. Since the existing site is a parking lot, pre-development conditions did not generate sanitary flow. The post-development total peak flow was calculated to be 0.76 L/s as a result of residential population and a small portion of infiltration. Refer to Appendix D for further information on the calculated sanitary flows.

The City of Ottawa official was contacted regarding the remaining capacity of sanitary sewer along Morisset Ave. Based on the information received from the City (see Appendix D), the anticipated sanitary peak flow will not exceed the allowable flow.

7.2 Sanitary Sewer Servicing Design

The parameters used to calculate the anticipated sanitary flows are; residential average population per unit of 1.4 persons for 1-bedroom units and 2.1 persons for 2-bedroom units, a residential peaking factor of 4.0 and an infiltration rate of 0.33 L/s/ha. Based on these parameters and the total site area of 0.185 ha, the total anticipated sanitary flow was estimated to 0.76 L/s. Refer to Appendix D for the site sanitary sewer design sheet.

The proposed new building will be serviced with a new 200 mm dia. sanitary service which will connect to the existing 225 mm dia. sanitary sewer along Morisset Ave. The new service will be located at the south-east corner of the proposed building. The proposed 200mm dia. PVC sanitary service will be installed at a recommended gradient of 2% as per the City of Ottawa Sewer Design Guidelines. Refer to LRL drawing C401 for the proposed sanitary servicing.

8 EROSION AND SEDIMENT CONTROL

During construction, erosion and sediment controls will be provided primarily via a sediment control fence to be erected along the perimeter of the site where runoff has the potential of leaving the site. Inlet sediment control devices are also to be provided in any catch basin and/or manholes in and around the site that may be impacted by the site construction. Construction and maintenance requirements for erosion and sediment controls are to comply with Ontario Provincial Standard Specification OPSS 577. Refer to LRL Associates drawing C101 for erosion and sediment control details.

9 CONCLUSION

This Stormwater Management Report and Servicing Brief for the proposed development at 1435/1455 Morisset Avenue presents the rationale and details for the servicing requirements for the subject property.

In accordance with the report objectives, the servicing requirements for the development are summarized below:

Stormwater Management

- The storm water release rates from the proposed development will meet the predevelopment allowable release rate of 19.70 L/s onto Morisset Ave.
- Stormwater quantity control objectives will be met through on-site stormwater ponding on the roof and parking lot surface storage.

Water Service

- The anticipated maximum hour demand of the proposed development, based on estimated population & calculated peak factor, is 18.27 L/s.
- The maximum required fire flow was calculated at 75 L/s using the FUS method.
- There are three (3) existing fire hydrant along Morisset Ave in proximity to the proposed building: one (1) within 76 m and two (2) additional within 305 m which will provide the required fire flow.
- The proposed building will be serviced by a new 150 mm dia. water service connection to the existing 150 mm dia. watermain on Morisset Ave.

Sanitary Service

- The anticipated sanitary peak flow from the proposed development is 0.76 L/s.
- The proposed building will be serviced by a new 200 mm dia. sanitary service connection to the existing 225 mm dia. sanitary sewer on Morisset Ave.

10 REPORT CONDITIONS AND LIMITATIONS

The report conclusions are applicable only to this specific project described in the preceding pages. Any changes, modifications or additions will require a subsequent review by LRL Associates Ltd. to ensure the compatibility with the recommendations contained in this document. If you have any questions or comments, please contact the undersigned.

Prepared by: LRL Associates Ltd.



Mohan Basnet, P.Eng. Civil Engineer

APPENDIX A

Pre-consultation/Correspondence



Planning, Infrastructure and Economic Development Department Services de la planification, de l'infrastructure et du développement économique

Site Plan Pre - Application Consultation Notes

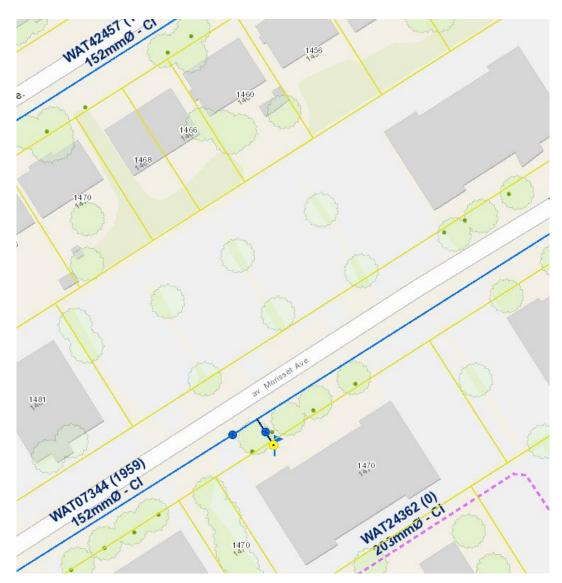
Infrastructure

Water

Please note that watermains in this area may experience lower than average pressures within the acceptable pressure range. Please submit your water boundary request to confirm these details.

Existing nearest public services:

• Morisset – 152mm Cast Iron



Watermain Frontage Fees to be paid (\$190.00 per metre)
Ves
No

- Service areas with a basic demand greater than 50 m³/day shall be connected with a minimum of two water services, separated by an isolation valve, to avoid creation of vulnerable service area.
- A District Metering Area Chamber (DMA) is required for services 150mm or greater in diameter.

Boundary conditions:

Civil consultant must request boundary conditions from the City's assigned Project Manager prior to first submission.

- Water boundary condition requests must include the location of the service(s) and the expected loads required by the proposed developments. Please provide all the following information:
 - Location of service(s)
 - Type of development and the amount of fire flow required (as per FUS, 1999).
 - Average daily demand: ____ l/s.
 - Maximum daily demand: ____l/s.
 - Maximum hourly daily demand: ____ l/s.
 - Fire protection (Fire demand, Hydrant Locations)
- A water meter sizing questionnaire [water card] will have to be completed prior to receiving a water permit (water card will be provided post approval)

Sanitary Sewer

Existing public services:

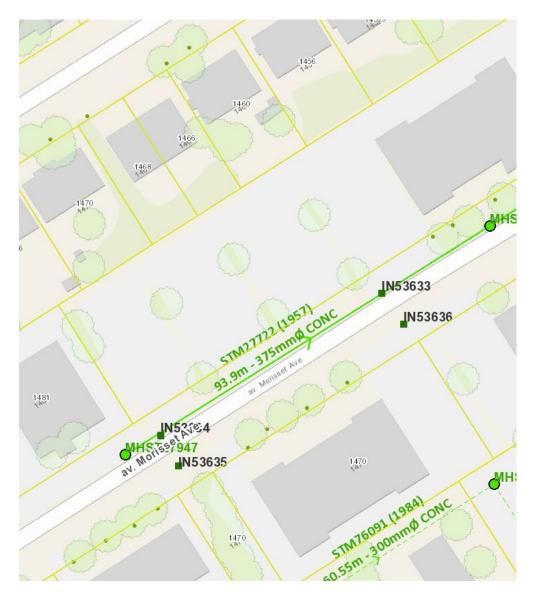
• Morisset – 225mm Conc.



- The designer should be aware there may be limited capacity in the downstream sanitary sewer system. The sanitary demand needs to be coordinated with the City Planning Dept. to determine if the existing sanitary sewer system has sufficient capacity to support the proposed rezoning. Provide sanitary demands to the City project manager for coordination.
- Any premise in which there is commercial or institutional food preparation shall install a grease and oil inceptor on all fixtures.

Existing public services:

• Morisset – 375mm Conc.



Storm Sewer Notes:

- For concrete sewer pipe, maintenance holes shall be installed when the service is greater than 50% of the diameter of the mainline concrete pipe
- The Environmental Site Assessment (ESA) may provide recommendations where site contamination may be present. The recommendations from the ESA need to be coordinated with the servicing report to ensure compliance with the Sewer Use By-Law.

Stormwater Management

Quantity Control:

- Stormwater quantity controls will depend upon which outlet is chosen for the site. Once an outlet
 has been chosen, please confirm with the Project Manager the SWM criteria for the property. For
 the nearby public services, the criteria is as follows
 - The pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less (§ 8.3.7.3).
 - A calculated time of concentration (Cannot be less than 10 minutes).
 - Flows to the storm sewer in excess of the 2-year storm release rate, up to and including the 100-year storm event, must be detained on site.

Ministry of Environment, Conservation and Parks (MECP)

All development applications should be considered for an Environmental Compliance Approval, under MECP regulations.

- a. The consultants determine if an approval for sewage works under Section 53 of OWRA is required and determines what type of application. The City's project manager may help confirm and coordinate with the MECP as required.
- b. The project will be either transfer of review (standard), transfer of review (additional), direct submission, or exempt as per O. Reg. 525/98.
- c. Pre-consultation is not required if applying for standard or additional works (Schedule A of the Agreement) under Transfer Review.
- d. Pre-consultation with local District office of MECP is recommended for direct submission.
- e. Consultant completes an MECP request form for a pre-consultation. Sends request to <u>moeccottawasewage@ontario.ca</u>
- f. ECA applications are required to be submitted online through the MECP portal. A business account required to submit ECA application. For more information visit <u>https://www.ontario.ca/page/environmental-compliance-approval</u>
- g. It is unclear if the proposed development will remain as one property. An ECA will be required where the stormwater management services more than one property parcel.

NOTE: Site Plan Approval, or Draft Approval, is required before any Ministry of the Environment and Climate Change (MOECC) application is sent

General Service Design Comments

- The City of Ottawa requests that all new services be located within the existing service trench to minimize necessary road cuts.
- Monitoring manholes should be located within the property near the property line in an accessible location to City forces and free from obstruction (i.e. not a parking).
- Where service length is greater than 30 m between the building and the first maintenance hole / connection, a cleanout is required.
- The City of Ottawa Standard Detail Drawings should be referenced where possible for all work within the Public Right-of-Way.
- The upstream and downstream manhole top of grate and invert elevations are required for all new sewer connections.
- Services crossing the existing watermain or sewers need to clearly provide the obvert/invert elevations to demonstration minimum separation distances. A watermain crossing table may be provided.

Other

Are there are Capital Works Projects scheduled that will impact the application?
Yes No

References and Resources

- As per section 53 of the Professional Engineers Act, O. Reg 941/40, R.S.O. 1990, all documents prepared by engineers must be signed and dated on the seal.
- All required plans are to be submitted on standard A1 size sheets (594mm x 841mm) sheets, utilizing a reasonable and appropriate metric scale as per City of Ottawa Servicing and Grading Plan Requirements: title blocks are to be placed on the right of the sheets and not along the bottom. Engineering plans may be combined, but the Site Plans must be provided separately. Plans shall include the survey monument used to confirm datum. Information shall be provided to enable a non-surveyor to locate the survey monument presented by the consultant.
- All required plans & reports are to be provided in *.pdf format (at application submission and for any, and all, re-submissions)
- Please find relevant City of Ottawa Links to Preparing Studies and Plans below: https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#standards-policies-and-guidelines
- To request City of Ottawa plan(s) or report information please contact the City of Ottawa Information Centre: <u>InformationCentre@ottawa.ca<mailto:InformationCentre@ottawa.ca</u>> (613) 580-2424 ext. 44455
- geoOttawa http://maps.ottawa.ca/geoOttawa/

SITE PLAN APPLICATION – Municipal servicing

For information on preparing required studies and plans refer to:

http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

S/A	Number of copies	ENGINEERING			Number of copies
S		1. Site Servicing Plan	2. Site Servicing Study	S	
S		3. Grade Control and Drainage Plan	 Geotechnical Study 	S	
		5. Composite Utility Plan	6. Groundwater Impact Study		
		7. Servicing Options Report	8. Wellhead Protection Study		
		9. Community Transportation Study			
		and/or Transportation Impact Study / Brief	10. Erosion and Sediment Control Plan / Brief	S	
S		11. Storm water Management Report	12. Hydro-geological and Terrain Analysis		
		13. Water main Analysis	14. <mark>Noise Study</mark>	S	
		15. Roadway Modification Design Plan	16. Confederation Line Proximity Study		

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, City Planning will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the City.

Notes:

4. Geotechnical Study / Slope Stability Study – required as per Official Plan section 4.8.3. All site plan applications need to demonstrate the soils are suitable for development. A Slope Stability Study may be required with unique circumstances (Schedule K or topography may define slope stability concerns).

10. Erosion and Sediment Control Plan - required with all site plan applications as per Official Plan section 4.7.3.

11. Stormwater Management Report/Brief - required with all site plan applications as per Official Plan section 4.7.6.

14. Noise and Vibration Study – a Noise Study will be required if the noise sensitive development is proposed within 250 metres of an existing or proposed highway or a railway right-of-way, or 100 metres of an arterial or collector roadway or rapid-transit corridor. A Vibration Study will be required if the proposed development is within 75 metres of either an existing or proposed railway ROW. A Noise Study may also be required if the proposed development is adjacent to an existing or proposed stationary noise source..

APPENDIX B

Stormwater Management Calculations

Mohan Basnet

From: Sent: To: Cc: Subject:	Jamie Batchelor <jamie.batchelor@rvca.ca> April 29, 2021 9:25 PM Mohan Basnet Eric Lalande RE: 200572-1435/1455 Morisset Ave Proposed Residential Development - Stormwater Quality Control</jamie.batchelor@rvca.ca>
Follow Up Flag:	Follow up
Flag Status:	Flagged

Good Evening Mohan,

Based on the distance of the downstream outlet being more than 2km from the site, the RVCA accepts that no additional on-site water quality treatment measures are required save and except best management practices. The RVCA strongly encourages that you explore the opportunity to incorporate LID measures in the stormwater management plan.

Jamie Batchelor, MCIP, RPP Planner, ext. 1191 Jamie.batchelor@rvca.ca



3889 Rideau Valley Drive PO Box 599, Manotick ON K4M 1A5 T 613-692-3571 | 1-800-267-3504 F 613-692-0831 | www.rvca.ca

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From: Mohan Basnet <mbasnet@lrl.ca> Sent: Wednesday, April 28, 2021 4:00 PM To: Jamie Batchelor <jamie.batchelor@rvca.ca> Subject: 200572-1435/1455 Morisset Ave Proposed Residential Development - Stormwater Quality Control

Hello Jamie,

We are working for a proposed 4-Storey Residential Development at 1435/1455 Morisset Ave, Ottawa. The existing site is a paved surface parking lot. The proposed development will also include paved parking lot providing 28 parking spots.

For stormwater management, we are planning to discharge controlled stormwater in a municipal storm sewer along Morisset Ave. Could you please advise stormwater quality control requirement for this site?

Thank you.

Mohan Mohan Basnet, P.Eng.



Civil Engineering Services

LRL Associates Ltd. 5430 Canotek Road Ottawa, Ontario K1J 9G2

T (613) 842-3434 or (877) 632-5664 ext 213

- F. (613) 842-4338
- mbasnet@lrl.ca Е
- W www.lrl.ca

Given the current COVID-19 situation, please be aware that LRL has implemented alternative working conditions for our team. Many of us have now transitioned to working from home; however, communication and workability remains one of our top priorities.

We will continue to be reachable by cell phone or by calling LRL at 613-842-3434 which will prompt you to enter the extension of the person you are trying to reach.

In addition, we will continue to have access to all e-mail correspondence and do our best to return all inquiries in a timely manner.

Mohan Basnet

From:	Baker, Adam <adam.baker@ottawa.ca></adam.baker@ottawa.ca>
Sent:	May 27, 2021 8:22 AM
To:	Mohan Basnet
Subject:	RE: 200572-1435 Morisset Ave-Proposed Service Connection/SWM criteria
Follow Up Flag:	Follow up
Flag Status:	Flagged

Hi Mohan,

Please accept the following information with regards to the servicing criteria -

- SWM criteria for an outlet on Morisset Avenue:
 - The 2-yr storm or 5-yr storm event using the IDF information derived from the Meteorological Services of Canada rainfall data, taken from the MacDonald Cartier Airport, collected 1966 to 1997.
 - The pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less (§ 8.3.7.3).
 - A calculated time of concentration (Cannot be less than 10 minutes).
 - Flows to the storm sewer in excess of the 2-year storm release rate, up to and including the 100-year storm event, must be detained on site.
 - Please confirm any stormwater quality requirements with the relevant Conservation Authority.
- Sanitary Constraints:
 - Please confirm if the anticipated sanitary flow provided (0.76 L/s) is peak or average. From an average flow of 0.76 L/s, a resulting peak sanitary flow of 3.24 L/s is acceptable for this system. If the peak sanitary flows will exceed this, please let me know and I will re-confirm for you.

Thank you, Adam

Adam Baker, EIT

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - South Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 26552, <u>Adam.Baker@ottawa.ca</u>

From: Mohan Basnet <mbasnet@lrl.ca> Sent: May 12, 2021 2:09 PM To: Baker, Adam <adam.baker@ottawa.ca> Subject: 200572-1435 Morisset Ave-Proposed Service Connection/SWM criteria CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Good afternoon Adam,

ı

ı

At LRL, we are working for STM and servicing study for a proposed 3-storey residential apartment building at 1435 Morisset Ave.

As a follow-up from the pre-consultation notes (Aug 28, 2020):

- Can you please advise SWM criteria of the subject site with a proposed STM outlet at Morisset Ave?
- Can you also advise remaining capacity of SAN sewer along Morisset Ave? Anticipated SAN flow from the proposed building is 0.76 L/s

For your reference I have also attached schematic of proposed service connections.

Please let me know if you have any question.



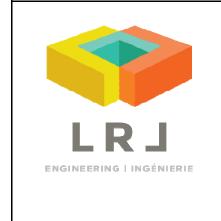
Given the current COVID-19 situation, please be aware that LRL has implemented alternative working conditions for our team. Many of us have now transitioned to working from home; however, communication and workability remains one of our top priorities.

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In addition, we will continue to have access to all e-mail correspondence and do our best to return all inquiries in a timely manner.

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LRL File No. 200572

Project: Proposed 3-Storey Low-rise Apartment
Location: 1435/1455 Morisset Ave, Ottawa
Date: June 16, 2021
Designed: Ayo Oni
Checked: Mohan Basnet

Dwg Reference: C701, C702

Pre-Development Catchments

Watershed	C = 0.20	C = 0.80	C = 0.90	Total Area (ha)	Combined C
EWS-01 (uncontrolled)	0.000	0.000	0.185	0.185	0.90
Total	0.000	0.000	0.185	0.185	0.90

Post-Development Catchments

Watershed	C = 0.20	C = 0.8	C = 0.90	Total Area (ha)	Combined C
WS-01 (controlled)	0.012	0.003	0.000	0.015	0.32
WS-02 (controlled)	0.005	0.000	0.044	0.049	0.82
WS-03 (controlled)	0.001	0.000	0.041	0.042	0.88
WS-04 (controlled)	0.000	0.000	0.057	0.057	0.90
WS-05 (uncontrolled)	0.003	0.000	0.002	0.005	0.49
WS-06 (uncontrolled)	0.000	0.000	0.003	0.003	0.90
WS-07 (uncontrolled)	0.004	0.000	0.000	0.004	0.20
WS-08 (uncontrolled)	0.000	0.000	0.0004	0.000	0.90
WS-09 (uncontrolled)	0.003	0.000	0.007	0.010	0.69
Total	0.028	0.003	0.154	0.185	0.79



Stormwater Management

STORM - 100 YEAR

Runoff Equation

- Q = 2.78CIA (L/s)
 - C = Runoff coefficient
 - I = Rainfall intensity (mm/hr) = A / $(Td + C)^{B}$
 - A = Area (ha) Td = Time of duration (min)

Pre-Development Catchments within Development Area

Total Area =	0.185	ha	∑R =	0.90
EWS-01 (uncontrolled)	0.185	ha	R =	0.90
Total Un-controlled =	0.185	ha	∑R =	0.90

Pre-Development Release Rate

IDF Curve Equations 100-Year, I ₁₀₀ = 1735.688 / (Td + 6.014) ^{0.820}	A	x = 1735.688	B = 0.820	C = 6.014
5-Year, I ₅ = 998.071 / (Td + 6.053) ^{0.814}	A	x = 998.071	B = 0.814	C = 6.053
2-Year, I ₂ = 732.951 / (Td + 6.199) ^{0.810}	A	x = 732.951	B = 0.810	C = 6.199
C = I ₁₀₀ = I ₅ = I ₂ = Td =	0.50 178.6 104.2 76.8 10	(max 0.5 as per C mm/hr mm/hr mm/hr min	ity Guidelines-Sewer)	
100-year Release Rate =	45.79	L/s		

L/s

L/s

L/s

 100-year Release Rate =
 45.79

 5-year Release Rate =
 26.72

 2-year Release Rate =
 19.70

 Allowable Release Rate =
 19.70

(2-year pre-development level)

Post-development Stormwater Management

					∑R _{2&5}	∑R ₁₀₀
	Total Site Area =	0.185	ha	∑R =	0.79	0.99
	WS-01 (controlled)	0.015	ha	R =	0.32	0.40
	WS-02 (controlled)	0.049	ha	R =	0.82	1.00
	WS-03 (controlled)	0.042	ha	R =	0.88	1.00
Roof	WS-04 (controlled)	0.057	ha	R =	0.90	1.00
	Total (Controlled)	0.163	ha	R =	0.82	1.00
	WS-05 (uncontrolled)	0.005	ha	R =	0.49	0.61
	WS-06 (uncontrolled)	0.003	ha	R =	0.90	1.00
	WS-07 (uncontrolled)	0.004	ha	R =	0.20	0.25
	WS-08 (uncontrolled)	0.000	ha	R =	0.90	1.00
	WS-09 (uncontrolled)	0.010	ha	R =	0.69	0.86
	Total (Uncontrolled)	0.022	ha	R =	0.59	0.74
	Total	0.185	ha	R =	0.79	0.99

100-Year Post-development Stormwater Management (WS-04 Roof)

				Controlled		
	Intensity	Controlled Runoff	Storage	Release Rate	Uncontrolled	Total Release
Time (min)	(mm/hr)	(L/s)	Volume (m ³)	(L/s)	Runoff (L/s)	Rate (L/s)
10	178.56	28.29	15.46	2.53	0.00	2.53
15	142.89	22.64	18.10	2.53	0.00	2.53
20	119.95	19.01	19.78	2.53	0.00	2.53
25	103.85	16.46	20.89	2.53	0.00	2.53
30	91.87	14.56	21.66	2.53	0.00	2.53
35	82.58	13.09	22.17	2.53	0.00	2.53
40	75.15	11.91	22.51	2.53	0.00	2.53
45	69.05	10.94	22.72	2.53	0.00	2.53
50	63.95	10.13	22.82	2.53	0.00	2.53



Stormwater Management

55	59.62	9.45	22.84	2.53	0.00	2.53
60	55.89	8.86	22.79	2.53	0.00	2.53
65	52.65	8.34	22.68	2.53	0.00	2.53
70	49.79	7.89	22.52	2.53	0.00	2.53
75	47.26	7.49	22.33	2.53	0.00	2.53
80	44.99	7.13	22.09	2.53	0.00	2.53
85	42.95	6.81	21.83	2.53	0.00	2.53
90	41.11	6.51	21.53	2.53	0.00	2.53
95	39.43	6.25	21.22	2.53	0.00	2.53
100	37.90	6.01	20.88	2.53	0.00	2.53
105	36.50	5.78	20.52	2.53	0.00	2.53
110	35.20	5.58	20.14	2.53	0.00	2.53
115	34.01	5.39	19.75	2.53	0.00	2.53
120	32.89	5.21	19.34	2.53	0.00	2.53

On-site stormwater detention		
Storage required = <u>Storage provided</u>	22.84	m ³
Avaiable roof surface for storage =	536.45	m ²
Maximum ponding depth =	150	mm
Avaialbe roof storage =	26.82	m³
Proposed roof drains: W	/ATTS adju	stable roof drain w/ weir opening-closed
Maximum flow per roof drain =	0.63	L/s
Number of roof drain =	4	
Total flow from roof drains =	2.53	L/s

100-Year Post-c	100-Year Post-development Stormwater Management (except WS-04 Roof)							
				Controlled				
	Intensity	Controlled Runoff	Storage	Release Rate	Uncontrolled	Total Release		
Time (min)	(mm/hr)	(L/s)	Volume (m ³)	(L/s)	Runoff (L/s)	Rate (L/s)		
10	178.56	48.08	23.44	9.02	8.15	17.17		
15	142.89	38.47	26.51	9.02	6.52	15.54		
20	119.95	32.30	27.94	9.02	5.48	14.49		
25	103.85	27.96	28.42	9.02	4.74	13.76		
30	91.87	24.74	28.29	9.02	4.19	13.21		
35	82.58	22.23	27.76	9.02	3.77	12.79		
40	75.15	20.23	26.92	9.02	3.43	12.45		
45	69.05	18.59	25.85	9.02	3.15	12.17		
50	63.95	17.22	24.61	9.02	2.92	11.94		
55	59.62	16.05	23.22	9.02	2.72	11.74		
60	55.89	15.05	21.72	9.02	2.55	11.57		
65	52.65	14.18	20.12	9.02	2.40	11.42		
70	49.79	13.41	18.43	9.02	2.27	11.29		
75	47.26	12.72	16.68	9.02	2.16	11.17		
80	44.99	12.11	14.86	9.02	2.05	11.07		
85	42.95	11.57	13.00	9.02	1.96	10.98		
90	41.11	11.07	11.08	9.02	1.88	10.89		
95	39.43	10.62	9.12	9.02	1.80	10.82		
100	37.90	10.21	7.13	9.02	1.73	10.75		
105	36.50	9.83	5.10	9.02	1.67	10.68		
110	35.20	9.48	3.04	9.02	1.61	10.62		
115	34.01	9.16	0.96	9.02	1.55	10.57		
120	32.89	8.86	0.00	9.02	1.50	10.52		

On-site stormwater detention

Storage required =28.42m³Storage provided =29.34m³

(Parking lot surface storage)



Stormwater Management

STORM - 5 YEAR

Runoff Equation

- Q = 2.78CIA (L/s)
- C = Runoff coefficient
- I = Rainfall intensity (mm/hr) = A / $(Td + C)^{B}$
- A = Area (ha)

Td = Time of duration (min) Pre-Development Catchments within Development Area

Total Area =	0.185	ha	∑R =	0.90
EWS-01 (uncontrolled)	0.185	ha	R =	0.90
Total Un-controlled =	0.185	ha	∑R =	0.90

Pre-Development Release Rate

IDF Curve Equations 100-Year, I ₁₀₀ = 1735.688 / (Td + 6.014) ^{0.820}	ļ	A = 1735.688	B = 0.820	C = 6.014
5-Year, $I_5 = 998.071 / (Td + 6.053)^{0.814}$	ļ	A = 998.071	B = 0.814	C = 6.053
2-Year, $I_2 = 732.951 / (Td + 6.199)^{0.810}$	ļ	A = 732.951	B = 0.810	C = 6.199
C = I ₁₀₀ =	0.50 178.6	(max 0.5 as per City mm/hr	Guidelines-Sewer)	
le =	104.2	mm/br		

I ₅ —	104.2	mm/m	
I ₂ =	76.8	mm/hr	
Td =	10	min	
100-year Release Rate =	45.79	L/s	
5-year Release Rate =	26.72	L/s	
2-year Release Rate =	19.70	L/s	
Allowable Release Rate =	19.70	L/s	
(2-year pre-development level)			

Post-development Stormwater Management

					∑ R 2&5	∑ R 100
	Total Site Area =	0.185	ha	∑R =	0.79	0.99
	WS-01 (controlled)	0.015	ha	R =	0.32	0.40
	WS-02 (controlled)	0.049	ha	R =	0.82	1.00
	WS-03 (controlled)	0.042	ha	R =	0.88	1.00
Roof	WS-04 (controlled)	0.057	ha	R =	0.90	1.00
	Total (Controlled)	0.163	ha	R =	0.82	1.00
	WS-05 (uncontrolled)	0.005	ha	R =	0.49	0.61
	WS-06 (uncontrolled)	0.003	ha	R =	0.90	1.00
	WS-07 (uncontrolled)	0.004	ha	R =	0.20	0.25
	WS-08 (uncontrolled)	0.000	ha	R =	0.90	1.00
	WS-09 (uncontrolled)	0.010	ha	R =	0.69	0.86
	Total (Uncontrolled)	0.022	ha	R =	0.59	0.74
	Total	0.185	ha	R =	0.79	0.99

5-Year Post-development Stormwater Management (WS-04 Roof)

				Controlled		
	Intensity	Controlled Runoff	Storage	Release Rate	Uncontrolled	Total Release
Time (min)	(mm/hr)	(L/s)	Volume (m ³)	(L/s)	Runoff (L/s)	Rate (L/s)
10	104.19	14.86	7.40	2.53	0.00	2.53
15	83.56	11.92	8.45	2.53	0.00	2.53
20	70.25	10.02	8.99	2.53	0.00	2.53
25	60.90	8.68	9.24	2.53	0.00	2.53
30	53.93	7.69	9.30	2.53	0.00	2.53
35	48.52	6.92	9.22	2.53	0.00	2.53
40	44.18	6.30	9.06	2.53	0.00	2.53
45	40.63	5.79	8.82	2.53	0.00	2.53
50	37.65	5.37	8.53	2.53	0.00	2.53



Stormwater Management

55	35.12	5.01	8.19	2.53	0.00	2.53
60	32.94	4.70	7.82	2.53	0.00	2.53
65	31.04	4.43	7.41	2.53	0.00	2.53
70	29.37	4.19	6.98	2.53	0.00	2.53
75	27.89	3.98	6.53	2.53	0.00	2.53
80	26.56	3.79	6.05	2.53	0.00	2.53
85	25.37	3.62	5.57	2.53	0.00	2.53
90	24.29	3.46	5.06	2.53	0.00	2.53
95	23.31	3.32	4.54	2.53	0.00	2.53
100	22.41	3.20	4.01	2.53	0.00	2.53
105	21.58	3.08	3.47	2.53	0.00	2.53
110	20.82	2.97	2.92	2.53	0.00	2.53
115	20.12	2.87	2.36	2.53	0.00	2.53
120	19.47	2.78	1.80	2.53	0.00	2.53

9.30

On-site stormwater detention

Storage required =

Storage provided Avaiable roof surface for storage =

536.45 150 Maximum ponding depth =

Avaialbe roof storage = 26.82

Proposed roof drains = Four (4) WATTS adjustable roof drain w/ weir opening-closed

m³

m²

mm

m³

5-Year Post-development Stormwater Management (except WS-04 Roof)

			Characte	Controlled			
	Intensity	Controlled Runoff	Storage	Release Rate	Uncontrolled	Total Release	
Time (min)	(mm/hr)	(L/s)	Volume (m ³)	(L/s)	Runoff (L/s)	Rate (L/s)	
10	104.19	23.79	8.86	9.02	3.81	12.82	
15	83.56	19.07	9.05	9.02	3.05	12.07	
20	70.25	16.04	8.42	9.02	2.57	11.58	
25	60.90	13.90	7.33	9.02	2.22	11.24	
30	53.93	12.31	5.93	9.02	1.97	10.99	
35	48.52	11.08	4.32	9.02	1.77	10.79	
40	44.18	10.09	2.57	9.02	1.61	10.63	
45	40.63	9.27	0.70	9.02	1.48	10.50	
50	37.65	8.60	0.00	9.02	1.38	10.39	
55	35.12	8.02	0.00	9.02	1.28	10.30	
60	32.94	7.52	0.00	9.02	1.20	10.22	
65	31.04	7.09	0.00	9.02	1.13	10.15	
70	29.37	6.71	0.00	9.02	1.07	10.09	
75	27.89	6.37	0.00	9.02	1.02	10.04	
80	26.56	6.06	0.00	9.02	0.97	9.99	
85	25.37	5.79	0.00	9.02	0.93	9.94	
90	24.29	5.54	0.00	9.02	0.89	9.90	
95	23.31	5.32	0.00	9.02	0.85	9.87	
100	22.41	5.12	0.00	9.02	0.82	9.84	
105	21.58	4.93	0.00	9.02	0.79	9.81	
110	20.82	4.75	0.00	9.02	0.76	9.78	
115	20.12	4.59	0.00	9.02	0.73	9.75	
120	19.47	4.44	0.00	9.02	0.71	9.73	

On-site stormwater detention

Storage required = Storage provided =

29.34

(Parking lot surface storage)



Stormwater Management

STORM - 2 YEAR

Runoff Equation

- Q = 2.78CIA (L/s)
- C = Runoff coefficient
- I = Rainfall intensity (mm/hr) = A / (Td + C)^B
- A = Area (ha)
- Td = Time of duration (min)

Pre-Development Catchments within Development Area

Total Area =	0.185	ha	∑R =	0.90
EWS-01 (uncontrolled)	0.185	ha	R =	0.90
Total Un-controlled =	0.185	ha	∑R =	0.90

Pre-Development Release Rate

IDF Curve Equations 100-Year, I ₁₀₀ = 1735.688 / (Td + 6.014) ^{0.820}	ŀ	A = 1735.688	B = 0.820	C = 6.014
5-Year, I ₅ = 998.071 / (Td + 6.053) ^{0.814}	ŀ	A = 998.071	B = 0.814	C = 6.053
2-Year, I ₂ = 732.951 / (Td + 6.199) ^{0.810}	ļ	A = 732.951	B = 0.810	C = 6.199
C =	0.50	(max 0.5 as	per City Guidelines-Sewer)	
I ₁₀₀ =	178.6	mm/hr		
I ₅ =	104.2	mm/hr		
I ₂ =	76.8	mm/hr		
Td =	10	min		
100-year Release Rate =	45.79	L/s		
5-year Release Rate =	26.72	L/s		

L/s

L/s

19.70

19.70

Post-development Stormwater Management

					∑R _{2&5}	∑R ₁₀₀
	Total Site Area =	0.185	ha	∑R =	0.79	0.99
	WS-01 (controlled)	0.015	ha	R =	0.32	0.40
	WS-02 (controlled)	0.049	ha	R =	0.82	1.00
	WS-03 (controlled)	0.042	ha	R =	0.88	1.00
Roof	WS-04 (controlled)	0.057	ha	R =	0.90	1.00
	Total (Controlled)	0.163	ha	R =	0.82	1.00
	WS-05 (uncontrolled)	0.005	ha	R =	0.49	0.61
	WS-06 (uncontrolled)	0.003	ha	R =	0.90	1.00
	WS-07 (uncontrolled)	0.004	ha	R =	0.20	0.25
	WS-08 (uncontrolled)	0.000	ha	R =	0.90	1.00
	WS-09 (uncontrolled)	0.010	ha	R =	0.69	0.86
	Total (Uncontrolled)	0.022	ha	R =	0.59	0.74
	Total	0.185	ha	R =	0.79	0.99

2-Year Post-development Stormwater Management (WS-04 Roof)

2-year Release Rate =

Allowable Release Rate =

(2-year pre-development level)

				Controlled		
	Intensity	Controlled Runoff	Storage	Release Rate	Uncontrolled	Total Release
Time (min)	(mm/hr)	(L/s)	Volume (m ³)	(L/s)	Runoff (L/s)	Rate (L/s)
10	76.81	10.95	5.06	2.53	0.00	2.53
15	61.77	8.81	5.65	2.53	0.00	2.53
20	52.03	7.42	5.87	2.53	0.00	2.53
25	45.17	6.44	5.87	2.53	0.00	2.53
30	40.04	5.71	5.73	2.53	0.00	2.53
35	36.06	5.14	5.49	2.53	0.00	2.53
40	32.86	4.69	5.18	2.53	0.00	2.53



LRL File No. 200572 **Project:** Proposed 3-Storey Low-rise Apartment **Location:** 1435/1455 Morisset Ave, Ottawa Date: June 16, 2021

Designed: M. Basnet Drawing Ref.: C601

Stormwater Management

45	30.24	4.31	4.82	2.53	0.00	2.53
50	28.04	4.00	4.42	2.53	0.00	2.53
55	26.17	3.73	3.98	2.53	0.00	2.53
60	24.56	3.50	3.51	2.53	0.00	2.53
65	23.15	3.30	3.02	2.53	0.00	2.53
70	21.91	3.13	2.51	2.53	0.00	2.53
75	20.81	2.97	1.99	2.53	0.00	2.53
80	19.83	2.83	1.45	2.53	0.00	2.53
85	18.94	2.70	0.89	2.53	0.00	2.53
90	18.14	2.59	0.33	2.53	0.00	2.53
95	17.41	2.48	0.00	2.53	0.00	2.53
100	16.75	2.39	0.00	2.53	0.00	2.53
105	16.13	2.30	0.00	2.53	0.00	2.53
110	15.57	2.22	0.00	2.53	0.00	2.53
115	15.05	2.15	0.00	2.53	0.00	2.53
120	14.56	2.08	0.00	2.53	0.00	2.53

On-site stormwater detention Storage required = 5.87

Storage provided #REF! Avaiable roof surface for storage = Maximum ponding depth = 150

#REF! Avaialbe roof storage =

Proposed roof drains = Four (4) WATTS adjustable roof drain w/ weir opening-closed

m³

 ${\rm m}^2$

mm

 m^3

2-Year Post-dev	elopment St	ormwater Manageme	ent (except WS	6-04 Roof)		
	Intensity	Controlled Runoff	Storage	Controlled Release Rate	Uncontrolled	Total Release
Time (min)	(mm/hr)	(L/s)	Volume (m ³)	(L/s)	Runoff (L/s)	Rate (L/s)
10	76.81	17.53	5.11	9.02	2.81	11.82
15	61.77	14.10	4.58	9.02	2.26	11.27
20	52.03	11.88	3.43	9.02	1.90	10.92
25	45.17	10.31	1.94	9.02	1.65	10.67
30	40.04	9.14	0.22	9.02	1.46	10.48
35	36.06	8.23	0.00	9.02	1.32	10.33
40	32.86	7.50	0.00	9.02	1.20	10.22
45	30.24	6.90	0.00	9.02	1.10	10.12
50	28.04	6.40	0.00	9.02	1.02	10.04
55	26.17	5.97	0.00	9.02	0.96	9.97
60	24.56	5.61	0.00	9.02	0.90	9.91
65	23.15	5.29	0.00	9.02	0.85	9.86
70	21.91	5.00	0.00	9.02	0.80	9.82
75	20.81	4.75	0.00	9.02	0.76	9.78
80	19.83	4.53	0.00	9.02	0.72	9.74
85	18.94	4.32	0.00	9.02	0.69	9.71
90	18.14	4.14	0.00	9.02	0.66	9.68
95	17.41	3.98	0.00	9.02	0.64	9.65
100	16.75	3.82	0.00	9.02	0.61	9.63
105	16.13	3.68	0.00	9.02	0.59	9.61
110	15.57	3.55	0.00	9.02	0.57	9.59
115	15.05	3.43	0.00	9.02	0.55	9.57
120	14.56	3.32	0.00	9.02	0.53	9.55

On-site stormwater detention

Storage required = 5.11 29.34 m³

m³

Storage provided =

(Parking lot surface storage)



Rational Method Q = 2.78CIA Q = Peak flow (L/s) A = Drainage area (ha) C = Runoff coefficient I = Rainfall intensity (mm/hr) Runoff coefficient (C) Grass = 0.2 Gravel = 0.8 Asphalt / rooftop = 0.9 <u>IDF curve</u> Ottawa Macdonald-Cartier International Airport Storm event: 2 Years <u>Intensity equation:</u> $I_2 = 732.951 / (Td + 6.199)^{0.810} (mm/hr)$

<u>Pipe Design Parameters</u> Minimum velocity = 0.80 m/s

Manning's "n" = 0.013

LC	OCATION			AREA (ha)			FLOW							STORM	SEWER				
WATERSHED / STREET	From MH	To MH	C = 0.20	C = 0.80	C = 0.90	Indiv. 2.78AC	Accum. 2.78AC	Time of Conc.	Rainfall Intensity	Peak Flow (Q)	Controlled Flow (Q)	Pipe Dia.	Туре	Slope	Length	Capacity Full (Q _{FULL})	Velocity Full	Time of Flow	Ratio Q /Q _{FULL}
								(min)	(mm/hr)	(L/s)	(L/s)	(mm)		(%)	(m)	(L/s)	(m/s)	(min)	
WS-01	CB01	CBMH02	0.012	0.003	0.000	0.01	0.01	10.00	76.81	1.01		250	PVC	0.50%	24.7	42.05	0.86	0.48	0.02
WS-02	CBMH02	CBMH03	0.005	0.000	0.044	0.11	0.13	10.48	75.01	9.46		250	PVC	0.50%	16.3	42.05	0.86	0.32	0.23
WS-04			0.000	0.000	0.057	0.14	0.14	10.00	76.81	10.95		150	PVC	2.00%	21.5	21.54	1.22	0.29	0.51
WS-06			0.000	0.000	0.003	0.01	0.01	10.00	76.81	0.58		150	PVC	2.00%	24.7	21.54	1.22	0.34	0.03
WS-08			0.000	0.000	0.000	0.00	0.00	10.00	76.81	0.08		150	PVC	2.00%	23.0	21.54	1.22	0.31	0.00
WS-03	*CBMH03	MH04	0.001	0.000	0.041	0.10	0.38	10.80	73.87	28.03	9.02	300	PVC	1.00%	13.0	96.70	1.37	0.16	0.29

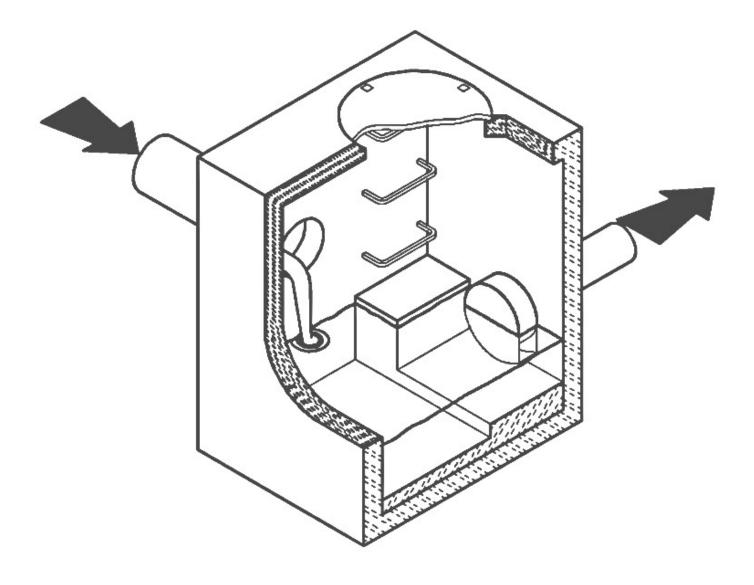
Note:

An ICD installed at CBMH03 will control flow at 9.02 L/s (H=1.98 m)

CSO/STORMWATER MANAGEMENT



[®] HYDROVEX[®] VHV / SVHV Vertical Vortex Flow Regulator



JOHN MEUNIER

HYDROVEX® VHV / SVHV VERTICAL VORTEX FLOW REGULATOR

APPLICATIONS

One of the major problems of urban wet weather flow management is the runoff generated after a heavy rainfall. During a storm, uncontrolled flows may overload the drainage system and cause flooding. Due to increased velocities, sewer pipe wear is increased dramatically and results in network deterioration. In a combined sewer system, the wastewater treatment plant may also experience significant increases in flows during storms, thereby losing its treatment efficiency.

A simple means of controlling excessive water runoff is by controlling excessive flows at their origin (manholes). John Meunier Inc. manufactures the HYDROVEX[®] VHV / SVHV line of vortex flow regulators to control stormwater flows in sewer networks, as well as manholes.

The vortex flow regulator design is based on the fluid mechanics principle of the forced vortex. This grants flow regulation without any moving parts, thus reducing maintenance. The operation of the regulator, depending on the upstream head and discharge, switches between orifice flow (gravity flow) and vortex flow. Although the concept is quite simple, over 12 years of research have been carried out in order to get a high performance.

The **HYDROVEX**[®] **VHV** / **SVHV** Vertical Vortex Flow Regulators (**refer to Figure 1**) are manufactured entirely of stainless steel, and consist of a hollow body (1) (in which flow control takes place) and an outlet orifice (7). Two rubber "O" rings (3) seal and retain the unit inside the outlet pipe. Two stainless steel retaining rings (4) are welded on the outlet sleeve to ensure that there is no shifting of the "O" rings during installation and use.

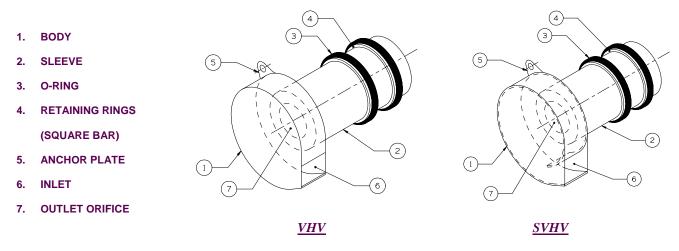


FIGURE 1: HYDROVEX[®] VHV-SVHV VERTICAL VORTREX FLOW REGULATORS

ADVANTAGES

- The **HYDROVEX[®] VHV** / **SVHV** line of flow regulators are manufactured entirely of stainless steel, making them durable and corrosion resistant.
- Having no moving parts, they require minimal maintenance.
- The geometry of the **HYDROVEX**[®] **VHV** / **SVHV** flow regulators allows a control equal to an orifice plate, having a cross section area 4 to 6 times smaller. This decreases the chance of blockage of the regulator, due to sediments and debris found in stormwater flows. **Figure 2** illustrates the comparison between a regulator model 100 SVHV-2 and an equivalent orifice plate. One can see that for the same height of water, the regulator controls a flow approximately four times smaller than an equivalent orifice plate.
- Installation of the **HYDROVEX**[®] **VHV** / **SVHV** flow regulators is quick and straightforward and is performed after all civil works are completed.
- Installation requires no special tools or equipment and may be carried out by any contractor.
- Installation may be carried out in existing structures.

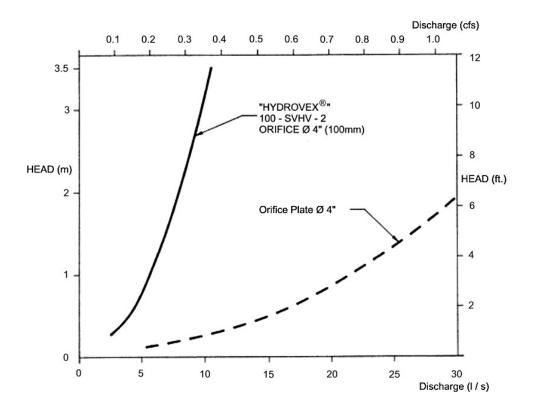


FIGURE 2: DISCHARGE CURVE SHOWING A HYDROVEX® FLOW REGULATOR VS AN ORIFICE PLATE

SELECTION

Selection of a VHV or SVHV regulator can be easily made using the selection charts found at the back of this brochure (see Figure 3). These charts are a graphical representation of the maximum upstream water pressure (head) and the maximum discharge at the manhole outlet. The maximum design head is the difference between the maximum upstream water level and the invert of the outlet pipe. All selections should be verified by John Meunier Inc. personnel prior to fabrication.

Example:

- 2m (6.56 ft.) ✓ Maximum design head
- ✓ Maximum discharge ✓ Using Figure 3 - VHV

6 L/s (0.2 cfs) model required is a 75 VHV-1

INSTALLATION REQUIREMENTS

All HYDROVEX[®] VHV / SVHV flow regulators can be installed in circular or square manholes. Figure 4 gives the various minimum dimensions required for a given regulator. It is imperative to respect the minimum clearances shown to ensure easy installation and proper functioning of the regulator.

SPECIFICATIONS

In order to specify a **HYDROVEX**[®] regulator, the following parameters must be defined:

- The model number (ex: 75-VHV-1)
- The diameter and type of outlet pipe (ex: 6" diam. SDR 35)
- The desired discharge (ex: 6 l/s or 0.21 CFS)
- The upstream head (ex: 2 m or 6.56 ft.) *
- The manhole diameter (ex: 36" diam.)
- The minimum clearance "H" (ex: 10 inches)
- The material type (ex: 304 s/s, 11 Ga. standard)
- * Upstream head is defined as the difference in elevation between the maximum upstream water level and the invert of the outlet pipe where the HYDROVEX[®] flow regulator is to be installed.

PLEASE NOTE THAT WHEN REQUESTING A PROPOSAL, WE SIMPLY REQUIRE THAT YOU PROVIDE US WITH THE FOLLOWING:

- project design flow rate
- > pressure head
- chamber's outlet pipe diameter and type



Typical VHV model in factory



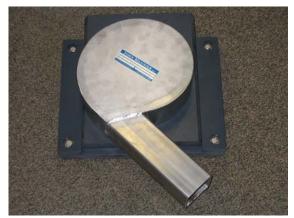
VHV-1-O (standard model with odour control inlet)



VHV with Gooseneck assembly in existing chamber without minimum release at the bottom



FV – SVHV (mounted on sliding plate)

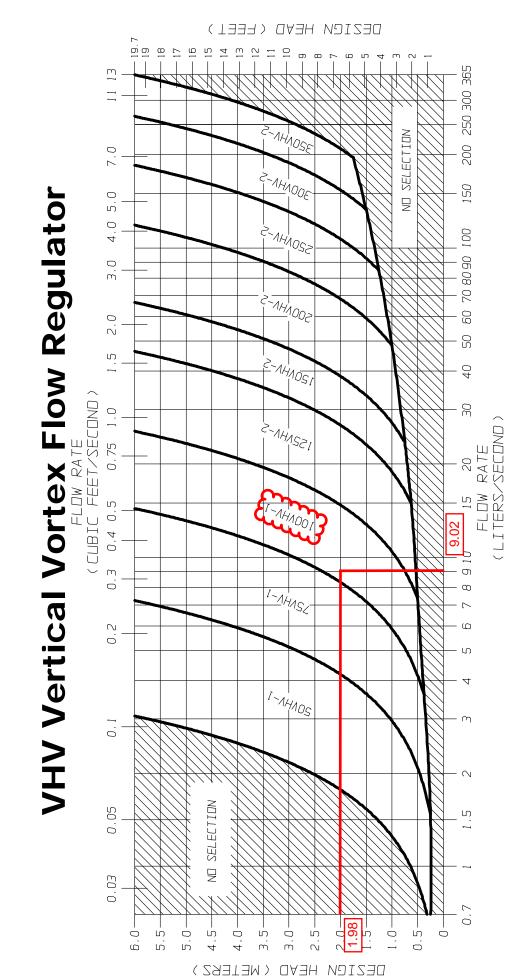


FV – *VHV-O* (mounted on sliding plate with odour control inlet)



VHV with air vent for minimal slopes

A[®] HYDROVEX[®]

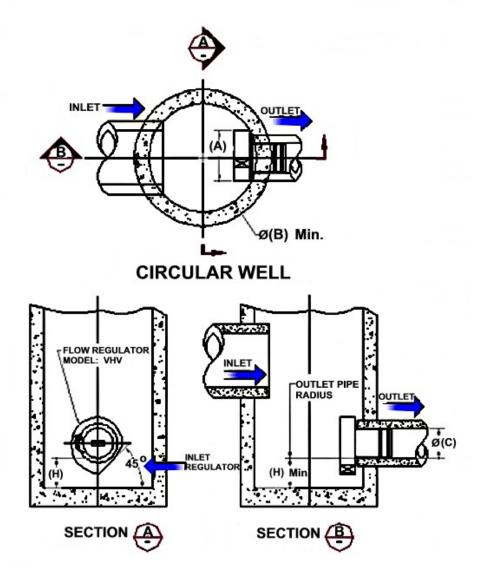


JOHN MEUNIER

FIGURE 3 - VHV

Model Number	Regulator Diameter		Minimum Manhole Diameter			n Outlet ameter	Minimum Clearance		
	A (mm)	A (in.)	B (mm)	B (in.)	C (mm)	C (in.)	H (mm)	H (in.)	
50VHV-1	150	6	600	24	150	6	150	6	
75VHV-1	250	10	600	24	150	6	150	6	
100VHV-1	325	13	900	36	150	6	200	8	
125VHV-2	275	11	900	36	150	6	200	8	
150VHV-2	350	14	900	36	150	6	225	9	
200VHV-2	450	18	1200	48	200	8	300	12	
250VHV-2	575	23	1200	48	250	10	350	14	
300VHV-2	675	27	1600	64	250	10	400	16	
350VHV-2	800	32	1800	72	300	12	500	20	

FLOW REGULATOR TYPICAL INSTALLATION IN CIRCULAR MANHOLE FIGURE 4 (MODEL VHV)



INSTALLATION

The installation of a HYDROVEX[®] regulator may be undertaken once the manhole and piping is in place. Installation consists of simply fitting the regulator into the outlet pipe of the manhole. John Meunier Inc. recommends the use of a lubricant on the outlet pipe, in order to facilitate the insertion and orientation of the flow controller.

MAINTENANCE

HYDROVEX[®] regulators are manufactured in such a way as to be maintenance free; however, a periodic inspection (every 3-6 months) is suggested in order to ensure that neither the inlet nor the outlet has become blocked with debris. The manhole should undergo periodically, particularly after major storms, inspection and cleaning as established by the municipality

GUARANTY

The HYDROVEX[®] line of VHV / SVHV regulators are guaranteed against both design and manufacturing defects for a period of 5 years. Should a unit be defective, John Meunier Inc. is solely responsible for either modification or replacement of the unit.

John Meunier Inc. ISO 9001 : 2008 Head Office 4105 Sartelon Saint-Laurent (Quebec) Canada H4S 2B3 Tel.: 514-334-7230 www.johnmeunier.com Fax: 514-334-5070 cso@johnmeunier.com

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USA Office 2209 Menlo Avenue Glenside, PA USA 19038 Tel.: 412-417-6614 www.johnmeunier.com





Adjustable Flow Control for Roof Drains

ADJUSTABLE ACCUTROL (for Large Sump Roof Drains only)

WATTS®

For more flexibility in controlling flow with heads deeper than 2", Watts Drainage offers the Adjustable Accutrol. The Adjustable Accutrol Weir is designed with a single parabolic opening that can be covered to restrict flow above 2" of head to less than 5 gpm per inch, up to 6" of head. To adjust the flow rate for depths over 2" of head, set the slot in the adjustable upper cone according to the flow rate required. Refer to Table 1 below. Note: Flow rates are directly proportional to the amount of weir opening that is exposed.

EXAMPLE:

For example, if the adjustable upper cone is set to cover 1/2 of the weir opening, flow rates above 2" of head will be restricted to 2-1/2 gpm per inch of head.

Therefore, at 3" of head, the flow rate through the Accutrol Weir that has 1/2 the slot exposed will be: [5 gpm(per inch of head) x 2 inches of head] + $2 \cdot 1/2$ gpm(for the third inch of head) = $12 \cdot 1/2$ gpm.

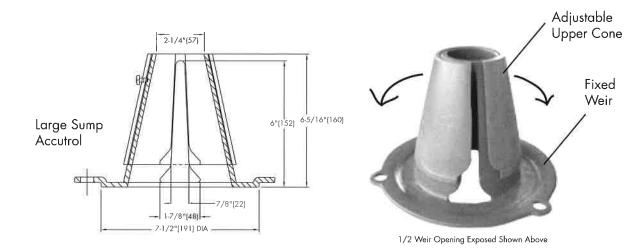


TABLE 1. Adjustable Accutrol Flow Rate Settings

		Head of Water								
Weir Opening Exposed	1"	2"	3"	4"	5"	6"				
Exposed	Flow Rate (gallons per minute)									
Fully Exposed	5	10	15	20	25	30				
3/4	5	10	13.75	17.5	21.25	25				
1/2	5	10	12.5	15	17.5	20				
1/4	5	10	11.25	12.5	13.75	15				
Closed	5	10	10	10	10	10				
Job Name Model No. Job Location Contractor Engineer Representative										
		WATTS Drainage reserves the rig any obligation to make similar ch representative for any clarification	anges and modifications to pro	ducts previously or subsequer						
Specification Drainage Products CANADA: 5435 North Service Road, Burlington, ON, L7L 5H7 TEL: 905-332-6718 TOLL-FREE: 1-888-208-8927 Website: www.wattscanada.ca										

ES-WD-RD-ACCUTROLADJ CANADA 0110

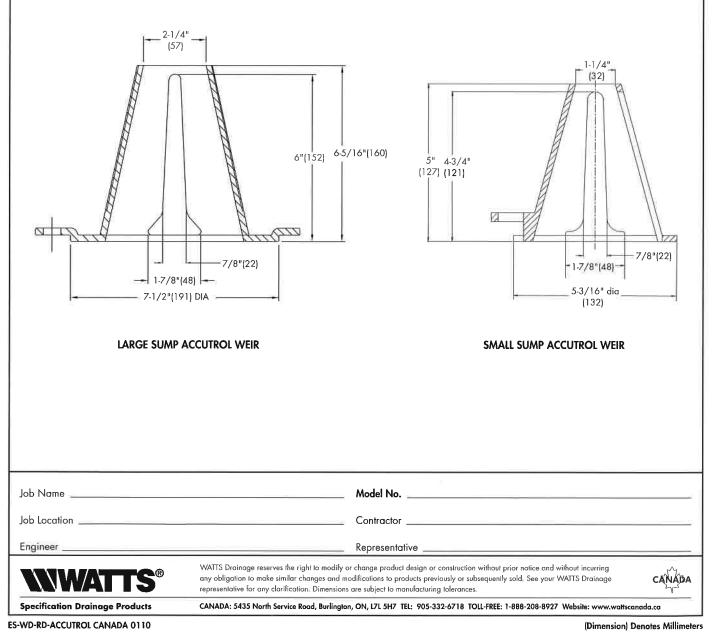
(Dimension) Denotes Millimeters



ACCUTROL WEIR FLOW CONTROL

SPECIFICATION: Watts Drainage Products epoxy coated cast iron Accutrol Weir is designed with parabolic openings which limit the flow of rain water off a roof. Each weir slot controls flow to 5 gpm per inch of head to a maximum of 30 gpm at 6" head(for large sump), 25 gpm at 5" head(for small sump) . The Accutrol Weir is secured to the flashing clamp of the roof drain. The Accutrol Weir is available with 1 to 4 slots for the large sump drain and up to 3 slots for the small sump drain.

For Large Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-100-A2" for two slot weir) For Small Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-200-A1" for one slot weir)



APPENDIX C

Water Supply Calculations



Water Supply Calculations LRL File No. 200572 Project: Proposed 3-Storey Low-Rise Apartment Location: 1435/1455 Morisset Ave, Ottawa, ON Date: June 16, 2021 Prepared by: M. Basnet

Residential Demand based on the City of Ottawa Design Guidelines-Water Distribution, 2010

Unit Type	Persons Per Uni	Number of Units	Population	
1-Bedroom Apartment	1.4	16	22.4	
2-Bedroom Apartment	2.1	15	31.5	
	Total	31	53.9	
				_
Average Water Consumption Rate	350	L/c/d		
Average Day Demand	18,865	L/d	0.22	L/s
Maximum Day Factor	7.5		(MOE Table 3-3))
Maximum Daily Demand	140,721	L/d	1.63	L/s
Peak Hour Factor	11.2		(MOE Table 3-3))
Maximum Hour Demand	1,578,953	L/d	18.27	L/s

Water Service Pipe Sizing

Q = VA	Where:
	V = velocity
	A = area of pipe
	Q = flow rate

Assuming a maximum velocity of 1.8m/s, the diameter of pipe is calculated as:

Minimum pipe diameter (d) = $(4Q/\pi V)^{1/2}$ = 0.114 m = 114 mm

Proposed pipe diameter (d) = 150 mm

(considering the presence of sprinkler system)



Fire Flow Calculations

LRL File No. 200572

LRL File No. 200572 Project: Proposed 3-Storey Low-Rise Apartment Location: 1435/1455 Morisset Ave, Ottawa Date: June 16, 2021 Method: Fire Underwriter's Survey (FUS) Prepared by: M. Basnet

Task	Term	Options	Multiplier	Choose: V	alue	Unit	Fire Flow
		Structural Framing	Material	÷			
		Wood Frame	1.5				
Choose frame	Coefficient C	Ordinary Construction	1.0				
	related to the type	Non-combustible construction	0.8	Ordinary Construction	1		
used for building	of construction	Fire resistive construction <2 hrs	0.7				
		Fire resistive construction >2 hrs	0.6				1
			a (A)				
		*Total area		1	,198	m ²	
Obtain fire flow	Poquirod fire flow	Equired fire flow Eiro Elow = $220 \times C \times A^{0.5}$					
before reductions	Required life now	Fire F	$10W = 220 \times C$	XA		L/!!!!!	7,613
		Reductions or surcharge due to fa	ctors affectin	g burning			
			-25%				
Choose		Limited combustible	-15%				
combustibility of	reduction or	Combustible	0%	Limited combustible -	15%	L/min	6,471
contents	surcharge	Free burning	15%				
			25%				
		Full automatic sprinklers	-30%	True -	30%		
Choose reduction for sprinklers	Sprinkler reduction	,	-10%	True -	10%	L/min	3,236
			10%	Truo	100/		
					10 /0		
Choose separation				÷		L/min	4,530
					10%		
			· ·	2070	10 / 0		
Obtain fire flow		•		fire flow rate (rounded to neares	st 100)	L/min	4,500
duration, and		/		75.0			
volume				Required duration of fi		hr	1.75
	Choose frame used for building Obtain fire flow before reductions Choose combustibility of contents Choose reduction for sprinklers Choose separation Obtain fire flow, duration, and	Choose frame used for building Coefficient C related to the type of construction Obtain fire flow before reductions Required fire flow Choose combustibility of contents Occupancy hazard reduction or surcharge Choose reduction for sprinklers Sprinkler reduction Choose separation Exposure distance between units Obtain fire flow, duration, and	Structural Framing Choose frame used for building Coefficient C related to the type of construction Wood Frame Ordinary Construction Non-combustible construction Fire resistive construction <2 hrs Fire resistive construction >2 hrs Obtain fire flow before reductions Required fire flow Fire resistive construction >2 hrs Obtain fire flow before reductions Required fire flow Fire resistive construction >2 hrs Obtain fire flow before reductions Required fire flow Fire F Choose combustibility of contents Occupancy hazard reduction or surcharge Non-combustible Combustible Choose reduction for sprinklers Occupancy hazard reduction or surcharge Non-combustible Combustible Choose reduction for sprinklers Sprinkler reduction for sprinklers Full automatic sprinklers Choose separation for sprinklers Sprinkler reduction between units North side East side Choose separation for fire flow, duration, and Exposure distance between units North side South side West side	Structural Framing Material Choose frame used for building Coefficient C related to the type of construction Wood Frame 1.5 Ordinary Construction 0.8 1.0 0.7 Fire resistive construction <2 hrs	Structural Framing Material Choose frame used for building Coefficient C related to the type of construction Wood Frame Drainary Construction 1.5 Ordinary Construction Ordinary Construction Structural Framing Material 1.0 0	Structural Framing Material Choose frame used for building Coefficient C related to the type of construction Structural Framing Material Mon-combustible construction 1.0 0 0 1 Non-combustible construction >2 hrs 0.7 0 0 1 Fire resistive construction >2 hrs 0.6 1 1 1 Obtain fire flow before reductions Required fire flow Fire Flow = 220 x C x A ^{0.5} 1,198 Choose combustibility of contents Occupancy hazard funited combustible -25% Combustible 1 Choose contents Occupancy hazard for sprinklers Non-combustible -25% Combustible 1 Choose reduction for sprinklers Sprinkler reduction for sprinklers Full automatic sprinklers -30% True -10% Choose separation Exposure distance between units East side >45m 0% 10% Choose separation for sprinklers Exposure distance between units South side 20.1 to 30m 10% 10% Choose separation Exposure distance between units South side 20.1 to 30m	Structural Framing Material Choose frame used for building Coefficient C related to the type of construction Wood Frame (notinary Construction) 1.5 (notinary Construction) Ordinary Construction 1 Choose frame used for building Coefficient C related to the type of construction Non-combustible construction < 2 hrs

Note: *The floor between level 2 and 3 will be separated by a 2h fire-resistance assembly to compartmentalize the building into two

(see confirmation letter by the Architect)



architects collective

June 10, 2021

Kelby Lodoen Unseth | Planner II, Development Review (South Services) Planning, Infrastructure and Economic Development Department

Re: Letter of Confirmation- Fire Separations

Proposed 3-storey low-rise apartment building 1435-1455 Morisset Avenue

Kelby,

In coordination with available fire flow rates on Morisset Ave., the proposed 3-storey low rise apartment building will be sprinklered and be constructed with the following fire separations. All floor assemblies will be separated by 1 h fire-resistance assemblies as per OBC section 9.10.9.11 (1). The floor between level 2 and level 3 will be separated by a 2 h fire-resistance assembly to compartmentalize the building into two. All walls at stair wells and shafts will also have a fire-resistance rating of 2 h.

Regards,

Roberto Campos, Architect | OAA | M.Arch. | MRAIC | ORSA Partner

FIG. 1 3550, Saint-Antoine O. Montréal, Québec H4C 1A9 T 514 861-5122 FIG. 2 190 Somerset St. West Suite 206 Ottawa ON K2P 0J4 T 613 695-6122



Mohan Basnet

From: Sent: To: Subject: Attachments: Baker, Adam <adam.baker@ottawa.ca> May 26, 2021 3:38 PM Mohan Basnet RE: 200572-1435 Morisset Ave-Water Boundary Conditions 1435 Morisset Avenue May 2021.pdf

Hi Mohan,

Please find attached the water boundary results modelled for 1435 Morisset. Please note the available flow would not meet the current fire flow requirements –

The following are boundary conditions, HGL, for hydraulic analysis at 1435 Morisset (zone ME) assumed to be connected to 152 mm on Morisset Avenue (see attached PDF for location).

Minimum HGL = 143.7 m

Maximum HGL = 158.2 m

Available Flow at 20 psi = 85 L/s, assuming ground elevation of 97.6 m

The maximum pressure is estimated to be more than 80 psi. A pressure check at completion of construction is recommended to determine if pressure control is required.

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Thank you, Adam

Adam Baker, EIT

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - South Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 26552, Adam.Baker@ottawa.ca

From: Mohan Basnet <mbasnet@lrl.ca>
Sent: May 11, 2021 4:03 PM
To: Baker, Adam <adam.baker@ottawa.ca>
Subject: 200572-1435 Morisset Ave-Water Boundary Conditions

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Hello Adam,

We are working on serviceability of the proposed residential development at 1435 Morisset Ave and require boundary conditions at this site to proceed. Please use the following data to provide the required boundary conditions.

- Service location: please see schematic below
- Type of development: proposed 3-Storey apartment
- Average daily demand: 0.22 L/s
- Maximum daily demand: 1.63 L/s
- Peak hourly demand: 18.27 L/s
- FUS fire flow demand: 320 L/s



For your reference, I have also included copies of domestic water demand calculations, FUS fire flow calculations and nearby hydrant location along with this email.

Thank you and please let me know if you have any questions.

Mohan

Mohan Basnet, P.Eng.



Civil Engineering Services LRL Associates Ltd. 5430 Canotek Road Ottawa, Ontario K1J 9G2 T (613) 842-3434 or (877) 632-5664 ext 213 F (613) 842-4338 E mbasnet@Irl.ca W www.Irl.ca Given the current COVID-19 situation, please be aware that LRL has implemented alternative working conditions for our team. Many of us have now transitioned to working from home; however, communication and workability remains one of our top priorities.

We will continue to be reachable by cell phone or by calling LRL at 613-842-3434 which will prompt you to enter the extension of the person you are trying to reach.

In addition, we will continue to have access to all e-mail correspondence and do our best to return all inquiries in a timely manner.

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APPENDIX D

Sanitary Calculations

Mohan Basnet

From:	Baker, Adam <adam.baker@ottawa.ca></adam.baker@ottawa.ca>
Sent:	May 27, 2021 8:22 AM
To:	Mohan Basnet
Subject:	RE: 200572-1435 Morisset Ave-Proposed Service Connection/SWM criteria
Follow Up Flag:	Follow up
Flag Status:	Flagged

Hi Mohan,

Please accept the following information with regards to the servicing criteria -

- SWM criteria for an outlet on Morisset Avenue:
 - The 2-yr storm or 5-yr storm event using the IDF information derived from the Meteorological Services of Canada rainfall data, taken from the MacDonald Cartier Airport, collected 1966 to 1997.
 - The pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less (§ 8.3.7.3).
 - A calculated time of concentration (Cannot be less than 10 minutes).
 - Flows to the storm sewer in excess of the 2-year storm release rate, up to and including the 100-year storm event, must be detained on site.
 - Please confirm any stormwater quality requirements with the relevant Conservation Authority.
- Sanitary Constraints:
 - Please confirm if the anticipated sanitary flow provided (0.76 L/s) is peak or average. From an average flow of 0.76 L/s, a resulting peak sanitary flow of 3.24 L/s is acceptable for this system. If the peak sanitary flows will exceed this, please let me know and I will re-confirm for you.

Thank you, Adam

Adam Baker, EIT

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - South Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 26552, <u>Adam.Baker@ottawa.ca</u>

From: Mohan Basnet <mbasnet@lrl.ca> Sent: May 12, 2021 2:09 PM To: Baker, Adam <adam.baker@ottawa.ca> Subject: 200572-1435 Morisset Ave-Proposed Service Connection/SWM criteria CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

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Good afternoon Adam,

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I

At LRL, we are working for STM and servicing study for a proposed 3-storey residential apartment building at 1435 Morisset Ave.

As a follow-up from the pre-consultation notes (Aug 28, 2020):

- Can you please advise SWM criteria of the subject site with a proposed STM outlet at Morisset Ave?
- Can you also advise remaining capacity of SAN sewer along Morisset Ave? Anticipated SAN flow from the proposed building is 0.76 L/s

For your reference I have also attached schematic of proposed service connections.

Please let me know if you have any question.



Given the current COVID-19 situation, please be aware that LRL has implemented alternative working conditions for our team. Many of us have now transitioned to working from home; however, communication and workability remains one of our top priorities.

We will continue to be reachable by cell phone or by calling LRL at 613-842-3434 which will prompt you to enter the extension of the person you are trying to reach.

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							Sanitary Design Parameters Average Daily Flow = 280 L/p/day							Pipe Design Parameters												
		LRL File No.: Project: Location: Date: Designed:	ļ			0	ON			Commercial & Institutional Flow = 28000 L/ha/day Light Industrial Flow = 35000 L/ha/day Heavy Industrial Flow = 55000 L/ha/day			Industrial Peak Factor = as per Appendix 4-B Extraneous Flow = 0.33 L/s/gross ha (as Per Tech Bulleting ISTB-2018-01)				Minimum Velocity = 0.60 m/s Manning's n = 0.013									
	LOCATIO	N		RESIDEN	TIAL AREA	AND POPL	JLATION		COMMERCIAL INDUSTRIAL INSTITUTIONAL C+I+I							PIPE										
STREET/ SITE	FROM MH	ТО МН	AREA (Ha)	POP.	CUMMU AREA (Ha)	POP.	PEAK FACT.	PEAK FLOW (I/s)	AREA (Ha)	ACCU. AREA (Ha)	AREA (Ha)	ACCU. AREA (Ha)	PEAK FACT.	AREA (Ha)	ACCU. AREA (Ha)	PEAK FLOW (I/s)	TOTAL AREA (Ha)	ACCU. AREA (Ha)	INFILT. FLOW (I/s)	FLOW (I/s)	LENGTH (m)	DIA. (mm)	SLOPE (%)	MATERIAL	CAP. (FULL) (I/s)	VEL. (FULL) (m/s)
	Bldg.	SAN MH01	0.185	53.9	0.19	53.9	4.0	0.70									0.19	0.19	0.06	0.76	12.5	200	2.00%	PVC	46.38	1.48

APPENDIX E

Civil Engineering Drawings

PROPOSED 3-STOREY LOW-RISE APARTMENT 1435 &1455 MORISSET AVE, OTTAWA, ON

REVISION 00



KEY PLAN (N.T.S.)

DRAWING INDEX

TITLE PAGE SEDIMENT AND EROSION CONTROL PLAN DEMOLITION PLAN GRADING AND DRAINAGE PLAN SERVICING PLAN STORMWATER MANAGEMENT PLAN PRE-DEVELOPMENT WATERSHED PLAN POST-DEVELOPMENT WATERSHED PLAN CONSTRUCTION DETAIL PLAN





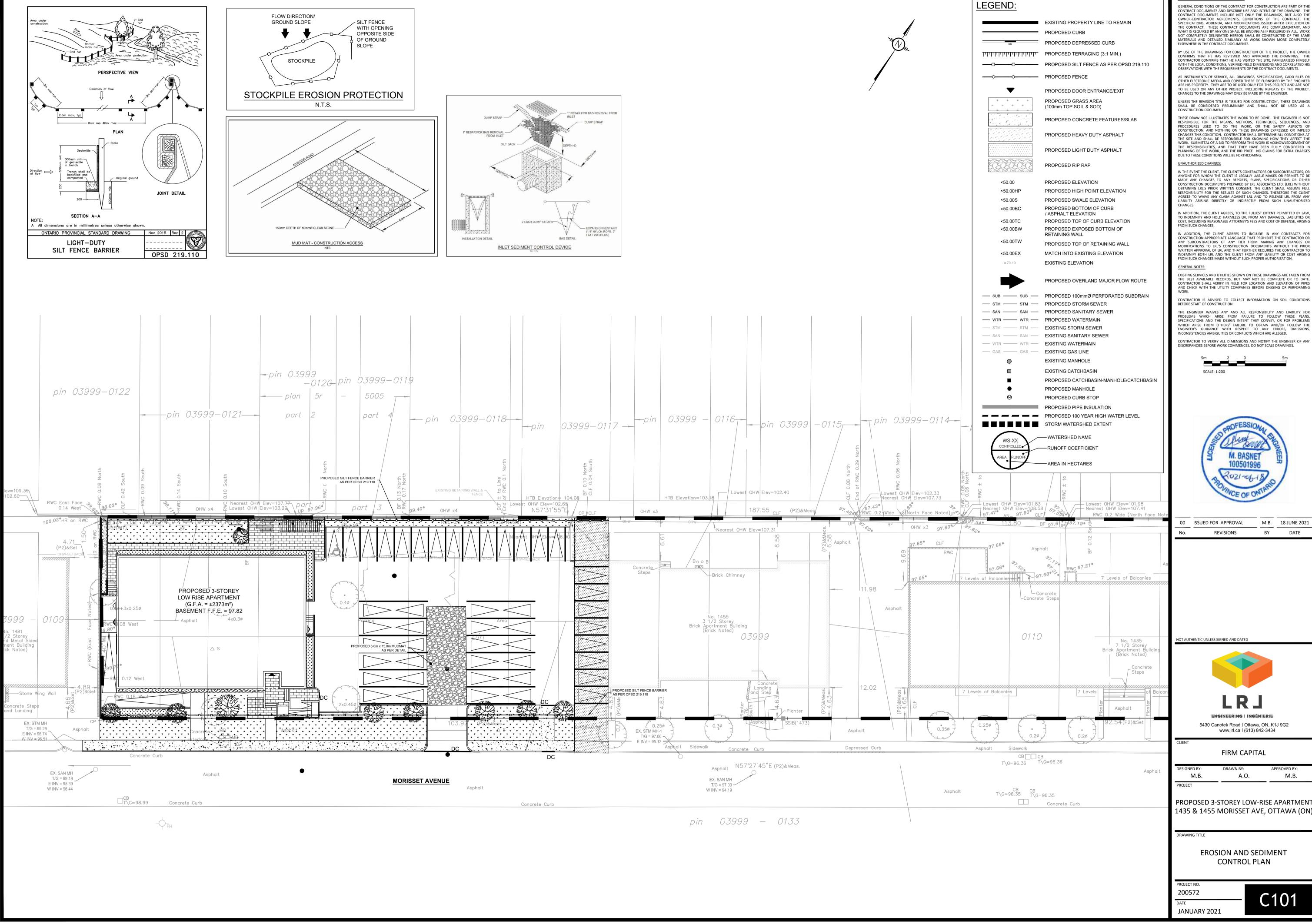
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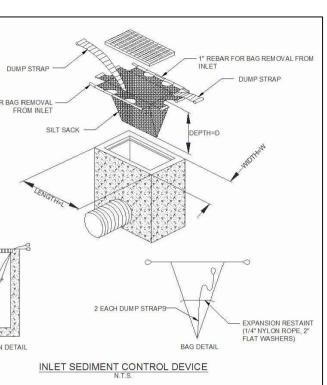
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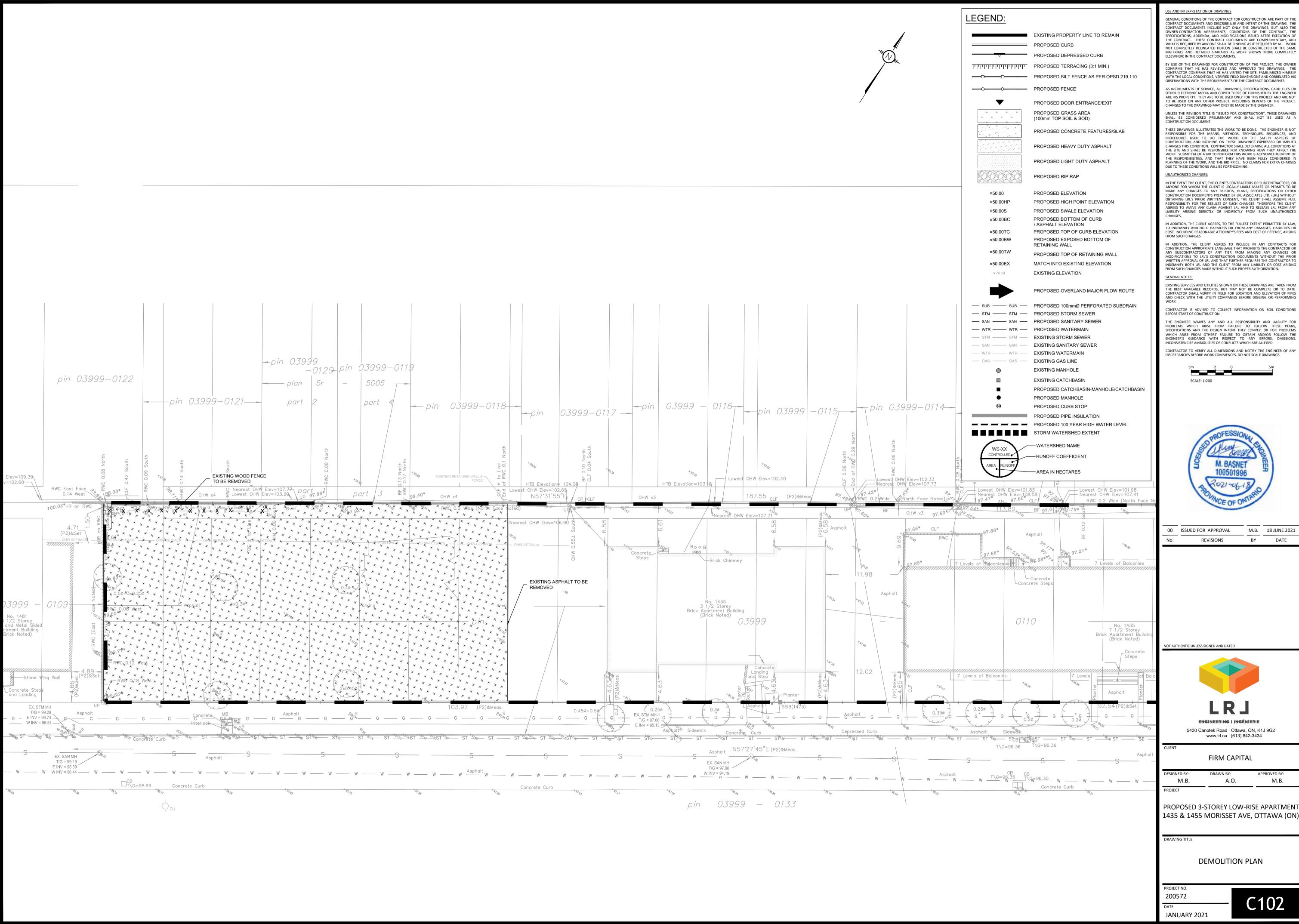






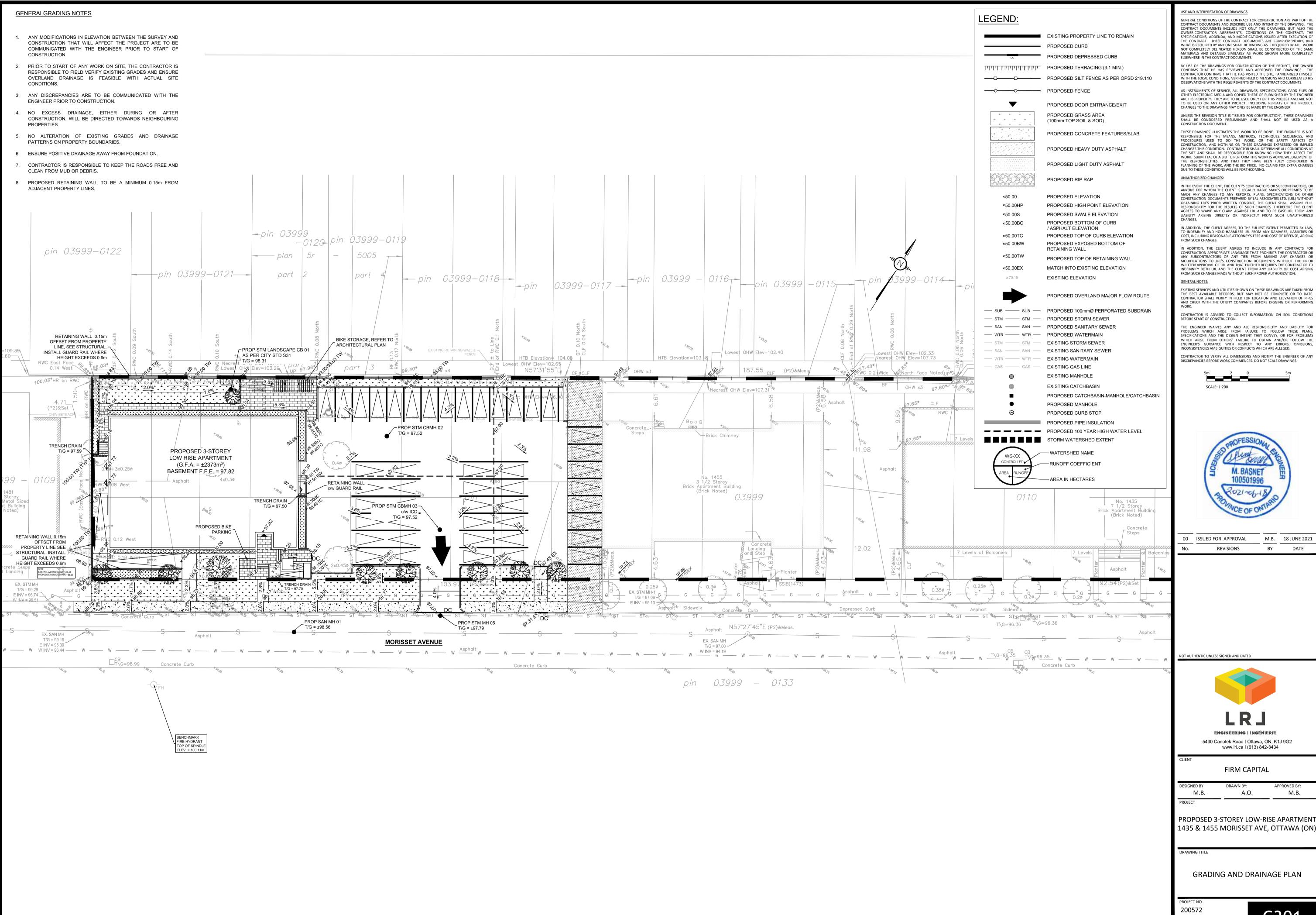


USE AND INTERPRETATION OF DRAWINGS



- CONSTRUCTION.
- CONDITIONS.
- PROPERTIES.

- PROPOSED RETAINING WALL TO BE A MINIMUM 0.15m FROM



JANUARY 2021

DATE



NOTES: GENERAL

- CONTRACTOR IS RESPONSIBLE FOR ALL LAYOUT FOR CONSTRUCTION PURPOSES.
- DETENTION AREAS ARE PROVIDED STRIP AND REMOVE ALL TOPSOIL FROM IMPROVED AREAS.
- CURBS TO BE BARRIER, CONSTRUCTED AS PER OPSD 600.110. ALL MATERIAL SUPPLIED AND PLACED FOR PARKING LOT AND ACCESS ROAD CONSTRUCTION SHALL BE TO OPSS STANDARDS AND SPECIFICATIONS UNLESS OTHERWISE NOTED.
- ABUTTING PROPERTY GRADE TO BE MATCHED.
- MINIMIZE DISTURBANCE TO EXISTING VEGETATION DURING THE EXECUTION OF ALL WORKS.
- THE ENGINEER. EXCAVATE AND REMOVE ALL ORGANIC MATERIAL AND DEBRIS, IF ANY, LOCATED WITHIN THE PROPOSED BUILDING, PARKING AND ROADWAY LOCATIONS. THE APPROVAL OF THIS PLAN DOES NOT EXEMPT THE CONTRACTOR FROM THE
- PERMITS, WATER PERMIT, ETC.
- ENGINEER BEFORE COMMENCING WORK.
- REQUIREMENTS.
- OPSD): SEWER AND WATERMAIN MATERIAL TYPES AND DISINFECTION. SUPPLY AND CONSTRUCT ALL SEWERS AND APPURTENANCES IN ACCORDANCE WITH
- SPECIFICATIONS (OPSS & OPSD); ROADS AND PUBLIC WORKS.

NOTES: SEWERS

- SEWER BEDDING AS PER PIPE TRENCH DETAIL WITH GRANULAR 'A' BEDDING COMPACTED TO 95% OF ITS SPMDD. ALL WORK SHALL BE PERFORMED, AS APPLICABLE IN ACCORDANCE WITH OPSS 407, AND 410.
- 5.7.1.3 & 5.10 AS WELL. ALSO REFER TO COO STD. DWGS. W21-23.
- TYPE JOINTS TO CONFORM TO CSA

- STANDARD B182.2 OR LATEST AMENDMENT, UNLESS SPECIFIED OTHERWISE.
- EXISTING MAINTENANCE STRUCTURES TO BE RE-BECNHED WHERE A NEW CONNECTION IS MADE.
- BEDDING, UNLESS SPECIFIED OTHERWISE.

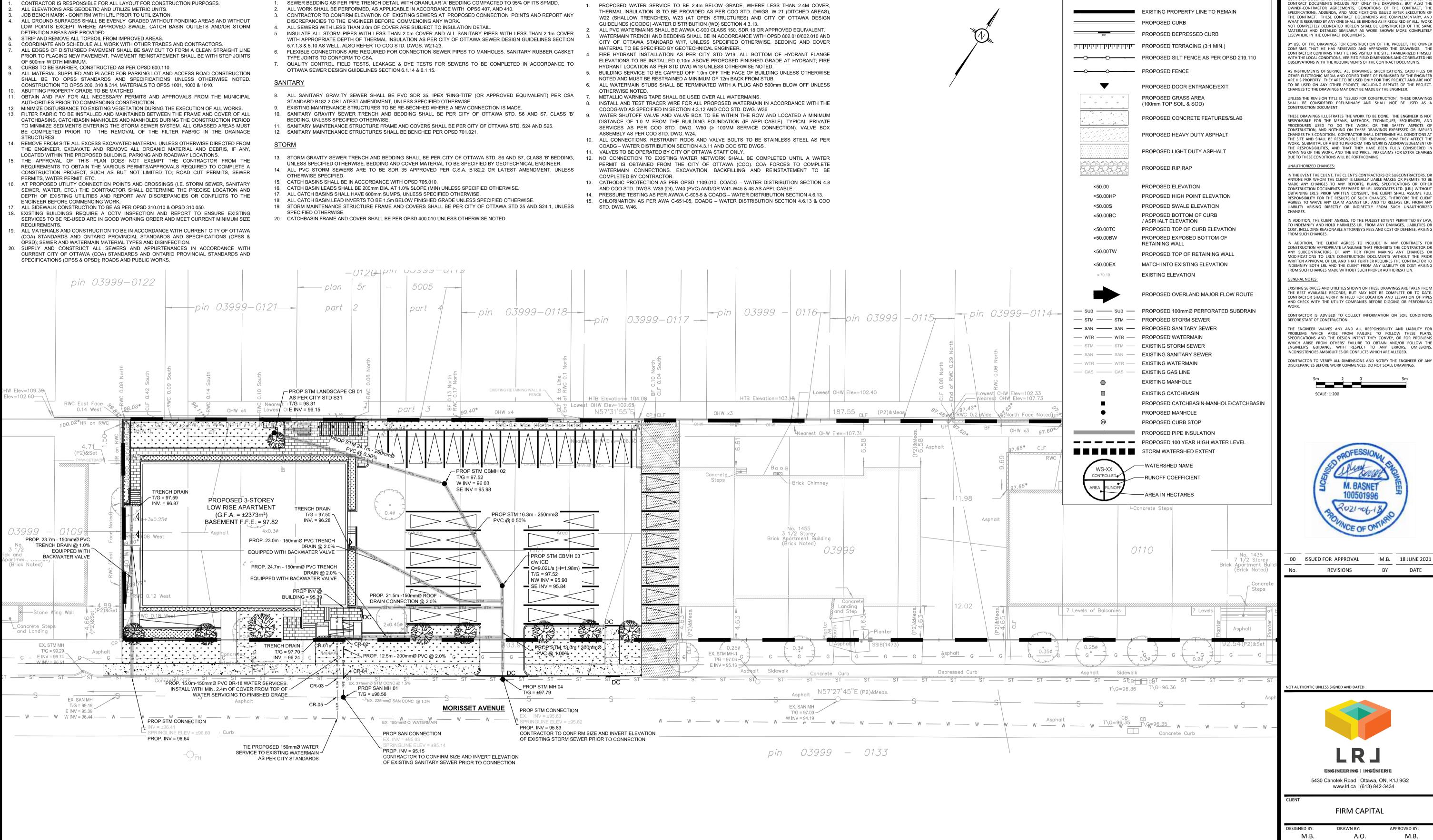
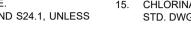


TABLE: PIPE CROSSING

Crossing #	PROP. SAN	PROP. WM	PROP. STM	EX. STM (375mm)	EX. SAN (225mm)	Depth Separation (m)
CR-01	N/A	95.70 (obv.)	96.21 (inv.)	N/A	N/A	±0.5
CR-02	95.51 (obv.)	N/A	96.18 (inv.)	N/A	N/A	±0.7
CR-03	N/A	95.70 (obv.)	N/A	±95.99 (inv.)	N/A	±0.3
CR-04	95.39 (obv.)	N/A	N/A	±95.97 (inv.)	N/A	±0.6
CR-05	N/A	95.60 (inv.)	N/A	N/A	±95.27 (obv.)	±0.3

NOTES: WATERMAIN



LEGEND:

PROJEC

USE AND INTERPRETATION OF DRAWINGS

GENERAL CONDITIONS OF THE CONTRACT FOR CONSTRUCTION ARE PART OF THE CONTRACT DOCUMENTS AND DESCRIBE USE AND INTENT OF THE DRAWING. T

PROPOSED 3-STOREY LOW-RISE APARTMENT 1435 & 1455 MORISSET AVE, OTTAWA (ON)

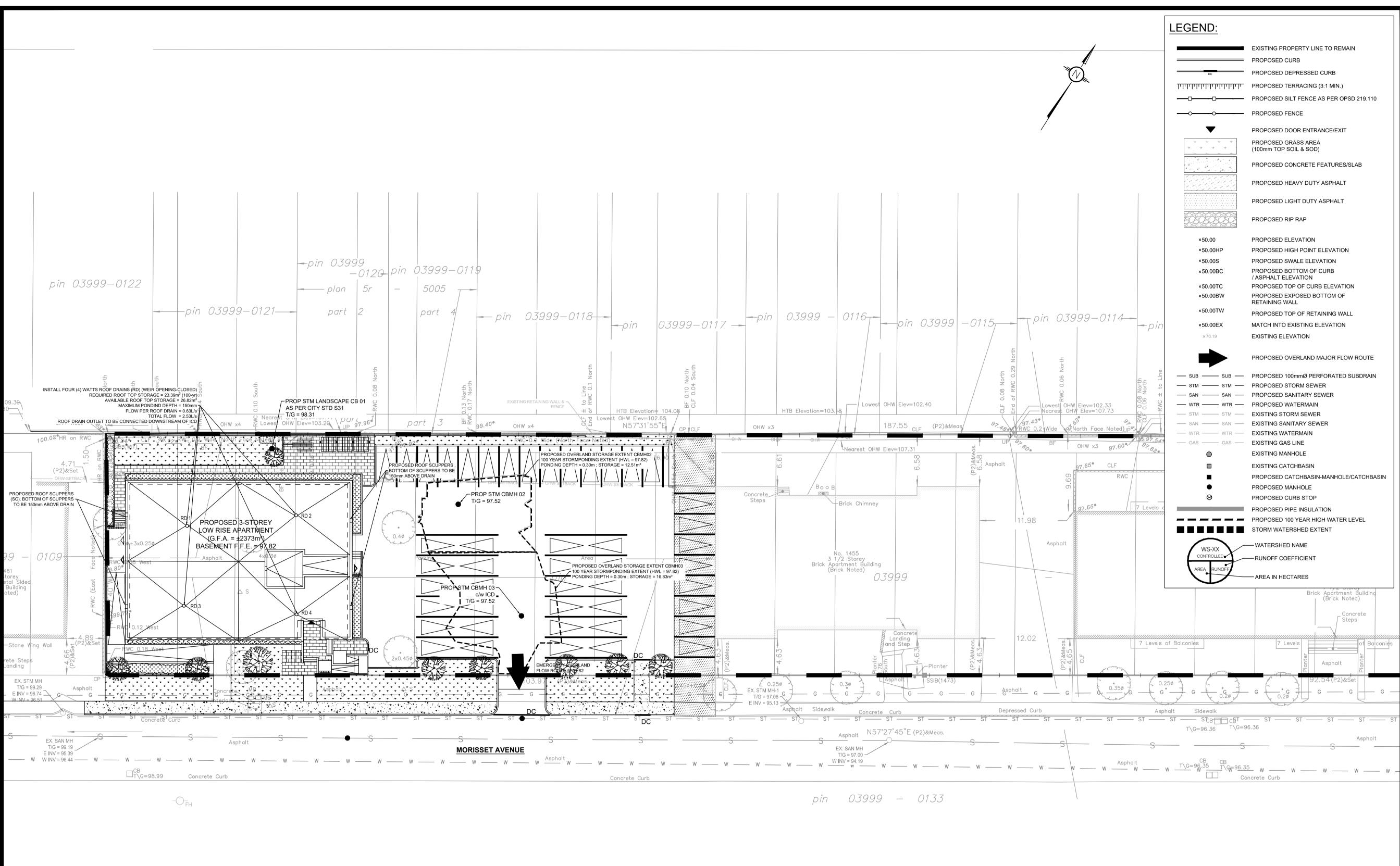
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SERVICING PLAN

PROJECT NO 200572

JANUARY 2021

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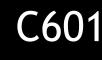


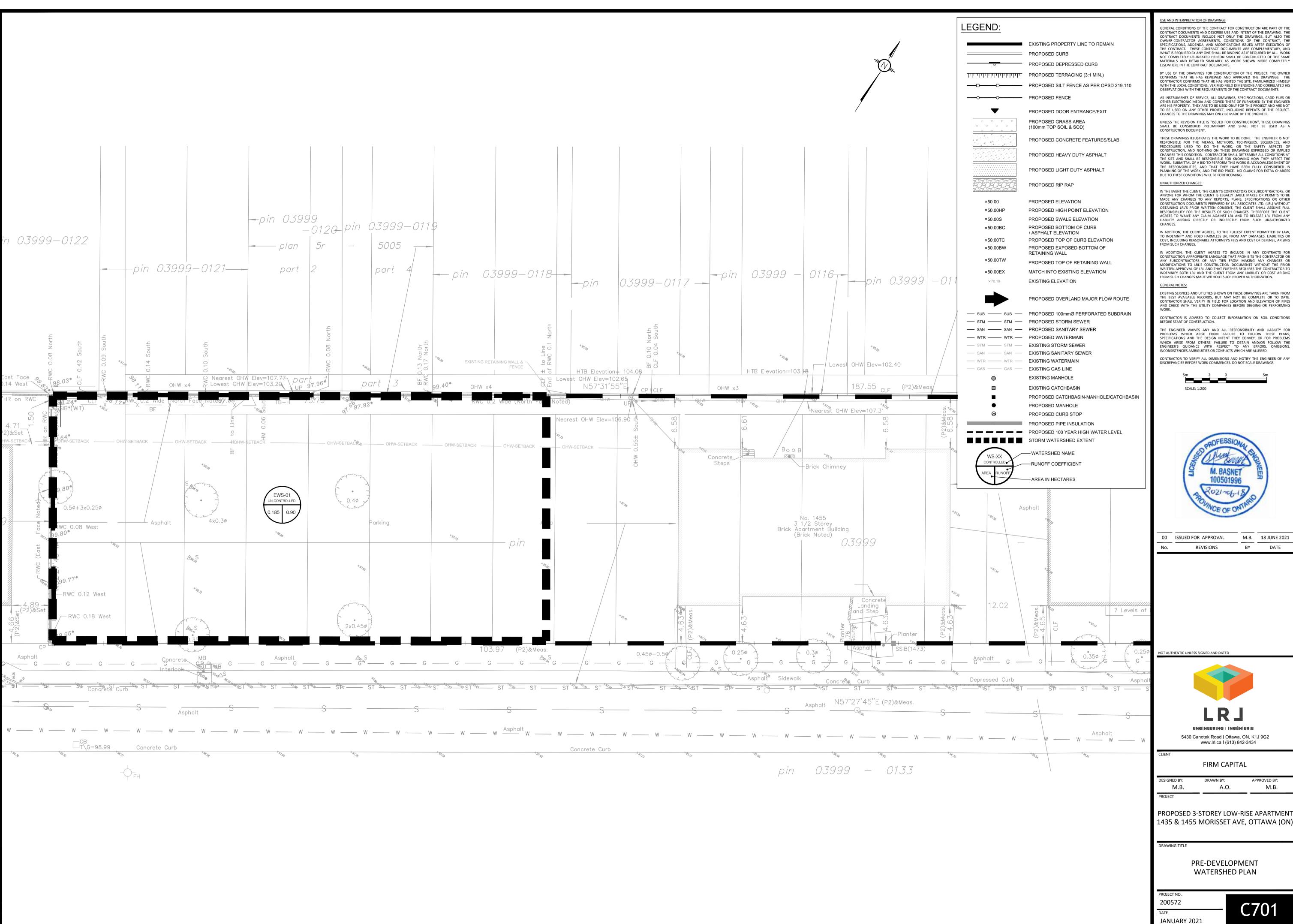
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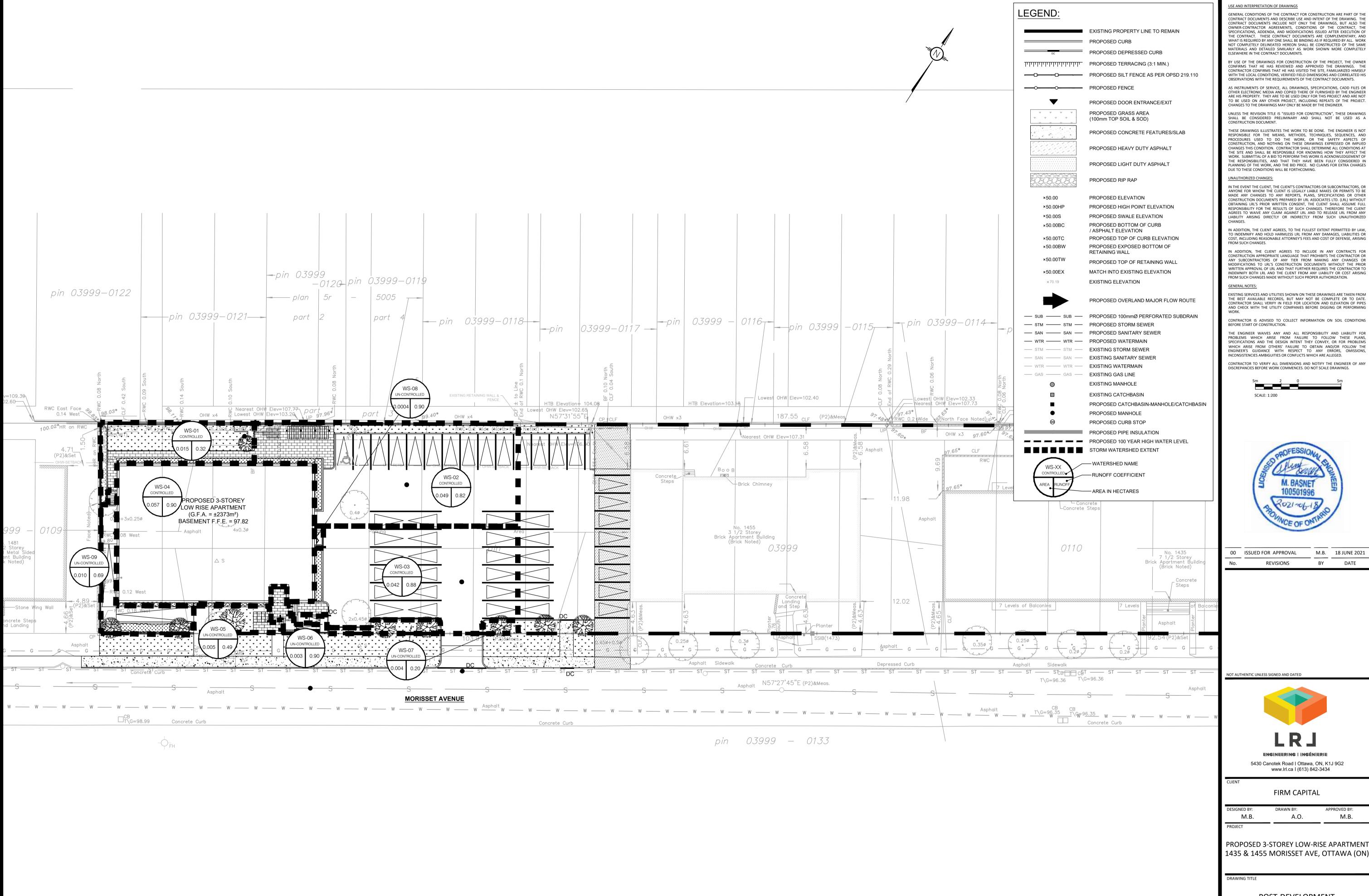
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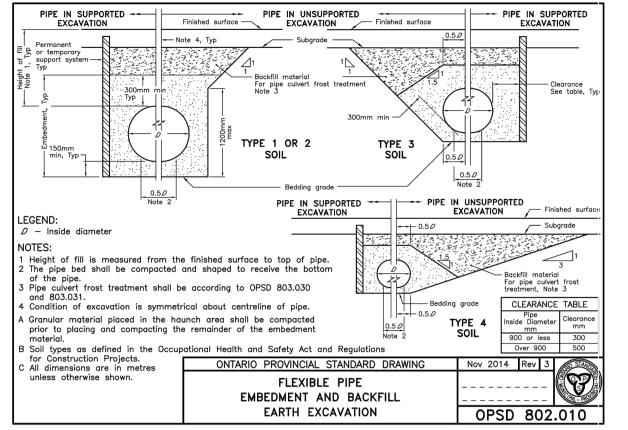


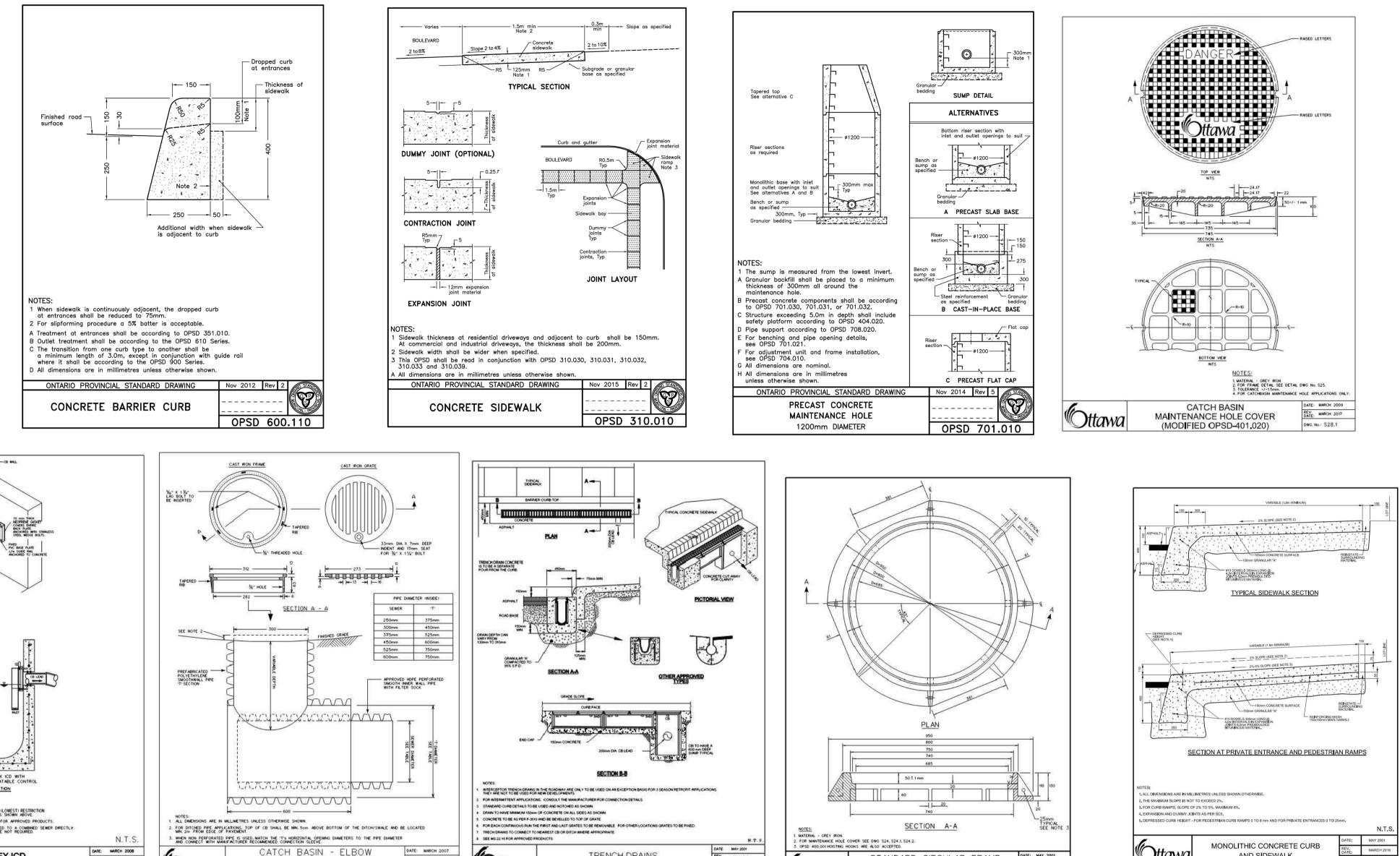
POST-DEVELOPMENT WATERSHED PLAN

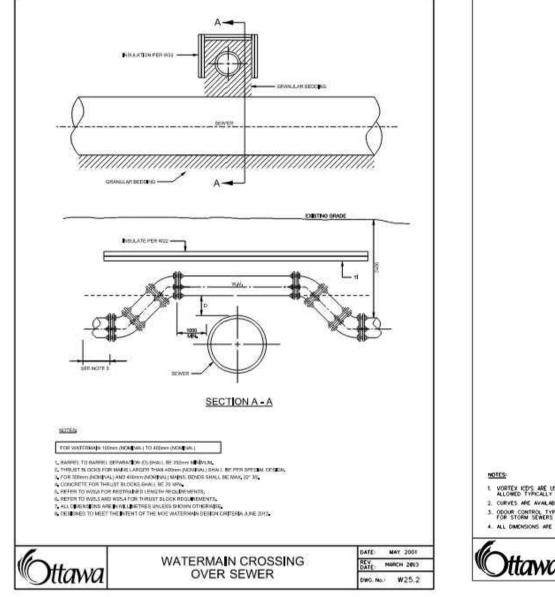
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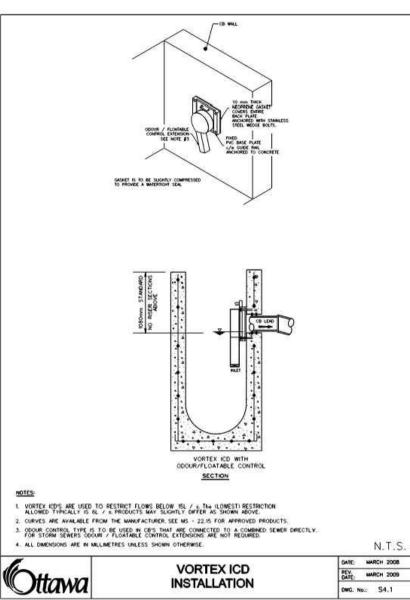
DATE JANUARY 2021

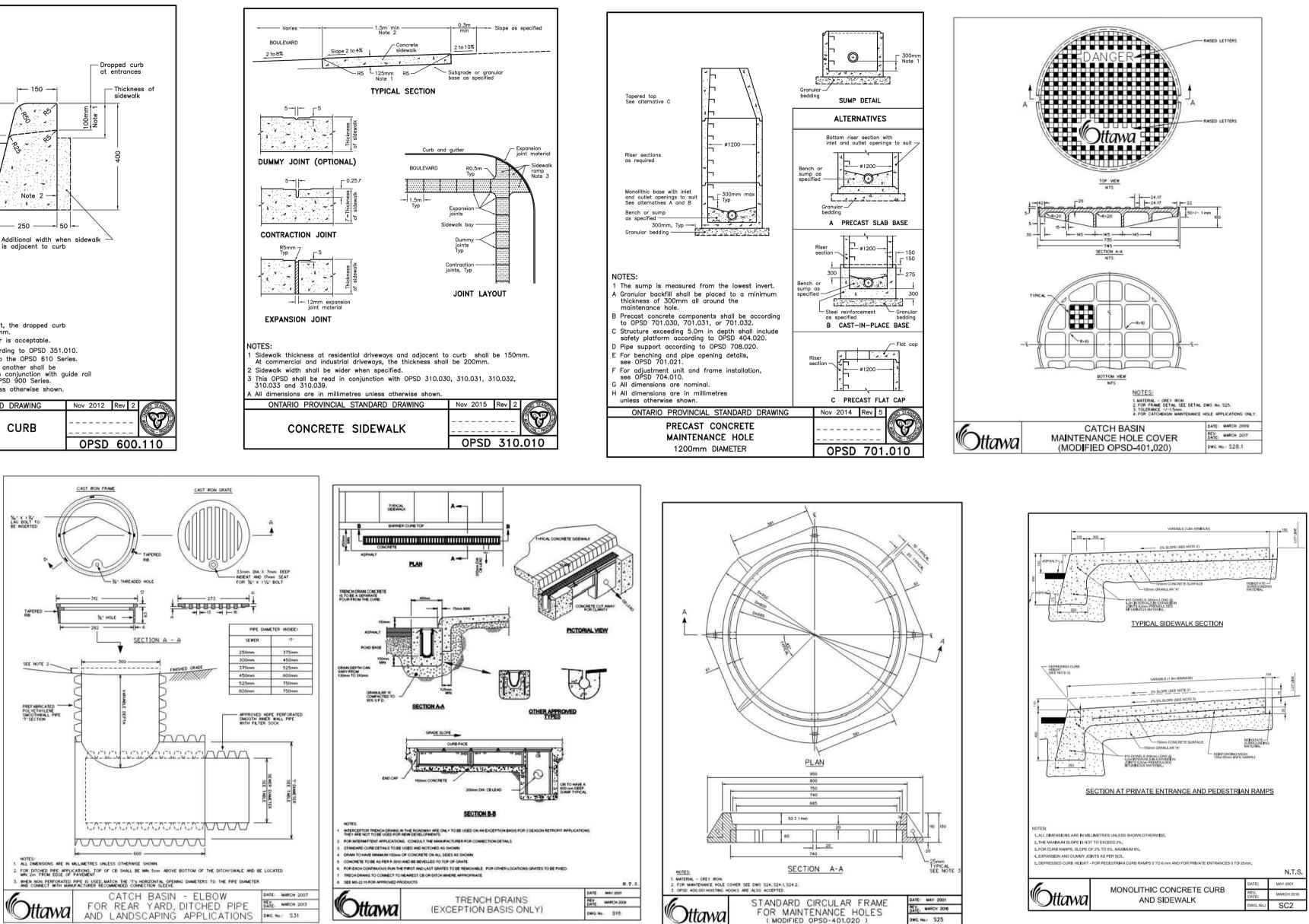


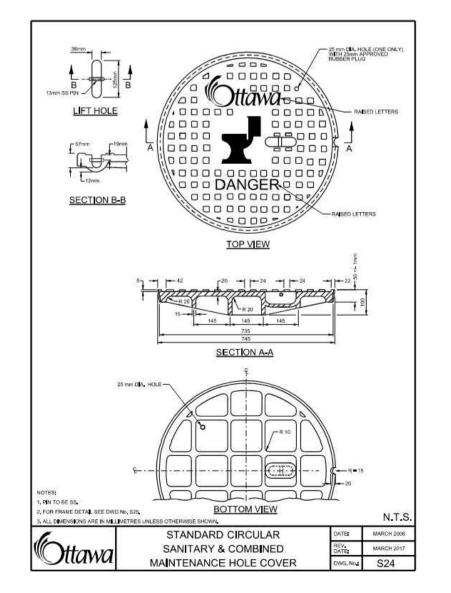


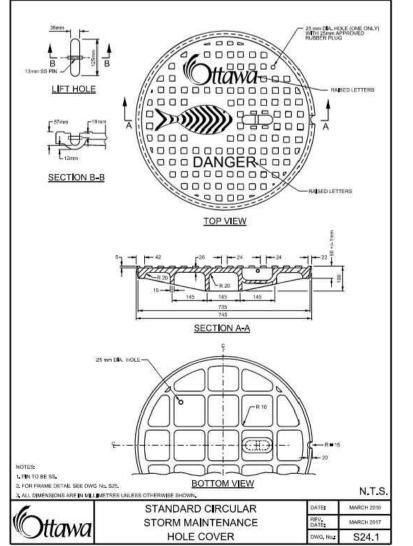


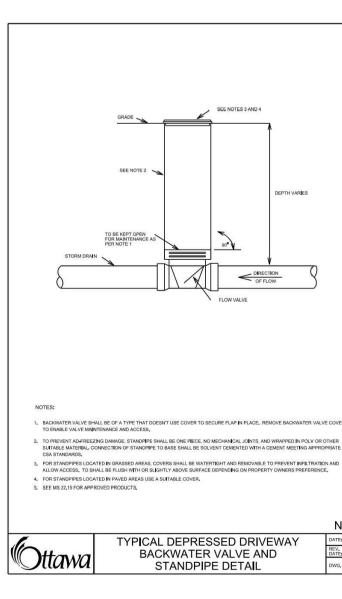


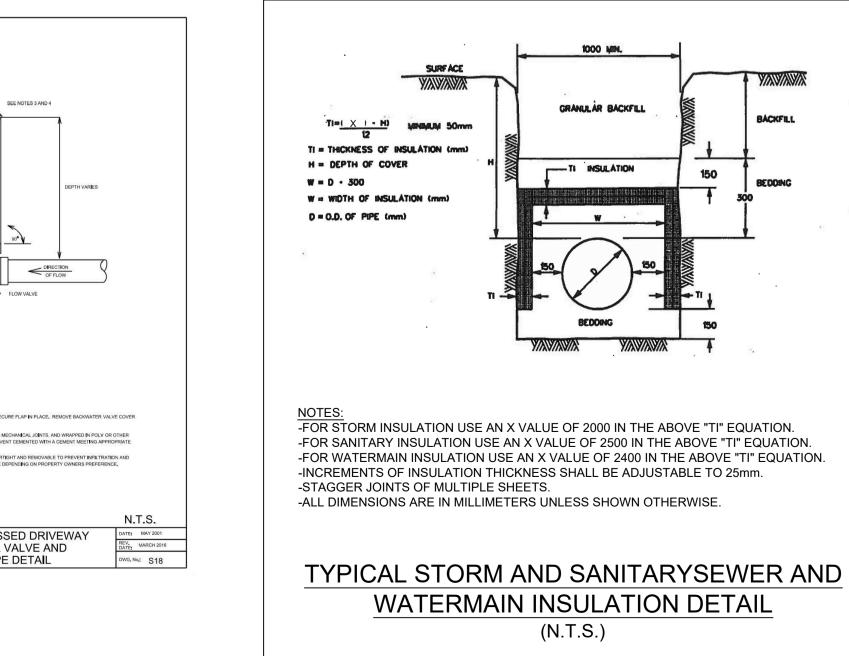


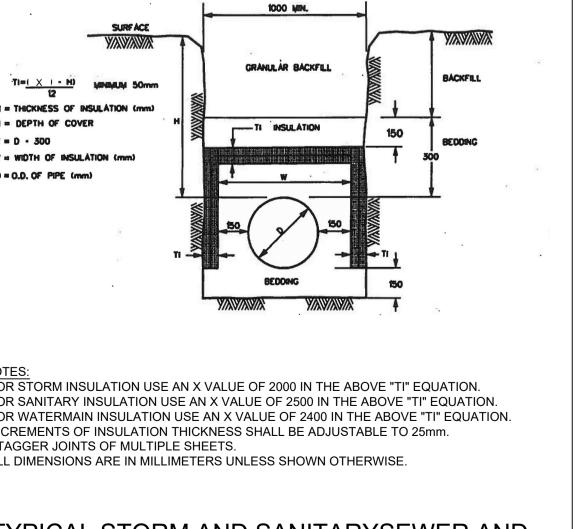












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UNAUTHORIZED CHANGES:

ANYONE FOR WHOM THE CLIENT IS LEGALLY LIABLE MAKES OR PERMITS TO BE MADE ANY CHANGES TO ANY REPORTS, PLANS, SPECIFICATIONS OR OTHER CONSTRUCTION DOCUMENTS PREPARED BY LRL ASSOCIATES LTD. (LRL) WITHOUT OBTAINING LRL'S PRIOR WRITTEN CONSENT, THE CLIENT SHALL ASSUME FULL RESPONSIBILITY FOR THE RESULTS OF SUCH CHANGES. THEREFORE THE CLIENT AGREES TO WAIVE ANY CLAIM AGAINST LRL AND TO RELEASE LRL FROM ANY LIABILITY ARISING DIRECTLY OR INDIRECTLY FROM SUCH UNAUTHORIZED CHANGES.

IN ADDITION, THE CLIENT AGREES, TO THE FULLEST EXTENT PERMITTED BY LAW, TO INDEMNIFY AND HOLD HARMLESS LRL FROM ANY DAMAGES, LIABILITIES OR COST, INCLUDING REASONABLE ATTORNEY'S FEES AND COST OF DEFENSE, ARISING FROM SUCH CHANGES

IN ADDITION, THE CLIENT AGREES TO INCLUDE IN ANY CONTRACTS FOR CONSTRUCTION APPROPRIATE LANGUAGE THAT PROHIBITS THE CONTRACTOR OR ANY SUBCONTRACTORS OF ANY TIER FROM MAKING ANY CHANGES OR MODIFICATIONS TO LRL'S CONSTRUCTION DOCUMENTS WITHOUT THE PRIOR WRITTEN APPROVAL OF LRL AND THAT FURTHER REQUIRES THE CONTRACTOR TO INDEMNIFY BOTH LRL AND THE CLIENT FROM ANY LIABILITY OR COST ARISING FROM SUCH CHANGES MADE WITHOUT SUCH PROPER AUTHORIZATION. GENERAL NOTES:

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CONTRACTOR IS ADVISED TO COLLECT INFORMATION ON SOIL CONDITIONS BEFORE START OF CONSTRUCTION. THE ENGINEER WAIVES ANY AND ALL RESPONSIBILITY AND LIABILITY FOR

PROBLEMS WHICH ARISE FROM FAILURE TO FOLLOW THESE PLANS, SPECIFICATIONS AND THE DESIGN INTENT THEY CONVEY, OR FOR PROBLEMS WHICH ARISE FROM OTHERS' FAILURE TO OBTAIN AND/OR FOLLOW THE ENGINEER'S GUIDANCE WITH RESPECT TO ANY ERRORS, OMISSIONS INCONSISTENCIES AMBIGUITIES OR CONFLICTS WHICH ARE ALLEGED. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES BEFORE WORK COMMENCES. DO NOT SCALE DRAWINGS.



00 ISSUED FOR APPROVAL M.B. 18 JUNE 2021 No. REVISIONS BY DATE

NOT AUTHENTIC UNLESS SIGNED AND DATED



FIRM CAPITAL

DESIGNED BY:	DRAWN BY:	APPROVED BY:
M.B.	A.O.	M.B.
PROJECT		

PROPOSED 3-STOREY LOW-RISE APARTMENT 1435 & 1455 MORISSET AVE, OTTAWA (ON)

DRAWING TITLE

CONSTRUCTION DETAIL PLAN

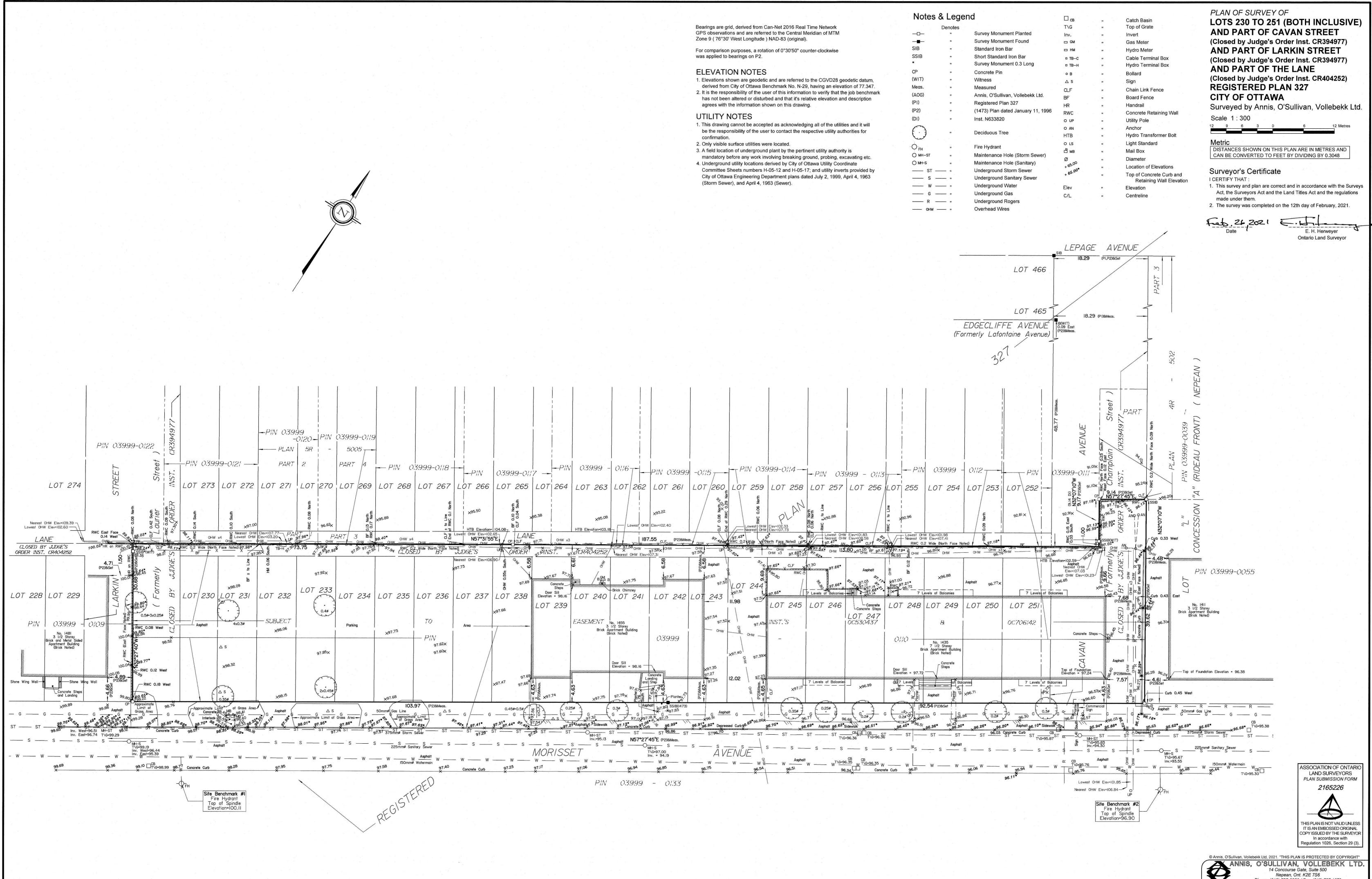
PROJECT NO. 200572

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JANUARY 2021

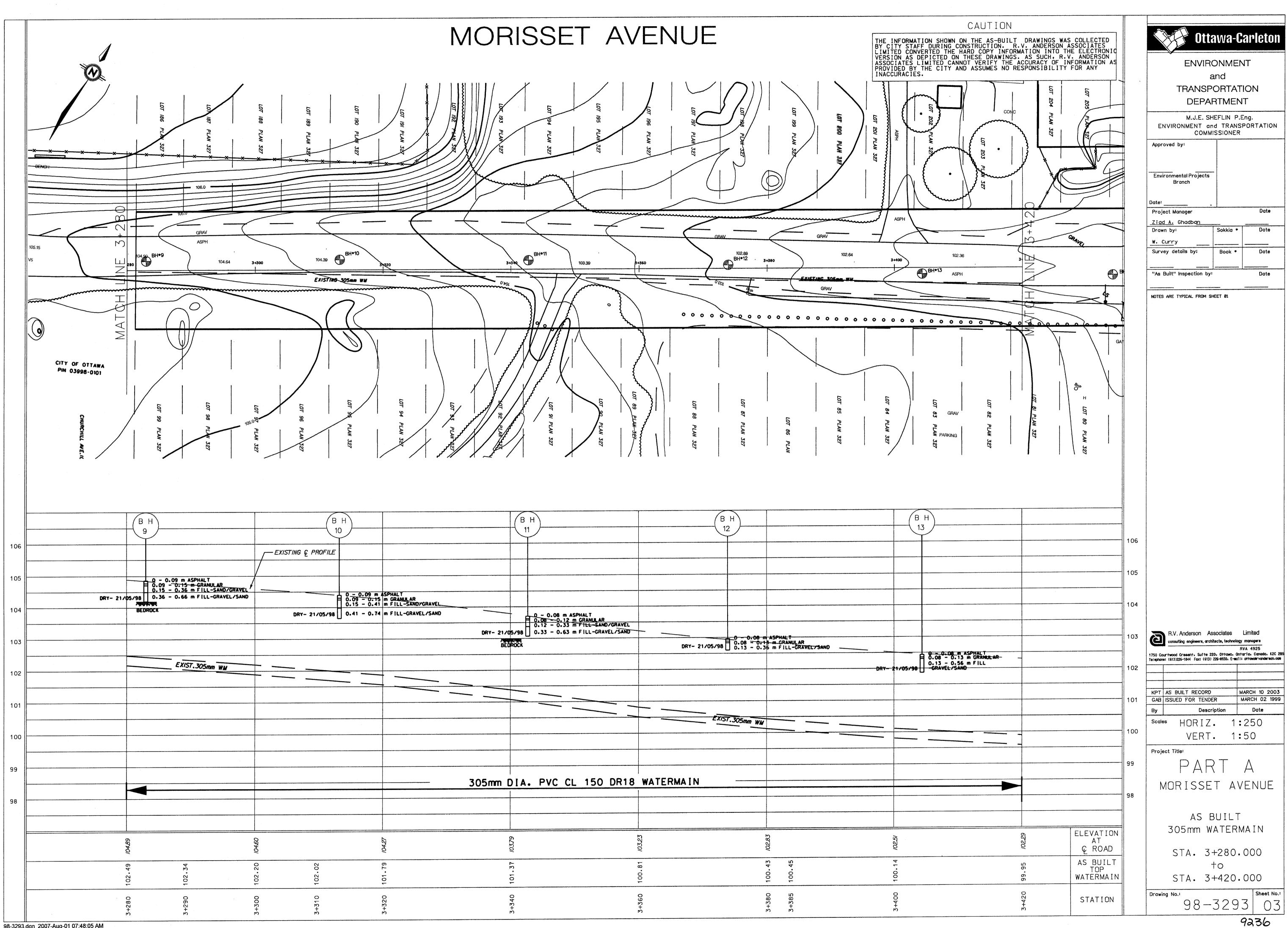
APPENDIX F

Survey As-Built Architectural Drawings



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Phone: (613) 727-0850 / Fax: (613) 727-1079 Email: Nepean@aovltd.com Job No. 21193-21 Firm Capital Lts230-251 PL327 T DI



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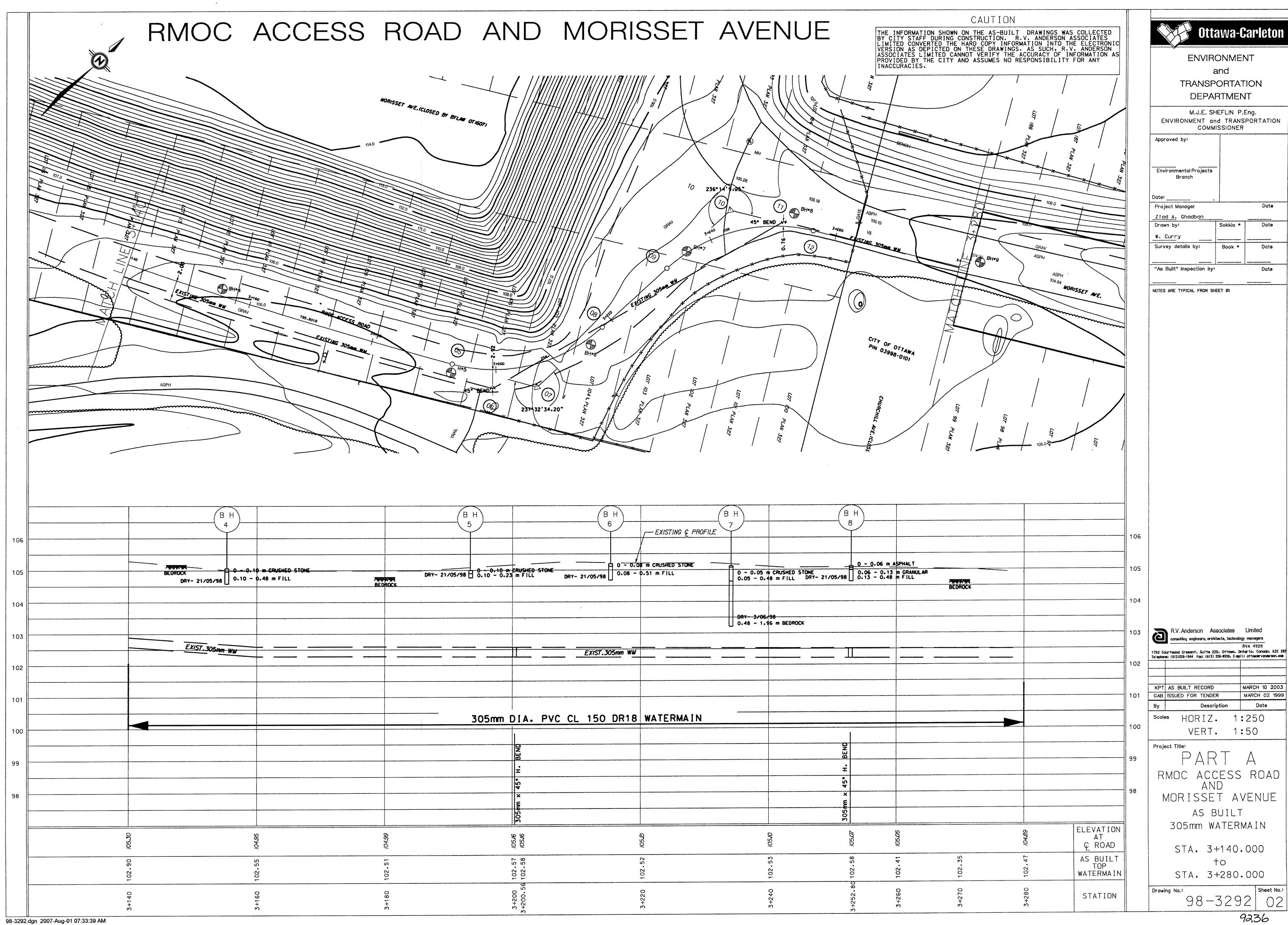
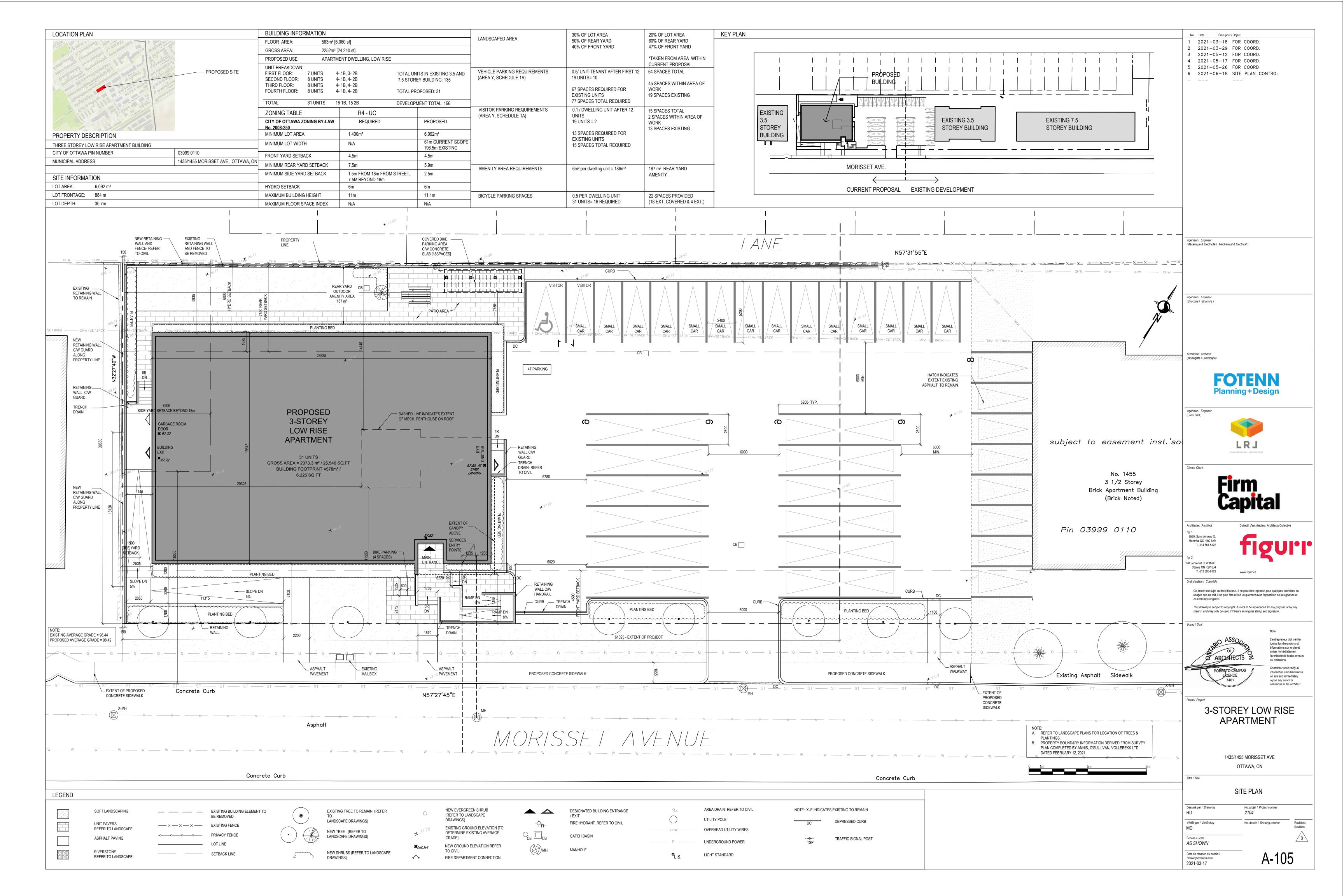
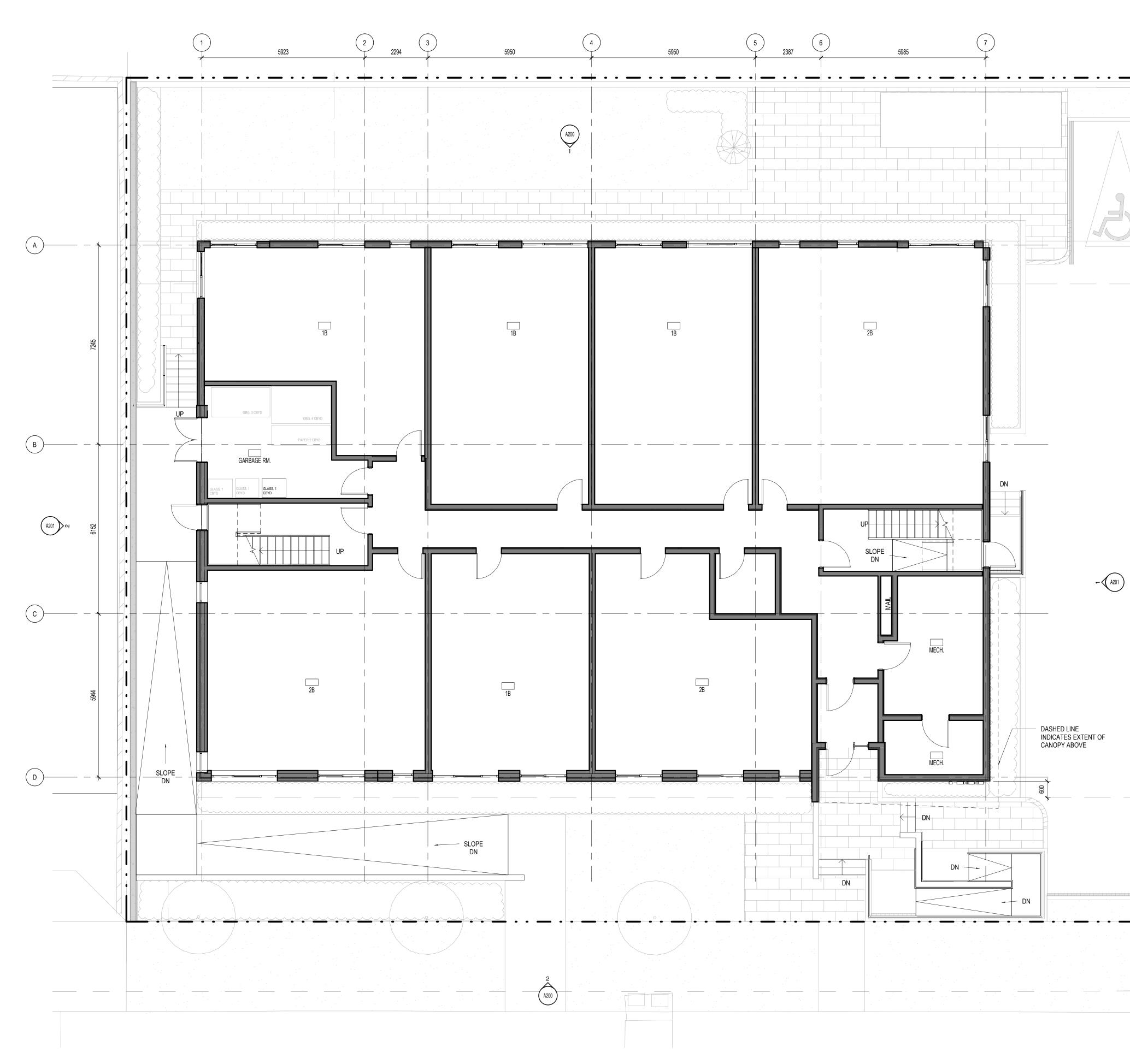


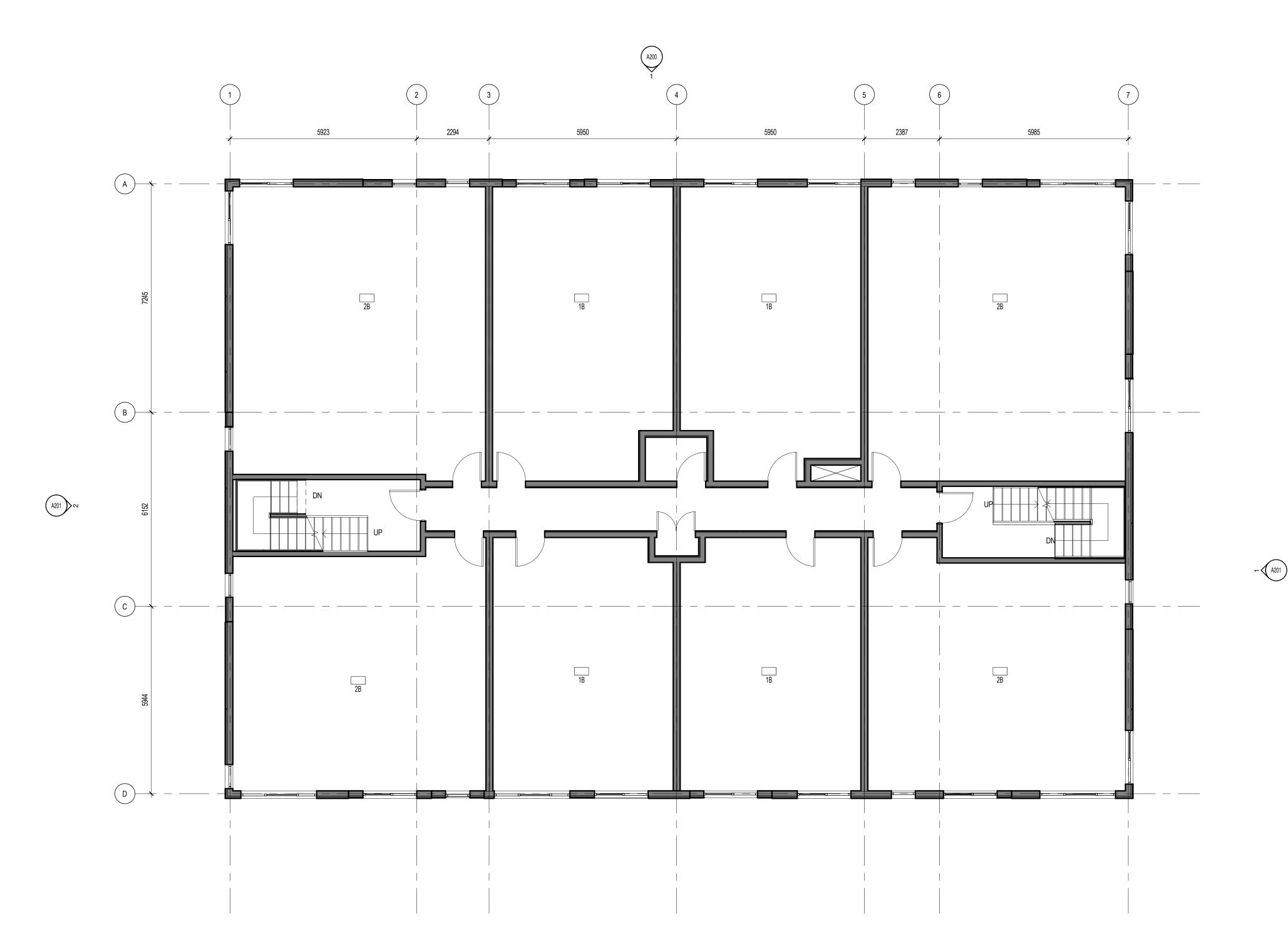
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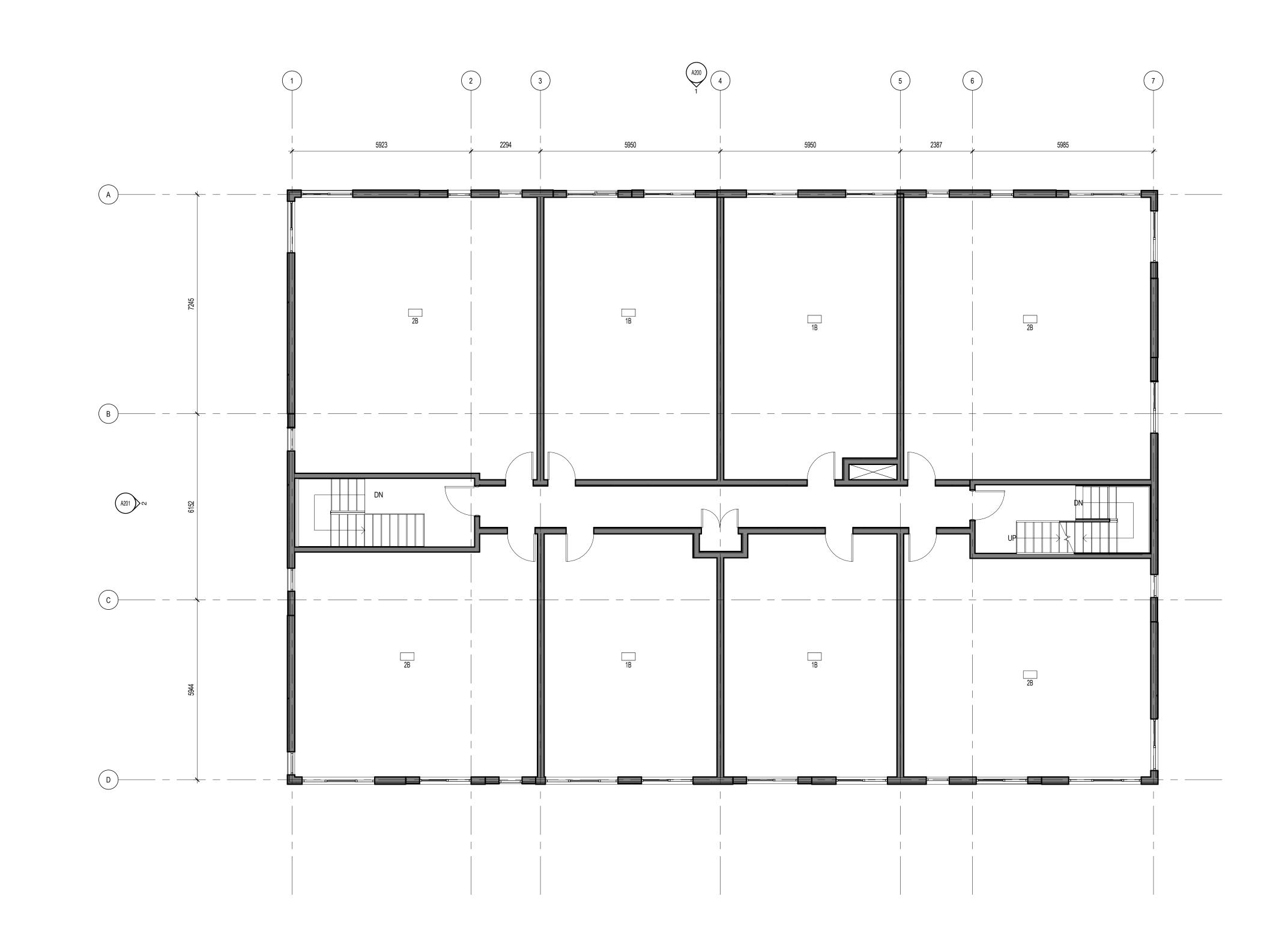


GENERAL NOTES	No. Date Émis pour / Object 2 2021-06-18 SITE PLAN APPLICATION
	Ingénieur / Engineer (paysagiste / Landscape)
	FOTENN
	Planning + Design
	Ingénieur / <i>Engineer</i> (Civil / <i>Civi</i> l)
	Client / Client
	Firm
	Capital
	Architecte / Architect Collectif d'architectes fig. 1 3550, Saint-Antoine O.
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	Sceau / Sea/
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	ROBERTO CAMPOS INFORMATION CONTRACTOR Shall verify all information and dimensions on site and immediately report any errors or omissions to the architect
	report any errors or ormissions to the architect.
	Projet / Project
	3-STOREY LOW RISE
	APARTMENT
	1435 & 1455 MORISSET AVE. OTTAWA, ON
	Titre / Title LEVEL 1 (BASEMENT) PLAN
	Dessiné par / Drawn by No. projet / Project number RD 2104
	Vérifié par / Verified by No. dessin / Drawing number Révision / MD
	Échelle / Scale 1:75
	Date de création du dessin / Drawing creation date Δ120
	2021-05-28 AIZU



GENERAL NOTES	No. Date Émis pour / Object
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	Ingénieur / Engineer (paysagiste / Landscape)
	FOTENN
	Planning + Design
	Ingénieur / Engineer (Civil / Civil)
	Client / Client
	Firm
	Capital
	Capitai
	Architecte / Architect Collectif d'architectes
	3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122
	3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122
	190 Somerset St W #206 Ottawa ON K2P 0.14 T. 613 695-6122 www.figurr.ca
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	ROBERTO CAMPOS UCENCE 7401 7401 7401 7401 7401 7401 7401 7401
	report any errors or omissions to the architect.
	Projet / Project
	3-STOREY LOW RISE APARTMENT
	1435 & 1455 MORISSET AVE. OTTAWA, ON
	LEVEL 2-3 PLAN
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	Dessiné par / Drawn by No. projet / Project number RD 2104
	Vérifié par / Verified by No. dessin / Drawing number Révision / Revision
	Échelle / Scale 1 : 75
	Date de création du dessin / Drawing creation date 2021-05-28 A121
	2021-05-28





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	Ingénieur / Engineer (paysagiste / Landscape)
	FOTENN
	Planning + Design
	Ingénieur / Engineer (Civil / Civil)
	LRJ
	ENGINEERING INGENIERIE Client / Client
	Firm
	Firm Capital
	Architecte / Architect Collectif d'architectes fig. 1 3550, Saint-Antoine O. Montréal QC H4C 1A9
	3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122
	190 Somerset St W #206 Ottawa ON K2P 0J4 T. 613 695-6122 www.figurr.ca
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	Note: L'entrepreneur doit vérifier toutes les dimensions et
	OF ARCHITECTS 2 informations sur le site et aviser immédiatement l'architecte de toutes erreurs ou omissions.
	ROBERTO CAMPOS UCENCE 7401 Contractor shall verify all information and dimensions on site and immediately report any errors or omissions to the architect.
	omissions to the architect.
	Projet / Project
	3-STOREY LOW RISE
	APARTMENT
	1435 & 1455 MORISSET AVE. OTTAWA, ON
	LEVEL 4 PLAN
	Dessiné par / Drawn by No. projet / Project number RD 2104
	RD 2104 Vérifié par / Verified by No. dessin / Drawing number Révision / Revision
	Échelle / Scale 1 : 75
	Date de création du dessin / Drawing creation date 2021-05-28 A122
	2021-05-28



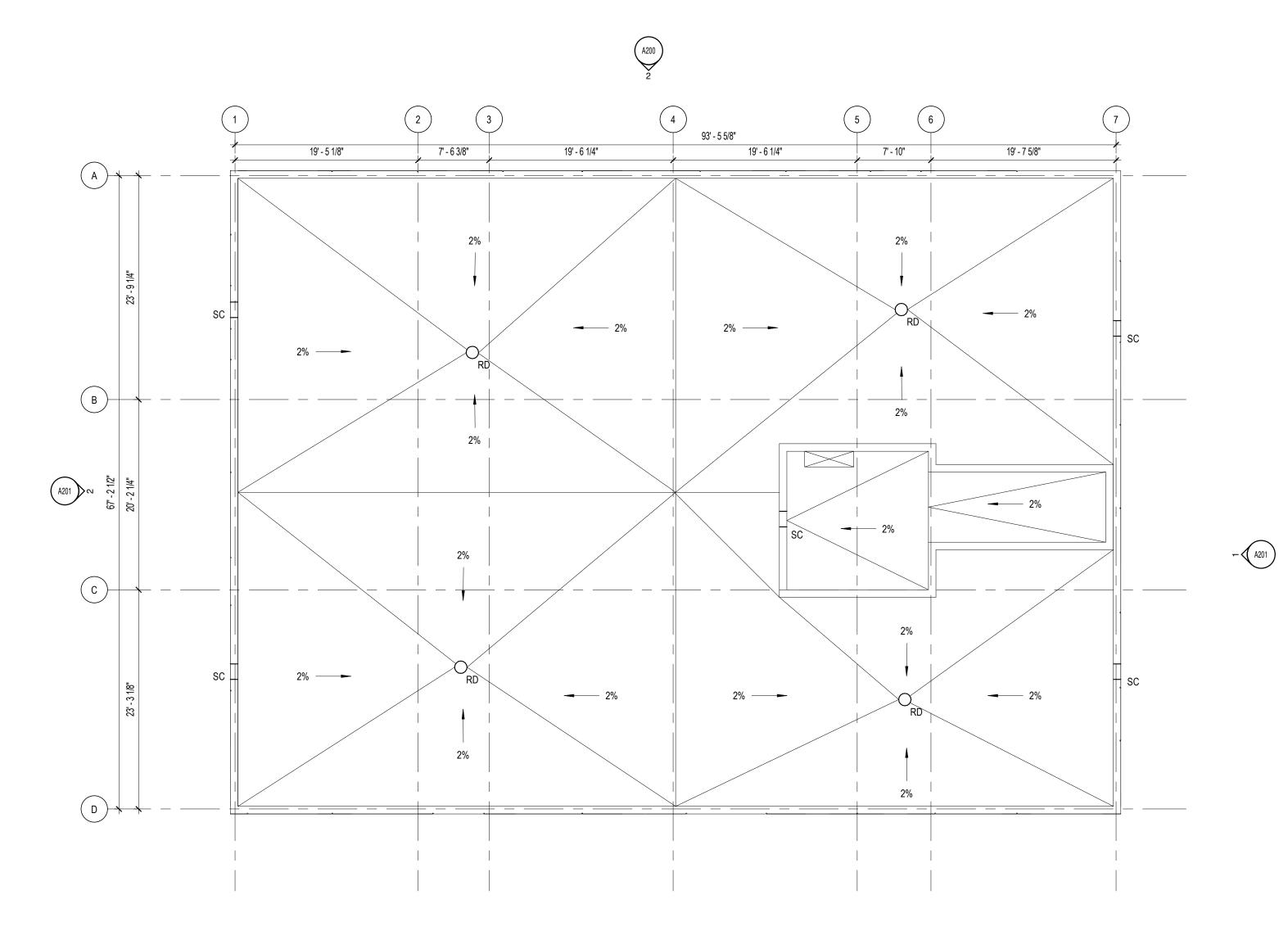


LEGEND		No. Date Émis pour / Object 2 2021-06-18 SITE PLAN APPLICATION
	CONCRETE PARGED FOUNDATION WALL MIN. 6" BELOW GRADE COLOUR: MEDIUM GREY	
2	BRICK VENEER SIZE:	
	BY: COLOUR: DARK GREY	
3	METAL PANELS BY: FINISH: SMOOTH COLOUR: DARK GREY	
4	METAL PANELS BY: FINISH: COLOUR: WOOD SIMULATION	
5	METAL PANELS BY: FINISH: CORRUGATED COLOUR: DARK GREY	
	OPERABLE WINDOW	
		Ingénieur / Engineer (paysagiste / Landscape)
		FOTENN Planning + Design
		Ingénieur / Engineer (Civil / Civil)
		Firm Capital
		Architecte / Architect Collectif d'architectes fig. 1 3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122
		3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122 fig. 2 190 Somerset St W #206 Ottawa ON K2P 0J4 T. 613 695-6122 www.figurr.ca
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		Projet / Project
		3-STOREY LOW RISE APARTMENT
		1435 & 1455 MORISSET AVE. OTTAWA, ON
		Dessiné par / Drawn by No. projet / Project number
		RD 2104 Vérifié par / Verified by No. dessin / Drawing number MD Révision Échelle / Scale 2
		As indicated Date de création du dessin / Drawing creation date 2021-05-28





	LEGEND		No. Date Émis pour / Object 2 2021-06-18 SITE PLAN APPLICATION
		Concrete Parged Foundation Wall Min. 6" Below Grade Colour: Medium Grey	
	2>	BRICK VENEER SIZE: BY: COLOUR: DARK GREY	
	3>	METAL PANELS BY: FINISH: SMOOTH COLOUR: DARK GREY	
109569	4	METAL PANELS BY: FINISH: COLOUR: WOOD SIMULATION	
	5	METAL PANELS BY: FINISH: CORRUGATED COLOUR: DARK GREY	
106673		OPERABLE WINDOW	
103777			
100881			Ingénieur / <i>Engineer</i> (paysagiste / <i>Landscape</i>)
98437			FOTENN Planning + Design
			Ingénieur / Engineer (Civil / Civil)
			LRJ ENGINEERING I INGÉMIERIE
			Firm Capital
			Architecte / Architect fig. 1 3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122 fig. 2
109569			190 Somerset St W #206 Ottawa ON K2P 0J4 T. 613 695-6122 www.figurr.ca Droit d'auteur / <i>Copyright</i> Ce dessin est sujet au droit d'auteur. Il ne peut être reproduit pour quelques intentions ou usages que ce soit, il ne peut être utilisé uniquement avec l'apposition de la signature et
106673			de l'estampe originale. This drawing is subject to copyright. It is not to be reproduced for any purpose or by any means, and may only be used if it bears an original stamp and signature. Sceau / Seal Note:
			Contractor shall verify all informations are uncertainty and the state of a s
103777			Projet / Project
G WALL			3-STOREY LOW RISE APARTMENT
			1435 & 1455 MORISSET AVE. OTTAWA, ON Titre / Title ELEVATIONS- EAST & WEST
98437			Dessiné par / Drawn by No. projet / Project number RD 2104
			MD Revision
			Date de création du dessin / Drawing creation date 2021-05-28 A201



A200



No. Date Émis pour 1 2021-04-22 COORDIN	
Ingénieur / Engineer	
(Mécanique & Électricité / Mechanic	al & Electrical)
Ingénieur / Engineer	
(Structure / Structure)	
Client / Client	
Gient / Gient	
Architecte / Architect	Collectif d'architectes
fig. 1	_
3550, Saint-Antoine O. Montréal QC H4C 1A9 T. 514 861-5122	figue
fig. 2	figurr
190 Somerset St W #206 Ottawa ON K2P 0J4	
T. 613 695-6122	www.figurr.ca
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Sceau / Seal	
	Note:
	L'entrepreneur doit vérifier toutes les dimensions et informations sur le site et
	aviser immédiatement l'architecte de toutes erreurs
	ou omissions. Contractor shall verify all
	information and dimensions on site and immediately report any errors or
	omissions to the architect.
Projet / Project	
3-STOR	EY APARTMENT
4.	155 MORISSET AVE.
Titre / Title	
	ROOF PLAN
Dessiné par / Drawn by Author	No. projet / Project number
Vérifié par / Verified by	No. dessin / Drawing number Révision /
Checker	Revision
Échelle / Scale 1:100	
Date de création du dessin / Drawing creation date	A130
06/05/18	